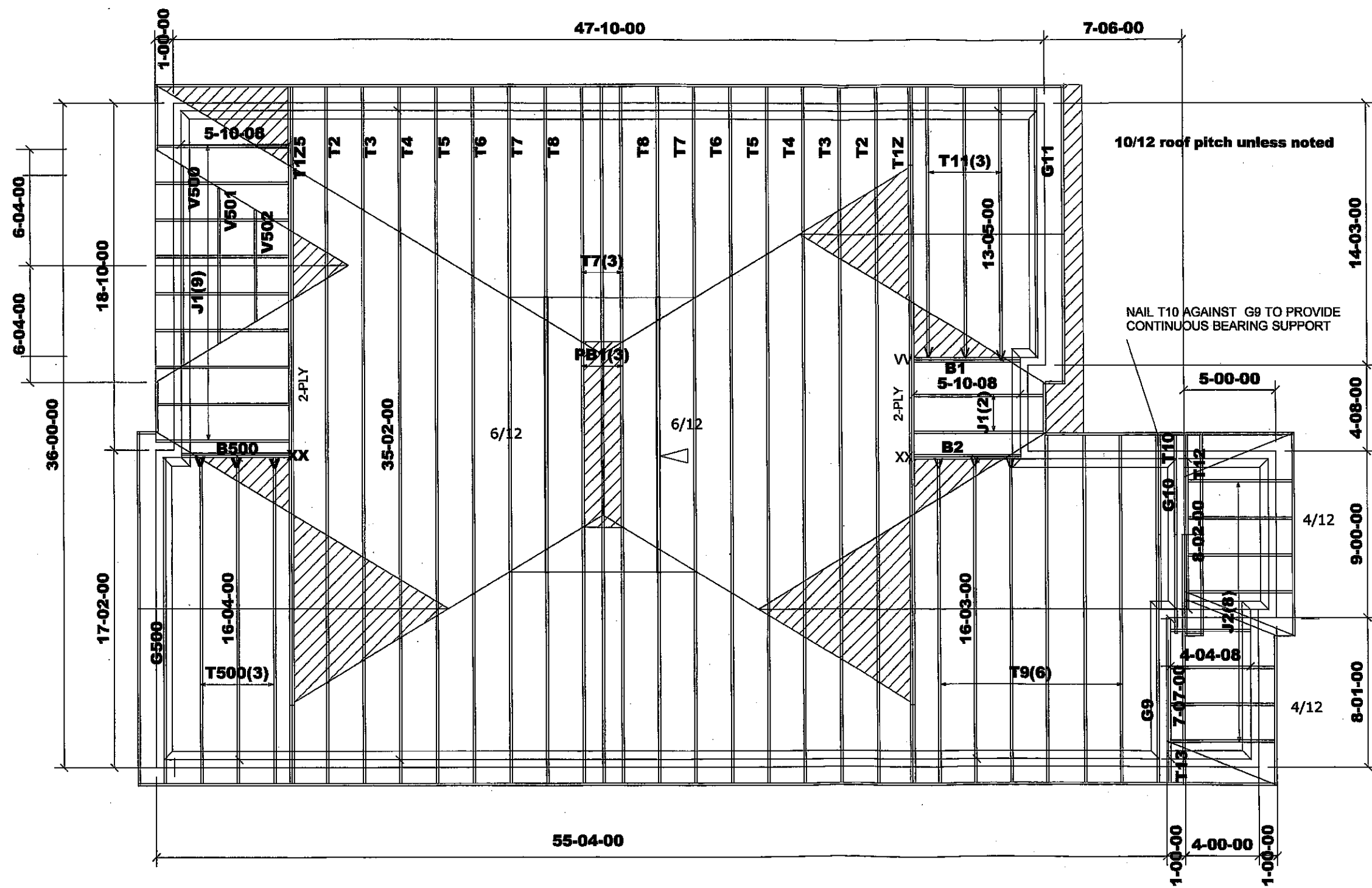




ASPHALT SHINGLES
 FINISHED OVERHANG: 12"
 2x6 EXTERIOR WALLS
 2x6 FASCIA BOARD
 HEEL: R.T.M.C.

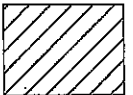
All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
 Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
 TCSL = 25.6 psf
 TCDL = 6.0 psf
 BCLL = 0.0 psf
 BCDL = 7.4 psf

HARDWARE:
 LUS24 - (O)
 LJS26DS - (V)
 HGUS26-2 - (XX)
 LUS26-2-(VV)

BEAMS:
 B1= B2 = B500=
 2 - 2x10 SPF #2

 DENOTES: CONVENTIONAL FRAMING

CITY OF HAMILTON BUILDING DIVISION Planning & Development Department	
DEC 10 2020	
REC'D BY _____	DATE _____
REF'D TO _____	DATE _____

M13126



Job Track: 51225	Builder / Location: GREEN PARK HOMES / WATERDOWN	Model / Elevation: MOUNTAINASH 5-011/1-REAR UPGRADE	Mitek ver 8.3.3.247
Plan Log: 202406	Project: RUSSELL GARDENS PH.3	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.	
Layout ID: 412345	Date: 2020-09-02	Sales: Mario DiCano	Designer: JG



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 5-011
 Lot #:
 Elevation: 1-REAR UPGRADE

Job Track: 51225
 PlanLog: 202406
 Layout ID: 412345
 Ref #
 Page: 1 of 2
 Date: 09-01-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T12 Hip Girder	10 / 12	35-02-00	4-01-11	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	365.29 218.00		
	1 2-ply	T125 Hip Girder	10 / 12	35-02-00	4-01-11	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	365.29 218.00		
	2	T2 Hip	10 / 12	35-02-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	293.44 185.67		
	2	T3 Hip	10 / 12	35-02-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	296.43 188.67		
	2	T4 Hip	10 / 12	35-02-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	323.09 204.33		
	2	T5 Hip	10 / 12	35-02-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	326.19 204.67		
	2	T6 Hip	10 / 12	35-02-00	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	363.03 228.67		
	5	T7 Hip	10 / 12	35-02-00	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	866.6 540.00		
	2	T8 Hip	10 / 12	35-02-00	11-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	380.36 236.67		
	6	T9 Common	10 / 12	16-03-00	8-04-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	480.92 308.00		
	1	G9 GABLE	10 / 12	16-03-00	8-04-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	81.49 53.33		
	1	T10 Common	10 / 12	8-02-00	5-00-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	38.47 25.83		
	1	G10 GABLE	10 / 12	8-02-00	5-00-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	40.07 27.83		
	3	T11 Common	10 / 12	13-05-00	7-02-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	191.27 120.60		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST				
	Lumber Yard:	TAMARACK LUMBER		Job Track:	51225
	Builder:	GREEN PARK HOMES		PlanLog:	202406
	Project:	RUSSELL GARDENS PH.3		Layout ID:	412345
	Location:	WATERDOWN		Ref #	
	Model:	MOUNTAINASH 5-011		Page:	2 of 2
Lot #:			Date:	09-01-2020	
Elevation:	1-REAR UPGRADE		Designer:		
			Sales Rep:	Mario DiCano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	G11 GABLE	10 / 12	13-05-00	7-02-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	65.46 42.17		
	1	T12 Hip Girder	10 / 12	8-02-00	2-02-08	2 x 4	1-03-08 1-03-08	1-00-07 1-00-07	34.43 25.33		
	1	T13 Half Hip Girder	10 / 12	7-07-00	2-02-08	2 x 4	1-03-08	1-00-07 1-09-07	31.77 22.50		
	3	T500 Common	10 / 12	16-04-00	8-05-05	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	241.52 154.00		
	1	G500 GABLE	10 / 12	16-04-00	8-05-05	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	81.97 53.33		
	3	PB1 Piggyback	10 / 12	14-10-03	2-00-00	2 x 4			135.57 90.00		
	1	V500 Valley	10 / 12	10-09-13	4-06-01	2 x 4			34.04 21.67		
	1	V501 Valley	10 / 12	8-05-00	3-06-01	2 x 4			23.25 14.17		
	1	V502 Valley	10 / 12	6-00-03	2-06-01	2 x 4			16.13 10.33		
	11	J1 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	184.74 117.33		
	8	J2 Jack-Open	4 / 12	4-04-08	2-02-06	2 x 4	1-03-08	3-15 1-09-07	95.29 64.00		

TOTAL # TRUSS= 65

TOTAL BFT OF ALL TRUSSES= 3375

BFT.

TOTAL WEIGHT OF ALL TRSSES 5356.09 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
9	Hardware	LJS26DS	
1	Hardware	LUS26-2	

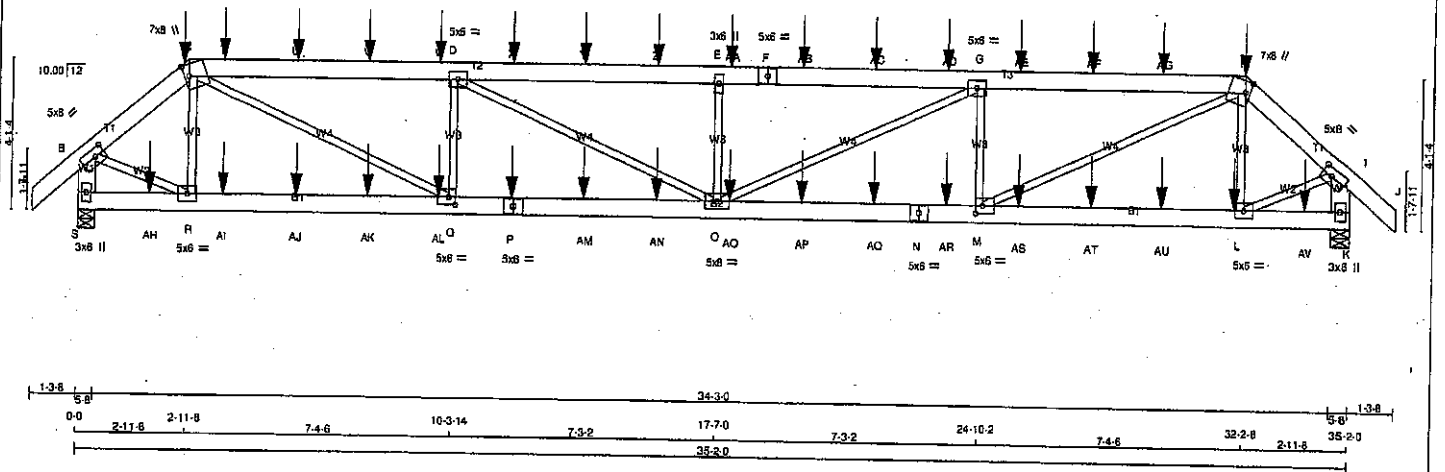
TOTAL NUMBER OF ITEMS= 12

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408168	T1	1	2	GREEN PARK HOMES	

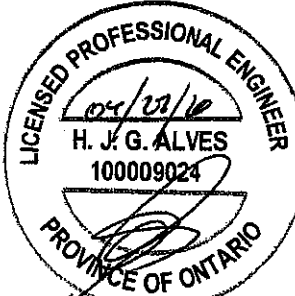
Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 28 12:34:48 2020 Page 1
ID:DMCubINVR6TstFoe31v6L_zns1H-Mcnq2FKiqQuZwGH_Yq9JS_3jk34wGOWeSzNBRL

Scale = 1/8" = 1'-0"



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x6 DRY No.2 SPF C - F 2x6 DRY No.2 SPF F - H 2x6 DRY No.2 SPF H - J 2x6 DRY No.2 SPF K - I 2x6 DRY No.2 SPF S - P 2x6 DRY 2100F 1.8E SPF P - N 2x6 DRY 2100F 1.8E SPF N - K 2x6 DRY 2100F 1.8E SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF) TOP CHORDS : (0.122"x3") SPIRAL NAILS A-C 2 12 SIDE(122.0) C-F 2 12 SIDE(81.0) F-H 2 12 SIDE(81.0) H-J 2 12 SIDE(122.0) S-B 2 12 TOP K-I 2 12 TOP BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS S-P 2 12 SIDE(0.0) P-N 2 12 SIDE(0.0) N-K 2 12 SIDE(197.8) WEBS : (0.122"x3") SPIRAL NAILS 2x3 1 6 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.			DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REORD JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX S 3330 0 3330 0 0 5-8 5-8 K 3379 0 3379 0 0 5-8 5-8 UNFACTORED REACTIONS 1ST CASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL S 2354 1553 / 0 0 / 0 0 / 0 801 / 0 0 / 0 K 2388 1572 / 0 0 / 0 0 / 0 816 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 (PLF) MAX. CSI (LC) UNBRACED LENGTH FR-TO MEMB. MAX. FACTORED FORCE (LBS) MAX. CSI (LC) FR-TO A-B 0 / 42 -91.8 -91.8 0.04 (1) 10.00 R-C -850 / 0 0.08 (1) B-C -3367 / 0 -91.8 -91.8 0.05 (1) 8.02 C-Q 0 / 4509 0.56 (1) C-T -8602 / 0 -91.8 -91.8 0.38 (1) 4.31 Q-D -1799 / 0 0.22 (1) T-U -8602 / 0 -91.8 -91.8 0.38 (1) 4.31 D-O 0 / 1370 0.17 (1) U-V -8602 / 0 -91.8 -91.8 0.38 (1) 4.31 O-E -987 / 0 0.12 (1) V-W -8602 / 0 -91.8 -91.8 0.38 (1) 4.31 E-G 0 / 1362 0.17 (1) W-D -8602 / 0 -91.8 -91.8 0.38 (1) 4.31 G-H -1787 / 0 0.21 (1) D-X -7826 / 0 -91.8 -91.8 0.42 (1) 3.98 H-I 0 / 4481 0.55 (1) X-Y -7826 / 0 -91.8 -91.8 0.42 (1) 3.98 I-J -861 / 0 0.08 (1) Y-Z -7826 / 0 -91.8 -91.8 0.42 (1) 3.98 J-K 0 / 2738 0.34 (1) Z-E -7826 / 0 -91.8 -91.8 0.42 (1) 3.98 K-L 0 / 2771 0.34 (1) E-AA -7826 / 0 -91.8 -91.8 0.42 (1) 3.98 AA-F -7826 / 0 -91.8 -91.8 0.42 (1) 3.98 F-AB -7826 / 0 -91.8 -91.8 0.42 (1) 3.98 AB-AC -7826 / 0 -91.8 -91.8 0.42 (1) 3.98 AC-AD -7826 / 0 -91.8 -91.8 0.42 (1) 3.98 AD-G -7826 / 0 -91.8 -91.8 0.42 (1) 3.98 G-AE -8608 / 0 -91.8 -91.8 0.38 (1) 4.31 AE-AF -8608 / 0 -91.8 -91.8 0.38 (1) 4.31 AF-AG -8608 / 0 -91.8 -91.8 0.38 (1) 4.31 AG-H -8608 / 0 -91.8 -91.8 0.38 (1) 4.31 H-I -3410 / 0 -91.8 -91.8 0.05 (1) 6.00 I-J -3410 / 0 -91.8 -91.8 0.04 (1) 10.00 S-B -3375 / 0 0.0 0.0 12 (1) 7.58 K-I -3414 / 0 0.0 0.0 12 (1) 7.55 S-AH 0 / 0 -18.5 -18.5 0.05 (4) 10.00 AH-R 0 / 0 -18.5 -18.5 0.05 (4) 10.00 R-AI 0 / 2553 -18.5 -18.5 0.12 (1) 10.00 AI-AJ 0 / 2553 -18.5 -18.5 0.12 (1) 10.00 AJ-AK 0 / 2553 -18.5 -18.5 0.12 (1) 10.00 AK-AL 0 / 2553 -18.5 -18.5 0.12 (1) 10.00 AL-Q 0 / 2553 -18.5 -18.5 0.12 (1) 10.00 Q-P 0 / 8602 -18.5 -18.5 0.22 (1) 10.00 P-AM 0 / 8602 -18.5 -18.5 0.22 (1) 10.00 AM-AN 0 / 8602 -18.5 -18.5 0.22 (1) 10.00 AN-O 0 / 8602 -18.5 -18.5 0.22 (1) 10.00 O-AO 0 / 8602 -18.5 -18.5 0.22 (1) 10.00 AO-AP 0 / 8602 -18.5 -18.5 0.22 (1) 10.00 AP-AQ 0 / 8602 -18.5 -18.5 0.22 (1) 10.00 AQ-N 0 / 8602 -18.5 -18.5 0.22 (1) 10.00 N-AR 0 / 8602 -18.5 -18.5 0.22 (1) 10.00 AR-M 0 / 8602 -18.5 -18.5 0.22 (1) 10.00			DESIGN CRITERIA *** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE SPECIFIED LOADS: TOP CH. LL = 25.6 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 39.0 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 *** NON STANDARD GIRDER *** ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, OBC 2012, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 088-09, CSA 086-14 - TPIC 2011, TPIC 2014 (55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (1.17) CALCULATED VERT. DEFL.(LL) = L/989 (0.20) ALLOWABLE DEFL.(TL) = L/360 (1.17) CALCULATED VERT. DEFL.(TL) = L/989 (0.37) CSI: TC=0.42/1.00 (D-E:1), BC=0.22/1.00 (D-Q:1), WB=0.56/1.00 (C-Q:1), SS=0.22/1.00 (C-D:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) MAX MIN MAX MIN MAX MIN MT20 818 354 1687 788 1987 1686 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.89 (H) INPUT = 0.90 JSI METAL= 0.65 (N) INPUT = 1.00		
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Structural component only
DWG# T-2007129 1/2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.25
C	TTWW+m	MT20	7.0	8.0	3.75	1.50
D	TMWW-t	MT20	5.0	8.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	8.0		
G	TMWW-t	MT20	5.0	8.0		
H	TTWW+m	MT20	7.0	8.0	3.75	1.50
I	TMVW-t	MT20	5.0	8.0	2.50	3.25
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	8.0		
M	BMWW-t	MT20	5.0	8.0	2.50	2.25
N	BS-t	MT20	5.0	8.0		
O	BMWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	8.0		
Q	BMWW-t	MT20	5.0	8.0	2.50	2.25
R	BMWW-t	MT20	5.0	8.0		
S	BMV1+p	MT20	3.0	6.0		

LOADING

TOTAL LOAD CASES: (4)

CHORDS

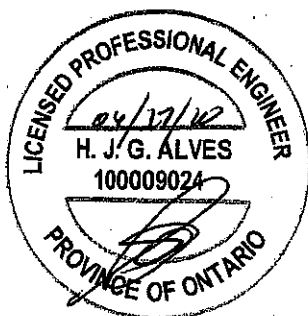
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 (PLF)	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM	TO					
M-AS	0 / 2586	-18.5	-18.5	0.12 (1)	10.00			
AS-AT	0 / 2586	-18.5	-18.5	0.12 (1)	10.00			
AT-AU	0 / 2586	-18.5	-18.5	0.12 (1)	10.00			
AU-L	0 / 2586	-18.5	-18.5	0.12 (1)	10.00			
L-AV	0 / 0	-18.5	-18.5	0.05 (1)	10.00			
AV-K	0 / 0	-18.5	-18.5	0.05 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-38	-42		FRONT	VERT	DEAD		C1
H	32-2-8	-191	-191		FRONT	VERT	SNOW		C1
H	32-2-8	-116	-116		FRONT	VERT	DEAD		C1
H	32-2-8	-191	-191		FRONT	VERT	TOTAL		C1
L	31-11-4	-26	-26		FRONT	VERT	SNOW		C1
P	11-11-4	-26	-26		FRONT	VERT	TOTAL		C1
T	3-11-4	-111	-111		FRONT	VERT	TOTAL		C1
U	5-11-4	-110	-110		FRONT	VERT	TOTAL		C1
V	7-11-4	-110	-110		FRONT	VERT	TOTAL		C1
W	9-11-4	-110	-110		FRONT	VERT	TOTAL		C1
X	11-11-4	-110	-110		FRONT	VERT	TOTAL		C1
Y	13-11-4	-110	-110		FRONT	VERT	TOTAL		C1
Z	15-11-4	-110	-110		FRONT	VERT	TOTAL		C1
AA	17-11-4	-110	-110		FRONT	VERT	TOTAL		C1
AB	19-11-4	-110	-110		FRONT	VERT	TOTAL		C1
AC	21-11-4	-110	-110		FRONT	VERT	TOTAL		C1
AD	23-11-4	-110	-110		FRONT	VERT	TOTAL		C1
AE	25-11-4	-110	-110		FRONT	VERT	TOTAL		C1
AF	27-11-4	-110	-110		FRONT	VERT	TOTAL		C1
AG	29-11-4	-110	-110		FRONT	VERT	TOTAL		C1
AH	1-11-4	-26	-26		FRONT	VERT	TOTAL		C1
AI	3-11-4	-26	-26		FRONT	VERT	TOTAL		C1
AJ	5-11-4	-26	-26		FRONT	VERT	TOTAL		C1
AK	7-11-4	-26	-26		FRONT	VERT	TOTAL		C1
AL	9-11-4	-26	-26		FRONT	VERT	TOTAL		C1
AM	13-11-4	-26	-26		FRONT	VERT	TOTAL		C1
AN	15-11-4	-26	-26		FRONT	VERT	TOTAL		C1
AO	17-11-4	-26	-26		FRONT	VERT	TOTAL		C1
AP	19-11-4	-26	-26		FRONT	VERT	TOTAL		C1
AQ	21-11-4	-26	-26		FRONT	VERT	TOTAL		C1
AR	23-11-4	-26	-26		FRONT	VERT	TOTAL		C1
AS	25-11-4	-26	-26		FRONT	VERT	TOTAL		C1
AT	27-11-4	-26	-26		FRONT	VERT	TOTAL		C1
AU	29-11-4	-26	-26		FRONT	VERT	TOTAL		C1
AV	33-11-4	-26	-26		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



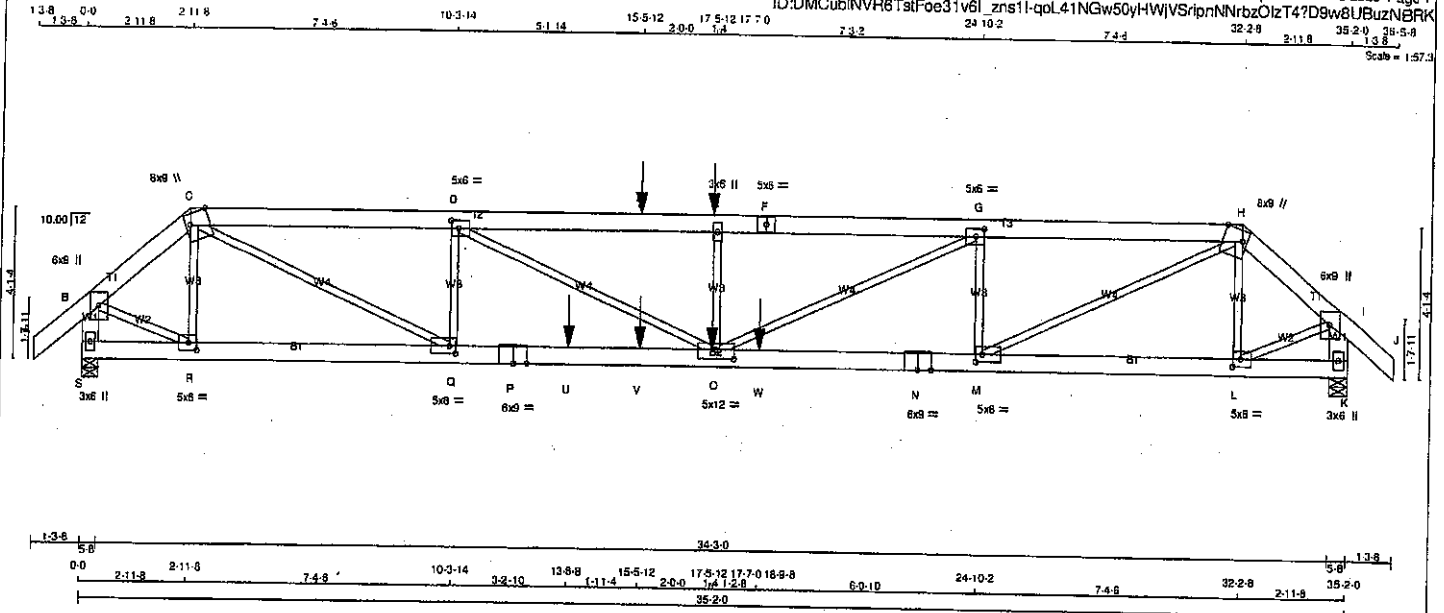
Structural component only
DWG# T-2007129

JOB NAME 408168	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF		
C - F	2x6	DRY	No.2	SPF		
F - H	2x6	DRY	No.2	SPF		
H - J	2x6	DRY	No.2	SPF		
S - B	2x6	DRY	No.2	SPF		
K - I	2x6	DRY	No.2	SPF		
S - P	2x6	DRY	2100F 1.8E	SPF		
P - N	2x6	DRY	2100F 1.8E	SPF		
N - K	2x6	DRY	2100F 1.8E	SPF		
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	2	12
C - F	2	12
F - H	2	12
H - J	2	12
S - B	2	12
K - I	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S - P	2	12
P - N	2	12
N - K	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
S	3963	0	0	5-8
K	3687	0	0	5-8

UNFACTORED REACTIONS	1ST CASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
S	2793	1890 / 0 0 / 0 0 / 0 902 / 0 0 / 0
K	2599	1756 / 0 0 / 0 0 / 0 843 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING	TOTAL LOAD CASES: (4)
CHORDS	WEBS
MEMB.	MEMB.
MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
VERT. LOAD LC1	MAX. FACTORED FORCE (LBS)
FROM TO	FROM TO
A-B	0 / 42
B-C	-3988 / 0
C-D	-9524 / 0
D-E	-12331 / 0
E-F	-12331 / 0
F-G	-12331 / 0
G-H	-8575 / 0
H-I	-3755 / 0
I-J	0 / 42
J-K	-3945 / 0
K-L	-3733 / 0
S-R	0 / 0
R-Q	0 / 3013
Q-P	0 / 9534
P-U	0 / 9534
U-V	0 / 9534
V-O	0 / 9534
O-W	0 / 8575
W-N	0 / 8575
N-M	0 / 8575
M-L	0 / 2844
L-K	0 / 0

S-R	0/0	-18.5	-18.5	0.02 (4)	10.00
R-Q	0/3013	-18.5	-18.5	0.21 (1)	10.00
Q-P	0/9534	-18.5	-18.5	0.57 (1)	10.00
P-U	0/9534	-18.5	-18.5	0.67 (1)	10.00
J-V	0/9534	-18.5	-18.5	0.67 (1)	10.00
V-Q	0/9534	-18.5	-18.5	0.67 (1)	10.00
D-W	0/8575	-18.5	-18.5	0.49 (1)	10.00
W-N	0/8575	-18.5	-18.5	0.49 (1)	10.00
N-M	0/8575	-18.5	-18.5	0.49 (1)	10.00
M-L	0/2844	-18.5	-18.5	0.12 (1)	10.00
A-K	0/0	-18.5	-18.5	0.05 (1)	10.00

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 183 = 365 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 089-09, CSA 089-14
- TPIC 2011, TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17)
CALCULATED VERT. DEFL.(LL) = L/898 (0.35)
ALLOWABLE DEFL.(TL) = L/360 (1.17)
CALCULATED VERT. DEFL.(TL) = L/657 (0.84)

CSI: TC=0.52/1.00 (D-E:1), BC=0.57/1.00 (O-Q:1), WB=0.50/1.00 (C-Q:1), SS=0.53/1.00 (M-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1668

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.69 (C) (INPUT = 0.90)

JSI METAL = 0.95 (P) (INPUT = 1.00)



Structural component only
DWG# T-2007130

JOB NAME 408168	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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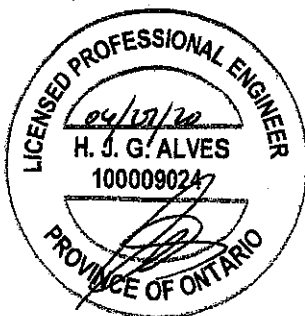
Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 12:34:49 2020 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	9.0		
C	TTWW+m	MT20	8.0	9.0	Edge	6.50
D	TMWW-l	MT20	5.0	6.0	2.50	2.50
E	TMW+w	MT20	3.0	6.0		
F	TS-l	MT20	5.0	6.0		
G	TMWW-l	MT20	5.0	6.0	2.50	2.50
H	TTWW+m	MT20	8.0	9.0	Edge	6.50
I	TMVW+p	MT20	6.0	9.0		
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-l	MT20	5.0	6.0	2.50	2.75
M	BMWW-l	MT20	5.0	8.0	2.50	2.00
N	BS-l	MT20	6.0	9.0		
O	BMWWW-l	MT20	5.0	12.0	3.00	6.00
P	BS-l	MT20	6.0	9.0		
Q	BMWW-l	MT20	5.0	8.0	2.50	2.00
R	BMWW-l	MT20	5.0	6.0	2.50	2.75
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



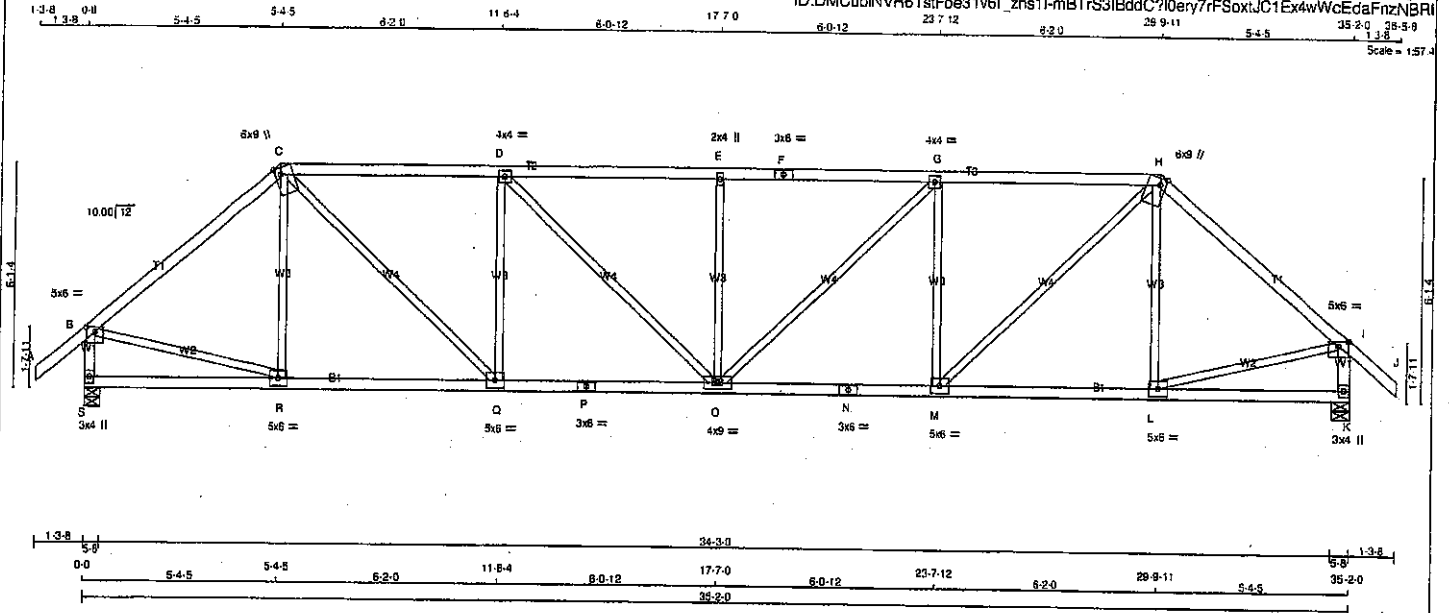
Structural component only
DWG# T-2007130 92

Structural component only
DWG# T-2007131

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408168	T3	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 148 = 296 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
L - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - P	2x4	DRY	No.2
P - Q	2x4	DRY	No.2
Q - R	2x4	DRY	No.2
R - S	2x4	DRY	No.2
S - T	2x4	DRY	No.2
T - U	2x4	DRY	No.2
U - V	2x4	DRY	No.2
V - W	2x4	DRY	No.2
W - X	2x4	DRY	No.2
X - Y	2x4	DRY	No.2
Y - Z	2x4	DRY	No.2
Z - A	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	6.0	1.50	3.00
C TTWW+m	MT20	6.0	9.0	Edge	1.75
D TMVW-t	MT20	4.0	4.0		
E TMVW-w	MT20	2.0	4.0		
F TS-t	MT20	3.0	6.0		
G TMVW-t	MT20	4.0	4.0		
H TTWW+m	MT20	6.0	9.0	Edge	1.75
I TMVW-p	MT20	5.0	6.0	1.50	3.00
K BMV1-p	MT20	3.0	4.0		
L, M, Q, R					
N BMVW-t	MT20	5.0	6.0		
O BS-t	MT20	3.0	6.0		
P BMVW-w	MT20	4.0	9.0		
Q BS-t	MT20	3.0	6.0		
S BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UP
S	2066	0	0	5-8
K	2066	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN.	COMPONENT REACTIONS
JT	COMBINED	SNOW
S	1458	971 / 0
K	1458	971 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1	MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRAC LENGTH	MAX CSI (LC)	
FR-TO		FROM TO			FR-TO				
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	R-C	-246 / 7	0.14 (1)	
B-C	-2004 / 0	-91.8	-91.8	0.84 (1)	4.11	C-Q	0 / 1538	0.35 (1)	
C-D	-2653 / 0	-91.8	-91.8	0.74 (1)	3.50	Q-D	-943 / 0	0.55 (1)	
D-E	-2978 / 0	-91.8	-91.8	0.78 (1)	3.28	O-E	0 / 450	0.10 (1)	
E-F	-2978 / 0	-91.8	-91.8	0.78 (1)	3.28	O-E	-513 / 0	0.30 (1)	
F-G	-2978 / 0	-91.8	-91.8	0.78 (1)	3.28	O-G	0 / 450	0.10 (1)	
G-H	-2653 / 0	-91.8	-91.8	0.74 (1)	3.50	M-G	-943 / 0	0.55 (1)	
H-I	-2004 / 0	-91.8	-91.8	0.84 (1)	4.11	M-H	0 / 1538	0.35 (1)	
I-J	0 / 41	-91.8	-91.8	0.13 (1)	10.00	L-H	-246 / 7	0.14 (1)	
S-B	-2026 / 0	0.0	0.0	0.22 (1)	5.94	B-R	0 / 1580	0.38 (1)	
K-I	-2026 / 0	0.0	0.0	0.22 (1)	5.94	L-I	0 / 1580	0.36 (1)	
S-R	0 / 0	-18.5	-18.5	0.14 (4)	10.00				
R-Q	0 / 1532	-18.5	-18.5	0.33 (1)	10.00				
Q-P	0 / 2653	-18.5	-18.5	0.49 (1)	10.00				
P-Q	0 / 2859	-18.5	-18.5	0.49 (1)	10.00				
O-N	0 / 2653	-18.5	-18.5	0.49 (1)	10.00				
N-M	0 / 2653	-18.5	-18.5	0.49 (1)	10.00				
M-L	0 / 1532	-18.5	-18.5	0.33 (1)	10.00				
L-K	0 / 0	-18.5	-18.5	0.14 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

SPECIFIED LOADS:			
TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN CC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CSI: TC=0.78/1.00 (E-G:1), BC=0.49/1.00 (M-O:1),
WS=0.55/1.00 (G-M:1), SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618	354
	1667	788
	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (B) (INPUT = 0.90)
JSI METAL = 0.85 (N) (INPUT = 1.00)

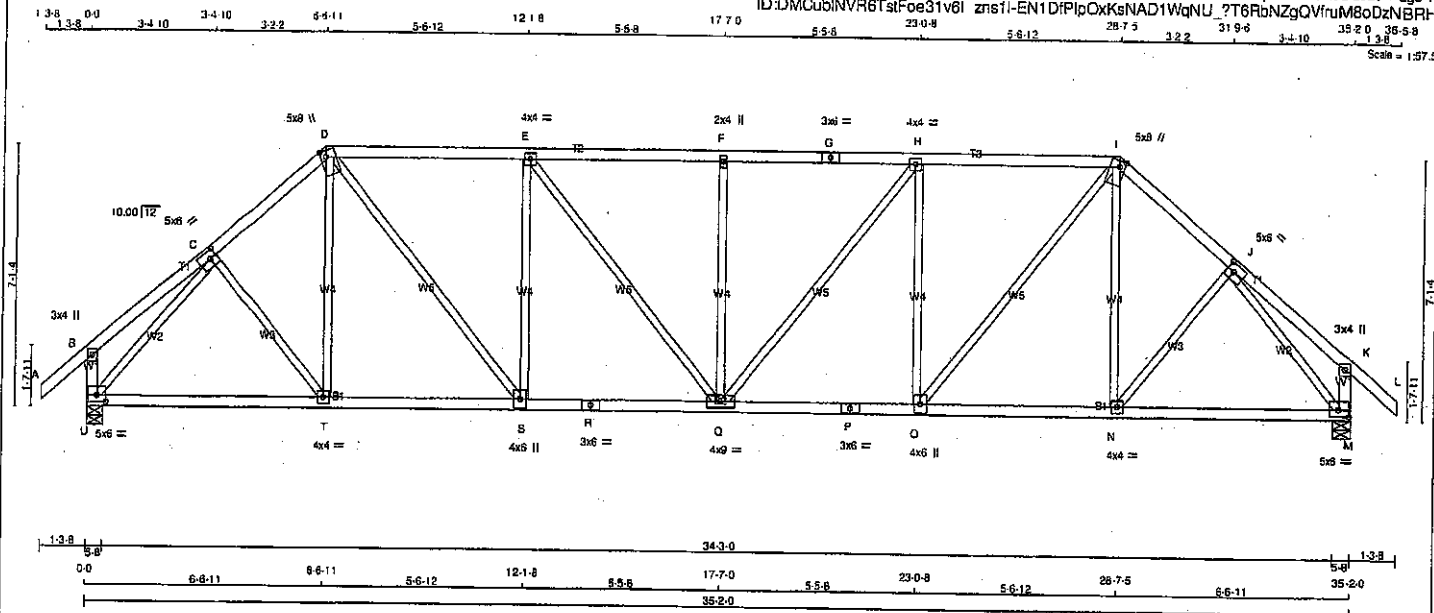


Structural component only
DWG# T-2007132

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408168	T4	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES					
CHORDS	SIZE		LUMBER	DESCR	
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - L	2x4	DRY	No.2	SPF	
U - B	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
U - R	2x4	DRY	No.2	SPF	
R - P	2x4	DRY	No.2	SPF	
P - M	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV-p	MT20	3.0	4.0		
C TMVW-t	MT20	5.0	6.0	2.50	2.25
D TTMW-m	MT20	5.0	8.0	2.00	1.75
E TMVW-t	MT20	4.0	4.0		
F TMVW-w	MT20	2.0	4.0		
G TS-t	MT20	3.0	6.0		
H TMVW-t	MT20	4.0	4.0		
I TTMW-m	MT20	5.0	8.0	2.00	1.75
J TMVW-t	MT20	5.0	6.0	2.50	2.25
K TMV-p	MT20	3.0	4.0		
M BMVW-t	MT20	5.0	6.0	2.25	3.00
N BMVW-t	MT20	4.0	4.0		
O BMVW-t	MT20	4.0	8.0		
P BS-t	MT20	3.0	8.0		
Q BMVW-t	MT20	4.0	9.0		
R BS-t	MT20	3.0	6.0		
S BMVW-t	MT20	4.0	6.0		
T BMVW-t	MT20	4.0	4.0		
U BMVW-t	MT20	5.0	6.0	2.25	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG	IN-SX
U	2086	0	2086	0	0	5-8	5-8	5-8	5-8
M	2086	0	2086	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
M	1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (LBS)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 41	-91.8	-91.8 0.13 (1)	C-T	0 / 158
B-C	0 / 19	-91.8	-91.8 0.14 (1)	T-D	0 / 76
C-D	-2023 / 0	-91.8	-91.8 0.20 (1)	D-S	0 / 1220
D-E	-2308 / 0	-91.8	-91.8 0.55 (1)	S-E	-850 / 0
E-F	-2534 / 0	-91.8	-91.8 0.57 (1)	F-Q	0 / 361
F-G	-2534 / 0	-91.8	-91.8 0.57 (1)	Q-F	-461 / 0
G-H	-2534 / 0	-91.8	-91.8 0.55 (1)	H-Q	0 / 361
H-I	-2308 / 0	-91.8	-91.8 0.55 (1)	I-Q	-850 / 0
I-J	-2023 / 0	-91.8	-91.8 0.20 (1)	Q-I	0 / 1220
J-K	0 / 19	-91.8	-91.8 0.14 (1)	K-I	0 / 76
K-L	0 / 41	-91.8	-91.8 0.13 (1)	L-J	0 / 158
U-B	-246 / 0	0.0	0.0 0.03 (1)	U-C	-2277 / 0
M-K	-246 / 0	0.0	0.0 0.03 (1)	K-M	-2277 / 0
U-T	0 / 1433	-18.5	-18.5 0.33 (1)		
T-S	0 / 1535	-18.5	-18.5 0.35 (1)		
S-R	0 / 2308	-18.5	-18.5 0.42 (1)		
R-Q	0 / 2308	-18.5	-18.5 0.42 (1)		
Q-P	0 / 2308	-18.5	-18.5 0.42 (1)		
P-O	0 / 2308	-18.5	-18.5 0.42 (1)		
O-N	0 / 1535	-18.5	-18.5 0.35 (1)		
N-M	0 / 1433	-18.5	-18.5 0.33 (1)		

TOTAL WEIGHT = 2 X 182 = 323 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 089-08, CSA 089-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/979 (0.23")
CSI: TC=0.57/1.00 (E-F:1), BC=0.42/1.00 (O-C:1),
WB=0.98/1.00 (J-M:1), SS=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

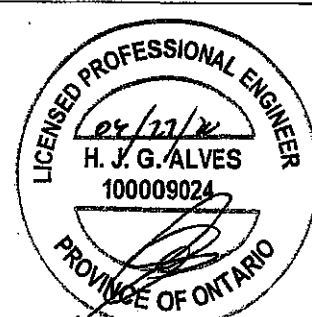
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)

JSI METAL= 0.72 (P) (INPUT = 1.00)

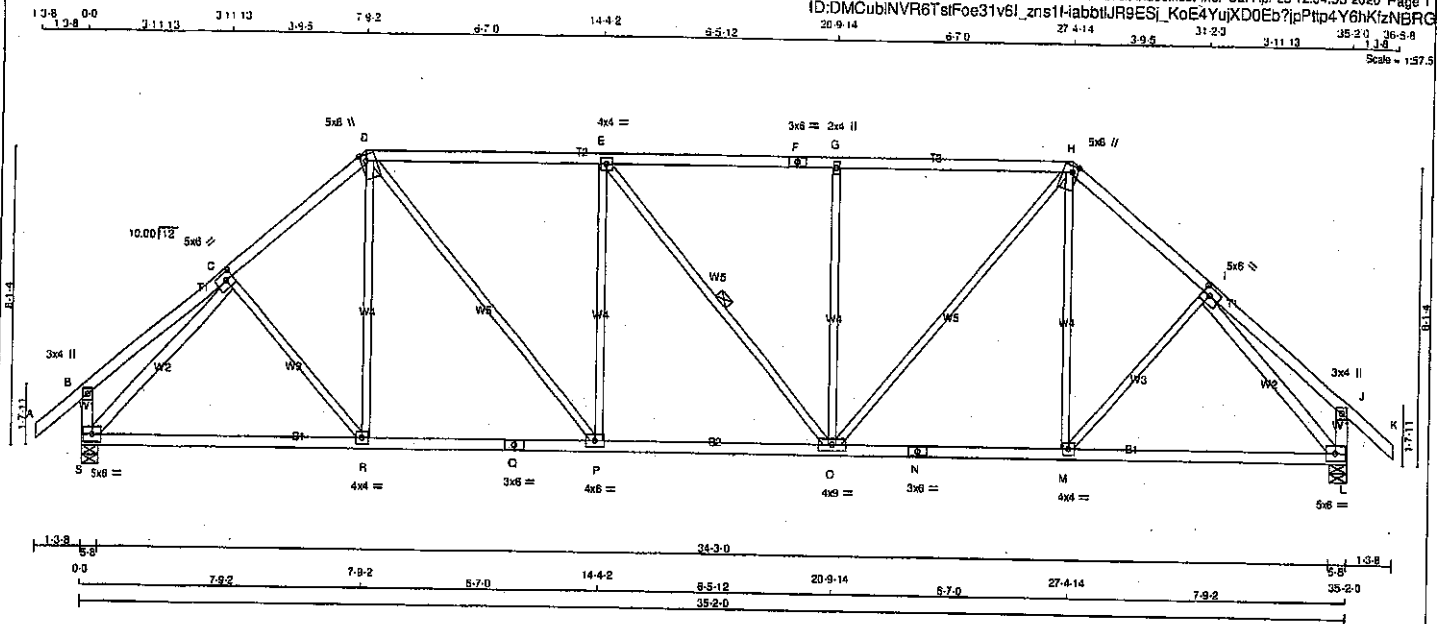


Structural component only
DWG# T-2007133

JOB NAME 408168	TRUSS NAME T5	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6I_zns1HabbtJR9ESJ_KoE4YujXD0Eb7pPtp4Y6hKizNBRC
Scale = 1:37.5



LUMBER	SIZE	DRY	LUMBER
N. L. G. A. RULES			
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
S - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY, SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMV+p	MT20	3.0	4.0	
C TMWw-t	MT20	5.0	6.0	2.50 2.50
D TTWW+m	MT20	5.0	8.0	2.00 1.75
E TMWw-t	MT20	4.0	4.0	
F TS-t	MT20	3.0	6.0	
G TMWw-w	MT20	2.0	4.0	
H TTWW+m	MT20	5.0	8.0	2.00 1.75
I TMWw-t	MT20	5.0	6.0	2.50 2.50
J TMV+p	MT20	3.0	4.0	
L BMVW-t	MT20	5.0	6.0	
M BMWw-t	MT20	4.0	4.0	
N BS-t	MT20	3.0	6.0	
O BMWw-w	MT20	4.0	9.0	
P BMWw-t	MT20	4.0	5.0	
Q BS-t	MT20	3.0	5.0	
R BMWw-t	MT20	4.0	4.0	
S BMVW-t	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	IN-SX	IN-SX
S	2066	0	5-8	5-8
L	2066	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
S	1458 871 / 0 0 / 0 0 / 0 0 / 0 488 / 0 0 / 0
L	1458 871 / 0 0 / 0 0 / 0 0 / 0 488 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
FR-TO						FR-TO			
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	C-R	0 / 64	0.02 (4)	
B-C	0 / 26	-91.8	-91.8	0.21 (1)	10.00	R-D	0 / 126	0.04 (4)	
C-D	-2006 / 0	-91.8	-91.8	0.29 (1)	4.50	D-P	0 / 998	0.22 (1)	
D-E	-2162 / 0	-91.8	-91.8	0.73 (1)	3.81	P-E	-650 / 0	0.82 (1)	
E-F	-2162 / 0	-91.8	-91.8	0.73 (1)	3.81	E-O	-2 / 0	0.00 (1)	
F-G	-2162 / 0	-91.8	-91.8	0.73 (1)	3.81	O-G	-649 / 0	0.82 (1)	
G-H	-2162 / 0	-91.8	-91.8	0.73 (1)	3.82	H-O	0 / 986	0.22 (1)	
H-I	-2006 / 0	-91.8	-91.8	0.29 (1)	4.50	M-H	0 / 126	0.04 (4)	
I-J	0 / 26	-91.8	-91.8	0.21 (1)	10.00	M-I	0 / 64	0.02 (4)	
J-K	0 / 41	-91.8	-91.8	0.13 (1)	10.00	S-C	-2291 / 0	0.97 (1)	
S-B	-264 / 0	0.0	0.0	0.03 (1)	7.81	I-L	-2291 / 0	0.97 (1)	
L-J	-264 / 0	0.0	0.0	0.03 (1)	7.81				
S-R	0 / 1487	-18.5	-18.5	0.38 (1)	10.00				
R-Q	0 / 1519	-18.5	-18.5	0.39 (1)	10.00				
Q-P	0 / 1519	-18.5	-18.5	0.39 (1)	10.00				
P-O	0 / 2163	-18.5	-18.5	0.42 (1)	10.00				
O-N	0 / 1519	-18.5	-18.5	0.39 (1)	10.00				
N-M	0 / 1519	-18.5	-18.5	0.39 (1)	10.00				
M-L	0 / 1487	-18.5	-18.5	0.38 (1)	10.00				

TOTAL WEIGHT = 2 X 163 = 326 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/999 (0.10")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = L/999 (0.21")

CSI: TC=0.73/1.00 (D-E:1), 8C=0.42/1.00 (O-P:1),
WB=0.97/1.00 (H-L:1), SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

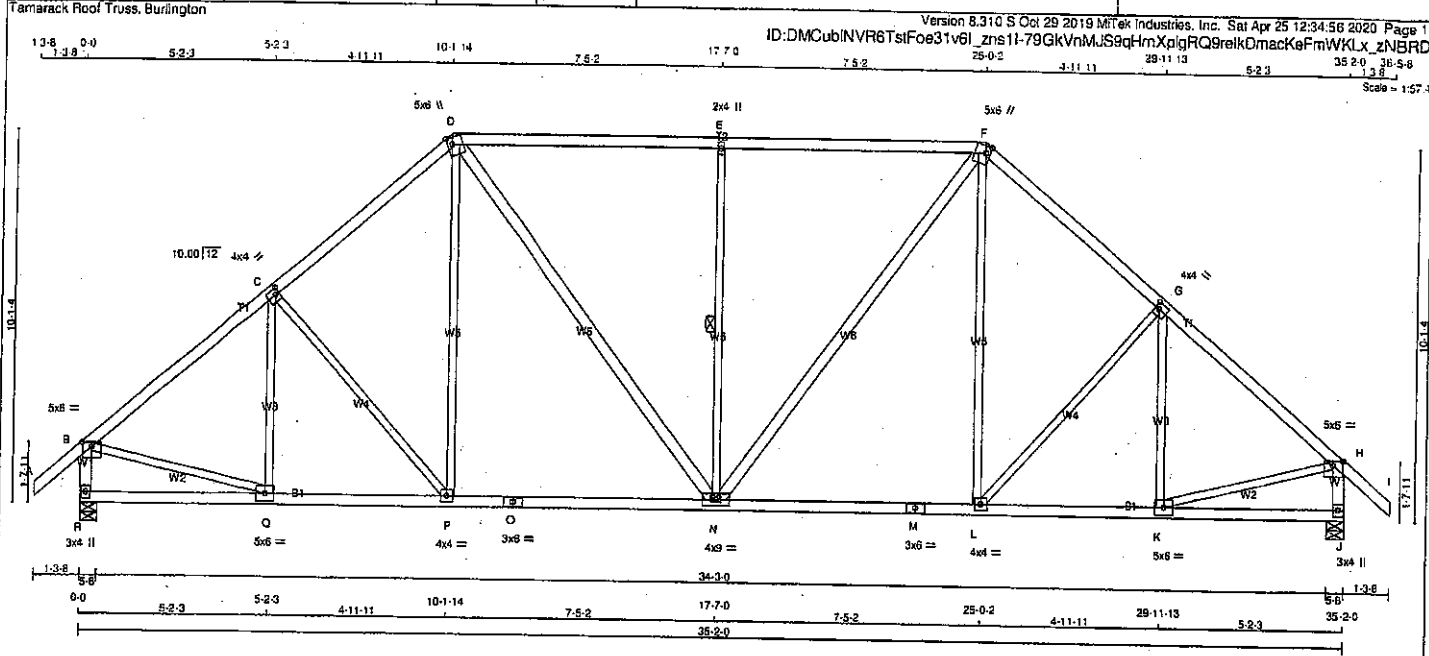
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (I) (INPUT = 0.90)
JSI METAL= 0.56 (I) (INPUT = 1.00)



Structural component only
DWG# T-2007134

Structural component only
DWG# T-2007135



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - F	2x4	DRY No.2
F - I	2x4	DRY No.2
R - B	2x4	DRY No.2
J - H	2x4	DRY No.2
R - O	2x4	DRY No.2
O - M	2x4	DRY No.2
M - J	2x4	DRY No.2
ALL WEBS	2x3	DRY No.2
EXCEPT		
D - N	2x4	DRY No.2
N - F	2x4	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMVW-i	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	6.0	2.25	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	1.50
G	TMVW-i	MT20	4.0	4.0	2.00	1.25
H	TMVW-p	MT20	5.0	6.0	1.50	3.00
J	BMV1-p	MT20	3.0	4.0		
K	BMVW-i	MT20	5.0	6.0		
L	BMVW-i	MT20	4.0	4.0		
M	BS-i	MT20	3.0	8.0		
N	BMVWW-1	MT20	4.0	9.0		
O	BS-i	MT20	3.0	8.0		
P	BMVW-i	MT20	4.0	4.0		
Q	BMVW-i	MT20	5.0	6.0		
R	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
R	2068	0	2098	0	5-8	5-8
J	2068	0	2068	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1458	971/0	0/0	0/0	0/0	488/0	0/0
J	1458	971/0	0/0	0/0	0/0	488/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			MAX. UNBRACED LENGTH	MEMB.	WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (LC)			MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (LC)
FR-TO		FROM	TO		FR-TO		FROM	TO
A-B	0/41	-91.8	-91.8	0.13 (1)	10.00	Q-C	-302/0	0.17 (1)
B-C	-2021/0	-91.8	-91.8	0.39 (1)	4.41	C-P	-217/0	0.25 (1)
C-D	-1908/0	-91.8	-91.8	0.37 (1)	4.52	P-D	0/278	0.08 (1)
D-E	-1809/0	-91.8	-91.8	0.74 (1)	3.96	D-N	0/615	0.10 (1)
E-F	-1809/0	-91.8	-91.8	0.74 (1)	3.96	N-E	-838/0	0.58 (1)
F-G	-1908/0	-91.8	-91.8	0.37 (1)	4.52	N-F	0/615	0.10 (1)
G-H	-2021/0	-91.8	-91.8	0.39 (1)	4.41	L-F	0/278	0.08 (1)
H-I	0/41	-91.8	-91.8	0.13 (1)	10.00	L-G	-217/0	0.25 (1)
R-B	-2024/0	0.0	0.0	0.22 (1)	5.94	K-G	-302/0	0.17 (1)
J-H	-2024/0	0.0	0.0	0.22 (1)	5.94	B-Q	0/1829	0.37 (1)
R-Q	0/0	-18.5	-18.5	0.10 (4)	10.00	K-H	0/1829	0.37 (1)
Q-P	0/1580	-18.5	-18.5	0.34 (1)	10.00			
P-O	0/1437	-18.5	-18.5	0.35 (1)	10.00			
O-N	0/1437	-18.5	-18.5	0.35 (1)	10.00			
N-M	0/1437	-18.5	-18.5	0.35 (1)	10.00			
M-L	0/1437	-18.5	-18.5	0.35 (1)	10.00			
L-K	0/1580	-18.5	-18.5	0.34 (1)	10.00			
K-J	0/0	-18.5	-18.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17)
CALCULATED VERT. DEFL.(LL) = L/999 (0.08)
ALLOWABLE DEFL.(TL) = L/360 (1.17)
CALCULATED VERT. DEFL.(TL) = L/999 (0.17)

CSI: TC=0.74/1.00 (D-E:1), BC=0.35/1.00 (L-N:1), WB=0.59/1.00 (E-N:1), SSI=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

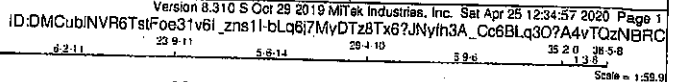
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (H) (INPUT = 0.90)
JSI METAL = 0.48 (O) (INPUT = 1.00)

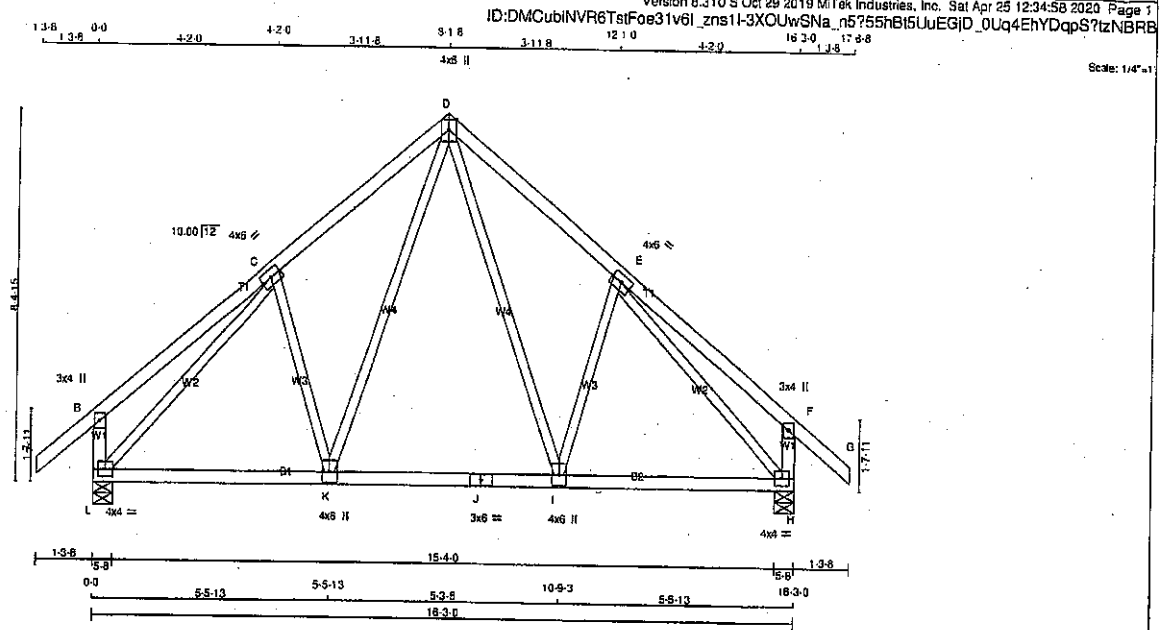


Tamarack Roof Truss, Burlington



CONTINUED ON PAGE 2

JOB NAME 408168	TRUSS NAME T9	QUANTITY 6	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-i	MT20	4.0	8.0		
D	TTWW+p	MT20	4.0	8.0		
E	TMWW-i	MT20	4.0	8.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-i	MT20	4.0	4.0		
I	BMVW1-i	MT20	4.0	6.0		
J	BS-i	MT20	3.0	6.0		
K	BMVW1-i	MT20	4.0	6.0		
L	BMVW1-i	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECD BRG
JT	VERT	HORZ	DOWN	HORZ
L	1023	0	1023	0
H	1023	0	1023	0

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	721	487 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0
H	721	487 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	MAX. FACTORED (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (PLF)	MAX. FACTORED (LBS)
FR-TO				FR-TO			
A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00	D-I	0 / 390	0.07 (1)
B-C	0 / 28	-91.8	-91.8 0.24 (1)	10.00	I-E	-216 / 0	0.09 (1)
C-D	-745 / 0	-91.8	-91.8 0.19 (1)	6.25	K-D	0 / 390	0.07 (1)
D-E	-745 / 0	-91.8	-91.8 0.19 (1)	6.25	C-K	-216 / 0	0.09 (1)
E-F	0 / 28	-91.8	-91.8 0.24 (1)	10.00	L-C	-943 / 0	0.61 (1)
F-G	0 / 41	-91.8	-91.8 0.13 (1)	10.00	E-H	-943 / 0	0.61 (1)
L-B	-267 / 0	0.0	0.0 0.03 (1)	7.81			
H-F	-267 / 0	0.0	0.0 0.03 (1)	7.81			
L-K	0 / 616	-18.5	-18.5 0.17 (4)	10.00			
K-J	0 / 449	-18.5	-18.5 0.16 (4)	10.00			
J-I	0 / 449	-18.5	-18.5 0.16 (4)	10.00			
I-H	0 / 616	-18.5	-18.5 0.17 (4)	10.00			

TOTAL WEIGHT = 6 X 80 = 481 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6 PSF
	DL	= 6.0 PSF
BOT CH.	LL	= 0.0 PSF
	DL	= 7.4 PSF
TOTAL LOAD		= 39.0 PSF

SPACING = 24.8 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/899 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/899 (0.04")

CSI: TC=0.24/1.00 (B-C:1), BC=0.17/1.00 (H-I:4), WB=0.61/1.00 (C-L:1), SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

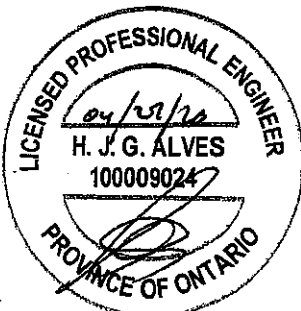
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	518 354	1667 789

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.76 (C) (INPUT = 0.90)
JSI METAL = 0.25 (C) (INPUT = 1.00)

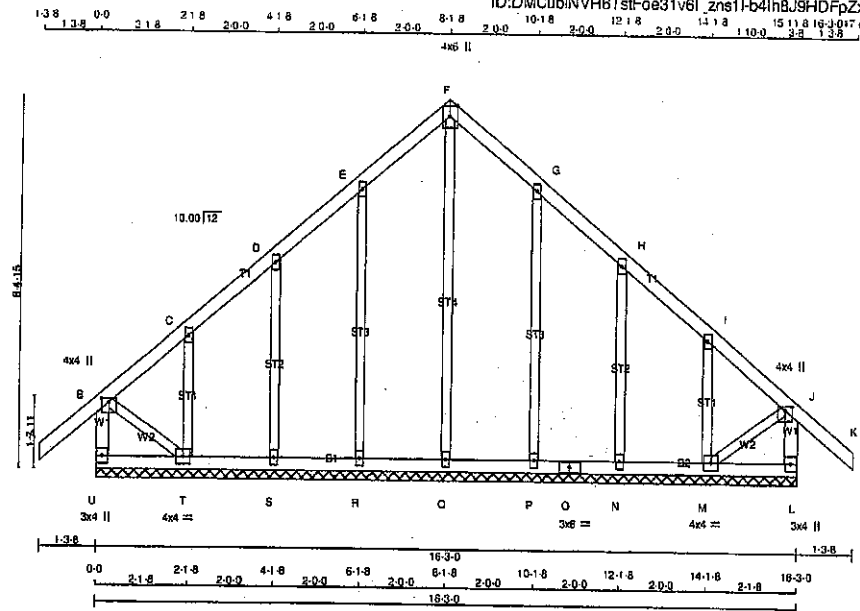


Structural component only
DWG# T-2007138

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC	DRWG NO.
408168	G9	1	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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Scale: 1/4"=1'

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - F	2x4 DRY	No.2		
F - K	2x4 DRY	No.2		
U - B	2x4 DRY	No.2		
L - J	2x4 DRY	No.2		
U - O	2x4 DRY	No.2		
O - L	2x4 DRY	No.2		

ALL WEBS	2x3 DRY	No.2
ALL GABLE WEBS	2x3 DRY	No.2
DRY: SEASONED LUMBER.		

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00	
C, D, E, G, H, I						
C TMVW+w	MT20	2.0	4.0			
F TTW+p	MT20	4.0	6.0	Edge		
J TMVW+p	MT20	4.0	4.0	1.00	2.00	
L BMV1+p	MT20	3.0	4.0			
M BMVW1+t	MT20	4.0	4.0			
N, P, Q, R, S						
N BMV1+w	MT20	2.0	4.0			
O BS-t	MT20	3.0	6.0			
T BMVW1-t	MT20	4.0	4.0			
U BMV1+p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)
FR-TO				FR-TO			
A-B	0/41	-81.8	-81.8 0.13 (1)	Q-F	-124/0	0.17 (1)	0.17 (1)
B-C	-28/0	-81.8	-81.8 0.05 (1)	R-E	-210/0	0.16 (1)	0.16 (1)
C-D	-36/0	-81.8	-81.8 0.05 (1)	S-D	-169/0	0.06 (1)	0.06 (1)
D-E	-24/0	-81.8	-81.8 0.05 (1)	T-C	-205/0	0.04 (1)	0.04 (1)
E-F	-36/0	-81.8	-81.8 0.05 (1)	P-G	-210/0	0.16 (1)	0.16 (1)
F-G	-36/0	-81.8	-81.8 0.05 (1)	N-H	-169/0	0.06 (1)	0.06 (1)
G-H	-24/0	-81.8	-81.8 0.05 (1)	M-I	-205/0	0.04 (1)	0.04 (1)
H-I	-36/0	-81.8	-81.8 0.05 (1)	B-T	0/37	0.01 (1)	0.01 (1)
I-J	-28/0	-81.8	-81.8 0.05 (1)	M-J	0/37	0.01 (1)	0.01 (1)
J-K	0/41	-81.8	-81.8 0.13 (1)				
U-B	-246/0	0.0	0.0 0.03 (1)				
L-J	-246/0	0.0	0.0 0.03 (1)				
U-T	0/0	-18.5	-18.5 0.02 (4)				
T-S	0/25	-18.5	-18.5 0.02 (4)				
S-R	0/21	-18.5	-18.5 0.02 (4)				
R-Q	0/18	-18.5	-18.5 0.02 (4)				
Q-P	0/19	-18.5	-18.5 0.02 (4)				
P-O	0/21	-18.5	-18.5 0.02 (4)				
O-N	0/21	-18.5	-18.5 0.02 (4)				
N-M	0/25	-18.5	-18.5 0.02 (4)				
M-L	0/0	-18.5	-18.5 0.02 (4)				

TOTAL WEIGHT = 81 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6 PSF
DL =	6.0 PSF	
BOT CH.	LL =	0.0 PSF
DL =	7.4 PSF	
TOTAL LOAD	=	39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2016, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (M-N:4), WB=0.17/1.00 (F-G:1), SS1=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	
MAX	MIN	MAX	MIN
MT20	618	354	1657 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (T) (INPUT = 0.50)

JSI METAL= 0.11 (G) (INPUT = 1.00)



Structural component only
DWG# T-2007123

DRWG NO.

ID:DMCubINVR6TstFoe31v6l_zns1l-Xjys8OOCi4DsiFFNQo?7nUGOUQrOpqnhSUZ?XJzNBRA

Scale = 1:30.3



N. L. G. A. RULES

CHORDS	SIZE		LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	577	0	577	0	0	5-B	5-B
F	577	0	577	0	0	5-B	5-B

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
	406	280 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0
F	406	280 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FORCE (LBS)	FACTORED MAX CSI (LC)
FR-TO		FROM TO		FR-TO			
A-B	0/41	-91.8	-91.8 0.13 (1)	10.00	G-C	-37/57	0.02 (4)
B-C	-284/0	-91.8	-91.8 0.20 (1)	6.25	B-G	0/213	0.05 (1)
C-D	-264/0	-91.8	-91.8 0.20 (1)	6.25	G-D	0/213	0.05 (1)
D-E	0/41	-91.8	-91.8 0.13 (1)	10.00			
H-B	-548/0	0.0	0.0 0.08 (1)	7.81			
F-D	-548/0	0.0	0.0 0.08 (1)	7.81			
H-G	0/0	-18.5	-18.5 0.09 (4)	10.00			
G-F	0/0	-18.5	-18.5 0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD =		39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.20/1.00 (B-C:1), BC=0.09/1.00 (F-G:4),
WB=0.05/1.00 (D-G:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

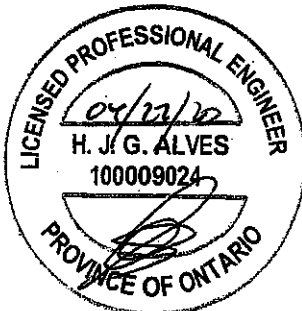
NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GR/P= 0.46 (D) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)

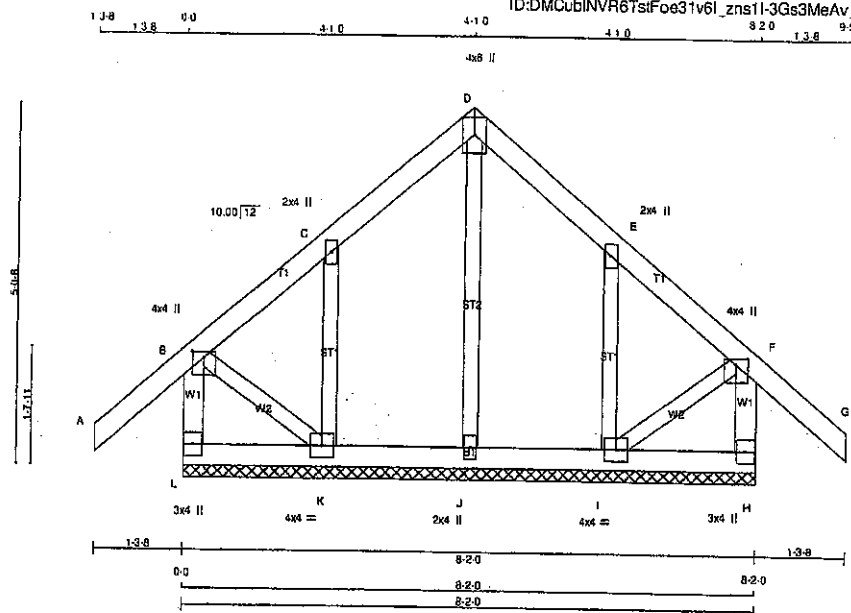


Structural component only
DWG# T-2007139

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 12:34:41 2020 Page 1
ID:DMCubINVR6TstFoe31v6l_zns1l-3Gs3MeAv_YyQYUwN0gv2hWL7AiCchl2loC3vMzNBBS

Scale = 1:39.



TOTAL WEIGHT = 40 LB

LUMBER		
N. L. G. A. RULES		
CHORDS	SIZE	LUMBER
L - B	2x4	DRY No.2
A - D	2x4	DRY No.2
D - G	2x4	DRY No.2
H - F	2x4	DRY No.2
L - H	2x4	DRY No.2

ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS			
	2x3	DRY	No.2
DRY: SEASONED LUMBER			

GABLE STUDS SPACED AT 2'-0" O.C.

PLATES (tablets in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV+p	MT20	4.0	4.0	1.00	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMWV+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWV1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWV1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
L-B	-224 / 0	0.0	0.0	0.02 (†)	7.81	J-D	-126 / 0	0.05 (†)	
A-B	0 / 41	-91.8	-91.8	0.15 (†)	10.00	K-C	-227 / 0	0.04 (†)	
B-C	-11 / 0	-91.8	-91.8	0.06 (†)	8.25	I-E	-227 / 0	0.04 (†)	
C-D	-31 / 0	-91.8	-91.8	0.09 (†)	8.25	B-K	0 / 24	0.01 (†)	
D-E	-31 / 0	-91.8	-91.8	0.06 (†)	8.25	I-F	0 / 24	0.01 (†)	
E-F	-11 / 0	-91.8	-91.8	0.06 (†)	8.25				
F-G	0 / 41	-91.8	-91.8	0.13 (†)	10.00				
H-F	-224 / 0	0.0	0.0	0.02 (†)	7.81				
L-K	0 / 0	-18.5	-18.5	0.02 (4)	10.00				
K-J	0 / 13	-18.5	-18.5	0.02 (4)	10.00				
J-I	0 / 13	-18.5	-18.5	0.02 (4)	10.00				
I-H	0 / 0	-18.5	-18.5	0.02 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (J-K:4),
WB=0.05/1.00 (D-J:1), SS=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

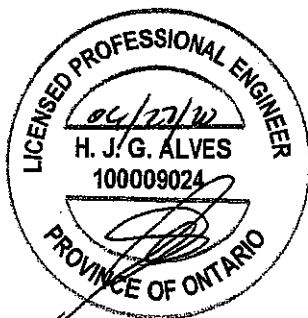
NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	708	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.17 (C) (INPUT = 0.90)
JSI METAL= 0.12 (C) (INPUT = 1.00)

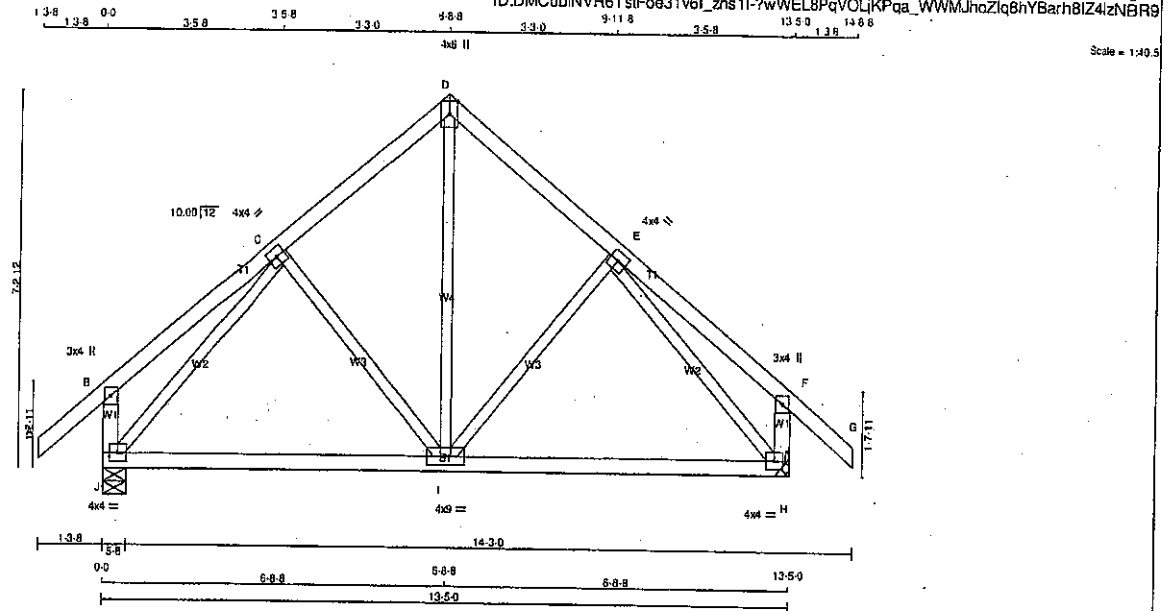


Structural component only
DWG# T-2007124

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408168	T11	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubiNVR6TstFoe31v6t_zns11-7wWEL8PqVOLiKPqa_WWMJhoZlq8hYBarh8IZ4izNBR9



Scale = 1/40.5

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2		
D - G	2x4	DRY	No.2		
J - B	2x4	DRY	No.2		
H - F	2x4	DRY	No.2		
J - H	2x4	DRY	No.2		

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)	PLATES	W	LEN	Y	X
JT TYPE	MT20	3.0	4.0		
B TMV+p	MT20	4.0	4.0		
C TMVW-t	MT20	4.0	4.0		
D TTW+p	MT20	4.0	6.0		Edge
E TMVW-t	MT20	4.0	4.0		
F TMV+p	MT20	3.0	4.0		
H BMVW-t	MT20	4.0	4.0		
I BMVW-t	MT20	4.0	9.0		
J BMVW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UPLIFT	IN-SX
J	867	0	0	5-8
H	867	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX /MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
J	611	414 / 0	0 / 0	0 / 0	0 / 0	198 / 0
H	611	414 / 0	0 / 0	0 / 0	0 / 0	198 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD LC1	MAX	MAX	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. (PLF)	CS (LC)	UNBRAC	MEMB.	FORCE (LBS)	MAX
FR-TO				LENGTH	FR-TO		
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	I-D	0 / 382
B-C	0 / 23	-91.8	-91.8	0.16 (1)	10.00	I-E	-149 / 0
C-D	-509 / 0	-91.8	-91.8	0.13 (1)	8.25	C-I	-149 / 0
D-E	-509 / 0	-91.8	-91.8	0.13 (1)	6.25	J-C	-742 / 0
E-F	0 / 23	-91.8	-91.8	0.16 (1)	10.00	E-H	-742 / 0
F-G	0 / 41	-91.8	-91.8	0.13 (1)	10.00		
J-B	-244 / 0	0.0	0.0	0.03 (1)	7.81		
H-F	-244 / 0	0.0	0.0	0.03 (1)	7.81		
J-I	0 / 489	-18.5	-18.5	0.27 (4)	10.00		
I-H	0 / 489	-18.5	-18.5	0.27 (4)	10.00		

TOTAL WEIGHT = 3 X 64 = 191 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2018 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.45")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.45")
CALCULATED VERT. DEFL. (TL) = L/999 (0.05")

CSI: TC=0.16/1.00 (E-F:1), BC=0.27/1.00 (I-J:4),
WB=0.33/1.00 (E-H:1), SSI=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=t.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

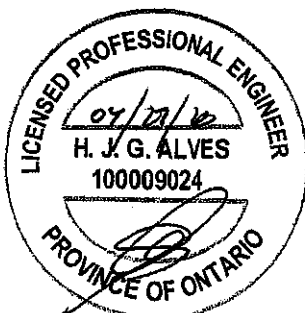
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.84 (E) (INPUT = 0.90)
JSI METAL=0.27 (E) (INPUT = 1.00)

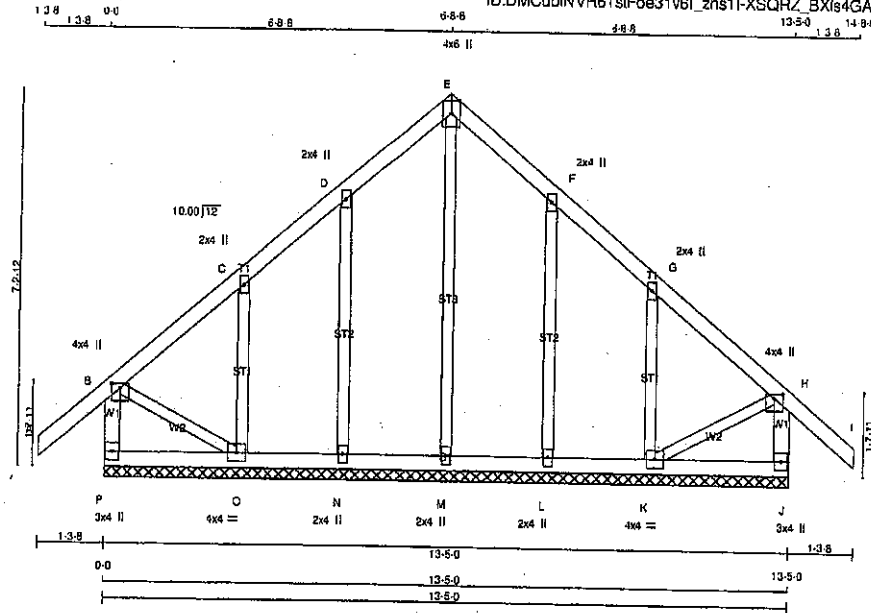


Structural component only
DWG# T-2007140

JOB NAME 408168	TRUSS NAME G11	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTEK industries, Inc. Sat Apr 25 12:34:42 2020 Page 1
ID:DMCubINVR6TstFoe31v6L_zns1I-XSQRZ_BXlg4GAeS6xkBBau3WIZ3HL6XBYKxcRozNBRR



Scale = 1/40.5

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF	
A - E	2x4	DRY	No.2	SPF	
E - I	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
P - J	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2
ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" O.C.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00	
C, D, F, G						
E TMVW+w	MT20	2.0	4.0			
H TTW+p	MT20	4.0	6.0	Edge		
J BMV1+p	MT20	3.0	4.0			
K BMVW1-t	MT20	4.0	4.0			
L, M, N						
L BMV1+w	MT20	2.0	4.0			
O BMVW1-t	MT20	4.0	4.0			
P BMV1+p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CHORDS	MAX. FACTORED	VERT. LOAD	FACTORED	MAX.	MAX.	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	LC1	FROM TO	CS1 (LC)	UNBRAC LENGTH	MEMB.	FORCE (LBS)
FR-TO						FR-TO	
P-B	-263 / 0	0.0	0.0	0.03 (1)	7.81	M-E	-128 / 0
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	N-D	-190 / 0
B-C	-24 / 0	-91.8	-91.8	0.05 (1)	6.25	O-C	-239 / 0
C-D	-36 / 0	-91.8	-91.8	0.08 (1)	6.25	L-F	-190 / 0
D-E	-35 / 0	-91.8	-91.8	0.05 (1)	6.25	K-G	-239 / 0
E-F	-35 / 0	-91.8	-91.8	0.08 (1)	6.25	B-O	0 / 32
F-G	-36 / 0	-91.8	-91.8	0.08 (1)	6.25	K-H	0 / 32
G-H	-24 / 0	-91.8	-91.8	0.08 (1)	6.25		
H-I	0 / 41	-91.8	-91.8	0.13 (1)	10.00		
J-H	-263 / 0	0.0	0.0	0.03 (1)	7.81		
P-O	0 / 0	-18.5	-18.5	0.03 (4)	10.00		
O-N	0 / 22	-18.5	-18.5	0.03 (4)	10.00		
N-M	0 / 19	-18.5	-18.5	0.02 (4)	10.00		
M-L	0 / 19	-18.5	-18.5	0.02 (4)	10.00		
L-K	0 / 22	-18.5	-18.5	0.03 (4)	10.00		
K-J	0 / 0	-18.5	-18.5	0.03 (4)	10.00		

TOTAL WEIGHT = 65 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.13/1.00 (H-I:1), BC=0.03/1.00 (K-L:4), WB=0.11/1.00 (E-M:1), SS=0.09/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 768 1987 1656

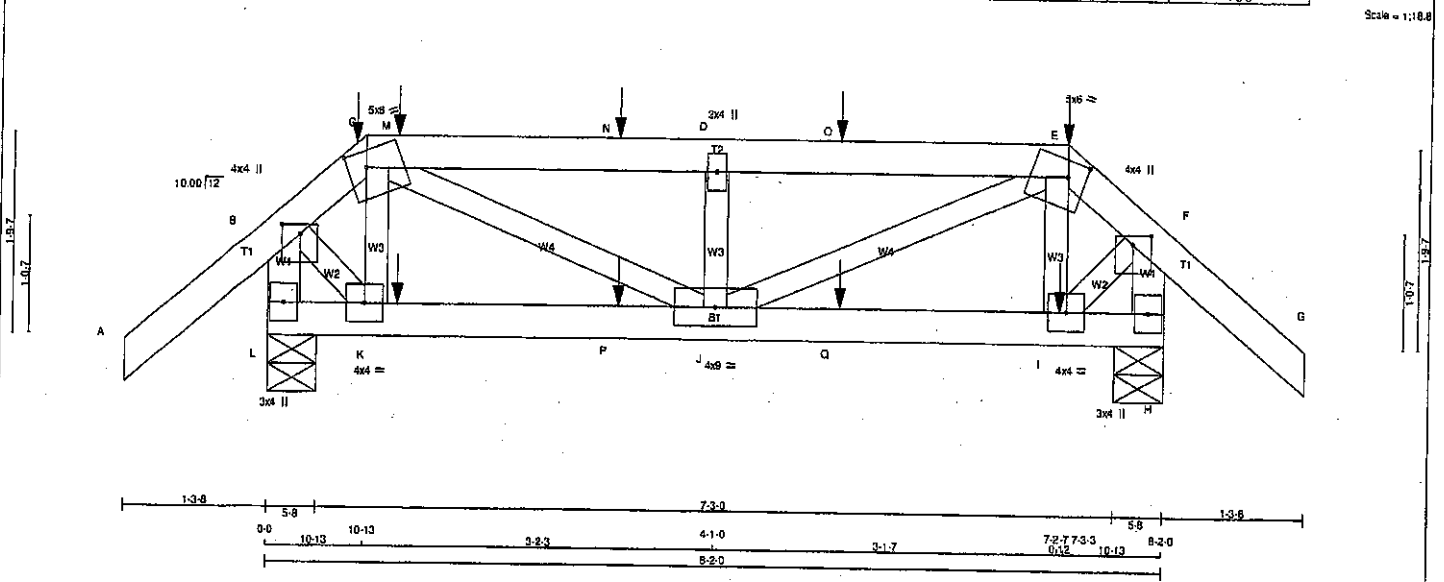
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.20 (B) (INPUT = 0.90)
JSI METAL = 0.13 (G) (INPUT = 1.00)



Structural component only
DWG# T-2007125



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW-m	MT20	5.0	5.0	1.75	2.00
D	TMVW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	5.0	1.75	2.00
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	9.0		
J	BMVWW-t	MT20	4.0	4.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	DOWN	BRG	BRG
L 913	0	IN-SX	IN-SX
H 911	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT COMBINED	SNOW	LIVE	PERM. LIVE	
L 642	442 / 0	0 / 0	0 / 0	189 / 0
H 641	437 / 0	0 / 0	0 / 0	204 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.82 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	W. B. S.	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO					FR-TO			
A-B	0 / 41	-91.8	-91.8	0.14 (1)	10.00	J-D	-487 / 0	0.08 (1)
B-C	-631 / 0	-91.8	-91.8	0.14 (1)	6.25	K-C	-156 / 0	0.02 (1)
C-M	-1054 / 0	-91.8	-91.8	0.23 (1)	5.82	C-J	0 / 700	0.17 (1)
M-N	-1054 / 0	-91.8	-91.8	0.23 (1)	5.82	B-K	0 / 525	0.13 (1)
N-D	-1054 / 0	-91.8	-91.8	0.23 (1)	5.82	I-E	-149 / 0	0.02 (1)
D-O	-1054 / 0	-91.8	-91.8	0.23 (1)	5.82	J-E	0 / 698	0.17 (1)
O-E	-1054 / 0	-91.8	-91.8	0.23 (1)	5.82	I-F	0 / 526	0.13 (1)
E-F	-632 / 0	-91.8	-91.8	0.14 (1)	6.25			
F-G	0 / 41	-91.8	-91.8	0.14 (1)	10.00			
L-B	-936 / 0	0.0	0.0	0.10 (1)	7.81			
H-F	-937 / 0	0.0	0.0	0.10 (1)	7.81			
L-K	0 / 0	-18.5	-18.5	0.05 (1)	10.00			
K-P	0 / 420	-18.5	-18.5	0.13 (1)	10.00			
P-J	0 / 420	-18.5	-18.5	0.13 (1)	10.00			
J-Q	0 / 421	-18.5	-18.5	0.14 (1)	10.00			
Q-I	0 / 421	-18.5	-18.5	0.14 (1)	10.00			
I-H	0 / 0	-18.5	-18.5	0.05 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	10-13	-9	-10	-10	FRONT	VERT	TOTAL		C1
C	1-2-7	-114	-114	-114	BACK	VERT	TOTAL		C1
C	10-13	-44	-44	-44	FRONT	VERT	SNOW		C1
E	7-3-3	-9	-10	-10	FRONT	VERT	DEAD		C1
E	7-3-3	-90	-90	-90	BACK	VERT	TOTAL		C1
E	7-3-3	-44	-44	-44	FRONT	VERT	SNOW		C1
I	7-2-7	-49	-49	-49	BACK	VERT	TOTAL		C1
K	1-2-7	-49	-49	-49	BACK	VERT	TOTAL		C1
N	3-2-7	-82	-82	-82	BACK	VERT	TOTAL		C1
O	5-2-7	-82	-82	-82	BACK	VERT	TOTAL		C1
P	3-2-7	-49	-49	-49	BACK	VERT	TOTAL		C1
Q	5-2-7	-49	-49	-49	BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 34 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT

NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

*** NON STANDARD GIRDER ***

ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.27")

CALCULATED VERT. DEFL.(LL) = L/999 (0.02")

ALLOWABLE DEFL.(TL) = L/360 (0.27")

CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.23/1.00 (C-D:1), BC=0.14/1.00 (I-J:1), WB=0.17/1.00 (C-J:1), SS=0.22/1.00 (C-D:1)

DOI LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (FLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.



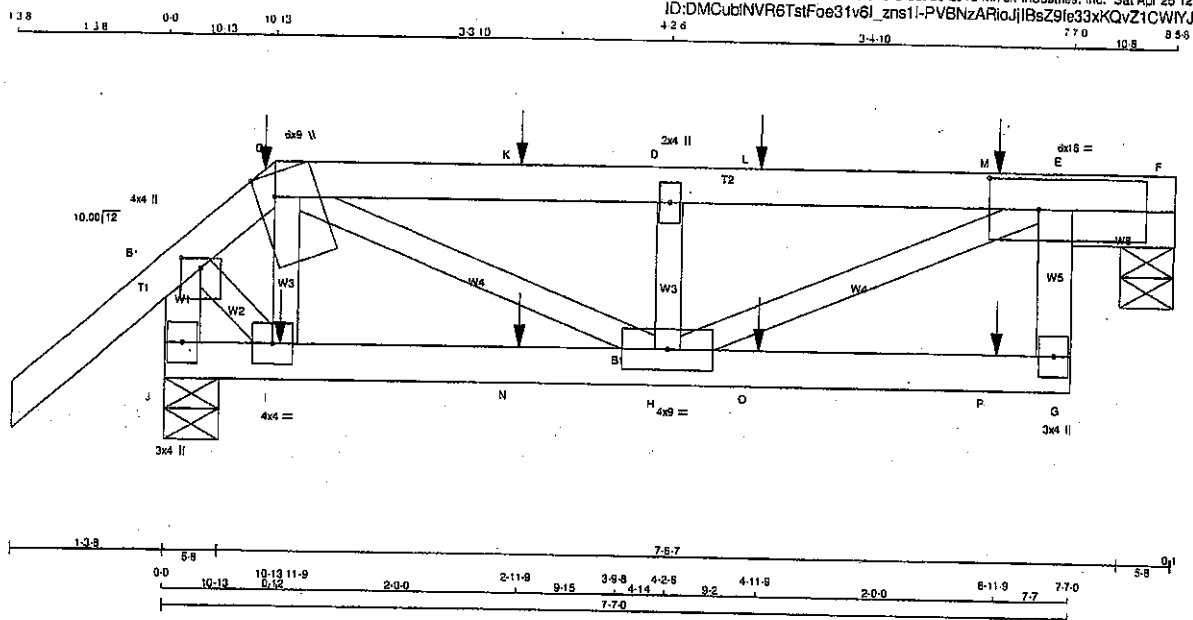
Structural component only

DWG# T-2007141

JOB NAME 408168	TRUSS NAME T13	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:12.5

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - B	2x4	DRY	No.2
J - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	8.0	9.0	Edge	1.75
D	TMVW-w	MT20	2.0	4.0		
E	TMVW-w	MT20	6.0	16.0	3.00	5.00
G	BMV+p	MT20	3.0	4.0		
H	BMVW-w	MT20	4.0	9.0		
I	BMVW-w	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
F	685	0	685	0	5-8
J	936	0	936	0	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): F

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	490	327 / 0	0 / 0	0 / 0	0 / 0	183 / 0	0 / 0
J	659	449 / 0	0 / 0	0 / 0	0 / 0	210 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8	-91.8 0.14 (1)	I-C	-158 / 0	0.03 (1)	
B-C	-656 / 0	-91.8	-91.8 0.14 (1)	B-I	0 / 548	0.14 (1)	
C-K	-1230 / 0	-91.8	-91.8 0.46 (1)	H-D	-767 / 0	0.12 (1)	
K-D	-1230 / 0	-91.8	-91.8 0.46 (1)	C-H	0 / 867	0.21 (1)	
D-L	-1230 / 0	-91.8	-91.8 0.89 (1)	H-E	0 / 1345	0.33 (1)	
L-M	-1230 / 0	-91.8	-91.8 0.89 (1)				
M-E	-1230 / 0	-91.8	-91.8 0.89 (1)				
E-F	0 / 0	-91.8	-91.8 0.35 (1)				
G-E	0 / 83	0.0	0.0 0.02 (4)				
J-B	-956 / 0	0.0	0.0 0.11 (1)				
J-I	0 / 0	-18.5	-18.5 0.06 (1)				
I-N	0 / 438	-18.5	-18.5 0.14 (1)				
N-H	0 / 438	-18.5	-18.5 0.14 (1)				
H-O	0 / 0	-18.5	-18.5 0.09 (1)				
O-P	0 / 0	-18.5	-18.5 0.09 (1)				
P-G	0 / 0	-18.5	-18.5 0.09 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	10-13	-9	-10		FRONT	VERT	DEAD		C1
C	10-13	-90	-30		FRONT	VERT	TOTAL		C1
C	10-13	-44	-44		FRONT	VERT	SNOW		C1
I	11-9	-49	-49		FRONT	VERT	TOTAL		C1
K	2-11-9	-82	-82		FRONT	VERT	TOTAL		C1
L	4-11-9	-82	-82		FRONT	VERT	TOTAL		C1
M	6-11-9	-82	-82		FRONT	VERT	TOTAL		C1
N	2-11-9	-49	-49		FRONT	VERT	TOTAL		C1
O	4-11-9	-49	-49		FRONT	VERT	TOTAL		C1
P	6-11-9	-53	-53		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 32 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/960 (0.25")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/960 (0.25")
CALCULATED VERT. DEFL.(TL) = L/688 (0.13")

CSI: TC=0.98/1.00 (D-E:1), BC=0.14/1.00 (H-I:1),
WB=0.33/1.00 (E-H:1), SSI=0.42/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

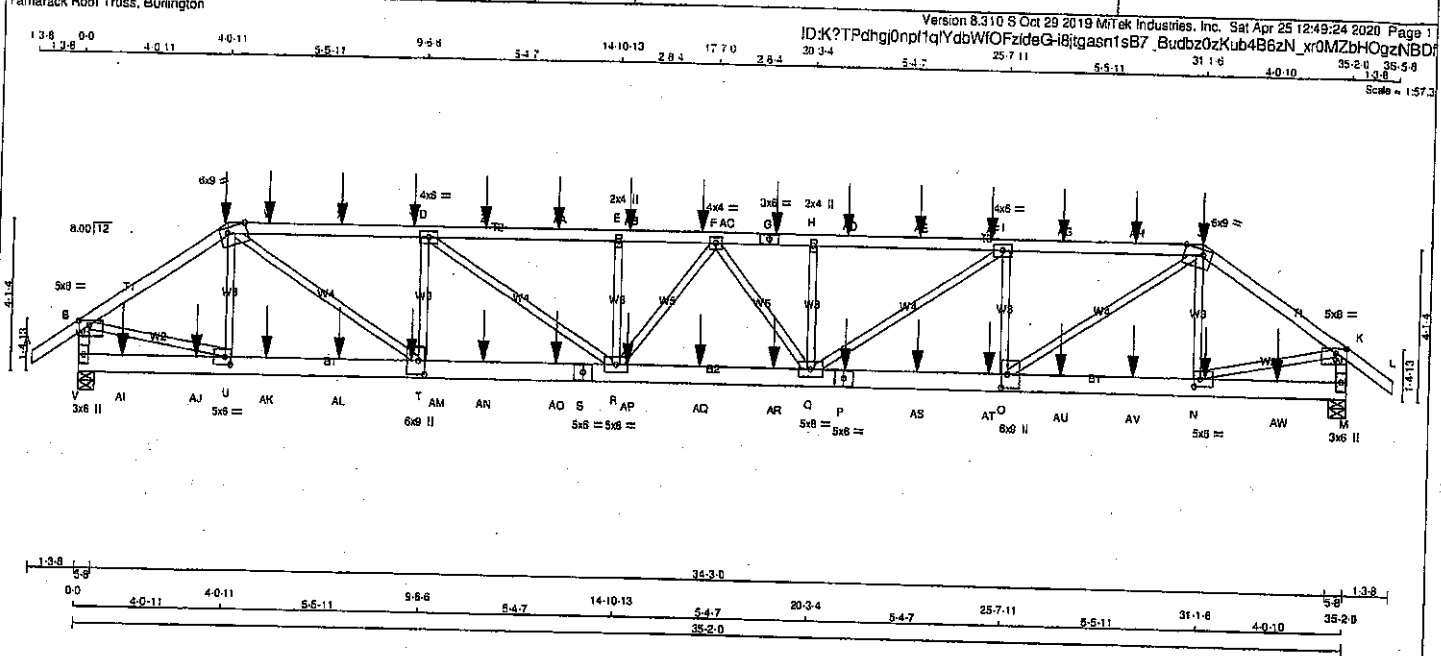
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.30 (H) (INPUT = 1.00)



Structural component only
DWG# T-2007142

JOB NAME 408169	TRUSS NAME T20	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
V - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
V - S	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE(61.0)
C - G	12	SIDE(61.0)
G - J	12	SIDE(61.0)
J - L	12	SIDE(61.0)
V - B	12	TOP
M - K	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
V - S	12	SIDE(183.1)
S - P	12	SIDE(183.1)
P - M	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	IN-SX	IN-SX
V 3342	0	5-8	5-8
M 3393	0	5-8	5-8

UNFACTORED REACTIONS			
1ST LOSE	MAX./MIN. COMPONENT REACTIONS		
JT	SNOW	LIVE	PERM.LIVE
V 2363	1555 / 0	0 / 0	0 / 0
M 2397	1585 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)
FR-TO			
A-B	0 / 35	-91.8	10.00
B-C	-3951 / 0	-91.8	4.58
C-W	-8188 / 0	-91.8	3.41
W-X	-8188 / 0	-91.8	3.41
X-Y	-8188 / 0	-91.8	3.41
Y-D	-8188 / 0	-91.8	3.41
D-Z	-7502 / 0	-91.8	3.03
Z-AA	-7502 / 0	-91.8	3.03
AA-E	-7502 / 0	-91.8	3.03
E-AB	-7502 / 0	-91.8	3.03
AB-AC	-7502 / 0	-91.8	3.03
AC-F	-7502 / 0	-91.8	3.03
F-G	-7509 / 0	-91.8	3.31
G-H	-7509 / 0	-91.8	3.31
H-AD	-7509 / 0	-91.8	3.03
AD-AE	-7509 / 0	-91.8	3.03
AE-AF	-7509 / 0	-91.8	3.03
AF-I	-7509 / 0	-91.8	3.03
I-AG	-6217 / 0	-91.8	3.41
AG-AH	-6217 / 0	-91.8	3.41
AH-J	-6217 / 0	-91.8	3.41
J-K	-4025 / 0	-91.8	4.55
K-L	0 / 35	-91.8	10.00
V-B	-3298 / 0	0.0	6.41
M-K	-3358 / 0	0.0	8.36

DESIGN CRITERIA
*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17)
CALCULATED VERT. DEFL.(LL) = L/999 (0.23)
ALLOWABLE DEFL.(TL) = L/360 (1.17)
CALCULATED VERT. DEFL.(TL) = L/984 (0.44)

CSI: TC=0.71/1.00 (D-E-1), BC=0.56/1.00 (Q-R-1), WB=0.44/1.00 (C-T-1), SS=0.23/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

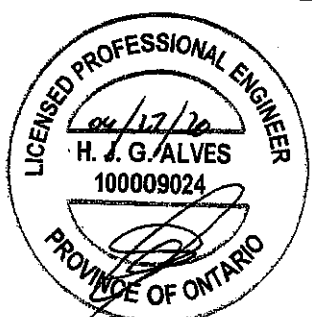
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (G) (INPUT = 0.90)
JSI METAL= 0.59 (S) (INPUT = 1.00)



Structural component only
DWG# T-2007145 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408169	T20	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	1.50	3.50
C TTWW-m	MT20	6.0	9.0	Edge	6.25
D TMWW-l	MT20	4.0	6.0		
E TMW-w	MT20	2.0	4.0		
F TMWW-l	MT20	4.0	4.0		
G TS-l	MT20	3.0	6.0		
H TMW-w	MT20	2.0	4.0		
I TMWW-l	MT20	4.0	6.0		
J TTWW-m	MT20	6.0	9.0	Edge	6.25
K TMVW-p	MT20	5.0	8.0	1.50	3.50
M BMV1-p	MT20	3.0	6.0		
N BMWW-l	MT20	5.0	6.0	2.50	2.00
O BMWW-l	MT20	6.0	9.0	4.50	2.00
P BS-l	MT20	5.0	8.0		
Q BMWW-l	MT20	5.0	8.0		
R BMWW-l	MT20	5.0	8.0		
S BS-l	MT20	5.0	6.0		
T BMWW-l	MT20	6.0	9.0	4.50	2.00
U BMWW-l	MT20	5.0	5.0	2.50	2.00
V BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		LENGTH FR-TO			
AS-AT	0 / 6217	-18.5	-18.5	0.48 (1)	10.00		
AT-O	0 / 6217	-18.5	-18.5	0.48 (1)	10.00		
O-AU	0 / 3326	-18.5	-18.5	0.26 (1)	10.00		
AU-AV	0 / 3326	-18.5	-18.5	0.26 (1)	10.00		
AV-N	0 / 3326	-18.5	-18.5	0.26 (1)	10.00		
N-AW	0 / 0	-18.5	-18.5	0.04 (4)	10.00		
AW-M	0 / 0	-18.5	-18.5	0.04 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-49	-55		FRONT	VERT	DEAD		C1
C	4-0-11	-254	-254		FRONT	VERT	SNOW		C1
G	19-2-12	-110	-110		BACK	VERT	TOTAL		C1
J	31-1-6	-49	-55		FRONT	VERT	DEAD		C1
J	31-1-6	-117	-117		BACK	VERT	TOTAL		C1
N	31-1-6	-254	-254		FRONT	VERT	SNOW		C1
P	31-2-12	-26	-26		BACK	VERT	TOTAL		C1
W	5-2-12	-26	-26		BACK	VERT	TOTAL		C1
X	5-2-12	-110	-110		BACK	VERT	TOTAL		C1
Y	7-2-12	-110	-110		BACK	VERT	TOTAL		C1
Y	9-2-12	-110	-110		BACK	VERT	TOTAL		C1
Z	11-2-12	-110	-110		BACK	VERT	TOTAL		C1
AA	13-2-12	-110	-110		BACK	VERT	TOTAL		C1
AB	15-2-12	-110	-110		BACK	VERT	TOTAL		C1
AC	17-2-12	-110	-110		BACK	VERT	TOTAL		C1
AD	21-2-12	-110	-110		BACK	VERT	TOTAL		C1
AE	23-2-12	-110	-110		BACK	VERT	TOTAL		C1
AF	25-2-12	-110	-110		BACK	VERT	TOTAL		C1
AG	27-2-12	-110	-110		BACK	VERT	TOTAL		C1
AH	29-2-12	-110	-110		BACK	VERT	TOTAL		C1
AI	1-2-12	-26	-26		BACK	VERT	TOTAL		C1
AJ	3-2-12	-26	-26		BACK	VERT	TOTAL		C1
AK	5-2-12	-26	-26		BACK	VERT	TOTAL		C1
AL	7-2-12	-26	-26		BACK	VERT	TOTAL		C1
AM	9-2-12	-26	-26		BACK	VERT	TOTAL		C1
AN	11-2-12	-26	-26		BACK	VERT	TOTAL		C1
AO	13-2-12	-26	-26		BACK	VERT	TOTAL		C1
AP	15-2-12	-26	-26		BACK	VERT	TOTAL		C1
AQ	17-2-12	-26	-26		BACK	VERT	TOTAL		C1
AR	19-2-12	-26	-26		BACK	VERT	TOTAL		C1
AS	23-2-12	-26	-26		BACK	VERT	TOTAL		C1
AT	25-2-12	-26	-26		BACK	VERT	TOTAL		C1
AU	27-2-12	-26	-26		BACK	VERT	TOTAL		C1
AV	29-2-12	-26	-26		BACK	VERT	TOTAL		C1
AW	33-2-12	-26	-26		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007145

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	T20Z	1	2	GREEN PARK HOMES	

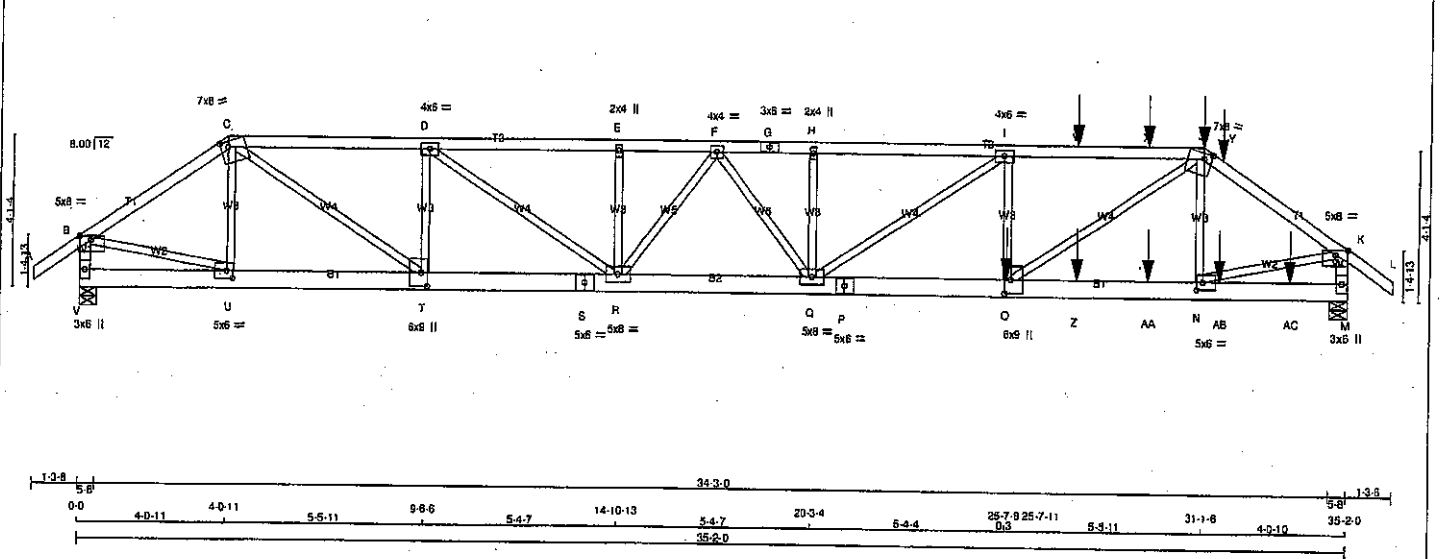
Tamarack Roof Truss, Burlington

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1.38 0.0 4.0-11 4.0-11 5.5-11 9.6-6 5.4-7 14.10-13 5.4-7 20.3-4 5.4-7 25.7-11 27.6-12 29.6-12 31.1-8 31.6-12 35.2-0 35.2-0 35.2-0

Scale = 1:57.3



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD	*** SPECIAL LOADS ANALYSIS ***	
A - C	2x4	DRY	No.2	SPF	JOINT	GROSS REACTION	GROSS REACTION	BRG	BRG	GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.	
C - G	2x4	DRY	No.2	SPF	JOINT	VERT	DOWN	UPLIFT	IN-SX	LOADS WERE DERIVED FROM USER INPUT	
G - J	2x4	DRY	No.2	SPF	JOINT	VERT	DOWN	UPLIFT	IN-SX	NO FURTHER MODIFICATIONS WERE MADE	
J - L	2x4	DRY	No.2	SPF	JOINT	VERT	DOWN	UPLIFT	IN-SX	SPECIFIED LOADS:	
V - B	2x4	DRY	No.2	SPF	JOINT	VERT	DOWN	UPLIFT	IN-SX	TOP CH. LL = 25.6 PSF	
M - K	2x4	DRY	No.2	SPF	JOINT	VERT	DOWN	UPLIFT	IN-SX	DL = 6.0 PSF	
V - S	2x6	DRY	No.2	SPF	JOINT	VERT	DOWN	UPLIFT	IN-SX	BOT CH. LL = 0.0 PSF	
S - P	2x6	DRY	No.2	SPF	JOINT	VERT	DOWN	UPLIFT	IN-SX	DL = 7.4 PSF	
P - M	2x6	DRY	No.2	SPF	JOINT	VERT	DOWN	UPLIFT	IN-SX	TOTAL LOAD = 35.0 PSF	
ALL WEBS	2x3	DRY	No.2	SPF	JOINT	VERT	DOWN	UPLIFT	IN-SX	SPACING = 24.0 IN. C/C	
EXCEPT					JOINT	VERT	DOWN	UPLIFT	IN-SX	LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF)

TOP CHORDS : (0.122"x3") SPIRAL NAILS

A-C 1 12 TOP

C-G 1 12 TOP

G-J 1 12 SIDE(81.0)

J-L 1 12 SIDE(81.0)

V-B 1 12 TOP

M-K 1 12 TOP

BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS

V-S 2 12 TOP

S-P 2 12 TOP

P-M 2 12 SIDE(183.1)

WEBS : (0.122"x3") SPIRAL NAILS

2x3 1 6

I-O 1 3

D-T 1 3

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.57 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FORCE (LBS)

MAX. FACTORED

FACTORED

VERT. LOAD LCI (PLF)

MAX. CSI (LC)

MAX. UNBRAC

WEBS

MEMB. FORCE (LBS)

MAX. FACTORED

MAX. CSI (LC)

FR-TO

A-B 0 / 35

B-C -3442 / 0

C-D -5905 / 0

D-E -7947 / 0

E-F -7947 / 0

F-G -9222 / 0

G-H -9222 / 0

H-I -9222 / 0

I-J -9222 / 0

J-K -9222 / 0

K-L -9222 / 0

L-M -9222 / 0

M-N -9222 / 0

N-O -9222 / 0

O-P -9222 / 0

P-Q -9222 / 0

Q-R -9222 / 0

R-S -9222 / 0

S-T -9222 / 0

T-U -9222 / 0

U-V -9222 / 0

V-W -9222 / 0

W-X -9222 / 0

X-Y -9222 / 0

Y-Z -9222 / 0

Z-AA -9222 / 0

AA-N -9222 / 0

N-AB -9222 / 0

AB-AC -9222 / 0

AC-M -9222 / 0

<

JOB NAME 408169	TRUSS NAME T20Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 12:49:25 2020 Page 2 ID:K?TPdhi0npl1clYdbWfOFzdeG-AKHFWtPo9K cKTp9gXCi67DKWF2Jk9bCKqW7zNBDe	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TTWW-m	MT20	7.0	8.0	1.75	2.50
D	TMWW-t	MT20	4.0	6.0		
E	TMW-w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW-w	MT20	2.0	4.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW-m	MT20	7.0	8.0	1.75	2.50
K	TMVW-p	MT20	5.0	8.0	1.50	3.50
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	8.0	2.50	2.00
O	BMWW-t	MT20	6.0	9.0	4.50	2.00
P	BS-t	MT20	5.0	6.0		
Q	BMWWWW-t	MT20	5.0	8.0		
R	BMWWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	6.0	9.0	4.50	2.00
U	BMWW-t	MT20	5.0	6.0	2.50	2.00
V	BMV1+p	MT20	3.0	6.0		

CONNECTION REQUIREMENTS

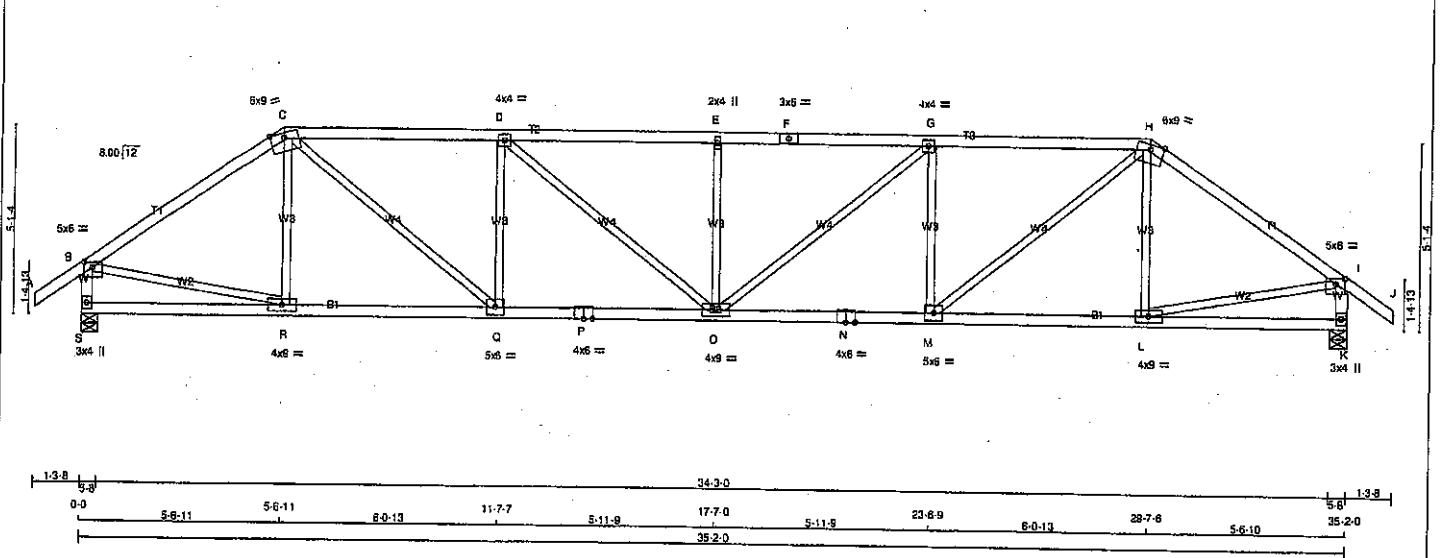
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007146 42

JOB NAME 408169	TRUSS NAME T21	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:57.3



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
S - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
S - P	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TTWW-m	MT20	6.0	9.0	Edge	
D	TMVW-t	MT20	4.0	4.0		
E	TMVW-w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	9.0	Edge	
I	TMVW-p	MT20	5.0	6.0	1.75	2.75
K	BMV1-p	MT20	3.0	4.0		
L	BMVW-t	MT20	4.0	9.0		
M	BMVW-t	MT20	5.0	5.0		
N	BS-t	MT20	4.0	6.0		
O	BMVWV-t	MT20	4.0	9.0		
P	BS-t	MT20	4.0	6.0		
Q	BMVW-t	MT20	5.0	5.0		
R	BMVW-t	MT20	4.0	9.0		
S	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
S	2065	0	2065	0	5-8
K	2065	0	2065	0	5-8

UNFACTORED REACTIONS

JT	1ST LOSE	MAX/MIN	COMPONENT REACTIONS	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0	0 / 0
K	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	LENGTH FR-TO
A-B	0 / 35	-91.8 -91.8	0.12 (1)
B-C	-2297 / 0	-91.8 -91.8	0.70 (1)
C-D	-3213 / 0	-91.8 -91.8	0.80 (1)
D-E	-3594 / 0	-91.8 -91.8	0.86 (1)
E-F	-3594 / 0	-91.8 -91.8	0.88 (1)
F-G	-3594 / 0	-91.8 -91.8	0.86 (1)
G-H	-3213 / 0	-91.8 -91.8	0.80 (1)
H-I	-2298 / 0	-91.8 -91.8	0.70 (1)
I-J	0 / 35	-91.8 -91.8	0.12 (1)
J-K	-2023 / 0	0.0 0.0	0.21 (1)
S-R	0 / 0	-18.5 -18.5	0.15 (4)
R-Q	0 / 1904	-18.5 -18.5	0.39 (1)
Q-P	0 / 3213	-18.5 -18.5	0.58 (1)
P-O	0 / 3213	-18.5 -18.5	0.58 (1)
O-N	0 / 3213	-18.5 -18.5	0.58 (1)
N-M	0 / 3213	-18.5 -18.5	0.58 (1)
M-L	0 / 1903	-18.5 -18.5	0.39 (1)
L-K	0 / 0	-18.5 -18.5	0.15 (4)

TOTAL WEIGHT = 2 X 140 = 280 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/998 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/969 (0.39")

CSK TC=0.89/1.00 (E-G:1), BC=0.58/1.00 (O-Q:1),
WB=0.44/1.00 (B-R:1), SSI=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	
MT20	618	354	1667
			788
			1957
			1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.87 (B) (INPUT = 0.90)
JSI METAL=0.74 (P) (INPUT = 1.00)



Structural component only
DWG# T-2007147

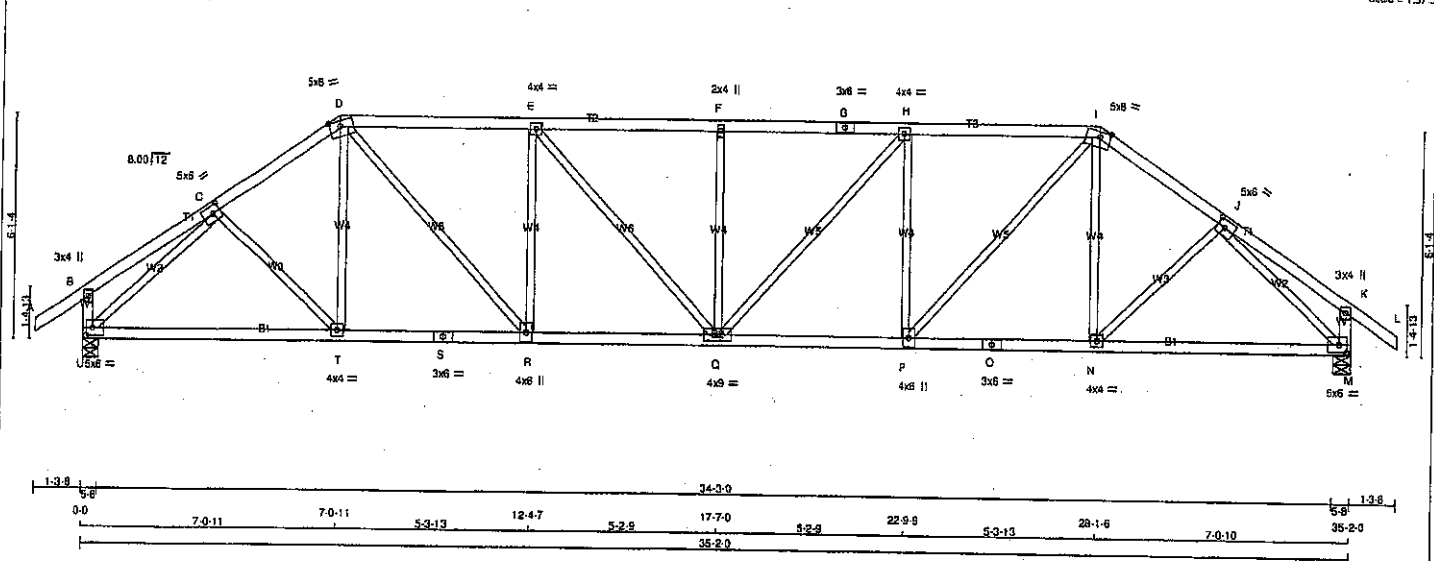
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	T22	2	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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Scale = 1:57.3



LUMBER

N.L.G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - G	2x4	DRY No.2
G - I	2x4	DRY No.2
I - L	2x4	DRY No.2
U - B	2x4	DRY No.2
M - K	2x4	DRY No.2
U - S	2x4	DRY No.2
S - O	2x4	DRY No.2
O - M	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY, SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
D	TMWW-t	MT20	5.0	6.0	2.50	2.50
C	TTWW-m	MT20	5.0	8.0	1.75	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	8.0	1.75	3.50
J	TMWW-t	MT20	5.0	6.0	2.50	2.50
K	TMV+p	MT20	3.0	4.0		
M	BMVW-t	MT20	5.0	8.0	2.50	2.25
N	BMVW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	8.0		
Q	BMVW-t	MT20	4.0	8.0		
R	BMVW-t	MT20	4.0	8.0		
S	BS-t	MT20	3.0	6.0		
T	BMVW-t	MT20	4.0	4.0		
U	BMVW-t	MT20	5.0	6.0	2.50	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
U	2065	0	5-8	5-8
M	2065	0	5-8	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN COMPONENT REACTIONS	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM.LIVE
U	1458	970/0	0/0	0/0
M	1458	970/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	UNBRAC LENGTH
FR-TO				FR-TO			
A-B	0/35	-91.8	-91.8 0.12 (1)	10.00	C-T	0/135	0.03 (4)
B-C	0/18	-91.8	-91.8 0.16 (1)	10.00	T-D	0/90	0.03 (4)
C-D	-2305/0	-91.8	-91.8 0.23 (1)	4.31	D-F	0/1218	0.27 (1)
D-E	-2723/0	-91.8	-91.8 0.54 (1)	3.69	R-E	-814/0	0.48 (1)
E-F	-2966/0	-91.8	-91.8 0.56 (1)	3.53	E-Q	0/363	0.08 (1)
F-G	-2966/0	-91.8	-91.8 0.56 (1)	3.53	Q-F	-440/0	0.26 (1)
G-H	-2966/0	-91.8	-91.8 0.58 (1)	3.53	Q-H	0/363	0.08 (1)
H-I	-2723/0	-91.8	-91.8 0.54 (1)	3.69	P-H	-814/0	0.48 (1)
I-J	-2305/0	-91.8	-91.8 0.23 (1)	4.31	P-I	0/1218	0.27 (1)
J-K	0/18	-91.8	-91.8 0.16 (1)	10.00	N-I	0/90	0.03 (4)
K-L	0/35	-91.8	-91.8 0.12 (1)	10.00	N-J	0/135	0.03 (4)
U-B	-255/0	0.0	0.0 0.03 (1)	7.81	U-C	-2515/0	0.97 (1)
M-K	-255/0	0.0	0.0 0.03 (1)	7.81	J-M	-2514/0	0.97 (1)
U-T	0/1802	-18.5	-18.5 0.40 (1)	10.00			
T-S	0/1901	-18.5	-18.5 0.41 (1)	10.00			
S-R	0/1901	-18.5	-18.5 0.41 (1)	10.00			
R-Q	0/2724	-18.5	-18.5 0.49 (1)	10.00			
Q-P	0/2723	-18.5	-18.5 0.49 (1)	10.00			
P-O	0/1900	-18.5	-18.5 0.41 (1)	10.00			
O-N	0/1900	-18.5	-18.5 0.41 (1)	10.00			
N-M	0/1801	-18.5	-18.5 0.40 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")

CALCULATED VERT. DEFL.(LL) = L/999 (0.16")

ALLOWABLE DEFL.(TL) = L/360 (1.17")

CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CSI: TC=0.58/1.00 (E-F:1), BC=0.48/1.00 (Q-R:1), WB=0.97/1.00 (C-U:1), SSI=0.23/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

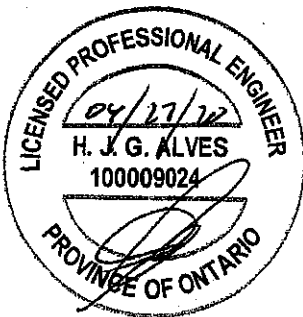
MT20 618 354 1667 768 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

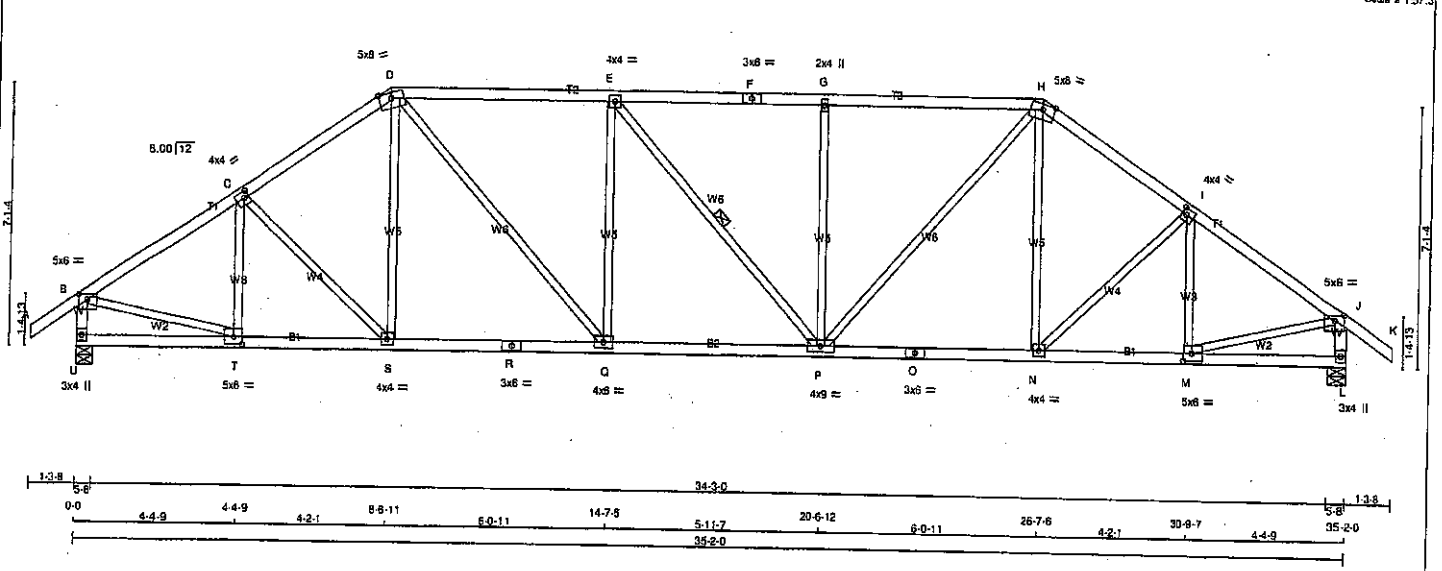
JSI GRIP=0.90 (M) (INPUT = 0.90)

JSI METAL=0.63 (C) (INPUT = 1.00)



Structural component only

DWG# T-2007148



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
U - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
U - R	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TMVW-m	MT20	4.0	4.0	2.00	1.50
D	TTWV-m	MT20	5.0	8.0	Edge	
E	TMVW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-w	MT20	2.0	4.0		
H	TTWV-m	MT20	5.0	6.0	Edge	
I	TMVW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	1.75	2.75
L	BMV1-p	MT20	3.0	4.0		
M	BMVW-t	MT20	5.0	6.0	2.50	2.75
N	BMVW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMVW-t	MT20	4.0	4.0		
S	BMVW-t	MT20	5.0	6.0	2.50	2.75
T	BMVW-t	MT20	5.0	6.0		
U	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	IN-SX	IN-SX
U	HORZ	UPLIFT	IN-SX	IN-SX
L				

UNFACTORED REACTIONS

	1ST CASE	MAX MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE			
U						
L						

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED		MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
		VERT. LOAD	LC1					
FR-TO						FR-TO		
A-B	0/35	-91.8	-91.8	0.12 (1)	10.00	T-C	-396/0	0.11 (1)
B-C	-2270/0	-91.8	-91.8	0.37 (1)	4.19	C-S	-79/0	0.05 (1)
C-D	-2254/0	-91.8	-91.8	0.38 (1)	4.21	S-D	0/156	0.04 (4)
D-E	-2485/0	-91.8	-91.8	0.84 (1)	3.72	D-Q	0/949	0.21 (1)
E-F	-2483/0	-91.8	-91.8	0.84 (1)	3.72	Q-E	-587/0	0.52 (1)
F-G	-2483/0	-91.8	-91.8	0.84 (1)	3.72	E-P	-2/0	0.00 (1)
G-H	-2483/0	-91.8	-91.8	0.83 (1)	3.74	P-G	-596/0	0.52 (1)
H-I	-2254/0	-91.8	-91.8	0.38 (1)	4.21	P-H	0/947	0.21 (1)
I-J	-2258/0	-91.8	-91.8	0.37 (1)	4.19	H-N	0/157	0.04 (4)
J-K	0/35	-91.8	-91.8	0.12 (1)	10.00	N-I	-78/0	0.05 (1)
U-B	-2027/0	0.0	0.0	0.21 (1)	5.94	M-I	-397/0	0.11 (1)
L-J	-2028/0	0.0	0.0	0.21 (1)	5.94	B-T	0/1965	0.44 (1)
						M-J	0/1965	0.44 (1)
U-T	0/0	-18.5	-18.5	0.08 (4)	10.00			
T-S	0/1909	-18.5	-18.5	0.37 (1)	10.00			
S-R	0/1854	-18.5	-18.5	0.38 (1)	10.00			
R-Q	0/1854	-18.5	-18.5	0.38 (1)	10.00			
Q-P	0/2485	-18.5	-18.5	0.47 (1)	10.00			
P-O	0/1854	-18.5	-18.5	0.38 (1)	10.00			
O-N	0/1854	-18.5	-18.5	0.38 (1)	10.00			
N-M	0/1908	-18.5	-18.5	0.37 (1)	10.00			
M-L	0/0	-18.5	-18.5	0.08 (4)	10.00			

TOTAL WEIGHT = 2 X 155 = 309 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.8	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD = 39.0 PSF			

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")

CALCULATED VERT. DEFL.(LL) = L/999 (0.12")

ALLOWABLE DEFL.(TL) = L/360 (1.17")

CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.64/1.00 (D-E:1), BC=0.47/1.00 (P-Q:1), WB=0.52/1.00 (E-Q:1), SS=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

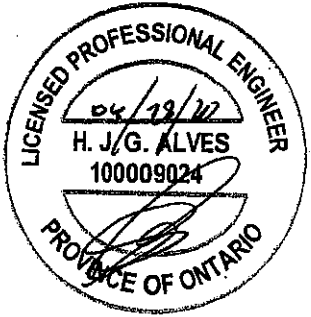
(PSI)	(PL)	(PL)
MAX	MIN	MAX
818	354	1667
818	354	1667

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)

JSI METAL= 0.62 (D) (INPUT = 1.00)



Structural component only
DWG# T-2007149

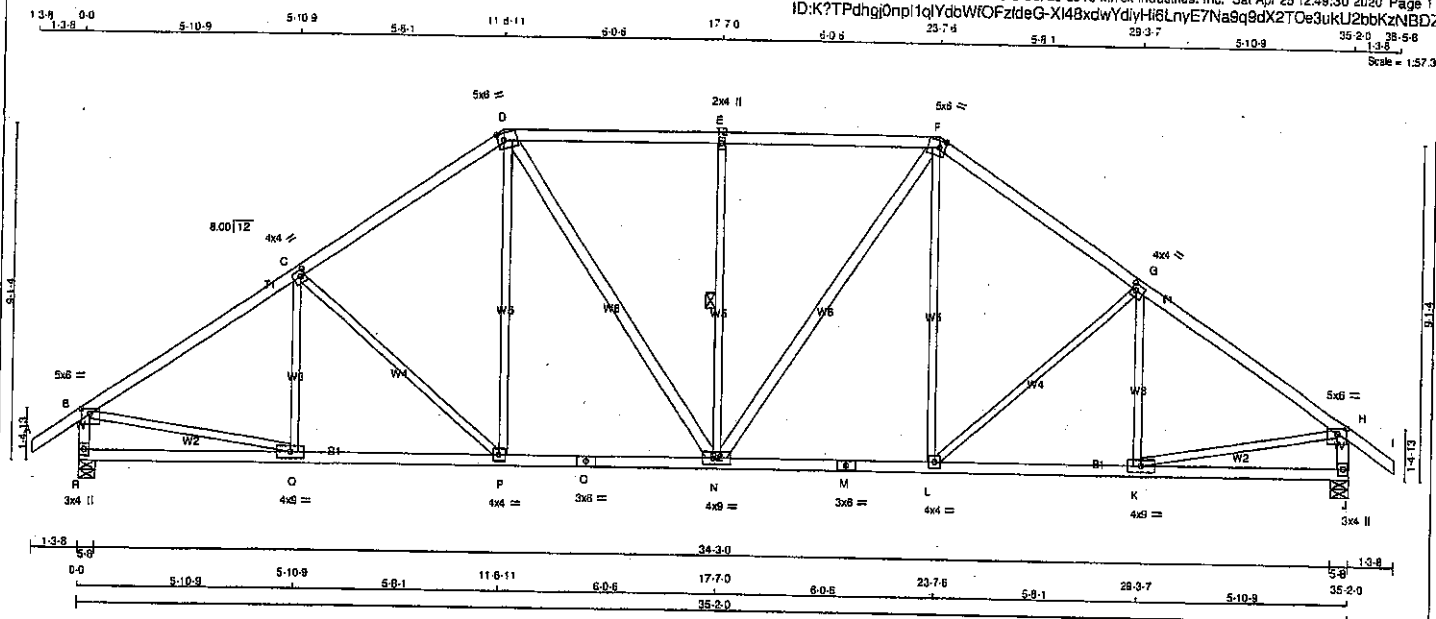
Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Sat Apr 25 12:49:29 2020 Page 1
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7-3-6	25.1-6	4.11.1	30-0.7	5-1.9	35.2-0	26-5-6	1.3-6
							Scale = 1:57.3



JOB NAME 408169	TRUSS NAME T25	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 Scale = 1:57.3



LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TMVW-l	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-l	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.75	2.75
J	BMVW-p	MT20	3.0	4.0		
K	BMVW-l	MT20	4.0	9.0		
L	BMVW-l	MT20	4.0	4.0		
M	BS-l	MT20	3.0	6.0		
N	BMVWW-l	MT20	4.0	9.0		
O	BS-l	MT20	3.0	6.0		
P	BMVW-l	MT20	4.0	4.0		
Q	BMVW-l	MT20	4.0	9.0		
R	BMVW-l	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
R	2065	0	2065	0	5-8
J	2065	0	2065	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN. COMPONENT REACTIONS	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
R	1458	970 / 0	0 / 0	0 / 0	488 / 0
J	1458	970 / 0	0 / 0	0 / 0	488 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF EN.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FACTORED	MEMB.	FACTORED
FR-TO	FORCE (LBS)	FR-TO	FORCE (LBS)
A-B	0 / 35	Q-C	-248 / 11
B-C	-2347 / 0	C-P	-356 / 0
C-D	-2096 / 0	P-D	0 / 342
D-E	-1985 / 0	D-N	0 / 477
E-F	-1985 / 0	N-E	-677 / 0
F-G	-2096 / 0	N-F	0 / 477
G-H	-2346 / 0	L-F	0 / 341
H-I	0 / 35	L-G	-356 / 0
R-B	-2018 / 0	K-G	-249 / 11
J-H	-2018 / 0	B-Q	0 / 2014
		K-H	0 / 2014
R-Q	0 / 0		
Q-P	0 / 1981		
P-O	0 / 1718		
O-N	0 / 1718		
N-M	0 / 1716		
M-L	0 / 1716		
L-K	0 / 1980		
K-J	0 / 0		

TOTAL WEIGHT = 2 X 165 = 329 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 8 OF BCBC 2018, OBC 2012, A6C 2019
 - PART 8 OF OBC 2012 (2019 AMENDMENT)
 - CSA 088-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.10")
 ALLOWABLE DEFL. (TL) = L/360 (1.17")
 CALCULATED VERT. DEFL. (TL) = L/999 (0.10")

CSI: TC=0.52/1.00 (B-C:1), BC=0.39/1.00 (P-Q:1), WB=0.45/1.00 (B-Q:1), SS=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

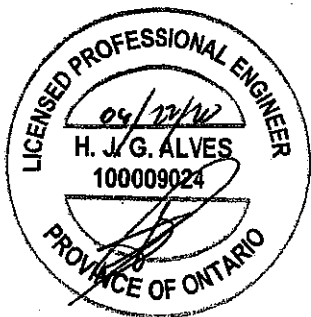
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1867 788 1987 1865

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (B) (INPUT = 0.90)
 JSI METAL = 0.58 (M) (INPUT = 1.00)



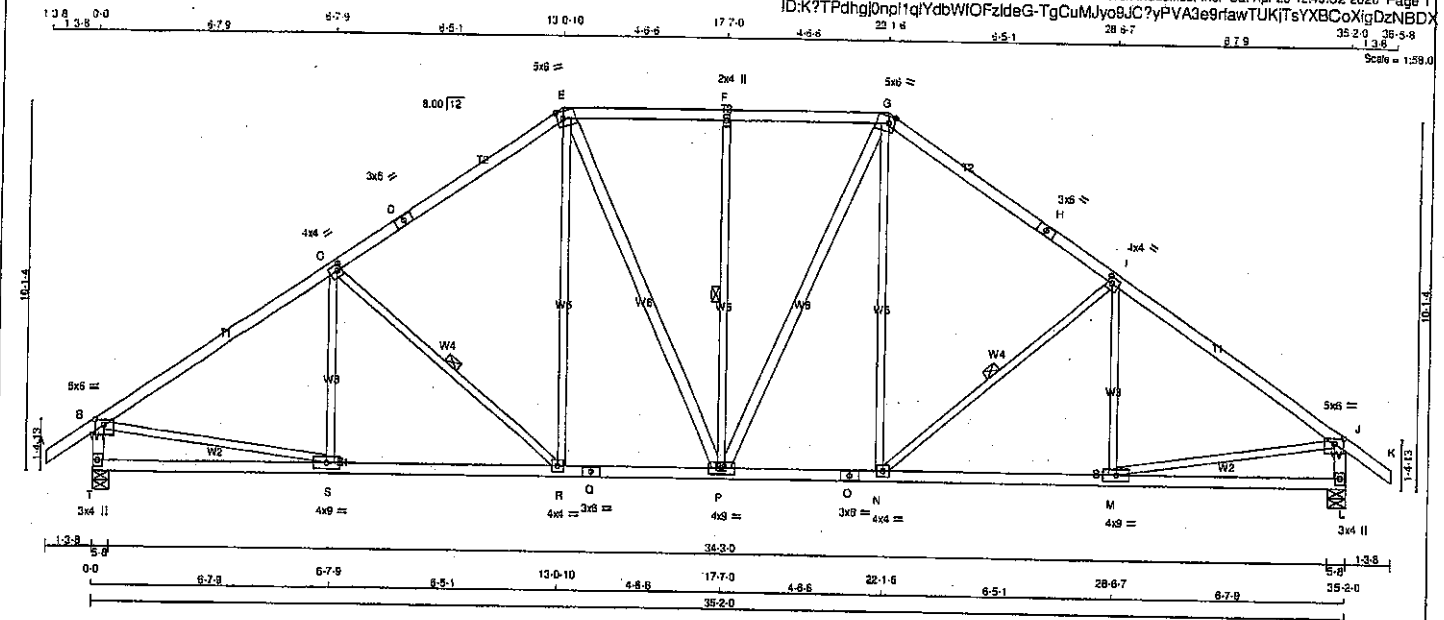
Structural component only
 DWG# T-2007151

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408169	T26	5	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:K?TPdhgJ0np1qYdbWIOFzIdG-TgCuMjyo8JC?yPVA3e9riawTUKITsYXBCoXigDzNBDX



TOTAL WEIGHT = 5 X 172 = 858 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES					
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
G - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
T - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
T - Q	2x4	DRY	No.2	SPF	
Q - O	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
E - P	2x4	DRY	No.2	SPF	
P - G	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMVW-w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TS-t	MT20	3.0	6.0		
I	TMVW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	6.0	1.75	2.75
L	BMV1-p	MT20	3.0	4.0		
M	BMVW-t	MT20	4.0	9.0		
N	BMVW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		
R	BMVW-t	MT20	4.0	4.0		
S	BMVW-t	MT20	4.0	9.0		
T	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORZ	UPLIFT
T	2085	0	0	5-8
L	2085	0	0	5-8

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
T	1458	970 / 0
L	1458	970 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-R, F-P, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (LC1)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (LC1)
FR-TO						FR-TO				
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	S-C	-189 / 46	0.10 (1)		
B-C	-2359 / 0	-91.8	-91.8	0.69 (1)	3.77	C-R	-485 / 0	0.24 (1)		
C-D	-1996 / 0	-91.8	-91.8	0.62 (1)	4.11	R-E	0 / 416	0.08 (1)		
D-E	-1996 / 0	-91.8	-91.8	0.62 (1)	4.11	E-P	0 / 315	0.05 (1)		
E-F	-1761 / 0	-91.8	-91.8	0.27 (1)	4.77	P-F	-502 / 0	0.35 (1)		
F-G	-1761 / 0	-91.8	-91.8	0.27 (1)	4.77	G-P	0 / 315	0.05 (1)		
G-H	-1996 / 0	-91.8	-91.8	0.62 (1)	4.11	N-G	0 / 415	0.09 (1)		
H-I	-1996 / 0	-91.8	-91.8	0.62 (1)	4.11	I-N	-484 / 0	0.24 (1)		
I-J	-2359 / 0	-91.8	-91.8	0.69 (1)	3.77	M-I	-189 / 46	0.10 (1)		
J-K	0 / 35	-91.8	-91.8	0.12 (1)	10.00	B-S	0 / 2022	0.45 (1)		
T-B	-2014 / 0	0.0	0.0	0.21 (1)	5.95	M-J	0 / 2021	0.45 (1)		
L-J	-2014 / 0	0.0	0.0	0.21 (1)	5.95					
T-S	0 / 0	-18.5	-18.5	0.19 (4)	10.00					
S-R	0 / 1996	-18.5	-18.5	0.42 (1)	10.00					
R-Q	0 / 1629	-18.5	-18.5	0.32 (1)	10.00					
Q-P	0 / 1629	-18.5	-18.5	0.32 (1)	10.00					
P-O	0 / 1629	-18.5	-18.5	0.32 (1)	10.00					
O-N	0 / 1629	-18.5	-18.5	0.32 (1)	10.00					
N-M	0 / 1995	-18.5	-18.5	0.42 (1)	10.00					
M-L	0 / 0	-18.5	-18.5	0.19 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = L/360 (1.17")
CALCULATED VERT. DEFL (LL) = L/999 (0.09")
ALLOWABLE DEFL (TL) = L/360 (1.17")
CALCULATED VERT. DEFL (TL) = L/999 (0.18")

CSI: TC=0.69/1.00 (B-C:1), BC=0.42/1.00 (R-S:1), WS=0.45/1.00 (B-S:1), SS=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

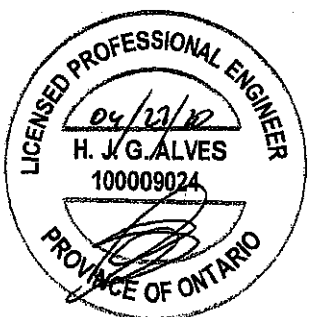
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

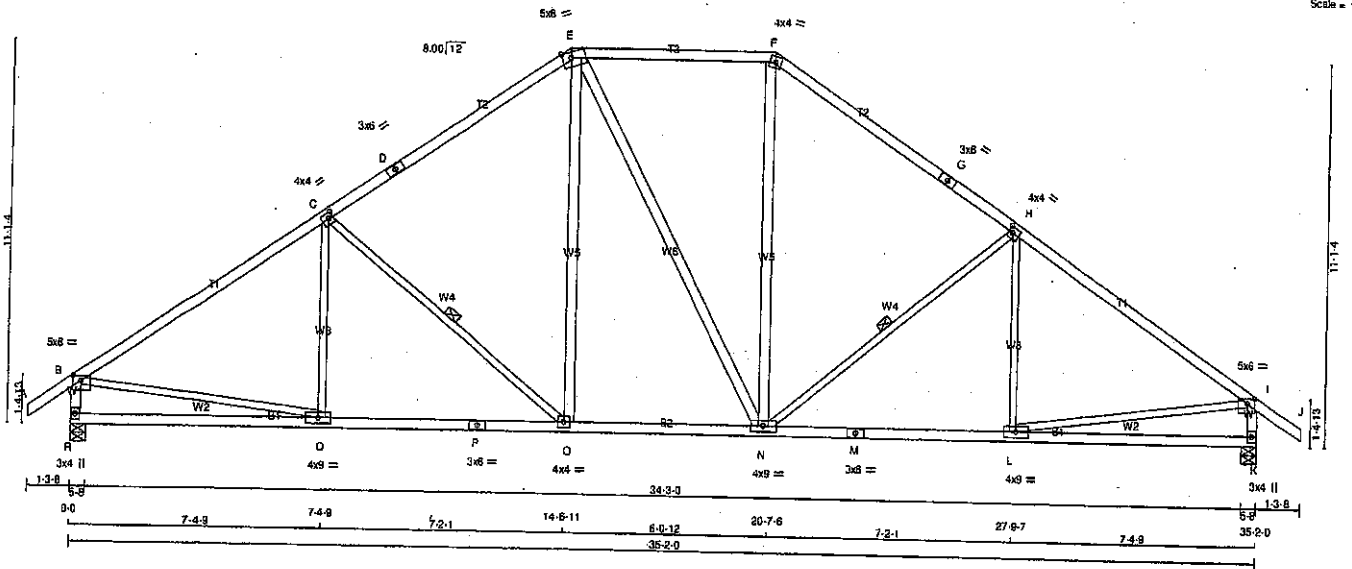
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.50)
JSI METAL= 0.50 (D) (INPUT = 1.00)



Structural component only
DWG# T-2007152

JOB NAME 408169	TRUSS NAME T27	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S.Oct 28 2019 Mitek Industries, Inc. Sat Apr 25 12:49:33 2020 Page 1	
ID:K?TPdngi0npl1qlydbWIOFzldG-xlmGZfzRwdKfZZ4MdMg4C0Sb3k2Cb?nKQSHGcizNBOW				Scale = 1/2" = 1'-0"	



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No. 2
D - E	2x4	DRY	No. 2
E - F	2x4	DRY	No. 2
F - G	2x4	DRY	No. 2
G - J	2x4	DRY	No. 2
R - B	2x4	DRY	No. 2
K - I	2x4	DRY	No. 2
R - P	2x4	DRY	No. 2
P - M	2x4	DRY	No. 2
M - K	2x4	DRY	No. 2

ALL WEBS	2x3	DRY	No. 2
EXCEPT			
O - E	2x4	DRY	No. 2
E - N	2x4	DRY	No. 2
N - F	2x4	DRY	No. 2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	2.75
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWV-m	MT20	5.0	8.0	2.00	3.00
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-p	MT20	5.0	8.0	1.75	2.75
K	BMV1-p	MT20	3.0	4.0		
L	BMVW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMVWV-t	MT20	4.0	9.0		
O	BMVW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	6.0		
Q	BMVW-t	MT20	4.0	9.0		
R	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
R	2065	0	2065	0	5-8
K	2065	0	2065	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0	0 / 0
K	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 3.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 CSI (LC)	MAX. LEBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	Q-C	-139 / 76	0.09 (1)
B-C	-2359 / 0	-91.8	-91.8 0.89 (1)	3.45	C-O	-805 / 0	0.38 (1)
C-D	-1898 / 0	-91.8	-91.8 0.78 (1)	3.91	O-E	0 / 499	0.08 (1)
D-E	-1898 / 0	-91.8	-91.8 0.78 (1)	3.91	E-N	0 / 0	0.00 (1)
E-F	-1538 / 0	-91.8	-91.8 0.47 (1)	4.74	N-F	0 / 500	0.08 (1)
F-G	-1898 / 0	-91.8	-91.8 0.78 (1)	3.91	H-H	-604 / 0	0.38 (1)
G-H	-1898 / 0	-91.8	-91.8 0.78 (1)	3.91	L-H	-140 / 75	0.09 (1)
H-I	-2357 / 0	-91.8	-91.8 0.89 (1)	3.45	B-Q	0 / 2020	0.45 (1)
I-J	0 / 35	-91.8	-91.8 0.12 (1)	10.00	L-I	0 / 2019	0.45 (1)
R-B	-2009 / 0	0.0	0.0 0.21 (1)	5.95			
K-I	-2009 / 0	0.0	0.0 0.21 (1)	5.95			
R-Q	0 / 0	-18.5	-18.5 0.25 (4)	10.00			
Q-P	0 / 1999	-18.5	-18.5 0.45 (1)	10.00			
P-O	0 / 1999	-18.5	-18.5 0.45 (1)	10.00			
O-N	0 / 1538	-18.5	-18.5 0.33 (1)	10.00			
N-M	0 / 1999	-18.5	-18.5 0.45 (1)	10.00			
M-L	0 / 1998	-18.5	-18.5 0.45 (1)	10.00			
L-K	0 / 0	-18.5	-18.5 0.24 (4)	10.00			

TOTAL WEIGHT = 2 X 166 = 333 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17')
CALCULATED VERT. DEFL. (LL) = L/999 (0.09')
ALLOWABLE DEFL. (TL) = L/360 (1.17')
CALCULATED VERT. DEFL. (TL) = L/999 (0.20')

CSI: TC=0.89/1.00 (B-C:1), BC=0.45/1.00 (O-Q:1),
WB=0.45/1.00 (B-Q:1), SSI=0.27/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

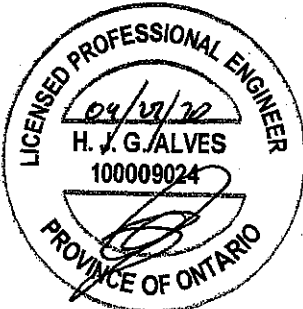
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354
	1867	788
	1867	1867

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (F) (INPUT = 0.90)
JSI METAL=0.68 (F) (INPUT = 1.00)



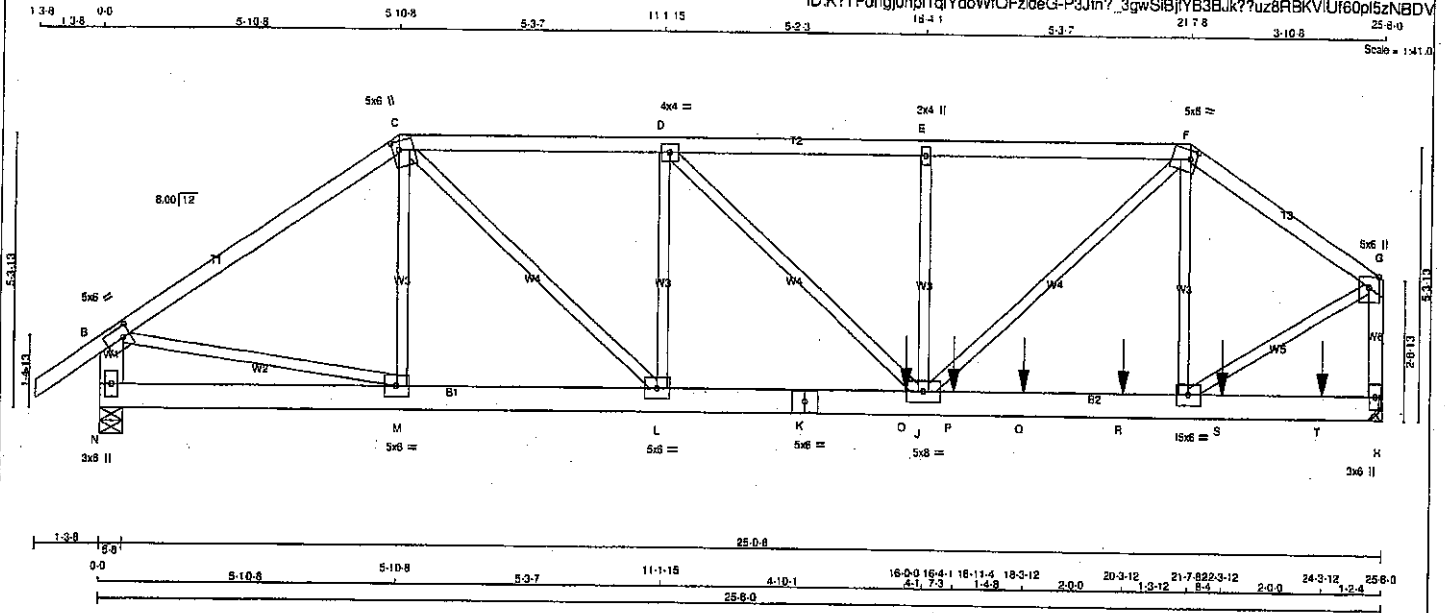
Structural component only
DWG# T-2007153

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408169	T28	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:K?TPdngj0npl1qYdbWfCFzideG-P3Jfn?_3gwSiBjYB3Bjk?uz8RBKVIU60pISzNBDV



LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES	A - C	2x4	DRY	No.2	SPF
	C - F	2x4	DRY	No.2	SPF
	F - G	2x4	DRY	No.2	SPF
	N - B	2x6	DRY	No.2	SPF
	H - G	2x4	DRY	No.2	SPF
	N - K	2x6	DRY	No.2	SPF
	K - H	2x6	DRY	No.2	SPF
	ALL WEBS	2x3	DRY	No.2	SPF
	EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
C-F	12	TOP
F-G	12	TOP
H-G	12	TOP
N-B	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
N-K	12	TOP
K-H	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
E-J	6	SIDE(183.1)
2x3	6	SIDE(77.0)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
N	2088	0	0	5-8
H	2758	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED
N	1470
H	1941

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.76 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	MEMB.	FORCE
	(LBS)	(PLF)		(LBS)
FR-TO	FROM	TO	FR-TO	FROM
A-B	0/35	-91.8	M-C	-218/22
B-C	-2363/0	-91.8	C-L	0/1550
C-D	-3093/0	-91.8	L-D	-928/0
D-E	-3519/0	-91.8	D-J	0/589
E-F	-3519/0	-91.8	J-E	-508/0
F-G	-2352/0	-91.8	J-F	0/2156
N-B	-2037/0	0.0	I-F	-548/0
H-G	-2632/0	0.0	B-M	0/1991
			I-G	0/2264
N-M	0/0	-18.5		
M-L	0/1959	-18.5		
L-K	0/3093	-18.5		
K-O	0/3093	-18.5		
O-J	0/3093	-18.5		
J-P	0/1942	-18.5		
P-Q	0/1942	-18.5		
Q-R	0/1942	-18.5		
R-I	0/1942	-18.5		
I-S	0/0	-18.5		
S-T	0/0	-18.5		
T-H	0/0	-18.5		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	16-0-0	-997	-997	-	FRONT	VERT	TOTAL	-	C1
P	16-11-4	-187	-187	-	FRONT	VERT	TOTAL	-	C1
Q	18-3-12	-187	-187	-	FRONT	VERT	TOTAL	-	C1
R	20-3-12	-187	-187	-	FRONT	VERT	TOTAL	-	C1
S	22-3-12	-174	-174	-	FRONT	VERT	TOTAL	-	C1
T	24-3-12	-173	-173	-	FRONT	VERT	TOTAL	-	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 122 = 245 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 35.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/989 (0.11")

CS: TC=0.37/1.00 (B-C:1), BC=0.27/1.00 (J-L:1), WB=0.28/1.00 (G-I:1), SS=0.34/1.00 (J-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.45 (G) (INPUT = 1.00)



Structural component only
DWG# T-2007154 1/2

CONTINUED ON PAGE 2

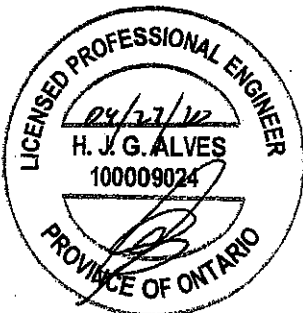
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	T28	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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 ID:K?TPdha0npl1aYdbWtOFzIdeG-P3Jfn? 3gwSIBtYB3B.jk??uz8RBKVIUf6op15zNBDV

PLATES (table is in inches)

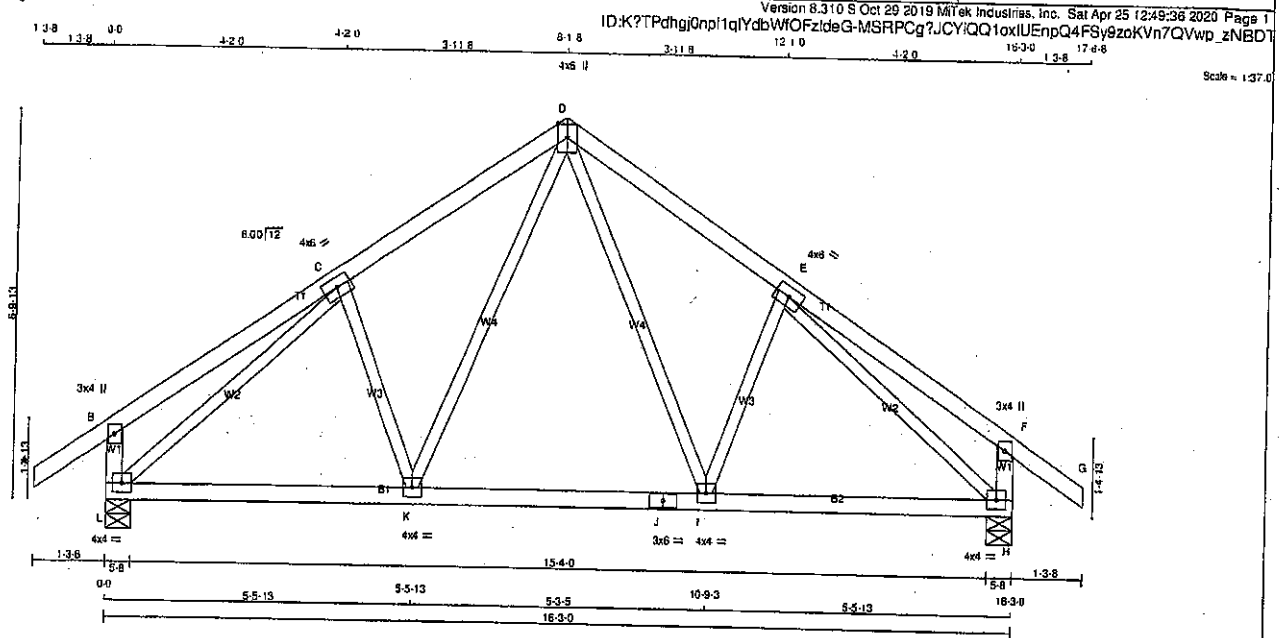
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.75
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMVW-t	MT20	4.0	4.0		
E	TMVW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	1.50
G	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I, L, M						
I	BMVW-t	MT20	5.0	6.0		
J	BMVW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
 TOUCHES EDGE OF CHORD.



Structural component only
 DWG# T-2007 154 *HL*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	T30	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV-p	MT20	3.0	4.0		
C TMVW-t	MT20	4.0	6.0		
D TTWW-p	MT20	4.0	6.0		Edge
E TMVW-t	MT20	4.0	6.0		
F TMV-p	MT20	3.0	4.0		
H BMVW-t	MT20	4.0	4.0		
I BMVW-t	MT20	4.0	4.0		
J BS-t	MT20	3.0	6.0		
K BMVW-t	MT20	4.0	4.0		
L BMVW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	HORZ	UPLIFT
L	1022	0	1022	0
H	1022	0	1022	0

UNFACTORED REACTIONS

1ST CASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	720	486 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0
H	720	486 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED TORSION (LBS-FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED TORSION (LBS-FT)
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	D-I	0 / 335	0.08 (1)		
B-C	0 / 24	-91.8	-91.8	0.24 (1)	10.00	I-E	-217 / 0	0.06 (1)		
C-D	-845 / 0	-91.8	-91.8	0.19 (1)	6.25	K-D	0 / 335	0.08 (1)		
D-E	-845 / 0	-91.8	-91.8	0.19 (1)	6.25	C-K	-217 / 0	0.06 (1)		
E-F	0 / 24	-91.8	-91.8	0.24 (1)	10.00	L-C	-1042 / 0	0.52 (1)		
F-G	0 / 35	-91.8	-91.8	0.12 (1)	10.00	E-H	-1042 / 0	0.52 (1)		
L-B	-288 / 0	0.0	0.0	0.03 (1)	7.81					
H-F	-288 / 0	0.0	0.0	0.03 (1)	7.81					
L-K	0 / 762	-18.5	-18.5	0.19 (1)	10.00					
K-J	0 / 557	-18.5	-18.5	0.17 (4)	10.00					
J-I	0 / 557	-18.5	-18.5	0.17 (4)	10.00					
I-H	0 / 762	-18.5	-18.5	0.19 (1)	10.00					

TOTAL WEIGHT = 2 X 72 = 145 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSK: TC=0.24/1.00 (B-C:1), BC=0.19/1.00 (H-I:1), WB=0.52/1.00 (C-L:1), SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.29 (L) (INPUT = 1.00)



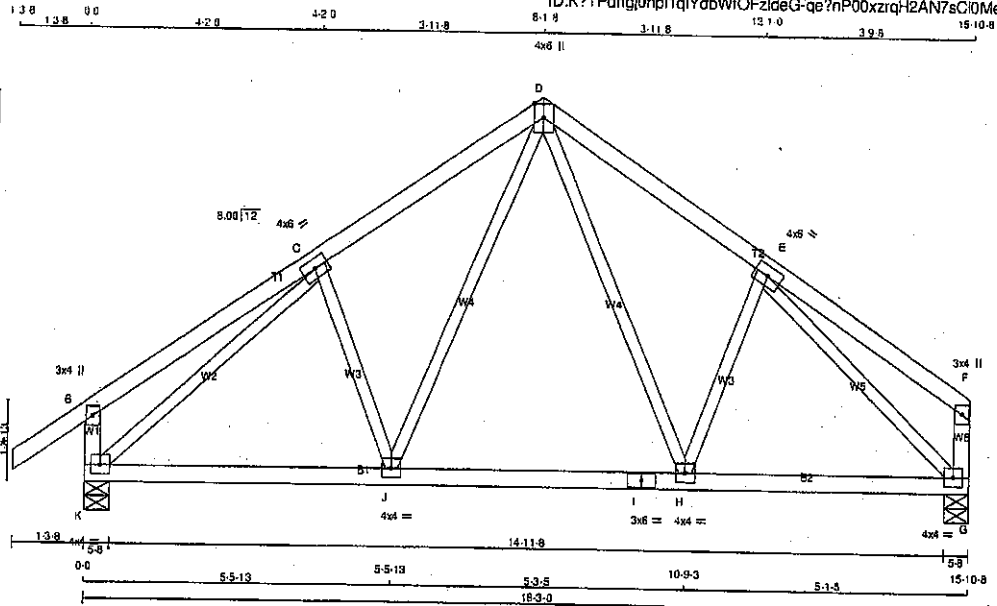
Structural component only
DWG# T-2007 156

JOB NAME 408169	TRUSS NAME T30A	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/32" = 1'-0"

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES	A - D	2x4	DRY	No.2	SPF
	D - F	2x4	DRY	No.2	SPF
	K - B	2x4	DRY	No.2	SPF
	G - F	2x4	DRY	No.2	SPF
	K - I	2x4	DRY	No.2	SPF
	I - G	2x4	DRY	No.2	SPF
	ALL WEBS	2x3	DRY	No.2	SPF
	EXCEPT				
	DRY: SEASONED LUMBER.				

PLATES (tablets in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0			
C TMWW-t	MT20	4.0	8.0			
D TTWW-t	MT20	4.0	8.0			
E TMWW-t	MT20	4.0	8.0			
F TMV+p	MT20	3.0	4.0			
G BMVW1-t	MT20	4.0	4.0			
H BMVW1-t	MT20	4.0	4.0			
I BS-t	MT20	3.0	8.0			
J BMVW1-t	MT20	4.0	4.0			
K BMVW1-t	MT20	4.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HORZ
K	1001	0	1001	0
G	875	0	875	0

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	706	476 / 0	0 / 0	0 / 0	0 / 0	229 / 0	0 / 0
G	619	408 / 0	0 / 0	0 / 0	0 / 0	213 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX. UNBRAC	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. (PL)	CS1 (LC)	LENGTH	FR-TO	MEMB.	FORCE (LBS)	MAX (LC)
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	D-H	0 / 272	0.08 (1)
B-C	0 / 24	-91.8	-91.8	0.24 (1)	10.00	H-E	-160 / 2	0.05 (1)
C-D	-817 / 0	-91.8	-91.8	0.19 (1)	6.25	J-D	0 / 342	0.08 (1)
D-E	-783 / 0	-91.8	-91.8	0.18 (1)	6.25	C-J	-222 / 0	0.06 (1)
E-F	0 / 24	-91.8	-91.8	0.22 (1)	10.00	K-C	-1012 / 0	0.51 (1)
K-B	-268 / 0	0.0	0.0	0.03 (1)	7.81	E-G	-982 / 0	0.44 (1)
G-F	-124 / 0	0.0	0.0	0.01 (1)	7.81			
K-J	0 / 740	-18.5	-18.5	0.18 (1)	10.00			
J-I	0 / 531	-18.5	-18.5	0.17 (4)	10.00			
I-H	0 / 531	-18.5	-18.5	0.17 (4)	10.00			
H-G	0 / 691	-18.5	-18.5	0.17 (1)	10.00			

TOTAL WEIGHT = 2 X 70 = 140 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
BOT CH.	LL = 8.0 PSF
	LL = 0.0 PSF
	LL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/899 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CS1: TC=0.24/1.00 (B-C:1), BC=0.18/1.00 (J-K:1), WB=0.51/1.00 (C-K:1), SS1=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MT20	618	354
	1667	789
	1997	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.28 (K) (INPUT = 1.00)

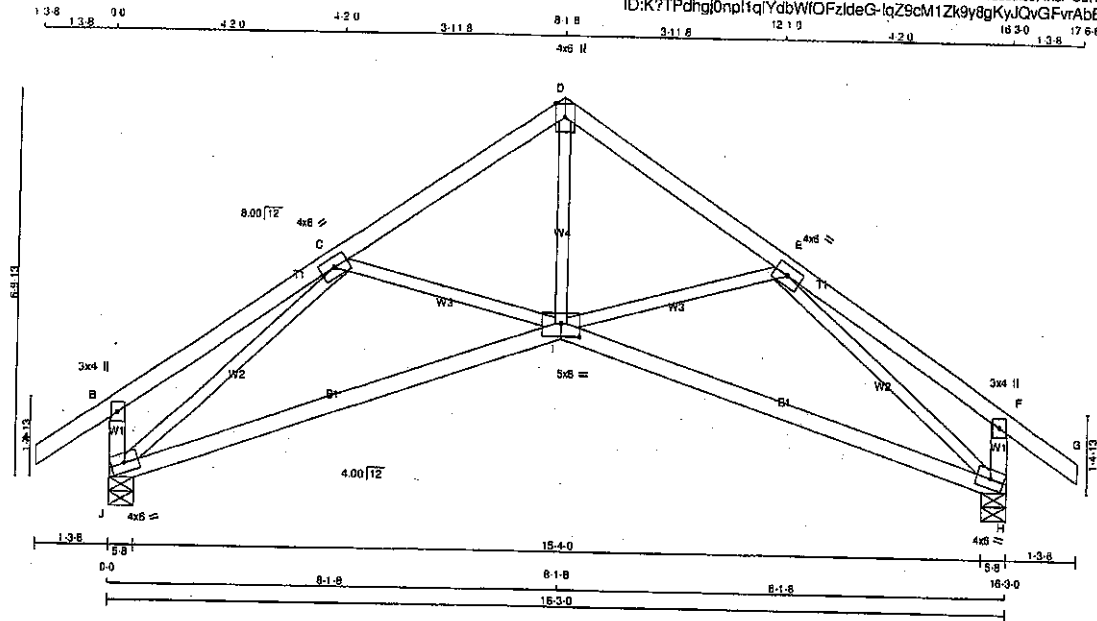


Structural component only
DWG# T-2007157

JOB NAME 408169	TRUSS NAME T30S	QUANTITY 2	PLY. 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/32"

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	8.0		
D	TTW+p	MT20	4.0	8.0	Edge	
E	TMWW-t	MT20	4.0	8.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	8.0		
I	BBWWV+p	MT20	5.0	8.0	2.75	4.00
J	BMVW-t	MT20	4.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	RECORD
	GROSS REACTION	DOWN	GROSS REACTION	DOWN		
JT	VERT	HORZ	DOWN	HORZ	UP/LIFT	IN-SX
J	1022	0	1022	0	0	5-8
H	1022	0	1022	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	720	486 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0
H	720	486 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (LC)
FR-TO				FR-TO			
A-B	0 / 35	-91.8	-91.8 0.12 (1)	I-D	0 / 942	0.21 (1)	
B-C	0 / 22	-91.8	-91.8 0.22 (1)	I-E	-143 / 11	0.05 (1)	
C-D	-1134 / 0	-91.8	-91.8 0.19 (1)	C-I	-143 / 11	0.05 (1)	
D-E	-1134 / 0	-91.8	-91.8 0.19 (1)	J-C	-1478 / 0	0.78 (1)	
E-F	0 / 22	-91.8	-91.8 0.22 (1)	E-H	-1478 / 0	0.78 (1)	
F-G	0 / 35	-91.8	-91.8 0.12 (1)				
J-B	-273 / 0	0.0	0.0 0.03 (1)				
H-F	-273 / 0	0.0	0.0 0.03 (1)				
J-I	0 / 1120	-18.5	-18.5 0.44 (4)				
I-H	0 / 1120	-18.5	-18.5 0.44 (4)				

TOTAL WEIGHT = 2 X 67 = 133 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/769 (0.25")

CSI: TC=0.22/1.00 (E-F:1), BC=0.44/1.00 (H-I:4), WB=0.78/1.00 (C-J:1), SS=0.15/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

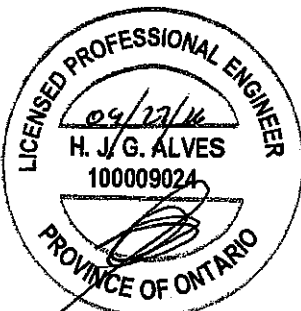
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PS)	(PL)	(PL)
MT20	618	354	1887 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.35 (H) (INPUT = 1.00)

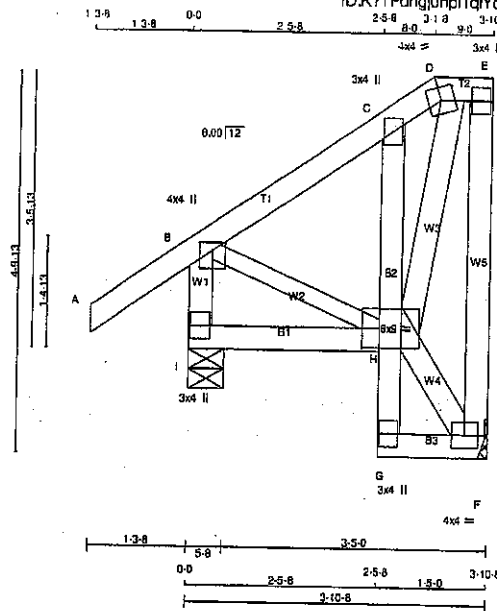


Structural component only
DWG# T-2007158

JOB NAME 408169	TRUSS NAME T31S	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/8" = 1'-0"

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
I - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
G - C	2x4	DRY	No.2
G - F	2x4	DRY	No.2

ALL WEBS 2x4 DRY No.2

EXCEPT

B - H 2x3 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMV+p	MT20	3.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-i	MT20	4.0	4.0		
G	BMV+p	MT20	3.0	4.0		
H	BMVW1-i	MT20	6.0	9.0	3.00	2.75
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	193	0	193	0
I	361	0	361	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS
	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
F	198	88 / 0 0 / 0 0 / 0 0 / 0 49 / 0 0 / 0
I	252	161 / 0 0 / 0 0 / 0 0 / 0 72 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) :

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	FR-TO	CHORDS				WEBS			
		MAX. FACTORED	FACTORED	VERT. LOAD	LC1 MAX	MAX. FACTORED	FACTORED	VERT. LOAD	LC1 MAX
		(LBS)	(PLF)		(LC)	(LBS)	(PLF)		(LC)
A-B	0 / 35			-91.8	-91.8 0.14 (5)	10.00		-27 / 0	0.00 (1)
B-C	-87 / 0			-91.8	-91.8 0.09 (1)	6.25		0 / 88	0.02 (1)
C-D	-99 / 0			-91.8	-91.8 0.07 (1)	6.25		0 / 173	0.03 (1)
D-E	-11 / 0			-91.8	-91.8 0.07 (1)	6.25			
E-F	-148 / 0			0.0	0.0 0.04 (1)	7.81			
F-G	-327 / 0			0.0	0.0 0.02 (4)	7.81			
I-H	0 / 8			-18.5	-18.5 0.03 (4)	10.00			
G-H	-2 / 8			0.0	0.0 0.05 (1)	10.00			
H-C	-203 / 0			0.0	0.0 0.04 (1)	7.81			
G-F	0 / 30			-18.5	-18.5 0.03 (1)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 29 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.14/1.00 (A-B-S), BC=0.05/1.00 (G-H-I), WB=0.03/1.00 (D-H-I), SS=0.12/1.00 (D-E-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



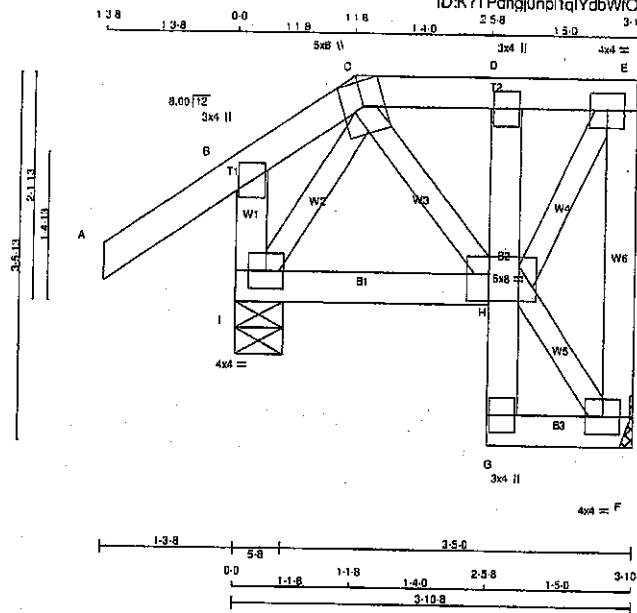
Structural component only
DWG# T-2007159

JOB NAME	TRUSS NAME	QUANTITY	PLY.	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408169	T32S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:20.1



CHORDS	SIZE	LUMBER
A - C	2x4	DRY No.2
C - E	2x4	DRY No.2
F - E	2x4	DRY No.2
I - B	2x4	DRY No.2
I - H	2x4	DRY No.2
G - D	2x4	DRY No.2
G - F	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	Edge	
D	TMV+p	MT20	3.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMV+p	MT20	3.0	4.0		
H	BMVWW1-t	MT20	5.0	8.0	3.00	2.50
I	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
F	VERT	DOWN	IN-SX	IN-SX
I	362	0	0	MECHANICAL
			5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX / MIN	COMPONENT REACTIONS
F	136	87 / -12	0 / 0
I	253	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MEMB.	MAX. FACTORED	FORCE (LBS)	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	FORCE (LBS)	MAX
FR-TO								FR-TO			
A-B	0 / 35	-31.8	-31.8	0.14	(5)	10.00	C-H	0 / 80	0.01	(1)	
B-C	-41 / 0	-31.8	-31.8	0.13	(5)	6.25	H-F	-7 / 0	0.00	(1)	
C-D	-89 / 0	-31.8	-31.8	0.02	(1)	6.25	H-E	0 / 153	0.03	(1)	
D-E	-86 / 0	-31.8	-31.8	0.02	(1)	6.25	I-C	-100 / 34	0.02	(1)	
F-E	-175 / 0	0.0	0.0	0.03	(1)	7.81					
I-B	-253 / 0	0.0	0.0	0.03	(1)	7.81					
I-H	-18 / 53	-18.5	-18.5	0.04	(4)	6.25					
G-H	0 / 13	0.0	0.0	0.01	(1)	10.00					
H-D	-142 / 0	0.0	0.0	0.01	(1)	7.81					
G-F	0 / 5	-18.5	-18.5	0.01	(4)	10.00					

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 25 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.0W/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (A-B-5), BC=0.04/1.00 (H-I-4), WB=0.03/1.00 (E-H-1), SS=0.03/1.00 (A-B-5)

DO: LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	619	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.18 (E) (INPUT = 0.90)
JSI METAL = 0.09 (B) (INPUT = 1.00)

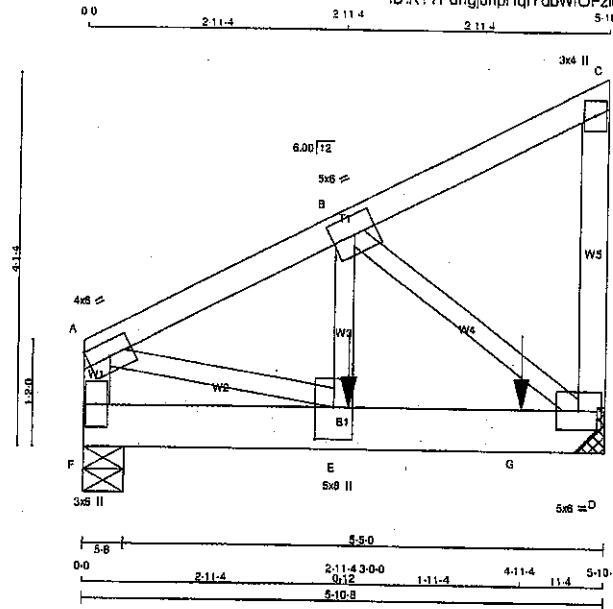


Structural component only
DWG# T-2007160

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408169	T33	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/2" = 1'-0"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A	12	TOP
A - C	12	TOP
C - D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
B - E	12	SIDE(305.2)
2x3	12	SIDE(408.9)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	Edge
B	TMVW-t	MT20	5.0	6.0	
C	TMVW-t	MT20	3.0	4.0	
D	BMVW-t	MT20	5.0	6.0	
E	BMVW-t	MT20	5.0	8.0	4.25 2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	1914	0	1914	0
D	2859	0	2859	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED
F	1348 915 / 0 0 / 0 0 / 0 0 / 0 0 / 0 0
D	2015 1359 / 0 0 / 0 0 / 0 0 657 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.59 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MEMB.	FORCE (LBS)
FR-TO			FR-TO	
F - A	-1877 / 0	0.0 0.0 0.10 (1)	A - E	0 / 2380
A - B	-2586 / 0	-91.8 -91.8 0.08 (1)	E - B	0 / 2608
B - C	-7 / 0	-91.8 -91.8 0.08 (1)	B - D	-2695 / 0
D - C	-120 / 0	0.0 0.0 0.01 (1)		
F - E	0 / 0	-18.5 -18.5 0.01 (1)		
E - G	0 / 2301	-18.5 -18.5 0.49 (1)		
G - D	0 / 2301	-18.5 -18.5 0.49 (1)		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-0-0	-2737	-2737	---	BACK	VERT	TOTAL	---	C1
G	4-11-4	-1388	-1388	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6 PSF
	DL	= 6.0 PSF
BOT CH.	LL	= 0.0 PSF
	DL	= 7.4 PSF
TOTAL LOAD		= 39.0 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCS 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $\frac{1}{360}$ (0.20")
CALCULATED VERT. DEFL.(LL) = $\frac{1}{899}$ (0.02")
ALLOWABLE DEFL.(TL) = $\frac{1}{360}$ (0.20")
CALCULATED VERT. DEFL.(TL) = $\frac{1}{899}$ (0.03")

CSI: TC=0.10/1.00 (A-F:1), 8C=0.49/1.00 (D-E:1), WB=0.35/1.00 (B-D:1), 6SI=0.32/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

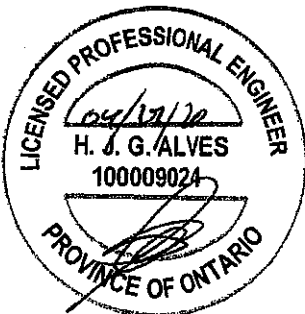
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618 354 1667 788 1667 1668	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (B) (INPUT = 0.90)
JSI METAL = 0.44 (E) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2007161 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408169	T33	1	2	TRUSS DESC.		

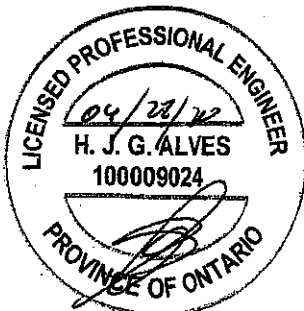
Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 12:49:41 2020 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



Structural component only
DWG# T-2007161 22

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T40	1	2	GREEN PARK HOMES	

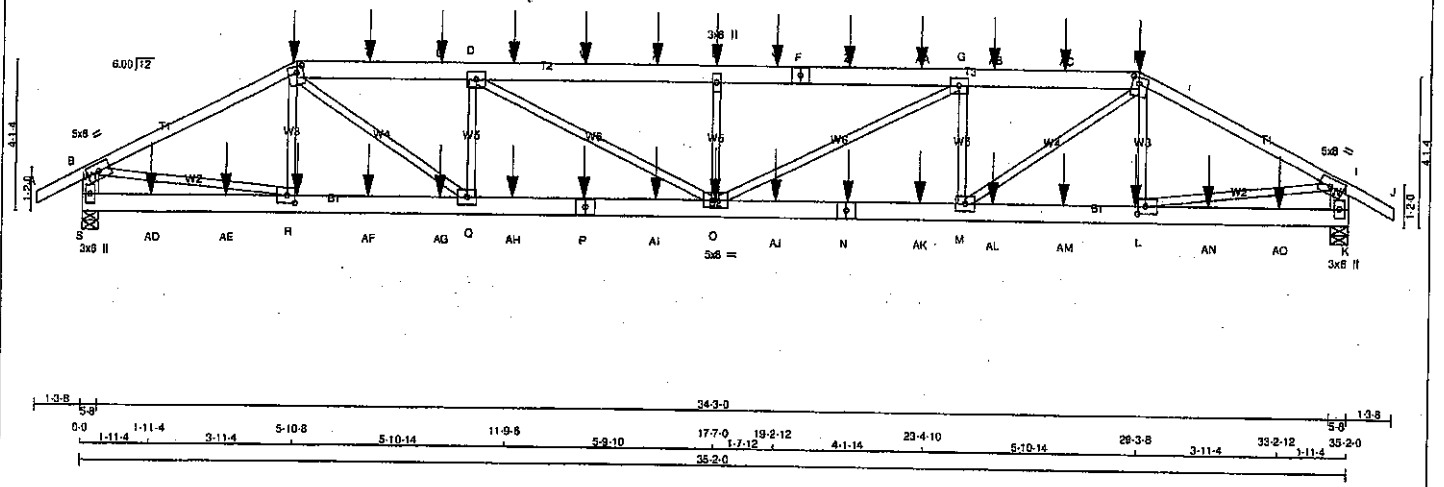
Tamarack Roof Truss, Burlington

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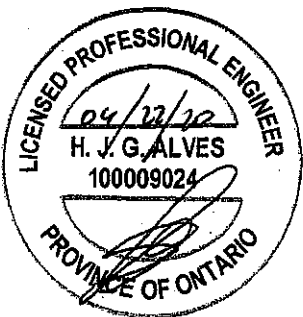
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13-8 0-0 5-10-8 5-10-8 5-10-14 11-9-6 5-9-10 17-7-0 19-2-12 3-1-14 23-4-10 5-10-14 29-3-8 5-10-8 35-2-0 38-5-8 13-8

Scale = 1/8" = 1'-0"



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER A - C 2x4 DRY No.2 C - F 2x6 DRY No.2 F - H 2x6 DRY No.2 H - J 2x4 DRY No.2 S - B 2x6 DRY No.2 K - I 2x6 DRY No.2 S - P 2x6 DRY No.2 P - N 2x6 DRY No.2 N - K 2x6 DRY No.2 ALL WEBS 2x3 DRY No.2 EXCEPT DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF) TOP CHORDS : (0.122"x3") SPIRAL NAILS A-C 1 12 SIDE(61.0) H-J 1 12 SIDE(61.0) C-F 2 12 SIDE(61.0) F-H 2 12 SIDE(61.0) S-B 2 12 TOP K-I 2 12 TOP BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS S-P 2 12 SIDE(183.1) P-N 2 12 SIDE(183.1) N-K 2 12 SIDE(183.1) WEBS : (0.122"x3") SPIRAL NAILS 2x3 1 6 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.			DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX JT 3331 0 2331 0 0 5-8 5-8 S 3331 0 3331 0 0 5-8 5-8 K 3331 0 3331 0 0 5-8 5-8 UNFACTORED REACTIONS 1ST CASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL S 2356 1544 / 0 0 / 0 0 / 0 812 / 0 0 / 0 K 2356 1544 / 0 0 / 0 0 / 0 812 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MEMB. MAX. FACTORED FORCE (LBS) VERT. LOAD LC1 MAX (PLF) CSI (LC) MAX. UNBRACED LENGTH FR-TO A-B 0 / 28 -91.8 -91.8 0.07 (1) 10.00 B-C -5069 / 0 -91.8 -91.8 0.52 (1) 3.87 C-T -6876 / 0 -91.8 -91.8 0.24 (1) 4.35 T-U -6976 / 0 -91.8 -91.8 0.24 (1) 4.35 U-D -6976 / 0 -91.8 -91.8 0.24 (1) 4.35 D-V -8155 / 0 -91.8 -91.8 0.31 (1) 4.02 V-W -8155 / 0 -91.8 -91.8 0.31 (1) 4.02 W-X -8155 / 0 -91.8 -91.8 0.31 (1) 4.02 X-E -8155 / 0 -91.8 -91.8 0.31 (1) 4.02 E-Y -8155 / 0 -91.8 -91.8 0.31 (1) 4.02 Y-F -8155 / 0 -91.8 -91.8 0.31 (1) 4.02 F-Z -8155 / 0 -91.8 -91.8 0.31 (1) 4.02 Z-AA -8155 / 0 -91.8 -91.8 0.31 (1) 4.02 AA-G -8155 / 0 -91.8 -91.8 0.31 (1) 4.02 G-AB -6976 / 0 -91.8 -91.8 0.24 (1) 4.35 AB-AC -6976 / 0 -91.8 -91.8 0.24 (1) 4.35 AC-H -6976 / 0 -91.8 -91.8 0.24 (1) 4.35 H-I -5069 / 0 -91.8 -91.8 0.52 (1) 3.87 I-J 0 / 28 -91.8 -91.8 0.07 (1) 10.00 S-B -3250 / 0 0.0 0.0 0.11 (1) 7.89 K-I -3250 / 0 0.0 0.0 0.11 (1) 7.89 WEBS MEMB. MAX. FACTORED FORCE (LBS) MAX. UNBRACED LENGTH FR-TO R-C -449 / 0 10.00 C-Q 0 / 3021 0.37 (1) Q-D -1808 / 0 0.19 (1) D-O 0 / 1339 0.17 (1) O-E -1056 / 0 0.13 (1) E-G 0 / 1339 0.17 (1) G-H -1808 / 0 0.19 (1) H-I 0 / 3021 0.37 (1) I-H -449 / 0 0.06 (1) B-R 0 / 4585 0.57 (1) L-I 0 / 4585 0.57 (1) S-AD 0 / 0 -18.5 -18.5 0.07 (4) 10.00 AD-AE 0 / 0 -18.5 -18.5 0.07 (4) 10.00 AE-R 0 / 0 -18.5 -18.5 0.07 (4) 10.00 R-AF 0 / 4557 -18.5 -18.5 0.33 (1) 10.00 AF-AG 0 / 4557 -18.5 -18.5 0.33 (1) 10.00 AG-Q 0 / 4557 -18.5 -18.5 0.33 (1) 10.00 Q-AH 0 / 6975 -18.5 -18.5 0.52 (1) 10.00 AH-P 0 / 6975 -18.5 -18.5 0.52 (1) 10.00 P-AI 0 / 6975 -18.5 -18.5 0.52 (1) 10.00 AI-O 0 / 6975 -18.5 -18.5 0.52 (1) 10.00 O-AJ 0 / 6975 -18.5 -18.5 0.52 (1) 10.00 AJ-N 0 / 6975 -18.5 -18.5 0.52 (1) 10.00 N-AK 0 / 6975 -18.5 -18.5 0.52 (1) 10.00 AK-M 0 / 6975 -18.5 -18.5 0.52 (1) 10.00 M-AL 0 / 4557 -18.5 -18.5 0.33 (1) 10.00 AL-AM 0 / 4557 -18.5 -18.5 0.33 (1) 10.00 AM-L 0 / 4557 -18.5 -18.5 0.33 (1) 10.00 L-AN 0 / 0 -18.5 -18.5 0.07 (4) 10.00 AN-AO 0 / 0 -18.5 -18.5 0.07 (4) 10.00 AO-K 0 / 0 -18.5 -18.5 0.07 (4) 10.00			DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 25.6 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 39.0 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF NBCC 2018, CBC 2012, ABC 2019 - PART 9 OF CBC 2012 (2019 AMENDMENT) - CSA 086-09, CSA 086-14 - TPIC 2011, TPIC 2014 (65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (1.17") CALCULATED VERT. DEFL.(LL) = L/999 (0.21") ALLOWABLE DEFL.(TL) = L/360 (1.17") CALCULATED VERT. DEFL.(TL) = L/999 (0.39") CSI: TC=0.52/1.00 (B-C:1), BC=0.52/1.00 (C-Q:1), WL=0.57/1.00 (B-R:1), SSL=0.16/1.00 (D-E:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 518 354 1687 788 1987 1656 PLATE PLACEMENT TOL = 0.250 inches PLATE ROTATION TOL = 5.0 Deg. JSI GRIP= 0.90 (H) (INPUT = 0.90) JSI METAL= 0.67 (P) (INPUT = 1.00)		
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Structural component only
DWG# T-2007168 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T40	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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ID:K?TPdhg0npl1qlydbWIOFzideG-dTolVCBPTkKk gKpGOLn4U0ubv6N8cIVf?sayUzNB1c

PLATES (table is in inches)

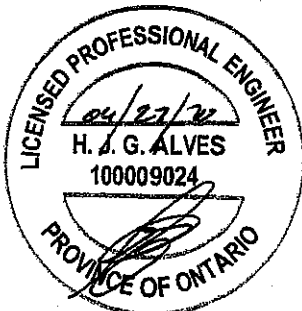
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TTWW+m	MT20	5.0	5.0	2.00	2.25
D	TMWW-t	MT20	5.0	6.0		
E	TMWW-w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	5.0	6.0	2.00	2.25
I	TMWW-t	MT20	5.0	8.0		
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	8.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.50
S	BMV1+p	MT20	3.0	6.0		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-437	-437	---	FRONT	VERT	TOTAL	---	C1
E	17-7-0	-110	-110	---	FRONT	VERT	TOTAL	---	C1
H	28-3-8	-437	-437	---	FRONT	VERT	TOTAL	---	C1
L	29-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
N	21-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	17-7-0	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	13-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
R	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
T	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
U	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	11-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	15-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	19-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	21-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	23-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	25-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	27-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	1-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
AE	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AF	7-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AG	9-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AH	11-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AI	15-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AJ	19-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AK	23-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AL	25-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AM	27-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AN	31-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AO	33-2-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007168

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T40Z	1	2	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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ID:K?TPdghi0no!1qlYqbWIOFzIdeG-arwSvuDf MaSDzUBNpNF9v6Buik?cSGo6JLh0NzNB1a

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	2.00	3.50
C	TTWW-m	MT20	8.0	9.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMW-w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW-m	MT20	6.0	9.0	Edge	
I	TMWW-p	MT20	5.0	8.0	2.00	3.50
K	BMVt-p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	3.25
M	BMWW-t	MT20	6.0	9.0	4.50	2.50
N	BS-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	6.0	9.0	4.50	2.50
R	BMWW-t	MT20	5.0	8.0	2.50	3.25
S	BMVt-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

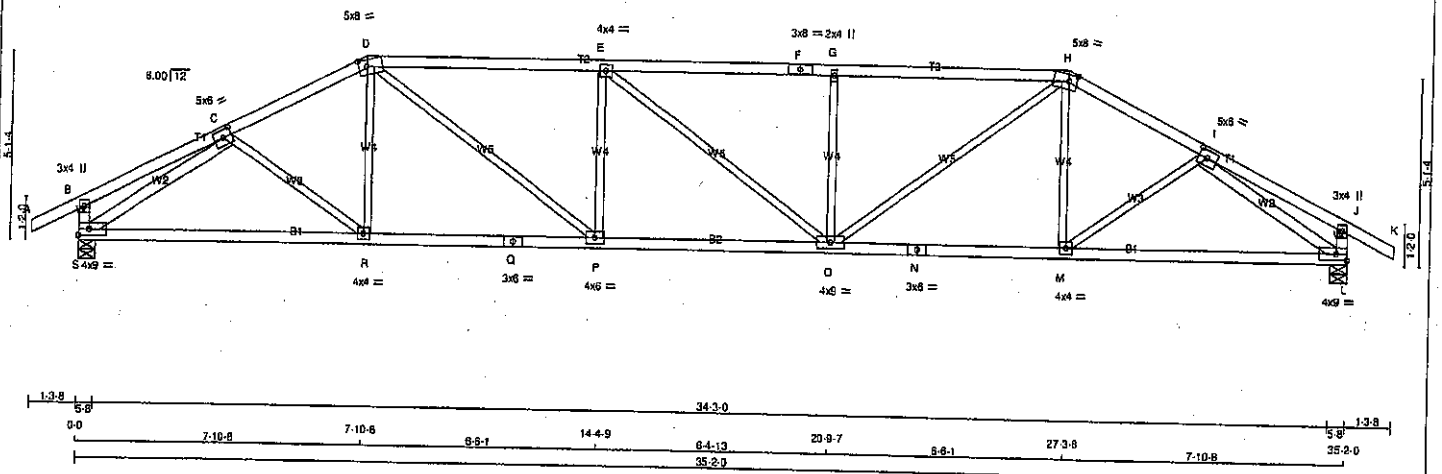
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007169

JOB NAME 408170	TRUSS NAME T41	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 ID:K?TPdngj0npi1qIYdbWfOFzIdG-22UT7EDHJR730xVWU6eJ865BLvLxLz4FZpzNB1Z
 1-3-8 0-0 4-0-8 3-10-0 7-10-5 8-8-1 14-4-9 6-4-13 20-9-7 6-8-1 27-3-8 3-10-0 31-1-8 4-0-8 35-2-0 36-5-8 1-3-8
 Scale = 1:57.3



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
Q - Q	2x4	DRY	No.2
S - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
S - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-1	MT20	5.0	6.0	2.50 2.75
D	TTWW-m	MT20	5.0	8.0	2.25 2.75
E	TMWW-1	MT20	4.0	4.0	
F	TS-1	MT20	3.0	8.0	
G	TMW+w	MT20	2.0	4.0	
H	TTWW-m	MT20	5.0	8.0	2.25 2.75
I	TMWW-1	MT20	5.0	6.0	2.50 2.75
J	TMV+p	MT20	3.0	4.0	
L	BMVW-1	MT20	4.0	9.0	Edge
M	BMVW-1	MT20	4.0	4.0	
N	BS-1	MT20	3.0	5.0	
O	BMVW-1	MT20	4.0	9.0	
P	BMVW-1	MT20	4.0	9.0	
Q	BS-1	MT20	3.0	6.0	
R	BMVW-1	MT20	4.0	4.0	
S	BMVW-1	MT20	4.0	9.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
S	2083	0	2083	0	5-8
L	2083	0	2083	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS				
S	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
L	1457	989 / 0	0 / 0	0 / 0	0 / 0	489 / 0	0 / 0	
L	1457	989 / 0	0 / 0	0 / 0	0 / 0	489 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.94 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 28	C-R	0 / 93
B-C	0 / 16	R-D	0 / 121
C-D	-2799 / 0	D-P	0 / 1267
D-E	-3508 / 0	P-E	-643 / 0
E-F	-3507 / 0	E-O	-2 / 0
F-G	-3507 / 0	O-G	-643 / 0
G-H	-3507 / 0	C-H	0 / 1268
H-I	-2789 / 0	M-I	0 / 93
I-J	0 / 16	S-C	-2974 / 0
J-K	0 / 28	I-L	-2974 / 0
K-L	-270 / 0		
L-J	-270 / 0		
S-R	0 / 2417		
R-Q	0 / 2489		
Q-P	0 / 2489		
P-O	0 / 3509		
O-N	0 / 2489		
N-M	0 / 2489		
M-L	0 / 2417		

TOTAL WEIGHT = 2 X 139 = 278 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 240 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, OBC 2012, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17')
 CALCULATED VERT. DEFL.(LL) = L/999 (0.21')
 ALLOWABLE DEFL.(TL) = L/360 (1.17')
 CALCULATED VERT. DEFL.(TL) = L/999 (0.41')

CSI: TC=0.85/1.00 (D-E-1), BC=0.64/1.00 (O-P-1),
 WB=0.83/1.00 (H-I-1), SS=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	618	354
	1687	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

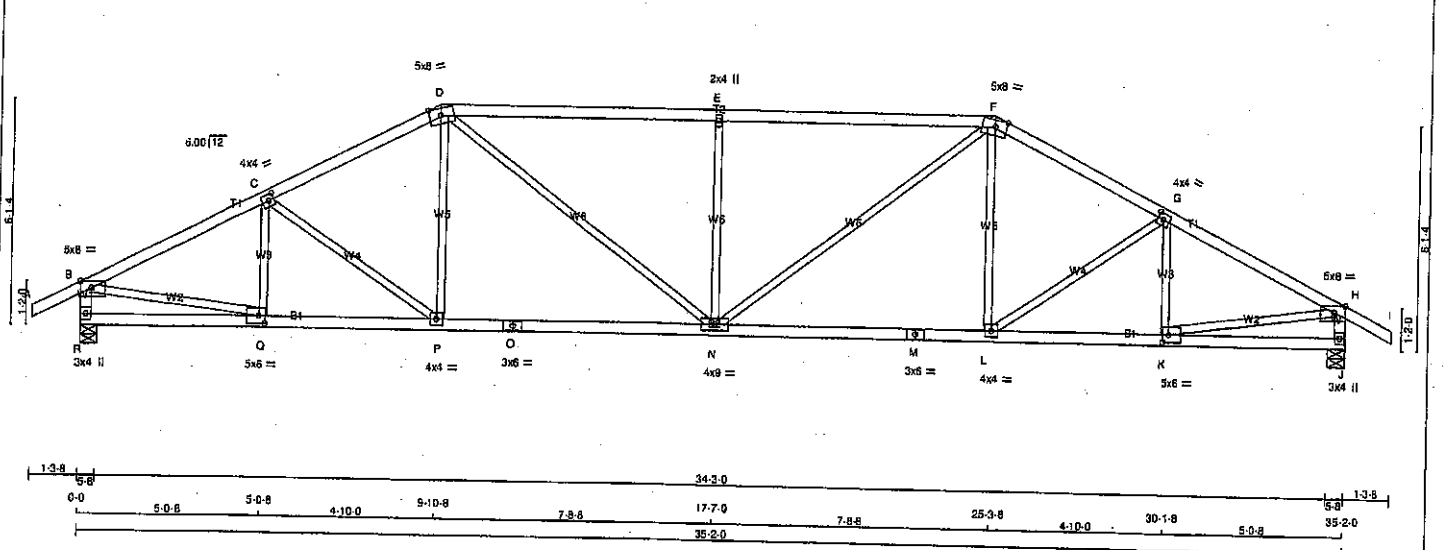
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
 JSI METAL= 0.77 (N) (INPUT = 1.00)



Structural component only
 DWG# T-2007170

JOB NAME 408170	TRUSS NAME T42	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 13:02:19 2020 Page 1	
ID:K?TPdngi0nph1qiYdbWfOfZldeG-WE2rKaEwWzr9THdaVEQIEKBSUWUW4PP4adqo5FzNB1Y				Scale = 1:57.3	



LUMBER
N.L.G.A. RULES
CHORDS SIZE LUMBER
A - D 2x4 DRY No.2
D - F 2x4 DRY No.2
F - I 2x4 DRY No.2
R - B 2x4 DRY No.2
J - H 2x4 DRY No.2
R - O 2x4 DRY No.2
O - M 2x4 DRY No.2
M - J 2x4 DRY No.2
ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY		BUILDING DESIGNER		BEARINGS	
FACTORED	MAXIMUM FACTORED	INPUT	REQD	FACTORED	MAXIMUM FACTORED
GROSS REACTION	GROSS REACTION	BRG	BRG	GROSS REACTION	GROSS REACTION
VERT	HORZ	DOWN	UPLIFT	IN-SX	IN-SX
JT 2063 0	2063 0	0 0	5-8 5-8	JT 2063 0	2063 0
R 2063 0	2063 0	0 0	5-8 5-8	R 2063 0	2063 0
J 2063 0	2063 0	0 0	5-8 5-8	J 2063 0	2063 0

UNFACTORED REACTIONS
1ST LOASE MAX./MIN. COMPONENT REACTIONS
JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL
R 1457 969 / 0 0 / 0 0 / 0 498 / 0 0 / 0
J 1457 969 / 0 0 / 0 0 / 0 498 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B 0 / 28	-91.8 -91.8 0.12 (1)	10.00	Q-C -353 / 0 0.08 (1)
B-C -2834 / 0	-91.8 -91.8 0.39 (1)	3.81	C-P -210 / 0 0.13 (1)
C-D -2687 / 0	-91.8 -91.8 0.37 (1)	3.91	P-D 0 / 246 0.06 (4)
D-E -3060 / 0	-91.8 -91.8 0.97 (1)	2.78	D-N 0 / 844 0.19 (1)
E-F -3060 / 0	-91.8 -91.8 0.97 (1)	2.78	N-E -872 / 0 0.51 (1)
F-G -2687 / 0	-91.8 -91.8 0.37 (1)	3.91	N-F 0 / 844 0.19 (1)
G-H -2834 / 0	-91.8 -91.8 0.39 (1)	3.81	L-F 0 / 246 0.06 (4)
H-I 0 / 28	-91.8 -91.8 0.12 (1)	10.00	L-G -210 / 0 0.13 (1)
R-B -2018 / 0	0.0 0.0 0.20 (1)	5.94	K-G -353 / 0 0.08 (1)
J-H -2018 / 0	0.0 0.0 0.20 (1)	5.94	B-Q 0 / 2591 0.58 (1)
R-Q 0 / 0	-18.5 -18.5 0.10 (4)	10.00	K-H 0 / 2591 0.58 (1)
Q-P 0 / 2555	-18.5 -18.5 0.51 (1)	10.00	
P-O 0 / 2386	-18.5 -18.5 0.49 (1)	10.00	
O-N 0 / 2386	-18.5 -18.5 0.49 (1)	10.00	
N-M 0 / 2386	-18.5 -18.5 0.49 (1)	10.00	
M-L 0 / 2386	-18.5 -18.5 0.49 (1)	10.00	
L-K 0 / 2555	-18.5 -18.5 0.51 (1)	10.00	
K-J 0 / 0	-18.5 -18.5 0.10 (4)	10.00	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.34")

CSI: TC=0.97/1.00 (D-E:1), BC=0.51/1.00 (P-Q:1),
WB=0.58/1.00 (B-Q:1), SS=0.34/1.00 (D-E:1)

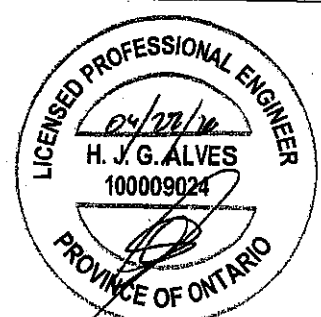
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

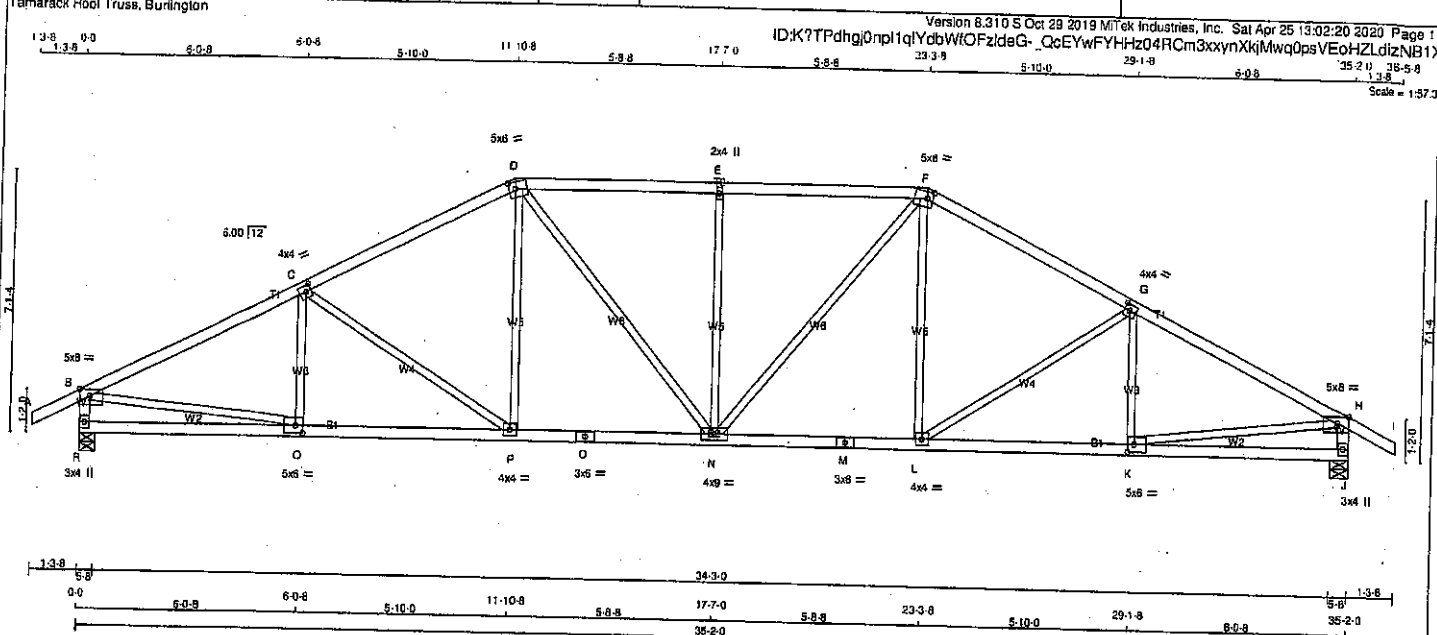
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1666

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP=0.97 (F) (INPUT = 0.90)
JSI METAL=0.75 (M) (INPUT = 1.00)



Structural component only
DWG# T-2007171

JOB NAME 408170	TRUSS NAME T43	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	X
B	TMWV-p	MT20	5.0	8.0 Edge 3.50
C	TMWV-w	MT20	4.0	4.0 2.00 1.75
D	TTWV-m	MT20	5.0	8.0 2.25 2.00
E	TMWV-w	MT20	2.0	4.0
F	TTWV-m	MT20	5.0	6.0 2.25 2.00
G	TMWV-w	MT20	4.0	4.0 2.00 1.75
H	TMWV-p	MT20	5.0	8.0 Edge 3.50
J	BMV1-p	MT20	3.0	4.0
K	BMWV-t	MT20	5.0	8.0 2.50 2.25
L	BMWV-t	MT20	4.0	4.0
M	BS-t	MT20	3.0	6.0
N	BMWV-w	MT20	4.0	9.0
O	BS-t	MT20	3.0	6.0
P	BMWV-w	MT20	4.0	4.0
Q	BMWV-w	MT20	5.0	6.0 2.50 2.25
R	BMV1-p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	HORZ	UPLIFT
JT 2063	0	2063	0
J 2063	0	2063	0

UNFACTORED REACTIONS			
1ST LCASE	MAX./MIN.	COMPONENT REACTIONS	
JT COMBINED	SNOW	LIVE	PERM.LIVE
R 1457	969 / 0	0 / 0	0 / 0
J 1457	969 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)
FR-TO			
A-B	0 / 28	-91.8 -91.8 0.12 (1)	10.00
B-C	-2889 / 0	-91.8 -91.8 0.58 (1)	3.58
C-D	-2539 / 0	-91.8 -91.8 0.52 (1)	3.83
D-E	-2559 / 0	-91.8 -91.8 0.48 (1)	3.85
E-F	-2559 / 0	-91.8 -91.8 0.48 (1)	3.85
F-G	-2539 / 0	-91.8 -91.8 0.52 (1)	3.83
G-H	-2889 / 0	-91.8 -91.8 0.58 (1)	3.58
H-I	0 / 28	-91.8 -91.8 0.12 (1)	10.00
I-J	-2014 / 0	0.0 0.0 0.20 (1)	5.95
J-K	-2014 / 0	0.0 0.0 0.20 (1)	5.95
K-L	0 / 0	-18.5 -18.5 0.15 (4)	10.00
L-M	0 / 2809	-18.5 -18.5 0.49 (1)	10.00
M-N	0 / 2249	-18.5 -18.5 0.43 (1)	10.00
N-O	0 / 2249	-18.5 -18.5 0.43 (1)	10.00
O-P	0 / 2249	-18.5 -18.5 0.43 (1)	10.00
P-Q	0 / 2609	-18.5 -18.5 0.49 (1)	10.00
Q-R	0 / 0	-18.5 -18.5 0.15 (4)	10.00

TOTAL WEIGHT = 2 X 145 = 289 lb

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL	=	25.8 PSF
BOT CH.	LL	=	6.0 PSF
	DL	=	0.0 PSF
	DL	=	7.4 PSF
TOTAL LOAD	=	39.0 PSF	

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.26")
CSI: TC=0.58/1.00 (B-C:1), BC=0.49/1.00 (P-Q:1), WB=0.59/1.00 (B-Q:1), SS=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

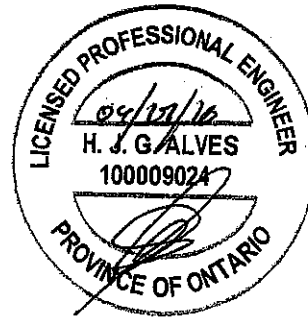
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PL)	SECTION (PL)	
(PSI)	(PL)		
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1697 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (Q) (INPUT = 0.90)
JSI METAL = 0.69 (M) (INPUT = 1.00)

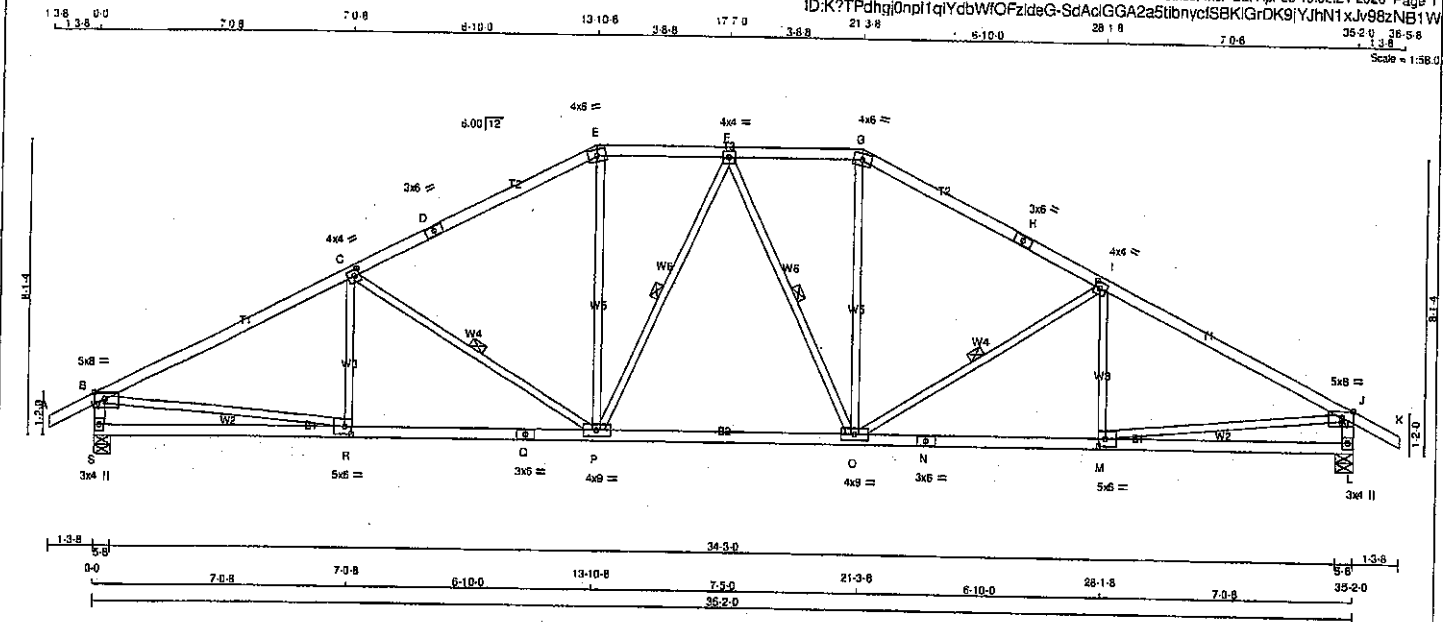


Structural component only
DWG# T-2007172

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T44	2	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 145 = 291 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - K	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
O - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2

ALL WEBS EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge 3.50	
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	6.0		
F	TMVW-t	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	6.0		
H	TS-t	MT20	3.0	6.0		
I	TMVW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-p	MT20	5.0	8.0	Edge 3.50	
L	BMV-t	MT20	3.0	4.0		
M	BMVW-t	MT20	5.0	6.0	2.50	2.25
N	BS-t	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	9.0		
P	BMVW-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		
R	BMVW-t	MT20	5.0	6.0	2.50	2.25
S	BMV-t	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORIZ	DOWN	HORIZ		
S	2093	0	2093	0	5-8	5-8
L	2093	0	2093	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1457	999 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
L	1457	999 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-P, F-O, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	R-C	-182 / 52	0.08 (1)
B-C	-2908 / 0	-91.8	-91.8 0.83 (1)	3.25	C-P	-837 / 0	0.29 (1)
C-D	-2373 / 0	-91.8	-91.8 0.72 (1)	3.66	P-E	0 / 874	0.15 (1)
D-E	-2373 / 0	-91.8	-91.8 0.72 (1)	3.66	P-F	-222 / 0	0.11 (1)
E-F	-2100 / 0	-91.8	-91.8 0.19 (1)	4.53	F-O	-222 / 0	0.11 (1)
F-G	-2100 / 0	-91.8	-91.8 0.19 (1)	4.53	O-G	0 / 874	0.15 (1)
G-H	-2373 / 0	-91.8	-91.8 0.72 (1)	3.66	O-I	-837 / 0	0.29 (1)
H-I	-2373 / 0	-91.8	-91.8 0.72 (1)	3.66	M-I	-182 / 52	0.08 (1)
I-J	-2908 / 0	-91.8	-91.8 0.83 (1)	3.25	B-R	0 / 2649	0.60 (1)
J-K	0 / 28	-91.8	-91.8 0.12 (1)	10.00	M-J	0 / 2649	0.60 (1)
S-B	-2008 / 0	0.0	0.0 0.20 (1)	5.96			
L-J	-2008 / 0	0.0	0.0 0.20 (1)	5.96			
S-R	0 / 0	-18.5	-18.5 0.20 (4)	10.00			
R-Q	0 / 2630	-18.5	-18.5 0.52 (1)	10.00			
C-P	0 / 2630	-18.5	-18.5 0.52 (1)	10.00			
F-O	0 / 2193	-18.5	-18.5 0.45 (1)	10.00			
C-N	0 / 2630	-18.5	-18.5 0.52 (1)	10.00			
N-M	0 / 2630	-18.5	-18.5 0.52 (1)	10.00			
M-L	0 / 0	-18.5	-18.5 0.20 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/899 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.83/1.00 (B-C:1), BC=0.52/1.00 (P-R:1), WB=0.60/1.00 (B-R:1), SS=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

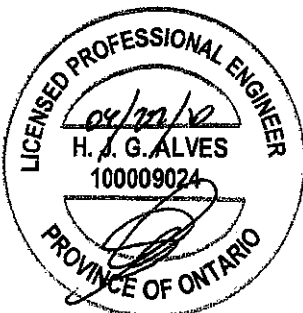
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1567 768 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (M) (INPUT = 0.90)
JSI METAL = 0.79 (Q) (INPUT = 1.00)



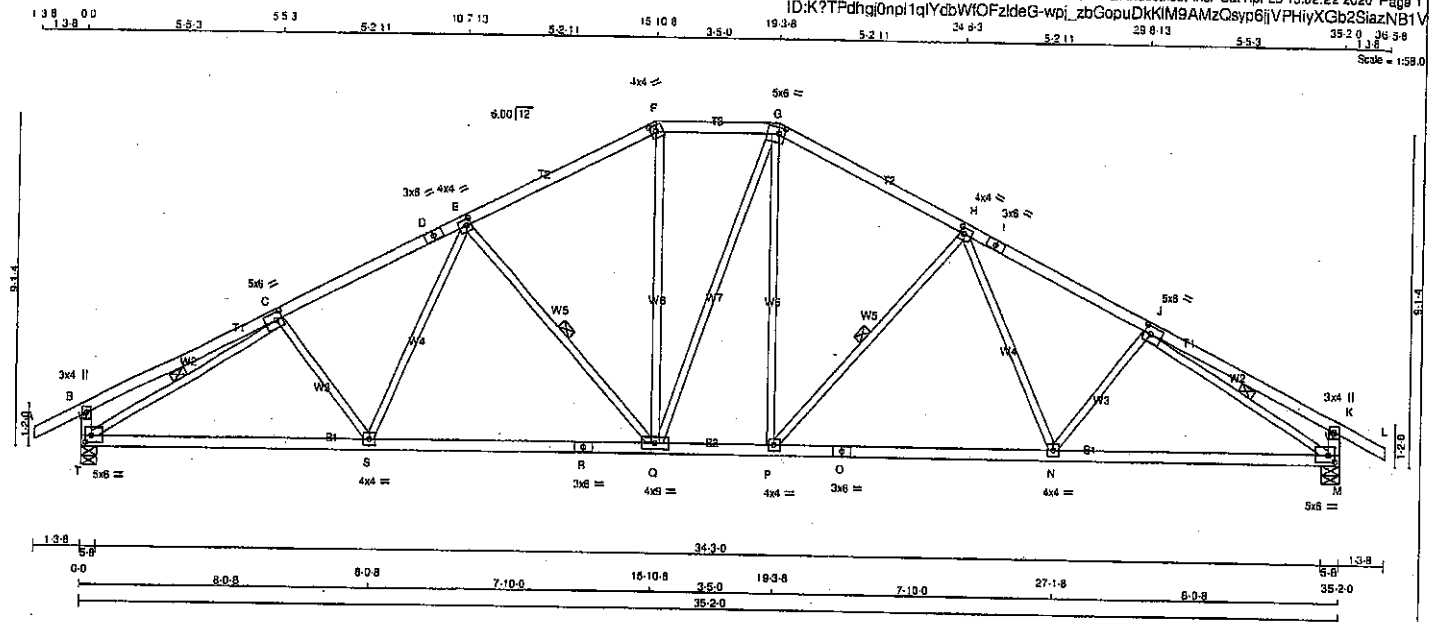
Structural component only
DWG# T-2007173

JOB NAME 408170	TRUSS NAME T45	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
L - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
T - R	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMV+p MT20	3.0	4.0		
C TMWW-t MT20	5.0	6.0	2.50	2.25
D TS-t MT20	3.0	6.0		
E TMWW-t MT20	4.0	4.0	2.00	1.50
F TTW-h MT20	4.0	4.0	2.00	1.75
G TTWW-m MT20	5.0	6.0	2.00	2.00
H TMWW-t MT20	4.0	4.0	2.00	1.50
I TS-t MT20	3.0	6.0		
J TMWW-t MT20	5.0	6.0	2.50	2.25
K TMV+p MT20	3.0	4.0		
M BMVW-t MT20	5.0	6.0	2.25	2.00
N, P, S				
N BMVW-t MT20	4.0	4.0		
O BS-t MT20	3.0	6.0		
Q BMVWW-t MT20	4.0	9.0		
R BS-t MT20	3.0	6.0		
T BMVW-t MT20	5.0	6.0	2.25	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT 2063	DOWN 2063	0	5-8
M	0	0	0	5-8

UNFACTORED REACTIONS	1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
T	1457	989 / 0 0 / 0 0 / 0 0 / 0 488 / 0 0 / 0
M	1457	989 / 0 0 / 0 0 / 0 0 / 0 488 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, H-P, C-T, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	C-S	-110 / 37	0.04 (1)	
B-C	0 / 19	-91.8	-91.8	0.32 (1)	S-E	0 / 276	0.03 (1)	
C-D	-2759 / 0	-91.8	-91.8	0.40 (1)	E-Q	-681 / 0	0.31 (1)	
D-E	-2759 / 0	-91.8	-91.8	0.40 (1)	Q-F	0 / 613	0.14 (1)	
E-F	-2167 / 0	-91.8	-91.8	0.36 (1)	F-G	0 / 4	0.00 (1)	
F-G	-1925 / 0	-91.8	-91.8	0.18 (1)	G-H	0 / 609	0.14 (1)	
G-H	-2168 / 0	-91.8	-91.8	0.36 (1)	H-N	-682 / 0	0.31 (1)	
H-I	-2760 / 0	-91.8	-91.8	0.40 (1)	I-J	0 / 278	0.06 (1)	
I-J	-2760 / 0	-91.8	-91.8	0.40 (1)	J-K	-110 / 37	0.04 (1)	
J-K	0 / 19	-91.8	-91.8	0.32 (1)	K-L	-3040 / 0	0.85 (1)	
K-L	0 / 28	-91.8	-91.8	0.12 (1)	L-M	-3041 / 0	0.85 (1)	
L-M	-326 / 0	0.0	0.0	0.03 (1)				
M-K	-326 / 0	0.0	0.0	0.03 (1)				
T-S	0 / 2536	-18.5	-18.5	0.55 (1)				
S-R	0 / 2362	-18.5	-18.5	0.53 (1)				
R-Q	0 / 2362	-18.5	-18.5	0.53 (1)				
Q-P	0 / 1924	-18.5	-18.5	0.40 (1)				
P-O	0 / 2362	-18.5	-18.5	0.53 (1)				
O-N	0 / 2362	-18.5	-18.5	0.53 (1)				
N-M	0 / 2537	-18.5	-18.5	0.55 (1)				

TOTAL WEIGHT = 2 X 151 = 302 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2018 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.32")

CSI: TC=0.40/1.00 (H-J:1), BC=0.56/1.00 (M-N:1),
WB=0.85/1.00 (J-M:1), SSI=0.20/1.00 (S-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

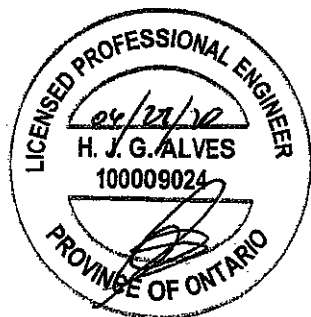
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1655

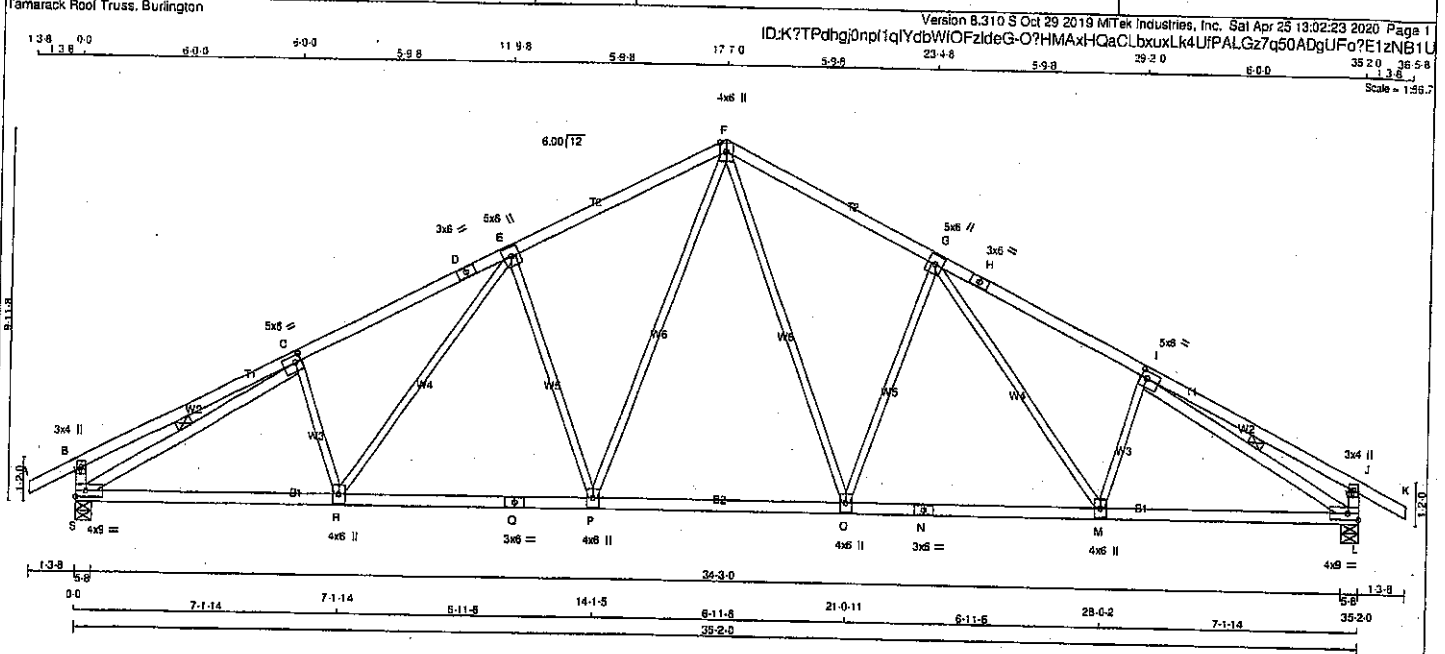
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (M) (INPUT = 0.90)
JSI METAL = 0.78 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007174



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY										TOTAL WEIGHT = 6 X 152 = 910 lb			
N. L. G. A. RULES				BUILDING DESIGNER										(M/F)			
CHORDS		SIZE	LUMBER	DESCR.	BEARINGS										DESIGN CRITERIA		
A - D	2x4	DRY	No.2	SPF	FACTORED					MAXIMUM FACTORED		INPUT	RECORD	SPECIFIED LOADS:			
D - F	2x4	DRY	No.2	SPF	GROSS REACTION					GROSS REACTION		BRG	BRG	TOP CH. LL = 25.8 PSF			
F - H	2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BOT CH. LL = 6.0 PSF				
H - K	2x4	DRY	No.2	SPF	S	2083	0	2083	0	0	5-8	5-8	DL = 0.0 PSF				
K - L	2x4	DRY	No.2	SPF	L	2083	0	2083	0	0	5-8	5-8	DL = 7.4 PSF				
L - J	2x4	DRY	No.2	SPF	UNFACTORED REACTIONS										TOTAL LOAD = 39.0 PSF		
J - S	2x4	DRY	No.2	SPF	1ST CASE										SPACING = 24.0 IN. O.C.		
S - Q	2x4	DRY	No.2	SPF	JT	MAX/MIN. COMPONENT REACTIONS								THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR			
Q - N	2x4	DRY	No.2	SPF	S	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	SMALL BUILDING REQUIREMENTS OF PART 9,				
N - L	2x4	DRY	No.2	SPF	L	1457	989 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0	NBCC 2010, NBCC 2015				
				SPF		1457	989 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0	THIS DESIGN CONFORMS				
ALL WEBS				2x3	DRY	No.2	SPF	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S. L.									
EXCEPT																	
S - C		2x4	DRY	No.2	SPF												
I - L		2x4	DRY	No.2	SPF												

DRY: SEASONED LUMBER.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, A8C 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")

CALCULATED VERT. DEFL.(LL) = L/999 (0.14")

ALLOWABLE DEFL.(TL) = L/360 (1.17")

CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.49/1.00 (G-I), BC=0.53/1.00 (R-S), WB=0.72/1.00 (E-P), SSI=0.22/1.00 (B-C)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (FL)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (C) (INPUT = 0.80)

JSI METAL = 0.77 (I) (INPUT = 1.00)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO				FR-TO			
A-B	0/28	-91.8	-91.8 0.12 (1)	F-O	0/866	0.19 (1)	
B-C	0/22	-91.8	-91.8 0.40 (1)	C-G	-726/0	0.72 (1)	
G-D	-2808/0	-91.8	-91.8 0.49 (1)	G-M	0/391	0.09 (1)	
D-E	-2808/0	-91.8	-91.8 0.49 (1)	M-I	-192/18	0.05 (1)	
E-F	-2322/0	-91.8	-91.8 0.46 (1)	P-F	0/866	0.19 (1)	
F-G	-2322/0	-91.8	-91.8 0.46 (1)	E-P	-726/0	0.72 (1)	
G-H	-2808/0	-91.8	-91.8 0.49 (1)	R-E	0/391	0.09 (1)	
H-I	-2808/0	-91.8	-91.8 0.49 (1)	C-R	-192/18	0.05 (1)	
I-J	0/22	-91.8	-91.8 0.40 (1)	S-C	-3084/0	0.72 (1)	
J-K	0/28	-91.8	-91.8 0.12 (1)	I-L	-3084/0	0.72 (1)	
S-B	-344/0	0.0	0.0 0.03 (1)				
L-J	-344/0	0.0	0.0 0.03 (1)				

S-R	0/2574	-18.5	-18.5 0.53 (1)	10.00
R-Q	0/2291	-18.5	-18.5 0.47 (1)	10.00
Q-P	0/2291	-18.5	-18.5 0.47 (1)	10.00
P-O	0/1782	-18.5	-18.5 0.36 (1)	10.00
O-N	0/2291	-18.5	-18.5 0.47 (1)	10.00
N-M	0/2291	-18.5	-18.5 0.47 (1)	10.00
M-L	0/2574	-18.5	-18.5 0.53 (1)	10.00

Structural component only

DWG# T-2007175

LICENSED PROFESSIONAL ENGINEER

H. J. G. ALVES

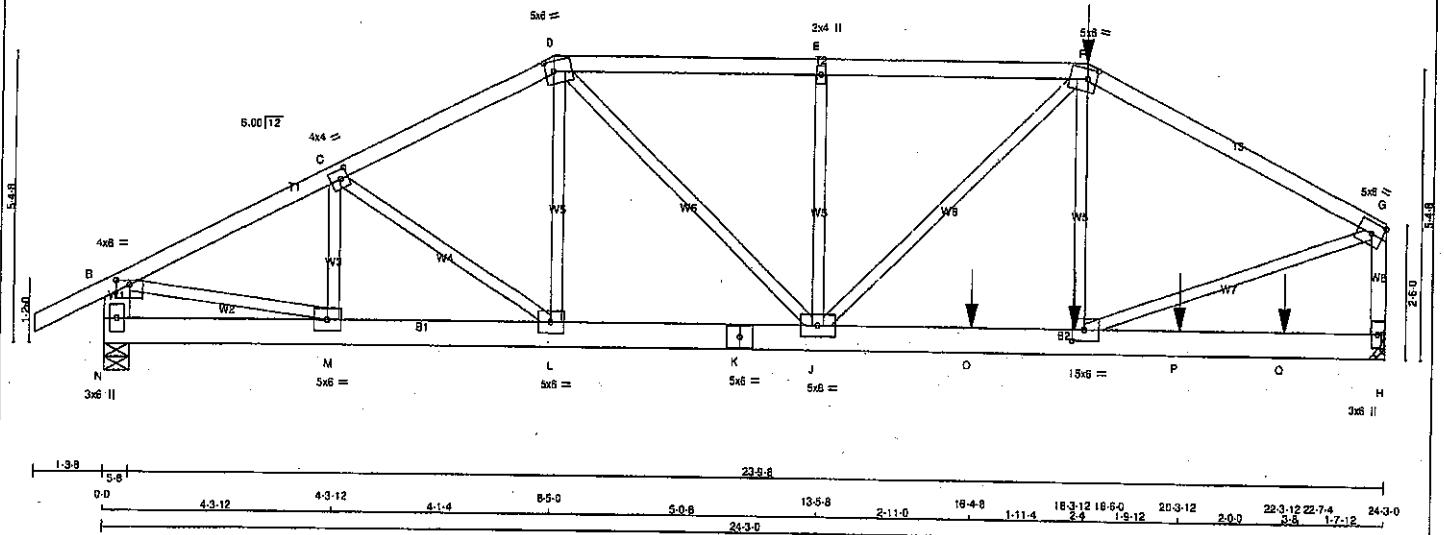
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PROVINCE OF ONTARIO

JOB NAME 408170	TRUSS NAME T47	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Sat Apr 25 13:02:24 2020 Page 1
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13.8 0-0 4-3-12 4-3-12 21-4 85-0 5-0-8 13-5-8 18-6-0 4-1-4 22-7-4 1-7-12 24-3-0
5c90 = 139.1



LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES					
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
N - B	2x6	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
N - K	2x6	DRY	No.2	SPF	
K - H	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1	12	TOP
D-F 1	12	SIDE(61.0)
F-G 1	12	SIDE(61.0)
H-G 1	12	TOP
N-B 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
N-K 2	12	TOP
K-H 2	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UP	IN-SX
N	2120	0	2120	0
H	3119	0	3119	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 4'-0.

UNFACTORED REACTIONS

1ST LOASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL
N	1485 1008 / 0 0 / 0 0 / 0 492 / 0 0 / 0
H	2201 1472 / 0 0 / 0 0 / 0 729 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (CS)	LC1 MAX (CS)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (CS)	LC1 MAX (CS)
FR-TO							FR-TO				
A-B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	10.00	M-C	-408 / 0	0.04 (1)	10.00	10.00
B-C	-2914 / 0	-91.8	-91.8	0.17 (1)	5.22	5.22	C-L	-66 / 0	0.01 (3)	10.00	10.00
C-D	-2892 / 0	-91.8	-91.8	0.17 (1)	5.24	5.24	L-D	0 / 70	0.01 (4)	10.00	10.00
D-E	-3517 / 0	-91.8	-91.8	0.27 (1)	4.74	4.74	D-J	0 / 1336	0.17 (1)	10.00	10.00
E-F	-3517 / 0	-91.8	-91.8	0.27 (1)	4.74	4.74	J-E	-555 / 0	0.12 (1)	10.00	10.00
F-G	-3485 / 0	-91.8	-91.8	0.39 (1)	4.64	4.64	J-F	0 / 575	0.07 (1)	10.00	10.00
N-B	-2084 / 0	0.0	0.0	0.07 (1)	7.81	7.81	I-F	0 / 522	0.06 (1)	10.00	10.00
H-G	-2933 / 0	0.0	0.0	0.20 (1)	8.72	8.72	I-G	0 / 3293	0.41 (1)	10.00	10.00
N-M	0 / 0	-18.5	-18.5	0.03 (1)	10.00	10.00	B-M	0 / 2664	0.33 (1)	10.00	10.00
M-L	0 / 2622	-18.5	-18.5	0.21 (1)	10.00	10.00					
L-K	0 / 2568	-18.5	-18.5	0.33 (1)	10.00	10.00					
K-J	0 / 2568	-18.5	-18.5	0.33 (1)	10.00	10.00					
J-I	0 / 3109	-18.5	-18.5	0.69 (1)	10.00	10.00					
I-P	0 / 3109	-18.5	-18.5	0.69 (1)	10.00	10.00					
P-Q	0 / 0	-18.5	-18.5	0.32 (1)	10.00	10.00					
Q-H	0 / 0	-18.5	-18.5	0.32 (1)	10.00	10.00					

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
F	18-6-0	-178	-178	---	FRONT	VERT	TOTAL	---	C1
I	18-3-12	-35	-35	---	FRONT	VERT	TOTAL	---	C1
O	16-4-8	-1661	-1661	---	FRONT	VERT	TOTAL	---	C1
P	20-3-12	-283	-283	---	FRONT	VERT	TOTAL	---	C1
Q	22-3-12	-284	-284	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 33.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(58 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/899 (0.15")

CSI: TC=0.39/1.00 (F-G:1), SC=0.89/1.00 (I-J:1), WB=0.41/1.00 (G-I:1), SS=0.37/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.87 (I) (INPUT = 0.90)
JSI METAL = 0.44 (G) (INPUT = 1.00)

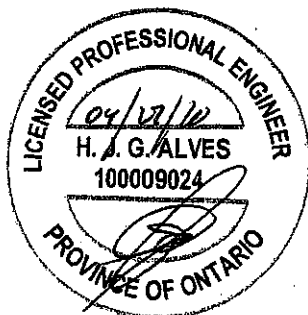
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T47	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mitak Industries, Inc. Set Apr 25 13:02:24 2020 Page 2
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PLATES (table is in inches)

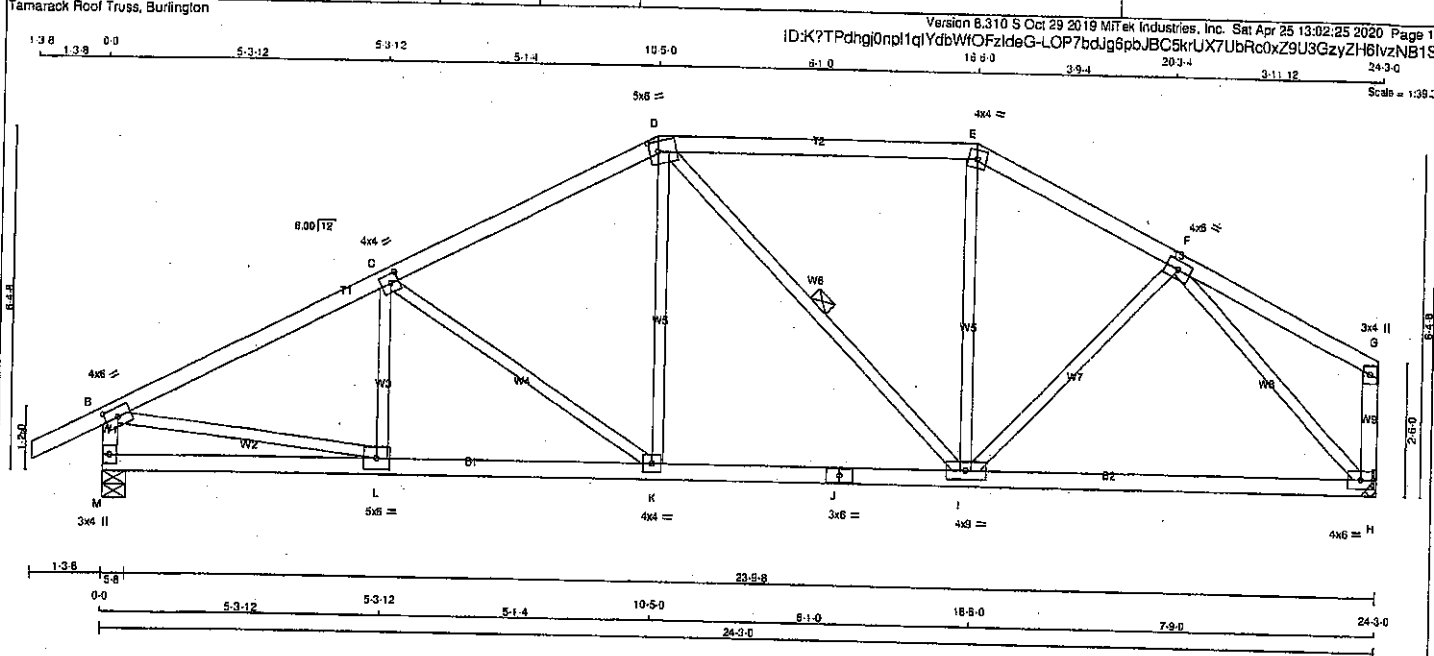
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	5.0	1.00	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-t	MT20	5.0	5.0		Edge
H	BMV1+p	MT20	3.0	6.0		
I	BMVW-t	MT20	5.0	6.0	2.50	2.75
J	BMVWW-t	MT20	5.0	8.0		
K	BS-t	MT20	5.0	6.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMVW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



Structural component only
DWG# T-2007176 112

JOB NAME 408170	TRUSS NAME T48	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER		CHORDS		SIZE		LUMBER		DESCR.	
A - D	2x4	DRY	No.2	SPF					
D - E	2x4	DRY	No.2	SPF					
E - F	2x4	DRY	No.2	SPF					
M - B	2x4	DRY	No.2	SPF					
H - G	2x4	DRY	No.2	SPF					
M - J	2x4	DRY	No.2	SPF					
J - H	2x4	DRY	No.2	SPF					
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF					

DRY: SEASONED LUMBER.

PLATES (table is in inches)		W		LEN		Y		X	
JT	TYPE	PLATES							
B	TMVW-t	MT20	4.0	8.0	2.00	3.00			
C	TMVW-t	MT20	4.0	4.0	2.00	1.75			
D	TTW-m	MT20	5.0	6.0	2.25	2.00			
E	TTW-m	MT20	4.0	4.0					
F	TMVW-t	MT20	4.0	6.0					
G	TMV-p	MT20	3.0	4.0					
H	BMVW-t	MT20	4.0	6.0					
I	BMVW-t	MT20	4.0	6.0					
J	BS-t	MT20	3.0	6.0					
K	BMVW-t	MT20	4.0	4.0					
L	BMVW-t	MT20	5.0	6.0					
M	BMV-t-p	MT20	3.0	4.0					

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
M	1031	890/0	0/0	0/0	0/0	341/0	0/0		
H	945	620/0	0/0	0/0	0/0	325/0	0/0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING		CHORDS		WEBS	
MEMB.		MAX. FACTORED		MAX. FACTORED	
FR-TO		FORCE		FORCE	
(LBS)		(PLF)		(LBS)	
FROM TO		CS1 (LC)		CS1 (LC)	
LENGTH		FR-TO		LENGTH	
FR-TO		FR-TO		FR-TO	
A-B		0/28		0/32	
B-C		-1844/0		-438/0	
C-D		-1488/0		0/394	
D-E		-1181/0		-189/0	
E-F		-1335/0		0/175	
F-G		0/18		0/158	
G-H		-1419/0		0/1692	
H-I		-139/0		-1563/0	
I-J		0/0		0/0	
J-K		0/1871		0/0	
K-L		0/1913		0/0	
L-M		0/1913		0/0	
M-N		0/1089		0/0	

TOTAL LOAD CASES: (4)

CS1: TC=0.46/1.00 (D-E:1), BC=0.36/1.00 (I-K:4), WB=0.82/1.00 (F-H:1), SS1=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLU) (PLU)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 1.00)

TOTAL WEIGHT = 99 lb

Scale = 1:38.0

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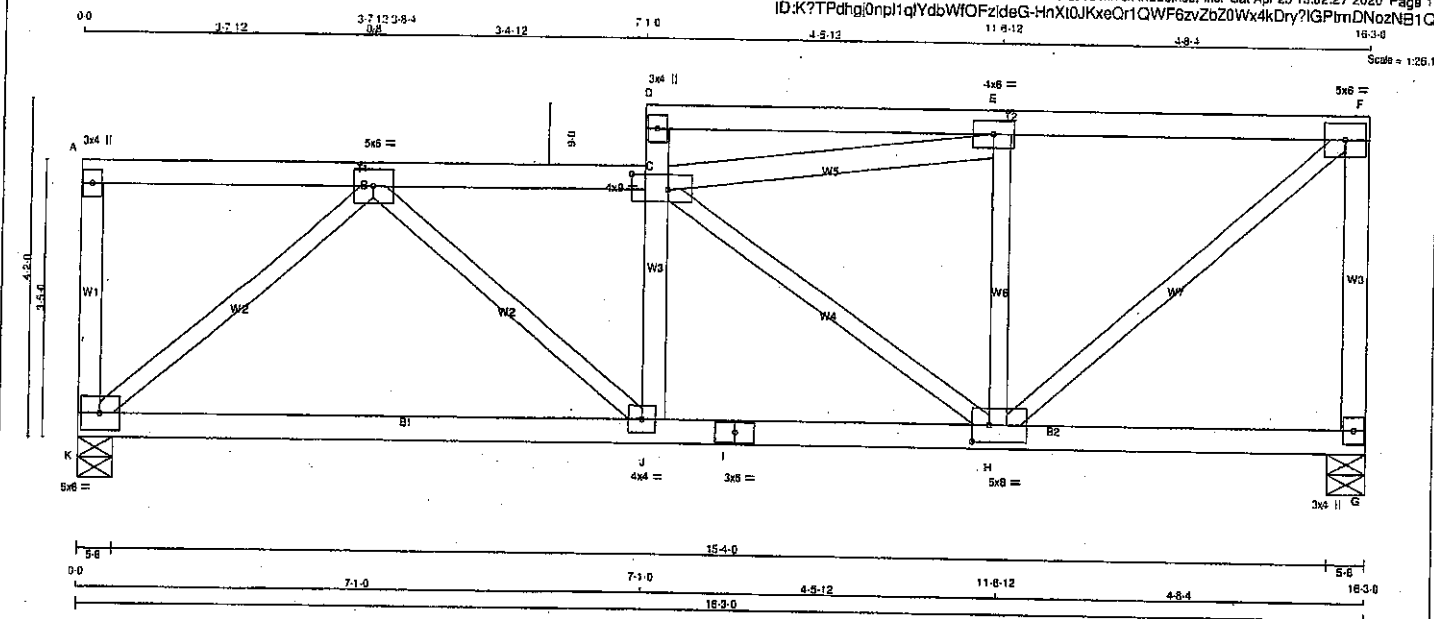
Structural component only
DWG# T-2007177

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

JOB NAME 408170	TRUSS NAME T49	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 72 = 216 lb
[M/R]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
K - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
J - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
C - E	2x4	DRY	No.2	SPF

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMVW-t	MT20	5.0	6.0		
C	TMVWV-t	MT20	4.0	9.0	2.25	5.50
D	TMV+p	MT20	3.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	5.0	8.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVWV-t	MT20	5.0	8.0	2.50	2.50
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMVW-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
	VERT	HORZ	DOWN	HORZ
JT	1079	0	1079	0
K	1079	0	1079	0
G	1079	0	1079	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	780	416 / 0	0 / 0	0 / 0	0 / 0	364 / 0	0 / 0
G	780	416 / 0	0 / 0	0 / 0	0 / 0	364 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
K-A	-163 / 0	0.0	0.0 0.04 (1)	7.81	K-B	-1328 / 0	0.65 (1)
A-B	0 / 0	-114.3	-114.3 0.32 (1)	10.00	B-J	0 / 538	0.18 (1)
B-C	-1416 / 0	-114.3	-114.3 0.37 (1)	4.47	C-E	-961 / 0	0.30 (1)
J-C	-226 / 0	0.0	0.0 0.08 (1)	7.81	C-H	-489 / 0	0.30 (1)
C-D	-183 / 0	0.0	0.0 0.08 (1)	7.81	H-E	-471 / 0	0.18 (1)
D-E	-58 / 0	-114.3	-114.3 0.51 (1)	6.25	H-F	0 / 1303	0.39 (1)
E-F	-1004 / 0	-114.3	-114.3 0.55 (1)	4.81			
G-F	-1039 / 0	0.0	0.0 0.37 (1)	8.91			
K-J	0 / 1013	-18.5	-18.5 0.41 (4)	10.00			
J-I	0 / 1402	-18.5	-18.5 0.48 (4)	10.00			
I-H	0 / 1402	-18.5	-18.5 0.48 (4)	10.00			
H-G	0 / 0	-18.5	-18.5 0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 15.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD	= 48.0	PSF

SPACING = 24.0 IN./C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 085-09, CSA 086-14
- TPIC 2011, TPIC 2014

(85% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.54")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.54")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.11")

CSI: TC=0.55/1.00 (E-F:1), BC=0.48/1.00 (H-J:4), WB=0.85/1.00 (B-C:1), SSI=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

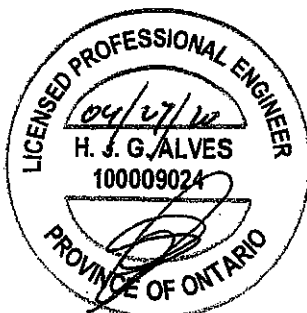
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 788	1987 1656

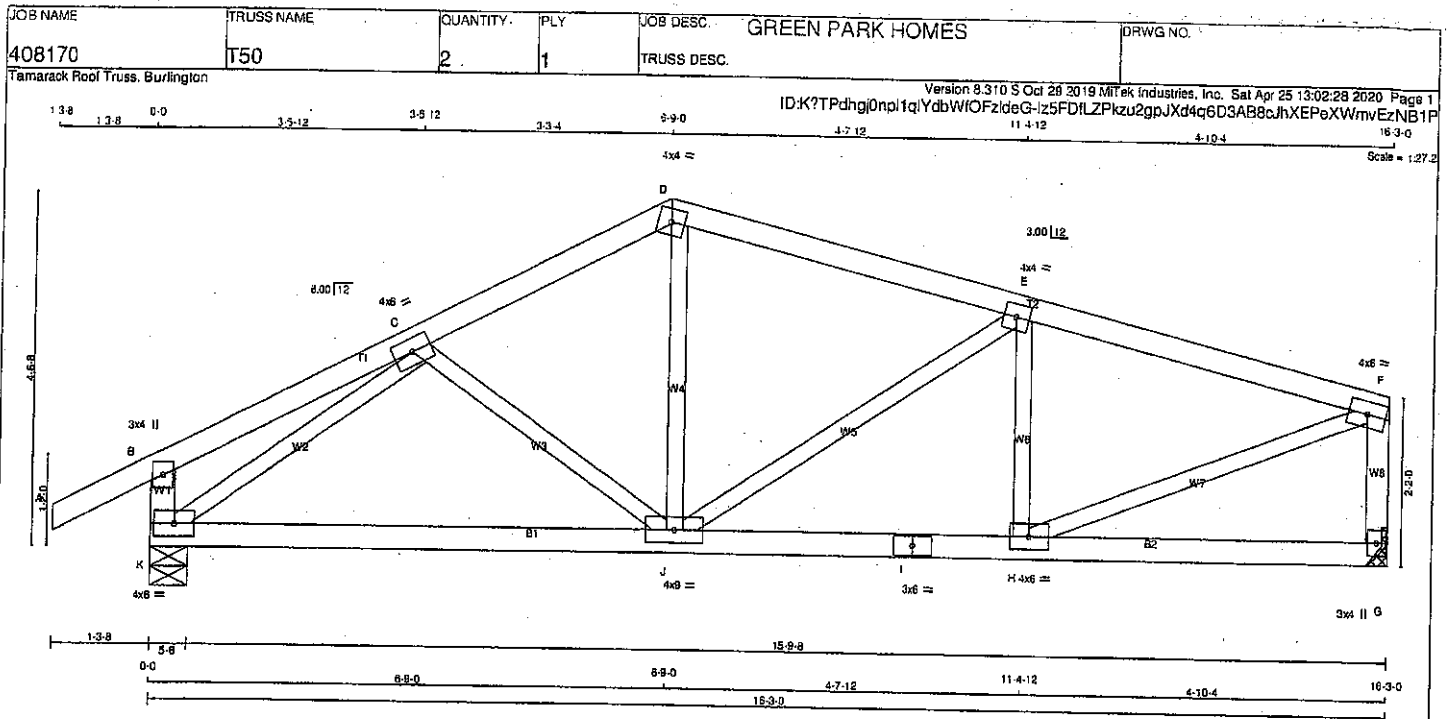
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.82 (I) (INPUT = 1.00)



Structural component only
DWG# T-2007178



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - J	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-1	MT20	4.0	8.0		
D	TTW-1	MT20	4.0	4.0		
E	TMWW-1	MT20	4.0	4.0		
F	TMVW-1	MT20	4.0	6.0		
G	BMV+p	MT20	3.0	4.0		
H	BMVW-1	MT20	4.0	6.0		
I	BS-1	MT20	3.0	6.0		
J	BMWW-1	MT20	4.0	8.0		
K	BMVW-1	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	DOWN	IN-SX	IN-SX
K 1020	0	0	5-8
G 898	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3'-8".

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT COMBINED							
K	719	485 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0
G	633	416 / 0	0 / 0	0 / 0	0 / 0	218 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO				FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	C-J	-135 / 7	0.04 (1)
B-C	0 / 14	-91.8	-91.8 0.15 (1)	10.00	J-D	0 / 355	0.08 (1)
C-D	-502 / 0	-91.8	-91.8 0.13 (1)	6.20	J-E	-263 / 0	0.13 (1)
D-E	-834 / 0	-91.8	-91.8 0.26 (1)	8.18	H-E	-335 / 0	0.07 (1)
E-F	-1094 / 0	-91.8	-91.8 0.27 (1)	5.73	K-C	-1198 / 0	0.35 (1)
K-B	-248 / 0	0.0	0.0 0.03 (1)	7.81	H-F	0 / 1150	0.26 (1)
G-F	-855 / 0	0.0	0.0 0.10 (1)	7.81			
K-J	0 / 856	-18.5	-18.5 0.26 (1)	10.00			
J-I	0 / 1073	-18.5	-18.5 0.27 (1)	10.00			
I-H	0 / 1073	-18.5	-18.5 0.27 (1)	10.00			
H-G	0 / 0	-18.5	-18.5 0.10 (4)	10.00			

TOTAL WEIGHT = 2 X 63 = 126 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.27/1.00 (E-F:1), BC=0.27/1.00 (H-J:1), WB=0.35/1.00 (C-K:1), SS=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

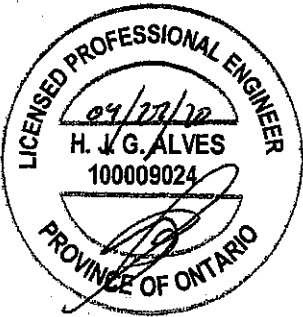
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (H) (INPUT = 0.90)
JSI METAL= 0.32 (I) (INPUT = 1.00)

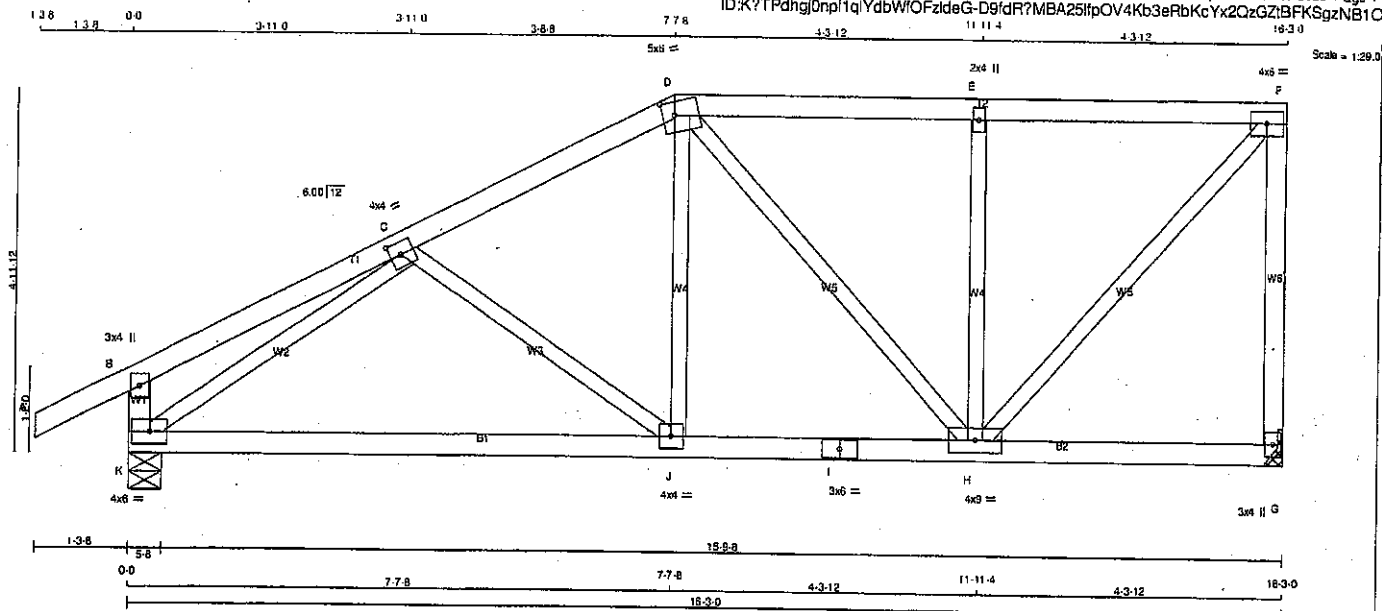


Structural component only
DWG# T-2007179

JOB NAME 408170	TRUSS NAME T51	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 70 lb

LUMBER	SIZE	DRY	LUMBER
N. L. G. A. RULES			
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMWW-t	MT20	4.0	4.0	2.00	1.75
D TTWW-m	MT20	5.0	6.0	2.25	2.00
E TMV+w	MT20	2.0	4.0		
F TMVW-t	MT20	4.0	6.0		
G BMV1+p	MT20	3.0	4.0		
H BMVWW-t	MT20	4.0	9.0		
I BS-t	MT20	3.0	6.0		
J BMVW-t	MT20	4.0	4.0		
K BMVW-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORZ	UPLIFT
G	896	0	896	0
K	1020	0	1020	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS	DEAD	SOIL
G	833	416 / 0	0 / 0	0 / 0	0 / 0
K	719	465 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO					FR-TO		
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	C-J	-215 / 0	0.08 (1)
B-C	0 / 17	-91.8	-91.8 0.20 (1)	10.00	J-D	0 / 266	0.06 (1)
C-D	-901 / 0	-91.8	-91.8 0.22 (1)	6.21	D-H	-210 / 0	0.14 (1)
D-E	-654 / 0	-91.8	-91.8 0.29 (1)	6.25	H-E	-488 / 0	0.18 (1)
E-F	-654 / 0	-91.8	-91.8 0.29 (1)	6.25	H-F	0 / 951	0.22 (1)
F-G	-658 / 0	0.0	0.0 0.35 (1)	7.81	K-C	-1192 / 0	0.43 (1)
G-H	-261 / 0	0.0	0.0 0.03 (1)	7.81			
K-J	0 / 965	-18.5	-18.5 0.30 (4)	10.00			
J-I	0 / 798	-18.5	-18.5 0.30 (4)	10.00			
I-H	0 / 796	-18.5	-18.5 0.30 (4)	10.00			
H-G	0 / 0	-18.5	-18.5 0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.54")
CALCULATED VERT. DEFL. (LL) = L/939 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.54")
CALCULATED VERT. DEFL. (TL) = L/939 (0.12")

CS1: TC=0.35/1.00 (F-G:1), BC=0.30/1.00 (J-K:4), WB=0.43/1.00 (C-K:1), SS=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.86 (C) (INPUT = 0.90)
JSI METAL = 0.40 (C) (INPUT = 1.00)

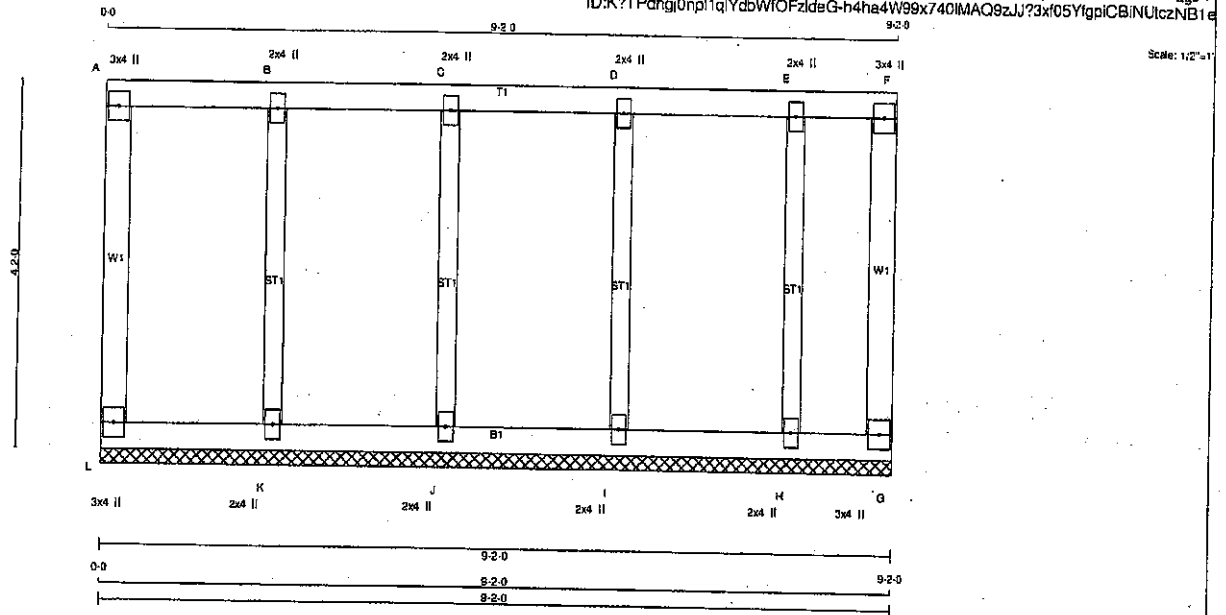


Structural component only
DWG# T-2007180

JOB NAME 408170	TRUSS NAME G52	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
L - A	2x4	DRY	No.2
A - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y
A	TMV+p	MT20	3.0	4.0	
B, C, D, E					
B	TMV+w	MT20	2.0	4.0	
F	TMV+p	MT20	3.0	4.0	
G	BMV+p	MT20	3.0	4.0	
H, I, J, K					
H	BMV+w	MT20	2.0	4.0	
L	BMV+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO	LENGTH	FR-TO				
L-A	-100 / 0	0.0	0.02 (1)	7.81	K-B	-245 / 0	0.09 (1)	
A-B	-4 / 0	-114.3	-114.3	0.08 (1)	10.00	J-C	-224 / 0	0.08 (1)
B-C	-4 / 0	-114.3	-114.3	0.08 (1)	10.00	I-D	-238 / 0	0.08 (1)
C-D	-4 / 0	-114.3	-114.3	0.07 (1)	10.00	H-E	-186 / 0	0.06 (1)
D-E	-4 / 0	-114.3	-114.3	0.07 (1)	10.00			
E-F	-4 / 0	-114.3	-114.3	0.05 (1)	10.00			
G-F	-54 / 0	0.0	0.02 (1)	7.81				
L-K	0 / 4	-18.5	-18.5	0.02 (4)	10.00			
K-J	0 / 4	-18.5	-18.5	0.02 (4)	10.00			
J-I	0 / 4	-18.5	-18.5	0.02 (4)	10.00			
I-H	0 / 4	-18.5	-18.5	0.02 (4)	10.00			
H-G	0 / 4	-18.5	-18.5	0.01 (4)	10.00			

TOTAL WEIGHT = 41 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 15.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 48.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (B-C:1), BC=0.02/1.00 (J-K:4), WB=0.09/1.00 (B-K:1), SSL=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

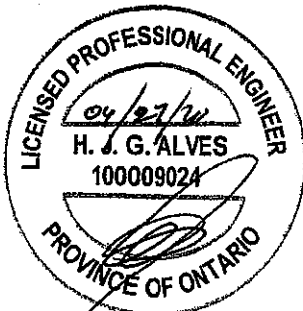
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1668

PLATE PLACEMENT TOL. = 0.250 inches

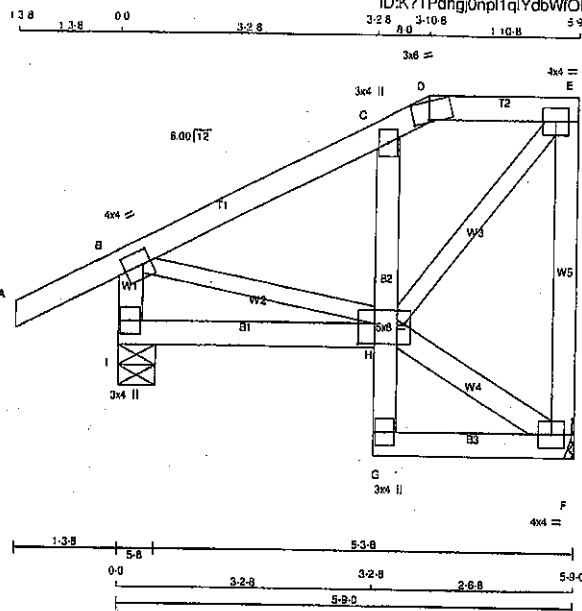
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.07 (K) (INPUT = 1.00)



Structural component only
DWG# T-2007166

JOB NAME 408170	TRUSS NAME T53S	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER				DESCR.
N. L. G. A. RULES	SIZE	LUMBER		
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
G - C	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
B - H	2x3	DRY	No.2	SPF
H - E	2x3	DRY	No.2	SPF

PLATES (tablets in inches)					
JT	TYPE	PLATES	W	LEN	X
B	TMVW-1	MT20	4.0	4.0	2.00 1.25
C	TMV+p	MT20	3.0	4.0	
D	TF-m	MT20	3.0	8.0	0.50 2.50
E	TMVW-1	MT20	4.0	4.0	
F	BMVW1-t	MT20	4.0	4.0	
G	BMV+p	MT20	3.0	4.0	
H	BMVWW-1	MT20	5.0	8.0	3.00 2.50
I	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	HORIZ	DOWN	HORIZ	BRG	IN-SX	BRG	IN-SX
F	302	0	0	302	0	0	MECHANICAL	0	0
I	457	0	0	457	0	0	5-8	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	214	138 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0
I	320	225 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	H-F	-10 / 0	0.00 (1)
B-C	-217 / 0	-91.8 -91.8	0.14 (1)	6.25	B-H	0 / 153	0.03 (1)
C-D	-200 / 0	-91.8 -91.8	0.05 (1)	6.25	H-E	0 / 247	0.08 (1)
D-E	-167 / 0	-91.8 -91.8	0.03 (1)	6.25			
F-E	-273 / 0	0.0 0.0	0.03 (1)	7.81			
I-B	-420 / 0	0.0 0.0	0.04 (4)	7.81			
F-H	0 / 41	-18.5 -18.5	0.05 (4)	10.00			
G-H	0 / 25	0.0 0.0	0.02 (1)	10.00			
H-C	-168 / 0	0.0 0.0	0.04 (1)	7.81			
G-F	0 / 16	-18.5 -18.5	0.02 (4)	10.00			

TOTAL WEIGHT = 33 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA C88-09, CSA C88-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (B-C:1), BC=0.05/1.00 (H-I:4), WB=0.09/1.00 (E-H:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

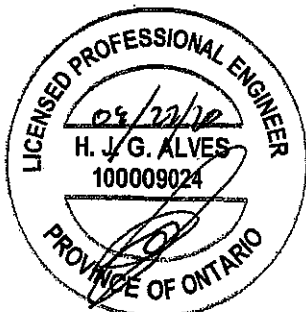
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1667 788 1987 1658	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.36 (H) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

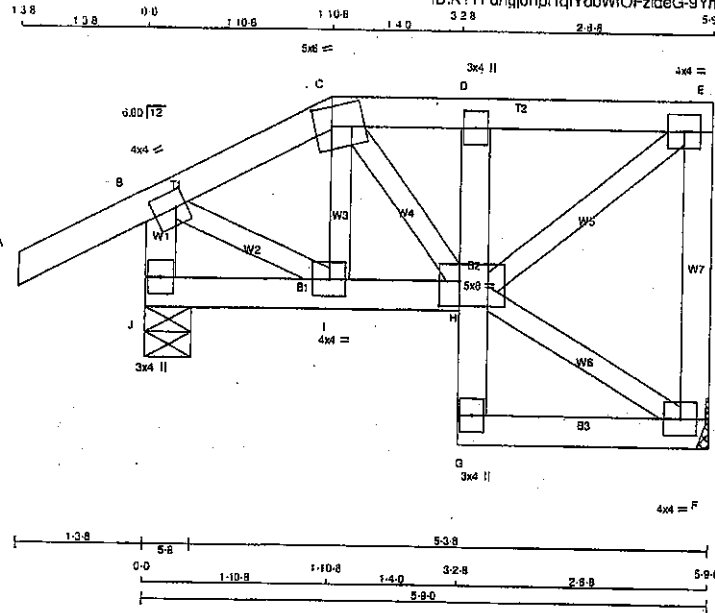


Structural component only
DWG# T-2007181

JOB NAME 408170	TRUSS NAME T54S	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/2" = 1'-0"

TOTAL WEIGHT = 31 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2	
C - E	2x4	DRY	No.2	
F - E	2x4	DRY	No.2	
J - B	2x4	DRY	No.2	
J - H	2x4	DRY	No.2	
G - D	2x4	DRY	No.2	
G - F	2x4	DRY	No.2	

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMVW-p	MT20	3.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMVW-p	MT20	3.0	4.0		
H	BVMWWW-t	MT20	5.0	8.0	3.00	2.50
I	BMVW-t	MT20	4.0	4.0		
J	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	GROSS REACTION	DOWN	HORIZ	BRG	BRG	IN-SX	IN-SX
F	302	0	302	0	0	MECHANICAL			
J	456	0	456	0	0	5-8	5-8		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1'-8".

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
F	214	139 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0
J	320	225 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	I-C	-50 / 10	0.01 (1)
B-C	-240 / 0	-91.8	-91.8 0.12 (1)	6.25	C-H	0 / 84	0.02 (1)
C-D	-246 / 0	-91.8	-91.8 0.05 (1)	6.25	H-F	-9 / 0	0.00 (1)
D-E	-241 / 0	-91.8	-91.8 0.07 (1)	6.25	H-E	0 / 302	0.07 (1)
F-E	-276 / 0	0.0	0.0 0.05 (1)	7.81	B-I	0 / 214	0.05 (1)
J-B	-439 / 0	0.0	0.0 0.04 (1)	7.81			
J-I	0 / 0	-18.5	-18.5 0.02 (4)	10.00			
I-H	0 / 193	-18.5	-18.5 0.04 (1)	10.00			
G-H	0 / 25	0.0	0.0 0.02 (1)	10.00			
H-D	-219 / 0	0.0	0.0 0.01 (1)	7.81			
G-F	0 / 8	-18.5	-18.5 0.03 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TO = 0.12/1.00 (A-B:1), BC = 0.04/1.00 (H-I:1),
WB = 0.07/1.00 (E-H:1), SS = 0.10/1.00 (D-E:1)

DOL LUMBER = 1.00 NAIL = 1.00 LS BEND = 1.10
COMP = 1.10 SHEAR = 1.10 TENS = 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

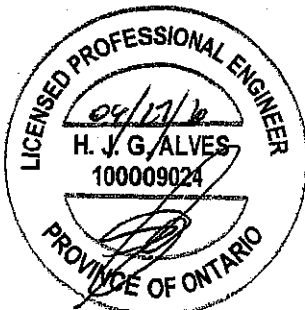
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1657 788 1987 1656	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.33 (B) (INPUT = 0.90)
JSI METAL = 0.13 (B) (INPUT = 1.00)



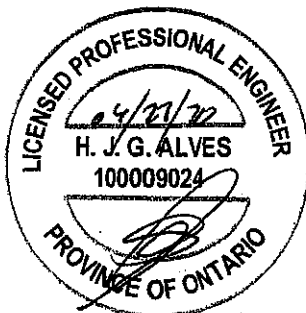
Structural component only
DWG# T-2007182

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC	GREEN PARK HOMES	DWG NO.
408170	T55	1	2	TRUSS DESC.		

Tamareck Roof Truss, Burlington

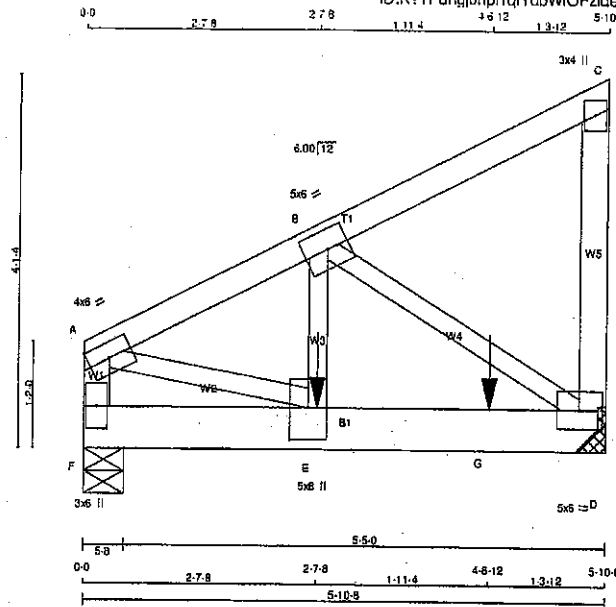
Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 13:02:32 2020 Page 2
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PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
F BMV1+p MT20 3.0 6.0



Structural component only
DWG# T-2007183

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T56	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



TOTAL WEIGHT = 2 X 29 = 57 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY, SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
F-A	(0.122"x3") SPIRAL NAILS	TOP
A-C	12	TOP
C-D	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
F-D	2	SIDE(549.3)
WEBS: (0.122"x3") SPIRAL NAILS		
B-E	1	SIDE(259.5)
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TM/VW-t	MT20	4.0	6.0		Edge
B	TM/VW-t	MT20	5.0	6.0	2.50	2.75
C	TM/V-p	MT20	3.0	4.0		
D	BM/VW-t	MT20	5.0	6.0		
E	BM/VW-t	MT20	5.0	6.0	4.25	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	2334	0	2334	0
D	2733	0	2733	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4'-0.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
F	1846	1103 / 0	0 / 0
D	1929	1290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)	UNBRAC	MEMB.	MAX. FACTORED	WEBS	MAX. FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)
FR-TO															
F-A	-2316 / 0	0.0	0.0	0.13 (1)	7.37	A-E	0 / 2898	0.36 (1)							
A-B	-3101 / 0	-91.8	-91.8	0.08 (1)	5.20	E-B	0 / 2880	0.36 (1)							
B-C	-5 / 0	-91.8	-91.8	0.08 (1)	10.00	B-D	-3303 / 0	0.42 (1)							
D-C	-138 / 0	0.0	0.0	0.02 (1)	7.81										
F-E	0 / 0	-18.5	-18.5	0.01 (4)	10.00										
E-G	0 / 2779	-18.5	-18.5	0.57 (1)	10.00										
G-D	0 / 2779	-18.5	-18.5	0.57 (1)	10.00										

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-7-8	-3101	-3101	---	BACK	VERT	TOTAL	---	C1
G	4-6-12	-1318	-1318	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.8	PSF
	DL	= 8.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(56 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CS1: TC=0.13/1.00 (A-F:1), BC=0.57/1.00 (D-E:1), WB=0.42/1.00 (B-D:1), SS=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (E) (INPUT = 0.90)
JSI METAL = 0.50 (E) (INPUT = 1.00)



Structural component only
DWG# T-2007184 1/2

CONTINUED ON PAGE 2

JOB NAME 408170	TRUSS NAME T56	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
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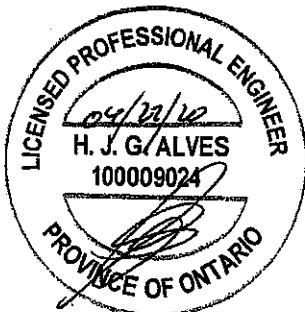
Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Sat Apr 25 13:02:33 2020 Page 2
ID:K?TPchq0npl1qlydbWIOFzdeG-8wu8GMPIDGbA8RiGJAq?pHm499FsMnS8opDXbRzNB1K

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

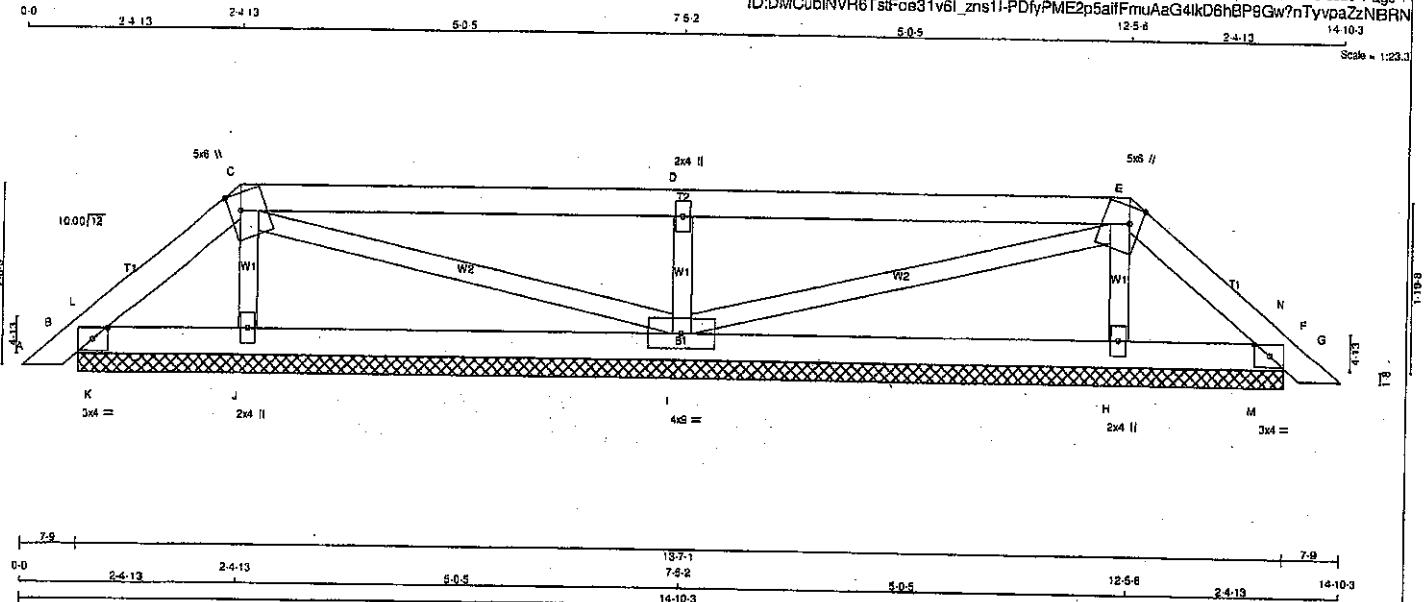


Structural component only
DWG# T-2007184 3/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC	GREEN PARK HOMES	DRWG NO.
408168	PB1	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6L_zns11-PDfyPME2p5aifFmuAaG4lkD6hBP8Gw?nTyypaZzNBRN



LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES	A - C	2x4	DRY	No.2	SPF
	C - D	2x4	DRY	No.2	SPF
	D - E	2x4	DRY	No.2	SPF
	E - F	2x4	DRY	No.2	SPF
	ALL WEBS	2x3	DRY	No.2	SPF
			DRY: SEASONED LUMBER.		

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
	B	TMB1-I	MT20	3.0	4.0	1.50	2.00
	C	TTWW+m	MT20	5.0	6.0	2.25	1.50
	D	TTWW+m	MT20	2.0	4.0		
	E	TTWW+m	MT20	5.0	6.0	2.25	1.50
	F	TMB1-I	MT20	3.0	4.0	1.50	2.00
	H	BMW1+w	MT20	2.0	4.0		
	I	BMWW1-I	MT20	4.0	9.0		
	J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	RECORD
	GROSS REACTION	DOWN	GROSS REACTION	DOWN		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
B	169	0	169	0	0	13-7-1
F	169	0	169	0	0	13-7-1
J	288	0	288	0	0	13-7-1
I	683	0	683	0	0	13-7-1
H	288	0	288	0	0	13-7-1

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	116	93/0	0/0	0/0	0/0	23/0	0/0
F	116	93/0	0/0	0/0	0/0	23/0	0/0
J	207	118/0	0/0	0/0	0/0	99/0	0/0
I	481	327/0	0/0	0/0	0/0	154/0	0/0
H	207	118/0	0/0	0/0	0/0	99/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO						
A-B	0/14	-91.8	-91.8 0.02 (1)	10.00	J-C	-194/0 0.03 (1)
B-L	-31/4	-91.8	-91.8 0.01 (1)	6.25	C-I	-22/0 0.01 (1)
L-C	-60/0	-91.8	-91.8 0.03 (1)	6.25	I-D	-578/0 0.08 (1)
C-D	-8/0	-91.8	-91.8 0.39 (1)	10.00	D-E	-22/0 0.01 (1)
D-E	-8/0	-91.8	-91.8 0.39 (1)	10.00	E-H	-194/0 0.03 (1)
E-N	-60/0	-91.8	-91.8 0.03 (1)	6.25	H-K	-121/0 0.00 (1)
N-F	-31/4	-91.8	-91.8 0.01 (1)	6.25	M-N	-121/0 0.00 (1)
F-G	0/14	-91.8	-91.8 0.02 (1)	10.00		
B-K	0/42	-18.5	-18.5 0.04 (1)	10.00		
K-J	0/42	-18.5	-18.5 0.07 (4)	10.00		
J-I	0/29	-18.5	-18.5 0.10 (4)	10.00		
I-H	0/29	-18.5	-18.5 0.10 (4)	10.00		
H-M	0/42	-18.5	-18.5 0.07 (4)	10.00		
M-F	0/42	-18.5	-18.5 0.04 (1)	10.00		

TOTAL WEIGHT = 3 X 45 = 136 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(56 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.39/1.00 (C-D:1), BC=0.10/1.00 (H-I:4), WB=0.08/1.00 (D-I:1), SS=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

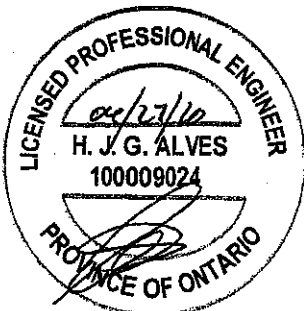
PLATE GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX	MIN	MAX
618	354	1667
788	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

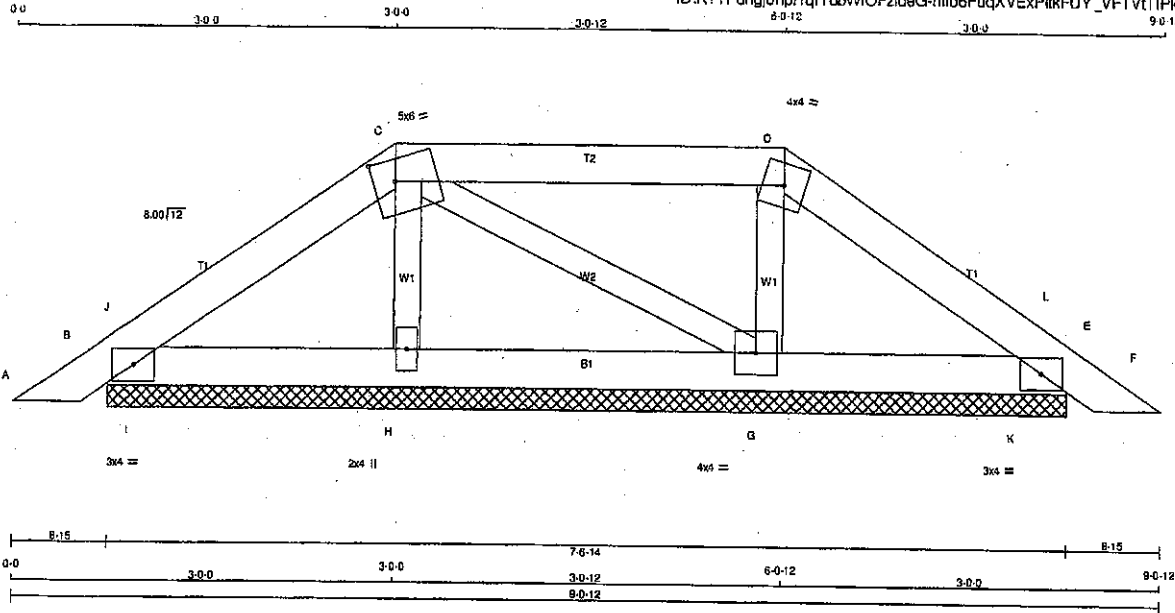
JSI GRIP = 0.30 (D) (INPUT = 0.90)

JSI METAL = 0.12 (D) (INPUT = 1.00)



Structural component only
DWG# T-2007128

JOB NAME 408169	TRUSS NAME PB20	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 12:49:22 2020 Page 1 ID:K7TPghg0np1q1YdbWIOFzIdG-mlb6FuqXVExPltkFUY_VFTVtTIPkW7jvF6AJozNBDH	



LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS	2x4	DRY	No.2
A - C	2x4	DRY	No.2
D - F	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW-m	MT20	5.0	6.0	2.00 2.00
D	TTW-m	MT20	4.0	4.0	
E	TMB1-I	MT20	3.0	4.0	
G	BMW1-I	MT20	4.0	4.0	
H	BMW1-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
B	221	0	221	0	0	7-6-14	7-6-14	7-6-14	7-6-14
E	209	0	209	0	0	7-6-14	7-6-14	7-6-14	7-6-14
H	248	0	248	0	0	7-6-14	7-6-14	7-6-14	7-6-14
G	280	0	280	0	0	7-6-14	7-6-14	7-6-14	7-6-14

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	DEAD	SOIL
B	154	113 / 0	0 / 0	0 / 0	0 / 0	41 / 0	0 / 0	0 / 0	0 / 0
E	146	106 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0	0 / 0	0 / 0
H	176	109 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0	0 / 0	0 / 0
G	198	127 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	FORCE (LBS)	MAX. CS1 (LC)	MAX. CS1 (LC)
FR-TO		FROM TO	LENGTH	FR-TO		LENGTH	
A-B	0 / 15	-91.8	-91.8 0.03 (1)	10.00	H-C	-178 / 0	0.03 (1)
B-J	-63 / 0	-91.8	-91.8 0.01 (1)	8.25	C-G	-21 / 0	0.03 (1)
J-C	-74 / 0	-91.8	-91.8 0.05 (1)	8.25	G-D	-188 / 0	0.03 (1)
C-D	-30 / 0	-91.8	-91.8 0.15 (1)	8.25	I-J	-121 / 0	0.00 (1)
D-L	-54 / 0	-91.8	-91.8 0.05 (1)	6.25	K-L	-123 / 0	0.00 (1)
L-E	-41 / 0	-91.8	-91.8 0.01 (1)	6.25			
E-F	0 / 15	-91.8	-91.8 0.03 (1)	10.00			
B-I	0 / 60	-18.5	-18.5 0.06 (1)	10.00			
I-H	0 / 60	-18.5	-18.5 0.06 (1)	10.00			
H-G	0 / 48	-18.5	-18.5 0.03 (1)	10.00			
G-K	0 / 43	-18.5	-18.5 0.06 (1)	10.00			
K-E	0 / 43	-18.5	-18.5 0.06 (1)	10.00			

TOTAL WEIGHT = 3 X 25 = 75 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 9.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.15/1.00 (C-D:1), BC=0.08/1.00 (B-I:1), WB=0.03/1.00 (D-G:1), SS1=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

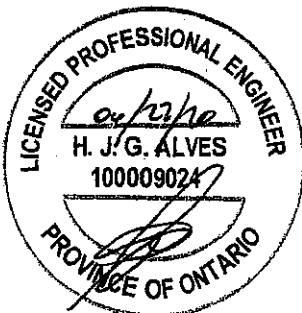
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1867 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



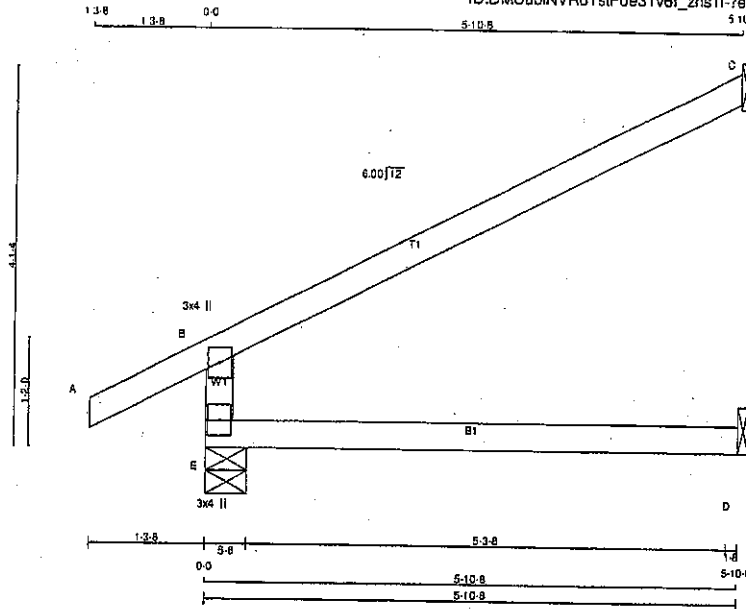
Structural component only
DWG# T-2007144

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408168	J1	19	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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Scale = 1:22.9



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
E BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	525	0	525	0	5-8	5-8
C	202	0	202	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE				
E	389	257 / 0	0 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	38	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	VERT. LOAD (PLF)	MAX. CS (LC)			MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
FR-TO								
E-B	-461 / 0	0.0	0.0	0.13 (4)	7.81			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-30 / 0	-91.8	-91.8	0.54 (1)	6.25			
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

TOTAL WEIGHT = 19 X 17 = 319 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TRC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
	1887	788
	1987	1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



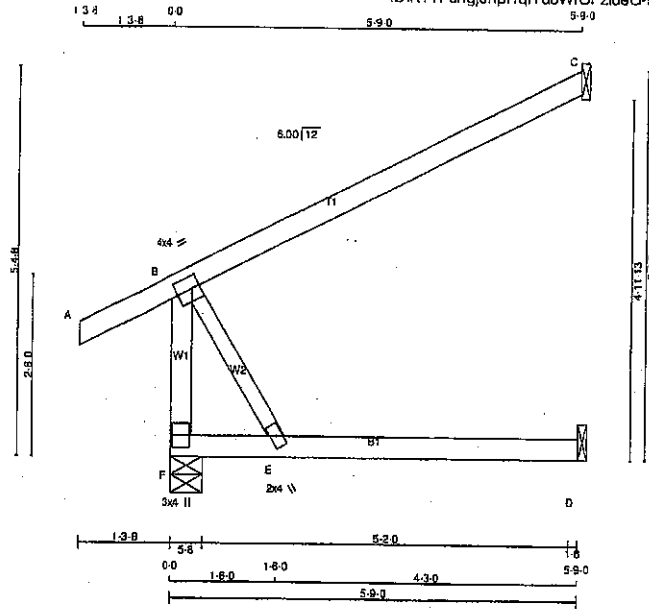
Structural component only
DWG# T-2007126

JOB NAME 408170	TRUSS NAME J40	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:20.2



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMW+1	MT20	4.0	4.0	2.00	1.25
E BMW+1	MT20	2.0	4.0		
F BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	441	0	441	0	0	5-8	5-8
C	284	0	284	0	0	1-8	1-8
D	53	0	60	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN COMPONENT REACTIONS					
		COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
F	310	217 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0
C	182	147 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LC)	
FR-TO		FROM TO			FR-TO			
F-B	-388 / 0	0.0	3.0	0.05 (1)	7.81	0 / 0	0.00 (1)	
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B-C	0 / 0	-91.8	-91.8	0.52 (1)	10.00			
F-E	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
E-D	0 / 0	-18.5	-18.5	0.18 (4)	10.00			

TOTAL WEIGHT = 20 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
BOT CH.	LL =	6.0	PSF
	DL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.05")

CSI: TC=0.52/1.00 (B-C:1), BC=0.18/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SS=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

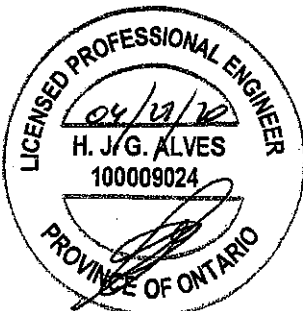
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	518 354	1567 788

PLATE PLACEMENT TOL. = 0.250 inches

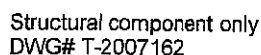
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.24 (B) (INPUT = 0.50)
JSI METAL = 0.07 (B) (INPUT = 1.00)



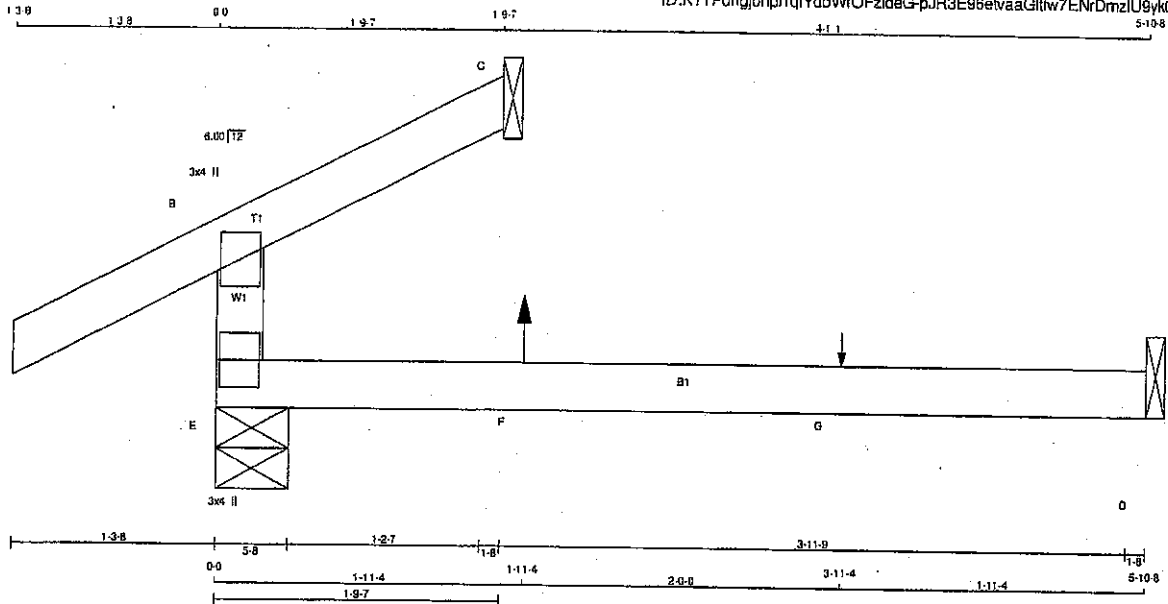
Structural component only
DWG# T-2007167

Scale = 1:17.9



JOB NAME 408170	TRUSS NAME C41	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 13:02:09 2020 Page 1
ID:K7TPchg0npl1q1YdbWfOFzldG-pJR3E96etvaaGltiw7ENrDmzIU9yK0tG4PGkrzNB1j



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER
E - B	2x4	DRY No.2
A - C	2x4	DRY No.2
E - D	2x4	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	284	0	284	0	5-8	5-8
C	63	0	63	0	1-8	1-8
D	44	0	52	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
E	200	137/0	0/0	0/0	0/0	62/0	0/0
C	46	21/0	0/0	0/0	0/0	25/0	0/0
D	35	0/-3	0/0	0/0	0/0	37/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO				
E-B	-227/0	0.0	0.0	0.11 (4)
A-B	0/28	-91.8	-91.8	0.12 (1)
B-C	-9/9	-91.8	-91.8	0.08 (4)
E-F	0/0	-18.5	-18.5	0.14 (4)
F-G	0/0	-18.5	-18.5	0.14 (4)
G-D	0/0	-18.5	-18.5	0.14 (4)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	BACK	VERT	TOTAL	-	C1
G	3-11-4	1	1	-	BACK	VERT	TOTAL	-	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT. CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.04")

CSI: TC=0.12/1.00 (A-B:1), BC=0.14/1.00 (D-E:1),
WB=0.09/1.00 (n/a:0), SS=0.09/1.00 (A-B:1)

DOL LUMBER=0.98 NAIL=0.98 LS @END=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

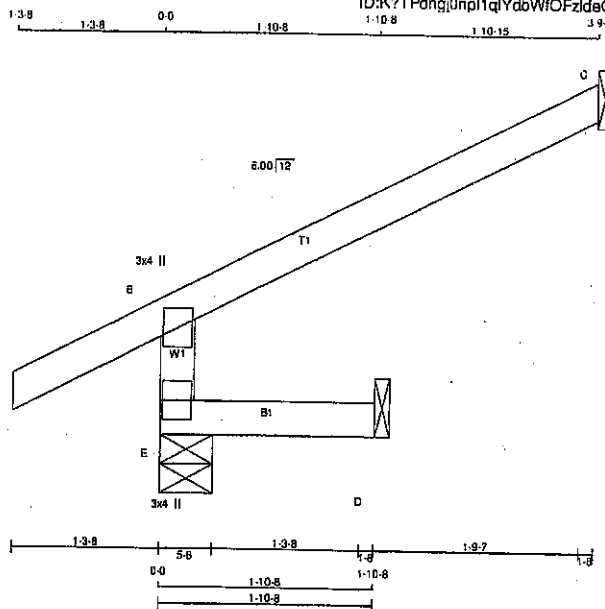


Structural component only
DWG# T-2007163

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	C42	3	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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ID:K7TPdhg0npl1qYdbWfOFzIdaG-HV7SRV7GcCfRuvR7TricNQJ6SuW4TTHmVkBpHhZNB1h



LUMBER

N. L. G. A. RULES	SIZE	LUMBER
CHORDS	2x4	DRY No.2
E - B	2x4	DRY No.2
A - C	2x4	DRY No.2
E - D	2x4	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
E	361	361	0	5-8
C	130	130	0	1-8
D	16	17	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX MIN COMPONENT REACTIONS	PERM LIVE	WIND	DEAD	SOIL
E	250	190 / 0	0 / 0	0 / 0	80 / 0	0 / 0
C	90	73 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB. MAX. FACTORED FORCE (LBS)
E-B	-342 / 0	0.0	0.0	0.01 (4)	7.81
A-B	0 / 28	-91.8	-91.8	0.13 (5)	10.00
B-C	-19 / 0	-91.8	-91.8	0.22 (1)	6.25
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 10 = 29 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/989 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/989 (0.00")

CSI: TC=0.22/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (A-B:0), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI)	(PLJ)	(PLI)
MAX MIN MAX MIN MAX MIN		
MT20	618	354
	1667	786
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

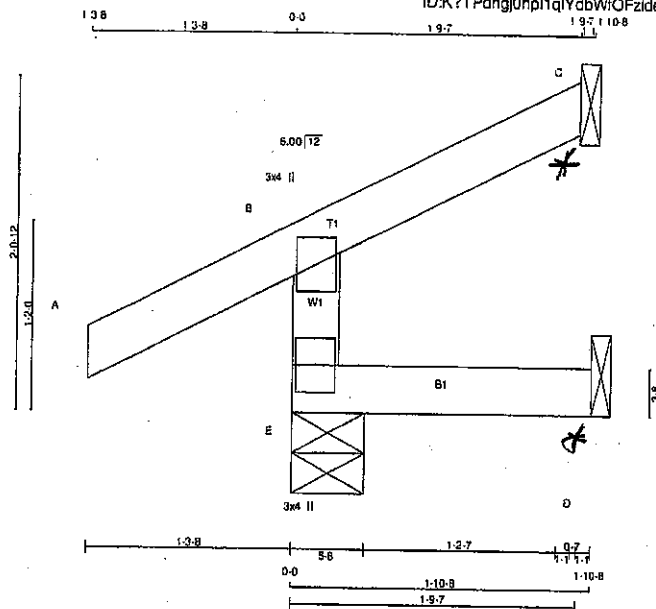


Structural component only
DWG# T-2007164

Tamarack Roof Truss, Burlington

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Scale = 1:13.1



TOTAL WEIGHT = $3 \times 7 = 21$ lb

LUMBER

COMBEN

N. L. G. A. RULES

CHORDS		SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ UPLIFT	IN-SX	IN-SX
E	271	0	271	0	5-8	5-8
C	45	0	45	0	-23	1-8
D	8	0	17	0	-2	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	188	141/0	0/0	0/0	0/0	47/0	0/0
C	31	24/-18	0/0	0/0	0/0	7/0	0/0
D	7	0/-8	0/0	0/0	0/0	12/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E. C.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBB			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LF)	MAX. FACTORED LCL (LC)	MAX. UNBRAC LCL (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LCL (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
E-B	-244 / 0	0.0	0.0	0.04 (S)	7.81		
A-B	0 / 28	-91.8	-91.8	0.12 (I)	10.00		
B-C	-17 / 0	-91.8	-91.8	0.08 (I)	6.25		
E-D	0 / 0	-18.5	-18.5	0.04 (S)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD =		39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL)= $L/380$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5),
WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

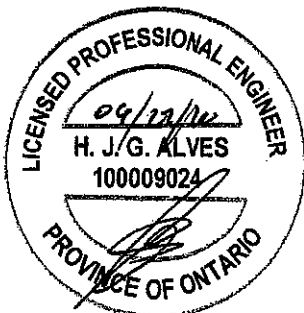
NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007165

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

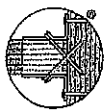
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

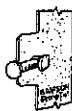
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options Information on p. 126.



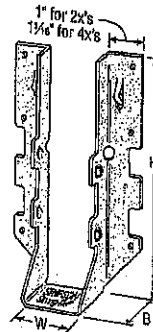
Double-Shear
Nailing
Top View



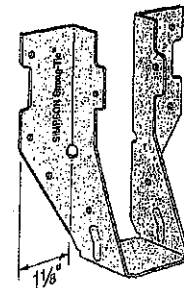
Double-Shear
Nailing
Side View;
Do not
bend tab



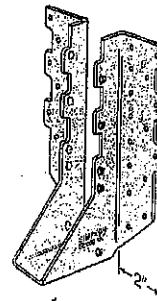
Dome Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580



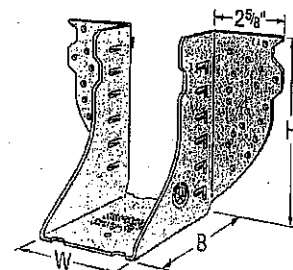
LUS28



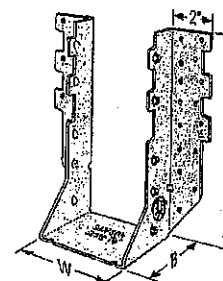
LU26L



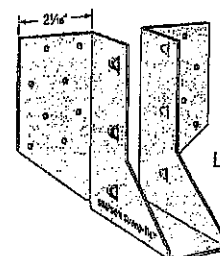
HUS210
(HUS26, HUS28,
and HHUS similar)



HGUS28-2

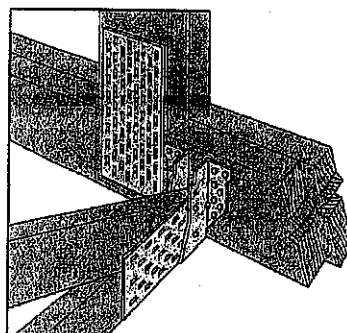


HHUS210-2



LJS26DS

Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS**HHUS/HGUS**

See Hanger Options Information on pp. 125–127.

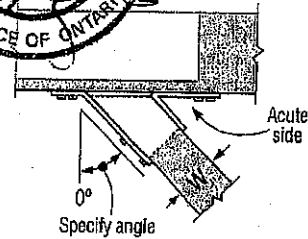
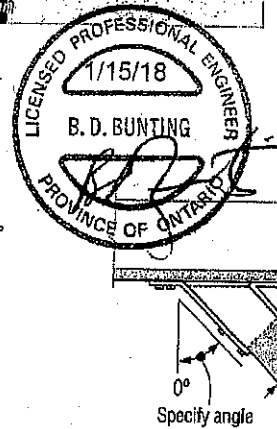
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
$W < 2"$	Bevel or square cut	0.62 of table value	0.46 of table value
$2" < W < 6"$	Bevel cut	0.67 of table value	0.41 of table value
$2" < W < 6"$	Square cut	0.46 of table value	0.41 of table value
$W > 6"$	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger
Skewed Right
(joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance				
		W	H	B	d _g ³	Header	Joist	D.Fir-L		S-P-F		
								Uplift	Normal	Uplift	Normal	
								(K _D = 1.15) lb.	(K _D = 1.00) lb.	(K _D = 1.15) lb.	(K _D = 1.00) lb.	
Single 2x Sizes												
■	LUS24	18	1 1/8	3 1/2	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155
	LU24L	22	1 1/8	3	1 1/4	2 1/4	(4) 10d	(2) 10d x 1 1/2"	360	723	2.87	5.14
■	LU26L	22	1 1/8	5	1 1/4	4 1/4	(8) 10d	(4) 10d x 1 1/2"	360	1020	320	725
	LUS26	18	1 1/8	4 1/4	1 1/4	3 1/4	(4) 10d	(4) 10d	1.60	4.54	1.42	3.22
■	HUS26	16	1 1/8	5 1/4	3	3 1/4	(14) 16d	(6) 16d	720	1605	645	1140
	LJS26DS	18	1 1/8	5	3 1/4	4 1/4	(16) 16d	(8) 16d	3.20	7.14	2.87	5.07
■	HGUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 16d	(8) 16d	1420	2170	1290	1630
	LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"	8.32	9.65	5.74	7.25
■	LUS28	18	1 1/8	6 1/4	1 1/4	3 1/4	(6) 10d	(4) 10d	2705	4940	2065	3875
	HUS28	16	1 1/8	7 1/4	3	6 1/4	(22) 16d	(8) 16d	11.30	21.97	9.20	17.24
■	HGUS28	12	1 1/8	7 1/4	5	6 1/4	(36) 16d	(12) 16d	2055	4265	1460	4115
	LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(6) 10d x 1 1/2"	9.14	18.97	6.49	18.31
■	LUS210	18	1 1/8	7 1/4	1 1/4	3 1/4	(8) 10d	(4) 10d	2685	6625	2685	5700
									11.96	29.51	11.96	25.35
■									1140	2185	1020	1550
									5.07	9.72	4.54	8.89
■									1420	2520	1290	1790
									6.32	11.21	5.74	7.96
■									3605	5365	2675	4345
									16.04	23.86	11.90	19.33
■									3310	7675	3310	6900
									14.74	34.19	14.74	30.73
■									1140	2495	1020	1770
									5.07	11.10	4.54	7.87
■									1420	2785	1290	2210
									6.32	12.39	5.74	9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_0 is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSON and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

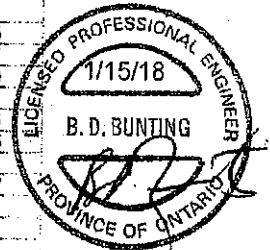
Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _e ³	Header	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _Q = 1.15)	(K _Q = 1.00)	(K _Q = 1.15)	(K _Q = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
LUS24-2	18	3 1/8	3 1/8	2	1 1/2	(4) 16d	(2) 16d	835	2020	590	1435
SS LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	3.71	8.99	2.62	6.38
HHUS26-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	1720	2595	1545	1920
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	7.65	11.54	6.87	8.54
SS LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	2850	7335	2065	5205
HHUS28-2	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	12.66	32.83	9.20	23.15
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	4385	8950	3110	6355
SS LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	19.51	39.81	13.83	28.27
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	7.65	14.79	6.87	11.45
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
								6070	12980	4310	9215
SS LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	27.00	57.74	19.17	40.99
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	2580	4500	2320	3195
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	11.48	20.02	10.32	14.21
								4670	9660	4235	7000
								29.77	42.97	18.84	31.14
								6840	14015	4855	10270
								30.43	62.34	21.60	45.69
Triple 2x Sizes											
HGUS26-3	12	4 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d	4670	9670	4235	6865
								20.77	43.02	18.84	30.54
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-4	14	6 1/8	8 1/8	3	7 1/8	(30) 16d	(10) 16d	4670	10155	4235	7210
								20.77	45.17	18.84	32.07
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14985	5425	10645
								33.98	66.70	24.13	47.35
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
LUS46	18	3 1/8	4 1/8	2	3 1/8	(4) 16d	(4) 16d	1720	2595	1545	1920
SS HHUS46	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	7.65	11.54	6.87	8.54
HGUS46	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	2540	7335	2065	5205
								11.30	32.83	9.20	23.15
SS LUS48	18	3 1/8	6 1/8	2	3 1/8	(6) 16d	(4) 16d	4385	8950	3110	6355
HHUS48	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	19.51	39.81	13.83	28.27
HGUS48	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	7.65	14.79	6.87	11.45
								3765	8940	2675	6345
								16.75	39.77	11.90	28.22
								6070	12980	4310	9215
SS LUS410	18	3 1/8	8 1/8	2	5 1/8	(8) 16d	(6) 16d	27.00	57.74	19.17	40.99
HGUS410	12	3 1/8	9	4	8 1/8	(46) 16d	(16) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HGUS412	12	3 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
HGUS414	12	3 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	7640	14985	5425	10645
								33.98	66.70	24.13	47.35
								10130	16400	7195	11645
								45.06	72.95	32.00	51.80



Plated Truss Connectors

See footnotes
on p. 258.

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

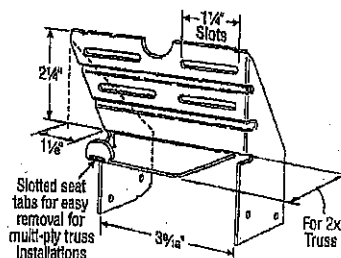
Design: Factored resistances are in accordance with CSA 086-14

Installation:

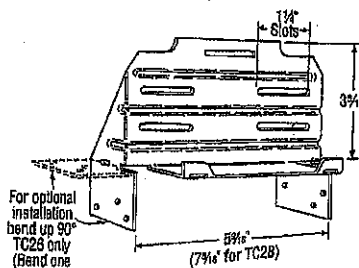
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside and is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

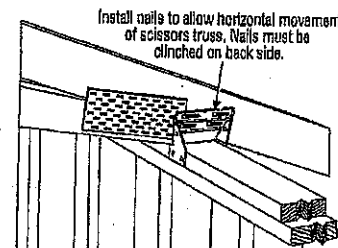
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



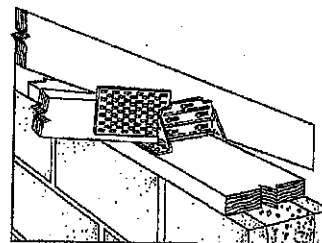
TC24
U.S. Patent 4,932,173



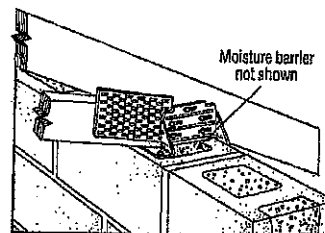
TC26
(TC28 Similar)



Typical TC24 Installation



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer (8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _o =1.15)	Uplift (K _o =1.15)
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _o =1.15)	Uplift (K _o =1.15)
TC26	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC28 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) - ½" x 2¼" Titen screws has a factored uplift resistance of 275 lb.



U.S. & CANADIAN
DESIGN

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H/TSP

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Seismic and Hurricane Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Fasteners			Factored Resistance ($K_0 = 1.15$)					
					D.Fir-L			S-P-F		
		To Rafters/Truss	To Plates	To Studs	Uplift	Lateral		Uplift	Lateral	
						F ₁	F ₂		F ₁	F ₂
					lb.	lb.	lb.	lb.	lb.	lb.
					kN	kN	kN	kN	kN	kN
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
					3.29	3.05	1.33	3.02	2.16	0.96
SS H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
					3.69	0.98	0.33	2.62	0.69	0.24
SS H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
					3.58	0.71	0.71	3.36	0.71	0.71
H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
					3.71	0.78	0.93	3.29	0.71	0.93
SS H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
					3.29	0.80	1.18	2.74	0.56	0.85
H6	16	—	(8) 8d	(8) 8d	1585	1085	—	1125	770	—
					7.05	4.83	—	5.00	3.43	—
H7Z	16	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
					6.18	2.98	—	4.40	2.11	—
SS H8 ³	18	(5) 10d x 1½"	(5) 10d x 1½"	—	1120	—	—	1025	—	—
					4.98	—	—	4.56	—	—
SS H10A ³	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290
					7.72	3.54	1.82	6.69	2.51	1.29
H10AR	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1485	690	430	1220	570	305
					6.61	3.07	1.91	5.43	2.54	1.36
H10A-2	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1835	1275	430	1645	880	305
					8.16	5.67	1.91	7.32	3.91	1.36
H10S ^{7,3}	18	(6) 8d x 1½"	(8) 8d x 1½"	(6) 8d	1465	795	315	1040	565	225
					6.52	3.54	1.40	4.68	2.51	1.00
H11Z	18	(6) 16d x 2½"	(6) 16d x 2½"	—	1095	920	545	780	655	390
					4.87	4.09	2.42	3.47	2.91	1.73
H14	18	① (12) 8d x 1½"	(13) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
		② (12) 8d x 1½"	(15) 8d	—	2390	855	320	1805	610	230
TSP	16	(9) 10d x 1½"	(6) 10d x 1½"	—	1295	440	—	920	310	—
					5.76	1.96	—	4.09	1.38	—
		(9) 10d x 1½"	(6) 10d	—	1560	440	—	1105	310	—
					6.94	1.96	—	4.92	1.38	—

- Factored resistances have been increased 15% for short term loading; no further increase is allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
- H8 factored uplift resistances for stud-to-bottom plate installations are 595 lb. (2.65 kN) for D.Fir-L and 390 lb. (1.74 kN) for S-P-F.
- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-HTIECONPATH).
- Factored resistances in the F₁ direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.

- H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.39 kN) D.Fir-L and 1015 lb. (4.51 kN) S-P-F.
- H10S nails to plates are optional for uplift but required for lateral loads.
- H10A may be field-bent up to a slope of 8/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
- The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SSNAILS at strongtie.com.
- D.Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1½" nails into the top plates and (3) 8d x 1½" nails into the lowest three flange holes into the truss bottom chord is 495 lb. (2.20 kN).
- Nails: 16d x 2½" = 0.162" dia. x 2½" long, 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long, 8d x 1½" = 0.131" dia. x 1½" long. See pp. 27–28 for other nail sizes and information.

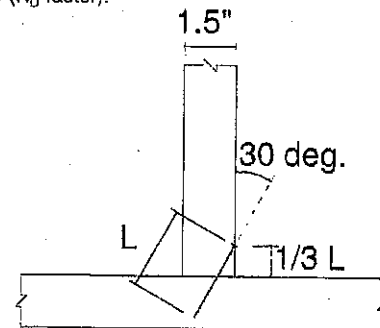
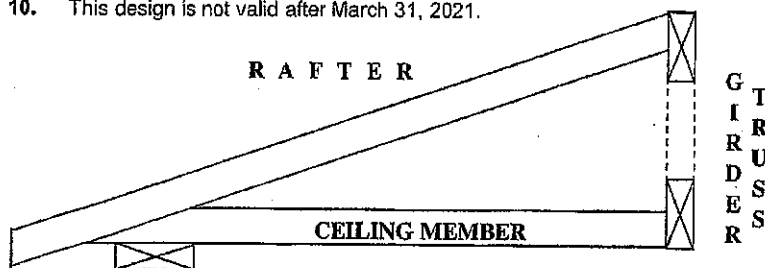
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.



TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

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December 2, 2019

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BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

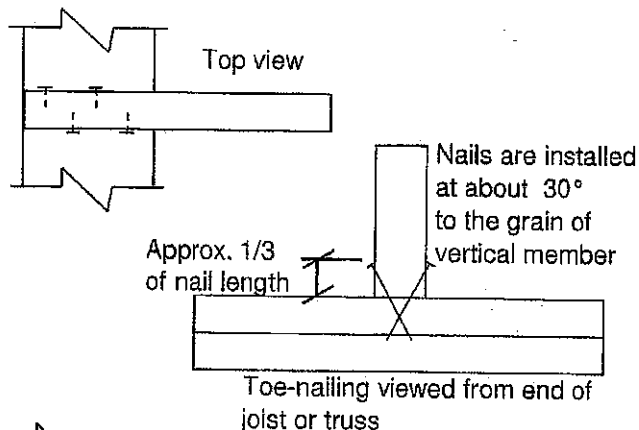
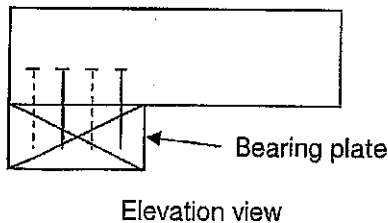
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

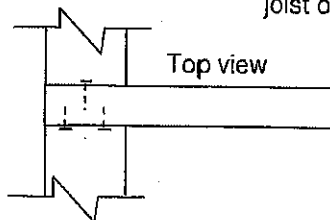
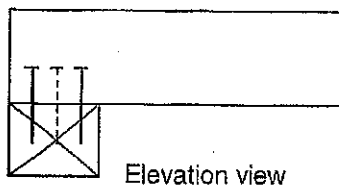
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



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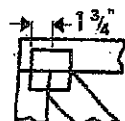
December 2, 2019

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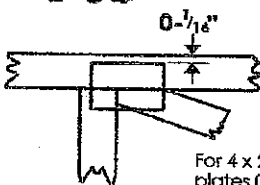


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint, unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0 1/16 inch from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



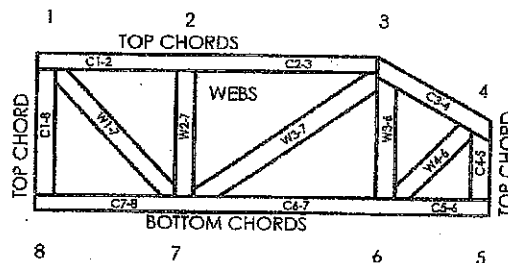
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

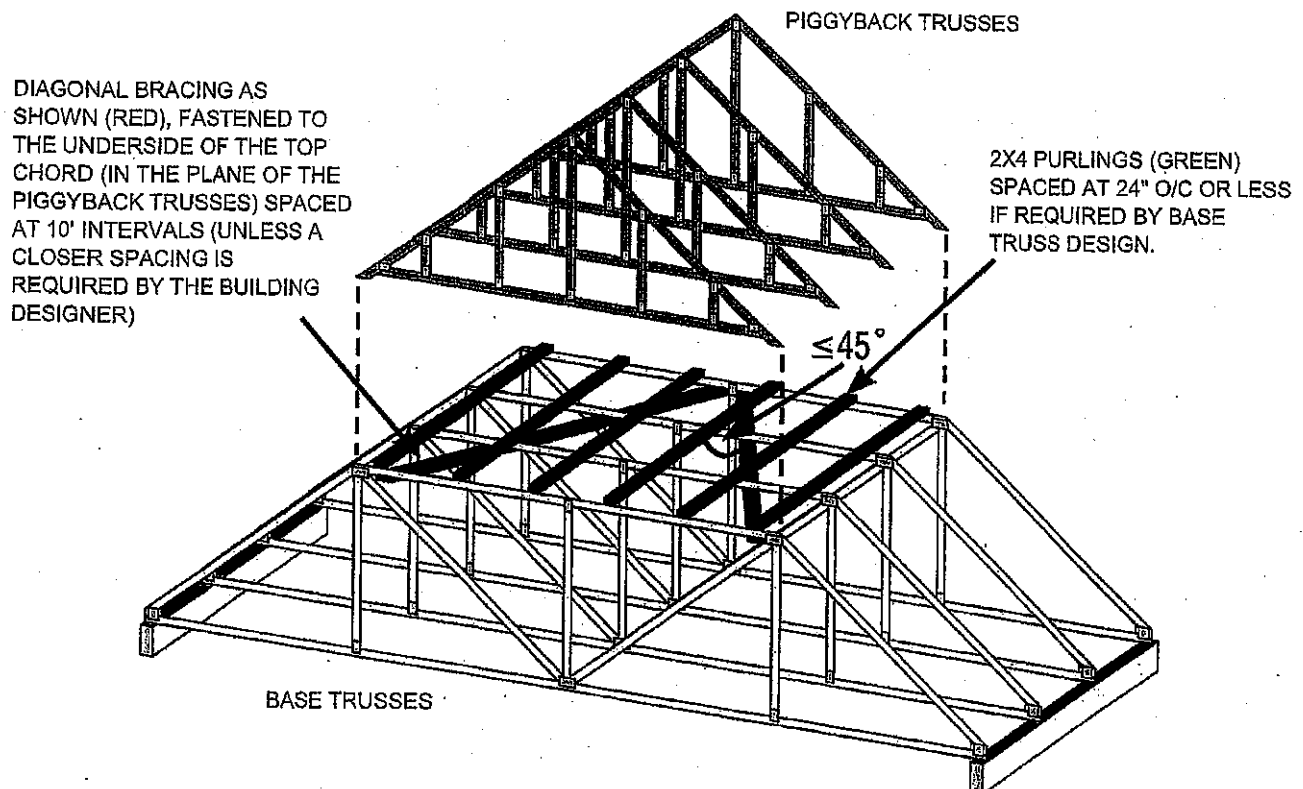
- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane of joint locations are regulated by TPIC.
- Design assumes trusses will be suitably protected from the environment in accord with TPIC.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or putlins provided at spacing indicated on design.
- Bottom chords require lateral bracing of 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with TPIC Quality Criteria.

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

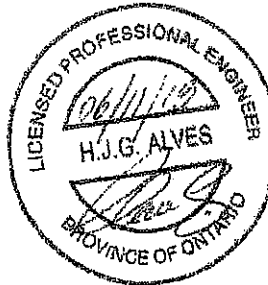


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1300213

Feb 09, 2018