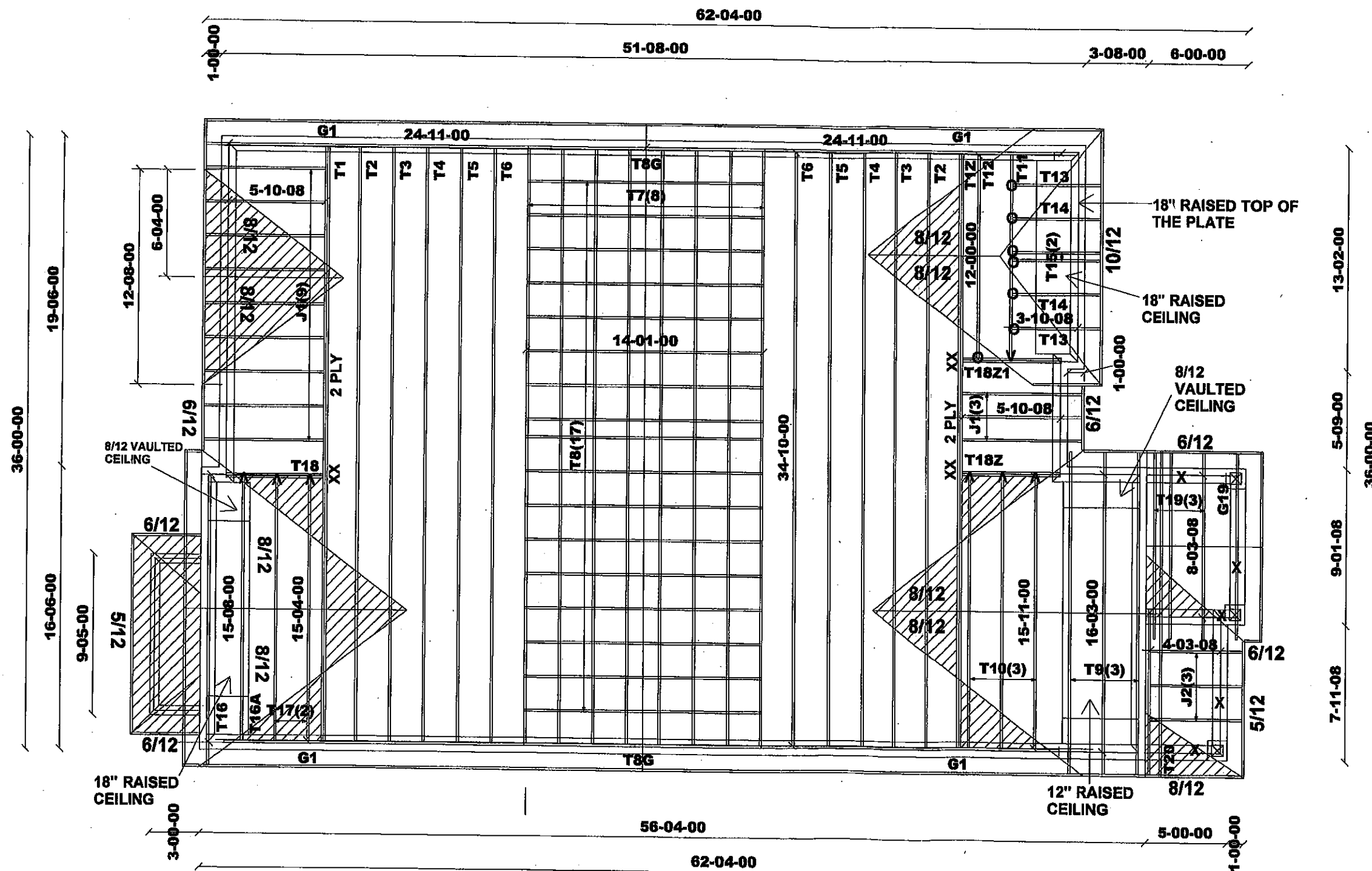


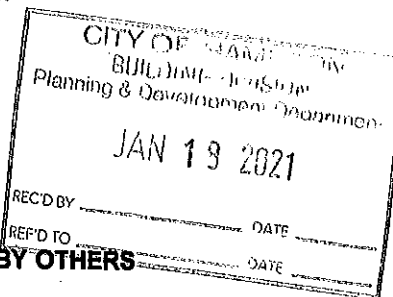


ASPHALT SHINGLES
 FINISHED OVERHANG: 12"
 2x6 EXTERIOR WALLS
 2x6 FASCIA BOARD
 HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
 OCCUPANCY: RESIDENTIAL | PART: 9
 (2019 AMENDMENT)
 Ss = 31.3 psf | Sr = 8.4 psf

DESIGN LOADS:
 TCSL = 25.6 psf
 TCDL = 6.0 psf
 BCLL = 0.0 psf
 BCDL = 7.4 psf

HARDWARE:
 LUS24 - (O)
 LJS26DS - (V)
 HGUS26-2 - (XX)



DENOTES:

X : DROPPED BEAM BY OTHERS

DENOTES: CONVENTIONAL FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

M13613



Job Track: 51225
 Plan Log: 202419
 Layout ID: 414964

Builder / Location:
GREEN PARK HOMES / WATERDOWN
 Project: **RUSSELL GARDENS PH.3**
 Date: 2020-12-02 Sales: Mario DiCano Designer: ND

Model / Elevation:
MOUNTAINASH 6 - 009/2

Mitek ver 8.3.3.247

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 6 - 009
 Lot #:
 Elevation: 2

Job Track: 51225
 PlanLog: 202419
 Layout ID: 414964
 Ref #
 Page: 1 of 3
 Date: 12-03-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Flat Girder	0/12	34-10-00	4-01-04	2 x 6		4-01-04 4-01-04	364.56 222.67		
	1 2-ply	T1Z Flat Girder	0/12	34-10-00	4-01-04	2 x 6		4-01-04 4-01-04	364.56 222.67		
	4	G1 GABLE	6/12	24-11-00	10-01-04	2 x 4	1-03-08	1-02-00 10-01-04	537.25 346.00		
	2	T2 Flat	0/12	34-10-00	5-01-04	2 x 4		5-01-04 5-01-04	286.17 179.67		
	2	T3 Flat	0/12	34-10-00	6-01-04	2 x 4		6-01-04 6-01-04	305.44 187.33		
	2	T4 Flat	0/12	34-10-00	7-01-04	2 x 4		7-01-04 7-01-04	325.33 201.00		
	2	T5 Flat	0/12	34-10-00	8-01-04	2 x 4		8-01-04 8-01-04	345.71 214.67		
	2	T6 Flat	0/12	34-10-00	9-01-04	2 x 4		9-01-04 9-01-04	406.18 250.33		
	8	T7 Flat	0/12	34-10-00	10-01-04	2 x 6 2 x 4		10-01-04 10-01-04	1887.43 1133.33		
	17	T8 Common	6/12	14-01-00	3-06-04	2 x 4			680.95 433.50		
	2	T8G GABLE	6/12	14-01-00	3-06-04	2 x 4			75.96 47.00		
	3	T9 Roof Special	8/12 8/12	16-03-00	6-09-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	201.39 133.00		
	3	T10 Common	8/12	16-01-00	6-09-13	2 x 4		1-06-02 1-04-13	206.48 132.50		
	1	T11 Hip Girder	8/12	12-02-00	6-04-07	2 x 4 2 x 6		3-00-03 2-10-14	66.45 43.67		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 6 - 009
 Lot #:
 Elevation: 2

Job Track: 51225
 PlanLog: 202419
 Layout ID: 414964
 Ref #
 Page: 2 of 3
 Date: 12-03-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T12 Common	8 /12	12-02-00	7-00-03	2 x 4		3-00-03 2-10-14	58.87 38.83		
	2	T13 Half Hip	10 /12	3-10-08	4-01-14	2 x 4	1-03-08	1-07-11 4-01-14	57.38 41.00		
	2	T14 Half Hip	10 /12	3-10-08	5-05-14	2 x 4	1-03-08	1-07-11 5-05-14	63.76 44.33		
	2	T15 Monopitch	10 /12	3-10-08	6-04-07	2 x 4	1-03-08	1-07-11 6-04-07	61.98 41.67		
	1	T16 Roof Special	8 /12 8 /12	15-08-00	6-07-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	65.19 43.83		
	1	T16A Roof Special	8 /12 8 /12	15-06-00	6-07-08	2 x 4		1-08-08 1-04-13	61.11 41.17		
	2	T17 Common	8 /12	15-06-00	6-07-08	2 x 4		1-06-02 1-04-13	125.78 80.00		
	1 2-ply	T18 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T18Z Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T18Z1 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	3	T19 Common	6 /12	8-03-08	3-00-09	2 x 4	1-03-08 1-03-08	5-07 5-07	79.61 53.00		
	1	G19 GABLE	6 /12	8-03-08	3-00-09	2 x 4	1-03-08 1-03-08	5-07 5-07	25.21 16.33		
	1	T20 Half Hip Girder	8 /12	8-05-00	2-07-12	2 x 4	1-03-08	8-05 2-01-08	32.93 23.00		
	12	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	201.63 128.00		

 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: RUSSELL GARDENS PH.3 Location: WATERDOWN Model: MOUNTAINASH 6 - 009 Lot #: Elevation: 2	Job Track: 51225 PlanLog: 202419 Layout ID: 414964 Ref # Page: 3 of 3 Date: 12-03-2020 Designer: Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	3	J2 Jack-Open	5 /12	4-03-08	2-07-11	2 x 4	1-03-08	4-01 2-01-08	35.79 24.00		

TOTAL # TRUSS= 89 TOTAL BFT OF ALL TRUSSES= 4433.5 BFT. TOTAL WEIGHT OF ALL TRSSES 7095.94 LBS

HARDWARE

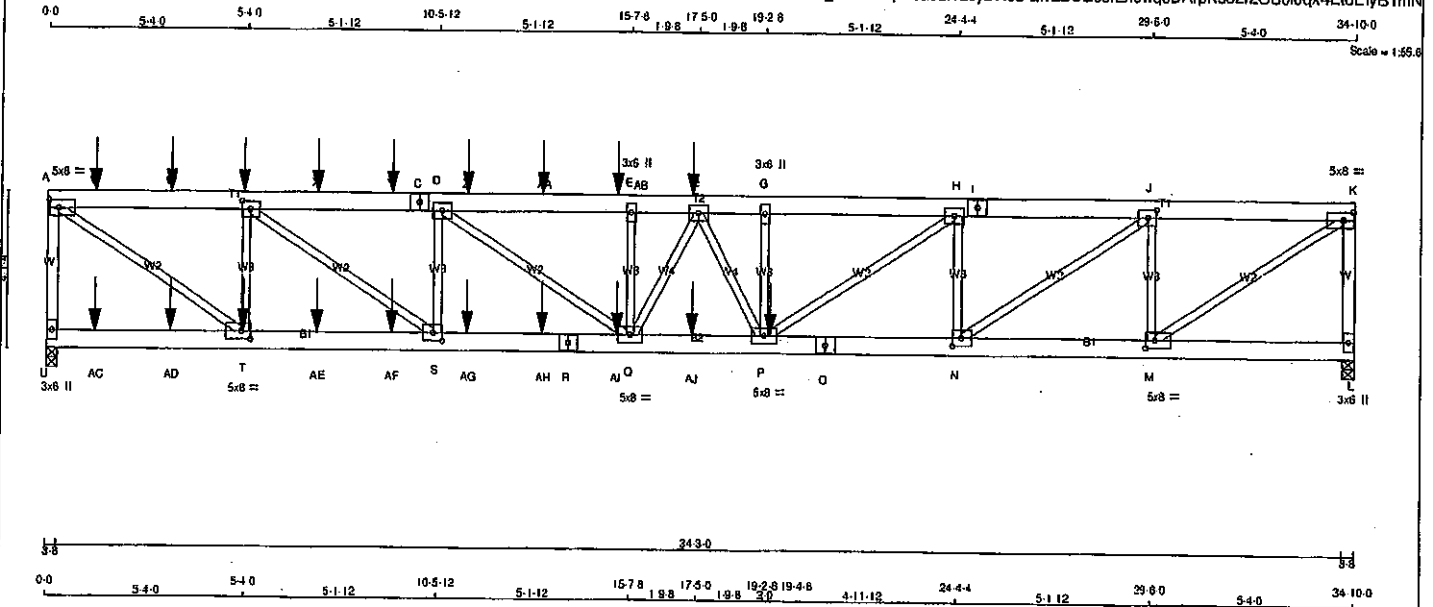
QTY	TYPE	MODEL	LENGTH
7	Hardware	LUS24	
7	Hardware	LJS26DS	
3	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 17

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
414964	T1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MTek Industries, Inc. Tue Dec 8 12:44:06 2020 Page 1
ID:76L61_X31PWpZ5adLTzJyDN6u-awLD6GJ9rLn0wqoDArpK552fZO56loqx4L6ElyB1mN



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
U - A	2x4 DRY	No.2	SPF
A - C	2x6 DRY	No.2	SPF
C - I	2x6 DRY	No.2	SPF
I - K	2x6 DRY	No.2	SPF
L - K	2x4 DRY	No.2	SPF
U - R	2x6 DRY	2100F 1.8E	SPF
R - O	2x6 DRY	2100F 1.8E	SPF
O - L	2x6 DRY	2100F 1.8E	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
U - A	12	TOP
K - L	12	TOP
A - C	12	SIDE(183.1)
C - I	12	SIDE(183.1)
I - K	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
U - R	12	SIDE(197.8)
R - O	12	SIDE(197.8)
O - L	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
P - G	6	SIDE(215.0)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
		GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
U	3533	0	3533	0	3-8
L	3121	0	3121	0	3-8

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2499	1836	0	0	0	883	0
L	2208	1456	0	0	0	750	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
U - A	-3451	P - G	-259
A - V	-4592	Q - E	-533
V - W	-4592	Q - F	-539
W - B	-4592	F - P	0
B - X	-7572	D - Q	0
X - Y	-7572	A - T	0
Y - C	-7572	T - B	-1948
C - D	-7572	S - D	-2955
D - Z	-9238	B - S	0
Z - AA	-9238	M - K	0
AA - AB	-9238	P - H	0
AB - E	-9238	M - J	-2765
E - F	-9238	N - J	-2106
F - G	-9602		
G - H	-9602		
H - I	-7339		
I - J	-7339		
J - K	-4181		
L - K	-3066		

U-AC	0	0	-18.5	-18.5	0.03	(4)	10.00
AC-AD	0	0	-18.5	-18.5	0.03	(4)	10.00
AD-T	0	0	-18.5	-18.5	0.03	(4)	10.00
T-AE	0	4592	-18.5	-18.5	0.15	(1)	10.00
AE-AF	0	4592	-18.5	-18.5	0.15	(1)	10.00
AF-S	0	4592	-18.5	-18.5	0.15	(1)	10.00
S-AG	0	7572	-18.5	-18.5	0.24	(1)	10.00
AG-AH	0	7572	-18.5	-18.5	0.24	(1)	10.00
AH-R	0	7572	-18.5	-18.5	0.24	(1)	10.00
R-AI	0	7572	-18.5	-18.5	0.24	(1)	10.00
AI-Q	0	7572	-18.5	-18.5	0.24	(1)	10.00
Q-AJ	0	9476	-18.5	-18.5	0.30	(1)	10.00
AJ-P	0	9476	-18.5	-18.5	0.30	(1)	10.00
P-O	0	7339	-18.5	-18.5	0.24	(1)	10.00
O-N	0	7339	-18.5	-18.5	0.24	(1)	10.00
N-M	0	4181	-18.5	-18.5	0.13	(1)	10.00
M-L	0	0	-18.5	-18.5	0.02	(4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LCI	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
B	5-3-4	-78	-78	---	FRONT	VERT	TOTAL	C1
F	17-3-4	-76	-76	---	FRONT	VERT	TOTAL	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
ALLOWABLE DEFL.(TL) = L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/972 (0.43")

CSI: TC=0.40/1.00 (A-U:1), BC=0.30/1.00 (P-Q:1),
WB=0.69/1.00 (A-T:1), SS=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (A) (INPUT = 0.90)
JSI METAL= 0.73 (O) (INPUT = 1.00)



Structural component only
DWG# T-2028120 1/1

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
414964	T1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.330 S May 8 2020 Mtek Industries, Inc. Tue Dec 8 12:44:08 2020 Page 2
ID:76L61 X31PWioZ5adLITzJyDNdu-awLD6GJ8rt-n0wqcDAmK552fZOS6look4Ll6ElyB1mN

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW-1	MT20	5.0	8.0	2.50	3.00
B TMWW-1	MT20	5.0	8.0	2.50	2.75
C TS-1	MT20	5.0	8.0		
D, F, H					
D TMWW-1	MT20	5.0	8.0		
E TMW+w	MT20	3.0	8.0		
G TMW+w	MT20	3.0	8.0		
I TS-1	MT20	5.0	8.0		
J TMWW-1	MT20	5.0	6.0	2.50	2.75
K TMVW-1	MT20	5.0	8.0	2.50	3.00
L BMV1+p	MT20	3.0	6.0		
M BMWW-1	MT20	5.0	8.0	2.50	3.00
N BMWW-1	MT20	5.0	8.0	2.50	2.75
O BS-1	MT20	5.0	8.0		
P BMWWWW-1	MT20	5.0	8.0		
Q BS-1	MT20	5.0	8.0		
R BS-1	MT20	5.0	8.0		
S BMWW-1	MT20	5.0	8.0	2.50	2.75
T BMWW-1	MT20	5.0	8.0	2.50	3.00
U BMV1+p	MT20	3.0	6.0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	19-4-8	-1115	-1115	---	FRONT	VERT	TOTAL	---	C1
T	5-3-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
V	1-3-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
W	3-3-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
X	7-3-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
Y	9-3-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
Z	11-3-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AA	13-3-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AB	15-3-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AC	1-3-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AD	3-3-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AE	7-3-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AF	9-3-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AG	11-3-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AH	13-3-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AI	15-3-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AJ	17-3-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

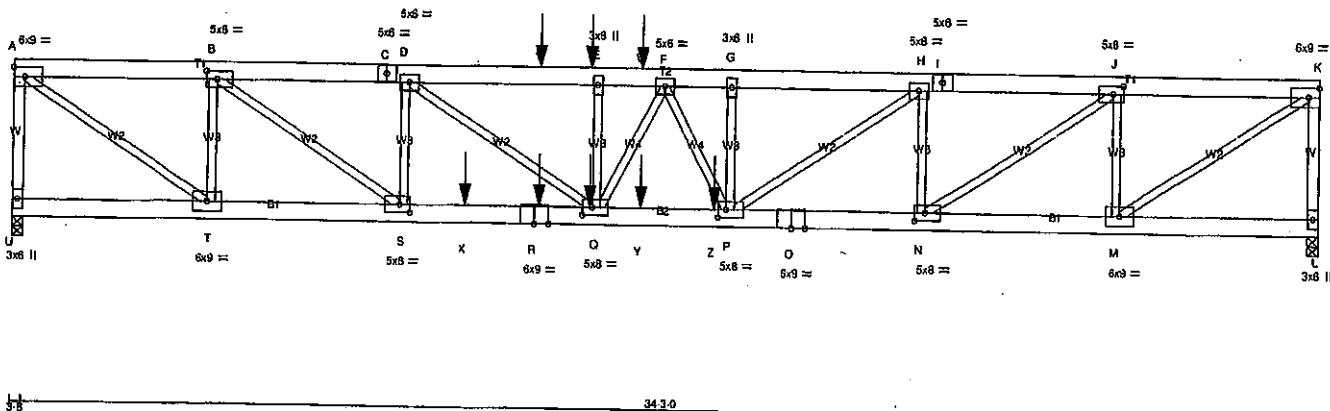
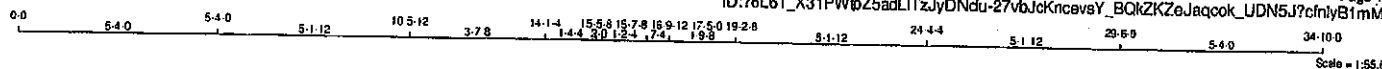
1) C1: A SUITABLE HANGERMechanical CONNECTION IS REQUIRED.



Structural component only
DWG# T-2028120 *M*

JOB NAME 414964	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.330 S May 8 2020 Mitek Industries, Inc. Tue Dec 8 12:44:07 2020 Page 1	

ID:26L61_X31PWpZ5adLITzJyDNdu-27vbJckNcav5Y_BQkZKZeJaqook_UDN5J7cfnyB1mm



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
U - A	2x4	DRY	No.2
A - C	2x6	DRY	No.2
C - I	2x6	DRY	No.2
I - K	2x6	DRY	No.2
L - K	2x4	DRY	No.2
U - R	2x6	DRY	2100F 1.8E
R - O	2x6	DRY	2100F 1.8E
O - L	2x6	DRY	2100F 1.8E
ALL WEBS 2x3 DRY No.2 SPF			
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
U-A	12	TOP
K-L	12	TOP
A-C	12	TOP
C-I	12	TOP
I-K	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
U-R	12	SIDE(0.0)
R-O	12	SIDE(107.8)
O-L	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3/8 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT VERT	3921	DOWN	3921	UPLIFT	IN-SX	IN-SX	
L	3578	0	3578	0	3-8	3-8	

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS		LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW		PERM.							
U	2768	1847	0	0	0	0	0	920	0	0	0
L	2526	1681	0	0	0	0	0	846	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.35 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT.	LOAD (PLF)	MAX. CSI (LC)	UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO									
U-A	-3844	0	0.0	0.44 (1)	6.01	P-G	232	0	0.03 (1)
A-B	-5317	0	-91.8	0.15 (1)	4.97	Q-E	-541	0	0.08 (1)
B-C	-9748	0	-91.8	0.25 (1)	3.79	Q-F	0	73	0.01 (1)
C-D	-9748	0	-91.8	0.25 (1)	3.79	F-P	-370	0	0.05 (1)
D-V	-11780	0	-91.8	0.40 (1)	3.35	D-Q	0	2490	0.31 (1)
V-E	-11780	0	-91.8	0.40 (1)	3.35	A-T	0	6440	0.80 (1)
E-W	-11780	0	-91.8	0.28 (1)	3.46	S-D	-1942	0	0.23 (1)
W-F	-11780	0	-91.8	0.28 (1)	3.46	T-B	-3675	0	0.44 (1)
F-G	-11585	0	-91.8	0.28 (1)	3.49	B-S	0	5431	0.67 (1)
G-H	-11585	0	-91.8	0.37 (1)	3.41	M-K	0	5872	0.73 (1)
H-I	-8836	0	-91.8	0.21 (1)	4.03	P-H	0	3814	0.45 (1)
I-J	-8836	0	-91.8	0.21 (1)	4.03	M-J	-3211	0	0.38 (1)
J-K	-4848	0	-91.8	0.14 (1)	5.17	N-H	-2594	0	0.31 (1)
L-K	-3523	0	0.0	0.40 (1)	6.24	N-J	0	4642	0.57 (1)
U-T									
T-S	0	0	-18.5	-18.5	0.03 (1)	10.00			
S-X	0	5317	-18.5	-18.5	0.22 (1)	10.00			
X-R	0	9748	-18.5	-18.5	0.51 (1)	10.00			
R-Q	0	9748	-18.5	-18.5	0.51 (1)	10.00			
Q-Y	0	9748	-18.5	-18.5	0.51 (1)	10.00			
Y-Z	0	11748	-18.5	-18.5	0.45 (1)	10.00			
Z-P	0	11748	-18.5	-18.5	0.45 (1)	10.00			
P-O	0	8836	-18.5	-18.5	0.28 (1)	10.00			
O-N	0	8836	-18.5	-18.5	0.28 (1)	10.00			
N-M	0	4848	-18.5	-18.5	0.15 (1)	10.00			
M-L	0	0	-18.5	-18.5	0.02 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)		LOC.		MAX.		FACE		DIR.		TYPE		HEEL		CONN.	
JT	15-5.8	-78	-78			BACK	VERT	TOTAL							C1
E	15-5.8	-21	-21			BACK	VERT	TOTAL							C1
Q	14-1.4	-21	-21			BACK	VERT	TOTAL							C1
R	14-1.4	-78	-78			BACK	VERT	TOTAL							C1
V	16.9-12	-78	-78			BACK	VERT	TOTAL							C1
W	12-1.8	-1018	-1018			BACK	VERT	TOTAL							C1
X	16.9-12	-21	-21			BACK	VERT	TOTAL							C1
Y	18-9.8	-1268	-1268			BACK	VERT	TOTAL							C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBCG 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014.

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.18")
CALCULATED VERT. DEFL.(LL) = U/899 (0.29")
ALLOWABLE DEFL.(TL) = L/360 (1.18")
CALCULATED VERT. DEFL.(TL) = U/791 (0.53")

CSI: TC=0.44:1.00 (A-U:1), BC=0.51:1.00 (Q-S:1), WB=0.80:1.00 (A-T:1), SS=0.49:1.00 (P-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (T) (INPUT = 0.90)
JSI METAL = 0.84 (P) (INPUT = 1.00)



Structural component only
DWG# T-2028121 1/2

JOB NAME 414964	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.330 S May 6 2020 MTEK Industries, Inc. Tue Dec 8 12:44:07 2020 Page 2 ID:76L61 X31PWtpZ5adLITzJyDNdu-27vbJckKncvysY BQkZKZeJagcok UDN5J?cfnlyB1mM	

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	6.0	9.0	Edge
B	TMWW-t	MT20	5.0	8.0	2.50 3.25
C	TS-t	MT20	5.0	6.0	
D, F, H					
D	TMWW-t	MT20	5.0	6.0	
E	TMW+w	MT20	3.0	6.0	
G	TMW+w	MT20	3.0	6.0	
I	TS-t	MT20	5.0	6.0	
J	TMWW-t	MT20	5.0	8.0	2.50 3.25
K	TMVW-t	MT20	6.0	9.0	Edge
L	BMV1+p	MT20	3.0	6.0	
M	BMWW-t	MT20	6.0	9.0	
N	BMWW-t	MT20	5.0	8.0	2.50 3.25
O	BS-t	MT20	6.0	9.0	
P	BMWWW-t	MT20	5.0	8.0	2.50 2.50
Q	BMWWW-t	MT20	5.0	8.0	2.50 3.00
R	BS-t	MT20	6.0	9.0	
S	BMWW-t	MT20	5.0	8.0	2.50 3.25
T	BMWW-t	MT20	6.0	9.0	
U	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

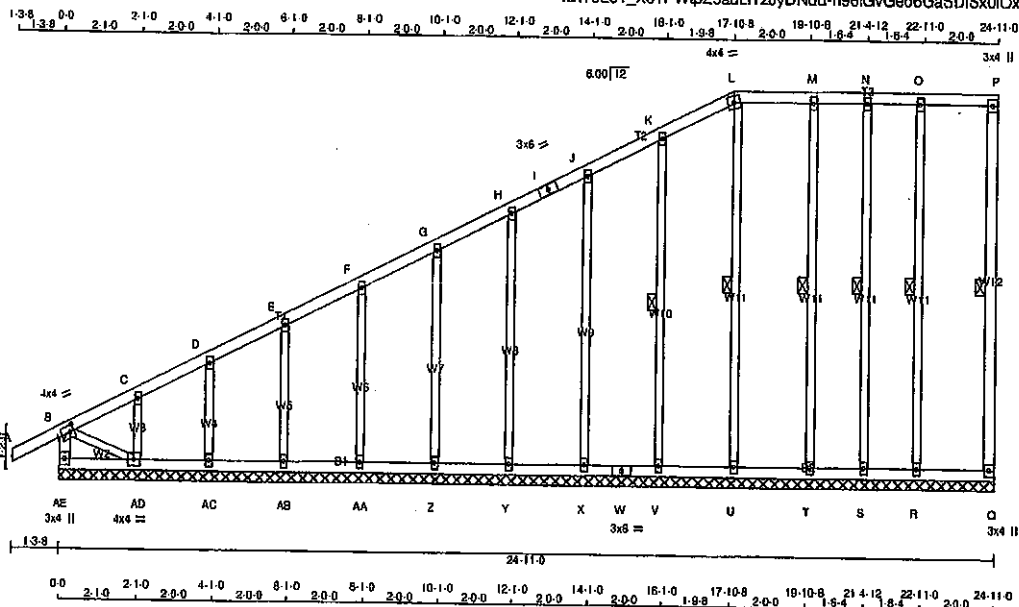
CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2028121 *me*

JOB NAME 414964	TRUSS NAME G1	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.330 S May 6 2020 MTak Industries, Inc. Tue Dec 8 12:44:02 2020 Page 1	



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
AE - B	2x4	DRY	No.2
A - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
L - P	2x4	DRY	No.2
Q - W	2x4	DRY	No.2
AE - W	2x4	DRY	No.2
W - Q	2x4	DRY	No.2
ALL WEBS 2x3 DRY			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	4.0	2.00 1.25
C, D, E, F, G, H, J, K, M, N, O					
C	TMVW-w	MT20	2.0	4.0	
I	TS-1	MT20	3.0	6.0	
L	TTW-m	MT20	4.0	4.0	
P	TMV1-p	MT20	3.0	4.0	
Q	BMV1-p	MT20	3.0	4.0	
R, S, T, U, V, X, Y, Z, AA, AB, AC					
R	BMV1-w	MT20	2.0	4.0	
W	BS-1	MT20	3.0	6.0	
AD	BMVW1-1	MT20	4.0	4.0	
AE	BMV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
AE	237	0	237	0	0	24-11-0 (14-12421-0)	
Q	99	0	99	0	0	24-11-0 (14-12421-0)	
U	181	0	191	0	0	24-11-0 (14-12421-0)	
T	218	0	218	0	0	24-11-0 (14-12421-0)	
R	221	0	221	0	0	24-11-0 (14-12421-0)	
S	134	0	134	0	0	24-11-0 (14-12421-0)	
V	208	0	208	0	0	24-11-0 (14-12421-0)	
X	217	0	217	0	0	24-11-0 (14-12421-0)	
Y	220	0	220	0	0	24-11-0 (14-12421-0)	
Z	220	0	220	0	0	24-11-0 (14-12421-0)	
AA	219	0	219	0	0	24-11-0 (14-12421-0)	
AB	222	0	222	0	0	24-11-0 (14-12421-0)	
AC	203	0	203	0	0	24-11-0 (14-12421-0)	
AD	232	0	232	0	0	24-11-0 (14-12421-0)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LOASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AE	165	123 / 0	0 / 0	0 / 0	0 / 0	41 / 0	0 / 0
Q	70	47 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0
U	138	85 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0
T	154	103 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0
R	157	101 / 0	0 / 0	0 / 0	0 / 0	56 / 0	0 / 0
S	95	64 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
V	168	112 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
X	154	100 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
Y	155	102 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
Z	155	102 / 0	0 / 0	0 / 0	0 / 0	54 / 0	0 / 0
AA	155	101 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
AB	157	103 / 0	0 / 0	0 / 0	0 / 0	54 / 0	0 / 0
AC	144	94 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
AD	164	106 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AE, Q, U, T, R, S, V, X, Y, Z, AA, AB, AC, AD

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF P-Q, L-U, M-T, O-R, N-S, K-V.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
AE-B	-221 / 0	U-L	-153 / 0
A-B	0 / 28	T-M	-185 / 0
B-C	-20 / 0	R-O	-181 / 0
C-D	-22 / 0	S-N	-115 / 0
D-E	-15 / 0	V-K	-201 / 0
E-F	-12 / 0	X-J	-180 / 0
F-G	-9 / 0	Y-H	-183 / 0
G-H	-7 / 0	Z-G	-183 / 0
H-I	-5 / 0	AA-F	-182 / 0
I-J	-5 / 0	AB-E	-185 / 0

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (A-B:1), SC=0.02/1.00 (AC-AD:4), WB=0.23/1.00 (J-X:1), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

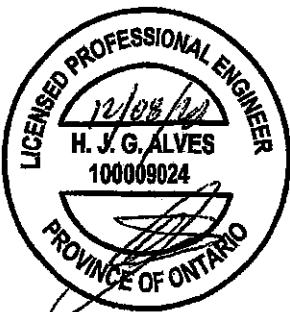
PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MT20	618	354
	1667	788
	1667	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (B) (INPUT = 0.90)
JSI METAL= 0.08 (K) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2028116 1/2

JOB NAME 414964	TRUSS NAME G1	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.330 S May 6 2020 Mitek Industries, Inc. Tue Dec 8 12:44:03 2020 Page 2	
ID:76L61 X31PWipZ5adLITzIvDNdu-ALq4UFHGGZQPR3NtoVIGdTTQDdBUjYYBVONeRezyB1mC					

LOADING
TOTAL LOAD CASES: (4)

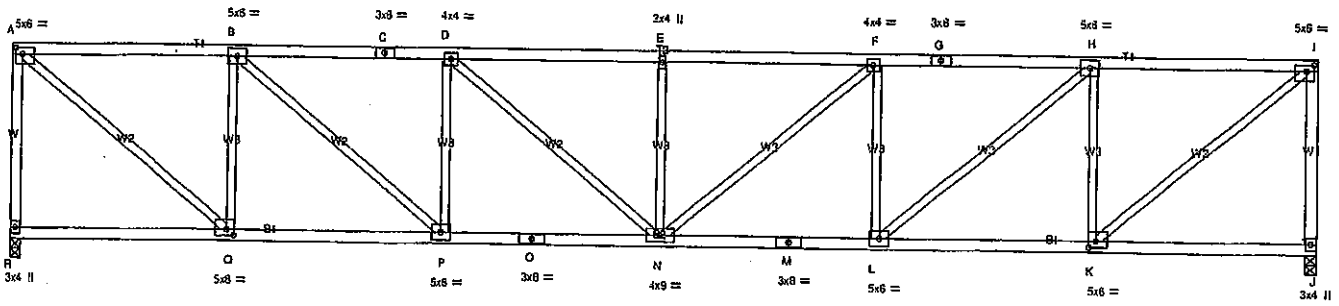
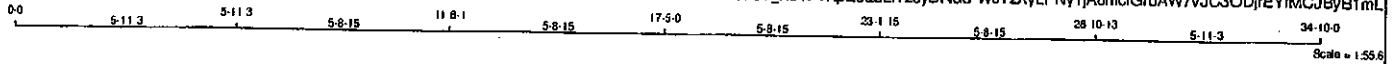
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 MAX. CSI (LC)	
FR-TO		FROM TO		LENGTH FR-TO			
J-K	-270	-91.8	-91.8 0.05 (1)	10.00	AC-D	-16970	0.03 (1)
K-L	-1070	-91.8	-91.8 0.05 (1)	10.00	AD-C	-20070	0.03 (1)
L-M	070	-91.8	-91.8 0.05 (1)	10.00	B-AD	0727	0.01 (1)
M-N	070	-91.8	-91.8 0.04 (1)	10.00			
N-O	070	-91.8	-91.8 0.04 (1)	10.00			
O-P	070	-91.8	-91.8 0.05 (1)	10.00			
Q-P	-8470	0.0	0.0 0.04 (1)	6.25			
AE-AD	070	-18.5	-18.5 0.02 (4)	10.00			
AD-AC	0718	-18.5	-18.5 0.02 (4)	10.00			
AC-AB	0714	-18.5	-18.5 0.02 (4)	10.00			
AB-AA	0711	-18.5	-18.5 0.02 (4)	10.00			
AA-Z	078	-18.5	-18.5 0.01 (4)	10.00			
Z-Y	076	-18.5	-18.5 0.01 (4)	10.00			
Y-X	075	-18.5	-18.5 0.01 (4)	10.00			
X-W	073	-18.5	-18.5 0.01 (4)	10.00			
W-V	073	-18.5	-18.5 0.01 (4)	10.00			
V-U	072	-18.5	-18.5 0.02 (4)	10.00			
U-T	070	-18.5	-18.5 0.02 (4)	10.00			
T-S	070	-18.5	-18.5 0.01 (4)	10.00			
S-R	070	-18.5	-18.5 0.02 (4)	10.00			
R-Q	070	-18.5	-18.5 0.02 (4)	10.00			



Structural component only
DWG# T-2028116 *mr*

JOB NAME 414964	TRUSS NAME T2	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.330 S May 6 2020 Mitek Industries, Inc. Tue Dec 8 12:44:08 2020 Page 1
ID:76L61_X31PWtpZ5adLITzJyDNdu-WJTzXyLPNy1A8mclGroAW7vJC3ODjREYfMCJByB1mL



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
R - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
J - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER.			

PLATES (tablets in inches)					
JT	TYPE	PLATES	W	LEN	X
A	TMVW-I	MT20	5.0	6.0	2.00 2.25
B	TMVW-I	MT20	5.0	6.0	
C	TS-I	MT20	3.0	6.0	
D	TMVW-I	MT20	4.0	4.0	
E	TMVW-I	MT20	2.0	4.0	
F	TMVW-I	MT20	4.0	4.0	
G	TS-I	MT20	3.0	6.0	
H	TMVW-I	MT20	5.0	6.0	
I	TMVW-I	MT20	5.0	6.0	2.00 2.25
J	BMV1-p	MT20	3.0	4.0	
K	BMVW-I	MT20	5.0	6.0	2.00 2.25
L	BMVW-I	MT20	5.0	6.0	
M	BS-I	MT20	3.0	6.0	
N	BMVW-I	MT20	4.0	9.0	
O	BS-I	MT20	3.0	6.0	
P	BMVW-I	MT20	5.0	6.0	
Q	BMVW-I	MT20	5.0	6.0	2.00 2.25
R	BMV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	HORIZ	UPLIFT	BRG	IN-SX	BRG	IN-SX
R	1920	0	1920	0	0	3-8	3-8		
J	1920	0	1920	0	0	3-8	3-8		

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	VERT	DOWN	HORIZ	UPLIFT	BRG	IN-SX	BRG	IN-SX					
R	1358	891.0	0.0	0.0	0.0	0.0	0.0	0.0	467.0	0.0	0.0	0.0	0.0	0.0	0.0
J	1358	891.0	0.0	0.0	0.0	0.0	0.0	0.0	467.0	0.0	0.0	0.0	0.0	0.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

CHORDS		MEMB.		MAX. FACTORED		FORCE		VERT. LOAD		MAX. FACTORED		MEMB.		MAX. FACTORED		FORCE		VERT. LOAD	
FR-TO	FROM	TO	FROM	TO	FROM	TO	FROM	TO	FROM	TO	FROM	TO	FROM	TO	FROM	TO	FROM	TO	FROM
R-A	-1875.0	0.0	0.0	0.83 (1)	6.13	K-I	0	2629	0.59 (1)										
A-B	-2042.0	-18.5	-18.5	0.40 (1)	10.00	A-Q	0	2629	0.59 (1)										
B-C	-3149.0	-18.5	-18.5	0.74 (1)	3.28	K-H	-1535.0	0.59 (1)											
C-D	-3149.0	-18.5	-18.5	0.74 (1)	3.28	Q-B	-1535.0	0.59 (1)											
D-E	-3533.0	-18.5	-18.5	0.68 (1)	3.23	L-H	0	1444	0.33 (1)										
E-F	-3533.0	-18.5	-18.5	0.68 (1)	3.23	B-P	0	1444	0.33 (1)										
F-G	-3149.0	-18.5	-18.5	0.74 (1)	3.28	L-F	-826.0	0.32 (1)											
G-H	-3149.0	-18.5	-18.5	0.74 (1)	3.28	P-D	-826.0	0.32 (1)											
H-I	-2042.0	-18.5	-18.5	0.59 (1)	4.10	N-F	0	502	0.11 (1)										
I-J	-1875.0	0.0	0.0	0.83 (1)	6.13	D-N	0	502	0.11 (1)										
R-Q	0.0	-18.5	-18.5	0.15 (4)	10.00	N-E	-537.0	0.21 (1)											
Q-P	0.0	-18.5	-18.5	0.40 (1)	10.00														
P-O	0.0	-18.5	-18.5	0.58 (1)	10.00														
O-N	0.0	-18.5	-18.5	0.56 (1)	10.00														
N-M	0.0	-18.5	-18.5	0.58 (1)	10.00														
M-L	0.0	-18.5	-18.5	0.58 (1)	10.00														
L-K	0.0	-18.5	-18.5	0.40 (1)	10.00														
K-J	0.0	-18.5	-18.5	0.15 (4)	10.00														

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.16")
CALCULATED DEFL. (LL) = L/999 (0.23")
ALLOWABLE DEFL. (TL) = L/360 (1.16")
CALCULATED VERT. DEFL. (TL) = L/998 (0.42")

CSI: TC=0.83/1.00 (A-R:1), BC=0.58/1.00 (L-N:1), WB=0.59/1.00 (I-K:1), SS=0.25/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

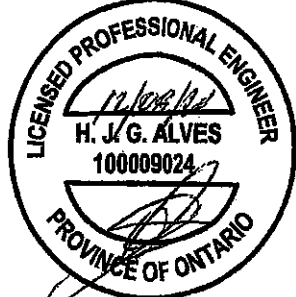
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (A) (INPUT = 0.90)
JSI METAL = 0.99 (C) (INPUT = 1.00)

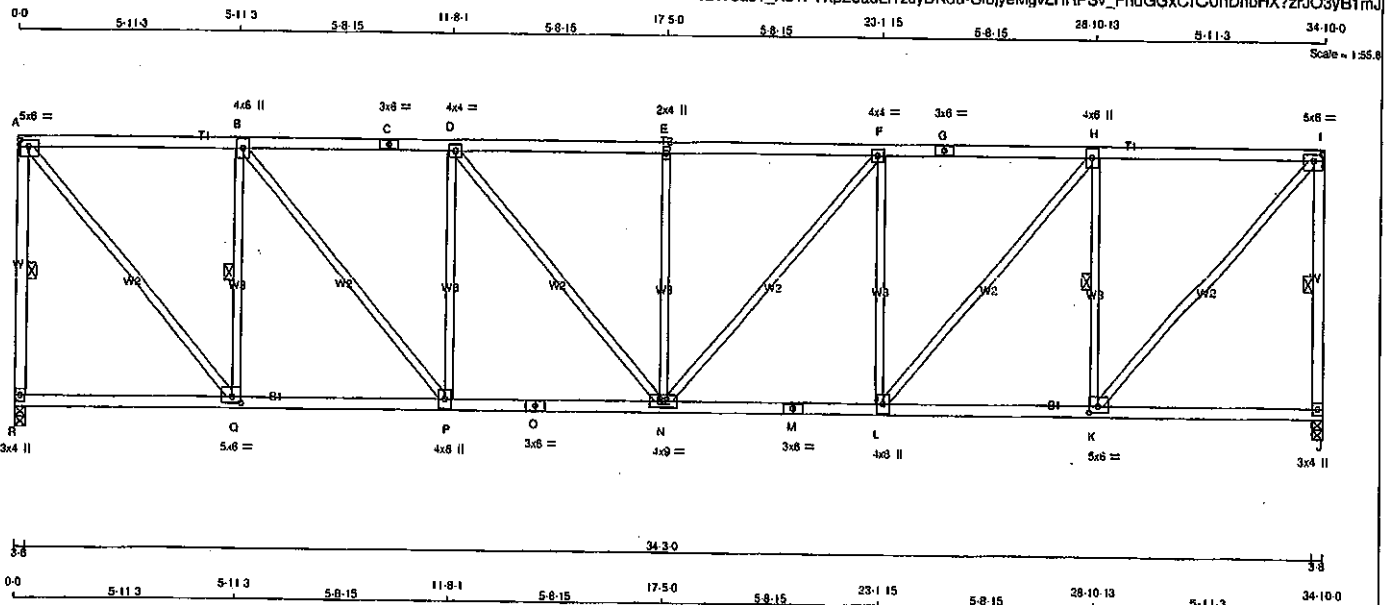


Structural component only
DWG# T-2028122

JOB NAME 414964	TRUSS NAME T4	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
---------------------------	-------------------------	----------------------	-----------------	--------------------------------------	-------------	----------

Tamarack Roof Truss, Burlington

Version 8.330 S May 8 2020 MTEK Industries, Inc. Tue Dec 8 12:44:10 2020 Page 1
ID:76L61_X31PWtpZ5adLITzJyDNdu-SibjyeMgvZHRPSv_PhuGGxCICOnDhbHX?zrJO3yB1mj



LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
R - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
J - L	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY-SEASONED LUMBER.			

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-I	MT20	5.0	6.0	2.00	2.75
B	TMVW-H	MT20	4.0	6.0		
C	TS-I	MT20	3.0	6.0		
D	IMVW-I	MT20	4.0	4.0		
E	TMVW-I	MT20	2.0	4.0		
F	TMVW-I	MT20	4.0	4.0		
G	TS-I	MT20	3.0	6.0		
H	TMVW-H	MT20	4.0	6.0		
I	TMVW-I	MT20	5.0	6.0	2.00	2.75
J	BMV1+P	MT20	3.0	4.0		
K	BMVW-I	MT20	5.0	6.0	2.00	2.75
L	BMVW-I	MT20	4.0	6.0		
M	BS-I	MT20	3.0	6.0		
N	BS-I	MT20	4.0	6.0		
O	BS-I	MT20	3.0	6.0		
P	BMVW-I	MT20	4.0	6.0		
Q	BMVW-I	MT20	5.0	6.0	2.00	2.75
R	BMV1+P	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
R	1920	0	1920	0	3.8
J	1920	0	1920	0	3.8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1358	891.0	0.0	0.0	0.0	467.0	0.0
J	1358	891.0	0.0	0.0	0.0	467.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-R, I-J, H-K, B-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX
FR-TO		FROM	TO	FR-TO		FROM	TO
R-A	-1876.0	0.0	0.0 0.43 (1)	K-I	0 2199	0.49 (1)	
A-B	-1444.0	-91.8	-91.8 0.53 (1)	A-Q	0 2199	0.49 (1)	
B-C	-2226.0	-91.8	-91.8 0.61 (1)	K-H	-1536.0	0.49 (1)	
C-D	-2226.0	-91.8	-91.8 0.61 (1)	Q-B	-1536.0	0.49 (1)	
D-E	-2497.0	-91.8	-91.8 0.53 (1)	L-H	0 1214	0.27 (1)	
E-F	-2497.0	-91.8	-91.8 0.53 (1)	B-P	0 1214	0.27 (1)	
F-G	-2226.0	-91.8	-91.8 0.61 (1)	L-F	-826.0	0.72 (1)	
G-H	-2226.0	-91.8	-91.8 0.61 (1)	P-D	-826.0	0.72 (1)	
H-I	-1444.0	-91.8	-91.8 0.53 (1)	N-F	0 421	0.09 (1)	
I-J	-1876.0	0.0	0.0 0.43 (1)	D-N	0 421	0.09 (1)	
				N-E	-537.0	0.47 (1)	
R-Q	0 0	-18.5	-18.5 0.15 (4)				
Q-P	0 1444	-18.5	-18.5 0.31 (1)				
P-O	0 2226	-18.5	-18.5 0.41 (1)				
O-N	0 2226	-18.5	-18.5 0.41 (1)				
N-M	0 2226	-18.5	-18.5 0.41 (1)				
M-L	0 2226	-18.5	-18.5 0.41 (1)				
L-K	0 1444	-18.5	-18.5 0.31 (1)				
K-J	0 0	-18.5	-18.5 0.15 (4)				

TOTAL WEIGHT = 2 X 183 = 325 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.18")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.18")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.61:1.00 (F:H:1), BC=0.41:1.00 (L:N:1),
WB=0.72:1.00 (F:L:1), SSI=0.26:1.00 (A:B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	618	354
	1687	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

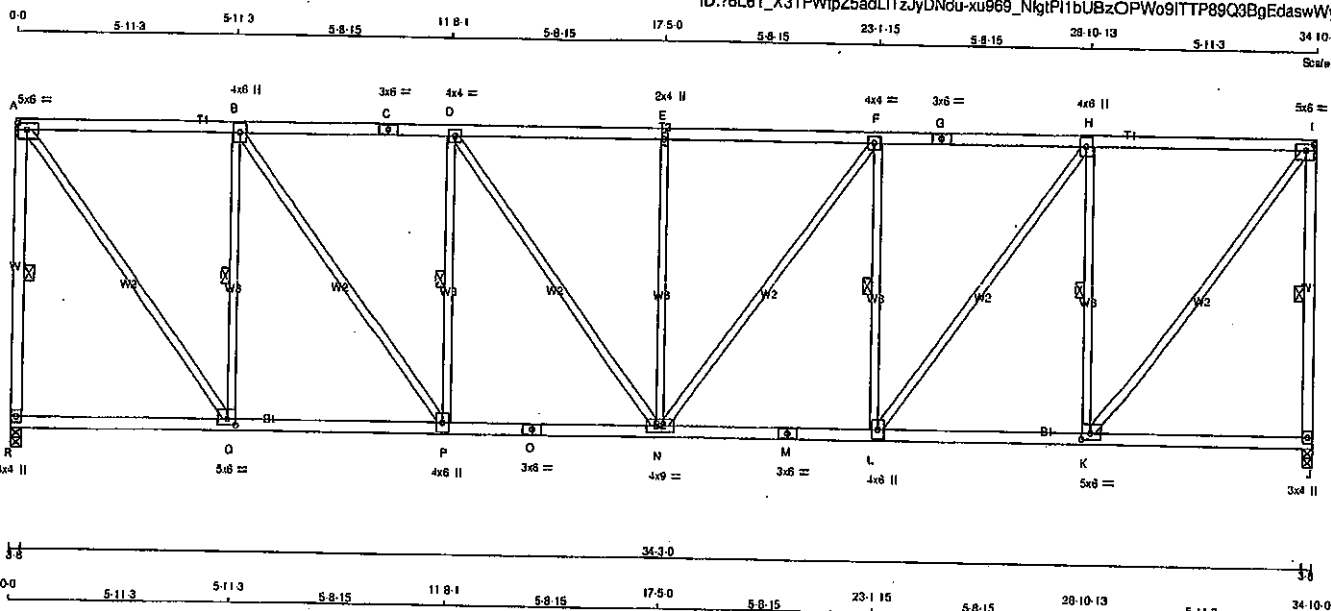
JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.72 (M) (INPUT = 1.00)



Structural component only
DWG# T-2028124

JOB NAME 414964	TRUSS NAME T5	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.390 S May 8 2020 Mitek Industries, Inc. Tue Dec 8 12:44:11 2020 Page 1
ID:76L61_X31PWpZ5adLITzJyDNdu-xu969_Nkg1PI1bUBzOPW09ITTP89Q3BgEdaswWyB1ml



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
R - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
J - I	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMWVH	MT20	5.0	6.0 2.00 2.75
B	TMWVH	MT20	4.0	6.0
C	TS-1	MT20	3.0	6.0
D	TMWVH	MT20	4.0	4.0
E	TMWVH	MT20	2.0	4.0
F	TMWVH	MT20	4.0	4.0
G	TS-1	MT20	3.0	6.0
H	TMWVH	MT20	4.0	6.0
I	TMWVH	MT20	5.0	6.0 2.00 2.75
J	BMV1+p	MT20	3.0	4.0
K	BMWVH	MT20	5.0	6.0 2.00 2.75
L	BMWVH	MT20	4.0	6.0
M	BS-1	MT20	3.0	6.0
N	BMWVH	MT20	4.0	9.0
O	BS-1	MT20	3.0	6.0
P	BMWVH	MT20	4.0	6.0
Q	BMWVH	MT20	5.0	6.0 2.00 2.75
R	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT VERT	HORZ	JT VERT	HORZ	IN-SK		IN-SK	
R 1920	0	1920	0	3-8		3-8	
J 1920	0	1920	0	3-8		3-8	

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1358	891.0	0.0	0.0	0.0	467.0	0.0
J	1358	891.0	0.0	0.0	0.0	467.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-R, I-J, H-K, B-Q, F-L, D-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
R-A	-1876.0	0.0	0.0 0.56 (1)	K-I	0 2082	0.47 (1)	
A-B	-1259.0	-91.8	-91.8 0.52 (1)	A-Q	0 2082	0.47 (1)	
B-C	-1941.0	-91.8	-91.8 0.58 (1)	K-H	-1536.0	0.64 (1)	
C-D	-1941.0	-91.8	-91.8 0.58 (1)	Q-B	-1536.0	0.64 (1)	
D-E	-2178.0	-91.8	-91.8 0.49 (1)	L-H	0 1152	0.26 (1)	
E-F	-2178.0	-91.8	-91.8 0.49 (1)	B-P	0 1152	0.26 (1)	
F-G	-1941.0	-91.8	-91.8 0.58 (1)	L-F	-826.0	0.34 (1)	
G-H	-1941.0	-91.8	-91.8 0.58 (1)	P-D	-826.0	0.34 (1)	
H-I	-1259.0	-91.8	-91.8 0.52 (1)	N-F	0 400	0.09 (1)	
I-J	-1876.0	0.0	0.0 0.56 (1)	D-N	0 400	0.09 (1)	
				N-E	-537.0	0.68 (1)	
R-Q	0 0	-18.5	-18.5 0.15 (4)				
Q-P	0 1259	-18.5	-18.5 0.29 (1)				
P-O	0 1941	-18.5	-18.5 0.37 (1)				
O-N	0 1941	-18.5	-18.5 0.37 (1)				
N-M	0 1941	-18.5	-18.5 0.37 (1)				
M-L	0 1941	-18.5	-18.5 0.37 (1)				
L-K	0 1259	-18.5	-18.5 0.29 (1)				
K-J	0 0	-18.5	-18.5 0.15 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
DL	= 6.0	PSF	
BOT. CH.	LL	= 0.0	PSF
DL	= 7.4	PSF	
TOTAL LOAD	= 35.0	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.58/1.00 (B-D:1), BC=0.37/1.00 (N-P:1), WB=0.68/1.00 (E-N:1), SS=0.26/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667 788 1987 1858

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.90 (Q) (INPUT = 0.90)
JSI METAL=0.64 (M) (INPUT = 1.00)

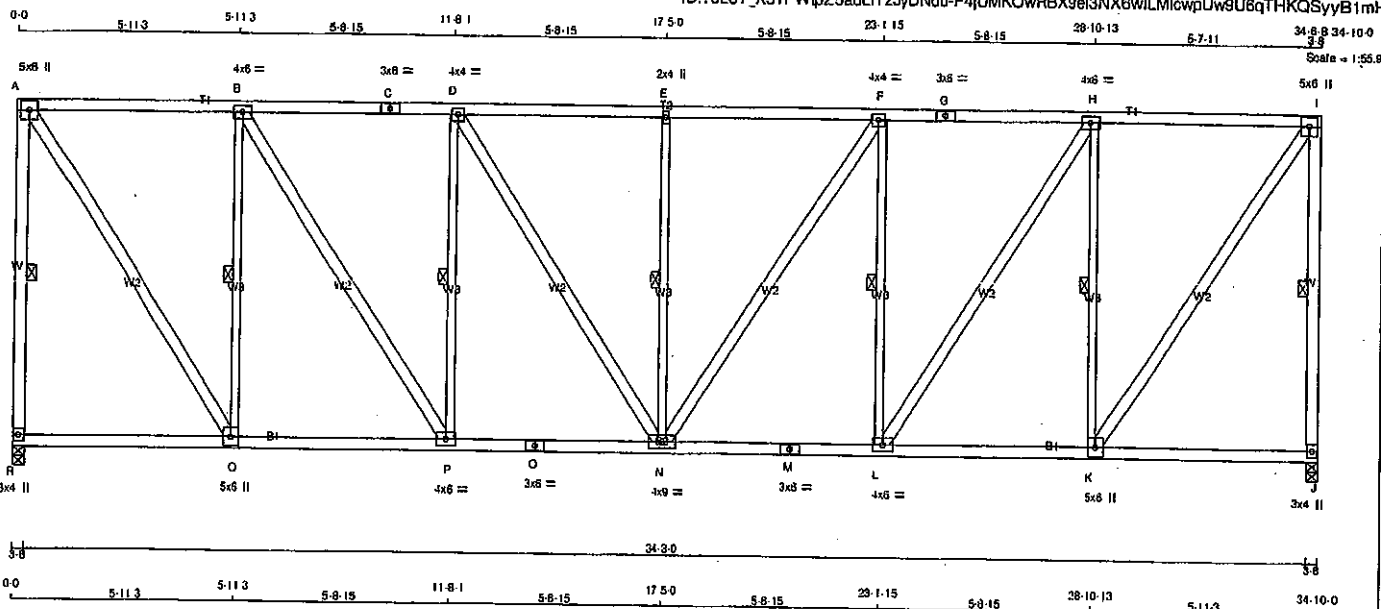


Structural component only
DWG# T-2028125

JOB NAME 414964	TRUSS NAME T6	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
---------------------------	-------------------------	----------------------	-----------------	--------------------------------------	-------------	----------

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 Mitek Industries, Inc. Tue Dec 8 12:44:12 2020 Page 1
ID:76L61_X31PWpZ5adLITzJyDNdu-P4JUMKOWRBX9eI3NX6wLMicwpUw9U8qTHKQSyYB1m



LUMBER				DESIGN CRITERIA			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	SPF	FACTORED	MAXIMUM FACTORED
R - A	2x4	DRY	No.2	SPF	JT	VERT	GROSS REACTION
A - C	2x4	DRY	No.2	SPF	R	1920	0
C - G	2x4	DRY	No.2	SPF	J	1920	0
G - I	2x4	DRY	No.2	SPF	J	1920	0
J - L	2x4	DRY	No.2	SPF	J	1920	0
L - O	2x4	DRY	No.2	SPF	J	1920	0
O - M	2x4	DRY	No.2	SPF	J	1920	0
M - J	2x4	DRY	No.2	SPF	J	1920	0
ALL WEBS	2x4	DRY	No.2	SPF	JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS
EXCEPT					R	1358	891.0
K - H	2x3	DRY	No.2	SPF	R	1358	891.0
Q - B	2x3	DRY	No.2	SPF	R	1358	891.0
L - F	2x3	DRY	No.2	SPF	R	1358	891.0
P - D	2x3	DRY	No.2	SPF	R	1358	891.0
N - E	2x3	DRY	No.2	SPF	R	1358	891.0
DRY: SEASONED LUMBER.				TOTAL WEIGHT = 2 X 203 = 406 lb			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	
B	TMVW-i	MT20	4.0	6.0	
C	TS-i	MT20	3.0	6.0	
D	TMVW-i	MT20	4.0	4.0	
E	TMVW-w	MT20	2.0	4.0	
F	TMVW-i	MT20	4.0	4.0	
G	TS-i	MT20	3.0	6.0	
H	TMVW-i	MT20	4.0	6.0	
I	TMVW-p	MT20	5.0	6.0	
J	BMV1-p	MT20	3.0	4.0	
K	BMVW-i	MT20	5.0	6.0	
L	BMVW-i	MT20	4.0	6.0	
M	BS-i	MT20	3.0	6.0	
N	BMVW-i	MT20	4.0	9.0	
O	BS-i	MT20	3.0	6.0	
P	BMVW-i	MT20	4.0	6.0	
Q	BMVW-i	MT20	5.0	6.0	
R	BMV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG
VERT	HORZ	DOWN	HORZ
JT	1920	0	0
R	1920	0	0
J	1920	0	0

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED
R	1358
J	1358

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-R, I-J, H-K, B-Q, F-L, D-P, E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO		
R-A	-1876 0	0.0 0.0	0.72 (1)	4.90	K-I	0 2000	0.32 (1)
A-B	-1117 0	-91.8 -91.8	0.51 (1)	5.29	A-Q	0 2000	0.32 (1)
B-C	-1721 0	-91.8 -91.8	0.58 (1)	4.41	K-H	-1536 0	0.83 (1)
C-D	-1721 0	-91.8 -91.8	0.58 (1)	4.41	Q-B	-1536 0	0.83 (1)
D-E	-1931 0	-91.8 -91.8	0.47 (1)	4.37	L-H	0 1107	0.18 (1)
E-F	-1931 0	-91.8 -91.8	0.47 (1)	4.37	B-P	0 1107	0.18 (1)
F-G	-1721 0	-91.8 -91.8	0.56 (1)	4.41	L-F	-826 0	0.45 (1)
G-H	-1721 0	-91.8 -91.8	0.56 (1)	4.41	P-D	-826 0	0.45 (1)
H-I	-1117 0	-91.8 -91.8	0.51 (1)	5.29	N-F	0 384	0.08 (1)
I-J	-1876 0	0.0 0.0	0.72 (1)	4.90	D-N	0 384	0.08 (1)
					N-E	-537 0	0.29 (1)
R-Q	0 0	-18.5 -18.5	0.16 (4)	10.00			
Q-P	0 1117	-18.5 -18.5	0.27 (1)	10.00			
P-O	0 1721	-18.5 -18.5	0.33 (1)	10.00			
O-N	0 1721	-18.5 -18.5	0.33 (1)	10.00			
N-M	0 1721	-18.5 -18.5	0.33 (1)	10.00			
M-L	0 1721	-18.5 -18.5	0.33 (1)	10.00			
L-K	0 1117	-18.5 -18.5	0.27 (1)	10.00			
K-J	0 0	-18.5 -18.5	0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
OL	=	6.0	PSF	
BOT CH.	LL	=	9.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.16")
CALCULATED VERT. DEFL. (LL) = L/999 (0.12")
ALLOWABLE DEFL. (TL) = L/360 (1.16")
CALCULATED VERT. DEFL. (TL) = L/999 (0.21")

CSI: TO=0.72/1.00 (A-R:1), BO=0.33/1.00 (N-P:1),
WB=0.63/1.00 (H-K:1), SSI=0.28/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

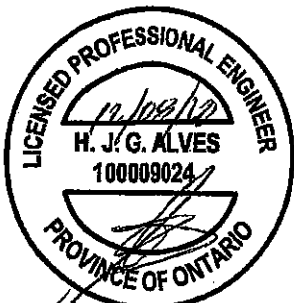
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (K) (INPUT = 0.90)
JSI METAL = 0.57 (M) (INPUT = 1.00)

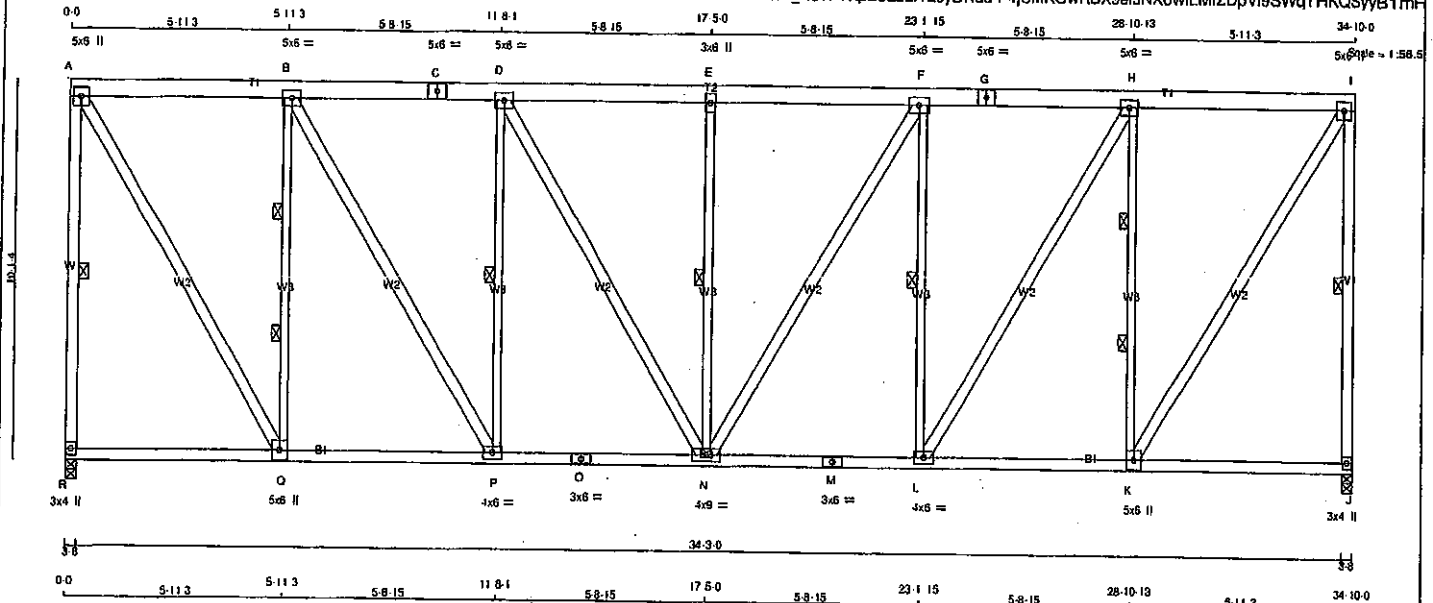


Structural component only
DWG# T-2028126

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
414964	T7	8	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

Version 8.330 5 May 8 2020 M Tek Industries, Inc. Tue Dec 8 12:44:12 2020 Page 1
ID:26L61_X31PWpZ5adLITzJyDNdu-P4jUMKOWRBX9el3NX6wLMIZDpV9SWqTHKQSYyB1mH



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
R - A	2x4	DRY	No.2	SPF	
A - C	2x6	DRY	No.2	SPF	
C - G	2x6	DRY	No.2	SPF	
G - I	2x6	DRY	No.2	SPF	
J - L	2x4	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x4	DRY	No.2	SPF	
EXCEPT					
K - H	2x3	DRY	No.2	SPF	
O - B	2x3	DRY	No.2	SPF	
L - F	2x3	DRY	No.2	SPF	
P - D	2x3	DRY	No.2	SPF	
N - E	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0		
B, D, F, H						
B	TMVW-t	MT20	5.0	6.0		
C	TS-t	MT20	5.0	6.0		
E	TMVW+p	MT20	5.0	6.0		
G	TS-t	MT20	5.0	6.0		
I	TMVW+p	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	4.0		
K	BMVW-t	MT20	5.0	6.0		
L	BMVW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	6.0		
Q	BMVW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORIZ	DOWN	HORIZ
R	1920	0	0
J	1920	0	0

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
R	1358	891
J	1358	891

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-R, J-L, F-L, D-P, E-N.
2 LATERAL BRACE(S) AT 1/3 LENGTH OF H-K, B-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	WEBS
MEMB.	MEMB.
MAX. FACTORED	MAX. FACTORED
FORCE	FORCE
(LBS)	(LBS)
VERT. LOAD	VERT. LOAD
LC1	LC1
MAX	MAX
CSI (LC)	CSI (LC)
FROM TO	FROM TO
FR-TO	FR-TO
R-A	-1876
A-B	-1009
B-C	-1558
C-D	-1558
D-E	-1747
E-F	-1747
F-G	-1558
G-H	-1558
H-I	-1009
J-I	-1876
R-Q	0
Q-P	0
P-O	0
O-N	0
N-M	0
M-L	0
L-K	0
K-J	0

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT. CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.2 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2018 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.90/1.00 (A-R:1), BC=0.31/1.00 (N-P:1),
WB=0.93/1.00 (H-K:1), SS=0.20/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.82 (I) (INPUT = 0.90)
JSI METAL=0.52 (M) (INPUT = 1.00)



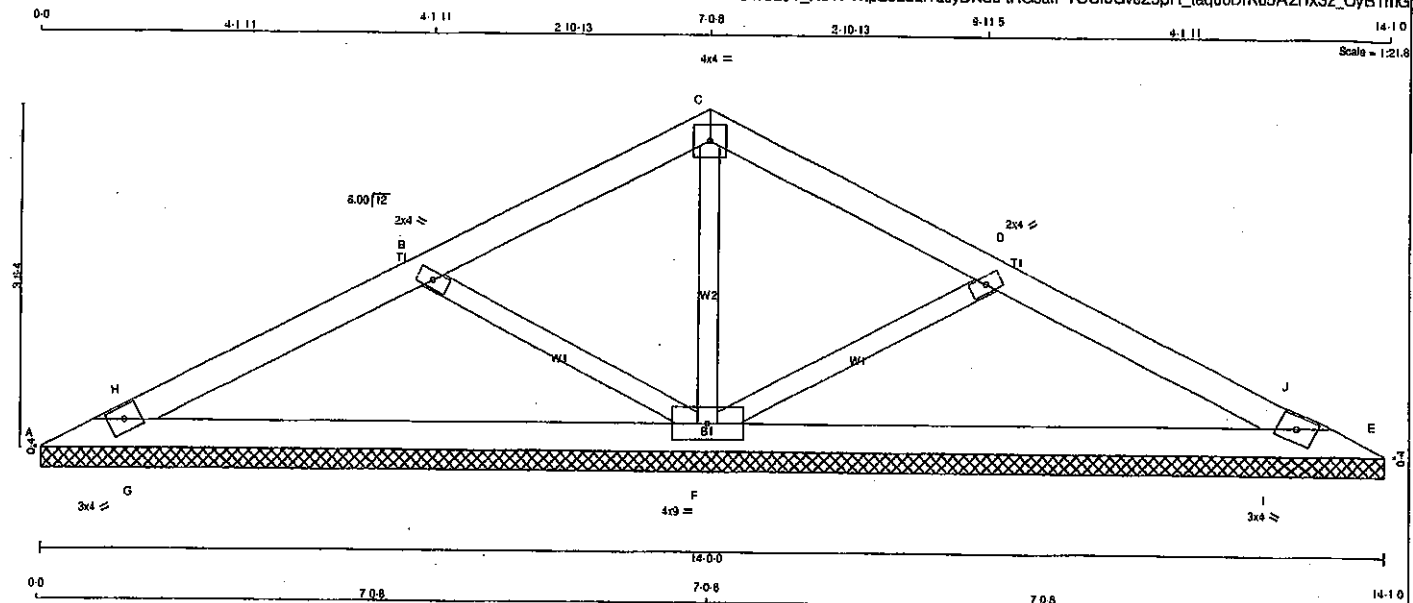
Structural component only
DWG# T-2028127

JOB NAME 414964	TRUSS NAME T8	QUANTITY 17	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
--------------------	------------------	----------------	----------	-------------------------------	-------------	----------

Tamarack Roof Truss, Burlington

Version 8.330 S May 8 2020 MTEK Industries, Inc. Tue Dec 8 12:44:13 2020 Page 1

ID:76L61_X31PWlpZ5adLITzJyDNdu-IHGsaIPYCUi0GveZ5pR_laquoDrKu5Azhx3z_OyB1mG



TOTAL WEIGHT = 17 X 40 = 681 LBS

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF				
DRY: SEASONED LUMBER				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TBM1-h	MT20	3.0	4.0	
B	TMW-w	MT20	2.0	4.0	
C	TTW-p	MT20	4.0	4.0	
D	TMW-w	MT20	2.0	4.0	
E	TBM1-h	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	9.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		RECORD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
A	144	0	144	0	0	14-0-0	14-0-0
E	144	0	144	0	0	14-0-0	14-0-0
F	1256	0	1256	0	0	14-0-0	14-0-0

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	100	74	0	0	0	26	0
E	100	74	0	0	0	26	0
F	891	588	0	0	0	323	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-H	0 115	-91.8 -91.8	0.12 (4)	10.00	F-C	-674 0	0.14 (1)
H-B	0 179	-91.8 -91.8	0.21 (1)	10.00	F-D	-418 0	0.10 (1)
B-C	0 529	-91.8 -91.8	0.27 (1)	10.00	B-F	-416 0	0.10 (1)
C-D	0 529	-91.8 -91.8	0.27 (1)	10.00	G-H	-22 56	0.00 (1)
D-J	0 179	-91.8 -91.8	0.21 (1)	10.00	I-J	-22 56	0.00 (1)
J-E	0 115	-91.8 -91.8	0.12 (4)	10.00			
A-G	-139 0	-18.5 -18.5	0.08 (1)	6.25			
G-F	-132 0	-18.5 -18.5	0.19 (4)	6.25			
F-I	-132 0	-18.5 -18.5	0.19 (4)	6.25			
I-E	-139 0	-18.5 -18.5	0.08 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 20.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.8 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2016, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.27/1.00 (B-C:1), BC=0.19/1.00 (F-I:4), WB=0.14/1.00 (C-F:1), SS=0.15/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	818	354	1667 768 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (C) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)



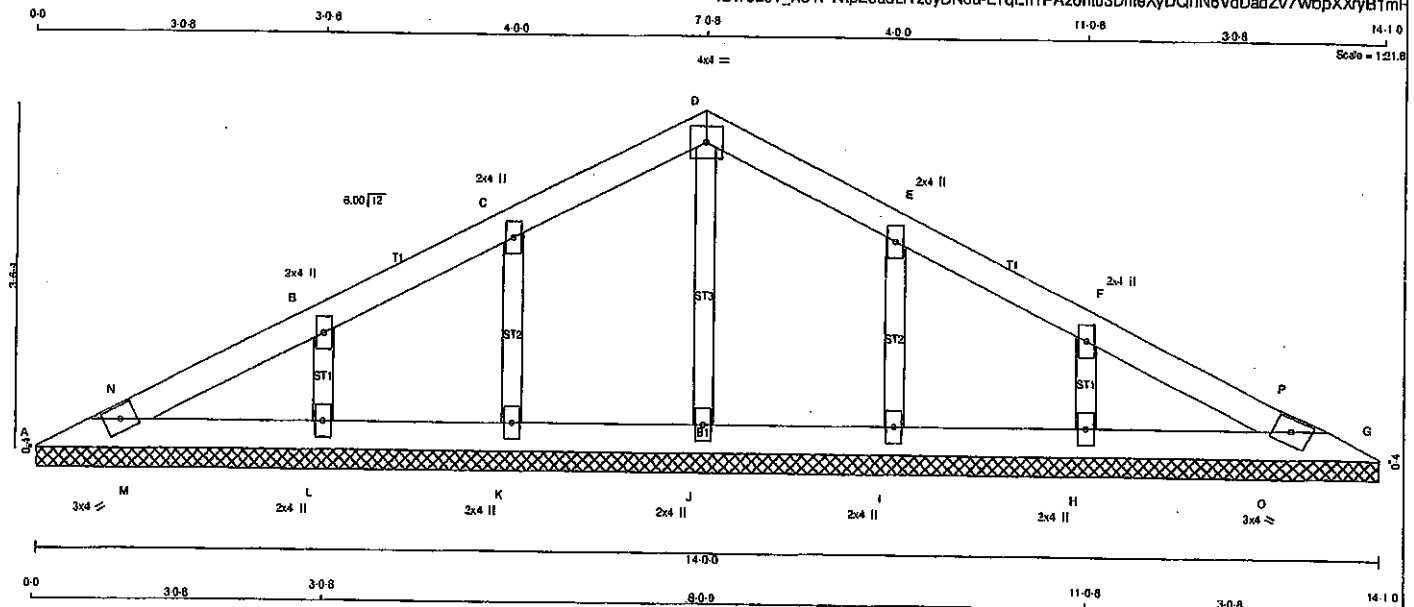
Structural component only
DWG# T-2028128

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
414964	T8G	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.330 5 May 6 2020 Mitek Industries, Inc. Tue Dec 8 12:44:14 2020 Page 1

ID:26L61_X31PWtpZ5adLITzJyDNDu-LTqEn?PAZonlu3DmeXyDQnN6VdDadZv7wbpXxyB1mf



TOTAL WEIGHT = 2 X 40 = 79 lb

LUMBER	SIZE	DRY	NO.2	DESCR.
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF

ALL WEBS: 2x3 DRY No.2 SPF
ALL GABLE WEBS: 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B, C, E, F						
B	TBW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0		
G	TBM1-h	MT20	3.0	4.0		
H, I, J, K, L						
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSF (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSF (LC)	
FR-TO		FROM TO		FR-TO			
A-N	0.27	-91.8 -91.8 0.04 (1)	10.00	J-D	-207.0	0.04 (1)	
N-B	0.66	-91.8 -91.8 0.08 (1)	10.00	K-C	-193.0	0.03 (1)	
B-C	0.48	-91.8 -91.8 0.08 (1)	10.00	I-E	-193.0	0.03 (1)	
C-D	0.60	-91.8 -91.8 0.08 (1)	10.00	H-F	-222.0	0.03 (1)	
D-E	0.50	-91.8 -91.8 0.08 (1)	10.00	L-B	-222.0	0.03 (1)	
E-F	0.48	-91.8 -91.8 0.08 (1)	10.00	M-N	-85.2	0.00 (1)	
F-P	0.56	-91.8 -91.8 0.08 (1)	10.00	O-P	-85.2	0.00 (1)	
P-G	0.27	-91.8 -91.8 0.04 (1)	10.00				
A-M	-40.0	-18.5 -18.5 0.07 (1)	6.25				
M-L	-34.0	-18.5 -18.5 0.07 (1)	6.25				
L-K	-46.0	-18.5 -18.5 0.04 (1)	6.25				
K-J	-52.0	-18.5 -18.5 0.02 (4)	6.25				
J-I	-52.0	-18.5 -18.5 0.02 (4)	6.25				
I-H	-46.0	-18.5 -18.5 0.04 (1)	6.25				
H-O	-34.0	-18.5 -18.5 0.07 (1)	6.25				
O-G	-40.0	-18.5 -18.5 0.07 (1)	6.25				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT. CH. LL = 0.0 PSF

DE = 1.447.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, OBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSF: TC=0.08/1.00 (F-P:1), BC=0.07/1.00 (H-O:1), WB=0.04/1.00 (D-J:1), SS=0.09/1.00 (F-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
	MAX	MIN	MAX
MT20	618	354	1667
	788	1987	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)

JSI METAL= 0.09 (F) (INPUT = 1.00)

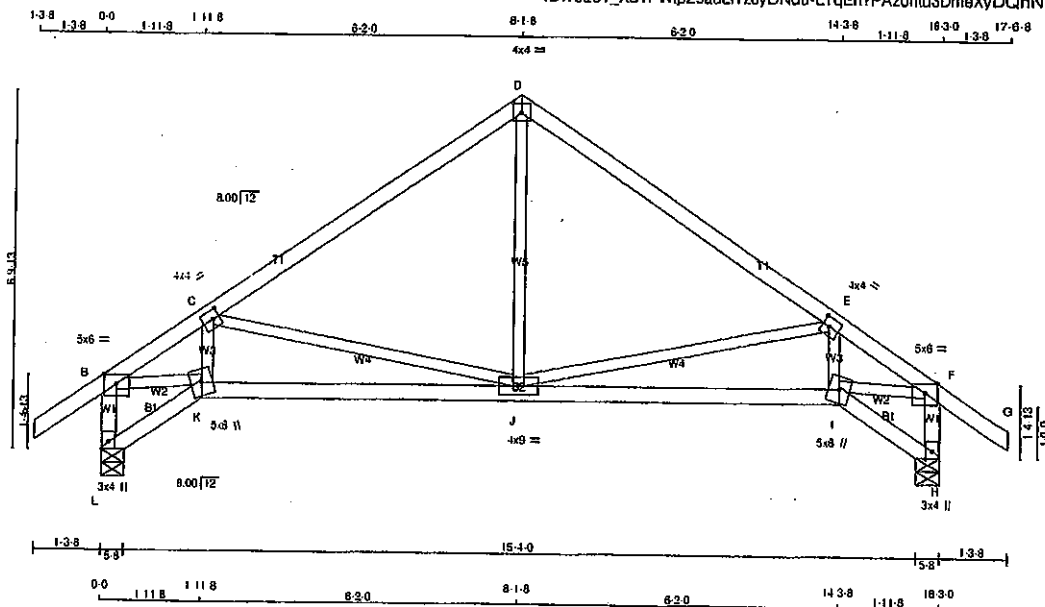


Structural component only
DWG# T-2028129

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
414964	T9	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.330 S May 8 2020 MTek Industries, Inc. Tue Dec 8 12:44:14 2020 Page 1
ID:76L61_X31PWpZ5adLTzJyDNdu-LTqEn7PAzontu3DmeXyDQnN1Ed9UdSI7wbpXXryB1mF



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
L - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	6.0	Edge
C	TMWW-I	MT20	4.0	4.0	2.00 1.50
D	TTW-p	MT20	4.0	4.0	2.25 2.00
E	TMWW-I	MT20	4.0	4.0	2.00 1.50
F	TMVW-p	MT20	5.0	6.0	Edge
H	BMV1-p	MT20	3.0	4.0	Edge
I	BBWW-m	MT20	5.0	6.0	
J	BMWW-m	MT20	4.0	6.0	
K	BBWW-m	MT20	5.0	6.0	
L	BMV1-p	MT20	3.0	4.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
L	1022	0	1022	0	5-8
H	1022	0	1022	0	5-8

UNFACTORED REACTIONS		MAX/MIN. COMPONENT REACTIONS		1ST LCASE	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
L	720	488	0	0	0
H	720	488	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
L-B	-1004 0	0.0 0.0	0.11 (1)	J-D	0 444	0.10 (1)	
A-B	0 35	-91.8 -91.8	0.12 (1)	J-E	-861 0	0.47 (1)	
B-C	-1510 0	-91.8 -91.8	0.33 (1)	I-E	0 56	0.02 (4)	
C-D	-841 0	-91.8 -91.8	0.42 (1)	I-F	0 1322	0.30 (1)	
D-E	-841 0	-91.8 -91.8	0.42 (1)	C-J	-861 0	0.47 (1)	
E-F	-1510 0	-91.8 -91.8	0.33 (1)	K-C	0 56	0.02 (4)	
F-G	0 35	-91.8 -91.8	0.12 (1)	B-K	0 1322	0.30 (1)	
H-F	-1004 0	0.0 0.0	0.11 (1)				
L-K	0 0	-18.5 -18.5	0.02 (4)	10.00			
K-J	0 1320	-18.5 -18.5	0.33 (1)	10.00			
J-I	0 1320	-18.5 -18.5	0.33 (1)	10.00			
I-H	0 0	-18.5 -18.5	0.02 (4)	10.00			

TOTAL WEIGHT = 3 X 67 = 201 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (0.54")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.03")
ALLOWABLE DEFL. (TL) = 1/360 (0.54")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.08")

CSI: TC=0.42/1.00 (D-E:1), BC=0.33/1.00 (J-I:1), WB=0.47/1.00 (E-J:1), SS=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP (DRY)	SHEAR	SECTION	
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1867
	788	1907	1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.47 (I) (INPUT = 1.00)



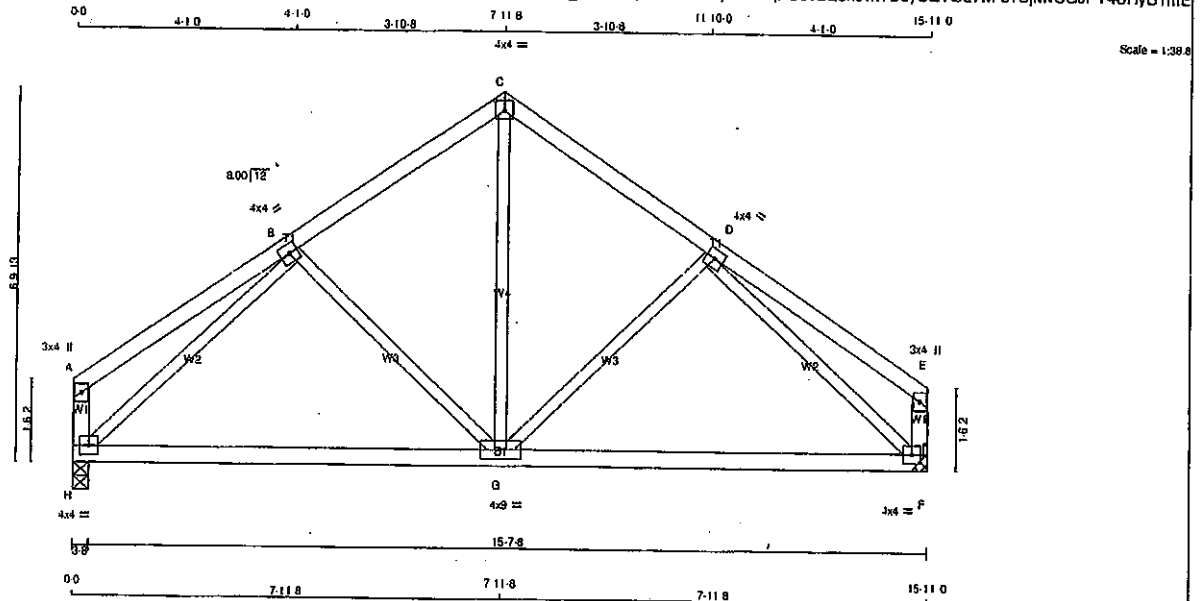
Structural component only
DWG# T-2028130

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
414964	T10	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MTek Industries, Inc. Tue Dec 8 12:44:15 2020 Page 1

ID:76L61_X31PWpZ5adLITzJyDNdu-pfOc?LQok6vkvDoyCETSz?wFstUjMvOG9FY43HyB1mE



TOTAL WEIGHT = 3 X 65 = 194 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - A	2x4	DRY	No.2
F - E	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
A	TMV-p	MT20	3.0	4.0	
B	TMVW-1	MT20	4.0	4.0	
C	TTW-p	MT20	4.0	4.0	2.25 2.00
D	TMVW-1	MT20	4.0	4.0	
E	TMV-p	MT20	3.0	4.0	
F	BMVW-1	MT20	4.0	4.0	
G	BMVW-1	MT20	4.0	4.0	
H	BMVW-1	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
H	877	0	877	0	3-8
F	877	0	877	0	3-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	621	407.0	0.0	0.0	0.0	213.0	0.0
F	621	407.0	0.0	0.0	0.0	213.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	FR-TO
A-B	0.23	-91.8 -91.8 0.23 (1)	10.00 G-C
B-C	-895.0	-91.8 -91.8 0.18 (1)	6.25 G-D
C-D	-895.0	-91.8 -91.8 0.18 (1)	6.25 B-G
D-E	0.23	-91.8 -91.8 0.23 (1)	10.00 H-B
H-A	-140.0	0.0 0.0 0.01 (1)	7.81 D-F
F-E	-140.0	0.0 0.0 0.01 (1)	7.81
H-G	0.705	-18.5 -18.5 0.39 (4)	10.00
G-F	0.705	-18.5 -18.5 0.39 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL.	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.23/1.00 (D-E:1), BC=0.39/1.00 (F-G:4),
WB=0.49/1.00 (D-F:1), SS=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	618	354
	1667	788
	1597	1656

PLATE PLACEMENT TOL. = 0.250 inches

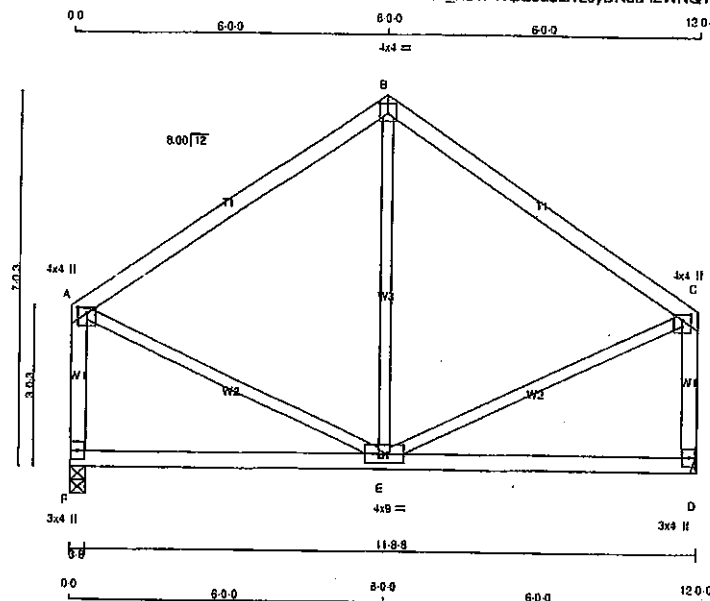
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (D) (INPUT = 0.90)
JSI METAL= 0.35 (B) (INPUT = 1.00)



Structural component only
DWG# T-2028131

JOB NAME 414964	TRUSS NAME T12	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.330 S May 8 2020 Mtek Industries, Inc. Tue Dec 8 12:44:17 2020 Page 1			
		ID:76L61_X31PWtpZ5adLITzJyDNdu-I2WNQ1S3Gj9SIWYlKlWw2Q?YKqEJqvVZcZ1B79yB1mC			



Scale = 1:30.8

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - B 2x4 DRY No.2

B - C 2x4 DRY No.2

F - A 2x4 DRY No.2

D - C 2x4 DRY No.2

F - D 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTW+p	MT20	4.0	4.0	2.25	2.00
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
O	BMV1+p	MT20	3.0	4.0		
E	BMVWW-I	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	662	0	662	0
D	662	0	662	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
F	468	307	0	0	0	0	161	0
D	468	307	0	0	0	0	161	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSF (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSF (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-371	0	-31.8 -91.8 0.42 (1)	E-B	-139	52	0.12 (1)
B-C	-371	0	-31.8 -91.8 0.42 (1)	A-E	0	338	0.08 (1)
F-A	-619	0	0.0 0.0 0.10 (1)	E-C	0	338	0.08 (1)
D-C	-619	0	0.0 0.0 0.10 (1)				
F-E	0	0	-18.5 -18.5 0.19 (4)				
E-D	0	0	-18.5 -18.5 0.19 (4)				

TOTAL WEIGHT = 52 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. G.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/899 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.42/1.00 (A-B:1), BC=0.19/1.00 (D-E:4),
WB=0.12/1.00 (B-E:1), GSI=0.18/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.43 (C) (INPUT = 0.90)
JSI METAL = 0.13 (C) (INPUT = 1.00)

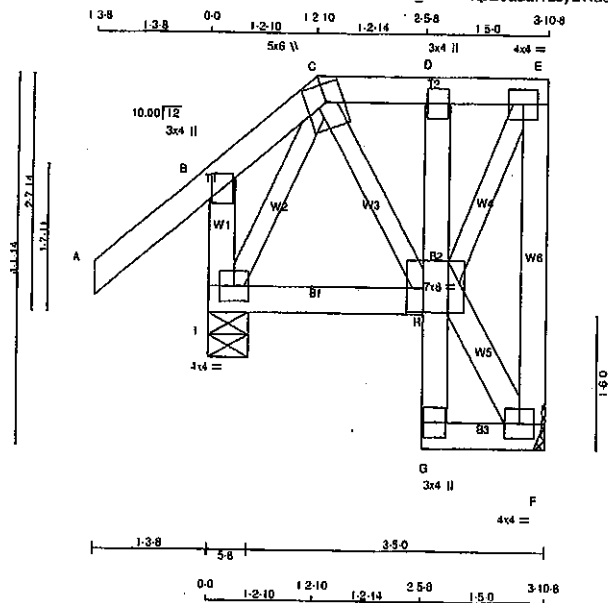


Structural component only
DWG# T-2028133

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
414964	T13	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.330 S May 8 2020 MTEx Industries, Inc. Tue Dec 8 12:44:17 2020 Page 1
ID:76L61_X31PWpZ5adLITzJyDNdu-l2VWNQ1S3G9SIWYKfWw2C?cJqGgqwpZcZ1B79yB1mC



TOTAL WEIGHT = 2 X 29 = 57 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - E	2x4 DRY	No.2	SPF		
F - E	2x4 DRY	No.2	SPF		
I - B	2x4 DRY	No.2	SPF		
I - H	2x4 DRY	No.2	SPF		
G - D	2x4 DRY	No.2	SPF		
G - F	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					
H - F	2x4 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
C	TTW-W-m	MT20	5.0	6.0	2.50	2.25
D	TMV-p	MT20	3.0	4.0		
E	TMV-W-1	MT20	4.0	4.0		
F	BMV-W-1	MT20	4.0	4.0		
G	BMV-p	MT20	3.0	4.0		
H	BVMW-W-1	MT20	7.0	8.0	3.25	2.25
I	BMV-W-1	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	191	0	191	0
I	363	0	363	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1'-8".

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
F	135	88	13	0	0	0	49	0
I	254	183	0	0	0	0	72	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 : 41	-91.8	-91.8 0.14 (5)	10.00	H-F	-6 : 0	0.00 (1)
B-C	-44 : 0	-91.8	-91.8 0.14 (5)	8.25	H-E	0 : 141	0.03 (1)
C-D	-69 : 0	-91.8	-91.8 0.02 (1)	6.25	I-C	-96 : 27	0.02 (1)
D-E	-67 : 0	-91.8	-91.8 0.02 (1)	6.25	C-H	0 : 48	0.01 (1)
F-E	-175 : 0	0.0	0.0 0.05 (1)	7.81			
I-B	-255 : 0	0.0	0.0 0.03 (1)	7.81			
I-H	-13 : 45	-18.5	-18.5 0.04 (4)	8.25			
G-H	0 : 13	0.0	0.0 0.01 (1)	10.00			
H-D	-135 : 0	0.0	0.0 0.01 (1)	7.81			
G-F	0 : 4	-18.5	-18.5 0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/399 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CS1: TC=0.14/1.00 (A-B:5), BC=0.04/1.00 (H-I:4), WB=0.03/1.00 (E-H:1), SS=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.18 (E) (INPUT = 0.90)
JSI METAL = 0.09 (B) (INPUT = 1.00)

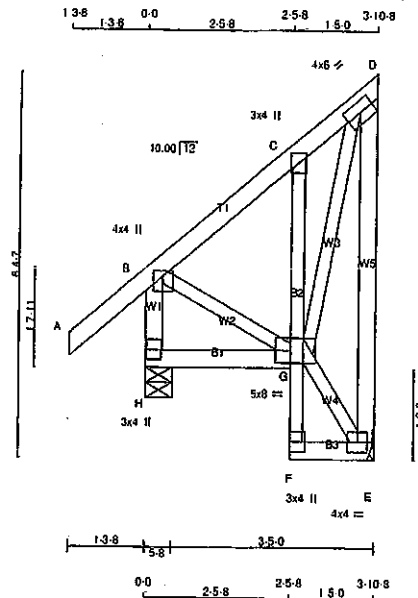


Structural component only
DWG# T-2028134

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
414964	T15	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.330 3 May 6 2020 MTek Industries, Inc. Tue Dec 8 12:44:19 2020 Page 1
ID:76L61_X31PWpZ5adLITzJyDNdu-iQe7qTJnKQ8_q5JR4YO7q4x1eyDlQ0e4tWIC2yB1mA



Scale = 1:35.1

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
F - C	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW+p	MT20	3.0	4.0		
D	TMVW+p	MT20	4.0	6.0	2.00	3.00
E	BMVW+p	MT20	4.0	4.0		
F	BMVW+p	MT20	3.0	4.0		
G	BMVW+p	MT20	5.0	8.0	2.50	3.00
H	BMVW+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED			MAXIMUM FACTORED			INPUT	REQD
GROSS REACTION			GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	214	0	214	0	0	MECHANICAL	5-8
H	341	0	341	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	151	99.0	0.0	0.0	0.0	52.0	0.0
H	239	170.0	0.0	0.0	0.0	69.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LCI (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LCI (LBS)
FR-TO						FR-TO			
A-B	0.41	-91.8	-91.8	0.14 (5)	10.00	G-E	-4.0	0.00 (1)	
B-C	-80.0	-91.8	-91.8	0.07 (1)	6.25	G-D	0.223	0.05 (1)	
C-D	-99.0	-91.8	-91.8	0.06 (1)	6.25	B-G	0.78	0.02 (1)	
E-D	-198.0	0.0	0.0	0.18 (1)	7.81				
H-B	-317.0	0.0	0.0	0.03 (1)	7.81				
H-G	0.0	-18.5	-18.5	0.04 (4)	10.00				
F-G	0.14	0.0	0.0	0.01 (1)	10.00				
G-C	-217.0	0.0	0.0	0.02 (1)	7.81				
F-E	0.3	-18.5	-18.5	0.01 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 31 = 62 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TO=0.18/1.00 (D-E:1), BC=0.04/1.00 (G-H:4), WB=0.05/1.00 (D-G:1), SS=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

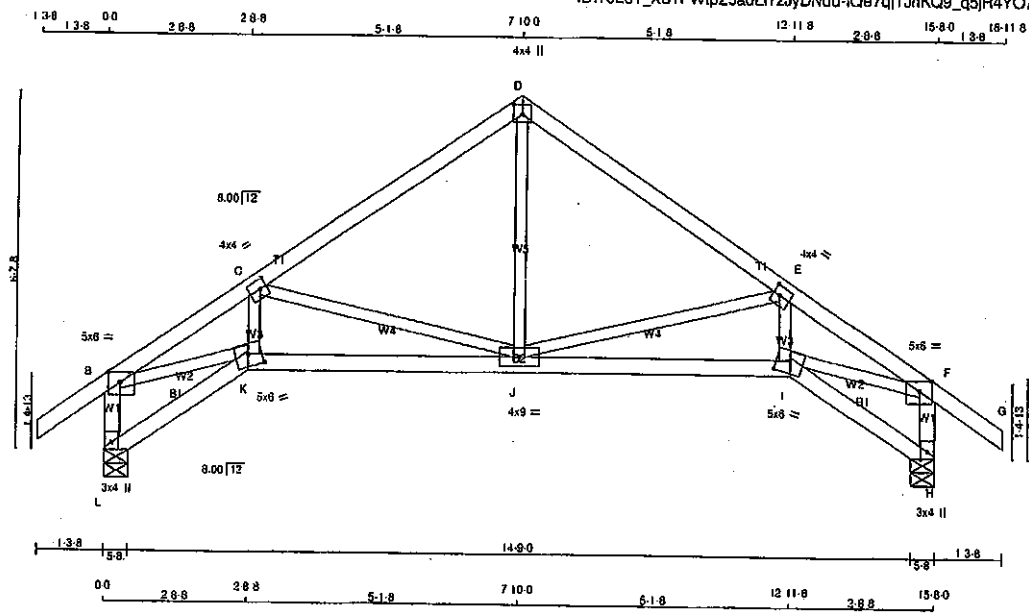
JSI GRIP = 0.25 (D) (INPUT = 0.90)
JSI METAL = 0.08 (G) (INPUT = 1.00)



Structural component only
DWG# T-2028136

JOB NAME 414964	TRUSS NAME T16	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.330 S May 6 2020 Mitek Industries, Inc. Tue Dec 8 12:44:19 2020 Page 1
ID:76L61_X31PWipZ5adLITzJyDNdu-iQe7qITJnKQ9_q5JR4YO7q4vretmilus4tWIC2yB1mA



Scale = 1/32"

TOTAL WEIGHT = 65 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
L - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY, SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
GROSS REACTION		GROSS REACTION		BRG	BRG	BRG	BRG
JT VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX
L 990	0	990	0	0	5-8	5-8	5-8
H 990	0	990	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS	
1ST LCASE		SNOW	LIVE
JT COMBINED			
L 697	471.0	0.0	0.0
H 697	471.0	0.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO	FROM	FR-TO	FROM
L-B	-965.0	J-D	0.564
A-B	0.35	J-E	-818.0
B-C	-1798.0	I-E	0.311
C-D	-880.0	I-F	0.1544
D-E	-880.0	C-J	-818.0
E-F	-1798.0	B-K	0.311
F-G	0.735		
H-F	-965.0		
L-K	0.0		
K-J	0.1502		
J-I	0.1502		
I-H	0.0		

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMW-p	MT20	5.0	6.0	2.25 2.75
C TMW-w	MT20	4.0	4.0	2.00 1.50
D TTW-p	MT20	4.0	4.0	2.25 2.00
E TMW-w	MT20	4.0	4.0	2.00 1.50
F TMW-p	MT20	5.0	6.0	2.25 2.75
H BMV-p	MT20	3.0	4.0	Edge
J BBWW-m	MT20	5.0	6.0	2.75 3.00
K BBWW-w	MT20	4.0	9.0	
L BMV-p	MT20	5.0	6.0	2.75 3.00
L BMV-p	MT20	3.0	4.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	25.6 PSF
DL	6.0 PSF
BOT. CH. LL	0.0 PSF
DL	7.4 PSF
TOTAL LOAD	39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.30/1.00 (D-E:1), BC=0.32/1.00 (I-J:1), WB=0.30/1.00 (E-J:1), SS=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MT20	618	354	1667
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

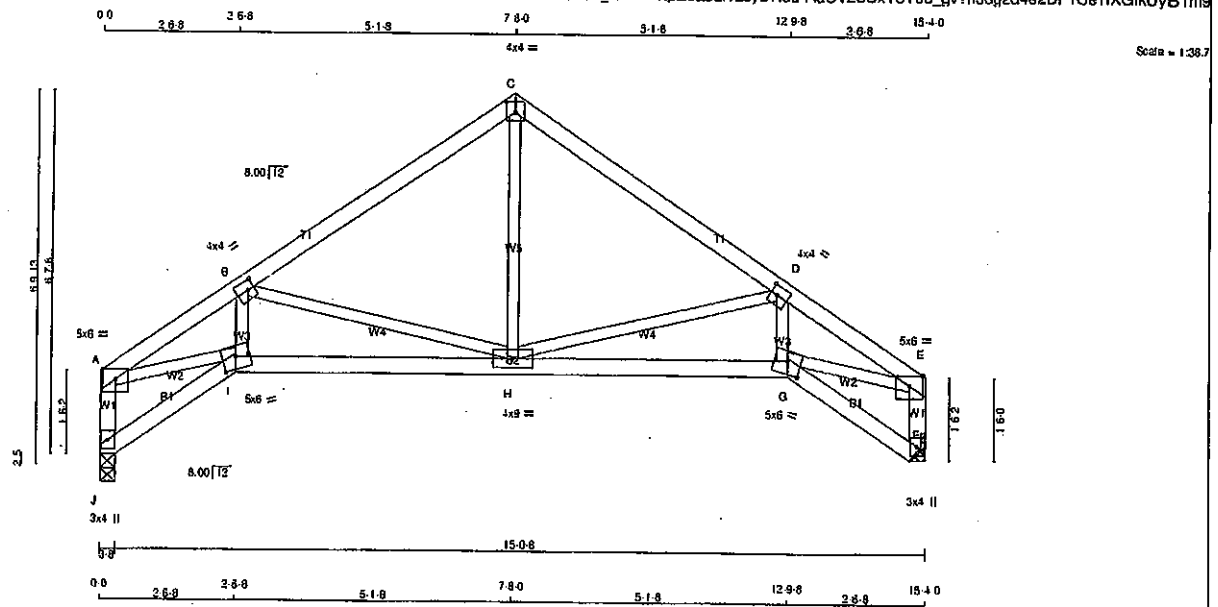
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (K) (INPUT = 0.90)
JSI METAL= 0.98 (K) (INPUT = 1.00)



Structural component only
DWG# T-2028137

JOB NAME 414964	TRUSS NAME T16A	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.330 S May 6 2020 Mitek Industries, Inc. Tue Dec 8 12:44:20 2020 Page 1		ID:76L61_X31PWlpZ5adLITzJyDNdu-AdCV23UxYsY0c_gv?n3dp2d4e2DF1C67IXGikUyB1m9	
		TRUSS DESC.			



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
J - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - J	2x4	DRY	No.2
J - F	2x4	DRY	No.2
F - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
D - H	2x4	DRY	No.2
H - G	2x4	DRY	No.2
G - F	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER.			

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN	X
A - TMVW-p	MT20	5.0	8.0	Edge
B - TMVW-t	MT20	4.0	4.0	2.00 1.50
C - TTVW-p	MT20	4.0	4.0	2.25 2.00
D - TMVW-t	MT20	4.0	4.0	2.00 1.50
E - TMVW-p	MT20	5.0	8.0	Edge
F - BMV1+p	MT20	3.0	4.0	
G - BBWW-m	MT20	5.0	5.0	2.75 3.00
H - BBWW-t	MT20	4.0	9.0	
I - BBWW-m	MT20	5.0	8.0	2.75 3.00
J - BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS	REACTION	GROSS	REACTION	BRG	BRG	IN-SX	IN-SX
J	845	0	845	0	0	3-8	3-8		
F	845	0	845	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM.LIVE
J	598	392.0	0.0	0.0
F	598	392.0	0.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
J-A	-822.0	A-I	0.1434
A-B	-1673.0	I-B	0.245
B-C	-845.0	B-H	-748.0
C-D	-845.0	H-C	0.526
D-E	-1673.0	E-D	-748.0
E-F	-822.0	F-G	0.245
J-I	0.0	G-E	0.1434
I-H	0.1405		
H-G	0.1405		
G-F	0.0		

TOTAL WEIGHT = 61 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
TOP CH.	DL	=	6.0	PSF
BOY CH.	LL	=	0.0	PSF
BOY CH.	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.29/1.00 (C-D:1), BC=0.30/1.00 (G-H:1), WB=0.35/1.00 (D-H:1), SS=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX	MIN	MAX MIN
MT20	618	354 1667 788 1887 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (C) (INPUT = 0.90)
JSI METAL = 0.35 (G) (INPUT = 1.00)



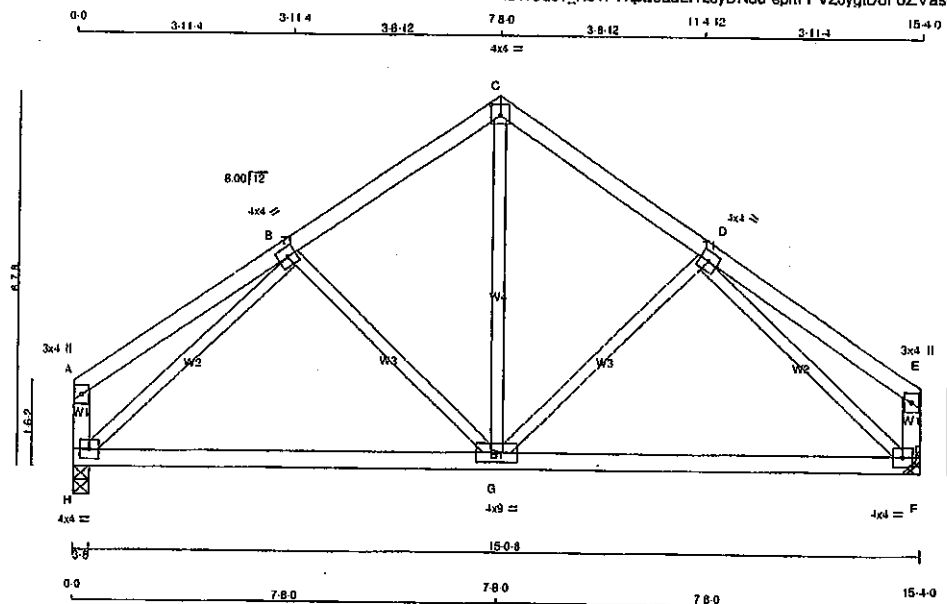
Structural component only
DWG# T-2028138

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
414964	T17	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 Mtek Industries, Inc. Tue Dec 8 12:44:21 2020 Page 1

ID:76L61_X31PWtpZ5adLITzJyDNdu-epItFPVZJyglD8F6ZVasCFAGcRYZmeX9XB7OGxyB1m8



Scale = 1:37.7

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - E	2x4 DRY	No.2	SPF		
H - A	2x4 DRY	No.2	SPF		
F - E	2x4 DRY	No.2	SPF		
H - F	2x4 DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - TMV+p	MT20	3.0	4.0		
B - TMWW-I	MT20	4.0	4.0		
C - TTW-p	MT20	4.0	4.0	2.25	2.00
D - TMWW-I	MT20	4.0	4.0		
E - TMV+p	MT20	3.0	4.0		
F - BMWW-I	MT20	4.0	4.0		
G - BMWW-I	MT20	4.0	9.0		
H - BMWW-I	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	HORIZ	UPLIFT
H 845 0	845 0	0 0	3-8 3-8
F 845 0	845 0	0 0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERMLIVE WIND DEAD SOIL
H 598 392 0 0 0 0 0 0	205 0 0 0
F 598 392 0 0 0 0 0 0	205 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)
FR-TO									
A-B	0 22	-91.8	-91.8	0.21 (1)	10.00	G-C	0 440	0.10 (1)	
B-C	-665 0	-91.8	-91.8	0.17 (1)	6.25	G-D	-190 0	0.09 (1)	
C-D	-665 0	-91.8	-91.8	0.17 (1)	6.25	B-G	-190 0	0.09 (1)	
D-E	0 22	-91.8	-91.8	0.21 (1)	10.00	H-B	-938 0	0.43 (1)	
H-A	-135 0	0.0	0.0	0.01 (1)	7.81	D-F	-938 0	0.43 (1)	
F-E	-135 0	0.0	0.0	0.01 (1)	7.81				
H-G	0 671	-18.5	-18.5	0.36 (4)	10.00				
G-F	0 671	-18.5	-18.5	0.36 (4)	10.00				

TOTAL WEIGHT = 2 X 63 = 125 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.21/1.00 (A-B:1), BC=0.36/1.00 (F-G:4), WB=0.43/1.00 (D-F:1), SS=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

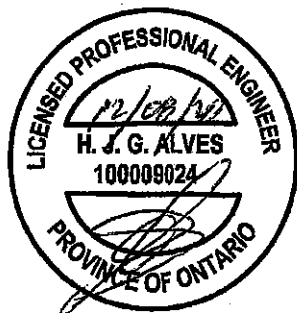
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 768	1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.81 (B) (INPUT = 0.80)
JSI METAL= 0.33 (B) (INPUT = 1.00)

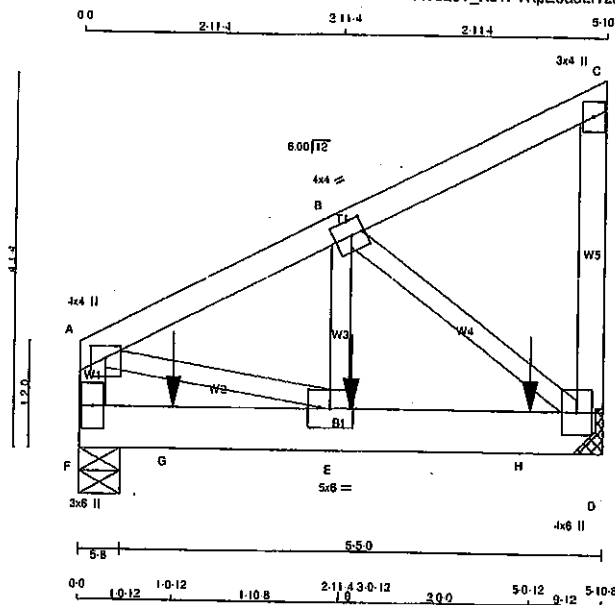


Structural component only
DWG# T-2028139

JOB NAME 414964	TRUSS NAME T18	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
---------------------------	--------------------------	----------------------	-----------------	--------------------------------------	-------------	----------

Tamarack Roof Truss, Burlington

Version 8.330 5 May '8 2020 Mitek Industries, Inc. Tue Dec 8 12:44:22 2020 Page 1
ID:76L61_X31PWtpZ5adLITzJyDNdu-67JGTRWB4Fokrlq6C65ITUnrwV9ilm4ypNyB1m7



Scale = 1/2\"

TOTAL WEIGHT = 2 X 29 = 58 lb

CHORDS	SIZE	DRY	NO.2	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122\"X3\") SPIRAL NAILS		
A - G	12	TOP
C - D	12	TOP
F - A	12	TOP
BOTTOM CHORDS : (0.122\"X3\") SPIRAL NAILS		
F - D	12	SIDE(183.1)
WEBS : (0.122\"X3\") SPIRAL NAILS		
B - E	8	SIDE(32.6)
2x3	1	8

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW+p	MT20	4.0	4.0	1.50	2.00
B TMVW-1	MT20	4.0	4.0	2.00	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORIZ	DOWN	HORIZ
D	1601	0	1601
F	1529	0	1529

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LGASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	1129	755	0	0	0	0	374	0
F	1079	722	0	0	0	0	357	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LG1 MAX (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO									
A-B	-1348	0	-91.8	-91.8	0.06 (1)	6.25	E-B	0	1219
B-C	-11	0	-91.8	-91.8	0.05 (1)	6.25	B-D	-1529	0
D-C	-111	0	0.0	0.0	0.01 (1)	7.81	A-E	0	1257
F-A	-1039	0	0.0	0.0	0.06 (1)	7.81			
F-G	0	0	-18.5	-18.5	0.19 (1)	10.00			
G-E	0	0	-18.5	-18.5	0.19 (1)	10.00			
E-H	0	1215	-18.5	-18.5	0.24 (1)	10.00			
H-D	0	1215	-18.5	-18.5	0.24 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LG1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-0-12	-583	-583	---	FRONT	VERT	TOTAL	---	C1
G	1-0-12	-583	-583	---	FRONT	VERT	TOTAL	---	C1
H	5-0-12	-584	-584	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT. CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CS1: TC=0.06/1.00 (A-B:1), BC=0.24/1.00 (D-E:1), WB=0.18/1.00 (B-D:1), SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
MT20	1667	788
MT20	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (B) (INPUT = 0.90)
JSI METAL= 0.27 (D) (INPUT = 1.00)



Structural component only
DWG# T-2028140 1/1

CONTINUED ON PAGE 2

JOB NAME 414964	TRUSS NAME T18	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.330 S May 6 2020 Mitek Industries, Inc. Tue Dec 8 12:44:22 2020 Page 2
ID:76L61 X31PWtpZ5adLITzJyDNdu-67JGTkWB4Fokriql6C85ITIUnrwfV9ilimrlypNvB1m7

PLATES (table is in inches)

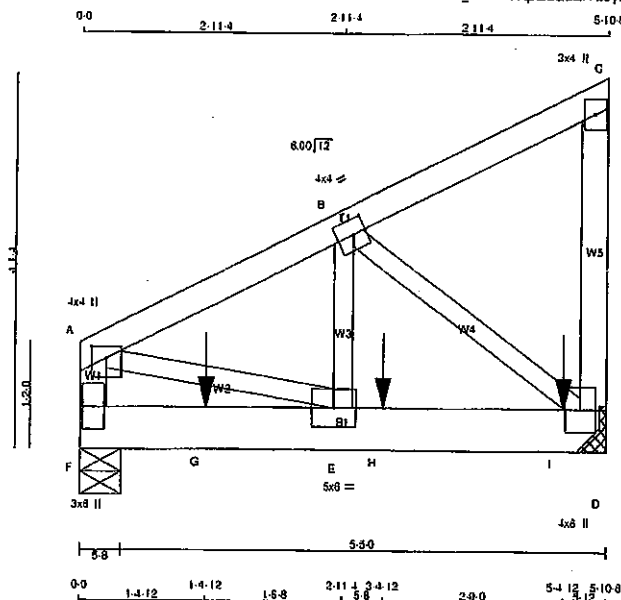
JT	TYPE	PLATES	W	LEN	Y	X
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	8.0		
E	BMVW-1	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		



Structural component only
DWG# T-2028140 812

JOB NAME 414964	TRUSS NAME T18Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.390 S May 6 2020 MTak Industries, Inc. Tue Dec 8 12:44:22 2020 Page 1
ID:76L61_X31PWpZ5adLTzJyDNdu-6?JGKWB4Fokrlq6C65TlUnrw7V9HfmrtpNyB1m7



TOTAL WEIGHT = 2 X 29 = 58 lb

LUMBER	SIZE	DRY	DESCR.
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
C-D	12	SIDE (103.3)
F-A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D	12	SIDE (122.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50 2.00
B	TMVW-i	MT20	4.0	4.0	2.00 1.75
C	TMV+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
D	1818	0	1818	0	MECHANICAL	
F	1412	0	1412	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
D	1283	856	0	0	0	428	0
F	998	666	0	0	0	330	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO		FR-TO				
A-B	-1350 0	-91.8	-91.8	0.06 (1)	6.25	E-B	0 1229	0.15 (1)	
B-C	-11 0	-91.8	-91.8	0.05 (1)	6.25	B-D	-1540 0	0.18 (1)	
D-C	-111 0	0.0	0.0	0.01 (1)	7.81	A-E	0 1266	0.18 (1)	
F-A	-1045 0	0.0	0.0	0.06 (1)	7.81				
F-G	0 0	-18.5	-18.5	0.18 (1)	10.00				
G-E	0 0	-18.5	-18.5	0.18 (1)	10.00				
E-H	0 1224	-18.5	-18.5	0.21 (1)	10.00				
H-I	0 1224	-18.5	-18.5	0.21 (1)	10.00				
I-D	0 1224	-18.5	-18.5	0.21 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-4-12	-806	-806	...	BACK	VERT	TOTAL	...	C1
H	3-4-12	-806	-806	...	BACK	VERT	TOTAL	...	C1
I	5-4-12	-810	-810	...	BACK	VERT	TOTAL	...	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2010
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TO=0.06/1.00 (A-B:1), BC=0.21/1.00 (D-E:1), WB=0.18/1.00 (B-D:1), SSI=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687
MT20	618	354	1687

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.84 (B) (INPUT = 0.80)
JSI METAL = 0.28 (D) (INPUT = 1.00)



Structural component only
DWG# T-2028141

CONTINUED ON PAGE 2

JOB NAME 414964	TRUSS NAME T18Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.330 S May 6 2020 MTek Industries, Inc. Tue Dec 8 12:44:23 2020 Page 2
ID:76L61 X31PWpZ5adLITzJyDNdu-aCieq4WpZw6TRPUjwdKHqFIWFGMEcwR?VUVLpy81m6

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW1-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

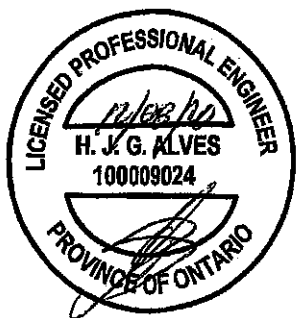


Structural component only
DWG# T-2028141 *mn*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
414964	T18Z1	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		Version 8.330 S May 6 2020 Mitek Industries, Inc. Tue Dec 8 12:44:23 2020 Page 2			
		ID:76L61 X31PWlpZ5adLITzJvDNdu-aCten4WprZwbTRPUJawdKHqFWFG2Ec3R7VUVIovB1m6			
TRUSS DESC.					

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-1	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

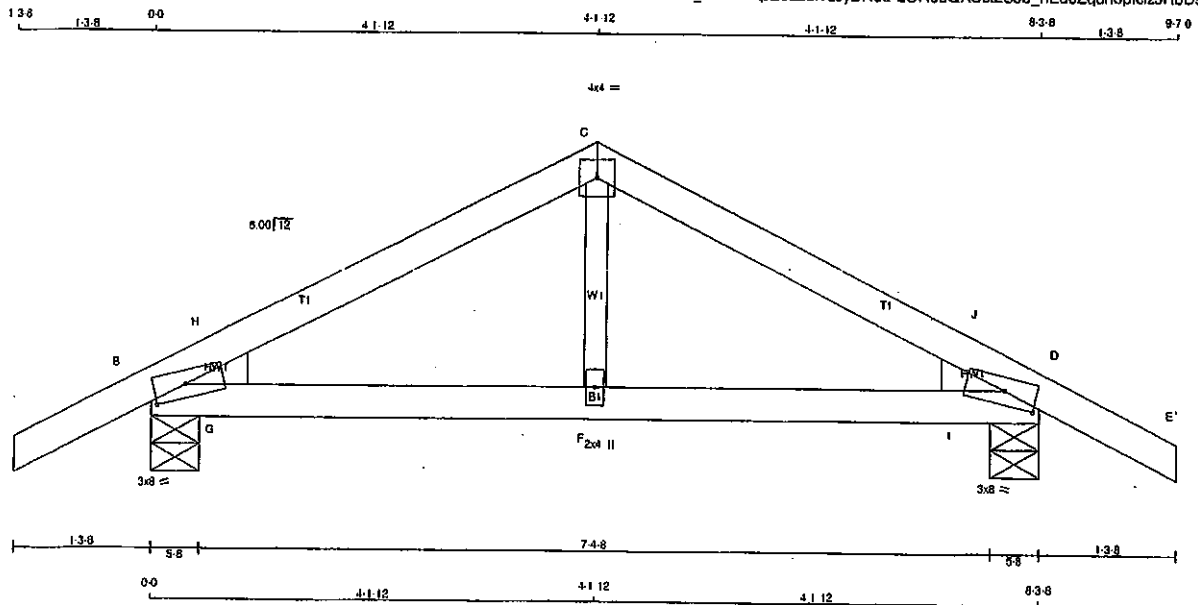


Structural component only
DWG# T-2028142 412

JOB NAME 414964	TRUSS NAME T19	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRAWG NO.
---------------------------	--------------------------	----------------------	-----------------	--------------------------------------	-------------	-----------

Tamarack Roof Truss, Burlington

Version 8.330 \$ May 6 2020 MTEK Industries, Inc. Tue Dec 8 12:44:24 2020 Page 1
ID:76L61_X31PWlpZ5adLITzJyDNdu-2OR0uQXScl2SS5b_hEd8Zqunopclz5RbD9E2tFyB1m5



Scale = 1:10.4

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBH1-m	MT20	3.0	8.0	1.50 3.75
C	TTW-p	MT20	4.0	4.0	
D	TMBH1-m	MT20	3.0	8.0	1.50 3.75
F	BMW-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
GROSS REACTION		GROSS REACTION		BRG	HEEL		
JT VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE
B 582	0	582	0	0	5-8	5-8	2x4 L
D 582	0	582	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	409	282.0	0.0	0.0	0.0	127.0	0.0
D	409	282.0	0.0	0.0	0.0	127.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED (CSI) (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED (CSI) (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 23	-91.8	-91.8 0.12 (1)	10.00	F-C	0 170	0.04 (1)
B-H	-539.0	-91.8	-91.8 0.04 (1)	6.25	G-H	-158.3	0.00 (1)
H-C	-525.0	-91.8	-91.8 0.15 (1)	6.25	I-J	-158.3	0.00 (1)
C-J	-525.0	-91.8	-91.8 0.15 (1)	6.25			
J-D	-539.0	-91.8	-91.8 0.04 (1)	6.25			
D-E	0 23	-91.8	-91.8 0.12 (1)	10.00			
B-G	0 485	-18.5	-18.5 0.21 (1)	10.00			
G-F	0 485	-18.5	-18.5 0.21 (1)	10.00			
F-I	0 485	-18.5	-18.5 0.21 (1)	10.00			
I-D	0 485	-18.5	-18.5 0.21 (1)	10.00			

TOTAL WEIGHT = 3 X 27 = 80 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.15/1.00 (C-J:1), BC=0.21/1.00 (F-I:1),
WB=0.04/1.00 (G-F:1), SSI=0.12/1.00 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

MAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	818	354	1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.25 (B) (INPUT = 0.80)
JSI METAL = 0.11 (D) (INPUT = 1.00)



Structural component only
DWG# T-2028143

JOB NAME

414964

TRUSS NAME

G19

QUANTITY

1

PLY

1

JOB DESC.

GREEN PARK HOMES

DRWG NO.

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 M&T Industries, Inc. Tue Dec 8 12:44:03 2020 Page 1

ID:76L61_X31PWtpZ5adLITzJyDNdu-ALg4UFHGZQPR3NteVjGdTTQCZBSMYbgVONeRezyB1mC

13.8

13.8

0.0

41.12

41.12

41.12

83.8

13.8

97.0

4x4 =

C

8.00'12"

T1

H

B

A

G

3x4 =

F

2x4 ||

I

3x4 =

J

D

E

13.8

0.0

83.8

83.8

13.8

83.8

Scale = 1:19.4

LUMBER

N. L. G. A. RULES

CHORDS

SIZE

DRY

LUMBER

No.2

DESCR.

SPF

A - C

2x4

DRY

No.2

SPF

C - E

2x4

DRY

No.2

SPF

B - D

2x4

DRY

No.2

SPF

ALL WEBS

2x3

DRY

No.2

SPF

ALL GABLE WEBS

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table in inches)

JT TYPE

PLATES

W

LEN

Y

X

B TMB1-i

MT20

3.0

4.0

Edge

C TTW-p

MT20

4.0

4.0

Edge

D TMB1-i

MT20

3.0

4.0

Edge

F BMW1+w

MT20

2.0

4.0

Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.

MAX. FACTORED FORCE (LBS)

VERT. LOAD (PLF)

LC1

MAX

CS1 (LC)

UNBRAC LENGTH

FR-TO

FR-TO

MEMB.

MAX. FACTORED FORCE (LBS)

MAX

CS1 (LC)

A-B

0 : 23

-91.8

-91.8

0.12 (1)

10.00

F-C

-68 : 7

0.01 (1)

B-H

-120 : 0

-91.8

-91.8

0.11 (1)

6.25

G-H

-401 : 0

0.00 (1)

H-C

-288 : 0

-91.8

-91.8

0.19 (1)

6.25

I-J

-401 : 0

0.00 (1)

C-J

-288 : 0

-91.8

-91.8

0.19 (1)

6.25

J-D

-120 : 0

-91.8

-91.8

0.11 (1)

6.25

D-E

0 : 23

-91.8

-91.8

0.12 (1)

10.00

B-G

0 : 249

-18.5

-18.5

0.17 (1)

10.00

G-F

0 : 249

-18.5

-18.5

0.17 (1)

10.00

F-I

0 : 249

-18.5

-18.5

0.17 (1)

10.00

I-D

0 : 249

-18.5

-18.5

0.17 (1)

10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

PART 9 OF CBC 2018, CBC 2012, ABC 2019

PART 9 OF CBC 2012 (2019 AMENDMENT)

CSA 086-09, CSA 086-14

TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.19/1.00 (C-J:1), BC=0.17/1.00 (F-I:1), WB=0.01/1.00 (C-F:1), SS1=0.31/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 818 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (B) (INPUT = 0.90)

JSI METAL= 0.12 (B) (INPUT = 1.00)

100009024

H. J. G. ALVES

PROVINCE OF ONTARIO

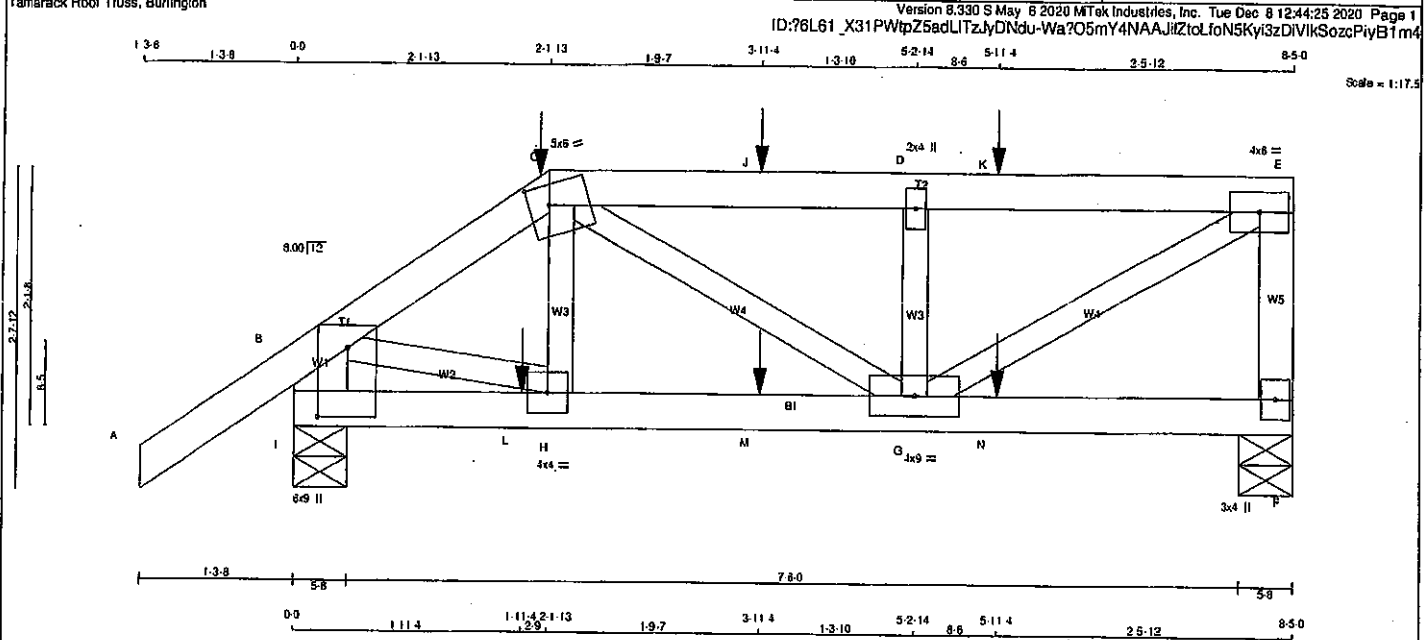
Structural component only

DWG# T-2028117

TOTAL WEIGHT = 25 lb

[M]

JOB NAME 414964	TRUSS NAME T20	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.330 S May 6 2020 MTek Industries, Inc. Tue Dec 8 12:44:25 2020 Page 1			
		ID:76L61_X31PWtpZ5adLITzJyDNdu-Wa?O5mY4NAAJifZioLfoN5Kyiz3zDIVIKSozcPiyB1m4			
		Scale = 1:17.5			



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
I - B	2x6	DRY	No.2
I - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B					
C	TTWW-m	MT20	5.0	6.0	2.00 2.00
D	TMW-w	MT20	2.0	4.0	
E	TMW-l	MT20	4.0	6.0	
F	BMV1-p	MT20	3.0	4.0	
G	BMWW-l	MT20	4.0	9.0	
H	BMWW-l	MT20	4.0	4.0	
I					
J	TMBMW1-p	MT20	6.0	9.0	6.75 3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	873	0	873	0	5-8	5-8
I	876	0	876	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE MAX. MIN. COMPONENT REACTIONS		LIVE	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW					
F	478	314 / 0	0 / 0	0 / 0	0 / 0	161 / 0	0 / 0
I	616	424 / 0	0 / 0	0 / 0	0 / 0	193 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		VERT. LOAD	LC1	MAX	MAX. UNBRACED LENGTH	MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED (PLF)						MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO							FR-TO		
A-B	0 / 35	-91.8	-91.8	0.14 (1)	10.00	H-C	-8	47	0.02 (4)
B-C	-909 / 0	-91.8	-91.8	0.09 (1)	6.25	C-G	0	164	0.04 (1)
C-D	-894 / 0	-91.8	-91.8	0.21 (1)	6.21	G-D	-474	0	0.08 (1)
J-D	-894 / 0	-91.8	-91.8	0.21 (1)	6.21	G-E	0	1033	0.26 (1)
D-K	-895 / 0	-91.8	-91.8	0.21 (1)	6.21	B-H	0	764	0.19 (1)
K-E	-895 / 0	-91.8	-91.8	0.21 (1)	6.21				
F-E	-839 / 0	0.0	0.0	0.08 (1)	7.81				
I-B	-858 / 0	0.0	0.0	0.06 (1)	7.81				
I-L	0 / 0	-18.5	-18.5	0.04 (4)	10.00				
L-H	0 / 0	-18.5	-18.5	0.04 (4)	10.00				
H-M	0 / 753	-18.5	-18.5	0.19 (1)	10.00				
M-G	0 / 753	-18.5	-18.5	0.19 (1)	10.00				
G-N	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
N-F	0 / 0	-18.5	-18.5	0.06 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2'-11"	-15	-15	---	FRONT	VERT	DEAD	---	C1
C	2'-11"	-60	-60	---	FRONT	VERT	TOTAL	---	C1
C	2'-11"	-61	-61	---	FRONT	VERT	SNOW	---	C1
J	3'-11"	-55	-55	---	FRONT	VERT	TOTAL	---	C1
K	5'-11"	-55	-55	---	FRONT	VERT	TOTAL	---	C1
L	1'-11"	-34	-34	---	FRONT	VERT	TOTAL	---	C1
M	3'-11"	-34	-34	---	FRONT	VERT	TOTAL	---	C1
N	5'-11"	-34	-34	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT. NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TO=0.21/1.00 (D-E:1), BC=0.19/1.00 (G-H:1), WB=0.26/1.00 (E-G:1), SS=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	818	354	1687 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.84 (H) (INPUT = 0.80)

JSI METAL = 0.28 (H) (INPUT = 1.00)



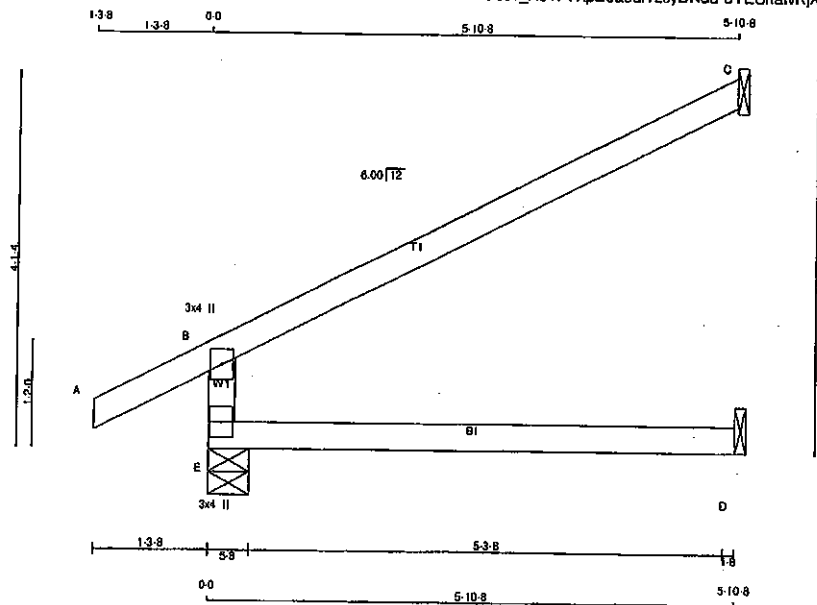
Structural component only
DWG# T-2028144

JOB NAME 414964	TRUSS NAME J1	QUANTITY 12	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.330 S May 6 2020 Mitek Industries, Inc. Tue Dec 8 12:44:04 2020 Page 1

ID:76L61_X31PWtpZ5adLTzJyDNdu-sYEShalvK(X)hXSr3Qns0gyHpbodH24ed1O7AQyB1mP

Scale = 1:23.3



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY, SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
E	525	0	525	0	5-8	5-8
C	202	0	202	0	1-8	1-8
D	45	0	45	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	369	257 0	0 0	0 0	0 0	111 0	0 0
C	139	113 0	0 0	0 0	0 0	28 0	0 0
D	36	0 0	0 0	0 0	0 0	38 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	UNBRAC LENGTH FR-TO	MEMB. FORCE (LBS)	MAX CS1 (LC)
E-B	-461	0	0.0	0.0	0.13 (4)	7.81		
A-B	0	28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-30	0	-91.8	-91.8	0.54 (1)	6.25		
E-D	0	0	-18.5	-18.5	0.13 (4)	10.00		

TOTAL WEIGHT = 12 X 17 = 202 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 39.0 PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CS1: TC=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (A-B:0), SS1=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	618	354	1867 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

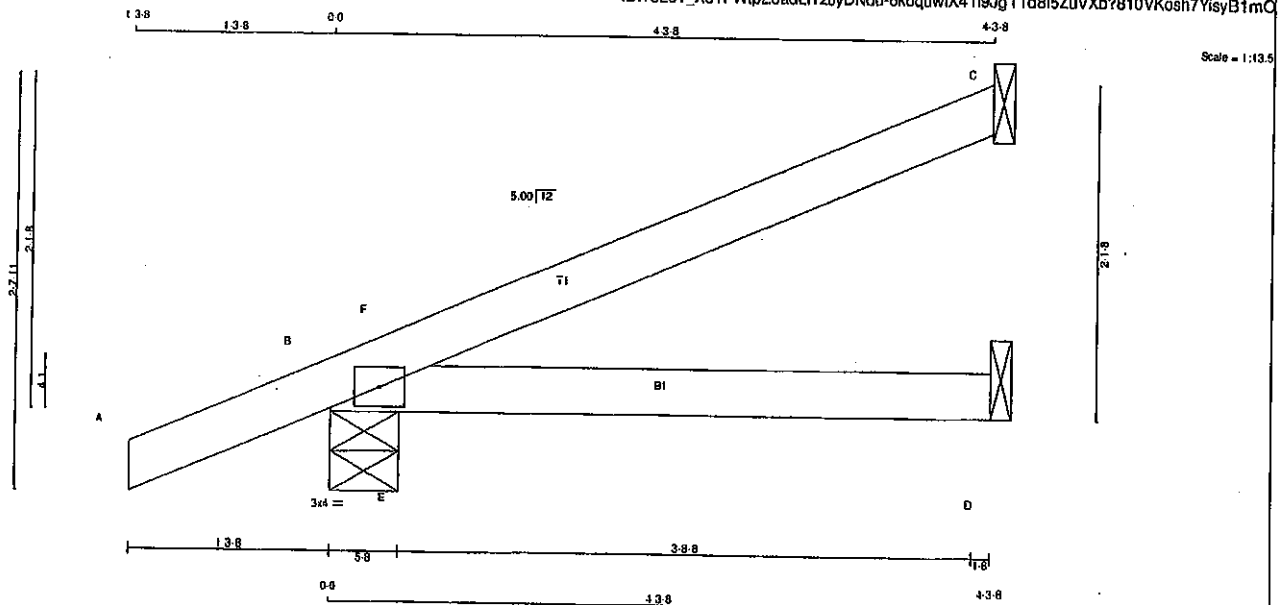
JSI GRIP = 0.19 (E) (INPUT = 0.90)
JSI METAL = 0.13 (B) (INPUT = 1.00)



Structural component only
DWG# T-2028118

JOB NAME 414964	TRUSS NAME J2	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.330 9 May 6 2020 Mitek Industries, Inc. Tue Dec 8 12:44:05 2020 Page 1
ID:76L61_X31PWtpZ5adLITzJyDNdu-6koquwIX419Jg11d8I5ZuVxb7810VKosh7YisyB1mC



Scale = 1:13.5

LUMBER				
N. L. & A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
C	171	0	171	0	1-8
B	360	0	360	0	1-8
D	65	0	65	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
C	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	118	92.0	0.0	0.0	0.0	28.0	0.0
B	252	179.0	0.0	0.0	0.0	74.0	0.0
D	49	18.0	0.0	0.0	0.0	31.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	WEBS		MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	FORCE (LBS)	MAX (LC)
FR-TO		FROM	TO	FR-TO		
A-B	0.23	-91.8	-91.8 0.12 (1)	10.00	E-F	227.7 0.00 (1)
B-F	-17.12	-91.8	-91.8 0.04 (4)	6.25		
F-C	-1.2	-91.8	-91.8 0.22 (1)	10.00		
B-E	0.0	-18.5	-18.5 0.16 (1)	10.00		
E-D	0.0	-18.5	-18.5 0.16 (1)	10.00		

TOTAL WEIGHT = 3 X 12 = 36 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.02")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.04")

CSI: TC=0.22/1.00 (C-F:1), BC=0.16/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.19/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PL)	SECTION (PL)
	MAX	MIN	MAX MIN
MT20	618	354	1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.26 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2028119

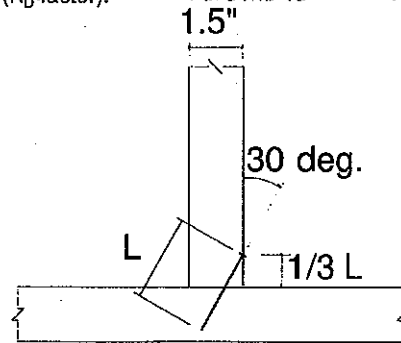
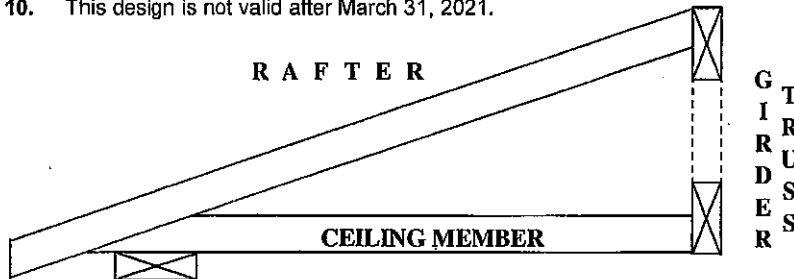
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_p factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.

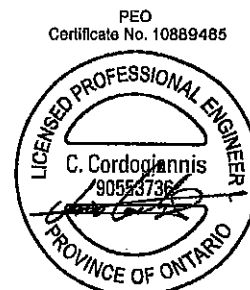


TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

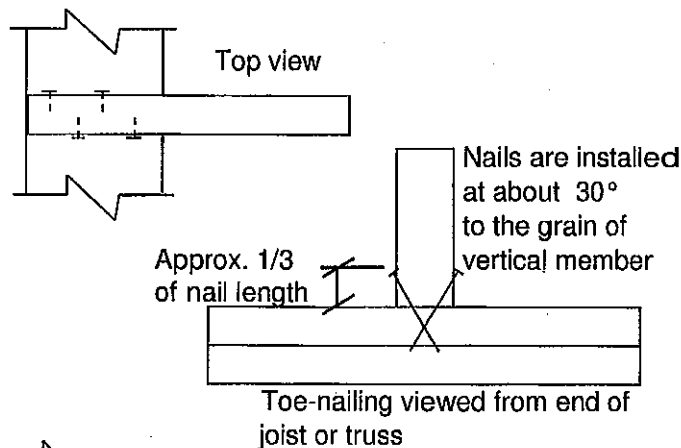
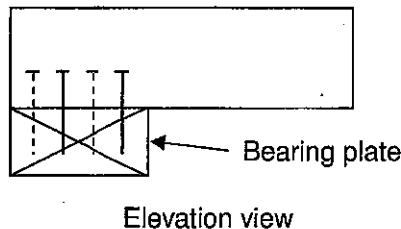
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

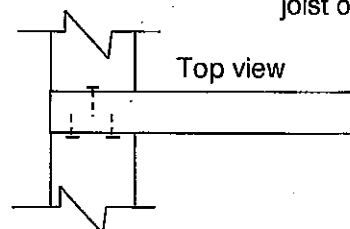
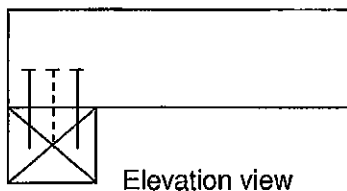
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5.
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate

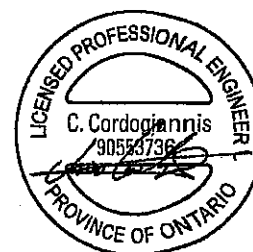


Toe-nailing on 2x4 Bearing Plate



MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



December 2, 2019

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

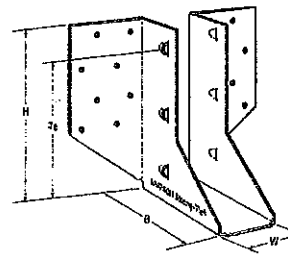
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

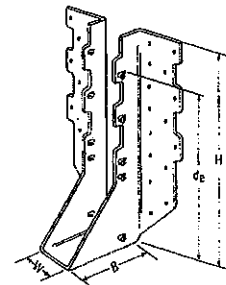
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

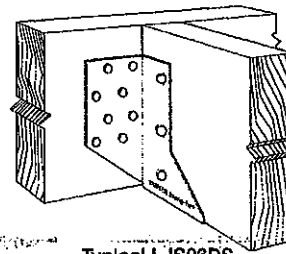
- See current catalogue for options



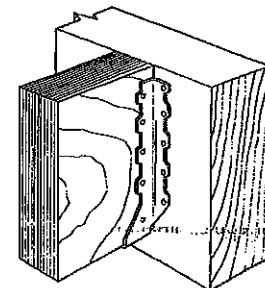
LJS26DS



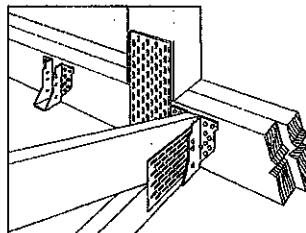
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



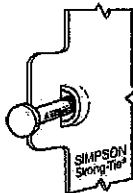
Typical HUS
Installation



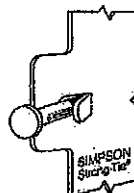
Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LJS26DS	18	1 1/8	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/2	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 3/32	3	6 5/32	(22) 16d	(8) 16d	3605	5385	2675	4345
HUS210	16	1 1/8	9 3/32	3	7 3/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.8 1/10	16	1 1/8	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

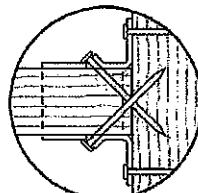
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Double
Shear
Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

LUS – Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

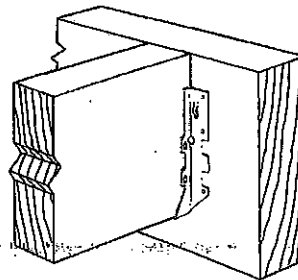
Finish: G90 galvanized

Design:

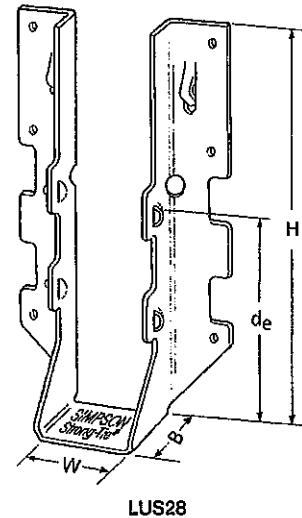
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.



Typical LUS Installation

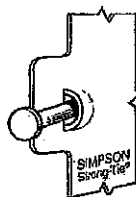


Options:

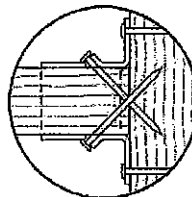
- These hangers cannot be modified

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _o ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LUS24	18	1 1/8	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/8	4 3/4	1 3/4	3 3/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 3/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 3/8	4 3/8	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 3/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 3/8	6 1/4	2	3 3/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 13/16	1 3/4	3 3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 3/8	8 3/8	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

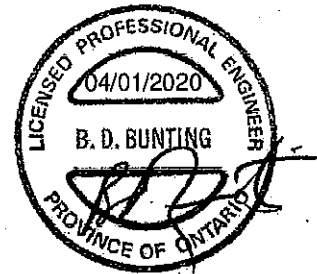
1. d_o is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPEC LUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HGUS – Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

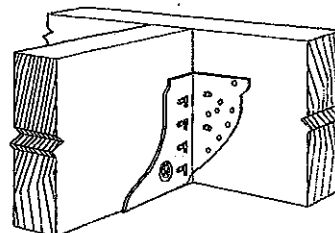
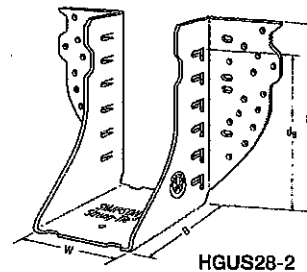
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

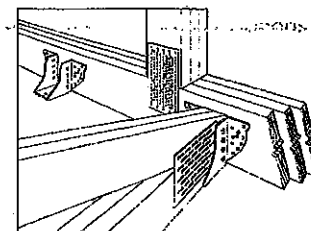
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



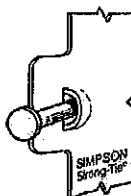
Typical HGUS Installation



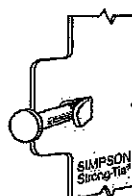
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (In.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _u =1.00)	Uplift (K _u =1.15)	Normal (K _u =1.00)
HGUS26	12	1 1/8	5 3/8	5	4 5/8	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 1/8	5 7/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 7/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/4	4	6 3/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1 1/8	9 1/8	5	7 7/8	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3 1/8	9 3/8	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 1/8	9 1/4	4	8 3/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 3/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 3/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 3/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

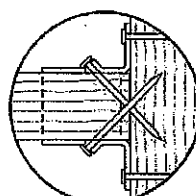
1. d_e is the distance from the seat of the hanger to the highest joist nail.



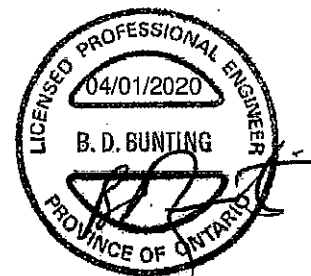
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPECHGUS20 3/20 exp. 6/22

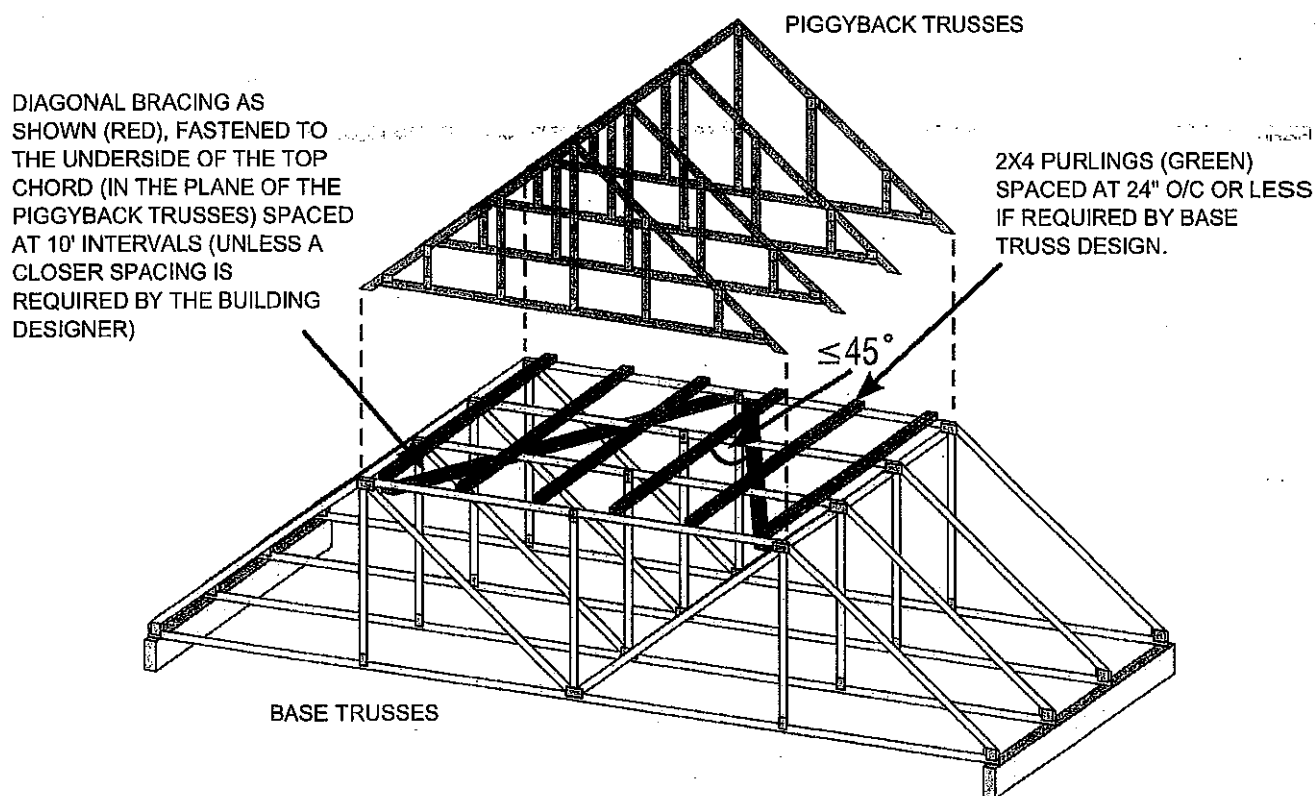
(800) 999-5099
strongtie.com

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

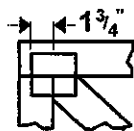
SKETCH FROM BCSI-CANADA 2013

Disclaimer:

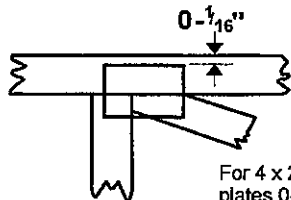
OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4

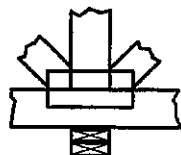
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

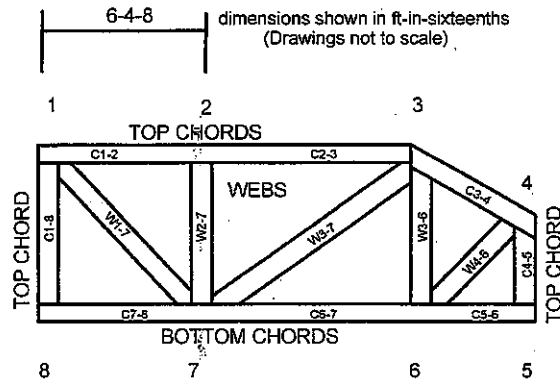


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

© 2012 MiTek® All Rights Reserved



MiTek Engineering Reference Sheet MII-7473 rev. 10/03/2015



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design).

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900213

Feb 09, 2018