

		Products	-		
PlotID	Length	Product	Plies	Net Qty	Fab Type
J1	18-00-00	11 7/8" NI-40x	1	29	MFD
J2	16-00-00	11 7/8" NI-40x	1	12	MFD
J3	14-00-00	11 7/8" NI-40x	1	14	MFD
J4	12-00-00	11 7/8" NI-40x	1	40	MFD
J5	10-00-00	11 7/8" NI-40x	1	1	MFD
J6	8-00-00	11 7/8" NI-40x	1	3	MFD
J7	2-00-00	11 7/8" NI-40x	1	3	MFD
J8	20-00-00	11 7/8" NI-80	1	8	MFD
B17	18-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2	MFD
B15	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1	MFD
B22	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2	MFD
B14	8-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2	MFD
B16	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1	MFD
B10	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2	MFD
B12	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2	MFD

CITY OF HAMILTON
BUILDING DIVISION
Planeling & Dayelogment Department

FEB 0 9 2021

DATE\_\_

REC'D BY

REF'D TO

Connector Summary						
Qty	Manuf	Product				
2	H1	IUS2.56/11.88				
36	H1	IUS2.56/11.88				
1	H2	HUS1.81/10				
4	H3	IUS3.56/11.88				

CITY OF HAMILTON
Building Division

Permit No. 21-106977

THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE

THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW

These drawings and or specifications have been reviewed by

0 2/2 4/2 1

FOR CHIEF BY LDING OFFICIAL DATE



FROM PLAN DATED:

**BUILDER: GREENPARK HOMES** 

SITE: RUSSELL GARDENS PH 3

MODEL: MOUNTAINASH 4

**ELEVATION: 3** 

LOT: 3/2

CITY: WATERDOWN

SALESMAN: MARIO DICIANO

DESIGNER: AJ REVISION:

#### NOTES:

REFER TO THE NORDIC INSTALLATION **GUIDE** FOR PROPER STORAGE AND INSTALLATION, SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE **SQUASH BLOCKS REQ'D UNDER** CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE, CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6

LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 20.0 lb/ft<sup>2</sup>

SUBFLOOR: 5/8" GLUED AND NAILED

**DATE:** 2020-02-18

2nd FLOOR

# NORDIC STRUCTURES

**COMPANY** Apr. 9, 2020 09;44

PROJECT
J1 1ST FLOOR,wwb

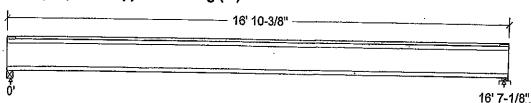
## **Design Check Calculation Sheet**

Nordic Sizer - Canada 7.2

#### Loads:

Load	Type	Distribution Pat-	,	Magnitude Start End	Unit
Load1 Load2	Dead Live	Full Area Full Area		20.00	psf psf

# Maximum Reactions (lbs) and Support Bearing (in):



<u> </u>			10 7-170,
Unfactored: Dead Live	221 442		221 442
Factored: Total Bearing:	940		940
Capacity Joist Support	2137 4036		2102 3981
Des ratio Joist Support Load case	0.44 0.23 #2		0.45
Length Min req'd Stiffener	2-5/8 1-3/4		#2 2-3/8 1-3/4
KD KB support	1.00 1.00	<u>.</u>	No 1.00 1.00
fcp sup Kzcp sup	769 1.00		769

# Nordic 11-7/8" NI-40x Floor joist @ 16" o.c.

Supports: 1 - Lumber Beam, No.1/No.2; 2 - Lumber Sill plate, No.1/No.2; Total length: 16' 10-3/8"; Clear span: 16' 5-3/8"; 3/4" nailed and glued OSB sheathing This section PASSES the design code check.

# Limit States Design using CSA O86-14 and Vibration Criterion:

	<u> </u>			
Criterion	Analysis Value	Design Value	Unit	Analysis/Design
Shear	Vf ≈ 940	Vr = 2336	lbs	$\frac{\text{Vf/Vr} = 0.40}{\text{Vf/Vr} = 0.40}$
Moment(+)	Mf = 3901	Mr = 6255	lbs-ft	Mf/Mr = 0.62
Perm. Defl'n	0.11 = < L/999	0.55 = L/360	in in	OFESSION 0.20
Live Defl'n	0.23 = L/879	0.41 = L/480		
Total Defl'n	0.34 = L/586	0.83 = L/240	in S	0.55
Bare Defl'n	0.27 = L/727	0.55 = L/360	in /2 🛴	4-1620 0.41
Vibration	Lmax = 16'-7.1	Lv = 18'-1.3	f+ # 4#	M L AA
Defl'n	= 0.029	= 0.038	in	KATSOULAKOS 👹 🕽 . 92
<u></u>				

AWR HO. TAM 6 035 = 28 STRUCTURAL COMPONENT ONLY

### WoodWorks® Sizer

#### for NORDIC STRUCTURES

#### J1 1ST FLOOR,wwb

### Nordic Sizer - Canada 7.2

Page 2

	t								····		
	Additional D										,
	FACTORS:	•	KD	KH	KZ	KL	KT	KS	KN	LC#	
	Vr	2336	1.00	1.00	_	_		-	_	#2	
	Mr+	6255	1.00	1.00		1,000	_	-	<b></b>	#2	
	EI 3	371.1 mi	llion	_	_	_	_	_	_	π2 #2	
ı	CRITICAL LOAI	D COMBIN	VATIONS	:						πΔ	
	Shear :	: LC #2	= 1.25	D + 1.5L							
1	Moment(+):										
ļ	Deflection:	LC #1	= 1.0D	(perma	nent)						
١		LC #2	= 1.0D	+ 1.0L	(live)						
ı	ĺ	LC #2	= 1.0D	+ 1.0L	(total	3					
ľ		LC #2	= 1.0D	+ 1.0L	(bare	ioist)					
ł	Bearing :	Support	t 1 – L	C #2 = 1	25D +	1.5t.					
1	_	Support	t 2 - L	C #2 ≈ 1	.25D +	1.75T.					
1	Load Types:	D=dead	W=wind	d S=snot	v H≕ea	rth.arour	dwater	F=0ari	thought		
Į		L=live	(use,oco	cupancyl	Ls=li	ve(storaç	e anui:	nment)	frfire		
Ī	Load Patter	ns: s=S/	∕2 L=L·	+Ls =no	patte	rn load i	n thie	onan Suerre)	r~rrre		
1	All Load Co	mbinatio	ons (LC:	s) are li	sted	n the Ans	lveie	outout			
1	CALCULATIONS	S:	,			00 11110	TAPTO (	oucput			
ĺ	EIeff = 459		ln^2 K=	= 6.18eC	6 1hs				Gan	FORMS TN	OBC 2012
1	"Live" defl	ection i	s due t	to all no	n-dead	loade /I	1370 137	ind and			
-					acaa	TOUND (T	±v∈, W.	ina, snc	)W+1.1	AMENDED	202B
- 1											H M

#### **Design Notes:**

- 1. WoodWorks analysis and design are in accordance with the 2015 National Building Code of Canada (NBC), Division B, Part 4, and the CSA 086-14 Engineering Design in Wood standard, Update No. 2 (June 2017).
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to Nordic Structures technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.



OWO HO. YAM 6035 -20 STRUCTURAL COMPONENT ONLY

# NORDIC STRUCTURES

COMPANY Apr. 9, 2020 09:44 PROJECT
J1 2ND FLOOR.wwb

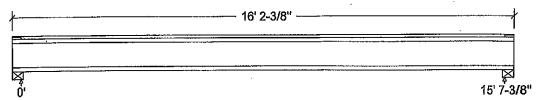
### **Design Check Calculation Sheet**

Nordic Sizer - Canada 7.2

#### Loads:

Load	Type	Distribution	Pat- tern	Location Start	[ft] End	Magnitude Start End	Unit
Load1 Load2	Dead Live	Full Area Full Area				20.00 40.00	psf psf

### Maximum Reactions (lbs) and Support Bearing (in):



Unfactored: Dead Live	208 416		208 416
Factored: Total	885		885
Bearing:			
Capacity			0000
Joi.st	2336		2336
Support	6734		6734
Des ratio	•		
Joist	0.38		0.38
Support	0.13	•	0.13
Load case	#2		#2
Length	4-3/8		4-3/8
Min req'd	1-3/4		1-3/4
Stiffener	No		No
KD	1.00		1.00
KB support	1.00		1.00
fcp sup	769		769
Kzco sup	1.00		1.00

### Nordic 11-7/8" NI-40x Floor joist @ 16" o.c.

Supports: All - Lumber Beam, No.1/No.2

Total length: 16' 2-3/8"; Clear span: 15' 5-5/8"; 5/8" nailed and glued OSB sheathing with 1/2" gypsum ceiling

This section PASSES the design code check.

### Limit States Design using CSA O86-14 and Vibration Criterion:

Criterion	Analysis Value	Design Value	Unit	Analysis/Design
Shear	Vf = 885	Vr = 2336	lbs	Vf/Vr = 0.38
Moment(+)	Mf = 3454	Mr = 6255	lbs-ft	Mf/Mr = 0.55
Perm. Defl'n	$0.09 = \langle L/999 \rangle$	0.52 = L/360	in	FESSIONAL 0.18
Live Defl'n	$0.18 = \langle L/999 \rangle$	0.39 = L/480	ر *م∕اک با	0.47
Total Defl'n	0.28 = L/676	0.78 = L/240	in	11600 0 0.35
Bare Defl'n	0.22 = L/861	0.52 = L/360 $Lv = 17'-8.1$	in / 5 C	0.35 0.42 0.88
Vibration	Lmax = 15'-7.4	0.044	I J. L. H 826	ATSOULANDS \$6.68
Defl'n	= 0.028	= 0.041	IN S V	AISUULANNO 30 10.68

AWA NO. YAN *6036*-20 Structural Component only

POVINCE OF ON THE

#### WoodWorks® Sizer

#### for NORDIC STRUCTURES

#### J1 2ND FLOOR.wwb

#### Nordic Sizer - Canada 7.2

Page 2

· ·											
Additional	Data:										
FACTORS:	f/E	KD	KH	KZ	KL	KT	KS	KN	LC#	•	
Vr	2336	1.00	1.00	•••	•••	-	_	-	#2	•	
Mrt	6255	1.00	1.00	_	1.000	-	-	-	#2		
EI	371.1 m		_	-	_	-	-		#2		
CRITICAL LO	AD COME	SINATIONS	S:								
Shear	: LC #2	= 1.25	5D + 1.51								
Moment(+)	: LC #2	= 1.2	5D + 1.51								
Deflectio	n: LC #1	= 1.01	D (perma	anent)							
			0 + 1.0L								
			0 + 1.0L								
			0 + 1.0L								
Bearing			LC #2 = 3								
_			LC #2 = 1								
Load Type					arth,grou						
	L=liv	re (use, o	ccupancy	Ls=1	ive(stora	ge,equi	pment)	f=fire			
Load Patt	erns: s=	:S/2 L=1	[.+Ls _=1	10 patt	ern load	in this	s span				
All Load	Combinat	ions (L	Cs) are l	Listed	in the An	alysis	output				

**CALCULATIONS:** 

Eleff = 447.63 lb-in^2 K= 6.18e06 lbs

"Live" deflection is due to all non-dead loads (live, wind, snow ...)

CONFORMS TO OBG 2012

AMENDED 2020

#### Design Notes:

- 1. WoodWorks analysis and design are in accordance with the 2015 National Building Code of Canada (NBC), Division B, Part 4, and the CSA 086-14 Engineering Design in Wood standard, Update No. 2 (June 2017).
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to Nordic Structures technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.



ows no. yam 6036-20 structural component only

# NORDIC STRUCTURES

**COMPANY** Apr. 9, 2020 09:47 PROJECT
J8 2ND FLOOR.wwb

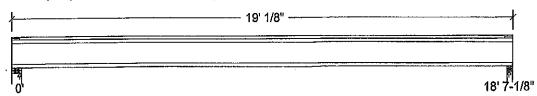
### **Design Check Calculation Sheet**

Nordic Sizer - Canada 7.2

#### Loads:

	Load	Туре	Distribution	Pat- tern	Location Start	[ft] End	Magnitude Start End	Unit
ļ	Load1 Load2	Dead Live	Full Area Full Area	COLII	Beare		20.00	psf psf

### Maximum Reactions (lbs) and Support Bearing (in):



Unfactored: Dead	186		186 372
Live	372		314
Factored: Total	790		790
Bearing:			
Capacity			
Joist	2336		2188
Support	10841		5573
Des ratio		·	, ,,,
Joist	0.34		0.36
Support	0.07	•	0.14 #2
Load case	#2		2-3/8
Length	4-3/8 1-3/4		1-3/4
Min req'd Stiffener	No No		No
KD	1.00		1.00
KB support			-
fcp sup	769		769
Kzcp sup	-	·	-

Bearing for wall supports is perpendicular-to-grain bearing on top plate. No stud design included.

### Nordic 11-7/8" Ni-80 Floor joist @ 12" o.c.

Supports: All - Lumber Wall, No.1/No.2

Total length: 19' 1/8"; Clear span: 18' 5-3/8"; 5/8" nailed and glued OSB sheathing with 1/2" gypsum celling

This section PASSES the design code check.

### Limit States Design using CSA O86-14 and Vibration Criterion:

Criterion	Analysis Value	Design Value	Unit	Analysis/Design
Shear	Vf = 790	Vr = 2336	lbs	Vf/Vr = 0.34
Moment (+)	Mf = 3673	Mr = 11609	lbs~ft	Mf/Mr = 0.32
Perm. Defl'n	0.10 = < L/999	0.62 = L/360	in 🖋	OVE 3310/Va 0.16
Live Defl'n	0.20 = < L/999	0.46 = L/480	in o	0.44
Total Defl'n	0.30 = L/735	0.93 = L/240	in W	41620 0.33
Bare Defl'n	0.22 = L/998	0.62 = L/360		36
Vibration	Lmax = 18'-7.1	Lv = 20'-5.8	ft is a	KATSOULAKOS \$3.91
Defl'n	= 0.027	<b>≈</b> 0.034	in [3] 5.	MAISUULENSU 201.79
Deri II	0,021			and the second of the second o

OWE NO. YAME 037 -20 STRUCTURAL COMPONENT ONLY

OVIVOE OF ONLY

#### WoodWorks® Sizer

#### for NORDIC STRUCTURES

#### J8 2ND FLOOR.wwb

#### Nordic Sizer - Canada 7.2

Page 2

Additional	Data:							·	
FACTORS:	f/E	KD	KH	KZ	$_{ m KL}$	KT	KS	KN	LC#
Vr	2336	1.00	1.00	-	-	-	-	-	#2
Mr+	11609	1.00	1.00	-	1.000	-	<b>~</b> .	-	#2
EI	547.1 m	illion	-	-	-	-	-	_	#2
CRITICAL LO	DAD COMB	INATIONS	3:						
Shear	: LC #2	= 1.25	5D + 1.5I						
Moment (+)	: LC #2	= 1.25	5D + 1.51						
Deflection	on: LC #1	= 1.01	) (perma	nent)					
	LC #2	= 1.01	+1.0L	(live	)				
	LC #2	= 1.0I	0 + 1.0L	(tota	.1)				
			0 + 1.0L						
Bearing	: Suppo	rt 1 - 1	LC #2 = 1	25D +	1.5L				
-			LC #2 = 1						
Load Type	es: D≔dea	d W=wir	nd S≔sno	w H=e	arth, grou	ndwate	c E=ear	thquake	•
					ive(stora			f=fire	
					ern load				
All Load	Combinat	ions (LO	Cs) are l	.isted	in the An	alysis	output		· · · · · · · · · · · · · · · · · · ·
CALCULATION	:RAC							(	CONFORMS TO OBG 2012
Eleff = 6		-in^2 H	<= 6.18e	06 lbs				- 1	
					d loads (	live, v	vind, sn	ow)	AMENDED 2020

#### **Design Notes:**

- 1. WoodWorks analysis and design are in accordance with the 2015 National Building Code of Canada (NBC), Division B. Part 4, and the CSA O86-14 Engineering Design in Wood standard, Update No. 2 (June 2017).
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to Nordic Structures technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.



owa no . yan 6037 - 20 STRUCTURAL COMPONENT 'ONLY





PASSED

### 2ND FLR FRAMING\Dropped Beams\B19A DR(I2421) (Dropped Beam)

**BC CALC® Member Report** 

Dry | 1 span | No cant.

February 18, 2020 08:54:51

Bulld 7239 Job name:

Address:

City, Province, Postal Code: WATERDOWN

CCMC 12472-R

Customer:

Code reports:

•

File name:

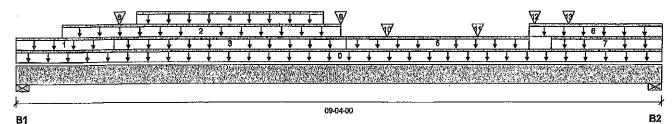
MOUNTAINASH 4 EL 1.mmdl

Description: 2ND FLR FRAMING\Dropped Beams\B19A DR(i2421)

Specifier:

AJ

Designer: Company:



Total Horizontal Product Length = 09-04-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	
B1, 4"	999 / 0	945 / 0	405 / 0	
B2, 5-1/2"	. 1034 / 0	996 / 0	444 / 0	

Lo	ad Summary	i					Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	09-04-00	Тор		10			00-00-00
1	R1(i1840)	Unf. Lin. (lb/ft)	L	00-00-00	01-05-00	Тор		41			n\a
2	Smoothed Load	Unf. Lin. (lb/ft)	L	00-88-00	04-08-00	Тор	205	103			n\a
3	R1(i1840)	Unf. Lin. (lb/ft)	L	01-05-00	04-09-00	Top		81			n\a
4	R1(11840)	Unf. Lin. (lb/ft)	L,	01-09-00	04-05-00	Тор	44	40	92		n\a
5	R1(i1840)	Unf. Lin. (lb/ft)	L	04-09-00	07-05-00	Тор		41			n\a
6	R1(i1840)	Unf, Lin. (lb/ft)	L	07-05-00	09-04-00	Тор		81			n\a
7	R1(i1840)	Unf. Lin. (lb/ft)	L	07-09-00	09-04-00	Тор	44	40	92		n\a
8	R1(i1840)	Conc. Pt. (lbs)	L	01-06-00	01-06-00	Тор	72	90	151		n\a
9	R1(i1840)	Conc. Pt. (lbs)	L	04-08-00	04-08-00	Тор	75	93	156		n\a
10	J4(i1832)	Conc. Pt. (lbs)	L	05-04-00	05-04-00	Тор	273	137			n\a
11	J4(i1774)	Conc. Pt. (lbs)	L	06-08-00	06-08-00	Top	273	137			n\a
12	R1(i1840)	Conc. Pt. (lbs)	L	07-06-00	07-06-00	Тор	72	90	151		n\a
13	J4(j1726)	Conc. Pt. (lbs)	L	08-00-00	08-00-00	Тор	253	126			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	7076 ft-lbs	23220 ft-ibs	30.5%	1	04-05-00
End Shear	3013 lbs	11571 lbs	26.0%	1	01-01-08
Total Load Deflection	L/721 (0.144")	n\a	33.3%	35	04-06-08
Live Load Deflection	L/999 (0.087")	n\a	n\a·	51	04-06-08
Max Defl.	0.144"	n\a	n\a	35	04-06-08
Span / Depth	10,9				

Bearing	Supports	Dlm. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	4" x 3-1/2"	3084 lbs	16.5%	18.1%	Spruce-Pine-Fir
B2	Wall/Plate	5-1/2" x 3-1/2"	3240 lbs	12.6%	13.8%	Spruce-Pine-Fir



owo no.tam 6036-20 Structural Component only





Passed

### 2ND FLR FRAMING\Dropped Beams\B19A DR(i2421) (Dropped Beam)

**BC CALC® Member Report** 

Dry | 1 span | No cant.

February 18, 2020 08:54:51

Build 7239

Job name:

Address: City, Province, Postal Code: WATERDOWN

Description: 2ND FLR FRAMING\Dropped Beams\B19A DR(i2421) Specifier:

Designer:

File name:

Customer: Code reports:

CCMC 12472-R

ΑJ

Company:

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

COMPORMS TO OBC 2012

MOUNTAINASH 4 EL 1.mmdl

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Unbalanced snow loads determined from building geometry were used in selected product's

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

PROVIDE 3 ROWS OF 3½" ARDOX SPIRAL NAILS @ 12. 0/C FOR MULTI-PLY NAILING, MAINTAIN A MIN. Z"LUMBER EDGE/END DISTANCE, DO NOT USE AIR NAILS



DWG NO. TAN 6038-20 STRUCTURAL COMPONENT ONLY

**Disclosure** 

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PASSED

### 2ND FLR FRAMING\Dropped Beams\B20A DR(i2266) (Dropped Beam)

BC CALC® Member Report

Dry | 1 span | No cant.

February 18, 2020 08:54:51

**Bulld 7239** Job name:

Address:

City, Province, Postal Code: WATERDOWN

Customer:

Code reports:

CCMC 12472-R

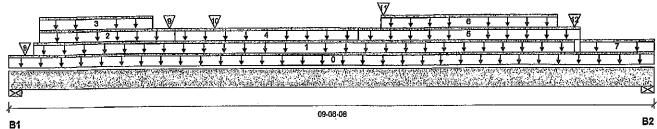
MOUNTAINASH 4 EL 1.mmdl File name:

Description: 2ND FLR FRAMING\Dropped Beams\B20A DR(i2266)

Specifier:

Designer: AJ

Company:



Total Horizontal Product Length = 09-96-08

ru (Down / Hnlift) (lbs)

Reaction St	Milliai A (Dosser : Obs	nity (iwa)				
Bearing	Live	Dead	Snow	Wind		
B1, 2"	1272 / 0	1070 / 0	399 / 0		•	
B2, 4"	1109 / 0	1043 / 0	462 / 0			

l o	ad Summary		:				Live	Dead	Snow	Wind	Tributary
Tag		Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lln. (lb/ft)	L	00-00-00	09-06-08	Тор		10			00-00-00
1	Smoothed Load	Unf. Lin. (lb/ft)	L	00-04-08	08-04-08	Тор	102	52			n\a
2	R1(j1840)	Unf. Lin. (lb/ft)	L	00-05-08	02-05-08	Тор		81			n\a
3	R1(i1840)	Unf. Lin. (lb/ft)	L	00-05-08	02-01-08	Тор	44	40	92		n\a
4	R1(11840)	Unf. Lin. (lb/ft)	L	02-05-08	05-01-08	Тор		41			n\a
5	R1(i1840)	Unf. Lin. (lb/ft)	L	05-01-08	08-05-08	Тор		81			n\a
6	R1(i1840)	Unf. Lin. (lb/ft)	L	05-05-08	08-01-08	Тор	44	40	92		n\a
7	R1(i1840)	Unf. Lin. (lb/ft)	L	08-05-08	09-06-08	Тор		41			n\a
8	-	Conc. Pt. (lbs)	L	00-02-14	00-02-14	Тор	328	164			n\a
9	R1(i1840)	Conc. Pt. (lbs)	L	02-04-08	02-04-08	Top	75	93	156		n\a
10	J4(11794)	Conc. Pt. (lbs)	L	03-00-08	03-00-08	Тор	273	137			n\a
11		Conc. Pt. (lbs)	L	05-05-14	05-05-14	Тор	345	227	151		n\a
12	_	Conc. Pt. (lbs)	L	08-04-08	08-04-08	Тор	348	230	156		n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	7786 ft-lbs	23220 ft-lbs	33.5%	1	05-02-08
End Shear	3399 lbs	11571 lbs	29.4%	1	08-05-00
Total Load Deflection	L/614 (0.179")	n\a	39.1%	35	04-09-00
Live Load Deflection	L/999 (0.108")	n\a	n\a	51	04-09-00
Max Defl.	0.179"	n\a	n\a	35	04-09-00
Span / Depth	11.6				

Beari	ng Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material	
B1	Wall/Plate	2" x 3-1/2"	3645 lbs	39.0%	42.7%	Spruce-Pine-Fir	_
B2	Wall/Plate	4" x 3-1/2"	3430 lbs	18.4%	20.1%	Spruce-Pine-Fir	



STRUCTURAL COMPONENT ONLY





Passed

#### 2ND FLR FRAMING\Dropped Beams\B20A DR(I2266) (Dropped Beam)

**BC CALC® Member Report** 

Dry | 1 span | No cant.

February 18, 2020 08:54:51

**Bulld 7239** 

Job name:

Address: City, Province, Postal Code: WATERDOWN

Customer:

Code reports:

CCMC 12472-R

File name:

MOUNTAINASH 4 EL 1.mmdl

Description: 2ND FLR FRAMING\Dropped Beams\B20A DR(i2266)

Specifier:

Designer: ΑJ

Company:

**Notes** 

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CLNFDAMS TO OBE 2012

Calculations assume unbraced length of Top: 00-01-11, Bottom: 00-01-11.

AMENDED 2020

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadlan Limit States Design, as per NBCC 2015 and CSA 086.

Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

PROVIDES ROWS OF 311" ARDOX SPIRAL NAILS @ /2 " D/C FOR MULTI-PLY NAILING, MAINTAIN A MIN.2 LUMBER EDGE/END DISTANCE DO NOT USE AIR HAILS



#### Disclosure

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**BC CALC® Member Report** 



### Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

#### 2ND FLR FRAMING\Flush Beams\B12(i2257) (Flush Beam)

Dry | 1 span | No cant.

February 18, 2020 08:54:51

**Bulld 7239** 

Job name:

Address:

City, Province, Postal Code: WATERDOWN

**Customer:** 

Code reports:

CCMC 12472-R

File name:

**MOUNTAINASH 4 EL 1.mmdl** 

Description: 2ND FLR FRAMING\Flush Beams\B12(i2257)

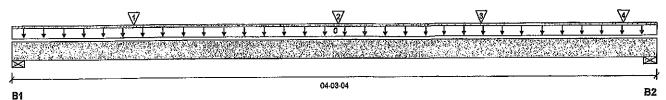
Specifier:

Designer:

ΑJ

Wind

Company:



#### Total Horizontal Product Length = 04-03-04

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	LÌve	Dead
B1, 2-3/4"	602 / 0	326 / 0
B2, 4"	903 / 0	477 / 0

Los	ad Summary						Live	Dead
Tag	•	Load Type	Ref.	Start	End	Loc.	1.00	0.65
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	04-03-04	Тор		12
1	-	Conc. Pt. (lbs)	L	00-09-12	00-09-12	Тор	382	191
2	<b>.</b> .	Conc. Pt. (lbs)	L	02-01-13	02-01-13	Тор	428	215
3	J2(i2353)	Conc. Pt. (lbs)	L	03-01-04	03-01-04	Тор	317	158
4	-	Conc. Pt. (lbs)	L	04-00-10	04-00-10	Тор	378	188

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1459 ft-lbs	35392 ft-lbs	4.1%	1	02-01-04
End Shear	1045 lbs	14464 lbs	7.2%	1	02-11-06
Total Load Deflection	L/999 (0.003")	n\a	n\a	4	02-01-04
Live Load Deflection	L/999 (0.002")	n\a	n\a	5	02-01-04
Max Defl.	0.003"	n\a	n\a	4	02-01-04
Snan / Denth	3.9				

Bearine	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material	
B1	Wall/Plate	2-3/4" x 3-1/2"	1310 lbs	22.1%	11.2%	Spruce-Pine-Fir	•
B2	Wall/Plate	4" x 3-1/2"	1951 lbs	22.7%	11.4%	Spruce-Pine-Fir	

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CANFDAMS TO OBC 2012

Calculations assume member is fully braced.

AMENDED 2020 Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

PROVIDE3 ROWS OF 3½" ARDOX SPIRAL NAILS @ 6 "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN.2 "LUMBER EDGE/END DISTANCE. DO NOT USE AIR NAILS

### own nn. fam 6040-20 STRUCTURAL COMPONENT ONLY

ONINCE OF ON

#### Disclosure

Snow

1.00

Wind

1.15

Tributary

00-00-00 n\a

n\a

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Passed

В2

Tributary

00-00-00 n\a n\a n\a

February 18, 2020 08:54:51

2ND FLR FRAMING\Flush Beams\B13(i2387) (Flush Beam) Dry | 1 span | No cant.

**BC CALC® Member Report** 

**Build 7239** Job name:

Address:

City, Province, Postal Code: WATERDOWN

Customer:

**B**1

Code reports:

CCMC 12472-R

File name:

MOUNTAINASH 4 EL 1.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B13(i2387)

Specifier:

Designer:

Company:

ΑJ

Wind

04-01-12 Total Horizontal Product Length = 04-01-12

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead
B1, 4"	1278 / 0	664/0
B2, 3-3/4"	1032 / 0	541/0

Load 9	Summary						1.170	Deau	SHOW	MAILO	Itibu
	scription	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
	elf-Weight	Unf. Lin. (lb/ft)	L	00-00-00	04-01-12	Top		12			00-00
1 -		Conc. Pt. (lbs)	L.	01-03-07	01-03-07	Top	694	347			
2 -		Conc. Pt. (lbs)	L.	02-03-07	02-03-07	Тор	694	347			
3 -		Conc. Pt. (lbs)	L	03-03-05	03-03-05	Тор	545	273		arista (distribution)	Patenta.
4 J8	(i2418) ·	Conc. Pt. (lbs)	L	00-03-12	00-03-12	Тор	376	188	SACTOR SECTION	OFESS	ONA

Controls Summary	Factored Demand	Factored Resistance	Demand <i>l</i> Resistance	Case	Location
Pos. Moment	2393 ft-lbs	35392 ft-lbs	6.8%	1	02-03-00
End Shear	1870 lbs	14464 lbs	12.9%	1	01-03-14
Total Load Deflection	L/999 (0.004")	n\a	n\a	4	02-00-15
Live Load Deflection	L/999 (0.003")	n\a	n\a	5	02-00-15
Max Defl.	0.004"	n\a	n\a	4	02-00-15
Span / Depth	3.7				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	4" x 3-1/2"	2747 lbs	31.9%	16.1%	Spruce-Pine-Fir
B2	Wall/Plate	3-3/4" x 3-1/2"	2224 lbs	27.5%	13,9%	Spruce-Pine-Fir

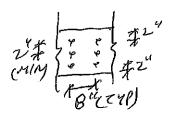
Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

AMENDED 2020 Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086, Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



PROVIDE 3 ROWS OF 312" ARDOX SPIRAL NAILS @ 8 "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN.2 LUMBER EDGE/END DISTANCE, DO NOT USE AIR NAILS und No. Tan 6041 -20 STRUCTURAL COMPONENT ONLY

ONINCE OF ONE

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Passed

February 18, 2020 08:54:51

#### 2ND FLR FRAMING\Flush Beams\B14(i1719) (Flush Beam)

**BC CALC® Member Report** 

**Build 7239** 

Job name:

Address:

City, Province, Postal Code: WATERDOWN

**Customer:** Code reports:

CCMC 12472-R

Dry | 1 span | No cant.

File name:

MOUNTAINASH 4 EL 1.mmdl

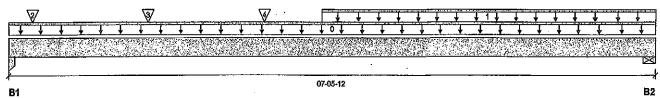
Description: 2ND FLR FRAMING\Flush Beams\B14(i1719)

Wind

Specifier:

Designer: AJ

Company:



#### Total Horizontal Product Length = 07-05-12

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	_
B1, 1-3/4"	748 / 0	417 / 0	
B2 5-1/2"	893 / 0	492 / 0	

L,Oa	ad Summary						• •	LIVE	
Tag	Description	Load Type	Ref.	Start	End	Loc.	<del>.</del> .	1.00	_ {
0	Self-Weight	Ūnf. Lin. (lb/ft)	L	00-00-00	07-05-12	Top			
1	Smoothed Load	Unf. Lin. (lb/ft)	L	03-07-06	07-05-12	Тор		222	
2	J4(i1797)	Conc. Pt. (lbs)	L	00-03-06	00-03-06	Top		206	
3	J4(i1843)	Conc. Pt. (lbs)	L	01-07-06	01-07-06	Тор		163	8
4	J4(i2389)	Conc. Pt. (lbs)	L	02-11-06	02-11-06	Тор		413	2

Controls Summary	Factored Demand	Factored Resistance	Demand <i>l</i> Resistance	Case	Location
Pos. Moment	3010 ft-lbs	35392 ft-lbs	8.5%	1	02-11-06
End Shear	1337 lbs	14464 lbs	9.2%	1	06-00-06
Total Load Deflection	L/999 (0.019")	n\a	n\a	4	03-07-06
Live Load Deflection	L/999 (0.012")	n\a	n\a	5	03-07-06
Max Defl.	0.019"	n\a	n\a	4	03-07-06
Span / Depth	7.1			•	

Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Column	1-3/4" x 3-1/2"	1643 lbs	33.0%	22.0%	Unspecified
B2	Wall/Plate	5-1/2" x 3-1/2"	1955 lbs	16.5%	8.3%	Spruce-Pine-Fir

Design meets Code minimum (L/240) Total load deflection criteria.

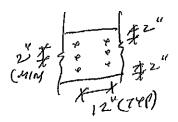
Design meets Code minimum (L/360) Live load deflection criteria.

CANPARMS TO OBE 2012

Calculations assume member is fully braced.

AMENDED 2020 Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086. Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



PROVIDE3 ROWS OF 31/2" ARDOX SPIRAL NAILS @ /2 "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN. 2 LUMBER EDGE/END Distance. Do not use air nails



Wind

Tributary

00-00-00

n\a

n\a

n\a

n\a

Snow

Dead

0.65

12 111

103

82

206

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PASSED

February 18, 2020 08:54:51

#### 2ND FLR FRAMING\Flush Beams\B15(i1830) (Flush Beam)

**BC CALC® Member Report** 

**Build 7239** Job name:

Address:

City, Province, Postal Code: WATERDOWN

Customer:

Code reports:

Load Summary

CCMC 12472-R

Dry I 1 span | No cant.

File name:

MOUNTAINASH 4 EL 1.mmdl

Live

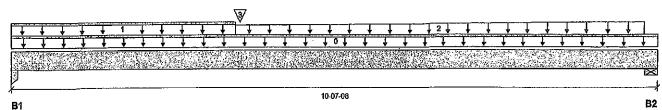
Dead

Description: 2ND FLR FRAMING\Flush Beams\B15(i1830)

Specifier:

Designer: AJ

Company:



#### Total Horizontal Product Length = 10-07-08

Reaction Summary (Down / Uplift) (lbs)

Bearing	Lìve	Dead
B1, 3-1/2"	501/0	290 / 0
R2 5.4/2"	394 / 0	234 / 0

	ad Calliniary	1									
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.16	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	10-07-08	Top		6			00-0
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	03-08-00	Top	3			•	
2	FC2 Floor Material	Trapezoidai (ib	/ft) L	03-08-00		Тор	30	15			
		•			10-04-12		40	20	n Jest id Street	FESSIC	1/1/2016
3 -	B16(i1764)	Conc. Pt. (lbs)	L	03-08-14	03-08-14	Тор	647	336	60 W	CONTRACTOR OF STREET	THE STATE OF
•	and the Community		Factored		and/	_				416	

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos, Moment	3813 ft-lbs	17696 ft-lbs	21.5%	1	03-08-14
End Shear	1096 lbs	7232 lbs	15.2%	1	01-03-06
Total Load Deflection	L/999 (0.085")	n\a	n\a	4	04-11-02
Live Load Deflection	L/999 (0.054")	n\a	n\a	5	04-11-02
Max Defl.	0,085"	n\a	n\a	4	04-11-02
Span / Depth	10.1	a.			

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Column	3-1/2" x 1-3/4"	1113 lbs	22.4%	14.9%	Unspecified
B2	Wall/Plate	5-1/2" x 1-3/4"	883 lbs	14.9%	7.5%	Spruce-Pine-Fir

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

CONFORMS TO OBC 2012

Resistance Factor phi has been applied to all presented results per CSA 086. AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

importance Factor: Normal Part code: Part 9

648 NO. 7AM 6043 -20 STRUCTURAL COMPONENT 'ONLY

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S. KATSOULAKOS

Wind

Tributary

00-00-00 n\a n\a

Snow

Disclosure Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain installation Guide or ask questions, please call (800)232-0788 before installation.





PASSED

2ND FLR FRAMING\Flush Beams\B16(i1764) (Flush Beam)

**BC CALC® Member Report** 

Dry | 1 span | No cant.

February 18, 2020 08:54:51

**Build 7239** Job name:

Address: City, Province, Postal Code: WATERDOWN

File name: Description: MOUNTAINASH 4 EL 1.mmdl

Customer:

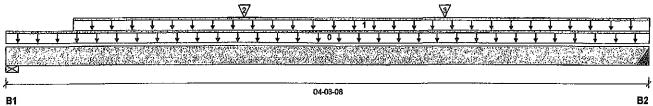
Specifier: Designer: 2ND FLR FRAMING\Flush Beams\B16(i1764)

Code reports:

CCMC 12472-R

ΑJ

Company:



#### Total Horizontal Product Length = 04-03-08

Reaction Sur	nmary (Down / U	plift) (lbs)			
Bearing	Live	Dead	Snow	Wind	 
B1, 5-1/2"	632 / 0	330 / 0			-
B2 2"	664 / 0	345/0			

Load Summary						Live	Dead	Snow	Wind	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	: 1.00	1.15	
0 Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	04-03-08	Тор		6			00-00-00
1 STAIR	Unf. Lin. (lb/ft)	L.	00-05-08	04-03-08	Top	240	120			n\a
2 J6(i1825)	Conc. Pt. (lbs)	L,	01-07-00	01-07-00	Тор	181	91			n\a
3 J6(l1743)	Conc. Pt. (lbs)	L	02-11-00	02-11-00	Тор	191	96	alestron a	FESSIC	n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1419 ft-lbs	17696 ft-lbs	8.0%	1	02-04-00
End Shear	837 lbs	7232 lbs	11.6%	1	01-05-06
Total Load Deflection	L/999 (0.005")	n <b>\a</b>	n\a	4	02-03-08
Live Load Deflection	L/999 (0.004")	n\a	n\a	5	02-03-08
Max Defl.	0.005"	n\a	n\a	4	02-03-08
Span / Depth	3.8				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	5-1/2" x 1-3/4"	1361 lbs	23.0%	11.6%	Spruce-Pine-Fir
B2	Hanger	2" x 1-3/4"	1427 lbs	n\a	33.4%	HUS1.81/10

Header for the hanger HUS1.81/10 at B2 is a Single 1-3/4" x 11-7/8" VERSA-LAM® 1.7 2400 DF. Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

CAMPORMS TO OBO 2012

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86. AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



#### Disclosure

Use of the Boise Cascade Software Is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of sultability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current installation Guide and applicable building codes. To obtain installation Guide or ask questions, please call (800)232-0788 before installation.



**BC CALC® Member Report** 



### Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

#### 2ND FLR FRAMING\Flush Beams\B17(I2170) (Flush Beam)

Dry | 2 spans | No cant.

February 18, 2020 08:54:51

**Build 7239** 

Job name:

Address:

City, Province, Postal Code: WATERDOWN

Customer:

Code reports:

CCMC 12472-R

File name:

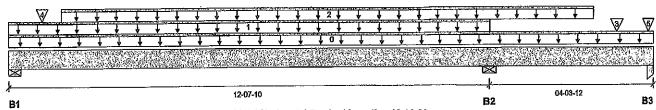
MOUNTAINASH 4 EL 1.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B17(i2170)

Specifier:

Designer: ΑJ

Company:



Total Horizontal Product Length = 16-11-06

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing Live Dead 906/0 1695 / 16 B1, 4-3/8" 2482 / 0 4638 / 0 B2, 5-1/2" 0/164 B3, 3-1/2" 939 / 1231

l o	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	16-11-06	Тор		12			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	12-07-10	Тор	19	9			n\a
,	Smoothed Load	Unf. Lin. (lb/ft)	L	01-04-14	15-04-14	Top	333	167			nla
3	-	Conc. Pt. (lbs)	L	15-11-09	15-11-09	Top	377	158			n\a
1	J1(i2296)	Conc. Pt. (lbs)	L	00-10-14	00-10-14	Top	293	147			n\a
5	J1(i2172)	Conc. Pt. (lbs)	Ĺ	16-09-10	16-09-10	,	316	158			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand <i>i</i> Resistance	Case	Location
Pos. Moment	9514 ft-lbs	35392 ft-lbs	26.9%	2	04-10-14
Neg. Moment	-11329 ft-lbs	-35392 ft-lbs	32.0%	1	12-07-10
End Shear	3318 lbs	14464 lbs	22.9%	2	01-04-04
Cont. Shear	5044 lbs	14464 lbs	34.9%	1	11-05-00
Total Load Deflection	L/934 (0.158")	n\a	25.7%	9	05-10-14
Live Load Deflection	L/999 (0.104")	n\a	n\a	12	05-10-14
Total Neg. Defl.	L/999 (-0.014")	n\a	n\a	9	14-04-02
Max Defl.	0.158"	n\a	n\a	9	05-10-14
Span / Depth	12.5				

Beari	ng Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	4-3/8" x 3-1/2"	3675 lbs	39.0%	19.7%	Spruce-Pine-Flr
B2	Wall/Plate	5-1/2" x 3-1/2"	10059 lbs	84.9%	42.8%	Spruce-Pine-Fir
B3	Column	3-1/2" x 3-1/2"	1262 lbs	12.7%	8.4%	Unspecified
B3	Uplift		2051 lbs			

846 NO. TAN 6045 -20 STRUCTURAL COMPONENT ONLY

Uplift of 2051 lbs found at bearing B3. (51 M/Son





Passed

2ND FLR FRAMING\Flush Beams\B17(i2170) (Flush Beam)

Dry | 2 spans | No cant.

February 18, 2020 08:54:51

**BC CALC® Member Report** 

**Build 7239** 

Job name: Address:

City, Province, Postal Code: WATERDOWN

Customer:

Code reports:

CCMC 12472-R

File name:

MOUNTAINASH 4 EL 1.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B17(i2170)

Specifier:

Designer: AJ

Company:

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBC 2012

Calculations assume member is fully braced.

AMENDED 2020

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

PROVIDE 3 ROWS OF 3½" ARBOX
SPIRAL NAILS @ 12 " O/G FOR
MULTI-PLY NAILING, MAINTAIN
A MIN. 2"LUMBER EDGE/END
DISTANCE. BONOTUSE AIR NAILS

POLINCE OF ONE

DWG NO. TAM 6045 STRUCTURAL COMPONENT ONLY

#### Disclosure

Use of the Bolse Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.



Passed

### 1ST FLR FRAMING\Flush Beams\B18(i3879) (Flush Beam)

**BC CALC® Member Report** 

Dry | 2 spans | No cant.

February 18, 2020 08:54:51

**Build 7239** 

Job name:

Address: City, Province, Postal Code: WATERDOWN

**Customer:** 

Code reports:

CCMC 12472-R

File name:

MOUNTAINASH 4 EL 1.mmdl

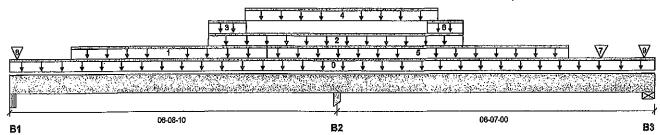
Description: 1ST FLR FRAMING\Flush Beams\B18(i3879)

Specifier:

Designer:

ΑJ

Company:



Total Horizontal Product Length = 13-03-10

Reaction Summary (Down / Unlift) (lbs)

Neaction out	Illiary (Bossii , Spi	iit) (103)			
Bearing	Live	Dead	Snow	Wind	
B1, 5-1/4"	1388 / 344	605 / 0			
B2, 6"	7432 / 0	4243 / 0			
B3, 5-1/2"	2738 / 310	2808 / 0	ī		

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	•	Load Typs	Ref.	Start	End	Loc.	1.00	0.65	1.00	1,16	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	13-03-10	Тор		12			00-00-00
1	Smoothed Load	Unf. Lin. (lb/ft)	L.	01-03-04	05-03-04	Тор	363	182			n\a
2	5(1592)	Unf. Lin. (lb/ft)	Ĺ	04-01-02	09-04-02	Тор		81			n\a
3	5(1592)	Unf. Lin. (lb/ft)	L	04-01-02	04-10-02	Top	1209	638			n\a
4	5(i592)	Unf, Lin. (lb/ft)	L	04-09-14	08-09-14	Top	698	349			n\a
5	Smoothed Load	Unf. Lin. (lb/ft)	L	05-03-04	11-06-08	Тор	348	174			n\a
6	5(1592)	Unf. Lin. (lb/ft)	L	08-07-02	09-04-02	Тор	2107	1087			n\a
7	J2(i3755)	Conc. Pt. (lbs)	L.	12-02-08	12-02-08	Тор	522	267			n\a
8	4(1589)	Conc. Pt. (lbs)	L	00-01-14	00-01-14	Тор	178	118			n\a
9	1(i533)	Conc. Pt. (lbs)	L	13-00-14	13-00-14	Тор	1208	2139			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	6231 ft-lbs	35392 ft-lbs	17.6%	3	09-04-02
Neg. Moment	-9047 ft-lbs	-35392 ft-lbs	25.6%	1	06-08-10
End Shear	2710 lbs	14464 lbs	18.7%	3	11-10-04
Cont. Shear	7571 lbs	14464 lbs	52.3%	1	07-11-08
Total Load Deflection	L/999 (0.027")	n\a	n\a	10	09-09-08
Live Load Deflection	L/999 (0.02")	n\a	n\a	13	09-09-08
Total Neg. Defl.	L/999 (-0.006")	n\a	n\a	10	04-10-02
Max Defi.	0.027"	n\a	n\a	10	09-09-08
Span / Depth	6.4				,

Bearing	g Supports	Dlm. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Beam	5-1/4" x 3-1/2"	2838 lbs	28.9%	12.7%	Unspecified
B2	Column	6" x 3-1/2"	16451 lbs	96.5%	64.2%	Unspecified
B3	Wall/Plate	5-1/2" x 3-1/2"	7617 lbs	64.3%	32.4%	Spruce-Pine-Fir



DWB NO. TAM 6046-20 STRUCTURAL COMPONENT ONLY





Passed

### 1ST FLR FRAMING\Flush Beams\B18(i3879) (Flush Beam)

Dry | 2 spans | No cant.

February 18, 2020 08:54:51

**BC CALC® Member Report Build 7239** 

Job name: Address:

Code reports:

City, Province, Postal Code: WATERDOWN Customer:

CCMC 12472-R

File name:

MOUNTAINASH 4 EL 1.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B18(i3879)

Specifier:

Designer: AJ

Company:

Notes -

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBC 2012

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

PROVIDE3 ROWS OF 312" ARDOX SPIRAL NAILS @ 6 "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN. 2"LUMBER EDGE/END DISTANCE, DO NOT USE AIR NAILS



DWG NO. TAN 6046-20 STRUCTURAL COMPONENT ONLY

#### Disclosure

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Passed

February 18, 2020 08:54:51

1ST FLR FRAMING\Flush Beams\B2(i3880) (Flush Beam)

**BC CALC® Member Report** 

**Bulld 7239** 

Job name: Address:

City, Province, Postal Code: WATERDOWN

Customer: Code reports:

CCMC 12472-R

Dry | 2 spans | L cant.

File name:

MOUNTAINASH 4 EL 1.mmdl

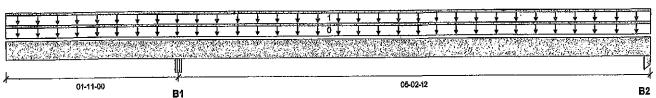
Wind

Description: 1ST FLR FRAMING\Flush Beams\B2(i3880)

Specifier:

Designer: AJ

Company:



Total Horizontal Product Length = 07-01-12

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead		
B1, 5-1/4"	90/0	103/0		
B2. 1-3/4"	49/7	49/0		

l o	ad Summary					. 1	Live	Dead	Snow	Wind	Tributary
Tag		Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Welght	Unf. Lin. (lb/ft)	L	00-00-00	07-01-12	Top		12			00-00-00
1	FC1 Floor Material	Unf, Lin. (lb/ft)	L	00-00-00	07-01-12	Тор	19	9	•		n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	157 ft-lbs	35392 ft-lbs	0.4%	3	04-07-14
Neg. Moment	-100 ft-lbs	-35392 ft-lbs	0.3%	1	01-11-00
End Shear	73 lbs	14464 lbs	0.5%	3	06-00-02
Cont. Shear	94 lbs	14464 lbs	0.6%	1	03-01-08
Total Load Deflection	L/999 (0.001")	n\a	n\a	10	04-06-13
Live Load Deflection	2xL/1998 (-0")	n\a	n\a	13	00-00-00
Total Neg. Defl.	2xL/1998 (-0")	n\a	n\a	10	00-00-00
Max Defl.	0.001"	n\a	n\a	10	04-06-13
Snan / Denth	5.2				

Bearing	Supports	Dlm. (LxW)	Demand	Demand/ Resistance Support	Demand <i>i</i> Resistance Member	Material
B1	Beam	5-1/4" x 3-1/2"	264 lbs	2.7%	1.2%	Unspecified
B2	Column	1-3/4" x 3-1/2"	135 lbs	2.7%	1.8%	Unspecified

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets User specified (2xL/360) Live load deflection criteria.

CONFORMS TO OBC 2012

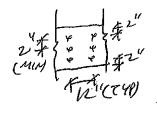
Calculations assume member is fully braced.

AMENDED 2020 Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Cantilevers require sheathed bottom flanges, blocking at cantilever support and closure at ends.



PROVIDE3 ROWS OF 3½" ARDOX SPIRAL NAILS @ 12 "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN.2"LUMBER EDGE/END DISTANCE. DO NOT USE AIR NAILS



### DWA NO. TAN 6047 -20 STRUCTURAL COMPONENT ONLY

#### Disclosure

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Passed

### 1ST FLR FRAMING\Flush Beams\B3(l3832) (Flush Beam)

Dry | 1 span | No cant.

February 18, 2020 08:54:51

**Build 7239** 

Job name:

Address:

**BC CALC® Member Report** 

City, Province, Postal Code: WATERDOWN

Customer:

Code reports:

CCMC 12472-R

File name:

**MOUNTAINASH 4 EL 1.mmdl** 

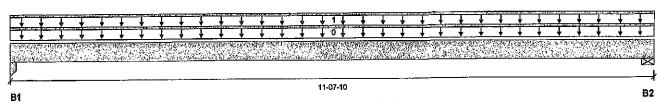
1ST FLR FRAMING\Flush Beams\B3(i3832) Description:

Wind

Specifier:

Designer: AJ

Company:



Total Horizontal Product Length = 11-07-10

Snow

Reaction Summary (Down / Uplift) (lbs)

Dead Live B1, 1-3/4" 124 / 0 108 / 0 124 / 0 B2, 1-7/8" 108 / 0

	Los	ad Summary						Live	Dead	Snow	Wind	Tributary
		Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
•	0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	11-07-10	Тор		12			00-00-00
	1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	11-07-10	Top	19	9			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	894 ft-lbs	35392 ft-lbs	2.5%	1	05-09-12
End Shear	255 lbs	14464 lbs	1.8%	1	01-01-10
Total Load Deflection	L/999 (0.016")	n\a	n\a	4	05-09-12
Live Load Deflection	L/999 (0.007")	n\a	n\a	5	05-09-12
Max Defl.	0.016"	n\a	n\a	4	05-09-12
Span / Depth	11.6				

Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Column	1-3/4" x 3-1/2"	316 lbs	6.4%	4.2%	Unspecified
<b>2</b> 2	Mol/Dieto	1.7/8" v 3.1/2"	317 lbs	7.9%	4.0%	Spruce-Pine-Fir

<u>Disclosure</u>

CUMP 17/1/13 7/1 OBR 2012 Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of sultability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain installation Guide or ask questions, please call (800)232-0788 before installation.

> BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

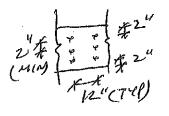
Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

AMENDED 2020 Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



PROVIDE ROWS OF 3%" ARDOX SPIRAL NAILS @ 12-" 0/C FOR MULTE-PLY NAILING, MAINTAIN A MIN.2" LUMBER EDGE/END DISTANCE, BOHOT USE AIR NAILS



ung no. Tan 6048 -20 STRUCTURAL COMPONENT ONLY



Passed

1ST FLR FRAMING\Flush Beams\B4(i3765) (Flush Beam)

Dry [ 1 span | No cant.

February 18, 2020 08:54:51

**BC CALC® Member Report Build 7239** 

Job name:

Address:

City, Province, Postal Code: WATERDOWN

Customer:

Code reports:

CCMC 12472-R

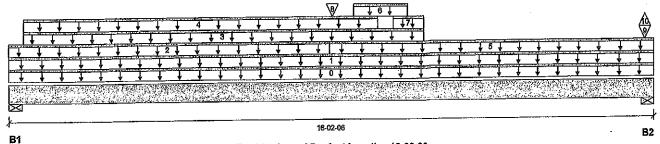
File name: MOUNTAINASH 4 EL 1.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B4(i3765)

Specifier:

Designer: ΑJ

Company:



Total Horizontal Product Length = 16-02-06

Reaction Summary (Down / Uplift) (lbs)

	minary (mounts of		Chau	Wind		
Bearing	Live	Dead	Snow	Aktio		
B1, 1-7/8"	1167 / 0	1263 / 0				
B2. 5-1/2"	1277 / 5	1062 / 0				

Los	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	*	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	16-02-06	Тор		12			00-00-00
1	FC1 Floor Material	Unf, Lin. (lb/ft)	L	00-00-00	16-02-06	Тор	26	13			n\a
2	FC1 Floor Material	Unf, Lin. (lb/ft)	L	00-00-00	08-00-08	Тор	6	3			n\a
3	6(i614)	Unf. Lin. (lb/ft)	L	00-04-06	10-04-10	Тор		81			n\a
4	6(1614)	Unf. Lin. (lb/ft)	L	00-04-06	09-02-14	Top	35	23			n\a
5	FC1 Floor Material	Unf, Lin, (lb/ft)	Ĺ	08-00-08	16-02-06	Тор	7	4			n\a
6	6(i614)	Unf. Lin. (lb/ft)	Ĺ	08-07-12	09-11-12	Top	487	254			n\a
7	6(1614)	Unf. Lin. (lb/ft)	Ĺ	09-07-10	10-04-10	qoT	279	139			n\a
8	, ,	Conc. Pt. (lbs)	ī	08-01-06	08-01-06	Top	709	365			n\a
_	B5(i3861)	Conc. Pt. (lbs)	ī	15-11-10	15-11-10		36	41			n\a
9 10	2(1532) 2(1532)	Conc. Pt. (lbs)	Ĺ	15-11-10	15-11-10	Тор	-5				n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	18495 ft-lbs	35392 ft-lbs	52.3%	1	08-01-06
End Shear	3234 lbs	14464 lbs	22.4%	1	01-01-12
Total Load Deflection	L/343 (0.549")	n\a	69.9%	6	80-00-80
Live Load Deflection	L/642 (0.294")	n\a	56.1%	8	08-01-06
Max Defl.	0.549"	n\a	n\a	6	80-00-80
Span / Depth	15.9				

Rearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	1-7/8" x 3-1/2"	3330 lbs	82.5%	41.6%	Spruce-Pine-Fir
B2	Wall/Plate	5-1/2" x 3-1/2"	3244 lbs	27.4%	13.8%	Spruce-Pine-Fir



646 NO. TAM6049 -20 STRUCTURAL COMPONENT ONLY





Passed

1ST FLR FRAMING\Flush Beams\B4(i3765) (Flush Beam)

BC CALC® Member Report

Dry | 1 span | No cant.

February 18, 2020 08:54:51

**Build 7239** 

Job name: Address:

Customer:

Code reports:

City, Province, Postal Code: WATERDOWN

Specifier: Designer:

File name:

Description:

CCMC 12472-R

ΑJ

Company:

**Notes** 

Design meets Code minimum (L/240) Total load deflection criteria,

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBC 2012

MOUNTAINASH 4 EL 1.mmdl

1ST FLR FRAMING\Flush Beams\B4(i3765)

Calculations assume member is fully braced.

AMENDED 2020

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

PROVIDE∃ROWS OF 3½" ARDOX SPIRAL NAILS @ 12-" 0/C FOR MULTI-PLY NAILING, MAINTAIN A MIN 2"LUMBER EDGE/END DISTANCE, DO HOT USE AIR NAILS



AWS NO. TAN 6049-20 STRUCTURAL COMPONENT ONLY

#### Disclosure

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1ST FLR FRAMING\Flush Beams\B5(i3861) (Flush Beam)

**BC CALC® Member Report** 

Dry | 1 span | No cant.

February 18, 2020 08:54:51

Passed

**Build 7239** Job name:

Address: City, Province, Postal Code: WATERDOWN File name:

MOUNTAINASH 4 EL 1.mmdl

Live

Description:

1ST FLR FRAMING\Flush Beams\B5(l3861)

Specifier:

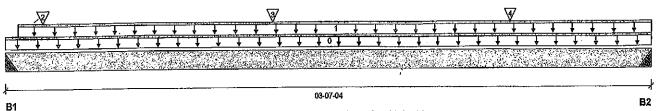
Designer: ΑJ

Customer: Code reports:

**Load Summary** 

Company:

CCMC 12472-R



#### Total Horizontal Product Length = 03-07-04

Reaction Su	mmary (Down / U	plift) (lbs)			
Bearing	Live	Dead	Snow	Wind	<u></u>
B1, 2"	699 / 0	360 / 0			
B2, 2"	663 / 0	343/0			

Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-07-04	Тор		6		;	00-00-00
1	STAIR	Unf. Lin. (lb/ft)	L	00-00-14	03-07-04	Top	240	120			n\a
2	J5(i3844)	Conc. Pt. (lbs)	L	00-02-08	-00-02-08	Top	129	64			n\a
2	J5(13855)	Conc. Pt. (lbs)	L	01-05-12	01-05-12	Top	206	103		يوس دمير	n\a
1	J5(13872)	Conc. Pt. (lbs)	L	02-09-12	02-09-12	Top	180	90	ower C	FESSIO	n\a
	33(13072)	••••••••••••••••••••••••••••••••••••••				•			A Section of the second	PARTIE NAME OF THE PARTY OF THE	42 . Y.
_			Factored	Dem				ý		11/1	
Co	ntrois Summary	Factored Demand	Resistance	Resi	stance	Case	Location	1.	S & word	SOME PROPERTY.	annana 🔊 🖡

Controls Summary	Factored Demand	ractored Resistance	Resistance	Case	Location
Pos. Moment	1204 ft-lbs	17696 ft-lbs	6.8%	1	01-07-03
End Shear	684 lbs	7232 lbs	9.5%	1	02-05-06
Total Load Deflection	L/999 (0.004")	n\a	n\a	4	01-09-12
Live Load Deflection	L/999 (0.002")	n\a	n\a	5	01-09-12
Max Defl.	0.004"	n\a	n\a	4	01-09-12
Snan / Depth	3.4				

Bearing	g Supports	Dlm. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Hanger	2" x 1-3/4"	1499 lbs	n\a	35.1%	HUS1.81/10
B2	Hanger	2" x 1-3/4"	1423 lbs	n/a	33.3%	HUS1.81/10

#### **Cautions**

Header for the hanger HUS1.81/10 at B1 is a Double 1-3/4" x 11-7/8" VERSA-LAM® 1.7 2400 DF. Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

Header for the hanger HUS1.81/10 at B2 is a Double 1-3/4" x 11-7/8" VERSA-LAM® 1.7 2400 DF.

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

AMENDED 2020

Hanger Manufacturer: Unassigned Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### 148 NO. TAM 6050 -20 STRUCTURAL COMPONENT ONLY

ONNICE OF ON

#### Disclosure

Snow

Dead

Wind

Tributary

Use of the Bolse Cascade Software Is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Bolse Cascade engineered wood products must be in CUNFORMS TO OBC 2012 accordance with current installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.





PASSED

Tributary

1ST FLR FRAMING\Flush Beams\B6(i3876) (Flush Beam)

**BC CALC® Member Report** 

**Build 7239** Job name: Dry | 1 span | No cant.

February 18, 2020 08:54:51

Address: City, Province, Postal Code: WATERDOWN

Customer: Code reports:

CCMC 12472-R

File name:

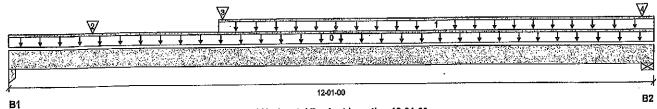
MOUNTAINASH 4 EL 1.mmdl

1ST FLR FRAMING\Flush Beams\B6(i3876) Description:

Specifier:

Designer: ΑJ

Company:



Total Horizontal Product Length = 12-01-00

/Davin / Hallfth /lbal

Reaction Sun	Illiary (Down of	mity (ina)		****
Bearing	Live	Dead	Snow	Wind
B1, 3-1/2"	2577 / 0	1554 / 0		
B2. 5-1/2"	772 / 0	520 / 0		•

Lo Tag	ad Summary Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15
0	Self-Weight	Unf. Lin. (lb/ft)	L	0.0-00-00	12-01-00	Тор		- 12		
1	FC1 Floor Material	Unf. Lin. (lb/ft)	,.L	03-11-02	12-01-00	Тор	27	13		
'n	-	Conc. Pt. (lbs)	L	01-07-02	01-07-02	Тор	2325	1374		
3	B5(i3861)	Conc. Pt. (lbs)	L	04-00-00	04-00-00	Тор	654	338	and .	والمالة الأصاب استأرين
4	2(i532)	Conc. Pt. (lbs)	L	11-10-04	11-10-04	Тор	127	95	6201	FESSIO

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	9181 ft-lbs	35392 ft-lbs	25.9%	1	04-00-00
End Shear	5775 lbs	14464 lbs	39.9%	1	01-03-06
Total Load Deflection	L/995 (0.138")	n\a	24.1%	4	05-04-03
Live Load Deflection	L/999 (0.086")	n\a	n\a	5	05-04-03
Max Defi.	0.138"	n\a	n\a	4	05-04-03
Span / Denth	11.6				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	<u>Material</u>
B1	Column	3-1/2" x 3-1/2"	5808 lbs	58.4%	38.9%	Unspecified
B2	Wall/Plate	5-1/2" x 3-1/2"	1807 lbs	15.3%	7.7%	Spruce-Pine-Fir

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

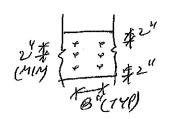
CONFORMS TO OBC 2012

Calculations assume member is fully braced.

AMENDED 2020 Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadlan Limit States Design, as per NBCC 2015 and CSA 086. Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



PROVIDE 3 ROWS OF 312" ARDOX SPIRAL NAILS @ 8 "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN.2" LUMBER EDGE/END DISTANCE. DO NOT USE AIR NAILS



#### Disclosure

Use of the Boise Cascade Software Is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of sultability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Bolse Cascade engineered wood products must be in accordance with current installation Gulde and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before Installation.





PASSED

### 1ST FLR FRAMING\Flush Beams\B7(i3864) (Flush Beam)

Dry | 1 span | No cant.

February 18, 2020 08:54:51

**Build 7239** 

Job name: Address:

City, Province, Postal Code: WATERDOWN

**BC CALC® Member Report** 

Customer: Code reports:

CCMC 12472-R

File name:

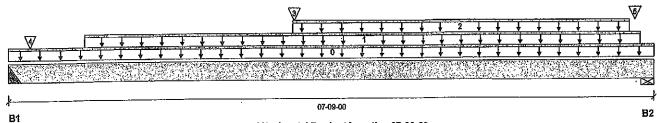
MOUNTAINASH 4 EL 1.mmdl

1ST FLR FRAMING\Flush Beams\B7(i3864) Description:

Specifier:

ΑJ

Designer: Company:



#### Total Horizontal Product Length = 07-09-00

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead
B1, 4"	2280 / 0	1354 / 0
B2 3-4/2"	1726 / 0	976 / 0

Las	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	<u>;</u>
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	07-09-00	Тор		12			00-00-00
1	Smoothed Load	Unf. Lin. (lb/ft)	L.	00-11-00	07-07-00	Тор	216	108			. n\a
,	STAIR	Unf. Lin. (lb/ft)	L	03-04-11	07-05-08	Тор	120	60			n\a
3	B9(i3871)	Conc. Pt. (lbs)	L	03-04-14	03-04-14	Top	63	36			n\a
4	-	Conc. Pt. (lbs)	L	00-03-04	00-03-04	Top	1439	884			n\a
5	E10(i425)	Conc. Pt. (lbs)	L	07-06-04	07-06-04	Top	545	338			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	4497 ft-lbs	35392 ft-lbs	12.7%	1	04-03-00
End Shear	2014 lbs	14464 lbs	13.9%	1	06-05-10
Total Load Deflection	L/999 (0.03")	n\a	n\a	4	03-11-08
Live Load Deflection	L/999 (0.019")	n\a	n\a	5	03-11-08
Max Defl.	0.03"	n\a	n\a	4	03-11-08
Snan / Denth	7.3				

Bearing	g Supports	Dim. (LxW)	Demand	Demand <i>i</i> Resistance Support	Demand/ Resistance Member	Material	
B1	Hanger	4" x 3-1/2"	5113 lbs	n\a	29.9%	HGUS410	
B2	Wall/Plate	3-1/2" x 3-1/2"	3809 lbs	50.5%	25.5%	Spruce-Pine-Fir	

### Cautions

Header for the hanger HGUS410 at B1 is a Double 1-3/4" x 11-7/8" VERSA-LAM® 1.7 2400 DF. Hanger model HGUS410 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.



DWG NO. TAM 6052 -20 STRUCTURAL COMPONENT ONLY



**BC CALC® Member Report** 



# Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

1ST FLR FRAMING\Flush Beams\B7(i3864) (Flush Beam)

Dry | 1 span | No cant.

February 18, 2020 08:54:51

**Bulld 7239** 

Job name: Address:

City, Province, Postal Code: WATERDOWN

File name:

MOUNTAINASH 4 EL 1.mmdl

Description:

1ST FLR FRAMING\Flush Beams\B7(i3864) Specifier:

Designer:

Company:

Customer:

Code reports:

CCMC 12472-R

AJ

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

CAMPORMS TO OBC 2012

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

PROVIDE 3 ROWS OF 31/2" ARDOX SPIRAL NAILS @ 8 "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN.2"LUMBER EDGE/END DISTANCE DO HOT USE AIR NAILS



DWG NO. TAM6052-28 STRUCTURAL COMPONENT ONLY

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of sultability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Bolse Cascade engineered wood products must be in accordance with current installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.





City, Province, Postal Code: WATERDOWN

### Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

February 18, 2020 08:54:51

1ST FLR FRAMING\Flush Beams\B8(i3867) (Flush Beam)

BC CALC® Member Report

**Build 7239** 

Customer:

**B**1

Job name: Address:

Dry I 1 span I No cant.

File name:

MOUNTAINASH 4 EL 1.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B8(i3867)

Specifier:

Designer: ΑJ

Company:



CCMC 12472-R

03-09-04

B2

#### Total Horizontal Product Length = 03-09-04

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead
B1, 5-1/4"	52 / 0	38/0
B2. 3-1/2"	48 / 0	35/0

Lo: Tag	ad Summary  Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-09-04	Тор		6			00-00-00
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	03-09-04	Top	27	13			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	80 ft-lbs	17696 ft-lbs	0.5%	1	01-11-08
End Shear	34 lbs	7232 lbs	0.5%	1	01-05-02
Total Load Deflection	L/999 (0")	n\a	n\a	4	01-11-08
Live Load Deflection	L/999 (0")	n\a	n\a	5	01-11-08
Max Defl,	0"	n\a	n\a	4	01-11-08
Span / Depth	3.2				

Bearing	Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Column	5-1/4" x 1-3/4"	126 lbs	1.7%	1.1%	Unspecified
B2	Column	3-1/2" x 1-3/4"	116 lbs	2.3%	1.6%	Unspecified

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86. AMENDED 2020 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



### ONG NO. TAM6053 STRUCTURAL COMPONENT ONLY

#### Disclosure

COMPORMS TO OBC 2012

Use of the Bolse Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of Input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.







February 18, 2020 08:54:51

1ST FLR FRAMING\Flush Beams\B9(i3871) (Flush Beam) Dry | 1 span | No cant.

**BC CALC® Member Report** 

**Build 7239** 

Job name:

Address:

City, Province, Postal Code: WATERDOWN

Customer:

Code reports:

CCMC 12472-R

Description:

File name:

MOUNTAINASH 4 EL 1.mmdl

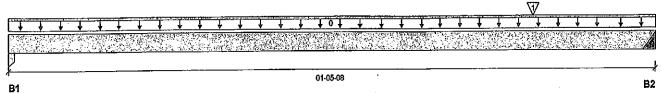
1ST FLR FRAMING\Flush Beams\B9(i3871)

Wind

Specifier:

Designer: ΑJ

Company:



Total Horizontal Product Length = 01-05-08

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead		
B1, 1-3/4"	8/0	8/0		
B2. 2"	55 / 0	32/0		

Lo	ad Summary							Live	Dead	Snow	Wind	Tributary
Tag	•	Load Type	Ref.	Start	End	Loc.	<u> </u>	1.00	0.65	1.00	1.15	
0	Self-Welght	Unf. Lin. (lb/ft)	L	00-00-00	01-05-08	Top	-		6			00-00-00
1	J6(i3869)	Conc. Pt. (lbs)	L	01-02-04	01-02-04	Top		63	32			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand <i>i</i> Resistance	Case	Location
Pos. Moment	20 ft-lbs	17696 ft-lbs	0.1%	1	01-02-04
End Shear	14 lbs	7232 lbs	0.2%	1	01-01-10
Snan / Denth	1.3				

Bearing	, Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material	
B1	Column	1-3/4" x 1-3/4"	23 lbs	0.9%	.0.6%	Unspecified	
Dο	Danger	2" v 1_3///"	123 lbs	n\a	2.9%	HUS1.81/10	

**Cautions** 

Header for the hanger HUS1.81/10 at B2 is a Double 1-3/4" x 11-7/8" VERSA-LAM® 1.7 2400 DF. Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

#### Notes

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

CANPORMS TO OBE 201

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



UWA NO. TAM GOSY -20 STRUCTURAL COMPONENT ONLY

### **Disclosure**

Use of the Bolse Cascade Software is subject to the terms of the End User License Agreement (EULA). 2 Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.





Passed

February 18, 2020 09:06:05

2ND FLR FRAMING\Dropped Beams\B19 DR(i4489) (Dropped Beam)

**BC CALC® Member Report** 

Build 7239 Job name:

Address:

City, Province, Postal Code: WATERDOWN

Customer:

Code reports:

Dry | 1 span | No cant.

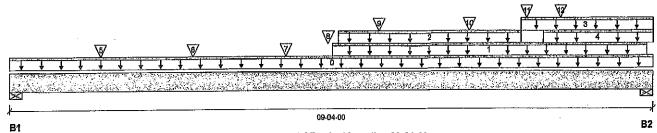
MOUNTAINASH 4 EL 2.mmdl File name:

2ND FLR FRAMING\Dropped Beams\B19 DR(i4489) Description:

Specifier:

Designer:

Company:



#### Total Horizontal Product Length = 09-04-00

Reaction Summary (Down / Uplift) (lbs)

CCMC 12472-R

Medicing duminal from the first									
Bearing	Live	Dead	Snow	Wind					
B1. 4"	1206 / 0	1090 / 0	524 / 0						
B2, 5-1/2"	1274 / 0	1230 / 0	780 / 0						

Los	ad Summary						Live	Dead	Snow	·Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf, Lin. (lb/ft)	L.	00-00-00	09-04-00	Top		10			00-00-00
1	ROOF	Unf. Lin. (lb/ft)	L	04-08-00	09-02-14	Top	33	30	63		n\a
2	R1(i4465)	Unf, Lin. (lb/ft)	L	04-09-00	07-05-00	Тор		41			`n <b>\a</b>
3	R1(i4465)	Unf. Lin. (lb/ft)	L	07-05-00	09-04-00	Top		81			n\a
4	R1(i4465)	Unf. Lin. (lb/ft)	L	07-09-00	09-04-00	Тор	44	40	92		n\a
5	J4((4437)	Conc. Pt. (lbs)	L	01-04-00	01-04-00	Тор	414	342	170		n\a
6	J4(i4460)	Conc. Pt. (lbs)	L	02-08-00	02-08-00	Тор	394	331	127		n\a
7	J4(j4484)	Conc. Pt. (lbs)	L	04-00-00	04-00-00	Top	286	237	87		n\a
8	B21(i4467)	Conc. Pt. (lbs)	L	04-07-04	04-07-04	Top	282	331	197		n\a
9	J4(14447)	Conc. Pt. (lbs)	L.	05-04-00	05-04-00	Тор	244	136	69		n\a
10	J4((4129)	Conc. Pt. (lbs)	L	06-08-00	06-08-00	Top	273	137			n\a
11	R1(14465)	Conc. Pt. (lbs)	L	07-06-00	07-06-00	Top	72	90	151		n\a
12	J4(i3973)	Conc. Pt. (lbs)	L	08-00-00	08-00-00	Тор	286	156	69		ુહ્, n∖a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	9223 ft-lbs	23220 ft-lbs	39.7%	1	04-07-04
End Shear	3683 lbs	11571 lbs	31,8%	1	01-01-08
Total Load Deflection	L/561 (0.185")	nla	42.8%	35	04-07-04
Live Load Deflection	L/999 (0.114")	n\a	n\a	51	04-07-04
Max Defl.	0.185"	n\a	n\a	35	04-07-04
Span / Depth	10.9				

Bea	ring Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	<u>Material</u>	
B1	Wall/Plate	4" x 3-1/2"	3696 lbs	32.5%	21.6%	Spruce-Pine-Fir	
B2	Wall/Plate	5-1/2" x 3-1/2"	4227 lbs	16.5%	18.0%	Spruce-Pine-Fir	



6W6 NO. TAM 6055 -20 STRUCTURAL CONFONENT ONLY





City, Province, Postal Code: WATERDOWN

### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING\Dropped Beams\B19 DR(i4489) (Dropped Beam)

Passed

**BC CALC® Member Report** 

**Bulld 7239** 

Job name: Address:

Dry | 1 span | No cant.

February 18, 2020 09:06:05

MOUNTAINASH 4 EL 2.mmdl File name:

2ND FLR FRAMING\Dropped Beams\B19 DR(i4489) Description:

Specifier:

Designer:

Customer: Code reports:

CCMC 12472-R

Company:

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBC 2012

Calculations assume unbraced length of Top: 01-01-08, Bottom: 01-01-08.

AMENDED 2020

Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Unbalanced snow loads determined from building geometry were used in selected product's

verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

PROVIDE3 ROWS OF 3½" ARDOX SPIRAL NAILS @ 8 "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN. 2 LUMBER EDGE/END DISTANCE, DO NOT USE AIR NAILS



6w6 No . Tan 6*055 -* 20 STRUCTURAL COMPONENT ONLY

Disclosure

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PASSED

February 18, 2020 09:06:05

2ND FLR FRAMING\Dropped Beams\B20 DR(i4444) (Dropped Beam)

BC CALC® Member Report

**Build 7239** Job name:

Address:

City, Province, Postal Code: WATERDOWN

Customer:

Code reports:

CCMC 12472-R

Dry | 1 span | No cant.

File name:

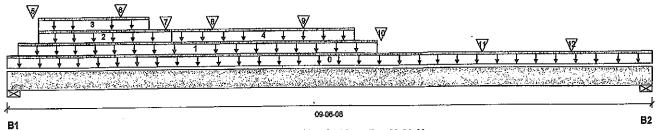
MOUNTAINASH 4 EL 2.mmdl

2ND FLR FRAMING\Dropped Beams\B20 DR(i4444) Description:

Specifier:

Designer:

Company:



Total Horizontal Product Length = 09-06-08

Reaction Summary (Down / Uplift) (lbs)

IZGAGUUN GU	IIIIII AI A IS CAMILLA C	0111t) (100)		•	
Bearing	Live	Dead	Snow	Wind	
B1. 2"	1316 / 0	1199 / 0	738 / 0		
B2, 4"	1285 / 0	1195 / 0	623 / 0		

l o	ad Summary						Live	Dead	Snow	. Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	09-06-08	Тор		10	"		00-00-00
1	ROOF	Unf. Lin. (lb/ft)	L	00-02-00	05-05-08	Тор	33	30	63		n\a
2	R1(i4438)	Unf. Lin. (lb/ft)	L	00-05-08	02-05-08	Top		81			n\a
3	R1(i4438)	Unf. Lin. (lb/ft)	L	00-05-08	02-01-08	Тор	44	40	92		n <b>\</b> a
4	R1(I4438)	Unf. Lin. (lb/ft)	L	02-05-08	05-01-08	Тор		41			n\a
5	J4(14000)	Conc. Pt. (lbs)	L	00-04-08	00-04-08	Тор	174	87 .			n\a
6	J4(i3957)	Conc. Pt. (lbs)	L.	01-08-08	01-08-08	Тор	303	165	69		n\a
7	R1(i4438)	Conc. Pt. (lbs)	L	02-04-08	02-04-08	Тор	75	93	156		n\a
8	J4(j4129)	Conc. Pt. (lbs)	L	03-00-08	03-00-08	Тор	273	137			n\a
9	J4(i4461)	Conc. Pt. (lbs)	L	04-04-08	04-04-08	Top	295	161	69		n\a
10	0-1(1-10-1) .	Conc. Pt. (lbs)	L.	05-06-03	05-06-03	Top	430	478	260		n\a
11	J4(i4479)	Conc. Pt. (lbs)	L	07-00-08	07-00-08	Тор	371	320	125		n\a
12	J4(14471)	Conc. Pt. (lbs)	L	08-04-08	08-04-08	Тор	427	362	195		n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	9800 ft-lbs	23220 ft-lbs	42,2%	1	04-09-00
End Shear	4031 lbs	11571 lbs	34.8%	1	08-05-00
Total Load Deflection	L/482 (0.228")	ก\ล	49.8%	<b>35</b> .	04-09-00
Live Load Deflection	Ľ777 (0.142")	n\a	46.3%	51	04-09-00
Max Defl.	0.228"	n\a	n <b>\</b> a	35	04-09-00
Span / Depth	11.6				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	2" x 3-1/2"	4211 lbs	45.1%	49.3%	Spruce-Pine-Fir
B2	Wall/Plate	4" x 3-1/2"	4044 lbs	35.6%	23.7%	Spruce-Pine-Fir



040 NO. TAN 6056 -20 STRUCTURAL COMPONENT ONLY





City, Province, Postal Code: WATERDOWN

### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING\Dropped Beams\B20 DR(i4444) (Dropped Beam)

Passed

**BC CALC® Member Report** 

Dry | 1 span | No cant.

February 18, 2020 09:06:05

**Bulld 7239** 

Job name: Address:

File name:

MOUNTAINASH 4 EL. 2.mmdl

2ND FLR FRAMING\Dropped Beams\B20 DR(i4444) Description:

Specifier:

Designer:

Customer: Code reports:

CCMC 12472-R

Company:

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

COMPORMS TO OBC 2012

Calculations assume unbraced length of Top: 01-02-04, Bottom: 01-02-04.

AMENDED 2020

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Unbalanced snow loads determined from building geometry were used in selected product's

verification.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

PROVIDE 3 ROWS OF 312" ARDOX SPIRAL NAILS @ & "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN. 2"LUMBER EDGE/END DISTANCE, DO NOT USE AIR NAILS



DWG NO. TAM 6056 -20 STRUCTURAL COMPONENT ONLY

#### Disclosure

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### Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING\Flush Beams\B21(i4467) (Flush Beam)

PASSED

**BC CALC® Member Report** 

Dry | 2 spans | No cant.

February 18, 2020 09:06:05

**Build 7239** 

Job name: Address:

City, Province, Postal Code: WATERDOWN

Customer:

Code reports:

CCMC 12472-R

File name:

MOUNTAINASH 4 EL 2.mmdl

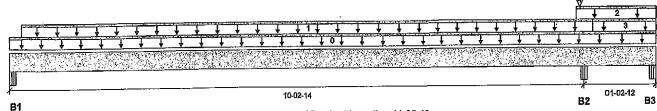
Description: 2ND FLR FRAMING\Flush Beams\B21(i4467)

AJ

Specifier:

Designer:

Company:



Total Horizontal Product Length = 11-05-10

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	
B1. 5-1/4"	105 / 0	103 / 0	0/0	
B2. 3-1/2"	620 / 0	650 / 0	206 / 0	
B3, 5-1/4"	38 / 350	0 / 293	50 / 0	

Lac	ad Cummons						Live	Dead	Snow	Wind	Tributary
Tag	ad Summary  Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L.	00-00-00	11-05-10	Тор		12			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-02-10	10-04-10	Тор	27	13			n\a
2	ROOF	Unf. Lin. (lb/ft)	L	10-01-02	11-05-10	Тор	33	30	63		n\a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	10-04-10	11-05-10	Top	15	7			n\a
4	E21(i1667)	Conc. Pt. (lbs)	L.	10-01-14	10-01-14	Тор	81	136	169		n\a
~	LZ I(11001)	Q01141 7 11 (1)				•					

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	517 ft-lbs	35392 ft-lbs	1.5%	44	04-02-04
Neg. Moment	-806 ft-lbs	-35392 ft-lbs	2.3%	1	10-02-14
End Shear	919 lbs	14464 lbs	6.4%	44	11-00-06
Cont. Shear	998 lbs	14464 lbs	6.9%	19	10-04-10
Total Load Deflection	L/999 (0.005")	n\a	n\a	107	04-08-00
Live Load Deflection	L/999 (0.003")	n\a	n\a	159	04-08-00
	L/999 (-0")	n\a	n\a	107	10-07-03
Total Neg. Defl. Max Defl.	0.005"	n\a	n\a	107	04-08-00
Snan / Denth	10.0				

Bearing	y Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1 B2 B3 B3	Beam Beam Beam Uplift	5-1/4" x 3-1/2" 3-1/2" x 3-1/2" 5-1/4" x 3-1/2"	287 lbs 1948 lbs 0 lbs 892 lbs	2.9% 13.0% n\a	1.3% 13.0% n\a	Unspecified VL 2.0 3100 SP Unspecified

Uplift of 892 lbs found at bearing B3. (SIMPSON 2-H2-54 @ 17. 33)



OWG NO. TAM6057 -20 STRUCTURAL COMPONENT ONLY





# Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

2ND FLR FRAMING\Flush Beams\B21(i4467) (Flush Beam)

**BC CALC® Member Report Build 7239** 

Job name: Address:

Customer:

Code reports:

File name:

February 18, 2020 09:06:05

Dry | 2 spans | No cant.

MOUNTAINASH 4 EL 2.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B21(i4467)

Specifier:

Designer:

Company:

Notes Design meets Code minimum (L/240) Total load deflection criteria.

CCMC 12472-R

CONFORMS TO OBC 2012

Design meets Code minimum (L/360) Live load deflection criteria.

City, Province, Postal Code: WATERDOWN

AMENDED 2020

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086. Unbalanced snow loads determined from building geometry were used in selected product's

verification.

Design based on Dry Service Condition. Importance Factor : Normal Part code : Part 9

PROVIDE 3 ROWS OF 311" ARDOX SPIRAL NAILS @/2 "0/C FOR MULTI-PLY NAILING, MAINTAIN A MIN.2" LUMBER EDGE/END DISTANCE. DO HOT USE AIR NAILS



ung no . Tan *6057* = 20 STRUCTURAL COMPONENT ONLY

#### Disclosure

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BC CALC®, BC FRAMER® , AJSTM, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





### Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING\Flush Beams\B22(i4454) (Flush Beam)

PASSED

**BC CALC® Member Report** 

Dry | 2 spans | No cant.

February 18, 2020 09:06:05

Build 7239 Job name:

Address:

City, Province, Postal Code: WATERDOWN

Customer: Code reports:

CCMC 12472-R

File name:

MOUNTAINASH 4 EL 2.mmdl

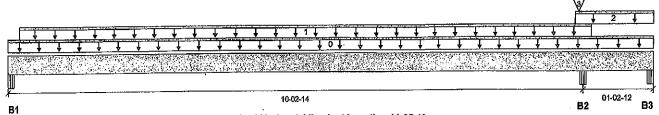
2ND FLR FRAMING\Flush Beams\B22(i4454) Description:

Wind

Specifier:

Designer:

ΑJ Company:



Total Horizontal Product Length = 11-05-10

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	
B1, 5-1/4"	211 / 0	156 / 0	0/0	
B2, 3-1/2"	1088 / 0	876 / 0	118/0	
B3. 5-1/4"	27 / 699	0 / 474	50 / 0	

10	ad Summary				;		Live	Dead	Snow	Wind	Tributary
Tag	_	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	11-05-10	Тор		12			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-02-10	10-04-10	Тор	53	27			n\a
2	ROOF	Unf. Lin. (lb/ft)	L	10-01-02	11-05-10	Тор	33	30	63		nla
3	E19(i1665)	Conc. Pt. (lbs)	Ĺ	10-01-14	10-01-14	Top	39	107	81		n\a

Controls Summary	Factored Demand	Factored Resistance	Demand <i>l</i> Resistance	Case	Location
Pos. Moment	926 ft-lbs	35392 ft-lbs	2.6%	44	04-02-04
Neg. Moment	-1440 ft-lbs	-35392 ft-lbs	4.1%	1	10-02-14
End Shear	1664 lbs	14464 lbs	11.5%	44	11-00-06
Cont. Shear	1731 lbs	14464 lbs	12.0%	1	10-04-10
Total Load Deflection	L/999 (0,009")	n\a	n\a	107	04-08-00
Live Load Deflection	L/999 (0.005")	n\a	n\a	159	04-08-00
Total Neg. Defl.	L/999 (-0")	n\a	n\a	107	10-07-03
Max Defl.	0.009"	n\a	n\a	107	04-08-00
Span / Depth	10.0				

Bearing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Wember	Material
B1 Beam B2 Beam B3 Beam B3 Uplift	5-1/4" x 3-1/2" 3-1/2" x 3-1/2" 5-1/4" x 3-1/2"	511 lbs 2845 lbs 0 lbs 1641 lbs	5.2% 19.0% n\a	2.3% 19.0% nla	Unspecified VL 2.0 3100 SP Unspecified

<u>Cauti</u>ons

Uplift of 1641 lbs found at bearing B3. (SIMPSON 2-12-54 + 4-3/2" AMBOX SIMAL TOE-NAILS @ 157- 83).



ova no . yan*6056-2*0 STRUCTURAL COMPONENT ONLY





City, Province, Postal Code: WATERDOWN

# Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING\Flush Beams\B22(I4464) (Flush Beam)

PASSED

February 18, 2020 09:06:05

**BC CALC® Member Report** 

Build 7239

Job name: Address: Dry | 2 spans | No cant.

File name: M

MOUNTAINASH 4 EL 2.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B22(i4454)

Specifier:

Designer:

esigner:

Customer: Code reports:

CCMC 12472-R

Company:

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBC 2012

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Unbalanced snow loads determined from building geometry were used in selected product's

verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

2" \$ \\ \frac{1}{12"CTYP)

PROVIDE 3 ROWS OF 3½" ARDOX
SPIRAL NAILS @ /2—" O/C FOR
MULTI-PLY HAILING, MAINTAIN
A MIN.2"LUMBER EDGE/END
DISTANCE.DONUTUSE AIR NAILS



awa no.tam*6058*-26 Structural Component Only

#### Disclosure

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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



**BC CALC® Member Report** 



# Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

2ND FLR FRAMING\Flush Beams\B22(i4484) (Flush Beam)

Dry | 2 spans | No cant.

February 18, 2020 09:19:26

Build 7239

Job name: Address:

City, Province, Postal Code: WATERDOWN

Customer:

Code reports:

CCMC 12472-R

File name:

MOUNTAINASH 4 EL 3 OPT.mmdl

2ND FLR FRAMING\Flush Beams\B22(i4484) Description:

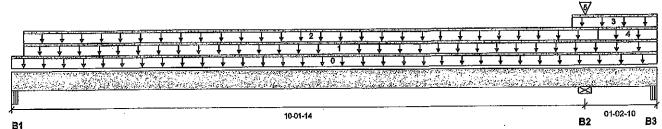
Specifier:

Designer:

ΑJ

Wind

Company:



Total Horizontal Product Length = 11-04-08

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow
B1, 5-1/4"	105 / 0	102 / 0	0/0
B2, 5-1/2"	504/0	494 / Ö	46/0
B3, 4-1/8"	34 / 311	0 / 260	46 / 0

Los	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag		Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	11-04-08	Тор		12			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-02-10	11-04-08	Top	7	4			n\a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-02-10	10-04-10	Тор	20	10			n\a
3	ROOF	Unf. Lin. (lb/ft)	L	09-11-02	11-04-08	Тор	33	30	63		n\a
4	FC2 Floor Material	Unf. Lin. (lb/ft)	L	10-04-10	11-04-08	Тор	6	3			n\a
5	E20(i1666)	Conc. Pt. (lbs)	L	10-01-14	10-01-14	Тор		14			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	511 ft-lbs	35392 ft-lbs	1.4%	44	04-01-08
Neg, Moment	-788 ft-lbs	-35392 ft-lbs	2,2%	1	10-01-14
End Shear	813 lbs	14464 lbs	5.6%	44	11-00-06
Cont. Shear	886 lbs	14464 lbs	6.1%	1	10-04-10
Total Load Deflection	L/999 (0.005")	n\a	n\a	107	04-07-02
Live Load Deflection	L/999 (0.003")	n\a	n\a	159	04-07-02
Total Neg. Defl.	L/999 (-0")	n\a	n\a	107	10-06-09
Max Defl.	0.005"	nla	n\a	107	04-07-02
Span / Depth	9.9				

Bearii	ng Supports	Dlm. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Beam	5-1/4" x 3-1/2"	285 lbs	2.9%	1.3%	Unspecified
B2	Wall/Plate	5-1/2" x 3-1/2"	1420 lbs	12.0%	6.0%	Spruce-Plne-Fir
B3	Beam	4-1/8" x 3-1/2"	0 lbs	n\a	n\a	Unspecified
B3	Uplift		792 lbs			

Cautions

Uplift of 792 lbs found at bearing B3. (SIMPSON



COMPONENT ONLY





### Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING\Flush Beams\B22(14484) (Flush Beam)

Passed

February 18, 2020 09:19:26

**BC CALC® Member Report** 

**Build 7239** 

Job name:

Address:

**Notes** 

City, Province, Postal Code: WATERDOWN

Design meets Code minimum (L/240) Total load deflection criteria.

Customer:

Code reports:

**CCMC 12472-R** 

Dry | 2 spans | No cant.

File name:

MOUNTAINASH 4 EL 3 OPT.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B22(i4484)

Specifier:

Designer: ΑJ

Company:

CONPORAS TO UBO 2012

Design meets Code minimum (L/360) Live load deflection criteria. Calculations assume member is fully braced.

AMENDED 2020

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 088.

Unbalanced snow loads determined from building geometry were used in selected product's

verification.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

PROVIDE 3 ROWS OF 3½" ARDOX SPIRAL NAILS @ /2-"O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN. 2"LUMBER EDGE/END DISTANCE, BOHOT USE AIR NAILS



146 NO. TAN 6059 -20 STRUCTURAL COMPONENT ONLY

#### Disclosure

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BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAMIM, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





## Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

April 16, 2020 07:54:23

1ST FLR FRAMING\Flush Beams\B1A(i4890) (Flush Beam)

**BC CALC® Member Report** 

**Build 7239** Job name:

Address:

City, Province, Postal Code: WATERDOWN

**Customer:** Code reports:

CCMC 12472-R

Dry | 1 span | No cant.

File name:

MOUNTAINASH 4 EL 1 DECK CONDITION, mmdl

1ST FLR FRAMING\Flush Beams\B1A(i4890)

Description: Specifier:

Designer:

Company:

	₩		₹	
		++++++1	+ + + + + + + + + + + + + + + + + + +	<b>,</b>
⊴				D
		03-01-	00	

Total Horizontal Product Length = 03-01-00

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	LÌve	Dead
B1, 3"	818 / 0	552/0
B2. 3"	689 / 0	487 / 0

١٨	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-01-00	Тор		12	;		00-00-00
1	E1(1429)	Unf. Lin. (lb/ft)	L	00-00-00	03-01-00	Top	262	212			n\a
,	J3(i4853)	Conc. Pt. (lbs)	L	00-07-08	00-07-08	Top	349	174			n\a
3	J3(i4853)	Conc. Pt. (lbs)	L	01-11-08	01-11-08	Top	349	174		FSSIO	nla
3	J3(14893)	Conc. Ft. (iba)	-	01-11-00	01-11-00	100	Ģ-10	11-1	OF COMPANY	ESSIO <sub>A</sub>	Land On

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1140 ft-lbs	35392 ft-lbs	3.2%	1	01-08-12.
End Shear	722 lbs	14464 lbs	5.0%	1	01-10-02
Total Load Deflection	L/999 (0.001")	n\a	n <b>\a</b>	4	01-06-08
Live Load Deflection	L/999 (0.001")	n\a	n\a	5	01-06-08
Max Defl.	0.001"	n\a	n\a	4	01-06-08
Span / Depth	2.7				

Bearing	Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material_	
B1	Wall/Plate	3" x 3-1/2"	1917 lbs	29.7%	15.0%	Spruce-Pine-Fir	
B2	Wall/Plate	3" x 3-1/2"	1643 lbs	25.4%	12.8%	Spruce-Pine-Fir	

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live toad deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

PROVIDE3 ROWS OF 31/2" ARDOX SPIRAL NAILS @ 6 "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN. 2"LUMBER EDGE/END DISTANCE, DO NOT USE AIR NAILS

conforms to obc 2012



1 NG NO. TAN 6060 - 20 STRUCTURAL COMPONENT ONLY

#### Disclosure

Use of the Bolse Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. installation of Bolse Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAMIM, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





#### Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

1ST FLR FRAMING\Flush Beams\B1B(i4889) (Flush Beam)

PASSED

April 16, 2020 07:54:23

**BC CALC® Member Report** 

**Build 7239** 

Job name:

Address:

City, Province, Postal Code: WATERDOWN

Customer: Code reports:

CCMC 12472-R

Dry | 1 span | No cant.

File name:

MOUNTAINASH 4 EL 1 DECK CONDITION.mmdl

1ST FLR FRAMING\Flush Beams\B1B(i4889) Description:

Specifier:

Designer: ΑJ

Wind

Company:

			+	
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	7 7 7 7 7 1 T	1 121 1 1 1	<b> </b>	<del>↑                                    </del>
		4 4 4 4 4 4	_	<b>+</b> + +
	<del>                                     </del>	1 101 1 1 1		<b>1 1 1</b>
	Laster Control			1 / 1 / 1 / 1 / 1 / 1 / 1 / 1 / 1 / 1 /
		03-01-00		В

**B**1

Total Horizontal Product Length = 03-01-00

Reaction Summary (Down / Unlift) (lbs)

IZGAVUOII QU	Illitary (Down / D	ואסון (וייטן	
Bearing	Live	Dead	Snow
B1, 3"	79 / 0	192 / 0	
B2. 3"	79 / 0	192 / 0	

Load		d Summary						Live	Dead	Snow	Wind	Tributary
		Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
-	0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-01-00	Top		12			00-00,00
	1	E5(1427)	Unf. Lin. (lb/ft)	 L	00-00-00	03-01-00	Top	25	99			n\a
	2	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	03-01-00	Top	27	13			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	160 ft-lbs	23005 ft-lbs	0.7%	0	01-06-08
End Shear	53 lbs	9401 lbs	0.6%	0	01-02-14
Total Load Deflection	L/999 (0")	n\a	n\a	4	01-06-08
Live Load Deflection	L/999 (0")	n\a	n\a	5	01-06-08
Max Defl.	0"	n\a	n\a	4	01-06-08
Span / Depth	2.7				

Bearir	ng Supports	Dim. (LxW)	Demand.	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	3" x 3-1/2"	268 lbs	6.4%	3.2%	Spruce-Pine-Fir
B2	Mall/Plate	3" x 3-1/2"	268 lbs	6.4%	3.2%	Spruce-Pine-Fir

#### Notes

A Commence of the

Design meets Code minimum (L/240) Total load deflection criteria.

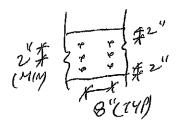
Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



PROVIDE 3 ROWS OF 312" ARDOX SPIRAL NAILS @8 "O/C FOR MULTI-PLY NAILING, MAINTAIN A MIN. 2 "LUMBER EDGE/END DISTANCE DO NOT USE AIR BALLS

CONFORMS TO OBC 2012



DVS MO. FAN 6061 =20 STRUCTURAL COMPONENT ONLY

#### Disclosure

Use of the Bolse Cascade Software Is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAMTM, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



Live Load = 40 þsf. Þead Load = 30 psf Simþle Spans, L/480 Þeflection Limit 3/4" OSB G&N Sheathing







			B	are		1	1/2" Gyp:	sum Ceiling	
Depth	Series	-	On Centi	e Spacing		T	On Cent	re Spacing	
•		12"	16"	19.2"	24"	12"	16"	19.2"	24 <sup>h</sup>
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-0"	16'-0"	15'-1"	13'-11"	17'-5"	16'-1"	15'-1"	13'-11"
9-1/2"	NI-60	17'-2"	16'-2"	15'-5"	14'-3"	17'-6"	16'-5"	15'-5"	14'-3"
•	NI-70	18'-0"	16'-11"	16'-3"	15'-6"	18'-5"	17'-3"	16'-7"	15'-6"
9-1/2" 11-7/8"	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	15'-10'
	NI-20	17'-10"	16'-10"	16'-0"	14'-10"	18'-6"	17'-1"	16'-0"	14'-10'
11-7/8"	NI-40x	19'-4"	17'-11"	17'-3"	15'-10"	19'-11"	18'-6"	17'-9"	15'-10"
	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-1"
	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10'
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	14-2" 13-4" 16-1" 15-1" 16-5" 15-5" 17-3" 16-7" 17-5" 16-9" 17-1" 16-0" 18-6" 17-9" 18-9" 17-11" 19-9" 18-10"	18'-6"	
	N1-40x	21'-5"	19'-10"	18'-11"	17'-5"	22'-1"	201-6"	19'-6"	17'-5"
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
14"	Nt-70	23'-0"	21'-3"	20'-3"	19'-2"	231-8"	21'-11"	20'-10"	19'-9'
	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'- <del>9</del> "	20'-7"
	NI-50	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22"-9"	21'-8"	20'-6"
	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	221-9"	21'-6"
16"	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	N1-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

			Mld-Spa	n Blocking		Mid-S	pan Blocking an	id 1/2" Gγpsum	Celling
11-7/8"	Serles		On Cent	re Spacing		1	On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	Nt-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-9"	16'-1"	15'-1"	13'-11"	17'-9"	16'-1"	15'-1"	13'-11'
9-1/2"	NI-60	18'-1"	16'-5"	15'-5"	14'-3"	18'-1"	16'-5"	15'-5"	14'-3"
	NI-70	19'-10"	17'-11"	16'-9"	15'-6"	19'-10"	17'-11"	16'-9"	15'-6"
	NI-80	20'-2"	18'-3"	17'-1"	15'-10"	20'-2"	18'-3"	17'-1"	15'-10'
	NI-20	18'-10"	17'-1"	16'-0"	14'-10"	18'-10"	17'-1"	16'-0"	14'-10'
11-7/8"	NI-40x	21'-3"	19'-3"	17'-9"	15'-10"	21'-3"	19'-3"	17'-9"	15'-10'
	NI-60	21'-9"	19*-8"	18'-5"	17'-1"	21'-9"	19'-8"	18'-5"	17'-1"
	NI-70	23'-4"	21'-5"	20'-1"	18'-6"	23'-8"	21'-5"	20'-1"	18'-6"
	พ80	23'-7"	21'-10"	20'-5"	18'-11"	24'-1"	21'-10"	20'-5"	18'-11'
	NI-90x	24'-3"	22'-6"	21'-3"	19'-7"	24'-8"	22'-7" -	21'-3"	19'-7"
	NI-40x	24'-2"	21'-5"	19'-6"	17'-5"	24'-2"	21'-5"	19'-6"	17'-5"
	NI-60	24'-9"	22'-5"	21'-0"	19'-6"	24'-9"	22'-5"	21'-0"	19'-6"
14"	NI-70	26'-1"	24'-3"	22'-9"	21'-0"	26'-8"	24'-3"	22'-9"	21'-0"
	NI-80	26'-6"	24'-7"	23'-3"	21'-6"	27'-1"	24'-10"	23'-3"	21'-6"
	NI-90x	27'-3"	25'-4"	24'-1"	22'-4"	27'-9"	25'-10"	24'-3"	22¹-4"
	NI-60	27'-3"	24'-11"	23'-5"	21'-7"	^27'-6 <sup>a</sup>	24'-11"	23'-5"	21'-7"
	NI-70	28'-8"	26'-8"	25'-3"	23'-4"	29'-3"	26'-11"	25'-3"	23'-4"
16"	NI-80	29'-1"	27'-0"	25'-9"	23'-10"	29'-8"	27'-6"	25'-10"	23'-10'
	NI-90x	29'-11"	27'-10"	26'-6"	24'-10"	30'-6"	28'-5"	26'-11"	24'-10"

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum celling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum celling attached to joists.

3. Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA 086-09, NBC 2010, and OBC 2012.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-Joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 5/8" OSB GRN Sheathing







			B	are		1.	1/2" Gyp:	sum Ceiling	
Depth	Series		On Centi	re Spacing			On Cent	re Spacing	
•		12"	16"	19.2"	24"	12"	16"		24"
	NI-20	15'-1"	14'-2"	13'-9"	N/A	15'-7"	14'-8"	14'-2"	N/A
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
11-7/8°	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	N1-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A
14"	NI-70	21'-7"	20'-0"	19'-1"	. N/A	22'-3"	20'-7"	19'-8"	N/A
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	Nt-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
16"	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21 9"	N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

			Mld-Spai	n Blocking		Mid-S	pan Blocking a	nd 1/2" Gypsum	Ceiling	
	Series		On Centr	e Spacing		On Centre Spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	16'-8"	15'-3"	14'-5"	N/A	16'-8"	15'-3"	14'-S"	N/A	
	NI-40x	17'-11"	16'-11"	16'-1"	N/A	18'-5"	17'-1"	16'-1"	N/A	
9-1/2"	NI-60	18'-2"	17'-1"	16'-4"	N/A	18'-7"	17'-4"	16'-4"	N/A	
• · ·	NI-70	19¹-2"	17'-10"	17'-2"	N/A	19'-7"	18'-3"	17'-7"	N/A	
	NI-80	19'-5"	18'-0"	17'-4"	N/A	19'-10"	18'-5"	17'-8"	N/A	
	NI-20	19'-6"	18'-1"	17'-3"	N/A	19'-11"	18'-3"	17'-3"	N/A	
11-7/8"	NI-40x	21'-0"	19'-6"	18'-8"	N/A	21'-7"	20'-2"	19'-2"	N/A	
	NI-60	21'-4"	19'-9"	18'-11"	N/A	21'-11"	20'-4"	19'-6"	N/A	
	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-5"	20'-5"	N/A	
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20′-8™	N/A	
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A	
	NI-40x	23'-7"	21'-11"	20'-11"	N/A	24'-3"	22'-7"	21'-7"	N/A	
	NI-60	24'-0"	22'-3"	21'-3"	N/A	24'-8"	22'-11"	21'-11"	N/A	
14"	Nt-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-11"	N/A	
	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A	
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A	
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	25'-3"	24'-2"	N/A	
	N1-70	27'- <del>9</del> "	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A	
16"	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A	
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26'-2"	N/A	

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum celling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum celling attached to joists.

<sup>3.</sup> Minimum bearing length shall be 1-3/4 inches for the end bearings.

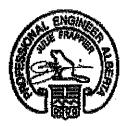
<sup>4.</sup> Bearing stiffeners are not required when i-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf. Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			Ba	ire		l	1/2" Gyps	um Celling	
Depth	Series		On Centr	e Spacing			On Centr	e Spacing	
o cpa i	••••	12"	16°	19.2"	24"	12"	16"	19.2"	24"
	NJ-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"
	N1-40x	17'-0"	16'-0"	15'-5"	14'-9"	17'-5"	16'-5"	15'-10"	15'-2"
9-1/2°	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-6"	16'-7"	15'-11"	15'-3"
,-	N1-70	18'-0"	16'-11"	16'-3"	15'-7"	18'-5"	17'-3"	16'-7"	15'-11"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
	NI-20	17'-10"	16'-10"	16'-2"	15'-6"	18'-6"	17'-4"	16'-9"	16'-1"
	NI-40x	19¹-4°	17'-11"	17'-3"	16'-6"	19'-11"	18'-6"	17'-9"	17'-0"
11-7/8"	NI-60	19'-7"	18'-2"	17'-5"	16'- <del>9</del> "	20'-2"	18'-9"	17'-11"	17'-2"
	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
	N1-40x	21'-5"	19'-10"	18'-11"	17'-11*	22'-1"	20'-6"	19'-7"	18'-7"
	N1-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
14"	NI-70	23'-0"	21'-3"	201-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"
24	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90x	24'-1"	221-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"
	NI-60	23'-9"	22'-0"	201-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22'-9"	21'-6"
16"	N(-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

			Mid-Spar	n Blocking		Mid-s	pan Blocking an	d 1/2" Gypsum	Ceiling
14 <b>"</b>	Series		On Centr	e Spacing		On Centre Spacing			
ocpu.	Denias	12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
	Nt-40x	18'-8"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"
9.1/2"	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"
J-1/2	NI-70	20'-0"	18'-7"	17'-9"	16'-7"	20'-5"	18'-11"	17'-10"	16'-7"
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10'
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"
	NI-40x	21'-10"	20'-4"	19'-4"	17'-8"	22'-5"	20'-6"	19'-4"	17'-8"
11-7/8"	NI-60	22'-1"	20'-7"	19'-7"	18 <sup>7</sup> -4"	22'-8"	20'-10"	19'-8"	18'-4"
	NI-70	23'-4"	21'-8"	20'-8"	19'-7"	23'-10"	22'-3"	21'-2"	19'-9"
	NI-80	23'-7"	21'-11"	20'-11"	19'-9"	24'-1"	22'-6"	21'-5"	20'-0"
	NI-90x	24'-3"	22'-6"	21'-6"	20'-4"	24'-8"	23'-0"	22'-0"	20'-9"
	NI-30x	24'-5"	22'-9"	21'-8"	19'-5"	25'-1"	23'-2"	21'-9"	19'-5"
	NI-60	24'-10"	23'-1"	22'-0"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10'
1.49	NI-70	26'-1"	24'-3"	23'-2"	21'-10"	26'-8"	24'-11"	23'-9"	22'-4"
14	NI-80	26'-6"	24'-7"	23'-5"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"
	NI-90x	. 27'-3"	25'-4"	24'-1"	22'-9"	27'-9"	25'-11"	24'-8"	23'-4"
	NI-60	27'-3"	25'-5"	24'-2"	22'-10"	28'-0"	26'-2"	24.9"	23'-1"
	NI-70	28'-8"	26'-8"	25'-4"	23'-11"	29'-3"	27'-4"	26'-1"	24'-8"
16"	NI-70	29'-1"	27'-0"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"
	NI-90x	29'-11"	27'-10"	26'-6"	25'-0"	30'-6"	28'-5"	27'-2"	25'-8"

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

3. Minimum bearing length shall be 1-3/4 inches for the end bearings.

4. Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

5. This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Unit States Design per CSA 086-09, NBC 2010, and OBC 2012.

6. Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.

<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum celling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum celling attached to joists.



Live Load = 40 psf, Pead Load = 30 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







Depth	Series	Bare On Centre Spacing				1/2" Gypsum Celling On Centre Spacing			
		9-1/2"	NI-20	15'-1"	14'-1"	13'-3"	N/A	15'-7"	14'-1"
NI-40x	16'-1"		15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A
NI-60	16'-3"		15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A
Nt-70	17'-1"		16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
NI-80	17'-3"		16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
11-7/8"	NJ-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	N1-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
	N1-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
	N1-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
14"	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A
	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	201-011	N/A
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
16" .	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

Depth	Series	Mid-Span Biocking On Centre Spacing				Mid-Span Blocking and 1/2" Gypsum Celling On Centre Spacing			
		9-1/2"	NI-20	15'-7"	14'-1"	13'-3"	N/A	15'-7"	14'-1"
NI-40x	17'-9"		16'-1"	15'-1"	N/A	17'-9"	16'-1"	15'-1"	N/A
NI-60	18'-1"		16'-4"	15'-4"	N/A	18'-1"	16'-4"	15'-4"	N/A
NI-70	19'-2"		17'-10"	16'-9"	N/A	19'-7"	17'-10"	16'-9"	N/A
NI-80	19'-5"		18'-0"	17'-1"	N/A	19'-10"	18'-3"	17'-1"	N/A
11-7/8"	Nt-20	18'-9"	17'-0"	16'-0"	N/A	18'-9"	17'-0"	16'-0"	N/A
	NI-40x	21'-0"	19'-3"	17'-9"	N/A	21'-3"	19'-3"	17'-9"	N/A
	NI-60	21'-4"	19'-8"	18'-5"	N/A	21'-8"	19'-8"	18'-5"	N/A
	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-4"	20'-0"	N/A
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-5"	N/A
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A
	NI-40x	23'-7"	21'-5"	19'-6"	N/A	24'-1"	21'-5"	19'-6"	N/A
14"	NI-60	24'-0"	22'-3"	21'-0"	N/A	24'-8"	22'-5"	21'-0"	N/A
	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-9"	N/A
	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A
16"	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	24'-10"	23'-4"	N/A
	N1-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A
	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A
	NI-90x	29'-0".	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26'-2"	N/A

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

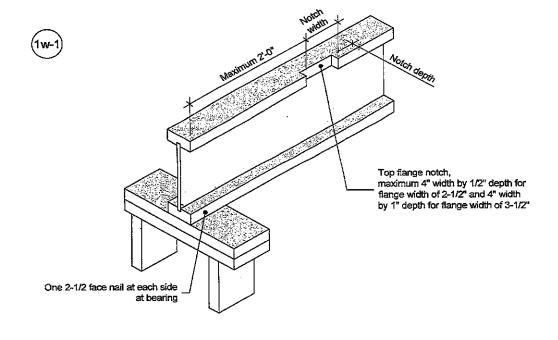
<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attacked to joists.

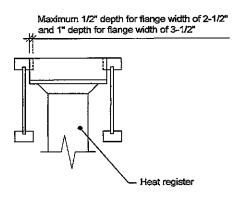
<sup>3.</sup> Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when t-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA 086-09, NBC 2010, and OBC 2012.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.





DOCUMENT

- Blocking required at bearing for lateral support, not shown for clarity.
- 2. The maximum dimensions for a notch on the side of the top flange are 4-inch width by 1/2-inch depth for flange width of 2-1/2 inches, and 4-inch width by 1-inch depth for flange width of 3-1/2 inches.
- 3. This detail applies to simple-span joists and multiple-span joists where the notch is located at the end half-span.
- 4. For other applications, contact Nordic Structures.

This document supersedes all previous versions. If the document has been in effect for more than one year, consult nordic.ca or contact Nordic Structures. All nails shown in the details are assumed to be common nails unless otherwise noted. Nails shall have a diameter not less than 0.128 inch for 2-1/2-inch nails, or 0.144 inch for 3-inch nails, Individual components not shown to scale for clarity.



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Notch in I-joist for Heat Register DATE NUMBER I-joist - Typical Floor Framing and Construction Details 2018-04-10 1w-1

# **Construction Detail**



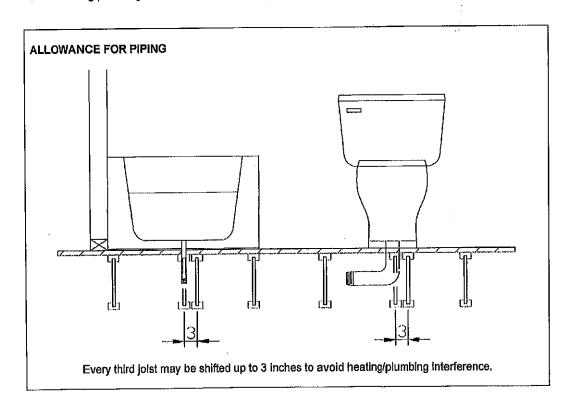
Limit States Design

# Allowance for Piping (Installation Notes)

The floor layouts have usually not been checked for heating and/or plumbing interference. On-site adjustment of joists of up to 3 inches is permitted to avoid interferences. When moving a joist, the subfloor thickness shall be checked with code requirements when the joist spacing exceeds 19.2 inches. Except for cutting to length, I-joist flanges should never be cut, drilled, or notched.

Installation of Nordic I-joists shall be as per *Nordic Joist Installation Guide for Residential Floors*. Refer to Tables 1 and 2 for maximum web hole and duct chase openings, respectively. These tables are based on the I-joists being used at their maximum spans. The minimum distance given may be reduced for shorter spans; contact your distributor for additional information.

The detail below shows the 3-inch allowance for piping. Every third joist may be shifted up to 3 inches to avoid heating/plumbing interference. For other applications, please contact your distributor.



Revised April 12, 2012