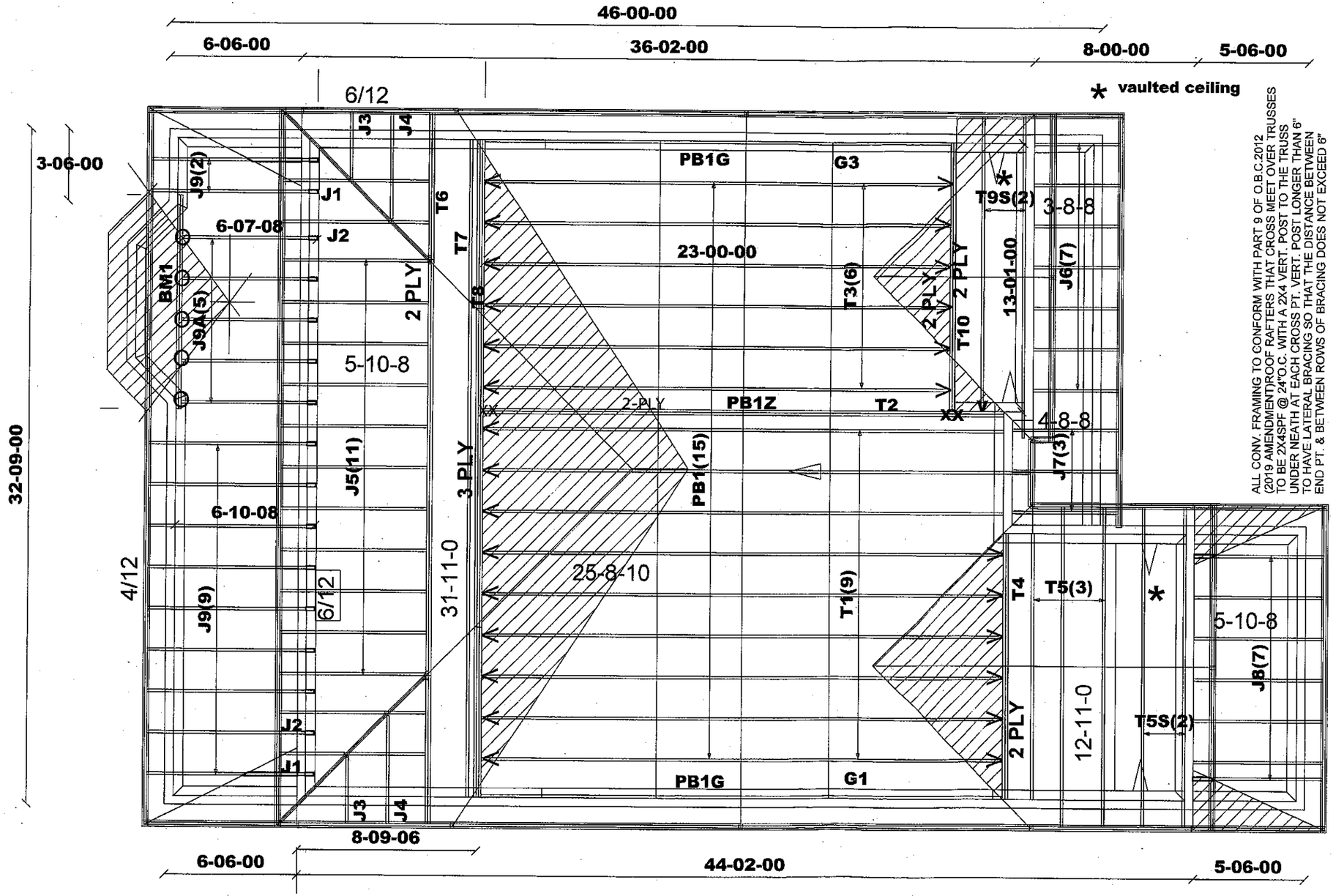
9/12 roof pitch unless noted



ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C.2012 (2019 AMENDMENT)ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4SPF @ 24"O.C. WITH A 2X4 VERT. POST TO THE TRUSS UNDER NEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6" TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6"

ASPHALT SHINGLES
12"FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2- (XX)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C.2012 PART 9 (2019 AMENDMENT)
DESIGN LOADS:
SNOW LOAD 25.6 PSF
TC DEAD 6 PSF
BC LIVE 0 PSF
BC DEAD 7.4 PSF

BM1 = 2-2X10

CITY OF HAMILTON
Building Division
Permit No. 20-187703

THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE

THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW

These drawings and/or specifications have been reviewed by

FOR CHIEF BUILDING OFFICIAL

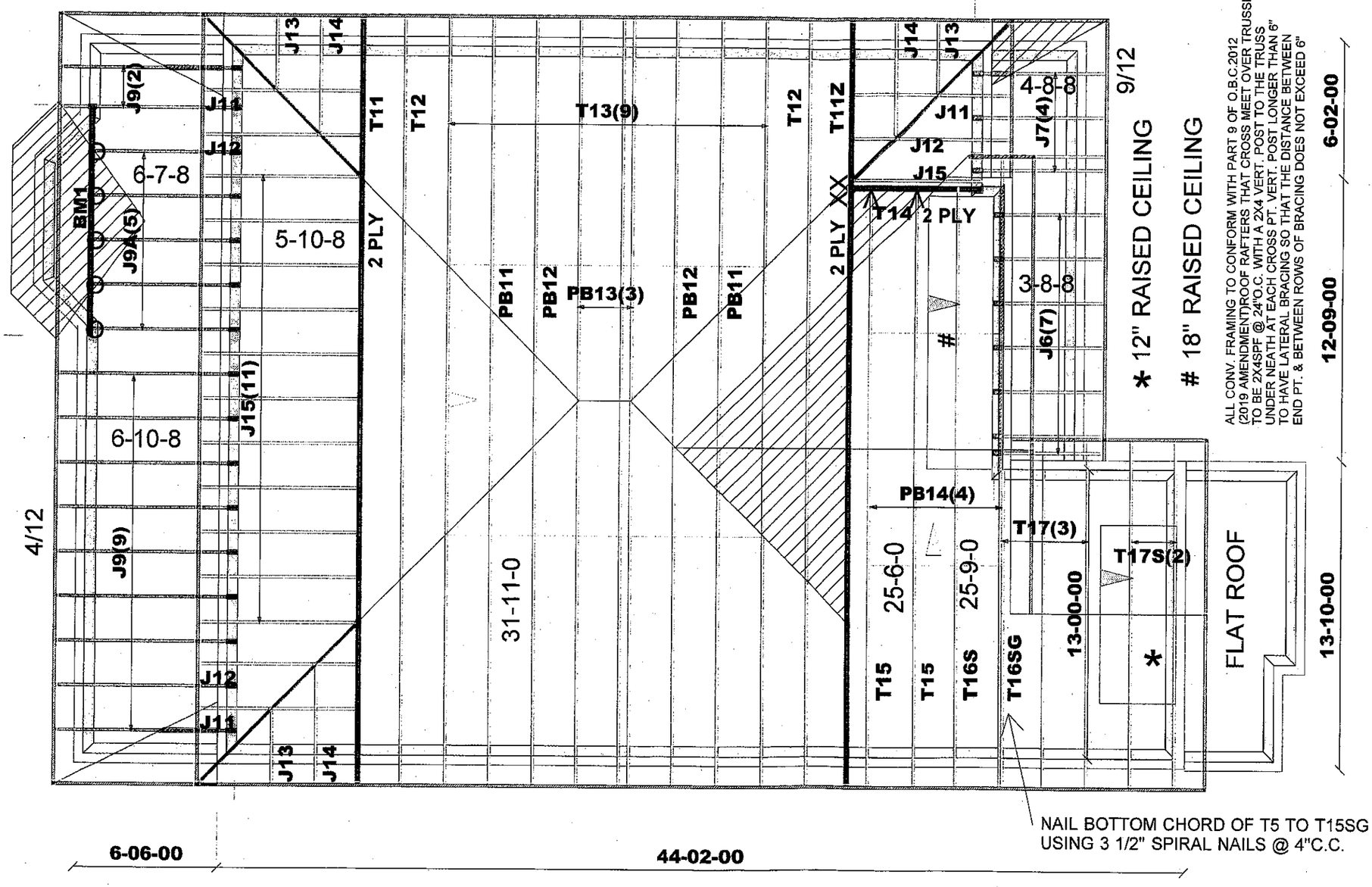
DEC 14/20
DATE

M13106

TAMARACK ROOF TRUSSES INC.

Job Track: 51225	Builder / Location: GREEN PARK HOMES / WATERDOWN	Model / Elevation: VALLEYCREEK 1/1	Mitek ver: 8.02.00
Plan Log: , 202394	Project: RUSSELL GARDENS PH.3		
Layout ID: 408128	Date: 2020-04-13 Sales: Mario DiCano Designer: AC/JG		

9/12 ROOF PITCH UNLESS NOTED



NAIL BOTTOM CHORD OF T5 TO T15SG USING 3 1/2" SPIRAL NAILS @ 4"C.C.

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C.2012 (2019 AMENDMENT)ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4SPF @ 24"O.C. WITH A 2X4 VERT. POST TO THE TRUSS UNDER NEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6" TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6"

ASPHALT SHINGLES
12"FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)
HGUS26-2 - (XX)
LUS24 -(O)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C.2012 PART 9 (2019 AMENDMENT)
DESIGN LOADS:
SNOW LOAD 25.6 PSF
TC DEAD 6 PSF
BC LIVE 0 PSF
BC DEAD 7.4 PSF

BM1 = 2-2X10

CITY OF HAMILTON
Building Division
Permit No. 20-187703
THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE
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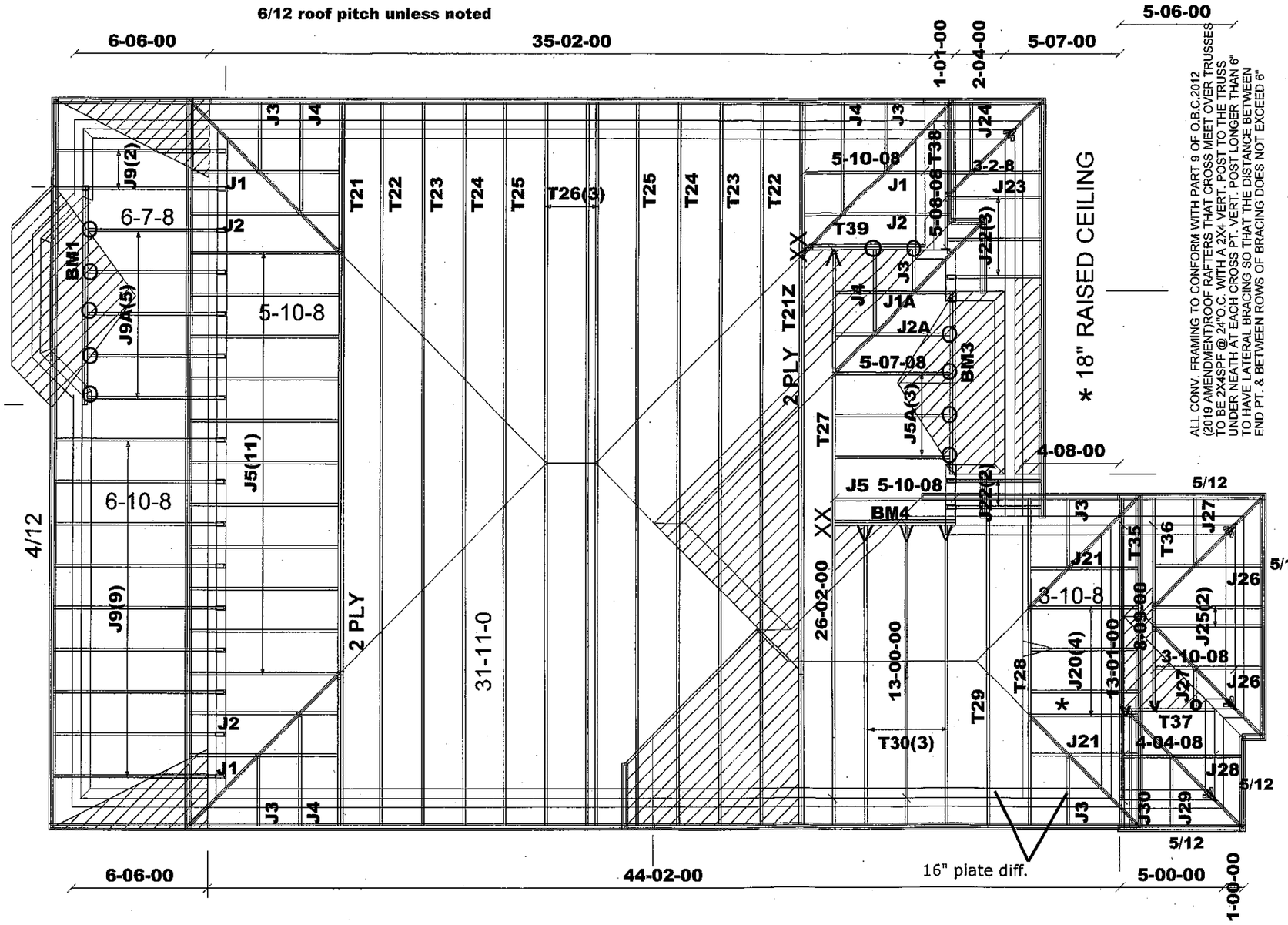
These drawings and/or specifications have been reviewed by
FOR CHIEF BUILDING OFFICIAL
DEC. 14/20
DATE

M13106

TAMARACK ROOF TRUSSES INC.

Job Track: 51225	Builder / Location: GREEN PARK HOMES / WATERDOWN	Model / Elevation: VALLEYCREEK 1/2	Mitek ver 8.3.1.215
Plan Log: 202394			
Layout ID: 408129	Project: RUSSELL GARDENS PH.3		
	Date: 2020-04-13 Sales: Mario DiCano Designer: AC/JG		



ASPHALT SHINGLES
12"FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C.2012 PART 9 (2019 AMENDMENT)
DESIGN LOADS:
SNOW LOAD 25.6 PSF
TC DEAD 6 PSF
BC LIVE 0 PSF
BC DEAD 7.4 PSF

BM1 = BM4 = 2-2X10
= BM3

CITY OF HAMILTON
Building Division
Permit No. 20-187703
THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE
THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW
These drawings and/or specifications have been reviewed by
CR DEC 14/20
FOR CHIEF BUILDING OFFICIAL DATE

M13106

TAMARACK ROOF TRUSSES INC.

Job Track: 51225	Builder / Location: GREEN PARK HOMES / WATERDOWN	Model / Elevation: VALLEYCREEK 1/3	Mitek ver 8.3.1.215
Plan Log: 202394	Project: RUSSELL GARDENS PH.3		
Layout ID: 408130	Date: 2020-04-14 Sales: Mario DiCano	Designer: AC/JG	

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51225
Builder:	GREEN PARK HOMES	PlanLog:	202394
Project:	RUSSELL GARDENS PH.3	Layout ID:	408128
Location:	WATERDOWN	Ref #	
Model:	VALLEYCREEK 1	Page:	1 of 3
Lot #:		Date:	04-13-2020
Elevation:	1	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

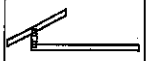
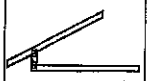
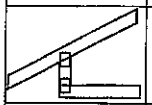
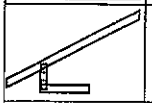
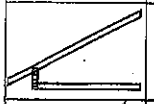
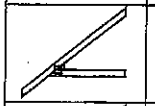
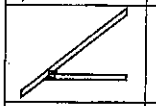
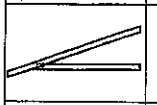
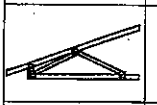
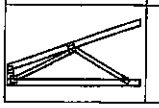
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	9	T1 Piggyback Base	9 /12	25-08-10	8-00-00	2 x 4		1-06-04 1-06-04	1095.39 711.00		
	1	G1 GABLE	9 /12	25-08-10	8-00-00	2 x 4		1-06-04 1-06-04	136.53 88.00		
	1 2-ply	T2 Piggyback Base Girder	9 /12	25-08-10	8-00-00	2 x 4 2 x 6		1-06-04 1-06-04	269.01 172.00		
	6	T3 Piggyback Base	9 /12	23-00-00	8-00-00	2 x 4		1-06-04 3-06-12	663.68 431.00		
	1	G3 GABLE	9 /12	23-00-00	8-00-00	2 x 4		1-06-04 3-06-12	123.39 80.83		
	1 2-ply	T4 Common Girder	9 /12	12-11-00	6-04-06	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	142.58 92.67		
	3	T5 Common	9 /12	12-11-00	6-04-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	177.12 113.00		
	2	T5S Scissor	9 /12 5 /12	12-11-00	6-04-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	113.51 75.33		
	1 2-ply	T6 Hip Girder	6 /12	31-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	288.99 180.00		
	1	T7 Hip	6 /12	31-11-00	5-01-04	2 x 4		1-02-00 1-02-00	123.2 76.50		
	1 3-ply	T8 Hip Girder	6 /12	31-11-00	5-03-07	2 x 6		1-02-00 1-02-00	539.49 324.00		
	2	T9S Scissor	9 /12 5 /12	13-01-00	6-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	114.74 78.00		
	1 2-ply	T10 Common Girder	9 /12	13-01-00	6-05-02	2 x 4 2 x 6		1-06-04 1-06-04	133.88 86.67		
	15	PB1 Piggyback	9 /12	8-05-05	3-02-00	2 x 4			330.77 222.50		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51225
Builder:	GREEN PARK HOMES	PlanLog:	202394
Project:	RUSSELL GARDENS PH.3	Layout ID:	408128
Location:	WATERDOWN	Ref #	
Model:	VALLEYCREEK 1	Page:	2 of 3
Lot #:		Date:	04-13-2020
Elevation:	1	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	PB1G GABLE	9 /12	8-05-05	3-02-00	2 x 4			47.24 31.67		
	1 2-ply	PB1Z Piggyback	9 /12	8-05-05	3-02-00	2 x 4			44.1 29.67		
	2	J1 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 3-11-09	1-02-00 2-01-08	23.48 14.67		
	2	J2 Jack-Open	6 /12	3-10-15	3-01-08	2 x 4	1-03-08 1-11-09	1-02-00 3-01-08	28.58 17.33		
	2	J3 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 1-01	1-02-00 2-01-08	14.65 9.33		
	2	J4 Jack-Open	6 /12	2-00-00	3-01-08	2 x 4	1-03-08 1-10-15	1-02-00 2-02-00	19.75 12.00		
	11	J5 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	184.74 117.33		
	7	J6 Jack-Open	9 /12	3-08-08	4-01-06	2 x 4	1-03-08	4-10 2-06-14	84.34 56.00		
	3	J7 Jack-Open	9 /12	4-08-08	4-10-06	2 x 4	1-03-08	4-10 3-03-14	43.84 28.00		
	7	J8 Jack-Open	4 /12	5-10-00	2-08-03	2 x 4	1-03-08	3-15 2-00-04	107.29 65.33		
	11	J9 Jack-Partial	4 /12	6-10-08	3-02-11	2 x 4	1-03-08	11-03 2-11-08	259.01 168.67		
	5	J9A Jack-Partial	4 /12	6-07-08	3-02-11	2 x 4		1-00-03 3-02-11	108.41 73.33		

TOTAL # TRUSS= **107** TOTAL BFT OF ALL TRUSSES= **3354.83** BFT. TOTAL WEIGHT OF ALL TRSSES **5217.7** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
28	Hardware	LJS26DS	
5	Hardware	LUS24	

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51225
Builder:	GREEN PARK HOMES	PlanLog:	202394
Project:	RUSSELL GARDENS PH.3	Layout ID:	408128
Location:	WATERDOWN	Ref #	
Model:	VALLEYCREEK 1	Page:	3 of 3
Lot #:		Date:	04-13-2020
Elevation:	1	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

HARDWARE

QTY	TYPE	MODEL	LENGTH
TOTAL NUMBER OF ITEMS=	33		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51225
Builder:	GREEN PARK HOMES	PlanLog:	202394
Project:	RUSSELL GARDENS PH.3	Layout ID:	408129
Location:	WATERDOWN	Ref #	
Model:	VALLEYCREEK 1	Page:	1 of 2
Lot #:		Date:	04-13-2020
Elevation:	2	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

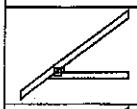
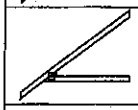
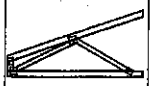
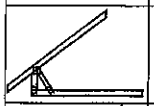
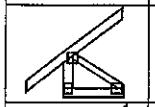
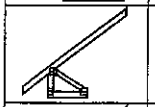
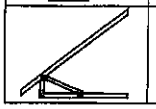
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T11 Hip Girder	9 /12	31-11-00	5-11-02	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	335.45 210.67		
	1 2-ply	T11Z Hip Girder	9 /12	31-11-00	5-11-02	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	335.45 210.67		
	2	T12 Hip	9 /12	31-11-00	7-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	296.23 185.67		
	9	T13 Hip	9 /12	31-11-00	8-11-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	1394.54 880.50		
	1 2-ply	T14 Monopitch Girder	0 /12	5-10-08	1-06-00	2 x 6		1-06-00 1-06-00	55.18 35.00		
	2	T15 Piggyback Base	9 /12	25-06-00	8-00-00	2 x 4	1-03-08	1-06-04 3-02-08	245.98 155.33		
	1	T16S Piggyback Base	9 /12	25-09-00	8-00-00	2 x 4	1-03-08	1-06-04 1-06-04	127.94 81.67		
	1	T16SG GABLE	9 /12	25-09-00	8-00-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	137.69 88.17		
	3	T17 Common	9 /12	13-00-00	6-04-12	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	178.09 113.00		
	2	T17S Roof Special	9 /12	13-00-00	6-04-12	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	130.27 85.67		
	2	PB11 Piggyback	9 /12	12-02-00	1-06-00	2 x 4			71.07 45.67		
	2	PB12 Piggyback	9 /12	12-02-00	3-00-00	2 x 4			71.58 45.67		
	3	PB13 Piggyback	9 /12	12-02-00	4-06-12	2 x 4			98.23 60.00		
	4	PB14 Piggyback	9 /12	10-05-11	3-11-02	2 x 4			111.51 72.00		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51225
Builder:	GREEN PARK HOMES	PlanLog:	202394
Project:	RUSSELL GARDENS PH.3	Layout ID:	408129
Location:	WATERDOWN	Ref #	
Model:	VALLEYCREEK 1	Page:	2 of 2
Lot #:		Date:	04-13-2020
Elevation:	2	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	7	J6 Jack-Open	9 /12	3-08-08	4-01-06	2 x 4	1-03-08	4-10 3-02-00	84.34 56.00		
	4	J7 Jack-Open	9 /12	4-08-08	4-10-06	2 x 4	1-03-08	4-10 3-11-00	58.46 37.33		
	11	J9 Jack-Partial	4 /12	6-10-08	3-02-11	2 x 4	1-03-08	11-03 3-02-11	262.82 168.67		
	5	J9A Jack-Partial	4 /12	6-07-08	3-02-11	2 x 4		1-00-03 3-02-11	108.41 73.33		
	3	J11 Jack-Open	9 /12	1-10-15	2-11-07	2 x 4	1-03-08 3-11-09	1-06-04 2-11-07	42.4 29.00		
	3	J12 Jack-Open	9 /12	3-10-15	4-05-07	2 x 4	1-03-08 1-11-09	1-06-04 4-05-07	50.15 33.00		
	3	J13 Jack-Open	9 /12	1-10-15	2-11-07	2 x 4	1-03-08 1-01	1-06-04 2-11-07	29.94 22.50		
	3	J14 Jack-Open	9 /12	2-00-00	4-05-07	2 x 4	1-03-08 1-10-15	1-06-04 3-00-04	38.49 26.50		
	12	J15 Jack-Open	9 /12	5-10-08	5-11-02	2 x 4	1-03-08	1-06-04 5-11-02	248.17 162.00		

TOTAL # TRUSS= **88** TOTAL BFT OF ALL TRUSSES= **2878.02** BFT. TOTAL WEIGHT OF ALL TRSSES **4512.4** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LJS26DS	
5	Hardware	LUS24	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= **8**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51225
Builder:	GREEN PARK HOMES	PlanLog:	202394
Project:	RUSSELL GARDENS PH.3	Layout ID:	408130
Location:	WATERDOWN	Ref #	
Model:	VALLEYCREEK 1	Page:	1 of 3
Lot #:		Date:	04-14-2020
Elevation:	3	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T21 Hip Girder	6 /12	**31-11-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	288.19 184.00		
	1 2-ply	T21Z Hip Girder	6 /12	**31-11-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	288.19 184.00		
	2	T22 Hip	6 /12	**31-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	259.03 161.67		
	2	T23 Hip	6 /12	**31-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	258.25 161.33		
	2	T24 Hip	6 /12	**31-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	271.21 168.33		
	2	T25 Hip	6 /12	**31-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	262.57 165.00		
	3	T26 Hip	6 /12	**31-11-00**	9-01-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	387.71 242.50		
	1	T27 Hip Girder	6 /12	**26-02-00**	4-01-04	2 x 6	1-03-08	1-02-00 2-08-00	141.84 89.00		
	1	T28 Hip Girder	6 /12	**13-00-00**	3-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	51.64 33.00		
	1	T29 Hip	6 /12	**13-00-00**	5-07-04	2 x 4	1-03-08 1-03-08	2-08-00 2-08-00	61.95 41.00		
	3	T30 Common	6 /12	**13-00-00**	5-11-00	2 x 4	1-03-08 1-03-08	2-08-00 2-08-00	179.75 113.00		
	1	T35 Hip Girder	5 /12	**13-01-00**	2-08-02	2 x 4	1-03-08 1-03-08	4-01 4-01	40.14 26.50		
	1	T36 Hip Girder	5 /12	**8-09-00**	2-05-10	2 x 4	1-03-08	4-01 4-01	26 17.00		
	1	T37 Half Hip Girder	5 /12	**4-04-08**	2-01-01	2 x 4 2 x 6		4-01 2-01-01	14.93 9.67		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

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Model:	VALLEYCREEK 1	Page:	2 of 3
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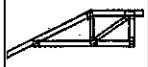
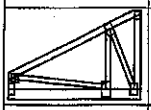
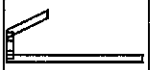
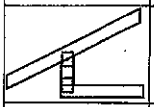
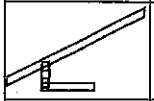
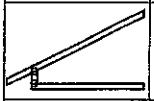
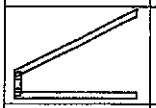
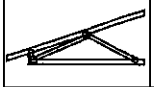
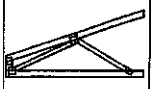
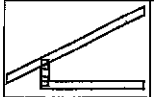
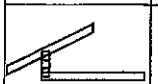
Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T38 Half Hip Girder	6 /12	5-08-08	2-06-15	2 x 4	1-03-08	4-03 1-11-07	20.42 13.83		
	1 2-ply	T39 Monopitch Girder	6 /12	5-10-08	3-11-00	2 x 4 2 x 6		1-02-00 3-11-00	60.57 39.00		
	3	J1 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 3-11-09	1-02-00 2-01-08	35.22 22.00		
	1	J1A Jack-Open	6 /12	5-07-08	2-01-08	2 x 4		1-03-08 4-01-04	9.63 6.67		
	3	J2 Jack-Open	6 /12	3-10-15	3-01-08	2 x 4	1-03-08 1-11-09	1-02-00 3-01-08	42.87 26.00		
	1	J2A Jack-Open	6 /12	5-07-08	3-01-08	2 x 4		1-03-08 4-01-04	12.18 8.00		
	6	J3 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 1-01	1-02-00 2-01-08	43.94 28.00		
	4	J4 Jack-Open	6 /12	2-00-00	3-01-08	2 x 4	1-03-08 1-10-15	1-02-00 2-02-00	39.49 24.00		
	12	J5 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	201.63 128.00		
	3	J5A Jack-Open	6 /12	5-07-08	4-01-04	2 x 4		1-03-08 4-01-04	44.06 28.00		
	11	J9 Jack-Partial	4 /12	6-10-08	3-02-11	2 x 4	1-03-08	11-03 3-02-11	264.25 168.67		
	5	J9A Jack-Partial	4 /12	6-07-08	3-02-11	2 x 4		1-00-03 3-02-11	108.41 73.33		
	4	J20 Jack-Open	6 /12	3-10-08	3-01-04	2 x 4	1-03-08	1-02-00 3-01-04	47.86 29.33		
	2	J21 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 1-11-09	1-02-00 2-01-08	18.92 12.00		

TAMARACK
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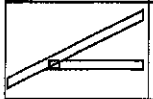
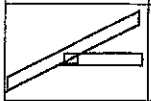
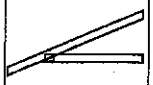
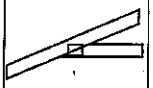
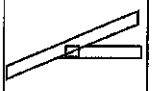
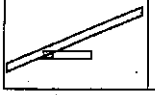
Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	5	J22 Jack-Open	6 /12	3-02-08	2-06-15	2 x 4	1-03-08	4-03 1-11-07	47.81 33.33		
	1	J23 Jack-Open	6 /12	1-10-15	1-11-02	2 x 4	1-03-08 1-03-09	4-03 1-03-10	7.91 5.33		
	1	J24 Jack-Open	6 /12	1-10-15	1-11-02	2 x 4	1-03-08 1-01	4-03 1-03-10	6.53 4.00		
	2	J25 Jack-Open	5 /12	3-10-08	2-05-10	2 x 4	1-03-08	4-01 1-11-07	21.88 13.33		
	2	J26 Jack-Open	5 /12	1-09-07	1-07-03	2 x 4	1-03-08 2-01-01	4-01 1-01-00	16.72 10.67		
	2	J27 Jack-Open	5 /12	1-09-07	1-07-03	2 x 4	1-03-08 1-01	4-01 1-01-00	12.16 8.00		
	1	J28 Jack-Open	5 /12	1-10-15	1-07-13	2 x 4	1-03-08 2-05-09	4-01 1-01-10	9.08 6.00		
	1	J29 Jack-Open	5 /12	1-10-15	1-07-13	2 x 4	1-03-08 1-01	4-01 1-01-10	6.38 4.00		
	1	J30 Jack-Open	5 /12	2-00-00	2-05-13	2 x 4	1-03-08 1-10-15	4-01 1-02-01	8.85 5.33		

TOTAL # TRUSS= **98** TOTAL BFT OF ALL TRUSSES= **2292.82** BFT. TOTAL WEIGHT OF ALL TRSSES **3618.09** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
6	Hardware	LJS26DS	
12	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= **20**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	T1	9	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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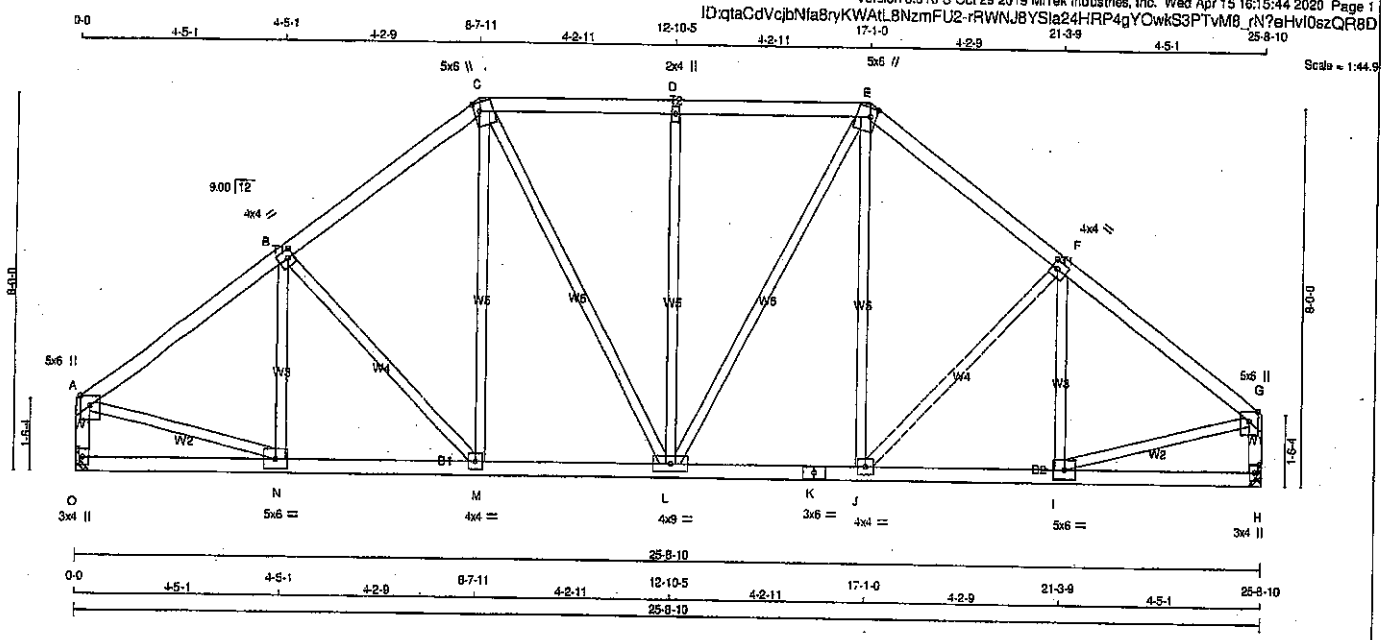


TOTAL WEIGHT = 9 X 122 = 1096 lb [M](F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1418	0	1418	0	0	MECHANICAL	
H	1418	0	1418	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT O. H. MINIMUM BEARING LENGTH AT JOINT O = 3-8, JOINT H = 3-8.

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1003	658 / 0	0 / 0	0 / 0	0 / 0	345 / 0	0 / 0
H	1003	658 / 0	0 / 0	0 / 0	0 / 0	345 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

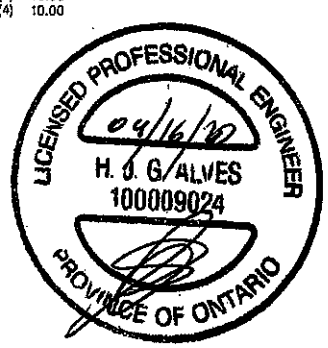
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-1475 / 0	-91.8	-91.8	0.25 (1)	5.15
B-C	-1355 / 0	-91.8	-91.8	0.24 (1)	5.33
C-D	-1210 / 0	-91.8	-91.8	0.22 (1)	5.58
D-E	-1210 / 0	-91.8	-91.8	0.22 (1)	5.58
E-F	-1355 / 0	-91.8	-91.8	0.24 (1)	5.33
F-G	-1475 / 0	-91.8	-91.8	0.25 (1)	5.15
O-A	-1382 / 0	0.0	0.0	0.14 (1)	6.91
H-G	-1382 / 0	0.0	0.0	0.14 (1)	6.91
O-N	0 / 0	-18.5	-18.5	0.08 (4)	10.00
N-M	0 / 1202	-18.5	-18.5	0.23 (1)	10.00
M-L	0 / 1062	-18.5	-18.5	0.21 (1)	10.00
L-K	0 / 1062	-18.5	-18.5	0.21 (1)	10.00
K-J	0 / 1062	-18.5	-18.5	0.21 (1)	10.00
J-I	0 / 1202	-18.5	-18.5	0.23 (1)	10.00
I-H	0 / 0	-18.5	-18.5	0.08 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-B	-238 / 0	0.08 (1)
B-M	-207 / 0	0.14 (1)
M-C	0 / 229	0.05 (1)
C-L	0 / 308	0.07 (1)
L-D	-467 / 0	0.57 (1)
L-E	0 / 308	0.07 (1)
J-E	0 / 229	0.05 (1)
J-F	-207 / 0	0.14 (1)
I-F	-238 / 0	0.08 (1)
A-N	0 / 1244	0.28 (1)
I-G	0 / 1244	0.28 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.25/1.00 (A-B:1) , BC=0.23/1.00 (M-N:1) , WB=0.57/1.00 (D-L:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (A) (INPUT = 0.90)
JSI METAL= 0.56 (A) (INPUT = 1.00)

Structural component only
DWG# T-2006271

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	G1	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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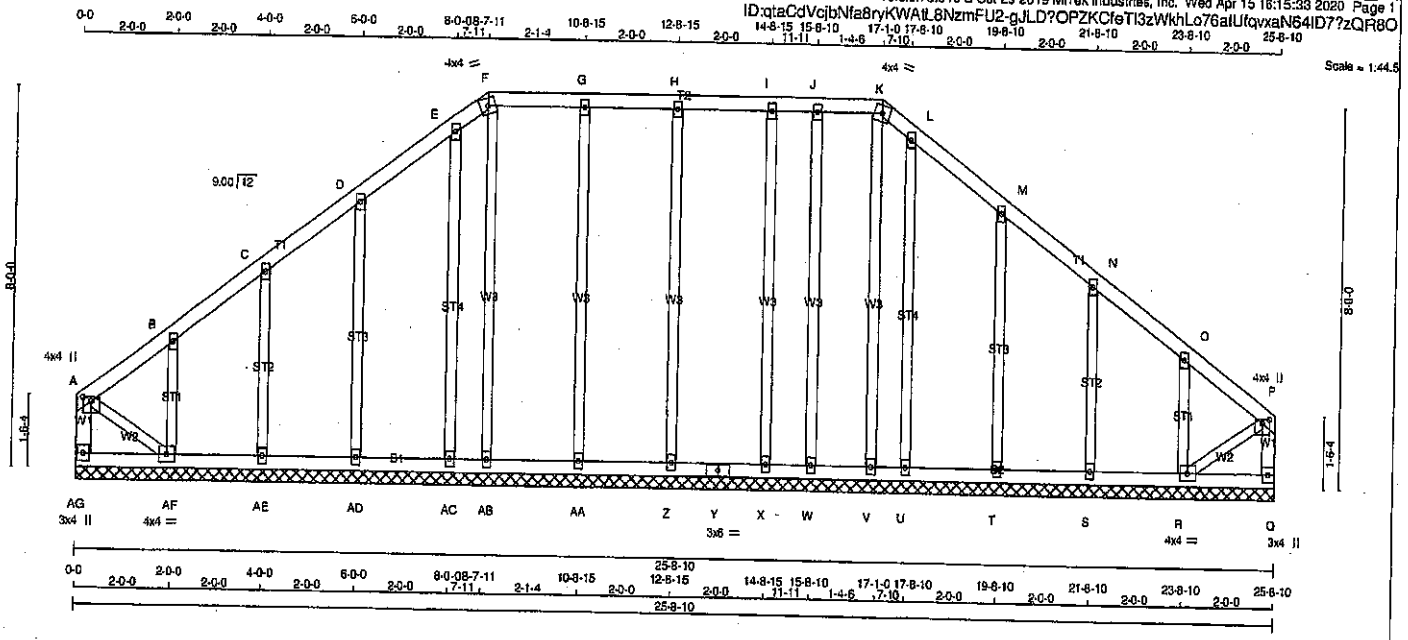

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AG - A	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
K - P	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
AG - Y	2x4	DRY	No.2	SPF
Y - Q	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B, C, D, E, G, H, I, J, L, M, N, O						
B	TMW+w	MT20	2.0	4.0		
F	TTW-m	MT20	4.0	4.0		
K	TTW-m	MT20	4.0	4.0		
P	TMVW+p	MT20	4.0	4.0	1.00	2.00
Q	BMV1+p	MT20	3.0	4.0		
R	BMWW1-t	MT20	4.0	4.0		
S, T, U, V, W, X, Z, AA, AB, AC, AD, AE						
S	BMW1+w	MT20	2.0	4.0		
Y	BS-t	MT20	3.0	6.0		
AF	BMWW1-t	MT20	4.0	4.0		
AG	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
AG- A	-97 / 0	0.0	0.0	0.01 (1)		7.81
A- B	-15 / 0	-91.8	-91.8	0.05 (1)		6.25
B- C	-20 / 0	-91.8	-91.8	0.05 (1)		6.25
C- D	-12 / 0	-91.8	-91.8	0.04 (1)		6.25
D- E	-14 / 0	-91.8	-91.8	0.04 (1)		6.25
E- F	-23 / 0	-91.8	-91.8	0.03 (1)		6.25
F- G	-5 / 0	-91.8	-91.8	0.05 (1)		10.00
G- H	-5 / 0	-91.8	-91.8	0.05 (1)		10.00
H- I	-5 / 0	-91.8	-91.8	0.04 (1)		10.00
I- J	-5 / 0	-91.8	-91.8	0.03 (1)		10.00
J- K	-5 / 0	-91.8	-91.8	0.02 (1)		10.00
K- L	-21 / 0	-91.8	-91.8	0.02 (1)		6.25
L- M	-14 / 0	-91.8	-91.8	0.04 (1)		6.25
M- N	-12 / 0	-91.8	-91.8	0.04 (1)		6.25
N- O	-19 / 0	-91.8	-91.8	0.05 (1)		6.25
O- P	-14 / 0	-91.8	-91.8	0.05 (1)		6.25
Q- P	-97 / 0	0.0	0.0	0.01 (1)		7.81
AG-AF	0 / 0	-18.5	-18.5	0.02 (4)		10.00
AF-AE	0 / 14	-18.5	-18.5	0.02 (4)		10.00
AE-AD	0 / 10	-18.5	-18.5	0.02 (4)		10.00
AD-AC	0 / 8	-18.5	-18.5	0.02 (4)		10.00
AC-AB	0 / 6	-18.5	-18.5	0.01 (4)		10.00
AB-AA	0 / 5	-18.5	-18.5	0.02 (4)		10.00
AA- Z	0 / 5	-18.5	-18.5	0.02 (4)		10.00
Z- Y	0 / 5	-18.5	-18.5	0.02 (4)		10.00
Y- X	0 / 5	-18.5	-18.5	0.02 (4)		10.00
X- W	0 / 5	-18.5	-18.5	0.01 (4)		10.00
W- V	0 / 5	-18.5	-18.5	0.00 (4)		10.00
V- U	0 / 5	-18.5	-18.5	0.01 (4)		10.00
U- T	0 / 7	-18.5	-18.5	0.02 (4)		10.00
T- S	0 / 10	-18.5	-18.5	0.02 (4)		10.00
S- R	0 / 14	-18.5	-18.5	0.02 (4)		10.00
R- Q	0 / 0	-18.5	-18.5	0.02 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- O	-197 / 0	0.04 (1)
S- N	-175 / 0	0.05 (1)
T- M	-191 / 0	0.11 (1)
U- L	-139 / 0	0.14 (1)
AF- B	-197 / 0	0.04 (1)
AE- C	-176 / 0	0.05 (1)
AD- D	-190 / 0	0.11 (1)
AC- E	-143 / 0	0.15 (1)
AB- F	-82 / 0	0.10 (1)
V- K	-59 / 0	0.07 (1)
Z- H	-185 / 0	0.23 (1)
AA- G	-209 / 0	0.25 (1)
X- I	-141 / 0	0.17 (1)
W- J	-107 / 0	0.13 (1)
A-AF	0 / 24	0.01 (1)
R- P	0 / 24	0.01 (1)

TOTAL WEIGHT = 137 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (F-G:1) , BC=0.02/1.00 (AE-AF:4) , WB=0.25/1.00 (G-AA:1) , SSI=0.09/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (K) (INPUT = 0.90)
JSI METAL= 0.10 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006256

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	T2	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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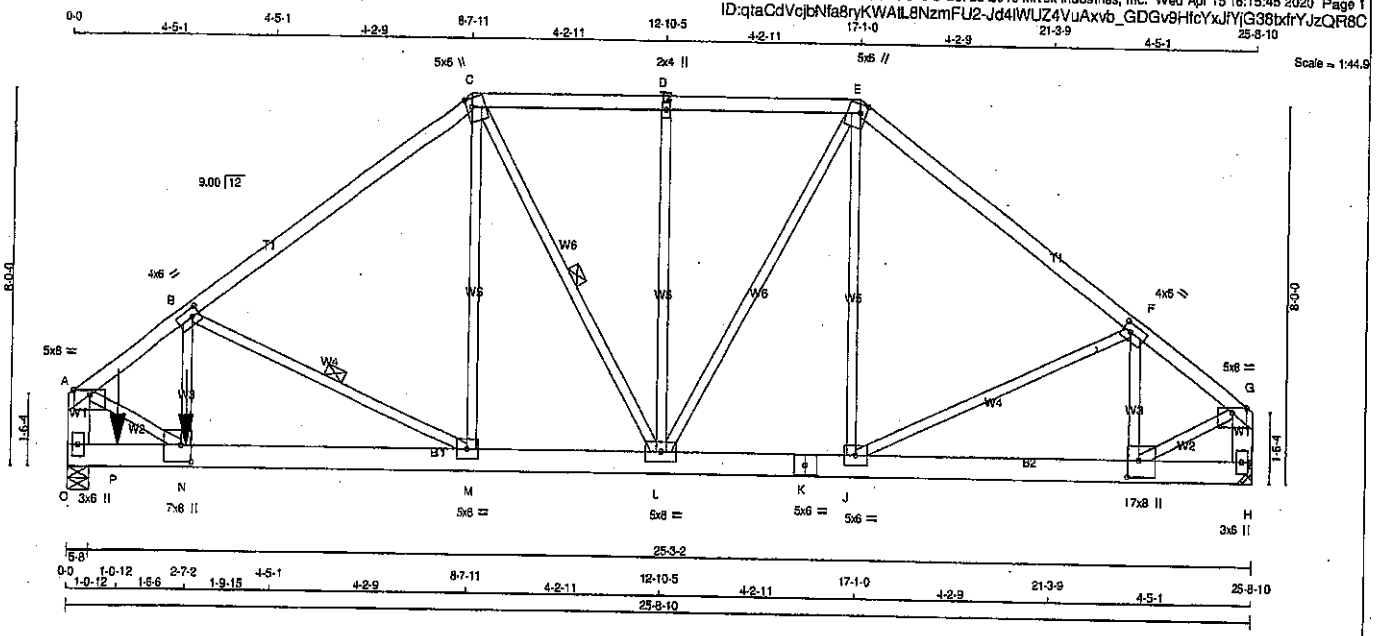

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
O - A	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-E	1	12	TOP
E-G	1	12	TOP
O-A	2	12	TOP
H-G	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O-K	2	5	SIDE(549.3)
K-H	2	5	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
B-N	1	2	SIDE(689.7)
2x3	1	6	
F-I	1	2	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	5.0	8.0	1.25	4.00
B	TMWW-t	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	6.0	2.00	2.00
G	TMWW-p	MT20	5.0	8.0	1.25	4.00
H	BMV1+p	MT20	3.0	6.0		
I	BMWW+t	MT20	7.0	8.0	4.25	2.75
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW+t	MT20	7.0	8.0	4.25	2.75
O	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	6484	0	6484	0	0	5-8	5-8
H	1931	0	1931	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	4575	3081 / 0	0 / 0	0 / 0	0 / 0	1513 / 0	0 / 0
H	1365	901 / 0	0 / 0	0 / 0	0 / 0	464 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-M, C-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-6159 / 0	-91.8	-91.8	0.33 (1)	3.64	
B-C	-2798 / 0	-91.8	-91.8	0.33 (1)	5.17	
C-D	-2086 / 0	-91.8	-91.8	0.16 (1)	5.94	
D-E	-2086 / 0	-91.8	-91.8	0.16 (1)	5.94	
E-F	-2108 / 0	-91.8	-91.8	0.29 (1)	5.79	
F-G	-1945 / 0	-91.8	-91.8	0.24 (1)	5.98	
O-A	-5879 / 0	0.0	0.0	0.21 (1)	6.09	
H-G	-1901 / 0	0.0	0.0	0.07 (1)	7.81	
O-P	0 / 0	-18.5	-18.5	0.23 (1)	10.00	
P-N	0 / 0	-18.5	-18.5	0.23 (1)	10.00	
N-M	0 / 4975	-18.5	-18.5	0.41 (1)	10.00	
M-L	0 / 2236	-18.5	-18.5	0.18 (1)	10.00	
L-K	0 / 1662	-18.5	-18.5	0.13 (1)	10.00	
K-J	0 / 1662	-18.5	-18.5	0.13 (1)	10.00	
J-I	0 / 1612	-18.5	-18.5	0.13 (1)	10.00	
I-H	0 / 0	-18.5	-18.5	0.02 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-B	0 / 2897	0.36 (1)
B-M	-3081 / 0	0.51 (1)
M-C	0 / 1546	0.19 (1)
C-L	-310 / 0	0.08 (1)
L-D	-458 0	0.27 (1)
L-E	0 / 874	0.11 (1)
J-E	0 / 109	0.02 (4)
J-F	0 / 54	0.01 (1)
I-F	-628 / 0	0.07 (1)
A-N	0 / 5411	0.67 (1)
I-G	0 / 1753	0.22 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR	TYPE	REF	CONN.
N	2-7-2	-4750	-4750	---	FRONT	VERT	TOTAL	H	C1
P	1-0-12	-829	-829	---	FRONT	VERT	TOTAL	H	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 135 = 269 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.33/1.00 (A-B:1) , BC=0.41/1.00 (M-N:1) , WB=0.67/1.00 (A-N:1) , SSI=0.20/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (S) (INPUT = 0.90)
JSI METAL= 0.84 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006272

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	T3	6	1	GREEN PARK HOMES TRUSS DESC.	

Tamareck Roof Truss, Burlington

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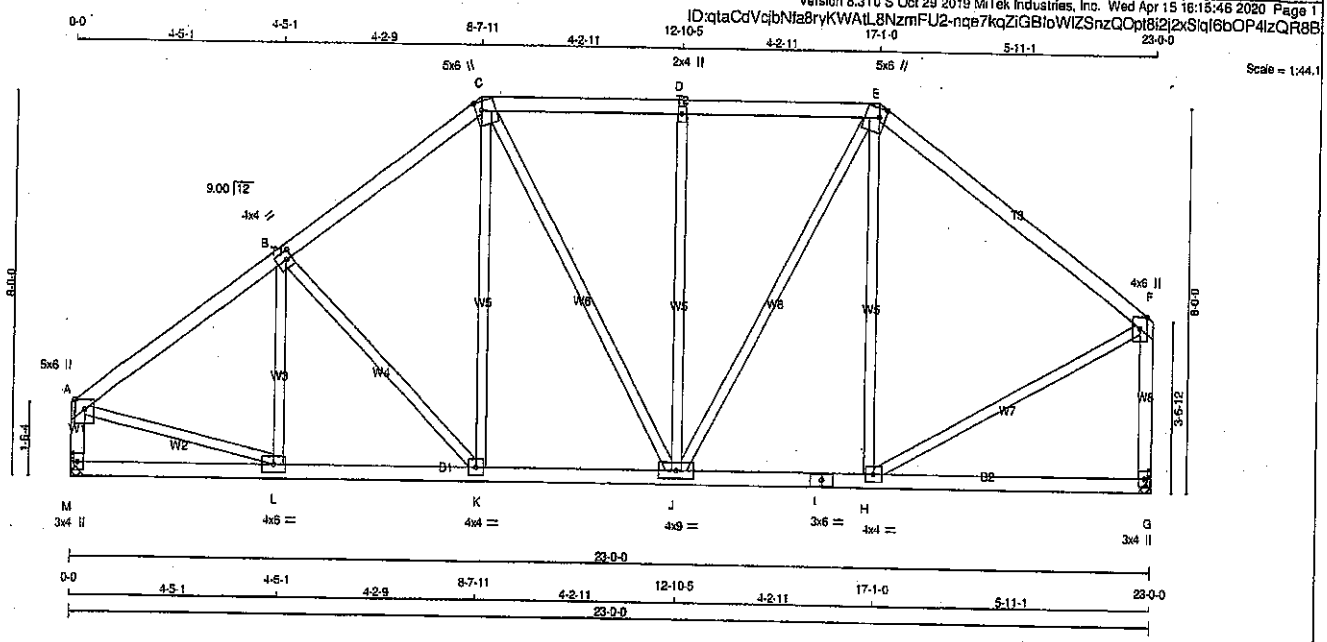

TOTAL WEIGHT = 6 X 111 = 664 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1268	0	1268	0	0	IN-SX	IN-SX
G	1268	0	1268	0	0	MECHANICAL	
						MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. G. MINIMUM BEARING LENGTH AT JOINT M = 3-8, JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	897	588 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0
G	897	588 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-1291 / 0	-91.8	-91.8	0.24 (1)	5.43
B-C	-1144 / 0	-91.8	-91.8	0.23 (1)	5.69
C-D	-958 / 0	-91.8	-91.8	0.21 (1)	6.10
D-E	-958 / 0	-91.8	-91.8	0.21 (1)	6.10
E-F	-923 / 0	-91.8	-91.8	0.43 (1)	5.84
M-A	-1233 / 0	0.0	0.0	0.13 (1)	7.23
G-F	-1223 / 0	0.0	0.0	0.26 (1)	7.25
M-L	0 / 0	-18.5	-18.5	0.08 (4)	10.00
L-K	0 / 1055	-18.5	-18.5	0.21 (1)	10.00
K-J	0 / 894	-18.5	-18.5	0.18 (1)	10.00
J-I	0 / 734	-18.5	-18.5	0.21 (4)	10.00
I-H	0 / 734	-18.5	-18.5	0.21 (4)	10.00
H-G	0 / 0	-18.5	-18.5	0.14 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-B	-199 / 4	0.07 (1)
B-K	-238 / 0	0.16 (1)
K-C	0 / 256	0.08 (1)
C-J	0 / 135	0.03 (1)
J-D	-469 / 0	0.57 (1)
J-E	0 / 467	0.11 (1)
H-E	-278 / 0	0.34 (1)
A-L	0 / 1093	0.25 (1)
H-F	0 / 837	0.19 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.43/1.00 (E-F:1) , BC=0.21/1.00 (K-L:1) , WB=0.57/1.00 (D-J:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	518	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.50 (A) (INPUT = 1.00)

Structural component only
DWG# T-2006273

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	G3	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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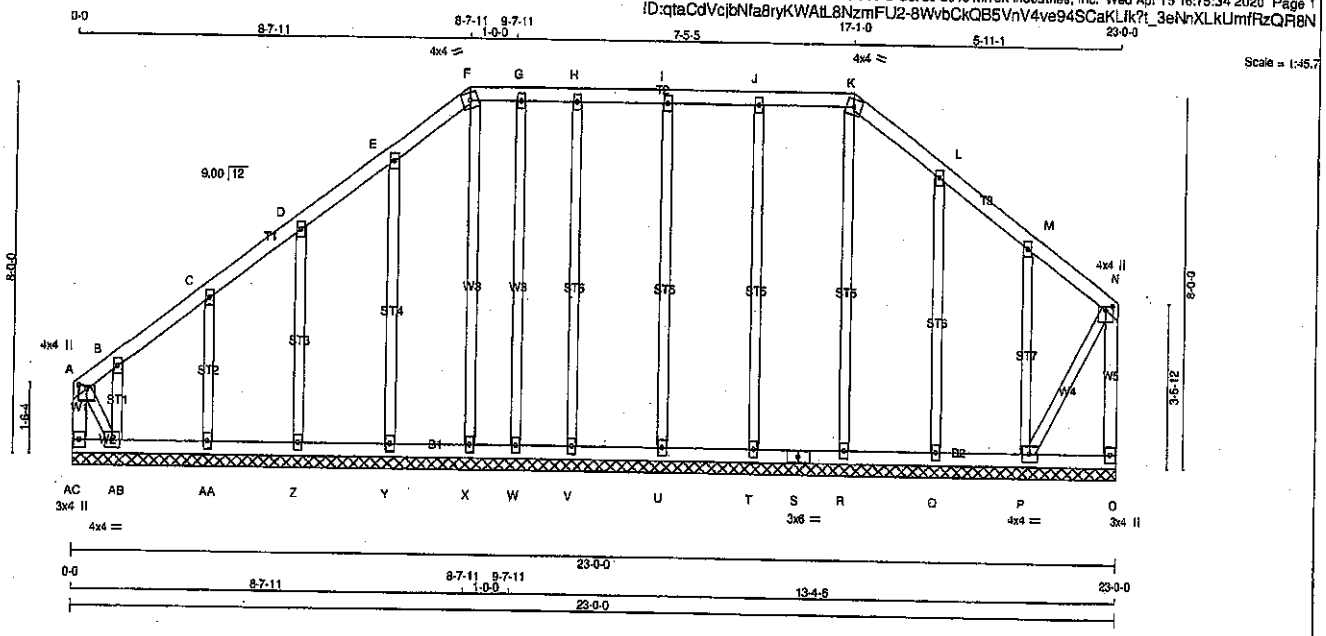

TOTAL WEIGHT = 123 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AC- A	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
K - N	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
AC- S	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B, C, D, E, G, H, I, J, L, M						
B	TMW+w	MT20	2.0	4.0		
F	TTW-m	MT20	4.0	4.0		
K	TTW-m	MT20	4.0	4.0		
N	TMVW+p	MT20	4.0	4.0	1.00	2.00
O	BMV1+p	MT20	3.0	4.0		
P	BMWW1-t	MT20	4.0	4.0		
Q, R, T, U, V, W, X, Y, Z, AA						
Q	BMW1+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	6.0		
AB	BMWW1-t	MT20	4.0	4.0		
AC	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
AC- A	-65 / 0	0.0	0.0	0.01 (1)		7.81
A- B	-16 / 0	-91.8	-91.8	0.03 (1)		6.25
B- C	-20 / 0	-91.8	-91.8	0.04 (1)		6.25
C- D	-17 / 0	-91.8	-91.8	0.04 (1)		6.25
D- E	-13 / 0	-91.8	-91.8	0.04 (1)		6.25
E- F	-20 / 0	-91.8	-91.8	0.04 (1)		6.25
F- G	-6 / 0	-91.8	-91.8	0.02 (1)		10.00
G- H	-6 / 0	-91.8	-91.8	0.03 (1)		10.00
H- I	-6 / 0	-91.8	-91.8	0.04 (1)		10.00
I- J	-6 / 0	-91.8	-91.8	0.05 (1)		10.00
J- K	-6 / 0	-91.8	-91.8	0.05 (1)		10.00
K- L	-21 / 0	-91.8	-91.8	0.05 (1)		6.25
L- M	-13 / 0	-91.8	-91.8	0.05 (1)		6.25
M- N	-8 / 0	-91.8	-91.8	0.05 (1)		10.00
O- N	-105 / 0	0.0	0.0	0.02 (1)		7.81
AC-AB	0 / 0	-18.5	-18.5	0.01 (4)		10.00
AB-AA	0 / 18	-18.5	-18.5	0.02 (4)		10.00
AA- Z	0 / 13	-18.5	-18.5	0.02 (4)		10.00
Z- Y	0 / 10	-18.5	-18.5	0.01 (4)		10.00
Y- X	0 / 7	-18.5	-18.5	0.01 (4)		10.00
X- W	0 / 6	-18.5	-18.5	0.01 (4)		10.00
W- V	0 / 6	-18.5	-18.5	0.01 (4)		10.00
V- U	0 / 6	-18.5	-18.5	0.02 (4)		10.00
U- T	0 / 6	-18.5	-18.5	0.02 (4)		10.00
T- S	0 / 6	-18.5	-18.5	0.02 (4)		10.00
S- R	0 / 6	-18.5	-18.5	0.02 (4)		10.00
R- Q	0 / 8	-18.5	-18.5	0.02 (4)		10.00
Q- P	0 / 11	-18.5	-18.5	0.02 (4)		10.00
P- O	0 / 0	-18.5	-18.5	0.02 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- K	-145 / 0	0.18 (1)
T- J	-210 / 0	0.26 (1)
U- I	-184 / 0	0.22 (1)
V- H	-152 / 0	0.19 (1)
Y- E	-187 / 0	0.14 (1)
Z- D	-180 / 0	0.07 (1)
AA- C	-186 / 0	0.04 (1)
AB- B	-148 / 0	0.02 (1)
Q- L	-199 / 0	0.14 (1)
P- M	-195 / 0	0.07 (1)
X- F	-97 / 0	0.12 (1)
W- G	-110 / 0	0.13 (1)
A-AB	0 / 40	0.01 (1)
P- N	0 / 27	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	5.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	38.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (J-K:1) , BC=0.02/1.00 (P-Q:4) , WB=0.26/1.00 (J-T:1) , SSI=0.09/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

(SI GRIP= 0.62 (K) (INPUT = 0.90)
(SI METAL= 0.11 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006257

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	T4	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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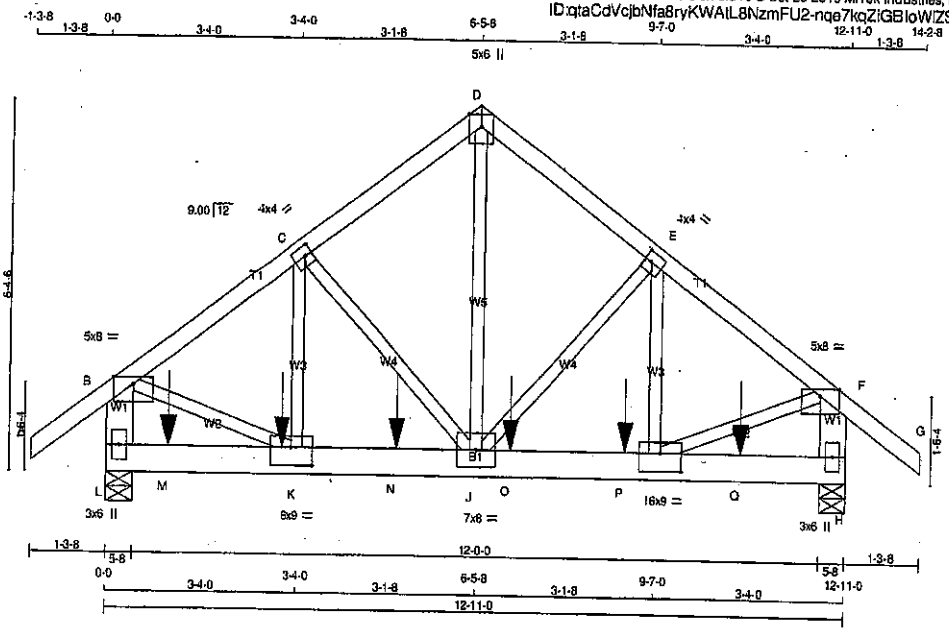

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- G	1	12	TOP
L- B	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- H	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
C- K	1	6	SIDE(182.0)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	5.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMW-p	MT20	5.0	8.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	6.0	9.0	3.75	4.50
J	BMWWW-t	MT20	7.0	8.0		
K	BMWW-t	MT20	6.0	9.0	3.75	4.50
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	5264	0	5264	0	0	IN-SX	IN-SX
H	4809	0	4809	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	3715	2482 / 0	0 / 0	0 / 0	0 / 0	1233 0	0 0
H	3394	2268 / 0	0 / 0	0 / 0	0 0	1126 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 36	-91.8	-91.8	0.07 (1)	10.00	
B- C	-4733 / 0	-91.8	-91.8	0.17 (1)	4.30	
C- D	-3712 / 0	-91.8	-91.8	0.12 (1)	4.80	
D- E	-3713 / 0	-91.8	-91.8	0.12 (1)	4.80	
E- F	-4665 / 0	-91.8	-91.8	0.16 (1)	4.33	
F- G	0 / 36	-91.8	-91.8	0.07 (1)	10.00	
L- B	-4373 / 0	0.0	0.0	0.16 (1)	6.87	
H- F	-4314 / 0	0.0	0.0	0.16 (1)	6.90	
L- M	0 / 0	-18.5	-18.5	0.34 (1)	10.00	
M- K	0 / 0	-18.5	-18.5	0.34 (1)	10.00	
K- N	0 / 3793	-18.5	-18.5	0.48 (1)	10.00	
N- J	0 / 3793	-18.5	-18.5	0.48 (1)	10.00	
J- O	0 / 3739	-18.5	-18.5	0.47 (1)	10.00	
O- P	0 / 3739	-18.5	-18.5	0.47 (1)	10.00	
P- I	0 / 3739	-18.5	-18.5	0.47 (1)	10.00	
I- Q	0 / 0	-18.5	-18.5	0.33 (1)	10.00	
Q- H	0 / 0	-18.5	-18.5	0.33 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	0 / 4187	0.52 (1)
J- E	-1185 / 0	0.22 (1)
I- E	0 / 1152	0.14 (1)
C- J	-1268 / 0	0.24 (1)
K- C	0 / 1256	0.16 (1)
B- K	0 / 3997	0.49 (1)
I- F	0 / 3940	0.49 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL CONN.
K	3-0-12	-1399	-1399	---	FRONT	VERT	TOTAL	C1
M	1-0-12	-1399	-1399	---	FRONT	VERT		
N	5-0-12	-1399	-1399	---	FRONT	VERT	TOTAL	C1
O	7-0-12	-1399	-1399	---	FRONT	VERT	TOTAL	C1
P	9-0-12	-1399	-1399	---	FRONT	VERT	TOTAL	C1
Q	11-0-12	-1399	-1399	---	FRONT	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G. ALVES
100090245
PROVINCE OF ONTARIO

TOTAL WEIGHT = 2 X 71 = 143 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.17/1.00 (B-C:1) , BC=0.48/1.00 (J-K:1) , WB=0.52/1.00 (D-J:1) , SSI=0.47/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.40 (D) (INPUT = 1.00)

Structural component only
DWG# T-2006274

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408128	T5	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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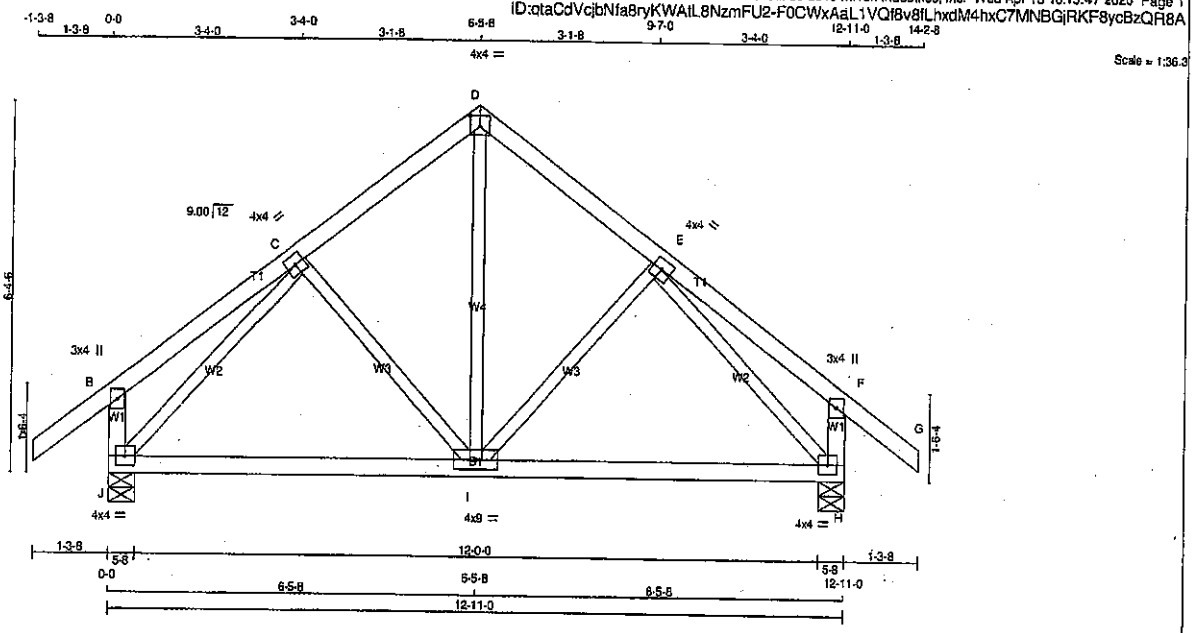


TOTAL WEIGHT = 3 X 59 = 177 lb

M|F

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	839	0	839	0	0	5-8	5-8
H	839	0	839	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	591	401 / 0	0 / 0	0 / 0	0 / 0	190 / 0	0 / 0
H	591	401 / 0	0 / 0	0 / 0	0 / 0	190 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

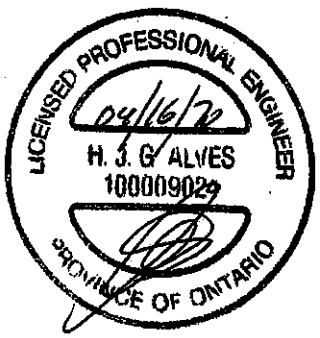
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 38	-91.8	-91.8	0.12 (1)	10.00
B-C	0 / 20	-91.8	-91.8	0.15 (1)	10.00
C-D	-514 / 0	-91.8	-91.8	0.12 (1)	6.25
D-E	-514 / 0	-91.8	-91.8	0.12 (1)	6.25
E-F	0 / 20	-91.8	-91.8	0.15 (1)	10.00
F-G	0 / 38	-91.8	-91.8	0.12 (1)	10.00
J-B	-240 / 0	0.0	0.0	0.03 (1)	7.81
H-F	-240 / 0	0.0	0.0	0.03 (1)	7.81
J-I	0 / 492	-18.5	-18.5	0.26 (4)	10.00
I-H	0 / 492	-18.5	-18.5	0.26 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-D	0 / 361	0.06 (1)
I-E	-144 / 0	0.06 (1)
C-I	-144 / 0	0.06 (1)
J-C	-740 / 0	0.27 (1)
E-H	-740 / 0	0.27 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.26/1.00 (I-J:4) , WB=0.27/1.00 (E-H:1) , SSI=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (C) (INPUT = 0.90)
JSI METAL= 0.27 (C) (INPUT = 1.00)

Structural component only
DWG# T-2006275

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	T5S	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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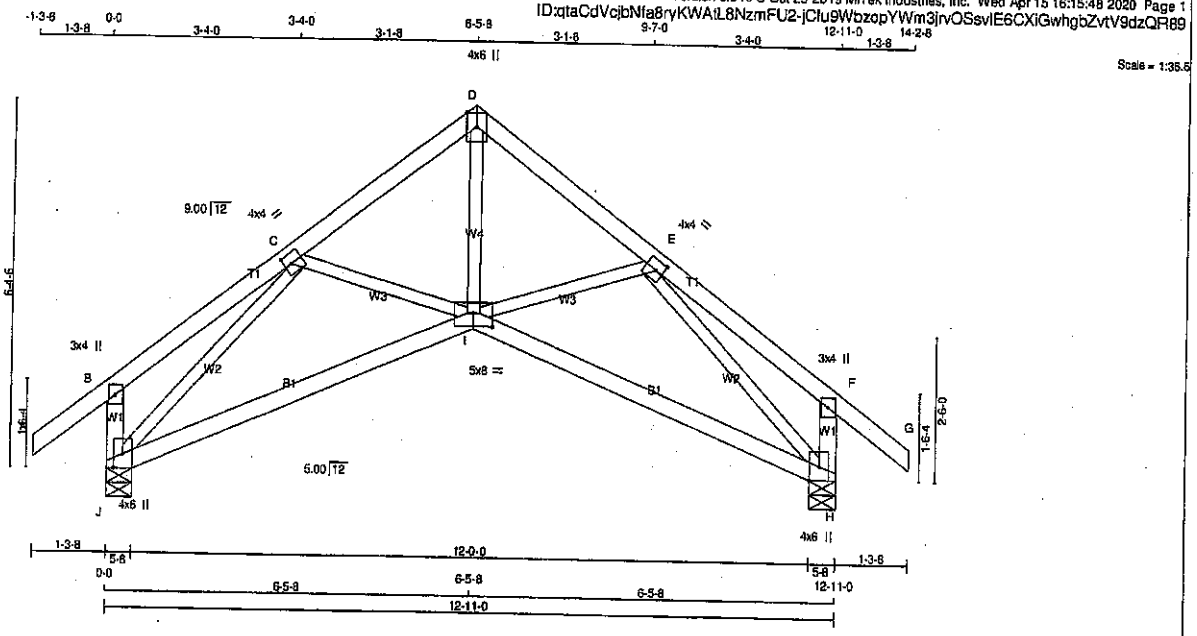

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1+p	MT20	4.0	8.0	Edge	
I	BBWWW-p	MT20	5.0	8.0	2.75	4.00
J	BMVW1+p	MT20	4.0	6.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	839	0	839	0	0	5-8	5-8
H	839	0	839	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	591	401 / 0	0 / 0	0 / 0	0 / 0	190 / 0	0 / 0
H	591	401 / 0	0 / 0	0 / 0	0 / 0	190 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 38	-91.8	-91.8	0.12 (1)		10.00
B-C	0 / 18	-91.8	-91.8	0.13 (1)		10.00
C-D	-833 / 0	-91.8	-91.8	0.11 (1)		6.25
D-E	-833 / 0	-91.8	-91.8	0.11 (1)		6.25
E-F	0 / 18	-91.8	-91.8	0.13 (1)		10.00
F-G	0 / 38	-91.8	-91.8	0.12 (1)		10.00
J-B	-245 / 0	0.0	0.0	0.03 (1)		7.81
H-F	-245 / 0	0.0	0.0	0.03 (1)		7.81
J-I	0 / 763	-18.5	-18.5	0.28 (4)		10.00
I-H	0 / 763	-18.5	-18.5	0.28 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-D	0 / 743	0.17 (1)
I-E	-55 / 21	0.01 (1)
C-I	-55 / 21	0.01 (1)
J-C	-1087 / 0	0.42 (1)
E-H	-1087 / 0	0.42 (1)

TOTAL WEIGHT = 2 X 57 = 114 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.13/1.00 (E-F:1) , BC=0.28/1.00 (H-I:4) , WB=0.42/1.00 (C-J:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.49 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G ALVES
100090247
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006276

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	T6	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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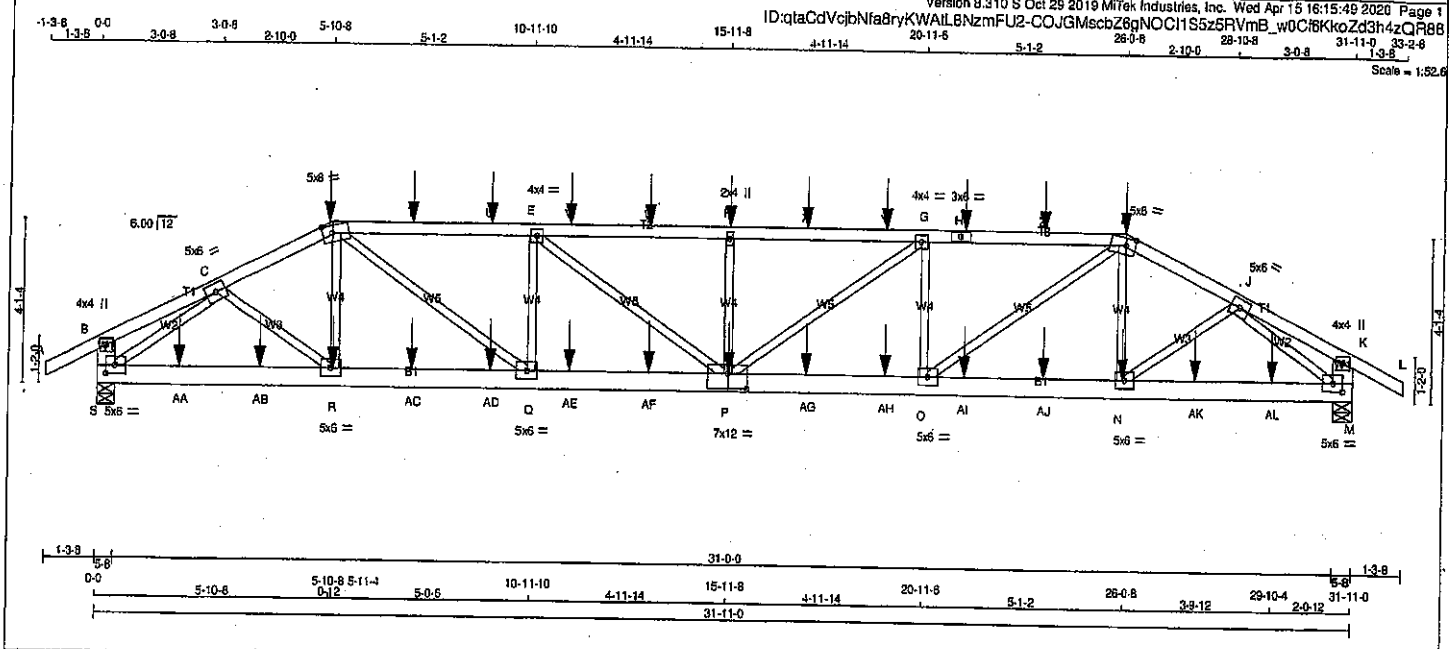


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-D	1	12	SIDE(61.0)
D-H	1	12	SIDE(61.0)
H-I	1	12	SIDE(61.0)
I-L	1	12	SIDE(61.0)
S-B	2	12	TOP
M-K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S-P	2	12	SIDE(183.1)
P-M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	8.0	2.25	3.00
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TTWW-m	MT20	5.0	8.0	2.25	3.00
J	TMWW-t	MT20	5.0	6.0		
K	TMV+p	MT20	4.0	4.0		
M	BMWW1-t	MT20	5.0	6.0	2.50	2.75
N, O, Q, R						
N	BMWW-t	MT20	5.0	6.0		
P	BSWWW-t	MT20	7.0	12.0	4.50	6.00
S	BMWW1-t	MT20	5.0	6.0	2.50	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2984	0	2984	0	0	5-8	5-8
M	2985	0	2985	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2110	1385 / 0	0 / 0	0 / 0	0 0	725 0	0 0
M	2111	1385 / 0	0 / 0	0 / 0	0 0	725 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, M

BRACING

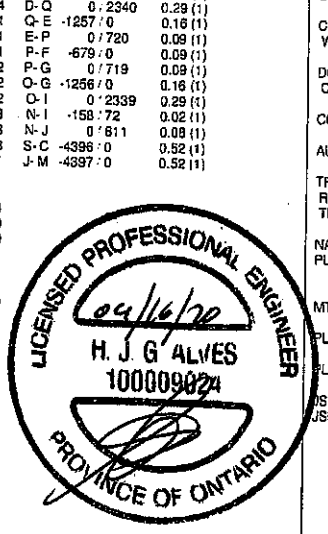
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A-B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	C-R	0 / 611	0.08 (1)
B-C	0 / 6	-91.8	-91.8	0.06 (1)	10.00	R-D	-158 / 72	0.02 (1)
C-D	-4468 / 0	-91.8	-91.8	0.13 (1)	4.44	D-Q	0 / 2340	0.29 (1)
D-T	-5872 / 0	-91.8	-91.8	0.48 (1)	3.61	Q-E	-1257 / 0	0.16 (1)
T-U	-5872 / 0	-91.8	-91.8	0.48 (1)	3.61	E-P	0 / 720	0.09 (1)
U-E	-5872 / 0	-91.8	-91.8	0.48 (1)	3.61	P-F	-679 / 0	0.09 (1)
E-V	-6448 / 0	-91.8	-91.8	0.52 (1)	3.42	P-G	0 / 719	0.09 (1)
V-W	-6448 / 0	-91.8	-91.8	0.52 (1)	3.42	O-G	-1256 / 0	0.16 (1)
W-F	-6448 / 0	-91.8	-91.8	0.52 (1)	3.42	O-I	0 / 2339	0.29 (1)
F-X	-6448 / 0	-91.8	-91.8	0.51 (1)	3.43	N-I	-158 / 72	0.02 (1)
X-Y	-6448 / 0	-91.8	-91.8	0.51 (1)	3.43	N-J	0 / 611	0.08 (1)
Y-G	-6448 / 0	-91.8	-91.8	0.51 (1)	3.43	S-C	-4396 / 0	0.52 (1)
G-H	-5872 / 0	-91.8	-91.8	0.48 (1)	3.61	J-M	-4397 / 0	0.52 (1)
H-Z	-5872 / 0	-91.8	-91.8	0.48 (1)	3.61			
Z-I	-5872 / 0	-91.8	-91.8	0.48 (1)	3.61			
I-J	-4470 / 0	-91.8	-91.8	0.13 (1)	4.44			
J-K	0 / 6	-91.8	-91.8	0.06 (1)	10.00			
K-L	0 / 28	-91.8	-91.8	0.07 (1)	10.00			
S-B	-247 / 0	0.0	0.0	0.01 (1)	7.81			
M-K	-247 / 0	0.0	0.0	0.01 (1)	7.81			
S-AA	0 / 3496	-18.5	-18.5	0.27 (1)	10.00			
AA-AB	0 / 3496	-18.5	-18.5	0.27 (1)	10.00			
AB-R	0 / 3496	-18.5	-18.5	0.27 (1)	10.00			
R-AC	0 / 3982	-18.5	-18.5	0.30 (1)	10.00			
AC-AD	0 / 3982	-18.5	-18.5	0.30 (1)	10.00			
AD-Q	0 / 3982	-18.5	-18.5	0.30 (1)	10.00			
Q-AE	0 / 5872	-18.5	-18.5	0.43 (1)	10.00			
AE-AF	0 / 5872	-18.5	-18.5	0.43 (1)	10.00			
AF-P	0 / 5872	-18.5	-18.5	0.43 (1)	10.00			
P-AG	0 / 5872	-18.5	-18.5	0.43 (1)	10.00			
AG-AH	0 / 5872	-18.5	-18.5	0.43 (1)	10.00			
AH-O	0 / 5872	-18.5	-18.5	0.43 (1)	10.00			
O-AI	0 / 3983	-18.5	-18.5	0.30 (1)	10.00			
AI-AJ	0 / 3983	-18.5	-18.5	0.30 (1)	10.00			
AJ-N	0 / 3983	-18.5	-18.5	0.30 (1)	10.00			
N-AK	0 / 3497	-18.5	-18.5	0.27 (1)	10.00			
AK-AL	0 / 3497	-18.5	-18.5	0.27 (1)	10.00			
AL-M	0 / 3497	-18.5	-18.5	0.27 (1)	10.00			



TOTAL WEIGHT = 2 X 144 = 289 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.52/1.00 (E-F:1) ; BC=0.43/1.00 (P-Q:1) , WB=0.52/1.00 (J-M:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (M) (INPUT = 0.90)
JSI METAL= 0.57 (J) (INPUT = 1.00)

Structural component only 1/2
DWG# T-2006277

CONTINUED ON PAGE 2

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-406	-406	--	FRONT	VERT	TOTAL	--	C1
F	15-11-12	-110	-110	--	FRONT	VERT	TOTAL	--	C1
H	21-11-12	-110	-110	--	FRONT	VERT	TOTAL	--	C1
I	26-0-8	-406	-406	--	FRONT	VERT	TOTAL	--	C1
N	25-11-12	-26	-26	--	FRONT	VERT	TOTAL	--	C1
P	15-11-12	-26	-26	--	FRONT	VERT	TOTAL	--	C1
R	5-11-4	-26	-26	--	FRONT	VERT	TOTAL	--	C1
T	7-11-12	-110	-110	--	FRONT	VERT	TOTAL	--	C1
U	9-11-12	-110	-110	--	FRONT	VERT	TOTAL	--	C1
V	11-11-12	-110	-110	--	FRONT	VERT	TOTAL	--	C1
W	13-11-12	-110	-110	--	FRONT	VERT	TOTAL	--	C1
X	17-11-12	-110	-110	--	FRONT	VERT	TOTAL	--	C1
Y	19-11-12	-110	-110	--	FRONT	VERT	TOTAL	--	C1
Z	23-11-12	-110	-110	--	FRONT	VERT	TOTAL	--	C1
AA	2-0-12	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AB	4-0-12	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AC	7-11-12	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AD	9-11-12	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AE	11-11-12	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AF	13-11-12	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AG	17-11-12	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AH	19-11-12	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AI	21-11-12	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AJ	23-11-12	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AK	27-10-4	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AL	29-10-4	-26	-26	--	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006277

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	T7	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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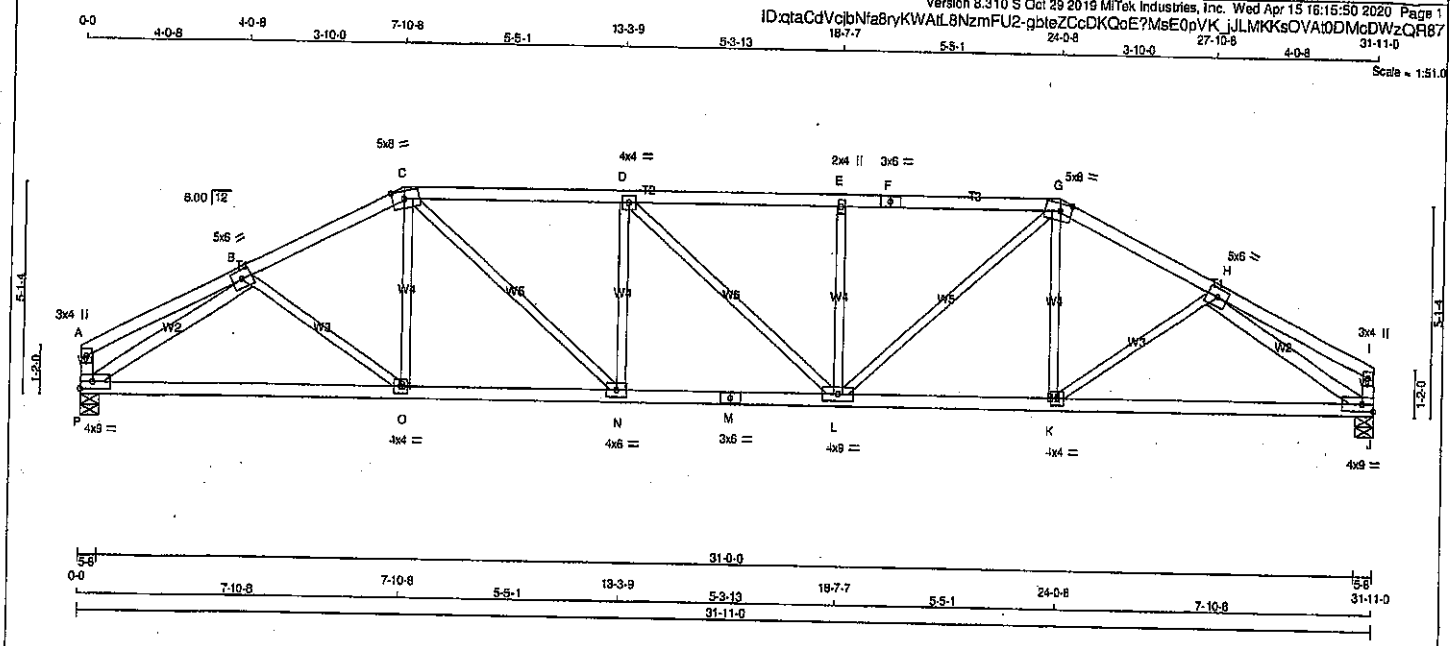


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - A	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW-m	MT20	5.0	6.0	2.25	3.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TTWW-m	MT20	5.0	6.0	2.25	3.50
H	TMWW-t	MT20	5.0	6.0		
I	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	9.0		Edge
K	BMWW-t	MT20	4.0	4.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1759	0	1759	0	0	5-8	5-8
J	1759	0	1759	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1244	817 / 0	0 / 0	0 / 0	0 : 0	428 : 0	0 0
J	1244	817 / 0	0 / 0	0 / 0	0 : 0	428 : 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

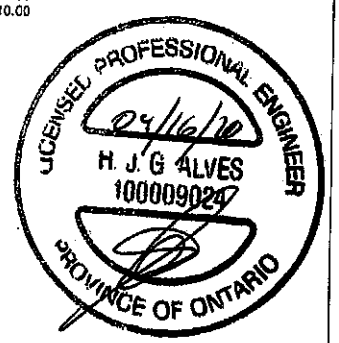
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 17	-91.8	-91.8	0.20 (1)	10.00	
B-C	-2469 / 0	-91.8	-91.8	0.30 (1)	4.12	
C-D	-2893 / 0	-91.8	-91.8	0.54 (1)	3.60	
D-E	-2891 / 0	-91.8	-91.8	0.53 (1)	3.61	
E-F	-2891 / 0	-91.8	-91.8	0.53 (1)	3.62	
F-G	-2891 / 0	-91.8	-91.8	0.53 (1)	3.62	
G-H	-2469 / 0	-91.8	-91.8	0.30 (1)	4.12	
H-I	0 / 17	-91.8	-91.8	0.20 (1)	10.00	
P-A	-145 / 0	0.0	0.0	0.01 (1)	7.81	
J-I	-145 / 0	0.0	0.0	0.01 (1)	7.81	
P-O	0 / 2167	-18.5	-18.5	0.49 (1)	10.00	
O-N	0 / 2194	-18.5	-18.5	0.48 (1)	10.00	
N-M	0 / 2893	-18.5	-18.5	0.53 (1)	10.00	
M-L	0 / 2893	-18.5	-18.5	0.53 (1)	10.00	
L-K	0 / 2194	-18.5	-18.5	0.49 (1)	10.00	
K-J	0 / 2168	-18.5	-18.5	0.49 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-O	0 / 74	0.03 (4)
O-C	0 / 133	0.04 (4)
C-N	0 / 933	0.21 (1)
N-D	-534 / 0	0.21 (1)
D-L	-3 / 0	0.00 (1)
L-E	-532 / 0	0.20 (1)
L-G	0 / 930	0.21 (1)
K-G	0 / 134	0.04 (4)
K-H	0 / 74	0.03 (4)
P-B	-2666 / 0	0.74 (1)
H-J	-2667 / 0	0.74 (1)



TOTAL WEIGHT = 126 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.54/1.00 (C-D:1) , BC=0.53/1.00 (L-N:1) , WB=0.74/1.00 (H-J:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (M) (INPUT = 0.90)
JSI METAL= 0.94 (M) (INPUT = 1.00)

Structural component only
DWG# T-2006278

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	T8	1	3	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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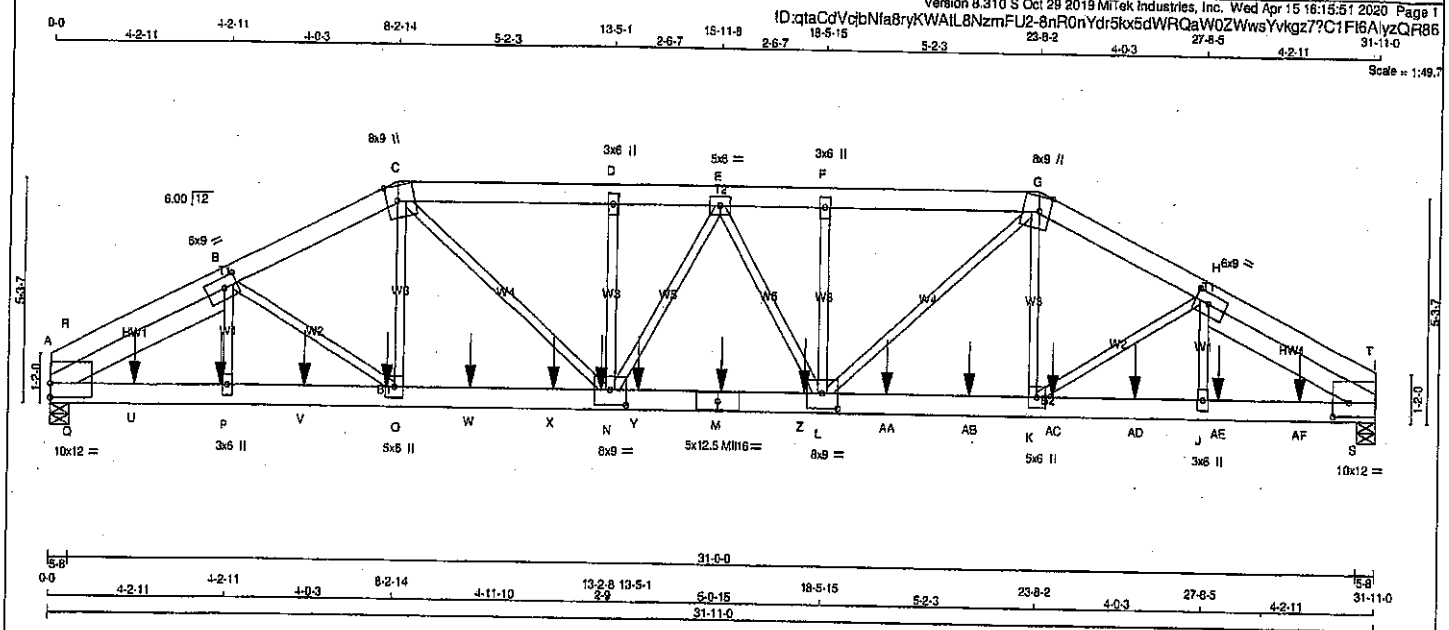


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
A - M	2x6	DRY	2100F 1.8E	SPF
M - I	2x6	DRY	2100F 1.8E	SPF

REINFORCING MEMBERS

HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	2	12	TOP
C-G	2	12	TOP
G-I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
A-M	2	10	SIDE(780.3)
M-I	2	10	SIDE(365.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
D-N	1	6	SIDE(137.8)
2x6	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-I	MT20	10.0	12.0	4.00	
B	TMWWW-I	MT20	6.0	9.0	3.00	4.00
C	TTWW+m	MT20	8.0	9.0	4.25	3.00
D	TMW+w	MT20	3.0	6.0		
E	TMWW-I	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TTWW+m	MT20	8.0	9.0	4.25	3.00
H	TMWWW-I	MT20	6.0	9.0	3.00	4.00
I	TMBMW1-I	MT20	10.0	12.0	4.00	4.50
J	BMW+w	MT20	3.0	6.0		
K	BMWW+I	MT20	5.0	6.0		
L	BMWWW-I	MT20	8.0	9.0	4.25	4.50
M	BS-I	MII16	5.0	12.5		
N	BMWWW-I	MT20	8.0	9.0	4.25	4.50
O	BMWW+I	MT20	5.0	6.0		
P	BMW+w	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
A	12581	0	12581	0	0	5-8	5-8
I	12942	0	12942	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	8883	5910 / 0	0 / 0	0 / 0	0 0	2972 0	0 0
I	9138	6077 / 0	0 / 0	0 / 0	0 0	3061 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, I

BEARING SIZE FACTOR = 1.15 AT JNT(S) A, I (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.92 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-R	-14496 / 0	-91.8	-91.8	0.29 (1)	3.73
R-B	-8445 / 0	-91.8	-91.8	0.23 (1)	4.77
B-C	-19831 / 0	-91.8	-91.8	0.33 (1)	3.21
C-D	-23338 / 0	-91.8	-91.8	0.42 (1)	2.92
D-E	-23338 / 0	-91.8	-91.8	0.35 (1)	2.97
E-F	-22722 / 0	-91.8	-91.8	0.32 (1)	3.03
F-G	-22722 / 0	-91.8	-91.8	0.40 (1)	2.98
G-H	-19744 / 0	-91.8	-91.8	0.33 (1)	3.22
H-T	-8751 / 0	-91.8	-91.8	0.24 (1)	4.70
T-I	-15059 / 0	-91.8	-91.8	0.30 (1)	3.65
A-Q	0 / 7403	-18.5	-18.5	0.12 (1)	10.00
Q-U	0 / 17570	-18.5	-18.5	0.42 (1)	10.00
U-P	0 / 17570	-18.5	-18.5	0.42 (1)	10.00
P-V	0 / 17525	-18.5	-18.5	0.42 (1)	10.00
V-O	0 / 17525	-18.5	-18.5	0.42 (1)	10.00
O-W	0 / 17838	-18.5	-18.5	0.42 (1)	10.00
W-X	0 / 17838	-18.5	-18.5	0.42 (1)	10.00
X-N	0 / 17838	-18.5	-18.5	0.42 (1)	10.00
N-Y	0 / 23136	-18.5	-18.5	0.54 (1)	10.00
Y-M	0 / 23136	-18.5	-18.5	0.54 (1)	10.00
M-Z	0 / 23136	-18.5	-18.5	0.54 (1)	10.00
Z-L	0 / 23136	-18.5	-18.5	0.54 (1)	10.00
L-AA	0 / 17773	-18.5	-18.5	0.46 (1)	10.00
AA-AB	0 / 17773	-18.5	-18.5	0.46 (1)	10.00
AB-K	0 / 17773	-18.5	-18.5	0.46 (1)	10.00
K-AC	0 / 17768	-18.5	-18.5	0.46 (1)	10.00
AC-AD	0 / 17768	-18.5	-18.5	0.46 (1)	10.00
AD-J	0 / 17768	-18.5	-18.5	0.46 (1)	10.00
J-AE	0 / 17821	-18.5	-18.5	0.44 (1)	10.00
AE-AF	0 / 17821	-18.5	-18.5	0.44 (1)	10.00
AF-S	0 / 17821	-18.5	-18.5	0.44 (1)	10.00
S-I	0 / 7675	-18.5	-18.5	0.13 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-B	0 / 2286	0.17 (1)
B-O	0 / 297	0.02 (1)
O-C	0 / 3164	0.24 (1)
C-N	0 / 7508	0.58 (1)
N-D	-143 / 23	0.02 (1)
L-F	-169 / 8	0.02 (1)
L-G	0 / 6755	0.51 (1)
K-G	0 3648	0.27 (1)
K-H	-91 / 0	0.01 (1)
J-H	0 2677	0.20 (1)
N-E	0 435	0.03 (1)
E-L	-888 / 0	0.14 (1)
Q-R	0 8219	0.00 (1)
Q-B	-12657 / 0	0.81 (1)
S-T	0 8604	0.00 (1)
S-H	-12631 0	0.81 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	PANEL	CONN.
M	16-0-12	-1399	-1399	---	BACK	VERT	TOTAL	---	C1
N	13-2-8	-1913	-1913	---	BACK	VERT	TOTAL	---	C1
O	8-0-12	-1250	-1250	---	BACK	VERT	TOTAL	---	C1
P	4-0-12	-1250	-1250	---	BACK	VERT	TOTAL	---	C1
U	2-0-12	-1250	-1250	---	BACK	VERT	TOTAL	---	C1
V	6-0-12	-1250	-1250	---	BACK	VERT	TOTAL	---	C1

TOTAL WEIGHT = 3 X 180 = 539 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/240 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 854 (0.45")

CSI: TC=0.42/1.00 (C-D:1) , BC=0.54/1.00 (L-N:1) , WB=0.81/1.00 (B-Q:1) , SSI=0.45/1.00 (L-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

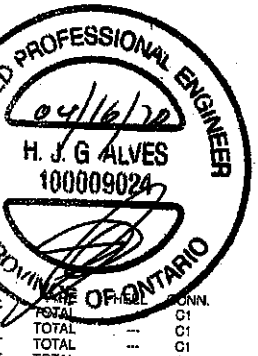
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656
MII16	438	276	2341	1245	4283	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.82 (M) (INPUT = 1.00)



Structural component only H/2

DWG# T-2006279

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408128	T8	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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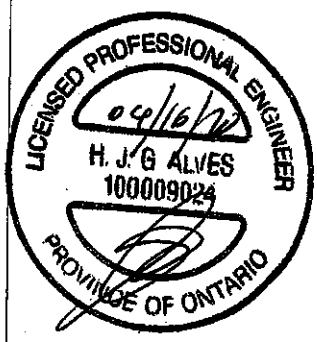
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
W	10-0-12	-1250	-1250	---	BACK	VERT	TOTAL	---	C1
X	12-0-12	-1250	-1250	---	BACK	VERT	TOTAL	---	C1
Y	14-0-12	-1399	-1399	---	BACK	VERT	TOTAL	---	C1
Z	16-0-12	-1399	-1399	---	BACK	VERT	TOTAL	---	C1
AA	20-0-12	-1399	-1399	---	BACK	VERT	TOTAL	---	C1
AB	22-0-12	-1399	-1399	---	BACK	VERT	TOTAL	---	C1
AC	24-0-12	-1399	-1399	---	BACK	VERT	TOTAL	---	C1
AD	26-0-12	-1399	-1399	---	BACK	VERT	TOTAL	---	C1
AE	28-0-12	-1399	-1399	---	BACK	VERT	TOTAL	---	C1
AF	30-0-12	-1399	-1399	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2006279

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	T9S	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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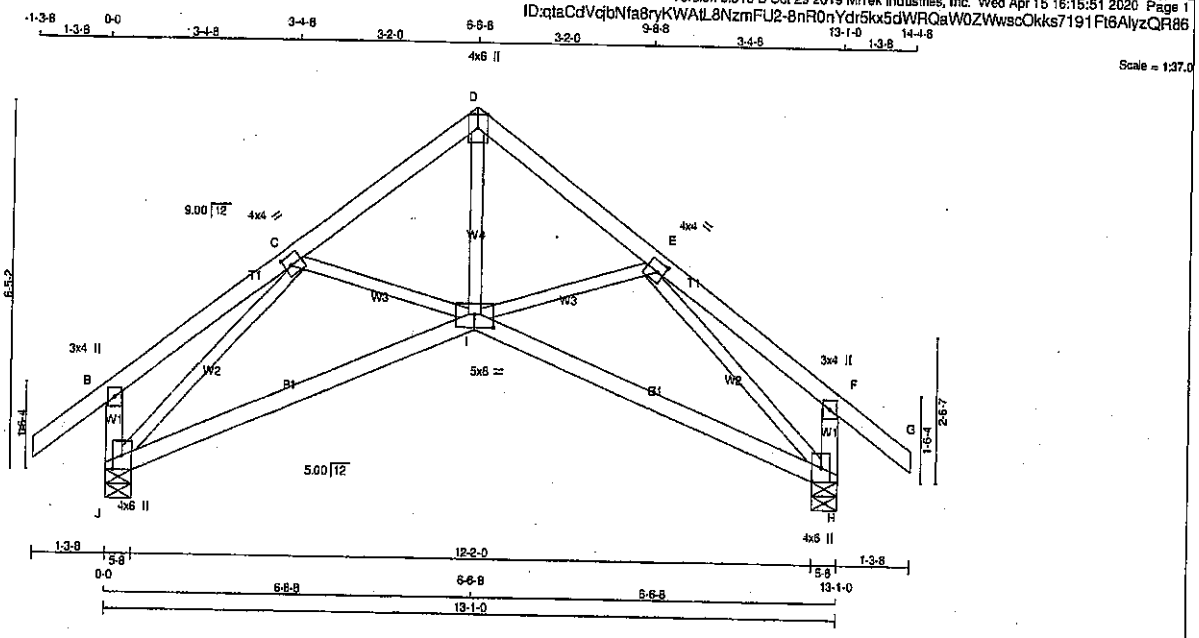

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1+p	MT20	4.0	6.0	Edge	
I	BBWWW-p	MT20	5.0	8.0	2.75	4.00
J	BMVW1+p	MT20	4.0	6.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	848	0	848	0	0	5-8	5-8
H	848	0	848	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	597	405 / 0	0 / 0	0 / 0	0 / 0	192 0	0 0
H	597	405 / 0	0 / 0	0 / 0	0 / 0	192 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 38	-91.8	-91.8	0.12 (1)	10.00
B-C	0 / 18	-91.8	-91.8	0.14 (1)	10.00
C-D	-848 / 0	-91.8	-91.8	0.11 (1)	6.25
D-E	-848 / 0	-91.8	-91.8	0.11 (1)	6.25
E-F	0 / 18	-91.8	-91.8	0.14 (1)	10.00
F-G	0 / 38	-91.8	-91.8	0.12 (1)	10.00
J-B	-247 / 0	0.0	0.0	0.03 (1)	7.81
H-F	-247 / 0	0.0	0.0	0.03 (1)	7.81
J-I	0 / 779	-18.5	-18.5	0.29 (4)	10.00
I-H	0 / 779	-18.5	-18.5	0.29 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-D	0 / 757	0.17 (1)
I-E	-58 / 21	0.01 (1)
C-I	-58 / 21	0.01 (1)
J-C	-1106 / 0	0.44 (1)
E-H	-1106 / 0	0.44 (1)

TOTAL WEIGHT = 2 X 57 = 115 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	5.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.14/1.00 (B-C:1) , BC=0.29/1.00 (H-I:4) , WB=0.44/1.00 (C-J:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (E) (INPUT = 0.90)
JSI METAL= 0.50 (J) (INPUT = 1.00)

Structural component only
DWG# T-2006280

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	T10	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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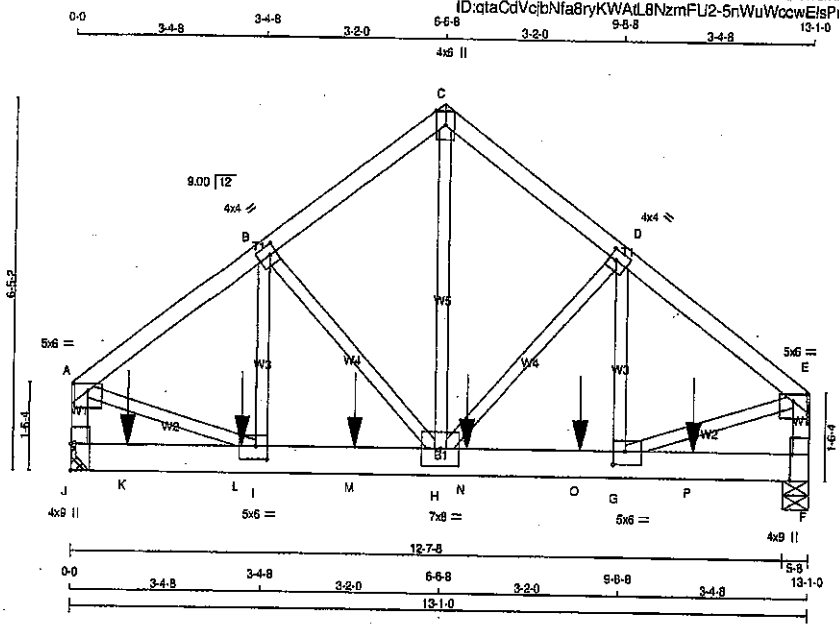

TOTAL WEIGHT = 2 X 67 = 134 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
J- A	1	12	TOP
F- E	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J- F	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- I	1	6	SIDE(142.9)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	5.0	6.0	1.50	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTW-p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TMWW-p	MT20	5.0	6.0	1.50	3.00
F	BMV1+t	MT20	4.0	9.0	Edge	0.50
G	BMWW-t	MT20	5.0	6.0	2.75	2.50
H	BMWWW-t	MT20	7.0	8.0		
I	BMWW-t	MT20	5.0	6.0	2.75	2.50
J	BMV1+t	MT20	4.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	4768	0	4768	0	0	MECHANICAL	
F	4171	0	4171	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	3366	2241 / 0	0 / 0	0 / 0	0 / 0	1126 / 0	0 / 0
F	2945	1960 / 0	0 / 0	0 / 0	0 / 0	966 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-4322 / 0	-91.8	-91.8	0.15 (1)		4.49
B- C	-3367 / 0	-91.8	-91.8	0.11 (1)		5.00
C- D	-3367 / 0	-91.8	-91.8	0.11 (1)		5.00
D- E	-4234 / 0	-91.8	-91.8	0.15 (1)		4.53
J- A	-3877 / 0	0.0	0.0	0.22 (1)		6.00
F- E	-3801 / 0	0.0	0.0	0.21 (1)		6.04
J- K	0 / 0	-18.5	-18.5	0.33 (1)		10.00
K- L	0 / 0	-18.5	-18.5	0.33 (1)		10.00
L- I	0 / 0	-18.5	-18.5	0.33 (1)		10.00
I- M	0 / 3466	-18.5	-18.5	0.41 (1)		10.00
M- H	0 / 3466	-18.5	-18.5	0.41 (1)		10.00
H- N	0 / 3396	-18.5	-18.5	0.41 (1)		10.00
N- O	0 / 3396	-18.5	-18.5	0.41 (1)		10.00
O- G	0 / 3396	-18.5	-18.5	0.41 (1)		10.00
G- P	0 / 0	-18.5	-18.5	0.27 (1)		10.00
P- F	0 / 0	-18.5	-18.5	0.27 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	0 / 3773	0.47 (1)
H- D	-1083 / 0	0.21 (1)
G- D	0 / 1015	0.13 (1)
B- H	-1190 / 0	0.23 (1)
I- B	0 / 1149	0.14 (1)
A- I	0 / 3648	0.45 (1)
G- E	0 / 3574	0.44 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	CONN.
K	1-0-4	-1250	-1250	--	BACK	VERT	TOTAL	C1
L	3-0-4	-1250	-1250	--	BACK	VERT	TOTAL	C1
M	5-0-4	-1250	-1250	--	BACK	VERT	TOTAL	C1
N	7-0-4	-1250	-1250	--	BACK	VERT	TOTAL	C1
O	9-0-4	-1250	-1250	--	BACK	VERT	TOTAL	C1
P	11-0-4	-1250	-1250	--	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.22/1.00 (A-J:1) , BC=0.41/1.00 (H-I:1) , WB=0.47/1.00 (C-H:1) , SSI=0.55/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.90 (I) (INPUT = 0.90)
USI METAL= 0.45 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. GALVES
100009028
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006281

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408129	T11	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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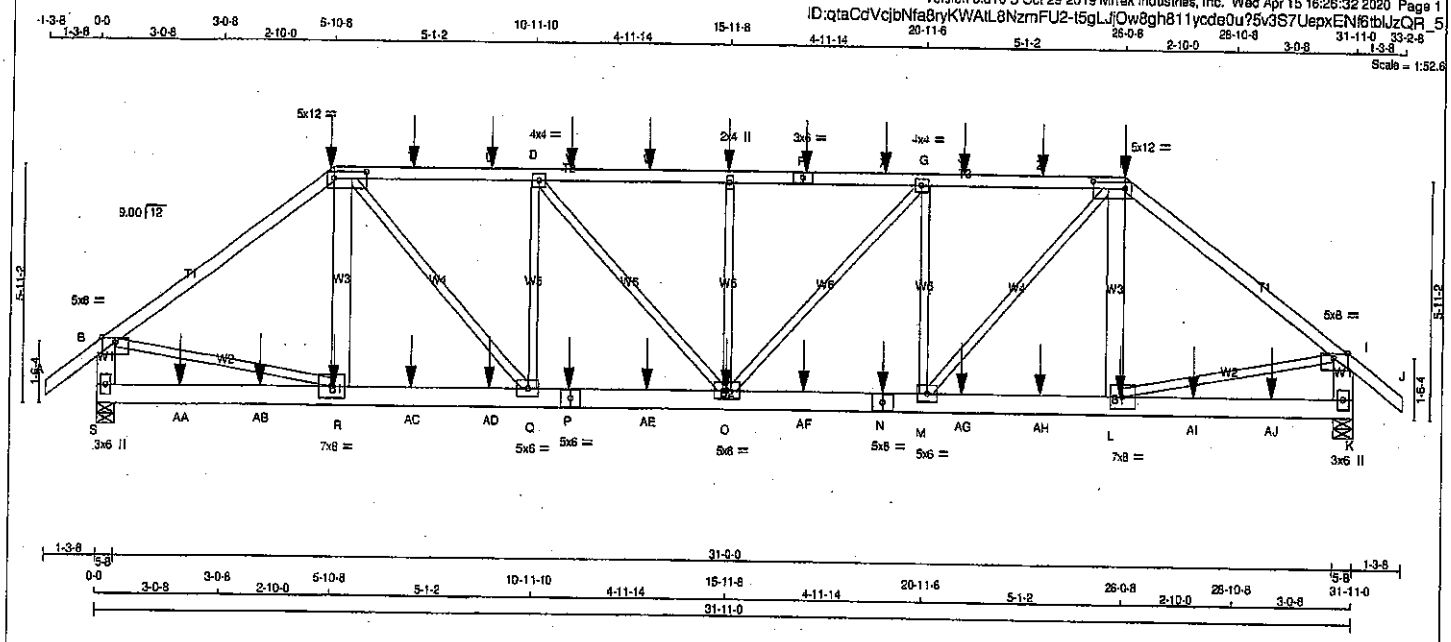


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - C	2x6	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-F	1	12	SIDE(61.0)
F-H	1	12	SIDE(61.0)
H-J	1	12	SIDE(61.0)
S-B	2	12	TOP
K-I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S-P	2	12	SIDE(183.1)
P-N	2	12	SIDE(183.1)
N-K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
R-C	2	6	SIDE(33.0)
L-H	2	6	SIDE(33.0)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-t	MT20	5.0	12.0	2.00	10.00
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-t	MT20	5.0	12.0	2.00	10.00
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	7.0	8.0		
M	BMWW-t	MT20	5.0	8.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3551	0	3551	0	0	5-8	5-8
K	3547	0	3547	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2508	1661 / 0	0 / 0	0 / 0	0 0	847 0	0 0
K	2508	1660 / 0	0 / 0	0 / 0	0 0	846 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 38	-91.8	-91.8	0.07 (1)	10.00
B-C	-4118 / 0	-91.8	-91.8	0.50 (1)	4.28
C-T	-4825 / 0	-91.8	-91.8	0.51 (1)	3.86
T-U	-4825 / 0	-91.8	-91.8	0.51 (1)	3.86
U-D	-4825 / 0	-91.8	-91.8	0.51 (1)	3.86
D-V	-5270 / 0	-91.8	-91.8	0.53 (1)	3.69
V-W	-5270 / 0	-91.8	-91.8	0.53 (1)	3.69
W-E	-5270 / 0	-91.8	-91.8	0.53 (1)	3.69
E-F	-5270 / 0	-91.8	-91.8	0.53 (1)	3.69
F-X	-5270 / 0	-91.8	-91.8	0.53 (1)	3.69
X-G	-5270 / 0	-91.8	-91.8	0.53 (1)	3.69
G-Y	-4821 / 0	-91.8	-91.8	0.51 (1)	3.87
Y-Z	-4821 / 0	-91.8	-91.8	0.51 (1)	3.87
Z-H	-4821 / 0	-91.8	-91.8	0.51 (1)	3.87
H-I	-4114 / 0	-91.8	-91.8	0.50 (1)	4.28
I-J	0 / 38	-91.8	-91.8	0.07 (1)	10.00
S-B	-3471 / 0	0.0	0.0	0.13 (1)	7.50
K-I	-3468 / 0	0.0	0.0	0.13 (1)	7.51
S-AA	0 / 0	-18.5	-18.5	0.07 (4)	10.00
AA-AB	0 / 0	-18.5	-18.5	0.07 (4)	10.00
AB-R	0 / 0	-18.5	-18.5	0.07 (4)	10.00
R-AC	0 / 3274	-18.5	-18.5	0.24 (1)	10.00
AC-AD	0 / 3274	-18.5	-18.5	0.24 (1)	10.00
AD-Q	0 / 3274	-18.5	-18.5	0.24 (1)	10.00
Q-P	0 / 4825	-18.5	-18.5	0.36 (1)	10.00
P-AE	0 / 4825	-18.5	-18.5	0.36 (1)	10.00
AE-O	0 / 4825	-18.5	-18.5	0.36 (1)	10.00
O-AF	0 / 4822	-18.5	-18.5	0.36 (1)	10.00
AF-N	0 / 4822	-18.5	-18.5	0.36 (1)	10.00
N-M	0 / 4822	-18.5	-18.5	0.36 (1)	10.00
M-AG	0 / 3271	-18.5	-18.5	0.24 (1)	10.00
AG-AH	0 / 3271	-18.5	-18.5	0.24 (1)	10.00
AH-L	0 / 3271	-18.5	-18.5	0.24 (1)	10.00
L-AI	0 / 0	-18.5	-18.5	0.07 (4)	10.00
AI-AJ	0 / 0	-18.5	-18.5	0.07 (4)	10.00
AJ-K	0 / 0	-18.5	-18.5	0.07 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-C	-371 / 60	0.04 (1)
C-Q	0 / 2290	0.28 (1)
Q-D	-1506 / 0	0.39 (1)
D-O	0 / 666	0.08 (1)
O-E	-838 / 0	0.22 (1)
O-G	0 / 671	0.08 (1)
M-G	-1506 / 0	0.39 (1)
M-H	0 / 2290	0.28 (1)
L-H	-371 / 60	0.04 (1)
B-R	0 / 3343	0.41 (1)
L-I	0 / 3340	0.41 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-594	-594	---	FRONT	VERT	TOTAL	---	C1
E	15-11-4	-178	-178	---	FRONT	VERT	TOTAL	---	C1
F	17-11-4	-178	-178	---	FRONT	VERT	TOTAL	---	C1

LICENSED PROFESSIONAL ENGINEER
04/16/20
H.J G ALVES
100009024
PROVINCE OF ONTARIO

TOTAL WEIGHT = 2 X 168 = 335 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.53/1.00 (D-E:1) , BC=0.36/1.00 (O-Q:1) , WB=0.41/1.00 (B-R:1) , SSI=0.23/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (F) (INPUT = 0.90)
JSI METAL= 0.44 (N) (INPUT = 1.00)

Structural component only 1/2
DWG# T-2006291

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408129	T11	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:gtaCdVcjbNfa8ryKWAtL8NzmFU2-t5gtJjOw8gh811ycde0u?5v3S7UepxENt6tbtJzQR_5

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMWW-t	MT20	7.0	8.0		
S	BMV1+p	MT20	3.0	6.0		

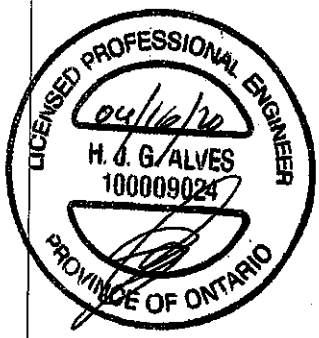
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	26-0-8	-594	-594	---	FRONT	VERT	TOTAL	--	C1
L	25-11-12	-36	-36	---	FRONT	VERT	TOTAL	--	C1
N	19-11-4	-36	-36	---	FRONT	VERT	TOTAL	--	C1
O	15-11-4	-36	-36	---	FRONT	VERT	TOTAL	--	C1
P	11-11-4	-36	-36	---	FRONT	VERT	TOTAL	--	C1
R	5-11-4	-36	-36	---	FRONT	VERT	TOTAL	--	C1
T	7-11-4	-178	-178	---	FRONT	VERT	TOTAL	--	C1
U	9-11-4	-178	-178	---	FRONT	VERT	TOTAL	--	C1
V	11-11-4	-178	-178	---	FRONT	VERT	TOTAL	--	C1
W	13-11-4	-178	-178	---	FRONT	VERT	TOTAL	--	C1
X	19-11-4	-178	-178	---	FRONT	VERT	TOTAL	--	C1
Y	21-11-4	-178	-178	---	FRONT	VERT	TOTAL	--	C1
Z	23-11-4	-178	-178	---	FRONT	VERT	TOTAL	--	C1
AA	2-0-12	-36	-36	---	FRONT	VERT	TOTAL	--	C1
AB	4-0-12	-36	-36	---	FRONT	VERT	TOTAL	--	C1
AC	7-11-4	-36	-36	---	FRONT	VERT	TOTAL	--	C1
AD	9-11-4	-36	-36	---	FRONT	VERT	TOTAL	--	C1
AE	13-11-4	-36	-36	---	FRONT	VERT	TOTAL	--	C1
AF	17-11-4	-36	-36	---	FRONT	VERT	TOTAL	--	C1
AG	21-11-4	-36	-36	---	FRONT	VERT	TOTAL	--	C1
AH	23-11-4	-36	-36	---	FRONT	VERT	TOTAL	--	C1
AI	27-10-4	-36	-36	---	FRONT	VERT	TOTAL	--	C1
AJ	29-10-4	-36	-36	---	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2006291

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408129	T11Z	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Wed Apr 15 16:26:33 2020 Page 1
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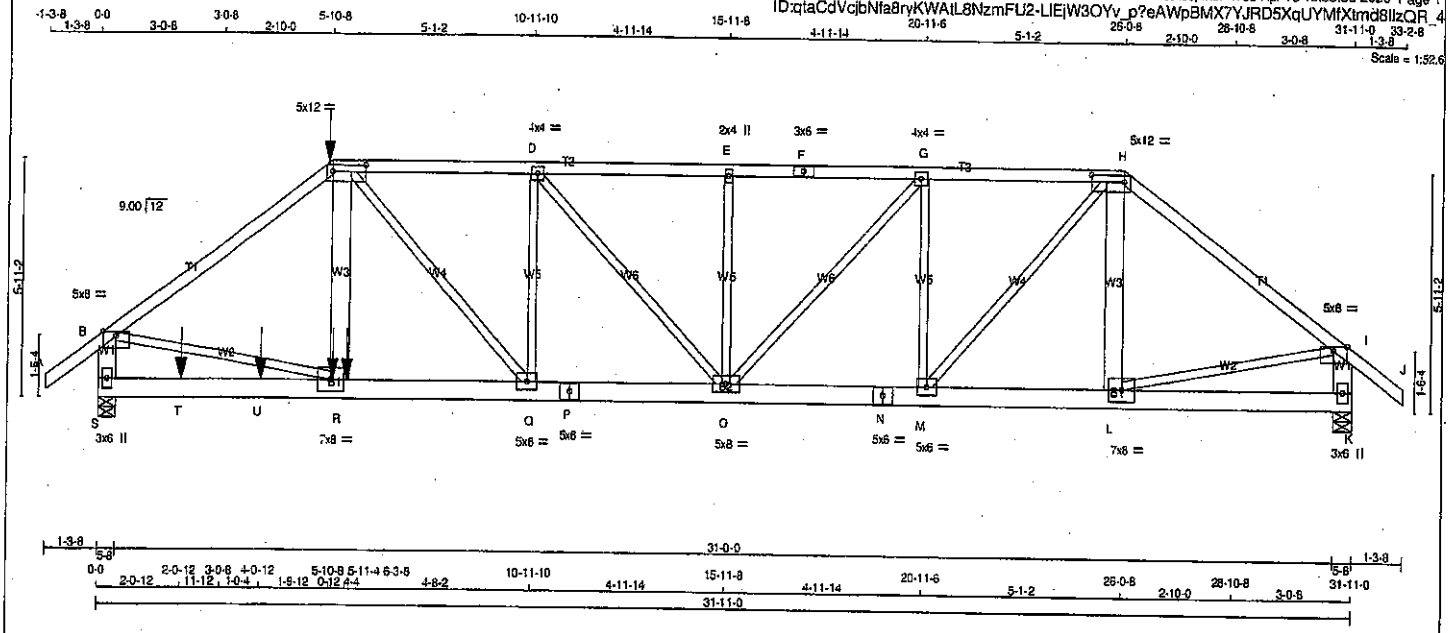

TOTAL WEIGHT = 2 X 168 = 335 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - C	2x6	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-F	1	12	SIDE(61.0)
F-H	1	12	TOP
H-J	1	12	TOP
S-B	2	12	TOP
K-I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S-P	2	12	SIDE(183.1)
P-N	2	12	TOP
N-K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
R-C	2	6	SIDE(461.8)
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	Edge	
C	TTWW-t	MT20	5.0	12.0	2.00	10.00
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-t	MT20	5.0	12.0	2.00	10.00
I	TMW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	7.0	8.0		
M	BMWW-t	MT20	5.0	8.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	4435	0	4435	0	0	5-8	5-8
K	2475	0	2475	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	3131	2083 / 0	0 / 0	0 / 0	0 0	1048 0	0 0
K	1747	1167 / 0	0 / 0	0 / 0	0 0	580 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B		0 / 38	-91.8	-91.8	0.07 (1)	10.00		R-C		0 / 1829	0.12 (1)
B-C		-5277 / 0	-91.8	-91.8	0.60 (1)	3.79		C-Q		0 / 381	0.05 (1)
C-D		-4549 / 0	-91.8	-91.8	0.29 (1)	4.25		Q-D		-167 / 8	0.04 (1)
D-E		-4239 / 0	-91.8	-91.8	0.28 (1)	4.39		D-O		-463 / 0	0.25 (1)
E-F		-4239 / 0	-91.8	-91.8	0.28 (1)	4.39		O-E		-420 / 0	0.11 (1)
F-G		-4239 / 0	-91.8	-91.8	0.28 (1)	4.39		O-G		0 / 1122	0.14 (1)
G-H		-3489 / 0	-91.8	-91.8	0.25 (1)	4.79		M-G		-1350 / 0	0.35 (1)
H-I		-2719 / 0	-91.8	-91.8	0.40 (1)	5.12		M-H		0 / 1963	0.24 (1)
I-J		0 / 38	-91.8	-91.8	0.07 (1)	10.00		L-H		-278 / 0	0.03 (1)
S-B		-4337 / 0	0.0	0.0	0.16 (1)	6.88		B-R		0 / 4285	0.53 (1)
K-I		-2426 / 0	0.0	0.0	0.09 (1)	7.81		L-I		0 / 2207	0.27 (1)
S-T		0 / 0	-18.5	-16.5	0.08 (4)	10.00					
T-U		0 / 0	-18.5	-18.5	0.08 (4)	10.00					
U-R		0 / 0	-18.5	-18.5	0.08 (4)	10.00					
R-Q		0 / 4291	-18.5	-18.5	0.30 (1)	10.00					
Q-P		0 / 4549	-18.5	-18.5	0.32 (1)	10.00					
P-O		0 / 4549	-18.5	-18.5	0.32 (1)	10.00					
O-N		0 / 3489	-18.5	-18.5	0.26 (1)	10.00					
N-M		0 / 3489	-18.5	-18.5	0.26 (1)	10.00					
M-L		0 / 2160	-18.5	-18.5	0.16 (1)	10.00					
L-K		0 / 0	-18.5	-18.5	0.04 (4)	10.00					

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	LOAD
C	5-10-8	-594	-594	---	BACK	VERT	TOTAL	C1
R	5-11-4	-36	-36	---	BACK	VERT	TOTAL	C1
R	6-3-8	-2436	-2436	---	BACK	VERT	TOTAL	C1
T	2-0-12	-36	-36	---	BACK	VERT	TOTAL	C1
U	4-0-12	-36	-36	---	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.60/1.00 (B-C:1) , BC=0.32/1.00 (O-Q:1) , WB=0.53/1.00 (B-R:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.55 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only 1/2
DWG# T-2006292

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408129	T11Z	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:qtaCdVcjbNfa8ryKWAtL8NzmFU2-LIEiW3OYv_p?eAWpBMX7YJRD5XqUYMfXtmd8IlzQR_4

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMWW-t	MT20	7.0	8.0		
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
Apr/16/20
H. J. G. ALVES
100009624
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006292

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408129	T12	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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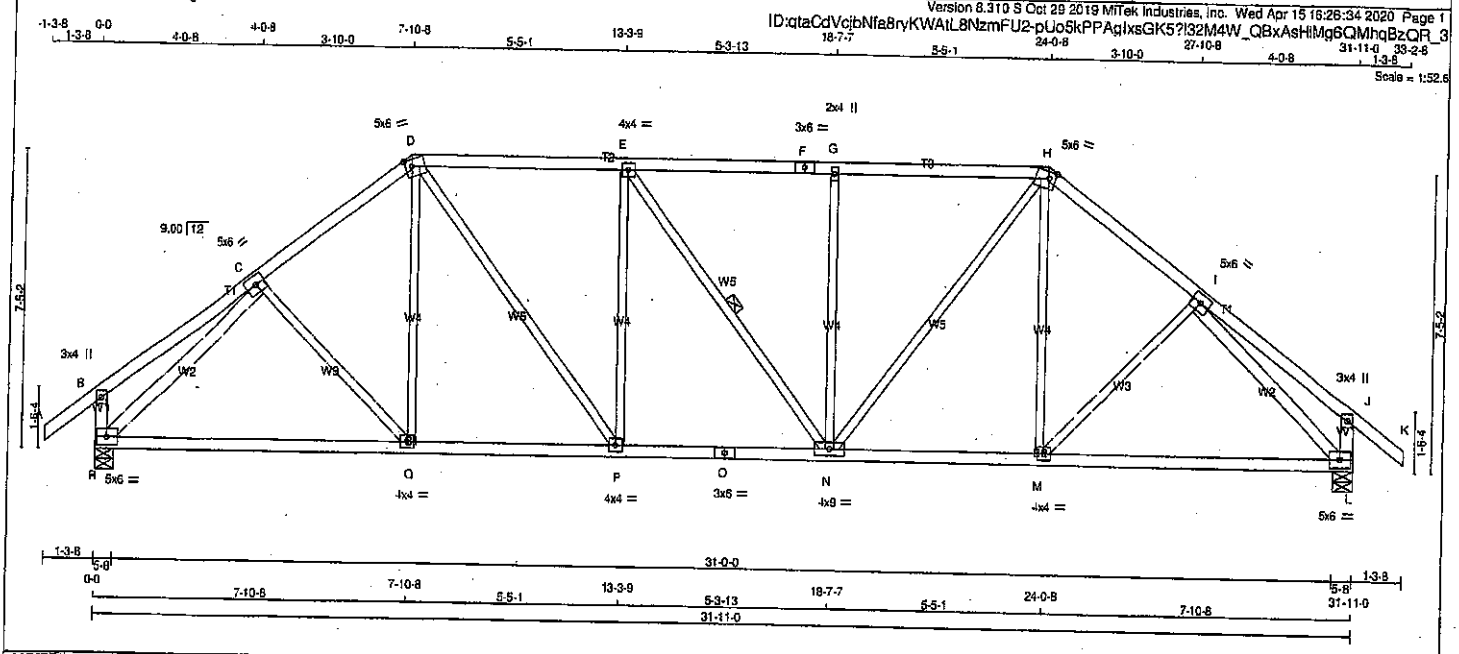


TOTAL WEIGHT = 2 X 148 = 296 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.00	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.00	2.00
I	TMWW-t	MT20	5.0	6.0		
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0		
M, P, Q						
M	BMWW-t	MT20	4.0	4.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
R	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1886	0	1886	0	0	5-8	5-8
L	1886	0	1886	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1331	887 / 0	0 / 0	0 / 0	0 0	444 0	0 0
L	1331	887 / 0	0 / 0	0 / 0	0 0	444 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

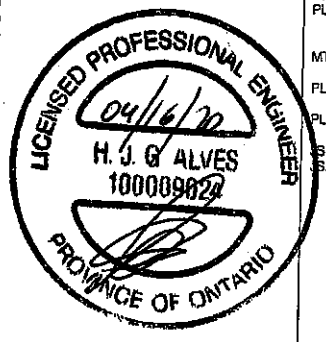
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 38	-91.8	-91.8	0.12 (1)	10.00
B-C	0 / 24	-91.8	-91.8	0.22 (1)	10.00
C-D	-1878 / 0	-91.8	-91.8	0.29 (1)	4.62
D-E	-1953 / 0	-91.8	-91.8	0.45 (1)	4.34
E-F	-1951 / 0	-91.8	-91.8	0.45 (1)	4.34
F-G	-1951 / 0	-91.8	-91.8	0.45 (1)	4.34
G-H	-1951 / 0	-91.8	-91.8	0.45 (1)	4.35
H-I	-1878 / 0	-91.8	-91.8	0.29 (1)	4.62
I-J	0 / 24	-91.8	-91.8	0.22 (1)	10.00
J-K	0 / 38	-91.8	-91.8	0.12 (1)	10.00
R-B	-266 0	0.0	0.0	0.03 (1)	7.81
L-J	-265 0	0.0	0.0	0.03 (1)	7.81
R-Q	0 / 1479	-18.5	-18.5	0.38 (1)	10.00
Q-P	0 / 1482	-18.5	-18.5	0.38 (1)	10.00
P-O	0 / 1953	-18.5	-18.5	0.37 (1)	10.00
O-N	0 / 1953	-18.5	-18.5	0.37 (1)	10.00
N-M	0 / 1482	-18.5	-18.5	0.38 (1)	10.00
M-L	0 / 1479	-18.5	-18.5	0.38 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-Q	0 52	0.02 (4)
Q-D	0 / 149	0.05 (4)
D-P	0 / 777	0.17 (1)
P-E	-533 / 0	0.53 (1)
E-N	-3 0	0.00 (1)
N-G	-532 / 0	0.53 (1)
N-H	0 / 774	0.17 (1)
M-H	0 / 150	0.05 (4)
M-I	0 52	0.02 (4)
R-C	-2149 / 0	0.82 (1)
I-L	-2149 0	0.82 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.45/1.00 (D-E:1) , BC=0.38/1.00 (M-N:1) , WB=0.82/1.00 (I-L:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	798	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (I) (INPUT = 0.90)
SI METAL= 0.86 (O) (INPUT = 1.00)

Structural component only
DWG# T-2006293

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408129	T13	9	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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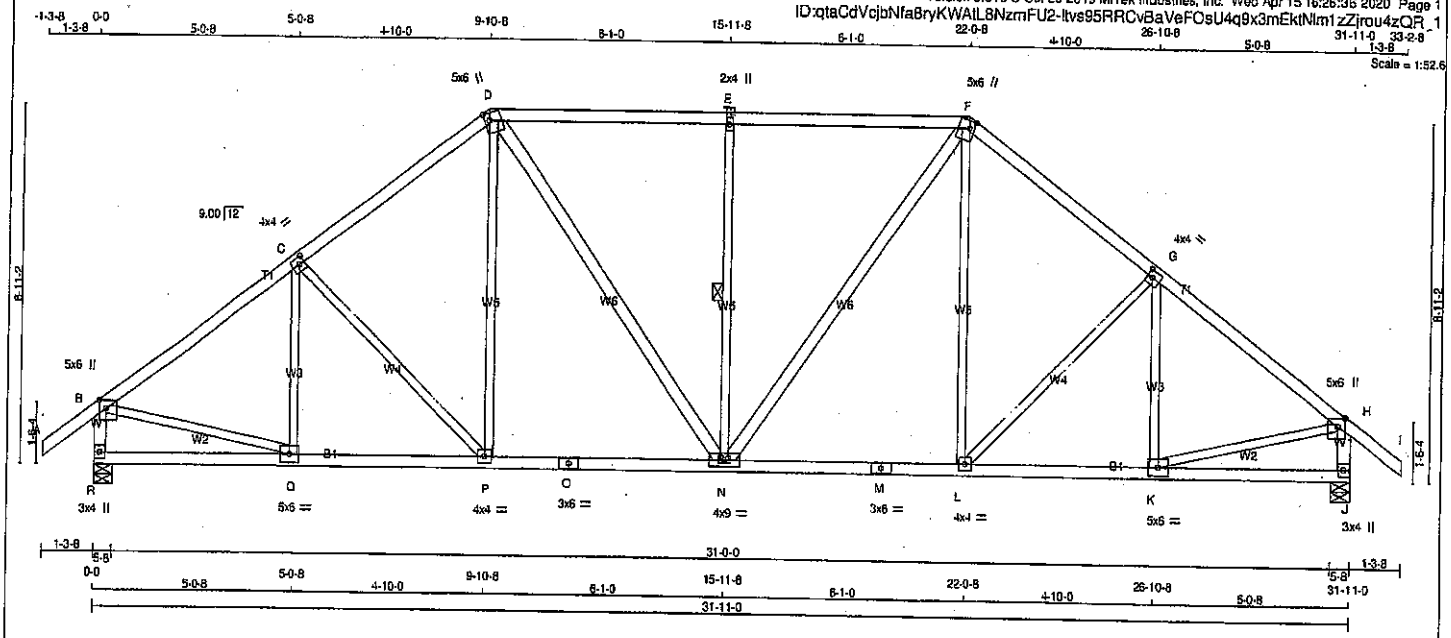

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4:0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW+p	MT20	5.0	6.0	2.50	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT.	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1886	0	1886	0	0	5-8	5-8
J	1886	0	1886	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1331	887 / 0	0 / 0	0 / 0	0 : 0	444 0	0 0
J	1331	887 / 0	0 / 0	0 / 0	0 : 0	444 0	0 : 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 38	-91.8	-91.8	0.12 (1)	10.00
B-C	-1920 / 0	-91.8	-91.8	0.35 (1)	4.53
C-D	-1780 / 0	-91.8	-91.8	0.33 (1)	4.68
D-E	-1680 / 0	-91.8	-91.8	0.48 (1)	4.57
E-F	-1680 / 0	-91.8	-91.8	0.48 (1)	4.57
F-G	-1780 / 0	-91.8	-91.8	0.33 (1)	4.68
G-H	-1920 / 0	-91.8	-91.8	0.35 (1)	4.53
H-I	0 / 38	-91.8	-91.8	0.12 (1)	10.00
R-B	-1845 : 0	0.0	0.0	0.19 (1)	6.16
J-H	-1845 : 0	0.0	0.0	0.19 (1)	6.16
R-Q	0 0	-18.5	-18.5	0.10 (4)	10.00
Q-P	0 1561	-18.5	-18.5	0.31 (1)	10.00
P-O	0 / 1398	-18.5	-18.5	0.30 (1)	10.00
O-N	0 / 1398	-18.5	-18.5	0.30 (1)	10.00
N-M	0 / 1398	-18.5	-18.5	0.30 (1)	10.00
M-L	0 / 1398	-18.5	-18.5	0.30 (1)	10.00
L-K	0 / 1561	-18.5	-18.5	0.31 (1)	10.00
K-J	0 : 0	-18.5	-18.5	0.10 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-C	-274 : 0	0.11 (1)
C-P	-236 / 0	0.22 (1)
P-D	0 / 270	0.06 (1)
D-N	0 : 488	0.08 (1)
N-E	-684 : 0	0.35 (1)
N-F	0 488	0.08 (1)
L-F	0 : 270	0.05 (1)
L-G	-236 / 0	0.22 (1)
K-G	-274 0	0.11 (1)
B-Q	0 1604	0.36 (1)
K-H	0 1604	0.36 (1)

TOTAL WEIGHT = 9 X 155 = 1395 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.48/1.00 (D-E:1) , BC=0.31/1.00 (P-Q:1) , WB=0.38/1.00 (B-Q:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (B) (INPUT = 0.90)
SI METAL= 0.48 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006294

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408129	T14	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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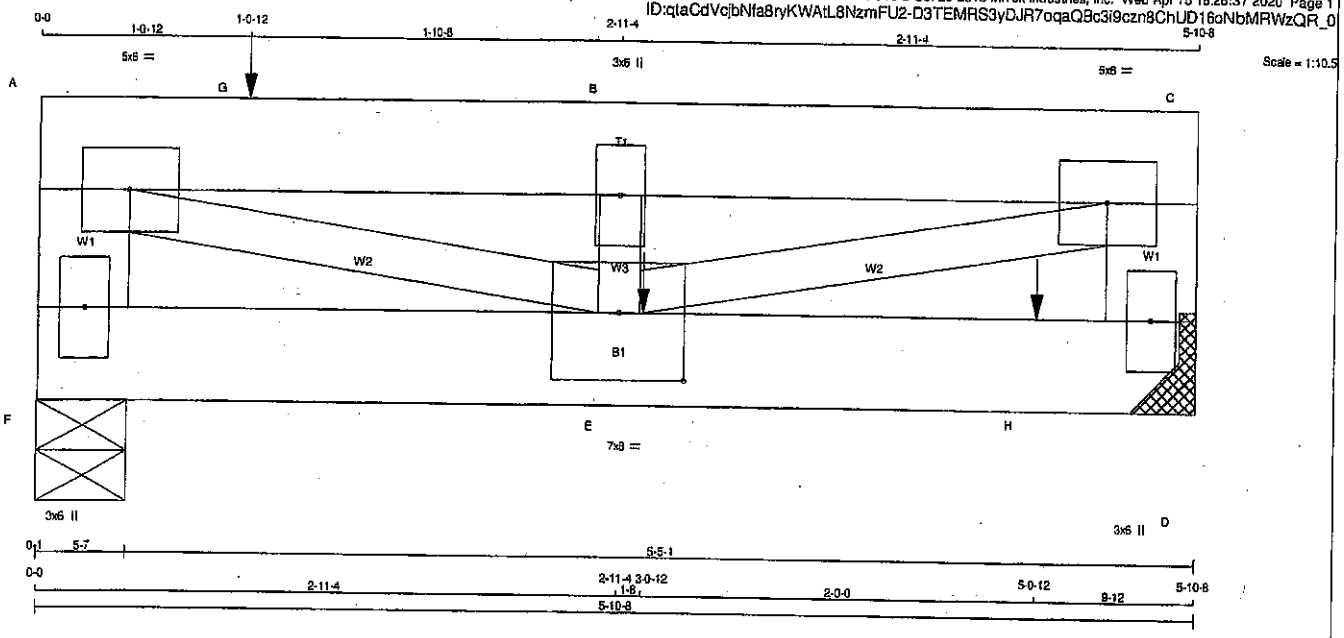


TOTAL WEIGHT = 2 X 29 = 55 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	2	12	TOP
C- D	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	7	SIDE(366.2)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		
B	TMW+w	MT20	3.0	6.0		
C	TMVW-t	MT20	5.0	6.0		
D	BMV1+p	MT20	3.0	6.0		
E	BMWWW-t	MT20	7.0	8.0	4.00	4.00
F	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	2297	0	2297	0	0	5-8 (5-7)	5-8
D	2455	0	2455	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1625	1064 / 0	0 / 0	0 / 0	0 / 0	560 / 0	0 / 0
D	1734	1149 / 0	0 / 0	0 / 0	0 / 0	585 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-2165 / 0	0.0	0.0	0.07 (1)		7.81	A- E	0 / 3354	0.38 (1)
A- G	-3161 / 0	-91.8	-91.8	0.37 (1)		5.73	E- B	-627 / 0	0.04 (1)
G- B	-3161 / 0	-91.8	-91.8	0.37 (1)		5.73	E- C	0 / 3354	0.38 (1)
B- C	-3161 / 0	-91.8	-91.8	0.08 (1)		8.15			
D- C	-1317 / 0	0.0	0.0	0.04 (1)		7.81			
F- E	0 / 0	-18.5	-18.5	0.10 (1)		10.00			
E- H	0 0	-18.5	-18.5	0.31 (1)		10.00			
H- D	0 0	-18.5	-18.5	0.31 (1)		10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-0-12	-1387	-1387	---	BACK	VERT	TOTAL	---	C1
G	1-0-12	-1328	-1328	---	TOP	VERT	TOTAL	---	C1
H	5-0-12	-1389	-1389	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD.	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.37/1.00 (A-B:1) , BC=0.31/1.00 (D-E:1) , WB=0.38/1.00 (A-E:1) , SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

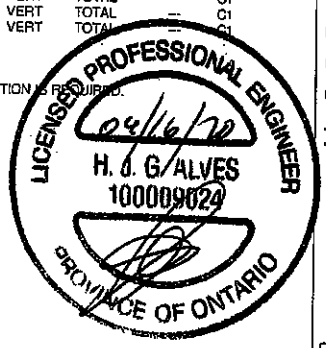
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	789	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (C) (INPUT = 0.90)
JSI METAL= 0.40 (E) (INPUT = 1.00)



Structural component only
DWG# T-2006295

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408129	T15	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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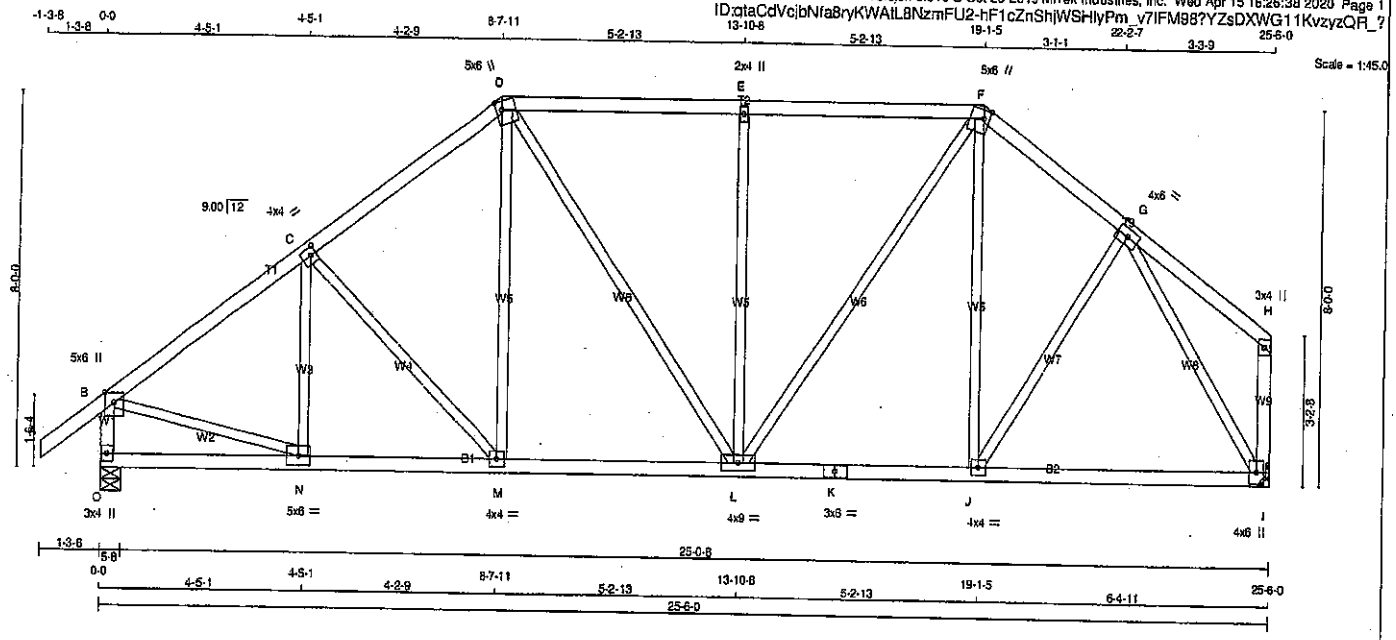


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	BMVW1+p	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	9.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1532	0	1532	0	0	IN-SX 5-8	IN-SX 5-8
I	1408	0	1408	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1081	723 / 0	0 / 0	0 / 0	0 0	358 0	0 / 0
I	994	652 / 0	0 / 0	0 / 0	0 0	342 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

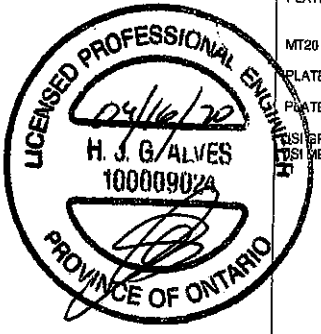
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 38	-91.8	-91.8	0.12 (1)	10.00
B-C	-1459 / 0	-91.8	-91.8	0.24 (1)	5.18
C-D	-1339 / 0	-91.8	-91.8	0.24 (1)	5.35
D-E	-1196 / 0	-91.8	-91.8	0.34 (1)	5.44
E-F	-1196 / 0	-91.8	-91.8	0.34 (1)	5.44
F-G	-1131 / 0	-91.8	-91.8	0.12 (1)	5.85
G-H	0 / 19	-91.8	-91.8	0.14 (1)	10.00
O-B	-1496 / 0	0.0	0.0	0.16 (1)	6.71
I-H	-116 / 0	0.0	0.0	0.02 (1)	7.81
O-N	0 / 0	-18.5	-18.5	0.07 (4)	10.00
N-M	0 / 1189	-18.5	-18.5	0.24 (1)	10.00
M-L	0 / 1050	-18.5	-18.5	0.22 (1)	10.00
L-K	0 / 885	-18.5	-18.5	0.25 (4)	10.00
K-J	0 / 885	-18.5	-18.5	0.25 (4)	10.00
J-I	0 / 755	-18.5	-18.5	0.23 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	-238 / 0	0.08 (1)
C-M	-206 / 0	0.14 (1)
M-D	0 / 242	0.05 (1)
D-L	0 / 260	0.06 (1)
L-E	-587 / 0	0.72 (1)
L-F	0 / 552	0.12 (1)
J-F	-66 / 37	0.10 (1)
J-G	0 / 251	0.06 (1)
B-N	0 / 1232	0.28 (1)
G-I	-1452 0	0.94 (1)



TOTAL WEIGHT = 2 X 123 = 246 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.34/1.00 (D-E:1) , BC=0.25/1.00 (J-L:4) , WB=0.94/1.00 (G-I:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.86 (B) (INPUT = 0.90)
SSI METAL= 0.59 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006296

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408129	T16S	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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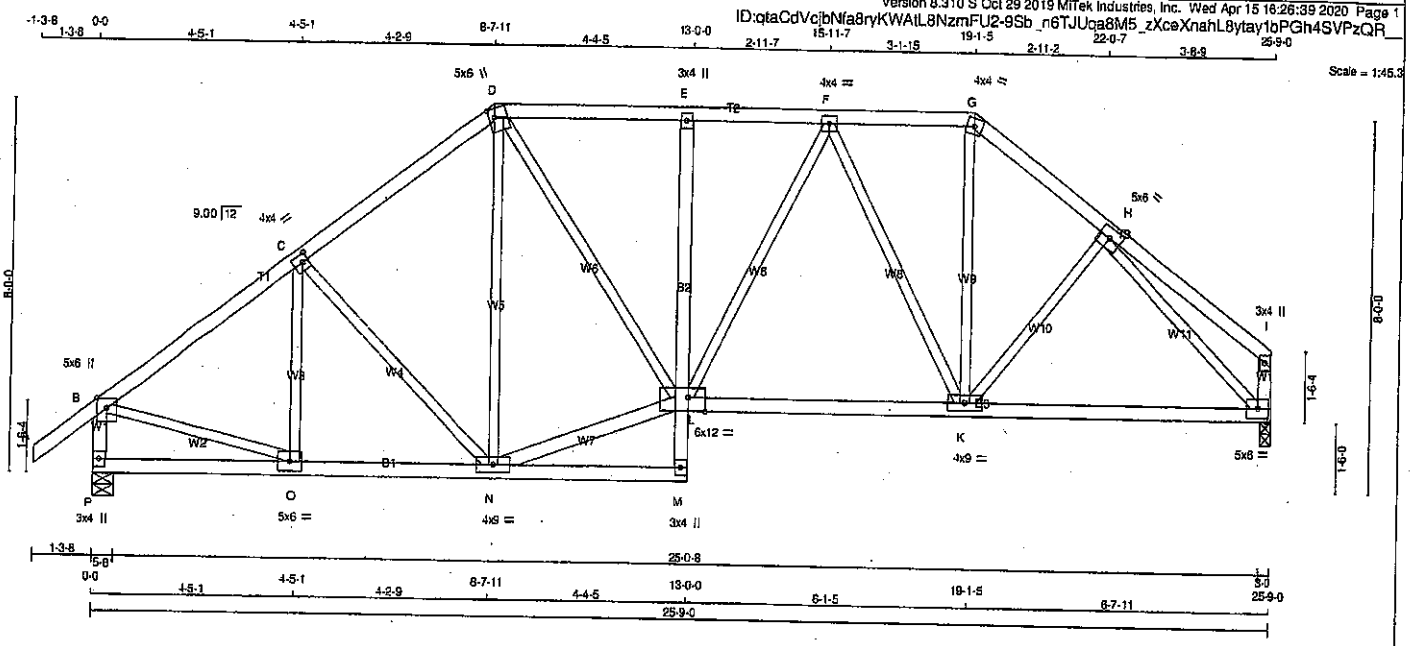


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMV+p	MT20	3.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	4.0		
H	TMWW-t	MT20	5.0	6.0		
I	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BVMWWW-t	MT20	6.0	12.0	Edge	4.50
M	BMV+p	MT20	3.0	4.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 128 lb [M][F]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1546	0	1546	0	0	5-8	5-8
J	1420	0	1420	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1091	729 / 0	0 / 0	0 / 0	0 / 0	362 0	0 0
J	1004	659 / 0	0 / 0	0 / 0	0 / 0	345 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

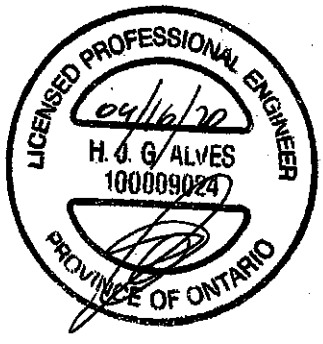
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 38	-91.8	-91.8	0.12 (1)	10.00
B-C	-1478 / 0	-91.8	-91.8	0.25 (1)	5.15
C-D	-1359 / 0	-91.8	-91.8	0.24 (1)	5.32
D-E	-1490 / 0	-91.8	-91.8	0.21 (1)	5.17
E-F	-1493 / 0	-91.8	-91.8	0.16 (1)	5.22
F-G	-1153 / 0	-91.8	-91.8	0.12 (1)	5.81
G-H	-1483 / 0	-91.8	-91.8	0.13 (1)	5.30
H-I	0 / 17	-91.8	-91.8	0.14 (1)	10.00
P-B	-1510 / 0	0.0	0.0	0.16 (1)	6.68
J-I	-135 / 0	0.0	0.0	0.01 (1)	7.81
P-O	0 / 0	-18.5	-18.5	0.07 (4)	10.00
O-N	0 / 1203	-18.5	-18.5	0.24 (1)	10.00
N-M	0 13	-18.5	-18.5	0.08 (4)	10.00
M-L	0 34	0.0	0.0	0.03 (1)	10.00
L-E	-381 0	0.0	0.0	0.16 (1)	7.81
L-K	0 1392	-18.5	-18.5	0.34 (1)	10.00
K-J	0 1141	-18.5	-18.5	0.31 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O-C	-242 0	0.08 (1)
C-N	-204 0	0.14 (1)
N-D	-144 / 11	0.18 (1)
N-L	0 1114	0.18 (1)
D-L	0 764	0.17 (1)
K-G	0 628	0.14 (1)
B-O	0 1246	0.28 (1)
K-H	0 / 45	0.02 (4)
H-J	-1681 0	0.76 (1)
L-F	0 227	0.05 (1)
F-K	-540 0	0.49 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.25/1.00 (B-C:1) , BC=0.34/1.00 (K-L:1) , WB=0.76/1.00 (H-J:t) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.59 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006297

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408129	T16SG	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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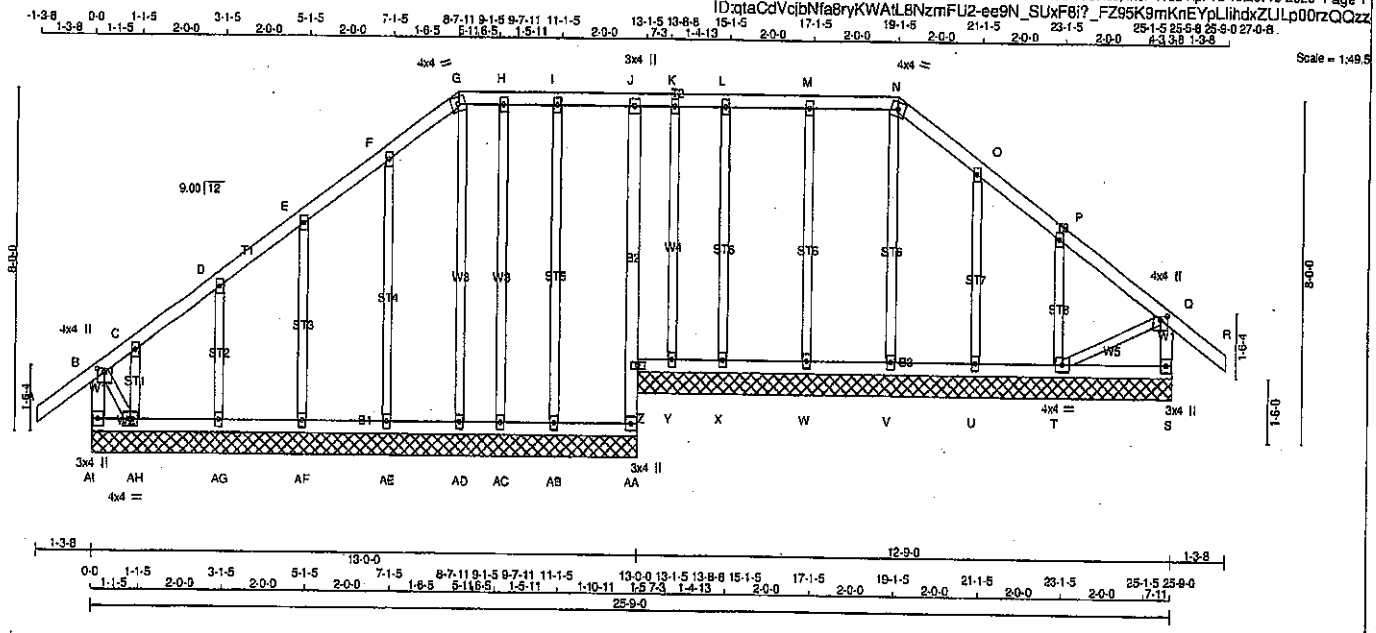

TOTAL WEIGHT = 138 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - N	2x4	DRY	No.2	SPF
N - R	2x4	DRY	No.2	SPF
AI - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
AI - AA	2x4	DRY	No.2	SPF
AA- J	2x4	DRY	No.2	SPF
Z - S	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, F, H, I, K, L, M, O, P						
C	TMW+w	MT20	2.0	4.0		
G	TTW-m	MT20	4.0	4.0		
J	TMV+p	MT20	3.0	4.0		
N	TTW-m	MT20	4.0	4.0		
Q	TMVW+p	MT20	4.0	4.0	1.00	2.00
S, AA, AI						
S	BMV1+p	MT20	3.0	4.0		
T	BMWW1-t	MT20	4.0	4.0		
U, V, W, X, Y, AB, AC, AD, AE, AF, AG						
U	BMW1+w	MT20	2.0	4.0		
Z	BVM1-I	MT20	2.0	4.0		
AH	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 38	-91.8	-91.8	0.12 (1)	10.00
B-C	-67 / 0	-91.8	-91.8	0.12 (1)	6.25
C-D	-9 / 0	-91.8	-91.8	0.05 (1)	10.00
D-E	-11 / 0	-91.8	-91.8	0.05 (1)	6.25
E-F	-6 / 0	-91.8	-91.8	0.04 (1)	10.00
F-G	-13 / 0	-91.8	-91.8	0.04 (1)	6.25
G-H	0 / 0	-91.8	-91.8	0.02 (1)	10.00
H-I	0 / 0	-91.8	-91.8	0.02 (1)	10.00
I-J	0 / 0	-91.8	-91.8	0.03 (1)	10.00
J-K	0 / 0	-91.8	-91.8	0.03 (1)	10.00
K-L	0 / 0	-91.8	-91.8	0.03 (1)	10.00
L-M	0 / 0	-91.8	-91.8	0.06 (1)	10.00
M-N	0 / 0	-91.8	-91.8	0.06 (1)	10.00
N-O	-13 / 0	-91.8	-91.8	0.04 (1)	6.25
O-P	-16 / 0	-91.8	-91.8	0.08 (1)	6.25
P-Q	-3 / 0	-91.8	-91.8	0.08 (1)	10.00
Q-R	0 / 38	-91.8	-91.8	0.12 (1)	10.00
AI-B	-278 / 0	0.0	0.0	0.03 (1)	7.81
S-Q	-242 / 0	0.0	0.0	0.03 (1)	7.81
AI-AH	0 / 0	-18.5	-18.5	0.01 (4)	10.00
AH-AG	0 / 13	-18.5	-18.5	0.02 (4)	10.00
AG-AF	0 / 8	-18.5	-18.5	0.02 (4)	10.00
AF-AE	0 / 4	-18.5	-18.5	0.01 (4)	10.00
AE-AD	0 / 2	-18.5	-18.5	0.01 (4)	10.00
AD-AC	0 / 0	-18.5	-18.5	0.01 (4)	10.00
AC-AB	0 / 0	-18.5	-18.5	0.01 (4)	10.00
AB-AA	0 / 0	-18.5	-18.5	0.01 (4)	10.00
AA-Z	0 / 0	0.0	0.0	0.00 (1)	10.00
Z-J	-145 / 0	0.0	0.0	0.02 (1)	7.81
Z-Y	0 / 0	-18.5	-18.5	0.00 (4)	10.00
Y-X	0 / 0	-18.5	-18.5	0.01 (4)	10.00
X-W	0 / 0	-18.5	-18.5	0.02 (4)	10.00
W-V	0 / 0	-18.5	-18.5	0.02 (4)	10.00
V-U	0 / 3	-18.5	-18.5	0.02 (4)	10.00
U-T	0 / 6	-18.5	-18.5	0.03 (4)	10.00
T-S	0 / 0	-18.5	-18.5	0.03 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V-N	-150 / 0	0.10 (1)
W-M	-218 / 0	0.15 (1)
X-L	-143 / 0	0.10 (1)
AB-I	-144 / 0	0.18 (1)
AE-F	-185 / 0	0.14 (1)
AF-E	-179 / 0	0.07 (1)
AG-D	-193 / 0	0.04 (1)
AH-C	-41 / 0	0.01 (1)
U-O	-182 / 0	0.07 (1)
T-P	-244 / 0	0.05 (1)
AD-G	-100 / 0	0.12 (1)
AC-H	-116 / 0	0.14 (1)
Y-K	-78 / 0	0.05 (1)
B-AH	0 / 22	0.01 (1)
T-Q	0 - 14	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-09, CSA O86-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/694 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.03/1.00 (T-U:4) , WB=0.18/1.00 (I-AB:1) , SSI=0.09/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (G) (INPUT = 0.90)
JSI METAL= 0.13 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006298

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408129	T17	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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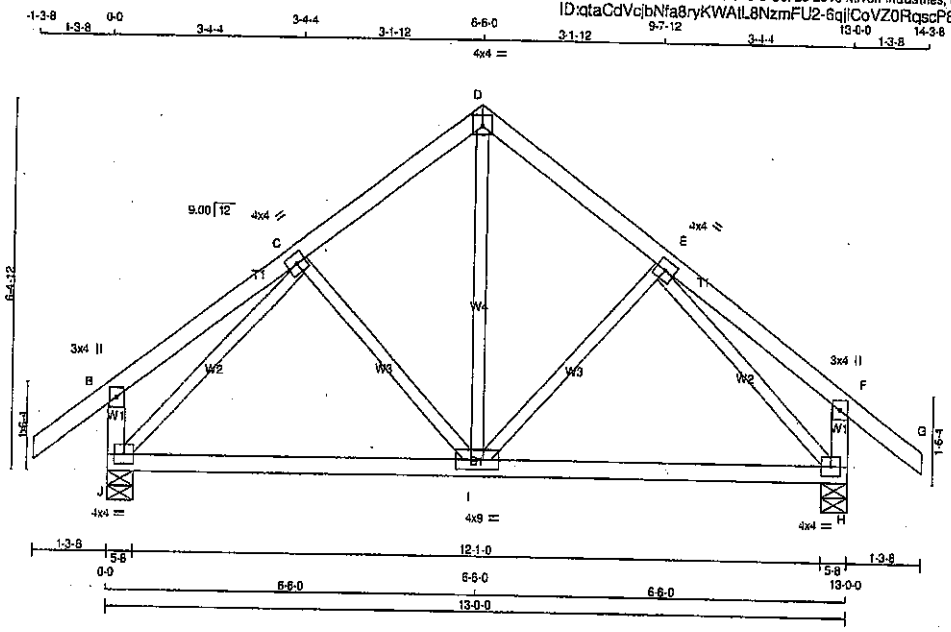


TOTAL WEIGHT = 3 X 59 = 178 lb

MI[F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	843	0	843	0	0	5-8	5-8
H	843	0	843	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	594	403 / 0	0 / 0	0 / 0	0 0	191 0	0 0
H	594	403 / 0	0 / 0	0 / 0	0 0	191 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

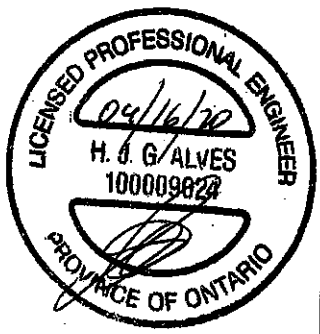
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 38	-91.8	-91.8	0.12 (1)	10.00	
B-C	0 / 20	-91.8	-91.8	0.15 (1)	10.00	
C-D	-518 / 0	-91.8	-91.8	0.12 (1)	6.25	
D-E	-518 / 0	-91.8	-91.8	0.12 (1)	6.25	
E-F	0 / 20	-91.8	-91.8	0.15 (1)	10.00	
F-G	0 / 38	-91.8	-91.8	0.12 (1)	10.00	
J-B	-241 / 0	0.0	0.0	0.03 (1)	7.81	
H-F	-241 / 0	0.0	0.0	0.03 (1)	7.81	
J-I	0 / 497	-18.5	-18.5	0.26 (4)	10.00	
I-H	0 497	-18.5	-18.5	0.26 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-D	0 / 365	0.08 (1)
I-E	-146 / 0	0.06 (1)
C-I	-146 / 0	0.06 (1)
J-C	-746 / 0	0.28 (3)
E-H	-746 / 0	0.28 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.26/1.00 (I-J:4) , WB=0.28/1.00 (C-J:1) , SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (C) (INPUT = 0.90)
JSI METAL= 0.27 (C) (INPUT = 1.00)

Structural component only
DWG# T-2006299

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408129	T17S	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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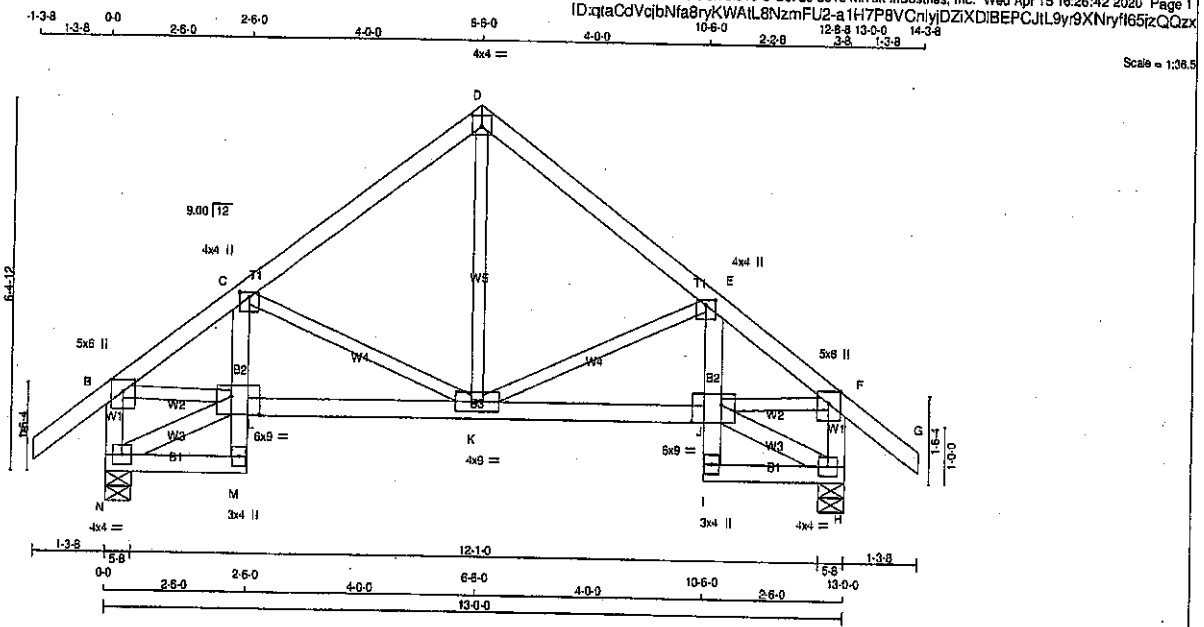


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-l	MT20	6.0	9.0	3.75	6.00
K	BMWWW-t	MT20	4.0	9.0		
L	BVMWW-l	MT20	6.0	9.0	3.75	6.00
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	843	0	843	0	0	IN-SX	IN-SX
H	843	0	843	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	594	403 / 0	0 / 0	0 / 0	0 0	191 0	0 0
H	594	403 / 0	0 / 0	0 / 0	0 0	191 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

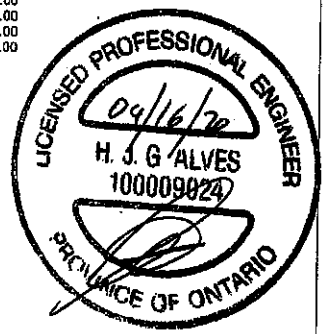
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-91.8	-91.8	0.12 (1)		10.00
B- C	-956 / 0	-91.8	-91.8	0.15 (1)		6.19
C- D	-602 / 0	-91.8	-91.8	0.19 (1)		6.25
D- E	-602 / 0	-91.8	-91.8	0.19 (1)		6.25
E- F	-956 / 0	-91.8	-91.8	0.15 (1)		6.19
F- G	0 / 38	-91.8	-91.8	0.12 (1)		10.00
N- B	-811 / 0	0.0	0.0	0.08 (1)		7.81
H- F	-811 / 0	0.0	0.0	0.08 (1)		7.81
N- M	0 / 24	-18.5	-18.5	0.03 (4)		10.00
M- L	0 / 24	0.0	0.0	0.04 (1)		10.00
L- C	0 / 46	0.0	0.0	0.04 (1)		10.00
L- K	0 / 805	-18.5	-18.5	0.18 (1)		10.00
K- J	0 / 805	-18.5	-18.5	0.18 (1)		10.00
I- J	0 / 24	0.0	0.0	0.04 (1)		10.00
J- E	0 / 46	0.0	0.0	0.04 (1)		10.00
I- H	0 / 24	-18.5	-18.5	0.03 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- D	0 / 383	0.09 (1)
K- E	-374 / 0	0.13 (1)
C- K	-374 / 0	0.13 (1)
N- L	-26 / 0	0.00 (1)
B- L	0 / 793	0.18 (1)
J- H	-26 / 0	0.00 (1)
J- F	0 / 793	0.18 (1)



TOTAL WEIGHT = 2 X 65 = 130 lb (M)[F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.19/1.00 (D-E:1) , BC=0.18/1.00 (K-L:1) , WB=0.18/1.00 (B-L:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	786	1987	1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (D) (INPUT = 0.90)
JSI METAL= 0.39 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006300

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	T21	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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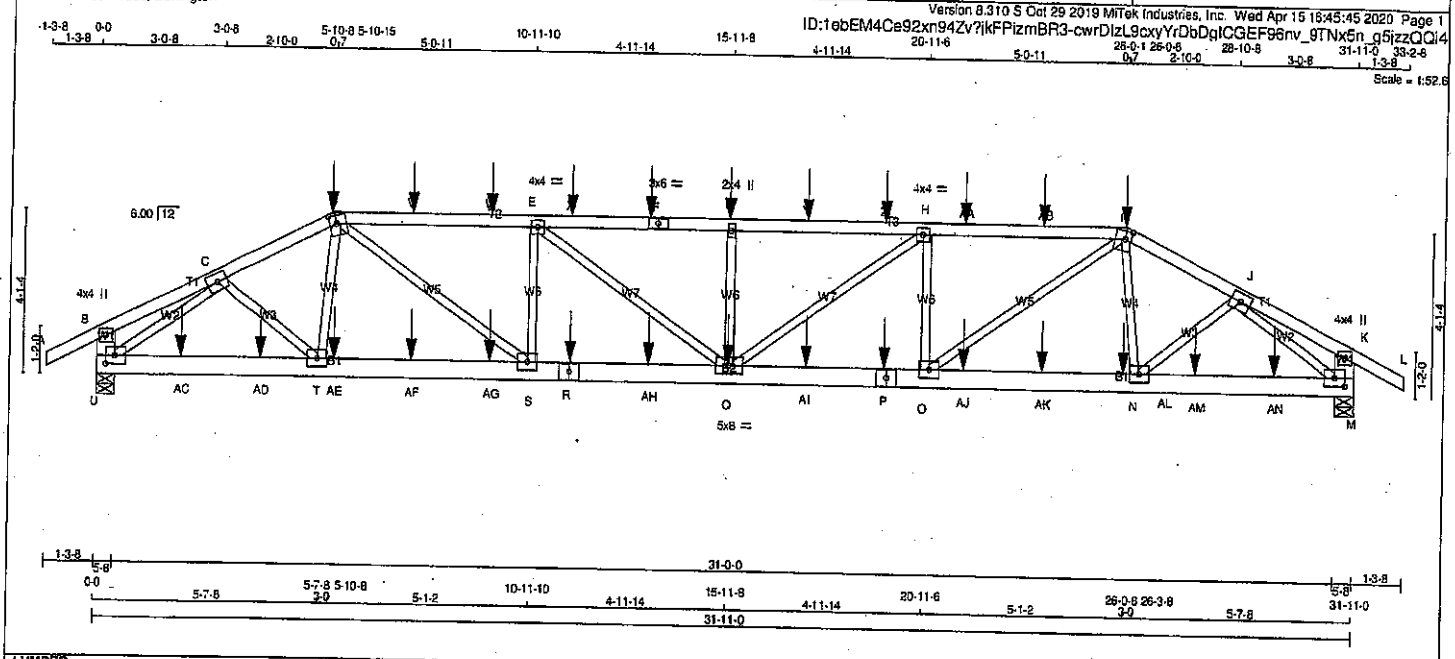


TOTAL WEIGHT = 2 X 144 = 288 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	SIDE(61.0)
D- F	1	12	SIDE(61.0)
F- I	1	12	SIDE(61.0)
I- L	1	12	SIDE(61.0)
U- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- R	2	12	SIDE(183.1)
R- P	2	12	SIDE(183.1)
P- M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.50	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	5.0	6.0	2.50	2.00
J	TMWW-t	MT20	5.0	6.0		
K	TMV+p	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	2.50	2.75
N, O, S, T						
N	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
U	BMVW1-t	MT20	5.0	6.0	2.50	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2995	0	2995	0	0	5-8	5-8
M	2995	0	2995	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2119	1385 / 0	0 / 0	0 / 0	0 0	734 0	0 0
M	2119	1385 / 0	0 / 0	0 / 0	0 0	734 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.07 (1)	10.00
B- C	0 / 8	-91.8	-91.8	0.06 (1)	10.00
C- D	-4453 / 0	-91.8	-91.8	0.13 (1)	4.44
D- V	-5899 / 0	-91.8	-91.8	0.48 (1)	3.61
V- W	-5899 / 0	-91.8	-91.8	0.48 (1)	3.61
W- E	-5899 / 0	-91.8	-91.8	0.48 (1)	3.61
E- X	-6444 / 0	-91.8	-91.8	0.51 (1)	3.43
X- F	-6444 / 0	-91.8	-91.8	0.51 (1)	3.43
F- G	-6444 / 0	-91.8	-91.8	0.51 (1)	3.43
G- Y	-6444 / 0	-91.8	-91.8	0.51 (1)	3.43
Y- Z	-6444 / 0	-91.8	-91.8	0.51 (1)	3.43
Z- H	-6444 / 0	-91.8	-91.8	0.51 (1)	3.43
H-AA	-5903 / 0	-91.8	-91.8	0.48 (1)	3.60
AA-AB	-5903 / 0	-91.8	-91.8	0.48 (1)	3.60
AB- I	-5903 / 0	-91.8	-91.8	0.48 (1)	3.60
I- J	-4453 / 0	-91.8	-91.8	0.13 (1)	4.44
J- K	0 / 8	-91.8	-91.8	0.06 (1)	10.00
K- L	0 / 28	-91.8	-91.8	0.07 (1)	10.00
U- B	-247 / 0	0.0	0.0	0.01 (1)	7.81
M- K	-247 / 0	0.0	0.0	0.01 (1)	7.81
U-AC	0 / 3514	-18.5	-18.5	0.27 (1)	10.00
AC-AD	0 / 3514	-18.5	-18.5	0.27 (1)	10.00
AD- T	0 / 3514	-18.5	-18.5	0.27 (1)	10.00
T-AE	0 / 3995	-18.5	-18.5	0.30 (1)	10.00
AE-AF	0 / 3995	-18.5	-18.5	0.30 (1)	10.00
AF-AG	0 / 3995	-18.5	-18.5	0.30 (1)	10.00
AG- S	0 / 3995	-18.5	-18.5	0.30 (1)	10.00
S- R	0 / 5899	-18.5	-18.5	0.44 (1)	10.00
R-AH	0 / 5899	-18.5	-18.5	0.44 (1)	10.00
AH- Q	0 / 5899	-18.5	-18.5	0.44 (1)	10.00
Q-AI	0 / 5903	-18.5	-18.5	0.44 (1)	10.00
AI- P	0 / 5903	-18.5	-18.5	0.44 (1)	10.00
P- O	0 / 5903	-18.5	-18.5	0.44 (1)	10.00
O-AJ	0 / 3994	-18.5	-18.5	0.30 (1)	10.00
AJ-AK	0 / 3994	-18.5	-18.5	0.30 (1)	10.00
AK-AL	0 / 3994	-18.5	-18.5	0.30 (1)	10.00
AL- N	0 / 3994	-18.5	-18.5	0.30 (1)	10.00
N-AM	0 / 3514	-18.5	-18.5	0.27 (1)	10.00
AM-AN	0 / 3514	-18.5	-18.5	0.27 (1)	10.00
AN- M	0 / 3514	-18.5	-18.5	0.27 (1)	10.00

WEBS

MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
C- T	0 / 608	0.08 (1)
S- E	-1235 / 0	0.16 (1)
E- Q	0 / 680	0.08 (1)
Q- G	-684 / 0	0.09 (1)
Q- H	0 / 676	0.08 (1)
O- H	-1237 / 0	0.16 (1)
N- J	0 / 608	0.08 (1)
U- C	-4419 / 0	0.52 (1)
J- M	-4419 / 0	0.52 (1)
T- D	-203 / 53	0.03 (1)
D- S	0 2366	0.29 (1)
I- N	-203 / 53	0.03 (1)
O- I	0 / 2371	0.29 (1)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. A. G. ALVES
100009024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.51/1.00 (G-H:1) , BC=0.44/1.00 (O-Q:1) , WB=0.52/1.00 (C-U:1) , SSI=0.19/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (D) (INPUT = 0.90)
JSI METAL= 0.57 (J) (INPUT = 1.00)

Structural component only
DWG# T-2006315

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408130	T21	1	2	TRUSS DESC.	

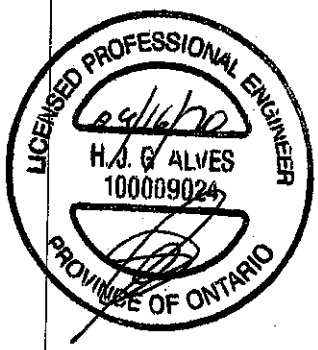
Tamarack Roof Truss, Burlington

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-15	-416	-416	--	FRONT	VERT	TOTAL	--	C1
F	13-11-4	-110	-110	--	FRONT	VERT	TOTAL	--	C1
G	15-11-4	-110	-110	--	FRONT	VERT	TOTAL	--	C1
I	26-0-1	-416	-416	--	FRONT	VERT	TOTAL	--	C1
P	19-11-4	-26	-26	--	FRONT	VERT	TOTAL	--	C1
Q	15-11-4	-26	-26	--	FRONT	VERT	TOTAL	--	C1
R	11-11-4	-26	-26	--	FRONT	VERT	TOTAL	--	C1
V	7-11-4	-110	-110	--	FRONT	VERT	TOTAL	--	C1
W	9-11-4	-110	-110	--	FRONT	VERT	TOTAL	--	C1
X	11-11-4	-110	-110	--	FRONT	VERT	TOTAL	--	C1
Y	17-11-4	-110	-110	--	FRONT	VERT	TOTAL	--	C1
Z	19-11-4	-110	-110	--	FRONT	VERT	TOTAL	--	C1
AA	21-11-4	-110	-110	--	FRONT	VERT	TOTAL	--	C1
AB	23-11-4	-110	-110	--	FRONT	VERT	TOTAL	--	C1
AC	2-0-12	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AD	4-0-12	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AE	5-11-4	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AF	7-11-4	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AG	9-11-4	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AH	13-11-4	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AI	17-11-4	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AJ	21-11-4	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AK	23-11-4	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AL	25-11-12	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AM	27-10-4	-26	-26	--	FRONT	VERT	TOTAL	--	C1
AN	29-10-4	-26	-26	--	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2006315

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	T21Z	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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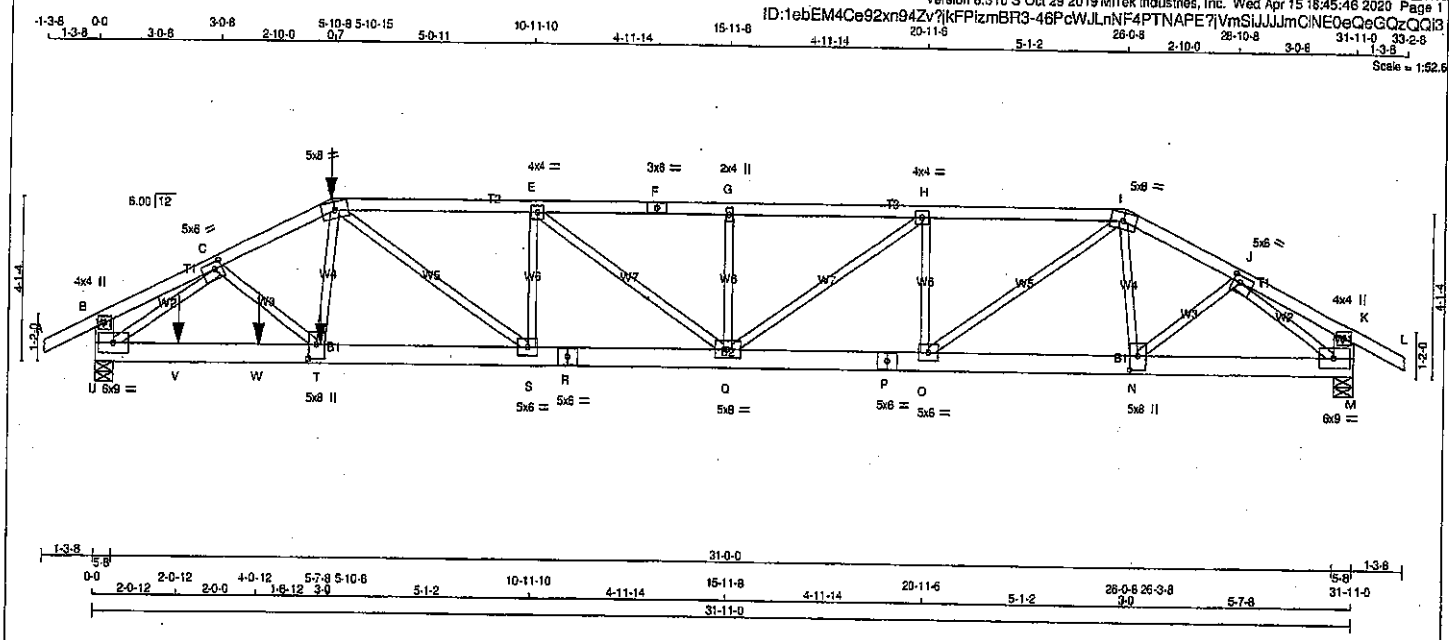


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-D	1	12	TOP
D-F	1	12	TOP
F-I	1	12	TOP
I-L	1	12	TOP
U-B	2	12	TOP
M-K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U-R	2	12	SIDE(183.1)
R-P	2	12	TOP
P-M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
T-D	1	3	SIDE(593.4)
2x3	1	6	
N-I	1	3	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.00	2.25
D	TTWW-m	MT20	5.0	8.0		
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMV+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	8.0		
J	TMWW-t	MT20	5.0	6.0	2.00	2.25
K	TMV+p	MT20	4.0	4.0		
M	BMVW1-t	MT20	6.0	9.0		
N	BMWW+t	MT20	5.0	8.0	4.25	2.50
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	4618	0	4618	0	0	5-8	5-8
M	2464	0	2464	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	3253	2208 / 0	0 / 0	0 / 0	0 0	1044 0	0 0
M	1738	1169 / 0	0 / 0	0 / 0	0 0	588 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B		0 / 28	-91.8	-91.8	0.07 (1)	10.00
B-C		-3 / 0	-91.8	-91.8	0.08 (1)	10.00
C-D		-7232 / 0	-91.8	-91.8	0.26 (1)	3.47
D-E		-6715 / 0	-91.8	-91.8	0.40 (1)	3.51
E-F		-6255 / 0	-91.8	-91.8	0.37 (1)	3.65
F-G		-6255 / 0	-91.8	-91.8	0.37 (1)	3.65
G-H		-6255 / 0	-91.8	-91.8	0.37 (1)	3.64
H-I		-5158 / 0	-91.8	-91.8	0.31 (1)	4.02
I-J		-3581 / 0	-91.8	-91.8	0.10 (1)	4.90
J-K		0 / 8	-91.8	-91.8	0.06 (1)	10.00
K-L		0 / 28	-91.8	-91.8	0.07 (1)	10.00
U-B		-264 / 0	0.0	0.0	0.01 (1)	7.81
M-K		-243 / 0	0.0	0.0	0.01 (1)	7.81
U-V		0 / 5578	-18.5	-18.5	0.44 (1)	10.00
V-W		0 5578	-18.5	-18.5	0.44 (1)	10.00
W-T		0 / 5578	-18.5	-18.5	0.44 (1)	10.00
T-S		0 / 6281	-18.5	-18.5	0.46 (1)	10.00
S-R		0 / 6715	-18.5	-18.5	0.48 (1)	10.00
R-Q		0 / 6715	-18.5	-18.5	0.48 (1)	10.00
Q-P		0 / 5158	-18.5	-18.5	0.38 (1)	10.00
P-O		0 / 5158	-18.5	-18.5	0.38 (1)	10.00
O-N		0 / 3192	-18.5	-18.5	0.23 (1)	10.00
N-M		0 / 2850	-18.5	-18.5	0.21 (1)	10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-T		0 / 1189	0.14 (1)
S-E		-178 / 0	0.02 (1)
E-Q		-574 / 0	0.20 (1)
Q-G		-422 / 0	0.05 (1)
Q-H		0 / 1369	0.17 (1)
Q-H		-1343 / 0	0.17 (1)
N-J		0 / 431	0.05 (1)
U-C		-7015 / 0	0.93 (1)
J-M		-3584 / 0	0.43 (1)
T-D		0 / 2309	0.29 (1)
D-S		0 / 539	0.07 (1)
I-N		-167 / 9	0.02 (1)
Q-I		0 / 2442	0.30 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYP	CONN.
D	5-10-15	-285	-285	---	TOP	VERT	TOTAL	C1
T	5-7-8	-2976	-2976	---	BACK	VERT	TOTAL	C1
V	2-0-12	-26	-26	---	BACK	VERT	TOTAL	C1
W	4-0-12	-26	-26	---	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G. ALVES
100089925
PROVINCE OF ONTARIO

TOTAL WEIGHT = 2 X 144 = 288 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.40/1.00 (D-E:1) , BC=0.48/1.00 (Q-S:1) , WB=0.83/1.00 (C-U:1) , SSI=0.12/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 0.92 (J) (INPUT = 1.00)

Structural component only
DWG# T-2006316

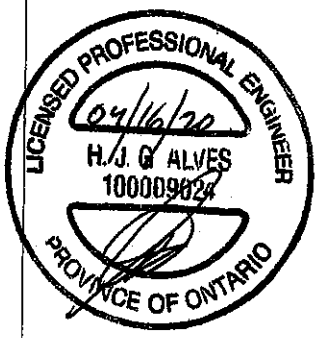
CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408130	T21Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
T	BMWW+t	MT20	5.0	8.0	4.25	2.50
U	BMVW1-t	MT20	6.0	9.0		



Structural component only
DWG# T-2006316

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	T22	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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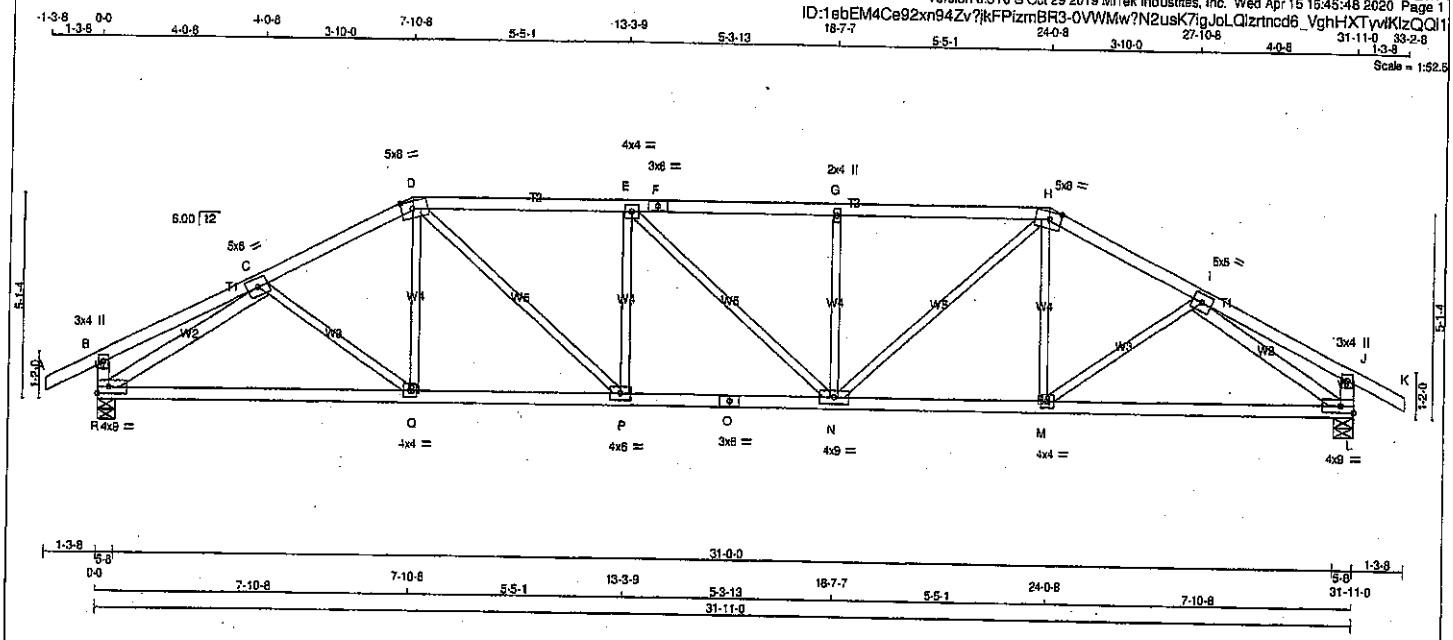


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	8.0	2.25	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.25	3.50
I	TMWW-t	MT20	5.0	6.0		
J	TMV+p	MT20	3.0	4.0		
L	BMWW1-t	MT20	4.0	9.0		Edge
M	BMWW-t	MT20	4.0	4.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1884	0	1884	0	0	5-8	5-8
L	1884	0	1884	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1330	886 / 0	0 / 0	0 / 0	0 . 0	444 0	0 . 0
L	1330	886 / 0	0 / 0	0 / 0	0 . 0	444 0	0 . 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

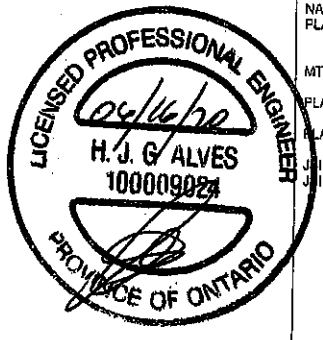
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
B- C	0 / 17	-91.8	-91.8	0.20 (1)	10.00	
C- D	-2469 / 0	-91.8	-91.8	0.30 (1)	4.12	
D- E	-2893 / 0	-91.8	-91.8	0.54 (1)	3.61	
E- F	-2891 / 0	-91.8	-91.8	0.53 (1)	3.61	
F- G	-2891 / 0	-91.8	-91.8	0.53 (1)	3.61	
G- H	-2891 / 0	-91.8	-91.8	0.53 (1)	3.61	
H- I	-2469 / 0	-91.8	-91.8	0.30 (1)	4.12	
I- J	0 / 17	-91.8	-91.8	0.20 (1)	10.00	
J- K	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
R- B	-269 / 0	0.0	0.0	0.03 (1)	7.81	
L- J	-269 / 0	0.0	0.0	0.03 (1)	7.81	
R- Q	0 / 2167	-18.5	-18.5	0.49 (1)	10.00	
Q- P	0 / 2194	-18.5	-18.5	0.48 (1)	10.00	
P- O	0 / 2893	-18.5	-18.5	0.53 (1)	10.00	
O- N	0 / 2893	-18.5	-18.5	0.53 (1)	10.00	
N- M	0 / 2194	-18.5	-18.5	0.49 (1)	10.00	
M- L	0 / 2168	-18.5	-18.5	0.49 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- Q	0 / 74	0.03 (4)
Q- D	0 / 133	0.04 (4)
D- P	0 / 933	0.21 (1)
P- E	-534 / 0	0.21 (1)
E- N	-2 / 0	0.00 (1)
N- G	-533 / 0	0.21 (1)
N- H	0 / 931	0.21 (1)
M- H	0 / 134	0.04 (4)
M- I	0 / 74	0.03 (4)
R- C	-2666 / 0	0.74 (1)
I- L	-2667 / 0	0.74 (1)



TOTAL WEIGHT = 2 X 130 = 259 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.54/1.00 (D-E:1) , BC=0.53/1.00 (N-P:1) , WB=0.74/1.00 (H-L:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JOINT GRIP= 0.84 (O) (INPUT = 0.90)
JOINT METAL= 0.94 (O) (INPUT = 1.00)

Structural component only
DWG# T-2006317

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	T23	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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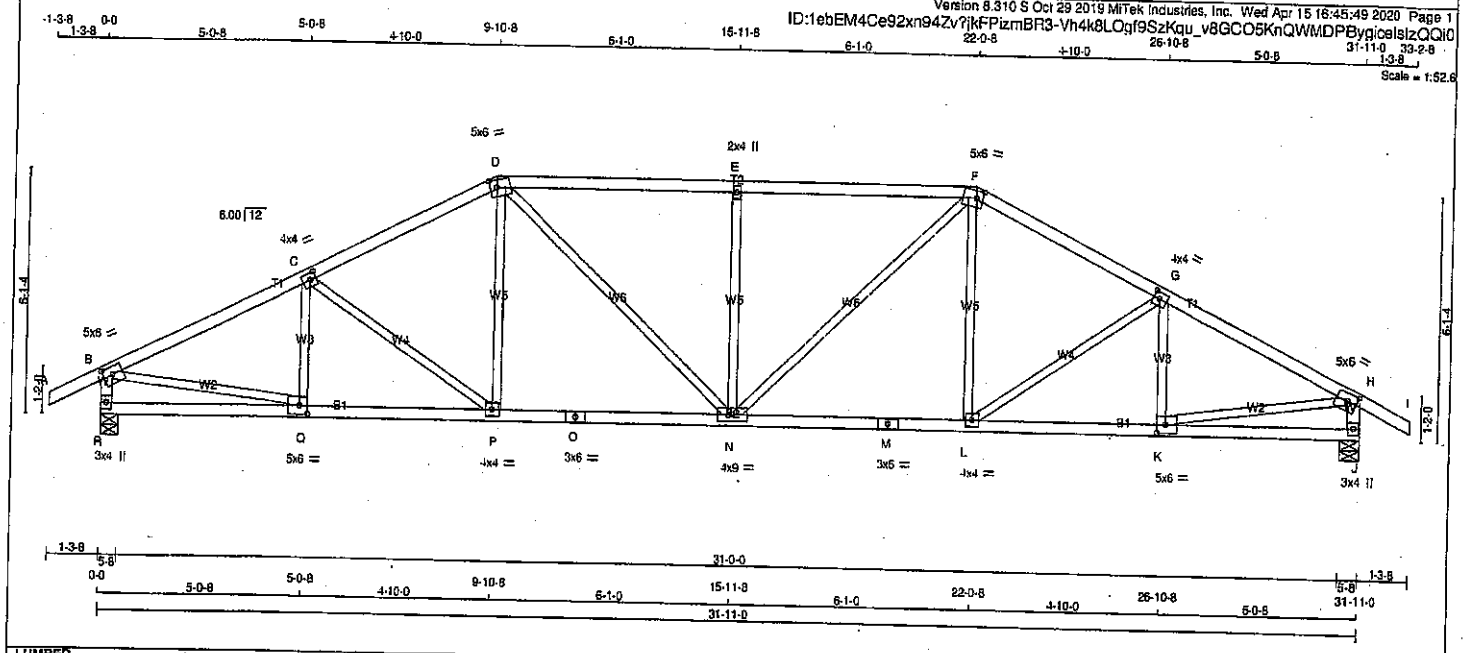

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.25	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	6.0	2.25	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.50
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1884	0	1884	0	0	5-8	5-8
J	1884	0	1884	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1330	886 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0
J	1330	886 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B-C	-2538 / 0	-91.8	-91.8	0.37 (1)	4.02
C-D	-2342 / 0	-91.8	-91.8	0.35 (1)	4.17
D-E	-2493 / 0	-91.8	-91.8	0.54 (1)	3.82
E-F	-2493 / 0	-91.8	-91.8	0.54 (1)	3.82
F-G	-2342 / 0	-91.8	-91.8	0.35 (1)	4.17
G-H	-2538 / 0	-91.8	-91.8	0.37 (1)	4.02
H-I	0 / 28	-91.8	-91.8	0.12 (1)	10.00
R-B	-1841 / 0	0.0	0.0	0.19 (1)	6.17
J-H	-1841 / 0	0.0	0.0	0.19 (1)	6.17
R-Q	0 / 0	-18.5	-18.5	0.10 (4)	10.00
Q-P	0 / 2290	-18.5	-18.5	0.43 (1)	10.00
P-O	0 / 2077	-18.5	-18.5	0.40 (1)	10.00
O-N	0 / 2077	-18.5	-18.5	0.40 (1)	10.00
N-M	0 / 2077	-18.5	-18.5	0.40 (1)	10.00
M-L	0 / 2077	-18.5	-18.5	0.40 (1)	10.00
L-K	0 / 2290	-18.5	-18.5	0.43 (1)	10.00
K-J	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-C	-301 / 0	0.07 (1)
C-P	-264 / 0	0.16 (1)
P-D	0 / 257	0.06 (1)
D-N	0 / 575	0.13 (1)
N-E	-685 / 0	0.40 (1)
N-F	0 / 575	0.13 (1)
L-F	0 / 257	0.06 (1)
L-G	-264 / 0	0.16 (1)
K-G	-301 / 0	0.07 (1)
B-Q	0 / 2323	0.52 (1)
K-H	0 / 2323	0.52 (1)

TOTAL WEIGHT = 2 X 129 = 258 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.54/1.00 (D-E:1) , BC=0.43/1.00 (K-L:1) , WB=0.52/1.00 (H-K:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.90 (B) (INPUT = 0.90)
SSI METAL= 0.67 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006318

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	T24	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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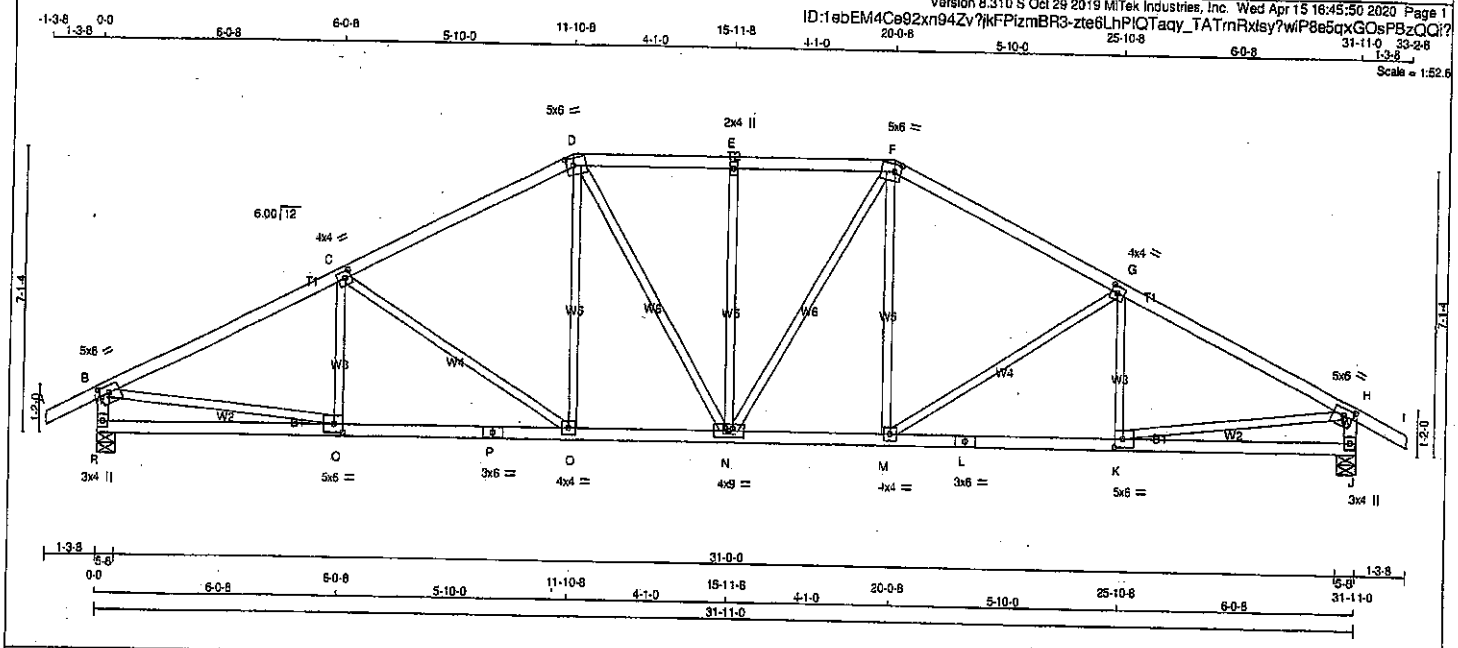


TOTAL WEIGHT = 2 X 136 = 271 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	6.0	2.00	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.50
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1884	0	1884	0	0	5-8	5-8
J	1884	0	1884	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1330	886 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0
J	1330	886 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

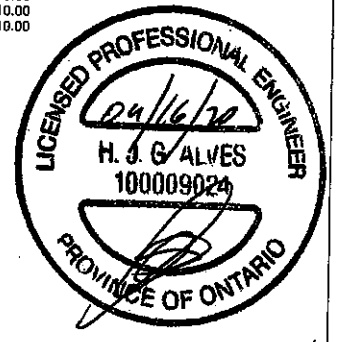
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.12	(1)		10.00
B-C	-2579 / 0	-91.8	-91.8	0.55	(1)		3.79
C-D	-2188 / 0	-91.8	-91.8	0.49	(1)		4.11
D-E	-2088 / 0	-91.8	-91.8	0.24	(1)		4.49
E-F	-2088 / 0	-91.8	-91.8	0.24	(1)		4.49
F-G	-2188 / 0	-91.8	-91.8	0.49	(1)		4.11
G-H	-2579 / 0	-91.8	-91.8	0.55	(1)		3.79
H-I	0 / 28	-91.8	-91.8	0.12	(1)		10.00
R-B	-1836 / 0	0.0	0.0	0.19	(1)		6.18
J-H	-1836 / 0	0.0	0.0	0.19	(1)		6.18
R-Q	0 / 0	-18.5	-18.5	0.15	(4)		10.00
Q-P	0 / 2332	-18.5	-18.5	0.44	(1)		10.00
P-O	0 / 2332	-18.5	-18.5	0.44	(1)		10.00
O-N	0 / 1935	-18.5	-18.5	0.37	(1)		10.00
N-M	0 / 1935	-18.5	-18.5	0.37	(1)		10.00
M-L	0 / 2332	-18.5	-18.5	0.44	(1)		10.00
L-K	0 / 2332	-18.5	-18.5	0.44	(1)		10.00
K-J	0 / 0	-18.5	-18.5	0.15	(4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
Q-C	-212 / 29	0.06	(1)
C-O	-480 / 0	0.46	(1)
Q-D	0 / 362	0.08	(1)
D-N	0 / 296	0.07	(1)
N-E	-451 / 0	0.39	(1)
N-F	0 / 296	0.07	(1)
M-F	0 / 362	0.08	(1)
M-G	-480 / 0	0.46	(1)
K-G	-212 / 29	0.06	(1)
B-Q	0 / 2355	0.53	(1)
K-H	0 / 2355	0.53	(1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.55/1.00 (G-H:1) , BC=0.44/1.00 (K-M:1) , WB=0.53/1.00 (H-K:1) , SSI=0.24/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.74 (P) (INPUT = 1.00)

Structural component only
DWG# T-2006319

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	T25	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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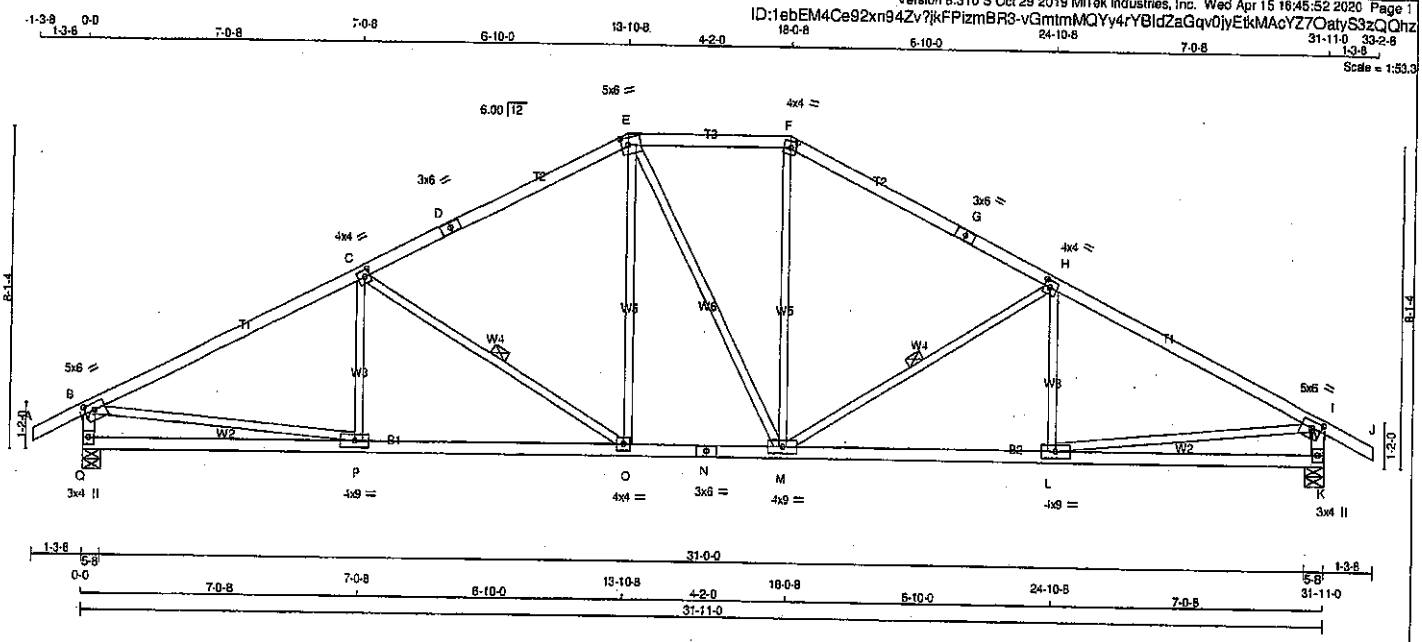

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TTW-m	MT20	4.0	4.0	2.00	1.75
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.75
I	TMVW-t	MT20	5.0	6.0	2.00	2.75
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	4.0	9.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BS-t	MT20	3.0	8.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	9.0		
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 2 X 131 = 263 lb

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1884	0	1884	0	0	5-8	5-8
K	1884	0	1884	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1330	886 / 0	0 / 0	0 / 0	0 / 0	444 0	0 0
K	1330	886 / 0	0 / 0	0 / 0	0 / 0	444 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	
B-C	-2587 / 0	-91.8 -91.8	0.78 (1)	3.47	
C-D	-2013 / 0	-91.8 -91.8	0.68 (1)	3.97	
D-E	-2013 / 0	-91.8 -91.8	0.68 (1)	3.97	
E-F	-1775 / 0	-91.8 -91.8	0.24 (1)	4.79	
F-G	-2013 / 0	-91.8 -91.8	0.68 (1)	3.97	
G-H	-2013 / 0	-91.8 -91.8	0.68 (1)	3.97	
H-I	-2587 / 0	-91.8 -91.8	0.78 (1)	3.47	
I-J	0 / 28	-91.8 -91.8	0.12 (1)	10.00	
Q-B	-1830 / 0	0.0 0.0	0.18 (1)	6.19	
K-I	-1830 / 0	0.0 0.0	0.18 (1)	6.19	
Q-P	0 / 0	-18.5 -18.5	0.22 (4)	10.00	
P-O	0 / 2344	-18.5 -18.5	0.48 (1)	10.00	
O-N	0 / 1774	-18.5 -18.5	0.35 (1)	10.00	
N-M	0 / 1774	-18.5 -18.5	0.35 (1)	10.00	
M-L	0 / 2344	-18.5 -18.5	0.48 (1)	10.00	
L-K	0 / 0	-18.5 -18.5	0.22 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-C	-140 / 71	0.05 (1)
C-O	-681 / 0	0.31 (1)
O-E	0 / 458	0.10 (1)
E-M	0 2	0.00 (1)
M-F	0 460	0.10 (1)
M-H	-680 0	0.31 (1)
L-H	-142 70	0.05 (1)
B-P	0 2362	0.53 (1)
L-I	0 2361	0.53 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.78/1.00 (B-C:1) , BC=0.48/1.00 (O-P:1) , WB=0.53/1.00 (B-P:1) , SSI=0.28/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	789	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.89 (B) (INPUT = 0.90)
SSI METAL= 0.70 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006320

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	T26	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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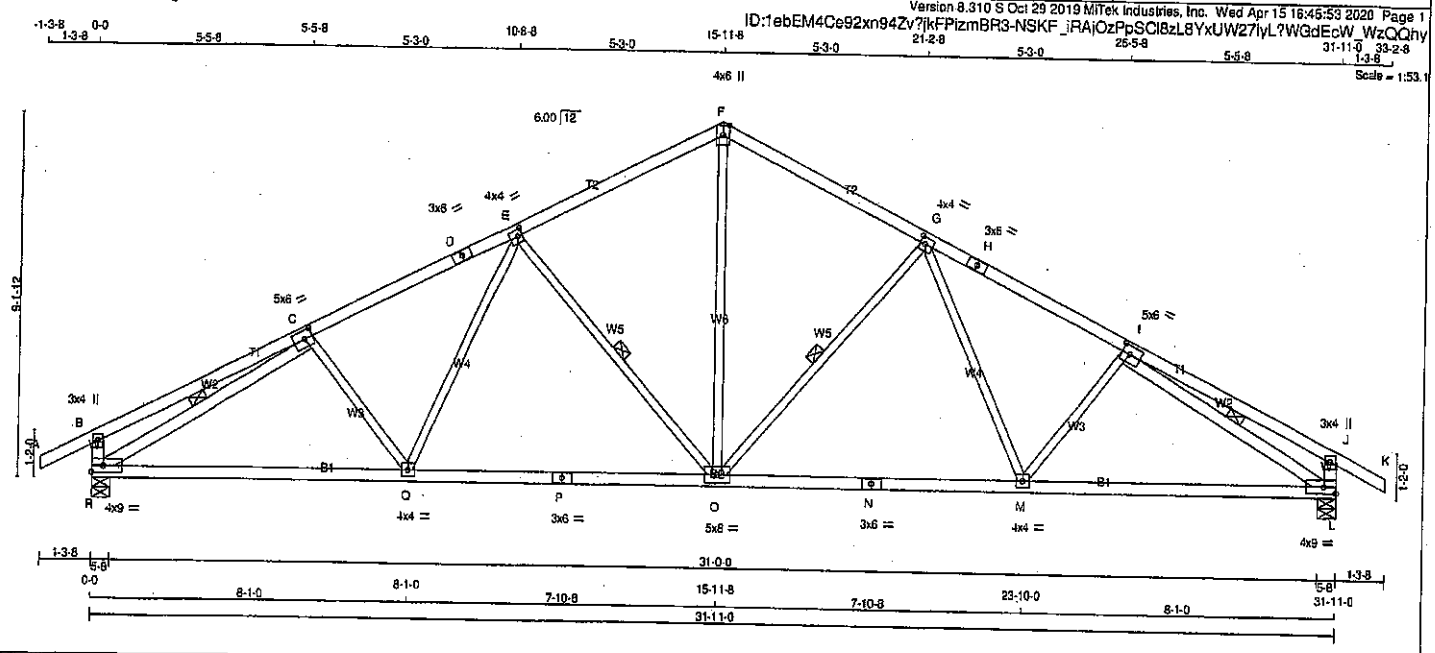

TOTAL WEIGHT = 3 X 133 = 400 lb (M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.50
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	9.0		Edge
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1884	0	1884	0	0	5-8	5-8
L	1884	0	1884	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1330	886 / 0	0 / 0	0 / 0	0 0	444 0	0 0
L	1330	886 / 0	0 / 0	0 / 0	0 0	444 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-O, E-O, C-R, I-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
B- C	0 / 20	-91.8	-91.8	0.33 (1)	10.00	
C- D	-2428 / 0	-91.8	-91.8	0.37 (1)	4.11	
D- E	-2428 / 0	-91.8	-91.8	0.37 (1)	4.11	
E- F	-1808 / 0	-91.8	-91.8	0.34 (1)	4.65	
F- G	-1808 / 0	-91.8	-91.8	0.34 (1)	4.65	
G- H	-2428 / 0	-91.8	-91.8	0.37 (1)	4.11	
H- I	-2428 / 0	-91.8	-91.8	0.37 (1)	4.11	
I- J	0 / 20	-91.8	-91.8	0.33 (1)	10.00	
J- K	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
R- B	-325 / 0	0.0	0.0	0.03 (1)	7.81	
L- J	-325 / 0	0.0	0.0	0.03 (1)	7.81	
R- Q	0 / 2268	-18.5	-18.5	0.51 (1)	10.00	
Q- P	0 / 2051	-18.5	-18.5	0.48 (1)	10.00	
P- O	0 / 2051	-18.5	-18.5	0.48 (1)	10.00	
O- N	0 / 2051	-18.5	-18.5	0.48 (1)	10.00	
N- M	0 / 2051	-18.5	-18.5	0.48 (1)	10.00	
M- L	0 / 2268	-18.5	-18.5	0.51 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- F	0 / 1211	0.27 (1)
O- G	-701 / 0	0.32 (1)
G- M	0 / 312	0.07 (1)
M- I	-156 / 21	0.05 (1)
E- O	-701 / 0	0.32 (1)
Q- E	0 / 312	0.07 (1)
C- Q	-156 / 21	0.05 (1)
R- C	-2717 / 0	0.55 (1)
I- L	-2717 / 0	0.55 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.37/1.00 (G-I:1) , BC=0.51/1.00 (L-M:1) , WB=0.55/1.00 (I-L:1) , SSI=0.20/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.85 (I) (INPUT = 0.90)
JT METAL= 0.70 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006321

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	T27	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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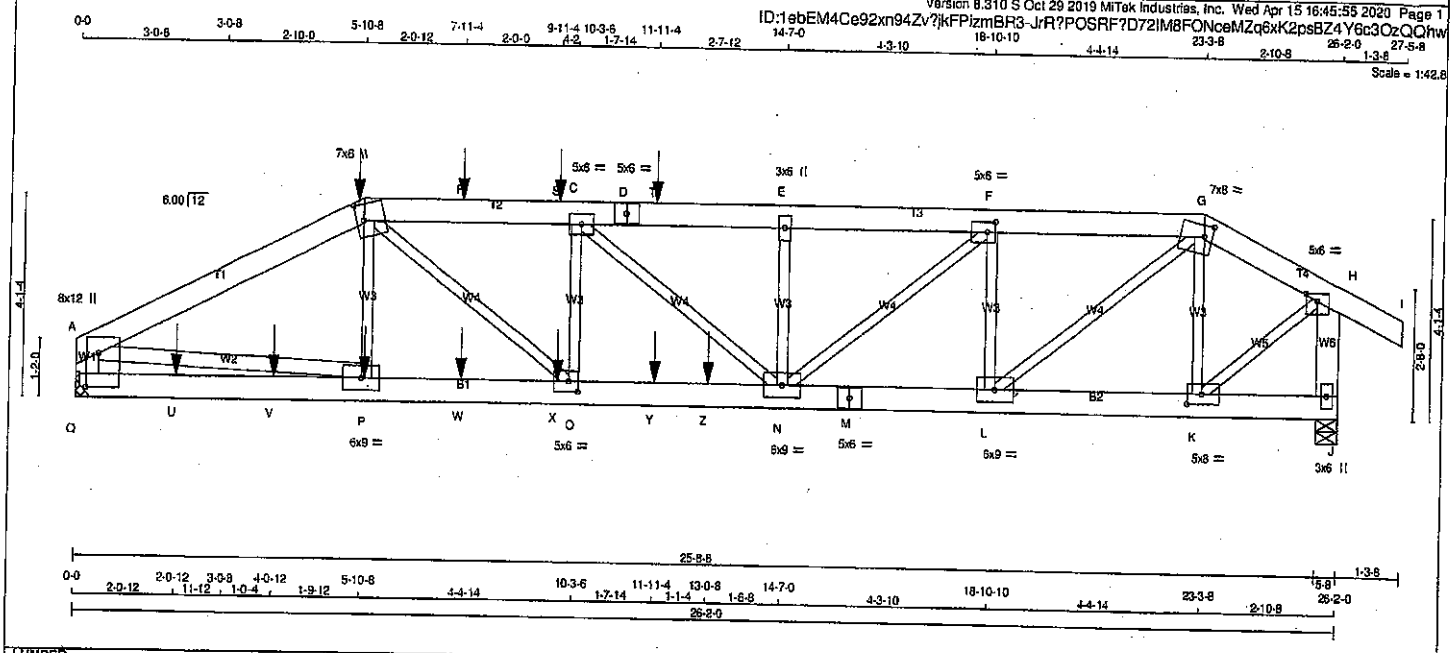

TOTAL WEIGHT = 142 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x6	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
Q - A	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	2100F 1.8E	SPF
M - J	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
A - P	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A						
A	TMBMVW1"+p	MT20	6.0	12.0	8.25	3.00
B	TTWW+m	MT20	7.0	8.0	4.00	1.75
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TMWW-t	MT20	5.0	6.0	2.50	2.00
G	TTWW-m	MT20	7.0	8.0	2.75	2.00
H	TMVW-p	MT20	5.0	6.0	1.75	3.00
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	3.75
L	BMWW-t	MT20	6.0	9.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	6.0	9.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.25
P	BMWW-t	MT20	6.0	9.0		
Q						

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2867	0	2887	0	0	MECHANICAL	
J	2551	0	2551	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT Q. MINIMUM BEARING LENGTH AT JOINT Q = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2039	1359 / 0	0 / 0	0 / 0	0 - 0	586 0	0 - 0
J	1798	1214 / 0	0 / 0	0 / 0	0 0	584 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)
A-B	-4593 / 0	-91.8	-91.8	0.46 (1)	
B-R	-5805 / 0	-91.8	-91.8	0.45 (1)	
R-S	-5805 / 0	-91.8	-91.8	0.45 (1)	
S-C	-5805 / 0	-91.8	-91.8	0.45 (1)	
C-D	-5767 / 0	-91.8	-91.8	0.36 (1)	
D-T	-5767 / 0	-91.8	-91.8	0.36 (1)	
T-E	-5767 / 0	-91.8	-91.8	0.36 (1)	
E-F	-5767 / 0	-91.8	-91.8	0.36 (1)	
F-G	-4032 / 0	-91.8	-91.8	0.22 (1)	
G-H	-2087 / 0	-91.8	-91.8	0.09 (1)	
H-I	0 - 29	-91.8	-91.8	0.07 (1)	
Q-A	-2799 / 0	0.0	0.0	0.20 (1)	
J-H	-2565 / 0	0.0	0.0	0.22 (1)	
Q-U	0 / 0	-18.5	-18.5	0.08 (4)	
U-V	0 / 0	-18.5	-18.5	0.08 (4)	
V-P	0 / 0	-18.5	-18.5	0.08 (4)	
P-W	0 / 4095	-18.5	-18.5	0.33 (1)	
W-X	0 / 4095	-18.5	-18.5	0.33 (1)	
X-O	0 / 4095	-18.5	-18.5	0.33 (1)	
O-Y	0 / 5805	-18.5	-18.5	0.73 (1)	
Y-Z	0 - 5805	-18.5	-18.5	0.73 (1)	
Z-N	0 / 5805	-18.5	-18.5	0.73 (1)	
N-M	0 / 4032	-18.5	-18.5	0.35 (1)	
M-L	0 - 4032	-18.5	-18.5	0.35 (1)	
L-K	0 / 1629	-18.5	-18.5	0.16 (1)	
K-J	0 / 0	-18.5	-18.5	0.05 (1)	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	MAX. UNBRAC LENGTH
P-B	-319 / 48	0.08 (1)	
B-O	0 - 2216	0.55 (1)	
O-C	-705 / 0	0.17 (1)	
C-N	-51 / 0	0.03 (1)	
N-E	-388 / 0	0.09 (1)	
N-F	0 / 2274	0.56 (1)	
L-F	-1913 / 0	0.46 (1)	
L-G	0 2855	0.71 (1)	
K-G	-1193 / 0	0.29 (1)	
A-P	0 - 4131	0.73 (1)	
K-H	0 2310	0.57 (1)	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	CONC.
B	5-10-8	-439	-439	---	BACK	VERT	TOTAL	---
P	5-11-4	-58	-58	---	BACK	VERT	TOTAL	---
R	7-11-4	-144	-144	---	BACK	VERT	TOTAL	---
S	9-11-4	-144	-144	---	BACK	VERT	TOTAL	---
T	11-11-4	-110	-110	---	BACK	VERT	TOTAL	---
U	2-0-12	-28	-28	---	BACK	VERT	TOTAL	---
V	4-0-12	-37	-37	---	BACK	VERT	TOTAL	---
W	7-11-4	-56	-56	---	BACK	VERT	TOTAL	---
X	9-11-4	-56	-56	---	BACK	VERT	TOTAL	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.87")
CALCULATED VERT. DEFL.(TL) = L/ 852 (0.37")

CSI: TC=0.46/1.00 (A-B:1) , BC=0.73/1.00 (N-O:1) , WB=0.73/1.00 (A-P:1) , SSI=0.61/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

MSI GRIP= 0.90 (B) (INPUT = 0.90)
MSI METAL= 0.81 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only 1/2
DWG# T-2006322

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	T27	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Y	11-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
Z	13-0-8	-1329	-1329	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. O. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006322

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	T28	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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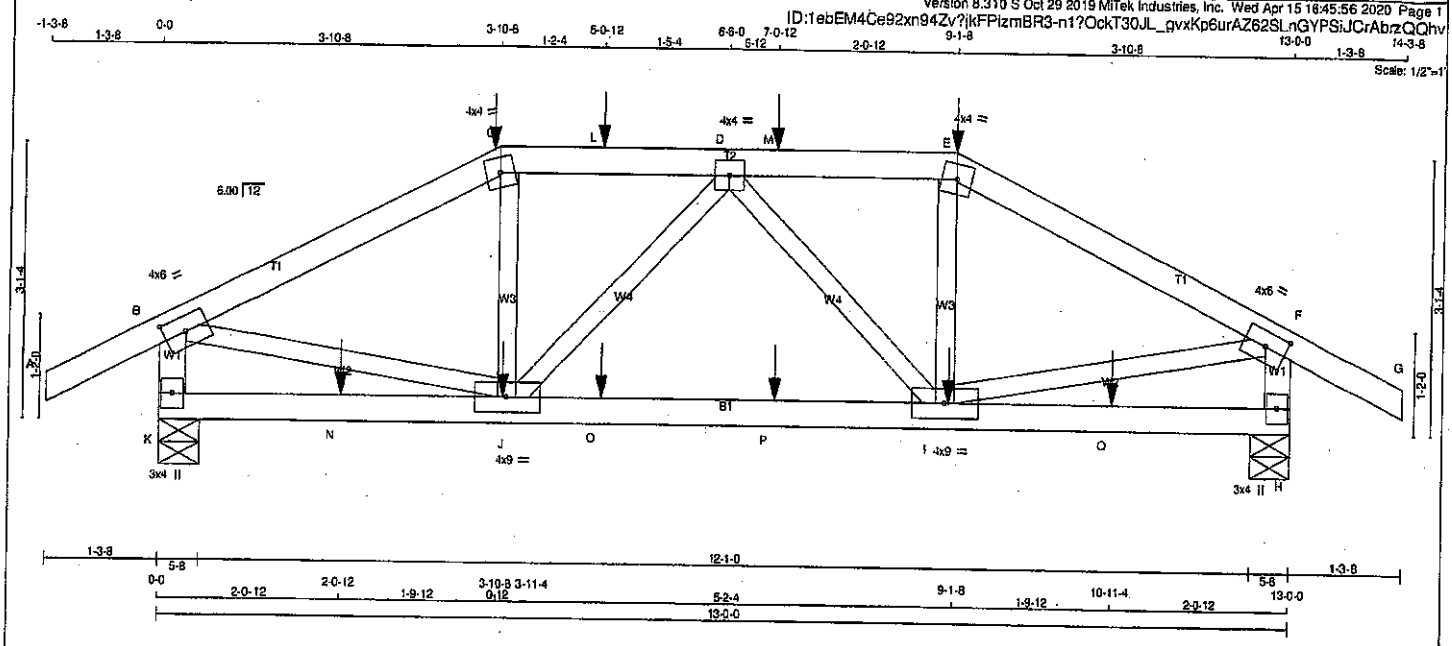


TOTAL WEIGHT = 52 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0	2.00	3.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1132	0	1132	0	0	5-8	5-8
H	1124	0	1124	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	797	540 / 0	0 / 0	0 / 0	0 0	256 0	0 0
H	792	536 / 0	0 / 0	0 / 0	0 0	256 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.13 (1)	10.00
B-C	-1246 / 0	-91.8	-91.8	0.29 (1)	5.36
C-L	-1117 / 0	-91.8	-91.8	0.14 (1)	5.81
L-D	-1117 / 0	-91.8	-91.8	0.14 (1)	5.81
D-M	-1107 / 0	-91.8	-91.8	0.14 (1)	5.83
M-E	-1107 / 0	-91.8	-91.8	0.14 (1)	5.83
E-F	-1234 / 0	-91.8	-91.8	0.29 (1)	5.39
F-G	0 / 28	-91.8	-91.8	0.13 (1)	10.00
K-B	-1097 / 0	0.0	0.0	0.12 (1)	7.52
H-F	-1090 / 0	0.0	0.0	0.12 (1)	7.54
K-N	0 / 0	-18.5	-18.5	0.12 (4)	10.00
N-J	0 / 0	-18.5	-18.5	0.12 (4)	10.00
J-O	0 / 1263	-18.5	-18.5	0.28 (1)	10.00
O-P	0 / 1263	-18.5	-18.5	0.28 (1)	10.00
P-I	0 / 1263	-18.5	-18.5	0.28 (1)	10.00
I-Q	0 / 0	-18.5	-18.5	0.11 (4)	10.00
Q-H	0 / 0	-18.5	-18.5	0.11 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-C	0 / 90	0.03 (4)
J-D	-223 / 0	0.06 (1)
D-I	-238 / 0	0.06 (1)
I-E	0 / 90	0.03 (4)
B-J	0 / 1138	0.28 (1)
I-F	0 / 1127	0.28 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL/TOE	CONN
C	3-10-8	-209	-209	---	FRONT	VERT	TOTAL	---	C1
E	9-1-8	-209	-209	---	FRONT	VERT	TOTAL	---	C1
I	9-0-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1
J	3-11-4	-12	-12	---	FRONT	VERT	TOTAL	---	C1
L	5-0-12	-42	-42	---	FRONT	VERT	TOTAL	---	C1
M	7-0-12	-42	-42	---	FRONT	VERT	TOTAL	---	C1
N	2-0-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1
O	5-0-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1
P	7-0-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1
Q	10-11-4	-12	-12	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.28/1.00 (I-J:1) , WB=0.28/1.00 (B-J:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

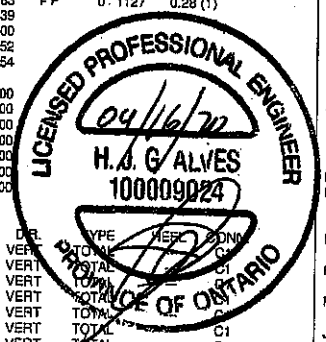
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (J) (INPUT = 0.60)
JSI METAL= 0.35 (F) (INPUT = 1.00)



Structural component only
DWG# T-2006323

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	T29	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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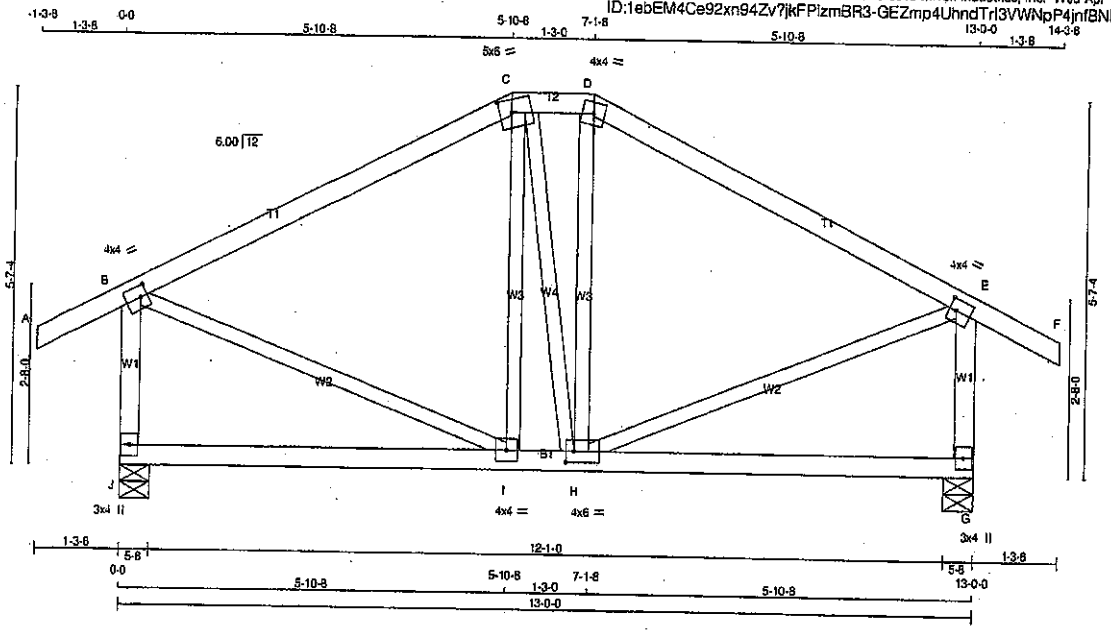


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	6.0	2.25	2.25
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0	2.00	1.50
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	841	0	841	0	0	5-8	5-8
G	841	0	841	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	593	402 / 0	0 / 0	0 / 0	0 0	190 0	0 / 0
G	593	402 / 0	0 / 0	0 / 0	0 0	190 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

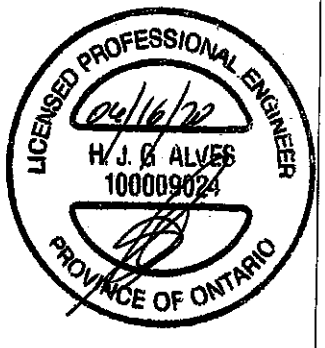
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B-C	-502 / 0	-91.8	-91.8	0.41 (1)	6.25
C-D	-446 / 0	-91.8	-91.8	0.02 (1)	6.25
D-E	-501 / 0	-91.8	-91.8	0.41 (1)	6.25
E-F	0 / 28	-91.8	-91.8	0.12 (1)	10.00
J-B	-796 / 0	0.0	0.0	0.11 (1)	7.81
G-E	-796 / 0	0.0	0.0	0.11 (1)	7.81
J-I	0 / 0	-18.5	-18.5	0.15 (4)	10.00
I-H	0 / 447	-18.5	-18.5	0.19 (4)	10.00
H-G	0 / 0	-18.5	-18.5	0.15 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-C	-98 / 20	0.05 (1)
C-H	-4 / 0	0.00 (1)
H-D	-102 / 21	0.05 (1)
B-I	0 / 483	0.11 (1)
H-E	0 / 482	0.11 (1)



TOTAL WEIGHT = 62 lb
MI(F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2018 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.41/1.00 (B-C:1) , BC=0.19/1.00 (H-I:4) ; WB=0.11/1.00 (B-I:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (B) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006324

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	T30	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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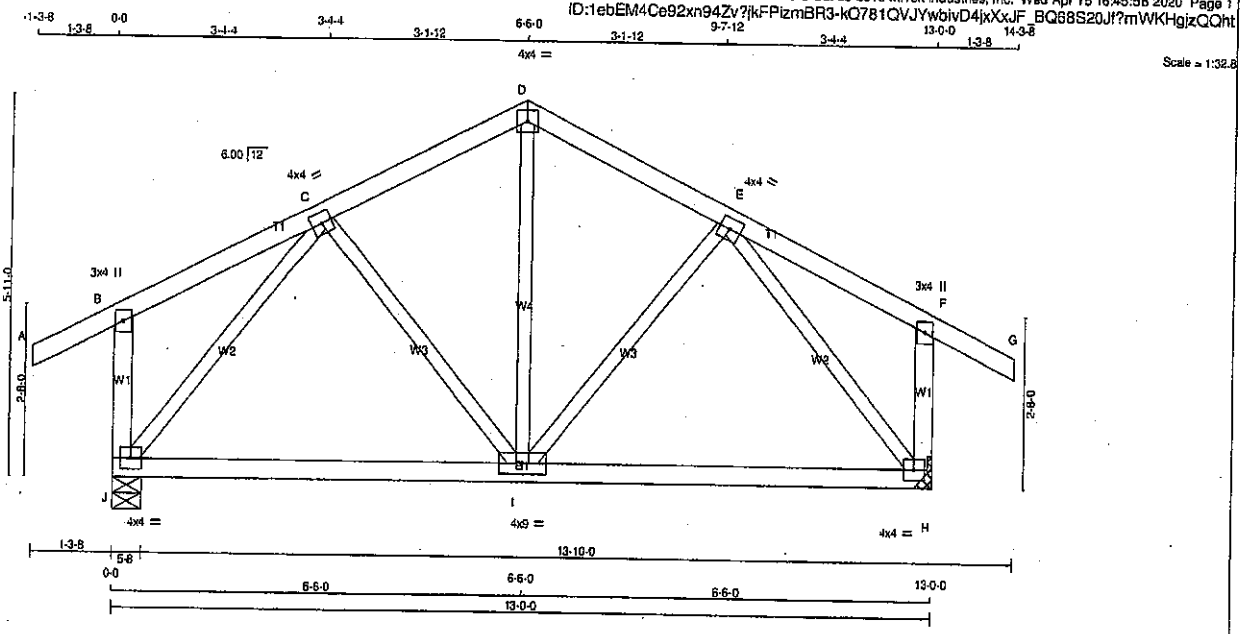


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	841	0	841	0	0	5-8	5-8
H	841	0	841	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	593	402 / 0	0 / 0	0 / 0	0 0	190 0	0 0
H	593	402 / 0	0 / 0	0 / 0	0 0	190 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

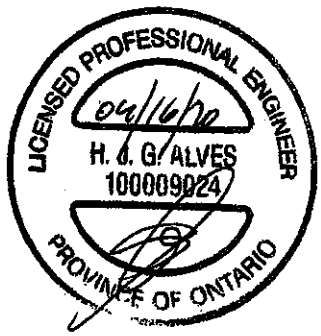
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.12 (1)		10.00
B-C	0 / 15	-91.8	-91.8	0.15 (1)		10.00
C-D	-498 / 0	-91.8	-91.8	0.12 (1)		6.25
D-E	-498 / 0	-91.8	-91.8	0.12 (1)		6.25
E-F	0 / 15	-91.8	-91.8	0.15 (1)		10.00
F-G	0 / 28	-91.8	-91.8	0.12 (1)		10.00
J-B	-240 / 0	0.0	0.0	0.03 (1)		7.81
H-F	-240 / 0	0.0	0.0	0.03 (1)		7.81
J-I	0 / 454	-18.5	-18.5	0.26 (4)		10.00
I-H	0 / 454	-18.5	-18.5	0.26 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-D	0 203	0.05 (1)
I-E	-35 32	0.02 (1)
C-I	-35 / 32	0.02 (1)
J-C	-717 / 0	0.30 (1)
E-H	-717 0	0.30 (1)



TOTAL WEIGHT = 3 X 60 = 180 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.15/1.00 (E-F:1) , BC=0.26/1.00 (H-I:4) , WB=0.30/1.00 (E-H:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (E) (INPUT = 0.90)
JSI METAL= 0.24 (E) (INPUT = 1.00)

Structural component only
DWG# T-2006325

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	T35	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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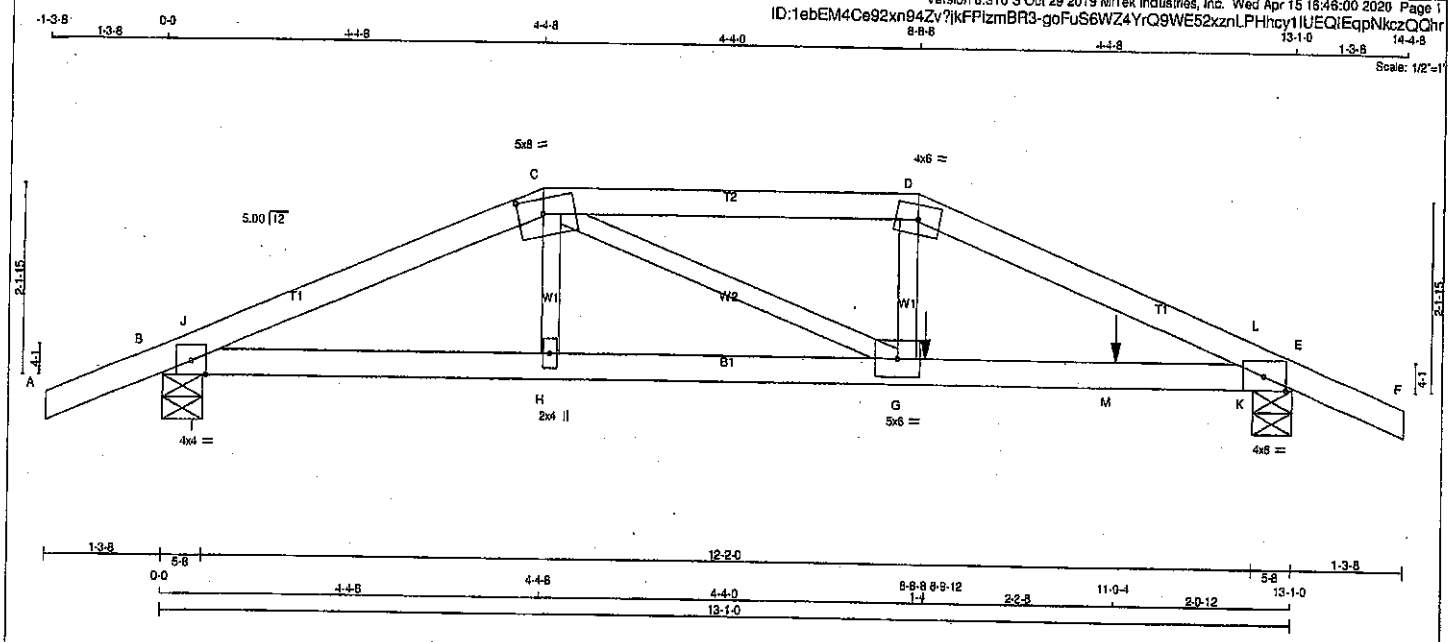


TOTAL WEIGHT = 40 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	4.0	Edge	
C	TTWW-m	MT20	5.0	8.0	2.00	3.50
D	TTW-m	MT20	4.0	6.0		
E	TMB1-I	MT20	4.0	6.0	Edge	
G	BMWW-t	MT20	5.0	6.0		
H	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	1203	0	1203	0	0	5-8	5-8
E	1585	0	1585	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	846	583 / 0	0 / 0	0 / 0	0 / 0	262 0	0 0
E	1113	773 / 0	0 / 0	0 / 0	0 / 0	340 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

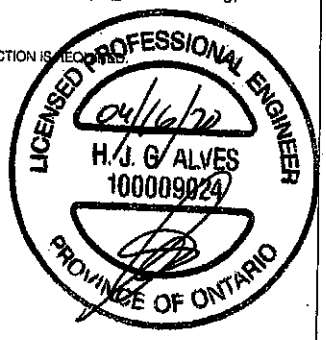
CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 23	-91.8	-91.8	0.13 (1)	10.00	H-C	0 / 109		0.04 (4)
B-J	-2159 / 0	-91.8	-91.8	0.13 (1)	4.49	C-G	0 / 882		0.22 (1)
J-C	-2154 / 0	-91.8	-91.8	0.31 (1)	4.30	G-D	0 / 778		0.19 (1)
C-D	-2796 / 0	-91.8	-91.8	0.47 (1)	3.64	I-J	-183 / 14		0.00 (1)
D-L	-2984 / 0	-91.8	-91.8	0.41 (1)	3.62	K-L	-46 / 56		0.00 (1)
L-E	-3048 / 0	-91.8	-91.8	0.13 (1)	3.88				
E-F	0 / 23	-91.8	-91.8	0.13 (1)	10.00				
B-I	0 / 1983	-18.5	-18.5	0.49 (1)	10.00				
I-H	0 / 1983	-18.5	-18.5	0.49 (1)	10.00				
H-G	0 / 1989	-18.5	-18.5	0.40 (1)	10.00				
G-M	0 / 2752	-18.5	-18.5	0.72 (1)	10.00				
M-K	0 / 2752	-18.5	-18.5	0.72 (1)	10.00				
K-E	0 / 2752	-18.5	-18.5	0.58 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	8-9-12	-1000	-1000	---	BACK	VERT	TOTAL	---	C1
M	11-0-4	-98	-98	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.47/1.00 (C-D:1) , BC=0.72/1.00 (G-K:1) , WB=0.22/1.00 (C-G:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.84 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006326

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	T36	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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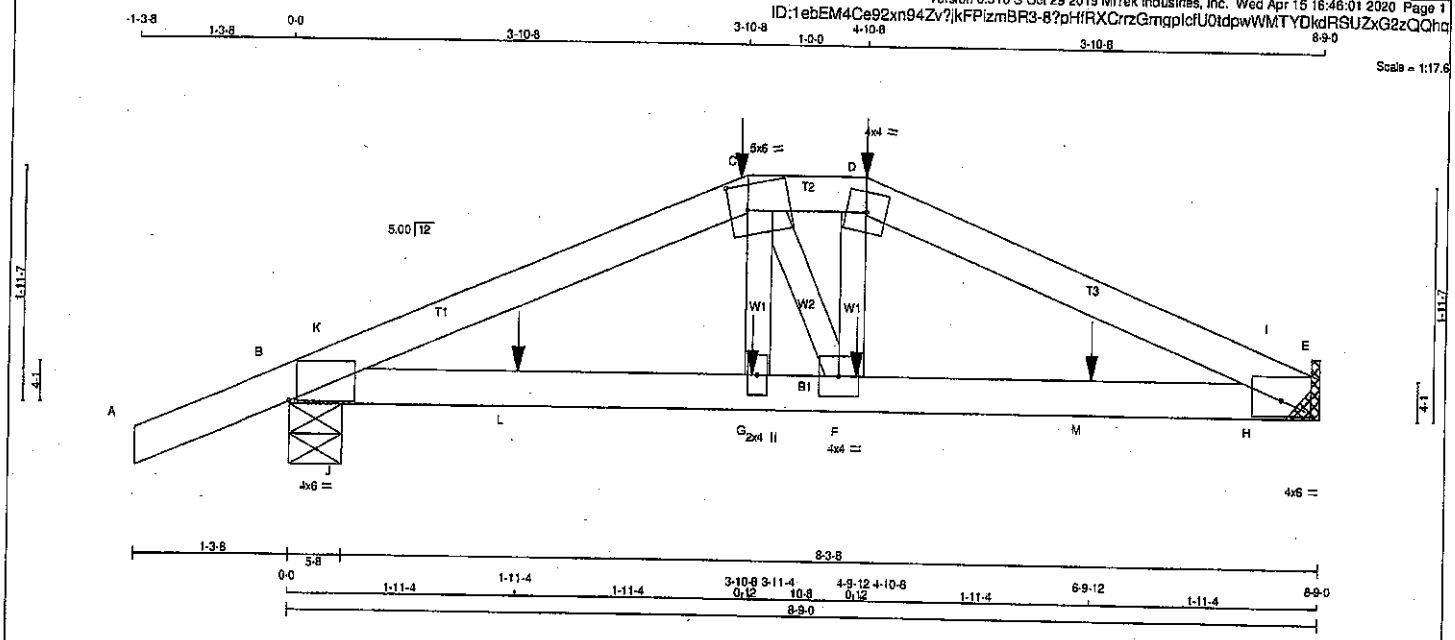

TOTAL WEIGHT = 26 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	6.0		0.75
C	TTWW-m	MT20	5.0	6.0	2.50	1.75
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	4.0	6.0	1.50	3.00
F	BMWW-t	MT20	4.0	4.0		
G	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	728	0	728	0	0	MECHANICAL	
B	851	0	851	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	514	340 / 0	0 / 0	0 / 0	0 / 0	175 0	0 0
B	589	409 / 0	0 / 0	0 / 0	0 / 0	191 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 23	-91.8	-91.8	0.13 (1)	10.00
B- K	-1327 / 0	-91.8	-91.8	0.06 (1)	5.54
K- C	-1320 / 0	-91.8	-91.8	0.21 (1)	5.36
C- D	-1224 / 0	-91.8	-91.8	0.04 (1)	5.75
D- I	-1322 / 0	-91.8	-91.8	0.21 (1)	5.36
I- E	-1328 / 0	-91.8	-91.8	0.06 (1)	5.54
B- J	0 / 1215	-18.5	-18.5	0.34 (1)	10.00
J- L	0 / 1215	-18.5	-18.5	0.34 (1)	10.00
L- G	0 / 1215	-18.5	-18.5	0.34 (1)	10.00
G- F	0 / 1222	-18.5	-18.5	0.27 (1)	10.00
F- M	0 / 1217	-18.5	-18.5	0.34 (1)	10.00
M- H	0 / 1217	-18.5	-18.5	0.34 (1)	10.00
H- E	0 / 1217	-18.5	-18.5	0.34 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 110	0.03 (4)
C- F	0 / 5	0.00 (4)
F- D	0 / 115	0.03 (4)
H- I	-161 / 16	0.00 (1)
J- K	-159 / 17	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-201	-201	---	BACK	VERT	TOTAL	--	C1
D	4-10-8	-201	-201	---	BACK	VERT	TOTAL	--	C1
F	4-9-12	-41	-41	---	BACK	VERT	TOTAL	--	C1
G	3-11-4	-41	-41	---	BACK	VERT	TOTAL	--	C1
L	1-11-4	-4	-5	---	BACK	VERT	TOTAL	--	C1
M	6-9-12	-4	-5	---	BACK	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.21/1.00 (C-K:1) , BC=0.34/1.00 (F-H:1) , WB=0.03/1.00 (D-F:4) , SSI=0.15/1.00 (E-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (D) (INPUT = 0.90)
JSI METAL= 0.29 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006327

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	T37	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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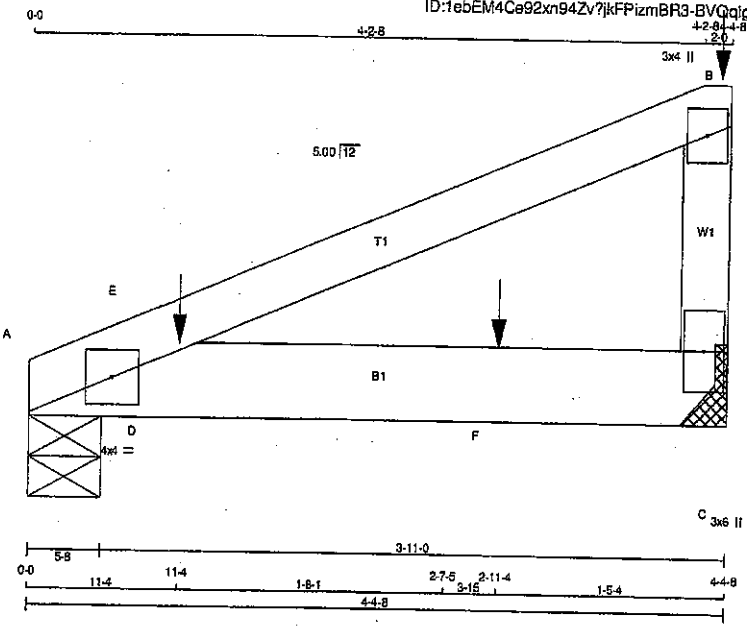

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMB1-I	MT20	4.0	4.0		
B	TMV+p	MT20	3.0	4.0		
C	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	683	0	683	0	0	5-8	5-8
C	1019	0	1019	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	479	339 / 0	0 / 0	0 / 0	0 / 0	140 / 0	0 / 0
C	714	504 / 0	0 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-E	-34 / 0	-91.8	-91.8	0.10 (1)		6.25
E-B	0 / 7	-91.8	-91.8	0.28 (1)		10.00
C-B	-439 / 0	0.0	0.0	0.05 (1)		7.81
A-D	-35 / 0	-18.5	-18.5	0.32 (1)		6.25
D-F	0 / 0	-18.5	-18.5	0.60 (1)		10.00
F-C	0 / 0	-18.5	-18.5	0.60 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D-E	-93 / 19	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	4-4-8	-255	-255	---	BACK	VERT	TOTAL	---	C1
D	11-4	-255	-255	---	TOP	VERT	TOTAL	---	C1
F	2-11-4	-709	-709	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G. ALVES
100009029
PROVINCE OF ONTARIO

TOTAL WEIGHT = 15 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 696 (0.08")

CSI: TC=0.28/1.00 (B-E:1) , BC=0.60/1.00 (C-D:1) , WB=0.00/1.00 (D-E:1) , SSI=0.38/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	766	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (A) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006328

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	T38	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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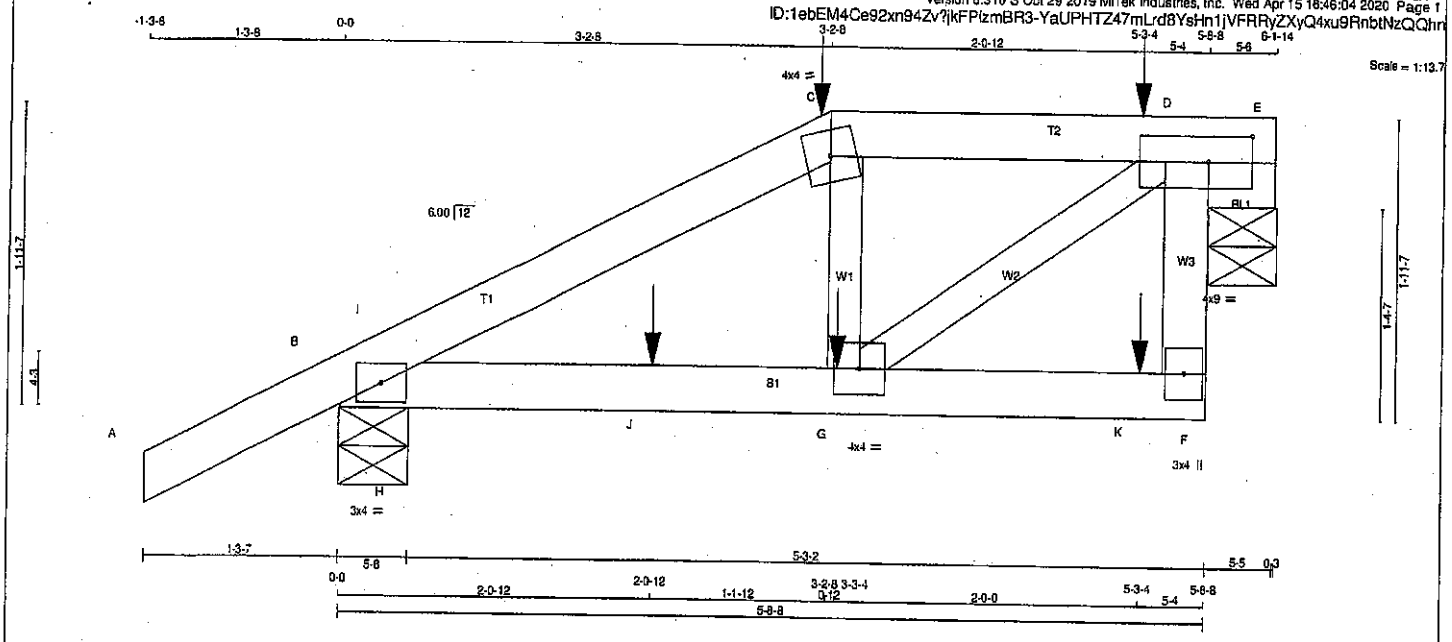

TOTAL WEIGHT = 21 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMVWK1-t	MT20	4.0	9.0	2.00	3.50
F	BMV+p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	530	0	530	0	0	5-8 (5-5)	5-8
B	505	0	505	0	0	5-8 (5-7)	5-8

(** SEE "BEARING NOTE" **)

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	375	248 / 0	0 / 0	0 / 0	0 / 0	127 / 0	0 / 0
B	355	246 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 27	-91.8	-91.8	0.13 (1)	10.00	G-C	-146 / 0	0.02 (1)
B-I	-449 / 0	-91.8	-91.8	0.02 (1)	6.25	G-D	0 / 489	0.12 (1)
I-C	-450 / 0	-91.8	-91.8	0.13 (1)	6.25	H-I	-147 / 3	0.00 (1)
C-D	-391 / 0	-91.8	-91.8	0.10 (1)	6.25			
D-E	0 / 0	-91.8	-91.8	0.01 (1)	10.00			
F-D	0 / 38	0.0	0.0	0.01 (4)	10.00			
B-H	0 / 400	-18.5	-18.5	0.16 (1)	10.00			
H-J	0 / 400	-18.5	-18.5	0.16 (1)	10.00			
J-G	0 / 400	-18.5	-18.5	0.16 (1)	10.00			
G-K	-3 / 0	-18.5	-18.5	0.05 (1)	10.00			
K-F	-3 / 0	-18.5	-18.5	0.05 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-2-8	-113	-113	---	BACK	VERT	TOTAL	---	C1
D	5-3-4	-40	-40	---	BACK	VERT	TOTAL	---	C1
G	3-3-4	-31	-31	---	TOP	VERT	TOTAL	---	C1
J	2-0-12	-19	-19	---	BACK	VERT	TOTAL	---	C1
K	5-3-4	-37	-37	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.16/1.00 (G-H:1) , WB=0.12/1.00 (D-G:1) , SSI=0.13/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (G) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H.J.G ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006329

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	T39	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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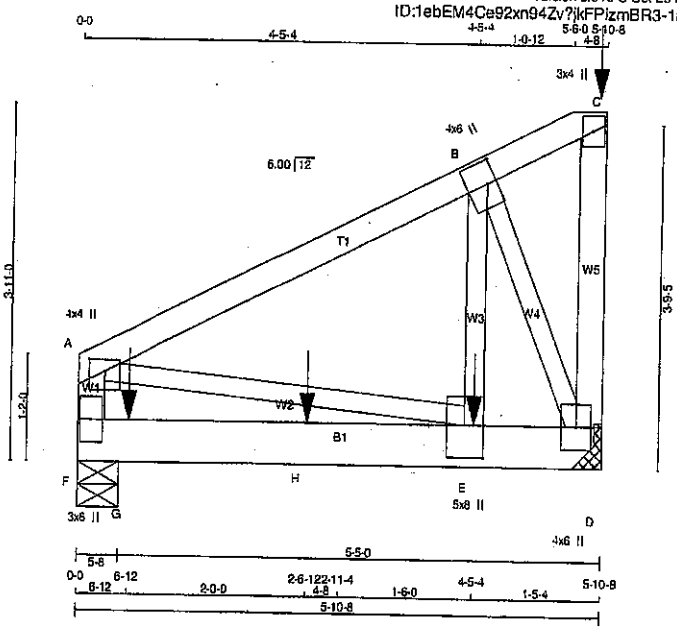

TOTAL WEIGHT = 2 X 30 = 61 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-D	1	12	TOP
F-A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F-D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B-E	1	2	SIDE(585.2)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (tablefs in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B	TMWW+l	MT20	4.0	6.0	3.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	BMWW1+p	MT20	4.0	6.0		
E	BMWW+l	MT20	5.0	8.0	4.25	2.50
F	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	2995	0	2995	0	0	MECHANICAL	
F	1442	0	1442	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	2105	1452 / 0	0 / 0	0 / 0	0 / 0	653 0	0 0
F	1012	709 / 0	0 / 0	0 / 0	0 / 0	303 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A-B	-1401 / 0	-91.8	-91.8	0.14 (1)	6.25	A-E	0 / 1284	0.16 (1)
B-C	-41 / 0	-91.8	-91.8	0.10 (1)	6.25	E-B	0 2876	0.36 (1)
D-C	-303 / 0	0.0	0.0	0.04 (1)	7.81	B-D	-2922 0	0.32 (1)
F-A	-1027 / 0	0.0	0.0	0.06 (1)	7.81			
F-G	0 / 0	-18.5	-18.5	0.19 (1)	10.00			
G-H	0 / 0	-18.5	-18.5	0.19 (1)	10.00			
H-E	0 / 0	-18.5	-18.5	0.19 (1)	10.00			
E-D	0 / 1265	-18.5	-18.5	0.11 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-328	-328	---	TOP	VERT	TOTAL	---	C1
E	4-5-4	-2868	-2868	---	BACK	VERT	TOTAL	---	C1
G	6-12	-242	-242	---	TOP	VERT	TOTAL	---	C1
H	2-6-12	-351	-351	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.19/1.00 (E-F:1) , WB=0.36/1.00 (B-E:1) , SSI=0.14/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.43 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006330

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	PB1	15	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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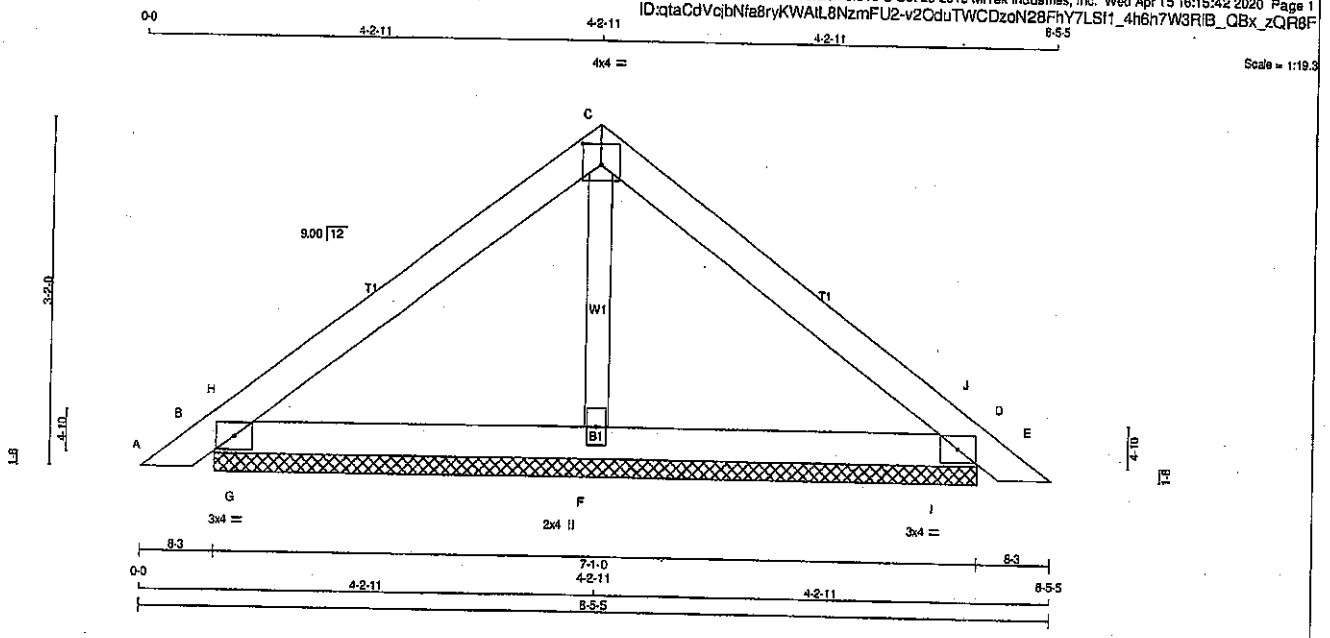

TOTAL WEIGHT = 15 X 22 = 331 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	304	0	304	0	0	7-1-0	7-1-0
D	304	0	304	0	0	7-1-0	7-1-0
F	281	0	281	0	0	7-1-0	7-1-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	213	153 / 0	0 / 0	0 / 0	0 / 0	60 . 0	0 / 0
D	213	153 / 0	0 / 0	0 / 0	0 / 0	60 . 0	0 / 0
F	201	118 / 0	0 / 0	0 / 0	0 / 0	83 . 0	0 . 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-91.8	-91.8	0.02 (1)	10.00
B-H	-28 / 0	-91.8	-91.8	0.07 (1)	6.25
H-C	-133 / 0	-91.8	-91.8	0.14 (1)	6.25
C-J	-133 / 0	-91.8	-91.8	0.14 (1)	6.25
J-D	-28 / 0	-91.8	-91.8	0.07 (1)	6.25
D-E	0 / 15	-91.8	-91.8	0.02 (1)	10.00
B-G	0 / 99	-18.5	-18.5	0.14 (1)	10.00
G-F	0 / 99	-18.5	-18.5	0.14 (1)	10.00
F-I	0 / 99	-18.5	-18.5	0.14 (1)	10.00
I-D	0 / 99	-18.5	-18.5	0.14 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	-121 / 0	0.02 (1)
G-H	-343 / 0	0.00 (1)
I-J	-343 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (C-H:1) , BC=0.14/1.00 (B-G:1) , WB=0.02/1.00 (C-F:1) , SSI=0.26/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (D) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. D. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006268

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	PB1G	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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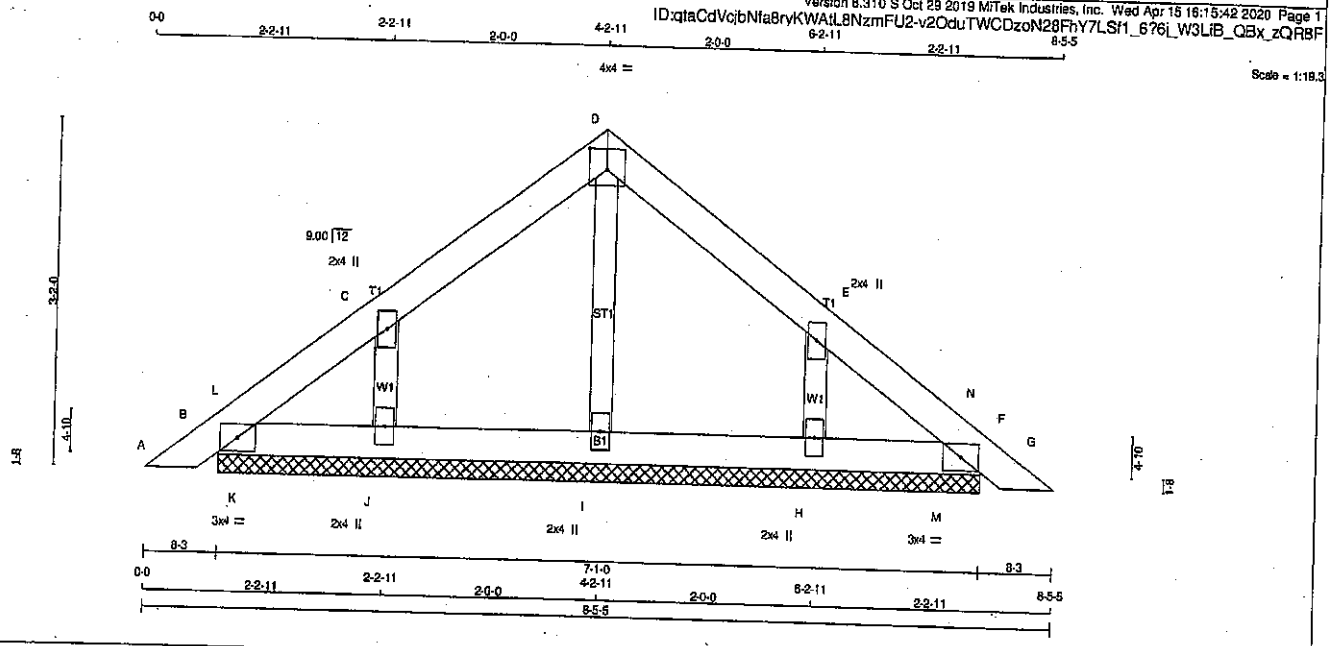

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMB1-I	MT20	3.0	4.0		
H, I, J						
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-91.8	-91.8	0.02 (1)	10.00
B-L	-65 / 0	-91.8	-91.8	0.00 (1)	6.25
L-C	-31 / 0	-91.8	-91.8	0.05 (1)	6.25
C-D	-44 / 0	-91.8	-91.8	0.05 (1)	6.25
D-E	-44 / 0	-91.8	-91.8	0.05 (1)	6.25
E-N	-31 / 0	-91.8	-91.8	0.05 (1)	6.25
N-F	-65 / 0	-91.8	-91.8	0.00 (1)	6.25
F-G	0 / 15	-91.8	-91.8	0.02 (1)	10.00
B-K	0 / 41	-18.5	-18.5	0.01 (1)	10.00
K-J	0 / 41	-18.5	-18.5	0.02 (1)	10.00
J-I	0 / 28	-18.5	-18.5	0.02 (4)	10.00
I-H	0 / 26	-18.5	-18.5	0.02 (4)	10.00
H-M	0 / 41	-18.5	-18.5	0.02 (1)	10.00
M-F	0 / 41	-18.5	-18.5	0.01 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-D	-116 / 0	0.02 (1)
J-C	-192 / 0	0.03 (1)
H-E	-192 / 0	0.03 (1)
K-L	-12 / 4	0.00 (1)
M-N	-12 / 4	0.00 (1)

TOTAL WEIGHT = 2 X 24 = 47 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (C-D:1) , BC=0.02/1.00 (I-J:4) , WB=0.03/1.00 (C-J:1) , SSI=0.07/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.10 (E) (INPUT = 1.00)

Structural component only
DWG# T-2006269

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	PB1Z	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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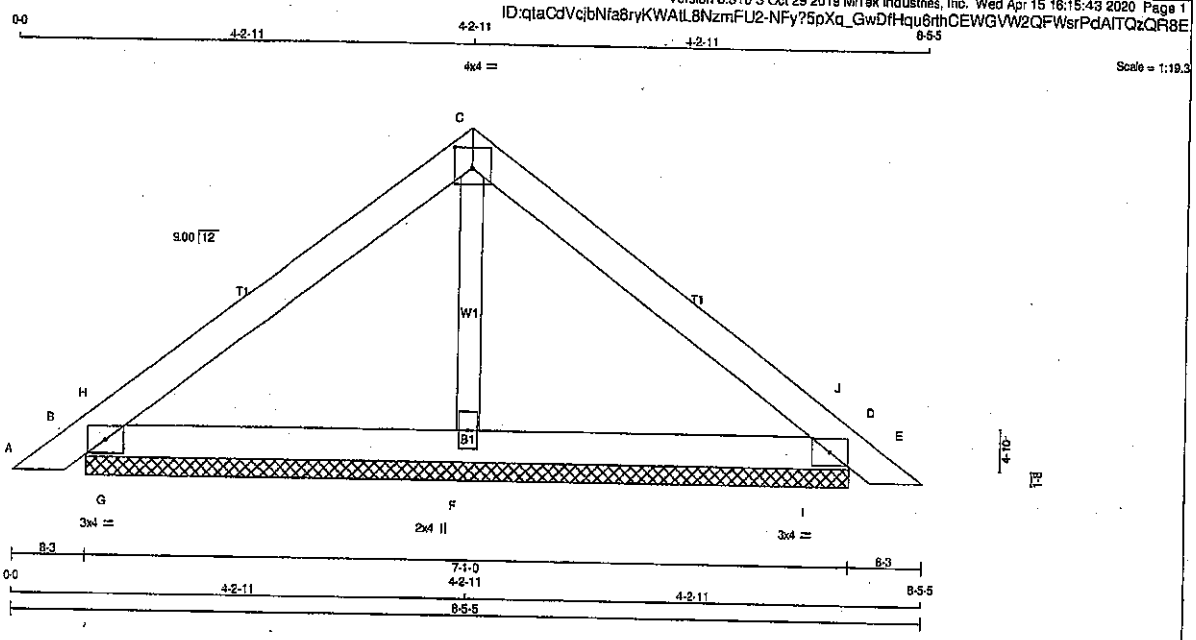

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-E	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B-D	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION UPLIFT	INPUT BRG	REQRD BRG
B	305	0	305	0	0	7-1-0	7-1-0
D	305	0	305	0	0	7-1-0	7-1-0
F	279	0	279	0	0	7-1-0	7-1-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	213	153 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
D	213	153 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
F	200	117 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-91.8	-91.8	0.01 (1)		10.00
B-H	-28 / 0	-91.8	-91.8	0.03 (1)		6.25
H-C	-134 / 0	-91.8	-91.8	0.07 (1)		6.25
C-J	-134 / 0	-91.8	-91.8	0.07 (1)		6.25
J-D	-28 / 0	-91.8	-91.8	0.03 (1)		6.25
D-E	0 / 15	-91.8	-91.8	0.01 (1)		10.00
B-G	0 / 100	-18.5	-18.5	0.07 (1)		10.00
G-F	0 / 100	-18.5	-18.5	0.07 (1)		10.00
F-I	0 / 100	-18.5	-18.5	0.07 (1)		10.00
I-D	0 / 100	-18.5	-18.5	0.07 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	-120 / 0	0.01 (1)
G-H	-338 / 0	0.00 (1)
I-J	-338 / 0	0.00 (1)

TOTAL WEIGHT = 2 X 22 = 44 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07/1.00 (C-J:1) , BC=0.07/1.00 (D-I:1) , WB=0.01/1.00 (C-F:1) , SSI=0.13/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (D) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006270

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408129	PB11	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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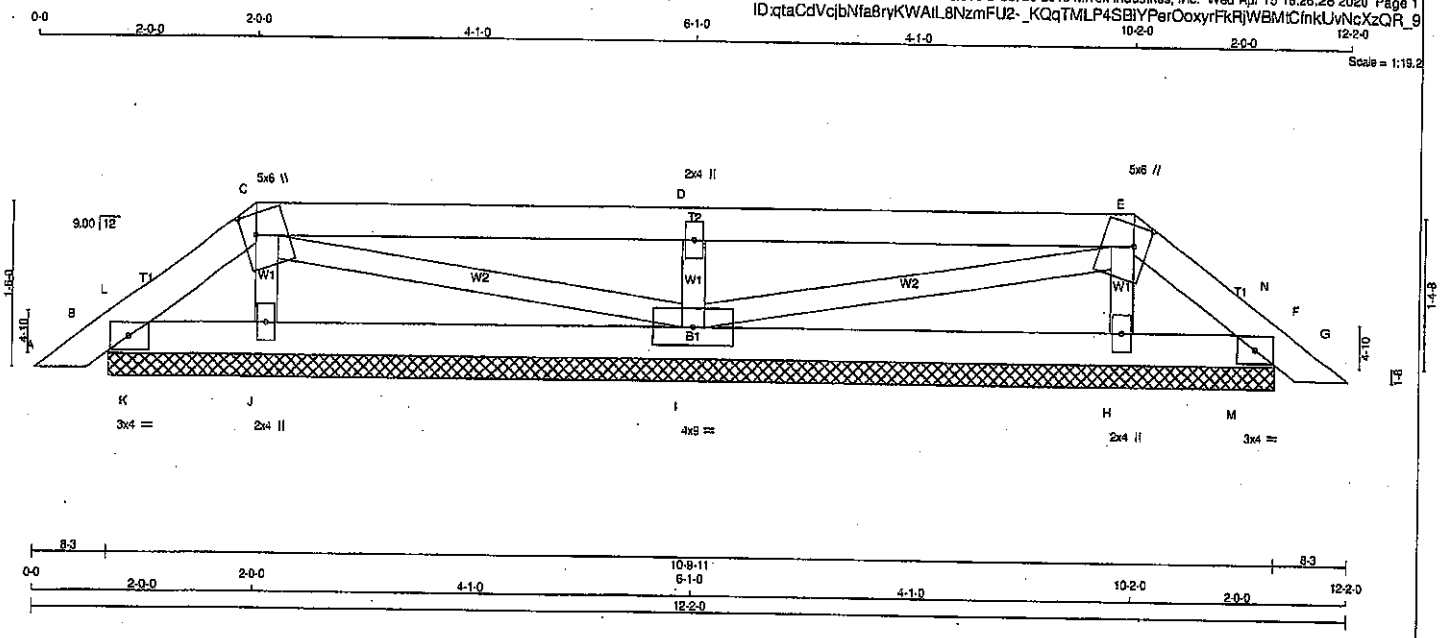


TOTAL WEIGHT = 2 X 36 = 71 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMB1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-t	MT20	4.0	9.0		
J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	140	0	140	0	0	10-9-11	10-9-11
F	140	0	140	0	0	10-9-11	10-9-11
J	235	0	235	0	0	10-9-11	10-9-11
I	550	0	550	0	0	10-9-11	10-9-11
H	235	0	235	0	0	10-9-11	10-9-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	96	78 / 0	0 / 0	0 / 0	0 / 0	18 0	0 0
F	96	78 / 0	0 / 0	0 / 0	0 / 0	18 0	0 0
J	169	97 / 0	0 / 0	0 / 0	0 / 0	72 0	0 0
I	387	263 / 0	0 / 0	0 / 0	0 / 0	124 0	0 0
H	169	97 / 0	0 / 0	0 / 0	0 / 0	72 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

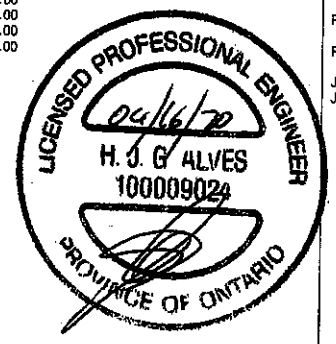
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 15	-91.8	-91.8	0.02 (1)	10.00	J-C	-159 / 0		0.02 (1)
B-L	-33 / 0	-91.8	-91.8	0.01 (4)	6.25	C-I	-12 / 0		0.00 (1)
L-C	-42 / 0	-91.8	-91.8	0.01 (1)	6.25	I-D	-468 / 0		0.07 (1)
C-D	-4 / 0	-91.8	-91.8	0.26 (1)	10.00	I-E	-12 / 0		0.00 (1)
D-E	-4 / 0	-91.8	-91.8	0.26 (1)	10.00	H-E	-159 / 0		0.02 (1)
E-N	-42 / 0	-91.8	-91.8	0.01 (1)	6.25	K-L	-73 / 0		0.00 (1)
N-F	-33 / 0	-91.8	-91.8	0.01 (4)	6.25	M-N	-73 / 0		0.00 (1)
F-G	0 / 15	-91.8	-91.8	0.02 (1)	10.00				
B-K	0 / 31	-18.5	-18.5	0.03 (1)	10.00				
K-J	0 / 31	-18.5	-18.5	0.05 (4)	10.00				
J-I	0 / 16	-18.5	-18.5	0.08 (4)	10.00				
I-H	0 / 16	-18.5	-18.5	0.06 (4)	10.00				
H-M	0 / 31	-18.5	-18.5	0.05 (4)	10.00				
M-F	0 / 31	-18.5	-18.5	0.03 (1)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.26/1.00 (D-E:1) , BC=0.06/1.00 (I-J:4) , WB=0.07/1.00 (D-I:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

Structural component only
DWG# T-2006287

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408129	PB12	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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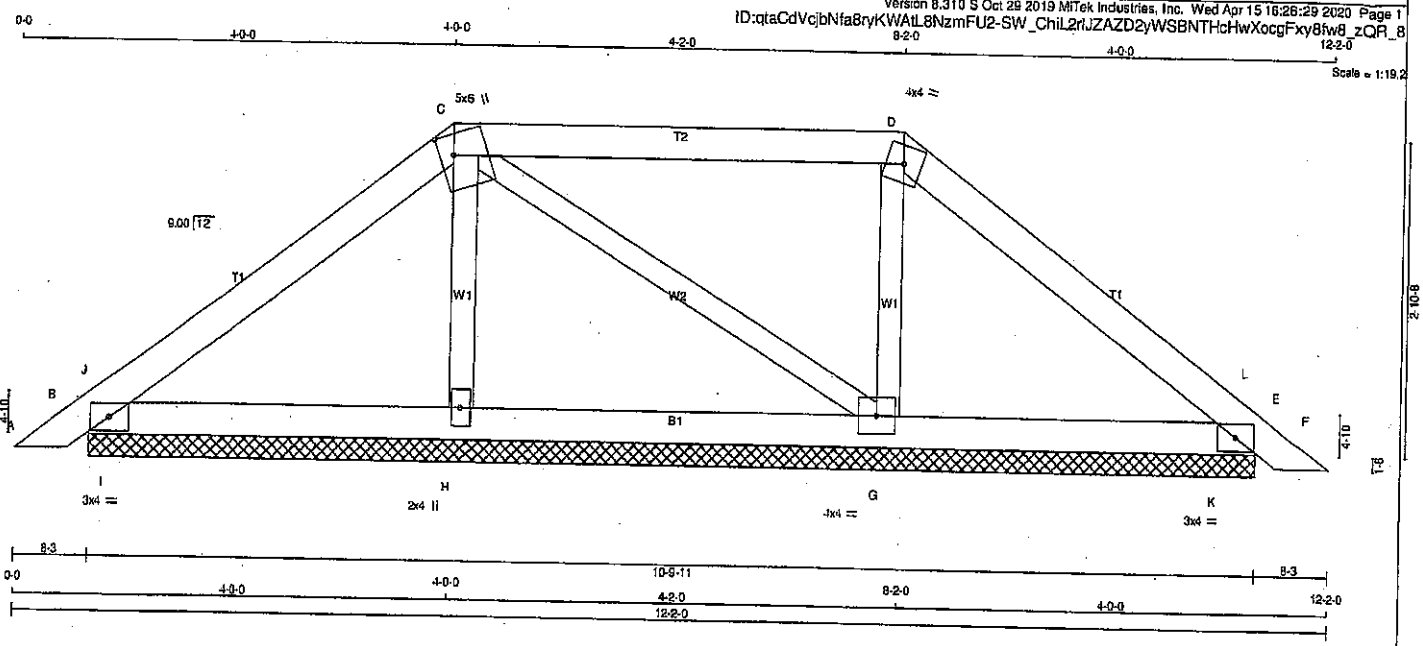

TOTAL WEIGHT = 2 X 36 = 72 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	8.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	311	0	311	0	0	10-9-11	10-9-11
E	290	0	290	0	0	10-9-11	10-9-11
H	321	0	321	0	0	10-9-11	10-9-11
G	378	0	378	0	0	10-9-11	10-9-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	217	158 / 0	0 / 0	0 / 0	0 / 0	59 0	0 0
E	203	146 / 0	0 / 0	0 / 0	0 0	57 0	0 0
H	229	139 / 0	0 / 0	0 / 0	0 0	90 0	0 0
G	268	171 / 0	0 / 0	0 / 0	0 0	97 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-91.8	-91.8	0.02 (1)	10.00
B-J	-60 / 0	-91.8	-91.8	0.05 (1)	6.25
J-C	-142 / 0	-91.8	-91.8	0.12 (1)	6.25
C-D	-70 / 0	-91.8	-91.8	0.27 (1)	6.25
D-L	-109 / 0	-91.8	-91.8	0.12 (1)	6.25
L-E	-27 / 0	-91.8	-91.8	0.05 (1)	6.25
E-F	0 / 15	-91.8	-91.8	0.02 (1)	10.00
B-I	0 / 107	-18.5	-18.5	0.12 (1)	10.00
I-H	0 107	-18.5	-18.5	0.12 (1)	10.00
H-G	0 / 98	-18.5	-18.5	0.07 (1)	10.00
G-K	0 / 81	-18.5	-18.5	0.11 (1)	10.00
K-E	0 81	-18.5	-18.5	0.11 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-C	-221 / 0	0.04 (1)
C-G	-33 / 0	0.01 (1)
G-D	-259 / 0	0.04 (1)
I-J	-270 / 0	0.00 (1)
K-L	-274 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.27/1.00 (C-D:1) , BC=0.12/1.00 (B-I:1) , WB=0.04/1.00 (D-G:1) , SSI=0.21/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006288

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408129	PB13	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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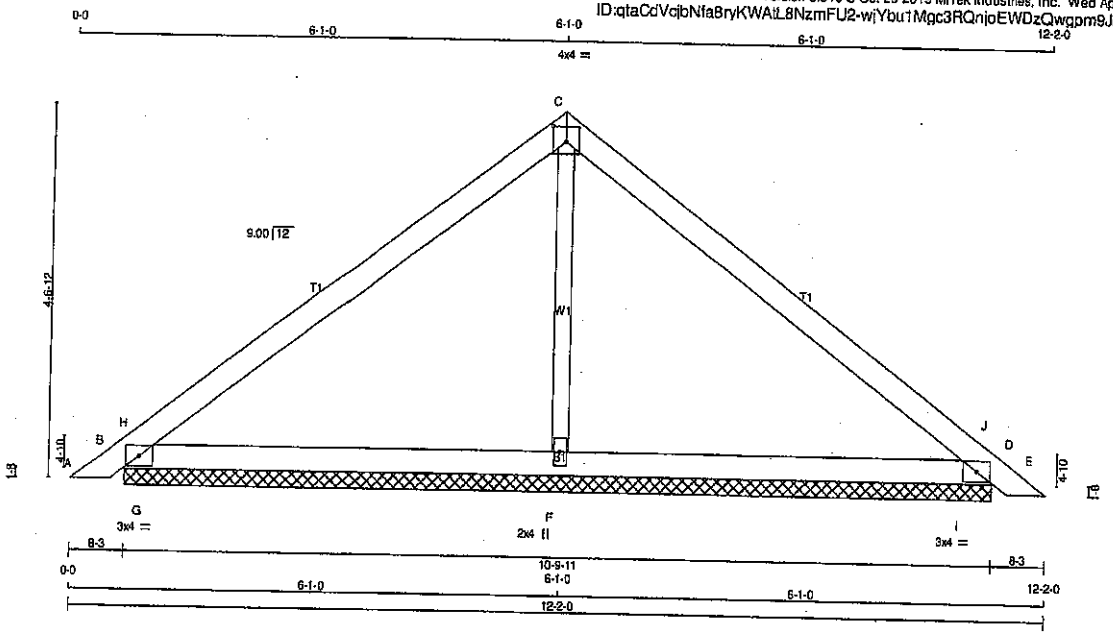


TOTAL WEIGHT = 3 X 33 = 98 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	467	0	467	0	0	10-9-11	10-9-11
D	467	0	467	0	0	10-9-11	10-9-11
F	367	0	367	0	0	10-9-11	10-9-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	327	232 / 0	0 / 0	0 / 0	0 0	95 0	0 0
D	327	232 / 0	0 / 0	0 / 0	0 0	95 0	0 0
F	264	149 / 0	0 / 0	0 / 0	0 0	115 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

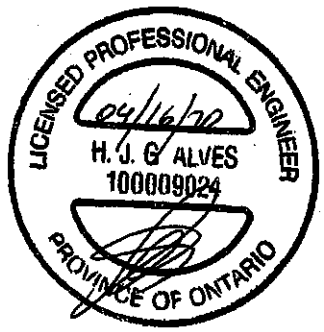
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-91.8	-91.8	0.02 (1)	10.00
B-H	-42 / 97	-91.8	-91.8	0.21 (1)	6.25
H-C	-256 / 0	-91.8	-91.8	0.33 (1)	6.25
C-J	-256 / 0	-91.8	-91.8	0.33 (1)	6.25
J-D	-42 / 97	-91.8	-91.8	0.21 (1)	6.25
D-E	0 / 15	-91.8	-91.8	0.02 (1)	10.00
B-G	0 / 192	-18.5	-18.5	0.30 (1)	10.00
G-F	0 192	-18.5	-18.5	0.30 (1)	10.00
F-I	0 192	-18.5	-18.5	0.30 (1)	10.00
I-D	0 / 192	-18.5	-18.5	0.30 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	-128 / 0	0.04 (1)
G-H	-759 / 0	0.00 (1)
I-J	-759 / 0	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.33/1.00 (C-J:1) , BC=0.30/1.00 (B-G:1) , WB=0.04/1.00 (C-F:1) , SSI=0.58/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (B) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)

Structural component only
DWG# T-2006289

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408129	PB14	4	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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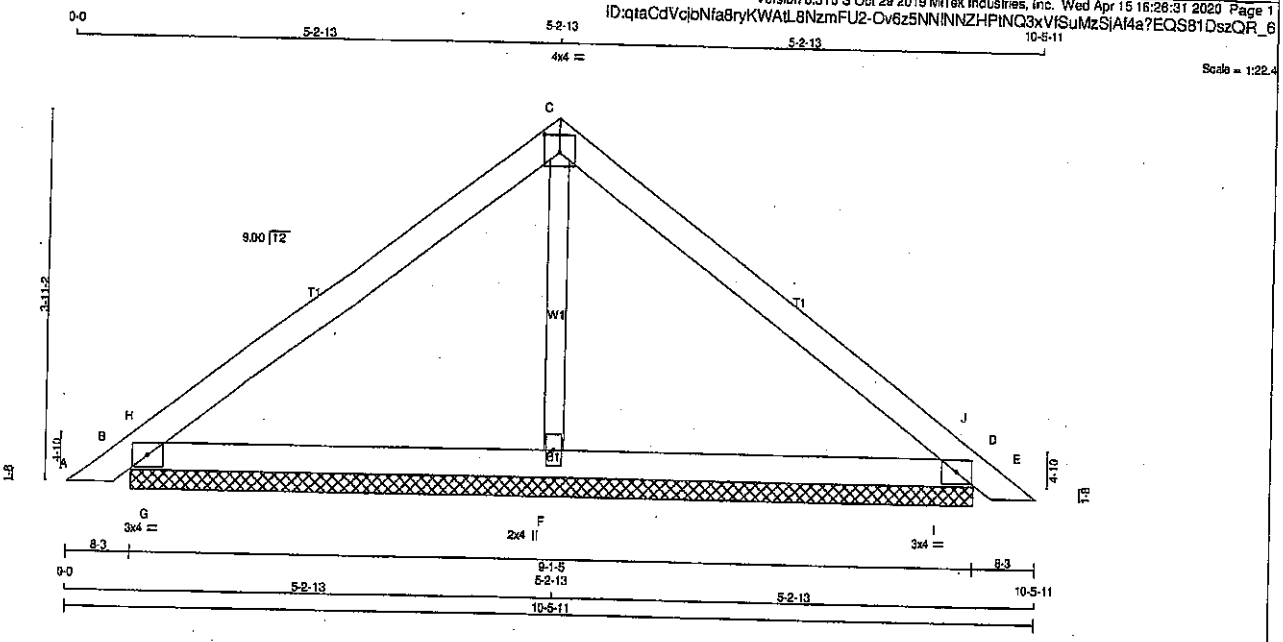

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	390	0	390	0	0	9-1-5	9-1-5
D	390	0	390	0	0	9-1-5	9-1-5
F	333	0	333	0	0	9-1-5	9-1-5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	273	195 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0
D	273	195 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0
F	239	137 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-91.8	-91.8	0.02 (1)	10.00
B- H	-36 / 39	-91.8	-91.8	0.13 (1)	6.25
H- C	-196 / 0	-91.8	-91.8	0.23 (1)	6.25
C- J	-196 / 0	-91.8	-91.8	0.23 (1)	6.25
J- D	-36 / 39	-91.8	-91.8	0.13 (1)	6.25
D- E	0 / 15	-91.8	-91.8	0.02 (1)	10.00
B- G	0 / 146	-18.5	-18.5	0.22 (1)	10.00
G- F	0 / 146	-18.5	-18.5	0.22 (1)	10.00
F- I	0 / 146	-18.5	-18.5	0.22 (1)	10.00
I- D	0 / 146	-18.5	-18.5	0.22 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-130 / 0	0.03 (1)
G- H	-549 / 0	0.00 (1)
I- J	-549 / 0	0.00 (1)

TOTAL WEIGHT = 4 X 28 = 112 lb

DESIGN CRITERIA [M]

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (C-J:1) , BC=0.22/1.00 (D-I:1) , WB=0.03/1.00 (C-F:1) , SSI=0.42/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (B) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006290

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	J1	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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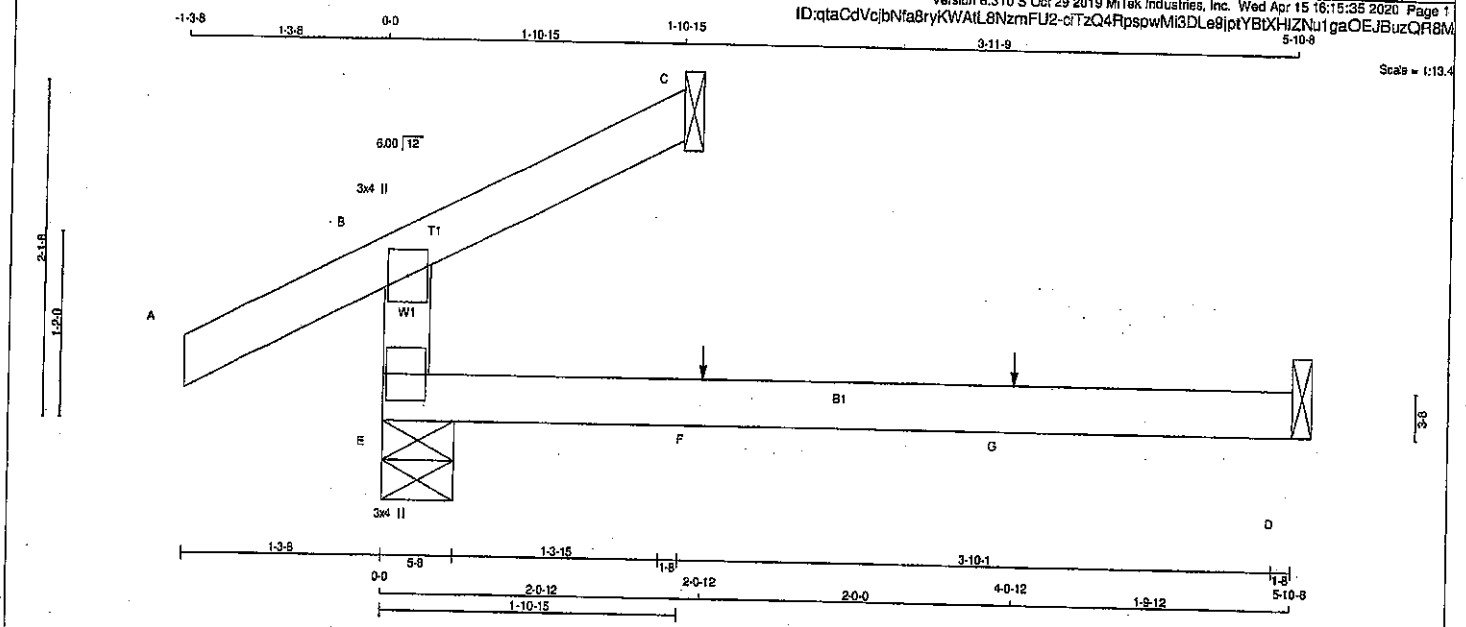


TOTAL WEIGHT = 2 X 12 = 23 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	297	0	297	0	0	5-8	5-8
C	66	0	66	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	212	130 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
C	46	37 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

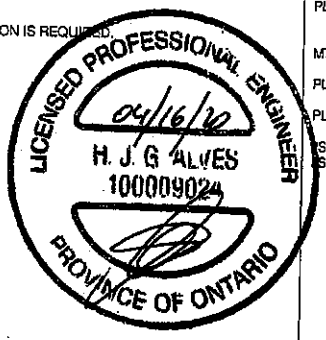
CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-234 / 0	0.0	0.0	0.13 (4)	7.81			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-10 / 0	-91.8	-91.8	0.06 (1)	10.00			
E-F	0 / 0	-18.5	-18.5	0.13 (4)	10.00			
F-G	0 / 0	-18.5	-18.5	0.13 (4)	10.00			
G-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	1	1	---	BACK	VERT	TOTAL	---	C1
G	4-0-12	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



DESIGN CRITERIA [M]

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2015 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.13/1.00 (B-E:4) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.10 (E) (INPUT = 0.90)
SI METAL= 0.07 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006258

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	J1A	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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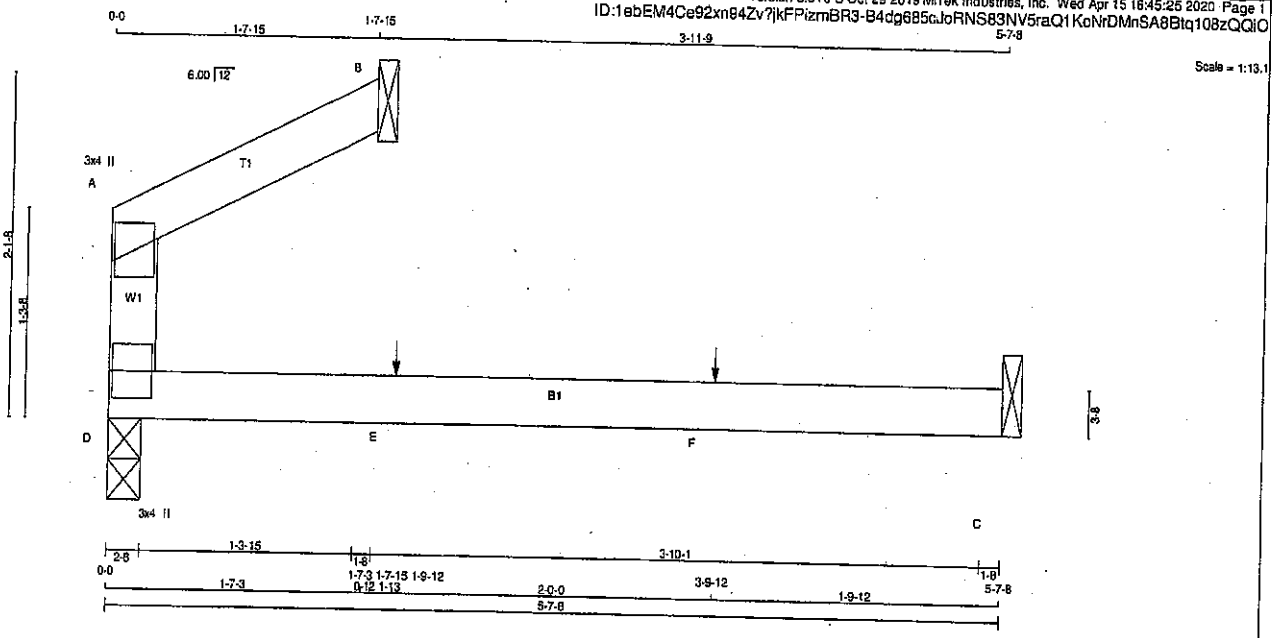

TOTAL WEIGHT = 10 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	114	0	114	0	0	2-8	2-8
B	97	0	97	0	0	1-8	1-8
C	46	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	82	44 / 0	0 / 0	0 0	0 0	39 0	0 0
B	69	41 / 0	0 / 0	0 / 0	0 / 0	29 0	0 / 0
C	37	1 / 0	0 / 0	0 / 0	0 / 0	36 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B .

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D-A	-56 / 12	0.0	0.0	0.09 (4)		7.81			
A-B	0 / 12	-91.8	-91.8	0.09 (4)		10.00			
D-E	0 / 0	-18.5	-18.5	0.13 (4)		10.00			
E-F	0 / 0	-18.5	-18.5	0.13 (4)		10.00			
F-C	0 / 0	-18.5	-18.5	0.13 (4)		10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-9-12	1	1	---	FRONT	VERT	TOTAL	---	C1
F	3-9-12	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.09/1.00 (A-D:4) , BC=0.13/1.00 (C-D:4) , WB=0.00/1.00 (n/a:0) , SSI=0.08/1.00 (C-D:4)

DOL LUMBER=0.87 NAIL=0.87 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.03 (D) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 1.00)

Structural component only
DWG# T-2006301

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	J2	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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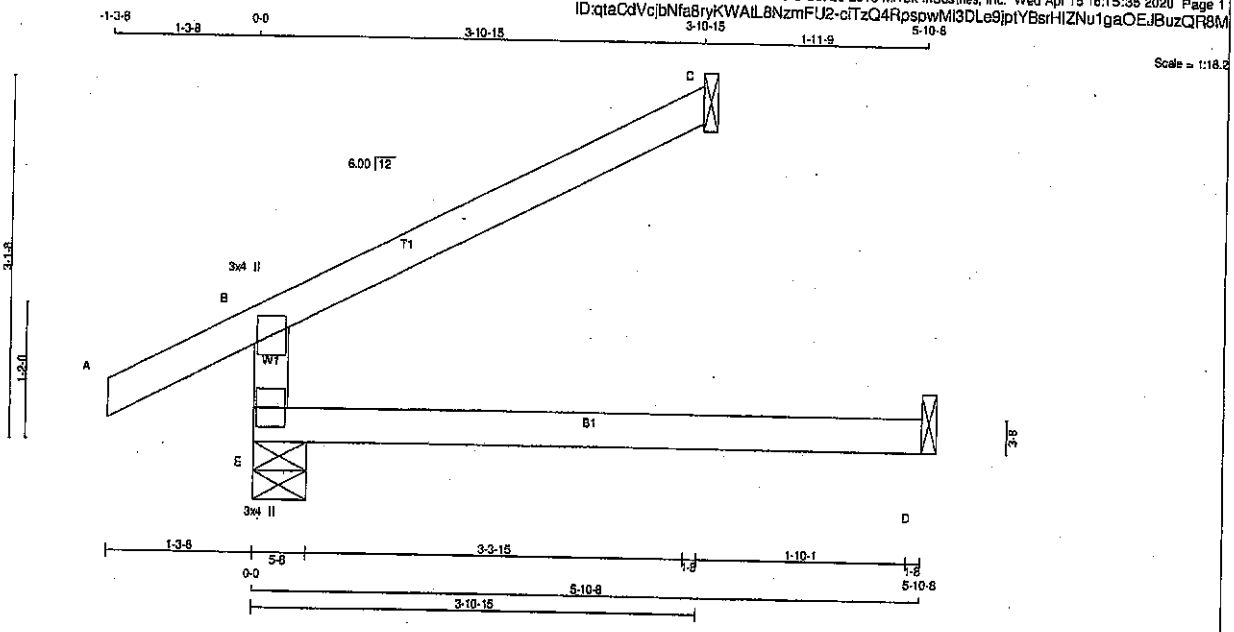

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	412	0	412	0	0	5-8	5-8
C	135	0	135	0	0	1-8	1-8
D	45	0	60	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	291	194 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
C	93	75 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	MAX. UNBRAC LENGTH
E-B	-349 / 0	0.0	0.0		0.13 (4)				7.81
A-B	0 / 28	-91.8	-91.8		0.12 (1)				10.00
B-C	-20 / 0	-91.8	-91.8		0.24 (1)				6.25
E-D	0 / 0	-18.5	-18.5		0.13 (4)				10.00

TOTAL WEIGHT = 2 X 14 = 29 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006259

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408130	J2A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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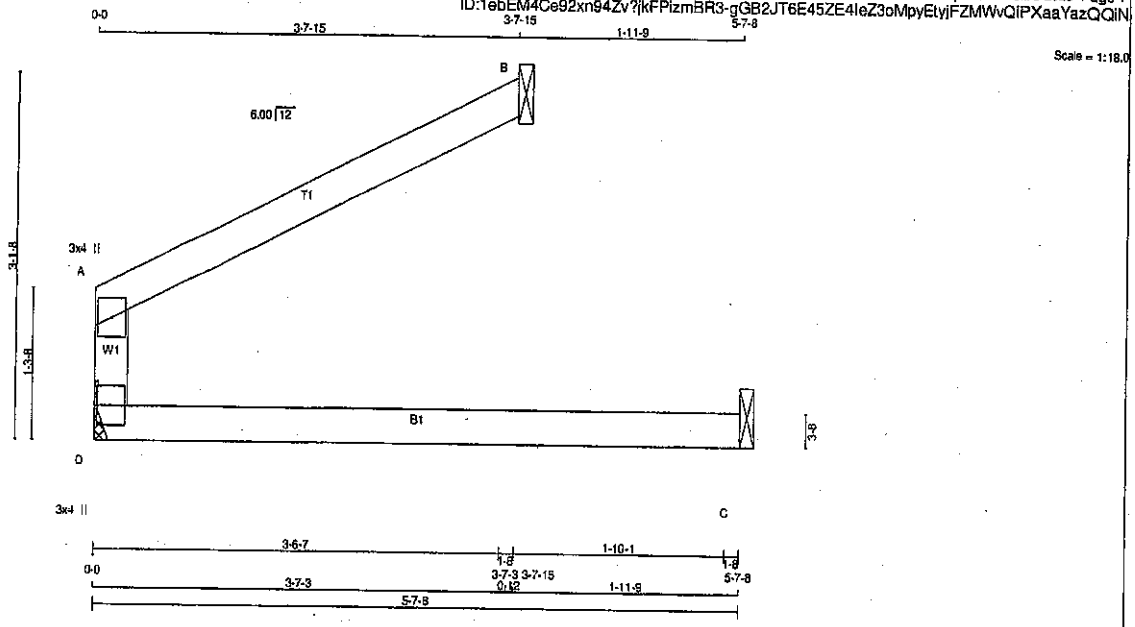


TOTAL WEIGHT = 12 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	222	0	222	0	0	MECHANICAL	
B	163	0	-163	0	0	1-8	1-8
C	55	0	55	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	158	96 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
B	113	86 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
C	43	5 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- A	-173 / 0	0.0	0.0	0.05 (4)	7.81			
A- B	-2 / 3	-91.8	-91.8	0.18 (1)	10.00			
D- C	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.18/1.00 (A-B:1) , BC=0.14/1.00 (C-D:4) , WB=0.00/1.00 (n/a:0) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

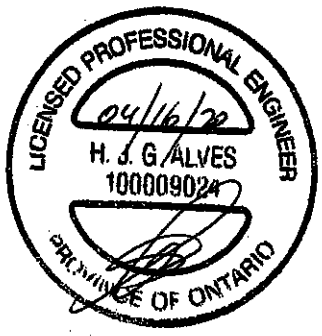
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.07 (D) (INPUT = 0.90)
JSI METAL= 0.05 (A) (INPUT = 1.00)



Structural component only
DWG# T-2006302

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	J3	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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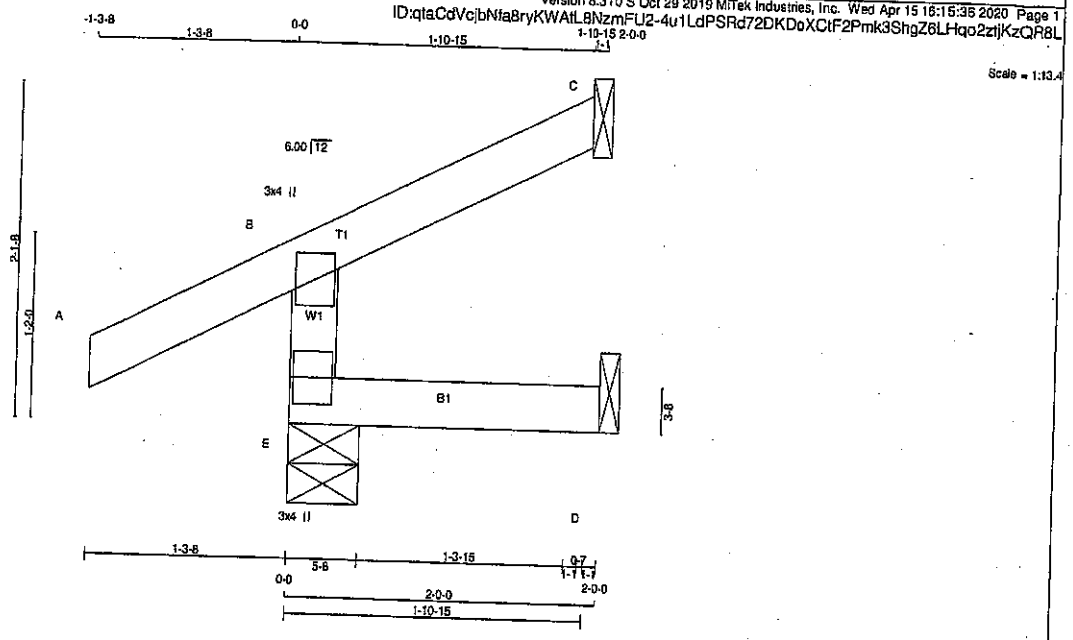


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	254	0	254	0	0	5-8	5-8
C	66	0	66	0	0	1-8	1-8
D	16	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	177	130 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	46	37 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

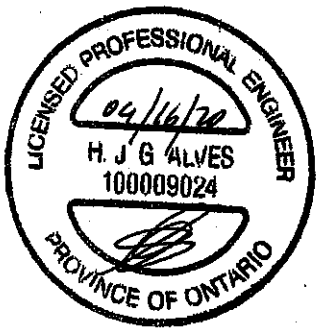
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-234 / 0	0.0	0.0	0.01 (4)	7.81			
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B- C	-10 / 0	-91.8	-91.8	0.06 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



TOTAL WEIGHT = 2 X 7 = 15 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006260

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	J4	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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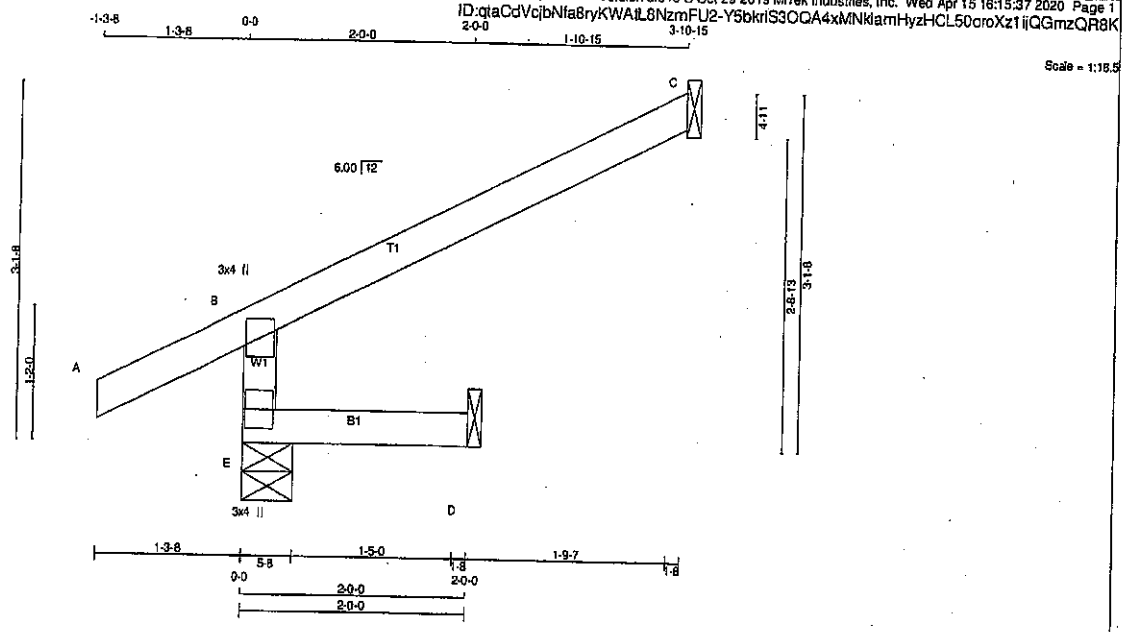


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	369	0	369	0	0	5-8	5-8
C	135	0	135	0	0	1-8	1-8
D	16	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	256	194 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	93	75 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

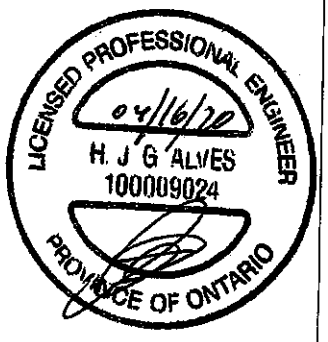
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
E-B	-349 / 0	0.0	0.0		0.01 (4)	7.81			
A-B	0 / 28	-91.8	-91.8		0.13 (5)	10.00			
B-C	-20 / 0	-91.8	-91.8		0.24 (1)	6.25			
E-D	0 / 0	-18.5	-18.5		0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



TOTAL WEIGHT = 2 X 10 = 20 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006261

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	J5	11	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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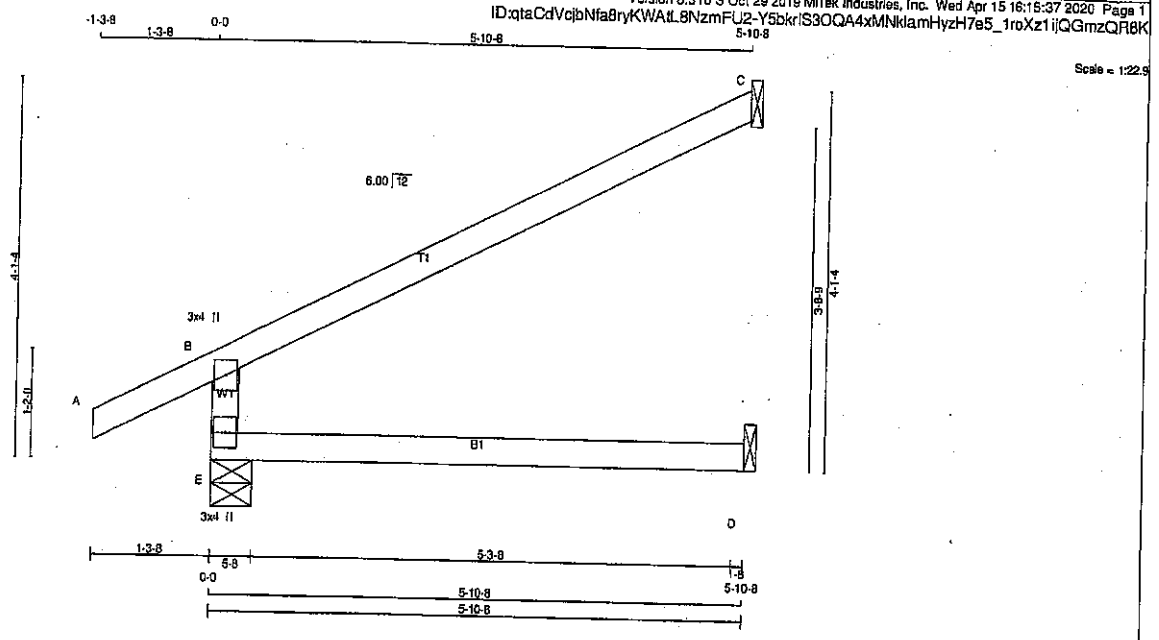

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	525	0	525	0	0	5-8	5-8
C	202	0	202	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	369	257 / 0	0 / 0	0 / 0	0 / 0	111 0	0 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-461 / 0	0.0	0.0	0.13 (4)	7.81			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-30 / 0	-91.8	-91.8	0.54 (1)	6.25			
E-D	0 0	-18.5	-18.5	0.13 (4)	10.00			

TOTAL WEIGHT = 11 X 17 = 185 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006262

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	J5A	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Wed Apr 15 16:45:28 2020 Page 1
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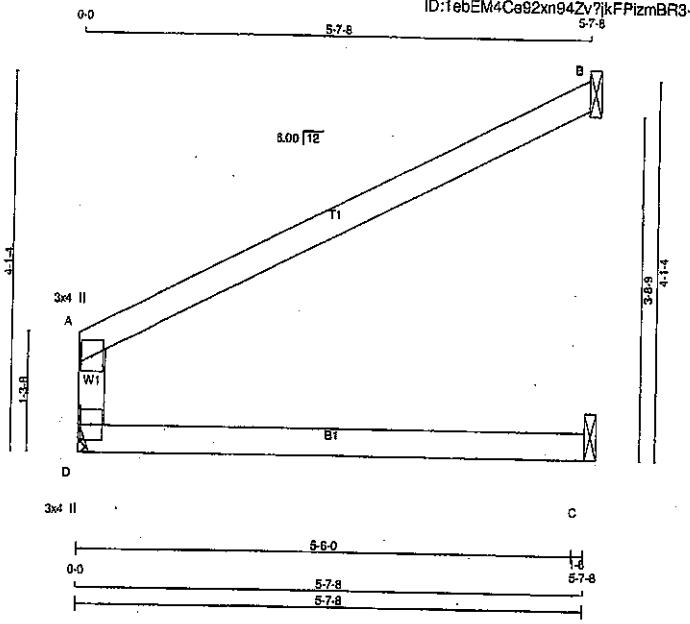

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	310	0	310	0	0	MECHANICAL	
B	236	0	236	0	0	1-8	1-8
C	74	0	74	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	219	144 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0
B	163	129 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0
C	56	15 / 0	0 / 0	0 / 0	0 / 0	41 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
D- A	-280 / 0	0.0	0.0	0.15 (1)	7.81
A- B	-10 / 0	-91.8	-91.8	0.41 (1)	6.25
D- C	0 / 0	-18.5	-18.5	0.20 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

TOTAL WEIGHT = 3 X 15 = 44 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 691 (0.10")

CSI: TC=0.41/1.00 (A-B:1) , BC=0.20/1.00 (C-D:1) , WB=0.00/1.00 (n/a:0) , SSI=0.20/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (D) (INPUT = 0.90)
JSI METAL= 0.08 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G. ALVES
100090247
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006303

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	J6	7	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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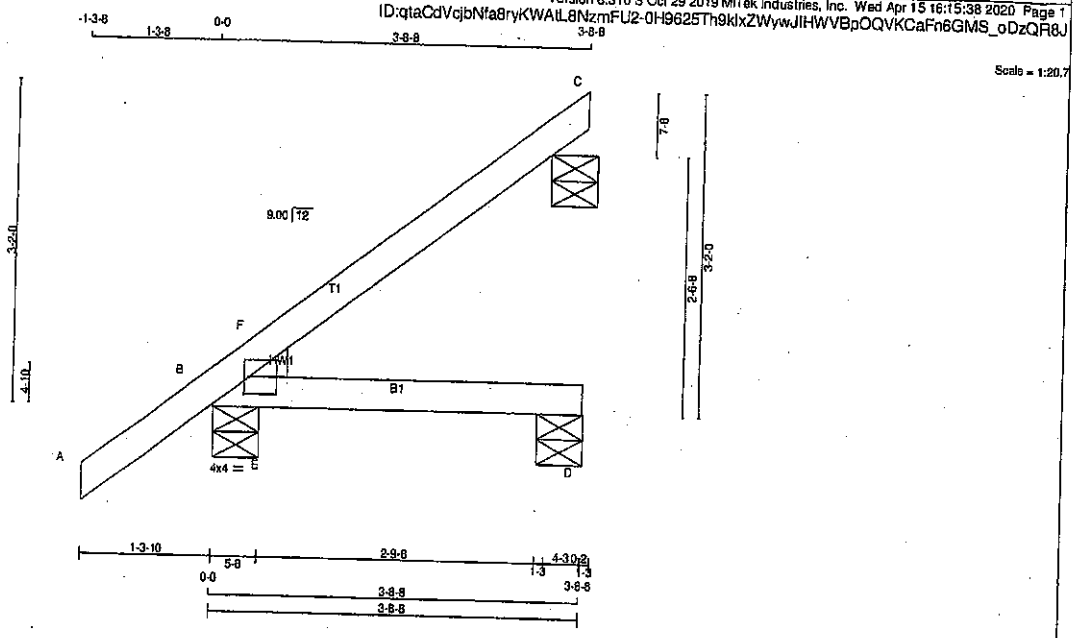


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	4.0	2.00	0.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 7 X 12 = 84 lb [M]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
C	144	0	144	0	0	5-8 (4-3)	5-8	
B	331	0	331	0	0	5-8	5-8	2x4 L
D	61	0	61	0	0	5-8 (5-6)	5-8	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	100	77 / 0	0 / 0	0 / 0	0 / 0	22 / 0	0 / 0
B	232	185 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0
D	45	17 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 97	-91.8	-91.8	6	0.14 (6)	10.00
B-F	-17 / 2	-91.8	-91.8	4	0.03 (4)	6.25
F-C	0 / 3	-91.8	-91.8	1	0.15 (1)	10.00
B-E	0 / 0	-18.5	-18.5	1	0.13 (1)	10.00
E-D	0 / 0	-18.5	-18.5	1	0.13 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-F	-173 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.15/1.00 (C-F:1) , BC=0.13/1.00 (B-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.14/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

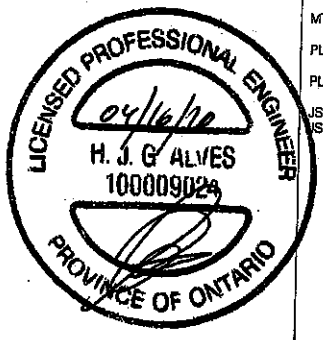
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



Structural component only
DWG# T-2006263

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	J7	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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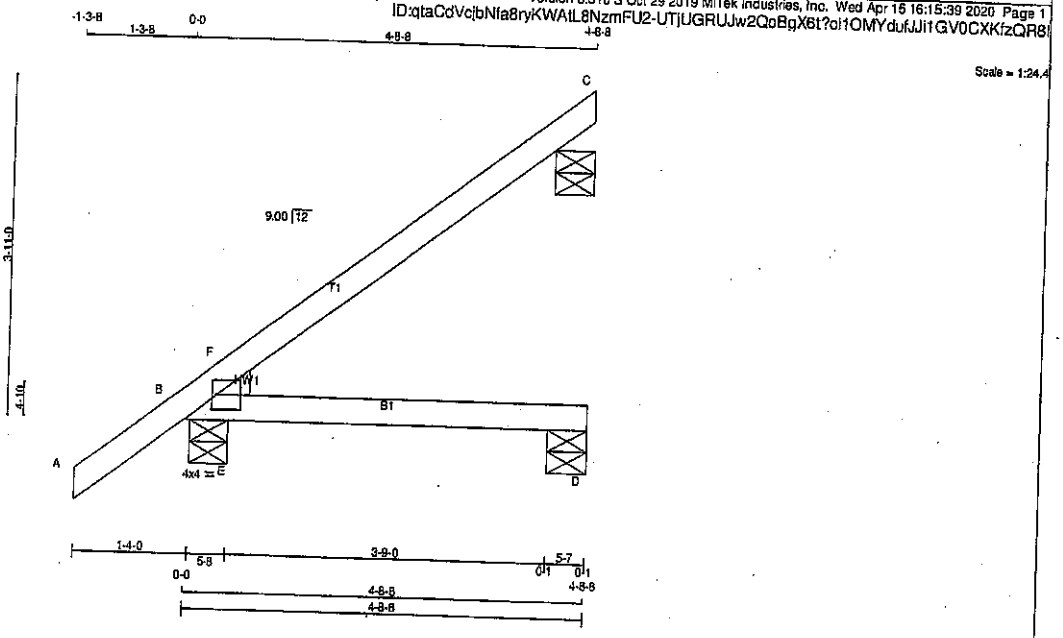

TOTAL WEIGHT = 3 X 15 = 44 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	4.0	2.00	0.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
C	185	0	185	0	0	5-8 (5-6)	5-8	
B	386	0	386	0	0	5-8	5-8	2x4 L
D	75	0	75	0	0	5-8	5-8	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	128	100 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0
B	271	191 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0
D	56	21 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 37	-91.8	-91.8	0.12 (1)	10.00
B-F	-22 / 27	-91.8	-91.8	0.04 (4)	5.25
F-C	-3 / 3	-91.8	-91.8	0.25 (1)	10.00
B-E	0 / 0	-18.5	-18.5	0.21 (1)	10.00
E-D	0 / 0	-18.5	-18.5	0.21 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-F	-263 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 874 (0.06")

CSI: TC=0.25/1.00 (C-F:1) , BC=0.21/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.21/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006264

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	J8	7	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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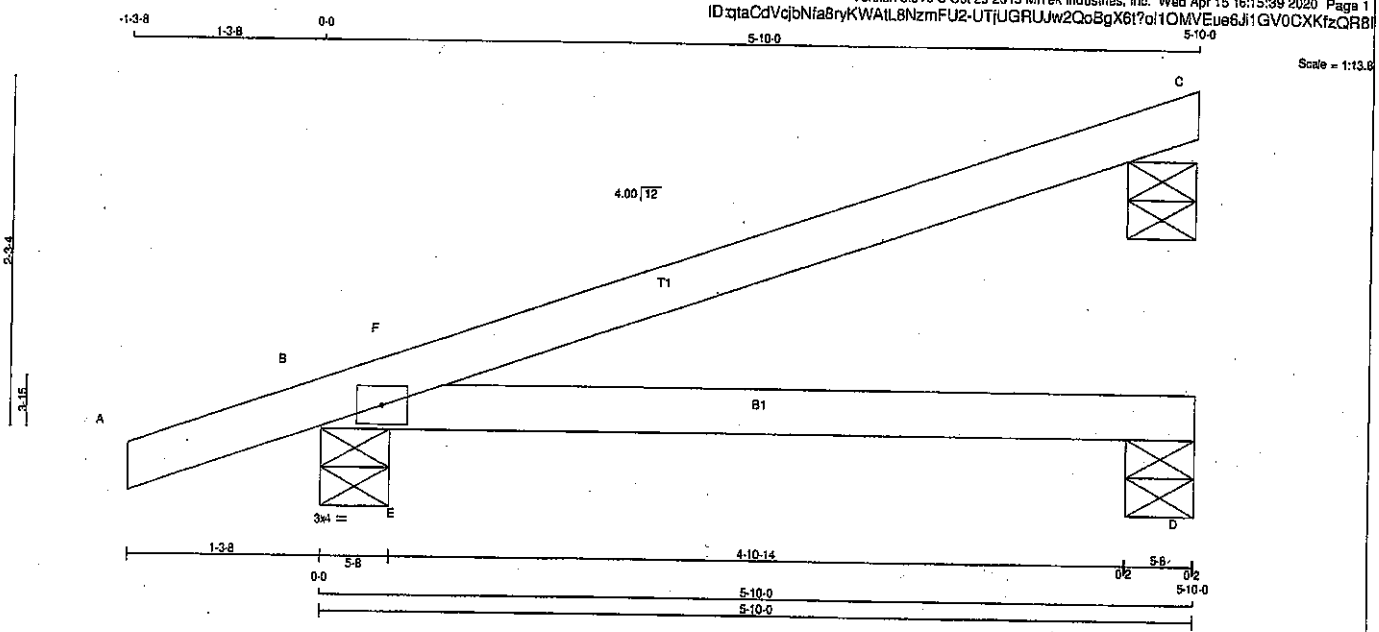


TOTAL WEIGHT = 7 X 15 = 107 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	234	0	234	0	0	5-8	5-8
B	444	0	444	0	0	5-8	5-8
D	88	0	88	0	0	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	182	126 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0
B	312	218 / 0	0 / 0	0 / 0	0 / 0	94 / 0	0 / 0
D	66	23 / 0	0 / 0	0 / 0	0 / 0	42 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 18	-91.8	-91.8	0.11 (1)	10.00	
B-F	-20 / 18	-91.8	-91.8	0.07 (4)	6.25	
F-C	-2 / 2	-91.8	-91.8	0.41 (1)	10.00	
B-E	0 / 0	-18.5	-18.5	0.28 (1)	10.00	
E-D	0 / 0	-18.5	-18.5	0.28 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-F	-324 / 12	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-09, CSA O86-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/501 (0.14")

CSI: TC=0.41/1.00 (C-F:1) , BC=0.28/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.27/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

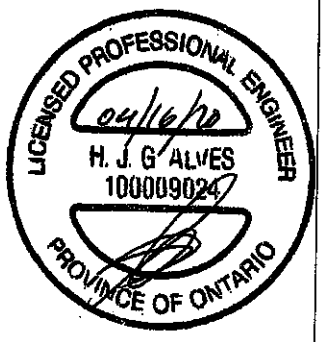
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



Structural component only
DWG# T-2006265

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	J9	11	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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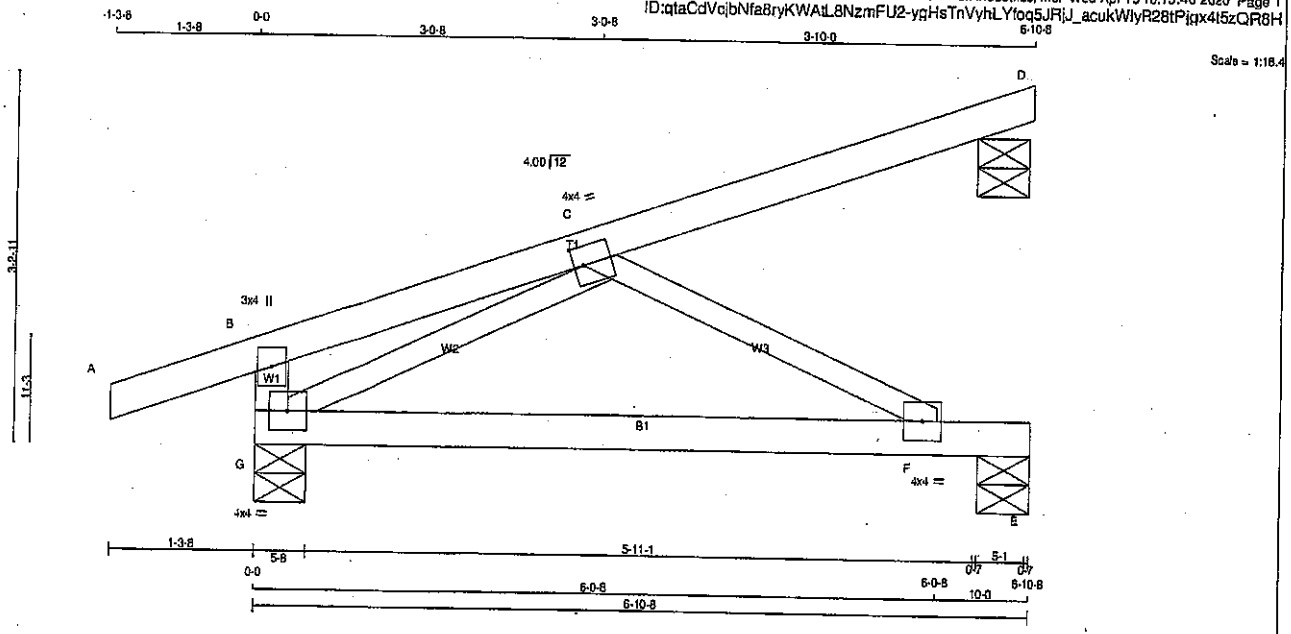


TOTAL WEIGHT = 11 X 24 = 259 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
G - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
F	BMW-t	MT20	4.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	502	0	502	0	0	5-8	5-8
D	161	0	161	0	0	5-8	5-8
E	218	0	218	0	0	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	353	244 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0
D	111	85 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
E	157	90 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, D, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
G-B	-236 / 0	0.0	0.0	0.02 (1)	7.81	
A-B	0 / 19	-91.8	-91.8	0.11 (1)	10.00	
B-C	-1 / 7	-91.8	-91.8	0.10 (1)	10.00	
C-D	-6 / 0	-91.8	-91.8	0.18 (1)	10.00	
G-F	0 / 325	-18.5	-18.5	0.34 (1)	10.00	
F-E	0 / 0	-18.5	-18.5	0.27 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-F	-371 / 0	0.09 (1)
G-C	-370 / 0	0.08 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 405 (0.20")

CSI: TC=0.18/1.00 (C-D:1) , BC=0.34/1.00 (F-G:1) , WB=0.09/1.00 (C-F:1) , SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

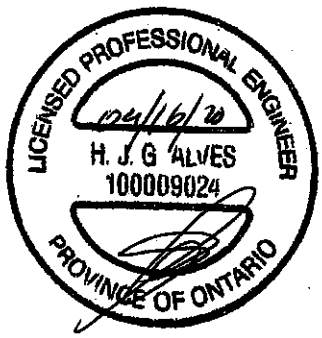
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (G) (INPUT = 0.90)
JSI METAL= 0.13 (F) (INPUT = 1.00)



Structural component only
DWG# T-2006266

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408128	J9A	5	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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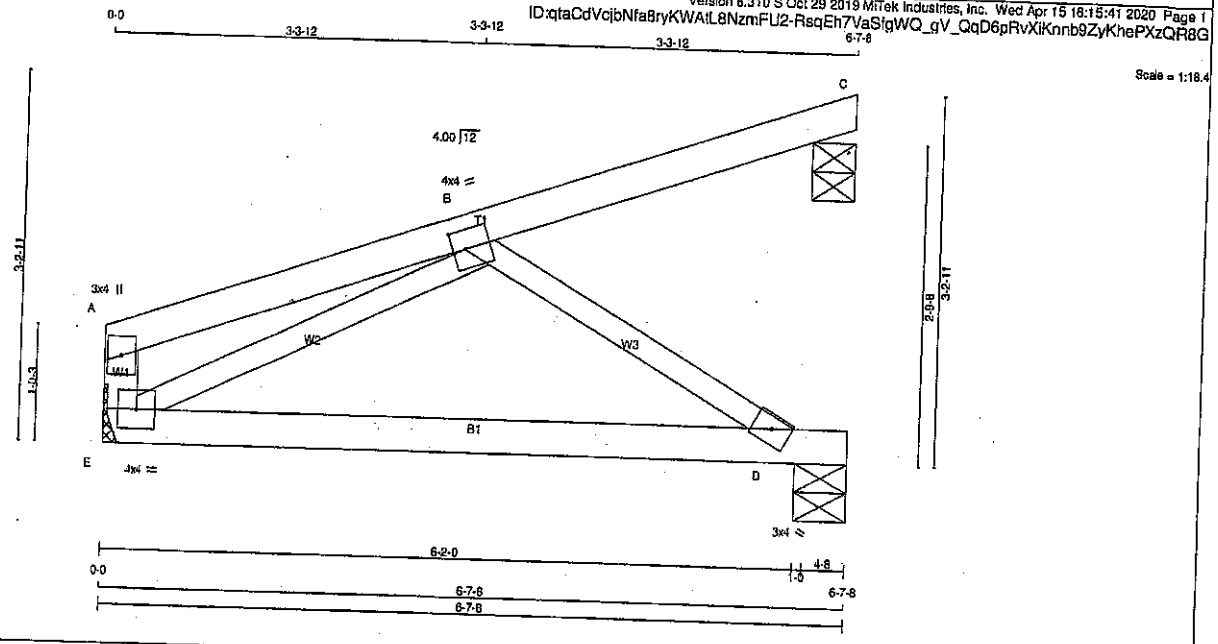

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	BMW1+w	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	338	0	338	0	0	MECHANICAL	
C	120	0	120	0	0	4-8	4-8
D	259	0	259	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	239	168 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
C	83	67 / 0	0 / 0	0 / 0	0 / 0	16 / 0	0 / 0
D	185	114 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
E-A	-113 / 0	0.0	0.0	0.01 (1)		7.81
A-B	0 / 11	-91.8	-91.8	0.16 (1)		10.00
B-C	-11 / 0	-91.8	-91.8	0.12 (1)		6.25
E-D	0 / 308	-18.5	-18.5	0.21 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-352 / 0	0.09 (1)
B-D	-369 / 0	0.09 (1)

TOTAL WEIGHT = 5 X 22 = 108 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD .

ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.21/1.00 (D-E:4) , WB=0.09/1.00 (B-D:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (E) (INPUT = 0.90)
JSI METAL= 0.14 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006267

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408129	J11	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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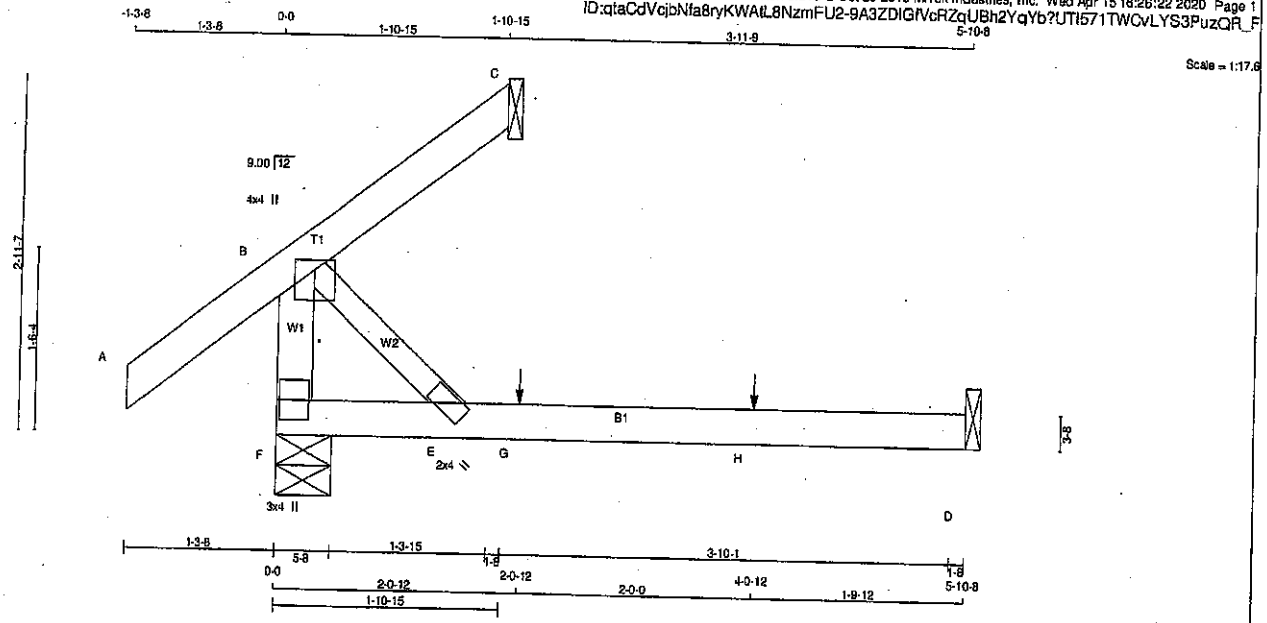

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	314	0	314	0	0	5-8	5-8
C	42	0	42	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	222	145 / 0	0 / 0	0 / 0	0 / 0	77 0	0 0
C	29	23 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-260 / 0	0.0	0.0	0.03 (1)	7.81	
A- B	0 / 38	-91.8	-91.8	0.13 (1)	10.00	
B- C	-27 / 0	-91.8	-91.8	0.12 (1)	6.25	
F- E	0 / 0	-18.5	-18.5	0.14 (4)	10.00	
E- G	0 / 0	-18.5	-18.5	0.19 (4)	10.00	
G- H	0 / 0	-18.5	-18.5	0.19 (4)	10.00	
H- D	0 / 0	-18.5	-18.5	0.19 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 0	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1
H	4-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

TOTAL WEIGHT = 3 X 14 = 42 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=0.98 NAIL=0.98 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006282

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408129	U12	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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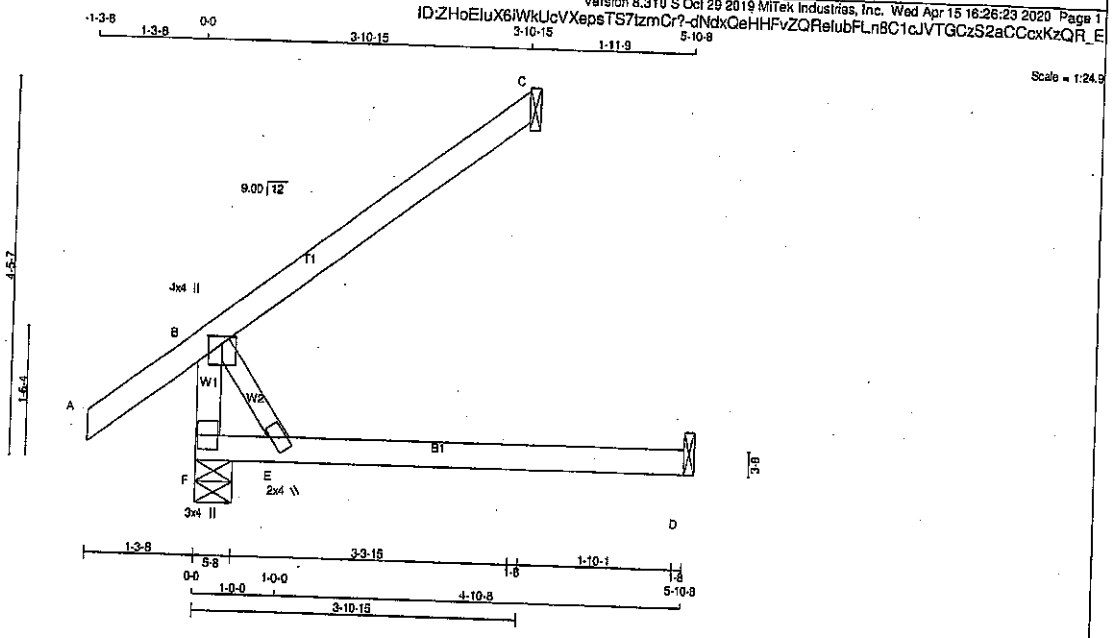

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 3 X 17 = 50 lb (M)

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	360	0	360	0	0	5-8	5-8
C	179	0	179	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	254	171 / 0	0 / 0	0 / 0	0 / 0	83 0	0 0
C	124	100 / 0	0 / 0	0 / 0	0 / 0	23 0	0 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-306 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 38	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 0	-91.8	-91.8	0.24 (1)	10.00
F- E	0 / 0	-18.5	-18.5	0.11 (4)	10.00
E- D	0 / 0	-18.5	-18.5	0.19 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO
04/16/20

Structural component only
DWG# T-2006283

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408129	J13	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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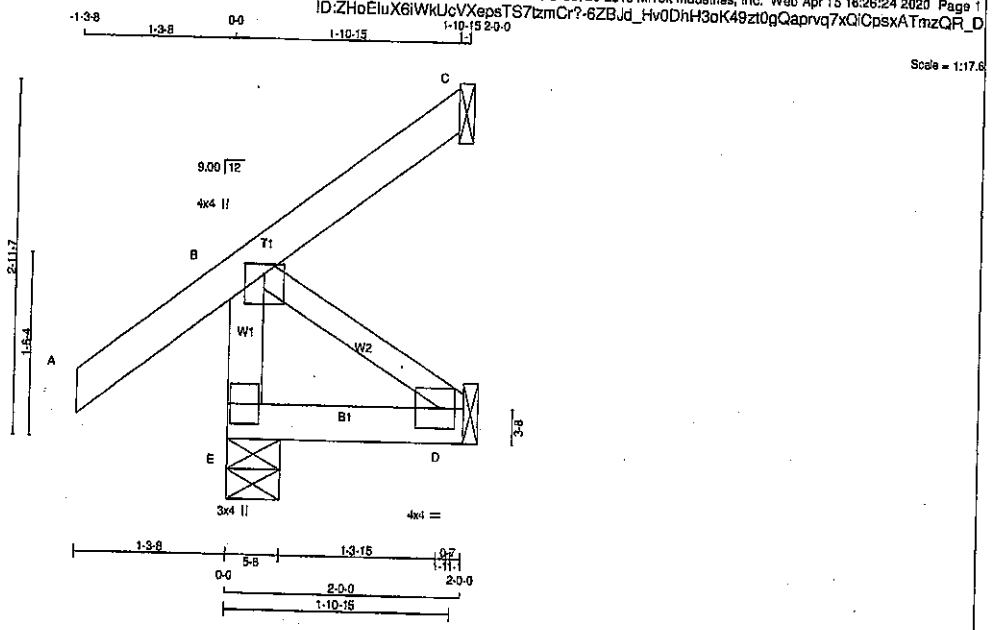


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMW1-t	MT20	4.0	4.0	2.00	1.50
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	277	0	277	0	0	IN-SX	IN-SX
C	42	0	42	0	-35	1-8	1-8
D	18	0	20	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	193	145 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0
C	29	23 / -25	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
E- B	-260 / 0	0.0	0.0	0.03 (1)		7.81
A- B	0 / 38	-91.8	-91.8	0.12 (1)		10.00
B- C	-27 / 0	-91.8	-91.8	0.12 (1)		6.25
E- D	0 / 0	-18.5	-18.5	0.02 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- D	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 10 = 30 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

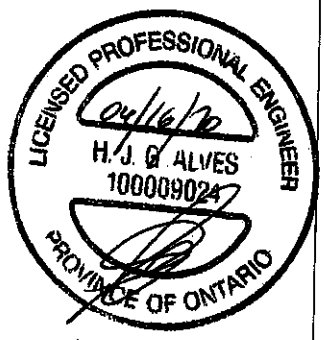
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2006284

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408129	J14	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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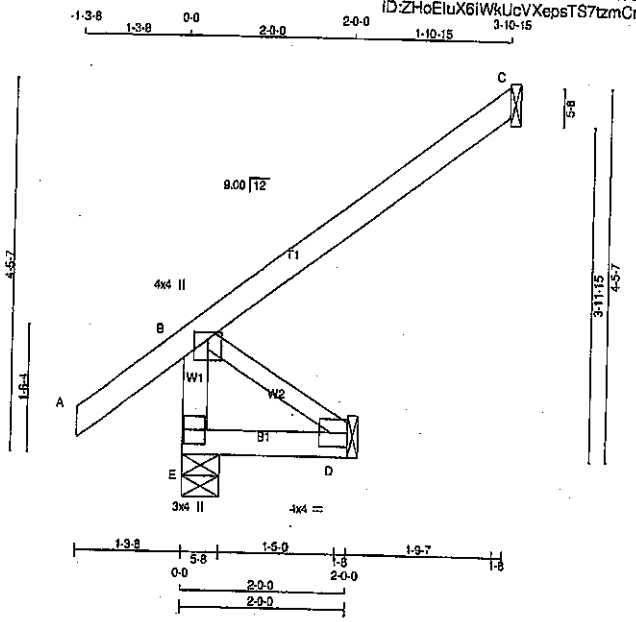


TOTAL WEIGHT = 3 X 13 = 38 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMW1-t	MT20	4.0	4.0	2.00	Edge
E	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	324	0	324	0	0	5-8	5-8
C	179	0	179	0	0	1-8	1-8
D	18	0	20	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	225	171 / 0	0 / 0	0 / 0	0 0	54 0	0 0
C	124	100 / 0	0 / 0	0 / 0	0 0	23 0	0 0
D	14	0 / 0	0 / 0	0 / 0	0 0	14 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (S)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-306 / 0	0.0	0.0	0.03 (1)	7.81	B- D	0 0	0.00 (1)
A- B	0 / 38	-91.8	-91.8	0.13 (5)	10.00			
B- C	0 / 0	-91.8	-91.8	0.24 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

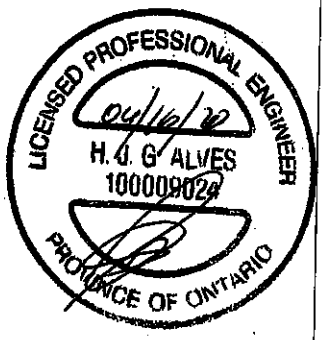
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2006285

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408129	J15	12	1	TRUSS DESC.	

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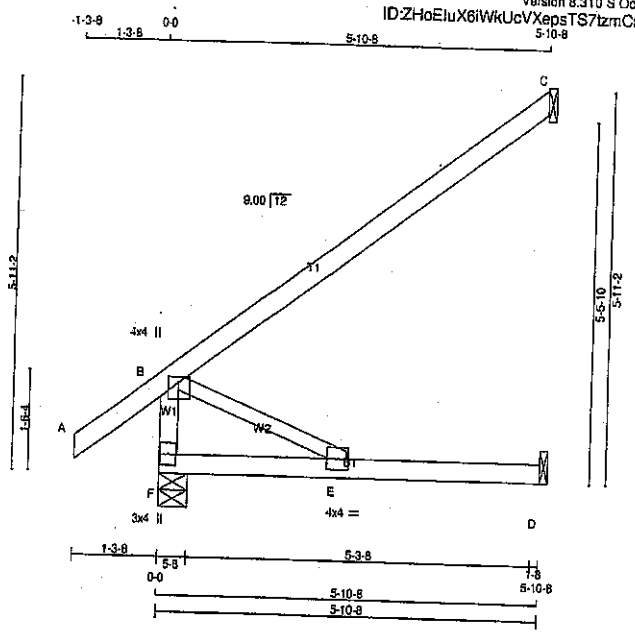


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 12 X 21 = 248 lb

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	450	0	450	0	0	IN-SX	IN-SX
C	270	0	270	0	0	5-8	5-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	316	221 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
C	166	150 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

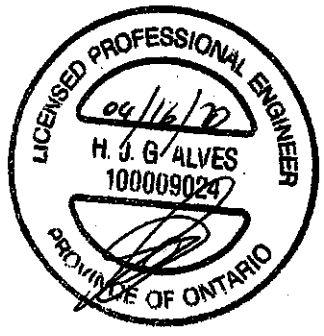
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
F-B	-396 / 0	0.0	0.0	0.04 (1)		7.81
A-B	0 / 38	-91.8	-91.8	0.12 (1)		10.00
B-C	0 / 0	-91.8	-91.8	0.54 (1)		10.00
F-E	0 / 0	-18.5	-18.5	0.19 (4)		10.00
E-D	0 / 0	-18.5	-18.5	0.19 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-E	0 / 0	0.00 (1)



DESIGN CRITERIA [M]

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-09, CSA O86-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006286

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408130	J20	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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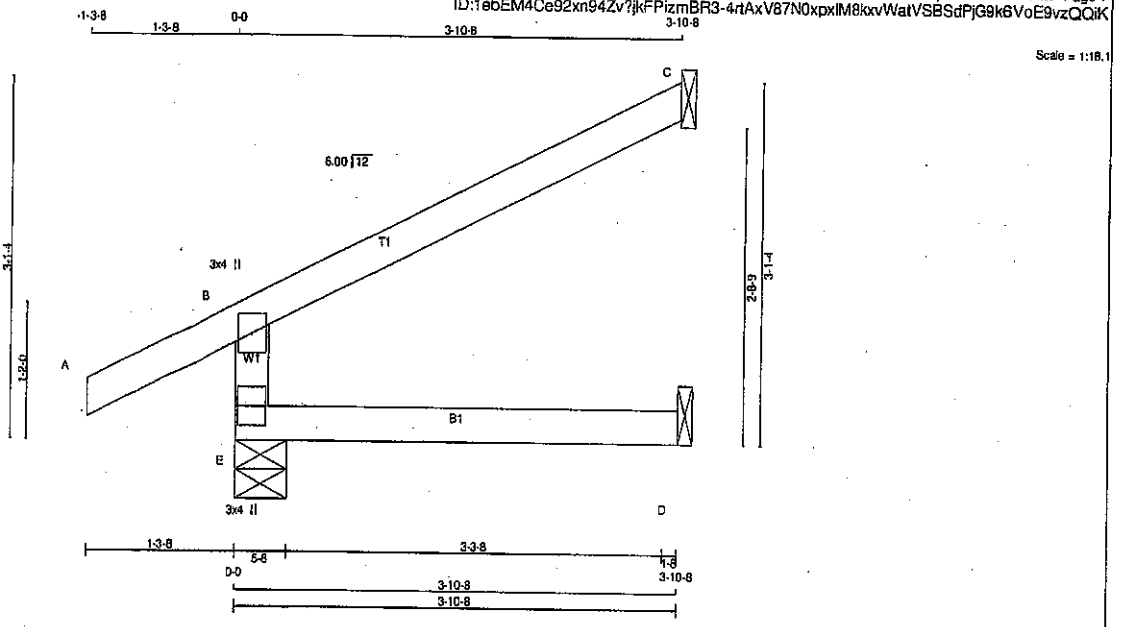


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	388	0	388	0	0	5-8	5-8
C	133	0	133	0	0	1-8	1-8
D	30	0	34	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	272	193 / 0	0 / 0	0 / 0	0 / 0	78 0	0 0
C	92	74 / 0	0 / 0	0 / 0	0 / 0	17 0	0 0
D	24	0 / 0	0 / 0	0 / 0	0 0	24 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

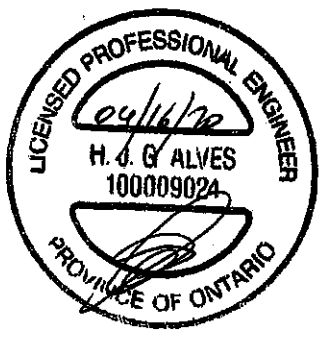
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-347 / 0	0.0	0.0	0.05 (4)	7.81			
A- B	0 / 28	-91.8	-91.8	0.14 (5)	10.00			
B- C	-20 / 0	-91.8	-91.8	0.23 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.05 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



TOTAL WEIGHT = 4 X 12 = 48 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.06/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006304

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	J21	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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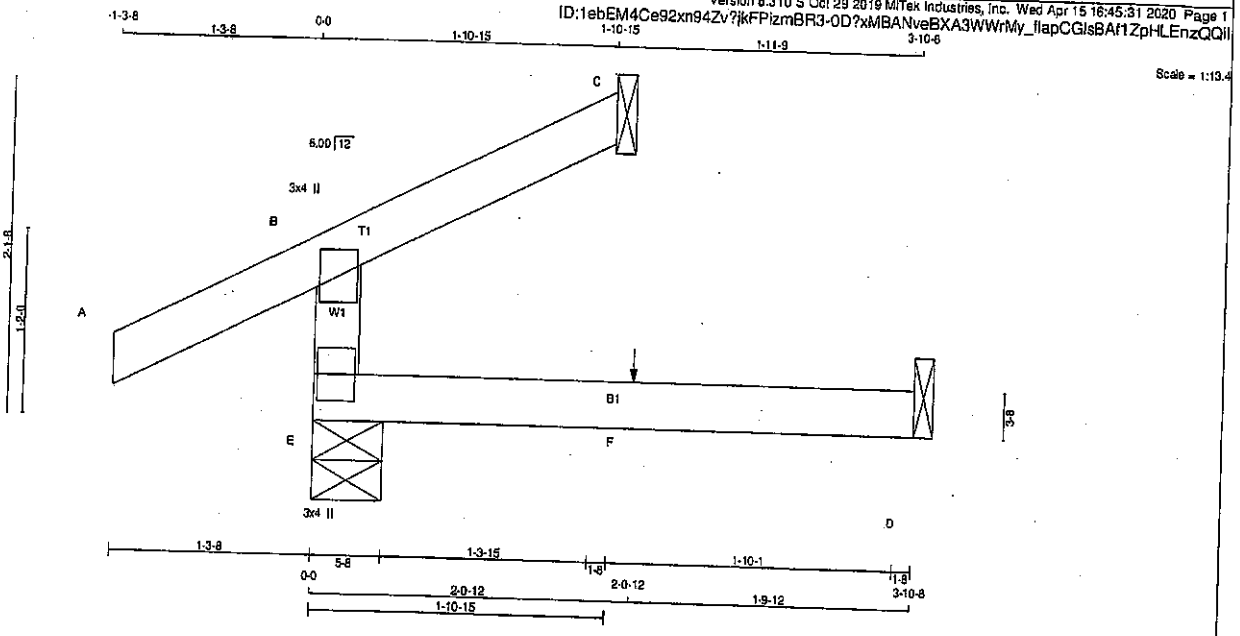


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	275	0	275	0	0	5-8	5-8
C	66	0	66	0	0	1-8	1-8
D	30	0	34	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	194	130 / 0	0 / 0	0 / 0	0 / 0	64 0	0 0
C	46	37 / 0	0 / 0	0 / 0	0 / 0	9 0	0 0
D	24	0 / 0	0 / 0	0 / 0	0 / 0	24 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-234 / 0	0.0	0.0	0.05 (4)	7.81			
A-B	0 / 28	-91.8	-91.8	0.14 (5)	10.00			
B-C	-10 / 0	-91.8	-91.8	0.06 (1)	10.00			
E-F	0 / 0	-18.5	-18.5	0.05 (4)	10.00			
F-D	0 / 0	-18.5	-18.5	0.05 (4)	10.00			

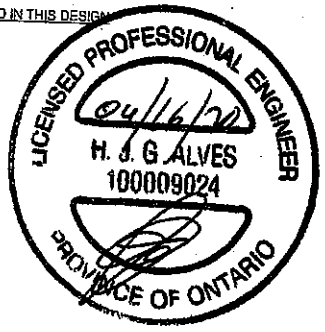
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



TOTAL WEIGHT = 2 X 9 = 19 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.8	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14/1.00 (A-B:5) , BC=0.06/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.10/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006305

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408130	J22	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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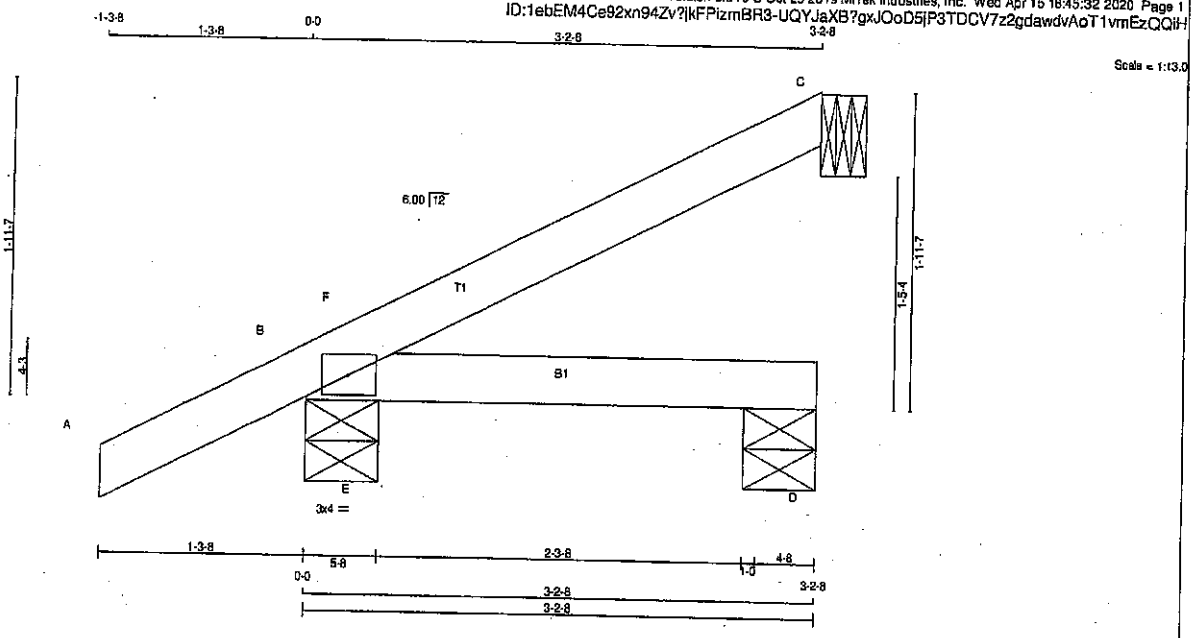

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
C	127	0	127	0	0	4-8	4-8
B	301	0	301	0	0	5-8	5-8
D	50	0	50	0	0	5-8	5-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	88	88 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0
B	211	152 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
D	37	14 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 27	-91.8	-91.8	0.13 (5)	10.00	E-F	-159 / 3	0.00 (1)	
B-F	-14 / 5	-91.8	-91.8	0.02 (4)	6.25				
F-C	0 / 2	-91.8	-91.8	0.12 (1)	10.00				
B-E	0 / 0	-18.5	-18.5	0.09 (1)	10.00				
E-D	0 / 0	-18.5	-18.5	0.09 (1)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 5 X 10 = 48 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.09/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.13/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006306

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	J23	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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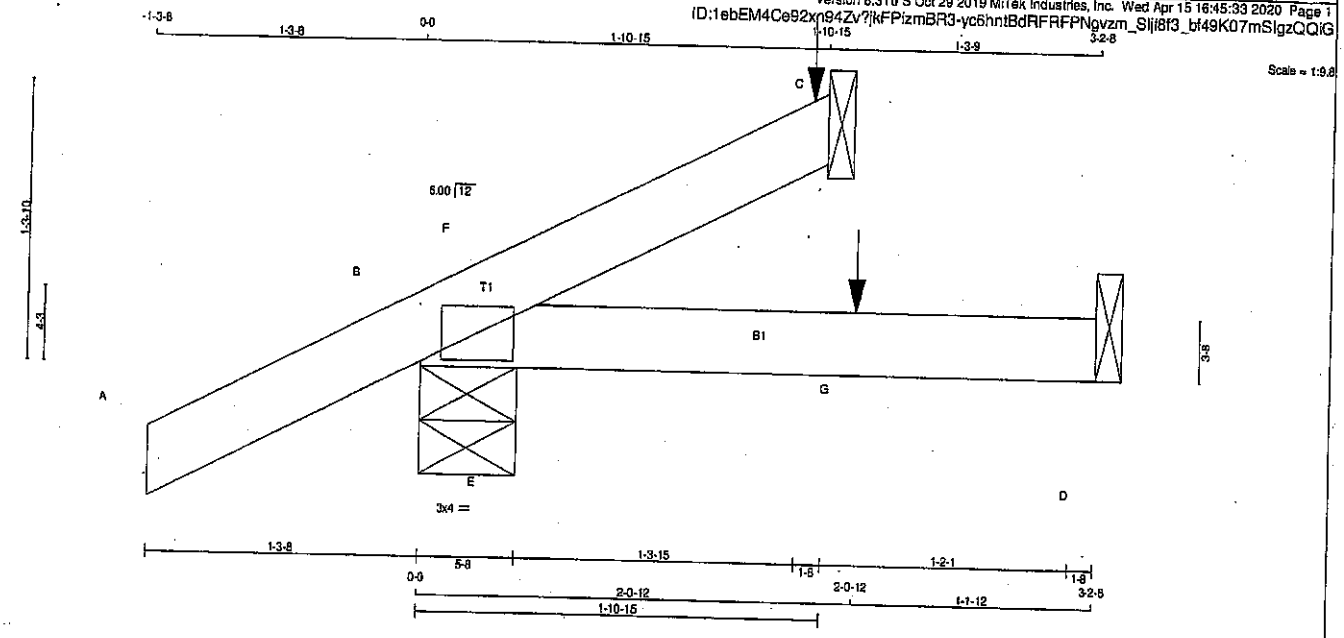


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 8 lb [M]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	125	0	125	0	0	1-8	1-8
B	246	0	246	0	0	5-8	5-8
D	37	0	37	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	87	62 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
B	172	123 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
D	28	8 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 27	-91.8	-91.8	0.14 (5)	10.00	E- F	0 / 44	0.00 (1)
B- F	-38 / 0	-91.8	-91.8	0.04 (1)	6.25			
F- C	0 / 9	-91.8	-91.8	0.06 (1)	10.00			
B- E	0 / 0	-18.5	-18.5	0.01 (1)	10.00			
E- G	0 / 0	-18.5	-18.5	0.04 (1)	10.00			
G- D	0 / 0	-18.5	-18.5	0.04 (1)	10.00			

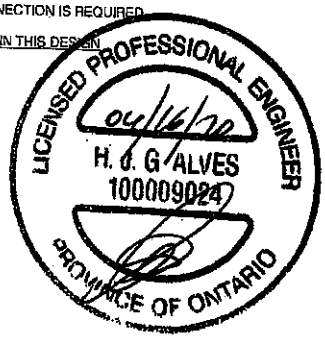
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-10-15	-36	-36	---	FRONT	VERT	TOTAL	---	C1
G	2-0-12	-13	-13	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN.



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14/1.00 (A-B:5) , BC=0.04/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.10/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)

Structural component only.
DWG# T-2006307

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	J24	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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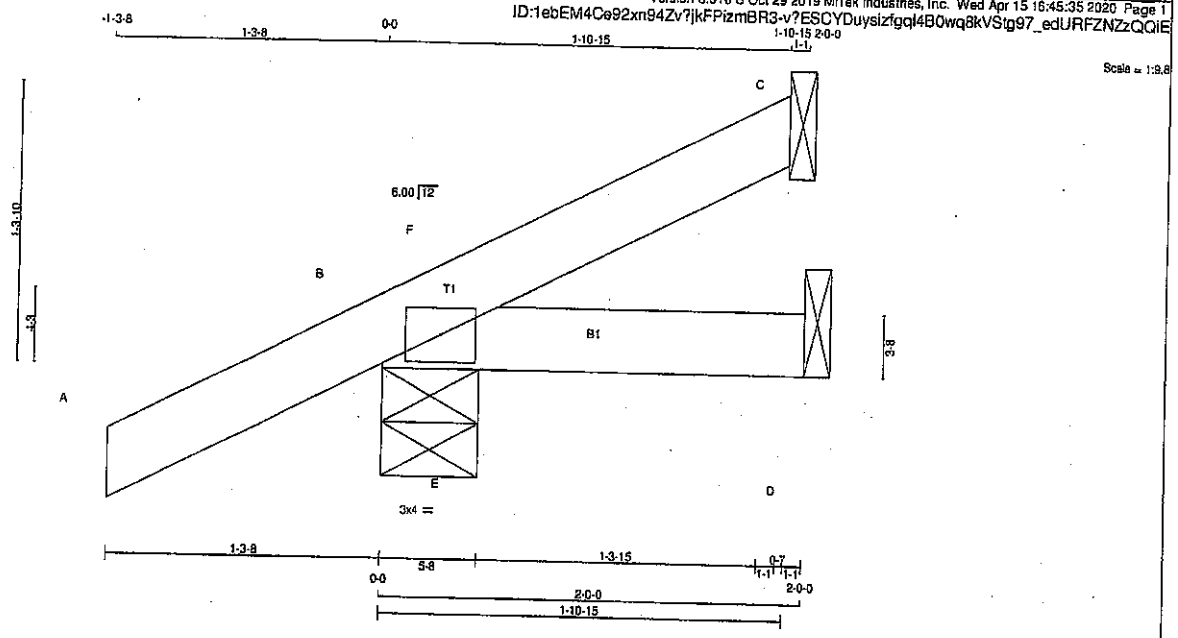

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	75	0	75	0	0	1-8	1-8
B	231	0	231	0	0	5-8	5-8
D	31	0	31	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	52	40 / 0	0 / 0	0 / 0	0 / 0	12 0	0 0
B	161	119 / 0	0 / 0	0 / 0	0 / 0	43 0	0 0
D	23	9 / 0	0 / 0	0 / 0	0 / 0	14 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 27	-91.8	-91.8	0.12 (1)	10.00	E- F	-61 4	0.00 (1)	
B- F	-11 / 0	-91.8	-91.8	0.01 (4)	6.25				
F- C	0 / 3	-91.8	-91.8	0.04 (1)	10.00				
B- E	0 / 0	-18.5	-18.5	0.04 (1)	10.00				
E- D	0 / 0	-18.5	-18.5	0.04 (1)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 7 lb

M

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.04/1.00 (B-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006308

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	J25	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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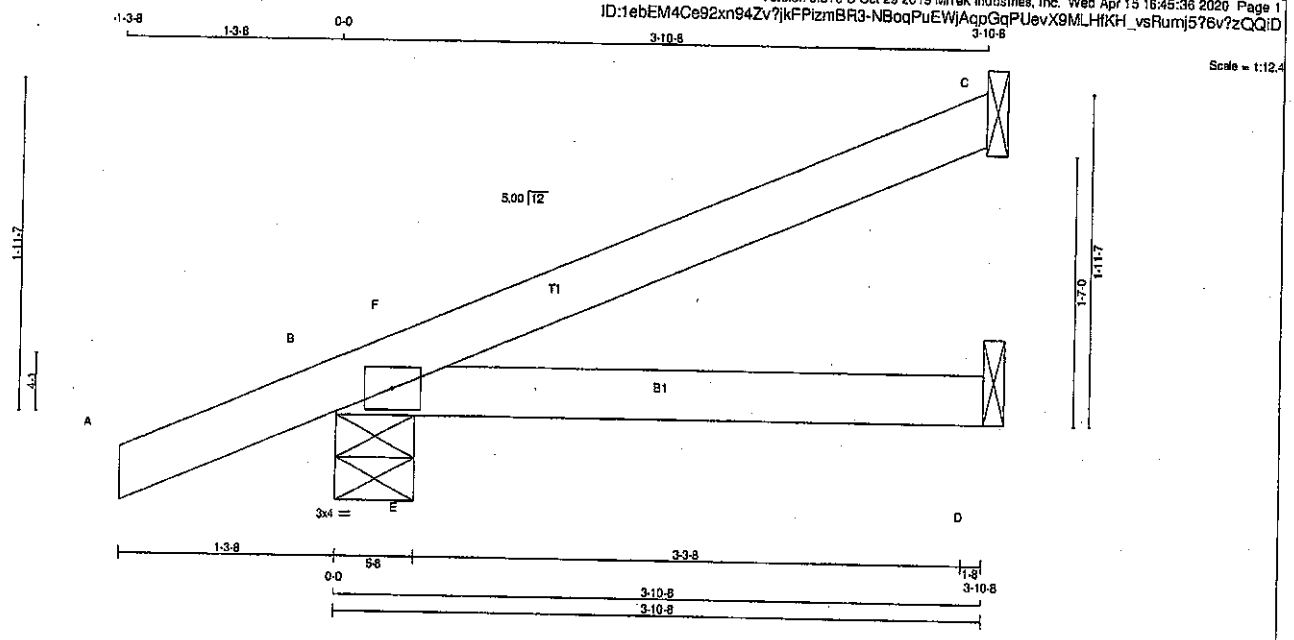

TOTAL WEIGHT = 2 X 11 = 22 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	154	0	154	0	0	1-8	1-8
B	337	0	337	0	0	5-8	5-8
D	60	0	60	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	107	83 / 0	0 / 0	0 / 0	0 / 0	24 0	0 0
B	236	168 / 0	0 / 0	0 / 0	0 / 0	68 0	0 0
D	44	16 / 0	0 / 0	0 / 0	0 / 0	28 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 23	-91.8	-91.8	0.13 (5)		10.00
B- F	-15 / 4	-91.8	-91.8	0.03 (4)		6.25
F- C	0 / 2	-91.8	-91.8	0.18 (1)		10.00
B- E	0 / 0	-18.5	-18.5	0.13 (1)		10.00
E- D	0 / 0	-18.5	-18.5	0.13 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- F	-189 / 8	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.18/1.00 (C-F:1) , BC=0.13/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.16/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006309

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.		DRWG NO.
408130	J26	2	1	TRUSS DESC.	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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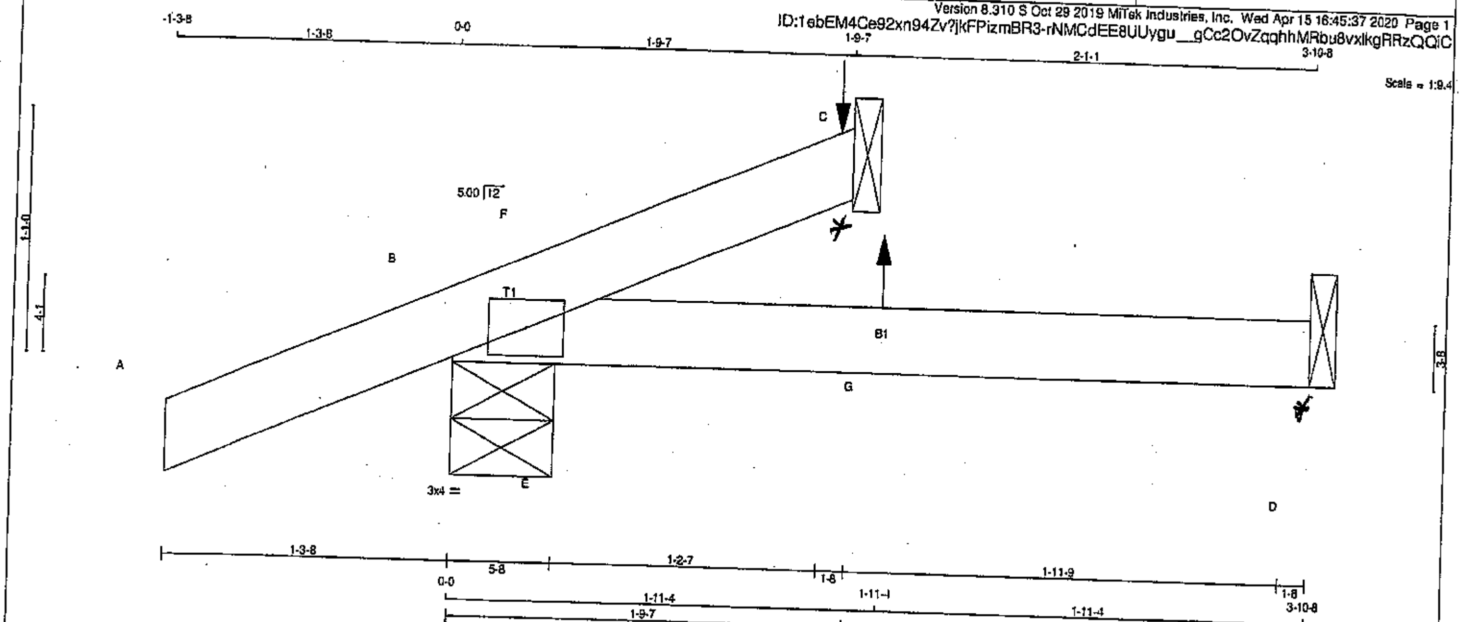

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	66	0	66	0	-7	1-8	1-8
B	273	0	273	0	0	5-8	5-8
D	22	0	30	0	0	1-8	1-8

* SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
19204615967690640075821451	8@Ai·ø!;·@î·/ -18		0/0	0/0	0/0	24 . 0	0 0
19204615967690640075821451	9@Ai·ø!;·@î·/ 0		0/0	0/0	0 . 0	53 . 0	0 0
D	16	0/-6	0/0	0/0	0/0	22 / 0	0 . 0

HORIZONTAL REACTIONS

62051C6314938249891020	80/0	0/0	0/0	0/0	0 0	0/0
62051B6314938249891020	80/0	0/0	0/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (8)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 23	-91.8	-91.8	0.14 (5)	10.00
B- F	-70 / 0	-91.8	-91.8	0.13 (5)	6.25
F- C	-7 / 7	-91.8	-91.8	0.05 (4)	10.00
B- E	0 / 0	-18.5	-18.5	0.05 (5)	10.00
E- G	0 / 0	-18.5	-18.5	0.05 (5)	10.00
G- D	0 / 0	-18.5	-18.5	0.05 (5)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- F	0 . 96	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-9-7	-11	-11	44	BACK	VERT	TOTAL	---	C1
G	1-11-4	9	1	22	BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 8 = 17 lb
[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.14/1.00 (A-B:5) , BC=0.05/1.00 (B-E:5) , WB=0.00/1.00 (E-F:1) , SSI=0.13/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

LICENCED PROFESSIONAL ENGINEER
04/16/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006310

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	J27	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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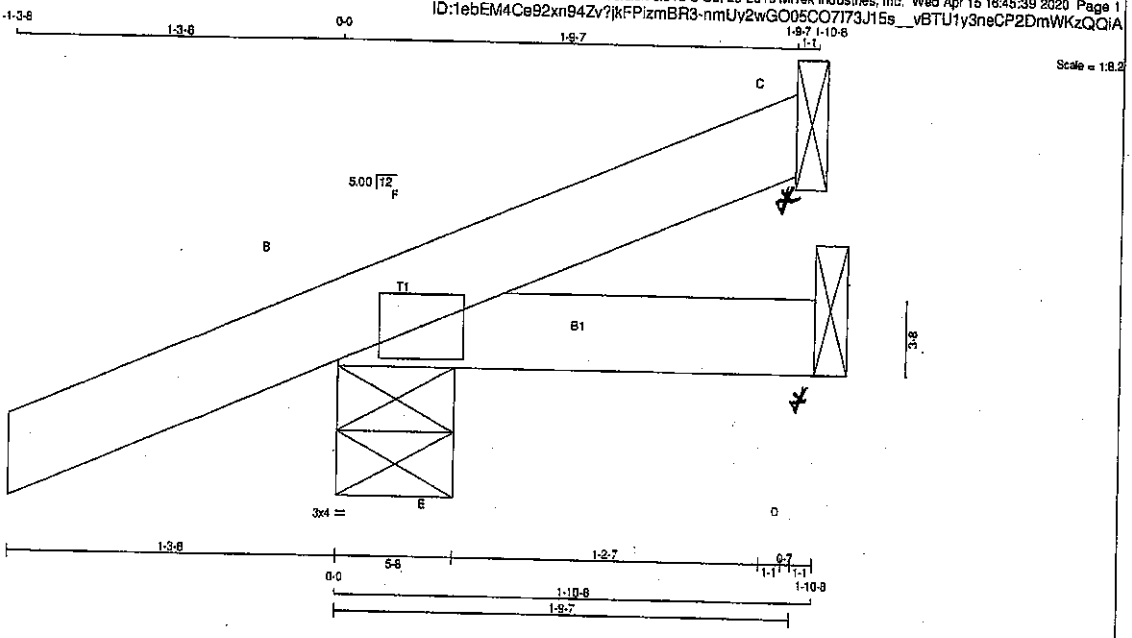


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	50	0	50	0	-9	1-8	1-8
B	269	0	269	0	0	5-8	5-8
D	4	0	14	0	-14	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	35	26 / -11	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
B	187	140 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
D	4	0 / -15	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

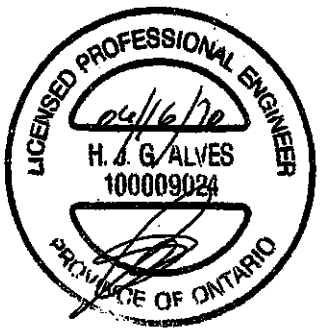
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 23	-91.8	-91.8	0.12	(1)	10.00	E- F	0 / 103	0.00 (1)
B- F	-53 / 0	-91.8	-91.8	0.11	(1)	6.25			
F- C	-8 / 0	-91.8	-91.8	0.03	(5)	10.00			
B- E	0 / 0	-18.5	-18.5	0.04	(5)	10.00			
E- D	0 / 0	-18.5	-18.5	0.04	(5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



TOTAL WEIGHT = 2 X 6 = 12 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.04/1.00 (D-E:5) , WB=0.00/1.00 (E-F:1) , SSI=0.12/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006311

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408130	J28	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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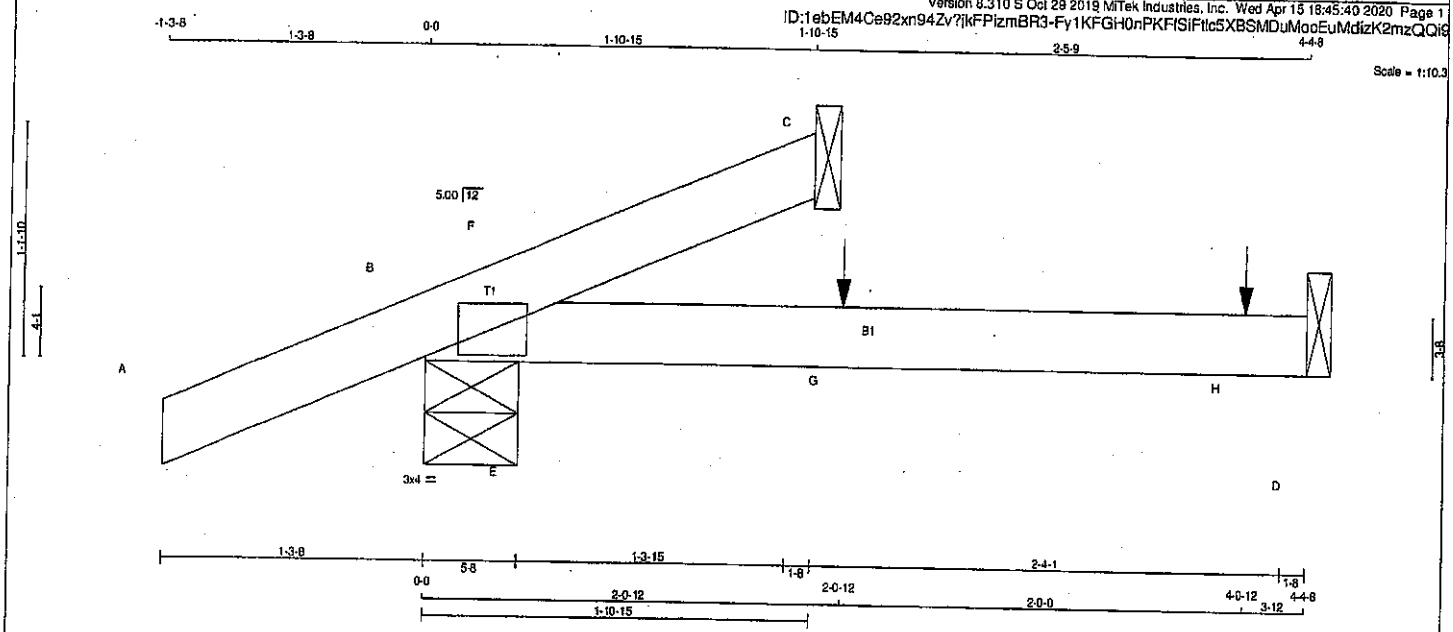


TOTAL WEIGHT = 9 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	112	0	112	0	0	1-8	1-8
B	251	0	251	0	0	5-8	5-8
D	116	0	116	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL 997791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	80	51 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0
B	176	125 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0
D	84	46 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI	(LC)	MAX. UNBRAC LENGTH
A-B	0 / 23	-91.8	-91.8	0.12	(1)		10.00
B-F	-60 / 0	-91.8	-91.8	0.09	(1)		6.25
F-C	0 / 18	-91.8	-91.8	0.10	(1)		10.00
B-E	0 / 0	-18.5	-18.5	0.04	(4)		10.00
E-G	0 / 0	-18.5	-18.5	0.07	(1)		10.00
G-H	0 / 0	-18.5	-18.5	0.07	(1)		10.00
H-D	0 / 0	-18.5	-18.5	0.07	(1)		10.00

WEBS

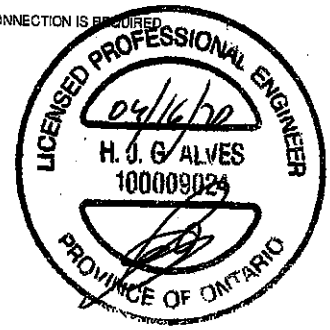
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-F	0 / 97	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-14	-14	---	BACK	VERT	TOTAL	---	C1
H	4-0-12	-86	-86	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.07/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.11/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	798	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006312

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408130	J29	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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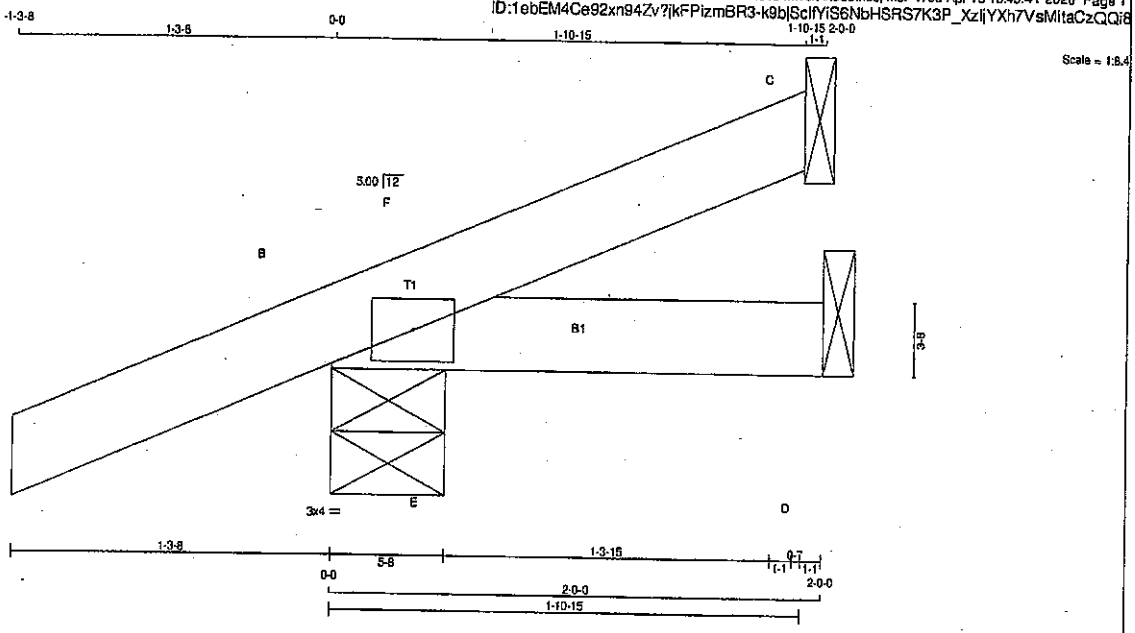


TOTAL WEIGHT = 6 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	74	0	74	0	0	1-8	1-8
B	230	0	230	0	0	5-8	5-8
D	32	0	32	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	51	39 / 0	0 / 0	0 / 0	0 0	12 0	0 0
B	161	118 / 0	0 / 0	0 / 0	0 0	42 0	0 0
D	23	9 / 0	0 / 0	0 / 0	0 0	14 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

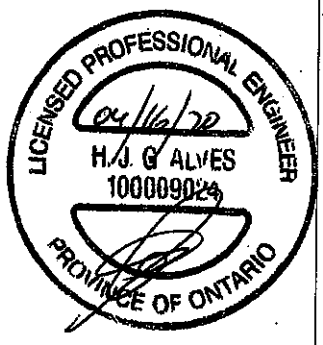
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 23	-91.8	-91.8	0.12 (1)	10.00	E-F	-54 / 4	0.00 (1)
B-F	-11 / 0	-91.8	-91.8	0.02 (1)	6.25			
F-C	0 / 3	-91.8	-91.8	0.04 (1)	10.00			
B-E	0 / 0	-18.5	-18.5	0.04 (1)	10.00			
E-D	0 / 0	-18.5	-18.5	0.04 (1)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.04/1.00 (B-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

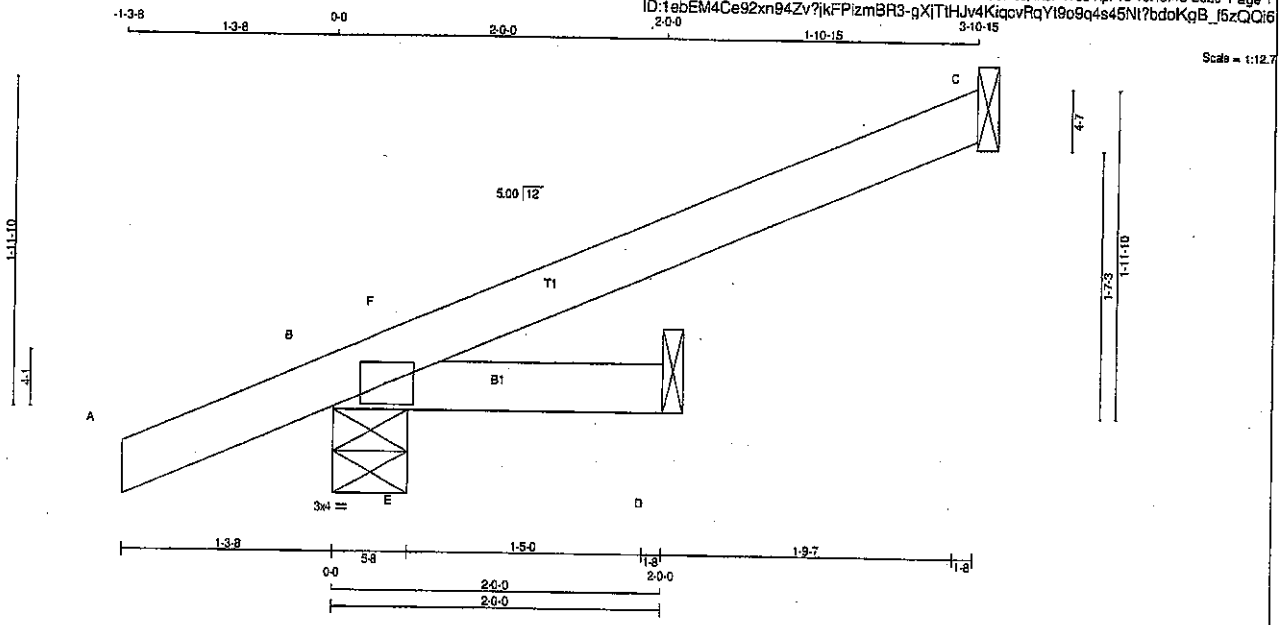
JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006313

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408130	J30	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Wed Apr 15 16:45:43 2020 Page 1
ID:1ebEM4Ce92xn94Zv?jkFPizm8R3-gXjTtHJv4KiqcvRqYt9o9q4s45Nl?bdoKqB_f5zQQi6



TOTAL WEIGHT = 9 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	139	0	139	0	0	1-8	1-8
B	283	0	283	0	0	5-8	5-8
D	98	0	98	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	96	77 / 0	0 / 0	0 / 0	0 / 0	19 0	0 0
B	197	147 / 0	0 / 0	0 / 0	0 / 0	50 0	0 0
D	70	45 / 0	0 / 0	0 / 0	0 / 0	24 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

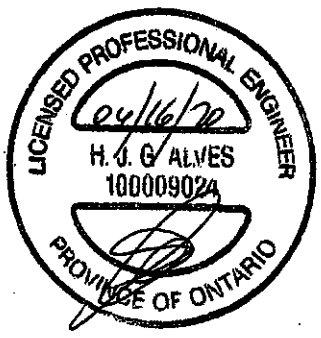
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 23	-91.8	-91.8	0.12 (5)	10.00	E-F	-327 / 0	0.00 (1)
B-F	0 / 56	-91.8	-91.8	0.09 (1)	10.00			
F-C	-7 / 0	-91.8	-91.8	0.14 (1)	10.00			
B-E	0 / 0	-18.5	-18.5	0.17 (1)	10.00			
E-D	0 / 0	-18.5	-18.5	0.17 (1)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14/1.00 (C-F:1) , BC=0.17/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.21/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.20 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006314

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

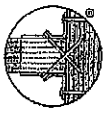
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

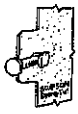
Installation:

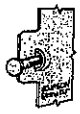
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

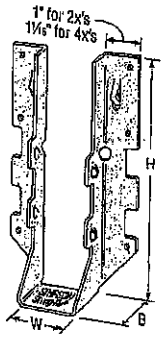
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.


Double-Shear Nailing Top View

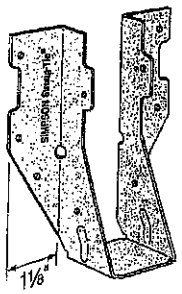

Double-Shear Nailing Side View; Do not bend tab


Dome Double-Shear Nailing Side View *(available on some models)*

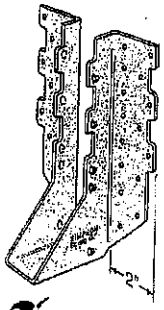
U.S. Patent 5,603,580



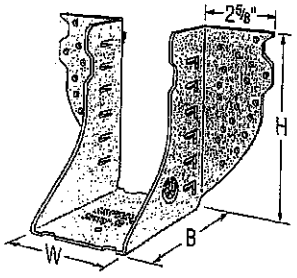
LUS28



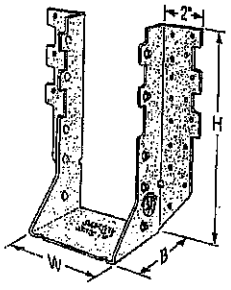
LU26L



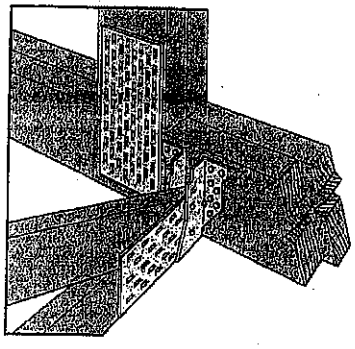
HUS210
(HUS26, HUS28, and HHUS similar)

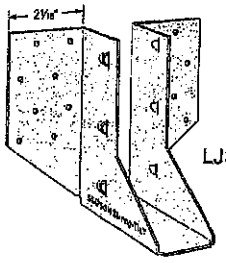


HGUS28-2



HHUS210-2


Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)



LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON Strong-Tie

HHUS/HGUS

See Hanger Options information on pp. 125–127.

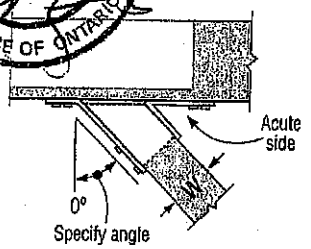
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Single 2x Sizes											
LUS24	18	1⅜	3⅛	1¾	2⅛	(4) 10d	(2) 10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
LU24L	22	1⅜	3	1⅝	2⅛	(4) 10d	(2) 10d x 1½"	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
LU26L	22	1⅜	5	1⅝	4⅝	(6) 10d	(4) 10d x 1½"	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
LUS26	18	1⅜	4¾	1¾	3¾	(4) 10d	(4) 10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
HUS26	16	1⅝	5⅜	3	3⅞	(14) 16d	(6) 16d	2705 / 11.80	4940 / 21.97	2065 / 9.20	3875 / 17.24
LJS26DS	18	1⅜	5	3½	4⅝	(16) 16d	(6) 16d	2055 / 9.14	4265 / 18.97	1460 / 6.49	4115 / 18.31
HGUS26	12	1⅝	5⅜	5	4⅛	(20) 16d	(8) 16d	2685 / 11.96	6625 / 29.51	2685 / 11.96	5700 / 25.35
LU28L	20	1⅜	6¾	1⅝	5⅞	(8) 10d	(6) 10d x 1½"	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
LUS28	18	1⅜	6⅝	1¾	3¾	(6) 10d	(4) 10d	1420 / 6.32	2520 / 11.21	1290 / 5.74	1790 / 7.96
HUS28	16	1⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3605 / 16.04	5365 / 23.86	2675 / 11.90	4345 / 19.33
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310 / 14.74	7675 / 34.19	3310 / 14.74	6900 / 30.73
LU210L	20	1⅜	8	1⅝	7⅝	(10) 10d	(6) 10d x 1½"	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
LUS210	18	1⅜	7⅞	1¾	3¾	(8) 10d	(4) 10d	1420 / 6.32	2785 / 12.39	1290 / 5.74	2210 / 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. **Nails:** 16d = 0.162" dia. x 3½" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

SIMPSON Strong-Tie®

■ These products are available with additional corrosion protection. For more information, see p. 24.

◤ These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Double 2x Sizes											
■ LUS24-2	18	3⅛	3⅛	2	1½	(4) 16d	(2) 16d	835 / 3.71	2020 / 8.99	590 / 2.62	1435 / 6.38
SS ■ LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
■ HHUS26-2	14	3⁵⁄₁₆	5¾	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2850 / 12.68	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS26-2	12	3⁵⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
SS ■ LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
■ HHUS28-2	14	3⁵⁄₁₆	7⁵⁄₁₆	3	6⅛	(22) 16d	(8) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS28-2	12	3⁵⁄₁₆	7⁵⁄₁₆	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
SS ■ LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
■ HHUS210-2	14	3⁵⁄₁₆	9³⁄₁₆	3	8	(30) 16d	(10) 16d	4670 / 20.77	9660 / 42.97	4235 / 18.84	7000 / 31.14
HGUS210-2	12	3⁵⁄₁₆	9⁵⁄₁₆	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
Triple 2x Sizes											
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
■ HHUS210-3	14	4¹⁵⁄₁₆	9	3	7¹⁵⁄₁₆	(30) 16d	(10) 16d	4670 / 20.77	9670 / 43.02	4235 / 18.84	6865 / 30.54
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
Quadruple 2x Sizes											
HGUS26-4	12	6⁵⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-4	12	6⁵⁄₁₆	7⁵⁄₁₆	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
■ HHUS210-4	14	6⅛	8⅞	3	7¹³⁄₁₆	(30) 16d	(10) 16d	4670 / 20.77	10155 / 45.17	4235 / 18.84	7210 / 32.07
HGUS210-4	12	6⁵⁄₁₆	9³⁄₁₆	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
HGUS212-4	12	6⁵⁄₁₆	10⅝	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS214-4	12	6⁵⁄₁₆	12⅝	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80
4x Sizes											
■ LUS46	18	3⁵⁄₁₆	4¾	2	3⅞	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
■ HHUS46	14	3⅝	5¼	3	3¹³⁄₁₆	(14) 16d	(6) 16d	2540 / 11.30	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
■ LUS48	18	3⁵⁄₁₆	6¾	2	3⅞	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
■ HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS48	12	3⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
■ LUS410	18	3⁵⁄₁₆	8¾	2	5⅞	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
HGUS410	12	3⅝	9	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
HGUS412	12	3⅝	10⅞	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS414	12	3⅝	12⅞	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80

See footnotes on p. 258.

LICENSED PROFESSIONAL ENGINEER 1/15/18 B. D. BUNTING PROVINCE OF ONTARIO

TECHNICAL BULLETIN

TC – Truss Connectors

SIMPSON Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

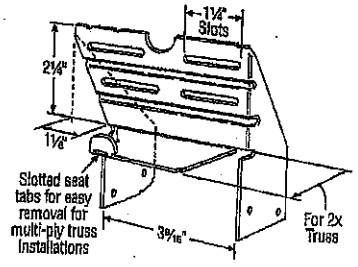
Design: Factored resistances are in accordance with CSA 086-14

Installation:

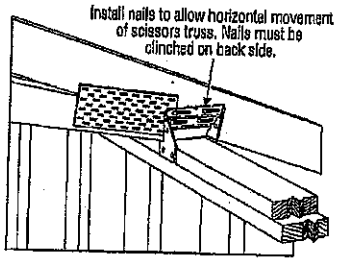
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½ = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss–allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

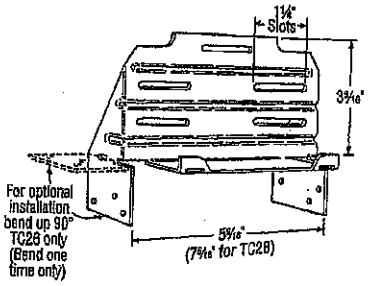
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



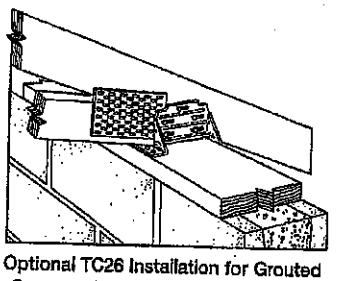
TC24
U.S. Patent 4,932,173



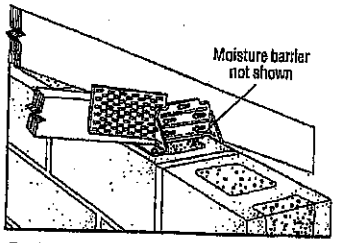
Typical TC24 Installation



TC26
(TC28 Similiar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC26	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC28 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) – ⅜" x 2¼" Titen screws has a factored uplift resistance of 275 lb.

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4/3/2017
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H/TSP

SIMPSON Strong-Tie®

Seismic and Hurricane Ties (cont.)

■ These products are available with additional corrosion protection. For more information, see p. 24.

◤ These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Fasteners			Factored Resistance (K_D = 1.15)					
					D.Fir-L			S-P-F		
					Uplift	Lateral		Uplift	Lateral	
		To Rafters/ Truss	To Plates	To Studs		F_1	F_2		F_1	F_2
					lb.	lb.	lb.	lb.	lb.	lb.
					kN	kN	kN	kN	kN	kN
■	H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
						3.29	3.05	1.33	3.02	2.16	0.96
SS	H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
						3.69	0.98	0.33	2.62	0.69	0.24
SS	H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
						3.58	0.71	0.71	3.36	0.71	0.71
	H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
						3.71	0.78	0.93	3.29	0.71	0.93
SS	H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
						3.29	0.80	1.18	2.74	0.56	0.85
■	H6	16	—	(8) 8d	(8) 8d	1585	1085	—	1125	770	—
						7.05	4.83	—	5.00	3.43	—
■	H7Z	16	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
						6.18	2.98	—	4.40	2.11	—
SS	H8[3]	18	(5) 10d x 1½"	(5) 10d x 1½"	—	1120	—	—	1025	—	—
						4.98	—	—	4.56	—	—
SS	H10A[9]	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290
						7.72	3.54	1.82	6.69	2.51	1.29
	H10AR	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1485	690	430	1220	570	305
						6.61	3.07	1.91	5.43	2.54	1.36
■	H10A-2	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1835	1275	430	1645	880	305
						8.16	5.67	1.91	7.32	3.91	1.36
	H10S[7,8]	18	(8) 8d x 1½"	(8) 8d x 1½"	(8) 8d	1465	795	315	1040	555	225
						6.52	3.54	1.40	4.63	2.51	1.00
■	H11Z	18	(6) 16d x 2½"	(6) 16d x 2½"	—	1095	920	545	780	655	390
						4.87	4.09	2.42	3.47	2.91	1.73
	H14	18	[1] (12) 8d x 1½"	(13) 8d	—	2390	855	320	1805	610	230
						10.63	3.80	1.42	8.03	2.71	1.02
			[2] (12) 8d x 1½"	(15) 8d	—	2390	855	320	1805	610	230
						10.63	3.80	1.42	8.03	2.71	1.02
	TSP	16	(9) 10d x 1½"	(6) 10d x 1½"	—	1295	440	—	920	310	—
						5.76	1.96	—	4.09	1.38	—
			(9) 10d x 1½"	(6) 10d	—	1560	440	—	1105	310	—
						6.94	1.96	—	4.92	1.38	—

1. Factored resistances have been increased 15% for short term loading; no further increase is allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
3. H8 factored uplift resistances for stud-to-bottom plate installations are 595 lb. (2.65 kN) for D.Fir-L and 390 lb. (1.74 kN) for S-P-F.
4. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
5. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-HTIECONPATH).
6. Factored resistances in the F_1 direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.
7. H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.38 kN) D.Fir-L and 1015 lb. (4.51 kN) S-P-F.
8. H10S nails to plates are optional for uplift but required for lateral loads.
9. H10A may be field-bent up to a slope of 6/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
10. The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SSNAILS at strongtie.com.
11. D.Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1 ½" nails into the top plates and (3) 8d x 1 ½" nails into the lowest three flange holes into the truss bottom chord is 495 lb. (2.20 kN).
12. **Nails:** 16d x 2½" = 0.162" dia. x 2½" long, 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long, 8d x 1½" = 0.131" dia. x 1½" long. See pp. 27–28 for other nail sizes and information.

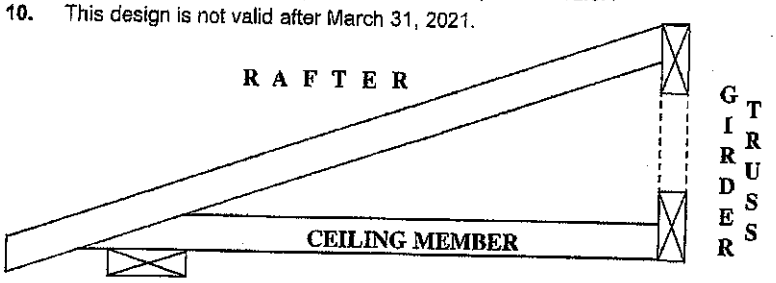
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

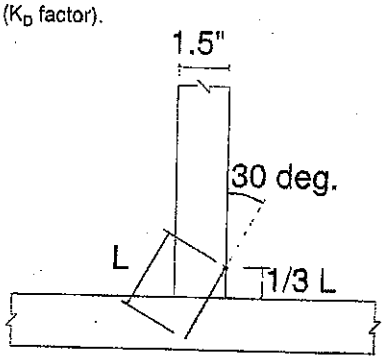
B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.





TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		**(3" and 3.25" nail)**	
LUMBER SIZE	**MAXIMUM NUMBER OF TOE-NAILS**			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

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Certificate No. 10889485

December 2, 2019

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

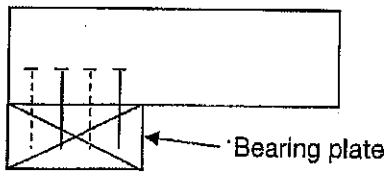
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

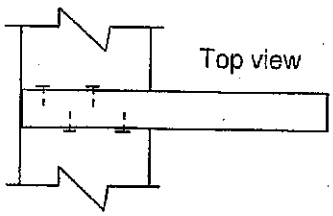
NOTES:

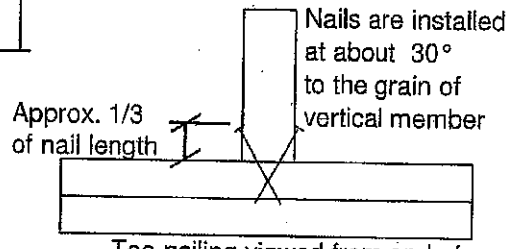
1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



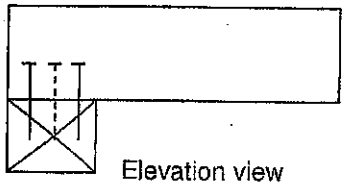
Elevation view



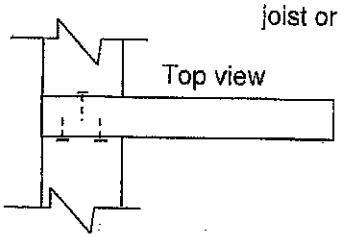


Toe-nailing viewed from end of joist or truss

Toe-nailing on 2x4 Bearing Plate



Elevation view



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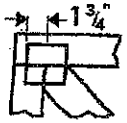
Bradford, Ontario L3Z 3G7

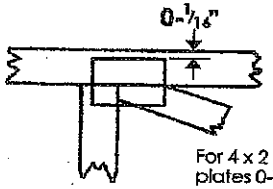
December 2, 2019

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

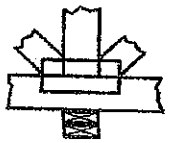
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

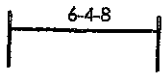


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

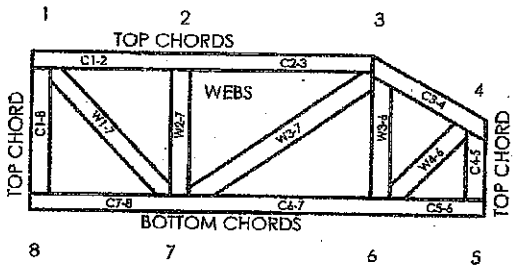
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

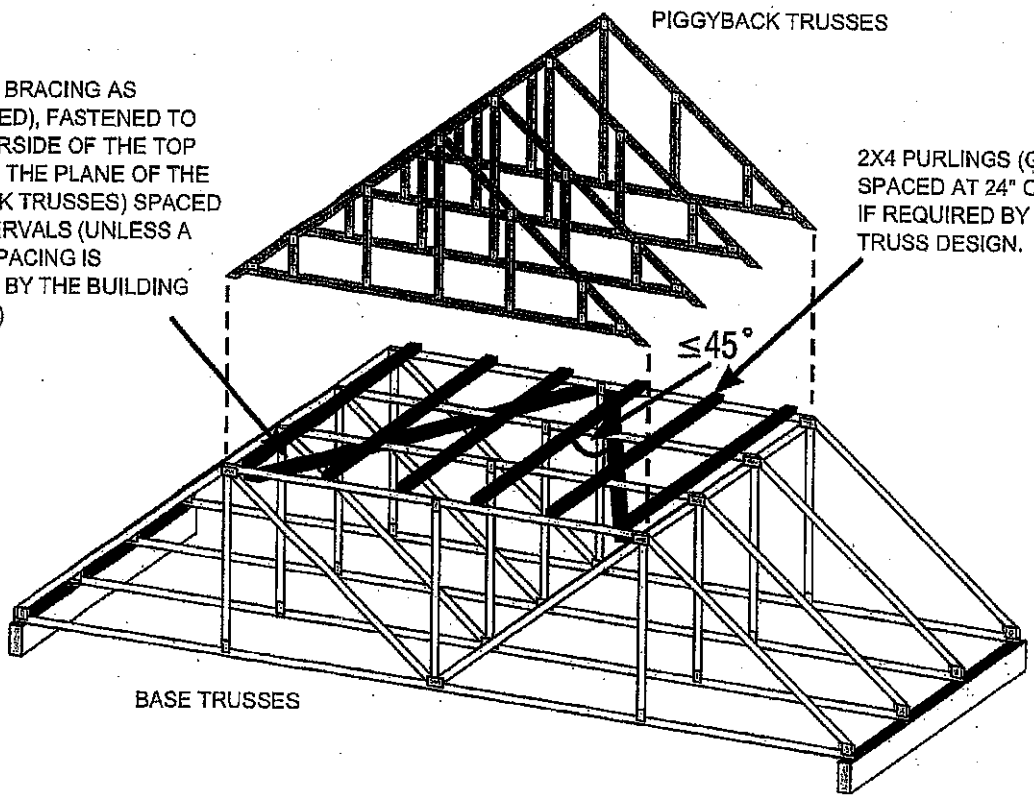
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

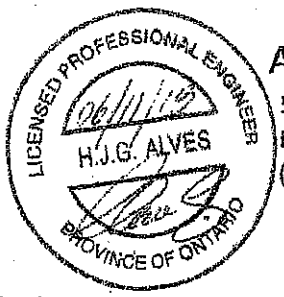
Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013



Alves Engineering Services Inc.
5208 Easton road
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(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-18002/18 Feb 09, 2018