

Permit No. 21-102417

THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE
THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH
THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW

These drawings and/or specifications have been reviewed by

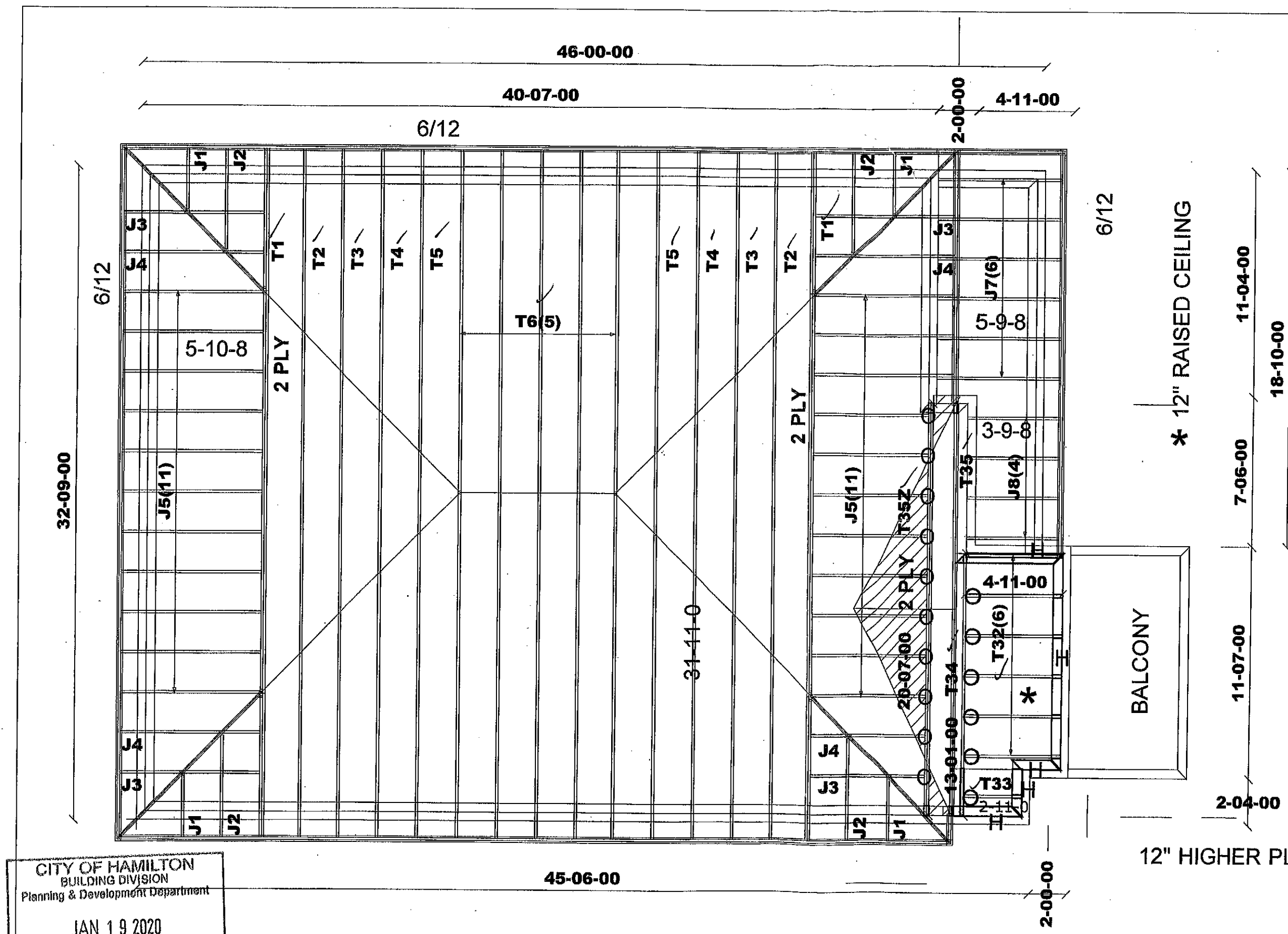
SDU FOR CHIEF BUILDING OFFICIAL DATE Feb 2021

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)

DENOTES
CONV.
FRAMING

DESIGN CONFORMS
WITH O.B.C.2012 PART 9
(2019 AMENDMENT)
DESIGN LOADS:
SNOW LOAD 25.6 PSF
TC DEAD 6 PSF
BC LIVE 0 PSF
BC DEAD 7.4 PSF



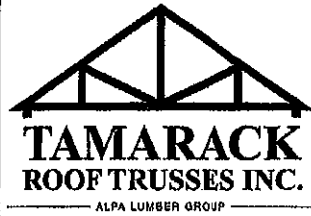
CITY OF HAMILTON
BUILDING DIVISION
Planning & Development Department
JAN 19 2020
REC BY _____ DATE _____
REF'D TO _____ DATE _____

M13107



Job Track: 51225	Builder / Location: GREEN PARK HOMES / WATERDOWN	Model / Elevation: VALLEYCREEK 2S/3	Lot 95
Plan Log: 202634	Project: RUSSELL GARDENS PH.3	Milek ver 8.3.1.215	
Layout ID: 409086	Date: 2020-04-14	Sales: Mario DiCano	Designer: AC/IG

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DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: VALLEYCREEK 2S
 Lot #: 095
 Elevation: 3

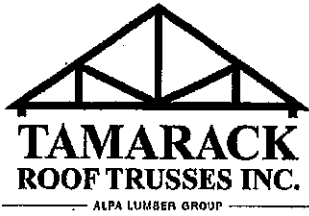
Job Track: 51225
 PlanLog: 202634
 Layout ID: 409086
 Ref #
 Page: 1 of 2
 Date: 04-14-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

2ND Copy

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2 2-ply	T1 Hip Girder	6 / 12	31-11-00	4-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	498.39 314.87		
	2	T2 Hip	6 / 12	31-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	259.03 161.87		
	2	T3 Hip	6 / 12	31-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	258.25 161.33		
	2	T4 Hip	6 / 12	31-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	271.21 168.33		
	2	T5 Hip	6 / 12	31-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	282.57 165.00		
	5	T6 Hip	6 / 12	31-11-00	9-01-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	646.18 404.17		
	6	T32 Flat	0 / 12	4-11-00	2-06-00	2 x 4		2-06-00 2-06-00	116.66 71.00		
	1	T33 Flat	0 / 12	2-11-00	1-06-00	2 x 4		1-06-00 1-06-00	10.78 8.83		
	1	T34 Roof Special Girder	0 / 12	13-01-00	2-06-00	2 x 4		1-06-00 2-06-00	51.97 33.00		
	1	T35 Common	3 / 12	20-07-00	3-00-06	2 x 4 2 x 6		5-08 5-08	72.66 45.17		
	1 2-ply	T36Z Common Girder	3 / 12	20-07-00	3-00-06	2 x 4 2 x 6		5-08 5-08	147.26 93.00		
	4	J1 Jack-Open	6 / 12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	28.08 18.87		
	4	J2 Jack-Open	6 / 12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	38.28 24.00		
	4	J3 Jack-Open	6 / 12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	46.33 29.33		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: VALLEYCREEK 2S
 Lot #: 095
 Elevation: 3

Job Track: 51225
 PlanLog: 202634
 Layout ID: 409086
 Ref #
 Page: 2 of 2
 Date: 04-14-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	4	J4 Jack-Open	6 / 12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	58.53 34.87		
	22	J6 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	369.48 234.67		
	6	J7 Jack-Open	6 / 12	5-10-00	3-10-11	2 x 4	1-03-08	4-03 2-10-03	95.41 60.00		
	4	J8 Jack-Open	6 / 12	3-09-08	2-10-07	2 x 4	1-03-08	4-03 1-10-03	46.54 29.33		

TOTAL # TRUSS= 76

TOTAL BFT OF ALL TRUSSES= 2054.84 BFT.

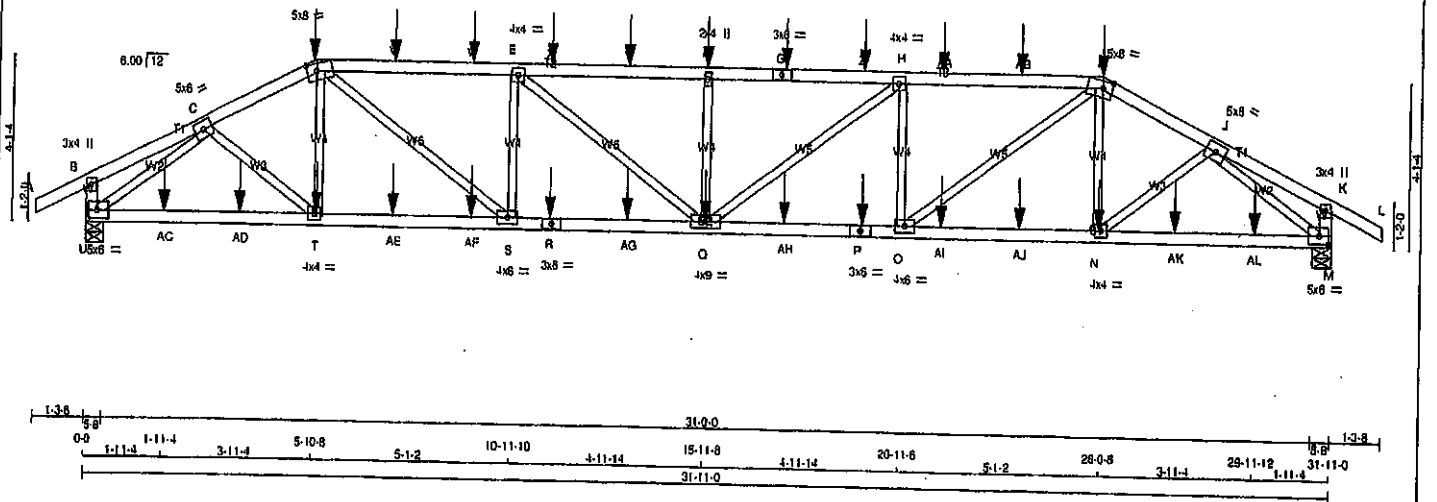
TOTAL WEIGHT OF ALL TRSSES 3275.61 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
16	Hardware	LUS24	

TOTAL NUMBER OF
ITEMS= 16

JOB NAME 408146	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 MTK Industries, Inc. Wed Apr 15 14:02:28 2020 Page 1	
1-3.8 0.0 3-0.8 3-0.8 2-10.0 5-10.8 5-1.2 10-11-10 4-11-14 15-11.8 4-11-14 20-11.6 5-1.2 26-0.8 2-10.0 28-10.8 3-0.8 31-11.0 32-0.8 1-3.8 Scale = 1:52.6				ID: CxSRXnRaDYJRNhTisra7zlylg-z8MZ71npped7d7JFFxrfutGgF8WVz3zGzBMKzOT58 31-11-0 32-0.8 1-3.8	



LUMBER				DESCR.	SPF
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER			
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - L	2x4	DRY	No.2	SPF	
U - B	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
U - R	2x4	DRY	No.2	SPF	
R - P	2x4	DRY	No.2	SPF	
P - M	2x4	DRY	No.2	SPF	
ALL WEBS 2x3 DRY No.2				SPF	
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D	12	SIDE(61.0)
D-G	12	SIDE(61.0)
G-I	12	SIDE(61.0)
I-L	12	SIDE(61.0)
U-B	12	TOP
M-K	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
U-R	12	SIDE(61.0)
R-P	12	SIDE(61.0)
P-M	12	SIDE(61.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

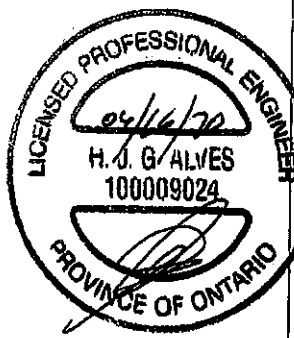
TOP: COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY

SIDE: PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)				
TYPE	PLATES	W	LEN	Y X
TMV+p	MT20	3.0	4.0	
TMWV-1	MT20	5.0	6.0	
TTWV-m	MT20	5.0	8.0	2.25 3.00
TMWV-w	MT20	4.0	4.0	
TMW+w	MT20	2.0	4.0	
TS-1	MT20	3.0	6.0	
TMWV-1	MT20	4.0	4.0	
TTWV-m	MT20	5.0	8.0	2.25 3.00
TMWV-1	MT20	5.0	8.0	
TMV+p	MT20	3.0	4.0	
BMWV-1	MT20	5.0	8.0	2.50 2.75
BMWV-1	MT20	4.0	4.0	
BMWV-1	MT20	4.0	6.0	
BS-1	MT20	3.0	6.0	
BMWVW-1	MT20	4.0	9.0	
BS-1	MT20	3.0	8.0	
BMWV-1	MT20	4.0	6.0	
BMWV-1	MT20	4.0	4.0	
BMWV-1	MT20	5.0	8.0	2.50 2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
U	3035	0	3035	0	0	5-8	5-8		
M	3034	0	3034	0	0	5-8	5-8		
UNFACTORED REACTIONS									
	1ST CASE	MAX. MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
U	2146	1410	0	0	0	0	736	0	0
M	2145	1409	0	0	0	0	736	0	0
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	FR-TO	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO							
A-B	0 28	-91.8 -91.8	0.07 (1)	10.00	C-T	0 693	0 099 (1)		
B-C	0 6	-91.8 -91.8	0.08 (1)	10.00	T-D	-223 55	0 03 (1)		
C-D	-4467 0	-91.8 -91.8	0.13 (1)	4.44	D-S	0 2320	0 28 (1)		
D-E	-5839 0	-91.8 -91.8	0.48 (1)	3.63	S-E	-1244 0	0 17 (1)		
E-F	-5839 0	-91.8 -91.8	0.48 (1)	3.63	E-Q	0 693	0 09 (1)		
F-G	-5839 0	-91.8 -91.8	0.51 (1)	3.45	Q-F	-882 0	0 09 (1)		
G-H	-6390 0	-91.8 -91.8	0.51 (1)	3.45	O-H	0 697	0 09 (1)		
H-I	-6390 0	-91.8 -91.8	0.51 (1)	3.45	I-O	-223 55	0 03 (1)		
I-J	-6390 0	-91.8 -91.8	0.51 (1)	3.45	N-I	0 692	0 09 (1)		
J-K	-6390 0	-91.8 -91.8	0.51 (1)	3.45	U-C	-4363 0	0 56 (1)		
K-L	-6390 0	-91.8 -91.8	0.48 (1)	3.63	J-M	-4362 0	0 56 (1)		
L-M	-6390 0	-91.8 -91.8	0.48 (1)	3.63					
AB-I	-5839 0	-91.8 -91.8	0.48 (1)	3.63					
I-J	-4466 0	-91.8 -91.8	0.13 (1)	4.44					
J-K	0 5	-91.8 -91.8	0.08 (1)	10.00					
K-L	0 28	-91.8 -91.8	0.07 (1)	10.00					
U-B	-249 0	0.0 0.0	0.01 (1)	7.81					
M-K	-249 0	0.0 0.0	0.01 (1)	7.81					
U-AC	0 3438	-18.5 -18.5	0.38 (1)	10.00					
AC-AD	0 3438	-18.5 -18.5	0.38 (1)	10.00					
AD-T	0 3438	-18.5 -18.5	0.38 (1)	10.00					
T-AE	0 3981	-18.5 -18.5	0.42 (1)	10.00					
AE-AF	0 3981	-18.5 -18.5	0.42 (1)	10.00					
AF-S	0 3981	-18.5 -18.5	0.42 (1)	10.00					
S-R	0 5840	-18.5 -18.5	0.57 (1)	10.00					
R-AG	0 5840	-18.5 -18.5	0.57 (1)	10.00					
AG-Q	0 5840	-18.5 -18.5	0.57 (1)	10.00					
Q-AH	0 8837	-18.5 -18.5	0.57 (1)	10.00					
AH-P	0 8837	-18.5 -18.5	0.57 (1)	10.00					
P-O	0 8837	-18.5 -18.5	0.57 (1)	10.00					
O-AI	0 3979	-18.5 -18.5	0.42 (1)	10.00					
AI-AJ	0 3979	-18.5 -18.5	0.42 (1)	10.00					
AJ-N	0 3979	-18.5 -18.5	0.42 (1)	10.00					
N-AK	0 3437	-18.5 -18.5	0.38 (1)	10.00					
AK-AL	0 3437	-18.5 -18.5	0.38 (1)	10.00					
AL-M	0 3437	-18.5 -18.5	0.38 (1)	10.00					

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL	=	25.6 PSF
	DL	=	8.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.4 PSF
TOTAL LOAD		=	39.0 PSF
SPACING = 24.0 IN. C/C			
LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM			
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015			
THIS DESIGN COMPLIES WITH:			
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019			
- PART 9 OF CBC 2012 (2019 AMENDMENT)			
- CSA 088-09, CSA 088-14			
- TPIC 2011, TPIC 2014			
(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD			
ALLOWABLE DEFL. (LL) = L/380 (1.08")			
CALCULATED VERT. DEFL. (LL) = U/999 (0.18")			
ALLOWABLE DEFL. (TL) = L/380 (1.08")			
CALCULATED VERT. DEFL. (TL) = L/999 (0.34")			
CSI: TC=0.51 1.00 (E-F:1), BC=0.57 1.00 (Q-S:1), WB=0.58 1.00 (C-U:1), SS=0.19 1.00 (D-E:1)			
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00			
COMP=1.00 SHEAR=1.00 TENS=1.00			
COMPANION LIVE LOAD FACTOR = 1.00			
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
NAIL VALUES			
PLATE GRIP(DRY)	SHEAR	SECTION	
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 788	1987 1656
PLATE PLACEMENT TOL. = 0.250 inches			
PLATE ROTATION TOL. = 5.0 Deg.			
JS GRIP=0.85 (R) INPUT=0.80			
JT METAL=0.87 (R) INPUT=1.00			
Structural component only 1/2			
DIM/G# T.0006731			



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408146	T1	1	2	TRUSS DESC.		

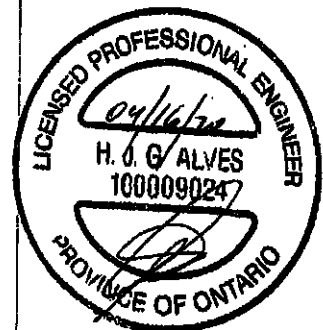
Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitel Industries, Inc. Wed Apr 15 14:02:28 2020 Page 2
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FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-457	-457	---	FRONT	VERT	TOTAL	---	C1
F	15-11-8	-110	-110	---	FRONT	VERT	TOTAL	---	C1
G	17-11-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
I	28-0-8	-457	-457	---	FRONT	VERT	TOTAL	---	C1
N	25-11-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
P	19-11-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
Q	15-11-8	-28	-28	---	FRONT	VERT	TOTAL	---	C1
R	11-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
T	5-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
V	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	11-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	19-11-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	21-11-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	23-11-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	1-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AD	3-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AE	7-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AF	9-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AG	13-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AH	17-11-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AI	21-11-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AJ	23-11-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AK	27-11-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AL	29-11-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only. *m*
DWG# T-2008231

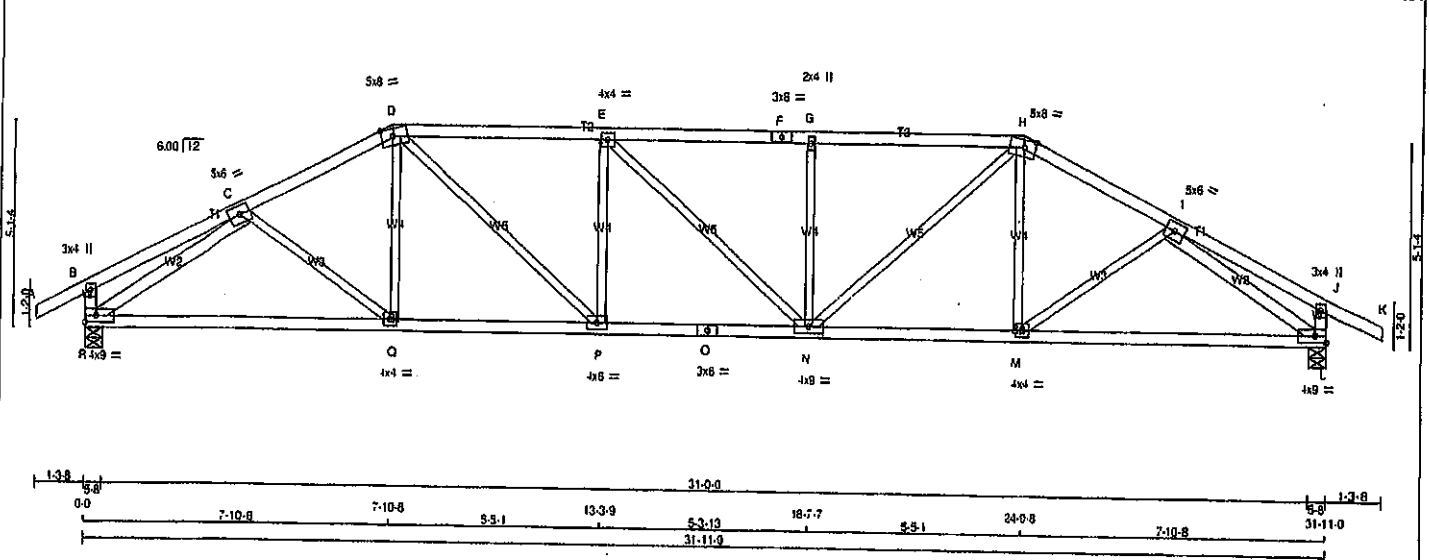
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408146	T2	1	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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Scale = 1:52.6



LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
R - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)					
JT	TYPE	PLATES	W	LEN	X
B	TMVp	MT20	3.0	4.0	
C	TMVW-1	MT20	5.0	8.0	
D	TTWW-m	MT20	5.0	8.0	2.25 3.50
E	TMVW-1	MT20	4.0	4.0	
F	TS-1	MT20	3.0	4.0	
G	TMVW-m	MT20	2.0	4.0	
H	TTWW-m	MT20	5.0	8.0	2.25 3.50
I	TMVW-1	MT20	5.0	8.0	
J	TMVp	MT20	3.0	4.0	
L	BMVW-1	MT20	4.0	9.0	Edge
M	BMVW-1	MT20	4.0	4.0	
N	BMVW-1	MT20	4.0	9.0	
O	BS-1	MT20	3.0	6.0	
P	BMVW-1	MT20	4.0	6.0	
Q	BMVW-1	MT20	4.0	4.0	
R	BMVW-1	MT20	4.0	9.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED	MAXIMUM FACTORED	INPUT	REQD	
	GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
R	1884	0	1884	0	0
L	1884	0	1884	0	0

UNFACTORED REACTIONS					
	1ST LCASE	MAX.	MIN.	COMPONENT	REACTIONS
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND
R	1330	886	0	0	0
L	1330	886	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)					
CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED VERT. LOAD LC1 (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 28	-91.8	-91.8 0.12 (1)	C-Q	0 74
B-C	0 17	-91.8	-91.8 0.20 (1)	D-P	0 133
C-D	-2469 0	-91.8	-91.8 0.30 (1)	E-R	0 933
D-E	-2893 0	-91.8	-91.8 0.54 (1)	F-S	-534 0
E-F	-2891 0	-91.8	-91.8 0.53 (1)	G-T	-2 0
F-G	-2891 0	-91.8	-91.8 0.53 (1)	H-U	-533 0
G-H	-2891 0	-91.8	-91.8 0.53 (1)	I-V	0 931
H-I	-2469 0	-91.8	-91.8 0.30 (1)	J-W	0 134
I-J	0 17	-91.8	-91.8 0.20 (1)	K-X	0 74
J-K	0 28	-91.8	-91.8 0.12 (1)	L-Y	-2688 0
K-L	-289 0	0 0	0 0.03 (1)	M-Z	-2687 0
L-M	-289 0	0 0	0 0.03 (1)		
R-Q	0 2167	-18.5	-18.5 0.49 (1)		
Q-P	0 2194	-18.5	-18.5 0.48 (1)		
P-O	0 2893	-18.5	-18.5 0.53 (1)		
O-N	0 2893	-18.5	-18.5 0.53 (1)		
N-M	0 2194	-18.5	-18.5 0.49 (1)		
M-L	0 2168	-18.5	-18.5 0.49 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
OL = 8.0 PSF
BOT CH. LL = 0.0 PSF
OL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00:12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2018 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPG 2011, TPG 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.06")
CALCULATED VERT. DEFL. (LL) = L/999 (0.15")
ALLOWABLE DEFL. (TL) = L/360 (1.06")
CALCULATED VERT. DEFL. (TL) = L/999 (0.29")

CSI: TC=0.54:1.00 (D-E:1), BC=0.53:1.00 (M-P:1), WB=0.74:1.00 (I-L:1), SS=0.23:1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE GRIP (DRY)	SHEAR	SECTION	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN			
MT20	618 354	1687 788	1987 1656		

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

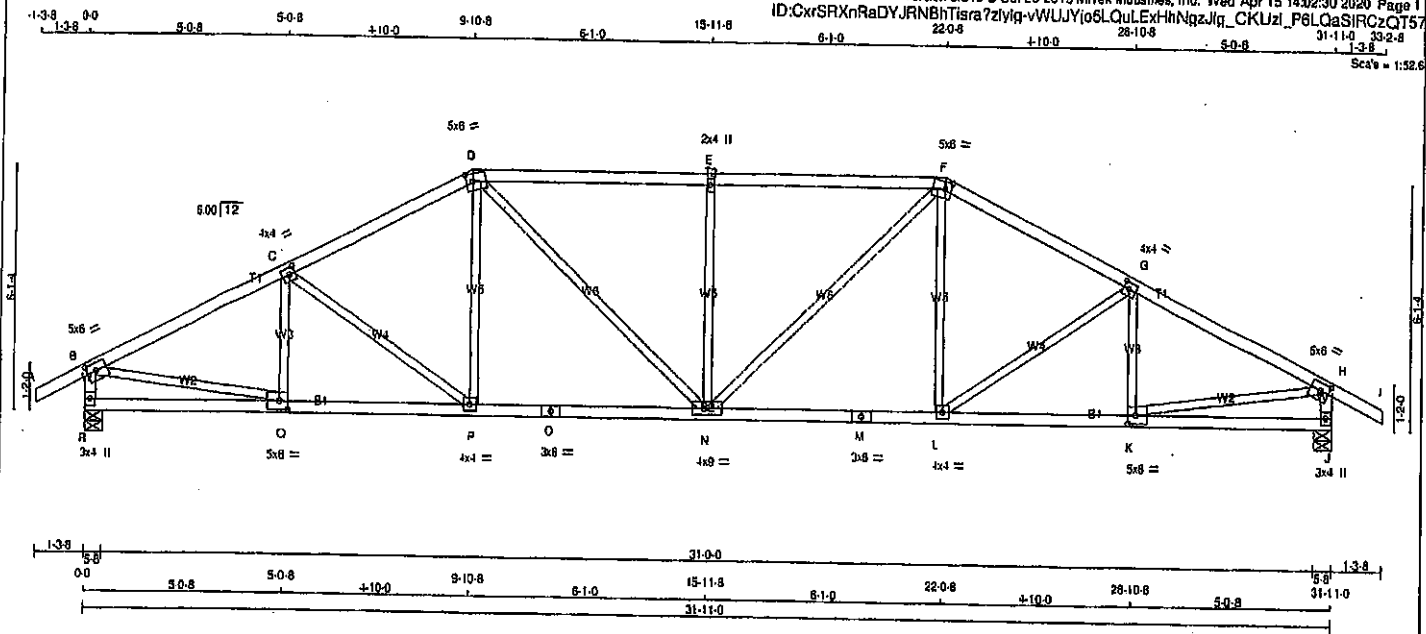
JSI GRP= 0.84 (O) (INPUT = 0.90)
JSI METAL= 0.94 (O) (INPUT = 1.00)



JOB NAME 408146	TRUSS NAME T3	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Wed Apr 15 14:02:30 2020 Page 1
ID: CxrSRXnRaDYJRNbHtIsra?zlylg-vWUJYj05LQuLExHhNgzJ/g_CKUZl_P6LQaSIRCzQT57



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-1	MT20	5.0	8.0	2.25 2.75
C	BMVW-1	MT20	4.0	4.0	2.00 1.75
D	TTWV-m	MT20	5.0	8.0	2.25 2.00
E	TMVW-w	MT20	2.0	4.0	
F	TTWV-m	MT20	5.0	8.0	2.25 2.00
G	BMVW-1	MT20	4.0	4.0	2.00 1.75
H	TMVW-1	MT20	5.0	8.0	2.25 2.75
J	BMV1+p	MT20	3.0	4.0	
K	BMVW-1	MT20	5.0	8.0	2.50 2.50
L	BMVW-1	MT20	4.0	4.0	
M	BS-1	MT20	3.0	8.0	
N	BMVWW-1	MT20	4.0	9.0	
O	BS-1	MT20	3.0	8.0	
P	BMVW-1	MT20	4.0	4.0	
Q	BMVW-1	MT20	5.0	8.0	2.50 2.50
R	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
R	1884	0	1884	0	5-8
J	1884	0	1884	0	5-8

UNFACTORED REACTIONS		IST CASE	MAX. MIN. COMPONENT REACTIONS		
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
R	1330	888	0	0	0
J	1330	888	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 28	Q-C	-301 0
B-C	-2538 0	P-D	-284 0
C-D	-2342 0	N-E	-685 0
D-E	-2493 0	M-F	-575 0
E-F	-2493 0	L-G	-284 0
F-G	-2342 0	K-H	-301 0
G-H	-2538 0	B-Q	0 2323
H-I	0 28	K-H	0 2323
R-B	-1841 0		
J-H	-1841 0		
R-Q	0 0		
O-P	0 2290		
P-O	0 2077		
O-N	0 2077		
N-M	0 2077		
M-L	0 2077		
L-K	0 2290		
K-J	0 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, NBCC 2012, ABC 2019
- PART 9 OF NBCC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.08')
CALCULATED VERT. DEFL. (LL) = L/999 (0.13')
ALLOWABLE DEFL. (TL) = L/360 (1.08')
CALCULATED VERT. DEFL. (TL) = L/999 (0.24')

CSL: TC=0.54/1.00 (D-E-I), BC=0.43/1.00 (K-L-I), WB=0.52/1.00 (H-K-I), SS=0.27/1.00 (E-F-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

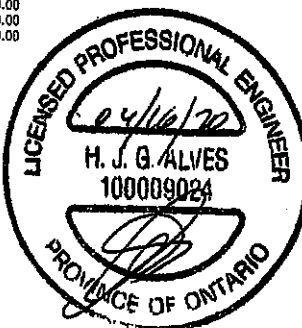
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PL) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (B) (INPUT = 0.90)
JSI METAL = 0.87 (H) (INPUT = 1.00)

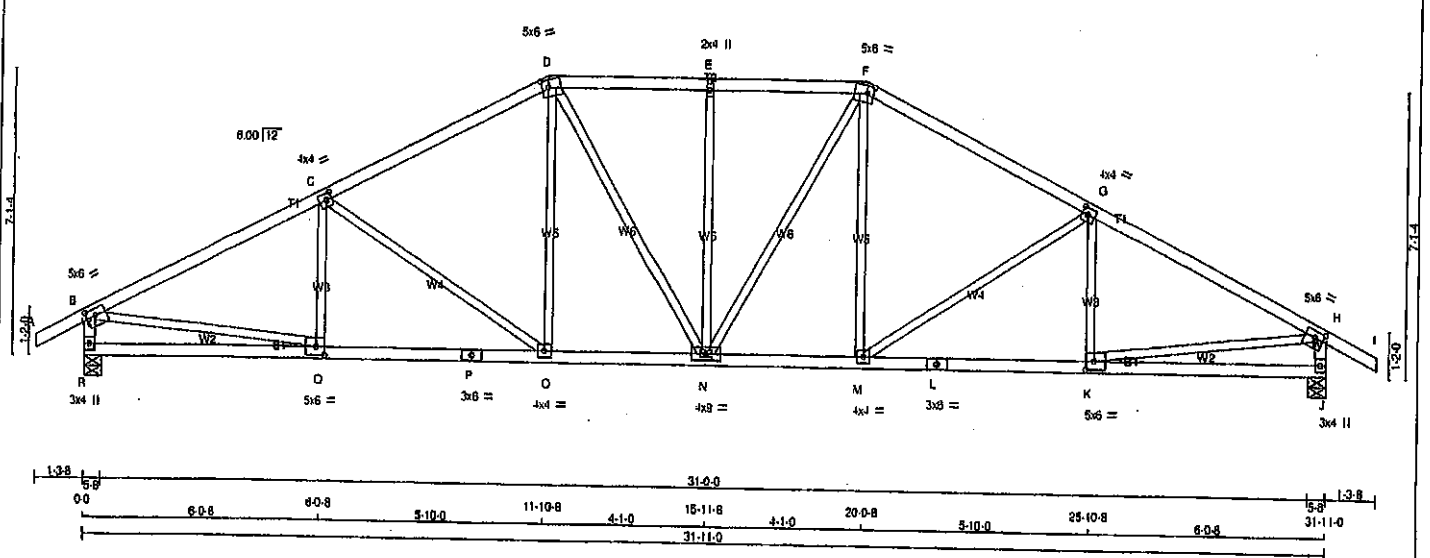


Structural component only
DWG# T-2006233

JOB NAME 408146	TRUSS NAME T4	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Wed Apr 15 14:02:31 2020 Page 1
ID: CxrSRXnRaDYJRNhTisra?zlylg-Nj2h13pk6k0Cs6uwNUYHuWNvuJJaFVIECrzQ156
31-11-0 33-2-8
Scale = 1:52.8



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	ORY	No.2	SPF	
D - F	2x4	ORY	No.2	SPF	
F - I	2x4	ORY	No.2	SPF	
R - B	2x4	ORY	No.2	SPF	
J - H	2x4	ORY	No.2	SPF	
R - P	2x4	ORY	No.2	SPF	
P - L	2x4	ORY	No.2	SPF	
L - J	2x4	ORY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	5.0	8.0	2.00	2.75	
C	TMVW-1	MT20	4.0	4.0	2.00	1.75	
D	TTWV-m	MT20	5.0	8.0	2.25	2.00	
E	TMVW-m	MT20	2.0	4.0			
F	TTWV-m	MT20	5.0	8.0	2.25	2.00	
G	TMVW-1	MT20	4.0	4.0	2.00	1.75	
H	TMVW-1	MT20	5.0	8.0	2.00	2.75	
J	BMV1+p	MT20	3.0	4.0			
K	BMVW-1	MT20	5.0	8.0	2.50	2.50	
L	BS-1	MT20	3.0	8.0			
M	BMVW-1	MT20	4.0	4.0			
N	BMVW-1	MT20	4.0	9.0			
O	BMVW-1	MT20	4.0	4.0			
P	BS-1	MT20	3.0	8.0			
Q	BMVW-1	MT20	5.0	8.0	2.50	2.50	
R	BMV1+p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	DOWN	UPLIFT	BRG
JT	VERT	HORZ	DOWN	HORZ
R	1884	0	1884	0
J	1884	0	1884	0

UNFACTORED REACTIONS							
	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
	COMBINED	SNOW.	LIVE	PERM. LIVE	WIND	DEAD	SO
JT							
R	1330	888 0	0 0	0 0	0 0	444 0	0
J	1330	888 0	0 0	0 0	0 0	444 0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	MAX
MEMB.	FORCE	(PLF)	(PLF)	(LC)	UNBRAC	LENGTH	FR-TO	FORCE	(LC)
FR-TO									
A-B	0 28	-91.8	-91.8	0.12 (1)	10.00	Q-C	-212 29	0.08 (1)	
B-C	-2579 0	-91.8	-91.8	0.55 (1)	3.75	C-D	-480 0	0.48 (1)	
C-D	-2188 0	-91.8	-91.8	0.49 (1)	4.11	D-E	0 362	0.08 (1)	
D-E	-2088 0	-91.8	-91.8	0.24 (1)	4.49	E-F	0 296	0.07 (1)	
E-F	-2088 0	-91.8	-91.8	0.24 (1)	4.49	F-G	-451 0	0.39 (1)	
F-G	-2188 0	-91.8	-91.8	0.49 (1)	4.11	G-H	0 296	0.07 (1)	
G-H	-2579 0	-91.8	-91.8	0.55 (1)	3.75	H-I	0 362	0.08 (1)	
H-I	0 28	-91.8	-91.8	0.12 (1)	10.00	I-J	-480 0	0.48 (1)	
I-J	-1836 0	0.0	0.0	0.19 (1)	6.18	J-K	-212 29	0.08 (1)	
J-K	-1836 0	0.0	0.0	0.19 (1)	6.18	K-L	0 2355	0.53 (1)	
K-L	0 0	-18.5	-18.5	0.15 (4)	10.00	L-M	0 2355	0.53 (1)	
L-M	0 2332	-18.5	-18.5	0.44 (1)	10.00	M-N	0 2332	0.44 (1)	
M-N	0 2332	-18.5	-18.5	0.44 (1)	10.00	N-O	0 1935	0.37 (1)	
N-O	0 1935	-18.5	-18.5	0.37 (1)	10.00	O-P	0 1935	0.37 (1)	
O-P	0 1935	-18.5	-18.5	0.37 (1)	10.00	P-Q	0 2332	0.44 (1)	
P-Q	0 2332	-18.5	-18.5	0.44 (1)	10.00	Q-R	0 2332	0.44 (1)	
Q-R	0 2332	-18.5	-18.5	0.44 (1)	10.00	R-S	0 2332	0.44 (1)	
R-S	0 2332	-18.5	-18.5	0.44 (1)	10.00	S-T	0 2332	0.44 (1)	
S-T	0 2332	-18.5	-18.5	0.44 (1)	10.00	T-U	0 2332	0.44 (1)	
T-U	0 2332	-18.5	-18.5	0.44 (1)	10.00	U-V	0 2332	0.44 (1)	
U-V	0 2332	-18.5	-18.5	0.44 (1)	10.00	V-W	0 2332	0.44 (1)	
V-W	0 2332	-18.5	-18.5	0.44 (1)	10.00	W-X	0 2332	0.44 (1)	
W-X	0 2332	-18.5	-18.5	0.44 (1)	10.00	X-Y	0 2332	0.44 (1)	
X-Y	0 2332	-18.5	-18.5	0.44 (1)	10.00	Y-Z	0 2332	0.44 (1)	
Y-Z	0 2332	-18.5	-18.5	0.44 (1)	10.00	Z-A	0 2332	0.44 (1)	
Z-A	0 2332	-18.5	-18.5	0.44 (1)	10.00	A-B	0 2332	0.44 (1)	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2018
- PART 9 OF CBC 2012 (2019-AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08')
CALCULATED VERT. DEFL.(LL) = L/999 (0.12')
ALLOWABLE DEFL.(TL) = L/360 (1.08')
CALCULATED VERT. DEFL.(TL) = L/999 (0.21')

CSI: TC=0.55/1.00 (B-C:1), BC=0.44/1.00 (O-C:1),
WB=0.53/1.00 (H-K:1), SS=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

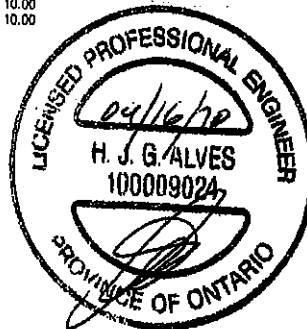
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

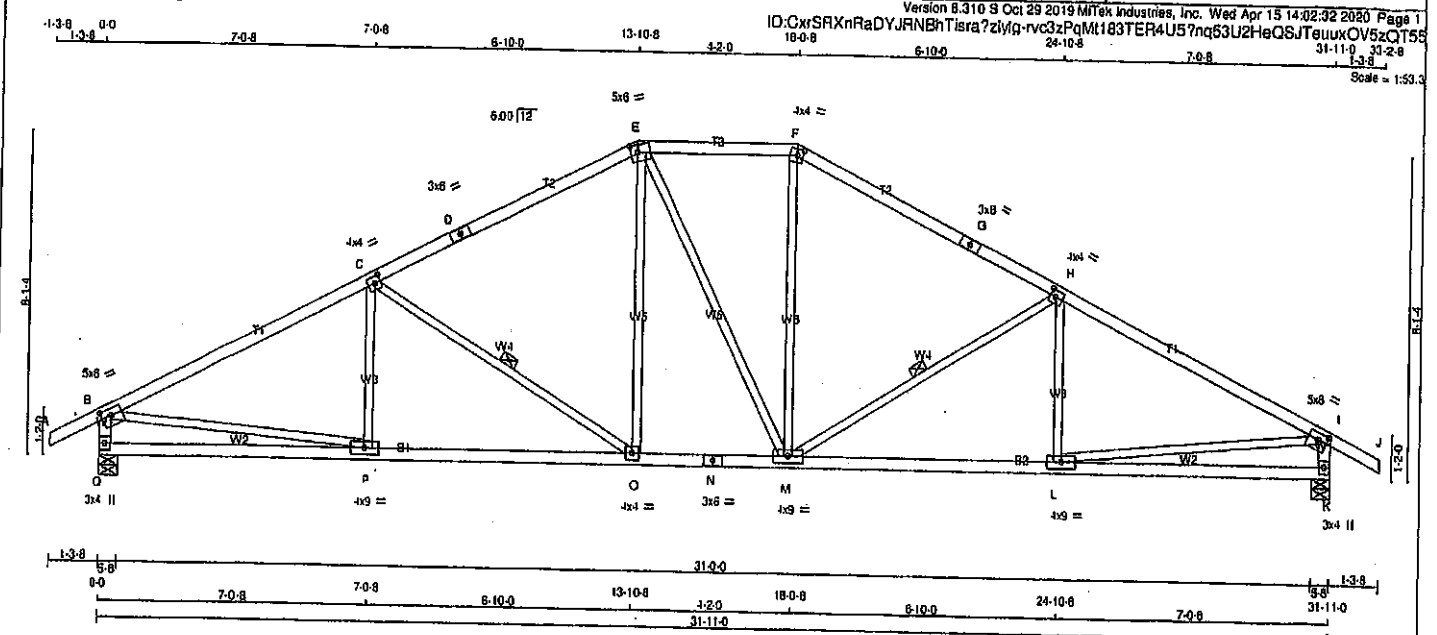
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL = 0.74 (P) (INPUT = 1.00)



Structural component only
DWG# T-2006234

JOB NAME 408146	TRUSS NAME T5	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

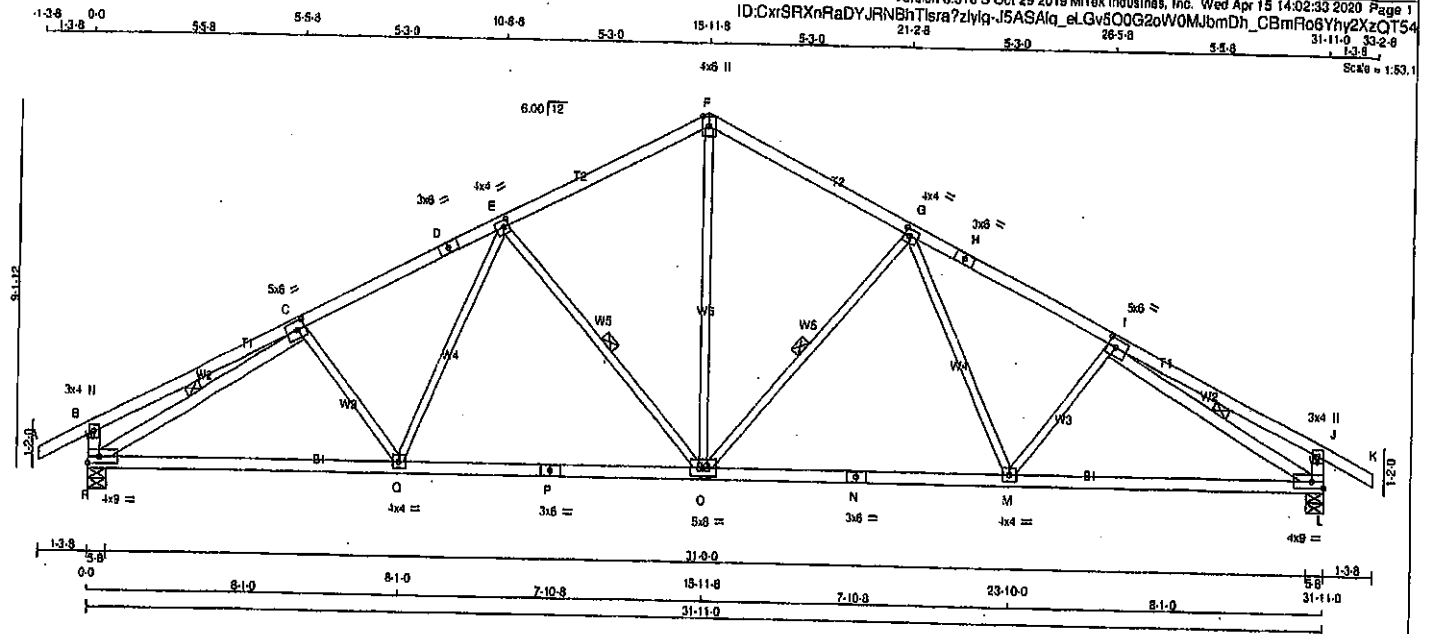


LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS		2x3	DRY
EXCEPT			No.2
DRY: SEASONED LUMBER.			

JOB NAME 408146	TRUSS NAME T6	QUANTITY 6	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRAWG NO.
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Tamarack Roof Truss, Burlington

Version 8.316 S Oct 29 2019 Mittek Industries, Inc. Wed Apr 15 14:02:33 2020 Page 1
ID: CxSRXnRAdYJRNhTlra7zlyq-J5ASAlq_eLGv5QOG2oW0MjbmDh_CBMRo8Yhy2XzQT54
31-11-0 33-2-8
Scale = 1:50.1



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
R - P	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
R - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y
B	TMV+p	MT20	3.0	4.0	
C	TMVW+1	MT20	5.0	8.0	2.50
D	TS-1	MT20	3.0	8.0	
E	TMVW+1	MT20	4.0	4.0	2.00
F	TTW+p	MT20	4.0	8.0	Edge
G	TMVW+1	MT20	4.0	4.0	2.00
H	TS-1	MT20	3.0	8.0	
I	TMVW+1	MT20	5.0	8.0	2.50
J	TMV+p	MT20	3.0	4.0	
L	BMVW+1	MT20	4.0	9.0	Edge
M	BMVW+1	MT20	4.0	4.0	
N	BS-1	MT20	3.0	8.0	
O	BMVW+1	MT20	5.0	9.0	
P	BS-1	MT20	3.0	8.0	
R	BMVW+1	MT20	4.0	4.0	
O	BMVW+1	MT20	4.0	9.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	BRG	BRG	IN-SX
R	1884	0	0	1884	0	0	5-8	5-8	5-8
L	1884	0	0	1884	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS
1ST CASE - MAX. MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1330	886	0	0	0	444	0
L	1330	886	0	0	0	444	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-Q, E-O, C-R, H-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO			
A-B	0 28	-91.8 -91.8 0.12 (1)	10.00	O-F	0 1211	0.27 (1)	
B-C	0 20	-91.8 -91.8 0.33 (1)	10.00	O-G	-701 0	0.32 (1)	
C-D	-2428 0	-91.8 -91.8 0.37 (1)	4.11	G-M	0 312	0.07 (1)	
D-E	-2428 0	-91.8 -91.8 0.37 (1)	4.11	M-I	-158 21	0.05 (1)	
E-F	-1808 0	-91.8 -91.8 0.34 (1)	4.85	E-O	-701 0	0.32 (1)	
F-G	-1808 0	-91.8 -91.8 0.34 (1)	4.85	O-E	0 312	0.07 (1)	
G-H	-2428 0	-91.8 -91.8 0.37 (1)	4.11	C-Q	-158 21	0.05 (1)	
H-I	-2428 0	-91.8 -91.8 0.37 (1)	4.11	R-C	-2717 0	0.55 (1)	
I-J	0 20	-91.8 -91.8 0.33 (1)	10.00	I-L	-2717 0	0.55 (1)	
J-K	0 28	-91.8 -91.8 0.12 (1)	10.00				
R-B	-325 0	0.0 0.0 0.03 (1)	7.81				
L-J	-325 0	0.0 0.0 0.03 (1)	7.81				
R-Q	0 2288	-18.5 -18.5 0.51 (1)	10.00				
O-P	0 2051	-18.5 -18.5 0.48 (1)	10.00				
P-O	0 2051	-18.5 -18.5 0.48 (1)	10.00				
O-N	0 2051	-18.5 -18.5 0.48 (1)	10.00				
N-M	0 2051	-18.5 -18.5 0.48 (1)	10.00				
M-L	0 2288	-18.5 -18.5 0.51 (1)	10.00				

TOTAL WEIGHT = 6 X 133 = 798 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2018
- PART 9 OF OBC 2012 (2018 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = L/989 (0.24")

CS: TC=0.37/1.00 (C-E:1), BC=0.51/1.00 (C-R:1), WR=0.59/1.00 (C-R:1), SR=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

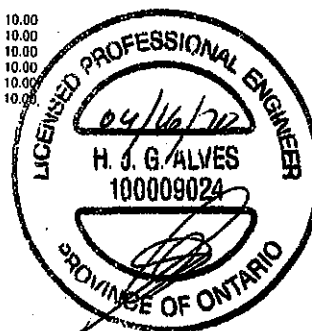
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 918 354 1687 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (I) (INPUT = 0.90)
JSI METAL = 0.70 (I) (INPUT = 1.00)



Structural component only
DWG# T-20002338

PLATES (tablets in package)					
J	TYPE	PLATES	W	LEN	Y X
B	TMWV-p	MT20	5.0	6.0	1.25 3.00
C	TTWWV-m	MT20	5.0	6.0	1.75 3.00
D	TMWV-l	MT20	4.0	4.0	
E	TMWV-w	MT20	2.0	4.0	
F	TS-l	MT20	3.0	6.0	
G	TMWV-l	MT20	4.0	4.0	
H	TTWWV-m	MT20	5.0	6.0	1.75 3.00
I	TMWV-p	MT20	3.0	6.0	1.25 3.00
K	BMV1-p	MT20	3.0	6.0	
L, M, Q, R					
L	BMWV-l	MT20	5.0	6.0	
N	BS-l	MT20	5.0	6.0	
O	BMWVWV-l	MT20	3.0	6.0	
P	BS-l	MT20	5.0	6.0	
S	BMV1-p	MT20	3.0	6.0	

FACTORED CONCENTRATED LOADS (LBS)							
LOC.	LC1	MAX.	FACE	DIF.	TYPE	HEEL	CONN.
4-5-9	-38	-42	---	FRONT	VERT	DEAD	... C1
4-5-9	-173	-173	---	FRONT	VERT	SNOW	... C1
12-2-8	-760	-760	---	BACK	VERT	TOTAL	... C1
5-0-12	-104	-104	---	BACK	VERT	TOTAL	... C1
7-0-12	-86	-86	---	BACK	VERT	TOTAL	... C1
9-0-12	-86	-86	---	BACK	VERT	TOTAL	... C1
11-0-12	-86	-86	---	BACK	VERT	TOTAL	... C1
1-0-12	-17	-17	---	BACK	VERT	TOTAL	... C1
3-0-12	-17	-17	---	BACK	VERT	TOTAL	... C1
5-0-12	-17	-17	---	BACK	VERT	TOTAL	... C1

Structural component only 1/2
DWG# T-2006237

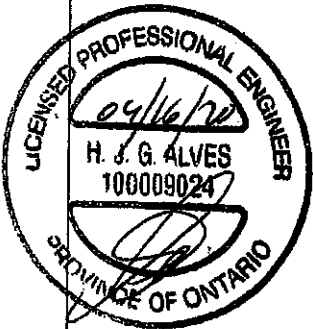
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408146	T7	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Wed Apr 15 14:02:34 2020 Page 2
ID: CxSRXnRaDYJRNhTlra7zlvlg-nllkoO5rcPQmiYbTcW2FvW8v25lqwG3xLCQVa zQT53

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AA	7-0-12	-17	-17	---	BACK	VERT	TOTAL	---	C1
AB	9-0-12	-17	-17	---	BACK	VERT	TOTAL	---	C1
AC	11-0-12	-17	-17	---	BACK	VERT	TOTAL	---	C1

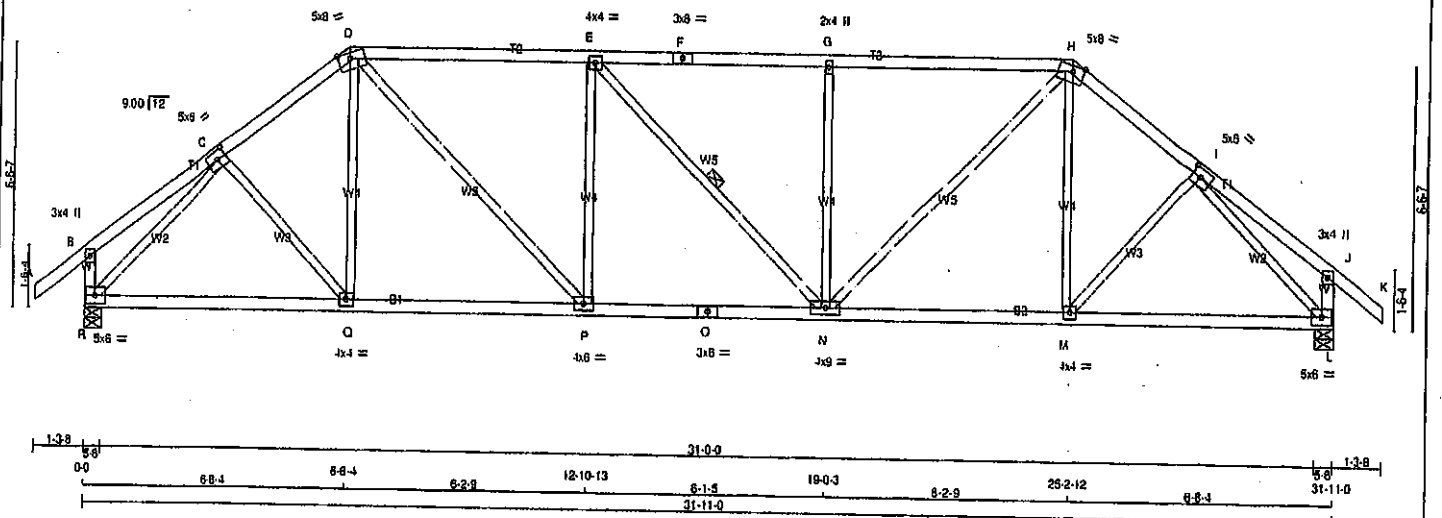
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



JOB NAME 408146	TRUSS NAME T8	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 5 Oct 29 2019 Mittek Industries, Inc. Wed Apr 15 14:02:35 2020 Page 1
 ID: CxrSRXnRaDYJRNhTlra7zlyg-FUICbRsE9yWdKIA9DZUSkh1KVg5bO5asA26QzQT52
 1-3.8 0.0 3-5.6 3-5.6 2-2.14 8-8.4 6-2.9 12-10-13 8-1.5 19-0.3 6-2.9 25-2-12 3-2-14 28-5-10 31-11-0 33-2-8
 Scale = 1:52.6



LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR
CHORDS			
A - D	2x4	DRY	No.2
O - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMW+p	MT20	3.0	4.0	
C	TMWW-1	MT20	5.0	6.0	2.50 2.75
D	TTWW-m	MT20	5.0	6.0	Edge 3.50
E	TMWW-1	MT20	4.0	4.0	
F	TS-1	MT20	3.0	6.0	
G	TMW+w	MT20	2.0	4.0	
H	TTWW-m	MT20	5.0	6.0	Edge 3.50
I	TMWW-1	MT20	5.0	6.0	2.50 2.75
J	TMW+p	MT20	3.0	4.0	
L	BMVW-1	MT20	5.0	6.0	
M	BMWW-1	MT20	4.0	4.0	
N	BMWW-1	MT20	4.0	9.0	
O	GS-1	MT20	3.0	6.0	
P	BMWW-1	MT20	4.0	6.0	
Q	BMWW-1	MT20	4.0	4.0	
R	BMVW-1	MT20	5.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEADINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
R	VERT	DOWN	UPLIFT	IN-SX	
L	HORZ	HORZ	IN-SX	IN-SX	

UNFACTORED REACTIONS					
JT	1ST LOASE COMBINED	MAX MIN. COMPONENT REACTIONS			
R		SNOW	LIVE	PERM.LIVE	WIND
L					DEAD
					SOIL

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.
 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO				FR-TO			
A-B	0 38	-91.8	-91.8 0.12 (1)	C-D	0 110	0.03 (4)	
B-C	0 19	-91.8	-91.8 0.14 (1)	C-D	0 95	0.03 (4)	
C-D	-1907 0	-91.8	-91.8 0.19 (1)	D-P	0 1019	0.23 (1)	
D-E	-2227 0	-91.8	-91.8 0.85 (1)	E-N	-613 0	0.43 (1)	
E-F	-2225 0	-91.8	-91.8 0.84 (1)	E-N	-2 0	0.00 (1)	
F-G	-2226 0	-91.8	-91.8 0.84 (1)	N-G	-612 0	0.43 (1)	
G-H	-2226 0	-91.8	-91.8 0.64 (1)	N-H	0 1017	0.23 (1)	
H-I	-1907 0	-91.8	-91.8 0.19 (1)	M-H	0 95	0.03 (4)	
I-J	0 19	-91.8	-91.8 0.14 (1)	M-I	0 110	0.03 (4)	
J-K	0 38	-91.8	-91.8 0.12 (1)	R-C	-2137 0	0.84 (1)	
R-B	-248 0	0.0	0.0 0.03 (1)	L-L	-2137 0	0.84 (1)	
L-J	-248 0	0.0	0.0 0.03 (1)				
R-Q	0 1431	-18.5	-18.5 0.34 (1)				
Q-P	0 1507	-18.5	-18.5 0.35 (1)				
P-O	0 2227	-18.5	-18.5 0.42 (1)				
O-N	0 2227	-18.5	-18.5 0.42 (1)				
N-M	0 1608	-18.5	-18.5 0.35 (1)				
M-L	0 1431	-18.5	-18.5 0.34 (1)				

TOTAL WEIGHT = 2 X 138 = 277 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 8.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2019, OBC 2012, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 088-09, CSA 088-14
 - TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
 ALLOWABLE DEFL.(TL) = L/360 (1.08")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.68/1.00 (D-E:1), BC=0.42/1.00 (N-P:1), WB=0.84/1.00 (L-L:1), SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

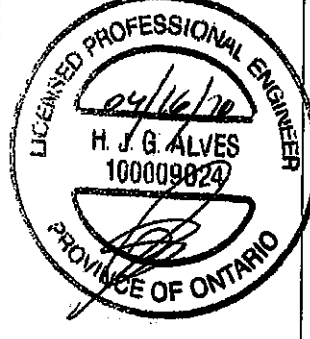
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1668

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP=0.87 (L) (INPUT = 0.90)
 SI METAL=0.74 (O) (INPUT = 1.00)



1-3-8 0-0 4-6-11 4-6-11 6-10-15 7-0-9 15-11-0
1-3-8 1-3-8 4-6-11 4-6-11 6-10-15 7-0-9 15-11-0
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23-0-1 27-4-5 4-6-11 31-11-0 33-2-6
7-0-9 4-6-11 31-11-0 33-2-6
Scale = 1:52.6



PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVV+ _p	MT20	5.0	8.0	2.50	2.00
C	TMVV+ _i	MT20	4.0	4.0	2.00	1.50
D	TTWW+ _m	MT20	5.0	6.0	2.25	1.50
E	TMV-w	MT20	2.0	4.0		
F	TTWW+ _m	MT20	5.0	8.0	2.25	1.50
G	TMVV+ _i	MT20	4.0	4.0	2.00	1.50
H	TMVV+ _p	MT20	5.0	8.0	2.50	2.00
I	BMV+ _p	MT20	3.0	4.0		
K	8MWW+ _i	MT20	5.0	8.0		
L	8MWW+ _i	MT20	4.0	4.0		
M	8S+ _t	MT20	3.0	6.0		
N	8MWWWW+ _t	MT20	4.0	9.0		
O	8S+ _t	MT20	3.0	6.0		
P	8MWW+ _i	MT20	4.0	4.0		
Q	8MWW+ _t	MT20	5.0	6.0		
R	8MV+ _p	MT20	3.0	4.0		

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

0.00
0.00
0.00
0.00
0.00
0.00
0.00

LICENSED PROFESSIONAL ENGINEER

04/16/20

H. J. G. ALVES

100009024

PROVINCE OF ONTARIO

TOTAL WEIGHT = 3 X 150 = 451 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2018 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

**CSI: TC=0.88/1.00 (D-E:1), BC=0.34/1.00 (N-P:1),
WB=0.38/1.00 (B-Q:1), SSI=0.31/1.00 (D-E:1)**

**DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10**

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20 618 354	1667 788	1987 1656

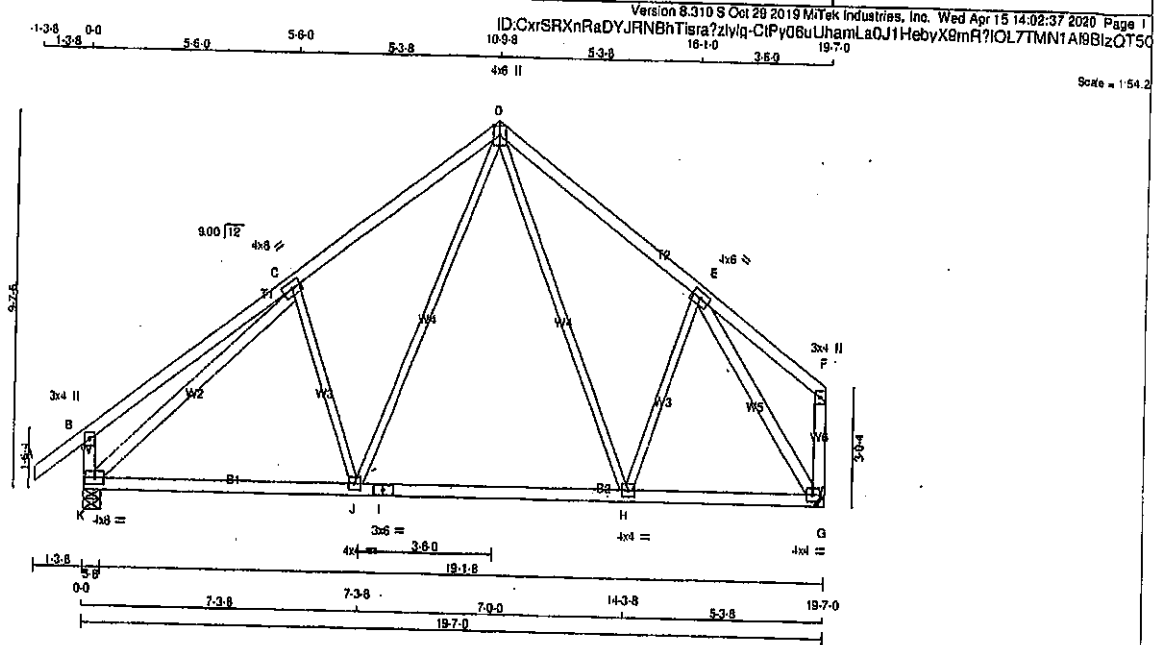
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (B) (INPUT = 0.90)
JSI METAL = 0.42 (C) (INPUT = 1.00)

Structural component only
NAI# T-2006220

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC	DRWG NO.
408146	T10	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR
CHORDS			
A - D	2x4 DRY	No.2	SPF
D - F	2x4 DRY	No.2	SPF
K - B	2x4 DRY	No.2	SPF
G - F	2x4 DRY	No.2	SPF
K - I	2x4 DRY	No.2	SPF
I - G	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
EXCEPT			
K - C	2x4 DRY	No.2	SPF
E - G	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN V X
B	TMV+p	MT20	3.0	4.0
C	TMWW-1	MT20	4.0	6.0
D	TTWW+p	MT20	4.0	6.0
E	TMWW-1	MT20	4.0	6.0
F	TMV+p	MT20	3.0	4.0
G	BMVW1-1	MT20	4.0	4.0
H	BMVW-1	MT20	4.0	4.0
I	SS-1	MT20	3.0	6.0
J	BMVW-1	MT20	4.0	4.0
K	BMVW1-1	MT20	4.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	JT	GROSS REACTION	BRG	BRG	BRG	BRG
K	VERT	K	DOWN	IN-SX	IN-SX	IN-SX	IN-SX
K	1208	K	0	0	5-8	5-8	5-8
G	1080	G	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	COMBINED	851	572.0	0.0	0.0	279.0	0.0
G		763	501.0	0.0	0.0	282.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	MAX. UNBRACED LENGTH (LC)
FR-TO				FR-TO			
A-B	0.38	-91.8	-91.8 0.12 (1)	C-J	-352.0	0.19 (1)	
B-C	0.38	-91.8	-91.8 0.44 (1)	J-D	0.516	0.12 (1)	
C-D	-985.0	-91.8	-91.8 0.38 (1)	D-H	0.157	0.04 (1)	
D-E	-797.0	-91.8	-91.8 0.31 (1)	H-E	-27.54	0.02 (4)	
E-F	0.42	-91.8	-91.8 0.34 (1)	K-C	-1208.0	0.94 (1)	
K-B	-311.0	0.0	0.0 0.03 (1)	E-G	-1141.0	0.54 (1)	
G-F	-88.0	0.0	0.0 0.01 (1)				
K-J	0.884	-18.5	-18.5 0.31 (4)				
J-I	0.559	-18.5	-18.5 0.28 (4)				
I-H	0.559	-18.5	-18.5 0.28 (4)				
H-G	0.624	-18.5	-18.5 0.20 (4)				

TOTAL WEIGHT = 96 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-00, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.65")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.65")
CALCULATED VERT. DEFL. (TL) = L/999 (0.09")

CSI: TC=0.44/1.00 (B-C:1), BC=0.31/1.00 (J-K:4), WB=0.94/1.00 (C-K:1), SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

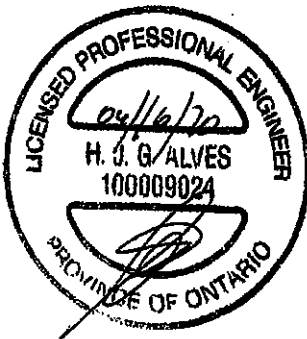
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1687 788 1887 1656

PLATE PLACEMENT TOL = 0.250 inches

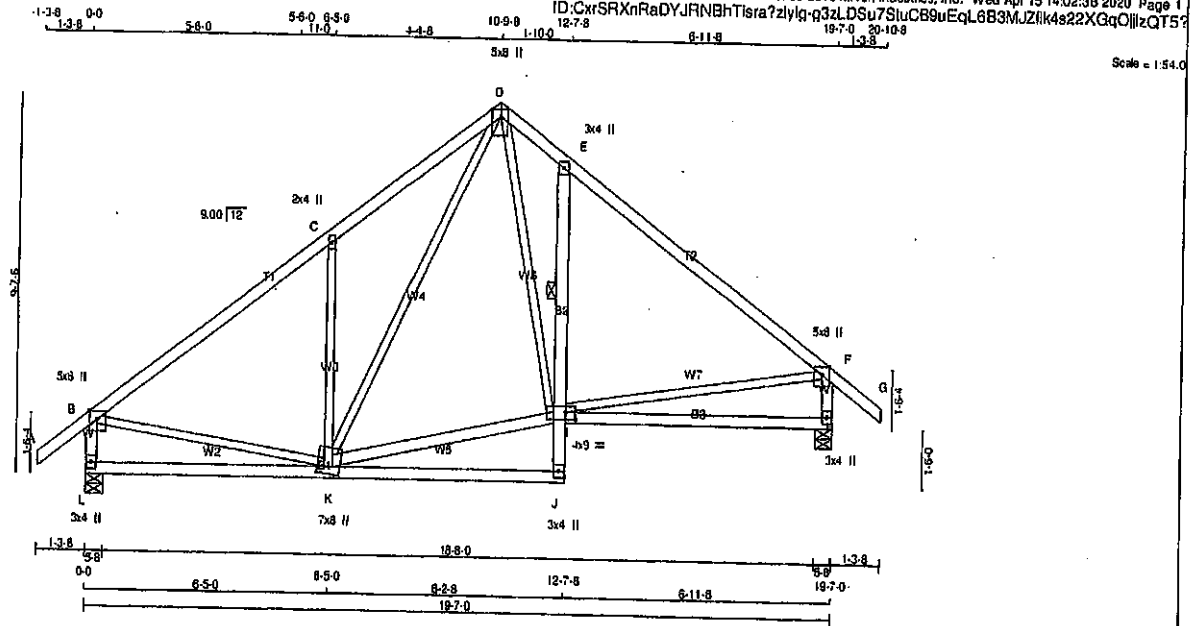
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.34 (I) (INPUT = 1.00)



Structural component only
DWG# T-20008240

JOB NAME 408146	TRUSS NAME T10S	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - E	2x4	DRY	No.2
I - H	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
EXCEPT			
K - I	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	5.0	6.0	Edge
C	TMW+w	MT20	2.0	4.0	
D	TTWW+p	MT20	5.0	8.0	Edge 2.75
E	TMV+p	MT20	3.0	4.0	
F	TMVW+p	MT20	5.0	6.0	Edge
H	BMV1+p	MT20	3.0	4.0	
I	BVMVWW-I	MT20	4.0	9.0	2.25 3.00
J	BMV+p	MT20	3.0	4.0	
K	BMMVWW+MT20	MT20	7.0	8.0	3.50 2.25
L	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
L	1208	0	1208	0	0
H	1208	0	1208	0	0

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT COMBINED					
L	851	572.0	0.0	0.0	0.0
H	851	572.0	0.0	0.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-I

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 38	B-K	0 888
B-C	-1034.0	K-C	-817.0
C-D	-1084.0	D-I	0 640
D-E	-1158.0	I-F	0 844
E-F	-988.0	F-G	0 492
F-G	0 38	G-H	0 832
H-I	-1159.0		
I-H	-1140.0		
L-K	0 0		
K-J	0 10		
J-I	0 46		
I-E	-700.0		
E-H	0 0		

TOTAL WEIGHT = 2 X 102 = 204 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPC 2011, TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.65")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.65")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.14")

CSI: TC=0.58/1.00 (E-F:1), BC=0.27/1.00 (H-I:4), WB=0.40/1.00 (C-K:1), ST=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 L3 BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HELLS OFF

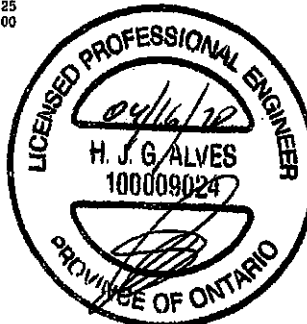
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 384 1867 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (B) (INPUT = 0.90)
JSI METAL = 0.45 (B) (INPUT = 1.00)

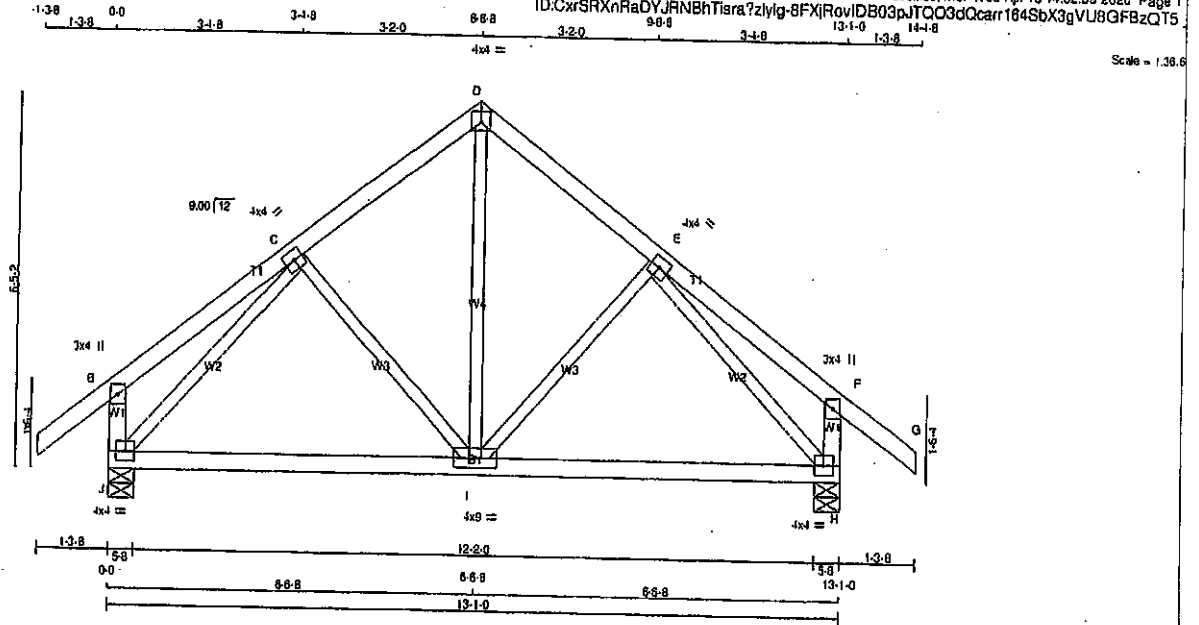


Structural component only
DWG# T-2006241

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408146	T11	1	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - G	2x4	DRY	No.2	SPF		
J - B	2x4	DRY	No.2	SPF		
H - F	2x4	DRY	No.2	SPF		
J - H	2x4	DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-1	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-1	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-1	MT20	4.0	4.0		
I	BMVWW-1	MT20	4.0	9.0		
J	BMVW1-1	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	UPLIFT
J	848	0	0	5-8
H	848	0	0	5-8

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	597	405.0	0.0	0.0	0.0	192.0	0.0
H	597	405.0	0.0	0.0	0.0	192.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
A-B	0.38	-91.8	-91.8	0.12 (1)	10.00	1-D	0.388	0.08 (1)	
B-C	0.20	-91.8	-91.8	0.15 (1)	10.00	1-E	-148.0	0.08 (1)	
C-D	-522.0	-91.8	-91.8	0.12 (1)	6.25	C-1	-148.0	0.08 (1)	
D-E	-522.0	-91.8	-91.8	0.12 (1)	6.25	J-C	-751.0	0.28 (1)	
E-F	0.20	-91.8	-91.8	0.15 (1)	10.00	E-H	-751.0	0.28 (1)	
F-G	0.38	-91.8	-91.8	0.12 (1)	10.00				
J-B	-242.0	0.0	0.0	0.03 (1)	7.81				
H-F	-242.0	0.0	0.0	0.03 (1)	7.81				
J-I	0.501	-18.5	-18.5	0.26 (4)	10.00				
I-H	0.501	-18.5	-18.5	0.26 (4)	10.00				

TOTAL WEIGHT = 60 lb (MIF)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015.

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OSC 2012, ABC 2019
- PART 9 OF OSC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.44")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.44")
CALCULATED VERT. DEFL. (TL) = L/999 (0.05")

CSI: TC=0.15/1.00 (E-F:1), BC=0.28/1.00 (J-H:4), WB=0.28/1.00 (E-H:1), SS=0.12/1.00 (D-E:1)

DOL: LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

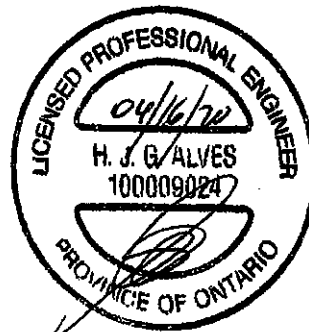
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (C) (INPUT = 0.90)
JSI METAL= 0.27 (C) (INPUT = 1.00)

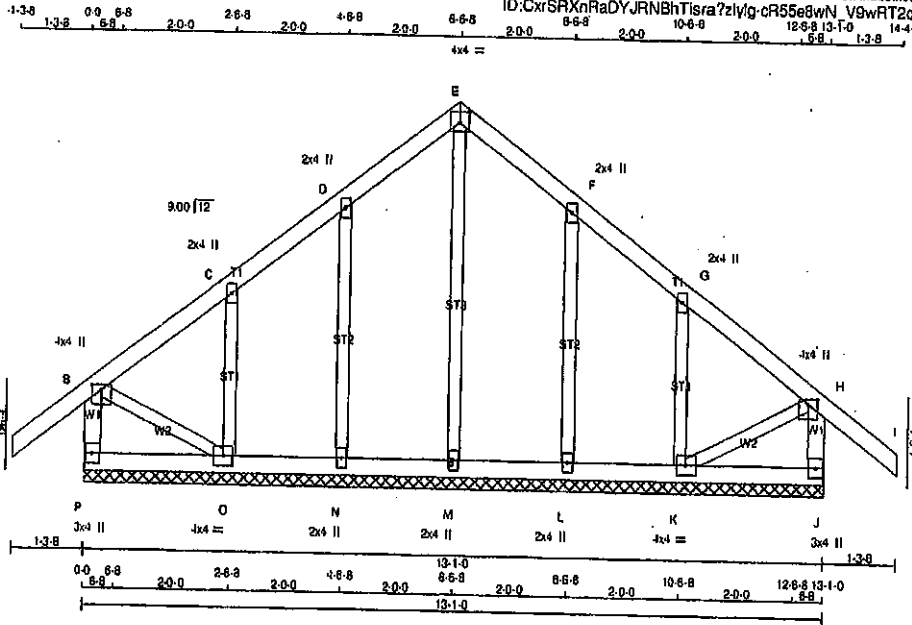


Structural component only
DWG# T-2006242

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408146	T11G	1	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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Scale = 1/32"

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER

GABLE STUDS SPACED AT 2-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G	TMVW+p	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	4.0	2.25	2.00
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMVW1+1	MT20	4.0	4.0		
L, M, N	BMV1+w	MT20	2.0	4.0		
O	BMVW1+1	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LBS)	MAX. UNBRACED LENGTH (LBS)
FR-TO				FR-TO			
A-B	0.38	-91.8	-91.8 0.12 (1)	M-E	-133.0	0.09 (1)	
B-C	-22.0	-91.8	-91.8 0.07 (1)	N-D	-194.0	0.07 (1)	
C-D	-30.0	-91.8	-91.8 0.07 (1)	O-C	-228.0	0.05 (1)	
D-E	-31.0	-91.8	-91.8 0.05 (1)	L-F	-194.0	0.07 (1)	
E-F	-31.0	-91.8	-91.8 0.05 (1)	K-G	-228.0	0.05 (1)	
F-G	-30.0	-91.8	-91.8 0.07 (1)	B-O	0.30	0.01 (1)	
G-H	-22.0	-91.8	-91.8 0.07 (1)	K-H	0.30	0.01 (1)	
H-I	0.38	-91.8	-91.8 0.12 (1)				
P-B	-252.0	0.0	0.0 0.03 (1)				
J-H	-252.0	0.0	0.0 0.03 (1)				
P-O	0.0	-18.5	-18.5 0.03 (4)				
O-N	0.21	-18.5	-18.5 0.03 (4)				
N-M	0.17	-18.5	-18.5 0.02 (4)				
M-L	0.17	-18.5	-18.5 0.02 (4)				
L-K	0.21	-18.5	-18.5 0.03 (4)				
K-J	0.0	-18.5	-18.5 0.03 (4)				

TOTAL WEIGHT = 81 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (H:1), BC=0.03/1.00 (N:O:4), WB=0.09/1.00 (E:M:1), SS=0.08/1.00 (G:H:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

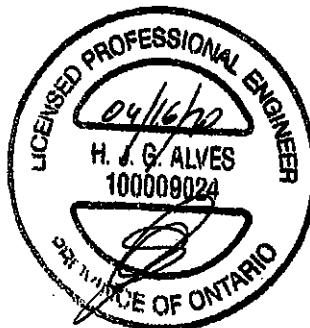
PLATE GRIP (DRY)	SHEAR SECTION (PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	818	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.20 (H) (INPUT = 0.90)

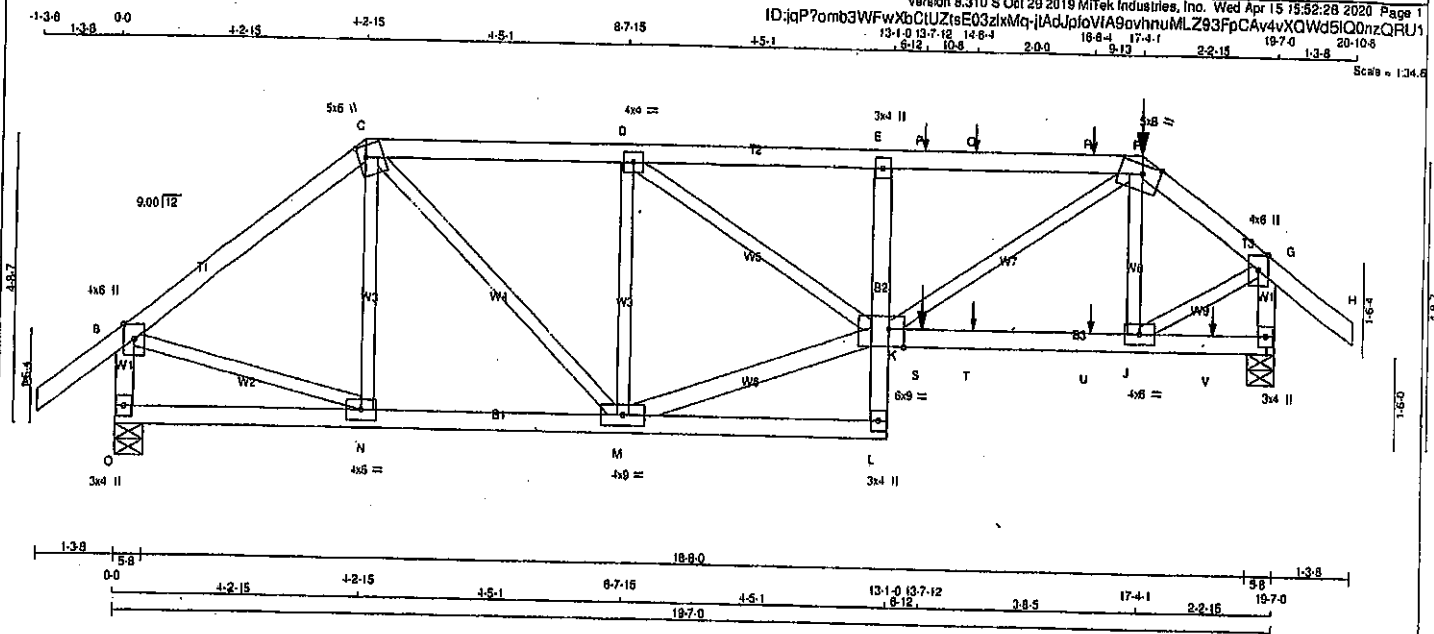
JSI METAL = 0.12 (C) (INPUT = 1.00)



Structural component only
DWG# T-2008243

JOB NAME 408147	TRUSS NAME T21S	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.
A - C 2x4 DRY No.2 SPF
C - F 2x4 DRY No.2 SPF
F - H 2x4 DRY No.2 SPF
O - B 2x4 DRY No.2 SPF
I - G 2x4 DRY No.2 SPF
O - L 2x4 DRY No.2 SPF
L - E 2x4 DRY No.2 SPF
K - I 2x4 DRY No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
M - K 2x4 DRY No.2 SPF
ORY: SEASONED LUMBER.

PLATES (table in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMVW+p MT20	4.0	6.0	Edge	
C TTWW+m MT20	5.0	6.0	2.25	1.50
D TMVW-i MT20	4.0	4.0		
E TMV+p MT20	3.0	4.0		
F TTWW-m MT20	5.0	6.0	Edge	3.50
G TMVW+p MT20	4.0	6.0	Edge	
I BMV1+p MT20	3.0	4.0		
J BMVW-i MT20	4.0	6.0		
K BMVWWW-i MT20	6.0	9.0	Edge	3.00
L BMV+p MT20	3.0	4.0		
M BMVWW-i MT20	4.0	9.0		
N BMVW-i MT20	4.0	6.0		
O BMV1+p MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
JT	1210	0	1210	0	5-8	5-8	5-8
O	1239	0	1239	0	5-8	5-8	5-8

UNFACTORED REACTIONS		MAX. MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
1ST LCASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	WIND	SOIL
JT	854	574	0	0	0	0	280	0	0
O	873	589	0	0	0	0	284	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. CS (LC)	MEMB.	FORCE (LBS)	MAX. CS (LC)	MAX. CS (LC)
FR-TO		FROM TO		FR-TO			
A-B	0.38	-91.8	-91.8 0.14 (1)	N-C	-140	20	0.05 (1)
B-C	-1043.0	-91.8	-91.8 0.35 (1)	C-M	0	573	0.14 (1)
C-D	-1236.0	-91.8	-91.8 0.32 (1)	M-D	-742	0	0.25 (1)
D-E	-1683.0	-91.8	-91.8 0.33 (1)	K-F	0	1100	0.27 (1)
E-P	-1688.0	-91.8	-91.8 0.29 (1)	J-F	-319	0	0.08 (1)
P-Q	-1688.0	-91.8	-91.8 0.29 (1)	B-N	0	683	0.21 (1)
Q-R	-1688.0	-91.8	-91.8 0.29 (1)	J-G	0	687	0.22 (1)
R-F	-1688.0	-91.8	-91.8 0.29 (1)	M-K	0	1278	0.23 (1)
F-G	-992.0	-91.8	-91.8 0.10 (1)	D-K	0	541	0.13 (1)
G-H	0.38	-91.8	-91.8 0.14 (1)				
O-B	-1178.0	0.0	0.0 0.13 (1)				
I-G	-1231.0	0.0	0.0 0.14 (1)				
O-N	0.0	-18.5	-18.5 0.09 (4)				
N-M	0.829	-18.5	-18.5 0.19 (1)				
M-L	0.30	-18.5	-18.5 0.09 (4)				
L-K	0.38	0.0	0.0 0.07 (1)				
K-E	-420.0	0.0	0.0 0.07 (1)				
K-S	0.779	-18.5	-18.5 0.19 (1)				
S-T	0.779	-18.5	-18.5 0.19 (1)				
T-U	0.779	-18.5	-18.5 0.19 (1)				
U-J	0.779	-18.5	-18.5 0.19 (1)				
J-V	0.0	-18.5	-18.5 0.08 (4)				
V-I	0.0	-18.5	-18.5 0.08 (4)				

FACTORED CONCENTRATED LOADS (LBS)		FACE		DIP		TYPE		COR	
JT	LOC.	LC1	MAX.	FRONT	VERT	DEAD	SNOW	TYPE	COR
F	17-4-1	-8	-7	---	---	---	---	---	---
P	17-4-1	-30	-30	---	---	---	---	---	---
Q	13-7-12	1	1	118	FRONT	VERT	TOTAL	---	---
R	14-6-4	1	1	118	FRONT	VERT	TOTAL	---	---
S	16-8-4	1	1	109	FRONT	VERT	TOTAL	---	---
T	13-7-12	-1	-1	---	FRONT	VERT	TOTAL	---	---
U	14-6-4	1	1	---	FRONT	VERT	TOTAL	---	---
V	16-8-4	1	1	---	FRONT	VERT	TOTAL	---	---

FACTORED CONCENTRATED LOADS (LBS)						
LOC.	LC1	MAX.	MAX+	FACE	DIP	PROTYPE
17-4-1	-8	-7	---	FRONT	VERT	DEAD
17-4-1	-30	-30	---	FRONT	VERT	SNOW
13-7-12	1	1	118	FRONT	VERT	TOTAL
14-6-4	1	1	118	FRONT	VERT	TOTAL
18-6-4	1	1	109	FRONT	VERT	TOTAL
13-7-12	-1	-1	---	FRONT	VERT	TOTAL
14-6-4	1	1	---	FRONT	VERT	TOTAL
18-6-4	1	1	---	FRONT	VERT	TOTAL
18-6-4	1	1	---	FRONT	VERT	TOTAL

TOTAL WEIGHT = 89 lb

DESIGN CRITERIA
*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF
SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.65")
CALCULATED VERT. DEFL. (LL) = 0.999 (0.05")
ALLOWABLE DEFL. (TL) = L/360 (0.65")
CALCULATED VERT. DEFL. (TL) = 0.999 (0.09")

CS1: TC=0.35/1.00 (B-C:1), BC=0.19/1.00 (M-N:1), WB=0.27/1.00 (F-K:1), SS=0.21/1.00 (C-D:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00
COMPANION LIVE LOAD FACTOR = 1.00

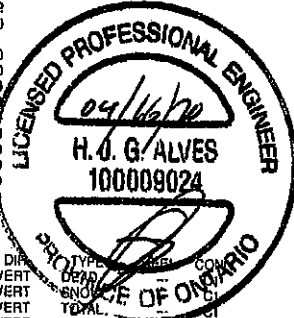
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1686

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.55 (B) (INPUT = 1.00)

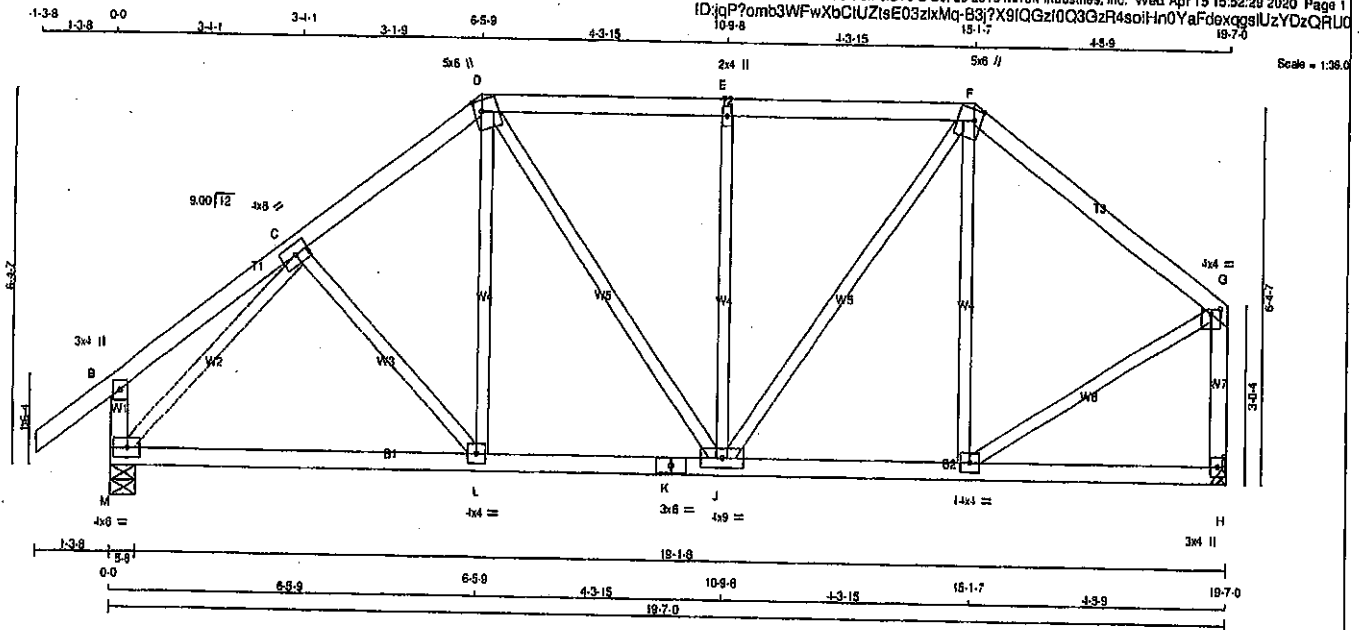
Structural component only
DWG# T-2008247



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408147	T22	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/32"

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	8.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW-p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	4.0		
J	BMVW-w	MT20	4.0	4.0		
K	BS-t	MT20	3.0	8.0		
L	BMVW-t	MT20	4.0	4.0		
M	BMVW-t	MT20	4.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
M	1206 0	1206 0	5-8	5-8
H	1080 0	1080 0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
M	COMBINED	572 0	0 0	0 0	279 0	0 0
H	COMBINED	501.0	0 0	0 0	262 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO			FROM TO			FR-TO			
A-B	0 38	-91.8	-91.8 0.12 (1)	10.00	C-L	-50 24	0.02 (1)		
B-C	0 19	-91.8	-91.8 0.14 (1)	10.00	L-D	0 162	0.04 (4)		
C-D	-989 0	-91.8	-91.8 0.12 (1)	6.13	D-J	0 191	0.04 (1)		
D-E	-694 0	-91.8	-91.8 0.22 (1)	8.24	J-E	-483 0	0.32 (1)		
E-F	-694 0	-91.8	-91.8 0.22 (1)	8.24	J-F	0 469	0.11 (1)		
F-G	-785 0	-91.8	-91.8 0.24 (1)	8.25	I-F	-270 0	0.18 (1)		
M-B	-242 0	0.0	0.0 0.03 (1)	7.81	M-C	-1225 0	0.45 (1)		
H-G	-1046 0	0.0	0.0 0.17 (1)	7.69	I-G	0 727	0.16 (1)		
M-L	0 315	-18.5	-18.5 0.23 (4)	10.00					
L-K	0 784	-18.5	-18.5 0.24 (4)	10.00					
K-J	0 784	-18.5	-18.5 0.24 (4)	10.00					
J-I	0 822	-18.5	-18.5 0.18 (1)	10.00					
I-H	0 0	-18.5	-18.5 0.10 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.8 PSF
DL	= 6.0 PSF
BOY CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65')
CALCULATED VERT. DEFL.(LL) = L/999 (0.02')
ALLOWABLE DEFL.(TL) = L/360 (0.65')
CALCULATED VERT. DEFL.(TL) = L/999 (0.07')

CSF: TC=0.24/1.00 (F/G:1), BC=0.24/1.00 (J-L:4),
WB=0.45/1.00 (C-M:1), SS=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

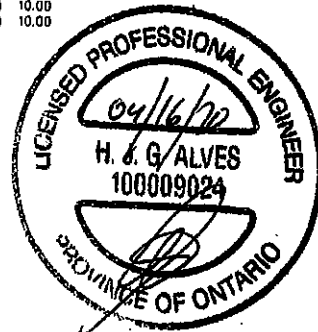
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1887	798	1887	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

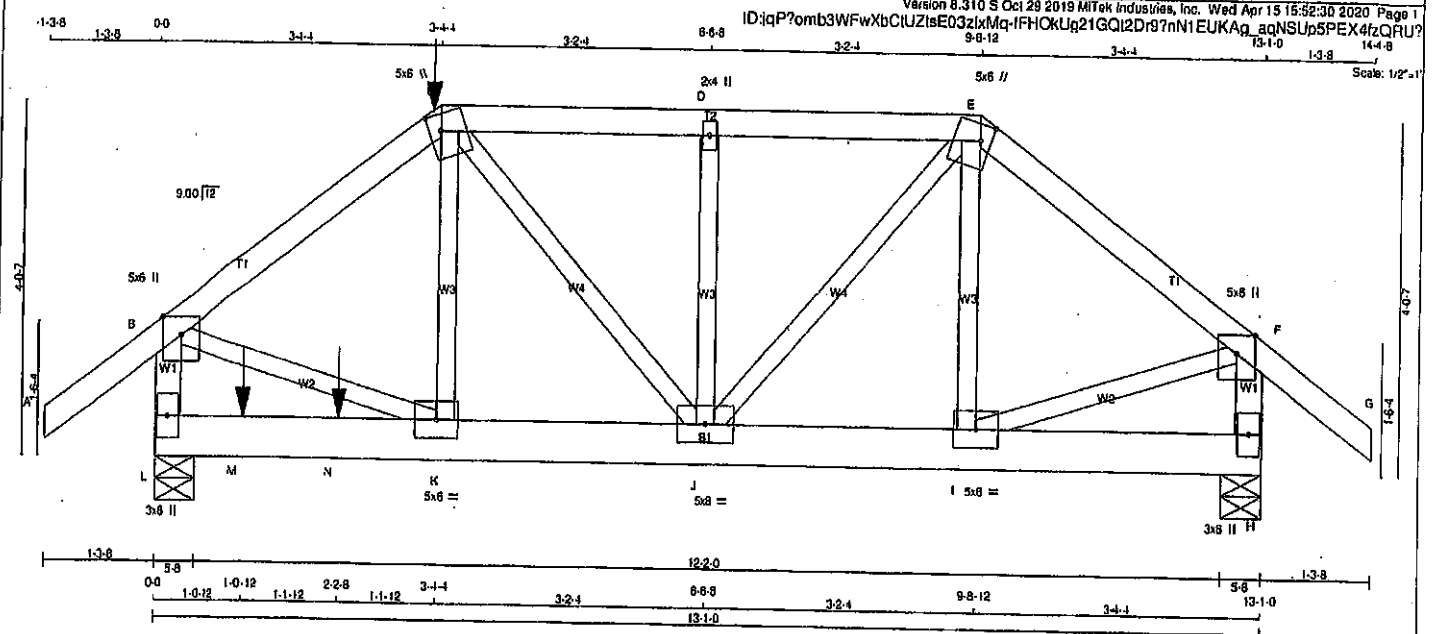
JSI GRIP = 0.89 (G) (INPUT = 0.90)

JSI METAL = 0.30 (C) (INPUT = 1.00)



Structural component only
DWG# T-2006248

JOB NAME 408147	TRUSS NAME T23	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



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Scale: 1/2"=1'

LUMBER				DESIGN CRITERIA			
N. L. G. A. RULES				*** SPECIAL LOADS ANALYSIS ***			
CHORDS	SIZE	LUMBER	DESCR.	GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.			
A - C	2x4	DRY	No.2	LOADS WERE DERIVED FROM USER INPUT			
C - E	2x4	DRY	No.2	NO FURTHER MODIFICATIONS WERE MADE			
E - G	2x4	DRY	No.2				
L - B	2x4	DRY	No.2				
H - F	2x4	DRY	No.2				
L - H	2x6	DRY	No.2				
ALL WEBS EXCEPT	2x3	DRY	No.2				

FACTORED REACTIONS				UNFACTORED REACTIONS			
JT	VERT	HORZ	DOWN	JT	VERT	HORZ	DOWN
L	1470	0	1470	L	1033	717	0
H	985	0	985	H	693	475	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS L, H
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED LENGTH (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LENGTH (LBS)	
FR-TO				FR-TO			
A-B	0:38	-91.8	-91.8 0.14 (1)	K-C	0:328	0.08 (1)	
B-C	-1140:0	-91.8	-91.8 0.22 (1)	C-J	-29:0	0.01 (1)	
C-D	-900:0	-91.8	-91.8 0.17 (1)	J-D	-347:0	0.09 (1)	
D-E	-900:0	-91.8	-91.8 0.17 (1)	E-J	0:412	0.10 (1)	
E-F	-791:0	-91.8	-91.8 0.21 (1)	I-E	-108:17	0.03 (1)	
F-G	0:38	-91.8	-91.8 0.14 (1)	B-K	0:955	0.24 (1)	
L-B	-1259:0	0.0	0.0 0.14 (1)	I-F	0:683	0.16 (1)	
H-F	-960:0	0.0	0.0 0.11 (1)				
L-M	0:0	-18.5	-18.5 0.30 (1)				
M-N	0:0	-18.5	-18.5 0.30 (1)				
N-K	0:0	-18.5	-18.5 0.30 (1)				
K-J	0:919	-18.5	-18.5 0.27 (1)				
J-I	0:628	-18.5	-18.5 0.13 (1)				
I-H	0:0	-18.5	-18.5 0.03 (4)				

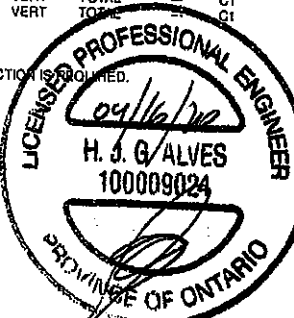
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.
TOTAL LOAD CASES: (4)
MAXIMUM FACTORED REACTION: 1470 LBS
MAXIMUM FACTORED REACTION: 985 LBS
MAXIMUM FACTORED REACTION: 1033 LBS
MAXIMUM FACTORED REACTION: 693 LBS

FACTORED CONCENTRATED LOADS (LBS)				CONNECTION REQUIREMENTS			
JT	LOC	LC1	MAX. MAX.	1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.			
C	3-4.4	-19	-21				
C	3-4.4	-90	-90				
M	1-0.12	-7	-7				
N	2-2.8	-844	-844				

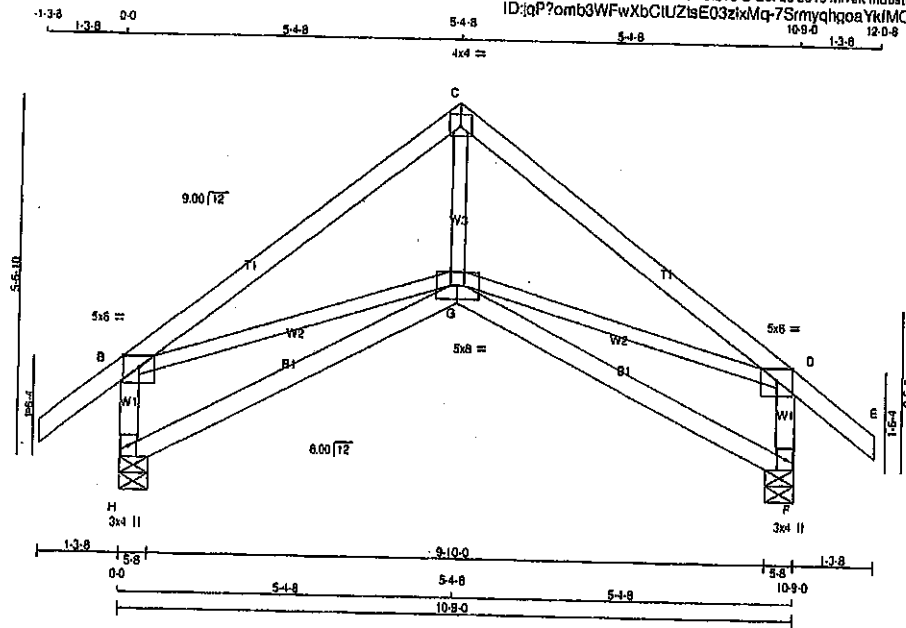
ALLOWABLE DEFLECTION (LL) = L/360 (0.44")
CALCULATED VERT. DEFLECTION (LL) = L/999 (0.02")
ALLOWABLE DEFLECTION (TL) = L/360 (0.44")
CALCULATED VERT. DEFLECTION (TL) = L/999 (0.03")
CS: TC=0.22 (1.00) (B-C:1), BC=0.30 (1.00) (K-L:1)
WB=0.24 (1.00) (B-K:1), SS=0.33 (1.00) (K-L:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP (P) SHEAR SECTION (PL)
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 789 1987 1656
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.84 (F) (INPUT = 0.90)
JSI METAL=0.46 (S) (INPUT = 1.00)
Structural component only



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408147	T24S	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Wed Apr 15 15:52:31 2020 Page 1					
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10-0-0 12-0-8					
Scale = 1:32.0					



LUMBER			
N. L. O. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - G	2x4	DRY	No.2
G - F	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	6.0	Edge
C	TTW-p	MT20	4.0	4.0	2.25 2.00
D	TMVW-p	MT20	5.0	6.0	Edge
F	BMV1-p	MT20	3.0	4.0	
G	BBWW-p	MT20	5.0	8.0	
H	BMV1-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
H	719 0	719 0	0 0	5-8	5-8
F	719 0	719 0	0 0	5-8	5-8

UNFACTORED REACTIONS		1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
H	506	346 0	0 0
F	506	346 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS: H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PL)	MAX. CS (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 38	-91.8	-91.8 0.12 (1)	G-C	0 376
B-C	-724 0	-91.8	-91.8 0.35 (1)	B-G	0 598
C-D	-724 0	-91.8	-91.8 0.35 (1)	G-D	0 598
D-E	0 38	-91.8	-91.8 0.12 (1)		
H-B	-689 0	0 0	0 0 0.07 (1)		
F-D	-689 0	0 0	0 0 0.07 (1)		
H-G	0 0	-18.5	-18.5 0.16 (4)		
G-F	0 0	-18.5	-18.5 0.16 (4)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCG 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN (LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.35/1.00 (B-C:1), BC=0.16/1.00 (G-H:4), WB=0.13/1.00 (B-G:1), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

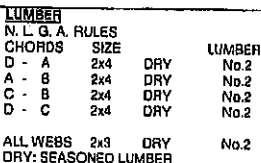
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.67 (C) (INPUT = 0.99)
JSI METAL = 0.19 (F) (INPUT = 1.00)



Structural component only
DWG# T-2008250

Tamarack Roof Truss, Burlington



DESCR.
SPF
SPF
SPF
SPF

BEARINGS

JT D C	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQ'D BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	328	0	328	0	0	5-8	5-8
	328	0	328	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

TOTAL WEIGHT = 6 X 19 = 117 lb
(M/R)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	48.0	PSF

SPACING 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.63/1.00 (A-B:1), BC=0.17/1.00 (C-D:4).
WB=0.00/1.00 (A-C:1), SS=0.29/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

	PLATE GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	818	354	1667	788	1987	1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

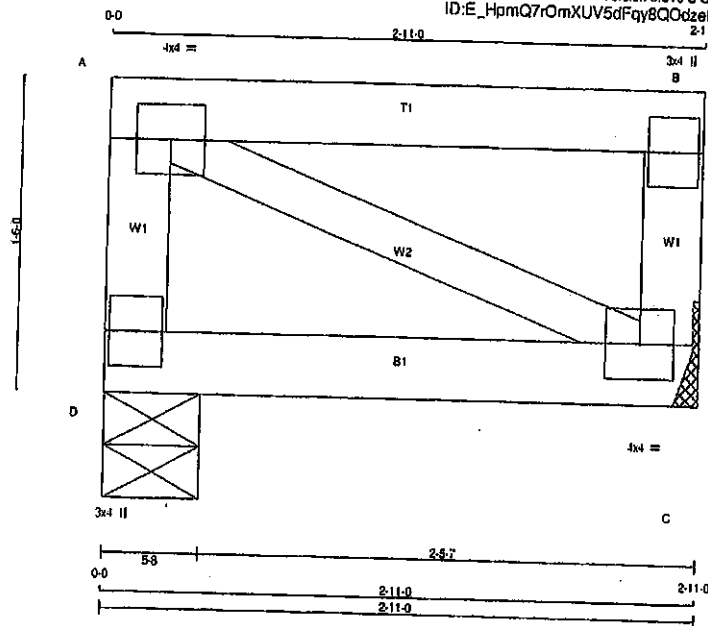
JSI GRIP= 0.22 (A) (INPUT = 0.90)
JSI METAL= 0.06 (A) (INPUT = 1.00)



Structural component only
DWG# T-2006251

JOB NAME 408148	TRUSS NAME T33	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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2-11-0



Scale = 1/10.1

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
D - A	2x4	DRY No.2
A - B	2x4	DRY No.2
C - B	2x4	DRY No.2
D - C	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DESCR.
SPF
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
D	226	0	226	0
C	226	0	226	0

MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	168	75/0	0/0	0/0	0/0	0/0	92/0	0/0
C	168	75/0	0/0	0/0	0/0	0/0	92/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CS (LBS)	MAX. FACTORED UNBRACED LENGTH (LBS)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LBS)
FR-TO							
D-A	-199	0	0.0	0.03 (1)	7.81	0	0.00 (1)
A-B	0	0	-138.8	-138.8	0.30 (1)	10.00	
C-B	-199	0	0.0	0.03 (1)	7.81		
D-C	0	0	-18.5	-18.5	0.07 (4)	10.00	

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 15.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 48.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2018 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

15% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (TL) = L/360 (0.18")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.30/1.00 (A-B:1), BC=0.07/1.00 (C-D:1)
WB=0.00/1.00 (A-C:1), SS=0.24/1.00 (A-B:1)

DOL LUMBER=0.98 NAIL=0.98 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

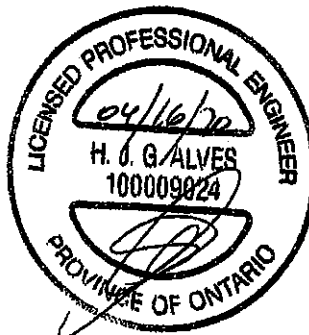
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1867
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.17 (A) (INPUT = 0.90)
JSI METAL = 0.04 (A) (INPUT = 1.00)

Structural component only
DWG# T-2008252

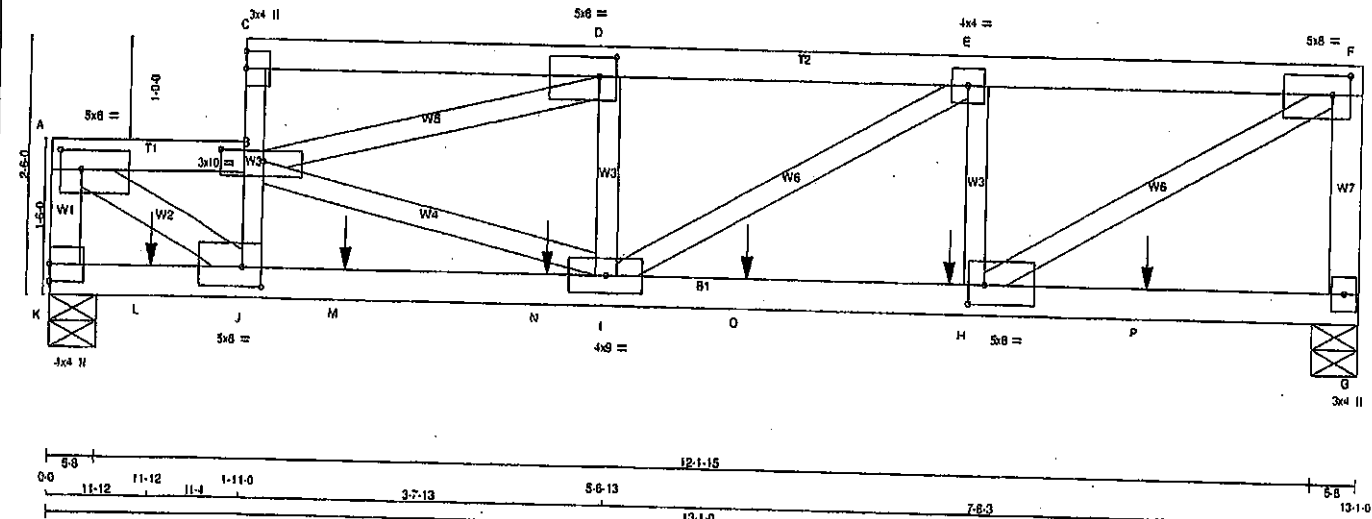


JOB NAME 408148	TRUSS NAME T34	QUANTITY 1	PLV 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:20.5



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
K - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
J - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - G	2x4	DRY	1850F 1.5E

ALL WEBS 2x3 DRY No.2
EXCEPT
A - J 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (Table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-I	MT20	5.0	8.0	2.25	2.50
B	TMVW-W	MT20	3.0	10.0	1.25	5.25
C	TMV-W	MT20	3.0	4.0		
D	TMVW-I	MT20	5.0	8.0	2.25	2.00
E	TMVW-I	MT20	4.0	4.0		
F	TMVW-I	MT20	5.0	8.0	2.25	2.00
G	BMV1-p	MT20	3.0	4.0		
H	BMVW-I	MT20	5.0	8.0	2.25	2.00
I	BMVW-W	MT20	4.0	9.0		
J	BMVW-I	MT20	5.0	8.0	2.25	2.50
K	BMV1-p	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
K	1773	0	0	5-8
G	1711	0	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	1280	895	0	0	0	585	0
G	1233	878	0	0	0	565	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
K-A	-1889 0	0.0 0.0	0.25 (1)	5.82	A-J	0 3408	0.80 (1)
A-B	-2929 0	-114.3 -114.3	0.31 (1)	3.28	B-D	-2838 0	1.00 (1)
J-B	-1293 0	0.0 0.0	0.39 (1)	4.88	B-I	-74 0	0.03 (1)
B-C	-170 0	0.0 0.0	0.32 (1)	7.81	I-D	0 412	0.14 (1)
C-D	-72 0	-114.3 -114.3	0.25 (1)	6.25	I-E	0 451	0.15 (1)
D-E	-2799 0	-114.3 -114.3	0.46 (1)	3.19	H-E	-702 0	0.18 (1)
E-F	-2416 0	-114.3 -114.3	0.47 (1)	3.40	H-F	0 2785	0.92 (1)
G-F	-1572 0	0.0 0.0	0.28 (1)	5.80			
K-L	0 0	-18.5 -18.5	0.14 (1)	10.00			
L-J	0 0	-18.5 -18.5	0.14 (1)	10.00			
J-M	0 2849	-18.5 -18.5	0.80 (1)	10.00			
M-N	0 2849	-18.5 -18.5	0.80 (1)	10.00			
N-I	0 2849	-18.5 -18.5	0.80 (1)	10.00			
I-O	0 2416	-18.5 -18.5	0.71 (1)	10.00			
O-H	0 2416	-18.5 -18.5	0.71 (1)	10.00			
H-P	0 0	-18.5 -18.5	0.42 (1)	10.00			
P-G	0 0	-18.5 -18.5	0.42 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
H	8-11-12	-308	-308	---	FRONT	VERT	TOTAL	C1
L	11-12	-208	-208	---	FRONT	VERT	TOTAL	C1
M	2-11-12	-308	-308	---	FRONT	VERT	TOTAL	C1
N	4-11-12	-308	-308	---	FRONT	VERT	TOTAL	C1
O	6-11-12	-308	-308	---	FRONT	VERT	TOTAL	C1
P	10-11-12	-308	-308	---	FRONT	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

TOTAL WEIGHT = 52 lb (M/R)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
DL	=	15.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	48.0	PSF	

SPACING = 24.0 IN/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.44")
CALCULATED VERT. DEFL. (LL) = L/999 (0.07")
ALLOWABLE DEFL. (TL) = L/360 (0.44")
CALCULATED VERT. DEFL. (TL) = L/994 (0.16")

CSL: TC=0.47/1.00 (E-F:1), BC=0.80/1.00 (I-J:1), WB=1.00/1.00 (B-D:1), SS=0.45/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	818	354	1687	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

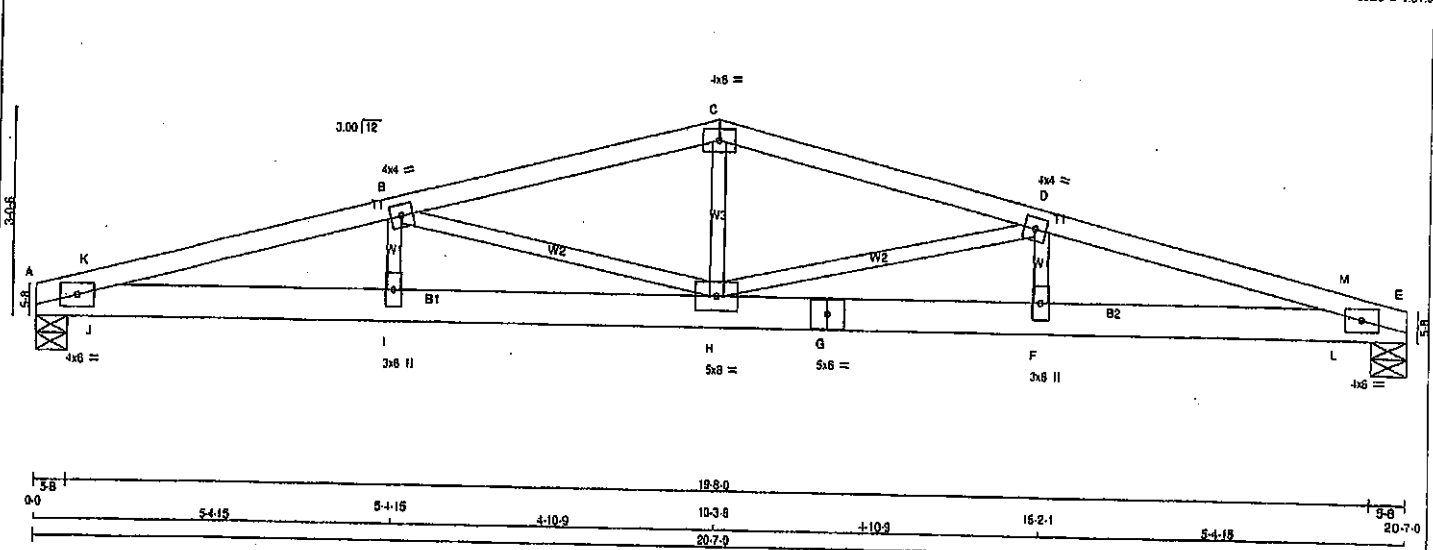
JSI GRIP = 0.90 (F) (INPUT = 0.90)
JSI METAL = 0.77 (D) (INPUT = 1.00)

Structural component only
DWG# T-2002020



JOB NAME 408148	TRUSS NAME T35	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Wed Apr 15 15:57:12 2020 Page 1
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 20-7-0
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WOOD		CHORDS		WEBS		PLATES		ALL WEBS		DRY, SEASONED LUMBER.	
N. L. G. A. RULES		CHORDS	SIZE	WEBS	SIZE	PLATES	TYPE	ALL WEBS	2x3	DRY	No.2
A - C	2x4	DRY	No.2								
C - E	2x4	DRY	No.2								
G - E	2x8	DRY	No.2								
G - E	2x8	DRY	No.2								

BEARINGS		UNFACTORED REACTIONS		BRACING		LOADING		DESIGN CRITERIA	
JT	VERT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	JT	VERT	TOTAL LOAD CASES: (4)		SPECIFIED LOADS:	
A	1135	0	1135	A	802	0		TOP CH. LL = 25.8 PSF	
E	1135	0	1135	E	802	0		BOT CH. LL = 8.0 PSF	
								DL = 0.0 PSF	
								DL = 7.4 PSF	
								TOTAL LOAD = 39.0 PSF	

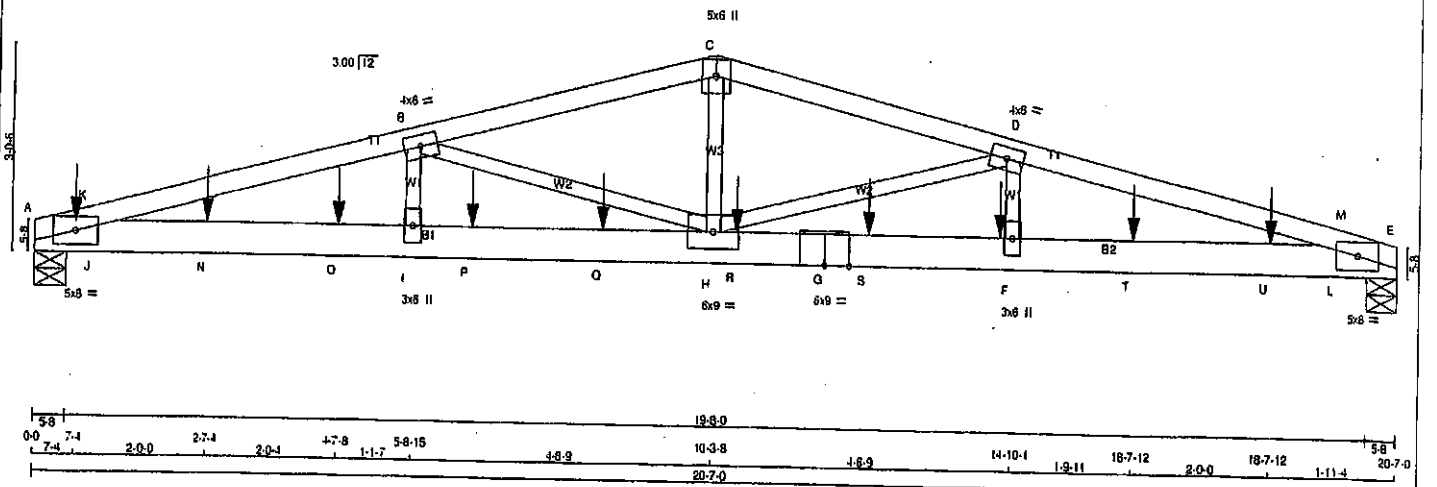
CHORDS		WEBS		DESIGN CRITERIA	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)	THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015	
FR-TO	FROM TO	FR-TO	FROM TO		
A-K	-3281.0	H-C	0 748	THIS DESIGN COMPLIES WITH: - PART 9 OF NBCC 2010, CBC 2012, ABC 2019 - PART 9 OF CBC 2012 (2019 AMENDMENT) - CSA 086-09, CSA 086-14 - TPIC 2011, TPIC 2014	
K-B	-3223.0	H-D	-921.0		
B-C	-2326.0	F-D	-9 89	(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD	
C-D	-2326.0	B-H	-921.0		
D-M	-3223.0	I-B	-9 89	ALLOWABLE DEFL.(LL) = L/360 (0.69") CALCULATED VERT. DEFL.(LL) = L/999 (0.15") ALLOWABLE DEFL.(TL) = L/360 (0.69") CALCULATED VERT. DEFL.(TL) = L/873 (0.28")	
M-E	-3281.0	J-K	-28.43		
		L-M	-28.43	CS: TC=0.33/1.00 (B-C:1), BC=0.62/1.00 (I-J:1), WB=0.40/1.00 (B-H:1), SSI=0.20/1.00 (B-C:1)	
A-J	0 3132				
J-I	0 3132			DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10	
I-H	0 3132				
H-G	0 3132			COMPANION LIVE LOAD FACTOR = 1.00	
G-F	0 3132				
F-L	0 3132			TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.	
L-E	0 3132				

NAIL VALUES		PLATE GRIP(DRY) SHEAR SECTION		PLATE PLACEMENT TOL.	
PLATE	GRIP(DRY)	SHEAR	SECTION	PLATE ROTATION TOL. = 5.0 Deg.	
(PSI)	(PLI)	(PLI)	(PLI)		
MT20	818	354	1687 788 1887 1656	JSI GRIP = 0.76 (E) (INPUT = 0.90) JSI METAL = 0.73 (A) (INPUT = 1.00)	

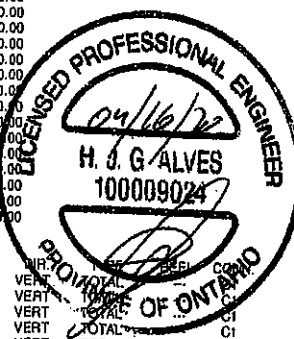


Structural component only
 DWG# T-2006254

JOB NAME 408148	TRUSS NAME T35Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Wed Apr 15 15:57:13 2020 Page 1	
ID: CxrSRXnRaDYJRNbHtIsra?zlylg-tluN8G5ZOD4jVizY4jOC9YR733KANRTAxH7WmzQRP				20-7-0	
Scale = 1/31.0					



LUMBER N.L.G.A. RULES CHORDS SIZE LUMBER DESCR. SPF A-C 2x4 DRY No.2 SPF C-E 2x4 DRY No.2 SPF A-G 2x6 DRY 1650F 1.5E SPF G-E 2x6 DRY 1650F 1.5E SPF ALL WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER.				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 25.6 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 38.0 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, OBC 2012, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-09, CSA 086-14 - TRC 2011, TRC 2014 (55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL. (LL) = L/360 (0.89") CALCULATED VERT. DEFL. (LL) = L/981 (0.25") ALLOWABLE DEFL. (TL) = L/360 (0.89") CALCULATED VERT. DEFL. (TL) = L/559 (0.44") CSI: TC=0.88/1.00 (B-K:1), BC=0.87/1.00 (I-J:1), WB=0.88/1.00 (B-H:1), CSI=0.33/1.00 (F-H:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE RIGHT HEEL ONLY TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 788 1987 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.85 (C) (INPUT = 0.90) JSI METAL= 0.87 (E) (INPUT = 1.00)			
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING(IN) LOAD(PLF) TOP CHORDS : (0.122"x3") SPIRAL NAILS A-C 1 12 TOP C-E 1 12 TOP BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS A-G 2 12 SIDE(0.0) G-E 2 12 SIDE(183.1) WEBS : (0.122"x3") SPIRAL NAILS 2x3 1 6 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY. SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.				FACTORED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX. MAX. FACE F 14-7-12 -507 -507 --- FRONT VERT J 7-4 -510 -510 --- FRONT VERT N 2-7-4 -507 -507 --- FRONT VERT O 4-7-8 -507 -507 --- FRONT VERT P 6-7-12 -507 -507 --- FRONT VERT Q 8-7-12 -507 -507 --- FRONT VERT R 10-7-12 -507 -507 --- FRONT VERT S 12-7-12 -507 -507 --- FRONT VERT T 16-7-12 -387 -387 --- TOP VERT U 18-7-12 -265 -265 --- TOP VERT			
PLATES (table in inches) JT TYPE PLATES W LEN Y X A TMB1-1 MT20 5.0 8.0 B TMWW-1 MT20 4.0 8.0 C TTW+p MT20 5.0 8.0 D TMWW-1 MT20 4.0 8.0 E TMB1-1 MT20 5.0 8.0 F BMWW+w MT20 3.0 8.0 G BS-1 MT20 6.0 9.0 H BMWWWW-1 MT20 6.0 9.0 I BMWW+w MT20 3.0 8.0				CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.			

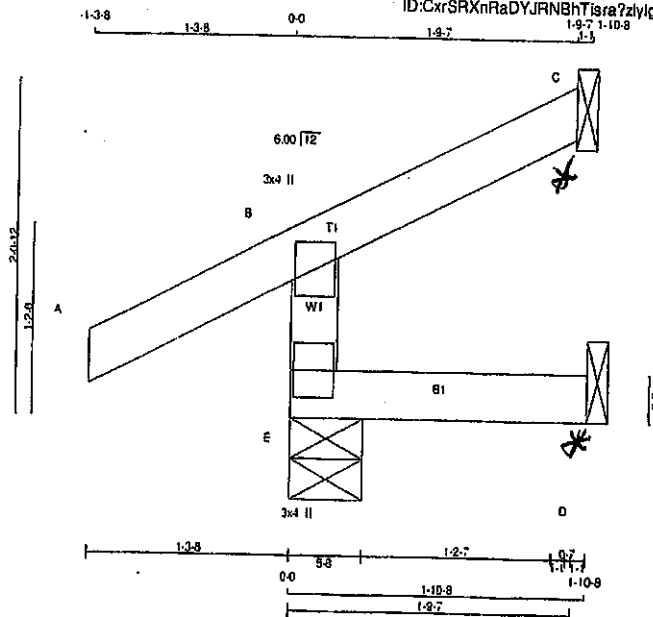


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408146	J1	2	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Wed Apr 15 14:02:18 2020 Page 1
ID: CxrSRXnRaDYJRNbTisra7zlylg-FDmn1dAaK2o5MOg8UG8UGsez6A6lagjY10vzQT5J

Scale = 1/16"



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	DOWN	UPLIFT	IN-SX
E	271	0	0	5-8
C	45	0	23	1-8
D	8	0	2	1-8

SEE MITTEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 160 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 160 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE			
E	188	141	0	0	0	0
C	31	24	-18	0	0	0
D	7	0	-8	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1(LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1(LC)
FR-TO		FROM TO			FR-TO		
E-B	-244	0.0	0.0	0.04 (5)			
A-B	0	28	-91.8	-91.8			
B-C	-17	0	-91.8	-91.8			
E-D	0	0	-18.5	-18.5			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 7 = 14 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
OL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/380 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:1)
WB=0.00/1.00 (m/a:0), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	{PSI}		{PLI}		{PLI}	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1687	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E); INPUT= 0.80 ;
JSI METAL= 0.07 (B); INPUT= 1.00 ;

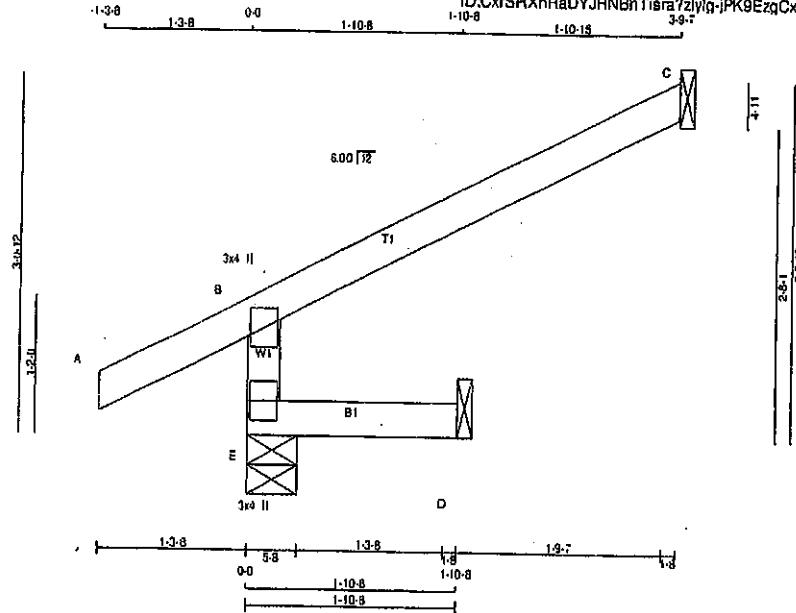


Structural component only
DWG# T-2006223

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408146	J2	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
TRUSS DESC.					

Version 8.310 S Oct 29 2018 Mitek Industries, Inc. Wed Apr 15 14:02:19 2020 Page 1
ID: CxrSRXnRaDYJRNbHtTara?zlylg-jPK9EzqCx2VvQFxaDsGkoM0P_2JqvYYkuNICYLzQT5

Scale = 1:18.2



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - 8	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - 0	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
E	381	0	381	0	0	5.8	5.8	5.8	5.8
C	130	0	130	0	0	1.8	1.8	1.8	1.8
D	16	0	17	0	0	1.8	1.8	1.8	1.8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS		1ST LCASE		MAX. MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
E	250	190	0	0	0	0	0	E	250	190	0	0	0
C	90	73	0	0	0	0	0	C	90	73	0	0	0
D	12	0	0	0	0	0	0	D	12	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		W E B S	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (PLF)	MEMB.	FORCE (LBS)
E-B	-342	0	0.0	E-B	-342
A-B	0	20	-91.8	A-B	0
B-C	-19	0	-91.8	B-C	-19
E-D	0	0	-18.5	E-D	0

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

55% OF 31.3 P.S.F. Q.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CS: TC=0.22/1.00 (B-C:1), BC=0.02/1.00 (D-E:1), WB=0.00/1.00 (m:0), SS=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	818 354	1687 788 1687 1656

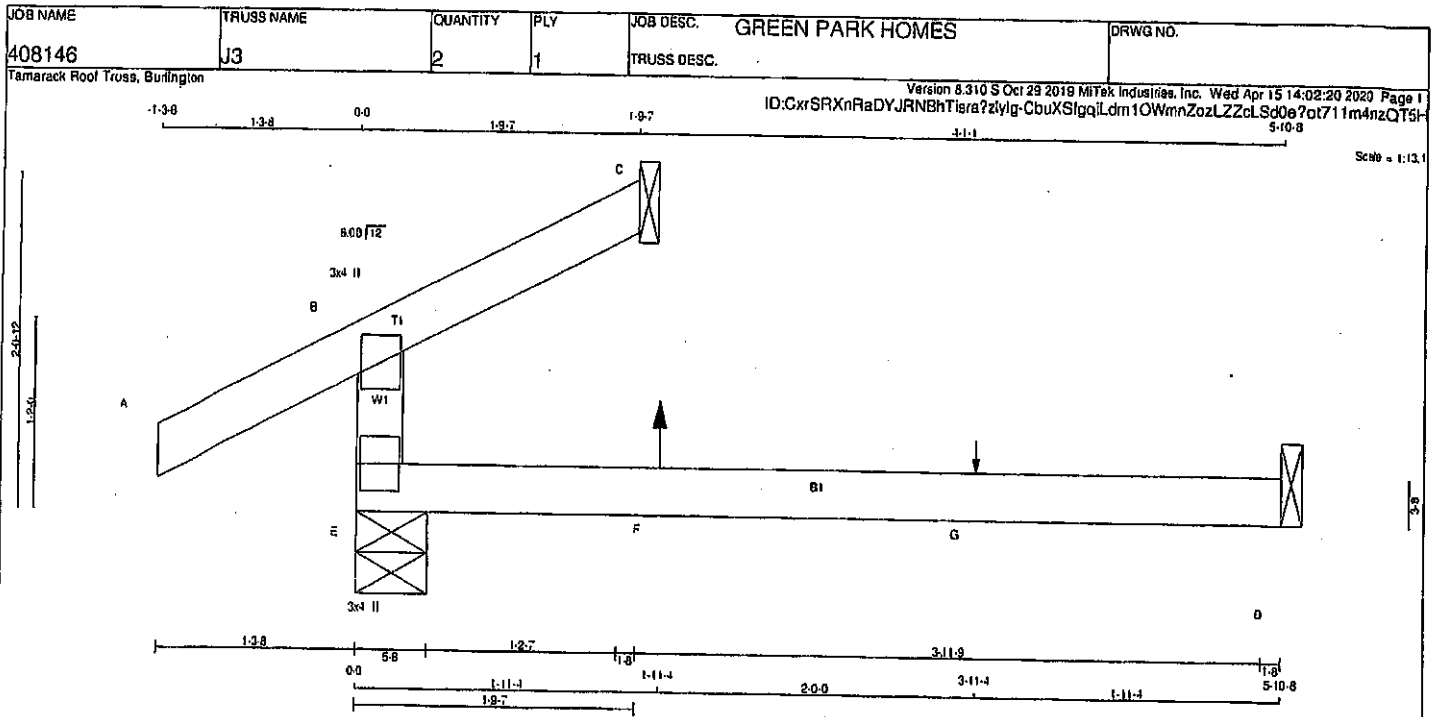
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.14 (E:1) INPUT = 0.90
JSI METAL = 0.09 (B:1) INPUT = 1.00



Structural component only
DWG# T-2006224



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (1/4" to 1/2" in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	284	0	284	0	5-8	5-8
C	83	0	83	0	1-8	1-8
D	44	0	52	0	1-8	1-8

SEE MITEK STANDARD DETAIL 89791H FOR CONNECTION TO JOINTS C, D

UNFACTORED REACTIONS								
1ST CASE		MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
E	200	137 0	0 0	0 0	0 0	82 0	0 0	
C	46	21 0	0 0	0 0	0 0	25 0	0 0	
D	35	0 3	0 0	0 0	0 0	37 0	0 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS			WEBS					
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CS1 (LC)
FR-TO		FROM TO			FR-TO			
E-B	227 0	0.0	0.0 (11'4)	7.81				
A-B	0 28	-91.8	-91.8 (12'11)	10.00				
B-C	9 9	-91.8	-91.8 (08'4)	10.00				
E-F	0 0	-18.5	-18.5 (14'4)	10.00				
F-G	0 0	-18.5	-18.5 (14'4)	10.00				
G-D	0 0	-18.5	-18.5 (14'4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX	MAX+	FACE	DIR	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	BACK	VERT	TOTAL		C1
G	3-11-4	1	1		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

11 C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 12 = 23.0

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.04")

CSK: TC=0.12/1.00 (A-B:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (m:a:0), SS=0.09/1.00 (A-B:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1 10
COMP=1.10 SHEAR=1.10 TENS=1 10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1887 1650

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.10 (E) INPUT = 0.90
JSI METAL = 0.08 (B) INPUT = 1.00



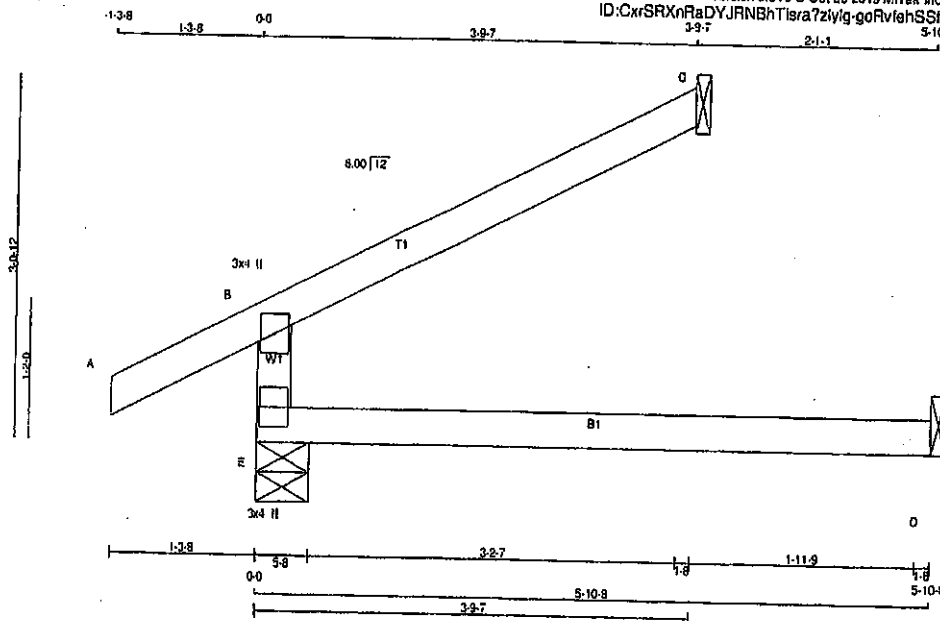
Structural component only
DWG# T-2006225

JOB NAME 408146	TRUSS NAME J4	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:17.3



LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	
E - B	2x4	DRY	No.2	
A - C	2x4	DRY	No.2	
E - D	2x4	DRY	No.2	

DRY: SEASONED LUMBER.

PLATES (table in inches)	PLATES	W	LEN	Y	X
8 TMV+p	MT20	3.0	4.0		
8 BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
E	405	0	405	0	5-8	5-8
C	130	0	130	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX. MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	286	190	0	0	0	96	0
C	90	73	0	0	0	17	0
D	38	0	0	0	0	38	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	WEBS		MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)
			MAX. VERT. LOAD (PL)	MAX. HORIZ. LOAD (PL)		
FR-TO		FROM TO				
E-B	-342	0	0.0	0.0	13.14	7.81
A-B	0	28	-91.8	-91.8	12.11	10.00
B-C	-19	0	-91.8	-91.8	22.11	6.25
E-D	0	0	-18.5	-18.5	0.13	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF

156 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CSI: TC=0.22/1.00 (B-C-1), BC=0.13/1.00 (D-E-4), WB=0.00/1.00 (A-B-0), SS=0.15/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

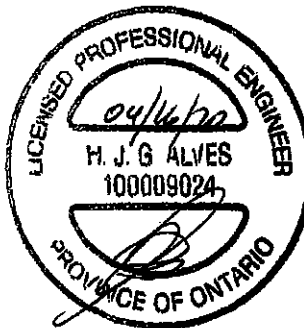
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

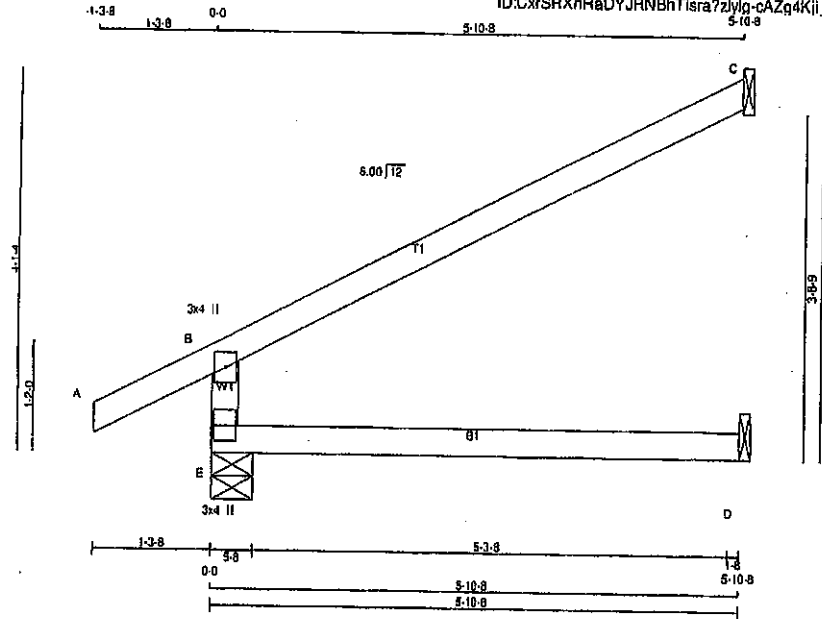
JSI GRIP = 0.14 (E) INPUT = 0.90
JSI METAL = 0.09 (B) INPUT = 1.00



Structural component only
DATE: T. 2008226

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408146	J5	11	1	GREEN PARK HOMES		
Tamarack Roof Truss, Burlington						

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ID: CxrSRXnRaDYJRNhTisra?zlylg-cAZg4Kji_G?KusFLSiLqzCB01fepmXKp_GQh6zQT5E



Scale = 1:22.9

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2	
A - C	2x4	DRY	No.2	
E - D	2x4	DRY	No.2	

DRY: SEASONED LUMBER.

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
E	525	0	5-8	5-8
C	202	0	1-8	1-8
D	45	0	1-8	1-8

TOTAL WEIGHT = 11 X 17 = 185 lb

DESIGN CRITERIA

TOP CH.	LL = 25.6	PSF
	DL = 8.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CSI: TC=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (In:0), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20 818 354	1687 768	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.19 (E) INPUT = 0.90
JSI METAL = 0.13 (B) INPUT = 1.00

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	V	X
B	TMV+p	MT20	3.0	4.0		
E	BMV+p	MT20	3.0	4.0		

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE		
E	389	257	0	0	0
C	139	113	0	0	0
D	38	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
MEMB.				MEMB.		
FR-TO				FR-TO		
E-B	-481	0	0.0	0.13 (4)	7.81	
A-B	0	28	-91.8	-91.8	0.12 (1)	10.00
B-C	-30	0	-91.8	-91.8	0.54 (1)	6.25
E-D	0	0	-18.5	-18.5	0.13 (4)	10.00

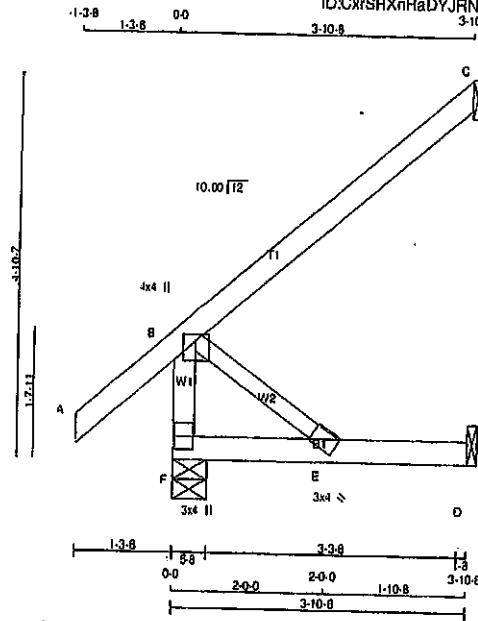


Structural component only
DWG# T-2006227

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408146	J6	6	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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ID: CxrSRXnRaDYJRNbHtIsra7zlylg-4M72HgklJa8BW0qY0PsvVPkGY3?papnT2e7zDZzQT5D



Scale = 1/27.0

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	100	2.00
E BMVW+p	MT20	3.0	4.0		
F BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	341	0	341	0	5-8	5-8
C	178	0	178	0	1-8	1-8
D	38	0	40	0	1-8	1-8

TOTAL WEIGHT = 8 X 16 = 93 lb

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINTS C, D

UNFACTORED REACTIONS

JT	IST LCASE	MAX. MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
F	239	170	0	0	0	89
C	122	99	0	0	0	23
D	29	0	0	0	0	28

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

FR-TO	CHORDS		FACTORED		WEBS	
	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS (LC)	MEMB.	FORCE (LBS)
F-B	305	0	0.0	0.0	0.03 (1)	7.81
A-B	0	41	-91.8	-91.8	0.14 (5)	10.00
B-C	0	0	-91.8	-91.8	0.23 (1)	10.00
F-E	0	0	-18.5	-18.5	0.08 (4)	10.00
E-D	0	0	-18.5	-18.5	0.08 (4)	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19')
CALCULATED VERT. DEFL. (LL) = L/989 (0.00')
ALLOWABLE DEFL. (TL) = L/360 (0.19')
CALCULATED VERT. DEFL. (TL) = L/989 (0.01')

CSI: TC=0.23/1.00 (B-C:1), BC=0.08/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1887 788 1887 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.24 (B) (INPUT = 0.90)
JSI METAL=0.05 (B) (INPUT = 1.00)

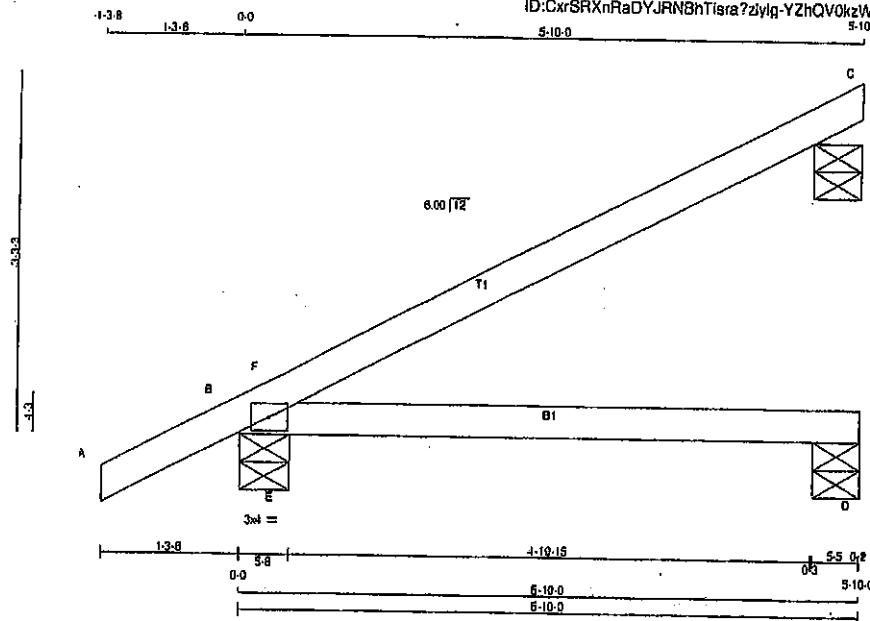


Structural component only
DWG# T-20008228

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408146	J7	7	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:19.3



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS	2x4	DRY	SPF
A - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (tablets in inches)			
JT TYPE	PLATES	W	LEN Y X
B	TMB1-1	MT20	3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

JT	FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
	GROSS REACTION	GROSS REACTION	DOWN	HORIZ	BRG	BRG	IN-SX	IN-SX
C	235	0	235	0	0	5-8	5-8	5-8
B	448	0	448	0	0	5-8	5-8	5-8
D	87	0	87	0	0	5-8	5-8	5-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LOSE		MAX. MIN. COMPONENT REACTIONS		LIVE	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	DOWN	HORIZ					
C	162	127.0	0.0	0.0	0.0	0.0	36.0	0.0	0.0
B	313	219.0	0.0	0.0	0.0	0.0	94.0	0.0	0.0
D	65	22.0	0.0	0.0	0.0	0.0	43.0	0.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LOAD (LBS)	MAX. CS (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LBS)
FR-TO		FROM TO		UNBRACED LENGTH	FR-TO		
A-B	0 28	-91.8	-91.8	0.12 (11)	10.00	E-F	477 11
B-F	-29 100	-91.8	-91.8	0.12 (11)	6.25		
F-C	-6 2	-91.8	-91.8	0.41 (11)	10.00		
B-E	0 0	-18.5	-18.5	0.27 (11)	10.00		
E-D	0 0	-18.5	-18.5	0.28 (11)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/507 (0.14")

CSI: TC=0.41/1.00 (C-F:1), BC=0.28/1.00 (D-E:1),
WB=0.00/1.00 (E-F:1), SB=0.39/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

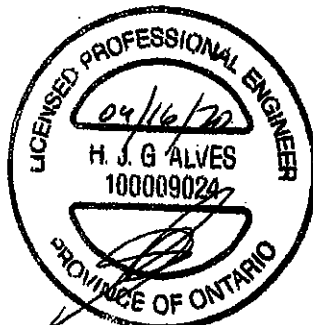
NAIL VALUES

PLATE	GRIP/DRY	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MT20	818	354	1687 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.36 (8) (INPUT = 0.90)
JSI METAL = 0.08 (8) (INPUT = 1.00)

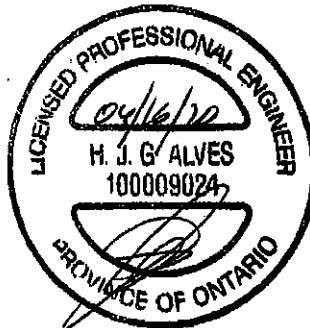


Structural component only
DWG# T-2006229

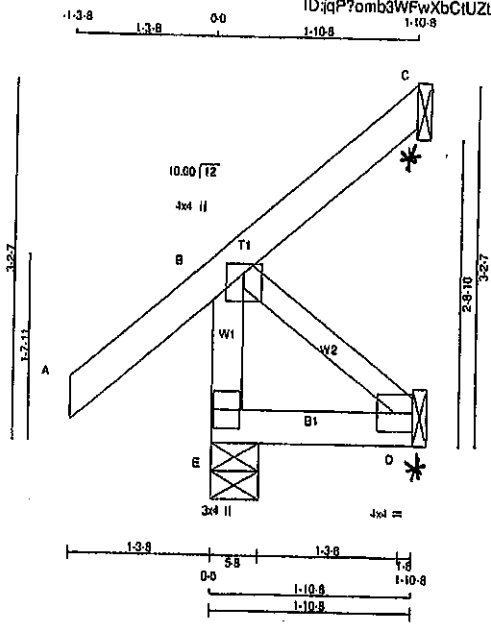
Version 8.310 S Oct 29 2019 MITEK Industries, Inc. Wed Apr 15 14:02:26 2020 Page 1
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Structural component only
DWG# T-2006230



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408147	J21	4	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	7MMW+p	MT20	4.0	4.0	1.00 2.00
D	8MMW1-i	MT20	4.0	4.0	2.00 Edge
E	8MMW1-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		RECORD BRG	
VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX
JT							
E	277	0	277	0	5-8	5-8	
C	39	0	39	0	1-8	1-8	
D	17	0	19	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS		MAX/MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
1ST CASE	2ND CASE	SNOW	LIVE	PERM. LIVE					
JT									
E	192	145.0	0.0	0.0	0.0	47.0	0.0	0.0	0.0
C	27	22.28	0.0	0.0	0.0	5.0	0.0	0.0	0.0
D	13	0.0	0.0	0.0	0.0	13.0	0.0	0.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	MEMB.	FORCE (LBS)	MAX. (LBS)	MAX. (LC1)
FR-TO		FROM	TO	FR-TO			
E-B	-260.0	0.0	0.0 0.03 (1)	8-0	0.0	0.00 (1)	
A-B	0.41	-91.8	-91.8 0.13 (1)				
B-C	-30.0	-91.8	-91.8 0.12 (1)				
E-D	0.0	-18.5	-18.5 0.02 (4)				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 10 = 40 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = 0.999 (0.00")

CSI: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (B-D:1), SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 856

PLATE PLACEMENT TOL. = 0.250 inches

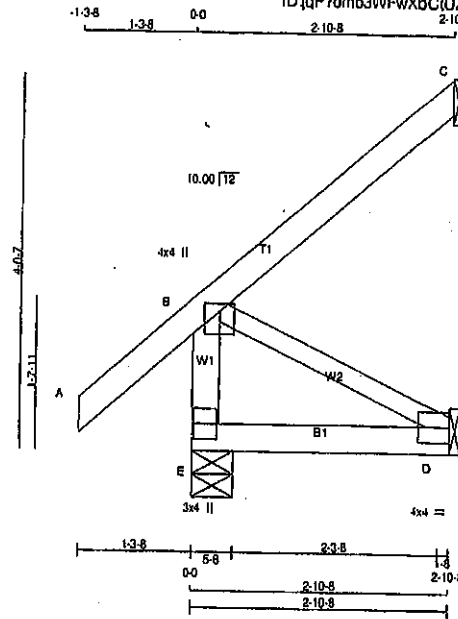
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408147	J22	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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Scale = 1/2\"/>

LUMBER	DESCR.
N.L.G.A. RULES	SPF
CHORDS SIZE	SPF
E - B 2x4 DRY No.2	SPF
A - C 2x4 DRY No.2	
E - D 2x4 DRY No.2	
ALL WEBS 2x3 DRY No.2	
DRY: SEASONED LUMBER.	

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMVW+p MT20	4.0	4.0	1.00	2.00
D BMV1-i MT20	4.0	4.0	2.00	Edge
E BMV1+p MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
JT						
E	284	0	284	0	5-8	5-8
C	132	0	132	0	1-8	1-8
D	25	0	25	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINTS: C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX/MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
E	199	144.0	0.0	0.0	0.0	54.0
C	91	74.0	0.0	0.0	0.0	17.0
D	20	0.0	0.0	0.0	0.0	20.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS: E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	LENGTH FR-TO	
E-B	-259.0	0.0	0.0	0.0
A-B	0.41	-91.8	-91.8	10.00
B-C	0.0	-91.8	-91.8	10.00
E-D	0.0	-18.5	-18.5	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 13 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF ORC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/989 (0.00")

CSI: TC=0.13/1.00 (A-B-S), BC=0.04/1.00 (D-E-4),
WB=0.00/1.00 (B-C-1), SS=0.08/1.00 (A-B-S)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

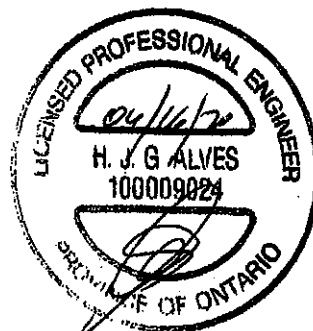
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1867 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.20 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)

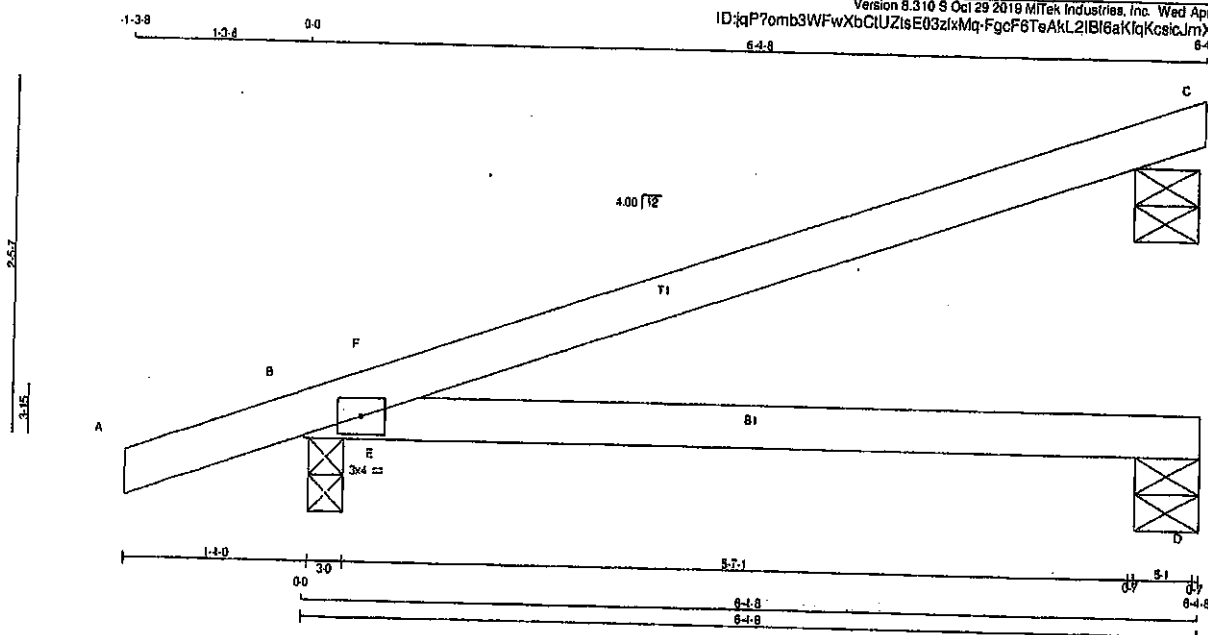


Structural component only
DWG# T-20062145

JOB NAME 408147	TRUSS NAME J23	QUANTITY 6	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 9 Oct 2019 MITek Industries, Inc. Wed Apr 15 15:52:27 2020 Page 1
 ID:qP7omb3WFwXbCIUZIsE03zlxMq-FgcF6TsAkL2IBI6aKlqKscJmXaA8QNPR?ULzQRU2
 6-4-8

Scale = 1:14.6



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER
 A - C 2x4 DRY No.2
 B - D 2x4 DRY No.2
 DRY: SEASONED LUMBER.

PLATES (table in inches)
 JT TYPE PLATES W LEN Y X
 B TMB1-1 MT20 3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
C	256	0	256	0	5-8	5-8	5-8	5-8
B	474	0	474	0	3-0	3-0	3-0	3-0
D	95	0	95	0	5-8 1 5-7 1	5-8	5-8	5-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	COMBINED	MAX. MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	177	138 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0
B	333	232 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0
D	71	25 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		LENGTH FR-TO
A-B	0 18	-91.8 -91.8 0.11 (1)	10.00	E-F -381 -15 0.00 (1)
B-F	-22 28	-91.8 -91.8 0.07 (1)	8.25	
F-C	-3 2	-91.8 -91.8 0.48 (1)	10.00	
B-E	0 0	-18.5 -18.5 0.33 (1)	10.00	
E-D	0 0	-18.5 -18.5 0.33 (1)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 8.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBC 2010, NBC 2012, ABC 2019
 - PART 9 OF NBC 2012 (2019 AMENDMENT)
 - CSA 088-09, CSA 088-14
 - TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.21")
 CALCULATED VERT. DEFL.(LL) = L/785 (0.10")
 ALLOWABLE DEFL.(TL) = L/360 (0.21")
 CALCULATED VERT. DEFL.(TL) = L/387 (0.20")

CS: TC=0.48/1.00 (C-F:1), BC=0.33/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.31/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

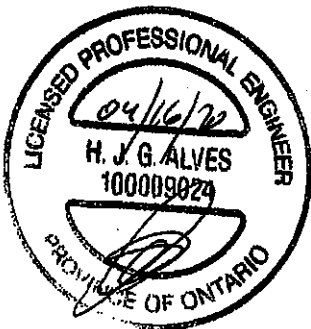
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1687 788 1987 1858	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.32 (B) (INPUT = 0.80)
 JSI METAL = 0.09 (B) (INPUT = 1.00)



LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing -- an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258-259.

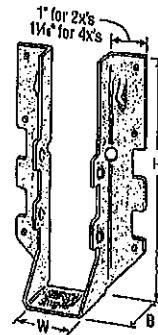
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20-24.

Installation:

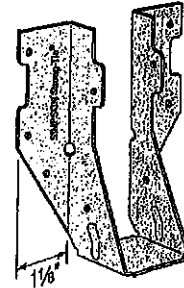
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/4" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

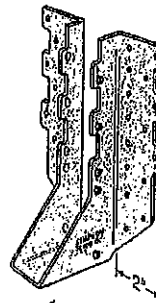
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



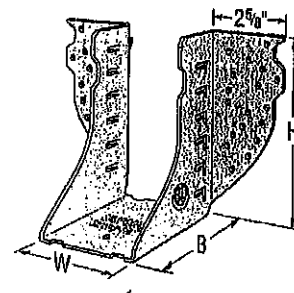
LUS28



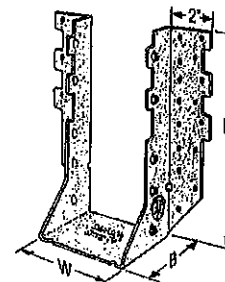
LU26L



HUS210
(HUS26, HUS28,
and HHUS similar)



HGUS28-2



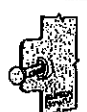
HHUS210-2



Double-Shear
Nailing
Top View

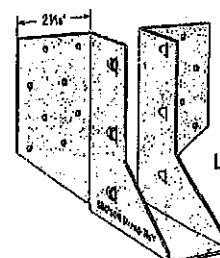
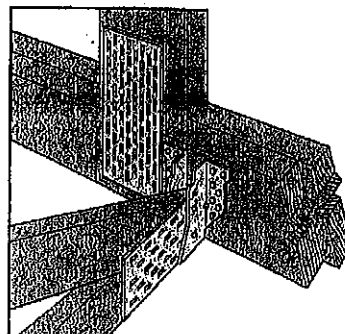


Double-Shear
Nailing
Side View;
Do not
bend tab



Dome Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580

Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LJS26DS

Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS**HHUS/HGUS**

See Hanger Options Information on pp. 125-127.

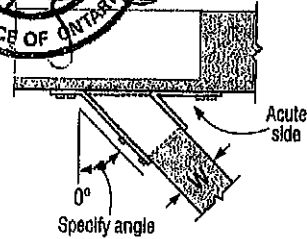
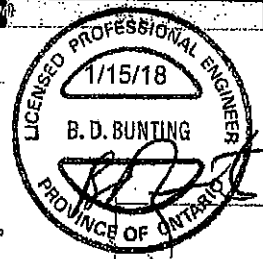
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.82 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _g ¹	Header	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15) lb. kN	(K _D = 1.00) lb. kN	(K _D = 1.15) lb. kN	(K _D = 1.00) lb. kN
Single 2x Sizes											
LUS24	18	1 1/8	3 1/4	1 1/4	2 1/4	(4) 10d	(2) 10d	710 3.16	1625 7.23	645 2.87	1155 5.14
LU24L	22	1 1/8	3	1 1/4	2 1/8	(4) 10d	(2) 10d x 1 1/2"	360 1.60	1020 4.54	320 1.42	726 3.22
LU26L	22	1 1/8	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"	720 3.20	1805 7.14	645 2.87	1140 5.07
LUS26	18	1 1/8	4 1/4	1 1/4	3 3/4	(4) 10d	(4) 10d	1420 6.32	2170 9.65	1290 5.74	1630 7.25
HUS28	18	1 1/8	5 1/4	3	3 3/8	(14) 16d	(8) 16d	2705 11.30	4940 21.97	2065 9.20	3875 17.24
LJS26DS	18	1 1/8	5	3 1/2	4 1/4	(16) 16d	(8) 16d	2055 9.14	4265 18.97	1480 6.49	4115 18.31
HGUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 16d	(8) 16d	2685 11.96	6625 29.51	2685 11.96	5700 25.35
LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(8) 10d x 1 1/2"	1140 5.07	2185 9.72	1020 4.54	1550 6.89
LUS28	18	1 1/8	6 1/4	1 1/4	3 3/4	(8) 10d	(4) 10d	1420 6.32	2520 11.21	1290 5.74	1780 7.96
HUS28	16	1 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3805 16.04	5365 23.88	2675 11.90	4345 19.33
HGUS28	12	1 1/8	7 1/8	5	6 1/4	(36) 16d	(12) 16d	3310 14.74	7675 34.19	3310 14.74	6800 30.73
LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(8) 10d x 1 1/2"	1140 5.07	2495 11.10	1020 4.54	1770 7.87
LUS210	18	1 1/8	7 1/8	1 1/4	3 3/4	(8) 10d	(4) 10d	1420 6.32	2785 12.39	1290 5.74	2210 9.83

1. Factored uplift resistances have been increased 16% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_g is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSON and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27-28 for other nail sizes and information.

SIMPSON
Strong-Tie

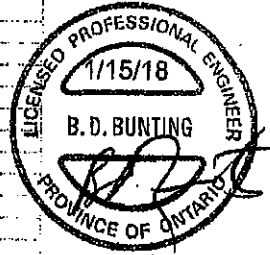
Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _g ³	Header	Jolst	D.Fir-L		S-P-F	
								Uplift (K _D = 1.15)	Normal (K _D = 1.00)	Uplift (K _D = 1.15)	Normal (K _D = 1.00)
								lb. kN	lb. kN	lb. kN	lb. kN
Double 2x Sizes											
■ LUS24-2	18	3 1/8	3 1/8	2	1 1/2	(4) 16d	(2) 16d	835	2020	580	1435
SS LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	3.71	8.99	2.82	6.38
■ HHUS26-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	1720	2595	1545	1920
■ HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	7.85	11.54	8.87	8.54
SS LUS28-2	18	3 1/8	7	2	4	(8) 16d	(4) 16d	2850	7335	2065	5205
■ HHUS28-2	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	12.68	32.83	9.20	23.15
■ HGUS28-2	12	3 1/8	7 1/8	4	8 1/8	(36) 16d	(12) 16d	4385	8950	3110	6355
SS LUS210-2	18	3 1/8	9	2	8	(8) 16d	(8) 16d	19.51	39.81	13.83	28.27
■ HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	1720	3325	1545	2575
■ HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	7.85	14.79	6.87	11.45
								3785	8940	2675	6345
								16.75	39.77	11.90	28.22
								6070	12980	4310	9215
SS LUS210-2	18	3 1/8	9	2	8	(8) 16d	(8) 16d	27.00	57.74	19.17	40.99
■ HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	2680	4500	2320	3195
■ HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	11.48	20.02	10.32	14.21
								4670	9680	4235	7000
								20.77	42.97	18.84	31.14
								6840	14015	4855	10270
								30.43	62.34	21.60	45.60
Triple 2x Sizes											
■ HGUS26-3	12	4 1/8	6 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
■ HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	19.51	39.81	13.83	28.27
■ HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d	6070	12980	4310	9215
■ HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	27.00	57.74	19.17	40.99
								4670	9670	4235	6865
								20.77	43.02	18.84	30.54
								6840	14645	4855	10400
								30.43	65.14	21.60	46.28
Quadruple 2x Sizes											
■ HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
■ HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	19.51	39.81	13.83	28.27
■ HHUS210-4	14	6 1/8	8 1/8	3	7 1/8	(30) 16d	(10) 16d	6070	12980	4310	9215
■ HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	27.00	57.74	19.17	40.99
								4670	10155	4235	7210
								20.77	45.17	18.84	32.07
								6840	14845	4855	10400
								30.43	65.14	21.60	46.28
■ HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
■ HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	33.98	68.70	24.13	47.35
								10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
■ LUS46	18	3 1/8	4 1/8	2	3 1/8	(4) 16d	(4) 16d	1720	2505	1545	1920
■ HHUS46	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	7.85	11.54	6.87	8.54
■ HGUS46	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	2540	7335	2065	5205
■ LUS48	18	3 1/8	6 1/8	2	3 1/8	(6) 16d	(4) 16d	11.30	32.83	9.20	23.15
■ HHUS48	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	4385	8950	3110	6355
■ HGUS48	12	3 1/8	7 1/8	4	8 1/8	(36) 16d	(12) 16d	19.51	39.81	13.83	28.27
■ LUS410	18	3 1/8	8 1/8	2	5 1/8	(8) 16d	(6) 16d	1720	3325	1545	2575
■ HHUS410	14	3 1/8	9 1/8	3	8 1/8	(30) 16d	(10) 16d	7.85	14.79	6.87	11.45
■ HGUS410	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	3785	8940	2675	6345
■ LUS412	18	3 1/8	10 1/8	2	6 1/8	(10) 16d	(8) 16d	16.75	39.77	11.90	28.22
■ HHUS412	14	3 1/8	11 1/8	3	9 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
■ HGUS412	12	3 1/8	11 1/8	4	10 1/8	(56) 16d	(20) 16d	27.00	57.74	19.17	40.99
■ LUS414	18	3 1/8	12 1/8	2	7 1/8	(12) 16d	(10) 16d	2680	4500	2320	3195
■ HHUS414	14	3 1/8	13 1/8	3	10 1/8	(46) 16d	(16) 16d	11.48	20.02	10.32	14.21
■ HGUS414	12	3 1/8	13 1/8	4	11 1/8	(66) 16d	(22) 16d	4670	9680	4235	7000
								20.77	43.02	18.84	30.54
								6840	14015	4855	10270
								30.43	62.34	21.60	45.60
								7640	14995	5425	10645
								33.98	68.70	24.13	47.35
								10130	16400	7195	11645
								45.06	72.95	32.00	51.80



Plated Truss Connectors

See footnotes
on p. 258.

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1/4". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

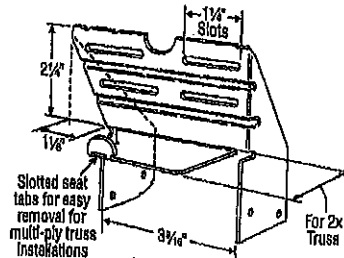
Design: Factored resistances are in accordance with CSA 086-14

Installation:

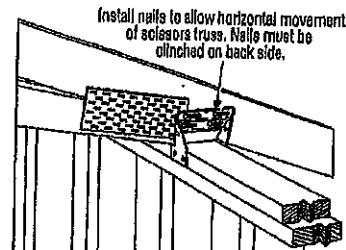
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

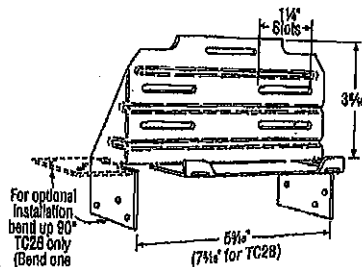
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



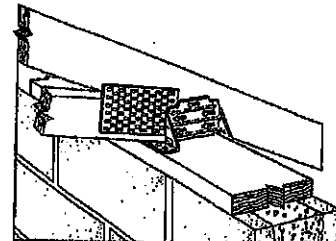
TC24
U.S. Patent 4,932,173



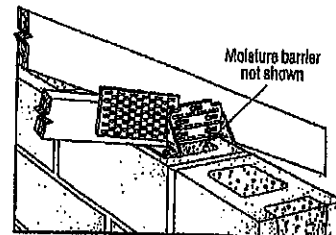
Typical TC24 Installation



TC26
(TC28 Similar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer (8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift ($K_p=1.15$)	Uplift ($K_p=1.15$)
TC24	(4) 10d	(4) 10d	605	430
TC26	(6) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift ($K_p=1.15$)	Uplift ($K_p=1.15$)
TC28	(5) 10d	(6) 10d x 1 1/2"	810	660
	(6) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC28 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) - 3/8" x 2 1/4" Titen screws has a factored uplift resistance of 275 lb.



MP
TIES
DESIGN

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.

2. Grout strength is 15 MPa minimum.

3. Optional TC28 installation with 10d nails requires minimum 3" top plate thickness.

4. TC26 fastened to grouted concrete block with (6) - 3/8" x 2 1/4" Titen screws has a factored uplift resistance of 275 lb.

(800) 999-5099
strongtie.com

H/TSP**SIMPSON**
Strong-Tie**Seismic and Hurricane Ties (cont.)**

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Fasteners			Factored Resistance ($K_F = 1.15$)					
					D.Fir-L			S-P-F		
		To Rafter/Truss	To Plates	To Studs	Uplift	Lateral		Uplift	Lateral	
					lb.	F ₁	F ₂	lb.	F ₁	F ₂
					kN	kN	kN	kN	kN	kN
H1	18	(6) 8d x 1 1/2"	(4) 8d	—	740	685	300	680	485	215
					3.29	3.05	1.33	3.02	2.16	0.96
H2A	18	(5) 8d x 1 1/2"	(2) 8d x 1 1/2"	(5) 8d x 1 1/2"	890	220	75	590	155	55
					3.89	0.98	0.33	2.62	0.69	0.24
H2.5A	18	(5) 8d	(5) 8d	—	805	180	180	755	180	180
					3.58	0.71	0.71	3.36	0.71	0.71
H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
					3.71	0.78	0.93	3.29	0.71	0.93
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
					3.29	0.80	1.18	2.74	0.56	0.85
H6	16	—	(8) 8d	(8) 8d	1585	1085	—	1125	770	—
					7.05	4.83	—	5.00	3.43	—
H7Z	16	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
					6.18	2.98	—	4.40	2.11	—
H8 ³	18	(5) 10d x 1 1/2"	(5) 10d x 1 1/2"	—	1120	—	—	1025	—	—
					4.98	—	—	4.56	—	—
H10A ³	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1736	795	410	1505	565	290
					7.72	3.54	1.82	6.69	2.51	1.29
H10AR	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1485	690	430	1220	570	305
					6.61	3.07	1.91	5.43	2.54	1.36
H10A-2	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1835	1275	430	1645	880	305
					8.16	5.67	1.91	7.32	3.91	1.36
H10S ^{3,4}	18	(8) 8d x 1 1/2"	(8) 8d x 1 1/2"	(8) 8d	1465	795	315	1040	585	225
					6.52	3.54	1.40	4.63	2.51	1.00
H11Z	18	(6) 16d x 2 1/2"	(6) 16d x 2 1/2"	—	1095	920	545	780	685	390
					4.87	4.09	2.42	3.47	2.91	1.73
H14	18	[1] (12) 8d x 1 1/2"	(13) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
		[2] (12) 8d x 1 1/2"	(15) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
TSP	18	(9) 10d x 1 1/2"	(6) 10d x 1 1/2"	—	1295	440	—	920	310	—
					5.76	1.96	—	4.09	1.38	—
		(9) 10d x 1 1/2"	(6) 10d	—	1560	440	—	1105	310	—
					6.94	1.96	—	4.92	1.38	—

- Factored resistances have been increased 15% for short term loading; no further increase is allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2 1/2" must be used when framing anchors are installed on the same side of the plate (exception: H2.6A).
- H8 factored uplift resistances for stud-to-bottom plate installations are 595 lb. (2.65 kN) for D.Fir-L and 390 lb. (1.74 kN) for S-P-F.
- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-HTIECONPATH).
- Factored resistances in the F₁ direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.

- H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.38 kN) D.Fir-L and 1015 lb. (4.61 kN) S-P-F.
- H10S nails to plates are optional for uplift but required for lateral loads.
- H10A may be field-bent up to a slope of 6/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
- The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SSNAILS at strongtie.com.
- D.Fir-L/S-P-F factored uplift resistances for the H2.6A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1 1/2" nails into the top plates and (3) 8d x 1 1/2" nails into the lowest three flange holes into the truss bottom chord is 495 lb. (2.20 kN).
- Nails: 16d x 2 1/2" = 0.162" dia. x 2 1/2" long, 10d = 0.148" dia. x 3" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 8d = 0.131" dia. x 2 1/2" long, 8d x 1 1/2" = 0.131" dia. x 1 1/2" long. See pp. 27-28 for other nail sizes and information.

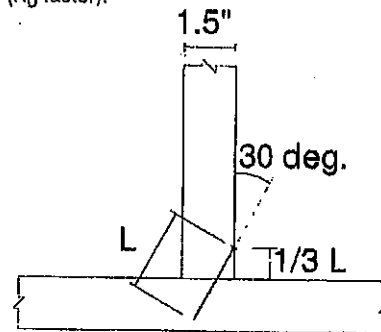
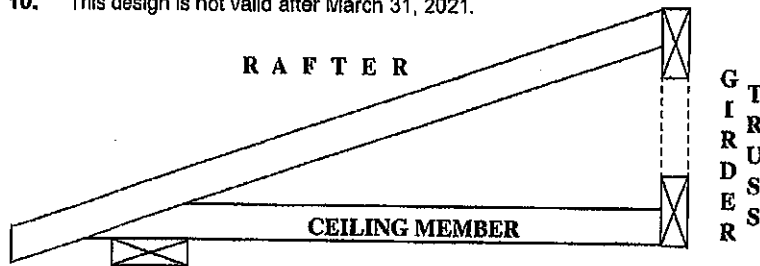
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.



TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (In)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

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100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019

PEO
Certificate No. 10889485



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

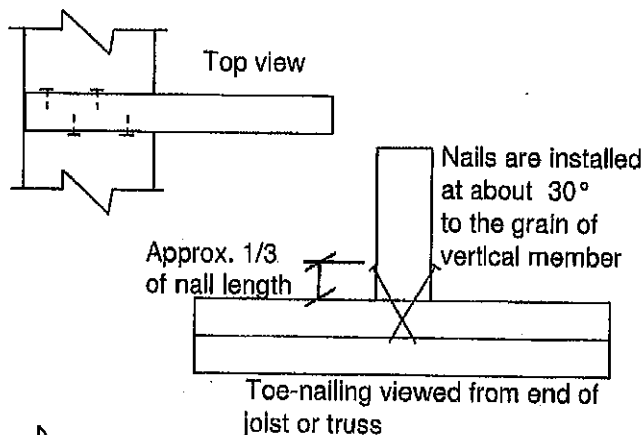
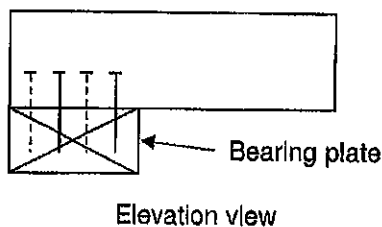
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

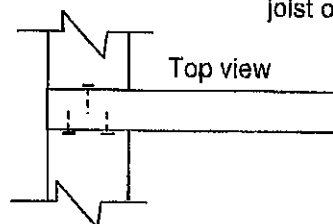
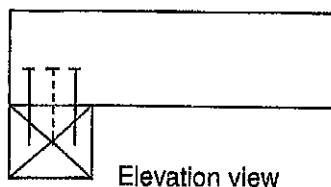
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37679H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



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100 Industrial Rd.
Bradford, Ontario L3Z 3G7

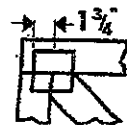
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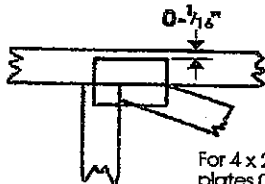
December 2, 2019

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint, unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

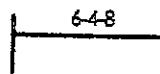


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

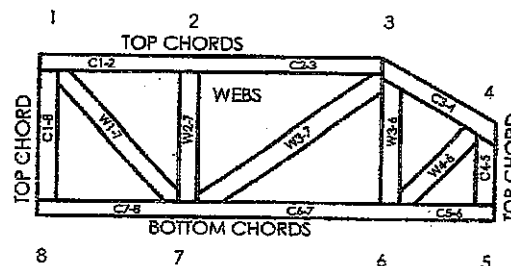
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L 10319-L 13270-L 12691-R

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MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MH-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

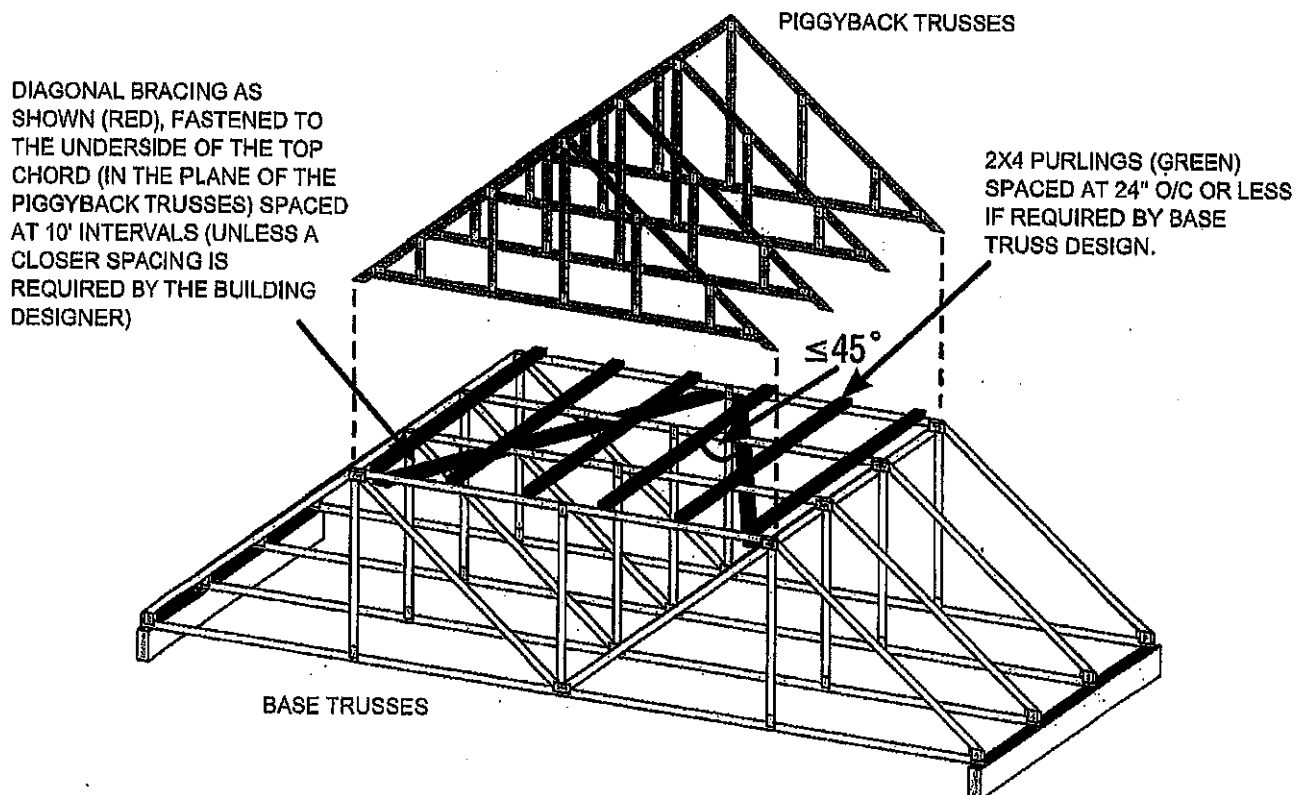
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

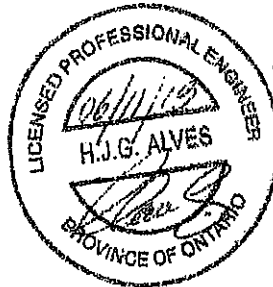


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufacturer's responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet M117473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800213

Feb 09, 2018