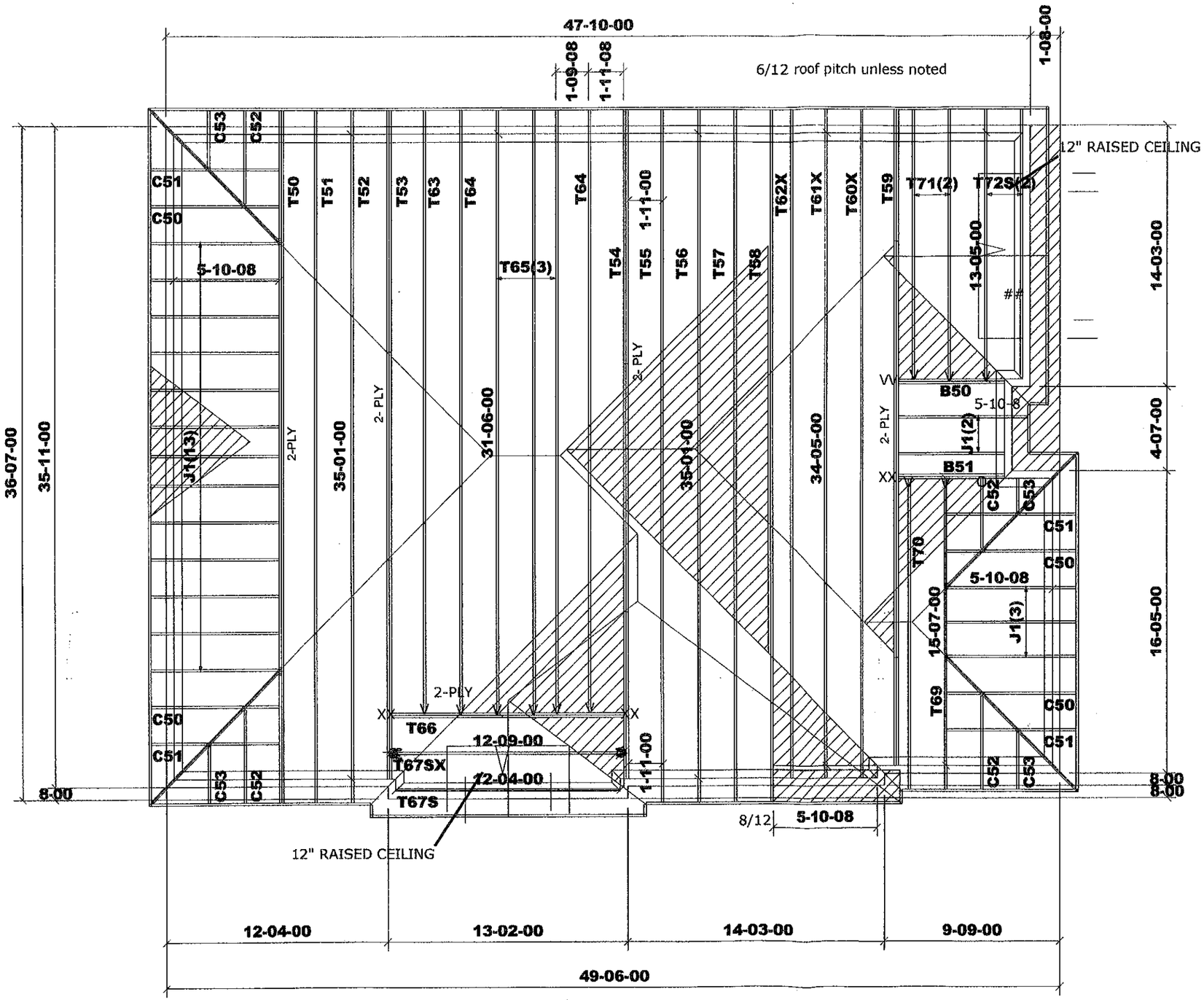


ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
HGUS26-2 - (XX)
LJS26DS - (V)
LUS24 - (O)
LUS26-2-(VV)

BEAMS:
B50= B51 =
2 - 2x10 SPF #2

CITY OF HAMILTON
BUILDING DIVISION
Planning & Development Department
FEB 05 2021
REC'D BY _____ DATE _____
REF'D TO _____ DATE _____

CITY OF HAMILTON
Building Division
Permit No. 21 - 105920
THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE
THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW
These drawings and/or specifications have been reviewed by
Feb 22 2021
FOR CHIEF BUILDING OFFICIAL DATE

DENOTES:
CONVENTIONAL FRAMING

M13140

TAMARACK ROOF TRUSSES INC.

Job Track: **51225**	Builder / Location: **GREEN PARK HOMES / WATERDOWN**	Model / Elevation: **MOUNTAINASH 12/3**	Mitek ver 8.3.1.215
Plan Log: **202439**			LOT 156
Layout ID: **408317**	Project: **RUSSELL GARDENS PH.3**		
	Date: 2020-04-30 Sales: Mario DiCano Designer: JG		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	51225
Builder:	GREEN PARK HOMES	PlanLog:	202439
Project:	RUSSELL GARDENS PH.3	Layout ID:	408317
Location:	WATERDOWN	Ref #	
Model:	MOUNTAINASH 12	Page:	1 of 3
Lot #:	156	Date:	04-30-2020
Elevation:	3	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

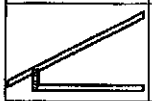
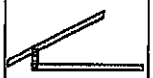
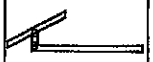
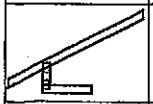
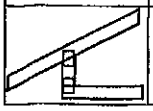
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T50 Hip Girder	6 /12	35-01-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	308 192.00		
	1	T51 Hip	6 /12	35-01-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	138.75 87.00		
	1	T52 Hip	6 /12	35-01-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	138.32 87.33		
	1 2-ply	T53 Hip Girder	6 /12	35-01-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	354.2 219.00		
	1 2-ply	T54 Roof Special Girder	6 /12	35-01-00	7-05-04	2 x 4 2 x 6	1-03-08	1-02-00 1-04-13	342.61 208.33		
	1	T55 Roof Special	6 /12	35-01-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-04-13	144.8 90.50		
	1	T56 Roof Special	6 /12	35-01-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-04-13	144.65 90.50		
	1	T57 Roof Special	6 /12	35-01-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-04-13	142.13 88.67		
	1	T58 Roof Special	6 /12	35-01-00	7-07-12	2 x 4	1-03-08 1-03-08	1-02-00 1-04-13	155.1 99.67		
	1 2-ply	T59 Hip Girder	6 /12	34-05-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	335 210.00		
	1	T60X Hip	6 /12	35-01-00	5-01-04	2 x 4	1-03-08	1-02-00 10-00	138.56 86.83		
	1	T61X Hip	6 /12	35-01-00	6-01-04	2 x 4	1-03-08	1-02-00 10-00	137.06 84.83		
	1	T62X Hip	6 /12	35-01-00	7-01-04	2 x 4	1-03-08	1-02-00 10-00	143.34 89.50		
	1	T63 Hip	6 /12	31-06-00	8-01-04	2 x 4	1-03-08	1-02-00 2-11-08	134.66 84.83		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51225
Builder:	GREEN PARK HOMES	PlanLog:	202439
Project:	RUSSELL GARDENS PH.3	Layout ID:	408317
Location:	WATERDOWN	Ref #	
Model:	MOUNTAINASH 12	Page:	2 of 3
Lot #:	156	Date:	04-30-2020
Elevation:	3	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	T64 Hip	6 /12	**31-06-00**	9-01-04	2 x 4	1-03-08	1-02-00 2-11-08	284.64 180.00		
	3	T65 Common	6 /12	**31-06-00**	9-11-04	2 x 4	1-03-08	1-02-00 2-11-08	412.8 259.00		
	1 2-ply	T66 Common Girder	6 /12	**12-09-00**	4-03-00	2 x 4 2 x 6		1-00-12 1-00-12	117.89 78.00		
	1	T67S Roof Special	6 /12	**12-04-00**	4-03-00	2 x 4		1-02-00 1-02-00	51.59 34.83		
	1	T67SX Roof Special	6 /12	**12-09-00**	4-03-00	2 x 4		1-00-12 1-00-12	53.06 36.17		
	1	T69 Hip Girder	6 /12	**15-07-00**	4-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	61.23 39.17		
	1	T70 Common	6 /12	**15-07-00**	5-00-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	61.14 39.17		
	2	T71 Common	6 /12	**13-05-00**	4-06-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	106.99 68.00		
	2	T72S Roof Special	6 /12	**13-05-00**	4-06-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	113.78 75.67		
	18	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	302.3 192.00		
	4	C50 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	56.53 34.67		
	4	C51 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	46.33 29.33		
	4	C52 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	38.28 24.00		
	4	C53 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	28.08 18.67		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51225
Builder:	GREEN PARK HOMES	PlanLog:	202439
Project:	RUSSELL GARDENS PH.3	Layout ID:	408317
Location:	WATERDOWN	Ref #	
Model:	MOUNTAINASH 12	Page:	3 of 3
Lot #:	156	Date:	04-30-2020
Elevation:	3	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS

TOTAL # TRUSS= **67** TOTAL BFT OF ALL TRUSSES= **2827.67** BFT. TOTAL WEIGHT OF ALL TRSSES **4491.81** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
11	Hardware	LJS26DS	
3	Hardware	LUS24	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= **18**

CITY OF HAMILTON
BUILDING DIVISION
Planning & Development Department
FEB 05 2021
REC'D BY _____ DATE _____
REF'D TO _____ DATE _____

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	T1	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:31 2020 Page 1
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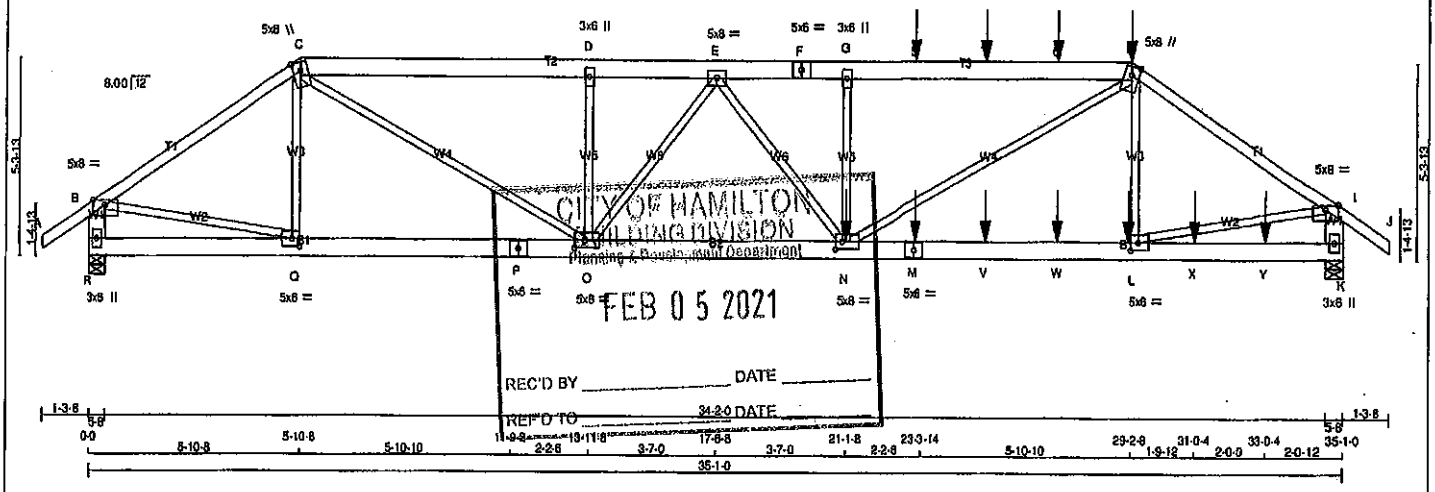

TOTAL WEIGHT = 2 X 176 = 350 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
H-J	1	12	SIDE(61.0)
C-F	2	12	TOP
F-H	2	12	SIDE(61.0)
R-B	2	12	TOP
K-I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R-P	2	12	TOP
P-M	2	12	SIDE(183.1)
M-K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
L-H	1	6	SIDE(18.3)
N-G	1	6	SIDE(240.7)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J/G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008005 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2990	0	2990	0	0	5-8	5-8
K	4041	0	4041	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2109	1413 / 0	0 / 0	0 / 0	0 / 0	696 / 0	0 / 0
K	2853	1899 / 0	0 / 0	0 / 0	0 / 0	953 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.07 (1)	10.00
B-C	-3676 / 0	-91.8	-91.8	0.45 (1)	4.50
C-D	-6111 / 0	-91.8	-91.8	0.30 (1)	4.58
D-E	-6111 / 0	-91.8	-91.8	0.23 (1)	4.60
E-F	-7562 / 0	-91.8	-91.8	0.41 (1)	3.98
F-G	-7562 / 0	-91.8	-91.8	0.41 (1)	3.98
G-S	-7563 / 0	-91.8	-91.8	0.56 (1)	3.86
S-T	-7563 / 0	-91.8	-91.8	0.56 (1)	3.86
T-U	-7563 / 0	-91.8	-91.8	0.56 (1)	3.86
U-H	-7563 / 0	-91.8	-91.8	0.56 (1)	3.86
H-I	-5157 / 0	-91.8	-91.8	0.56 (1)	3.84
I-J	0 / 35	-91.8	-91.8	0.07 (1)	10.00
R-B	-2954 / 0	0.0	0.0	0.11 (1)	7.81
K-I	-3985 / 0	0.0	0.0	0.14 (1)	7.11
R-Q	0 / 0	-18.5	-18.5	0.06 (4)	10.00
Q-P	0 / 3073	-18.5	-18.5	0.24 (1)	10.00
P-O	0 / 3073	-18.5	-18.5	0.24 (1)	10.00
O-N	0 / 6782	-18.5	-18.5	0.48 (1)	10.00
N-M	0 / 4309	-18.5	-18.5	0.36 (1)	10.00
M-V	0 / 4309	-18.5	-18.5	0.36 (1)	10.00
V-W	0 / 4309	-18.5	-18.5	0.36 (1)	10.00
W-L	0 / 4309	-18.5	-18.5	0.36 (1)	10.00
L-X	0 / 0	-18.5	-18.5	0.12 (4)	10.00
X-Y	0 / 0	-18.5	-18.5	0.12 (4)	10.00
Y-K	0 / 0	-18.5	-18.5	0.12 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-C	-371 / 0	0.07 (1)
C-O	0 / 3550	0.44 (1)
N-H	0 / 3802	0.47 (1)
L-H	-434 / 84	0.09 (1)
B-Q	0 / 3110	0.38 (1)
L-I	0 / 4383	0.54 (1)
O-D	-796 / 0	0.15 (1)
N-G	-1225 / 0	0.23 (1)
O-E	-1131 / 0	0.35 (1)
E-N	0 / 1314	0.16 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	29-2-8	-538	-538	---	FRONT	VERT	TOTAL	---	C1
L	29-1-12	-38	-38	---	FRONT	VERT	TOTAL	---	C1
M	23-1-12	-38	-38	---	FRONT	VERT	TOTAL	---	C1
N	21-1-8	-1624	-1624	---	FRONT	VERT	TOTAL	---	C1
S	23-1-12	-178	-178	---	FRONT	VERT	TOTAL	---	C1
T	25-1-12	-178	-178	---	FRONT	VERT	TOTAL	---	C1
U	27-1-12	-178	-178	---	FRONT	VERT	TOTAL	---	C1
V	25-1-12	-38	-38	---	FRONT	VERT	TOTAL	---	C1
W	27-1-12	-38	-38	---	FRONT	VERT	TOTAL	---	C1
X	31-0-4	-38	-38	---	FRONT	VERT	TOTAL	---	C1
Y	33-0-4	-38	-38	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN.C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.56/1.00 (H-I:1) , BC=0.48/1.00 (N-O:1) , WB=0.54/1.00 (I-L:1) , SSI=0.25/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (L) (INPUT = 0.90)
JSI METAL= 0.54 (L) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408315	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:32 2020 Page 2
ID:DMCubINVR6TstFoe31v6I_zns1I-VsUaKwFKIBus9rltCMOYrO00NF3whfGAeBZc9QzLaf9

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMW+w	MT20	3.0	6.0		
E	TMWW-t	MT20	5.0	8.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	5.0	8.0	Edge	
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0	2.50	2.25
O	BMWWW-t	MT20	5.0	8.0	2.50	3.25
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CITY OF HAMILTON
BUILDING DIVISION
Planning & Development Department

FEB 05 2021

REC'D BY _____ DATE _____

REF'D TO _____ DATE _____

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008005 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408316	T100	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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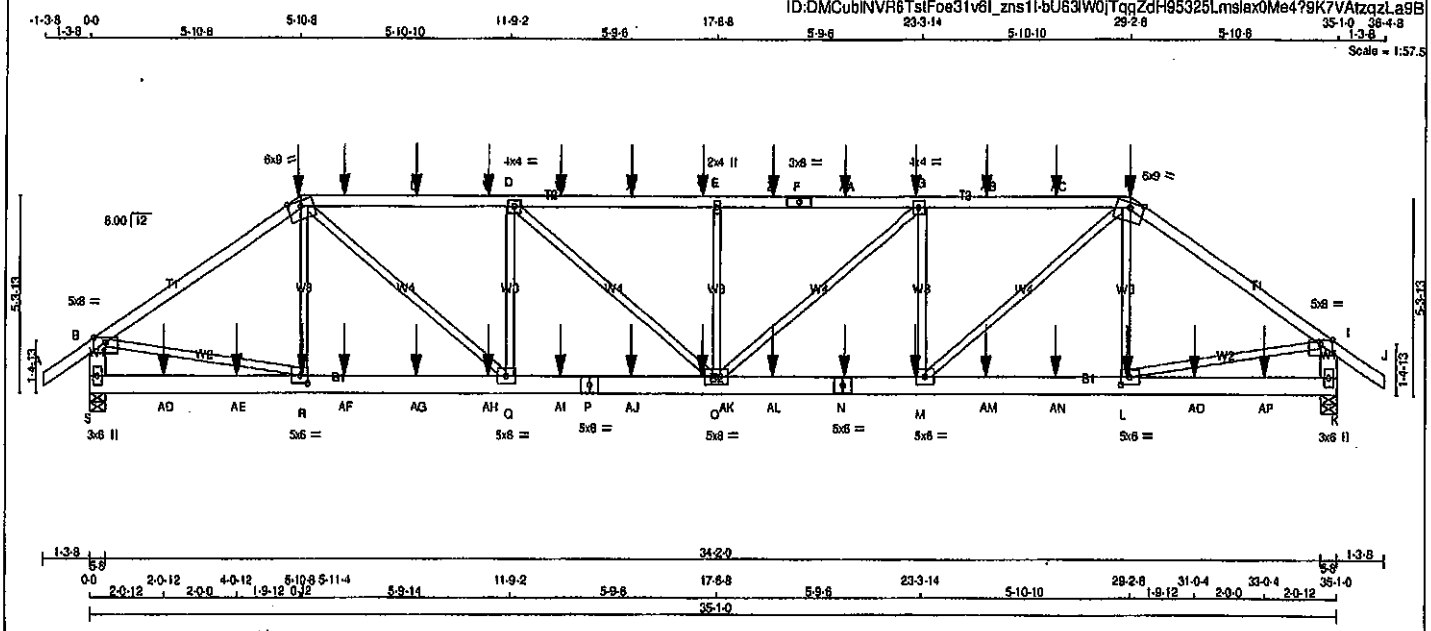

TOTAL WEIGHT = 2 X 164 = 328 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-F	1	12	SIDE(61.0)
F-H	1	12	SIDE(61.0)
H-J	1	12	SIDE(61.0)
S-B	2	12	TOP
K-I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S-P	2	12	SIDE(183.1)
P-N	2	12	SIDE(0.0)
N-K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
R-C	1	6	SIDE(19.3)
L-H	1	6	SIDE(19.3)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008047 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3909	0	3909	0	0	5-8	5-8
K	3859	0	3859	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2783	1823 / 0	0 / 0	0 / 0	0 / 0	940 / 0	0 / 0
K	2727	1799 / 0	0 / 0	0 / 0	0 / 0	928 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.07 (1)	10.00
B-C	-4949 / 0	-91.8	-91.8	0.55 (1)	3.92
C-T	-6443 / 0	-91.8	-91.8	0.84 (1)	3.05
T-U	-6443 / 0	-91.8	-91.8	0.84 (1)	3.05
U-V	-6443 / 0	-91.8	-91.8	0.84 (1)	3.05
V-D	-6443 / 0	-91.8	-91.8	0.84 (1)	3.05
D-W	-7094 / 0	-91.8	-91.8	0.89 (1)	2.84
W-X	-7094 / 0	-91.8	-91.8	0.89 (1)	2.84
X-Y	-7094 / 0	-91.8	-91.8	0.89 (1)	2.84
Y-E	-7094 / 0	-91.8	-91.8	0.89 (1)	2.84
E-Z	-7094 / 0	-91.8	-91.8	0.84 (1)	2.93
Z-F	-7094 / 0	-91.8	-91.8	0.84 (1)	2.93
F-AA	-7094 / 0	-91.8	-91.8	0.84 (1)	2.93
AA-G	-7094 / 0	-91.8	-91.8	0.84 (1)	2.93
G-AB	-6405 / 0	-91.8	-91.8	0.79 (1)	3.14
AB-AC	-6405 / 0	-91.8	-91.8	0.79 (1)	3.14
AC-H	-6405 / 0	-91.8	-91.8	0.79 (1)	3.14
H-I	-4875 / 0	-91.8	-91.8	0.64 (1)	3.95
I-J	0 / 35	-91.8	-91.8	0.07 (1)	10.00
S-B	-3833 / 0	0.0	0.0	0.14 (1)	7.22
K-I	-3782 / 0	0.0	0.0	0.14 (1)	7.26
S-AD	0 / 0	-18.5	-18.5	0.07 (4)	10.00
AD-AE	0 / 0	-18.5	-18.5	0.07 (4)	10.00
AE-R	0 / 0	-18.5	-18.5	0.07 (4)	10.00
R-AF	0 / 4104	-18.5	-18.5	0.31 (1)	10.00
AF-AG	0 / 4104	-18.5	-18.5	0.31 (1)	10.00
AG-AH	0 / 4104	-18.5	-18.5	0.31 (1)	10.00
AH-Q	0 / 4104	-18.5	-18.5	0.31 (1)	10.00
Q-AI	0 6443	-18.5	-18.5	0.48 (1)	10.00
AI-P	0 / 6443	-18.5	-18.5	0.48 (1)	10.00
P-AJ	0 / 6443	-18.5	-18.5	0.48 (1)	10.00
AJ-AK	0 6443	-18.5	-18.5	0.48 (1)	10.00
AK-O	0 / 6443	-18.5	-18.5	0.48 (1)	10.00
O-AL	0 6405	-18.5	-18.5	0.48 (1)	10.00
AL-N	0 6405	-18.5	-18.5	0.48 (1)	10.00
N-M	0 6405	-18.5	-18.5	0.48 (1)	10.00
M-AM	0 4043	-18.5	-18.5	0.30 (1)	10.00
AM-AN	0 4043	-18.5	-18.5	0.30 (1)	10.00
AN-L	0 4043	-18.5	-18.5	0.30 (1)	10.00
L-AO	0 0	-18.5	-18.5	0.07 (4)	10.00
AO-AP	0 0	-18.5	-18.5	0.07 (4)	10.00
AP-K	0 0	-18.5	-18.5	0.07 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-C	-431 / 54	0.09 (1)
C-Q	0 / 3051	0.38 (1)
Q-D	-1748 / 0	0.35 (1)
D-O	0 / 857	0.11 (1)
O-E	-957 / 0	0.19 (1)
O-G	0 / 907	0.11 (1)
M-G	-1769 / 0	0.36 (1)
M-H	0 / 3081	0.38 (1)
L-H	-433 / 44	0.09 (1)
B-R	0 / 4170	0.52 (1)
L-I	0 / 4108	0.51 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.35")

CSI: TC=0.89/1.00 (D-E:1) , BC=0.48/1.00 (O-Q:1) , WB=0.52/1.00 (B-R:1) , SSI=0.29/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (R) (INPUT = 0.90)
JSI METAL= 0.61 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408316	T100	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	9.0	Edge	
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.75
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-542	-542	---	FRONT	VERT	TOTAL	--	C1
G	23-1-12	-178	-178	---	FRONT	VERT	TOTAL	--	C1
H	29-2-8	-542	-542	---	FRONT	VERT	TOTAL	--	C1
L	29-1-12	-36	-36	---	FRONT	VERT	TOTAL	--	C1
M	23-1-12	-36	-36	---	FRONT	VERT	TOTAL	--	C1
N	21-1-12	-36	-36	---	FRONT	VERT	TOTAL	--	C1
R	5-11-4	-36	-36	---	FRONT	VERT	TOTAL	--	C1
T	7-1-12	-178	-178	---	FRONT	VERT	TOTAL	--	C1
U	9-1-12	-178	-178	---	FRONT	VERT	TOTAL	--	C1
V	11-1-12	-178	-178	---	FRONT	VERT	TOTAL	--	C1
W	13-1-12	-178	-178	---	FRONT	VERT	TOTAL	--	C1
X	15-1-12	-178	-178	---	FRONT	VERT	TOTAL	--	C1
Y	17-1-12	-178	-178	---	FRONT	VERT	TOTAL	--	C1
Z	19-1-12	-178	-178	---	FRONT	VERT	TOTAL	--	C1
AA	21-1-12	-178	-178	---	FRONT	VERT	TOTAL	--	C1
AB	25-1-12	-178	-178	---	FRONT	VERT	TOTAL	--	C1
AC	27-1-12	-178	-178	---	FRONT	VERT	TOTAL	--	C1
AD	2-0-12	-36	-36	---	FRONT	VERT	TOTAL	--	C1
AE	4-0-12	-36	-36	---	FRONT	VERT	TOTAL	--	C1
AF	7-1-12	-36	-36	---	FRONT	VERT	TOTAL	--	C1
AG	9-1-12	-36	-36	---	FRONT	VERT	TOTAL	--	C1
AH	11-1-12	-36	-36	---	FRONT	VERT	TOTAL	--	C1
AI	13-1-12	-36	-36	---	FRONT	VERT	TOTAL	--	C1
AJ	15-1-12	-36	-36	---	FRONT	VERT	TOTAL	--	C1
AK	17-1-12	-36	-36	---	FRONT	VERT	TOTAL	--	C1
AL	19-1-12	-36	-36	---	FRONT	VERT	TOTAL	--	C1
AM	25-1-12	-36	-36	---	FRONT	VERT	TOTAL	--	C1
AN	27-1-12	-36	-36	---	FRONT	VERT	TOTAL	--	C1
AO	31-0-4	-36	-36	---	FRONT	VERT	TOTAL	--	C1
AP	33-0-4	-36	-36	---	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008047 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408315	T2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:33 2020 Page 1

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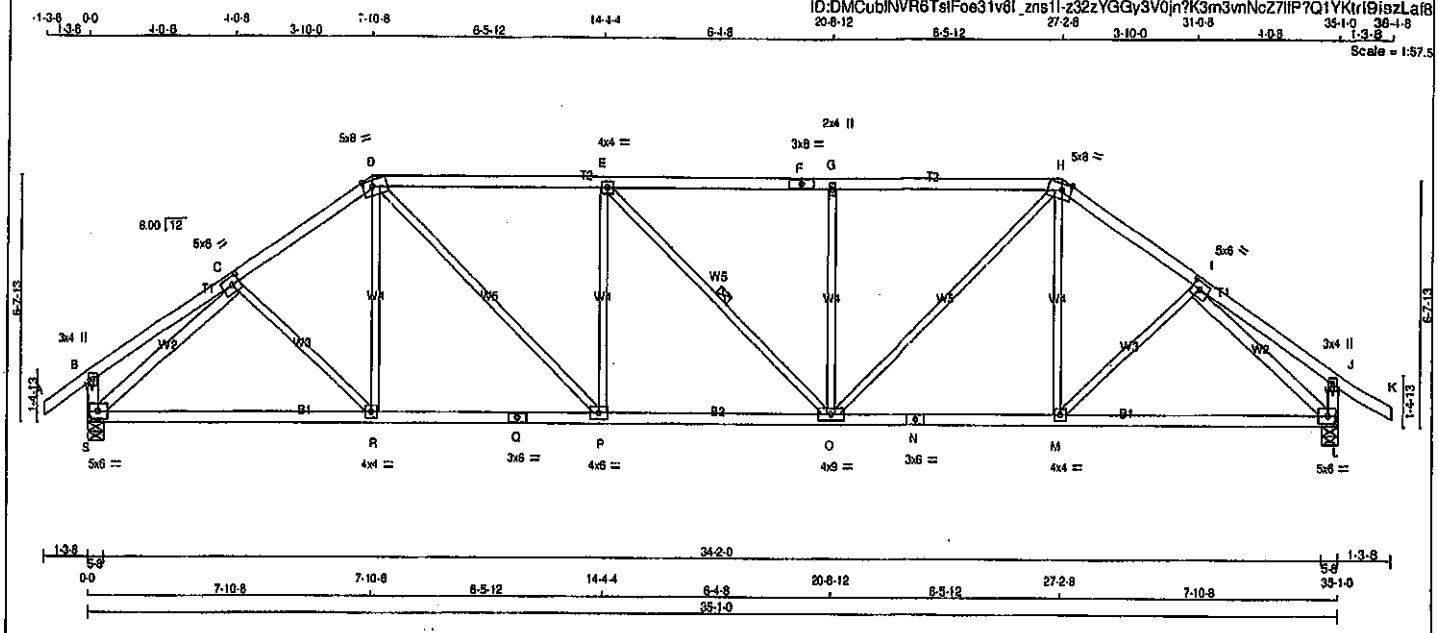

TOTAL WEIGHT = 151 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	8.0	2.00	3.25
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.00	3.25
I	TMWW-t	MT20	5.0	6.0	2.50	2.75
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.50	2.75
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	5.0	6.0	2.50	2.75

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008006

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	FACTORED UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2060	0	2060	0	0	5-8	5-8
L	2060	0	2060	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1454	968 / 0	0 / 0	0 / 0	0 / 0	487 / 0	0 / 0
L	1454	968 / 0	0 / 0	0 / 0	0 / 0	487 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B-C	0 / 22	-91.8	-91.8	0.21 (1)	10.00
C-D	-2282 / 0	-91.8	-91.8	0.30 (1)	4.25
D-E	-2644 / 0	-91.8	-91.8	0.76 (1)	3.47
E-F	-2643 / 0	-91.8	-91.8	0.77 (1)	3.47
F-G	-2643 / 0	-91.8	-91.8	0.77 (1)	3.47
G-H	-2643 / 0	-91.8	-91.8	0.77 (1)	3.49
H-I	-2282 / 0	-91.8	-91.8	0.30 (1)	4.25
I-J	0 / 22	-91.8	-91.8	0.21 (1)	10.00
J-K	0 / 35	-91.8	-91.8	0.12 (1)	10.00
S-B	-267 / 0	0.0	0.0	0.03 (1)	7.81
L-J	-267 / 0	0.0	0.0	0.03 (1)	7.81
S-R	0 / 1840	-18.5	-18.5	0.44 (1)	10.00
R-Q	0 / 1879	-18.5	-18.5	0.45 (1)	10.00
Q-P	0 / 1879	-18.5	-18.5	0.45 (1)	10.00
P-O	0 / 2645	-18.5	-18.5	0.49 (1)	10.00
O-N	0 / 1880	-18.5	-18.5	0.45 (1)	10.00
N-M	0 / 1880	-18.5	-18.5	0.45 (1)	10.00
M-L	0 / 1840	-18.5	-18.5	0.44 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-R	0 / 72	0.02 (4)
R-D	0 / 126	0.04 (4)
D-P	0 / 1071	0.24 (1)
P-E	-640 / 0	0.47 (1)
E-O	-2 / 0	0.00 (1)
O-G	-640 / 0	0.47 (1)
O-H	0 / 1069	0.24 (1)
M-H	0 / 126	0.04 (4)
M-I	0 / 72	0.02 (4)
S-C	-2527 / 0	0.86 (1)
I-L	-2527 / 0	0.86 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.78/1.00 (D-E:1) , BC=0.48/1.00 (O-P:1) , WB=0.86/1.00 (I-L:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	518	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.83 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	T3	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:34 2020 Page 1
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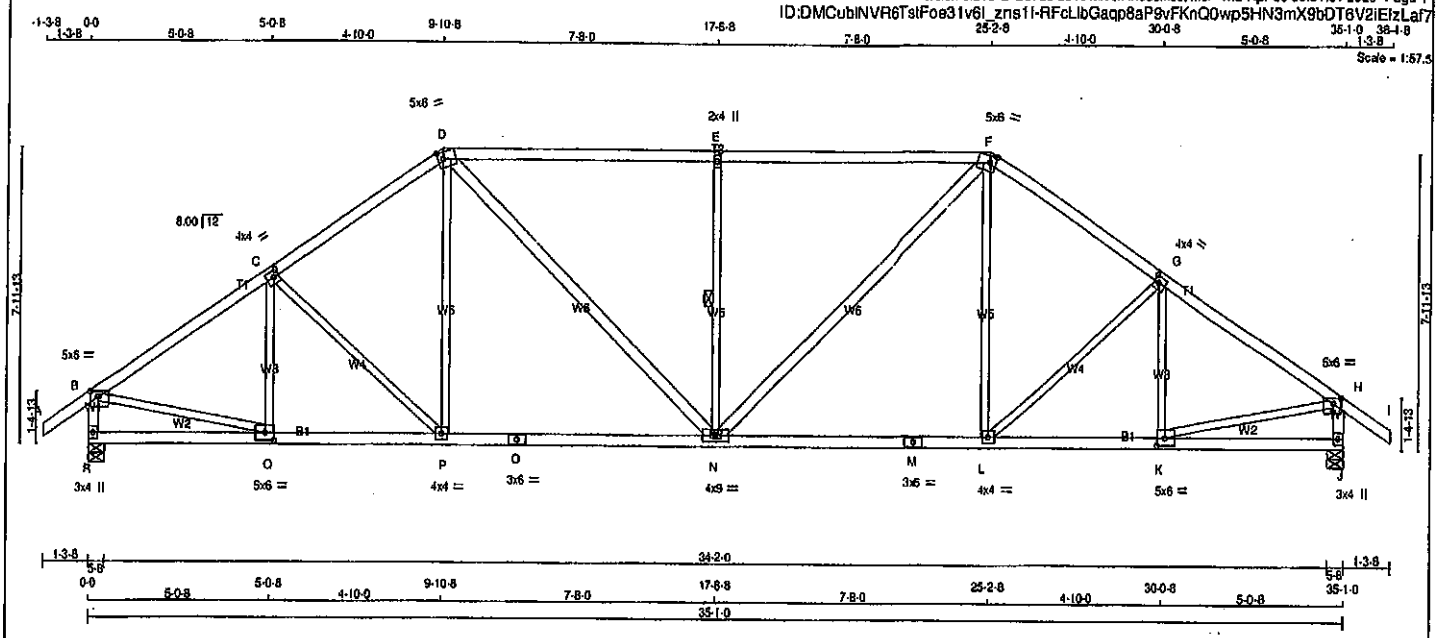

TOTAL WEIGHT = 157 lb
[M](F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - O	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	1.75	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	1.75
E	TMW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	1.75
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMW-p	MT20	5.0	6.0	1.75	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.75
R	BMV1+p	MT20	3.0	4.0		

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008007

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2060	0	2060	0	0	5-8	5-8
J	2060	0	2060	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1454	968 / 0	0 / 0	0 / 0	0 / 0	487 / 0	0 / 0
J	1454	968 / 0	0 / 0	0 / 0	0 / 0	487 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
B-C	-2307 / 0	-91.8	-91.8	0.37 (1)	4.19	
C-D	-2188 / 0	-91.8	-91.8	0.35 (1)	4.29	
D-E	-2302 / 0	-91.8	-91.8	0.85 (1)	3.38	
E-F	-2302 / 0	-91.8	-91.8	0.85 (1)	3.38	
F-G	-2188 / 0	-91.8	-91.8	0.35 (1)	4.29	
G-H	-2307 / 0	-91.8	-91.8	0.37 (1)	4.19	
H-I	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
R-B	-2017 / 0	0.0	0.0	0.21 (1)	5.94	
J-H	-2017 / 0	0.0	0.0	0.21 (1)	5.94	
R-Q	0 / 0	-18.5	-18.5	0.10 (4)	10.00	
Q-P	0 / 1944	-18.5	-18.5	0.40 (1)	10.00	
P-O	0 / 1797	-18.5	-18.5	0.41 (1)	10.00	
O-N	0 / 1797	-18.5	-18.5	0.41 (1)	10.00	
N-M	0 / 1797	-18.5	-18.5	0.41 (1)	10.00	
M-L	0 / 1797	-18.5	-18.5	0.41 (1)	10.00	
L-K	0 / 1944	-18.5	-18.5	0.40 (1)	10.00	
K-J	0 / 0	-18.5	-18.5	0.10 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-C	-329 / 0	0.11 (1)
C-P	-202 / 0	0.16 (1)
P-D	0 / 257	0.06 (4)
D-N	0 / 716	0.12 (1)
N-E	-887 / 0	0.35 (1)
N-F	0 / 716	0.12 (1)
L-F	0 / 257	0.06 (4)
L-G	-202 / 0	0.16 (1)
K-G	-329 / 0	0.11 (1)
B-Q	0 / 1988	0.45 (1)
K-H	0 / 1988	0.45 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC-2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.85/1.00 (E-F:1) , BC=0.41/1.00 (N-P:1) , WB=0.45/1.00 (B-Q:1) , SSI=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (8) (INPUT = 0.90)
JSI METAL= 0.59 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	T4	1	3	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 11:19:20 2020 Page 1
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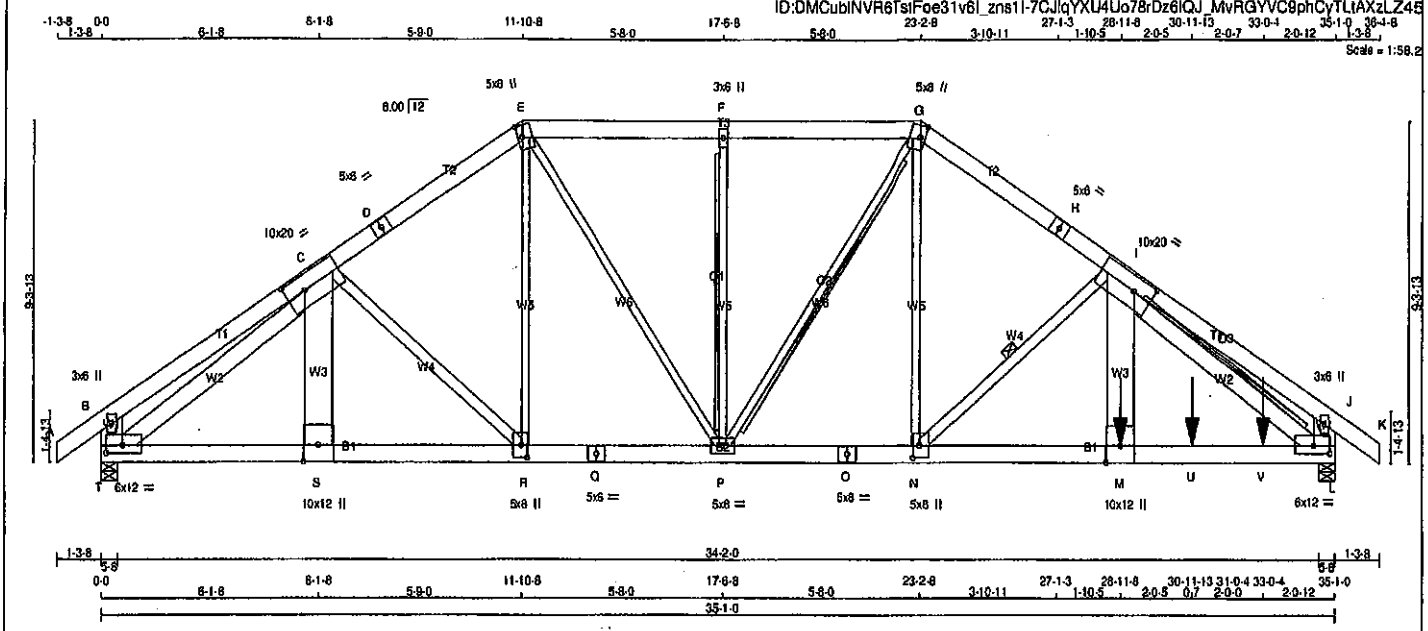

TOTAL WEIGHT = 3 X 250 = 750 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	1650F 1.5E	SPF
Q - O	2x6	DRY	1650F 1.5E	SPF
O - L	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x10	DRY	No.2	SPF
C - R	2x4	DRY	No.2	SPF
N - I	2x4	DRY	No.2	SPF
T - C	2x6	DRY	No.2	SPF
M - I	2x10	DRY	No.2	SPF
I - L	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-D	2	12	TOP
D-E	2	12	TOP
E-G	2	12	TOP
G-H	2	12	TOP
H-K	2	12	TOP
T-B	2	12	TOP
L-J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T-Q	2	7	TOP
Q-O	2	12	TOP
O-L	2	7	SIDE(549.3)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	8	
I-M	5	4	SIDE(3345.0)
2x10	5	4	

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008008 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	FACTORED UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	4365	0	4365	0	0	5-8	5-8
L	14398	0	14398	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	3082	2051 / 0	0 / 0	0 / 0	0 / 0	1031 / 0	0 / 0
L	10166	6764 / 0	0 / 0	0 / 0	0 / 0	3402 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF I-N.
2x6 DRY SPF No.2 T-BRACE AT F-P, G-P, H-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX	MAX. UNBRAC LENGTH
A-B	0 / 36	-91.8	-91.8	0.02 (1)		10.00
B-C	0 / 29	-91.8	-91.8	0.07 (1)		10.00
C-D	-5831 / 0	-91.8	-91.8	0.10 (1)		5.69
D-E	-5831 / 0	-91.8	-91.8	0.10 (1)		5.69
E-F	-8527 / 0	-91.8	-91.8	0.09 (1)		5.46
F-G	-8527 / 0	-91.8	-91.8	0.09 (1)		5.46
G-H	-9190 / 0	-91.8	-91.8	0.17 (1)		4.71
H-I	-9190 / 0	-91.8	-91.8	0.17 (1)		4.71
I-J	-8 / 0	-91.8	-91.8	0.10 (1)		10.00
J-K	0 / 36	-91.8	-91.8	0.02 (1)		10.00
T-B	-356 / 0	0.0	0.0	0.01 (1)		7.81
L-J	-421 / 0	0.0	0.0	0.01 (1)		7.81
T-S	0 / 4843	-18.5	-18.5	0.14 (1)		10.00
S-R	0 / 4844	-18.5	-18.5	0.13 (1)		10.00
R-Q	0 / 4820	-18.5	-18.5	0.15 (1)		10.00
Q-P	0 / 4820	-18.5	-18.5	0.15 (1)		10.00
P-O	0 / 7715	-18.5	-18.5	0.25 (1)		10.00
O-N	0 / 7715	-18.5	-18.5	0.25 (1)		10.00
N-M	0 / 15707	-18.5	-18.5	0.58 (1)		10.00
M-U	0 / 15528	-18.5	-18.5	0.80 (1)		10.00
U-V	0 / 15528	-18.5	-18.5	0.80 (1)		10.00
V-L	0 / 15528	-18.5	-18.5	0.80 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S-C	0 / 128	0.01 (4)
C-R	-33 / 0	0.01 (1)
R-E	0 / 122	0.01 (4)
E-P	0 / 3162	0.24 (1)
P-F	-569 / 0	0.09 (1)
P-G	-2201 / 0	0.52 (1)
N-G	0 / 6460	0.46 (1)
N-I	-10507 / 0	0.94 (1)
T-C	-6246 / 0	0.87 (1)
M-I	0 / 13739	0.38 (1)
I-L	-20028 / 0	0.98 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	28-11-8	-11723	-11723	---	BACK	VERT	TOTAL	---	C1
U	31-0-4	-1456	-1456	---	BACK	VERT	TOTAL	---	C1
V	33-0-4	-1456	-1456	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	OL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.17/1.00 (G-I:1) , BC=0.80/1.00 (L-M:1) , WB=0.98/1.00 (I-L:1) , SSI=0.34/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (R) (INPUT = 0.90)
JSI METAL= 0.98 (R) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408315	T4	1	3	TRUSS DESC.	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	6.0		
C	TMWWW-t	MT20	10.0	20.0	4.75	6.75
D	TS-t	MT20	5.0	6.0		
E	TTWW+m	MT20	5.0	8.0	4.00	1.50
F	TMW+w	MT20	3.0	6.0		
G	TTWW+m	MT20	5.0	8.0	4.00	1.50
H	TS-t	MT20	5.0	6.0		
I	TMWWW-t	MT20	10.0	20.0	4.75	6.75
J	TMV+p	MT20	3.0	6.0		
L	BMW1-t	MT20	6.0	12.0	2.50	5.50
M	BMW+w	MT20	10.0	12.0	Edge	
N	BMWW+t	MT20	5.0	8.0	4.00	2.00
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW+t	MT20	5.0	8.0	4.00	2.00
S	BMW+w	MT20	10.0	12.0	Edge	
T	BMW1-t	MT20	6.0	12.0	2.50	5.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008008 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	T5	6	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:37 2020 Page 1

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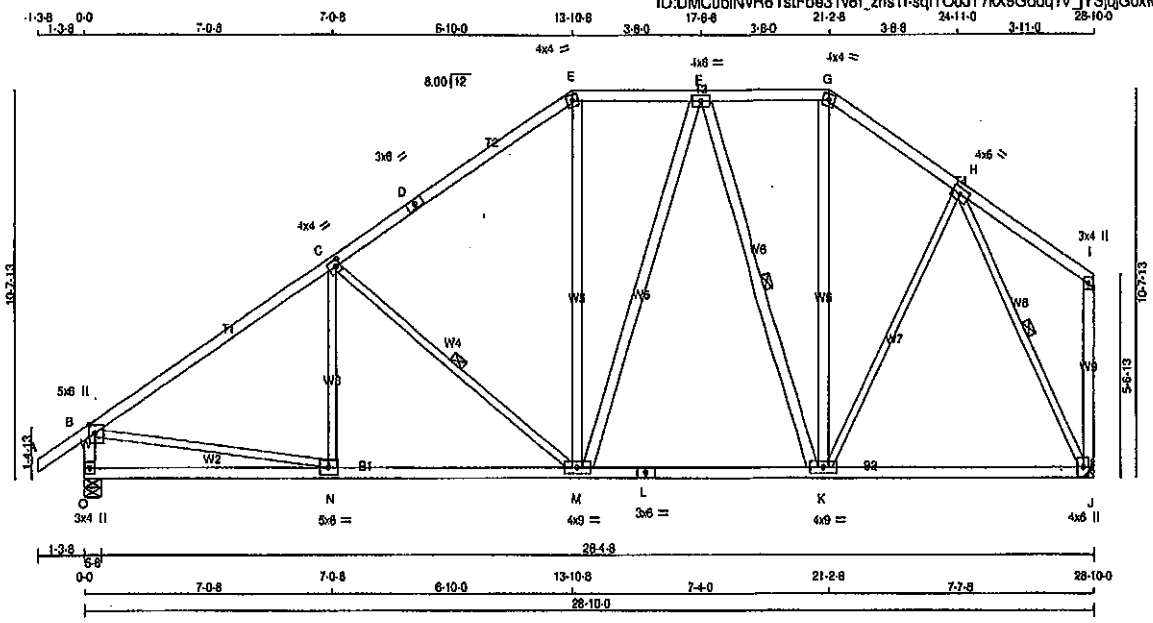

TOTAL WEIGHT = 6 X 158 = 948 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
N - C	2x3	DRY	No.2	SPF
C - M	2x3	DRY	No.2	SPF
K - H	2x3	DRY	No.2	SPF
B - N	2x3	DRY	No.2	SPF
H - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.75	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TTW-m	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	TMV+p	MT20	3.0	4.0		
J	BMVW1+p	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008009

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1715	0	1715	0	0	5-8	5-8
J	1589	0	1589	0	0	MECHANICAL	IN-SX IN-SX

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1211	808 / 0	0 / 0	0 / 0	0 / 0	403 / 0	0 / 0
J	1124	738 / 0	0 / 0	0 / 0	0 / 0	386 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, F-K, H-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
B- C	-1846 / 0	-91.8	-91.8	0.72 (1)	4.08	
C- D	-1374 / 0	-91.8	-91.8	0.64 (1)	4.68	
D- E	-1374 / 0	-91.8	-91.8	0.64 (1)	4.68	
E- F	-1111 / 0	-91.8	-91.8	0.16 (1)	5.83	
F- G	-881 / 0	-91.8	-91.8	0.16 (1)	6.25	
G- H	-1078 / 0	-91.8	-91.8	0.17 (1)	5.89	
H- I	0 / 22	-91.8	-91.8	0.21 (1)	10.00	
O- B	-1882 / 0	0.0	0.0	0.17 (1)	6.43	
J- I	-137 / 0	0.0	0.0	0.08 (1)	7.81	
O- N	0 / 0	-18.5	-18.5	0.22 (4)	10.00	
N- M	0 / 1573	-18.5	-18.5	0.37 (1)	10.00	
M- L	0 / 1065	-18.5	-18.5	0.35 (4)	10.00	
L- K	0 / 1065	-18.5	-18.5	0.35 (4)	10.00	
K- J	0 / 687	-18.5	-18.5	0.32 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-97 / 84	0.08 (1)
C- M	-607 / 0	0.34 (1)
M- E	0 / 372	0.06 (1)
M- F	0 / 143	0.02 (1)
F- K	-572 / 0	0.37 (1)
K- G	0 / 322	0.05 (1)
K- H	0 / 422	0.09 (1)
B- N	0 / 1592	0.38 (1)
H- J	-1555 / 0	0.79 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.72/1.00 (B-C:1) , BC=0.37/1.00 (M-N:1) , WB=0.79/1.00 (H-J:1) , SSI=0.26/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (B) (INPUT = 0.90)
JSI METAL= 0.77 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	T6	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:39 2020 Page 1

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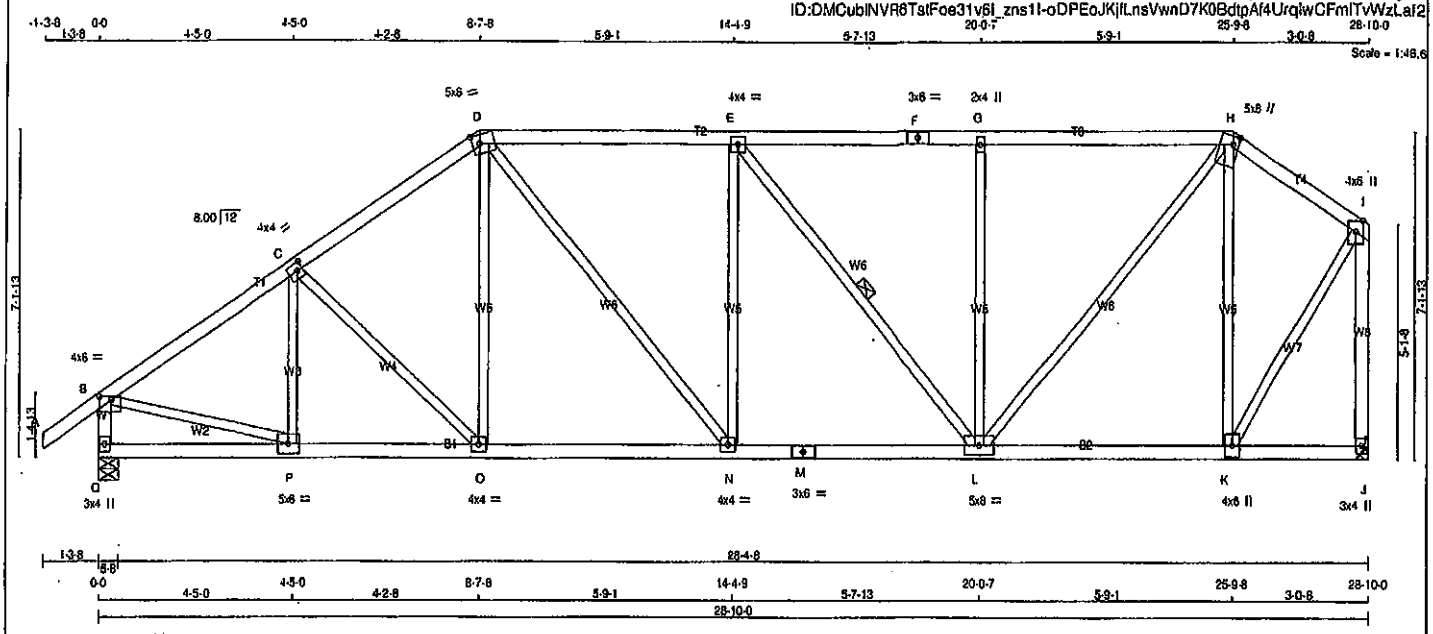

TOTAL WEIGHT = 135 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-I	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	5.0	6.0	2.25	1.50
I	TMVW+p	MT20	4.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW+t	MT20	4.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008010

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	UPLIFT	FACTORED INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1715	0	1715	0	0	5-8	5-8
J	1589	0	1589	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

MAX/MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1211	808 / 0	0 / 0	0 / 0	0 / 0	403 - 0	0 / 0
J	1124	738 / 0	0 / 0	0 / 0	0 / 0	388 - 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-1814 / 0	-91.8	-91.8	0.35 (1)	4.61
C- D	-1720 / 0	-91.8	-91.8	0.33 (1)	4.72
D- E	-1720 / 0	-91.8	-91.8	0.50 (1)	4.50
E- F	-1464 / 0	-91.8	-91.8	0.47 (1)	4.82
F- G	-1464 / 0	-91.8	-91.8	0.47 (1)	4.82
G- H	-1464 / 0	-91.8	-91.8	0.47 (1)	4.83
H- I	-786 / 0	-91.8	-91.8	0.15 (1)	8.25
Q- B	-1878 / 0	0.0	0.0	0.17 (1)	6.41
J- I	-1574 / 0	0.0	0.0	0.75 (1)	6.58
Q- P	0 / 0	-18.5	-18.5	0.07 (4)	10.00
P- O	0 / 1530	-18.5	-18.5	0.30 (1)	10.00
O- N	0 / 1410	-18.5	-18.5	0.29 (1)	10.00
N- M	0 / 1720	-18.5	-18.5	0.34 (1)	10.00
M- L	0 / 1720	-18.5	-18.5	0.34 (1)	10.00
L- K	0 / 639	-18.5	-18.5	0.17 (4)	10.00
K- J	0 / 0	-18.5	-18.5	0.09 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-295 / 0	0.08 (1)
C- O	-168 / 0	0.10 (1)
O- D	0 / 211	0.05 (1)
D- N	0 / 481	0.11 (1)
N- E	-261 / 2	0.23 (1)
E- L	-402 / 0	0.22 (1)
L- G	-569 / 0	0.51 (1)
L- H	0 / 1282	0.29 (1)
K- H	-900 / 0	0.80 (1)
B- P	0 / 1575	0.35 (1)
K- I	0 / 1191	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.75/1.00 (I-J:1) , BC=0.34/1.00 (L-N:1) , WB=0.80/1.00 (H-K:1) , SSI=0.25/1.00 (O-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.53 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	T7	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:40 2020 Page 1
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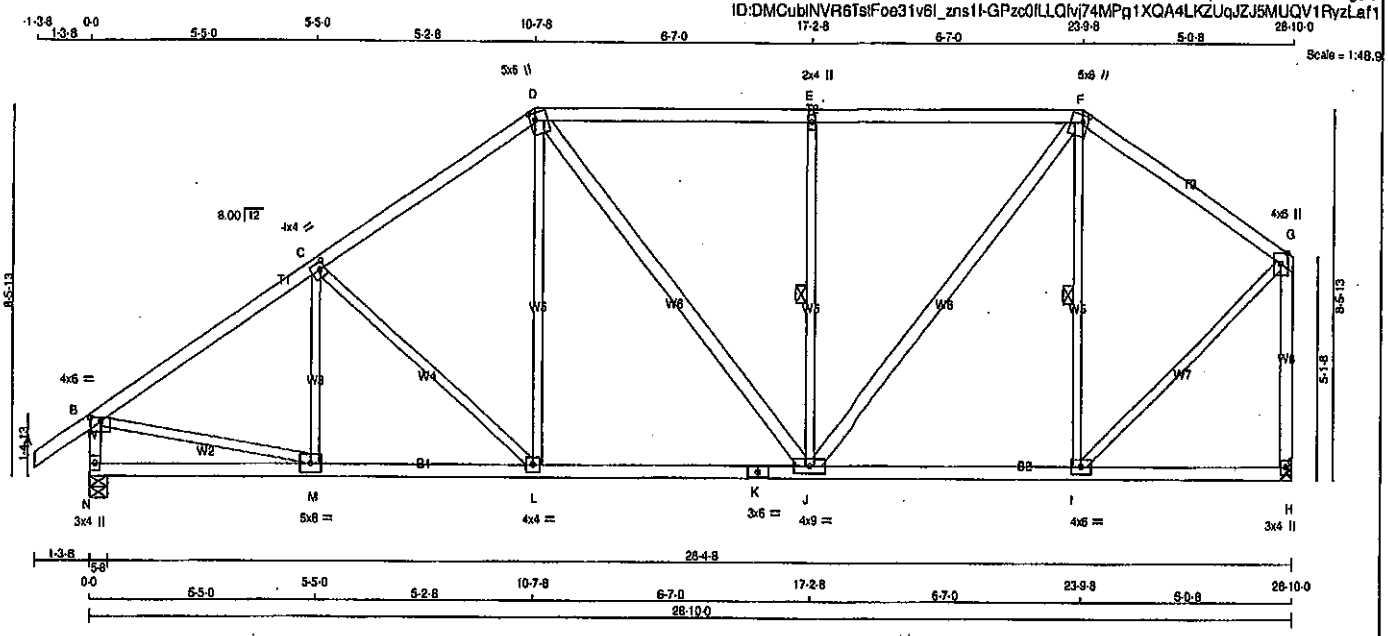

TOTAL WEIGHT = 139 lb
(M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
D - J	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	4.0	8.0	1.00	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	1.50
G	TMWW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008011

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	FACTORED UPLIFT	INPUT BRG	REQRD BRG
N	1715	0	1715	0	0	5-8	5-8
H	1589	0	1589	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX.MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1211	808 / 0	0 / 0	0 / 0	0 / 0	403 / 0	0 / 0
H	1124	738 / 0	0 / 0	0 / 0	0 / 0	386 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-J, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
B-C	-1848 / 0	-91.8	-91.8	0.39 (1)	4.54	
C-D	-1606 / 0	-91.8	-91.8	0.37 (1)	4.83	
D-E	-1412 / 0	-91.8	-91.8	0.55 (1)	4.77	
E-F	-1412 / 0	-91.8	-91.8	0.55 (1)	4.77	
F-G	-993 / 0	-91.8	-91.8	0.31 (1)	5.87	
N-B	-1672 / 0	0.0	0.0	0.17 (1)	6.42	
H-G	-1554 / 0	0.0	0.0	0.74 (1)	6.61	
N-M	0 / 0	-18.5	-18.5	0.11 (4)	10.00	
M-L	0 / 1564	-18.5	-18.5	0.32 (1)	10.00	
L-K	0 / 1311	-18.5	-18.5	0.29 (1)	10.00	
K-J	0 / 1311	-18.5	-18.5	0.29 (1)	10.00	
J-I	0 / 817	-18.5	-18.5	0.23 (4)	10.00	
I-H	0 / 0	-18.5	-18.5	0.14 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-C	-209 / 15	0.08 (1)
C-L	-343 / 0	0.33 (1)
L-D	0 / 338	0.08 (1)
D-J	0 / 162	0.03 (1)
J-E	-745 / 0	0.34 (1)
J-F	0 / 950	0.15 (1)
I-F	-650 / 0	0.30 (1)
B-M	0 / 1595	0.36 (1)
I-G	0 / 1129	0.25 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.74/1.00 (G-H:1) , BC=0.32/1.00 (L-M:1) , WB=0.36/1.00 (B-M:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.52 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	T8	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:42 2020 Page 1

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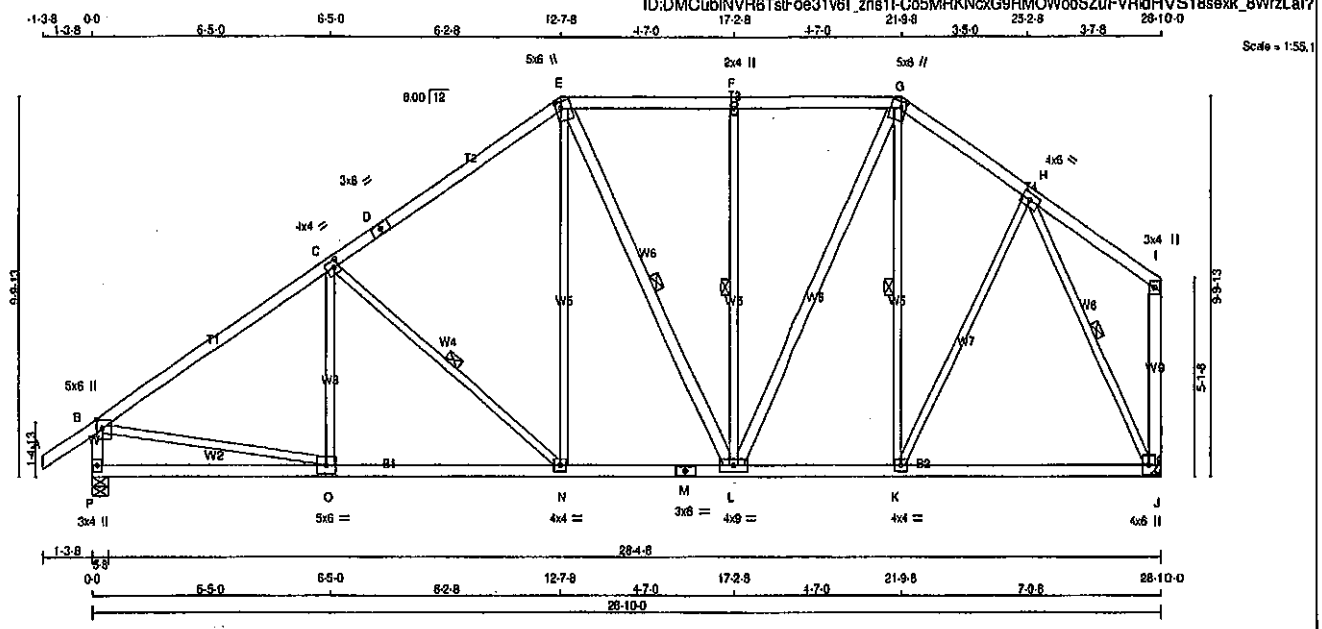

TOTAL WEIGHT = 153 lb [M|F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - L	2x4	DRY	No.2	SPF
L - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	2.75	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.00	1.50
H	TMWW-t	MT20	4.0	6.0		
I	TMV+p	MT20	3.0	4.0		
J	BMVW1+p	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008012

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
P	1716	0	1716	0	0	5-8	5-8
J	1589	0	1589	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1211	808 / 0	0 / 0	0 / 0	0 / 0	403 / 0	0 / 0
J	1124	738 / 0	0 / 0	0 / 0	0 / 0	386 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-N, E-L, F-L, G-K, H-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-1855 / 0	-91.8	-91.8	0.58 (1)	4.29
C- D	-1467 / 0	-91.8	-91.8	0.52 (1)	4.77
D- E	-1467 / 0	-91.8	-91.8	0.52 (1)	4.77
E- F	-1182 / 0	-91.8	-91.8	0.28 (1)	5.58
F- G	-1182 / 0	-91.8	-91.8	0.28 (1)	5.58
G- H	-1107 / 0	-91.8	-91.8	0.15 (1)	5.85
H- I	0 / 21	-91.8	-91.8	0.18 (1)	10.00
P- B	-1686 / 0	0.0	0.0	0.17 (1)	6.42
J- I	-126 / 0	0.0	0.0	0.06 (1)	7.81
P- O	0 / 0	-18.5	-18.5	0.18 (4)	10.00
O- N	0 / 1576	-18.5	-18.5	0.34 (1)	10.00
N- M	0 / 1191	-18.5	-18.5	0.25 (1)	10.00
M- L	0 / 1191	-18.5	-18.5	0.25 (1)	10.00
L- K	0 / 899	-18.5	-18.5	0.28 (4)	10.00
K- J	0 / 699	-18.5	-18.5	0.26 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-134 / 61	0.07 (1)
C- N	-510 / 0	0.24 (1)
N- E	0 / 435	0.10 (1)
E- L	-23 / 0	0.01 (4)
L- F	-512 / 0	0.33 (1)
L- G	0 / 653	0.10 (1)
K- G	-269 / 0	0.17 (1)
K- H	0 / 454	0.10 (1)
B- O	0 / 1598	0.38 (1)
H- J	-1573 / 0	0.66 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.58/1.00 (B-C:1) , BC=0.34/1.00 (N-O:1) , WB=0.66/1.00 (H-J:1) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.77 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	T9	4	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:43 2020 Page 1
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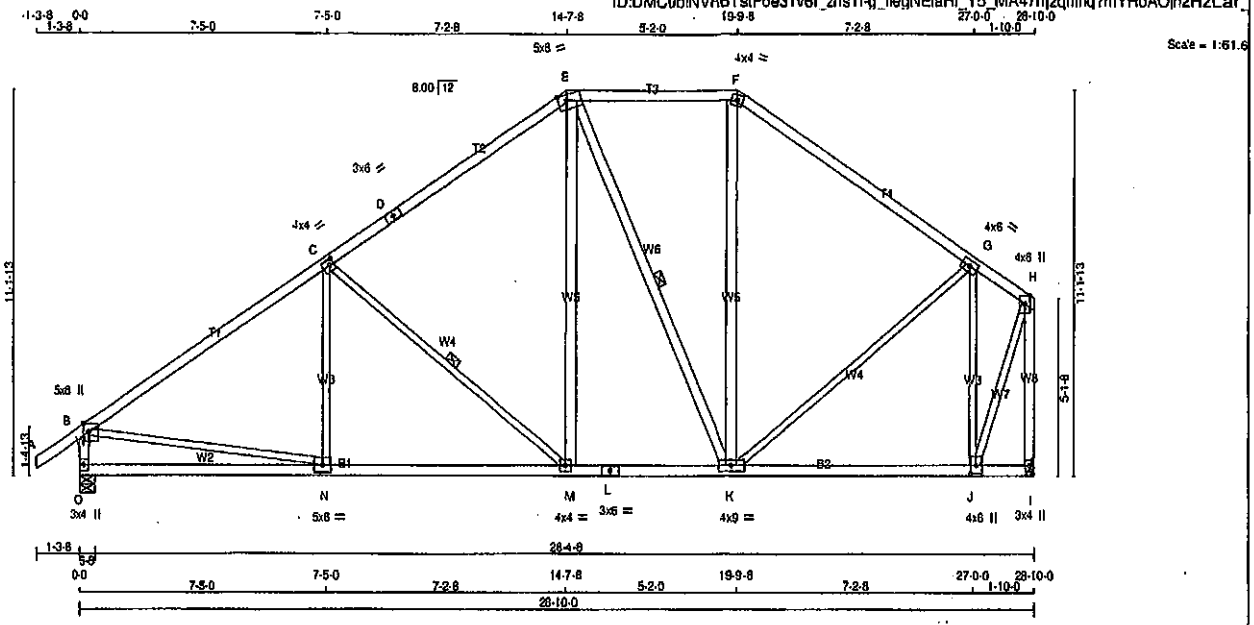

TOTAL WEIGHT = 4 X 151 = 605 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF
E - K	2x4	DRY	No.2	SPF
K - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.75	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.00	3.00
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMVW+p	MT20	4.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW+t	MT20	4.0	6.0	2.75	2.00
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1715	0	1715	0	0	5-8	5-8
I	1589	0	1589	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1211	808 / 0	0 / 0	0 / 0	0 / 0	403 / 0	0 / 0
I	1124	738 / 0	0 / 0	0 / 0	0 / 0	386 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, E-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
B- C	-1842 / 0	-91.8	-91.8	0.81 (1)	3.91	
C- D	-1314 / 0	-91.8	-91.8	0.72 (1)	4.56	
D- E	-1314 / 0	-91.8	-91.8	0.72 (1)	4.56	
E- F	-921 / 0	-91.8	-91.8	0.32 (1)	6.02	
F- G	-1141 / 0	-91.8	-91.8	0.62 (1)	5.07	
G- H	-509 / 0	-91.8	-91.8	0.49 (1)	6.25	
O- B	-1660 / 0	0.0	0.0	0.17 (1)	6.44	
I- H	-1608 / 0	0.0	0.0	0.77 (1)	6.52	
O- N	0 / 0	-18.5	-18.5	0.26 (4)	10.00	
N- M	0 / 1570	-18.5	-18.5	0.39 (1)	10.00	
M- L	0 / 1061	-18.5	-18.5	0.28 (1)	10.00	
L- K	0 / 1061	-18.5	-18.5	0.28 (1)	10.00	
K- J	0 / 540	-18.5	-18.5	0.21 (4)	10.00	
J- I	0 / 0	-18.5	-18.5	0.17 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-72 / 101	0.05 (1)
C- M	-667 / 0	0.42 (1)
M- E	0 / 525	0.08 (1)
E- K	-335 / 0	0.27 (1)
K- F	0 / 119	0.03 (4)
K- G	0 / 490	0.11 (1)
J- G	-1297 / 0	0.84 (1)
B- N	0 / 1587	0.38 (1)
J- H	0 / 1514	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.81/1.00 (B-C:1) , BC=0.39/1.00 (M-N:1) , WB=0.84/1.00 (G-J:1) , SSI=0.27/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.77 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008013

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	T10	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Rool Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:45 2020 Page 1

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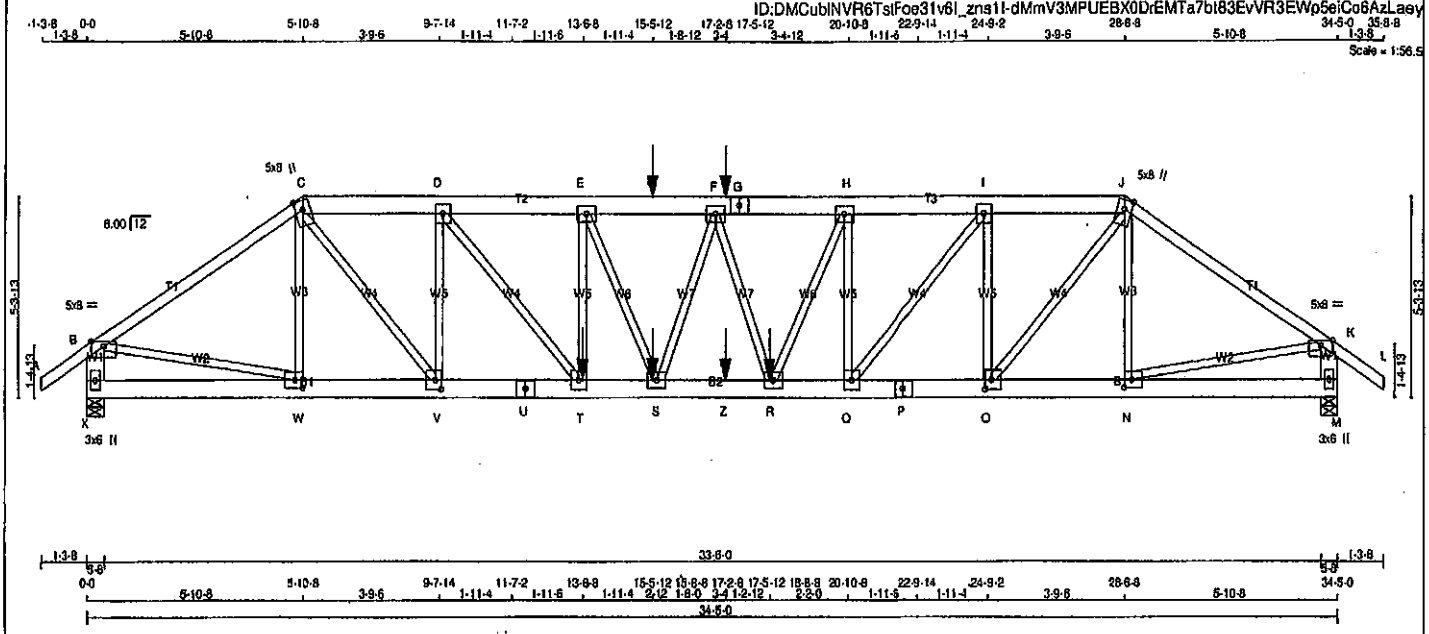

TOTAL WEIGHT = 2 X 190 = 380 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
X - U	2x6	DRY	No.2	SPF
U - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
J-L	1	12	TOP
C-G	2	12	SIDE(0.0)
G-J	2	12	TOP
X-B	2	12	TOP
M-K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
X-U	2	12	TOP
U-P	2	12	SIDE(183.1)
P-M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
T-E	1	6	SIDE(237.8)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008014 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	4128	0	4128	0	0	5-8	5-8
M	3945	0	3945	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX.MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	2908	1970 / 0	0 / 0	0 / 0	0 / 0	938 / 0	0 / 0
M	2780	1881 / 0	0 / 0	0 / 0	0 / 0	899 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	
B-C	-5285 / 0	-91.8	-91.8	0.57 (1)	3.79	
C-D	-6908 / 0	-91.8	-91.8	0.13 (1)	4.49	
D-E	-9033 / 0	-91.8	-91.8	0.19 (1)	3.96	
E-Y	-9338 / 0	-91.8	-91.8	0.25 (1)	3.83	
Y-F	-9338 / 0	-91.8	-91.8	0.25 (1)	3.83	
F-G	-9416 / 0	-91.8	-91.8	0.20 (1)	3.87	
G-H	-9416 / 0	-91.8	-91.8	0.20 (1)	3.87	
H-I	-8557 / 0	-91.8	-91.8	0.18 (1)	4.08	
I-J	-6544 / 0	-91.8	-91.8	0.12 (1)	4.60	
J-K	-5026 / 0	-91.8	-91.8	0.55 (1)	3.89	
K-L	0 / 35	-91.8	-91.8	0.07 (1)	10.00	
X-B	-4074 / 0	0.0	0.0	0.15 (1)	7.05	
M-K	-3894 / 0	0.0	0.0	0.14 (1)	7.18	
X-W	0 / 0	-18.5	-18.5	0.04 (4)	10.00	
W-V	0 / 4421	-18.5	-18.5	0.30 (1)	10.00	
V-U	0 / 6908	-18.5	-18.5	0.52 (1)	10.00	
U-T	0 / 6908	-18.5	-18.5	0.52 (1)	10.00	
T-S	0 / 9033	-18.5	-18.5	0.66 (1)	10.00	
S-Z	0 / 9480	-18.5	-18.5	0.73 (1)	10.00	
Z-R	0 / 9480	-18.5	-18.5	0.73 (1)	10.00	
R-Q	0 / 8557	-18.5	-18.5	0.67 (1)	10.00	
Q-P	0 / 6544	-18.5	-18.5	0.44 (1)	10.00	
P-O	0 / 6544	-18.5	-18.5	0.44 (1)	10.00	
O-N	0 / 4204	-18.5	-18.5	0.29 (1)	10.00	
N-M	0 / 0	-18.5	-18.5	0.04 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W-C	-681 / 0	0.14 (1)
N-J	-628 / 0	0.12 (1)
B-W	0 / 4472	0.55 (1)
N-K	0 / 4253	0.53 (1)
S-F	-462 / 0	0.10 (1)
F-R	-209 / 0	0.04 (1)
T-E	-1127 / 0	0.21 (1)
Q-H	-2292 / 0	0.43 (1)
R-H	0 / 2176	0.27 (1)
E-S	0 / 773	0.10 (1)
C-V	0 / 4063	0.50 (1)
V-D	-3073 / 0	0.58 (1)
D-T	0 / 3403	0.42 (1)
O-J	0 / 3824	0.47 (1)
O-I	-2988 / 0	0.56 (1)
Q-I	0 / 3223	0.40 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	17-5-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
R	18-8-8	-1986	-1986	---	BACK	VERT	TOTAL	---	C1
S	15-5-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
T	13-6-8	-1613	-1613	---	BACK	VERT	TOTAL	---	C1
Y	15-5-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
Z	17-5-12	-36	-36	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.15")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.15")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.36")

CSI: TC=0.57/1.00 (B-C:1) , BC=0.73/1.00 (R-S:1) , WB=0.68/1.00 (D-V:1) , SSI=0.10/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.85 (U) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408315	T10	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	Edge	
D	TMWW+t	MT20	5.0	6.0		
E, F, H						
E	TMWW-t	MT20	5.0	6.0		
G	TS-t	MT20	5.0	6.0		
I	TMWW+t	MT20	5.0	6.0		
J	TTWW+m	MT20	5.0	6.0	Edge	
K	TMVW-p	MT20	5.0	6.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMWW+t	MT20	5.0	6.0	3.00	2.00
P	BS-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMWW+t	MT20	5.0	6.0		
U	BS-t	MT20	5.0	6.0		
V	BMWW+t	MT20	5.0	6.0	3.00	2.00
W	BMWW-t	MT20	5.0	6.0	2.50	2.50
X	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008014

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	T11	1	3	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:46 2020 Page 1

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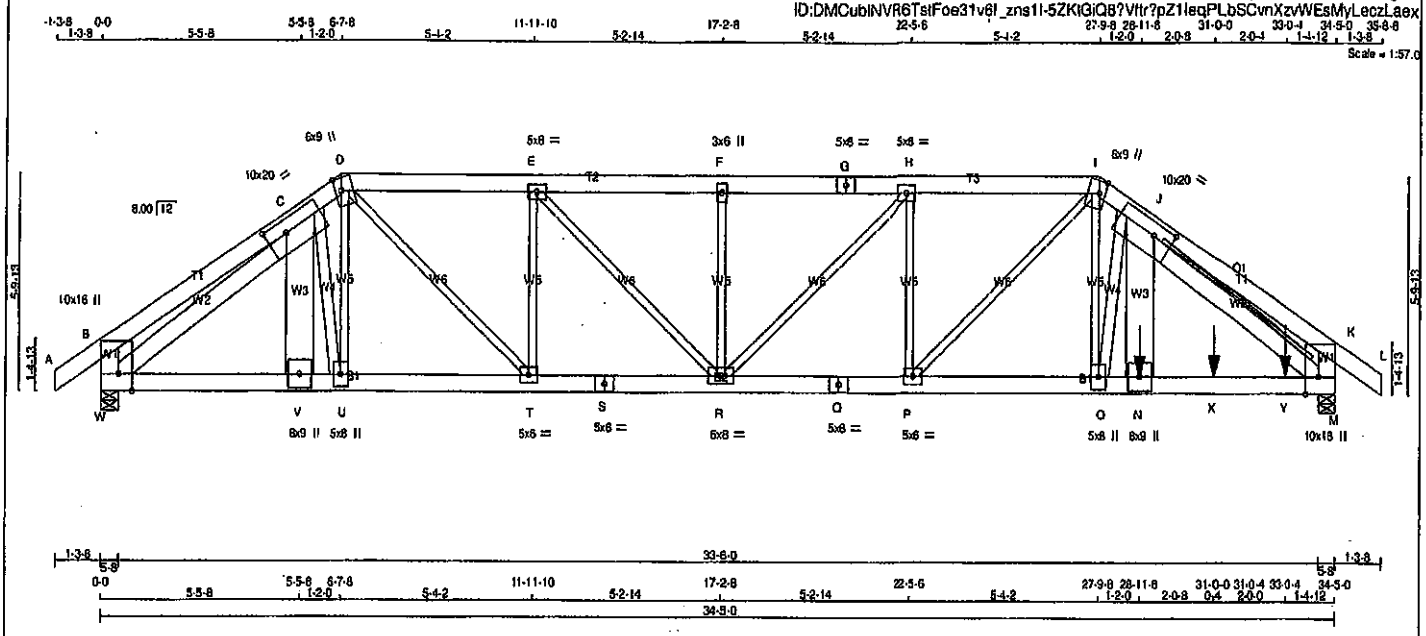

TOTAL WEIGHT = 3 X 238 = 713 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - L	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - S	2x6	DRY	1650F 1.5E	SPF
S - Q	2x6	DRY	1650F 1.5E	SPF
Q - M	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
V - C	2x10	DRY	No.2	SPF
C - U	2x4	DRY	No.2	SPF
W - C	2x6	DRY	No.2	SPF
N - J	2x10	DRY	No.2	SPF
O - J	2x4	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	TOP
D- G	2	12	TOP
G- I	2	12	TOP
I- L	2	12	TOP
W- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- S	2	5	TOP
S- Q	2	12	TOP
Q- M	2	5	SIDE(824.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	
J- N	5	4	SIDE(2940.4)
2x10	5	4	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008015

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	4072	0	4072	0	0	5-8	5-8
M	14508	0	14508	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2875	1913 / 0	0 / 0	0 / 0	0 / 0	962 / 0	0 / 0
M	10243	6815 / 0	0 / 0	0 / 0	0 / 0	3428 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M
BEARING SIZE FACTOR = 1.15 AT JNT(S) M (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT J-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 36	-91.8	-91.8	0.02 (1)	10.00
B- C	0 / 15	-91.8	-91.8	0.06 (1)	10.00
C- D	-5481 / 0	-91.8	-91.8	0.04 (1)	5.87
D- E	-7359 / 0	-91.8	-91.8	0.09 (1)	5.22
E- F	-9618 / 0	-91.8	-91.8	0.10 (1)	4.69
F- G	-9618 / 0	-91.8	-91.8	0.10 (1)	4.69
G- H	-9618 / 0	-91.8	-91.8	0.10 (1)	4.69
H- I	-11353 / 0	-91.8	-91.8	0.14 (1)	4.35
I- J	-15043 / 0	-91.8	-91.8	0.35 (1)	3.59
J- K	-114 / 0	-91.8	-91.8	0.25 (1)	6.25
K- L	0 / 36	-91.8	-91.8	0.02 (1)	10.00
W- B	-349 / 0	0.0	0.0	0.01 (1)	7.81
M- K	-580 / 0	0.0	0.0	0.01 (1)	7.81
W- V	0 / 4381	-18.5	-18.5	0.13 (1)	10.00
V- U	0 / 4377	-18.5	-18.5	0.13 (1)	10.00
U- T	0 / 4472	-18.5	-18.5	0.14 (1)	10.00
T- S	0 / 7359	-18.5	-18.5	0.20 (1)	10.00
S- R	0 / 7359	-18.5	-18.5	0.20 (1)	10.00
R- Q	0 / 11353	-18.5	-18.5	0.30 (1)	10.00
Q- P	0 / 11353	-18.5	-18.5	0.30 (1)	10.00
P- O	0 / 12609	-18.5	-18.5	0.38 (1)	10.00
O- N	0 / 15181	-18.5	-18.5	0.48 (1)	10.00
N- X	0 / 14810	-18.5	-18.5	0.72 (1)	10.00
X- Y	0 / 14810	-18.5	-18.5	0.72 (1)	10.00
Y- M	0 / 14810	-18.5	-18.5	0.72 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-150 / 21	0.00 (1)
C- U	0 / 321	0.02 (1)
U- D	-14 / 14	0.00 (1)
D- T	0 / 4079	0.31 (1)
T- E	-2816 / 0	0.42 (1)
E- R	0 / 3233	0.24 (1)
R- F	-447 / 0	0.07 (1)
R- H	-2481 / 0	0.87 (1)
P- H	0 / 1284	0.10 (1)
P- I	-1775 / 0	0.83 (1)
O- I	0 / 8859	0.66 (1)
W- C	-5708 / 0	0.60 (1)
N- J	0 / 13381	0.37 (1)
O- J	-9338 / 0	0.85 (1)
J- M	-19292 / 0	0.79 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	28-11-8	-11612	-11612	---	FRONT	VERT	TOTAL	---	C1
X	31-0-4	-1456	-1456	---	FRONT	VERT	TOTAL	---	C1
Y	33-0-4	-1456	-1456	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN.C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.15")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.15")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.35/1.00 (I-J:1) , BC=0.72/1.00 (M-N:1) , WB=0.87/1.00 (H-R:1) , SSI=0.31/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.92 (N) (INPUT = 1.00)

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, K, M, W						
B						
B	TMBMVW1*+p	MT20	10.0	16.0	Edge	4.50
C	TMWWW-t	MT20	10.0	20.0	4.00	6.75
D	TTWW+m	MT20	6.0	9.0	4.00	2.00
E	TMWW-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0		
I	TTWW+m	MT20	6.0	9.0	4.00	2.00
J	TMWWW-t	MT20	10.0	20.0	4.00	6.75
M	TMBMVW1*+p	MT20	10.0	16.0	Edge	4.50
N	BMW+w	MT20	8.0	9.0		
O	BMWW+t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	6.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMWW+t	MT20	5.0	6.0		
V	BMW+w	MT20	8.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008016

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	T12	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:48 2020 Page 1

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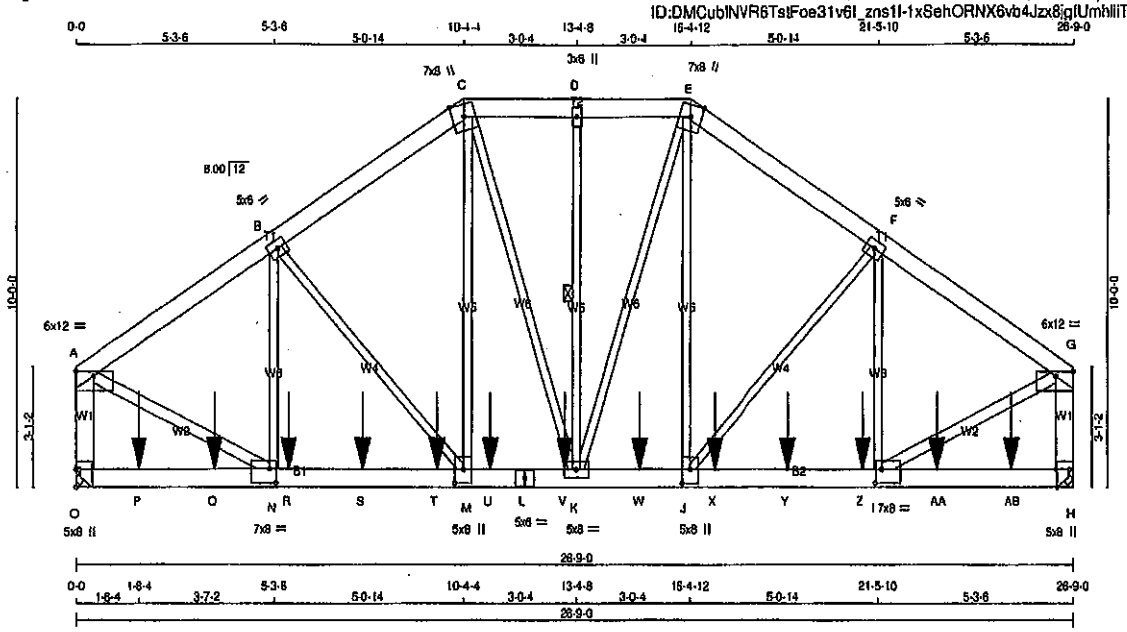

TOTAL WEIGHT = 2 X 184 = 369 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
O - A	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
O - L	2x6	DRY	2100F 1.8E	SPF
L - H	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
A - N	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	2	12	TOP
C-E	2	12	TOP
E-G	2	12	TOP
O-A	2	12	TOP
H-G	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O-L	2	12	SIDE(0.0)
L-H	2	12	SIDE(197.8)
WEBS : (0.122"X3") SPIRAL NAILS			
D-K	1	6	SIDE(212.1)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008017 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	11741	0	11741	0	0	MECHANICAL	
H	11631	0	11631	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT O, H. MINIMUM BEARING LENGTH AT JOINT O = 5-0, JOINT H = 5-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	8292	5505 / 0	0 / 0	0 / 0	0 / 0	2787 / 0	0 / 0
H	8214	5453 / 0	0 / 0	0 / 0	0 / 0	2761 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LG1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-10728 / 0	-91.8	-91.8	0.30 (1)	3.56
B-C	-10099 / 0	-91.8	-91.8	0.25 (1)	3.89
C-D	-8728 / 0	-91.8	-91.8	0.16 (1)	4.02
D-E	-8728 / 0	-91.8	-91.8	0.16 (1)	4.02
E-F	-9982 / 0	-91.8	-91.8	0.26 (1)	3.72
F-G	-10612 / 0	-91.8	-91.8	0.29 (1)	3.59
O-A	-10565 / 0	0.0	0.0	0.54 (1)	4.64
H-G	-10453 / 0	0.0	0.0	0.53 (1)	4.66
O-P	0 / 0	-18.5	-18.5	0.44 (1)	10.00
P-Q	0 / 0	-18.5	-18.5	0.44 (1)	10.00
Q-N	0 / 0	-18.5	-18.5	0.44 (1)	10.00
N-R	0 / 8933	-18.5	-18.5	0.67 (1)	10.00
R-S	0 / 8933	-18.5	-18.5	0.67 (1)	10.00
S-T	0 / 8933	-18.5	-18.5	0.67 (1)	10.00
T-M	0 / 8933	-18.5	-18.5	0.67 (1)	10.00
M-U	0 / 8425	-18.5	-18.5	0.42 (1)	10.00
U-L	0 / 8425	-18.5	-18.5	0.42 (1)	10.00
L-V	0 / 8425	-18.5	-18.5	0.42 (1)	10.00
V-K	0 / 8425	-18.5	-18.5	0.42 (1)	10.00
K-W	0 / 8305	-18.5	-18.5	0.43 (1)	10.00
W-J	0 / 8305	-18.5	-18.5	0.43 (1)	10.00
J-X	0 / 8837	-18.5	-18.5	0.67 (1)	10.00
X-Y	0 / 8837	-18.5	-18.5	0.67 (1)	10.00
Y-Z	0 / 8837	-18.5	-18.5	0.67 (1)	10.00
Z-I	0 / 8837	-18.5	-18.5	0.67 (1)	10.00
I-AA	0 / 0	-18.5	-18.5	0.44 (1)	10.00
AA-AB	0 / 0	-18.5	-18.5	0.44 (1)	10.00
AB-H	0 / 0	-18.5	-18.5	0.44 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-B	0 / 483	0.06 (1)
B-M	-858 / 0	0.55 (1)
M-C	0 / 4208	0.52 (1)
C-K	0 / 1003	0.12 (1)
K-D	-70 / 24	0.02 (1)
K-E	0 / 1399	0.17 (1)
J-E	0 / 3755	0.46 (1)
J-F	-887 / 0	0.57 (1)
I-F	0 / 515	0.06 (1)
A-N	0 / 9949	0.88 (1)
I-G	0 / 9843	0.87 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	1-8-4	-1571	-1571	---	BACK	VERT	TOTAL	---	C1
Q	3-8-4	-1571	-1571	---	BACK	VERT	TOTAL	---	C1
R	5-8-4	-1571	-1571	---	BACK	VERT	TOTAL	---	C1
S	7-8-4	-1571	-1571	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.89")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.89")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.54/1.00 (A-O:1) , BC=0.67/1.00 (M-N:1) , WB=0.88/1.00 (A-N:1) , SSI=0.93/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.84 (N) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408315	T12	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:48 2020 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	6.0	12.0	1.50	Edge
B	TMWW-t	MT20	5.0	8.0		
C	TTWW+m	MT20	7.0	8.0	Edge	
D	TMW+w	MT20	3.0	6.0		
E	TTWW+m	MT20	7.0	8.0	Edge	
F	TMWW-t	MT20	5.0	6.0		
G	TMWW-p	MT20	6.0	12.0	1.50	Edge
H	BMV1+t	MT20	5.0	8.0	Edge	0.50
I	BMWW-t	MT20	7.0	8.0	4.25	2.00
J	BMWW+t	MT20	5.0	8.0	4.25	2.50
K	BMWWW-t	MT20	5.0	8.0		
L	BS-t	MT20	5.0	6.0		
M	BMWW+t	MT20	5.0	8.0	4.25	2.50
N	BMWW-t	MT20	7.0	8.0	4.25	2.00
O	BMV1+t	MT20	5.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	9-8-4	-1571	-1571	---	BACK	VERT	TOTAL	---	C1
U	11-0-12	-1571	-1571	---	BACK	VERT	TOTAL	---	C1
V	13-0-12	-1571	-1571	---	BACK	VERT	TOTAL	---	C1
W	15-0-12	-1571	-1571	---	BACK	VERT	TOTAL	---	C1
X	17-0-12	-1571	-1571	---	BACK	VERT	TOTAL	---	C1
Y	19-0-12	-1571	-1571	---	BACK	VERT	TOTAL	---	C1
Z	21-0-12	-1571	-1571	---	BACK	VERT	TOTAL	---	C1
AA	23-0-12	-1571	-1571	---	BACK	VERT	TOTAL	---	C1
AB	25-0-12	-1571	-1571	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008017 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	T13	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:49 2020 Page 1

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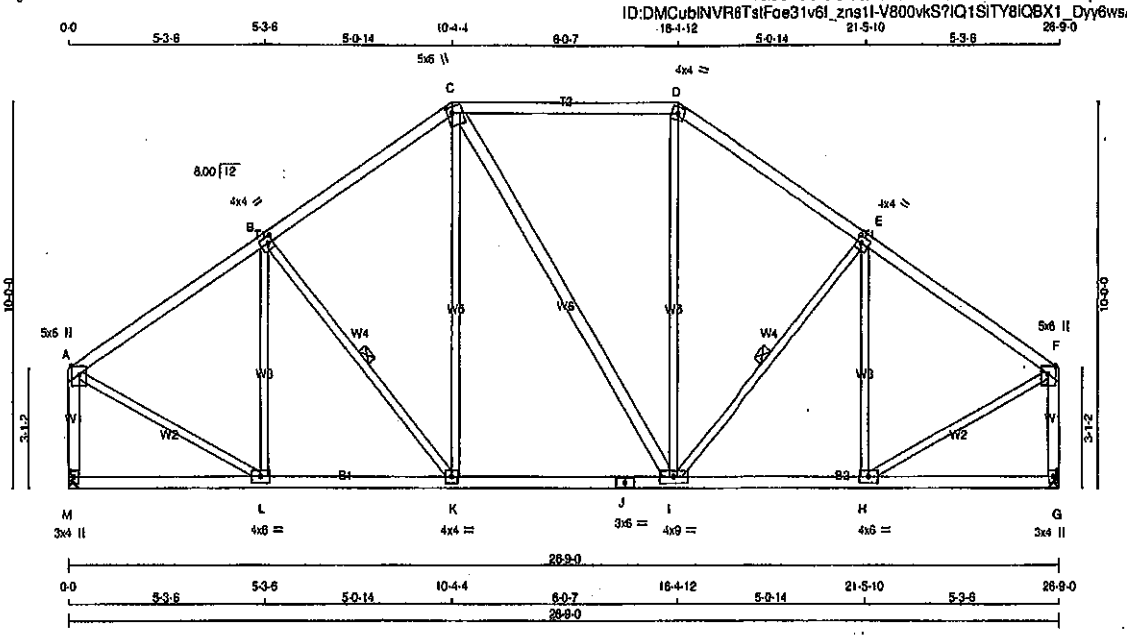

TOTAL WEIGHT = 132 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMWW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1475	0	1475	0	0	MECHANICAL	
G	1475	0	1475	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M, G. MINIMUM BEARING LENGTH AT JOINT M = 3-8, JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1043	684 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0
G	1043	684 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF B-K, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1227 / 0	-91.8	-91.8	0.34 (1)	5.39
B- C	-1198 / 0	-91.8	-91.8	0.34 (1)	5.44
C- D	-971 / 0	-91.8	-91.8	0.44 (1)	5.70
D- E	-1199 / 0	-91.8	-91.8	0.34 (1)	5.44
E- F	-1227 / 0	-91.8	-91.8	0.34 (1)	5.39
M- A	-1434 / 0	0.0	0.0	0.24 (1)	6.82
G- F	-1434 / 0	0.0	0.0	0.24 (1)	6.82
M- L	0 / 0	-18.5	-18.5	0.12 (4)	10.00
L- K	0 / 1047	-18.5	-18.5	0.23 (1)	10.00
K- J	0 / 971	-18.5	-18.5	0.22 (1)	10.00
J- I	0 / 971	-18.5	-18.5	0.22 (1)	10.00
I- H	0 / 1047	-18.5	-18.5	0.23 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.11 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	-445 / 0	0.32 (1)
B- K	-124 / 0	0.06 (1)
K- C	0 / 197	0.05 (4)
C- I	0 / 0	0.00 (1)
I- D	0 / 198	0.05 (4)
I- E	-124 / 0	0.06 (1)
H- E	-445 / 0	0.32 (1)
A- L	0 / 1163	0.27 (1)
H- F	0 / 1163	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.89")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.89")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.44/1.00 (C-D:1) , BC=0.23/1.00 (H-I:1) , WB=0.32/1.00 (E-H:1) , SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (A) (INPUT = 0.90)
JSI METAL= 0.50 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008018

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	T14S	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:50 2020 Page 1

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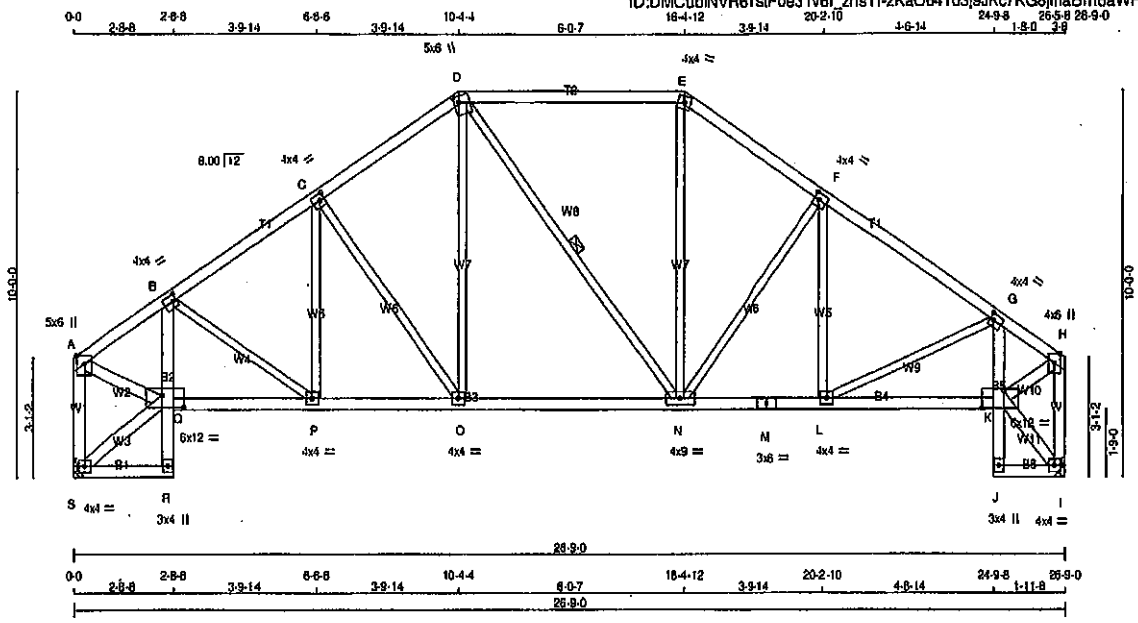

TOTAL WEIGHT = 138 lb

(M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
S - A	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
A - Q	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW-t	MT20	4.0	4.0	2.00	1.00
H	TMVW+p	MT20	4.0	6.0	Edge	
I	BMVW1-t	MT20	4.0	4.0		
J	BMV+p	MT20	3.0	4.0		
K	BVMWW-t	MT20	6.0	12.0	5.25	7.25
L, O, P						
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
Q	BVMWW-t	MT20	6.0	12.0	3.75	7.00
R	BMV+p	MT20	3.0	4.0		
S	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008019

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	1475	0	1475	0	0	MECHANICAL	
I	1475	0	1475	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT S, I. MINIMUM BEARING LENGTH AT JOINT S = 3-8, JOINT I = 3-8.

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1043	684 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0
I	1043	684 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-1511 / 0	-91.8	-91.8	0.11 (1)	5.25	
B-C	-1653 / 0	-91.8	-91.8	0.19 (1)	4.99	
C-D	-1440 / 0	-91.8	-91.8	0.18 (1)	5.28	
D-E	-1184 / 0	-91.8	-91.8	0.45 (1)	5.28	
E-F	-1445 / 0	-91.8	-91.8	0.21 (1)	5.23	
F-G	-1664 / 0	-91.8	-91.8	0.22 (1)	4.94	
G-H	-1355 / 0	-91.8	-91.8	0.14 (1)	5.43	
S-A	-1442 / 0	0.0	0.0	0.24 (1)	6.81	
I-H	-1444 / 0	0.0	0.0	0.24 (1)	6.80	
S-R	0 / 13	-18.5	-18.5	0.04 (4)	10.00	
R-Q	0 / 27	0.0	0.0	0.03 (1)	10.00	
Q-B	-460 / 0	0.0	0.0	0.03 (1)	7.81	
Q-P	0 / 1280	-18.5	-18.5	0.25 (1)	10.00	
P-O	0 / 1383	-18.5	-18.5	0.28 (1)	10.00	
O-N	0 / 1184	-18.5	-18.5	0.25 (1)	10.00	
N-M	0 / 1390	-18.5	-18.5	0.28 (1)	10.00	
M-L	0 / 1390	-18.5	-18.5	0.28 (1)	10.00	
L-K	0 / 1169	-18.5	-18.5	0.24 (1)	10.00	
J-K	0 / 19	0.0	0.0	0.04 (1)	10.00	
K-G	-613 / 0	0.0	0.0	0.03 (1)	7.81	
J-I	0 / 15	-18.5	-18.5	0.02 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-P	0 / 124	0.03 (1)
P-C	0 / 55	0.02 (4)
C-O	-345 / 0	0.28 (1)
O-D	0 / 375	0.08 (1)
D-N	0 / 0	0.00 (1)
N-E	0 / 383	0.09 (1)
N-F	-357 / 0	0.29 (1)
L-F	-24 / 54	0.02 (4)
L-G	0 / 244	0.05 (1)
S-Q	-16 / 0	0.00 (1)
A-Q	0 / 1370	0.22 (1)
K-I	-20 / 0	0.00 (1)
K-H	0 / 1330	0.21 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.89")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.89")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.45/1.00 (D-E:1) , BC=0.28/1.00 (L-N:1) , WB=0.29/1.00 (F-N:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.64 (H) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	G15	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:15 2020 Page 1

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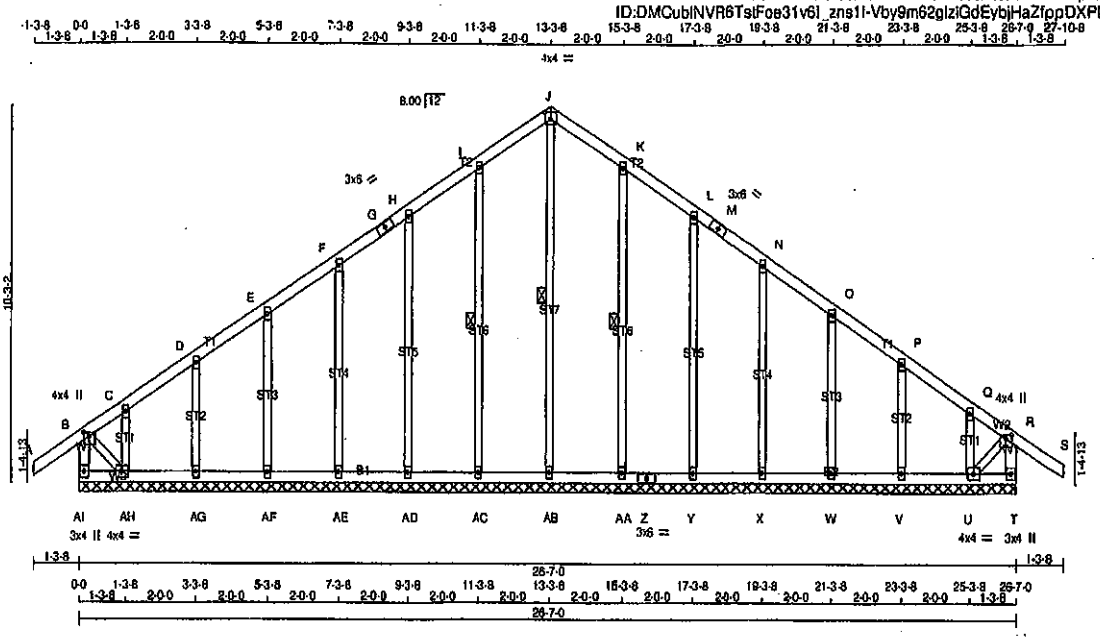

TOTAL WEIGHT = 132 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
M - S	2x4	DRY	No.2	SPF
AI - B	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
AI - Z	2x4	DRY	No.2	SPF
Z - T	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, F, H, I, K, L, N, O, P, Q						
C	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
J	TTW-p	MT20	4.0	4.0	2.25	2.00
M	TS-t	MT20	3.0	6.0		
R	TMVW+p	MT20	4.0	4.0	1.25	2.00
T	BMV1+p	MT20	3.0	4.0		
U	BMWW1-t	MT20	4.0	4.0		
V, W, X, Y, AA, AB, AC, AD, AE, AF, AG						
V	BMW1+w	MT20	2.0	4.0		
Z	BS-t	MT20	3.0	6.0		
AH	BMWW1-t	MT20	4.0	4.0		
AI	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF J-AB, I-AC, K-AA.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B-C	-70 / 0	-91.8	-91.8	0.12 (1)	6.25
C-D	-24 / 0	-91.8	-91.8	0.05 (1)	6.25
D-E	-27 / 0	-91.8	-91.8	0.05 (1)	6.25
E-F	-21 / 0	-91.8	-91.8	0.04 (1)	6.25
F-G	-19 / 0	-91.8	-91.8	0.04 (1)	6.25
G-H	-19 / 0	-91.8	-91.8	0.04 (1)	6.25
H-I	-14 / 0	-91.8	-91.8	0.05 (1)	6.25
I-J	-24 / 0	-91.8	-91.8	0.05 (1)	6.25
J-K	-24 / 0	-91.8	-91.8	0.05 (1)	6.25
K-L	-14 / 0	-91.8	-91.8	0.05 (1)	6.25
L-M	-19 / 0	-91.8	-91.8	0.04 (1)	6.25
M-N	-19 / 0	-91.8	-91.8	0.04 (1)	6.25
N-O	-21 / 0	-91.8	-91.8	0.04 (1)	6.25
O-P	-27 / 0	-91.8	-91.8	0.05 (1)	6.25
P-Q	-24 / 0	-91.8	-91.8	0.05 (1)	6.25
Q-R	-70 / 0	-91.8	-91.8	0.12 (1)	6.25
R-S	0 / 35	-91.8	-91.8	0.12 (1)	10.00
AI-B	-288 / 0	0.0	0.0	0.03 (1)	7.81
T-R	-288 / 0	0.0	0.0	0.03 (1)	7.81
AI-AH	0 / 0	-18.5	-18.5	0.01 (4)	10.00
AH-AG	0 / 26	-18.5	-18.5	0.02 (4)	10.00
AG-AF	0 / 21	-18.5	-18.5	0.02 (4)	10.00
AF-AE	0 / 18	-18.5	-18.5	0.02 (4)	10.00
AE-AD	0 / 15	-18.5	-18.5	0.02 (4)	10.00
AD-AC	0 / 13	-18.5	-18.5	0.02 (4)	10.00
AC-AB	0 / 11	-18.5	-18.5	0.01 (4)	10.00
AB-AA	0 / 11	-18.5	-18.5	0.01 (4)	10.00
AA-Z	0 / 13	-18.5	-18.5	0.02 (4)	10.00
Z-Y	0 / 13	-18.5	-18.5	0.02 (4)	10.00
Y-X	0 / 15	-18.5	-18.5	0.02 (4)	10.00
X-W	0 / 18	-18.5	-18.5	0.02 (4)	10.00
W-V	0 / 21	-18.5	-18.5	0.02 (4)	10.00
V-U	0 / 26	-18.5	-18.5	0.02 (4)	10.00
U-T	0 / 0	-18.5	-18.5	0.01 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AB-J	-139 / 0	0.10 (1)
AC-I	-208 / 0	0.11 (1)
AD-H	-177 / 0	0.19 (1)
AE-F	-183 / 0	0.11 (1)
AF-E	-178 / 0	0.08 (1)
AG-D	-195 / 0	0.04 (1)
AH-C	-70 / 0	0.01 (1)
AA-K	-208 / 0	0.11 (1)
Y-L	-177 / 0	0.19 (1)
X-N	-183 / 0	0.11 (1)
W-O	-178 / 0	0.08 (1)
V-P	-195 / 0	0.04 (1)
U-Q	-70 / 0	0.01 (1)
B-AH	0 / 38	0.01 (1)
U-R	0 / 38	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL.	=	25.6	PSF
	DL.	=	6.0	PSF
BOT CH.	LL.	=	0.0	PSF
	DL.	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (AG-AH:4) , WB=0.19/1.00 (L-Y:1) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (U) (INPUT = 0.90)
JSI METAL= 0.11 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2007993

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	T16	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:52 2020 Page 1
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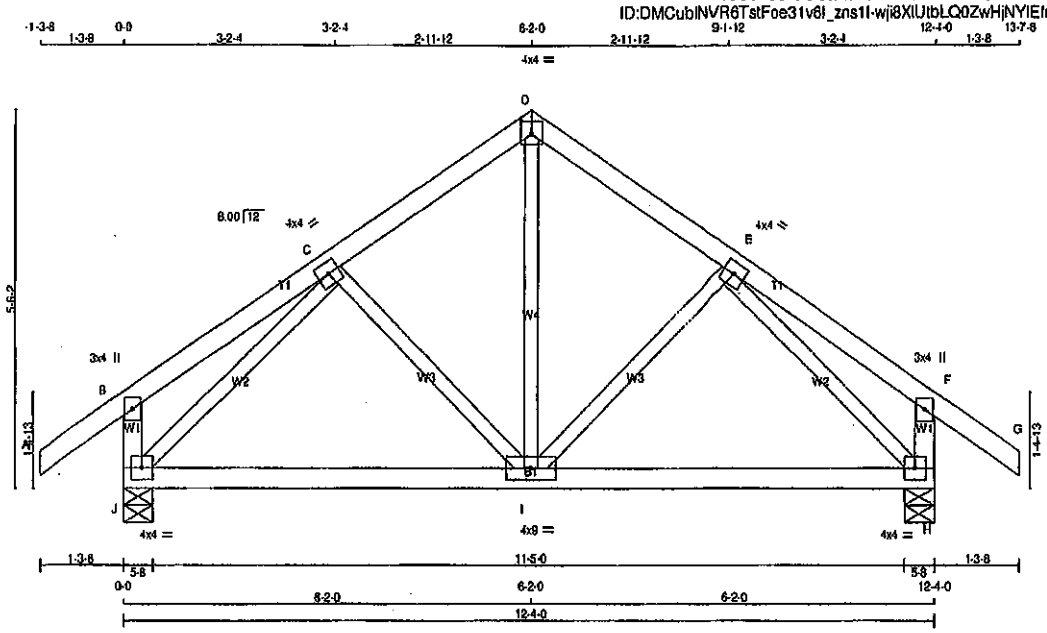

TOTAL WEIGHT = 54 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	806	0	806	0	0	5-8	5-8
H	806	0	806	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	568	386 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0
H	568	386 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
B- C	0 / 17	-91.8	-91.8	0.13 (1)	10.00	
C- D	-521 / 0	-91.8	-91.8	0.10 (1)	6.25	
D- E	-521 / 0	-91.8	-91.8	0.10 (1)	6.25	
E- F	0 / 17	-91.8	-91.8	0.13 (1)	10.00	
F- G	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
J- B	-236 / 0	0.0	0.0	0.02 (1)	7.81	
H- F	-236 / 0	0.0	0.0	0.02 (1)	7.81	
J- I	0 / 517	-18.5	-18.5	0.24 (4)	10.00	
I- H	0 / 517	-18.5	-18.5	0.24 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 337	0.08 (1)
I- E	-139 / 0	0.04 (1)
C- I	-139 / 0	0.04 (1)
J- C	-737 / 0	0.23 (1)
E- H	-737 / 0	0.23 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN.C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010. NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.13/1.00 (B-C:1) , BC=0.24/1.00 (H-I:4) , WB=0.23/1.00 (C-J:1) , SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.26 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008020

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	G16	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:16 2020 Page 1
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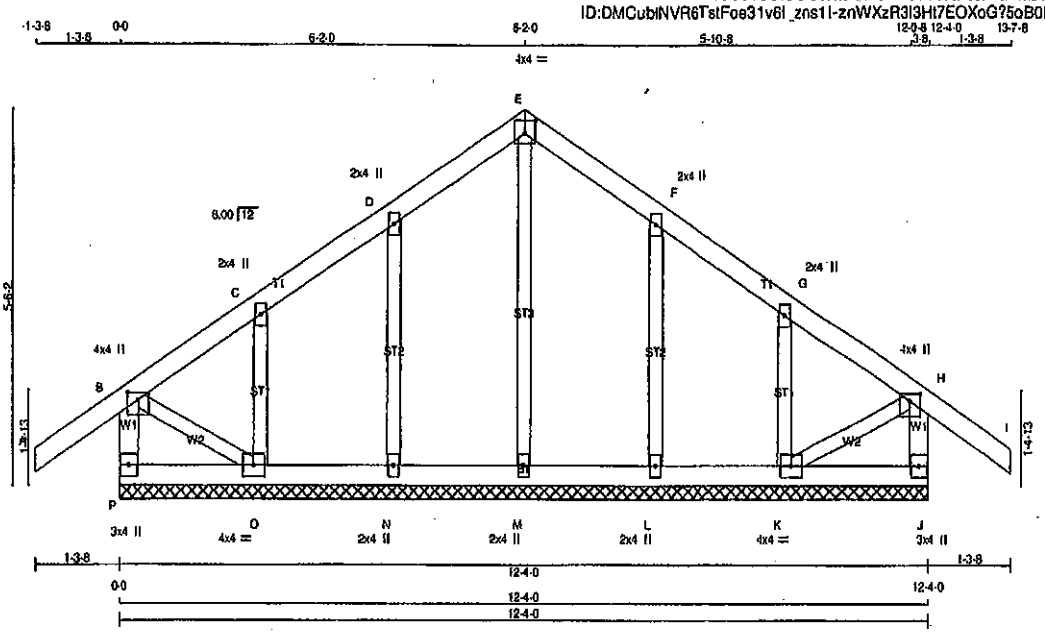

TOTAL WEIGHT = 54 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0	1.25	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	2.25	2.00
H	TMW+p	MT20	4.0	4.0	1.25	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-1	MT20	4.0	4.0		
L, M, N						
L	BMV1+w	MT20	2.0	4.0		
O	BMWW1-1	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
P-B	-232 / 0	0.0	0.0	0.02 (1)	7.81	
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
B-C	-19 / 0	-91.8	-91.8	0.05 (1)	6.25	
C-D	-22 / 0	-91.8	-91.8	0.05 (1)	6.25	
D-E	-27 / 0	-91.8	-91.8	0.05 (1)	6.25	
E-F	-27 / 0	-91.8	-91.8	0.05 (1)	6.25	
F-G	-22 / 0	-91.8	-91.8	0.05 (1)	6.25	
G-H	-19 / 0	-91.8	-91.8	0.05 (1)	6.25	
H-I	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
J-H	-232 / 0	0.0	0.0	0.02 (1)	7.81	
P-O	0 / 0	-18.5	-18.5	0.02 (4)	10.00	
O-N	0 / 18	-18.5	-18.5	0.02 (4)	10.00	
N-M	0 / 14	-18.5	-18.5	0.02 (4)	10.00	
M-L	0 / 14	-18.5	-18.5	0.02 (4)	10.00	
L-K	0 / 18	-18.5	-18.5	0.02 (4)	10.00	
K-J	0 / 0	-18.5	-18.5	0.02 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-E	-137 / 0	0.08 (1)
N-D	-202 / 0	0.05 (1)
O-C	-202 / 0	0.03 (1)
L-F	-202 / 0	0.05 (1)
K-G	-202 / 0	0.03 (1)
B-O	0 / 28	0.01 (1)
K-H	0 / 28	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (H-I:1) , BC=0.02/1.00 (K-L:4) , WB=0.08/1.00 (E-M:1) , SSI=0.08/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.10 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2007994

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	T17	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:53 2020 Page 1
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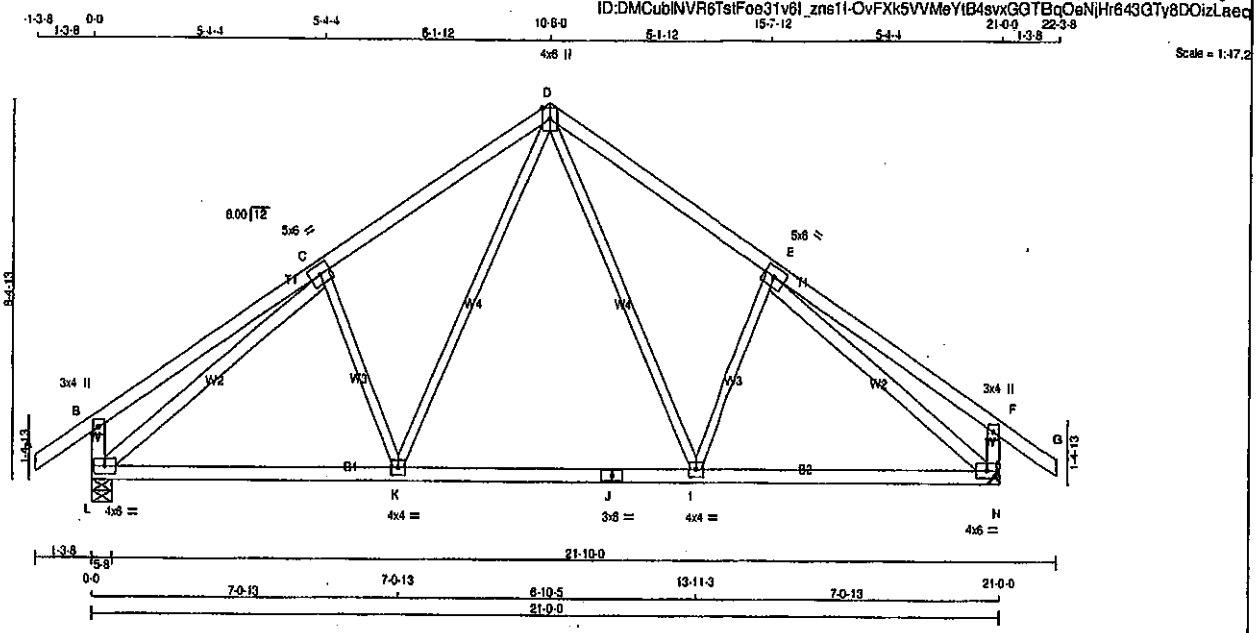

TOTAL WEIGHT = 2 X 95 = 191 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCH.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1284	0	1284	0	0	5-8	5-8
H	1284	0	1284	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	905	608 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
H	905	608 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 32	-91.8	-91.8	0.42 (1)	10.00
C- D	-1155 / 0	-91.8	-91.8	0.34 (1)	5.52
D- E	-1155 / 0	-91.8	-91.8	0.34 (1)	5.52
E- F	0 / 32	-91.8	-91.8	0.42 (1)	10.00
F- G	0 / 35	-91.8	-91.8	0.12 (1)	10.00
L- B	-307 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-307 / 0	0.0	0.0	0.03 (1)	7.81
L- K	0 / 1056	-18.5	-18.5	0.29 (4)	10.00
K- J	0 / 749	-18.5	-18.5	0.27 (4)	10.00
J- I	0 / 749	-18.5	-18.5	0.27 (4)	10.00
I- H	0 / 1056	-18.5	-18.5	0.29 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 475	0.11 (1)
I- E	-321 / 0	0.13 (1)
K- D	0 / 475	0.11 (1)
C- K	-321 / 0	0.13 (1)
L- C	-1403 / 0	0.89 (1)
E- H	-1403 / 0	0.89 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.42/1.00 (B-C:1) , BC=0.29/1.00 (K-L:4) , WB=0.89/1.00 (C-L:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (E) (INPUT = 0.80)
JSI METAL= 0.36 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008021

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	G17	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:17 2020 Page 1
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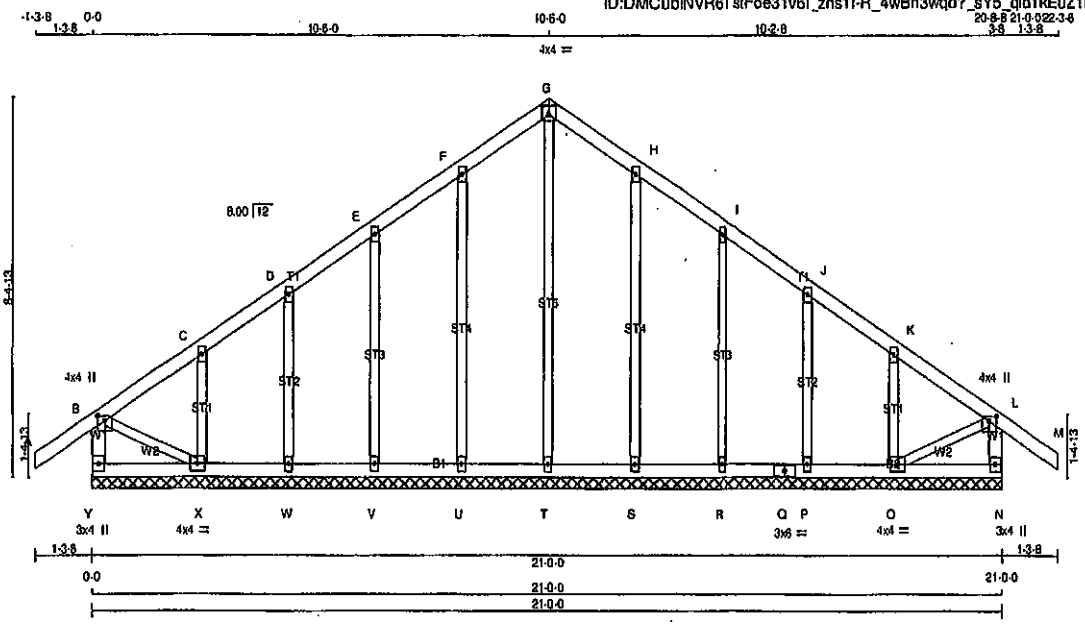

TOTAL WEIGHT = 98 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Y - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW-p	MT20	4.0	4.0	2.25	2.00
L	TMVW+p	MT20	4.0	4.0	1.25	2.00
N	BMV1+p	MT20	3.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P, R, S, T, U, V, W						
P	BMW1+w	MT20	2.0	4.0		
Q	BS-t	MT20	3.0	6.0		
X	BMWW1-t	MT20	4.0	4.0		
Y	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LCI	MAX CSI (LC)	MAX. UNBRAC LENGTH
Y- B	-250 / 0	0.0	0.0	0.03 (1)	7.81	
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
B- C	-26 / 0	-91.8	-91.8	0.07 (1)	6.25	
C- D	-36 / 0	-91.8	-91.8	0.07 (1)	6.25	
D- E	-24 / 0	-91.8	-91.8	0.04 (1)	6.25	
E- F	-20 / 0	-91.8	-91.8	0.05 (1)	6.25	
F- G	-29 / 0	-91.8	-91.8	0.05 (1)	6.25	
G- H	-29 / 0	-91.8	-91.8	0.05 (1)	6.25	
H- I	-20 / 0	-91.8	-91.8	0.05 (1)	6.25	
I- J	-24 / 0	-91.8	-91.8	0.04 (1)	6.25	
J- K	-36 / 0	-91.8	-91.8	0.07 (1)	6.25	
K- L	-26 / 0	-91.8	-91.8	0.07 (1)	6.25	
L- M	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
N- L	-250 / 0	0.0	0.0	0.03 (1)	7.81	
Y- X	0 / 0	-18.5	-18.5	0.03 (4)	10.00	
X- W	0 / 24	-18.5	-18.5	0.03 (4)	10.00	
W- V	0 / 21	-18.5	-18.5	0.02 (4)	10.00	
V- U	0 / 18	-18.5	-18.5	0.02 (4)	10.00	
U- T	0 / 18	-18.5	-18.5	0.02 (4)	10.00	
T- S	0 / 18	-18.5	-18.5	0.02 (4)	10.00	
S- R	0 / 18	-18.5	-18.5	0.02 (4)	10.00	
R- Q	0 / 21	-18.5	-18.5	0.02 (4)	10.00	
Q- P	0 / 21	-18.5	-18.5	0.02 (4)	10.00	
P- O	0 / 24	-18.5	-18.5	0.03 (4)	10.00	
O- N	0 / 0	-18.5	-18.5	0.03 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- G	-133 / 0	0.18 (1)
U- F	-203 / 0	0.18 (1)
V- E	-180 / 0	0.09 (1)
W- D	-167 / 0	0.05 (1)
X- C	-231 / 0	0.04 (1)
S- H	-203 / 0	0.18 (1)
R- I	-180 / 0	0.09 (1)
P- J	-167 / 0	0.05 (1)
O- K	-231 / 0	0.04 (1)
B- X	0 / 34	0.01 (1)
O- L	0 / 34	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (L-M:1) , BC=0.03/1.00 (W-X:4) , WB=0.18/1.00 (G-T:1) , SSI=0.09/1.00 (B-C:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (X) (INPUT = 0.90)
JSI METAL= 0.12 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2007995

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408315	T18	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:55 2020 Page 1
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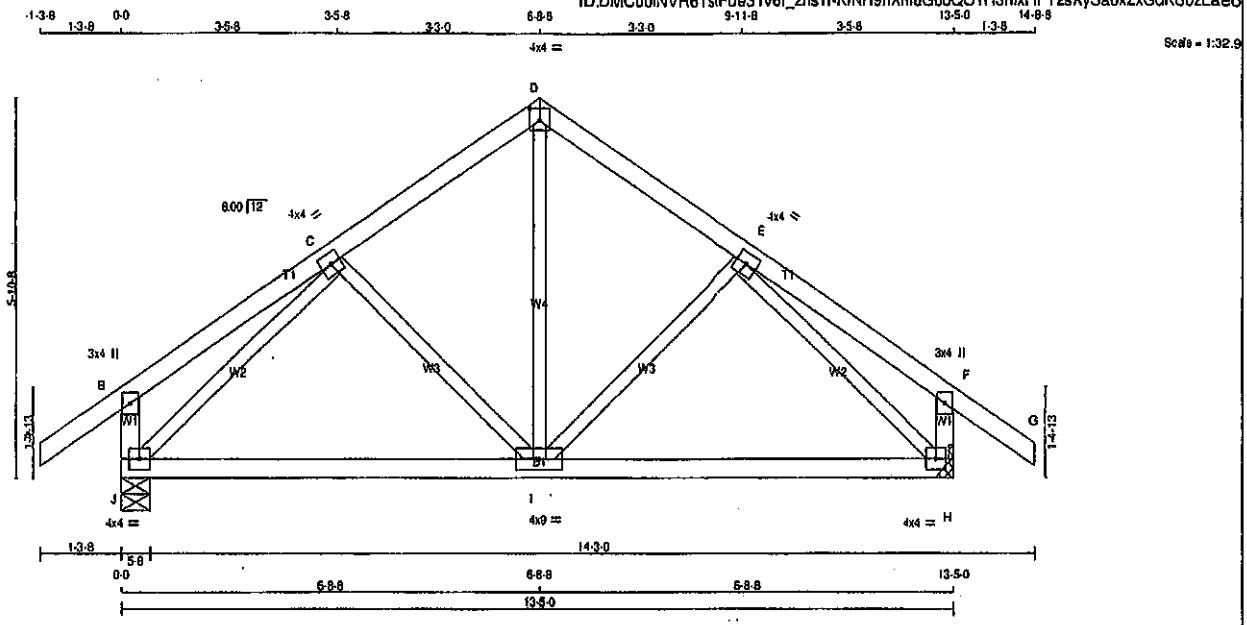

TOTAL WEIGHT = 3 X 58 = 175 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	866	0	866	0	0	5-8	5-8
H	866	0	866	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	610	413 / 0	0 / 0	0 / 0	0 / 0	195 / 0	0 / 0
H	610	413 / 0	0 / 0	0 / 0	0 / 0	196 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
B-C	0 / 19	-91.8	-91.8	0.16 (1)	10.00	
C-D	-577 / 0	-91.8	-91.8	0.12 (1)	8.25	
D-E	-577 / 0	-91.8	-91.8	0.12 (1)	8.25	
E-F	0 / 19	-91.8	-91.8	0.16 (1)	10.00	
F-G	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
J-B	-245 / 0	0.0	0.0	0.03 (1)	7.81	
H-F	-245 / 0	0.0	0.0	0.03 (1)	7.81	
J-I	0 / 579	-18.5	-18.5	0.28 (4)	10.00	
I-H	0 / 579	-18.5	-18.5	0.28 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-D	0 / 379	0.09 (1)
I-E	-161 / 0	0.08 (1)
C-I	-161 / 0	0.08 (1)
J-C	-815 / 0	0.29 (1)
E-H	-815 / 0	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.16/1.00 (B-C:1) , BC=0.28/1.00 (H-I:4) , WB=0.29/1.00 (C-J:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	766	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (E) (INPUT = 0.90)
JSI METAL= 0.29 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008022

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	G18	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:19 2020 Page 1
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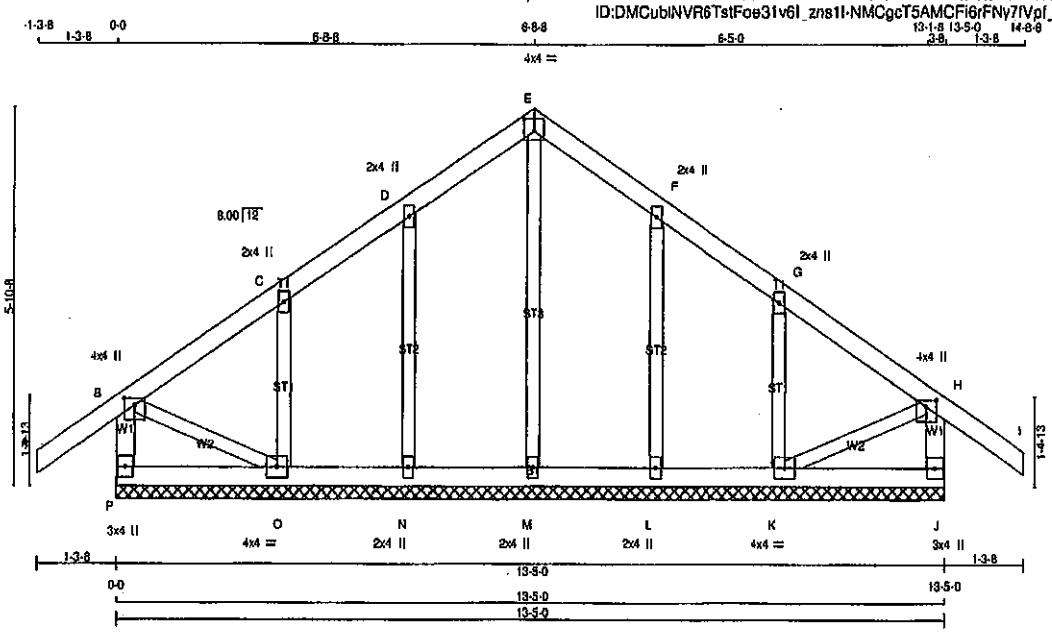

TOTAL WEIGHT = 59 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	2.25	2.00
H	TMVW+p	MT20	4.0	4.0	1.25	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
P-B	-252 / 0	0.0	0.0	0.03 (1)	7.81	
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
B-C	-19 / 0	-91.8	-91.8	0.08 (1)	6.25	
C-D	-29 / 0	-91.8	-91.8	0.08 (1)	6.25	
D-E	-27 / 0	-91.8	-91.8	0.05 (1)	6.25	
E-F	-27 / 0	-91.8	-91.8	0.05 (1)	6.25	
F-G	-29 / 0	-91.8	-91.8	0.08 (1)	6.25	
G-H	-19 / 0	-91.8	-91.8	0.08 (1)	6.25	
H-I	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
J-H	-252 / 0	0.0	0.0	0.03 (1)	7.81	
P-O	0 / 0	-18.5	-18.5	0.03 (4)	10.00	
O-N	0 / 19	-18.5	-18.5	0.03 (4)	10.00	
N-M	0 / 15	-18.5	-18.5	0.02 (4)	10.00	
M-L	0 / 15	-18.5	-18.5	0.02 (4)	10.00	
L-K	0 / 19	-18.5	-18.5	0.03 (4)	10.00	
K-J	0 / 0	-18.5	-18.5	0.03 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
M-E	-138 / 0	0.07 (1)
N-D	-189 / 0	0.06 (1)
O-C	-241 / 0	0.05 (1)
L-F	-189 / 0	0.06 (1)
K-G	-241 / 0	0.05 (1)
B-O	0 / 28	0.01 (1)
K-H	0 / 28	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (H-I:1) , BC=0.03/1.00 (N-O:4) , WB=0.07/1.00 (E-M:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (H) (INPUT = 0.90)
JSI METAL= 0.12 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2007996

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408316	T18S	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:21 2020 Page 1

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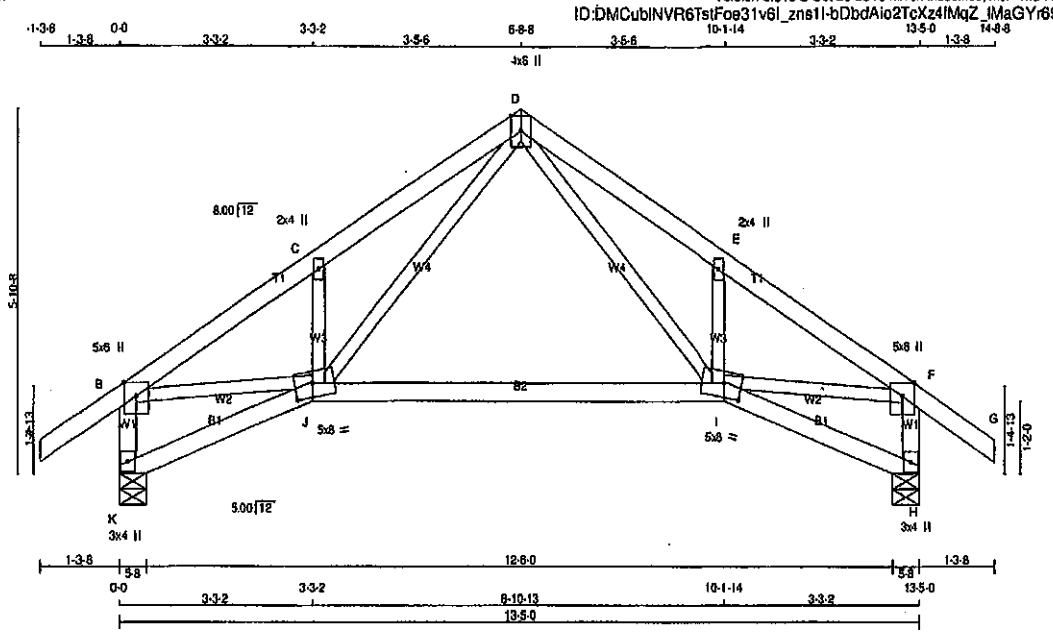

TOTAL WEIGHT = 2 X 57 = 115 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	5.0	6.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BBWWW-m	MT20	5.0	8.0	2.75	3.50
J	BBWWW-m	MT20	5.0	8.0	2.75	3.50
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	FACTORED UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	866	0	866	0	0	5-8	5-8
H	866	0	866	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	610	413 / 0	0 / 0	0 / 0	0 / 0	196 / 0	0 / 0
H	610	413 / 0	0 / 0	0 / 0	0 / 0	196 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LCI	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
B-C	-1092 / 0	-91.8	-91.8	0.12 (1)	5.93	
C-D	-1130 / 0	-91.8	-91.8	0.13 (1)	5.84	
D-E	-1130 / 0	-91.8	-91.8	0.13 (1)	5.84	
E-F	-1092 / 0	-91.8	-91.8	0.12 (1)	5.93	
F-G	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
K-B	-835 / 0	0.0	0.0	0.09 (1)	7.81	
H-F	-835 / 0	0.0	0.0	0.09 (1)	7.81	
K-J	0 / 0	-18.5	-18.5	0.06 (4)	10.00	
J-I	0 / 541	-18.5	-18.5	0.30 (4)	10.00	
I-H	0 / 0	-18.5	-18.5	0.06 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D-I	0 / 623	0.14 (1)
I-E	-369 / 0	0.08 (1)
J-D	0 / 623	0.14 (1)
J-C	-369 / 0	0.08 (1)
B-J	0 / 923	0.21 (1)
I-F	0 / 923	0.21 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.13/1.00 (C-D:1) , BC=0.30/1.00 (I-J:4) , WB=0.21/1.00 (F-I:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	786	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (F) (INPUT = 0.90)
JSI METAL= 0.46 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008027

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	T19	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:56 2020 Page 1

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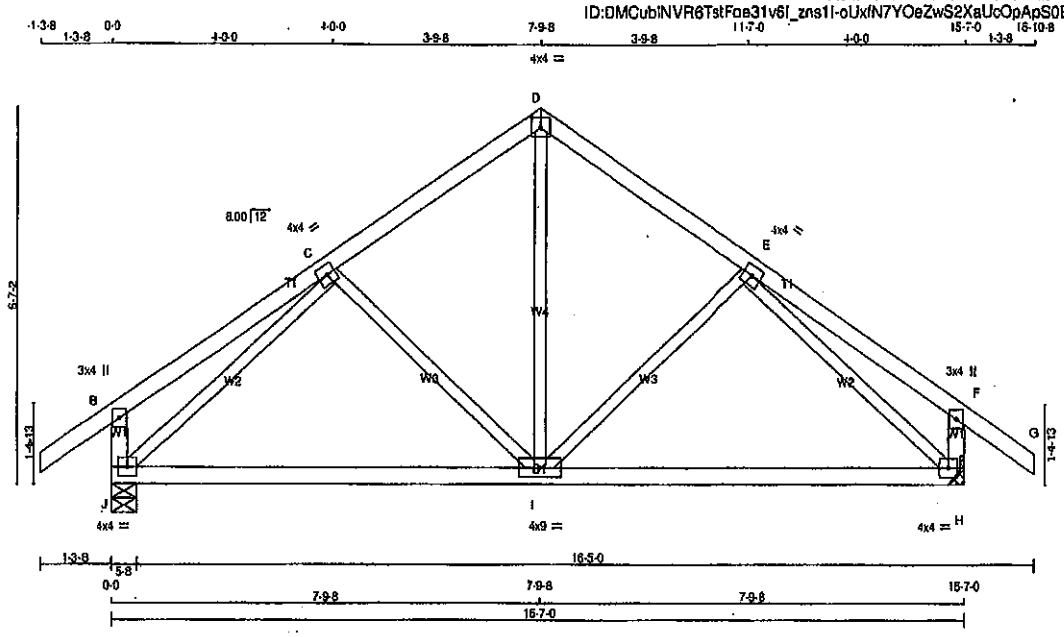

TOTAL WEIGHT = 3 X 66 = 199 lb

(M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-I	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-I	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-I	MT20	4.0	4.0		
I	BMWWW-I	MT20	4.0	9.0		
J	BMVW1-I	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	985	0	985	0	0	5-8	5-8
H	985	0	985	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	694	469 / 0	0 / 0	0 / 0	0 / 0	225 / 0	0 / 0
H	694	469 / 0	0 / 0	0 / 0	0 / 0	225 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
B-C	0 / 23	-91.8	-91.8	0.22 (1)	10.00	
C-D	-689 / 0	-91.8	-91.8	0.17 (1)	6.25	
D-E	-689 / 0	-91.8	-91.8	0.17 (1)	6.25	
E-F	0 / 23	-91.8	-91.8	0.22 (1)	10.00	
F-G	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
J-B	-263 / 0	0.0	0.0	0.03 (1)	7.81	
H-F	-263 / 0	0.0	0.0	0.03 (1)	7.81	
J-I	0 / 706	-18.5	-18.5	0.38 (4)	10.00	
I-H	0 / 706	-18.5	-18.5	0.38 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-D	0 / 463	0.10 (1)
I-E	-208 / 0	0.10 (1)
C-I	-208 / 0	0.10 (1)
J-C	-971 / 0	0.45 (1)
E-H	-971 / 0	0.45 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.38/1.00 (I-J:4) , WB=0.45/1.00 (C-J:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (J) (INPUT = 0.90)
JSI METAL= 0.34 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	T20S	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:57 2020 Page 1
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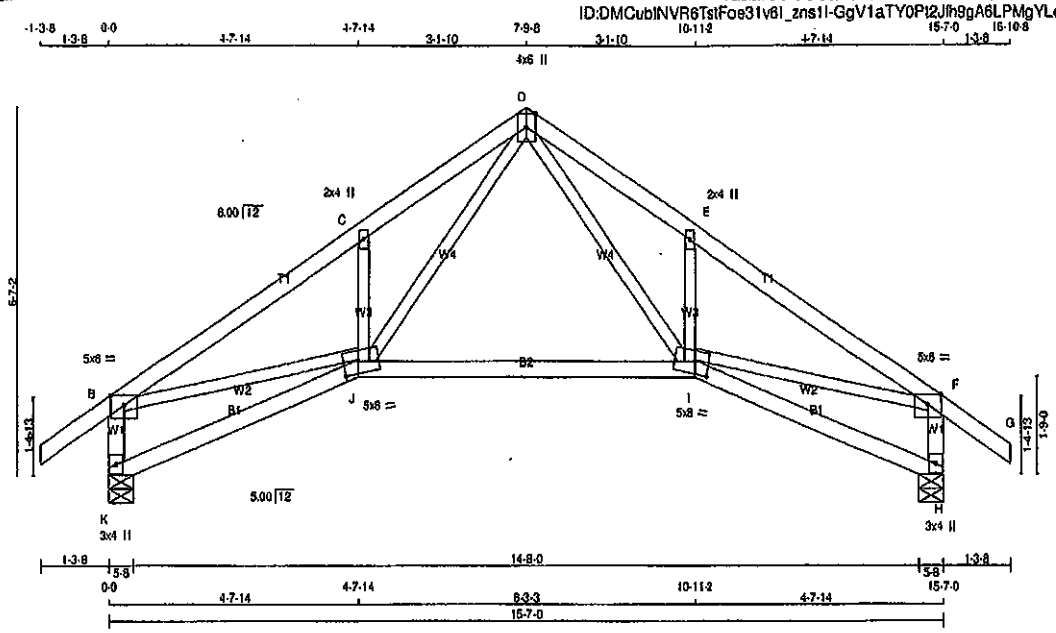

TOTAL WEIGHT = 2 X 66 = 132 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	8.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMW-p	MT20	5.0	8.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BBWWW-m	MT20	5.0	8.0	2.75	3.25
J	BBWWW-m	MT20	5.0	8.0	2.75	3.25
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	985	0	985	0	0	5-8	5-8
H	985	0	985	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	694	469 / 0	0 / 0	0 / 0	0 / 0	225 / 0	0 / 0
H	894	469 / 0	0 / 0	0 / 0	0 / 0	225 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-1445 / 0	-91.8	-91.8	0.27 (1)	5.16
C- D	-1497 / 0	-91.8	-91.8	0.19 (1)	5.18
D- E	-1497 / 0	-91.8	-91.8	0.19 (1)	5.18
E- F	-1445 / 0	-91.8	-91.8	0.27 (1)	5.16
F- G	0 / 35	-91.8	-91.8	0.12 (1)	10.00
K- B	-942 / 0	0.0	0.0	0.10 (1)	7.81
H- F	-942 / 0	0.0	0.0	0.10 (1)	7.81
K- J	0 / 0	-18.5	-18.5	0.12 (4)	10.00
J- I	0 / 721	-18.5	-18.5	0.27 (4)	10.00
I- H	0 / 0	-18.5	-18.5	0.12 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 880	0.20 (1)
I- E	-445 / 0	0.08 (1)
J- D	0 / 880	0.20 (1)
J- C	-445 / 0	0.08 (1)
B- J	0 / 1231	0.28 (1)
I- F	0 / 1231	0.28 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN.C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.27/1.00 (B-C:1) , BC=0.27/1.00 (I-J:4) , WB=0.28/1.00 (F-I:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1997	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (F) (INPUT = 0.90)
JSI METAL= 0.30 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J/G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008024

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408316	T20S1	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:22 2020 Page 1

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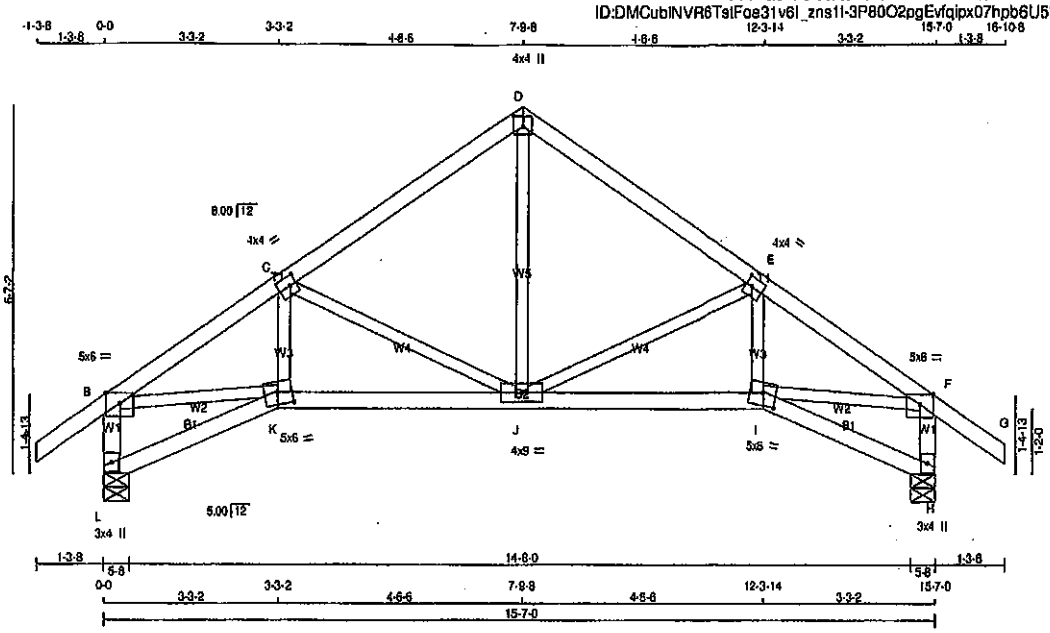

TOTAL WEIGHT = 2 X 66 = 132 lb [M](F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMWW-p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BBWW-m	MT20	5.0	6.0	2.75	3.00
J	BMWWW-t	MT20	4.0	9.0		
K	BBWW-m'	MT20	5.0	6.0	2.75	3.00
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	985	0	985	0	0	5-8	5-8
H	985	0	985	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	694	469 / 0	0 / 0	0 / 0	0 / 0	225 / 0	0 / 0
H	694	469 / 0	0 / 0	0 / 0	0 / 0	225 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B-C	-1334 / 0	-91.8	-91.8	0.19 (1)	5.41
C-D	-818 / 0	-91.8	-91.8	0.21 (1)	6.25
D-E	-818 / 0	-91.8	-91.8	0.21 (1)	6.25
E-F	-1334 / 0	-91.8	-91.8	0.19 (1)	5.41
F-G	0 / 35	-91.8	-91.8	0.12 (1)	10.00
L-B	-955 / 0	0.0	0.0	0.10 (1)	7.81
H-F	-955 / 0	0.0	0.0	0.10 (1)	7.81
L-K	0 / 0	-18.5	-18.5	0.06 (4)	10.00
K-J	0 / 1127	-18.5	-18.5	0.24 (1)	10.00
J-I	0 / 1127	-18.5	-18.5	0.24 (1)	10.00
I-H	0 / 0	-18.5	-18.5	0.06 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-D	0 / 551	0.12 (1)
J-E	-516 / 0	0.21 (1)
I-E	0 / 96	0.03 (4)
G-J	-516 / 0	0.21 (1)
K-C	0 / 96	0.03 (4)
B-K	0 / 1132	0.25 (1)
I-F	0 / 1132	0.25 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.21/1.00 (D-E:1) , BC=0.24/1.00 (J-K:1) , WB=0.25/1.00 (B-K:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (D) (INPUT = 0.90)
JSI METAL= 0.27 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008028

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	T21	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:59 2020 Page 1
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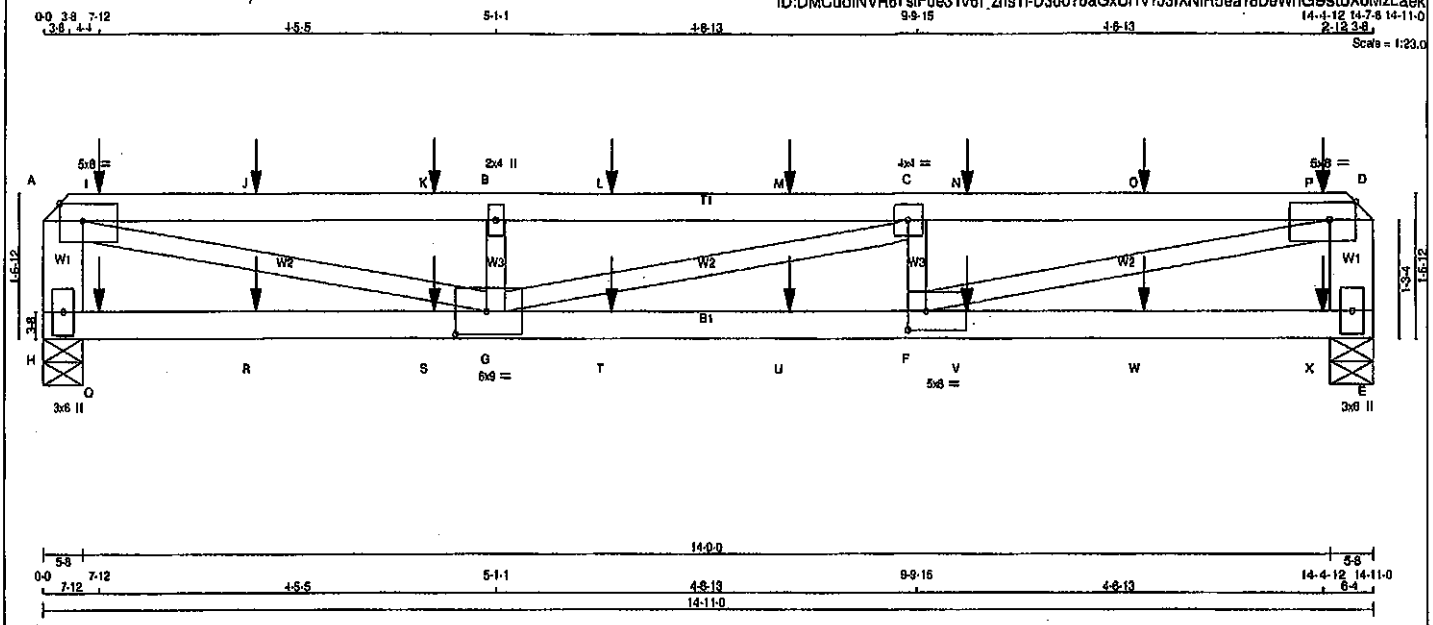

TOTAL WEIGHT = 50 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
H - A	2x6	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	5.0	8.0	Edge	3.25
B	TMW+w	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TMWW-t	MT20	5.0	8.0	Edge	3.25
E	BMV1+p	MT20	3.0	6.0		
F	BMWW-t	MT20	5.0	8.0	2.50	2.50
G	BMWWW-t	MT20	6.0	9.0	3.00	4.25
H	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1220	0	1220	0	0	5-8	5-8
E	1224	0	1224	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	863	566 / 0	0 / 0	0 / 0	0 / 0	297 / 0	0 / 0
E	865	568 / 0	0 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-I	-3296 / 0	-91.8	-91.8	0.73 (1)	3.06
I-J	-3296 / 0	-91.8	-91.8	0.73 (1)	3.06
J-K	-3296 / 0	-91.8	-91.8	0.73 (1)	3.06
K-B	-3296 / 0	-91.8	-91.8	0.73 (1)	3.06
B-L	-3296 / 0	-91.8	-91.8	0.55 (1)	3.31
L-M	-3296 / 0	-91.8	-91.8	0.55 (1)	3.31
M-C	-3296 / 0	-91.8	-91.8	0.55 (1)	3.31
C-N	-3289 / 0	-91.8	-91.8	0.71 (1)	3.09
N-O	-3289 / 0	-91.8	-91.8	0.71 (1)	3.09
O-P	-3289 / 0	-91.8	-91.8	0.71 (1)	3.09
P-D	-3289 / 0	-91.8	-91.8	0.71 (1)	3.09
H-A	-1113 / 0	0.0	0.0	0.08 (1)	7.61
E-D	-1116 / 0	0.0	0.0	0.08 (1)	7.61
H-Q	0 / 0	-18.5	-18.5	0.19 (1)	10.00
Q-R	0 / 0	-18.5	-18.5	0.19 (1)	10.00
R-S	0 / 0	-18.5	-18.5	0.19 (1)	10.00
S-G	0 / 0	-18.5	-18.5	0.19 (1)	10.00
G-T	0 / 3289	-18.5	-18.5	0.71 (1)	10.00
T-U	0 / 3289	-18.5	-18.5	0.71 (1)	10.00
U-F	0 / 3289	-18.5	-18.5	0.71 (1)	10.00
F-V	0 / 0	-18.5	-18.5	0.19 (1)	10.00
V-W	0 / 0	-18.5	-18.5	0.19 (1)	10.00
W-X	0 / 0	-18.5	-18.5	0.19 (1)	10.00
X-E	0 / 0	-18.5	-18.5	0.19 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-D	0 / 3390	0.84 (1)
A-G	0 / 3397	0.84 (1)
F-C	-630 / 0	0.10 (1)
G-B	-632 / 0	0.10 (1)
G-C	0 / 7	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	7-12	-71	-71	---	FRONT	VERT	TOTAL	--	C1
J	2-4-12	-54	-54	---	FRONT	VERT	TOTAL	--	C1
K	4-4-12	-54	-54	---	FRONT	VERT	TOTAL	--	C1
L	6-4-12	-54	-54	---	FRONT	VERT	TOTAL	--	C1
M	8-4-12	-54	-54	---	FRONT	VERT	TOTAL	--	C1
N	10-4-12	-54	-54	---	FRONT	VERT	TOTAL	--	C1
O	12-4-12	-54	-54	---	FRONT	VERT	TOTAL	--	C1
P	14-4-12	-76	-76	---	FRONT	VERT	TOTAL	--	C1
Q	7-12	-43	-43	---	FRONT	VERT	TOTAL	--	C1
R	2-4-12	-40	-40	---	FRONT	VERT	TOTAL	--	C1
S	4-4-12	-40	-40	---	FRONT	VERT	TOTAL	--	C1
T	6-4-12	-40	-40	---	FRONT	VERT	TOTAL	--	C1
U	8-4-12	-40	-40	---	FRONT	VERT	TOTAL	--	C1
V	10-4-12	-40	-40	---	FRONT	VERT	TOTAL	--	C1
W	12-4-12	-40	-40	---	FRONT	VERT	TOTAL	--	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN.C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.0/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , AEC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/ 882 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/ 477 (0.38")

CSI: TC=0.73/1.00 (A-B:1) , BC=0.71/1.00 (F-G:1) , WB=0.84/1.00 (A-G:1) , SSI=0.31/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.82 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008025 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408315	T21	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:59 2020 Page 2
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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
X	14-4-12	-44	-44	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008025 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408316	T30	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Rool Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:23 2020 Page 1

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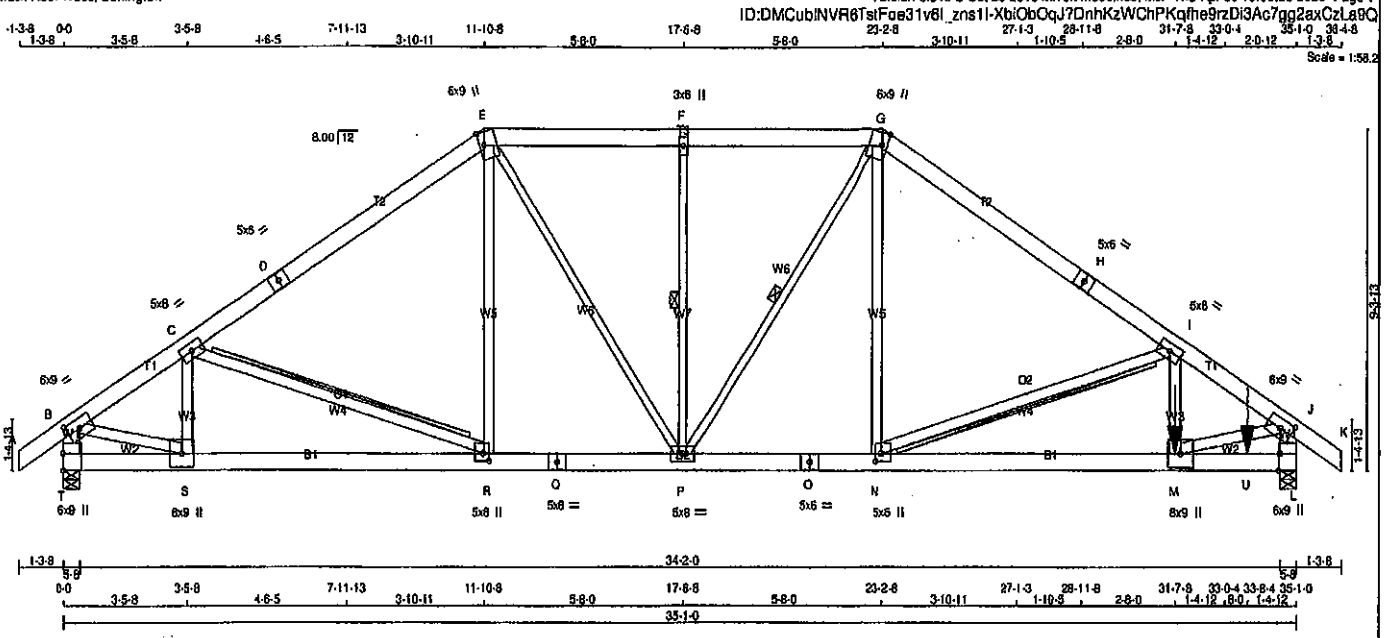

TOTAL WEIGHT = 2 X 219 = 437 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
P - G	2x3	DRY	No.2	SPF
E - P	2x3	DRY	No.2	SPF
P - F	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-D	2	12	TOP
D-E	2	12	TOP
E-G	2	12	TOP
G-H	2	12	TOP
H-K	2	12	TOP
T-B	2	12	TOP
L-J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T-Q	2	12	TOP
Q-O	2	12	TOP
O-L	2	12	SIDE(163.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
M-I	2	2	SIDE(1289.7)
2x4	1	6	
S-C	2	2	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008029 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2640	0	2640	0	0	5-8	5-8
L	7741	0	7741	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1864	1241 / 0	0 / 0	0 / 0	0 / 0	623 / 0	0 / 0
L	5484	3643 / 0	0 / 0	0 / 0	0 / 0	1822 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-P, F-P.
2x4 DRY SPF No.2 T-BRACE AT C-R
2x6 DRY SPF No.2 T-BRACE AT I-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 36	-91.8	-91.8	0.04 (1)	10.00	
B-C	-3133 / 0	-91.8	-91.8	0.22 (1)	5.96	
C-D	-3051 / 0	-91.8	-91.8	0.25 (1)	6.02	
D-E	-3051 / 0	-91.8	-91.8	0.25 (1)	6.02	
E-F	-3089 / 0	-91.8	-91.8	0.13 (1)	6.12	
F-G	-3089 / 0	-91.8	-91.8	0.13 (1)	6.12	
G-H	-3902 / 0	-91.8	-91.8	0.31 (1)	5.43	
H-I	-3902 / 0	-91.8	-91.8	0.31 (1)	5.43	
I-J	-9121 / 0	-91.8	-91.8	0.28 (1)	3.85	
J-K	0 / 36	-91.8	-91.8	0.04 (1)	10.00	
T-B	-2610 / 0	0.0	0.0	0.09 (1)	7.81	
L-J	-7254 / 0	0.0	0.0	0.28 (1)	5.55	
T-S	0 / 0	-18.5	-18.5	0.04 (4)	10.00	
S-R	0 / 2682	-18.5	-18.5	0.22 (1)	10.00	
R-Q	0 / 2509	-18.5	-18.5	0.21 (1)	10.00	
Q-P	0 / 2509	-18.5	-18.5	0.21 (1)	10.00	
P-O	0 / 3253	-18.5	-18.5	0.27 (1)	10.00	
O-N	0 / 3253	-18.5	-18.5	0.27 (1)	10.00	
N-M	0 / 7639	-18.5	-18.5	0.57 (1)	10.00	
M-U	0 / 0	-18.5	-18.5	0.24 (1)	10.00	
U-L	0 / 0	-18.5	-18.5	0.24 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-R	-188 / 0	0.07 (1)
R-E	0 / 236	0.03 (4)
N-G	0 / 1853	0.16 (1)
N-I	-4716 / 0	0.62 (1)
P-G	-302 / 0	0.12 (1)
E-P	0 / 1075	0.13 (1)
P-F	-605 / 0	0.16 (1)
S-C	-596 / 0	0.05 (1)
B-S	0 / 2770	0.24 (1)
M-I	0 / 3891	0.34 (1)
M-J	0 / 7892	0.70 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	31-7-8	-5569	-5569	---	BACK	VERT	TOTAL	--	C1
U	33-8-4	-684	-684	---	BACK	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.31/1.00 (G-I:1) , BC=0.57/1.00 (M-N:1) , WB=0.70/1.00 (J-M:1) , SSI=0.18/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (R) (INPUT = 0.90)
JSI METAL= 0.66 (M) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408316	T30	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	6.0	9.0	3.00	4.25
C	TMWW-t	MT20	5.0	8.0		
D	TS-t	MT20	5.0	6.0		
E	TTWW+m	MT20	6.0	9.0	4.25	1.50
F	TMW+w	MT20	3.0	6.0		
G	TTWW+m	MT20	6.0	9.0	4.25	1.50
H	TS-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	8.0		
J	TMWW-t	MT20	6.0	9.0	3.00	4.25
L	BMV1+t	MT20	6.0	9.0	Edge	0.50
M	BMWW+t	MT20	8.0	9.0		
N	BMWW+t	MT20	5.0	6.0	2.50	2.00
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW+t	MT20	5.0	6.0	2.50	2.00
S	BMWW+t	MT20	8.0	9.0		
T	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008029 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408316	T31	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:24 2020 Page 1

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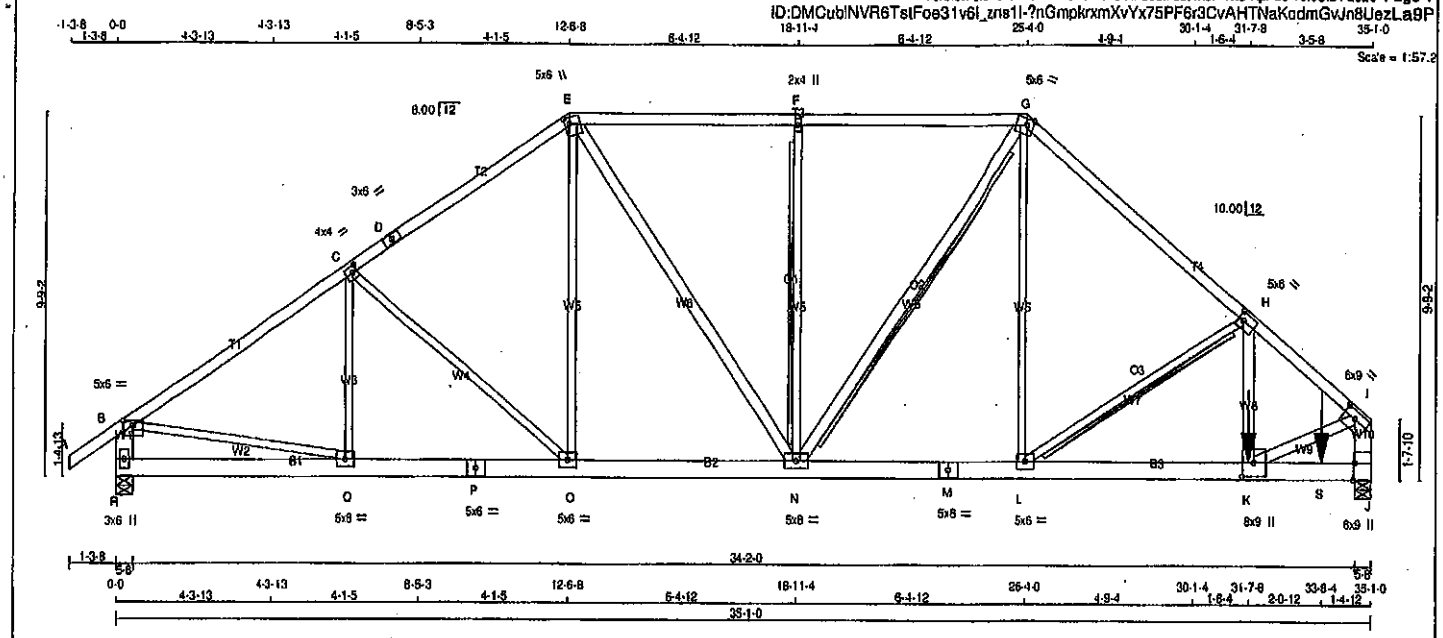

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
N - G	2x4	DRY	No.2	SPF
E - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-D	1	12	TOP
D-E	1	12	TOP
E-G	1	12	TOP
G-I	1	12	TOP
R-B	2	12	TOP
J-I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R-P	2	12	TOP
P-M	2	12	TOP
M-J	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
H-K	2	3	SIDE(1410.3)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG#, T-2008030 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2689	0	2689	0	0	5-8	5-8
J	8097	0	8097	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1699	1264 / 0	0 / 0	0 / 0	0 / 0	635 / 0	0 / 0
J	5718	3798 / 0	0 / 0	0 / 0	0 / 0	1920 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT H-L, G-N
2x4 DRY SPF No.2 T-BRACE AT F-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	
B- C	-3291 / 0	-91.8	-91.8	0.49 (1)	4.68	
C- D	-3058 / 0	-91.8	-91.8	0.45 (1)	4.82	
D- E	-3058 / 0	-91.8	-91.8	0.45 (1)	4.82	
E- F	-3118 / 0	-91.8	-91.8	0.44 (1)	4.76	
F- G	-3118 / 0	-91.8	-91.8	0.44 (1)	4.76	
G- H	-4126 / 0	-91.8	-91.8	0.49 (1)	4.36	
H- I	-8211 / 0	-91.8	-91.8	0.51 (1)	3.04	
R- B	-2628 / 0	0.0	0.0	0.09 (1)	7.81	
J- I	-7603 / 0	0.0	0.0	0.28 (1)	5.44	
R- Q	0 / 0	-18.5	-18.5	0.05 (4)	10.00	
Q- P	0 / 2769	-18.5	-18.5	0.20 (1)	10.00	
P- O	0 / 2769	-18.5	-18.5	0.20 (1)	10.00	
O- N	0 / 2512	-18.5	-18.5	0.19 (1)	10.00	
N- M	0 / 3167	-18.5	-18.5	0.24 (1)	10.00	
M- L	0 / 3167	-18.5	-18.5	0.24 (1)	10.00	
L- K	0 / 6348	-18.5	-18.5	0.48 (1)	10.00	
K- S	0 / 0	-18.5	-18.5	0.25 (1)	10.00	
S- J	0 / 0	-18.5	-18.5	0.25 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-321 / 0	0.08 (1)
C- O	-345 / 0	0.24 (1)
O- E	0 / 356	0.04 (1)
L- G	0 / 2223	0.28 (1)
L- H	-3808 / 0	0.70 (1)
B- Q	0 / 2801	0.35 (1)
K- H	0 / 4142	0.37 (1)
K- I	0 / 6730	0.59 (1)
N- G	-86 / 0	0.02 (1)
E- N	0 / 1075	0.10 (1)
N- F	-711 / 0	0.34 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
K	31-7-8	-6108	-6108	---	FRONT	VERT	TOTAL	---	CI
S	33-8-4	-684	-684	---	FRONT	VERT	TOTAL	---	CI

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.51/1.00 (H-I:1) , BC=0.46/1.00 (K-L:1) , WB=0.70/1.00 (H-L:1) , SSI=0.16/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	798	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.72 (K) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408316	T32	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:25 2020 Page 1

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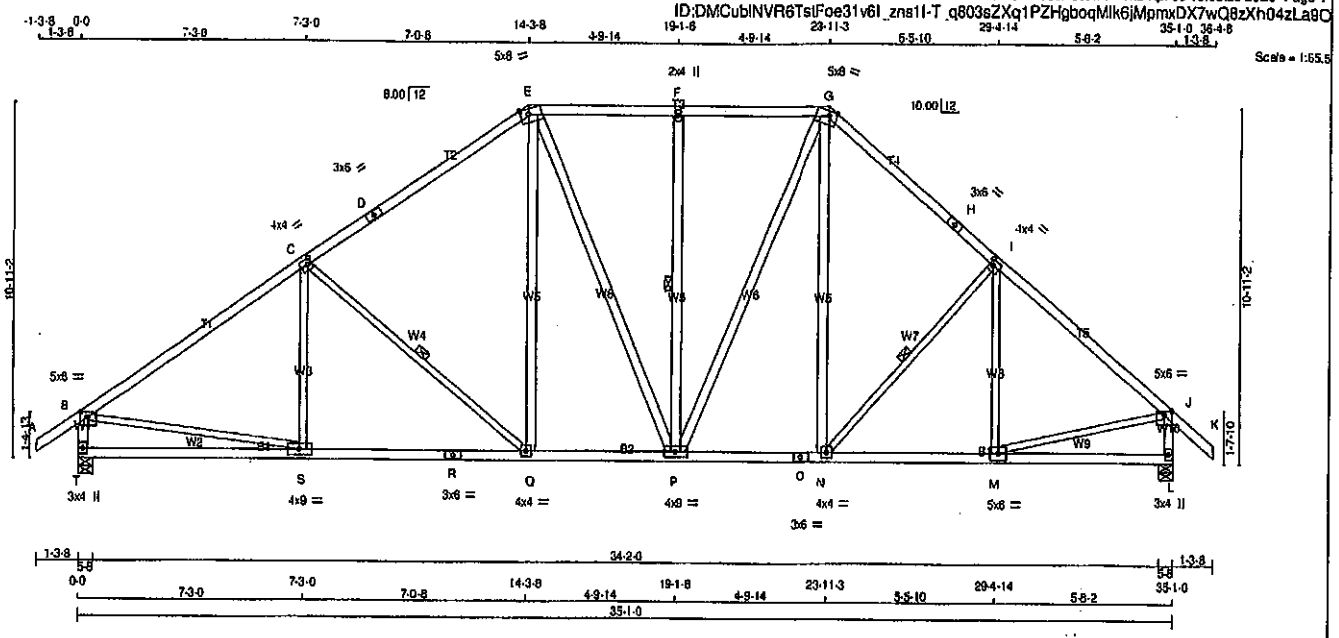


TOTAL WEIGHT = 188 lb (M)(F)

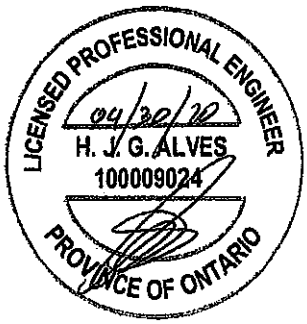
LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
S - C	2x3	DRY	No.2	SPF
C - Q	2x3	DRY	No.2	SPF
N - I	2x3	DRY	No.2	SPF
M - I	2x3	DRY	No.2	SPF
B - S	2x3	DRY	No.2	SPF
M - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	1.75	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	2.00	3.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	3.00
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-p	MT20	5.0	6.0	1.50	3.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	4.0	9.0		
T	BMV1+p	MT20	3.0	4.0		



Structural component only
DWG# T-2008031

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2060	0	2060	0	0	5-8	5-8
L	2061	0	2061	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1454	968 / 0	0 / 0	0 / 0	0 / 0	487 / 0	0 / 0
L	1455	968 / 0	0 / 0	0 / 0	0 / 0	487 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-Q, F-P, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
B-C	-2352 / 0	-91.8	-91.8	0.85 (1)	3.53	
C-D	-1899 / 0	-91.8	-91.8	0.75 (1)	3.96	
D-E	-1899 / 0	-91.8	-91.8	0.75 (1)	3.96	
E-F	-1808 / 0	-91.8	-91.8	0.80 (1)	4.80	
F-G	-1808 / 0	-91.8	-91.8	0.80 (1)	4.80	
G-H	-1848 / 0	-91.8	-91.8	0.45 (1)	4.49	
H-I	-1848 / 0	-91.8	-91.8	0.45 (1)	4.49	
I-J	-2032 / 0	-91.8	-91.8	0.48 (1)	4.29	
J-K	0 / 41	-91.8	-91.8	0.13 (1)	10.00	
T-B	-2005 / 0	0.0	0.0	0.21 (1)	5.98	
L-J	-2017 / 0	0.0	0.0	0.21 (1)	5.94	
T-S	0 / 0	-18.5	-18.5	0.24 (4)	10.00	
S-R	0 / 1993	-18.5	-18.5	0.44 (1)	10.00	
R-Q	0 / 1993	-18.5	-18.5	0.44 (1)	10.00	
Q-P	0 / 1548	-18.5	-18.5	0.32 (1)	10.00	
P-O	0 / 1390	-18.5	-18.5	0.28 (1)	10.00	
O-N	0 / 1390	-18.5	-18.5	0.28 (1)	10.00	
N-M	0 / 1591	-18.5	-18.5	0.32 (1)	10.00	
M-L	0 / 0	-18.5	-18.5	0.14 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S-C	-148 / 72	0.09 (1)
C-Q	-585 / 0	0.35 (1)
Q-E	0 / 484	0.08 (1)
E-P	0 / 145	0.02 (1)
P-F	-536 / 0	0.33 (1)
P-G	0 / 526	0.08 (1)
N-G	0 / 318	0.05 (1)
N-I	-302 / 0	0.14 (1)
M-I	-250 / 9	0.16 (1)
B-S	0 / 2015	0.45 (1)
M-J	0 / 1633	0.37 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.85/1.00 (B-C:1) , BC=0.44/1.00 (Q-S:1) , WB=0.45/1.00 (B-S:1) , SSI=0.27/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (J) (INPUT = 0.90)
JSI METAL= 0.67 (R) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408316	T33	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:25 2020 Page 1

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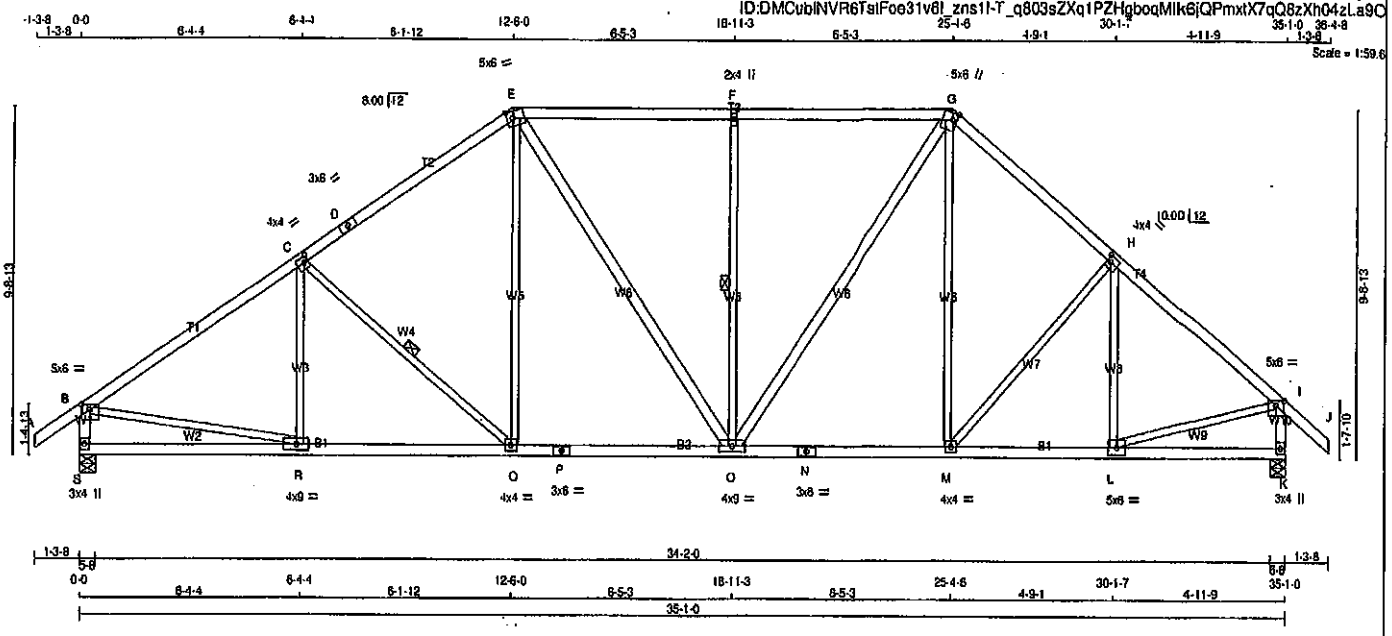

TOTAL WEIGHT = 170 lb

(M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - O	2x4	DRY	No.2	SPF
O - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	1.75	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.25	1.50
H	TMWW-t	MT20	4.0	4.0	2.00	1.25
I	TMWW-p	MT20	5.0	6.0	1.50	3.00
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMWW-t	MT20	4.0	9.0		
S	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2060	0	2060	0	0	5-8	5-8
K	2061	0	2061	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1454	968 / 0	0 / 0	0 / 0	0 / 0	487 / 0	0 / 0
K	1455	968 / 0	0 / 0	0 / 0	0 / 0	487 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-Q, F-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
B-C	-2349 / 0	-91.8	-91.8	0.62 (1)	3.66	
C-D	-2028 / 0	-91.8	-91.8	0.56 (1)	4.16	
D-E	-2028 / 0	-91.8	-91.8	0.56 (1)	4.16	
E-F	-1840 / 0	-91.8	-91.8	0.55 (1)	4.30	
F-G	-1840 / 0	-91.8	-91.8	0.55 (1)	4.30	
G-H	-1921 / 0	-91.8	-91.8	0.34 (1)	4.55	
H-I	-2008 / 0	-91.8	-91.8	0.35 (1)	4.46	
I-J	0 / 41	-91.8	-91.8	0.13 (1)	10.00	
S-B	-2011 / 0	0.0	0.0	0.21 (1)	5.95	
K-I	-2021 / 0	0.0	0.0	0.21 (1)	5.94	
S-R	0 / 0	-18.5	-18.5	0.17 (4)	10.00	
R-Q	0 / 1986	-18.5	-18.5	0.40 (1)	10.00	
Q-P	0 / 1657	-18.5	-18.5	0.35 (1)	10.00	
P-O	0 / 1657	-18.5	-18.5	0.35 (1)	10.00	
O-N	0 / 1448	-18.5	-18.5	0.32 (1)	10.00	
N-M	0 / 1448	-18.5	-18.5	0.32 (1)	10.00	
M-L	0 / 1568	-18.5	-18.5	0.32 (1)	10.00	
L-K	0 / 0	-18.5	-18.5	0.09 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-C	-211 / 32	0.10 (1)
C-Q	-437 / 0	0.20 (1)
Q-E	0 / 398	0.09 (1)
E-O	0 / 325	0.05 (1)
O-F	-723 / 0	0.48 (1)
O-G	0 / 696	0.11 (1)
M-G	0 / 243	0.05 (1)
M-H	-184 / 0	0.19 (1)
L-H	-319 / 0	0.18 (1)
B-R	0 / 2014	0.45 (1)
L-I	0 / 1621	0.36 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 989 (0.17")

CSI: TC=0.62/1.00 (B-C:1) , BC=0.40/1.00 (Q-R:1) , WB=0.46/1.00 (F-O:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.48 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008032

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408316	T34	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 11:34:36 2020 Page 1

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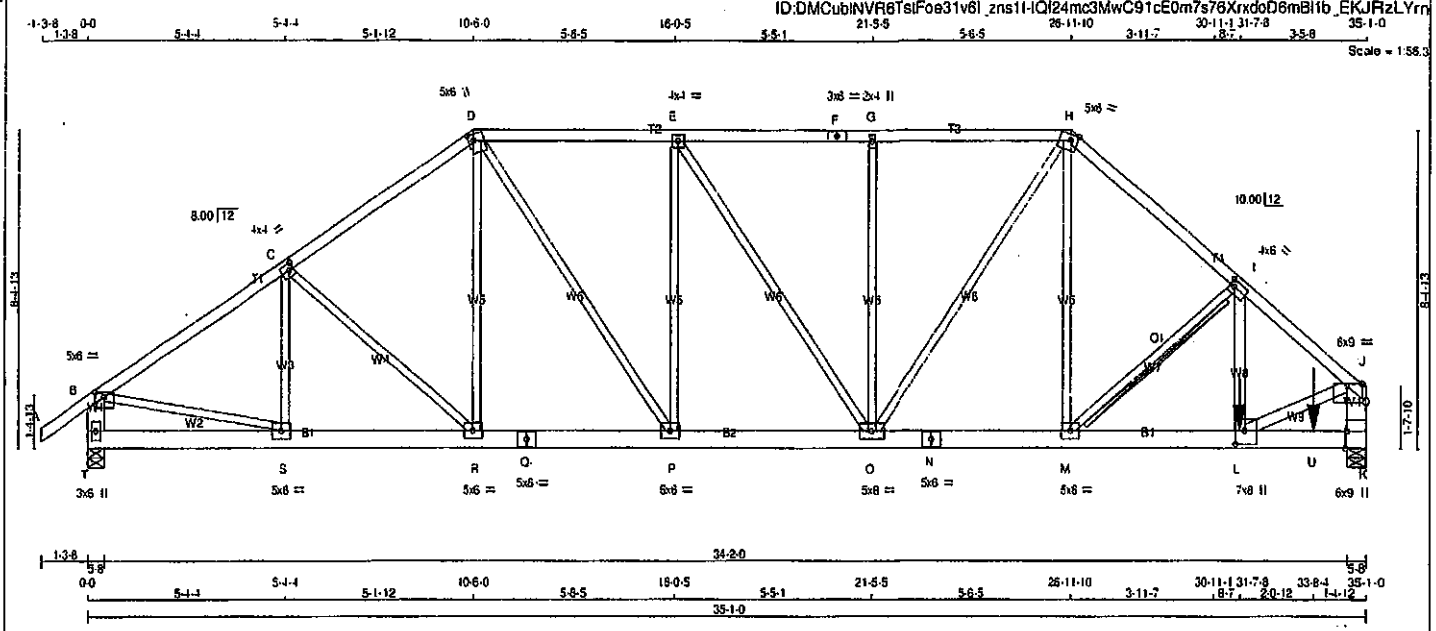

TOTAL WEIGHT = 2 X 187 = 374 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
K - J	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
L - I	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
F- H	1	12	TOP
H- J	1	12	TOP
T- B	2	12	TOP
K- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T- Q	2	12	TOP
Q- N	2	12	TOP
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
I- L	2	3	SIDE(1194.0)
2x4	1	8	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008033 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2603	0	2603	0	0	5-8	5-8
K	7230	0	7230	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1837	1223 0	0 0	0 0	0 0	614 0	0 0
K	5108	3392 0	0 0	0 0	0 0	1714 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT I-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)		FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0	35	-91.8	-91.8	0.07 (1)		10.00
B- C	-3111	0	-91.8	-91.8	0.30 (1)		4.96
C- D	-3032	0	-91.8	-91.8	0.30 (1)		5.01
D- E	-3217	0	-91.8	-91.8	0.27 (1)		4.92
E- F	-3492	0	-91.8	-91.8	0.28 (1)		4.75
F- G	-3492	0	-91.8	-91.8	0.28 (1)		4.75
G- H	-3492	0	-91.8	-91.8	0.27 (1)		4.76
H- I	-4347	0	-91.8	-91.8	0.26 (1)		4.40
I- J	-7316	0	-91.8	-91.8	0.33 (1)		3.42
T- B	-2548	0	0.0	0.0	0.09 (1)		7.81
K- J	-6805	0	0.0	0.0	0.25 (1)		5.71
T- S	0	0	-18.5	-18.5	0.03 (4)		10.00
S- R	0	2614	-18.5	-18.5	0.19 (1)		10.00
R- Q	0	2496	-18.5	-18.5	0.19 (1)		10.00
Q- P	0	2496	-18.5	-18.5	0.19 (1)		10.00
P- O	0	3217	-18.5	-18.5	0.23 (1)		10.00
O- N	0	3345	-18.5	-18.5	0.24 (1)		10.00
N- M	0	3345	-18.5	-18.5	0.24 (1)		10.00
M- L	0	5640	-18.5	-18.5	0.44 (1)		10.00
L- U	0	0	-18.5	-18.5	0.21 (1)		10.00
U- K	0	0	-18.5	-18.5	0.21 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)		MAX CSI (LC)
S- C	-416	0	0.07 (1)
C- R	-160	0	0.07 (1)
R- D	0	231	0.03 (1)
D- P	0	1270	0.16 (1)
P- E	-951	0	0.64 (1)
E- O	0	492	0.06 (1)
O- G	-534	0	0.36 (1)
O- H	0	259	0.03 (1)
M- H	0	2163	0.27 (1)
M- I	-3065	0	0.58 (1)
L- I	0	3477	0.31 (1)
B- S	0	2659	0.33 (1)
L- J	0	5980	0.53 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
L	31-7-8	-5279	-5279	---	BACK	VERT	TOTAL	---	C1
U	33-8-4	-560	-560	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.33/1.00 (I-J:1) , BC=0.44/1.00 (L-M:1) , WB=0.64/1.00 (E-P:1) , SSI=0.14/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.73 (L) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408316	T34	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMWW-l	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.00	2.25
I	TMWW-t	MT20	4.0	6.0	2.00	1.50
J	TMVW-p	MT20	6.0	9.0	Edge	
K	BMV1+t	MT20	6.0	9.0	Edge	0.50
L	BMWW+t	MT20	7.0	8.0	4.25	3.00
M, P, R, S						
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008033 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408316	T35	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:28 2020 Page 1

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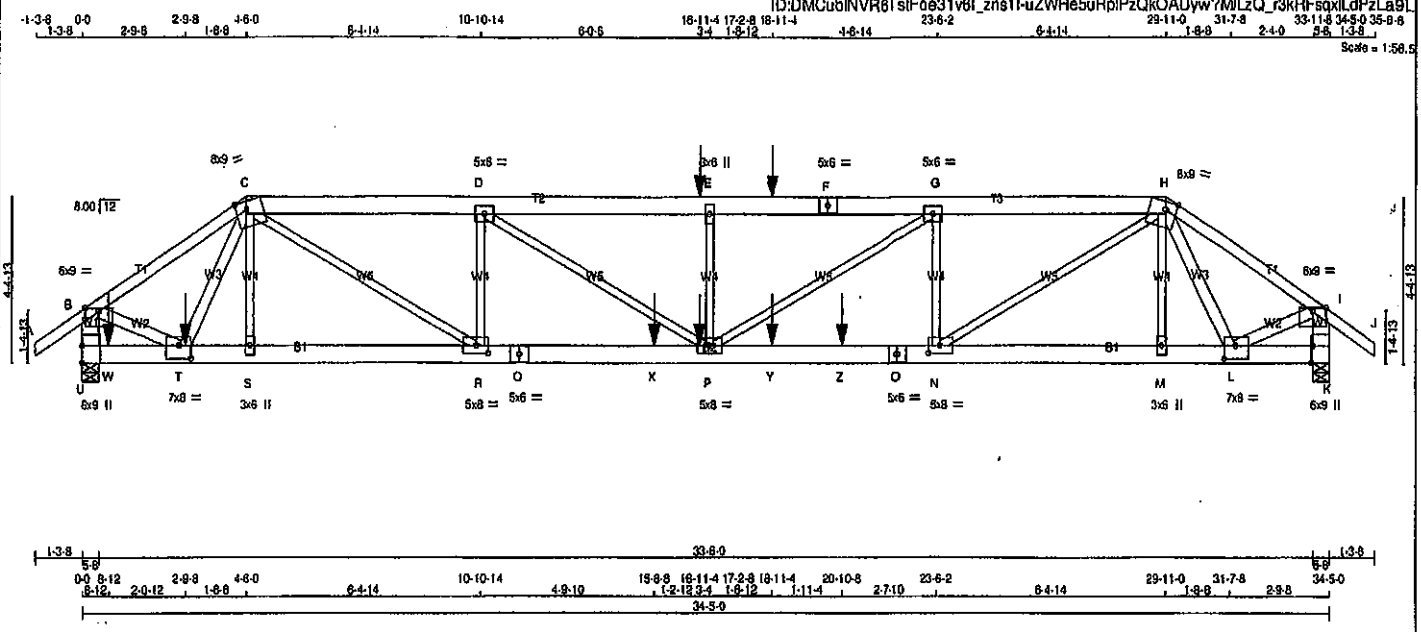

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
B - T	2x4	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
H-J	1	12	TOP
C-F	2	12	SIDE(183.1)
F-H	2	12	TOP
U-B	2	12	TOP
K-I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U-Q	2	12	SIDE(0.0)
Q-O	2	12	SIDE(183.1)
O-K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008034 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	7874	0	7874	0	0	5-8	5-8
K	3710	0	3710	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	5554	3726 / 0	0 / 0	0 / 0	0 / 0	1828 / 0	0 / 0
K	2615	1766 / 0	0 / 0	0 / 0	0 / 0	849 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	
B-C	-8031 / 0	-91.8	-91.8	0.51 (1)	3.14	
C-D	-9165 / 0	-91.8	-91.8	0.30 (1)	3.65	
D-E	-10772 / 0	-91.8	-91.8	0.32 (1)	3.56	
E-V	-10772 / 0	-91.8	-91.8	0.36 (1)	3.53	
V-F	-10772 / 0	-91.8	-91.8	0.36 (1)	3.53	
F-G	-10772 / 0	-91.8	-91.8	0.36 (1)	3.53	
G-H	-8408 / 0	-91.8	-91.8	0.27 (1)	4.02	
H-I	-3632 / 0	-91.8	-91.8	0.26 (1)	4.60	
I-J	0 / 35	-91.8	-91.8	0.07 (1)	10.00	
U-B	-7249 / 0	0.0	0.0	0.26 (1)	5.56	
K-1	-3633 / 0	0.0	0.0	0.13 (1)	7.37	
U-W	0 / 0	-18.5	-18.5	0.19 (1)	10.00	
W-T	0 / 0	-18.5	-18.5	0.19 (1)	10.00	
T-S	0 / 5967	-18.5	-18.5	0.58 (1)	10.00	
S-R	0 / 5978	-18.5	-18.5	0.45 (1)	10.00	
R-Q	0 / 9165	-18.5	-18.5	0.93 (1)	10.00	
Q-X	0 / 9165	-18.5	-18.5	0.93 (1)	10.00	
X-P	0 / 9165	-18.5	-18.5	0.93 (1)	10.00	
P-Y	0 / 8408	-18.5	-18.5	0.94 (1)	10.00	
Y-Z	0 / 8408	-18.5	-18.5	0.94 (1)	10.00	
Z-O	0 / 8408	-18.5	-18.5	0.94 (1)	10.00	
O-N	0 / 8408	-18.5	-18.5	0.94 (1)	10.00	
N-M	0 / 3795	-18.5	-18.5	0.36 (1)	10.00	
M-L	0 / 3792	-18.5	-18.5	0.30 (1)	10.00	
L-K	0 / 0	-18.5	-18.5	0.05 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S-C	0 / 428	0.05 (1)
M-H	0 / 134	0.02 (4)
L-I	0 / 3411	0.30 (1)
B-T	0 / 7148	0.63 (1)
T-C	0 / 1764	0.16 (1)
H-L	-1428 / 0	0.17 (1)
N-H	0 / 5426	0.67 (1)
C-R	0 / 3749	0.46 (1)
N-G	-2204 / 0	0.29 (1)
R-D	-1690 / 0	0.22 (1)
P-G	0 / 2789	0.35 (1)
D-P	0 / 1896	0.23 (1)
P-E	-727 / 0	0.10 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	16-11-4	-115	-115	---	FRONT	VERT	TOTAL	--	C1
P	16-11-4	-23	-23	---	FRONT	VERT	TOTAL	--	C1
T	2-9-8	-4435	-4435	---	BACK	VERT	TOTAL	--	C1
V	18-11-4	-115	-115	---	FRONT	VERT	TOTAL	--	C1
W	8-12	-563	-563	---	BACK	VERT	TOTAL	--	C1
X	15-8-8	-1259	-1259	---	FRONT	VERT	TOTAL	--	C1
Y	18-11-4	-23	-23	---	FRONT	VERT	TOTAL	--	C1
Z	20-10-8	-1006	-1006	---	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.15")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (1.15")
CALCULATED VERT. DEFL.(TL) = L/ 881 (0.47")

CSI: TC=0.51/1.00 (B-C:1) , BC=0.94/1.00 (N-P:1) , WB=0.67/1.00 (H-N:1) , SSI=0.38/1.00 (P-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.84 (O) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408316	T35	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWWW-m	MT20	8.0	9.0	2.50	3.50
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWWW-m	MT20	8.0	9.0	2.50	3.50
I	TMVW-p	MT20	6.0	9.0	Edge	
K	BMV1+t	MT20	6.0	9.0	Edge	0.50
L	BMWW-t	MT20	7.0	8.0	4.25	3.75
M	BMW+w	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	8.0	2.50	3.75
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	8.0	2.50	3.75
S	BMW+w	MT20	3.0	6.0		
T	BMWW-t	MT20	7.0	8.0	4.25	3.75
U	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008034 YL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408316	T36	6	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:28 2020 Page 1

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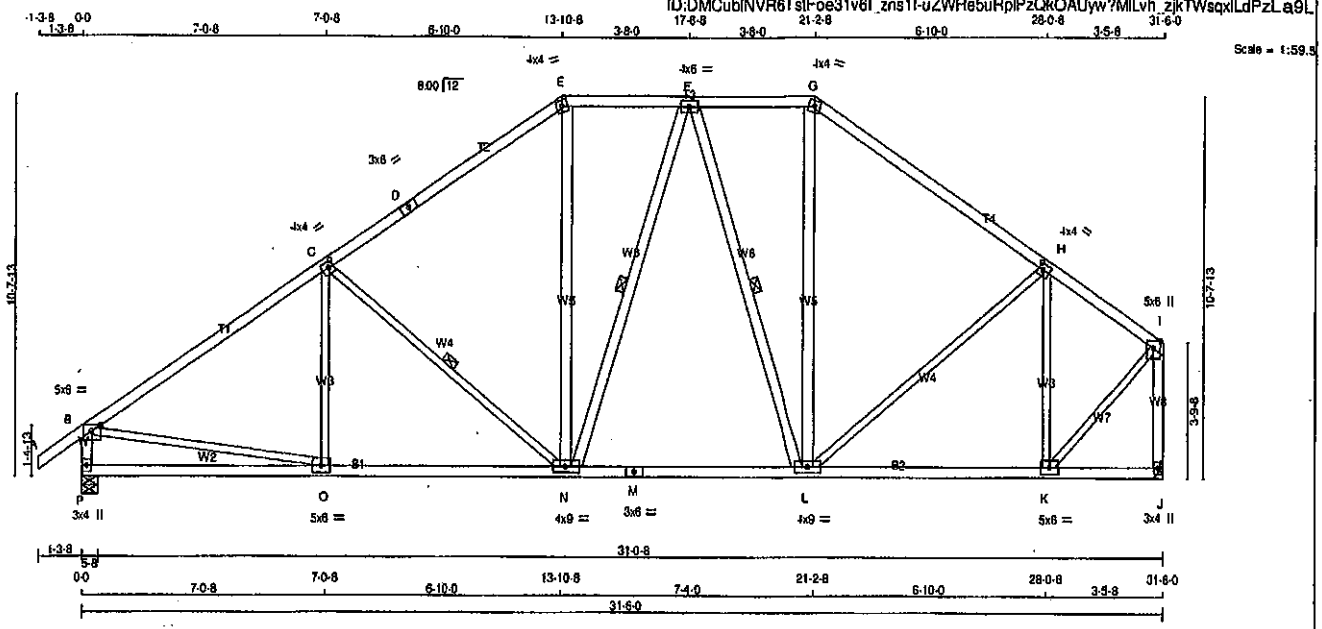

TOTAL WEIGHT = 6 X 184 = 984 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
N - E	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
F - L	2x4	DRY	No.2	SPF
L - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TTW-m	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/20/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008035

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
P	1862	0	1862	0	0	5-8	5-8
J	1736	0	1736	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1315	876 / 0	0 / 0	0 / 0	0 / 0	439 / 0	0 / 0
J	1228	806 / 0	0 / 0	0 / 0	0 / 0	422 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-N, F-N, F-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B-C	-2063 / 0	-91.8	-91.8	0.75 (1)	3.87
C-D	-1612 / 0	-91.8	-91.8	0.67 (1)	4.36
D-E	-1612 / 0	-91.8	-91.8	0.67 (1)	4.36
E-F	-1311 / 0	-91.8	-91.8	0.17 (1)	5.48
F-G	-1177 / 0	-91.8	-91.8	0.16 (1)	5.71
G-H	-1442 / 0	-91.8	-91.8	0.56 (1)	4.73
H-I	-1142 / 0	-91.8	-91.8	0.43 (1)	5.38
P-B	-1809 / 0	0.0	0.0	0.19 (1)	6.22
J-I	-1719 / 0	0.0	0.0	0.41 (1)	6.35
P-O	0 / 0	-18.5	-18.5	0.21 (4)	10.00
O-N	0 / 1752	-18.5	-18.5	0.39 (1)	10.00
N-M	0 1313	-18.5	-18.5	0.32 (1)	10.00
M-L	0 / 1313	-18.5	-18.5	0.32 (1)	10.00
L-K	0 / 1002	-18.5	-18.5	0.27 (4)	10.00
K-J	0 / 0	-18.5	-18.5	0.13 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O-C	-126 / 72	0.07 (1)
C-N	-561 / 0	0.33 (1)
N-E	0 / 503	0.08 (1)
N-F	-5 / 32	0.01 (4)
F-L	-420 / 0	0.27 (1)
L-G	0 / 393	0.06 (1)
L-H	0 / 220	0.05 (1)
K-H	-898 / 0	0.53 (1)
B-O	0 / 1772	0.40 (1)
K-I	0 / 1426	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2016 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.05")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.05")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.75/1.00 (B-C:1) , BC=0.39/1.00 (N-O:1) , WB=0.53/1.00 (H-K:1) , SSI=0.26/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.45 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408316	T37	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:29 2020 Page 1
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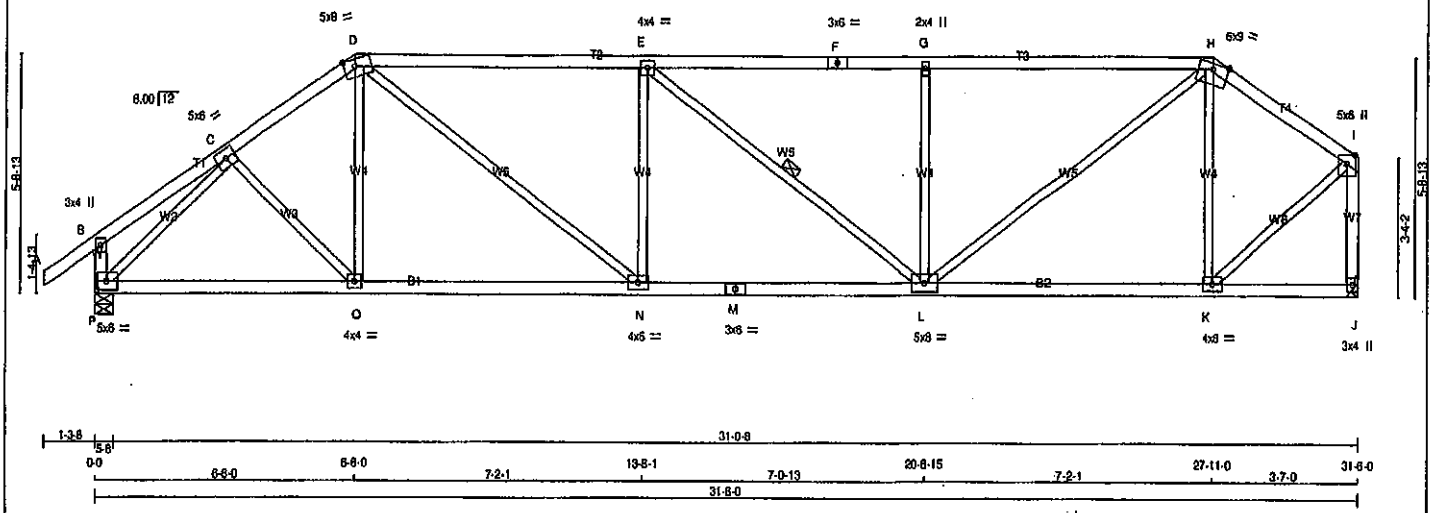

TOTAL WEIGHT = 129 lb. [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	8.0	2.00	3.25
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	6.0	9.0	Edge	
I	TMVW-p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMVW1-t	MT20	5.0	6.0	2.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008036

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1862	0	1862	0	0	5-8	5-8
J	1736	0	1736	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1315	876 / 0	0 / 0	0 / 0	0 / 0	439 / 0	0 / 0
J	1228	808 / 0	0 / 0	0 / 0	0 / 0	422 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
B-C	0 / 18	-91.8	-91.8	0.13 (1)	10.00	
C-D	-2023 / 0	-91.8	-91.8	0.18 (1)	4.60	
D-E	-2564 / 0	-91.8	-91.8	0.99 (1)	3.25	
E-F	-2355 / 0	-91.8	-91.8	0.93 (1)	3.41	
F-G	-2355 / 0	-91.8	-91.8	0.93 (1)	3.41	
G-H	-2355 / 0	-91.8	-91.8	0.93 (1)	3.42	
H-I	-1257 / 0	-91.8	-91.8	0.23 (1)	5.47	
P-B	-246 / 0	0.0	0.0	0.03 (1)	7.81	
J-I	-1720 / 0	0.0	0.0	0.33 (1)	6.35	
P-O	0 / 1568	-18.5	-18.5	0.37 (1)	10.00	
O-N	0 / 1687	-18.5	-18.5	0.38 (1)	10.00	
N-M	0 / 2564	-18.5	-18.5	0.50 (1)	10.00	
M-L	0 / 2564	-18.5	-18.5	0.50 (1)	10.00	
L-K	0 / 1029	-18.5	-18.5	0.27 (4)	10.00	
K-J	0 / 0	-18.5	-18.5	0.14 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-O	0 / 136	0.03 (4)
O-D	0 / 97	0.03 (4)
N-E	0 / 1124	0.25 (1)
E-L	-551 / 0	0.28 (1)
L-G	-263 / 0	0.14 (1)
G-H	-711 / 0	0.36 (1)
L-H	0 / 1662	0.37 (1)
K-H	-739 / 0	0.37 (1)
P-C	-2218 / 0	0.74 (1)
K-I	0 / 1350	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.05")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.05")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.99/1.00 (D-E:1) , BC=0.50/1.00 (L-N:1) , WB=0.74/1.00 (C-P:1) , SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.82 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408316	T38	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:30 2020 Page 1
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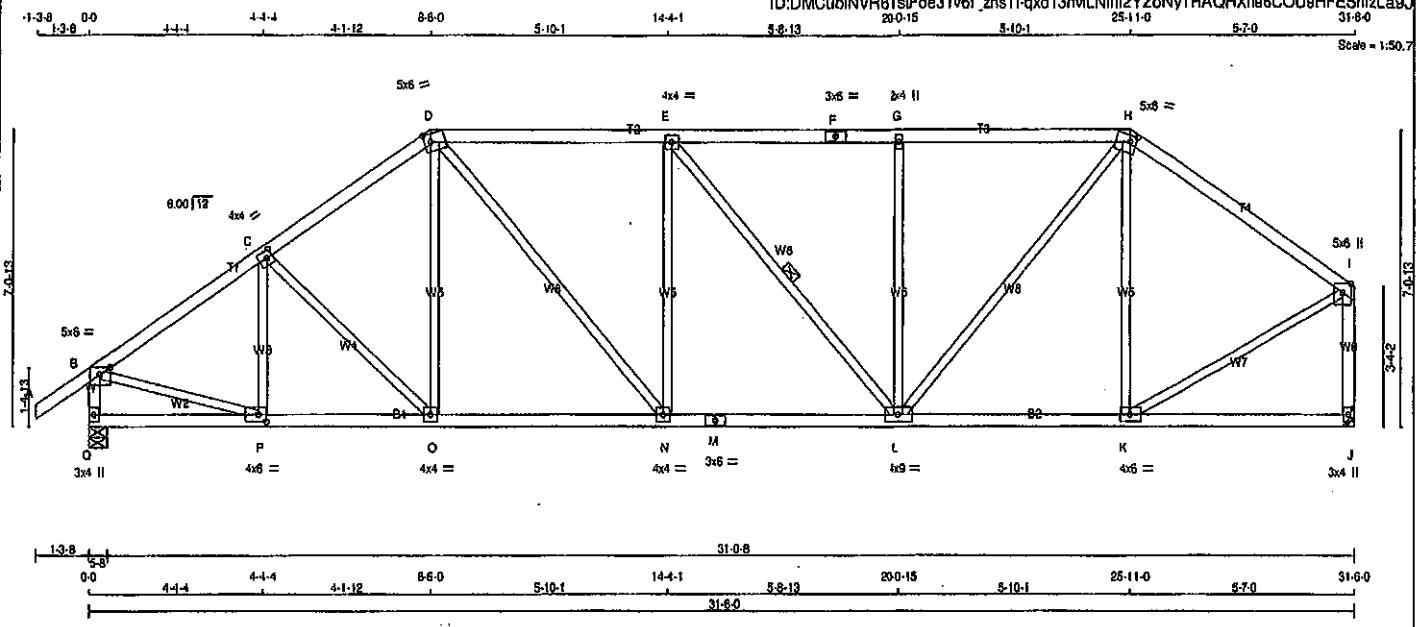

TOTAL WEIGHT = 139 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	1.75	1.75
I	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	6.0	2.00	2.25
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008037

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT.	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	1862	0	1862	0	0	5-8	5-8
J	1736	0	1736	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1315	876 / 0	0 / 0	0 / 0	0 / 0	439 / 0	0 / 0
J	1228	806 / 0	0 / 0	0 / 0	0 / 0	422 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FROM	FACTORED VERT. LOAD LC1 (PLF) TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B-C	-2003 / 0	-91.8	-91.8	0.35 (1)	4.44
C-D	-1949 / 0	-91.8	-91.8	0.34 (1)	4.49
D-E	-2053 / 0	-91.8	-91.8	0.54 (1)	4.14
E-F	-1916 / 0	-91.8	-91.8	0.53 (1)	4.27
F-G	-1915 / 0	-91.8	-91.8	0.53 (1)	4.27
G-H	-1914 / 0	-91.8	-91.8	0.52 (1)	4.28
H-I	-1442 / 0	-91.8	-91.8	0.60 (1)	4.67
Q-B	-1825 / 0	0.0	0.0	0.19 (1)	6.20
J-I	-1695 / 0	0.0	0.0	0.32 (1)	6.38
Q-P	0 / 0	-18.5	-18.5	0.07 (4)	10.00
P-O	0 / 1687	-18.5	-18.5	0.33 (1)	10.00
O-N	0 / 1601	-18.5	-18.5	0.32 (1)	10.00
N-M	0 / 2053	-18.5	-18.5	0.40 (1)	10.00
M-L	0 / 2053	-18.5	-18.5	0.40 (1)	10.00
L-K	0 / 1190	-18.5	-18.5	0.28 (1)	10.00
K-J	0 / 0	-18.5	-18.5	0.15 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-C	-342 / 0	0.09 (1)
C-O	-122 / 0	0.07 (1)
O-D	0 / 181	0.04 (4)
D-N	0 / 692	0.16 (1)
N-E	-414 / 0	0.36 (1)
E-L	-215 / 0	0.12 (1)
L-G	-576 / 0	0.50 (1)
L-H	0 / 1108	0.25 (1)
K-H	-515 / 0	0.44 (1)
B-P	0 / 1738	0.39 (1)
K-I	0 / 1357	0.31 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.05")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.05")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.60/1.00 (H-I:1) , BC=0.40/1.00 (L-N:1) , WB=0.50/1.00 (G-L:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.59 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408316	T39	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:31 2020 Page 1
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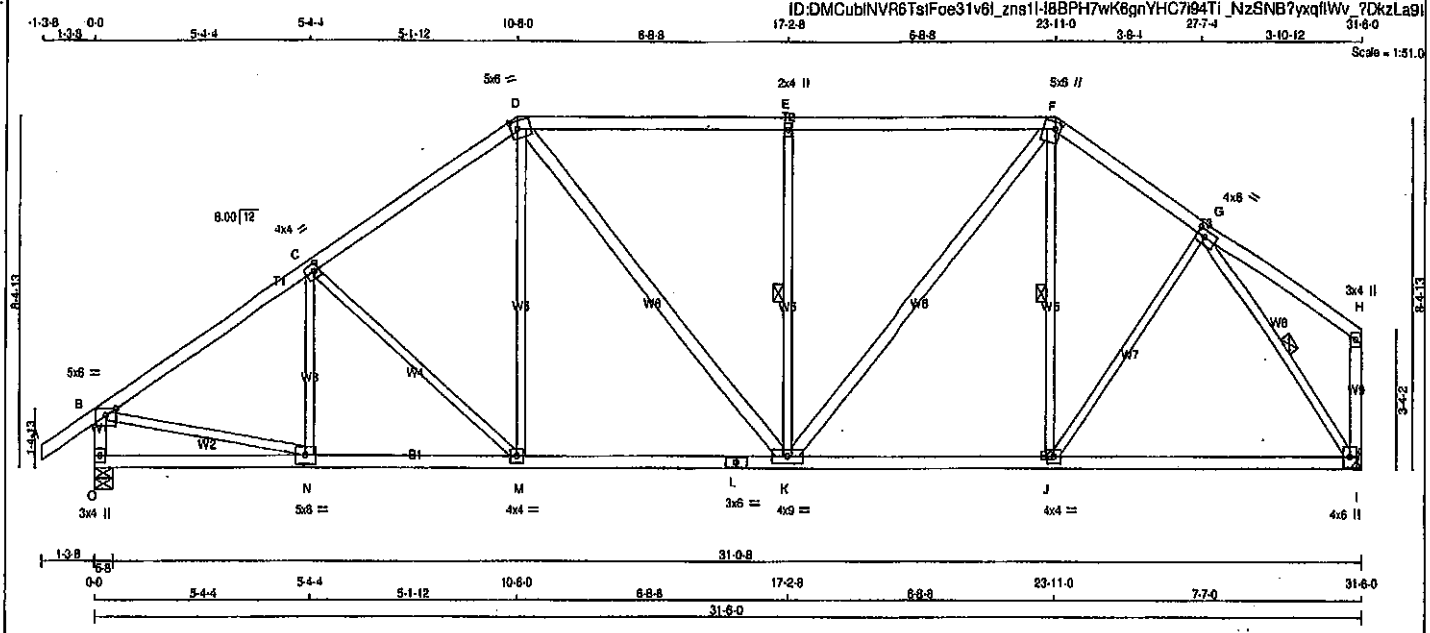

TOTAL WEIGHT = 148 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	1.50
G	TMWW-t	MT20	4.0	6.0	2.00	2.50
H	TMV+p	MT20	3.0	4.0		
I	BMVW1+p	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008038

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1862	0	1862	0	0	5-8	5-8
I	1736	0	1736	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1315	876 / 0	0 / 0	0 / 0	0 / 0	439 / 0	0 / 0
I	1228	808 / 0	0 / 0	0 / 0	0 / 0	422 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-K, F-J, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
B- C	-2050 / 0	-91.8	-91.8	0.40 (1)	4.36	
C- D	-1844 / 0	-91.8	-91.8	0.38 (1)	4.57	
D- E	-1740 / 0	-91.8	-91.8	0.59 (1)	4.32	
E- F	-1740 / 0	-91.8	-91.8	0.59 (1)	4.32	
F- G	-1527 / 0	-91.8	-91.8	0.19 (1)	5.15	
G- H	0 / 22	-91.8	-91.8	0.21 (1)	10.00	
O- B	-1819 / 0	0.0	0.0	0.19 (1)	6.20	
I- H	-138 / 0	0.0	0.0	0.03 (1)	7.81	
O- N	0 / 0	-18.5	-18.5	0.11 (4)	10.00	
N- M	0 / 1731	-18.5	-18.5	0.36 (1)	10.00	
M- L	0 / 1510	-18.5	-18.5	0.32 (1)	10.00	
L- K	0 / 1510	-18.5	-18.5	0.32 (1)	10.00	
K- J	0 / 1249	-18.5	-18.5	0.35 (4)	10.00	
J- I	0 / 1057	-18.5	-18.5	0.33 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-250 / 0	0.09 (1)
C- M	-302 / 0	0.28 (1)
M- D	0 / 315	0.07 (1)
D- K	0 / 362	0.06 (1)
K- E	-758 / 0	0.34 (1)
K- F	0 / 771	0.12 (1)
J- F	-125 / 39	0.06 (1)
J- G	0 / 338	0.06 (1)
B- N	0 / 1766	0.40 (1)
G- I	-1870 / 0	0.57 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.05")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.05")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.59/1.00 (D-E:1) , BC=0.38/1.00 (M-N:1) , WB=0.57/1.00 (G-I:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.48 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408316	T40	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:31 2020 Page 1
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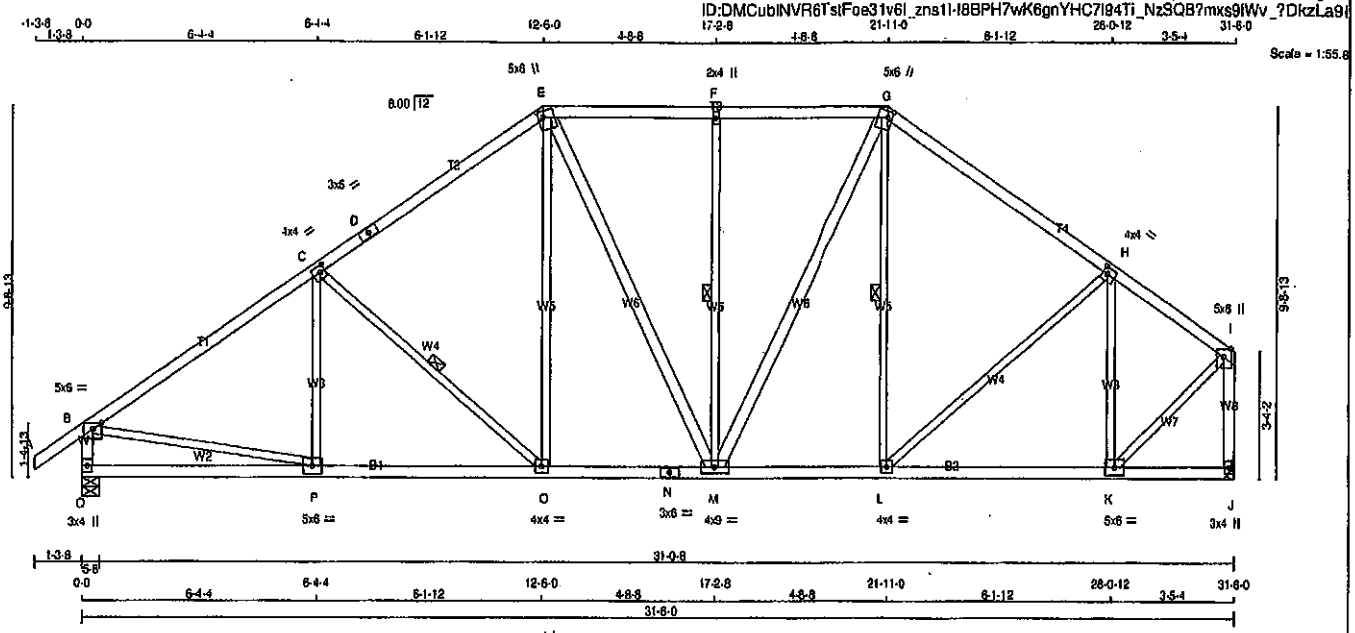

TOTAL WEIGHT = 158 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - M	2x4	DRY	No.2	SPF
M - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.00	1.50
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008039

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1862	0	1862	0	0	5-8	5-8
J	1736	0	1736	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1315	878 / 0	0 / 0	0 / 0	0 / 0	439 / 0	0 / 0
J	1228	806 / 0	0 / 0	0 / 0	0 / 0	422 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, F-M, G-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B-C	-2066 / 0	-91.8	-91.8	0.59 (1)	4.11
C-D	-1711 / 0	-91.8	-91.8	0.53 (1)	4.49
D-E	-1711 / 0	-91.8	-91.8	0.53 (1)	4.49
E-F	-1463 / 0	-91.8	-91.8	0.28 (1)	5.11
F-G	-1463 / 0	-91.8	-91.8	0.28 (1)	5.11
G-H	-1514 / 0	-91.8	-91.8	0.46 (1)	4.82
H-I	-1232 / 0	-91.8	-91.8	0.35 (1)	5.35
Q-B	-1814 / 0	0.0	0.0	0.19 (1)	6.21
J-I	-1717 / 0	0.0	0.0	0.33 (1)	6.35
Q-P	0 / 0	-18.5	-18.5	0.18 (4)	10.00
P-O	0 / 1751	-18.5	-18.5	0.37 (1)	10.00
O-N	0 / 1394	-18.5	-18.5	0.28 (1)	10.00
N-M	0 / 1394	-18.5	-18.5	0.28 (1)	10.00
M-L	0 / 1234	-18.5	-18.5	0.27 (1)	10.00
L-K	0 / 1068	-18.5	-18.5	0.24 (1)	10.00
K-J	0 / 0	-18.5	-18.5	0.11 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
P-C	-168 / 49	0.08 (1)
C-O	-474 / 0	0.22 (1)
O-E	0 / 410	0.09 (1)
E-M	0 / 155	0.02 (1)
M-F	-525 / 0	0.33 (1)
M-G	0 / 513	0.08 (1)
L-G	-33 / 66	0.02 (4)
L-H	0 / 219	0.05 (1)
K-H	-839 / 0	0.41 (1)
B-P	0 / 1776	0.40 (1)
K-I	0 / 1422	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.05")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.05")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.59/1.00 (B-C:1) , BC=0.37/1.00 (O-P:1) , WB=0.41/1.00 (H-K:1) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.49 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408316	T41	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:32 2020 Page 1
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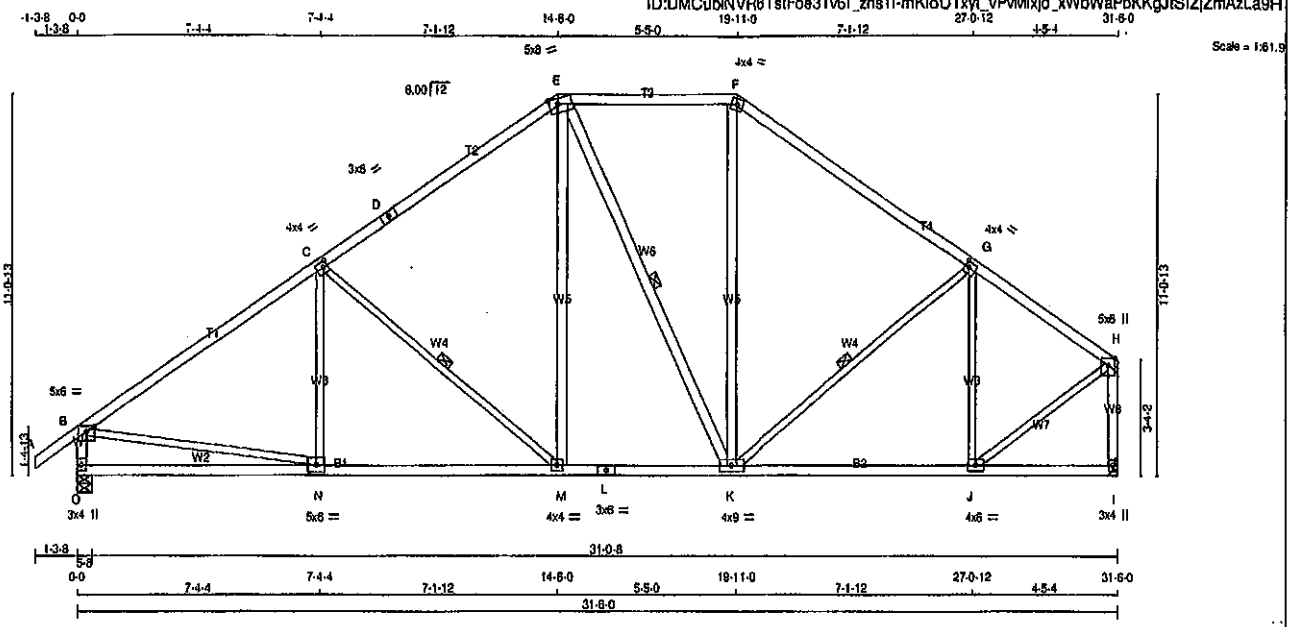

TOTAL WEIGHT = 155 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
M - E	2x4	DRY	No.2	SPF
E - K	2x4	DRY	No.2	SPF
K - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	2.00	3.00
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

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100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008040

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1882	0	1882	0	0	5-8	5-8
I	1736	0	1736	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1315	876 / 0	0 / 0	0 / 0	0 / 0	439 / 0	0 / 0
I	1228	806 / 0	0 / 0	0 / 0	0 / 0	422 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, E-K, G-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B-C	-2050 / 0	-91.8	-91.8	0.83 (1)	3.72
C-D	-1563 / 0	-91.8	-91.8	0.73 (1)	4.28
D-E	-1563 / 0	-91.8	-91.8	0.73 (1)	4.28
E-F	-1191 / 0	-91.8	-91.8	0.37 (1)	5.41
F-G	-1462 / 0	-91.8	-91.8	0.64 (1)	4.60
G-H	-1376 / 0	-91.8	-91.8	0.49 (1)	4.91
O-B	-1607 / 0	0.0	0.0	0.19 (1)	6.22
I-H	-1710 / 0	0.0	0.0	0.32 (1)	6.36
O-N	0 / 0	-18.5	-18.6	0.25 (4)	10.00
N-M	0 / 1751	-18.5	-18.5	0.41 (1)	10.00
M-L	0 / 1268	-18.5	-18.5	0.29 (1)	10.00
L-K	0 / 1268	-18.5	-18.5	0.29 (1)	10.00
K-J	0 / 1191	-18.5	-18.5	0.28 (1)	10.00
J-I	0 / 0	-18.5	-18.5	0.16 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	-102 / 89	0.07 (1)
C-M	-634 / 0	0.40 (1)
M-E	0 / 508	0.08 (1)
E-K	-176 / 0	0.14 (1)
K-F	0 / 279	0.04 (1)
K-G	-5 / 7	0.00 (1)
J-G	-689 / 0	0.44 (1)
B-N	0 / 1770	0.40 (1)
J-H	0 / 1441	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.05")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.05")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.83/1.00 (B-C:1) , BC=0.41/1.00 (M-N:1) , WB=0.44/1.00 (G-J:1) , SSI=0.27/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.55 (H) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408316	T42	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:33 2020 Page 1
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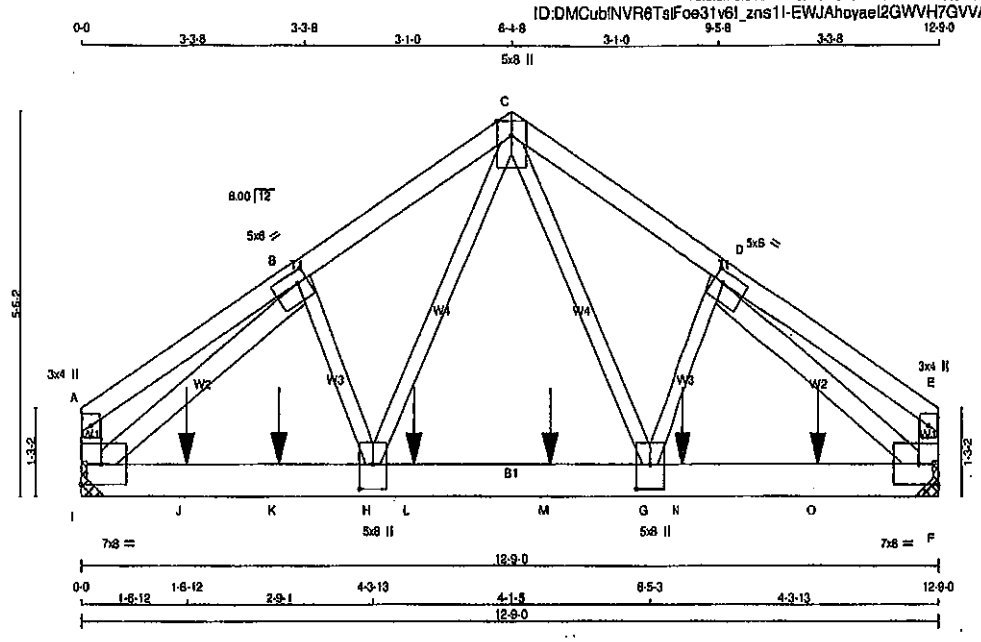

TOTAL WEIGHT = 2 X 64 = 128 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x6	DRY	1650F 1.5E	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - B	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-E	1	12	TOP
I-A	1	12	TOP
F-E	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
I-F	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

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100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008041 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	6126	0	6126	0	0	MECHANICAL	
F	5588	0	5588	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I, F. MINIMUM BEARING LENGTH AT JOINT I = 4-0, JOINT F = 4-0.

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	4327	2870 / 0	0 / 0	0 / 0	0 / 0	1457 / 0	0 / 0
F	3947	2618 / 0	0 / 0	0 / 0	0 / 0	1329 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-3 / 1	-91.8	-91.8	0.08 (1)	10.00
B-C	-6393 / 0	-91.8	-91.8	0.24 (1)	3.70
C-D	-6154 / 0	-91.8	-91.8	0.22 (1)	3.78
D-E	-2 / 3	-91.8	-91.8	0.08 (1)	10.00
I-A	-142 / 0	0.0	0.0	0.01 (1)	7.81
F-E	-139 / 0	0.0	0.0	0.01 (1)	7.81
I-J	0 / 4984	-18.5	-18.5	0.71 (1)	10.00
J-K	0 / 4984	-18.5	-18.5	0.71 (1)	10.00
K-H	0 / 4984	-18.5	-18.5	0.71 (1)	10.00
H-L	0 / 3876	-18.5	-18.5	0.58 (1)	10.00
L-M	0 / 3876	-18.5	-18.5	0.58 (1)	10.00
M-G	0 / 3876	-18.5	-18.5	0.58 (1)	10.00
G-N	0 / 4802	-18.5	-18.5	0.62 (1)	10.00
N-O	0 / 4802	-18.5	-18.5	0.62 (1)	10.00
O-F	0 / 4802	-18.5	-18.5	0.62 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-G	0 / 3327	0.41 (1)
G-D	0 / 857	0.11 (1)
H-C	0 / 3882	0.49 (1)
B-H	0 / 903	0.11 (1)
I-B	-6839 / 0	0.76 (1)
D-F	-6588 / 0	0.73 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
J	1-6-12	-1718	-1718	---	FRONT	VERT	TOTAL	--	C1
K	2-11-4	-1718	-1718	---	FRONT	VERT	TOTAL	--	C1
L	4-11-4	-1718	-1718	---	FRONT	VERT	TOTAL	--	C1
M	6-11-4	-1718	-1718	---	FRONT	VERT	TOTAL	--	C1
N	8-11-4	-1718	-1718	---	FRONT	VERT	TOTAL	--	C1
O	10-11-4	-1718	-1718	---	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.71/1.00 (H-I:1) , WB=0.76/1.00 (B-I:1) , SSI=0.88/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.82 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408316	T42	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0	2.00	1.75
C	TTWW+p	MT20	5.0	8.0	Edge	
D	TMWW-t	MT20	5.0	6.0	2.00	1.75
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	7.0	8.0		Edge
G	BMWW+t	MT20	5.0	8.0	4.25	2.50
H	BMWW+t	MT20	5.0	8.0	4.25	2.50
I	BMVW1-t	MT20	7.0	8.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
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100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008041 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408316	T43S	1	1	TRUSS DESC.	

Tamarack Rool Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:34 2020 Page 1

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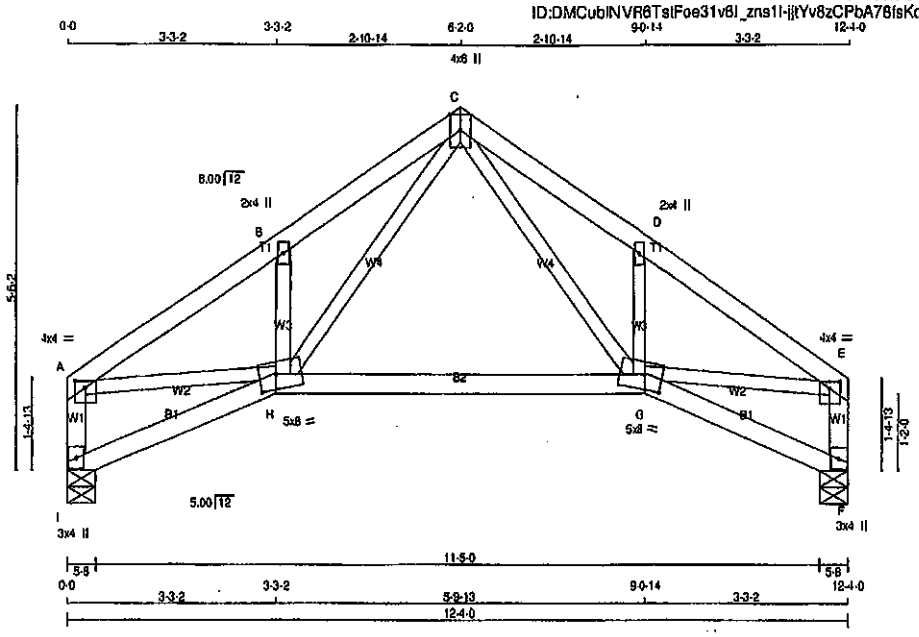

TOTAL WEIGHT = 50 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	4.0	1.25	2.00
B	TMW+w	MT20	2.0	4.0		
C	TTWW+p	MT20	4.0	8.0	Edge	
D	TMW+w	MT20	2.0	4.0		
E	TMVW-p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BBWWW-m	MT20	5.0	8.0	2.75	3.25
H	BBWWW-m	MT20	5.0	8.0	2.75	3.25
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008042

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	680	0	680	0	0	5-8	5-8
F	680	0	680	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	481	316 / 0	0 / 0	0 / 0	0 / 0	165 / 0	0 / 0
F	481	316 / 0	0 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LOC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-973 / 0	-91.8	-91.8	0.12	(1)	6.18
B- C	-1008 / 0	-91.8	-91.8	0.10	(1)	6.13
C- D	-1008 / 0	-91.8	-91.8	0.10	(1)	6.13
D- E	-973 / 0	-91.8	-91.8	0.12	(1)	6.18
I- A	-650 / 0	0.0	0.0	0.07	(1)	7.81
F- E	-650 / 0	0.0	0.0	0.07	(1)	7.81
I- H	0 / 0	-18.5	-18.5	0.06	(4)	10.00
H- G	0 / 502	-18.5	-18.5	0.22	(4)	10.00
G- F	0 / 0	-18.5	-18.5	0.06	(4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- G	0 / 551	0.12 (1)
G- D	-339 / 0	0.05 (1)
H- C	0 / 551	0.12 (1)
H- B	-339 / 0	0.05 (1)
A- H	0 / 822	0.18 (1)
G- E	0 / 822	0.18 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.22/1.00 (G-H:4) , WB=0.18/1.00 (A-H:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (A) (INPUT = 0.90)
JSI METAL= 0.28 (A) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408316	T43SX	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:34 2020 Page 1
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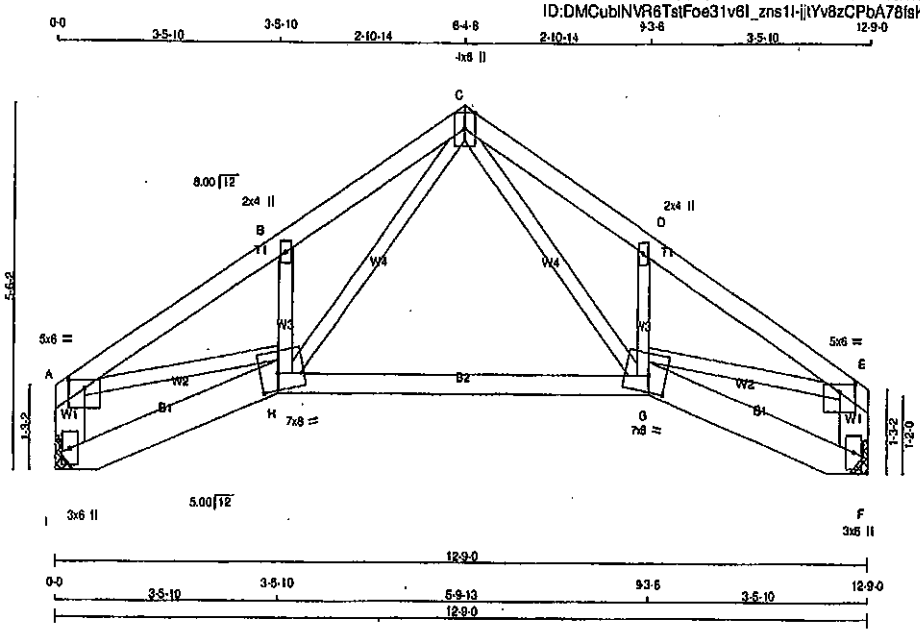

TOTAL WEIGHT = 57 lb
(M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
I - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW-p	MT20	5.0	6.0	1.50	3.00
B	TMW+w	MT20	2.0	4.0		
C	TTWW+p	MT20	4.0	6.0	Edge	
D	TMW+w	MT20	2.0	4.0		
E	TMW-p	MT20	5.0	6.0	1.50	3.00
F	BMV1+p	MT20	3.0	6.0		
G	BBWWW-m	MT20	7.0	8.0	3.00	3.25
H	BBWWW-m	MT20	7.0	8.0	3.00	3.25
I	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

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04/30/20
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100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008043

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	703	0	703	0	0	MECHANICAL	
F	703	0	703	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I, F. MINIMUM BEARING LENGTH AT JOINT I = 3-8, JOINT F = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	497	326 / 0	0 / 0	0 / 0	0 / 0	171 / 0	0 / 0
F	497	326 / 0	0 / 0	0 / 0	0 / 0	171 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-1084 / 0	-91.8	-91.8	0.14 (1)	5.92
B-C	-1119 / 0	-91.8	-91.8	0.11 (1)	5.89
C-D	-1119 / 0	-91.8	-91.8	0.11 (1)	5.89
D-E	-1084 / 0	-91.8	-91.8	0.14 (1)	5.92
I-A	-671 / 0	0.0	0.0	0.04 (1)	7.81
F-E	-671 / 0	0.0	0.0	0.04 (1)	7.81
I-H	0 / 0	-18.5	-18.5	0.03 (4)	10.00
H-G	0 / 544	-18.5	-18.5	0.23 (4)	10.00
G-F	0 / 0	-18.5	-18.5	0.03 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-G	0 / 631	0.14 (1)
G-D	-351 / 0	0.06 (1)
H-C	0 / 631	0.14 (1)
H-B	-351 / 0	0.06 (1)
A-H	0 / 916	0.21 (1)
G-E	0 / 916	0.21 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.23/1.00 (G-H:4) , WB=0.21/1.00 (A-H:1) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (E) (INPUT = 0.90)
JSI METAL= 0.22 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408316	T44	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:35 2020 Page 1

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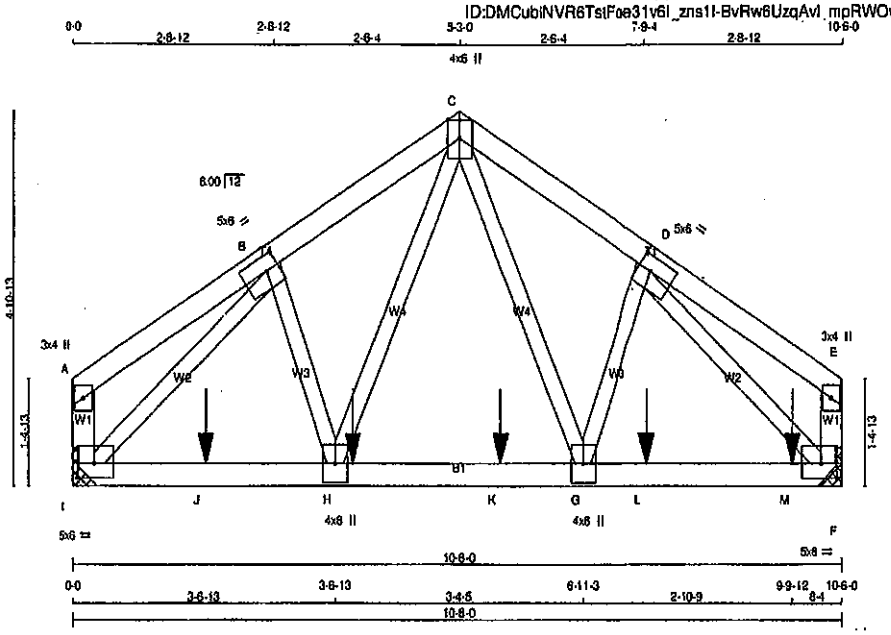

TOTAL WEIGHT = 2 X 47 = 93 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-E	1	12	TOP
I-A	1	12	TOP
F-E	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
I-F	1	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0	2.25	2.00

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008044 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	4453	0	4453	0	0	MECHANICAL	
F	5297	0	5297	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I, F. MINIMUM BEARING LENGTH AT JOINT I = 4-0, JOINT F = 4-0.

UNFACTORED REACTIONS

1ST LOASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	3145	2086 / 0	0 / 0	0 / 0	0 / 0	1059 / 0	0 / 0
F	3742	2481 / 0	0 / 0	0 / 0	0 / 0	1261 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-5 / 0	-91.8	-91.8	0.07 (1)	10.00	
B-C	-4431 / 0	-91.8	-91.8	0.14 (1)	4.45	
C-D	-4422 / 0	-91.8	-91.8	0.14 (1)	4.45	
D-E	-5 / 0	-91.8	-91.8	0.07 (1)	10.00	
I-A	-128 / 0	0.0	0.0	0.01 (1)	7.81	
F-E	-127 / 0	0.0	0.0	0.01 (1)	7.81	
I-J	0 / 3361	-18.5	-18.5	0.74 (1)	10.00	
J-H	0 / 3361	-18.5	-18.5	0.74 (1)	10.00	
H-K	0 / 2833	-18.5	-18.5	0.66 (1)	10.00	
K-G	0 / 2833	-18.5	-18.5	0.66 (1)	10.00	
G-L	0 / 3354	-18.5	-18.5	0.68 (1)	10.00	
L-M	0 / 3354	-18.5	-18.5	0.68 (1)	10.00	
M-F	0 / 3354	-18.5	-18.5	0.68 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-G	0 / 2469	0.31 (1)
G-D	0 / 1006	0.12 (1)
H-C	0 / 2491	0.31 (1)
B-H	0 / 1009	0.12 (1)
I-B	-4953 / 0	0.65 (1)
D-F	-4943 / 0	0.65 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	3-9-12	-1718	-1718	---	FRONT	VERT	TOTAL	--	C1
J	1-9-12	-1718	-1718	---	FRONT	VERT	TOTAL	--	C1
K	5-9-12	-1718	-1718	---	FRONT	VERT	TOTAL	--	C1
L	7-9-12	-1718	-1718	---	FRONT	VERT	TOTAL	--	C1
M	9-9-12	-1721	-1721	---	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.14/1.00 (B-C:1) , BC=0.74/1.00 (H-I:1) , WB=0.65/1.00 (B-I:1) , SSI=0.84/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.57 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408316	T44	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TTWW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	5.0	6.0	2.25	2.00
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	5.0	6.0	2.25	2.75
G	BMWW+t	MT20	4.0	6.0		
H	BMWW+t	MT20	4.0	6.0		
I	BMVW1-t	MT20	5.0	6.0	2.25	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008044 yz

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408316	T45	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:36 2020 Page 1
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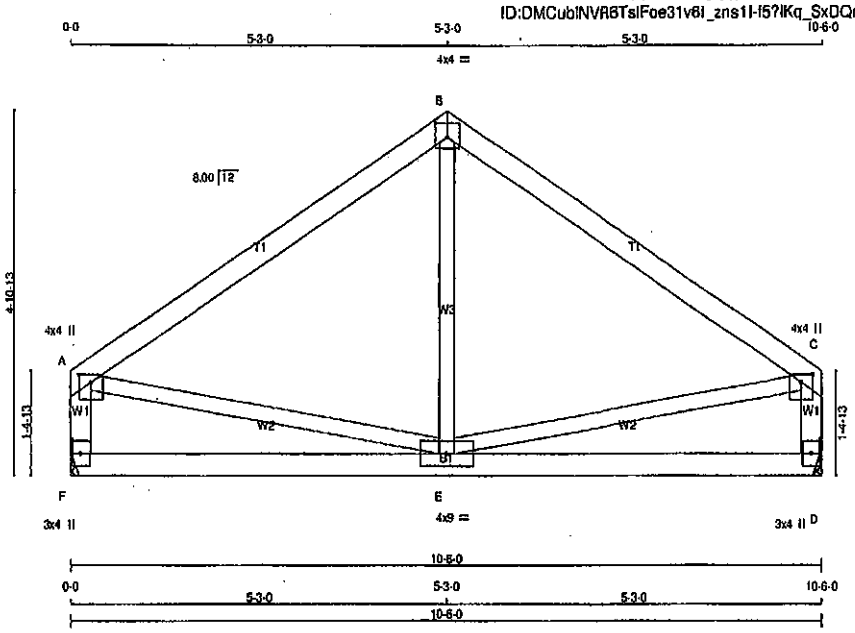

TOTAL WEIGHT = 41 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTW-p	MT20	4.0	4.0	2.25	2.00
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	579	0	579	0	0	MECHANICAL	
D	579	0	579	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	409	269 / 0	0 / 0	0 / 0	0 / 0	141 / 0	0 / 0
D	409	269 / 0	0 / 0	0 / 0	0 / 0	141 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-414 / 0	-91.8	-91.8	0.33 (1)	6.25
B-C	-414 / 0	-91.8	-91.8	0.33 (1)	6.25
F-A	-541 / 0	0.0	0.0	0.06 (1)	7.81
D-C	-541 / 0	0.0	0.0	0.06 (1)	7.81
F-E	0 / 0	-18.5	-18.5	0.14 (4)	10.00
E-D	0 / 0	-18.5	-18.5	0.14 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-22 / 82	0.03 (4)
A-E	0 / 352	0.08 (1)
E-C	0 / 352	0.08 (1)

(M)(F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.33/1.00 (B-C:1) , BC=0.14/1.00 (E-F:4) , WB=0.08/1.00 (A-E:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (C) (INPUT = 0.90)
JSI METAL= 0.13 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008045

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408316	T45S	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:38 2020 Page 1
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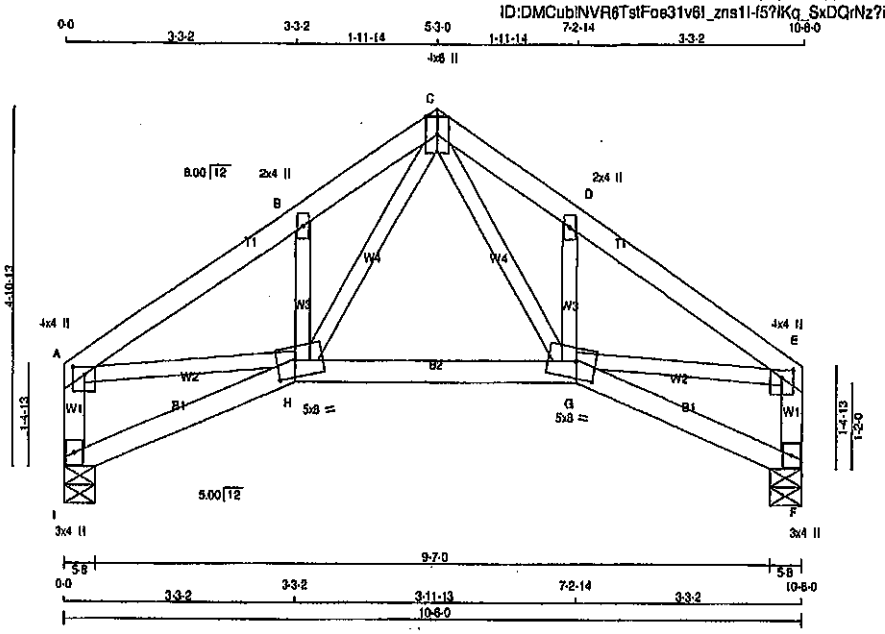

TOTAL WEIGHT = 44 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (tables is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW+p	MT20	4.0	4.0	1.25	2.00
B	TMW+w	MT20	2.0	4.0		
C	TTWW+p	MT20	4.0	6.0	Edge	
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BBWWW-m	MT20	5.0	8.0	2.75	3.25
H	BBWWW-m	MT20	5.0	8.0	2.75	3.25
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	579	0	579	0	0	5-8	5-8
F	579	0	579	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	409	269 / 0	0 / 0	0 / 0	0 / 0	141 / 0	0 / 0
F	409	269 / 0	0 / 0	0 / 0	0 / 0	141 · 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-778 / 0	-91.8	-91.8	0.12 (1)	8.25	
B- C	-813 / 0	-91.8	-91.8	0.08 (1)	8.25	
C- D	-813 / 0	-91.8	-91.8	0.08 (1)	8.25	
D- E	-778 / 0	-91.8	-91.8	0.12 (1)	8.25	
I- A	-549 / 0	0.0	0.0	0.06 (1)	7.81	
F- E	-549 / 0	0.0	0.0	0.06 (1)	7.81	
I- H	0 / 0	-18.5	-18.5	0.06 (4)	10.00	
H- G	0 / 435	-18.5	-18.5	0.12 (4)	10.00	
G- F	0 / 0	-18.5	-18.5	0.06 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- G	0 / 446	0.10 (1)
G- D	-299 / 0	0.05 (1)
H- C	0 / 446	0.10 (1)
H- B	-299 / 0	0.05 (1)
A- H	0 / 657	0.15 (1)
G- E	0 / 657	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.12/1.00 (D-E:1) , BC=0.12/1.00 (G-H:4) , WB=0.15/1.00 (A-H:1) , SSI=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (A) (INPUT = 0.80)
JSI METAL= 0.20 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008046

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T50	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:31 2020 Page 1

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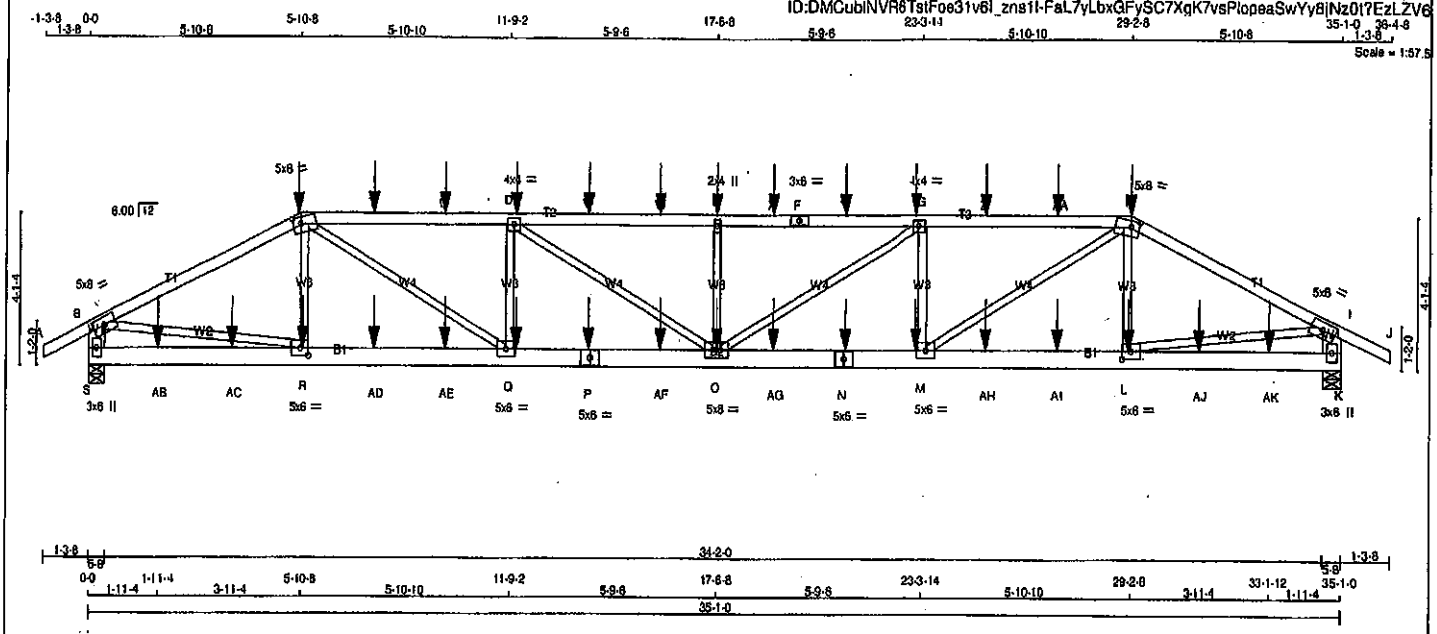

TOTAL WEIGHT = 2 X 154 = 308 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-F	1	12	SIDE(61.0)
F-H	1	12	SIDE(61.0)
H-J	1	12	SIDE(61.0)
S-B	2	12	TOP
K-I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S-P	2	12	SIDE(183.1)
P-N	2	12	SIDE(183.1)
N-K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008054

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3303	0	3303	0	0	5-8	5-8
K	3303	0	3303	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2336	1528 / 0	0 / 0	0 / 0	0 / 0	808 / 0	0 / 0
K	2336	1528 / 0	0 / 0	0 / 0	0 / 0	808 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	
B-C	-5023 / 0	-91.8	-91.8	0.52 (1)	3.89	
C-T	-7076 / 0	-91.8	-91.8	0.74 (1)	3.11	
T-U	-7076 / 0	-91.8	-91.8	0.74 (1)	3.11	
U-D	-7076 / 0	-91.8	-91.8	0.74 (1)	3.11	
D-V	-7846 / 0	-91.8	-91.8	0.80 (1)	2.89	
V-W	-7846 / 0	-91.8	-91.8	0.80 (1)	2.89	
W-E	-7846 / 0	-91.8	-91.8	0.80 (1)	2.89	
E-X	-7846 / 0	-91.8	-91.8	0.80 (1)	2.89	
X-F	-7846 / 0	-91.8	-91.8	0.80 (1)	2.89	
F-Y	-7846 / 0	-91.8	-91.8	0.80 (1)	2.89	
Y-G	-7846 / 0	-91.8	-91.8	0.80 (1)	2.89	
G-Z	-7076 / 0	-91.8	-91.8	0.74 (1)	3.11	
Z-AA	-7076 / 0	-91.8	-91.8	0.74 (1)	3.11	
AA-H	-7076 / 0	-91.8	-91.8	0.74 (1)	3.11	
H-I	-5023 / 0	-91.8	-91.8	0.52 (1)	3.89	
I-J	0 / 28	-91.8	-91.8	0.07 (1)	10.00	
S-B	-3228 / 0	0.0	0.0	0.11 (1)	7.71	
K-I	-3227 / 0	0.0	0.0	0.11 (1)	7.71	
S-AB	0 / 0	-18.5	-18.5	0.06 (4)	10.00	
AB-AC	0 / 0	-18.5	-18.5	0.06 (4)	10.00	
AC-R	0 / 0	-18.5	-18.5	0.06 (4)	10.00	
R-AD	0 / 4480	-18.5	-18.5	0.34 (1)	10.00	
AD-AE	0 / 4480	-18.5	-18.5	0.34 (1)	10.00	
AE-Q	0 / 4480	-18.5	-18.5	0.34 (1)	10.00	
Q-P	0 / 7076	-18.5	-18.5	0.53 (1)	10.00	
P-AF	0 / 7076	-18.5	-18.5	0.53 (1)	10.00	
AF-O	0 / 7076	-18.5	-18.5	0.53 (1)	10.00	
O-AG	0 / 7076	-18.5	-18.5	0.53 (1)	10.00	
AG-N	0 / 7076	-18.5	-18.5	0.53 (1)	10.00	
N-M	0 / 7076	-18.5	-18.5	0.53 (1)	10.00	
M-AH	0 / 4480	-18.5	-18.5	0.34 (1)	10.00	
AH-AI	0 / 4480	-18.5	-18.5	0.34 (1)	10.00	
AI-L	0 / 4480	-18.5	-18.5	0.34 (1)	10.00	
L-AJ	0 / 0	-18.5	-18.5	0.06 (4)	10.00	
AJ-AK	0 / 0	-18.5	-18.5	0.06 (4)	10.00	
AK-K	0 / 0	-18.5	-18.5	0.06 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-C	-377 / 33	0.05 (1)
C-Q	0 / 3072	0.38 (1)
Q-D	-1459 / 0	0.19 (1)
D-O	0 / 916	0.11 (1)
O-E	-821 / 0	0.10 (1)
O-G	0 / 916	0.11 (1)
M-G	-1459 / 0	0.19 (1)
M-H	0 / 3072	0.38 (1)
L-H	-378 / 33	0.05 (1)
B-R	0 / 4528	0.56 (1)
L-I	0 / 4528	0.56 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-414	-414	---	FRONT	VERT	TOTAL	---	C1
D	11-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/901 (0.47")

CSI: TC=0.80/1.00 (D-E:1) , BC=0.53/1.00 (O-Q:1) , WB=0.56/1.00 (B-R:1) , SSI=0.20/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.68 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408317	T50	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:31 2020 Page 2

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TTWW-m	MT20	5.0	8.0	2.00	2.75
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.00	2.75
I	TMVW-t	MT20	5.0	8.0		
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.75
S	BMV1+p	MT20	3.0	6.0		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	17-8-8	-110	-110	---	FRONT	VERT	TOTAL	---	C1
G	23-1-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
H	29-2-8	-414	-414	---	FRONT	VERT	TOTAL	---	C1
L	29-1-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
M	23-1-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
N	21-1-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	17-6-6	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	13-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Q	11-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
R	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
T	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
U	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	15-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	19-1-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	21-1-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	25-1-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	27-1-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	1-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
AC	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AD	7-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AE	9-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AF	15-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AG	19-1-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AH	25-1-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AI	27-1-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AJ	31-1-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AK	33-1-12	-25	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008054

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T51	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:32 2020 Page 1
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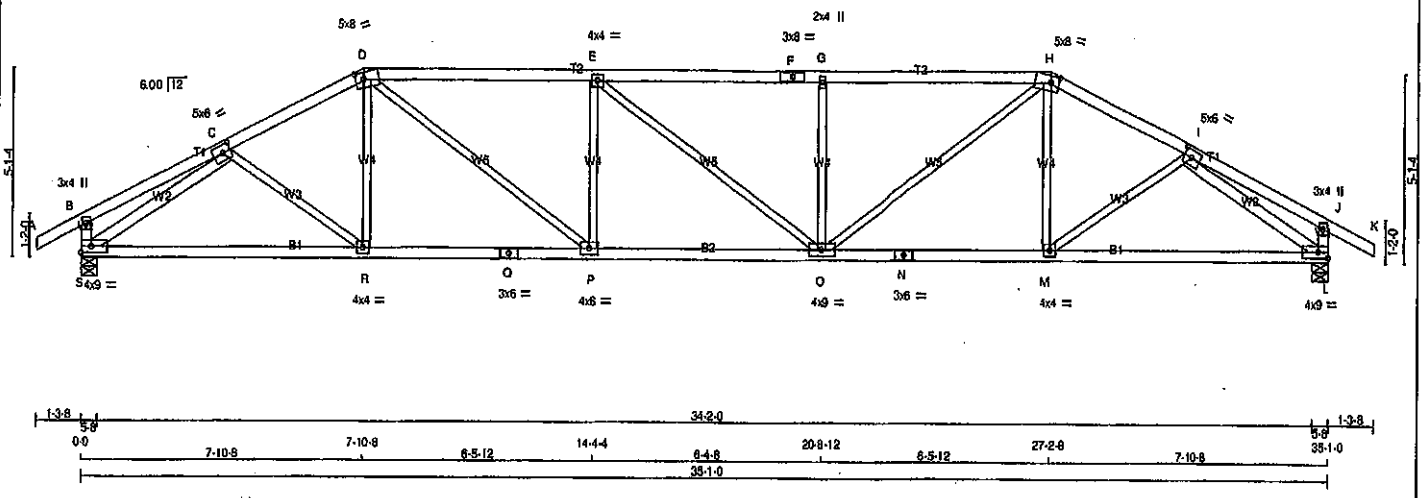

TOTAL WEIGHT = 139 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	8.0	2.25	2.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.25	2.75
I	TMWW-t	MT20	5.0	6.0	2.50	2.75
J	TMV+p	MT20	3.0	4.0		
L	BMW1-t	MT20	4.0	9.0		Edge
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008055

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	FACTORED UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2059	0	2059	0	0	5-8	5-8
L	2059	0	2059	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1453	967 / 0	0 / 0	0 / 0	0 / 0	486 / 0	0 / 0
L	1453	967 / 0	0 / 0	0 / 0	0 / 0	486 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
B-C	0 / 16	-91.8	-91.8	0.20 (1)	10.00	
C-D	-2791 / 0	-91.8	-91.8	0.31 (1)	3.89	
D-E	-3492 / 0	-91.8	-91.8	0.93 (1)	2.95	
E-F	-3491 / 0	-91.8	-91.8	0.92 (1)	2.95	
F-G	-3491 / 0	-91.8	-91.8	0.92 (1)	2.95	
G-H	-3491 / 0	-91.8	-91.8	0.93 (1)	2.97	
H-I	-2791 / 0	-91.8	-91.8	0.31 (1)	3.89	
I-J	0 / 16	-91.8	-91.8	0.20 (1)	10.00	
J-K	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
S-B	-270 / 0	0.0	0.0	0.03 (1)	7.81	
L-J	-270 / 0	0.0	0.0	0.03 (1)	7.81	
S-R	0 / 2411	-18.5	-18.5	0.53 (1)	10.00	
R-Q	0 / 2481	-18.5	-18.5	0.54 (1)	10.00	
Q-P	0 / 2481	-18.5	-18.5	0.54 (1)	10.00	
P-O	0 / 3492	-18.5	-18.5	0.64 (1)	10.00	
O-N	0 / 2482	-18.5	-18.5	0.54 (1)	10.00	
N-M	0 / 2482	-18.5	-18.5	0.54 (1)	10.00	
M-L	0 / 2411	-18.5	-18.5	0.53 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-R	0 / 93	0.03 (4)
R-D	0 / 121	0.04 (4)
D-P	0 / 1258	0.28 (1)
P-E	-641 / 0	0.25 (1)
E-O	-2 / 0	0.00 (1)
O-G	-640 / 0	0.25 (1)
O-H	0 / 1256	0.28 (1)
M-H	0 / 121	0.04 (4)
M-I	0 / 93	0.03 (4)
S-C	-2966 / 0	0.82 (1)
I-L	-2966 / 0	0.82 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.41")

CSI: TC=0.93/1.00 (D-E:1) , BC=0.84/1.00 (O-P:1) , WB=0.82/1.00 (I-L:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.77 (N) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T52	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:33 2020 Page 1
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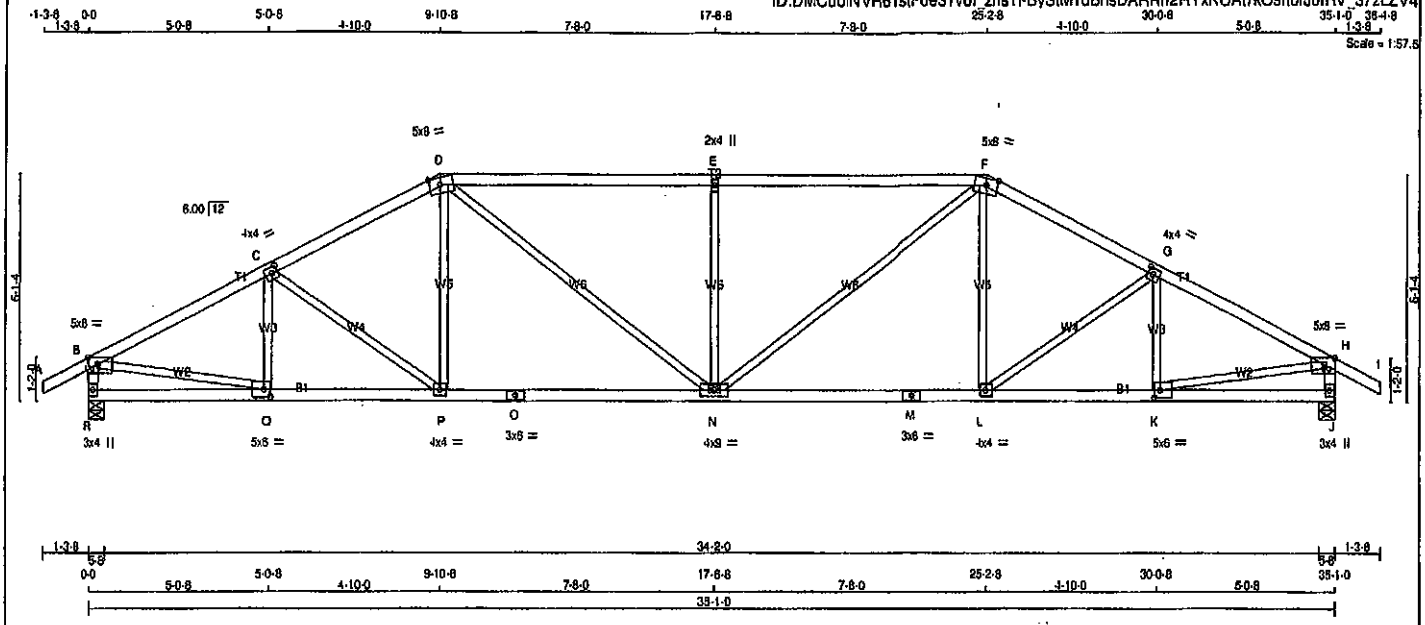

TOTAL WEIGHT = 138 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	Edge	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	2.25	3.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	2.25	3.75
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	5.0	8.0	Edge	3.50
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.00
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.00
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION VERT	HORZ	FACTORED UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2059	0	2059	0	0	5-8	5-8
J	2059	0	2059	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1453	987 / 0	0 / 0	0 / 0	0 / 0	466 / 0	0 / 0
J	1453	987 / 0	0 / 0	0 / 0	0 / 0	466 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-91.8	-91.8	0.12 (1)	10.00
B-C	-2626 / 0	-91.8	-91.8	0.39 (1)	3.81
C-D	-2679 / 0	-91.8	-91.8	0.37 (1)	3.92
D-E	-3044 / 0	-91.8	-91.8	0.96 (1)	2.83
E-F	-3044 / 0	-91.8	-91.8	0.96 (1)	2.83
F-G	-2678 / 0	-91.8	-91.8	0.37 (1)	3.92
G-H	-2626 / 0	-91.8	-91.8	0.39 (1)	3.81
H-I	0 / 26	-91.8	-91.8	0.12 (1)	10.00
R-B	-2014 / 0	0.0	0.0	0.20 (1)	5.95
J-H	-2014 / 0	0.0	0.0	0.20 (1)	5.95
R-Q	0 / 0	-18.5	-18.5	0.10 (4)	10.00
Q-P	0 / 2548	-18.5	-18.5	0.50 (1)	10.00
P-O	0 / 2378	-18.5	-18.5	0.49 (1)	10.00
O-N	0 / 2378	-18.5	-18.5	0.49 (1)	10.00
N-M	0 / 2378	-18.5	-18.5	0.49 (1)	10.00
M-L	0 / 2378	-18.5	-18.5	0.49 (1)	10.00
L-K	0 / 2548	-18.5	-18.5	0.50 (1)	10.00
K-J	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-C	-352 / 0	0.08 (1)
C-P	-211 / 0	0.13 (1)
P-D	0 / 246	0.06 (4)
D-N	0 / 836	0.19 (1)
N-E	-867 / 0	0.51 (1)
N-F	0 / 836	0.19 (1)
L-F	0 / 246	0.06 (4)
L-G	-211 / 0	0.13 (1)
K-G	-352 / 0	0.08 (1)
B-Q	0 / 2584	0.58 (1)
K-H	0 / 2584	0.58 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CSI: TC=0.96/1.00 (D-E:1) , BC=0.50/1.00 (P-Q:1) , WB=0.58/1.00 (B-Q:1) , SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (F) (INPUT = 0.90)
JSI METAL= 0.75 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008056

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T53	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 11:40:05 2020 Page 1

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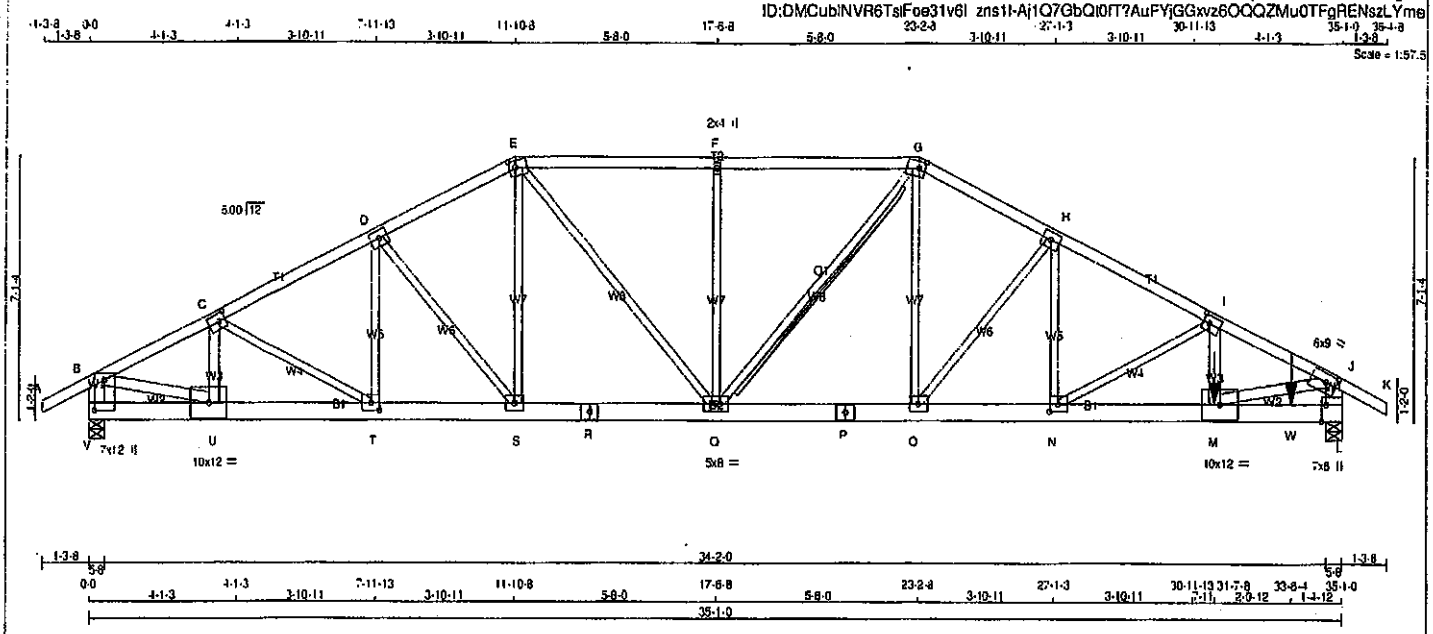

TOTAL WEIGHT = 2 X 177 = 354 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
V - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
B - U	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-E	1	12	TOP
E-G	1	12	TOP
G-K	1	12	TOP
V-B	2	12	TOP
L-J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V-R	2	12	TOP
R-P	2	12	TOP
P-L	2	6	SIDE(427.3)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
I-M	2	2	SIDE(1083.7)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008057 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2649	0	2649	0	0	5-8	5-8
L	7868	0	7868	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1870	1245 0	0 0	0 0	0 0	626 0	0 0
L	5554	3702 0	0 0	0 0	0 0	1852 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT G-Q

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0	28	-91.8	-91.8	0.07 (1)	10.00	
B-C	-3600	0	-91.8	-91.8	0.14 (1)	4.84	
C-D	-3931	0	-91.8	-91.8	0.19 (1)	4.63	
D-E	-3718	0	-91.8	-91.8	0.18 (1)	4.74	
E-F	-4114	0	-91.8	-91.8	0.37 (1)	4.33	
F-G	-4114	0	-91.8	-91.8	0.37 (1)	4.33	
G-H	-4803	0	-91.8	-91.8	0.23 (1)	4.21	
H-I	-6580	0	-91.8	-91.8	0.34 (1)	3.59	
I-J	-10833	0	-91.8	-91.8	0.53 (1)	2.59	
J-K	0	28	-91.8	-91.8	0.07 (1)	10.00	
V-B	-2591	0	0.0	0.0	0.09 (1)	7.81	
L-J	-7297	0	0.0	0.0	0.26 (1)	5.54	
V-U	0	0	-18.5	-18.5	0.03 (1)	10.00	
U-T	0	3234	-18.5	-18.5	0.25 (1)	10.00	
T-S	0	3519	-18.5	-18.5	0.25 (1)	10.00	
S-R	0	3312	-18.5	-18.5	0.23 (1)	10.00	
R-Q	0	3312	-18.5	-18.5	0.23 (1)	10.00	
Q-P	0	4305	-18.5	-18.5	0.30 (1)	10.00	
P-O	0	4305	-18.5	-18.5	0.30 (1)	10.00	
O-N	0	5900	-18.5	-18.5	0.39 (1)	10.00	
N-M	0	9890	-18.5	-18.5	0.79 (1)	10.00	
M-W	0	0	-18.5	-18.5	0.29 (1)	10.00	
W-L	0	0	-18.5	-18.5	0.29 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)		MAX CSI (LC)
U-C	-692	0	0.04 (1)
C-T	0	326	0.04 (1)
T-D	-82	33	0.02 (1)
D-S	-331	0	0.11 (1)
S-E	0	367	0.05 (1)
O-G	0	2035	0.25 (1)
O-H	-2534	0	0.66 (1)
N-H	0	2276	0.28 (1)
N-I	-4330	0	0.97 (1)
M-I	0	3662	0.32 (1)
B-U	0	3314	0.29 (1)
M-J	0	9930	0.88 (1)
Q-G	-297	0	0.12 (1)
E-Q	0	1243	0.15 (1)
Q-F	-630	0	0.26 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	31-7-8	-5715	-5715	---	BACK	VERT	TOTAL	---	C1
W	33-8-4	-684	-684	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER-MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.23")

CSI: TC=0.53(1.00 (J-1)) , BC=0.79(1.00 (M-N:1)) , WB=0.97(1.00 (I-N:1)) , SSI=0.19(1.00 (L-M:1))

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (1) (INPUT = 0.90)
JSI METAL= 0.91 (1) (INPUT = 1.00)

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B						
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TMWW-t	MT20	5.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TMWW-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.25
J	TMVW-t	MT20	6.0	9.0	2.75	3.75
L	BMV1+t	MT20	7.0	8.0	Edge	1.50
M	BMWW-t	MT20	10.0	12.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.75
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.75
U	BMWW-t	MT20	10.0	12.0		
V						
V	TMBMVW1'+pMT20		7.0	12.0	9.75	3.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
09/20/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008057 n2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T54	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:36 2020 Page 1

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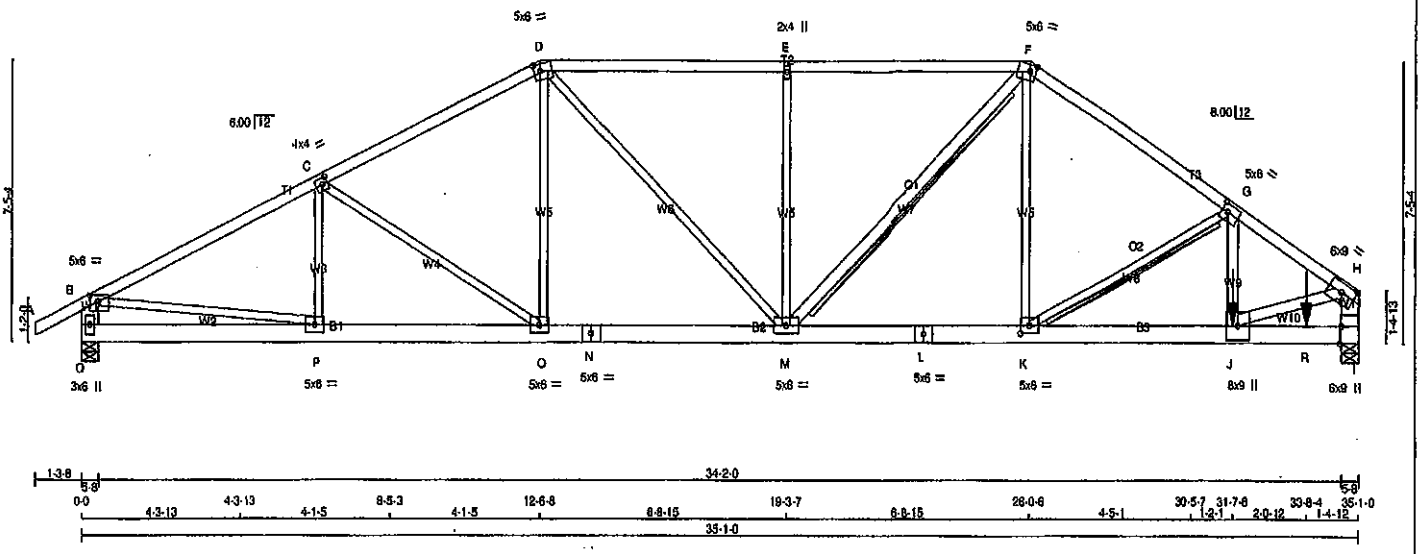

TOTAL WEIGHT = 2 X 171 = 343 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
J - G	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
M - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-D	1	12	TOP
D-F	1	12	TOP
F-H	1	12	TOP
Q-B	2	12	TOP
I-H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q-N	2	12	TOP
N-L	2	12	TOP
L-I	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
G-J	2	2	SIDE(1372.1)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008058 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2673	0	2673	0	0	5-8	5-8
I	7985	0	7985	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1887	1256 / 0	0 / 0	0 / 0	0 / 0	631 / 0	0 / 0
I	5625	3736 / 0	0 / 0	0 / 0	0 / 0	1888 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT G-K, F-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	
B-C	-4025 / 0	-91.8	-91.8	0.51 (1)	4.29	
C-D	-3735 / 0	-91.8	-91.8	0.46 (1)	4.44	
D-E	-4129 / 0	-91.8	-91.8	0.56 (1)	4.14	
E-F	-4129 / 0	-91.8	-91.8	0.56 (1)	4.14	
F-G	-4951 / 0	-91.8	-91.8	0.40 (1)	4.05	
G-H	-9200 / 0	-91.8	-91.8	0.47 (1)	2.91	
Q-B	-2603 / 0	0.0	0.0	0.09 (1)	7.81	
I-H	-7441 / 0	0.0	0.0	0.27 (1)	5.49	
Q-P	0 / 0	-18.5	-18.5	0.05 (4)	10.00	
P-O	0 / 3828	-18.5	-18.5	0.27 (1)	10.00	
O-N	0 / 3316	-18.5	-18.5	0.25 (1)	10.00	
N-M	0 / 3316	-18.5	-18.5	0.25 (1)	10.00	
M-L	0 / 4132	-18.5	-18.5	0.31 (1)	10.00	
L-K	0 / 4132	-18.5	-18.5	0.31 (1)	10.00	
K-J	0 / 7980	-18.5	-18.5	0.61 (1)	10.00	
J-R	0 / 0	-18.5	-18.5	0.28 (1)	10.00	
R-I	0 / 0	-18.5	-18.5	0.28 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-C	-350 / 0	0.05 (1)
C-O	-377 / 0	0.20 (1)
O-D	0 / 363	0.04 (1)
K-F	0 / 2280	0.28 (1)
K-G	-4139 / 0	0.86 (1)
B-P	0 / 3852	0.45 (1)
J-G	0 / 3950	0.35 (1)
J-H	0 / 7993	0.71 (1)
M-F	-8 / 0	0.00 (4)
D-M	0 / 1175	0.15 (1)
M-E	-753 / 0	0.36 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
J	31-7-6	-5962	-5962	---	FRONT	VERT	TOTAL	--	C1
R	33-8-4	-684	-684	---	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-09, CSA O86-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.56/1.00 (O-E:1) , BC=0.61/1.00 (J-K:1) , WB=0.86/1.00 (G-K:1) , SSI=0.17/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.87 (J) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408317	T54	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	2.75
C	TMWW-l	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-l	MT20	5.0	6.0	2.50	2.00
H	TMVW-t	MT20	6.0	9.0	2.75	4.50
I	BMV1+t	MT20	6.0	9.0	Edge	0.50
J	BMWW+t	MT20	8.0	9.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BS-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008058 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T55	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:37 2020 Page 1

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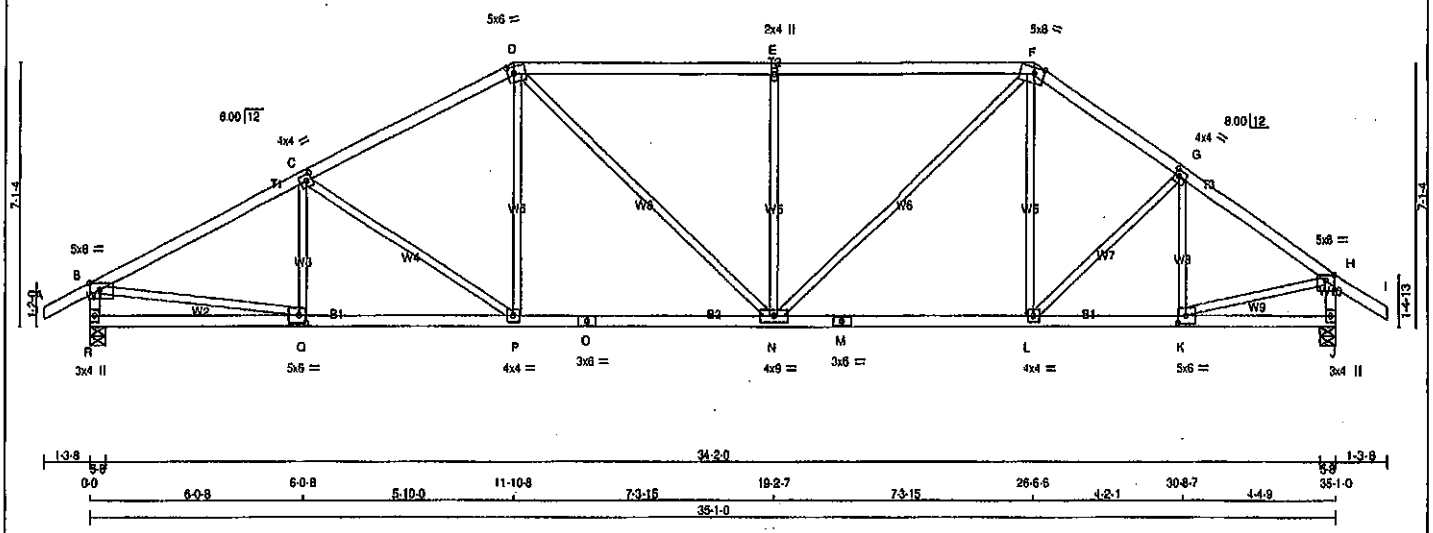

TOTAL WEIGHT = 145 lb

(M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	Edge	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	2.00	3.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMWW-p	MT20	5.0	6.0	1.75	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.25
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008059

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2059	0	2059	0	0	5-8	5-8
J	2060	0	2060	0	0	5-8	5-8

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1453	987 / 0	0 / 0	0 / 0	0 / 0	466 / 0	0 / 0
J	1454	968 / 0	0 / 0	0 / 0	0 / 0	487 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
B- C	-2880 / 0	-91.8	-91.8	0.58 (1)	3.58	
C- D	-2533 / 0	-91.8	-91.8	0.52 (1)	3.84	
D- E	-2566 / 0	-91.8	-91.8	0.80 (1)	3.34	
E- F	-2566 / 0	-91.8	-91.8	0.80 (1)	3.34	
F- G	-2250 / 0	-91.8	-91.8	0.27 (1)	4.33	
G- H	-2262 / 0	-91.8	-91.8	0.27 (1)	4.32	
H- I	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
R- B	-2009 / 0	0.0	0.0	0.20 (1)	5.95	
J- H	-2021 / 0	0.0	0.0	0.21 (1)	5.94	
R- O	0 / 0	-18.5	-18.5	0.14 (4)	10.00	
O- P	0 / 2600	-18.5	-18.5	0.51 (1)	10.00	
P- O	0 / 2244	-18.5	-18.5	0.46 (1)	10.00	
O- N	0 / 2244	-18.5	-18.5	0.46 (1)	10.00	
N- M	0 / 1850	-18.5	-18.5	0.40 (1)	10.00	
M- L	0 / 1850	-18.5	-18.5	0.40 (1)	10.00	
L- K	0 / 1902	-18.5	-18.5	0.39 (1)	10.00	
K- J	0 / 0	-18.5	-18.5	0.08 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-258 / 8	0.07 (1)
C- P	-433 / 0	0.42 (1)
P- D	0 / 383	0.08 (1)
D- N	0 / 440	0.10 (1)
N- E	-828 / 0	0.72 (1)
N- F	0 / 977	0.22 (1)
L- F	0 / 169	0.05 (4)
L- G	-74 / 0	0.04 (1)
K- G	-401 / 0	0.11 (1)
B- Q	0 / 2627	0.59 (1)
K- H	0 / 1959	0.44 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.80/1.00 (D-E:1) , BC=0.61/1.00 (P-Q:1) , WB=0.72/1.00 (E-N:1) , SSI=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.70 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T56	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:39 2020 Page 1
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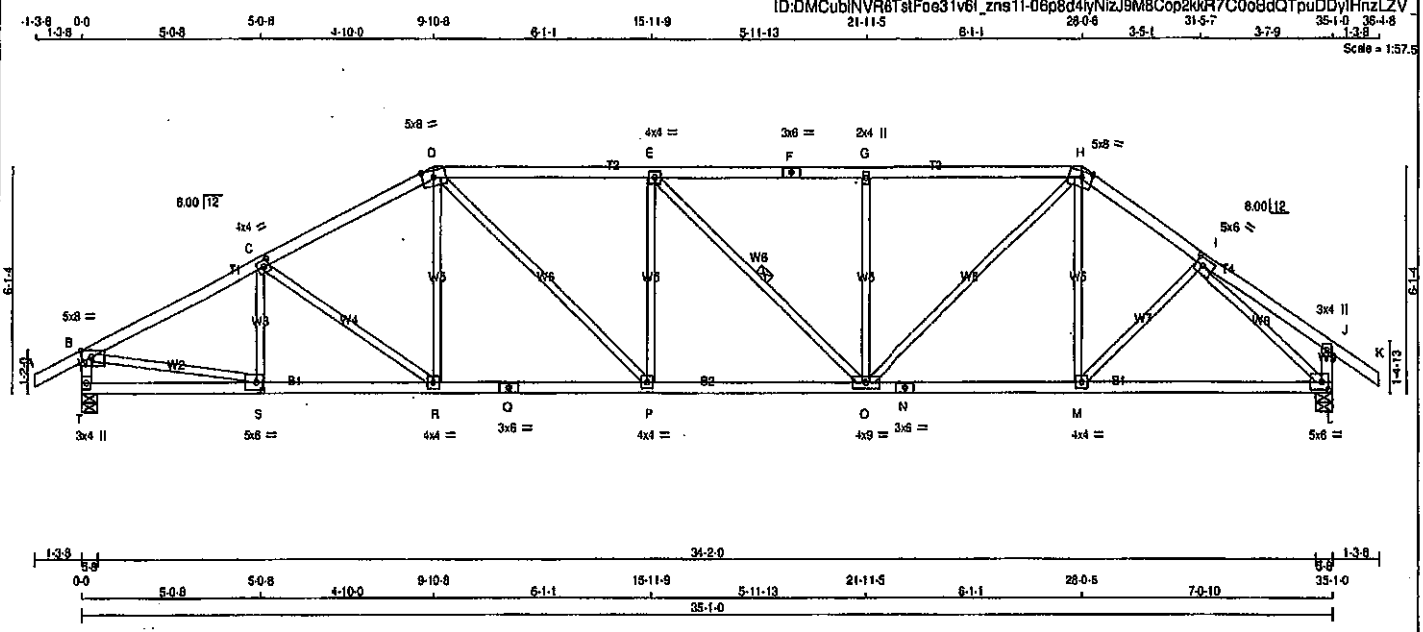

TOTAL WEIGHT = 145 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	Edge	3.50
C	TMWW-l	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	2.25	3.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	1.75	3.50
I	TMWW-t	MT20	5.0	8.0	2.50	2.50
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	8.0	2.50	2.25
M, P, R						
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	8.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.00
T	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008060

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2059	0	2059	0	0	5-8	5-8
L	2060	0	2080	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1453	967 / 0	0 / 0	0 / 0	0 / 0	486 / 0	0 / 0
L	1454	968 / 0	0 / 0	0 / 0	0 / 0	487 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B-C	-2828 / 0	-91.8	-91.8	0.54 (1)	3.65
C-D	-2674 / 0	-91.8	-91.8	0.51 (1)	3.75
D-E	-2958 / 0	-91.8	-91.8	0.72 (1)	3.39
E-F	-2795 / 0	-91.8	-91.8	0.68 (1)	3.50
F-G	-2795 / 0	-91.8	-91.8	0.68 (1)	3.50
G-H	-2795 / 0	-91.8	-91.8	0.68 (1)	3.50
H-I	-2299 / 0	-91.8	-91.8	0.23 (1)	4.32
I-J	0 / 18	-91.8	-91.8	0.16 (1)	10.00
J-K	0 / 35	-91.8	-91.8	0.12 (1)	10.00
T-B	-2015 / 0	0.0	0.0	0.20 (1)	5.95
L-J	-255 / 0	0.0	0.0	0.03 (1)	7.81
T-S	0 / 0	-18.5	-18.5	0.10 (4)	10.00
S-R	0 / 2549	-18.5	-18.5	0.48 (1)	10.00
R-Q	0 / 2374	-18.5	-18.5	0.45 (1)	10.00
Q-P	0 / 2374	-18.5	-18.5	0.45 (1)	10.00
P-O	0 / 2958	-18.5	-18.5	0.54 (1)	10.00
O-N	0 / 1895	-18.5	-18.5	0.42 (1)	10.00
N-M	0 / 1895	-18.5	-18.5	0.42 (1)	10.00
M-L	0 / 1797	-18.5	-18.5	0.40 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-G	-346 / 0	0.08 (1)
C-R	-218 / 0	0.14 (1)
R-D	0 / 231	0.05 (1)
D-P	0 / 808	0.18 (1)
P-E	-444 / 0	0.26 (1)
E-O	-228 / 0	0.11 (1)
O-G	-599 / 0	0.35 (1)
O-H	0 / 1243	0.28 (1)
M-H	0 / 95	0.03 (4)
M-I	0 / 134	0.03 (4)
B-S	0 / 2586	0.58 (1)
I-L	-2508 / 0	0.86 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.72/1.00 (D-E:1) , BC=0.54/1.00 (O-P:1) , WB=0.98/1.00 (I-L:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.74 (Q) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T57	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:40 2020 Page 1

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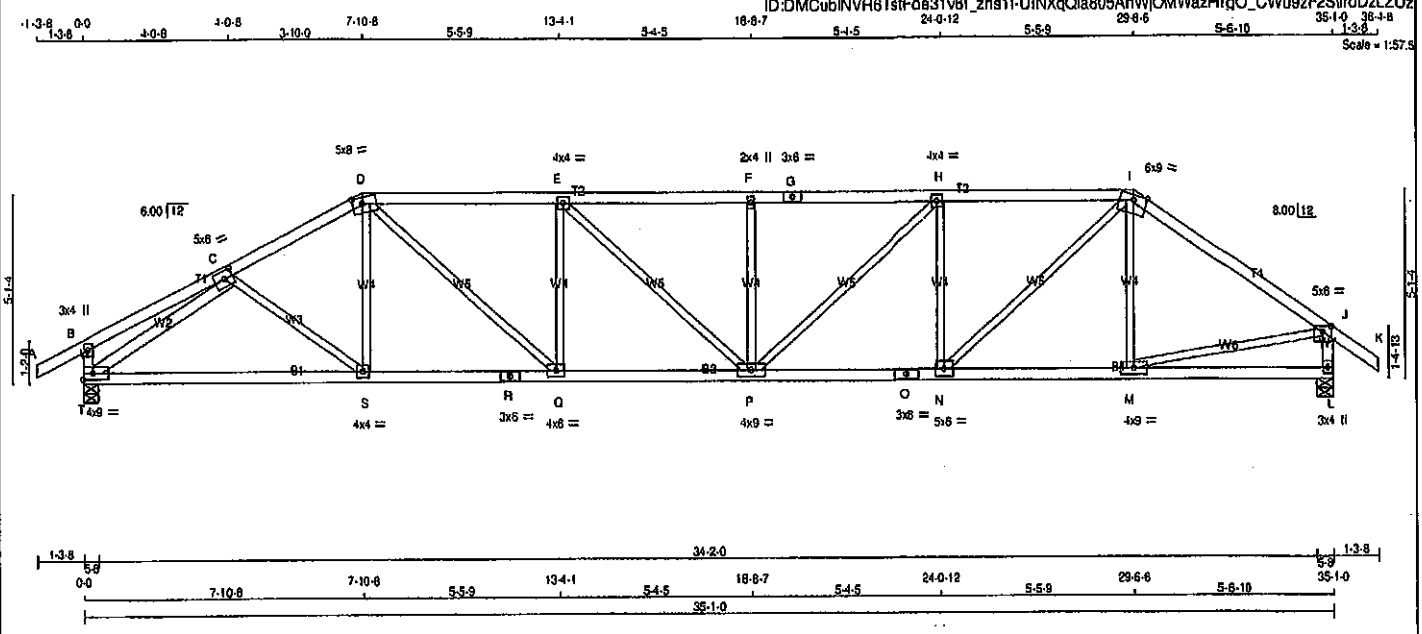

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	8.0	2.00	3.25
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	6.0	9.0	Edge	
J	TMVW-p	MT20	5.0	6.0	1.75	3.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	4.0	9.0		
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	3.0	8.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BMWW-t	MT20	4.0	6.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	4.0	4.0		
T	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008061

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2059	0	2059	0	0	5-8	5-8
L	2060	0	2060	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1453	967 / 0	0 / 0	0 / 0	0 / 0	486 / 0	0 / 0
L	1454	968 / 0	0 / 0	0 / 0	0 / 0	487 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
B-C	0 / 18	-91.8	-91.8	0.20 (1)	10.00	
C-D	-2790 / 0	-91.8	-91.8	0.31 (1)	3.89	
D-E	-3383 / 0	-91.8	-91.8	0.65 (1)	3.28	
E-F	-3551 / 0	-91.8	-91.8	0.67 (1)	3.16	
F-G	-3551 / 0	-91.8	-91.8	0.67 (1)	3.16	
G-H	-3551 / 0	-91.8	-91.8	0.67 (1)	3.16	
H-I	-3100 / 0	-91.8	-91.8	0.61 (1)	3.42	
I-J	-2289 / 0	-91.8	-91.8	0.70 (1)	3.79	
J-K	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
T-B	-270 / 0	0.0	0.0	0.03 (1)	7.81	
L-J	-2017 / 0	0.0	0.0	0.21 (1)	5.94	
T-S	0 / 2410	-18.5	-18.5	0.53 (1)	10.00	
S-R	0 / 2480	-18.5	-18.5	0.53 (1)	10.00	
R-Q	0 / 2480	-18.5	-18.5	0.53 (1)	10.00	
Q-P	0 / 3383	-18.5	-18.5	0.60 (1)	10.00	
P-O	0 / 3100	-18.5	-18.5	0.55 (1)	10.00	
O-N	0 / 3100	-18.5	-18.5	0.55 (1)	10.00	
N-M	0 / 1897	-18.5	-18.5	0.38 (1)	10.00	
M-L	0 / 0	-18.5	-18.5	0.14 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-S	0 / 92	0.03 (4)
S-D	0 / 117	0.04 (4)
D-Q	0 / 1201	0.27 (1)
Q-E	-712 / 0	0.27 (1)
E-P	0 / 228	0.05 (1)
P-F	-454 / 0	0.17 (1)
P-H	0 / 607	0.14 (1)
N-H	-963 / 0	0.37 (1)
N-I	0 / 1601	0.36 (1)
M-I	-247 / 5	0.09 (1)
T-C	-2965 / 0	0.82 (1)
M-J	0 / 1936	0.44 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CSI: TC=0.70/1.00 (I-J:1) , BC=0.60/1.00 (P-Q:1) , WB=0.82/1.00 (C-T:1) , SSI=0.23/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.90 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T58	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:41 2020 Page 1

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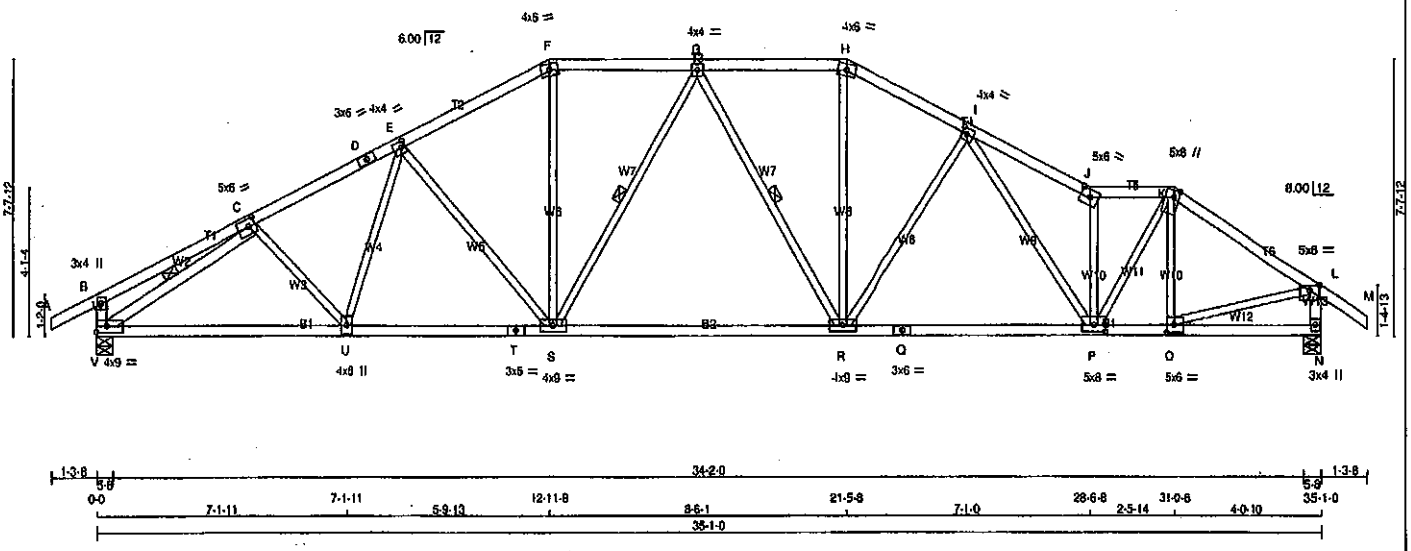

TOTAL WEIGHT = 156 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
K - M	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
V - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TTW-m	MT20	4.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTW-m	MT20	4.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TTW-h	MT20	5.0	6.0	2.50	3.25
K	TTWW+m	MT20	5.0	8.0	2.25	1.75
L	TMVW-p	MT20	5.0	6.0	1.75	3.00
N	BMV1+p	MT20	3.0	4.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.75
P	BMWWW-t	MT20	5.0	8.0	2.25	4.00
Q	BS-t	MT20	3.0	6.0		
R	BMWWW-t	MT20	4.0	9.0		
S	BMWWW-t	MT20	4.0	9.0		
T	BS-t	MT20	3.0	6.0		
U	BMWW+t	MT20	4.0	6.0		
V	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008062

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2059	0	2059	0	0	5-8	5-8
N	2060	0	2060	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1453	967 / 0	0 / 0	0 / 0	0 / 0	486 / 0	0 / 0
N	1454	968 / 0	0 / 0	0 / 0	0 / 0	487 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-S, G-R, C-V.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.13 (1)	10.00	
B- C	0 / 15	-91.8	-91.8	0.23 (1)	10.00	
C- D	-2780 / 0	-91.8	-91.8	0.39 (1)	3.78	
D- E	-2780 / 0	-91.8	-91.8	0.39 (1)	3.78	
E- F	-2429 / 0	-91.8	-91.8	0.36 (1)	4.04	
F- G	-2167 / 0	-91.8	-91.8	0.37 (1)	4.21	
G- H	-2213 / 0	-91.8	-91.8	0.37 (1)	4.16	
H- I	-2481 / 0	-91.8	-91.8	0.30 (1)	4.05	
I- J	-3018 / 0	-91.8	-91.8	0.35 (1)	3.67	
J- K	-2674 / 0	-91.8	-91.8	0.20 (1)	4.03	
K- L	-2219 / 0	-91.8	-91.8	0.39 (1)	4.15	
L- M	0 / 35	-91.8	-91.8	0.14 (1)	10.00	
V- B	-291 / 0	0.0	0.0	0.03 (1)	7.81	
N- L	-2027 / 0	0.0	0.0	0.23 (1)	5.88	
V- U	0 / 2454	-18.5	-18.5	0.57 (1)	10.00	
U- T	0 / 2460	-18.5	-18.5	0.55 (1)	10.00	
T- S	0 / 2460	-18.5	-18.5	0.55 (1)	10.00	
S- R	0 / 2322	-18.5	-18.5	0.58 (1)	10.00	
R- Q	0 / 2510	-18.5	-18.5	0.61 (1)	10.00	
Q- P	0 / 2510	-18.5	-18.5	0.61 (1)	10.00	
P- O	0 / 1832	-18.5	-18.5	0.36 (1)	10.00	
O- N	0 / 0	-18.5	-18.5	0.08 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- U	0 / 77	0.03 (4)
U- E	0 / 86	0.04 (4)
E- S	-474 / 0	0.42 (1)
S- F	0 / 769	0.19 (1)
S- G	-317 / 0	0.16 (1)
G- R	-222 / 0	0.11 (1)
R- H	0 / 826	0.20 (1)
R- I	-563 / 0	0.47 (1)
I- P	0 / 378	0.09 (1)
P- J	-1585 / 0	0.43 (1)
V- C	-2991 / 0	0.50 (1)
P- K	0 / 1583	0.39 (1)
O- L	0 / 1904	0.47 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.39/1.00 (C-E:1) , BC=0.61/1.00 (P-R:1) , WB=0.50/1.00 (C-V:1) , SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.75 (T) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T59	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 6.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:43 2020 Page 1

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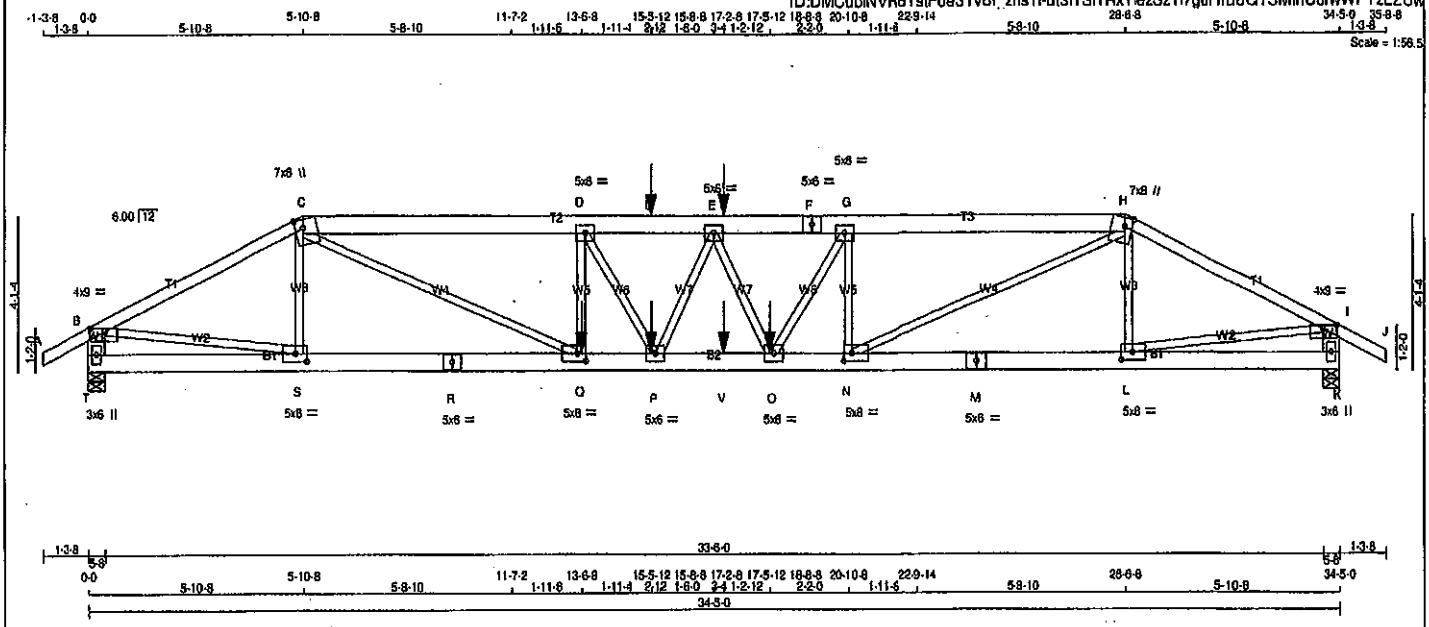

TOTAL WEIGHT = 2 X 167 = 335 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
K - I	2x8	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - M	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
H- J	1	12	TOP
C- F	2	12	SIDE(0.0)
F- H	2	12	TOP
T- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T- R	2	12	TOP
R- M	2	12	SIDE(183.1)
M- K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
Q- D	1	6	SIDE(237.3)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008063 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	4094	0	4094	0	0	5-8	5-8
K	3933	0	3933	0	0	5-8	5-8

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2685	1951 / 0	0 / 0	0 / 0	0 / 0	933 0	0 / 0
K	2772	1872 / 0	0 / 0	0 / 0	0 / 0	900 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	
B- C	-6510 / 0	-91.8	-91.8	0.64 (1)	3.38	
C- D	-11956 / 0	-91.8	-91.8	0.54 (1)	3.22	
D- U	-12303 / 0	-91.8	-91.8	0.34 (1)	3.31	
U- E	-12303 / 0	-91.8	-91.8	0.34 (1)	3.31	
E- F	-12466 / 0	-91.8	-91.8	0.33 (1)	3.30	
F- G	-12466 / 0	-91.8	-91.8	0.33 (1)	3.30	
G- H	-11395 / 0	-91.8	-91.8	0.51 (1)	3.31	
H- I	-6215 / 0	-91.8	-91.8	0.62 (1)	3.47	
I- J	0 / 28	-91.8	-91.8	0.07 (1)	10.00	
T- B	-4062 / 0	0.0	0.0	0.14 (1)	7.06	
K- I	-3896 / 0	0.0	0.0	0.14 (1)	7.17	
T- S	0 / 0	-18.5	-18.5	0.08 (4)	10.00	
S- R	0 / 5853	-18.5	-18.5	0.48 (1)	10.00	
R- Q	0 / 5853	-18.5	-18.5	0.48 (1)	10.00	
Q- P	0 / 11956	-18.5	-18.5	0.87 (1)	10.00	
P- V	0 / 12462	-18.5	-18.5	0.93 (1)	10.00	
V- O	0 / 12462	-18.5	-18.5	0.93 (1)	10.00	
O- N	0 / 11395	-18.5	-18.5	0.86 (1)	10.00	
N- M	0 / 5588	-18.5	-18.5	0.43 (1)	10.00	
M- L	0 5588	-18.5	-18.5	0.43 (1)	10.00	
L- K	0 / 0	-18.5	-18.5	0.06 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-588 / 0	0.07 (1)
L- H	-577 / 0	0.07 (1)
B- S	0 / 5888	0.73 (1)
L- I	0 / 5621	0.70 (1)
P- E	-392 / 0	0.05 (1)
E- O	0 / 60	0.01 (1)
Q- D	-1367 / 0	0.16 (1)
N- G	-2629 / 0	0.31 (1)
D- P	0 / 708	0.09 (1)
O- G	0 / 2219	0.27 (1)
C- Q	0 / 6770	0.84 (1)
N- H	0 / 6441	0.80 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	17-5-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
O	18-8-8	-2098	-2098	---	BACK	VERT	TOTAL	---	C1
P	15-5-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
Q	13-6-8	-1611	-1611	---	BACK	VERT	TOTAL	---	C1
U	15-5-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
V	17-5-12	-26	-26	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-09, CSA O86-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.15")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.34")
ALLOWABLE DEFL.(TL)= L/360 (1.15")
CALCULATED VERT. DEFL.(TL) = L/ 671 (0.62")

CSI: TC=0.64/1.00 (B-C:1) , BC=0.93/1.00 (O-P:1) , WB=0.84/1.00 (C-Q:1) , SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (N) (INPUT = 0.90)
JSI METAL= 0.60 (R) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408317	T59	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	9.0	1.00	4.75
C	TTWW+m	MT20	7.0	8.0	2.75	2.50
D, E, G						
D	TMWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
H	TTWW+m	MT20	7.0	8.0	2.75	2.50
I	TMVW-p	MT20	4.0	9.0	1.00	4.75
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	8.0	2.50	3.75
M	BS-t	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	8.0	2.50	2.75
O	BMWW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	8.0	2.50	2.75
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	8.0	2.50	3.75
T	BMV1+p	MT20	3.0	6.0		

LICENSED PROFESSIONAL ENGINEER
04/30/20
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100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008063

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T60X	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 6.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:44 2020 Page 1
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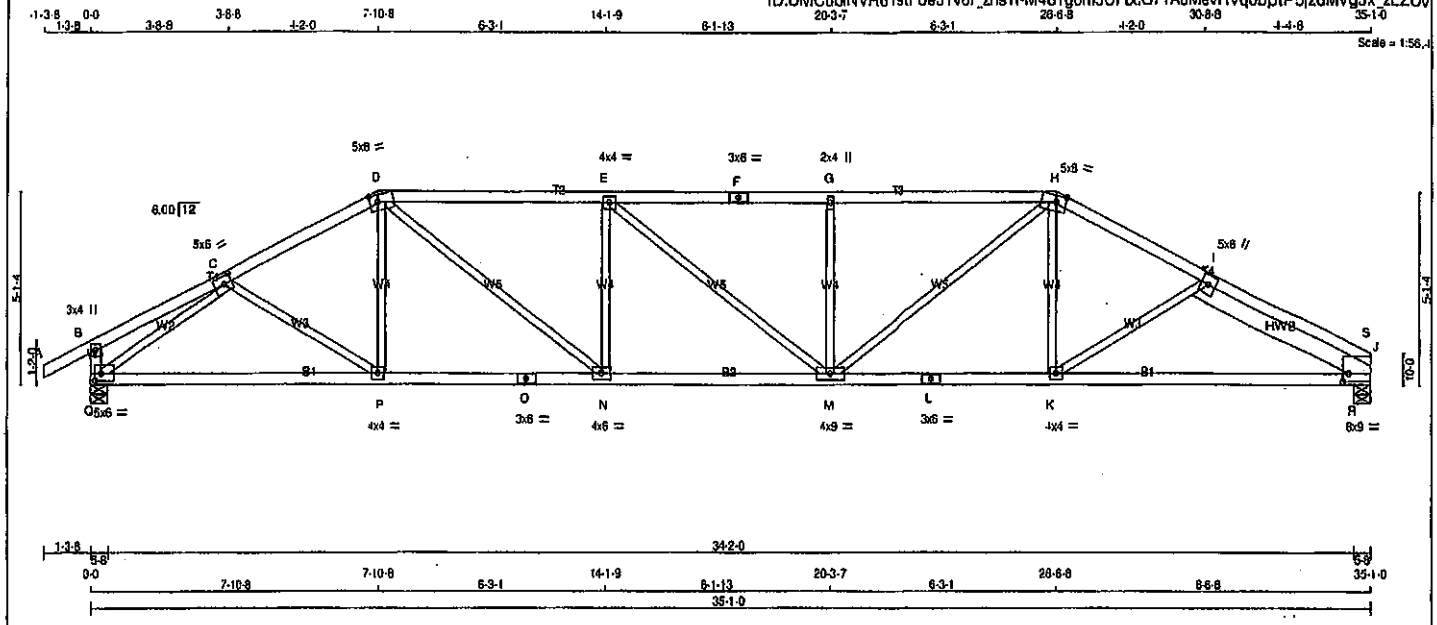

TOTAL WEIGHT = 139 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW2	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	8.0	2.50	2.75
D	TTWW-m	MT20	5.0	8.0	2.25	2.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.25	3.25
I	TMWW+t	MT20	5.0	6.0		
J	TMBMW1-t	MT20	8.0	9.0	2.50	2.00
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMWW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMVW1-t	MT20	5.0	6.0	2.25	2.00

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008064

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1934	0	1934	0	0	5-8	5-8
Q	2059	0	2059	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1368	898 / 0	0 / 0	0 / 0	0 / 0	470 / 0	0 / 0
Q	1453	967 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, Q

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
B-C	0 / 17	-91.8	-91.8	0.19 (1)	10.00	
C-D	-2789 / 0	-91.8	-91.8	0.32 (1)	3.89	
D-E	-3469 / 0	-91.8	-91.8	0.85 (1)	3.05	
E-F	-3515 / 0	-91.8	-91.8	0.85 (1)	3.02	
F-G	-3515 / 0	-91.8	-91.8	0.85 (1)	3.02	
G-H	-3515 / 0	-91.8	-91.8	0.85 (1)	3.03	
H-I	-2942 / 0	-91.8	-91.8	0.33 (1)	3.80	
I-S	-1483 / 0	-91.8	-91.8	0.23 (1)	5.13	
S-J	-2458 / 0	-91.8	-91.8	0.16 (1)	4.25	
Q-B	-252 / 0	0.0	0.0	0.03 (1)	7.81	
Q-P	0 / 2368	-18.5	-18.5	0.52 (1)	10.00	
P-O	0 / 2481	-18.5	-18.5	0.53 (1)	10.00	
O-N	0 / 2481	-18.5	-18.5	0.53 (1)	10.00	
N-M	0 / 3469	-18.5	-18.5	0.63 (1)	10.00	
M-L	0 / 2621	-18.5	-18.5	0.54 (1)	10.00	
L-K	0 / 2621	-18.5	-18.5	0.54 (1)	10.00	
K-R	0 / 2754	-18.5	-18.5	0.56 (1)	10.00	
R-J	0 / 1333	-18.5	-18.5	0.35 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-P	0 / 130	0.04 (4)
P-D	0 / 118	0.04 (4)
D-N	0 / 1246	0.28 (1)
N-E	-655 / 0	0.25 (1)
E-M	0 / 59	0.01 (1)
M-G	-617 / 0	0.24 (1)
M-H	0 / 1128	0.25 (1)
K-H	0 / 231	0.06 (4)
K-I	-184 / 0	0.07 (1)
Q-C	-2942 0	0.97 (1)
R-S	0 / 1217	0.00 (1)
R-I	-1727 0	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.40")

CSI: TC=0.85/1.00 (G-H:1) , BC=0.63/1.00 (M-N:1) , WB=0.97/1.00 (C-Q:1) , SSI=0.28/1.00 (J-R:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.81 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T61X	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:45 2020 Page 1

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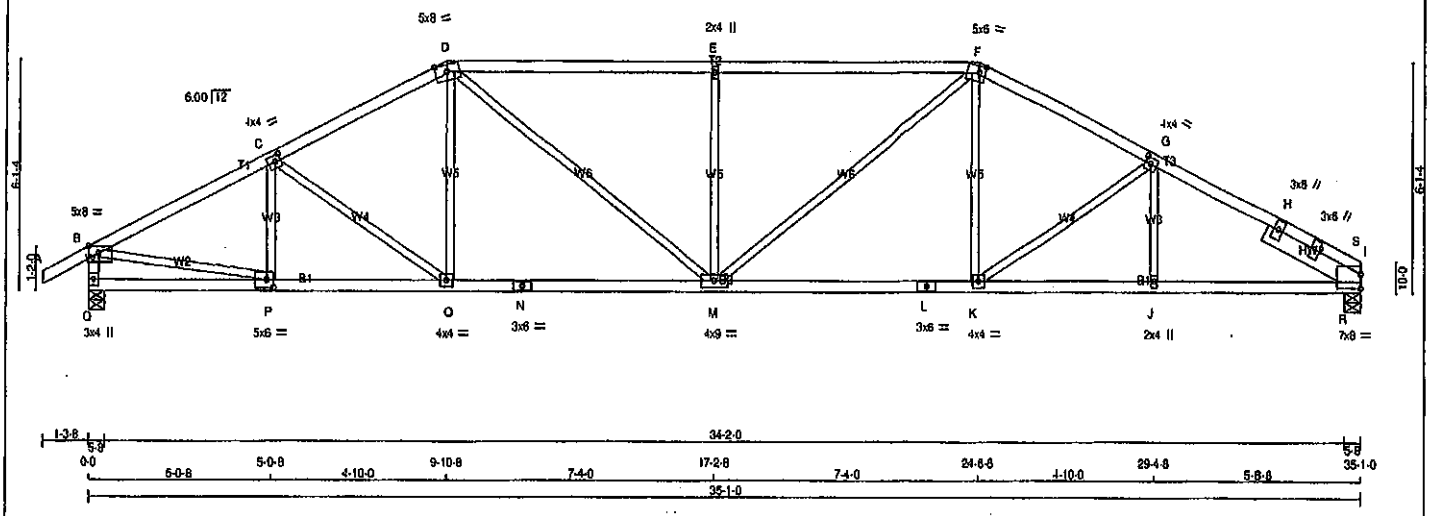

TOTAL WEIGHT = 137 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW2	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	Edge	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	3.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
I	TMBMR1-t	MT20	7.0	8.0	4.50	
I	RT+t	MT20	3.0	6.0		
I	RT+t	MT20	3.0	6.0		
J	BMW+w	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.00
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008065

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1934	0	1934	0	0	5-8	5-8
Q	2059	0	2059	0	0	5-8	5-8

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1368	898 / 0	0 / 0	0 / 0	0 / 0	470 / 0	0 / 0
Q	1453	967 / 0	0 / 0	0 / 0	0 / 0	486 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
B-C	-2827 / 0	-91.8	-91.8	0.39 (1)	3.81	
C-D	-2678 / 0	-91.8	-91.8	0.37 (1)	3.92	
D-E	-3031 / 0	-91.8	-91.8	0.86 (1)	3.03	
E-F	-3031 / 0	-91.8	-91.8	0.86 (1)	3.03	
F-G	-2763 / 0	-91.8	-91.8	0.45 (1)	3.77	
G-H	-2210 / 0	-91.8	-91.8	0.53 (1)	4.02	
H-S	-2210 / 0	-91.8	-91.8	0.53 (1)	4.02	
S-I	-2714 / 0	-91.8	-91.8	0.17 (1)	4.09	
Q-B	-2014 / 0	0.0	0.0	0.20 (1)	5.95	
Q-P	0 / 0	-18.5	-18.5	0.10 (4)	10.00	
P-O	0 / 2548	-18.5	-18.5	0.50 (1)	10.00	
O-N	0 / 2377	-18.5	-18.5	0.47 (1)	10.00	
N-M	0 / 2377	-18.5	-18.5	0.47 (1)	10.00	
M-L	0 / 2472	-18.5	-18.5	0.51 (1)	10.00	
L-K	0 / 2472	-18.5	-18.5	0.51 (1)	10.00	
K-J	0 / 2727	-18.5	-18.5	0.55 (1)	10.00	
J-R	0 / 2727	-18.5	-18.5	0.73 (1)	10.00	
R-I	0 / 1301	-18.5	-18.5	0.50 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-C	-351 / 0	0.08 (1)
C-O	-213 / 0	0.13 (1)
O-D	0 / 244	0.06 (4)
D-M	0 / 835	0.19 (1)
M-E	-828 / 0	0.48 (1)
M-F	0 / 713	0.16 (1)
K-F	0 / 321	0.07 (1)
K-G	-315 / 0	0.20 (1)
J-G	0 / 71	0.02 (4)
B-P	0 / 2585	0.58 (1)
R-S	0 / 1839	0.00 (1)
R-H	-1903 / 0	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL.	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.35")

CSI: TC=0.86/1.00 (D-E:1) , BC=0.73/1.00 (J-R:1) , WB=0.58/1.00 (B-P:1) , SSI=0.37/1.00 (I-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.98 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T62X	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Rool Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:46 2020 Page 1

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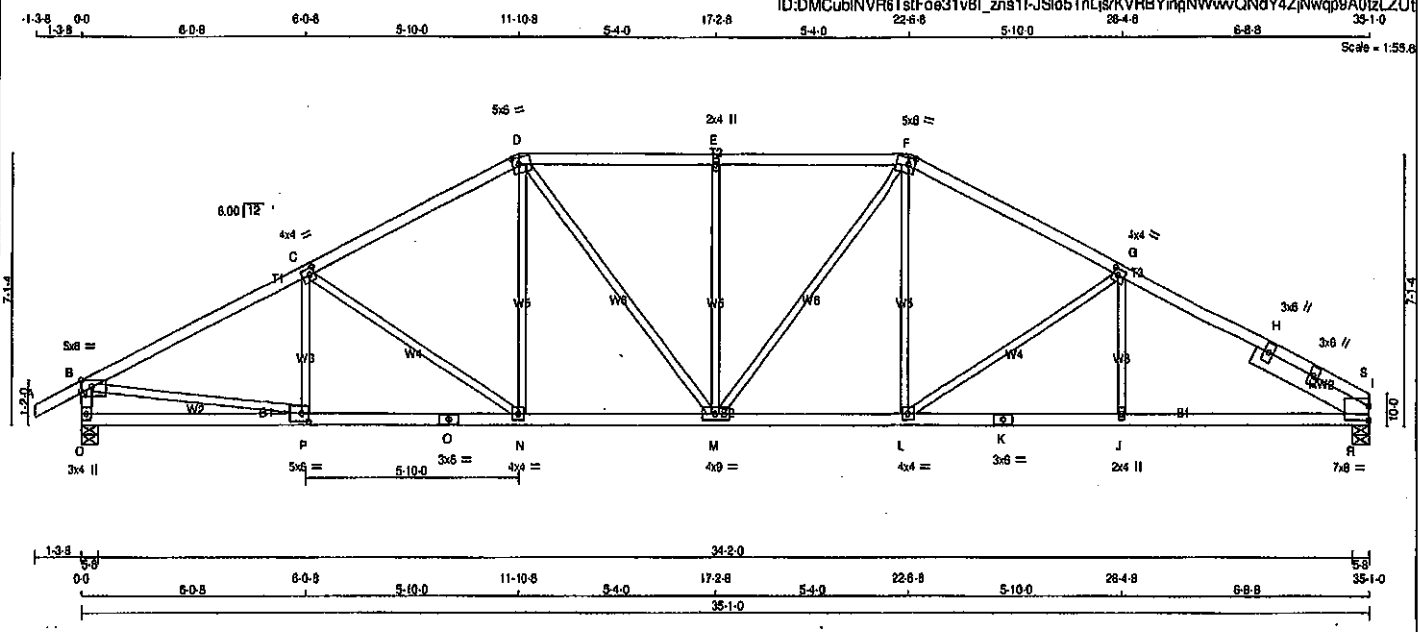

TOTAL WEIGHT = 143 lb

[M][F]

LUMBER

N. L. G. A RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

REINFORCING MEMBERS

HW2	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	3.50
C	TMWW-l	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
I	TMBMR1-l	MT20	7.0	8.0	4.50	
I	RT+t	MT20	3.0	6.0		
I	RT+t	MT20	3.0	6.0		
J	BMW+w	MT20	2.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.25
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008066

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1934	0	1934	0	0	5-8	5-8
Q	2059	0	2059	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1388	898 / 0	0 / 0	0 / 0	0 / 0	470 / 0	0 / 0
Q	1453	987 / 0	0 / 0	0 / 0	0 / 0	466 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
B-C	-2881 / 0	-91.8	-91.8	0.58 (1)	3.58	
C-D	-2530 / 0	-91.8	-91.8	0.52 (1)	3.84	
D-E	-2538 / 0	-91.8	-91.8	0.42 (1)	3.94	
E-F	-2538 / 0	-91.8	-91.8	0.42 (1)	3.94	
F-G	-2579 / 0	-91.8	-91.8	0.56 (1)	3.75	
G-H	-2225 / 0	-91.8	-91.8	0.61 (1)	4.08	
H-S	-2225 / 0	-91.8	-91.8	0.61 (1)	4.08	
S-I	-2847 / 0	-91.8	-91.8	0.13 (1)	4.17	
Q-B	-2009 / 0	0.0	0.0	0.20 (1)	5.95	
Q-P	0 / 0	-18.5	-18.5	0.15 (4)	10.00	
P-O	0 / 2602	-18.5	-18.5	0.49 (1)	10.00	
O-N	0 / 2602	-18.5	-18.5	0.49 (1)	10.00	
N-M	0 / 2241	-18.5	-18.5	0.43 (1)	10.00	
M-L	0 / 2301	-18.5	-18.5	0.46 (1)	10.00	
L-K	0 / 2744	-18.5	-18.5	0.53 (1)	10.00	
K-J	0 / 2744	-18.5	-18.5	0.53 (1)	10.00	
J-R	0 / 2744	-18.5	-18.5	0.68 (1)	10.00	
R-I	0 / 1303	-18.5	-18.5	0.45 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-C	-263 / 13	0.07 (1)
C-N	-437 / 0	0.42 (1)
N-D	0 / 349	0.08 (1)
D-M	0 / 481	0.11 (1)
M-E	-597 / 0	0.52 (1)
M-F	0 / 384	0.09 (1)
L-F	0 / 417	0.09 (1)
L-G	-537 / 0	0.52 (1)
J-G	0 / 111	0.04 (4)
B-P	0 / 2628	0.59 (1)
R-S	0 / 1848	0.00 (1)
R-H	-1650 / 0	0.19 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2016 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.58/1.00 (B-C:1) , BC=0.68/1.00 (J-R:1) , WB=0.59/1.00 (B-P:1) , SSI=0.31/1.00 (I-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.98 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T63	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:48 2020 Page 1
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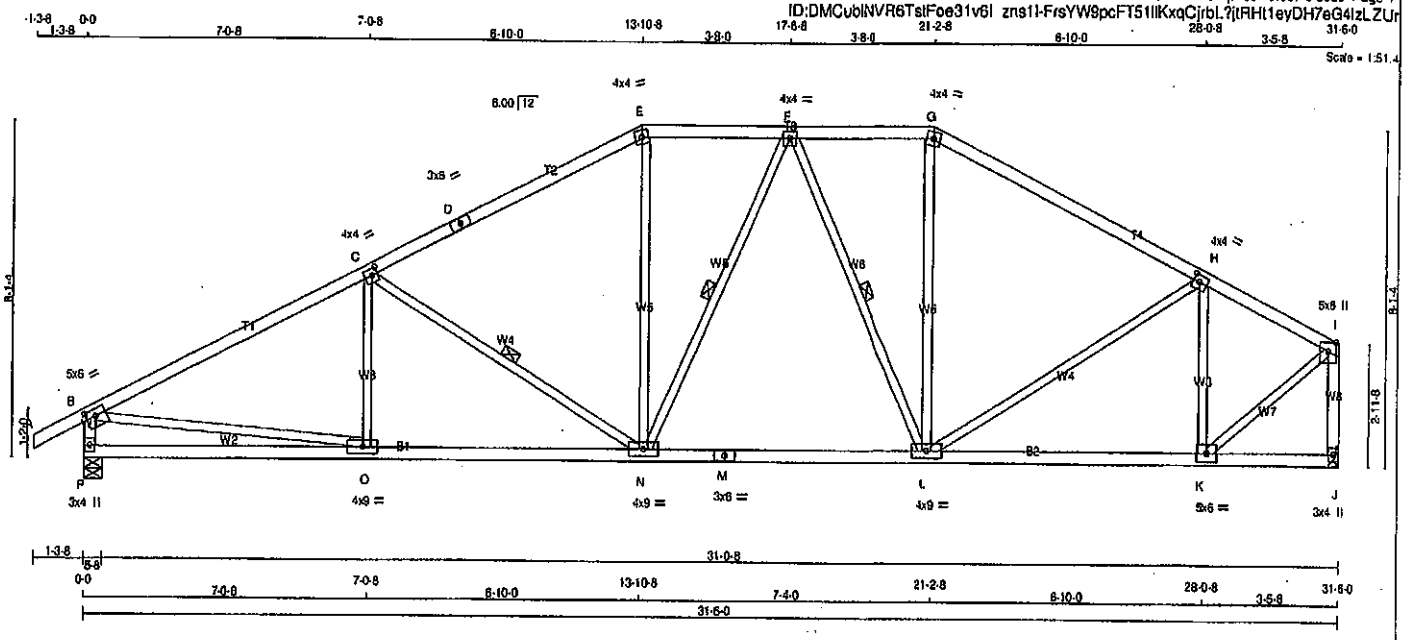

TOTAL WEIGHT = 135 lb

[M|F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0	2.00	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.75
I	TMWW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	9.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008067

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
P	1861	0	1861	0	0	IN-SX	IN-SX
J	1736	0	1736	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1314	875 / 0	0 / 0	0 / 0	0 / 0	438 / 0	0 / 0
J	1228	806 / 0	0 / 0	0 / 0	0 / 0	422 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-N, F-N, F-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
B-C	-2544 / 0	-91.8	-91.8	0.77 (1)	3.50	
C-D	-1971 / 0	-91.8	-91.8	0.67 (1)	4.01	
D-E	-1971 / 0	-91.8	-91.8	0.67 (1)	4.01	
E-F	-1737 / 0	-91.8	-91.8	0.18 (1)	4.90	
F-G	-1558 / 0	-91.8	-91.8	0.17 (1)	5.12	
G-H	-1783 / 0	-91.8	-91.8	0.59 (1)	4.35	
H-I	-1428 / 0	-91.8	-91.8	0.43 (1)	4.93	
P-B	-1808 / 0	0.0	0.0	0.18 (1)	6.23	
J-I	-1718 / 0	0.0	0.0	0.27 (1)	6.35	
P-O	0 / 0	-18.5	-18.5	0.21 (4)	10.00	
O-N	0 / 2306	-18.5	-18.5	0.47 (1)	10.00	
N-M	0 / 1740	-18.5	-18.5	0.39 (1)	10.00	
M-L	0 / 1740	-18.5	-18.5	0.39 (1)	10.00	
L-K	0 / 1320	-18.5	-18.5	0.32 (1)	10.00	
K-J	0 / 0	-18.5	-18.5	0.12 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O-C	-142 / 66	0.05 (1)
C-N	-680 / 0	0.31 (1)
N-E	0 / 497	0.11 (1)
N-F	-6 / 33	0.01 (4)
F-L	-438 / 0	0.22 (1)
L-G	0 / 387	0.09 (1)
L-H	0 / 275	0.06 (1)
K-H	-907 / 0	0.29 (1)
B-O	0 / 2323	0.52 (1)
K-I	0 / 1663	0.37 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.05")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.05")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.77/1.00 (B-C:1) , BC=0.47/1.00 (N-O:1) , WB=0.52/1.00 (B-O:1) , SSI=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL: = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.69 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T64	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:49 2020 Page 1
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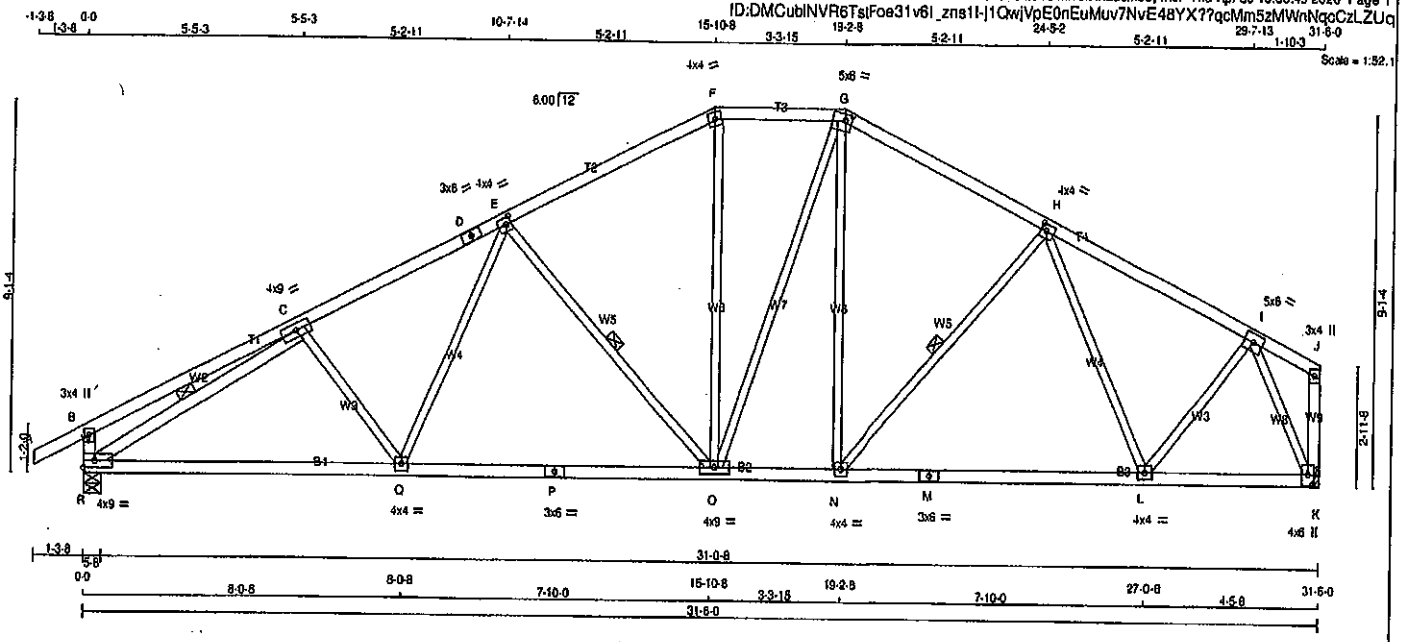

TOTAL WEIGHT = 2 X 142 = 285 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	9.0		
D	TS-I	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TTW-m	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.00	2.00
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMWW-I	MT20	5.0	6.0		
J	TMV+p	MT20	3.0	4.0		
K	BMVW1+p	MT20	4.0	6.0		
L, N, Q						
L	BMWW-t	MT20	4.0	4.0		
M	BS-I	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BS-t	MT20	3.0	6.0		
R	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008068

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	1861	0	1861	0	0	5-8	5-8
K	1736	0	1736	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1314	875 / 0	0 / 0	0 / 0	0 / 0	438 / 0	0 / 0
K	1228	808 / 0	0 / 0	0 / 0	0 / 0	422 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-O, H-N, C-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 20	-91.8	-91.8	0.33 (1)	10.00
C- D	-2389 / 0	-91.8	-91.8	0.36 (1)	4.14
D- E	-2389 / 0	-91.8	-91.8	0.36 (1)	4.14
E- F	-1758 / 0	-91.8	-91.8	0.33 (1)	4.71
F- G	-1557 / 0	-91.8	-91.8	0.16 (1)	5.14
G- H	-1684 / 0	-91.8	-91.8	0.34 (1)	4.76
H- I	-1501 / 0	-91.8	-91.8	0.33 (1)	5.01
I- J	0 / 33	-91.8	-91.8	0.18 (1)	10.00
R- B	-324 / 0	0.0	0.0	0.03 (1)	7.81
K- J	-6 / 0	0.0	0.0	0.00 (1)	7.81
R- Q	0 / 2233	-18.5	-18.5	0.52 (1)	10.00
Q- P	0 / 2014	-18.5	-18.5	0.48 (1)	10.00
P- O	0 / 2014	-18.5	-18.5	0.48 (1)	10.00
O- N	0 / 1487	-18.5	-18.5	0.33 (1)	10.00
N- M	0 / 1575	-18.5	-18.5	0.36 (1)	10.00
M- L	0 / 1575	-18.5	-18.5	0.36 (1)	10.00
L- K	0 / 849	-18.5	-18.5	0.27 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- Q	-156 / 21	0.05 (1)
Q- E	0 / 318	0.07 (1)
E- O	-708 / 0	0.32 (1)
O- F	0 / 434	0.10 (1)
O- G	0 / 206	0.05 (1)
N- G	0 / 211	0.05 (1)
N- H	-140 / 0	0.06 (1)
H- L	-570 / 0	0.48 (1)
L- I	0 / 829	0.19 (1)
R- C	-2678 / 0	0.54 (1)
I- K	-1906 / 0	0.49 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.05")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.05")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.38/1.00 (C-E:1) , BC=0.52/1.00 (O-R:1) , WB=0.54/1.00 (C-R:1) , SSI=0.20/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.73 (P) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T65	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:50 2020 Page 1

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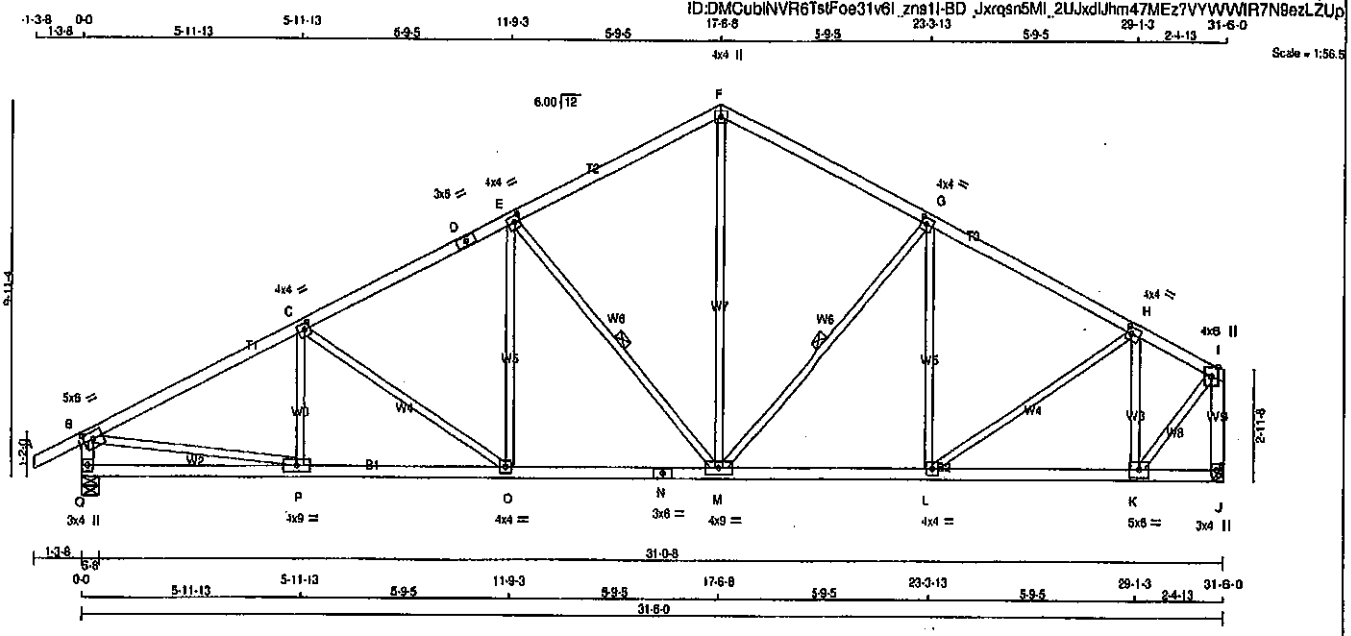

TOTAL WEIGHT = 3 X 138 = 413 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0	2.25	2.75
C, E, G						
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
F	TTW+p	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMWW+p	MT20	4.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	9.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. GALVES
100090247
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008069

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	1861	0	1861	0	0	5-8	5-8
J	1736	0	1736	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1314	875 / 0	0 / 0	0 / 0	0 / 0	438 / 0	0 / 0
J	1226	806 / 0	0 / 0	0 / 0	0 / 0	422 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M, G-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
B-C	-2522 / 0	-91.8	-91.8	0.52 (1)	3.88	
C-D	-2179 / 0	-91.8	-91.8	0.41 (1)	4.24	
D-E	-2179 / 0	-91.8	-91.8	0.41 (1)	4.24	
E-F	-1596 / 0	-91.8	-91.8	0.40 (1)	4.60	
F-G	-1599 / 0	-91.8	-91.8	0.41 (1)	4.79	
G-H	-1794 / 0	-91.8	-91.8	0.41 (1)	4.58	
H-I	-1145 / 0	-91.8	-91.8	0.18 (1)	5.74	
Q-B	-1813 / 0	0.0	0.0	0.18 (1)	6.21	
J-I	-1726 / 0	0.0	0.0	0.27 (1)	6.33	
Q-P	0 / 0	-18.5	-18.5	0.14 (4)	10.00	
P-O	0 / 2276	-18.5	-18.5	0.43 (1)	10.00	
O-N	0 / 1949	-18.5	-18.5	0.39 (1)	10.00	
N-M	0 / 1949	-18.5	-18.5	0.39 (1)	10.00	
M-L	0 / 1616	-18.5	-18.5	0.33 (1)	10.00	
L-K	0 / 1051	-18.5	-18.5	0.23 (1)	10.00	
K-J	0 / 0	-18.5	-18.5	0.09 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-C	-209 / 26	0.06 (1)
C-O	-393 / 0	0.36 (1)
O-E	0 / 322	0.07 (1)
E-M	-832 / 0	0.45 (1)
M-F	0 / 985	0.22 (1)
M-G	-320 / 0	0.17 (1)
L-G	-269 / 0	0.23 (1)
L-H	0 / 678	0.15 (1)
K-H	-1072 / 0	0.28 (1)
B-P	0 / 2299	0.52 (1)
K-I	0 / 1565	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.05")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.05")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.52/1.00 (B-C:1) , BC=0.43/1.00 (O-P:1) , WB=0.52/1.00 (B-P:1) , SSb=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.67 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T66	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:52 2020 Page 1
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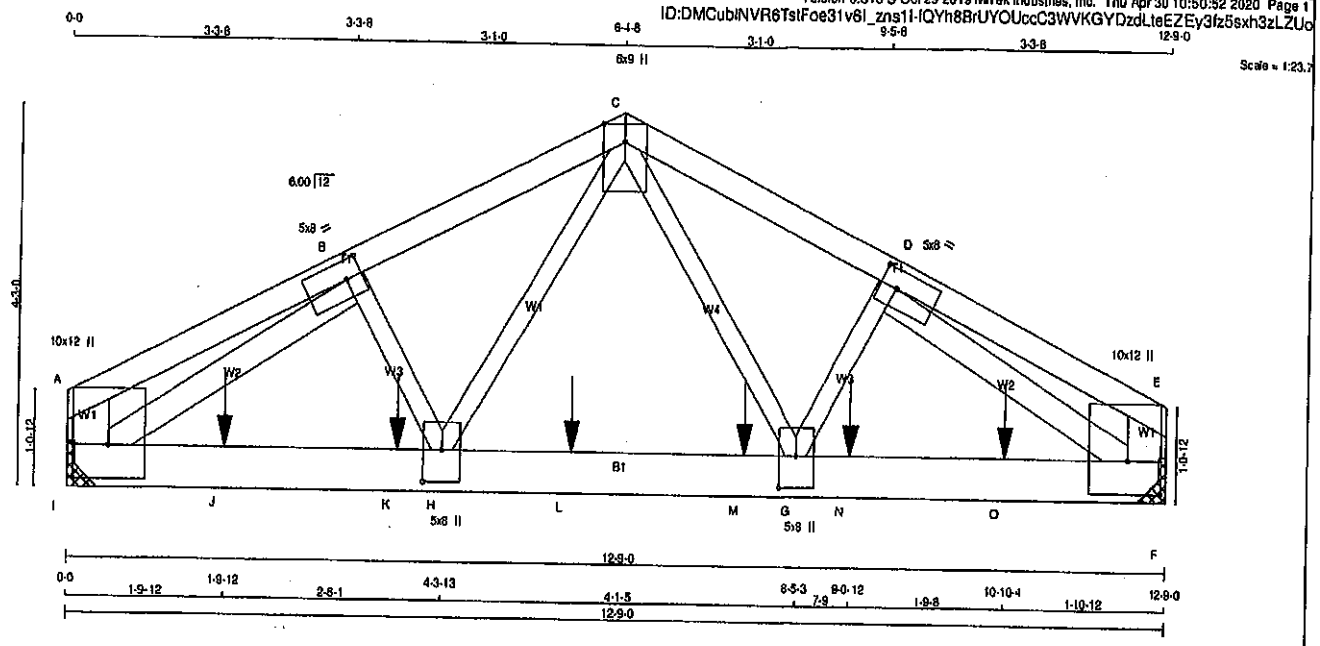

TOTAL WEIGHT = 2 X 59 = 118 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
I - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
I - F	2x6	DRY	1650F 1.5E	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - B	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-E	1	12	TOP
I-A	2	12	TOP
F-E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
I-F	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

LICENSED PROFESSIONAL ENGINEER
04/20/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008070

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	5734	0	5734	0	0	MECHANICAL	
F	5980	0	5980	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I, F. MINIMUM BEARING LENGTH AT JOINT I = 4-0, JOINT F = 4-0.

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	4050	2686 / 0	0 / 0	0 / 0	0 / 0	1364 / 0	0 / 0
F	4224	2802 / 0	0 / 0	0 / 0	0 / 0	1422 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-3 / 0	-91.8	-91.8	0.09 (1)	10.00	
B-C	-7793 / 0	-91.8	-91.8	0.31 (1)	3.31	
C-D	-8218 / 0	-91.8	-91.8	0.34 (1)	3.19	
D-E	-4 / 0	-91.8	-91.8	0.10 (1)	10.00	
I-A	-150 / 0	0.0	0.0	0.01 (1)	7.81	
F-E	-154 / 0	0.0	0.0	0.01 (1)	7.81	
I-J	0 / 6530	-18.5	-18.5	0.69 (1)	10.00	
J-K	0 / 6530	-18.5	-18.5	0.69 (1)	10.00	
K-H	0 / 6530	-18.5	-18.5	0.69 (1)	10.00	
H-L	0 / 5336	-18.5	-18.5	0.57 (1)	10.00	
L-M	0 / 5336	-18.5	-18.5	0.57 (1)	10.00	
M-G	0 / 5336	-18.5	-18.5	0.57 (1)	10.00	
G-N	0 / 6878	-18.5	-18.5	0.72 (1)	10.00	
N-O	0 / 6878	-18.5	-18.5	0.72 (1)	10.00	
O-F	0 / 6878	-18.5	-18.5	0.72 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-G	0 / 4292	0.53 (1)
G-D	0 / 1052	0.13 (1)
H-C	0 / 3477	0.43 (1)
B-H	0 / 978	0.12 (1)
I-B	-8011 / 0	0.72 (1)
D-F	-8439 / 0	0.75 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
J	1-9-12	-1718	-1718	---	BACK	VERT	TOTAL	---	C1
K	3-9-12	-1718	-1718	---	BACK	VERT	TOTAL	---	C1
L	5-9-12	-1718	-1718	---	BACK	VERT	TOTAL	---	C1
M	7-9-12	-1718	-1718	---	BACK	VERT	TOTAL	---	C1
N	9-0-12	-1718	-1718	---	BACK	VERT	TOTAL	---	C1
O	10-10-4	-1718	-1718	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.34/1.00 (C-D:1) , BC=0.72/1.00 (F-G:1) , WB=0.75/1.00 (D-F:1) , SSI=0.88/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS SEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.78 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408317	T66	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:52 2020 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A, E, F, I						
A						
A	TM8MVW1*+p	MT20	10.0	12.0	4.50	4.75
B	TMWW-t	MT20	5.0	8.0	2.50	2.25
C	TTWW+p	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	5.0	8.0	2.50	2.25
E	TM8MVW1*+p	MT20	10.0	12.0	4.50	4.75
G	BMWW+t	MT20	5.0	8.0	4.25	2.50
H	BMWW+t	MT20	5.0	8.0	4.25	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
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100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008070 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T67S	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:53 2020 Page 1
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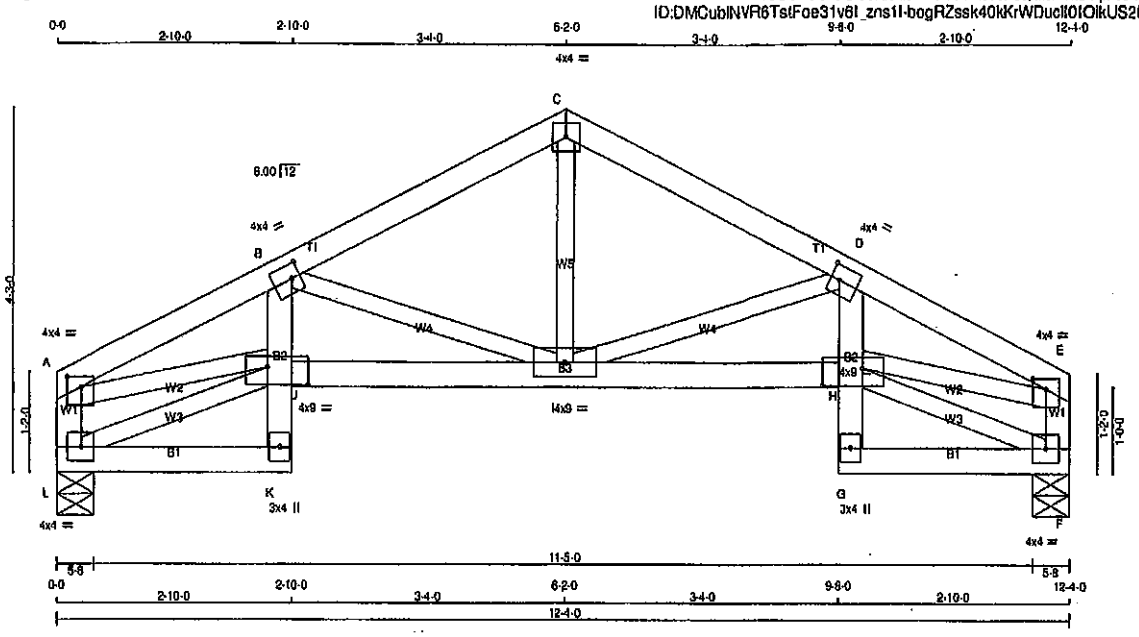

TOTAL WEIGHT = 52 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW-p	MT20	4.0	4.0	1.50	2.00
B	TMW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-p	MT20	4.0	4.0		
D	TMW-t	MT20	4.0	4.0	2.00	1.25
E	TMW-p	MT20	4.0	4.0	1.50	2.00
F	BMWW1-t	MT20	4.0	4.0		
G	BMV+p	MT20	3.0	4.0		
H	BVMWW-t	MT20	4.0	9.0	2.50	6.00
I	BMWWW-t	MT20	4.0	9.0		
J	BVMWW-t	MT20	4.0	9.0	2.50	6.00
K	BMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	680	0	680	0	0	5-8	5-8
F	680	0	680	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
L	481	316 / 0	0 / 0	0 / 0	0 / 0	165 / 0	0 / 0
F	481	316 / 0	0 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-1320 / 0	-91.8	-91.8	0.09 (1)	5.55
B-C	-818 / 0	-91.8	-91.8	0.14 (1)	6.25
C-D	-818 / 0	-91.8	-91.8	0.14 (1)	6.25
D-E	-1320 / 0	-91.8	-91.8	0.09 (1)	5.55
L-A	-629 / 0	0.0	0.0	0.06 (1)	7.81
F-E	-629 / 0	0.0	0.0	0.06 (1)	7.81
L-K	0 / 69	-18.5	-18.5	0.04 (4)	10.00
K-J	0 / 28	0.0	0.0	0.10 (1)	10.00
J-B	0 / 87	0.0	0.0	0.11 (1)	10.00
J-I	0 / 1249	-18.5	-18.5	0.24 (1)	10.00
I-H	0 / 1249	-18.5	-18.5	0.24 (1)	10.00
G-H	0 / 28	0.0	0.0	0.10 (1)	10.00
H-D	0 / 87	0.0	0.0	0.11 (1)	10.00
G-F	0 / 69	-18.5	-18.5	0.04 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-C	0 / 443	0.10 (1)
I-D	-558 / 0	0.13 (1)
B-I	-558 / 0	0.13 (1)
L-J	-74 / 0	0.01 (1)
A-J	0 / 1193	0.27 (1)
H-F	-74 / 0	0.01 (1)
H-E	0 / 1193	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.14/1.00 (B-C:1) , BC=0.24/1.00 (I-J:1) , WB=0.27/1.00 (A-J:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (E) (INPUT = 0.90)
JSI METAL= 0.40 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008071

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408317	T67SX	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:54 2020 Page 1
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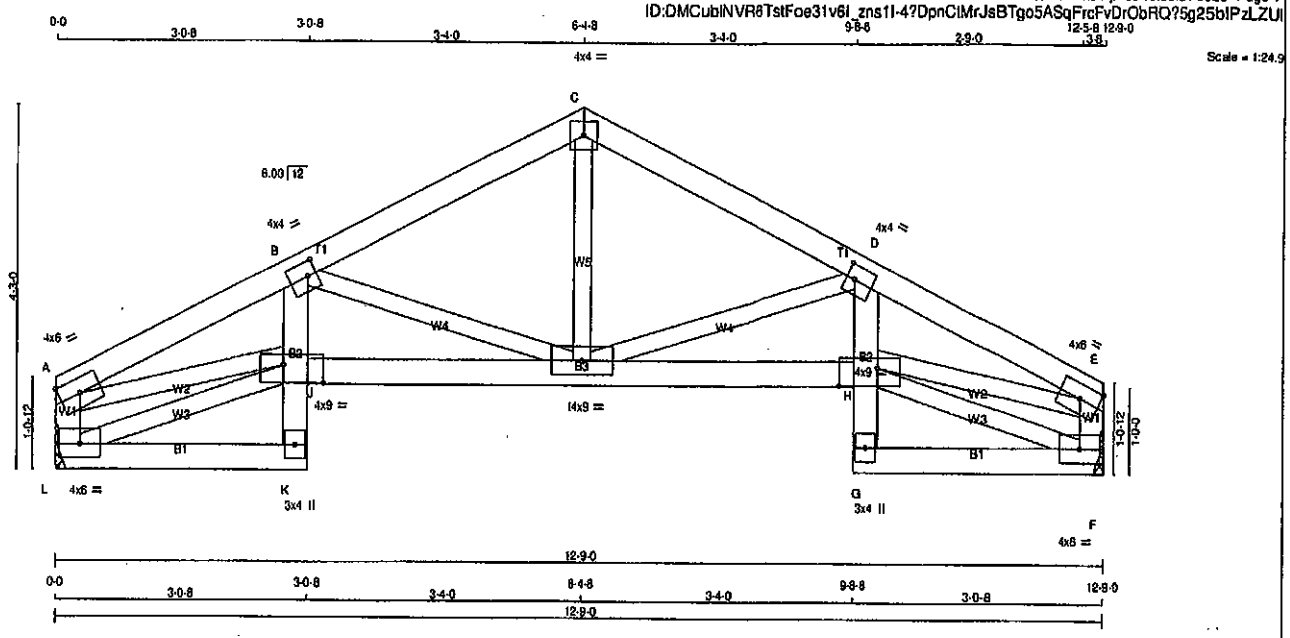

TOTAL WEIGHT = 53 lb
[M|F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	4.0	6.0		Edge
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-p	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TMWW-t	MT20	4.0	6.0		Edge
F	BMVW1-t	MT20	4.0	6.0		
G	BMV+p	MT20	3.0	4.0		
H	BVMWW-l	MT20	4.0	9.0	2.50	5.75
I	BMWWW-t	MT20	4.0	9.0		
J	BVMWW-l	MT20	4.0	9.0	2.50	5.75
K	BMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	703	0	703	0	0	MECHANICAL	
F	703	0	703	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L, F. MINIMUM BEARING LENGTH AT JOINT L = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	497	326 / 0	0 / 0	0 / 0	0 / 0	171 / 0	0 / 0
F	497	326 / 0	0 / 0	0 / 0	0 / 0	171 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1445 / 0	-91.8	-91.8	0.10 (1)		5.35
B- C	-871 / 0	-91.8	-91.8	0.14 (1)		6.25
C- D	-871 / 0	-91.8	-91.8	0.14 (1)		6.25
D- E	-1444 / 0	-91.8	-91.8	0.10 (1)		5.35
L- A	-648 / 0	0.0	0.0	0.07 (1)		7.81
F- E	-648 / 0	0.0	0.0	0.07 (1)		7.81
L- K	0 / 81	-18.5	-18.5	0.05 (4)		10.00
K- J	0 / 30	0.0	0.0	0.11 (1)		10.00
J- B	0 / 135	0.0	0.0	0.13 (1)		10.00
J- I	0 / 1370	-18.5	-18.5	0.25 (1)		10.00
I- H	0 / 1369	-18.5	-18.5	0.25 (1)		10.00
G- H	0 / 30	0.0	0.0	0.11 (1)		10.00
H- D	0 / 134	0.0	0.0	0.13 (1)		10.00
G- F	0 / 81	-18.5	-18.5	0.05 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- I	-634 / 0	0.15 (1)
I- C	0 / 493	0.11 (1)
I- D	-633 / 0	0.15 (1)
L- J	-86 / 0	0.02 (1)
A- J	0 / 1307	0.29 (1)
H- F	-86 / 0	0.02 (1)
H- E	0 / 1306	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.14/1.00 (C-D:1) , BC=0.25/1.00 (I-J:1) , WB=0.29/1.00 (A-J:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1997	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (A) (INPUT = 0.90)
JSI METAL= 0.38 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G/ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008072

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T68	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:54 2020 Page 1
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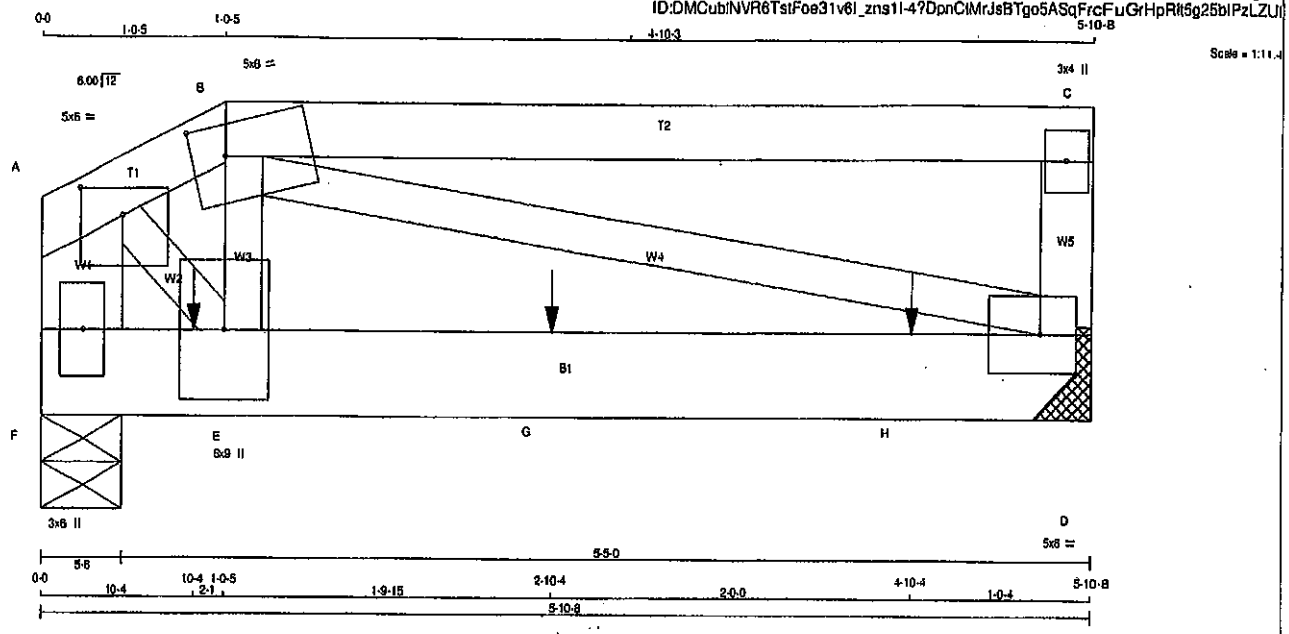

TOTAL WEIGHT = 2 X 25 = 49 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	TOP
B- C	1	12	TOP
C- D	1	12	TOP
F- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	6	SIDE(427.3)
WEBS : (0.122"X3") SPIRAL NAILS			
E- B	1	2	SIDE(95.7)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	1.75	3.00

LICENSED PROFESSIONAL ENGINEER
04/20/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008073

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	3340	0	3340	0	0	MECHANICAL	
F	3319	0	3319	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	2356	1580 / 0	0 / 0	0 / 0	0 / 0	775 / 0	0 / 0
F	2341	1570 / 0	0 / 0	0 / 0	0 / 0	771 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	-4326 / 0	-91.8	-91.8	0.07 (1)	4.57
B- C	0 / 0	-91.8	-91.8	0.20 (1)	10.00
D- C	-222 / 0	0.0	0.0	0.01 (1)	7.81
F- A	-4647 / 0	0.0	0.0	0.16 (1)	6.70
F- E	0 / 0	-18.5	-18.5	0.39 (1)	10.00
E- G	0 / 4080	-18.5	-18.5	0.69 (1)	10.00
G- H	0 / 4080	-18.5	-18.5	0.69 (1)	10.00
H- D	0 / 4080	-18.5	-18.5	0.69 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	0 / 2791	0.35 (1)
B- D	-4223 / 0	0.81 (1)
A- E	0 / 4673	0.58 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	10-4	-2005	-2005	---	FRONT	VERT	TOTAL	---	C1
G	2-10-4	-2003	-2003	---	FRONT	VERT	TOTAL	---	C1
H	4-10-4	-2003	-2003	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/547 (0.13")

CSI: TC=0.20/1.00 (B-C:1) , BC=0.69/1.00 (D-E:1) , WB=0.81/1.00 (B-D:1) , SSI=0.69/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.61 (E) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408317	T68	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TTWW-m	MT20	5.0	8.0	2.00	2.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0	2.50	2.50
E	BMWW+l	MT20	6.0	9.0		
F	BMV1+p	MT20	3.0	6.0		

LICENSED PROFESSIONAL ENGINEER
04/20/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008073

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T69	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:56 2020 Page 1
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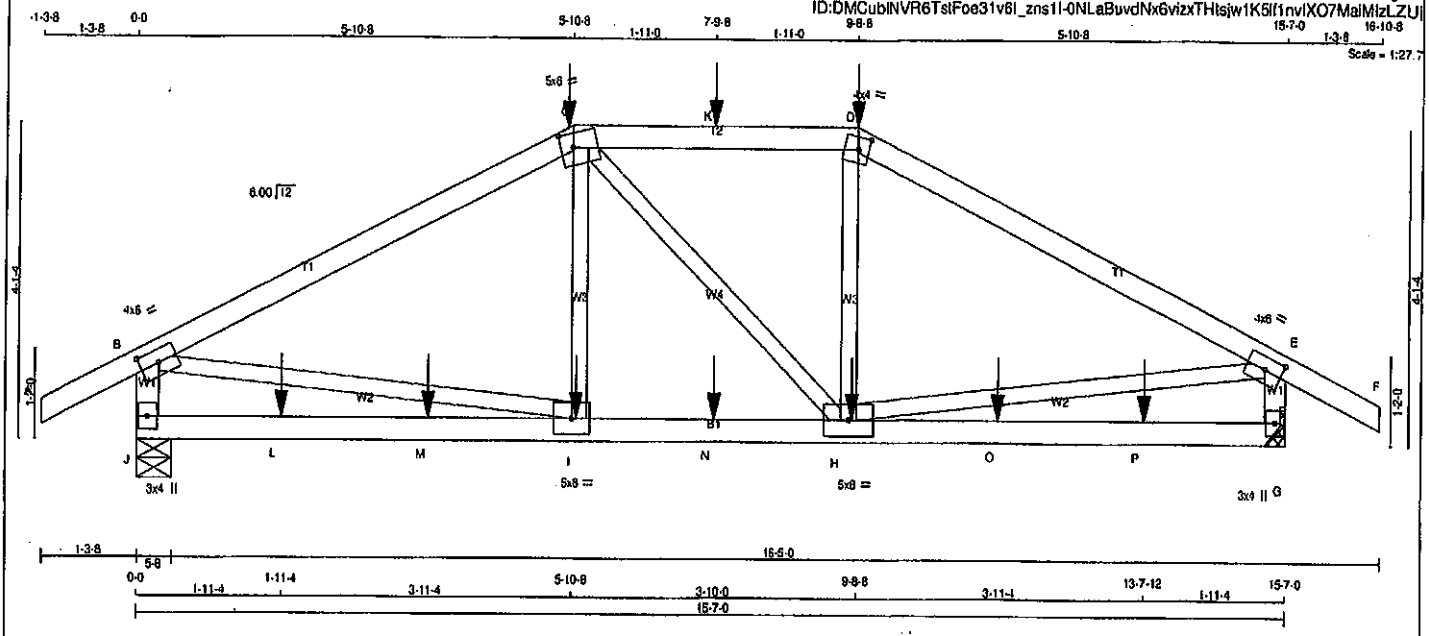

TOTAL WEIGHT = 61 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	6.0	2.00	3.00
C	TTWW-m	MT20	5.0	8.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0	2.00	1.75
E	TMVW-t	MT20	4.0	6.0	2.00	3.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1544	0	1544	0	0	5-8	5-8
G	1543	0	1543	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1091	722 / 0	0 / 0	0 / 0	0 / 0	369 / 0	0 / 0
G	1090	722 / 0	0 / 0	0 / 0	0 / 0	369 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.13 (1)	10.00	
B-C	-1673 / 0	-91.8	-91.8	0.78 (1)	3.86	
C-K	-1674 / 0	-91.8	-91.8	0.47 (1)	4.49	
K-D	-1674 / 0	-91.8	-91.8	0.47 (1)	4.49	
D-E	-1676 / 0	-91.8	-91.8	0.78 (1)	3.85	
E-F	0 / 28	-91.8	-91.8	0.13 (1)	10.00	
J-B	-1474 / 0	0.0	0.0	0.18 (1)	6.71	
G-E	-1473 / 0	0.0	0.0	0.16 (1)	6.71	
J-L	0 / 0	-18.5	-18.5	0.28 (4)	10.00	
L-M	0 / 0	-18.5	-18.5	0.28 (4)	10.00	
M-I	0 / 0	-18.5	-18.5	0.28 (4)	10.00	
I-N	0 / 1672	-18.5	-18.5	0.40 (1)	10.00	
N-H	0 / 1672	-18.5	-18.5	0.40 (1)	10.00	
H-O	0 / 0	-18.5	-18.5	0.28 (4)	10.00	
O-P	0 / 0	-18.5	-18.5	0.28 (4)	10.00	
P-G	0 / 0	-18.5	-18.5	0.28 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-C	-76 / 105	0.04 (4)
C-H	0 / 3	0.00 (4)
H-D	-73 / 107	0.04 (4)
B-I	0 / 1691	0.42 (1)
H-E	0 / 1694	0.42 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-414	-414	---	FRONT	VERT	TOTAL	---	C1
D	9-8-8	-414	-414	---	FRONT	VERT	TOTAL	---	C1
H	9-7-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
I	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
K	7-9-8	-110	-110	---	FRONT	VERT	TOTAL	---	C1
L	1-11-4	-25	-26	---	FRONT	VERT	TOTAL	---	C1
M	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
N	7-9-8	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	11-7-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	13-7-12	-25	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.78/1.00 (D-E:1) , BC=0.40/1.00 (H-I:1) , WB=0.42/1.00 (E-H:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.51 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008074

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T70	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:57 2020 Page 1
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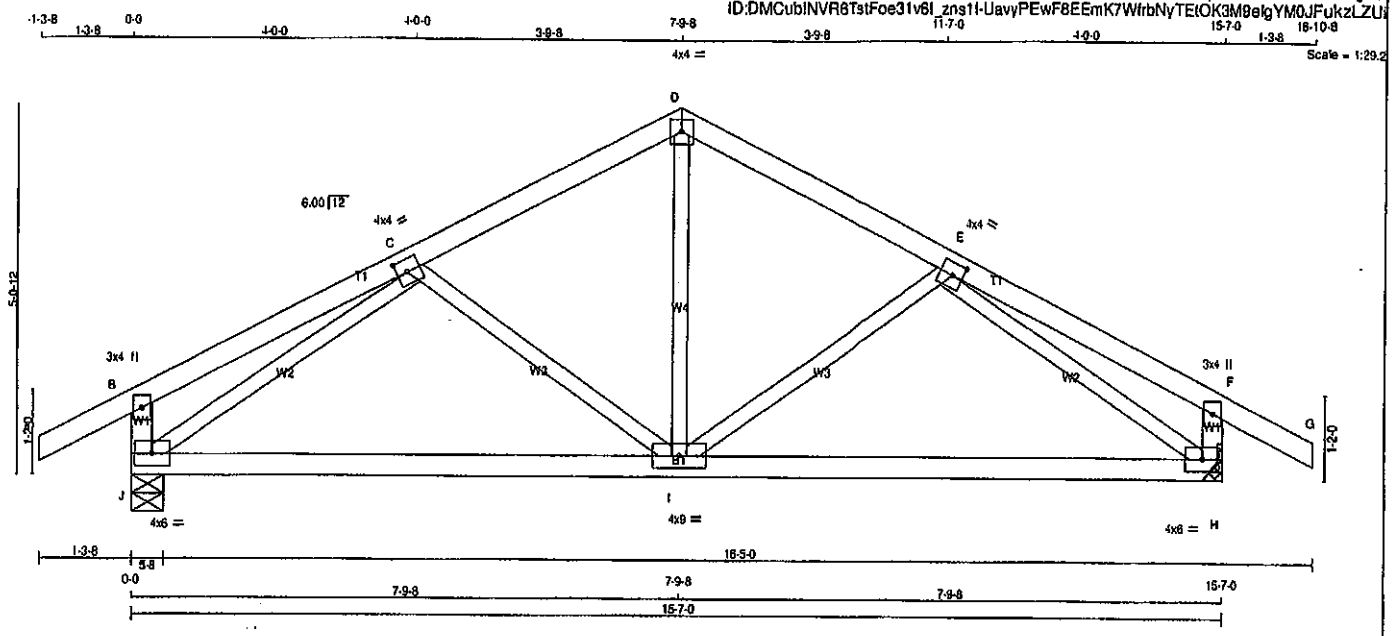

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (tables in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-I	MT20	4.0	4.0	2.00	1.75
D	TTW-p	MT20	4.0	4.0		
E	TMWW-I	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-I	MT20	4.0	6.0		
I	BMWWW-I	MT20	4.0	9.0		
J	BMVW1-I	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	984	0	984	0	0	5-8	5-8
H	984	0	984	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	693	468 / 0	0 / 0	0 / 0	0 / 0	225 / 0	0 / 0
H	693	468 / 0	0 / 0	0 / 0	0 / 0	225 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LCI	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
B- C	0 / 18	-91.8	-91.8	0.21 (1)	10.00	
C- D	-835 / 0	-91.8	-91.8	0.17 (1)	6.25	
D- E	-835 / 0	-91.8	-91.8	0.17 (1)	6.25	
E- F	0 / 18	-91.8	-91.8	0.21 (1)	10.00	
F- G	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
J- B	-264 / 0	0.0	0.0	0.03 (1)	7.81	
H- F	-264 / 0	0.0	0.0	0.03 (1)	7.81	
J- I	0 / 923	-18.5	-18.5	0.39 (4)	10.00	
I- H	0 / 923	-18.5	-18.5	0.39 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 455	0.10 (1)
I- E	-237 / 0	0.09 (1)
C- I	-237 / 0	0.09 (1)
J- C	-1137 / 0	0.43 (1)
E- H	-1137 / 0	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.21/1.00 (E-F:1) , BC=0.39/1.00 (I-J:4) , WB=0.43/1.00 (C-J:1) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (E) (INPUT = 0.90)
JSI METAL= 0.38 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008075

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T71	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:58 2020 Page 1
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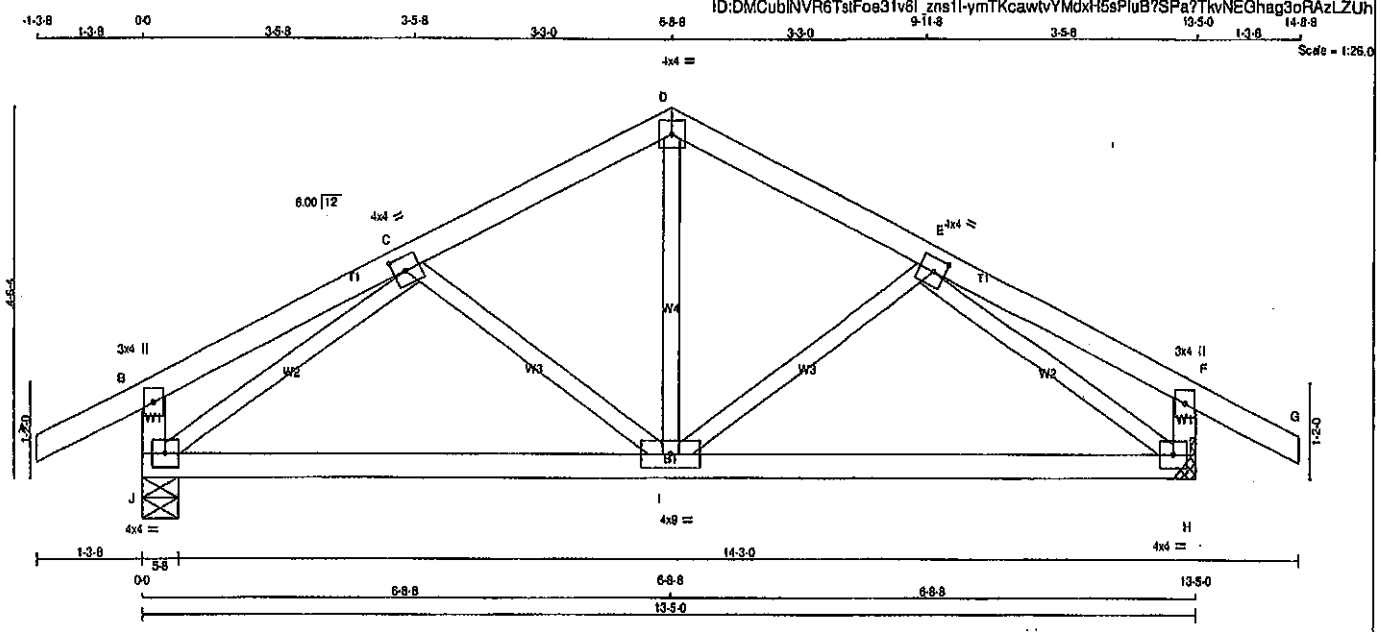

TOTAL WEIGHT = 2 X 53 = 107 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-p	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	664	0	664	0	0	5-8	5-8
H	664	0	664	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	609	413 / 0	0 / 0	0 / 0	0 / 0	196 / 0	0 / 0
H	609	413 / 0	0 / 0	0 / 0	0 / 0	196 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.12 (1)		10.00
B-C	0 / 15	-91.8	-91.8	0.15 (1)		10.00
C-D	-698 / 0	-91.8	-91.8	0.12 (1)		6.25
D-E	-698 / 0	-91.8	-91.8	0.12 (1)		6.25
E-F	0 / 15	-91.8	-91.8	0.15 (1)		10.00
F-G	0 / 28	-91.8	-91.8	0.12 (1)		10.00
J-B	-246 / 0	0.0	0.0	0.02 (1)		7.81
H-F	-246 / 0	0.0	0.0	0.02 (1)		7.81
J-I	0 / 755	-18.5	-18.5	0.29 (4)		10.00
I-H	0 / 755	-18.5	-18.5	0.29 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-D	0 / 371	0.08 (1)
I-E	-181 / 0	0.05 (1)
C-I	-181 / 0	0.05 (1)
J-C	-946 / 0	0.28 (1)
E-H	-946 / 0	0.28 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.15/1.00 (E-F:1) , BC=0.29/1.00 (H-I:4) , WB=0.28/1.00 (E-H:1) , SSI=0.13/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (J) (INPUT = 0.90)
JSI METAL= 0.32 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008076

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	T72S	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:59 2020 Page 1
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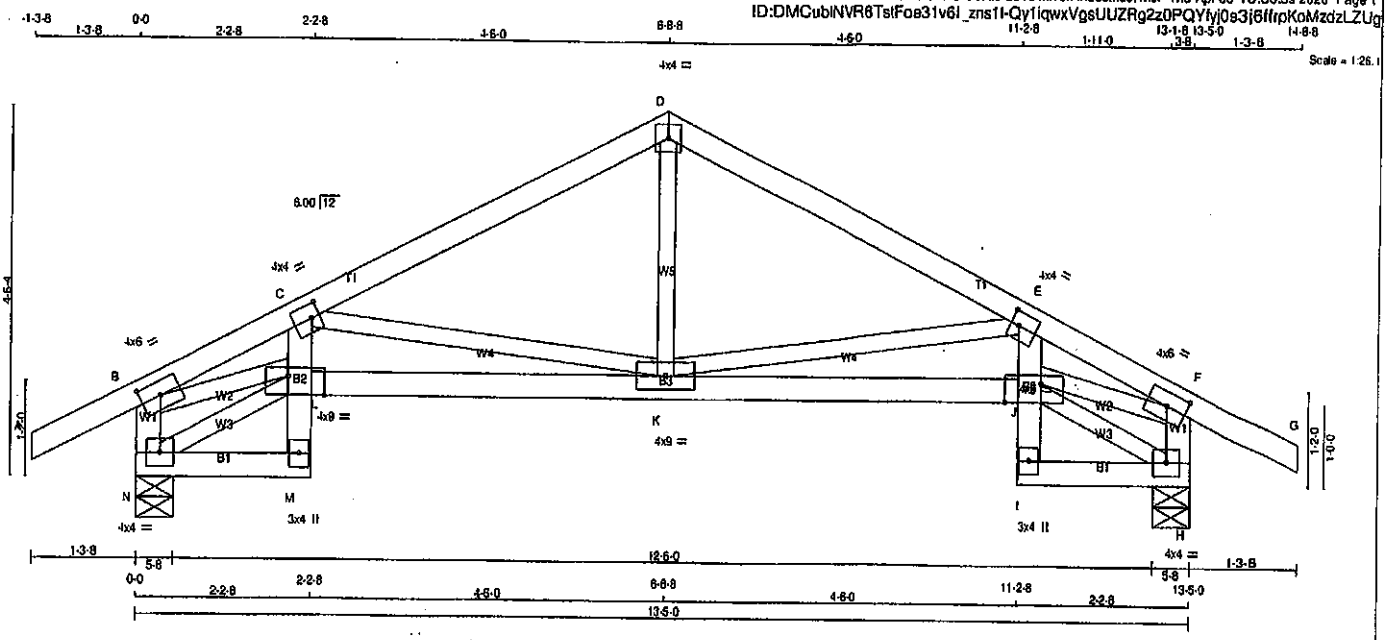

TOTAL WEIGHT = 2 X 57 = 114 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-p	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	TMVW-t	MT20	4.0	6.0	2.00	3.00
H	BMVW1-t	MT20	4.0	4.0		
I	BMV-p	MT20	3.0	4.0		
J	BVMWW-l	MT20	4.0	9.0	2.75	5.50
K	BMWWW-t	MT20	4.0	9.0		
L	BVMWW-l	MT20	4.0	9.0	2.75	5.50
M	BMV-p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	864	0	864	0	0	5-8	5-8
H	864	0	864	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	609	413 / 0	0 / 0	0 / 0	0 / 0	196 / 0	0 / 0
H	609	413 / 0	0 / 0	0 / 0	0 / 0	196 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B-C	-1616 / 0	-91.8	-91.8	0.15 (1)	5.07
C-D	-892 / 0	-91.8	-91.8	0.26 (1)	6.18
D-E	-892 / 0	-91.8	-91.8	0.26 (1)	6.18
E-F	-1616 / 0	-91.8	-91.8	0.15 (1)	5.07
F-G	0 / 28	-91.8	-91.8	0.12 (1)	10.00
N-B	-795 / 0	0.0	0.0	0.08 (1)	7.81
H-F	-795 / 0	0.0	0.0	0.08 (1)	7.81
N-M	0 / 104	-18.5	-18.5	0.03 (4)	10.00
M-L	0 / 21	0.0	0.0	0.14 (1)	10.00
L-C	0 / 102	0.0	0.0	0.16 (1)	10.00
L-K	0 / 1585	-18.5	-18.5	0.32 (1)	10.00
K-J	0 / 1585	-18.5	-18.5	0.32 (1)	10.00
I-J	0 / 21	0.0	0.0	0.14 (1)	10.00
J-E	0 / 102	0.0	0.0	0.16 (1)	10.00
I-H	0 / 104	-18.5	-18.5	0.03 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-D	0 / 412	0.09 (1)
K-E	-812 / 0	0.30 (1)
C-K	-812 / 0	0.30 (1)
N-L	-116 / 0	0.02 (1)
B-L	0 / 1472	0.33 (1)
J-H	-116 / 0	0.02 (1)
J-F	0 / 1472	0.33 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.26/1.00 (D-E:1) , BC=0.32/1.00 (J-K:1) , WB=0.33/1.00 (F-J:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008077

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408316	V30	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:39 2020 Page 1

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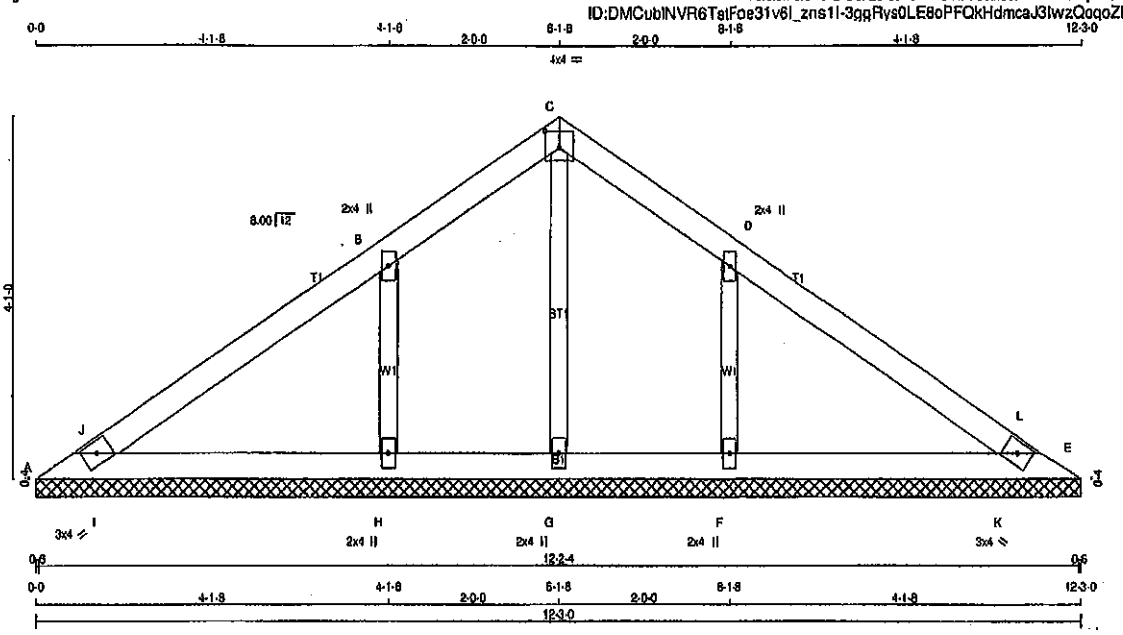

TOTAL WEIGHT = 36 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW+w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMW+w	MT20	2.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F, G, H						
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	102	0	102	0	0	12-3-0 (12-2-4	2-3-0
E	102	0	102	0	0	12-3-0 (12-2-4	2-3-0
G	252	0	252	0	0	12-3-0 (12-2-4	2-3-0
H	444	0	444	0	0	12-3-0 (12-2-4	2-3-0
F	444	0	444	0	0	12-3-0 (12-2-4	2-3-0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	72	48 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0
E	72	48 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0
G	179	115 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
H	314	206 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0
F	314	206 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-J	0 / 150	-91.8	-91.8	0.04 (4)	10.00
J-B	0 / 183	-91.8	-91.8	0.17 (1)	10.00
B-C	0 / 151	-91.8	-91.8	0.16 (1)	10.00
C-D	0 / 151	-91.8	-91.8	0.16 (1)	10.00
D-L	0 / 183	-91.8	-91.8	0.17 (1)	10.00
L-E	0 / 150	-91.8	-91.8	0.04 (4)	10.00
A-I	-156 / 0	-18.5	-18.5	0.10 (1)	6.25
I-H	-137 / 0	-18.5	-18.5	0.10 (1)	6.25
H-G	-148 / 0	-18.5	-18.5	0.08 (1)	6.25
G-F	-148 / 0	-18.5	-18.5	0.08 (1)	6.25
F-K	-137 / 0	-18.5	-18.5	0.10 (1)	6.25
K-E	-156 / 0	-18.5	-18.5	0.10 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-C	-291 / 0	0.07 (1)
H-B	-318 / 0	0.05 (1)
F-D	-318 / 0	0.05 (1)
I-J	-119 / 3	0.00 (1)
K-L	-119 / 3	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (B-J:1) , BC=0.10/1.00 (H-I:1) , WB=0.07/1.00 (C-G:1) , SSI=0.12/1.00 (B-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (C) (INPUT = 0.90)
JSI METAL= 0.16 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008048

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408316	V31	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:40 2020 Page 1
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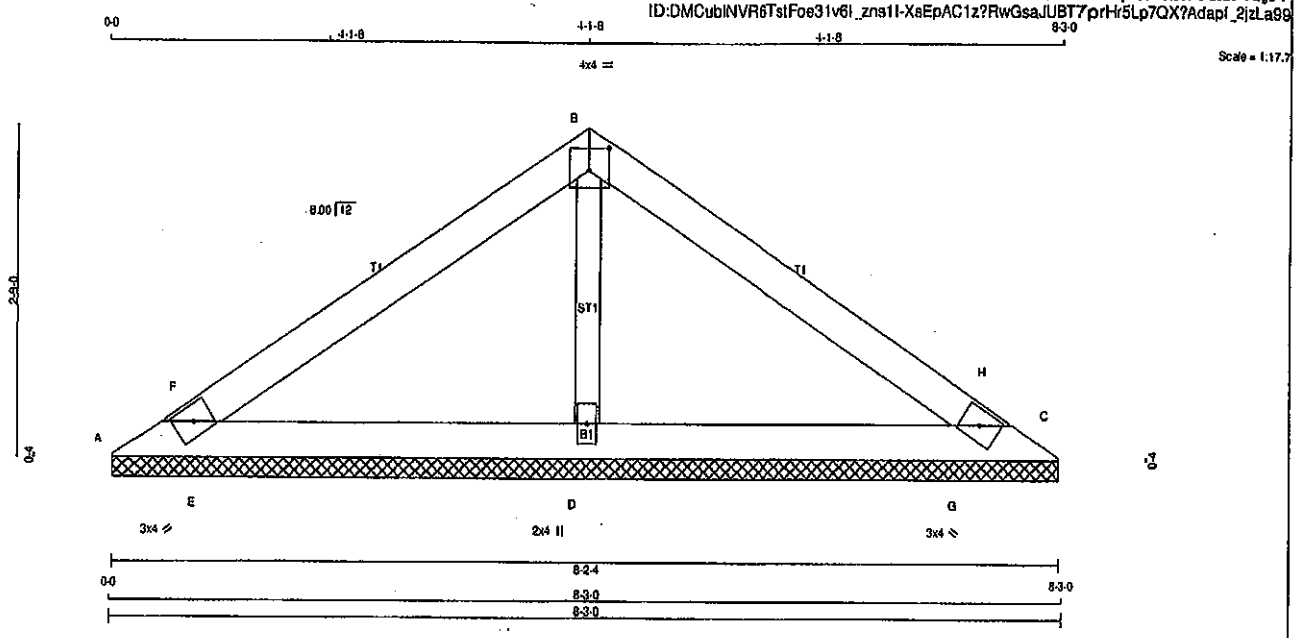

TOTAL WEIGHT = 21 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBMI-h	MT20	3.0	4.0		
B	TTW-p	MT20	4.0	4.0	2.25	2.00
C	TBMI-h	MT20	3.0	4.0		
D	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	61	0	61	0	0	8-2-4	8-2-4
C	61	0	61	0	0	8-2-4	8-2-4
D	780	0	780	0	0	8-2-4	8-2-4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	43	28 / 0	0 / 0	0 / 0	0 / 0	15 · 0	0 / 0
C	43	28 / 0	0 / 0	0 / 0	0 / 0	15 · 0	0 / 0
D	552	362 / 0	0 / 0	0 / 0	0 / 0	190 · 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- F	0 / 260	-91.8	-91.8	0.09 (1)	10.00
F- B	0 / 279	-91.8	-91.8	0.19 (1)	10.00
B- H	0 / 279	-91.8	-91.8	0.19 (1)	10.00
H- C	0 / 260	-91.8	-91.8	0.09 (1)	10.00
A- E	-264 / 0	-18.5	-18.5	0.14 (1)	6.25
E- D	-237 / 0	-18.5	-18.5	0.14 (1)	6.25
D- G	-237 / 0	-18.5	-18.5	0.14 (1)	6.25
G- C	-264 / 0	-18.5	-18.5	0.14 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- B	-598 / 0	0.10 (1)
E- F	-195 / 0	0.00 (1)
G- H	-195 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2010, NBCO 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19/1.00 (B-F:1) , BC=0.14/1.00 (D-E:1) , WB=0.10/1.00 (B-D:1) , SSI=0.10/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008049

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	PB1	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:23 2020 Page 1
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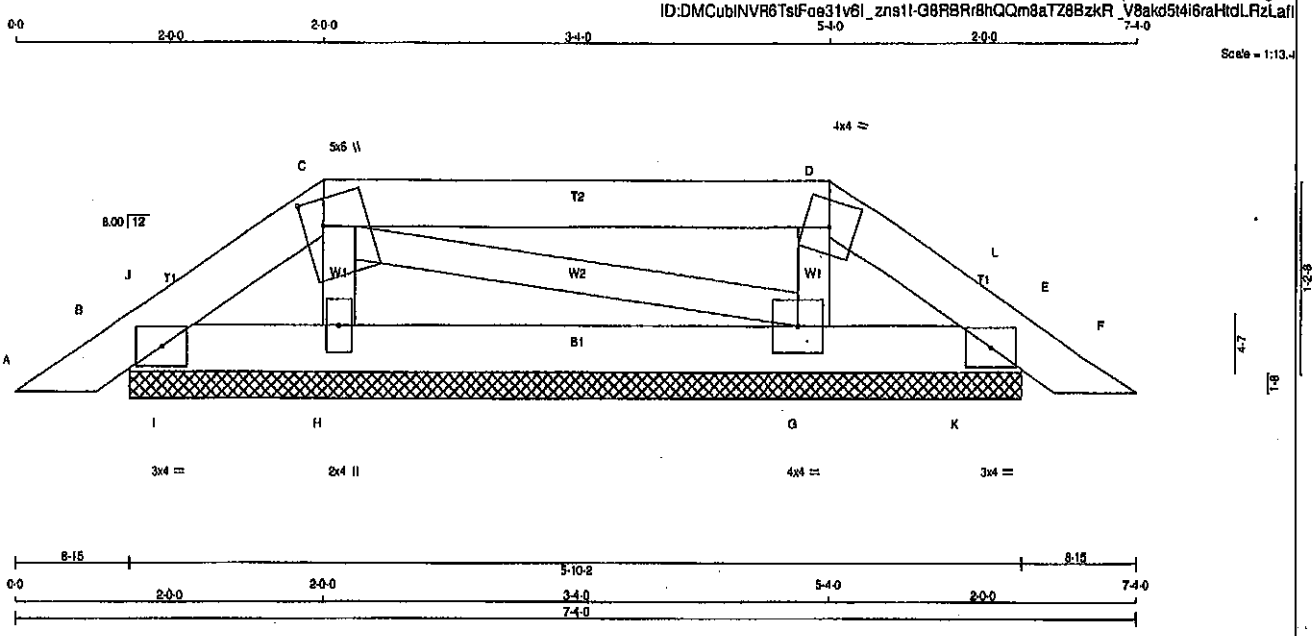

TOTAL WEIGHT = 19 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-I	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	148	0	148	0	0	5-10-2	5-10-2
E	144	0	144	0	0	5-10-2	5-10-2
H	233	0	233	0	0	5-10-2	5-10-2
G	241	0	241	0	0	5-10-2	5-10-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	102	81 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0
E	99	78 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0
H	166	101 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0
G	172	106 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-91.8	-91.8	0.03 (1)	10.00	
B- J	-45 / 0	-91.8	-91.8	0.00 (4)	6.25	
J- C	-46 / 0	-91.8	-91.8	0.01 (1)	6.25	
C- D	-11 / 0	-91.8	-91.8	0.17 (1)	6.25	
D- L	-38 / 0	-91.8	-91.8	0.01 (1)	6.25	
L- E	-37 / 0	-91.8	-91.8	0.00 (4)	6.25	
E- F	0 / 15	-91.8	-91.8	0.03 (1)	10.00	
B- I	0 / 37	-18.5	-18.5	0.02 (1)	10.00	
I- H	0 / 37	-18.5	-18.5	0.03 (4)	10.00	
H- G	0 / 18	-18.5	-18.5	0.03 (4)	10.00	
G- K	0 / 31	-18.5	-18.5	0.03 (4)	10.00	
K- E	0 - 31	-18.5	-18.5	0.02 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-169 / 0	0.02 (1)
C- G	-7 / 0	0.00 (1)
G- D	-175 / 0	0.02 (1)
I- J	-58 / 0	0.00 (1)
K- L	-59 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (C-D:1) , BC=0.03/1.00 (H-I:4) , WB=0.02/1.00 (D-G:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (D) (INPUT = 0.90)
JSI METAL= 0.04 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2007999

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	PB2	4	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:24 2020 Page 1
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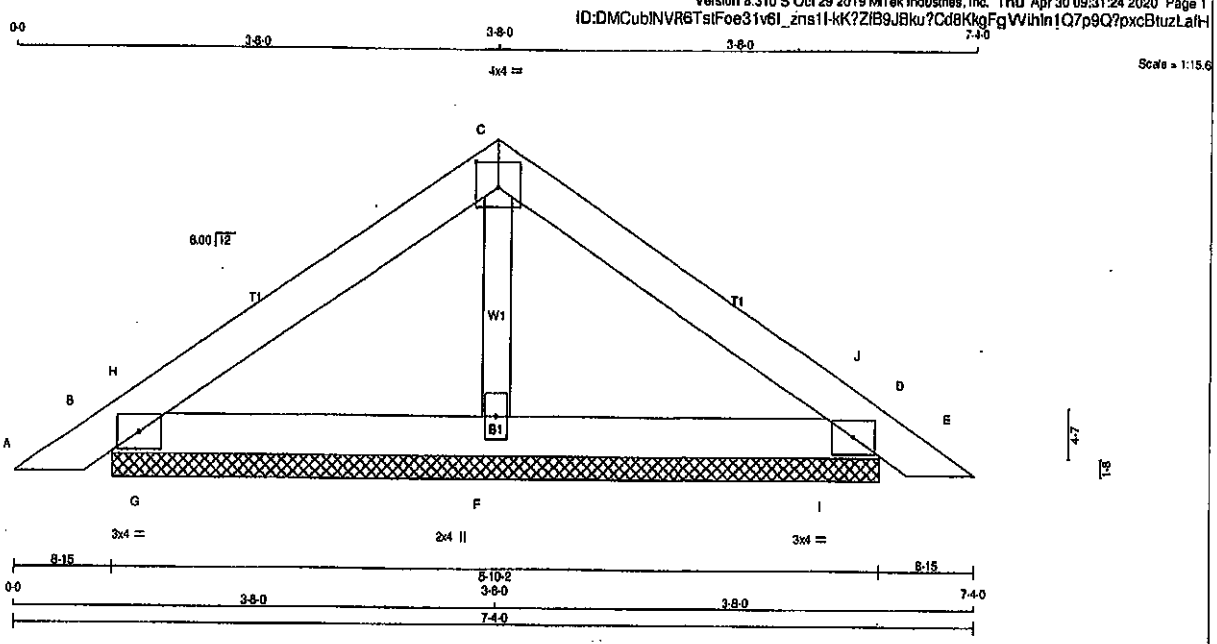

TOTAL WEIGHT = 4 X 18 = 72 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TM81-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TM81-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	252	0	252	0	0	5-10-2	5-10-2
D	252	0	252	0	0	5-10-2	5-10-2
F	262	0	262	0	0	5-10-2	5-10-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	176	127 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
D	176	127 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
F	187	113 / 0	0 / 0	0 / 0	0 / 0	74 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-91.8	-91.8	0.03 (1)	10.00
B- H	-37 / 0	-91.8	-91.8	0.04 (1)	6.25
H- C	-90 / 0	-91.8	-91.8	0.09 (1)	6.25
C- J	-90 / 0	-91.8	-91.8	0.09 (1)	6.25
J- D	-37 / 0	-91.8	-91.8	0.04 (1)	6.25
D- E	0 / 15	-91.8	-91.8	0.03 (1)	10.00
B- G	0 / 71	-18.5	-18.5	0.10 (1)	10.00
G- F	0 / 71	-18.5	-18.5	0.10 (1)	10.00
F- I	0 / 71	-18.5	-18.5	0.10 (1)	10.00
I- D	0 / 71	-18.5	-18.5	0.10 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-126 / 0	0.02 (1)
G- H	-219 / 0	0.00 (1)
I- J	-219 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (C-H:1) , BC=0.10/1.00 (B-G:1) , WB=0.02/1.00 (C-F:1) , SSI=0.16/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (D) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008000

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	PB3	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:26 2020 Page 1
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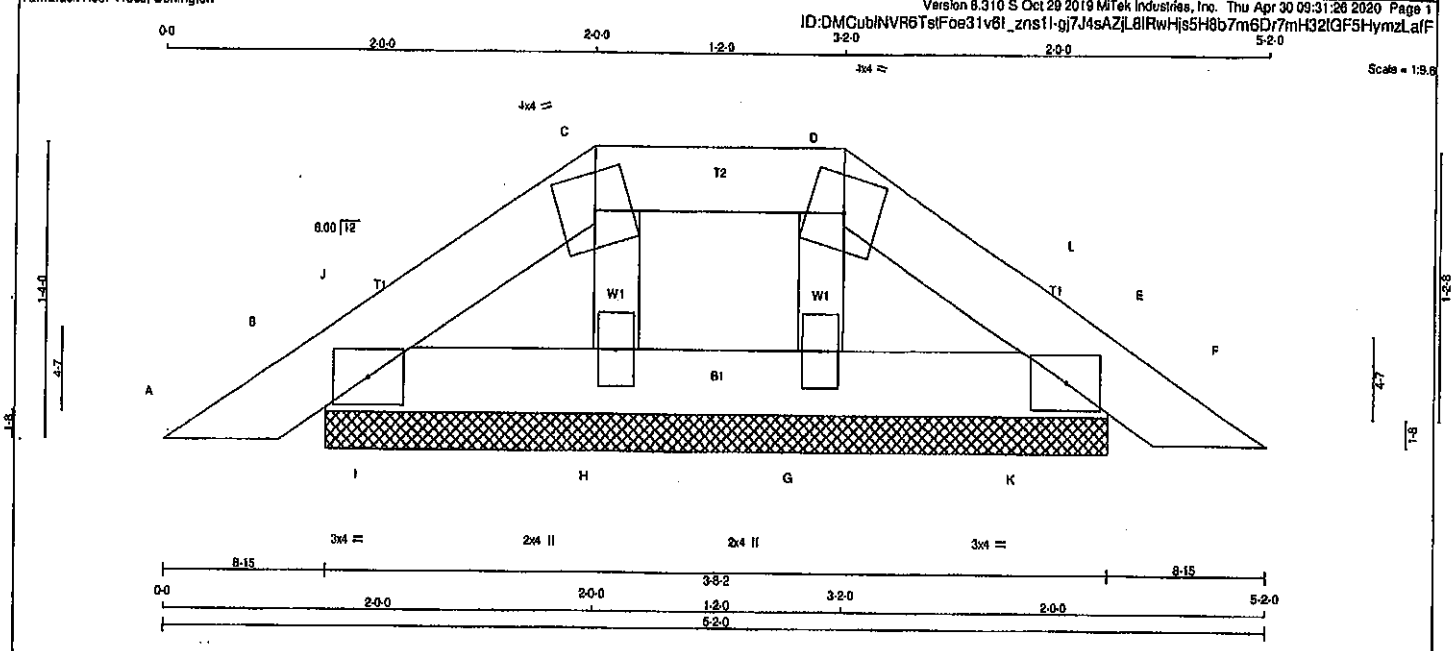

TOTAL WEIGHT = 12 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMB1-t	MT20	3.0	4.0		
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	144	0	144	0	0	3-8-2	3-8-2
E	144	0	144	0	0	3-8-2	3-8-2
H	119	0	119	0	0	3-8-2	3-8-2
G	119	0	119	0	0	3-8-2	3-8-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	101	74 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
E	101	74 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
H	85	54 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
G	85	54 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-91.8	-91.8	0.03 (1)	10.00
B-J	-34 / 0	-91.8	-91.8	0.00 (4)	6.25
J-C	-27 / 0	-91.8	-91.8	0.02 (1)	6.25
C-D	-14 / 0	-91.8	-91.8	0.02 (1)	6.25
D-L	-27 / 0	-91.8	-91.8	0.02 (1)	6.25
L-E	-34 / 0	-91.8	-91.8	0.00 (4)	6.25
E-F	0 / 15	-91.8	-91.8	0.03 (1)	10.00
B-I	0 / 23	-18.5	-18.5	0.02 (1)	10.00
I-H	0 / 23	-18.5	-18.5	0.02 (1)	10.00
H-G	0 / 14	-18.5	-18.5	0.01 (1)	10.00
G-K	0 / 23	-18.5	-18.5	0.02 (1)	10.00
K-E	0 / 23	-18.5	-18.5	0.02 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-C	-84 / 0	0.01 (1)
G-D	-84 / 0	0.01 (1)
I-J	-46 / 0	0.00 (1)
K-L	-46 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.03/1.00 (A-B:1) , BC=0.02/1.00 (E-K:1) , WB=0.01/1.00 (D-G:1) , SSI=0.04/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (E) (INPUT = 0.90)
JSI METAL= 0.02 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008001

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	PB4	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:27 2020 Page 1
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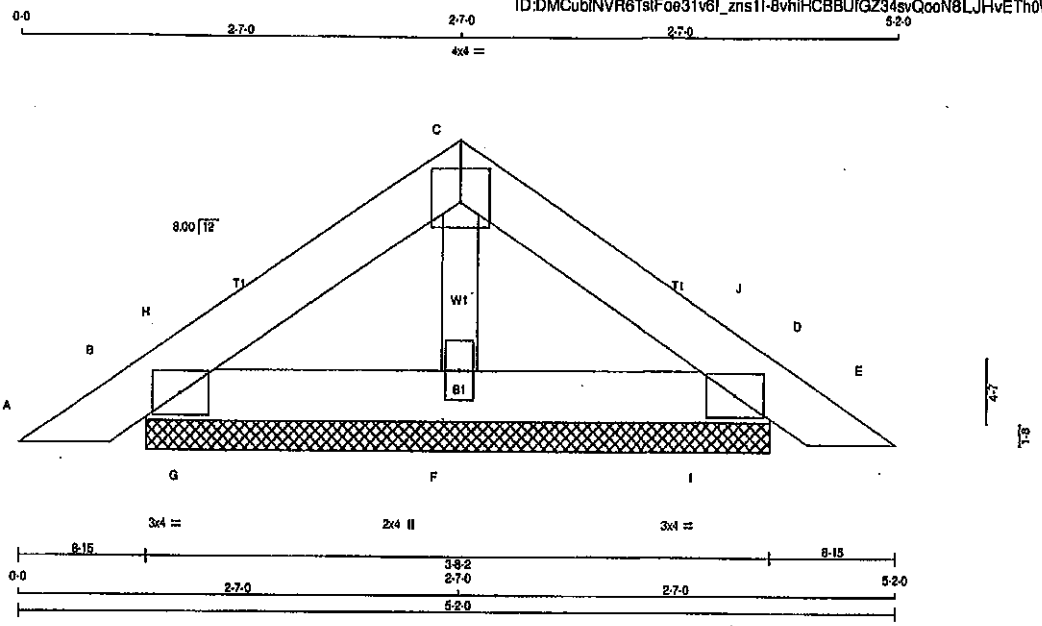

TOTAL WEIGHT = 2 X 12 = 24 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY; SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	175	0	175	0	0	3-8-2	3-8-2
D	175	0	175	0	0	3-8-2	3-8-2
F	178	0	178	0	0	3-8-2	3-8-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	122	89 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0
D	122	89 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0
F	127	78 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-91.8	-91.8	0.03 (1)	10.00	
B- H	-35 / 0	-91.8	-91.8	0.01 (1)	6.25	
H- C	-45 / 0	-91.8	-91.8	0.03 (1)	6.25	
C- J	-45 / 0	-91.8	-91.8	0.03 (1)	6.25	
J- D	-35 / 0	-91.8	-91.8	0.01 (1)	6.25	
D- E	0 / 15	-91.8	-91.8	0.03 (1)	10.00	
B- G	0 / 36	-18.5	-18.5	0.04 (1)	10.00	
G- F	0 / 38	-18.5	-18.5	0.04 (1)	10.00	
F- I	0 / 36	-18.5	-18.5	0.04 (1)	10.00	
I- D	0 / 36	-18.5	-18.5	0.04 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-86 / 0	0.01 (1)
G- H	-99 / 0	0.00 (1)
I- J	-99 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.03/1.00 (C-J:1) , BC=0.04/1.00 (F-I:1) , WB=0.01/1.00 (C-F:1) , SSI=0.07/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	789	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (D) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008002

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	PB5	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:29 2020 Page 1
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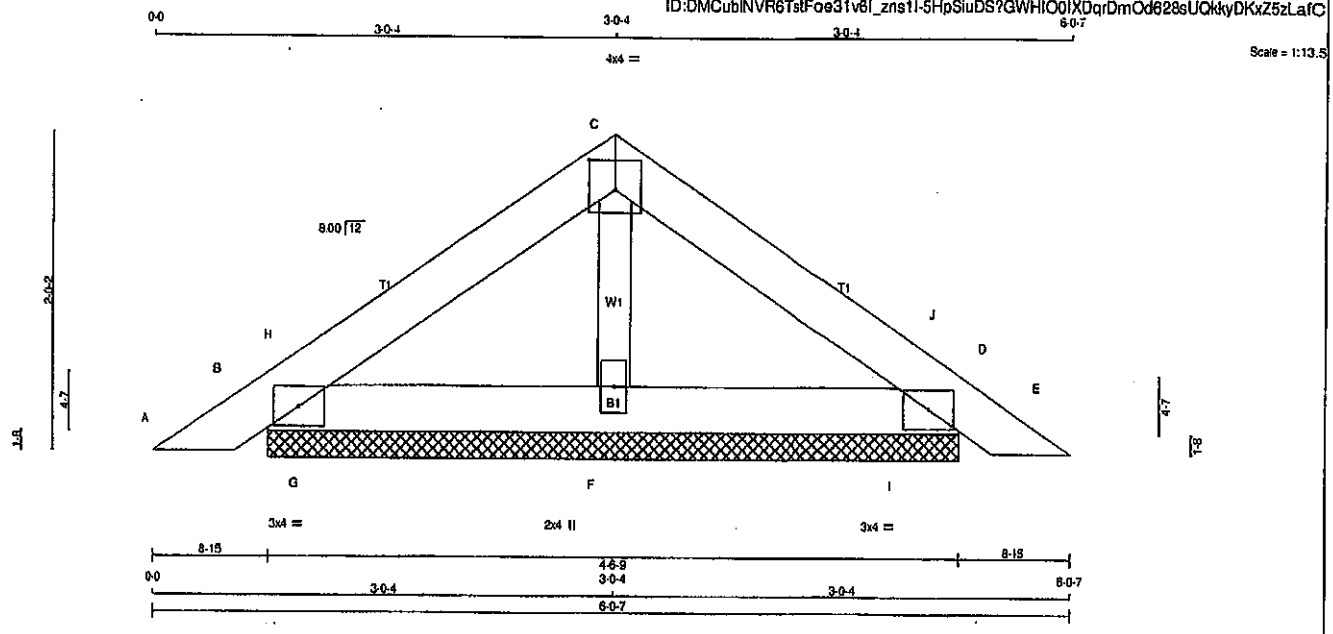

TOTAL WEIGHT = 2 X 14 = 29 lb.

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	RECORD BRG IN-SX
B	205	0	205	0	0	4-6-9	4-6-9
D	205	0	205	0	0	4-6-9	4-6-9
F	213	0	213	0	0	4-6-9	4-6-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	143	104 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0
D	143	104 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0
F	152	92 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-91.8	-91.8	0.03 (1)	10.00
B-H	-38 / 0	-91.8	-91.8	0.02 (1)	6.25
H-C	-62 / 0	-91.8	-91.8	0.05 (1)	6.25
C-J	-62 / 0	-91.8	-91.8	0.05 (1)	6.25
J-D	-38 / 0	-91.8	-91.8	0.02 (1)	6.25
D-E	0 / 15	-91.8	-91.8	0.03 (1)	10.00
B-G	0 / 49	-18.5	-18.5	0.06 (1)	10.00
G-F	0 / 49	-18.5	-18.5	0.06 (1)	10.00
F-I	0 / 49	-18.5	-18.5	0.06 (1)	10.00
I-D	0 / 49	-18.5	-18.5	0.06 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	-104 / 0	0.02 (1)
G-H	-141 / 0	0.00 (1)
I-J	-141 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (C-H:1) , BC=0.06/1.00 (B-G:1) , WB=0.02/1.00 (C-F:1) , SSI=0.10/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008003

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408315	PB5Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:30 2020 Page 1
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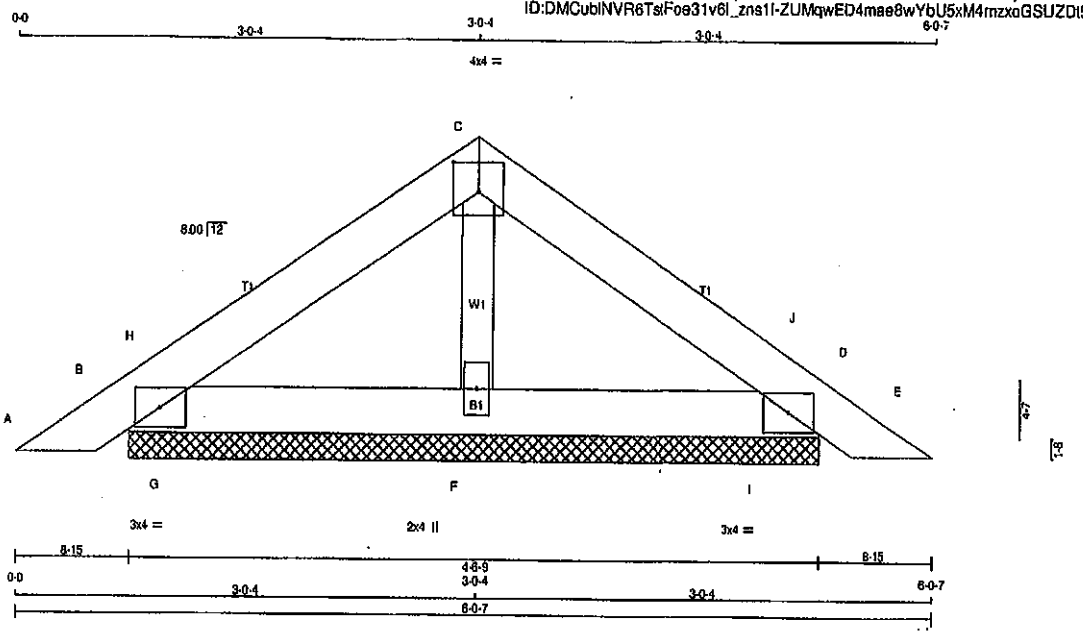

TOTAL WEIGHT = 2 X 14 = 29 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- C 1	12	TOP
C- E 1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
B- D 1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	206	0	206	0	0	4-6-9	4-6-9
D	206	0	206	0	0	4-6-9	4-6-9
F	212	0	212	0	0	4-6-9	4-6-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	144	104 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0
D	144	104 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0
F	151	92 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-91.8	-91.8	0.01 (1)	10.00	
B- H	-40 / 0	-91.8	-91.8	0.01 (1)	6.25	
H- C	-62 / 0	-91.8	-91.8	0.03 (1)	6.25	
C- J	-62 / 0	-91.8	-91.8	0.03 (1)	6.25	
J- D	-40 / 0	-91.8	-91.8	0.01 (1)	6.25	
D- E	0 / 15	-91.8	-91.8	0.01 (1)	10.00	
B- G	0 / 49	-18.5	-18.5	0.03 (1)	10.00	
G- F	0 / 49	-18.5	-18.5	0.03 (1)	10.00	
F- I	0 / 49	-18.5	-18.5	0.03 (1)	10.00	
I- D	0 / 49	-18.5	-18.5	0.03 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-104 / 0	0.01 (1)
G- H	-139 / 0	0.00 (1)
I- J	-139 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.03/1.00 (C-H:1) , BC=0.03/1.00 (B-G:1) , WB=0.01/1.00 (C-F:1) , SSI=0.08/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (B) (INPUT = 0.90)
JSI METAL= 0.02 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008004

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	J1	6	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:20 2020 Page 1
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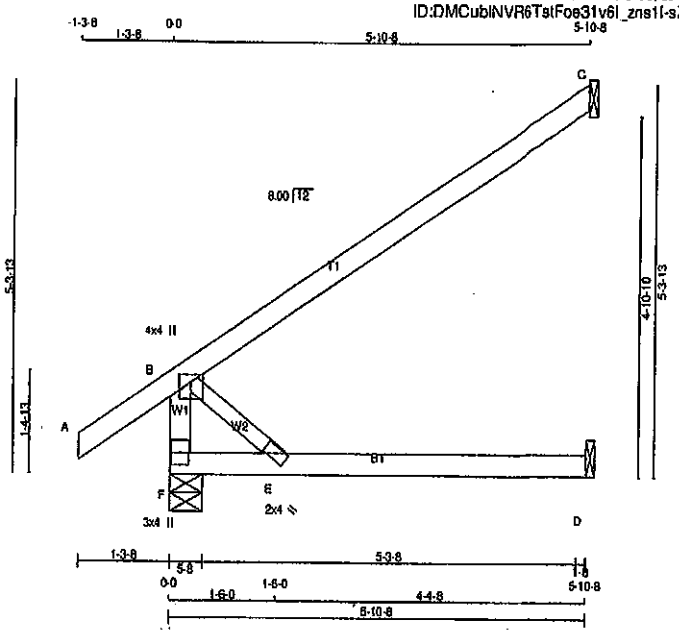

TOTAL WEIGHT = 6 X 19 = 115 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	450	0	450	0	0	5-8	5-8
C	270	0	270	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	316	221 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
C	186	150 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F-B	-395 / 0	0.0	0.0	0.04 (1)	7.81
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B-C	0 / 0	-91.8	-91.8	0.54 (1)	10.00
F-E	0 / 0	-18.5	-18.5	0.14 (4)	10.00
E-D	0 / 0	-18.5	-18.5	0.19 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-E	0 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2007997

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408315	J2	8	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:22 2020 Page 1
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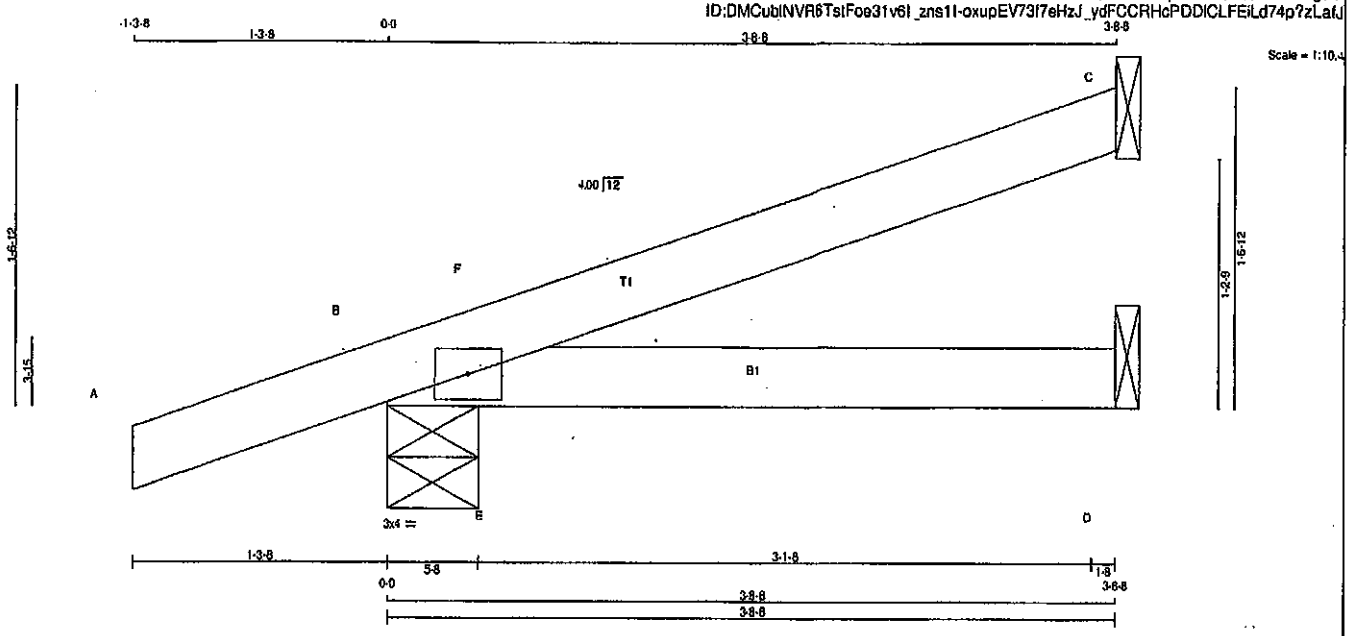

TOTAL WEIGHT = 8 X 10 = 83 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMS1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	146	0	146	0	0	1-8	1-8
B	327	0	327	0	0	5-8	5-8
D	58	0	58	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	101	78 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
B	229	163 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0
D	43	17 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 18	-91.8	-91.8	0.13 (5)	10.00	
B-F	-11 / 0	-91.8	-91.8	0.04 (4)	6.25	
F-C	0 / 2	-91.8	-91.8	0.16 (1)	10.00	
B-E	0 / 0	-18.5	-18.5	0.12 (1)	10.00	
E-D	0 / 0	-18.5	-18.5	0.12 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-F	-146 / 6	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.16/1.00 (C-F:1) , BC=0.12/1.00 (B-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.12/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2007998

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408316	J30	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 6.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:05:21 2020 Page 1
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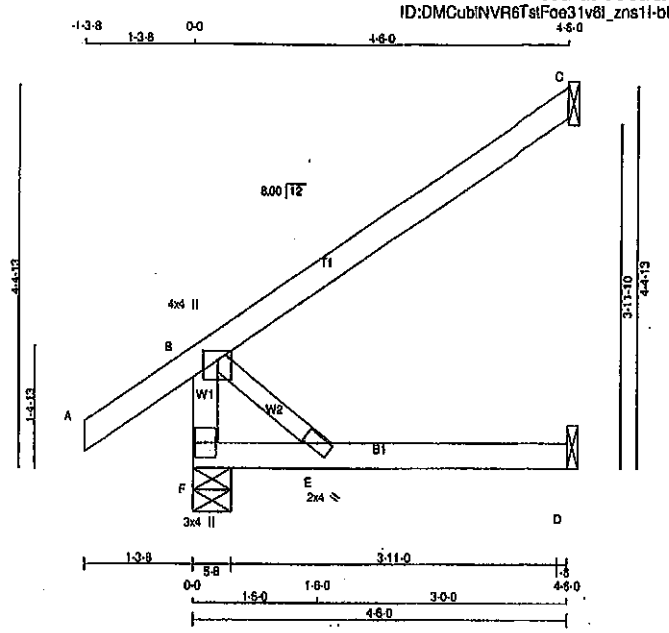

TOTAL WEIGHT = 2 X 16 = 31 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	374	0	374	0	0	5-8	5-8
C	208	0	208	0	0	1-8	1-8
D	42	0	47	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C ,D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	262	185 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
C	142	115 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
D	33	0 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
F-B	-332 / 0	0.0	0.0	0.03 (1)	7.81
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B-C	0 / 0	-91.8	-91.8	0.32 (1)	10.00
F-E	0 / 0	-18.5	-18.5	0.10 (4)	10.00
E-D	0 / 0	-18.5	-18.5	0.11 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-E	0 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 989 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.32/1.00 (B-C:1) , BC=0.11/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	786	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J/G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008026

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	C1	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:09 2020 Page 1
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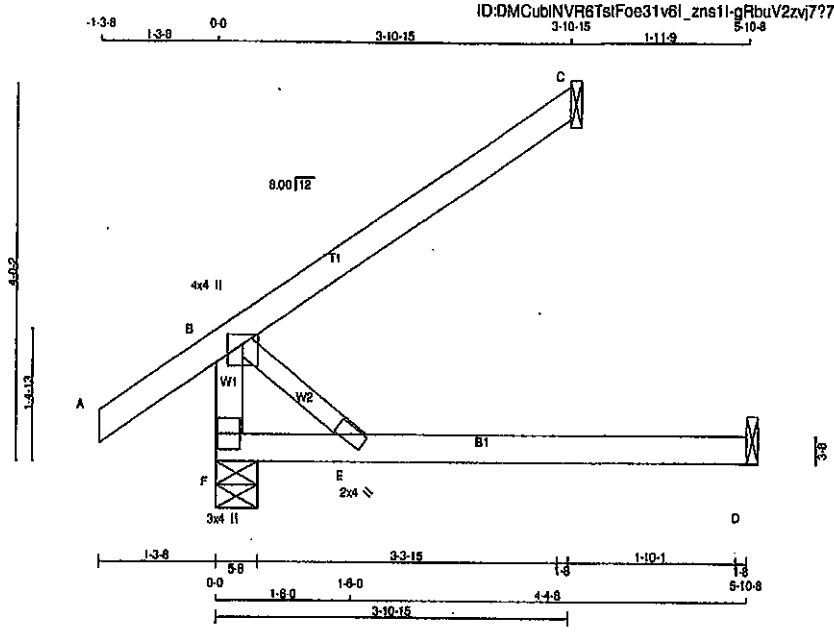

TOTAL WEIGHT = 17 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	360	0	360	0	0	5-8	5-8
C	179	0	179	0	0	1-8	1-8
D	54	0	81	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX.MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	254	170 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0
C	124	100 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-B	-305 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.03 (1)
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00			
B-C	0 / 0	-91.8	-91.8	0.24 (1)	10.00			
F-E	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
E-D	0 / 0	-18.5	-18.5	0.19 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J/G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2007989

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	C2	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:10 2020 Page 1
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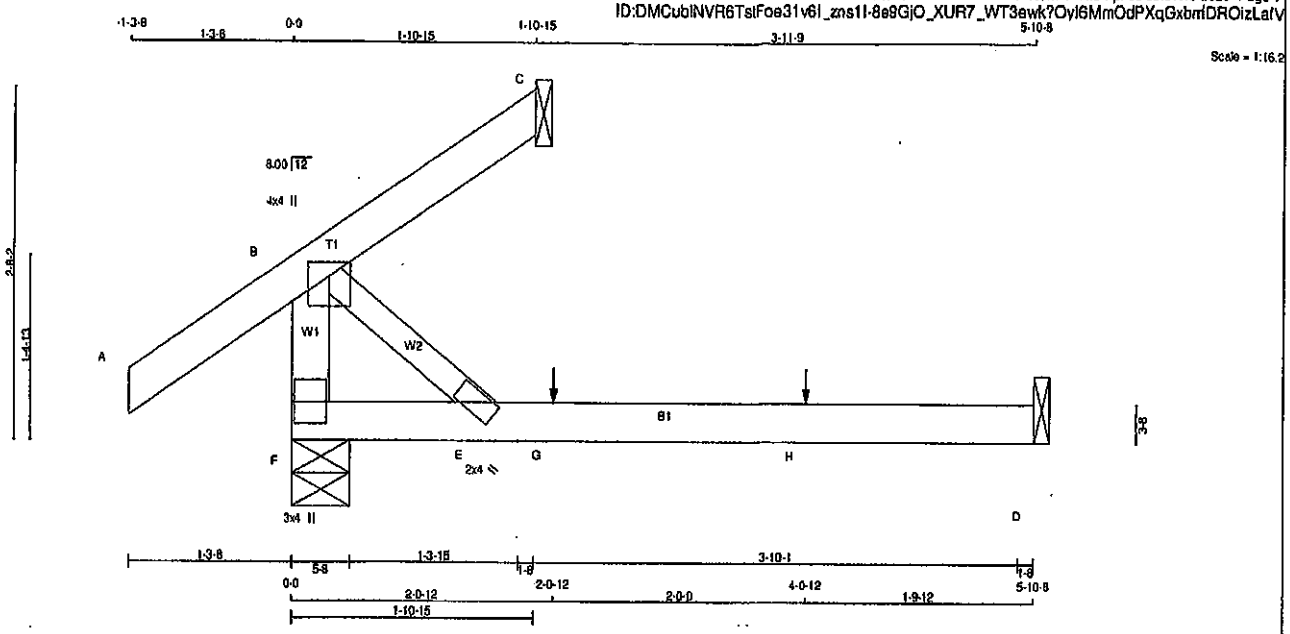

TOTAL WEIGHT = 14 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	313	0	313	0	0	5-8	6-8
C	42	0	42	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	222	144 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
C	29	24 / 0	0 / 0	0 / 0	0 / 0	6 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-259 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 35	-91.8	-91.8	0.13 (1)	10.00
B- C	-25 / 0	-91.8	-91.8	0.12 (1)	6.25
F- E	0 / 0	-18.5	-18.5	0.14 (4)	10.00
E- G	0 / 0	-18.5	-18.5	0.19 (4)	10.00
G- H	0 / 0	-18.5	-18.5	0.19 (4)	10.00
H- D	0 / 0	-18.5	-18.5	0.19 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1
H	4-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=0.98 NAIL=0.98 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2007990

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408315	C3	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:12 2020 Page 1

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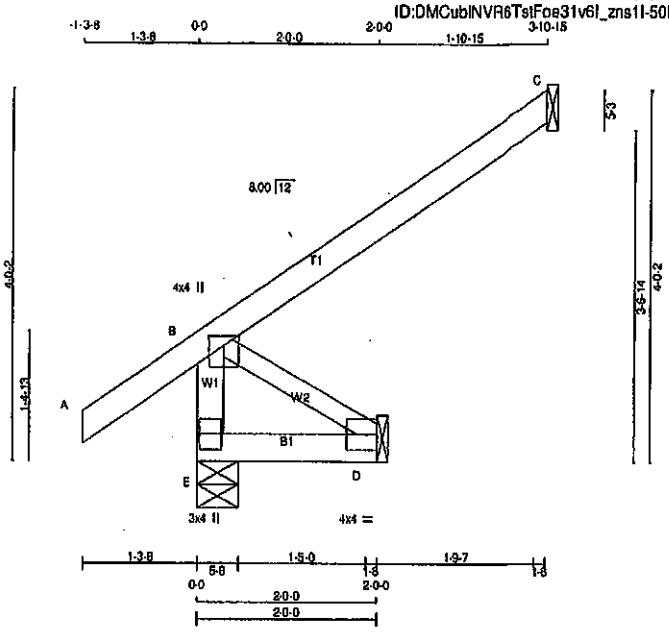

TOTAL WEIGHT = 12 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	4.0	1.25	2.00
D	BMW1-1	MT20	4.0	4.0	2.00	Edge
E	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	323	0	323	0	0	5-8	5-8
C	179	0	179	0	0	1-8	1-8
D	18	0	20	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

1ST LCASE — MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	224	170 / 0	0 / 0	0 / 0	0 / 0	54 / 0	0 / 0
C	124	100 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
E- B	-305 / 0	0.0	0.0	0.03 (1)	7.61
A- B	0 / 35	-91.8	-91.8	0.13 (6)	10.00
B- C	0 / 0	-91.8	-91.8	0.24 (1)	10.00
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- D	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2007991

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
08315	C4	1	1	GREEN PARK HOMES TRUSS DESC.	

amarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 09:31:13 2020 Page 1
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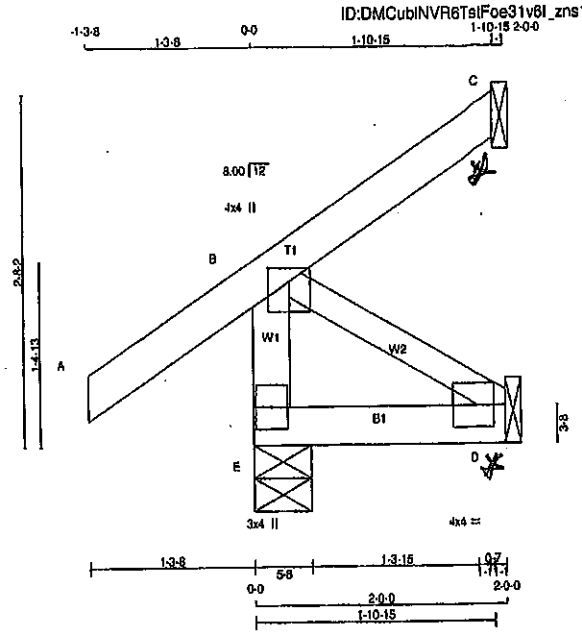

TOTAL WEIGHT = 10 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	BMW1-1	MT20	4.0	4.0	2.00	1.50
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	276	0	276	0	0	5-8	5-8
C	42	0	42	0	-35	1-8	1-8
D	18	0	20	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	192	144 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0
C	29	24 / -26	0 / 0	0 / 0	0 / 0	6 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
E-B	-259 / 0	0.0	0.0	0.03 (1)	7.81	
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
B-C	-25 / 0	-91.8	-91.8	0.12 (1)	6.25	
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-D	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2007992

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
08317	C50	4	1	GREEN PARK HOMES TRUSS DESC.	

amarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:25 2020 Page 1
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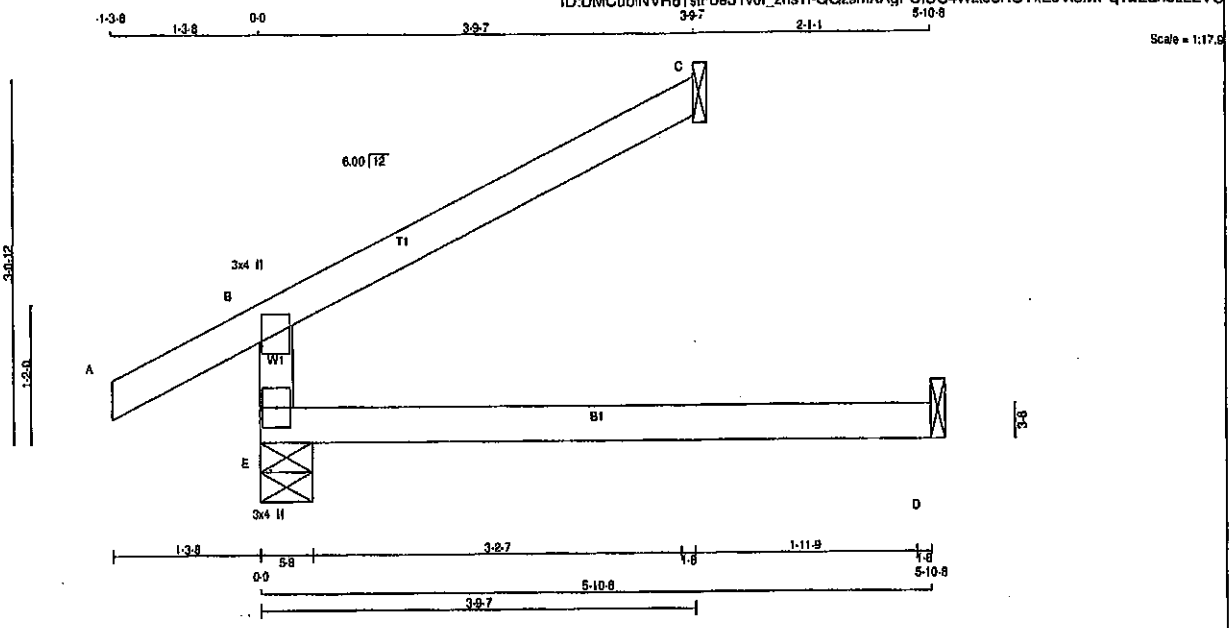

TOTAL WEIGHT = 4 X 14 = 57 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	405	0	405	0	0	5-8	5-8
C	130	0	130	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	288	190 / 0	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-342 / 0	0.0	0.0		0.13 (4)	7.61			
A-B	0 / 28	-91.8	-91.8		0.12 (1)	10.00			
B-C	-19 / 0	-91.8	-91.8		0.22 (1)	6.25			
E-D	0 / 0	-18.5	-18.5		0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008050

JB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
08317	C51	4	1	GREEN PARK HOMES TRUSS DESC.	

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:26 2020 Page 1
marack Roof Truss, Burlington
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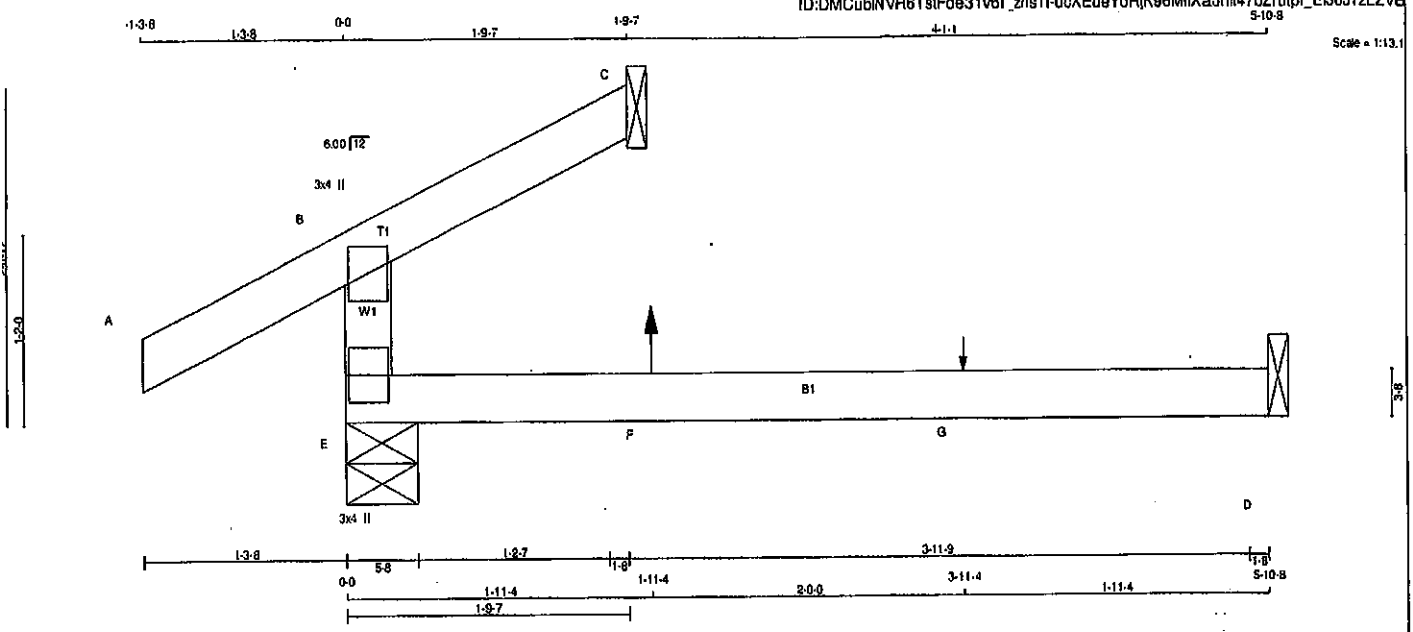

TOTAL WEIGHT = 4 X 12 = 46 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	284	0	284	0	0	5-8	5-8
C	63	0	63	0	0	1-8	1-8
D	44	0	52	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	200	137 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	46	21 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
D	35	0 / -3	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
E-B	-227 / 0	0.0	0.0	0.11 (4)	7.81
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B-C	-9 / 9	-91.8	-91.8	0.08 (4)	10.00
E-F	0 / 0	-18.5	-18.5	0.14 (4)	10.00
F-G	0 / 0	-18.5	-18.5	0.14 (4)	10.00
G-D	0 / 0	-18.5	-18.5	0.14 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	BACK	VERT	TOTAL	---	C1
G	3-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.14/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H.J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008051

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
08317	C52	4	1	TRUSS DESC.	

amarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:28 2020 Page 1
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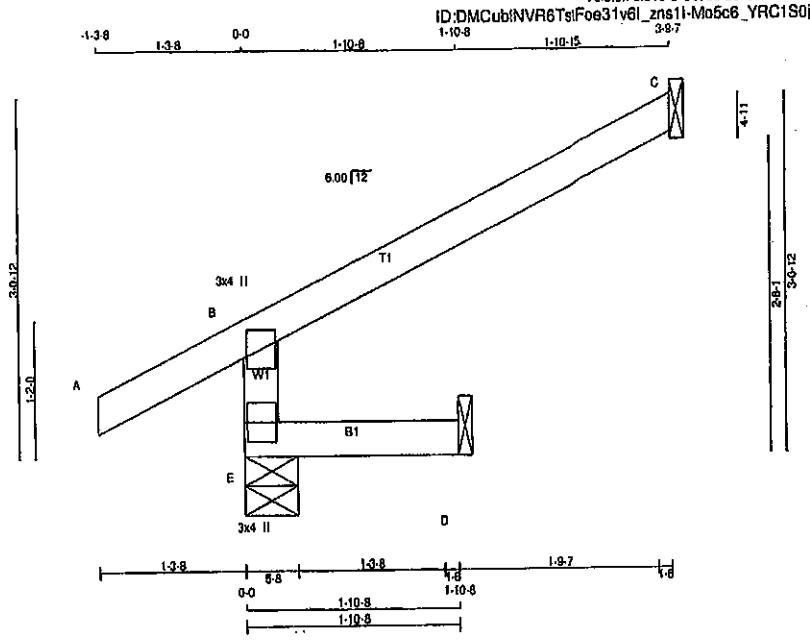

TOTAL WEIGHT = 4 X 10 = 38 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	361	0	361	0	0	5-8	5-8
C	130	0	130	0	0	1-8	1-8
D	18	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	250	190 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-342 / 0	0.0	0.0	0.01	(4)	7.81			
A- B	0 / 28	-91.8	-91.8	0.13	(5)	10.00			
B- C	-19 / 0	-91.8	-91.8	0.22	(1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.02	(4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H.J. J. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008052

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408317	C53	4	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Thu Apr 30 10:50:29 2020 Page 1
ID:DMCubINVR6TstFoe31v6I_zns1I-IBDMXfahkeikzqNHCitOKKiesmv94AOQwgXmwMzLZV8

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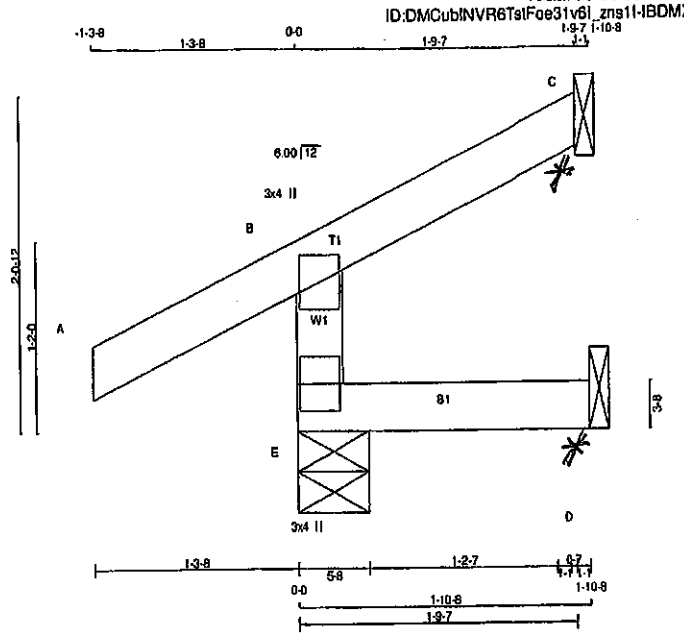


TOTAL WEIGHT = 4 X 7 = 28 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	271	0	271	0	0	5-8	5-8
C	45	0	45	0	-23	1-8	1-8
D	8	0	17	0	-2	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	188	141 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	31	24 / -18	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0
D	7	0 / -8	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
E- B	-244 / 0	0.0	0.0	0.04 (5)		7.81
A- B	0 / 28	-91.8	-91.8	0.12 (1)		10.00
B- C	-17 / 0	-91.8	-91.8	0.09 (1)		6.25
E- D	0 / 0	-18.5	-18.5	0.04 (5)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.04/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/30/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2008053

SIMPSON Strong-Tie

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

VALUE ENGINEERED

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

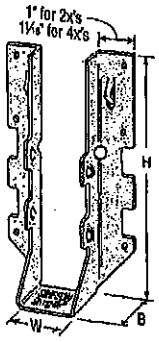
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

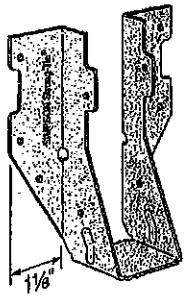
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

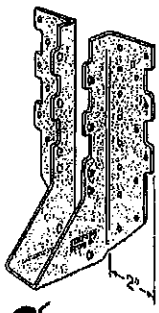
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



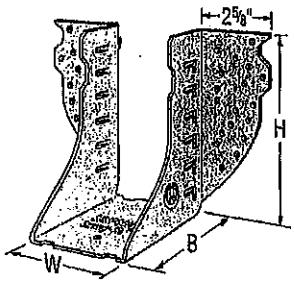
LUS28



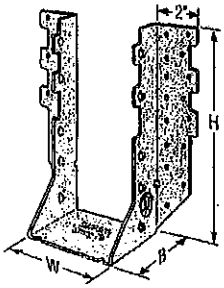
LU26L



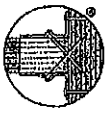
HUS210
(HUS26, HUS28, and HHUS similar)



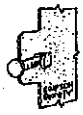
HGUS28-2



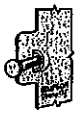
HHUS210-2



Double-Shear Nailing Top View



Double-Shear Nailing Side View; Do not bend tab

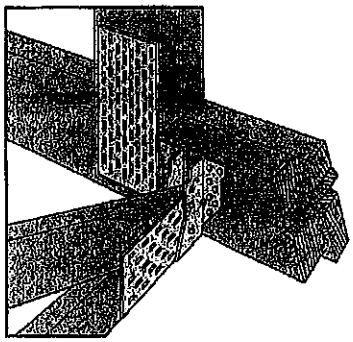


Dome Double-Shear Nailing Side View *(available on some models)*

U.S. Patent 5,603,580

Typical HUS26 Installation with Reduced Heel Height

(Truss Designer to provide fastener quantity for connecting multiple members together)



LJS26DS

Plated Truss Connectors

SIMPSON Strong-Tie

LUL/LUS/LJS/HUS/HHUS/HGUS

LICENSED PROFESSIONAL ENGINEER 1/15/18 B. D. BUNTING PROVINCE OF ONTARIO

HHUS/HGUS

See Hanger Options information on pp. 125–127.

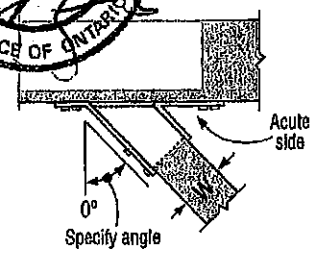
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

	Model No.	Ga.	Dimensions (in.) W	H	B	d_a[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
	Single 2x Sizes											
	LUS24	18	1 9/16	3 1/8	1 3/4	2 1/4	(4) 10d	(2) 10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
	LU24L	22	1 9/16	3	1 5/8	2 11/16	(4) 10d	(2) 10d x 1 1/2"	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
	LU26L	22	1 9/16	5	1 5/8	4 5/8	(6) 10d	(4) 10d x 1 1/2"	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
SS	LUS26	18	1 9/16	4 3/4	1 3/4	3 3/4	(4) 10d	(4) 10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
	HUS26	16	1 5/8	5 5/8	3	3 15/16	(14) 16d	(6) 16d	2705 / 11.30	4940 / 21.97	2065 / 9.20	3875 / 17.24
	LJS26DS	18	1 9/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055 / 9.14	4265 / 18.97	1460 / 6.49	4115 / 18.31
	HGUS26	12	1 5/8	5 5/8	5	4 1/8	(20) 16d	(8) 16d	2685 / 11.96	6625 / 29.51	2685 / 11.96	5700 / 25.35
	LU28L	20	1 9/16	6 3/4	1 5/8	5 7/8	(8) 10d	(6) 10d x 1 1/2"	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
SS	LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(4) 10d	1420 / 6.32	2520 / 11.21	1290 / 5.74	1790 / 7.96
	HUS28	16	1 5/8	7 1/16	3	6 1/8	(22) 16d	(8) 16d	3605 / 16.04	5365 / 23.86	2675 / 11.90	4345 / 19.33
	HGUS28	12	1 5/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310 / 14.74	7675 / 34.19	3310 / 14.74	6900 / 30.73
	LU210L	20	1 9/16	8	1 5/8	7 5/8	(10) 10d	(6) 10d x 1 1/2"	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
SS	LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420 / 6.32	2785 / 12.39	1290 / 5.74	2210 / 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_a is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

SIMPSON Strong-Tie®

Face-Mount Hangers

■ These products are available with additional corrosion protection. For more information, see p. 24.

◤ These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_6[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Double 2x Sizes											
■ LUS24-2	18	3⅛	3⅛	2	1½	(4) 16d	(2) 16d	835	2020	590	1435
								3.71	8.99	2.62	6.38
SS LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
■ HHUS26-2	14	3 9/16	5⅜	3	3 15/16	(14) 16d	(6) 16d	2850	7335	2065	5205
								12.68	32.63	9.20	23.15
HGUS26-2	12	3 9/16	5⅞	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
SS LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
■ HHUS28-2	14	3 9/16	7 9/16	3	6⅛	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS28-2	12	3 9/16	7 9/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
SS LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
■ HHUS210-2	14	3 9/16	9 9/16	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
								20.77	42.97	18.84	31.14
HGUS210-2	12	3 5/16	9 9/16	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
Triple 2x Sizes											
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
■ HHUS210-3	14	4 11/16	9	3	7 15/16	(30) 16d	(10) 16d	4670	9670	4235	6865
								20.77	43.02	18.84	30.54
HGUS210-3	12	4 11/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	6 9/16	5 9/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-4	12	6 9/16	7 9/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
■ HHUS210-4	14	6⅛	8⅞	3	7 13/16	(30) 16d	(10) 16d	4670	10155	4235	7210
								20.77	45.17	18.84	32.07
HGUS210-4	12	6 9/16	9 9/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
HGUS212-4	12	6 9/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS214-4	12	6 9/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
■ LUS46	18	3 9/16	4¾	2	3 7/16	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
■ HHUS46	14	3⅝	5¼	3	3 15/16	(14) 16d	(6) 16d	2640	7335	2065	5205
								11.30	32.63	9.20	23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
■ LUS48	18	3 9/16	6¾	2	3 7/16	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
■ HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS48	12	3⅝	7 1/16	4	6 1/16	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
■ LUS410	18	3 9/16	8¾	2	5 9/16	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HGUS410	12	3⅝	9	4	8 9/16	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
HGUS412	12	3⅝	10 7/16	4	10 9/16	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS414	12	3⅝	12 7/16	4	11 9/16	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80

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See footnotes on p. 258.

TC – Truss Connectors

SIMPSON Strong-Tie®

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

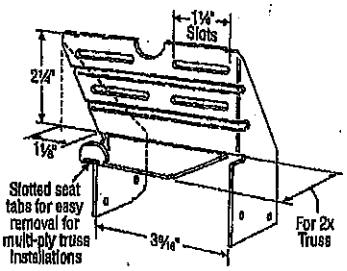
Design: Factored resistances are in accordance with CSA 086-14

Installation:

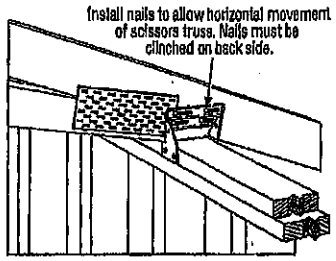
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½ = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss–allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

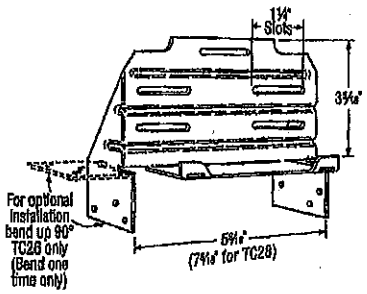
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



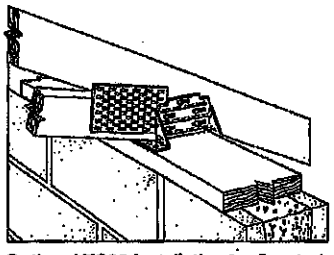
TC24
U.S. Patent 4,932,173



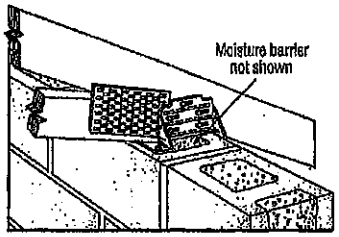
Typical TC24 Installation



TC26
(TC28 Similar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L.	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC24	(4) 10d	(4) 10d	605	430
TC26	(6) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC26	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC28 fastened to grouted concrete block with (6) – ⅜" x 2¼" Titen screws has a factored uplift resistance of 275 lb.

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H/TSP

SIMPSON Strong-Tie®

Seismic and Hurricane Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Fasteners: To Rafters/Truss	Fasteners: To Plates	Fasteners: To Studs	Factored Resistance (K_D = 1.15) D.Fir-L Uplift	D.Fir-L Lateral F_1	D.Fir-L Lateral F_2	S-P-F Uplift	S-P-F Lateral F_1	S-P-F Lateral F_2
					lb. / kN	lb. / kN	lb. / kN	lb. / kN	lb. / kN	lb. / kN
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
					3.29	3.05	1.33	3.02	2.16	0.96
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
					3.69	0.98	0.33	2.62	0.69	0.24
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
					3.58	0.71	0.71	3.36	0.71	0.71
H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
					3.71	0.78	0.93	3.29	0.71	0.93
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
					3.29	0.80	1.18	2.74	0.56	0.85
H6	16	—	(8) 8d	(8) 8d	1585	1085	—	1125	770	—
					7.05	4.83	—	5.00	3.43	—
H7Z	16	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
					6.18	2.98	—	4.40	2.11	—
H8[3]	18	(5) 10d x 1½"	(5) 10d x 1½"	—	1120	—	—	1025	—	—
					4.98	—	—	4.56	—	—
H10A[9]	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290
					7.72	3.54	1.82	6.69	2.51	1.29
H10AR	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1485	690	430	1220	570	305
					6.61	3.07	1.91	5.43	2.54	1.36
H10A-2	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1835	1275	430	1645	880	305
					8.16	5.67	1.91	7.32	3.91	1.36
H10S[7,8]	18	(8) 8d x 1½"	(8) 8d x 1½"	(8) 8d	1465	795	315	1040	565	225
					6.52	3.54	1.40	4.63	2.51	1.00
H11Z	18	(6) 16d x 2½"	(6) 16d x 2½"	—	1095	920	545	780	655	390
					4.87	4.09	2.42	3.47	2.91	1.73
H14	18	[1] (12) 8d x 1½"	(13) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
		[2] (12) 8d x 1½"	(15) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
TSP	16	(9) 10d x 1½"	(6) 10d x 1½"	—	1295	440	—	920	310	—
					5.76	1.96	—	4.09	1.38	—
		(9) 10d x 1½"	(6) 10d	—	1560	440	—	1105	310	—
					6.94	1.96	—	4.92	1.38	—

1. Factored resistances have been increased 15% for short term loading; no further increase is allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
3. H8 factored uplift resistances for stud-to-bottom plate installations are 595 lb. (2.65 kN) for D.Fir-L and 390 lb. (1.74 kN) for S-P-F.
4. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
5. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-HTIECONPATH).
6. Factored resistances in the F_1 direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.
7. H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.38 kN) D.Fir-L and 1015 lb. (4.51 kN) S-P-F.
8. H10S nails to plates are optional for uplift but required for lateral loads.
9. H10A may be field-bent up to a slope of 8/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
10. The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SSNAILS at strongtie.com.
11. D.Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1½" nails into the top plates and (3) 8d x 1½" nails into the lowest three flange holes into the truss bottom chord is 495 lb. (2.20 kN).
12. **Nails:** 16d x 2½" = 0.162" dia. x 2½" long, 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long, 8d x 1½" = 0.131" dia. x 1½" long. See pp. 27–28 for other nail sizes and information.

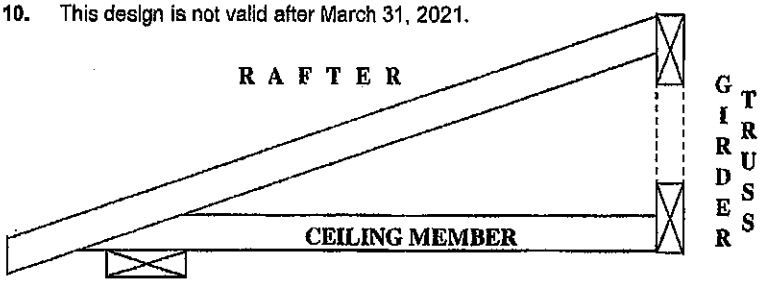
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

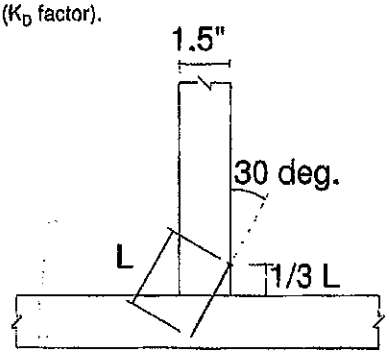
B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.





TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	**MAXIMUM NUMBER OF TOE-NAILS**			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

PEO
Certificate No. 10889485

LICENSED PROFESSIONAL ENGINEER
C. Cordogiannis
90553736
PROVINCE OF ONTARIO

MiTek®
MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

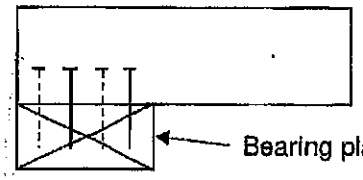
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

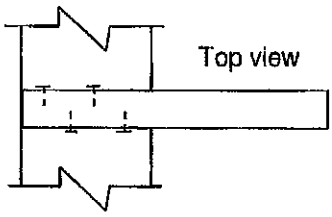
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37679H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

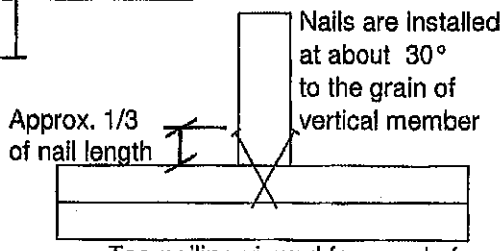
Toe-nailing on 2x6 Bearing Plate



Elevation view

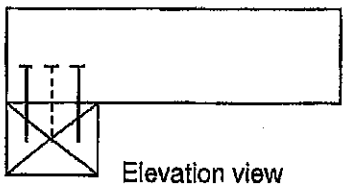


Top view

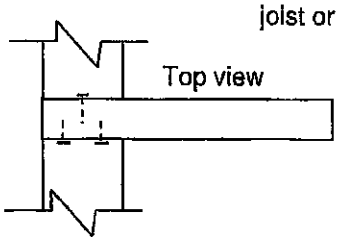


Toe-nailing viewed from end of joist or truss

Toe-nailing on 2x4 Bearing Plate



Elevation view



Top view

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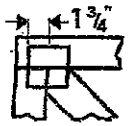
MiTek®
MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

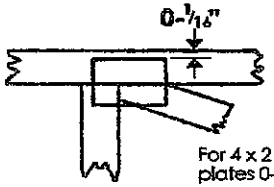
December 2, 2019

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

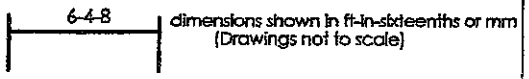
BEARING

Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

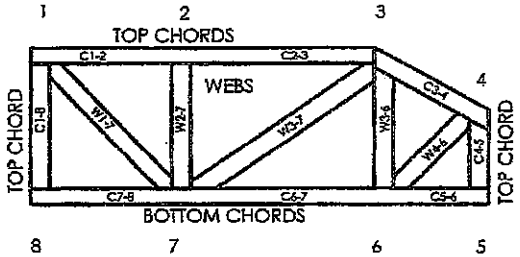
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

ONTARIO WOOD TRUSS
FABRICATORS ASSOCIATION

TECH-NOTES

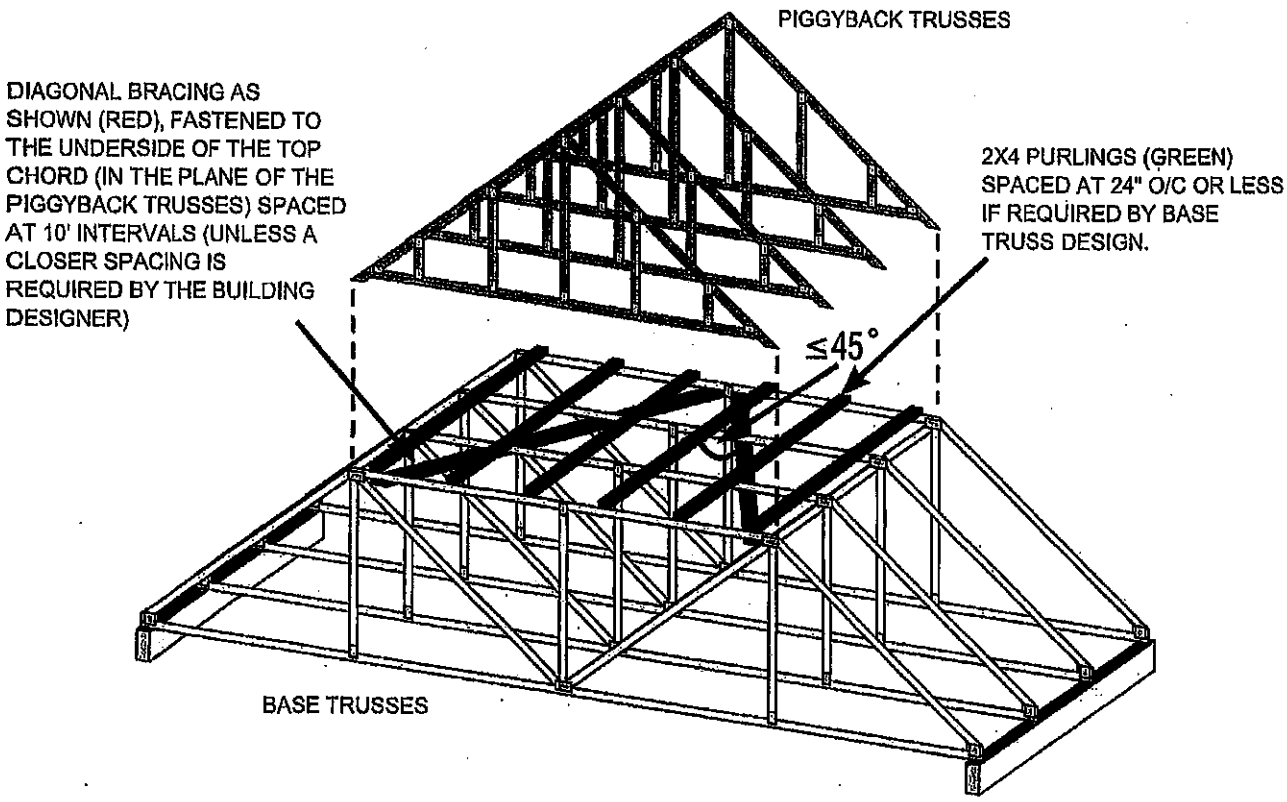
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

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LICENSED PROFESSIONAL ENGINEER
H.J.G. ALVES
PROVINCE OF ONTARIO

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900215 Feb 09, 2018