



FROM PLAN DATED: JAN 2020

**BUILDER: GREENPARK HOMES** 

SITE: RUSSELL GARDENS PH 3

**MODEL:** MOUNTAINASH 6

**ELEVATION: 1** 

LOT: 214

**CITY: WATERDOWN** 

**SALESMAN: MARIO DICIANO** 

DESIGNER: AJ REVISION: CH

NOTES:

REFER TO THE **NORDIC INSTALLATION**GUIDE FOR PROPER STORAGE AND
INSTALLATION.

SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. CERAMIC TILE APPLICATION AS PER O.B.C 9.30.6.

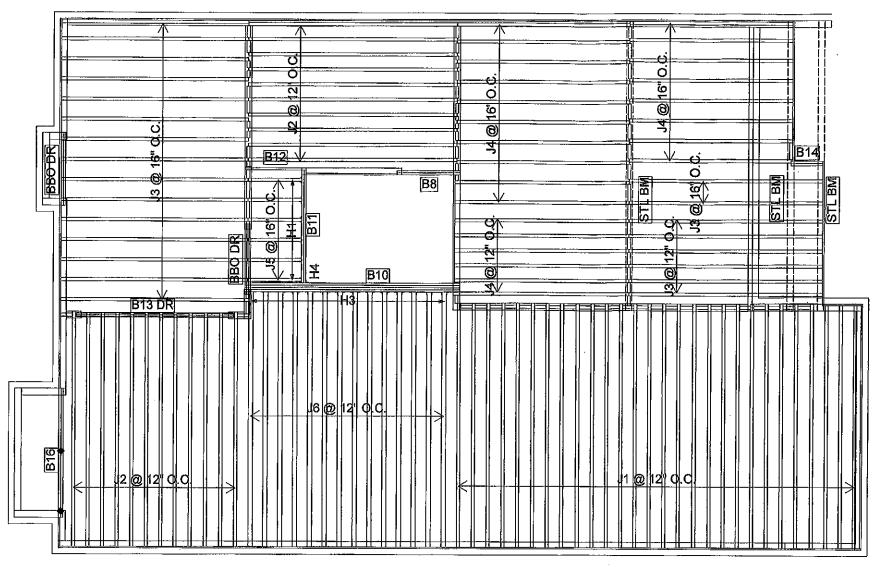
LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft 2 DEAD LOAD: 20.0 lb/ft 2

SUBFLOOR: 3/4" GLUED AND NAILED

**DATE:** 9/24/20

# 1st FLOOR



L.						
	PlotID	Length	Product	Plies	Net Qty	Fab Type
	J1	18-00-00	9 1/2" NI-40x	1	28	MFD
·	J2	16-00-00	9 1/2" NI-40x	1	22	MFD
	J3	14-00-00	9 1/2" NI-40x	1	23	MFD
	J4	12-00-00	9 1/2" NI-40x	1	24	MFD
	J5	4-00-00	9 1/2" NI-40x	1	6	MFD
	J6	18-00-00	9 1/2" NI-80	1	14	MFD
	B10	16-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	4	4	MFD
CITY OF HAMILT		12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	MFD
Building Division	B16	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	MFD
Building Division  Permit No. 21-1	B11	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	MFD
Permit No. 21-	B12	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1	MFD
THESE STAMPED DRAWINGS SHALL BE AVA	B8	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	MFD
THE OWNER AND/OR CONTRACTOR SHALL OHTER ONTARIO BUILDING CODE AND ALL OTHER	RABITATELE LAW	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	MFD
THE ONTARIO BUILDING CODE AND ALL OTTE						

e drawings and/or specifications have been reviewed by

FOR SHIEF EUILDING OFFICIAL

**Products** 

C	Connector Summary						
Qty	Manuf	Product					
6	H1	IUS2.56/9.5					
14	Н3	IUS3.56/9.5					
1	H4	HGUS410					



FROM PLAN DATED: JAN 2020

**BUILDER: GREENPARK HOMES** 

**SITE:** RUSSELL GARDENS PH 3

**MODEL:** MOUNTAINASH 6

**ELEVATION: 11** 

LOT: 214

**CITY: WATERDOWN** 

SALESMAN: MARIO DICIANO

DESIGNER: AJ REVISION: CH

#### NOTES:

REFER TO THE NORDIC INSTALLATION **GUIDE FOR PROPER STORAGE AND** INSTALLATION. SQUASH BLOCKS OF 2x4, 2x6. 2x8 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE **SQUASH BLOCKS** REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD **CUT OPENINGS** SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE, CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6

#### LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 20.0 lb/ft<sup>2</sup>

SUBFLOOR: 5/8" GLUED AND NAILED

**DATE:** 9/24/20

# 2nd FLOOR

## NORDIC STRUCTURES

**COMPANY** Apr. 9, 2020 10:01

PROJECT
J1 1ST FLOOR.wwb

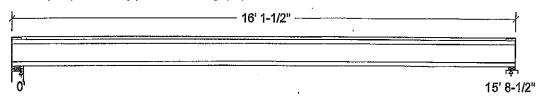
## **Design Check Calculation Sheet**

Nordic Sizer - Canada 7,2

#### Loads:

Load	Туре	Distribution	Pat-	Location	[ft]	Magnitude		Unit
			tern	Start	End	Start E	nd	
Load1	Dead	Full Area				20.00		psf
Load2	Live	Full Area				40.00		psf

#### Maximum Reactions (lbs) and Support Bearing (in):



i			
Unfactored: Dead Live Factored:	157 314		157 314
Total	668		668
Bearing:			<del> </del>
Capacity			
Joist	1893	•	1865
Support	7744	•	3971
Des ratio			
Joist	0.35		0.36
Support	0.09		0.17
Load case	#2		#2
Length	4-3/8		2-3/8
Min req'd	1-3/4		1-3/4
Stiffener	No		No
KD	1.00		1.00
KB support	_		1.00
fcp sup	769		769
Kzcp sup	-		1.09

Bearing for wall supports is perpendicular-to-grain bearing on top plate. No stud design included.

#### Nordic 9-1/2" NI-40x Floor joist @ 12" o.c.

Supports: 1 - Lumber Wali, No.1/No.2; 2 - Lumber Sill plate, No.1/No.2; Total length: 16' 1-1/2"; Clear span: 15' 6-3/4"; 3/4" nailed and glued OSB sheathing This section PASSES the design code check.

#### Limit States Design using CSA O86-14 and Vibration Criterion:

Criterion	Analysis Value.	Design Value	Unit	Analysis/Design
Shear	Vf = 668	Vr = 1895	lbs	Vf/Vr = 0.35
Moment(+)	Mf = 2622	Mr = 4824	lbs-ft	Mf/Mr = 0.54
Perm. Defl'n	0.12 = < L/999	0.52 = L/360	in	ESSION 0.22
Live Defl'n	0.23 = L/817	0.39 = L/480	in part	0.59
Total Defl'n	0.35 = L/545	0.79 = L/240	in/s	0.44
Bare Defl'n	0.28 = L/684	0.52 = L/360	ing Ca	0.44 0.53 0.53 0.92 0.76
Vibration	Lmax = 15'-8.5	Lv = 17'-1.8	ff	0.92
Defl'n	= 0.031	= 0.041	149 S.K	TSOULANDS \$ 0.76
			1 413	Lancar 1

DWO NO. TAN 6078 -20 STRUCTURAL COME ONE MY ONLY

POWER OF OR

## WoodWorks® Sizer

#### for NORDIC STRUCTURES

#### J1 1ST FLOOR.wwb

#### Nordic Sizer - Canada 7.2

Page 2

Additional Data	a:									
FACTORS: f/		KH	KZ	KL	KT	KS	KN	LC#		
Vr 18	95 1.00	1.00		-	-	-	_	#2		
Mr+ 48	24 1.00	1.00	-	1.000	-	-	-	#2		
EI 218	.1 million	_	-	_	-	-	_	#2		
CRITICAL LOAD O	OMBINATIONS	<b>3</b> :								
Shear : L	C #2 = 1.25	5D + 1.5L	ı							
Moment(+): L	C #2 = 1.25	5D + 1.5L	1							
Deflection: L	C #1 = 1.0C	) (perma	nent)							
	C #2 = 1.0D								•	
	C #2 = 1.00									
Ь	C #2 = 1.0D	) + 1.0L	(bare	joist)						
Bearing : S	upport 1 - I	C # 2 = 1	.25D +	1.5L						
S	upport 2 - I	LC #2 = 1	.25D +	1.5L						
	=dead W=win			- 0			-			
	=live(use,oc						f=fire			
Load Patterns										
All Load Comb	inations (LC	cs) are l	isted i	n the Ana	alysis	output				
CALCULATIONS:							· C	anporms to	OBC 2012	
Eleff = $265.2$								11 Times hade to be		
"Live" deflec	tion is due	to all n	on-dead	loads (	Live,∶w	ind, sno	w)	AMENDED	2020	
1										

#### Design Notes:

- 1. WoodWorks analysis and design are in accordance with the 2015 National Building Code of Canada (NBC), Division B, Part 4, and the CSA 086-14 Engineering Design in Wood standard, Update No. 2 (June 2017).
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to Nordic Structures technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.



SWB HO. TANGOT 6-20 STRUCTURAL COMPONENT ONLY

## NORDIC STRUCTURES

**COMPANY** Apr. 9, 2020 09:59 PROJECT
J1 2ND FLOOR.wwb

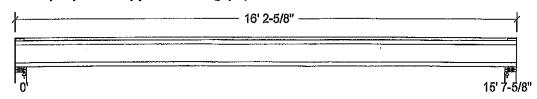
## **Design Check Calculation Sheet**

Nordic Sizer - Canada 7.2

#### Loads:

ſ	Load	Туре	Distribution	Pat-	Location	[ft]	Magnitude		Unit
				tern	Start	End	Start	End	<u> </u>
	Load1	Dead	Full Area				20.00		psf
Ì	Load2	Live	Full Area	j			40.00		psf

## Maximum Reactions (lbs) and Support Bearing (in):



		······································	<del></del>
Unfactored: Dead Live	156 313		156 313
Factored: Total	.664		664.
Bearing:			
Capacity	1002		1000
Joist	1893		1893
Support	7744		7744
Des ratio			
Joist	0.35		0.35
Support	0.09		0.09
Load case	′ #2		#2
Length	4-3/8	•	4-3/8
Min req'd	1-3/4		1-3/4
Stiffener	No		No
KD	1.00		1.00
KB support	i -		-
fcp sup	769		769
Kzcp sup	-		

Bearing for wall supports is perpendicular-to-grain bearing on top plate. No stud design included.

#### Nordic 9-1/2" NI-40x Floor joist @ 12" o.c.

Supports: All - Lumber Wall, No.1/No.2

Total length: 16' 2-5/8"; Clear span: 15' 5-7/8"; 5/8" nailed and glued OSB sheathing with 1/2" gypsum ceiling

This section PASSES the design code check.

### Limit States Design using CSA O86-14 and Vibration Criterion:

	<u> </u>			
Criterion	Analysis Value	Design Value	Unit	Analysis/Design
Shear	Vf = 664	Vr = 1895	lbs	Vf/Vr = 0.35
Moment (+)	Mf = 2597	Mr = 4824.	lbs-ft	Mr = 0.54
Perm. Defl'n	0.12 = < L/999	0.52 = L/360	in in	FESSION 0.22
Live Defl'n	0.23 = L/808	0.39 = L/480	in 🔊	
Total Defl'n	0.35 = L/539	0.78 = L/240	in 🚱	0.45 0.52 ATSOULAKOS (10.94
Bare Defl'n	0.27 = L/694	0.52 = L/360	in/2	0.45
Vibration	Lmax = 15'-7.6	$Lv = 16^{7} - 8.5$	ft 🖁 🖁	AVOC 18 10.94
Defl'n	= 0.034	= 0,041	in S S.K	ATSOULAKUS \$ 10.81
DETT II			1.63	

864

BOUNCE OF ONTE STRUCTURAL COMMONIENT ONLY

### WoodWorks® Sizer

#### for NORDIC STRUCTURES

### J1 2ND FLOOR.wwb

#### Nordic Sizer - Canada 7.2

Page 2

						· · · · · · · · · · · · · · · · · · ·					
Additiona	l Data:		•								
FACTORS:	f/E	KD	KH	KZ	KL	KT	KS .	KN	LC#		
Vr	1895	1.00	1.00	_		-	-	-	#2		
Mr+	4824	1.00	1.00	_	1.000	, <b></b>	-	-	#2		
ΕI	218.1 m	illion	-	-	-	-	-	-	#2		
CRITICAL LO	OAD COMB	INATIONS	3:								
Shear	: LC #2	= 1.2	5D + 1.51	<u></u>							
Moment (+	); LC #2	= 1.2	5D + 1.51	Ĺ							
Deflection	on: LC #1	= 1.0	D (perma	anent)							
			D + 1.0L		)						
	LC #2	= 1.01	D + 1.0L	(tota	1)						
	LC #2	= 1.01	D + 1.0L	(bare	joist)						
Bearing	: Suppo	rt 1 - :	LC #2 = 3	1.25D +	1.5L						
			LC #2 = 3								
Load Type					arth, grou						
	L≕liv	e (use, o	ccupancy)	Ls=l:	ive(stora	ge, equi	(pment	f=fire			
Load Pat	terns: s=	S/2 L=	L+Ls _=r	no patt	ern load	in this	s span				
All Load	Combinat	ions (L	Cs) are	Listed :	in the An	alysis	output				
CALCULATI									CANFORMS	agn ar	2019
Eleff =	258.29 lb	-in^2	K = 4.946	e06 lbs					o chai nauno	10 000	1 4 U I A
"Live" d	eflection	is due	to all r	non-dead	d loads (	live, w	ind, sn	ow):	AMENI	DED 20:	2 R
									74 CH PG 40 7		AT W

## **Design Notes:**

- 1. WoodWorks analysis and design are in accordance with the 2015 National Building Code of Canada (NBC), Division B, Part 4, and the CSA 086-14 Engineering Design in Wood standard, Update No. 2 (June 2017).
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to Nordic Structures technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.



DWG NO. TAW 6079-2 STRUCTURAL COMPONENT ONLY NORDIC **STRUCTURES** 

**COMPANY** Apr. 9, 2020 10:02 **PROJECT** J6 2ND FLOOR.wwb

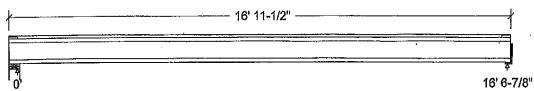
## **Design Check Calculation Sheet**

Nordic Sizer - Canada 7.2

#### Loads:

Load	Туре	Distribution	Pat- tern	Location Start	[ft] End	Magnitude Start	e End	Unit
Loadl Load2	Dead Live	Full Area Full Area				20.00 40.00		psf psf

## Maximum Reactions (lbs) and Support Bearing (in):



			·······
Unfactored:			
Dead	166		166
Live	331		331
Factored:			
Total	704		704
Bearing:			<del>   </del>
Capacity			
Joist	1893		1893
Support	10841		-
Des ratio	•		
Joist	0.37	·	0.37
Support	0.06		
Load case			#2
Length	4-3/8		2
Min req'd	1-3/4		1-3/4
Stiffener	No		No
KD	1.00		1.00
KB support	-		
fcp sup	769		-
Kzcp sup	· -		-

Bearing for wall supports is perpendicular-to-grain bearing on top plate. No stud design included.

#### Nordic 9-1/2" NI-80 Floor joist @ 12" o.c.

Supports: 1 - Lumber Wall, No.1/No.2; 2 - Hanger; Total length: 16' 11-1/2"; Clear span: 16' 5-1/8"; 5/8" nailed and glued OSB sheathing with 1/2" gypsum ceiling This section PASSES the design code check.

## Limit States Design using CSA O86-14 and Vibration Criterion:

Criterion	Analysis Value	Design Value	Unit	Analysis/Design
Shear	Vf = 704	$\nabla r = 1895$	lbs	Vf/Vr = 0.37
Moment (+)	Mf == 2918	Mr = 8958	lbs-ft 🎤	ccsMT/Mr = 0.33
Perm. Defl'n	$0.11 = \langle L/999$	0.55 = L/360	in de	0.19
Live Defl'n	0.21 = L/940	0.41 = L/480	in /o	0.51
Total Defl'n	0.32 = L/626	0.83 = L/240	in / 3 /	41600 2 0.38
Bare Defl'n	0.24 = L/841	0.55 ⇒ L/360	in/2	新 10.43
Vibration	Lmax = 16'-6.9	Lv = 17' - 9.5	ft 🦁 e i	ATSOUDANOS \$ 0.93
Defl'n	= 0.031	= 0.038	in 3	0.80
	<del></del>		N Green	William II

ove No. 7AN 6060-20 STRUCTURAL component only

BUNCE OF ONE

### WoodWorks® Sizer

#### for NORDIC STRUCTURES

#### J6 2ND FLOOR.wwb

#### Nordic Sizer - Canada 7.2

Page 2

Additiona	I Data:									
FACTORS:	f/E	KD	KH	KZ	KL	KT	KS	KN	LC#	
Vr	1895	1.00	1.00	-	-	-	-	-	#2 ·	
Mr+	8958	1.00	1.00	_	1.000		_	-	#2	•
EI	324.1 m	illion	-		-	_	-	- '	#2	
CRITICAL L			3:							
Shear	: LC #2	= 1.2!	5D + 1.51	ū						
Moment(+	) : LC #2	= 1.2	5D + 1.5I	.i						
Deflecti	on: LC #1	. = 1.0I	D (perma	anent)						
			0 + 1.0L		)					
	LC #2	= 1.01	0 + 1.0L	(tota:	L)					
			0 + 1.0L							
Bearing	: Suppo	rt 1 - 1	LC #2 = 1	1.25D +	1.5L					
	Suppo	rt 2 - 1	LC #2 = 1	1.25D +	1.5L					
Load Typ					arth, grou	ndwater	. E≔ear	thquake		
					lve (stora					
Load Pat	terns: s=	S/2 L=1	L+Ls =r	no patte	ern load	in this	sspan			
All Load	l Combinat	ions (L	Cs) are l	listed :	in the An	alysis	output			
CALCULAT	IONS:								canforms to	OBG 2012
Eleff =	367.27 lk	-in^2	K⇔ 4.946	e06 lbs						
"Live" d	eflection	is due	to all r	ion-dead	i loads (	live, w	vind, sn	ow)	AMENDED	Z U Z U

#### **Design Notes:**

- 1. WoodWorks analysis and design are in accordance with the 2015 National Building Code of Canada (NBC), Division B, Part 4, and the CSA O86-14 Engineering Design in Wood standard, Update No. 2 (June 2017).
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to Nordic Structures technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.



DWG NO. TAN 6080-28 STRUCTURAL COMPONENT ONLY





PASSED

November 25, 2020 08:03:28

2ND FLR FRAMING\Flush Beams\B17D(i3135) (Flush Beam) Dry | 2 spans | L cant.

**BC CALC® Member Report** 

Build 7493

Job name:

Address:

City, Province, Postal Code: WATERDOWN

Customer:

Code reports:

CCMC 12472-R

File name:

MOUNTAINASH 6 EL 3 LOT 12.mmdl

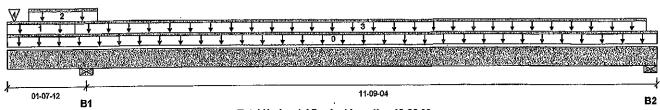
2ND FLR FRAMING\Flush Beams\B17D(i3135)

Description: Specifier:

Designer:

ΑĴ

Company:



Total Horizontal Product Length = 13-05-00

Reaction 5un	nmary (Down / O				
Bearing	Live	Dead	Snow	Wind	
B1, 5-1/2"	179 / 0	884 / 0	367 / 0		- · · · · · · · · · · · · · · · · · · ·
B2, 5-1/2"	117/6	38/0	0/43		

Los	ad Summary		;			-	Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	13-05-00	Тор		10			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	, L	00-00-00	01-05-00	Top	15	7			n\a
2	E30((3261)	Unf, Lin. (lb/ft)	L	00-05-08	01-10-08	Тор		81			nla
3	FC2 Floor Material	Unf. Lin. (lb/ft)	L	01-05-00	13-02-04	Top	20	10			n\a
4	-	Conc. Pt. (lbs)	L.	00-01-13	00-01-13	Top	33	550	324		n\a

Controls Summary	Factored Demand	Factored Resistance	Demand <i>i</i> Resistance	Case	Location
Pos. Moment	419 ft-lbs	23220 ft-lbs	1.8%	36	08-10-13
Neg. Moment	-1265 ft-lbs	-15093 ft-lbs	8.4%	0	01-07-12
End Shear	165 lbs	11571 lbs	1.4%	33	12-02-00
Cont. Shear	1246 lbs	11571 lbs	10.8%	37	00-07-08
Total Load Deflection	2xL/1998 (0.029")	n\a	n\a	82	00-00-00
Live Load Deflection	L/999 (-0.016")	n\a	n\a	120	06-06-05
Total Neg. Defl.	L/999 (-0.028")	n\a	n\a	82	05-11-03
Max Deff.	-0.028"	n\a	n\a	82	05-11-03
Span / Depth	14.4				•

Bear	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	5-1/2" x 3-1/2"	1238 lbs	16.1%	8.1%	Spruce-Pine-Fir
B2	Wall/Plate	5-1/2" x 3-1/2"	223 lbs	1.9%	0.9%	Spruce-Pine-Fir

**Cautions** 

Concentrated side load(s) 2 are closer than 18" from end of member. Please consult a technical representative or Professional of Record.



STRUCTURAL COMPONENT ONLY





## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING\Flush Beams\B17D(i3135) (Flush Beam)

PASSED

November 25, 2020 08:03:28

**BC CALC® Member Report** 

**Build 7493** 

Job name:

Address: City, Province, Postal Code: WATERDOWN

Customer:

Code reports:

Dry | 2 spans | L cant.

File name:

MOUNTAINASH 6 EL 3 LOT 12.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B17D(i3135)

Specifier:

Designer:

Company:

Design meets User specified (2xL/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO 033 2112

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

CCMC 12472-R

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Unbalanced snow loads determined from building geometry were used in selected product's

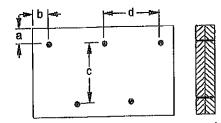
verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Cantilevers require sheathed bottom flanges, blocking at cantilever support and closure at ends.

## Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3"

c = 5-1/2" d=@ 8"

Calculated Side Load = 227.3 lb/ft

Connectors are: .

ு் பெ, Nails

3-1/2" ARDOX SPIRAL



6W8 NO. FAN 16085-20 STRUCTURAL COMPONENT ONLY

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of Input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Gulde and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAMM, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING\Flush Beams\B18D(i3150) (Flush Beam)

PASSED

**BC CALC® Member Report** 

Dry | 2 spans | L cant.

November 25, 2020 08:03:28

**Build 7493** Job name:

File name:

MOUNTAINASH 6 EL 3 LOT 12.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B18D(i3150)

Specifier:

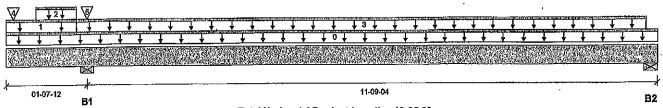
City, Province, Postal Code: WATERDOWN Customer:

Designer:

Code reports:

CCMC 12472-R

Company:



Total Horizontal Product Length = 13-05-00

Reaction Sur	וןט <i>ו</i> mary ושסעו) mary ו	ant) (ibs)			
Bearing	Live	Dead	Snow	Wind	
B1, 5-1/2"	222 / 0	906/0	387 / 0		
B2 5-1/2"	117 / 12	34/0	0/45	•	

Los	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag		Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	13-05-00	Тор	-	10			00-00-00
1	E28(13262)	Unf. Lin. (lb/ft)	L	00-00-00	01-05-00	Top		81			n\a
,	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-07-04	01-05-00	Top	10				n\a
3	FC2 Floor Material	Unf. Lin. (lb/ft)	L	01-05-00	13-02-04	Top	20	10			n\a
4	H	Conc. Pt. (lbs)	L	00-01-13	00-01-13	Top	84	550	342		n\a
5	E23(13255)	Conc. Pt. (lbs)	L.	01-07-12	01-07-12	Top		24			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand <i>l</i> Resistance	Case	Location
Pos. Moment	403 ft-lbs	23220 ft-lbs	1.7%	36	08-10-13
Neg, Moment	-2083 ft-lbs	-23220 ft-lbs	9.0%	37	01-07-12
End Shear	159 lbs	11571 lbs	1.4%	33	12-02-00
Cont. Shear	1356 lbs	11571 lbs	11.7%	37	00-07-08
Total Load Deflection	2xL/1998 (0.032")	n\a	n\a	82	00-00-00
Live Load Deflection	L/999 (-0.018")	n\a	n\a	120	06-06-05
Total Neg. Defl.	L/999 (-0.031")	n\a	n\a	82	06-01-00
Max Defi.	-0.031"	n\a	n\a	82	06-01-00
Span / Depth	14.4				

Bearing	supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	5-1/2" x 3-1/2"	1268 lbs	16.5%	8.3%	Spruce-Pine-Fir
B2	Wall/Plate	5-1/2" x 3-1/2"	218 lbs	1.8%	0.9%	Spruce-Pine-Fir

Cautions

Concentrated side load(s) 2 are closer than 18" from end of member Please consult a technical representative or Professional of Record.



046 NO. TAN 160%-20 STRUCTURAL COMPONENT ONLY





# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING/Flush Beams/B18D(i3150) (Flush Beam)

PASSED

**BC CALC® Member Report** 

Build 7493 Job name: Address:

Customer:

Code reports:

Dry | 2 spans | L cant.

November 25, 2020 08:03:28

File name:

ame: MOUNTAINASH 6 EL 3 LOT 12.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B18D(i3150)

City, Province, Postal Code: WATERDOWN Specifier:

Designer: AJ

Company:

Notes

Design meets User specified (2xL/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBC 2012

Calculations assume member is fully braced.

AMENDED 2020

Resistance Factor phi has been applied to all presented results per CSA O86.

CCMC 12472-R

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Unbalanced snow loads determined from building geometry were used in selected product's

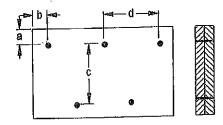
verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Cantilevers require sheathed bottom flanges, blocking at cantilever support and closure at ends.

## Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3" c = 5-1/2" d = 200 6"

Calculated Side Load = 231.8 lb/ft

Connectors are: .

.. Nails

3-1/2" ARDOX SPIRAL

A



uwa no . tan 16096-20 Structural Component only

#### Disclosure

Use of the Bolse Cascade Software Is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of sultability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

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November 25, 2020 08:03:28

2ND FLR FRAMING\Flush Beams\B19D(I3184) (Flush Beam)

Dry | 1 span | No cant. **BC CALC® Member Report** 

**Build 7493** 

Job name: Address:

City, Province, Postal Code: WATERDOWN

Customer: Code reports:

**CCMC 12472-R** 

File name:

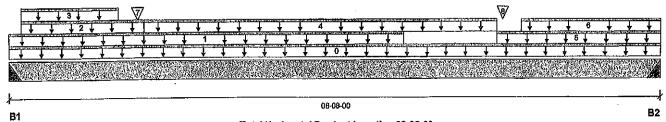
MOUNTAINASH 6 EL 3 LOT 12.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B19D(i3184) Specifier:

Designer:

Wind

Company:



Total Horizontal Product Length = 08-08-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow-	
B1, 2"	78 / 0	528 / 0	301 / 0	
B2. 2"	33/0	523 / 0	311 / 0	

Los	ad Summary					:	Live	Dead	Snow	Wind	Tributary
Tag	· · · · · · · · · · · · · · · · · · ·	Load Type	Ref.	Start	End .	Loc.	1.00	0.65	1.00	1.15	<u> </u>
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-08-00	Тор		10			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	05-02-08	Top `	22	11			n\a
2	E29(i3263)	Unf. Lin. (lb/ft)	L	00-02-00	01-09-08	Тор		81			n\a
3	E29(i3263)	Unf. Lin. (lb/ft)	· L	00-02-00	01-05-08	Top		28	72		n\a
4	E31(i3264)	Unf. Lin. (lb/ft)	L	01-09-08	06-05-08	Top		61			n\a
5	E32(i3265)	Unf. Lin. (lb/ft)	L	06-05-08	08-08-00	Top		81			n\a
6	E32(I3265)	Unf. Lin. (lb/ft)	L	06-09-08	08-08-00	Top		28	72		n\a
7	E29(i3263)	Conc. Pt. (lbs)	L	01-08-08	01-08-08	Top		115	193		n\a
8	E32(i3265)	Conc. Pt. (lbs)	L	06-06-08	06-06-08	Тор		114	191		n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1364 ft-lbs	15093 ft-lbs	9.0%	0	04-04-04
End Shear	978 lbs	11571 lbs	8.5%	13	00-11-08
Total Load Deflection	L/999 (0.042")	n\a	n\a	35	04-04-04
Live Load Deflection	L/999 (0.016")	n\a	n\a	51	04-04-04
Max Defl.	0.042"	n\a	n\a	35	04-04-04
Snan / Denth	10.7				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Hanger	2" x 3-1/2"	1189 lbs	n\a	13.9%	Hanger
B2	Hanger	2" x 3-1/2"	1154 lbs	n\a	13.5%	Hanger

Hanger model Hanger was not found. Hanger has not been analyzed for adequate capacity.



646 NO . TAM 16097 = 20 STRUCTURAL COM, ONEHT ONLY





## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING\Flush Beams\B19D(i3184) (Flush Beam)

PASSED

**BC CALC® Member Report** 

Job name:

Dry | 1 span | No cant.

November 25, 2020 08:03:28

**Build 7493** 

Address:

Customer:

Code reports:

City, Province, Postal Code: WATERDOWN

CCMC 12472-R

File name:

MOUNTAINASH 6 EL 3 LOT 12.mmdl

2ND FLR FRAMING\Flush Beams\B19D(i3184) Description:

Specifier:

Designer: AJ

Company.

**Notes** 

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

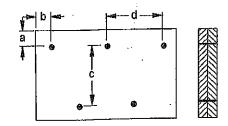
CONFORMS TO OBC 2012 AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Unbalanced snow loads determined from building geometry were used in selected product's

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

## Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3"

c = 5-1/2"d=100 (1

Connectors are: '

Nails نواند کارد. 3-1/2" ARDOX SPIRAL

ONNOE OF ONT

BWS NO. YAW 16027-20 STRUCTURAL COMPONENT ONLY

#### Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of sultability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Bolse Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

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Passed

August 26, 2020 09:26:19

2ND FLR FRAMING\Flush Beams\B21(i2792) (Flush Beam)

**BC CALC® Member Report** Build 7493

Job name: ...

Address:

City, Province, Postal Code: HAMILTON

Customer:

Code reports:

CCMC 12472-R

Dry | 2 spans | L cant.

File name:

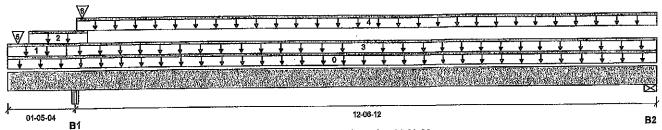
MOUNTAINASH 6

Description: 2ND FLR FRAMING\Flush Beams\B21(i2792)

Specifier:

Designer:

CH Company:



Total Horizontal Product Length = 14-00-00

Reaction Sun	mary (שסשוו ו	ant) (ana)			
Bearing	Live	Dead	Snow	Wind	
B1, 4-1/2"	278 / 0	799 / 0	805/0		
B2, 2-3/4"	255/2	171 / 0	0/12		• •

Los	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.16	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	14-00-00	Тор		10			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	01-03-00	Тор	20	10			n\a
2	E75(i1685)	Unf. Lin. (lb/ft)	L	00-05-08	01-08-08	Тор		176	144		n\a
3	FC2 Floor Material	Unf. Lin. (lb/ft)	L	01-03-00	14-00-00	Тор	23	11			n\a
4	FC2 Floor Material	Unf. Lin. (lb/ft)	L	01-05-12	14-00-00	Top	17	9			n\a
5	E86(l1925)	Conc. Pt. (lbs)	L	00-02-12	00-02-12	Top		95	67		n\a
6	E75(11685)	Conc. Pt. (lbs)	L	01-07-04	01-07-04	Тор		255	547		n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1733 ft-lbs	23220 ft-lbs	7,5%	45	07-10-06
Neg. Moment	-520 ft-lbs	-23220 ft-lbs	2,2%	49	01-05-04
End Shear	497 lbs	11571 lbs	4.3%	45	12-11-12
Cont. Shear	572 lbs	11571 lbs	4.9%	1	02-05-00
Total Load Deflection	L/999 (0.067")	n\a	n\a	108	07-08-06
Live Load Deflection	L/999 (0.042")	n\a	n\a	160	07-08-06
Total Neg. Defl.	2xL/1998 (-0.023")	n\a	n\a	108	00-00-00
Max Defl.	0.067"	n\a	n\a	108	07-08-06
Span / Depth	15.7				

Bearing	Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1		4-1/2" x 3-1/2"	2485 lbs	29.5%	12.9%	Unspecified
B2		2-3/4" x 3-1/2"	596 lbs	10.1%	5.1%	Spruce-Pine-Fir



STRUCTURAL COMPONENT ONLY





## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING\Flush Beams\B21(i2792) (Flush Beam)

PASSED

August 26, 2020 09:26:19

**BC CALC® Member Report** 

**Build 7493** 

Job name: Address:

City, Province, Postal Code: HAMILTON

Customer: Code reports:

CCMC 12472-R

Dry | 2 spans | L cant.

File name:

**MOUNTAINASH 6** 

Description: 2ND FLR FRAMING\Flush Beams\B21(i2792)

Specifier:

Designer: CH

Company:

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

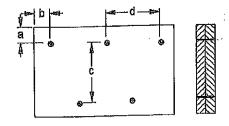
Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Cantilevers require sheathed bottom flanges, blocking at cantilever support and closure at ends.

## Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3"

c = 5-1/2" d= # 6"

· 1 · ·

Connectors are:

Naits

3.1/2" ARDOX SPIRAL



BWG NO. FAM 12885-20 STRUCTURAL COMPONENT ONLY

#### Disclosure

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BC CALC®, BC FRAMER® , AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



Passed

2ND FLR FRAMING\Flush Beams\B20(i3104) (Flush Beam) BC CALC® Member Report

Dry | 2 spans | L cant.

August 26, 2020 09:26:19

Build 7493

Job name:

Address:

City, Province, Postal Code: HAMILTON

Customer: Code reports:

CCMC 12472-R

File name:

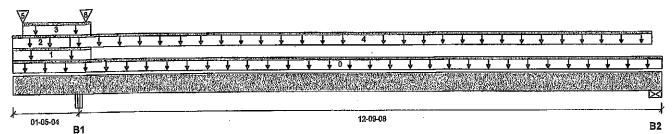
MOUNTAINASH 6

Description: 2ND FLR FRAMING\Flush Beams\B20(i3104)

Specifier:

Designer:

CH Company:



Total Horizontal Product Length = 14-02-12

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 4-1/2"	139 / 0	702 / 0	782 / 0	
B2, 5-1/2"	127 / 1	113 / 0	0/10	

l oa	ad Summary						Live	Dead	Snow	Wind	Tributary
		Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	14-02-12	Тор		10			00-00-00
1	E73(11679)	Unf. Lin. (lb/ft)	L	00-00-00	01-08-08	Тор		100			n\ <b>a</b>
2	FC2 Floor Material	Unf, Lin. (lb/ft)	L	00-00-00	01-03-00	Тор	8				n\a
3	E73(11679)	Unf. Lin. (lb/ft)	L	00-02-12	01-08-08	Тор		76	144		n\a
4	FC2 Floor Material	Unf, Lin. (lb/ft)	L	01-03-00	14-00-00	Тор	20	10			n\a
5	E73(I1679)	Conc. Pt. (lbs)	L	00-02-12	00-02-12	Тор		7	13		n\a
6	E73(i1679)	Conc. Pt. (lbs)	L	01-07-04	01-07-04	Тор		255	547		n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	940 ft-lbs	23220 ft-lbs	4.1%	45	08-00-05
Neg. Moment	-417 ft-lbs	-23220 ft-lbs	1.8%	49	01-05-04
End Shear	273 lbs	11571 lbs	2.4%	43	12-11-12
Cont. Shear	342 lbs	11571 lbs	3.0%	1	02-05-00
Total Load Deflection	L/999 (0.036")	n\a	n\a	108	07-08-06
Live Load Deflection	L/999 (0.021")	n\a	n <b>\</b> a	160	07-08-06
Total Neg. Defl.	2xL/1998 (-0.012")	n\a	n\a	108	00-00-00
Max Defl.	0.036"	n\a	n <b>\</b> a	108	07-08-06
Span / Depth	15.7				

Bearing	Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
	Beam	4-1/2" x 3-1/2"	2190 lbs	26.0%	11.4%	Unspecified
B2	Wall/Plate	5-1/2" x 3-1/2"	332 lbs	2.8%	1.4%	Spruce-Pine-Fir



8W8 NO. YAM/2686-20 STRUCTURAL COMPONENT ONLY





## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING\Flush Beams\B20(I3104) (Flush Beam)

PASSED

August 26, 2020 09;26:19

**BC CALC® Member Report** 

**Build 7493** Job name:

Address:

City, Province, Postal Code: HAMILTON

Customer: Code reports:

CCMC 12472-R

Dry | 2 spans | L cant.

File name: MOUNTAINASH 6

2ND FLR FRAMING\Flush Beams\B20(i3104) Description:

Specifier:

Designer: CH ·

Company:

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

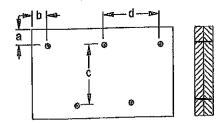
Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Cantilevers require sheathed bottom flanges, blocking at cantilever support and closure at ends.

## Connection Diagram: Full Length of Wember



a minlmum = 2" b minimum = 3" c = 5-1/2" d= 9988"

Connectors are: ...

3-1/2" ARDOX SPIRAL



6WG NO. THM 12886-20 STRUCTURAL COMPONENT ONLY

#### Disclosure

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BC CALC®, BC FRAMER® , AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





## Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B16 H(I6553) (Flush Beam)

Passed

B2

**BC CALC® Member Report** 

**Build 7493** Address:

Job name:

Dry | 1 span | No cant.

September 17, 2020 07:57:48

File name:

**MOUNTAINASH 6** 

1ST FLR FRAMING\Flush Beams\B16 H(i6553)

Description: Specifier:

Designer:

Customer: Code reports:

**B**1

**CCMC 12472-R** 

Company:

City, Province, Postal Code: WATERDOWN

16-01-08

Total Horizontal Product Length = 16-01-08

Reaction Sun	nmary (Down / U <sub>l</sub>	plift) (lbs)			
Bearing	Live	Dead	Snow	Wind	 
B1, 1-7/8"	110 / 0	94 / 0			
B2. 4-1/8"	124 / 0	101/0			

.	ا م	ad Summary						Live	Dead	Snow	Wind	Tributary
		Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.16	
: =	U.	Self-Weight	Unf. Lin. (lb/ft)	L.	00-00-00	16-01-08	Тор		5			00-00-00
	1	FC1 Floor Material	Unf. Lin. (lb/ft)	L.	00-00-00	16-01-08	Top	14	7			n\a
	2	FC1 Floor Material	Unf. Lin. (lb/ft)	L	10-06-14	16-01-08	Top	3	1			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1122 ft-lbs	11610 ft-lbs	9.7%	1	08-02-00
End Shear	266 lbs	5785 lbs	4.6%	1	14-11-14
Total Load Deflection	L/1302 (0.145")	n\a	18.4%	4	08-00-03
Live Load Deflection	L/999 (0.079")	n\a	n\a	5	08-00-03
Max Defl.	0.145"	n\a	n\a	4	08-00-03
Span / Depth	19.9				•

Bearin	g Supports	Dim, (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	1-7/8" x 1-3/4"	283 lbs	14.0%	7.1%	Spruce-Pine-Fir /Spruce-Pine-Fir
B2	Wall/Plate	4-1/8" x 1-3/4"	313 lbs	7.0%	3.6%	

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

cunforms to obc 2012

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



644 NO , YAM 12867-26 STRUCTURAL Disclosure ONLY

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## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B18(i6765) (Flush Beam)

PASSED

**BC CALC® Member Report** 

**Build 7493** 

Dry | 1 span | No cant.

September 17, 2020 07:57:48

Job name: Address:

City, Province, Postal Code: WATERDOWN

Customer: Code reports:

CCMC 12472-R

**MOUNTAINASH 6** 

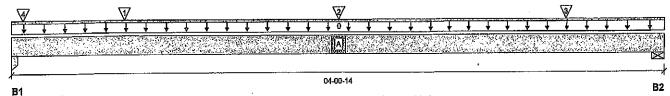
Wind

File name: 1ST FLR FRAMING\Flush Beams\B18(16765) Description:

Specifier:

Designer: ΑJ

Company:



Total Horizontal Product Length = 04-00-14

Snow

Reaction Summary (Down / Uplift) (lbs)

Live Dead Bearing 1009 / 0 535 / 0 B1, 1-3/4" 1207 / 0 662 / 0 B2, 5-1/2"

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag Description		Load Type	Ref.	Start	End.	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	04-00-14	Тор		10		•••	00-00-00
1	J2(i6650)	Conc. Pt. (lbs)	L	00-08-06	00-08-06	Top	690	345			n\a
,	J2(16682)	Conc. Pt. (lbs)	L	02-00-06	02-00-06	Top	751	375			n\a
3	-	Conc. Pt. (lbs)	L	03-05-10	03-05-10	Top	733	404			n\a
4	B7(i6767)	Conc. Pt. (lbs)	L	00-00-14	00-00-14	Тор	42	33			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	2066 ft-lbs	23220 ft-lbs	8.9%	1	02-00-06
End Shear	1623 lbs	11571 lbs	14.0%	1	00-11-04
Total Load Deflection	L/999 (0.006")	n\a	n\a	4	01-10-12
Live Load Deflection	L/999 (0.004")	n\a	n\a	5	01-10-12
Max Defl.	0,006"	n\a	n\a	4	01-10-12
Span / Depth	4.5				

Bearing	y Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Column	1-3/4" x 3-1/2"	2182 lbs	54.8%	29.2%	Unspecified
B2	Wall/Plate	5-1/2" x 3-1/2"	2637 lbs	22.3%	11.2%	Spruce-Pine-Fir

**Cautions** 

Concentrated side load(s) 2,4 are closer than 18" from end of member Please consult a technical representative or Professional of Record.

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBC 2012

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

OVINCE OF ONLY

DWG HO. YAM 1288820 STRUCTURAL COMPONENT ONLY





## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B18(i6765) (Flush Beam)

PASSED

**BC CALC® Member Report** 

**Build 7493** Job name: Address:

Dry | 1 span | No cant.

September 17, 2020 07:57:48

File name: **MOUNTAINASH 6** 

Description: 1ST FLR FRAMING\Flush Beams\B18(i6765)

Specifier:

Designer: ΑJ

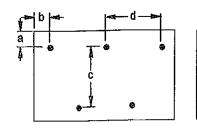
City, Province, Postal Code: WATERDOWN

Customer: Code reports:

CCMC 12472-R

Company:

#### Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3" c = 5-1/2"

Calculated Side Load = 52.1 lb/ft

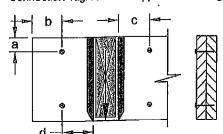
Connectors are:

. Nails

## Connection Diagrams: Concentrated Side Loads

Connection Tag: A

Applies to load tag(s): 3



a minimum = 2"

b minimum = 4"

c minimum = 4"

d maximum = 12"

Connectors are: 16d : Mails

3-1/2" ARDOX SPIRAL



## DWG NO. TAN /2006-20 STRUCTURAL COMPONENT ONLY

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PASSED

## 1ST FLR FRAMING\Flush Beams\B1(16766) (Flush Beam)

Dry | 1 span | No cant.

September 17, 2020 07:57:48

BC CALC® Member Report Build 7493

Job name: Address:

File name:

MOUNTAINASH 6 Description:

Wind

Specifier:

1ST FLR FRAMING\Flush Beams\B1(i6766)

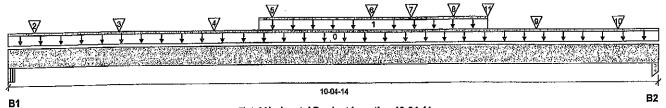
City, Province, Postal Code: WATERDOWN

Customer: Code reports:

CCMC 12472-R

Designer:

Company:



Total Horizontal Product Length = 10-04-14

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	<u>Dead</u>	
B1, 5-1/2"	3601 / 0	1979 / 0	
B2. 2-3/4"	2544 / 0	1352 / 0	

Los	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	10-04-14	Тор	:	10			00-00-00
1	STAIR	Unf. Lin. (lb/ft)	L	03-11-12	07-07-12	Top	240	120			n\a
,		Conc. Pt. (lbs)	L	00-05-02	00-05-02	Тор	1528	850			n\a
3	J2(i6759)	Conc. Pt. (lbs)	L	01-09-04	01-09-04	Тор	431	215			n\a
. 4	02(10700)	Conc. Pt. (lbs)	L	03-03-06	03-03-06	Тор	522	261			n\a
5	_	Conc. Pt. (lbs)	L	04-02-04	04-02-04	Top	707	415			n\a
6	J2(i6757)	Conc. Pt. (lbs)	L	05-09-04	05-09-04	Top	295	147			n\a
7	J2DJ(i6760)	Conc. Pt. (lbs)	L	06-04-12	06-04-12	Top	255	127			n\a
8	J2(i6756)	Conc. Pt. (lbs)	L.	07-01-04	07-01-04	Тор	332	166			n\a
9	-	Conc. Pt. (lbs)	L.	08-05-04	08-05-04	Top	547	273			n <b>\a</b>
10	-	Conc. Pt. (lbs)	L	09-09-04	09-09-04	Top	568	284			n\a
11	B5(i6752)	Conc. Pt. (lbs)	Ĺ	07-07-12	07-07-12	Top		34			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand <i>i</i> Resistance	Case	Location
Pos. Moment	14154 ft-lbs	23220 ft-lbs	61.0%	1	05-07-04
End Shear	4906 lbs	11571 lbs	42.4%	1	09-04-10
Total Load Deflection	L/342 (0.345")	n\a	70.2%	4	05-03-04
Live Load Deflection	L/526 (0.225")	n\a	68.5%	5	05-03-04
Max Defi.	0.345"	n\a	n\a	4	05-03-04
Span / Depth	12.4		'		

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand <i>l</i> Resistance Member	Materia
B1		5-1/2" x 3-1/2"	7875 lbs	95.8%	33.5%	Unspecified
B2	Column	2-3/4" x 3-1/2"	5506 lbs	88.1%	46.9%	Unspecified

Concentrated side load(s) 4 are closer than 18" from end of member. Please consult a technical representative or Professional of Record.



044 NO. YAW 12889-20 STRUCTURAL COMPONENT ONLY





## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B1(16766) (Flush Beam)

PASSED

**BC CALC® Member Report** 

Job name:

Dry | 1 span | No cant.

September 17, 2020 07:57:48

Build 7493

Address:

Customer:

Code reports:

City, Province, Postal Code: WATERDOWN

CCMC 12472-R

File name: Description:

**MOUNTAINASH 6** 

1ST FLR FRAMING\Flush Beams\B1(i6766)

Specifier:

ΑJ

Designer: Company:

**Notes** 

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

CONFORMS TO UBC 2012

AMENDED 2020

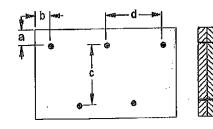
Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

## Connection Diagram: Full Length of Member



a minimum = 2" b minimum ≈ 3"

c = 5-1/2" d = 6"

Calculated Side Load = 1012.3 lb/ft

Connectors are:

Nails

3-1/2" ARDOX SPIRAL



bws no . Yam 12*80*9 STRUCTURAL COMPONENT ONLY

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Passed

1ST FLR FRAMING\Flush Beams\B17(I2700) (Flush Beam)

Dry | 1 span | No cant.

August 26, 2020 09:26:19

Build 7493

Job name: Address:

Customer:

Code reports:

City, Province, Postal Code: HAMILTON

**BC CALC® Member Report** 

CCMC 12472-R

File name:

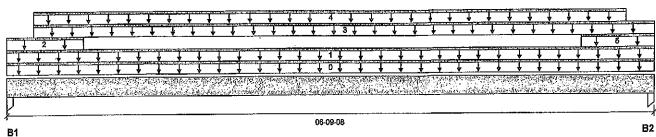
**MOUNTAINASH 6 EL 3** 

1ST FLR FRAMING\Flush Beams\B17(i2700) Description:

Specifier:

Designer:

Company:



Total Horizontal Product Length = 06-09-08

Reaction Sun	milaly (Down / O	hiari (ins)			
Bearing	Live	Dead	Snow	Wind	
B1, 3-1/2"	538 / 0	1073 / 0			
DO 2 4/0"	227 / 0	768 / 0			

Los	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	Ĺ.	00-00-00	06-09-08	Тор		10			00-00-00
1	5(1782)	Unf. Lin. (lb/ft)	L	00-00-00	06-09-08	Тор		81			n\a
2	5(1782)	Unf. Lin. (lb/ft)	L.	00-00-00	00-09-12	Тор	481	612			n\a
3	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-03-08	06-09-08	Тор	20	10			n\a
4	5(i782)	Unf. Lin. (lb/ft)	L	00-03-08	06-06-00	Тор	31	80			n\a
5	5(1782)	Unf. Lin. (lb/ft)	L	06-00-08	06-09-08	Тор	71	221			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1363 ft-lbs	15093 ft-lbs	9.0%	0	03-03-14
End Shear	1010 lbs	7521 lbs	13.4%	0	01-01-00
Total Load Deflection	L/999 (0.019")	n\a	n\a	4	03-04-12
Live Load Deflection	L/999 (0.004")	n\a	n <b>\a</b>	5.	03-03-14
Max Defl.	0.019"	n\a	n\a	4	03-04-12
Span / Depth	8.0				

Bearing	y Supports	Dim, (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Column	3-1/2" x 3-1/2"	1503 lbs	23.2%	15.5%	Unspecified
B2.	Column	3-1/2" x 3-1/2"	1075 lbs	16.6%	11.1%	Unspecified

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBG 2012

Calculations assume member is fully braced.

AMENDED 2020

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

TOVINCE OF OTHER

090 NO. 74812117 -80 STRUCTURAL COMPONENT ONLY





1ST FLR FRAMING\Flush Beams\B17(i2700) (Flush Beam)

Dry | 1 span | No cant.

PASSED

August 26, 2020 09:26:19

**BC CALC® Member Report** 

Bulld 7493

Job name:

Address:

City, Province, Postal Code: HAMILTON

Customer:

Code reports:

MOUNTAINASH 6 EL 3

File name: MOU

Description:

: 1ST FLR FRAMING\Flush Beams\B17(i2700)

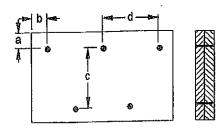
Specifier:

Designer:

Company:

Connection Diagram: Full Length of Member

CCMC 12472-R



a minimum = 2" b minimum = 3" c = 5-1/2" d = 66 " 66"

Connectors are:

... . ... , Nails

3-1/2" ARDOX SPIRAL



846 HB. TAM 12117 - 20 STRUCTURAL COMPONENT ONLY

#### Disclosure

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Passed

2ND FLR FRAMING/Flush Beams/B18(I2803) (Flush Beam)

Dry | 1 span | No cant.

August 26, 2020 09:26:19

**BC CALC® Member Report** Build 7493

Job name: Address:

Customer:

**MOUNTAINASH 6 EL 3** File name:

Wind

2ND FLR FRAMING\Flush Beams\B18(i2803) Description:

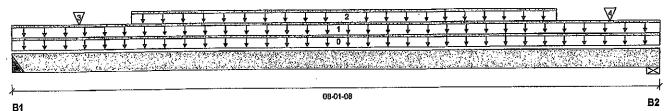
Specifier:

Designer: CH

Code reports:

City, Province, Postal Code: HAMILTON

Company: CCMC 12472-R



Total Horizontal Product Length = 08-01-08

Reaction Summary (Down / Unlift) (lbs)

Reaction Summary (Down / Opins) (1885)									
Bearing	Live	Dead	Snow						
B1. 4"	310 / 0	440 / 0							
B2, 3-1/2"	307 / 0	435 / 0							

Load Summary							Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	;
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-01-08	Тор		10			00-00-00
1	WALL	Unf. Lin. (lb/ft)	L	00-00-00	08-01-08	Top		60			n\a
2	Smoothed Load	Unf. Lin. (lb/ft)	L	01-06-00	06-10-00	Top	82	40			n\a
3	J6(i2742)	Conc. Pt. (lbs)	L	00-10-00	00-10-00	Top	101	50			n\a
⊿	J6(12742)	Conc. Pt. (lbs)	L	07-06-00	07-06-00	Тор	82	41			n\a

	Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
•	Pos. Moment	1874 ft-lbs	23220 ft-lbs	8,1%	1	03-10-00
	End Shear	839 lbs	11571 lbs	7.2%	1	01-01-08
	Total Load Deflection	L/999 (0.029")	n\a	n\a	4	04-01-00
	Live Load Deflection	L/999 (0.012")	n\a	n\a	5	04-01-00
	Max Defl.	0,029"	n\a	n\a	4	04-01-00
	Span / Depth	9.6				

Bea	ring Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Hanger	4" x 3-1/2"	1015 lbs	n\a	5.9%	HGUS410
B2	Wall/Plate	3-1/2" x 3-1/2"	1004 lbs	13.3%	6.7%	Spruce-Pine-Fir

#### Cautions

Header for the hanger HGUS410 is a Quadruple 1-3/4" x 9-1/2" LVL Beam.

Hanger model HGUS410 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-00-00.

CANFORMS TO OBC 2012

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



DWG NO. TAMI 2/16-20 STRUCTURAL COMPONENT ONLY





## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING\Flush Beams\B18(i2803) (Flush Beam)

Passed

August 26, 2020 09:26:19

**BC CALC® Member Report** 

**Build 7493** 

Job name:

Address:

City, Province, Postal Code: HAMILTON

Customer:

Code reports:

CCMC 12472-R

Dry | 1 span | No cant.

File name:

**MOUNTAINASH 6 EL 3** 

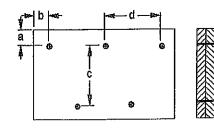
Description: 2ND FLR FRAMING\Flush Beams\B18(i2803)

Specifier:

Designer: CH

Company:

### Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3"

c = 5-1/2" d = 200 B 11

Calculated Side Load = 232.3 lb/ft

. Nails

3-1/2" ARDOX SPIRAL



946 NO. TAN 12118-20 STRUCTURAL COMPONENT ONLY

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Passed

#### 2ND FLR FRAMING\Flush Beams\B19(12627) (Flush Beam)

**BC CALC® Member Report** 

Dry | 1 span | No cant.

August 26, 2020 09:26:19

**Bulld 7493** Job name:

File name:

MOUNTAINASH 6 EL 3

Address:

City, Province, Postal Code: HAMILTON

Specifier:

Description: 2ND FLR FRAMING\Flush Beams\B19(i2627)

Çustomer:

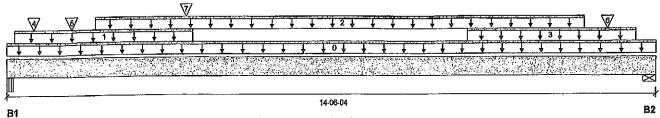
Designer:

CH

Code reports:

CCMC 12472-R

Company:



Total Horizontal Product Length = 14-06-04

Summary (Down / Unlift) (lbs)

Reaction Suit	Illiary (Down of	mit) (ina)		
Bearing	Live	Dead	Snow	Wine
B1. 4-1/2"	2856 / 0	1769 / 0		
D2 5 4/2"	3171 / 0	1798 / 0		

100	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	<u> </u>	00-00-00	14-06-04	Тор		19			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-02-04	04-01-04	Top	22	11			n\a
2	Smoothed Load	Unf. Lin. (lb/ft)	L	01-11-04	12-11-04	Top	342	170			n\a
3	STAIRS	Unf. Lin. (lb/ft)	L	10-03-04	14-00-12	Top	240	120			n\a
4	J7(j2837)	Conc. Pt. (lbs)	L	00-07-04	00-07-04	Top	324	162			n\a
5	J7(12847)	Conc. Pt. (lbs)	L.	01-05-04	01-05-04	Top	305	152			n\a
6	J7(i3121)	Conc. Pt. (lbs)	L	13-05-04	13-05-04	Top	325	162			n\a
7	B18((2803)	Conc. Pt. (lbs)	L	03-11-08	03-11-08	Тор	301	426			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	21829 ft-lbs	48297 ft-lbs	45.2%	1	07-05-04
End Shear	6416 lbs	23142 lbs	27.7%	1	13-03-04
Total Load Deflection	L/306 (0.542")	n\a	78.6%	4	07-02-04
Live Load Deflection	L/489 (0.339")	n\a	73.6%	5	07-02-04
Max Defl.	0.542"	n\a	n\a	4	07-02-04
Span / Depth	17.4				

Bearing	y Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material_
B1	Beam	4-1/2" x 7"	6495 lbs	38.6%	16.9%	Unspecified
B2	Wall/Plate	5-1/2" x 7"	7004 lbs	29.6%	14.9%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBC 2012

Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-00-00.

AMENDED 2020

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition. importance Factor: Normal Part code: Part 9

OWS NO. TAN /2/19 -20 STRUCTURAL COMPONENT ONLY

POLINCE OF ONE





Passed

August 26, 2020 09:26:19

2ND FLR FRAMING\Flush Beams\B19(i2627) (Flush Beam)

**BC CALC® Member Report** 

**Build 7493** 

Job name:

Address: City, Province, Postal Code: HAMILTON

Customer: Code reports: CCMC 12472-R

Dry | 1 span | No cant.

File name: **MOUNTAINASH 6 EL 3** 

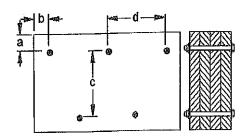
2ND FLR FRAMING\Flush Beams\B19(i2627) Description:

Specifier:

CH

Designer: Company:

Connection Diagram: Full Length of Member



a minimum = 2-1/2" b minimum = 3-1/8" c = 4-1/2" d = 185"/

Calculated Side Load = 731.0 lb/ft Bolts are assumed to be Grade A307 or Grade 2 or higher. Connectors are: 5/8 in. Staggered Through Bolt



DWG NO. TAM /2-1/9 -20 STRUCTURAL COMPONENT ONLY

### **Disclosure**

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain installation Guide or ask questions, please call (800)232-0788 before Installation.

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PASSED

2ND FLR FRAMING\Dropped Beams\B13 DR(I1726) (Dropped Beam)

**BC CALC® Member Report** 

Dry | 1 span | No cant.

February 10, 2020 11:38:52

**Build 7239** Job name:

MOUNTAINASH 6 EL 1.mmdl

Address:

File name: Description: 2ND FLR FRAMING\Dropped Beams\B13 DR(I1726)

Wind

City, Province, Postal Code:

Specifier:

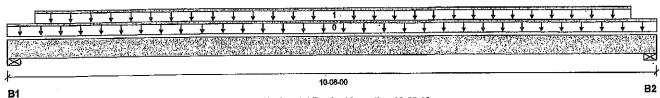
Customer:

Designer:

Code reports:

CCMC 12472-R

Company:



Total Horizontal Product Length = 10-08-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow
B1, 4"	1619 / 0	861 / 0	
B2, 4"	· 1618 / 0	861 / 0	

1	oad Summary							Live	Dead	Snow	Wind	Tributary
	g Description	÷	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight		Unf. Lin. (lb/ft)	L,	00-00-00	10-08-00	Тор		10			00-00-00
1	Smoothed Load	-	Unf. Lin. (lb/ft)	L	00-05-04	10-02-12	Top	313	156			nla

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	8926 ft-lbs	23220 ft-lbs	38.4%	1	05-08-12
End Shear	3236 lbs	11571 lbs	28,0%	1	01-01-08
Total Load Deflection	L/523 (0.232")	n\a	45.9%	4	04-11-04
Live Load Deflection	L/800 (0.152")	n\a	45.0%	5	04-11-04
Max Defl.	0.232"	n\a	n\a	4	04-11-04
Span / Depth	12.8				

Bearing	Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand <i>i</i> Resistance <u>Member</u>	Material	
B1	Wall/Plate	4" x 3-1/2"	3504 lbs	18.8%	20.5%	Spruce-Pine-Fir	
B2	Wall/Plate	4" x 3-1/2"	3503 lbs	18.8%	20.5%	Spruce-Pine-Fir	

OVINCE OF

148 NO. TAM6001 -20 STRUCTURAL COMPONENT ONLY

#### **Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

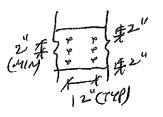
CONFORMS TO OBC 2012

Calculations assume unbraced length of Top: 00-02-08, Bottom: 00-02-08.

Resistance Factor phi has been applied to all presented results per CSA O86. AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



PROVIDE 3 ROWS OF 3½" ARDOX SPIRAL NAILS @ /2\_"O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN.2"LUMBER EDGE/END DISTANCE, DO NOT USE AIR NAILS

#### Disclosure

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Passed

2ND FLR FRAMING\Flush Beams\B10(i2111) (Flush Beam)

Dry | 1 span | No cant.

February 10, 2020 11:38:52

**BC CALC® Member Report Build 7239** 

Job name: Address:

City, Province, Postal Code:

Customer:

Code reports:

File name:

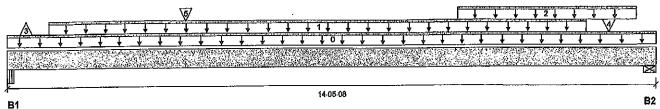
MOUNTAINASH 6 EL 1.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B10(i2111)

Specifier:

Designer:

Company:



Total Horizontal Product Length = 14-05-08

Reaction Summary (Down / Uplift) (lbs	Reaction Su	mmary (Down	/U	(tilla	(lbs)
---------------------------------------	-------------	-------------	----	--------	-------

CCMC 12472-R

Reaction Sur	ımary (Down / Upl	ift) (lbs)			
Bearing	Live	Dead	Snow	Wind	 
B1, 4-1/2"	2574 / 597	1313 / 0			
B2, 5-1/2"	2883 / 0	1649 / 0			

ما	ad Summary						Live	Dead	Snow
Tag	•	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00
0	Self-Weight	Unf. Lin. (lb/ft)	L,	00-00-00	14-05-08	Top		19	
1	Smoothed Load	Unf. Lin. (lb/ft)	L	00-11-00	12-11-00	Тор	353	.176	
2	STAIR	Unf. Lin. (lb/ft)	L	10-00-00	14-00-00	Тор	120	60	
3	J6(i1586)	Conc. Pt. (lbs)	L	00-05-00	00-05-00	Top	-597	-282	
4	J6(12121)	Conc. Pt. (lbs)	L	13-05-00	13-05-00	Top	338	169	PROFE
5	B11(i2153)	Conc. Pt. (lbs)	L	03-11-10	03-11-10	Тор	305	406	O STATE

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	21228 ft-lbs	48297 ft-lbs	44.0%	1	07-05-00
End Shear	5935 lbs	23142 lbs	25.6%	1	13-02-08
Total Load Deflection	L/317 (0.52")	n\a	75.6%	6	07-02-00
Live Load Deflection	L/508 (0.325")	n\a	70.9%	8	07-02-00
Max Defl.	0.52"	n\a	n\a	6	07-02-00
Span / Depth	17.4	·			

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Beam	4-1/2" x 7"	5502 lbs	28.4%	14.3%	Spruce-Pine-Fir
B2	Wall/Plate	5-1/2" x 7"	6387 lbs	27.0%	13.6%	Spruce-Pine-Fir

#### **Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

CONFORMS TO OBE 2012

Resistance Factor phi has been applied to all presented results per CSA O86. AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086. Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

The BOLT MULTI-PLY MOMBERS USING

of the Language of the Columbians I Notes & 16"0/L.

8W8 NO. TAM 6092-20 STRUCTURAL DISCIOSURE ONLY

ON NOE OF ONE

Wind

1.15

Tributary

00-00-00 n\a n\a n\a n\a

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## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING\Flush Beams\B11(I2153) (Flush Beam)

Passed

February 10, 2020 11:38:52

**BC CALC® Member Report** 

**Build 7239** Job name:

Address:

City, Province, Postal Code:

Customer:

Code reports:

Dry | 1 span | No cant.

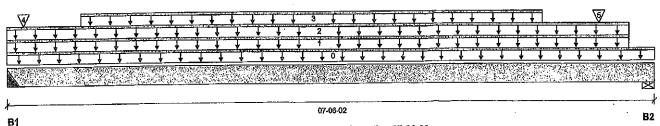
MOUNTAINASH 6 EL 1.mmdi

File name: Description: 2ND FLR FRAMING\Flush Beams\B11(i2153)

Specifier:

Designer:

Company:



Total Horizontal Product Length = 07-06-02

Seaction Summary (Down / Uplift) (lbs)

CCMC 12472-R

Mearing Ann	minimy (Dominio		_	
Bearing	Live	Dead	Snow	Wind
B1, 2"	307 / 0	409 / 0		
B2, 3-3/4"	290 / 0	393 / 0		

Lac	ad Summary	•					Live	Dead	Snow	Wind
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.66	1.00	1.15
0	Self-Weight	Unf. Lln. (lb/ft)	L	00-00-00	07-06-02	Тор		10		
1	WALL	Unf. Lin. (lb/ft)	L	00-00-00	07-02-06	Тор		60		
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	07-02-06	Тор	6	3	.1186	g winderstand the same of the same of
3	Smoothed Load	Unf. Lin. (lb/ft)	L	00-10-06	06-02-06	Тор	76	38	ORON	essio
4	J5(11619)	Conc. Pt. (lbs)	L	00-02-06	00-02-06	Тор	70			
5	J5(11506)	Conc. Pt. (lbs)	L	06-10-06	06-10-06	Тор	77	39	a a u	162
			Factored	Dem	and/			14	3 40	TOOLII A

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1645 ft-lbs	23220 ft-lbs	7.1%	1	03-11-06
End Shear	744 lbs	11571 lbs	6.4%	1	. 06-04-14
Total Load Deflection	L/999 (0.022")	n\a	n\a	4	03-08-06
Live Load Deflection	L/999 (0.009")	n\a	n\a	5	03-08-06
Max Defl.	0,022"	n\a	n\a	4	03-08-06
Span / Depth	9.0				

Rea	ring Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material	
B1	Hanger	2" x 3-1/2"	972 lbs	n\a	11.4%	Hanger	
B2	\Mail/Plate	3-3/4" x 3-1/2"	926 lbs	11.5%	5.8%	Spruce-Pine-Fir	

#### **Cautions**

Hanger model Hanger was not found. Hanger has not been analyzed for adequate capacity.

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86. AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086. Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

PROVIDE 3 ROWS OF 3½" ARDOX SPIRAL NAILS @ /2-" 0/C FOR MULTI-PLY NAILING, MAINTAIN A MIN.2" LUMBER EDGE/END DISTANCE. DO NOT USE AIR NAILS

00-00-00 n\a n∖a SIONA NOWNCE OF ON

Tributary

dwe nd . Tan*6063* -20 STRUCTURAL COMPONENT ONLY

#### Disclosure

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> BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAMIM, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





Passed

February 10, 2020 11:38:52

2ND FLR FRAMING\Flush Beams\B12(i1651) (Flush Beam)

**BC CALC® Member Report** 

CCMC 12472-R

**Build 7239** Job name:

Address: City, Province, Postal Code:

Customer:

Code reports:

Dry | 1 span | No cant.

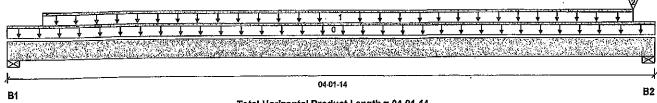
MOUNTAINASH 6 EL 1.mmdl

File name: 2ND FLR FRAMING\Flush Beams\B12(I1651)

Description: Specifier:

Designer:

Company:



Total Horizontal Product Length = 04-01-14

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead		
B1, 5-1/2"	52 / 0	36/0		
B2, 3-1/2"	51/0	35/0		

Load Summary						Live	Dead	Snow	Wind	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0 Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	04-01-14	Top		5			00-00-00
1 FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-02-12	04-00-02	Top	27	13			n\a
2 FC2 Floor Material	Conc. Pt. (lbs)	L	04-00-02	04-00-02	Тор	2	1		CESSA	n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	98 ft-lbs	11610 ft-lbs	0.8%	1	02-01-15
End Shear	57 lbs	5785 lbs	1.0%	1	01-03-00
Total Load Deflection	L/999 (0.001")	n\a	n\a	4	02-01-15
Live Load Deflection	L/999 (0")	n\a	n\a	5	02-01-15
Max Defl.	0.001"	n\a	n\a	4	02-01-15
Span / Depth	4.5				

Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	5-1/2" x 1-3/4"	123 lbs	2.1%	1.0%	Spruce-Pine-Fir
B2	Wall/Plate	3-1/2" x 1-3/4"	121 lbs	3.2%	1.6%	Spruce-Pine-Fir

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBC 2012

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086. AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9



BWG NO. TAM 6084-28 STRUCTURAL COMPONENT ONLY

#### **Disclosure**

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## 2ND FLR FRAMING\Flush Beams\B14(i2151) (Flush Beam)

Dry | 1 span | No cant.

February 10, 2020 11:38:52

Passed

Tributary

00-00-00

**BC CALC® Member Report Build 7239** 

Job name:

Address: City, Province, Postal Code:

Customer:

Code reports:

File name:

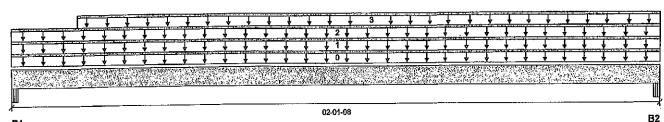
MOUNTAINASH 6 EL 1.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B14(i2151)

Specifier:

Designer:

Company. CCMC 12472-R



**B**1

Total Horizontal Product Length = 02-01-08

Reaction Sun	nmary (Down / G	ibiliri (ins)				
Bearing	Live	Dead	Snow	Wind		
B1, 2-5/8"	43/0	120 / 0	42 / 0			
B2. 4-1/8"	55 / 0	138 / 0	47 / 0	•	•	

Load Summary						Live	Dead	Snow	Wind
Tag Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15
0 Self-Weight	Unf. Lin. (lb/ft)	L ,.	00-00-00	02-01-08	Тор		10		
1 SNOW	Unf. Lin. (lb/ft)	L	00-00-00	02-01-08	Тор	22	20	42	•
2 WALL	Unf, Lin, (lb/ft)	L	00-00-00	02-01-08	Тор		80		THE PARTY NAMED IN
3 FC2 Floor Material	Unf. Lin. (lb/ft)	Ĺ.	00-02-10	02-01-08	Тор	27	13	COFE	SSION

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	96 ft-lbs	23220 ft-lbs	0.4%	1	01-00-02
End Shear	2 lbs	7521 lbs	n\a	0	01-00-02
Total Load Deflection	L/999 (0")	n\a	n\a	35	01-00-02
Max Defl.	0"	n\a	n\a	35	01-00-02
Span / Depth	2.1				

Rearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Materia!	
B1	Beam	2-5/8" x 3-1/2"	168 lbs	5.3%	2.3%	Unspecified	_
B2	Ream	4-1/8" x 3-1/2"	302 lbs	3.9%	1.7%	Unspecified	

#### Notes

verification.

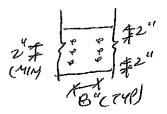
Design meets Code minimum (L/240) Total load deflection criteria.

CONFORMS TO OBC 2012

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA Q86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086. Unbalanced snow loads determined from building geometry were used in selected product's

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9



PROVINE 3 ROWS OF 312" ARDOX SPIRAL NAILS @ 8 "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN. 2" LUMBER EDGE/END DISTANCE, DO NOT USE AIR NAILS



## DWG NO. YAN 6085-20 STRUCTURAL COMPONENT ONLY

#### Disclosure

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BC CALC®, BC FRAMER®, AJSTM ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAMIM, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING\Flush Beams\B16(i2154) (Flush Beam)

PASSED

February 10, 2020 11:38:52

**BC CALC® Member Report** 

**Build 7239** 

Job name: Address:

City, Province, Postal Code:

Customer: Code reports: Dry | 1 span | No cant.

File name: MOUNTAINASH 6 EL 1.mmdl

2ND FLR FRAMING\Flush Beams\B16(i2154) Description:

Specifier:

Designer:

Company:

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Walter Str.	CONTROL	400000			120			34.511		4 feet 19 feet		E PAR		影响	3.73			M. 37 (3) 40	
Britan Warra	oma <del>dini ad</del>	A Seminar	SP 554 W	Herry Car	. (3+3y (45 t	College States	P. Fallowski	No. of Parties	11.489	CALLEST NATE	(Massalla 3	igus jarjis	( ) [ v · a ]	mile 27,850	विविध्य करन	Section Section	1200 ALC:	4-9-10/2003 4-9-10/2003	W. F. 1984

В1

Total Horizontal Product Length = 08-07-00

Reaction Summary (Down / Uplift) (lbs)

CCMC 12472-R

( /cashon can	minute in a second	A		
Bearing	Live	Dead	Snow	Wind
B1, 5-1/2"	184 / 0	517/0	187 / 0	
B2, 5-1/2"	120 / 0	452 / 0	65 / 0	

l na	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag		Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-07-00	Тор		10			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	08-07-00	Тор	20	10			n\a
2	WALL	Unf. Lin. (lb/ft)	L	00-00-00	08-06-01	Top		80			n\a
3	WINDOW	Conc. Pt. (lbs)	L	00-05-00	00-05-00	Top	66	60	126		n\a
4	WINDOW	Conc. Pt. (lbs)	L	04-05-00	04-05-00	Тор	66	60	126	- PERSONAL PROPERTY OF	n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1221 ft-lbs	15093 ft-lbs	8.1%	0	04-05-00
End Shear	468 lbs	7521 lbs	6.2%	0	07-04-00
Total Load Deflection	L/999 (0.028")	n\a	n\a	35	04-03-13
Live Load Deflection	L/999 (0.01")	n\a	n\a	51	04-03-13
Max Defl.	0.028"	n\a	n\a	35	04-03-13
Span / Depth	9.8				

Bearing	Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1 \	/Vall/Plate	5-1/2" x 3-1/2" 5-1/2" x 3-1/2"	723 lbs 633 lbs	9.4% 8.2%	4.7% 4.1%	Spruce-Pine-Fir Spruce-Pine-Fir

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86. AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086. Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

PROVIDE 3 ROWS OF 3½" ARDOX SPIRAL NAILS @ /2- 0/C FOR MULTI-PLY NAILING, MAINTAIN A MIN.2"LUMBER EDGE/END DISTANCE, BONOTUSE AIR NAILS

conforms to obc 2012



#### Disclosure

Use of the Bolse Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of Input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of sultability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™, ·· ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

DWG NO. FAM 6086-20 STRUCTURAL





PASSED

2ND FLR FRAMING\Flush Beams\B8(i2152) (Flush Beam)

Dry | 1 span | No cant.

February 10, 2020 11:38:52

**BC CALC® Member Report Build 7239** 

Job name: Address:

City, Province, Postal Code:

Customer:

Code reports:

File name:

MOUNTAINASH 6 EL 1.mmdl

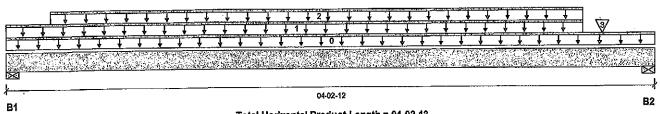
Description: 2ND FLR FRAMING\Flush Beams\B8(i2152)

Wind

Specifier:

Designer:

Company:



Total Horizontal Product Length = 04-02-12

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead
B1, 3-1/2"	41/0	144 / 0
B2. 5-1/2"	36 / 0	143 / 0

CCMC 12472-R

d Summary				:		Live
	Load Type	Ref.	Start	End	Loc.	1.00
	Unf. Lin. (lb/ft)	L	00-00-00	04-02-12	Top	
		L	00-00-00	03-09-04	Тор	20
	• •	L	00-03-08	03-09-04	Top	
	Conc. Pt. (lbs)	L	03-10-10	03-10-10	Тор	1
	d Summary Description Self-Weight FC2 Floor Material WALL FC2 Floor Material	DescriptionLoad TypeSelf-WeightUnf. Lin. (lb/ft)FC2 Floor MaterialUnf. Lin. (lb/ft)WALLUnf. Lin. (lb/ft)	Description     Load Type     Ref.       Self-Weight     Unf. Lin. (lb/ft)     L       FC2 Floor Material     Unf. Lin. (lb/ft)     L       WALL     Unf. Lin. (lb/ft)     L	Description         Load Type         Ref.         Start           Self-Weight         Unf. Lin. (lb/ft)         L         00-00-00           FC2 Floor Material         Unf. Lin. (lb/ft)         L         00-00-00           WALL         Unf. Lin. (lb/ft)         L         00-03-08	Description         Load Type         Ref.         Start         End           Self-Weight         Unf. Lin. (lb/ft)         L         00-00-00         04-02-12           FC2 Floor Material         Unf. Lin. (lb/ft)         L         00-00-00         03-09-04           WALL         Unf. Lin. (lb/ft)         L         00-03-08         03-09-04	Description         Load Type         Ref.         Start         End         Loc.           Self-Weight         Unf. Lin. (lb/ft)         L         00-00-00         04-02-12         Top           FC2 Floor Material         Unf. Lin. (lb/ft)         L         00-00-00         03-09-04         Top           WALL         Unf. Lin. (lb/ft)         L         00-03-08         03-09-04         Top

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	181 ft-lbs	15093 ft-lbs	1.2%	0	02-00-06
End Shear	106 lbs	7521 lbs	1.4%	0	01-01-00
Total Load Deflection	L/999 (0.001")	n\a	n\a	4	02-00-06
Live Load Deflection	L/999 (0")	n\a	n\a	5	02-00-06
Max Defl.	0.001"	n\a	n\a	4	02-00-06
Snan / Denth	4.6				

Bearing Supports Dim. (LxW)			Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material	
٠	B1		3-1/2" x 3-1/2"	202 lbs	4.1%	2.1%	Spruce-Pine-Fir
	B2	Wall/Plate	5-1/2" x 3-1/2"	201 lbs	2.6%	1.3%	Spruce-Pine-Fir

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

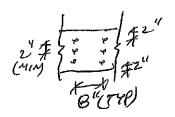
Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86. AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



PROVIDE 3 ROWS OF 3½" ARDOX SPIRAL NAILS @ & "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN. 2" LUMBER EDGE/END DISTANCE, BONOTUSE AIR NAILS 146 HO. TAW 6087-20 STRUCTURAL COMPONENT ONLY

#### Disclosure

ROVINCE OF

Dead

0.65

10 10

60

Snow

1.00

Wind

1.15

Tributary

00-00-00

n\a

n\a n\a

Use of the Bolse Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input GINFORMS TO OBC 2012 must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current installation Gulde and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

> BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAMIM, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

PASSED

1ST FLR FRAMING\Flush Beams\B1(i1955) (Flush Beam)

Dry | 2 spans | No cant.

February 10, 2020 11:38:52

**BC CALC® Member Report Bulld 7239** 

Job name: Address:

City, Province, Postal Code:

Customer:

Code reports:

MOUNTAINASH 6 EL 1.mmdl

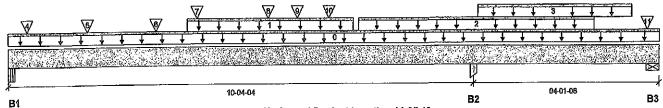
File name: 1ST FLR FRAMING\Flush Beams\B1(i1955) Description:

Wind

Specifier:

Designer:

Company:



Total Horizontal Product Length = 14-05-10

CCMC 12472-R

Reaction Summary (Down / Uplift) (lbs)
Bearing Live Dead 3149 / 27 1723 / 0 B1, 5-1/4" 5407 / 0 2876 / 0 B2, 4-1/2" 4368 / 1288 3245 / 0 B3, 5-1/2"

دم ا	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Seif-Weight	Unf, Lin. (lb/ft)	Ļ	00-00-00	14-05-10	Тор		10			00-00-00
1	STAIR	Unf. Lin. (lb/ft)	L	03-11-10	07-07-10	Тор	240	120			n\a
2	Smoothed Load	Unf. Lin. (lb/ft)	L	07-09-02	13-01-02	Тор	323	161			n\a
3	STAIR	Unf. Lin. (lb/ft)	L	10-05-06	13-10-10	Тор	240	120			n\a
4	J2(i2040)	Conc. Pt. (lbs)	L	00-05-02	00-05-02	Тор	1454	789			n\a
5	J2(i2099)	Conc. Pt. (lbs)	L	01-09-02	01-09-02	Top	431	215			n\a
6	-	Conc. Pt. (lbs)	L	03-03-04	03-03-04	Top	522	261			n\a
7	_	Conc. Pt. (lbs)	L	04-02-02	04-02-02	Top	710	417			n\a
8	J2(I2016)	Conc. Pt. (lbs)	L	05-09-02	05-09-02	Тор	295	147			n\a
9	J2DJ(i2081)	Conc. Pt. (lbs)	L	06-04-10	06-04-10	Тор	255	127			n\a
10	J2(i1959)	Conc. Pt. (lbs)	L	07-01-02	07-01-02	Top	332	166			n\a
11		Conc. Pt. (lbs)	L	14-02-06	14-02-06	Тор	3733	3598			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	9867 ft-lbs	23220 ft-lbs	42.5%	2	04-05-02
Neg. Moment	-10975 ft-lbs	-23220 ft-lbs	47.3%	1	10-04-04
End Shear	3496 lbs	11571 lbs	30.2%	2	01-02-12
Cont. Shear	5922 lbs	11571 lbs	51.2%	1	09-04-08
Total Load Deflection	L/584 (0.205")	n\a	41.1%	9	04-11-02
Live Load Deflection	L/886 (0.135")	nla	40.7%	12	04-11-02
Total Neg. Defl.	L/999 (-0.021")	n\a	n\a	9	11-10-09
Max Defl.	0.205"	n\a	n\a	9	04-11-02
Span / Depth	12.6				

Bearing	Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Beam	5-1/4" x 3-1/2"	6878 lbs	70.1%	30.7%	Unspecified
B2	Column	4-1/2" x 3-1/2"	11705 lbs	91.5%	60.9%	Unspecified
			40007 lbs	00.00/	4E 00/	Compo Dino Eir
B3	Wall/Plate	5-1/2" x 3-1/2"	10607 lbs	89.6%	45.2%	Spruce-Pine-Fir



8W8 NB . FAM*6086* -20 STRUCTURAL COMPONENT ONLY





# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B1(i1955) (Flush Beam)

Passed

February 10, 2020 11:38:52

BC CALC® Member Report

**Build 7239** Job name: Address:

City, Province, Postal Code:

Customer: Code reports: Dry | 2 spans | No cant.

File name:

MOUNTAINASH 6 EL 1.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B1(i1955)

Specifier:

Designer:

Company:

**CCMC 12472-R** 

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

COMPORMS TO OBG 2012

Calculations assume member is fully braced.

AMENDED 2020

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

PROVIDES ROWS OF 31/2" ARDOX SPIRAL NAILS @ 8 " 0/C FOR MULTI-PLY NAILING, MAINTAIN A MIN.2" LUMBER EDGE/END BISTANGE, BONOT UBS AIN NAILS



DWG NO . TAN 6069-20 STRUCTURAL COMPONENT ONLY

Disclosure

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# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

Passed

February 10, 2020 11:38:52

1ST FLR FRAMING\Flush Beams\B2(i1994) (Flush Beam) Dry | 1 span | No cant.

BC CALC® Member Report

**Build 7239** Job name:

Address:

City, Province, Postal Code:

Customer:

Code reports:

MOUNTAINASH 6 EL 1.mmdl

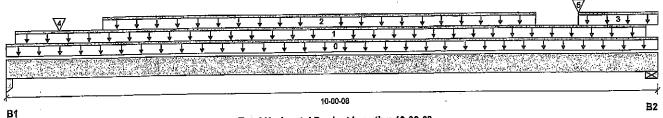
File name: Description: 1ST FLR FRAMING\Flush Beams\B2(i1994)

Specifier:

Designer:

**CCMC 12472-R** 

Company:



#### Total Horizontal Product Length = 10-00-08

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead
B1, 1-3/4"	985 / 0	834 / 0
B2, 1-7/8"	942 / 0	810 / 0

اما	d Cummanı						Live
Tag	ad Summary  Description	Load Type	Ref.	Start	End	Loc.	1.00
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	10-00-08	Top	
4	WALL	Unf. Lin. (lb/ft)	L.	00-01-12	09-10-02	Top	
2	Smoothed Load	Unf. Lin. (lb/ft)	L	01-05-10	08-01-10	Тор	210
_		Unf. Lin. (lb/ft)	ì	08-09-10	10-00-08	Top	22
3	FC1 Floor Material	Conc. Pt. (lbs)	ī	00-09-10	00-09-10	Top	244
4	J4(i1985)		<u>-</u>	08-09-10	08-09-10	Top	262
5	J4(i1973)	Conc. Pt. (lbs)	L	00-08-10	00-09-10	тор	202

Controls Summary	Factored Demand	Factored Resistance	Demand <i>l</i> Resistance	Case	Location
Pos. Moment	6498 ft-lbs	23220 ft-lbs	28.0%	1	04-09-10
End Shear	2370 lbs	11571 lbs	20.5%	1	09-01-02
Total Load Deflection	L/728 (0.163")	n\a	33.0%	4	05-00-10
Live Load Deflection	L/999 (0.089")	n\a	n\a	5	05-00-10
Max Defl.	0.163"	n\a	n\a	4	05-00-10
Span / Depth	12.5				•

Bearing	Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Resistance Member	<u> </u>
B1		1-3/4" x 3-1/2" 1-7/8" x 3-1/2"	2520 lbs 2426 lbs	50.7% 60.1%	33.7% 30.3%	Unspecified Spruce-Pine-Fir

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

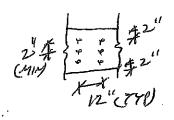
CONFORMS TO OBC 2012

Calculations assume member is fully braced.

AMENDED 2020 Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



PROVIDE3 ROWS OF 31/2" ARDOX SPIRAL NAILS @ /2- "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN. 2" LUMBER EDGE/END DISTANCE DO NOT USE AIR NAILS



Wind

Snow

Dead

Tributary

### Disclosure

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# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B3(i1931) (Flush Beam)

PASSED

February 10, 2020 11:38:52

**BC CALC® Member Report** 

**CCMC 12472-R** 

**Build 7239** 

Job name: Address:

City, Province, Postal Code:

Customer: Code reports: Dry | 1 span | No cant.

File name:

MOUNTAINASH 6 EL 1.mmdl

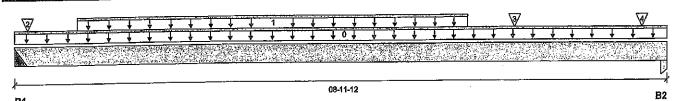
Wind

Description: 1ST FLR FRAMING\Flush Beams\B3(i1931)

Specifier:

Designer:

Company:



Total Horizontal Product Length = 08-11-12

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead
B1, 2"	358 / 0	222 / 0
B2, 3-1/2"	810 / 0	999 / 0

۱.	ad Cummoni						Live	Dead	Snow	Wind	Tributary
Tag	ad Summary  Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-11-12	Top		10			00-00-00
4	Smoothed Load	Unf. Lin. (lb/ft)	L	00-10-04	06-02-04	Top	80	40			n\a
2	J7(12069)	Conc. Pt. (lbs)	L	00-02-04	00-02-04	Top	67	34			n\a
2	J7(i2009) J7(i2079)	Conc. Pt. (lbs)	Ĺ	06-10-04	06-10-04	Top	108	54			n\a
ې. ا	3/(120/8)	Conc. Pt. (lbs)	L	08-07-07	08-07-07	Top	559	830			_ n∖a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1714 ft-lbs	23220 ft-lbs	7.4%	1	04-02-04
End Shear	706 lbs	11571 lbs	6.1%	1	07-10-12
Total Load Deflection	L/999 (0.032")	n\a	n\a	4	04-04-04
Live Load Deflection	L/999 (0.02")	n\a	n\a	5	04-04-04
Max Defl.	0.032"	n\a	n\a	4	04-04-04
Span / Denth	10.9				

Rearing	ı Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Hanger	2" x 3-1/2"	813 lbs	n\a	9.5%	Hanger
B2	Column	3-1/2" x 3-1/2"	2463 lbs	24.8%	16.5%	Unspecified

Hanger model Hanger was not found. Hanger has not been analyzed for adequate capacity.

Design meets Code minimum (L/240) Total load deflection criteria.

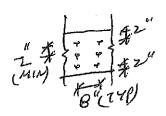
Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-00-00. CANFORMS TO OBC 2012

Hanger Manufacturer: Unassigned

AMENDED 2020 Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86. Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9



PROVIDE3 ROWS OF 3½" ARDOX SPIRAL NAILS @ 8 " O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN 2" LUMBER EDGE/END DISTANCE, DO NOT USE AIR NAILS



OVO NO. TAN 6090-20 STRUCTURAL COMPONENT ONLY

### **Disclosure**

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# Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B4(i2084) (Flush Beam)

PASSED

**BC CALC® Member Report** 

Dry | 1 span | No cant.

February 10, 2020 11:38:52

**Build 7239** 

Job name:

Address: City, Province, Postal Code:

Customer:

Code reports:

File name:

MOUNTAINASH 6 EL 1.mmdi

Description: 1ST FLR FRAMING\Flush Beams\B4(i2084)

Specifier:

Designer:

Company:

<b>†</b> †	4	4	<b></b>	¥	+	ŧ_		- ₩	₩			_ +	4	, 4	<del>↓</del>	<b>+</b>	₩		<b>+</b>	<u> </u>	₩.	+		+	<u>+</u>	<u> </u>	_₩	₩	₩.
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R1

06-06-00 Total Horizontal Product Length = 06-05-00

Reaction Summary (Down / Uplift) (lbs)

I WALLAND II AMI	Ittitudi & (m. a.e m.			
Bearing	Live	Dead	Snow	Wind
B1, 2"	344 / 0	618 / 0		
B2. 3-1/2"	167 / 0	496 / 0		

**CCMC 12472-R** 

Loa	ad Summary	1					Live
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	06-05-00	Top	
1	3(i760)	Unf. Lin. (lb/ft)	L	00-00-00	06-05-00	Тор	
2	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	06-05-00	Top	19
3	3(i760)	Unf. Lin. (lb/ft)	L	00-00-00	00-06-06	Top	427
4	3(i760)	Unf. Lin. (lb/ft)	L	00-00-02	06-01-08	Top	20
5	3(i760)	Unf. Lin. (lb/ft)	L	05-08-00	06-05-00	Top	53

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	748 ft-lbs	7546 ft-lbs	9.9%	0	03-01-12
End Shear	594 lbs	3761 lbs	15.8%	0	00-11-08
Total Load Deflection	L/999 (0.02")	n\a	n\a	4	03-01-12
Live Load Deflection	L/999 (0.006")	n\a	n\a	5	03-00-15
Max Defl.	0.02"	n\a	n\a	4	03-01-12
Snan / Denth	7.7				

Bearing	ı Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Hanger	2" x 1-3/4"	865 lbs	n\a	31.2%	Hanger
B2	Column	3-1/2" x 1-3/4"	694 lbs	21.5%	14.3%	Unspecified

Hanger model Hanger was not found. Hanger has not been analyzed for adequate capacity.

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

AMENDED 2020

Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

CANPORMS TO OBC 2012



Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of sultability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain installation Guide or ask questions, please call (800)232-0788 before installation.





# Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B5(i2038) (Fiush Beam)

PASSED

Dry | 1 span | No cant.

February 10, 2020 11:38:52

BC CALC® Member Report

**Build 7239** Job name:

Address: City, Province, Postal Code:

Customer: Code reports:

**CCMC 12472-R** 

File name:

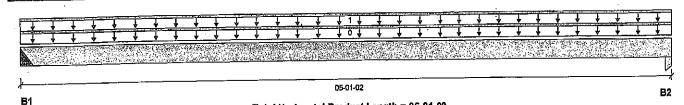
MOUNTAINASH 6 EL 1.mmdl

1ST FLR FRAMING\Flush Beams\B5(i2038) Description:

Specifier:

Designer:

Company:



Total Horizontal Product Length = 05-01-02

Reaction Summary (Down / Uplift) (lbs) Dead Live Bearing 34/0 B1, 2" 44/0

34/0 44/0 B2, 1-3/4"

	- I O						Live	Dead	Snow	Wind	Tributary
	ad Summary  Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
Tag	Self-Weight	Unf. Lin. (lb/ft)	Ĺ	00-00-00	05-01-02	Top		5			00-00-00
1	FC1 Floor Material.	Unf. Lin. (lb/ft)	L	00-00-00	05-01-02	Top	17	9			n\a

Snow

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	129 ft-lbs	11610 ft-libs	1.1%	1	02-06-11
	68 lbs	5785 lbs	1.2%	1	00-11-08
End Shear	L/999 (0.002")	n\a	n\a	4	02-06-11
Total Load Deflection	L/999 (0.001")	n/a	n\a	5	02-06-11
Live Load Deflection Max Defl.	0.002"	n\a	n\a	4	02-06-11
Span / Depth	6.2				

Dooring '	Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Wember	Material	
	-langer	2" x 1-3/4"	109 lbs	n\a	2.6%	Hanger	
	Taligo:	1-3/4" x 1-3/4"	108 lbs	4.4%	2.9%	Unspecified	

Hanger model Hanger was not found. Hanger has not been analyzed for adequate capacity.

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

CANFORMS TO OBO 2012

Hanger Manufacturer: Unassigned

AMENDED 2020

Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

ON MICE OF O

# UVE NO. TAN 6092-20 STRUCTURAL COMPONENT ONLY

#### Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.





# Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B6(i2036) (Flush Beam)

PASSED

February 10, 2020 11:38:52

**BC CALC® Member Report** 

**Bulld 7239** 

Job name: Address:

City, Province, Postal Code:

Customer:

Code reports:

Dry | 1 span | No cant.

MOUNTAINASH 6 EL 1.mmdl

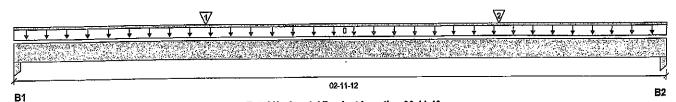
File name: 1ST FLR FRAMING\Flush Beams\B6(i2036) Description:

Wind

Specifier:

Designer:

Company: CCMC 12472-R



Total Horizontal Product Length = 02-11-12

Reaction Summary (Down / Uplift) (lbs)

Meachon ean	illiai y (moisii . o		• ,
Bearing	Live	Dead	Snow
B1, 3-1/2"	113 / 0	64/0	<del></del>
B2. 3-1/2"	119/0	67 / 0	

١a	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag		Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
<u> 128</u>	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	02-11-12	Тор		5			00-00-00
1	J6(i1935)	Conc. Pt. (lbs)	L	00-10-06	00-10-06	Top	118	59			n\a
2	J6(i2002)	Conc. Pt. (lbs)	L	02-02-06	02-02-06	Top	114	57	, es	THE PERSON NAMED IN	n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	157 ft-lbs	11610 ft-lbs	1.3%	1	00-10-11
End Shear	174 lbs	5785 lbs	3.0%	1	01-01-00
Total Load Deflection	L/999 (0.001")	n\a	n\a	4	01-05-14
Live Load Deflection	L/999 (0")	n\a	n\a	5	01-05-14
Max Defl.	0.001"	n\a	n\a	4	01-05-14
Span / Depth	3.2	•			

Bearing	Supports	Dim, (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material	
B1	Column	3-1/2" x 1-3/4"	249 lbs	5.0%	3.3%	Unspecified	
B2	Column	3-1/2" x 1-3/4"	261 lbs	5.3%	3.5%	Unspecified	

146 ND. TAN 6093-20 STRUCTURAL COMPONENT ONLY

OLIVOE OF ON

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

CONFORMS TO OBC 2012

Resistance Factor phi has been applied to all presented results per CSA 086. AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

#### Disclosure

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# Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B7(I2063) (Flush Beam)

PASSED

February 10, 2020 11:38:52

**BC CALC® Member Report** 

**Build 7239** 

Job name: Address:

City, Province, Postal Code:

Customer: Code reports: CCMC 12472-R

Dry | 1 span | No cant.

File name:

MOUNTAINASH 6 EL 1.mmdl

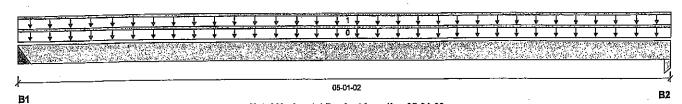
Wind

Description: 1ST FLR FRAMING\Flush Beams\B7(i2063)

Specifier:

Designer:

Company:



Total Horizontal Product Length = 05-01-02

Reaction Summary (Down / Uplift) (lbs)

Dead Bearing Live 40 / 0 32 / 0 B1, 2" 32/0 40 / 0 B2, 1-3/4"

Load Summary						Live	Dead	Snow	Wind	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	,
0 Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	05-01-02	Тор		5			00-00-00
1 FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	05-01-02	Тор	16	8			n\a

Snow

Controls Summary	Factored Demand	Factored Resistance	Demand <i>i</i> Resistance	Case	Location
Pos. Moment	118 ft-lbs	11610 ft-lbs	1.0%	1	02-06-11
End Shear	63 lbs	5785 lbs	1.1%	1	00-11-08
Total Load Deflection	L/999 (0.001")	n\a	n\a	4	02-06-11
Live Load Deflection	L/999 (0.001")	n\a	n\a	5	02-06-11
Max Defl.	0.001"	n\a	n\a	4	02-06-11
Span / Depth	6.2				

Bearing	Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Hanger	2" x 1-3/4"	100 lbs	n\a	2.3%	Hanger
B2	Column	1-3/4" x 1-3/4"	99 lbs	4.0%	2.7%	Unspecified

Cautions

Hanger model Hanger was not found. Hanger has not been analyzed for adequate capacity.

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

CAMPORMS TO OBC 2012

Hanger Manufacturer: Unassigned

AMENDED 2020

Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

ONINCE OF

ung no. Tan 6094 -20 STRUCTURAL COMPONENT ONLY

#### Disclosure

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# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING\Flush Beams\B14B(i2247) (Flush Beam)

Passed

February 10, 2020 12:41:53

**BC CALC® Member Report** 

**Build 7239** 

Job name:

Address:

City, Province, Postal Code: WATERDOWN

Customer:

Code reports:

Dry | 1 span | No cant.

File name: Description: MOUNTAINASH 6 EL 2.mmdl

2ND FLR FRAMING\Flush Beams\B14B(i2247)

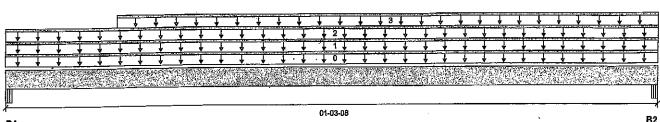
Wind

Specifier:

Designer:

Company:

CCMC 12472-R



**B**1

#### Total Horizontal Product Length = 01-03-08

Reaction Summary (Down / Uplift) (lbs)

IZEACTION OWN	illiaty (Domini -		_
Bearing	Live	Dead	Snow
B1, 2-5/8"	12/0	59 / 0	23 / 0
D2 5 4//"	17/0	83 / 0	32 / 0

Loa	ad Summary	•				i	Live
Tag		Load Type	Ref.	Start	End	Loc.	1.00
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	01-03-08	Top	
1	SNOW	Unf. Lin. (lb/ft)	L ·	00-00-00	01-03-08	Top	22
2	WALL	Unf. Lin. (lb/ft)	L	00-00-00	01-03-08	Тор	
3	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-02-10	01-03-08	Top	1

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	11 ft-lbs	15093 ft-lbs	n\a	0	00-06-07
End Shear	49 lbs	7521 lbs	0.6%	0	00-02-10
Span / Depth	1.0				

Bearing	i Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Beam	2-5/8" x 3-1/2"	82 lbs	2.6%	1.1%	Unspecified
B2	Beam	5-1/4" x 3-1/2"	116 <b>l</b> bs	1.8%	0.8%	Unspecified

ove no . tan*6095* -20 STRUCTURAL COMPONENT ONLY

ROVINCE OF

#### Notes

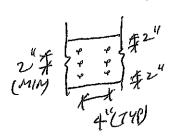
Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86. Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9 EQNFORMS TO OBC 2012

AMENDED 2020



PROVIDE3 ROWS OF 3½" ARDOX SPIRAL NAILS @ \( '' O / C FOR MULTI-PLY NAILING, MAINTAIN A MIN.2" LUMBER EDGE/END DISTANCE. DO NOT USE AIR NAILS

#### Disclosure

Dead

0.65

10

20

80

Snow

1.00

42

Wind

1.15

**Tributary** 

00-00-00

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# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B1A(i2350) (Flush Beam)

Passed

**BC CALC® Member Report** 

Dry | 1 span | No cant.

April 16, 2020 08;43:03

**Build 7239** 

Job name:

Customer:

Code reports:

Address: City, Province, Postal Code: WATERDOWN

CCMC 12472-R

File name:

MOUNTAINASH 6 EL. 1 DECK CONDITION.mmdl

1ST FLR FRAMING\Flush Beams\B1A(i2350) Description:

Specifier:

Designer:

ΑJ Company:

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4.5.0			Ü		3.		i i si		3	100	46	, ue	41	10.00	34	Value	1.3 4.50	2	100		6		54.5	ć.		ili. Nagio	ijā.	$\hat{\mathbf{g}}_{i}$		10	4.7	s de	10 TeV			eil.	37	2	3.1	4,7		2	į.
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																																											_
																					03	-01-	00																				

Total Horizontal Product Length = 03-01-00

a. (Down / Haliff) (lhe)

Reaction 5	Billitigia / Possili o					
Bearing	Live	Dead	\$now	Wind	·	
B1, 3"	41/0	160 / 0			•	
B2, 3"	41 / 0	160 / 0				

	1 ^4	d Summary						Live	Dead	Snow	Wind	Tributary
		Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
_	n n	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-01-00	Тор		10			00-00-00
	1	E3(i585)	Unf. Lin. (lb/ft)	L	00-00-00	03-01-00	Top		81			n/a
	2	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	03-01-00	Top	27	13	- Aller Carrier	the state of the s	. n\a
	_	101110111111111	•							CFE	SS/On?	**************************************

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	133 ft-lbs	15093 ft-lbs	0.9%	0	01-06-08
End Shear	73 lbs	7521 lbs	1.0%	0	01-00-08
Total Load Deflection	L/999 (0")	n\a	n\a	4	01-06-08
Live Load Deflection	L/999 (0")	n\a	n\a	5	01-06-08
Max Defl.	0"	n\a	n\a	4	01-06-08
Span / Depth	3.4				

	Bearing Suppor	ts Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Wember	Material	
٠	B1 Wall/Plate		224 lbs	5.3%	2.7%	Spruce-Pine-Fir	_
	B2 Wall/Plate	3" x 3-1/2"	224 lbs	5.3%	2.7%	Spruce-Pine-Fir	

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CAMPORNS TO OBC 2012

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86. AWENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086. Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

PROVIDE 3 ROWS OF 3½" ARDOX SPIRAL NAILS @ 8 " 0/C FOR MULTI-PLY NAILING, MAINTAIN A MIN. 2"LUMBER EDGE/END DISTANCE, DO NOT USE AIR NAILS



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#### Disclosure

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# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B1B(i2351) (Flush Beam)

passed

**BC CALC® Member Report** 

Dry | 1 span | No cant,

April 16, 2020 08:43:03

**Build 7239** 

Job name:

Customer:

Code reports:

Load Summary

Address:

City, Province, Postal Code: WATERDOWN

CCMC 12472-R

File name:

MOUNTAINASH 6 EL 1 DECK CONDITION.mmdi

Dead

Snow

Wind

Tributary

00-00-00 n\a n\a

Description: 1ST FLR FRAMING\Flush Beams\B1B(i2351)

Specifier:

Designer: ΑJ

Company:

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1,295	74. 75E	10.50	S	0.00	- Michie	e Mer Sell	No like	1000	4	ja (e.	Air	×.	4.4.	1,32.7	<u> </u>	2. 7. 7.	Por Sec	2.00	i de la composición dela composición de la composición de la composición dela composición dela composición dela composición de la composición de la composición de la composición dela composición de la composición dela c		10 10	reisi.	720,00	8,8-36	A Section	arr.	. Terror in		er inch	Jane 1	20,77.1	(12. jan	4194	1	2.4	(4.25)	1.000	100	8743	0.40		A. W. &	4/62
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Total Horizontal Product Length = 03-01-00

Reaction Su	Illinary (Down of	hillr) (ine)			
Bearing	Live	Dead	Snow	Wind	
B1, 3"	938 / 0	610 / 0			
B2 3"	928 / 0	604 / 0			

	ace Cumming									
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-01-00	Тор		10		
1	E9(i588)	Unf. Lin. (lb/ft)	L	00-00-00	03-01-00	Тор	270	216		
2	J3(i2302)	Conc. Pt. (lbs)	L	00-02-00	00-02-00	Тор	345	173		
3	J3(i2284)	Conc. Pt. (lbs)	L	01-06-00	01-06-00	Top	345	173	1 market	FESS
4	J3(i2227)	Conc. Pt. (lbs)	L	02-10-00	02-10-00	Тор	345	173	100	
		`	Factored	Dem	and/			f		116

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1125 ft-lbs	23220 ft-lbs	4.8%	1	01-06-00
End Shear	721 lbs	11571 lbs	6.2%	1	01-00-08
Total Load Deflection	L/999 (0.002")	n\a	n\a	4	01-06-07
Live Load Deflection	L/999 (0.001")	n\a	n\a	5	01-06-07
Max Defl.	0.002"	n\a	n\a	4	01-06-07
Span / Depth	3.4				

Bearing	, Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	3" x 3-1/2"	2170 lbs	33.6%	16.9%	Spruce-Pine-Fir
-B2	Wall/Plate	3" x 3-1/2"	2147 lbs	33.2%	16.8%	Spruce-Pine-Fir

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CINFORMS TO OBC 2012

Calculations assume member is fully braced.

AMENDED 2020 Resistance Factor phi has been applied to all presented results per CSA 086. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

PROVIDE 3 ROWS OF 31/2" ARDOX SPIRAL NAILS @ 8 "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN. Z"LUMBER EDGE/END DISTANCE, DO NOT USE AIR MAILS



OUNCE OF ONLY

### Disclosure

Use of the Boise Cascade Software Is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on . building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current installation Guide and applicable building codes. To obtain installation Guide or ask questions, please call (800)232-0788 before installation.



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### Maximum Floor Spans

Live Load = 40 psf, Pead Load = 30 psf Simple Spans; L/480 Deflection Limit 3/4" OSB G&N Sheathing







				are			1/2" Gyp	sum Celling	
Depth	Series		On Cent	re Spacing		i	On Cent	re Spacing	
-		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	N1-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-0"	16'-0"	15'-1"	13'-11"	17'-5"	16'-1"	15'-1°	13'-11"
9-1/2"	NI-60	17'-2"	16'-2"	15'-5"	14'-3"	17'-6"	16'-5"	15'-5"	14'-3"
	NI-70	18'-0"	16'-11"	16'-3"	15'-6"	18'-5"	17'-3"	16'-7"	15'-6"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	15'-10"
	NI-20	17'-10"	16'-10"	16'-0"	14'-10"	18'-6"	17'-1"	16'-0"	14'-10"
	NI-40x	19'-4"	17'-11"	17'-3"	15'-10"	19'-11"	18'-6"	17'-9"	15'-10"
11-7/8"	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-1"
	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	1 <b>9'-</b> 9"	18'-10"	<b>17'-1</b> 0"
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
	NI-40x	21'-5"	19'-10"	18'-11"	17'-5"	22'-1"	20'-6"	19'-6"	17'-5°
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"
	08-1N	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
4.611	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22'-9"	21'-6"
16"	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

			Mid-Spa	n Blocking		Mid-	Span Blocking at	nd 1/2" Gypsum	Celling
Depth	Serles		On Cent	re Spacing			On Cent	re Spacing	-
•		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14¹-2 <sup>tt</sup>	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-9"	16'-1"	15'-1"	13'-11"	17'-9"	16'-1"	15'-1"	13'-11'
9-1/2"	NI-60	18'-1"	16'-5"	15'-5"	14'-3"	18'-1"	16'-5"	15'-5"	14'-3"
•	NI-70	19'-10"	17'-11"	16'-9"	15'-6"	19'-10"	17'-11"	16'-9"	15'-6"
	NI-80	20'-2"	18'-3"	17' <b>-1</b> "	15'-10"	20'-2"	18'-3"	17'-1"	15'-10'
	NI-20	18'-10"	17'-1"	16'-0"	14'-10"	18'-10"	17'-1"	16'-0"	14'-10
	NI-40x	21'-3"	19'-3"	27'- <del>9</del> "	15'-10"	21'-3"	19'-3"	17-9"	15'-10
11-7/8"	NJ-60	21'-9"	19'-8"	18'-5"	17'-1"	21'-9"	19'-8"	18'-5"	17'-1"
	NJ-70	23'-4"	21'-5"	20'-1"	18'- <del>6</del> "	23'-8"	21'-5"	20'-1"	18'-6"
	N1-80	23'-7"	21'-10"	20'-5"	18'-11"	24'-1"	21'-10"	20'-5"	18'-11
	N1-90x	24'-3"	22'-6"	21'-3"	19'-7"	24'-8"	22'-7" ·	21'-3"	19'-7"
	NI-40x	24'-2"	21'-5"	19'-6"	17'-5"	24'-2"	21'-5"	19'-6"	17'-5"
	NI-60	24'-9"	22'-5"	21'-0"	19'-6"	24'-9"	22'-5"	21'-0"	19'-5"
14"	พเ-70	26'-1"	24'-3"	22'-9"	21'-0"	26'-8"	24'-3"	22'-9"	21'-0"
	08-1N	26'-6"	24'-7"	23'-3"	21'-6"	27'-1"	24'-10"	23'-3"	21'-6"
	NI-90x	27'-3"	25'-4"	24'-1"	22'-4"	27'-9"	25'-10"	24'-3"	22'-4"
	NI-60	27'-3"	24'-11"	23'-5"	21.'-7"	27'-6"	24'-11"	23'-5"	21'-7"
	NI-70	28'-8"	26'-8"	25'-3"	23'-4"	29'-3"	26'-11"	25'-3"	23'-4"
16"	NI-80	29'-1"	27'-0"	25'-9"	23'-10"	29'-8"	27'-6"	25'-10"	23'-10'
	NI-90x	29'-11"	27'-10"	26'-6"	24'-10"	30'-6"	28'-5"	26'-11"	24'-10"

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>2.</sup> Spans are based on a composite floor with glued-nalled oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

<sup>3.</sup> Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers. 5. This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required

based on the use of the design properties. Tables are based on Limit States Design per CSA 086-09, NBC 2010, and OBC 2012, 6. Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



### **Maximum Floor Spans**

Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







			В	are		I	1/2" Gyp:	sum Ceiling	
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
<b>4-</b> p		12"	16"	19.2"	24"	12"	16°	19.2"	24"
	NI-20	15'-1"	14'-2"	13'-9"	N/A	15'-7"	14'-8"	14'-2"	N/A
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
_ 1=0	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
11-7/8"	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	N1-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
	N1-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A
14°	N1-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NJ-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
11	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
16"	NI-80	23'-11"	22'- <b>1</b> "	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A
	N1-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

			Mid-Spa	n Blocking		Mid-S	Mid-Span Blocking and 1/2" Gypsum Celling On Centre Spacing				
Depth	Series		On Cent	re Spacing							
ocp		12"	16"	19.2°	24"	12"	16"	19.2"	24"		
	NI-20	16'-8"	15'-3"	14'-5"	N/A	16'-8"	15'-3"	14'-5"	N/A		
	NI-40x	17'-11"	16'-11"	16'-1"	N/A	18'-5"	17'-1"	<b>16'-1"</b>	N/A		
9-1/2"	NI-60	18'-2"	17'-1"	16'-4"	N/A	18'-7"	17'-4"	16'-4"	N/A		
·	NI-70	19'-2"	17'-10"	17'-2"	N/A	19'-7"	18'-3"	17'-7"	N/A		
	NI-80	19'-5"	18'-0"	17'-4"	N/A	19'-10"	18'-5"	17'-8"	N/A		
	NI-20	19'-6"	18'-1"	17'-3"	N/A	19'-11"	18'-3"	17'-3"	N/A		
	NI-40x	21'-0"	19'-6"	18'-8"	N/A	21'-7"	20'-2"	19'-2"	N/A		
	NI-60	21'-4"	19'-9"	18'-11"	N/A	21'-11"	20'-4"	19'-6"	N/A		
11-7/8"	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-5"	20'-5"	N/A		
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-8"	N/A		
	NI-90×	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A		
	NI-40x	23'-7"	21'-11"	20'-11"	N/A	241-3"	22'-7"	21'-7"	N/A		
	NI-60	24'-0"	22'-3"	21'-3"	N/A	24'-8"	22'-11"	21'-11"	N/A		
14"	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-11"	N/A		
	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A		
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23' <del>-9</del> "	N/A		
	Nt-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	25'-3"	24'-2"	N/A		
	NI-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A		
16"	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A		
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26'-2"	N/A		

Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The
ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration,
a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceilling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

3. Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA 086-09, NBC 2010, and OBC 2012.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



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Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			Ba	ire		l	1/2" Gyp:	sum Ceiling	
Depth	Series		On Centr	e Spacing			On Cent	re Spacing	
Doptii	<b>54</b> =	12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5°
	NI-40x	17'-0"	16'-0"	15'-5"	14'-9"	17'-5"	16'-5"	15'-10"	15'-2"
9-1/2"	NI-60	17'-2"	16'-2"	1.5°-7"	14'-11"	17'-6"	16'-7"	15'-11"	15'-3"
2 4/2	N1-70	18'-0"	16'-11"	16'-3"	15'-7"	18'-5"	17'-3"	16'-7"	15'-11"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
	NI-20	17'-10"	16'-10"	16'-2"	15'-6"	18'-6"	17'-4"	16'-9"	16'-1"
	N1-40x	19'-4"	17'-11"	17'-3"	16'-6"	19'-11"	18'-6"	17'-9"	17'-0"
	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"
11-7/8"	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
	NI-40x	21'-5"	19'-10"	18'-11"	17'-11"	22'-1"	20'-6"	19'-7"	18'-7"
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	231-8"	21'-11"	20'-10"	19'-9"
24	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'- <del>9</del> "	20'-7"
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
	NJ-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22'-9"	21'-5"
16"	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

			Mid-Spar	n Blocking		Mid-Span Blocking and 1/2" Gypsum Ceiling On Centre Spacing				
Depth	Series		On Centi	e Spacing						
Deptii	501.65	12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"	
	N1-40x	18'-8"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"	
9-1/2"	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-S"	
J 4/2	NI-70	20'-0"	18'-7"	17'-9"	16'-7"	20'-5"	18'-11"	17'-10"	16'-7"	
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10"	
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"	
	N1-40x	21'-10"	20'-4"	19'-4"	17'-8"	22'-5"	20'-6"	19'~4"	17'-8"	
	NI-60	22'-1"	20'-7"	19'-7"	18 <sup>1</sup> -4"	22'-8"	20'-10"	19'-8"	18'-4"	
11-7/8"	NI-70	23'-4"	21'-8"	20'-8"	19'-7"	23'-10"	22'-3"	21'-2"	19'-9"	
	N1-80	23'-7"	21'-11"	20'-11"	19'-9"	24'-1"	22' <del>-6</del> "	21'-5"	20'-0"	
	NI-90x	24'-3"	22'-6"	21'-6"	20'-4"	24'-8"	23'-0"	22'-0"	20'-9"	
	NI-40x	24'-5"	22'-9"	21'-8"	19'-5"	25'-1"	23'-2"	21'-9"	19'-5"	
	NI-60	24'-10"	23'-1"	22'-0"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10"	
14"	NI-70	26'-1"	24'-3"	23'-2"	21'-10"	26'-8"	24'-11"	23'-9"	22'-4"	
14	NI-80	26'-6"	24'-7"	23'-5"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"	
	NI-90x	27'-3"	25'-4"	24'-1"	22'-9"	27'-9"	25'-11"	24'-8"	23'-4"	
	NI-60	27'-3"	25'-5"	24'-2"	22'-10"	28'-0"	26'-2"	24'-9"	23'-1"	
	NI-70	28'-8"	26'-8"	25'-4"	23'-11"	29'-3"	27'-4"	26'-1"	24'-8"	
16"		29'-1"	27'-0"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"	
	NI-80 NI-90x	29'-11"	27'-10"	26'-6"	25'-0"	30'-6"	28'-5"	27'-2"	25'-8"	

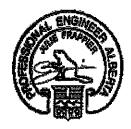
- Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The
  ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration,
  a live load deflection limit of L/480 and a total load deflection limit of L/240.
- 2. Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum celling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.
- 3. Minimum bearing length shall be 1-3/4 inches for the end bearings.
- 4. Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.
- 5. This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA 086-09, NBC 2010, and OBC 2012.
- 6. Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



 $\Box$ 

## **Maximum Floor Spans**

Live Load = 40 psf; Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&M Sheathing







			B	are		1	1/2" Gyps	sum Ceiling	
Depth	Series		On Cent	re Spacing		1	On Centre Spacing		
Depa.		12°	16"	19.2"	24"	12"	16"	19.2"	24"
	NJ-20	15'-1"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	Ŋ/A
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	<b>15'-1"</b>	N/A
9-1/2°	NI-60	16¹-3"	15'-4"	14'-10"	N/A	16'-8"	15′-9"	15'-3"	N/A
J 1/1-	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
11-7/8"	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4	18'-10"	17'-11"	N/A
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A
	NJ-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
	N1-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A
14"	NI-70	21 7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
14	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
	NJ-70	23'-5"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
16"	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10°	21'-9"	N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

	_		Mid-Spar	n Blocking		Mid-Span Blocking and 1/2" Gypsum Ceiling				
Depth	Series		On Centr	re Spacing		On Centre Spacing				
DCp4		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	15'-7"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A	
	N1-40x	17'-9"	16'-1"	15'-1"	N/A	17'-9"	16'-1"	15'-1"	N/A	
9-1/2"	NI-60	18'-1"	16'-4"	15'-4"	N/A	18'-1"	16'-4"	15'-4"	N/A	
3 1/2	NI-70	19'-2"	17'-10"	16'-9"	N/A	19'-7"	17'-10"	16'-9"	N/A	
	NI-80	19'-5"	18'-0"	17'-1"	N/A	19'-10"	18'-3"	17'-1"	N/A	
	NJ-20	18'-9"	17'-0"	16'-0"	N/A	18'-9"	17'-0"	16'-0"	N/A	
	NJ-40x	21'-0"	19'-3"	17'-9"	N/A	21 -3"	19'-3"	17'-9"	N/A	
	NI-60	21'-4"	19'-8"	18'-5"	N/A	21'-8"	19'-8"	18'-5"	N/A	
11-7/8"	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-4"	20'-0"	N/A	
	N1-80	221-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-5"	N/A	
	NJ-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A	
	NI-40x	23'-7"	21'-5"	19'-6"	N/A	24'-1"	21'-5"	19'-6"	N/A	
	NJ-60	24'-0"	22'-3"	21'-0"	N/A	24'-8"	22'-5"	21'-0"	N/A	
14"	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-9"	N/A	
17	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A	
	N1-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A	
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	24'-10"	23'-4"	N/A	
	NI-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A	
16"	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A	
	N1-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26'-2"	N/A	

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of 1/480 and a total load deflection limit of 1/240.

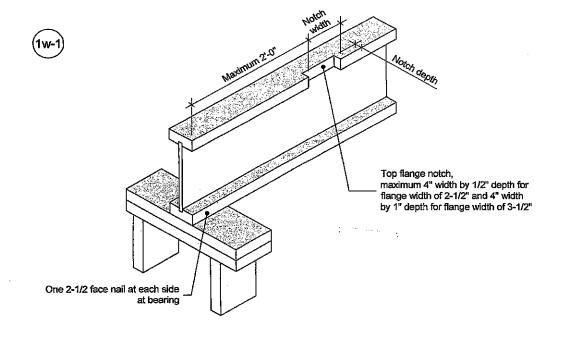
a new load deleted in mice of 2400 and the first place of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

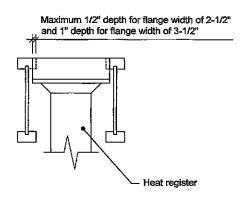
<sup>3.</sup> Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.





#### Notes:

- Blocking required at bearing for lateral support, not shown for clarity.
- The maximum dimensions for a notch on the side of the top flange are 4-inch width by 1/2-inch depth for flange width of 2-1/2 inches, and 4-inch width by 1-inch depth for flange width of 3-1/2 inches.
- 3. This detail applies to simple-span joists and multiple-span joists where the notch is located at the end half-span.
- 4. For other applications, contact Nordic Structures.

This document supersedes all previous versions. If the document has been in effect for more than one year, consult nordic.ca or contact Nordic Structures.

All nails shown in the details are assumed to be common nails unless otherwise noted. Nails shall have a diameter not less than 0.128 inch for 2-1/2-inch nails, or 0.144 inch for 3-inch nails, individual components not shown to scale for clarity.



T 514-871-8526 1 866 817-3418 nordic.ca Notch in I-joist for Heat Register

CATEGO

I-joist - Typical Floor Framing and Construction Details

DOCUMENT

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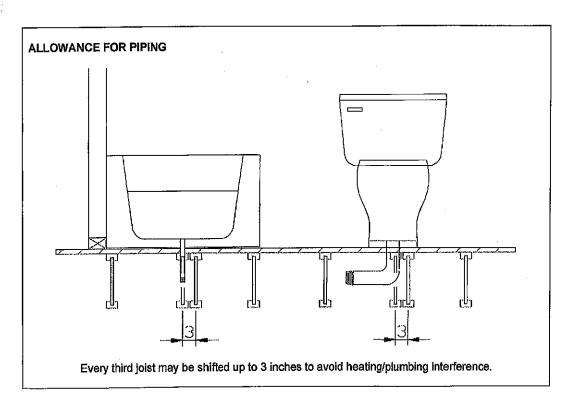


# Allowance for Piping (Installation Notes)

The floor layouts have usually not been checked for heating and/or plumbing interference. On-site adjustment of joists of up to 3 inches is permitted to avoid interferences. When moving a joist, the subfloor thickness shall be checked with code requirements when the joist spacing exceeds 19.2 inches. Except for cutting to length, I-joist flanges should never be cut, drilled, or notched.

Installation of Nordic I-joists shall be as per *Nordic Joist Installation Guide for Residential Floors*. Refer to Tables 1 and 2 for maximum web hole and duct chase openings, respectively. These tables are based on the I-joists being used at their maximum spans. The minimum distance given may be reduced for shorter spans; contact your distributor for additional information.

The detail below shows the 3-inch allowance for piping. Every third joist may be shifted up to 3 inches to avoid heating/plumbing interference. For other applications, please contact your distributor.



Revised April 12, 2012