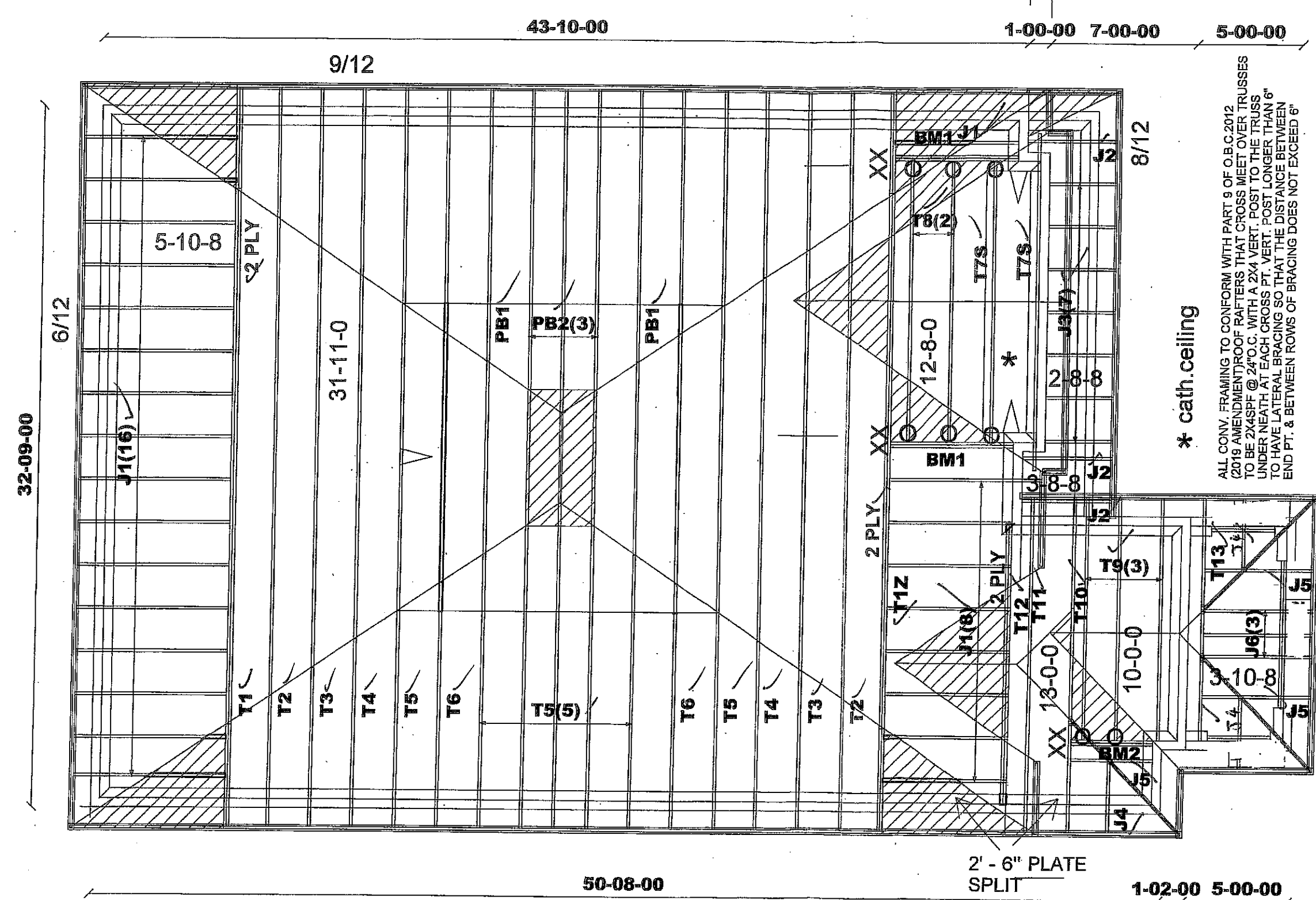




ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
HGUS26-2 - (XX)

DENOTES
CONV.
FRAMING

DESIGN CONFORMS
WITH O.B.C.2012 PART 9
(2019 AMENDMENT)
DESIGN LOADS:
SNOW LOAD 25.6 PSF
TC DEAD 6 PSF
BC LIVE 0 PSF
BC DEAD 7.4 PSF

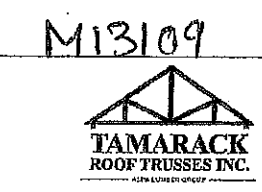
BM1, BM2 = 2X10
Building Division

Permit No. 187710

THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE

THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH
THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW

These drawings and/or specifications have been reviewed by
JDC
FOR CHIEF BUILDING OFFICIAL
DATE Dec 10/2020



Job Track: 51225
Plan Log: 202403
Layout ID: 408157

Builder / Location:
GREEN PARK HOMES / WATERDOWN
Project: RUSSELL GARDENS PH.3
Date: 2020-04-15 Sales: Mario DiCano Designer: ACJ/JG

Model / Elevation:
VALLEYCREEK 3/1

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

CITY OF HAMILTON
BUILDING DIVISION
Planning & Development Department
OCT 06 2020
REC BY _____ DATE _____
REF'D TO _____ DATE _____

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

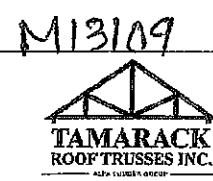
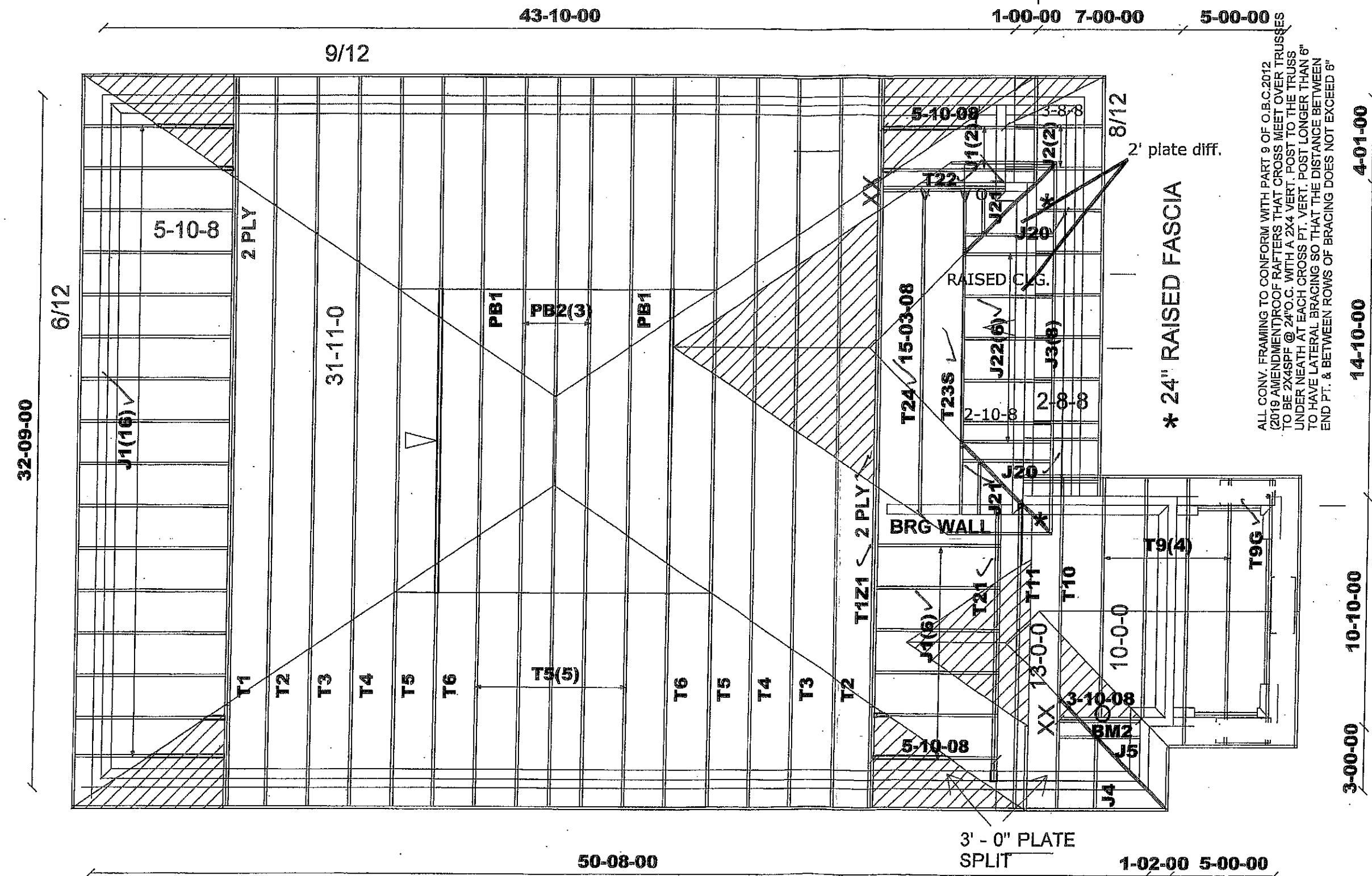
HARDWARE:
LUS24 - (O)
HGUS26-2 - (XX)

DENOTES
CONV.
FRAMING

DESIGN CONFORMS
WITH O.B.C.2012 PART 9
(2019 AMENDMENT)
DESIGN LOADS:
SNOW LOAD 25.6 PSF
TC DEAD 6 PSF
BC LIVE 0 PSF
BC DEAD 7.4 PSF

CITY OF HAMILTON
BM2 = 2-2X10 Building Division

Permit No. 187710
THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE
THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH
THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW
Milex ver 8.3.1.215
These drawings and/or specifications have been reviewed by
JDC
FOR CHIEF BUILDING OFFICIAL
DATE Dec 10/20



Job Track: 51225
Plan Log: 202403
Layout ID: 408158

Builder / Location:
GREEN PARK HOMES / WATERDOWN
Project: RUSSELL GARDENS PH.3
Date: 2020-04-15 Sales: Mario DiCano

Model / Elevation:
VALLEYCREEK 3/2

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

LUS26-2 - (V)



BC DEAD 7.4 PSF

CITY OF HAMILTON
Building Division

DAT8

FLAT ROOF

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE

Milek ver 8.3.1.215



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: VALLEYCREEK 3
 Lot #:
 Elevation: 1

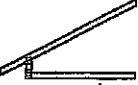
Job Track: 51225
 PlanLog: 202403
 Layout ID: 408157
 Ref #
 Page: 1 of 2
 Date: 04-15-2020
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	9/12	31-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	289.32 181.33		
	1 2-ply	T12 Hip Girder	9/12	31-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	289.32 181.33		
	2	T2 Hip	9/12	31-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	261.72 163.00		
	2	T3 Hip	9/12	31-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	270.76 172.33		
	2	T4 Hip	9/12	31-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	291.19 187.00		
	7	T5 Hip	9/12	31-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	1048.33 666.00		
	2	T6 Hip	9/12	31-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	312.24 194.33		
	2	T7S Scissor	9/12 6/12	13-02-00	6-03-04	2 x 4	1-03-08 1-03-08	6-04 6-04	104.77 70.00		
	2	T8 Common	9/12	12-08-00	6-05-08	2 x 4		1-08-08 1-08-08	110.86 72.67		
	3	T9 Common	9/12	9-11-08	5-03-04	2 x 4	1-03-12 1-03-12	1-06-04 1-06-04	131.2 86.50		
	1	T10 Hip Girder	9/12	13-00-00	4-05-02	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	65 42.83		
	1	T11 Hip	9/12	13-00-00	5-11-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	62.02 41.00		
	1 2-ply	T12 Flat	0/12	13-00-00	2-06-00	2 x 4		2-06-00 2-06-00	95.96 59.00		
	1	T13 Hip Girder	9/12	10-00-00	4-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	47.5 31.17		

 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: RUSSELL GARDENS PH.3 Location: WATERDOWN Model: VALLEYCREEK 3 Lot #: Elevation: 1	Job Track: 51225 PlanLog: 202403 Layout ID: 408157 Ref # Page: 2 of 2 Date: 04-15-2020 Designer: Andrew Conway Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	PB1 Piggyback	9/12	14-04-05	2-00-00	2 x 4			86.07 57.33		
	3	PB2 Piggyback	9/12	14-04-05	3-00-00	2 x 4			127.21 79.50		
	25	J1 Jack-Open	6/12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	419.86 266.67		
	3	J2 Jack-Open	8/12	3-08-08	3-08-03	2 x 4	1-03-08	4-07 2-03-13	35.4 24.00		
	7	J3 Jack-Open	8/12	2-08-08	3-00-03	2 x 4	1-03-08	4-07 1-07-13	65.04 42.00		
	3	J4 Jack-Open	9/12	1-10-15	2-11-07	2 x 4	1-03-08 1-01	1-06-04 2-11-07	29.94 22.50		
	3	J5 Jack-Open	9/12	1-10-15	2-11-07	2 x 4	1-03-08 1-11-09	1-06-04 2-11-07	35.56 25.00		
	3	J6 Jack-Open	9/12	3-10-08	4-05-02	2 x 4	1-03-08	1-06-04 4-05-02	45.15 30.50		

TOTAL # TRUSS= 80 TOTAL BFT OF ALL TRUSSES= 2694.99 BFT. TOTAL WEIGHT OF ALL TRSSES 4224.41 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
8	Hardware	LUS24	
3	Hardware	HGUS26-2	

TOTAL NUMBER OF
ITEMS= 11



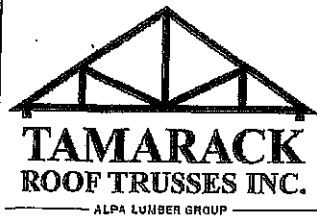
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: VALLEYCREEK 3
 Lot #:
 Elevation: 2

Job Track: 51225
 PlanLog: 202403
 Layout ID: 408158
 Ref #
 Page: 1 of 2
 Date: 04-15-2020
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	9/12	31-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	289.32 181.33		
	1 2-ply	T1Z1 Hip Girder	9/12	31-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	289.32 181.33		
	2	T2 Hip	9/12	31-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	281.72 163.00		
	2	T3 Hip	9/12	31-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	270.76 172.33		
	2	T4 Hip	9/12	31-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	284.63 183.00		
	7	T5 Hip	9/12	31-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	1048.33 665.00		
	2	T6 Hip	9/12	31-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	312.24 194.33		
	4	T9 Common	9/12	10-00-00	5-03-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	175.08 115.33		
	1	T9G GABLE	9/12	10-00-00	5-03-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	46.07 31.83		
	1	T10 Hip Girder	9/12	13-00-00	4-05-02	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	63.58 40.17		
	1	T11 Hip	9/12	13-00-00	5-11-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	62.02 41.00		
	1 2-ply	T21 Flat	0/12	13-00-00	3-00-00	2 x 4		3-00-00 3-00-00	101.03 63.67		
	1 2-ply	T22 Monopitch Girder	6/12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1	T23S Hip Girder	9/12	15-03-08	5-08-02	2 x 4		3-02-02 3-06-04	78.49 51.67		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: VALLEYCREEK 3
 Lot #:
 Elevation: 2

Job Track: 51225
 PlanLog: 202403
 Layout ID: 408158
 Ref #
 Page: 2 of 2
 Date: 04-15-2020
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	✓ T24 Hip	9/12	15-03-08	7-02-02	2 x 4		3-02-02 3-06-04	71.42 46.17		
	2	PB1 Piggyback	9/12	14-04-05	2-00-00	2 x 4			86.07 57.33		
	3	PB2 Piggyback	9/12	14-04-05	3-00-00	2 x 4			127.21 79.50		
	24	J1 Jack-Open	6/12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	403.07 256.00		
	2	J2 Jack-Open	8/12	3-08-08	3-08-03	2 x 4	1-03-08	4-07 2-03-13	23.6 16.00		
	8	J3 Jack-Open	8/12	2-08-08	3-00-03	2 x 4	1-03-08	4-07 1-07-13	74.33 48.00		
	1	J4 Jack-Open	9/12	1-10-15	2-11-07	2 x 4	1-03-08 1-01	1-06-04 2-11-07	9.72 7.00		
	1	J5 Jack-Open	9/12	3-10-08	2-11-07	2 x 4		1-06-04 4-05-02	10.3 7.50		
	2	J20 Jack-Open	9/12	1-10-15	4-11-07	2 x 4	1-03-08 11-09	3-06-04 4-11-07	28.7 20.00		
	2	J21 Jack-Open	9/12	2-05-08	4-11-07	2 x 4		3-02-02 5-00-04	24.49 17.33		
	6	J22 Jack-Open	9/12	2-10-08	3-08-02	2 x 4	1-03-08	1-06-04 3-08-02	74.9 53.00		

TOTAL # TRUSS= 83

TOTAL BFT OF ALL TRUSSES= 2728.82

BFT.

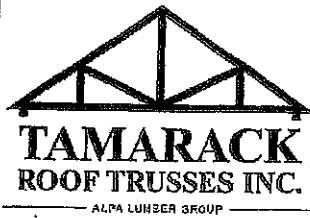
TOTAL WEIGHT OF ALL TRSSES 4274.01 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LUS24	
2	Hardware	HGUS26-2	

TOTAL NUMBER OF
ITEMS=

4



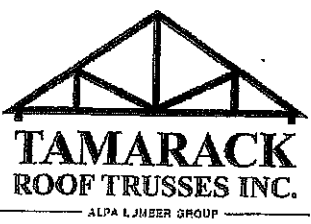
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: VALLEYCREEK 3
 Lot #:
 Elevation: 3

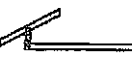
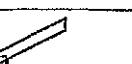
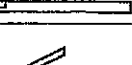
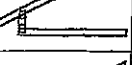
Job Track: 51225
 PlanLog: 202403
 Layout ID: 408159
 Ref #
 Page: 1 of 2
 Date: 04-15-2020
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T31 Hip Girder	6 /12	31-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	285.1 176.00		
	1 2-ply	T31Z Hip Girder	6 /12	31-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	285.1 176.00		
	2	T32 Hip	6 /12	31-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	259.03 161.67		
	2	T33 Hip	6 /12	31-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	258.25 161.33		
	2	T34 Hip	6 /12	31-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	271.21 168.33		
	2	T35 Hip	6 /12	31-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	262.57 165.00		
	7	T36 Hip	6 /12	31-11-00	9-01-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	904.65 565.83		
	1	T37 Roof Special Girder	6 /12	13-00-00	3-08-00	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	62.12 40.50		
	1	T38 Hip	6 /12	13-00-00	4-00-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	54.06 35.33		
	3	T39 Common	6 /12	13-00-00	4-05-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	155.89 96.00		
	3	T41 Common	4 /12	14-00-00	2-09-08	2 x 4		5-08 5-08	110.38 68.50		
	18	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	302.3 192.00		
	6	J31 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	42.13 28.00		
	5	J32 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	47.85 30.00		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP		DELIVERY SHIPLIST	
Lumber Yard:	TAMARACK LUMBER	Job Track:	51225
Builder:	GREEN PARK HOMES	PlanLog:	202403
Project:	RUSSELL GARDENS PH.3	Layout ID:	408159
Location:	WATERDOWN	Ref #	
Model:	VALLEYCREEK 3	Page:	2 of 2
Lot #:		Date:	04-15-2020
Elevation:	3	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	3	J33 Jack-Open	6 /12	5-10-08	2-00-12	2 x 4	1-03-08	1-02-00 4-01-04	34.75 22.00		
	3	J34 Jack-Open	6 /12	5-10-08	3-00-12	2 x 4	1-03-08	1-02-00 4-01-04	42.4 26.00		
	1	J35 Jack-Open	6 /12	3-09-08	2-01-08	2 x 4		1-02-00 3-00-12	7.72 5.33		
	2	J38 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 3-00-09	1-02-00 2-01-08	21.39 13.33		
	2	J39 Jack-Open	6 /12	3-10-15	3-01-08	2 x 4	1-03-08 1-00-09	1-02-00 3-01-08	26.49 16.00		
	3	J41 Jack-Open	6 /12	5-08-08	3-09-15	2 x 4	1-03-08	4-03 2-09-11	48.8 30.00		

TOTAL # TRUSS= 70

TOTAL BFT OF ALL TRUSSES= 2177.15

BFT.

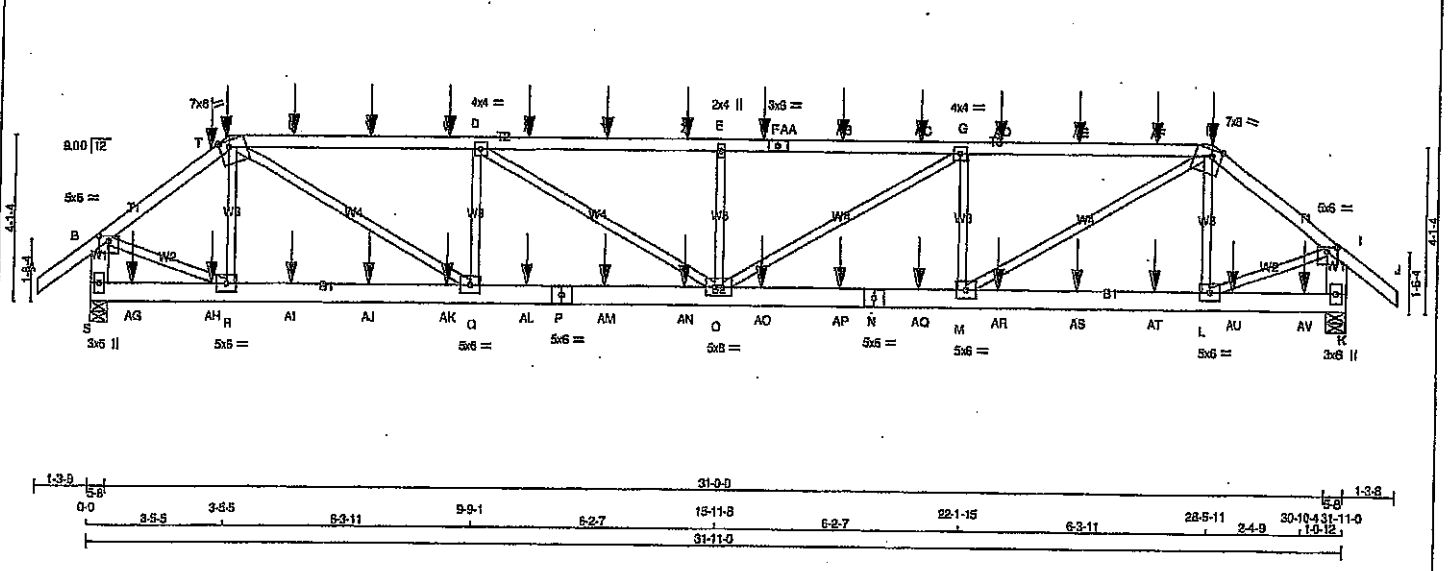
TOTAL WEIGHT OF ALL TRSSES 3482.19 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LJS26DS	
2	Hardware	LUS24	
2	Hardware	LUS26-2	

TOTAL NUMBER OF
ITEMS=

7



LUMBER

N. L. & A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	2100F 1.8E	SPF
F - H	2x4	DRY	2100F 1.8E	SPF
H - J	2x4	DRY	No.2	SPF
K - L	2x6	DRY	No.2	SPF
L - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF)

CHORDS	#ROWS	SURFACE	SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS				
A-C	1	12	SIDE(61.0)	
C-F	1	12	SIDE(65.9)	
F-H	1	12	SIDE(85.9)	
H-J	1	12	SIDE(81.0)	
K-L	2	12	TOP	
L-P	2	12	TOP	
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS				
S-P	2	12	SIDE(0.0)	
P-N	2	12	SIDE(0.0)	
N-K	2	12	SIDE(0.0)	

WEBS: (0.122"x3") SPIRAL NAILS

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT	TYPE	W	LEN	Y	X
B	TMWV-p	MT20	5.0	6.0	1.25 3.00
C	TTWV-m	MT20	7.0	8.0	Edge 3.00
D	TMWV-t	MT20	4.0	4.0	
E	TMWV-w	MT20	2.0	4.0	
F	TS-t	MT20	3.0	6.0	
G	TMWV-t	MT20	4.0	4.0	
H	TTWV-m	MT20	7.0	8.0	Edge 3.00
I	TMWV-p	MT20	5.0	6.0	1.25 3.00
K	BMV-t+p	MT20	3.0	6.0	
L, M, Q, R					
L	BMWV-t	MT20	5.0	6.0	
N	BS-t	MT20	5.0	6.0	
O	BMWVW-t	MT20	5.0	6.0	
P	BS-t	MT20	5.0	6.0	
S	BMV-t+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	UPLIFT	IN-SX
S 3148	0	0	5-8
K 3047	0	0	5-8

UNFACTORED REACTIONS

1ST CASE	MAX/MIN COMPONENT REACTIONS
JT COMBINED SNOW	LIVE PERM. LIVE WIND DEAD SOIL
S 2224 1474 / 0	0 / 0 0 / 0 750 / 0 0 / 0
K 2155 1417 / 0	0 / 0 0 / 0 738 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO			
A-B	0 / 38	-91.8	-91.8	0.07 (1)	R-C	-619 / 0	0.08 (1)	
B-T	-3265 / 0	-91.8	-91.8	0.17 (1)	C-Q	0 / 3382	0.42 (1)	
T-C	-3265 / 0	-91.8	-91.8	0.17 (1)	Q-D	-1538 / 0	0.20 (1)	
C-U	-5545 / 0	-91.8	-91.8	0.40 (1)	D-O	0 / 993	0.12 (1)	
U-V	-5545 / 0	-91.8	-91.8	0.40 (1)	O-E	-941 / 0	0.11 (1)	
V-W	-5545 / 0	-91.8	-91.8	0.40 (1)	E-G	0 / 1028	0.13 (1)	
W-D	-5545 / 0	-91.8	-91.8	0.40 (1)	G-M	-1562 / 0	0.20 (1)	
D-X	-6388 / 0	-91.8	-91.8	0.43 (1)	M-H	0 / 3429	0.42 (1)	
X-Y	-6388 / 0	-91.8	-91.8	0.43 (1)	L-H	-590 / 0	0.07 (1)	
Y-Z	-6388 / 0	-91.8	-91.8	0.43 (1)	H-R	0 / 2774	0.34 (1)	
Z-E	-6388 / 0	-91.8	-91.8	0.43 (1)	R-I	0 / 2690	0.33 (1)	
E-AA	-6388 / 0	-91.8	-91.8	0.43 (1)				
AA-F	-6388 / 0	-91.8	-91.8	0.43 (1)				
F-AB	-6388 / 0	-91.8	-91.8	0.43 (1)				
AB-AC	-6388 / 0	-91.8	-91.8	0.43 (1)				
AC-G	-6388 / 0	-91.8	-91.8	0.43 (1)				
G-AD	-5506 / 0	-91.8	-91.8	0.40 (1)				
AD-AE	-5506 / 0	-91.8	-91.8	0.40 (1)				
AE-AF	-5506 / 0	-91.8	-91.8	0.40 (1)				
AF-H	-5506 / 0	-91.8	-91.8	0.40 (1)				
H-I	-3217 / 0	-91.8	-91.8	0.16 (1)				
I-J	0 / 38	-91.8	-91.8	0.07 (1)				
S-B	-3127 / 0	0.0	0.0	0.11 (1)				
K-I	-3026 / 0	0.0	0.0	0.11 (1)				

PLATES

PLATE	TYPE	W	LEN	Y	X
S-AG	0 / 0	-18.5	-18.5	0.08 (4)	10.00
AG-AH	0 / 0	-18.5	-18.5	0.08 (4)	10.00
AH-F	0 / 0	-18.5	-18.5	0.08 (4)	10.00
F-AI	0 / 2630	-18.5	-18.5	0.21 (1)	10.00
AI-AJ	0 / 2630	-18.5	-18.5	0.21 (1)	10.00
AJ-AK	0 / 2630	-18.5	-18.5	0.21 (1)	10.00
AK-Q	0 / 2630	-18.5	-18.5	0.21 (1)	10.00
Q-AL	0 / 5545	-18.5	-18.5	0.42 (1)	10.00
AL-P	0 / 5545	-18.5	-18.5	0.42 (1)	10.00
P-AM	0 / 5545	-18.5	-18.5	0.42 (1)	10.00
AM-AN	0 / 5545	-18.5	-18.5	0.42 (1)	10.00
AN-O	0 / 5545	-18.5	-18.5	0.42 (1)	10.00
O-AO	0 / 5508	-18.5	-18.5	0.42 (1)	10.00
AO-AP	0 / 5508	-18.5	-18.5	0.42 (1)	10.00
AP-N	0 / 5508	-18.5	-18.5	0.42 (1)	10.00
N-AQ	0 / 5508	-18.5	-18.5	0.42 (1)	10.00
AQ-M	0 / 5508	-18.5	-18.5	0.42 (1)	10.00
M-AR	0 / 2550	-18.5	-18.5	0.21 (1)	10.00

TOTAL WEIGHT = 2 X 145 = 289 lb

DESIGN CRITERIA
*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TC10 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.08")
CALCULATED VERT. DEFL. (LL) = L/999 (0.15")
ALLOWABLE DEFL. (TL) = L/360 (1.08")
CALCULATED VERT. DEFL. (TL) = L/999 (0.28")

CSI: TC-0.43/1.00 (E-G:1), BC-0.42/1.00 (Q-O:1), WB-0.42/1.00 (H-M:1), SS-0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

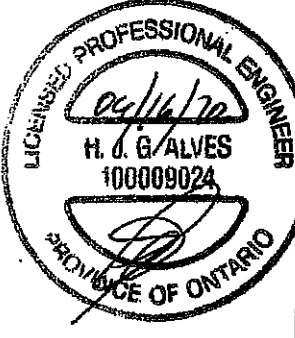
PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.83 (Q) (INPUT = 0.80)
JSI METAL=0.54 (N) (INPUT = 1.00)

Structural component only 1/2
DWG# T-2006181



LOADING
TOTAL LOAD CASES: (4)

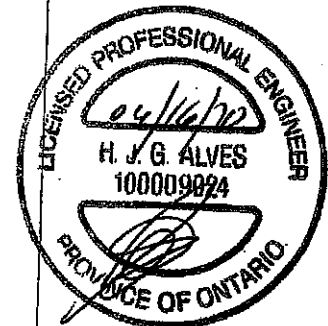
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	UNBRAG LENGTH FR-TO
FR-TO		FROM	TO				
AR-AS	0 / 2550	-18.5	-18.5 0.21 (1)	10.00			
AS-AT	0 / 2550	-18.5	-18.5 0.21 (1)	10.00			
AT-L	0 / 2550	-18.5	-18.5 0.21 (1)	10.00			
L-AU	0 / 0	-18.5	-18.5 0.05 (4)	10.00			
AU-AV	0 / 0	-18.5	-18.5 0.05 (4)	10.00			
AV-K	0 / 0	-18.5	-18.5 0.05 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-5-5	-44	-49	---	FRONT	VERT	DEAD	---	C1
C	3-5-5	-225	-225	---	FRONT	VERT	SNOW	---	C1
H	28-5-11	-44	-49	---	FRONT	VERT	DEAD	---	C1
H	28-5-11	-225	-225	---	FRONT	VERT	SNOW	---	C1
T	3-0-12	-139	-139	---	FRONT	VERT	TOTAL	---	C1
U	5-0-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	7-0-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	9-0-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	11-0-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	13-0-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	15-0-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	17-0-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	19-0-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AG	21-0-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	23-0-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AE	25-0-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AF	27-0-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AG	1-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AH	3-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AI	5-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AJ	7-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AK	9-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AL	11-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AM	13-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AN	15-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AO	17-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AP	19-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AQ	21-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AR	23-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AS	25-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AT	27-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AU	29-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AV	30-10-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1

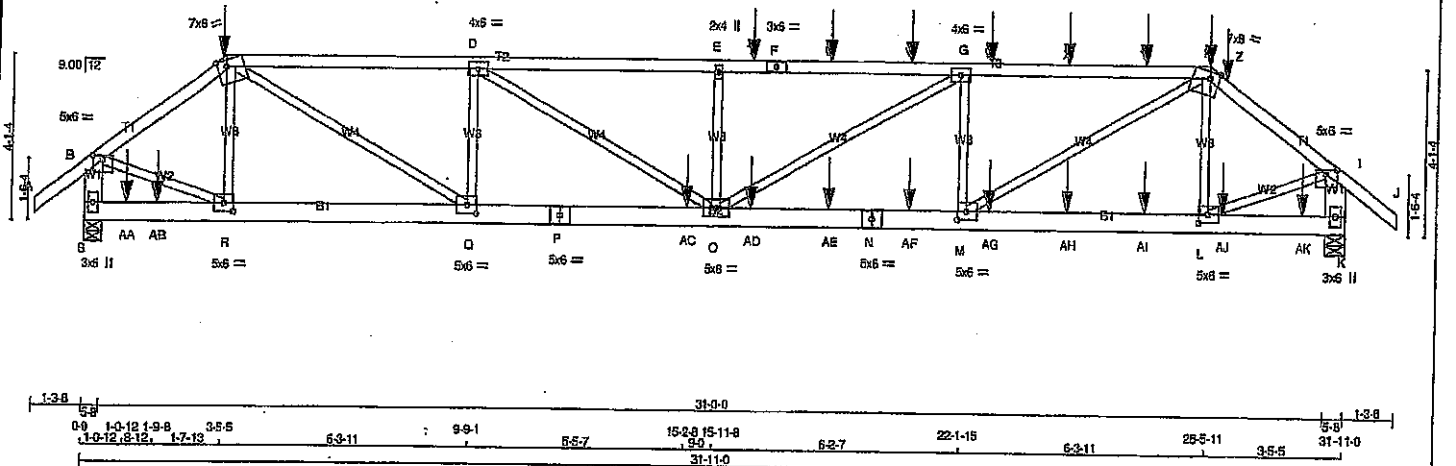
CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



JOB NAME 408157	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Floor Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2018 Mitek Industries, Inc. Wed Apr 15 12:13:25 2020 Page 1
 ID:7R4Dg_C9iAuONuKpIv?nzzlwC4-1NW1umbfTH7X6RoSc2U_TPKY97UzK3_PpWSD3zQUhC
 265-11 28-104 31-110 33-2-8 1-3-8
 Scale = 1/8"=1'-0"



LUMBER N.L.G.A. RULES CHORDS A - C 2x4 DRY No.2 C - F 2x4 DRY 2100F 1.8E F - H 2x4 DRY 2100F 1.8E H - J 2x4 DRY No.2 S - B 2x6 DRY No.2 K - I 2x6 DRY No.2 S - P 2x6 DRY No.2 P - N 2x6 DRY No.2 N - K 2x6 DRY No.2 ALL WEBS 2x3 DRY No.2 EXCEPT DRY, SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF) TOP CHORDS : (0.122"x3") SPIRAL NAILS A-C 1 12 SIDE (61.0) C-F 1 12 SIDE (65.9) F-H 1 12 SIDE (65.9) H-J 1 12 SIDE (61.0) S-B 2 12 TOP K-I 2 12 TOP BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS S-P 2 12 SIDE (0.0) P-N 2 12 SIDE (0.0) N-K 2 12 SIDE (0.0) WEBS : (0.122"x3") SPIRAL NAILS 2x3 1 6 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3/4 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY. SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP. PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMVW-p MT20 5.0 8.0 1.25 3.00 G TTWW-m MT20 7.0 8.0 Edge 3.00 D TMVW-t MT20 4.0 8.0 E TMVW-n MT20 2.0 4.0 F TS-t MT20 3.0 6.0 G TMVW-t MT20 4.0 8.0 H TTWW-m MT20 7.0 8.0 Edge 3.00 I TMVW-p MT20 5.0 6.0 1.25 3.00 K BMV1-p MT20 3.0 6.0 L, M, Q, R L BMVW-t MT20 5.0 8.0 2.50 2.75 N BS-t MT20 5.0 8.0 O BMVW-t MT20 5.0 8.0 P BS-t MT20 5.0 6.0 S BMV1-p MT20 3.0 6.0				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX S 4216 0 4216 0 0 5-8 5-8 K 3532 0 3532 0 0 5-8 5-8 UNFACTORED REACTIONS 1ST LCASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERMALIVE WIND DEAD SOIL S 3082 1457 / 0 0 / 0 0 / 0 1624 / 0 0 / 0 K 2532 1498 / 0 0 / 0 0 / 0 1064 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX (PLF) MAX. UNBRACED LENGTH FR-TO A-B 0 / 38 -91.8 -91.8 0.07 (1) 10.00 R-C -144 / 216 0.04 (4) B-C -3965 / 0 -91.8 -91.8 0.17 (1) 4.63 C-Q 0 / 3656 0.45 (1) C-D -5312 / 0 -91.8 -91.8 0.28 (1) 4.55 Q-D -1680 / 0 0.21 (1) D-E -8075 / 0 -91.8 -91.8 0.34 (1) 4.08 D-O 0 / 2057 0.25 (1) E-F -8075 / 0 -91.8 -91.8 0.50 (1) 3.89 C-E -590 / 0 0.09 (1) F-G -8075 / 0 -91.8 -91.8 0.50 (1) 3.89 C-G 0 / 1805 0.25 (4) G-H -8075 / 0 -91.8 -91.8 0.50 (1) 3.89 M-H -1981 / 0 0.25 (1) H-I -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 M-H 0 / 4098 0.51 (1) I-J -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 B-H -704 / 0 0.09 (1) J-K -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 L-I 0 / 3159 0.39 (1) K-L -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 L-M -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 M-N -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 N-O -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 O-P -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 P-Q -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 Q-R -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 R-S -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 S-T -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 T-U -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 U-V -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 V-W -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 W-X -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 X-Y -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 Y-Z -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 Z-A -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 A-B -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 B-C -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 C-D -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 D-E -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 E-F -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 F-G -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 G-H -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 H-I -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 I-J -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 J-K -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 K-L -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 L-M -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 M-N -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 N-O -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 O-P -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 P-Q -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 Q-R -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 R-S -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 S-T -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 T-U -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 U-V -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 V-W -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 W-X -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 X-Y -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 Y-Z -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 Z-A -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 A-B -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 B-C -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 C-D -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 D-E -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 E-F -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 F-G -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 G-H -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 H-I -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 I-J -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 J-K -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 K-L -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 L-M -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 M-N -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 N-O -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 O-P -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 P-Q -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 Q-R -8075 / 0 -91.8 -91.8 0.45 (1) 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4.30 K-L -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 L-M -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 M-N -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 N-O -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 O-P -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 P-Q -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 Q-R -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 R-S -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 S-T -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 T-U -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 U-V -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 V-W -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 W-X -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 X-Y -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 Y-Z -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 Z-A -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 A-B -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 B-C -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 C-D -8075 / 0 -91.8 -91.8 0.45 (1) 4.30 D-E -8075 / 0 -91.8 -91
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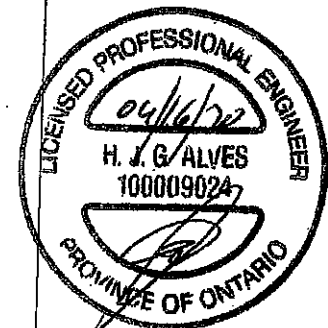
JOB NAME 408157	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 8 Oct 29 2019 MITek Industries, Inc. Wed Apr 15 12:13:25 2020 Page 2 ID:7R4Dg C8IAuIONuKpIv?nzzlwQ4-1NW1umbfITH7X6RoSc2U TPKY97Uzk3 PoWSD3zQUhC	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	28-5-11	-44	-49	---	FRONT	VERT	DEAD	---	C1
H	28-5-11	-225	-225	---	FRONT	VERT	SNOW	---	C1
T	18-10-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
U	18-10-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
V	20-10-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
W	22-10-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
X	24-10-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
Y	26-10-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
Z	28-10-4	-139	-139	---	BACK	VERT	TOTAL	---	C1
AA	1-0-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AB	1-9-8	-1200	-1200	---	BACK	VERT	TOTAL	---	C1
AC	15-2-8	-1200	-1200	---	BACK	VERT	TOTAL	---	C1
AD	16-10-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AE	18-10-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AF	20-10-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AG	22-10-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AH	24-10-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AI	26-10-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AJ	28-10-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AK	30-10-4	-26	-26	---	BACK	VERT	TOTAL	---	C1

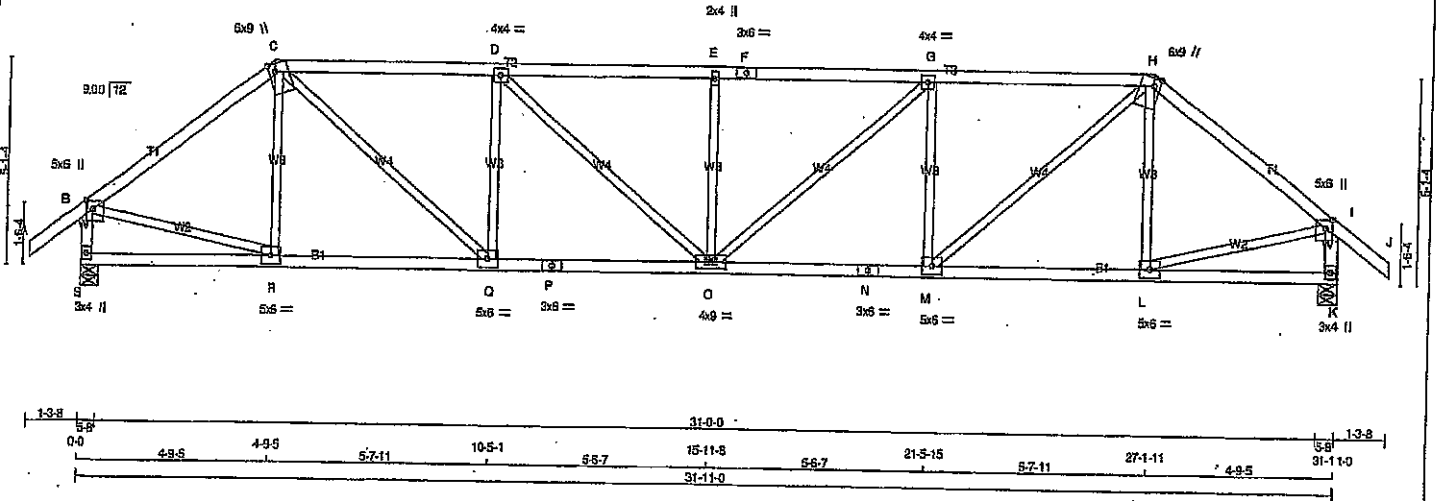
CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



JOB NAME 408157	TRUSS NAME T2	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Wed Apr 15 12:13:26 2020 Page 1
 ID:7R4Dg_C9IAuicNUKpiv?nzzlwO4-WZ4P68cHTnPs8G0_0JZjWhxIGZV4pn7eTF?IWzQUhN
 1-3-8 0-0 4-9-5 4-9-5 5-7-11 10-5-1 5-6-7 15-11-8 5-6-7 21-5-15 5-7-11 27-1-11 4-9-5 31-1-10 1-3-8
 Scale = 1:52.6



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - J	2x4	DRY No.2	SPF
K - B	2x4	DRY No.2	SPF
K - I	2x4	DRY No.2	SPF
S - P	2x4	DRY No.2	SPF
P - N	2x4	DRY No.2	SPF
N - K	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV+p	MT20	5.0	6.0	2.50	2.00
C	TTWW+m	MT20	6.0	9.0	Edge	2.00
D	TMWV+m	MT20	4.0	4.0		
E	TMWV+w	MT20	2.0	4.0		
F	TS-1	MT20	3.0	6.0		
G	TMWV-1	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	8.0	Edge	2.00
I	TMWV+p	MT20	5.0	6.0	2.50	2.00
K	BMV1+p	MT20	3.0	4.0		
L, M, Q, R						
L	BMWV-1	MT20	5.0	6.0		
N	BS-1	MT20	3.0	6.0		
O	BMWVW-1	MT20	4.0	9.0		
P	BS-1	MT20	3.0	6.0		
S	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
S	1886	0	1886	0
K	1886	0	1886	0

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERMALIVE	WIND	DEAD	SOIL
S	1331	887 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0
K	1331	887 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 38	-91.8	-91.8 0.12 (1)	10.00	R-C	-259 / 0	0.10 (1)
B-C	-1889 / 0	-91.8	-91.8 0.47 (1)	4.39	C-Q	0 / 1485	0.33 (1)
C-D	-2634 / 0	-91.8	-91.8 0.60 (1)	3.68	Q-D	-861 / 0	0.33 (1)
D-E	-2962 / 0	-91.8	-91.8 0.63 (1)	3.46	D-O	0 / 434	0.10 (1)
E-F	-2962 / 0	-91.8	-91.8 0.63 (1)	3.46	O-E	-468 / 0	0.18 (1)
F-G	-2962 / 0	-91.8	-91.8 0.63 (1)	3.46	C-G	0 / 434	0.10 (1)
G-H	-2634 / 0	-91.8	-91.8 0.60 (1)	3.68	M-G	-861 / 0	0.33 (1)
H-I	-1889 / 0	-91.8	-91.8 0.47 (1)	4.39	M-H	0 / 1485	0.33 (1)
I-J	0 / 38	-91.8	-91.8 0.12 (1)	10.00	L-H	-259 / 0	0.10 (1)
S-B	-1851 / 0	0.0	0.0 0.19 (1)	6.16	B-R	0 / 1552	0.35 (1)
K-I	-1851 / 0	0.0	0.0 0.19 (1)	6.16	L-I	0 / 1552	0.35 (1)
S-R	0 / 0	-18.5	-18.5 0.12 (4)	10.00			
R-Q	0 / 1502	-18.5	-18.5 0.31 (1)	10.00			
Q-P	0 / 2634	-18.5	-18.5 0.47 (1)	10.00			
P-O	0 / 2634	-18.5	-18.5 0.47 (1)	10.00			
O-N	0 / 2634	-18.5	-18.5 0.47 (1)	10.00			
N-M	0 / 2634	-18.5	-18.5 0.47 (1)	10.00			
M-L	0 / 1502	-18.5	-18.5 0.31 (1)	10.00			
L-K	0 / 0	-18.5	-18.5 0.12 (4)	10.00			

TOTAL WEIGHT = 2 X 131 = 262 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.08")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.15")
ALLOWABLE DEFL. (TL) = $L/360$ (1.08")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.27")

CSI: TC=0.63/1.00 (D-E-1), BC=0.47/1.00 (M-O-1),
WB=0.35/1.00 (I-L-1), SS=0.24/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

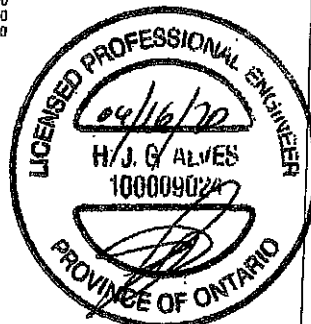
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1867

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

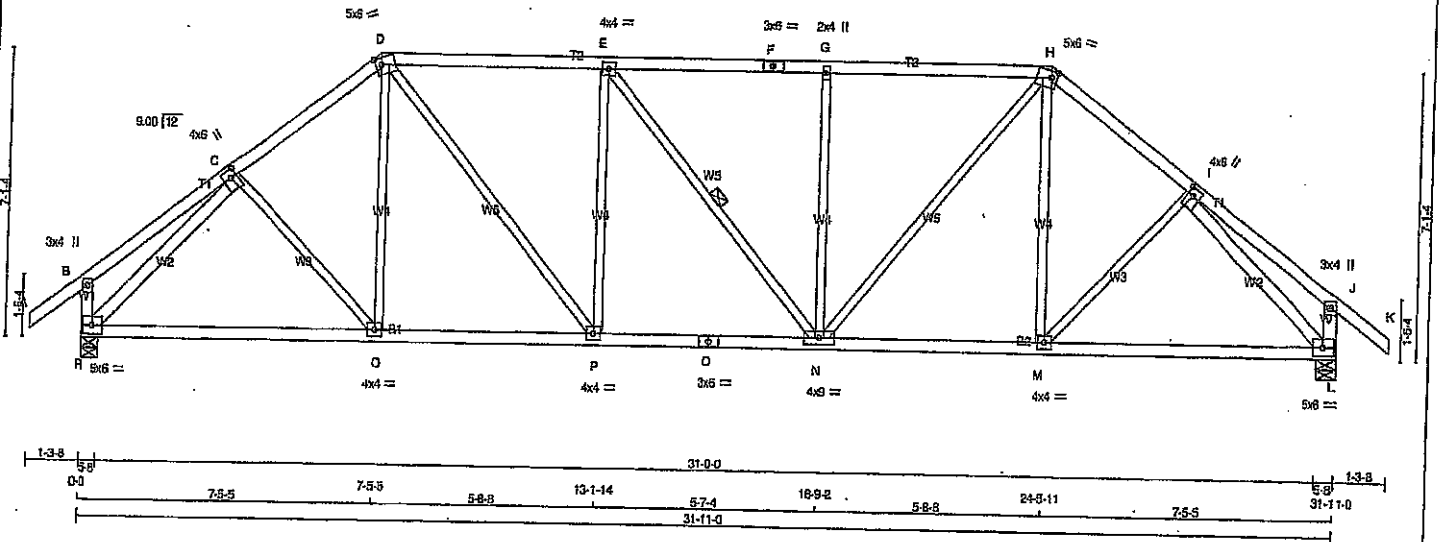
SI GRIP= 0.90 (I) (INPUT= 0.90)
SI METAL= 0.80 (P) (INPUT= 1.00)



Structural component only
DWG# T-2006183

Structural component only
DWG# T-2006184

JOB NAME 408157	TRUSS NAME T4	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Wed Apr 15 12:13:28 2020 Page 1	
ID:7R4Dg_C9iAuiOnuKplv?nzzlwO4-SyBAWndX_OgaOZ9N7kbBc51EcMBxAcGQ5mk6pOzQUhL				10-9-2 20-5-11 37-7 29-1-1 39-15 31-11-0 33-2-4 1-3-8	
1-3-8 0-0 3-9-15 3-9-15 3-7-7 7-5-5 5-9-8 13-1-14 5-7-4 10-9-2 5-9-8 20-5-11 37-7 29-1-1 39-15 31-11-0 33-2-4 1-3-8				Scale = 1:82.5	



LUMBER			
N.L.G.A. RULES			
CHORDS	SIZE	LUMBER	DESGR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
R - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW+t	MT20	4.0	8.0	2.25 1.75
D	TTWW-m	MT20	5.0	6.0	2.00 1.75
E	TMVW+t	MT20	4.0	4.0	
F	TS-t	MT20	3.0	6.0	
G	TMVW+t	MT20	2.0	4.0	
H	TTWW-m	MT20	5.0	6.0	2.00 1.75
I	TMVW+t	MT20	4.0	6.0	2.25 1.75
J	TMV+p	MT20	3.0	4.0	
L	BMVW1-t	MT20	5.0	6.0	
M, P, Q					
M	BMVW1-t	MT20	4.0	4.0	
N	BMVW1-t	MT20	4.0	9.0	
O	BS-t	MT20	3.0	6.0	
R	BMVW1-t	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG	
R	VERT	DOWN	DOWN	UP/LIFT	IN-SX
L	1888	0	1888	0	5-8
L	1888	0	1888	0	5-8

UNFACTORED REACTIONS		1ST CASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
R	1331	887/0	0/0
L	1331	887/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)			
R-TO		FROM	TO			FR-TO					
A-B	0/38	-91.8	-91.8	0.12 (1)	10.00	C-Q	0/61	0.82 (4)			
B-C	0/23	-91.8	-91.8	0.19 (1)	10.00	Q-D	0/118	0.04 (4)			
C-D	-1891/0	-91.8	-91.8	0.25 (1)	4.64	D-P	0/857	0.19 (1)			
D-E	-2044/0	-91.8	-91.8	0.52 (1)	4.18	P-E	-562/0	0.49 (1)			
E-F	-2043/0	-91.8	-91.8	0.51 (1)	4.18	E-N	-2/0	0.00 (1)			
F-G	-2043/0	-91.8	-91.8	0.51 (1)	4.18	N-G	-561/0	0.49 (1)			
G-H	-2043/0	-91.8	-91.8	0.51 (1)	4.19	H-M	0/854	0.19 (1)			
H-I	-1891/0	-91.8	-91.8	0.25 (1)	4.64	M-H	0/119	0.04 (4)			
I-J	0/23	-91.8	-91.8	0.19 (1)	10.00	M-I	0/61	0.02 (4)			
J-K	0/38	-91.8	-91.8	0.12 (1)	10.00	R-C	-2146/0	0.73 (1)			
R-B	-259/0	0.0	0.0	0.03 (1)	7.81	I-L	-2147/0	0.73 (1)			
L-J	-259/0	0.0	0.0	0.03 (1)	7.81						
R-Q	0/1464	-18.5	-18.5	0.36 (1)	10.00						
Q-P	0/1493	-18.5	-18.5	0.37 (1)	10.00						
P-O	0/2044	-18.5	-18.5	0.38 (1)	10.00						
O-N	0/2044	-18.5	-18.5	0.39 (1)	10.00						
N-M	0/1493	-18.5	-18.5	0.37 (1)	10.00						
M-L	0/1464	-18.5	-18.5	0.36 (1)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.5 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 8 OF CBC 2012 (2019 AMENDMENT)
- CSA C86-09, CSA C86-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (1.06")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.09")
ALLOWABLE DEFL.(TL) = 1/360 (1.06")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.17")
CS1: TC=0.52/1.00 (D-E:1), BC=0.39/1.00 (N-P:1), WB=0.73/1.00 (L-I:1), SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

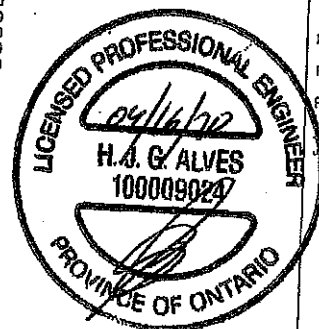
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 708 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CS1 GRIP = 0.89 (P) (INPUT = 0.90)
CS1 METAL = 0.94 (I) (INPUT = 1.00)



Structural component only
DWG# T-2006185

Tamarack Roof Truss, Burlington

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1-3-B 0-0 4-5-15 4-5-15 6-9-5 15-11-8

ID:7R4Dg_C9IAvIOuNuKpIv?mzzlwO4-w8Yk7e9lioR?jkZnR7QBjZMamYtv9OaKQIuMzQzUJhK

1-3-B 4-5-15 7-2-3 23-1-11 4-3-7 27-5-1 4-5-15 31-11-0 33-2-8

1-3-B 4-5-15 7-2-3 23-1-11 4-3-7 27-5-1 4-5-15 31-11-0 33-2-8

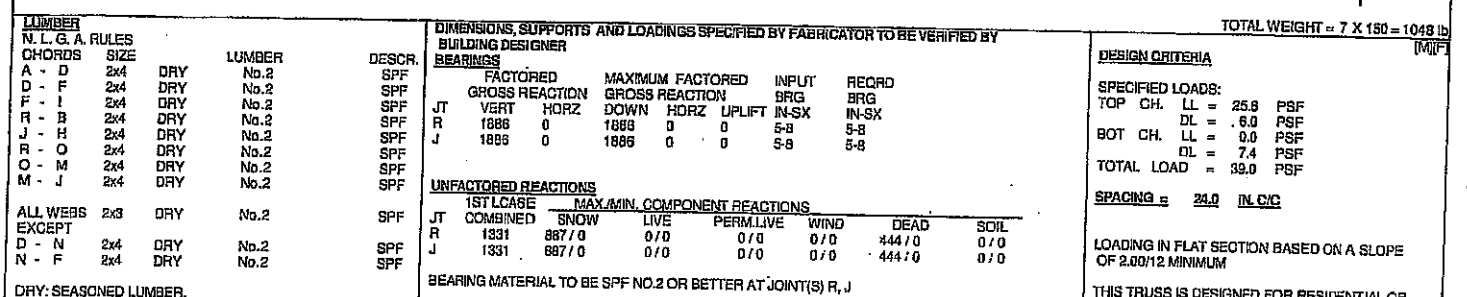
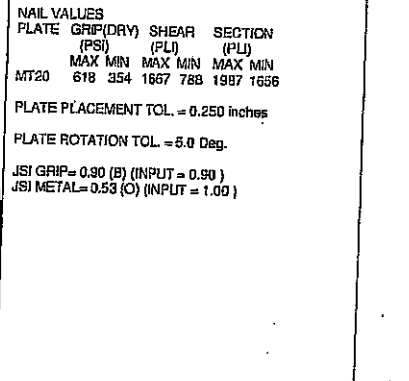
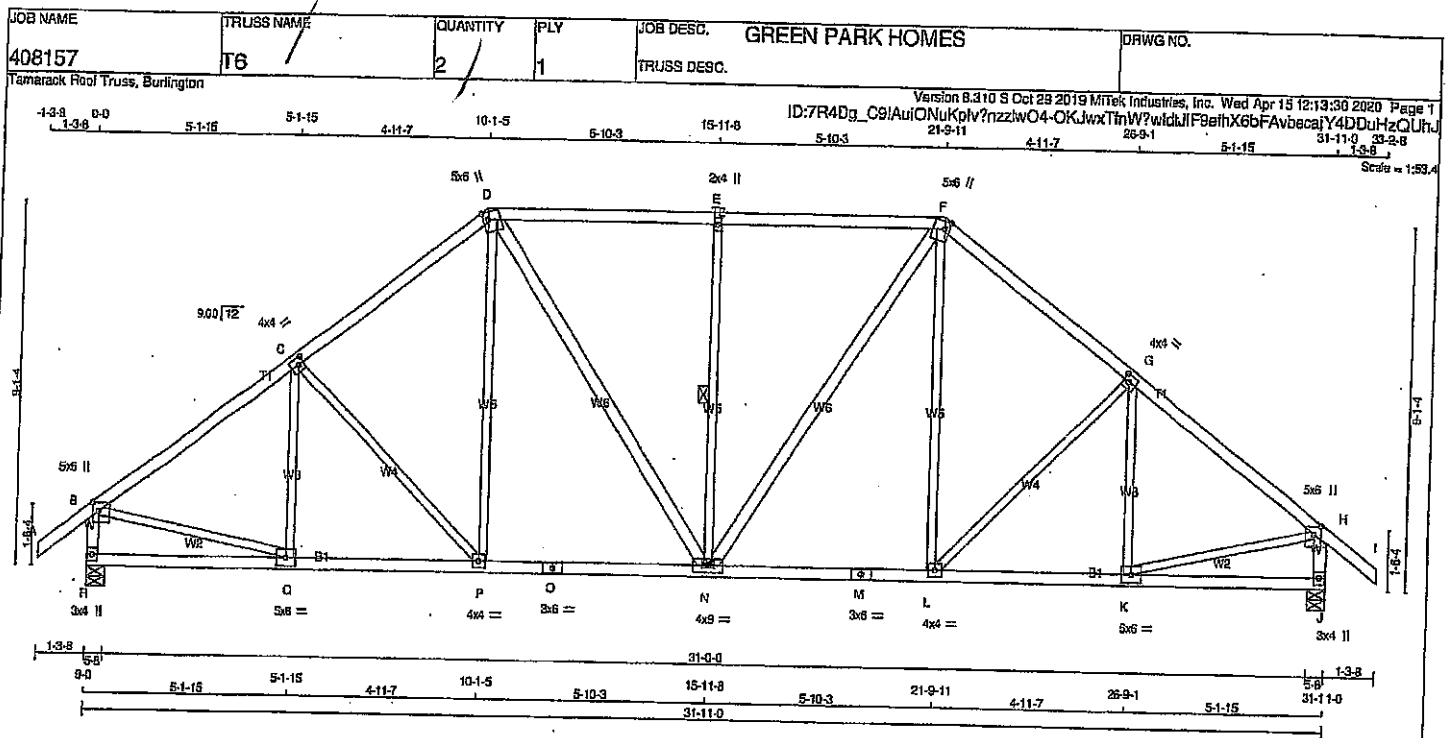
[illegible]

PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.53 (C) (INPUT = 1.00)



Structural component only
DWG# T-2006186



TOTAL WEIGHT = 2 X 156 = 312 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UP	IN-SX
R	1888	0	0	5-8
J	1888	0	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
R	1331	887 / 0 0 / 0 0 / 0 444 / 0 0 / 0
J	1331	887 / 0 0 / 0 0 / 0 444 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 38	-91.8 -91.8 0.12 (1)	10.00	C-C	-262 / 0	0.11 (1)	
B-C	-1924 / 0	-91.8 -91.8 0.37 (1)	4.51	C-P	-256 / 0	0.25 (1)	
C-D	-1767 / 0	-91.8 -91.8 0.35 (1)	4.68	P-D	0 / 282	0.06 (1)	
D-E	-1642 / 0	-91.8 -91.8 0.44 (1)	4.67	D-N	0 / 459	0.07 (1)	
E-F	-1767 / 0	-91.8 -91.8 0.44 (1)	4.67	N-E	-656 / 0	0.35 (1)	
F-G	-1767 / 0	-91.8 -91.8 0.37 (1)	4.51	N-F	0 / 459	0.07 (1)	
G-H	-1924 / 0	-91.8 -91.8 0.37 (1)	4.51	L-F	0 / 282	0.06 (1)	
H-I	0 / 38	-91.8 -91.8 0.12 (1)	10.00	L-G	-256 / 0	0.25 (1)	
R-B	-1845 / 0	0.0 0.0 0.19 (1)	6.18	K-G	-262 / 0	0.11 (1)	
J-H	-1845 / 0	0.0 0.0 0.19 (1)	6.18	B-Q	0 / 1807	0.36 (1)	
R-D	0 / 0	-18.5 -18.5 0.11 (4)	10.00	K-H	0 / 1807	0.36 (1)	
Q-P	0 / 1565	-18.5 -18.5 0.31 (1)	10.00				
P-O	0 / 1398	-18.5 -18.5 0.29 (1)	10.00				
O-N	0 / 1368	-18.5 -18.5 0.29 (1)	10.00				
N-M	0 / 1388	-18.5 -18.5 0.29 (1)	10.00				
M-L	0 / 1368	-18.5 -18.5 0.29 (1)	10.00				
L-K	0 / 1565	-18.5 -18.5 0.31 (1)	10.00				
K-J	0 / 0	-18.5 -18.5 0.11 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 085-09, CSA 085-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.06')
CALCULATED VERT. DEFL. (LL) = U/999 (0.07')
ALLOWABLE DEFL. (TL) = L/360 (1.06')
CALCULATED VERT. DEFL. (TL) = U/999 (0.13')

CSK: TC=0.44/1.00 (D-E-1), BC=0.31/1.00 (K-L-1), WB=0.35/1.00 (H-K-1), SE=0.26/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

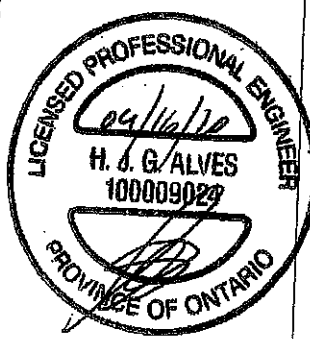
NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP = 0.89 (B) (INPUT = 0.90)

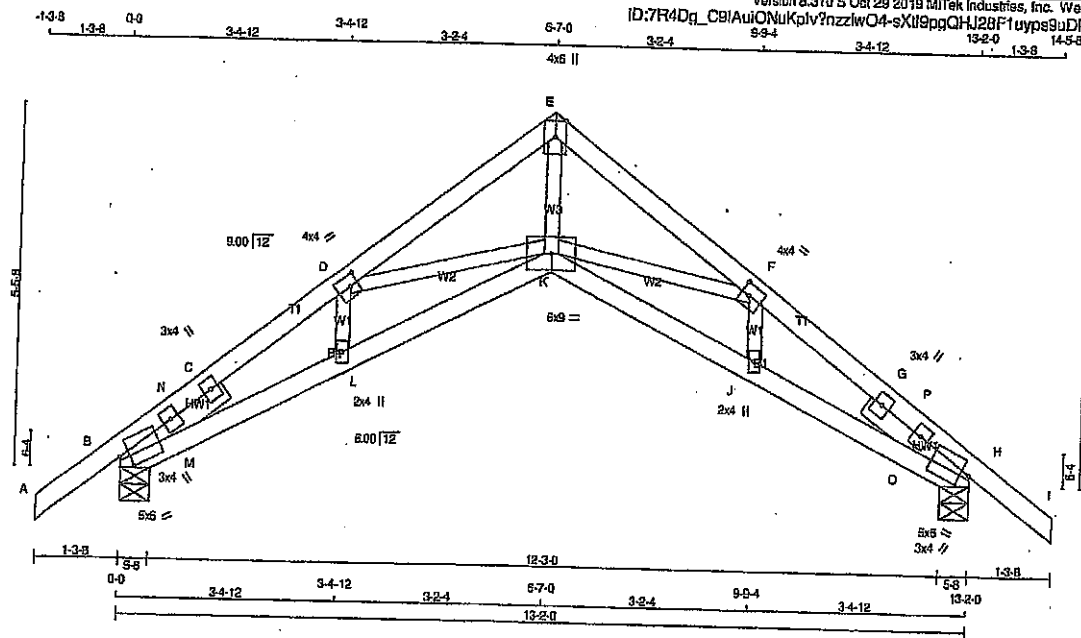
SI METAL = 0.44 (C) (INPUT = 1.00)



Structural component only
DWG# T-2006187

JOB NAME 408157	TRUSS NAME T7S	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington



Scale: 3/8\"/>

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF	
E - I	2x4	DRY	No.2	SPF	
B - K	2x4	DRY	No.2	SPF	
K - H	2x4	DRY	No.2	SPF	

REINFORCING MEMBERS	SIZE	LUMBER	DESCR.
HW1	2x4	DRY	No.2
HW2	2x4	DRY	No.2

ALL WEBS	SIZE	LUMBER	DESCR.
2x3	DRY	No.2	SPF

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B, B, H, H	MT20	5.0	6.0	2.75	1.50	
B, B, H, H	MT20	3.0	4.0			
D, TMWW-H	MT20	4.0	4.0	2.00	1.50	
E, TTW-p	MT20	4.0	4.0	Edge		
F, TMWW-H	MT20	4.0	4.0	2.00	1.50	
H, TMWW-H	MT20	5.0	6.0	2.75	1.50	
J, BMW-w	MT20	2.0	4.0			
K, BSWW-p	MT20	6.0	9.0			
L, BMW-w	MT20	2.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

<u>BEARINGS</u>		FACTORED		MAXIMUM FACTORED			INPUT	REQD
	GROSS REACTION		GROSS REACTION			BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
B	852	0	852	0	0	5-8	5-8	
H	852	0	852	0	0	5-8	5-8	

UNFACTORED REACTIONS							
1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT.	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	600	407 / 0	0 / 0	0 / 0	0 / 0	193 / 0	0 / 0
H	600	407 / 0	0 / 0	0 / 0	0 / 0	193 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. FACTORED CS1 (LC)	MAX. UNBRACED LENGTH	FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)
A-B	0 / 26	-91.8	-91.8	0.12 (1)	10.00	K-E	0 / 1448	0.33 (1)	
B-N	-1620 / 0	-91.8	-91.8	0.07 (1)	5.15	K-F	-230 / 0	0.05 (1)	
N-C	-1389 / 0	-91.8	-91.8	0.17 (1)	5.35	J-F	-20 / 41	0.01 (4)	
C-D	-1389 / 0	-91.8	-91.8	0.17 (1)	5.35	D-K	-230 / 0	0.05 (1)	
D-E	-1454 / 0	-91.8	-91.8	0.18 (1)	5.25	L-D	-20 / 41	0.01 (4)	
E-F	-1454 / 0	-91.8	-91.8	0.18 (1)	5.25	M-N	0 / 828	0.00 (1)	
F-G	-1389 / 0	-91.8	-91.8	0.17 (1)	5.35	M-C	-937 / 0	0.10 (1)	
G-P	-1389 / 0	-91.8	-91.8	0.17 (1)	5.35	O-P	0 / 828	0.00 (1)	
P-H	-1620 / 0	-91.8	-91.8	0.07 (1)	5.15	O-G	-937 / 0	0.10 (1)	
H-I	0 / 26	-91.8	-91.8	0.12 (1)	10.00				
B-M	0 / 1174	-18.5	-18.5	0.28 (1)	10.00				
M-L	0 / 1537	-18.5	-18.5	0.34 (1)	10.00				
L-K	0 / 1568	-18.5	-18.5	0.34 (1)	10.00				
K-J	0 / 1568	-18.5	-18.5	0.34 (1)	10.00				
J-O	0 / 1537	-18.5	-18.5	0.34 (1)	10.00				
O-H	0 / 1174	-18.5	-18.5	0.28 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.44")
CALCULATED VERT. DEFL. (LL) = L/989 (0.08")
ALLOWABLE DEFL. (TL) = L/360 (0.44")
CALCULATED VERT. DEFL. (TL) = L/989 (0.15")

CSI: TC=0.18/1.00 (E-F:1), BC=0.34/1.00 (J-K:1),
WB=0.33/1.00 (E-K:1), SS=0.26/1.00 (F-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

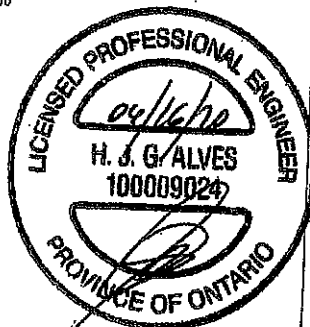
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

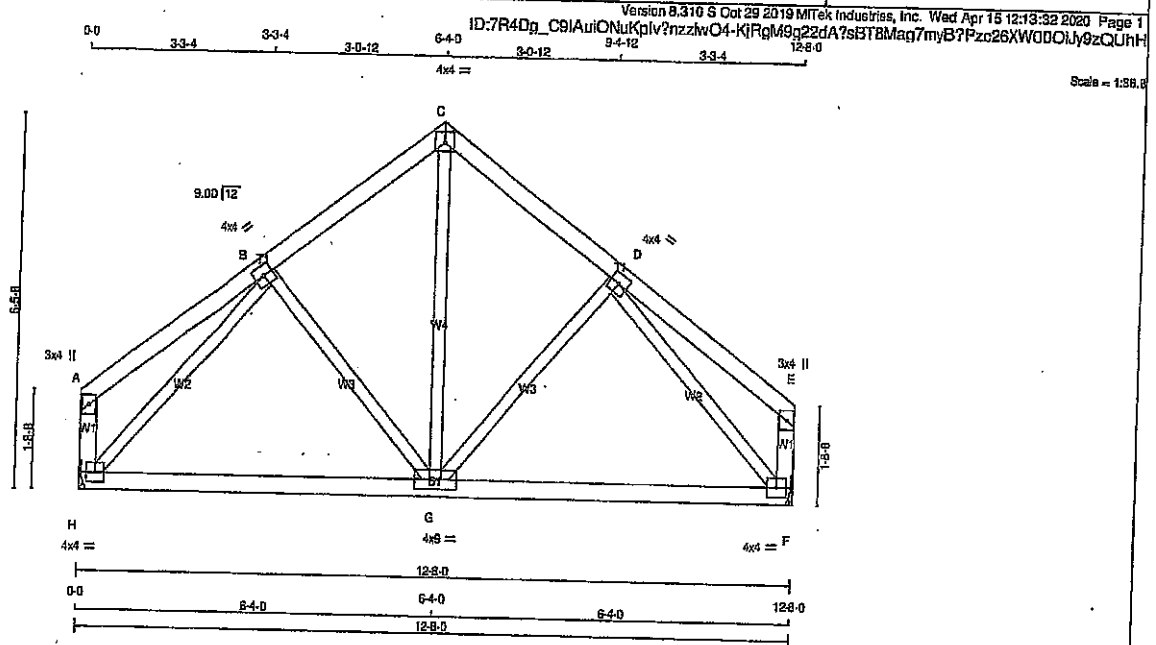
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (K) (INPUT = 0.90)
JSI METAL= 0.60 (H) (INPUT = 1.00)



Structural component only
DWG# T-2006188

JOB NAME 408157	TRUSS NAME T8	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - A	2x4	DRY	No.2
F - E	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT TYPE	PLATES	W	LEN	Y	X
A	TMV+p MT20	3.0	4.0		
B	TMVW-t MT20	4.0	4.0		
C	TTV-p MT20	4.0	4.0	2.25	2.00
D	TMVW-t MT20	4.0	4.0		
E	TMV+p MT20	3.0	4.0		
F	BMVW-t MT20	4.0	4.0		
G	BMVW-t MT20	4.0	4.0		
H	BMVW-t MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	BRG	IN-SX	IN-SX
H	698	0	398	0	0	MECHANICAL	MECHANICAL		
F	698	0	398	0	0	MECHANICAL	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, F. MINIMUM BEARING LENGTH AT JOINT H = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	494	324 / 0	0 / 0	0 / 0	0 / 0	170 / 0	0 / 0
F	494	324 / 0	0 / 0	0 / 0	0 / 0	170 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PL)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	MAX. CSI (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 20	-91.8 -91.8 0.14 (1)	10.00	G-C	0 / 334	0.08 (1)	
B-C	-489 / 0	-91.8 -91.8 0.11 (1)	6.25	G-D	-125 / 0	0.05 (1)	
C-D	-489 / 0	-91.8 -91.8 0.11 (1)	6.25	B-G	-125 / 0	0.05 (1)	
D-E	0 / 20	-91.8 -91.8 0.14 (1)	10.00	H-B	-708 / 0	0.27 (1)	
H-A	-111 / 0	0.0 0.0 0.01 (1)	7.81	D-F	-708 / 0	0.27 (1)	
F-E	-111 / 0	0.0 0.0 0.01 (1)	7.81				
H-G	0 / 458	-18.5 -18.5 0.25 (4)	10.00				
G-F	0 / 458	-18.5 -18.5 0.25 (4)	10.00				

TOTAL WEIGHT = 2 X 55 = 111 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.8 PSF
DL	= 5.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 81.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.42")
CALCULATED VERT. DEFL.(LL) = $L/899$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.42")
CALCULATED VERT. DEFL.(TL) = $L/899$ (0.04")

CSI: TC=0.14/1.00 (D-E:1), BC=0.25/1.00 (G-H:4),
WB=0.27/1.00 (D-F:1), SS=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1667 768	1967 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.77 (B) (INPUT = 0.90)
JSI METAL = 0.26 (C) (INPUT = 1.00)

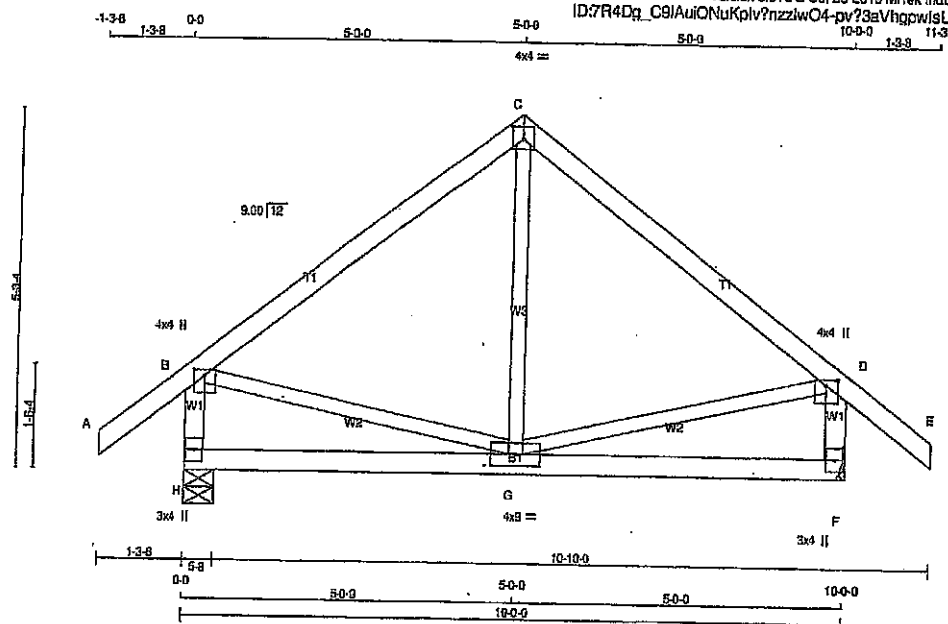


Structural component only
DWG# T-2006189

JOB NAME 408157	TRUSS NAME T9	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 44 = 132 lb (M/F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
H - B	2x4	DRY No.2	SPF
F - D	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
3 TMVW+p	MT20	4.0	4.0	1.00	2.00
C TTW-p	MT20	4.0	4.0	2.25	2.00
D TMVW+p	MT20	4.0	4.0	1.00	2.00
F BMV1+p	MT20	3.0	4.0		
G BMVWW-I	MT20	4.0	9.0		
H BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
JT VERT	678	0	0
H VERT	678	0	0
F VERT	678	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS							
1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	477	326 / 0	0 / 0	0 / 0	0 / 0	151 / 0	0 / 0
F	477	326 / 0	0 / 0	0 / 0	0 / 0	151 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO				FR-TO			
A-B	0/38	-91.8	-91.8 0.12 (1)	10.00	G-C	-24/77	0.03 (4)
B-C	-362/0	-91.8	-91.8 0.30 (1)	6.25	B-G	0/298	0.07 (1)
C-D	-362/0	-91.8	-91.8 0.30 (1)	6.25	G-D	0/298	0.07 (1)
D-E	0/38	-91.8	-91.8 0.12 (1)	10.00			
H-B	-842/0	0.0	0.0 0.07 (1)	7.81			
F-D	-842/0	0.0	0.0 0.07 (1)	7.81			
H-G	0/0	-18.5	-18.5 0.13 (4)	10.00			
G-F	0/0	-18.5	-18.5 0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 098-09, CSA 098-14
- TPC 2011, TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.30/1.00 (C-D:1), BC=0.13/1.00 (F-G:4), WB=0.07/1.00 (B-G:1), SB=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

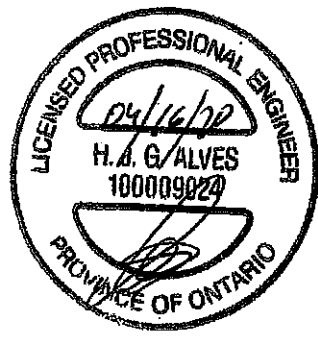
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1957 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

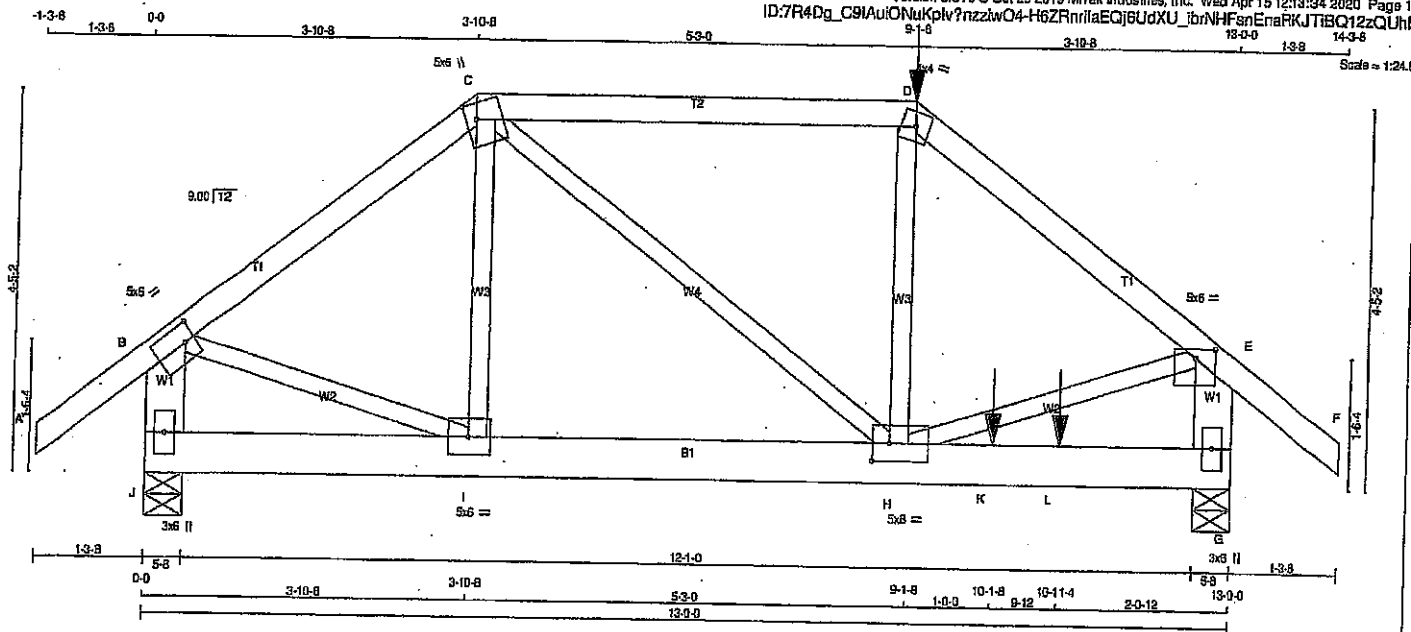
JSI GRIP = 0.59 (D) (INPUT = 0.90)
JSI METAL = 0.15 (D) (INPUT = 1.00)



Structural component only
DWG# T-2006190

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 65' b

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x6	DRY	No.2
G - E	2x6	DRY	No.2
J - G	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-1	MT20	5.0	6.0	2.50	1.50
C	TTWW-m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMW-p	MT20	5.0	6.0	1.25	3.00
G	BMW1+p	MT20	3.0	6.0		
H	BMWW-1	MT20	5.0	6.0	2.50	2.50
I	BMWW-1	MT20	5.0	6.0		
J	BMW1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	0	0
J	HORIZ	HORIZ	0	0
G	VERT	DOWN	0	0

UNFACTORED REACTIONS

1ST LOOSE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	802	553 / 0	0 / 0	0 / 0	0 / 0	249 / 0	0 / 0
G	1282	892 / 0	0 / 0	0 / 0	0 / 0	390 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.06 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO							
A-B	0 / 38	-91.8	-91.8	0.14 (1)	10.00	1-C	-214 / 6
B-C	-954 / 0	-91.8	-91.8	0.28 (1)	5.96	C-H	0 / 599
C-D	-1228 / 0	-91.8	-91.8	0.53 (1)	4.98	H-D	0 / 302
D-E	-1528 / 0	-91.8	-91.8	0.31 (1)	4.94	E-I	0 / 791
E-F	0 / 38	-91.8	-91.8	0.14 (1)	10.00	I-E	0 / 1266
J-B	-1091 / 0	0.0	0.0	0.08 (1)	7.81		
G-E	-1565 / 0	0.0	0.0	0.11 (1)	7.80		
J-I	0 / 0	-18.5	-18.5	0.05 (1)	10.00		
I-H	0 / 756	-18.5	-18.5	0.33 (1)	10.00		
H-K	0 / 0	-18.5	-18.5	0.49 (1)	10.00		
K-L	0 / 0	-18.5	-18.5	0.49 (1)	10.00		
L-G	0 / 0	-18.5	-18.5	0.49 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	9'-1'-8"	-200	-200	---	BACK	VERT	TOTAL	---	C1
K	10'-1'-8"	-1063	-1063	---	BACK	VERT	TOTAL	---	C1
L	10'-11'-4"	-17	-17	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. G/G

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 089-09, CSA 088-14
- TPIC 2011, TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/599 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/599 (0.06")

CSI: TC=0.59/1.00 (C-D:1), BC=0.49/1.00 (G-H:1),
WB=0.31/1.00 (E-H:1), SSI=0.59/1.00 (G-H:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

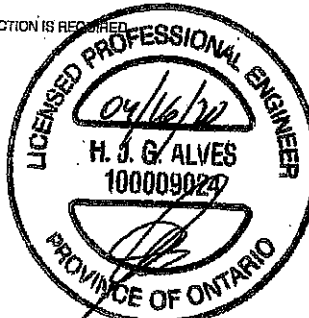
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
	MAX	MIN	MAX
MT20	618	354	1667

PLATE PLACEMENT TOL = 0.250 inches

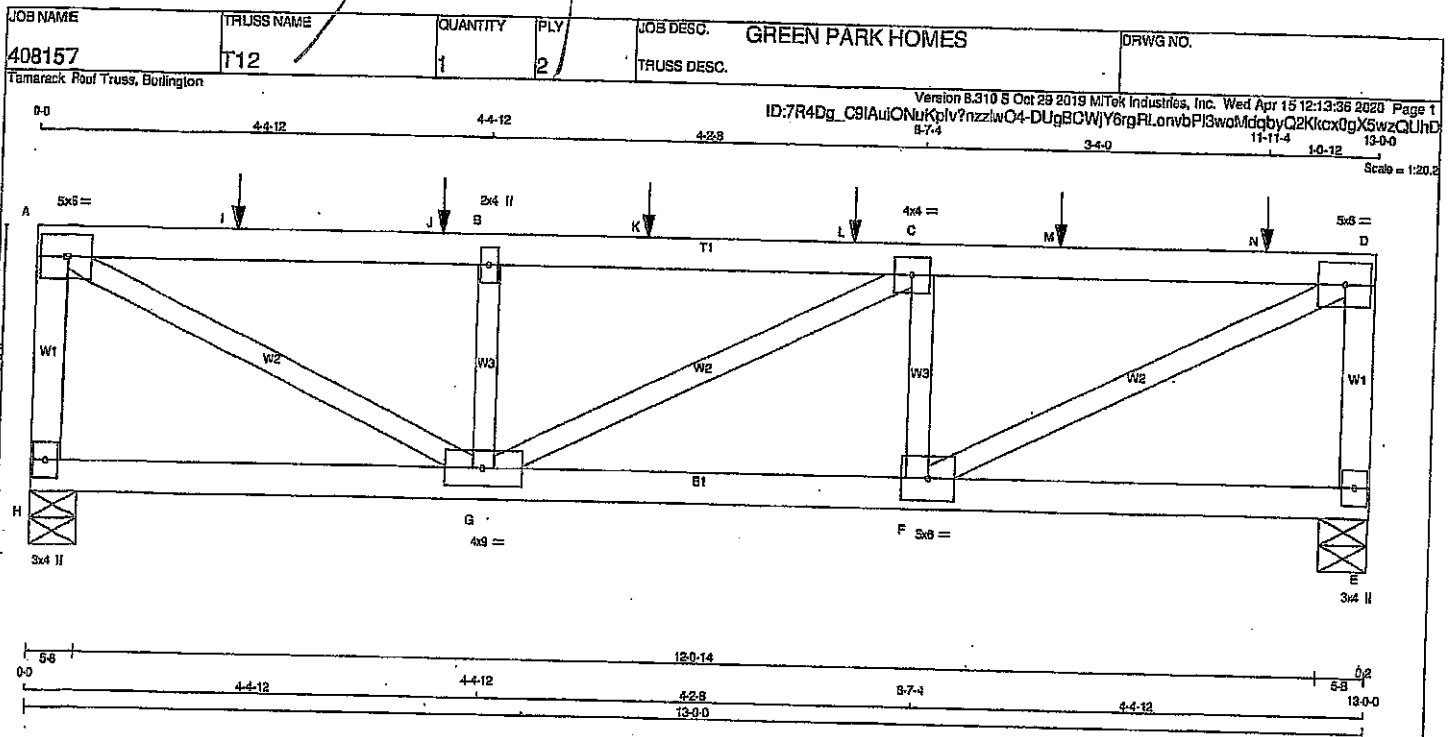
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.85 (D) (INPUT=0.90)
JSI METAL=0.34 (E) (INPUT=1.00)



Structural component only
DWG# T-2006191

Structural component only
DWG# T-2006192



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR. SPF
H - A 2x4 DRY No.2 SPF
A - D 2x4 DRY No.2 SPF
E - D 2x4 DRY No.2 SPF
H - E 2x4 DRY No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF)

TOP CHORDS: (0.122"x3") SPIRAL NAILS

H-A 1 12 TOP
A-D 1 12 SIDE(0.0)
D-E 1 12 TOP

BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS

H-E 1 12 TOP
E-D 1 12 TOP

WEBS: (0.122"x3") SPIRAL NAILS

2x3 1 6

NAILED TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER MAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING MAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	5.0	6.0		
B	TMVW-1	MT20	2.0	4.0		
C	TMVW-1	MT20	4.0	4.0		
D	TMVW-1	MT20	5.0	6.0		
E	BMVW-1	MT20	3.0	4.0		
F	BMVW-1	MT20	5.0	6.0		
G	BMVW-1	MT20	4.0	9.0		
H	BMVW-1	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
H	1929	0	1929	0	5-8	5-8
E	2104	0	2104	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX. MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
		SNOW	LIVE				
H	1362	909 / 0	0 / 0	0 / 0	0 / 0	452 / 0	0 / 0
E	1465	993 / 0	0 / 0	0 / 0	0 / 0	492 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
		FROM	TO					
FR-TO								
H-A	-1885 / 0	0.0	0.0	0.12 (1)	7.81	F-D	0 / 3404	0.39 (1)
A-I	-3008 / 0	-91.8	-91.8	0.43 (1)	4.81	A-G	0 / 3367	0.98 (1)
I-J	-3008 / 0	-91.8	-91.8	0.43 (1)	4.81	F-C	-1450 / 0	0.11 (1)
J-B	-3008 / 0	-91.8	-91.8	0.43 (1)	4.81	G-B	-1419 / 0	0.11 (1)
B-K	-3008 / 0	-91.8	-91.8	0.43 (1)	4.79	G-C	-37 / 0	0.01 (1)
K-L	-3008 / 0	-91.8	-91.8	0.43 (1)	4.79			
L-G	-3008 / 0	-91.8	-91.8	0.43 (1)	4.79			
C-M	-3041 / 0	-91.8	-91.8	0.44 (1)	4.77			
M-N	-3041 / 0	-91.8	-91.8	0.44 (1)	4.77			
N-D	-3041 / 0	-91.8	-91.8	0.44 (1)	4.77			
E-D	-2061 / 0	0.0	0.0	0.13 (1)	7.73			
H-G	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
G-F	0 / 3041	-18.5	-18.5	0.28 (1)	10.00			
F-E	0 / 0	-18.5	-18.5	0.04 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
I	1-11-4	-433	-433	-	BACK	VERT
J	3-11-4	-433	-433	-	BACK	VERT
K	5-11-4	-433	-433	-	BACK	VERT
L	7-11-4	-433	-433	-	BACK	VERT
M	9-11-4	-433	-433	-	BACK	VERT
N	11-11-4	-433	-433	-	BACK	VERT

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 48 = 96 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 99.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(85 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.43")
CALCULATED VERT. DEFL. (LL) = L/999 (0.05")
ALLOWABLE DEFL. (TL) = L/360 (0.43")
CALCULATED VERT. DEFL. (TL) = L/999 (0.05")

CSI: TC=0.44/1.00 (C-D:1), BC=0.28/1.00 (F-G:1), WB=0.39/1.00 (D-F:1), SE=0.35/1.00 (A-B:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 789 1987 1856

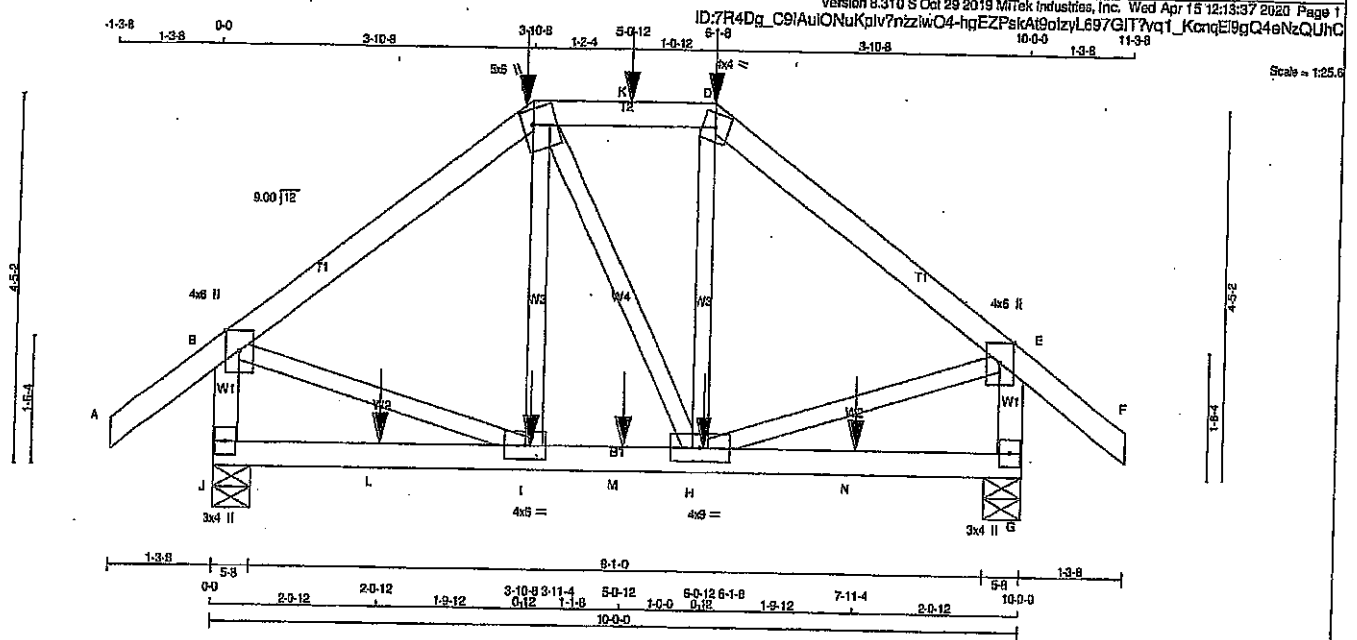
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (3) (INPUT = 0.90)
JSI METAL= 0.38 (2) (INPUT = 1.00)

Structural component only
DWG# T-2006193

JOB NAME 408157	TRUSS NAME T13	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESOR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT DRY, SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	4.0	8.0	Edge	
C TTWW-m	MT20	5.0	6.0	2.25	1.50
D TTW-m	MT20	4.0	4.0		
E TMVW-p	MT20	4.0	6.0	Edge	
G BMV1-p	MT20	3.0	4.0		
H BMVWW-t	MT20	4.0	9.0		
I BMVW-l	MT20	4.0	6.0		
J BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
J	1058	0	0	5-8
G	1058	0	0	5-8

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	743	508 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0
G	744	510 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

SHACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO									
A-B	0 / 38	-91.8	-91.8	0.14 (1)	10.00	I-C	-105 / 44	0.03 (1)	
B-C	-847 / 0	-91.8	-91.8	0.28 (1)	6.23	C-H	0 / 2	0.00 (4)	
C-K	-674 / 0	-91.8	-91.8	0.17 (1)	6.25	H-D	-103 / 45	0.03 (1)	
K-D	-874 / 0	-91.8	-91.8	0.17 (1)	6.25	D-E	0 / 706	0.17 (1)	
D-E	-848 / 0	-91.8	-91.8	0.28 (1)	6.23	E-F	0 / 707	0.17 (1)	
E-F	0 / 38	-91.8	-91.8	0.14 (1)	10.00				
J-B	-1018 / 0	0.0	0.0	0.12 (1)	7.74				
G-E	-1018 / 0	0.0	0.0	0.12 (1)	7.74				
J-L	0 / 0	-18.5	-18.5	0.10 (4)	10.00				
L-I	0 / 0	-18.5	-18.5	0.10 (4)	10.00				
I-M	0 / 674	-18.5	-18.5	0.16 (1)	10.00				
M-H	0 / 674	-18.5	-18.5	0.16 (1)	10.00				
H-N	0 / 0	-18.5	-18.5	0.10 (4)	10.00				
N-G	0 / 0	-18.5	-18.5	0.10 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LG1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-293	-293	---	BACK	VERT	TOTAL	C1
D	6-1-8	-293	-293	---	BACK	VERT	TOTAL	C1
H	6-0-12	-17	-17	---	BACK	VERT	TOTAL	C1
I	3-11-4	-17	-17	---	BACK	VERT	TOTAL	C1
K	5-0-12	-88	-88	---	BACK	VERT	TOTAL	C1
L	2-0-12	-17	-17	---	BACK	VERT	TOTAL	C1
M	5-0-12	-17	-17	---	BACK	VERT	TOTAL	C1
N	7-11-4	-17	-17	---	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 47 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.33")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.33")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.28/1.00 (D-E:1), BC=0.16/1.00 (H-I:1), WB=0.17/1.00 (E-H:1), SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

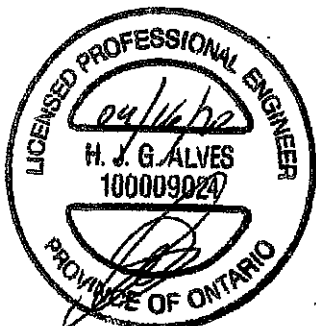
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

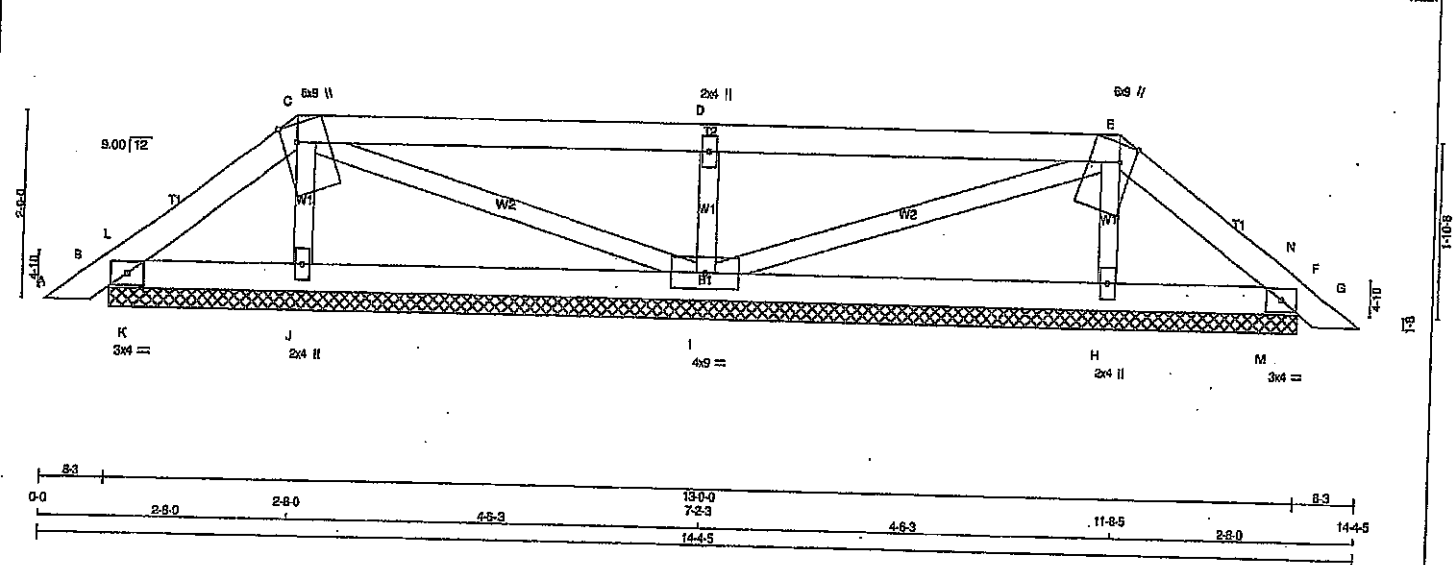
JSI GRIP = 0.71 (E) (INPUT = 0.90)
JSI METAL = 0.46 (E) (INPUT = 1.00)

Structural component only
DWG# T-2006194



JOB NAME 408157	TRUSS NAME PB1	QUANTITY 2	FLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 ID:7F4Dg_C9IAuIONuKplv?mzzhwO4-9bGW2OY8eEnZ2V71DmzYpdE5UYu41cqDUJBYE4lzQUHS
 118-5 14-4-5 14-4-5
 Scale = 1:22.7



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	6.0	9.0	Edge	2.00
D	TMMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	6.0	9.0	Edge	2.00
F	TMB1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-I	MT20	4.0	9.0		
J	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	185	0	185	0	13-0-0	13-0-0
F	185	0	185	0	13-0-0	13-0-0
J	278	0	278	0	13-0-0	13-0-0
I	616	0	616	0	13-0-0	13-0-0
H	278	0	278	0	13-0-0	13-0-0

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	129	99/0	0/0	0/0	0/0	30/0	0/0
F	129	99/0	0/0	0/0	0/0	30/0	0/0
J	199	117/0	0/0	0/0	0/0	82/0	0/0
I	433	285/0	0/0	0/0	0/0	138/0	0/0
H	199	117/0	0/0	0/0	0/0	82/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 (PLF)	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO			LENGTH	FR-TO			
A-B	0/15	-91.8	-91.8	0.02 (1)	10.00	J-C	-189 / 0	0.03 (1)	
B-L	-35/0	-91.8	-91.8	0.01 (1)	6.25	C-D	-24 / 0	0.01 (1)	
L-C	-57/0	-91.8	-91.8	0.04 (1)	6.25	I-D	-517 / 0	0.08 (1)	
C-D	-7/0	-91.8	-91.8	0.32 (1)	10.00	D-E	-24 / 0	0.01 (1)	
D-E	-7/0	-91.8	-91.8	0.32 (1)	10.00	H-E	-189 / 0	0.03 (1)	
E-N	-57/0	-91.8	-91.8	0.04 (1)	6.25	K-L	-123 / 0	0.01 (1)	
N-F	-35/0	-91.8	-91.8	0.01 (1)	6.25	M-N	-123 / 0	0.01 (1)	
F-G	0/15	-91.8	-91.8	0.02 (1)	10.00				
B-K	0/43	-18.5	-18.5	0.05 (1)	10.00				
K-J	0/43	-18.5	-18.5	0.06 (4)	10.00				
J-I	0/30	-18.5	-18.5	0.08 (4)	10.00				
I-H	0/30	-18.5	-18.5	0.08 (4)	10.00				
H-M	0/43	-18.5	-18.5	0.05 (4)	10.00				
M-F	0/43	-18.5	-18.5	0.05 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.8 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 81.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.32/1.00 (C-D:1), BC=0.08/1.00 (H-I:4), WB=0.08/1.00 (D-I:1), SS=0.20/1.00 (C-D:1)

DDL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

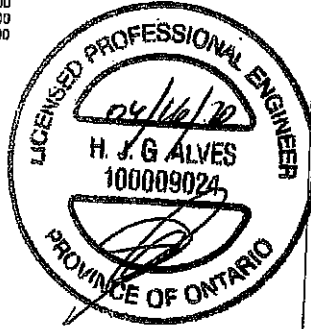
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687
	618	354	1687

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.27 (D) (INPUT = 0.90)

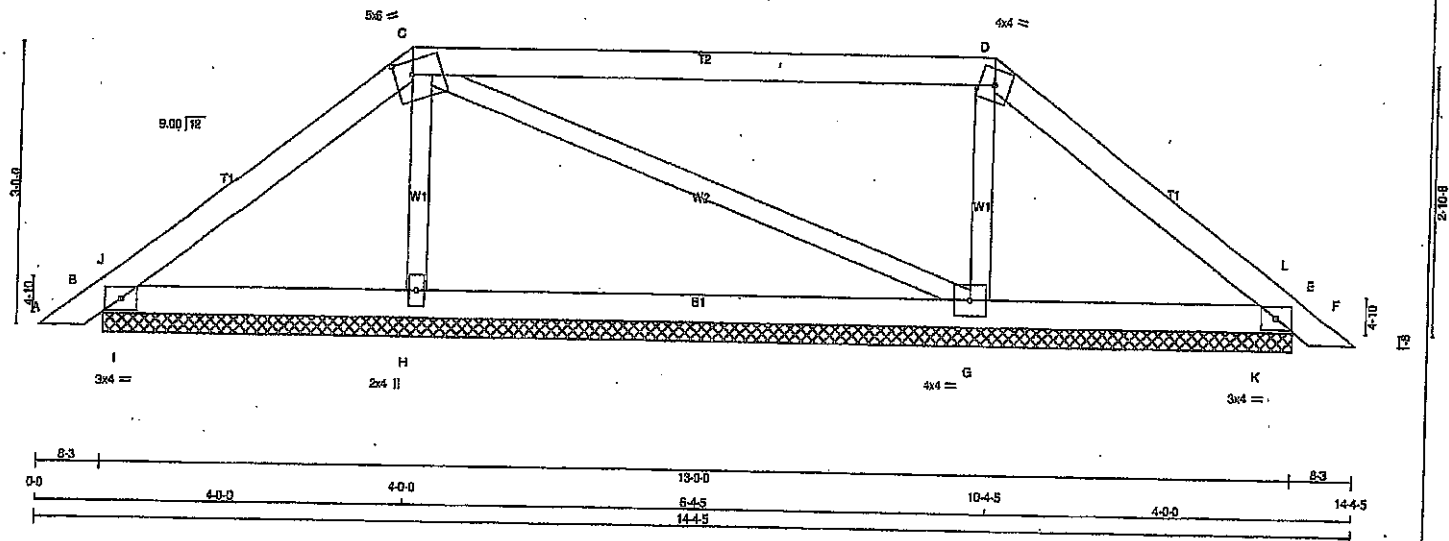
JSI METAL=0.11 (D) (INPUT = 1.00)



Structural component only
 DWG# T-2006179

JOB NAME 408157	TRUSS NAME PB2	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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10-4-5 14-4-5
Scale = 1:22.7



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
B - TMB1-I	MT20	3.0	4.0		
C - TTWW-m	MT20	5.0	6.0	1.75	2.25
D - TTWW-m	MT20	4.0	4.0		
E - TMB1-I	MT20	3.0	4.0		
G - BMW1-I	MT20	4.0	4.0		
H - BMW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
JT	306	0	306	0	13-0-0	13-0-0
B	288	0	288	0	13-0-0	13-0-0
E	456	0	456	0	13-0-0	13-0-0
G	493	0	493	0	13-0-0	13-0-0

UNFACTORED REACTIONS

1ST LOASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD
JT	212	161 / 0	0 / 0	0 / 0	0 / 0	52 / 0
E	200	151 / 0	0 / 0	0 / 0	0 / 0	49 / 0
G	325	197 / 0	0 / 0	0 / 0	0 / 0	128 / 0
G	351	217 / 0	0 / 0	0 / 0	0 / 0	133 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX		MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	
		FROM	TO			CS1 (LC)	CS1 (LC)
FR-TO							
A-B	0 / 15	-91.8	-91.8	0.02 (1)	H-Q	-327 / 0	0.06 (1)
B-J	-48 / 0	-91.8	-91.8	0.06 (1)	C-G	-26 / 0	0.02 (1)
J-C	-145 / 0	-91.8	-91.8	0.12 (1)	G-D	-354 / 0	0.06 (1)
C-D	-72 / 0	-91.8	-91.8	0.03 (1)	I-J	-292 / 0	0.00 (1)
D-L	-116 / 0	-91.8	-91.8	0.12 (1)	K-L	-295 / 0	0.00 (1)
L-E	-17 / 0	-91.8	-91.8	0.05 (1)			
E-F	0 / 15	-91.8	-91.8	0.02 (1)			
B-I	0 / 109	-18.5	-18.5	0.12 (1)			
I-H	0 / 109	-18.5	-18.5	0.13 (4)			
H-G	0 / 88	-18.5	-18.5	0.13 (4)			
G-K	0 / 88	-18.5	-18.5	0.12 (4)			
K-E	0 / 88	-18.5	-18.5	0.12 (1)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.09/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.63/1.00 (C-D:1), BC=0.13/1.00 (H-I:4), WS=0.06/1.00 (D-G:1), SS=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRV)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1887 788	1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)

JSI METAL= 0.07 (H) (INPUT = 1.00)



Structural component only
DWG# T-2006180

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408158	T1Z1	1	2	TRUSS DESC.		

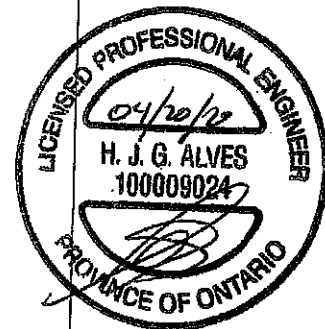
Tamarack Roof Truss, Burlington

Version 8.310 5 Oct 2019 MITek Industries, Inc. Mon Apr 20 14:09:16 2020 Page 2
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FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
U	20-10-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
V	22-10-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
W	24-10-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
X	26-10-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
Y	28-10-4	-139	-139	---	BACK	VERT	TOTAL	---	C1
Z	1-0-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
AA	3-0-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
AB	3-11-8	-1285	-1285	---	BACK	VERT	TOTAL	---	C1
AC	20-10-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AD	22-10-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AE	24-10-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AF	26-10-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AG	28-10-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AH	30-10-4	-26	-26	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

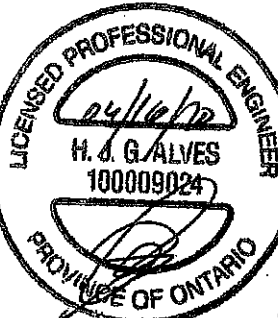
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2006618



GRIP= 0.81 (F) (INPUT = 0.90)
METAL= 0.53 (F) (INPUT = 1.00)



Structural component only *1/2*
DWG# T-2006201

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408158	T23S	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

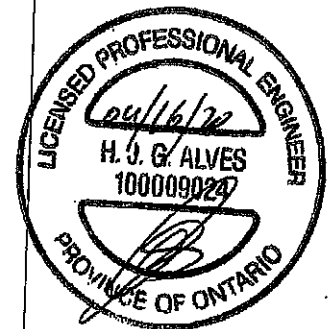
Version 8.310 S Oct 29 2019 MITek Industries, Inc. Wed Apr 15 15:04:49 2020 Page 2
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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	4-3-12	-8	-8	---	FRONT	VERT	TOTAL	---	C1
U	8-3-12	-8	-8	---	FRONT	VERT	TOTAL	---	C1
V	8-3-12	-8	-8	---	FRONT	VERT	TOTAL	---	C1
W	10-3-12	-8	-8	---	FRONT	VERT	TOTAL	---	C1
X	12-2-12	-15	-15	---	FRONT	VERT	TOTAL	---	C1

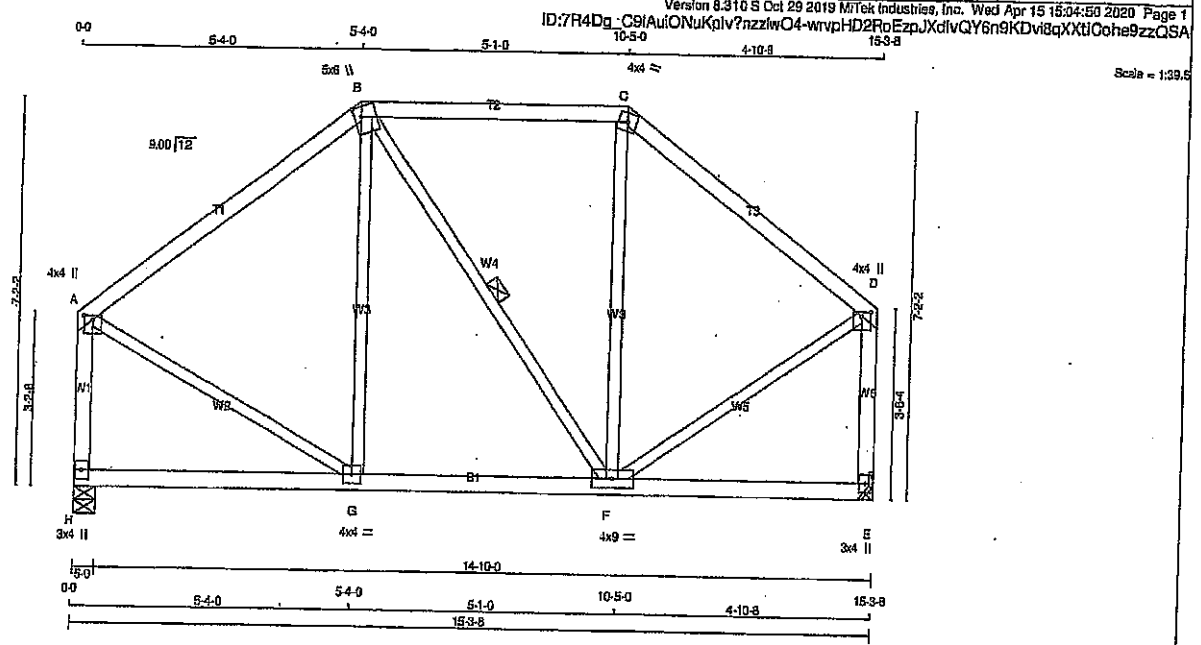
CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
 DWG# T-2006201

JOB NAME 408158	TRUSS NAME T24	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - B	2x4	DRY No.2
B - C	2x4	DRY No.2
C - D	2x4	DRY No.2
H - A	2x4	DRY No.2
E - D	2x4	DRY No.2
H - E	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY, SEASONED LUMBER.

PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - TMVW+p	MT20	4.0	4.0	1.00	2.00
B - TTWW+m	MT20	5.0	6.0	2.25	1.50
C - TTW-m	MT20	4.0	4.0		
D - TMVW+p	MT20	4.0	4.0	1.00	2.00
E - BMV1+p	MT20	3.0	4.0		
F - BMVWW-1	MT20	4.0	9.0		
G - BMVWW-1	MT20	4.0	4.0		
H - BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
H	841	0	841	0	5-0	5-0
E	841	0	841	0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 3'-5".

UNFACTORED REACTIONS

1ST CASE MAX/MIN COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	595	390 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0
E	595	390 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO				FR-TO			
A-B	-542 / 0	-91.8	-91.8	0.33 (1)	6.25	G-B	-125 / 37	0.11 (1)	
B-C	-413 / 0	-91.8	-91.8	0.30 (1)	6.25	B-F	-31 / 0	0.01 (1)	
C-D	-520 / 0	-91.8	-91.8	0.28 (1)	6.25	F-C	-144 / 26	0.13 (1)	
H-A	-800 / 0	0.0	0.0	0.14 (1)	7.81	A-G	0 / 490	0.11 (1)	
E-D	-803 / 0	0.0	0.0	0.17 (1)	7.81	F-D	0 / 493	0.11 (1)	
H-G	0 / 0	-18.5	-18.5	0.12 (4)	10.00				
G-F	0 / 431	-18.5	-18.5	0.15 (4)	10.00				
F-E	0 / 0	-18.5	-18.5	0.10 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 5.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.0/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.51")

CALCULATED VERT. DEFL.(LL) = L/999 (0.01")

ALLOWABLE DEFL.(TL) = L/360 (0.51")

CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.33/1.00 (A-B:1), BC=0.15/1.00 (F-G:4), WB=0.13/1.00 (C-F:1), SS=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

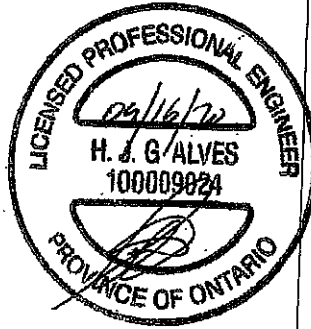
PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	819	354	1687

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP = 0.69 (A) (INPUT = 0.50)

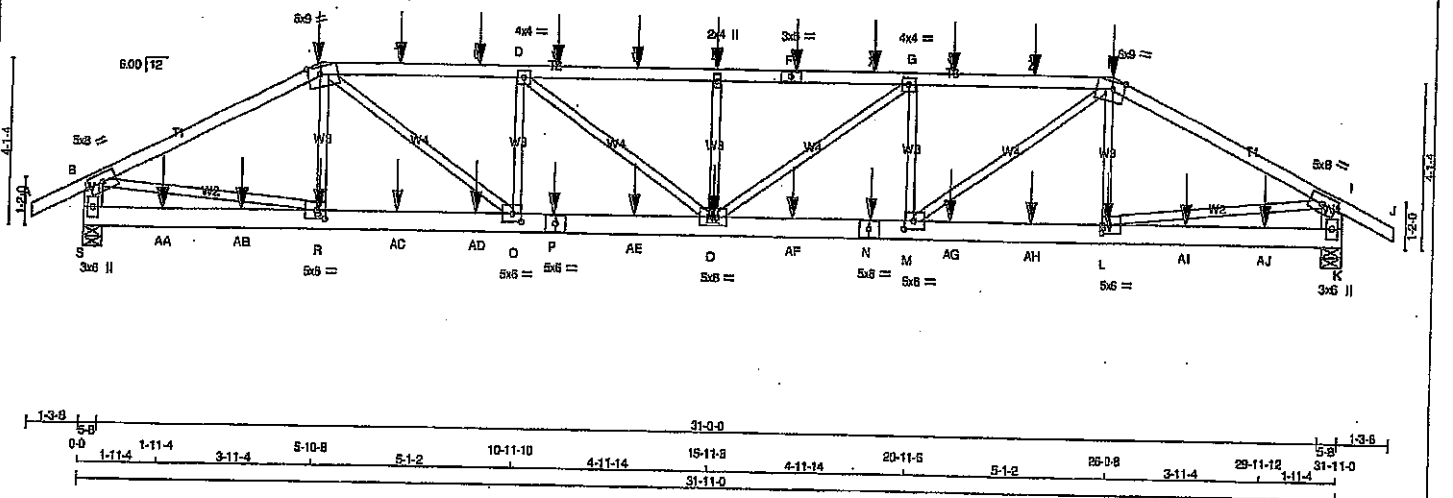
SI METAL = 0.18 (A) (INPUT = 1.00)

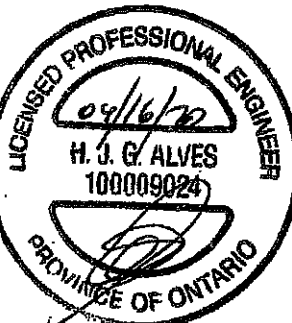


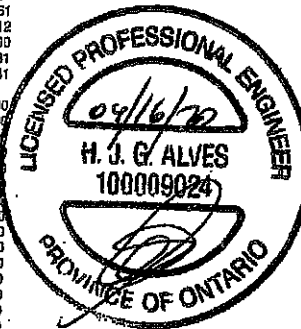
Structural component only
DWG# T-2006202

JOB NAME 408159	TRUSS NAME T31	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 15-11-8 4-11-14 20-11-6 5-1-2 25-0-8 31-11-0 33-2-8
 Scale = 1:50



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x4 DRY No.2 SPF C - F 2x4 DRY No.2 SPF F - H 2x4 DRY No.2 SPF H - J 2x4 DRY No.2 SPF S - B 2x6 DRY No.2 SPF K - I 2x6 DRY No.2 SPF S - P 2x6 DRY 1650F 1.5E SPF P - N 2x6 DRY 1650F 1.5E SPF N - K 2x6 DRY 1650F 1.5E SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF) TOP CHORDS: (0.122"x3") SPIRAL NAILS A-C 1 12 SIDE(61.0) C-F 1 12 SIDE(61.0) F-H 1 12 SIDE(61.0) H-J 1 12 SIDE(61.0) S-B 2 12 TOP K-I 2 12 TOP BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS S-P 2 12 SIDE(183.1) P-N 2 12 SIDE(183.1) N-K 2 12 SIDE(183.1) WEBS: (0.122"x3") SPIRAL NAILS 2x3 1 6 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 8-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY. SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP. PLATES (table in inches) <table> <tr> <th>JT</th> <th>TYPE</th> <th>PLATES</th> <th>W</th> <th>LEN</th> <th>Y</th> <th>X</th> </tr> <tr> <td>B</td> <td>TMWV-1</td> <td>MT20</td> <td>5.0</td> <td>8.0</td> <td></td> <td></td> </tr> <tr> <td>C</td> <td>TMWV-m</td> <td>MT20</td> <td>6.0</td> <td>9.0</td> <td>2.25</td> <td>3.75</td> </tr> <tr> <td>D</td> <td>TMWV-1</td> <td>MT20</td> <td>4.0</td> <td>4.0</td> <td></td> <td></td> </tr> <tr> <td>E</td> <td>TMWV-w</td> <td>MT20</td> <td>2.0</td> <td>4.0</td> <td></td> <td></td> </tr> <tr> <td>F</td> <td>TS-1</td> <td>MT20</td> <td>3.0</td> <td>6.0</td> <td></td> <td></td> </tr> <tr> <td>G</td> <td>TMWV-1</td> <td>MT20</td> <td>4.0</td> <td>4.0</td> <td></td> <td></td> </tr> <tr> <td>H</td> <td>TMWV-m</td> <td>MT20</td> <td>6.0</td> <td>9.0</td> <td>2.25</td> <td>3.75</td> </tr> <tr> <td>I</td> <td>TMWV-1</td> <td>MT20</td> <td>5.0</td> <td>8.0</td> <td></td> <td></td> </tr> <tr> <td>K</td> <td>BMWV-1</td> <td>MT20</td> <td>3.0</td> <td>6.0</td> <td></td> <td></td> </tr> <tr> <td>L</td> <td>BMWV-1</td> <td>MT20</td> <td>5.0</td> <td>8.0</td> <td>2.50</td> <td>2.50</td> </tr> <tr> <td>M</td> <td>BMWV-1</td> <td>MT20</td> <td>5.0</td> <td>8.0</td> <td>2.50</td> <td>2.75</td> </tr> <tr> <td>N</td> <td>BS-1</td> <td>MT20</td> <td>5.0</td> <td>8.0</td> <td></td> <td></td> </tr> <tr> <td>O</td> <td>BMWV-1</td> <td>MT20</td> <td>5.0</td> <td>8.0</td> <td></td> <td></td> </tr> <tr> <td>P</td> <td>BS-1</td> <td>MT20</td> <td>5.0</td> <td>8.0</td> <td></td> <td></td> </tr> <tr> <td>Q</td> <td>BMWV-1</td> <td>MT20</td> <td>5.0</td> <td>8.0</td> <td>2.50</td> <td>2.75</td> </tr> <tr> <td>R</td> <td>BMWV-1</td> <td>MT20</td> <td>5.0</td> <td>8.0</td> <td>2.50</td> <td>2.50</td> </tr> <tr> <td>S</td> <td>BMWV-1</td> <td>MT20</td> <td>3.0</td> <td>6.0</td> <td></td> <td></td> </tr> </table>	JT	TYPE	PLATES	W	LEN	Y	X	B	TMWV-1	MT20	5.0	8.0			C	TMWV-m	MT20	6.0	9.0	2.25	3.75	D	TMWV-1	MT20	4.0	4.0			E	TMWV-w	MT20	2.0	4.0			F	TS-1	MT20	3.0	6.0			G	TMWV-1	MT20	4.0	4.0			H	TMWV-m	MT20	6.0	9.0	2.25	3.75	I	TMWV-1	MT20	5.0	8.0			K	BMWV-1	MT20	3.0	6.0			L	BMWV-1	MT20	5.0	8.0	2.50	2.50	M	BMWV-1	MT20	5.0	8.0	2.50	2.75	N	BS-1	MT20	5.0	8.0			O	BMWV-1	MT20	5.0	8.0			P	BS-1	MT20	5.0	8.0			Q	BMWV-1	MT20	5.0	8.0	2.50	2.75	R	BMWV-1	MT20	5.0	8.0	2.50	2.50	S	BMWV-1	MT20	3.0	6.0			DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table> <tr> <th>JT</th> <th>FACTORED GROSS REACTION</th> <th>MAXIMUM FACTORED GROSS REACTION</th> <th>INPUT BRG</th> <th>RECORD BRG</th> </tr> <tr> <td>S</td> <td>2988</td> <td>2988</td> <td>0</td> <td>5-8</td> </tr> <tr> <td>K</td> <td>2988</td> <td>2988</td> <td>0</td> <td>5-8</td> </tr> </table> UNFACTORED REACTIONS <table> <tr> <th>JT</th> <th>1ST CASE</th> <th>MAX/MIN. COMPONENT REACTIONS</th> </tr> <tr> <td>S</td> <td>COMBINED</td> <td>SNOW LIVE PERM.LIVE WIND DEAD SOIL</td> </tr> <tr> <td>S</td> <td>2114</td> <td>1385 / 0 0 / 0 0 / 0 730 / 0 0 / 0</td> </tr> <tr> <td>K</td> <td>2114</td> <td>1384 / 0 0 / 0 0 / 0 730 / 0 0 / 0</td> </tr> </table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.48 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. LOADING TOTAL LOAD CASES: (4) <table> <tr> <th>CHORDS</th> <th>MEMB.</th> <th>MAX. FACTORED FORCE (LBS)</th> <th>FACTORED VERT. LOAD (PLF)</th> <th>MAX. CS1 (LC)</th> <th>MAX. UNBRACED LENGTH</th> <th>WEBS</th> <th>MEMB.</th> <th>MAX. FACTORED FORCE (LBS)</th> <th>MAX. CS1 (LC)</th> </tr> <tr> <td>FR-TO</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>FR-TO</td> <td></td> <td></td> <td></td> </tr> <tr> <td>A-B</td> <td>0 / 28</td> <td>-91.8</td> <td>-91.8</td> <td>0.07 (1)</td> <td>10.00</td> <td>R-C</td> <td>-331 / 42</td> <td>0.04 (1)</td> <td></td> </tr> <tr> <td>B-C</td> <td>-4461 / 0</td> <td>-91.8</td> <td>-91.8</td> <td>0.48 (1)</td> <td>4.12</td> <td>C-Q</td> <td>0 / 2360</td> <td>0.29 (1)</td> <td></td> </tr> <tr> <td>C-T</td> <td>-5884 / 0</td> <td>-91.8</td> <td>-91.8</td> <td>0.49 (1)</td> <td>3.60</td> <td>Q-D</td> <td>-1239 / 0</td> <td>0.16 (1)</td> <td></td> </tr> <tr> <td>T-U</td> <td>-5884 / 0</td> <td>-91.8</td> <td>-91.8</td> <td>0.49 (1)</td> <td>3.60</td> <td>D-O</td> <td>0 / 674</td> <td>0.08 (1)</td> <td></td> </tr> <tr> <td>U-D</td> <td>-5884 / 0</td> <td>-91.8</td> <td>-91.8</td> <td>0.51 (1)</td> <td>3.60</td> <td>O-E</td> <td>-678 / 0</td> <td>0.09 (1)</td> <td></td> </tr> <tr> <td>D-V</td> <td>-6424 / 0</td> <td>-91.8</td> <td>-91.8</td> <td>0.52 (1)</td> <td>3.43</td> <td>E-G</td> <td>0 / 877</td> <td>0.08 (1)</td> <td></td> </tr> <tr> <td>V-W</td> <td>-6424 / 0</td> <td>-91.8</td> <td>-91.8</td> <td>0.52 (1)</td> <td>3.43</td> <td>G-H</td> <td>-1229 / 0</td> <td>0.16 (1)</td> <td></td> </tr> <tr> <td>W-E</td> <td>-6424 / 0</td> <td>-91.8</td> <td>-91.8</td> <td>0.52 (1)</td> <td>3.43</td> <td>H-M</td> <td>0 / 2350</td> <td>0.29 (1)</td> <td></td> </tr> <tr> <td>E-F</td> <td>-6424 / 0</td> <td>-91.8</td> <td>-91.8</td> <td>0.51 (1)</td> <td>3.43</td> <td>M-G</td> <td>-331 / 42</td> <td>0.04 (1)</td> <td></td> </tr> <tr> <td>F-X</td> <td>-6424 / 0</td> <td>-91.8</td> <td>-91.8</td> <td>0.51 (1)</td> <td>3.43</td> <td>B-R</td> <td>0 / 4021</td> <td>0.50 (1)</td> <td></td> </tr> <tr> <td>X-G</td> <td>-6424 / 0</td> <td>-91.8</td> <td>-91.8</td> <td>0.51 (1)</td> <td>3.43</td> <td>L-I</td> <td>0 / 4019</td> <td>0.50 (1)</td> <td></td> </tr> <tr> <td>G-Y</td> <td>-5881 / 0</td> <td>-91.8</td> <td>-91.8</td> <td>0.48 (1)</td> <td>3.61</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>Y-Z</td> <td>-5881 / 0</td> <td>-91.8</td> <td>-91.8</td> <td>0.48 (1)</td> <td>3.61</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>Z-H</td> <td>-5881 / 0</td> <td>-91.8</td> <td>-91.8</td> <td>0.48 (1)</td> <td>3.61</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>H-I</td> <td>-4459 / 0</td> <td>-91.8</td> <td>-91.8</td> <td>0.48 (1)</td> <td>4.12</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>I-J</td> <td>0 / 28</td> <td>-91.8</td> <td>-91.8</td> <td>0.07 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>S-B</td> <td>-2910 / 0</td> <td>0.0</td> <td>0.0</td> <td>0.10 (1)</td> <td>7.81</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>K-I</td> <td>-2909 / 0</td> <td>0.0</td> <td>0.0</td> <td>0.10 (1)</td> <td>7.81</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>S-AA</td> <td>0 / 0</td> <td>-18.5</td> <td>-18.5</td> <td>0.05 (4)</td> <td>10.00</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>AA-AB</td> <td>0 / 0</td> <td>-18.5</td> <td>-18.5</td> <td>0.05 (4)</td> <td>10.00</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>AB-R</td> <td>0 / 0</td> <td>-18.5</td> <td>-18.5</td> <td>0.05 (4)</td> <td>10.00</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>R-AC</td> <td>0 / 3978</td> <td>-18.5</td> <td>-18.5</td> <td>0.19 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>AC-AD</td> <td>0 / 3978</td> <td>-18.5</td> <td>-18.5</td> <td>0.19 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>AD-Q</td> <td>0 / 3978</td> <td>-18.5</td> <td>-18.5</td> <td>0.19 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>Q-P</td> <td>0 / 5884</td> <td>-18.5</td> <td>-18.5</td> <td>0.28 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>P-AE</td> <td>0 / 5884</td> <td>-18.5</td> <td>-18.5</td> <td>0.28 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>AE-O</td> <td>0 / 5884</td> <td>-18.5</td> <td>-18.5</td> <td>0.28 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>O-AF</td> <td>0 / 5881</td> <td>-18.5</td> <td>-18.5</td> <td>0.28 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>AF-N</td> <td>0 / 5881</td> <td>-18.5</td> <td>-18.5</td> <td>0.28 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>N-M</td> <td>0 / 5881</td> <td>-18.5</td> <td>-18.5</td> <td>0.28 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>M-AG</td> <td>0 / 3977</td> <td>-18.5</td> <td>-18.5</td> <td>0.19 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>AG-AH</td> <td>0 / 3977</td> <td>-18.5</td> <td>-18.5</td> <td>0.19 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>AH-L</td> <td>0 / 3977</td> <td>-18.5</td> <td>-18.5</td> <td>0.19 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>L-AI</td> <td>0 / 0</td> <td>-18.5</td> <td>-18.5</td> <td>0.05 (4)</td> <td>10.00</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>AI-AJ</td> <td>0 / 0</td> <td>-18.5</td> <td>-18.5</td> <td>0.05 (4)</td> <td>10.00</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>AJ-K</td> <td>0 / 0</td> <td>-18.5</td> <td>-18.5</td> <td>0.05 (4)</td> <td>10.00</td> <td></td> <td></td> <td></td> <td></td> </tr> </table> FACTORED CONCENTRATED LOADS (LBS) <table> <tr> <th>JT</th> <th>LOC.</th> <th>LOC1</th> <th>MAX-</th> <th>MAX+</th> <th>FACE</th> <th>DIR.</th> <th>TYPE</th> <th>HEEL</th> <th>CONN.</th> </tr> <tr> <td>C</td> <td>5-10-8</td> <td>-411</td> <td>-110</td> <td>-</td> <td>FRONT</td> <td>VERT</td> <td>TOTAL</td> <td>-</td> <td>C1</td> </tr> <tr> <td>E</td> <td>15-11-8</td> <td>-110</td> <td>-110</td> <td>-</td> <td>FRONT</td> <td>VERT</td> <td>TOTAL</td> <td>-</td> <td>C1</td> </tr> <tr> <td>F</td> <td>17-11-12</td> <td>-110</td> <td>-110</td> <td>-</td> <td>FRONT</td> <td>VERT</td> <td>TOTAL</td> <td>-</td> <td>C1</td> </tr> </table>	JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG	S	2988	2988	0	5-8	K	2988	2988	0	5-8	JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	S	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL	S	2114	1385 / 0 0 / 0 0 / 0 730 / 0 0 / 0	K	2114	1384 / 0 0 / 0 0 / 0 730 / 0 0 / 0	CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	FR-TO						FR-TO				A-B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	R-C	-331 / 42	0.04 (1)		B-C	-4461 / 0	-91.8	-91.8	0.48 (1)	4.12	C-Q	0 / 2360	0.29 (1)		C-T	-5884 / 0	-91.8	-91.8	0.49 (1)	3.60	Q-D	-1239 / 0	0.16 (1)		T-U	-5884 / 0	-91.8	-91.8	0.49 (1)	3.60	D-O	0 / 674	0.08 (1)		U-D	-5884 / 0	-91.8	-91.8	0.51 (1)	3.60	O-E	-678 / 0	0.09 (1)		D-V	-6424 / 0	-91.8	-91.8	0.52 (1)	3.43	E-G	0 / 877	0.08 (1)		V-W	-6424 / 0	-91.8	-91.8	0.52 (1)	3.43	G-H	-1229 / 0	0.16 (1)		W-E	-6424 / 0	-91.8	-91.8	0.52 (1)	3.43	H-M	0 / 2350	0.29 (1)		E-F	-6424 / 0	-91.8	-91.8	0.51 (1)	3.43	M-G	-331 / 42	0.04 (1)		F-X	-6424 / 0	-91.8	-91.8	0.51 (1)	3.43	B-R	0 / 4021	0.50 (1)		X-G	-6424 / 0	-91.8	-91.8	0.51 (1)	3.43	L-I	0 / 4019	0.50 (1)		G-Y	-5881 / 0	-91.8	-91.8	0.48 (1)	3.61					Y-Z	-5881 / 0	-91.8	-91.8	0.48 (1)	3.61					Z-H	-5881 / 0	-91.8	-91.8	0.48 (1)	3.61					H-I	-4459 / 0	-91.8	-91.8	0.48 (1)	4.12					I-J	0 / 28	-91.8	-91.8	0.07 (1)	10.00					S-B	-2910 / 0	0.0	0.0	0.10 (1)	7.81					K-I	-2909 / 0	0.0	0.0	0.10 (1)	7.81					S-AA	0 / 0	-18.5	-18.5	0.05 (4)	10.00					AA-AB	0 / 0	-18.5	-18.5	0.05 (4)	10.00					AB-R	0 / 0	-18.5	-18.5	0.05 (4)	10.00					R-AC	0 / 3978	-18.5	-18.5	0.19 (1)	10.00					AC-AD	0 / 3978	-18.5	-18.5	0.19 (1)	10.00					AD-Q	0 / 3978	-18.5	-18.5	0.19 (1)	10.00					Q-P	0 / 5884	-18.5	-18.5	0.28 (1)	10.00					P-AE	0 / 5884	-18.5	-18.5	0.28 (1)	10.00					AE-O	0 / 5884	-18.5	-18.5	0.28 (1)	10.00					O-AF	0 / 5881	-18.5	-18.5	0.28 (1)	10.00					AF-N	0 / 5881	-18.5	-18.5	0.28 (1)	10.00					N-M	0 / 5881	-18.5	-18.5	0.28 (1)	10.00					M-AG	0 / 3977	-18.5	-18.5	0.19 (1)	10.00					AG-AH	0 / 3977	-18.5	-18.5	0.19 (1)	10.00					AH-L	0 / 3977	-18.5	-18.5	0.19 (1)	10.00					L-AI	0 / 0	-18.5	-18.5	0.05 (4)	10.00					AI-AJ	0 / 0	-18.5	-18.5	0.05 (4)	10.00					AJ-K	0 / 0	-18.5	-18.5	0.05 (4)	10.00					JT	LOC.	LOC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.	C	5-10-8	-411	-110	-	FRONT	VERT	TOTAL	-	C1	E	15-11-8	-110	-110	-	FRONT	VERT	TOTAL	-	C1	F	17-11-12	-110	-110	-	FRONT	VERT	TOTAL	-	C1	DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 25.6 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 39.0 PSF SPACING = 24.0 IN/LC LOADING IN FLAT SECTION BASED ON A SLOPE OF 200/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, OBC 2012, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA S85-09, CSA S85-14 - TPIC 2011, TPIC 2014 (55% OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (1.08") CALCULATED VERT. DEFL.(LL) = L/999 (0.17") ALLOWABLE DEFL.(TL) = L/360 (1.08") CALCULATED VERT. DEFL.(TL) = L/999 (0.32") CSR: TC=0.52/1.00 (D-E:1), BC=0.28/1.00 (O-Q:1), WS=0.50/1.00 (R-R:1), SSI=0.19/1.00 (C-D:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES <table> <tr> <th>PLATE</th> <th>GRIP(DRY)</th> <th>SHEAR</th> <th>SECTION (PSI)</th> <th>(PLI)</th> <th>(PLI)</th> </tr> <tr> <td>MAX MIN</td> <td>MAX MIN</td> <td>MAX MIN</td> <td>MAX MIN</td> <td>MAX MIN</td> <td>MAX MIN</td> </tr> <tr> <td>MT20</td> <td>618 354</td> <td>1867 788</td> <td>1987 1656</td> <td></td> <td></td> </tr> </table> PLATE PLACEMENT TOL = 0.250 inches PLATE ROTATION TOL = 5.0 Deg. JSI GRIP= 0.73 (LI) (INPUT = 0.90) JSI METAL= 0.55 (RV) (INPUT = 1.00)	PLATE	GRIP(DRY)	SHEAR	SECTION (PSI)	(PLI)	(PLI)	MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN	MT20	618 354	1867 788	1987 1656			
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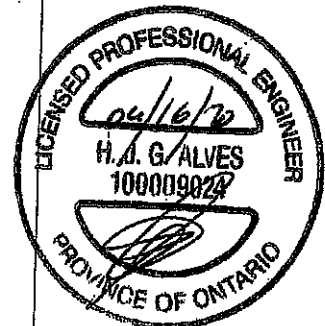
JOB NAME 408159	TRUSS NAME T31	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Building				Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Wed Apr 15 12:51:43 2020 Page 2		
ID:MTW7E0QKeRW7Mm2BLF?Kizlsx2-ITmMBXP6WVXpDGS4DHN64aQFUVSP6EqKA3stzQU7L						

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	25-0-8	-411	-411	---	FRONT	VERT	TOTAL	---	C1
L	25-11-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
N	19-11-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	15-11-8	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	11-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
R	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
T	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
U	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	11-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	19-11-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	21-11-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	23-11-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	1-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AB	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AC	7-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AD	9-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AE	13-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AF	17-11-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AG	21-11-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AH	23-11-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AI	27-11-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AJ	29-11-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

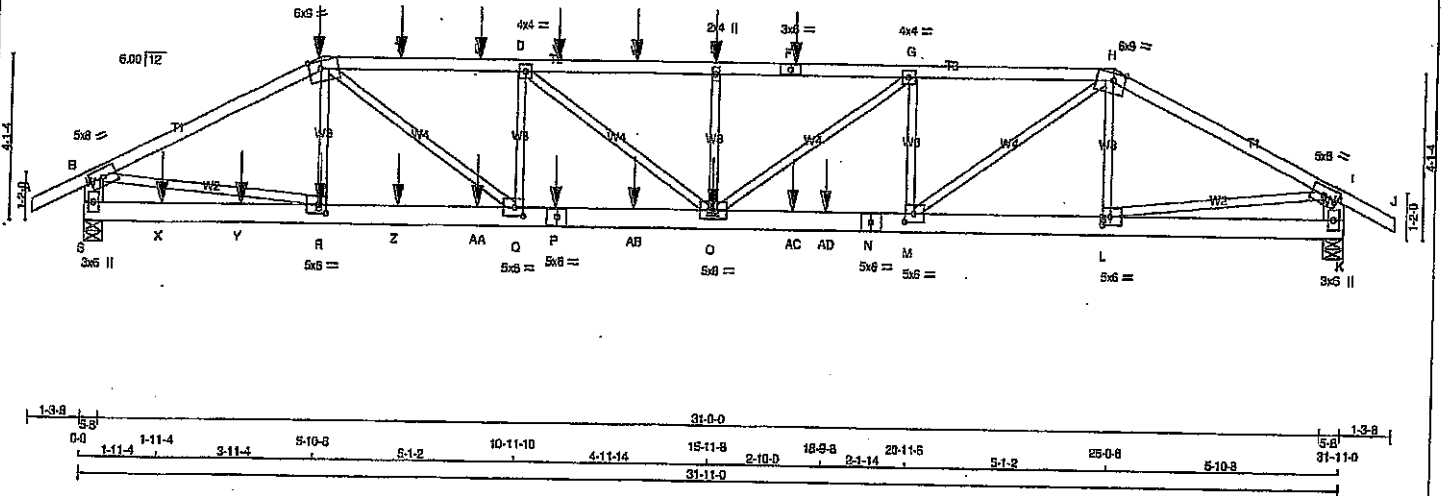
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2006212

JOB NAME 408159	TRUSS NAME T31Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Wed Apr 16 12:51:45 2020 Page 1
 ID: MTW7E0QKerW7Mm2iBLF7kizsk?isUxCQM283fSZGrBeJrCVkelSVWGXHaf9kizCU7S
 1-3-8 0-0 5-10-8 5-10-8 5-1-2 10-11-10 4-11-14 15-11-8 4-11-14 20-11-8 5-1-2 25-0-8 5-10-8 31-11-0 33-2-8
 Scale = 1:52.6



LUMBER	N.L.G.A. RULES	CHORDS	SIZE	DRY	LUMBER	DESOR.
A - C	2x4	DRY	No.2	SPF		
C - F	2x4	DRY	No.2	SPF		
F - H	2x4	DRY	No.2	SPF		
H - J	2x4	DRY	No.2	SPF		
S - B	2x6	DRY	No.2	SPF		
K - I	2x6	DRY	No.2	SPF		
S - P	2x6	DRY	1650F 1.5E	SPF		
P - N	2x6	DRY	1650F 1.5E	SPF		
N - K	2x6	DRY	1650F 1.5E	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C 1	12	SIDE(61.0)
C-F 1	12	SIDE(61.0)
F-H 1	12	SIDE(61.0)
H-J 1	12	TOP
S-B 2	12	TOP
K-I 2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
S-P 2	12	SIDE(183.1)
P-N 2	12	SIDE(183.1)
N-K 2	12	TOP
WEBS: (0.122"x3") SPIRAL NAILS		
2x3 1	8	TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	PLATES	W	LEN	Y	X
JT TYPE					
B TMW-t	MT20	5.0	8.0		
C TTWV-m	MT20	6.0	9.0	2.25	3.75
D TMWV-t	MT20	4.0	4.0		
E TMW-w	MT20	2.0	4.0		
F TS-t	MT20	3.0	6.0		
G TMWV-t	MT20	4.0	4.0		
H TTWV-m	MT20	6.0	9.0	2.25	3.75
I TMW-t	MT20	5.0	8.0		
K BMW-t	MT20	3.0	6.0		
L BMW-t	MT20	5.0	6.0	2.50	2.50
M BMW-t	MT20	5.0	6.0	2.50	2.75
N BS-t	MT20	5.0	8.0		
O BMWV-t	MT20	5.0	8.0		
P BS-t	MT20	5.0	8.0		
Q BMW-t	MT20	5.0	6.0	2.50	2.75
R BMW-t	MT20	5.0	6.0	2.50	2.50
S BMV-t	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	UP/LIFT	IN-SX
S 3419	0	0	5-8
K 3223	0	0	5-8

UNFACTORED REACTIONS	1ST CASE	MAX/MIN. COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
S 2415	1801 / 0	0 / 0
K 2273	1527 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS (LC)	MEMB.	FORCE (LBS)
FR-TO				FR-TO	
A-B	0 / 28	-91.8	-91.8 0.07 (1)	R-C	-385 / 27
B-C	-3223 / 0	-91.8	-91.8 0.53 (1)	C-D	0 / 3031
C-T	-7110 / 0	-91.8	-91.8 0.57 (1)	D-E	-1783 / 5
T-U	-7110 / 0	-91.8	-91.8 0.57 (1)	E-F	0 / 1599
U-D	-7110 / 0	-91.8	-91.8 0.57 (1)	F-G	-661 / 0
D-V	-8391 / 0	-91.8	-91.8 0.65 (1)	G-H	0 / 1060
V-W	-8391 / 0	-91.8	-91.8 0.65 (1)	H-I	-1180 / 0
W-E	-8391 / 0	-91.8	-91.8 0.63 (1)	I-J	0 / 3962
E-F	-8391 / 0	-91.8	-91.8 0.63 (1)	J-K	-621 / 0
F-G	-8391 / 0	-91.8	-91.8 0.53 (1)	K-L	0 / 4713
G-H	-7542 / 0	-91.8	-91.8 0.47 (1)	L-I	0 / 4398
H-I	-4877 / 0	-91.8	-91.8 0.51 (1)		
I-J	0 / 28	-91.8	-91.8 0.07 (1)		
S-B	-3348 / 0	0.0	0.0 0.12 (1)		
K-I	-3145 / 0	0.0	0.0 0.11 (1)		
S-X	0 / 0	-18.5	-18.5 0.04 (4)		
X-Y	0 / 0	-18.5	-18.5 0.04 (4)		
Y-R	0 / 0	-18.5	-18.5 0.04 (4)		
R-Z	0 / 4863	-18.5	-18.5 0.24 (1)		
Z-AA	0 / 4863	-18.5	-18.5 0.24 (1)		
AA-Q	0 / 4863	-18.5	-18.5 0.24 (1)		
Q-P	0 / 7110	-18.5	-18.5 0.39 (1)		
P-AB	0 / 7110	-18.5	-18.5 0.39 (1)		
AB-O	0 / 7110	-18.5	-18.5 0.39 (1)		
O-AC	0 / 7542	-18.5	-18.5 0.69 (1)		
AC-AD	0 / 7542	-18.5	-18.5 0.69 (1)		
AD-N	0 / 7542	-18.5	-18.5 0.69 (1)		
N-M	0 / 7542	-18.5	-18.5 0.69 (1)		
M-L	0 / 4342	-18.5	-18.5 0.31 (1)		
L-K	0 / 0	-18.5	-18.5 0.04 (1)		

FACTORED CONCENTRATED LOADS (LBS)	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
JT								
C	5-10-8	-411	-411	BACK	VERT	TOTAL	-	C1
E	15-11-8	-110	-110	BACK	VERT	TOTAL	-	C1
F	17-11-12	-110	-110	BACK	VERT	TOTAL	-	C1
O	15-11-8	-26	-26	BACK	VERT	TOTAL	-	C1
P	11-11-4	-26	-26	BACK	VERT	TOTAL	-	C1
R	5-11-4	-26	-26	BACK	VERT	TOTAL	-	C1
T	7-11-4	-110	-110	BACK	VERT	TOTAL	-	C1
U	9-11-4	-110	-110	BACK	VERT	TOTAL	-	C1
V	11-11-4	-110	-110	BACK	VERT	TOTAL	-	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08")

CALCULATED VERT. DEFL.(LL) = 1/999 (0.25")

ALLOWABLE DEFL.(TL) = L/360 (1.08")

CALCULATED VERT. DEFL.(TL) = 1/852 (0.45")

CSI: TC=0.56/1.00 (D-E-1), BC=0.59/1.00 (M-O-1), WB=0.58/1.00 (B-R-1), SS=0.32/1.00 (M-O-1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

T20 518 354 1667 789 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRP=0.87 (M) (INPUT = 0.90)

JSI METAL=0.85 (N) (INPUT = 1.00)

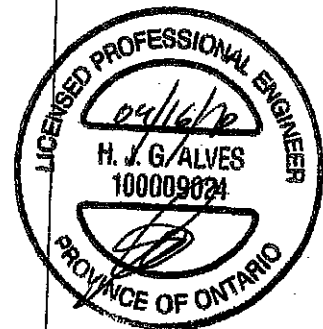
JOB NAME 408159	TRUSS NAME T31Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.910 S Oct 28 2019 MTE Industries, Inc. Wed Apr 15 12:51:45 2020 Page 2 ID:MTW7E0CKeRW7Mm2IBLF7kizlsx7-lsUXcCOM283ISZGRBeJrCVikelsVWGXHef9xtzQU7S	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
W	13-11-4	-110	-110	--	BACK	VERT	TOTAL	--	C1
X	1-11-4	-25	-25	--	BACK	VERT	TOTAL	--	C1
Y	3-11-4	-25	-25	--	BACK	VERT	TOTAL	--	C1
Z	7-11-4	-25	-25	--	BACK	VERT	TOTAL	--	C1
AA	9-11-4	-25	-25	--	BACK	VERT	TOTAL	--	C1
AB	13-11-4	-25	-25	--	BACK	VERT	TOTAL	--	C1
AC	17-11-12	-25	-25	--	BACK	VERT	TOTAL	--	C1
AD	18-9-8	-1564	-1564	--	BACK	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

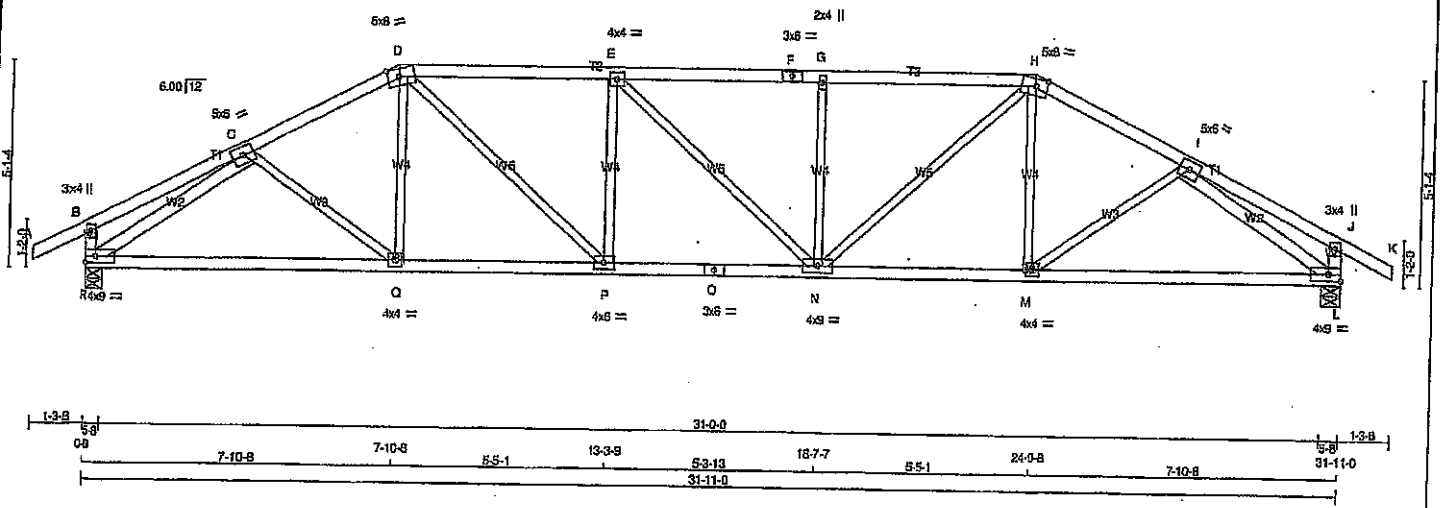
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2006213

Handwritten initials

JOB NAME 408159	TRUSS NAME T32	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	
Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Wed Apr 15 12:51:46 2020 Page 1				ID:MTW7E0QKerW7Mm2BLF7Kdzlx7-A22vqYR_pSBW4j1Mq4KiCxiTKcw2gWIPITCzQU7R	
1-3-8 0.0 4-0-8 4-0-8 3-10-8 7-10-8 5-5-1 13-3-9 5-3-13 18-7-7 5-5-1 24-0-8 3-10-0 27-10-8 4-0-8 31-11-0 33-2-8 1-3-8				Scale = 1:50.6	



LUMBER	N.L.G.A. RULES	SIZE	DRY	LUMBER
CHORDS				
A - D	2x4	DRY	No.2	
E - H	2x4	DRY	No.2	
I - K	2x4	DRY	No.2	
L - M	2x4	DRY	No.2	
N - P	2x4	DRY	No.2	
Q - R	2x4	DRY	No.2	
S - T	2x4	DRY	No.2	
U - V	2x4	DRY	No.2	
W - X	2x4	DRY	No.2	
Y - Z	2x4	DRY	No.2	

ALL WEBS	2x3	DRY	No.2
R - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY-SEASONED LUMBER.

PLATES (table in inches)	PLATES	W	LEN	Y	X
JT	TMV-p	MT20	3.0	4.0	
B	TMV-p	MT20	5.0	6.0	
C	TMVW-t	MT20	5.0	6.0	
D	TMVW-m	MT20	5.0	6.0	2.25 3.50
E	TMVW-t	MT20	4.0	4.0	
F	TS-t	MT20	3.0	6.0	
G	TMVW-w	MT20	2.0	4.0	
H	TMVW-m	MT20	5.0	6.0	2.25 3.50
I	TMVW-t	MT20	5.0	6.0	
J	TMV-p	MT20	3.0	4.0	
L	BMVW-t	MT20	4.0	6.0	Edge
M	BMVW-t	MT20	4.0	4.0	
N	BMVW-t	MT20	4.0	6.0	
O	BS-t	MT20	3.0	6.0	
P	BMVW-t	MT20	4.0	6.0	
Q	BMVW-t	MT20	4.0	4.0	
R	BMVW-t	MT20	4.0	6.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
R	1884	0	1884	0
L	1884	0	1884	0

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE
R	1330	888 / 0	0 / 0	0 / 0
L	1330	888 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	W E B S	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
MEMB.				MEMB.		
FR-TO				FR-TO		
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	C-Q	0 / 74
B-C	0 / 17	-91.8	-91.8 0.20 (1)	10.00	Q-D	0 / 133
C-D	-2469 / 0	-91.8	-91.8 0.30 (1)	4.12	D-P	0 / 933
D-E	-2893 / 0	-91.8	-91.8 0.54 (1)	3.61	P-E	-534 / 0
E-F	-2891 / 0	-91.8	-91.8 0.53 (1)	3.61	E-N	-2 / 0
F-G	-2891 / 0	-91.8	-91.8 0.53 (1)	3.61	N-G	-533 / 0
G-H	-2891 / 0	-91.8	-91.8 0.53 (1)	4.12	H-I	0 / 933
H-I	-2469 / 0	-91.8	-91.8 0.30 (1)	10.00	I-H	0 / 134
I-J	0 / 17	-91.8	-91.8 0.20 (1)	10.00	J-K	0 / 74
J-K	0 / 28	-91.8	-91.8 0.12 (1)	10.00	K-L	-2668 / 0
K-L	-289 / 0	0.0	0.0 0.03 (1)	7.81	L-K	-2667 / 0
L-M	-289 / 0	0.0	0.0 0.03 (1)	7.81	M-L	-2667 / 0
R-Q	0 / 2167	-18.5	-18.5 0.49 (1)	10.00		
Q-P	0 / 2194	-18.5	-18.5 0.45 (1)	10.00		
P-O	0 / 2893	-18.5	-18.5 0.53 (1)	10.00		
O-N	0 / 2893	-18.5	-18.5 0.53 (1)	10.00		
N-M	0 / 2194	-18.5	-18.5 0.49 (1)	10.00		
M-L	0 / 2168	-18.5	-18.5 0.49 (1)	10.00		

TOTAL WEIGHT = 2 X 130 = 260 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	DL	= 25.6	PSF
BOT CH.	DL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 33.0	PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CSI: TC=0.54/1.00 (D-E:1), BC=0.53/1.00 (N-P:1),
WB=0.74/1.00 (I-L:1), SS=0.23/1.00 (D-E:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE FEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

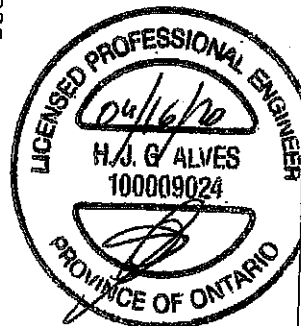
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	818 354	1667 788

PLATE PLACEMENT TOL = 0.250 inches

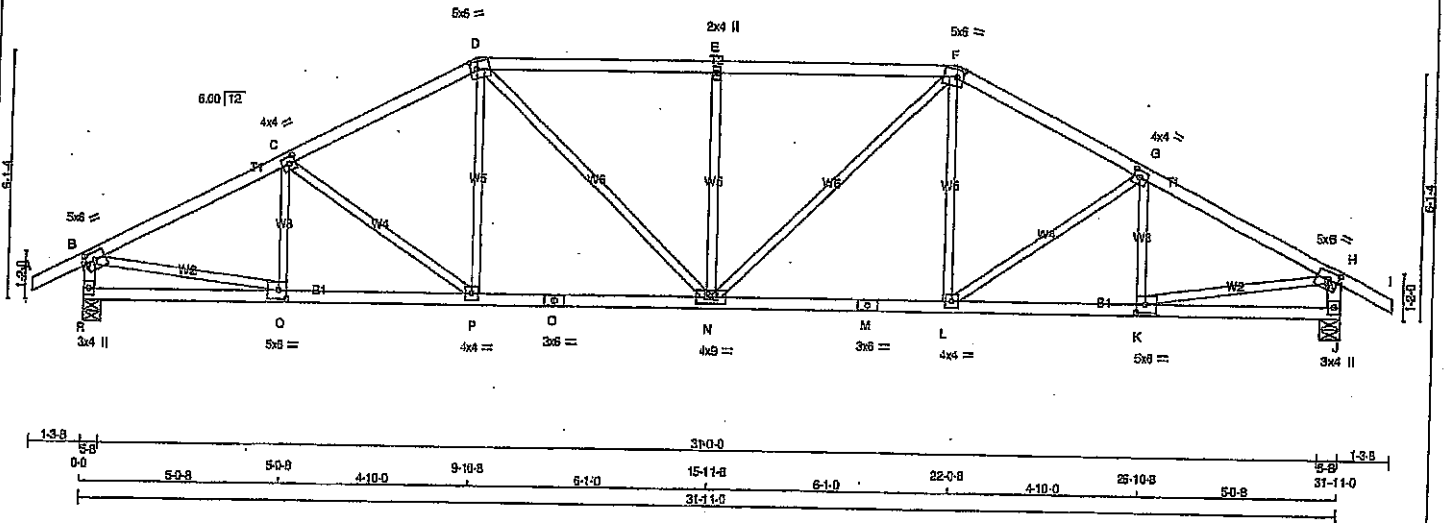
PLATE ROTATION TOL = 5.0 Deg.

SI GRIP=0.84 (O) (INPUT = 0.80)
SI METAL=0.84 (O) (INPUT = 1.00)



Structural component only
DWG# T-2006214

JOB NAME 408159	TRUSS NAME T33	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	
Version 8.310 S Oct 29 2019 MTek Industries, Inc. Wed Apr 15 12:51:47 2020 Page 1					
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Scale = 1:52.6					



LUMBER	N L G A RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
R - H	2x4	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table in inches)	W	LEN	Y	X
JT TYPE				
B TMWW-1	MT20	5.0	6.0	2.25 2.75
C TMWW-1	MT20	4.0	4.0	2.00 1.75
D TTWW-m	MT20	5.0	6.0	2.25 2.00
E TMWW-w	MT20	2.0	4.0	
F TTWW-m	MT20	5.0	6.0	2.25 2.00
G TMWW-1	MT20	4.0	4.0	2.00 1.75
H TMWW-1	MT20	5.0	6.0	2.25 2.75
J BMV1-p	MT20	3.0	4.0	
K BMWW-1	MT20	5.0	6.0	2.50 2.50
L BMWW-1	MT20	4.0	4.0	
M BS-1	MT20	3.0	6.0	
N BMWW-1	MT20	4.0	9.0	
O BS-1	MT20	3.0	6.0	
P BMWW-1	MT20	4.0	4.0	
Q BMWW-1	MT20	5.0	6.0	2.50 2.50
R BMV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
R	1884	0	1884	0	0	5-8	5-8	5-8	5-8
J	1884	0	1884	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST CASE		MAX/MIN COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
R	1830	886 / 0	0 / 0	0 / 0	0 / 0
J	1830	886 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MEMB.		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS (LC)	UNBRACED LENGTH (FT)	MEMB.	FORCE (LBS)	MAX. CS (LC)
R-Q	0 / 0	-18.5	-18.5 0.10 (4)	10.00	Q-C	-301 / 0	0.07 (1)
Q-P	0 / 2290	-18.5	-18.5 0.43 (1)	10.00	C-P	-254 / 0	0.16 (1)
P-O	0 / 2077	-18.5	-18.5 0.40 (1)	10.00	P-D	0 / 257	0.05 (1)
O-N	0 / 2077	-18.5	-18.5 0.40 (1)	10.00	D-N	0 / 575	0.13 (1)
N-M	0 / 2077	-18.5	-18.5 0.40 (1)	10.00	N-E	-695 / 0	0.40 (3)
M-L	0 / 2077	-18.5	-18.5 0.40 (1)	10.00	E-F	0 / 575	0.13 (1)
L-K	0 / 2290	-18.5	-18.5 0.43 (1)	10.00	F-G	-2342 / 0	0.40 (3)
K-J	0 / 0	-18.5	-18.5 0.10 (4)	10.00	G-H	-2538 / 0	0.40 (3)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 6.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 085-09, CSA 085-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.06")
CALCULATED VERT. DEFL. (LL) = L/899 (0.13")
ALLOWABLE DEFL. (TL) = L/360 (1.06")
CALCULATED VERT. DEFL. (TL) = L/899 (0.24")

CS: TC=0.54/1.00 (D-E:1), BC=0.48/1.00 (K-L:1), WB=0.52/1.00 (H-K:1), SS=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

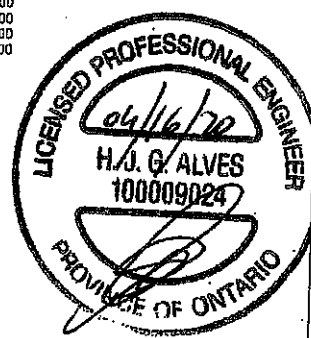
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP=0.90 (B) (INPUT = 0.80)
ISI METAL=0.67 (B) (INPUT = 1.00)



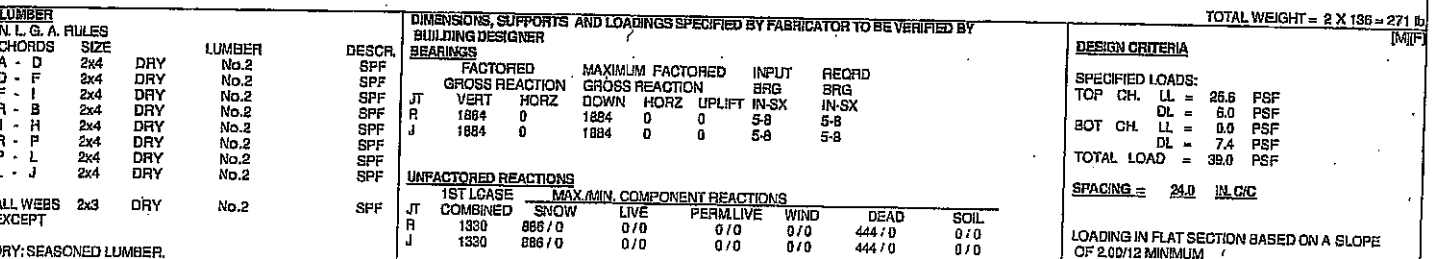
Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Wed Apr 15 12:51:48 2020 Page 1

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1-3-8 0-0 5-0-8 6-0-8 11-10-5 15-11-8 4-1-0 4-1-0 20-0-8 5-10-0 25-10-8 31-11-0 1-3-8

Scale = 1/8"=1'-0"



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	1884	0	1884	0	0	5-8	5-8
J	1884	0	1884	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LEASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1330	886 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0
J	1330	886 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
 MAX. UNSHEATHED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN.C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.0D/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBCG 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2018 AMENDMENT)
- CSA 08-09, CSA 08-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFLL (LL) = L/360 (1.06")

CALCULATED VERT. DEFLL (LL) = L/939 (0.12")

ALLOWABLE DEFLL (TL) = L/360 (1.08")

CALCULATED VERT. DEFLL (TL) = L/939 (0.21")

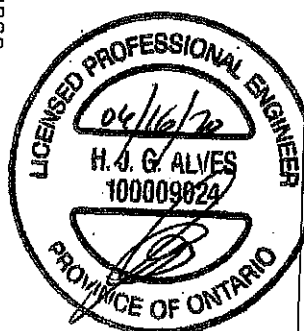
CSI: TO=0.55/1.0 (G+H-I), BC=0.44/1.0 (K-M-I),
WB=0.53/1.0 (H-K-I), SI=0.24/1.0 (G-H-I)

DOL LUMBER=1.0 NAIL=1.0 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

SI GRIP= 0.88 (H) (INPUT = 0.90)
SI METAL= 0.74 (P) (INPUT = 1.00)

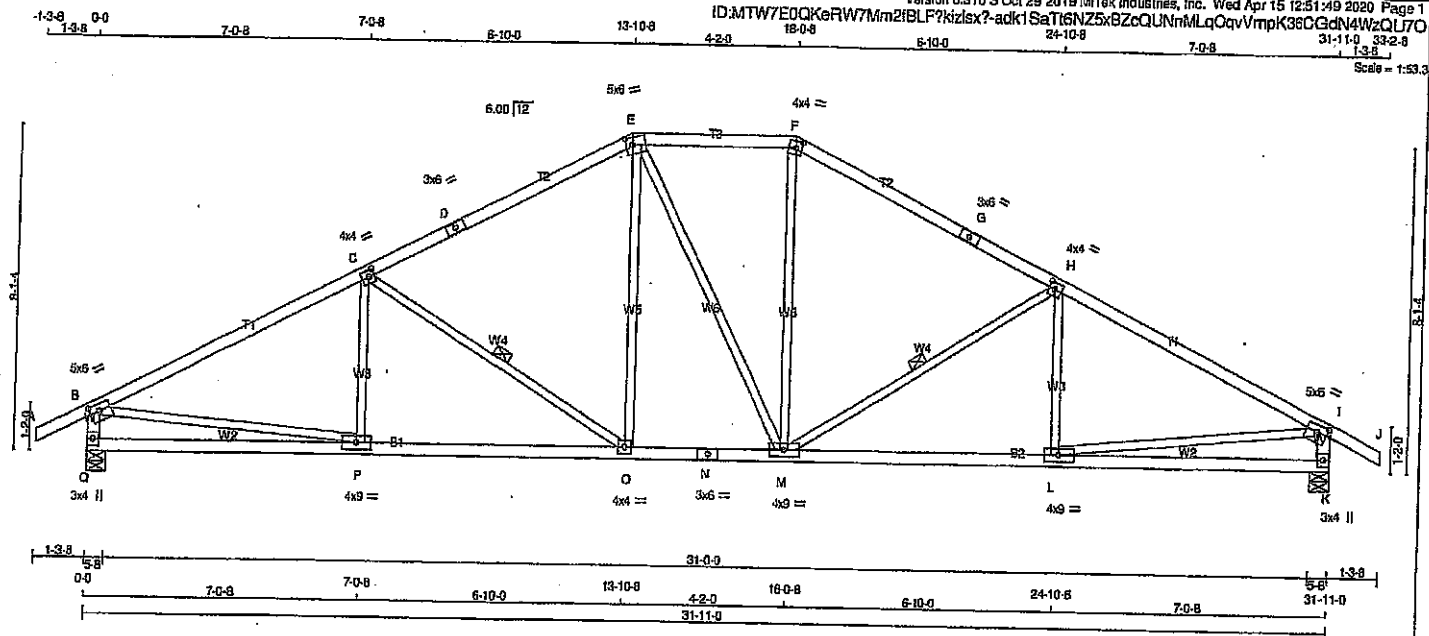
Structural component only
DWG# T-2006216



JOB NAME 408159	TRUSS NAME T35	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Wed Apr 15 12:51:49 2020 Page 1
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31-11-0 33-2-8
Scale = 1:53.3



LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS			
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.75
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTWV-m	MT20	5.0	6.0	2.25	2.00
F	TTW-m	MT20	4.0	4.0	2.00	1.75
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	4.0	2.00	1.75
I	TMVW-t	MT20	5.0	6.0	2.00	2.75
K	BMV-t	MT20	3.0	4.0		
L	BMVW-t	MT20	4.0	9.0		
M	BMVWV-t	MT20	4.0	9.0		
N	BS-t	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	4.0	9.0		
Q	BMV-t	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	BRG	BRG
Q	1884	0	1884	0
K	1884	0	1884	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1330	885 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0
K	1330	885 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	P-C	-140 / 71	0.05 (1)
B-C	-2587 / 0	-91.8	-91.8 0.78 (1)	3.47	C-O	-581 / 0	0.31 (1)
C-D	-2013 / 0	-91.8	-91.8 0.68 (1)	3.97	D-E	0 / 458	0.10 (1)
D-E	-2013 / 0	-91.8	-91.8 0.68 (1)	3.97	E-M	0 / 2	0.00 (1)
E-F	-1775 / 0	-91.8	-91.8 0.24 (1)	4.79	M-F	0 / 450	0.10 (1)
F-G	-2013 / 0	-91.8	-91.8 0.68 (1)	3.97	M-H	-680 / 0	0.31 (1)
G-H	-2013 / 0	-91.8	-91.8 0.68 (1)	3.97	L-H	-142 / 70	0.05 (1)
H-I	-2587 / 0	-91.8	-91.8 0.78 (1)	3.47	B-P	0 / 2362	0.53 (1)
I-J	0 / 28	-91.8	-91.8 0.12 (1)	10.00	L-I	0 / 2361	0.53 (1)
Q-B	-1830 / 0	0.0	0.0 0.18 (1)	6.19			
K-I	-1830 / 0	0.0	0.0 0.18 (1)	6.19			
Q-P	0 / 0	-18.5	-18.5 0.22 (4)	10.00			
P-O	0 / 2344	-18.5	-18.5 0.48 (1)	10.00			
O-N	0 / 1774	-18.5	-18.5 0.35 (1)	10.00			
N-M	0 / 1774	-18.5	-18.5 0.35 (1)	10.00			
M-L	0 / 2344	-18.5	-18.5 0.48 (1)	10.00			
L-K	0 / 0	-18.5	-18.5 0.22 (4)	10.00			

TOTAL WEIGHT = 2 X 131 = 263 lb

(MIF)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.78/1.00 (B-C:1), BC=0.48/1.00 (O-P:1),
WB=0.53/1.00 (B-P:1), SSI=0.28/1.00 (H-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

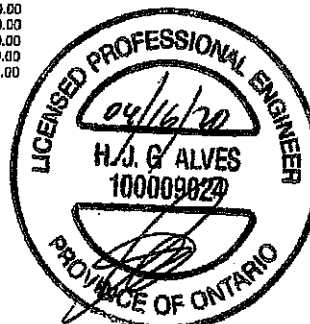
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (B) (INPUT = 0.90)
JSI METAL = 0.70 (B) (INPUT = 1.00)

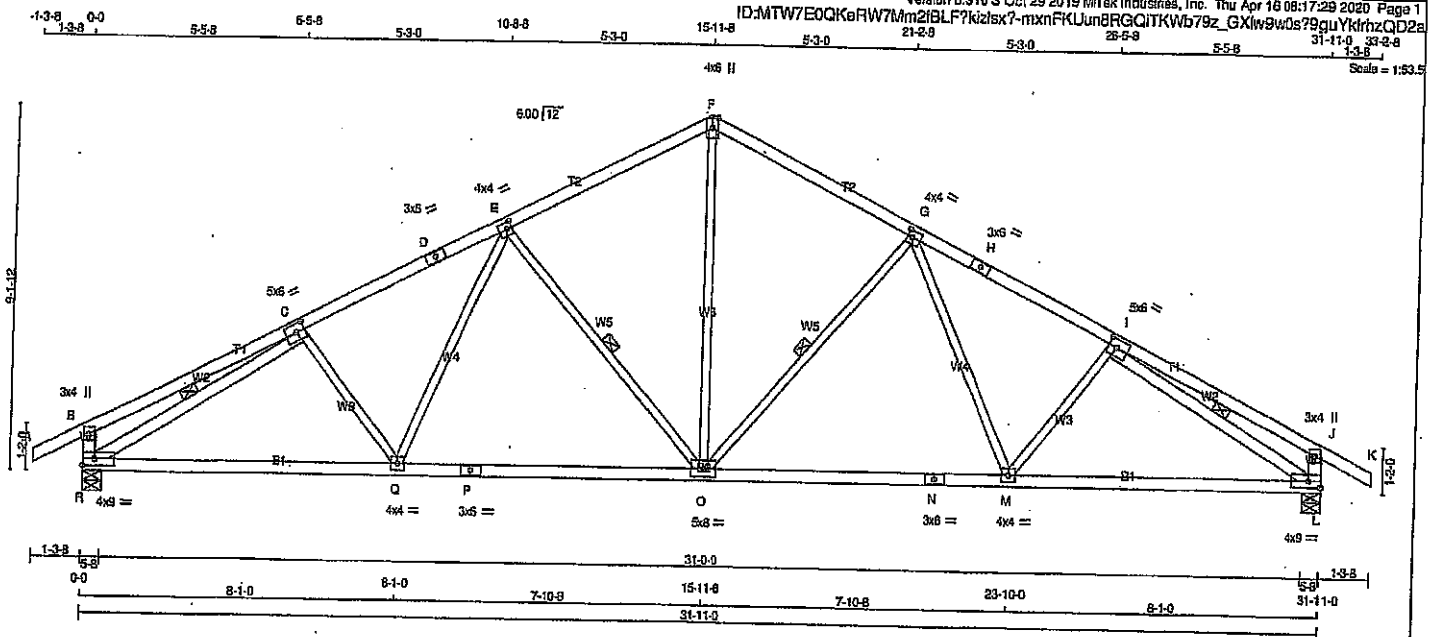


Structural component only
DWG# T-2006217

JOB NAME 408159	TRUSS NAME T36	QUANTITY 7	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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21-2-9 5-3-0 26-5-9 31-11-0 33-2-8
Scale = 1:53.5



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESOR.
A - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	No.2	SPF		
F - H	2x4	DRY	No.2	SPF		
H - K	2x4	DRY	No.2	SPF		
R - B	2x4	DRY	No.2	SPF		
L - J	2x4	DRY	No.2	SPF		
R - P	2x4	DRY	No.2	SPF		
P - N	2x4	DRY	No.2	SPF		
N - L	2x4	DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF		
R - C	2x4	DRY	No.2	SPF		
I - L	2x4	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (tablets in inches)	W	LEN	Y	X
JT TYPE				
B TMV+p	MT20	3.0	4.0	
C TMVW-1	MT20	5.0	6.0	2.50 2.50
D TS-1	MT20	3.0	6.0	
E TMVW-1	MT20	4.0	4.0	2.00 1.50
F TMV+p	MT20	4.0	6.0	Edge
G TMVW-1	MT20	4.0	4.0	2.00 1.50
H TS-1	MT20	3.0	6.0	
I TMVW-1	MT20	5.0	6.0	2.50 2.50
J TMV+p	MT20	3.0	4.0	
L BMVW-1	MT20	4.0	9.0	Edge
M BMVW-1	MT20	4.0	4.0	
N BS-1	MT20	3.0	6.0	
O BMVW-1	MT20	5.0	6.0	
P BS-1	MT20	3.0	6.0	
Q BMVW-1	MT20	4.0	4.0	
R BMVW-1	MT20	4.0	9.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
R	1884	0	1884	0	5-8	5-8
L	1884	0	1884	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
R	1330	885 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0
L	1330	885 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-O, E-O, C-R, J-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	Q-F	0 / 1211	0.27 (1)		
B-C	0 / 20	-91.8	-91.8 0.33 (1)	10.00	C-G	-701 / 0	0.32 (1)		
C-D	-2428 / 0	-91.8	-91.8 0.37 (1)	4.11	G-M	0 / 312	0.07 (1)		
D-E	-2428 / 0	-91.8	-91.8 0.37 (1)	4.11	M-I	-166 / 21	0.05 (1)		
E-F	-1808 / 0	-91.8	-91.8 0.34 (1)	4.65	E-O	-701 / 0	0.32 (1)		
F-G	-1808 / 0	-91.8	-91.8 0.34 (1)	4.65	C-E	0 / 312	0.07 (1)		
G-H	-2428 / 0	-91.8	-91.8 0.37 (1)	4.11	C-Q	-156 / 21	0.05 (1)		
H-I	-2428 / 0	-91.8	-91.8 0.37 (1)	4.11	R-Q	-2717 / 0	0.55 (1)		
I-J	0 / 20	-91.8	-91.8 0.33 (1)	10.00	I-L	-2717 / 0	0.55 (1)		
J-K	0 / 28	-91.8	-91.8 0.12 (1)	10.00					
R-B	-325 / 0	0.0	0.0 0.03 (1)	7.81					
L-J	-325 / 0	0.0	0.0 0.03 (1)	7.81					
R-Q	0 / 2268	-18.5	-18.5 0.51 (1)	10.00					
Q-P	0 / 2051	-18.5	-18.5 0.48 (1)	10.00					
P-O	0 / 2051	-18.5	-18.5 0.48 (1)	10.00					
O-N	0 / 2051	-18.5	-18.5 0.48 (1)	10.00					
N-M	0 / 2051	-18.5	-18.5 0.48 (1)	10.00					
M-L	0 / 2268	-18.5	-18.5 0.51 (1)	10.00					

TOTAL WEIGHT = 7 X 133 = 932 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/350$ (1.06")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.11")
ALLOWABLE DEFL.(TL) = $L/350$ (1.06")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.24")

CS1: TC=0.37/1.00 (G-I), BC=0.51/1.00 (L-M), WB=0.55/1.00 (I-L-1), SS=0.20/1.00 (J-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE REELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

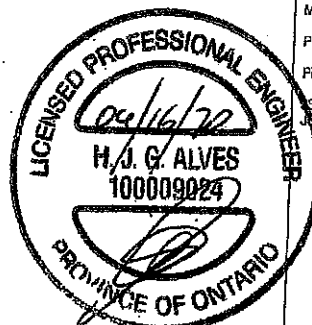
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1657 788	1987 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

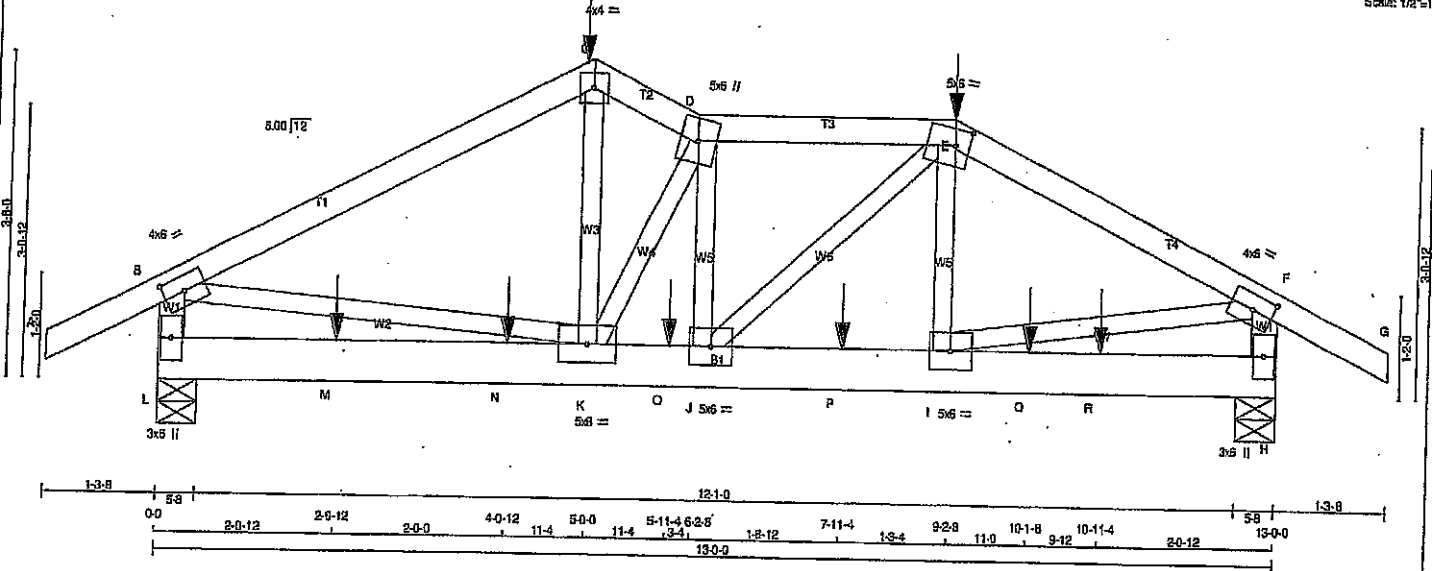
SI GRIP= 0.85 (1) (INPUT = 0.90)
SI METAL= 0.67 (1) (INPUT = 1.00)



Structural component only
DWG# T-2006218

JOB NAME 408159	TRUSS NAME T37	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER

N. L. G. A. RULES

CHORDS

A - C 2x4 DRY No.2

C - D 2x4 DRY No.2

D - E 2x4 DRY No.2

E - G 2x4 DRY No.2

L - R 2x4 DRY No.2

H - F 2x4 DRY No.2

L - H 2x6 DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE PLATES W LEN Y X

B TMW-t MT20 4.0 6.0 2.00 3.00

C TTW-p MT20 4.0 4.0

D TTWW-m MT20 5.0 6.0

E TTWW-m MT20 5.0 6.0 2.25 2.00

F TMW-t MT20 4.0 6.0 2.00 3.00

H BMV1-p MT20 3.0 6.0

I BMW-t MT20 5.0 6.0

J BMW-t MT20 5.0 6.0

K BMW-t MT20 5.0 6.0

L BMV1-p MT20 3.0 6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION

JT VERT 1368

L 1368

H 1587

MAXIMUM FACTORED GROSS REACTION

DOWN 1368

UP 0

INPUT BRG 5-8

RECORD IN-SX 5-8

UNFACTORED REACTIONS

1ST LOASE COMBINED

JT 983

L 983

H 1114

MAX/MIN COMPONENT REACTIONS

SNOW 658 / 0

LIVE 0 / 0

PERM LIVE 0 / 0

WIND 0 / 0

DEAD 305 / 0

SOIL 332 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. MAX. FACTORED FORCE (LBS)

FR-TO 0 / 28

A-B -1630 / 0

B-C -1630 / 0

C-D -1630 / 0

D-E -1758 / 0

E-F -1839 / 0

F-G 0 / 28

G-H -1808 / 0

H-F -1451 / 0

L-M 0 / 0

M-N 0 / 0

N-K 0 / 0

K-O 0 / 1765

O-J 0 / 1765

J-P 0 / 1654

P-I 0 / 1654

I-Q 0 / 0

Q-R 0 / 0

R-H 0 / 0

MAX. FACTORED FORCE (LBS)

FR-TO 0 / 28

A-B -1630 / 0

B-C -1630 / 0

C-D -1630 / 0

D-E -1758 / 0

E-F -1839 / 0

F-G 0 / 28

G-H -1808 / 0

H-F -1451 / 0

L-M 0 / 0

M-N 0 / 0

N-K 0 / 0

K-O 0 / 1765

O-J 0 / 1765

J-P 0 / 1654

P-I 0 / 1654

I-Q 0 / 0

Q-R 0 / 0

R-H 0 / 0

MAX. FACTORED FORCE (LBS)

FR-TO 0 / 28

A-B -1630 / 0

B-C -1630 / 0

C-D -1630 / 0

D-E -1758 / 0

E-F -1839 / 0

F-G 0 / 28

G-H -1808 / 0

H-F -1451 / 0

L-M 0 / 0

M-N 0 / 0

N-K 0 / 0

K-O 0 / 1765

O-J 0 / 1765

J-P 0 / 1654

P-I 0 / 1654

I-Q 0 / 0

Q-R 0 / 0

R-H 0 / 0

MAX. FACTORED FORCE (LBS)

FR-TO 0 / 28

A-B -1630 / 0

B-C -1630 / 0

C-D -1630 / 0

D-E -1758 / 0

E-F -1839 / 0

F-G 0 / 28

G-H -1808 / 0

H-F -1451 / 0

L-M 0 / 0

M-N 0 / 0

N-K 0 / 0

K-O 0 / 1765

O-J 0 / 1765

J-P 0 / 1654

P-I 0 / 1654

I-Q 0 / 0

Q-R 0 / 0

R-H 0 / 0

MAX. FACTORED FORCE (LBS)

FR-TO 0 / 28

A-B -1630 / 0

B-C -1630 / 0

C-D -1630 / 0

D-E -1758 / 0

E-F -1839 / 0

F-G 0 / 28

G-H -1808 / 0

H-F -1451 / 0

L-M 0 / 0

M-N 0 / 0

N-K 0 / 0

K-O 0 / 1765

O-J 0 / 1765

J-P 0 / 1654

P-I 0 / 1654

I-Q 0 / 0

Q-R 0 / 0

R-H 0 / 0

MAX. FACTORED FORCE (LBS)

FR-TO 0 / 28

A-B -1630 / 0

B-C -1630 / 0

C-D -1630 / 0

D-E -1758 / 0

E-F -1839 / 0

F-G 0 / 28

G-H -1808 / 0

H-F -1451 / 0

L-M 0 / 0

M-N 0 / 0

N-K 0 / 0

K-O 0 / 1765

O-J 0 / 1765

J-P 0 / 1654

P-I 0 / 1654

I-Q 0 / 0

Q-R 0 / 0

R-H 0 / 0

MAX. FACTORED FORCE (LBS)

FR-TO 0 / 28

A-B -1630 / 0

B-C -1630 / 0

C-D -1630 / 0

D-E -1758 / 0

E-F -1839 / 0

F-G 0 / 28

G-H -1808 / 0

H-F -1451 / 0

L-M 0 / 0

M-N 0 / 0

N-K 0 / 0

K-O 0 / 1765

O-J 0 / 1765

J-P 0 / 1654

P-I 0 / 1654

I-Q 0 / 0

Q-R 0 / 0

R-H 0 / 0

MAX. FACTORED FORCE (LBS)

FR-TO 0 / 28

A-B -1630 / 0

B-C -1630 / 0

C-D -1630 / 0

D-E -1758 / 0

E-F -1839 / 0

F-G 0 / 28

G-H -1808 / 0

H-F -1451 / 0

L-M 0 / 0

M-N 0 / 0

N-K 0 / 0

K-O 0 / 1765

O-J 0 / 1765

J-P 0 / 1654

P-I 0 / 1654

I-Q 0 / 0

Q-R 0 / 0

R-H 0 / 0

MAX. FACTORED FORCE (LBS)

FR-TO 0 / 28

A-B -1630 / 0

B-C -1630 / 0

C-D -1630 / 0

D-E -1758 / 0

E-F -1839 / 0

F-G 0 / 28

G-H -1808 / 0

H-F -1451 / 0

L-M 0 / 0

M-N 0 / 0

N-K 0 / 0

K-O 0 / 1765

O-J 0 / 1765

J-P 0 / 1654

P-I 0 / 1654

I-Q 0 / 0

Q-R 0 / 0

R-H 0 / 0

MAX. FACTORED FORCE (LBS)

FR-TO 0 / 28

A-B -1630 / 0

B-C -1630 / 0

C-D -1630 / 0

D-E -1758 / 0

E-F -1839 / 0

F-G 0 / 28

G-H -1808 / 0

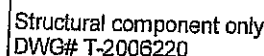
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L-M 0 / 0

M-N 0 / 0

N-K 0 / 0

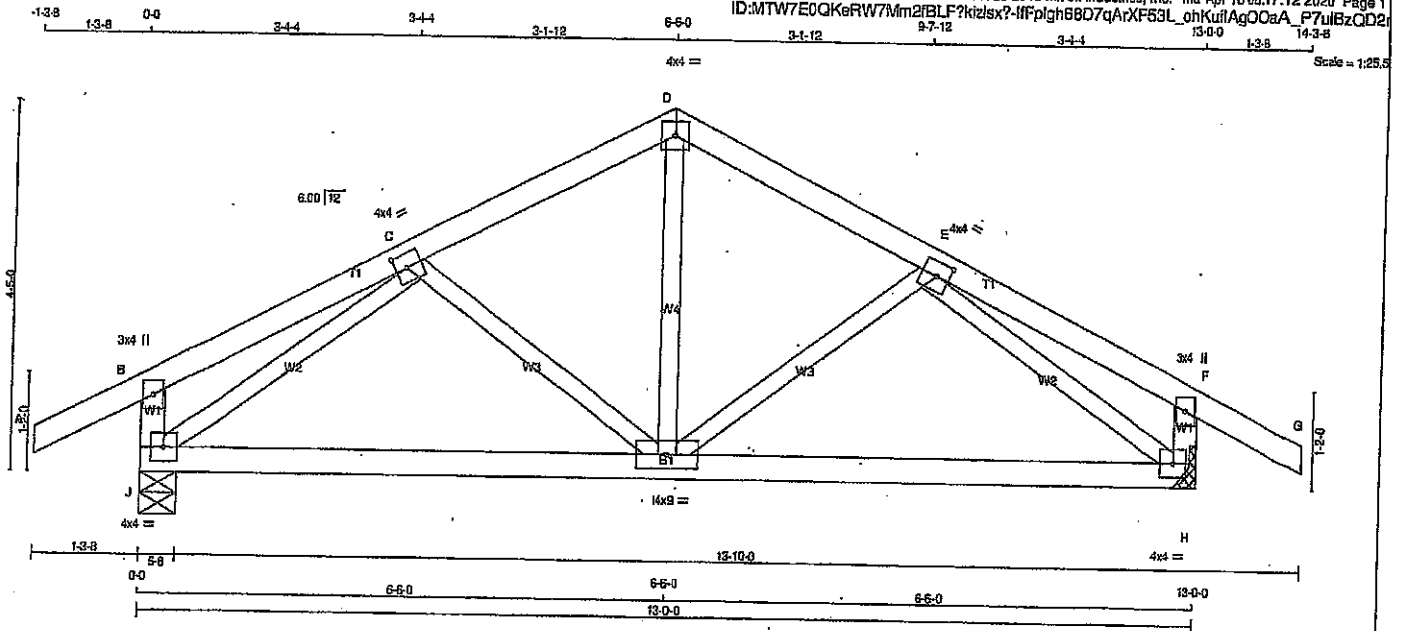
amarack Fast Trues, Burlington



JOB NAME 408159	TRUSS NAME T39	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMVW-t	MT20	4.0	4.0		
J	BMVW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	UPLIFT
J	841	0	0	5-8
H	841	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	593	402 / 0	0 / 0	0 / 0	0 / 0	0 / 0	190 / 0	0 / 0
H	593	402 / 0	0 / 0	0 / 0	0 / 0	0 / 0	190 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED MAX. CSF (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSF (LC)
FR-TO					FR-TO		
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	I-D	0 / 355	0.08 (1)
B-C	0 / 14	-91.8	-91.8 0.14 (1)	10.00	I-E	-171 / 0	0.05 (1)
C-D	-872 / 0	-91.8	-91.8 0.11 (1)	6.25	C-L	-171 / 0	0.05 (1)
D-E	-872 / 0	-91.8	-91.8 0.11 (1)	6.25	J-C	-910 / 0	0.25 (1)
E-F	0 / 14	-91.8	-91.8 0.14 (1)	10.00	E-H	-910 / 0	0.25 (1)
F-G	0 / 28	-91.8	-91.8 0.12 (1)	10.00			
J-B	-242 / 0	0.0	0.0 0.02 (1)	7.81			
H-F	-242 / 0	0.0	0.0 0.02 (1)	7.81			
J-I	0 / 724	-18.5	-18.5 0.28 (4)	10.00			
I-H	0 / 724	-18.5	-18.5 0.28 (4)	10.00			

TOTAL WEIGHT = 3 X 52 = 156 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 36.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 085-09, CSA 085-14
- TPO 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.43")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.02")
ALLOWABLE DEFL. (TL) = $L/360$ (0.43")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.05")

CSF: TC=0.14/1.00 (E-F), BC=0.28/1.00 (I-J), WS=0.25/1.00 (C-J), SS=0.18/1.00 (B-C)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

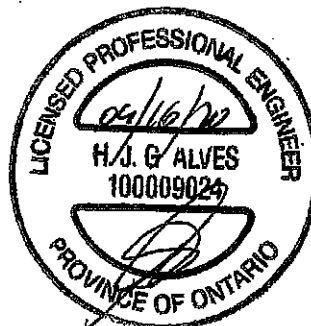
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354	1667 798 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

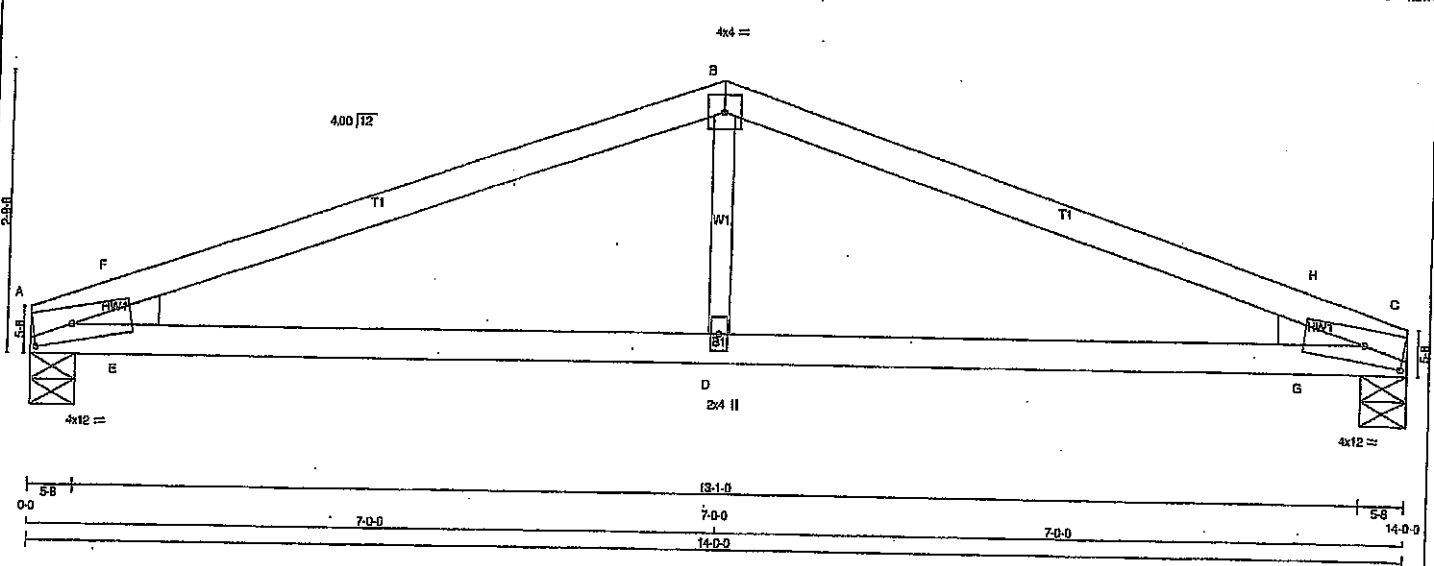
JSI GRIP = 0.81 (H) (INPUT = 0.90)
JSI METAL = 0.31 (C) (INPUT = 1.00)



Structural component only
DWG# T-2006221

JOB NAME 408159	TRUSS NAME T41	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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14-0-0
Scale = 1:21.1



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
A - C	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-m	MT20	4.0	12.0	2.00	4.75
B	TTW-p	MT20	4.0	4.0		
C	TMBH1-m	MT20	4.0	12.0	2.00	4.75
D	BMW-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	HEEL
JT	VERT	DOWN	UP	IN-SX	WEDGE
A	772	0	0	5-8	2x4 L
C	772	0	0	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LOSE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
A	546	358 / 0	0 / 0	0 / 0	188 / 0	0 / 0
C	546	358 / 0	0 / 0	0 / 0	188 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO							
A-F	-1258 / 0	-91.8	-91.8	0.16 (1)	5.57	0 / 280	0.08 (1)
F-B	-1253 / 0	-91.8	-91.8	0.51 (1)	5.07	-292 / 8	0.00 (1)
B-H	-1253 / 0	-91.8	-91.8	0.51 (1)	5.07	0 / 280	0.00 (1)
H-C	-1258 / 0	-91.8	-91.8	0.16 (1)	5.57	-292 / 8	0.00 (1)
A-E	0 / 1181	-18.5	-18.5	0.55 (1)	10.00		
E-D	0 / 1181	-18.5	-18.5	0.55 (1)	10.00		
D-G	0 / 1181	-18.5	-18.5	0.55 (1)	10.00		
G-C	0 / 1181	-18.5	-18.5	0.55 (1)	10.00		

TOTAL WEIGHT = 3 X 37 = 110 lb

(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:-

- PART 9 OF BCBC 2016, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/800 (0.47")
CALCULATED VERT. DEFL. (LL) = L/999 (0.09")
ALLOWABLE DEFL. (TL) = L/800 (0.47")
CALCULATED VERT. DEFL. (TL) = L/999 (0.16")

CSI: TC=0.51/1.00 (B-F:1), BC=0.56/1.00 (A-E:1),
WB=0.08/1.00 (B-D:1), SS=0.22/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

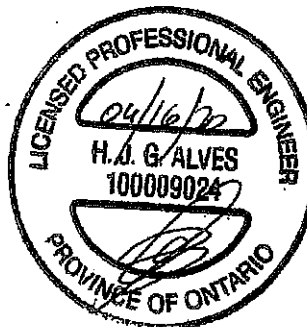
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1657 798

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

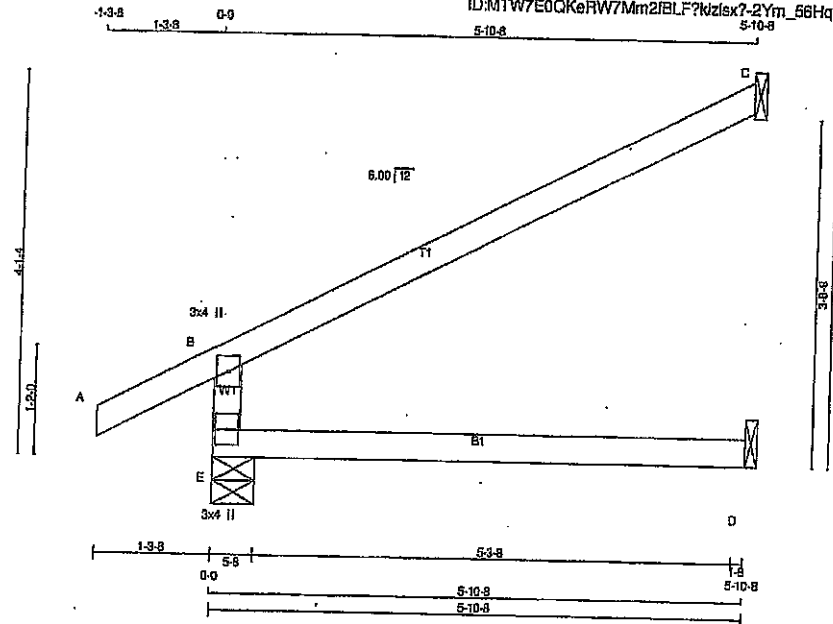
JSI GRIP= 0.48 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)



Structural component only
DWG# T-2006222

JOB NAME 408159	TRUSS NAME J1	QUANTITY 22	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Wed Apr 15 12:51:33 2020 Page 1
 ID:MTW7E0QKerW7Mm2BLF?Kzlsx7-2Ym_56HqISYM0LXU751jaS7VSDPmXnIKWSzQU76



Scale = 1:22.5

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER
E - B	2x4	DRY No.2
A - C	2x4	DRY No.2
E - D	2x4	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	525	0	525	0	5-8	5-8
C	202	0	202	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	369	257 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
E-B	-461 / 0	0.0 0.0	0.13 (4)	7.81			
A-B	0 / 29	-91.8 -91.8	0.12 (1)	10.00			
B-C	-30 / 0	-91.8 -91.8	0.54 (1)	6.25			
E-D	0 / 0	-18.5 -18.5	0.13 (4)	10.00			

TOTAL WEIGHT = 18 X 17 = 302 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 33.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, CBC 2012, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 088-09, CSA 088-14
 - TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20')
 CALCULATED VERT. DEFL. (LL) = L/999 (0.00')
 ALLOWABLE DEFL. (TL) = L/600 (0.20')
 CALCULATED VERT. DEFL. (TL) = L/999 (0.03')

CS1: TO=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS1=0.24/1.00 (B-C:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

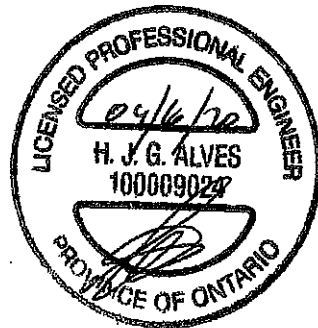
NAIL VALUES

PLATE GRIP (DRY) (PS)	SHEAR (PL)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20 618 354	1657 788	1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

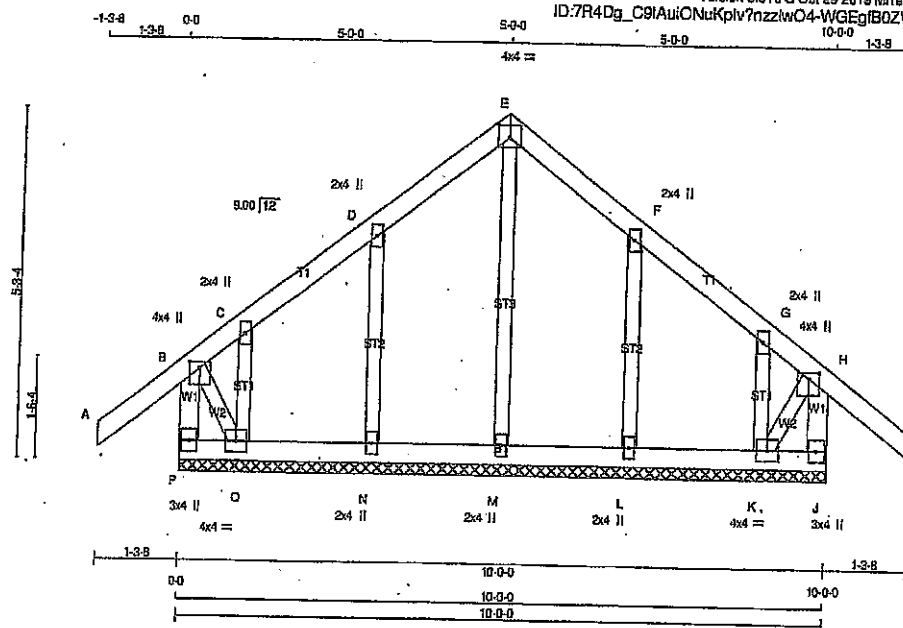
JSI GRIP = 0.19 (E) (INPUT = 0.90)
 JSI METAL = 0.13 (B) (INPUT = 1.00)



Structural component only
 DWG# T-2006203

Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Wed Apr 15 15:04:46 2020 Page 1
ID:7R4Dg_C9IAuIONuKplv?nzzlwO4-WGEgIBQZVJaFS4ujEI?P9XinwU9IKCDFWqS_YgzQSA

Scale: 3/8"=1'



LUMBER		
N. L. & A. RULES		
CHORDS	SIZE	LUMBER
P - B	2x4	No.2
A - E	2x4	No.2
E - I	2x4	No.2
J - H	2x4	No.2
P - J	2x4	No.2

ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" O.C.

PLATES (tablets in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMWV+p	MT20	4.0	4.0 1.00 2.00
C, D, F, G				
C	TMW+w	MT20	2.0	4.0
E	TTW+p	MT20	4.0	4.0 2.25 2.00
H	TMWV+p	MT20	4.0	4.0 1.00 2.00
J	BMV1+p	MT20	3.0	4.0
K	BMWV1-1	MT20	4.0	4.0
L, M, N				
L	BMV1+w	MT20	2.0	4.0
O	BMWV1-1	MT20	4.0	4.0
P	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBB				
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED		
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM		LENGTH	FR-TO			
P-B	-286 / 0	0.0	0.0	0.03 (1)	7.81	M-E	-150 / 0	0.05 (1)
A-B	0 / 38	-91.8	-91.8	0.12 (1)	10.00	N-D	-221 / 0	0.05 (1)
B-C	-59 / 0	-91.8	-91.8	0.12 (1)	8.25	Q-C	-31 / 0	0.00 (1)
C-D	0 / 3	-91.8	-91.8	0.06 (1)	10.00	L-F	-221 / 0	0.05 (1)
D-E	-12 / 0	-91.8	-91.8	0.06 (1)	6.25	K-G	-31 / 0	0.00 (1)
E-F	-12 / 0	-91.8	-91.8	0.06 (1)	6.25	S-O	0 / 11	0.00 (1)
F-G	0 / 3	-91.8	-91.8	0.06 (1)	10.00	K-H	0 / 11	0.00 (1)
G-H	-59 / 0	-91.8	-91.8	0.12 (1)	6.25			
H-I	0 / 38	-91.8	-91.8	0.12 (1)	10.00			
J-H	-286 / 0	0.0	0.0	0.03 (1)	7.81			
P-D	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
C-N	0 / 6	-18.5	-18.5	0.02 (4)	10.00			
N-M	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
M-L	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
L-K	0 / 6	-18.5	-18.5	0.02 (4)	10.00			
K-J	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCG 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (N-O:4),
WB=0.05/1.00 (E-M:1), SSI=0.05/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

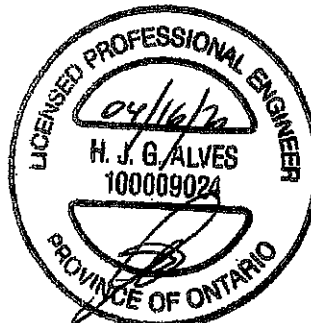
NAFL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)



Structural component only
DWG# T-2006198

Tamarack Roof Truss, Burlington

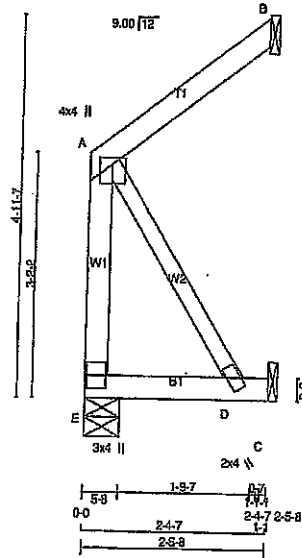
ANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

```
GRIP= 0.21 (B) (INPUT = 0.90 )
METAL= 0.05 (B) (INPUT = 1.00 )
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JOB NAME 408158	TRUSS NAME J21	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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 ID:7RADg_C9IAuIONuKplv7nzzlwO4-24gIR7xi7SOqwuXgaUAJAc4poblv6HAJQ0DzQ8Am
 2-4-7 2-5-8 1-7
 Scale = 1:27.7



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - A	2x4 DRY	No.2		
A - B	2x4 DRY	No.2		
E - C	2x4 DRY	No.2		
ALL WEBS	2x3 DRY	No.2		
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00	
D	BMVW+p	MT20	2.0	4.0			
E	BMV1+p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	131	0	131	0	5-8	5-8
B	109	0	109	0	1-8	1-8
C	23	0	23	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS					
JT	1ST CASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	33	61/0	0/0	0/0	0/0	32/0	0/0
B	75	61/0	0/0	0/0	0/0	14/0	0/0
C	18	0/0	0/0	0/0	0/0	18/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
E-A	-109/0	0.0	0.0	0.02 (1)			
A-B	0/0	-91.8	-91.8	0.09 (1)	7.81	0/0	0.00 (1)
				10.00			
E-D	0/0	-18.5	-18.5	0.03 (4)	10.00		
D-C	0/0	-18.5	-18.5	0.02 (4)	10.00		

TOTAL WEIGHT = 2 X 12 = 24 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 8.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBC 2010, NBC 2012, ABC 2019
 - PART 9 OF NBC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/350 (0.19")
 CALCULATED VERT. DEFL. (LL) = 1/999 (0.00")
 ALLOWABLE DEFL. (TL) = 1/350 (0.19")
 CALCULATED VERT. DEFL. (TL) = 1/999 (0.00")

CS: TC=0.09/1.00 (A-B:1), SC=0.03/1.00 (D-E:4),
 WB=0.00/1.00 (A-D:1), SE=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

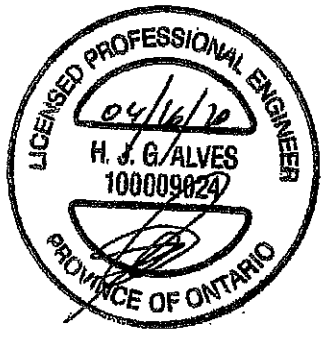
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 613 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.09 (A) (INPUT = 0.90)
 JSI METAL = 0.02 (A) (INPUT = 1.00)



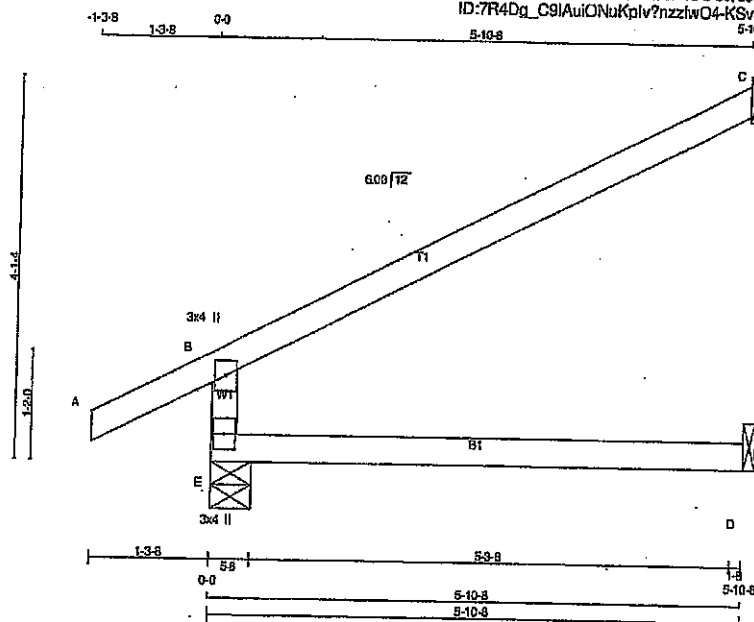
Structural component only
 DWG# T-2006196

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Wed Apr 16 15:04:45 2020 Page 1
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2-10-8



Structural component only
DWG# T-2006197

JOB NAME 408157	TRUSS NAME J1	QUANTITY 25	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Wed Apr 15 12:13:16 2020 Page 1	



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	No.2
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
E BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
E	525	525	5-8	5-8
C	202	202	1-8	1-8
D	45	50	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERMILIVE	WIND	DEAD	SOIL
E	369	257/0	0/0	0/0	0/0	111/0	0/0
C	139	113/0	0/0	0/0	0/0	25/0	0/0
D	36	0/0	0/0	0/0	0/0	38/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
E-B	FR-TO	-461/0	0.0	0.0	0.13 (4)	7.31	FR-TO	0.0	0.0
A-B		0/28	-91.8	-91.8	0.12 (1)	10.00			
B-C		-30/0	-91.8	-91.8	0.54 (1)	6.25			
E-D		0/0	-18.5	-18.5	0.13 (4)	10.00			

TOTAL WEIGHT = 25 X 17 = 425 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2018, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 51.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")

CALCULATED VERT. DEFL. (LL) = L/999 (0.00")

ALLOWABLE DEFL. (TL) = L/360 (0.20")

CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CS: TO=0.54/1.00 (B-C:1), BC=0.12/1.00 (D-E:4), WB=0.00/1.00 (A-B:0), SS=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

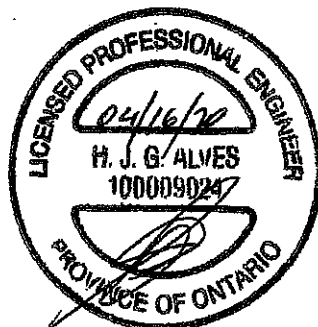
NAIL VALUES

PLATE GRIP (PS)	DRY (PL)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.19 (E) (INPUT = 0.90)
JSI METAL = 0.13 (B) (INPUT = 1.00)



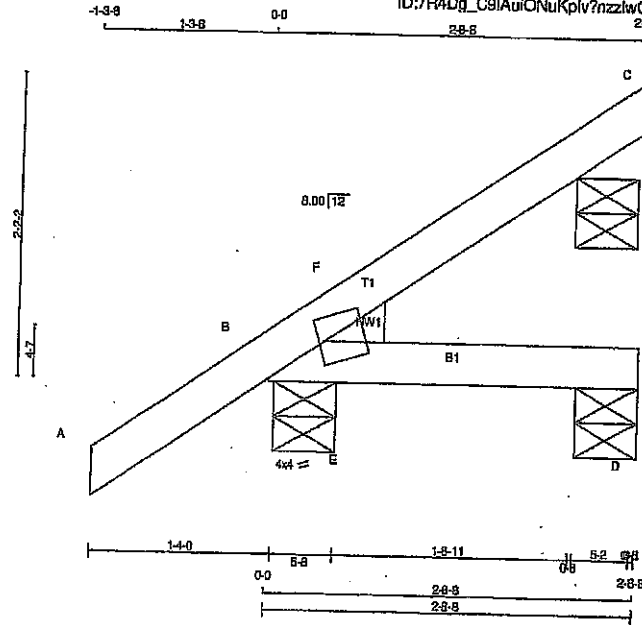
Structural component only
DWG# T-2006173

JOB NAME 408157	TRUSS NAME J3	QUANTITY 7	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:15.3



LUMBER

N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
A - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	4.0	4.0	2.00 0.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
C	103	0	103	0	5-8	5-8	
B	275	0	275	0	5-8	5-8	
D	47	0	47	0	5-8 (5-7)	5-8	2x4 L

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	71	55/0	0/0	0/0	0/0	18/0	0/0
B	182	140/0	0/0	0/0	0/0	58/0	0/0
D	34	14/0	0/0	0/0	0/0	20/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)
FR-TO				
A-B	0/34	-91.8 -91.8 0.13 (5)	10.00	
B-F	-15/0	-91.8 -91.8 0.02 (4)	6.25	
F-C	0/4	-91.8 -91.8 0.08 (1)	10.00	
B-E	0/0	-18.5 -18.5 0.08 (1)	10.00	
E-D	0/0	-18.5 -18.5 0.08 (1)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 7 X 9 = 63 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.8	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24" IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPOC 2011, TPOC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSR: TC=0.13/1.00 (A-B-S), BC=0.08/1.00 (D-E-1),
WB=0.00/1.00 (E-F-1), SS=0.09/1.00 (A-B-S)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

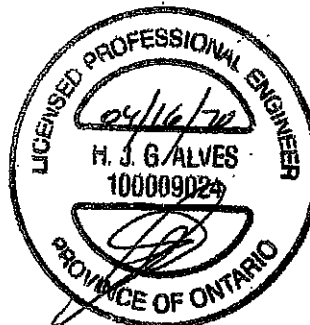
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	818 354	1857 788	1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.18 (B) (INPUT = 0.90)
JSI METAL=0.04 (B) (INPUT = 1.00)



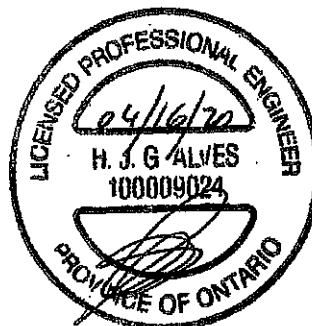
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DWG# T-2006175

Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Wed Apr 15 12:13:18 2020 Page 1
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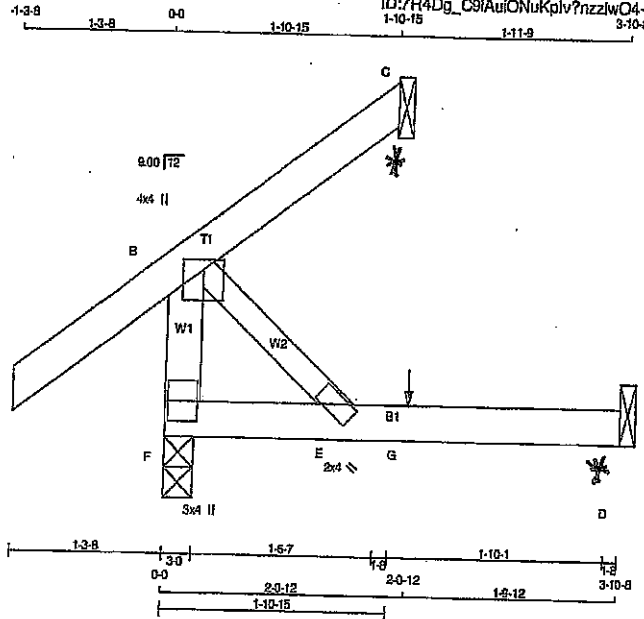
Structural component only
DWG# T-2006176

ANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408157	J5	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington



Scale = 1:17.6

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ID:7R4Dg_C9iAuONuKpIv?nzzlwO4-DD9meIwu8dXpBze6Lx4kC9n[kCaZ]V51137?QzQUhU

LUMBER

N. L. G. A. RULES	SIZE	LUMBER
CHORDS	2x4	No.2
F - B	2x4	DRY
A - C	2x4	DRY
F - D	2x4	DRY
ALL WEBS	2x3	DRY
DRY: SEASONED LUMBER.		No.2

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
E BMV+w	MT20	2.0	4.0		
F BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	296	0	296	0
C	42	0	42	0
D	36	0	40	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LOSE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	29	23/-25	0/0	0/0	0/0	63/0	0/0
D	29	0/0	0/0	0/0	0/0	5/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORZ. LOAD (LC1)	MAX. FACTORED LENGTH (LC2)	W E B S	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LC1)
FR-TO									
F-B	-260/0	0.0	0.0	0.03 (1)	7.81	B-E	0/0	0.00 (1)	
A-B	0/39	-91.8	-91.8	0.14 (5)	10.00				
B-C	-27/0	-91.8	-91.8	0.13 (5)	6.25				
F-E	0/0	-18.5	-18.5	0.08 (4)	10.00				
E-G	0/0	-18.5	-18.5	0.08 (4)	10.00				
G-D	0/0	-18.5	-18.5	0.08 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	1	1	1	BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 12 = 36 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 26.6 PSF
BOT CH.	LL = 6.0 PSF
	DL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.14/1.00 (A-B-5), BC=0.03/1.00 (D-E-4),
WB=0.00/1.00 (B-E-1), SS=0.09/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

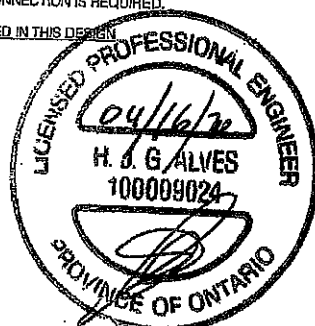
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1867 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.21 (B) (INPUT = 0.30)
JSI METAL = 0.05 (B) (INPUT = 1.00)

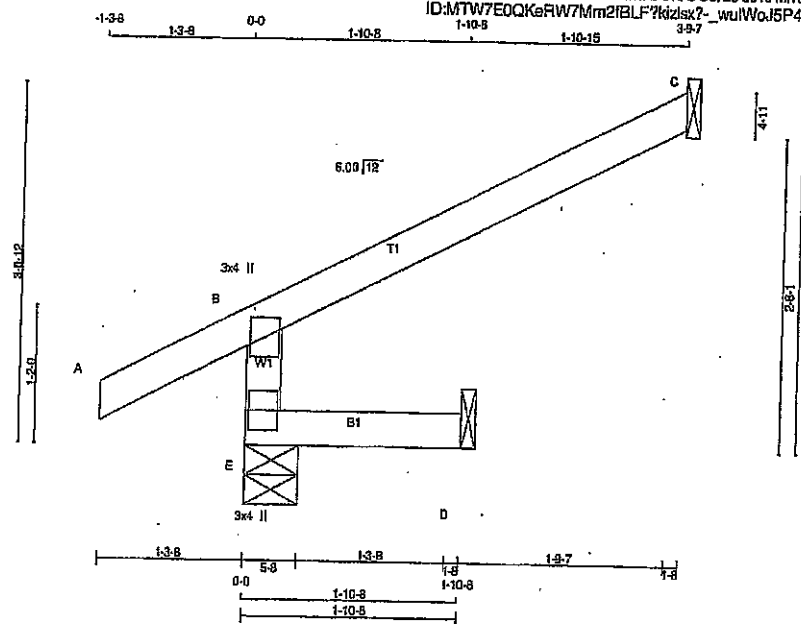


Structural component only
DWG# T-2006177

Structural component only
DWG# T-2006204

JOB NAME 406159	TRUSS NAME J32	QUANTITY 5	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.310.5 Oct 29 2019 Mitek Industries, Inc. Wed Apr 15 12:51:35 2020 Page 1
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Scale = 1:18.2

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B - TMV+p	MT20	3.0	4.0		
E - BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	361	0	361	0	5-8	5-8
C	130	0	130	0	1-8	1-8
D	16	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM.LIVE	WIND	DEAD
E	250	190 / 0	0 / 0	0 / 0	0 / 0	60 / 0
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)		MAX. CS1 (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB. FORCE (LBS)	MAX. CS1 (LC)
		FROM	TO				
FR-TO							
E-B	-342 / 0	0.0	0.0	0.01 (4)	7.81		
A-B	0 / 28	-91.8	-91.8	0.13 (5)	10.00		
B-C	-19 / 0	-91.8	-91.8	0.22 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 5 X 10 = 48 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, CBC 2012, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/899 (0.00")
 ALLOWABLE DEFL.(TL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/899 (0.00")

CS1: TC=0.22/1.00 (B-C-1), BC=0.02/1.00 (D-E-4), WB=0.09/1.00 (n=0), SS=0.15/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

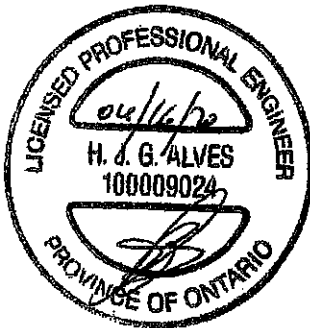
NAIL VALUES

PLATE GRIP (DRY) SHEAR (PSI)	SECTION (PLJ)	MAX MIN	MAX MIN	MAX MIN
MT20	818	354	1657	788 1997 1656

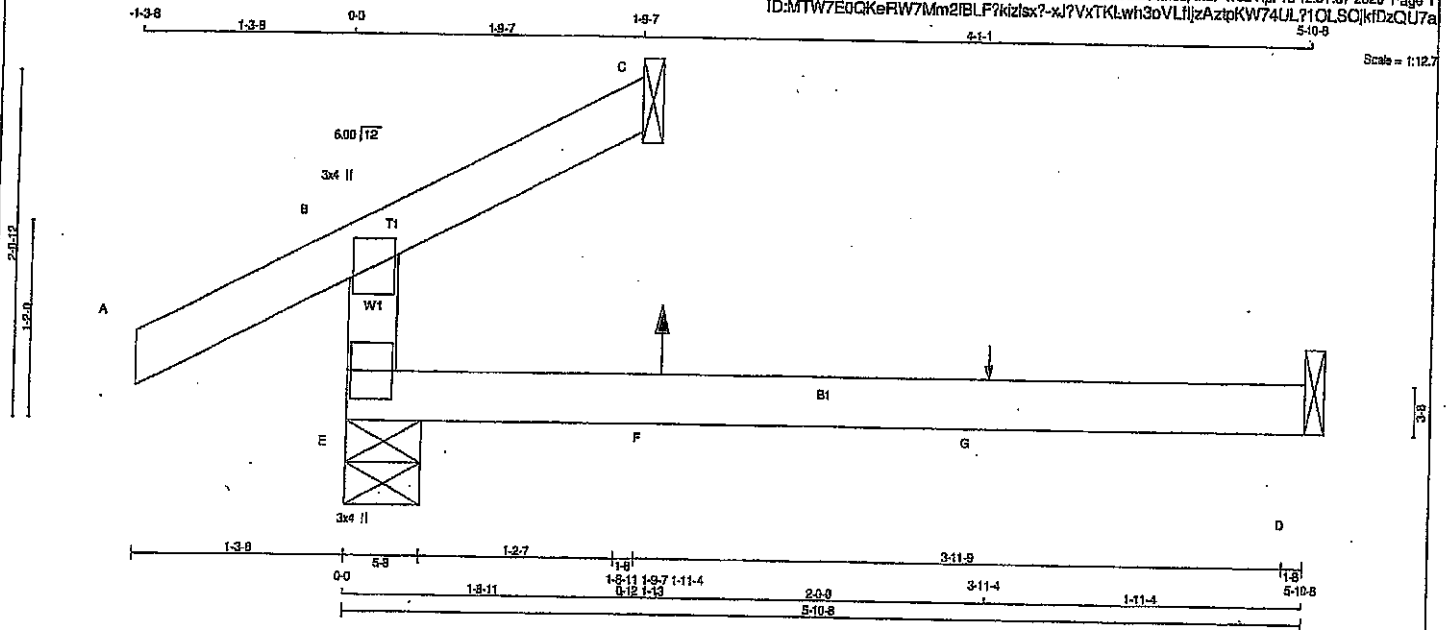
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.14 (E) (INPUT = 0.90)
 JSI METAL = 0.09 (B) (INPUT = 1.00)



Structural component only
 DWG# T-2006205



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	NO.2
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
E	284	0	284	0	5-8	5-8
C	83	0	83	0	1-8	1-8
D	44	0	52	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) G, D

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN. COMPONENT REACTIONS				
		1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND
E	200	137 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	45	21 / 0	0 / 0	0 / 0	25 / 0	0 / 0
D	35	0 / -3	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS		WEBS	
		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)
E-B	-227 / 0	0.0	0.0	0.11 (4)	7.81
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B-C	-9 / 9	-91.8	-91.8	0.08 (4)	10.00
E-F	0 / 0	-18.5	-18.5	0.14 (4)	10.00
F-G	0 / 0	-18.5	-18.5	0.14 (4)	10.00
G-D	0 / 0	-18.5	-18.5	0.14 (4)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11.4	7	1	12	FRONT	VERT	TOTAL	—	C1
G	3-11.4	1	1	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 9 X 12 = 35 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN/C/G

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-09, CSA 085-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.12/1.00 (A-B:1), SC=0.14/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SS=0.09/1.00 (A-B:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

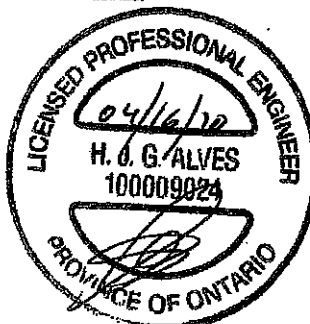
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX
MT20	918	354
	1687	788
	1987	1686

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



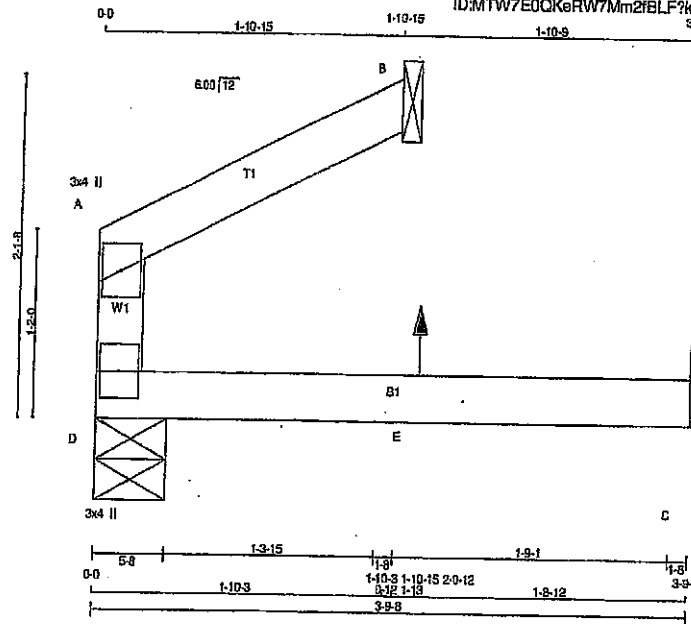
Structural component only
DWG# T-2006206

JOB NAME 408159	TRUSS NAME J35	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Wed Apr 15 12:51:39 2020 Page 1
ID:MTW7E0QKerW7Mm2fBLF?kizax?-i7GM9MbSIUVkaohrNCRyEPatCyTxueviCrj6zQU7Y

Scale = 1:12.2



TOTAL WEIGHT = 8 lb

LUMBER	N. L. G. A. RULES
CHORDS	SIZE
D - A	2x4 DRY No.2
A - B	2x4 DRY No.2
D - C	2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0	
D	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
D	119	0	119	0	5-8	5-8
B	89	0	89	0	1-8	1-8
C	31	0	35	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D, C

UNFACTORED REACTIONS

JT	1ST LOSE	MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	85	49/0	0/0	0/0	0/0	37/0	0/0
B	62	45/0	0/0	0/0	0/0	17/0	0/0
C	25	0/-2	0/0	0/0	0/0	25/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1	MAX. FACTORED (PL)	MAX. FACTORED (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LC)	WEBS	
										FR-TO	FR-TO
D-A	0/0	-87/0	0.0	0.0	0.03 (4)	7.81					
A-B	0/3	-91.8	-91.8	0.06 (1)	10.00						
D-E	0/0	-18.5	-18.5	0.05 (4)	10.00						
E-C	0/0	-18.5	-18.5	0.05 (4)	10.00						

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-0-12	7	1	12	BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.01")

CSI: TC=0.08/1.00 (A-B:1), BC=0.08/1.00 (C-D:4),
WB=0.02/1.00 (W:0), SS=0.06/1.00 (A-B:1)

DOI LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

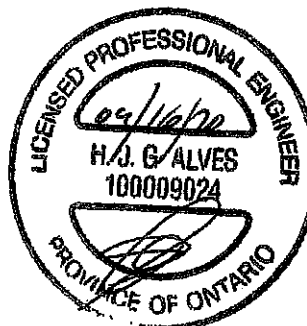
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PS)	(PL)	(PL)
MT20	618	354	1667 788 1987 1656

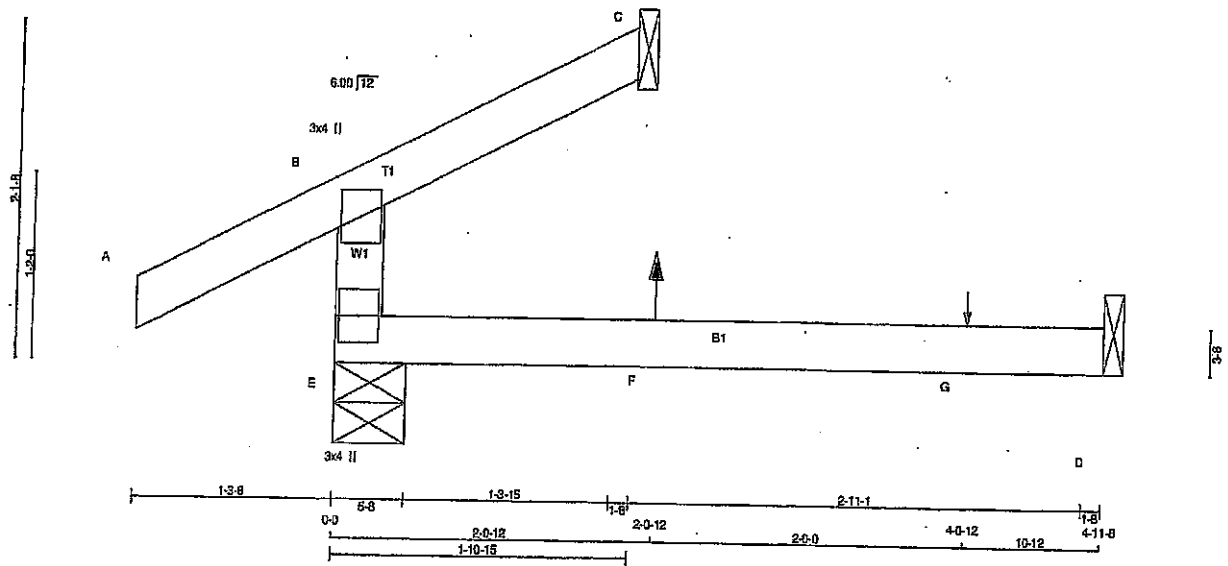
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.04 (D) (INPUT = 0.80)
JSI METAL= 0.02 (A) (INPUT = 1.00)



Structural component only
DWG# T-2006208



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2	
A - G	2x4	DRY	No.2	
E - D	2x4	DRY	No.2	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	282	0	282	0	5-8	5-8
C	66	0	66	0	1-8	1-8
D	36	0	43	0	1-8	1-8

SEE MITEK STANDARD DETAIL B87791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	200	127/0	0/0	0/0	0/0	73/0	0/0
C	46	37/0	0/0	0/0	0/0	9/0	0/0
D	29	0/-2	0/0	0/0	0/0	31/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. CSI (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
			FROM	TO						
FR-TO										
E-B	-234/0	0.0	0.0	0.09 (4)	7.81					
A-B	0/28	-91.8	-91.8	0.12 (1)	10.00					
B-C	-10/0	-91.8	-91.8	0.06 (1)	10.00					
E-F	0/0	-18.5	-18.5	0.09 (4)	10.00					
F-G	0/0	-18.5	-18.5	0.09 (4)	10.00					
G-D	0/0	-18.5	-18.5	0.09 (4)	10.00					

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	7	1	12	BACK	VERT	TOTAL	---	C1
G	4-0-12	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 11 = 21 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.02")

CSI: TC=0.12/1.00 (A-B:1), BC=0.09/1.00 (D-E:4),
WB=0.09/1.00 (n=2:0), SSI=0.09/1.00 (A-B:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

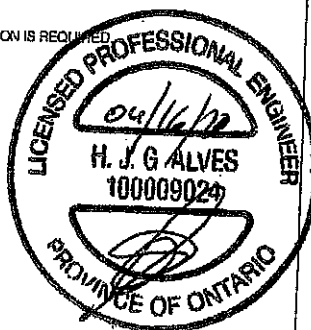
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

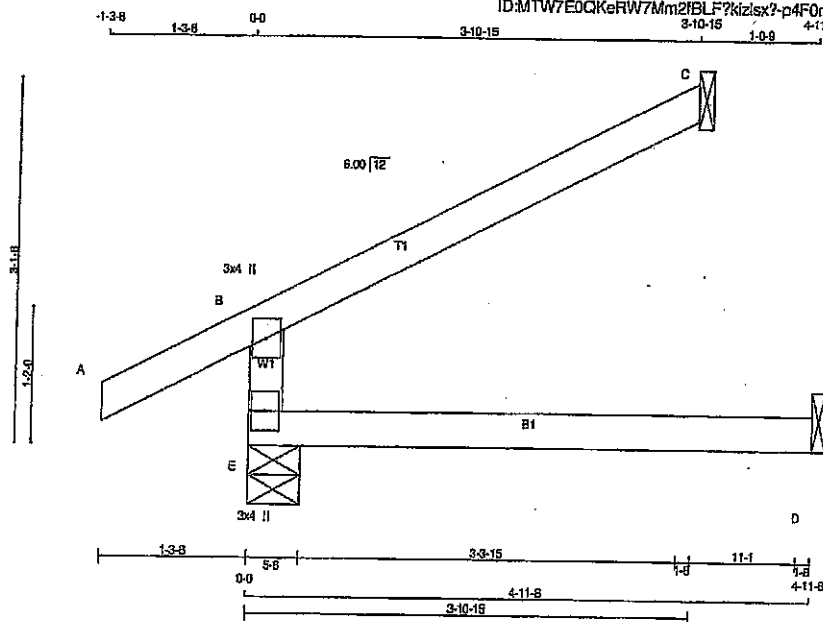
SI GRIP= 0.10 (E) (INPUT = 0.90)
SI METAL= 0.06 (B) (INPUT = 1.00)



Tamarack Roof Truss, Burlington

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Scale = 1:1B 2



TOTAL WEIGHT = 2 X 13 = 26 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQD
GROSS REACTION		GROSS REACTION				BRG	BRG
JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT	IN-SX	IN-SX
E	402	0	402	0	0	5-8	5-8
C	135	0	135	0	0	1-8	1-8
D	38	0	43	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791 H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	283	184 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0
C	93	75 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
D	31	0 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E. C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: {4}

CHORDS				WEBBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LO1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		FR-TO			
E-B	-349 / 0	0.0	0.0	0.09 (4)			7.81
A-B	0 / 28	-91.8	-91.8	0.12 (3)			10.00
B-C	-20 / 0	-91.8	-91.8	0.24 (1)			6.25
E-D	0 / 0	-18.5	-18.5	0.09 (4)			10.00

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-TO		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD =		39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF ECBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA OBC-09, CSA OBC-14
- TPK 2011, TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= 1/360 (0.19")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL)= 1/360 (0.19")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.02")

CSI: TC=0.24/1.00 (B-C:1), EC=0.09/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

	PLATE GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
TT20	618	354	1667	788	1987	1656

LATE PLACEMENT TOL. = 0.250 inches

LATE ROTATION TOL = 5.0 Deg.

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SI GRIP= 0.15 (E) (INPUT = 0.90 )
SI METAL= 0.10 (B) (INPUT = 1.00 )
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Structural component only
DWG# T-2006210

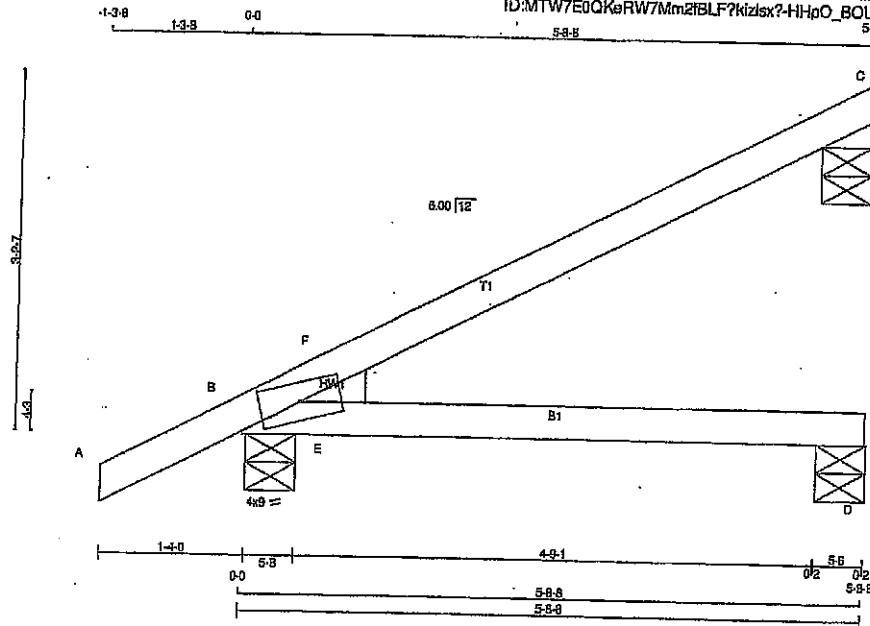


JOB NAME 408159	TRUSS NAME J41	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/16"

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	4.0	9.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
C	225	0	225	0	5-8 (5-7)	5-8	
B	439	0	439	0	5-8	5-8	2x4 L
D	90	0	90	0	5-8 (5-5)	5-8	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	155	121/0	0/0	0/0	0/0	35/0	0/0
B	308	215/0	0/0	0/0	0/0	93/0	0/0
D	67	25/0	0/0	0/0	0/0	42/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	FACTORED HORZ. LOAD (LC)	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LC)
FR-TO						
A-B	0/25	-91.8	-91.8 0.12 (1)	10.00	E-F	-254/8 0.00 (1)
B-F	-22/0	-91.8	-91.8 0.08 (4)	6.25		
F-C	0/4	-91.8	-91.8 0.37 (1)	10.00		
B-E	0/0	-18.5	-18.5 0.30 (1)	10.00		
E-D	0/0	-18.5	-18.5 0.30 (1)	10.00		

TOTAL WEIGHT = 3 X 16 = 48 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
DL	6.0	PSF
BOT CH.	LL = 0.0	PSF
DL	7.4	PSF
TOTAL LOAD	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/998 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/500 (0.14")

CS: TC=0.37/1.00 (C-F:1), BC=0.30/1.00 (B-E:1), WB=0.00/1.00 (E-F:1), SS=0.21/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (B) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)



Structural component only
DWG# T-2006211

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

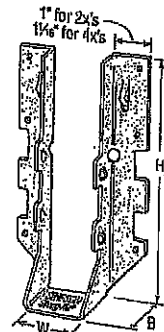
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

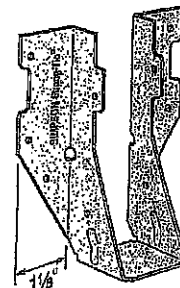
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

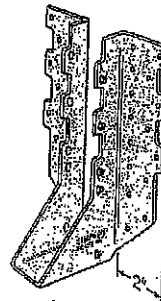
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options Information on p. 126.



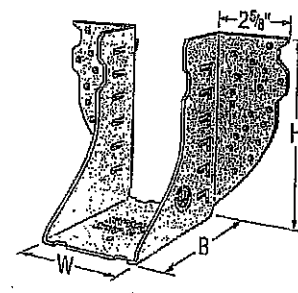
LUS26



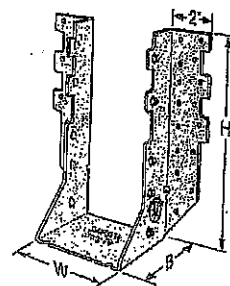
LU26L



HUS210
(HUS26, HUS28, and HHUS similar)



HGUS28-2



HHUS210-2



Double-Shear Nailing Top View

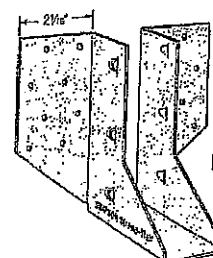
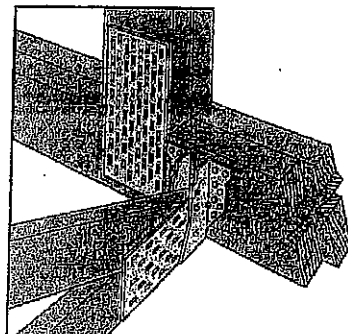


Double-Shear Nailing Side View; Do not bend tab



Double-Shear Nailing Side View (available on some models)
U.S. Patent 5,603,560

Typical HUS26 Installation with Reduced Heel Height (Truss Designer to provide fastener quantity for connecting multiple members together)



LJS26DS

Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS**HHUS/HGUS**

See Hanger Options information on pp. 125-127.

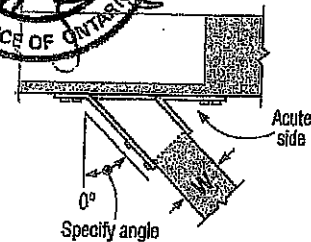
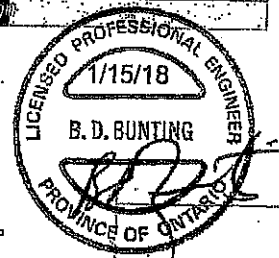
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.82 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _a ⁵	Header	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15) lb. kN	(K _D = 1.00) lb. kN	(K _D = 1.15) lb. kN	(K _D = 1.00) lb. kN
Single 2x Sizes											
LUS24	18	1 1/8	3 1/4	1 1/4	2 1/4	(4) 10d	(2) 10d	710 3.16	1625 7.23	645 2.87	1155 5.14
LU24L	22	1 1/8	3	1 1/4	2 1/8	(4) 10d	(2) 10d x 1 1/2"	360 1.60	1020 4.54	320 1.42	725 3.22
LU26L	22	1 1/8	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"	720 3.20	1605 7.14	545 2.87	1140 5.07
LUS26	18	1 1/8	4 1/4	1 1/4	3 1/4	(4) 10d	(4) 10d	1420 6.32	2170 9.65	1290 5.74	1630 7.25
HUS28	16	1 1/8	5 1/4	3	3 3/8	(14) 16d	(6) 16d	2705 11.90	4940 21.97	2065 9.20	3875 17.24
LJS26DS	18	1 1/8	5	3 1/4	4 1/4	(16) 16d	(6) 16d	2055 9.14	4265 18.97	1460 6.49	4115 18.31
HGUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 16d	(8) 16d	2685 11.96	6625 29.51	2685 11.96	5700 25.35
LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"	1140 5.07	2185 9.72	1020 4.54	1550 6.89
LUS28	18	1 1/8	6 1/4	1 1/4	3 3/4	(6) 10d	(4) 10d	1420 6.32	2520 11.21	1290 5.74	1790 7.96
HUS28	16	1 1/8	7 1/4	3	6 1/8	(22) 16d	(8) 16d	3605 16.04	5365 23.86	2675 11.90	4345 19.33
HGUS28	12	1 1/8	7 1/4	5	6 1/4	(36) 16d	(12) 16d	3310 14.74	7675 34.19	3310 14.74	6900 30.73
LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(6) 10d x 1 1/2"	1140 5.07	2495 11.10	1020 4.54	1770 7.87
LUS210	18	1 1/8	7 1/4	1 1/4	3 3/4	(8) 10d	(4) 10d	1420 6.32	2785 12.39	1290 5.74	2210 9.83

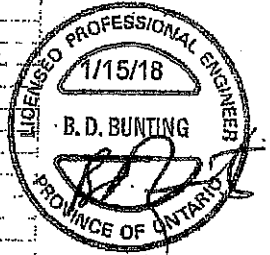
1. Factored uplift resistances have been increased 16% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_a is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin TC-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27-28 for other nail sizes and information.

Face-Mount Hangers

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _b ^a	Header	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
LUS24-2	18	3 1/8	3 1/8	2	1 1/2	(4) 16d	(2) 16d	835	2020	590	1435
								3.71	8.99	2.62	6.38
LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS26-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2850	7335	2065	5205
								12.68	32.63	9.20	23.15
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS28-2	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	4670	9560	4235	7000
								20.77	42.97	18.84	31.14
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
Triple 2x Sizes											
HGUS28-3	12	4 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS26-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d	4670	9670	4235	6865
								20.77	49.02	18.84	30.54
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.29
Quadruple 2x Sizes											
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-4	14	8 1/8	8 1/8	3	7 1/8	(30) 16d	(10) 16d	4670	10155	4235	7210
								20.77	45.17	18.84	32.07
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
LUS46	18	3 1/8	4 1/8	2	3 1/8	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS46	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2540	7335	2065	5205
								11.30	32.63	9.20	23.15
HGUS46	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS48	18	3 1/8	6 1/8	2	3 1/8	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS48	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS48	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS410	18	3 1/8	8 1/8	2	5 1/8	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HGUS410	12	3 1/8	9	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
HGUS412	12	3 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS414	12	3 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80



Plated Truss Connectors

See footnotes on p. 258.

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1 1/4". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

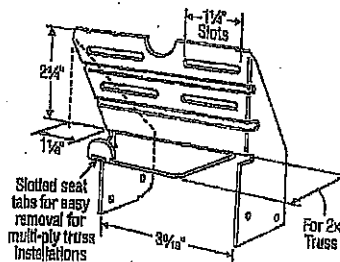
Design: Factored resistances are in accordance with CSA 086-14

Installation:

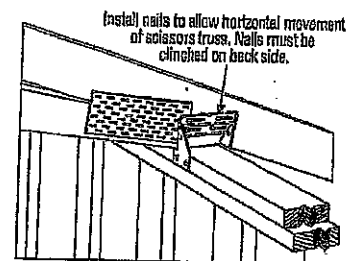
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

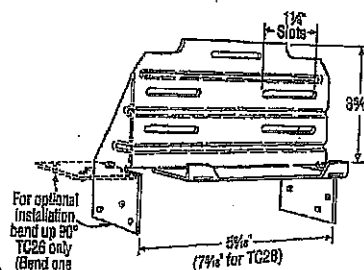
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



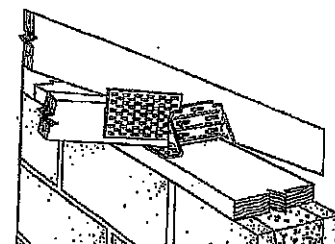
TC24
U.S. Patent 4,932,173



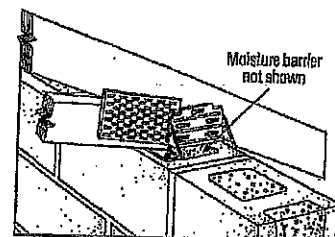
Typical TC24 Installation



TC26
(TC28 Similar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



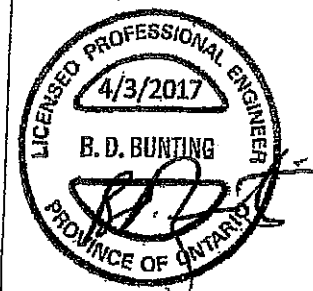
Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K ₀ =1.15)	Uplift (K ₀ =1.15)
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(5) 10d	1015	720
TC28	(5) 10d	(5) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K ₀ =1.15)	Uplift (K ₀ =1.15)
TC26	(5) 10d	(3) 10d x 1 1/2"	810	660
	(5) 10d	(3) 10d	930	680

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (3) - 9/16" x 2 3/4" Titen screws has a factored uplift resistance of 275 lb.



MIT
STATES
DESIGN

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H/TSP

SIMPSON
Strong-Tie

Seismic and Hurricane Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Fasteners			Factored Resistances (K _p = 1.15)					
					D.Fir-L			S-P-F		
		To Rafters/ Truss	To Plates	To Studs	Uplift	Lateral		Uplift	Lateral	
						F ₁	F ₂		F ₁	F ₂
					lb.	lb.	lb.	lb.	lb.	lb.
					kN	kN	kN	kN	kN	kN
H1	18	(6) 8d x 1 1/2"	(4) 8d	—	740	685	300	680	485	215
					3.29	3.05	1.33	3.02	2.16	0.96
SS H2A	18	(5) 8d x 1 1/2"	(2) 8d x 1 1/2"	(5) 8d x 1 1/2"	890	220	75	590	155	55
					3.69	0.98	0.33	2.62	0.69	0.24
SS H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	765	160	160
					3.58	0.71	0.71	3.36	0.71	0.71
H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
					3.71	0.78	0.93	3.29	0.71	0.93
SS H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
					3.29	0.80	1.18	2.74	0.56	0.85
H6	16	—	(8) 8d	(8) 8d	1585	1085	—	1125	770	—
					7.05	4.83	—	5.00	3.43	—
H7Z	16	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
					6.18	2.98	—	4.40	2.11	—
SS H8 ¹	18	(5) 10d x 1 1/2"	(5) 10d x 1 1/2"	—	1120	—	—	1025	—	—
					4.98	—	—	4.56	—	—
SS H10A ²	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1735	795	410	1505	665	290
					7.72	3.54	1.82	6.69	2.51	1.29
H10AR	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1485	690	430	1220	570	305
					6.61	3.07	1.91	5.43	2.54	1.36
H10A-2	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1835	1275	490	1645	880	305
					8.16	5.67	1.91	7.32	3.91	1.36
H10S ^{2,3}	18	(8) 8d x 1 1/2"	(8) 8d x 1 1/2"	(8) 8d	1465	795	315	1040	585	225
					6.52	3.54	1.40	4.63	2.51	1.00
H11Z	18	(6) 16d x 2 1/2"	(6) 16d x 2 1/2"	—	1095	920	545	780	655	390
					4.87	4.09	2.42	3.47	2.91	1.73
H14	18	[1] (12) 8d x 1 1/2"	(13) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
		[2] (12) 8d x 1 1/2"	(15) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
TSP	16	(9) 10d x 1 1/2"	(6) 10d x 1 1/2"	—	1295	440	—	920	310	—
					5.76	1.96	—	4.09	1.38	—
		(9) 10d x 1 1/2"	(6) 10d	—	1560	440	—	1105	310	—
					6.94	1.96	—	4.92	1.38	—

- Factored resistances have been increased 15% for short term loading; no further increase is allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2 1/2" must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
- H8 factored uplift resistances for stud-to-bottom plate installations are 595 lb. (2.65 kN) for D.Fir-L and 390 lb. (1.74 kN) for S-P-F.
- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-HTECONPATH).
- Factored resistances in the F₁ direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.

- H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.38 kN) D.Fir-L and 1015 lb. (4.51 kN) S-P-F.
- H10S nails to plates are optional for uplift but required for lateral loads.
- H10A may be field-bent up to a slope of 6/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
- The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SSNAIL.S at strongtie.com.
- D.Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1 1/2" nails into the top plates and (3) 8d x 1 1/2" nails into the lowest three flange holes into the truss bottom chord is 495 lb. (2.20 kN).
- Nails: 16d x 2 1/2" = 0.162" dia. x 2 1/2" long, 10d = 0.148" dia. x 3" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 8d = 0.131" dia. x 2 1/2" long, 8d x 1 1/2" = 0.131" dia. x 1 1/2" long. See pp. 27-28 for other nail sizes and information.

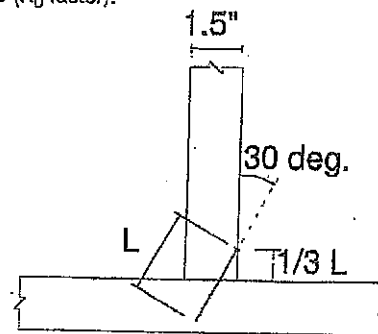
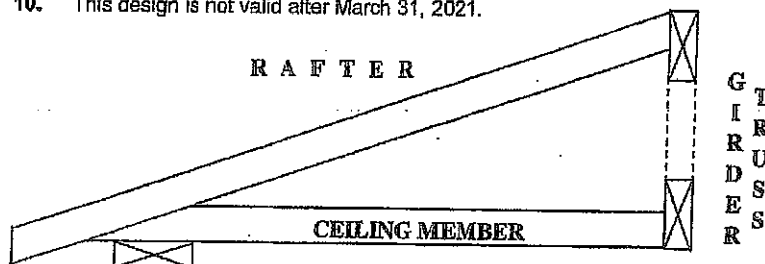
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.



TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

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Bradford, Ontario L3Z 3G7

December 2, 2019



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

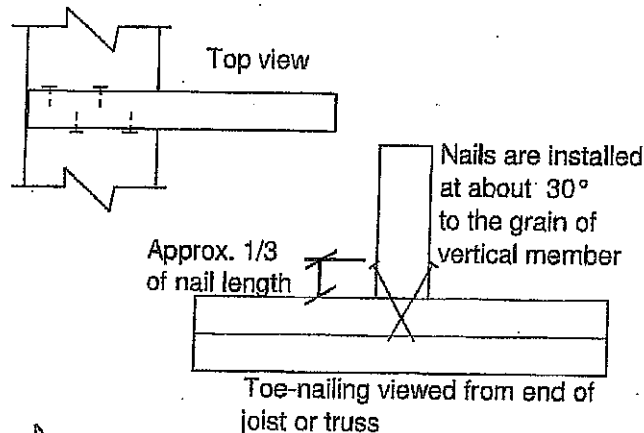
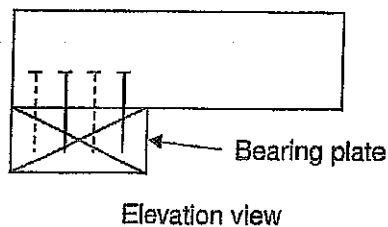
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

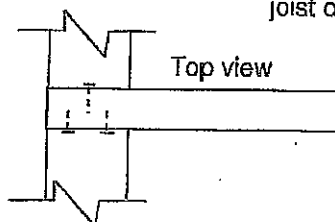
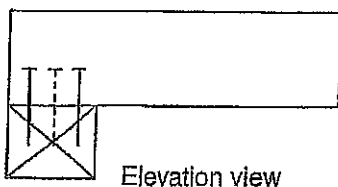
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to wind or earthquake load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate

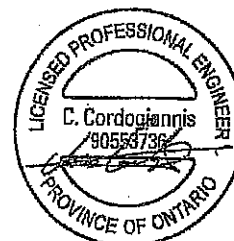


Toe-nailing on 2x4 Bearing Plate



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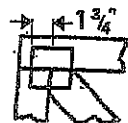
PEO
Certificate No. 10889485



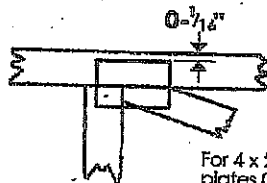
December 2, 2019

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/2" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

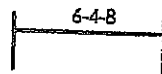


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

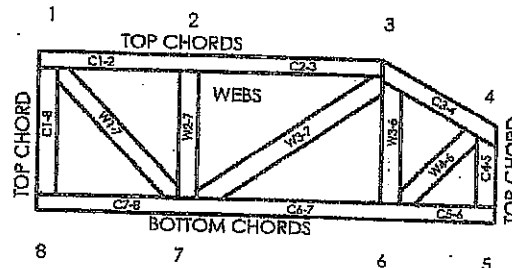
Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

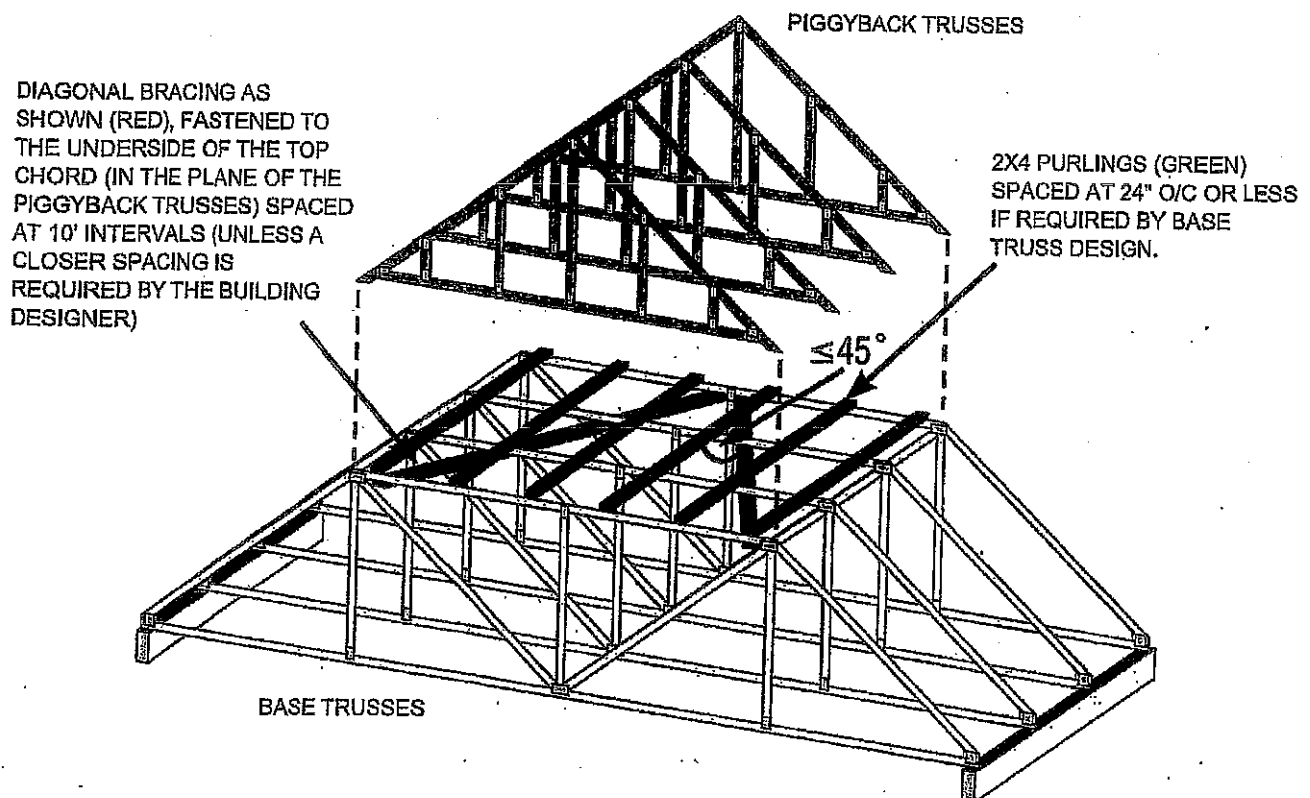
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

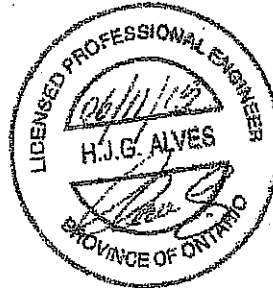


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSCI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800219

Feb 09, 2018