

	•	Products				Connector Summary			
PlotID	Length	Product	Plies	Net Qty	Fab Type	Qty	Manuf	Product	
J1	18-00-00	9 1/2" NI-40x	1	6	MFD	3	H1	IUS2.56/9.5	
J2	16-00-00	9 1/2" NI-40x	1	48	MFD	8	H1	IUS2.56/9.5	
J3	14-00-00	9 1/2" NI-40x	1	16	MFD	1	H3	HUS1.81/10	
J4	12-00-00	9 1/2" NI-40x	1	3	MFD	1	H9	L90	
J5	10-00-00	9 1/2" NI-40x	1	7	MFD	,			
J6	4-00-00	9 1/2" NI-40x	1	3	MFD	-			
/B12	20-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	MFD		C	ITY OF HAMILTON	
B9 DR	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	MFD			Building Division	
₆ ∕87	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1	MFD		Permit No.	21-102417	
B11	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	MFD			ED DRAWINGS SHALL BE AVAILABLE ON SIT	
1B13	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1	MFD		THE OWNER	ND//OR CONTRACTOR CULL COLUMN	
/ B14	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	MFD	•	THE ONTARIO BU	ILDING CODE AND ALL OTHER APPLICABLE	



FROM PLAN DATED: MAR 2020

BUILDER: GREENPARK HOMES

SITE: RUSSELL GARDENS PH 3

MODEL: VALLEYCREEK 2S

ELEVATION: 3

LOT:

CITY: WATERDOWN

SALESMAN: MARIO DICIANO

DESIGNER: AJ **REVISION:** Ibv

NOTES:

REFER TO THE NORDIC INSTALLATION **GUIDE FOR PROPER STORAGE AND** INSTALLATION. **SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE **SQUASH BLOCKS** REQ'D UNDER CONCENTRATED LOADS, SEE FIGURE 1. **CANTILEVERED JOISTS INCLUDING CANT'** OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6

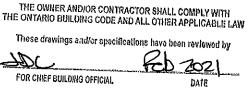
LOADING:

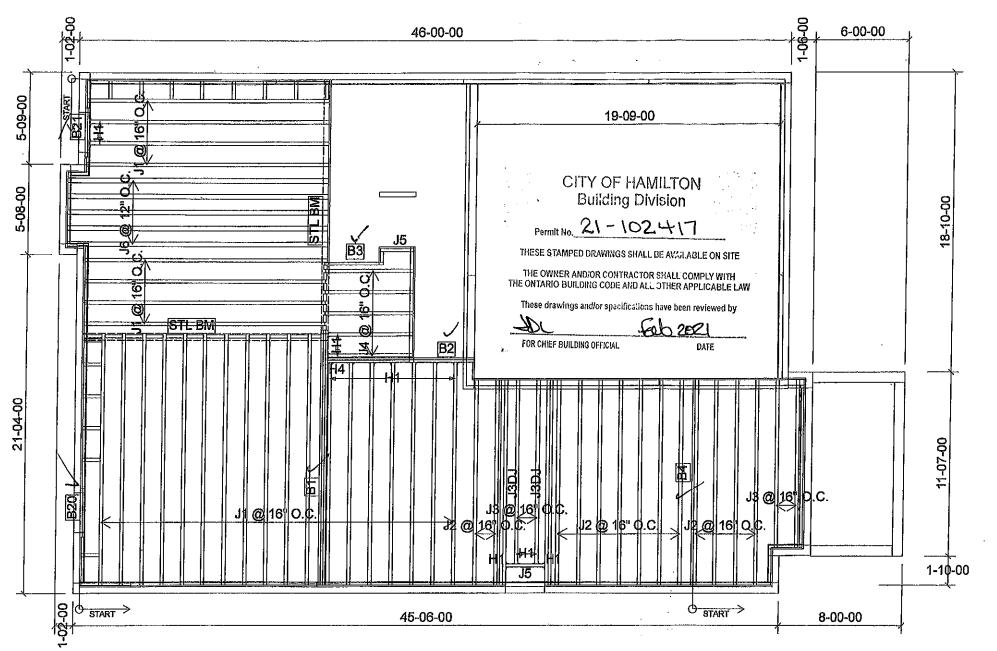
DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 20.0 lb/ft ²

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 2020-03-25

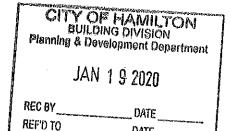
2nd FLOOR

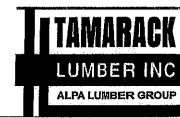




1		Products			•
PlotID	Length	Product	Plies	Net Qty	Fab Type
J1	16-00-00	9 1/2" NI-40x	1	26	MFD
J2	14-00-00	9 1/2" NJ-40x	1	13	MFD
J3DJ	14-00-00	9 1/2" NI-40x	2	4	MFD
J3	12-00-00	9 1/2" NI-40x	1	4	MFD
J4	6-00-00	9 1/2" NI-40x	1	5	MFD
J5	4-00-00	9 1/2" NI-40x	1	2	MFD
J6	18-00-00	9 1/2" NI-80	1	5	MFD
∕B1	18-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3	MFD
√B4	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1	MFD
√B2	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	MFD
∕B3	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1	MFD
B20	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	MFD
∕B21	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	MFD /

C	Connector	Connector Summary								
Qty	Manuf	Product								
9	H1	IUS2.56/9.5								
2	H1	IUS2.56/9.5								
2	H1	IUS2.56/9.5								
2	H1	IUS2.56/9.5								
1	H4	HGUS410								





FROM PLAN DATED: MAR 2020

BUILDER: GREENPARK HOMES

SITE: RUSSELL GARDENS PH 3

MODEL: VALLEYCREEK 2S

ELEVATION: 3

LOT: 95

CITY: WATERDOWN

SALESMAN: MARIO DICIANO

DESIGNER: AJ **REVISION:** Ibv

NOTES:

REFER TO THE **NORDIC INSTALLATION**GUIDE FOR PROPER STORAGE AND
INSTALLATION.

SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ.

IJOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. CERAMIC TILE

APPLICATION AS PER O.B.C 9.30.6.

LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 20.0 lb/ft²

SUBFLOOR: 3/4" GLUED AND NAILED

DATE: 2020-03-25

1st FLOOR

DECK CONDITION

NORDIC **STRUCTURES**

COMPANY

PROJÉCT

Mar. 25, 2020 10:00 J7 1ST FLOOR.wwb

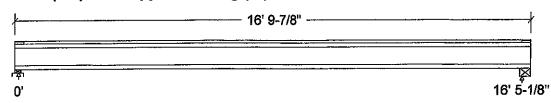
Design Check Calculation Sheet

Nordic Sizer - Canada 7.2

Loads:

Load	Туре	Distribution	Pat-	Location	[ft]	Magnitud	de	Unit
	"-		tern	Start	End	Start	End	
Load1	Dead	Full Area				20.00		psf
Load2	Live	Full Area				40.00		psf

Maximum Reactions (lbs) and Support Bearing (in):



Unfactored: Dead Live	164 329		164 329
Factored: Total	698	CITY OF HAMILTON BUILDING DIVISION	698
Bearing: Capacity Joist Support Des ratio	1893 5573	Planning & Development Department FEB 2 6 2021	1893
Joist Support Load case Length Min reg'd	0.37 0.13 #2 2-3/8 1-3/4	REC BY DATE DATE	0.37 - #2 4-1/8 1-3/4
Stiffener KD	No 1.00		No 1.00
KB support fcp sup Kzcp sup	1.00 769 1.09		_

Nordic 9-1/2" NI-80 Floor joist @ 12" o.c.

Supports: 1 - Lumber Sill plate, No.1/No.2; 2 - Steel Beam, W; Total length: 16' 9-7/8"; Clear span: 16' 3-3/8"; 3/4" nailed and glued OSB sheathing This section PASSES the design code check.



DWG NO. TAM 55/2 -20. STRUCTURAL COMPONENT ONLY

WoodWorks® Sizer

for NORDIC STRUCTURES

J7 1ST FLOOR.wwb

Nordic Sizer - Canada 7.2

Page 2

Limit States Design using CSA-O86-09 and Vibration Criterion:

Criterion	Analysis Value	Design Value	Unit	Analysis/Design
Shear	Vf = 698	Vr = 1895	lbs	Vf/Vr = 0.37
Moment(+)	Mf = 2867	Mr = 8958	lbs-ft	Mf/Mr = 0.32
Perm. Defl'n	0.10 = < L/999	0.55 = L/360	in	0.18
Live Defl'n	0.20 = L/981	0.41 = L/480	in	0.49
Total Defl'n	0.30 = L/654	0.82 = L/240	in	0.37
Bare Defl'n	0.23 = L/862	0.55 = L/360	in	0.42
Vibration	Lmax = 16'-5.1	Lv = 18'-4.9	ft	0.89
Defl'n	= 0.028	= 0.039	in	0.71

Additional Data:

, idditions									11
FACTORS:	f/E	KD	KH	KZ	KL	KT	KS	KN	LC#
Vr	1895	1.00	1.00	· -	-	-	-	_	#2
Mr+			1.00	_	1.000	-	-	-	#2
ET.	224 1 m			_	_	_	_	_	#2

CRITICAL LOAD COMBINATIONS:

		= 1.25D + 1.5L
Moment(+):	LC #2	= 1.25D + 1.5L
Deflection:	LC #1	= 1.0D (permanent)
	LC #2	= 1.0D + 1.0L (live)
	LC #2	= 1.0D + 1.0L (total)
	T C #2	-1.00 ± 1.01 . (hare join

LC #2 = 1.0D + 1.0L (bare joist) Bearing : Support 1 - LC #2 = 1.25D + 1.5L

Support 2 - LC #2 = 1.25D + 1.5L

Load Types: D=dead W=wind S=snow H=earth,groundwater E=earthquake L=live(use,occupancy) Ls=live(storage,equipment) f=fire

Load Patterns: s=S/2 L=L+Ls _=no pattern load in this span

All Load Combinations (LCs) are listed in the Analysis output

CALCULATIONS:

Eleff = 375.38 lb-in² K= 4.94e06 lbs

"Live" deflection is due to all non-dead loads (live, wind, snow...)

CONFORMS TO OBC 2012

AMENDED 2020

Design Notes:

- 1. WoodWorks analysis and design are in accordance with the 2010 National Building Code of Canada (NBC), Division B, Part 4, and the CSA O86-09 Engineering Design in Wood standard, which includes Update No.1
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to Nordic Structures technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.



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STRUCTURAL COM. ONENT COM.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			Ba	are		1/2" Gypsum Celling				
Depth	Series		On Centr	re Spacing			On Cent	re Spacing		
Oupen		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"	
	NI-40x	17'-0"	16'-0"	15'-1"	13'-11"	17'-5"	16'-1"	15'-1"	13'-11"	
9-1/2"	NI-60	1 7 '-2"	16'-2"	15'-5"	14'-3"	17'-6"	16'-5"	15'-5"	14'-3"	
J =1-	NI-70	18'-0"	16'-11"	16'-3"	15'-6"	18'-5"	17'-3"	16'-7"	15'-6"	
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	15'-10"	
	Nt-20	17'-10"	16'-10"	16'-0"	14'-10"	18'-6"	17'-1"	16'-0"	14'-10"	
	NI-40x	19'-4"	17'-11"	17'-3"	15'-10"	19'-11"	18'-6"	17'-9"	15'-10"	
	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-1"	
11-7/8"	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"	
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"	
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"	
	N1-40x	21'-5"	19'-10"	18'-11"	17'-5"	22'-1"	20'-6"	19'-6"	17'-5"	
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"	
14"	N1-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21 -11	20'-10"	19'-9"	
	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"	
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"	
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22"-9"	21'-8"	20'-6"	
	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22'-9"	21'-6"	
16"	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"	
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"	

			Mid-Spar	n Blocking	Mid-Span Blocking and 1/2" Gypsum Celling						
Depth	Series		On Centr	e Spacing		On Centre Spacing					
осран	551165	12"	16"	19.2"	24"	12"	16"	19.2"	24"		
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"		
	NI-40x	17'-9"	16'-1"	15'-1"	13'-11"	17'-9"	16'-1"	15 '-1 "	13'-11'		
9-1/2"	NI-60	18'-1"	16'-5"	15'-5"	14'-3"	18'-1"	16'-5"	15 '-5 "	14'-3"		
J-3;2	NI-70	19'-10"	17'-11"	16'-9"	15'-6"	19'-10"	17'-11"	16'-9"	15'-6"		
	NI-80	20'-2"	18'-3"	17'-1"	15'-10"	20'-2"	18'-3"	17'-1"	15'-10'		
	NI-20	18'-10"	17'-1"	16'-0"	14'-10"	18'-10"	17'-1"	16'-0"	14'-10'		
	NI-40x	21'-3"	19'-3"	17'-9"	15'-10"	21'-3"	19'-3"	17'-9"	15'-10'		
	NI-60	21'-9"	19'-8"	18'-5"	17'-1"	21'-9"	19'-8"	18'-5"	17'-1"		
11-7/8"	NI-70	23'-4"	21'-5"	20'-1"	18'-6"	23'-8"	21'-5"	20'-1"	18'-6"		
	NI-70	23'-7"	21'-10"	20'-5"	18'-11"	24'-1"	21'-10"	20'-5"	18'-11'		
	NI-90x	24'-3"	22'-6"	21'-3"	19'-7"	24'-8"	22'-7"	21'-3"	19'-7"		
	NI-40x	24'-2"	21'-5"	19'-6"	17'-5"	24'-2"	21'-5"	19'-6"	17'-5"		
	NI-60	24'-9"	22'-5"	21'-0"	19'-6"	24'-9"	22'-5"	21'-0"	19'-6"		
14"	NI-70	26'-1"	24'-3"	22'-9"	21'-0"	26'-8"	24'-3"	22'-9"	21'-0"		
14	Nf-80	26'-6"	24'-7"	23'-3"	21'-6"	27'-1"	24'-10"	23'-3"	21'-6"		
	NI-90x	27'-3"	25'-4"	24'-1"	22'-4"	27'-9"	25'-10"	24'-3"	22'-4"		
	NI-60	27'-3"	24'-11"	23'-5"	21'-7"	27'-6"	24'-11"	23'-5"	21'-7"		
	N1-70	28'-8"	26'-8"	25'-3"	23'-4"	29'-3"	26'-11"	25'-3"	23'-4"		
16"	NI-80	29'-1"	27'-0"	25'-9"	23'-10"	29'-8"	27'-6"	25'-10"	23'-10"		
	NI-90x	29'-11"	27'-10"	26'-6"	24'-10"	30'-6"	28'-5"	26'-11"	24'-10"		

^{1.} Maximum dear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nalled oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







	Serles		Ва	are			1/2" Gypsum Ceiling On Centre Spacing				
Depth			On Centr	e Spacing							
ocpu.	*	12"	16"	19.2"	24"	12"	16"	19.2"	24"		
	NI-20	15'-1"	14'-2"	13'-9"	N/A	15'-7"	14'-8"	14'-2"	N/A		
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15 '-1 "	N/A		
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A		
J -/-	NI-70	17'-1"	16 -1"	15'-6"	N/A	17'-5"	16'-5" ,	15'-10"	N/A		
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A		
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A		
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A		
	MI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A		
11-7/8"	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A		
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A		
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A		
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A		
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A		
14"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A		
17	N1-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A		
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A		
	N1-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A		
	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A		
16"	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A		
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A		

			Mid-Spar	n Blocking	Mid-Span Blocking and 1/2" Gypsum Ceiling					
Depth	Series		On Centi	re Spacing		On Centre Spacing				
րեիա	361163	12"	16"	19.2"	24"	12"	16 ¹¹	19.2"	24"	
	NI-20	16'-8"	15'-3"	14'-5"	N/A	16'-8"	15'- 3 "	14'-5"	N/A	
	NI-40x	17'-11"	· 16'-11"	16'-1"	N/A	18'-5"	17'-1"	16'-1"	N/A	
9-1/2"	N1-60	18'-2"	17'-1"	16'-4"	N/A	18'-7"	17'-4"	16'-4"	N/A	
3-1/2	NI-70	19'-2"	17'-10"	17'-2"	N/A	19'-7"	18'-3"	17'-7"	N/A	
	NI-80	19'-5"	18'-0"	17'-4"	N/A	19'-10"	18'-5"	17'-8"	N/A	
	NI-20	19'-6"	18'-1"	17'-3"	N/A	19'-11"	18'-3"	17'-3"	N/A	
	NI-40x	21'-0"	19'-6"	18'-8"	N/A	21'-7"	20'-2"	19'-2"	N/A	
11-7/8"	NI-60	21'-4"	19'-9"	18'-11"	N/A	21'-11"	20'-4"	19'-6"	N/A	
		22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-5"	20'-5"	N/A	
•	NI-70	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-8"	N/A	
	NI-80	22 -9 23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A	
	NI-90x		21'-11"	20'-11"	N/A	24'-3"	22'-7"	21'-7"	N/A	
	N1-40x	23'-7"	21-11	21'-3"	N/A	24'-8"	22'-11"	21'-11"	N/A	
	N1-60	24'-0"				25'-10"	24'-0"	22'-11"	N/A	
14°	NI-70	25'-3"	23'-4"	22'-3"	N/A	26'-2"	24'-4"	23'-2"		
	NI-80	25'-7"	23'-8"	22'-7"	N/A	1			N/A	
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A	
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	25'-3"	24'-2"	N/A	
	NI-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A	
16"	NI-80	28'-2"	26'- 1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A	
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26'-2"	N/A	

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design five load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nalled oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{4.} Bearing summers are not required what is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			Ba	are			1/2" Gyp:	sum Ceiling	
Depth	Series	On Centre Spacing			On Centre Spacing				
Depui		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"
	NI-40x	17'-0"	16'-0"	15'-5"	14'-9"	17'-5"	16'-5"	15'-10"	15'-2"
9-1/2"	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-6"	16'-7"	15'-11"	15'-3"
J 1/4	NI-70	18'-0"	16'-11"	16'-3"	15'-7"	18'-5"	17'-3"	16'-7"	15'-11"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
	NI-20	17'-10"	16'-10"	16'-2"	15'-6"	18'-6"	17'-4"	16'-9"	16'-1"
	NI-40x	19'-4"	17'-11"	17'-3"	16'-6"	19'-11"	18'-6"	17'-9"	17'-0"
	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"
11-7/8"	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"
	N1-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
	NI-40x	21'-5"	19'-10"	18'-11"	17'-11"	22'-1"	20'-6"	19'-7"	18'-7"
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"
**	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"
	N1-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22'-9"	21'-6"
16"	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	Nt-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

			Mid-Spar	Blocking		Mid-S	pan Blocking an	d 1/2" Gypsum	Celling
Depth	Series	On Centre Spacing			On Centre Spacing				
Dehm		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
	NI-40x	18'-8"	17'-2"	16'-3"	15' - 2"	18'-10"	17'-2"	16'-3"	15'-2"
9-1/2"	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"
	NI-70	20'-0"	18'-7"	17'-9"	16'-7"	20'-5"	18'- 1 1"	17'-10"	16'-7"
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10"
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"
	NI-40x	21'-10"	20'-4"	19'-4"	17'-8"	22'-5"	20'-6"	19'-4"	17'-8"
	NI-60	22'-1"	20'-7"	19'-7"	18 ¹ -4"	22'-8"	20'-10"	19'-8"	18'-4"
11-7/8"	NI-70	23'-4"	21'-8"	20'-8"	19'-7"	23'-10"	22'-3"	21'-2"	19'-9"
	NI-80	23'-7"	21'-11"	20'-11"	19'-9"	24'-1"	22'-6"	21'-5"	20'-0"
	N1-90x	24'-3"	22'-6"	21'-6"	20'-4"	24'-8"	23'-0"	22'-0"	20'-9"
	NI-40x	24'-5"	22'-9"	21'-8"	19'-5"	25'-1"	23'-2"	21'-9"	19'-5"
	NI-60	24'-10"	23'-1"	22'-0"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10"
14"	NI-70	26'-1"	24'-3"	23'-2"	21'-10"	26'-8"	24'-11"	23'-9"	22'-4"
17	NI-80	26'-6"	24'-7"	23'-5"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"
	NI-90x	. 27'-3"	25'-4"	24'-1"	22'-9"	27'-9"	25'-11"	24'-8"	23'-4"
	NI-60	27'-3"	25'-5"	24'-2"	22'-10"	28'-0"	26'-2"	24'-9"	23'-1"
	NI-70	281-8"	26'-8"	25'-4"	23'-11"	29'-3"	27'-4"	26'-1"	24'-8"
16"	NI-80	29'-1"	27'-0"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"
	NI-90x	29'-11"	27'-10"	26'-6"	25'-0"	30'-6"	28'-5"	27'-2"	25'-8"

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 Inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 Inches for the end bearings.
4. Bearing stiffeners are not required when I-Joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







			B:	are		1	1/2" Gyp:	sum Celling	
9-1/2" 11-7/8"	Şeries	On Centre Spacing				On Centre Spacing			
	4	12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-1"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
11-7/8"	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'- 6 "	16'-11"	N/A
	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
	N1-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A
14"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
	Nt-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
16"	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A
•	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

			Mid-Spar	n Blocking	Mid-Span Blocking and 1/2" Gypsum Ceiling				
Depth	Serles	On Centre Spacing			On Centre Spacing				
Deptii	501120	12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A
	NI-40x	17'-9"	16'-1"	15'-1"	N/A	17'-9"	16'-1"	15'-1"	N/A
9-1/2"	NI-60	18'-1"	16'-4"	15'-4"	N/A	18'-1"	16'-4"	15'-4"	N/A
3-1/2	NI-70	19'-2"	17'-10"	16'-9"	N/A	19'-7"	17'-10"	16'-9"	N/A
	NI-80	19'-5"	18'-0"	17'-1"	N/A	19'-10"	18'-3"	17'-1"	N/A
	NI-20	18'-9"	17'-0"	16'-0"	N/A	18'-9"	17'-0"	16'-0"	N/A
	NI-40x	21'-0"	19'-3"	17'-9"	N/A	21'-3"	19'-3"	17'-9"	N/A
	NI-60	21'-4"	19'-8"	18'-5"	N/A	21'-8"	19'-8"	18'-5"	N/A
11-7/8"	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-4"	20'-0"	N/A
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-5"	N/A
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A
	NI-40x	23'-7"	21'-5"	19'-6"	N/A	24'-1"	21'-5"	19'-6"	N/A
	NI-60	24'-0"	22'-3"	21'-0"	N/A	24'-8"	22'-5"	21'-0"	N/A
a All	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-9"	N/A
14"	NI-70	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	24'-10"	23'-4"	N/A
	NI-70	20-5 27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A
16"		27-3 28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A
	NI-80	20 -2 29'-0".	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26'-2"	N/A
	NI-90x	Z3-U ,	20-10	40 /					

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

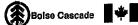
^{2.} Spans are based on a composite floor with glued-nalled oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

3. Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{5.} Minimum gearing rength shall be 1-374 inches for the classic state of the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic 1-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.





PASSED

2ND FLR FRAMING\Dropped Beams\B9 DR(i2601) (Dropped Beam)

BC CALC® Member Report

Dry | 1 span | No cant.

March 25, 2020 09:20:17

Build 7239

Job name:

Address:

City, Province, Postal Code: WATERDOWN

File name:

VALLEYCREEK 2S EL 1.mmdl

2ND FLR FRAMING\Dropped Beams\B9 DR(i2601) Description:

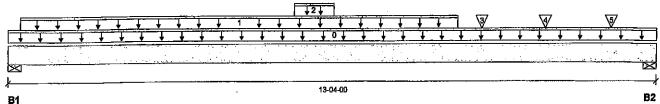
Specifier:

Designer: ΑJ

Customer: Code reports:

CCMC 12472-R

Company:



Total Horizontal Product Length = 13-04-00

Snow

Reaction Summary (Down / Uplift) (Ibs)

Bearing	Live	Dead
B1, 5"	2160 / 0	1147 / 0
B2 5"	2134 / 0	1132 / 0

LO	ad Summary										
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	13-04-00	Top		10	•		-00
1	Smoothed Load	Unf. Lin. (lb/ft)	L	00-03-00	09-03-00	Тор	307	154			
2	Bk1(i2586)	Unf. Lin. (lb/ft)	L	05-10-04	06-07-12	Top	351	175	_		
3	J2(12400)	Conc. Pt. (lbs)	L	09-09-00	09-09-00	Top	359	179	PRO	ESSIO	AL THE
4	J2(i2446)	Conc. Pt. (lbs)	L	11-01-00	11-01-00	Top	410	205	O OFFICE		~ X
5	J2(i2446)	Conc. Pt. (lbs)	L	12-05-00	12-05-00	Top	410	205	ST 8 2	16	
_	4 1 0		Factored	Dem		_	• 4	NO.	O I/V	reall A	
Co	ntrols Summary	Factored Demand	Resistance	Resi	stance	Case	Location	13	5. KA	rsoula	JNU O

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	15541 ft-lbs	23220 ft-lbs	66.9%	1	06-07-12
End Shear	4281 lbs	11571 lbs	37.0%	1	01-02-08
Total Load Deflection	L/246 (0.616")	n\a	97.6%	4	06-07-12
Live Load Deflection	L/376 (0.403")	h\a	95.8%	5	06-07-12
Max Defl.	0.616"	n\a	n\a	4	06-07-12
Span / Depth	15.9				

Bear	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	5" x 3-1/2"	4674 lbs	20.0%	21.9%	Spruce-Pine-Fir
B2	Wall/Plate	5" x 3-1/2"	4616 lbs	19.8%	21.6%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 00-02-13, Bottom: 00-02-13.

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

PROVIDE ROWS OF 31/4" ARBOX SPIRAL NAILS @ 8 "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN.2"LUMBER EDGE/END DISTANCE, DO NOT USE AIR HAILS



POLINCE OF ONTH

Disclosure

Snow

Wind

Tributary

00-00-00 n\a n\a n\a n\a

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate CONFORMS 18 OBC 2012 expert to assure its adequacy, prior to evidence of sultability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.



PASSED

2ND FLR FRAMING\Flush Beams\B11(i2558) (Flush Beam)

Dry | 1 span | No cant.

March 25, 2020 09:20:17

Build 7239 Job name:

Address:

City, Province, Postal Code: WATERDOWN

BC CALC® Member Report

Customer: Code reports:

CCMC 12472-R

File name:

VALLEYCREEK 2S EL 1.mmdi

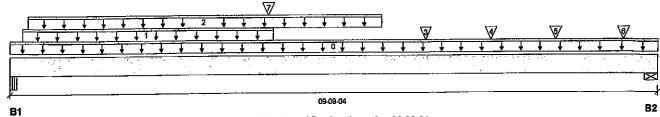
2ND FLR FRAMING\Flush Beams\B11(i2558) Description:

Wind

Specifier:

Designer: ΑJ

Company:



Total Horizontal Product Length = 09-09-04

Snow

Reaction Summary (Down / Uplift) (Ibs)

Bearing	Live	Dead
B1, 4-1/2"	1403 / 0	752 / 0
B2, 5-1/2"	1451 / 0	775 / 0

Los	d Summary	•					Live	
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	09-09-04	Тор		
1	FC3 Floor Material	Unf. Lin. (ib/ft)	Ł	00-02-04	03-11-10	Тор	14	
2	Smoothed Load	Unf. Lin. (lb/ft)	Ł	00-03-04	05-07-04	Тор	282	
3	J2(j2113)	Conc. Pt. (lbs)	L	06-03-04	06-03-04	Тор	335	
4	J2(i2121)	Conc. Pt. (lbs)	Ĺ	07-03-04	07-03-04	Тор	287	
5	J2(i2121)	Conc. Pt. (lbs)	L	08-03-04	08-03-04	Тор	287	
6	J2(12120)	Conc. Pt. (lbs)	L	09-03-04	09-03-04	Тор	260	
7	B13(i2593)	Conc. Pt. (lbs)	L	03-10-12	03-10-12	Тор	122	

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	6956 ft-lbs	23220 ft-lbs	30.0%	1	04-11-04
End Shear	2771 lbs	11571 lbs	23.9%	1	01-02-00
Total Load Deflection	L/757 (0.144")	n\a	31.7%	4	04-09-13
Live Load Deflection	L/999 (0.094")	n\a	n\a	5	04-09-13
Max Defl.	0.144"	n\a	n\a	4	04-09-13
Spen / Denth	11.4				

Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Beam	4-1/2" x 3-1/2"	3044 lbs	31.4%	15.8%	Spruce-Pine-Fir
B2	Wall/Plate	5-1/2" x 3-1/2"	3146 lbs	26.6%	13.4%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

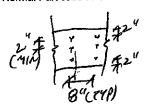
Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86. AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



PROVIDE 3 ROWS OF 31/2" ARDOX SPIRAL NAILS @ 8 "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN.2"LUMBER EDGE/END DISTANCE DO NOT USE AIR NAILS



STRUCTURAL DISCIOSURE ONLY

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of sultability for a particular application. The output here is based on CONFORMS TO OBC 2012building code-accepted design. properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain installation Guide or ask questions, please call (800)232-0788

> BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

before installation.



Passed

2ND FLR FRAMING\Flush Beams\B12(i2612) (Flush Beam)

Dry [2 spans] R cant.

March 25, 2020 09:20:17

BC CALC® Member Report **Build 7239**

Job name:

Customer:

Address:

Code reports:

City, Province, Postal Code: WATERDOWN

File name:

VALLEYCREEK 2S EL 1.mmdl

2ND FLR FRAMING\Flush Beams\B12(i2612) Description:

Specifier:

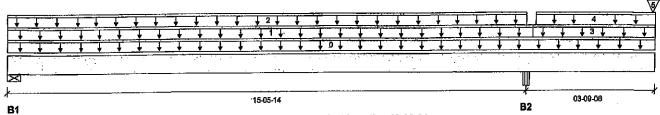
Designer:

CCMC 12472-R

Company:

ΑJ

Wind



Total Horizontal Product Length = 19-03-04

Snow

Reaction	Summary (Down	/ Uplift)	(lbs)
B = = = loo ==	Livo		Dogri

Bearing	Live	Dead
B1, 4-3/8"	201 / 98	120 / 0
B2 4-1/2"	924 / 0	589 / 0

Loa	ad Summary						LIVE
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00
0	Self-Weight	Unf. Lin. (lb/ft)	Ĺ	00-00-00	19-03-04	Top	
1	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	15-08-02	Тор	5
2	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	15-05-14	Top	20
3	FC3 Floor Material	Unf. Lin. (lb/ft)	L.	15-08-02	19-03-04	Тор	23
4	STAIR	Unf. Lin. (lb/ft)	L	15-09-04	19-03-04	Top	120
5	B13(i2593)	Conc. Pt. (lbs)	L	19-02-06	19-02-06	Тор	128

Controls Summary	Factored Demand	Factored Resistance	Demand <i>i</i> Resistance	Case	Location
Pos. Moment	1408 ft-lbs	23220 ft-lbs	6.1%	2	06-09-08
Neg. Moment	-3307 ft-lbs	-23220 ft-lbs	14.2%	1	15-05-14
End Shear	375 lbs	11571 lbs	3.2%	2	01-01-14
Cont. Shear	1349 lbs	11571 lbs	11.7%	1	16-05-10
Total Load Deflection	2xL/627 (0.145")	n\a	38.3%	10	19-03-04
Live Load Deflection	2xL/1998 (0.119")	n\a	n\a	13	19-03-04
Total Neg. Defl.	L/999 (-0.069")	n\a	n\a	10	09-11-01
Max Defl.	0.072"	n\a	n\a	9	07-03-12
Span / Depth	19.2				

Beari	ng Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	4-3/8" x 3-1/2"	451 lbs	4.8%	2.4%	Spruce-Plne-Fir
B2	Beam	4-1/2" x 3-1/2"	2122 lbs	21.9%	11.0%	Spruce-Pine-Fir

Notes

Design meets User specified (2xL/240) Total load deflection criteria.

Design meets User specified (2xL/360) Live load deflection criteria.

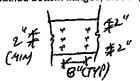
Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86. AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

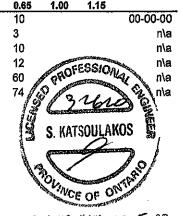
Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Cantilevers require sheathed bottom flanges, blocking at cantilever support and closure at ends.



PROVIDE 3 ROWS OF 31/2" ARDOX SPIRAL NAILS @ & "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN. 2" LUMBER EDGE/END DISTANCE, DO NOT USE AIR NAILS



Wind

Tributary

Dead

Snow

9W6 NO. TAM 55/5-20 STRUCTURAL

COMPONENT Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design CONFORMS TO OBC 2012 properties and analysis methods. engineered wood products must be in accordance with current installation Guide and applicable building codes. To obtain installation Guide or ask questions, please call (800)232-0788 before installation.





PASSED

2ND FLR FRAMING\Flush Beams\B13(i2593) (Flush Beam)

Dry | 1 span | No cant.

March 25, 2020 09:20:17

Build 7239

Job name:

Customer:

Code reports:

Address: City, Province, Postal Code: WATERDOWN

BC CALC® Member Report

File name:

VALLEYCREEK 2S EL 1.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B13(i2593)

Specifier:

Designer:

AJ

CCMC 12472-R

Company:

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															_															
31															01-00	0														E

Total Horizontal Product Length = 04-01-00

Reaction Summary (Down / Opinic) (los)										
Bearing	Live	Dead	Snow	Wind						
B1. 3"	124 / 0	72/0								
B2. 2"	126 / 0	72 / 0								

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	-	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	Ľ	00-00-00	04-01-00	Тор		5			00-00-00
1	J7(12627)	Conc. Pt. (lbs)	L.	00-06-08	00-06-08	Тор	79	40			n\a
2	J7(i2645)	Conc. Pt. (lbs)	Ĺ	01-10-08	01-10-08	Top	51	25			. n\a
3	J7(i2555)	Conc. Pt. (lbs)	L	03-02-08	03-02-08	Тор	120	60	PRO	-E35/()	Na n\a
								á	0 10	_	W. (2)

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	232 ft-lbs	11610 ft-lbs	2.0%	1	01-10-08
End Shear	246 lbs	5785 lbs	4.3%	1	03-01-08
Total Load Deflection	L/999 (0.002")	n\a	n\a	4	02-01-13
Live Load Deflection	L/999 (0.001")	n\a	n\a	5	02-01-13
Max Defl.	0.002"	n\a	n\a	4	02-01-13
Span / Depth	4.8				

Bearing	ı Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material	
B1	Hanger	3" x 1-3/4"	277 lbs	n\a	4.3%	HUS1.81/10	
R2	Hanger	2" x 1-3/4"	279 lbs	n\a	6.5%	L90	

Cautions

Header for the hanger HUS1.81/10 at B1 is a Double 1-3/4" x 9-1/2" VERSA-LAM® 1.7 2400 DF. Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

Header for the hanger L90 at B2 is a Double 1-3/4" x 9-1/2" VERSA-LAM® 1.7 2400 DF.

Hanger model L90 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

CANFORMS TO OBE 2012

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86. AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

BUE NO. TAM 5516-20 STRÚCTURAL COMPONENT ONLY

OVINCE OF O

Disclosure

Use of the Bolse Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain installation Guide or ask questions, please call (800)232-0788 before installation.





PASSED

2ND FLR FRAMING\Flush Beams\B14(I1813) (Flush Beam)

Dry | 1 span | No cant. **BC CALC® Member Report**

March 25, 2020 09:20:17

Build 7239 Job name:

Address: City, Province, Postal Code: WATERDOWN

File name:

VALLEYCREEK 2S EL 1.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B14(i1813)

Specifier:

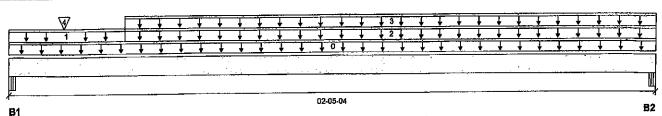
Designer:

ΑJ

Customer: Code reports:

CCMC 12472-R

Company:



Total Horizontal Product Length = 02-05-04

n Summani (Down / Unlift) (lhs)

Reaction Sun	_			
Bearing	Live	Dead	Snow	
B1, 5-1/4"	64/0	147 / 0	78 / 0	
B2, 5-1/4"	63/0	158 / 0	77 / 0	

Los	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	02-05-04	Тор		10			00-00-00
1	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	00-05-04	Тор	21	11			n\a
2	E28(i1210)	Unf. Lin. (lb/ft)	L	00-05-04	02-05-04	Top	33	111	63		n\a
2	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-05-04	02-05-04	Тор	- 18	9			n\a
ر ا	E27(i1214)	Conc. Pt. (lbs)	ī	00-02-08	00-02-08	Top	15	37	يسمو 29	OFESU	<i>O₂₁.</i> . n\a
4	E21(11214)	Conto. i t. (iba)				1			N OF	AND THE PERSON NAMED IN	and the

Controls Summary	Factored Demand	Factored Resi <u>stance</u>	Demand/ Resistance	Case	Location
Pos. Moment	109 ft-lbs	23220 ft-lbs	0.5%	13	01-02-12
End Shear	2 lbs	7521 lbs	n\a	0	01-02-12
Total Load Deflection	L/999 (0")	n\a	n\a	35	01-02-12
Live Load Deflection	L/999 (0")	n\a	n\a	51	01-02-12
Max Defl.	0"	n\a	n\a	35	01-02-12
Snan / Denth	21				

Beari	ng Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material	
B1	Beam	5-1/4" x 3-1/2"	366 lbs	4.7%	1.6%	Unspecified	
R2	Ream	5-1/4" x 3-1/2"	375 lbs	4.8%	1.7%	Unspecified	

Notes

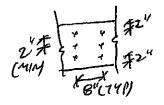
Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

AMENDED 2020 Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86. Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9



PROVIDE3 ROWS OF 3½" ARDOX SPIRAL NAILS @ 8 "O/G FOR MULTI-PLY HAILING, MAINTAIN A MIN.2" LUMBER EDGE/END DISTANCE. BO NOT USE AIR NAILS



DWG NO. TAM 5517 -20 STRUCTURAL COMPONENT ONLY

Disclosure

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PASSED

2ND FLR FRAMING\Flush Beams\B7(i1816) (Flush Beam)

Dry | 1 span | No cant.

March 25, 2020 09:20:17

BC CALC® Member Report Build 7239

Job name:

Address:

City, Province, Postal Code: WATERDOWN

Customer:

Code reports:

CCMC 12472-R

File name:

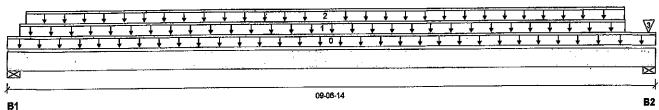
VALLEYCREEK 2S EL 1.mmdl

2ND FLR FRAMING\Flush Beams\B7(i1816) Description:

Specifier:

Designer: ΑJ

Company:



Total Horizontal Product Length = 09-06-14

Reaction Summany (Down / Uplift) (lbs)

Meacholl our	Intrary (Doming O	hine) (ma)			
Bearing	Live	Dead	Snow	Wind	
B1, 2-3/4"	99 / 0	336 / 0			
B2, 5-3/8"	100 / 0	340 / 0			

Lo	ad Summary						Lîve	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	09-06-14	Тор		5			00-00-00
1	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-02-04	09-01-08	Top	22	11			n\a
,	WALL	Unf. Lin. (lb/ft)	L.	00-03-06	09-01-06	Top		60			n\a
3	FC3 Floor Material	Conc. Pt. (lbs)	L	09-05-09	09-05-09	Тор	2		JE OF E	SSIO _N	n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1080 ft-lbs	7546 ft-lbs	14.3%	0	04-08-02
End Shear	454 lbs	3761 lbs	12.1%	0	08-04-00
Total Load Deflection	L/999 (0.058")	n/a	n\a	4	04-08-02
Live Load Deflection	L/999 (0.013")	n\a	n\a	5	04-08-02
Max Defl.	0.058"	n\a	n\a	4	04-08-02
Span / Depth	11.4				

Bearin	ng Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	2-3/4" x 1-3/4"	470 lbs	24.4%	12.3%	Spruce-Pine-Fir
B2	Wall/Plate	5-3/8" x 1-3/4"	476 lbs	12.6%	6.4%	Spruce-Pine-Fir

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86. Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

importance Factor: Normal Part code: Part 9

two no. tam5520 -20 STRUCTÚRAL COMPONENT ONLY **Disclosure**

ON TWEE OF OF

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input CONFORMS TO OBE 2012 must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Gulde and applicable building codes. To obtain installation Guide or ask questions, please call (800)232-0788 before installation.





Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B1(i2599) (Flush Beam)

PASSED

Dry | 1 span | No cant.

BC CALC® Member Report

March 25, 2020 09:20:17

Build 7239

Job name:

Address:

City, Province, Postal Code: WATERDOWN

Customer: Code reports:

CCMC 12472-R

File name:

VALLEYCREEK 2S EL 1.mmdl

Description:

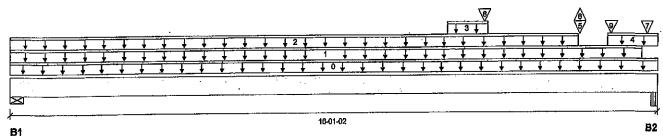
1ST FLR FRAMING\Flush Beams\B1(i2599)

Dead

Specifier: Designer:

ΑJ

Company:



Total Horizontal Product Length = 16-01-02

Snow

Reaction Summary (Down / Uplift) (lbs)

Live Dead 340/0 394/5 B1, 1-7/8' 1563 / 0 2442 / 54 B2, 7-7/8"

Lo	ad Summary						Live
	Description	Load Type	Ref.	Start	End	Loc.	1.00
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	16-01-02	Тор	
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	15-08-06	Тор	19
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	14-02-06	Top	8
3	13(i545)	Unf. Lin. (lb/ft)	L	10-10-14	11-10-14	Top	
4	10(i539)	Unf. Lin. (lb/ft)	L	14-10-14	16-01-02	Тор	
5	H	Conc. Pt. (lbs)	L	14-02-13	14-02-13	Top	781
6	-	Conc. Pt. (lbs)	L	14-02-13	14-02-13	Top	-59
7	J7(i1886)	Conc. Pt. (lbs)	L	15-09-14	15-09-14	Тор	152
8	13(1545)	Conc. Pt. (lbs)	L	11-09-14	11-09-14	Top	353
9	10(i539)	Conc. Pt. (lbs)	L	14-11-14	14-11-14	Top	1138

Controls Summary	Factored Demand	Factored Resista <u>nce</u>	Demand/ Resistance	Case	Location
Pos. Moment	6648 ft-lbs	36222 ft-lbs	18.4%	1	11-09-14
End Shear	4071 lbs	17356 lbs	23.5%	1	14-07-12
Total Load Deflection	L/740 (0.25")	n\a	32.4%	6	08-05-00
Live Load Deflection	L/1316 (0.141")	n\e	27,4%	8	08-06-14
Max Defl.	0,25"	n\a	n\a	6	08-05-00
Span / Depth	19.5				

Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	1-7/8" x 5-1/4"	1016 lbs 5617 lbs	16.8% 31.8%	8.5% 11.1%	Spruce-Pine-Fir Unspecified
B2	Beam	7-7/8" x 5-1/4"	801 / 10C	31.070	11,170	Cushacillad

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086. Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

PROVIDE3 ROWS OF 3½" ARDOX SPIRAL HAILS @ /2-"O/C FOR MULTI-PLY NAILING, MAINTAIN MIN. 24 LUMBER EDBE/END DO NOT USE AIR HALL

0.66 1.15 00-00-00 14 10 n\a n\a 4 81 n\a CROPE SOLVING 81 n\a n\a n\a n\a én\a n\a TO HAVE OF ONTE BYS HO. TAM 5521 -20

Wind

Tributary

Snow

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BC CALC® Member Report

Double 1-3/4" x 9-1/2" VERSA-LAM® 2,0 3100 SP



B3

1ST FLR FRAMING\Flush Beams\B2(i2592) (Flush Beam)

Dry | 2 spans | No cant.

March 25, 2020 09:20:17

Build 7239

Job name:

Customer:

B1

Address:

City, Province, Postal Code: WATERDOWN

File name:

VALLEYCREEK 2S EL 1.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B2(i2592)

Wind

Specifier:

Designer: ΑJ

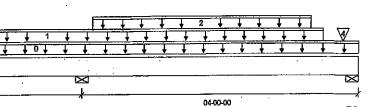
В2

Code reports:

CCMC 12472-R

05-03-14

Company:



Total Horizontal Product Length = 09-03-14

Snow

Reaction Summary (Down / Uplift) (lbs)
Bearing Live Dead 722 / 70 347/0 B1, 4" 1094/0 2083 / 0 B2, 3-1/2" 2201 / 146 1115/0 B3, 5-1/2"

Los	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	09-03-14	Тор		10			00-00-00
1	Smoothed Load	Unf. Lin. (lb/ft)	L.	00-09-14	08-09-14	Top	288	144			n\a
2	STAIR	Unf. Lin. (lb/ft)	L	05-05-10	08-07-14	Top	240	120			n\a
2	J2(i2563)	Conc. Pt. (lbs)	Ĺ	00-01-14	00-01-14	qoT	244	122			n\a
4	92(12303) 9/1521)	Conc. Pt. (lbs)	Ē	09-01-02	09-01-02	Тор	1449	797			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1372 ft-lbs	23220 ft-lbs	5.9%	2	02-09-14
Neg. Moment	-1903 ft-lbs	-23220 ft-lbs	8.2%	1	05-03-14
End Shear	1424 lbs	11571 lbs	12.3%	3	08-00-14
Cont. Shear	1641 lbs	11571 lbs	14.2%	1	04-04-10
Total Load Deflection	L/999 (0.007")	n\a	n\a	9	02-06-14
Live Load Deflection	L/999 (0.005")	n\a	n\a	12	02-07-14
Total Neg. Defl.	L/999 (-0.001")	n\a	n\a	9	06-04-00
Max Defl.	0.007"	n\a	n\a	9	02-06-14
Span / Depth	6.4				

Bearing	g Supports	Dim. (ŁxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Hanger	4" x 3-1/2"	1517 lbs	n\a	8.9%	HGUS410
B2	Wall/Plate	3-1/2" x 3-1/2"	4491 lbs	59.6%	30.0%	Spruce-Pine-Fir
B3	Wall/Plate	5-1/2" x 3-1/2"	4695 lbs	39.6%	20.0%	Spruce-Pine-Fir

Cautions

Header for the hanger HGUS410 at B1 is a Triple 1-3/4" x 9-1/2" VERSA-LAM® 1.7 2400 DF. Hanger model HGUS410 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.



TWO NO. TAMS 522 -20 STRUCTURAL COMPONENT ONLY





PASSED

1ST FLR FRAMING\Flush Beams\B2(i2592) (Flush Beam)

Dry | 2 spans | No cant.

March 25, 2020 09:20:17

Build 7239

Job name:

Address:

City, Province, Postal Code: WATERDOWN

BC CALC® Member Report

Customer: Code reports: CCMC 12472-R

File name:

VALLEYCREEK 2S EL 1.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B2(i2592)

ΑJ

Specifier:

Designer:

Company:

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

CONFORMS TO OBC 2012

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

PROVIDE PROWS OF 3½" ARDOX SPIRAL NAILS @ 8"0/8 FOR MULTI-PLY NAILING, MAINTAIN A MIN.2"LUMBER EDGE/END DISTANCE, DO NOT USE AIR NAILS



146 HO, TAN 5522420 STRUCTURAL

Disclosure ONLY

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PASSED

March 25, 2020 09:20:17

1ST FLR FRAMING\Flush Beams\B3(i2591) (Flush Beam)

BC CALC® Member Report

Build 7239

Job name:

Address:

City, Province, Postal Code: WATERDOWN

Customer:

Code reports:

CCMC 12472-R

Dry | 1 span | No cant.

File name:

VALLEYCREEK 2S EL 1.mmdl

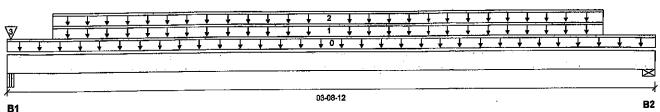
Description:

1ST FLR FRAMING\Flush Beams\B3(i2591)

Specifier:

Designer: AJ

Company:



Total Horizontal Product Length = 03-08-12

Snow

Reaction Summary (Down / Uplift) (lbs)

Dead Bearing Live 205/0 337/0 B1, 3-1/8" 113/0 207 / 0 B2, 3-1/2"

10	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	•	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-08-12	Тор		5			00-00-00
1	STAIR	Unf. Lin. (lb/ft)	L	00-03-02	03-05-04	Тор	120	60			n\a
9	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-03-02	03-05-04	Top	10	5			n\a
3	9(i538)	Conc. Pt. (lbs)	L	00-00-04	00-00-04	Top	129	92	.42	OFESS	n/a
•	0(1000)	• • •							W. O.	SOL COL	IONA

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	385 ft-lbs	11610 ft-lbs	3.3%	1	01-10-03
End Shear	226 lbs	5785 lbs	3.9%	1	01-00-10
Total Load Deflection	L/999 (0.002")	n\a	n\a	4	01-10-03
Live Load Deflection	L/999 (0.001")	n\a	n\a	5	01-10-03
Max Defl.	0.002"	n\a	n\a	4	01-10-03
Span / Depth	4.2				

Bearin	ng Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Beam	3-1/8" x 1-3/4"	763 lbs	32.6%	11.4%	Unspecified
B2	Wall/Plate	3-1/2" x 1-3/4"	451 lbs	12.0%	6.0%	Spruce-Pine-Fir

Notes

and the second

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

AMENDED 2020 Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



OWO NO. TAM 5523 -20 STRUCTURAL COMPONENT ONLY

Disclosure

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BC CALC® Member Report



Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

PASSED

1ST FLR FRAMING\Flush Beams\B4(i2632) (Flush Beam)

Dry | 1 span | No cant.

March 25, 2020 09:20:17

Build 7239

Job name:

Customer:

Address:

Code reports:

City, Province, Postal Code: WATERDOWN

CCMC 12472-R

File name:

VALLEYCREEK 2S EL 1.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B4(i2632)

Wind

Specifier:

Designer: ΑJ

Company:

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Total Horizontal Product Length = 12-11-04

Reaction Summary (Down / Uplift) (Ibs)

Dead Live <u>111 / 0</u> 86/0 B1, 1-7/8" 89/0 115/0 B2, 4-3/8"

	ad Summary	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
nay	Description Self-Weight	Unf. Lin. (lb/ft)	L		12-11-04	Тор	·····	5			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	12-11-04	Top	17	9			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	849 ft-lbs	11610 ft-lbs	7.3%	1	06-04-06
End Shear	234 lbs	5785 lbs	4.0%	1	00-11-06
Total Load Deflection	L/999 (0.069")	n\a	n\a	4	06-04-06
Live Load Deflection	∐/999 (0.039")	n\a	n\a	5	06-04-06
Max Defl.	0.069"	n\a	n\a	4	06-04-06
Span / Depth	15.8				

Bearing Supp	orts Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1 Wall/Pla	ate 1-7/8" x 1-3/4"	275 lbs	13.6%	6.9%	Spruce-Pine-Fir
B2 Wall/Pla		284 lbs	6.0%	3.0%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

AMENDED 2020 Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



646 NO. TAM 5524 -20 STRUCTURAL COMPONENT ONLY

Disclosure

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PASSED

1ST FLR FRAMING\Flush Beams\B20(i2839) (Flush Beam)

Dry | 1 span | No cant.

March 25, 2020 10:18:09

VALLEYCREEK 2S EL 1 DECK CONDITION.mmdi

Build 7239

Job name: Address:

City, Province, Postal Code: WATERDOWN

BC CALC® Member Report

File name: Description: 1ST FLR FRAMING\Flush Beams\B20(i2839)

> Specifier: Designer:

Customer: Code reports:

B1

CCMC 12472-R

Company:

ΑJ

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<u>an terrera</u>

Total Horizontal Product Length = 03-01-00

Snow

Reaction Summary (Down / Uplift) (lbs)

Dead Bearing Live 311/0 78/0 B1, 3" 311/0 B2, 3" 78/0

1.	ad Summary						Live	Dead	Snow	Wind	Tributary
	g Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-01-00	Тор		10			00-00-00
1	E13((513)	Unf. Lin. (lb/ft)	L	00-00-00	03-01-00	Top	24	179			n\a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	03-01-00	Тор	27	13		tion and	n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	259 ft-lbs	15093 ft-lbs	1.7%	0	01-06-08
End Shear	141 lbs	7521 lbs	1.9%	0	01-00-08
Total Load Deflection	L/999 (0.001")	n\a	n\a	4	01-06-08
Live Load Deflection	L/999 (0")	n\a	n\a	5	01-06-08
Max Defl.	0.001"	n\a	n\a	4	01-06-08
Snan / Denth	3.4				

Bearin	ng Supports	Dîm. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material	
B1	Wali/Plate	3" x 3-1/2"	435 lbs	10.4%	5.2%	Spruce-Pine-Fir	_
B2	Wall/Plate	3" x 3-1/2"	435 lbs	10.4%	5.2%	Spruce-Pine-Fir	

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86. AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

PROVIDE 3 ROWS OF 3½" ARBOX
SPIRAL NAILS @ 6 "0/6 FOR
MULTI-PLY NAILING, MAINTAIN
A MIN.Z" LUMBER EDGE/END
DISTANCE. DO NOT USE AIR NAILS

CONFORMS TO OBC 2012



DWE NO. YAN 5525-20 STRUCTURAL COMPONENT ONLY

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PASSED

1ST FLR FRAMING\Flush Beams\B21(i2840) (Flush Beam)

Dry | 1 span | No cant.

March 25, 2020 10:18:09

VALLEYCREEK 2S EL 1 DECK CONDITION.mmdl

1ST FLR FRAMING\Flush Beams\B21(i2840)

Build 7239

Job name:

Address: City, Province, Postal Code: WATERDOWN

BC CALC® Member Report

File name: Description:

Specifier:

Designer: ΑJ

Customer: Code reports:

CCMC 12472-R

Company:

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Total Horizontal Product Length = 03-01-00

Snow

Reaction Summary (Down / Uplift) (lbs)

Live Dead Bearing 718/0 908/0 B1, 3" 908/0 718/0 B2, 3"

Los	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-01-00	Тор		10			00-00-00
1	E17(i517)	Unf, Lin. (lb/ft)	L	00-00-00	03-01-00	Тор	316	320			n\a
2	J1(i2660)	Conc. Pt. (lbs)	L	00-10-08	00-10-08	Top	421	210	فقد		n\a
3	J1(i2664)	Conc. Pt. (lbs)	L	02-02-08	02-02-08	Тор	421	210	ARCO PRO	FESSI	O/V _A n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1427 ft-lbs	23220 ft-lbs	6.1%	1	01-06-08
End Shear	1148 lbs	11571 lbs	9.9%	1	01-00-08
Total Load Deflection	L/999 (0.003")	n\a	n\a	4	01-06-08
Live Load Deflection	L/999 (0.002")	n\a	n\a	5	01-06-08
Max Defl.	0.003"	n\a	n\a	4	01-06-08
Span / Depth	3.4				

Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	3" x 3-1/2"	2259 lbs	35.0%	17.6%	Spruce-Pine-Fir
B2	Wall/Plate	3" x 3-1/2"	2260 lbs	35.0%	17.6%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

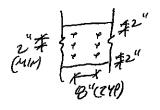
Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86. AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



PROVIDE3 ROWS OF 3½" ARDOX SPIRAL NAILS @ 8 "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN.2" LUMBER EDGE/END DISTANCE. DO NOTUSE AIR NAILS 948 NO. TAM 5526-20 STRUCTURAL COMPONENT ONLY

ONINCE OF

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Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 bellection Limit 3/4" OSB G&N Sheathing







			6:	are		1/2" Gypsum Ceiling					
Depth	Series		On Centi	e Spacing			On Centre Spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"		
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"		
	NI-40x	17'-0"	16'-0"	15'-1"	13'-11"	17'-5"	16'-1"	15'-1"	13'-11"		
9-1/2"	NI-60	17'-2"	16'-2"	15'-5"	14'-3"	17'-6"	16'-5"	15'-5"	14'-3"		
	NI-70	18'-0"	16'-11"	16'-3"	15'-6"	18'-5"	17'-3"	16'-7"	15'-6"		
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	15'-10"		
	NI-20	17'-10"	16'-10"	16'-0"	14'-10"	18'-6"	17'-1"	16'-0"	14'-10"		
	NI-40x	19'-4"	17'-11"	17'-3"	15'-10"	19'-11"	18'-6"	17'-9"	15'-10"		
	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-1"		
11-7/8"	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"		
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"		
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"		
	N1-40x	21'-5"	19'-10"	18'-11"	17'-5"	22'-1"	20'-6"	19'-6"	17'-5"		
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"		
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"		
	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"		
	NI-90x	24'-1"	221-3"	21'-2"	20'-0"	24'-8"	22'-10"	21′-9"	20'-7"		
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"		
. 411	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22'-9"	21'-6"		
16"	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"		
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"		

			Mid-Spar	n Blocking		Mid-Span Blocking and 1/2" Gypsum Ceiling				
Depth	Series		On Centr	re Spacing			On Cent	re Spacing		
Dept.	•••	12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"	
	NI-40x	17'-9"	16'-1"	15'-1"	13'-11"	17'-9"	16'-1"	15'-1"	13'-11'	
9-1/2"	NI-60	18'-1"	16'-5"	15'-5"	14'-3"	18'-1"	16'-5"	15'-5"	14'-3"	
,-	NI-70	19'-10"	17'-11"	16'-9"	15'-6"	19'-10"	17'-11"	16'-9"	15'-6"	
	NI-80	20'-2"	18'-3"	17'-1"	15'-10"	20'-2"	18'-3"	<u> 17'-1"</u>	15'-10'	
	NI-20	18'-10"	17'-1"	16'-0"	14'-10"	18'-10"	17'-1"	16'-0"	14'-10'	
	NI-40x	21'-3"	19'-3"	17'-9"	15'-10"	21'-3"	19'-3"	17'-9"	15'-10'	
	NI-60	21'-9"	19'-8"	18'-5"	17'-1"	21'-9"	19'-8"	18'-5"	17'-1"	
11-7/8"	NI-70	23'-4"	21'-5"	20'-1"	18'-6"	23'-8"	21'-5"	20'-1"	18'-6"	
	NI-80	23'-7"	21'-10"	20'-5"	18'-11"	24'-1"	21'-10"	20'-5"	18'-11"	
	NI-90x	24'-3"	22'-6"	21'-3"	19'-7"	24'-8"	22'-7"	21'-3"	19'-7"	
	NI-40x	24'-2"	21'-5"	19'-6"	17'-5"	24'-2"	21'-5"	19'-6"	17'-5"	
	NI-60	24'-9"	22'-5"	21'-0"	19'-6"	24'-9"	22'-5"	21'-0"	19'-6"	
14"	NI-70	26'-1"	24'-3"	22'-9"	21'-0"	26'-8"	24'-3"	22'-9"	21'-0"	
	NI-80	26'-6"	24'-7"	23'-3"	21'-6"	27'-1"	24'-10"	23'-3"	21'-6"	
	NI-90x	27'-3"	25'-4"	24'- <u>1"</u>	22'-4"	27'-9"	25'-10"	24'-3"	22'-4"	
	Nt-60	27'-3"	24'-11"	23'-5"	21'-7"	27'-6"	24'-11"	23'-5"	21'-7"	
	NI-70	28'-8"	26'-8"	25'-3"	23'-4"	29'-3"	26' -11 "	25'-3"	23'-4"	
16"	NI-80	29'-1"	27'-0"	25'-9"	23'-10"	29'-8"	27'-6"	25'-10"	23'-10"	
	NI-90x	29'-11"	27'-10"	26'-6"	24'-10"	30'-6"	28'-5"	26'-11"	24'-10"	

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

Minimum bearing length shall be 1-3/4 inches for the end bearings.
 Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA 086-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Maximum Floor Spans Live Load = 40 psf, Dead Load = 15 psf

Live Load = 40 psf, Déad Load = 15 psf Simple Spans, L/480 Déflettion Limit 5/8" OSB G&N Sheathing







			В	are		1/2" Gypsum Celling On Centre Spacing				
Depth	Series		On Centi	re Spacing						
υτραι		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	15'-1"	14'-2"	13'-9"	N/A	15'-7"	14'-8"	14'-2"	N/A	
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A	
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15 9"	15'-3"	N/A	
-1/2	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5" .	15'-10"	N/A	
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A	
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A	
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A	
	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A	
11-7/8"	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A	
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A	
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A	
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A	
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A	
a All	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8°	N/A	
14"	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A	
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A	
		22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A	
	NI-60	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A	
16"	NI-70	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A	
	N1-80		22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A	
	NI-90x	24'-8"	24.9	41-7	17/7	1 23 77		<u> </u>	.7/	

			Mid-Spar	n Blocking		Mid-Span Blocking and 1/2" Gypsum Ceiling				
Depth	Series		On Centr	e Spacing		On Centre Spacing				
Deptil	JENICS	12"	16"	19.2°	24"	12"	16"	19.2"	24"	
	NI-20	16'-8"	15'-3"	14'-5"	N/A	16'-8"	15'-3"	14'-5"	N/A	
	Nt-40x	17'-11"	16'-11"	16'-1"	N/A	18'-5"	17'-1"	16'-1"	N/A	
9-1/2"	NI-60	18'-2"	17'-1"	16'-4"	N/A	18'-7"	17'-4"	16'-4"	N/A	
J-1/2	NI-70	19'-2"	17'-10"	17'-2"	N/A	19'-7"	18'-3"	17'-7"	N/A	
	N1-80	19'-5"	18'-0"	17'-4"	N/A	19'-10"	18'-5"	17'-8"	N/A	
	NI-20	19'-6"	18'-1"	17'-3"	N/A	19'-11"	18'-3"	17'-3"	N/A	
	NI-40x	21'-0"	19¹-6"	18'-8"	N/A	21'-7"	20'-2"	19'-2"	N/A	
	NI-60	21'-4"	19'-9"	18'-11"	N/A	21'-11"	20'-4"	19'-6"	N/A	
11-7/8"	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-5"	20'-5"	N/A	
		22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-8"	N/A	
	NI-80	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A	
	NI-90x	23'-7"	21'-11"	20'-11"	N/A	24'-3"	22'-7"	21'-7"	N/A	
	NI-40x	25 - 7 24'-0"	22'-3"	21'-3"	N/A	24'-8"	22'-11"	21'-11"	N/A	
	NI-60		23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-11"	N/A	
14"	NI-70	25'-3"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A	
	NI-80	25'-7"		23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A	
	NI-90x	26'-4"	24'-4"			27'-2"	25'-3"	24'-2"	N/A	
	NI-60	26'-5"	24'-6"	23'-4"	N/A					
16"	NI-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A	
10	Nt-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A	
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26'-2"	N/A	

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

3. Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

Minimum deaning length shall be 1-3/3 method to the spans and spacings given in this table, except as required for hangers.
 Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA 086-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Umit 3/4" OSB G&N Sheathing







	Bare					1/2" Gypsum Ceiling					
Depth	Serles		On Cent	re Spacing			On Centre Spacing				
nehai	201100	12"	16"	19.2"	24"	12"	16"	19.2"	24"		
	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"		
	NI-40x	17'-0"	16'-0"	15'-5"	14'-9"	17'-5"	16'-5"	15'-10"	15'-2"		
9-1/2"	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-6"	16'-7"	15'-11"	15'-3"		
3-112	NI-70	18'-0"	16'-11"	16'-3"	15'-7"	18'-5"	17'-3"	16'-7"	15'-11"		
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	_ 17'-5"	16'-9"	16'-1"		
	NI-20	17'-10"	16'-10"	16'-2"	15'-6"	18'-6"	17'-4"	16'-9"	16'-1"		
	NI-40x	19'-4"	17'-11"	17'-3"	16'-6"	19'-11"	18'-6"	17'-9"	17'-0"		
	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"		
11-7/8"	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18 - 1 0"	17'-10'		
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"		
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"		
	NI-40x	21'-5"	19'-10"	18'-11"	17'-11"	22'-1"	20'~6"	19'-7"	18'-7"		
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"		
14*	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"		
14	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"		
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"		
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"		
	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22'-9"	21'-6"		
16"	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"		
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"		

			Mid-Spar	n Błocking		Mld-S	pan Blocking an	d 1/2" Gypsum	Ceiling	
Danels	Series		On Cente	e Spacing	.,,	On Centre Spacing				
Depth	361163	12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"	
	NI-40x	18'-8"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"	
9-1/2"	NI-60	18'-11"	17'-6*	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"	
3-1/ Z	NI-70	20'-0"	18'-7"	17'-9"	16'-7"	20'-5"	18'-11"	17'-10"	16'-7"	
	N1-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10"	
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"	
	NI-40x	21'-10"	20'-4"	19'-4"	17'-8"	22'-5"	20'-6"	19'-4"	17'-8"	
	NI-60	22'-1"	201-7*	19'-7"	18-4"	22'-8"	20'-10"	19'-8"	18'-4"	
11-7/8"	NI-00 NI-70	23'-4"	21'-8"	201-8"	19'-7"	23'-10"	22'-3"	21'-2"	19'-9"	
	NI-70 NI-80	23'-7"	21'-11"	20'-11"	19'-9"	24'-1"	22'-6"	21'-5"	20'-0"	
	NI-80 NI-90x	25-7 24'-3"	22'-6"	21'-6"	20'-4"	24'-8"	23'-0"	22'-0"	20'-9"	
		24'-5"	22'-9"	21-8"	19'-5"	25'-1"	23'-2"	21'-9"	19'-5"	
	NI-40x	24'-10"	23'-1"	22'-0"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10"	
	NI-60	26'-1"	24'-3"	23'-2"	21'-10"	26'-8"	24'-11"	23'-9"	22'-4"	
14"	NI-70	26'-6"	24'-7"	23'-5"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"	
	NI-80		25'-4"	24'-1"	22'-9"	27'-9"	25'-11"	24'-8"	23'-4"	
	NI-90x	27'-3"	25'-5"	24'-2"	22'-10"	28'-0"	26'-2"	24'-9"	23'-1"	
	NI-60	27'-3"		24 -2 25'-4"	23'-11"	29'-3"	20-2 27'-4"	26'-1"	24'-8"	
16"	NI-70	28'-8"	26'-8"	25*-4 25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"	
**	NI-80	29'-1" 29'-11"	27'-0" 27'-10*	25 -9 26'-6"	24 -4 25'-0"	30'-6"	27 -5" 28¹-5"	20-3 27'-2"	25'-8"	
	NI-90x	73-11	27-10	20.0		I				

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/240.

3. Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

5. This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA 086-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 5/8" OSS G&N Sheathing







			В	are		1/2" Gypsum Ceiling					
Depth	Series		On Cent	re Spacing			On Centre Spacing				
ocpui	J	12"	16"	19.2"	24"	12"	16"	19.2"	24"		
	NI-20	15'-1"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A		
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A		
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A		
,-	NI-70	17'-1"	16'- 1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A		
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A		
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A		
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A		
	N!-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A		
11-7/8"	N1-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A		
	N1-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A		
	NJ-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A		
	N1-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A		
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A		
14°	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A		
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A		
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A		
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23 -1"	21'-5"	20'-6"	N/A		
	NI-70	23'-6"	21'-9"	20' -9 "	N/A	24'-3"	22'-5"	21'-5"	N/A		
16"	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A		
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A		

			Mid-Spar	n Blocking		Mid-S	pan Blocking an	id 1/2" Gypsum	Ceiling		
Depth	Series			e Spacing			On Centre Spacing				
Depair	501120	12"	16"	19.2"	24"	12"	16"	19.2"	24"		
	NI-20	15'-7"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A		
	NI-40x	17'-9"	16'-1"	15' -1 "	N/A	17'-9"	16'-1"	15'-1"	N/A		
9-1/2"	NI-60	18'-1"	16'-4"	15'-4"	N/A	18'-1"	16'-4"	15'-4"	N/A		
3-112	NI-70	19'-2"	17'-10"	16'-9"	N/A	19'-7"	17'-10"	16'-9"	N/A		
	NI-80	19'-5"	18'-0"	17'-1"	N/A	19'-10"	18'-3"	17'-1"	N/A		
	NI-20	18'-9"	17'-0"	16'-0"	N/A	18'-9"	17'-0"	16'-0"	N/A		
	NI-40x	21'-0"	19'-3"	17'-9"	N/A	21'-3"	19'-3"	17¹-9 ⁱ ¹	N/A		
	NI-50	21'-4"	19'-8"	18'-5"	N/A	21'-8"	19'-8"	18'-5"	. N/A		
11-7/8"	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-4"	20'-0"	N/A		
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-5"	N/A		
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A		
	NI-40x	23'-7"	21'-5"	19'-6"	N/A	24'-1"	21'-5"	19'-6"	N/A		
	NI-60	24'-0"	22'-3"	21'-0"	N/A	24'-8"	22'-5"	21'-0"	N/A		
14"	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-9"	N/A		
14	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A		
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A		
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	24'-10"	23'-4"	N/A		
	NI-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A		
16"	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A		
	N1-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26'-2"	N/A		

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration,

Minimum bearing length shall be 1-3/4 inches for the end bearings.
 Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

a live load deflection limit of L/480 and a total load deflection limit of L/240.

2. Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA 086-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic 1-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.

Construction Detail



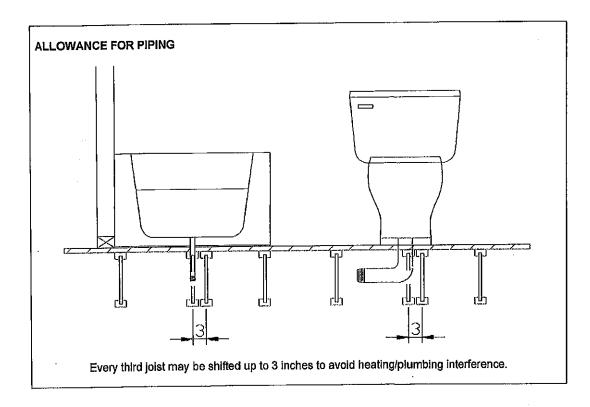
Limit States Design

Allowance for Piping (Installation Notes)

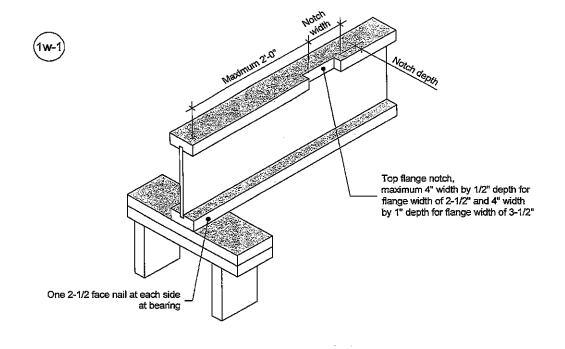
The floor layouts have usually not been checked for heating and/or plumbing interference. On-site adjustment of joists of up to 3 inches is permitted to avoid interferences. When moving a joist, the subfloor thickness shall be checked with code requirements when the joist spacing exceeds 19.2 inches. Except for cutting to length, I-joist flanges should never be cut, drilled, or notched.

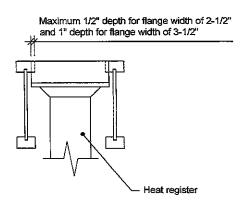
Installation of Nordic I-joists shall be as per *Nordic Joist Installation Guide for Residential Floors*. Refer to Tables 1 and 2 for maximum web hole and duct chase openings, respectively. These tables are based on the I-joists being used at their maximum spans. The minimum distance given may be reduced for shorter spans; contact your distributor for additional information.

The detail below shows the 3-inch allowance for piping. Every third joist may be shifted up to 3 Inches to avoid heating/plumbing interference. For other applications, please contact your distributor.



Revised April 12, 2012





Notes:

- 1. Blocking required at bearing for lateral support, not shown for clarity.
- The maximum dimensions for a notch on the side of the top flange are 4-inch width by 1/2-inch depth for flange width of 2-1/2 inches, and 4-inch width by 1-inch depth for flange width of 3-1/2 inches.
- 3. This detail applies to simple-span joists and multiple-span joists where the notch is located at the end half-span.
- 4. For other applications, contact Nordic Structures.

This document supersedes all previous versions. If the document has been in effect for more than one year, consult nordic, ca or contact Nordic Structures.

All nails shown in the details are assumed to be common nails unless otherwise noted. Nails shall have a diameter not less than 0.128 inch for 2-1/2-inch nails, or 0.144 inch for 3-inch nails. Individual components not shown to scale for clarity.

NORDIC STRUCTURES

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Notch in I-joist for Heat Register

CATEGOR

I-joist - Typical Floor Framing and Construction Details

DOCUMENT

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