



FROM PLAN DATED: JAN 2020

BUILDER: GREENPARK HOMES

SITE: RUSSELL GARDENS PH 3

MODEL: VALLEYCREEK 5

ELEVATION: 3

LOT: 96

CITY: WATERDOWN

SALESMAN: MARIO DICIANO

DESIGNER: AJ REVISION:

NOTES:

REFER TO THE **NORDIC INSTALLATION**GUIDE FOR PROPER STORAGE AND
INSTALLATION.

SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F
REQ'D UNDER INTERIOR UNIFORM LOAD
BEARING WALLS. MULTIPLE SQUASH
BLOCKS REQ'D UNDER CONCENTRATED
LOADS. SEE FIGURE 1. CANTILEVERED
JOISTS INCLUDING CANT' OVER BRICK REQ.
I-JOIST BLOCKING ALONG BEARING AND
RIMBOARD CLOSURE AT ENDS. SEE
FIGURES 4 & 5 FOR REINFORCEMENT
REQUIREMENTS. FOR HOLES INCLUDING
DUCT CHASE AND FIELD CUT OPENINGS
SEE FIGURE 7, TABLES 1 & 2. CERAMIC TILE
APPLICATION AS PER O.B.C 9.30.6.

LOADING:

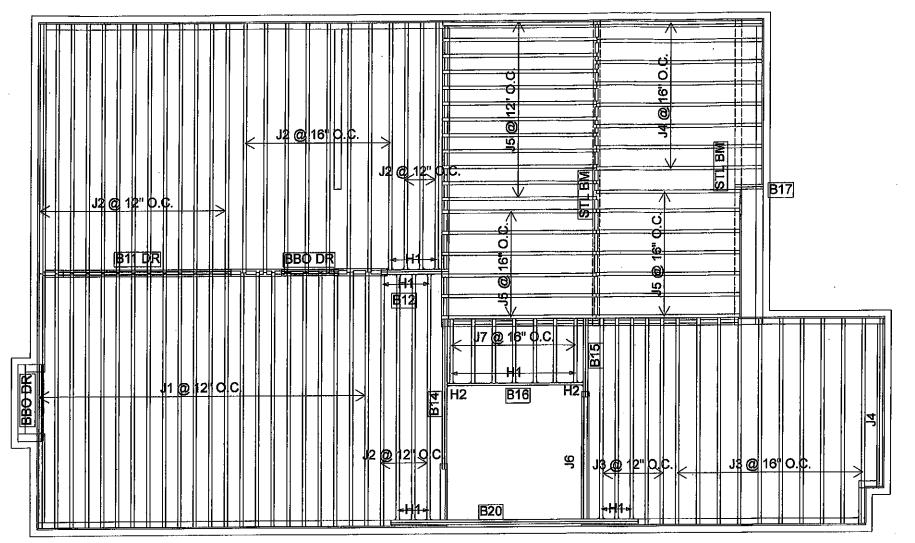
DESIGN LOADS: L/480.000-LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 20.0 lb/ft²

SUBFLOOR: 3/4" GLUED AND NAILED

DATE: 2020-03-24

1st FLOOR

DECK CONDITION



		Products			
PlotID	Length	Product	Plies	Net Qty	Fab Type
J1	18-00-00	9 1/2" NI-40x	1	22	MFD
J2	16-00-00	9 1/2" NI-40x	1	28	MFD
J3	14-00-00	9 1/2" NI-40x	1	15	MFD
J4	12-00-00	9 1/2" NI-40x	1	9	MFD
J5	10-00-00	9 1/2" NI-40x	1	25	MFD
J6	8-00-00	9 1/2" NI-40x	1	1	MFD
J7	6-00-00	9 1/2" NI-40x	1	7	MFD
B20	16-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	MFD
B11 DR	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3	MFD
B16	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1	MFD
B14	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	MFD
B12	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	MFD
B15	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	MFD
B17	2-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	_2	MFD

	Connector	Summary
Qty	Manuf	Product
7	H1	IUS2.56/9.5
14	H1	IUS2.56/9.5
2	H2	HUS1.81/10

CITY OF HAMILTON **Building Division**

THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE

THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW

se drawings and/or specifications have been reviewed by FEB 2 2 2021 FOR CHIEF BUILDING OFFICIAL

DATE



FROM PLAN DATED: JAN 2020

BUILDER: GREENPARK HOMES

SITE: RUSSELL GARDENS PH 3

MODEL: VALLEYCREEK 5

ELEVATION: 3

LOT: 96

CITY: WATERDOWN

SALESMAN: MARIO DICIANO

DESIGNER: AJ **REVISION:**

NOTES:

REFER TO THE NORDIC INSTALLATION **GUIDE FOR PROPER STORAGE AND** INSTALLATION. SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS, SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD **CUT OPENINGS** SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6

LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft2 DEAD LOAD: 20.0 lb/ft2

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 2020-03-24

2nd FLOOR

NORDIC STRUCTURES

COMPANYFeb. 13, 2020 15:27

PROJECT
J1 1ST FLOOR.wwb

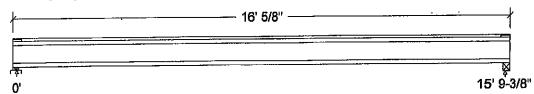
Design Check Calculation Sheet

Nordic Sizer - Canada 7.2

Loads:

Load	Туре	Distribution	Pat-	Location	[ft]	Magnitud	le	Unit
12044	-2.5		tern	Start	End	Start	End	
Load1	Dead	Full Area				20.00		psf
Load2	Live	Full Area	,			40.00		psf

Maximum Reactions (lbs) and Support Bearing (in):



Unfactored:	150	15
Dead	158	31
Live	316	31
Factored:		 67
Total	671	
Bearing:		
Capacity	1	186
Joist	1865	100
Support	3981	_
' Des ratio		ر م
Joist	0.36	0.30
Support	0.17	
Load case	#2	#:
Length	2-3/8	2-5/8
Min req'd	1-3/4	1-3/4
Stiffener	No	No
KD	1.00	1.00
KB support	1.00	-
fcp sup	769	_
Kzcp sup	1.09	

Nordic Joist 9-1/2" NI-40x Floor joist @ 12" o.c.

Supports: 1 - Lumber Sill plate, No.1/No.2; 2 - Steel Beam, W; Total length: 16' 5/8"; Clear span: 15' 7-5/8"; 3/4" nailed and glued OSB sheathing This section PASSES the design code check.

Limit States Design using CSA-O86-09 and Vibration Criterion:

Criterion	Analysis Value	Design Value	Unit	Analysis/Design
Shear	Vf = 671	Vr = 1895	lbs	Vf/Vr = 0.35
Moment (+)	Mf = 2646	Mr = 4824	lbs-ft	Mf/Mr = 0.55
Perm. Defl'n	0.12 = < L/999	0.53 = L/360	in 🥖	ROFESSION 0.22
Live Defl'n	0.23 = L/807	0.39 = L/480	in /o	0.59
Total Defl'n	0.35 = L/538	0.79 = L/240	in /6/2	32610 3.45
Bare Defl'n	0.28 = L/676	0.53 = L/360	in 🔏	M. 53
Vibration	Lmax = 15'-9.4	Lv = 17'-1.8	ft (9)	KATSOULAKOS 92
Defl'n	= 0.031	= 0.041	in 🕍 🕻	0.77
17011 II			3 4	W. W

NOTION ON THE NO. TAM 5459 -20
STRUCTURAL
COMPONENT ONLY

WoodWorks® Sizer

for NORDIC STRUCTURES

J1 1ST FLOOR.wwb

Nordic Sizer - Canada 7.2

Page 2

71 131 1 200)[\.\V									
Additiona	i Data:									_
	f/E	KD	KH	KZ	KL	KT	KS	KN	LC#	
Vr		1.00	1.00	-	_	_	-	-	#2	
	4824		1.00	_	1.000	-	-	-	#2	
ΕÏ	218.1 π	aillion	-	-	-		-	-	#2	
CRITICAL L			3:							
Shear	: LC #2	= 1.25	5D + 1.5I							
Moment (+) : LC #2	= 1.25	5D + 1.51	ն						
Deflecti	on: LC #1	= 1.01	D (perma	anent)						
	LC #2	· = 1.0	0 + 1.0L	(live	+)					•
	LC #2	= 1.01	D + 1.0L	(tota	.1)					
	LC #2	= 1.0	D + 1.0L	(bare	joist)					
Bearing	: Suppo	ort 1 - 1	LC #2 = 1	1.25D +	1.5L					
-	Suppo	ort 2 - 1	LC #2 = 1	1.25D +	1.5L					
Load Typ	es: D=dea	ıd W=wi	nd S≃sno	ом Н=е	arth,grou	indwate	r E≕ear	thquake		
	L=liv	re(use,o	ccupancy)) Ls=l	ive(stora	ge, equ:	ipment)	r=rire		
Load Pat	terns: s=	=S/2 L≕]	L+Ls _=1	no patt	ern load	in this	s span			
All Load	Combinat	ions (Lo	Cs) are l	Listed	in the An	alysis	output			,
CALCULATI	IONS:							enn	AT SURA	OBC 2012
EIeff =	265.29 lb	-in^2	K= 4.946	e06 lbs					I O A III O I O	
"Live" d	leflection	ı is due	to all r	non-dea	d loads (live, w	wind, sn	OW)	AMENDED	202 0

Design Notes:

- 1. WoodWorks analysis and design are in accordance with the 2010 National Building Code of Canada (NBC), Division B, Part 4, and the CSA O86-09 Engineering Design in Wood standard, which includes Update No.1
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to Nordic Structures technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.



BWG NO. TAM SYST -20 STRUCTURAL COMPONENT ONLY

NORDIC STRUCTURES

COMPANY Feb. 13, 2020 15:28

PROJECT
J1 2ND FLOOR.wwb

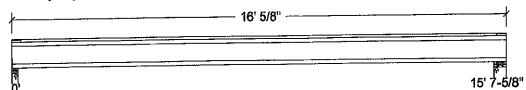
Design Check Calculation Sheet

Nordic Sizer - Canada 7.2

Loads:

Load	Type	Distribution	Pat- tern	Location Start	[ft] End	Magnitud Start	le End	Unit
Load1 Load2	Dead Live	Full Area Full Area		Deal o		20.00 40.00		psf psf

Maximum Reactions (lbs) and Support Bearing (in):



Unfactored: Dead Live	156 313		156 313
Factored: Total	664		664
Bearing:			
Capacity			1893
Joist	1865		7744
Support	3981		
Des ratio Joist	0.36	•	0.35
Support	0.17		0.09
Load case	#2		#2 4-3/8
Length	2-3/8	·	1-3/4
Min req'd	1-3/4		No
Stiffener	No 1.00		1.00
KD KB support			-
fcp sup	769		769
Kzcp sup		to the North declar included	

Bearing for wall supports is perpendicular-to-grain bearing on top plate. No stud design included.

Nordic Joist 9-1/2" NI-40x Floor joist @ 12" o.c.

Supports: All - Lumber Wall, No.1/No.2

Total length: 16' 5/8"; Clear span: 15' 5-7/8"; 5/8" nailed and glued OSB sheathing with 1/2" gypsum ceiling
This section PASSES the design code check.

Limit States Design using CSA-086-09 and Vibration Criterion:

	· · · · · · · · · · · · · · · · · · ·			
Criterion	Analysis Value	Design Value	Unit	Analysis/Design
	Vf = 664	Vr = 1895	lbs	$v_f/v_r = 0.35$
Shear	· · · · · · · · · · · · · · · · · · ·	T-11		$a0^{\text{FESS}}M^{\text{FWM}} = 0.54$
Moment(+)	Mf = 2597	Mr = 4824	lbs-ft	- MANUAL TO THE REAL PROPERTY OF THE PARTY O
	0.12 = < L/999	0.52 = L/360	in /a	0.22
Perm. Defl'n			in /S/	0.59
Live Defl'n	0.23 = L/808			3 45
Total Defl'n	0.35 = L/539	0.78 = L/240	in 🖁 🖛	0.45
		0.52 = L/360	in C c	MATSOULAKOS \$ 0.52
Bare Defl'n	0,2, , _,		Till 3.	M13000
Vibration	$I_{\text{max}} = 15 - 7.6$	$L_{V} = 16'-8.5$	ft 🕍 💆 🖰	0.94
. —	= 0.034	= 0.041	lin 🖁 🧬	0.81
Defl'n			100	N 12 1

OWO NO.YAM 5460-20 Structural Component only

WoodWorks® Sizer

for NORDIC STRUCTURES

J1 2ND FLOOR.wwb

Nordic Sizer - Canada 7.2

Page 2

, 1115 , 15	•••••									
Additiona	al Data:							****	* C.II	
FACTORS:	f/E	KD	KH	KZ	KL	KT	KS	KN	LC#	
۷r	1895	1.00	1.00	-	-	-	-	-	#2	
Mr+	4824	1.00	1.00		1.000	-	-	-	#2	
EI	218.1 m	illion		-	-	-			#2	
CRITICAL L	OAD COMB		3:							
Shear	: LC #2	= 1.2	5D + 1.5L							
Moment (1	-) : LC #2	= 1.2	5D + 1.5L							
Deflecti	on: LC #1	= 1.0	D (perma:	nent)						
2022000	LC #2	= 1.00	D + 1.0L	(live)					
	LC #2	= 1.0	D + 1.0L	(tota	.1)					
	T.C #2	= 1.0	D + 1.0L	(bare	joist)					
Bearing	: Suppo	rt 1 - 3	LC #2 = 1	.25D +	1.5L					
-	Suppo	rt 2 ~	I.C. #2 = 1	.25D +	1.5L					
Load Tyr	oe• D≕dea	d W=win	nd S≕sno	w H=e	arth, grou	ındwate:	r E=ear	thquake		
	T.=liv	e (use. o	ccupancy)	Ls=1	ive(stora	ıge, equ:	ipment)	f=fire		
Load Pat	terns: s=	s/2 L=	L+Ls =n	o patt	ern load	in this	s span			
All Load	d Combinat	ions (L	Cs) are l	isted	in the An	alysis	output			
CALCULAT	IONS:		•					e a	Neabus To	000 004
FIoff =	258 29 15	-in^2	K= 4.94e	06 lbs					informs to	088 5013
"Live" o	deflection	is due	to all n	on-dea	d loads (live, v	wind, sn	ow)	AMFUNEN	2820

Design Notes:

- 1. WoodWorks analysis and design are in accordance with the 2010 National Building Code of Canada (NBC), Division B, Part 4, and the CSA O86-09 Engineering Design in Wood standard, which includes Update No.1
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to Nordic Structures technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.



OWS NO. TAN 5460 -20 STRUCTURAL COMPONENT ONLY





PASSED

1ST FLR FRAMING\Flush Beams\B1(i1675) (Flush Beam)

Dry | 1 span | No cant.

February 13, 2020 14:51:27

BC CALC® Member Report Build 7239

Job name: Address:

City, Province, Postal Code: WATERDOWN

File name:

VALLEYCREEK 5 EL 2.mmdl

1ST FLR FRAMING\Flush Beams\B1(i1675) Description:

Wind

Specifier:

Designer: ΑJ

Customer: Code reports:

CCMC 12472-R

Company:

12-11-04	1			1	1	1	Ţ	-	Ţ	-	+	+	ŧ	¥		ı J	+	Ł	Ţ	T	Ţ	¥	Ŧ	Ţ	Ţ	Ţ	ţ	¥	¥	¥
		Ť	士	ţ	Ť	Ť	Ť	Ţ	¥	Ţ	¥	Ţ	¥	Ţ	Ţ () ↓	¥	¥	Ł	1	¥	¥	Ŧ	•	ţ	<u>+</u>	<u>+</u>	<u>+</u>	1	<u> </u>
						-														-										
							···												-											
															42.4	4.04														

Total Horizontal Product Length = 12-11-04

Snow

Reaction Summary (Down / Uplift) (lbs)

Live Bearing 118/0 B1, 1-7/8" 175/0 122 / 0 B2, 4-3/8" 181 / 0

	- d Crasses and						Live	Dead	Snow	Wind	Tributary
LO: Tag	ad Summary Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
nag	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	12-11-04	Тор		5			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	12-11-04	Тор	28	14			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1267 ft-lbs	11610 ft-lbs	10.9%	1	06-04-06
End Shear	349 lbs	5785 lbs	6.0%	1	00-11-06
Total Load Deflection	L/999 (0.103")	n\a	n\a	4	06-04-06
Live Load Deflection	L/999 (0.061")	n\a	n\a	5	06-04-06
Max Deft.	0.103"	n\a	n\a	4	06-04-06
Span / Denth	15.8				

В	earing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B'	1 Wall/Plate	1-7/8" x 1-3/4" 4-3/8" x 1-3/4"	410 lbs 424 lbs	20.3% 9.0%	10.2% 4.5%	Spruce-Pine-Fir Spruce-Pine-Fir

OVINCE OF ON

846 HO. FAN 5461 -20 STRUCTURAL COMPONENT ONLY

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live toad deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86. AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86. Resistance Factor phi has been applied to all presented results per CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Disclosure

Use of the Boise Cascade Software is CONFORMS TO OBC 2012 subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Bolse Cascade engineered wood products must be in accordance with current installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

> BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAMIM, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





PASSED

1ST FLR FRAMING\Flush Beams\B1A(I1683) (Flush Beam)

Dry | 1 span | No cant.

February 13, 2020 14:51:27

BC CALC® Member Report

Build 7239

Job name:

Address:

Code reports:

City, Province, Postal Code: WATERDOWN

Customer:

CCMC 12472-R

File name:

VALLEYCREEK 5 EL 2.mmdl

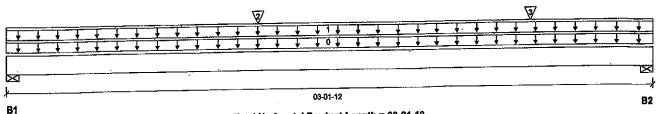
1ST FLR FRAMING\Flush Beams\B1A(i1683)

Description:

Specifier:

Designer: ΑJ

Company:



Total Horizontal Product Length = 03-01-12

Snow

Reaction Summary (Down / Uplift) (lbs)

Dead Live Bearing 552 / 0 824 / 0 B1, 3-3/16" 660 / 0 1037 / 0 B2, 3-7/16"

	l O						Live	Dead	Snow	Wind	Tributary
	ad Summary	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
Tag		Unf. Lin. (lb/ft)	L	00-00-00	03-01-12	Тор		10			00-00-00
0	Self-Weight	Unf. Lin. (lb/ft)	· ī	00-00-00	03-01-12	Top	321	241			n\a
7	E6(i389)	Conc. Pt. (lbs)	ī	01-02-12	01-02-12	•	425	212		. مر	n\a
2	J2(i1660)	, ,	ī	02-06-12	02-06-12	•	425	212	Carrie Col	ESSIO,	îv.∵∵ n\a
3	J2(i1710)	Conc. Pt. (lbs)	-	02-00-12	02,00 12	. •••			& SHOW	Common and	M. C.

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1389 ft-lbs	23220 ft-lbs	6.0%	1	01-03-06
	1465 lbs	11571 lbs	12.7%	1	01-00-11
End Shear	L/999 (0.003")	n\a	n\ a	4	01-06-08
Total Load Deflection	•	n\a	n\a	5	01-06-08
Live Load Deflection Max Defl.	L/999 (0.002") 0.003"	n\a	n\a	4	01-06-08
Span / Depth	3.4				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1		3-3/16" x 3-1/2"	1926 lbs	27.8%	14.0%	Spruce-Pine-Fir
B2		3-7/16" x 3-1/2"	2380 lbs	32.2%	16.3%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

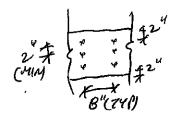
Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



PROVINE 3 ROWS OF 3½" ARDOX SPIRAL NAILS @ & "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN. 24 LUMBER EDGE/END DISTANCE, DO NOT USE AIR NAILS

THE HO. TAM 5462-28 STRUCTURAL COMPONENT ONLY

POVINCE OF

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). COMPleteness and accuracy of input CONFORMS TO OBC 2012 must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain installation Guide or ask questions, please call (800)232-0788 before installation.

> BC CALC®, BC FRAMER® , AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



Dry | 1 span | No cant.

PASSED

February 13, 2020 14:51:27

1ST FLR FRAMING\Flush Beams\B1B(i1738) (Flush Beam)

BC CALC® Member Report

Build 7239

Job name:

Address:

City, Province, Postal Code: WATERDOWN

File name:

VALLEYCREEK 5 EL 2.mmdl

Wind

ΑJ

Description: 1ST FLR FRAMING\Flush Beams\B1B(i1738)

Specifier:

Designer:

Customer: Code reports:

CCMC 12472-R

Company:

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																									<u> </u>	•	_

Total Horizontal Product Length = 03-02-00

Snow

Reaction Summary (Down / Uplift) (lbs)

Live Bearing 191/0 79/0 B1, 3-7/16" 79/0 191 / 0 B2, 3-7/16"

	ad Cummans						Live	Dead	Snow	Wind	Tributary
	oad Summary Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-02-00	Top		10			00-00-00
U		· · · · · · · · · · · · · · · · · · ·	ī	00-00-00	03-02-00	αοΤ	28	100			n\a
1	E1(l390)	Unf. Lin. (lb/ft)	<u>.</u>			1		44			n\a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	03-02-00	Тор	22	11			11/0

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	156 ft-lbs	15093 ft-lbs	1.0%	0	01-07-00
End Shear	85 lbs	7521 lbs	1,1%	0	01-00-15
Total Load Deflection	L/999 (0")	n\a	ˈn\a	4	01-07-00
Live Load Deflection	L/999 (0")	n\a	n\a	5	01-07-00
Max Defl.	0"	n\a	n\a	4	01-07-00
Span / Depth	3.4				

	Rearing	s Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
-	B1	Wall/Plate	3-7/16" x 3-1/2"	267 lbs	5.6%	2.8%	Spruce-Pine-Fir
	B2	\\/all/Plate	3-7/16" x 3-1/2"	267 lbs	5.6%	2.8%	Spruce-Pine-Fir

COMPONENT ONLY

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Notes

Design meets Code minimum (L/240) Total load deflection criteria.

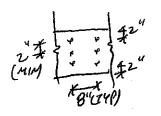
Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86. AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



PROVIDE3 ROWS OF 3½" ARDOX SPIRAL NAILS @ 8 "O/C FOR MULTI-PLY NATLING, MAINTAIN A MIN.2"LUMBER EDGE/END DISTANCE, BO NOT USE AIR NAILS



0W6 NO. TAN 5463 -28 STRUCTURĂL

Disclosure





PASSED

B2

1ST FLR FRAMING\Flush Beams\B2(I1694) (Flush Beam)

Dry | 1 span | No cant.

February 13, 2020 14:51:27

BC CALC® Member Report Build 7239

Job name:

Address:

City, Province, Postal Code: WATERDOWN

File name:

VALLEYCREEK 5 EL 2.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B2(i1694)

Specifier:

Designer: ΑJ

Customer: Code reports:

CCMC 12472-R

Company:

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В1

10-05-00 Total Horizontal Product Length = 10-05-00

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing Live Dead 507 / 0 73/0 B1, 1-3/4" 479 / 0 B2, 1-7/8" 73/0

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L.	00-00-00	10-05-00	Тор		10			00-00-00 .
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	10-05-00	Top	14	7			n\a
2	11(i450)	Unf. Lin. (lb/ft)	L	00-00-00	10-00-10	Top		81			n\a
2	11(1400)	Offic Entry (1871)	-			•			يتعلقكاد	OFESS	10MA

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1787 ft-lbs	15093 ft-lbs	11.8%	0	05-02-07
End Shear	649 lbs	7521 lbs	8.6%	0	09-05-10
Total Load Deflection	L/999 (0.055")	n\a	n\a	4	05-02-07
Live Load Deflection	L/999 (0.007")	n\a	n\a	5	05-02-07
Max Defl.	0.055"	n\a	n\a	4	05-02-07
Span / Depth	12.9				

Bearing Supports		Dim. (LxW) Demand		Demand/ Resistance Support	Demand/ Resistance Member	Material	
B1	Column	1-3/4" x 3-1/2"	710 lbs	22.0%	14.6%	Unspecified	
B2	Wail/Plate	1-7/8" x 3-1/2"	671 lbs	25.6%	12.9%	Spruce-Pine-Fir	

DWG NO. TAM*5464* -20 COMPONENT ONLY

NCE OF

Notes

1.4

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

AMENDED 2020 Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

PROVIDE3 ROWS OF 3½" ARDOX SPIRAL NAILS @ 12 " 0/C FOR MULTI-PLY NAILING, MAINTAIN A MIN.2" LUMBER EDGE/END DISTANCE. DO NOT USE AIR NAILS

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User CONFORMS TO OBC 2012 License Agreement (EULA). must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of sultability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

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PASSED

February 13, 2020 14:51:27

1ST FLR FRAMING\Flush Beams\B3(i1954) (Flush Beam)

BC CALC® Member Report

Build 7239 Job name:

Address: City, Province, Postal Code: WATERDOWN

Customer: Code reports:

CCMC 12472-R

Dry | 1 span | No cant.

File name:

VALLEYCREEK 5 EL 2.mmdl

1ST FLR FRAMING\Flush Beams\B3(i1954) Description:

Specifier:

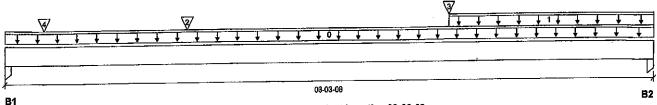
Designer:

AJ

Wind

CONFORMS TO OBC 2012

Company:



Total Horizontal Product Length = 03-03-08

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead
B1, 3-1/2"	141 / 0	81/0
B2, 3-1/2"	147 / 0	81/0

Los	ad Summary						LIVE	Dead	Show	AAIIIM	Imutaly
Tag		Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-03-08	Тор		5			00-00-00
4	FC2 Floor Material	Unf. Lin. (lb/ft)	L	02-03-02	03-03-08	Top	33	17			n\a
1		Conc. Pt. (lbs)	ī	00-11-02	00-11-02	του	135	67			n\a
2	J7(i1800)	Conc. Pt. (lbs)	ī	02-03-02	02-03-02	Top	116	58	1	CCC1/	n\a
3	J7(i1752)	• •	<u>-</u>	00-02-06	00-02-06	Τορ		3	A COR	le cool	n\a
4	11(i450)	Conc. Pt. (lbs)	L	00-02-00	00-02-00	ιυp		•	1 0 A		Sign of the state
			Englared	Dem	andl			j	810	162	0 N ON

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	220 ft-lbs	11610 ft-lbs	1.9%	1	02-03-02
End Shear	279 ibs	5785 lbs	4.8%	1	02-02-08
Total Load Deflection	L/999 (0.001")	n\a	n\a	4	01-07-12
Live Load Deflection	L/999 (0.001")	n\a	n\a	5	01-07-12
Max Defl.	0.001"	n\a	n\a	4	01-07-12
Span / Depth	3.6				

	Paaring	ı Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
•	B1	Column	3-1/2" x 1-3/4"	312 lbs	6,3%	4.2%	Unspecified
	B2	Column	3-1/2" x 1-3/4"	323 lbs	6.5%	4.3%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086. AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

OVINCE OF ON DWS NO. TAM SYGS STRUCTURAL COMPONENT ONLY

Mind

Disclosure

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PASSED

1ST FLR FRAMING\Flush Beams\B5(i1693) (Flush Beam)

Dry | 1 span | No cant.

February 13, 2020 14:51:27

BC CALC® Member Report

Build 7239 Job name:

Address:

City, Province, Postal Code: WATERDOWN

Customer:

Code reports:

CCMC 12472-R

File name:

VALLEYCREEK 5 EL 2.mmdl

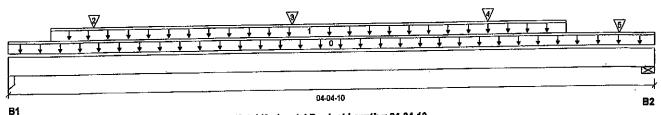
1ST FLR FRAMING\Flush Beams\B5(i1693)

Wind

Description: Specifier:

Designer: ΑJ

Company:



Total Horizontal Product Length = 04-04-10

Snow

Reaction Summary (Down / Uplift) (lbs)

Meaction Can	milery (months)	Dood		
Bearing	Live	Dead		
B1, 3-1/2"	410/0	215/0		
B2 5-1/2"	502 / 0	1155 / 0		

	l O						LIVE	pead	Snow
	ad Summary	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00
	Description	Unf. Lin. (lb/ft)	L	00-00-00	04-04-10	Top		5	
0	Self-Weight	Unf. Lin. (lb/ft)	ī	00-03-08	03-09-10	Top	120	60	
1	STAIR	• •	ī	00-07-02	00-07-02	•	118	59	
2	J7(i1525)	Conc. Pt. (lbs)	-	01-11-02	01-11-02		118	59	
3	J7(i15 2 5)	Conc. Pt. (lbs)	<u>ا</u>			-	108	54	AND S
4	J7(i1534)	Conc. Pt. (lbs)	L	03-03-02	03-03-02	•			PROPOS
5	7(i399)	Conc. Pt. (lbs)	L	04-01-14	04-01-14	Тор	147	966	10

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
	814 ft-lbs	11610 ft-lbs	7.0%	1	01-11-02
Pos. Moment	725 lbs	5785 lbs	12.5%	1	03-01-10
End Shear	L/999 (0.006")	n\a	n\a	4	02-01-06
Total Load Deflection	•	n/a	n\a	5	02-01-06
Live Load Deflection	L/999 (0.004")	n\a n\a	n\a	4	02-01-06
Max Defl.	0.006"	IINA	ITICA	•	
Span / Depth	4.7				

Bearing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1 Column B2 Wall/Plate	3-1/2" x 1-3/4"	884 lbs	17.8%	11.8%	Unspecified
	5-1/2" x 1-3/4"	1617 lbs	42.0%	21.2%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

COMPORMS TO OBC 2012

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



Wind

1.15

Tributary

00-00-00

OWG NO. TAM 5466 -20 STRUCTURAL COMPONENT ONLY

Disclosure

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PASSED

1ST FLR FRAMING\Flush Beams\B6(i1676) (Flush Beam)

Dry | 1 span | No cant.

February 13, 2020 14:51:27

BC CALC® Member Report

Build 7239 Job name:

Customer:

Code reports:

Address:

City, Province, Postal Code: WATERDOWN

CCMC 12472-R

File name:

VALLEYCREEK 5 EL 2.mmdl

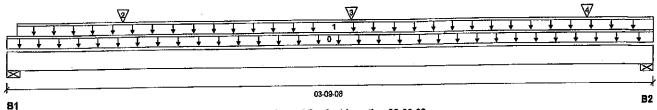
1ST FLR FRAMING\Flush Beams\B6(i1676) Description:

Wind

Specifier:

Designer: ΑJ

Company:



Total Horizontal Product Length = 03-09-08

Snow

Reaction Summary (Down / Uplift) (lbs)

Live Dead Bearing 179/0 340 / 0 B1, 2" 808 / 0 470 / 0 B2, 5-1/2"

La	ad Qummani						Live	Dead	Snow	Wind	Tributary
Tag	ad Summary Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
<u>π.υ.</u>	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-09-08	Тор		5			00-00-00
1	STAIR	Unf. Lin. (lb/ft)	L	00-00-11	03-09-08	Top	120	60			n\a
,	J7(i1698)	Conc. Pt. (lbs)	1	00-08-04	00-08-04	Top	100	50			n\a
2		Conc. Pt. (lbs)	ī	02-00-04	02-00-04	Top	125	63	Sept. 22.57	ESSIO.	n\a
3	J7(i1664)	• •	ī	03-05-00	03-05-00	Top	467	289	APPORTON OF	ESSIO/	n\a
4	-	Conc. Pt. (lbs)	_	05-05-00	00-00-00	TOP	,,,,	-00	O RESERVE	40	1. 18

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	613 ft-lbs	11610 ft-lbs	5.3%	1	02-00-04
End Shear	425 lbs	5785 lbs	7.3%	1	00-11-08
Total Load Deflection	L/999 (0.003")	n\a	n\a	4	01-09-01
Live Load Deflection	⊥/999 (0.002")	n\a	n\a	5	01-09-01
Max Defl. Span / Depth	0.003" 4.2	n\a	n\a	4	01-09-01

Bearing	Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	2" x 1-3/4"	734 lbs	34.1%	17.2%	Spruce-Pine-Fir
B2	Wall/Plate	5-1/2" x 1-3/4"	1800 lbs	30.4%	15.3%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

AMENDED 2020 Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

DWG NO. TAM 5467-20 STRUCTURAL COMPONENT ONLY

OLINCE OF ONTRE

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PASSED

1ST FLR FRAMING\Flush Beams\B7(I1667) (Flush Beam)

Dry | 1 span | No cant.

February 13, 2020 14:51:27

Wind

ROFESSION NO

POLINCE OF ONT DWG NO. TAM 5468 -20 STRUCTURAL

Tributary

00-00-00

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Snow

Build 7239

Job name:

Address:

BC CALC® Member Report

City, Province, Postal Code: WATERDOWN

Customer: Code reports:

CCMC 12472-R

File name:

VALLEYCREEK 5 EL 2.mmdl

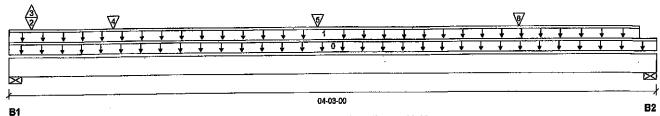
1ST FLR FRAMING\Flush Beams\B7(i1667) Description:

Wind

Specifier:

Designer: ΑJ

Company:



Total Horizontal Product Length = 04-03-00

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead
B1, 5-1/2"	533 / 3	277 / 0
B2, 3-1/2"	409 / 0	215/0

Lo	ad Summary						Live	Dead
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	04-03-00	Top		5
1	STAIR	Unf, Lin, (lb/ft)	L	00-00-00	04-01-10	Тор	120	60
2	J5(i1703)	Conc. Pt; (lbs)	L	00-01-12	00-01-12	Тор	70	34
3	J5(i1703)	Conc. Pt. (lbs)	L	00-01-12	00-01-12	Тор	-3	
4	J7(i1707)	Conc. Pt. (lbs)	L	00-08-04	00-08-04	Top	101	51
5	J7(i1661)	Conc. Pt. (lbs)	L	02-00-04	02-00-04	Top	145	73
6	J7(i1670)	Conc. Pt. (lbs)	Ĺ.	03-04-04	03-04-04	Тор	126	63 🖊

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	816 ft-lbs	11610 ft-lbs	7.0%	1	02-00-04
End Shear	565 lbs	5785 lbs	9.8%	1	03-02-00
Total Load Deflection	L/999 (0.005")	n\a	n\a	6	02-02-06
Live Load Deflection	L/999 (0.003")	n\a	n\a	8	02-02-06
Max Defl.	0.005"	n\a	n\a	6	02-02-06
Snan / Denth	4.6				

Beari	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	5-1/2" x 1-3/4"	1145 lbs	19.3%	9.8%	Spruce-Pine-Fir
B2	Wall/Plate	3-1/2" x 1-3/4"	883 lbs	23.4%	11.8%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

AMENDED 2020 Resistance Factor phi has been applied to all presented results per CSA 086. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

CONFORMS TO OBC 2012

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PASSED

February 13, 2020 14:51:27

2ND FLR FRAMING\Dropped Beams\B11 DR(i1959) (Dropped Beam)

BC CALC® Member Report

Build 7239

Job name:

Customer:

Address:

Code reports:

City, Province, Postal Code: WATERDOWN

CCMC 12472-R

Dry | 1 span | No cant.

VALLEYCREEK 5 EL 2.mmdi

File name: Description: 2ND FLR FRAMING\Dropped Beams\B11 DR(i1959)

Wind

Specifier:

Designer: AJ

Company:

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<i>}</i>					-		11-0											В	2

Total Horizontal Product Length = 11-02-00

Snow

Reaction Summary (Down / Uplift) (Ibs)

Dead Bearing Live 1703/0 3250/0 B1, 4" 1923 / 0 3690 / 0 B2, 4"

	and Commons						Live	Dead	Snow	Wind	Tributary
	ad Summary	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
Tag		Unf. Lin. (lb/ft)	1	00-00-00	11-02-00	Top		14			00-00-00
U	Self-Weight	•		00-04-14	10-04-14		629	314			n\a
1	Smoothed Load	Unf. Lin. (lb/ft)	L.								n\a
2	_	Conc. Pt. (lbs)	L	10-10-14	10-10-14	• Тор	650	325			11/2

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	19114 ft-lbs	36222 ft-lbs	52.8%	1	05-10-14
End Shear	6614 lbs	17356 lbs	38,1%	1	01-01-08
Total Load Deflection	L/349 (0.365")	n\a	68.7%	4	05-06-06
	L/531 (0.24")	n\a	67.7%	5	05-06-06
Live Load Deflection Max Defl.	0.365"	n\a	n\a _.	4	05-06-06
Span / Depth	13.4				

Bearing	ı Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Resistance Membe <u>r</u>	Material
B1	Wall/Plate	4" x 5-1/4"	7004 lbs	25.0%	27.3%	Spruce-Pine-Fir
B2	Wall/Plate	4" x 5-1/4"	7939 lbs	28.3%	31.0%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBC 2012

Calculations assume unbraced length of Top: 00-03-02, Bottom: 00-03-02.

AMENDED 2020 Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

PROVIDE3 ROWS OF 3½" AROOX SPIRAL HAILS @ /2 "O/C FOR MULTI-PLY NAILING, MAINTAIN MIN. 2" LUMBER EDGE/END DISTANCE, DO NOT USE AIR HAILS STAGGER NAILS GO BETWEEN PLIES.



DWG NO. PAM 5469 -20 STRUCTURAL COMPONENT ONLY

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of sultability for a particular application. The output here is based on building code-accepted design properties and analysis methods. installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Gulde or ask questions, please call (800)232-0788 before Installation.

BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





PASSED

2ND FLR FRAMING\Flush Beams\B12(i1747) (Flush Beam)

Dry | 1 span | No cant.

February 13, 2020 14:51:27

Build 7239

Job name:

Address:

BC CALC® Member Report

City, Province, Postal Code: WATERDOWN

Customer:

Code reports:

CCMC 12472-R

File name:

VALLEYCREEK 5 EL 2.mmdl

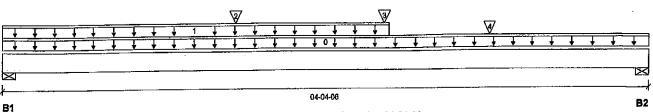
2ND FLR FRAMING\Flush Beams\B12(i1747) Description:

Wind

Specifier:

Designer: ΑJ

Company:



Total Horizontal Product Length = 04-04-06

Snow

Reaction Summary (Down / Uplift) (lbs)

Live Dead Bearing 779/0 1520./ 0 B1, 4-7/8" 1069 / 0 555/0 B2, 5-1/2"

1.	ad Summary						Live	Dead	Snow	Wind	Tributary
Ta		Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	04-04-06	Top		10			00-00-00
1	Smoothed Load	Unf. Lin. (lb/ft)	L.	00-00-00	02-07-04	Top	506	- 253			n\a
ż	J2(i1886)	Conc. Pt. (lbs)	L	01-06-14	01-06-14	Top	311	155			n\a
2	J2(11946)	Conc. Pt. (lbs)	Ĺ	02-06-14	02-06-14	Top	311	155		. مدوعه المغموري	n\a
3	J2(11840)	Conc. Pt. (lbs)	ī	03-03-08	03-03-08	Top	649	325	ALLEST AND	FESSIO	n\a
4	-	COIIC. Ft. (109)	-	00 00 00	00 00 00	יקטי	• • • •		POPORO		Red Ar

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	2308 ft-lbs	23220 ft-lbs	9.9%	1	02-01-04
End Shear	1963 lbs	11571 lbs	17.0%	1	03-01-06
Total Load Deflection	L/999 (0.008")	n\a	n\a	4.	02-01-15
Live Load Deflection	L/999 (0.005")	n\a	n\a	5	02-01-15
Max Defl.	0.008"	n\a	n\a	4	02-01-15
Span / Depth	4.6				

Bearing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1 Wall/Plate B2 Wall/Plate	4-7/8" x 3-1/2"	3254 lbs	31.0%	15.6%	Spruce-Pine-Fir
	5-1/2" x 3-1/2"	2297 lbs	19.4%	9.8%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBC 2012

Calculations assume member is fully braced.

AMENDED 2020 Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

PROVIDE 3 ROWS OF 31/2" ARDOX SPIRAL NAILS @ 6 " 0/C FOR MULTI-PLY HAILING, MAINTAIN A MIN. 2" LUMBER EDGE/END DISTANCE, DO NOT USE AIR NAILS

146 HO . PAN 5470 -20 STRUCTURAL COMPONENT ONLY Disclosure

NOE OF ONLY

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Bolse Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain installation Guide or ask questions, please call (800)232-0788 before installation.

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PASSED

February 13, 2020 14:51:27

2ND FLR FRAMING\Flush Beams\B14(i1681) (Flush Beam)

BC CALC® Member Report

Build 7239

Job name:

Address:

City, Province, Postal Code: WATERDOWN

Customer: Code reports:

CCMC 12472-R

Dry | 1 span | No cant.

File name:

VALLEYCREEK 5 EL 2.mmdl

Description:

2ND FLR FRAMING\Flush Beams\B14(i1681)

Specifier:

Designer:

AJ

Company:

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≤1																											
R1	-										09	-05-08															8

Total Horizontal Product Length = 09-05-08

Snow

Reaction Summary (Down / Uplift) (lbs)

Dead Live Bearing 304/0 206/0 B1, 4" 250 / 0 383/0 B2, 5-1/2"

١.	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	09-05-08	Тор		10			00-00-00
1	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	05-03-02	Тор	24	12			n\a
2	FC3 Floor Material	Unf. Lin. (lb/ft)	L	05-03-02	09-05-08	Top	27	13			n\a
3	R16/i1696)	Conc. Pt. (lbs)	Ĺ	05-04-00	05-04-00	Тор	451	247			" n\a

Controls Summary	Factored Demand	Factored Resistance	Demand <i>i</i> Resistance	Case	Location
Pos. Moment	2729 ft-lbs	23220 ft-lbs	11.8%	1	05-04-00
End Shear	801 lbs	11571 lbs	6.9%	1	08-02-08
Total Load Deflection	L/999 (0.046")	n\a	n\a	4	04-10-07
Live Load Deflection	L/999 (0.028")	n\a	n\a	5	04-10-07
Max Defl.	0.046"	n\a	n\a	4	04-10-07
Span / Depth	11.1	,			

Bear	ring Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	4" x 3-1/2"	713 lbs	8.3%	4.2%	Spruce-Pine-Fir
B2	Wall/Plate	5-1/2" x 3-1/2"	887 lbs	7.5%	3.8%	Spruce-Pine-Fir

Notes

. ja (37)

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-00-00. Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

AMENDED 2020

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

PROVIDE 3 ROWS OF 3½" ARDOX SPIRAL HAILS @ /2-"O/C FOR MULTI-PLY HAILING, MAINTAIN A MIN. 2"LUMBER EDGE/END DISTANCE. DO NOTUSE AIR NAILS



DWG NO. TAMSY2/ -20 STRUCTURAL COMPONENT ONLY

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input CONFORMS TO OBC 2012 must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of sultability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Bolse Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

> BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





PASSED

February 13, 2020 14:51:27

2ND FLR FRAMING\Flush Beams\B15(i1680) (Flush Beam)

BC CALC® Member Report

Build 7239

Job name:

Address:

City, Province, Postal Code: WATERDOWN

Customer: Code reports:

CCMC 12472-R

Dry | 1 span | No cant.

File name:

VALLEYCREEK 5 EL 2.mmdl

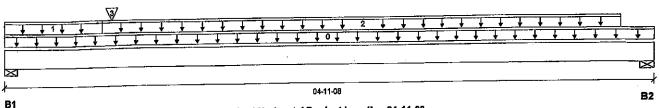
2ND FLR FRAMING\Flush Beams\B15(i1680) Description:

AJ

Specifier:

Designer:

Company:



Total Horizontal Product Length = 04-11-08

Snow

Reaction Summary (Down / Uplift) (lbs)

Dead Live Bearing 377/0 670 / 0 B1. 4" 110 / 0 166 / 0 B2. 5-1/2"

	- 10						Live	Dead	Snow	Wind	Tributary
	ad Summary Description	Load Type	Ref.	Start	End	Loc,	1.00	0.65	1.00	1.15	
Tag	Self-Weight	Unf. Lin. (lb/ft)		00-00-00	04-11-08	Top		10			00-00-00
v		Unf. Lin. (lb/ft)	ī	00-00-00	00-09-02	Top	30	15			n\a
1	FC3 Floor Material	• •	ī	00-09-02	04-08-12	Top	33	17			n\a
2	FC3 Floor Material	Unf. Lin. (lb/ft)	L .	•• ••		Top	681	362			n\a
3	B16(i1696)	Conc. Pt. (lbs)	L	00-10-00	00-10-00	ιυβ	QÐ I	JUZ	me.	್ಕಾಟಕಾಗಿ	

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	805 ft-lbs	23220 ft-lbs	3.5%	1	00-10-00
	898 lbs	11571 lbs	7.8%	1	01-01-08
End Shear	L/999 (0.003")	n\a	n\a	4	02-02-09
Total Load Deflection Live Load Deflection	L/999 (0.002")	n\a	n\a	5	02-02-02
Max Defl.	0.003"	n\a	n\a	4	02-02-09
Span / Depth	5.4				

Regrine	g Supports	Dim. (LxW)	Demand	Demand <i>i</i> Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	4" x 3-1/2"	1476 lbs	17.1%	8.6%	Spruce-Pine-Fir
B2		5-1/2" x 3-1/2"	387 lbs	3.3%	1.6%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criterla.

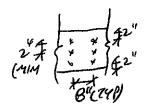
CONFORMS TO OBC 2012

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86. AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



PROVIDE3 ROWS OF 31/2" ARDOX SPIRAL NAILS @ 8 "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN. 2" LUMBER EDGE/END DISTANCE. BUNOT USE AIR HAILS



OWG NO. TAM5472-20 STRUCTÚRAL COMPONENT ONLY <u>Disclosure</u>

Use of the Bolse Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. installation of Boise Cascade engineered wood products must be in accordance with current installation Guide and applicable building codes. To obtain installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





PASSED

2ND FLR FRAMING\Flush Beams\B16(i1696) (Flush Beam)

Dry | 1 span | No cant.

February 13, 2020 14:51:27

BC CALC® Member Report Build 7239

Job name:

Address:

City, Province, Postal Code: WATERDOWN

Customer:

Code reports:

CCMC 12472-R

File name:

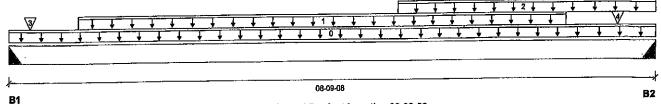
VALLEYCREEK 5 EL 2.mmdl

2ND FLR FRAMING\Flush Beams\B16(i1696) Description:

Specifier:

Designer: ΑJ

Company:



Total Horizontal Product Length = 08-09-08

Snow

Reaction Summary (Down / Uplift) (lbs)

Live Dead Bearing 244/0 B1, 2" 445 / 0 687 / 0 365/0 B2, 2"

1.00	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
n	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-09-08	Top		5			00-00-00
1	Smoothed Load	Unf. Lin. (lb/ft)	L	00-11-08	07-07-08	Top	83	41			n\a
2	STAIR	Unf. Lin. (lb/ft)	L	05-03-08	08-09-08	Top	120	60			n\a
2		Conc. Pt. (lbs)	Ī	00-03-08	00-03-08	Top	81	41			n\a
3	J6(i1673)	Conc. Pt. (ibs)	ī	08-03-08	08-03-08	Top	81	41	- Table	FESS/	n\a
4	J6(i1715)	COHO, Ft. (IDS)		00-00-00	00 00 00	. op	• •		3/50-45	んについげ	J/31 'O.

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	2473 ft-lbs	11610 ft-lbs	21.3%	1	05-07-08
End Shear	1136 lbs	5785 lbs	19.6%	1	07-10-00
Total Load Deflection	L/999 (0.092")	n\a	n\a	4	04-06-08
Live Load Deflection	1/999 (0.06")	n\a	n\a	5	04-06-08
Max Defl.	0.092"	n\a	n\a	4	04-06-08
Span / Depth	10.8				

Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Suppo <u>rt</u>	Demand/ Resistance Member	Material
B1	Hanger	2" x 1-3/4"	973 lbs	n\a	22.8%	HUS1.81/10
B2	Hanger	2" x 1-3/4"	1487 lbs	n\a	34.8%	HUS1.81/10

Cautions

Header for the hanger HUS1.81/10 at B1 is a Double 1-3/4" x 9-1/2" VERSA-LAM® 1.7 2400 DF. Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

Header for the hanger HUS1.81/10 at B2 is a Double 1-3/4" x 9-1/2" VERSA-LAM® 1.7 2400 DF.

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

COMPORMS TO OBC 2012

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86. AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

DWG HO. FAM 5473 -20 STRUCTURAL DISCHOULT ONLY

OVINCE OF

Use of the Bolse Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of sultability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

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PASSED

February 13, 2020 14:51:27

2ND FLR FRAMING\Flush Beams\B17(i1910) (Flush Beam)

BC CALC® Member Report

Build 7239

Job name:

Customer:

Code reports:

Address:

City, Province, Postal Code: WATERDOWN

CCMC 12472-R

Dry | 1 span | No cant.

VALLEYCREEK 5 EL 2.mmdl

File name: Description: 2ND FLR FRAMING\Flush Beams\B17(i1910)

Wind

Specifier:

Designer: ΑJ

Company:

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		-			===					_			<u> </u>	 		_																

B1

Total Horizontal Product Length = 01-10-02

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow
B1. 5-1/4"	56 / 0	122 / 0	57/0
B2 5-1/2"	56 / 0	124 / 0	59/0

	ad Commons						Live	Dead	
Tag	ad Summary Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	01-10-02	Top		10	
1	ROOF	Unf, Lin. (lb/ft)	L	00-00-00	01-10-02	Top	33	30	- (
2	WALL	Unf. Lin. (lb/ft)	L	00-00-00	01-10-02	Тор		80	
_	FC3 Floor Material	Unf. Lin. (lb/ft)	1.	00-00-00	00-05-04	Top	30	15	
		Unf. Lin. (lb/ft)	ī	00-05-04	01-10-02	Top	27	14	
4	FC3 Floor Material	Otti. Eiti. (ib/it)	-	00 00 0	•	,-		ø	15

Controls Summary_	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	46 ft-lbs	23220 ft-lbs	0.2%	13	00-10-15
End Shear	102 lbs	11571 lbs	0.9%	1	01-02-12
Span / Depth	1.4				

Bearing	Supports	Dim. (LxW)	Demand	Resistance Support	Resistance Member	Material
B1	Beam	5-1/4" x 3-1/2"	295 lbs	3.0%	1.3%	Unspecified
B2	Mall/Plate	5-1/2" x 3-1/2"	300 lbs	2.5%	1.3%	Spruce-Pine-Fir

Notes

7222 0000

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86. Unbalanced snow loads determined from building geometry were used in selected product's COMPORMS TO UBC 2012 verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

AMENDED 2020

PROVICE 3 ROWS OF 31/2" ARDOX SPIRAL NAILS @ 6 "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN. Z"LUMBER EDGE/END DISTANCE. DO NOT USE AIR NAILS



Wind

1.15

Snow 1.00

63

Tributary

00-00-00

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Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Bolse Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™. ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





PASSED

1ST FLR FRAMING\Fiush Beams\B21(i2663) (Flush Beam)

Dry | 1 span | No cant.

March 24, 2020 16:40:28

BC CALC® Member Report Build 7239

Job name: Address:

Customer:

Code reports:

City, Province, Postal Code: WATERDOWN

CCMC 12472-R

Description:

VALLEYCREEK 5 EL 1.mmdl

File name: 1ST FLR FRAMING\Flush Beams\B21(i2663)

Specifier:

Designer: ΑJ

Company:

<u> </u>	 		+ + + + + + + +
	+ + + + + +		
		A Company of the Comp	

Total Horizontal Product Length = 03-01-00

Snow

Reaction Summary (Down / Uplift) (lbs)

Live Dead B1, 3" 186 / 0 77/0 186 / 0 B2, 3" 77/0

l۸	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	_	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L.	00-00-00	03-01-00	Тор		10			00-00-00
1	E5(i395)	Unf. Lin. (lb/ft)	L	00-00-00	03-01-00	Тор	28	100			n\a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	03-01-00	Тор	22	11			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	155 ft-lbs	15093 ft-lbs	1 .0 %	0	01-06-08
End Shear	84 lbs	7521 lbs	1.1%	0	01-00-08
Total Load Deflection	L/999 (0")	n\a	n\a	4	01-06-08
Live Load Deflection	L/999 (0")	n\a	n\a	5	01-06-08
Max Defl.	0"	n\a	n\a	4	01-06-08
Span / Depth	3.4				

Bearing	Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material	
B1	Wall/Plate	3" x 3-1/2"	260 lbs	6.2%	3.1%	Spruce-Pine-Fir	_
B2	Wall/Plate	3" x 3-1/2"	260 lbs	6.2%	3.1%	Spruce-Pine-Fir	

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

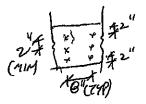
CONFORMS TO OBC 2012

Calculations assume member is fully braced.

AMENDED 2020 Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



PROVIDE3 ROWS OF 3½" ARDOX SPIRAL NAILS @8 "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN.2" LUMBER EDGE/END DISTANCE, DO NOT USE AIR HAILS



DWS NO. TAM 5425 -20 STRUCTURAL COMPONENT ONLY Disclosure

Use of the Bolse Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™. ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit, 3/4" OSB G&N Sheathing







			В	are		l	1/2" Gyp	sum Ceiling	
Depth	Series		On Centi	re Spacing			On Cent	re Spacing	
		12"	16"	19.2°	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-0"	16'-0"	15'-1"	13'-11"	17'-5"	16'-1"	15'-1"	13'-11"
9-1/2"	NI-60	17'-2"	16'-2"	15'-5"	14'-3"	17'-6"	16'-5"	15'-5"	14'-3"
	NI-70	18'-0"	16'-11"	16'-3"	15'-6"	18'-5"	17'-3"	16'-7"	15'-6"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	15'-10"
	NI-20	17'-10"	16'-10"	16'-0"	14'-10"	18'-6"	17'-1"	16'-0"	14'-10"
	NI-40x	19'-4"	17'-11"	17'-3"	15'-10"	19'-11"	18'-6"	17'-9"	15′-10"
44 7/00	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-1"
11-7/8"	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"
	Nt-80	21'-1"	19'-5"	18'- 6 "	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
	NI-40x	21'-5"	19'-10"	18'-11"	17'-5"	22'-1"	20'-6"	19'-6"	17'-5"
	NI-60	21'-10"	20'-2"	19'-3"	18 - 2"	22'-5"	20'-10"	19'-11"	18'-10"
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"
	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22'-9"	21'-6"
16"	N1-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

			Mid-Spar	n Blocking		Mid-S	pan Blocking ar	nd 1/2" Gypsum	Ceifing
Depth	Series		On Centr	re Spacing			On Cent	re Spacing	
D-104.		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-9"	16'-1"	15'-1"	13'-11"	17'-9"	16'-1"	15'-1"	13'-11'
9-1/2"	NI-60	18'-1"	16'-5"	15'-5"	14'-3"	18'-1"	16'-5"	15'-5"	14'-3"
· -,-	NI-70	19'-10"	17'-11"	16'-9"	15'-6"	19'-10"	17'-11"	16'-9"	15'-6"
	NI-80	20'-2"	18'-3"	17'-1"	15'-10"	20'-2"	18'-3"	17'-1"	15'-10
	NI-20	18'-10"	17'-1"	16'-0"	14'-10"	18'-10"	17'-1"	16'-0"	14'-10
	NI-40×	21'-3"	19'-3"	17'-9"	15'-10"	21'-3"	19'-3"	17'-9"	15 -10
	NI-60	21'-9"	19'-8"	18'-5"	17'-1"	21'-9"	19'-8"	18'-5"	17'-1"
11-7/8"	NI-70	23'-4"	21'-5"	20'-1"	18'-6"	231-8"	21'-5"	20'-1"	18'-6"
	NI-80	23'-7"	21'-10"	20'-5"	18'-11"	24'-1"	21'-10"	20'-5"	18-11
	NI-90x	24'-3"	22'-6"	21'-3"	19'-7"	24'-8"	22'-7" ·	21'-3"	19'-7"
	NI-40x	24'-2"	21'-5"	19'-6"	17'-5"	24'-2"	21'-5"	19'-6"	17'-5"
	NI-60	24'-9"	22'-5"	21'-0"	19'-6"	24'-9"	22'-5"	21'-0"	19'-6"
14"	NI-70	26'-1"	24'-3"	22'-9"	21'-0"	26'-8"	24'-3"	22'-9"	21'-0"
	NI-80	26'-6"	24'-7"	23'-3"	21'-6"	27'-1"	24'-10"	23'-3"	21'-6"
	NI-90x	27'-3"	25'-4"	24'-1"	22'-4"	27'-9"	25'-10"	24'-3"	22'-4"
	NI-60	27'-3"	24'-11"	23'-5"	21'-7"	27'-6"	24'-11"	23'-5"	21'-7"
	NI-70	28'-8"	26'-8"	25'-3"	23'-4"	29'-3"	26'-11"	25'-3"	23'-4"
16"	NI-80	29'-1"	27'-0"	25'-9"	23'-10"	29'-8"	27'-6"	25'-10"	23'-10"
	NI-90x	29'-11"	27'-10"	26'-6"	24'-10"	30'-6"	28'-5"	26'-11"	24'-10"

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of U/480 and a total load deflection limit of t/240.

3. Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum celling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum celling attached to joists.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA 086-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Déad Load = 15 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







			В	are		Ī	1/2" Gyp:	sum Ceiling	
Depth	Series		On Cent	re Spacing			· On Cent	re Spacing	
Dapo.		12"	16"	19.2"	24"	12"	16	19.2"	24"
	NI-20	15'-1"	14'-2"	13'-9"	N/A	15'-7"	14'-8"	14'-2"	N/A
	NI-40x	16'~1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A
<i>-</i> ,-	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	Nt-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
11-7/8"	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18' - 9"	N/A
14°	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
A-T	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
16"	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

			Mid-Spar	Blocking		Mid-S	pan Blocking an	d 1/2" Gypsum	Ceiling
Depth	Series		On Centr	e Spacing			On Centi	e Spacing	
Берен	5555	12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-8"	15'-3"	14'-5"	N/A	16'-8"	15'-3"	14'-5"	N/A
	NI-40x	17'-11"	16'-11"	16'-1"	N/A	18'-5"	17'-1"	16'-1"	N/A
9-1/2"	NI-60	18'-2"	17'-1"	16'-4*	N/A	18'-7"	17'-4"	16'-4"	N/A
2 1/2	NI-70	19'-2"	17'-10"	17'-2"	N/A	19'-7"	18'-3"	17'-7"	N/A
	NI-80	19'-5"	18'-0"	17'-4"	N/A	19'-10"	18'-5"	17'-8"	N/A
	NI-20	19'-6"	18'-1"	17'-3"	N/A	19'-11"	18'-3"	17'-3"	N/A
	NI-40x	21'-0"	19'-6"	18'-8"	N/A	21'-7"	20'-2"	19'-2"	N/A
	NI-60	21'-4"	19'-9"	18'-11"	N/A	21'-11"	20'-4"	19'-6"	N/A
11-7/8"	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-5"	201-5"	N/A
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-8"	N/A
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A
	NI-40x	23¹-7"	21'-11"	20'-11"	N/A	24'-3"	22'-7"	21'-7"	N/A
	NI-60	24'-0"	22'-3"	21'-3"	N/A	24'-8"	22'-11"	21'-11°	N/A
14"	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-11"	N/A
17	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	25'-3"	24'-2"	N/A
	NI-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A
16"	N1-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A
	NI-90x	29'-0"	26'-10"	251-7"	N/A	29'-7"	27'-5°	26'-2"	N/A

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

3. Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{2.} Spans are based on a composite floor with glued-halled oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{4.} Bearing stiffeners are not required when I-Joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA 086-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 bsf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 3/4" OS8 G&N Sheathing







			В	are			1/2" Gyp:	sum Ceiling	
Depth	Series		On Centi	re Spacing	· · · · · · · · · · · · · · · · · · ·		On Cent	re Spacing	
Depen	partos	12"	16"	19.2"	24"	12"	16"	19.2"_	24"
	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	· 16'-4"	15'-5"	14'-6"	13'-5"
	Nt-40x	17'-0"	16'-0"	15'-5"	14'-9"	17'-5"	16'-5"	15'-10"	15'-2"
9-1/2"	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-6"	16'-7"	15'-11"	15'-3"
J-1/2	NI-70	18'-0"	16'-11"	16'-3"	15'-7"	18'-5"	17'-3"	16'-7"	15'-11"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
	NI-20	17'-10"	16'-10"	16'-2"	15'-6"	18'-6"	17'-4"	16'-9"	16'-1"
	NI-40x	19'-4"	17'-11"	17'-3"	16'-6"	19'-11"	18'-6"	17'-9"	17'-0"
	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"
11-7/8	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
	NI-40x	21'-5"	19'-10"	18'-11"	17'-11"	22'-1"	20'-6"	19'-7"	18'-7"
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"
14	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"
	NI-60	231-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22'-9"	21'-6"
16	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

12" 16'-10" 18'-8" 18'-11" 20'-0" 20'-3"	16" 15'-5" 17'-2" 17'-6" 18'-7" 18'-10"	19.2" 14'-6" 16'-3" 16'-6" 17'-9"	24" 13'-5" 15'-2" 15'-5" 16'-7"	12" 16'-10" 18'-10" 19'-2"	On Centr 16" 15'-5" 17'-2" 17'-6"	e Spacing 19.2" 14'-6" 16'-3"	24" 13'-5" 15'-2"
16'-10" 18'-8" 18'-11" 20'-0" 20'-3"	15'-5" 17'-2" 17'-6" 18'-7" 18'-10"	14'-6" 16'-3" 16'-6" 17'-9"	13'-5" 15'-2" 15'-5"	16'-10" 18'-10"	15'-5" 17'-2"	14'-6" 16'-3"	13'-5"
18'-8" 18'-11" 20'-0" 20'-3"	17'-2" 17'-6" 18'-7" 18'-10"	16'-3" 16'-6" 17'-9"	15'-2" 15'-5"	18'-10"	17'-2"	16'-3"	
18'-8" 18'-11" 20'-0" 20'-3"	17'-6" 18'-7" 18'-10"	16'-6" 17'-9"	15'-5"				15'-2"
18'-11" 20'-0" 20'-3"	18'-7" 18'-10"	17'-9"		19'-2"	17'-6"		
20'-0" 20'-3"	18'-10"		16'-7"		T1 -0	16'-6"	15'-5"
20'-3"	18'-10"	471 4411		20'-5"	18'-11"	17'-10"	16'-7"
		17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10
	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"
21'-10"	20'-4"	19'-4"	17'-8"	22'-5"	20'-6"	19'-4"	17'-8"
22'-1"	20'-7"	19'-7"	18 ⁵ -4"	22'-8"	20'-10"	19'-8"	18'-4"
23'-4"	21'-8"	20'-8"	19'-7"	23'-10"	22'-3"	21'-2"	19'-9"
23'-7"	21'-11"	20'-11"	19'-9"	24'-1"	22'-6"	21'-5"	20'-0"
-			20'-4"	24'-8"	23'-0"	22'-0"	20'-9"
			19'-5"	25'-1"	23'-2"	21'-9"	19'-5"
			20'-10"	25'-6"	23'-8"	22'-4"	20'-10
				26'-8"	24'-11"	23'-9"	22'-4"
				27'-1"	251-3"	24'-1"	22'-9"
						24'-8"	23'-4"
					26'-2"		23'-1"
				1	27'-4"		24'-8"
				29'-8"	27'-9"		25'-0"
							25'-8"
	24'-3" 24'-5" 24'-10" 26'-1" 26'-6" 27'-3" 27'-3" 28'-8" 29'-1" 29'-11"	24'-3" 22'-6" 24'-5" 22'-9" 24'-10" 23'-1" 26'-1" 24'-3" 26'-6" 24'-7" 27'-3" 25'-4" 27'-3" 25'-5" 28'-8" 26'-8" 29'-1" 27'-0"	24'-3" 22'-6" 21'-6" 24'-5" 22'-9" 21'-8" 24'-10" 23'-1" 22'-0" 26'-1" 24'-3" 23'-2" 26'-6" 24'-7" 23'-5" 27'-3" 25'-4" 24'-1" 27'-3" 25'-5" 24'-2" 28'-8" 26'-8" 25'-4" 29'-1" 27'-0" 25'-9"	24'-3" 22'-6" 21'-6" 20'-4" 24'-5" 22'-9" 21'-8" 19'-5" 24'-10" 23'-1" 22'-0" 20'-10" 26'-1" 24'-3" 23'-2" 21'-10" 26'-6" 24'-7" 23'-5" 22'-2" 27'-3" 25'-4" 24'-1" 22'-9" 22'-3" 25'-5" 24'-2" 22'-10" 28'-8" 26'-8" 25'-4" 23'-11' 29'-1" 27'-0" 25'-9" 24'-4"	24'-3" 22'-6" 21'-6" 20'-4" 24'-8" 24'-5" 22'-9" 21'-8" 19'-5" 25'-1" 24'-10" 23'-1" 22'-0" 20'-10" 25'-6" 26'-1" 24'-3" 23'-2" 21'-10" 26'-8" 26'-6" 24'-7" 23'-5" 22'-2" 27'-1" 27'-3" 25'-4" 24'-1" 22'-9" 27'-9" 27'-3" 25'-5" 24'-2" 22'-10" 28'-0" 28'-8" 26'-8" 25'-4" 23'-11" 29'-3" 29'-1" 27'-0" 25'-9" 24'-4" 29'-8"	24'-3" 22'-6" 21'-6" 20'-4" 24'-8" 23'-0" 24'-5" 22'-9" 21'-8" 19'-5" 25'-1" 23'-2" 24'-10" 23'-1" 22'-0" 20'-10" 25'-6" 23'-8" 26'-1" 24'-3" 23'-2" 21'-10" 26'-8" 24'-11" 26'-6" 24'-7" 23'-5" 21'-10" 26'-8" 24'-11" 27'-3" 25'-4" 24'-1" 22'-9" 27'-9" 25'-11" 27'-3" 25'-5" 24'-2" 22'-10" 28'-0" 26'-2" 28'-8" 26'-8" 25'-4" 23'-11" 29'-3" 27'-4" 29'-1" 27'-0" 25'-9" 24'-4" 29'-8" 27'-9"	24'-3" 22'-6" 21'-6" 20'-4" 24'-8" 23'-0" 22'-0" 24'-5" 22'-9" 21'-8" 19'-5" 25'-1" 23'-2" 21'-9" 24'-10" 23'-1" 22'-0" 20'-10" 25'-6" 23'-8" 22'-4" 26'-1" 24'-3" 23'-2" 21'-10" 26'-8" 24'-11" 23'-9" 26'-6" 24'-7" 23'-5" 22'-2" 27'-1" 25'-3" 24'-1" 27'-3" 25'-4" 24'-1" 22'-9" 27'-9" 25'-11" 24'-8" 27'-3" 25'-5" 24'-2" 22'-10" 28'-0" 26'-2" 24'-9" 28'-8" 26'-8" 25'-4" 23'-11" 29'-3" 27'-4" 26'-1" 29'-1" 27'-0" 25'-9" 24'-4" 29'-8" 27'-9" 26'-5"

Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The
ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration,
a live load deflection limit of L/480 and a total load deflection limit of L/240.

a two load deflection filling it 4-be also a local road collection of the load of the load

Minimum bearing length shall be 1-3/4 inches for the end bearings.
 Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA 086-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf. Dead Load = 30 psf Simple Spans, L/480 Deflection Umit 5/8" OSB G&N Sheathing







1 Int Consum Calling

Depth	Series	8are On Centre Spading				1/2" Gypsum Ceiling On Centre Spacing			
		9-1/2"	NI-20	15'-1"	14'-1"	13'-3"	N/A	15'-7"	14'-1"
NI-40x	16'-1"		15'-Z"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A
NI-60	16'-3"		15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A
NI-70	17'-1"		16'-1"	15 '-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
NI-80	17'-3"		16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
11-7/8"	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	NI-40x	18'- 1 "	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
	MI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A_
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
14"	NI-60	20'-5"	18¹-11"	18'-1"	N/A	21'-2"	19'-7"	18'- 9 "	N/A
	NI-70	21'-7"	201-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
16"	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
	NJ-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

Depth	Serles	Mid-Span Blocking On Centre Spacing				Mid-Span Blocking and 1/2" Gypsum Ceiling On Centre Spacing			
		9-1/2"	NI-20	15'-7"	14'-1"	13'-3"	N/A	15'-7"	14'-1"
NI-40x	17'-9"		16'-1"	15'-1"	N/A	17'-9"	16'-1"	15'-1"	N/A
NI-60	18'-1"		16'-4"	15'-4"	N/A	18'-1"	16'-4"	15'-4"	N/A
NI-70	19'-2"		17'-10"	16'-9"	N/A	19'-7"	17'-10"	16'-9"	N/A
NI-80	19'-5"		18'-0"	17'-1"	N/A	19'-10"	18'-3"	17'-1"	N/A
11-7/8"	NI-20	18'-9"	17'-0"	16'-0"	N/A	18 -9	17'-0"	16'-0"	N/A
	NI-40x	21'-0"	19'-3"	17'-9"	N/A	21'-3"	19'-3"	17'-9"	N/A
	NI-60	21'-4"	19'-8"	18'-5"	N/A	21'-8"	19'-8"	18'-5"	N/A
	N1-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-4"	20'-0"	N/A
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-5"	N/A
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A
14 ^u	NI-40x	23'-7"	21'-5"	19'-6"	N/A	24'-1"	21'-5"	19'-6"	N/A
	NI-60	24'-0"	22'-3"	21'-0"	N/A	24'-8"	22'-5"	21'-0"	N/A
	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-9"	N/A
	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A
	Nt-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A
16"	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	24'-10"	23'-4"	N/A
	NI-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A
	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A
	N1-90x	29'-0".	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26'-2"	N/A

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf, The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

3. Minimum bearing length shall be 1-3/4 inches for the end bearings.

4. Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum celling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum celling attached to joists.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA 086-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.

Construction Detail



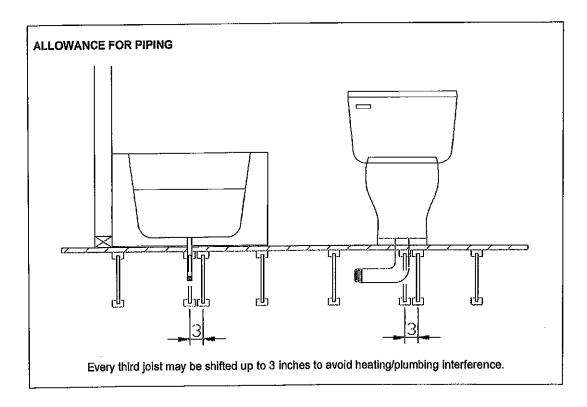
Limit States Design

Allowance for Piping (Installation Notes)

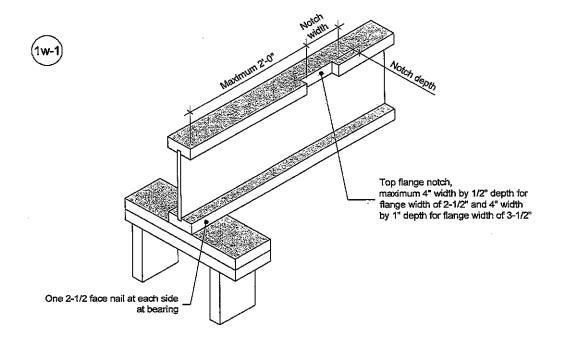
The floor layouts have usually not been checked for heating and/or plumbing interference. On-site adjustment of joists of up to 3 inches is permitted to avoid interferences. When moving a joist, the subfloor thickness shall be checked with code requirements when the joist spacing exceeds 19.2 inches. Except for cutting to length, I-joist flanges should never be cut, drilled, or notched.

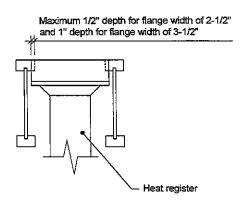
Installation of Nordic I-joists shall be as per *Nordic Joist Installation Guide for Residential Floors*. Refer to Tables 1 and 2 for maximum web hole and duct chase openings, respectively. These tables are based on the I-joists being used at their maximum spans. The minimum distance given may be reduced for shorter spans; contact your distributor for additional information.

The detail below shows the 3-inch allowance for plping. Every third joist may be shifted up to 3 inches to avoid heating/plumbing interference. For other applications, please contact your distributor.



Revised April 12, 2012





Notes:

- 1. Blocking required at bearing for lateral support, not shown for clarity.
- 2. The maximum dimensions for a notch on the side of the top flange are 4-inch width by 1/2-inch depth for flange width of 2-1/2 inches, and 4-inch width by 1-inch depth for flange width of 3-1/2 inches.
- 3. This detail applies to simple-span joists and multiple-span joists where the notch is located at the end half-span.
- 4. For other applications, contact Nordic Structures.

This document supersedes all previous versions. If the document has been in effect for more than one year, consult nordic, ca or contact Nordic Structures.

All nails shown in the details are assumed to be common nails unless otherwise noted. Nails shall have a diameter not less than 0.128 inch for 2-1/2-inch nails, or 0.144 inch for 3-inch nails. Individual components not shown to scale for clarity.

NORDIC STRUCTURES

T 514-871-8526 1 866 817-3419

nordic.ca

Notch in I-joist for Heat Register

CATEGORY

I-joist - Typical Floor Framing and Construction Details

DOCUMENT

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