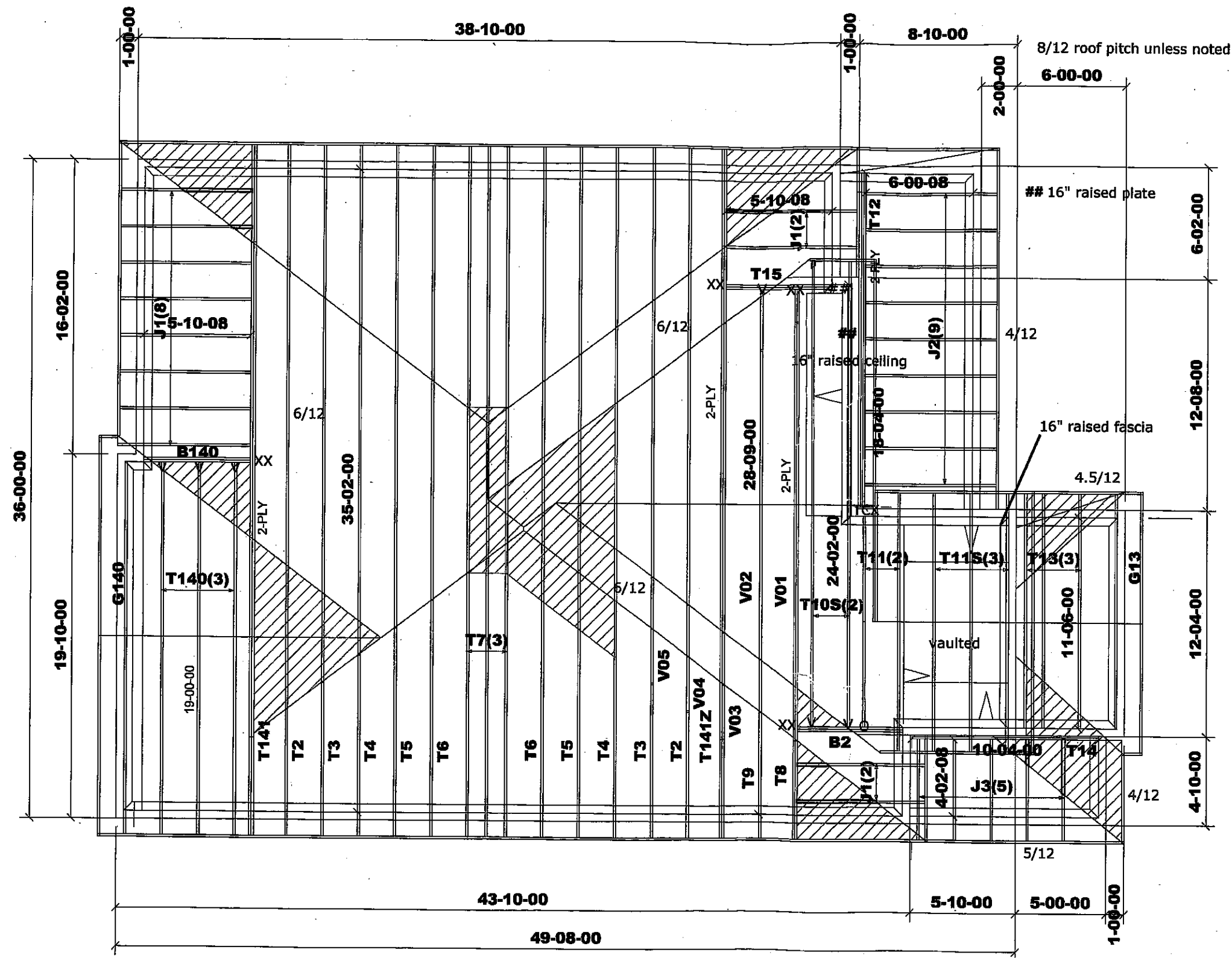




ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH
OBC 2012
(2019 amendment)
OCCUPANCY: RESIDENTIAL
| PART: 9

Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:

TCSL = 25.6 psf

TCDL = 6.0 psf

BCLL = 0.0 psf

BCDL = 7.4 psf

HARDWARE:

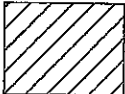
LUS24 - (O)

LJS26DS - (V)

HGUS26-2 - (XX)

BEAMS:

B2, B141 = 2 - 2x10 SPF #2

 DENOTES:
CONVENTIONAL
FRAMING

CITY OF HAMILTON
Building Division
Permit No. 199361
THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE
THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH
THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW
These drawings and/or specifications have been reviewed by
FOR CHIEF BUILDING OFFICIAL
JAN 11 2021
DATE

M13123



Job Track: 51225

Plan Log: 202376

Layout ID: 411918

Builder / Location:

GREEN PARK HOMES / WATERDOWN

Project: RUSSELL GARDENS PH.3

Date: 2020-08-25

Sales: Mario DiCano

Designer: JG

Model / Elevation:

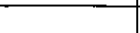
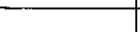
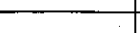
MOUNTAINASH 2-005/EL.1 W/4 BED REAR UPGRADE

Milek ver 8.3.3.247

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

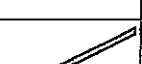
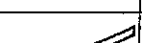
 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST			
	Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: RUSSELL GARDENS PH.3 Location: WATERDOWN Model: MOUNTAINASH 2-005 Lot #: Elevation: EL.1 W/4 BED REAR UPGRADE	Job Track: 51225 PlanLog: 202376 Layout ID: 411918 Ref # Page: 1 of 3 Date: 08-11-2020 Designer: Leo Chen Sales Rep: Mario DiCano		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T2 Hip	8 /12	35-02-00	5-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	279.73 177.00		
	2	T3 Hip	8 /12	35-02-00	6-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	302.74 191.33		
	2	T4 Hip	8 /12	35-02-00	7-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	309.1 195.67		
	2	T5 Hip	8 /12	35-02-00	8-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	316.76 196.67		
	2	T6 Hip	8 /12	35-02-00	9-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	329.25 205.00		
	3	T7 Hip	8 /12	35-02-00	10-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	514.72 323.00		
	1 2-ply	T8 Hip Girder	8 /12	28-09-00	4-01-04	2 x 4 2 x 6	1-03-08	1-04-13 2-10-13	264.12 165.00		
	1	T9 Hip	8 /12	28-09-00	5-01-04	2 x 4	1-03-08	1-04-13 2-10-13	115.01 71.67		
	2	T10S Roof Special	8 /12	24-02-00	10-09-08	2 x 4	1-03-08	2-08-13 1-04-13	243.66 152.67		
	2	T11 Common	8 /12	11-06-00	6-06-13	2 x 4		2-08-13 2-08-13	97.68 63.33		
	3	T11S Roof Special	8 /12 8 /12	11-06-00	6-06-13	2 x 4	1-03-08 1-03-08	2-08-13 2-08-13	170.35 110.00		
	1 2-ply	T12 Flat Girder	0 /12	18-04-00	2-04-02	2 x 6 2 x 4		2-04-02 2-04-02	154.37 94.00		
	3	T13 Common	4.5 /12	11-06-00	2-11-07	2 x 4	1-03-08 1-03-08	4-10 4-10	96.28 59.00		
	1	G13 GABLE	4.5 /12	11-06-00	2-11-07	2 x 4	1-03-08 1-03-08	4-10 4-10	34.1 21.67		

DELIVERY SHIPLIST				
 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track:	51225
	Builder:	GREEN PARK HOMES	PlanLog:	202376
	Project:	RUSSELL GARDENS PH.3	Layout ID:	411918
	Location:	WATERDOWN	Ref #	
	Model:	MOUNTAINASH 2-005	Page:	2 of 3
	Lot #:		Date:	08-11-2020
	Elevation:	EL.1 W/4 BED REAR UPGRADE	Designer:	Leo Chen
			Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T14 Half Hip Girder	4 /12	10-04-00	2-02-06	2 x 4		3-15 2-02-06	37.41 24.17		
	1 2-ply	T15 Flat Girder	0 /12	5-10-08	1-04-00	2 x 4 2 x 6		1-04-00 1-04-00	46.53 29.67		
	3	T140 Common	8 /12	19-00-00	7-08-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	249.57 157.00		
	1	G140 GABLE	8 /12	19-00-00	7-08-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	84.58 54.33		
	1 2-ply	T141 Hip Girder	8 /12	35-02-00	4-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	380.19 234.00		
	1 2-ply	T141Z Hip Girder	8 /12	35-02-00	4-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	380.19 234.00		
	1	V01 Valley	8 /12	19-10-07	6-07-08	2 x 4			58.93 36.33		
	1	V02 Valley	8 /12	16-10-07	5-07-08	2 x 4			48.95 30.17		
	1	V03 Valley	8 /12	22-08-07	6-07-08	2 x 4			67.82 42.33		
	1	V04 Valley	8 /12	19-08-07	5-07-08	2 x 4			57.84 36.17		
	1	V05 Valley	8 /12	16-08-07	4-07-08	2 x 4			46.74 29.67		
	12	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	201.51 128.00		
	9	J2 Jack-Open	4 /12	6-00-08	2-09-00	2 x 4	1-03-08	3-15 2-04-02	142.33 90.00		
	5	J3 Jack-Open	5 /12	4-02-08	2-07-05	2 x 4	1-03-08	5-06 2-02-06	62.65 43.33		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP		DELIVERY SHIPLIST	
		Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: RUSSELL GARDENS PH.3 Location: WATERDOWN Model: MOUNTAINASH 2-005 Lot #: Elevation: EL.1 W/4 BED REAR UPGRADE	Job Track: 51225 PlanLog: 202376 Layout ID: 411918 Ref # Page: 3 of 3 Date: 08-11-2020 Designer: Leo Chen Sales Rep: Mario DiCano

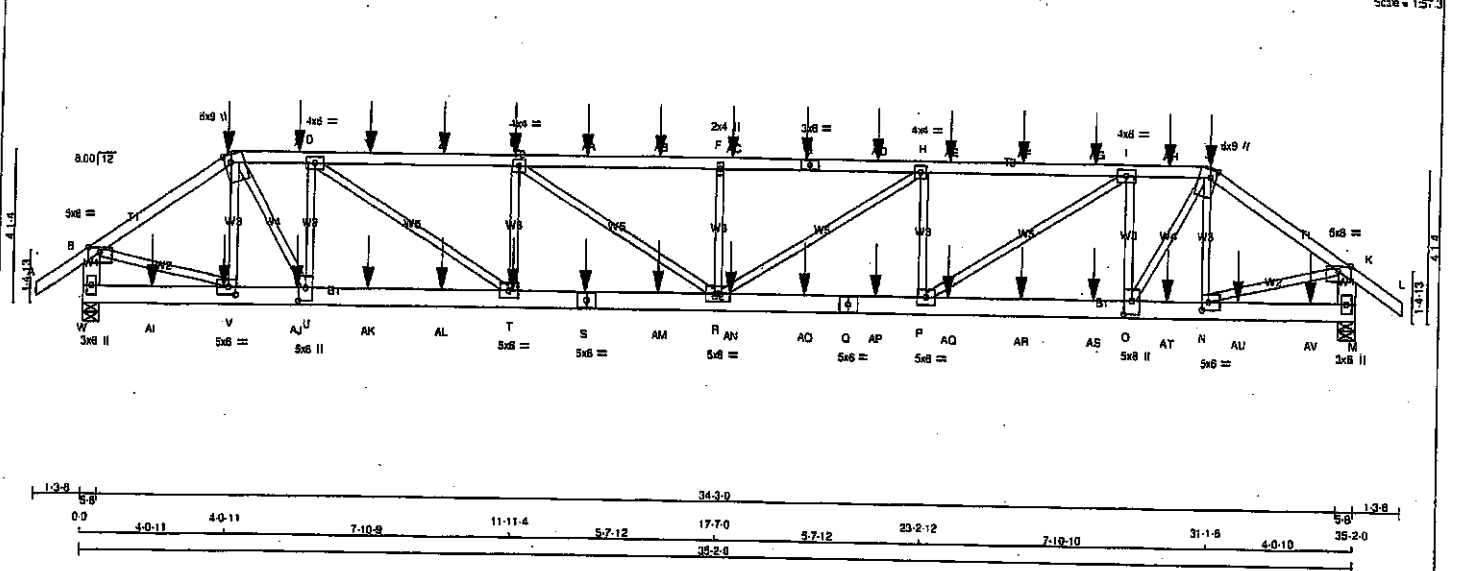
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
TOTAL # TRUSS= 71 TOTAL BFT OF ALL TRUSSES= 3195.18 BFT. TOTAL WEIGHT OF ALL TRSSES 5093.16 LBS											

HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	HGUS26-2	
6	Hardware	LJS26DS	
1	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 11



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - G	2x4	DRY No.2	SPF
G - J	2x4	DRY No.2	SPF
J - L	2x4	DRY No.2	SPF
W - B	2x6	DRY No.2	SPF
M - K	2x6	DRY No.2	SPF
W - S	2x6	DRY No.2	SPF
S - Q	2x6	DRY No.2	SPF
Q - M	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(81.0)
C-G	12	SIDE(81.0)
G-J	12	SIDE(81.0)
J-L	12	SIDE(81.0)
W-B	2	TOP
M-K	2	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
W-S	2	SIDE(183.1)
S-Q	2	SIDE(0.0)
Q-M	2	SIDE(0.0)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT 3393	DOWN 3393	0	5-8
W	3393	0	0	5-8
M	3342	0	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
W	2597	1586 / 0 0 / 0 0 / 0 0 / 0 0 / 0 0 / 0 0
M	2363	1555 / 0 0 / 0 0 / 0 0 / 0 0 / 0 0 / 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	V-C	-703 / 0 0.09 (1)
B-C	-4010 / 0	-91.8	-91.8	0.22 (1)	4.55	C-U	0 / 2685 0.33 (1)
C-X	-4696 / 0	-91.8	-91.8	0.29 (1)	4.16	U-D	-2159 / 0 0.27 (1)
X-D	-4696 / 0	-91.8	-91.8	0.29 (1)	4.16	D-I	-2187 / 0 0.28 (1)
D-Y	-6889 / 0	-91.8	-91.8	0.81 (1)	3.27	O-J	0 / 2713 0.34 (1)
Y-Z	-6889 / 0	-91.8	-91.8	0.81 (1)	3.27	N-J	-681 / 0 0.09 (1)
Z-E	-6889 / 0	-91.8	-91.8	0.81 (1)	3.27	B-V	0 / 3425 0.42 (1)
E-AA	-7711 / 0	-91.8	-91.8	0.81 (1)	3.07	N-K	0 / 3359 0.42 (1)
AA-AB	-7711 / 0	-91.8	-91.8	0.88 (1)	3.07	P-I	0 / 2778 0.34 (1)
AB-F	-7711 / 0	-91.8	-91.8	0.88 (1)	3.07	O-T	0 / 2747 0.34 (1)
F-AC	-7711 / 0	-91.8	-91.8	0.70 (1)	3.04	P-H	-1354 / 0 0.17 (1)
AC-G	-7711 / 0	-91.8	-91.8	0.70 (1)	3.04	T-E	-1337 / 0 0.17 (1)
G-AD	-7711 / 0	-91.8	-91.8	0.70 (1)	3.04	R-H	0 / 893 0.11 (1)
AD-H	-7711 / 0	-91.8	-91.8	0.70 (1)	3.04	E-R	0 / 885 0.11 (1)
H-AE	-6966 / 0	-91.8	-91.8	0.63 (1)	3.24	R-F	-808 / 0 0.10 (1)
AE-AF	-6966 / 0	-91.8	-91.8	0.63 (1)	3.24		
AF-AG	-6966 / 0	-91.8	-91.8	0.63 (1)	3.24		
AG-I	-6966 / 0	-91.8	-91.8	0.63 (1)	3.24		
I-AH	-4647 / 0	-91.8	-91.8	0.30 (1)	4.16		
AH-J	-4647 / 0	-91.8	-91.8	0.30 (1)	4.16		
J-K	-3933 / 0	-91.8	-91.8	0.22 (1)	4.59		
K-L	0 / 35	-91.8	-91.8	0.07 (1)	10.00		
W-B	-3343 / 0	0.0	0.0	0.12 (1)	7.61		
M-K	-3285 / 0	0.0	0.0	0.12 (1)	7.66		

W-AI	0 / 0	-18.5	-18.5	0.04 (4)	10.00
AI-V	0 / 0	-18.5	-18.5	0.04 (4)	10.00
V-AJ	0 / 3311	-18.5	-18.5	0.22 (1)	10.00
AJ-U	0 / 3311	-18.5	-18.5	0.22 (1)	10.00
U-AK	0 / 4697	-18.5	-18.5	0.35 (1)	10.00
AK-AL	0 / 4697	-18.5	-18.5	0.35 (1)	10.00
AL-T	0 / 4697	-18.5	-18.5	0.35 (1)	10.00
T-S	0 / 6989	-18.5	-18.5	0.52 (1)	10.00
S-AM	0 / 6989	-18.5	-18.5	0.52 (1)	10.00
AM-R	0 / 6989	-18.5	-18.5	0.52 (1)	10.00
R-AN	0 / 6986	-18.5	-18.5	0.52 (1)	10.00
AN-AO	0 / 6966	-18.5	-18.5	0.52 (1)	10.00
AO-Q	0 / 6966	-18.5	-18.5	0.52 (1)	10.00
Q-AP	0 / 6966	-18.5	-18.5	0.52 (1)	10.00
AP-P	0 / 6966	-18.5	-18.5	0.52 (1)	10.00
P-AQ	0 / 4648	-18.5	-18.5	0.35 (1)	10.00
AQ-AR	0 / 4648	-18.5	-18.5	0.35 (1)	10.00
AR-AS	0 / 4648	-18.5	-18.5	0.35 (1)	10.00

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = 1/360 (1.17")
CALCULATED VERT. DEF. (LL) = 1/999 (0.24")
ALLOWABLE DEF. (TL) = 1/360 (1.17")
CALCULATED VERT. DEF. (TL) = 1/982 (0.44")
CSI: TC=0.70/1.00 (F-H:1), BC=0.52/1.00 (P-R:1), WB=0.42/1.00 (B-V:1), SS=0.19/1.00 (H:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1667 788 1987 1856	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G); INPUT = 0.90;
JSI METAL= 0.67 (S); INPUT = 1.00;



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408087	T1	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mittek Industries, Inc. Sat Apr 25 08:53:14 2020 Page 2

ID:DMCubINVR6TslFoe31v6l zns11-b3W1:MNR57wh?ZdGV0c31kiYG5mqx6Jz 4T3 zNEh3

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge
C	TTWW-m	MT20	6.0	9.0	Edge 2.00
D	TMVW-t	MT20	4.0	6.0	
E	TMVW-t	MT20	4.0	4.0	
F	TMVW-w	MT20	2.0	4.0	
G	TS-t	MT20	3.0	6.0	
H	TMVW-t	MT20	4.0	4.0	
I	TMVW-t	MT20	4.0	6.0	
J	TTWW-m	MT20	6.0	9.0	Edge 2.00
K	TMVW-p	MT20	5.0	8.0	Edge
M	BMV1-p	MT20	3.0	6.0	
N	BMVW-t	MT20	5.0	6.0	2.50 2.25
O	BMVW-t	MT20	5.0	6.0	4.25 2.50
P	BMVW-t	MT20	5.0	6.0	
Q	BS-t	MT20	5.0	6.0	
R	BMVW-t	MT20	5.0	8.0	
S	BS-t	MT20	5.0	6.0	
T	BMVW-t	MT20	5.0	6.0	
U	BMVW-t	MT20	5.0	8.0	4.25 2.50
V	BMVW-t	MT20	5.0	6.0	2.50 2.25
W	BMV1-p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: (4)

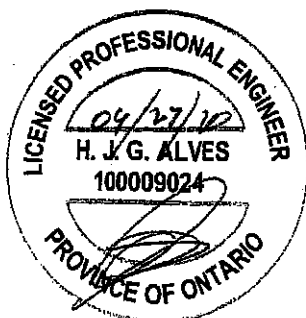
CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. MEMB. FORCE (LBS)	MAX. FACTORED CS1 (LC)	UNBRAC LENGTH	FR-TO
AS-O	0 / 4648	-18.5	-18.5	0.35 (1)	10.00		
O-AT	0 / 3248	-18.5	-18.5	0.22 (1)	10.00		
AT-N	0 / 3248	-18.5	-18.5	0.22 (1)	10.00		
N-AU	0 / 0	-18.5	-18.5	0.03 (4)	10.00		
AU-AV	0 / 0	-18.5	-18.5	0.03 (4)	10.00		
AV-M	0 / 0	-18.5	-18.5	0.03 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-49	-55		FRONT	VERT	DEAD		C1
C	4-0-11	-117	-117		BACK	VERT	TOTAL		C1
C	4-0-11	-254	-254		FRONT	VERT	SNOW		C1
E	11-11-4	-110	-110		BACK	VERT	TOTAL		C1
G	19-11-4	-110	-110		BACK	VERT	TOTAL		C1
J	31-1-6	-49	-55		FRONT	VERT	DEAD		C1
J	31-1-6	-254	-254		FRONT	VERT	SNOW		C1
S	13-11-4	-26	-26		BACK	VERT	TOTAL		C1
T	11-11-4	-26	-26		BACK	VERT	TOTAL		C1
V	3-11-4	-26	-26		BACK	VERT	TOTAL		C1
X	5-11-4	-110	-110		BACK	VERT	TOTAL		C1
Y	7-11-4	-110	-110		BACK	VERT	TOTAL		C1
Z	9-11-4	-110	-110		BACK	VERT	TOTAL		C1
AA	13-11-4	-110	-110		BACK	VERT	TOTAL		C1
AB	15-11-4	-110	-110		BACK	VERT	TOTAL		C1
AC	17-11-4	-110	-110		BACK	VERT	TOTAL		C1
AD	21-11-4	-110	-110		BACK	VERT	TOTAL		C1
AE	23-11-4	-110	-110		BACK	VERT	TOTAL		C1
AF	25-11-4	-110	-110		BACK	VERT	TOTAL		C1
AG	27-11-4	-110	-110		BACK	VERT	TOTAL		C1
AH	29-11-4	-110	-110		BACK	VERT	TOTAL		C1
AI	1-11-4	-26	-26		BACK	VERT	TOTAL		C1
AJ	5-11-4	-26	-26		BACK	VERT	TOTAL		C1
AK	7-11-4	-26	-26		BACK	VERT	TOTAL		C1
AL	9-11-4	-26	-26		BACK	VERT	TOTAL		C1
AM	13-11-4	-26	-26		BACK	VERT	TOTAL		C1
AN	17-11-4	-26	-26		BACK	VERT	TOTAL		C1
AO	19-11-4	-26	-26		BACK	VERT	TOTAL		C1
AP	21-11-4	-26	-26		BACK	VERT	TOTAL		C1
AQ	23-11-4	-26	-26		BACK	VERT	TOTAL		C1
AR	25-11-4	-26	-26		BACK	VERT	TOTAL		C1
AS	27-11-4	-26	-26		BACK	VERT	TOTAL		C1
AT	29-11-4	-26	-26		BACK	VERT	TOTAL		C1
AU	31-11-4	-26	-26		BACK	VERT	TOTAL		C1
AV	33-11-4	-26	-26		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2006952

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
<u>BEARINGS</u>									
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG			
	VERT	HORZ	DOWN	HORZ			UPLIFT	IN-SX	IN-SX
JT	2536	0	2536	0	0	5-8	5-8		
M	4405	0	4405	0	0	5-8	5-8		
<u>UNFACTORED REACTIONS</u>									
	1ST LCASE		MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
JT	1791	1198 / 0	0 / 0	0 / 0	0 / 0	803 / 0	0 / 0		
M	3114	2063 / 0	0 / 0	0 / 0	0 / 0	1663 / 0	0 / 0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
<u>BEARINGS</u>									
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG			
	VERT	HORZ	DOWN	HORZ			UPLIFT	IN-SX	IN-SX
JT	2536	0	2536	0	0	5-8	5-8		
M	4405	0	4405	0	0	5-8	5-8		
<u>UNFACTORED REACTIONS</u>									
	1ST L CASE		MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
JT	1791	1198 / 0	0 / 0	0 / 0	0 / 0	803 / 0	0 / 0		
M	3114	2063 / 0	0 / 0	0 / 0	0 / 0	1663 / 0	0 / 0		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CSI: TC=0.50/1.00 (H:1), BC=0.51/1.00 (P-R:1),
WB=0.57/1.00 (K-N:1), SS=0.12/1.00 (H:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVEHEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
J	31-1-6	-49	-55	---	FRONT	VERT	DEAD	---	C1
J	31-1-6	-118	-118	---	FRONT	VERT	TOTAL	---	C1
J	31-1-6	-254	-254	---	FRONT	VERT	SNOW	---	C1
N	31-0-12	-26	-28	---	FRONT	VERT	TOTAL	---	C1
O	28-10-8	-2340	-2340	---	FRONT	VERT	TOTAL	---	C1
X	33-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	818 354	1667 786	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (N) ; INPUT = 0.90 ;
JSI METAL = 0.85 (S) ; INPUT = 1.00 ;

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408087	T1Z	1	2	GREEN PARK HOMES	

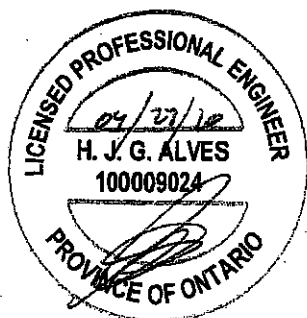
Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Sat Apr 25 08:53:15 2020 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	2.00
D	TMWW-l	MT20	4.0	6.0		
E	TMWW-l	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMWW-l	MT20	4.0	6.0		
J	TTWW+m	MT20	6.0	9.0	Edge	2.00
K	TMVW-p	MT20	5.0	8.0	Edge	
M	BMV1+p	MT20	3.0	5.0		
N	BMWW-l	MT20	5.0	6.0	2.50	2.00
O	BMWW-l	MT20	5.0	8.0	4.25	2.50
P	BMWW-l	MT20	5.0	6.0		
Q	SS-t	MT20	5.0	6.0		
R	BMWWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-l	MT20	5.0	6.0		
U	BMWW-l	MT20	5.0	8.0	4.25	2.50
V	BMWW-l	MT20	5.0	6.0	2.50	2.00
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

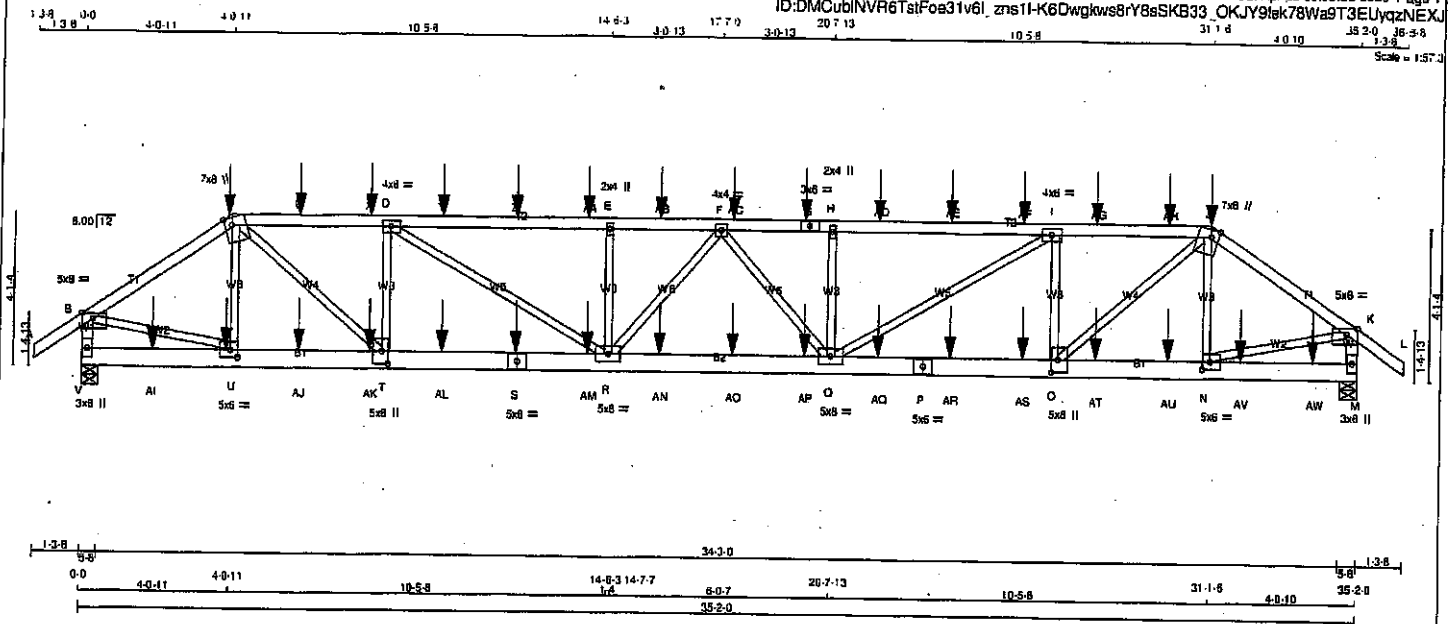


Structural component only
DWG# T-2006953 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408088	T100	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
V - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
V - S	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(61.0)
C-G	12	SIDE(61.0)
G-J	12	SIDE(61.0)
J-L	12	SIDE(61.0)
V-B	12	TOP
M-K	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
V-S	12	SIDE(183.1)
S-P	12	SIDE(0.0)
P-M	12	SIDE(0.0)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
V	3393	0	3393	0
M	3342	0	3342	0

UNFACTORED REACTIONS

	1ST LOASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
V	2397	1585 / 0	0 / 0	0 / 0	0 / 0	812 / 0	0 / 0
M	2363	1555 / 0	0 / 0	0 / 0	0 / 0	808 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO	
A-B	0 / 35	-91.8	-91.8 0.07 (1)	U-C	-658 / 0
B-C	-4013 / 0	-91.8	-91.8 0.22 (1)	C-T	0 / 3210
C-W	-5748 / 0	-91.8	-91.8 0.49 (1)	T-D	-1922 / 0
W-X	-5748 / 0	-91.8	-91.8 0.49 (1)	C-I	-1945 / 0
X-D	-5748 / 0	-91.8	-91.8 0.49 (1)	C-J	0 / 3243
D-Y	-7499 / 0	-91.8	-91.8 0.76 (1)	N-J	-639 / 0
Y-Z	-7499 / 0	-91.8	-91.8 0.76 (1)	B-U	0 / 3426
Z-AA	-7499 / 0	-91.8	-91.8 0.76 (1)	N-K	0 / 3381
AA-E	-7499 / 0	-91.8	-91.8 0.76 (1)	Q-I	0 / 2078
E-AB	-7499 / 0	-91.8	-91.8 0.48 (1)	D-R	0 / 2048
AB-F	-7499 / 0	-91.8	-91.8 0.48 (1)	Q-H	-770 / 0
F-AC	-7499 / 0	-91.8	-91.8 0.48 (1)	R-E	-769 / 0
AC-G	-7499 / 0	-91.8	-91.8 0.48 (1)	R-F	-141 / 0
G-H	-7499 / 0	-91.8	-91.8 0.48 (1)	F-Q	-161 / 0
H-AD	-7499 / 0	-91.8	-91.8 0.77 (1)		
AD-AE	-7499 / 0	-91.8	-91.8 0.77 (1)		
AE-AF	-7499 / 0	-91.8	-91.8 0.77 (1)		
AF-I	-7499 / 0	-91.8	-91.8 0.77 (1)		
I-AG	-5710 / 0	-91.8	-91.8 0.50 (1)		
AG-AH	-5710 / 0	-91.8	-91.8 0.50 (1)		
AH-J	-5710 / 0	-91.8	-91.8 0.50 (1)		
J-K	-3935 / 0	-91.8	-91.8 0.22 (1)		
K-L	0 / 35	-91.8	-91.8 0.07 (1)		
V-B	-3345 / 0	0.0	0.0 0.19 (1)		
M-K	-3286 / 0	0.0	0.0 0.18 (1)		
V-AI	0 / 0	-18.5	-18.5 0.03 (4)		
AI-U	0 / 0	-18.5	-18.5 0.03 (4)		
U-AJ	0 / 3315	-18.5	-18.5 0.24 (1)		
AJ-AK	0 / 3315	-18.5	-18.5 0.24 (1)		
AK-T	0 / 3315	-18.5	-18.5 0.24 (1)		
T-AL	0 / 5749	-18.5	-18.5 0.44 (1)		
AL-S	0 / 5749	-18.5	-18.5 0.44 (1)		
S-AM	0 / 5749	-18.5	-18.5 0.44 (1)		
AM-R	0 / 5749	-18.5	-18.5 0.44 (1)		
R-AN	0 / 7589	-18.5	-18.5 0.57 (1)		
AN-AO	0 / 7589	-18.5	-18.5 0.57 (1)		
AO-AP	0 / 7589	-18.5	-18.5 0.57 (1)		
AP-Q	0 / 7589	-18.5	-18.5 0.57 (1)		
Q-AQ	0 / 5710	-18.5	-18.5 0.43 (1)		
AQ-P	0 / 5710	-18.5	-18.5 0.43 (1)		
P-AR	0 / 5710	-18.5	-18.5 0.43 (1)		
AR-AS	0 / 5710	-18.5	-18.5 0.43 (1)		

TOTAL WEIGHT = 2 X 160 = 321 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/989 (0.23")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/984 (0.44")

CSI: TC=0.77/1.00 (H-I), BC=0.57/1.00 (Q-R), WB=0.42/1.00 (B-U), SS=0.20/1.00 (H-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	518 354	1687 789 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.73 (G); INPUT = 0.90 ;
JSI METAL = 0.57 (S); INPUT = 1.00 ;



Structural component only
DWG# T-2006982 1/2

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TTWW-m	MT20	7.0	8.0	Edge	2.50
D	TMVW-t	MT20	4.0	6.0		
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-w	MT20	2.0	4.0		
I	TMVW-t	MT20	4.0	6.0		
J	TTWW-m	MT20	7.0	8.0	Edge	2.50
K	TMVW-p	MT20	5.0	8.0	Edge	3.50
M	BMV1-p	MT20	3.0	6.0		
N	BMVW-t	MT20	5.0	8.0	2.50	2.50
O	BMVW-t	MT20	5.0	8.0	4.00	2.00
P	BS-t	MT20	5.0	8.0		
Q	BMVW-w	MT20	5.0	8.0		
R	BS-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	8.0		
T	BMVW-t	MT20	5.0	8.0	4.00	2.00
U	BMVW-t	MT20	5.0	8.0	2.50	2.50
V	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CS1 (LC)	UNBRAC	MEMB. MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM	TO		LENGTH	FR-TO	
AS-O	0 / 5710	-18.5	-18.5	0.43 (1)	10.00		
O-AT	0 / 3251	-18.5	-18.5	0.23 (1)	10.00		
AT-AU	0 / 3251	-18.5	-18.5	0.23 (1)	10.00		
AU-N	0 / 3251	-18.5	-18.5	0.23 (1)	10.00		
N-AV	0 / 0	-18.5	-18.5	0.03 (4)	10.00		
AV-AW	0 / 0	-18.5	-18.5	0.03 (4)	10.00		
AW-M	0 / 0	-18.5	-18.5	0.03 (4)	10.00		

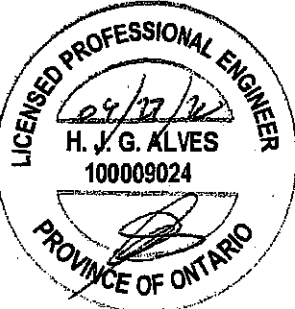
WEBS

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-49	-55		FRONT	VERT	DEAD		C1
C	4-0-11	-117	-117		BACK	VERT	TOTAL		C1
C	4-0-11	-254	-254		FRONT	VERT	SNOW		C1
G	19-11-4	-110	-110		BACK	VERT	TOTAL		C1
J	31-1-6	-49	-55		FRONT	VERT	DEAD		C1
J	31-1-6	-254	-254		FRONT	VERT	SNOW		C1
S	11-11-4	-26	-26		BACK	VERT	TOTAL		C1
U	3-11-4	-26	-26		BACK	VERT	TOTAL		C1
W	5-11-4	-110	-110		BACK	VERT	TOTAL		C1
X	7-11-4	-110	-110		BACK	VERT	TOTAL		C1
Y	9-11-4	-110	-110		BACK	VERT	TOTAL		C1
Z	11-11-4	-110	-110		BACK	VERT	TOTAL		C1
AA	13-11-4	-110	-110		BACK	VERT	TOTAL		C1
AB	15-11-4	-110	-110		BACK	VERT	TOTAL		C1
AC	17-11-4	-110	-110		BACK	VERT	TOTAL		C1
AD	21-11-4	-110	-110		BACK	VERT	TOTAL		C1
AE	23-11-4	-110	-110		BACK	VERT	TOTAL		C1
AF	25-11-4	-110	-110		BACK	VERT	TOTAL		C1
AG	27-11-4	-110	-110		BACK	VERT	TOTAL		C1
AH	29-11-4	-110	-110		BACK	VERT	TOTAL		C1
AI	1-11-4	-26	-26		BACK	VERT	TOTAL		C1
AJ	5-11-4	-26	-26		BACK	VERT	TOTAL		C1
AK	7-11-4	-26	-26		BACK	VERT	TOTAL		C1
AL	9-11-4	-26	-26		BACK	VERT	TOTAL		C1
AM	13-11-4	-26	-26		BACK	VERT	TOTAL		C1
AN	15-11-4	-26	-26		BACK	VERT	TOTAL		C1
AO	17-11-4	-26	-26		BACK	VERT	TOTAL		C1
AP	19-11-4	-26	-26		BACK	VERT	TOTAL		C1
AQ	21-11-4	-26	-26		BACK	VERT	TOTAL		C1
AR	23-11-4	-26	-26		BACK	VERT	TOTAL		C1
AS	25-11-4	-26	-26		BACK	VERT	TOTAL		C1
AT	27-11-4	-26	-26		BACK	VERT	TOTAL		C1
AU	29-11-4	-26	-26		BACK	VERT	TOTAL		C1
AV	31-11-4	-26	-26		BACK	VERT	TOTAL		C1
AW	33-11-4	-26	-26		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



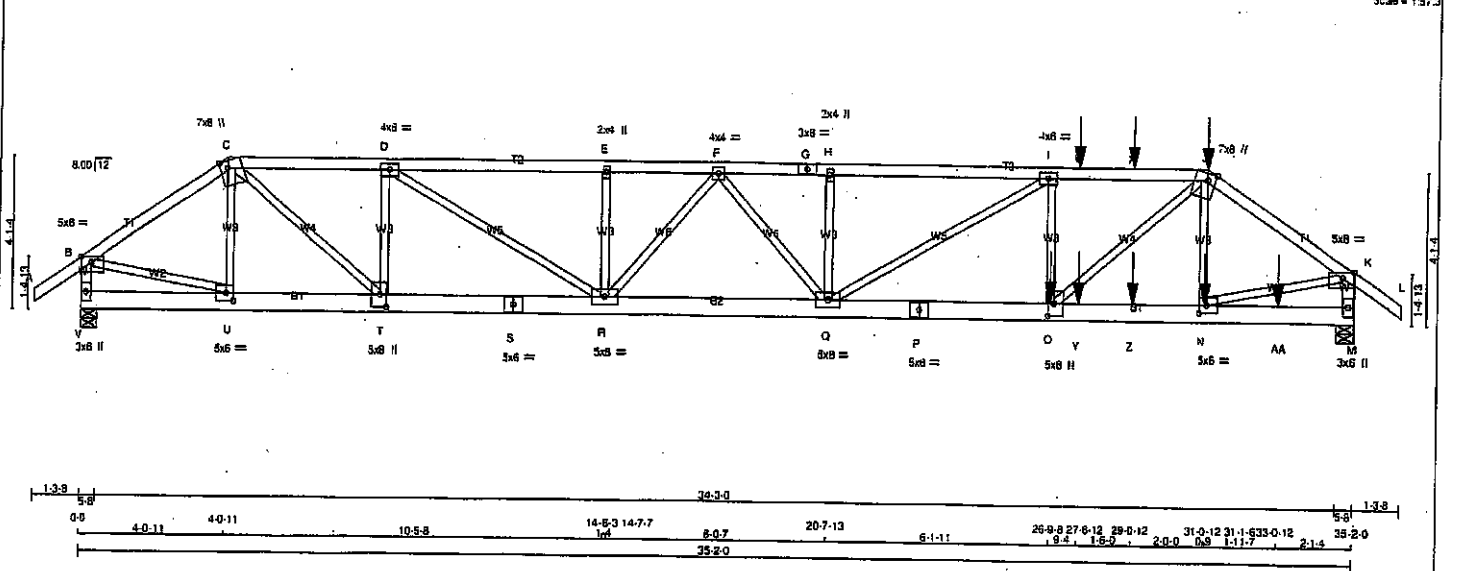
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408088	T100Z	1	2	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstF0e31v6l_znstt-0nlt3xUv9g?UcvOcnVdsX5MC14NtwUjh_1UGzNEXI

Scale = 1/8" = 1'-0"



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	NUMBER
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
V - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
V - S	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
C-G	12	TOP
G-J	12	TOP
J-L	12	TOP
V-B	12	TOP
M-K	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
V-S	12	TOP
S-P	12	TOP
P-M	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
I-O	5	SIDE(183.1)
D-T	5	SIDE(352.8)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
V	2660	0	2660	0	0
M	4289	0	4289	0	0

UNFACTORED REACTIONS					
	1ST CASE	MAX. MIN.	COMPONENT REACTIONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
V	1879	1243 / 0	0 / 0	0 / 0	0 / 0
M	3016	1988 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-91.8 -91.8	0.07 (1)	U-C	-525 / 0	0.07 (1)	
B-C	-3082 / 0	-91.8 -91.8	0.19 (1)	C-T	0 / 2900	0.36 (1)	
C-D	-4727 / 0	-91.8 -91.8	0.31 (1)	T-D	-1780 / 0	0.23 (1)	
D-E	-6774 / 0	-91.8 -91.8	0.53 (1)	O-I	-767 / 0	0.10 (1)	
E-F	-6774 / 0	-91.8 -91.8	0.32 (1)	C-J	0 / 4417	0.55 (1)	
F-G	-7769 / 0	-91.8 -91.8	0.37 (1)	N-J	-832 / 0	0.11 (1)	
G-H	-7769 / 0	-91.8 -91.8	0.37 (1)	B-U	0 / 2615	0.32 (1)	
H-I	-7633 / 0	-91.8 -91.8	0.56 (1)	N-K	0 / 4430	0.55 (1)	
I-W	-7633 / 0	-91.8 -91.8	0.51 (1)	Q-I	0 / 159	0.02 (1)	
W-X	-7633 / 0	-91.8 -91.8	0.51 (1)	D-R	0 / 2395	0.30 (1)	
X-J	-7633 / 0	-91.8 -91.8	0.51 (1)	Q-H	-461 / 0	0.08 (1)	
J-K	-5186 / 0	-91.8 -91.8	0.07 (1)	R-E	-472 / 0	0.08 (1)	
K-L	0 / 35	-91.8 -91.8	0.07 (1)	R-F	-887 / 0	0.17 (1)	
V-B	-2628 / 0	0.0 0.0	0.15 (1)	F-Q	0 / 678	0.08 (1)	
M-K	-4232 / 0	0.0 0.0	0.24 (1)				
V-U	0 / 0	-18.5 -18.5	0.02 (4)				
U-T	0 / 2529	-18.5 -18.5	0.17 (1)				
T-S	0 / 4727	-18.5 -18.5	0.36 (1)				
S-R	0 / 4727	-18.5 -18.5	0.36 (1)				
R-Q	0 / 7338	-18.5 -18.5	0.53 (1)				
Q-P	0 / 7633	-18.5 -18.5	0.57 (1)				
P-O	0 / 7633	-18.5 -18.5	0.57 (1)				
O-Y	0 / 4284	-18.5 -18.5	0.34 (1)				
Y-Z	0 / 4284	-18.5 -18.5	0.34 (1)				
Z-N	0 / 4284	-18.5 -18.5	0.34 (1)				
N-AA	0 / 0	-18.5 -18.5	0.03 (4)				
AA-M	0 / 0	-18.5 -18.5	0.03 (4)				

FACTORED CONCENTRATED LOADS (LBS)					
JT	LOC.	LC1	MAX.	FACE	DIR.
J	31-1-8	-49	-55	FRONT	VERT
J	31-1-8	-118	-118	FRONT	VERT
N	31-1-8	-254	-254	FRONT	VERT
N	31-0-12	-26	-26	FRONT	VERT
O	26-9-8	-2054	-2054	FRONT	VERT
W	27-6-12	-110	-110	FRONT	VERT
X	29-0-12	-110	-110	FRONT	VERT
Y	27-6-12	-26	-26	FRONT	VERT
Z	29-0-12	-26	-26	FRONT	VERT
AA	33-0-12	-26	-26	FRONT	VERT

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, NBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/960 (1.17")
CALCULATED VERT. DEFL.(LL) = L/959 (0.23")
ALLOWABLE DEFL.(TL) = L/960 (1.17")
CALCULATED VERT. DEFL.(TL) = L/992 (0.43")

CSI: TC=0.66/1.00 (H-I-1), BC=0.57/1.00 (O-Q-1),
WB=0.55/1.00 (K-N-1), SS=0.18/1.00 (J-I-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

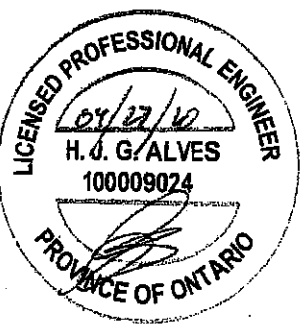
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PLI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) / INPUT = 0.90 }
JSI METAL= 0.72 (P) / INPUT = 1.00 }



Structural component only
DWG# T-2006983 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408088	T100Z	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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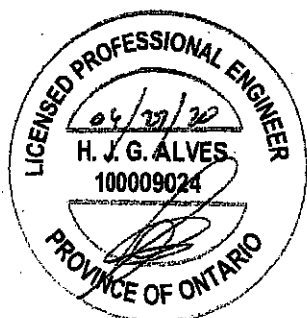
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TTWW+m	MT20	7.0	8.0	Edge	2.50
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.50
K	TMVW-p	MT20	5.0	8.0	Edge	3.50
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	8.0	2.50	2.50
O	BMWW+t	MT20	5.0	8.0	4.00	2.00
P	BS-t	MT20	5.0	6.0		
R	BMWWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW+t	MT20	5.0	8.0	4.00	2.00
U	BMWW-t	MT20	5.0	8.0	2.50	2.50
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

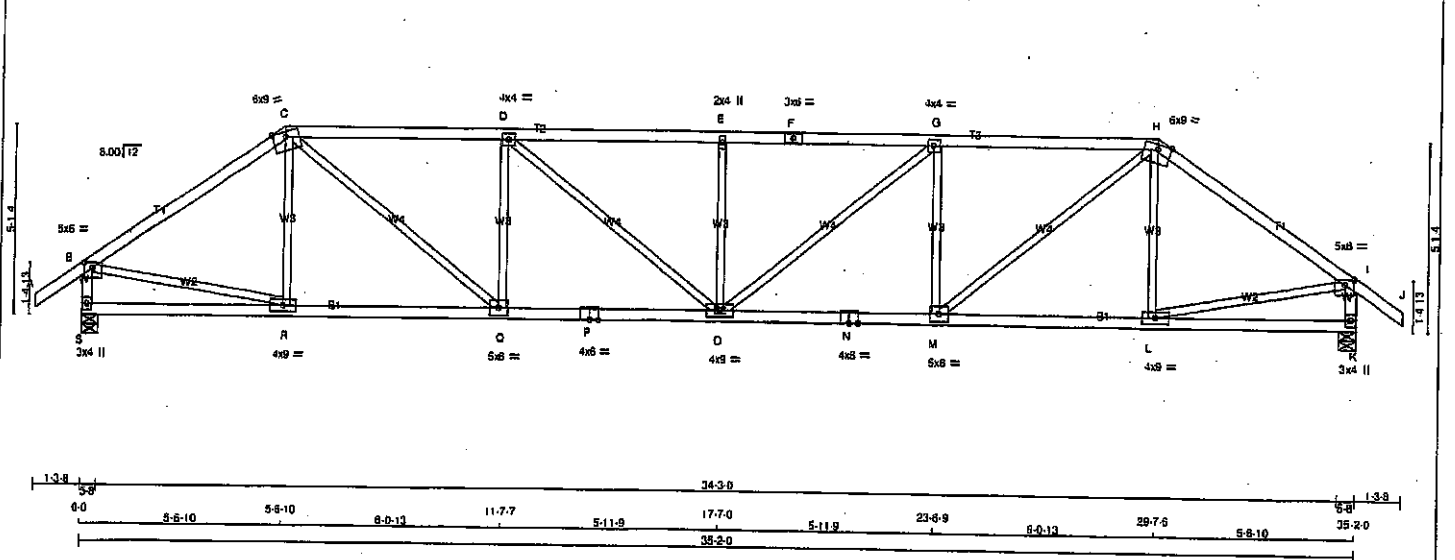
CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2006983 2/2

JOB NAME 408087	TRUSS NAME T2	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 08:33:16 2020 Page 1	
				ID:DMCubINVR6TstFoe31v6I_zns1-XSdn72OidkAPesmfRqX69orNvQSPCocRIZZ7tNEh1	
Scale = 1:57.3					



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	6.0	1.75	2.75
C TTWW-m	MT20	6.0	9.0	Edge	
D TMVW-l	MT20	4.0	4.0		
E TMVW-w	MT20	2.0	4.0		
F TS-l	MT20	3.0	6.0		
G TMVW-l	MT20	4.0	4.0		
H TTWW-m	MT20	6.0	9.0	Edge	
I TMVW-p	MT20	5.0	6.0	1.75	2.75
L BMV1-p	MT20	3.0	4.0		
M BMVW-l	MT20	4.0	9.0		
N BS-l	MT20	4.0	6.0		
O BMVW-w	MT20	4.0	9.0		
P BS-l	MT20	4.0	6.0		
Q BMVW-l	MT20	5.0	6.0		
R BMVW-l	MT20	4.0	9.0		
S BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	2065	0	0	5-8
S VERT	2065	0	0	5-8
K VERT	2065	0	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT COMBINED	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
S COMBINED	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
K COMBINED	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-91.8 -91.8	0.12 (1)	10.00	R-C	-243 / 10	0.09 (1)
B-C	-2297 / 0	-91.8 -91.8	0.70 (1)	3.78	C-Q	0 / 1670	0.38 (1)
C-D	-3214 / 0	-91.8 -91.8	0.80 (1)	3.18	Q-D	-923 / 0	0.36 (1)
D-E	-3595 / 0	-91.8 -91.8	0.98 (1)	2.97	D-O	0 / 490	0.11 (1)
E-F	-3695 / 0	-91.8 -91.8	0.88 (1)	2.97	O-E	-505 / 0	0.19 (1)
F-G	-3595 / 0	-91.8 -91.8	0.88 (1)	2.97	O-G	0 / 490	0.11 (1)
G-H	-3214 / 0	-91.8 -91.8	0.80 (1)	3.18	M-G	-929 / 0	0.35 (1)
H-I	-2297 / 0	-91.8 -91.8	0.70 (1)	3.78	M-H	0 / 1670	0.38 (1)
I-J	0 / 35	-91.8 -91.8	0.12 (1)	10.00	L-H	-243 / 10	0.09 (1)
S-B	-2023 / 0	0.0 0.0	0.21 (1)	5.94	B-R	0 / 1943	0.44 (1)
K-I	-2023 / 0	0.0 0.0	0.21 (1)	5.94	L-I	0 / 1943	0.44 (1)
S-R	0 / 0	-18.5 -18.5	0.15 (4)	10.00			
R-Q	0 / 1904	-18.5 -18.5	0.39 (1)	10.00			
Q-P	0 / 3214	-18.5 -18.5	0.58 (1)	10.00			
P-O	0 / 3214	-18.5 -18.5	0.58 (1)	10.00			
O-N	0 / 3214	-18.5 -18.5	0.58 (1)	10.00			
N-M	0 / 3214	-18.5 -18.5	0.58 (1)	10.00			
M-L	0 / 1904	-18.5 -18.5	0.39 (1)	10.00			
L-K	0 / 0	-18.5 -18.5	0.15 (4)	10.00			

TOTAL WEIGHT = 2 X 140 = 280 LB

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPC 2011, TPC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/380 (1.17")
CALCULATED VERT. DEFL. (LL) = L/999 (0.21")
ALLOWABLE DEFL. (TL) = L/380 (1.17")
CALCULATED VERT. DEFL. (TL) = L/999 (0.39")

CSI: TC=0.88/1.00 (E-G:1), BC=0.58/1.00 (O-Q:1),
WB=0.44/1.00 (B-R:1), SS=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION	(PSI)	(PLI)	(PLI)
MAX MIN MAX MIN MAX MIN			
MT20	618	354	1667 789 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (B) (INPUT = 0.90)
JSI METAL = 0.74 (P) (INPUT = 1.00)

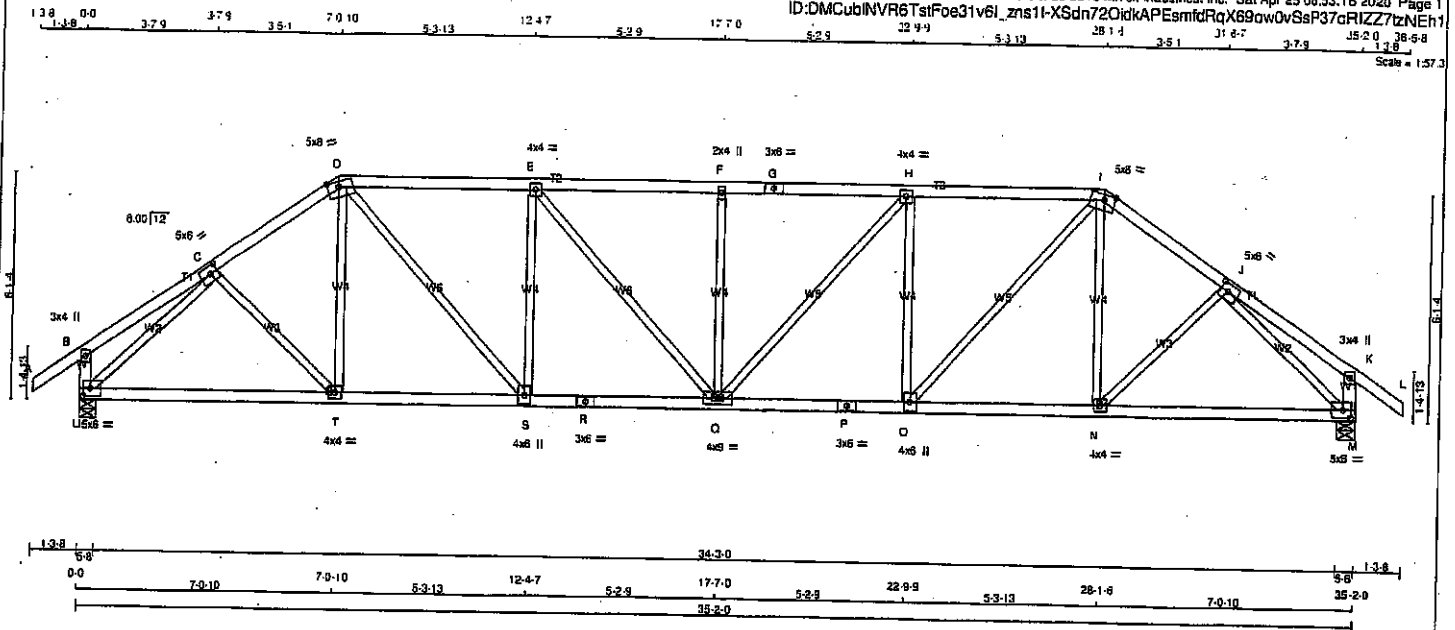


Structural component only
DWG# T-2006954

JOB NAME 408087	TRUSS NAME T3	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-i	MT20	5.0	6.0	2.50	2.50
D	TTWV-m	MT20	5.0	8.0	1.75	3.50
E	TMVW-i	MT20	4.0	4.0		
F	TMVW-i	MT20	2.0	4.0		
G	TS-i	MT20	3.0	6.0		
H	TMVW-i	MT20	4.0	4.0		
I	TTWV-m	MT20	5.0	8.0	1.75	3.50
J	TMVW-i	MT20	5.0	8.0	2.50	2.50
K	TMV+p	MT20	3.0	4.0		
M	BMVW-i	MT20	5.0	8.0		
N	BMVW-i	MT20	4.0	4.0	2.50	2.25
O	BMVW-i	MT20	4.0	8.0		
P	BS-i	MT20	3.0	6.0		
Q	BMVW-i	MT20	4.0	8.0		
R	BS-i	MT20	3.0	6.0		
S	BMVW-i	MT20	4.0	8.0		
T	BMVW-i	MT20	4.0	8.0		
U	BMVW-i	MT20	5.0	8.0	2.50	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	0	5-8
U	HORZ	HORZ	0	5-8
M	VERT	DOWN	0	5-8

UNFACTORED REACTIONS

1ST LOASE	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
JT	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
U	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
M	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO						FR-TO			
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	C-T	0 / 135	0.03 (4)	
B-C	0 / 18	-91.8	-91.8	0.16 (1)	10.00	T-D	0 / 90	0.03 (4)	
C-D	-2308 / 0	-91.8	-91.8	0.23 (1)	4.31	D-S	0 / 1218	0.27 (1)	
D-E	-2724 / 0	-91.8	-91.8	0.54 (1)	3.69	S-E	-814 / 0	0.48 (1)	
E-F	-2967 / 0	-91.8	-91.8	0.56 (1)	3.53	E-Q	0 / 363	0.08 (1)	
F-G	-2967 / 0	-91.8	-91.8	0.56 (1)	3.53	Q-F	-440 / 0	0.26 (1)	
G-H	-2967 / 0	-91.8	-91.8	0.56 (1)	3.53	H-Q	0 / 363	0.08 (1)	
H-I	-2724 / 0	-91.8	-91.8	0.54 (1)	3.69	Q-H	-814 / 0	0.48 (1)	
I-J	-2308 / 0	-91.8	-91.8	0.23 (1)	4.31	O-I	0 / 1218	0.27 (1)	
J-K	0 / 18	-91.8	-91.8	0.16 (1)	10.00	N-I	0 / 90	0.03 (4)	
K-L	0 / 35	-91.8	-91.8	0.12 (1)	10.00	N-J	0 / 135	0.03 (4)	
U-B	-258 / 0	0.0	0.0	0.03 (1)	7.81	U-C	-2515 / 0	0.97 (1)	
M-K	-258 / 0	0.0	0.0	0.03 (1)	7.81	J-M	-2515 / 0	0.97 (1)	
U-T	0 / 1802	-18.5	-18.5	0.40 (1)	10.00				
T-S	0 / 1801	-18.5	-18.5	0.41 (1)	10.00				
S-R	0 / 2724	-18.5	-18.5	0.49 (1)	10.00				
R-Q	0 / 2724	-18.5	-18.5	0.49 (1)	10.00				
Q-P	0 / 2724	-18.5	-18.5	0.49 (1)	10.00				
P-O	0 / 2724	-18.5	-18.5	0.49 (1)	10.00				
O-N	0 / 1801	-18.5	-18.5	0.41 (1)	10.00				
N-M	0 / 1802	-18.5	-18.5	0.40 (1)	10.00				

TOTAL WEIGHT = 2 X 151 = 302 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/998 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/998 (0.29")

CS: TC=0.56/1.00 (E-F:1) , BC=0.49/1.00 (C-S:1) ,
WB=0.97/1.00 (C-U:1) , SS=0.23/1.00 (D-E:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PS) (PL) (PLJ)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1667 1656

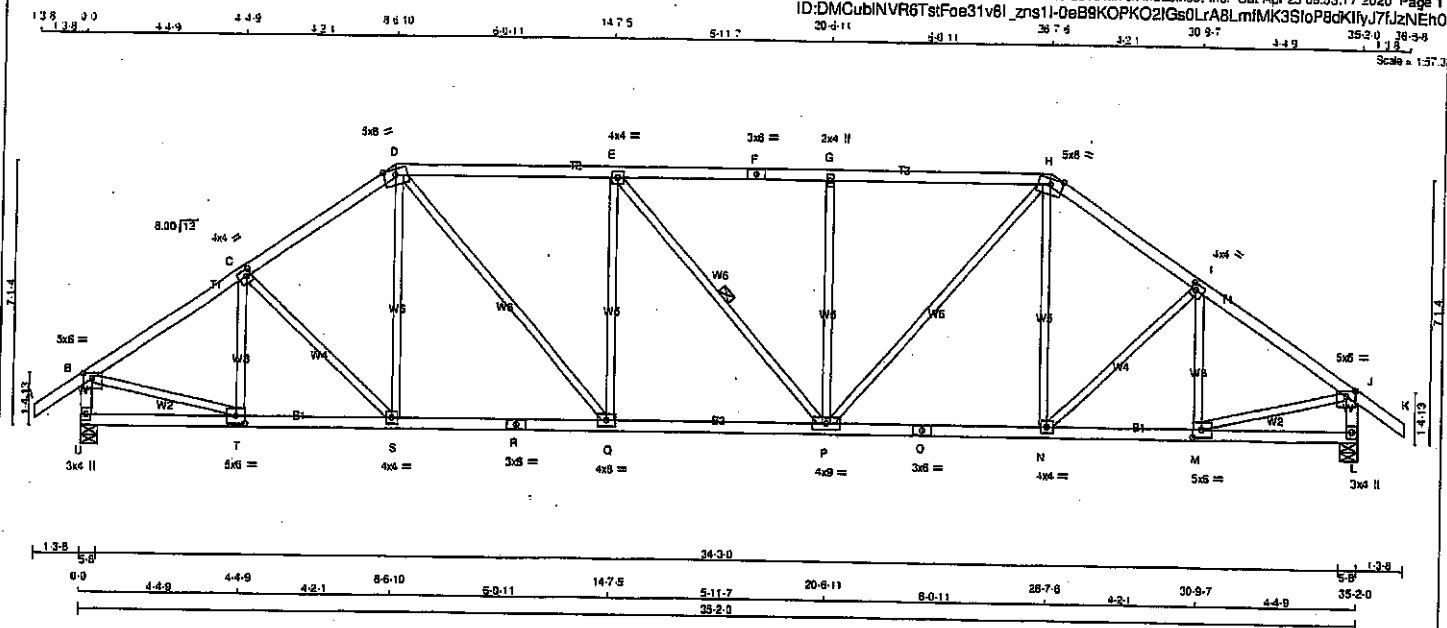
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (U) (INPUT = 0.90)
JSI METAL = 0.84 (P) (INPUT = 1.00)



Structural component only
DWG# T-2006955



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	2.75
C	TMVW-i	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	8.0	Edge	
E	TMVW-i	MT20	4.0	4.0		
F	TS-i	MT20	3.0	6.0		
G	TMVW-w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge	
I	TMVW-i	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	1.75	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMVW-i	MT20	5.0	8.0	2.50	2.75
N	BMVW-i	MT20	4.0	4.0		
O	RS-i	MT20	3.0	6.0		
P	BMVW-w	MT20	4.0	4.0		
Q	BMVW-i	MT20	4.0	6.0		
R	BS-i	MT20	3.0	6.0		
S	BMVW-i	MT20	4.0	4.0		
T	BMVW-i	MT20	5.0	8.0	2.50	2.75
U	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
U	2065	0	2065	0
L	2065	0	2065	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
L	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	T-C	-396 / 0	0.11 (1)
B-C	-2270 / 0	-91.8	-91.8 0.37 (1)	4.19	C-S	-79 / 0	0.05 (1)
C-D	-2254 / 0	-91.8	-91.8 0.36 (1)	4.21	S-D	0 / 156	0.04 (4)
D-E	-2485 / 0	-91.8	-91.8 0.84 (1)	3.72	D-Q	0 / 949	-0.21 (1)
E-F	-2484 / 0	-91.8	-91.8 0.84 (1)	3.72	Q-E	-587 / 0	0.52 (1)
F-G	-2484 / 0	-91.8	-91.8 0.83 (1)	3.74	E-P	-2 / 0	0.00 (1)
G-H	-2254 / 0	-91.8	-91.8 0.36 (1)	4.21	P-G	-587 / 0	0.52 (1)
H-I	-2254 / 0	-91.8	-91.8 0.37 (1)	4.19	N-H	0 / 947	0.21 (1)
I-J	-2270 / 0	-91.8	-91.8 0.12 (1)	10.00	N-I	0 / 157	0.04 (4)
J-K	0 / 35	-91.8	-91.8 0.12 (1)	10.00	M-I	-78 / 0	0.05 (1)
U-B	-2027 / 0	0.0	0.0 0.21 (1)	5.94	M-J	-397 / 0	0.11 (1)
L-J	-2028 / 0	0.0	0.0 0.21 (1)	5.94	B-T	0 / 1966	0.44 (1)
					M-J	0 / 1966	0.44 (1)
U-T	0 / 0	-18.5	-18.5 0.08 (4)	10.00			
T-S	0 / 1909	-18.5	-18.5 0.37 (1)	10.00			
S-R	0 / 1854	-18.5	-18.5 0.36 (1)	10.00			
R-Q	0 / 1854	-18.5	-18.5 0.36 (1)	10.00			
Q-P	0 / 2485	-18.5	-18.5 0.47 (1)	10.00			
P-O	0 / 1854	-18.5	-18.5 0.36 (1)	10.00			
O-N	0 / 1854	-18.5	-18.5 0.36 (1)	10.00			
N-M	0 / 1909	-18.5	-18.5 0.37 (1)	10.00			
M-L	0 / 0	-18.5	-18.5 0.08 (4)	10.00			

TOTAL WEIGHT = 2 X 155 = 309 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17')
CALCULATED VERT. DEFL.(LL) = L/999 (0.12')
ALLOWABLE DEFL.(TL) = L/360 (1.17')
CALCULATED VERT. DEFL.(TL) = L/999 (0.24')

CSI: TC=0.64/1.00 (D-E:1), BC=0.47/1.00 (P-Q:1), WB=0.52/1.00 (E-Q:1), SS=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

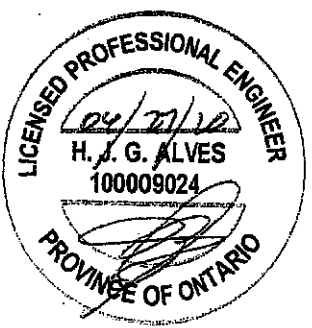
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

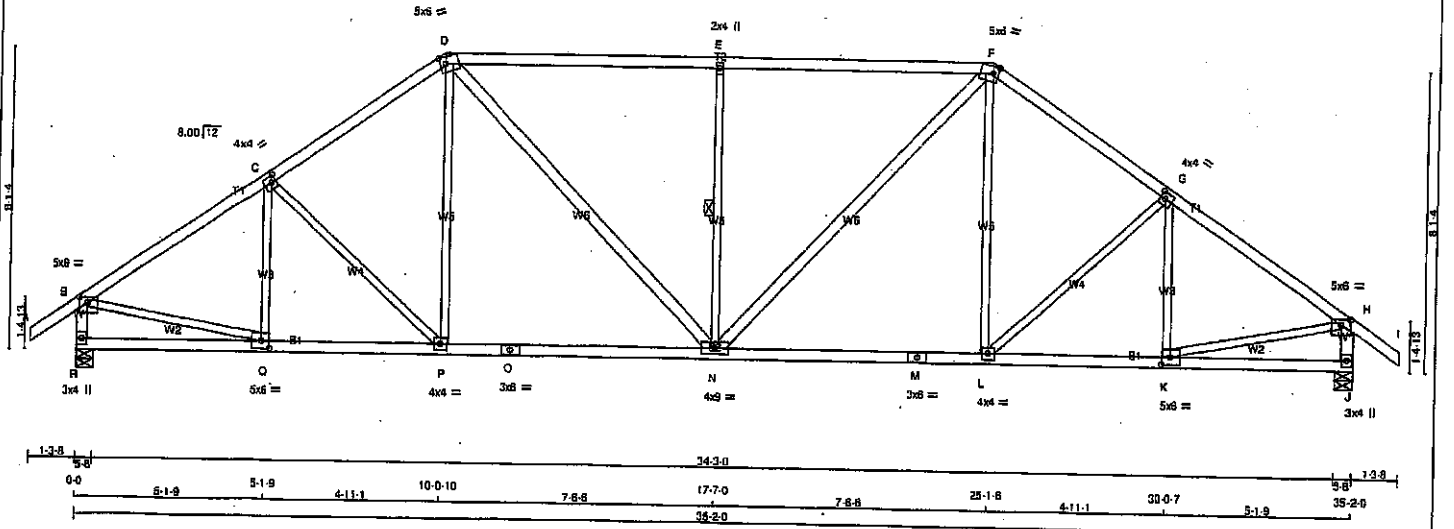
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (D) (INPUT = 0.90)
JSI METAL=0.82 (O) (INPUT = 1.00)





LUMBER					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 2 X 158 = 317 lb		
N. L. G. A. RULES					BEARINGS										DESIGN CRITERIA		
CHORDS	SIZE		LUMBER	DESCR.	SPF	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG		SPECIFIED LOADS:				
A - D	2x4	DRY	No.2	SPF		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	TOP CH. LL = 25.6 PSF				
D - F	2x4	DRY	No.2	SPF									OL = 8.0 PSF				
F - I	2x4	DRY	No.2	SPF									BOT CH. LL = 0.0 PSF				
R - B	2x4	DRY	No.2	SPF									OL = 7.4 PSF				
J - H	2x4	DRY	No.2	SPF									TOTAL LOAD = 39.0 PSF				
R - O	2x4	DRY	No.2	SPF													
O - M	2x4	DRY	No.2	SPF													
M - J	2x4	DRY	No.2	SPF													
ALL WEBS					UNFACTORED REACTIONS										SPACING = 24.0 IN./C		
EXCEPT					1ST LCASE MAX/MIN. COMPONENT REACTIONS										LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12		
D - N	2x4	DRY	No.2	SPF		JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL				
N - F	2x4	DRY	No.2	SPF		R	1458	970/0	0/0	0/0	0/0	488/0	0/0				
				SPF		J	1458	970/0	0/0	0/0	0/0	488/0	0/0				

DRY: SEASONED LUMBER.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED	FACTORED	
	FORCE	VERT. LOAD	LC1 MAX		FORCE	MAX	
	(LBS)	(PLF)	CS1 (LC)		(LBS)	CS1 (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0/35	-91.8	-91.8 0.12 (1)	10.00	Q-C	-321/0	0.11 (1)
B-C	-2318/0	-91.8	-91.8 0.38 (1)	4.16	C-P	-218/0	0.18 (1)
C-D	-2186/0	-91.8	-91.8 0.37 (1)	4.28	P-D	0/266	0.06 (1)
D-E	-2275/0	-91.8	-91.8 0.81 (1)	3.47	D-N	0/892	0.11 (1)
E-F	-2275/0	-91.8	-91.8 0.81 (1)	3.47	N-E	-851/0	0.35 (1)
F-G	-2186/0	-91.8	-91.8 0.37 (1)	4.28	N-F	0/692	0.11 (1)
G-H	-2318/0	-91.8	-91.8 0.38 (1)	4.16	L-F	0/266	0.06 (1)
H-I	0/35	-91.8	-91.8 0.12 (1)	10.00	L-G	-218/0	0.18 (1)
R-B	-2022/0	0.0	0.0 0.21 (1)	5.94	K-G	-321/0	0.11 (1)
J-H	-2022/0	0.0	0.0 0.21 (1)	5.94	K-Q	0/1996	0.45 (1)
					K-H	0/1996	0.45 (1)
R-Q	0/0	-18.5	-18.5 0.10 (4)	10.00			
Q-P	0/1953	-18.5	-18.5 0.40 (1)	10.00			
P-O	0/1794	-18.5	-18.5 0.41 (1)	10.00			
O-N	0/1794	-18.5	-18.5 0.41 (1)	10.00			
N-M	0/1794	-18.5	-18.5 0.41 (1)	10.00			
M-L	0/1794	-18.5	-18.5 0.41 (1)	10.00			
L-K	0/1953	-18.5	-18.5 0.40 (1)	10.00			
K-J	0/0	-18.5	-18.5 0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CS1: TC=0.81/1.00 (D-E:1), BC=0.41/1.00 (N-P:1),
WB=0.45/1.00 (B-Q:1), SS1=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

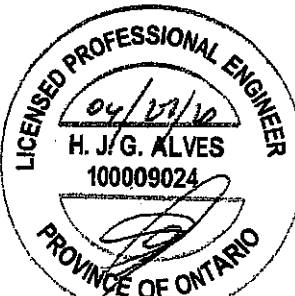
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 618 354 1667 788 1997 1656

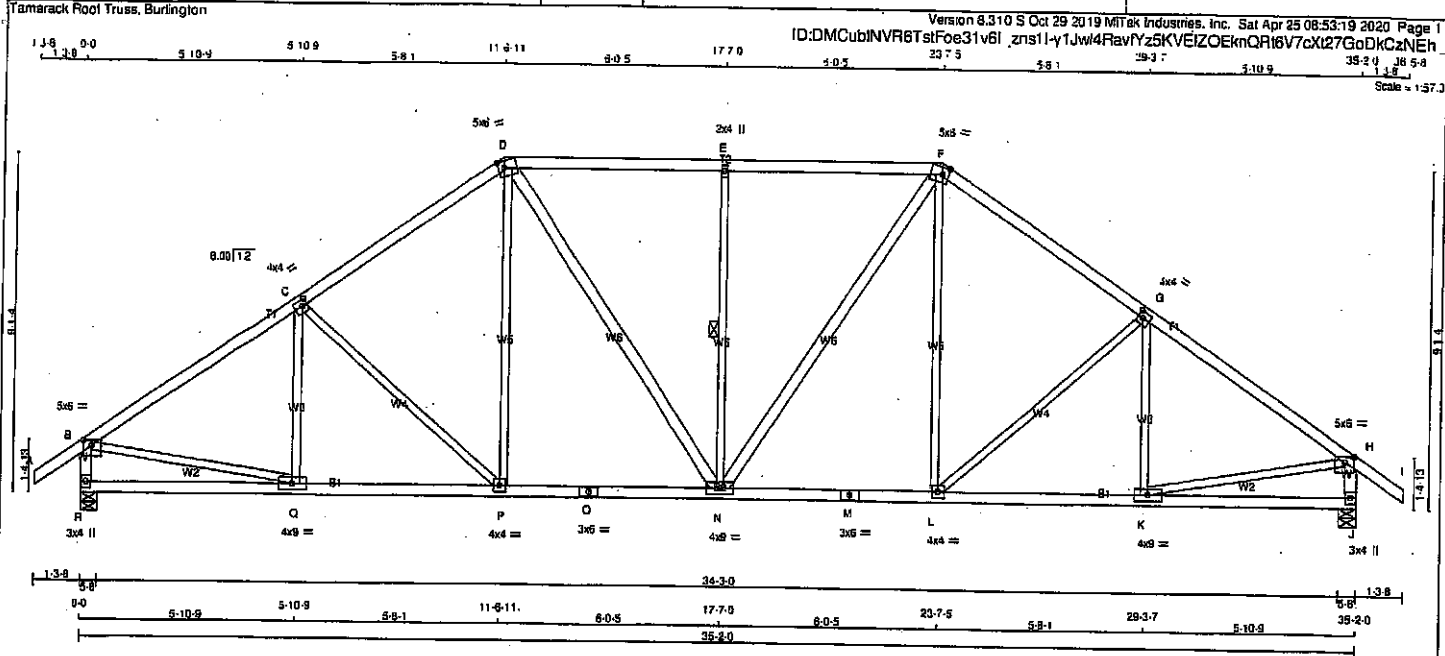
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.57 (C) (INPUT = 1.00)



Structural component only
DWG# T-2006957



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY, SEASONED LUMBER.

PLATES (table is in inches)						
PLATE	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TMVW-l	MT20	4.0	4.0	2.00	1.50
D	TMVW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-l	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.75	2.75
J	BMV1-p	MT20	3.0	4.0		
K	BMVW-l	MT20	4.0	9.0		
L	BMVW-l	MT20	4.0	4.0		
M	BS-l	MT20	3.0	8.0		
N	BMVW-l	MT20	4.0	9.0		
O	BS-l	MT20	3.0	6.0		
P	BMVW-l	MT20	4.0	4.0		
Q	BMVW-l	MT20	4.0	9.0		
R	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	0	0
R	HORIZ	UP/LIFT	5-8	5-8
J	VERT	DOWN	0	0
J	HORIZ	UP/LIFT	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED
R	SNOW
J	LIVE
J	PERM. LIVE
J	WIND
J	DEAD
J	SOIL

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-91.8 -91.8 0.12 (1)	10.00	Q-C	-248 / 11	0.11 (1)	
B-C	-2347 / 0	-91.8 -91.8 0.52 (1)	3.99	C-P	-358 / 0	0.42 (1)	
C-D	-2098 / 0	-91.8 -91.8 0.48 (1)	4.22	P-D	0 / 342	0.08 (1)	
D-E	-1985 / 0	-91.8 -91.8 0.49 (1)	4.26	D-N	0 / 477	0.08 (1)	
E-F	-1985 / 0	-91.8 -91.8 0.49 (1)	4.26	N-E	-877 / 0	0.37 (1)	
F-G	-2098 / 0	-91.8 -91.8 0.48 (1)	4.22	N-F	0 / 477	0.08 (1)	
G-H	-2347 / 0	-91.8 -91.8 0.52 (1)	3.99	L-F	0 / 342	0.08 (1)	
H-I	0 / 35	-91.8 -91.8 0.12 (1)	10.00	L-G	-358 / 0	0.42 (1)	
R-B	-2018 / 0	0.0 0.0 0.21 (1)	5.94	K-G	-248 / 11	0.11 (1)	
J-H	-2018 / 0	0.0 0.0 0.21 (1)	5.94	B-Q	0 / 2014	0.45 (1)	
R-Q	0 / 0	-18.5 -18.5 0.14 (4)	10.00	K-H	0 / 2014	0.45 (1)	
Q-P	0 / 1981	-18.5 -18.5 0.39 (1)	10.00				
P-O	0 / 1716	-18.5 -18.5 0.34 (1)	10.00				
O-N	0 / 1716	-18.5 -18.5 0.34 (1)	10.00				
N-M	0 / 1716	-18.5 -18.5 0.34 (1)	10.00				
M-L	0 / 1716	-18.5 -18.5 0.34 (1)	10.00				
L-K	0 / 1981	-18.5 -18.5 0.39 (1)	10.00				
K-J	0 / 0	-18.5 -18.5 0.14 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF, LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17')
CALCULATED VERT. DEFL. (LL) = L/999 (0.10')
ALLOWABLE DEFL. (TL) = L/360 (1.17')
CALCULATED VERT. DEFL. (TL) = L/999 (0.19')

CSI: TC=0.52/1.00 (B-C-1), BC=0.39/1.00 (K-L-1), WB=0.45/1.00 (H-K-1), SS=0.27/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

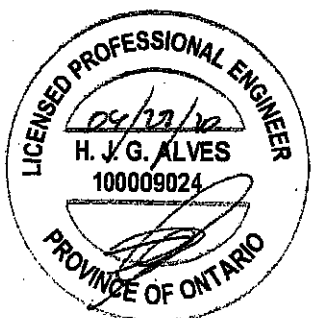
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

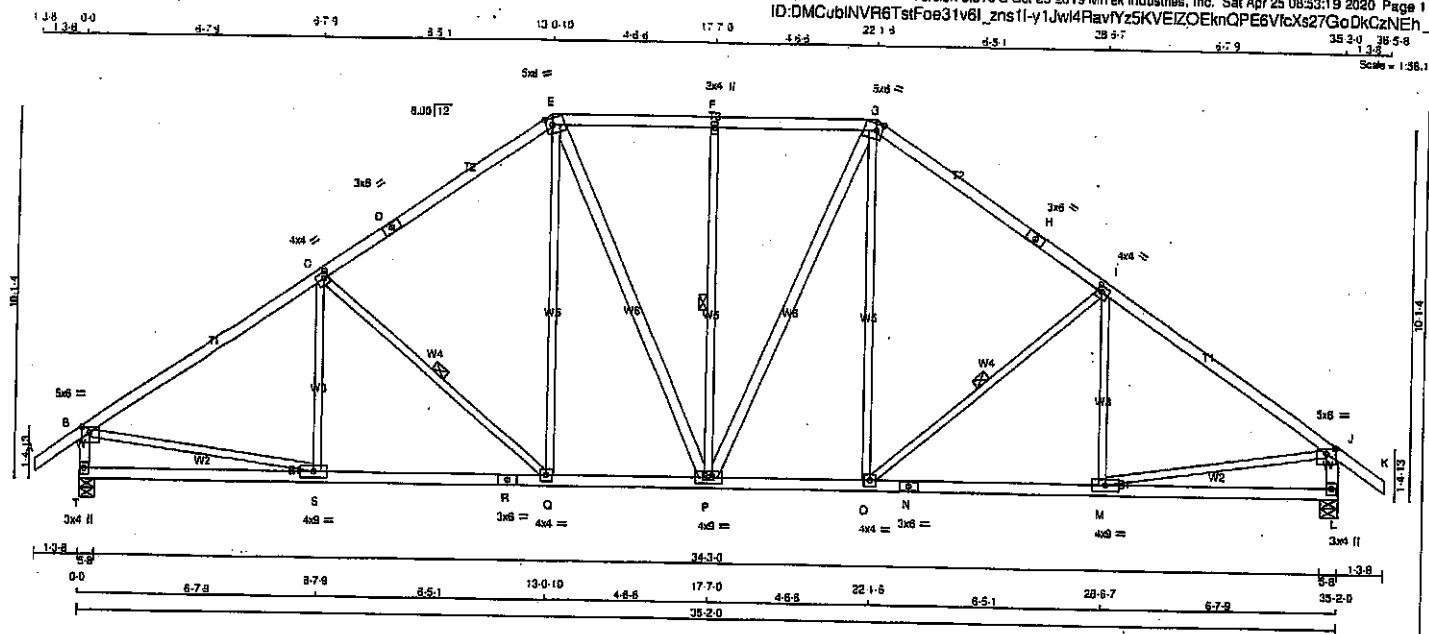
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.87 (B) (INPUT = 0.90)
JSI METAL=0.56 (M) (INPUT = 1.00)





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - R	2x4	DRY	No.2
R - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
E - P	2x4	DRY	No.2
P - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	2.75
C	TMVW-i	MT20	4.0	4.0	2.00	1.50
D	TS-i	MT20	3.0	6.0		
E	TTWV-m	MT20	5.0	8.0	2.25	2.00
F	TMVW-w	MT20	2.0	4.0		
G	TTWV-m	MT20	5.0	8.0	2.25	2.00
H	TS-i	MT20	3.0	6.0		
I	TMVW-i	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	1.75	2.75
L	BMV1-p	MT20	3.0	4.0		
M	BMVW-i	MT20	4.0	9.0		
N	BS-i	MT20	3.0	6.0		
O	BMVW-i	MT20	4.0	4.0		
P	BMVW-i	MT20	4.0	9.0		
Q	BMVW-i	MT20	4.0	4.0		
R	BS-i	MT20	3.0	6.0		
S	BMVW-i	MT20	4.0	9.0		
T	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
T	2065	0	0	5-8
L	2065	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
T	1458	970 / 0 0 / 0 0 / 0 0 / 0 488 / 0 0 / 0
L	1458	970 / 0 0 / 0 0 / 0 0 / 0 488 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-Q, F-P, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO					FR-TO		
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	S-C	-189 / 46	0.10 (1)
B-C	-2359 / 0	-91.8	-91.8 0.69 (1)	3.77	C-Q	-485 / 0	0.24 (1)
C-D	-1996 / 0	-91.8	-91.8 0.62 (1)	4.11	Q-E	0 / 416	0.09 (1)
D-E	-1996 / 0	-91.8	-91.8 0.62 (1)	4.11	E-P	0 / 315	0.05 (1)
E-F	-1762 / 0	-91.8	-91.8 0.27 (1)	4.77	P-F	-502 / 0	0.35 (1)
F-G	-1762 / 0	-91.8	-91.8 0.27 (1)	4.77	P-G	0 / 315	0.05 (1)
G-H	-1996 / 0	-91.8	-91.8 0.62 (1)	4.11	O-G	0 / 416	0.09 (1)
H-I	-1996 / 0	-91.8	-91.8 0.62 (1)	4.11	O-I	-485 / 0	0.24 (1)
I-J	-2359 / 0	-91.8	-91.8 0.69 (1)	3.77	M-I	-189 / 46	0.10 (1)
J-K	0 / 35	-91.8	-91.8 0.12 (1)	10.00	B-S	0 / 2022	0.45 (1)
T-B	-2014 / 0	0.0	0.0 0.21 (1)	5.95	M-J	0 / 2022	0.45 (1)
L-J	-2014 / 0	0.0	0.0 0.21 (1)	5.95			
T-S	0 / 0	-18.5	-18.5 0.19 (4)	10.00			
S-R	0 / 1996	-18.5	-18.5 0.42 (1)	10.00			
R-O	0 / 1996	-18.5	-18.5 0.42 (1)	10.00			
Q-P	0 / 1830	-18.5	-18.5 0.32 (1)	10.00			
P-O	0 / 1830	-18.5	-18.5 0.32 (1)	10.00			
O-N	0 / 1996	-18.5	-18.5 0.42 (1)	10.00			
N-M	0 / 1996	-18.5	-18.5 0.42 (1)	10.00			
M-L	0 / 0	-18.5	-18.5 0.19 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17')
CALCULATED VERT. DEFL.(LL) = L/999 (0.09')
ALLOWABLE DEFL.(TL) = L/360 (1.17')
CALCULATED VERT. DEFL.(TL) = L/999 (0.18')

CSH: TC=0.68/1.00 (I-J:1), BC=0.42/1.00 (M-O:1), WB=0.45/1.00 (J-M:1), SS=0.24/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1856

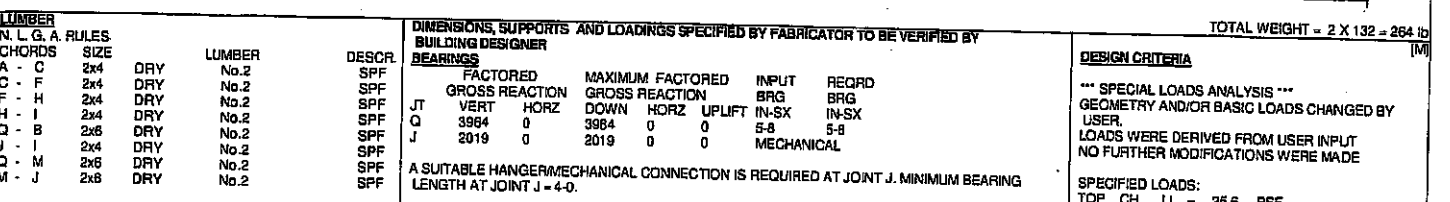
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.87 (B) (INPUT = 0.90)
JSI METAL=0.58 (N) (INPUT = 1.00)



138 00 4011488 388 209 148-15 309 2098 2113 26-11 d 19-10 Scale = 1:46.5



EXCEPT

RY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	2797	1871 / 0	0 / 0	0 / 0	0 / 0	926 / 0	0 / 0
J	1428	943 / 0	0 / 0	0 / 0	0 / 0	483 / 0	0 / 0

EOT CH. DL = 6.0 PSF
LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

CHORDS > ROWS	SURFACE SPACING (IN)	LOAD (PLF)	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q	LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12
OF CHORDS : (0.122"x3") SPIRAL NAILS			BRACING	*** NON STANDARD GIRDER ***
C 1 12		SIDE(61.0)	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08 FT.	ADDTL USER-DEFINED LOADS APPLIED TO ALL
H 1 12		SIDE(61.0)	MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.	LOAD CASES.
I 1 12		TOP	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.	
I 1 12		TOP		THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
I 2 12		TOP	LOADING	SMALL BUILDING REQUIREMENTS OF PART 9,
		TOP	TOTAL LOAD CASES: (4)	NBCQ 2010, DRG 701

[illegible]

Q-H	0/0	-18.5	-18.5	0.06 (1)	10.00	WS=0.51/1.00 (G-P:1), SS=0.15/1.00 (G-H:1)
R-S	0/0	-18.5	-18.5	0.06 (1)	10.00	
S-P	0/0	-18.5	-18.5	0.06 (1)	10.00	DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

C-N	0/4979	-18.5	-18.5 0.37 (1)	10.00
N-M	0/3457	-18.5	-18.5 0.36 (1)	10.00
M-L	0/3457	-18.5	-18.5 0.27 (1)	10.00
L-K	0/988	-18.5	-18.5 0.27 (1)	10.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HERE ONLY

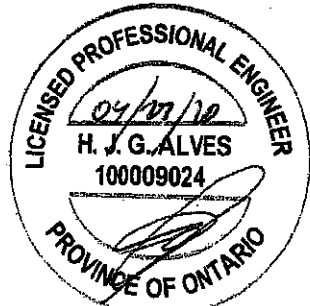
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

C	4-0-11	-254	-254	--	FRONT	VERT	SNOW			C1	NAIL VALUES
P	4-8-8	-2329	-2329	--	FRONT	VERT	TOTAL			C1	PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
R	9-4	-29	-29	--	FRONT	VERT	TOTAL			C1	MAX MIN MAX MIN MAX MIN
S	2-9-4	-26	-26	--	FRONT	VERT	TOTAL			C1	MT20 818 354 1687 788 1087 1666

1) CT: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only

DWG# T-2006960 1/2



Structural component only
DWG# T-2006960 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408087	T8	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

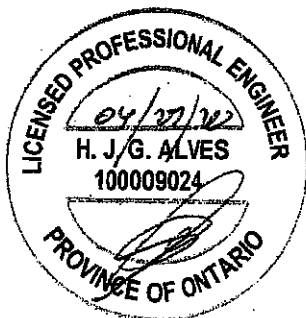
Version 8.310 S Oct 29 2019 MITek Industries, Inc. Sat Apr 25 08:53:20 2020 Page 2

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PLATES (table is in inches)

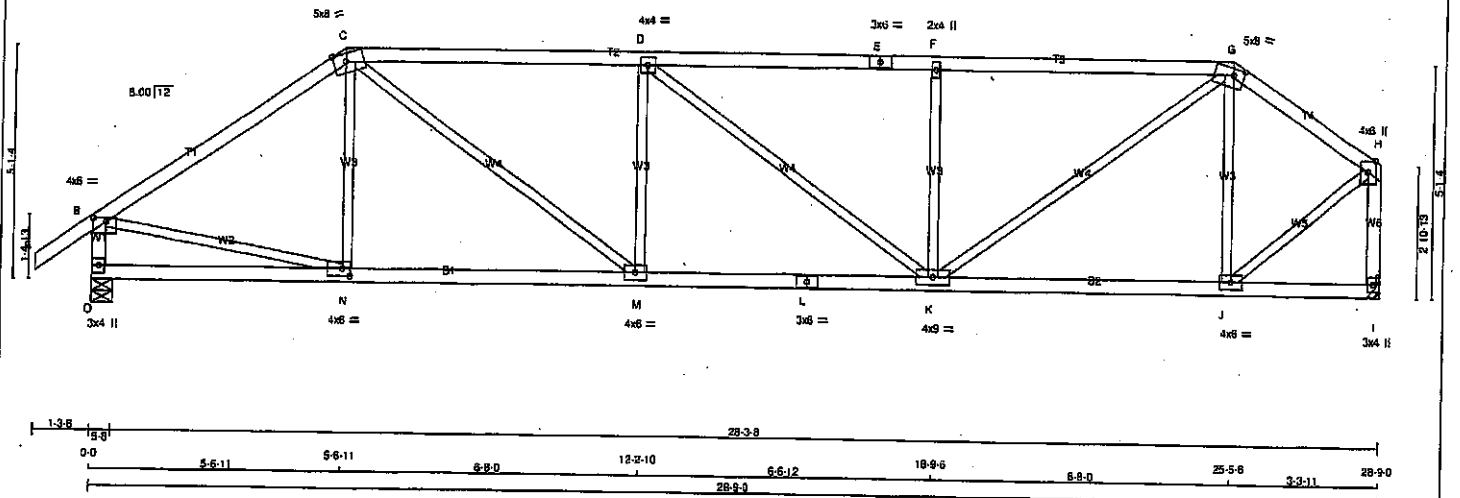
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTW-m	MT20	5.0	8.0	Edge	
D	TMWWW-t	MT20	4.0	9.0		
E	TMW-w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge 5.50	
I	TMVW-p	MT20	4.0	4.0	1.25 2.00	
J	BMV1-p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMW-w	MT20	3.0	6.0		
P	BMWWW-t	MT20	7.0	8.0	4.25 3.25	
Q	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



Structural component only
DWG# T-2006960 212

JOB NAME 408087	TRUSS NAME T9	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	
Version 8.310 S Oct 29 2019 MITek Industries, Inc. Sat Apr 25 08:53:21 2020 Page 1					
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Scale = 1:16.5					



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWV-p	MT20	4.0	6.0	1.00 3.25
C	TTWV-m	MT20	5.0	8.0	2.00 3.25
D	TMWV-l	MT20	4.0	4.0	
E	TS-l	MT20	3.0	6.0	
F	TMWV-w	MT20	2.0	4.0	
G	TTWV-m	MT20	5.0	8.0	1.75 2.75
H	TMWV-p	MT20	4.0	6.0	Edge
I	BMV1-p	MT20	3.0	4.0	
J	BMWV-l	MT20	4.0	6.0	
K	BMWV-w	MT20	4.0	6.0	
L	BS-l	MT20	3.0	6.0	
M	BMWV-l	MT20	4.0	6.0	
N	BMWV-w	MT20	4.0	6.0	2.00 2.00
O	BMV1-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG			
O	1711 0	1711 0	5-8	5-8			
I	1585 0	1585 0	MECHANICAL				

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS							
JT	1ST CASE COMBINED	MAX. MIN. COMPONENT REACTIONS					
O	1208	806 / 0	0 / 0	0 / 0	0 / 0	402 / 0	0 / 0
I	1121	736 / 0	0 / 0	0 / 0	0 / 0	385 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING							
TOTAL LOAD CASES: (4)							
CHORDS							
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORZ. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (PLF)
FR-TO					FR-TO		
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	N-C	-159 / 42	0.06 (1)
B-C	-1804 / 0	-91.8	-91.8 0.83 (1)	4.25	C-M	0 / 1121	0.25 (1)
C-D	-2406 / 0	-91.8	-91.8 0.79 (1)	3.59	M-D	-536 / 0	0.21 (1)
D-E	-2238 / 0	-91.8	-91.8 0.78 (1)	3.73	D-K	-210 / 0	0.29 (1)
E-F	-2238 / 0	-91.8	-91.8 0.78 (1)	3.73	K-F	-680 / 0	0.25 (1)
F-G	-2238 / 0	-91.8	-91.8 0.78 (1)	3.74	K-G	0 / 1548	0.35 (1)
G-H	-1196 / 0	-91.8	-91.8 0.19 (1)	5.62	J-G	-642 / 0	0.25 (1)
O-B	-1670 / 0	0.0	0.0 0.17 (1)	6.42	B-N	0 / 1526	0.34 (1)
I-H	-1570 / 0	0.0	0.0 0.24 (1)	6.56	J-H	0 / 1246	0.28 (1)
O-N	0 / 0	-18.5	-18.5 0.16 (4)	10.00			
N-M	0 / 1496	-18.5	-18.5 0.33 (1)	10.00			
M-L	0 / 2406	-18.5	-18.5 0.46 (1)	10.00			
L-K	0 / 2406	-18.5	-18.5 0.46 (1)	10.00			
K-J	0 / 980	-18.5	-18.5 0.24 (4)	10.00			
J-I	0 / 0	-18.5	-18.5 0.12 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSK: TC=0.79/1.00 (C-D:1), BC=0.46/1.00 (K-M:1), WB=0.35/1.00 (G-K:1), SB=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

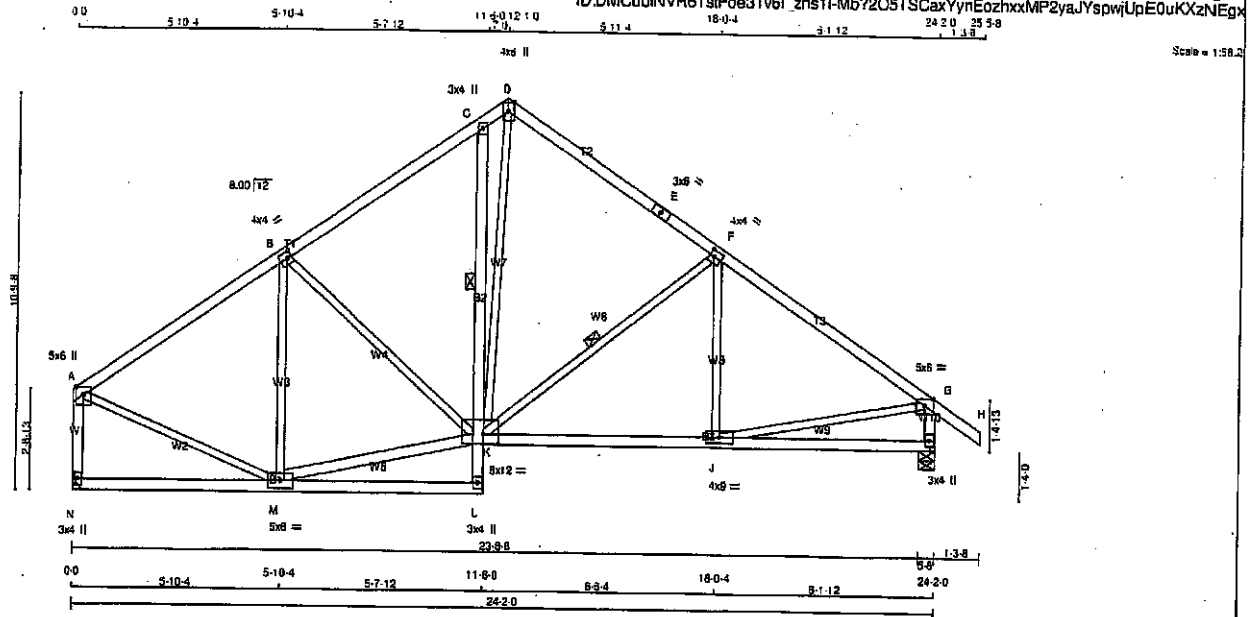
JSI GRIP = 0.88 (B) (INPUT = 0.90)
JSI METAL = 0.79 (L) (INPUT = 1.00)



Structural component only
DWG# T-2006961

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408087	T10S	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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ID:DMCubINVR6TstFoe31v6l_zns11-Mb72OSTScaxYynEozhxxMP2yaJYspwUpE0uKXzNEgX



CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
N - A	2x4	DRY	No.2
I - G	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - C	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS EXCEPT M - K	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge
B	TMVW-t	MT20	4.0	4.0	2.00 1.50
C	TMV+p	MT20	3.0	4.0	
D	TTW+p	MT20	4.0	6.0	Edge
E	TS-t	MT20	3.0	6.0	
F	TMVW-t	MT20	4.0	4.0	2.00 1.50
G	TMVW-p	MT20	5.0	6.0	Edge
I	BMV1+p	MT20	3.0	4.0	
J	BMVW-t	MT20	4.0	9.0	
K	BMVWVW-t	MT20	8.0	12.0	3.25 5.00
L	BMV+p	MT20	3.0	4.0	
M	BMVWVW-t	MT20	5.0	8.0	
N	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
N	1332	0	1332	0
I	1458	0	1458	0
			UPLIFT	IN-SX
			5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT N. MINIMUM BEARING LENGTH AT JOINT N = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	942	618 / 0	0 / 0	0 / 0	0 / 0	324 / 0	0 / 0
I	1029	669 / 0	0 / 0	0 / 0	0 / 0	340 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-K, F-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	W E B S	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH FR-TO	MEMB. FORCE (LBS)
FR-TO				
A-B	-1161 / 0	-91.8	-91.8 0.40 (1)	5.41
B-C	-1153 / 0	-91.8	-91.8 0.38 (1)	5.46
C-D	-1180 / 0	-91.8	-91.8 0.10 (1)	5.78
D-E	-1081 / 0	-91.8	-91.8 0.45 (1)	5.52
E-F	-1081 / 0	-91.8	-91.8 0.45 (1)	5.52
F-G	-1494 / 0	-91.8	-91.8 0.49 (1)	4.80
G-H	0 / 35	-91.8	-91.8 0.12 (1)	10.00
H-A	-1291 / 0	0.0	0.0 0.19 (1)	7.10
I-G	-1415 / 0	0.0	0.0 0.15 (1)	6.85
N-M	0 / 0	-18.5	-18.5 0.17 (4)	10.00
M-L	0 / 10	-18.5	-18.5 0.17 (4)	10.00
L-K	0 / 44	0.0	0.0 0.02 (1)	10.00
K-C	-337 / 0	0.0	0.0 0.03 (1)	6.25
K-J	0 / 1273	-18.5	-18.5 0.32 (4)	10.00
J-I	0 / 0	-18.5	-18.5 0.21 (4)	10.00

TOTAL WEIGHT = 2 X 122 = 244 lb (MIF)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI = TC=0.49/1.00 (F-G:1), BC=0.32/1.00 (J-K:4), WB=0.38/1.00 (B-M:1), SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

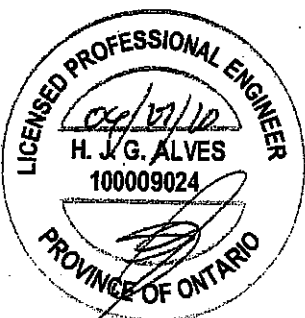
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

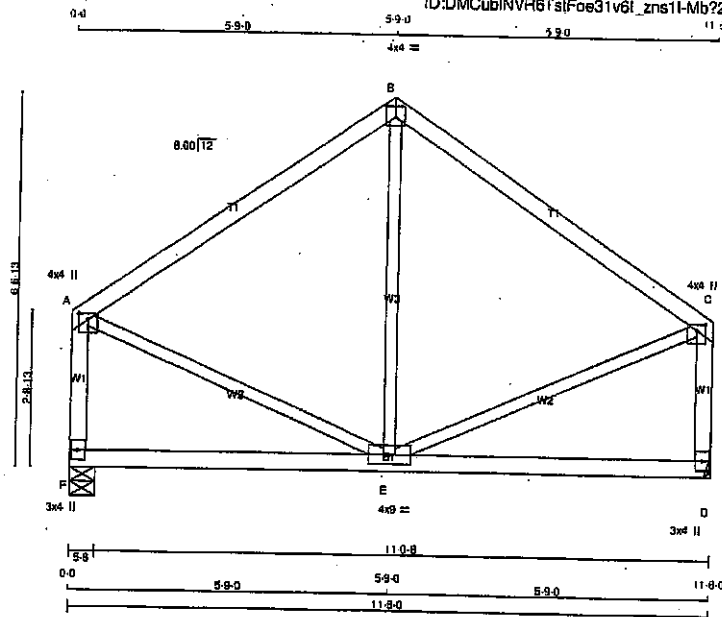
JSI GRIP=0.82 (G) (INPUT = 0.90)
JSI METAL=0.48 (A) (INPUT = 1.00)



Structural component only
DWG# T-2006962

JOB NAME 408087	TRUSS NAME T11	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1/2" = 1'-0"

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - B	2x4	DRY No.2
B - C	2x4	DRY No.2
F - A	2x4	DRY No.2
D - C	2x4	DRY No.2
F - D	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTW+p	MT20	4.0	4.0	2.25	2.00
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMVWW-t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
F	VERT 834	DOWN 834	IN-SX 5-8	IN-SX 5-8
D	HORZ 834	HORZ 834	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1'-8".

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	COMBINED	2947/0	0/0	0/0	0/0	1547/0	0/0
D		2947/0	0/0	0/0	0/0	1547/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-365/0	-91.8 -91.8	0.39 (1)	E-B	-123/54	0.09 (1)	
B-C	-365/0	-91.8 -91.8	0.39 (1)	A-E	0/329	0.07 (1)	
F-A	-593/0	0.0 0.0	0.09 (1)	E-C	0/329	0.07 (1)	
D-C	-593/0	0.0 0.0	0.09 (1)				
F-E	0/0	-18.5 -18.5	0.18 (4)				
E-D	0/0	-18.5 -18.5	0.18 (4)				

TOTAL WEIGHT = 2 X 49 = 98 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 8.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 8 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.39/1.00 (A-B:1), BC=0.18/1.00 (D-E:4),
WB=0.09/1.00 (B-E:1), SS=0.17/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

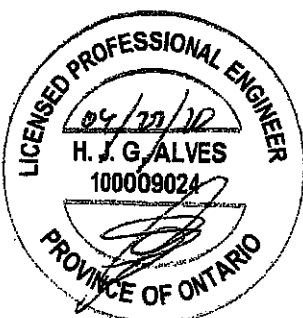
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1657 788

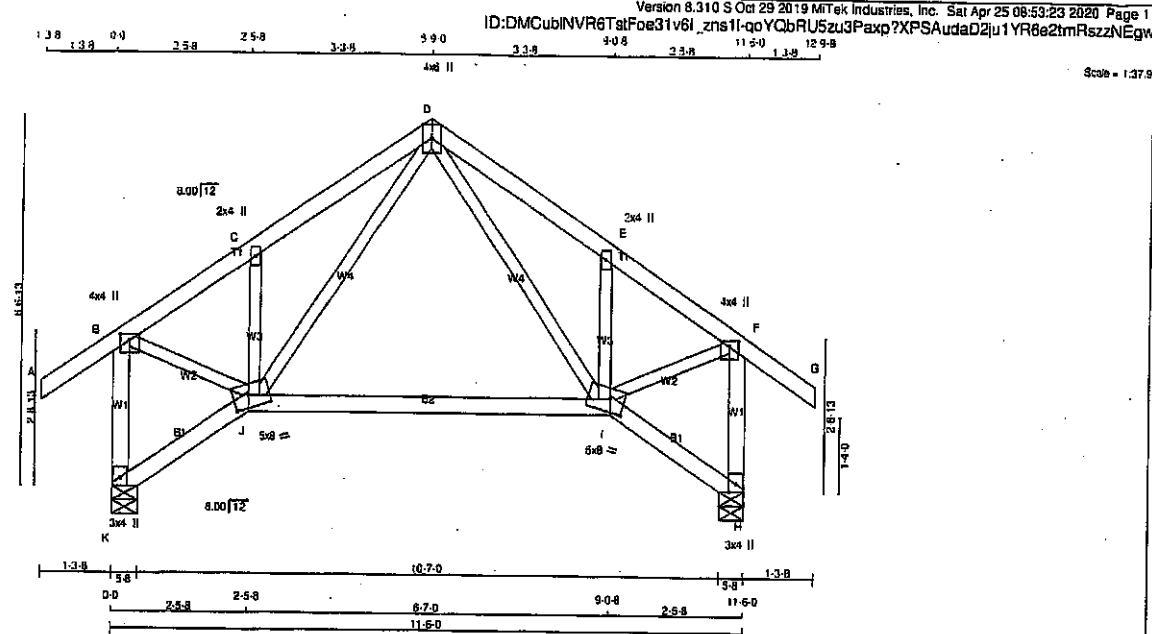
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.41 (C) (INPUT = 0.90)
JSI METAL = 0.12 (A) (INPUT = 1.00)



Structural component only
DWG# T-2006963



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
K - B	2x4	DRY No.2	SPF
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
K - J	2x4	DRY No.2	SPF
J - I	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0	1.25	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW+w	MT20	4.0	8.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0	Edge	
I	BBWW-m	MT20	5.0	8.0	2.75	3.25
J	BBWW-m	MT20	5.0	8.0	2.75	3.25
K	BMV1+p	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
K	760	0	760	0	5-8	5-8
H	760	0	760	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
K	535	384 / 0	0 / 0	0 / 0	0 / 0	171 / 0	0 / 0
H	535	384 / 0	0 / 0	0 / 0	0 / 0	171 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL FITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO			FR-TO				
K-B	-737 / 0	0.0	0.0	0.12 (1)	7.81	D-I	0 / 241		
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	I-E	-328 / 0		
B-C	-562 / 0	-91.8	-91.8	0.09 (1)	6.25	I-F	0 / 527		
C-D	-589 / 0	-91.8	-91.8	0.11 (1)	6.25	J-D	0 / 241		
D-E	-589 / 0	-91.8	-91.8	0.11 (1)	6.25	J-C	-325 / 0		
E-F	-562 / 0	-91.8	-91.8	0.09 (1)	6.25	B-J	0 / 527		
F-G	0 / 35	-91.8	-91.8	0.12 (1)	10.00				
H-F	-737 / 0	0.0	0.0	0.12 (1)	7.81				
K-J	0 / 0	-18.5	-18.5	0.03 (4)	10.00				
J-I	0 / 352	-18.5	-18.5	0.26 (4)	10.00				
I-H	0 / 0	-18.5	-18.5	0.03 (4)	10.00				

TOTAL WEIGHT = 3 X 57 = 170 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN LC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.12/1.00 (A-B:1), BC=0.26/1.00 (I-J:4), WB=0.12/1.00 (B-J:1), SS=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

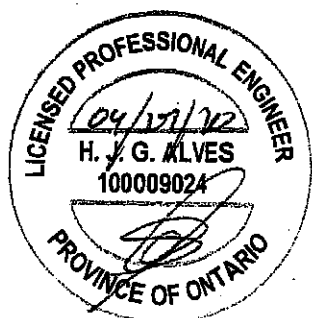
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

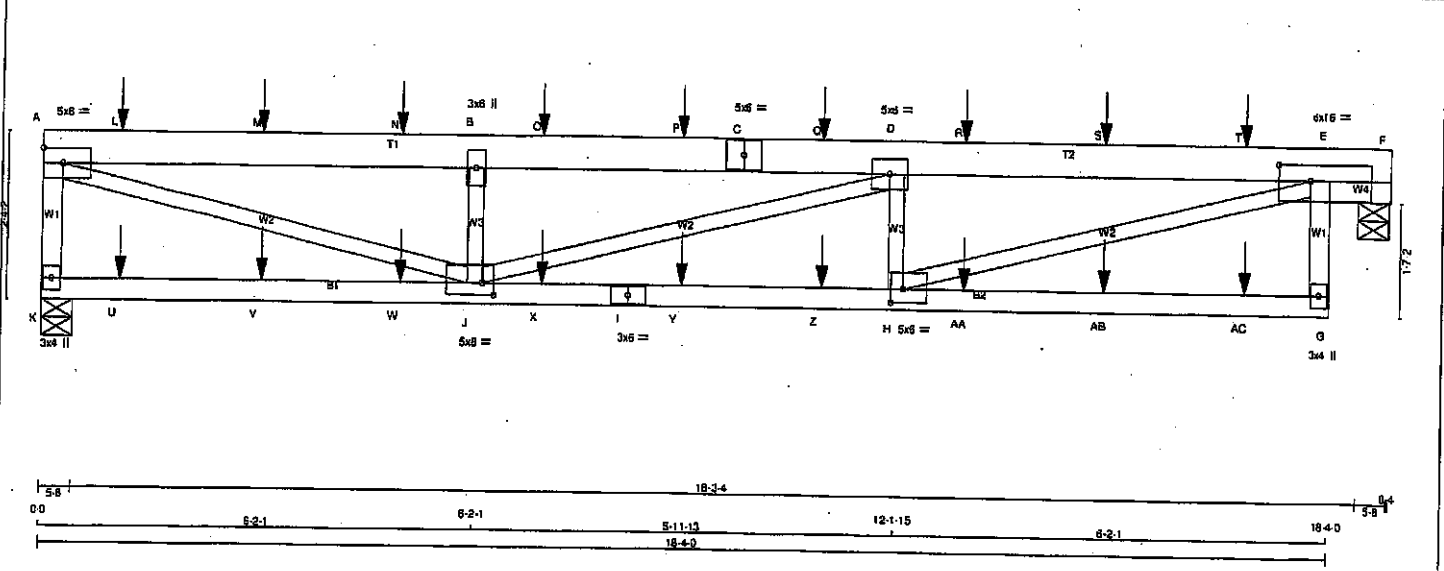
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1668

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.54 (F) (INPUT = 0.90)
JSI METAL= 0.24 (K) (INPUT = 1.00)





LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 2 X 77 = 154 lb																																	
N. L. G. A. RULES				BEARINGS										DESIGN CRITERIA																																	
CHORDS SIZE				LUMBER				DESCR.				FACTORED				MAXIMUM FACTORED				INPUT				REQRD																							
K - A				2x4				DRY				No.2				SPF				GROSS REACTION				BRG				BRG																			
A - C				2x6				DRY				No.2				SPF				JT				VERT				HORZ				UPLIFT				IN-SX				IN-SX							
C - F				2x6				DRY				No.2				SPF				K				2110				0				5-8				5-8											
E - F				2x4				DRY				No.2				SPF				F				1995				0				1995				0				5-8				5-8			
G - E				2x4				DRY				No.2				SPF																															
K - I				2x4				DRY				No.2				SPF																															
I - G				2x4				DRY				No.2				SPF																															
ALL WEBS				2x3				DRY				No.2				SPF																															
DRY: SEASONED LUMBER																																															

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF)

TOP CHORDS : (0.122"x3") SPIRAL NAILS

K-A 1 12 TOP

E-G 1 12 TOP

A-C 2 12 SIDE(0.0)

C-F 2 12 SIDE(0.0)

BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS

K-I 1 12 SIDE(0.0)

I-G 1 12 SIDE(0.0)

WEBS : (0.122"x3") SPIRAL NAILS

2x3 1 8

2x4 1 6

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.04 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
K-A	-1971 / 0	0.0 0.0 0.12 (1)	7.81	H-E	0 / 5502	0.68 (1)	
A-L	-4759 / 0	-91.8 -91.8 0.30 (1)	5.00	A-J	0 / 4985	0.62 (1)	
L-M	-4759 / 0	-91.8 -91.8 0.30 (1)	5.00	H-D	-1323 / 0	0.11 (1)	
M-N	-4759 / 0	-91.8 -91.8 0.30 (1)	5.00	J-B	-895 / 0	0.08 (1)	
N-B	-4759 / 0	-91.8 -91.8 0.30 (1)	5.00	J-D	-508 / 0	0.18 (1)	
B-O	-4759 / 0	-91.8 -91.8 0.33 (1)	4.95				
O-P	-4759 / 0	-91.8 -91.8 0.33 (1)	4.95				
P-C	-4759 / 0	-91.8 -91.8 0.33 (1)	4.95				
C-Q	-4759 / 0	-91.8 -91.8 0.33 (1)	4.95				
Q-D	-4759 / 0	-91.8 -91.8 0.33 (1)	4.95				
D-R	-5242 / 0	-91.8 -91.8 0.72 (1)	4.04				
R-S	-5242 / 0	-91.8 -91.8 0.72 (1)	4.04				
S-T	-5242 / 0	-91.8 -91.8 0.72 (1)	4.04				
T-E	-5242 / 0	-91.8 -91.8 0.72 (1)	4.04				
E-F	-788 / 0	-91.8 -91.8 0.38 (1)	6.25				
G-E	0 / 140	0.0 0.0 0.07 (4)	10.00				
K-U	0 / 0	-18.5 -18.5 0.16 (4)	10.00				
U-V	0 / 0	-18.5 -18.5 0.16 (4)	10.00				
V-W	0 / 0	-18.5 -18.5 0.16 (4)	10.00				
W-J	0 / 0	-18.5 -18.5 0.16 (4)	10.00				
J-X	0 / 5242	-18.5 -18.5 0.58 (1)	10.00				
X-I	0 / 5242	-18.5 -18.5 0.58 (1)	10.00				
I-Y	0 / 5242	-18.5 -18.5 0.58 (1)	10.00				
Y-Z	0 / 5242	-18.5 -18.5 0.58 (1)	10.00				
Z-H	0 / 5242	-18.5 -18.5 0.58 (1)	10.00				
H-AA	0 / 0	-18.5 -18.5 0.16 (4)	10.00				
AA-AB	0 / 0	-18.5 -18.5 0.16 (4)	10.00				
AB-AC	0 / 0	-18.5 -18.5 0.16 (4)	10.00				
AC-G	0 / 0	-18.5 -18.5 0.16 (4)	10.00				

STRUCTURAL COMPONENT ONLY
DWG# T-2006965 1/2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLES WITH:

- PART 9 OF NBC 2010, NBC 2012, ABC 2019

- PART 9 OF NBC 2012 (2019 AMENDMENT)

- CSA 086-09, CSA 086-14

- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.61")

CALCULATED VERT. DEFL.(LL) = L/999 (0.17")

ALLOWABLE DEFL.(TL) = L/360 (0.61")

CALCULATED VERT. DEFL.(TL) = L/704 (0.31")

CSI: TC=0.72/1.00 (D-E:1), GC=0.58/1.00 (H-J:1), WB=0.68/1.00 (E-H:1), SS=0.37/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1867 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (H) (INPUT = 0.90)

JSI METAL = 0.85 (H) (INPUT = 1.00)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
L	1-1-7	-151	-151	---	BACK	VERT	TOTAL	---	C1
M	3-1-7	-151	-151	---	BACK	VERT	TOTAL	---	C1
N	5-1-7	-151	-151	---	BACK	VERT	TOTAL	---	C1
O	7-1-7	-151	-151	---	BACK	VERT	TOTAL	---	C1
P	9-1-7	-151	-151	---	BACK	VERT	TOTAL	---	C1
Q	11-1-7	-151	-151	---	BACK	VERT	TOTAL	---	C1
R	13-1-7	-151	-151	---	BACK	VERT	TOTAL	---	C1

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408087	T12	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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ID:DMCubINVR6TslFos31v6l zns1H 6oonUik8BGC50856zPRq7EX79FHIZnGXV OPzNEqv

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0		Edge
B	TMVW-w	MT20	3.0	6.0		
C	TS-t	MT20	5.0	6.0		
D	TMVW-t	MT20	5.0	6.0		
E	TMVWW-t	MT20	6.0	16.0	2.75	5.50
G	BMV+p	MT20	3.0	4.0		
H	BMVW-t	MT20	5.0	6.0	2.25	2.00
I	BS-t	MT20	3.0	6.0		
J	BMVWW-t	MT20	5.0	8.0	2.00	2.00
K	BMV1+p	MT20	3.0	4.0		

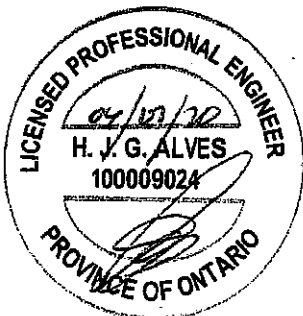
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
S	15-1-7	-151	-151	---	BACK	VERT	TOTAL	---	C1
T	17-1-7	-151	-151	---	BACK	VERT	TOTAL	---	C1
U	1-1-7	-72	-72	---	BACK	VERT	TOTAL	---	C1
V	3-1-7	-72	-72	---	BACK	VERT	TOTAL	---	C1
W	5-1-7	-72	-72	---	BACK	VERT	TOTAL	---	C1
X	7-1-7	-72	-72	---	BACK	VERT	TOTAL	---	C1
Y	9-1-7	-72	-72	---	BACK	VERT	TOTAL	---	C1
Z	11-1-7	-72	-72	---	BACK	VERT	TOTAL	---	C1
AA	13-1-7	-72	-72	---	BACK	VERT	TOTAL	---	C1
AB	15-1-7	-72	-72	---	BACK	VERT	TOTAL	---	C1
AC	17-1-7	-72	-72	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

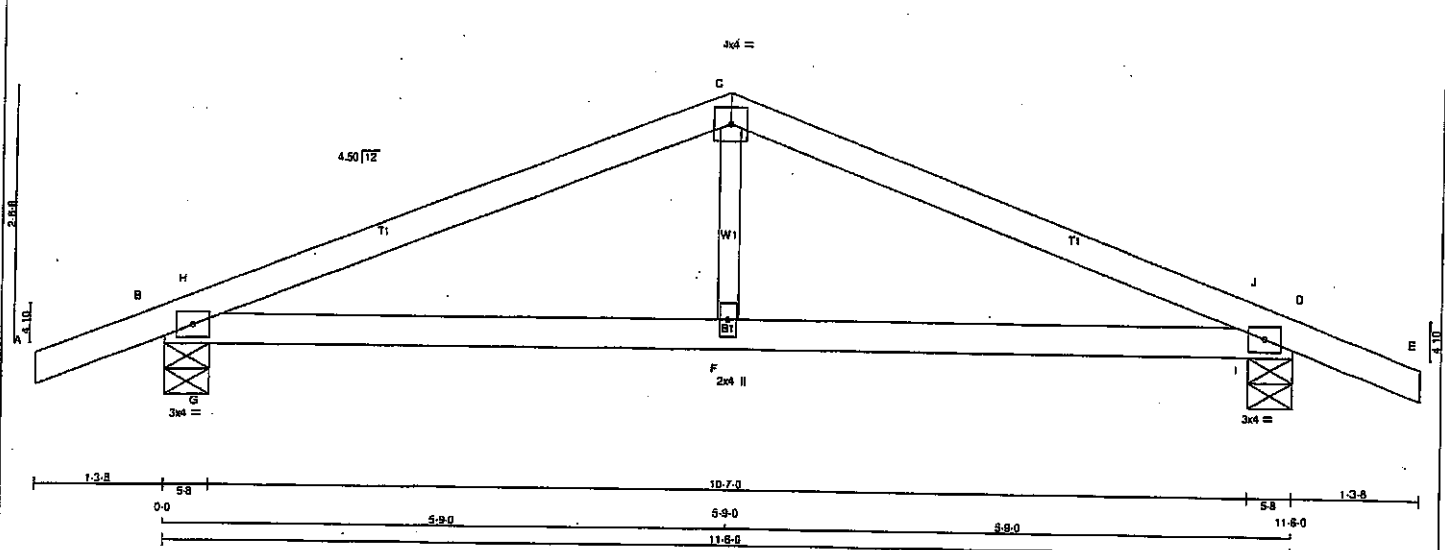
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2006965 2/2

JOB NAME 408087	TRUSS NAME T13	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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 ID:DMCubINVR6TstFoe31v6l_zns1H_6conUjk8BGC5OB56zPrq7K57CEHVOnGXV_OPzNEgv
 1.38 0.0 5.90 5.90 11.80 1.38 12.98
 Scale = 1/2" = 1'-0"



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE			LUMBER
A - C	2x4	DRY		No.2
C - E	2x4	DRY		No.2
B - D	2x4	DRY		No.2
ALL WEBS		2x3	DRY	No.2
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0			
C	TTW-p	MT20	4.0	4.0			
D	TMB1-I	MT20	3.0	4.0			
F	BMW-w	MT20	2.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
B	757	0	757	0	0	5-8	5-8	5-8	5-8
D	757	0	757	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS							
	1ST LCASE	MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	533	363 / 0	0 / 0	0 / 0	0 / 0	170 / 0	0 / 0
D	533	363 / 0	0 / 0	0 / 0	0 / 0	170 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.89 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS						
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)			
FR-TO		FROM	TO		FR-TO					
A-B	0 / 18	-91.8	-91.8	0.12 (1)	10.00	F-C	0 / 226 0.05 (4)			
B-H	-858 / 0	-91.8	-91.8	0.15 (1)	8.25	G-H	-435 / 0 0.00 (1)			
H-C	-952 / 0	-91.8	-91.8	0.36 (1)	5.89	I-J	-435 / 0 0.00 (1)			
C-J	-952 / 0	-91.8	-91.8	0.36 (1)	5.89					
J-D	-858 / 0	-91.8	-91.8	0.15 (1)	6.25					
D-E	0 / 18	-91.8	-91.8	0.12 (1)	10.00					
B-G	0 / 883	-18.5	-18.5	0.39 (1)	10.00					
G-F	0 / 883	-18.5	-18.5	0.39 (1)	10.00					
F-I	0 / 883	-18.5	-18.5	0.39 (1)	10.00					
I-D	0 / 883	-18.5	-18.5	0.39 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 8.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C
 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, OBC 2012, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.38")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
 ALLOWABLE DEFL.(TL) = L/380 (0.38")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CS1: TC=0.38/1.00 (C-H:1), BC=0.39/1.00 (B-G:1), WB=0.05/1.00 (C-F:4), SS=0.33/1.00 (D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (FSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.71 (D) (INPUT = 0.90)
 JSI METAL= 0.34 (D) (INPUT = 1.00)

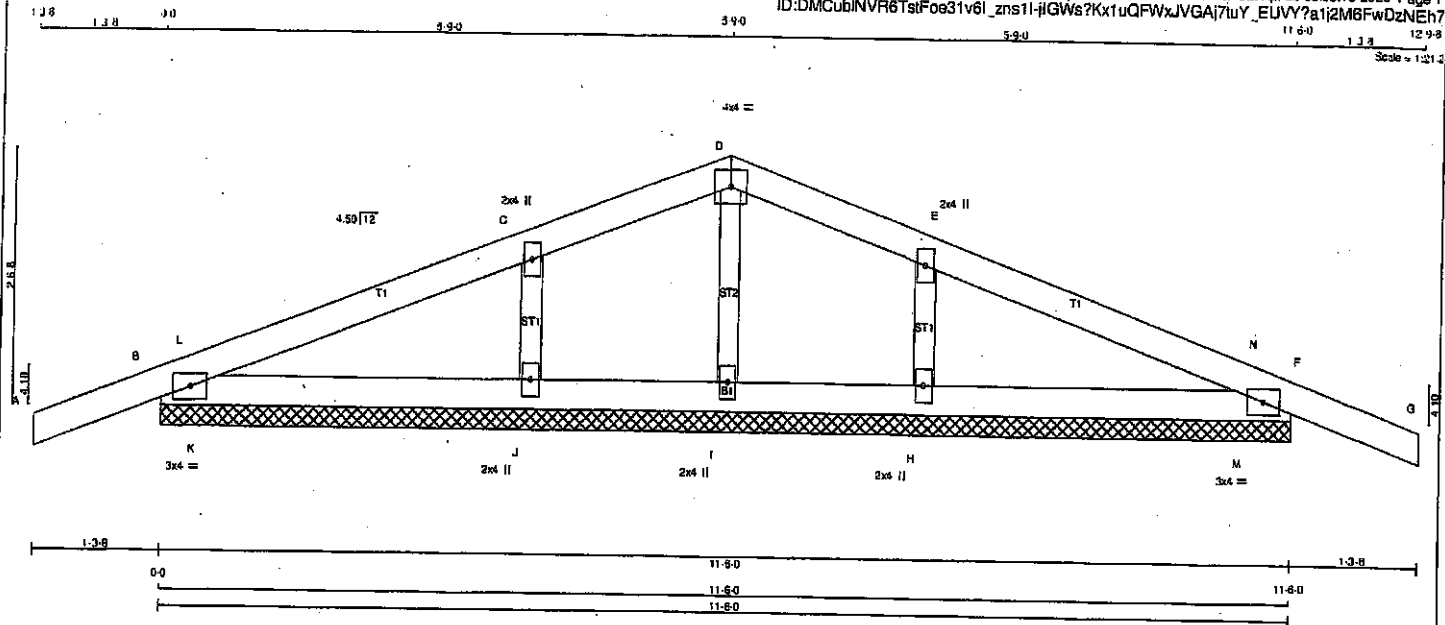


Structural component only
 DWG# T-2006966

JOB NAME 408087	TRUSS NAME G13	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
A - D	2x4	DRY	No.2		SPF
D - G	2x4	DRY	No.2		SPF
B - F	2x4	DRY	No.2		SPF
ALL WEBS		2x3	DRY	No.2	SPF
ALL GABLE WEBS		2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.					

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW-w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TMW-w	MT20	2.0	4.0		
F	TMB1-I	MT20	3.0	4.0		
H, I, J						
H	BNW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LCI (L/C)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LCI (L/C)	UNBRACED LENGTH (FT)
FR-TO		FROM TO		FR-TO			
A-B	0 / 18	-91.8 -91.8	0.12 (1)	10.00	I-D	-47 / 3	0.01 (1)
B-L	-80 / 0	-91.8 -91.8	0.02 (4)	6.25	J-C	-314 / 0	0.05 (1)
L-C	-71 / 0	-91.8 -91.8	0.13 (1)	6.25	H-E	-314 / 0	0.05 (1)
C-D	-85 / 0	-91.8 -91.8	0.13 (1)	6.25	K-L	-142 / 8	0.00 (1)
D-E	-85 / 0	-91.8 -91.8	0.13 (1)	6.25	M-N	-142 / 8	0.00 (1)
E-N	-71 / 0	-91.8 -91.8	0.13 (1)	6.25			
N-F	-80 / 0	-91.8 -91.8	0.02 (4)	6.25			
F-G	0 / 18	-91.8 -91.8	0.12 (1)	10.00			
B-K	0 / 75	-18.5 -18.5	0.10 (1)	10.00			
K-J	0 / 75	-18.5 -18.5	0.10 (1)	10.00			
J-I	0 / 64	-18.5 -18.5	0.07 (1)	10.00			
I-H	0 / 64	-18.5 -18.5	0.07 (1)	10.00			
H-M	0 / 75	-18.5 -18.5	0.10 (1)	10.00			
M-F	0 / 75	-18.5 -18.5	0.10 (1)	10.00			

TOTAL WEIGHT = 34 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (E-N:1), BC=0.10/1.00 (H-M:1), WB=0.05/1.00 (E-H:1), SS=0.13/1.00 (E-N:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

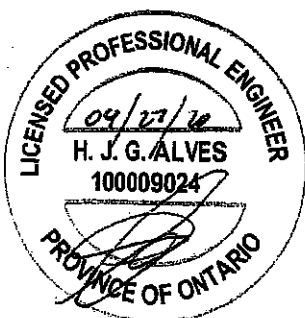
PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618	354
	1667	798
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

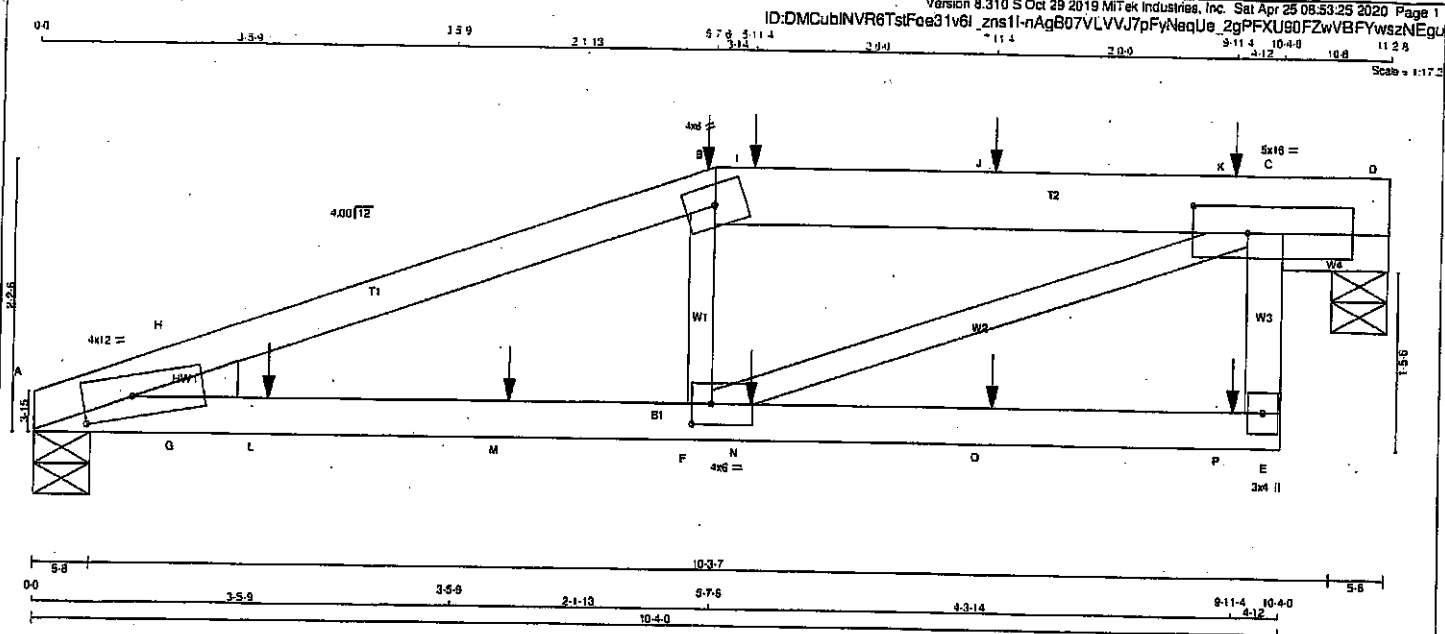
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.50)

JSI METAL= 0.10 (C) (INPUT = 1.00)



Structural component only
DWG# T-2006948



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - B	2x4	DRY No.2
B - D	2x6	DRY No.2
C - D	2x4	DRY No.2
E - C	2x4	DRY No.2
A - E	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-m	MT20	4.0	12.0	2.00	5.00
B	TTW-h	MT20	4.0	6.0		
C	TMVWV-t	MT20	5.0	16.0	2.50	5.50
E	BMV-p	MT20	3.0	4.0		
F	BMVW-t	MT20	4.0	6.0	2.00	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	MAXIMUM FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	HEEL WEDGE
D	1050	0	1050	0	5-8	5-8	2x4 L
A	953	0	953	0	5-8	5-8	2x4 L

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN COMPONENT REACTIONS	DEAD	SOIL
D	739	506/0	0/0	0/0
A	672	453/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, A

BRACING
TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED (LBS)	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED (LBS)
FR-TO				FR-TO			
A-H	-2013/0	-91.8	-91.8	F-B	-409/0	0.07(1)	0.07(1)
H-B	-1978/0	-91.8	-91.8	F-C	0/2038	0.50(1)	0.50(1)
B-I	-1905/0	-91.8	-91.8	G-H	-146/10	0.00(1)	0.00(1)
I-J	-1905/0	-91.8	-91.8				
J-K	-1905/0	-91.8	-91.8				
K-C	-1905/0	-91.8	-91.8				
C-D	0/0	-91.8	-91.8				
E-C	0/95	0.0	0.0				
A-G	0/1883	-18.5	-18.5				
G-L	0/1883	-18.5	-18.5				
L-M	0/1883	-18.5	-18.5				
M-F	0/1883	-18.5	-18.5				
F-N	0/0	-18.5	-18.5				
N-O	0/0	-18.5	-18.5				
O-P	0/0	-18.5	-18.5				
P-E	0/0	-18.5	-18.5				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	5-7-6	-45	-50		FRONT	VERT	DEAD		C1
B	5-7-6	-237	-237		FRONT	VERT	SNOW		C1
I	5-11-4	-103	-103		BACK	VERT	TOTAL		C1
J	7-11-4	-72	-72		BACK	VERT	TOTAL		C1
K	9-11-4	-72	-72		BACK	VERT	TOTAL		C1
L	1-11-4	-50	-50		BACK	VERT	TOTAL		C1
M	3-11-4	-50	-50		BACK	VERT	TOTAL		C1
N	5-11-4	-50	-50		BACK	VERT	TOTAL		C1
O	7-11-4	-50	-50		BACK	VERT	TOTAL		C1
P	9-11-4	-55	-55		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPC 2011, TPC 2014

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/677 (0.18")

CSI: TC=0.72/1.00 (B-C:1), BC=0.87/1.00 (F-G:1), WB=0.50/1.00 (C-F:1), SS=0.43/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

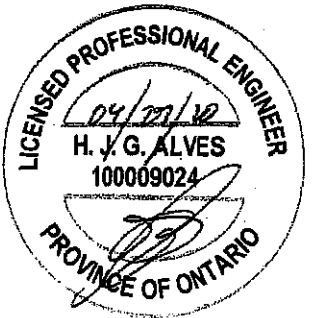
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.85 (P) (INPUT = 0.90)
JSI METAL = 0.47 (P) (INPUT = 1.00)

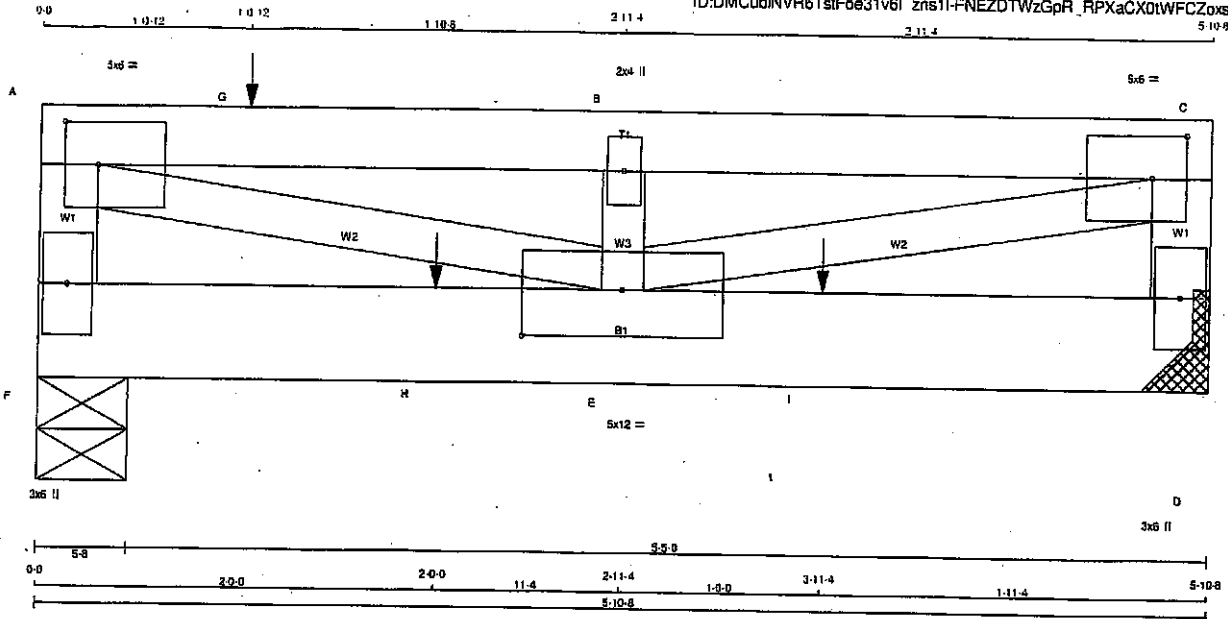


Structural component only
DWG# T-2006967

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408087	T15	1	2	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Sat Apr 25 08:53:26 2020 Page 1
ID:DMCubINVR6TstFoe31v6l zns1l-FNEZDTWzGpR_RPXaCX0tWFCZoxsllt4kr_5T1zNEQl



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	Lumber	DESCR
F - A	2x4	DRY	No.2	SPF		
A - C	2x4	DRY	No.2	SPF		
D - C	2x4	DRY	No.2	SPF		
F - D	2x6	DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #/ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
F - A	12	TOP
A - C	12	SIDE(0.0)
C - D	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE(14.0)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	5.0	6.0	2.50	2.00
B	TMVW-1	MT20	2.0	4.0		
C	TMVW-1	MT20	5.0	6.0	2.50	2.00
D	BMV1-p	MT20	3.0	6.0		
E	BMVWV-1	MT20	5.0	12.0	2.75	6.00
F	BMV1-p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	3277	0	3277	0
D	2358	0	2358	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4'-0".

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	2323	1495/0	0/0	0/0	0/0	828/0	0/0
D	1673	1070/0	0/0	0/0	0/0	602/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)	MAX. UNBRAC LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH
FR-TO						FR-TO			
F-A	-2531/0	0.0	0.0	0.14 (1)	7.12	A-E	0/5214	0.65 (1)	
A-G	-4858/0	-91.8	-91.8	0.80 (1)	3.00	E-B	-799/0	0.06 (1)	
G-B	-4858/0	-91.8	-91.8	0.80 (1)	3.00	B-C	0/5214	0.85 (1)	
B-C	-4858/0	-91.8	-91.8	0.16 (1)	4.22				
D-C	-1717/0	0.0	0.0	0.09 (1)	7.81				

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)	MAX. UNBRAC LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH
FR-TO						FR-TO			
F-H	0/0	-43.5	-43.5	0.52 (1)	10.00				
H-E	0/0	-43.5	-43.5	0.52 (1)	10.00				
E-I	0/0	-43.5	-43.5	0.43 (1)	10.00				
I-D	0/0	-43.5	-43.5	0.43 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LCC	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-0-12	-1275	-1275		BACK	VERT	TOTAL		C1
H	2-0-0	-2000	-2000		BACK	VERT	TOTAL		C1
I	3-11-4	-1568	-1568		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 23 = 47 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2018 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/822 (0.09")

CSI: TC=0.80/1.00 (A-B:1), BC=0.52/1.00 (E-F:1), WB=0.65/1.00 (A-E:1), SS=0.46/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

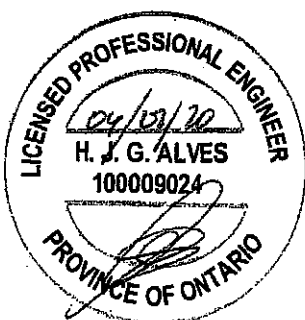
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
	1667	788
	1987	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.87 (A) (INPUT = 0.90)
JSI METAL = 0.62 (C) (INPUT = 1.00)

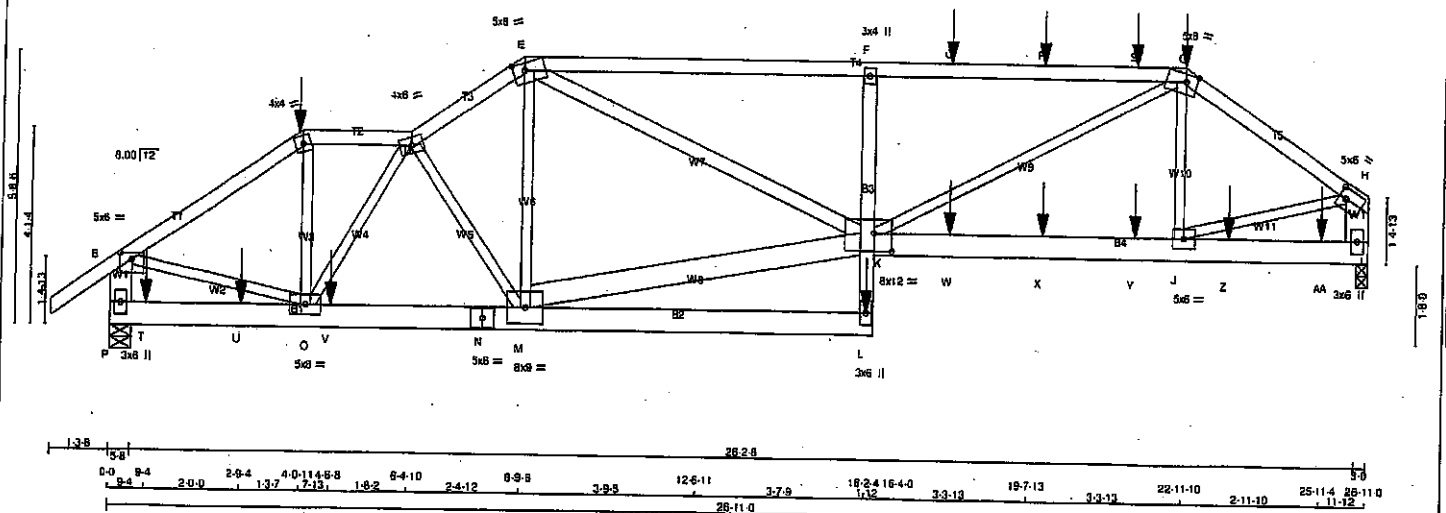


Structural component only
DWG# T-2006968

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Sat Apr 25 09:03:31 2020 Page 1
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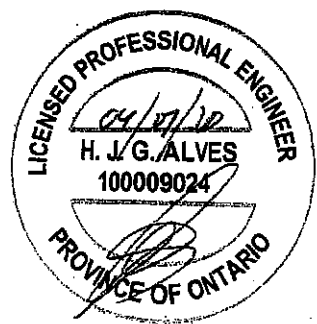


DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

NAILS TO BE DRIVEN FROM ONE SIDE ONLY

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAIL S.



Structural component only
DWG# T-2006976

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

UNFACTORED REACTIONS

1ST LCASE MAX MIN. COMPONENT REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P. 1

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS				FACTORED				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. FACTORED MAX CSI (LC)				
FR-TO		FROM TO		LENGTH	FR-TO						
A-B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	C-C	0 / 1291				
B-C	-3385 / 0	-91.8	-91.8	0.07 (1)	4.90	D-M	-1271 / 0				
C-D	-2689 / 0	-91.8	-91.8	0.07 (1)	5.38	M-E	0 / 690				
D-E	-3303 / 0	-91.8	-91.8	0.09 (1)	5.06	N-F	0 / 2773				
E-F	-4304 / 0	-91.8	-91.8	0.70 (1)	3.93	E-K	0 / 1729				
F-Q	-4327 / 0	-91.8	-91.8	0.86 (1)	3.92	K-G	0 / 2508				
Q-R	-4327 / 0	-91.8	-91.8	0.86 (1)	3.82	J-G	-368 / 0				
R-S	-4327 / 0	-91.8	-91.8	0.86 (1)	3.92	G-D	0 / 2889				
S-G	-4327 / 0	-91.8	-91.8	0.86 (1)	3.92	J-H	0 / 2191				
P-H	-2562 / 0	-91.8	-91.8	0.17 (1)	7.81	O-D	-1142 / 0				
G-B	-2851 / 0	0.0	0.0	0.10 (1)	7.81						
-H	-2131 / 0	0.0	0.0	0.08 (1)	7.81						
P-T	0 / 0	-18.5	-18.5	0.03 (1)	10.00						
T-U	0 / 0	-18.5	-18.5	0.03 (1)	10.00						
U-O	0 / 0	-18.5	-18.5	0.03 (1)	10.00						
O-V	0 / 3445	-18.5	-18.5	0.35 (1)	10.00						
V-N	0 / 3445	-16.5	-18.5	0.35 (1)	10.00						
N-M	0 / 3445	-18.5	-18.5	0.35 (1)	10.00						
M-L	0 / 49	-18.5	-18.5	0.06 (4)	10.00						
L-K	0 / 425	0.0	0.0	0.10 (1)	10.00						
K-F	-853 / 0	0.0	0.0	0.08 (1)	7.81						
K-W	0 / 2117	-18.5	-18.5	0.17 (1)	10.00						
W-X	0 / 2117	-18.5	-18.5	0.17 (1)	10.00						
X-Y	0 / 2117	-18.5	-18.5	0.17 (1)	10.00						
Y-J	0 / 2117	-18.5	-18.5	0.17 (1)	10.00						
Z	0 / 0	-18.5	-18.5	0.08 (4)	10.00						
-AA	0 / 0	-18.5	-18.5	0.06 (4)	10.00						
A-I	0 / 0	-18.5	-18.5	0.06 (4)	10.00						

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC	MAX	MAX	FACE	DIR.	TYPE	HEEL	CONN
C	4-0-11	-49	-55	---	FRONT	VERT	DEAD	---	C1
C	3-0-11	-254	-254	---	FRONT	VERT	SNOW	---	C1
G	22-11-10	-21	-24	---	FRONT	VERT	DEAD	---	C1
G	22-11-10	-105	-105	---	FRONT	VERT	SNOW	---	C1
L	16-2-4	-375	-375	---	FRONT	VERT	TOTAL	---	C1
Q	17-11-4	-40	-40	---	FRONT	VERT	TOTAL	---	C1
R	19-11-4	-40	-40	---	FRONT	VERT	TOTAL	---	C1
S	21-11-4	-40	-40	---	FRONT	VERT	TOTAL	---	C1
T	9-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
U	2-9-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
V	4-8-8	-953	-953	---	FRONT	VERT	TOTAL	---	C1

TOTAL WEIGHT = 2 X 142 = 285 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY
USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A
SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL
LOAD CASES.

**THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015**

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.70/1.00 (E-F:1), BC=0.35/1.00 (M-O:1),
WB=0.36/1.00 (B-O:1), SS=0.29/1.00 (M-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP = 0.88 (H) (INPUT = 0.90)
ISI METAL = 0.34 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408088	T20	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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ID:DMCubINVR6TstFoe31v6I zns1I-1mIHCKrTnhf6WNHr85MIXrm bpOu?Mu7sTZcCkzNEXC

PLATES (table is in inches)

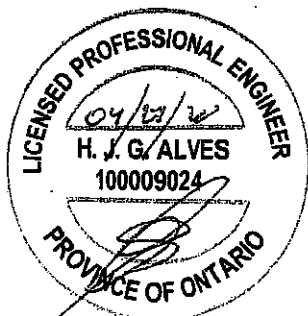
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.00
C	TTWW-m	MT20	4.0	4.0		
D	TTWW-m	MT20	4.0	6.0		
E	TTWW-m	MT20	5.0	8.0	2.00	2.75
F	TMV-p	MT20	3.0	4.0		
G	TTWW-m	MT20	5.0	8.0	2.00	3.00
H	TMVW-t	MT20	5.0	8.0	2.50	1.75
I	BMV1-p	MT20	3.0	6.0		
J	BMVW-t	MT20	5.0	6.0		
K	BMVWW-t	MT20	8.0	12.0	4.50	4.50
L	BMV-p	MT20	3.0	6.0		
M	BMVWW-t	MT20	8.0	9.0		
N	BS-t	MT20	5.0	6.0		
O	BMVWW-t	MT20	5.0	8.0		
P	BMV1-p	MT20	3.0	6.0		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
W	17-11-4	-8	-8	---	FRONT	VERT	TOTAL	---	C1
X	19-11-4	-8	-8	---	FRONT	VERT	TOTAL	---	C1
Y	21-11-4	-8	-8	---	FRONT	VERT	TOTAL	---	C1
Z	23-11-4	-8	-8	---	FRONT	VERT	TOTAL	---	C1
AA	25-11-4	-8	-8	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

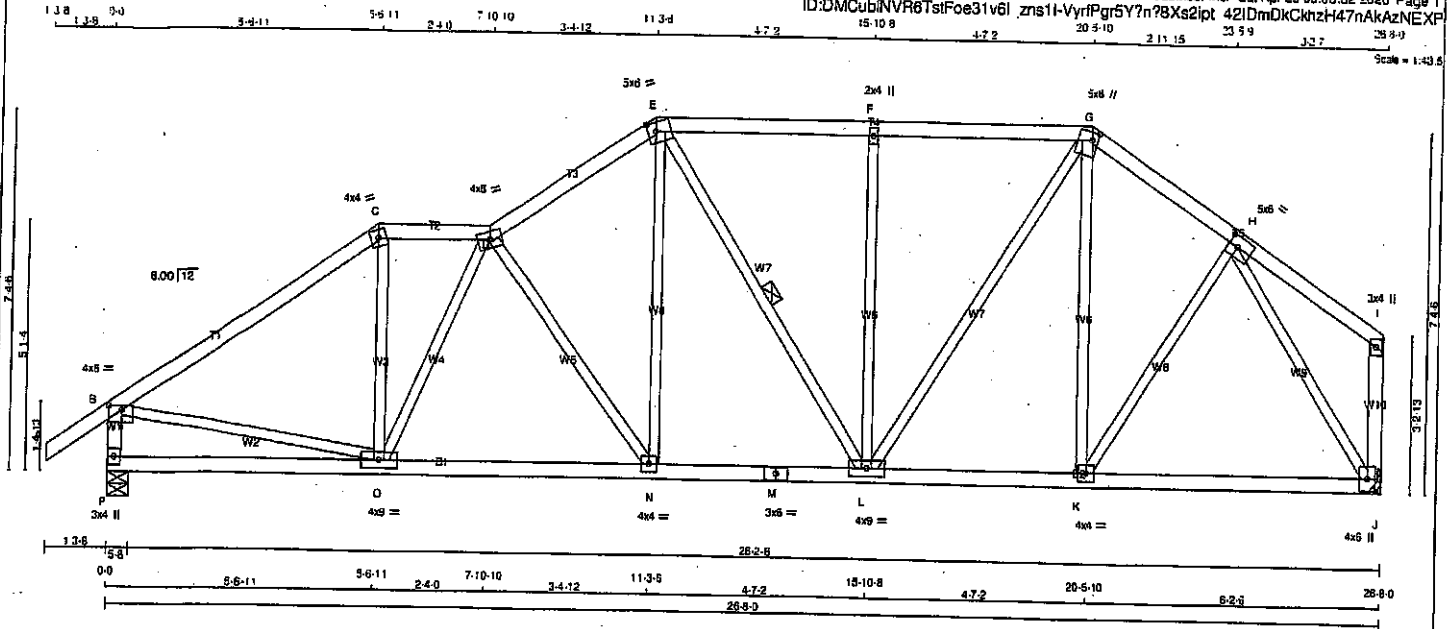


Structural component only
DWG# T-2006976

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408088	T21	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
K - L	2x4	DRY	No.2	SPF
L - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TTW-m	MT20	5.0	6.0	2.25	2.00
F	TTW-m	MT20	2.0	4.0		
G	TTW-m	MT20	5.0	6.0	2.00	1.50
H	TTW-m	MT20	5.0	6.0	2.50	2.25
I	TMV-p	MT20	3.0	4.0		
J	BMVW1-p	MT20	4.0	6.0		
K	BMVW1	MT20	4.0	4.0		
L	BMVW1	MT20	4.0	6.0		
M	SS-1	MT20	3.0	6.0		
N	BMVW1	MT20	4.0	4.0		
O	BMVW1	MT20	4.0	6.0		
P	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
P	1596	0	0	5-8
J	1470	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
P	1128	752 / 0	0 / 0	0 / 0	0 / 0	374 / 0	0 / 0
J	1040	662 / 0	0 / 0	0 / 0	0 / 0	357 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 35	-91.8 -91.8	0.12 (1)	10.00	O-C	0 / 552	0.12 (1)
B-C	-1842 / 0	-91.8 -91.8	0.42 (1)	4.72	D-N	-585 / 0	0.35 (1)
C-D	-1383 / 0	-91.8 -91.8	0.08 (1)	5.46	N-E	0 / 567	0.13 (1)
D-E	-1625 / 0	-91.8 -91.8	0.17 (1)	5.05	E-L	-9 / 15	0.00 (4)
E-F	-1368 / 0	-91.8 -91.8	0.26 (1)	5.27	L-F	-512 / 0	0.49 (1)
F-G	-1369 / 0	-91.8 -91.8	0.26 (1)	5.27	L-G	0 / 675	0.15 (1)
G-H	-1226 / 0	-91.8 -91.8	0.12 (1)	5.68	K-G	-170 / 6	0.18 (1)
H-I	0 / 17	-91.8 -91.8	0.13 (1)	10.00	K-H	0 / 349	0.05 (1)
P-B	-1552 / 0	0.0 0.0	0.16 (1)	6.61	B-O	0 / 1369	0.31 (1)
J-I	-110 / 0	0.0 0.0	0.02 (1)	7.91	H-J	-1544 / 0	0.88 (1)
					O-D	-760 / 0	0.36 (1)
P-O	0 / 0	-18.5 -18.5	0.14 (4)	10.00			
O-N	0 / 1697	-18.5 -18.5	0.34 (1)	10.00			
N-M	0 / 1361	-18.5 -18.5	0.26 (1)	10.00			
M-L	0 / 1381	-18.5 -18.5	0.26 (1)	10.00			
L-K	0 / 1001	-18.5 -18.5	0.25 (1)	10.00			
K-J	0 / 813	-18.5 -18.5	0.22 (4)	10.00			

TOTAL WEIGHT = 126 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. CC

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.89")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.89")
CALCULATED VERT. DEFL.(TL) = L/959 (0.12")

CSI: TC=0.42/1.00 (B-C:1), BC=0.34/1.00 (N-O:1), WB=0.66/1.00 (H-J:1), SS=0.20/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

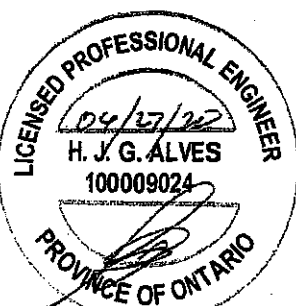
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354
	1657	788
	1987	1656

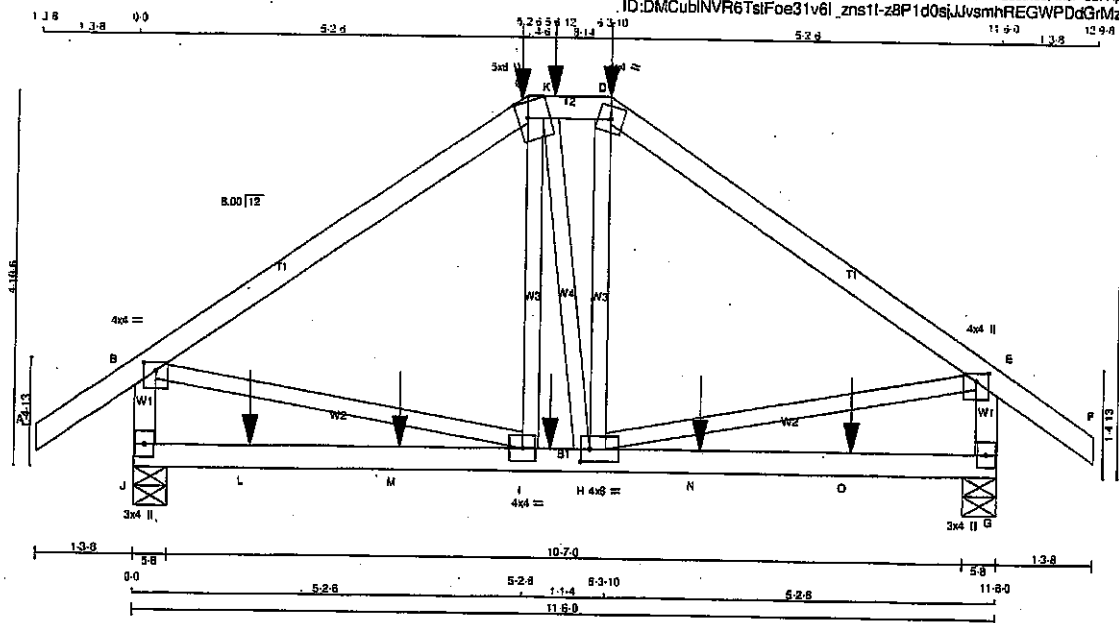
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (H) (INPUT = 0.90)
JSI METAL = 0.44 (M) (INPUT = 1.00)



Structural component only
DWG# T-2008977



TOTAL WEIGHT = 53 lb

LUMBER

N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	5.0	6.0	2.50	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW-p	MT20	4.0	4.0	1.25	2.00
G	BMVt-p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	5.0	2.00	1.50
I	BMVWW-t	MT20	4.0	4.0		
J	BMVt-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
J	1103	0	1103	0
G	1095	0	1095	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
J	776	531 / 0	0 / 0	PERM LIVE WIND DEAD SOIL
G	771	529 / 0	0 / 0	0 / 0 245 / 0 242 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	MAX. FACTORED FORCE (LBS)	WEBS MAX. FACTORED FORCE (LBS)
FR-TO					
A-B	0 / 35	-91.8	-91.8	0.14 (1)	10.00
B-C	-943 / 0	-91.8	-91.8	0.52 (1)	5.57
C-K	-778 / 0	-91.8	-91.8	0.07 (1)	6.25
K-D	-778 / 0	-91.8	-91.8	0.07 (1)	6.25
D-E	-939 / 0	-91.8	-91.8	0.52 (1)	5.58
E-F	0 / 35	-91.8	-91.8	0.14 (1)	10.00
J-B	-1049 / 0	0.0	0.0	0.12 (1)	7.64
G-E	-1042 / 0	0.0	0.0	0.12 (1)	7.67
J-L	0 / 0	-18.5	-18.5	0.19 (4)	10.00
L-M	0 / 0	-18.5	-18.5	0.19 (4)	10.00
M-I	0 / 0	-18.5	-18.5	0.19 (4)	10.00
I-H	0 / 783	-18.5	-18.5	0.27 (4)	10.00
H-N	0 / 0	-18.5	-18.5	0.20 (4)	10.00
N-O	0 / 0	-18.5	-18.5	0.20 (4)	10.00
O-G	0 / 0	-18.5	-18.5	0.20 (4)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX	FACE	DIR.	TYPE	HEEL	CONN.
C	5-2-6	-38	-42	---	FRONT	VERT	DEAD	---	C1
C	5-2-6	-195	-195	---	FRONT	VERT	SNOW	---	C1
D	8-3-10	-38	-42	---	FRONT	VERT	DEAD	---	C1
D	8-3-10	-195	-195	---	FRONT	VERT	SNOW	---	C1
I	5-6-12	-17	-17	---	BACK	VERT	TOTAL	---	C1
K	5-6-12	-127	-127	---	BACK	VERT	TOTAL	---	C1
L	1-6-12	-17	-17	---	BACK	VERT	TOTAL	---	C1
M	3-6-12	-17	-17	---	BACK	VERT	TOTAL	---	C1
N	7-6-12	-17	-17	---	BACK	VERT	TOTAL	---	C1
O	9-6-12	-17	-17	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL	LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.52/1.00 (B-C:1), BC=0.27/1.00 (H-I:4), WB=0.20/1.00 (B-I:1), SSI=0.17/1.00 (B-C:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

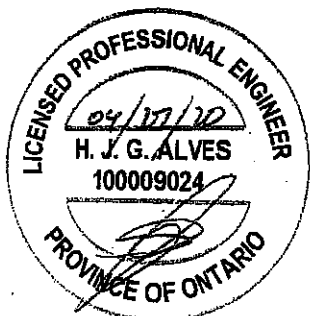
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1957 1650

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) INPUT = 0.90 ;
JSI METAL= 0.31 (B) INPUT = 1.00 ;

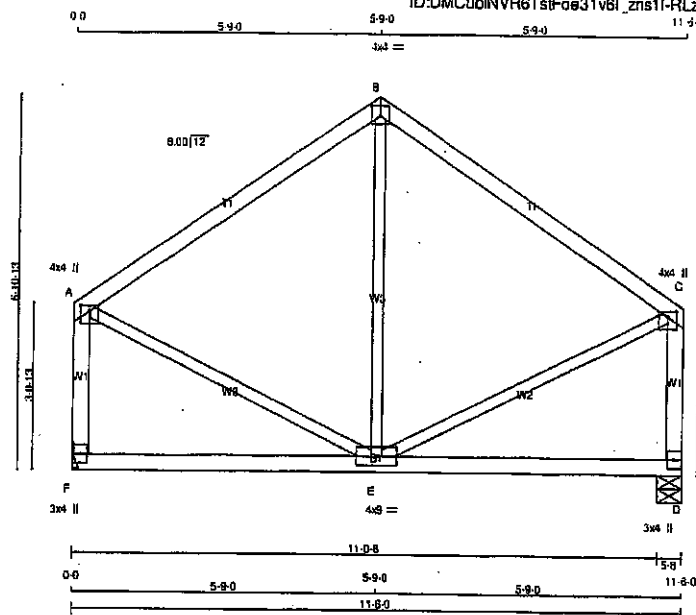


Structural component only
DWG# T-2006978

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408088	T23	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2	
B - C	2x4	DRY	No.2	
F - A	2x4	DRY	No.2	
D - C	2x4	DRY	No.2	
F - D	2x4	DRY	No.2	

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTW-p	MT20	4.0	4.0	2.25	2.00
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMVWW-t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
F	634	0	634	0
D	634	0	634	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
F	448	294 / 0	0 / 0	0 / 0	154 / 0	0 / 0
D	448	294 / 0	0 / 0	0 / 0	154 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LCI MAX	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
A-B	-346 / 0	-91.8	-91.8	0.39 (1)	6.25	E-B	-144 / 46	0.11 (1)	
B-C	-346 / 0	-91.8	-91.8	0.39 (1)	6.25	A-E	0 / 319	0.07 (1)	
F-A	-593 / 0	0.0	0.0	0.10 (1)	7.81	E-C	0 / 319	0.07 (1)	
D-C	-593 / 0	0.0	0.0	0.10 (1)	7.81				
F-E	0 / 0	-18.5	-18.5	0.18 (4)	10.00				
E-D	0 / 0	-18.5	-18.5	0.18 (4)	10.00				

TOTAL WEIGHT = 2 X 50 = 100 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.39/1.00 (B-C:1), BC=0.18/1.00 (D-E:4), WB=0.11/1.00 (B-E:1), SS=0.17/1.00 (B-C:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX	MIN	MAX
MT20	618	354
	1687	788
	1987	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.41 (A) (INPUT = 0.90)

JSI METAL= 0.12 (A) (INPUT = 1.00)

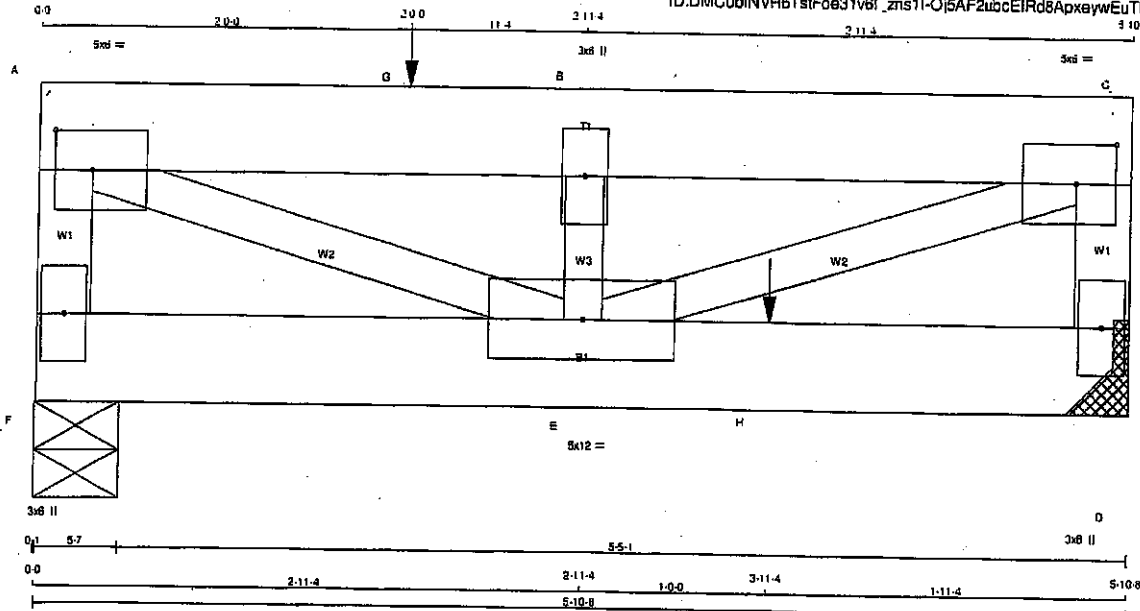


Structural component only
DWG# T-2006979

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408088	T24	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 28 = 55 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A	12	TOP
C - D	12	TOP
A - C	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE(14.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - TMVW-1	MT20	5.0	6.0	2.50	2.50
B - TMVW-1	MT20	3.0	6.0		
C - TMVW-1	MT20	5.0	6.0	2.50	2.50
D - BMV1+p	MT20	3.0	6.0		
E - BMVWW-1	MT20	5.0	12.0		
F - BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	UPLIFT	IN-SX
F 2237	0	2237	5-8 (5-7)
D 2073	0	2073	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LOSE	MAX/MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
F 1588	1010 / 0	0 / 0
D 1471	935 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	MEMB.	FORCE
(LBS)	(PLF)	LC1	UNBRAC	MAX
FR-TO	FROM	TO	LENGTH	FR-TO
F-A	-2132 / 0	0.0	0.0	0.12 (1)
A-G	-3319 / 0	-91.8	-91.8	0.51 (1)
G-B	-3319 / 0	-91.8	-91.8	0.51 (1)
B-C	-3319 / 0	-91.8	-91.8	0.04 (1)
D-C	-1473 / 0	0.0	0.0	0.09 (1)
F-E	0 / 0	-43.5	-43.5	0.05 (1)
E-H	0 / 0	-43.5	-43.5	0.40 (1)
H-D	0 / 0	-43.5	-43.5	0.40 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-0	-2084	-2084	---	TOP	VERT	TOTAL	---	C1
H	3-11-4	-1452	-1452	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6 PSF
	DL =	6.0 PSF
BOT CH.	LL =	0.0 PSF
	DL =	7.4 PSF
TOTAL LOAD	=	38.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/989 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/989 (0.05")

CSI: TC=0.51/1.00 (A-B:1), BC=0.40/1.00 (D-E:1), WB=0.44/1.00 (A-E:1), SBI=0.52/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

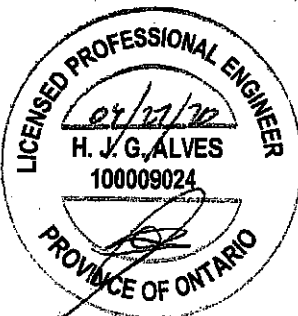
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY): SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 788 1987 1858

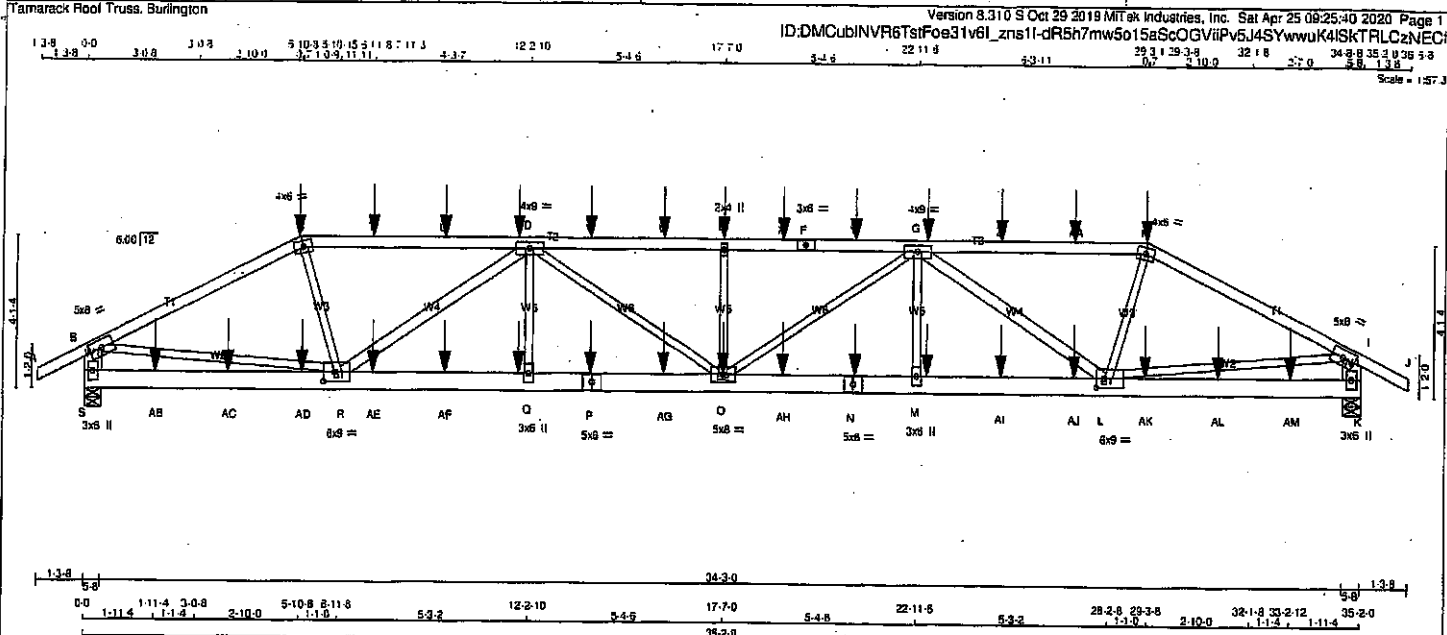
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (A) (INPUT = 0.90)
JSI METAL= 0.11 (C) (INPUT = 1.00)



Structural component only
DWG# T-2006981



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

R - D 2x4 DRY No.2 SPF

G - L 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(0.0)
C-F	12	SIDE(61.0)
F-H	12	SIDE(61.0)
H-J	12	SIDE(0.0)
S-B	2	TOP
K-I	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S-P	2	SIDE(183.1)
P-N	2	SIDE(183.1)
N-K	2	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1	5
2x4	1	5

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT 3321	DOWN 3321	0	0
S	HORIZ 0	HORIZ 0	5-8	5-8
K	UP/LIFT 0	UP/LIFT 0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	2351	1532 / 0	0 / 0	0 / 0	0 / 0	818 / 0	0 / 0	0 / 0
S	2351	1532 / 0	0 / 0	0 / 0	0 / 0	818 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.81 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED MEMB. FORCE (LBS)
FR-TO			
A-B	0 / 28	-91.8	0.07 (1)
B-C	-5188 / 0	-91.8	0.54 (1)
C-T	-5050 / 0	-91.8	0.84 (1)
T-U	-5050 / 0	-91.8	0.64 (1)
U-D	-5050 / 0	-91.8	0.64 (1)
D-V	-7883 / 0	-91.8	0.81 (1)
V-W	-7883 / 0	-91.8	0.81 (1)
W-E	-7883 / 0	-91.8	0.81 (1)
E-X	-7883 / 0	-91.8	0.81 (1)
X-F	-7883 / 0	-91.8	0.81 (1)
F-Y	-7883 / 0	-91.8	0.81 (1)
Y-G	-7883 / 0	-91.8	0.81 (1)
G-Z	-5050 / 0	-91.8	0.64 (1)
Z-AA	-5050 / 0	-91.8	0.64 (1)
AA-H	-5050 / 0	-91.8	0.64 (1)
H-I	-5188 / 0	-91.8	0.54 (1)
I-J	0 / 28	-91.8	0.07 (1)
S-B	-3222 / 0	0.0	0.11 (1)
K-I	-3222 / 0	0.0	0.11 (1)
S-AB	0 / 0	-18.5	0.09 (4)
AB-AC	0 / 0	-18.5	0.09 (4)
AC-AD	0 / 0	-18.5	0.09 (4)
AD-R	0 / 0	-18.5	0.09 (4)
R-AE	0 / 7272	-18.5	0.51 (1)
AE-AF	0 / 7272	-18.5	0.51 (1)
AF-Q	0 / 7272	-18.5	0.51 (1)
Q-P	0 / 7272	-18.5	0.53 (1)
P-AG	0 / 7272	-18.5	0.53 (1)
AG-O	0 / 7272	-18.5	0.53 (1)
O-AH	0 / 7272	-18.5	0.53 (1)
AH-N	0 / 7272	-18.5	0.53 (1)
N-M	0 / 7272	-18.5	0.53 (1)
M-AI	0 / 7272	-18.5	0.51 (1)
AI-AJ	0 / 7272	-18.5	0.51 (1)
AJ-L	0 / 7272	-18.5	0.51 (1)
L-AK	0 / 0	-18.5	0.09 (4)
AK-AL	0 / 0	-18.5	0.09 (4)
AL-AM	0 / 0	-18.5	0.09 (4)
AM-K	0 / 0	-18.5	0.09 (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")

CALCULATED VERT. DEFL.(LL) = L/998 (0.23")

ALLOWABLE DEFL.(TL) = L/360 (1.17")

CALCULATED VERT. DEFL.(TL) = L/971 (0.43")

CSI: TC=0.81/1.00 (D-E:1), SC=0.53/1.00 (M-O:1), WB=0.73/1.00 (D-R:1), SSI=0.21/1.00 (C-D:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

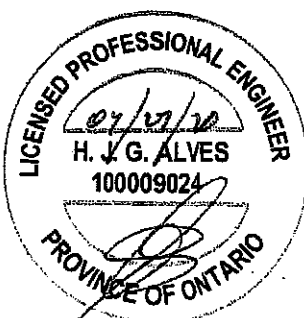
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.83 (R) (INPUT = 0.90)

JSI METAL = 0.68 (P) (INPUT = 1.00)



Structural component only

DWG# T-2006996

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408089	T30	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR6TstFoe31v8/ zns1I-dR5h7mw5q15aScOGVilPv5J4SYwwwuK4ISKTRLCzNEO

PLATES (table is in inches)

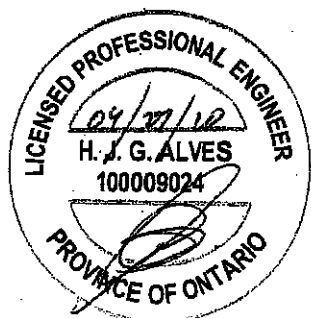
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	5.0	8.0		
C	TTW-m	MT20	4.0	6.0		
D	TMWVW-1	MT20	4.0	9.0		
E	TMW-w	MT20	2.0	4.0		
F	TS-1	MT20	3.0	6.0		
G	TMWVW-1	MT20	4.0	9.0		
H	TTW-m	MT20	4.0	6.0		
I	TMVW-1	MT20	5.0	8.0		
K	BMV1+p	MT20	3.0	8.0		
L	BMWVW-1	MT20	6.0	9.0	3.00	2.75
M	BMW-w	MT20	3.0	6.0		
N	BS-1	MT20	5.0	8.0		
O	BMWVW-1	MT20	5.0	8.0		
P	BS-1	MT20	5.0	8.0		
Q	BMW-w	MT20	3.0	6.0		
R	BMWVW-1	MT20	6.0	9.0	3.00	3.75
S	BMV1+p	MT20	3.0	8.0		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-15	-428	-428	---	FRONT	VERT	TOTAL	---	C1
D	11-11-3	-110	-110	---	FRONT	VERT	TOTAL	---	C1
E	17-7-0	-110	-110	---	FRONT	VERT	TOTAL	---	C1
G	23-2-13	-110	-110	---	FRONT	VERT	TOTAL	---	C1
H	29-3-1	-428	-428	---	FRONT	VERT	TOTAL	---	C1
M	23-2-13	-26	-26	---	FRONT	VERT	TOTAL	---	C1
N	21-2-13	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	17-7-0	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	13-11-3	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Q	11-11-3	-26	-26	---	FRONT	VERT	TOTAL	---	C1
T	7-11-3	-110	-110	---	FRONT	VERT	TOTAL	---	C1
U	9-11-3	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	13-11-3	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	15-11-3	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	19-2-13	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	21-2-13	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	25-2-13	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	27-2-13	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	1-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AC	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AD	5-11-3	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AE	7-11-3	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AF	9-11-3	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AG	15-11-3	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AH	19-2-13	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AI	25-2-13	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AJ	27-2-13	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AK	29-2-13	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AL	31-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AM	33-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2006996 3/2



DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S. K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

FACTORED CONCENTRATED LOADS (LBS)

CONNECTION REQUIREMENTS

1) G1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/913 (0.46")

CSI: TC=0.83/1.00 (B-C:1), BC=0.62/1.00 (Q-R:1),
WB=0.90/1.00 (B-R:1), SSI=0.15/1.00 (G-H:1)

DOZ LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (R) (INPUT = 0.90)
JSI METAL= 0.77 (P) (INPUT = 1.00)



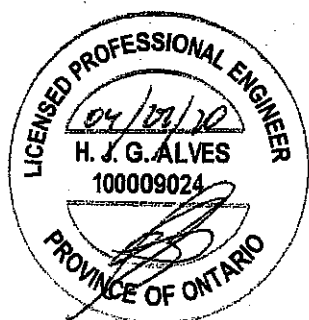
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408089	T30Z	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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ID:DMCubINVR6TstFoe31v6I zns1I-5ef4L6xjZKDO4mzt3QDeRjRExFsdkeuhOD?uezNECe

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	12.0	1.00	5.50
C	TTW-h	MT20	5.0	6.0	2.50	3.00
D	TMWWWW-t	MT20	4.0	9.0		
E	TMW-w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWWWW-t	MT20	4.0	9.0		
H	TTW-h	MT20	5.0	6.0	2.50	3.00
I	TMVW-p	MT20	4.0	12.0	1.00	5.50
K	BMV1-p	MT20	3.0	6.0		
L	BMWWWW-t	MT20	7.0	12.0	4.25	4.50
M	BMW-w	MT20	3.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMW-w	MT20	3.0	6.0		
R	BMWWWW-t	MT20	7.0	12.0	4.25	5.00
S	BMV1-p	MT20	3.0	6.0		

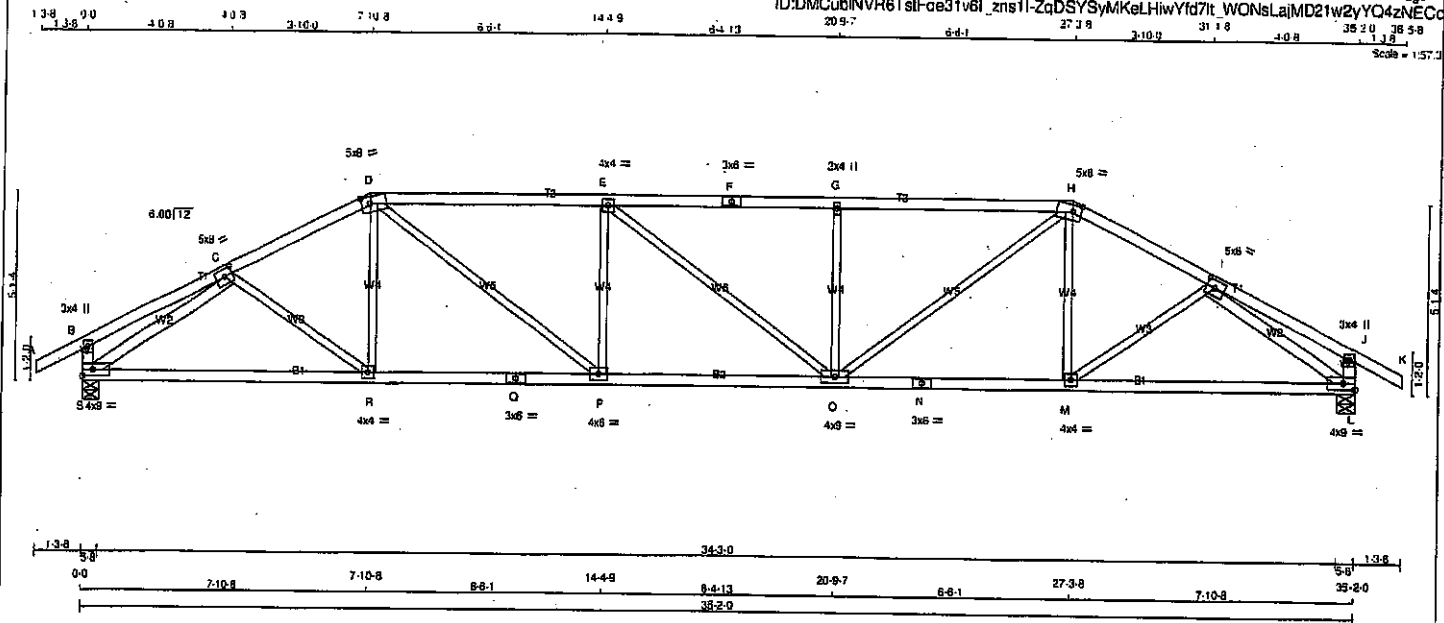


Structural component only
DWG# T-2006997 32

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408089	T31	2	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v8l_zns1l-ZqDSYSyMkaLHwYfd7it_WONsLajMD21w2yYQ4ZNECc



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - K	2x4	DRY No.2	SPF
S - B	2x4	DRY No.2	SPF
B - J	2x4	DRY No.2	SPF
J - L	2x4	DRY No.2	SPF
L - N	2x4	DRY No.2	SPF
N - L	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
S - C	2x4	DRY No.2	SPF
I - L	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWV-i	MT20	5.0	8.0	2.50 2.75
D	TTWV-m	MT20	5.0	8.0	2.25 2.75
E	TMWV-i	MT20	4.0	4.0	
F	TS-i	MT20	3.0	6.0	
G	TMWV-w	MT20	2.0	4.0	
H	TTWV-m	MT20	5.0	8.0	2.25 2.75
I	TMWV-i	MT20	5.0	6.0	2.50 2.75
J	TMV+p	MT20	3.0	4.0	
L	BMVWV-i	MT20	4.0	9.0	Edge
M	BMWV-i	MT20	4.0	4.0	
N	BS-i	MT20	3.0	6.0	
O	BMWVW-i	MT20	4.0	9.0	
P	BMWV-i	MT20	4.0	6.0	
Q	BS-i	MT20	3.0	6.0	
R	BMWV-i	MT20	4.0	4.0	
S	BMVWV-i	MT20	4.0	9.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
	VERT	HORZ	DOWN	HORZ
JT	2083	0	2083	0
S	2083	0	2083	0
L	2083	0	2083	0

UNFACTORED REACTIONS

	1ST CASE	MAX	MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE	PERM. LIVE
JT	1457	869 / 0	0 / 0	0 / 0
S	1457	869 / 0	0 / 0	0 / 0
L	1457	869 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	C-R	0 / 93	0.03 (4)
B-C	0 / 16	-91.8	-91.8 0.20 (1)	10.00	F-D	0 / 121	0.04 (4)
C-D	-2799 / 0	-91.8	-91.8 0.32 (1)	3.88	D-P	0 / 1287	0.29 (1)
D-E	-3508 / 0	-91.8	-91.8 0.95 (1)	2.94	E-F	-843 / 0	0.25 (1)
E-F	-3507 / 0	-91.8	-91.8 0.93 (1)	2.94	F-G	-2 / 0	0.00 (1)
F-G	-3507 / 0	-91.8	-91.8 0.93 (1)	2.94	G-H	-843 / 0	0.25 (1)
G-H	-3507 / 0	-91.8	-91.8 0.94 (1)	2.95	H-I	0 / 1266	0.28 (1)
H-I	-2799 / 0	-91.8	-91.8 0.32 (1)	3.88	I-J	0 / 121	0.04 (4)
I-J	0 / 16	-91.8	-91.8 0.20 (1)	10.00	J-K	0 / 93	0.03 (4)
J-K	0 / 28	-91.8	-91.8 0.12 (1)	10.00	K-L	-2974 / 0	0.83 (1)
K-L	-270 / 0	0.0	0.0 0.03 (1)	7.81	L-M	-2974 / 0	0.83 (1)
L-M	-270 / 0	0.0	0.0 0.03 (1)	7.81			
S-R	0 / 2417	-18.5	-18.5 0.53 (1)	10.00			
R-Q	0 / 2489	-18.5	-18.5 0.54 (1)	10.00			
Q-P	0 / 2489	-18.5	-18.5 0.54 (1)	10.00			
P-O	0 / 3509	-18.5	-18.5 0.64 (1)	10.00			
O-N	0 / 2489	-18.5	-18.5 0.54 (1)	10.00			
N-M	0 / 2489	-18.5	-18.5 0.54 (1)	10.00			
M-L	0 / 2417	-18.5	-18.5 0.53 (1)	10.00			

TOTAL WEIGHT = 2 X 139 = 278 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.41")

CSI: TC=0.95/1.00 (D-E:1), BC=0.84/1.00 (O-P:1),
WB=0.83/1.00 (I-L:1), SS=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX	MIN	MAX
MT20	618	354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

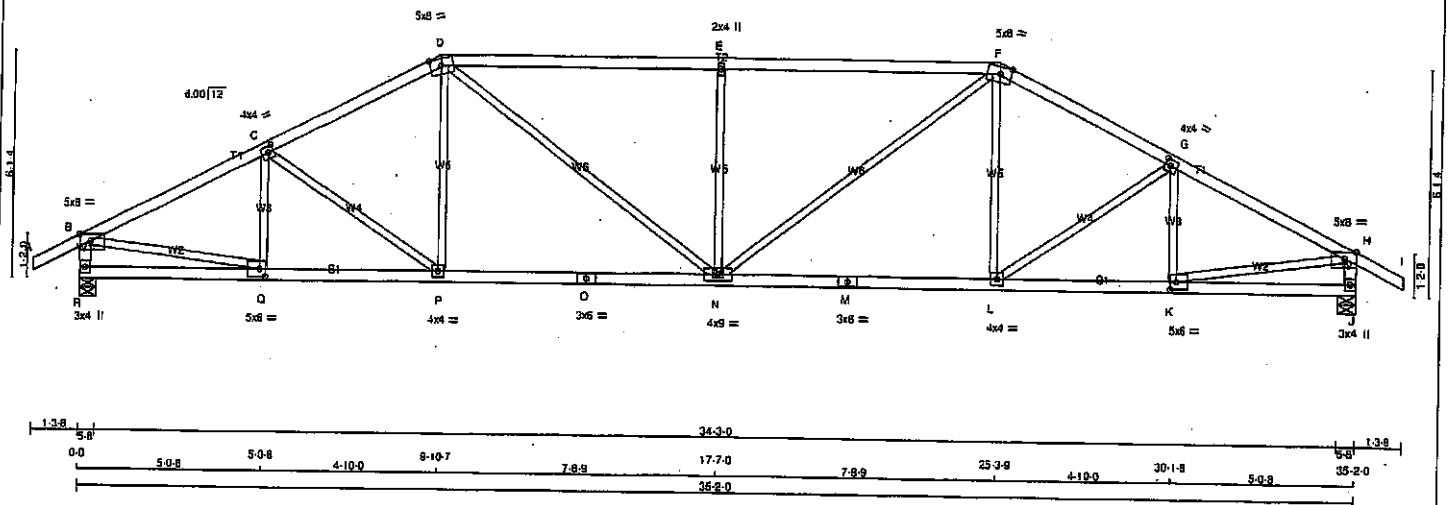
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (L) (INPUT = 0.90)
JSI METAL = 0.77 (N) (INPUT = 1.00)



Structural component only
DWG# T-2006998

JOB NAME 408089	TRUSS NAME T32	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mtek Industries, Inc. Sat Apr 25 09:25:43 2020 Page 1	
ID:DMCubINVR6TstFoe31v6l zns1-10nqloy 5yUBJ46rArG6WioxAlz25i6BBii5yXzNECc				Scale = 1/8" = 1'-0"	



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
R - S	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
R - O	2x4	DRY No.2	SPF
O - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	Edge 3.50
C	TMWW-l	MT20	4.0	4.0	2.00 1.75
D	TTWW-m	MT20	5.0	8.0	2.25 3.75
E	TMWW-w	MT20	2.0	4.0	
F	TTWW-m	MT20	5.0	8.0	2.25 3.75
G	TMWW-l	MT20	4.0	4.0	2.00 1.75
H	TMWW-p	MT20	5.0	8.0	Edge 3.50
J	BMV1+p	MT20	3.0	4.0	
K	BMWW-l	MT20	5.0	8.0	2.50 2.00
L	BMWW-l	MT20	4.0	4.0	
M	BS-l	MT20	3.0	6.0	
N	BMWWWW-l	MT20	4.0	9.0	
O	BS-l	MT20	3.0	6.0	
P	BMWW-l	MT20	4.0	4.0	
Q	BMWW-l	MT20	5.0	8.0	2.50 2.00
R	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ
R	2063	0	2063	0
J	2063	0	2063	0

UNFACTORED REACTIONS

	1ST LOOSE	MAX MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
R	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
J	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLINE SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

	CHORDS	WEBS
	MAX. FACTORED	MAX. FACTORED
MEMB.	FORCE (LBS)	FORCE (LBS)
FR-TO	FROM TO	FROM TO
A-B	0 / 28	-91.8 -91.8 0.12 (1)
B-C	-2834 / 0	-91.8 -91.8 0.39 (1)
C-D	-2688 / 0	-91.8 -91.8 0.37 (1)
D-E	-3061 / 0	-91.8 -91.8 0.97 (1)
E-F	-3061 / 0	-91.8 -91.8 0.97 (1)
F-G	-2688 / 0	-91.8 -91.8 0.37 (1)
G-H	-2834 / 0	-91.8 -91.8 0.39 (1)
H-I	0 / 28	-91.8 -91.8 0.12 (1)
I-J	-2018 / 0	0.0 0.0 0.20 (1)
J-K	-2018 / 0	0.0 0.0 0.20 (1)
R-O	0 / 0	-18.5 -18.5 0.10 (4)
Q-P	0 / 2554	-18.5 -18.5 0.51 (1)
P-O	0 / 2386	-18.5 -18.5 0.49 (1)
O-N	0 / 2386	-18.5 -18.5 0.49 (1)
N-M	0 / 2386	-18.5 -18.5 0.49 (1)
M-L	0 / 2386	-18.5 -18.5 0.49 (1)
L-K	0 / 2554	-18.5 -18.5 0.51 (1)
K-J	0 / 0	-18.5 -18.5 0.10 (4)

TOTAL WEIGHT = 2 X 139 = 277 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 8.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. O.C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/999 (0.17")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = L/999 (0.34")

CSI: TO=0.97/1.00 (E-F:1), BC=0.51/1.00 (K-L:1),
WB=0.58/1.00 (H-K:1), SS=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR	SECTION (PS)	(PL)	(PL)
MAX MIN MAX MIN			
MT20	618	354	1687 788 1987 1656

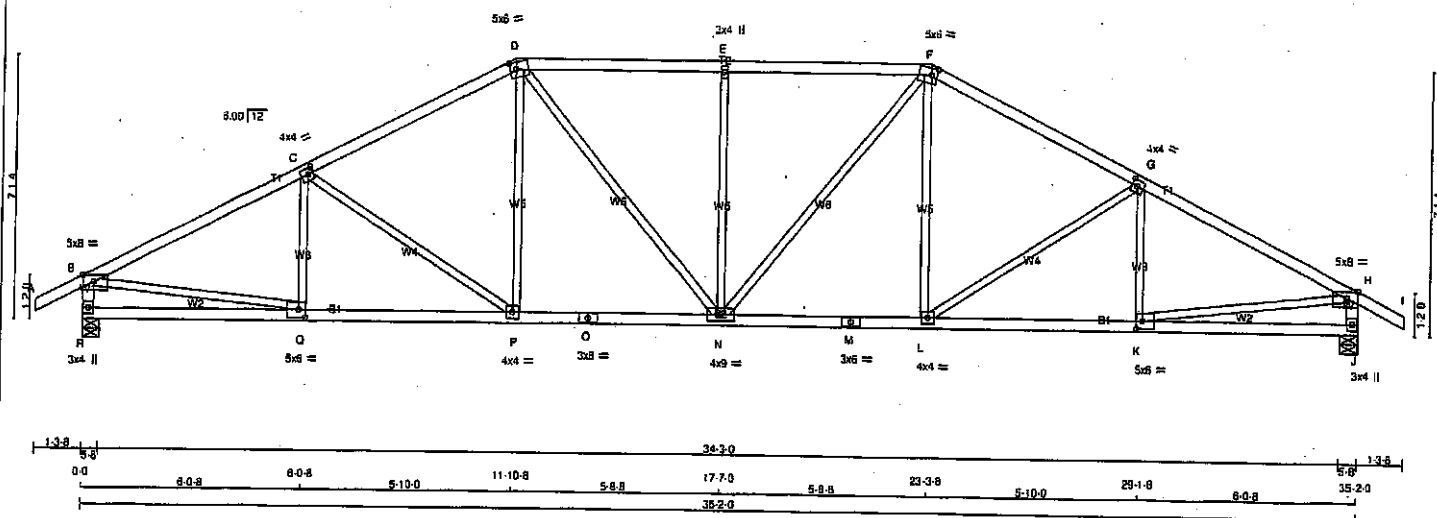
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (0) (INPUT = 0.90)
JSI METAL= 0.82 (M) (INPUT = 1.00)



Structural component only
DWG# T-2006999



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	Edge	3.50
C TMVW-i	MT20	4.0	4.0	2.00	1.75
D TTWW-m	MT20	5.0	6.0	2.25	2.00
E TMVW-w	MT20	2.0	4.0		
F TTWW-m	MT20	5.0	8.0	2.25	2.00
G TMVW-i	MT20	4.0	4.0	2.00	1.75
H TMVW-p	MT20	5.0	8.0	Edge	3.50
J BMV1-p	MT20	3.0	4.0		
K BMVW-i	MT20	5.0	6.0	2.50	2.25
L BMVW-i	MT20	4.0	4.0		
M BS-i	MT20	3.0	6.0		
N BMVW-i	MT20	4.0	9.0		
O BS-i	MT20	3.0	6.0		
P BMVW-i	MT20	4.0	4.0		
Q BMVW-i	MT20	5.0	6.0	2.50	2.25
R BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
R	2083	0	2083	0	5-8	5-8
J	2083	0	2083	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
R	1457	959 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
J	1457	959 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 (PLF)	MAX CSI (L/C)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (L/C)	
FR-TO		FROM TO			LENGTH	FR-TO			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	Q-C	-255 / 12	0.07 (1)	
B-C	-2889 / 0	-91.8	-91.8	0.58 (1)	3.58	C-P	-435 / 0	0.42 (1)	
C-D	-2539 / 0	-91.8	-91.8	0.52 (1)	1.32	P-D	0 / 350	0.08 (1)	
D-E	-2559 / 0	-91.8	-91.8	0.48 (1)	3.85	D-N	0 / 482	0.11 (1)	
E-F	-2559 / 0	-91.8	-91.8	0.48 (1)	3.85	N-E	-641 / 0	0.56 (1)	
F-G	-2539 / 0	-91.8	-91.8	0.52 (1)	3.83	N-F	0 / 482	0.11 (1)	
G-H	-2889 / 0	-91.8	-91.8	0.58 (1)	3.58	L-F	0 / 350	0.08 (1)	
H-I	0 / 28	-91.8	-91.8	0.12 (1)	10.00	L-G	-435 / 0	0.42 (1)	
R-B	-2014 / 0	0.0	0.0	0.20 (1)	5.95	K-G	-255 / 12	0.07 (1)	
J-H	-2014 / 0	0.0	0.0	0.20 (1)	5.95	B-Q	0 / 2635	0.58 (1)	
						K-H	0 / 2635	0.59 (1)	
R-Q	0 / 0	-18.5	-18.5	0.15 (4)	10.00				
Q-P	0 / 2609	-18.5	-18.5	0.49 (1)	10.00				
P-O	0 / 2249	-18.5	-18.5	0.43 (1)	10.00				
O-N	0 / 2249	-18.5	-18.5	0.43 (1)	10.00				
N-M	0 / 2249	-18.5	-18.5	0.43 (1)	10.00				
M-L	0 / 2249	-18.5	-18.5	0.43 (1)	10.00				
L-K	0 / 2609	-18.5	-18.5	0.49 (1)	10.00				
K-J	0 / 0	-18.5	-18.5	0.15 (4)	10.00				

TOTAL WEIGHT = 2 X 145 = 289 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPC 2011, TPC 2014

(55% OF 91.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.58/1.00 (G:H=1), BC=0.49/1.00 (K:L=1),
WB=0.59/1.00 (H:K=1), SS=0.25/1.00 (E:F=1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

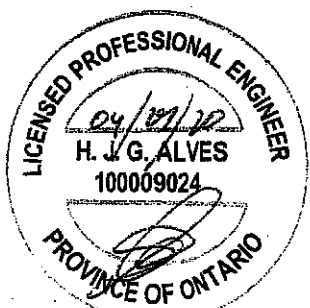
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.59 (M) (INPUT = 1.00)

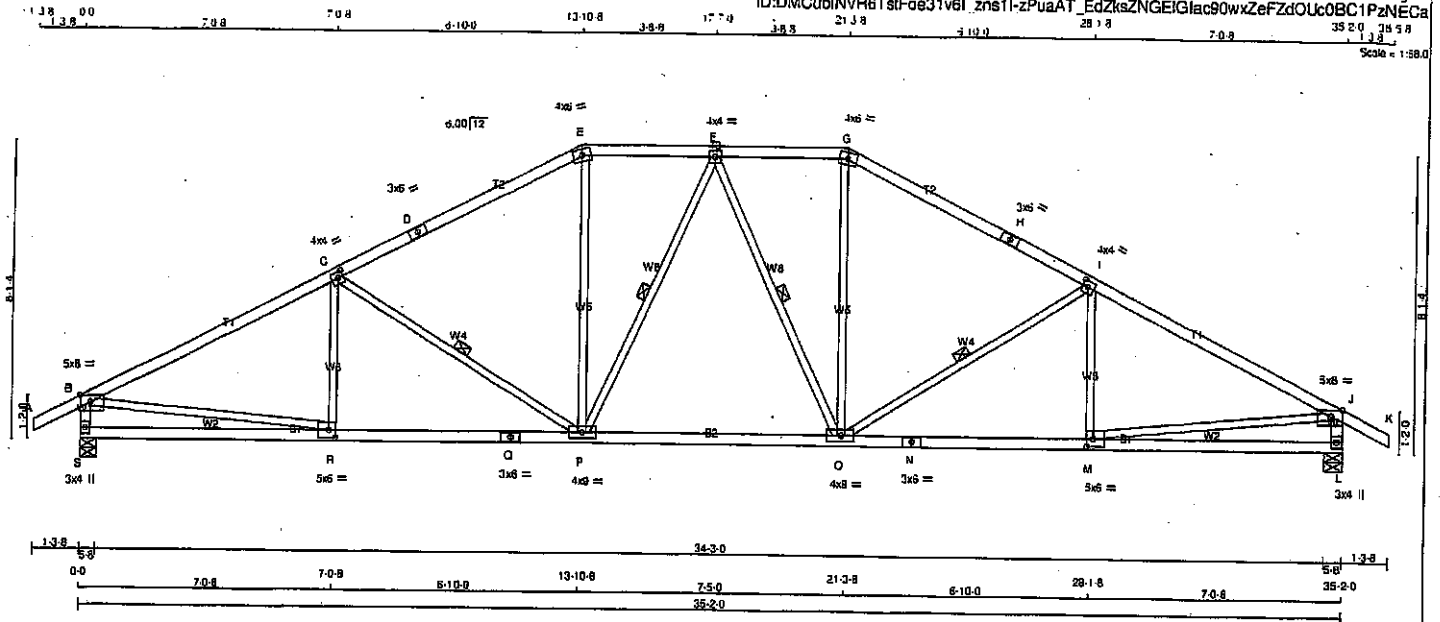


Structural component only
DWG# T-2007000

JOB NAME 408089	TRUSS NAME T34	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	3.0	Edge 3.50
C	TMWV-l	MT20	4.0	4.0	2.00 1.75
D	TS-l	MT20	3.0	6.0	
E	TTW-m	MT20	4.0	6.0	
F	TMWV-l	MT20	4.0	4.0	
G	TTW-m	MT20	4.0	6.0	
H	TS-l	MT20	3.0	6.0	
I	TMWV-l	MT20	4.0	4.0	2.00 1.75
J	TMWV-p	MT20	5.0	3.0	Edge 3.50
L	BMV1-p	MT20	3.0	4.0	
M	BMWV-l	MT20	5.0	6.0	2.50 2.25
N	BS-l	MT20	3.0	6.0	
O	BMWV-l	MT20	4.0	6.0	
P	BMWV-l	MT20	4.0	6.0	
Q	BS-l	MT20	3.0	6.0	
R	BMWV-l	MT20	5.0	6.0	2.50 2.25
S	BMV1-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT VERT	2063	0	0	5-8
S	2063	0	0	5-8
L	2063	0	0	5-8

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT COMBINED	1457	989 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
S	1457	989 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-P, F-O, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	FACTORED MAX. CS1 (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO					FR-TO			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	R-C	-182 / 52	0.06 (1)
B-C	-2906 / 0	-91.8	-91.8	0.83 (1)	3.25	C-P	-637 / 0	0.29 (1)
C-D	-2373 / 0	-91.8	-91.8	0.72 (1)	3.66	P-E	0 / 674	0.15 (1)
D-E	-2373 / 0	-91.8	-91.8	0.72 (1)	3.66	F-O	-222 / 0	0.11 (1)
E-F	-2100 / 0	-91.8	-91.8	0.19 (1)	4.53	O-G	-222 / 0	0.11 (1)
F-G	-2100 / 0	-91.8	-91.8	0.19 (1)	4.53	G-H	0 / 674	0.15 (1)
G-H	-2373 / 0	-91.8	-91.8	0.72 (1)	3.66	H-I	-637 / 0	0.29 (1)
H-I	-2373 / 0	-91.8	-91.8	0.72 (1)	3.66	I-J	-182 / 52	0.06 (1)
I-J	-2906 / 0	-91.8	-91.8	0.83 (1)	3.25	J-K	0 / 2649	0.60 (1)
J-K	0 / 28	-91.8	-91.8	0.12 (1)	10.00	K-L	0 / 2649	0.60 (1)
S-B	-2008 / 0	0.0	0.0	0.20 (1)	5.66			
L-J	-2008 / 0	0.0	0.0	0.20 (1)	5.66			
S-R	0 / 0	-18.5	-18.5	0.20 (4)	10.00			
R-Q	0 / 2630	-18.5	-18.5	0.52 (1)	10.00			
Q-P	0 / 2630	-18.5	-18.5	0.52 (1)	10.00			
P-O	0 / 2193	-18.5	-18.5	0.45 (1)	10.00			
O-N	0 / 2630	-18.5	-18.5	0.52 (1)	10.00			
N-M	0 / 2630	-18.5	-18.5	0.52 (1)	10.00			
M-L	0 / 0	-18.5	-18.5	0.20 (4)	10.00			

TOTAL WEIGHT = 2 X 145 = 291 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 39.0 PSF	

SPACING = 24.0 IN. G.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TRC 2011, TRC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CS1: TC=0.83/1.00 (B-C:1), BC=0.52/1.00 (M-O:1), WB=0.60/1.00 (B-R:1), SS=0.28/1.00 (K-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	818 354	1987 788 1987 1658

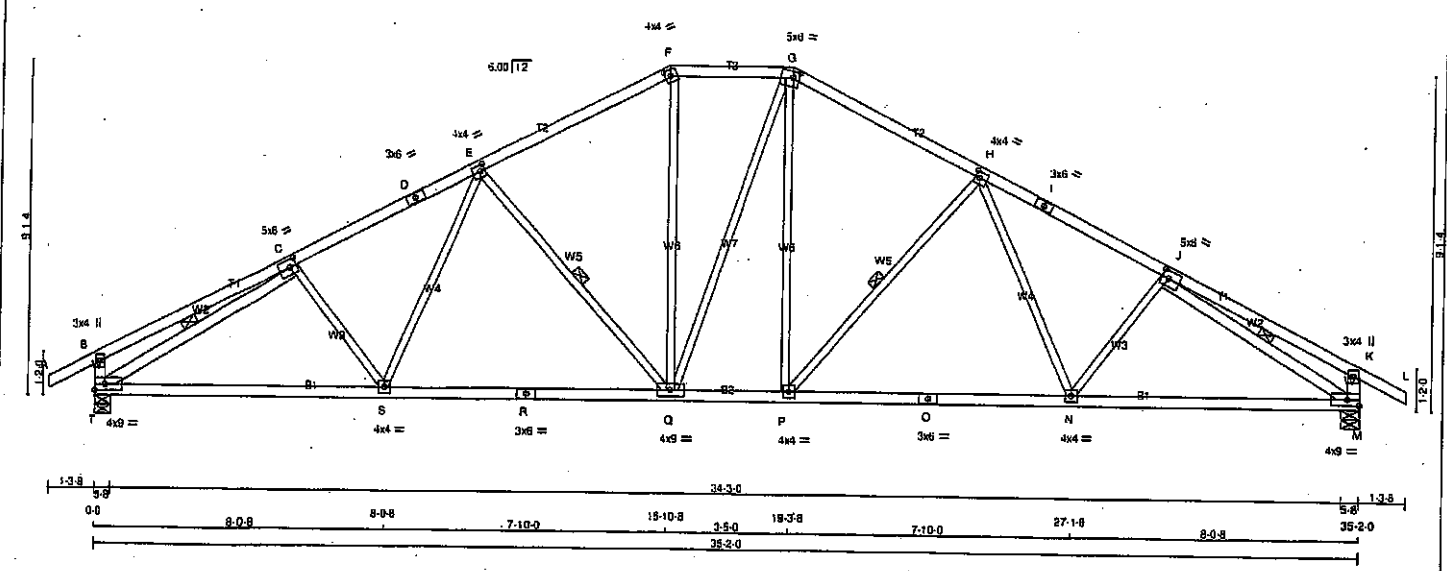
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 0.79 (O) (INPUT = 1.00)



Structural component only
DWG# T-2007001



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
L - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
T - R	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

T - C 2x4 DRY No.2

J - M 2x4 DRY No.2

DRY: SEASONED LUMBER:

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0	
C	TMWW-I	MT20	5.0	8.0	2.50 2.25
D	TS-I	MT20	3.0	6.0	
E	TMWW-I	MT20	4.0	4.0	2.00 1.50
F	TTW-h	MT20	4.0	4.0	2.00 1.75
G	TTWW-m	MT20	5.0	8.0	2.00 2.00
H	TMWW-I	MT20	4.0	4.0	2.00 1.50
I	TS-I	MT20	3.0	6.0	
J	TMWW-I	MT20	5.0	8.0	2.50 2.25
K	TMV-p	MT20	3.0	4.0	
M	BMVW-I	MT20	4.0	9.0	Edge
N, P, S					
N	BMWW-I	MT20	4.0	4.0	
O	BS-I	MT20	3.0	6.0	
Q	BMWWW-I	MT20	4.0	9.0	
R	BS-I	MT20	3.0	6.0	
T	BMVW-I	MT20	4.0	9.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	GROSS REACTION		MAXIMUM FACTORED		INPUT	REQD
	VERT	HORZ	DOWN	HORZ		
JT	2063	0	2063	0	5-8	5-8
T	2063	0	2063	0	5-8	5-8
M	2063	0	2063	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS					
	SNOW	LIVE	PERM	LIVE	WIND	DEAD
JT	1457	989 / 0	0 / 0	0 / 0	0 / 0	498 / 0
T	1457	989 / 0	0 / 0	0 / 0	0 / 0	498 / 0
M	1457	989 / 0	0 / 0	0 / 0	0 / 0	498 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, H-P, C-T, J-M

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PL)	MAX. FACTORED (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LC)	
FR-TO				FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	G-S	-110 / 37	0.04 (1)
B-C	0 / 20	-91.8	-91.8 0.32 (1)	10.00	S-E	0 / 277	0.06 (1)
C-D	-2760 / 0	-91.8	-91.8 0.40 (1)	3.88	E-Q	-681 / 0	0.31 (1)
D-E	-2760 / 0	-91.8	-91.8 0.40 (1)	3.88	Q-F	0 / 813	0.14 (1)
E-F	-2167 / 0	-91.8	-91.8 0.36 (1)	4.30	O-G	0 / 4	0.00 (1)
F-G	-1925 / 0	-91.8	-91.8 0.18 (1)	4.71	P-G	0 / 609	0.14 (1)
G-H	-2165 / 0	-91.8	-91.8 0.36 (1)	4.30	P-H	-683 / 0	0.31 (1)
H-I	-2761 / 0	-91.8	-91.8 0.40 (1)	3.88	H-N	0 / 279	0.06 (1)
I-J	-2761 / 0	-91.8	-91.8 0.40 (1)	3.88	N-J	-110 / 37	0.04 (1)
J-K	0 / 20	-91.8	-91.8 0.32 (1)	10.00	T-C	-3042 / 0	0.81 (1)
K-L	0 / 28	-91.8	-91.8 0.12 (1)	10.00	J-M	-3043 / 0	0.81 (1)
T-B	-325 / 0	0.0	0.0 0.03 (1)	7.81			
M-K	-325 / 0	0.0	0.0 0.03 (1)	7.81			
T-S	0 / 2538	-18.5	-18.5 0.56 (1)	10.00			
S-R	0 / 2362	-18.5	-18.5 0.53 (1)	10.00			
R-Q	0 / 2362	-18.5	-18.5 0.53 (1)	10.00			
Q-P	0 / 1924	-18.5	-18.5 0.40 (1)	10.00			
P-O	0 / 2362	-18.5	-18.5 0.53 (1)	10.00			
O-N	0 / 2362	-18.5	-18.5 0.53 (1)	10.00			
N-M	0 / 2538	-18.5	-18.5 0.56 (1)	10.00			

TOTAL WEIGHT = 2 X 155 = 310 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/999 (0.14")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = L/999 (0.29")

CS: TC=0.40/1.00 (H-J:1), BC=0.56/1.00 (M-N:1), WB=0.81/1.00 (J-M:1), SS=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

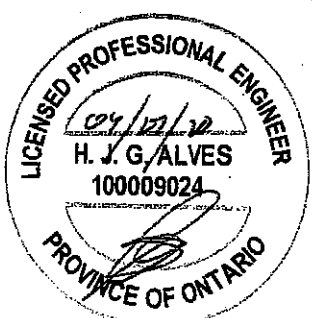
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1867 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (F) (INPUT = 0.90)
JSI METAL = 0.63 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007002

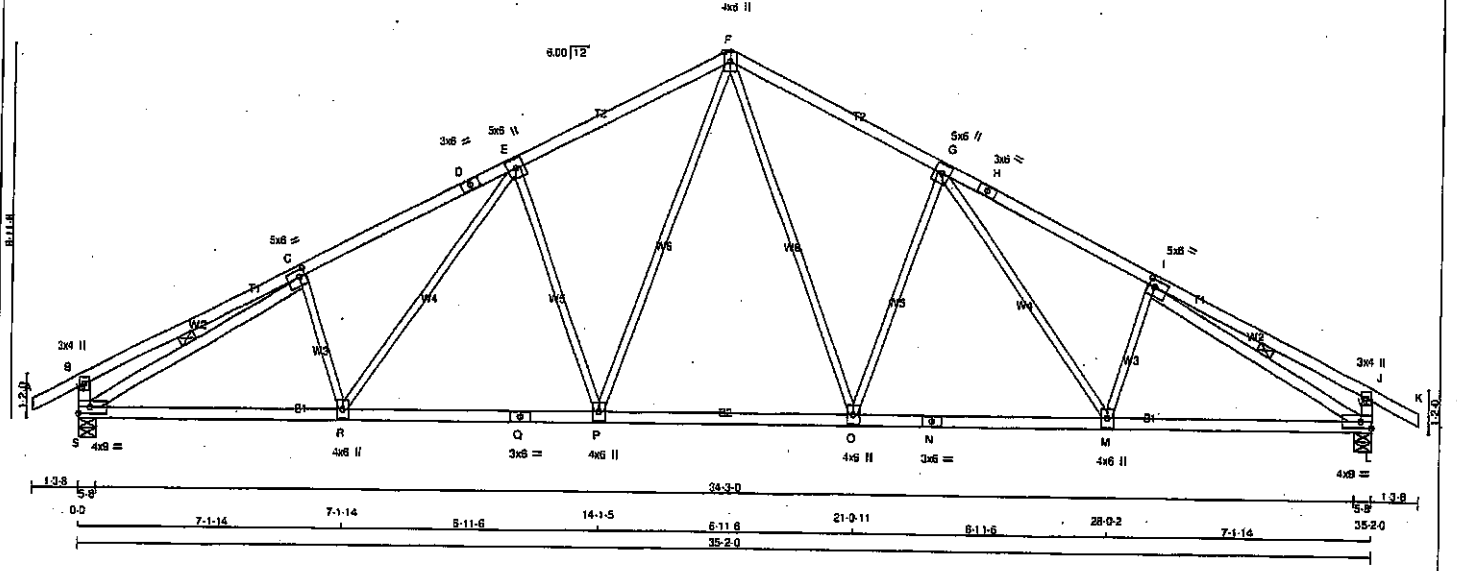
JOB NAME 408089	TRUSS NAME T36	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/8" = 1'-0"



LUMBER				DESIGN CRITERIA			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	SPECIFIED LOADS:			
CHORDS				TOP CH. LL = 25.6 PSF			
A - D	2x4	DRY	No.2	DL = 6.0 PSF			
D - F	2x4	DRY	No.2	BOT CH. LL = 0.0 PSF			
F - H	2x4	DRY	No.2	DL = 7.4 PSF			
H - K	2x4	DRY	No.2	TOTAL LOAD = 39.0 PSF			
S - B	2x4	DRY	No.2				
L - J	2x4	DRY	No.2				
S - Q	2x4	DRY	No.2				
Q - N	2x4	DRY	No.2				
N - L	2x4	DRY	No.2				
ALL WEBS	2x3	DRY	No.2	SPACING = 24.0 IN/C			
EXCEPT							
S - C	2x4	DRY	No.2				
I - L	2x4	DRY	No.2				

DRY: SEASONED LUMBER.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+1	MT20	5.0	6.0	2.25	2.00
D	TS-1	MT20	3.0	6.0		
E	TMVW+1	MT20	5.0	6.0		
F	TTVW+p	MT20	4.0	6.0	Edge	
G	TMVW+1	MT20	5.0	6.0		
H	TS-1	MT20	3.0	6.0		
I	TMVW+1	MT20	5.0	6.0	2.25	2.00
J	TMV+p	MT20	3.0	4.0		
L	BMVW+1	MT20	4.0	9.0	Edge	
M, Q, P, R						
M	BMVW+1	MT20	4.0	6.0		
N	BS-1	MT20	3.0	6.0		
Q	BS-1	MT20	3.0	6.0		
S	BMVW+1	MT20	4.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PL)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO				FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	F-O	0 / 866	0.19 (1)	
B-C	0 / 22	-91.8	-91.8 0.40 (1)	O-G	-728 / 0	0.72 (1)	
C-D	-2808 / 0	-91.8	-91.8 0.49 (1)	G-M	0 / 391	0.09 (1)	
D-E	-2808 / 0	-91.8	-91.8 0.49 (1)	M-I	-192 / 18	0.05 (1)	
E-F	-2322 / 0	-91.8	-91.8 0.46 (1)	P-F	0 / 866	0.19 (1)	
F-G	-2322 / 0	-91.8	-91.8 0.46 (1)	E-P	-728 / 0	0.72 (1)	
G-H	-2808 / 0	-91.8	-91.8 0.49 (1)	P-E	0 / 391	0.09 (1)	
H-I	-2808 / 0	-91.8	-91.8 0.49 (1)	C-R	-192 / 18	0.05 (1)	
I-J	0 / 22	-91.8	-91.8 0.12 (1)	S-C	-3084 / 0	0.72 (1)	
J-K	0 / 28	-91.8	-91.8 0.12 (1)	I-L	-3084 / 0	0.72 (1)	
S-B	-344 / 0	0.0	0.0 0.03 (1)				
L-J	-344 / 0	0.0	0.0 0.03 (1)				
S-R	0 / 2574	-18.5	-18.5 0.53 (1)				
R-Q	0 / 2291	-18.5	-18.5 0.47 (1)				
Q-P	0 / 2291	-18.5	-18.5 0.47 (1)				
P-O	0 / 1782	-18.5	-18.5 0.38 (1)				
O-N	0 / 2291	-18.5	-18.5 0.47 (1)				
N-M	0 / 2291	-18.5	-18.5 0.47 (1)				
M-L	0 / 2574	-18.5	-18.5 0.53 (1)				

Structural component only
DWG# T-2007003

COMpanion LIVE LOAD FACTOR = 1.00

AUTO SOLVEHEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MT20 618 354 1687 788 1987 1686

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (I) (INPUT = 0.89)

JSI METAL = 0.77 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408089	T37	1	2	GREEN PARK HOMES	

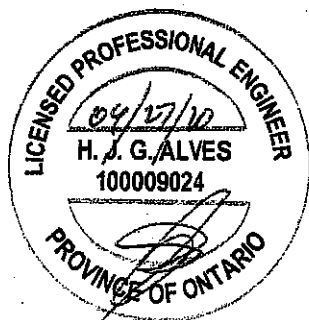
Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Sat Apr 25 09:25:49 2020 Page 2
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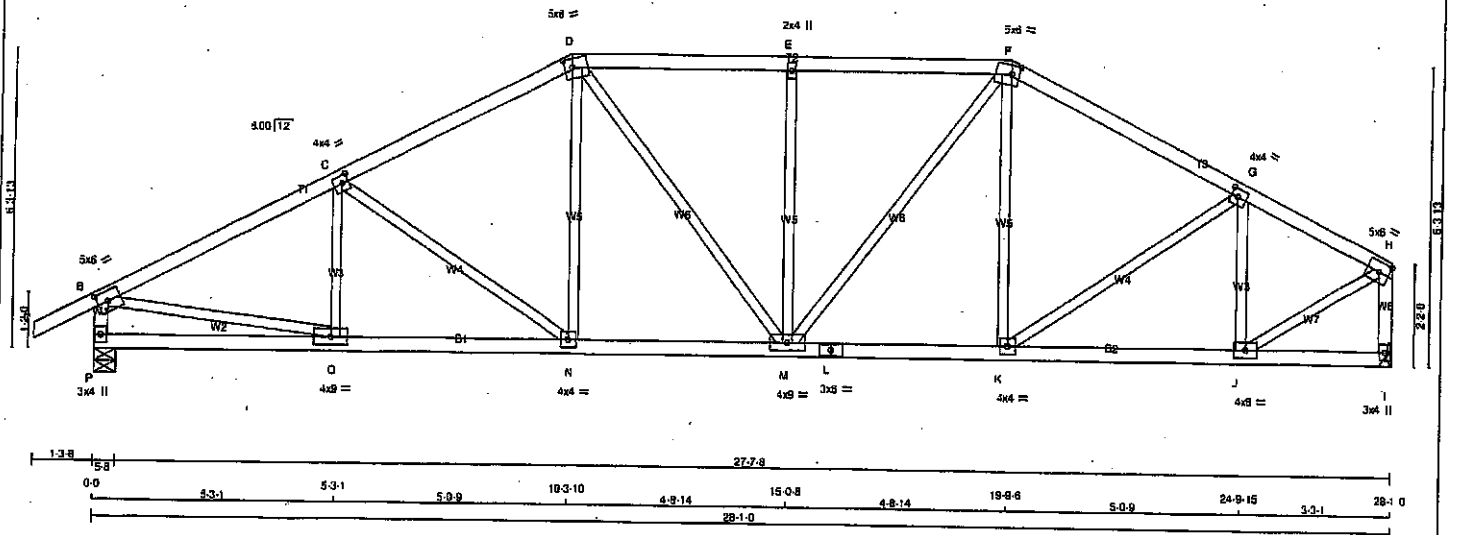
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	2.25	3.25
E	TMVW-m	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	1.50
G	TMVW-t	MT20	5.0	6.0		Edge
H	BMV1-p	MT20	3.0	6.0		
I	BMVW-t	MT20	5.0	6.0		
J	BMVWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
N	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



Structural component only
DWG# T-2007004 3/2



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR
A - D	2x4	DRY No.2	SPF
F - F	2x4	DRY No.2	SPF
D - H	2x4	DRY No.2	SPF
P - B	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
P - L	2x4	DRY No.2	SPF
L - I	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	5.0	6.0	2.50	2.75
C TMVW-t	MT20	4.0	4.0	2.00	1.75
D TTWW-m	MT20	5.0	6.0	2.25	2.00
E TMVW-w	MT20	2.0	4.0		
F TTWW-m	MT20	5.0	6.0	2.25	2.00
G TMVW-t	MT20	4.0	4.0	2.00	1.75
H TMVW-t	MT20	5.0	6.0		Edge
I BMVW-t	MT20	3.0	4.0		
J BMVW-t	MT20	4.0	6.0		
K BMVW-t	MT20	4.0	4.0		
L BS-t	MT20	3.0	6.0		
M BMVW-t	MT20	4.0	9.0		
N BMVW-t	MT20	4.0	4.0		
O BMVW-t	MT20	4.0	9.0		
P BMVW-t	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	1673	0	1673	0	0	5-8	5-8
I	1548	0	1548	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1181	788 / 0	0 / 0	0 / 0	0 / 0	393 / 0	0 / 0
I	1095	718 / 0	0 / 0	0 / 0	0 / 0	376 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED VERT. LOAD (PL)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PL)	MAX. FACTORED VERT. LOAD (PL)
FR-TO				FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	C-C	-224 / 10	0.05 (1)
B-C	-2197 / 0	-91.8	-91.8 0.38 (1)	4.25	C-N	-370 / 0	0.25 (1)
C-D	-1905 / 0	-91.8	-91.8 0.35 (1)	4.53	N-D	0 / 303	0.07 (1)
D-E	-1838 / 0	-91.8	-91.8 0.30 (1)	4.66	D-M	0 / 247	0.06 (1)
E-F	-1838 / 0	-91.8	-91.8 0.30 (1)	4.66	M-E	-530 / 0	0.34 (1)
F-G	-1711 / 0	-91.8	-91.8 0.31 (1)	4.79	M-F	0 / 527	0.12 (1)
G-H	-1474 / 0	-91.8	-91.8 0.25 (1)	5.14	K-F	-15 / 83	0.02 (1)
P-B	-1629 / 0	0.0	0.0 0.16 (1)	6.48	K-G	0 / 202	0.05 (1)
I-H	-1524 / 0	0.0	0.0 0.18 (1)	6.65	J-G	-687 / 0	0.16 (1)
					B-O	0 / 2012	0.45 (1)
P-O	0 / 0	-18.5	-18.5 0.11 (4)	10.00	J-H	0 / 1548	0.35 (1)
O-N	0 / 1986	-18.5	-18.5 0.37 (1)	10.00			
N-M	0 / 1685	-18.5	-18.5 0.32 (1)	10.00			
M-L	0 / 1512	-18.5	-18.5 0.30 (1)	10.00			
L-K	0 / 1512	-18.5	-18.5 0.30 (1)	10.00			
K-J	0 / 1345	-18.5	-18.5 0.27 (1)	10.00			
J-I	0 / 0	-18.5	-18.5 0.07 (4)	10.00			

TOTAL WEIGHT = 119 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 BOT CH. LL = 0.0 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24" IN LCIC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBCC 2010, CBC 2012, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360 (0.94")$
 CALCULATED VERT. DEFL.(LL) = $L/999 (0.08")$
 ALLOWABLE DEFL.(TL) = $L/360 (0.94")$
 CALCULATED VERT. DEFL.(TL) = $L/999 (0.14")$

CSI: TC=0.38/1.00 (B-C:1), BC=0.37/1.00 (N-O:1), WB=0.45/1.00 (B-O:1), SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

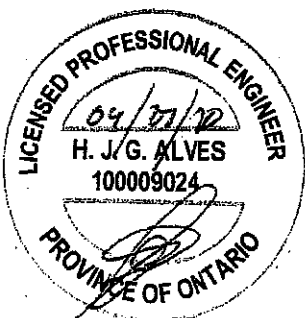
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL)
 MAX MIN MAX MIN MAX MIN
 MT20 518 354 1667 788 1987 1656

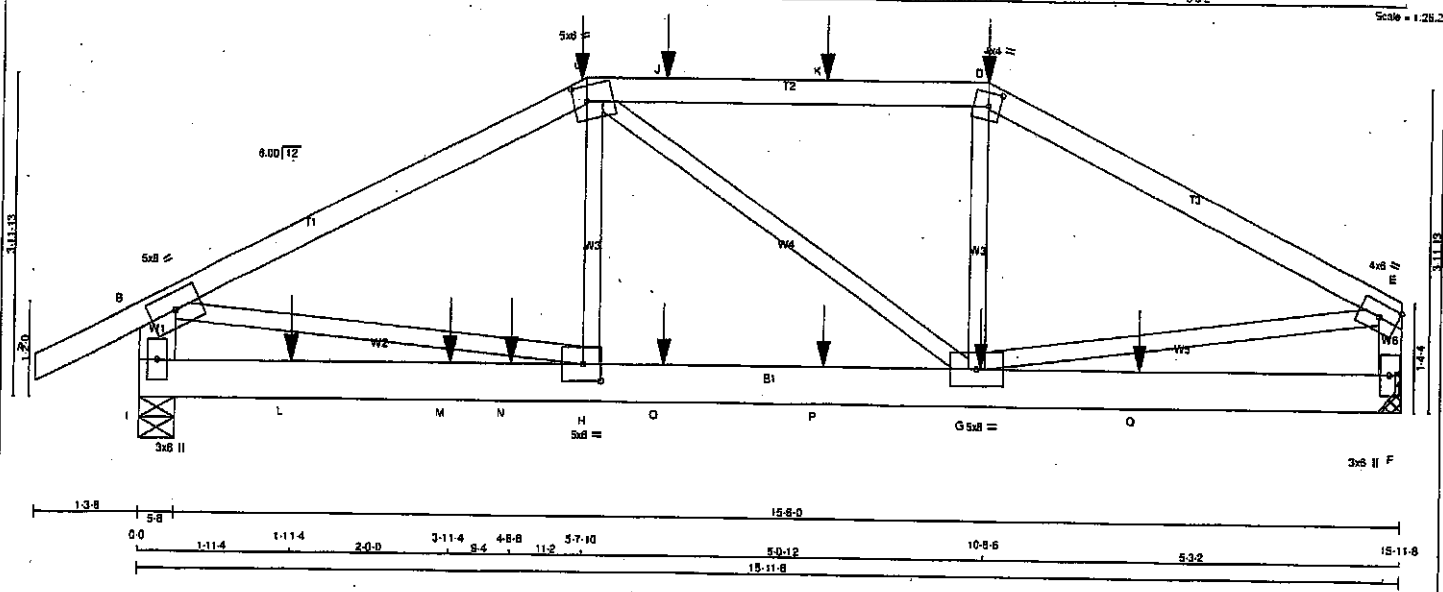
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.80)
 JSI METAL= 0.59 (B) (INPUT = 1.00)



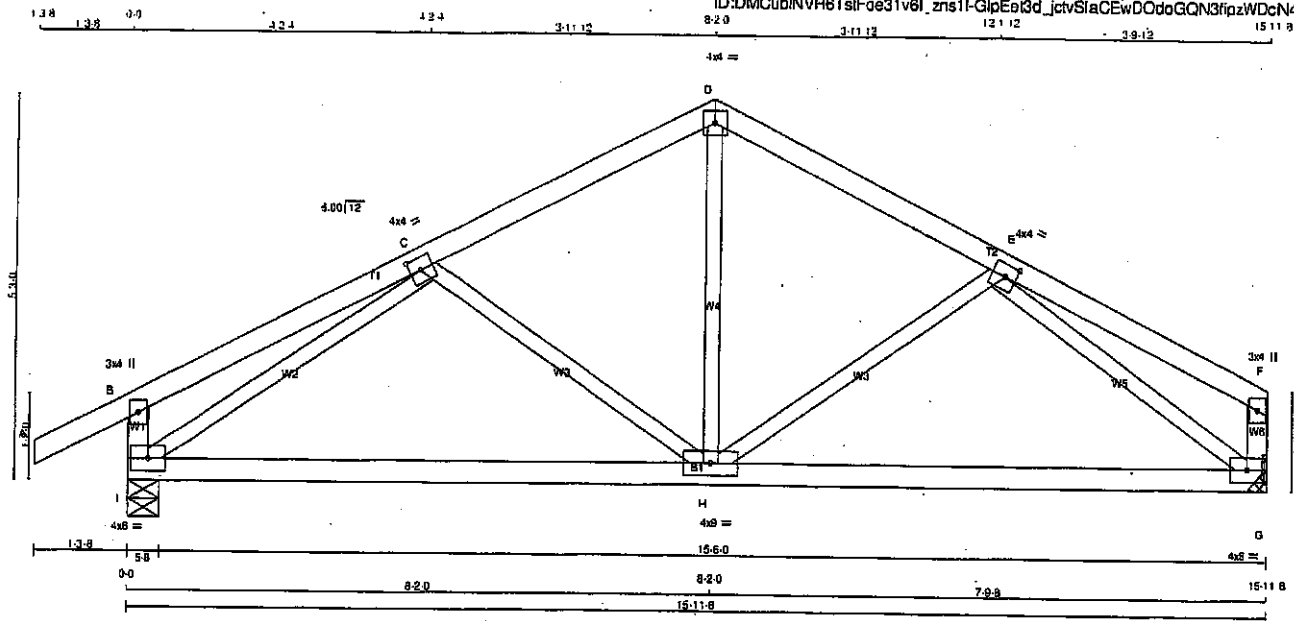
Structural component only
 DWG# T-2007005



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - D	2x4	DRY No.2	SPF
D - E	2x4	DRY No.2	SPF
E - F	2x4	DRY No.2	SPF
F - G	2x4	DRY No.2	SPF
G - H	2x4	DRY No.2	SPF
H - I	2x4	DRY No.2	SPF
I - J	2x4	DRY No.2	SPF
J - K	2x4	DRY No.2	SPF
K - L	2x4	DRY No.2	SPF
L - M	2x4	DRY No.2	SPF
M - N	2x4	DRY No.2	SPF
N - O	2x4	DRY No.2	SPF
O - P	2x4	DRY No.2	SPF
P - Q	2x4	DRY No.2	SPF
Q - R	2x4	DRY No.2	SPF
R - S	2x4	DRY No.2	SPF
S - T	2x4	DRY No.2	SPF
T - U	2x4	DRY No.2	SPF
U - V	2x4	DRY No.2	SPF
V - W	2x4	DRY No.2	SPF
W - X	2x4	DRY No.2	SPF
X - Y	2x4	DRY No.2	SPF
Y - Z	2x4	DRY No.2	SPF
Z - AA	2x4	DRY No.2	SPF
AA - AB	2x4	DRY No.2	SPF
AB - AC	2x4	DRY No.2	SPF
AC - AD	2x4	DRY No.2	SPF
AD - AE	2x4	DRY No.2	SPF
AE - AF	2x4	DRY No.2	SPF
AF - AG	2x4	DRY No.2	SPF
AG - AH	2x4	DRY No.2	SPF
AH - AI	2x4	DRY No.2	SPF
AI - AJ	2x4	DRY No.2	SPF
AJ - AK	2x4	DRY No.2	SPF
AK - AL	2x4	DRY No.2	SPF
AL - AM	2x4	DRY No.2	SPF
AM - AN	2x4	DRY No.2	SPF
AN - AO	2x4	DRY No.2	SPF
AO - AP	2x4	DRY No.2	SPF
AP - AQ	2x4	DRY No.2	SPF
AQ - AR	2x4	DRY No.2	SPF
AR - AS	2x4	DRY No.2	SPF
AS - AT	2x4	DRY No.2	SPF
AT - AU	2x4	DRY No.2	SPF
AU - AV	2x4	DRY No.2	SPF
AV - AW	2x4	DRY No.2	SPF
AW - AX	2x4	DRY No.2	SPF
AX - AY	2x4	DRY No.2	SPF
AY - AZ	2x4	DRY No.2	SPF
AZ - BA	2x4	DRY No.2	SPF
BA - BB	2x4	DRY No.2	SPF
BB - BC	2x4	DRY No.2	SPF
BC - BD	2x4	DRY No.2	SPF
BD - BE	2x4	DRY No.2	SPF
BE - BF	2x4	DRY No.2	SPF
BF - BG	2x4	DRY No.2	SPF
BG - BH	2x4	DRY No.2	SPF
BH - BI	2x4	DRY No.2	SPF
BI - BJ	2x4	DRY No.2	SPF
BJ - BK	2x4	DRY No.2	SPF
BK - BL	2x4	DRY No.2	SPF
BL - BM	2x4	DRY No.2	SPF
BM - BN	2x4	DRY No.2	SPF
BN - BO	2x4	DRY No.2	SPF
BO - BP	2x4	DRY No.2	SPF
BP - BQ	2x4	DRY No.2	SPF
BQ - BR	2x4	DRY No.2	SPF
BR - BS	2x4	DRY No.2	SPF
BS - BT	2x4	DRY No.2	SPF
BT - BU	2x4	DRY No.2	SPF
BU - BV	2x4	DRY No.2	SPF
BV - BW	2x4	DRY No.2	SPF
BW - BX	2x4	DRY No.2	SPF
BX - BY	2x4	DRY No.2	SPF
BY - BZ	2x4	DRY No.2	SPF
BZ - CA	2x4	DRY No.2	SPF
CA - CB	2x4	DRY No.2	SPF
CB - CC	2x4	DRY No.2	SPF
CC - CD	2x4	DRY No.2	SPF
CD - CE	2x4	DRY No.2	SPF
CE - CF	2x4	DRY No.2	SPF
CF - CG	2x4	DRY No.2	SPF
CG - CH	2x4	DRY No.2	SPF
CH - CI	2x4	DRY No.2	SPF
CI - CJ	2x4	DRY No.2	SPF
CJ - CK	2x4	DRY No.2	SPF
CK - CL	2x4	DRY No.2	SPF
CL - CM	2x4	DRY No.2	SPF
CM - CN	2x4	DRY No.2	SPF
CN - CO	2x4	DRY No.2	SPF
CO - CP	2x4	DRY No.2	SPF
CP - CQ	2x4	DRY No.2	SPF
CQ - CR	2x4	DRY No.2	SPF
CR - CS	2x4	DRY No.2	SPF
CS - CT	2x4	DRY No.2	SPF
CT - CU	2x4	DRY No.2	SPF
CU - CV	2x4	DRY No.2	SPF
CV - CW	2x4	DRY No.2	SPF
CW - CX	2x4	DRY No.2	SPF
CX - CY	2x4	DRY No.2	SPF
CY - CZ	2x4	DRY No.2	SPF
CZ - DA	2x4	DRY No.2	SPF
DA - DB	2x4	DRY No.2	SPF
DB - DC	2x4	DRY No.2	SPF
DC - DD	2x4	DRY No.2	SPF
DD - DE	2x4	DRY No.2	SPF
DE - DF	2x4	DRY No.2	SPF
DF - DG	2x4	DRY No.2	SPF
DG - DH	2x4	DRY No.2	SPF
DH - DI	2x4	DRY No.2	SPF
DI - DJ	2x4	DRY No.2	SPF
DJ - DK	2x4	DRY No.2	SPF
DK - DL	2x4	DRY No.2	SPF
DL - DM	2x4	DRY No.2	SPF
DM - DN	2x4	DRY No.2	SPF
DN - DO	2x4	DRY No.2	SPF
DO - DP	2x4	DRY No.2	SPF
DP - DQ	2x4	DRY No.2	SPF
DQ - DR	2x4	DRY No.2	SPF
DR - DS	2x4	DRY No.2	SPF
DS - DT	2x4	DRY No.2	SPF
DT - DU	2x4	DRY No.2	SPF
DU - DV	2x4	DRY No.2	SPF
DV - DW	2x4	DRY No.2	SPF
DW - DX	2x4	DRY No.2	SPF
DX - DY	2x4	DRY No.2	SPF
DY - DZ	2x4	DRY No.2	SPF
DZ - EA	2x4	DRY No.2	SPF
EA - EB	2x4	DRY No.2	SPF
EB - EC	2x4	DRY No.2	SPF
EC - ED	2x4	DRY No.2	SPF
ED - EE	2x4	DRY No.2	SPF
EE - EF	2x4	DRY No.2	SPF
EF - EG	2x4	DRY No.2	SPF
EG - EH	2x4	DRY No.2	SPF
EH - EI	2x4	DRY No.2	SPF
EI - EJ	2x4	DRY No.2	SPF
EJ - EK	2x4	DRY No.2	SPF
EK - EL	2x4	DRY No.2	SPF
EL - EM	2x4	DRY No.2	SPF
EM - EN	2x4	DRY No.2	SPF
EN - EO	2x4	DRY No.2	SPF
EO - EP	2x4	DRY No.2	SPF
EP - EQ	2x4	DRY No.2	SPF
EQ - ER	2x4	DRY No.2	SPF
ER - ES	2x4	DRY No.2	SPF
ES - ET	2x4	DRY No.2	SPF
ET - EU	2x4	DRY No.2	SPF
EU - EV	2x4	DRY No.2	SPF
EV - EW	2x4	DRY No.2	SPF
EW - EX	2x4	DRY No.2	SPF
EX - EY	2x4	DRY No.2	SPF
EY - EZ	2x4	DRY No.2	SPF
EZ - FA	2x4	DRY No.2	SPF
FA - FB	2x4	DRY No.2	SPF
FB - FC	2x4	DRY No.2	SPF
FC - FD	2x4	DRY No.2	SPF
FD - FE	2x4	DRY No.2	SPF
FE - FF	2x4	DRY No.2	SPF
FF - FG	2x4	DRY No.2	SPF
FG - FH	2x4	DRY No.2	SPF
FH - FI	2x4	DRY No.2	SPF
FI - FJ	2x4	DRY No.2	SPF
FJ - FK	2x4	DRY No.2	SPF
FK - FL	2x4	DRY No.2	SPF
FL - FM	2x4	DRY No.2	SPF
FM - FN	2x4	DRY No.2	SPF
FN - FO	2x4	DRY No.2	SPF
FO - FP	2x4	DRY No.2	SPF
FP - FQ	2x4	DRY No.2	SPF
FQ - FR	2x4	DRY No.2	SPF
FR - FS	2x4	DRY No.2	SPF
FS - FT	2x4	DRY No.2	SPF
FT - FU	2x4	DRY No.2	SPF
FU - FV	2x4	DRY No.2	SPF
FV - FW	2x4	DRY No.2	SPF
FW - FX	2x4	DRY No.2	SPF
FX - FY	2x4	DRY No.2	SPF
FY - FZ	2x4	DRY No.2	SPF
FZ - GA	2x4	DRY No.2	SPF
GA - GB	2x4	DRY No.2	SPF
GB - GC	2x4	DRY No.2	SPF
GC - GD	2x4	DRY No.2	SPF
GD - GE	2x4	DRY No.2	SPF
GE - GF	2x4	DRY No.2	SPF
GF - GG	2x4	DRY No.2	SPF
GG - GH	2x4	DRY No.2	SPF
GH - GI	2x4	DRY No.2	SPF
GI - GJ	2x4	DRY No.2	SPF
GJ - GK	2x4	DRY No.2	SPF
GK - GL	2x4	DRY No.2	SPF
GL - GM	2x4	DRY No.2	SPF
GM - GN	2x4	DRY No.2	SPF
GN - GO	2x4	DRY No.2	SPF
GO - GP	2x4	DRY No.2	SPF
GP - GQ	2x4	DRY No.2	SPF
GQ - GR	2x4	DRY No.2	SPF
GR - GS	2x4	DRY No.2	SPF
GS - GT	2x4	DRY No.2	SPF
GT - GU	2x4	DRY No.2	SPF
GU - GV	2x4	DRY No.2	SPF
GV - GW	2x4	DRY No.2	SPF
GW - GX	2x4	DRY No.2	SPF
GX - GY	2x4	DRY No.2	SPF
GY - GZ	2x4	DRY No.2	SPF
GZ - HA	2x4	DRY No.2	SPF
HA - HB	2x4	DRY No.2	SPF
HB - HC	2x4	DRY No.2	SPF
HC - HD	2x4	DRY No.2	SPF
HD - HE	2x4	DRY No.2	SPF
HE - HF	2x4	DRY No.2	SPF
HF - HG	2x4	DRY No.2	SPF
HG - HH	2x4	DRY No.2	SPF
HH - HI	2x4	DRY No.2	SPF
HI - HJ	2x4	DRY No.2	SPF
HJ - HK	2x4	DRY No.2	SPF
HK - HL	2x4	DRY No.2	SPF
HL - HM	2x4	DRY No.2	SPF
HM - HN	2x4	DRY No.2	SPF
HN - HO	2x4	DRY No.2	SPF
HO - HP	2x4	DRY No.2	SPF
HP - HQ	2x4	DRY No.2	SPF
HQ - HR	2x4	DRY No.2	SPF
HR - HS	2x4	DRY No.2	SPF
HS - HT	2x4	DRY No.2	SPF
HT - HU	2x4	DRY No.2	SPF
HU - HV	2x4	DRY No.2	SPF
HV - HW	2x4	DRY No.2	SPF
HW - HX	2x4	DRY No.2	SPF
HX - HY	2x4	DRY No.2	SPF
HY - HZ	2x4	DRY No.2	SPF
HZ - IA	2x4	DRY No.2	SPF
IA - IB	2x4	DRY No.2	SPF
IB - IC	2x4	DRY No.2	SPF
IC - ID	2x4	DRY No.2	SPF
ID - IE	2x4	DRY No.2	SPF
IE - IF	2x4	DRY No.2	SPF
IF - IG	2x4	DRY No.2	SPF
IG - IH	2x4	DRY No.2	SPF
IH - II	2x4	DRY No.2	SPF
II - IJ	2x4	DRY No.2	SPF
IJ - IK	2x4	DRY No.2	SPF
IK - IL	2x4	DRY No.2	SPF
IL - IM	2x4	DRY No.2	SPF
IM - IN	2x4	DRY No.2	SPF
IN - IO	2x4	DRY No.2	SPF
IO - IP	2x4	DRY No.2	SPF
IP - IQ	2x4	DRY No.2	SPF
IQ - IR	2x4	DRY No.2	SPF
IR - IS	2x4	DRY No.2	SPF
IS - IT	2x4	DRY No.2	SPF
IT - IU	2x4	DRY No.2	SPF
IU - IV	2x4	DRY No.2	SPF
IV - IV	2x4	DRY No.2	SPF
IV - IV	2x4	DRY No.2	SPF
IV - IV	2x4	DRY No.2	SPF
IV - IV	2x4	DRY No.2	SPF



TOTAL WEIGHT = 61 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - D 2x4 DRY No.2

D - F 2x4 DRY No.2

I - B 2x4 DRY No.2

G - F 2x4 DRY No.2

I - G 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY, SEASONED LUMBER.

DESCR.

SPF

SPF

SPF

SPF

SRF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
I	1004	0	1004	0
G	880	0	880	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
I	708	478 / 0	0 / 0	0 / 0	0 / 0	230 / 0	0 / 0	
G	622	408 / 0	0 / 0	0 / 0	0 / 0	214 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	C-H	-263 / 0	0.11 (1)
B-C	0 / 19	-91.8 -91.8	0.24 (1)	10.00	H-D	0 / 445	0.10 (1)
C-D	-844 / 0	-91.8 -91.8	0.19 (1)	6.25	H-E	-190 / 0	0.08 (1)
D-E	-842 / 0	-91.8 -91.8	0.17 (1)	6.25	I-C	-1167 / 0	0.48 (1)
E-F	0 / 19	-91.8 -91.8	0.22 (1)	10.00	E-G	-1132 / 0	0.41 (1)
I-B	-270 / 0	0.0 0.0	0.03 (1)	7.81			
G-F	-129 / 0	0.0 0.0	0.01 (1)	7.81			
I-H	0 / 952	-18.5 -18.5	0.41 (4)	10.00			
H-G	0 / 892	-18.5 -18.5	0.41 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
 ALLOWABLE DEFL.(TL) = L/360 (0.53")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSK TC=0.24/1.00 (B-C:1), BC=0.41/1.00 (H-I:4),
 WB=0.48/1.00 (C-I:1), SS=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

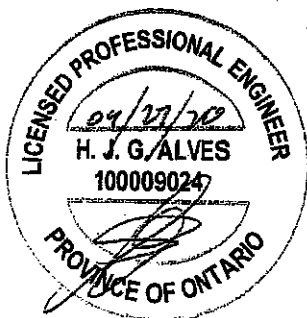
PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (E) (INPUT = 0.90)

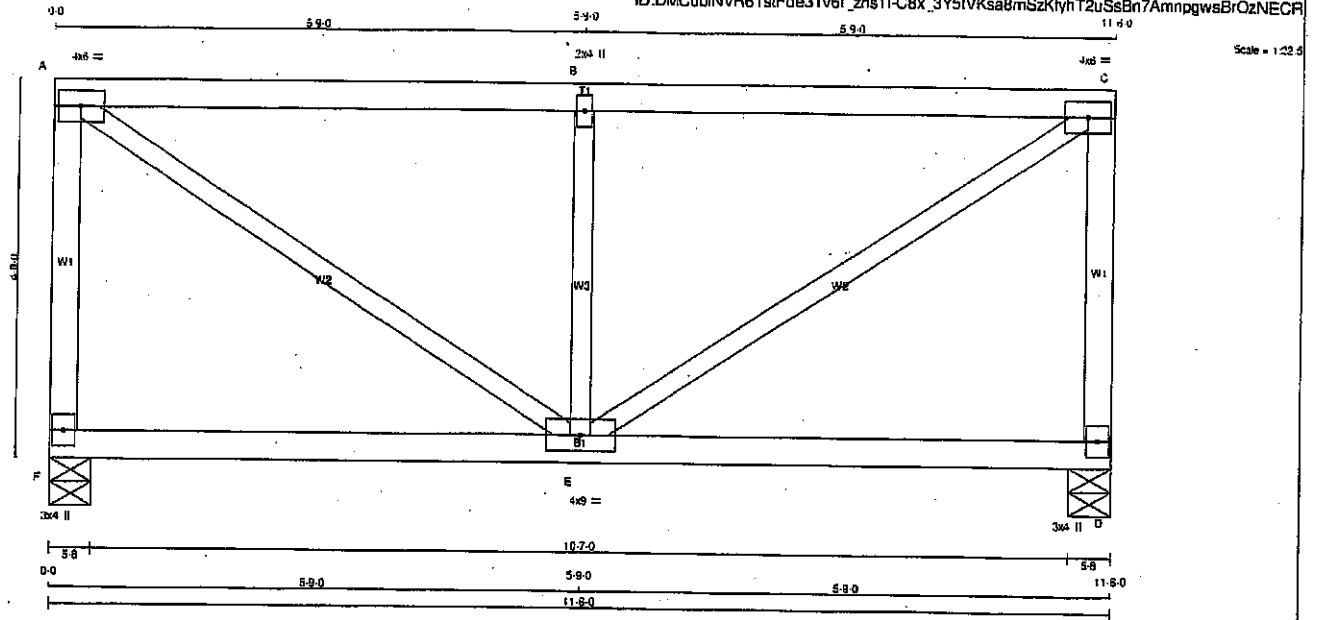
JSI METAL= 0.39 (C) (INPUT = 1.00)



Structural component only
 DWG# T-2007007

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408089	T41	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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ID:DMCubiNVR6TstFoe31v6f_zns11-C8x_3Y5IVKsa8mSzKfytT2uSsBn7AmnpqwsBrOzNECR



TOTAL WEIGHT = 3 X 47 = 141 lb [M/R]

LUMBER					
N. L. G. A. RULES					
CHORDS	SIZE		LUMBER		DESCR
F - A	2x4	DRY	No.2		SPF
A - C	2x4	DRY	No.2		SPF
D - C	2x4	DRY	No.2		SPF
F - D	2x4	DRY	No.2		SPF
ALL WEBS 2x3 DRY			No.2		SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMW-4	MT20	4.0	8.0			
B	TMW-4	MT20	2.0	4.0			
C	TMW-4	MT20	4.0	8.0			
D	BMV-4	MT20	3.0	4.0			
E	BMWV-4	MT20	4.0	8.0			
F	BMV-4	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
F	763	763	5-8	5-8
D	763	763	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS
F	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
F	552	294 / 0 0 / 0 0 / 0 0 / 0 258 / 0 0 / 0
D	552	294 / 0 0 / 0 0 / 0 0 / 0 258 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO									
F-A	-721 / 0	0.0	0.0	0.24 (1)	7.81	A-E	0 / 870	0.28 (1)	
A-B	-731 / 0	-114.3	-114.3	0.68 (1)	4.30	E-B	-815 / 0	0.27 (1)	
B-C	-731 / 0	-114.3	-114.3	0.68 (1)	4.30	E-C	0 / 870	0.26 (1)	
D-C	-721 / 0	0.0	0.0	0.24 (1)	7.81				
F-E	0 / 0	-18.5	-18.5	0.22 (4)	10.00				
E-D	0 / 0	-18.5	-18.5	0.22 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL	15.0	PSF
BOT CH.	LL = 0.0	PSF
DL	7.4	PSF
TOTAL LOAD	= 48.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 085-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.88/1.00 (B-C:1), BC=0.22/1.00 (E-F:4), WB=0.27/1.00 (B-E:1), SS=0.42/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

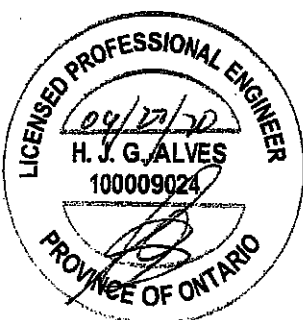
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION	(PSI)	(PL)	(PL)	(PL)
MAX MIN MAX MIN MAX MIN				
MT20	618	354	1667	788
				1987
				1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (A) (INPUT = 0.90)
JSI METAL = 0.27 (C) (INPUT = 1.00)

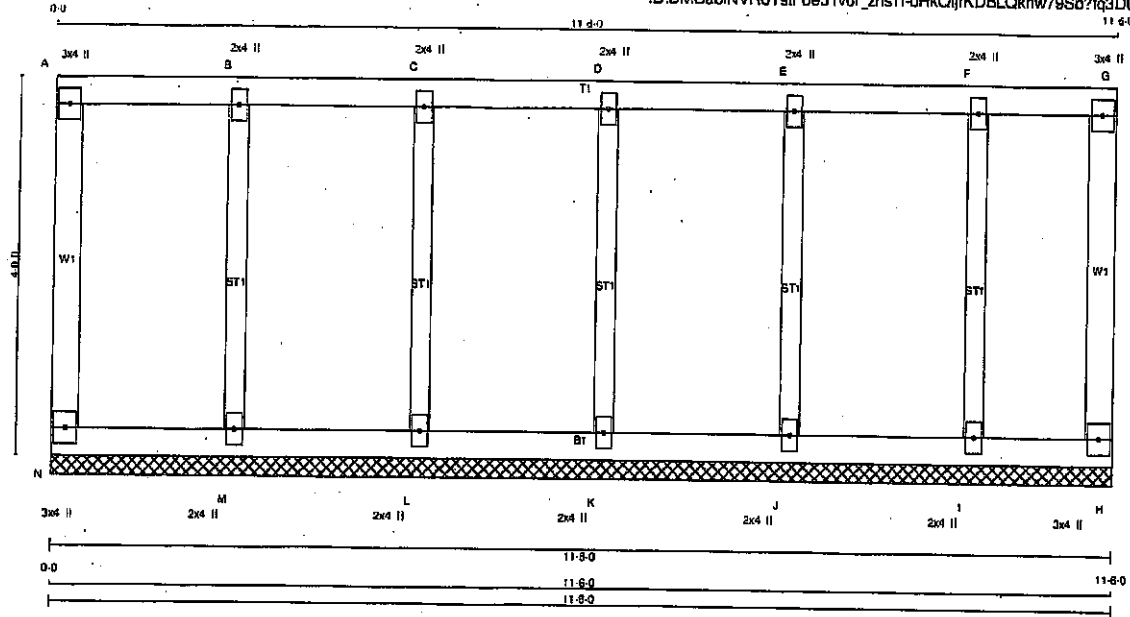


Structural component only
DWG# T-2007008

JOB NAME 408089	TRUSS NAME G41	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	ORWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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ID:DMCubINVR6TstFoe31v6l_zns1l-qHkCtjrKDBLQkhW79Sb2fq3DU7?WUms4p078YzNEC



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
N - A	2x4	DRY	No.2
A - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
N - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - TMV+p	MT20	3.0	4.0		
B, C, D, E, F					
G - TMV+w	MT20	2.0	4.0		
H - TMV+p	MT20	3.0	4.0		
I - BMV1+p	MT20	3.0	4.0		
J, K, L, M					
N - BMV1+w	MT20	2.0	4.0		
O - BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	UNBRACED LENGTH (FT)	LC1 MAX (LC)
N-A	-100 / 0	0.0	0.0 (1)	7.81	M-B	-244 / 0	0.08 (1)
A-B	-5 / 0	-114.3	-114.3 (1)	10.00	L-C	-226 / 0	0.07 (1)
B-C	-5 / 0	-114.3	-114.3 (1)	10.00	K-D	-228 / 0	0.07 (1)
C-D	-5 / 0	-114.3	-114.3 (1)	10.00	J-E	-234 / 0	0.08 (1)
D-E	-5 / 0	-114.3	-114.3 (1)	10.00	I-F	-207 / 0	0.07 (1)
E-F	-5 / 0	-114.3	-114.3 (1)	10.00			
F-G	-5 / 0	-114.3	-114.3 (1)	10.00			
H-G	-74 / 0	0.0	0.0 (1)	7.81			
N-M	0 / 5	-18.5	-18.5 (4)	10.00			
M-L	0 / 5	-18.5	-18.5 (4)	10.00			
L-K	0 / 5	-18.5	-18.5 (4)	10.00			
K-J	0 / 5	-18.5	-18.5 (4)	10.00			
J-I	0 / 5	-18.5	-18.5 (4)	10.00			
I-H	0 / 5	-18.5	-18.5 (4)	10.00			

TOTAL WEIGHT = 48 lb

(M/R)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 15.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 48.0	PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (B-C:1), BC=0.02/1.00 (M-N:4), WB=0.08/1.00 (B-M:1), SS=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00 FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	(PS)	(PL)	(PL)	SECTION
MT20	618	354	1987	788	1987 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (M) (INPUT = 0.90) JSI METAL= 0.07 (M) (INPUT = 1.00)

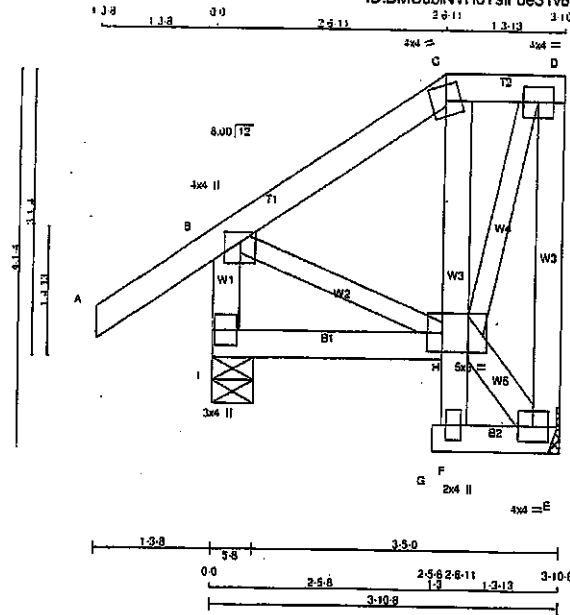


Structural component only
DWG# T-2006992

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408089	T43S	1	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4	DRY No.2
C - D	2x4	DRY No.2
E - D	2x4	DRY No.2
I - B	2x4	DRY No.2
I - H	2x4	DRY No.2
G - E	2x4	DRY No.2

ALL WEBS	2x4	DRY No.2
EXCEPT		
H - D	2x3	DRY No.2
B - H	2x3	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B - TMVW+p	MT20	4.0	4.0	1.25	2.00
C - TTW-m	MT20	4.0	4.0		
D - TMVW-i	MT20	4.0	4.0		
E - BMVW1-i	MT20	4.0	4.0		
F - BMVW-w	MT20	2.0	4.0		
H - BMWWWW+i	MT20	5.0	8.0	2.50	2.00
I - BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
E	216	0	216	0
I	341	0	341	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	153	98/0	0/0	0/0	0/0	0/0	54/0	0/0
I	239	169/0	0/0	0/0	0/0	0/0	89/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORIZ. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					MEMB.		
A-B	0/35	-91.8	-91.8	0.14 (5)	H-E	-10/0	0.00 (1)
B-C	-81/0	-91.8	-91.8	0.10 (1)	H-D	0/152	0.03 (1)
C-D	-61/0	-91.8	-91.8	0.03 (1)	F-H	0/17	0.01 (1)
E-D	-199/0	0.0	0.0	0.05 (1)	H-C	-133/0	0.01 (1)
I-B	-316/0	0.0	0.0	0.03 (1)	B-H	0/73	0.02 (1)
I-H	0/0	-18.5	-18.5	0.04 (4)			
G-F	0/0	-18.5	-18.5	0.00 (4)			
F-E	0/7	-18.5	-18.5	0.01 (4)			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 25 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 8.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 066-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (A-B-S), BC=0.04/1.00 (H-I-K), WB=0.03/1.00 (D-H-I), SS=0.08/1.00 (A-B-S)

DDL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

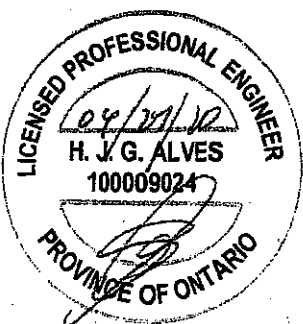
NAIL VALUES

PLATE GRIP(DRY)	(PSI)	(PL)	(PL)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667	798	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

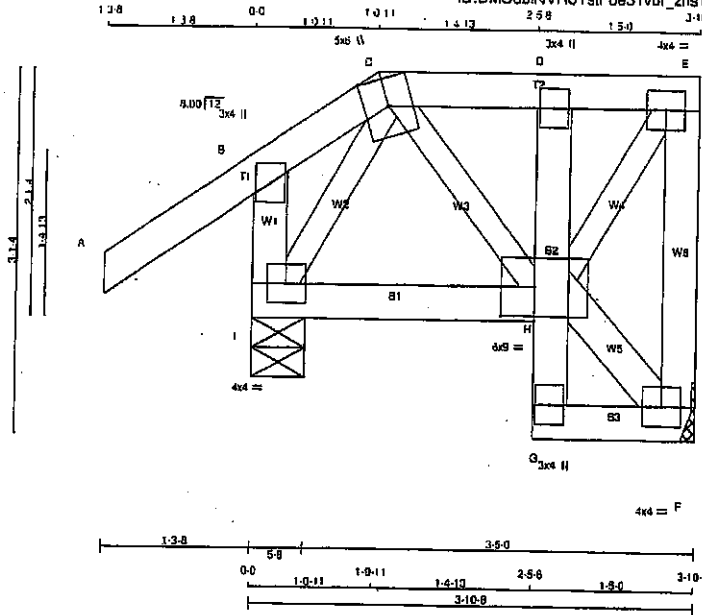


Structural component only
DWG# T-2007009

JOB NAME 408089	TRUSS NAME T44S	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4 DRY	No.2
C - E	2x4 DRY	No.2
F - H	2x4 DRY	No.2
I - B	2x4 DRY	No.2
I - H	2x4 DRY	No.2
G - D	2x4 DRY	No.2
G - F	2x4 DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT
H - F 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	Edge	
D	TMV+p	MT20	3.0	4.0		
E	TMVW+I	MT20	4.0	4.0		
F	BMVW+I	MT20	4.0	4.0		
G	BMV+p	MT20	3.0	4.0		
H	BMVWVW+I	MT20	6.0	9.0	3.00	3.50
I	BMVW+I	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	191	0	191	0
I	362	0	362	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	136	87/-12	0/0	0/0	0/0	49/0	0/0
I	253	182/0	0/0	0/0	0/0	71/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED VERT. LOAD LC1 MAX (PLF)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO
A-B	0/35	-91.8	-91.8	0.14 (5)	10.00	C-H	0/86	0.01 (1)
B-C	-44/0	-91.8	-91.8	0.13 (5)	6.25	H-F	-9/0	0.00 (1)
C-D	-93/0	-91.8	-91.8	0.03 (1)	6.25	H-E	0/155	0.03 (1)
D-E	-89/0	-91.8	-91.8	0.02 (1)	6.25	I-C	-97/38	0.01 (1)
E-F	-174/0	0.0	0.0	0.03 (1)	7.81			
F-B	-255/0	0.0	0.0	0.03 (1)	7.81			
I-H	-20/50	-18.5	-18.5	0.04 (4)	6.25			
G-H	0/13	0.0	0.0	0.01 (1)	10.00			
H-D	-147/0	0.0	0.0	0.01 (1)	7.81			
G-F	0/7	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 24 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/998 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/998 (0.00")

CSF: TC=0.14/1.00 (A-B:5), BC=0.04/1.00 (H-E:4), WB=0.03/1.00 (E-H:1), SS=0.09/1.00 (A-B:5)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

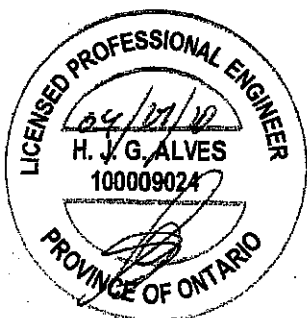
PLATE GRIP(DRY) SHEAR SECTION	(PSI)	(PL)	(PL)
MAX MIN MAX MIN MAX MIN			
MT20	818	354	1657 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.27 (H) INPUT = 0.90

JSI METAL = 0.09 (B) INPUT = 1.00

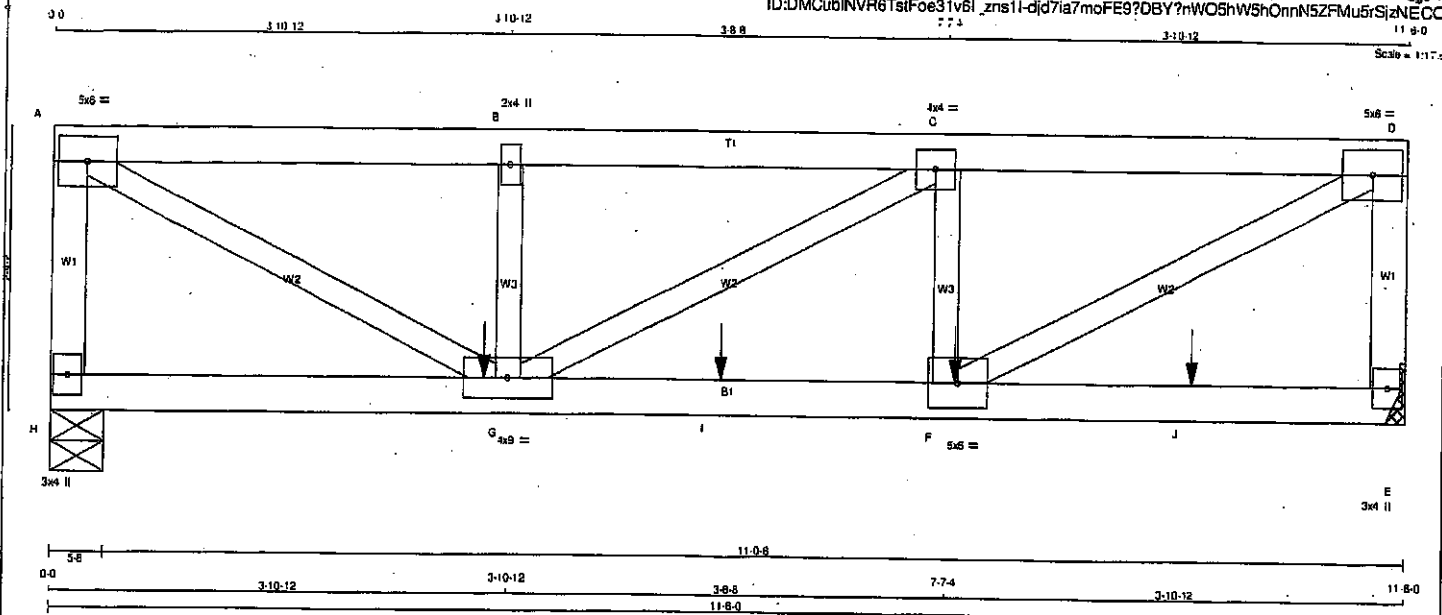


Structural component only
DWG# T-2007010

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408089	T45	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6l_zns11-djd7ia7moFE9?DBY?mW0ShWShOnnN5ZFmu8rSizNECO



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
H - A	2x4	DRY	No.2
A - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
H - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
H-A	1	12
A-D	1	12
D-E	1	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
H-E	1	12
WEBS : (0.122"x3") SPIRAL NAILS		
F-C	1	6
G-B	1	6
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)					
JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	5.0	6.0	
B	TMVW-w	MT20	2.0	4.0	
C	TMVW-1	MT20	4.0	4.0	
D	TMVW-1	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
H	1429	0	1429	0
E	1488	0	1488	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 2-0.

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED
H	1038
E	1086

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
H-A	-1355 / 0	0.0 0.0	0.12 (1)	7.81 F-D	0 / 2354	0.39 (1)	
A-B	-2030 / 0	-175.1 -175.1	0.27 (1)	5.34 A-G	0 / 2295	0.38 (1)	
B-C	-2030 / 0	-175.1 -175.1	0.25 (1)	5.26 F-C	-694 / 0	0.08 (1)	
C-D	-2083 / 0	-175.1 -175.1	0.27 (1)	5.18 G-B	-721 / 0	0.08 (1)	
E-D	-1383 / 0	0.0 0.0	0.12 (1)	7.81 G-C	-60 / 0	0.01 (1)	
H-G	0 / 0	-43.5 -43.5	0.10 (4)	10.00			
G-I	0 / 2083	-43.5 -43.5	0.35 (1)	10.00			
I-F	0 / 2083	-43.5 -43.5	0.35 (1)	10.00			
F-J	0 / 0	-43.5 -43.5	0.14 (1)	10.00			
J-E	0 / 0	-43.5 -43.5	0.14 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	7-8-4	-103	-103		FRONT	VERT	TOTAL		C1
G	3-8-4	-103	-103		FRONT	VERT	TOTAL		C1
I	5-8-4	-103	-103		FRONT	VERT	TOTAL		C1
J	9-8-4	-103	-103		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 43 = 85 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 15.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 48.0 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

GIRDER TYPE: CSldGirder
START DISTANCE = 0-0
START SPAN CARRIED = 4-0-0
END DISTANCE = 11-8-0
END SPAN CARRIED = 4-0-0
END WALL WIDTH = 2-0
APPLIED TO FRONT SIDE OF TOP CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CS: TC=0.27/1.00 (C-D:1), BC=0.35/1.00 (F-G:1), WB=0.39/1.00 (D-F:1), SS=0.23/1.00 (C-D:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1587 1656

CONTINUED ON PAGE 2



Structural component only
DWG# T-2007011

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408089	T45	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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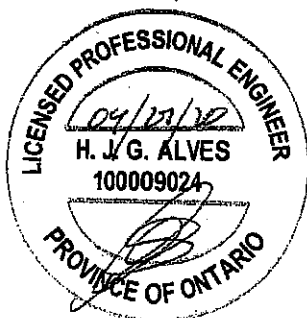
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	BMV1+p	MT20	3.0	4.0		
F	BMWW-1	MT20	5.0	6.0		
G	BMWWW-1	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

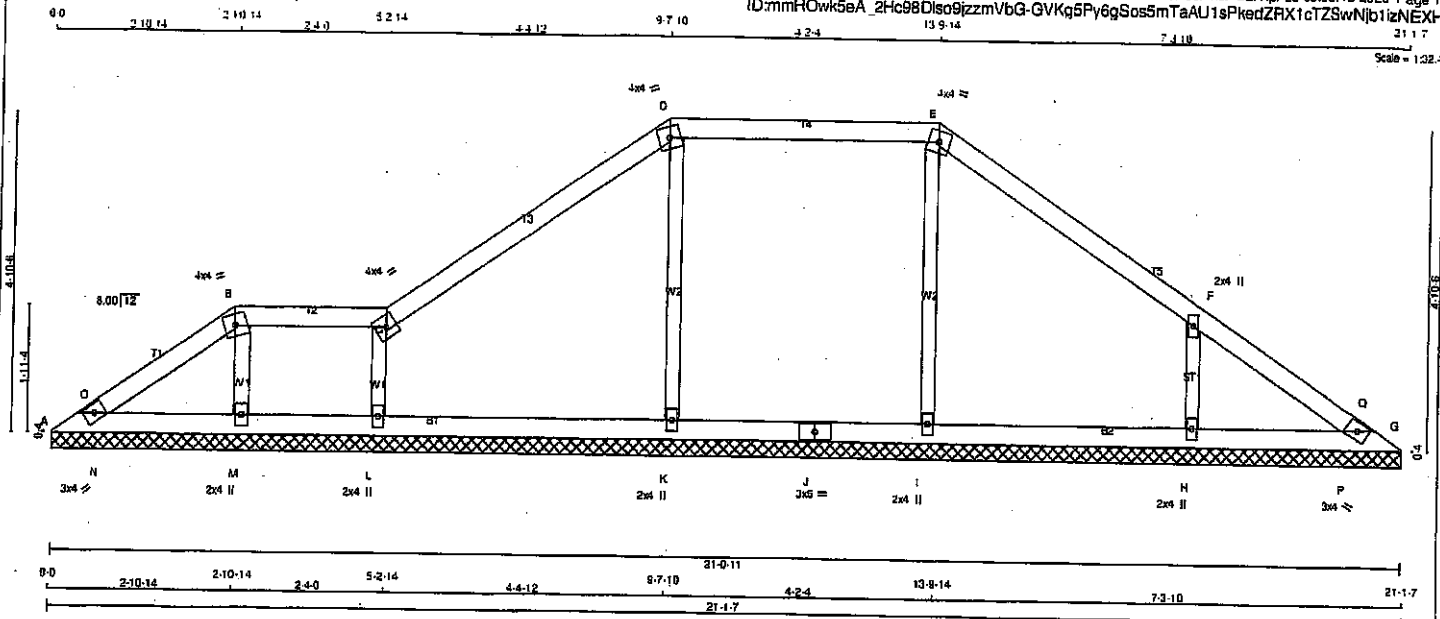
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.81 (G) (INPUT = 0.90)
JSI METAL = 0.35 (D) (INPUT = 1.00)



Structural component only
DWG# T-2007011



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - B	2x4	DRY No.2
B - C	2x4	DRY No.2
C - D	2x4	DRY No.2
D - E	2x4	DRY No.2
E - G	2x4	DRY No.2
A - J	2x4	DRY No.2
J - G	2x4	DRY No.2
ALL WEBS	2x3	DRY No.2
DRY: SEASONED LUMBER.		

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B, D, E						
B	TTW-m	MT20	4.0	4.0		
C	TTW-h	MT20	4.0	4.0		
F	TBW-w	MT20	2.0	4.0		
G	TBM1-h	MT20	3.0	4.0		
H, I, K, L, M						
H	BMW1-w	MT20	2.0	4.0		
J	BS-1	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
A	147	0	147	0	21-0-11 (9-1-21)0-11	
G	130	0	130	0	21-0-11 (9-1-21)0-11	
H	490	0	490	0	21-0-11 (9-1-21)0-11	
K	477	0	477	0	21-0-11 (9-1-21)0-11	
L	363	0	363	0	21-0-11 (9-1-21)0-11	
M	305	0	305	0	21-0-11 (9-1-21)0-11	
I	410	0	410	0	21-0-11 (9-1-21)0-11	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LC CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	104	70/0	0/0	0/0	0/0	33/0	0/0
G	81	52/0	0/0	0/0	0/0	29/0	0/0
H	348	231/0	0/0	0/0	0/0	115/0	0/0
K	338	217/0	0/0	0/0	0/0	121/0	0/0
L	257	167/0	0/0	0/0	0/0	90/0	0/0
M	216	145/0	0/0	0/0	0/0	71/0	0/0
I	281	185/0	0/0	0/0	0/0	108/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G, H, K, L, M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-O	-5/3	-91.8 -91.8 0.02 (1)	10.00	H-F	-400/0	0.06 (1)	
O-B	0/12	-91.8 -91.8 0.08 (1)	10.00	K-D	-392/0	0.14 (1)	
B-C	0/17	-91.8 -91.8 0.09 (1)	10.00	L-C	-311/0	0.05 (1)	
C-D	-4/9	-91.8 -91.8 0.30 (1)	10.00	M-B	-213/0	0.03 (1)	
D-E	0/9	-91.8 -91.8 0.27 (1)	10.00	E	-341/0	0.12 (1)	
E-F	-23/5	-91.8 -91.8 0.21 (1)	6.25	N-O	-103/0	0.00 (1)	
F-G	0/20	-91.8 -91.8 0.21 (1)	10.00	P-Q	-33/6	0.00 (1)	
Q-G	-38/0	-91.8 -91.8 0.04 (1)	6.25				
A-N	-18/0	-18.5 -18.5 0.09 (1)	6.25				
N-M	-8/0	-18.5 -18.5 0.09 (1)	10.00				
M-L	-17/0	-18.5 -18.5 0.06 (4)	6.25				
L-K	-7/3	-18.5 -18.5 0.07 (4)	10.00				
K-J	-9/0	-18.5 -18.5 0.07 (4)	10.00				
J-I	-9/0	-18.5 -18.5 0.07 (4)	10.00				
I-H	-7/2	-18.5 -18.5 0.06 (4)	10.00				
H-P	-4/19	-18.5 -18.5 0.06 (4)	10.00				
P-G	-8/13	-18.5 -18.5 0.05 (1)	10.00				

TOTAL WEIGHT = 61 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.30/1.00 (C-D:1), BC=0.09/1.00 (A-N:1),
WB=0.14/1.00 (D-K:1), SSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS B&ND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

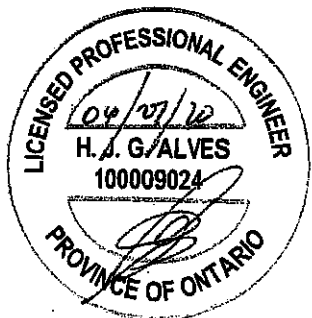
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI)	(PL)	(PL)
MAX MIN MAX MIN		
MT20 616 354 1667 769 1987 1658		

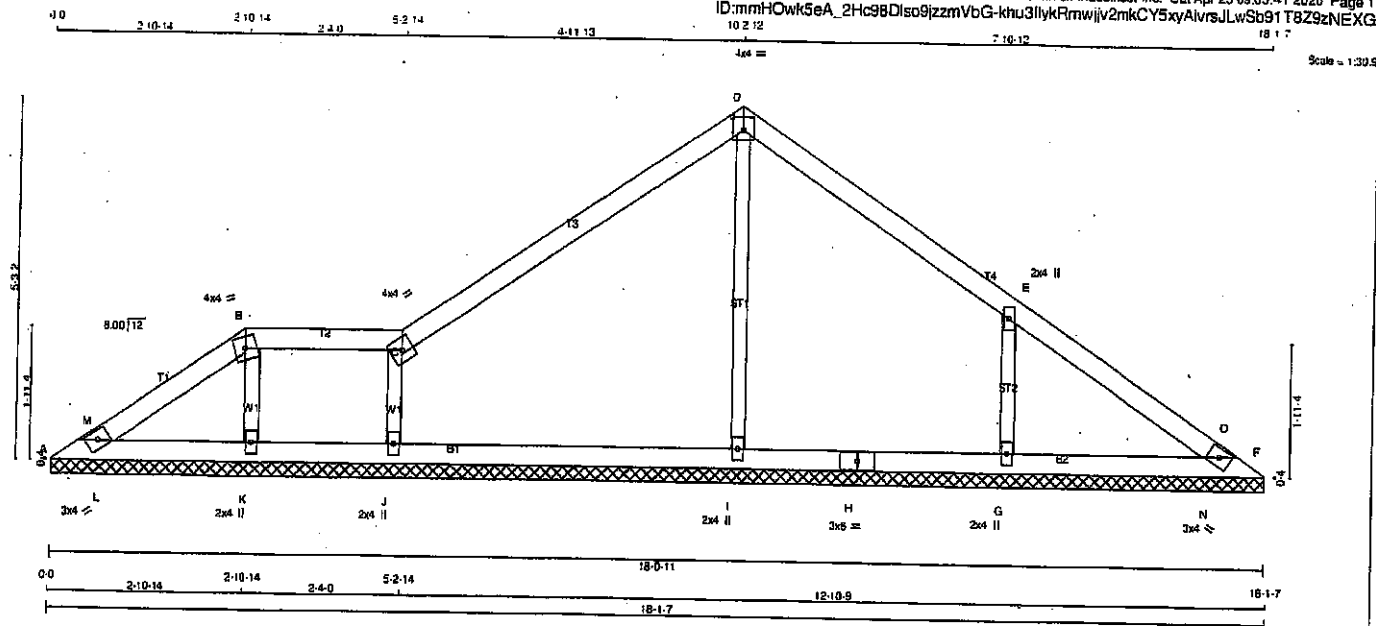
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (C) (INPUT = 0.90)
JSI METAL= 0.20 (F) (INPUT = 1.00)



JOB NAME 408088	TRUSS NAME V21	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 9.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 09:03:41 2020 Page 1	



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY No.2	SPF
B - C	2x4	DRY No.2	SPF
C - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
A - H	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TTW-m	MT20	4.0	4.0		
C	TTW-h	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMW-w	MT20	2.0	4.0		
F	TBM1-h	MT20	3.0	4.0		
G, I, J, K	BMW1-w	MT20	2.0	4.0		
H	BS-t	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	VERT	HORZ	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
			DOWN	HORZ	DOWN	HORZ		
A	136	0	136	0	0	0	18-0-11 (11-118-9-11)	
F	154	0	154	0	0	0	18-0-11 (11-118-9-11)	
I	482	0	482	0	0	0	18-0-11 (11-118-9-11)	
G	519	0	519	0	0	0	18-0-11 (11-118-9-11)	
K	310	0	310	0	0	0	18-0-11 (11-118-9-11)	
J	389	0	389	0	0	0	18-0-11 (11-118-9-11)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN. COMPONENT REACTIONS					SOIL
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	
A	98	68 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
F	109	74 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0
I	343	210 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0
G	366	247 / 0	0 / 0	0 / 0	0 / 0	119 / 0	0 / 0
K	218	149 / 0	0 / 0	0 / 0	0 / 0	70 / 0	0 / 0
J	278	178 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F, I, G, K, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	FR-TO	CHORDS		WEBS	
		MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)
A-M	0 / 18	-91.8	-91.8 0.03 (1)	10.00	10.00
M-B	0 / 33	-91.8	-91.8 0.08 (1)	10.00	10.00
B-C	0 / 38	-91.8	-91.8 0.09 (1)	10.00	10.00
C-D	0 / 21	-91.8	-91.8 0.39 (1)	10.00	10.00
D-E	-2 / 14	-91.8	-91.8 0.23 (1)	10.00	10.00
E-O	0 / 39	-91.8	-91.8 0.23 (1)	10.00	10.00
O-F	-15 / 5	-91.8	-91.8 0.04 (1)	6.25	6.25
A-L	-39 / 0	-18.5	-18.5 0.09 (1)	8.25	8.25
L-K	-24 / 0	-18.5	-18.5 0.09 (1)	8.25	8.25
K-J	-38 / 0	-18.5	-18.5 0.07 (4)	8.25	8.25
J-I	-17 / 0	-18.5	-18.5 0.06 (4)	8.25	8.25
I-H	-17 / 0	-18.5	-18.5 0.08 (4)	8.25	8.25
H-G	-17 / 0	-18.5	-18.5 0.08 (4)	8.25	8.25
G-N	-12 / 0	-18.5	-18.5 0.08 (1)	8.25	8.25
N-F	-18 / 0	-18.5	-18.5 0.08 (1)	8.25	8.25

TOTAL WEIGHT = 51 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL	6.0	PSF
BOT CH.	LL = 0.0	PSF
DL	7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.39/1.00 (C-D-1), 9C=0.09/1.00 (K-L-1), WB=0.16/1.00 (D-I-1), SSI=0.15/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

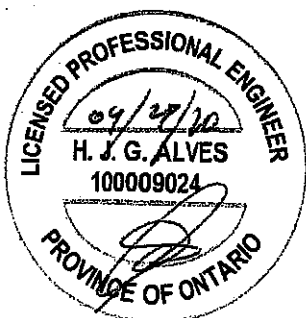
NAIL VALUES

PLATE	GRIP (DRY)	GRIP (WET)	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667

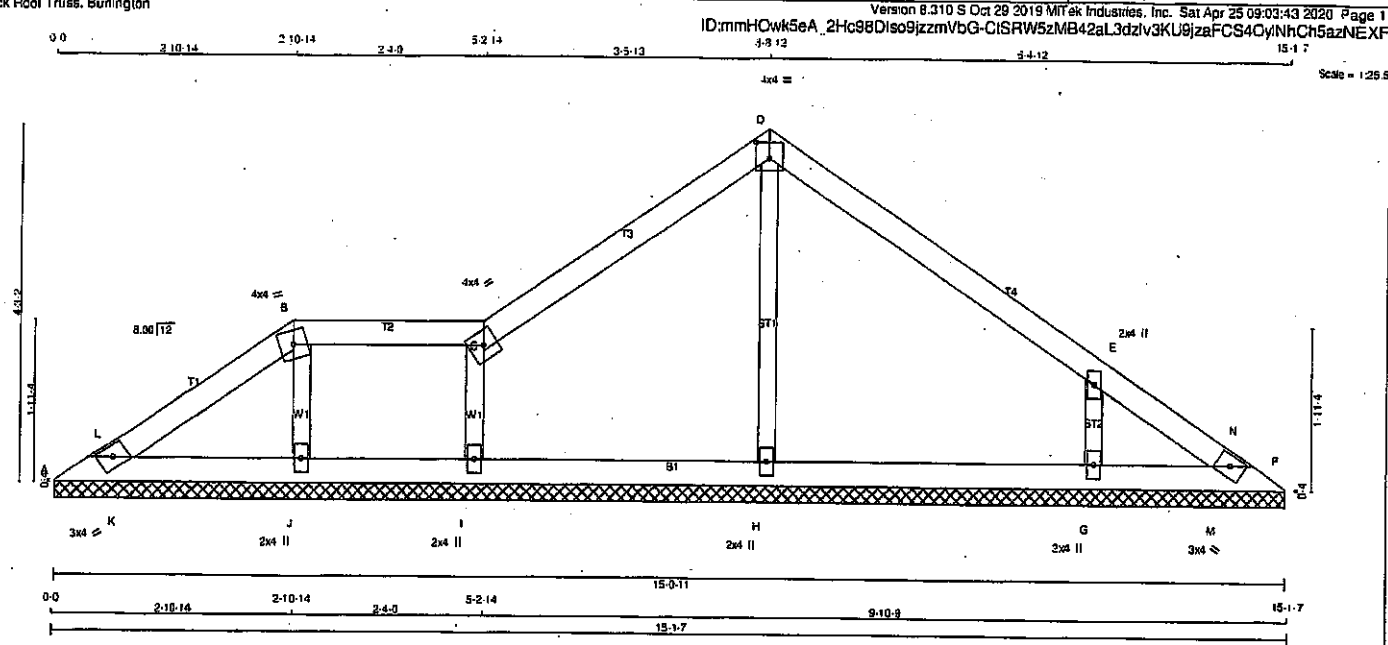
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.66 (C) (INPUT = 0.90)
JSI METAL = 0.21 (E) (INPUT = 1.00)



Structural component only
DWG# T-2006985



TOTAL WEIGHT = 42 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
A - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

DRY, SEASONED LUMBER.

PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - TBW1-h	MT20	3.0	4.0		
B - TTW-m	MT20	4.0	4.0		
C - TTW-h	MT20	4.0	4.0		
D - TTW-p	MT20	4.0	4.0	2.25	2.00
E - TWW-w	MT20	2.0	4.0		
F - TBW1-h	MT20	3.0	4.0		
G, H, I, J					
G - BW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
A	151	0	151	0	15-0-11	15-0-11
F	73	0	73	0	15-0-11	15-0-11
H	377	0	377	0	15-0-11	15-0-11
G	444	0	444	0	15-0-11	15-0-11
I	306	0	306	0	15-0-11	15-0-11
J	309	0	309	0	15-0-11	15-0-11

UNFACTORED REACTIONS							
1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	108	72 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0
F	52	35 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
H	289	184 / 0	0 / 0	0 / 0	0 / 0	104 / 0	0 / 0
G	313	211 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0
I	218	144 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0
J	219	143 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F, H, G, I, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)		MAX. CSI (LC)	UNBRACED LENGTH	MEMB. FORCE (LBS)	MAX. FACTORED CSI (LC)
		FROM	TO				
A-L	-15 / 0	-91.8	-91.8	0.02 (1)	6.25	H-D	-298 / 0
L-B	-2 / 7	-91.8	-91.8	0.08 (1)	10.00	G-E	-399 / 0
B-C	0 / 8	-91.8	-91.8	0.08 (1)	10.00	I-C	-274 / 0
C-D	-12 / 4	-91.8	-91.8	0.19 (1)	6.25	J-B	-207 / 0
D-E	-32 / 0	-91.8	-91.8	0.20 (1)	6.25	K-L	-105 / 0
E-N	0 / 21	-91.8	-91.8	0.20 (1)	10.00	M-N	0 / 28
N-F	-50 / 0	-91.8	-91.8	0.03 (1)	6.25		
A-K	-10 / 0	-18.5	-18.5	0.09 (1)	10.00		
K-J	-4 / 5	-18.5	-18.5	0.09 (1)	10.00		
J-I	-8 / 0	-18.5	-18.5	0.05 (1)	10.00		
I-H	-3 / 10	-18.5	-18.5	0.06 (4)	10.00		
H-G	-3 / 10	-18.5	-18.5	0.06 (4)	10.00		
G-M	0 / 35	-18.5	-18.5	0.05 (4)	10.00		
M-F	0 / 37	-18.5	-18.5	0.01 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.20/1.00 (E-N:1), BC=0.09/1.00 (J-K:1), WB=0.09/1.00 (D-H:1), SS=0.15/1.00 (D-E:1)

DDL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
618 354	1667 788	1987 1858

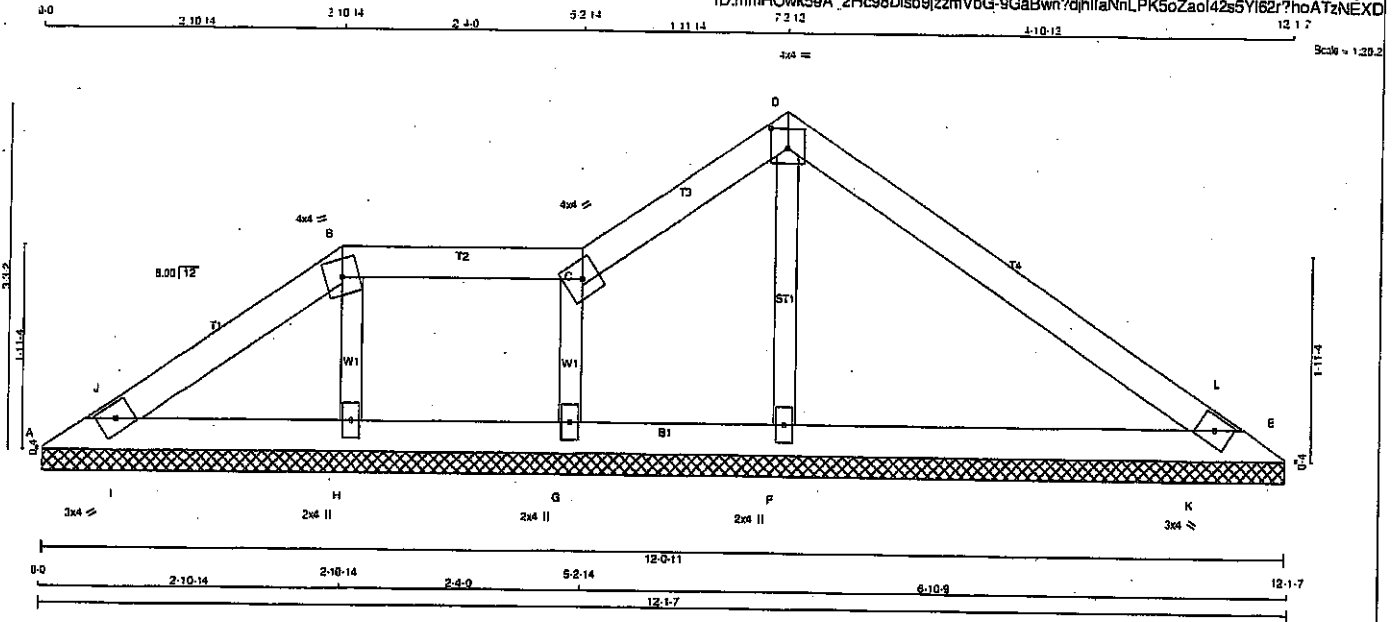
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.27 (C) (INPUT = 0.30)

JSI METAL = 0.20 (E) (INPUT = 1.00)





LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF TOTAL WEIGHT = 33 lb

CHORDS	SIZE	LUMBER	DESCR.	SPF
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TBMT-h	MT20	3.0	4.0			
B	TTW-m	MT20	4.0	4.0			
C	TTW-h	MT20	4.0	4.0			
D	TTW-p	MT20	4.0	4.0	2.25	2.00	
E	TBMT-h	MT20	3.0	4.0			
F, G, H	BMW1-w	MT20	2.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
A	41	0	41	0
E	122	0	122	0
F	720	0	720	0
H	433	0	433	0
G	13	0	13	0

PROVIDE ANCHORAGE AT BEARING JOINT G FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
A	29	19/0 0/0 0/0 0/0 10/0 0/0
E	88	58/0 0/0 0/0 0/0 29/0 0/0
F	510	331/0 0/0 0/0 0/0 179/0 0/0
H	306	200/0 0/0 0/0 0/0 108/0 0/0
G	8	9/0 0/0 0/0 0/0 0/-1 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, F, H, G

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
MEMB.				MEMB.		
FR-TO		FROM TO		FR-TO		
A-J	0/212	-91.8 -91.8 0.04 (1)	10.00	F-D	-541/0	0.10 (1)
J-B	0/227	-91.8 -91.8 0.10 (1)	10.00	H-B	-329/0	0.05 (1)
B-C	0/210	-91.8 -91.8 0.12 (1)	10.00	G-C	-82/9	0.01 (1)
C-D	0/247	-91.8 -91.8 0.10 (1)	10.00	I-J	-101/0	0.00 (1)
D-L	0/242	-91.8 -91.8 0.27 (1)	10.00	K-L	-247/0	0.00 (1)
L-E	0/251	-91.8 -91.8 0.10 (1)	10.00			
A-I	-203/0	-18.5 -18.5 0.06 (1)	6.25			
I-H	-189/0	-18.5 -18.5 0.08 (1)	6.25			
H-G	-210/0	-18.5 -18.5 0.05 (1)	6.25			
G-F	-208/0	-18.5 -18.5 0.14 (1)	6.25			
F-K	-208/0	-18.5 -18.5 0.21 (1)	6.25			
K-E	-244/0	-18.5 -18.5 0.21 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.8 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, CBC 2012, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 088-14
 - TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.27/1.00 (D-L:1), BC=0.21/1.00 (F-K:1),
 WB=0.10/1.00 (D-F:1), SSI=0.15/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

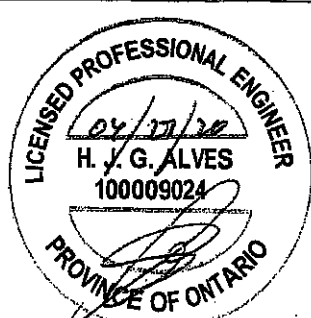
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PL) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 788 1987 1658

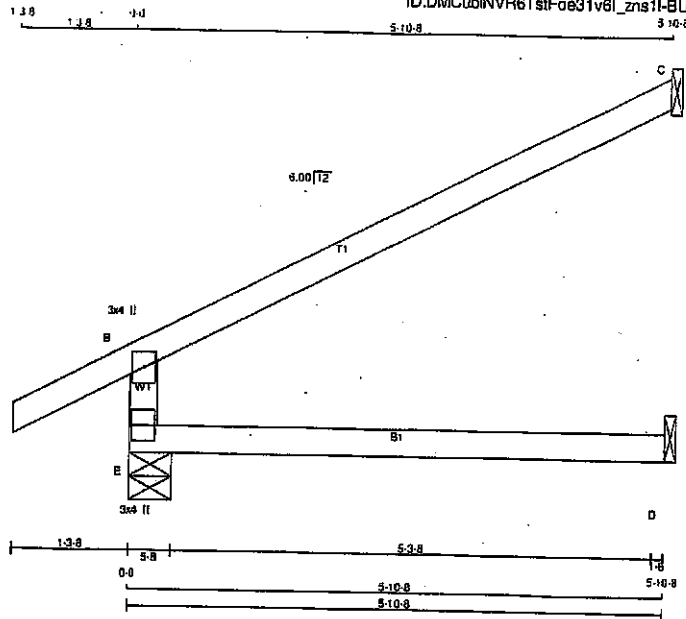
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.45 (D) (INPUT = 0.90)
 JSI METAL= 0.18 (D) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408087	J1	21	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
TRUSS DESC.					



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
E BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
E	525	0	0	5-8
C	202	0	0	1-8
D	45	0	0	1-8

SEE MITEK STANDARD DETAIL B97781H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX MIN. SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	389	257/0	0/0	0/0	0/0	111/0	0/0
C	139	113/0	0/0	0/0	0/0	26/0	0/0
D	36	0/0	0/0	0/0	0/0	38/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO							
E-B	-461/0	0.0	0.0	0.13 (4)	7.81		
A-B	0/28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-30/0	-91.8	-91.8	0.54 (1)	6.25		
E-D	0/0	-18.5	-18.5	0.13 (4)	10.00		

TOTAL WEIGHT = 21 X 17 = 353 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CS: TC=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (A-B:0), SS=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

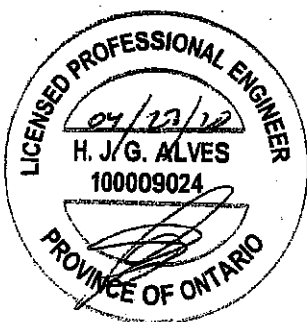
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667	768	1967 1656

PLATE PLACEMENT TOL. = 0.250 inches

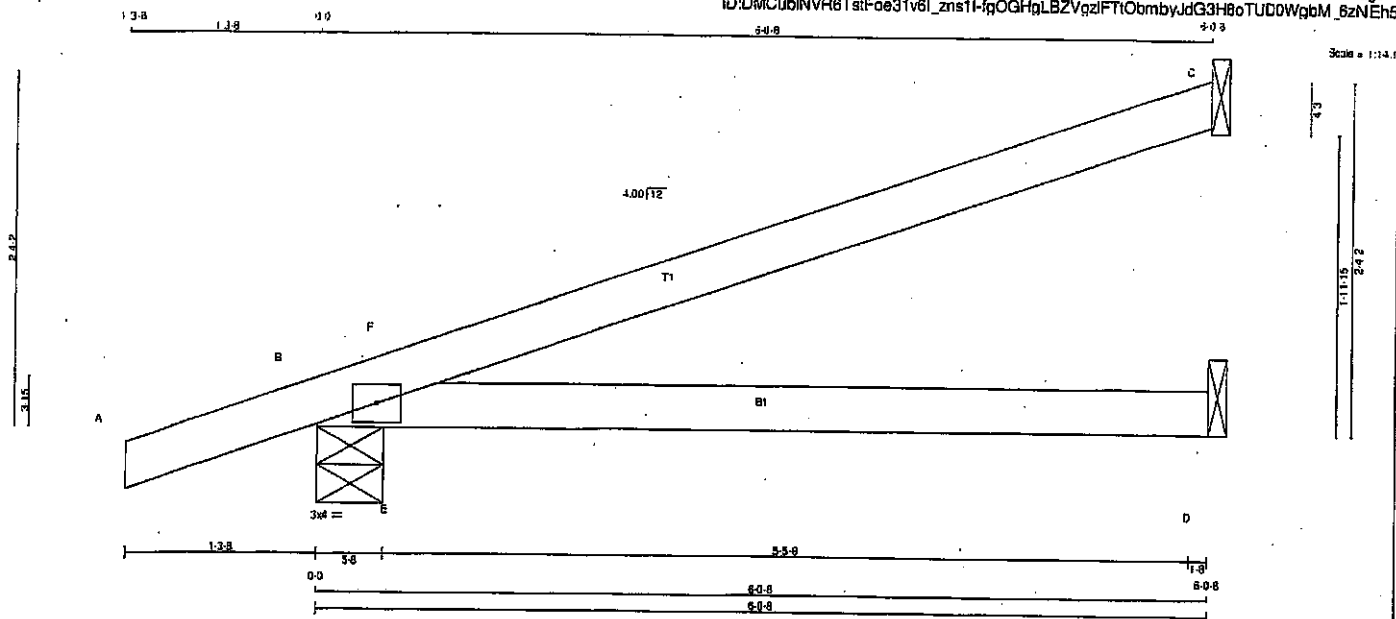
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.19 (E) (INPUT = 0.80)
JSI METAL = 0.13 (B) (INPUT = 1.00)



Structural component only
DWG# T-2006949

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408087	J2	9	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 08:53:12 2020 Page 1					
ID:DMCubINVR6TstFoe31v6I_zns1I-fpOGHgLBZVqzFTtObmbyJdG3H8oTUDOWqbm_6zNEH5					
Scale = 1:14.1					



LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
C	242	0	242	0	1-8	1-8
B	458	0	458	0	5-8	5-8
D	91	0	91	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
C	188	131/0	0/0	0/0	0/0	37/0	0/0
B	320	223/0	0/0	0/0	0/0	97/0	0/0
D	66	24/0	0/0	0/0	0/0	44/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MAX. CSI (LC)	UNBRACED LENGTH (FT)	MEMB. FORCE (LBS)	MAX. CSI (LC)
	FR-TO	FROM-TO	FR-TO	FROM-TO				
A-B	0/18	-91.8	-91.8	0.11 (1)	10.00	E-F	-345/13	0.00 (1)
B-F	-20/22	-91.8	-91.8	0.08 (4)	6.25			
F-C	-2/2	-91.8	-91.8	0.43 (1)	10.00			
B-E	0/0	-18.5	-18.5	0.30 (1)	10.00			
E-D	0/0	-18.5	-18.5	0.30 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/826$ (0.08")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/452$ (0.18")

CSI: TC=0.43/1.00 (C-F:1), BC=0.30/1.00 (D-E:1),
WB=0.00/1.00 (E-F:1), SSI=0.29/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

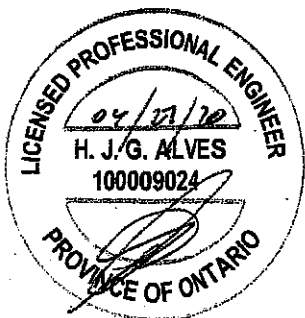
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

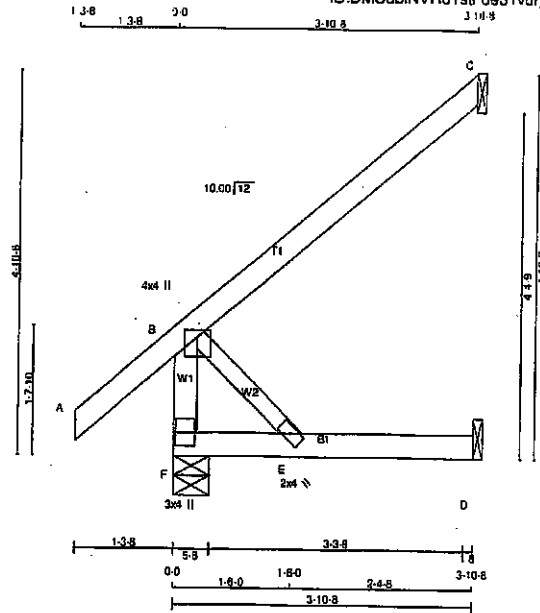
JSI GRIP= 0.30 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2006950

JOB NAME 408088	TRUSS NAME J20	QUANTITY 5	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

ID:DMCubiNVR6TstFos31v8f_zns11-dBc8ZJoaUmHZfwZGTzp2wC8aecQMo4h9WpybPzNEXT



Scale = 1:27.0

LUMBER

N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS			
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMVW+p	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
F	341	341	5-8	5-8
C	178	178	1-8	1-8
D	38	40	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	239	170 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0
C	122	99 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
F-B	-305 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)	
A-B	0 / 41	-91.8	-91.8	0.14 (5)	10.00				
B-C	0 / 0	-91.8	-91.8	0.23 (1)	10.00				
F-E	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
E-D	0 / 0	-18.5	-18.5	0.08 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 5 X 15 = 75 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 8.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.23/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PS)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1887 788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.24 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)

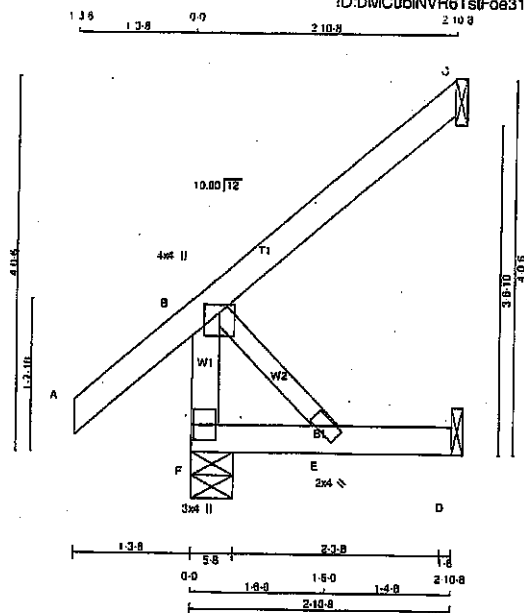


Structural component only
DWG# T-2006974

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408088	J21	5	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/2" = 1'-0"

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMVW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	MAXIMUM FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
F	286	0	286	0	5-8	5-8
C	132	0	132	0	1-8	1-8
D	27	0	30	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. SNOW	MAX/MIN. LIVE	MAX/MIN. PERM. LIVE	MAX/MIN. WIND	MAX/MIN. DEAD	MAX/MIN. SOIL
F	200	144 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
C	91	74 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	21	0 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	FACTORED LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO									
F-B		-269 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)
A-B		0 / 41	-91.8	-91.8	0.13 (5)	10.00			
B-C		0 / 0	-91.8	-91.8	0.13 (1)	10.00			
F-E		0 / 0	-18.5	-18.5	0.04 (4)	10.00			
E-D		0 / 0	-18.5	-18.5	0.04 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 5 X 13 = 65 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBCG 2010, CBC 2012, ABC 2019
- PART 9 OF CBCG 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B-S), BC=0.04/1.00 (E-F-A), WB=0.00/1.00 (B-E-1), SS=0.08/1.00 (A-B-S)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1887

PLATE PLACEMENT TOL. = 0.250 inches

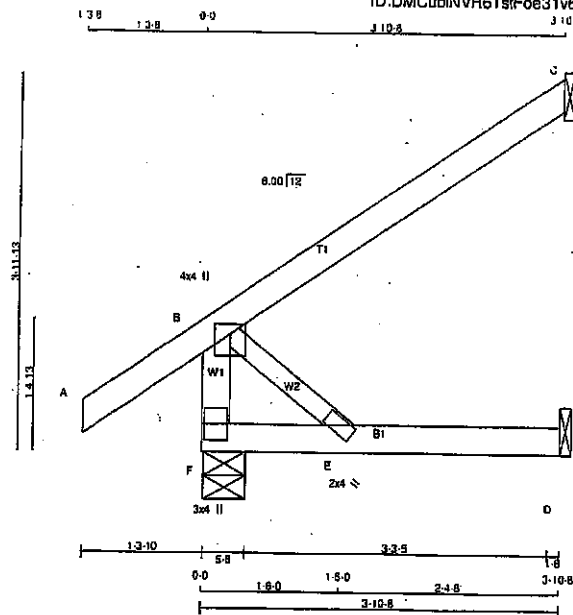
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.20 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2006975

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408089	J30	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 09:25:35 2020 Page 1					
ID:DMCubINVR6TstFoe31v6L_zns1l-GUto43sz_UTHMrVJ97EC2bMIXKqDEE0JTmgg_zNECK					
Scale = 1/2" = 1'-0"					



LUMBER

N. L. G. A. RULES	SIZE	DRY	LUMBER	DESCR
CHORDS				
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMVW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
F	340	340	0	0
C	178	178	0	0
D	38	40	0	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	238	169 / 0	0 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0
C	122	99 / 0	0 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED VERT. LOAD LC2 (PLF)	MAX. FACTORED VERT. LOAD LC3 (PLF)	MAX. FACTORED VERT. LOAD LC4 (PLF)	MAX. FACTORED VERT. LOAD LC5 (PLF)	MAX. FACTORED VERT. LOAD LC6 (PLF)
F-B		-304 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)
A-B		0 / 35	-91.8	-91.8	0.14 (5)	10.00			
B-C		0 / 0	-91.8	-91.8	0.23 (1)	10.00			
F-E		0 / 0	-18.5	-18.5	0.08 (4)	10.00			
E-D		0 / 0	-18.5	-18.5	0.08 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 14 = 28 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.23/1.00 (B-C:1), BC=0.08/1.00 (D-E:4),
WB=0.00/1.00 (B-E:1), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

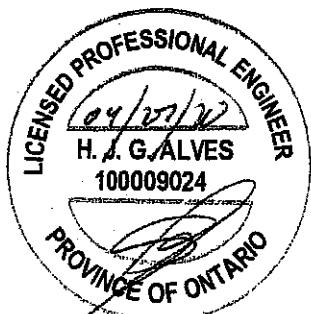
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1857 798 1987 1658	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.50)
JSI METAL= 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2006993

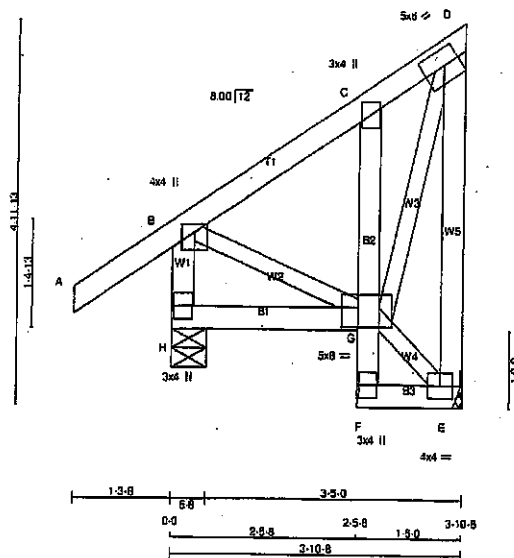
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408089	J31S	3	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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1-3-8 1-3-8 0-0 2-5-8 2-5-8 1-5-0 3-10-8

Scale = 1/2" = 1'-0"



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
H - B	2x4	DRY No.2
A - D	2x4	DRY No.2
H - G	2x4	DRY No.2
F - C	2x4	DRY No.2
F - E	2x4	DRY No.2

ALL WEBS	2x3	DRY No.2
EXCEPT		
E - D	2x4	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25 2.00
C	TMVW+p	MT20	3.0	4.0	
D	TMVW+p	MT20	5.0	6.0	2.50 2.50
E	BMVW+p	MT20	4.0	4.0	
F	BMVW+p	MT20	3.0	4.0	
G	BMVW+p	MT20	5.0	8.0	3.00 2.50
H	BMVW+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
H	332	0	332	0
E	205	0	205	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
H	232	168/0	0/0	0/0	0/0
E	145	95/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

	CHORDS	WEBS
	MAX. FACTORED	MAX. FACTORED
	MEMB. FORCE (LBS)	MEMB. FORCE (LBS)
	VERT. LOAD LC1	MAX. UNBRACED LENGTH FR-TO
	FROM TO	CS1 (LC)
H-B	-307/0	0.0 0.0 0.03 (1)
A-B	0/35	-91.8 -91.8 0.14 (5)
B-C	-83/0	-91.8 -91.8 0.07 (1)
C-D	-99/0	-91.8 -91.8 0.06 (1)
H-G	0/0	-18.5 -18.5 0.04 (4)
F-G	0/12	0.0 0.0 0.02 (1)
G-C	-218/0	0.0 0.0 0.02 (1)
F-E	0/13	-18.5 -18.5 0.01 (4)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 28 = 84 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 8.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.19")
CALCULATED VERT. DEFL.(LL) = L/989 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/989 (0.00")

CSI: TC=0.14/1.00 (A-B-S), BC=0.04/1.00 (G-H-4),
WB=0.05/1.00 (D-G-1), SS=0.09/1.00 (A-B-S)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

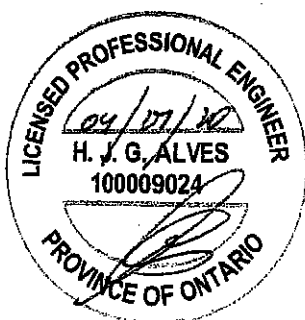
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(FSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 768 1987 1666

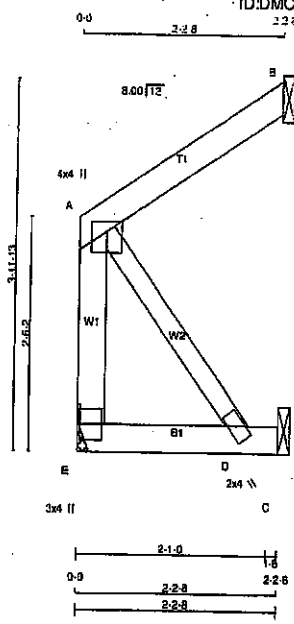
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.08 (G) (INPUT = 1.00)



Structural component only
DWG# T-2006994



LUMBER

N. L. G. A. RULES	SIZE	DRY	LUMBER
E - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
E - C	2x4	DRY	No.2

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	BMVW+w	MT20	2.0	4.0		
E	3MV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
E	122	0	122	0	MECHANICAL	IN-SX
B	101	0	101	0	1-8	1-8
C	20	0	23	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B, C.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	85	58/0	0/0	0/0	0/0	30/0	0/0
B	70	58/0	0/0	0/0	0/0	13/0	0/0
C	16	0/0	0/0	0/0	0/0	16/0	0/0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (PLF)	
FR-TO		FROM	TO		FR-TO		FROM	TO	
E-A	-101/0	0.0	0.0	0.01 (1)	7.81				
A-B	0/0	-91.8	-91.8	0.08 (1)	10.00	A-D	0/0	0.00 (1)	
E-D	0/0	-18.5	-18.5	0.03 (4)	10.00				
D-C	0/0	-18.5	-18.5	0.02 (4)	10.00				

TOTAL WEIGHT = 4 X 10 = 41 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 085-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.00")

CSI: TC=0.08/1.00 (A-B:1), BC=0.03/1.00 (D-E:4), WB=0.00/1.00 (A-D:1), SS=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

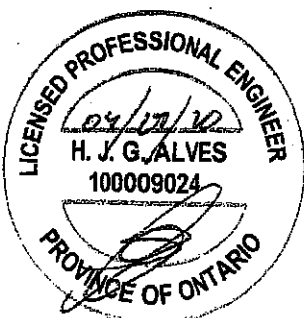
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

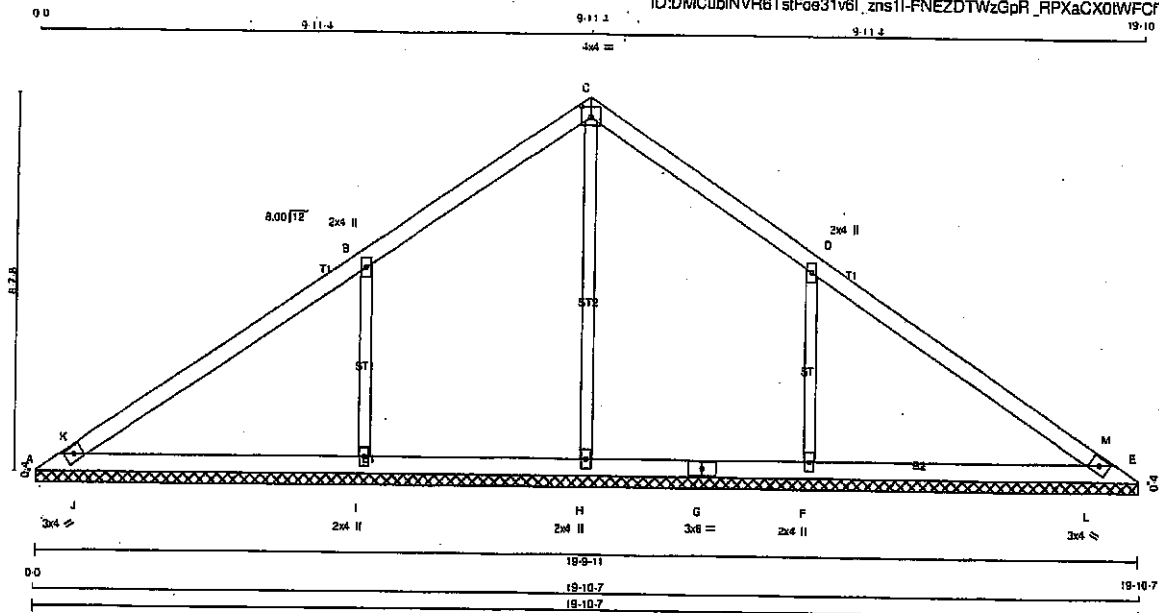
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.07 (A) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 1.00)



Structural component only
DWG# T-2006995

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408087	V01	1	1	GREEN PARK HOMES	
Tamarack Pool Truss, Burlington					
Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 08:53:26 2020 Page 1					
ID:DMCubINVR6TstPos31v6l_zns11-FNEZDTWzGpR_RPXaCX0iWFC7xyulj64kr_5TizNEg					
19-10-7					
Scale = 1:37.4					



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW-w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMW-w	MT20	2.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F, H, I						
F	BMW1-w	MT20	2.0	4.0		
G	BS-1	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQD
GROSS REACTION		GROSS REACTION		GROSS REACTION		BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
A	110	0	110	0	0	19-9-11 (7-9-18) 19-11	
E	110	0	110	0	0	19-9-11 (7-9-18) 19-11	
H	612	0	612	0	0	19-9-11 (7-9-18) 19-11	
I	676	0	676	0	0	19-9-11 (7-9-18) 19-11	
F	676	0	676	0	0	19-9-11 (7-9-18) 19-11	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	77	54/0	0/0	0/0	0/0	24/0	0/0
E	77	54/0	0/0	0/0	0/0	24/0	0/0
H	435	274/0	0/0	0/0	0/0	181/0	0/0
I	478	316/0	0/0	0/0	0/0	181/0	0/0
F	478	316/0	0/0	0/0	0/0	181/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, H, I, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-K	0/286	-91.8 -91.8	0.08 (1)	10.00	H-C	-597/0	0.42 (1)
K-B	0/322	-91.8 -91.8	0.39 (1)	10.00	I-B	-518/0	0.13 (1)
B-C	0/277	-91.8 -91.8	0.38 (1)	10.00	F-D	-518/0	0.13 (1)
C-D	0/277	-91.8 -91.8	0.38 (1)	10.00	J-K	-205/7	0.00 (1)
D-M	0/322	-91.8 -91.8	0.39 (1)	10.00	L-M	-205/7	0.00 (1)
M-E	0/286	-91.8 -91.8	0.08 (1)	10.00			
A-J	-281/0	-18.5 -18.5	0.18 (1)	6.25			
J-I	-247/0	-18.5 -18.5	0.19 (1)	6.25			
I-H	-258/0	-18.5 -18.5	0.14 (1)	6.25			
H-G	-258/0	-18.5 -18.5	0.14 (1)	6.25			
G-F	-258/0	-18.5 -18.5	0.14 (1)	6.25			
F-L	-247/0	-18.5 -18.5	0.19 (1)	6.25			
L-E	-281/0	-18.5 -18.5	0.18 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.39/1.00 (D-M:1), BC=0.19/1.00 (F-L:1), WB=0.42/1.00 (CH:1), SS1=0.18/1.00 (D-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (C) (INPUT = 0.90)
JSI METAL= 0.26 (D) (INPUT = 1.00)

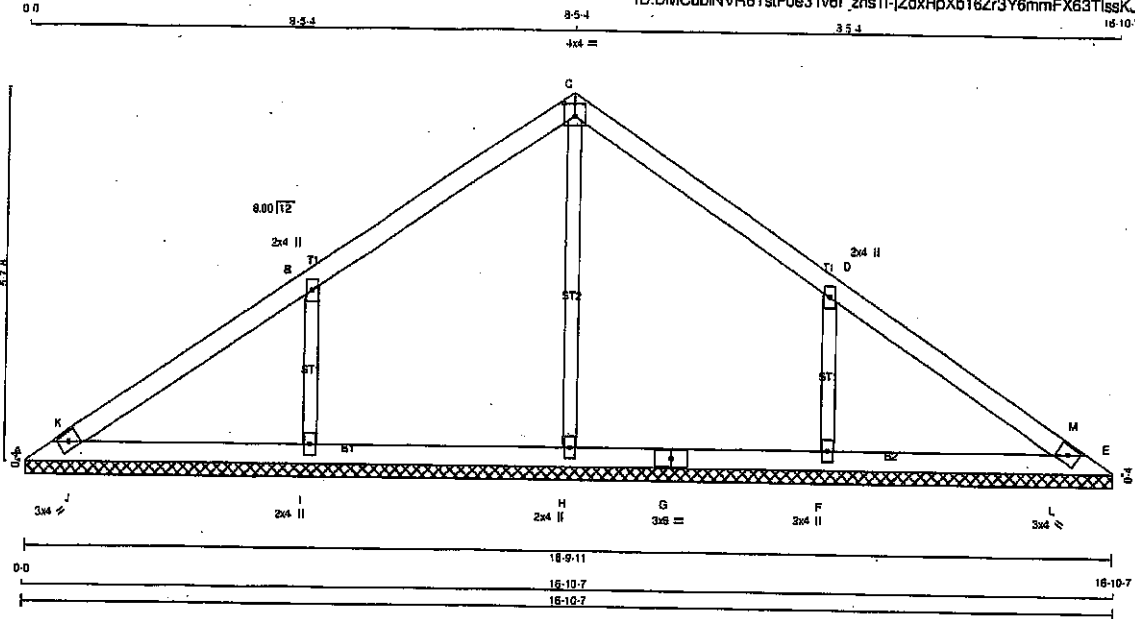


Structural component only
DWG# T-2006989

JOB NAME 408087	TRUSS NAME V02	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6t_zns11-jZoxRpxb16Zr3Y6mmFX63TissKJRUE_DzVkt?kzNEgs



TOTAL WEIGHT = 49 b

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
A - G	2x4	DRY No.2	SPF
G - E	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
DRY, SEASONED LUMBER.			

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TBM1-h	MT20	3.0	4.0		
B TMW-w	MT20	2.0	4.0		
C TTW-p	MT20	4.0	4.0	2.25	2.00
D TMW-w	MT20	2.0	4.0		
E TBM1-h	MT20	3.0	4.0		
F, H, I					
F BMW1-w	MT20	2.0	4.0		
G BS-t	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
A	140	140	0	0
E	140	140	0	0
H	455	455	0	0
I	559	559	0	0
F	559	559	0	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
A	98	88 / 0	0 / 0	0 / 0	31 / 0	0 / 0
E	98	88 / 0	0 / 0	0 / 0	31 / 0	0 / 0
H	325	198 / 0	0 / 0	0 / 0	127 / 0	0 / 0
I	395	264 / 0	0 / 0	0 / 0	131 / 0	0 / 0
F	395	264 / 0	0 / 0	0 / 0	131 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, H, I, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH FR-TO	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO
FR-TO						
A-K	0 / 73	-91.8	-91.8 0.04 (4)	H-C	-406 / 0	0.19 (1)
K-B	0 / 122	-91.8	-91.8 0.26 (1)	I-B	-440 / 0	0.08 (1)
B-C	0 / 83	-91.8	-91.8 0.26 (1)	F-D	-440 / 0	0.08 (1)
C-D	0 / 83	-91.8	-91.8 0.26 (1)	J-K	-103 / 8	0.00 (1)
D-M	0 / 122	-91.8	-91.8 0.26 (1)	L-M	-103 / 8	0.00 (1)
M-E	0 / 73	-91.8	-91.8 0.04 (4)			
A-J	-94 / 0	-18.5	-18.5 0.10 (1)			
J-I	-78 / 0	-18.5	-18.5 0.10 (1)			
I-H	-89 / 0	-18.5	-18.5 0.08 (4)			
H-G	-89 / 0	-18.5	-18.5 0.08 (4)			
G-F	-89 / 0	-18.5	-18.5 0.08 (4)			
F-L	-78 / 0	-18.5	-18.5 0.10 (1)			
L-E	-94 / 0	-18.5	-18.5 0.10 (1)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, NBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.28/1.00 (D-M:1), BC=0.10/1.00 (F-L:1), WB=0.19/1.00 (C-H:1), SS=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

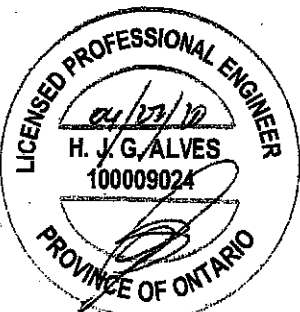
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	818	354	1867

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (C) (INPUT = 0.90)
JSI METAL= 0.22 (B) (INPUT = 1.00)

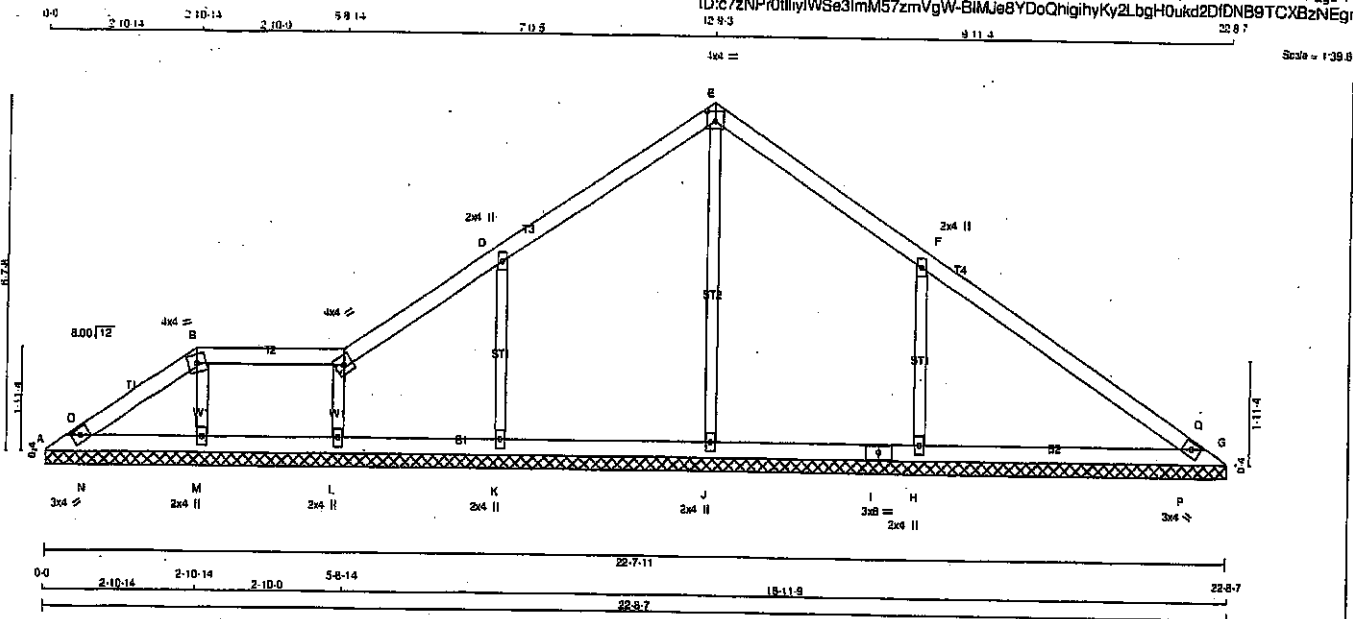


Structural component only
DWG# T-2006970

JOB NAME: 408087 TRUSS NAME: V03 QUANTITY: 1 PLY: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC: DRWG NO. 32 87

Tamarack Pool Truss, Burlington

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ID:c7zNPr0tlliyWSa3lmM57zmVgW-BIMJe8YDoQhigihyKy2LbgH0ukd2DIDNB9TCXBzNEgr



TOTAL WEIGHT = 68 lb

LUMBER

N, L, G, A, RULES

CHORDS SIZE LUMBER

A - B 2x4 DRY No.2

B - C 2x4 DRY No.2

C - E 2x4 DRY No.2

E - G 2x4 DRY No.2

A - I 2x4 DRY No.2

I - G 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

DRY, SEASONED LUMBER.

DESCR.

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
VERT	HORZ	DOWN	HORZ
UP	DOWN	UP	DOWN
JT			
A	85	0	0
G	187	0	0
J	493	0	0
K	485	0	0
H	681	0	0
L	166	0	0
M	399	0	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LOASE	MAX./MIN. COMPONENT REACTIONS					
COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT						
A	50	43/0	0/0	0/0	17/0	0/0
G	132	89/0	0/0	0/0	43/0	0/0
J	351	218/0	0/0	0/0	133/0	0/0
K	342	231/0	0/0	0/0	111/0	0/0
H	481	319/0	0/0	0/0	162/0	0/0
L	118	76/0	0/0	0/0	42/0	0/0
M	283	182/0	0/0	0/0	100/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G, J, K, H, L, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	WEBS	MEMB. MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		FR-TO		
A-O	0/121	-91.8	-91.8 0.03 (1)	J-E	-451/0	0.32 (1)
O-B	0/138	-91.8	-91.8 0.09 (1)	K-D	-406/0	0.10 (1)
B-C	0/131	-91.8	-91.8 0.15 (1)	H-F	-521/0	0.13 (1)
C-D	0/176	-91.8	-91.8 0.24 (1)	L-C	-132/0	0.02 (1)
D-E	0/138	-91.8	-91.8 0.23 (1)	M-B	-304/0	0.04 (1)
E-F	0/124	-91.8	-91.8 0.37 (1)	N-O	-101/0	0.00 (1)
F-Q	0/172	-91.8	-91.8 0.37 (1)	P-Q	-206/6	0.00 (1)
Q-G	0/138	-91.8	-91.8 0.08 (4)			

A-N	-128/0	-18.5	-18.5 0.07 (1)	6.25
N-M	-112/0	-18.5	-18.5 0.07 (1)	6.25
M-L	-131/0	-18.5	-18.5 0.05 (1)	6.25
L-K	-123/0	-18.5	-18.5 0.06 (4)	6.25
K-J	-132/0	-18.5	-18.5 0.06 (4)	6.25
J-I	-132/0	-18.5	-18.5 0.14 (1)	6.25
I-H	-132/0	-18.5	-18.5 0.14 (1)	6.25
H-P	-120/0	-18.5	-18.5 0.21 (1)	6.25
P-G	-156/0	-18.5	-18.5 0.21 (1)	6.25

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-09, CSA 086-14

- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.37/1.00 (F-Q:1), 6C=0.21/1.00 (H-P:1),
WB=0.32/1.00 (E-J:1), SSI=0.18/1.00 (F-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
CCMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1867 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

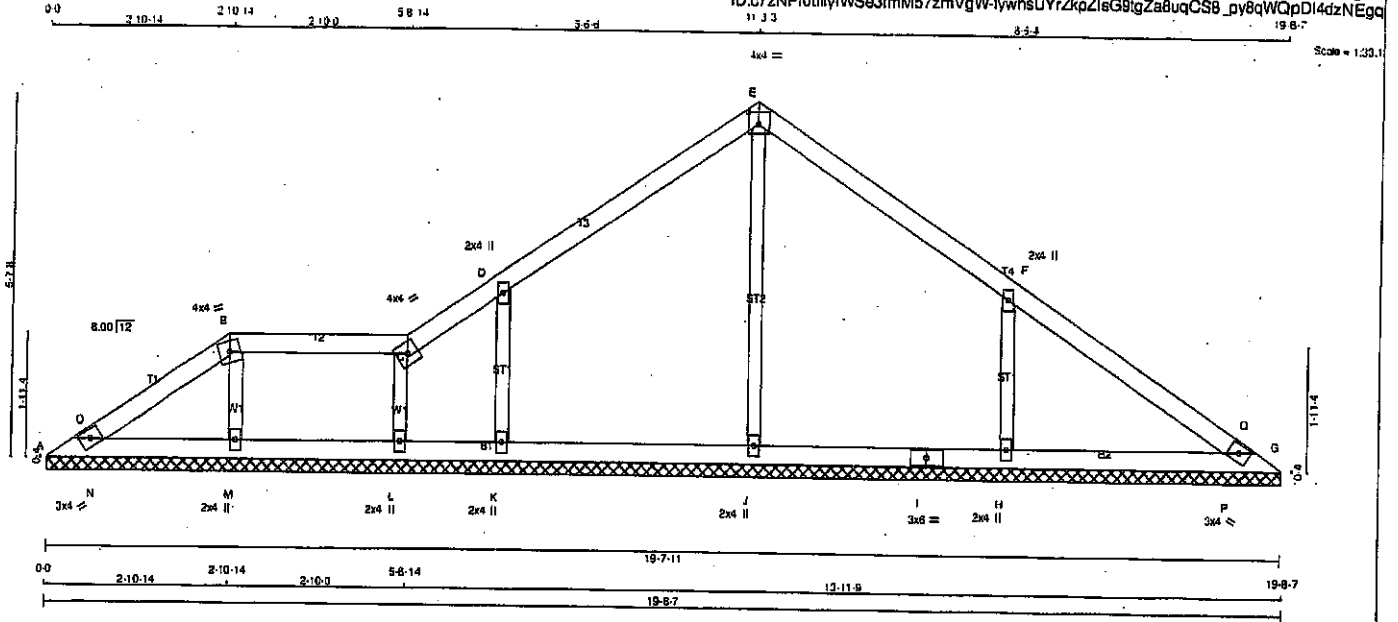
JSI GRIP= 0.66 (C) (INPUT = 0.90)

JSI METAL= 0.27 (F) (INPUT = 1.00)



Structural component only
DWG# T-2006971

JOB NAME 408087	TRUSS NAME V04	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	
				Version 8.310 S Oc 29 2019 Mitek Industries, Inc. Sat Apr 25 08:53:29 2020 Page 1	
				ID:c7zNP0tlllylWSe3imM57zmVgW-fywhsUYrZkpZleG9tgZa8uqCS8_py8qWQpDI4dzNEgq	
				Scale = 1/32.1	



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY No.2	SPF
B - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
A - I	2x4	DRY No.2	SPF
I - G	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
A - TBM1-h	MT20	3.0	4.0	
B - TTW-m	MT20	4.0	4.0	
C - TTW-h	MT20	4.0	4.0	
D - TMMW-w	MT20	2.0	4.0	
E - TTW-p	MT20	4.0	4.0	2.25 2.00
F - TMMW-w	MT20	2.0	4.0	
G - TBM1-h	MT20	3.0	4.0	
H, J, K, L, M	MT20	2.0	4.0	
I - SS-1	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
A	133	0	133	0	19-7-11 (13-1192-1)	
G	164	0	164	0	19-7-11 (13-1192-1)	
J	422	0	422	0	19-7-11 (13-1192-1)	
K	451	0	451	0	19-7-11 (13-1192-1)	
H	560	0	560	0	19-7-11 (13-1192-1)	
L	78	0	78	0	19-7-11 (13-1192-1)	
M	357	0	357	0	19-7-11 (13-1192-1)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
A	53	65/0	0/0	0/0	0/0	27/0
G	116	78/0	0/0	0/0	0/0	38/0
J	301	194/0	0/0	0/0	0/0	118/0
K	317	219/0	0/0	0/0	0/0	99/0
H	395	284/0	0/0	0/0	0/0	131/0
L	58	34/0	0/0	0/0	0/0	22/0
M	253	161/0	0/0	0/0	0/0	92/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G, J, K, H, L, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LBS)	
FR-TO		FROM TO		FR-TO			
A-O	0/22	-91.8 -91.8 0.02 (1)	10.00	J-E	-359/0	0.17 (1)	
O-B	0/34	-91.8 -91.8 0.08 (1)	10.00	K-D	-382/0	0.07 (1)	
B-C	0/41	-91.8 -91.8 0.13 (1)	10.00	H-F	-442/0	0.08 (1)	
C-D	0/99	-91.8 -91.8 0.21 (1)	10.00	L-G	-72/0	0.01 (1)	
D-E	0/39	-91.8 -91.8 0.20 (1)	10.00	M-B	-249/0	0.04 (1)	
E-F	0/34	-91.8 -91.8 0.25 (1)	10.00	N-O	-105/0	0.00 (1)	
F-G	0/74	-91.8 -91.8 0.28 (1)	10.00	P-Q	-103/6	0.00 (1)	
Q-G	0/26	-91.8 -91.8 0.04 (4)	10.00				
A-N	-40/0	-18.5 -18.5 0.09 (1)	6.25				
N-M	-28/0	-18.5 -18.5 0.09 (1)	6.25				
M-L	-41/0	-18.5 -18.5 0.05 (1)	6.25				
L-K	-37/0	-18.5 -18.5 0.05 (4)	6.25				
K-J	-49/0	-18.5 -18.5 0.06 (4)	6.25				
J-I	-49/0	-18.5 -18.5 0.08 (4)	6.25				
I-H	-49/0	-18.5 -18.5 0.08 (4)	6.25				
H-P	-35/0	-18.5 -18.5 0.11 (1)	6.25				
P-G	-53/0	-18.5 -18.5 0.11 (1)	6.25				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.26/1.00 (F-Q:1), BC=0.11/1.00 (H-P:1), WB=0.17/1.00 (E-J:1), SS=0.15/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

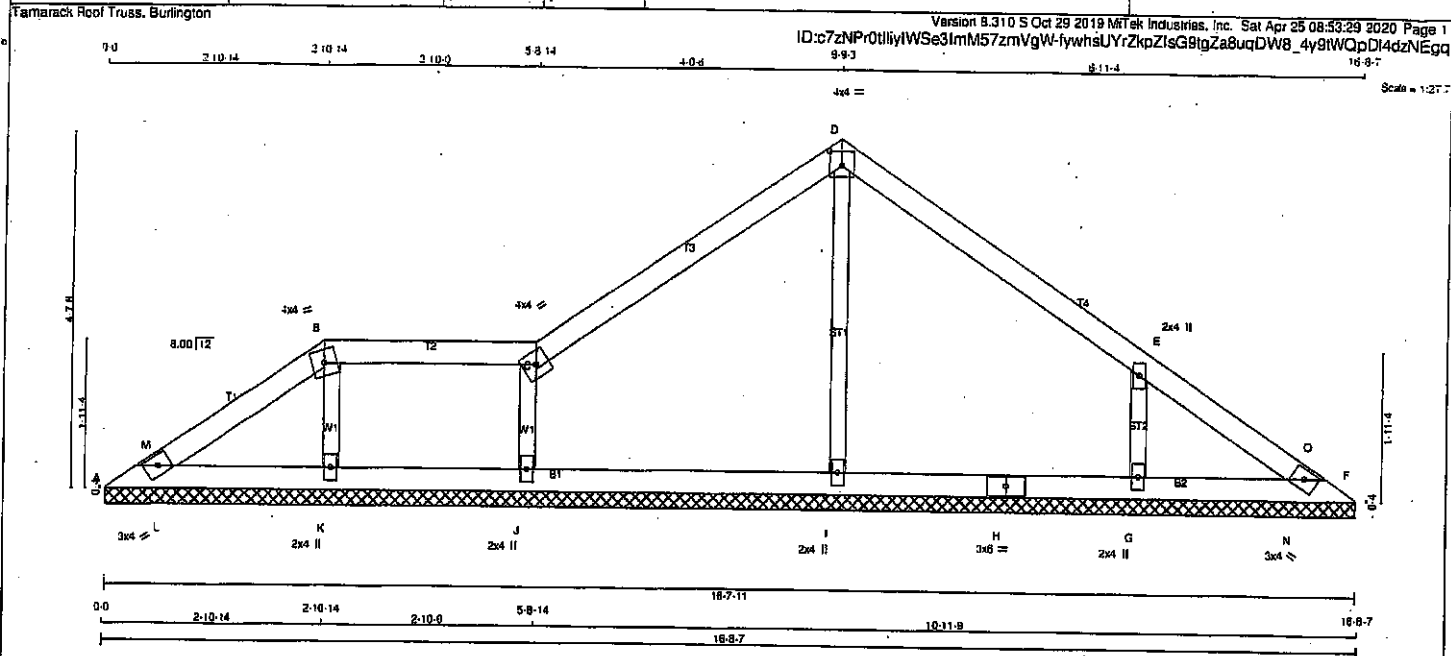
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.66 (C) (INPUT = 0.90)
JSI METAL = 0.23 (F) (INPUT = 1.00)



Structural component only
DWG# T-2006972



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TBM1-h	MT20	3.0	4.0		
B TTW-m	MT20	4.0	4.0		
C TTW-h	MT20	4.0	4.0		
D TTW-p	MT20	4.0	4.0	2.25	2.00
E TMW-w	MT20	2.0	4.0		
F TBM1-h	MT20	3.0	4.0		
G, I, J, K					
G BMW1-w	MT20	2.0	4.0		
H BS-1	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
A	148	0	148	0	16-7-11 (11-1162-11)	
F	109	0	109	0	16-7-11 (11-1162-11)	
I	409	0	409	0	16-7-11 (11-1162-11)	
G	466	0	466	0	16-7-11 (11-1162-11)	
J	369	0	369	0	16-7-11 (11-1162-11)	
K	333	0	333	0	16-7-11 (11-1162-11)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM.LIVE	WIND	DEAD
A	104	71/0	0/0	0/0	0/0	0/0
F	77	52/0	0/0	0/0	0/0	25/0
I	291	179/0	0/0	0/0	0/0	113/0
G	328	222/0	0/0	0/0	0/0	107/0
J	261	173/0	0/0	0/0	0/0	88/0
K	235	155/0	0/0	0/0	0/0	81/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F, I, G, J, K

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 MAX (PLF)	MAX. CS1 UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-M	-8/1	-91.8	-91.8 0.02 (1)	10.00	I-D	-329/0	0.10 (1)
M-B	0/10	-91.8	-91.8 0.08 (1)	10.00	G-E	-382/0	0.06 (1)
B-C	0/15	-91.8	-91.8 0.13 (1)	10.00	J-C	-319/0	0.05 (1)
C-D	-6/7	-91.8	-91.8 0.25 (1)	10.00	K-B	-234/0	0.03 (1)
D-E	-26/3	-91.8	-91.8 0.20 (1)	6.25	L-M	-104/0	0.00 (1)
E-O	0/19	-91.8	-91.8 0.21 (1)	10.00	N-O	-8/7	0.00 (1)
O-F	-43/0	-91.8	-91.8 0.04 (1)	6.25			
A-L	-16/0	-18.5	-18.5 0.09 (1)	6.25			
L-K	-8/0	-18.5	-18.5 0.09 (1)	10.00			
K-J	-15/0	-18.5	-18.5 0.05 (1)	6.25			
J-I	-6/5	-18.5	-18.5 0.07 (4)	10.00			
I-H	-8/5	-18.5	-18.5 0.07 (4)	10.00			
H-G	-6/5	-18.5	-18.5 0.07 (4)	10.00			
G-N	-2/25	-18.5	-18.5 0.05 (4)	10.00			
N-F	-3/22	-18.5	-18.5 0.03 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 8.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, OBC 2012, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 085-09, CSA 085-14
 - TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 9.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.25/1.00 (C-D-1), BC=0.09/1.00 (A-L-1), WB=0.10/1.00 (D-L-1), SS=0.15/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

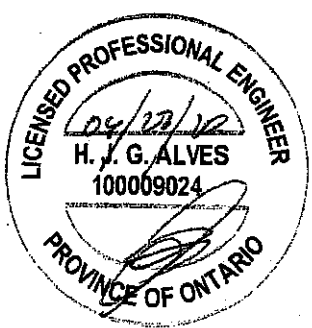
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (C) (INPUT = 0.90)
 JSI METAL= 0.20 (E) (INPUT = 1.00)



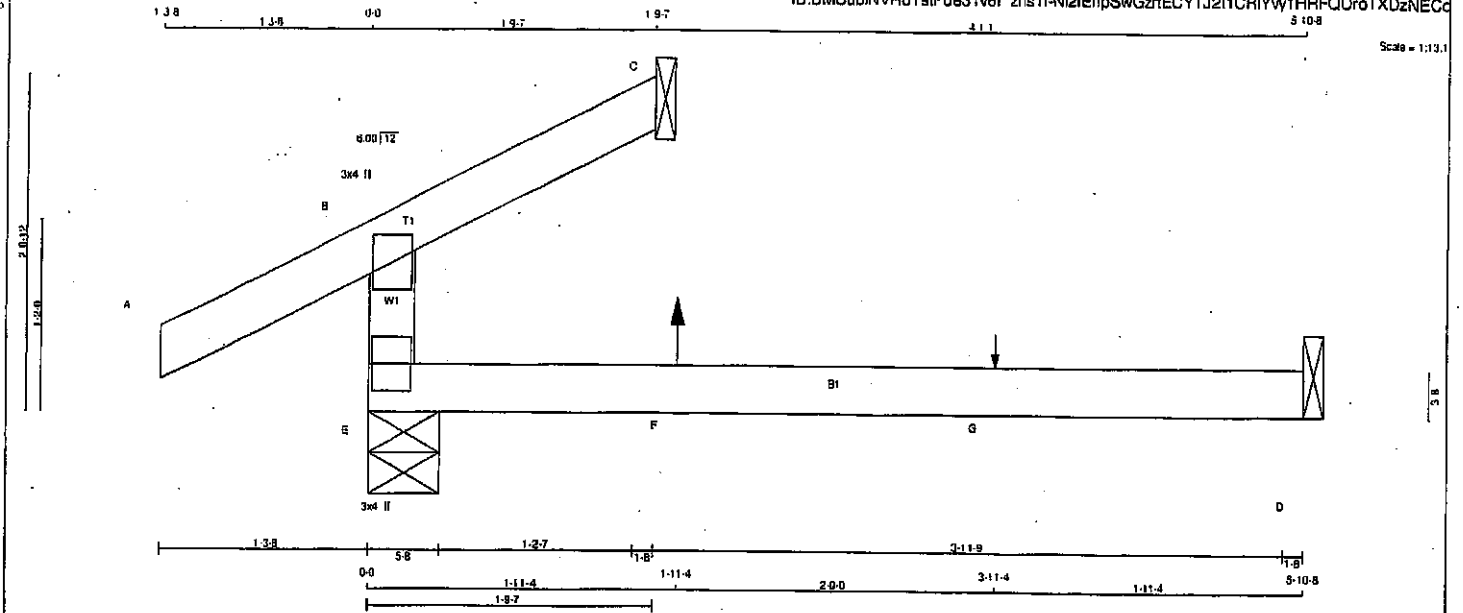


JSI GRIP=0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408089	C31	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER				DESIGN CRITERIA			
N. L. G. A. RULES				SPECIFIED LOADS:			
CHORDS	SIZE	DRY	LUMBER	TOP CH. LL	=	25.6	PSF
E - B	2x4	DRY	No.2	DL	=	6.0	PSF
A - C	2x4	DRY	No.2	BOT CH. LL	=	0.0	PSF
E - D	2x4	DRY	No.2	DL	=	7.4	PSF
DRY: SEASONED LUMBER.				TOTAL LOAD	=	39.0	PSF

PLATES (table is in inches)				UNFACTORED REACTIONS			
JT TYPE	PLATES	W	LEN Y X	1ST CASE	MAX. MIN. COMPONENT REACTIONS		
B TMV+p	MT20	3.0	4.0	E	200	137 / 0	0 / 0
E BMV+p	MT20	3.0	4.0	C	48	21 / 0	0 / 0
				D	35	0 / -3	0 / 0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) E, C, D

BEARINGS				DESIGN ASSUMPTIONS			
JT	VERT	HORZ	DOWN	FACTORED	MAXIMUM FACTORED	INPUT	REQD
E	284	0	284	0	0	5-8	5-8
C	63	0	63	0	0	1-8	1-8
D	44	0	52	0	0	1-8	1-8

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO	LENGTH FR-TO	FR-TO			
E-B	-227 / 0	0.0	0.0 0.11 (4)	7.81			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00			
B-C	-9 / 9	-91.8	-91.8 0.08 (4)	10.00			
E-F	0 / 0	-18.5	-18.5 0.14 (4)	10.00			
F-G	0 / 0	-18.5	-18.5 0.14 (4)	10.00			
G-D	0 / 0	-18.5	-18.5 0.14 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)				CONNECTION REQUIREMENTS			
JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL
F	1-11-4	7	1	12	FRONT	VERT	TOTAL
G	3-11-4	1	1	---	FRONT	VERT	TOTAL

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF
SPACING = 24.0 IN. C/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015
THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014
DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD
ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.04")
CS1: TC=0.12/1.00 (A-B-1), BC=0.14/1.00 (D-E-4), WB=0.00/1.00 (n/a-0), SS1=0.09/1.00 (A-B-1)
DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00
AUTOSOLVE RIGHT HEEL ONLY
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 518 354 1667 788 1987 1656
PLATE PLACEMENT TOL = 0.250 inches
PLATE ROTATION TOL = 5.0 Deg.
JSI GRIP = 0.10 (E) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)

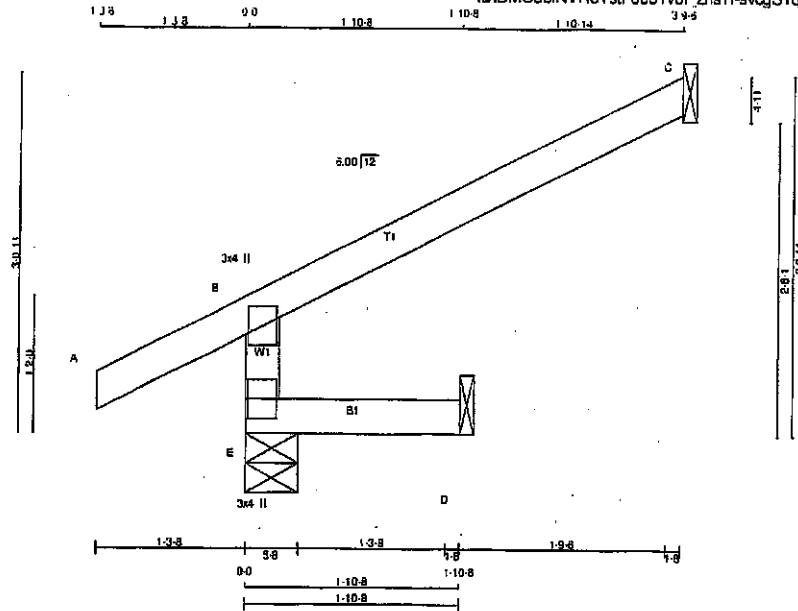


Structural component only
DWG# T-2006989

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408089	C32	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:18.1



TOTAL WEIGHT = 3 X 10 = 29 lb

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2		
A - C	2x4	DRY	No.2		
E - D	2x4	DRY	No.2		

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
E	361	0	361	0
C	130	0	130	0
D	16	0	17	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS	1ST LOASE	MAX MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED							
E	250	190/0	0/0	0/0	0/0	0/0	60/0	0/0
C	90	73/0	0/0	0/0	0/0	0/0	17/0	0/0
D	12	0/0	0/0	0/0	0/0	0/0	12/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. (PLF)	MEMB.	FORCE
FR-TO	(LBS)	FROM TO	FR-TO	(LBS)
E-B	-341/0	0.0 0.0 0.01 (4)	7.81	
A-B	0/28	-91.8 -91.8 0.13 (5)	10.00	
B-C	-19/0	-91.8 -91.8 0.22 (1)	6.25	
E-D	0/0	-18.5 -18.5 0.02 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CSI: TC=0.22/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (N/A:0), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

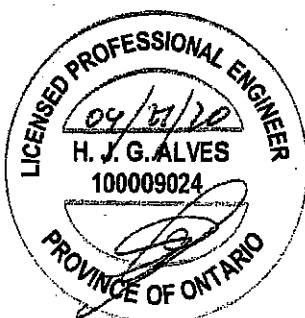
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	818 354	1667 788	1987 1656

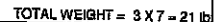
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2006990



DRY: SEASONED LUMBER.

DESCR
SPF
SPF
SPF

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQD
GROSS REACTION		GROSS REACTION		GROSS REACTION		BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
JT	271	0	271	0	0	5-8	5-8
E	44	0	44	0	-23	1-8	1-8
C	8	0	17	0	-2	1-8	1-8

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS							
1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	188	141 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	31	24 / -18	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
O	7	0 / -8	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E. C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBB			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX (LC)
FR-TO		FROM TO	CSI (LC)		FR-TO		
E-B	-244 / 0	0.0	0.0	0.04 (5)	7.81		
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-171 / 0	-91.8	-91.8	0.09 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.04 (5)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD =		39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5),
WB=0.00/1.00 (n/a:0), SS|=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

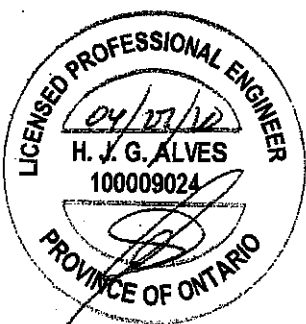
NAIL VALUES

PLATE	GRIP (DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	798	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00



Structural component only
DWG# T-2006991

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

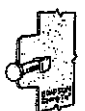
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



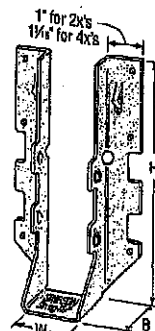
Double-Shear Nailing Top View



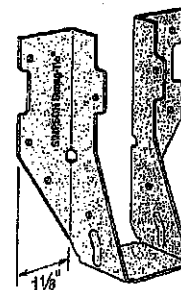
Double-Shear Nailing Side View; Do not bend tab



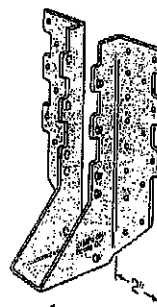
Double-Shear Nailing Side View (available on some models)
U.S. Patent 5,603,580



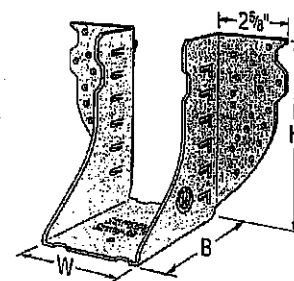
✓ LUS28



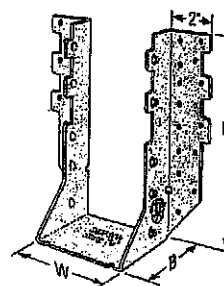
LU26L



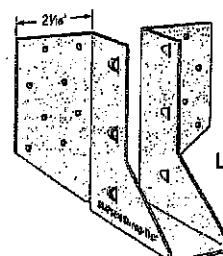
✓ HUS210
(HUS26, HUS28, and HHUS similar)



✓ HGUS28-2

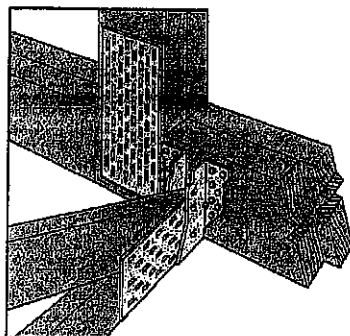


✓ HHUS210-2



LJS26DS

Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)



Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS**HHUS/HGUS**

See Hanger Options Information on pp. 125–127.

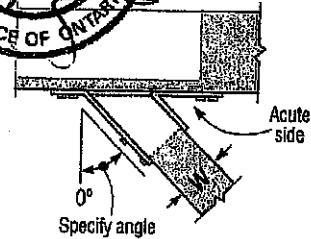
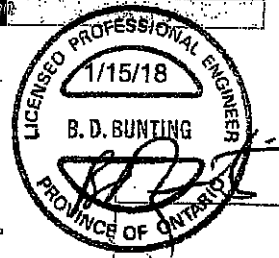
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
$W < 2"$	Bevel or square cut	0.62 of table value	0.46 of table value
$2" < W < 6"$	Bevel cut	0.67 of table value	0.41 of table value
$2" < W < 6"$	Square cut	0.46 of table value	0.41 of table value
$W > 6"$	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance											
		W	H	B	d _g ³	Header	Joist	D-Fir-L		S-P-F									
								Uplift	Normal	Uplift	Normal								
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)								
								lb.	lb.	lb.	lb.								
kN												kN	kN	kN					
Single 2x Sizes																			
LUS24	18	1 1/8	3 1/2	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155								
								316	723	287	514								
								360	1020	320	725								
								1.80	4.54	1.42	3.22								
LU24L	22	1 1/8	3	1 1/4	2 1/8	(4) 10d	(2) 10d x 1 1/2"	720	1605	645	1140								
								3.20	7.14	2.87	5.07								
								1420	2170	1290	1630								
								6.32	9.85	5.74	7.25								
LUS26	18	1 1/8	4 1/4	1 1/4	3 3/4	(4) 10d	(4) 10d	2705	4940	2085	3875								
								11.80	21.97	9.20	17.24								
								2055	4265	1480	4115								
								9.14	18.97	6.49	18.31								
HUS26	16	1 1/8	5 1/4	3	3 1/8	(14) 16d	(6) 16d	2685	6625	2685	5700								
								11.96	29.51	11.96	25.35								
								1140	2185	1020	1550								
								5.07	9.72	4.54	8.89								
LJS26DS	18	1 1/8	5	3 1/2	4 1/4	(16) 16d	(6) 16d	1420	2520	1290	1790								
								6.32	11.21	5.74	7.96								
								3605	5365	2675	4345								
								16.04	23.86	11.90	19.33								
HGUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 16d	(8) 16d	3310	7675	3310	6900								
								14.74	34.19	14.74	30.73								
								1140	2495	1020	1770								
								5.07	11.10	4.54	7.87								
LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"	1420	2785	1290	2210								
								6.32	12.39	5.74	9.83								
								LUS28	18	1 1/8	7 1/4	1 1/4	3 3/4	(6) 10d	(4) 10d	710	1625	645	1155
								316								723	287	514	
360	1020	320	725																
1.80	4.54	1.42	3.22																
LUS28	18	1 1/8	6 1/4	1 1/4	3 3/4	(6) 10d	(4) 10d	720	1605	645	1140								
								3.20	7.14	2.87	5.07								
								1420	2170	1290	1630								
								6.32	9.85	5.74	7.25								
HUS28	16	1 1/8	7 1/4	3	6 1/8	(22) 16d	(8) 16d	2705	4940	2085	3875								
								11.80	21.97	9.20	17.24								
								2055	4265	1480	4115								
								9.14	18.97	6.49	18.31								
HGUS28	12	1 1/8	7 1/4	5	6 1/4	(36) 16d	(12) 16d	2685	6625	2685	5700								
								11.96	29.51	11.96	25.35								
								1140	2185	1020	1550								
								5.07	9.72	4.54	8.89								
LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(6) 10d x 1 1/2"	1420	2520	1290	1790								
								6.32	11.21	5.74	7.96								
								3605	5365	2675	4345								
								16.04	23.86	11.90	19.33								
LUS210	18	1 1/8	7 1/4	1 1/4	3 3/4	(8) 10d	(4) 10d	3310	7675	3310	6900								
								14.74	34.19	14.74	30.73								
								1140	2495	1020	1770								
								5.07	11.10	4.54	7.87								
LUS210	18	1 1/8	7 1/4	1 1/4	3 3/4	(8) 10d	(4) 10d	1420	2785	1290	2210								
								6.32	12.39	5.74	9.83								

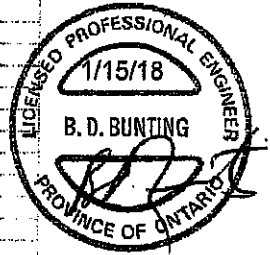
1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_g is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/4" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _h ³	Header	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _G = 1.15) lb. kN	(K _G = 1.00) lb. kN	(K _G = 1.15) lb. kN	(K _G = 1.00) lb. kN
Double 2x Sizes											
LUS24-2	18	3 1/8	3 1/8	2	1 1/8	(4) 16d	(2) 16d	835 3.71	2020 8.99	590 2.62	1435 6.38
LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	1720 7.65	2595 11.54	1545 6.87	1920 8.54
HHUS28-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2850 12.68	7335 32.63	2065 9.20	5205 23.15
HGUS28-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720 7.65	3325 14.79	1545 6.87	2575 11.45
HHUS28-2	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765 16.75	8940 39.77	2675 11.90	6345 28.22
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580 11.48	4500 20.02	2320 10.32	3195 14.21
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	4670 20.77	9650 42.97	4235 18.84	7000 31.14
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840 30.43	14015 62.34	4855 21.60	10270 45.69
Triple 2x Sizes											
HGUS26-3	12	4 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d	4670 20.77	9670 43.02	4235 18.84	6865 30.54
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840 30.43	14645 65.14	4855 21.60	10400 46.26
Quadruple 2x Sizes											
HGUS28-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
HHUS210-4	14	6 1/8	8 1/8	3	7 1/8	(30) 16d	(10) 16d	4670 20.77	10155 45.17	4235 18.84	7210 32.07
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840 30.43	14645 65.14	4855 21.60	10400 46.26
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640 33.98	14995 66.70	5425 24.13	10845 47.35
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130 45.06	16400 72.95	7195 32.00	11645 51.80
4x Sizes											
LUS46	18	3 1/8	4 1/8	2	3 1/8	(4) 16d	(4) 16d	1720 7.65	2595 11.54	1545 6.87	1920 8.54
HHUS46	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2540 11.30	7335 32.63	2065 9.20	5205 23.15
HGUS46	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
LUS48	18	3 1/8	6 1/8	2	3 1/8	(6) 16d	(4) 16d	1720 7.65	3325 14.79	1545 6.87	2575 11.45
HHUS48	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765 16.75	8940 39.77	2675 11.90	6345 28.22
HGUS48	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
LUS410	18	3 1/8	8 1/8	2	5 1/8	(8) 16d	(6) 16d	2580 11.48	4500 20.02	2320 10.32	3195 14.21
HGUS410	12	3 1/8	9	4	8 1/8	(46) 16d	(16) 16d	6840 30.43	14015 62.34	4855 21.60	10270 45.69
HGUS412	12	3 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640 33.98	14995 66.70	5425 24.13	10845 47.35
HGUS414	12	3 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130 45.06	16400 72.95	7195 32.00	11645 51.80



Plated Truss Connectors

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1 1/4". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

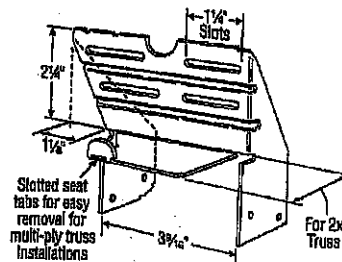
Design: Factored resistances are in accordance with CSA 086-14

Installation:

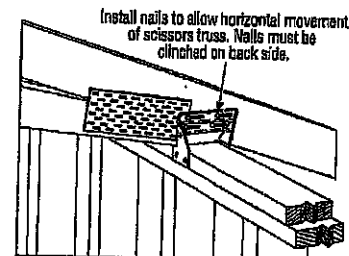
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

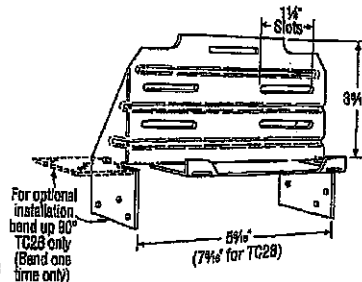
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



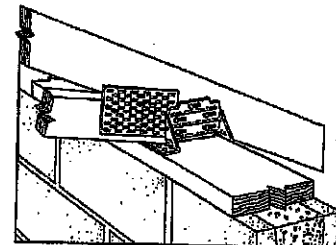
TC24
U.S. Patent 4,932,173



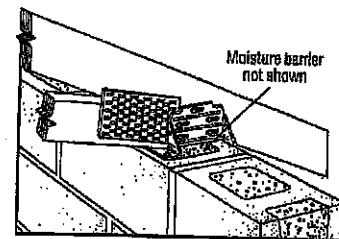
Typical TC24 Installation



TC26
(TC28 Similar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K ₀ =1.15)	Uplift (K ₀ =1.15)
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K ₀ =1.15)	Uplift (K ₀ =1.15)
TC28	(5) 10d	(6) 10d x 1 1/2"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.

2. Grout strength is 15 MPa minimum.

3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.

4. TC28 fastened to grouted concrete block with (8) - 3/8" x 2 1/4" Titen screws has a factored uplift resistance of 275 lb.



TIME
STATES
DESIGN

1. The design of this structure is based on the information provided by the client and the design professional. The design professional is not responsible for the accuracy of the information provided by the client. The design professional is not responsible for the accuracy of the information provided by the client. The design professional is not responsible for the accuracy of the information provided by the client.

(800) 999-5099
strongtie.com

H/TSP

Seismic and Hurricane Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Fasteners			Factored Resistance ($K_D = 1.15$)					
					D.Fir-L			S-P-F		
		To Rafters/Truss	To Plates	To Studs	Uplift	Lateral		Uplift	Lateral	
					lb.	F ₁ lb.	F ₂ lb.	lb.	F ₁ lb.	F ₂ lb.
					kN	kN	kN	kN	kN	kN
H1	18	(6) 8d x 1 1/2"	(4) 8d	—	740	685	300	680	485	215
					3.29	3.05	1.33	3.02	2.16	0.96
SS H2A	18	(5) 8d x 1 1/2"	(2) 8d x 1 1/2"	(5) 8d x 1 1/2"	830	220	75	590	155	55
					3.69	0.98	0.33	2.62	0.69	0.24
SS H2.5A	18	(5) 8d	(5) 8d	—	805	180	160	755	160	160
					3.58	0.71	0.71	3.38	0.71	0.71
H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
					3.71	0.78	0.93	3.29	0.71	0.93
SS H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
					3.29	0.80	1.18	2.74	0.56	0.85
H6	16	—	(8) 8d	(8) 8d	1585	1085	—	1125	770	—
					7.05	4.83	—	5.00	3.43	—
H7Z	16	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
					6.18	2.98	—	4.40	2.11	—
SS H8 ³	18	(5) 10d x 1 1/2"	(5) 10d x 1 1/2"	—	1120	—	—	1025	—	—
					4.98	—	—	4.56	—	—
SS H10A ⁶	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1735	795	410	1505	565	290
					7.72	3.54	1.82	6.69	2.51	1.29
H10AR	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1485	690	430	1220	570	305
					6.61	3.07	1.91	5.43	2.54	1.36
H10A-2	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1835	1275	430	1645	880	305
					8.16	5.67	1.91	7.32	3.91	1.36
H10S ^{7,8}	18	(8) 8d x 1 1/2"	(8) 8d x 1 1/2"	(8) 8d	1465	795	315	1040	565	225
					6.52	3.54	1.40	4.63	2.51	1.00
H11Z	18	(6) 16d x 2 1/2"	(6) 16d x 2 1/2"	—	1095	920	545	780	655	390
					4.87	4.08	2.42	3.47	2.81	1.73
H14	18	[1] (12) 8d x 1 1/2"	(13) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
		[2] (12) 8d x 1 1/2"	(15) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
TSP	16	(9) 10d x 1 1/2"	(6) 10d x 1 1/2"	—	1295	440	—	920	310	—
					5.76	1.96	—	4.09	1.38	—
		(9) 10d x 1 1/2"	(6) 10d	—	1560	440	—	1105	310	—
					6.94	1.96	—	4.92	1.38	—

- Factored resistances have been increased 15% for short term loading; no further increase is allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2 1/2" must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
- H8 factored uplift resistances for stud-to-bottom plate installations are 595 lb. (2.65 kN) for D.Fir-L and 390 lb. (1.74 kN) for S-P-F.
- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-HTECONPATH).
- Factored resistances in the F₁ direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.

- H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.38 kN) D.Fir-L and 1015 lb. (4.51 kN) S-P-F.
- H10S nails to plates are optional for uplift but required for lateral loads.
- H10A may be field-bent up to a slope of 8/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
- The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SSNAILS at strongtie.com.
- D.Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1 1/2" nails into the top plates and (3) 8d x 1 1/2" nails into the lowest three flange holes into the truss bottom chord is 485 lb. (2.20 kN).
- Nails: 16d x 2 1/2" = 0.162" dia. x 2 1/2" long, 10d = 0.148" dia. x 3" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 8d = 0.131" dia. x 2 1/2" long, 8d x 1 1/2" = 0.131" dia. x 1 1/2" long. See pp. 27–28 for other nail sizes and information.

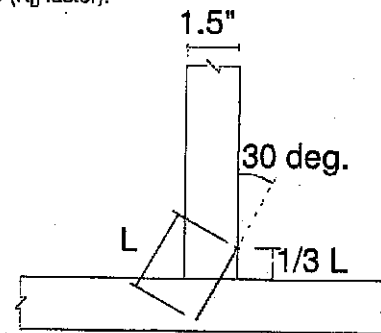
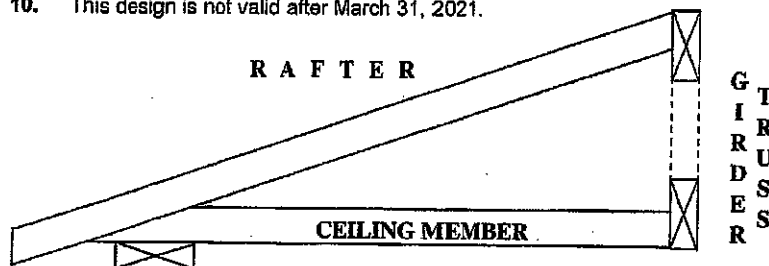
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4.
10. This design is not valid after March 31, 2021.

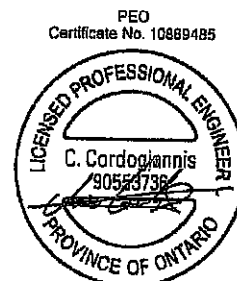


TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

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December 2, 2019



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

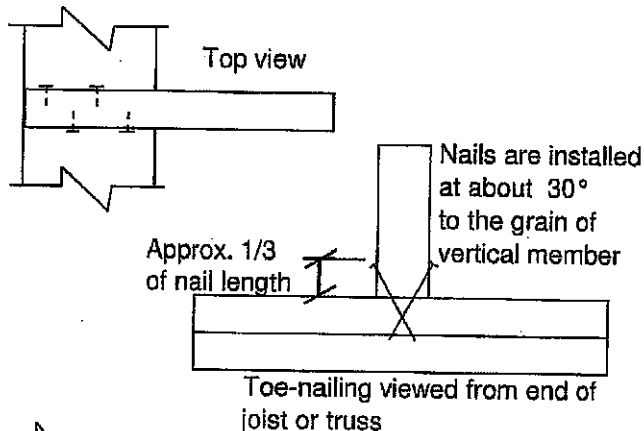
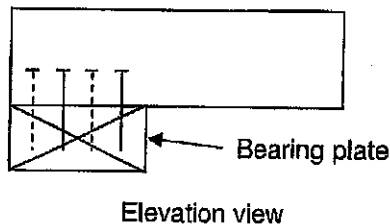
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

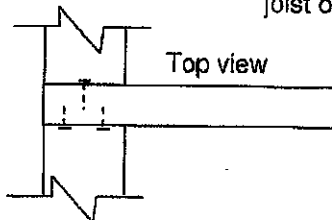
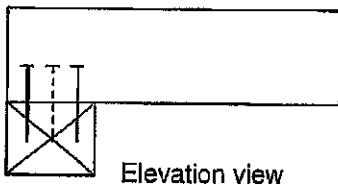
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to wind or earthquake load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate

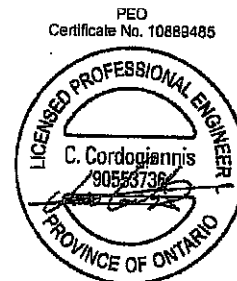


Toe-nailing on 2x4 Bearing Plate



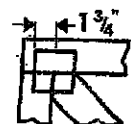
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December 2, 2019

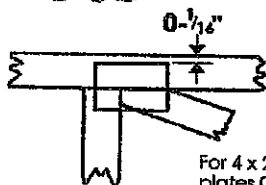


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint, unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

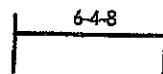


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

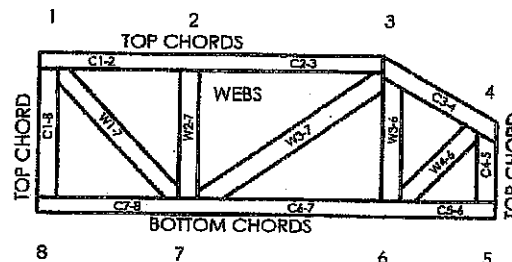
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

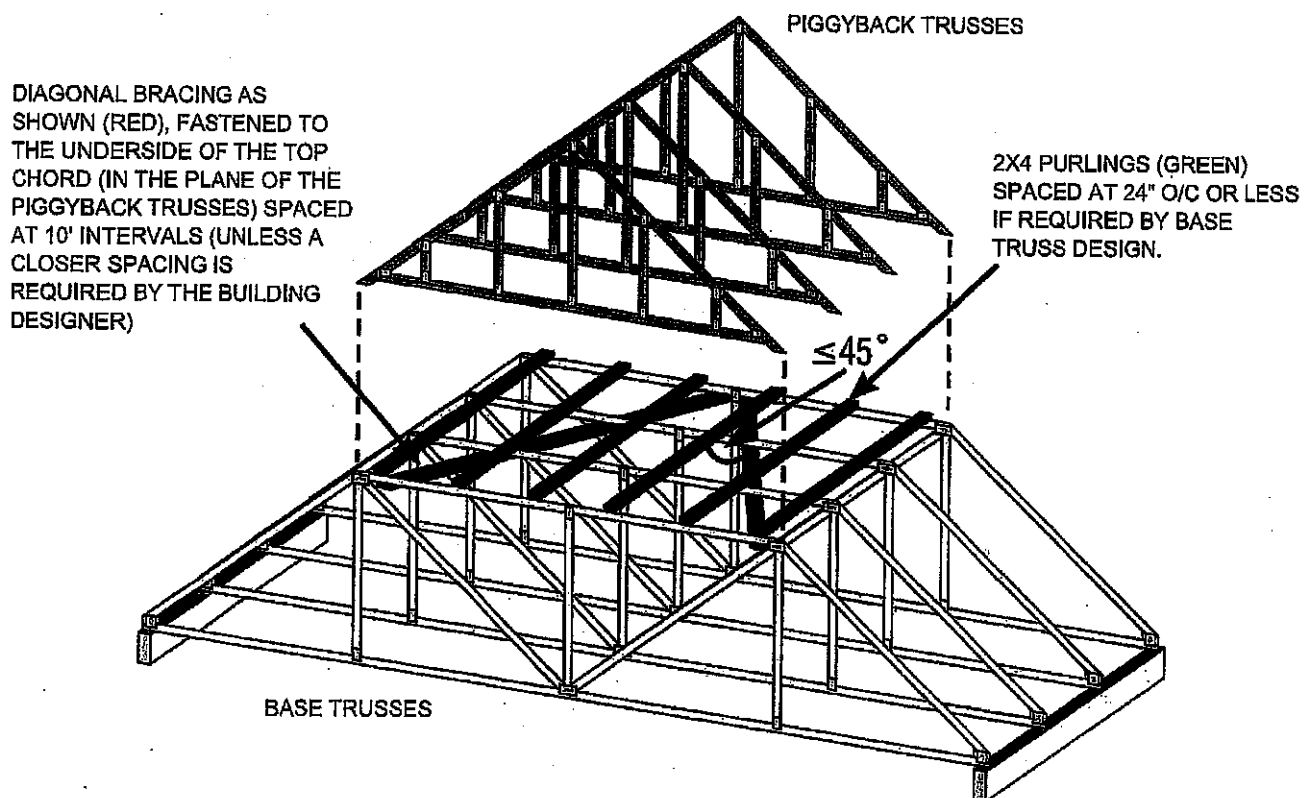
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane of joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

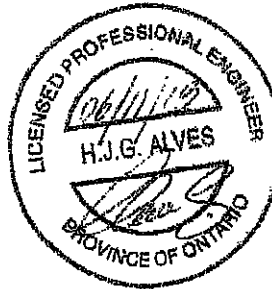


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.



Alves Engineering Services Inc.

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(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet M117473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1500213

Feb 09, 2018