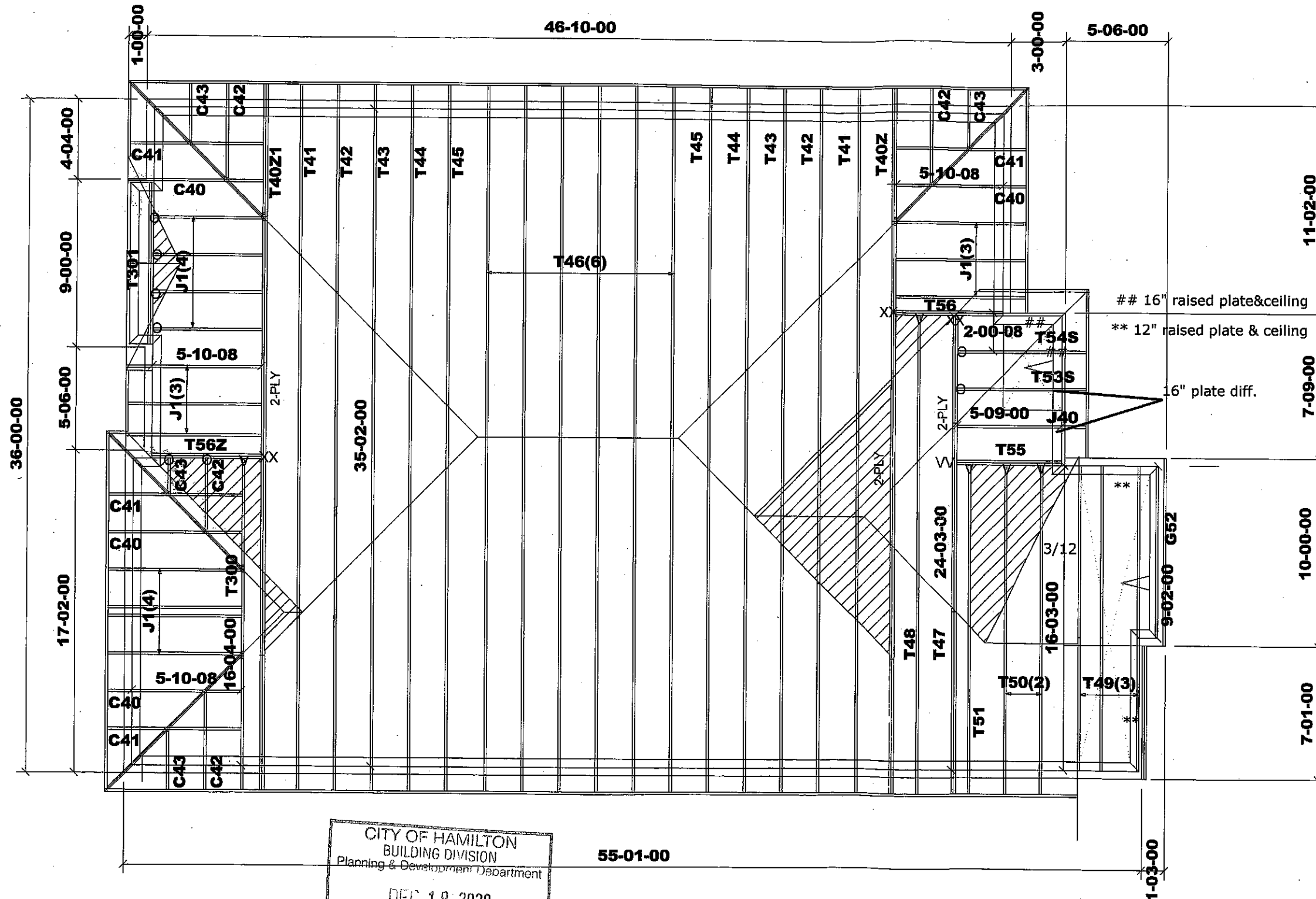


6/12 roof pitch unless noted



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
LUS26-2-(VV)

 DENOTES:
CONVENTIONAL
FRAMING



Job Track: 51225
Plan Log: 202406
Layout ID: 412082

Builder / Location:
GREEN PARK HOMES / WATERDOWN

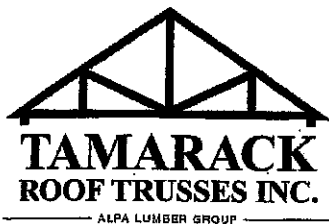
Project: **RUSSELL GARDENS PH.3**

Date: 2020-08-19 Sales: Mario DiCano

Model / Elevation:
MOUNTAINASH 5-05/3 · OPT. 5 BRM

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE

Milek ver 8.3.3.247



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 5-015
 Lot #:
 Elevation: 3- OPT. 5 BRM

Job Track: 51225
 PlanLog: 202406
 Layout ID: 412082
 Ref #
 Page: 1 of 3
 Date: 08-19-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T40Z Hip Girder	6 / 12	35-02-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	337.66 208.00		
	1 2-ply	T40Z1 Hip Girder	6 / 12	35-02-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	337.66 208.00		
	2	T41 Hip	6 / 12	35-02-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	277.99 174.00		
	2	T42 Hip	6 / 12	35-02-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	277.12 174.67		
	2	T43 Hip	6 / 12	35-02-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	289.07 179.67		
	2	T44 Hip	6 / 12	35-02-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	290.73 183.33		
	2	T45 Hip	6 / 12	35-02-00	9-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	301.67 191.33		
	6	T46 Common	6 / 12	35-02-00	9-11-08	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	910.08 558.00		
	1 2-ply	T47 Hip Girder	6 / 12	24-03-00	5-04-08	2 x 4 2 x 6	1-03-08	1-02-00 2-06-00	228.14 142.33		
	1	T48 Hip	6 / 12	24-03-00	6-04-08	2 x 4	1-03-08	1-02-00 2-06-00	99.13 63.33		
	3	T49 Roof Special	0 / 12	16-03-00	4-02-00	2 x 4		3-05-00 4-02-00	215.73 139.50		
	2	T50 Roof Special	6 / 12	16-03-00	4-06-08	2 x 4	1-03-08	1-02-00 2-02-00	125.84 80.33		
	1	T51 Half Hip	6 / 12	16-03-00	4-11-12	2 x 4	1-03-08	1-02-00 4-11-12	69.76 44.67		
	1	G52 GABLE	0 / 12	9-02-00	4-02-00	2 x 4		4-02-00 4-02-00	40.75 26.67		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 5-015
 Lot #:
 Elevation: 3- OPT. 5 BRM

Job Track: 51225
 PlanLog: 202406
 Layout ID: 412082
 Ref #
 Page: 2 of 3
 Date: 08-19-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T53S Half Hip	6 /12	5-09-00	4-05-04	2 x 4	1-03-08	1-02-00 4-05-04	32.51 21.50		
	1	T54S Half Hip	6 /12	5-09-00	3-05-04	2 x 4	1-03-08	1-02-00 3-05-04	30.62 20.67		
	1 2-ply	T55 Jack-Closed Girder	6 /12	5-09-00	5-04-08	2 x 4 2 x 6		2-06-00 5-04-08	67.19 43.67		
	1 2-ply	T56 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.17 37.00		
	1 2-ply	T56Z Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.17 37.00		
	1	T300 Hip Girder	6 /12	16-04-00	4-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	65.61 43.00		
	1 2-ply	T301 Common Girder	3 /12	8-08-00	1-10-12	2 x 4		9-12 9-12	56.79 38.00		
	14	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	235.12 149.33		
	1	J40 Jack-Open	6 /12	5-09-00	5-04-08	2 x 4	1-03-08	2-06-00 5-04-08	19.99 13.50		
	4	C40 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-05-00 2-01-01	1-02-00 3-00-12	57.17 34.67		
	4	C41 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	46.33 29.33		
	4	C42 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	38.28 24.00		
	4	C43 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	28.08 18.67		

TOTAL # TRUSS= 72

TOTAL BFT OF ALL TRUSSES= 2884.17

BFT.

TOTAL WEIGHT OF ALL TRSSES 4593.32 LBS



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
Builder: GREEN PARK HOMES
Project: RUSSELL GARDENS PH.3
Location: WATERDOWN
Model: MOUNTAINASH 5-015
Lot #:
Elevation: 3- OPT. 5 BRM

Job Track: 51225
PlanLog: 202406
Layout ID: 412082
Ref #
Page: 3 of 3
Date: 08-19-2020
Designer:
Sales Rep: Mario DiCano

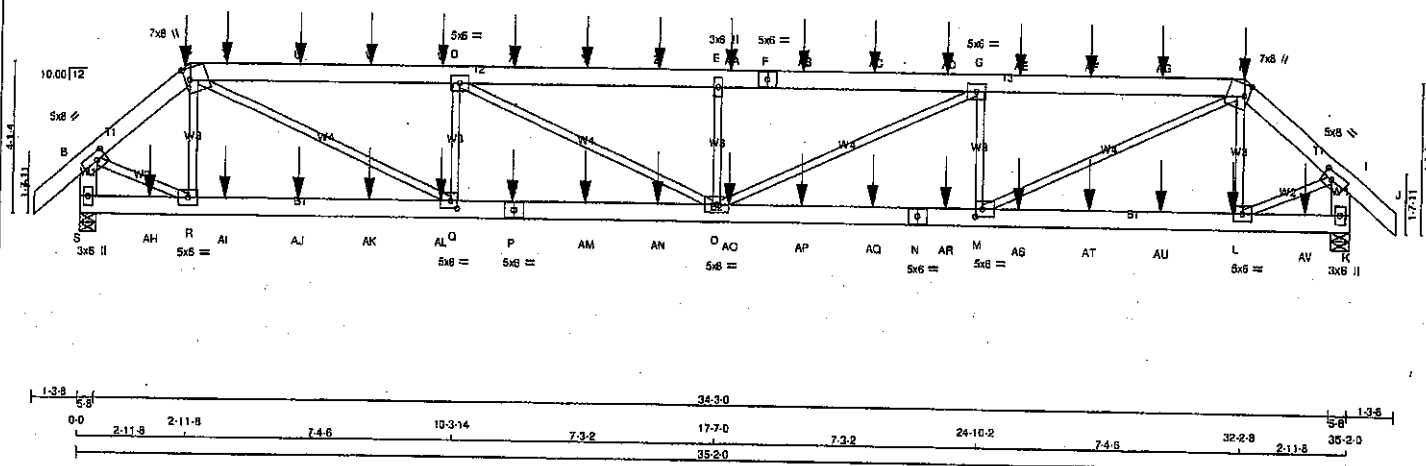
HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
5	Hardware	LJS26DS	
8	Hardware	LUS24	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 17

JOB NAME 408168	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 12:34:48 2020 Page 1
 ID:DMCubINVR6TstFoe31v6l_zns11-Mcnig2FIKqQuZwGH_IYq9JSI_3lk34wGOWeSzNBRL
 Scale = 1:57.2



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	2100F 1.8E	SPF
P - N	2x6	DRY	2100F 1.8E	SPF
N - K	2x6	DRY	2100F 1.8E	SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 2	12	SIDE(122.0)
C - F 2	12	SIDE(81.0)
F - H 2	12	SIDE(81.0)
H - J 2	12	SIDE(122.0)
S - B 2	12	TOP
K - I 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S - P 2	12	SIDE(0.0)
P - N 2	12	SIDE(0.0)
N - K 2	12	SIDE(197.8)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3/4" INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
JT VERT	3330	0	5-8	5-8
S	3330	0	5-8	5-8
K	3379	0	5-8	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX./MIN. COMPONENT REACTIONS	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT COMBINED	2954	1553 / 0	0 / 0	0 / 0	0 / 0	801 / 0	0 / 0
S	2988	1572 / 0	0 / 0	0 / 0	0 / 0	816 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	L1	MAX. CSI (LC)	UNBRAC LENGTH	FR-TO	WEBS	
							MEMB. FORCE (LBS)	MAX. FACTORED CSI (LC)
A-B	0 / 42	-91.8	-91.8	0.04 (1)	10.00	R-C	-850 / 0	0.08 (1)
B-C	-3387 / 0	-91.8	-91.8	0.05 (1)	6.02	C-Q	0 / 4509	0.56 (1)
C-T	-3602 / 0	-91.8	-91.8	0.38 (1)	4.31	Q-D	-1799 / 0	0.22 (1)
T-U	-3602 / 0	-91.8	-91.8	0.38 (1)	4.31	D-O	0 / 1370	0.17 (1)
U-V	-8602 / 0	-91.8	-91.8	0.38 (1)	4.31	O-E	-987 / 0	0.12 (1)
V-W	-8602 / 0	-91.8	-91.8	0.38 (1)	4.31	E-G	0 / 1362	0.17 (1)
W-O	-8602 / 0	-91.8	-91.8	0.38 (1)	4.31	G-H	-1787 / 0	0.21 (1)
D-X	-7826 / 0	-91.8	-91.8	0.42 (1)	3.98	M-H	0 / 4481	0.55 (1)
X-Y	-7826 / 0	-91.8	-91.8	0.42 (1)	3.98	L-H	-661 / 0	0.08 (1)
Y-Z	-7826 / 0	-91.8	-91.8	0.42 (1)	3.98	B-R	0 / 2736	0.34 (1)
Z-E	-7826 / 0	-91.8	-91.8	0.42 (1)	3.98	L-I	0 / 2771	0.34 (1)
E-AA	-7826 / 0	-91.8	-91.8	0.42 (1)	3.98			
AA-F	-7826 / 0	-91.8	-91.8	0.42 (1)	3.98			
F-AB	-7826 / 0	-91.8	-91.8	0.42 (1)	3.98			
AB-AC	-7826 / 0	-91.8	-91.8	0.42 (1)	3.98			
AC-AD	-7826 / 0	-91.8	-91.8	0.42 (1)	3.98			
AD-G	-7826 / 0	-91.8	-91.8	0.42 (1)	3.98			
G-AE	-6608 / 0	-91.8	-91.8	0.38 (1)	4.31			
AE-AF	-6608 / 0	-91.8	-91.8	0.38 (1)	4.31			
AF-AG	-6608 / 0	-91.8	-91.8	0.38 (1)	4.31			
AG-H	-6608 / 0	-91.8	-91.8	0.38 (1)	4.31			
H-I	-3410 / 0	-91.8	-91.8	0.05 (1)	6.00			
I-J	0 / 42	-91.8	-91.8	0.04 (1)	10.00			
S-B	-3375 / 0	0.0	0.0	0.12 (1)	7.58			
K-I	-3414 / 0	0.0	0.0	0.12 (1)	7.55			

S-AH	0 / 0	-18.5	-18.5	0.05 (4)	10.00
AH-R	0 / 0	-18.5	-18.5	0.05 (4)	10.00
R-AI	0 / 2553	-18.5	-18.5	0.12 (1)	10.00
AI-AJ	0 / 2553	-18.5	-18.5	0.12 (1)	10.00
AJ-AK	0 / 2553	-18.5	-18.5	0.12 (1)	10.00
AK-AL	0 / 2553	-18.5	-18.5	0.12 (1)	10.00
AL-Q	0 / 2553	-18.5	-18.5	0.12 (1)	10.00
Q-P	0 / 8602	-18.5	-18.5	0.22 (1)	10.00
P-AM	0 / 8602	-18.5	-18.5	0.22 (1)	10.00
AM-AN	0 / 8602	-18.5	-18.5	0.22 (1)	10.00
AN-O	0 / 8602	-18.5	-18.5	0.22 (1)	10.00
O-AQ	0 / 8609	-18.5	-18.5	0.22 (1)	10.00
AQ-AP	0 / 8609	-18.5	-18.5	0.22 (1)	10.00
AP-AQ	0 / 8609	-18.5	-18.5	0.22 (1)	10.00
AQ-N	0 / 8609	-18.5	-18.5	0.22 (1)	10.00
N-AR	0 / 8609	-18.5	-18.5	0.22 (1)	10.00
AR-M	0 / 8609	-18.5	-18.5	0.22 (1)	10.00

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
 ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBCC 2010, OBC 2012, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.20")
 ALLOWABLE DEFL.(TL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.37")

CSI: TC=0.42/1.00 (D-E:1), SC=0.22/1.00 (O-Q:1), WB=0.58/1.00 (C-Q:1), SS=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (H) (INPUT = 0.90)
 JSI METAL = 0.65 (N) (INPUT = 1.00)



Structural component only
 DWG# T-2007129 1/2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.25
C	TTWV+m	MT20	7.0	8.0	3.75	1.50
D	TMVW-t	MT20	5.0	6.0		
E	TMVW-w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMVW-t	MT20	5.0	6.0		
H	TTWV+m	MT20	7.0	8.0	3.75	1.50
I	TMVW-t	MT20	5.0	8.0	2.50	3.25
K	BMV1+p	MT20	3.0	6.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMVW-t	MT20	5.0	8.0	2.50	2.25
N	BS-t	MT20	5.0	6.0		
O	BMVWV-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.25
R	BMVW-t	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	6.0		

LOADING

TOTAL LOAD CASES: (4)

CHORDS

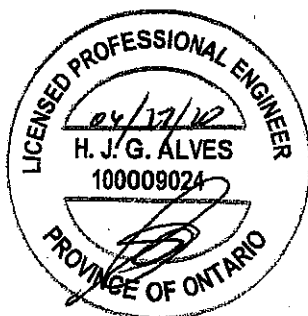
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		LENGTH FR-TO			
MAS	0 / 2586	-18.5	-18.5	0.12 (1)	10.00			
AS-AT	0 / 2586	-18.5	-18.5	0.12 (1)	10.00			
AT-AJ	0 / 2586	-18.5	-18.5	0.12 (1)	10.00			
AJ-L	0 / 2586	-18.5	-18.5	0.12 (1)	10.00			
L-AV	0 / 0	-18.5	-18.5	0.05 (1)	10.00			
AV-K	0 / 0	-18.5	-18.5	0.05 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-6	-38	-42	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-191	-191	---	FRONT	VERT	SNOW	---	C1
H	32-2-8	-38	-42	---	FRONT	VERT	DEAD	---	C1
H	32-2-8	-116	-116	---	FRONT	VERT	TOTAL	---	C1
H	32-2-8	-191	-191	---	FRONT	VERT	SNOW	---	C1
L	31-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	11-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
T	3-11-4	-111	-111	---	FRONT	VERT	TOTAL	---	C1
U	5-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	11-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	15-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	17-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	19-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	21-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	23-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AE	25-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AF	27-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AG	29-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AH	1-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AI	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AJ	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AK	7-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AL	9-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AM	13-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AN	15-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AO	17-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AP	19-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AQ	21-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AR	23-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AS	25-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AT	27-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AU	29-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AV	33-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1

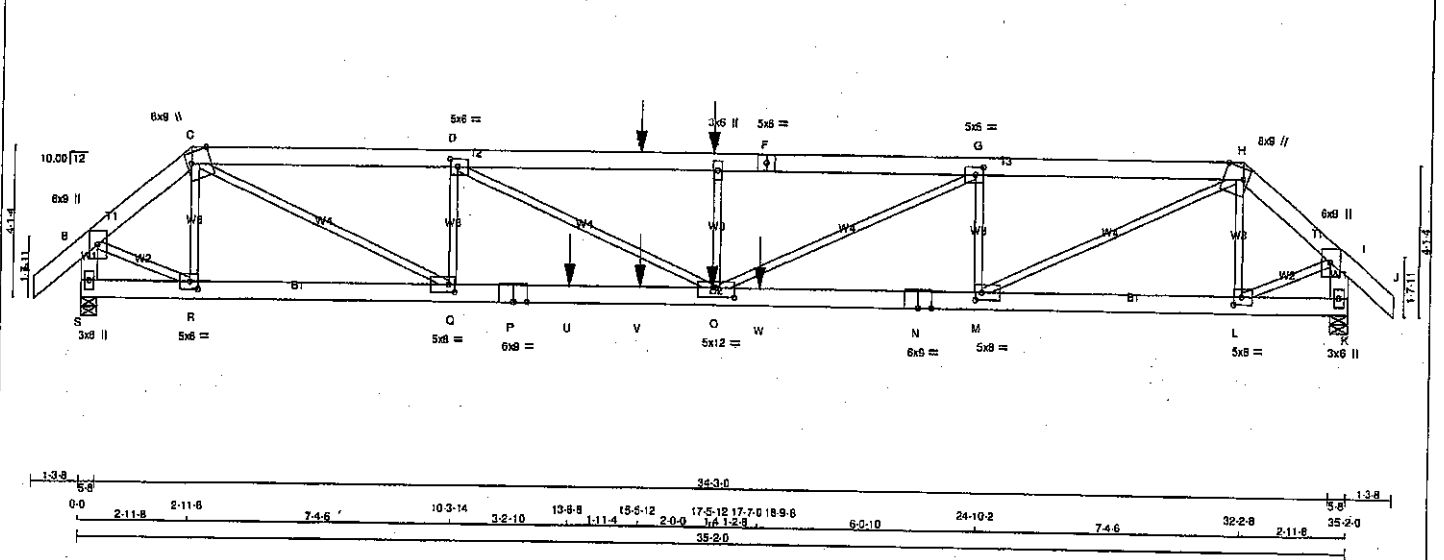
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007129

JOB NAME 408168	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mittek Industries, Inc. Sat Apr 25 12:34:49 2020 Page 1	
ID:DMCubINVR6TstFoe31v6l_zns1f-qol41NGw50yHWfVSqpnNNrbzOlzT4?D9w8UBuzNBK				Scale = 1/8" = 1'-0"	



LUMBER	SIZE	DESCR.
N. L. G. A. RULES		
CHORDS		
A - C	2x6 DRY	No. 2
O - F	2x6 DRY	No. 2
F - H	2x6 DRY	No. 2
H - J	2x6 DRY	No. 2
S - B	2x6 DRY	No. 2
K - I	2x6 DRY	No. 2
S - P	2x6 DRY	2100F 1.8E
P - N	2x6 DRY	2100F 1.8E
N - K	2x6 DRY	2100F 1.8E
ALL WEBS EXCEPT	2x3 DRY	No. 2

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	2 12	TOP
C-F	2 12	SIDE(183.1)
F-H	2 12	TOP
H-J	2 12	TOP
S-B	2 12	TOP
K-I	2 12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S-P	2 12	TOP
P-N	2 12	SIDE(197.8)
N-K	2 12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1 8	TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT 3963	DOWN 3963	0	5-8
S	HORIZ 0	HORIZ 0	0	5-8
K	VERT 3887	DOWN 3887	0	5-8

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE	VERT. LOAD	MAX.	MEMB.	FORCE	MAX.	MAX.
	(LBS)	(PLF)	CSI (LC)		(LBS)	CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 42	-91.8	-91.8 0.04 (1)	R-C	-1086 / 0	0.13 (1)	
B-C	-3988 / 0	-91.8	-91.8 0.06 (1)	C-Q	0 / 7263	0.90 (1)	
C-D	-9534 / 0	-91.8	-91.8 0.37 (1)	D-O	-2160 / 0	0.26 (1)	
D-E	-12331 / 0	-91.8	-91.8 0.52 (1)	E-F	0 / 3130	0.39 (1)	
E-F	-12331 / 0	-91.8	-91.8 0.52 (1)	F-G	-776 / 0	0.09 (1)	
F-G	-12331 / 0	-91.8	-91.8 0.50 (1)	G-H	0 / 4203	0.52 (1)	
G-H	-6575 / 0	-91.8	-91.8 0.50 (1)	H-I	-2613 / 0	0.31 (1)	
H-I	-6575 / 0	-91.8	-91.8 0.33 (1)	I-J	0 / 6383	0.79 (1)	
I-J	-3758 / 0	-91.8	-91.8 0.08 (1)	J-K	-843 / 0	0.10 (1)	
J-K	0 / 42	-91.8	-91.8 0.04 (1)	K-L	0 / 3239	0.40 (1)	
K-L	-3945 / 0	0.0	0.0 0.14 (1)	L-I	0 / 3052	0.38 (1)	
	-3733 / 0	0.0	0.0 0.13 (1)				
S-R	0 / 0	-18.5	-18.5 0.02 (4)				
R-Q	0 / 3013	-18.5	-18.5 0.21 (1)				
Q-P	0 / 9534	-18.5	-18.5 0.67 (1)				
P-U	0 / 9534	-18.5	-18.5 0.67 (1)				
U-V	0 / 9534	-18.5	-18.5 0.67 (1)				
V-O	0 / 9534	-18.5	-18.5 0.67 (1)				
O-W	0 / 6575	-18.5	-18.5 0.49 (1)				
W-N	0 / 6575	-18.5	-18.5 0.49 (1)				
N-M	0 / 6575	-18.5	-18.5 0.49 (1)				
M-L	0 / 2844	-18.5	-18.5 0.12 (1)				
L-K	0 / 0	-18.5	-18.5 0.05 (1)				

FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL
E	17-5-12	-110	-110	---	BACK	VERT	---
O	17-5-12	-26	-26	---	BACK	VERT	---
T	15-5-12	-110	-110	---	BACK	VERT	---
U	13-8-8	-1615	-1615	---	BACK	VERT	---
V	15-5-12	-26	-26	---	BACK	VERT	---
W	18-9-8	-1620	-1620	---	BACK	VERT	---

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 163 = 326 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.17')
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.35')
ALLOWABLE DEFL.(TL) = $L/360$ (1.17')
CALCULATED VERT. DEFL.(TL) = $L/657$ (0.64')
CSI: $TC=0.52/1.00$ (D-E:1), $BC=0.67/1.00$ (O-Q:1),
 $WB=0.80/1.00$ (C-Q:1), $SSI=0.59/1.00$ (M-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (C) (INPUT = 0.90)

JSI METAL = 0.95 (P) (INPUT = 1.00)



Structural component only
DWG# T-2007130 1/2

JOB NAME 408168	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6L zns1H-qoL41NGw50vH-WIVSnpnNNrbzOizT4?D9w8UBuzNBRK

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	9.0		
C	TTWW+m	MT20	8.0	9.0	Edge	6.50
D	TMWW-l	MT20	5.0	6.0	2.50	2.50
E	TMW+w	MT20	3.0	6.0		
F	TS-l	MT20	5.0	6.0		
G	TMWW-l	MT20	5.0	6.0	2.50	2.50
H	TTWW+m	MT20	8.0	9.0	Edge	6.50
I	TMVW+p	MT20	6.0	9.0		
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-l	MT20	5.0	6.0	2.50	2.75
M	BMWW-l	MT20	5.0	8.0	2.50	2.00
N	BS-l	MT20	6.0	9.0		
O	BMWWW-l	MT20	5.0	12.0	3.00	6.00
P	BS-l	MT20	6.0	9.0		
Q	BMWW-l	MT20	5.0	8.0	2.50	2.00
R	BMWW-l	MT20	5.0	8.0	2.50	2.75
S	BMV1+p	MT20	3.0	6.0		

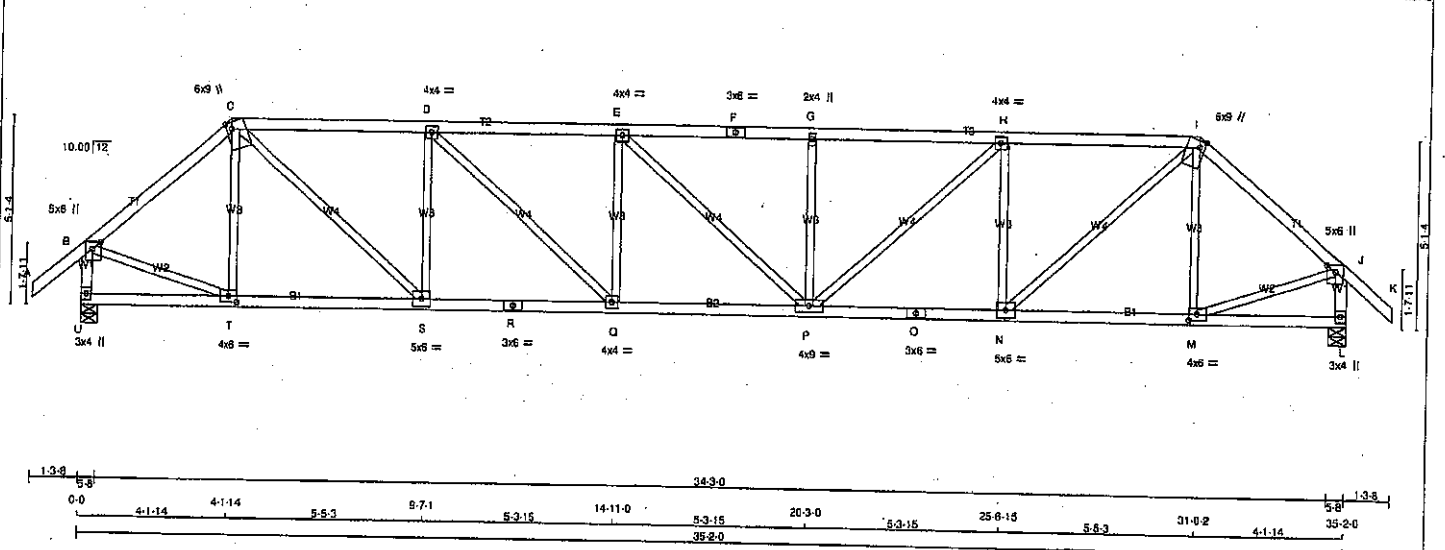
Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



Structural component only
DWG# T-2007130 92

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408168	T2	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR6TstFoe31v6L_zns1H7vSFHJZsJ487s3fPPK0vaCkaof0CiaMOat1IKzNBRJ
13-8 0-0 3-1-14 3-1-14 5-5-3 9-7-1 5-3-15 14-11-0 5-3-15 20-3-0 25-6-15 5-3-3 31-0-2 4-1-14 35-2-0 36-5-8 1-3-8
Scale = 1:57.3



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - F	2x4 DRY	No.2	SPF		
F - I	2x4 DRY	No.2	SPF		
I - K	2x4 DRY	No.2	SPF		
U - B	2x4 DRY	No.2	SPF		
L - J	2x4 DRY	No.2	SPF		
R - O	2x4 DRY	No.2	SPF		
O - L	2x4 DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75	
C	TTWW+m	MT20	6.0	9.0	Edge	1.75	
D, E, H							
D	TMVW-t	MT20	4.0	4.0			
F	TS-t	MT20	3.0	6.0			
G	TMVW+m	MT20	2.0	4.0			
I	TTWW+m	MT20	6.0	9.0	Edge	1.75	
J	TMVW+p	MT20	5.0	6.0	Edge	2.75	
L	BMV1+p	MT20	3.0	4.0			
M	BMVW-t	MT20	4.0	6.0	2.00	2.75	
N	BMVW-t	MT20	5.0	6.0			
O	BS-t	MT20	3.0	6.0			
P	BMVWVW-t	MT20	4.0	9.0			
Q	BMVW-t	MT20	4.0	4.0			
R	BS-t	MT20	3.0	6.0			
S	BMVW-t	MT20	5.0	6.0			
T	BMVW-t	MT20	4.0	6.0	2.00	2.75	
U	BMV1+p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
U	2066	0	2066	0
L	2066	0	2066	0

UNFACTORED REACTIONS	1ST LOASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
U	1458	971 / 0	0 / 0	0 / 0	488 / 0	0 / 0
L	1458	971 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)
FR-TO						
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	
B-C	-1960 / 0	-91.8	-91.8	0.39 (1)	4.46	
C-D	-2871 / 0	-91.8	-91.8	0.57 (1)	3.59	
D-E	-3508 / 0	-91.8	-91.8	0.85 (1)	3.21	
E-F	-3505 / 0	-91.8	-91.8	0.53 (1)	3.36	
F-G	-3505 / 0	-91.8	-91.8	0.66 (1)	3.20	
G-H	-3505 / 0	-91.8	-91.8	0.13 (1)	10.00	
H-I	-2871 / 0	-91.8	-91.8	0.39 (1)	4.46	
I-J	-1960 / 0	-91.8	-91.8	0.57 (1)	3.59	
J-K	0 / 41	-91.8	-91.8	0.13 (1)	10.00	
U-B	-2037 / 0	0.0	0.0	0.22 (1)	5.92	
L-J	-2037 / 0	0.0	0.0	0.22 (1)	5.92	
U-T	0 / 0	-18.5	-18.5	0.10 (4)	10.00	
T-S	0 / 1494	-18.5	-18.5	0.30 (1)	10.00	
S-R	0 / 2871	-18.5	-18.5	0.51 (1)	10.00	
R-Q	0 / 2871	-18.5	-18.5	0.51 (1)	10.00	
Q-P	0 / 3505	-18.5	-18.5	0.62 (1)	10.00	
P-O	0 / 2872	-18.5	-18.5	0.51 (1)	10.00	
O-N	0 / 2872	-18.5	-18.5	0.51 (1)	10.00	
N-M	0 / 1494	-18.5	-18.5	0.30 (1)	10.00	
M-L	0 / 0	-18.5	-18.5	0.10 (4)	10.00	

TOTAL WEIGHT = 2 X 147 = 293 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/989 (0.20")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = L/989 (0.39")

CSI: TC=0.66/1.00 (G-H:1), BC=0.62/1.00 (P-Q:1), WB=0.43/1.00 (D-S:1), SS=0.23/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

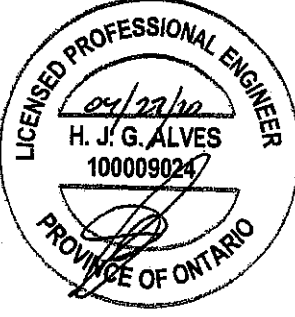
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY)	SHEAR SECTION
(PSI)	(PLI)	(PLI)
MT20	618	354
	1667	788
	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (N) (INPUT = 0.90)
JSI METAL = 0.90 (O) (INPUT = 1.00)

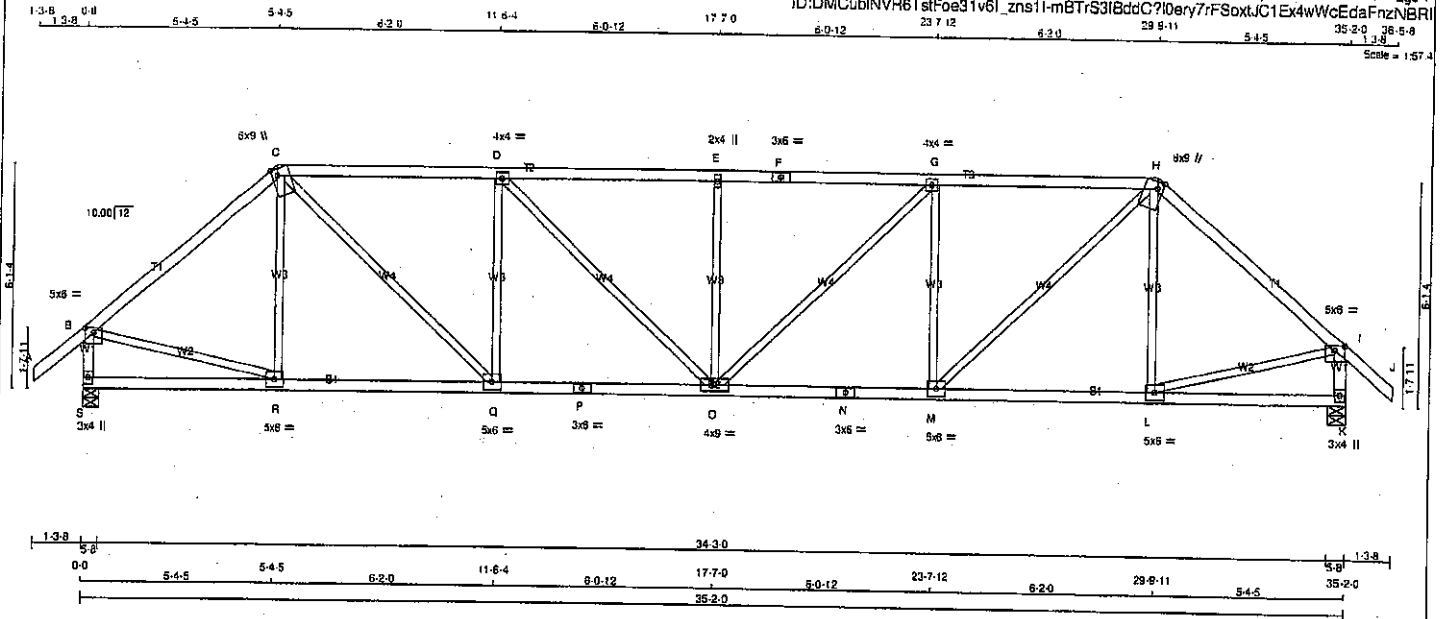


Structural component only
DWG# T-2007131

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408168	T3	2	1	GREEN PARK HOMES	

Tamareck Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
S - P	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2

ALL WEBS 2x3 DRY EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-m	MT20	5.0	9.0	Edge	1.75
D	TMWV-l	MT20	4.0	4.0		
E	TMWV-w	MT20	2.0	4.0		
F	TS-l	MT20	3.0	8.0		
G	TMWV-l	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	9.0	Edge	1.75
I	TMWV-p	MT20	5.0	6.0	1.50	3.00
K	BMV1-p	MT20	3.0	4.0		
L, M, Q, R						
N	BMWV-l	MT20	5.0	6.0		
O	BS-l	MT20	3.0	6.0		
P	BMWV-l	MT20	4.0	9.0		
S	BS-l	MT20	3.0	6.0		
S	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	RECD
JT	VERT	HORZ	DOWN	UPLIFT
S	2066	0	2066	0
K	2066	0	2066	0

UNFACTORED REACTIONS

	1ST LOASE	MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1458	971/0	0/0	0/0	0/0	489/0	0/0
K	1458	971/0	0/0	0/0	0/0	489/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORZ. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/41	-91.8	-91.8	R-C	-246/7	0.14	(1)
B-C	-2004/0	-91.8	-91.8	C-Q	0/1538	0.35	(1)
C-D	-2653/0	-91.8	-91.8	Q-D	-943/0	0.55	(1)
D-E	-2878/0	-91.8	-91.8	D-O	0/450	0.10	(1)
E-F	-2878/0	-91.8	-91.8	O-E	-513/0	0.30	(1)
F-G	-2653/0	-91.8	-91.8	G-Q	0/450	0.10	(1)
G-H	-2653/0	-91.8	-91.8	M-G	-943/0	0.55	(1)
H-I	-2004/0	-91.8	-91.8	M-H	0/1538	0.35	(1)
I-J	0/41	-91.8	-91.8	L-H	-246/7	0.14	(1)
S-B	-2026/0	0.0	0.0	B-R	0/1580	0.35	(1)
K-I	-2026/0	0.0	0.0	L-I	0/1580	0.35	(1)
S-R	0/0	-18.5	-18.5				
R-Q	0/1532	-18.5	-18.5				
Q-P	0/2653	-18.5	-18.5				
P-O	0/2653	-18.5	-18.5				
O-N	0/2653	-18.5	-18.5				
N-M	0/2653	-18.5	-18.5				
M-L	0/1532	-18.5	-18.5				
L-K	0/0	-18.5	-18.5				

TOTAL WEIGHT = 2 X 148 = 296 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 099-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")

CALCULATED VERT. DEFL.(LL) = L/998 (0.18")

ALLOWABLE DEFL.(TL) = L/360 (1.17")

CALCULATED VERT. DEFL.(TL) = L/998 (0.29")

CSI: TC=0.78/1.00 (E-G:1), BC=0.49/1.00 (M-O:1),

WS=0.55/1.00 (G-M:1), SS=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

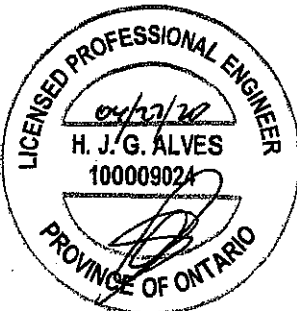
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1687

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)

JSI METAL= 0.85 (N) (INPUT = 1.00)

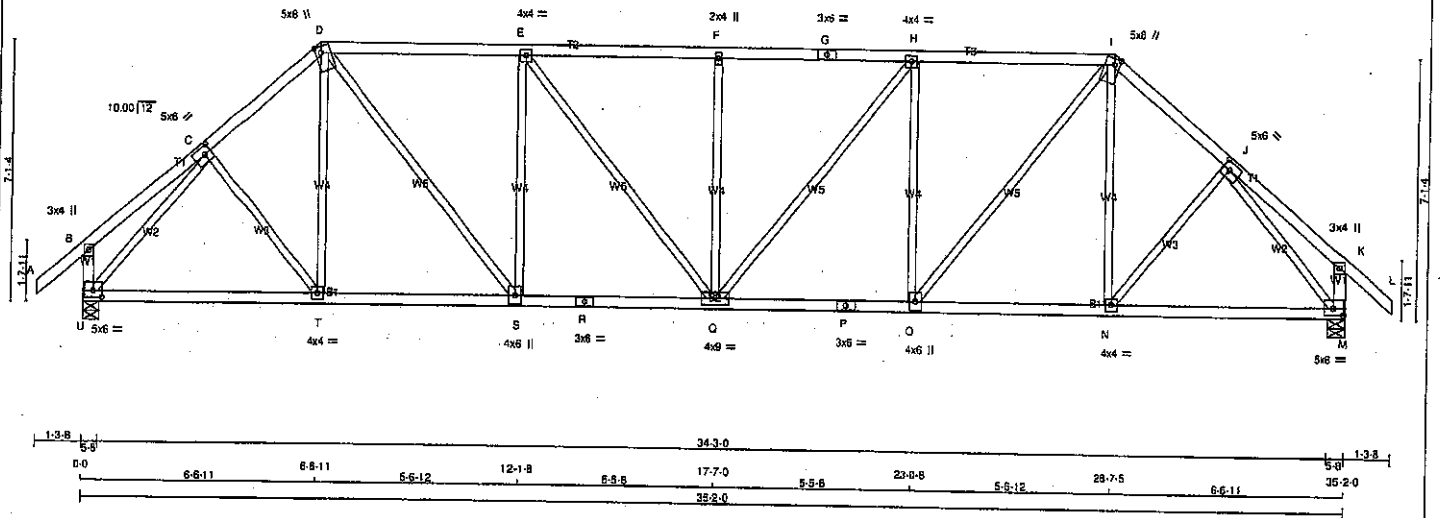


Structural component only
DWG# T-2007132

JOB NAME: 408168 TRUSS NAME: T4 QUANTITY: 2 PLY: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC: DRWG NO. Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Sat Apr 25 12:34:52 2020 Page 1
ID:DMCubINVR6TstFoe31v6I zns1I-EN1DIPiOxKsNAD1WqNU_?T6RbNZgQVruM8oDzNBRH

Scale = 1:57.5



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	8.0	2.50	2.25
D	TTWW+m	MT20	5.0	8.0	2.00	1.75
E	TMVW-t	MT20	4.0	4.0		
F	TMVW-w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	5.0	8.0	2.00	1.75
J	TMVW-t	MT20	5.0	8.0	2.50	2.25
K	TMV+p	MT20	3.0	4.0		
M	BMVW-t	MT20	5.0	8.0	2.25	3.00
N	BMVW-t	MT20	4.0	4.0		
O	BMVW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	6.0		
Q	BMVW-t	MT20	4.0	4.0		
R	BS-t	MT20	3.0	6.0		
S	BMVW-t	MT20	4.0	4.0		
T	BMVW-t	MT20	4.0	4.0		
U	BMVW-t	MT20	5.0	8.0	2.25	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
U	2066	0	0	5-8
M	2066	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
M	1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 41	-91.8 -91.8	0.13 (1)	10.00	C-T	0 / 156	0.04 (1)
B-C	0 / 19	-91.8 -91.8	0.14 (1)	10.00	T-D	0 / 76	0.03 (4)
C-D	-2023 / 0	-91.8 -91.8	0.20 (1)	4.59	D-S	0 / 1220	0.27 (1)
D-E	-2308 / 0	-91.8 -91.8	0.55 (1)	3.94	S-E	-850 / 0	0.74 (1)
E-F	-2534 / 0	-91.8 -91.8	0.57 (1)	3.77	E-Q	0 / 361	0.08 (1)
F-G	-2534 / 0	-91.8 -91.8	0.57 (1)	3.77	Q-F	-481 / 0	0.40 (1)
G-H	-2534 / 0	-91.8 -91.8	0.57 (1)	3.77	Q-H	0 / 361	0.08 (1)
H-I	-2308 / 0	-91.8 -91.8	0.55 (1)	3.94	C-H	-850 / 0	0.74 (1)
I-J	-2023 / 0	-91.8 -91.8	0.20 (1)	4.59	O-I	0 / 1220	0.27 (1)
J-K	0 / 19	-91.8 -91.8	0.14 (1)	10.00	N-I	0 / 76	0.03 (4)
K-L	0 / 41	-91.8 -91.8	0.13 (1)	10.00	N-J	0 / 156	0.04 (1)
U-B	-248 / 0	0.0 0.0	0.03 (1)	7.81	U-C	-2277 / 0	0.98 (1)
M-K	-248 / 0	0.0 0.0	0.03 (1)	7.81	J-M	-2277 / 0	0.98 (1)
U-T	0 / 1433	-18.5 -18.5	0.33 (1)	10.00			
T-S	0 / 1535	-18.5 -18.5	0.35 (1)	10.00			
S-R	0 / 2308	-18.5 -18.5	0.42 (1)	10.00			
R-Q	0 / 2308	-18.5 -18.5	0.42 (1)	10.00			
Q-P	0 / 2308	-18.5 -18.5	0.42 (1)	10.00			
P-O	0 / 2308	-18.5 -18.5	0.42 (1)	10.00			
O-N	0 / 1535	-18.5 -18.5	0.35 (1)	10.00			
N-M	0 / 1433	-18.5 -18.5	0.33 (1)	10.00			

TOTAL WEIGHT = 2 X 162 = 323 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 38.0	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17')

CALCULATED VERT. DEFL.(LL) = L/998 (0.13')

ALLOWABLE DEFL.(TL) = L/360 (1.17')

CALCULATED VERT. DEFL.(TL) = L/998 (0.23')

CS1: TC=0.57/1.00 (E-F:1), BC=0.42/1.00 (O-Q:1),

WB=0.98/1.00 (J-M:1), SS=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

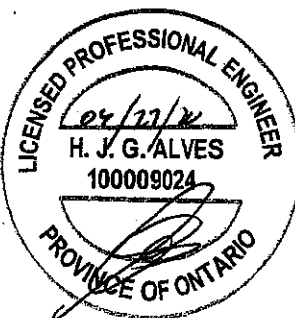
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PS)	SECTION (PLI)
MT20	618	354
	1667	788
	1667	1665

PLATE PLACEMENT TOL. = 0.250 inches

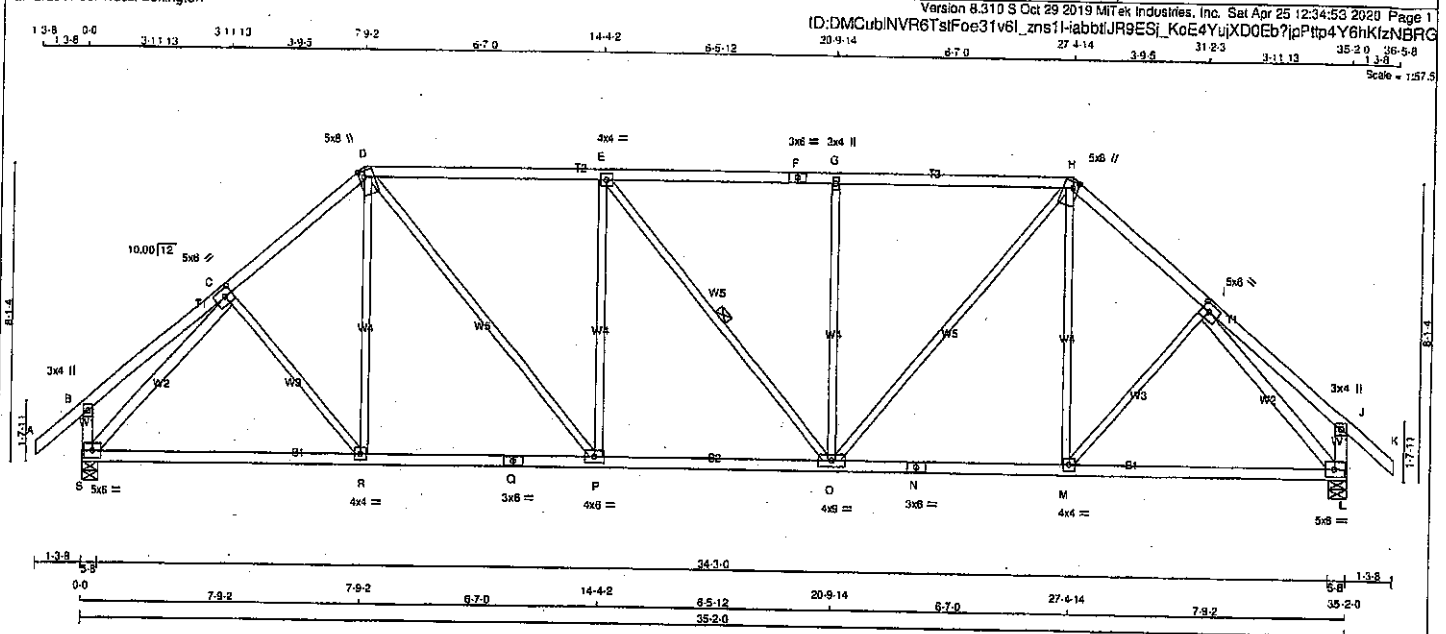
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (I) (INPUT = 0.90)
JSI METAL = 0.72 (P) (INPUT = 1.00)



Structural component only
DWG# T-2007133

JOB NAME 408168	TRUSS NAME T5	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
O - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
S - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV-p	MT20	3.0	4.0
C	TMVW-1	MT20	5.0	8.0 2.50 2.50
D	TTWW-m	MT20	5.0	8.0 2.00 1.75
E	TMVW-1	MT20	4.0	4.0
F	TS-1	MT20	3.0	8.0
G	TMVW-m	MT20	2.0	4.0
H	TTWW-m	MT20	5.0	8.0 2.00 1.75
I	TMVW-1	MT20	5.0	8.0 2.50 2.50
J	TMV-p	MT20	3.0	4.0
L	BMVW-1	MT20	5.0	6.0
M	BMVW-1	MT20	4.0	4.0
N	BS-1	MT20	3.0	6.0
O	BMVW-1	MT20	4.0	9.0
P	BMVW-1	MT20	4.0	6.0
Q	BS-1	MT20	3.0	6.0
R	BMVW-1	MT20	4.0	4.0
S	BMVW-1	MT20	5.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG	
S	2068 0	2068 0	0 0	5-8	5-8
L	2068 0	2068 0	0 0	5-8	5-8

UNFACTORED REACTIONS							
1ST CASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
L	1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO				FR-TO			
A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00	C-R	0 / 64	0.02 (4)
B-C	0 / 26	-91.8	-91.8 0.21 (1)	10.00	R-D	0 / 126	0.04 (4)
C-D	-2006 / 0	-91.8	-91.8 0.29 (1)	4.50	D-P	0 / 998	0.22 (1)
D-E	-2162 / 0	-91.8	-91.8 0.73 (1)	3.81	P-E	-850 / 0	0.82 (1)
E-F	-2162 / 0	-91.8	-91.8 0.73 (1)	3.81	E-O	-2 / 0	0.00 (1)
F-G	-2162 / 0	-91.8	-91.8 0.73 (1)	3.81	O-G	-849 / 0	0.82 (1)
G-H	-2162 / 0	-91.8	-91.8 0.73 (1)	3.81	C-H	0 / 998	0.22 (1)
H-I	-2006 / 0	-91.8	-91.8 0.29 (1)	4.50	M-H	0 / 126	0.04 (4)
I-J	0 / 26	-91.8	-91.8 0.21 (1)	10.00	M-I	0 / 64	0.02 (4)
J-K	0 / 41	-91.8	-91.8 0.13 (1)	10.00	S-C	-2291 / 0	0.97 (1)
S-B	-264 / 0	0.0	0.0 0.03 (1)	7.81	I-L	-2291 / 0	0.97 (1)
L-J	-264 / 0	0.0	0.0 0.03 (1)	7.81			
S-R	0 / 1487	-18.5	-18.5 0.38 (1)	10.00			
R-Q	0 / 1519	-18.5	-18.5 0.39 (1)	10.00			
Q-P	0 / 1519	-18.5	-18.5 0.39 (1)	10.00			
P-O	0 / 2163	-18.5	-18.5 0.42 (1)	10.00			
O-N	0 / 1519	-18.5	-18.5 0.39 (1)	10.00			
N-M	0 / 1519	-18.5	-18.5 0.39 (1)	10.00			
M-L	0 / 1487	-18.5	-18.5 0.38 (1)	10.00			

TOTAL WEIGHT = 2 X 163 = 326 lb

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, OBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 085-09, CSA 086-14
- TPIG 2011, TPIG 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSK: TC=0.73/1.00 (D-E-1), BC=0.42/1.00 (O-P-1), WB=0.97/1.00 (I-L-1), SS=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 518 354 1667 788 1987 1668

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (I) (INPUT = 0.90)
JSI METAL = 0.56 (I) (INPUT = 1.00)



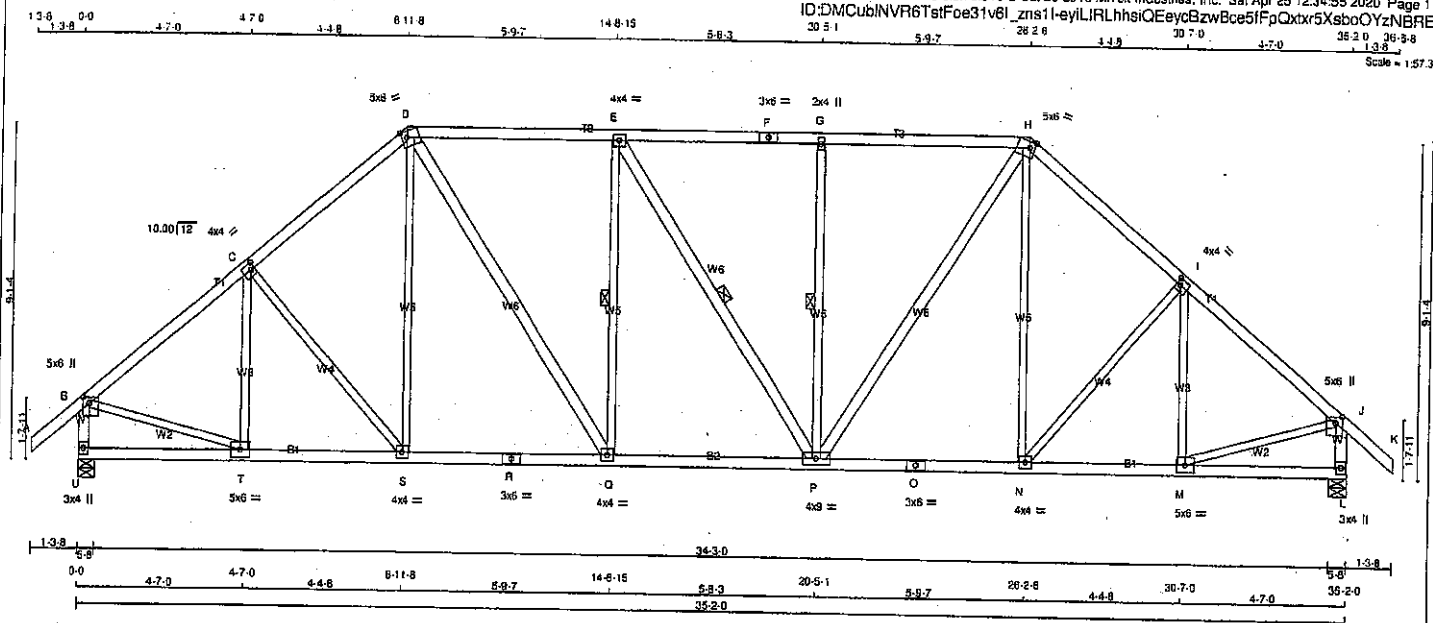
Structural component only
DWG# T-2007134

JOB NAME 408168	TRUSS NAME T6	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
U - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
U - R	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
D - Q	2x4	DRY	No.2
E - P	2x4	DRY	No.2
P - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	5.0	6.0	2.00	2.00
C TMVW-w	MT20	4.0	4.0	2.00	1.25
D TTWW-m	MT20	5.0	6.0	2.00	1.75
E TMVW-t	MT20	4.0	4.0		
F TS-t	MT20	3.0	4.0		
G TMVW-w	MT20	5.0	6.0		
H TTWW-m	MT20	5.0	6.0	2.00	1.75
I TMVW-t	MT20	4.0	4.0	2.00	1.25
J TMVW-p	MT20	5.0	6.0	2.00	2.00
L BMV1-p	MT20	3.0	4.0		
M BMVW-t	MT20	5.0	6.0		
N, Q, S					
N BMVW-t	MT20	4.0	4.0		
O BS-t	MT20	3.0	4.0		
P BMVW-w	MT20	4.0	4.0		
R BS-t	MT20	3.0	4.0		
T BMVW-t	MT20	5.0	6.0		
U BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
U	2086 0	2086 0	5-8	5-8
L	2066 0	2066 0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
U	1458	971 / 0	0 / 0	0 / 0	488 / 0	0 / 0
L	1458	971 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, E-P, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO						FR-TO			
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	T-C	-357 / 0	0.16 (1)	
B-C	-1894 / 0	-91.8	-91.8	0.30 (1)	4.53	C-S	-121 / 0	0.10 (1)	
C-D	-1957 / 0	-91.8	-91.8	0.29 (1)	4.57	S-D	0 / 189	0.05 (4)	
D-E	-1923 / 0	-91.8	-91.8	0.40 (1)	4.44	D-Q	0 / 810	0.13 (1)	
E-F	-1921 / 0	-91.8	-91.8	0.37 (1)	4.49	Q-E	-589 / 0	0.31 (1)	
F-G	-1921 / 0	-91.8	-91.8	0.37 (1)	4.49	E-P	-3 / 0	0.00 (1)	
G-H	-1921 / 0	-91.8	-91.8	0.40 (1)	4.45	P-G	-688 / 0	0.31 (1)	
H-I	-1957 / 0	-91.8	-91.8	0.29 (1)	4.57	P-H	0 / 807	0.13 (1)	
I-J	-1894 / 0	-91.8	-91.8	0.50 (1)	4.53	N-H	0 / 189	0.05 (4)	
J-K	0 / 41	-91.8	-91.8	0.13 (1)	10.00	N-I	-121 / 0	0.10 (1)	
U-B	-2028 / 0	0.0	0.0	0.22 (1)	5.94	M-I	-358 / 0	0.16 (1)	
L-J	-2028 / 0	0.0	0.0	0.22 (1)	5.94	B-T	0 / 1817	0.36 (1)	
U-T	0 / 0	-18.5	-18.5	0.09 (4)	10.00	M-J	0 / 1817	0.36 (1)	
T-S	0 / 1555	-18.5	-18.5	0.30 (1)	10.00				
S-R	0 / 1477	-18.5	-18.5	0.30 (1)	10.00				
R-Q	0 / 1477	-18.5	-18.5	0.30 (1)	10.00				
Q-P	0 / 1923	-18.5	-18.5	0.38 (1)	10.00				
P-O	0 / 1478	-18.5	-18.5	0.30 (1)	10.00				
O-N	0 / 1478	-18.5	-18.5	0.30 (1)	10.00				
N-M	0 / 1555	-18.5	-18.5	0.31 (1)	10.00				
M-L	0 / 0	-18.5	-18.5	0.09 (4)	10.00				

TOTAL WEIGHT = 2 X 182 = 363 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.40/1.00 (D-E-1), BC=0.38/1.00 (P-Q-1), WB=0.38/1.00 (B-T-1), SS=0.25/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1857 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

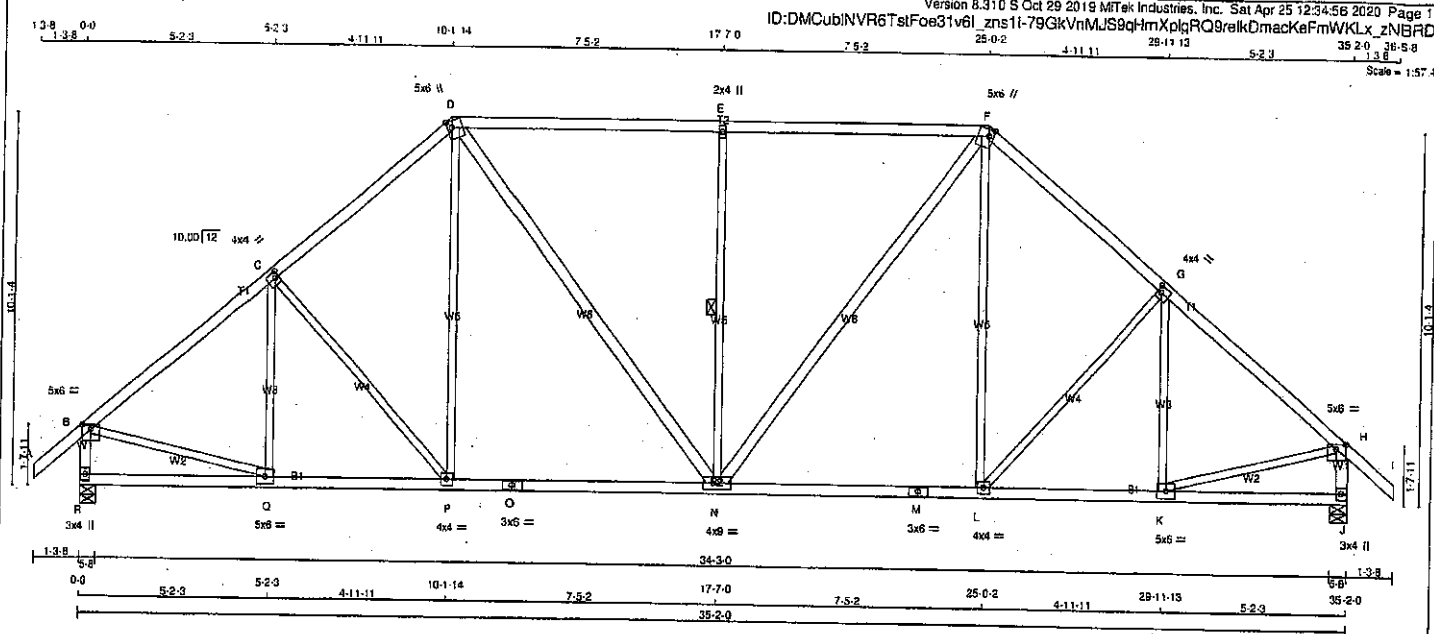
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.50 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007135

JOB NAME 408168 Tamarack Roof Truss, Burlington	TRUSS NAME T7	QUANTITY 5	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW-p	MT20	5.0	6.0	1.50	3.00
J	BMVt-p	MT20	3.0	4.0		
K	BMVW-t	MT20	5.0	6.0		
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	8.0		
N	BMVW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	5.0	6.0		
R	BMVt-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
R	2066	0	2066	0	5-8	5-8
J	2066	0	2066	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
J	1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.96 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8 -91.8	0.13 (1)	10.00	Q-C	-302 / 0	0.17 (1)
B-C	-2021 / 0	-91.8 -91.8	0.39 (1)	4.41	C-P	-217 / 0	0.25 (1)
C-D	-1908 / 0	-91.8 -91.8	0.37 (1)	4.52	P-D	0 / 278	0.06 (1)
D-E	-1809 / 0	-91.8 -91.8	0.74 (1)	3.96	D-N	0 / 615	0.10 (1)
E-F	-1809 / 0	-91.8 -91.8	0.74 (1)	3.96	N-E	-638 / 0	0.58 (1)
F-G	-1808 / 0	-91.8 -91.8	0.37 (1)	4.52	N-F	0 / 615	0.10 (1)
G-H	-2021 / 0	-91.8 -91.8	0.39 (1)	4.41	L-F	0 / 278	0.06 (1)
H-I	0 / 41	-91.8 -91.8	0.13 (1)	10.00	L-G	-217 / 0	0.25 (1)
R-B	-2024 / 0	0.0 0.0	0.22 (1)	5.94	K-G	-302 / 0	0.17 (1)
J-H	-2024 / 0	0.0 0.0	0.22 (1)	5.94	B-Q	0 / 1629	0.37 (1)
					K-H	0 / 1629	0.37 (1)
R-Q	0 / 0	-18.5 -18.5	0.10 (4)	10.00			
Q-P	0 / 1580	-18.5 -18.5	0.34 (1)	10.00			
P-O	0 / 1437	-18.5 -18.5	0.35 (1)	10.00			
O-N	0 / 1437	-18.5 -18.5	0.35 (1)	10.00			
N-M	0 / 1437	-18.5 -18.5	0.35 (1)	10.00			
M-L	0 / 1437	-18.5 -18.5	0.35 (1)	10.00			
L-K	0 / 1580	-18.5 -18.5	0.34 (1)	10.00			
K-J	0 / 0	-18.5 -18.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 085-09, CSA 085-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17)
CALCULATED VERT. DEFL.(LL) = L/989 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (1.17)
CALCULATED VERT. DEFL.(TL) = L/989 (0.17")

CSI: TC=0.74/1.00 (D-E:1), BC=0.35/1.00 (L-N:1), WB=0.58/1.00 (E-N:1), SS=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

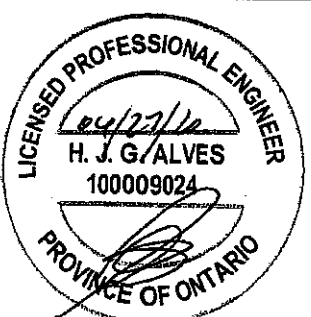
MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 788 1937 1668

PLATE PLACEMENT TOL. = 0.250 inches

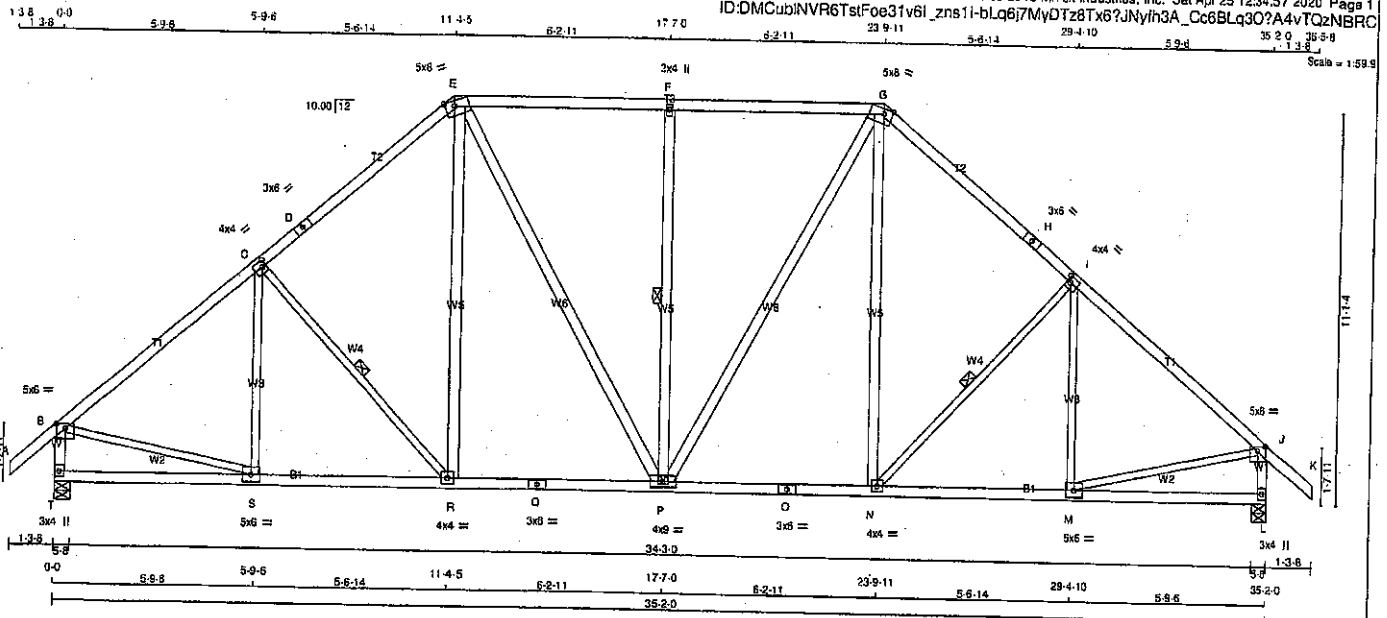
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.46 (D) (INPUT = 1.00)



Structural component only
DWG# T-2007136

JOB NAME 408168	TRUSS NAME T8	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
I - L	2x4	DRY	No.2	SPF	
M - P	2x4	DRY	No.2	SPF	
Q - R	2x4	DRY	No.2	SPF	
S - T	2x4	DRY	No.2	SPF	
U - V	2x4	DRY	No.2	SPF	
W - X	2x4	DRY	No.2	SPF	
Y - Z	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF	
S - C	2x3	DRY	No.2	SPF	
C - R	2x3	DRY	No.2	SPF	
N - I	2x3	DRY	No.2	SPF	
M - I	2x3	DRY	No.2	SPF	
B - S	2x3	DRY	No.2	SPF	
M - J	2x3	DRY	No.2	SPF	

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMW-p	MT20	5.0	6.0	1.50	3.00
C TMW-W-I	MT20	4.0	4.0	2.00	1.25
D TS-I	MT20	3.0	6.0		
E TTW-W-m	MT20	5.0	8.0	Edge	3.00
F TMW-W	MT20	2.0	4.0		
G TTW-W-m	MT20	5.0	8.0	Edge	3.00
H TS-I	MT20	3.0	6.0		
I TMW-W-I	MT20	4.0	4.0	2.00	1.25
J TMW-p	MT20	5.0	6.0	1.50	3.00
L BMW-I-p	MT20	3.0	4.0		
M BMW-W-I	MT20	5.0	6.0		
N BMW-W-I	MT20	4.0	4.0		
O BS-I	MT20	3.0	6.0		
P BMW-W-W-I	MT20	4.0	9.0		
Q BS-I	MT20	3.0	6.0		
R BMW-W-I	MT20	4.0	4.0		
S BMW-W-I	MT20	5.0	6.0		
T BMW-I-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORZ	DOWN	HORZ	UPLIFT
T	2066	0	2066	0
L	2066	0	2066	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
T	1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0	0 / 0
L	1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-R, F-P, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO		
A-B	0 / 41	-91.8	-91.8 0.13 (1)	S-C	-245 / 10	0.17 (1)
B-C	-2039 / 0	-91.8	-91.8 0.50 (1)	C-R	-314 / 0	0.15 (1)
C-D	-1844 / 0	-91.8	-91.8 0.46 (1)	E-F	0 / 338	0.05 (1)
D-E	-1844 / 0	-91.8	-91.8 0.46 (1)	F-P	0 / 489	0.08 (1)
E-F	-1621 / 0	-91.8	-91.8 0.50 (1)	P-F	-699 / 0	0.45 (1)
F-G	-1621 / 0	-91.8	-91.8 0.50 (1)	G-H	0 / 489	0.08 (1)
G-H	-1844 / 0	-91.8	-91.8 0.46 (1)	H-I	0 / 338	0.05 (1)
H-I	-1844 / 0	-91.8	-91.8 0.46 (1)	I-J	-314 / 0	0.15 (1)
I-J	-2039 / 0	-91.8	-91.8 0.50 (1)	J-K	-245 / 10	0.17 (1)
J-K	0 / 41	-91.8	-91.8 0.13 (1)	K-L	0 / 1637	0.37 (1)
K-L	-2021 / 0	0.0	0.0 0.21 (1)	L-M	0 / 1637	0.37 (1)
L-M	-2021 / 0	0.0	0.0 0.21 (1)			
T-S	0 / 0	-18.5	-18.5 0.14 (4)			
S-R	0 / 1597	-18.5	-18.5 0.33 (1)			
R-Q	0 / 1387	-18.5	-18.5 0.30 (1)			
Q-P	0 / 1387	-18.5	-18.5 0.30 (1)			
P-O	0 / 1387	-18.5	-18.5 0.30 (1)			
O-N	0 / 1387	-18.5	-18.5 0.30 (1)			
N-M	0 / 1597	-18.5	-18.5 0.33 (1)			
M-L	0 / 0	-18.5	-18.5 0.14 (4)			

TOTAL WEIGHT = 2 X 190 = 380 LB

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 085-00, CSA 085-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.17")
CALCULATED VERT. DEFL. (LL) = $L/599$ (0.07")
ALLOWABLE DEFL. (TL) = $L/360$ (1.17")
CALCULATED VERT. DEFL. (TL) = $L/599$ (0.14")

CSI: TC=0.50/1.00 (E-F:1), BC=0.33/1.00 (M-N:1), WB=0.45/1.00 (F-P:1), SS=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

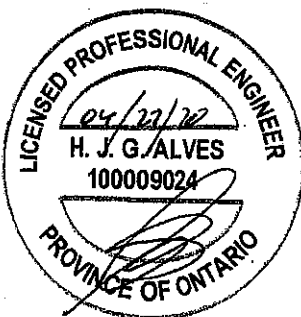
NAIL VALUES

PLATE GRIP (DRY)	SHEAR SECTION
(PSI)	(PLI)
MAX MIN MAX MIN MAX MIN	
MT20	618 354 1667 768 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.47 (O) (INPUT = 1.00)

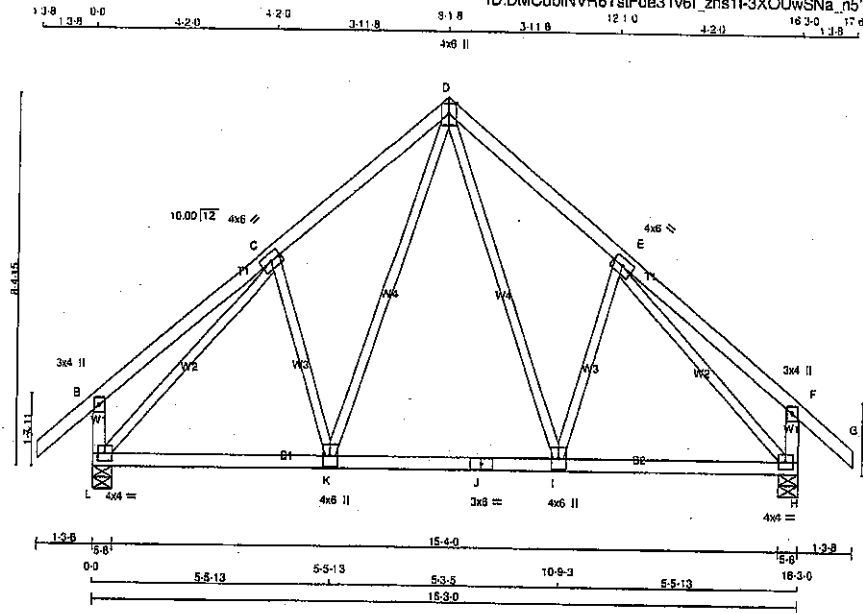


Structural component only
DWG# T-2007137

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408168	T9	6	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Sat Apr 25 12:34:58 2020 Page 1
ID:DMCubINVR6TstFoe31v6l_zns1t-3XOUwSNa_n5?55hBt5UwEGJD_0Uq4EhYDqpS?tzNBRB



Scale: 1/4"=1'

TOTAL WEIGHT = 6 X 80 = 481 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	8.0		
D	TMVW-p	MT20	4.0	8.0		
E	TMVW-t	MT20	4.0	8.0		Edge
F	TMV-p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMVW-t	MT20	4.0	8.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	8.0		
L	BMVW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UP	IN-SX
L	1023	0	0	5-8
H	1023	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	721	487 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0
H	721	487 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8	-91.8 0.13 (1)	D-I	0 / 330	0.07 (1)	
B-C	0 / 28	-91.8	-91.8 0.24 (1)	I-E	-216 / 0	0.09 (1)	
C-D	-745 / 0	-91.8	-91.8 0.19 (1)	K-D	0 / 330	0.07 (1)	
D-E	-745 / 0	-91.8	-91.8 0.19 (1)	C-K	-216 / 0	0.09 (1)	
E-F	0 / 28	-91.8	-91.8 0.24 (1)	L-C	-943 / 0	0.61 (1)	
F-G	0 / 41	-91.8	-91.8 0.13 (1)	E-H	-943 / 0	0.61 (1)	
L-B	-267 / 0	0.0	0.0 0.03 (1)				
H-F	-267 / 0	0.0	0.0 0.03 (1)				
L-K	0 / 616	-18.5	-18.5 0.17 (4)				
K-J	0 / 449	-18.5	-18.5 0.16 (4)				
J-I	0 / 449	-18.5	-18.5 0.16 (4)				
I-H	0 / 616	-18.5	-18.5 0.17 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	38.0	PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ASCE 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.54')
CALCULATED VERT. DEFL.(LL) = L/999 (0.02')
ALLOWABLE DEFL.(TL) = L/360 (0.54')
CALCULATED VERT. DEFL.(TL) = L/999 (0.04')

CSI: TC=0.24/1.00 (B-C:1), BC=0.17/1.00 (H-I:4), WB=0.81/1.00 (C-L:1), SSI=0.14/1.00 (C-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

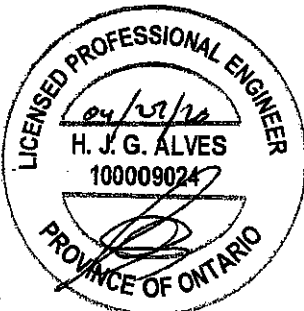
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	518 354 1667 788 1987 1655	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (C) (INPUT = 0.90)
JSI METAL= 0.25 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007138

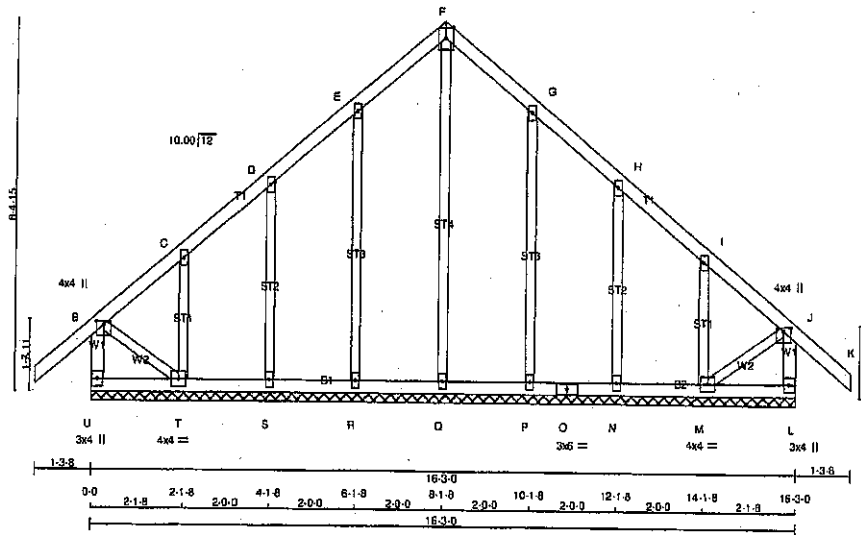
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408168	G9	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6l_zns1l-b4h8J9HDFpZxKJkpJ9gVTzANmNytCAu40SVMwzNBRT

Scale: 1/4"=1'



TOTAL WEIGHT = 81 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - F	2x4	DRY No.2
F - K	2x4	DRY No.2
U - B	2x4	DRY No.2
L - J	2x4	DRY No.2
U - O	2x4	DRY No.2
O - L	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I					
C TMVW+w	MT20	2.0	4.0		
F TTW+p	MT20	4.0	6.0	Edge	
J TMVW+p	MT20	4.0	4.0	1.00	2.00
L BMV1+p	MT20	3.0	4.0		
M BMVW1-t	MT20	4.0	4.0		
N, P, Q, R, S					
N BMV1+w	MT20	2.0	4.0		
O BS-t	MT20	3.0	6.0		
T BMVW1-t	MT20	4.0	4.0		
U BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (LC)	MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 41	-91.8 -91.8	0.13 (1)	10.00	Q-F	-124 / 0	0.17 (1)
B-C	-28 / 0	-91.8 -91.8	0.05 (1)	6.25	R-E	-210 / 0	0.16 (1)
C-D	-36 / 0	-91.8 -91.8	0.05 (1)	6.25	S-D	-169 / 0	0.08 (1)
D-E	-24 / 0	-91.8 -91.8	0.05 (1)	6.25	T-C	-205 / 0	0.04 (1)
E-F	-36 / 0	-91.8 -91.8	0.05 (1)	6.25	P-G	-210 / 0	0.16 (1)
F-G	-36 / 0	-91.8 -91.8	0.05 (1)	6.25	N-H	-169 / 0	0.06 (1)
G-H	-24 / 0	-91.8 -91.8	0.05 (1)	6.25	M-I	-205 / 0	0.04 (1)
H-I	-36 / 0	-91.8 -91.8	0.05 (1)	6.25	B-T	0 / 37	0.01 (1)
I-J	-28 / 0	-91.8 -91.8	0.05 (1)	6.25	M-J	0 / 37	0.01 (1)
J-K	0 / 41	-91.8 -91.8	0.13 (1)	10.00			
U-B	-246 / 0	0.0 0.0	0.03 (1)	7.81			
L-J	-246 / 0	0.0 0.0	0.03 (1)	7.81			
U-T	0 / 0	-18.5 -18.5	0.02 (4)	10.00			
T-S	0 / 25	-18.5 -18.5	0.02 (4)	10.00			
S-R	0 / 21	-18.5 -18.5	0.02 (4)	10.00			
R-Q	0 / 18	-18.5 -18.5	0.02 (4)	10.00			
Q-P	0 / 18	-18.5 -18.5	0.02 (4)	10.00			
P-O	0 / 21	-18.5 -18.5	0.02 (4)	10.00			
O-N	0 / 21	-18.5 -18.5	0.02 (4)	10.00			
N-M	0 / 25	-18.5 -18.5	0.02 (4)	10.00			
M-L	0 / 0	-18.5 -18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 6.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (M-N:4), WB=0.17/1.00 (F-Q:1), SS=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

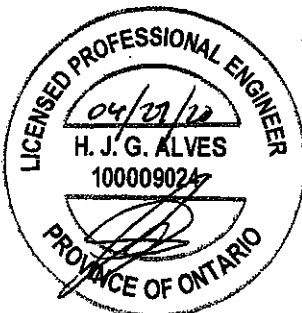
PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
	1667	768
	1967	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.52 (T) (INPUT = 0.90)

JSI METAL= 0.11 (G) (INPUT = 1.00)

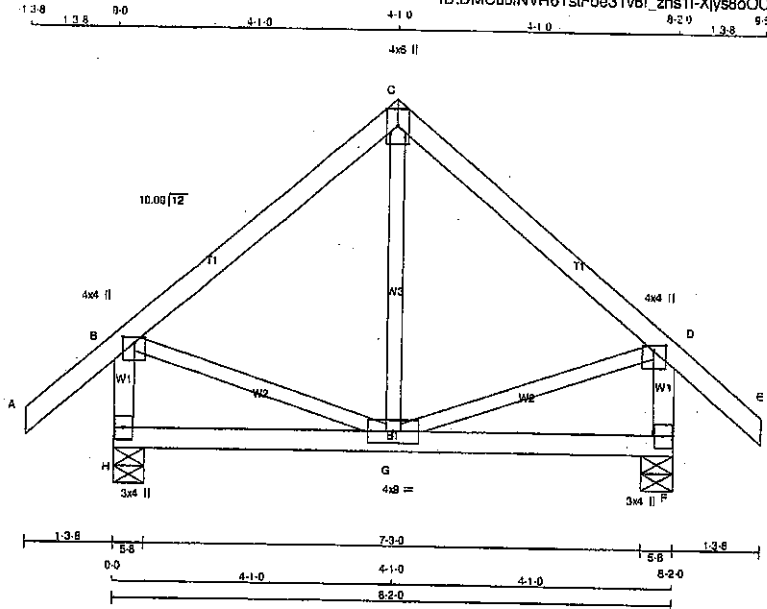


Structural component only
DWG# T-2007123

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408168	T10	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6l_zns1l-Xjys8oOC4DsiFFNqo77nUGOUQrOpqnhSUZ?XJzNBRA



Scale = 1/32"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2		
C - E	2x4	DRY	No.2		
H - B	2x4	DRY	No.2		
F - D	2x4	DRY	No.2		
H - F	2x4	DRY	No.2		

ALL WEBS 2x3 DRY No.2

EXCEPT

PLATES (table in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00	
C	TTW+p	MT20	4.0	6.0	Edge		
D	TMVW+p	MT20	4.0	4.0	1.00	2.00	
F	BMV1+p	MT20	3.0	4.0			
G	BMVWW-t	MT20	4.0	9.0			
H	BMV1+p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	HORZ	DOWN	UP/LIFT
H	577	0	577	0
F	577	0	577	0

UNFACTORED REACTIONS							
	1ST LOASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	406	280 / 0	0 / 0	0 / 0	0 / 0	126 / 0	0 / 0
F	406	280 / 0	0 / 0	0 / 0	0 / 0	126 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. (PLF)	CS1 (LC)	UNBRAC	LENGTH	FR-TO	MEMB.	FORCE (LBS)
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	G-C	-37 / 57	0.02 (4)
B-C	-264 / 0	-91.8	-91.8	0.20 (1)	6.25	B-G	0 / 213	0.05 (1)
C-D	-264 / 0	-91.8	-91.8	0.20 (1)	6.25	G-D	0 / 213	0.05 (1)
D-E	0 / 41	-91.8	-91.8	0.13 (1)	10.00			
H-B	-548 / 0	0.0	0.0	0.06 (1)	7.81			
F-D	-548 / 0	0.0	0.0	0.06 (1)	7.81			
H-G	0 / 0	-18.5	-18.5	0.09 (4)	10.00			
G-F	0 / 0	-18.5	-18.5	0.09 (4)	10.00			

TOTAL WEIGHT = 38 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CS1: TC=0.20/1.00 (B-C:1), BC=0.09/1.00 (F-G:4), WB=0.05/1.00 (D-G:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

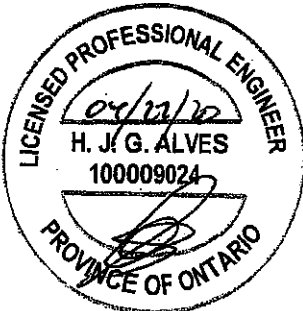
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 789 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (D) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)

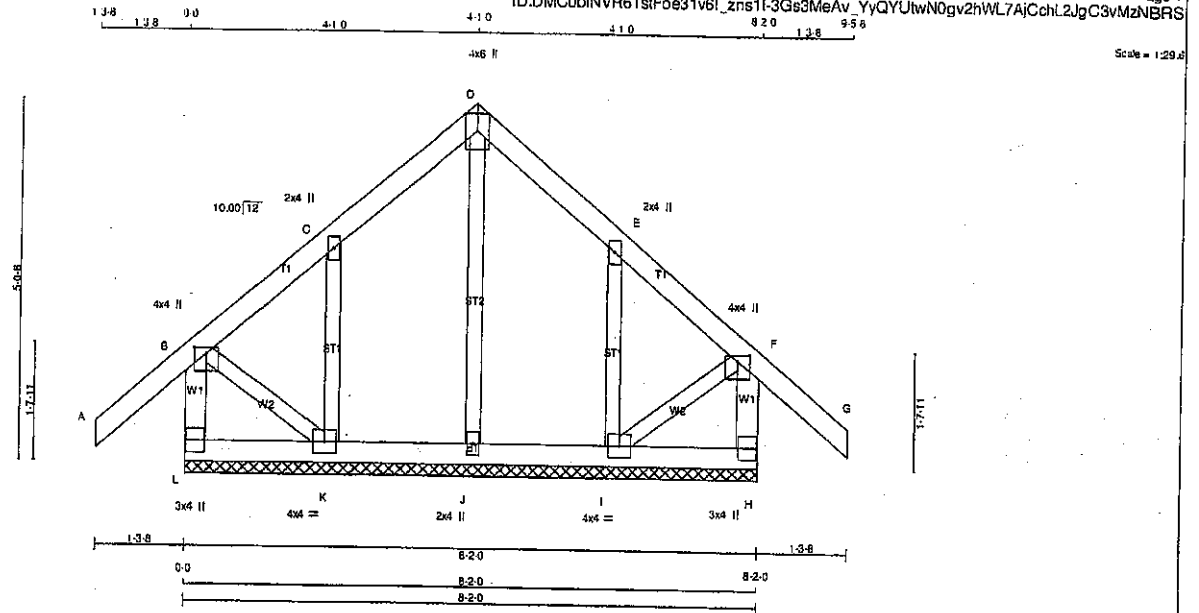


Structural component only
DWG# T-2007139

JOB NAME 408168	TRUSS NAME G10	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6l_zns1l-3Gs3MeAv_YyQYUtwN0gv2hWL7AjCchL2JgC3vMzNBBS



Scale = 1/20

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
L - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - H	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" O.C.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C TMVW+w	MT20	2.0	4.0		
D TTW+p	MT20	4.0	8.0	Edges	
E TMVW+w	MT20	2.0	4.0		
F TMVW+p	MT20	4.0	4.0	1.00	2.00
H BMV1+p	MT20	3.0	4.0		
I BMVW1+p	MT20	4.0	4.0		
J BMV1+w	MT20	2.0	4.0		
K BMVW1+p	MT20	4.0	4.0		
L BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LG1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED LG1 MAX (LC)	UNBRACED LENGTH (FT)
FR-TO		FROM	TO	FR-TO			
L-B	-224 / 0	0.0	0.0	0.02 (1)	J-D	-128 / 0	0.05 (1)
A-B	0 / 41	-91.8	-91.8	0.13 (1)	K-C	-227 / 0	0.04 (1)
B-C	-11 / 0	-91.8	-91.8	0.06 (1)	I-E	-227 / 0	0.04 (1)
C-D	-31 / 0	-91.8	-91.8	0.08 (1)	B-K	0 / 24	0.01 (1)
D-E	-31 / 0	-91.8	-91.8	0.08 (1)	I-F	0 / 24	0.01 (1)
E-F	-11 / 0	-91.8	-91.8	0.08 (1)			
F-G	0 / 41	-91.8	-91.8	0.13 (1)			
H-F	-224 / 0	0.0	0.0	0.02 (1)			
L-K	0 / 0	-18.5	-18.5	0.02 (4)			
K-J	0 / 13	-18.5	-18.5	0.02 (4)			
J-I	0 / 13	-18.5	-18.5	0.02 (4)			
I-H	0 / 0	-18.5	-18.5	0.02 (4)			

TOTAL WEIGHT = 43 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2018 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (J-K:4), WB=0.05/1.00 (D-J:1), SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

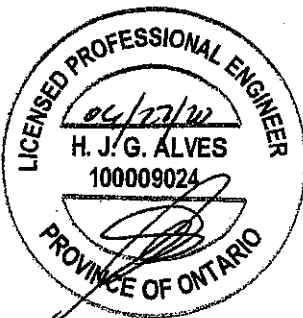
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1687 788 1937 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (C) (INPUT = 0.80)

JSI METAL= 0.12 (C) (INPUT = 1.00)

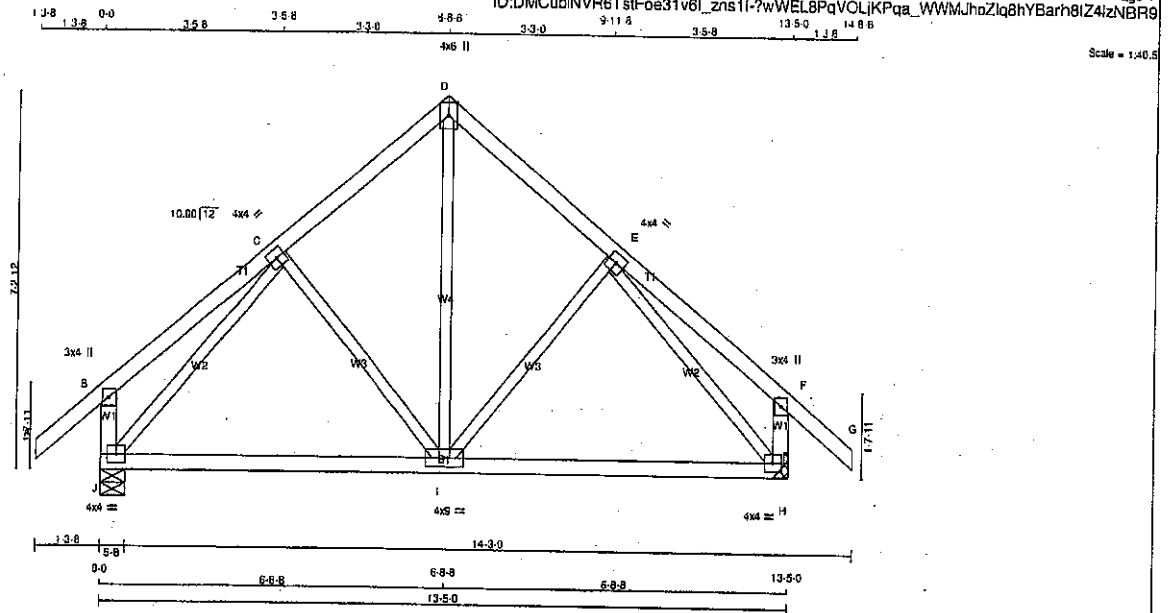


Structural component only
DWG# T-2007124

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408168	T11	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/40.5

TOTAL WEIGHT = 3 X 64 = 191 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVWW1-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
J	867	0	867	0
H	867	0	867	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	611	414 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0
H	611	414 / 0	0 / 0	0 / 0	0 / 0	196 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)	UNBRAC	LENGTH	FR-TO	WEBS	MAX. FACTORED	FORCE	MAX	CS1 (LC)
A-B	0 / 41	-91.8	-91.8	0.13	(1)	10.00	I-D	0 / 382	0.69 (1)					
B-C	0 / 23	-91.8	-91.8	0.16	(1)	10.00	I-E	-149 / 0	0.07 (1)					
C-D	-509 / 0	-91.8	-91.8	0.13	(1)	6.25	C-I	-149 / 0	0.07 (1)					
D-E	-509 / 0	-91.8	-91.8	0.13	(1)	6.25	J-C	-742 / 0	0.33 (1)					
E-F	0 / 23	-91.8	-91.8	0.16	(1)	10.00	E-H	-742 / 0	0.33 (1)					
F-G	0 / 41	-91.8	-91.8	0.13	(1)	10.00								
J-B	-244 / 0	0.0	0.0	0.03	(1)	7.81								
H-F	-244 / 0	0.0	0.0	0.03	(1)	7.81								
J-I	0 / 469	-18.5	-18.5	0.27	(4)	10.00								
I-H	0 / 469	-18.5	-18.5	0.27	(4)	10.00								

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/998 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CS: TC=0.16/1.00 (E-F:1), BC=0.27/1.00 (I-J:4),
WB=0.33/1.00 (E-H:1), SS=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

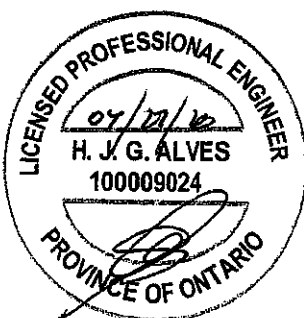
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618	354
	1667	788
	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

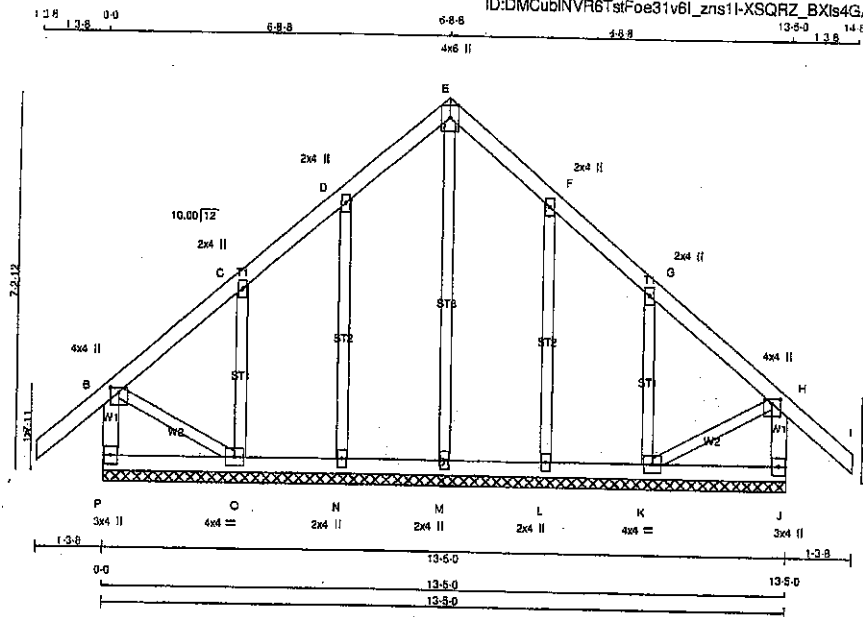
JSI GRIP= 0.84 (E) (INPUT = 0.90)
JSI METAL = 0.27 (E) (INPUT = 1.00)



Structural component only
DWG# T-2007140

JOB NAME 408168	TRUSS NAME G11	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington



Scale = 1/4" = 1'-0"

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LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
P - B	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G					
C TMW+w	MT20	2.0	4.0		
E TTW+p	MT20	4.0	6.0	Edge	
H TMVW+p	MT20	4.0	4.0	1.00	2.00
J BMV1+p	MT20	3.0	4.0		
K BMWW1-i	MT20	4.0	4.0		
L, M, N					
L BMW1+w	MT20	2.0	4.0		
O BMWW1-i	MT20	4.0	4.0		
P BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	LC2 MAX	UNBRAC LENGTH FR-TO	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
P-B	-263 / 0	0.0	0.0	0.09 (1)	7.81	M-E	-128 / 0	0.11 (1)
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	N-D	-190 / 0	0.09 (1)
B-C	-24 / 0	-91.8	-91.8	0.08 (1)	8.25	C-C	-239 / 0	0.08 (1)
C-D	-36 / 0	-91.8	-91.8	0.08 (1)	8.25	L-F	-190 / 0	0.09 (1)
D-E	-35 / 0	-91.8	-91.8	0.05 (1)	8.25	K-G	-239 / 0	0.08 (1)
E-F	-35 / 0	-91.8	-91.8	0.05 (1)	8.25	B-O	0 / 32	0.01 (1)
F-G	-24 / 0	-91.8	-91.8	0.08 (1)	8.25	K-H	0 / 32	0.01 (1)
G-H	-24 / 0	-91.8	-91.8	0.08 (1)	8.25			
H-I	0 / 41	-91.8	-91.8	0.13 (1)	10.00			
J-H	-263 / 0	0.0	0.0	0.03 (1)	7.81			
P-O	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
O-N	0 / 22	-18.5	-18.5	0.03 (4)	10.00			
N-M	0 / 19	-18.5	-18.5	0.02 (4)	10.00			
M-L	0 / 19	-18.5	-18.5	0.02 (4)	10.00			
L-K	0 / 22	-18.5	-18.5	0.03 (4)	10.00			
K-J	0 / 0	-18.5	-18.5	0.03 (4)	10.00			

TOTAL WEIGHT = 65 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
CL = 8.0 PSF
BOT. CH. LL = 0.0 PSF
CL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (H-K:1), BC=0.03/1.00 (K-L:4),
WB=0.11/1.00 (E-M:1), SSI=0.08/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.13 (G) (INPUT = 1.00)

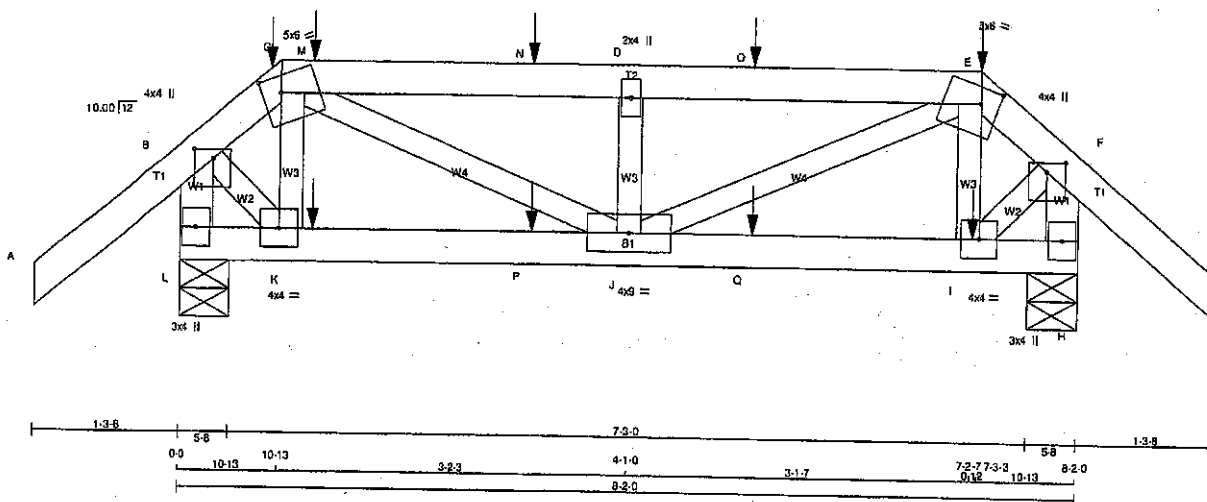
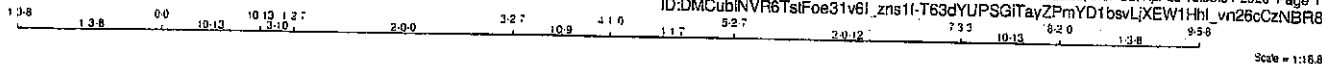


Structural component only
DWG# T-2007125

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408168	T12	1	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW-m	MT20	5.0	8.0	1.75	2.00
D	TMW-w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	1.75	2.00
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV+p	MT20	3.0	4.0		
I	BMVW-i	MT20	4.0	4.0		
J	BMVW-i	MT20	4.0	4.0		
K	BMVW-i	MT20	4.0	4.0		
L	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
L	913	0	913	0
H	911	0	911	0

UNFACTORED REACTIONS

JT	1ST LOSE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
L	642	442 / 0	0 / 0	0 / 0	199 / 0	0 / 0
H	641	437 / 0	0 / 0	0 / 0	204 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LENGTH (FT)	
FR-TO				FR-TO			
A-B	0 / 41	-91.8	10.00	J-D	-487 / 0	0.08 (1)	
B-C	-931 / 0	-91.8	10.00	K-C	-156 / 0	0.02 (1)	
C-M	-1054 / 0	-91.8	10.00	C-J	0 / 700	0.17 (1)	
M-N	-1054 / 0	-91.8	10.00	B-K	0 / 525	0.13 (1)	
N-D	-1054 / 0	-91.8	10.00	I-E	-149 / 0	0.02 (1)	
D-O	-1054 / 0	-91.8	10.00	J-E	0 / 698	0.17 (1)	
O-E	-1054 / 0	-91.8	10.00	I-F	0 / 526	0.13 (1)	
E-F	-632 / 0	-91.8	10.00				
F-G	0 / 41	-91.8	10.00				
L-B	-936 / 0	0.0	10.00				
H-F	-937 / 0	0.0	10.00				
L-K	0 / 0	-18.5	10.00				
K-P	0 / 420	-18.5	10.00				
P-J	0 / 420	-18.5	10.00				
J-Q	0 / 421	-18.5	10.00				
Q-I	0 / 421	-18.5	10.00				
I-H	0 / 0	-18.5	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	10-13	-9	-10	---	FRONT	VERT	DEAD	---	C1
C	1-2-7	-114	-114	---	BACK	VERT	TOTAL	---	C1
C	10-13	-44	-44	---	FRONT	VERT	SNOW	---	C1
E	7-3-3	-9	-10	---	FRONT	VERT	DEAD	---	C1
E	7-3-3	-90	-90	---	BACK	VERT	TOTAL	---	C1
E	7-3-3	-44	-44	---	FRONT	VERT	SNOW	---	C1
K	7-2-7	-49	-49	---	BACK	VERT	TOTAL	---	C1
K	1-2-7	-49	-49	---	BACK	VERT	TOTAL	---	C1
N	3-2-7	-82	-82	---	BACK	VERT	TOTAL	---	C1
O	5-2-7	-82	-82	---	BACK	VERT	TOTAL	---	C1
P	3-2-7	-49	-49	---	BACK	VERT	TOTAL	---	C1
Q	5-2-7	-49	-49	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 34 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDT. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/989 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/989 (0.03")

CSI: TC=0.23/1.00 (C-D:1), BC=0.14/1.00 (I-J:1),
WB=0.17/1.00 (C-J:1), SS=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

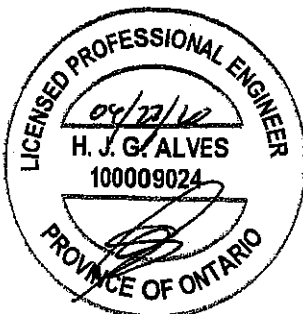
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

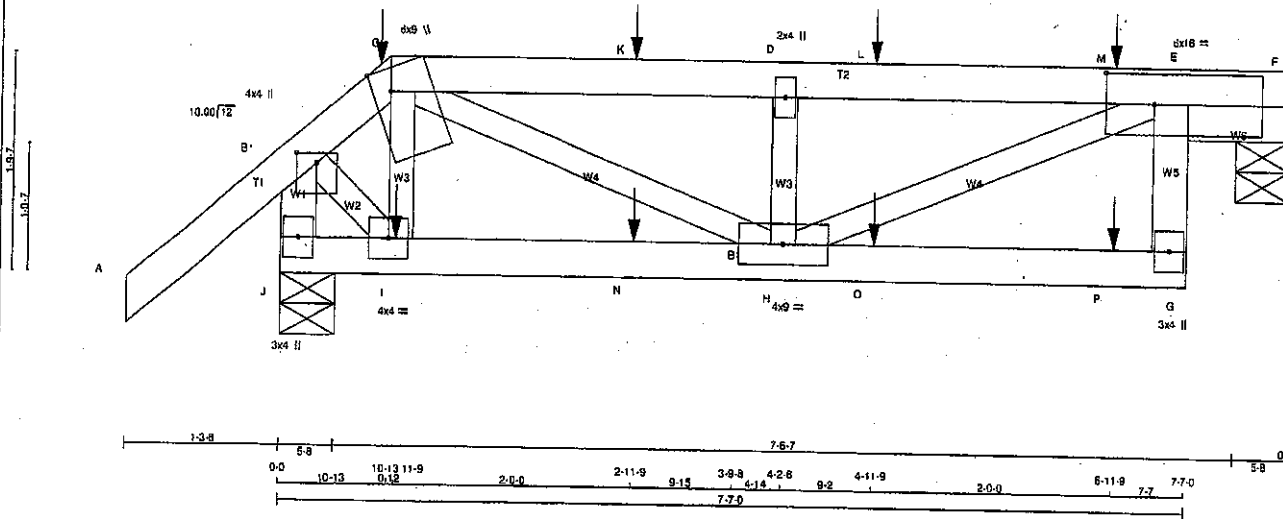
PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



Structural component only
DWG# T-2007141

JOB NAME 408168	TRUSS NAME T13	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					
Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 12:35:03 2020 Page 1 ID:DMCubINVR6TstFoe31v6I_zns11-PVBNzARicJlBsZ9fe33xKQvZ1CWYJHN5XDq4zNBR6					
Scale = 1:17.3					



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF

CHORDS	SIZE	LUMBER	DESCR.	SPF
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C TTWW+m	MT20	6.0	9.0	Edge	1.75
D TMVW-w	MT20	2.0	4.0		
E TMVWW-t	MT20	6.0	16.0	3.00	5.00
G BMV+p	MT20	3.0	4.0		
H BMVWW-t	MT20	4.0	9.0		
I BMVW-t	MT20	4.0	4.0		
J BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ
F	895	0	895	0
J	936	0	936	0

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): F

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS
F	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
F	490	327 / 0 0 / 0 0 / 0 163 / 0 0 / 0
J	859	449 / 0 0 / 0 0 / 0 210 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED HORZ. LOAD (LC)	MAX. FACTORED UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LC)
FR-TO						FR-TO			
A-B	0 / 41	-91.8	-91.8	0.14 (1)	10.00	I-C	-158 / 0	0.03 (1)	
B-C	-858 / 0	-91.8	-91.8	0.14 (1)	6.25	B-I	0 / 548	0.14 (1)	
C-K	-1230 / 0	-91.8	-91.8	0.46 (1)	5.09	H-D	-767 / 0	0.12 (1)	
K-D	-1230 / 0	-91.8	-91.8	0.46 (1)	5.09	C-H	0 / 867	0.21 (1)	
D-L	-1230 / 0	-91.8	-91.8	0.89 (1)	3.56	H-E	0 / 1345	0.33 (1)	
L-M	-1230 / 0	-91.8	-91.8	0.89 (1)	3.56				
M-E	-1230 / 0	-91.8	-91.8	0.89 (1)	3.56				
E-F	0 / 0	-91.8	-91.8	0.35 (1)	10.00				
G-E	0 / 83	0.0	0.0	0.02 (4)	10.00				
J-B	-966 / 0	0.0	0.0	0.11 (1)	7.81				
J-I	0 / 0	-18.5	-18.5	0.06 (1)	10.00				
I-N	0 / 438	-18.5	-18.5	0.14 (1)	10.00				
N-H	0 / 438	-18.5	-18.5	0.14 (1)	10.00				
H-O	0 / 0	-18.5	-18.5	0.09 (1)	10.00				
O-P	0 / 0	-18.5	-18.5	0.09 (1)	10.00				
P-G	0 / 0	-18.5	-18.5	0.09 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	10-13	-9	-10	---	---	FRONT	VERT	DEAD	---	C1
C	10-13	-90	-90	---	---	FRONT	VERT	TOTAL	---	C1
C	10-13	-44	-44	---	---	FRONT	VERT	SNOW	---	C1
I	11-9	-49	-49	---	---	FRONT	VERT	TOTAL	---	C1
K	2-11-9	-82	-82	---	---	FRONT	VERT	TOTAL	---	C1
L	4-11-9	-82	-82	---	---	FRONT	VERT	TOTAL	---	C1
M	8-11-9	-82	-82	---	---	FRONT	VERT	TOTAL	---	C1
N	2-11-9	-49	-49	---	---	FRONT	VERT	TOTAL	---	C1
O	4-11-9	-48	-48	---	---	FRONT	VERT	TOTAL	---	C1
P	6-11-9	-53	-53	---	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 32 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 8.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.25")
CALCULATED VERT. DEFL. (LL) = L/999 (0.07")
ALLOWABLE DEFL. (TL) = L/360 (0.25")
CALCULATED VERT. DEFL. (TL) = L/588 (0.13")

CSI: TC=0.89/1.00 (D-E-1), BC=0.14/1.00 (H-I-1),
WB=0.33/1.00 (E-H-1), SS=0.42/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

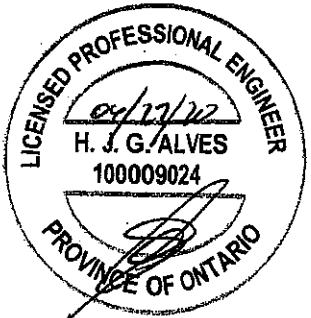
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1997 1658

PLATE PLACEMENT TOL. = 0.250 inches

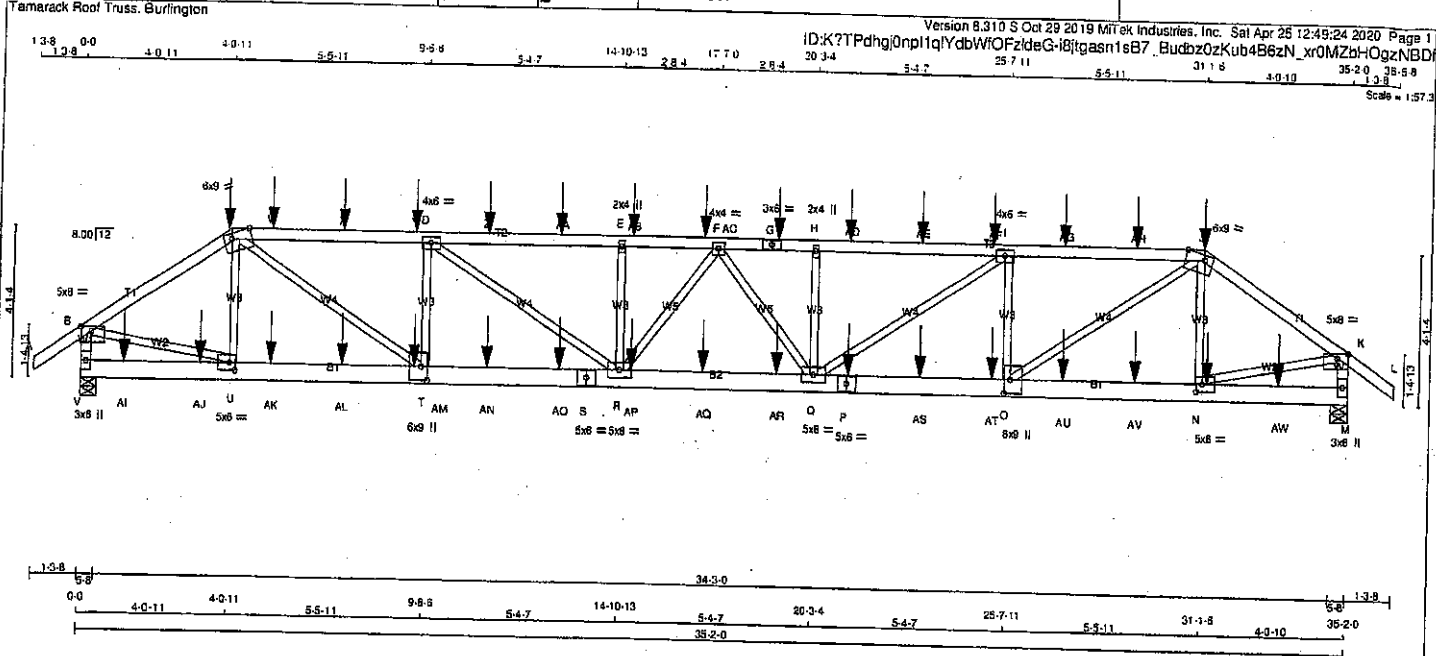
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.79 (C) (INPUT = 0.50)
JSI METAL = 0.30 (H) (INPUT = 1.00)

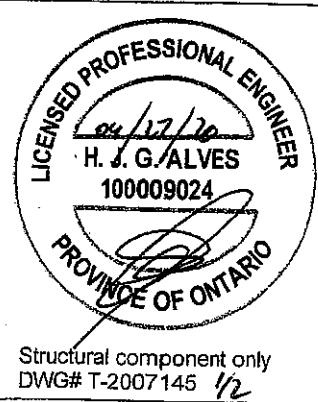


Structural component only
DWG# T-2007142

JOB NAME 408169	TRUSS NAME T20	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x4 DRY No.2 SPF C - G 2x4 DRY No.2 SPF G - J 2x4 DRY No.2 SPF J - L 2x4 DRY No.2 SPF V - B 2x4 DRY No.2 SPF M - K 2x4 DRY No.2 SPF S - P 2x6 DRY No.2 SPF P - M 2x6 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF) TOP CHORDS : (0.122"x3") SPIRAL NAILS C 1 12 SIDE(61.0) G 1 12 SIDE(61.0) J 1 12 SIDE(61.0) L 1 12 SIDE(61.0) K 1 12 TOP M 1 12 TOP BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS S 2 12 SIDE(183.1) P 2 12 SIDE(183.1) M 2 12 SIDE(183.1) WEBS : (0.122"x3") SPIRAL NAILS 3 1 6 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT BRG REQD BRG JT VERT 3342 V HORZ 3393 M 0 0 0 5-8 5-8 UNFACTORED REACTIONS 1ST LOASE MAX/MIN. COMPONENT REACTIONS JT COMBINED V SNOW 2363 1555 / 0 M LIVE 2397 1585 / 0 PERM. LIVE 0 / 0 WIND 0 / 0 DEAD 806 / 0 SOIL 812 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.03 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD (LBS) MAX. UNBRACED LENGTH (FT) WEBS MAX. FACTORED FORCE (LBS) MAX. UNBRACED LENGTH (FT) FR-TO
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Structural component only
 DWG# T-2007145 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	T20	1	2	GREEN PARK HOMES	

Tamareck Roof Truss, Burlington

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PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	8.0	1.50	3.50
C	TTWW-m	MT20	6.0	9.0	Edge 6.25	
D	TMWV-t	MT20	4.0	6.0		
E	TMW-w	MT20	2.0	4.0		
F	TMWV-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW-w	MT20	2.0	4.0		
I	TMWV-t	MT20	4.0	6.0		
J	TTWW-m	MT20	6.0	9.0	Edge 6.25	
K	TMWV-p	MT20	5.0	8.0	1.50	3.50
M	BMV1-p	MT20	3.0	6.0		
N	BMWV-t	MT20	5.0	6.0	2.50	2.00
O	BMWV-t	MT20	6.0	9.0	4.50	2.00
P	BS-t	MT20	5.0	6.0		
Q	BMWV-t	MT20	5.0	8.0		
R	BMWV-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
T	BMWV-t	MT20	6.0	9.0	4.50	2.00
U	BMWV-t	MT20	5.0	6.0	2.50	2.00
V	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CS1 (LC)	MAX. UNBRAC	MEMB. FORCE (LBS)	MAX. FACTORED CS1 (LC)
FR-TO		FROM TO			LENGTH FR-TO		
AS-AT	0 / 6217	-18.5	-18.5	0.46 (1)	10.00		
AT-O	0 / 6217	-18.5	-18.5	0.46 (1)	10.00		
O-AU	0 / 3326	-18.5	-18.5	0.26 (1)	10.00		
AU-AV	0 / 3326	-18.5	-18.5	0.26 (1)	10.00		
AV-N	0 / 3326	-18.5	-18.5	0.26 (1)	10.00		
N-AW	0 / 0	-18.5	-18.5	0.04 (4)	10.00		
AW-M	0 / 0	-18.5	-18.5	0.04 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-49	-55	---	FRONT	VERT	DEAD	C1
C	4-0-11	-254	-254	---	FRONT	VERT	SNOW	C1
G	19-2-12	-110	-110	---	BACK	VERT	TOTAL	C1
J	31-1-6	-49	-55	---	FRONT	VERT	DEAD	C1
J	31-1-6	-117	-117	---	BACK	VERT	TOTAL	C1
J	31-1-6	-254	-254	---	FRONT	VERT	SNOW	C1
N	31-2-12	-26	-26	---	BACK	VERT	TOTAL	C1
P	21-2-12	-26	-26	---	BACK	VERT	TOTAL	C1
W	5-2-12	-110	-110	---	BACK	VERT	TOTAL	C1
X	7-2-12	-110	-110	---	BACK	VERT	TOTAL	C1
Y	9-2-12	-110	-110	---	BACK	VERT	TOTAL	C1
Z	11-2-12	-110	-110	---	BACK	VERT	TOTAL	C1
AA	13-2-12	-110	-110	---	BACK	VERT	TOTAL	C1
AB	15-2-12	-110	-110	---	BACK	VERT	TOTAL	C1
AC	17-2-12	-110	-110	---	BACK	VERT	TOTAL	C1
AD	21-2-12	-110	-110	---	BACK	VERT	TOTAL	C1
AE	23-2-12	-110	-110	---	BACK	VERT	TOTAL	C1
AF	25-2-12	-110	-110	---	BACK	VERT	TOTAL	C1
AG	27-2-12	-110	-110	---	BACK	VERT	TOTAL	C1
AH	29-2-12	-110	-110	---	BACK	VERT	TOTAL	C1
AI	1-2-12	-26	-26	---	BACK	VERT	TOTAL	C1
AJ	3-2-12	-26	-26	---	BACK	VERT	TOTAL	C1
AK	5-2-12	-26	-26	---	BACK	VERT	TOTAL	C1
AL	7-2-12	-26	-26	---	BACK	VERT	TOTAL	C1
AM	9-2-12	-26	-26	---	BACK	VERT	TOTAL	C1
AN	11-2-12	-26	-26	---	BACK	VERT	TOTAL	C1
AO	13-2-12	-26	-26	---	BACK	VERT	TOTAL	C1
AP	15-2-12	-26	-26	---	BACK	VERT	TOTAL	C1
AQ	17-2-12	-26	-26	---	BACK	VERT	TOTAL	C1
AR	19-2-12	-26	-26	---	BACK	VERT	TOTAL	C1
AS	23-2-12	-26	-26	---	BACK	VERT	TOTAL	C1
AT	25-2-12	-26	-26	---	BACK	VERT	TOTAL	C1
AU	27-2-12	-26	-26	---	BACK	VERT	TOTAL	C1
AV	29-2-12	-26	-26	---	BACK	VERT	TOTAL	C1
AW	33-2-12	-26	-26	---	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007145

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	T20Z	1	2	GREEN PARK HOMES	

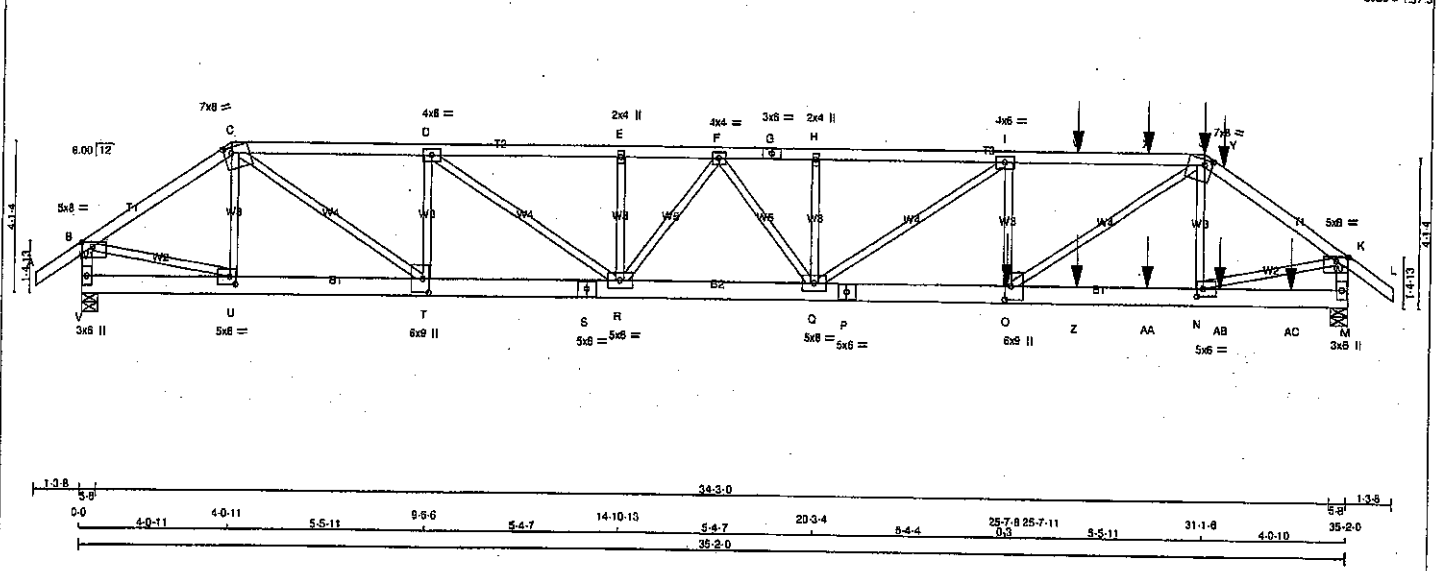
Tamarack Roof Truss, Burlington

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ID:K7TPdngj0npl1qYdbWOfzdeG-AKHFWuPc9K_cKTp9gXC67DKWF2jK9bCKqW7zNBDB

1:3.8 0.0 4.0-11 4.0-11 5.5-11 9.5-6 5.4-7 14-10-13 2.8-4 2.8-4 20.3-4 25.7-11 27.6-12 29.6-12 31.6-31 8-12 35.2-0 36.5-8 1:3.8

Scale = 1:57.3



LUMBER N. L. G. A. RULES CHORDS SIZE A - C 2x4 DRY No.2 C - G 2x4 DRY No.2 G - J 2x4 DRY No.2 J - L 2x4 DRY No.2 V - B 2x4 DRY No.2 M - K 2x4 DRY No.2 V - S 2x6 DRY No.2 S - P 2x6 DRY No.2 P - M 2x6 DRY No.2 ALL WEBS 2x3 DRY No.2 EXCEPT DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) TOP CHORDS : (0.122"x3") SPIRAL NAILS A-C 1 12 TOP C-G 1 12 TOP G-J 1 12 SIDE(61.0) J-L 1 12 SIDE(61.0) V-B 1 12 TOP M-K 1 12 TOP BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS V-S 2 12 TOP S-P 2 12 TOP P-M 2 12 SIDE(183.1) WEBS : (0.122"x3") SPIRAL NAILS 2x3 1 6 SIDE(557.9) F-O 1 3 D-T 1 3 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.			DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQ'D GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX V 2938 0 2938 0 0 5-8 5-8 M 4796 0 4796 0 0 5-8 5-8 UNFACTORED REACTIONS 1ST LOASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERMLIVE WIND DEAD SOIL V 2073 1388 / 0 0 / 0 0 / 0 884 / 0 0 / 0 M 3381 2278 / 0 0 / 0 0 / 0 1103 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.57 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 (PLF) MAX. CSI (LC) MAX. UNBRAC LENGTH FR-TO A-B 0 / 35 -91.8 -91.8 0.07 (1) 10.00 U-C -564 / 0 0.07 (1) B-C -3442 / 0 -91.8 -91.8 0.20 (1) 4.86 C-T 0 / 3700 0.48 (1) C-D -5905 / 0 -91.8 -91.8 0.43 (1) 3.70 T-D -1983 / 0 0.25 (1) D-E -7947 / 0 -91.8 -91.8 0.58 (1) 3.11 D-R 0 / 2485 0.31 (1) E-F -7947 / 0 -91.8 -91.8 0.31 (1) 3.30 R-E -368 / 0 0.05 (1) F-G -9222 / 0 -91.8 -91.8 0.38 (1) 3.01 Q-H -346 / 0 0.04 (1) G-H -9222 / 0 -91.8 -91.8 0.38 (1) 3.01 Q-I -503 / 0 0.19 (1) H-I -9222 / 0 -91.8 -91.8 0.77 (1) 2.73 O-I -397 / 0 0.05 (1) I-W -9635 / 0 -91.8 -91.8 0.88 (1) 2.57 C-J 0 / 5778 0.71 (1) W-X -9635 / 0 -91.8 -91.8 0.88 (1) 2.57 N-J -915 / 0 0.12 (1) X-J -9635 / 0 -91.8 -91.8 0.88 (1) 2.57 B-U 0 / 2840 0.36 (1) J-Y -5847 / 0 -91.8 -91.8 0.33 (1) 3.78 N-K 0 / 5020 0.62 (1) Y-K -5847 / 0 -91.8 -91.8 0.33 (1) 3.78 R-F -1209 / 0 0.21 (1) K-L 0 / 35 -91.8 -91.8 0.07 (1) 10.00 F-Q 0 / 974 0.12 (1) V-B -2914 / 0 0.0 0.0 0.16 (1) 6.74 M-K -4771 / 0 0.0 0.0 0.27 (1) 5.48 V-U 0 / 0 -18.5 -18.5 0.03 (4) 10.00 U-T 0 / 2843 -18.5 -18.5 0.21 (1) 10.00 T-S 0 / 5906 -18.5 -18.5 0.44 (1) 10.00 S-R 0 / 5906 -18.5 -18.5 0.44 (1) 10.00 R-Q 0 / 9635 -18.5 -18.5 0.62 (1) 10.00 Q-P 0 / 9635 -18.5 -18.5 0.62 (1) 10.00 P-O 0 / 9635 -18.5 -18.5 0.72 (1) 10.00 O-Z 0 / 4855 -18.5 -18.5 0.41 (1) 10.00 Z-AA 0 / 4855 -18.5 -18.5 0.41 (1) 10.00 AA-N 0 / 4855 -18.5 -18.5 0.41 (1) 10.00 N-AB 0 / 0 -18.5 -18.5 0.06 (4) 10.00 AB-AC 0 / 0 -18.5 -18.5 0.06 (4) 10.00 AC-M 0 / 0 -18.5 -18.5 0.06 (4) 10.00 FACTORED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX- MAX+ FACE DIR. TYPE HEEL CONN. J 31-1-6 -49 -55 --- FRONT VERT DEAD --- C1 J 31-1-6 -254 -254 --- FRONT VERT SNOW --- C1 O 25-7-8 -2840 -2840 --- FRONT VERT TOTAL --- C1 W 27-6-12 -110 -110 --- FRONT VERT TOTAL --- C1 X 29-6-12 -110 -110 --- FRONT VERT TOTAL --- C1 Y 31-6-12 -136 -136 --- FRONT VERT TOTAL --- C1 Z 27-6-12 -26 -26 --- FRONT VERT TOTAL --- C1 AA 39-6-12 -26 -26 --- FRONT VERT TOTAL --- C1 AB 31-6-12 -26 -26 --- FRONT VERT TOTAL --- C1 AC 33-6-12 -26 -26 --- FRONT VERT TOTAL --- C1			DESIGN CRITERIA *** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE SPECIFIED LOADS: TOP CH. LL = 25.6 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 38.0 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12 *** NON STANDARD GIRDER *** ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, OBC 2012, ABC 2018 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-08, CSA 086-14 - TPIC 2011, TPIC 2014 (55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL. (LL) = L/350 (1.17") CALCULATED VERT. DEFL. (LL) = L/989 (0.28") ALLOWABLE DEFL. (TL) = L/360 (1.17") CALCULATED VERT. DEFL. (TL) = L/832 (0.51") CSI: TC=0.88/1.00 (J-J:1), BC=0.72/1.00 (O-Q:1), WB=0.71/1.00 (J-Q:1), SSI=0.18/1.00 (J-J:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MT20 618 354 1667 788 1987 1655 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP=0.89 (C) (INPUT = 0.90) JSI METAL=0.87 (S) (INPUT = 1.00)		
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Structural component only
DWG# T-2007146 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	T20Z	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TTWW-m	MT20	7.0	8.0	1.75	2.50
D	TMWW-t	MT20	4.0	6.0		
E	TMW-w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW-w	MT20	2.0	4.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW-m	MT20	7.0	8.0	1.75	2.50
K	TMVW-p	MT20	5.0	8.0	1.50	3.50
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.00
O	BMWW-t	MT20	6.0	9.0	4.50	2.00
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	6.0	9.0	4.50	2.00
U	BMWW-t	MT20	5.0	6.0	2.50	2.00
V	BMV1+p	MT20	3.0	6.0		

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

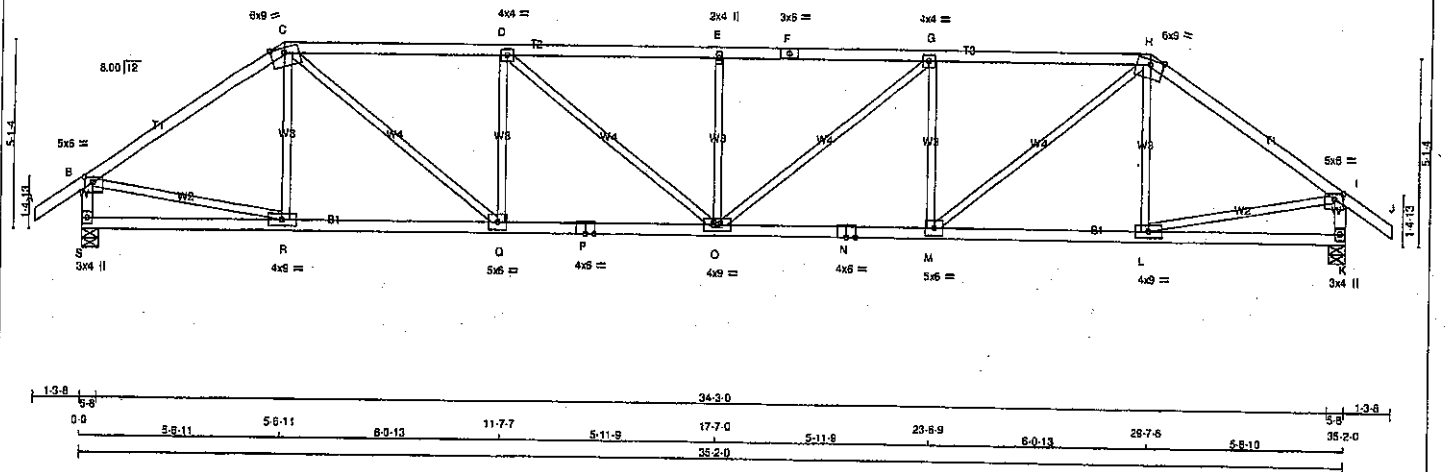


Structural component only
DWG# T-2007146 42

JOB NAME 408169	TRUSS NAME T21	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:57.0



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
S - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
S - P	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT
SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	6.0	1.75 2.75
C	TTWW-m	MT20	6.0	9.0	Edge
D	TMVW-t	MT20	4.0	4.0	
E	TMVW-w	MT20	2.0	4.0	
F	TS-t	MT20	3.0	6.0	
G	TMVW-m	MT20	4.0	4.0	
H	TTWW-m	MT20	6.0	9.0	Edge
I	TMVW-p	MT20	5.0	6.0	1.75 2.75
K	BMV1-p	MT20	3.0	4.0	
L	BMVW-t	MT20	4.0	9.0	
M	BMVW-t	MT20	5.0	6.0	
N	BS-t	MT20	4.0	6.0	
O	BMVW-t	MT20	4.0	9.0	
P	BS-t	MT20	4.0	6.0	
Q	BMVW-t	MT20	5.0	6.0	
R	BMVW-t	MT20	4.0	9.0	
S	BMV1-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	RECORD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
S	2065	0	2065	0	5-8
K	2065	0	2065	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
K	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 2.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
MEMB.	FR-TO	FR-TO	FR-TO	FR-TO	FR-TO	FR-TO	FR-TO	FR-TO	FR-TO	FR-TO	FR-TO	FR-TO	FR-TO
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	R-C	-243 / 10	0.09 (1)					
B-C	-2297 / 0	-91.8	-91.8	0.70 (1)	3.78	C-Q	0 / 1689	0.38 (1)					
C-D	-3213 / 0	-91.8	-91.8	0.50 (1)	3.19	Q-D	-929 / 0	0.36 (1)					
D-E	-3594 / 0	-91.8	-91.8	0.55 (1)	2.97	D-O	0 / 489	0.11 (1)					
E-F	-3594 / 0	-91.8	-91.8	0.55 (1)	2.97	O-E	-505 / 0	0.19 (1)					
F-G	-3594 / 0	-91.8	-91.8	0.55 (1)	2.97	G-G	0 / 490	0.11 (1)					
G-H	-3213 / 0	-91.8	-91.8	0.50 (1)	3.19	H-H	-929 / 0	0.36 (1)					
H-I	-2296 / 0	-91.8	-91.8	0.70 (1)	3.78	I-H	0 / 1670	0.38 (1)					
I-J	0 / 35	-91.8	-91.8	0.12 (1)	10.00	J-H	-243 / 10	0.09 (1)					
S-B	-2023 / 0	0.0	0.0	0.21 (1)	5.94	B-R	0 / 1943	0.44 (1)					
K-I	-2023 / 0	0.0	0.0	0.21 (1)	5.94	I-L	0 / 1943	0.44 (1)					
S-R	0 / 0	-18.5	-18.5	0.15 (4)	10.00								
R-Q	0 / 1904	-18.5	-18.5	0.39 (1)	10.00								
Q-P	0 / 3213	-18.5	-18.5	0.58 (1)	10.00								
P-O	0 / 3213	-18.5	-18.5	0.58 (1)	10.00								
O-N	0 / 3213	-18.5	-18.5	0.58 (1)	10.00								
N-M	0 / 3213	-18.5	-18.5	0.58 (1)	10.00								
M-L	0 / 1903	-18.5	-18.5	0.39 (1)	10.00								
L-K	0 / 0	-18.5	-18.5	0.15 (4)	10.00								

TOTAL WEIGHT = 2 X 140 = 280 LB

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT. CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS DESIGN IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.39")

CSK: TC=0.86/1.00 (E-G:1), BC=0.58/1.00 (O-Q:1), WB=0.44/1.00 (B-R:1), SSI=0.29/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1937 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.97 (B) (INPUT = 0.90)
JSI METAL= 0.74 (P) (INPUT = 1.00)

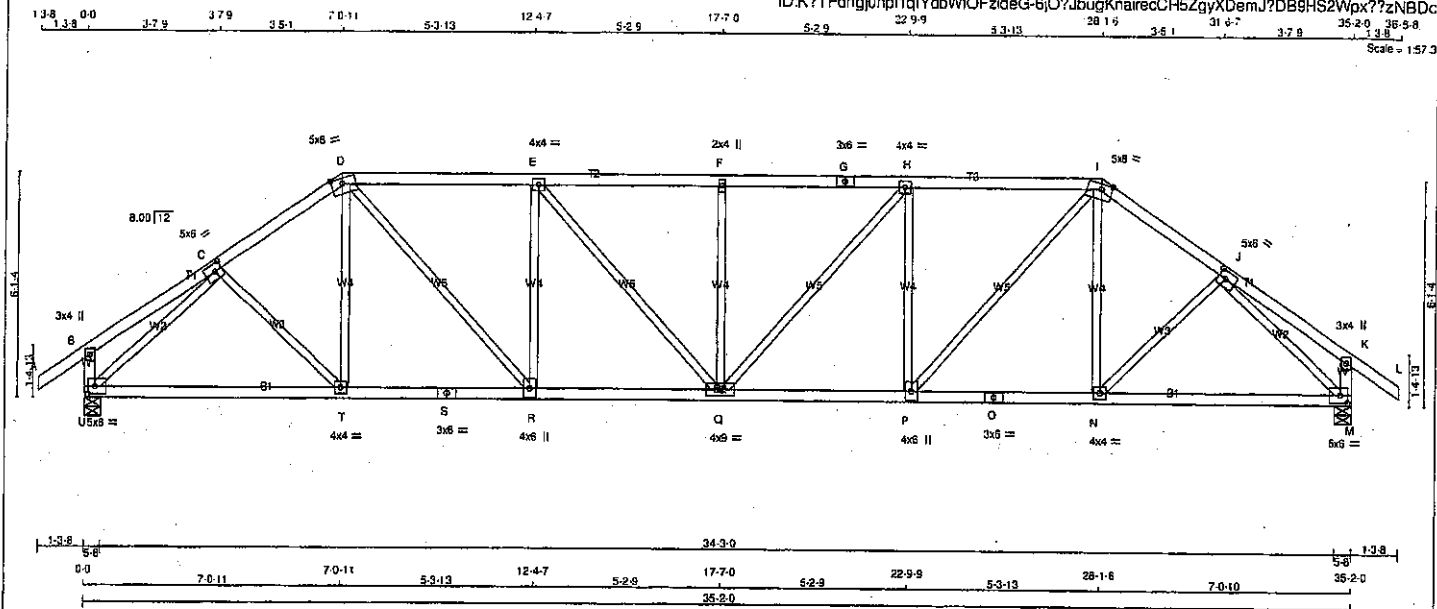


Structural component only
DWG# T-2007 147

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	T22	2	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMWWH	MT20	5.0	8.0	2.50	2.50
D TTWW-m	MT20	5.0	8.0	1.75	3.50
E TMWW-i	MT20	4.0	4.0		
F TMW-w	MT20	2.0	4.0		
G TS-i	MT20	3.0	6.0		
H TMWW-i	MT20	4.0	4.0		
I TTWW-m	MT20	5.0	8.0	1.75	3.50
J TMWW-i	MT20	5.0	8.0	2.50	2.50
K TMV+p	MT20	3.0	4.0		
M BMWW-i	MT20	5.0	6.0	2.50	2.25
N BMWW-i	MT20	4.0	4.0		
O BS-i	MT20	3.0	6.0		
P BMWW-i	MT20	4.0	8.0		
Q BMWW-i	MT20	4.0	9.0		
R BMWW-i	MT20	4.0	6.0		
S BS-i	MT20	3.0	6.0		
T BMWW-i	MT20	4.0	4.0		
U BMWW-i	MT20	5.0	6.0	2.50	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
U	2065	0	0	5-8
M	2065	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
U	1458	970 / 0	0 / 0	0 / 0	488 / 0	0 / 0
M	1458	970 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED		
	FORCE	VERT. LOAD	LC1 MAX		FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)		(LBS)	CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	C-T	0 / 135	0.03 (4)
B-C	0 / 18	-91.8	-91.8 0.16 (1)	10.00	T-D	0 / 90	0.03 (4)
C-D	-2305 / 0	-91.8	-91.8 0.23 (1)	4.31	D-R	0 / 1218	0.27 (1)
D-E	-2723 / 0	-91.8	-91.8 0.54 (1)	3.69	R-E	-814 / 0	0.48 (1)
E-F	-2986 / 0	-91.8	-91.8 0.59 (1)	3.53	E-Q	0 / 363	0.08 (1)
F-G	-2986 / 0	-91.8	-91.8 0.56 (1)	3.53	Q-F	-440 / 0	0.26 (1)
G-H	-2986 / 0	-91.8	-91.8 0.58 (1)	3.53	H-Q	0 / 363	0.08 (1)
H-I	-2723 / 0	-91.8	-91.8 0.54 (1)	3.69	P-H	-814 / 0	0.48 (1)
I-J	-2305 / 0	-91.8	-91.8 0.23 (1)	4.31	P-I	0 / 1218	0.27 (1)
J-K	0 / 18	-91.8	-91.8 0.16 (1)	10.00	N-J	0 / 90	0.03 (4)
K-L	0 / 35	-91.8	-91.8 0.12 (1)	10.00	N-I	0 / 135	0.03 (4)
U-B	-255 / 0	0.0	0.0 0.03 (1)	7.81	U-C	-2515 / 0	0.97 (1)
M-K	-285 / 0	0.0	0.0 0.03 (1)	7.81	J-M	-2514 / 0	0.97 (1)
U-T	0 / 1802	-18.5	-18.5 0.40 (1)	10.00			
T-S	0 / 1901	-18.5	-18.5 0.41 (1)	10.00			
S-R	0 / 1901	-18.5	-18.5 0.41 (1)	10.00			
R-Q	0 / 2724	-18.5	-18.5 0.49 (1)	10.00			
Q-P	0 / 2723	-18.5	-18.5 0.49 (1)	10.00			
P-O	0 / 1900	-18.5	-18.5 0.41 (1)	10.00			
O-N	0 / 1900	-18.5	-18.5 0.41 (1)	10.00			
N-M	0 / 1801	-18.5	-18.5 0.40 (1)	10.00			

TOTAL WEIGHT = 2 X 151 = 303 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CSI: TC=0.55/1.00 (E-F), BC=0.49/1.00 (Q-R), WB=0.97/1.00 (C-U), SS=0.23/1.00 (H-I)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	618	354
	1657	768
	1657	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (M) (INPUT = 0.90)
JSI METAL = 0.63 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007148

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13.8 0.0 4.39 4.39 4.21 3.41 14.75 20.612 28.76 30.97 35.20 38.68
1.39 4.39 4.21 3.41 20.612 28.76 30.97 35.20 38.68
Scale = 1.573



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-10
- TPIC 2011, TPIC 2014

ALLOWABLE DEFL.(LL) = $L/360$ (1.17")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.12")
ALLOWABLE DEFL.(TL) = $L/360$ (1.17")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.24")

CSK: TC=0.64/1.00 (D:E=1), BC=0.47/1.00 (P:Q=1),
WB=0.52/1.00 (E:C=1), SB=0.26/1.00 (D:E=1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT.

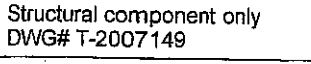
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(FSI)	(PLI)	(PLI)
MT20	818	354	1667
	798	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

(SI) GRIP=0.90 (D) (NAIL) TC=0.90

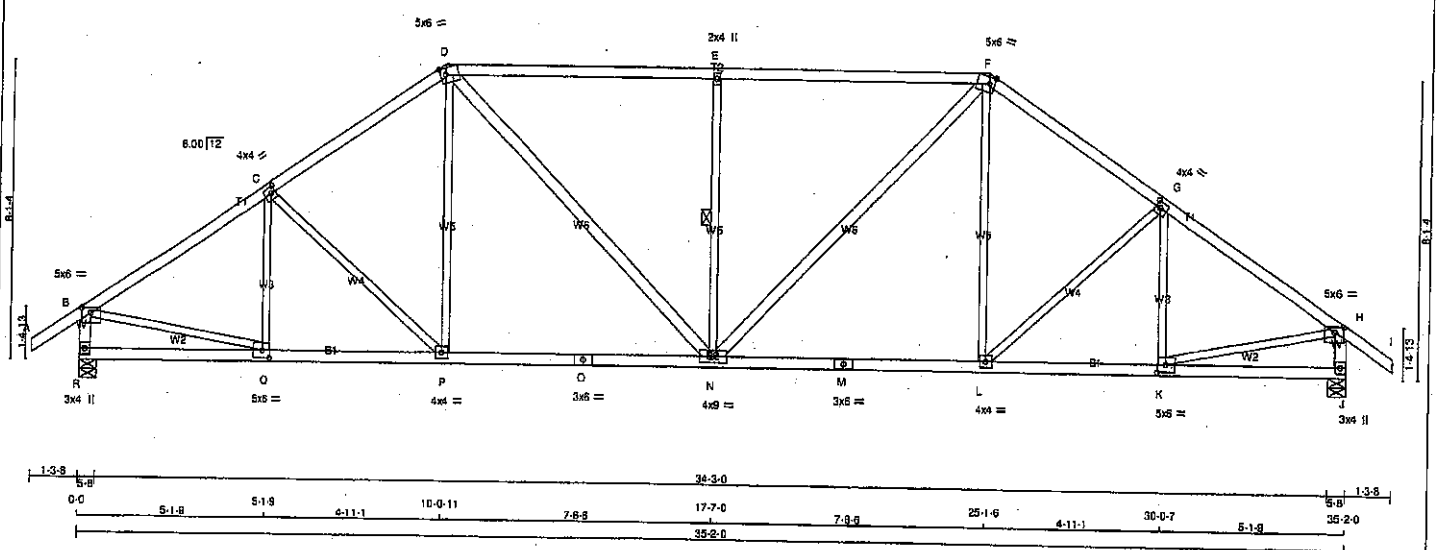


JOB NAME 408169	TRUSS NAME T24	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:57.3



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - B	2x4	DRY	No.2
B - H	2x4	DRY	No.2
H - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	X
B TMVW-p	MT20	5.0	6.0	1.75 2.75
C TMWW-t	MT20	4.0	4.0	2.00 1.50
D TTWW-m	MT20	5.0	6.0	2.25 1.75
E TMW-w	MT20	2.0	4.0	
F TTWW-m	MT20	5.0	6.0	2.25 1.75
G TMWW-t	MT20	4.0	4.0	2.00 1.50
H TMVW-p	MT20	5.0	6.0	1.75 2.75
J BMV1-p	MT20	3.0	4.0	
K BMWW-t	MT20	5.0	6.0	2.50 2.75
L BMWW-t	MT20	4.0	4.0	
M BS-t	MT20	3.0	6.0	
N BMWW-t	MT20	4.0	9.0	
O BS-t	MT20	3.0	6.0	
P BMWW-t	MT20	4.0	4.0	
Q BMWW-t	MT20	5.0	6.0	2.50 2.75
R BMV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX
R	2085 0	2085 0	5-8
J	2085 0	2085 0	5-8

UNFACTORED REACTIONS			
JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS	
R	1458 970/0	0/0	0/0
J	1458 970/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)
A-B	0/35	-91.8	-91.8
B-C	-2318/0	-91.8	-91.8
C-D	-2188/0	-91.8	-91.8
D-E	-2274/0	-91.8	-91.8
E-F	-2274/0	-91.8	-91.8
F-G	-2185/0	-91.8	-91.8
G-H	-2317/0	-91.8	-91.8
H-I	0/35	-91.8	-91.8
I-B	-2022/0	0.0	0.0
J-H	-2022/0	0.0	0.0
R-Q	0/0	-18.5	-18.5
Q-P	0/1953	-18.5	-18.5
P-O	0/1784	-18.5	-18.5
O-N	0/1784	-18.5	-18.5
N-M	0/1784	-18.5	-18.5
M-L	0/1784	-18.5	-18.5
L-K	0/1953	-18.5	-18.5
K-J	0/0	-18.5	-18.5

TOTAL WEIGHT = 2 X 150 = 317 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 085-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.81/1.00 (D-E-1), BC=0.41/1.00 (N-P-1), WB=0.45/1.00 (B-Q-1), SS=0.34/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.64 (M) (INPUT = 1.00)



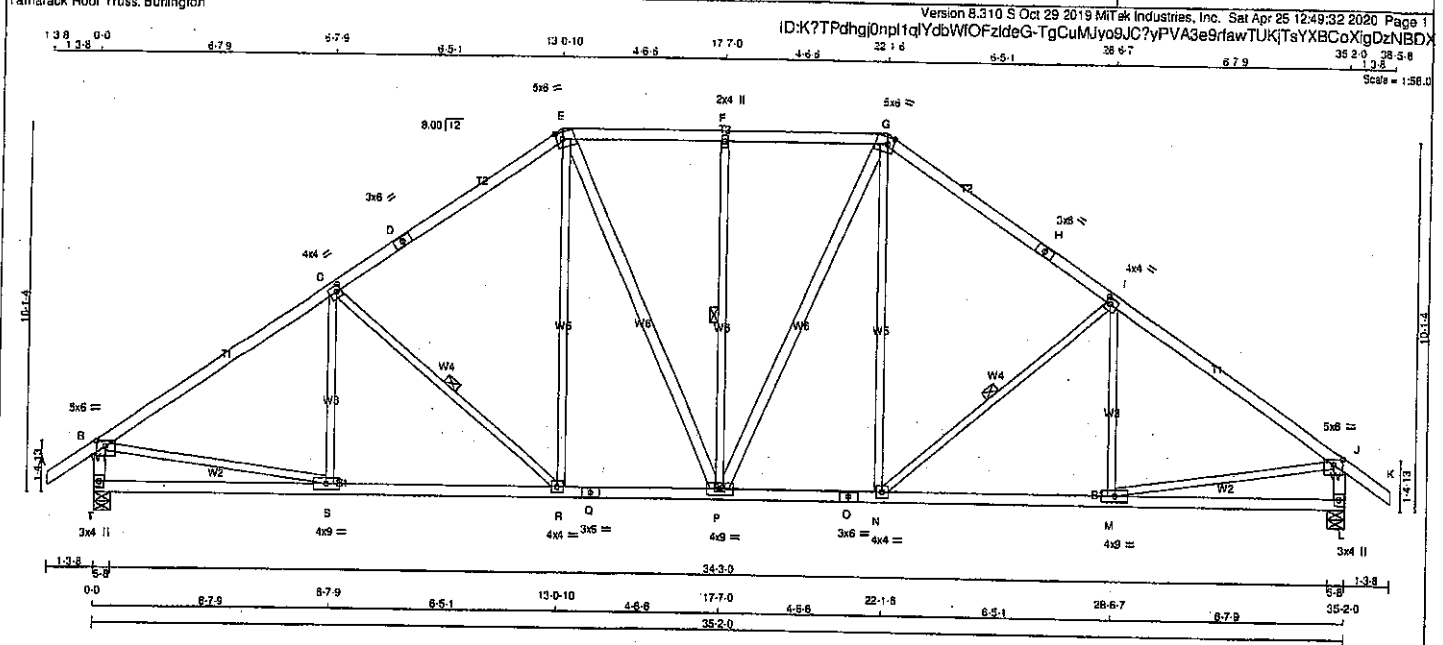
Structural component only
DWG# T-2007150

DAWG NO.	
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JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.56 (M) (INPUT = 1.00)





LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
T - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
T - Q	2x4	DRY	No.2	SPF	
Q - O	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
ALL WEBS		2x3	DRY	No.2	SPF
EXCEPT					
E - P	2x4	DRY	No.2	SPF	
P - G	2x4	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TMVW-i	MT20	4.0	4.0	2.00	1.50
D	TS-i	MT20	3.0	8.0		
E	TTVW-m	MT20	5.0	6.0	2.25	2.00
F	TMVW-w	MT20	2.0	4.0		
G	TTVW-m	MT20	5.0	6.0	2.25	2.00
H	TS-i	MT20	3.0	8.0		
I	TMVW-i	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	6.0	1.75	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMVW-i	MT20	4.0	9.0		
N	BMVW-i	MT20	4.0	4.0		
O	BS-i	MT20	3.0	6.0		
P	BMVWW-i	MT20	4.0	9.0		
Q	BS-i	MT20	3.0	6.0		
R	BMVW-i	MT20	4.0	4.0		
S	BMVW-i	MT20	4.0	9.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT 2065	DOWN 2065	0	5-8
T	HORZ 0	HORZ 0	0	5-8
L	VERT 2065	DOWN 2065	0	5-8

UNFACTORED REACTIONS

	1ST LOASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW 970 / 0	0 / 0	0 / 0	488 / 0	0 / 0
T		970 / 0	0 / 0	0 / 0	488 / 0	0 / 0
L		970 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-R, F-P, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	0 / 35	-91.8	0.12 (1)	10.00	S-C	-189 / 45	0.10 (1)
B-C	-2359 / 0	-91.8	0.69 (1)	3.77	C-R	-485 / 0	0.24 (1)
C-D	-1998 / 0	-91.8	0.62 (1)	4.11	R-E	0 / 416	0.09 (1)
D-E	-1996 / 0	-91.8	0.62 (1)	4.11	E-P	0 / 315	0.05 (1)
E-F	-1761 / 0	-91.8	0.27 (1)	4.77	P-F	-502 / 0	0.35 (1)
F-G	-1761 / 0	-91.8	0.27 (1)	4.77	P-G	0 / 315	0.05 (1)
G-H	-1996 / 0	-91.8	0.62 (1)	4.11	N-G	0 / 415	0.09 (1)
H-I	-1996 / 0	-91.8	0.62 (1)	4.11	N-I	-484 / 0	0.24 (1)
I-J	-2358 / 0	-91.8	0.69 (1)	3.77	M-I	-189 / 45	0.10 (1)
J-K	0 / 35	-91.8	0.12 (1)	10.00	B-S	0 / 2022	0.45 (1)
T-S	-2014 / 0	0.0	0.21 (1)	5.95	M-J	0 / 2021	0.45 (1)
L-J	-2014 / 0	0.0	0.21 (1)	5.95			

T-S	0 / 0	-18.5	-18.5	0.19 (4)	10.00
S-R	0 / 1996	-18.5	-18.5	0.42 (1)	10.00
R-Q	0 / 1629	-18.5	-18.5	0.32 (1)	10.00
Q-P	0 / 1629	-18.5	-18.5	0.32 (1)	10.00
P-O	0 / 1629	-18.5	-18.5	0.32 (1)	10.00
O-N	0 / 1629	-18.5	-18.5	0.32 (1)	10.00
N-M	0 / 1996	-18.5	-18.5	0.42 (1)	10.00
M-L	0 / 0	-18.5	-18.5	0.19 (4)	10.00



Structural component only
DWG# T-2007152

TOTAL WEIGHT = 5 X 172 = 858 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ORC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/998 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/998 (0.18")

CSI: TO=0.69/1.00 (B-C-1), BC=0.42/1.00 (R-S-1),
WB=0.45/1.00 (B-S-1), SS=0.24/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

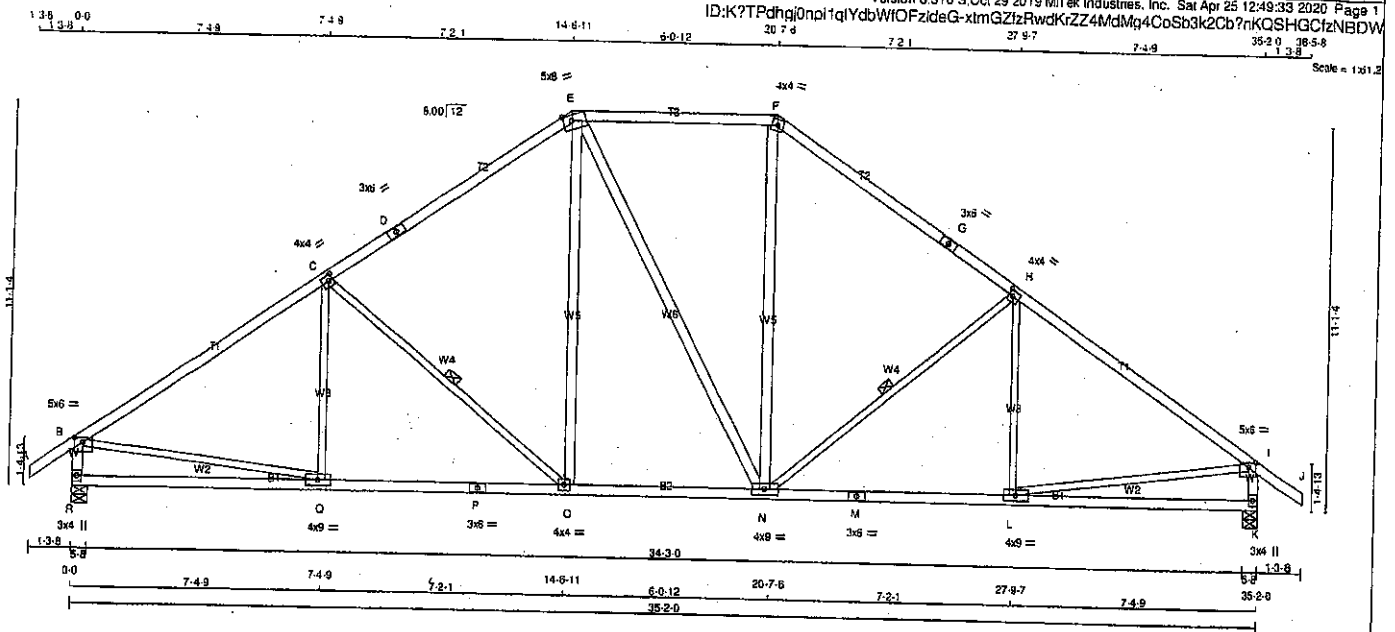
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.50 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	T27	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



LUMBER	SIZE	DRY	LUMBER
N. L. G. A. RULES			
CHORDS			
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - N	2x4	DRY	No.2
N - O	2x4	DRY	No.2
O - P	2x4	DRY	No.2
P - Q	2x4	DRY	No.2
Q - R	2x4	DRY	No.2
R - S	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
O - E	2x4	DRY	No.2
E - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWV-m	MT20	5.0	6.0	2.00	3.00
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-p	MT20	5.0	6.0	1.75	2.75
K	BMV1-p	MT20	3.0	4.0		
L	BMVW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	4.0	6.0		
O	BMVW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	6.0		
Q	BMVW-t	MT20	4.0	6.0		
R	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED		MAXIMUM FACTORED		INPUT	RECORD
	GROSS REACTION	DOWN	GROSS REACTION	UP		
R	2085	0	2085	0	5-8	5-8
K	2085	0	2085	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
R	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
K	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	FORCE	FACTORED	MAX.	MAX.	MEMB.	MAX.	FACTORED	
	(LBS)	VERT. LOAD	CSF (LC)	UNBRAC		FORCE	MAX	
		(PLF)	CSI (LC)	LENGTH		(LBS)	CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	Q-C	-139 / 75	0.09 (1)
B-C	-2358 / 0	-91.8	-91.8	0.89 (1)	3.45	C-O	-605 / 0	0.38 (1)
C-D	-1888 / 0	-91.8	-91.8	0.78 (1)	3.91	O-E	0 / 499	0.08 (1)
D-E	-1888 / 0	-91.8	-91.8	0.78 (1)	3.91	E-N	0 / 0	0.00 (1)
E-F	-1538 / 0	-91.8	-91.8	0.47 (1)	4.74	N-F	0 / 500	0.08 (1)
F-G	-1888 / 0	-91.8	-91.8	0.78 (1)	3.91	N-H	-604 / 0	0.38 (1)
G-H	-1888 / 0	-91.8	-91.8	0.78 (1)	3.91	L-H	-140 / 75	0.09 (1)
H-I	-2357 / 0	-91.8	-91.8	0.89 (1)	3.45	B-Q	0 / 2020	0.45 (1)
I-J	0 / 35	-91.8	-91.8	0.12 (1)	10.00	L-I	0 / 2019	0.45 (1)
R-B	-2009 / 0	0.0	0.0	0.21 (1)	5.95			
K-I	-2009 / 0	0.0	0.0	0.21 (1)	5.95			
R-Q	0 / 0	-18.5	-18.5	0.25 (4)	10.00			
Q-P	0 / 1999	-18.5	-18.5	0.45 (1)	10.00			
P-O	0 / 1999	-18.5	-18.5	0.45 (1)	10.00			
O-N	0 / 1638	-18.5	-18.5	0.33 (1)	10.00			
N-M	0 / 1998	-18.5	-18.5	0.45 (1)	10.00			
M-L	0 / 1998	-18.5	-18.5	0.45 (1)	10.00			
L-K	0 / 0	-18.5	-18.5	0.24 (4)	10.00			

TOTAL WEIGHT = 2 X 166 = 333 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17)
CALCULATED VERT. DEFL.(LL) = L/989 (0.09)
ALLOWABLE DEFL.(TL) = L/360 (1.17)
CALCULATED VERT. DEFL.(TL) = L/989 (0.20)

CSF: TC=0.89/1.00 (B-C:1), SC=0.45/1.00 (C-Q:1), WB=0.45/1.00 (B-Q:1), SS=0.27/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1887 788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

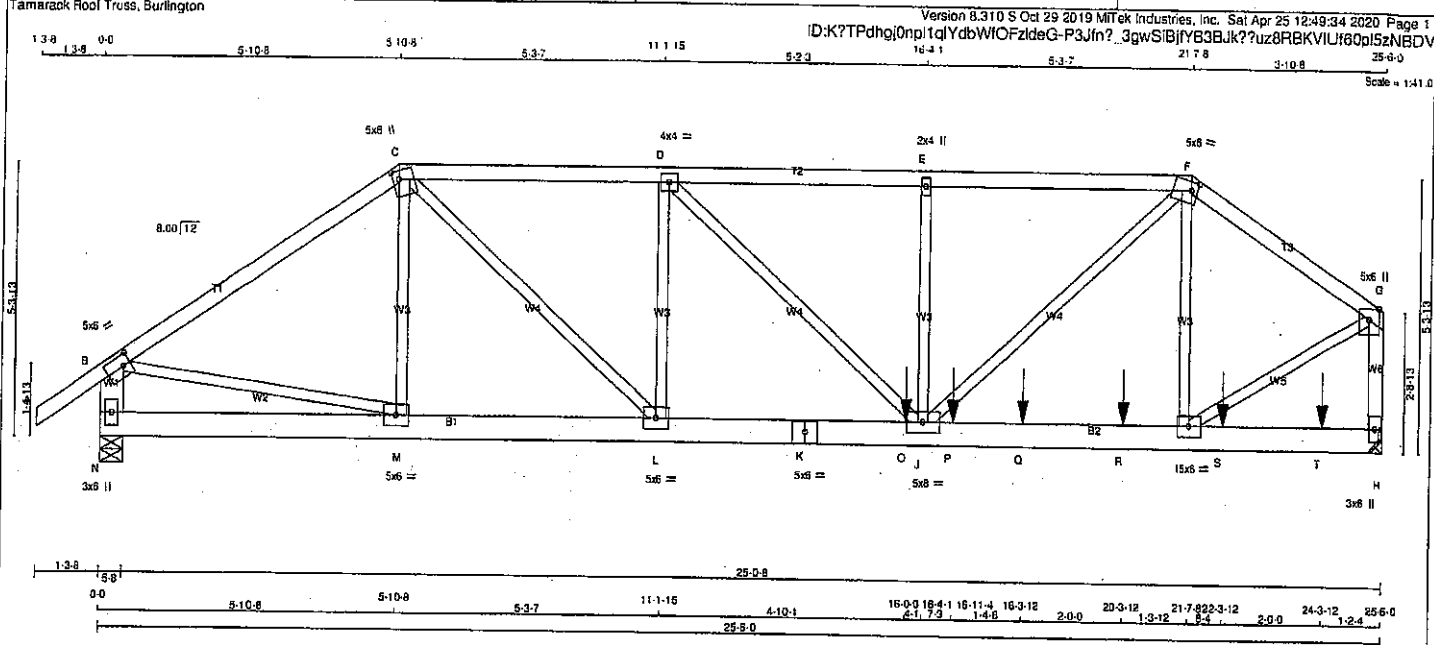
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (F) (INPUT = 0.90)
JSI METAL = 0.69 (F) (INPUT = 1.00)



Structural component only
DWG# T-2007153

JOB NAME 408169	TRUSS NAME T28	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 12:49:34 2020 Page 1			



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
N - B	2x6	DRY	No.2
N - G	2x4	DRY	No.2
N - K	2x6	DRY	No.2
K - H	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - F	12	TOP
F - G	12	TOP
H - G	12	TOP
N - B	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
N - K	12	TOP
K - H	12	TOP
WEBS: (0.122"x3") SPIRAL NAILS		
E - J	6	SIDE(77.0)
2x3	6	SIDE(183.1)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG	BRG	BRG	BRG
JT VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
N	2086	0	2086	0	5-8	5-8	
H	2756	0	2756	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LOOSE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	1470	994 / 0	0 / 0	0 / 0	0 / 0	476 / 0	0 / 0
H	1941	1320 / 0	0 / 0	0 / 0	0 / 0	620 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.76 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-91.8	-91.8 0.07 (1)	10.00	M-C	-218 / 22	0.04 (1)
B-C	-2363 / 0	-91.8	-91.8 0.37 (1)	5.41	C-L	0 / 1550	0.18 (1)
C-D	-3093 / 0	-91.8	-91.8 0.24 (1)	5.03	L-D	-928 / 0	0.19 (1)
D-E	-3519 / 0	-91.8	-91.8 0.25 (1)	4.76	D-J	0 / 589	0.07 (1)
E-F	-3519 / 0	-91.8	-91.8 0.24 (1)	4.79	J-E	-508 / 0	0.10 (1)
F-G	-2352 / 0	-91.8	-91.8 0.16 (1)	5.69	J-F	0 / 2158	0.27 (1)
N-B	-2037 / 0	0.0	0.0 0.07 (1)	7.81	I-F	-548 / 0	0.11 (1)
H-G	-2632 / 0	0.0	0.0 0.19 (1)	7.01	B-M	0 / 1991	0.25 (1)
				I-G	0 / 2264	0.28 (1)	
N-M	0 / 0	-18.5	-18.5 0.04 (4)	10.00			
M-L	0 / 1959	-18.5	-18.5 0.14 (1)	10.00			
L-K	0 / 3083	-18.5	-18.5 0.27 (1)	10.00			
K-O	0 / 3083	-18.5	-18.5 0.27 (1)	10.00			
O-J	0 / 3093	-18.5	-18.5 0.27 (1)	10.00			
J-P	0 / 1942	-18.5	-18.5 0.23 (1)	10.00			
P-Q	0 / 1942	-18.5	-18.5 0.23 (1)	10.00			
Q-R	0 / 1942	-18.5	-18.5 0.23 (1)	10.00			
R-I	0 / 1942	-18.5	-18.5 0.23 (1)	10.00			
I-S	0 / 0	-18.5	-18.5 0.10 (1)	10.00			
S-T	0 / 0	-18.5	-18.5 0.10 (1)	10.00			
T-H	0 / 0	-18.5	-18.5 0.10 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	16-0-0	-997	-997	---	FRONT	VERT	TOTAL	---	C1
P	16-11-4	-187	-187	---	FRONT	VERT	TOTAL	---	C1
Q	18-3-12	-187	-187	---	FRONT	VERT	TOTAL	---	C1
R	20-3-12	-187	-187	---	FRONT	VERT	TOTAL	---	C1
S	22-3-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1
T	24-3-12	-173	-173	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 8.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.85")

CALCULATED VERT. DEFL.(LL) = L/999 (0.06")

ALLOWABLE DEFL.(TL) = L/360 (0.85")

CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CS1: TC=0.37/1.00 (B-C:1), BC=0.27/1.00 (J-L:1), WB=0.28/1.00 (G-I:1), SS=0.34/1.00 (J-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PS) (PL) (PL)

MAX MIN MAX MIN MAX MIN

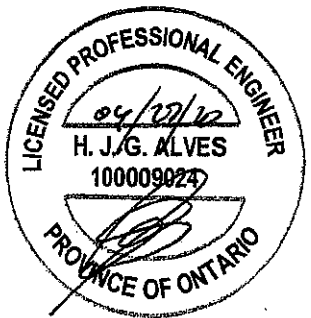
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)

JSI METAL= 0.45 (G) (INPUT = 1.00)



Structural component only

DWG# T-2007154 1/2

JOB NAME 408169	TRUSS NAME T28	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 28 2019 MITek Industries, Inc. Sat Apr 25 12:49:34 2020 Page 2
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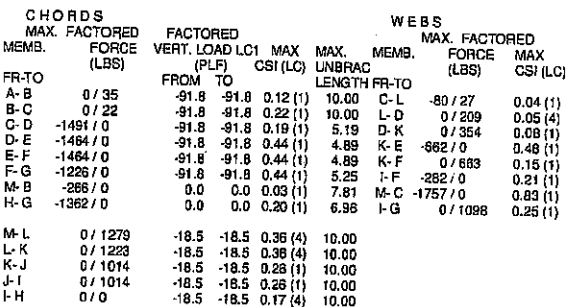
PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.75
C	TTWW-m	MT20	5.0	6.0	2.00	1.50
D	TMVW-t	MT20	4.0	4.0		
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	1.50
G	TMVW-p	MT20	5.0	6.0	Edge	
H	BMV1-p	MT20	3.0	6.0		
I, L, M						
I	BMVW-t	MT20	5.0	6.0		
J	BMVWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
N	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

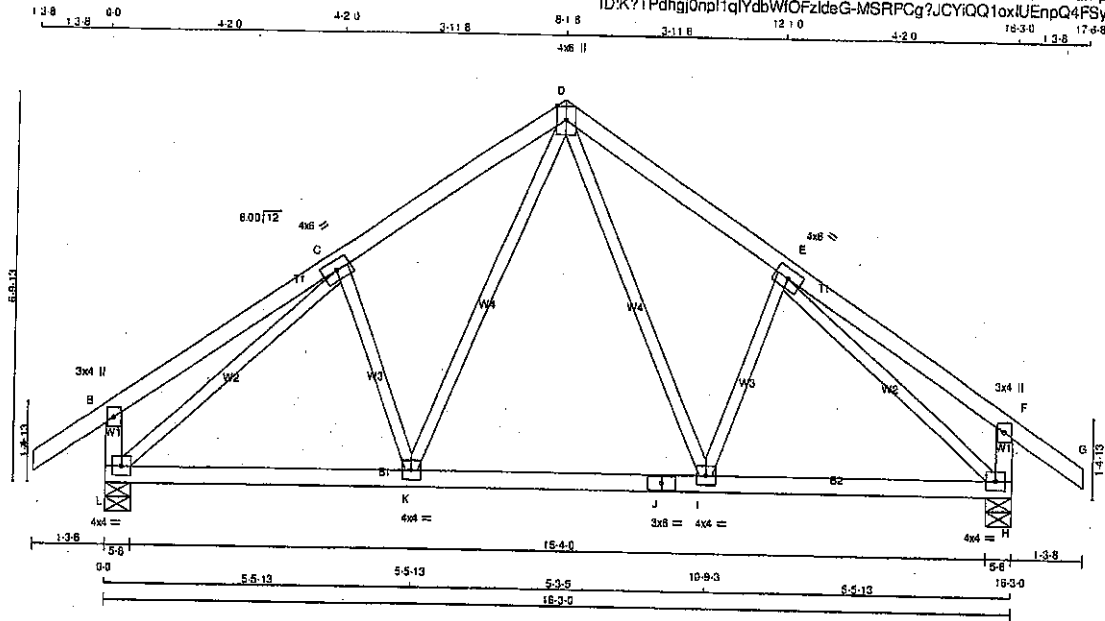
Structural component only
 DWG# T-2007154 *HL*

DRWG NO.

version 8.310 S Oct 29 2019 MTEK Industries, Inc. Sat Apr 25 12:49:35 2020 Page 1
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13-9-9



JOB NAME 408169	TRUSS NAME T30	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Sat Apr 25 12:49:36 2020 Page 1 ID:K7TPdhgi0npl1qIYdbWIOFzIdG-MSRPGG?JCYIQQ1oxIUEnpQ4FSy9zoKVn7QVwp_zNBDT	
Scale = 1/32.0					



LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-I	MT20	4.0	8.0		
D	TFWW+p	MT20	4.0	8.0	Edge	
E	TMWW-I	MT20	4.0	8.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVV1-I	MT20	4.0	4.0		
I	BMWW-I	MT20	4.0	4.0		
K	BS-I	MT20	3.0	8.0		
L	BMWW-I	MT20	4.0	4.0		
L	BMVV1-I	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
L	1022	0	1022	0	0	5-8	5-8	5-8	5-8
H	1022	0	1022	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	720	486 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0
H	720	486 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	UNBRACED LENGTH
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-91.8 -91.8	0.12 (1)	D-I	0 / 336	0.08 (1)	10.00
B-C	0 / 24	-91.8 -91.8	0.24 (1)	I-E	-217 / 0	0.08 (1)	10.00
C-D	-845 / 0	-91.8 -91.8	0.19 (1)	K-D	0 / 336	0.08 (1)	6.25
D-E	-845 / 0	-91.8 -91.8	0.19 (1)	C-K	-217 / 0	0.06 (1)	6.25
E-F	0 / 24	-91.8 -91.8	0.24 (1)	L-C	-1042 / 0	0.52 (1)	10.00
F-G	0 / 35	-91.8 -91.8	0.12 (1)	E-H	-1042 / 0	0.52 (1)	10.00
L-B	-268 / 0	0.0 0.0	0.03 (1)				7.81
H-F	-268 / 0	0.0 0.0	0.03 (1)				7.81
L-K	0 / 762	-16.5 -16.5	0.19 (1)				10.00
K-J	0 / 557	-16.5 -16.5	0.17 (4)				10.00
J-I	0 / 557	-16.5 -16.5	0.17 (4)				10.00
I-H	0 / 762	-16.5 -16.5	0.19 (1)				10.00

TOTAL WEIGHT = 2 X 72 = 145 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.54")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.54")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CSI: TC=0.24/1.00 (B-C:1), BC=0.19/1.00 (H-I:1),
WB=0.52/1.00 (C-L:1), SS=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

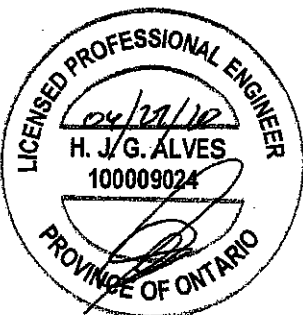
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

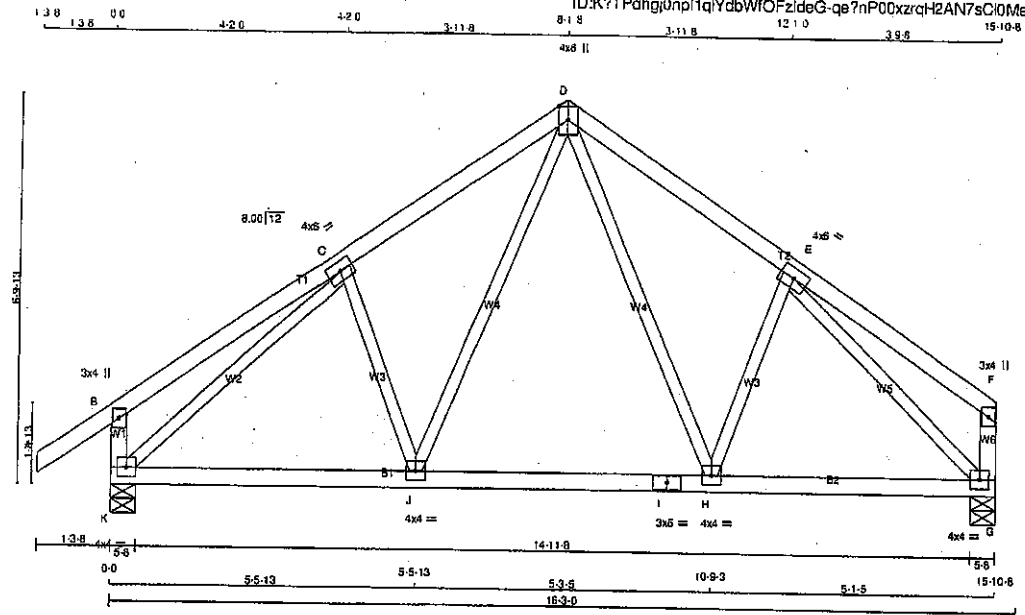
JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.29 (L) (INPUT = 1.00)



Structural component only
DWG# T-2007156

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 70 = 140 lb

DRY: SEASONED LUMBER.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

BEARINGS

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: {4}

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 35	-91.9	-91.9 0.12 (1)	10.00	D-H	0 / 272	0.06 (1)
B-C	0 / 24	-91.8	-91.9 0.24 (1)	10.00	H-E	-160 / 2	0.05 (1)
C-D	-817 / 0	-91.8	-91.8 0.19 (1)	6.25	J-D	0 / 342	0.08 (1)
D-E	-793 / 0	-91.8	-91.8 0.18 (1)	6.25	C-K	-222 / 0	0.08 (1)
E-F	0 / 24	-91.8	-91.8 0.22 (1)	10.00	K-C	-1012 / 0	0.51 (1)
K-B	-268 / 0	0.0	0.0 0.03 (1)	7.81	E-G	-992 / 0	0.44 (1)
G-F	-124 / 0	0.0	0.0 0.01 (1)	7.81			
K-J	0 / 740	-18.5	-18.5 0.18 (1)	10.00			
J-I	0 / 531	-18.5	-18.5 0.17 (4)	10.00			
I-H	0 / 531	-18.5	-18.5 0.17 (4)	10.00			
H-G	0 / 691	-18.5	-18.5 0.17 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.24/1.00 (B-C:1), BC=0.18/1.00 (J-K:1),
WB=0.51/1.00 (C-K:1), SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PSI)	(PLI)	(PLI)	(PLI)	(PLI)
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.28 (K) (INPUT = 1.00)



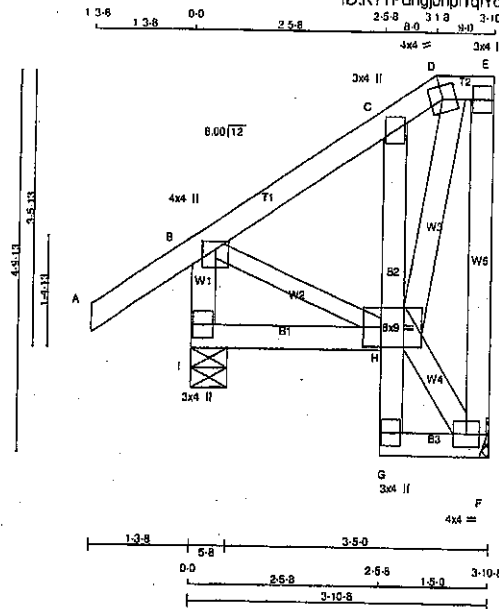
Structural component only
DWG# T-2007157

Structural component only
DWG# T-2007158

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	T31S	1	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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Scale = 1/25.8

TOTAL WEIGHT = 29 lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
I - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
G - C	2x4	DRY	No.2
G - F	2x4	DRY	No.2

ALL WEBS	2x4	DRY	No.2
EXCEPT			
B - H	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25 2.00
C	TMV+p	MT20	3.0	4.0	
D	TTW-m	MT20	4.0	4.0	
E	TMV+p	MT20	3.0	4.0	
F	BMVW1-t	MT20	4.0	4.0	
G	BMV+p	MT20	3.0	4.0	
H	BMVWW1	MT20	6.0	9.0	3.00 2.75
I	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
F	VERT	HORZ	DOWN	HORZ
I	193	0	193	0
I	361	0	361	0
			5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	138	88 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
I	262	181 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MEMB.	FORCE
(LBS)	(PLF)	CSI (LC)	(LBS)	CSI (LC)
FR-TO	FROM	TO	FR-TO	FROM
A-B	0 / 35	-91.8 -91.8 0.14 (5)	H-F	-27 / 0
B-C	-87 / 0	-91.8 -91.8 0.09 (1)	B-H	0 / 68
C-D	-99 / 0	-91.8 -91.8 0.07 (1)	H-D	0 / 173
D-E	-11 / 0	-91.8 -91.8 0.07 (1)		
F-E	-148 / 0	0.0 0.0 0.04 (1)		
I-B	-327 / 0	0.0 0.0 0.02 (4)		
I-H	0 / 8	-18.5 -18.5 0.03 (4)		
G-H	-2 / 8	0.0 0.0 0.05 (1)		
H-C	-203 / 0	0.0 0.0 0.04 (1)		
G-F	0 / 30	-18.5 -18.5 0.03 (1)		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
	DL = 8.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 38.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.01")

CSI: TC=0.14/1.00 (A-B-5), BC=0.05/1.00 (G-H-1),
WB=0.03/1.00 (D-H-1), SSI=0.12/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

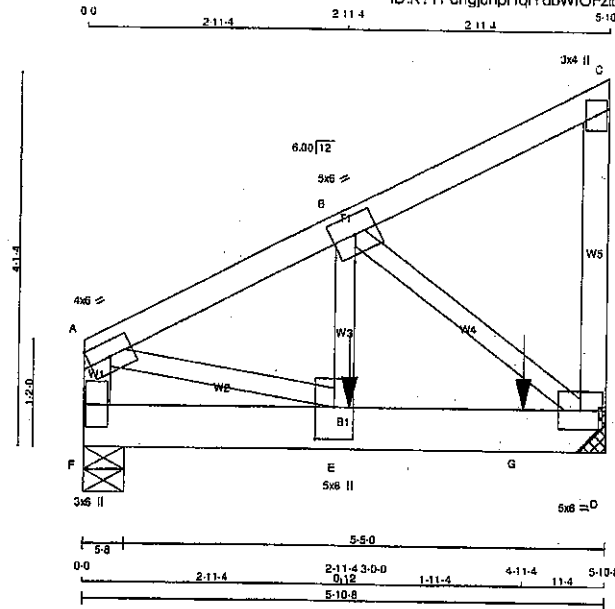


Structural component only
DWG# T-2007159

JOB NAME 408169	TRUSS NAME T33	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/23.3

LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A	12	TOP
A - C	12	TOP
C - D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	9	SIDE(305.2)
WEBS : (0.122"x3") SPIRAL NAILS		
B - E	2	SIDE(408.9)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UCL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMWV-1	MT20	4.0	8.0	Edge
B	TMWV-1	MT20	5.0	8.0	
C	TMWV-p	MT20	3.0	4.0	
D	BMWV-1	MT20	5.0	8.0	
E	BMWV-t	MT20	5.0	8.0	4.25 2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED		MAXIMUM FACTORED			INPUT	REQD
JT	GROSS REACTION		GROSS REACTION			BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1914	0	1914	0	0	5-8	5-8
D	2859	0	2859	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST LOASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1348	915 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0
D	2015	1359 / 0	0 / 0	0 / 0	0 / 0	657 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED		
MEMB.	FORCE	VERT. LOAD	LC1	MAX.	MEMB.	FORCE	MAX.	
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO			
F-A	-1877 / 0	0.0	0.0	0.10 (1)	7.81	A-E	0 / 2380	0.29 (1)
A-B	-2588 / 0	-91.8	-91.8	0.08 (1)	5.59	E-B	0 / 2608	0.32 (1)
B-C	-7 / 0	-91.8	-91.8	0.06 (1)	10.00	B-D	-2898 / 0	0.35 (1)
D-C	-120 / 0	0.0	0.0	0.01 (1)	7.81			
F-E	0 / 0	-18.5	-18.5	0.01 (1)	10.00			
E-G	0 / 2301	-18.5	-18.5	0.49 (1)	10.00			
G-D	0 / 2301	-18.5	-18.5	0.49 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIRL	TYPE	HEEL	CONN.
E	34-0	-2737	-2737	---	BACK	VERT	TOTAL	---	C1
G	4-11-4	-1388	-1388	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
	DL = 8.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 38.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.10/1.00 (A-F:1), BC=0.49/1.00 (D-E:1), WB=0.35/1.00 (B-D:1), SSI=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1867 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (B) (INPUT = 0.90)
JSI METAL = 0.44 (E) (INPUT = 1.00)

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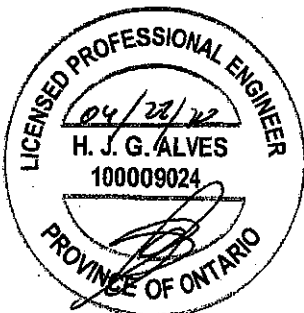


Structural component only
DWG# T-2007161 1/2

JOB NAME 408169	TRUSS NAME T33	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
F BMV1+p MT20 3.0 6.0
Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

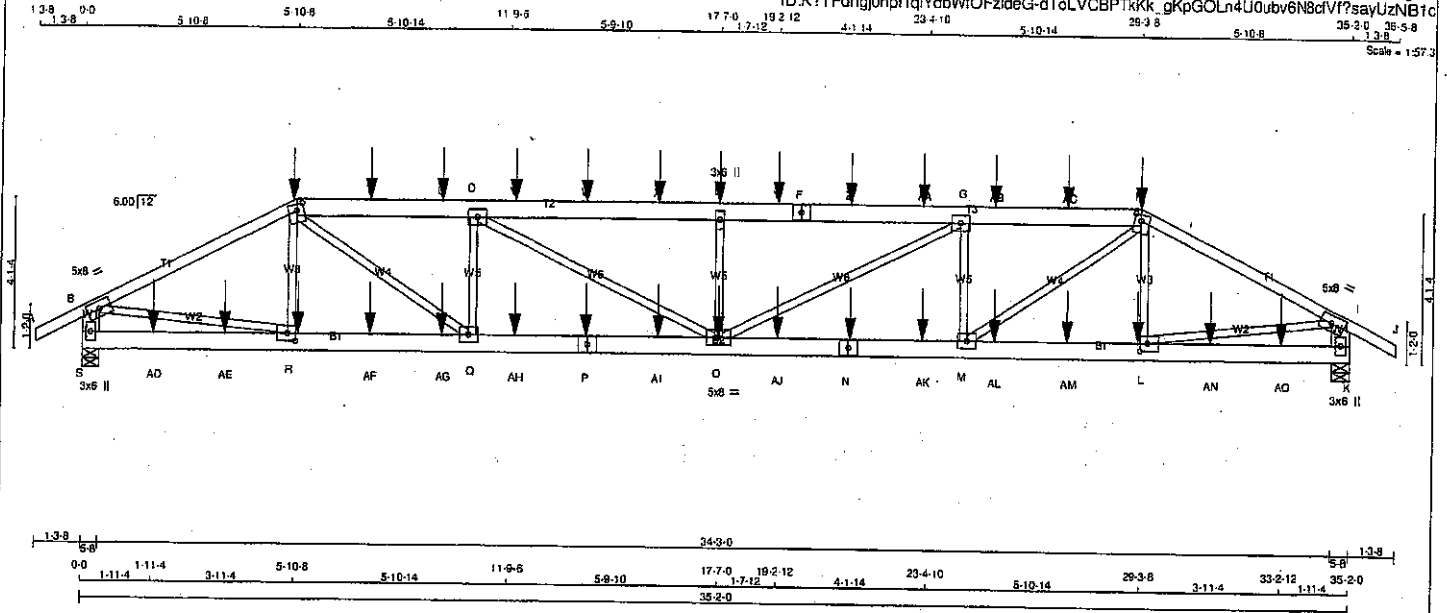


Structural component only
DWG# T-2007161 42

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408170	T40	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES	A - C	2x4	DRY	No.2	SPF
	C - F	2x6	DRY	No.2	SPF
	F - H	2x6	DRY	No.2	SPF
	H - J	2x4	DRY	No.2	SPF
	S - B	2x6	DRY	No.2	SPF
	K - I	2x6	DRY	No.2	SPF
	S - P	2x6	DRY	No.2	SPF
	P - N	2x6	DRY	No.2	SPF
	N - K	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	SIDE(61.0)
H - J 1	12	SIDE(61.0)
C - F 2	12	SIDE(61.0)
F - H 2	12	SIDE(61.0)
S - B 2	12	TOP
K - I 2	12	TOP

BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS
S - P 2 12 SIDE(183.1)
P - N 2 12 SIDE(183.1)
N - K 2 12 SIDE(183.1)

WEBS : (0.122"x3") SPIRAL NAILS
2x3 1 6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
S	3331	0	0	5-8
K	3331	0	0	5-8

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED SNOW	LIVE PERM.LIVE WIND DEAD SOIL
S	2356	1544 / 0 0 / 0 0 / 0 812 / 0 0 / 0
K	2356	1544 / 0 0 / 0 0 / 0 812 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

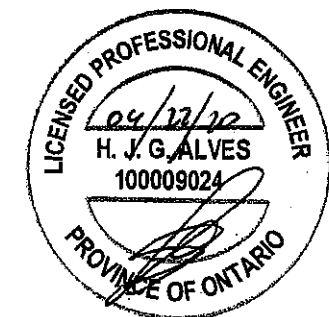
BRACING
TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 3.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO			
A-B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	R-C	-449 / 0	0.06 (1)
B-C	-5069 / 0	-91.8	-91.8	0.52 (1)	3.87	C-Q	0 / 3021	0.37 (1)
C-T	-8976 / 0	-91.8	-91.8	0.24 (1)	4.35	Q-D	-1508 / 0	0.19 (1)
T-U	-8976 / 0	-91.8	-91.8	0.24 (1)	4.35	D-O	0 / 1339	0.17 (1)
U-D	-8976 / 0	-91.8	-91.8	0.24 (1)	4.35	O-E	-1056 / 0	0.13 (1)
D-V	-8155 / 0	-91.8	-91.8	0.31 (1)	4.02	G-G	0 / 1339	0.17 (1)
V-W	-8155 / 0	-91.8	-91.8	0.31 (1)	4.02	M-G	-1608 / 0	0.19 (1)
W-X	-8155 / 0	-91.8	-91.8	0.31 (1)	4.02	M-H	0 / 3021	0.37 (1)
X-E	-8155 / 0	-91.8	-91.8	0.31 (1)	4.02	L-H	-449 / 0	0.06 (1)
E-Y	-8155 / 0	-91.8	-91.8	0.31 (1)	4.02	S-R	0 / 4585	0.57 (1)
Y-F	-8155 / 0	-91.8	-91.8	0.31 (1)	4.02	L-I	0 / 4585	0.57 (1)
F-Z	-8155 / 0	-91.8	-91.8	0.31 (1)	4.02			
Z-AA	-8155 / 0	-91.8	-91.8	0.31 (1)	4.02			
AA-G	-8155 / 0	-91.8	-91.8	0.31 (1)	4.02			
G-AB	-8976 / 0	-91.8	-91.8	0.24 (1)	4.35			
AB-AC	-8976 / 0	-91.8	-91.8	0.24 (1)	4.35			
AC-H	-8976 / 0	-91.8	-91.8	0.24 (1)	4.35			
H-I	-5069 / 0	-91.8	-91.8	0.52 (1)	3.87			
I-J	0 / 28	-91.8	-91.8	0.07 (1)	10.00			
S-B	-3250 / 0	0.0	0.0	0.11 (1)	7.89			
K-I	-3250 / 0	0.0	0.0	0.11 (1)	7.89			



Structural component only
DWG# T-2007168 1/2

TOTAL WEIGHT = 2 X 169 = 338 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.39")
CSI: TC=0.52/1.00 (B-C:1), BC=0.52/1.00 (C-Q:1),
WB=0.57/1.00 (B-R:1), SS=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 518 354 1887 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (H) (INPUT = 0.90)
JSI METAL = 0.67 (P) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T40	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TTWW+m	MT20	5.0	6.0	2.00	2.25
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	5.0	5.0	2.00	2.25
I	TMVW-t	MT20	5.0	8.0		
K	BMV1-p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.50
S	BMV1-p	MT20	3.0	6.0		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-437	-437	---	FRONT	VERT	TOTAL	---	C1
E	17-7-0	-110	-110	---	FRONT	VERT	TOTAL	---	C1
H	29-3-8	-437	-437	---	FRONT	VERT	TOTAL	---	C1
L	29-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
N	21-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	17-7-0	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	13-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
R	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
T	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
U	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	11-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	15-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	19-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	21-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	23-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	25-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	27-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	1-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AE	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AF	7-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AG	9-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AH	11-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AI	15-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AJ	19-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AK	23-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AL	25-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AM	27-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AN	31-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AO	33-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

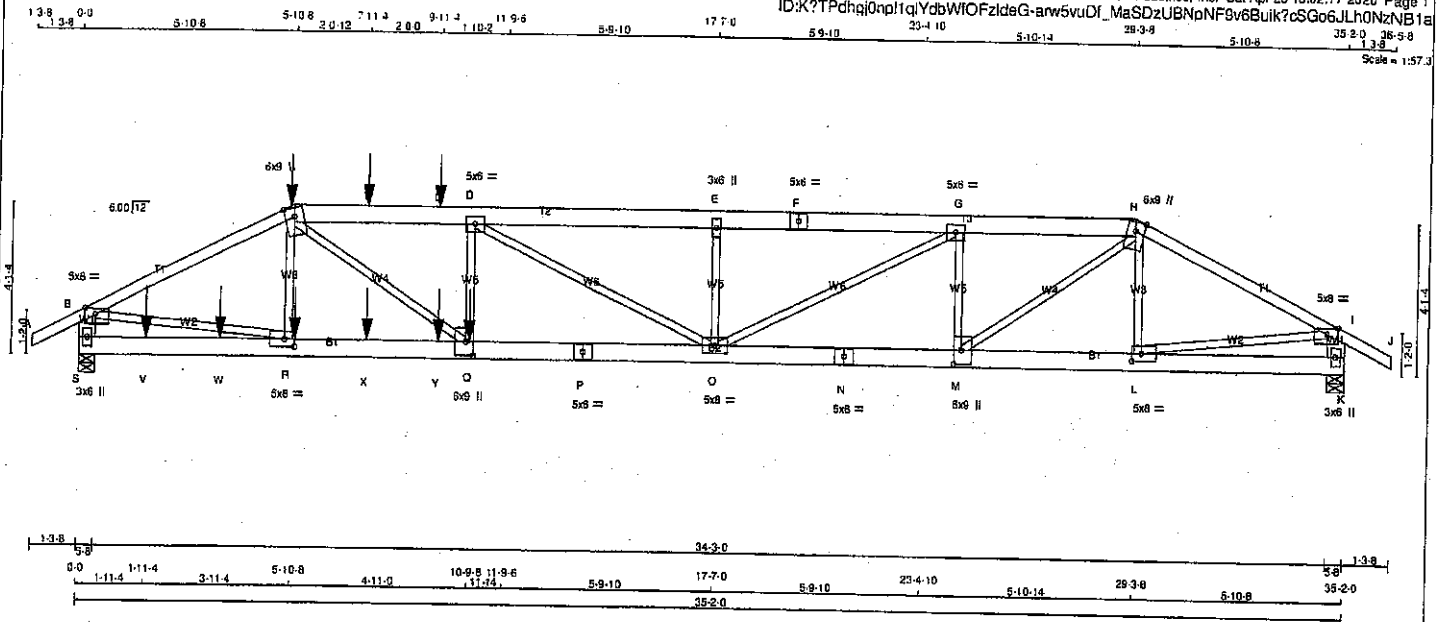


Structural component only
DWG# T-2007168

JOB NAME 408170	TRUSS NAME T40Z	QUANTITY 1	PLY. 2	JOB DESC GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x6	DRY	No.2	SPF	
F - H	2x6	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
S - B	2x6	DRY	No.2	SPF	
K - I	2x6	DRY	No.2	SPF	
S - P	2x6	DRY	No.2	SPF	
P - N	2x6	DRY	No.2	SPF	
N - K	2x6	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2
EXCEPT
SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122'X3") SPIRAL NAILS		
A-C	12	SIDE(61.0)
H-J	12	TOP
C-F	12	SIDE(61.0)
F-H	12	TOP
S-B	12	TOP
K-I	12	TOP
BOTTOM CHORDS: (0.122'X3") SPIRAL NAILS		
S-P	12	SIDE(183.1)
P-N	12	TOP
N-K	12	TOP
WEBS: (0.122'X3") SPIRAL NAILS		
2x3	6	SIDE(525.1)
D-Q	3	
G-M	3	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	JT	GROSS REACTION	JT	GROSS REACTION	JT	GROSS REACTION	JT	GROSS REACTION
S	4582	S	4582	S	4582	S	4582	S	4582
K	3047	K	3047	K	3047	K	3047	K	3047

UNFACTORED REACTIONS		1ST LC CASE		MAX/MIN. COMPONENT REACTIONS	
JT	COMBINED	JT	COMBINED	JT	COMBINED
S	3238	S	3238	S	3238
K	2151	K	2151	K	2151

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)	
CHORDS	MEMB.	CHORDS	MEMB.
MAX. FACTORED	MAX. FACTORED	MAX. FACTORED	MAX. FACTORED
FORCE (LBS)	FORCE (LBS)	FORCE (LBS)	FORCE (LBS)
FR-TO	FR-TO	FR-TO	FR-TO
A-B	0/28	A-B	0/28
B-C	-7293/0	B-C	-7293/0
C-T	-10549/0	C-T	-10549/0
T-U	-10549/0	T-U	-10549/0
U-D	-10549/0	U-D	-10549/0
D-E	-9485/0	D-E	-9485/0
E-F	-9485/0	E-F	-9485/0
F-G	-9485/0	F-G	-9485/0
G-H	-9485/0	G-H	-9485/0
H-I	-4608/0	H-I	-4608/0
I-J	0/28	I-J	0/28
J-B	-4503/0	J-B	-4503/0
K-I	-2990/0	K-I	-2990/0
S-V	0/0	S-V	0/0
V-W	0/0	V-W	0/0
W-R	0/0	W-R	0/0
R-X	0/8557	R-X	0/8557
X-Y	0/8557	X-Y	0/8557
Y-Q	0/6557	Y-Q	0/6557
Q-P	0/10548	Q-P	0/10548
P-O	0/10548	P-O	0/10548
O-N	0/8955	O-N	0/8955
N-M	0/8955	N-M	0/8955
M-L	0/4144	M-L	0/4144
L-K	0/0	L-K	0/0

FACTORED CONCENTRATED LOADS (LBS)	
JT	LOC.
C	5-10-8
Q	10-9-8
R	5-11-4
T	7-11-4
U	9-11-4
V	1-11-4
W	3-11-4
X	7-11-4
Y	9-11-4

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 169 = 338 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 8 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65% OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.26")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/881 (0.48")

GSI: TC=0.72/1.00 (B-C:1), BC=0.77/1.00 (O-Q:1), WB=0.82/1.00 (B-R:1), SS=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

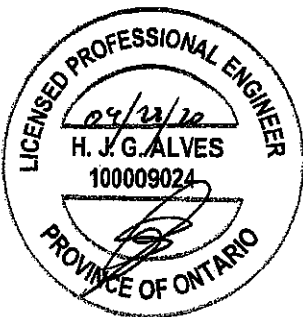
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)

JSI METAL= 0.99 (P) (INPUT = 1.00)



Structural component only
DWG# T-2007169

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T40Z	1	2	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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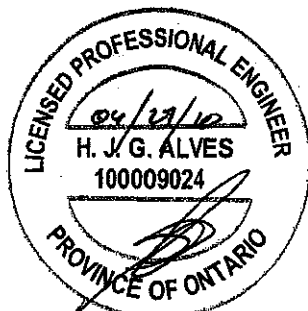
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	2.00	3.50
C	TTWW-m	MT20	6.0	9.0	Edge	
D	TMWW-i	MT20	5.0	6.0		
E	TMW-w	MT20	3.0	6.0		
F	TS-i	MT20	5.0	6.0		
G	TMWW-i	MT20	5.0	6.0		
H	TTWW-m	MT20	6.0	9.0	Edge	
I	TMVW-p	MT20	5.0	8.0	2.00	3.50
K	BMV1-p	MT20	3.0	6.0		
L	BMWW-i	MT20	5.0	8.0	2.50	3.25
M	BMWW-i	MT20	5.0	9.0	4.50	2.50
N	BS-i	MT20	5.0	6.0		
O	BMWWW-i	MT20	5.0	8.0		
P	BS-i	MT20	5.0	6.0		
Q	BMWW-w	MT20	8.0	9.0	4.50	2.50
R	BMWW-i	MT20	5.0	8.0	2.50	3.25
S	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

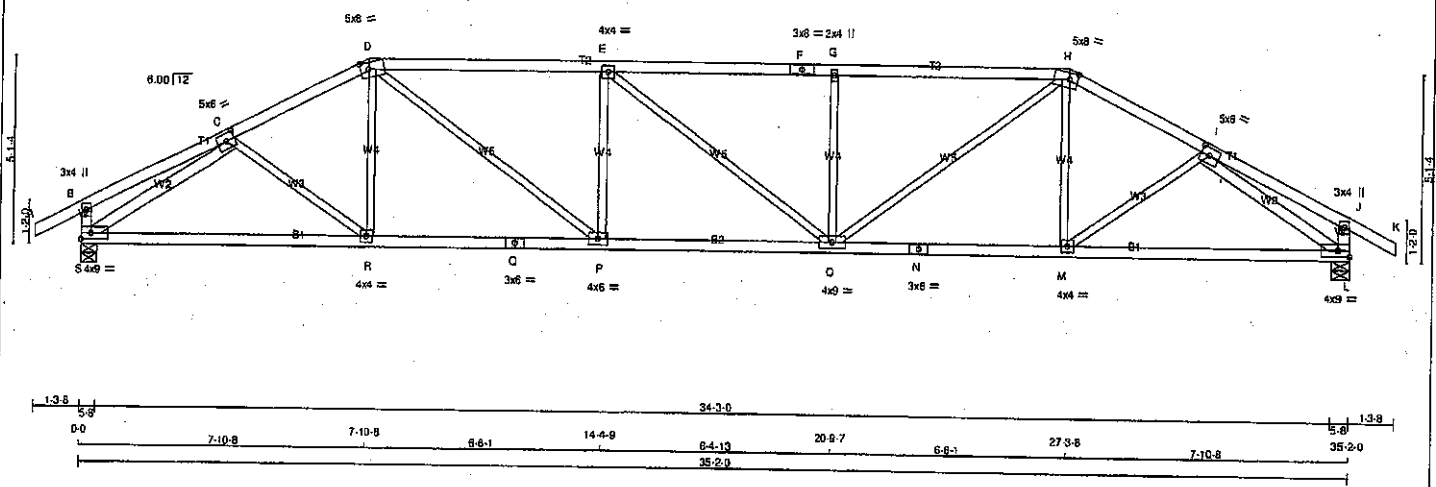
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007169 2/2

JOB NAME: 408170	TRUSS NAME: T41	QUANTITY: 2	PLY: 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 ID:K?TPdHgQnpIqYdbWOFzIdG-22UT7EDHfJ73OxWwUieJ865BLxLz4FzpzNB1Z
 20-9-7 27-3-8 31-1-8 40-8 35-2-0 38-5-8 1-3-8
 Scale = 1:37.2



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
S - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMWV-t	MT20	5.0	6.0	2.50	2.75
D TTWV-m	MT20	5.0	8.0	2.25	2.75
E TMWV-t	MT20	4.0	4.0		
F TS-t	MT20	3.0	8.0		
G TMWV-w	MT20	2.0	4.0		
H TTWV-m	MT20	5.0	8.0	2.25	2.75
I TMWV-t	MT20	5.0	6.0	2.50	2.75
J TMV+p	MT20	3.0	4.0		
K BMWV-t	MT20	4.0	9.0		Edge
M BMWV-t	MT20	4.0	4.0		
N BS-t	MT20	3.0	6.0		
O BMWV-t	MT20	4.0	9.0		
P BMWV-t	MT20	4.0	6.0		
Q BS-t	MT20	3.0	6.0		
R BMWV-t	MT20	4.0	4.0		
S BMWV-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT REQD BRG
JT	VERT	DOWN	BRG
S	2083	0	5-8
L	2083	0	5-8

UNFACTORED REACTIONS						
	1ST CASE	MAX	MIN	COMPONENT REACTIONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
S	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0
L	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.94 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)
B-C	0 / 16	-91.8	-91.8 0.20 (1)
C-D	-2799 / 0	-91.8	-91.8 0.32 (1)
D-E	-3508 / 0	-91.8	-91.8 0.95 (1)
E-F	-3507 / 0	-91.8	-91.8 0.93 (1)
F-G	-3507 / 0	-91.8	-91.8 0.93 (1)
G-H	-3507 / 0	-91.8	-91.8 0.94 (1)
H-I	-2799 / 0	-91.8	-91.8 0.32 (1)
I-J	0 / 16	-91.8	-91.8 0.20 (1)
J-K	0 / 28	-91.8	-91.8 0.12 (1)
S-B	-270 / 0	0.0	0.0 0.03 (1)
L-J	-270 / 0	0.0	0.0 0.03 (1)
S-R	0 / 2417	-18.5	-18.5 0.53 (1)
R-Q	0 / 2489	-18.5	-18.5 0.54 (1)
Q-P	0 / 2489	-18.5	-18.5 0.54 (1)
P-O	0 / 3509	-18.5	-18.5 0.84 (1)
O-N	0 / 2489	-18.5	-18.5 0.54 (1)
N-M	0 / 2489	-18.5	-18.5 0.54 (1)
M-L	0 / 2417	-18.5	-18.5 0.53 (1)

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 8.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBG 2018, CBC 2012, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 088-14
 - TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(LL) = L/989 (0.21")
 ALLOWABLE DEFL.(TL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(TL) = L/989 (0.41")

CSI: TC=0.85/1.00 (D-E:1), BC=0.64/1.00 (O-P:1), WB=0.83/1.00 (F-L:1), SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

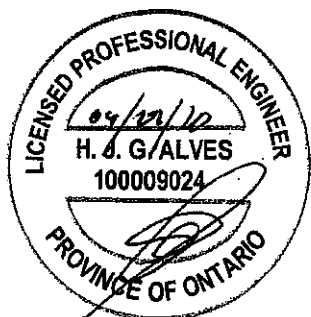
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1687 788	1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (L) (INPUT = 0.90)
 JSI METAL = 0.77 (N) (INPUT = 1.00)



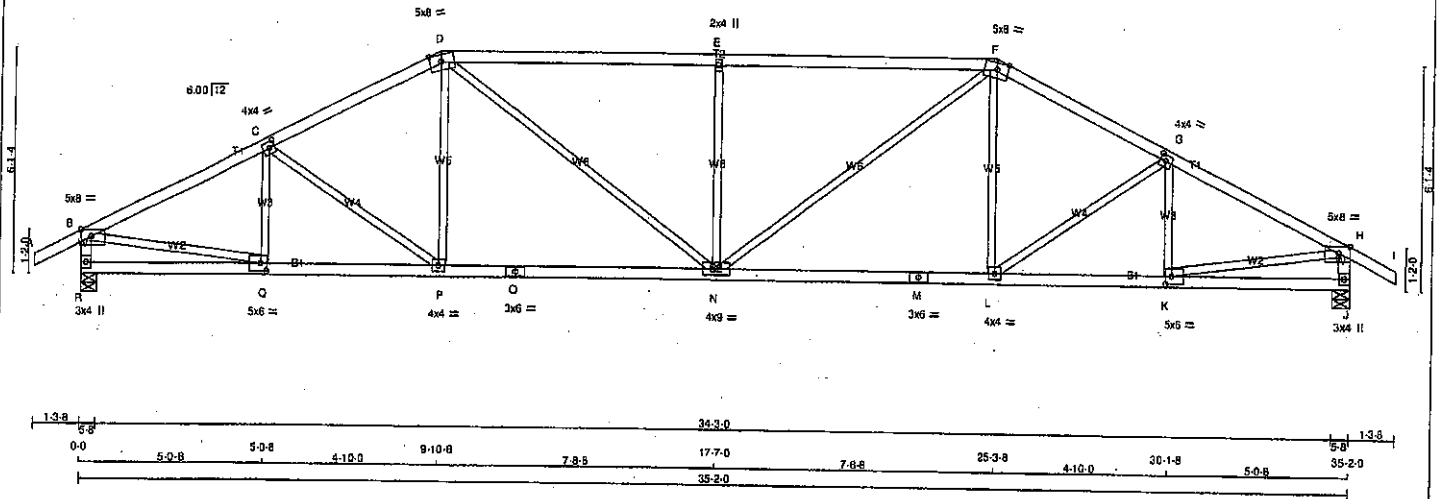
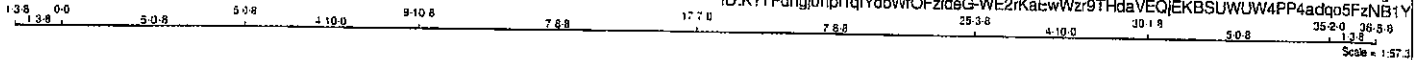
Structural component only
 DWG# T-2007170

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408170	T42	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 139 = 277 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
R - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
R	2063	0	0	2063	0	0	0	5-8	5-8
J	2063	0	0	2063	0	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
J	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	FORCE (LBS)	MAX. CS1 (LC)	MAX. CS1 (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	Q-C	-353 / 0	0.08 (1)
B-C	-2834 / 0	-91.8	-91.8 0.39 (1)	3.81	C-P	-210 / 0	0.13 (1)
C-D	-2687 / 0	-91.8	-91.8 0.37 (1)	3.91	P-D	0 / 246	0.06 (4)
D-E	-3060 / 0	-91.8	-91.8 0.37 (1)	2.78	D-N	0 / 844	0.19 (1)
E-F	-3020 / 0	-91.8	-91.8 0.39 (1)	3.91	N-F	-872 / 0	0.51 (1)
F-G	-2897 / 0	-91.8	-91.8 0.39 (1)	3.91	F-H	0 / 844	0.19 (1)
G-H	-2834 / 0	-91.8	-91.8 0.39 (1)	3.91	H-I	0 / 246	0.06 (4)
H-I	0 / 28	-91.8	-91.8 0.12 (1)	10.00	I-G	-210 / 0	0.13 (1)
I-B	-2018 / 0	0.0	0.0 0.20 (1)	5.94	K-G	-353 / 0	0.08 (1)
J-H	-2018 / 0	0.0	0.0 0.20 (1)	5.94	B-Q	0 / 2591	0.58 (1)
R-Q	0 / 0	-18.5	-18.5 0.10 (4)	10.00	K-H	0 / 2591	0.58 (1)
Q-P	0 / 2555	-18.5	-18.5 0.51 (1)	10.00			
P-O	0 / 2386	-18.5	-18.5 0.49 (1)	10.00			
O-N	0 / 2386	-18.5	-18.5 0.49 (1)	10.00			
N-M	0 / 2386	-18.5	-18.5 0.49 (1)	10.00			
M-L	0 / 2386	-18.5	-18.5 0.49 (1)	10.00			
L-K	0 / 2555	-18.5	-18.5 0.51 (1)	10.00			
K-J	0 / 0	-18.5	-18.5 0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
BOT CH.	DL = 8.0	PSF
	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/999 (0.17")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = L/999 (0.34")

CSI: TC=0.97/1.00 (D-E:1), BC=0.51/1.00 (P-Q:1), WB=0.58/1.00 (B-C:1), SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

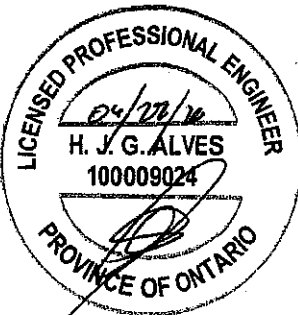
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (F) (INPUT = 0.90)
JSI METAL = 0.75 (M) (INPUT = 1.00)

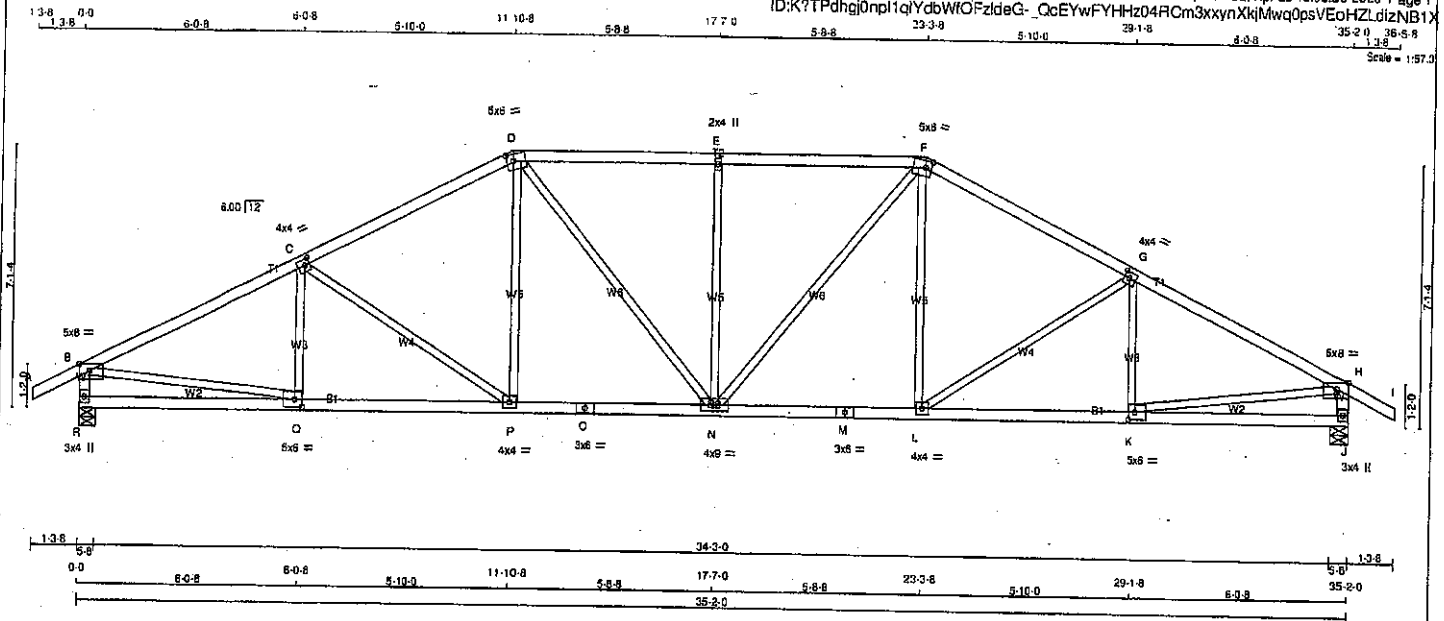


Structural component only
DWG# T-2007171

JOB NAME 408170	TRUSS NAME T43	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Pool Truss, Burlington

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<u>LUMBER</u>				
N. L. G. A. RULES				
CHORDS		SIZE		LUMBER
A - D	2x4	DRY		No.2
D - F	2x4	DRY		No.2
F - I	2x4	DRY		No.2
R - B	2x4	DRY		No.2
J - H	2x4	DRY		No.2
R - O	2x4	DRY		No.2
O - M	2x4	DRY		No.2
M - J	2x4	DRY		No.2

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	5.0	8.0	Edge	3.50
J	BMV1-p	MT20	3.0	4.0		
K	BMVW-t	MT20	5.0	6.0	2.50	2.25
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BSVWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.25
R	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HORZ
R	2083	0	2083	0
J	2083	0	2083	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1457	959 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
J	1457	959 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	FACTORED (PLF)	MAX. (LC)	MAX. UNBRAC LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (LC)
FR-TO							FR-TO			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	Q-C	-255 / 12	0.07 (1)		
B-C	-2889 / 0	-91.8	-91.8	0.58 (1)	3.58	C-P	-435 / 0	0.42 (1)		
C-D	-2539 / 0	-91.8	-91.8	0.52 (1)	3.83	P-D	0 / 360	0.08 (1)		
D-E	-2539 / 0	-91.8	-91.8	0.48 (1)	3.85	D-N	0 / 482	0.11 (1)		
E-F	-2539 / 0	-91.8	-91.8	0.48 (1)	3.85	N-E	-641 / 0	0.58 (1)		
F-G	-2539 / 0	-91.8	-91.8	0.52 (1)	3.83	N-F	0 / 492	0.11 (1)		
G-H	-2889 / 0	-91.8	-91.8	0.58 (1)	3.58	L-F	0 / 350	0.08 (1)		
H-I	0 / 28	-91.8	-91.8	0.12 (1)	10.00	L-G	-435 / 0	0.42 (1)		
I-B	-2014 / 0	0.0	0.0	0.20 (1)	5.95	K-G	-255 / 12	0.07 (1)		
J-H	-2014 / 0	0.0	0.0	0.20 (1)	5.95	B-Q	0 / 2635	0.59 (1)		
R-Q	0 / 0	-18.5	-18.5	0.15 (4)	10.00	K-H	0 / 2635	0.59 (1)		
C-P	0 / 2609	-18.5	-18.5	0.49 (1)	10.00					
P-O	0 / 2249	-18.5	-18.5	0.43 (1)	10.00					
O-N	0 / 2249	-18.5	-18.5	0.43 (1)	10.00					
N-M	0 / 2249	-18.5	-18.5	0.43 (1)	10.00					
M-L	0 / 2249	-18.5	-18.5	0.43 (1)	10.00					
L-K	0 / 2609	-18.5	-18.5	0.49 (1)	10.00					
K-J	0 / 0	-18.5	-18.5	0.15 (4)	10.00					

TOTAL WEIGHT = 2 X 145 = 289 lb (MIF)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/999 (0.15")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = L/899 (0.28")

CSI: TC=0.58/1.00 (B-C:1), BC=0.49/1.00 (P-Q:1), WB=0.59/1.00 (B-Q:1), SS=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

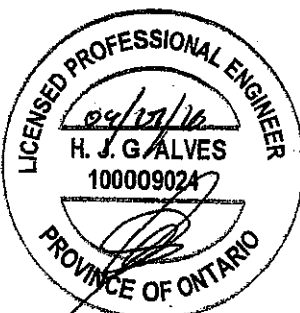
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 768 1987 1656

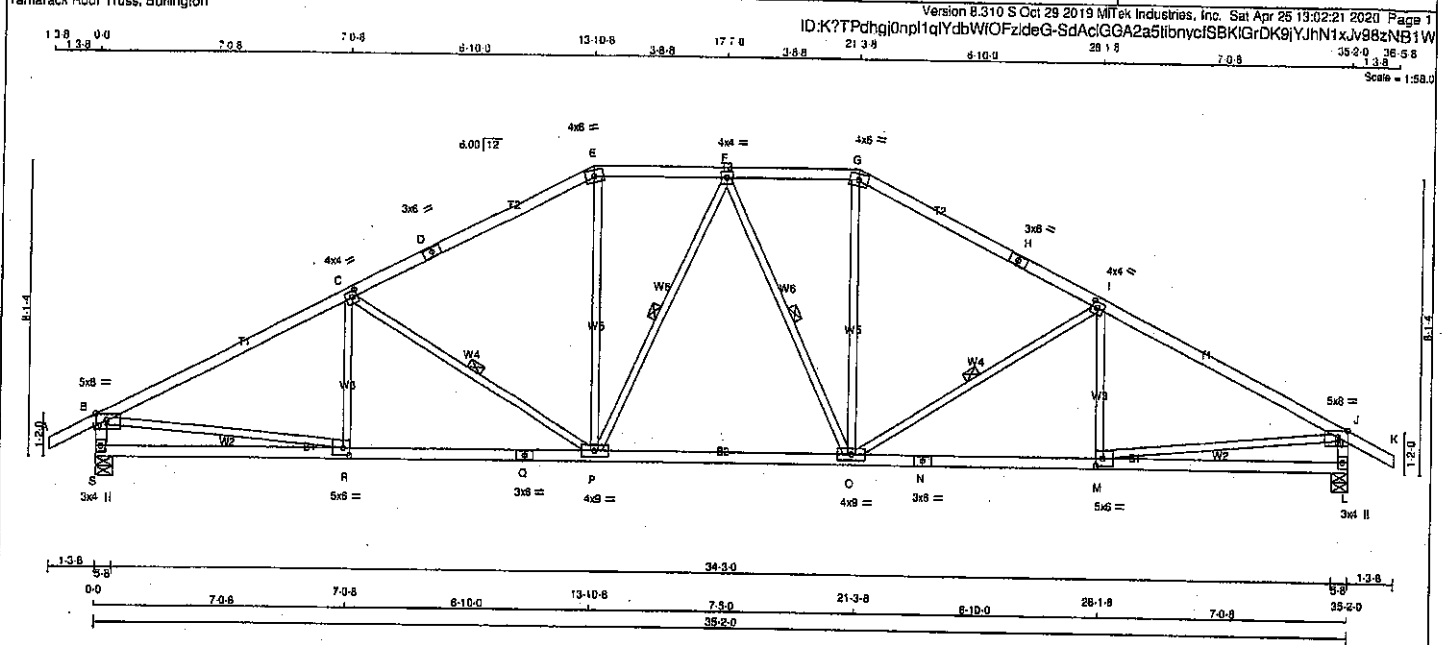
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.69 (Q) (INPUT = 0.90)
JSI METAL = 0.69 (M) (INPUT = 1.00)



Structural component only
DWG# T-2007172



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWV-p	MT20	5.0	8.0	Edge	3.50
C TMWW-I	MT20	4.0	4.0	2.00	1.75
D TS-I	MT20	3.0	6.0		
E TTW-m	MT20	4.0	6.0		
F TMWW-I	MT20	4.0	4.0		
G TTW-m	MT20	4.0	6.0		
H TS-I	MT20	3.0	6.0		
I TMWW-I	MT20	4.0	4.0	2.00	1.75
J TMWV-p	MT20	5.0	8.0	Edge	3.50
L BMWV-I	MT20	3.0	4.0		
M BS-I	MT20	5.0	6.0	2.50	2.25
N BS-I	MT20	3.0	6.0		
O BMWV-W-I	MT20	4.0	9.0		
P BMWV-W-I	MT20	4.0	9.0		
Q BS-I	MT20	3.0	6.0		
R BMWV-I	MT20	5.0	6.0	2.50	2.25
S BMWV-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
VERT				
DOWN				
UP				
IN-SX				
OUT-SX				

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
COMBINED						
SNOW						
LIVE						

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-P, F-Q, I-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO				FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	R-C	-182 / 52	0.06 (1)
B-C	-2908 / 0	-91.8	-91.8 0.83 (1)	3.25	C-P	-637 / 0	0.29 (1)
C-D	-2373 / 0	-91.8	-91.8 0.72 (1)	3.66	P-E	0 / 674	0.15 (1)
D-E	-2373 / 0	-91.8	-91.8 0.72 (1)	3.66	F-F	-222 / 0	0.11 (1)
E-F	-2100 / 0	-91.8	-91.8 0.19 (1)	4.53	F-O	-222 / 0	0.11 (1)
F-G	-2100 / 0	-91.8	-91.8 0.19 (1)	4.53	O-G	0 / 674	0.15 (1)
G-H	-2373 / 0	-91.8	-91.8 0.72 (1)	3.66	D-I	-637 / 0	0.29 (1)
H-I	-2373 / 0	-91.8	-91.8 0.72 (1)	3.66	M-I	-182 / 52	0.06 (1)
I-J	-2908 / 0	-91.8	-91.8 0.83 (1)	3.25	B-R	0 / 2649	0.80 (1)
J-K	0 / 28	-91.8	-91.8 0.12 (1)	10.00	M-J	0 / 2649	0.60 (1)
S-B	-2008 / 0	0.0	0.0 0.20 (1)	5.95			
L-J	-2008 / 0	0.0	0.0 0.20 (1)	5.95			
S-R	0 / 0	-18.5	-18.5 0.20 (4)	10.00			
R-Q	0 / 2630	-18.5	-18.5 0.52 (1)	10.00			
Q-P	0 / 2630	-18.5	-18.5 0.52 (1)	10.00			
P-O	0 / 2193	-18.5	-18.5 0.45 (1)	10.00			
O-N	0 / 2630	-18.5	-18.5 0.52 (1)	10.00			
N-M	0 / 2630	-18.5	-18.5 0.52 (1)	10.00			
M-L	0 / 0	-18.5	-18.5 0.20 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TRIC 2014

(58% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/989 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.83/1.00 (B-C:1), BC=0.52/1.00 (P-R:1), WB=0.60/1.00 (B-R:1), SS=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 0.79 (Q) (INPUT = 1.00)

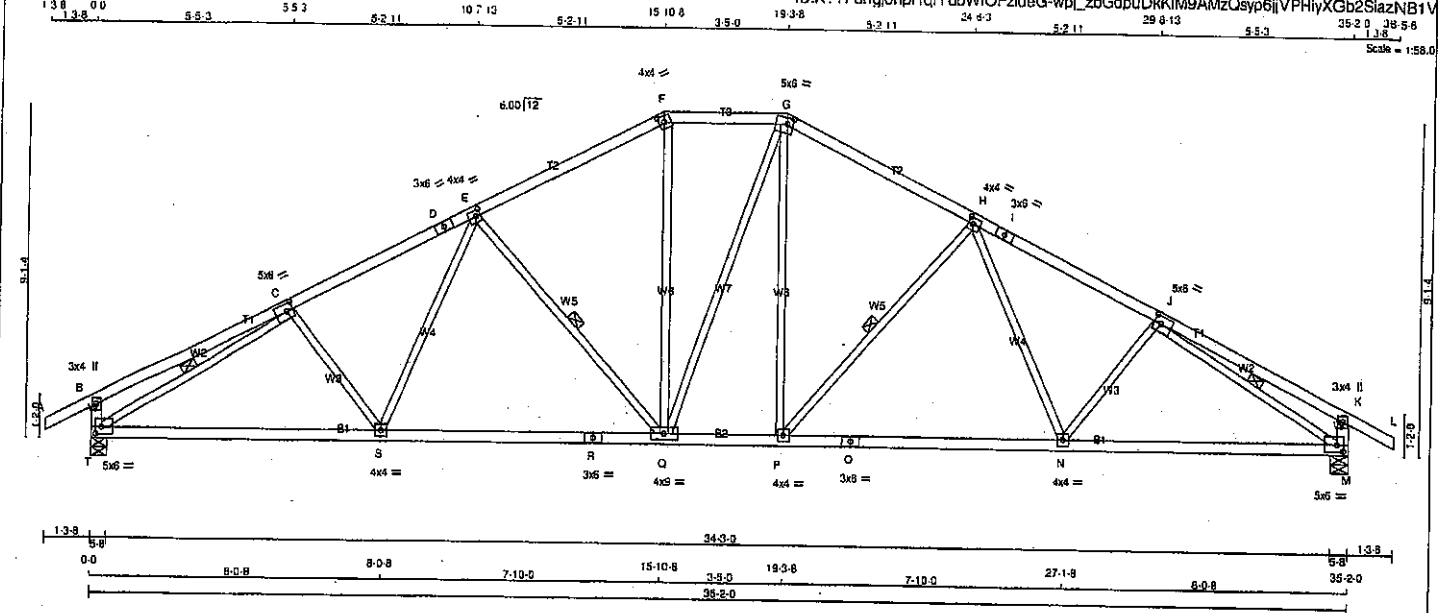


Structural component only
DWG# T-2007173

JOB NAME 408170	TRUSS NAME T45	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 13:02:22 2020 Page 1
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LUMBER

CHORDS		SIZE	LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
L - T	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
B - K	2x4	DRY	No.2	SPF
K - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-1	MT20	5.0	6.0	2.50	2.25
D	TS-1	MT20	3.0	6.0		
E	TMWW-1	MT20	4.0	4.0	2.00	1.50
F	TTW-h	MT20	4.0	4.0	2.00	1.75
G	TTWW-m	MT20	5.0	6.0	2.00	2.00
H	TMWW-1	MT20	4.0	4.0	2.00	1.50
I	TS-1	MT20	3.0	6.0		
J	TMWW-1	MT20	5.0	6.0	2.50	2.25
K	TMV+p	MT20	3.0	4.0		
M	BMVW-1	MT20	5.0	6.0	2.25	2.00
N, P, S						
N	BMWW-1	MT20	4.0	4.0		
O	BS-1	MT20	3.0	6.0		
Q	BMWW-1	MT20	4.0	4.0		
R	BS-1	MT20	3.0	6.0		
T	BMVW-1	MT20	5.0	6.0	2.25	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	BRG	IN-SX	IN-SX
T	2083	0	2083	0	0	5-8	5-8		
M	2083	0	2083	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
M	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, H-P, C-T, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0 / 28	-91.8	0.12 (1)	10.00	C-S	-110 / 37	0.04 (1)
B-C	0 / 19	-91.8	0.12 (1)	10.00	S-E	0 / 276	0.05 (1)
C-D	-2759 / 0	-91.8	0.12 (1)	3.86	E-Q	-681 / 0	0.31 (1)
D-E	-2759 / 0	-91.8	0.12 (1)	3.86	Q-F	0 / 613	0.14 (1)
E-F	-2167 / 0	-91.8	0.12 (1)	4.30	F-G	0 / 4	0.00 (1)
F-G	-1925 / 0	-91.8	0.12 (1)	4.71	G-H	0 / 609	0.14 (1)
G-H	-2168 / 0	-91.8	0.12 (1)	4.30	H-N	-682 / 0	0.31 (1)
H-I	-2760 / 0	-91.8	0.12 (1)	3.86	I-N	0 / 278	0.06 (1)
I-J	-2760 / 0	-91.8	0.12 (1)	3.86	N-J	-110 / 37	0.04 (1)
J-K	0 / 19	-91.8	0.12 (1)	10.00	T-C	-3040 / 0	0.85 (1)
K-L	0 / 28	-91.8	0.12 (1)	10.00	J-M	-3041 / 0	0.85 (1)
T-B	-325 / 0	0.0	0.03 (1)	7.81			
M-K	-326 / 0	0.0	0.03 (1)	7.81			
T-S	0 / 2536	-18.5	0.56 (1)	10.00			
S-R	0 / 2362	-18.5	0.53 (1)	10.00			
R-Q	0 / 2362	-18.5	0.53 (1)	10.00			
Q-P	0 / 1924	-18.5	0.40 (1)	10.00			
P-O	0 / 2362	-18.5	0.53 (1)	10.00			
O-N	0 / 2362	-18.5	0.53 (1)	10.00			
N-M	0 / 2537	-18.5	0.56 (1)	10.00			

TOTAL WEIGHT = 2 X 151 = 302 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.8	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.32")

CSI: TC=0.40/1.00 (H-J:1), BC=0.56/1.00 (M-N:1), WB=0.65/1.00 (J-M:1), SS=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10, COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

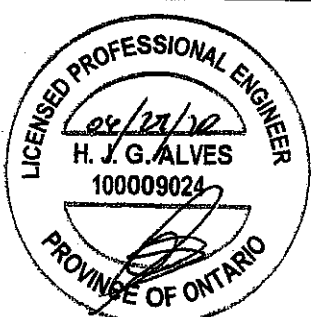
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1666

PLATE PLACEMENT TOL. = 0.250 inches

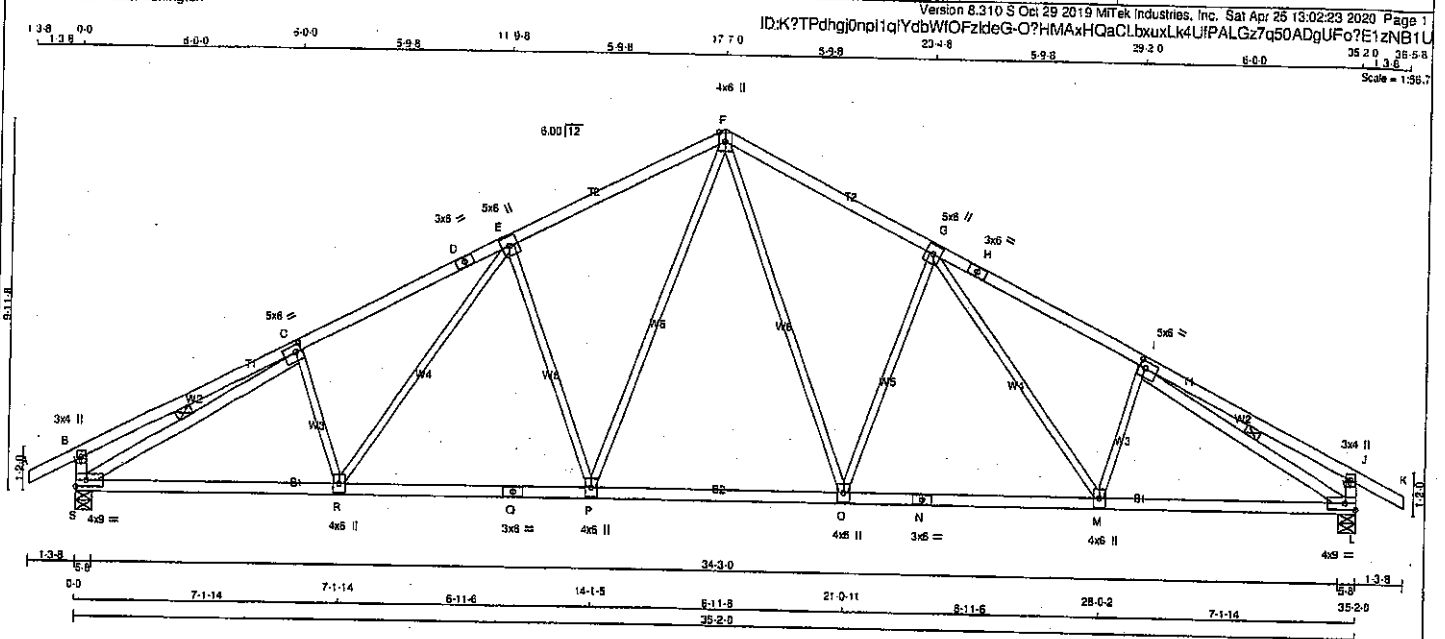
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (M) (INPUT = 0.80)
JSI METAL = 0.78 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007174

JOB NAME 408170	TRUSS NAME T46	QUANTITY 6	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



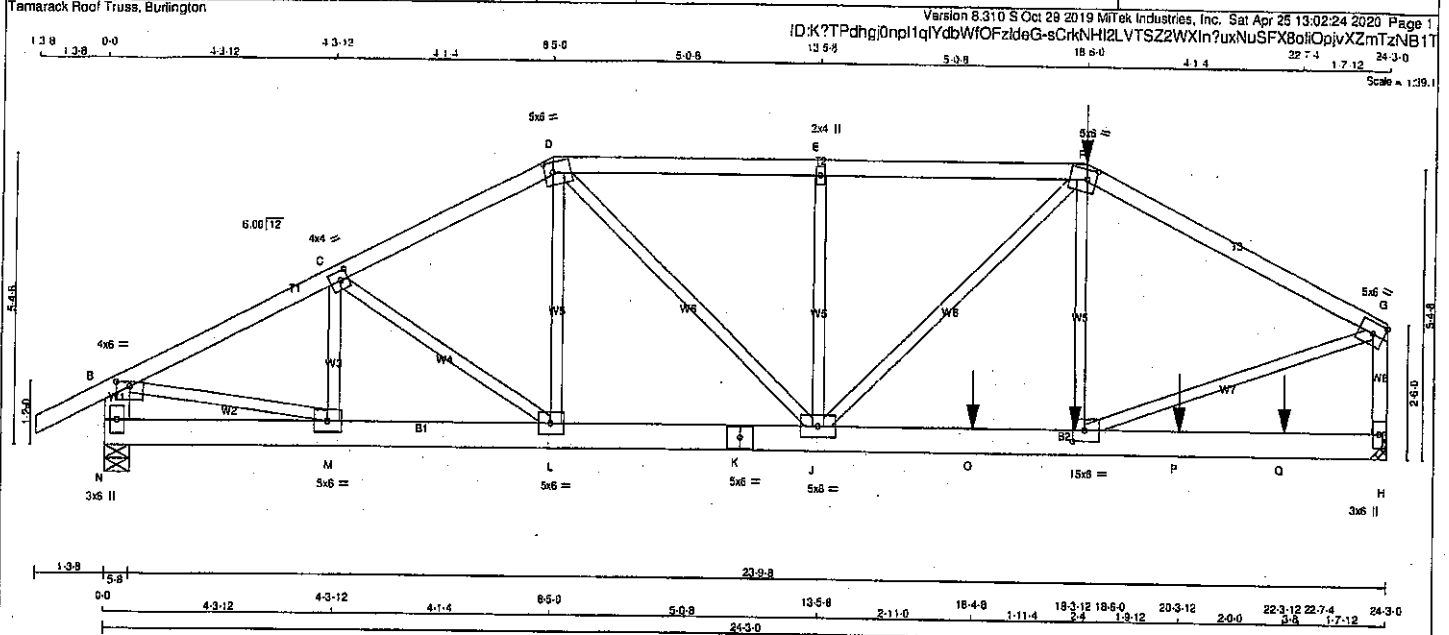
LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				TOTAL WEIGHT = 6 X 152 = 910 lb			
N. L. G. A. RULES				BEARINGS				DESIGN CRITERIA			
CHORDS	SIZE	LUMBER	DESCR.	FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
A - D	2x4	DRY	No.2	GROSS REACTION		GROSS REACTION		BRG		BRG	
D - F	2x4	DRY	No.2	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F - H	2x4	DRY	No.2	S	2063	0	2063	0	0	5-8	5-8
H - K	2x4	DRY	No.2	L	2063	0	2063	0	0	5-8	5-8
S - B	2x4	DRY	No.2	UNFACTORED REACTIONS							
L - J	2x4	DRY	No.2	1ST LOOSE							
S - Q	2x4	DRY	No.2	COMBINED							
Q - N	2x4	DRY	No.2	JT	1457	999 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
N - L	2x4	DRY	No.2	L	1457	999 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
ALL WEBS 2x3 DRY No.2				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L							
EXCEPT				BRACING							
S - C 2x4 DRY No.2				TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.							
I - L 2x4 DRY No.2				MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.							
DRY, SEASONED LUMBER.				ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.							

PLATES (table is in inches)				LOADING				DESIGN CRITERIA			
JT TYPE	PLATES	W	LEN	Y	X	TOTAL LOAD CASES: (4)		SPECIFIED LOADS:			
B TMV+p	MT20	3.0	4.0			1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-S, I-L.		TOP CH. LL = 25.6 PSF			
C TMWW-I	MT20	5.0	6.0	2.25	2.00	END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW		DL = 8.0 PSF			
D TS-I	MT20	3.0	6.0					BOT CH. LL = 0.0 PSF			
E TMWW+I	MT20	5.0	6.0					DL = 7.4 PSF			
F TTWW+p	MT20	4.0	6.0	Edge				TOTAL LOAD = 39.0 PSF			
G TMWW+I	MT20	5.0	6.0					SPACING = 24.0 IN. C/C			
H TS-I	MT20	3.0	6.0					THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015			
I TMWW-I	MT20	5.0	6.0	2.25	2.00			THIS DESIGN COMPLIES WITH:			
J TMV+p	MT20	3.0	4.0					- PART 9 OF BCBC 2018, CBC 2012, ABC 2019			
L BMVW-I	MT20	4.0	9.0	Edge				- PART 9 OF CBC 2012 (2019 AMENDMENT)			
M, O, P, R								- CSA 086-09, CSA 086-14			
N BS-I	MT20	3.0	6.0					- TPIC 2011, TPIC 2014			
Q BS-I	MT20	3.0	6.0					(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD			
S BMVW-I	MT20	4.0	9.0	Edge				ALLOWABLE DEFL.(LL) = L/360 (1.17")			
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.								CALCULATED VERT. DEFL.(LL) = L/999 (0.14")			
								ALLOWABLE DEFL.(TL) = L/360 (1.17")			
								CALCULATED VERT. DEFL.(TL) = L/999 (0.20")			
								CSI: TC=0.49/1.00 (G-I:1), BC=0.53/1.00 (R-S:1), WB=0.72/1.00 (E-P:1), SS=0.22/1.00 (B-C:1)			
								DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10			
								COMPANION LIVE LOAD FACTOR = 1.00			
								AUTOSOLVE HEELS OFF			
								TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
								NAIL VALUES			
								PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)			
								MAX MIN MAX MIN MAX MIN			
								MT20 618 354 1667 768 1987 1655			
								PLATE PLACEMENT TOL. = 0.250 inches			
								PLATE ROTATION TOL. = 5.0 Deg.			
								JSI GRIP = 0.87 (C) (INPUT = 0.90)			
								JSI METAL = 0.77 (I) (INPUT = 1.00)			



Structural component only
DWG# T-2007175

JOB NAME 408170	TRUSS NAME T47	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR.
A - D	2x4	DRY	No.2		SPF
D - F	2x4	DRY	No.2		SPF
F - G	2x4	DRY	No.2		SPF
N - B	2x6	DRY	No.2		SPF
H - G	2x4	DRY	No.2		SPF
N - K	2x6	DRY	No.2		SPF
K - H	2x6	DRY	No.2		SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D	12	TOP
D-F	12	SIDE(61.0)
F-G	12	SIDE(61.0)
H-G	12	TOP
N-B	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
N-K	12	TOP
K-H	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
	GROSS REACTION	VERT	GROSS REACTION	DOWN	BRG	BRG	IN-SX	IN-SX
JT	2120	0	2120	0	5-8	5-8		
N	3119	0	3119	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 4'-0.

UNFACTORED REACTIONS

1ST LOASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	1495	1008 / 0	0 / 0	0 / 0	0 / 0	492 / 0	0 / 0
N	2201	1472 / 0	0 / 0	0 / 0	0 / 0	729 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.84 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO				FR-TO			
A-B	0 / 28	-91.8	-91.8 0.07 (1)	M-C	-408 / 0	0.04 (1)	
B-C	-2914 / 0	-91.8	-91.8 0.17 (1)	C-L	-66 / 0	0.01 (1)	
C-D	-2892 / 0	-91.8	-91.8 0.17 (1)	L-D	0 / 70	0.01 (4)	
D-E	-3517 / 0	-91.8	-91.8 0.27 (1)	D-J	0 / 1336	0.17 (1)	
E-F	-3517 / 0	-91.8	-91.8 0.27 (1)	J-E	-555 / 0	0.12 (1)	
F-G	-3466 / 0	-91.8	-91.8 0.39 (1)	J-F	0 / 575	0.07 (1)	
N-B	-2064 / 0	0.0	0.0 0.07 (1)	F-G	0 / 522	0.06 (1)	
H-G	-2933 / 0	0.0	0.0 0.20 (1)	I-G	0 / 3293	0.41 (1)	
N-M	0 / 0	-18.5	-18.5 0.03 (1)	B-M	0 / 2694	0.33 (1)	
M-L	0 / 2822	-18.5	-18.5 0.21 (1)				
L-K	0 / 2588	-18.5	-18.5 0.33 (1)				
K-J	0 / 2568	-18.5	-18.5 0.33 (1)				
J-O	0 / 3109	-18.5	-18.5 0.69 (1)				
O-I	0 / 3109	-18.5	-18.5 0.69 (1)				
I-P	0 / 0	-18.5	-18.5 0.32 (1)				
P-Q	0 / 0	-18.5	-18.5 0.32 (1)				
Q-H	0 / 0	-18.5	-18.5 0.32 (1)				

FACTORED CONCENTRATED LOADS (LBS)				FACE DIR.		TYPE		HEEL		CONN.	
JT	LOC.	LC1	MAX.	MAX+							
F	18-6-0	-178	-178	---	FRONT	VERT	TOTAL	---	---	C1	
I	18-3-12	-35	-35	---	FRONT	VERT	TOTAL	---	---	C1	
O	16-4-8	-1661	-1661	---	FRONT	VERT	TOTAL	---	---	C1	
P	20-3-12	-283	-283	---	FRONT	VERT	TOTAL	---	---	C1	
Q	22-3-12	-284	-284	---	FRONT	VERT	TOTAL	---	---	C1	

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 089-09, CSA 085-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")

CALCULATED VERT. DEFL.(LL) = L/999 (0.08")

ALLOWABLE DEFL.(TL) = L/360 (0.81")

CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.39/1.00 (F-G:1), BC=0.69/1.00 (I-J:1), WB=0.41/1.00 (G-I:1), SS=0.37/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PS) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

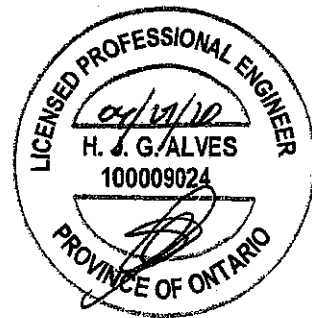
MT20 618 354 1667 788 1987 1866

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.67 (I) (INPUT = 0.90)

JSI METAL = 0.44 (G) (INPUT = 1.00)



Structural component only
DWG# T-2007176 92

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T47	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	8.0	1.00	3.00
C	TMWW-l	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-l	MT20	5.0	6.0		Edge
H	BMV1-p	MT20	3.0	6.0		
I	BMWW-l	MT20	5.0	6.0	2.50	2.75
J	BMWWW-l	MT20	5.0	8.0		
K	BS-l	MT20	5.0	6.0		
L	BMWW-l	MT20	5.0	6.0		
M	BMWW-l	MT20	5.0	6.0		
N	BMV1-p	MT20	3.0	6.0		

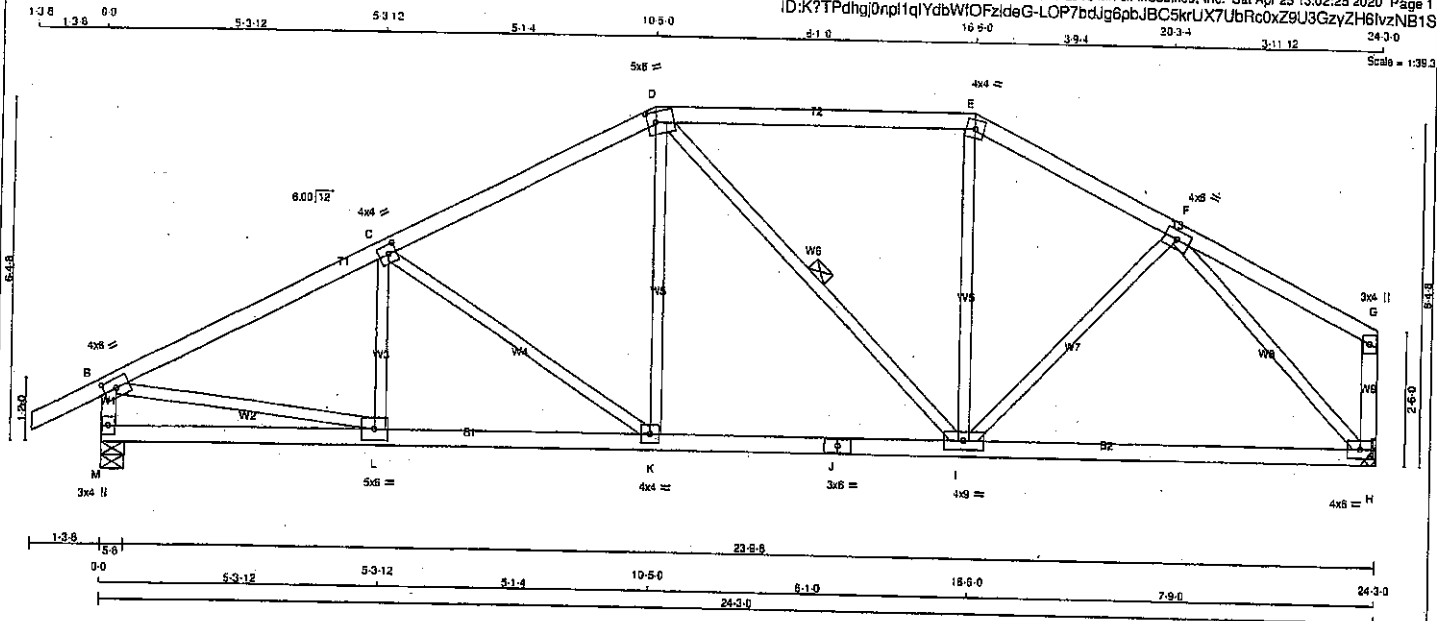
Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



Structural component only
DWG# T-2007176 12

JOB NAME 408170	TRUSS NAME T48	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS			
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	6.0	2.00 3.00
C	TMVW-t	MT20	4.0	4.0	2.00 1.75
D	TTWV-m	MT20	5.0	6.0	2.25 2.00
E	TTW-m	MT20	4.0	4.0	
F	TMVW-t	MT20	4.0	6.0	
G	TMV-p	MT20	3.0	4.0	
H	BMVW1-t	MT20	4.0	6.0	
I	BMVWW-t	MT20	4.0	9.0	
J	BS-t	MT20	3.0	5.0	
K	BMVW-t	MT20	4.0	4.0	
L	BMVW-t	MT20	5.0	6.0	
M	BMV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
M	1481 0	1481 0	0 0	5-8	5-8
H	1397 0	1397 0	0 0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1031	890 / 0	0 / 0	0 / 0	0 / 0	341 / 0	0 / 0
H	945	620 / 0	0 / 0	0 / 0	0 / 0	325 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	L-C	-165 / 32	0.04 (1)
B-C	-1844 / 0	-91.8 -91.8	0.36 (1)	4.56	C-K	-498 / 0	0.31 (1)
C-D	-1488 / 0	-91.8 -91.8	0.34 (1)	5.00	K-D	0 / 334	0.08 (1)
D-E	-1181 / 0	-91.8 -91.8	0.46 (1)	5.27	D-I	-189 / 0	0.09 (1)
E-F	-1336 / 0	-91.8 -91.8	0.18 (1)	5.42	I-E	0 / 175	0.05 (4)
F-G	0 / 18	-91.8 -91.8	0.21 (1)	10.00	I-F	0 / 156	0.04 (4)
M-B	-1419 / 0	0.0 0.0	0.14 (1)	6.85	B-L	0 / 1892	0.38 (1)
H-G	-138 / 0	0.0 0.0	0.02 (1)	7.81	F-H	-1563 / 0	0.82 (1)
M-L	0 / 0	-18.5 -18.5	0.12 (4)	10.00			
L-K	0 / 1871	-18.5 -18.5	0.32 (1)	10.00			
K-J	0 / 1313	-18.5 -18.5	0.35 (4)	10.00			
J-I	0 / 1313	-18.5 -18.5	0.35 (4)	10.00			
I-H	0 / 1069	-18.5 -18.5	0.34 (4)	10.00			

TOTAL WEIGHT = 99 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 38.0 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.48/1.00 (D-E:1), BC=0.36/1.00 (I-K:4), WB=0.82/1.00 (F-H:1), SS=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 1.00)

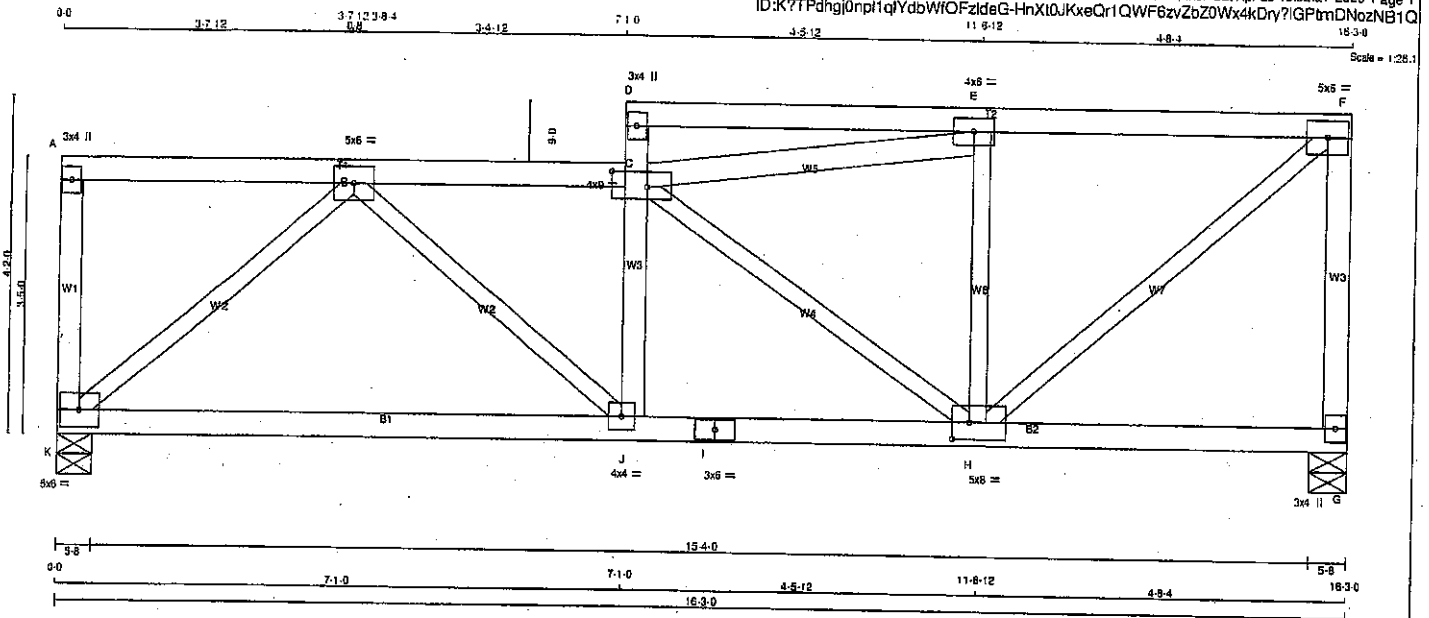


Structural component only
DWG# T-2007177

JOB NAME	TRUSS NAME	QUANTITY	PLY.	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408170	T49	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 72 = 216 lb (MTR)

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
K - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
J - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
EXCEPT			
C - E	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMVW-i	MT20	5.0	6.0		
C	TMVW-i	MT20	4.0	9.0	2.25	5.50
D	TMV+p	MT20	3.0	4.0		
E	TMVW-i	MT20	4.0	6.0		
F	TMVW-i	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-i	MT20	5.0	8.0	2.50	2.50
I	BS-i	MT20	3.0	6.0		
J	BMVW-i	MT20	4.0	4.0		
K	BMVW-i	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
	VERT	HORZ	DOWN	HORZ
JT				
K	1079	0	1079	0
G	1079	0	1079	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	780	416 / 0	0 / 0	0 / 0	0 / 0	364 / 0	0 / 0
G	780	416 / 0	0 / 0	0 / 0	0 / 0	364 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
K-A	-163 / 0	0.0	0.0 0.04 (1)	7.81	K-B	-1328 / 0	0.65 (1)
A-B	0 / 0	-114.3	-114.3 0.32 (1)	10.00	B-J	0 / 538	0.16 (1)
B-C	-1416 / 0	-114.3	-114.3 0.37 (1)	4.47	C-E	-961 / 0	0.30 (1)
J-C	-226 / 0	0.0	0.0 0.08 (1)	7.81	C-H	-489 / 0	0.30 (1)
C-D	-183 / 0	0.0	0.0 0.08 (1)	7.81	H-E	-471 / 0	0.16 (1)
D-E	-58 / 0	-114.3	-114.3 0.51 (1)	8.25	H-F	0 / 1303	0.39 (1)
E-F	-1004 / 0	-114.3	-114.3 0.56 (1)	4.81			
G-F	-1039 / 0	0.0	0.0 0.37 (1)	6.91			
K-J	0 / 1013	-18.5	-18.5 0.41 (4)	10.00			
J-I	0 / 1402	-18.5	-18.5 0.48 (4)	10.00			
I-H	0 / 1402	-18.5	-18.5 0.48 (4)	10.00			
H-G	0 / 0	-18.5	-18.5 0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 15.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 48.0	PSF

SPACING = 24.0 IN. G/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.03/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.54")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.54")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.11")

CSI: TC=0.55/1.00 (E-F-1), BC=0.49/1.00 (H-J-4), WB=0.65/1.00 (B-K-1), SSI=0.34/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

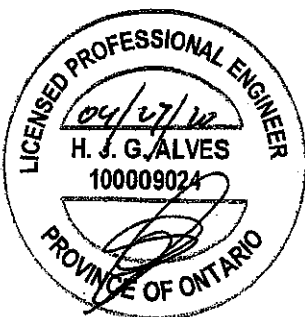
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	616 354	1667 788	1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL = 0.62 (I) (INPUT = 1.00)

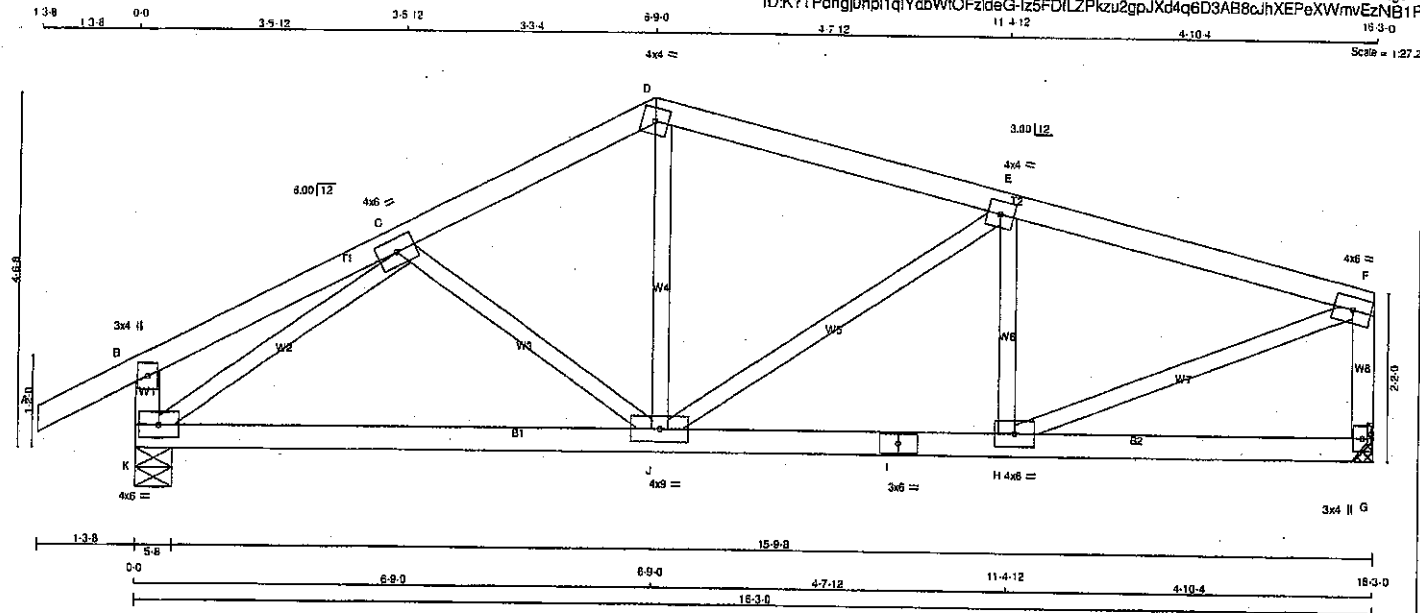


Structural component only
DWG# T-2007178

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408170	T50	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 63 = 126 lb
[M/F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-t	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	9.0		
K	BMV1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
K	1020	0	1020	0
G	896	0	896	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	719	465 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0
G	833	418 / 0	0 / 0	0 / 0	0 / 0	218 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	C-J	-135 / 7	0.04 (1)
B-C	0 / 14	-91.8	-91.8 0.15 (1)	10.00	J-D	0 / 355	0.08 (1)
C-D	-962 / 0	-91.8	-91.8 0.13 (1)	6.20	J-E	-283 / 0	0.13 (1)
D-E	-894 / 0	-91.8	-91.8 0.26 (1)	8.18	H-E	-335 / 0	0.07 (1)
E-F	-1094 / 0	-91.8	-91.8 0.27 (1)	5.73	K-C	-1199 / 0	0.35 (1)
K-B	-248 / 0	0.0	0.0 0.03 (1)	7.81	H-F	0 / 1150	0.26 (1)
G-F	-855 / 0	0.0	0.0 0.10 (1)	7.81			
K-J	0 / 866	-18.5	-18.5 0.26 (1)	10.00			
J-I	0 / 1073	-18.5	-18.5 0.27 (1)	10.00			
I-H	0 / 1073	-18.5	-18.5 0.27 (1)	10.00			
H-G	0 / 0	-18.5	-18.5 0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 8.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.54")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.54")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.27/1.00 (E-F:1), BC=0.27/1.00 (H-J:1), WB=0.25/1.00 (C-K:1), SS=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

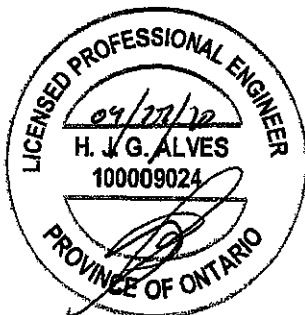
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PL)	SECTION (PL)
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618	354
	1657	788
	1987	1658

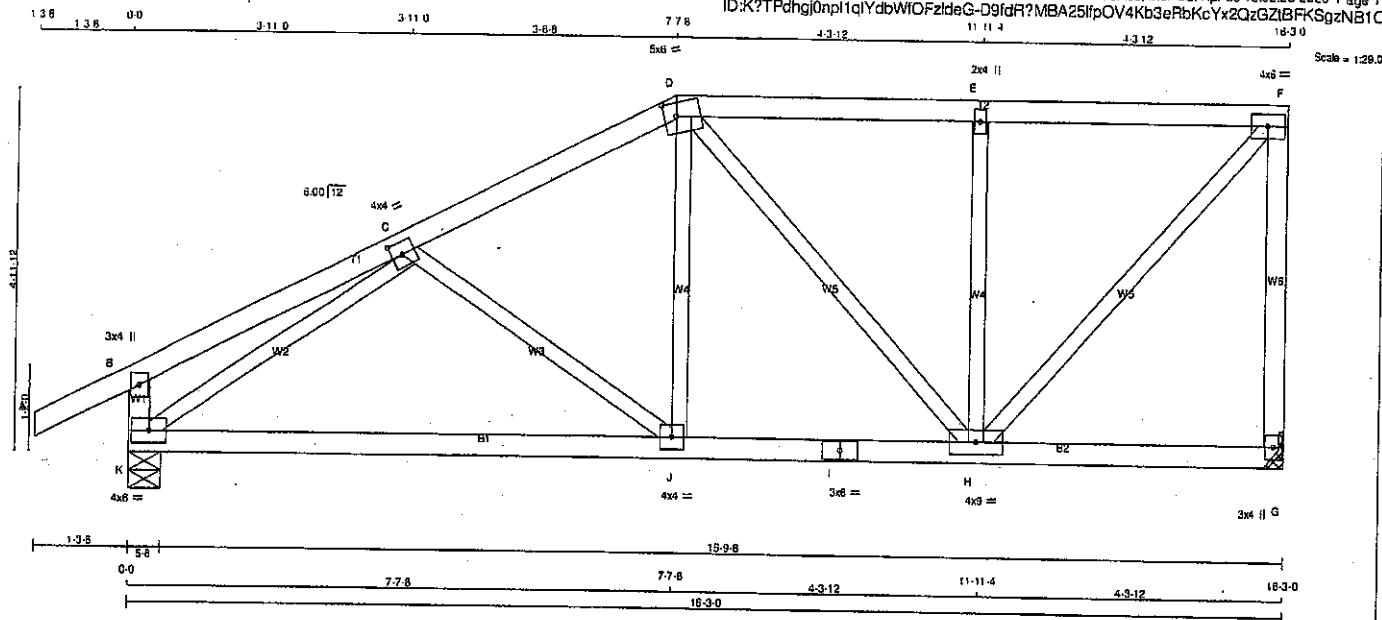
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (H) (INPUT = 0.90)
JSI METAL= 0.32 (I) (INPUT = 1.00)



Structural component only
DWG# T-2007179



TOTAL WEIGHT = 70 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS EXCEPT 2x3 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTWV-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-t	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVWV-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
G	896 0	896 0	0	MECHANICAL
K	1020 0	1020 0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

1ST LOASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED
G	633 418 / 0
K	719 485 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.21 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LC)	UNBRACED LENGTH FR-TO
FR-TO				FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	C-J	-215 / 0	0.08 (1)	10.00
B-C	0 / 17	-91.8	-91.8 0.20 (1)	J-D	0 / 256	0.06 (1)	10.00
C-D	-901 / 0	-91.8	-91.8 0.22 (1)	D-H	-210 / 0	0.14 (1)	6.21
D-E	-654 / 0	-91.8	-91.8 0.29 (1)	H-E	-488 / 0	0.18 (1)	6.25
E-F	-654 / 0	-91.8	-91.8 0.29 (1)	H-F	0 / 981	0.22 (1)	6.25
F-G	-859 / 0	0.0	0.0 0.35 (1)	K-C	-1192 / 0	0.43 (1)	7.81
G-B	-261 / 0	0.0	0.0 0.03 (1)				7.81
K-J	0 / 965	-18.5	-18.5 0.30 (4)				10.00
J-I	0 / 796	-18.5	-18.5 0.30 (4)				10.00
I-H	0 / 796	-18.5	-18.5 0.30 (4)				10.00
H-G	0 / 0	-18.5	-18.5 0.08 (4)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2016, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.54")

CALCULATED VERT. DEFL.(LL) = L/999 (0.02")

ALLOWABLE DEFL.(TL) = L/860 (0.54")

CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.35/1.00 (F-G:1), BC=0.30/1.00 (J-K:4)

WB=0.49/1.00 (C-K:1), SSI=0.19/1.00 (E-F:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

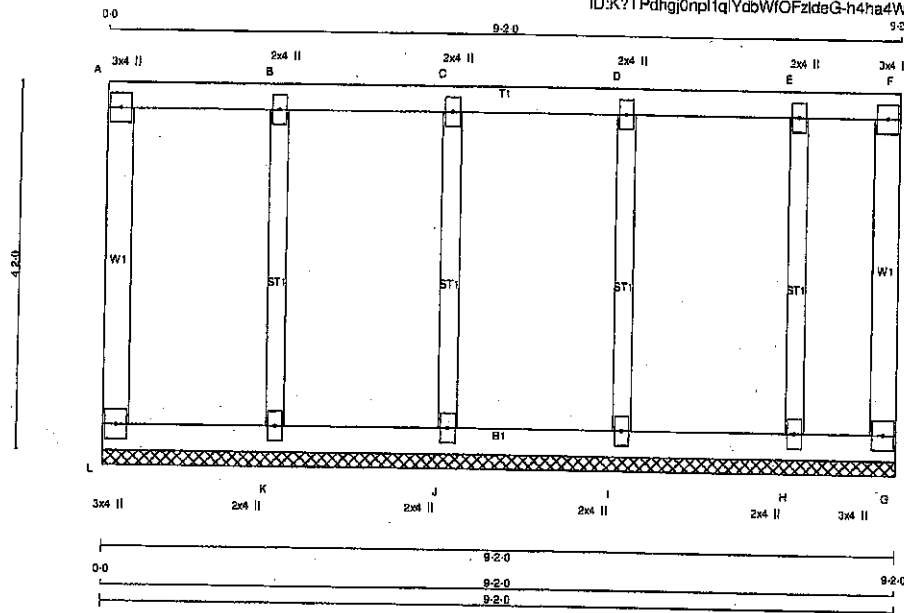
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.80)

JSI METAL= 0.40 (C) (INPUT = 1.00)



JOB NAME 408170	TRUSS NAME G52	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 13:02:13 2020 Page 1	



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
L - A	2x4	DRY	No.2	SPF	
A - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
L - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY, SEASONED LUMBER.					

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B, C, D, E						
B	TMW+w	MT20	2.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMV1+p	MT20	3.0	4.0		
H, I, J, K						
H	BMW1+w	MT20	2.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
L-A	-100 / 0	0.0	0.0	0.02 (1)	7.81	K-B	-245 / 0
A-B	-4 / 0	-114.3	-114.3	0.08 (1)	10.00	J-C	-224 / 0
B-C	-4 / 0	-114.3	-114.3	0.08 (1)	10.00	I-D	-238 / 0
C-D	-4 / 0	-114.3	-114.3	0.07 (1)	10.00	H-E	-188 / 0
D-E	-4 / 0	-114.3	-114.3	0.07 (1)	10.00		
E-F	-4 / 0	-114.3	-114.3	0.05 (1)	10.00		
G-F	-54 / 0	0.0	0.0	0.02 (1)	7.81		
L-K	0 / 4	-18.5	-18.5	0.02 (4)	10.00		
K-J	0 / 4	-18.5	-18.5	0.02 (4)	10.00		
J-I	0 / 4	-18.5	-18.5	0.02 (4)	10.00		
I-H	0 / 4	-18.5	-18.5	0.02 (4)	10.00		
H-G	0 / 4	-18.5	-18.5	0.01 (4)	10.00		

TOTAL WEIGHT = 41 lb (M/P)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 15.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 48.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 085-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (B-C:1), BC=0.02/1.00 (J-K:4), WB=0.09/1.00 (B-K:1), SS=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PS)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1657 788 1987 1656	

PLATE PLACEMENT TOL. = 0.250 inches

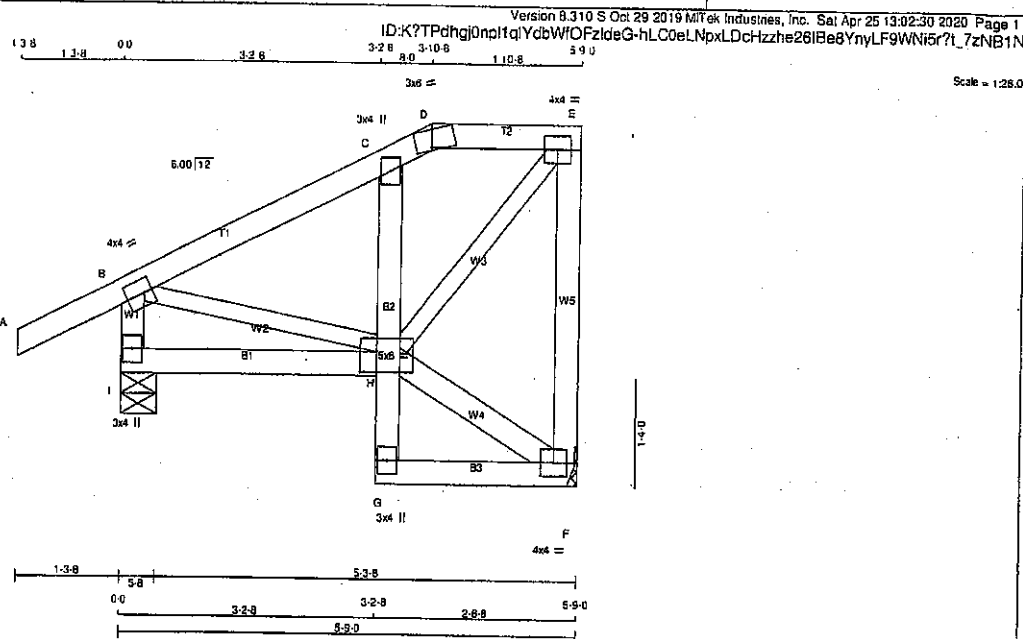
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.17 (B) (INPUT = 0.90)
JSI METAL = 0.07 (K) (INPUT = 1.00)



Structural component only
DWG# T-2007166

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T53S	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2			SPF
D - E	2x4	DRY	No.2			SPF
F - E	2x4	DRY	No.2			SPF
I - B	2x4	DRY	No.2			SPF
I - H	2x4	DRY	No.2			SPF
G - C	2x4	DRY	No.2			SPF
G - F	2x4	DRY	No.2			SPF
ALL WEBS	2x4	DRY	No.2			SPF
EXCEPT						
B - H	2x3	DRY	No.2			SPF
H - E	2x3	DRY	No.2			SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	TT-m	MT20	3.0	5.0	0.50	2.50
E	TMVW-t	MT20	4.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMV+p	MT20	3.0	4.0		
H	BVMWVW1-t	MT20	5.0	8.0	3.00	2.50
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	IN-SX	IN-SX	
F	302	0	302	0	0	MECHANICAL	
I	457	0	457	0	0	5-8	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS							
JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD
F	214	138 / 0	0 / 0	0 / 0	0 / 0	0 / 0	75 / 0
I	320	225 / 0	0 / 0	0 / 0	0 / 0	0 / 0	95 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING							
TOTAL LOAD CASES: (4)							
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
FR-TO					FR-TO		
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	H-F	-10 / 0	0.00 (1)
B-C	-217 / 0	-91.8	-91.8 0.14 (1)	6.25	B-H	0 / 153	0.03 (1)
C-D	-200 / 0	-91.8	-91.8 0.05 (1)	6.25	H-E	0 / 247	0.06 (1)
D-E	-167 / 0	-91.8	-91.8 0.03 (1)	6.25			
F-E	-273 / 0	0.0	0.0 0.03 (1)	7.81			
I-B	-420 / 0	0.0	0.0 0.04 (4)	7.81			
H-H	0 / 41	-18.5	-18.5 0.05 (4)	10.00			
G-H	0 / 25	0.0	0.0 0.02 (1)	10.00			
H-C	-185 / 0	0.0	0.0 0.04 (1)	7.81			
G-F	0 / 15	-18.5	-18.5 0.02 (4)	10.00			

TOTAL WEIGHT = 33 lb

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL =	25.6 PSF
DL =	5.0 PSF
BOT CH. LL =	0.0 PSF
DL =	7.4 PSF
TOTAL LOAD =	38.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (B-C:1), BC=0.05/1.00 (H-E:4), WB=0.08/1.00 (E-H:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

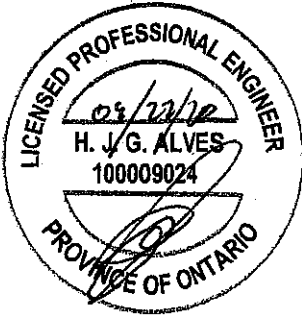
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	PLI	MAX MIN	MAX MIN
MT20	818	354	1687	788	1987

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.36 (H) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



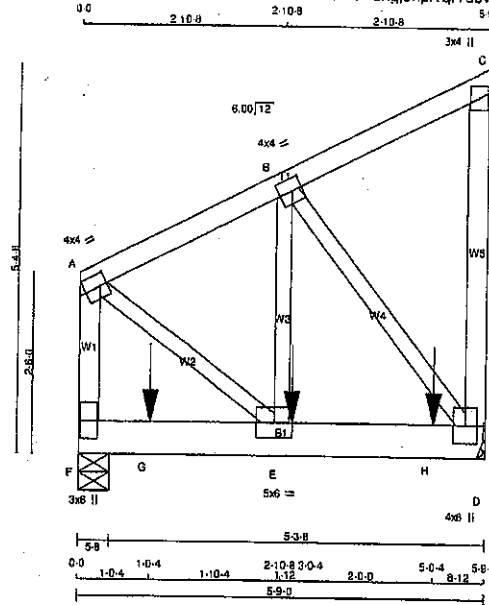
Structural component only
DWG# T-2007181

Structural component only
DWG# T-2007182

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408170	T55	1	2	TRUSS DESC.		

Tamareck Roof Truss, Burlington

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Scale = 1/29.2

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF		
A - C	2x4	DRY	No.2	SPF		
D - C	2x4	DRY	No.2	SPF		
F - D	2x6	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A 1 12	TOP	
A - C 1 12	TOP	
C - D 1 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D 2 12	SIDE (183.1)	
WEBS : (0.122"x3") SPIRAL NAILS		
B - E 1 6	SIDE (45.8)	
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW4	MT20	4.0	4.0	2.00	1.25
B	TMVW4	MT20	4.0	4.0	2.00	1.75
C	TMVW4	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW1	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UP
F	1589	0	1589	0
D	1680	0	1680	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	1121	750 / 0	0 / 0	0 / 0	0 / 0	371 / 0	0 / 0
D	1185	792 / 0	0 / 0	0 / 0	0 / 0	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL FITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 CSI (LC)
FR-TO		FROM TO			FR-TO		
F-A	-1074 / 0	0.0 0.0	0.07 (1)	7.81	A-E	0 / 975	0.12 (1)
A-B	-967 / 0	-91.8 -91.8	0.06 (1)	6.25	E-8	0 / 1044	0.13 (1)
B-C	-12 / 0	-91.8 -91.8	0.05 (1)	6.25	B-D	-1248 / 0	0.21 (1)
D-C	-105 / 0	0.0 0.0	0.02 (1)	7.81			
F-G	0 / 0	-18.5 -18.5	0.19 (1)	10.00			
G-E	0 / 0	-18.5 -18.5	0.19 (1)	10.00			
E-H	0 / 788	-18.5 -18.5	0.21 (1)	10.00			
H-D	0 / 788	-18.5 -18.5	0.21 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-0-4	-877	-877	---	BACK	VERT	TOTAL	---	C1
G	1-0-4	-877	-877	---	BACK	VERT	TOTAL	---	C1
H	5-0-4	-880	-880	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 34 = 67 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/380 (0.19")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.01")
ALLOWABLE DEFL. (TL) = 1/360 (0.19")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.01")

CSI: TC=0.07/1.00 (A-F:1), BC=0.21/1.00 (D-E:1), WB=0.21/1.00 (B-D:1), SS=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

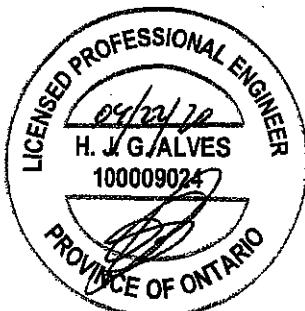
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1557 788 1957 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.72 (A) (INPUT = 0.90)
JSI METAL = 0.18 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

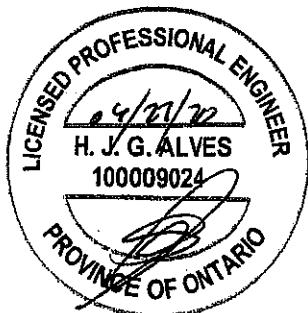


Structural component only
DWG# T-20071831/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T55	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

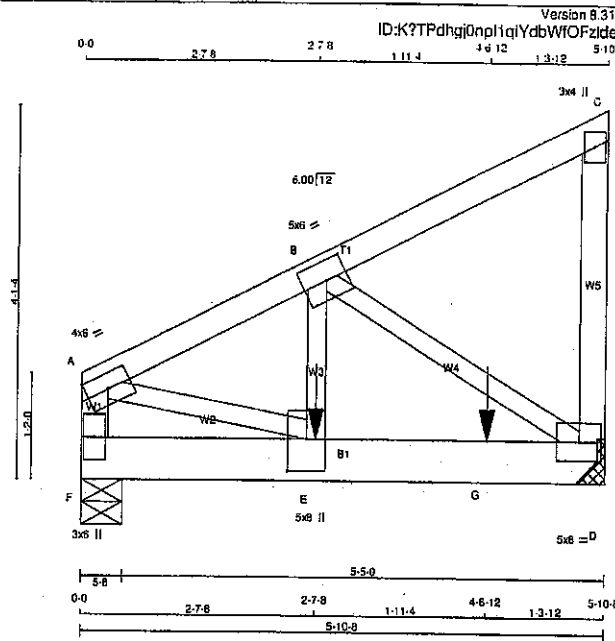
Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 13:02:32 2020 Page 2
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PLATES (table is in inches)
 JT TYPE PLATES W LEN Y X
 F BMV1+p M720 3.0 6.0



Structural component only
 DWG# T-2007183

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T56	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			



LUMBER	SIZE	DRY	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A	12	TOP
A - C	12	TOP
C - D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	5	SIDE (549.3)
WEBS : (0.122"x3") SPIRAL NAILS		
B - E	2	SIDE (259.5)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-1	MT20	4.0	8.0	Edge	
B	TMWW-1	MT20	5.0	8.0	2.50	2.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-1	MT20	5.0	6.0		
E	BMVW1-1	MT20	5.0	8.0	4.25	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	DOWN	BRG	BRG
F	2334	0	0	5-8
D	2733	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
F	1648	1103 / 0	0 / 0	0 / 0	543 / 0	0 / 0
D	1929	1280 / 0	0 / 0	0 / 0	639 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
F-A	-2316 / 0	0.0	0.0	0.13 (1)	7.37	A-E	0 / 2898
A-B	-3101 / 0	-91.8	-91.8	0.08 (1)	5.20	E-B	0 / 2880
B-C	-5 / 0	-91.8	-91.8	0.08 (1)	10.00	B-D	-3303 / 0
D-C	-138 / 0	0.0	0.0	0.02 (1)	7.81		
F-E	0 / 0	-18.5	-18.5	0.01 (4)	10.00		
E-G	0 / 2778	-18.5	-18.5	0.57 (1)	10.00		
G-D	0 / 2778	-18.5	-18.5	0.57 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
E	2-7-8	-3101	-3101	---	BACK	VERT	TOTAL	C1
G	4-6-12	-1318	-1318	---	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, OBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.13/1.00 (A-F:1), SC=0.57/1.00 (D-E:1), WB=0.42/1.00 (B-D:1), SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

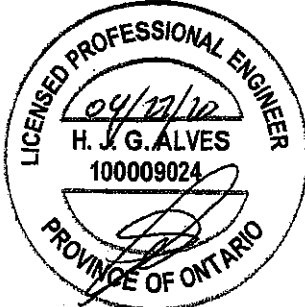
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (E) (INPUT = 0.90)
JSI METAL = 0.50 (E) (INPUT = 1.00)



Structural component only
DWG# T-2007184 1/2

JOB NAME 408170	TRUSS NAME T56	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
F BMV1+p MT20 3.0 6.0
Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



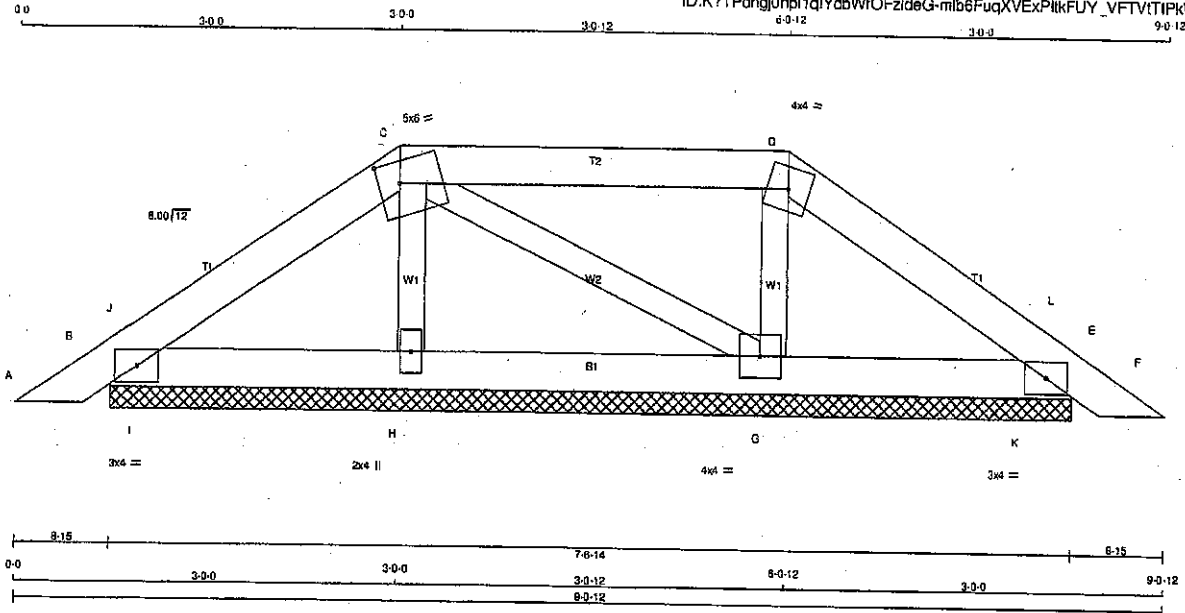
Structural component only
DWG# T-2007184 42

Structural component only
DWG# T-2007128

JOB NAME 408169	TRUSS NAME PB20	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
B - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
SPF DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.00	2.00
D	TTWW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMW1-I	MT20	4.0	4.0		
H	BMW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
JT	221	0	221	0	7-6-14	7-6-14
B	209	0	209	0	7-6-14	7-6-14
E	248	0	248	0	7-6-14	7-6-14
H	280	0	280	0	7-6-14	7-6-14

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM/LIVE	WIND	DEAD	SOIL
B	154	113/0	0/0	0/0	0/0	41/0	0/0
E	146	106/0	0/0	0/0	0/0	39/0	0/0
H	178	109/0	0/0	0/0	0/0	67/0	0/0
G	198	127/0	0/0	0/0	0/0	71/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED TORSION (LBS-FT)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/15	-91.8	-91.8	0.03 (1)	H-C	-176/0	0.03 (1)
B-C	-63/0	-91.8	-91.8	0.01 (1)	C-G	-21/0	0.00 (1)
C-D	-74/0	-91.8	-91.8	0.05 (1)	G-D	-198/0	0.03 (1)
D-E	-30/0	-91.8	-91.8	0.15 (1)	E-J	-121/0	0.00 (1)
E-F	-54/0	-91.8	-91.8	0.06 (1)	J-K	-123/0	0.00 (1)
F-G	-41/0	-91.8	-91.8	0.01 (1)			
G-H	0/15	-91.8	-91.8	0.03 (1)			
H-I	0/60	-18.5	-18.5	0.08 (1)			
I-H	0/60	-18.5	-18.5	0.08 (1)			
H-G	0/48	-18.5	-18.5	0.03 (1)			
G-K	0/43	-18.5	-18.5	0.05 (1)			
K-E	0/43	-18.5	-18.5	0.05 (1)			

TOTAL WEIGHT = 3 X 25 = 75 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2018 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (C-D:1), BC=0.08/1.00 (B-I:1), WB=0.03/1.00 (D-G:1), SS=0.11/1.00 (C-D:1)

DOL LUMBER=1.60 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

ISI GRIP = 0.18 (B) (INPUT = 0.90)
ISI METAL = 0.04 (B) (INPUT = 1.00)



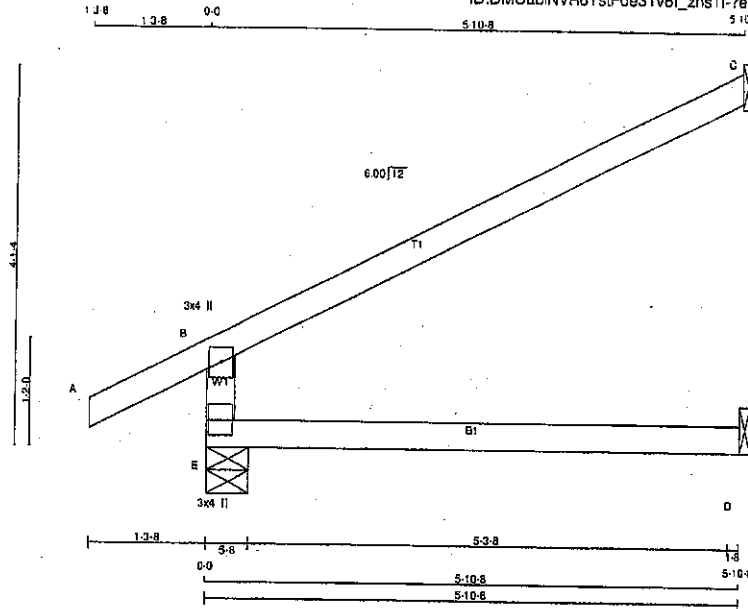
Structural component only
DWG# T-2007144

JOB NAME 408168	TRUSS NAME J1	QUANTITY 19	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6L_zns11-?e_pnKBWAC7oo1JURIN76ba9zNy3aYKn_h9zEzNBRO

Scale = 1/22.5



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS						RECORD	
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	525	0	525	0	0	5-8	5-8
C	202	0	202	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED							
E	389	257 / 0	0 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	UNBRAC LENGTH	MEMB. MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO			FR-TO	
E-B	-461 / 0	0.0	0.0	0.13 (4)	7.81		
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-30 / 0	-91.8	-91.8	0.54 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00		

TOTAL WEIGHT = 19 X 17 = 319 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/960$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/960$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

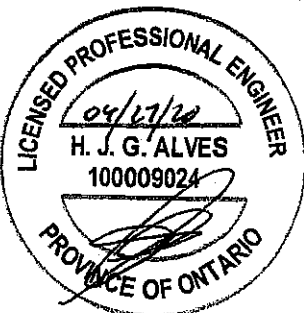
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1937 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

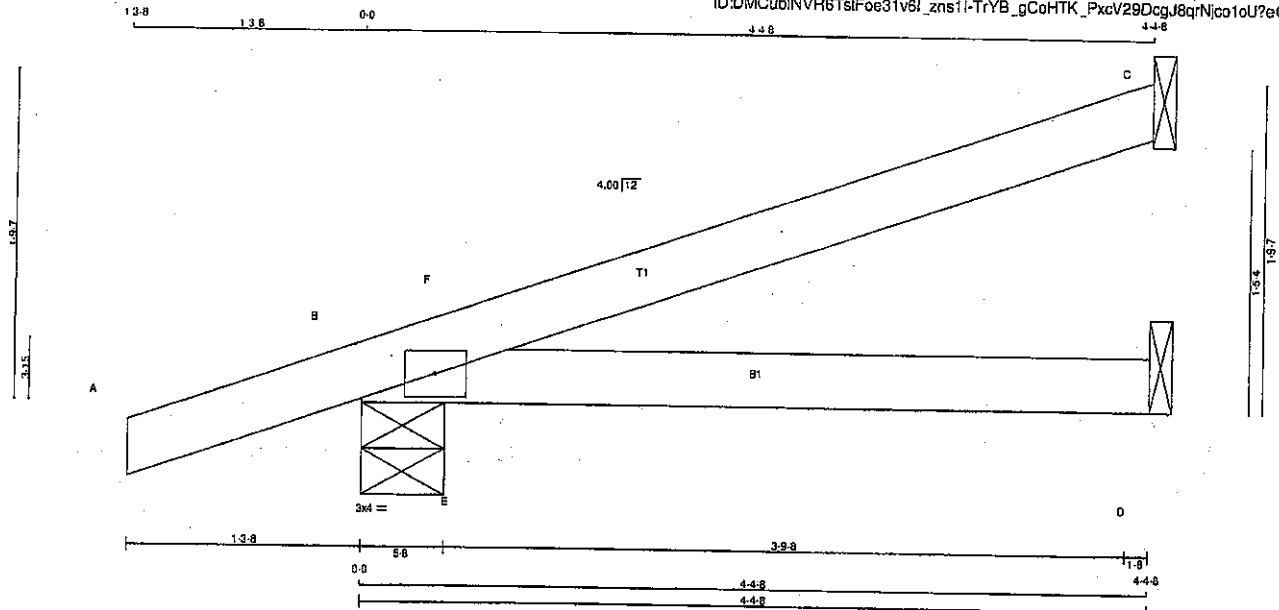


Structural component only
DWG# T-2007126

JOB NAME 408168	TRUSS NAME J2	QUANTITY 8	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TsfFoe31v6f_zns1i-TRYB_gCoHTK_PxcV29DcgJ8qrNico1oU?eQVhZNBRRP



Scale = 1/11.5

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
C	174	0	174	0	1-8	1-8
B	384	0	384	0	5-8	5-8
D	68	0	68	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	120	93/0	0/0	0/0	0/0	27/0	0/0
B	255	160/0	0/0	0/0	0/0	75/0	0/0
D	50	19/0	0/0	0/0	0/0	32/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. FACTORED	MEMB.	WEBS		MAX. FACTORED
	MAX. FORCE (LBS)	VERT. LOAD (LBS)	VERT. LOAD (LBS)	CS (LC)			MAX. FORCE (LBS)	CS (LC)	
FR-TO						FR-TO			
A-B	0/18	-91.8	-91.8	0.11 (1)	10.00	E-F	-194.7	0.00 (1)	
B-F	-14/0	-91.8	-91.8	0.05 (4)	8.25				
F-C	0/2	-91.8	-91.8	0.22 (1)	10.00				
B-E	0/0	-18.5	-18.5	0.17 (1)	10.00				
E-D	0/0	-18.5	-18.5	0.17 (1)	10.00				

TOTAL WEIGHT = 8 X 12 = 95 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.02")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.05")

CS: TC=0.22/1.00 (C-F:1), BC=0.17/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.16/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

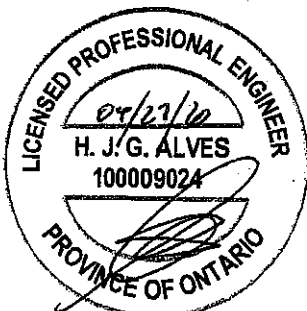
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JS: GRIP = 0.24 (B) (INPUT = 0.30)
JSI METAL = 0.07 (B) (INPUT = 1.00)

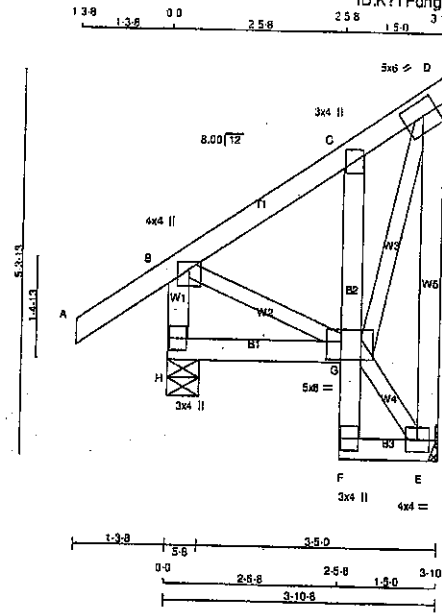


Structural component only
DWG# T-2007127

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	J20S	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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ID:K7TPdHgJOnp1qYdbWtOFzIdG-I21k2YpvkoxY7J92wrSGiGzisz3mg3ZgbMdnLzNBD

Scale = 1/20.3



TOTAL WEIGHT = 3 X 29 = 86 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF		
A - D	2x4	DRY	No.2	SPF		
H - G	2x4	DRY	No.2	SPF		
F - C	2x4	DRY	No.2	SPF		
F - E	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT	E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMV+p	MT20	3.0	4.0		
D	TMVW-H	MT20	5.0	8.0	2.50	2.50
E	BMVW-H	MT20	4.0	4.0		
F	BMV+p	MT20	3.0	4.0		
G	BMVW-H	MT20	5.0	8.0	3.00	2.50
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
H	332	0	332	0
E	206	0	206	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	232	166 / 0	0 / 0	0 / 0	0 / 0	88 / 0	0 / 0
E	145	85 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	MEMB.	FORCE
FR-TO	(LBS)	FR-TO	(LBS)
H-B	-307 / 0	B-G	0 / 83
A-B	0 / 35	E-D	-184 / 0
B-C	-83 / 0	G-E	-14 / 0
C-D	-100 / 0	G-D	0 / 220
H-G	0 / 0		
F-G	0 / 12		
G-C	-218 / 0		
F-E	0 / 9		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA C88-09, CSA C88-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.18")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/980 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TO=0.14/1.00 (A-B-5), BC=0.04/1.00 (G-H-4),
WB=0.05/1.00 (D-E-1), SSI=0.09/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

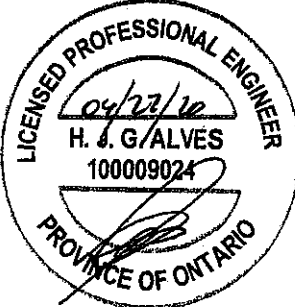
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.21 (B) (INPUT = 0.90)
JSI METAL = 0.07 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007143

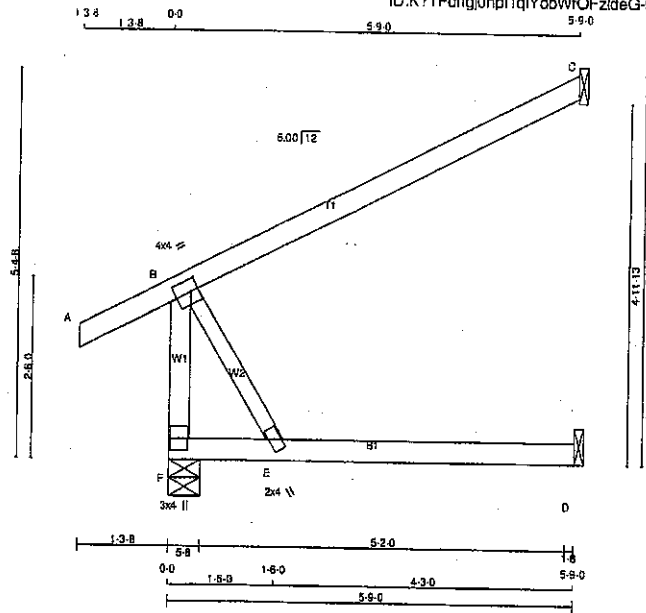
JOB NAME 408170	TRUSS NAME J40	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:K7TPdhg0npl1qYdbWfOFzIdeG-9GFyHsAniRCIMWfchqYYGJluVsSPHGLQM61Q2zNB1C

Scale = 1/29.2



LUMBER

N. L. G. A. RULES	SIZE	LUMBER
CHORDS		
F - B	2x4	DRY No.2
A - C	2x4	DRY No.2
F - D	2x4	DRY No.2
ALL WEBS	2x3	DRY No.2
DRY: SEASONED LUMBER.		

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
E	BMV-w	MT20	2.0	4.0		
F	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			MAXIMUM FACTORED			INPUT	REQD
	FACTORED GROSS REACTION		FACTORED GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	441	0	441	0	0	5-8	5-8
C	264	0	264	0	0	1-8	1-8
D	53	0	60	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	310	217 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0
C	182	147 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. CSI (LC)	UNBRACED LENGTH	MEMB.	WEBS		MAX. CSI (LC)
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	FROM	TO				MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO										
F-B	-388 / 0	0.0	0.0	0.05 (1)	7.81	B-E	0 / 0	0.00 (1)		
A-B	0 / 26	-91.8	-91.8	0.12 (1)	10.00					
B-C	0 / 0	-91.8	-91.8	0.52 (1)	10.00					
F-E	0 / 0	-18.5	-18.5	0.14 (4)	10.00					
E-D	0 / 0	-18.5	-18.5	0.18 (4)	10.00					

TOTAL WEIGHT = 20 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/380$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/899$ (0.00")
ALLOWABLE DEFL. (TL) = $L/380$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/899$ (0.05")

CSI: TC=0.52/1.00 (B-C:1), BC=0.18/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

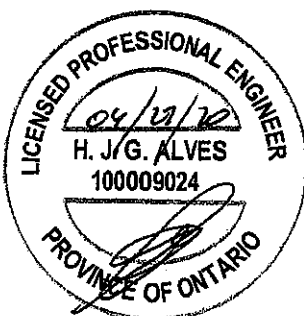
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.24 (B) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)

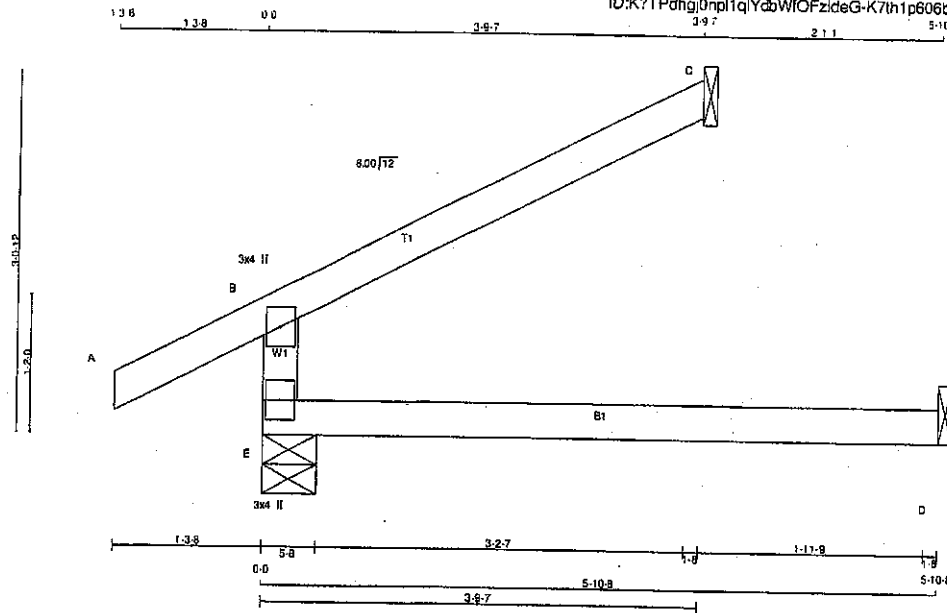


Structural component only
DWG# T-2007167

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	C40	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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Scale = 1:17.5



LUMBER

N. L. G. A. RULES	SIZE	LUMBER
CHORDS	2x4	DRY
E - B	2x4	DRY
A - C	2x4	DRY
E - D	2x4	DRY

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
E BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	405	0	405	0	5-8	5-8
C	130	0	130	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	285	190 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1	MAX. CSI (LC)	UNBRACED LENGTH	MEMB. FORCE (LBS)
FR-TO		FROM TO				
E-B	-342 / 0	0.0	0.0	0.13 (4)	7.81	
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
B-C	-19 / 0	-91.8	-91.8	0.22 (1)	6.25	
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00	

TOTAL WEIGHT = 3 X 14 = 42 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 8.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/869$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/998$ (0.03")

CSI: TC=0.22/1.00 (B-C-1), BC=0.13/1.00 (D-E-4),
WB=0.00/1.00 (n/a:0), SSI=0.15/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788

PLATE PLACEMENT TOL. = 0.250 inches

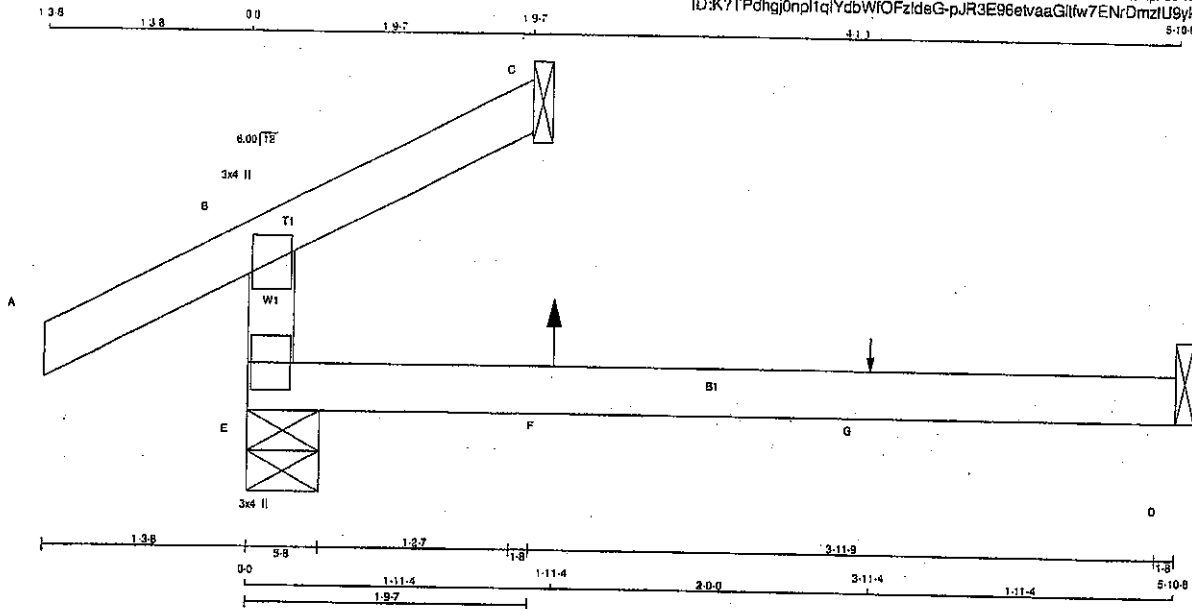
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.14 (E) (INPUT = 0.90)
JSI METAL = 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007162

JOB NAME 408170	TRUSS NAME C41	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 13:02:09 2020 Page 1 ID:K7TPdhgJ0nplqYdbWIOFzIdeG-pJR3E96eWaaGiltw7ENrDmzlU9yk0tcG4PGkrzNB1j	



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	
CHORDS	2x4	DRY	No.2
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y
B	TMV+p	MT20	3.0	4.0	X
E	BMV+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	VERT	GROSS REACTION	BRG	BRG
E	284	0	284	0	5-8
C	63	0	63	0	1-8
D	44	0	52	0	1-8

SEE MITEK STANDARD DETAIL B9779:1H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	2ND CASE	3RD CASE	4TH CASE	5TH CASE	6TH CASE	7TH CASE
E	200	137/0	0/0	0/0	0/0	62/0	0/0
C	46	21/0	0/0	0/0	0/0	25/0	0/0
D	35	0/3	0/0	0/0	0/0	37/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
E-B	-227/0	E-B	7.81
A-B	0/28	A-B	10.00
B-C	-9/9	B-C	10.00
E-F	0/0	E-F	10.00
F-G	0/0	F-G	10.00
G-D	0/0	G-D	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIRL	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	BACK	VERT	TOTAL	--	C1
G	3-11-4	1	1	--	BACK	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 3 X 12 = 35 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPI 2011, TPI 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/989 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.12/1.00 (A-B:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (A-B:0), SS=0.09/1.00 (A-B:1)

DOL LUMBER=0.98 NAIL=0.98 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



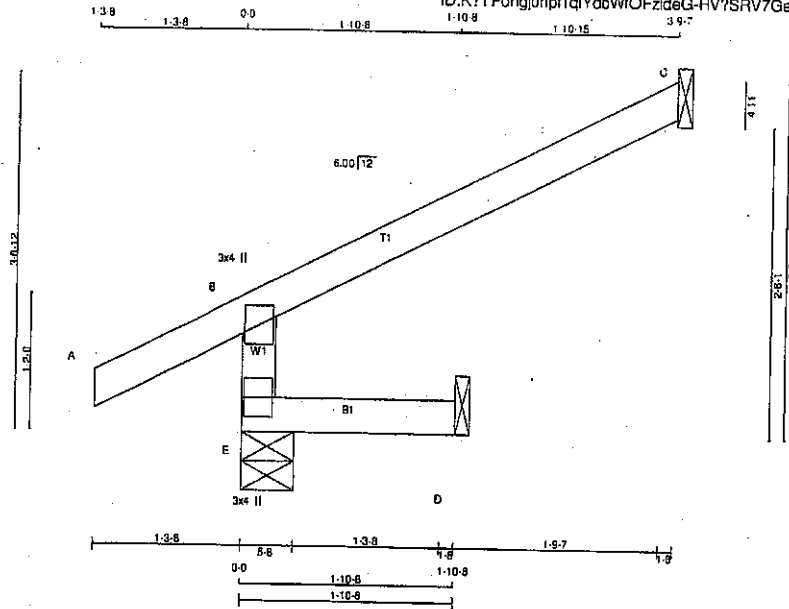
Structural component only
DWG# T-2007163

JOB NAME 408170	TRUSS NAME C42	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:10.2



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	361	0	361	0	5-8	5-8
C	130	0	130	0	1-8	1-8
D	16	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	250	190/0	0/0	0/0	0/0	60/0	0/0
C	90	73/0	0/0	0/0	0/0	17/0	0/0
D	12	0/0	0/0	0/0	0/0	12/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS	
	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO	
E-B	-342/0	0.0	0.0	0.01 (4)	7.81	
A-B	0/28	-91.8	-91.8	0.13 (5)	10.00	
B-C	-19/0	-91.8	-91.8	0.22 (1)	6.25	
E-D	0/0	-18.5	-18.5	0.02 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 10 = 29 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.18")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.18")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TO=0.22/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WS=0.03/1.00 (n/a:0), SS=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

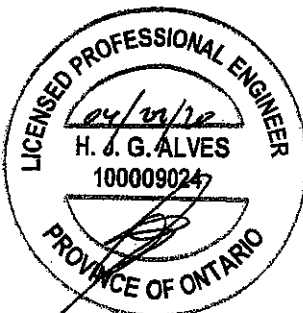
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1867 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

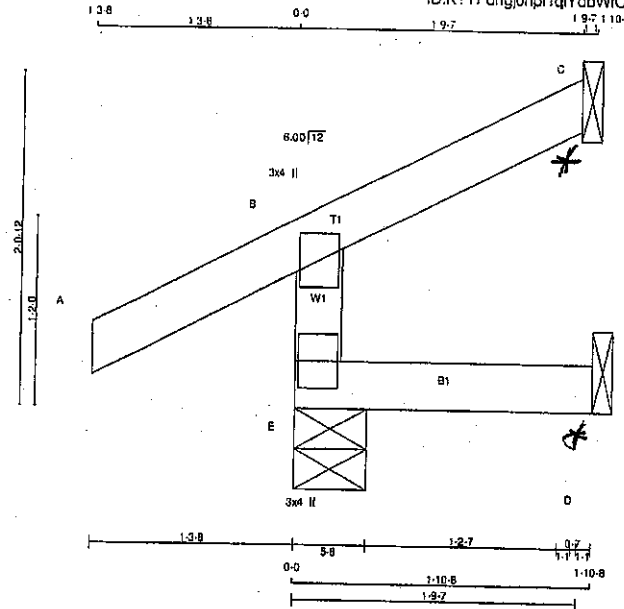
JSI GRIP = 0.14 (E) (INPUT = 0.90)
JSI METAL = 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007164

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	C43	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	
CHORDS	2x4	DRY	No.2
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPLT BRG	REORD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
E	271	0	271	0	5-8	5-8
C	45	0	45	0	1-8	1-8
D	8	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
E	188	141 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	31	24 / -18	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	7	0 / -8	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

FR-TO	CHORDS		FACTORED		WEBS	
	MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1 MAX (LBS)	MEMB.	FORCE (LBS)
E-B		-244 / 0	0.0	0.0 0.04 (5)		7.81
A-B		0 / 28	-91.8	-91.8 0.12 (1)		10.00
B-C		-17 / 0	-91.8	-91.8 0.08 (1)		6.25
E-D		0 / 0	-18.5	-18.5 0.04 (5)		10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 7 = 21 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5),
WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 788 1887 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.10 (E) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007165

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

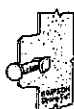
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



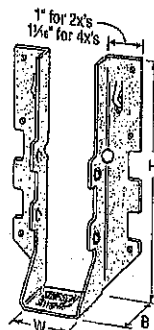
Double-Shear
Nailing
Top View



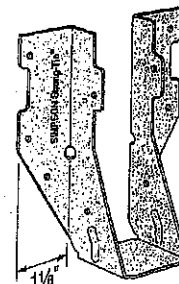
Double-Shear
Nailing
Side View;
Do not
bend tab



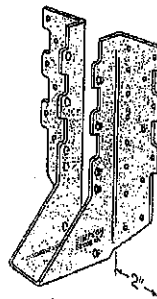
Dome Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,803,580



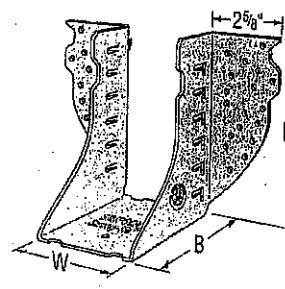
LUS28



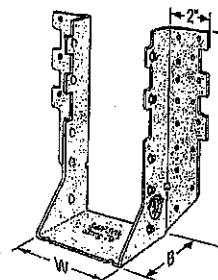
LU26L



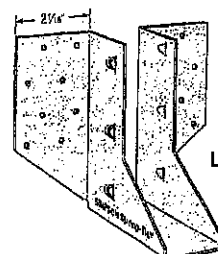
HUS210
(HUS26, HUS28,
and HHUS similar)



HGUS28-2

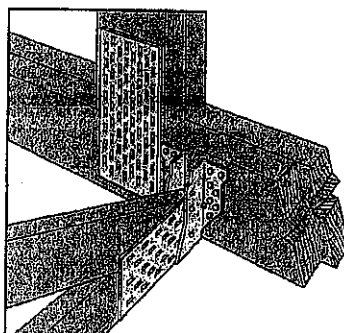


HHUS210-2



LJS26DS

Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS**SIMPSON**
Strong-Tie**HHUS/HGUS**

See Hanger Options information on pp. 125–127.

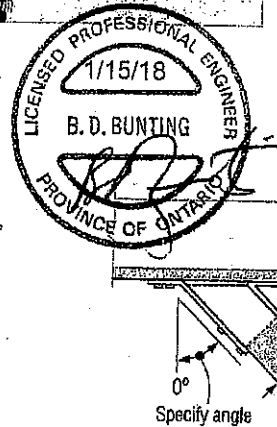
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.48 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger
Skewed Right
(Joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance				
		W	H	B	d _g ³	Header	Joist	D.Fir-L		S-P-F		
								Uplift	Normal	Uplift	Normal	
								(K _D = 1.15) lb.	(K _D = 1.00) lb.	(K _D = 1.15) lb.	(K _D = 1.00) lb.	
Single 2x Sizes												
■	LUS24	18	1 1/8	3 1/2	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155
									3.16	7.23	2.87	5.14
■	LU24L	22	1 1/8	3	1 1/4	2 1/4	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725
									1.60	4.54	1.42	3.22
■	LU26L	22	1 1/8	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"	720	1605	645	1140
									3.20	7.14	2.87	5.07
SS	LUS26	18	1 1/8	4 1/4	1 1/4	3 3/4	(4) 10d	(4) 10d	1420	2170	1290	1630
									6.32	9.65	5.74	7.25
■	HUS26	16	1 1/8	5 1/4	3	3 3/8	(14) 16d	(6) 16d	2705	4940	2065	3875
									11.90	21.97	9.20	17.24
■	LJS26DS	18	1 1/8	5	3 1/2	4 1/4	(16) 16d	(6) 16d	2055	4265	1460	4115
									9.14	18.97	6.49	18.31
■	HGUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 16d	(8) 16d	2685	6625	2685	5700
									11.96	29.51	11.96	25.35
■	LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1550
									5.07	9.72	4.54	6.89
SS	LUS28	18	1 1/8	6 1/4	1 1/4	3 3/4	(6) 10d	(4) 10d	1420	2520	1290	1790
									6.32	11.21	5.74	7.96
■	HUS28	16	1 1/8	7 1/4	3	6 1/8	(22) 18d	(8) 16d	3605	5365	2675	4345
									16.04	23.86	11.90	19.33
■	HGUS28	12	1 1/8	7 1/4	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3310	6900
									14.74	34.19	14.74	30.73
■	LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(6) 10d x 1 1/2"	1140	2495	1020	1770
									5.07	11.10	4.54	7.87
SS	LUS210	18	1 1/8	7 1/4	1 1/4	3 3/4	(8) 10d	(4) 10d	1420	2785	1290	2210
									6.32	12.39	5.74	9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_g is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSON and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

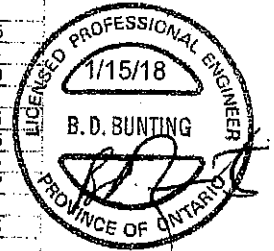
Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _c ¹	Header	Joist	D, Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _g = 1.15)	(K _g = 1.00)	(K _g = 1.15)	(K _g = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
LUS24-2	18	3%	3%	2	1½	(4) 16d	(2) 16d	835	2020	590	1435
SS LUS26-2	18	3%	4%	2	4	(4) 16d	(4) 16d	3.71	8.99	2.62	6.38
HHUS26-2	14	3%	5%	3	3½	(14) 16d	(8) 16d	1720	2595	1545	1920
HGUS26-2	12	3%	5%	4	4%	(20) 16d	(8) 16d	7.65	11.54	6.87	8.54
SS LUS28-2	18	3%	7	2	4	(6) 16d	(4) 16d	2850	7335	2065	5205
HHUS28-2	14	3%	7%	3	6%	(22) 16d	(8) 16d	12.68	32.63	9.20	23.15
HGUS28-2	12	3%	7%	4	6%	(36) 16d	(12) 16d	4385	8950	3110	6355
SS LUS210-2	18	3%	9	2	6	(8) 16d	(6) 16d	19.51	39.81	13.83	28.27
HHUS210-2	14	3%	9%	3	8	(30) 16d	(10) 16d	7.65	14.79	6.87	11.45
HGUS210-2	12	3%	9%	4	8%	(46) 16d	(16) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
								6070	12980	4310	9215
								27.00	57.74	19.17	40.99
								4670	9670	4235	7000
								20.77	43.02	18.84	31.14
								6840	14015	4855	10270
								30.43	62.34	21.60	45.69
Triple 2x Sizes											
HGUS26-3	12	4½%	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3110	6355
HGUS28-3	12	4½%	7½	4	6½	(36) 16d	(12) 16d	19.51	39.81	13.83	28.27
HHUS210-3	14	4½%	9	3	7½%	(30) 16d	(10) 16d	6070	12980	4310	9215
HGUS210-3	12	4½%	9½	4	8%	(46) 16d	(16) 16d	27.00	57.74	19.17	40.99
								4670	9670	4235	6865
								20.77	43.02	18.84	30.54
								6840	14645	4855	10400
								30.43	65.14	21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	6%	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3110	6355
HGUS28-4	12	6%	7½	4	6½	(36) 16d	(12) 16d	19.51	39.81	13.83	28.27
HHUS210-4	14	6%	8%	3	7½%	(30) 16d	(10) 16d	6070	12980	4310	9215
HGUS210-4	12	6%	9½	4	8½	(46) 16d	(16) 16d	27.00	57.74	19.17	40.99
								4670	10155	4235	7210
								20.77	45.17	18.84	32.07
								6840	14645	4855	10400
								30.43	65.14	21.60	46.26
HGUS212-4	12	6%	10%	4	10%	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6%	12%	4	11½	(66) 16d	(22) 16d	33.98	66.70	24.13	47.35
								10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
LUS46	18	3%	4%	2	3½	(4) 16d	(4) 16d	1720	2595	1545	1920
HHUS46	14	3%	5%	3	3½	(14) 16d	(8) 16d	7.65	11.54	6.87	8.54
HGUS46	12	3%	5%	4	4½	(20) 16d	(8) 16d	2540	7335	2065	5205
LUS48	18	3%	6%	2	3½	(6) 16d	(4) 16d	11.30	32.63	9.20	23.15
HHUS48	14	3%	7%	3	6%	(22) 16d	(8) 16d	4385	8950	3110	6355
HGUS48	12	3%	7%	4	6½	(36) 16d	(12) 16d	19.51	39.81	13.83	28.27
								1720	3325	1545	2575
								7.65	14.79	6.87	11.45
								3765	8940	2675	6345
								16.75	39.77	11.90	28.22
								6070	12980	4310	9215
								27.00	57.74	19.17	40.99
								2580	4500	2320	3195
								11.48	20.02	10.32	14.21
								6840	14015	4855	10270
								30.43	62.34	21.60	45.69
								7640	14995	5425	10645
								33.98	66.70	24.13	47.35
								10130	16400	7195	11645
								45.06	72.95	32.00	51.80



TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1/4". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

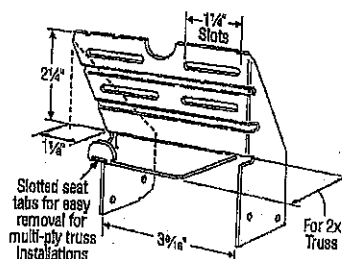
Design: Factored resistances are in accordance with CSA 086-14

Installation:

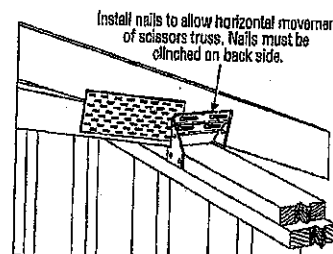
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

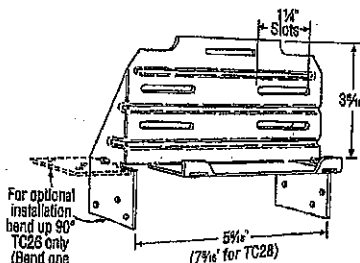
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



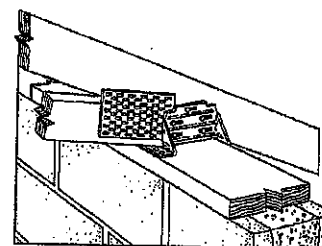
TC24
U.S. Patent 4,932,173



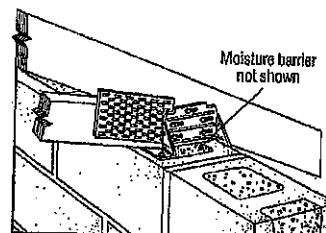
Typical TC24 Installation



TC26
(TC28 Similar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer (8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _u =1.15)	Uplift (K _u =1.15)
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _u =1.15)	Uplift (K _u =1.15)
TC26	(5) 10d	(6) 10d x 1 1/2"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.

2. Grout strength is 15 MPa minimum.

3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.

4. TC26 fastened to grouted concrete block with (6) - 9/16" x 2 1/4" Titen screws has a factored uplift resistance of 275 lb.



LIMITED
STATE
DESIGN

For more information, visit us online at www.strongtie.com.
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(800) 999-5099
strongtie.com

H/TSP

SIMPSON
Strong-Tie

Seismic and Hurricane Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Fasteners			Factored Resistance ($K_D = 1.15$)					
		To Rafters/Truss	To Plates	To Studs	D-Fir-L			S-P-F		
					Uplift	Lateral		Uplift	Lateral	
						F ₁	F ₂		F ₁	F ₂
					lb.	lb.	lb.	lb.	lb.	lb.
					kN	kN	kN	kN	kN	kN
H1	18	(5) 8d x 1½"	(4) 8d	—	740	685	300	980	485	215
					3.29	3.05	1.33	3.02	2.16	0.98
SS H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
					3.69	0.98	0.33	2.62	0.69	0.24
SS H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
					3.68	0.71	0.71	3.36	0.71	0.71
H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
					3.71	0.78	0.93	3.29	0.71	0.93
SS H3	18	(4) 8d	(4) 8d	—	740	180	265	815	125	190
					3.29	0.80	1.18	2.74	0.56	0.85
H6	16	—	(8) 8d	(8) 8d	1585	1085	—	1125	770	—
					7.05	4.83	—	5.00	3.43	—
H7Z	16	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
					6.18	2.99	—	4.40	2.11	—
SS H8 ³	18	(5) 10d x 1½"	(5) 10d x 1½"	—	1120	—	—	1025	—	—
					4.98	—	—	4.56	—	—
SS H10A ³	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1736	795	410	1505	565	290
					7.72	3.54	1.82	6.89	2.51	1.29
H10AR	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1485	690	430	1220	570	305
					6.61	3.07	1.91	5.43	2.54	1.36
H10A-2	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1835	1275	430	1645	880	305
					8.16	5.67	1.91	7.32	3.91	1.36
H10S ^{2,3}	18	(8) 8d x 1½"	(8) 8d x 1½"	(8) 8d	1465	795	315	1040	565	225
					6.52	3.54	1.40	4.63	2.51	1.00
H11Z	18	(6) 16d x 2½"	(6) 16d x 2½"	—	1095	920	545	780	655	390
					4.87	4.09	2.42	3.47	2.91	1.73
H14	18	1 (12) 8d x 1½"	(13) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
		2 (12) 8d x 1½"	(15) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
TSP	16	(9) 10d x 1½"	(6) 10d x 1½"	—	1295	440	—	920	310	—
					5.76	1.96	—	4.09	1.38	—
		(9) 10d x 1½"	(6) 10d	—	1560	440	—	1105	310	—
					6.94	1.96	—	4.92	1.38	—

- Factored resistances have been increased 15% for short term loading; no further increase is allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
- H8 factored uplift resistances for stud-to-bottom plate installations are 595 lb. (2.65 kN) for D-Fir-L and 390 lb. (1.74 kN) for S-P-F.
- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-HTIECONPATH).
- Factored resistances in the F₁ direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.

- H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.38 kN) D-Fir-L and 1015 lb. (4.61 kN) S-P-F.
- H10S nails to plates are optional for uplift but required for lateral loads.
- H10A may be field-bent up to a slope of 8/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
- The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SSNAILS at strongtie.com.
- D-Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1½" nails into the top plates and (3) 8d x 1½" nails into the lowest three flange holes into the truss bottom chord is 495 lb. (2.20 kN).
- Nails: 16d x 2½" = 0.162" dia. x 2½" long, 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long, 8d x 1½" = 0.131" dia. x 1½" long. See pp. 27-28 for other nail sizes and information.

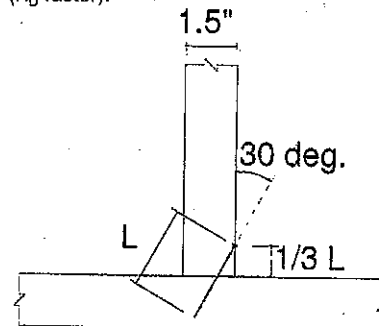
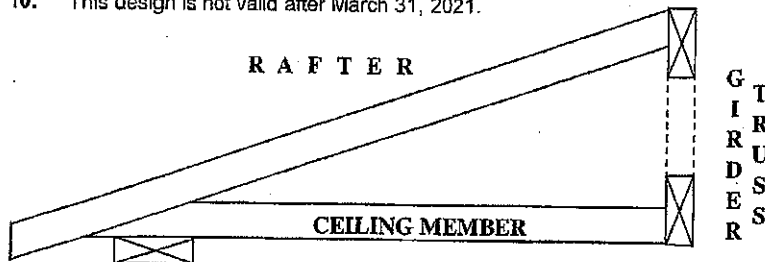
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.



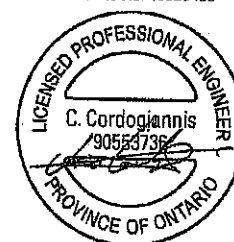
TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

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100 Industrial Rd.
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December 2, 2019

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BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

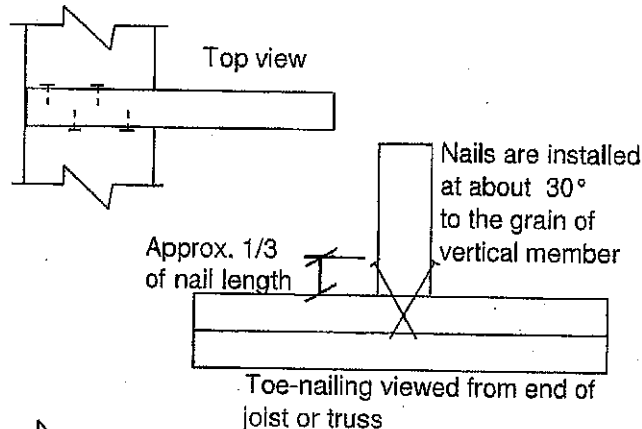
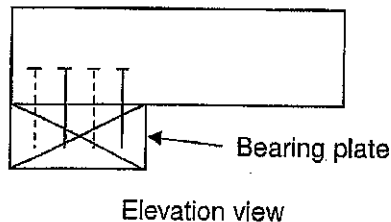
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

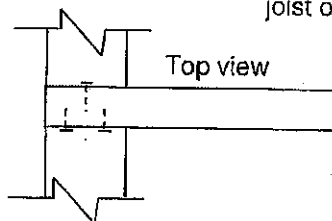
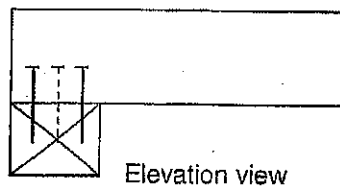
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind or earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



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Bradford, Ontario L3Z 3G7

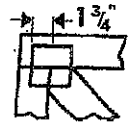
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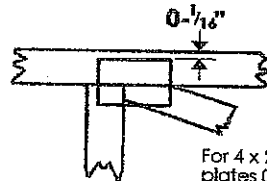
December 2, 2019

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint, unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/4\" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

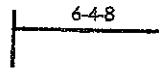


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

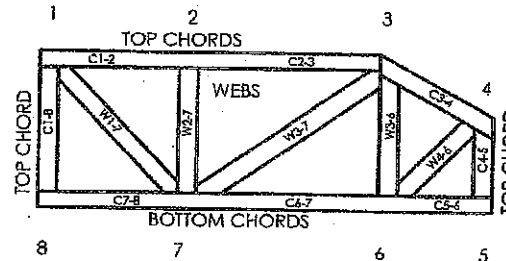
Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
- DSB-89: Design Standard for Bracing.
- BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MII-7473C rev. 10-08



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

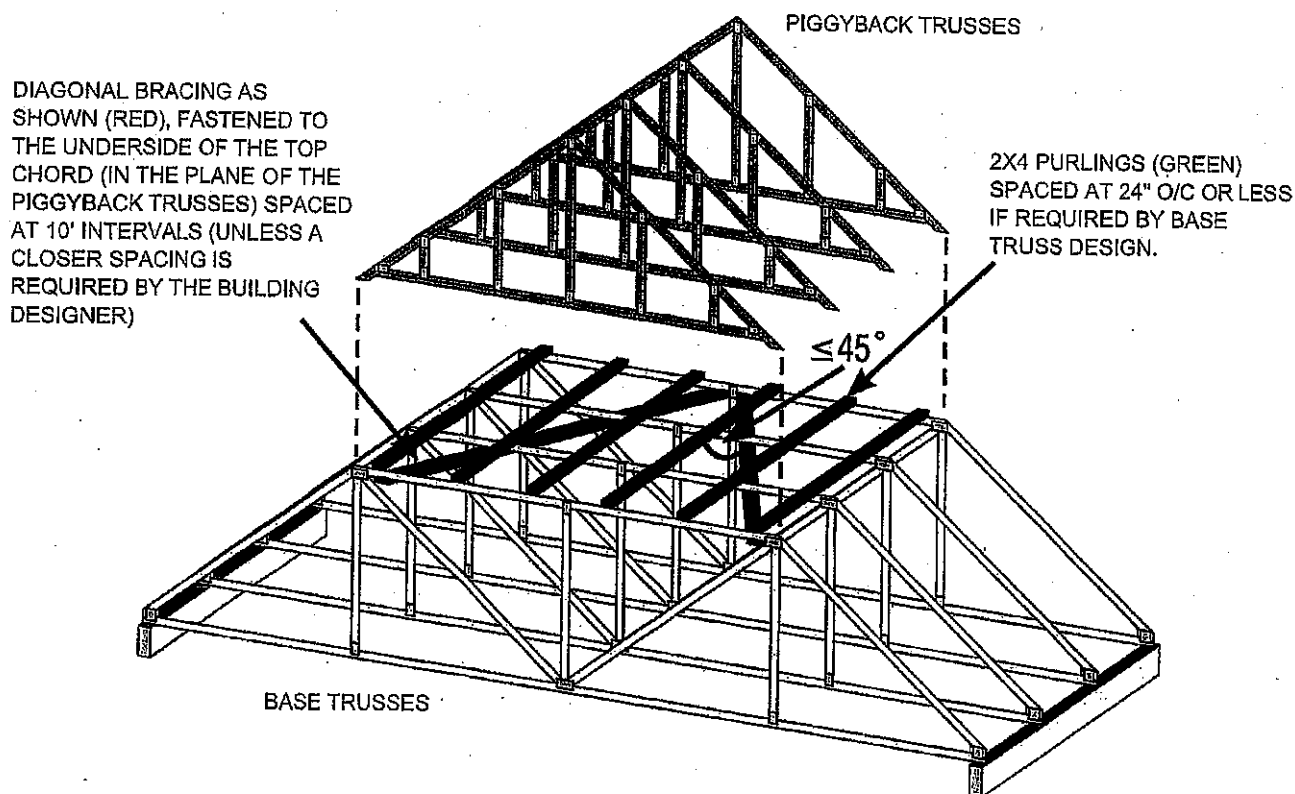
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

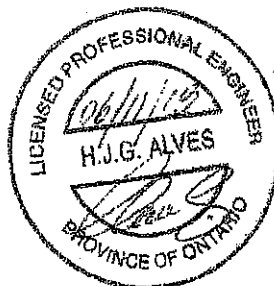


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800215

Feb 09, 2018