

FEB 05 2021

REC'D BY _____ DATE _____
REF'D TO _____ DATE _____

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:

RESIDENTIAL | PART: 9

✓ Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:

TCSL = 25.6 psf

TCDL = 6.0 psf

BCLL = 0.0 psf

BCDL = 7.4 psf

CITY OF HAMILTON
Building Division

Permit No. 21-105965

THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE

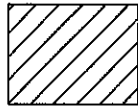
THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH
THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW

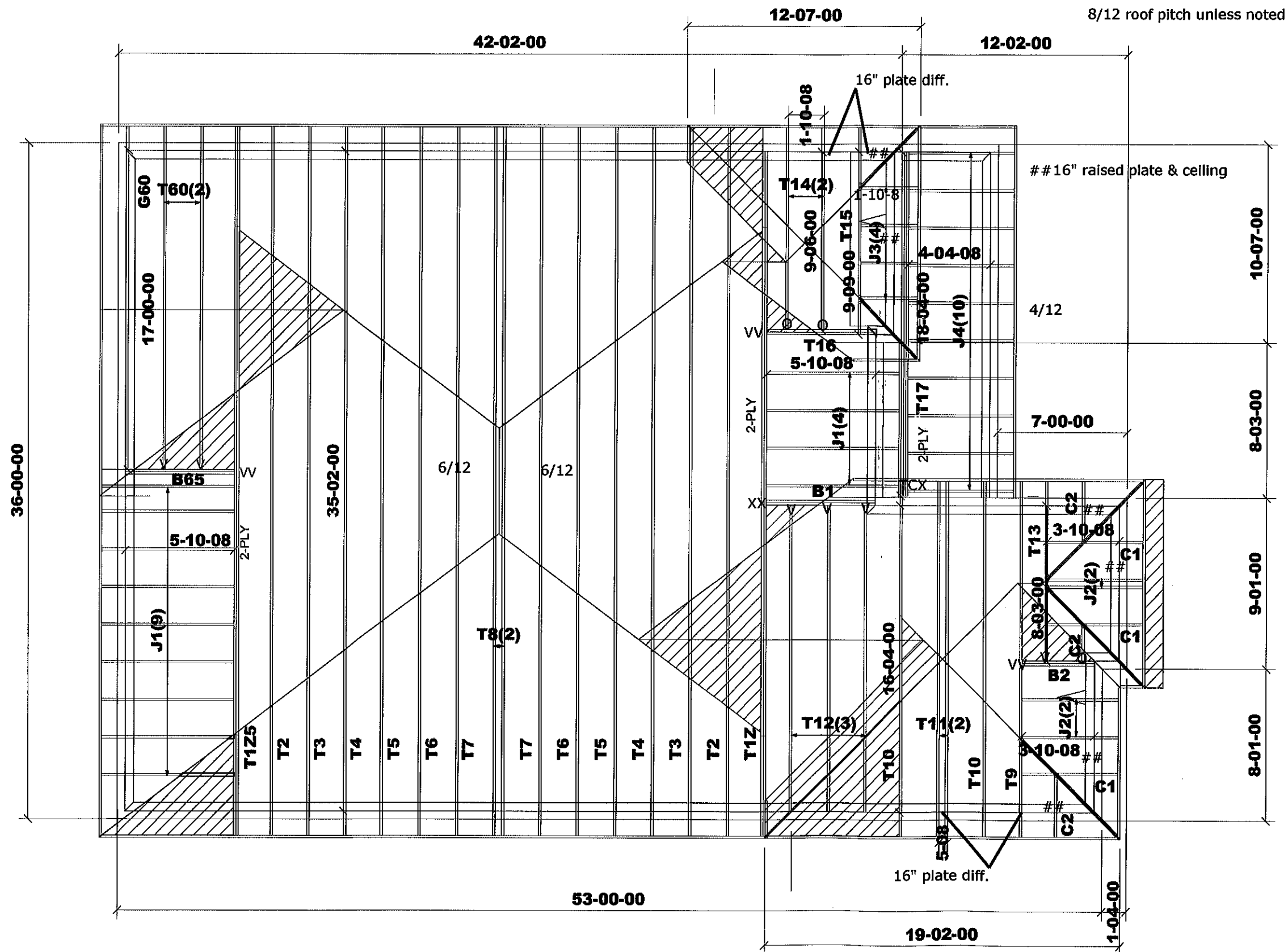
These drawings and/or specifications have been reviewed by

Kim Smith Feb 22, 2021
FOR CHIEF BUILDING OFFICIAL DATE

BEAMS:

B2 = B65 = 2 - 2x10 SPF #2

 **DENOTES:
CONVENTIONAL
FRAMING**



Job Track: 51225
Plan Log: 202395
Layout ID: 415100

Builder / Location:
GREEN PARK HOMES / WATERDOWN

Project: **RUSSELL GARDENS PH.3**

Date: 2020-12-08 Sales: Mario DiCano

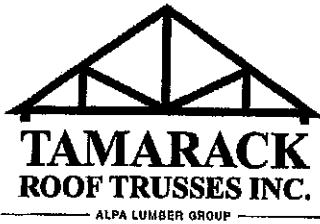
Designer: JG / ND

Model / Elevation:

MOUNTAINASH 3 | 176/1 -STD OR OPT.5 BR

Mike ver 8.3.3.247

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 3
 Lot #: 176
 Elevation: 1-STD OR OPT.5 BR

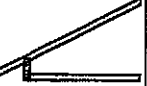
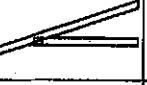
Job Track: 51225
 PlanLog: 202395
 Layout ID: 408131
 Ref #
 Page: 1 of 2
 Date: 04-27-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	8 / 12	35-02-00	4-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	359.48 218.67		CITY OF HAMILTON BUILDING DIVISION Planning & Development Department FEB 05 2021 REC'D BY _____ DATE _____ REF'D TO _____ DATE _____
	1 2-ply	T1Z Hip Girder	8 / 12	35-02-00	4-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	359.48 218.67		
	2	T2 Hip	8 / 12	35-02-00	5-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	279.73 177.00		
	2	T3 Hip	8 / 12	35-02-00	6-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	302.74 191.33		
	2	T4 Hip	8 / 12	35-02-00	7-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	309.1 195.67		
	2	T5 Hip	8 / 12	35-02-00	8-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	316.76 196.67		
	2	T6 Hip	8 / 12	35-02-00	9-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	329.25 205.00		
	2	T7 Hip	8 / 12	35-02-00	10-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	343.15 215.33		
	2	T8 Hip	8 / 12	35-02-00	11-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	332.52 206.67		
	1	T9 Hip Girder	8 / 12	16-04-00	3-11-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	77.68 49.67		
	2	T10 Hip	8 / 12	16-04-00	6-07-13	2 x 4	1-03-08 1-03-08	2-08-13 2-08-13	149.72 97.33		
	2	T11 Hip	8 / 12	16-04-00	7-11-13	2 x 4	1-03-08 1-03-08	2-08-13 2-08-13	161.88 103.67		
	3	T12 Common	8 / 12	16-04-00	7-06-02	2 x 4		1-04-13 2-08-13	214.34 135.00		
	1	T13 Hip Girder	8 / 12	8-03-00	3-11-13	2 x 4	1-03-08	1-04-13 1-04-13	34.02 22.00		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST				
	Lumber Yard:	TAMARACK LUMBER		Job Track:	51225
	Builder:	GREEN PARK HOMES		PlanLog:	202395
	Project:	RUSSELL GARDENS PH.3		Layout ID:	408131
	Location:	WATERDOWN		Ref #:	
	Model:	MOUNTAINASH 3		Page:	2 of 2
Lot #:	176		Date:	04-27-2020	
Elevation:	1 -STD OR OPT.5 BR		Designer:		
			Sales Rep:	Mario DiCano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE #	LOAD BY
	2	T14 Hip	8 /12	9-06-00	5-03-13	2 x 4	1-03-08	2-08-13 2-10-13	99.24 65.00		
	1	T15 Hip Girder	8 /12	9-09-00	2-07-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	42.84 27.83		
	1 2-ply	T16 Flat Girder	0 /12	5-10-08	1-04-00	2 x 4		1-04-00 1-04-00	40.07 25.67		
	1 2-ply	T17 Flat Girder	0 /12	18-04-00	1-09-07	2 x 6 2 x 4		1-09-07 1-09-07	149.42 94.00		
	21	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	352.64 224.00		
	4	J2 Jack-Open	8 /12	3-10-08	3-11-13	2 x 4	1-03-08	1-04-13 3-11-13	56.68 36.00		
	4	J3 Jack-Open	8 /12	1-10-08	2-07-13	2 x 4	1-03-08	1-04-13 2-07-13	37.17 22.67		
	10	J4 Jack-Open	4 /12	4-04-08	2-02-06	2 x 4	1-03-08	3-15 1-09-07	119.11 80.00		
	3	C1 Jack-Open	8 /12	1-10-15	2-08-02	2 x 4	1-03-08 1-11-09	1-04-13 2-08-02	34.33 23.00		
	3	C2 Jack-Open	8 /12	1-10-15	2-08-02	2 x 4	1-03-08 1-01	1-04-13 2-08-02	28.72 19.00		

TOTAL # TRUSS= 79

TOTAL BFT OF ALL TRUSSES= 2849.85 BFT.

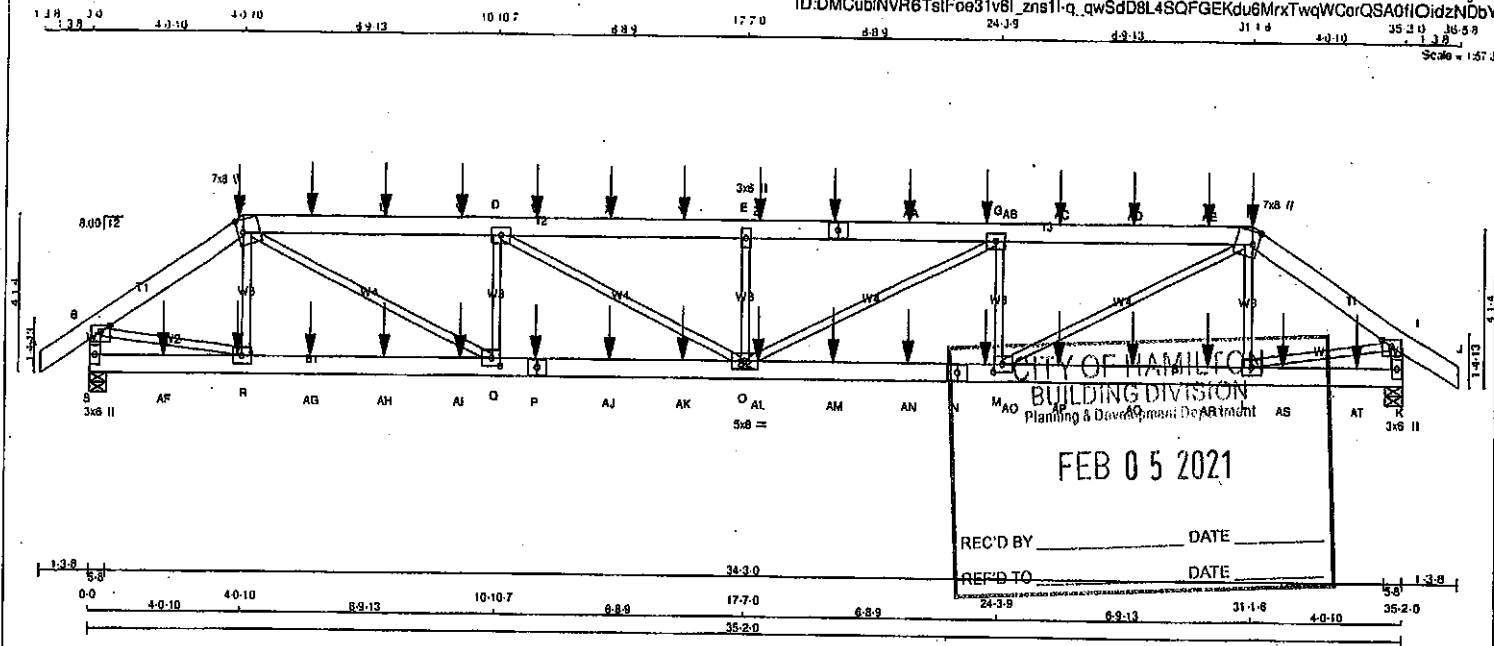
TOTAL WEIGHT OF ALL TRSSES 4530.03 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
2	Hardware	LJS26DS	
3	Hardware	LUS24	
3	Hardware	LUS26-2	

TOTAL NUMBER OF
ITEMS=

9



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x8	DRY	No.2	SPF
C - F	2x8	DRY	No.2	SPF
F - H	2x8	DRY	No.2	SPF
H - J	2x8	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
K - L	2x4	DRY	No.2	SPF
L - M	2x8	DRY	No.2	SPF
M - N	2x8	DRY	No.2	SPF
N - K	2x8	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C	2	12
C-F	2	12
F-H	2	12
H-J	2	12
J-K	1	12
K-L	1	12
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
S-P	2	12
P-N	2	12
N-K	2	12
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	1	8

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
S	3397	0	3397	0
K	3346	0	3346	0

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
S	2400	1588 / 0
K	2368	1557 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX FACTORED CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 36	-91.8	-91.8	0.04 (1)	10.00	R-C	-518 / 0	0.06 (1)
B-C	-4152 / 0	-91.8	-91.8	0.09 (1)	5.52	C-Q	0 / 3920	0.49 (1)
C-T	-6892 / 0	-91.8	-91.8	0.33 (1)	4.29	Q-D	-1649 / 0	0.20 (1)
T-U	-6892 / 0	-91.8	-91.8	0.33 (1)	4.29	D-O	0 / 1172	0.15 (1)
U-V	-6892 / 0	-91.8	-91.8	0.33 (1)	4.29	O-E	-918 / 0	0.11 (1)
V-D	-6892 / 0	-91.8	-91.8	0.33 (1)	4.29	O-G	0 / 1203	0.15 (1)
D-W	-7922 / 0	-91.8	-91.8	0.36 (1)	4.02	M-G	-1670 / 0	0.20 (1)
W-X	-7922 / 0	-91.8	-91.8	0.36 (1)	4.02	M-H	0 / 3993	0.49 (1)
X-Y	-7922 / 0	-91.8	-91.8	0.36 (1)	4.02	L-H	-501 / 0	0.08 (1)
Y-E	-7922 / 0	-91.8	-91.8	0.36 (1)	4.02	B-R	0 / 3525	0.44 (1)
E-Z	-7922 / 0	-91.8	-91.8	0.36 (1)	4.02	L-I	0 / 3457	0.43 (1)
Z-F	-7922 / 0	-91.8	-91.8	0.36 (1)	4.02			
F-AA	-7922 / 0	-91.8	-91.8	0.36 (1)	4.02			
AA-AB	-7922 / 0	-91.8	-91.8	0.36 (1)	4.02			
AB-G	-7922 / 0	-91.8	-91.8	0.36 (1)	4.02			
G-AC	-6865 / 0	-91.8	-91.8	0.33 (1)	4.30			
AC-AD	-6865 / 0	-91.8	-91.8	0.33 (1)	4.30			
AD-AE	-6865 / 0	-91.8	-91.8	0.33 (1)	4.30			
AE-H	-6865 / 0	-91.8	-91.8	0.33 (1)	4.30			
H-I	-4072 / 0	-91.8	-91.8	0.09 (1)	5.56			
I-J	0 / 36	-91.8	-91.8	0.04 (1)	10.00			
S-B	-3376 / 0	0.0	0.0	0.19 (1)	8.35			
K-I	-3317 / 0	0.0	0.0	0.18 (1)	8.39			

S-AF	0 / 0	-18.5	-18.5	0.06 (4)	10.00
AF-R	0 / 0	-18.5	-18.5	0.06 (4)	10.00
R-AG	0 / 3431	-18.5	-18.5	0.27 (1)	10.00
AG-AH	0 / 3431	-18.5	-18.5	0.27 (1)	10.00
AH-AI	0 / 3431	-18.5	-18.5	0.27 (1)	10.00
AI-AJ	0 / 3431	-18.5	-18.5	0.27 (1)	10.00
Q-P	0 / 6893	-18.5	-18.5	0.52 (1)	10.00
P-AJ	0 / 6893	-18.5	-18.5	0.52 (1)	10.00
AJ-AK	0 / 6893	-18.5	-18.5	0.52 (1)	10.00
AK-O	0 / 6893	-18.5	-18.5	0.52 (1)	10.00
O-AL	0 / 6865	-18.5	-18.5	0.51 (1)	10.00
AL-AM	0 / 6865	-18.5	-18.5	0.51 (1)	10.00
AM-AN	0 / 6865	-18.5	-18.5	0.51 (1)	10.00
AN-N	0 / 6865	-18.5	-18.5	0.51 (1)	10.00
N-AO	0 / 6865	-18.5	-18.5	0.51 (1)	10.00
AO-M	0 / 6865	-18.5	-18.5	0.51 (1)	10.00
M-AP	0 / 3365	-18.5	-18.5	0.27 (1)	10.00
AP-AQ	0 / 3365	-18.5	-18.5	0.27 (1)	10.00
AQ-AR	0 / 3365	-18.5	-18.5	0.27 (1)	10.00

TOTAL WEIGHT = 2 X 180 = 359 lb

DESIGN CRITERIA
*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/999 (0.20")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = L/999 (0.38")

CSI: TC=0.38/1.00 (D-E:1), BC=0.52/1.00 (O-Q:1), WB=0.49/1.00 (H-M:1), SS=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

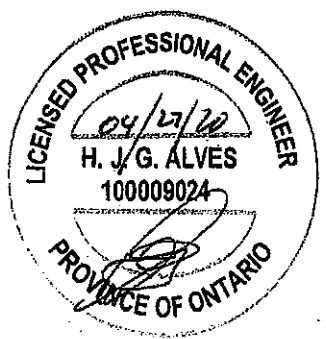
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) INPUT = 0.90
JSI METAL= 0.63 (N) INPUT = 1.00



Structural component only
DWG# T-2007017 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408131	T1	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Sat Apr 25 10:07:23 2020 Page 2
ID:DMCubINVR6TstFoe31v6l zns11-q qwSdD8L4SQF0EKdu6MrxTwqWCorQSA01QIdzND6Y

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	7.0	8.0	4.00	1.75
D	TMVW-t	MT20	5.0	8.0		
E	TMVW-w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	8.0		
G	TMVW-t	MT20	5.0	8.0		
H	TTWW+m	MT20	7.0	8.0	4.00	1.75
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMVW-t	MT20	5.0	8.0		
M	BMVW-t	MT20	5.0	8.0	2.50	2.75
N	BS-t	MT20	5.0	8.0		
O	BMVW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	8.0		
Q	BMVW-t	MT20	5.0	8.0	2.50	2.75
R	BMVW-t	MT20	5.0	8.0		
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: 4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. CS (LC)	MAX. UNBRAC. LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
FR-TO		FROM	TO				
AR-L	0 - 3365	-18.5	-18.5	0.27 (1)	10.00		
LA-S	0 0	-18.5	-18.5	0.07 (4)	10.00		
AS-AT	0 0	-18.5	-18.5	0.07 (4)	10.00		
AT-K	0 0	-18.5	-18.5	0.07 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-10	-49	-55	---	FRONT	VERT	DEAD	---	C1
C	4-0-10	-117	-117	---	BACK	VERT	TOTAL	---	C1
C	4-0-10	-254	-254	---	FRONT	VERT	SNOW	---	C1
F	19-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
H	31-1-6	-49	-55	---	FRONT	VERT	DEAD	---	C1
H	31-1-6	-254	-254	---	FRONT	VERT	SNOW	---	C1
P	11-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
R	3-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
T	5-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
U	7-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
V	9-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
W	11-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
X	13-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
Y	15-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
Z	17-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
AA	21-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
AB	23-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
AC	25-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
AD	27-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
AE	29-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
AF	1-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AG	5-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AH	7-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AI	9-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AJ	13-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AK	15-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AL	17-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AM	19-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AN	21-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AO	23-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AP	25-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AQ	27-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AR	29-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AS	31-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AT	33-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CITY OF HAMILTON
BUILDING DIVISION
Planning & Development Department

FEB 05 2021

REC'D BY _____ DATE _____
REF'D TO _____ DATE _____

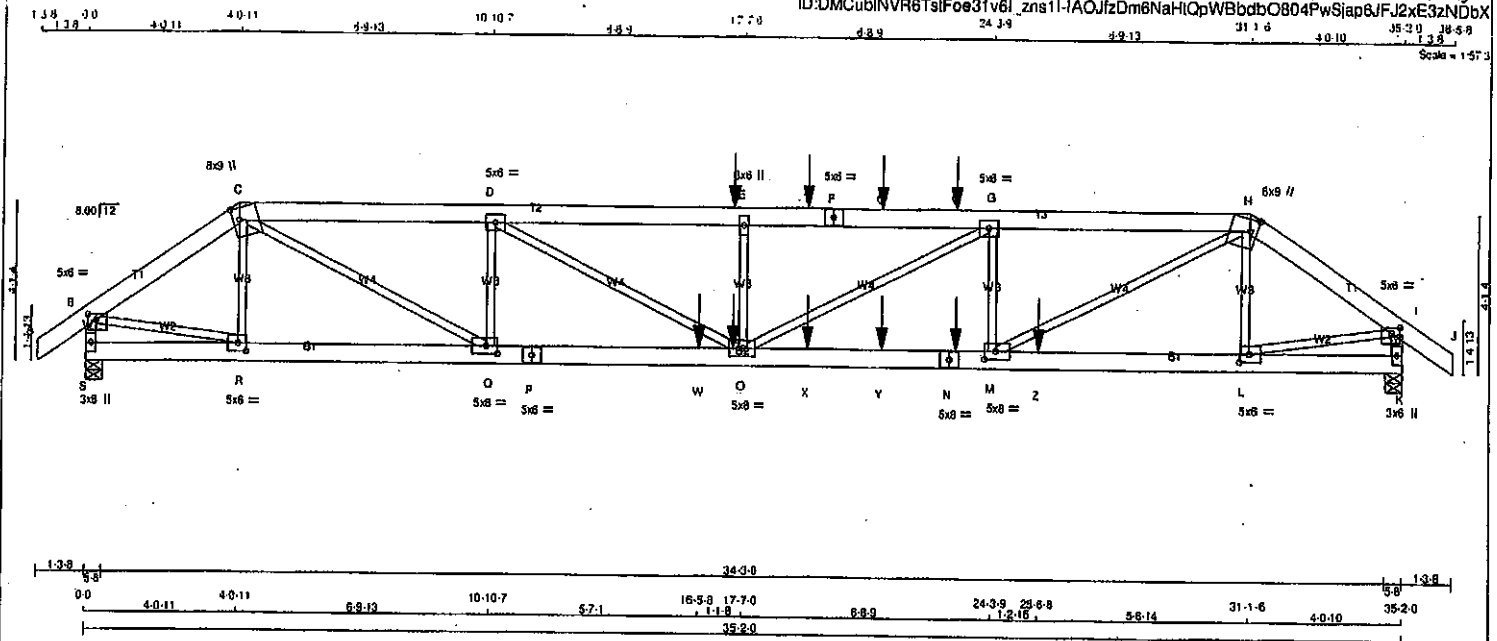


Structural component only
DWG# T-2007017

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408131	T1Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Sat Apr 25 10:07:24 2020 Page 1
ID:DMCubINVR6Tsf0e31v6l_zns11-1AOJfzDm6NaHlQpWBdbO804PwSjap6JFJ2xE3zNDbX



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6 DRY	No.2	SPF		
C - F	2x6 DRY	No.2	SPF		
F - H	2x6 DRY	No.2	SPF		
H - J	2x6 DRY	No.2	SPF		
S - B	2x4 DRY	No.2	SPF		
K - I	2x4 DRY	No.2	SPF		
S - P	2x6 DRY	No.2	SPF		
P - N	2x6 DRY	No.2	SPF		
N - K	2x6 DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 2	12	TOP
C - F 2	12	SIDE(183.1)
F - H 2	12	SIDE(0.0)
H - J 2	12	TOP
S - B 1	12	TOP
K - I 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S - P 2	12	TOP
P - N 2	12	SIDE(183.1)
N - K 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	UPLIFT
S	3388	0	3388	0
K	3900	0	3900	0

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
S	2383	1588 / 0	0 / 0	0 / 0	805 / 0	0 / 0
K	2757	1818 / 0	0 / 0	0 / 0	939 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING	TOTAL LOAD CASES: (4)
CHORDS	WEBS
MEMB.	MEMB.
MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO	FR-TO
FROM TO	FROM TO
FR-TO	FR-TO
A-B	0 / 36
B-C	-4130 / 0
C-D	-8076 / 0
D-E	-10755 / 0
E-F	-10755 / 0
F-G	-10755 / 0
G-H	-9242 / 0
H-I	-4959 / 0
I-J	0 / 36
J-K	-3359 / 0
K-L	-3970 / 0
S-R	0 / 0
R-Q	0 / 3408
Q-P	0 / 8076
P-W	0 / 8076
W-O	0 / 8076
O-X	0 / 9242
X-Y	0 / 9242
Y-N	0 / 9242
N-M	0 / 9242
M-Z	0 / 4102
Z-L	0 / 4102
L-K	0 / 0

FACTORED CONCENTRATED LOADS (LBS)	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	17-4.4	-110	-110	-110	FRONT	VERT	TOTAL	...	C1
N	23-4.4	-26	-26	-26	FRONT	VERT	TOTAL	...	C1
O	17-4.4	-26	-26	-26	FRONT	VERT	TOTAL	...	C1
T	19-4.4	-110	-110	-110	FRONT	VERT	TOTAL	...	C1
U	21-4.4	-110	-110	-110	FRONT	VERT	TOTAL	...	C1
V	23-4.4	-110	-110	-110	FRONT	VERT	TOTAL	...	C1
W	16-5.8	-1480	-1480	-1480	FRONT	VERT	TOTAL	...	C1
X	19-4.4	-26	-26	-26	FRONT	VERT	TOTAL	...	C1
Y	21-4.4	-26	-26	-26	FRONT	VERT	TOTAL	...	C1
Z	25-6.8	-1144	-1144	-1144	FRONT	VERT	TOTAL	...	C1

TOTAL WEIGHT = 2 X 180 = 359 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. - G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17)
CALCULATED VERT. DEFL. (LL) = U/999 (0.30")
ALLOWABLE DEFL. (TL) = L/360 (1.17)
CALCULATED VERT. DEFL. (TL) = U/775 (0.54")

CSI: TC=0.44/1.00 (E-G:1), BC=0.92/1.00 (O-Q:1), WB=0.72/1.00 (H-M:1), SS=0.43/1.00 (O-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

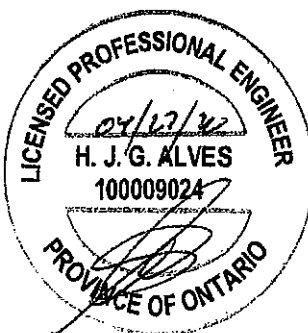
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.86 (N) (INPUT = 1.00)



Structural component only
DWG# T-2007018 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408131	T1Z	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR6TstFoe31v6I zns1I-IAOJfzDm8NaHIQpWBbDbO804PwSjap6JFJ2xE3zNDbX

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TTWW+m	MT20	8.0	9.0	3.75	2.00
D	TMVW-t	MT20	5.0	6.0		
E	TMVW-w	MT20	3.0	6.0		
F	TS-1	MT20	5.0	6.0		
G	TMVW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	8.0	9.0	3.75	2.00
I	TMVW-p	MT20	5.0	6.0	1.75	2.75
K	BMV1+p	MT20	3.0	6.0		
L	BMVW-1	MT20	5.0	6.0	2.50	2.75
M	BMVW-1	MT20	5.0	6.0	2.50	3.50
N	BS-1	MT20	5.0	6.0		
O	BMVWW-1	MT20	5.0	6.0		
P	BS-1	MT20	5.0	6.0		
Q	BMVW-1	MT20	5.0	6.0	2.50	3.50
R	BMVW-1	MT20	5.0	6.0	2.50	2.75
S	BMV1+p	MT20	3.0	6.0		

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

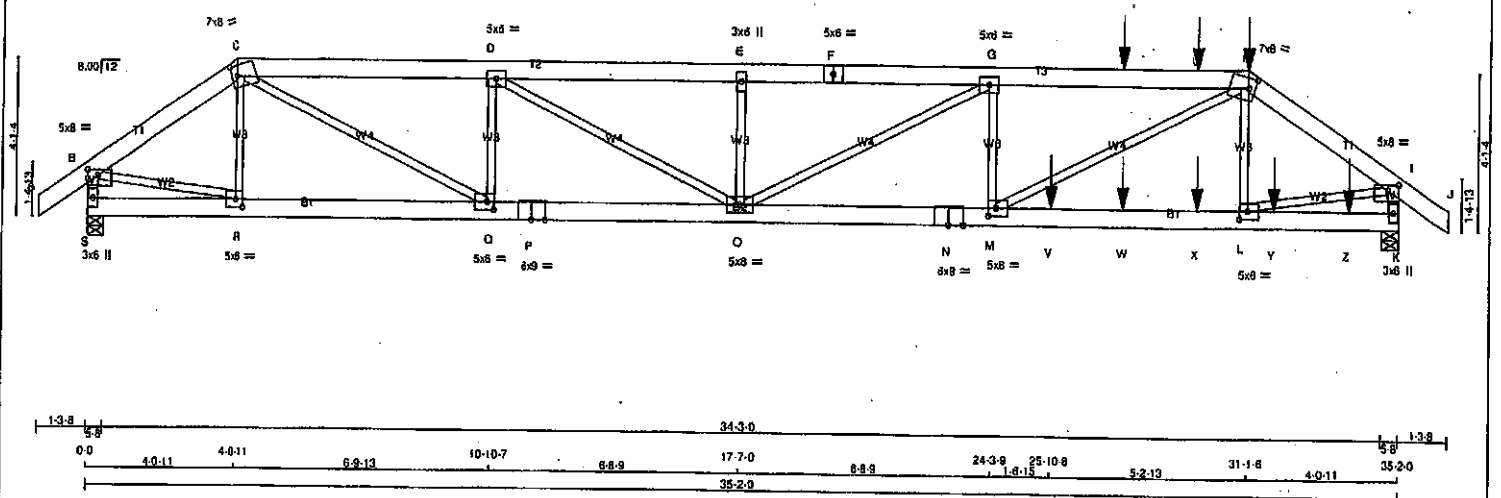
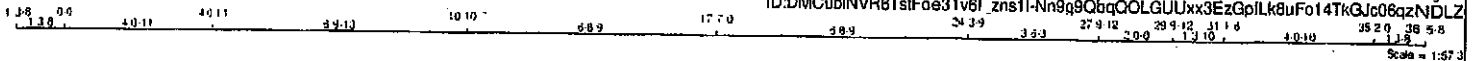


Structural component only
DWG# T-2007018 *2/2*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408132	T1Z2	1	2	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:24:28 2020 Page 1
ID:DMCubINVR6TstFos3Iv6I zns1I-Nn9g9QbqCOLGUUxx3EzGpIk8uFo14TkGJc08qzNDLZ



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF		
C - F	2x6	DRY	No.2	SPF		
F - H	2x6	DRY	No.2	SPF		
H - J	2x6	DRY	No.2	SPF		
S - B	2x4	DRY	No.2	SPF		
K - I	2x4	DRY	No.2	SPF		
S - P	2x6	DRY	No.2	SPF		
P - N	2x6	DRY	No.2	SPF		
N - K	2x6	DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 2	12	TOP
C - F 2	12	TOP
F - H 2	12	TOP
H - J 2	12	TOP
S - B 1	12	TOP
K - I 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S - P 2	12	TOP
P - N 2	12	TOP
N - K 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT 2892	DOWN 2892	0	5-8
S	HORZ 0	HORZ 0	0	5-8
K	VERT 4101	DOWN 4101	0	5-8

UNFACTORED REACTIONS

1ST LOSE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1900	1271 / 0	0 / 0	0 / 0	0 / 0	629 / 0	0 / 0
K	2893	1930 / 0	0 / 0	0 / 0	0 / 0	955 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORZ. LOAD (LBS)	MAX. FACTORED UNBRAC. LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LBS)
FR-TO						FR-TO			
A-B	0 / 38	-91.8	-91.8	0.04 (1)	10.00	R-C	-442 / 0	0.05 (1)	
B-C	-3201 / 0	-91.8	-91.8	0.08 (1)	6.10	C-Q	0 / 3750	0.46 (1)	
C-D	-5955 / 0	-91.8	-91.8	0.22 (1)	4.69	Q-D	-1603 / 0	0.19 (1)	
D-E	-7689 / 0	-91.8	-91.8	0.25 (1)	4.19	D-O	0 / 1951	0.24 (1)	
E-F	-7689 / 0	-91.8	-91.8	0.25 (1)	4.20	O-E	-575 / 0	0.07 (1)	
F-G	-7689 / 0	-91.8	-91.8	0.25 (1)	4.20	O-G	-915 / 0	0.53 (1)	
G-H	-8473 / 0	-91.8	-91.8	0.40 (1)	3.87	M-G	-301 / 2	0.04 (1)	
H-I	-8473 / 0	-91.8	-91.8	0.40 (1)	3.87	M-H	0 / 4612	0.57 (1)	
I-J	-5308 / 0	-91.8	-91.8	0.11 (1)	5.00	L-H	-128 / 99	0.02 (4)	
J-K	0 / 38	-91.8	-91.8	0.04 (1)	10.00	B-R	0 / 2717	0.34 (1)	
S-B	-2875 / 0	0.0	0.0	0.15 (1)	6.97	L-I	0 / 4508	0.56 (1)	
K-I	-4227 / 0	0.0	0.0	0.23 (1)	5.78				

S-R	0 / 0	-18.5	-18.5	0.04 (4)	10.00
R-Q	0 / 2644	-18.5	-18.5	0.20 (1)	10.00
Q-P	0 / 5955	-18.5	-18.5	0.48 (1)	10.00
P-O	0 / 5955	-18.5	-18.5	0.48 (1)	10.00
O-N	0 / 8473	-18.5	-18.5	0.86 (1)	10.00
N-M	0 / 8473	-18.5	-18.5	0.86 (1)	10.00
M-V	0 / 4401	-18.5	-18.5	0.82 (1)	10.00
V-W	0 / 4401	-18.5	-18.5	0.82 (1)	10.00
W-X	0 / 4401	-18.5	-18.5	0.82 (1)	10.00
X-L	0 / 4401	-18.5	-18.5	0.82 (1)	10.00
L-Y	0 / 0	-18.5	-18.5	0.29 (1)	10.00
Y-Z	0 / 0	-18.5	-18.5	0.29 (1)	10.00
Z-K	0 / 0	-18.5	-18.5	0.29 (1)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	31-1-6	-49	-55	---	FRONT	VERT	DEAD	---	C1
H	31-1-6	-254	-254	---	FRONT	VERT	SNOW	---	C1
T	27-9-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
U	29-9-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	25-10-8	-2027	-2027	---	FRONT	VERT	TOTAL	---	C1
W	27-9-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
X	29-9-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Y	31-9-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Z	33-9-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 180 = 359 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL	= 25.8 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCO 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, NBCO 2012, ABC 2019
- PART 9 OF NBCO 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.22")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.40")

CSI: TC=0.40/1.00 (G-H:1), BC=0.92/1.00 (L-M:1),
WB=0.57/1.00 (H-M:1), SS=0.54/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

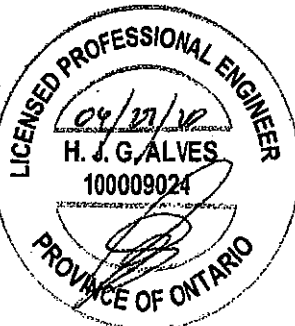
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1887 788	1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (C) (INPUT = 0.90)
JSI METAL = 0.81 (P) (INPUT = 1.00)



Structural component only
DWG# T-2007029 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408132	T1Z2	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 5 Oct 29 2019 MITek Industries, Inc. Sat Apr 25 10:24:28 2020 Page 2
ID:DMCubINVR6TslFos31v6I zns1I-Nn9a9QbaQOLGUUxx3EzGplLk8uFo14TkGJc08qzNDLZ

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	3.50
C	TTWW-m	MT20	7.0	8.0	3.25	2.00
D	TMVW-l	MT20	5.0	6.0		
E	TMVW-w	MT20	3.0	6.0		
F	TS-l	MT20	5.0	6.0		
G	TMVW-l	MT20	5.0	6.0		
H	TTWW-m	MT20	7.0	8.0	3.25	2.00
I	TMVW-p	MT20	5.0	8.0	1.75	3.50
K	BMV1-p	MT20	3.0	6.0		
L	BMVW-l	MT20	5.0	6.0	2.50	2.50
M	BMVW-l	MT20	5.0	6.0	2.50	2.25
N	BS-l	MT20	6.0	9.0		
O	BMVWW-l	MT20	5.0	6.0		
P	BS-l	MT20	6.0	9.0		
Q	BMVW-l	MT20	5.0	6.0	2.50	2.25
R	BMVW-l	MT20	5.0	6.0	2.50	2.50
S	BMV1-p	MT20	3.0	6.0		

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007029

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S. K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

LOADING

TOTAL LOAD CASES: (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	OL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	OL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (1.17")
CALCULATED VERT. DEFL.(LL) = $L/989$ (0.21")
ALLOWABLE DEFL.(TL)= $L/360$ (1.17")
CALCULATED VERT. DEFL.(TL) = $L/989$ (0.39")

CSL: TC=0.88/1.00 (E-G:1), BC=0.58/1.00 (O-Q:1),
WB=0.44/1.00 (B-R:1), SSL=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

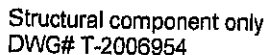
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

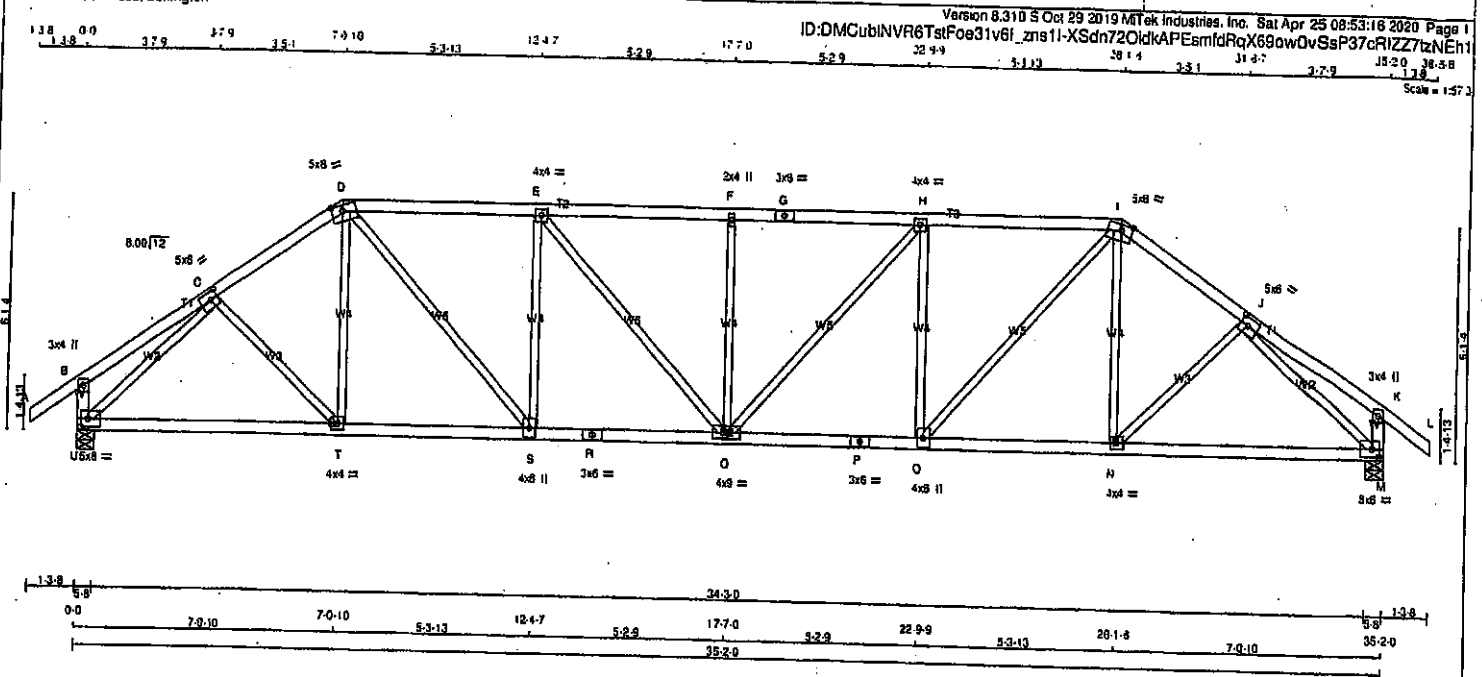
NAIL VALUES	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PL)	(PSI)	(PL)	(PSI)	(PL)
MT20	618	354	1667	788	1887	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP = 0.87 (B) ; INPUT = 0.90 ;
ISI METAL = 0.74 (P) (INPUT = 1.00)





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2 SPF
D - G	2x4	DRY	No.2 SPF
G - I	2x4	DRY	No.2 SPF
I - L	2x4	DRY	No.2 SPF
U - B	2x4	DRY	No.2 SPF
M - K	2x4	DRY	No.2 SPF
U - R	2x4	DRY	No.2 SPF
R - P	2x4	DRY	No.2 SPF
P - M	2x4	DRY	No.2 SPF
ALL WEBS	2x3	DRY	No.2 SPF
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-1	MT20	5.0	8.0	2.50	2.50
D	TTWV-m	MT20	5.0	8.0	1.75	3.50
E	TMVW-1	MT20	4.0	4.0		
F	TMVW-w	MT20	2.0	4.0		
G	TS-1	MT20	3.0	6.0		
H	TMVW-1	MT20	4.0	4.0		
I	TTWV-m	MT20	5.0	8.0	1.75	3.50
J	TMVW-1	MT20	8.0	8.0	2.50	2.50
K	TMV+p	MT20	3.0	4.0		
M	BMVW-1	MT20	5.0	8.0	2.50	2.25
N	BMVW-1	MT20	4.0	4.0		
O	BMVW-1	MT20	4.0	6.0		
P	BS-1	MT20	3.0	6.0		
Q	BMVW-1	MT20	4.0	9.0		
R	BS-1	MT20	3.0	8.0		
S	BMVW-1	MT20	4.0	8.0		
T	BMVW-1	MT20	4.0	4.0		
U	BMVW-1	MT20	5.0	8.0	2.50	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	REQD
U	2085	0	2085	0	0	5-8	5-8
M	2085	0	2085	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1455	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
M	1455	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PL)	FACTORED LC1 MAX CSI (LC)	MEMB.	FORCE (LBS)	VERT. LOAD (PL)	FACTORED LC1 MAX CSI (LC)
FR-TO				FR-TO			
A-B	0 / 35	-91.8	-91.8 0.12 (1)	C-T	0 / 135	0.03 (4)	
B-C	0 / 18	-91.8	-91.8 0.16 (1)	T-D	0 / 90	0.03 (4)	
C-D	-2308 / 0	-91.8	-91.8 0.23 (1)	D-S	0 / 1218	0.27 (1)	
D-E	-2724 / 0	-91.8	-91.8 0.54 (1)	S-E	-814 / 0	0.48 (1)	
E-F	-2987 / 0	-91.8	-91.8 0.58 (1)	E-O	0 / 383	0.08 (1)	
F-G	-2987 / 0	-91.8	-91.8 0.58 (1)	O-F	-440 / 0	0.28 (1)	
G-H	-2987 / 0	-91.8	-91.8 0.58 (1)	H-O	0 / 383	0.08 (1)	
H-I	-2724 / 0	-91.8	-91.8 0.54 (1)	O-H	-814 / 0	0.48 (1)	
I-J	-2308 / 0	-91.8	-91.8 0.23 (1)	H-I	0 / 1218	0.27 (1)	
J-K	0 / 18	-91.8	-91.8 0.16 (1)	I-J	0 / 90	0.03 (4)	
K-L	0 / 35	-91.8	-91.8 0.12 (1)	J-K	0 / 135	0.03 (4)	
U-B	-255 / 0	0.0	0.0 0.03 (1)	U-C	-2515 / 0	0.97 (1)	
M-K	-255 / 0	0.0	0.0 0.03 (1)	J-M	-2515 / 0	0.97 (1)	
U-T	0 / 1802	-18.5	-18.5 0.40 (1)				
T-S	0 / 1801	-18.5	-18.5 0.41 (1)				
S-R	0 / 2724	-18.5	-18.5 0.49 (1)				
R-Q	0 / 2724	-18.5	-18.5 0.49 (1)				
Q-P	0 / 2724	-18.5	-18.5 0.49 (1)				
P-O	0 / 2724	-18.5	-18.5 0.49 (1)				
O-N	0 / 1801	-18.5	-18.5 0.41 (1)				
N-M	0 / 1802	-18.5	-18.5 0.40 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/989 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/989 (0.29")

CSI: TC=0.58/1.00 (E-F:1), BC=0.49/1.00 (Q-S:1),
WB=0.97/1.00 (C-U:1), SS=0.23/1.00 (D-E:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PS)	SECTION (PL)
MT20	618	354
	1687	768
	1987	1656

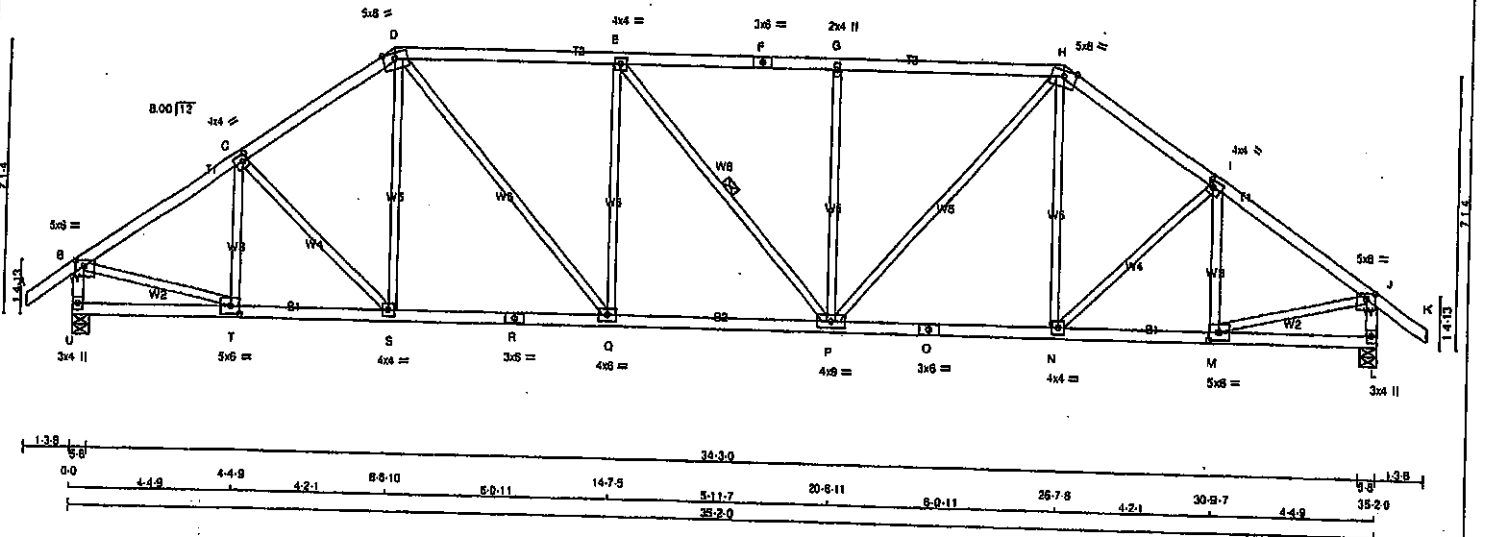
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.80 (U) (INPUT = 0.90)
JSI METAL = 0.84 (P) (INPUT = 1.00)



Structural component only
DWG# T-2006055



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER

A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
U - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
U - R	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	1.75	2.75
C	TMWW-l	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	Edge	
E	TMWW-l	MT20	4.0	4.0		
F	TS-l	MT20	3.0	4.0		
G	TMWW-l	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	Edge	
I	TMWW-l	MT20	4.0	4.0	2.00	1.50
J	TMWW-p	MT20	5.0	6.0	1.75	2.75
L	BMV1-p	MT20	3.0	4.0		
M	BMWW-l	MT20	5.0	6.0	2.50	2.75
N	BMWW-l	MT20	4.0	4.0		
O	BS-l	MT20	3.0	6.0		
P	BMWW-l	MT20	4.0	9.0		
Q	BMWW-l	MT20	4.0	8.0		
R	BS-l	MT20	3.0	6.0		
S	BMWW-l	MT20	4.0	4.0		
T	BMWW-l	MT20	5.0	6.0	2.50	2.75
U	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
U	2065	0	2065	0	5-8	5-8
L	2065	0	2065	0	5-8	5-8

UNFACTORED REACTIONS

JT	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
U	1458	970/0	0/0	0/0	0/0	488/0
L	1458	970/0	0/0	0/0	0/0	488/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX. UNBRACED LENGTH FR-TO	MEMB. MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
		FROM	TO			
FR-TO						
A-B	0/35	-91.8	-91.8	0.12 (1)	10.00	T-C -396/0 0.11 (1)
B-C	-2270/0	-91.8	-91.8	0.37 (1)	4.19	C-S -79/0 0.05 (1)
C-D	-2254/0	-91.8	-91.8	0.39 (1)	4.21	S-D 0/156 0.04 (4)
D-E	-2485/0	-91.8	-91.8	0.84 (1)	3.72	D-Q 0/849 0.21 (1)
E-F	-2484/0	-91.8	-91.8	0.84 (1)	3.72	Q-E -597/0 0.52 (1)
F-G	-2484/0	-91.8	-91.8	0.84 (1)	3.72	E-P -2/0 0.00 (1)
G-H	-2484/0	-91.8	-91.8	0.83 (1)	3.74	P-G -597/0 0.52 (1)
H-I	-2254/0	-91.8	-91.8	0.38 (1)	4.21	P-H 0/947 0.21 (1)
I-J	-2270/0	-91.8	-91.8	0.37 (1)	4.19	N-H 0/157 0.04 (4)
J-K	0/35	-91.8	-91.8	0.12 (1)	10.00	N-I -78/0 0.05 (1)
U-B	-2027/0	0.0	0.0	0.21 (1)	5.94	M-I -397/0 0.11 (1)
L-J	-2028/0	0.0	0.0	0.21 (1)	5.94	B-T 0/1966 0.44 (1)
U-T	0/0	-18.5	-18.5	0.08 (4)	10.00	M-J 0/1966 0.44 (1)
T-S	0/1909	-18.5	-18.5	0.37 (1)	10.00	
S-R	0/1854	-18.5	-18.5	0.38 (1)	10.00	
R-Q	0/1854	-18.5	-18.5	0.38 (1)	10.00	
Q-P	0/2485	-18.5	-18.5	0.47 (1)	10.00	
P-O	0/1854	-18.5	-18.5	0.38 (1)	10.00	
O-N	0/1854	-18.5	-18.5	0.38 (1)	10.00	
N-M	0/1909	-18.5	-18.5	0.37 (1)	10.00	
M-L	0/0	-18.5	-18.5	0.08 (4)	10.00	

TOTAL WEIGHT = 2 X 155 = 309 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN.C.C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, OBC 2012, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17)
 CALCULATED VERT. DEFL.(LL) = L/999 (0.12)
 ALLOWABLE DEFL.(TL) = L/360 (1.17)
 CALCULATED VERT. DEFL.(TL) = L/999 (0.24)

CS1: TC=0.84/1.00 (D-E:1), BC=0.47/1.00 (P-Q:1), WB=0.52/1.00 (E-Q:1), SS=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

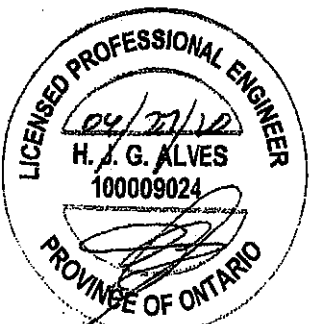
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 384 1867 788 1987 1858

PLATE PLACEMENT TOL = 0.250 inches

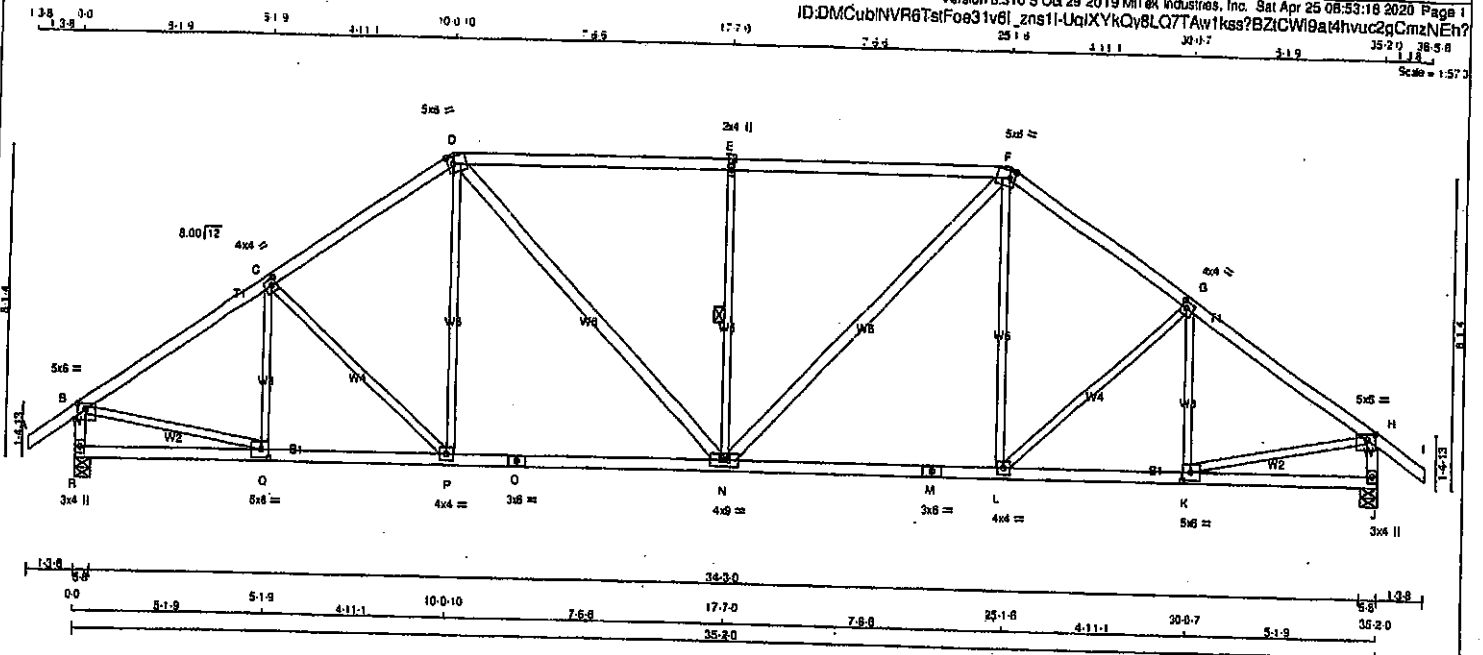
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.90 (D) (INPUT = 0.90)
 JSI METAL=0.62 (O) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408087	T5	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
TRUSS DESC.					

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Sat Apr 25 08:53:18 2020 Page 1
 ID:DMCubINVR6TsFoa31v6I_zns11-UqIXYkQy8LQ7TAw1kss?BZICW19at4hvuc2gCmzNEh?



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
H - J	2x4	DRY No.2	SPF
J - K	2x4	DRY No.2	SPF
K - L	2x4	DRY No.2	SPF
L - M	2x4	DRY No.2	SPF
M - N	2x4	DRY No.2	SPF
N - O	2x4	DRY No.2	SPF
O - P	2x4	DRY No.2	SPF
P - Q	2x4	DRY No.2	SPF
Q - R	2x4	DRY No.2	SPF
R - S	2x4	DRY No.2	SPF
S - T	2x4	DRY No.2	SPF
T - U	2x4	DRY No.2	SPF
U - V	2x4	DRY No.2	SPF
V - W	2x4	DRY No.2	SPF
W - X	2x4	DRY No.2	SPF
X - Y	2x4	DRY No.2	SPF
Y - Z	2x4	DRY No.2	SPF
Z - A	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	2.75
C	TMVW-l	MT20	4.0	4.0	2.00	1.50
D	TTVW-m	MT20	5.0	8.0	2.25	1.75
E	TMVW-w	MT20	2.0	4.0		
F	TTVW-m	MT20	5.0	8.0	2.25	1.75
G	TMVW-l	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	1.75	2.75
J	BMV1-p	MT20	3.0	4.0		
K	BMVW-l	MT20	5.0	8.0	2.50	2.75
L	BMVW-l	MT20	4.0	4.0		
M	BS-l	MT20	3.0	6.0		
N	BMVW-w	MT20	4.0	9.0		
O	BS-l	MT20	3.0	6.0		
P	BMVW-l	MT20	4.0	4.0		
Q	BMVW-l	MT20	5.0	8.0	2.50	2.75
R	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
DOWN	UP	DOWN	UP	IN-SX		IN-SX	
JT	VERT	HORZ					
R	2065	0	2065	0	5-8	5-8	
J	2065	0	2065	0	5-8	5-8	

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
J	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO	UNBRACED LENGTH	FR-TO		
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	Q-C	-321 / 0
B-C	-2318 / 0	-91.8	-91.8	0.35 (1)	4.18	C-P	-218 / 0
C-D	-2188 / 0	-91.8	-91.8	0.37 (1)	4.28	P-O	0 / 288
D-E	-2275 / 0	-91.8	-91.8	0.81 (1)	3.47	D-N	0 / 692
E-F	-2275 / 0	-91.8	-91.8	0.81 (1)	3.47	N-F	-851 / 0
F-G	-2188 / 0	-91.8	-91.8	0.37 (1)	4.28	N-F	0 / 692
G-H	-2318 / 0	-91.8	-91.8	0.38 (1)	4.16	L-F	0 / 288
H-I	0 / 35	-91.8	-91.8	0.12 (1)	10.00	L-G	-218 / 0
R-B	-2022 / 0	0.0	0.0	0.21 (1)	5.94	K-G	-321 / 0
J-H	-2022 / 0	0.0	0.0	0.21 (1)	5.94	B-Q	0 / 1998
						K-H	0 / 1998
R-Q	0 / 0	-18.5	-18.5	0.10 (4)	10.00		
O-P	0 / 1953	-18.5	-18.5	0.40 (1)	10.00		
P-O	0 / 1794	-18.5	-18.5	0.41 (1)	10.00		
O-N	0 / 1794	-18.5	-18.5	0.41 (1)	10.00		
N-M	0 / 1794	-18.5	-18.5	0.41 (1)	10.00		
M-L	0 / 1794	-18.5	-18.5	0.41 (1)	10.00		
L-K	0 / 1953	-18.5	-18.5	0.40 (1)	10.00		
K-J	0 / 0	-18.5	-18.5	0.10 (4)	10.00		

TOTAL WEIGHT = 2 X 158 = 317 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
 DL = 8.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
 ALLOWABLE DEFL.(TL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.81/1.00 (D-E-1), BC=0.41/1.00 (N-P-1), WB=0.49/1.00 (B-Q-1), SS=0.34/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 818 354 1867 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

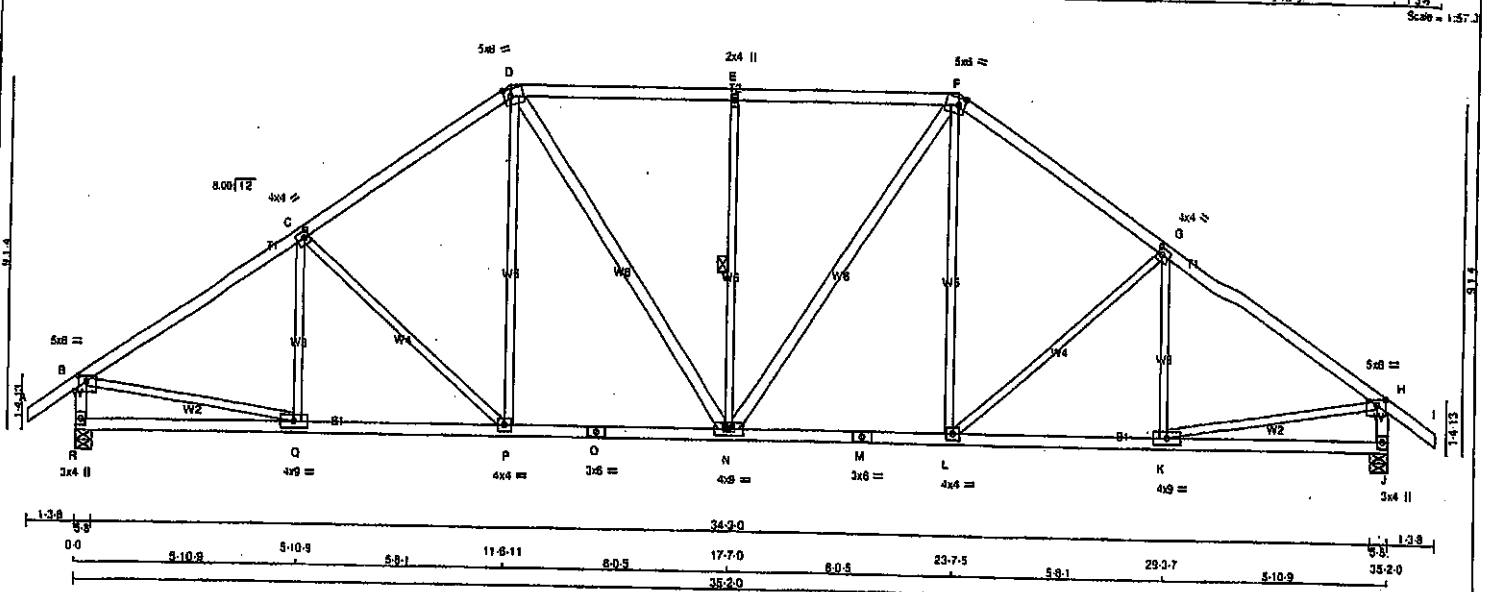
JSI GRIP = 0.87 (B) (INPUT = 0.90)
 JSI METAL = 0.57 (O) (INPUT = 1.00)



Structural component only
 DWG# T 0006057

JOB NAME 408087	TRUSS NAME T6	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - F	2x4	DRY No.2
F - I	2x4	DRY No.2
R - B	2x4	DRY No.2
J - H	2x4	DRY No.2
R - O	2x4	DRY No.2
O - M	2x4	DRY No.2
M - J	2x4	DRY No.2
ALL WEBS EXCEPT	2x3	DRY No.2
D - N	2x4	DRY No.2
N - F	2x4	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWW-p	MT20	5.0	6.0	1.75	2.75
C TMWW-l	MT20	4.0	4.0	2.00	1.50
D TTWW-m	MT20	5.0	6.0	2.25	2.00
E TMWW-w	MT20	2.0	4.0		
F TTWW-m	MT20	5.0	6.0	2.25	2.00
G TMWW-l	MT20	4.0	4.0	2.00	1.50
H TMWW-p	MT20	5.0	6.0	1.75	2.75
J BMV1-p	MT20	3.0	4.0		
K BMWW-l	MT20	4.0	9.0		
L BMWW-l	MT20	4.0	4.0		
M BS-l	MT20	3.0	6.0		
N BMWWW-l	MT20	4.0	6.0		
O BS-l	MT20	3.0	6.0		
P BMWW-l	MT20	4.0	4.0		
Q BMWW-l	MT20	4.0	9.0		
R BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
R	2065	0	2065	0	5-8	5-8
J	2065	0	2065	0	5-8	5-8

UNFACTORED REACTIONS							
JT	1ST LGASE COMBINED		MAX./MIN. COMPONENT REACTIONS				
	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
R	1458	970 / 0	0 / 0	0 / 0	488 / 0	0 / 0	
J	1458	970 / 0	0 / 0	0 / 0	488 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 35	-91.8 -91.8	0.12 (1)	10.00	Q-C	-248 / 11	0.11 (1)		
B-C	-2347 / 0	-91.8 -91.8	0.52 (1)	3.99	C-P	-356 / 0	0.42 (1)		
C-D	-2096 / 0	-91.8 -91.8	0.48 (1)	4.22	P-D	0 / 342	0.08 (1)		
D-E	-1885 / 0	-91.8 -91.8	0.49 (1)	4.26	D-N	0 / 477	0.08 (1)		
E-F	-1885 / 0	-91.8 -91.8	0.49 (1)	4.26	N-E	-677 / 0	0.37 (1)		
F-G	-2096 / 0	-91.8 -91.8	0.48 (1)	4.22	N-F	0 / 477	0.08 (1)		
G-H	-2347 / 0	-91.8 -91.8	0.52 (1)	3.99	L-F	0 / 342	0.08 (1)		
H-I	0 / 35	-91.8 -91.8	0.12 (1)	10.00	L-G	-356 / 0	0.42 (1)		
R-B	-2018 / 0	0.0 0.0	0.21 (1)	5.94	K-G	-248 / 11	0.11 (1)		
J-H	-2018 / 0	0.0 0.0	0.21 (1)	5.94	B-Q	0 / 2014	0.45 (1)		
					K-H	0 / 2014	0.45 (1)		
R-Q	0 / 0	-18.5 -18.5	0.14 (4)	10.00					
Q-P	0 / 1981	-18.5 -18.5	0.39 (1)	10.00					
P-O	0 / 1716	-18.5 -18.5	0.34 (1)	10.00					
O-N	0 / 1716	-18.5 -18.5	0.34 (1)	10.00					
N-M	0 / 1716	-18.5 -18.5	0.34 (1)	10.00					
M-L	0 / 1716	-18.5 -18.5	0.34 (1)	10.00					
L-K	0 / 1981	-18.5 -18.5	0.39 (1)	10.00					
K-J	0 / 0	-18.5 -18.5	0.14 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN O.C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPC 2011, TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/999 (0.10")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = L/999 (0.19")

CS1: TC=0.52/1.00 (B-C:1), BC=0.39/1.00 (K-L:1), WB=0.45/1.00 (H-K:1), SS1=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

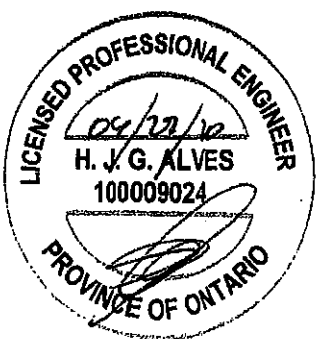
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.87 (B) (INPUT = 0.90)
JSI METAL=0.58 (M) (INPUT = 1.00)

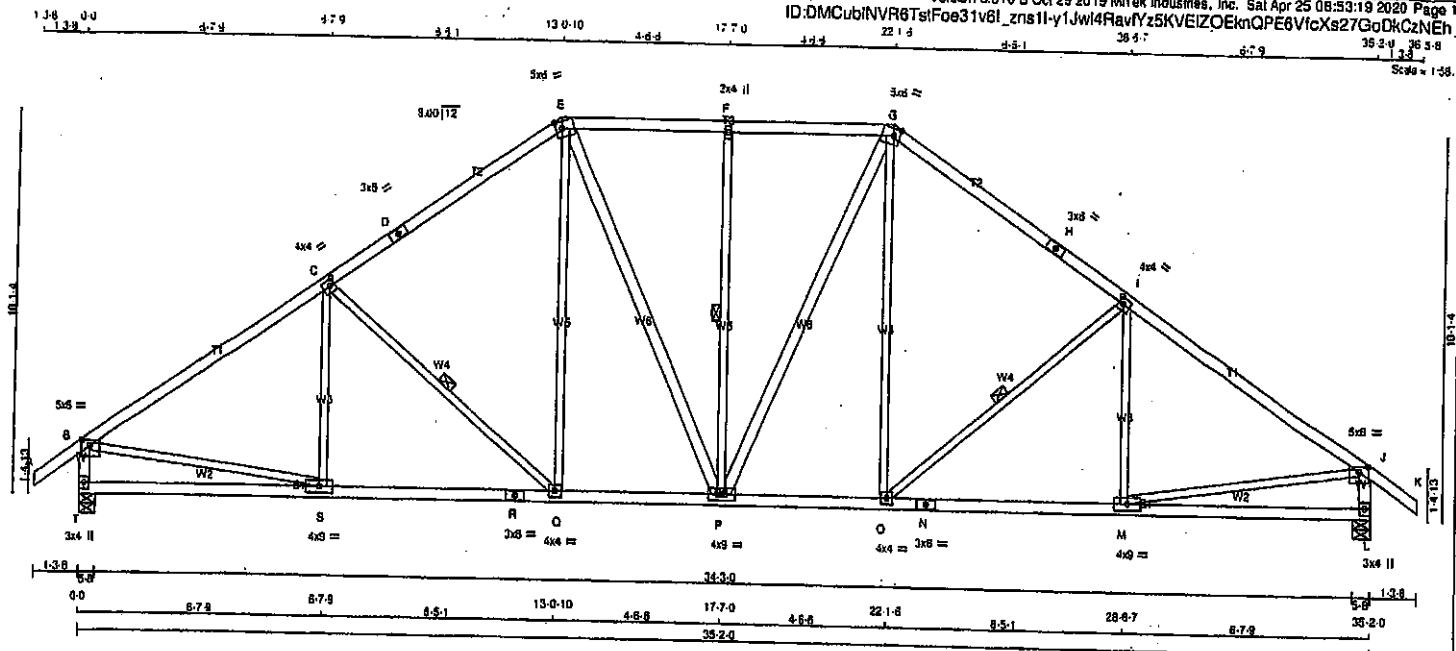


Structural component only

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ORWG NO.
408087	T7	3	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
T - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
T - R	2x4	DRY	No.2
R - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
E - P	2x4	DRY	No.2
P - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	2.75
C	TMVW-l	MT20	4.0	4.0	2.00	1.50
D	TS-l	MT20	3.0	8.0		
E	TTWW-m	MT20	5.0	8.0	2.25	2.00
F	TMVW-w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	2.25	2.00
H	TS-l	MT20	3.0	8.0		
I	TMVW-l	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	1.75	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMVW-l	MT20	4.0	9.0		
N	BS-l	MT20	3.0	8.0		
O	BMVW-l	MT20	4.0	9.0		
P	BMVW-l	MT20	4.0	9.0		
Q	BMVW-l	MT20	4.0	9.0		
R	BS-l	MT20	3.0	8.0		
S	BMVW-l	MT20	4.0	9.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
T	2065	0	2065	0	0	5-8	5-8		
L	2065	0	2065	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
T	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0						
L	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0						

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-Q, F-P, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MAX. UNBRACED	MEMB.	FORCE (LBS)	MAX. UNBRACED	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	LENGTH	FR-TO		FROM	TO	LENGTH
A-B	0 / 35	-81.8	-91.8	0.12 (1)	10.00	S-C	-189 / 46	0.10 (1)	
B-C	-2359 / 0	-91.8	-91.8	0.69 (1)	3.77	C-Q	-485 / 0	0.24 (1)	
C-D	-1998 / 0	-91.8	-91.8	0.82 (1)	4.11	Q-E	0 / 418	0.09 (1)	
D-E	-1998 / 0	-91.8	-91.8	0.82 (1)	4.11	E-P	0 / 315	0.05 (1)	
E-F	-1782 / 0	-91.8	-91.8	0.27 (1)	4.77	P-F	-502 / 0	0.35 (1)	
F-G	-1782 / 0	-91.8	-91.8	0.27 (1)	4.77	P-G	0 / 315	0.05 (1)	
G-H	-1998 / 0	-91.8	-91.8	0.82 (1)	4.11	C-G	0 / 418	0.09 (1)	
H-I	-1998 / 0	-91.8	-91.8	0.82 (1)	4.11	O-I	-485 / 0	0.24 (1)	
I-J	-2359 / 0	-91.8	-91.8	0.69 (1)	3.77	M-I	-189 / 46	0.10 (1)	
J-K	0 / 35	-91.8	-91.8	0.12 (1)	10.00	B-S	0 / 2022	0.45 (1)	
T-B	-2014 / 0	0.0	0.0	0.21 (1)	5.85	M-J	0 / 2022	0.45 (1)	
L-J	-2014 / 0	0.0	0.0	0.21 (1)	5.85				
T-S	0 / 0	-18.5	-18.5	0.19 (4)	10.00				
S-R	0 / 1998	-18.5	-18.5	0.42 (1)	10.00				
R-Q	0 / 1998	-18.5	-18.5	0.42 (1)	10.00				
Q-P	0 / 1830	-18.5	-18.5	0.32 (1)	10.00				
P-O	0 / 1830	-18.5	-18.5	0.32 (1)	10.00				
O-N	0 / 1998	-18.5	-18.5	0.42 (1)	10.00				
N-M	0 / 1998	-18.5	-18.5	0.42 (1)	10.00				
M-L	0 / 0	-18.5	-18.5	0.19 (4)	10.00				

TOTAL WEIGHT = 3 X 172 = 516 LBS

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF

DL = 8.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, CBC 2012, ABC 2019

- PART 9 OF CBC 2012 (2019 AMENDMENT)

- CSA 088-09, CSA 088-14

- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/380 (1.17")

CALCULATED VERT. DEFL. (LL) = L/999 (0.09")

ALLOWABLE DEFL. (TL) = L/380 (1.17")

CALCULATED VERT. DEFL. (TL) = L/999 (0.18")

CSI: TC=0.89/1.00 (I-J:1), BC=0.42/1.00 (M-O:1),

WB=0.45/1.00 (J-M:1), SS=0.24/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PL) (PL) (PL)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1657 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

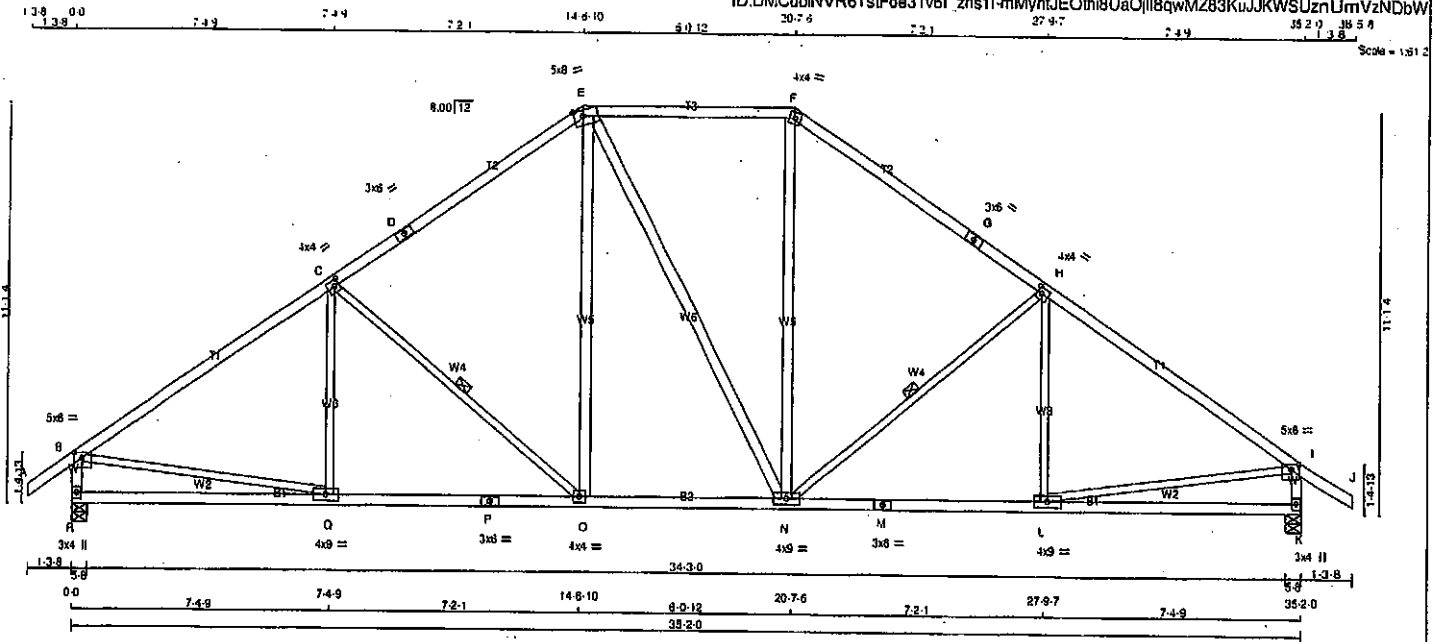
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.87 (B) (INPUT = 0.90)

JSI METAL = 0.59 (N) (INPUT = 1.00)



Structural component only
DWG# T-2006959



TOTAL WEIGHT = 2 X 166 = 333 lb [M/F]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
R - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
R - P	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
O - E	2x4	DRY	No.2
E - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN Y	X
B TMWV-p	MT20	5.0	6.0	1.75 2.75
C TMWW-1	MT20	4.0	4.0	2.00 1.50
D TS-1	MT20	3.0	6.0	
E TTWW-m	MT20	5.0	8.0	2.00 3.00
F TTW-m	MT20	4.0	4.0	
G TS-1	MT20	3.0	6.0	
H TMWV-1	MT20	4.0	4.0	2.00 1.50
I TMWV-p	MT20	5.0	6.0	1.75 2.75
K BMV1-p	MT20	3.0	4.0	
L BMWV-1	MT20	4.0	9.0	
M BS-1	MT20	3.0	6.0	
N BMWVW-1	MT20	4.0	4.0	
O BS-1	MT20	3.0	6.0	
P BS-1	MT20	3.0	6.0	
Q BMWV-1	MT20	4.0	9.0	
R BMV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
R	2085	0	2085	0	0	5-8	5-8	5-8	5-8
K	2085	0	2085	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS		PERM LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL	COMBINED	SNOW	LIVE	PERM LIVE
R	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0	1458	970 / 0	0 / 0	0 / 0
K	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0	1458	970 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	UNBRACED LENGTH
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-91.8 -91.8	0.12 (1)	10.00	Q-C	-139 / 76	0.09 (1)
B-C	-2358 / 0	-91.8 -91.8	0.89 (1)	3.45	C-O	-605 / 0	0.38 (1)
C-D	-1888 / 0	-91.8 -91.8	0.78 (1)	3.91	O-E	0 / 499	0.08 (1)
D-E	-1888 / 0	-91.8 -91.8	0.78 (1)	3.91	E-N	0 / 0	0.00 (1)
E-F	-1538 / 0	-91.8 -91.8	0.47 (1)	4.74	N-F	0 / 500	0.08 (1)
F-G	-1888 / 0	-91.8 -91.8	0.78 (1)	3.91	N-H	-604 / 0	0.38 (1)
G-H	-1888 / 0	-91.8 -91.8	0.78 (1)	3.91	H-I	-140 / 75	0.09 (1)
H-I	-2357 / 0	-91.8 -91.8	0.89 (1)	3.45	I-J	0 / 2020	0.45 (1)
I-J	0 / 35	-91.8 -91.8	0.12 (1)	10.00	L-I	0 / 2020	0.45 (1)
R-B	-2009 / 0	0.0 0.0	0.21 (1)	5.95			
K-I	-2009 / 0	0.0 0.0	0.21 (1)	5.95			
R-Q	0 / 0	-18.5 -18.5	0.25 (4)	10.00			
Q-P	0 / 1999	-18.5 -18.5	0.45 (1)	10.00			
P-O	0 / 1999	-18.5 -18.5	0.45 (1)	10.00			
O-N	0 / 1538	-18.5 -18.5	0.33 (1)	10.00			
N-M	0 / 1999	-18.5 -18.5	0.45 (1)	10.00			
M-L	0 / 1999	-18.5 -18.5	0.45 (1)	10.00			
L-K	0 / 0	-18.5 -18.5	0.24 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
BOT CH.	LL	= 6.0	PSF
	LL	= 0.0	PSF
	LL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2010, NBCO 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/999 (0.09")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = L/999 (0.20")

CSI: TC=0.89/1.00 (B-C:1), BC=0.45/1.00 (O-Q:1), WB=0.45/1.00 (B-C:1), SS=0.27/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (ORY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618	354
	1667	788
	1987	1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)

JSI METAL= 0.68 (P) (INPUT = 1.00)



Structural component only
DWG# T-2007019

Tamarack Roof Truss, Burlington

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DRY; SEASONED LUMBER.

TOTAL LOAD CASES: (4)

CHORDS				WEBBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LCI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LCI (LC)
FR-TO		FROM TO	CSI (LC)	FR-TO			
A-B	0/35	-91.8	-91.8	0.51 (4)	10.00	L-C	-218/28 0.05 (1)
B-C	-1751/0	-91.8	-91.8	0.33 (1)	4.88	C-K	0/874 0.22 (1)
C-N	-2118/0	-91.8	-91.8	0.42 (1)	4.17	K-D	-523/0 0.13 (1)
N-O	-2118/0	-91.8	-91.8	0.42 (1)	4.17	E-K	0/1120 0.28 (1)
D-E	-2118/0	-91.8	-91.8	0.42 (1)	4.17	I-E	-203/0 0.05 (1)
E-F	-1523/0	-91.8	-91.8	0.31 (1)	4.94	B-L	0/1498 0.37 (1)
F-G	0/35	-91.8	-91.8	0.14 (1)	10.00	I-F	0/1304 0.32 (1)
M-B	-1843/0	0.0	0.0	0.18 (1)	5.42		
H-F	-1469/0	0.0	0.0	0.16 (1)	6.71		
M-O	0/0	-18.5	-18.5	0.05 (4)	10.00		
O-L	0/0	-18.5	-18.5	0.05 (4)	10.00		
L-P	0/1448	-18.5	-18.5	0.26 (1)	10.00		
P-K	0/1448	-18.5	-18.5	0.26 (1)	10.00		
K-J	0/1259	-18.5	-18.5	0.23 (1)	10.00		
J-I	0/1259	-18.5	-18.5	0.23 (1)	10.00		
I-H	0/0	-18.5	-18.5	0.04 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX- MAX+	FACE	DIR.	TYPE	HEEL	CONN.	
K	3-10-8	255	255	---	FRONT	VERT	TOTAL	---	
C	7-11-8	-729	-729	---	FRONT	VERT	TOTAL	---	
N	4-0-4	-17	-17	---	FRONT	VERT	TOTAL	---	
L	5-0-4	-86	-86	---	FRONT	VERT	TOTAL	---	
O	2-0-12	-17	-17	---	FRONT	VERT	TOTAL	---	
P	5-0-4	-17	-17	---	FRONT	VERT	TOTAL	---	

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = 0.999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = 0.999 (0.08")

CSI: TC=0.42/1.00 (C-D:1), BC=0.26/1.00 (K-L:1),
WB=0.37/1.00 (B-L:1), SS=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP (DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	818	354	1867	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.40 (F) (INPUT = 1.00)

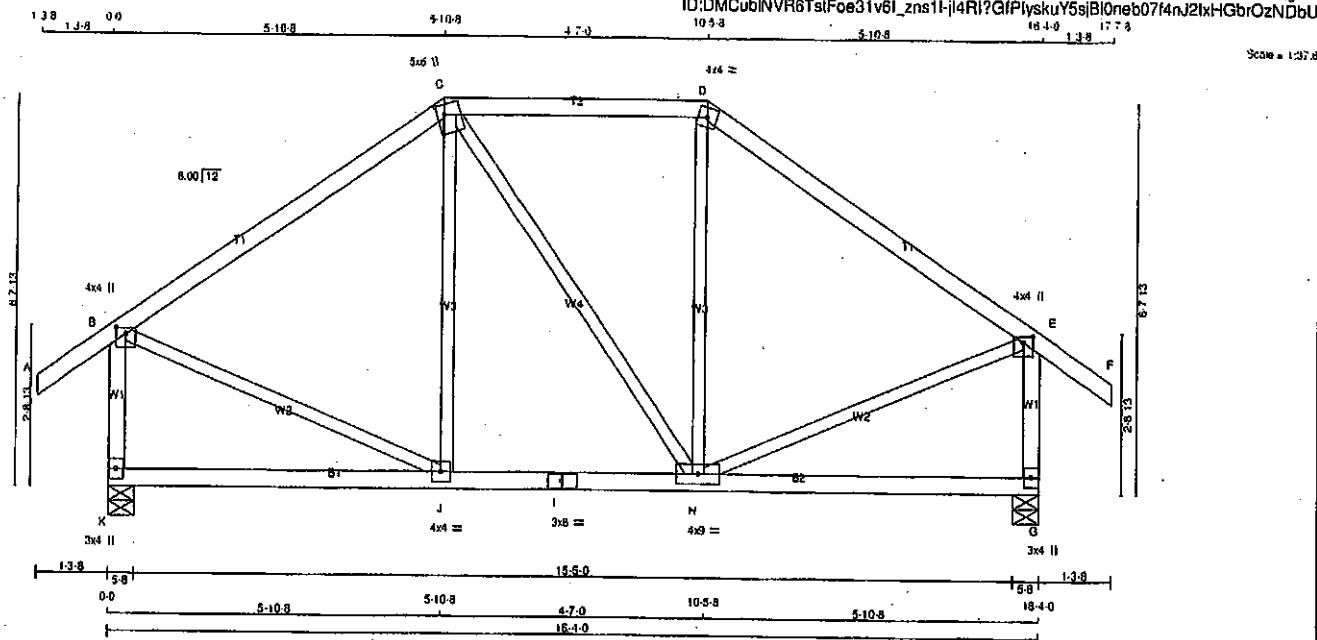


Structural component only
DWG# T-2007020

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408131	T10	2	1	GREEN PARK HOMES.	

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	No.2	SPF		
K - B	2x4	DRY	No.2	SPF		
G - E	2x4	DRY	No.2	SPF		
K - I	2x4	DRY	No.2	SPF		
I - G	2x4	DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (Tables in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMVt+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVt+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
K	1028	0	1028	0
G	1028	0	1028	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	723	488 / 0	0 / 0	0 / 0	0 / 0	0 / 0	235 / 0	0 / 0
G	723	488 / 0	0 / 0	0 / 0	0 / 0	0 / 0	235 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-91.8 -91.8	0.12 (1)	10.00	J-C	-115 / 40	0.08 (1)
B-C	-655 / 0	-91.8 -91.8	0.41 (1)	6.25	C-H	0 / 0	0.00 (1)
C-D	-542 / 0	-91.8 -91.8	0.25 (1)	6.25	H-O	-115 / 40	0.08 (1)
D-E	-655 / 0	-91.8 -91.8	0.41 (1)	6.25	B-J	0 / 587	0.13 (1)
E-F	0 / 35	-91.8 -91.8	0.12 (1)	10.00	H-E	0 / 587	0.13 (1)
K-B	-980 / 0	0.0 0.0	0.14 (1)	7.81			
G-E	-979 / 0	0.0 0.0	0.14 (1)	7.81			
K-J	0 / 0	-18.5 -18.5	0.14 (4)	10.00			
J-I	0 / 543	-18.5 -18.5	0.17 (4)	10.00			
I-H	0 / 543	-18.5 -18.5	0.17 (4)	10.00			
H-G	0 / 0	-18.5 -18.5	0.14 (4)	10.00			

TOTAL WEIGHT = 2 X 75 = 150 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 01.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/899 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/899 (0.05")

CSI: TC=0.41/1.00 (B-C:1), BC=0.17/1.00 (H-J:4), WB=0.13/1.00 (B-J:1), SS=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1687 788 1887 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.69 (B) (INPUT = 0.90)
JSI METAL = 0.21 (B) (INPUT = 1.00)

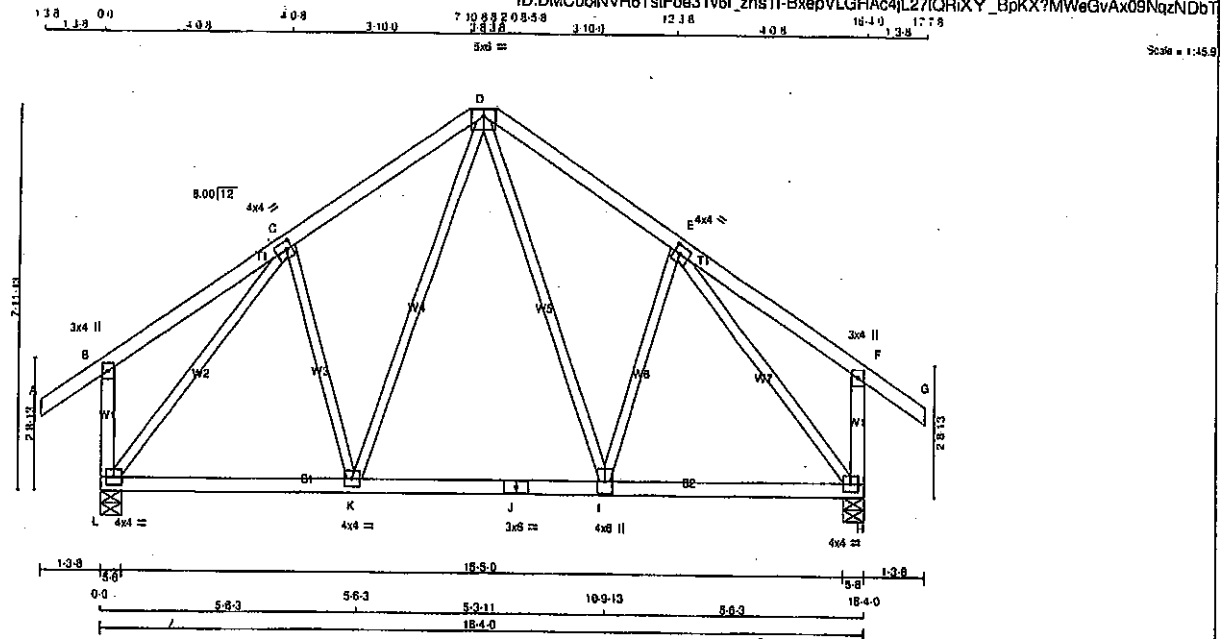


Structural component only
DWG# T-2007021

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408131	T11	2	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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Scale = 1:45.9

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR	
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0			
C	TMVW-1	MT20	4.0	4.0	2.00	1.50	
D	TMTMWm	MT20	5.0	6.0	1.50	3.00	
E	TMVW-1	MT20	4.0	4.0	2.00	1.50	
F	TMV-p	MT20	3.0	4.0			
H	BMVW-1	MT20	4.0	4.0			
I	BMVW-1	MT20	4.0	6.0			
J	BS-1	MT20	3.0	6.0			
K	BMVW-1	MT20	4.0	4.0			
L	BMVW-1	MT20	4.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
L	1026	0	1026	0	0	5-8	5-8	5-8	5-8
H	1026	0	1026	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST CASE		MAX/MIN COMPONENTS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL	DEAD	SOIL	SOIL
L	723	488 / 0	0 / 0	0 / 0	0 / 0	0 / 0	235 / 0	0 / 0	235 / 0	0 / 0	0 / 0
H	723	488 / 0	0 / 0	0 / 0	0 / 0	0 / 0	235 / 0	0 / 0	235 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM	TO	UNBRAC LENGTH	FR-TO			
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	C-K	-105 / 24	0.05 (1)
B-C	0 / 25	-91.8	-91.8	0.25 (1)	10.00	K-D	0 / 222	0.05 (1)
C-D	-871 / 0	-91.8	-91.8	0.20 (1)	8.25	D-I	0 / 222	0.05 (1)
D-E	-869 / 0	-91.8	-91.8	0.20 (1)	8.25	E-I	-105 / 24	0.05 (1)
E-F	0 / 25	-91.8	-91.8	0.25 (1)	10.00	L-C	-917 / 0	0.85 (1)
F-G	0 / 35	-91.8	-91.8	0.12 (1)	10.00	E-H	-917 / 0	0.85 (1)
L-B	-284 / 0	0.0	0.0	0.04 (1)	7.81			
H-F	-284 / 0	0.0	0.0	0.04 (1)	7.81			
L-K	0 / 567	-18.5	-18.5	0.17 (4)	10.00			
K-J	0 / 486	-18.5	-18.5	0.16 (4)	10.00			
J-I	0 / 466	-18.5	-18.5	0.16 (4)	10.00			
I-H	0 / 567	-18.5	-18.5	0.17 (4)	10.00			

TOTAL WEIGHT = 2 X 81 = 162 lb

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.54")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.54")
CALCULATED VERT. DEFL. (TL) = L/999 (0.04")

CSI: TC=0.25/1.00 (E-F:1), BC=0.17/1.00 (H-I:4), WB=0.65/1.00 (C-L:1), SSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1657 788	1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.34 (C) (INPUT = 1.00)

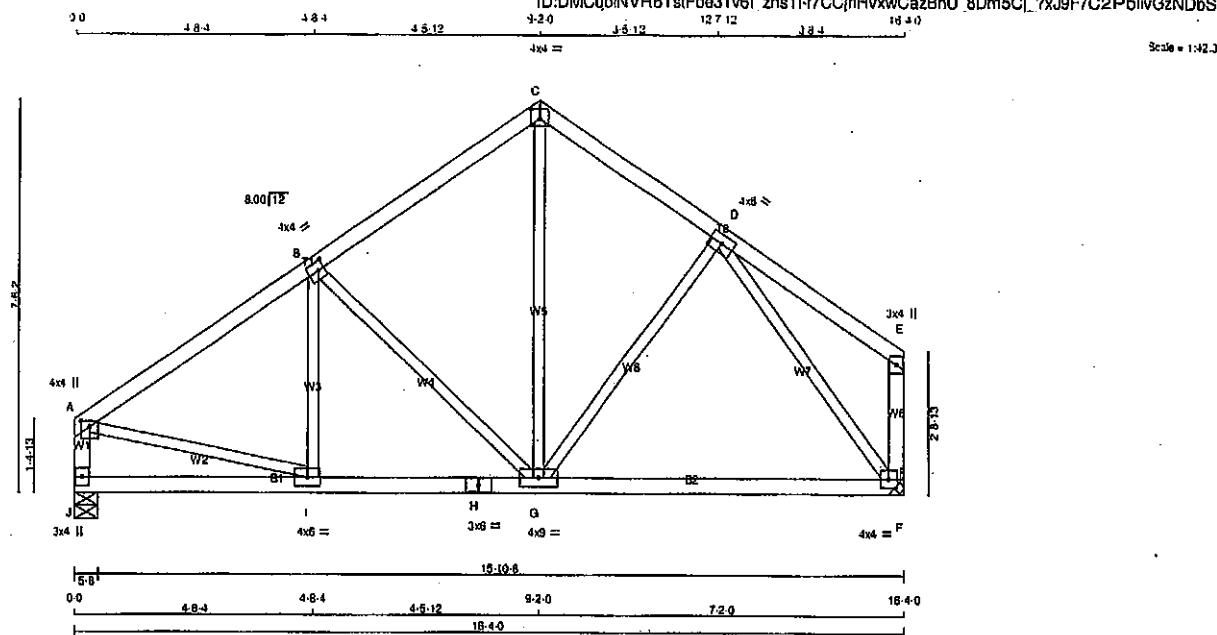


Structural component only
DWG# T-2007022

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408131	T12	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 71 = 214 lb (M/F)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - E	2x4	DRY	No.2	SPF		
J - A	2x4	DRY	No.2	SPF		
F - E	2x4	DRY	No.2	SPF		
H - H	2x4	DRY	No.2	SPF		
J - F	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMWW-l	MT20	4.0	4.0	2.00	1.50
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMWW-l	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-l	MT20	4.0	4.0		
G	BMVWW-l	MT20	4.0	9.0		
H	BS-l	MT20	3.0	6.0		
I	BMVW-l	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
J	800	0	900	0
F	900	0	900	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3'-8".

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
J	637	418 / 0	0 / 0	0 / 0	0 / 0	219 / 0	0 / 0
F	637	418 / 0	0 / 0	0 / 0	0 / 0	219 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC	LENGTH	FR-TO	(LBS)	CSI (LC)
FR-TO								
A-B	-904 / 0	-91.8	-91.8	0.25 (1)	6.17	I-B	-104 / 28	0.03 (1)
B-C	-848 / 0	-91.8	-91.8	0.25 (1)	6.25	8-G	-356 / 0	0.24 (1)
C-D	-640 / 0	-91.8	-91.8	0.15 (1)	6.25	G-C	0 / 407	0.09 (1)
D-E	0 / 21	-91.8	-91.8	0.19 (1)	10.00	G-D	-37 / 32	0.02 (1)
J-A	-881 / 0	0.0	0.0	0.09 (1)	7.81	A-I	0 / 795	0.18 (1)
F-E	-128 / 0	0.0	0.0	0.02 (1)	7.81	D-F	-886 / 0	0.54 (1)
J-I	0 / 0	-18.5	-18.5	0.09 (4)	10.00			
I-H	0 / 775	-18.5	-18.5	0.26 (4)	10.00			
H-G	0 / 775	-18.5	-18.5	0.26 (4)	10.00			
G-F	0 / 537	-18.5	-18.5	0.24 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CIC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.54")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.54")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.09")

CSI: TC=0.25/1.00 (A-B:1), BC=0.28/1.00 (G-I:4), WB=0.54/1.00 (D-F:1), SS=0.17/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1067 788
	1987	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (A) (INPUT = 0.90)

JSI METAL = 0.29 (I-I) (INPUT = 1.00)



Structural component only
DWG# T-2007023

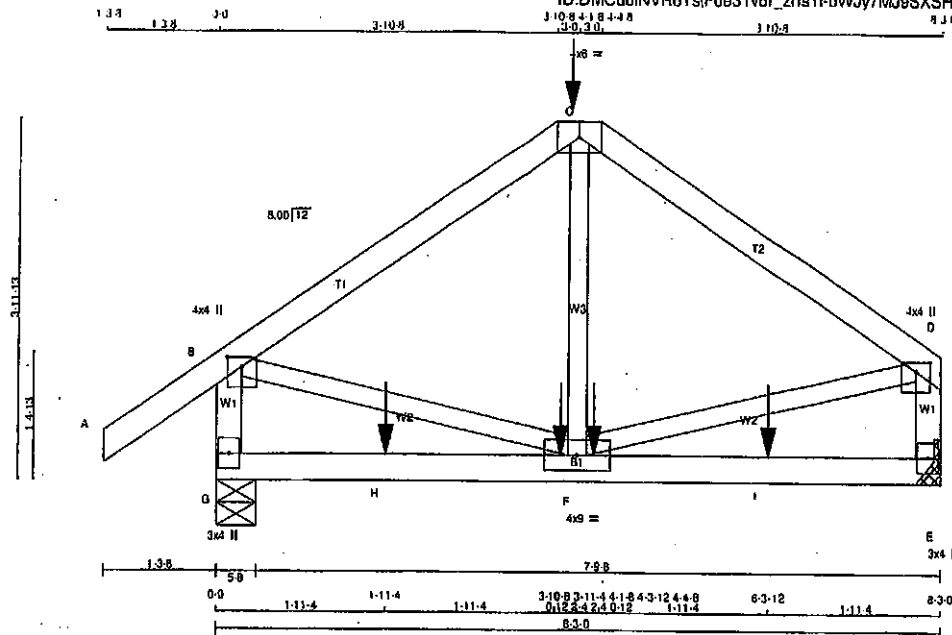
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408131	T13	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFos31v8I_zns11-bWJy7MJ9SXSCHCvrs5ZFEAdoJek2mj6qLsvEp_9zNDdQ

Scale = 1/24"



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - D	2x4 DRY	No.2	SPF		
E - B	2x4 DRY	No.2	SPF		
E - D	2x4 DRY	No.2	SPF		
G - E	2x4 DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW+p	MT20	4.0	8.0		
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMVWW-1	MT20	4.0	9.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

SEARINGS

SHEARINGS		FACTORED			MAXIMUM FACTORED		INPUT	RECORD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	BRG
G	879	0	879	0	0	5-8		5-8
E	753	0	753	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 3'-8".

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	819	420 / 0	0 / 0	0 / 0	0 / 0	0 / 0	199 / 0	0 / 0
E	533	350 / 0	0 / 0	0 / 0	0 / 0	0 / 0	183 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
FR-TO													
A-B		0 / 35	-91.8	-91.8	0.14 (1)	10.00							
B-C		-882 / 0	-91.8	-91.8	0.31 (1)	8.25							
C-D		-882 / 0	-91.8	-91.8	0.31 (1)	8.25							
G-B		-842 / 0	0.0	0.0	0.10 (1)	7.51							
E-D		-716 / 0	0.0	0.0	0.08 (1)	7.51							
G-H		0 / 0	-18.5	-18.5	0.13 (4)	10.00							
H-F		0 / 0	-18.5	-18.5	0.13 (4)	10.00							
F-I		0 / 0	-18.5	-18.5	0.13 (4)	10.00							
I-E		0 / 0	-18.5	-18.5	0.13 (4)	10.00							

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-1-8	-527	-527		BACK	VERT	TOTAL		C1
F	3-11-4	-17	-17		BACK	VERT	TOTAL		C1
F	4-3-12	-17	-17		BACK	VERT	TOTAL		C1
H	1-11-4	-17	-17		BACK	VERT	TOTAL		C1
I	8-3-12	-17	-17		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 34 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.28")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.28")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TC=0.31/1.00 (C-D:1), BC=0.13/1.00 (E-F:4), WB=0.15/1.00 (B-F:1), SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1867
	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.87 (B) (INPUT = 0.90)
JSI METAL=0.20 (B) (INPUT = 1.00)

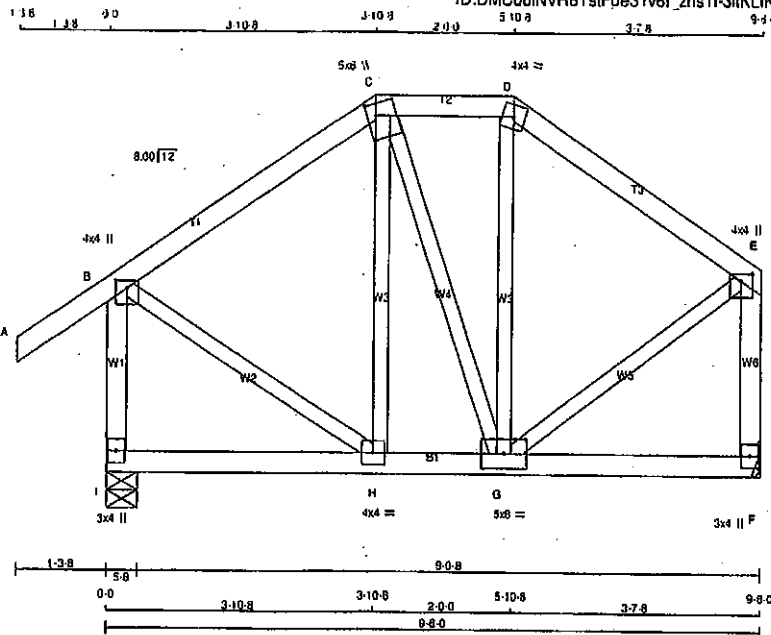


Structural component only
DWG# T-2007024

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408131	T14	2	1	TRUSS DESC.		

Farmack Roof Truss, Burlington

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ID:DMCubINVR6TstFpe31v6I_zns11-3IKLiKnDra8qIQ3IHnTiqLWS8NnSaKUSZ_MWbzNDpP



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2	
C - D	2x4	DRY	No.2	
D - E	2x4	DRY	No.2	
E - F	2x4	DRY	No.2	
F - G	2x4	DRY	No.2	
G - H	2x4	DRY	No.2	
H - I	2x4	DRY	No.2	
I - J	2x4	DRY	No.2	
J - K	2x4	DRY	No.2	
K - L	2x4	DRY	No.2	
L - M	2x4	DRY	No.2	
M - N	2x4	DRY	No.2	
N - O	2x4	DRY	No.2	
O - P	2x4	DRY	No.2	
P - Q	2x4	DRY	No.2	
Q - R	2x4	DRY	No.2	
R - S	2x4	DRY	No.2	
S - T	2x4	DRY	No.2	
T - U	2x4	DRY	No.2	
U - V	2x4	DRY	No.2	
V - W	2x4	DRY	No.2	
W - X	2x4	DRY	No.2	
X - Y	2x4	DRY	No.2	
Y - Z	2x4	DRY	No.2	

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)	PLATES	W	LEN	Y	X
JT	TMVW+p	MT20	4.0	4.0	1.25 2.00
B	TTWW+m	MT20	5.0	6.0	2.00 1.50
C	TTWW+m	MT20	4.0	4.0	
D	TTWW+m	MT20	4.0	4.0	
E	TMVW+p	MT20	4.0	4.0	1.25 2.00
F	BMV1+p	MT20	3.0	4.0	
G	BMWWW-I	MT20	5.0	8.0	
H	BMWWW-I	MT20	4.0	4.0	
I	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	VERT	GROSS REACTION	BRG	BRG
I	650	0	650	0
F	524	0	524	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM LIVE WIND DEAD SOIL
I	457	313 / 0 0 / 0 0 / 0 0 / 0 144 / 0 0 / 0
F	370	243 / 0 0 / 0 0 / 0 0 / 0 127 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)
FR-TO						FR-TO			
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	H-C	-87 / 16	0.04 (1)	
B-C	-297 / 0	-91.8	-91.8	0.18 (1)	6.25	C-G	-19 / 0	0.01 (1)	
C-D	-238 / 0	-91.8	-91.8	0.05 (1)	6.25	G-D	-87 / 9	0.04 (1)	
D-E	-289 / 0	-91.8	-91.8	0.15 (1)	6.25	B-H	0 / 288	0.08 (1)	
E-F	-618 / 0	0.0	0.0	0.09 (1)	7.81	G-E	0 / 292	0.07 (1)	
F-G	-493 / 0	0.0	0.0	0.08 (1)	7.81				
I-H	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
H-G	0 / 245	-18.5	-18.5	0.08 (4)	10.00				
G-F	0 / 0	-18.5	-18.5	0.05 (4)	10.00				

TOTAL WEIGHT = 2 X 50 = 99 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/989 (0.01")

CSI: TC=0.18/1.00 (B-C:1), BC=0.08/1.00 (G-H:4), WB=0.07/1.00 (E-G:1), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1686

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.43 (B) (INPUT = 0.90)
JSI METAL = 0.13 (B) (INPUT = 1.00)

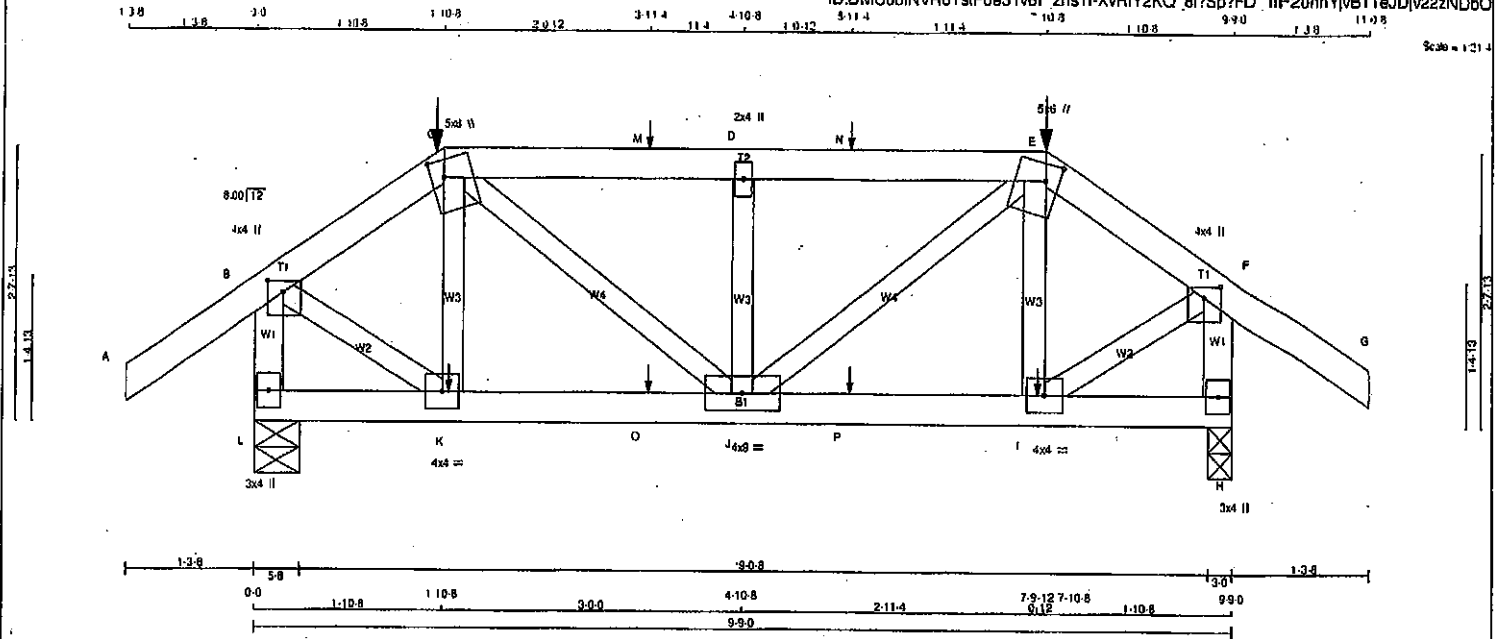


Structural component only
DWG# T-2007025

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408131	T15	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 43 lb (M)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
L - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+P	MT20	4.0	4.0	1.25	2.00	
C	TTWW+m	MT20	5.0	8.0	2.00	1.50	
D	TMVW+w	MT20	2.0	4.0			
E	TTWW+m	MT20	5.0	8.0	2.00	1.50	
F	TMVW+P	MT20	4.0	4.0	1.25	2.00	
H	BMV1+P	MT20	3.0	4.0			
I	BMWW-I	MT20	4.0	4.0			
J	BMWWW-I	MT20	4.0	9.0			
K	BMWW-I	MT20	4.0	4.0			
L	BMV1+P	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP/LIFT	IN-SX
L	708	0	0	5-8
H	708	0	0	3-0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	498	343 / 0	0 / 0	0 / 0	0 / 0	153 / 0	0 / 0
H	498	343 / 0	0 / 0	0 / 0	0 / 0	153 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLINE SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD LC1	MAX	MAX.	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	(PLF)	CSI (LC)	UNBRACED LENGTH	MEMB.	FORCE (LBS)
FR-TO						FR-TO	
A-B	0 / 35	-91.8	-91.8	0.14 (1)	10.00	K-C	-140 / 0
B-C	-458 / 0	-91.8	-91.8	0.13 (1)	6.25	C-J	0 / 304
C-M	-590 / 0	-91.8	-91.8	0.14 (1)	6.25	J-D	-329 / 0
M-D	-590 / 0	-91.8	-91.8	0.14 (1)	6.25	D-E	0 / 304
D-N	-590 / 0	-91.8	-91.8	0.14 (1)	6.25	E-F	-140 / 0
N-E	-590 / 0	-91.8	-91.8	0.14 (1)	6.25	B-K	0 / 407
E-F	-458 / 0	-91.8	-91.8	0.13 (1)	6.25	F-H	0 / 407
F-G	0 / 35	-91.8	-91.8	0.14 (1)	10.00		
L-B	-893 / 0	0.0	0.0	0.08 (1)	7.81		
H-F	-893 / 0	0.0	0.0	0.08 (1)	7.81		
L-K	0 / 0	-18.5	-18.5	0.03 (4)	10.00		
K-O	0 / 351	-18.5	-18.5	0.08 (1)	10.00		
O-J	0 / 351	-18.5	-18.5	0.08 (1)	10.00		
J-P	0 / 351	-18.5	-18.5	0.08 (1)	10.00		
P-I	0 / 351	-18.5	-18.5	0.08 (1)	10.00		
I-H	0 / 0	-18.5	-18.5	0.03 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	1-10.8	-42	-42	115	BACK	VERT	TOTAL		C1
E	7-10.8	-42	-42	115	BACK	VERT	TOTAL		C1
I	7-9-12	1	1		BACK	VERT	TOTAL		C1
K	1-11.4	1	1		BACK	VERT	TOTAL		C1
M	3-11.4	1	1	115	BACK	VERT	TOTAL		C1
N	5-11.4	1	1	115	BACK	VERT	TOTAL		C1
O	3-11.4	1	1		BACK	VERT	TOTAL		C1
P	5-11.4	1	1		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.14/1.00 (D-E:1), BC=0.08/1.00 (H-I:1),
WB=0.10/1.00 (F-I:1), SS=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667
	MAX MIN	MAX MIN	MAX MIN
	618 354	1667 788	1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.48 (F) (INPUT = 0.90)
JSI METAL= 0.14 (F) (INPUT = 1.00)



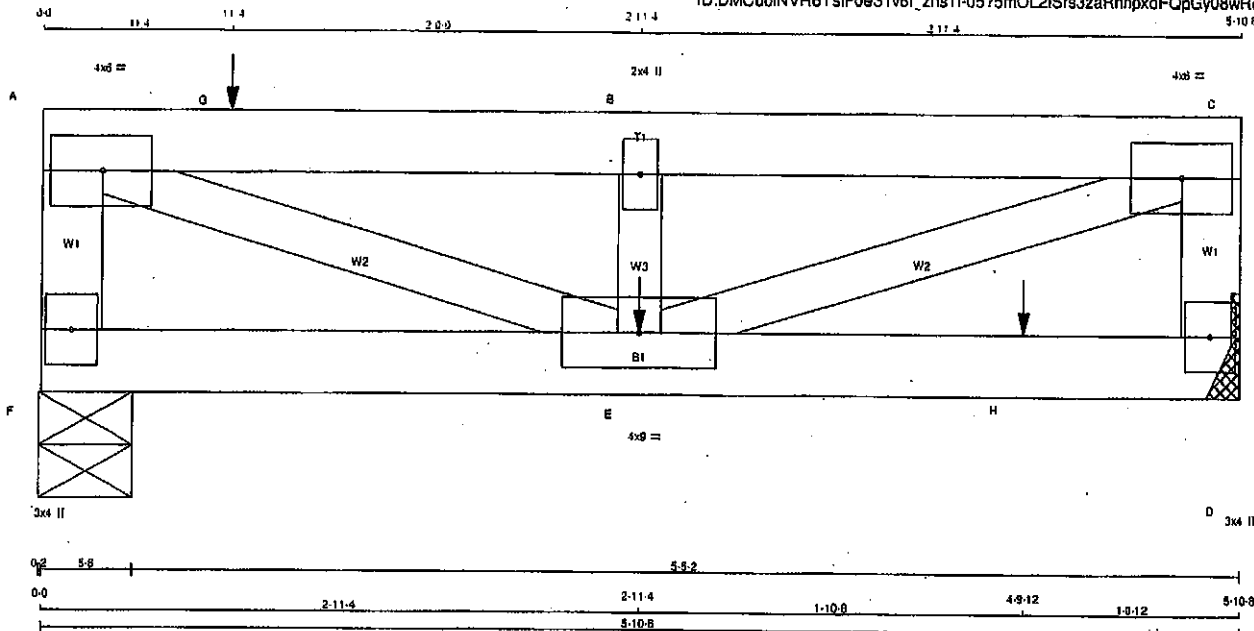
Structural component only
DWG# T-2007026

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408131	T16	1	2	GREEN PARK HOMES	
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TsiFos31v6l_zns11-0575mOL2ISr3aRnhpxoFQpGy08wRonYtTbUzND6n

Scale = 1:10.5



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
F-A	12	TOP
A-C	12	TOP
C-D	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
F-D	11	SIDE(97.1)
WEBS: (0.122"x3") SPIRAL NAILS		
B-E	8	SIDE(34.7)
2x3	1	8

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		
B	TMVW-w	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	BMV1-p	MT20	3.0	4.0		
E	BMVWW-t	MT20	4.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED			INPUT	REQD
		GROSS REACTION	GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1280	0	1280	0	0	5-8 (5-8)	5-8
D	1162	0	1162	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2'-0".

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM.LIVE	
F	895	563 / 0	0 / 0	0 / 0	333 / 0
D	826	518 / 0	0 / 0	0 / 0	308 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO					FR-TO		
F-A	-1214 / 0	0.0	0.0	0.07 (1)	7.81	A-E	0 / 2094
A-G	-1973 / 0	-91.8	-91.8	0.35 (1)	5.75	E-B	-548 / 0
G-B	-1973 / 0	-91.8	-91.8	0.35 (1)	5.75	E-C	0 / 2094
B-C	-1973 / 0	-91.8	-91.8	0.10 (1)	8.14		
D-C	-784 / 0	0.0	0.0	0.04 (1)	7.81		
F-E	0 / 0	-43.5	-43.5	0.08 (4)	10.00		
E-H	0 / 0	-43.5	-43.5	0.27 (1)	10.00		
H-D	0 / 0	-43.5	-43.5	0.27 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-11-4	-505	-505	---	---	FRONT	VERT	TOTAL	---	C1
G	11-4	-817	-817	---	---	TOP	VERT	TOTAL	---	C1
H	4-9-12	-505	-505	---	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.04")

CSI: TC=0.35/1.00 (A-B:1), BC=0.27/1.00 (D-E:1), WB=0.28/1.00 (A-E:1), SS=0.22/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

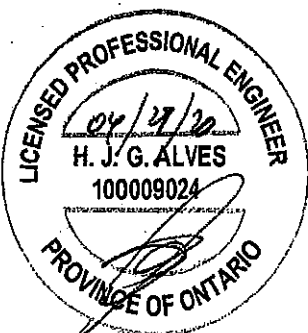
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.68 (C), INPUT = 0.90
JSI METAL = 0.25 (C), INPUT = 1.00



Structural component only
DWG# T-2007027 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408131	T16	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
F 8MV1+p	MT20	3.0	4.0		

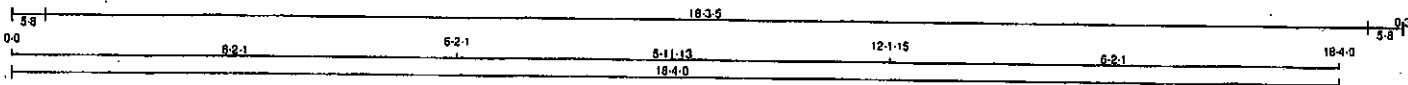
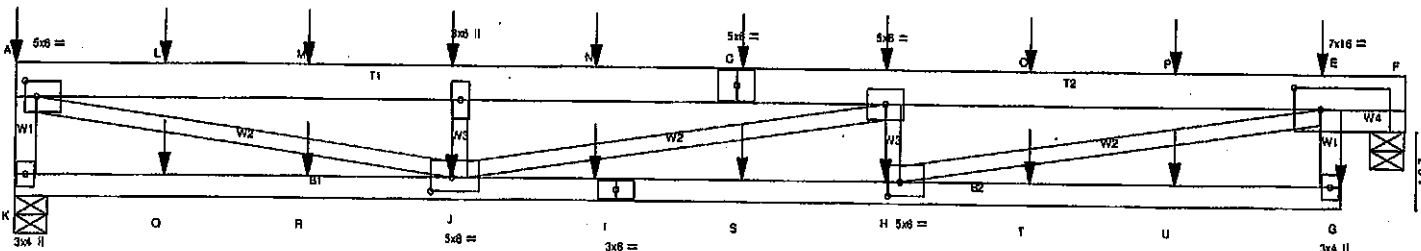
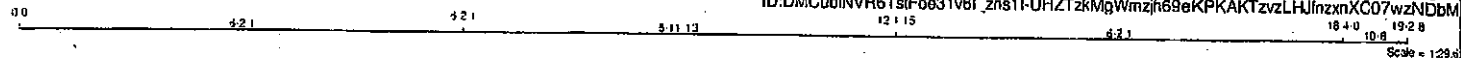


Structural component only
DWG# T-2007027 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408131	T17	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 75 = 149 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
K - A	2x4	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(P/LF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
K-A 1	12	TOP
E-G 1	6	SIDE(35.8)
A-C 2	12	SIDE(183.1)
C-F 2	12	SIDE(183.1)
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
K-I 1	12	SIDE(61.0)
I-G 1	12	SIDE(61.0)
WEBS : (0.122"x3") SPIRAL NAILS		
H-D 1	6	SIDE(34.0)
J-B 1	6	SIDE(34.0)
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
K	1740	0	0	5-8
F	1670	0	0	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): F

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED
K	1229 818 / 0 0 / 0 0 / 0 413 / 0 0 / 0
F	1180 779 / 0 0 / 0 0 / 0 401 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. (PLF)	LC1	MAX. (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	UNBRAC	FR-TO	
K-A	-1845 / 0	0.0	0.0	0.09 (1)	H-E	0 / 5759
A-L	-5118 / 0	-91.8	-91.8	0.28 (1)	A-J	0 / 5250
L-M	-5118 / 0	-91.8	-91.8	0.28 (1)	H-D	-1023 / 0
M-B	-5118 / 0	-91.8	-91.8	0.28 (1)	J-B	-775 / 0
B-N	-5118 / 0	-91.8	-91.8	0.19 (1)	J-D	-510 / 0
N-C	-5118 / 0	-91.8	-91.8	0.19 (1)		
C-D	-5118 / 0	-91.8	-91.8	0.19 (1)		
D-O	-5615 / 0	-91.8	-91.8	0.61 (1)		
O-P	-5615 / 0	-91.8	-91.8	0.61 (1)		
P-E	-5615 / 0	-91.8	-91.8	0.61 (1)		
E-F	-853 / 0	-91.8	-91.8	0.32 (1)		
G-E	0 / 154	0.0	0.0	0.01 (4)		
K-Q	0 / 0	-18.5	-18.5	0.12 (4)		
Q-R	0 / 0	-18.5	-18.5	0.12 (4)		
R-J	0 / 0	-18.5	-18.5	0.12 (4)		
J-I	0 / 5615	-18.5	-18.5	0.60 (1)		
I-S	0 / 5615	-18.5	-18.5	0.60 (1)		
S-H	0 / 5615	-18.5	-18.5	0.60 (1)		
H-T	0 / 0	-18.5	-18.5	0.60 (1)		
T-U	0 / 0	-18.5	-18.5	0.13 (1)		
U-G	0 / 0	-18.5	-18.5	0.13 (1)		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
A	0-0	-125	-125	---	BACK	VERT	TOTAL	---	C1
B	6-0-12	-82	-82	---	BACK	VERT	TOTAL	---	C1
C	10-0-12	-82	-82	---	BACK	VERT	TOTAL	---	C1
D	12-0-12	-82	-82	---	BACK	VERT	TOTAL	---	C1
E	18-0-12	-82	-82	---	BACK	VERT	TOTAL	---	C1
G	18-4-0	-56	-56	---	BACK	VERT	TOTAL	---	C1
H	12-0-12	-49	-49	---	BACK	VERT	TOTAL	---	C1
I	8-0-12	-49	-49	---	BACK	VERT	TOTAL	---	C1
J	6-0-12	-49	-49	---	BACK	VERT	TOTAL	---	C1
L	2-0-12	-82	-82	---	BACK	VERT	TOTAL	---	C1
M	-4-0-12	-82	-82	---	BACK	VERT	TOTAL	---	C1
N	8-0-12	-82	-82	---	BACK	VERT	TOTAL	---	C1
O	14-0-12	-82	-82	---	BACK	VERT	TOTAL	---	C1
P	16-0-12	-82	-82	---	BACK	VERT	TOTAL	---	C1
Q	2-0-12	-49	-49	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
OL	= 8.0 PSF
BOT CH. LL	= 0.0 PSF
OL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/984 (0.23")
ALLOWABLE DEFL.(TL) = L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/521 (0.42")

CSI: TC=0.61/1.00 (D-E:1), BC=0.60/1.00 (H-J:1), WB=0.71/1.00 (E-H:1), SS=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

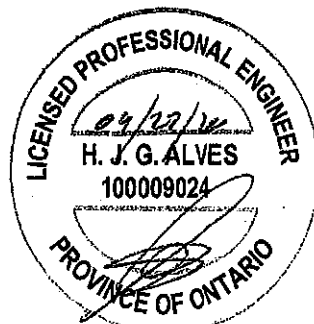
NAIL VALUES

PLATE GRIP(DRY) SHEAR	SECTION
(PSI)	(PLI)
MAX MIN MAX MIN	MAX MIN
MT20	618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (J) (INPUT = 0.90)
JSI METAL = 0.95 (I) (INPUT = 1.00)



Structural component only
DWG# T-2007028 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408131	T17	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table in inches)

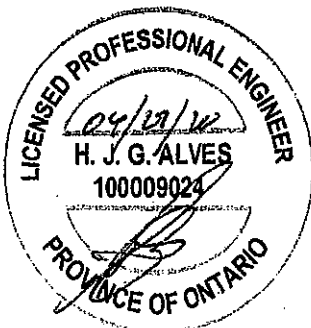
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-l	MT20	5.0	6.0	2.50	2.00
B	TMVW-w	MT20	3.0	6.0		
C	TS-l	MT20	5.0	6.0		
D	TMVW-l	MT20	5.0	6.0		
E	TMVW-l	MT20	7.0	16.0	3.50	4.50
G	BMV-p	MT20	3.0	4.0		
H	BMVW-l	MT20	5.0	8.0	2.25	2.00
I	SS-l	MT20	3.0	8.0		
J	BMVW-l	MT20	5.0	8.0	2.25	3.50
K	BMV-l-p	MT20	3.0	4.0		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	4-0-12	-49	-49	---	BACK	VERT	TOTAL	---	C1
S	10-0-12	-49	-49	---	BACK	VERT	TOTAL	---	C1
T	14-0-12	-49	-49	---	BACK	VERT	TOTAL	---	C1
U	16-0-12	-49	-49	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

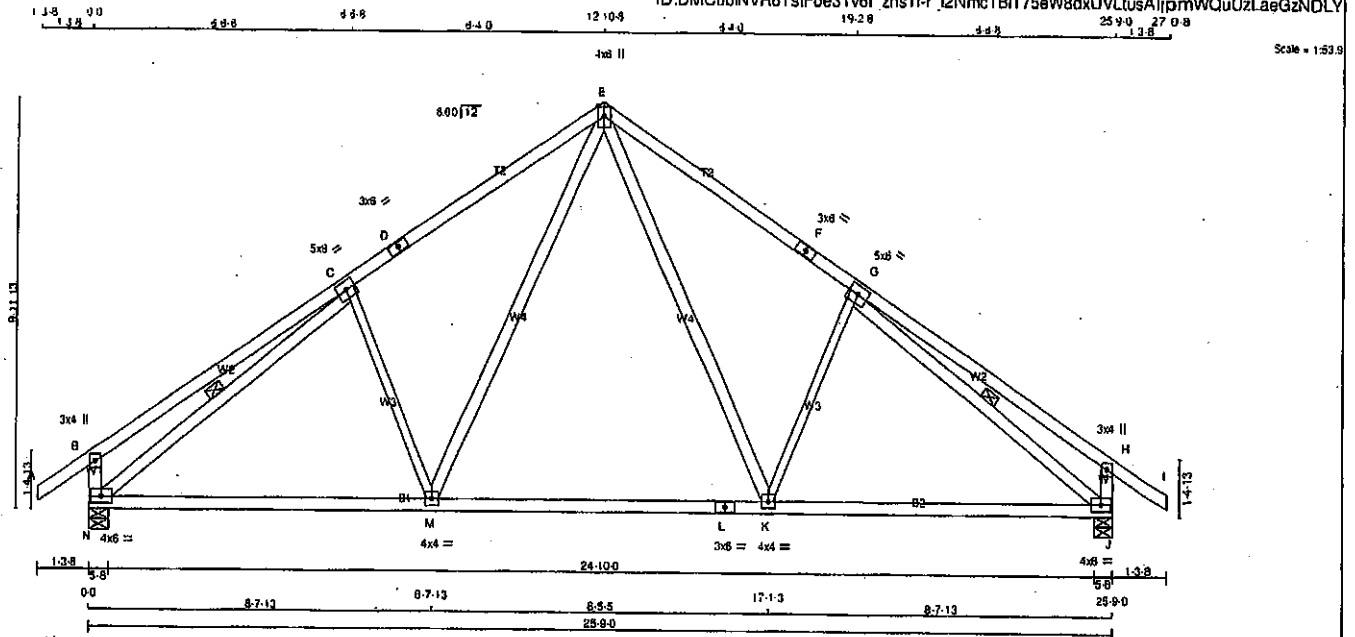
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007028

JOB NAME 408132	TRUSS NAME T20	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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25 9 0 27 0 8



TOTAL WEIGHT = 4 X 121 = 488 lb (M/F)

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
K - G	2x3	DRY	No.2	SPF
C - M	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMWW-t	MT20	5.0	6.0
D	TS-t	MT20	3.0	6.0
E	TTWW+p	MT20	4.0	8.0
F	TS-t	MT20	3.0	6.0
G	TMWW-t	MT20	5.0	8.0
H	TMV+p	MT20	3.0	4.0
J	BMVW-t	MT20	4.0	6.0
K	BMWW-t	MT20	4.0	4.0
L	BS-t	MT20	3.0	6.0
M	BMWW-t	MT20	4.0	4.0
N	BMVW-t	MT20	4.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
N	1545	0	0	1545	0	0	5-8	5-8	5-8
J	1545	0	0	1545	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
N	1091	729 / 0	0 / 0	0 / 0	0 / 0
J	1091	729 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-N, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING		TOTAL LOAD CASES: (4)	
CHORDS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD LC1	MAX
FR-TO		FROM	TO
A-B	0 / 35	-91.8	-91.8 0.12 (1)
B-C	0 / 40	-91.8	-91.8 0.03 (1)
C-D	-1469 / 0	-91.8	-91.8 0.58 (1)
D-E	-1469 / 0	-91.8	-91.8 0.58 (1)
E-F	-1469 / 0	-91.8	-91.8 0.58 (1)
F-G	-1469 / 0	-91.8	-91.8 0.58 (1)
G-H	0 / 40	-91.8	-91.8 0.63 (1)
H-I	0 / 35	-91.8	-91.8 0.12 (1)
N-B	-347 / 0	0.0	0.0 0.04 (1)
J-H	-347 / 0	0.0	0.0 0.04 (1)
N-M	0 / 1358	-18.5	-18.5 0.42 (4)
M-L	0 / 942	-18.5	-18.5 0.39 (4)
L-K	0 / 942	-18.5	-18.5 0.39 (4)
K-J	0 / 1358	-18.5	-18.5 0.42 (4)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/G

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.83/1.00 (B-C:1), BC=0.42/1.00 (J-K:4), WB=0.59/1.00 (C-N:1), SSI=0.24/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

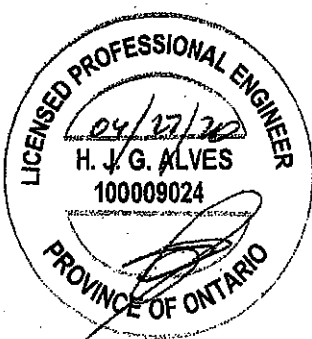
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1687 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (J) (INPUT = 0.90)

JSI METAL = 0.48 (G) (INPUT = 1.00)

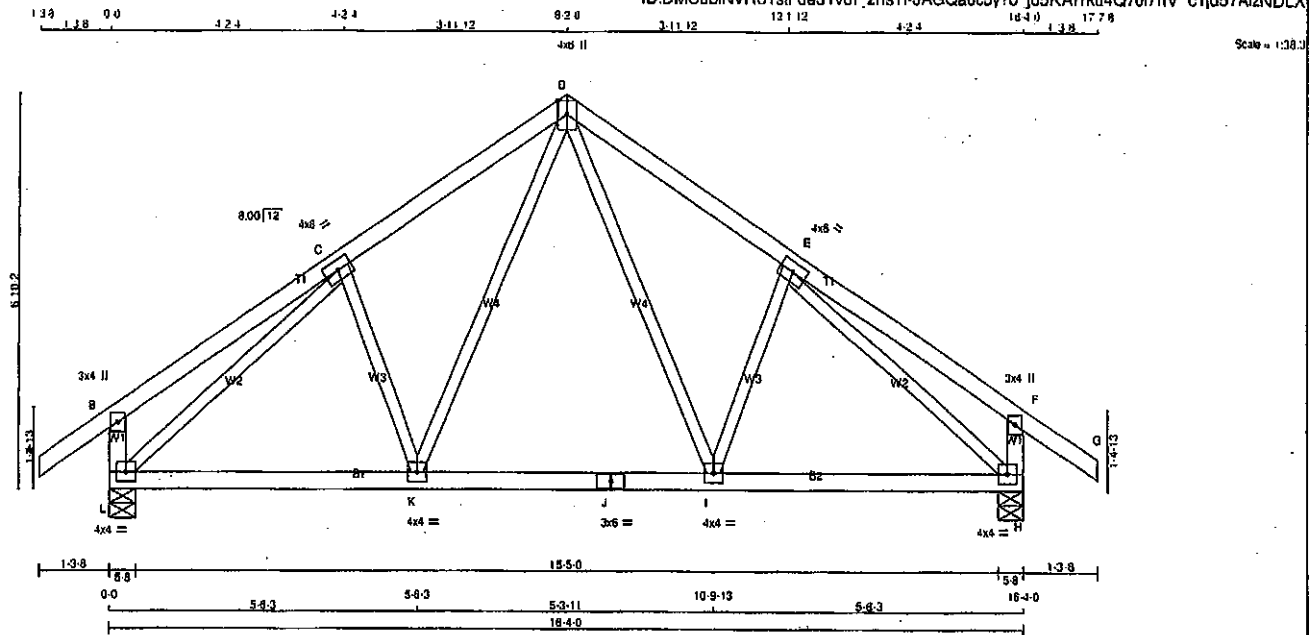


Structural component only
DWG# T-2007030

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408132	T21	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6l_zns11-JAGQa6c5y?b_jo5KAf?ku4Q70i7hV_ctjd57AizNDLX



TOTAL WEIGHT = 4 X 73 = 291 lb (M/F)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - G	2x4	DRY	No.2	SPF		
L - B	2x4	DRY	No.2	SPF		
H - F	2x4	DRY	No.2	SPF		
L - J	2x4	DRY	No.2	SPF		
J - H	2x4	DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVap	MT20	3.0	4.0		
C	TMWW-I	MT20	4.0	6.0		
D	TTWW-p	MT20	4.0	6.0	Edge	
E	TMWW-I	MT20	4.0	6.0		
F	TMVap	MT20	3.0	4.0		
H	BMVW-I	MT20	4.0	4.0		
I	BMVW-I	MT20	4.0	4.0		
J	BS-I	MT20	3.0	6.0		
K	BMVW-I	MT20	4.0	4.0		
L	BMVW-I	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
L	1026 0	1026 0	0	5-8
H	1026 0	1026 0	0	5-8

UNFACTORED REACTIONS

1ST LOSE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERMLIVE WIND DEAD SOIL
L	723 489 / 0 0 / 0 0 / 0 0 / 0 235 / 0 0 / 0
H	723 489 / 0 0 / 0 0 / 0 0 / 0 235 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX	MAX	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (L)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)
FR-TO					FR-TO	
A-B	0 / 35	-91.8 -91.8	0.12 (1)	10.00	D-I	0 / 338
B-C	0 / 24	-91.8 -91.8	0.24 (1)	10.00	E-E	-219 / 0
C-D	-851 / 0	-91.8 -91.8	0.19 (1)	6.25	K-D	0 / 338
D-E	-851 / 0	-91.8 -91.8	0.19 (1)	8.25	C-K	-219 / 0
E-F	0 / 24	-91.8 -91.8	0.24 (1)	10.00	L-C	-1048 / 0
F-G	0 / 35	-91.8 -91.8	0.12 (1)	10.00	E-H	-1048 / 0
L-B	-289 / 0	0.0 0.0	0.03 (1)	7.81		
H-F	-289 / 0	0.0 0.0	0.03 (1)	7.81		
L-K	0 / 787	-18.5 -18.5	0.19 (1)	10.00		
K-J	0 / 581	-18.5 -18.5	0.17 (4)	10.00		
J-I	0 / 581	-18.5 -18.5	0.17 (4)	10.00		
I-H	0 / 787	-18.5 -18.5	0.19 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.24/1.00 (B-C:1), BC=0.19/1.00 (H-I:1), WB=0.53/1.00 (C-L:1), SS=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

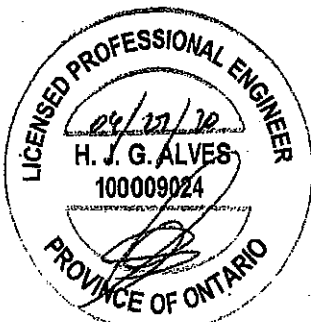
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1657 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.29 (L) (INPUT = 1.00)

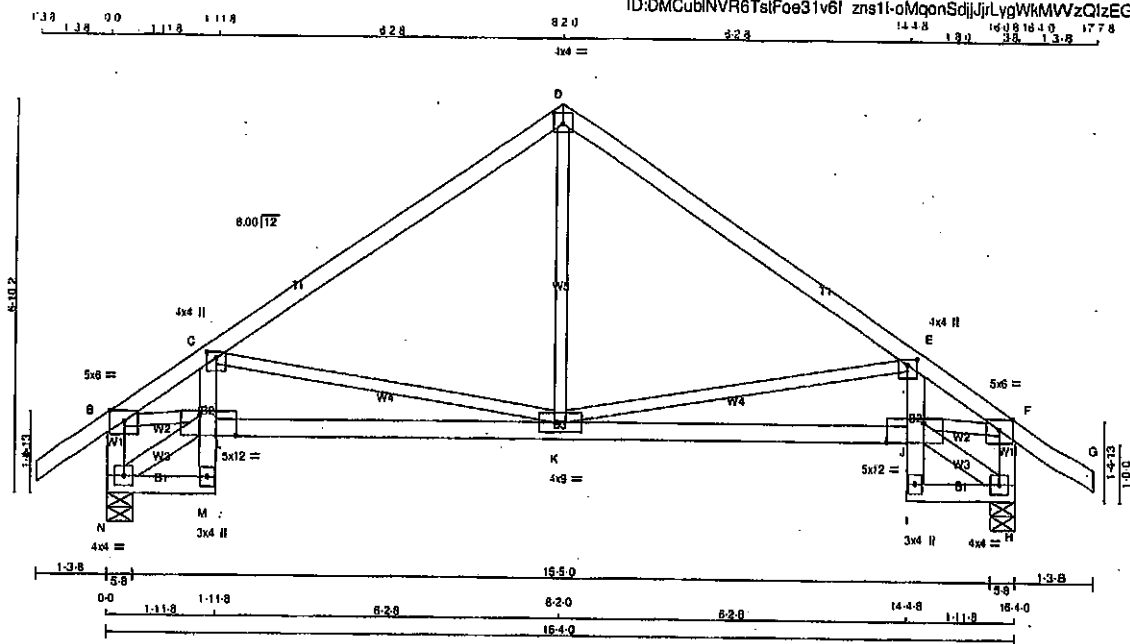


Structural component only
DWG# T-2007031

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408132	T21S	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/32

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

N - L 2x4 DRY No.2 SPF

J - H 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMVW-p	MT20	4.0	4.0	1.25	2.00
D	TMVW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-p	MT20	4.0	4.0	1.25	2.00
F	TMVW-p	MT20	5.0	6.0	Edge	
H	BMVW-1	MT20	4.0	4.0		
I	BMV-p	MT20	3.0	4.0		
J	BMVW-1	MT20	5.0	12.0	4.00	8.00
K	BMVW-1	MT20	4.0	9.0		
L	BMVW-1	MT20	5.0	12.0	4.00	8.00
M	BMV-p	MT20	3.0	4.0		
N	BMVW-1	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT	IN-SX
N	1026	0	1026	0
H	1026	0	1026	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	723	488 / 0	0 / 0	0 / 0	0 / 0	235 / 0	0 / 0
H	723	488 / 0	0 / 0	0 / 0	0 / 0	235 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS			MEMB.	MAX. FACTORED FORCE (LBS)	WEBS		
		VERT. LOAD	LC1	MAX			VERT. LOAD	LC1	MAX
FR-TO		FROM	TO	LENGTH	FR-TO		FROM	TO	LENGTH
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	K-D	0 / 405	0.09 (1)	10.00
B-C	-1592 / 0	-91.8	-91.8	0.36 (1)	4.83	K-E	-787 / 0	0.61 (1)	10.00
C-D	-829 / 0	-91.8	-91.8	0.47 (1)	6.01	C-K	-787 / 0	0.61 (1)	10.00
D-E	-829 / 0	-91.8	-91.8	0.47 (1)	6.01	N-L	-88 / 0	0.01 (1)	10.00
E-F	-1592 / 0	-91.8	-91.8	0.36 (1)	4.83	B-L	0 / 1383	0.31 (1)	10.00
F-G	0 / 35	-91.8	-91.8	0.12 (1)	10.00	J-H	-88 / 0	0.01 (1)	10.00
N-B	-974 / 0	0.0	0.0	0.10 (1)	7.81	J-F	0 / 1383	0.31 (1)	10.00
H-F	-974 / 0	0.0	0.0	0.10 (1)	7.81				
N-M	0 / 58	-18.5	-18.5	0.02 (4)	10.00				
M-L	0 / 19	0.0	0.0	0.09 (1)	10.00				
L-C	0 / 84	0.0	0.0	0.10 (1)	10.00				
L-K	0 / 1438	-18.5	-18.5	0.36 (1)	10.00				
K-J	0 / 1438	-18.5	-18.5	0.36 (1)	10.00				
I-J	0 / 19	0.0	0.0	0.09 (1)	10.00				
J-E	0 / 84	0.0	0.0	0.10 (1)	10.00				
I-H	0 / 58	-18.5	-18.5	0.02 (4)	10.00				

TOTAL WEIGHT = 2 X 72 = 145 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.54")
CALCULATED VERT. DEFL. (LL) = L/999 (0.03")
ALLOWABLE DEFL. (TL) = L/360 (0.54")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CSI: TC=0.47/1.00 (C-D:1), BC=0.36/1.00 (K-L:1), WB=0.61/1.00 (C-K:1), SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

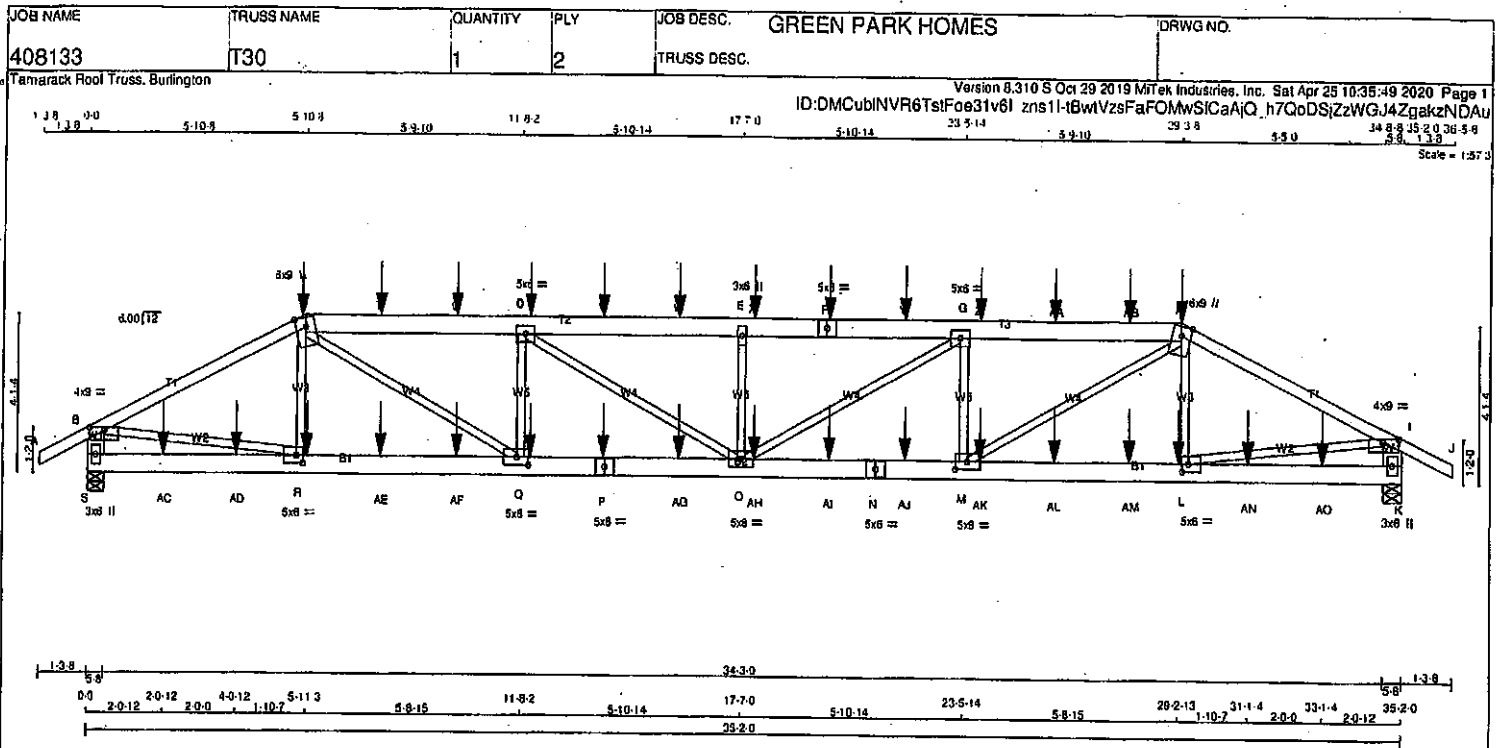
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.69 (F) (INPUT = 0.90)
JSI METAL=0.33 (L) (INPUT = 1.00)



Structural component only
DWG# T-2007032



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - F	2x6	DRY No.2	SPF
F - H	2x6	DRY No.2	SPF
H - J	2x4	DRY No.2	SPF
S - B	2x6	DRY No.2	SPF
K - I	2x6	DRY No.2	SPF
S - P	2x6	DRY No.2	SPF
P - N	2x6	DRY No.2	SPF
N - K	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(81.0)
H-J	12	SIDE(81.0)
C-F	12	SIDE(183.1)
F-H	12	SIDE(81.0)
S-B	12	TOP
K-I	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S-P	12	SIDE(183.1)
P-N	12	SIDE(0.0)
N-K	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	RECORD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
S	3322	0	3322	0	5-8
K	3350	0	3350	0	5-8

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
S	2350	1538 / 0	0 / 0	0 / 0	0 / 0	812 / 0
K	2370	1550 / 0	0 / 0	0 / 0	0 / 0	820 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED	WEBS	FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB. FORCE (LBS)
FR-TO				
A-B	0 / 28	-91.8 -91.8 0.07 (1)	10.00	R-C -413 / 18 0.05 (1)
B-C	-5084 / 0	-91.8 -91.8 0.52 (1)	3.87	L-H -411 / 24 0.05 (1)
C-T	-7207 / 0	-91.8 -91.8 0.26 (1)	4.28	B-R 0 / 4581 0.57 (1)
T-U	-7207 / 0	-91.8 -91.8 0.26 (1)	4.28	L-I 0 / 4625 0.57 (1)
U-D	-7207 / 0	-91.8 -91.8 0.26 (1)	4.28	M-H 0 / 3123 0.39 (1)
D-V	-8028 / 0	-91.8 -91.8 0.26 (1)	4.09	C-Q 0 / 3143 0.39 (1)
V-W	-8028 / 0	-91.8 -91.8 0.26 (1)	4.09	M-G -1484 / 0 0.18 (1)
W-E	-8028 / 0	-91.8 -91.8 0.26 (1)	4.09	Q-D -1496 / 0 0.18 (1)
E-X	-8028 / 0	-91.8 -91.8 0.27 (1)	4.08	O-G 0 / 933 0.12 (1)
X-F	-8028 / 0	-91.8 -91.8 0.27 (1)	4.08	D-O 0 / 985 0.12 (1)
F-Y	-8028 / 0	-91.8 -91.8 0.27 (1)	4.08	O-E -821 / 0 0.10 (1)
Y-G	-8028 / 0	-91.8 -91.8 0.27 (1)	4.08	
G-Z	-7234 / 0	-91.8 -91.8 0.26 (1)	4.27	
Z-AA	-7234 / 0	-91.8 -91.8 0.26 (1)	4.27	
AA-AB	-7234 / 0	-91.8 -91.8 0.26 (1)	4.27	
AB-H	-7234 / 0	-91.8 -91.8 0.26 (1)	4.27	
H-I	-5114 / 0	-91.8 -91.8 0.52 (1)	3.85	
I-J	0 / 28	-91.8 -91.8 0.07 (1)	10.00	
S-B	-3248 / 0	0.0 0.0 0.11 (1)	7.69	
K-I	-3278 / 0	0.0 0.0 0.12 (1)	7.67	

S-AC	0 / 0	-18.5 -18.5 0.06 (4)	10.00
AC-AD	0 / 0	-18.5 -18.5 0.06 (4)	10.00
AD-R	0 / 0	-18.5 -18.5 0.06 (4)	10.00
R-AE	0 / 4582	-18.5 -18.5 0.34 (1)	10.00
AE-AF	0 / 4582	-18.5 -18.5 0.34 (1)	10.00
AF-Q	0 / 4582	-18.5 -18.5 0.34 (1)	10.00
Q-P	0 / 7206	-18.5 -18.5 0.53 (1)	10.00
P-AG	0 / 7206	-18.5 -18.5 0.53 (1)	10.00
AG-O	0 / 7206	-18.5 -18.5 0.53 (1)	10.00
O-AH	0 / 7234	-18.5 -18.5 0.53 (1)	10.00
AH-AI	0 / 7234	-18.5 -18.5 0.53 (1)	10.00
AI-N	0 / 7234	-18.5 -18.5 0.53 (1)	10.00
N-AJ	0 / 7234	-18.5 -18.5 0.53 (1)	10.00
AJ-M	0 / 7234	-18.5 -18.5 0.53 (1)	10.00
M-AK	0 / 4598	-18.5 -18.5 0.35 (1)	10.00
AK-AL	0 / 4598	-18.5 -18.5 0.35 (1)	10.00
AL-AM	0 / 4598	-18.5 -18.5 0.35 (1)	10.00
AM-L	0 / 4598	-18.5 -18.5 0.35 (1)	10.00
L-AN	0 / 0	-18.5 -18.5 0.06 (4)	10.00
AN-AO	0 / 0	-18.5 -18.5 0.06 (4)	10.00
AO-K	0 / 0	-18.5 -18.5 0.06 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	8.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.39")

CSI: TC=0.52/1.00 (H-I:1), BC=0.53/1.00 (M-O:1), WB=0.57/1.00 (L-I:1), SS=0.18/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

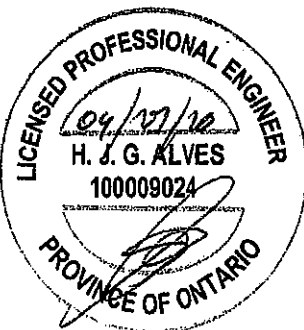
PLATE GRIP(DRY)	SHEAR	SECTION
(F8)	(PL)	(PL)
MT20	618	354 1887 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (R) (INPUT = 0.90)

JSI METAL= 0.68 (P) (INPUT = 1.00)



Structural component only
DWG# T-2007041

CONTINUED ON PAGE 2

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	9.0	1.00	4.75
C	TTWW+m	MT20	8.0	9.0	Edge	
D	TMVW-i	MT20	5.0	6.0		
E	TMVW-w	MT20	3.0	8.0		
F	TS-i	MT20	5.0	6.0		
G	TMVW-i	MT20	5.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	
I	TMVW-p	MT20	4.0	9.0	1.00	4.75
K	BMV1-p	MT20	3.0	6.0		
L	BMVW-i	MT20	5.0	6.0	2.50	2.00
M	BMVW-i	MT20	5.0	6.0	2.50	3.75
N	BS-i	MT20	5.0	6.0		
O	BMVW-i	MT20	5.0	6.0		
P	BS-i	MT20	5.0	6.0		
Q	BMVW-i	MT20	5.0	6.0	2.50	3.75
R	BMVW-i	MT20	5.0	6.0	2.50	2.00
S	BMV1-p	MT20	3.0	6.0		

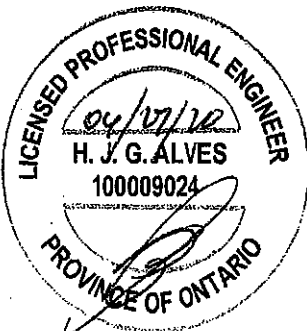
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

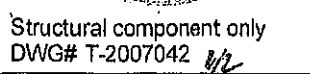
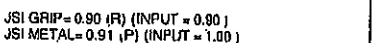
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-441	-441	---	BACK	VERT	TOTAL	---	C1
D	11-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
F	19-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
H	29-3-8	-441	-441	---	BACK	VERT	TOTAL	---	C1
L	29-2-13	-26	-26	---	BACK	VERT	TOTAL	---	C1
P	13-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
Q	11-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
R	5-11-3	-26	-26	---	BACK	VERT	TOTAL	---	C1
T	7-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
U	9-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
V	13-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
W	15-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
X	17-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
Y	21-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
Z	23-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
AA	25-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
AB	27-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
AC	2-0-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
AD	4-0-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
AE	7-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AF	9-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AG	15-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AH	17-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AI	19-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AJ	21-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AK	23-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AL	25-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AM	27-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AN	31-1-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AO	33-1-4	-26	-26	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007041

Scale = 1:57.3

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	17-4-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
G	23-4-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
M	23-4-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
N	21-4-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	17-4-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
T	18-4-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
U	21-4-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	18-7-8	-1364	-1364	---	FRONT	VERT	TOTAL	---	C1
W	19-4-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
X	23-11-8	-1443	-1443	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER MECHANICAL CONNECTION IS REQUIRED.

JOB NAME	THUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408133	T30Z	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 9 Oct 29 2019 MTek Industries, Inc. Sat Apr 25 10:35:50 2020 Page 2
ID:DMCubINVR6TstFoe31v6I zns1I-LNUFJtULYWDXbEP7uEfxvgaicGInbQYkID6BzNDAt

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	9.0	1.00	4.75
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMWW-l	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-l	MT20	5.0	6.0		
G	TMWW-l	MT20	5.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	
I	TMVW-p	MT20	4.0	9.0	1.00	4.75
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-l	MT20	5.0	6.0	2.50	2.00
M	BMWW-l	MT20	5.0	6.0	2.50	3.75
N	BS-l	MT20	5.0	6.0		
O	BMWW-l	MT20	5.0	6.0		
P	BS-l	MT20	5.0	6.0		
Q	BMWW-l	MT20	5.0	6.0	2.50	3.75
R	BMWW-l	MT20	5.0	6.0	2.50	2.00
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

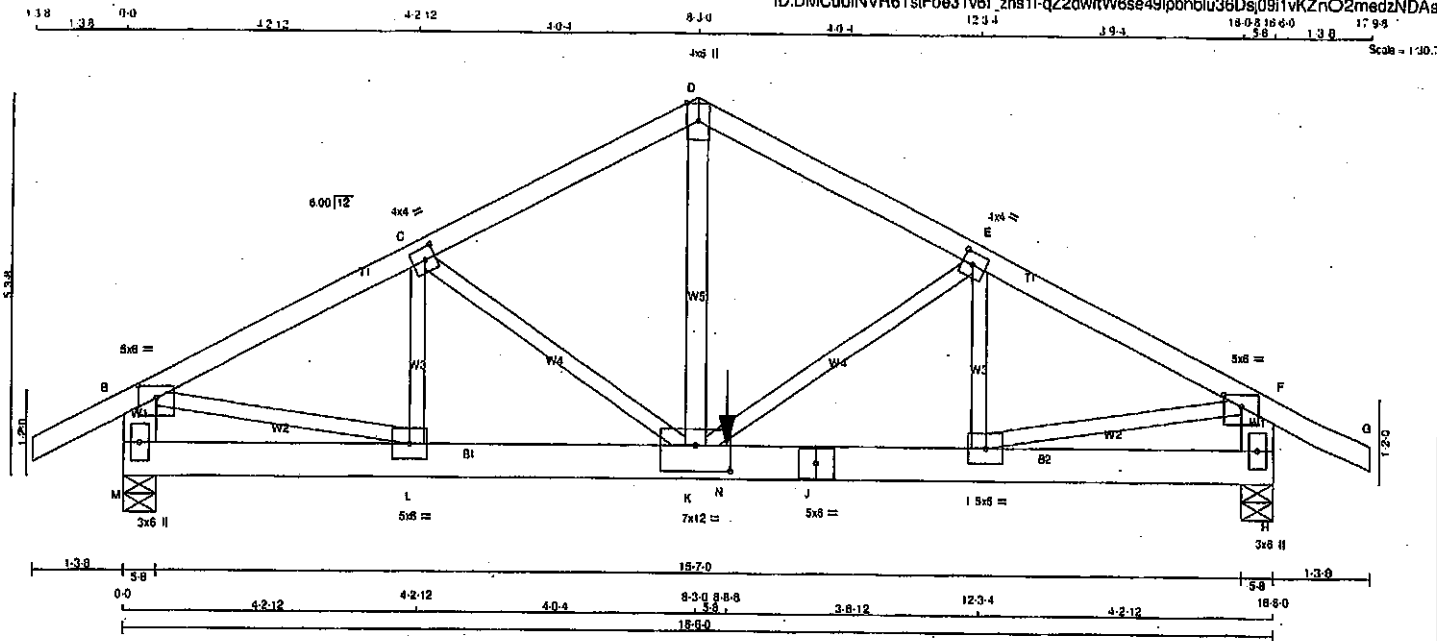
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007042 3/2

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:35:51 2020 Page 1
ID:DMCubINVR6Ts1Foe31v8l_zns11-qZ2dwfW8se49lphblu36Ds0911vKZnO2medzNDAs



TOTAL WEIGHT = 2 X 79 = 158 lb

CHORDS	SIZE	LUMBER	DESCR.
N - L, G - A	2x4	DRY	No.2
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
M - B	2x8	DRY	No.2
H - F	2x8	DRY	No.2
M - J	2x8	DRY	No.2
J - H	2x8	DRY	No.2

ALL WEBS	2x3	DRY	No.2
EXCEPT			
K - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A - D	12	TOP
D - G	12	TOP
M - B	2	TOP
H - F	2	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
M - J	2	SIDE (0.0)
J - H	2	TOP
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	6	
2x4	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
M	2447	0	2447	0
H	2813	0	2813	0

UNFACTORED REACTIONS	1ST LOASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
M	1733	1123 / 0
H	1851	1197 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
FR-TO												
A-B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	K-D	0 / 2958	0.26 (1)				
B-C	-3375 / 0	-91.8	-91.8	0.17 (1)	4.94	K-E	-150 / 0	0.03 (1)				
C-D	-3654 / 0	-91.8	-91.8	0.17 (1)	4.78	I-E	-301 / 0	0.03 (1)				
D-E	-3658 / 0	-91.8	-91.8	0.17 (1)	4.79	C-K	0 / 271	0.03 (1)				
E-F	-3758 / 0	-91.8	-91.8	0.17 (1)	4.72	L-C	-721 / 0	0.07 (1)				
F-G	0 / 28	-91.8	-91.8	0.07 (1)	10.00	B-L	0 / 3084	0.38 (1)				
M-B	-2354 / 0	0.0	0.0	0.08 (1)	7.81	I-F	0 / 3431	0.42 (1)				
H-F	-2590 / 0	0.0	0.0	0.08 (1)	7.81							
M-L	0 / 0	-18.5	-18.5	0.08 (1)	10.00							
L-K	0 / 3033	-18.5	-18.5	0.39 (1)	10.00							
K-N	0 / 3375	-18.5	-18.5	0.50 (1)	10.00							
N-J	0 / 3375	-18.5	-18.5	0.50 (1)	10.00							
J-I	0 / 3375	-18.5	-18.5	0.50 (1)	10.00							
I-H	0 / 0	-18.5	-18.5	0.03 (4)	10.00							

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
N	8-8	-2892	-2892	MAX-	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP CH.	LL	=	25.6 PSF
	DL	=	6.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.4 PSF
TOTAL LOAD		=	39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.55")
CALCULATED VERT. DEFL. (LL) = L/999 (0.05")
ALLOWABLE DEFL. (TL) = L/360 (0.55")
CALCULATED VERT. DEFL. (TL) = L/999 (0.10")

CSI: TC=0.17/1.00 (E-F:1), BC=0.50/1.00 (I-K:1), WB=0.42/1.00 (F-I:1), SS=0.93/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

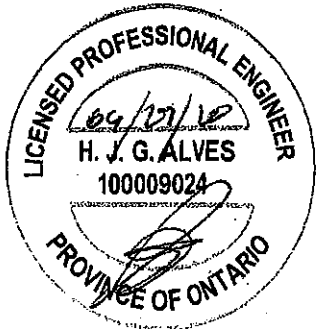
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL. IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)	
	MAX	MIN	MAX	MIN
MT20	618	354	1667	788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (B) (INPUT = 0.90)
JSI METAL = 0.52 (J) (INPUT = 1.00)



Structural component only
DWG# T-2007043 1/2

JOB NAME 408133	TRUSS NAME T37	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TIW+p	MT20	4.0	8.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMVW-p	MT20	5.0	6.0	2.00	3.00
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	7.0	12.0	4.25	6.00
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
 TOUCHES EDGE OF CHORD.

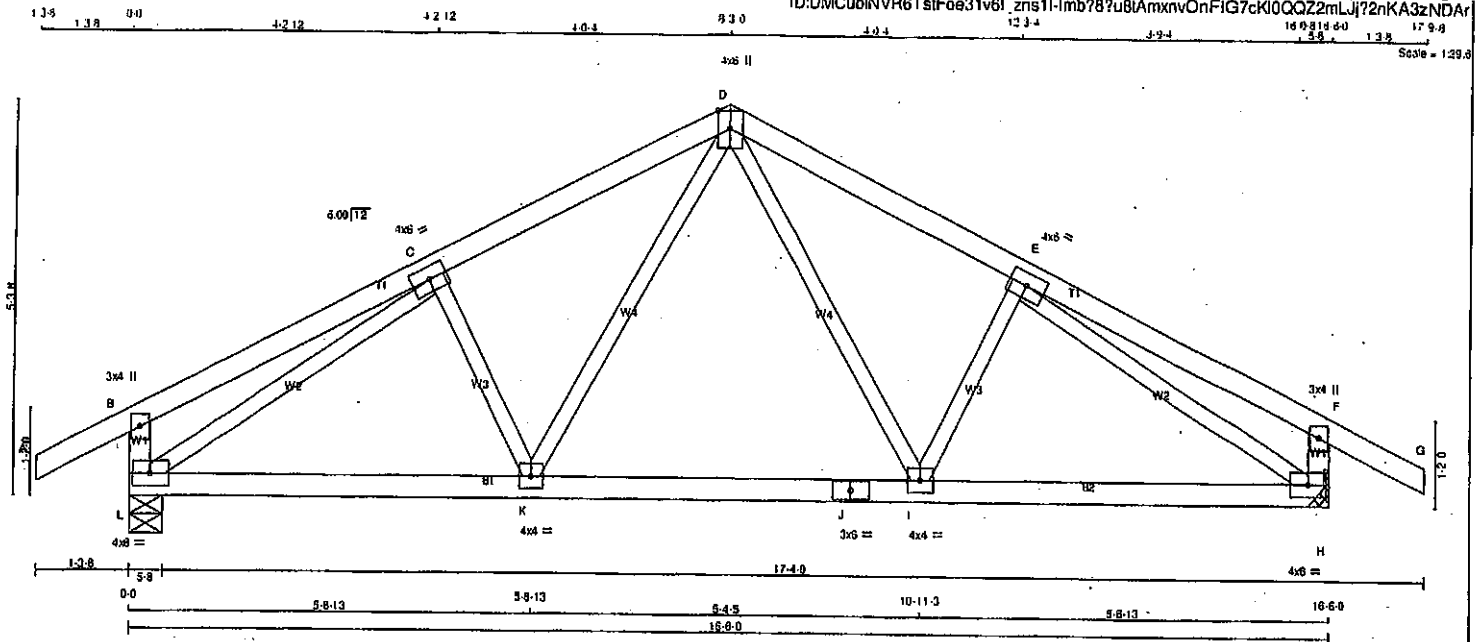


Structural component only
 DWG# T-2007043 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408133	T38	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mittek Industries, Inc. Sat Apr 25 10:35:52 2020 Page 1
ID:DMCubINVR6TstFoe31v6l_zns1l-lmb78?u8lAmxwOnFIG7cKl0QQZ2mLj72nKA3zNDAr



TOTAL WEIGHT = 2 X 66 = 133 lb
(MIP)

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF
D - G	2x4 DRY	No.2	SPF
L - B	2x4 DRY	No.2	SPF
H - F	2x4 DRY	No.2	SPF
L - J	2x4 DRY	No.2	SPF
J - H	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)			
JT	TYPE	PLATES	W LEN Y X
B	TMV+p	MT20	3.0 4.0
C	TMVW+1	MT20	4.0 8.0
D	TTWW+p	MT20	4.0 6.0 Edge
E	TMVW+1	MT20	4.0 8.0
F	TMV+p	MT20	3.0 4.0
H	BMVW+1	MT20	4.0 8.0
I	BMVW+1	MT20	4.0 4.0
J	BS+1	MT20	3.0 8.0
K	BMVW+1	MT20	4.0 4.0
L	BMVW+1	MT20	4.0 8.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERY	GROSS REACTION	GROSS REACTION	BRG	BRG
L	1034	0	1034	0	5-8
H	1034	0	1034	0	5-8
				MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-B.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	728	492 / 0	0 / 0	0 / 0	0 / 0	237 / 0	0 / 0
H	728	492 / 0	0 / 0	0 / 0	0 / 0	237 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	D-I	0 / 354	0.08 (1)
B-C	0 / 19	-91.8 -91.8	0.24 (1)	10.00	I-E	-228 / 0	0.05 (1)
C-D	-1044 / 0	-91.8 -91.8	0.20 (1)	5.93	K-D	0 / 354	0.08 (1)
D-E	-1044 / 0	-91.8 -91.8	0.20 (1)	5.93	C-K	-228 / 0	0.05 (1)
E-F	0 / 19	-91.8 -91.8	0.24 (1)	10.00	L-C	-1245 / 0	0.51 (1)
F-G	0 / 28	-91.8 -91.8	0.12 (1)	10.00	E-H	-1245 / 0	0.51 (1)
L-B	-271 / 0	0.0 0.0	0.03 (1)	7.81			
H-F	-271 / 0	0.0 0.0	0.03 (1)	7.81			
L-K	0 / 1016	-18.5 -18.5	0.23 (1)	10.00			
K-J	0 / 749	-18.5 -18.5	0.19 (4)	10.00			
J-I	0 / 749	-18.5 -18.5	0.19 (4)	10.00			
I-H	0 / 1016	-18.5 -18.5	0.23 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN.C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, A9C 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) =	L/360 (0.55")
CALCULATED VERT. DEFL.(LL) =	L/999 (0.03")
ALLOWABLE DEFL.(TL) =	L/360 (0.55")
CALCULATED VERT. DEFL.(TL) =	L/999 (0.06")

CSI: TC=0.24/1.00 (B-C:1), BC=0.23/1.00 (H-I:1), WB=0.51/1.00 (E-H:1), SSI=0.18/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP (DRY)	SHEAR (PS)	SECTION (PL)	
MT20	618	354	1667 768 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

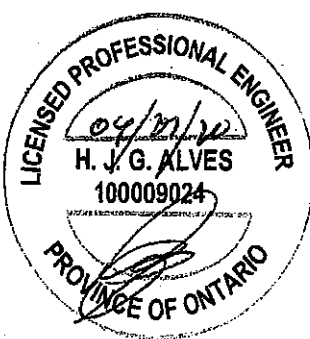
PLATE ROTATION TOL. = 5.0 Deg.

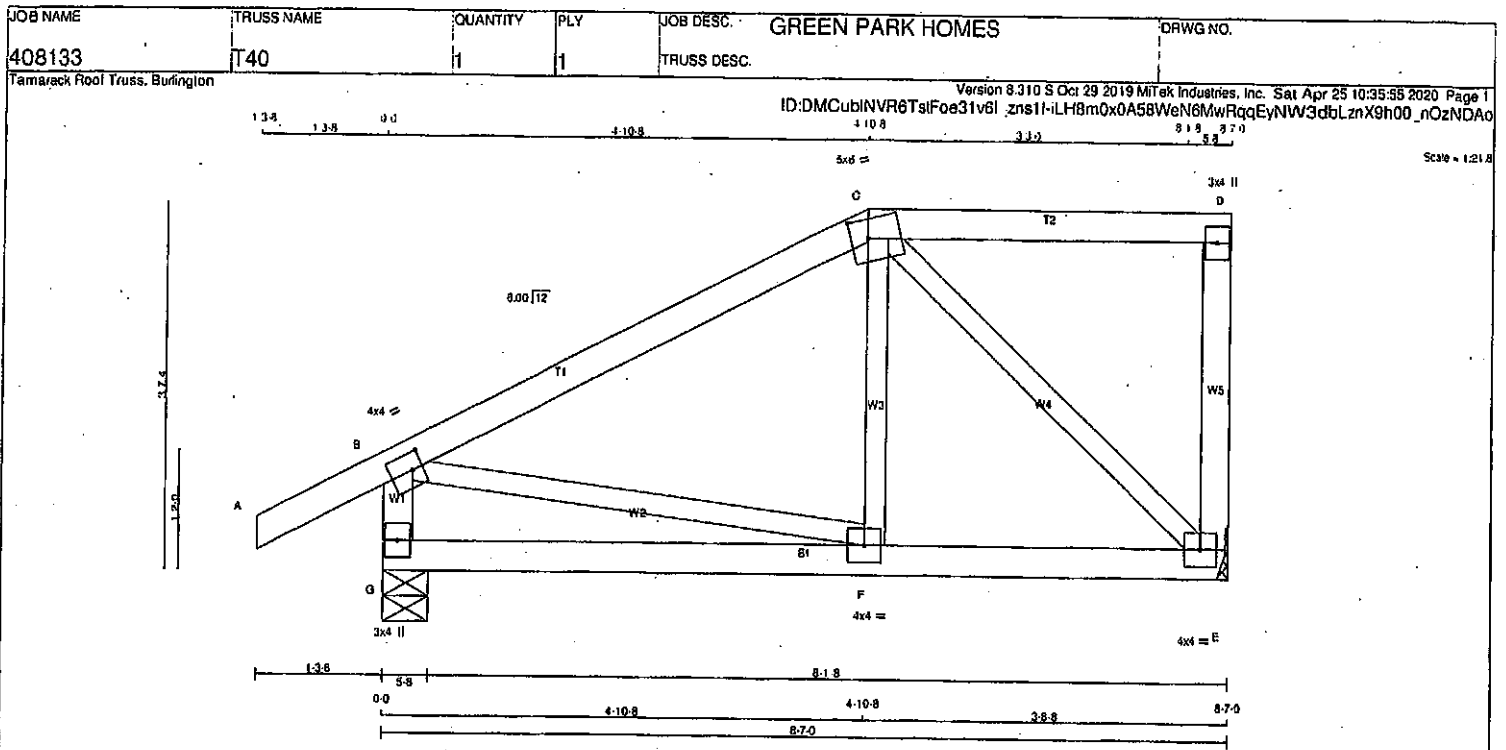
JSI GRIP = 0.76 (E) (INPUT = 0.90)

JSI METAL = 0.31 (E) (INPUT = 1.00)



Structural component only
DWG# T-2007044





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - D	2x4	DRY No.2	SPF
E - D	2x4	DRY No.2	SPF
G - B	2x4	DRY No.2	SPF
G - E	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	4.0	2.00 1.25
C	TTWW-m	MT20	5.0	6.0	2.25 2.00
D	TMVp	MT20	3.0	4.0	
E	BMVW-1	MT20	4.0	4.0	
F	BMVW-1	MT20	4.0	4.0	
G	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	473	0	473	0	5-8	5-8
G	598	0	598	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
E	335	220 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
G	420	289 / 0	0 / 0	0 / 0	0 / 0	131 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS		MAX. UNBRACED LENGTH
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	VERT. LOAD (LBS)	MAX. FACTORED FORCE (LBS)			MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	
FR-TO						FR-TO			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	F-C	0 / 82	0.03 (4)	
B-C	-360 / 0	-91.8	-91.8	0.28 (1)	6.25	C-E	-421 / 0	0.15 (1)	
C-D	0 / 0	-91.8	-91.8	0.21 (1)	10.00	B-F	0 / 318	0.07 (1)	
E-D	-170 / 0	0.0	0.0	0.03 (1)	7.81				
G-B	-559 / 0	0.0	0.0	0.08 (1)	7.81				
G-F	0 / 0	-18.5	-18.5	0.10 (4)	10.00				
F-E	0 / 315	-18.5	-18.5	0.13 (4)	10.00				

TOTAL WEIGHT = 36 lb

(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.29")
CALCULATED VERT. DEFL.(LL) = $L/989$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.29")
CALCULATED VERT. DEFL.(TL) = $L/989$ (0.02")

CSI: TC=0.28/1.00 (B-C:1), BC=0.13/1.00 (E-F:4), WB=0.15/1.00 (C-E:1), SB=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1887 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

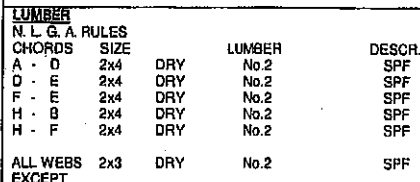
JSI GRIP = 0.57 (8) (INPUT = 0.90)
JSI METAL = 0.19 (8) (INPUT = 1.00)



Structural component only
DWG# T-2007046

Tamarack Roof Truss, Burlington

Scale = 1:250



DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
<u>BEARINGS</u>							
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQ'D BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
JT	473	0	473	0	0	MECHANICAL	
H	598	0	598	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	335	220 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
H	420	289 / 0	0 / 0	0 / 0	0 / 0	131 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING ≈ 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	0/28	-91.8	-91.8 0.12 (1)	10.00	C-G	-292/0	
B-C	0/16	-91.8	-91.8 0.17 (1)	10.00	G-D	-117/0	0.09 (1)
C-D	-199/0	-91.8	-91.8 0.13 (1)	8.25	G-E	0/460	0.04 (1)
D-E	-160/0	-91.8	-91.8 0.03 (1)	8.25	H-C	-498/0	0.10 (1)
F-E		0.0	0.0 0.17 (1)	7.81			0.15 (1)
H-B	-248/0	0.0	0.0 0.02 (1)	7.81			
H-G	0/399	-16.5	-16.5 0.23 (4)	10.00			
G-F	0/0	-18.5	-18.5 0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD =		39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/380$ (0.29")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL)= $L/380$ (0.29")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.07")

CSI: TC=0.17/1.00 (B-C:1), BC=0.23/1.00 (G-H:4), WB=0.15/1.00 (C-H:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

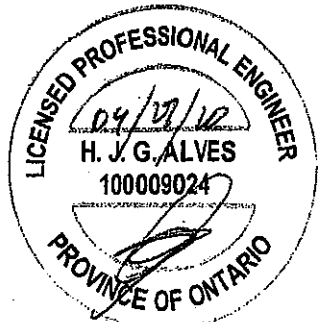
NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	610	354	1687	788	1987	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. ≈ 5.0 Deg.

JSI GRIP= 0.46 (G) (INPUT = 0.90)
JSI METAL= 0.15 (H) (INPUT = 1.00)

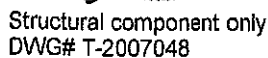


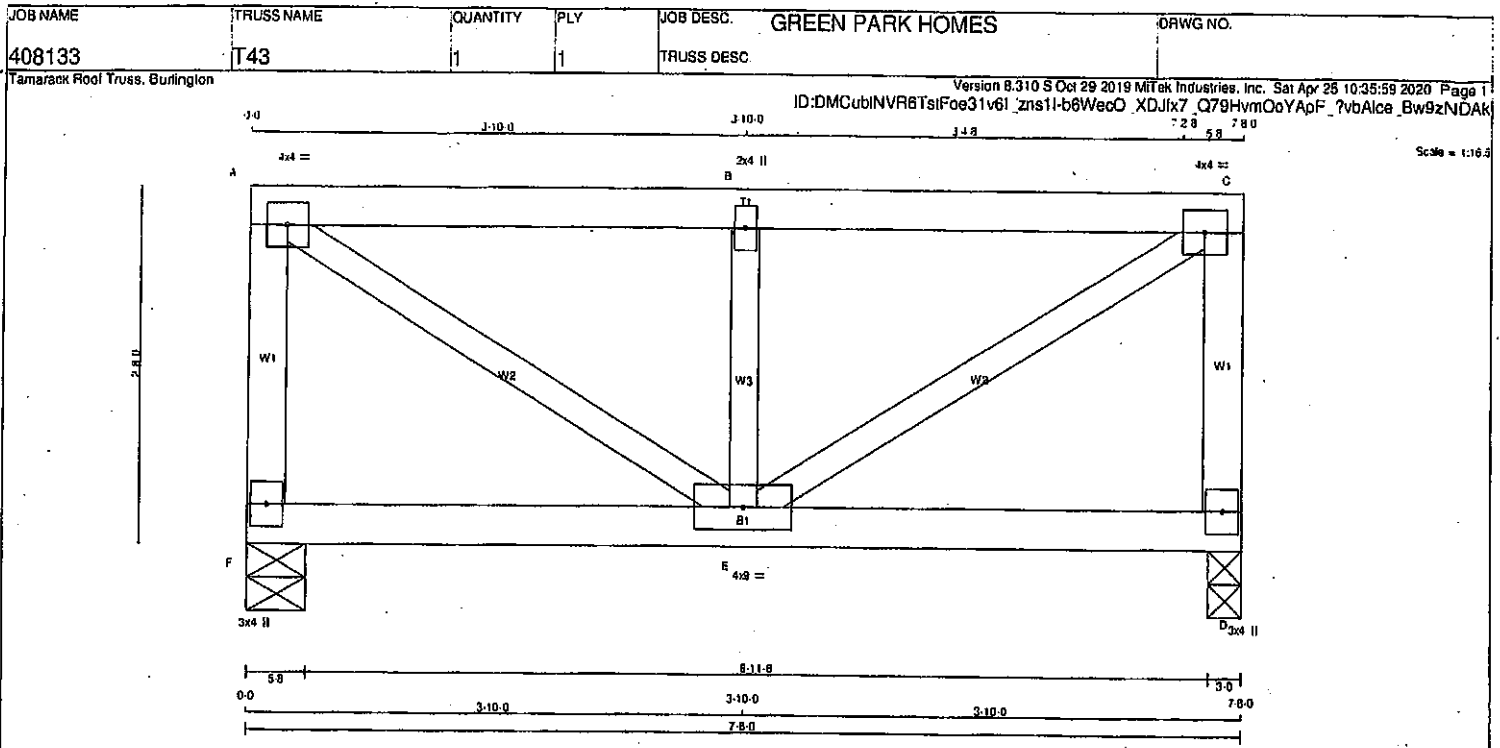
Structural component only
DWG# T-2007047



DRY: SEASONED LUMBER.

JSI GRIP= 0.73 (B) (INPUT = 0.90)
JSI METAL= 0.22 (B) (INPUT = 1.00)





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0		
B	TMVW-w	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
D	BMV1-p	MT20	3.0	4.0		
E	BMVW-w	MT20	4.0	9.0		
F	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
F	509	0	0	5-8
D	509	0	0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
F	368	196 / 0	0 / 0	0 / 0	172 / 0	0 / 0
D	368	196 / 0	0 / 0	0 / 0	172 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
F-A	-478 / 0	0.0 0.0	0.08 (1)	7.81	A-E	0 / 587 0.18 (1)	
A-B	-499 / 0	-114.3 -114.3	0.38 (1)	6.25	E-B	-538 / 0 0.12 (1)	
B-C	-499 / 0	-114.3 -114.3	0.38 (1)	6.25	E-C	0 / 587 0.18 (1)	
D-C	-478 / 0	0.0 0.0	0.08 (1)	7.81			
F-E	0 / 0	-18.5 -18.5	0.08 (4)	10.00			
E-D	0 / 0	-18.5 -18.5	0.08 (4)	10.00			

TOTAL WEIGHT = 30 lb

(M/R)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8 PSF
DL = 15.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 48.0 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.38/1.00 (A-B:1), BC=0.08/1.00 (E-F:4), WB=0.18/1.00 (A-E:1), SSI=0.28/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

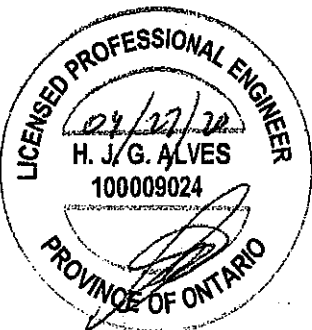
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (A) (INPUT = 0.90)
JSI METAL= 0.25 (A) (INPUT = 1.00)



Structural component only
DWG# T-2007049

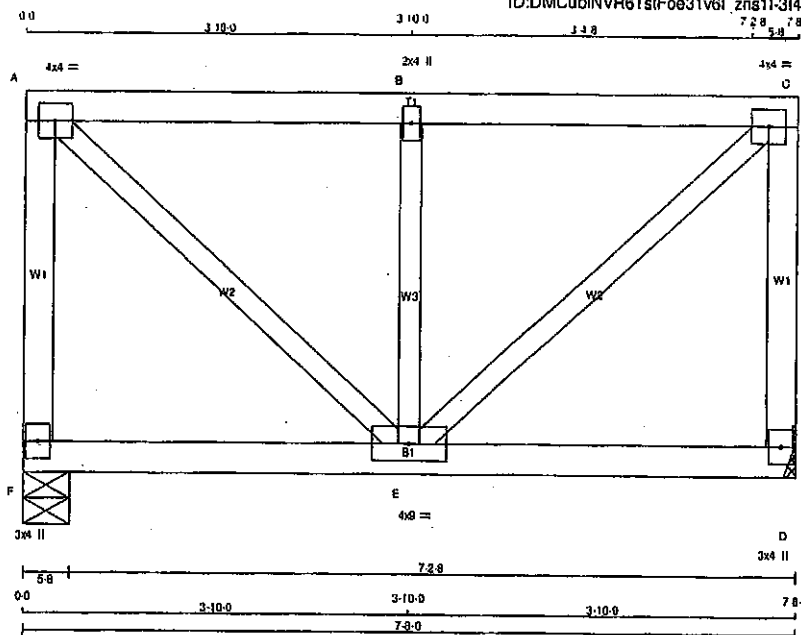


(MIR)

GABLE STUDS SPACED AT 2'-0" OC.

JSI GRIP= 0.17 (B) (INPUT = 0.80)
JSI METAL= 0.07 (I) (INPUT = 1.00)





LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY No.2	SPF
A - C	2x4	DRY No.2	SPF
D - C	2x4	DRY No.2	SPF
F - D	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2

DRY: SEASONED LUMBER.

PLATES (Labels in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-I	MT20	4.0	4.0		
B	TMVW-w	MT20	2.0	4.0		
C	TMVW-I	MT20	4.0	4.0		
D	BMV1-p	MT20	3.0	4.0		
E	BMVWW-I	MT20	4.0	9.0		
F	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	509	0	509	0	5-8	5-8
D	509	0	509	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1'-8".

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	368	198 / 0	0 / 0	0 / 0	0 / 0	172 / 0	0 / 0
D	368	198 / 0	0 / 0	0 / 0	0 / 0	172 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LBS)
FR-TO		FROM TO	CSI (LC)	UNBRAC LENGTH	FR-TO		CSI (LC)
F-A	-479 / 0	0.0 0.0	0.13 (1)	7.81	A-E	0 / 471	0.14 (1)
A-B	-354 / 0	-114.3 -114.3	0.36 (1)	6.25	E-B	-541 / 0	0.16 (1)
B-C	-354 / 0	-114.3 -114.3	0.36 (1)	6.25	E-C	0 / 471	0.14 (1)
D-C	-479 / 0	0.0 0.0	0.13 (1)	7.81			
F-E	0 / 0	-18.5 -18.5	0.09 (4)	10.00			
E-D	0 / 0	-18.5 -18.5	0.09 (4)	10.00			

TOTAL WEIGHT = 4 X 35 = 138 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 15.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 48.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.26")

CALCULATED VERT. DEFL. (LL) = L/999 (0.01")

ALLOWABLE DEFL. (TL) = L/360 (0.26")

CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.36/1.00 (B-C:1), BC=0.09/1.00 (E-F:4), WB=0.16/1.00 (B-E:1), SS=0.28/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)		SHEAR (PLI)	SECTION (PLI)
	(PSI)	(PLI)		
MT20	618	354	1667	788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

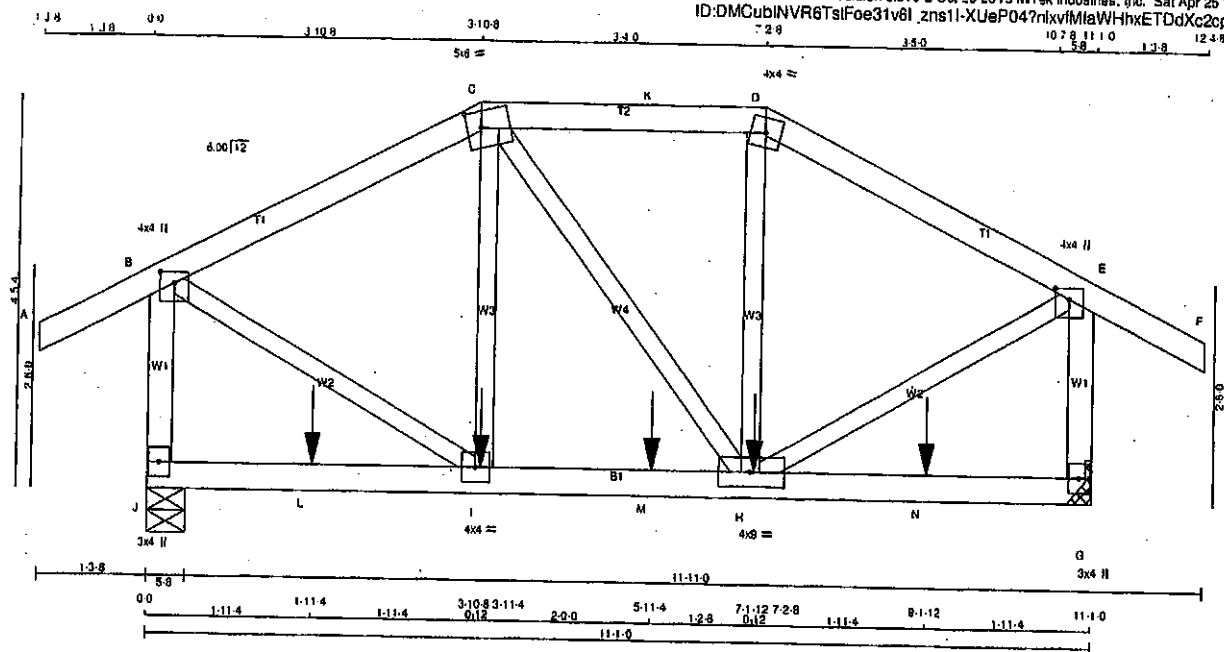
JSI GRIP = 0.84 (C) (INPUT = 0.90)

JSI METAL = 0.18 (A) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408133	T45	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/25

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.50	2.00
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.50	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-i	MT20	4.0	9.0		
I	BMWW-i	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORIZ	DOWN	HORIZ
J	1205	0	1205	0
G	1218	0	1218	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	845	592 / 0	0 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
G	855	599 / 0	0 / 0	0 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6-20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10-00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. (LC1)	MEMB.	FORCE (LBS)	MAX. (LC1)	MAX. (LC1)
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8	-91.8 0.13 (1)	I-C	0 / 51	0.02 (4)	
B-C	-844 / 0	-91.8	-91.8 0.28 (1)	C-H	0 / 25	0.01 (1)	
C-K	-771 / 0	-91.8	-91.8 0.20 (1)	H-D	0 / 58	0.02 (4)	
K-D	-771 / 0	-91.8	-91.8 0.20 (1)	B-I	0 / 881	0.21 (1)	
D-E	-861 / 0	-91.8	-91.8 0.28 (1)	H-E	0 / 878	0.22 (1)	
E-F	0 / 28	-91.8	-91.8 0.13 (1)				
J-B	-1098 / 0	0.0	0.0 0.15 (1)				
G-E	-1112 / 0	0.0	0.0 0.18 (1)				
J-L	0 / 0	-18.5	-18.5 0.26 (1)				
L-I	0 / 0	-18.5	-18.5 0.26 (1)				
I-M	0 / 755	-18.5	-18.5 0.32 (1)				
M-H	0 / 755	-18.5	-18.5 0.32 (1)				
H-N	0 / 0	-18.5	-18.5 0.25 (1)				
N-G	0 / 0	-18.5	-18.5 0.25 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
H	7-1-12	-187	-187	---	BACK	VERT	TOTAL	C1
I	3-11-4	-187	-187	---	BACK	VERT	TOTAL	C1
L	1-11-4	-195	-195	---	BACK	VERT	TOTAL	C1
M	5-11-4	-187	-187	---	BACK	VERT	TOTAL	C1
N	9-1-12	-195	-195	---	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 51 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.37")

CALCULATED VERT. DEFL.(LL) = 1/989 (0.03")

ALLOWABLE DEFL.(TL) = L/360 (0.37")

CALCULATED VERT. DEFL.(TL) = 1/989 (0.05")

CSI: TC=0.28/1.00 (D-E:1), BC=0.32/1.00 (H-I:1), WB=0.22/1.00 (E-H:1), SS=0.14/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

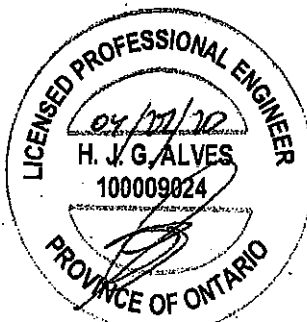
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

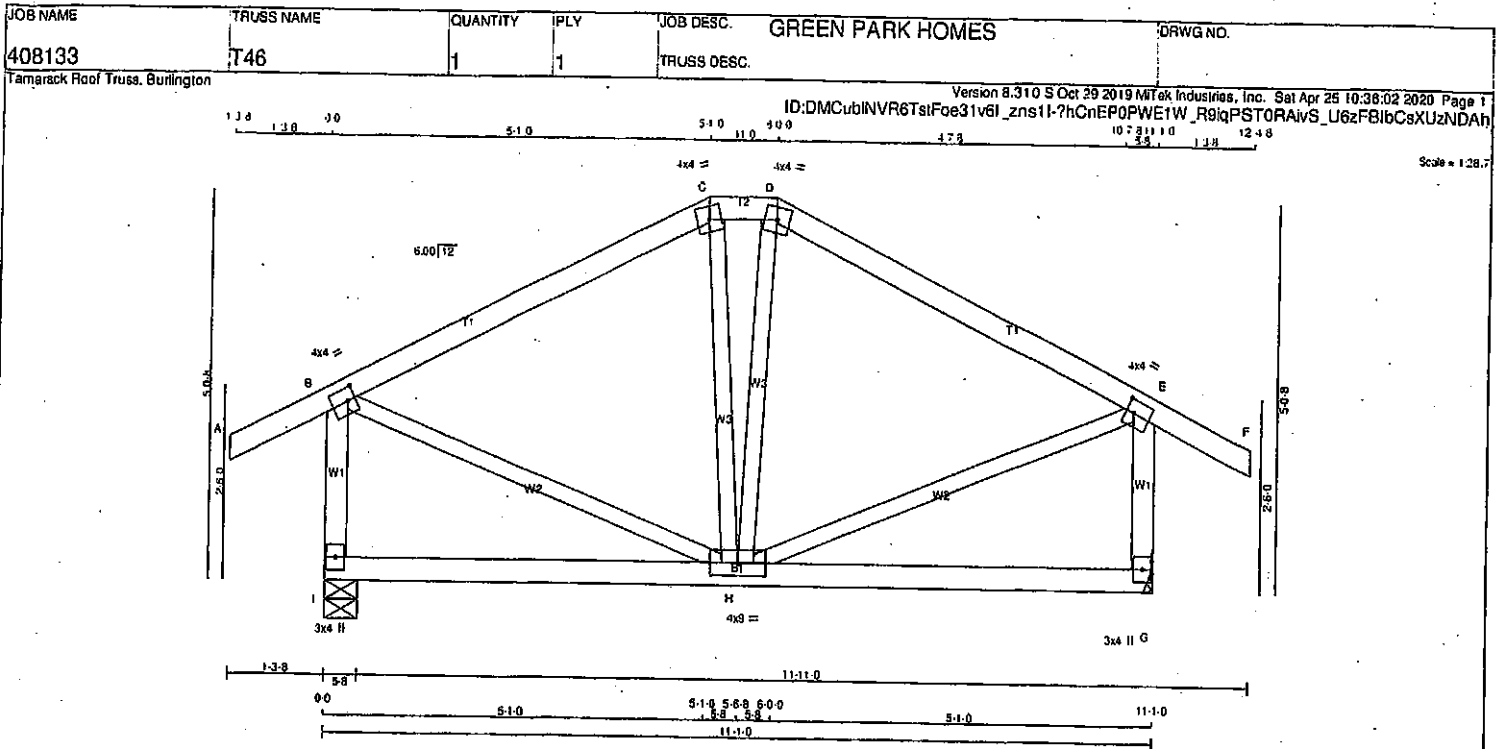
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)

JSI METAL= 0.28 (I) (INPUT = 1.00)



Structural component only
DWG# T-2007051



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	4.0	2.00	1.25
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMVW-1	MT20	4.0	4.0	2.00	1.25
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-1	MT20	4.0	9.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
I	735	0	735	0	5-8	5-8
G	735	0	735	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1'-8".

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX/MIN. SNOW	MAX/MIN. LIVE	MAX/MIN. PERM. LIVE	MAX/MIN. WIND	MAX/MIN. DEAD	MAX/MIN. SOIL
I	518	353 / 0	0 / 0	0 / 0	0 / 0	185 / 0	0 / 0
G	518	353 / 0	0 / 0	0 / 0	0 / 0	185 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (LBS)		MAX. CS (LC)		WEBS MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
	FR-TO	FROM TO	FR-TO	FROM TO	FR-TO	FROM TO	FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 28		-91.8	-91.8	0.12 (1)	10.00	B-H	0 / 405	0.09 (1)	
B-C	-422 / 0		-91.8	-91.8	0.31 (1)	6.25	H-E	0 / 405	0.09 (1)	
C-D	-369 / 0		-91.8	-91.8	0.01 (1)	6.25	C-H	-88 / 18	0.03 (1)	
D-E	-422 / 0		-91.8	-91.8	0.31 (1)	6.25	H-D	-86 / 18	0.03 (1)	
E-F	0 / 28		-91.8	-91.8	0.12 (1)	10.00				
I-B	-886 / 0		0.0	0.0	0.09 (1)	7.81				
G-E	-886 / 0		0.0	0.0	0.09 (1)	7.81				
I-H	0 / 0		-18.5	-18.5	0.16 (4)	10.00				
H-G	0 / 0		-18.5	-18.5	0.16 (4)	10.00				

TOTAL WEIGHT = 51 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012, ABC 2010
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.37")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.37")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CSI: TC=0.31/1.00 (D-E:1), BC=0.18/1.00 (G-H:4), WB=0.09/1.00 (B-H:1), SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

MAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1686

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (E) (INPUT = 0.90)
JSI METAL = 0.22 (E) (INPUT = 1.00)

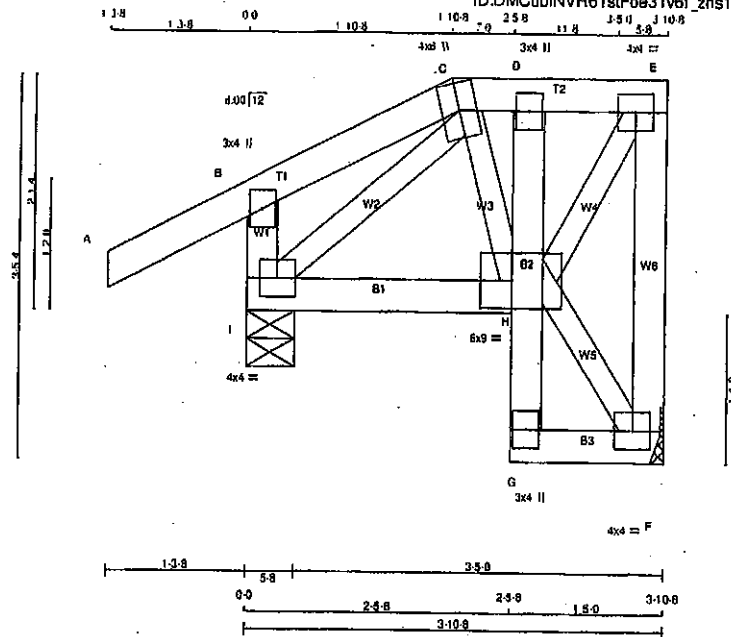


Structural component only
DWG# T-2007052

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408133	T47S	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:19.8

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - H	2x4	DRY	No.2
I - J	2x4	DRY	No.2
K - L	2x4	DRY	No.2
M - N	2x4	DRY	No.2
O - P	2x4	DRY	No.2
Q - R	2x4	DRY	No.2
S - T	2x4	DRY	No.2
U - V	2x4	DRY	No.2
W - X	2x4	DRY	No.2
Y - Z	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW+m	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
E	TMVW-1	MT20	4.0	4.0		
F	BMVW-1	MT20	4.0	4.0		
G	BMV+p	MT20	3.0	4.0		
H	BMVW-1	MT20	6.0	9.0	3.00	3.50
I	BMVW-1	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	214	0	214	0	0	MECHANICAL	
I	338	0	338	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	151	98 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0
I	237	189 / 0	0 / 0	0 / 0	0 / 0	88 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.		FORCE		MEMB.		FORCE	
FR-TO		(LBS)		FR-TO		(LBS)	
		VERT. LOAD					
		LC1 MAX					
		PLF					
		CSI (LC)					
		UNBRACED LENGTH					
A-B	0 / 28	-91.8	-91.8 0.14 (5)	10.00	C-H	-13 / 7	0.00 (4)
B-C	0 / 0	-91.8	-91.8 0.08 (1)	10.00	H-F	-8 / 0	0.00 (1)
C-D	-103 / 0	-91.8	-91.8 0.01 (1)	6.25	I-C	-148 / 0	0.03 (1)
D-E	-98 / 0	-91.8	-91.8 0.02 (1)	6.25	H-E	0 / 172	0.04 (1)
E-F	-196 / 0	0.0	0.0 0.04 (1)	7.81			
F-G	-212 / 0	0.0	0.0 0.02 (1)	7.81			
I-H	0 / 107	-18.5	-18.5 0.04 (4)	10.00			
G-H	0 / 13	0.0	0.0 0.01 (1)	10.00			
H-D	-98 / 0	0.0	0.0 0.01 (1)	7.81			
G-F	0 / 5	-18.5	-18.5 0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 24 = 48 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/898 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/898 (0.00")

CSI: TC=0.14/1.00 (A-B:5), BC=0.04/1.00 (H-I:4),
WB=0.04/1.00 (E-H:1), SS=0.10/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

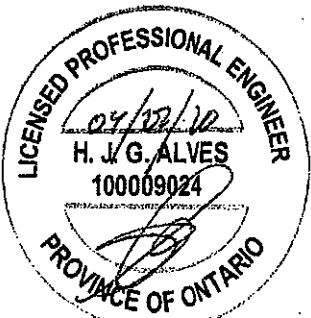
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PL) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

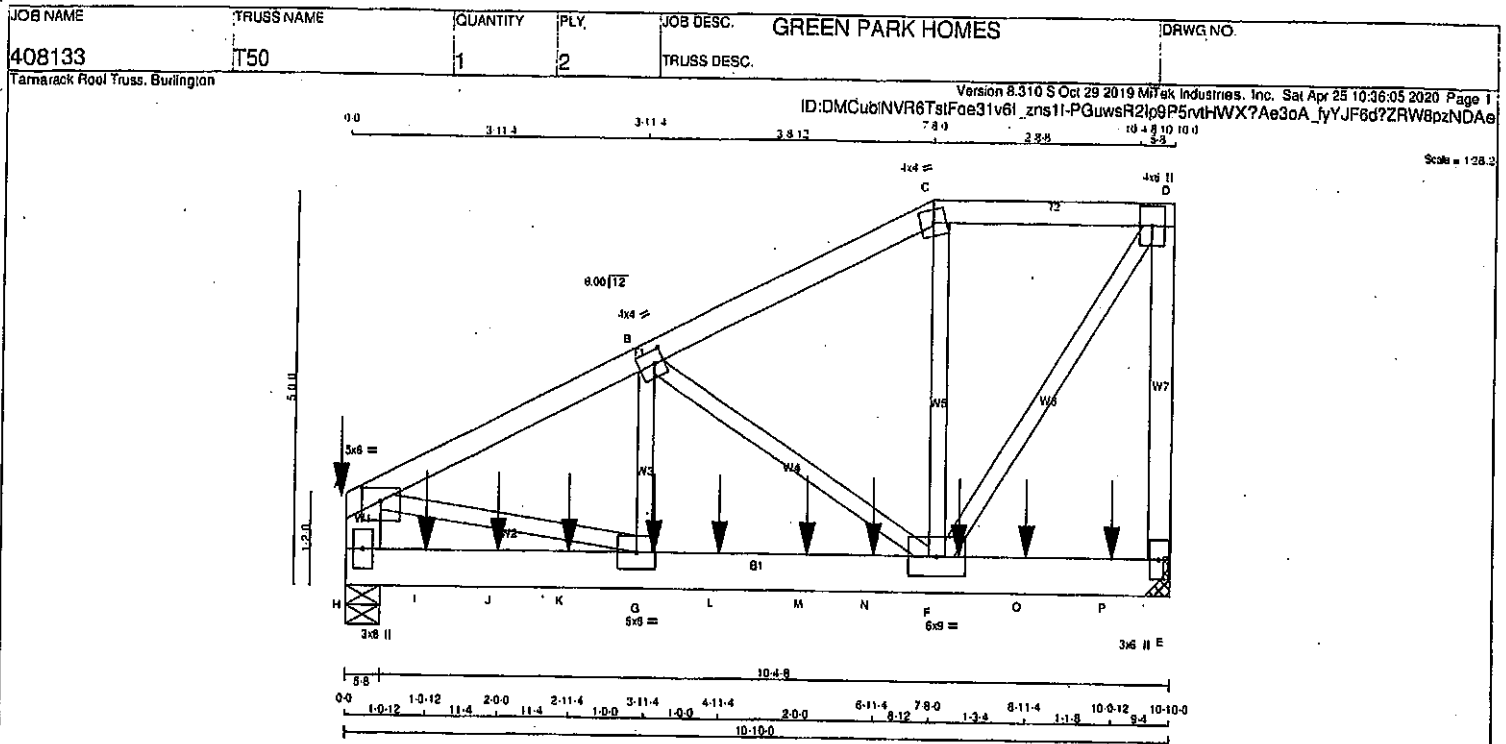
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.21 (H) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007053



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
H - A	2x6	DRY	No.2
H - E	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - D	12	TOP
D - E	12	TOP
H - A	12	TOP
H - E	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
H - E	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	8	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-0	MT20	5.0	8.0	2.00	3.00
B	TMWW-1	MT20	4.0	4.0	2.00	1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	IN-SX
E	3011	0	3011	0
H	2825	0	2825	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 4'-0".

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS
E	2142	1336 / 0	0 / 0
H	2004	1278 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO			
A-B	-3575 / 0	-91.8	-91.8
B-C	-1809 / 0	-91.8	-91.8
C-D	-1816 / 0	-91.8	-91.8
E-D	-2581 / 0	0.0	0.0
H-A	-2441 / 0	0.0	0.0
H-I	0 / 0	-18.5	-18.5
I-J	0 / 0	-18.5	-18.5
J-K	0 / 0	-18.5	-18.5
K-G	0 / 0	-18.5	-18.5
G-L	0 / 3210	-18.5	-18.5
L-M	0 / 3210	-18.5	-18.5
M-N	0 / 3210	-18.5	-18.5
N-F	0 / 3210	-18.5	-18.5
F-O	0 / 0	-18.5	-18.5
O-P	0 / 0	-18.5	-18.5
P-E	0 / 0	-18.5	-18.5

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
A	0-0	-53	-53	---	TOP	VERT	TOTAL	---	G1
F	8-0-12	-490	-490	---	FRONT	VERT	TOTAL	---	C1
G	4-0-12	-490	-490	---	FRONT	VERT	TOTAL	---	C1
I	1-0-12	-70	-70	---	BACK	VERT	TOTAL	---	C1
J	2-0-0	-499	-499	---	FRONT	VERT	TOTAL	---	C1
K	2-11-4	-705	-705	---	BACK	VERT	TOTAL	---	C1
L	4-11-4	-455	-455	---	BACK	VERT	TOTAL	---	C1
M	8-0-12	-490	-490	---	FRONT	VERT	TOTAL	---	C1
N	8-11-4	-455	-455	---	BACK	VERT	TOTAL	---	C1
O	8-11-4	-440	-440	---	BACK	VERT	TOTAL	---	C1
P	10-0-12	-493	-493	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) G1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 55 = 109 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.36")
CALCULATED VERT. DEFL. (LL) = L/999 (0.03")
ALLOWABLE DEFL. (TL) = L/360 (0.36")
CALCULATED VERT. DEFL. (TL) = L/999 (0.06")

CSI: TC=0.51/1.00 (D-E:1), BC=0.39/1.00 (F-G:1).
WB=0.40/1.00 (A-G:1), SS=0.32/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (RY)	SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX
618	354	1667
788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (B) (INPUT = 0.90)
JSI METAL = 0.40 (G) (INPUT = 1.00)

CONTINUED ON PAGE 2



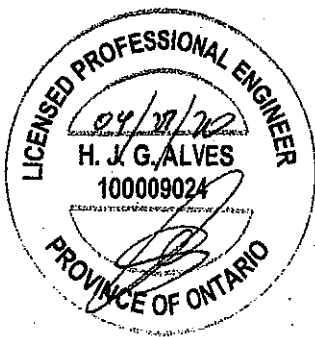
Structural component only
DWG# T-2007054 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408133	T50	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

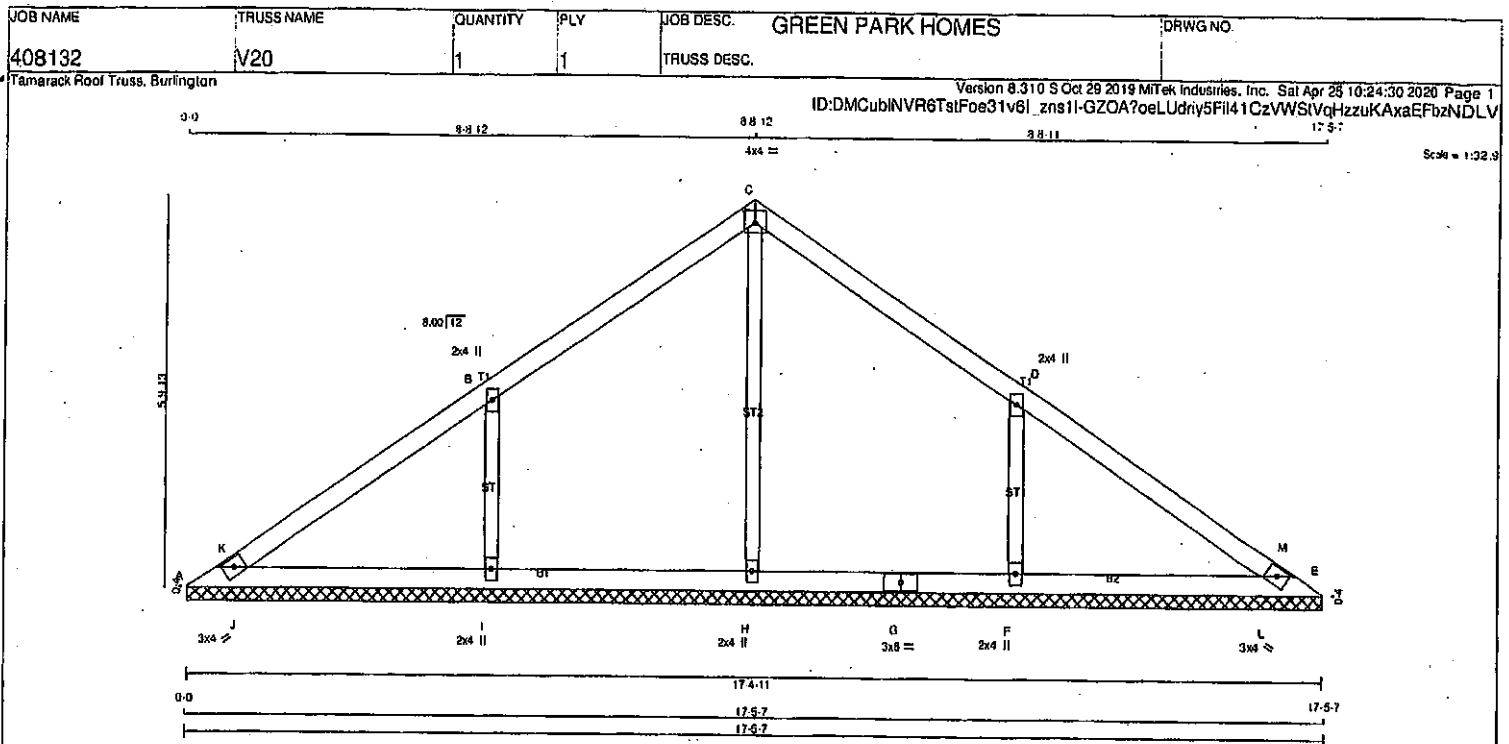
Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:36:05 2020 Page 2.
ID:DMCubINVR6TsiFoe31v6l zns1I-PGuwsR2ip9P5rvtHWX?Ae3oA fyYJF6d?ZRW8ozND Ae

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TTW-m	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	6.0		
F	BMWWW-t	MT20	6.0	9.0		
G	BMWWW-l	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	6.0		



Structural component only
DWG# T-2007054 3/2



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
A - G	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW+w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMW+w	MT20	2.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F, H, I	8MW1+w	MT20	2.0	4.0		
G	BS-l	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
A	137	0	137	0	17-4-11 (8-4-15/4-11)	IN-SX
E	137	0	137	0	17-4-11 (8-4-15/4-11)	IN-SX
H	482	0	482	0	17-4-11 (8-4-15/4-11)	IN-SX
I	580	0	580	0	17-4-11 (8-4-15/4-11)	IN-SX
F	580	0	580	0	17-4-11 (8-4-15/4-11)	IN-SX

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
A	97	88/0	0/0	0/0	0/0	30/0
E	97	88/0	0/0	0/0	0/0	30/0
H	344	211/0	0/0	0/0	0/0	133/0
I	410	273/0	0/0	0/0	0/0	137/0
F	410	273/0	0/0	0/0	0/0	137/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, H, I, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-K	0/109	-91.8 -91.8	0.05 (4)	10.00	H-C	-439/0	0.22 (1)
K-B	0/158	-91.8 -91.8	0.28 (1)	10.00	I-B	-453/0	0.08 (1)
B-C	0/118	-91.8 -91.8	0.28 (1)	10.00	F-D	-453/0	0.08 (1)
C-D	0/116	-91.8 -91.8	0.28 (1)	10.00	J-K	-122/8	0.00 (1)
D-M	0/158	-91.8 -91.8	0.28 (1)	10.00	L-M	-122/8	0.00 (1)
M-E	0/109	-91.8 -91.8	0.05 (4)	10.00			
A-J	-126/0	-18.5 -18.5	0.12 (1)	6.25			
J-I	-105/0	-18.5 -18.5	0.12 (1)	6.25			
I-H	-118/0	-18.5 -18.5	0.09 (1)	6.25			
H-G	-118/0	-18.5 -18.5	0.09 (1)	6.25			
G-F	-118/0	-18.5 -18.5	0.09 (1)	6.25			
F-L	-105/0	-18.5 -18.5	0.12 (1)	6.25			
L-E	-126/0	-18.5 -18.5	0.12 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.28/1.00 (B-K:1), BC=0.12/1.00 (I-J:1), WB=0.22/1.00 (C-H:1), SS=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

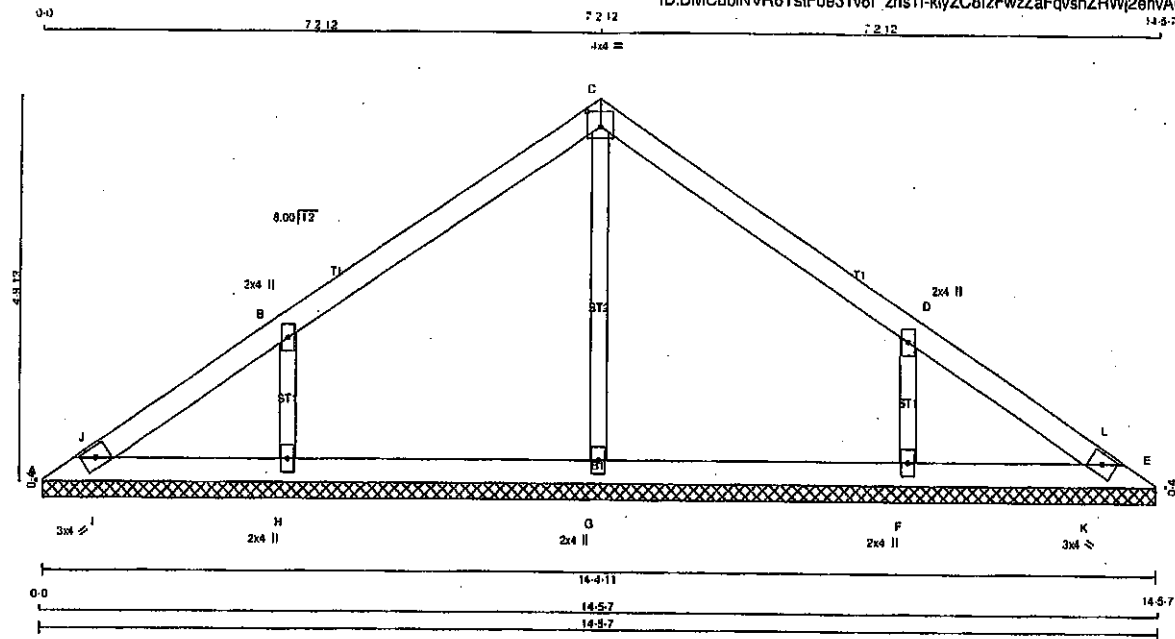
JSI GRIP = 0.40 (D) (INPUT = 0.90)
JSI METAL = 0.23 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007033

Tamarack Roof Truss, Burlington

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Scale = 1/27

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.	SPF
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
A - E	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TBM1-h	MT20	3.0	4.0		
B TMW+w	MT20	2.0	4.0		
C TTW-p	MT20	4.0	4.0	2.25	2.00
D TMW+w	MT20	2.0	4.0		
E TBM1-h	MT20	3.0	4.0		
F, G, H					
F BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REGRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
A	128	0	128	0	14-4-11	14-4-11
E	128	0	128	0	14-4-11	14-4-11
G	381	0	381	0	14-4-11	14-4-11
H	485	0	485	0	14-4-11	14-4-11
F	485	0	485	0	14-4-11	14-4-11

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	90	62 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0
E	90	62 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0
G	259	153 / 0	0 / 0	0 / 0	0 / 0	105 / 0	0 / 0
H	342	230 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
F	342	230 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED		
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	LENGTH		(LBS)	CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-J	-44 / 0	-91.8	-91.8	0.04 (1)	6.25	G-C	-292 / 0	0.10 (1)
J-B	0 / 15	-91.8	-91.8	0.21 (1)	10.00	H-B	-399 / 0	0.08 (1)
B-C	-28 / 4	-91.8	-91.8	0.21 (1)	6.25	F-D	-399 / 0	0.08 (1)
C-D	-28 / 4	-91.8	-91.8	0.21 (1)	6.25	I-J	-28 / 7	0.00 (1)
D-L	0 / 15	-91.8	-91.8	0.21 (1)	10.00	K-L	-28 / 7	0.00 (1)
L-E	-44 / 0	-91.8	-91.8	0.04 (1)	6.25			
A-I	-5 / 18	-18.5	-18.5	0.04 (1)	10.00			
I-H	-3 / 24	-18.5	-18.5	0.05 (4)	10.00			
H-G	-7 / 6	-18.5	-18.5	0.08 (4)	10.00			
G-F	-7 / 6	-18.5	-18.5	0.08 (4)	10.00			
F-K	-3 / 24	-18.5	-18.5	0.05 (4)	10.00			
K-E	-5 / 18	-18.5	-18.5	0.04 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (B-J-1), BC=0.08/1.00 (G-H-4), WB=0.10/1.00 (C-G-1), SS=0.15/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

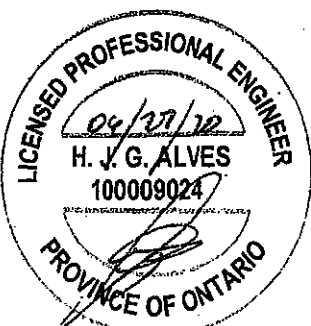
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

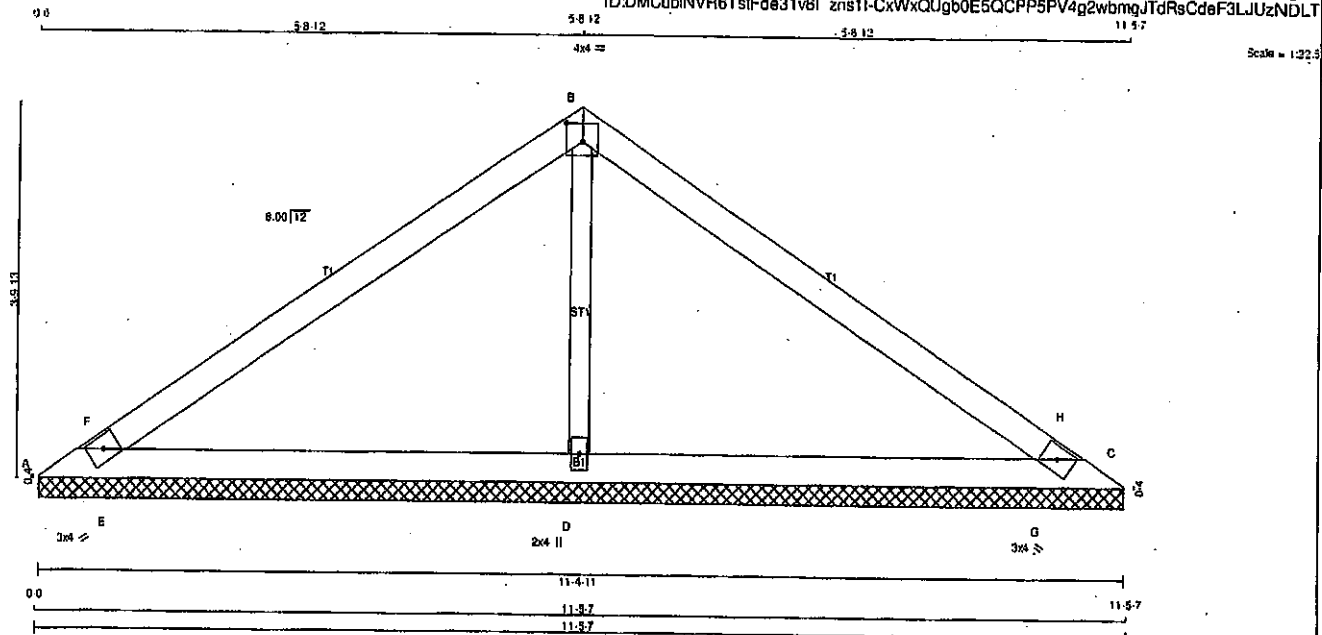
JSI GRIP = 0.27 (D) (INPUT = 0.90)
JSI METAL = 0.20 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007034

Tamarack Roof Truss, Burlington

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LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TTW-p	MT20	4.0	4.0	2.25	2.00
C	TBM1-h	MT20	3.0	4.0		
D	BMW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
A	-11	0	0	0	-11	11-4-11
C	-11	0	0	0	-11	11-4-11
D	1277	0	1277	0	0	11-4-11

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
A	-7	0/-7	0/0	0/0	0/0	0/0
C	-7	0/-7	0/0	0/0	0/0	0/0
D	902	597/0	0/0	0/0	0/0	305/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO		
A-F	0/591	-91.8 -91.8	0.25 (1)	10.00	D-B	-1043/0	0.23 (1)
F-B	0/584	-91.8 -91.8	0.39 (1)	10.00	E-F	-332/0	0.00 (1)
B-H	0/584	-91.8 -91.8	0.39 (1)	10.00	G-H	-332/0	0.00 (1)
H-C	0/591	-91.8 -91.8	0.25 (1)	10.00			
A-E	-528/0	-18.5 -18.5	0.25 (1)	6.25			
E-D	-480/0	-18.5 -18.5	0.25 (1)	6.25			
D-G	-480/0	-18.5 -18.5	0.25 (1)	6.25			
G-C	-528/0	-18.5 -18.5	0.25 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, OBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.39/1.00 (B-F:1), BC=0.25/1.00 (D-E:1), WB=0.23/1.00 (B-D:1), SS=0.19/1.00 (A-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

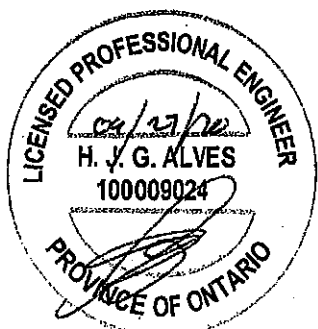
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

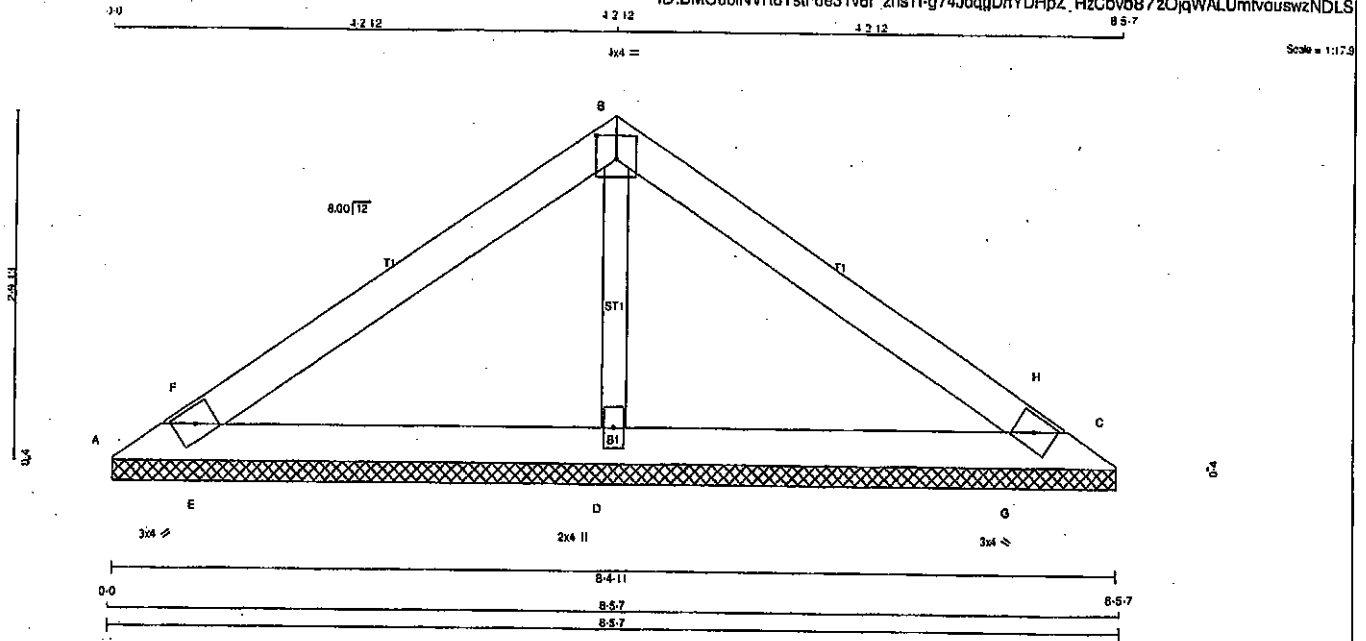
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (B) (INPUT = 0.90)
JSI METAL = 0.30 (B) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408132	V23	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.310 S Oct 29 2019 M&T Industries, Inc. Sat Apr 25 10:24:33 2020 Page 1
ID:DMCubINVR6TstFoe31v6I_znst11-g74JdqpDnYDhpZ_HzCbvb87zOjqWALUmtvouswzNDLS



LUMBER					DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF		
B - C	2x4	DRY	No.2	SPF		
A - C	2x4	DRY	No.2	SPF		
ALL WEBS					2x3	DRY
DRY: SEASONED LUMBER.					No.2	SPF

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TBM1-h	MT20	3.0	4.0	
B	TTW-p	MT20	4.0	4.0	2.25 2.00
C	TBM1-h	MT20	3.0	4.0	
D	BMW1-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REORD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
A	58	0	58	0	0	8-4-11	8-4-11		
C	58	0	58	0	0	8-4-11	8-4-11		
D	808	0	808	0	0	8-4-11	8-4-11		

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	41	27/0	0/0	0/0	0/0	14/0	0/0
C	41	27/0	0/0	0/0	0/0	14/0	0/0
D	572	375/0	0/0	0/0	0/0	198/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.26 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM TO		FR-TO			
A-F	0/297	-91.8 -91.8 0.10 (1)	10.00	D-B	-823/0	0.10 (1)	
F-B	0/294	-91.8 -91.8 0.20 (1)	10.00	E-F	-202/0	0.00 (1)	
B-H	0/294	-91.8 -91.8 0.20 (1)	10.00	G-H	-202/0	0.00 (1)	
H-C	0/297	-91.8 -91.8 0.10 (1)	10.00				
A-E	-278/0	-18.5 -18.5 0.15 (1)	8.25				
E-D	-250/0	-18.5 -18.5 0.15 (1)	8.25				
D-G	-250/0	-18.5 -18.5 0.15 (1)	8.25				
G-C	-278/0	-18.5 -18.5 0.15 (1)	8.25				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSF-TC=0.20/1.00 (B-F:1), BC=0.15/1.00 (D-E:1),
WB=0.10/1.00 (B-D:1), SSI=0.11/1.00 (A-E:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

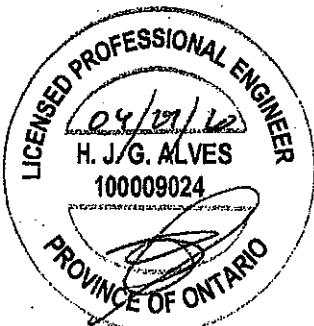
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1887 1886

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

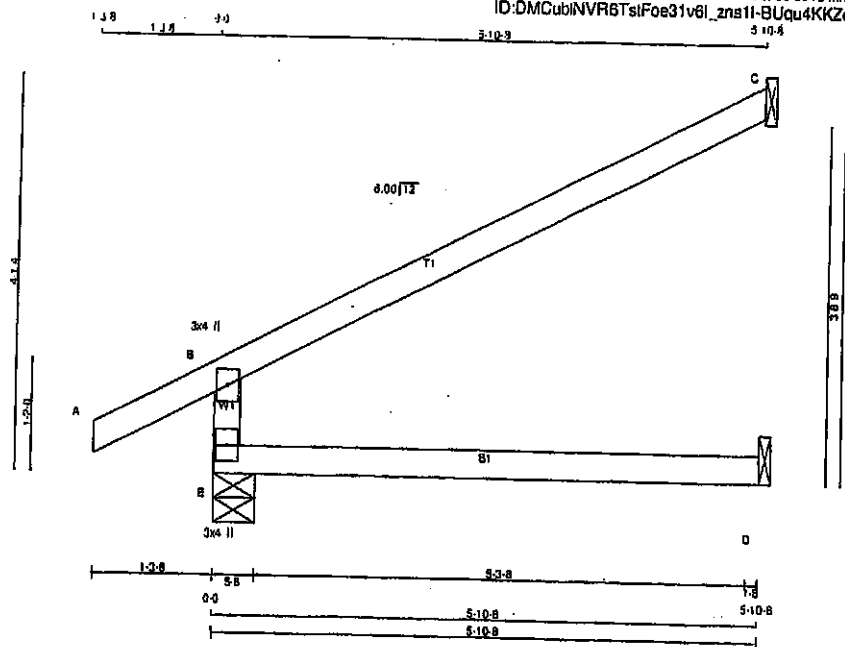
JSI GRIP= 0.52 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007036

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408087	J1	21	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubiNVR6TstFoe31v6l_zns11-BUqu4KKZoCY685uhquEMP543auqFk1_1tH0soSgzNEh6



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
E	525	0	525	0	0	6-8	5-8		
C	202	0	202	0	0	1-8	1-8		
D	45	0	50	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS		1ST LCASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
E	389	257/0	0/0	0/0	0/0
C	139	113/0	0/0	0/0	0/0
D	36	0/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	UNBRAC LENGTH	FR-TO	
E-B	-461/0	0.0 0.0 0.13 (4)	7.81		
A-B	0/29	-91.8 -91.8 0.12 (1)	10.00		
B-C	-30/0	-91.8 -91.8 0.54 (1)	6.25		
E-D	0/0	-18.5 -18.5 0.13 (4)	10.00		

TOTAL WEIGHT = 21 X 17 = 353 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (r/a:0), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	354	1887

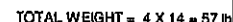
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

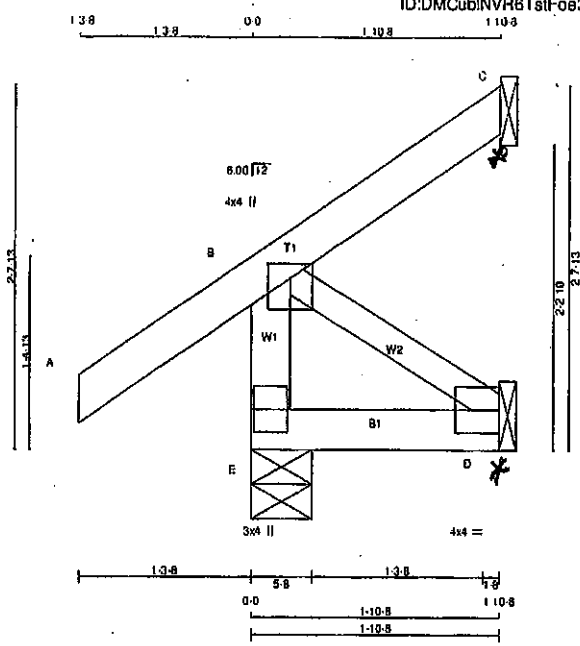
JSI GRIP = 0.19 (E) (INPUT = 0.90)
JSI METAL = 0.13 (B) (INPUT = 1.00)



Structural component only
DWG# T-20008940



Structural component only
DWG# T-2007014



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT. TYPE	PLATES	W	LEN	Y	X
B - TMVW+p	MT20	4.0	4.0	1.25	2.00
D - BMV1-l	MT20	4.0	4.0	2.00	Edge
E - BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	274	0	274	0	5-8	5-8
C	40	0	40	0	1-8	1-8
D	18	0	18	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
E	191	144 / 0	0 / 0	0 / 0	0 / 0	47 / 0
C	27	22 / 28	0 / 0	0 / 0	0 / 0	5 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

MEMB.	FR-TO	CHORDS		WEBS	
		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)
		FROM	TO	FROM	TO
E-B	-258 / 0	0.0	0.0	0.03 (1)	7.81
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B-C	-28 / 0	-91.8	-91.8	0.12 (1)	8.25
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B-1), BC=0.02/1.00 (D-E-4), WB=0.00/1.00 (B-D-1), SS=0.09/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 788

PLATE PLACEMENT TOL. = 0.250 inches

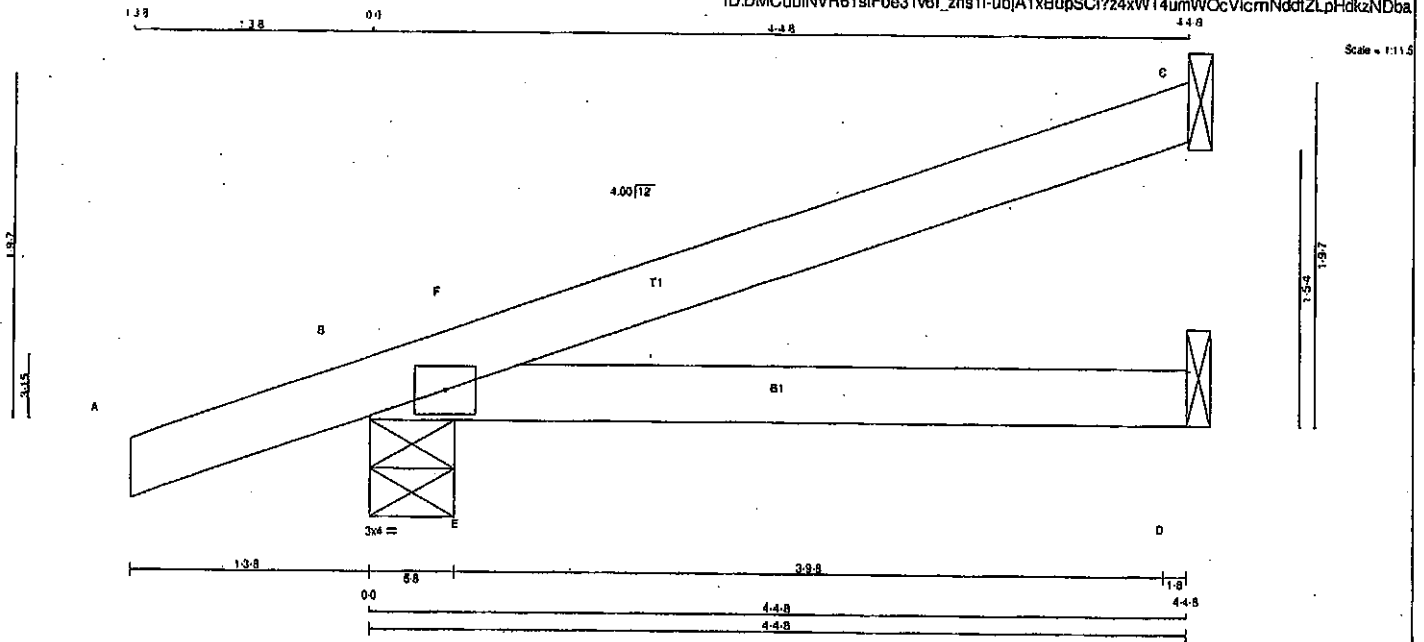
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.18 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408131	J4	10	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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ID:DMCubiNVR6TsiFoe31v6l_zns11-ubjA1xBupSCI?24xWT4umWOCVlcmNddtZLpHdkzNDba



TOTAL WEIGHT = 10 X 12 = 119 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS	2x4	No.2	SPF
A - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)			
JT TYPE	PLATES	W	LEN Y X
B TMB1-1	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		RECORD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
C	174	0	174	0	0	1-8	1-8	
B	364	0	364	0	0	5-8	5-8	
D	68	0	68	0	0	1-8	1-8	

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE					
C	120	93/0	0/0	0/0	0/0	0/0	27/0	0/0
B	255	180/0	0/0	0/0	0/0	0/0	75/0	0/0
D	50	19/0	0/0	0/0	0/0	0/0	32/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		W E B S		MAX. FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (LBS)	LC1 (PLF)	LC2 (LBS)	MAX. UNBRACED LENGTH	MEMB. FR-TO	MAX. FORCE (LBS)	MAX. CSI (LC)
FR-TO								
A-B	0/18	-91.8	-91.8	0.11 (1)	10.00	E-F	-194/7	0.00 (1)
B-F	-14/0	-91.8	-91.8	0.05 (4)	8.25			
F-C	0/2	-91.8	-91.8	0.22 (1)	10.00			
B-E	0/0	-18.5	-18.5	0.17 (1)	10.00			
E-D	0/0	-18.5	-18.5	0.17 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH. LL	= 25.6 PSF
DL	= 8.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. G.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.22/1.00 (C-F:1), BC=0.17/1.00 (D-E:1),
WB=0.00/1.00 (E-F:1), SS=0.16/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	610 354	1667 788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007016

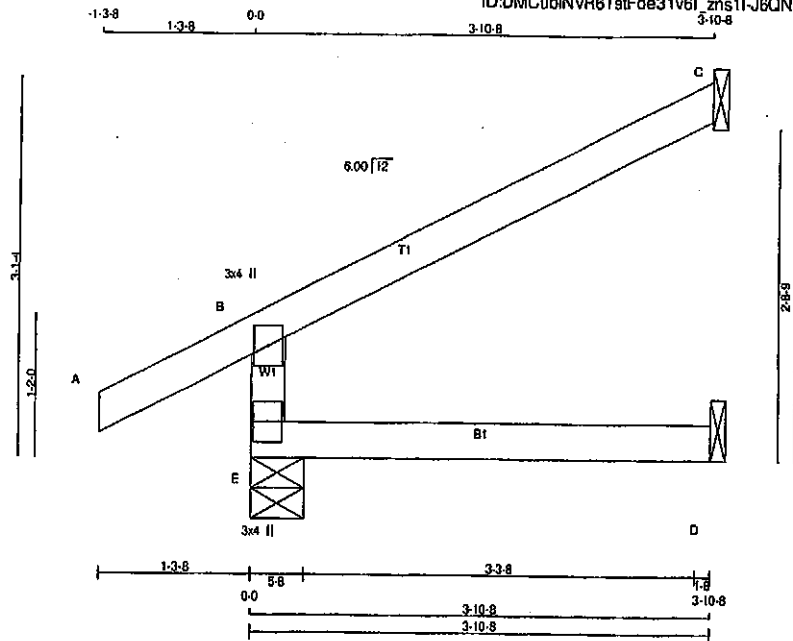
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408133	J30	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6I_zns1I-J6QNo3e7McJWaLKpv1nZJVMmex0A9P9Wb67zIZrX

Scale = 1:19.1



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
E	388	0	388	0	0	5-8	5-8	5-8	5-8
C	133	0	133	0	0	1-8	1-8	1-8	1-8
D	30	0	34	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS		1ST LCASE		MAX. MIN. COMPONENT REACTIONS		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW
E	272	193 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0	0 / 0	0 / 0
C	92	74 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0	0 / 0	0 / 0
D	24	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. MEMB.	MAX. MEMB.	FORCE (LBS)	MAX. MEMB.
FR-TO		FROM TO	LENGTH	FR-TO			
E-B	-347.0	0.0	0.05 (4)	7.81			
A-B	0.28	-91.8	-91.8	0.14 (5)	10.00		
B-C	-20.0	-91.8	-91.8	0.23 (1)	6.25		
E-D	0.0	-18.5	-18.5	0.06 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 12 = 24 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = U/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = U/999 (0.01")

CSI: TC=0.23/1.00 (B-C:1), BC=0.08/1.00 (D-E:1), WB=0.00/1.00 (n/a:0), SS=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



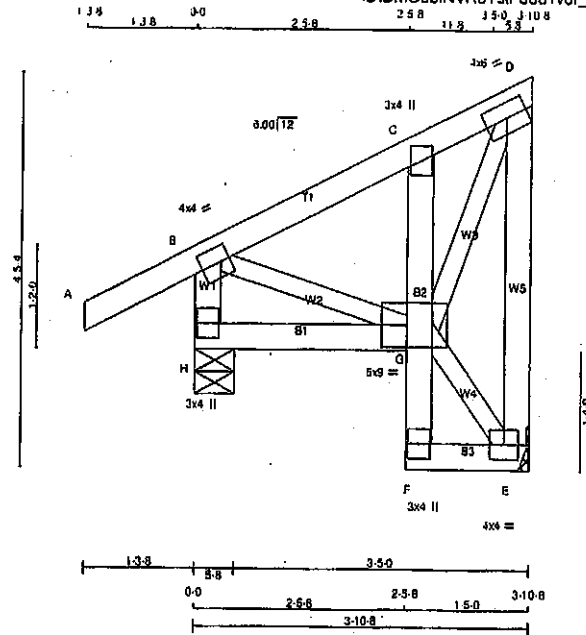
Structural component only
DWG# T-2008522

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408133	J31S	3	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6f_zns1f-xoo64Hq?2d8eg8VqSihyvG2AMPux5B9_sm4ZVszNDaw

Scale = 1/2" = 1'-0"



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
H - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
H - G	2x4	DRY	No.2
F - C	2x4	DRY	No.2
F - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-1	MT20	4.0	4.0	2.00 1.25
C	TMVW-1	MT20	3.0	4.0	
D	TMVW-1	MT20	4.0	6.0	2.00 3.00
E	BMVW-1	MT20	4.0	4.0	
F	BMVW-1	MT20	3.0	4.0	
G	BMVW-1	MT20	6.0	9.0	3.00 3.50
H	BMVW-1	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS								
	FACTORED			MAXIMUM FACTORED			INPUT	REQD
	GROSS REACTION			GROSS REACTION			BRG	BRG
JT	VERT	HORZ		DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	330	0		330	0	0	5-8	5-8
E	206	0		206	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1.8.

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	231	165 / 0	0 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0
E	145	95 / 0	0 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	MAX	LC1
FR-TO		FROM	TO	FR-TO		FROM	TO
H-B	-308 / 0	0.0	0.0 0.03 (1)	B-G	0 / 105	0.02 (1)	
A-B	0 / 28	-91.8	-91.8 0.13 (5)	E-D	-185 / 0	0.04 (1)	
B-C	-104 / 0	-91.8	-91.8 0.07 (1)	G-E	-14 / 0	0.00 (1)	
C-D	-114 / 0	-91.8	-91.8 0.05 (1)	G-D	0 / 229	0.05 (1)	
H-G	0 / 0	-18.5	-18.5 0.04 (4)				
F-G	0 / 12	0.0	0.0 0.02 (1)				
G-C	-221 / 0	0.0	0.0 0.02 (1)				
F-E	0 / 9	-18.5	-18.5 0.01 (4)				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 25 = 75 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/899 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/899 (0.00")

CSI: TC=0.13/1.00 (A-B-S), BC=0.04/1.00 (G-H-I),
WB=0.05/1.00 (D-G-I), SSI=0.10/1.00 (A-B-S)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.19 (D) (INPUT = 0.50)
JSI METAL = 0.08 (B) (INPUT = 1.00)

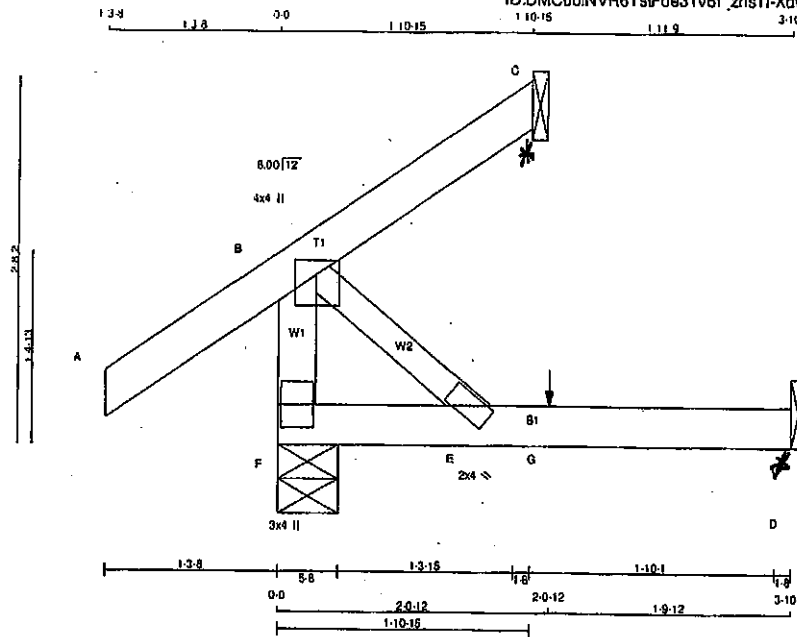


Structural component only
DWG# T-2007040

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408131	C1	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TsF0e31v6I_zns11-XdvH_E7I7wZQvCC_jvUj3Shn3hwxiMN8P36WyXzNDbf



Scale = 1/16" = 1'-0"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - B	2x4	DRY	No.2	
A - C	2x4	DRY	No.2	
F - D	2x4	DRY	No.2	
ALL WEBS	2x3	DRY	No.2	
DRY: SEASONED LUMBER.				

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ
F	295	0	295	0
C	42	0	42	0
D	38	0	40	0

* SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE			
F	207	144 / 0	0 / 0	0 / 0	83 / 0	0 / 0
C	29	24 / 25	0 / 0	0 / 0	8 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO								
F-B		-259 / 0	0.0	0.0	0.03 (1)	7.81	0 / 0	0.00 (1)
A-B		0 / 35	-91.8	-91.8	0.14 (5)	10.00		
B-C		-25 / 0	-91.8	-91.8	0.13 (5)	8.25		
F-E		0 / 0	-18.5	-18.5	0.08 (4)	10.00		
E-G		0 / 0	-18.5	-18.5	0.08 (4)	10.00		
G-D		0 / 0	-18.5	-18.5	0.08 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LG1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 11 = 34 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.01")

CSI: TC=0.14/1.00 (A-B:5), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SS=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX	MIN
MT20	818	354
	1687	788
	1987	1658

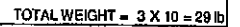
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

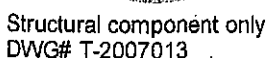
JSI GRIP = 0.18 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007012



JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

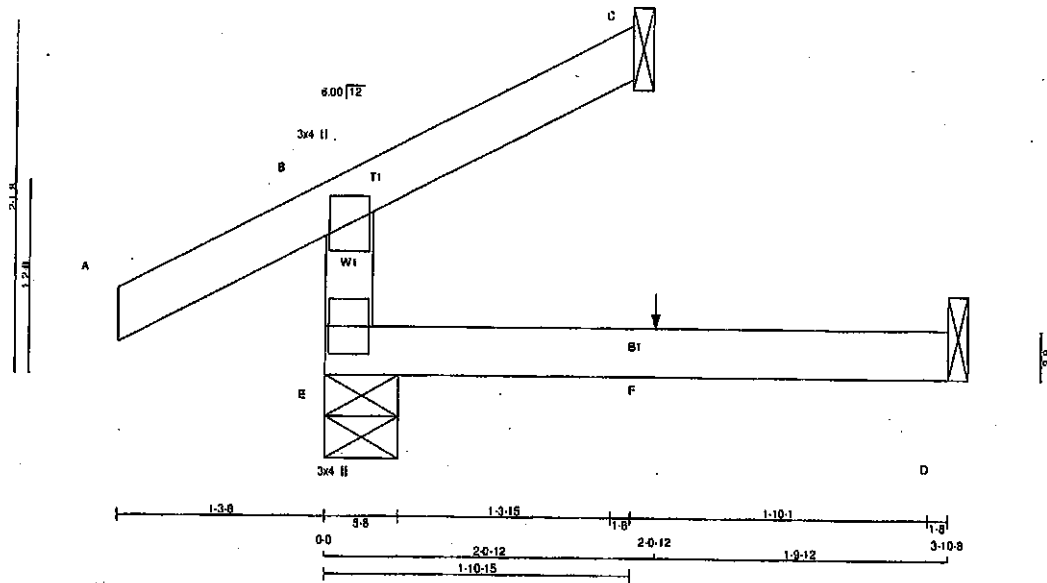


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408133	C34	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TslF0e3Tv6I_zns11-312bFwnV_PaDCXC3Dwc0IQIKoXf9OzOx86LN5zNDB

Scale = 1:12.4



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
E - B	2x4	DRY	No.2			
A - C	2x4	DRY	No.2			
E - D	2x4	DRY	No.2			

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	DOWN	IN-SX	IN-SX
E	275	0	5-8	5-8
C	68	0	1-8	1-8
D	30	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
E	194	130/0	0/0	0/0	0/0	64/0
C	46	37/0	0/0	0/0	0/0	9/0
D	24	0/0	0/0	0/0	0/0	24/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MAX. CS1 (LC) UNBRAC LENGTH FR-TO	FORCE MAX (LBS) CS1 (LC)
FR-TO		FROM TO		
E-B	-234/0	0.0 0.0 0.05 (4)	7.81	
A-B	0/28	-91.8 -91.8 0.14 (5)	10.00	
B-C	-10/0	-91.8 -91.8 0.06 (1)	10.00	
E-F	0/0	-18.5 -18.5 0.06 (4)	10.00	
F-D	0/0	-18.5 -18.5 0.06 (4)	10.00	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	1	1		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 9 = 19 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.14/1.00 (A-B.5), BC=0.06/1.00 (D-E.4), WB=0.00/1.00 (n/a.0), SS=0.10/1.00 (A-B.5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	810 354	1557 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.10 (E) (INPUT = 0.90)

JSI METAL=0.06 (B) (INPUT = 1.00)



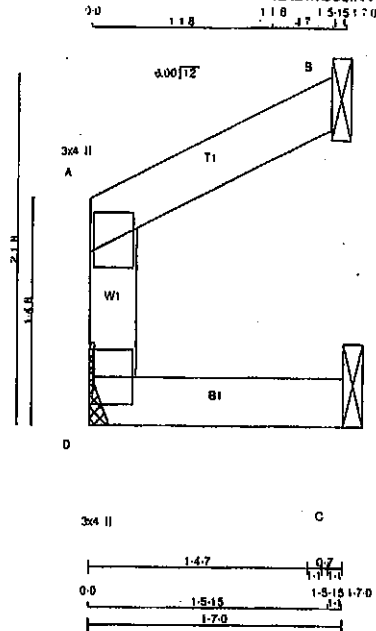
Structural component only
DWG# T-2007037

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO
408133	C35	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6I_zns11-XD6_SGo7llm4pgmFnd7FHeQhpCiburDX9orvuXzNDaz

Scale = 1:13.4



LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.	SPF
N. L. G. A. RULES						
D - A	2x4	DRY	No.2			
A - B	2x4	DRY	No.2			
D - C	2x4	DRY	No.2			

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
	DOWN	DOWN	IN-SX	IN-SX
	VERT	HORZ	UP	UP
D	83	0	0	MECHANICAL
B	85	0	0	1-8
C	18	0	0	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	59	38/0	0/0	0/0	0/0	0/0	21/0	0/0
B	45	36/0	0/0	0/0	0/0	0/0	9/0	0/0
C	14	2/0	0/0	0/0	0/0	0/0	12/0	0/0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	WEBS	MAX. FACTORED	MAX.
		FORCE	VERT.	VERT.	LC1	MAX	MAX.			FORCE	MAX
		(LBS)	(LBS)	(LBS)	(LBS)	CS1(LC)	UNBRAC			(LBS)	CS1(LC)
							LENGTH				
							FR-TO				
D-A		-72/0	0.0	0.0	0.0	0.01 (1)	7.81				
A-B		-2/0	-91.8	-91.8	0.03 (1)	10.00					
D-C		0/0	-18.5	-18.5	0.01 (4)	10.00					

TOTAL WEIGHT = 5 lb

(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/O

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD.

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.03/1.00 (A-B:1), BC=0.01/1.00 (C-D:4), WB=0.00/1.00 (n/a:0), SSI=0.05/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PS)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 788
	1987 1856	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

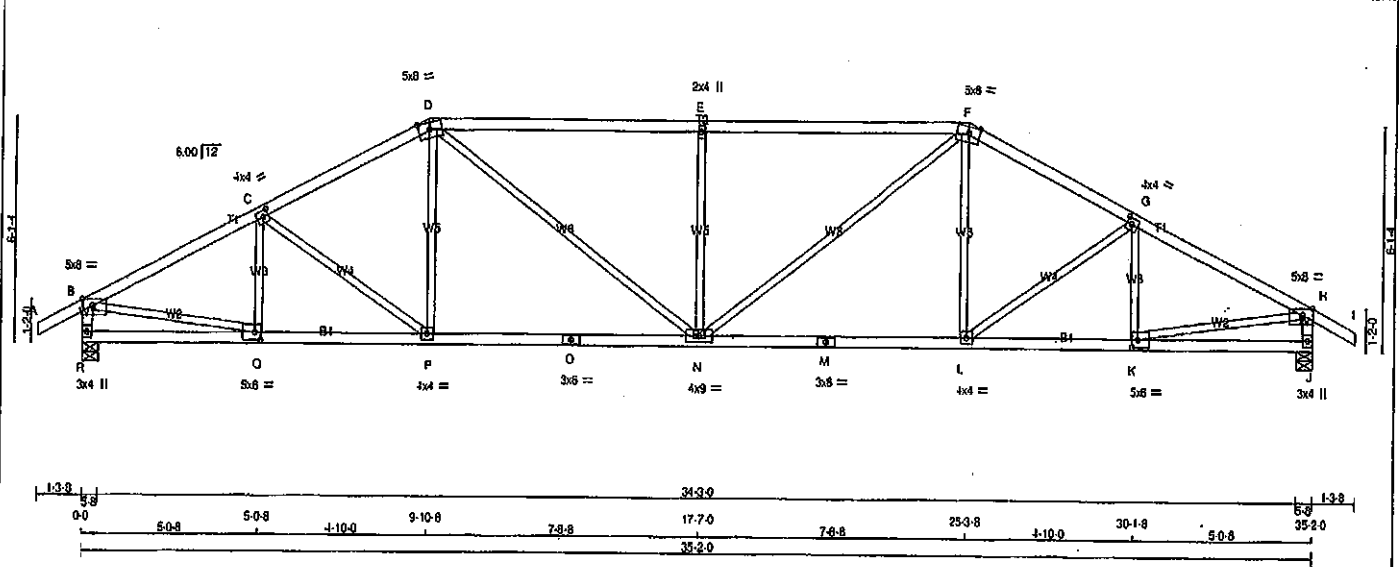
JSI GRIP= 0.03 (D) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 1.00)



Structural component only
DWG# T-2007038

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408268	T32	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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 ID:DMCubINVR6TstFos31v8l_zns11-5qB7FpkCjvDMQsWnoW9rbsdWd7WQB2ovGsM8dzMEVa
 352.0 16.5-8 13.8
 Scale = 1:57.6



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
R - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

PLATES (tablets in inches)	JT TYPE	PLATES	W	LEN	Y	X
A - B	TMWV-p	MT20	5.0	8.0	Edge	3.50
C - D	TMWV-1	MT20	4.0	4.0	2.00	1.75
D - E	TTWV-m	MT20	5.0	8.0	2.25	3.75
E - F	TMWV-w	MT20	2.0	4.0		
F - G	TTWV-m	MT20	5.0	8.0	2.25	3.75
G - H	TMWV-1	MT20	4.0	4.0	2.00	1.75
H - I	TMWV-p	MT20	5.0	8.0	Edge	3.50
J - K	BMV1-p	MT20	3.0	4.0		
K - L	BMWV-1	MT20	6.0	6.0	2.50	2.00
L - M	BMWV-1	MT20	4.0	4.0		
M - N	BS-1	MT20	3.0	6.0		
N - O	BMWVW-1	MT20	4.0	9.0		
O - P	BS-1	MT20	3.0	6.0		
P - Q	BMWV-1	MT20	4.0	4.0		
Q - R	BMWV-1	MT20	5.0	6.0	2.50	2.00
R - S	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
R	2063	0	2063	0	0	5-8	5-8		
J	2063	0	2063	0	0	5-8	5-8		
UNFACTORED REACTIONS									
	1ST CASE	MAX/MIN	COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
R	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 - 0	0 / 0		
J	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 - 0	0 / 0		
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (4)									

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	C-C	-353.0	0.08 (1)
B-C	-2834 / 0	-91.8 -91.8	0.39 (1)	3.81	O-P	-210.0	0.13 (1)
C-D	-2887 / 0	-91.8 -91.8	0.37 (1)	3.91	P-D	0 / 248	0.08 (4)
D-E	-3080 / 0	-91.8 -91.8	0.07 (1)	2.78	D-N	0 / 344	0.19 (1)
E-F	-3060 / 0	-91.8 -91.8	0.07 (1)	2.78	N-E	-872.0	0.51 (1)
F-G	-2887 / 0	-91.8 -91.8	0.37 (1)	3.91	N-F	0 / 844	0.19 (1)
G-H	-2834 / 0	-91.8 -91.8	0.39 (1)	3.81	L-F	0 / 248	0.08 (4)
H-I	0 / 28	-91.8 -91.8	0.12 (1)	10.00	L-G	-210.0	0.13 (1)
I-J	-2018 / 0	0.0 0.0	0.20 (1)	6.94	K-G	-353.0	0.08 (1)
J-H	-2018 / 0	0.0 0.0	0.20 (1)	6.94	B-Q	0 / 2591	0.58 (1)
R-Q	0 / 0	-18.5 -18.5	0.10 (4)	10.00	K-H	0 / 2591	0.58 (1)
Q-P	0 / 2555	-18.5 -18.5	0.51 (1)	10.00			
P-O	0 / 2386	-18.5 -18.5	0.49 (1)	10.00			
O-N	0 / 2386	-18.5 -18.5	0.49 (1)	10.00			
N-M	0 / 2386	-18.5 -18.5	0.49 (1)	10.00			
M-L	0 / 2386	-18.5 -18.5	0.49 (1)	10.00			
L-K	0 / 2555	-18.5 -18.5	0.51 (1)	10.00			
K-J	0 / 0	-18.5 -18.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 8.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, OBC 2012, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 088-09, CSA 088-14
 - TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
 ALLOWABLE DEFL.(TL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.34")

CSI: TC=0.97/1.00 (D-E:1), BC=0.51/1.00 (K-L:1),
 WB=0.58/1.00 (H-K:1), SS=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 788 1987 1636

PLATE PLACEMENT TOL. = 0.250 inches

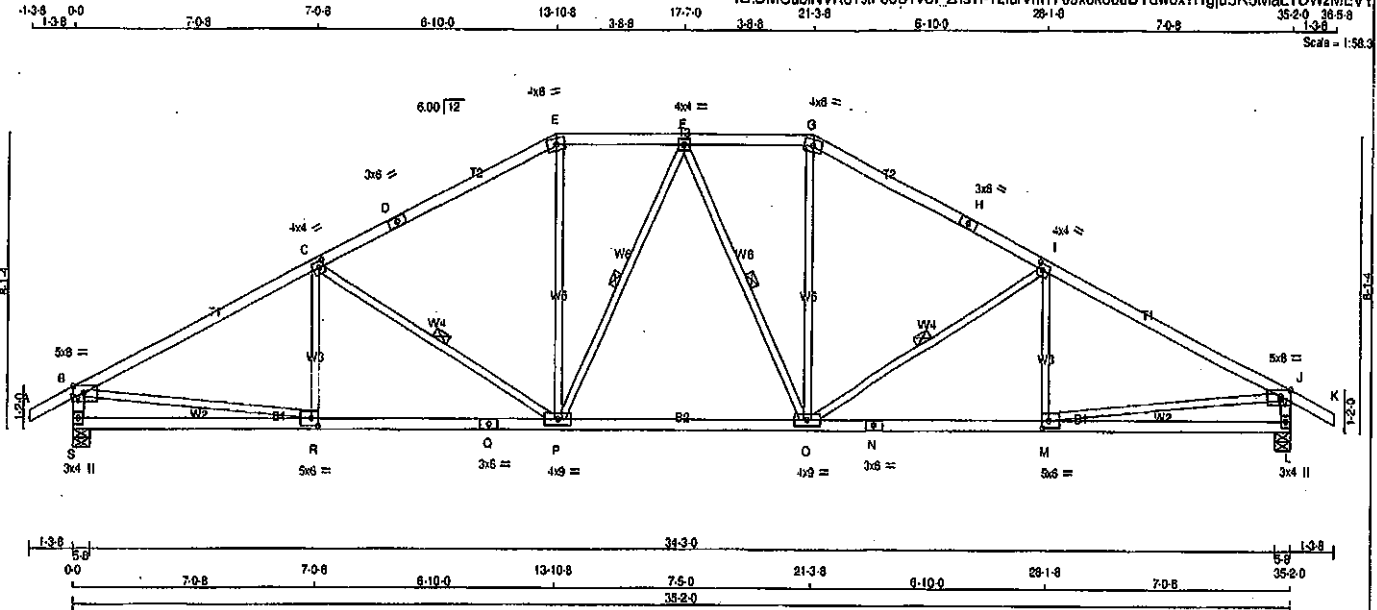
PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP=0.87 (D) (INPUT = 0.80)
 JSI METAL=0.82 (M) (INPUT = 1.00)



Structural component only
 DWG# T-2007716

JOB NAME 408268	TRUSS NAME T34	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 09:54:35 2020 Page 1	
ID:DMCubINVR6T5iF0e31v8i_zns11-12iUvMfJ9xbkouuDYdw0x?figju5K5MaLTOWzMEVY				35-20 38-58	
Scale = 1:50.8					



TOTAL WEIGHT = 2 X 145 = 291 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
O - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-p	MT20	5.0	8.0 Edge 3.50
C	TMVW-l	MT20	4.0	4.0 2.00 1.75
D	TS-l	MT20	3.0	6.0
E	TTW-m	MT20	4.0	6.0
F	TMVW-l	MT20	4.0	4.0
G	TTW-m	MT20	4.0	6.0
H	TS-l	MT20	3.0	8.0
I	TMVW-l	MT20	4.0	4.0 2.00 1.75
J	TMVW-p	MT20	5.0	8.0 Edge 3.50
L	BMV-l+p	MT20	3.0	4.0
M	BMVW-l	MT20	5.0	8.0 2.50 2.25
N	BS-l	MT20	3.0	8.0
O	BMVW-l	MT20	4.0	9.0
P	BMVW-l	MT20	4.0	9.0
Q	BS-l	MT20	3.0	6.0
R	BMVW-l	MT20	5.0	8.0 2.50 2.25
S	BMV-l+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
S	2083	0	2083	0	5-8
L	2083	0	2083	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1457	989 / 0	0 / 0	0 / 0	0 / 0	488 0	0 / 0
L	1457	989 / 0	0 / 0	0 / 0	0 / 0	488 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-P, F-O, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	FORCE (LBS)
FR-TO				FR-TO	
A-B	0 / 28	-91.8	-91.8 0.12 (1)	R-C	-182 52
B-C	-2908 / 0	-91.8	-91.8 0.83 (1)	C-P	-637 0
C-D	-2373 / 0	-91.8	-91.8 0.72 (1)	P-E	0 874
D-E	-2373 / 0	-91.8	-91.8 0.72 (1)	F-O	-222 0
E-F	-2100 / 0	-91.8	-91.8 0.19 (1)	O-G	0 874
F-G	-2100 / 0	-91.8	-91.8 0.19 (1)	G-H	-637 0
G-H	-2373 / 0	-91.8	-91.8 0.72 (1)	H-I	-182 52
H-I	-2373 / 0	-91.8	-91.8 0.72 (1)	I-J	0 2649
I-J	-2808 / 0	-91.8	-91.8 0.83 (1)	J-K	0 2649
J-K	0 / 28	-91.8	-91.8 0.12 (1)	K-L	0 2649
S-B	-2008 / 0	0.0	0.0 0.20 (1)	L-M	0 2649
L-J	-2008 / 0	0.0	0.0 0.20 (1)		
S-R	0 / 0	-18.5	-18.5 0.20 (4)		
R-Q	0 2630	-18.5	-18.5 0.52 (1)		
Q-P	0 2630	-18.5	-18.5 0.52 (1)		
P-O	0 2193	-18.5	-18.5 0.45 (1)		
O-N	0 2630	-18.5	-18.5 0.52 (1)		
N-M	0 2630	-18.5	-18.5 0.52 (1)		
M-L	0 / 0	-18.5	-18.5 0.20 (4)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.83/1.00 (J-M:1), BC=0.52/1.00 (M-O:1), WB=0.60/1.00 (J-M:1), SS=0.28/1.00 (B-C:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1656

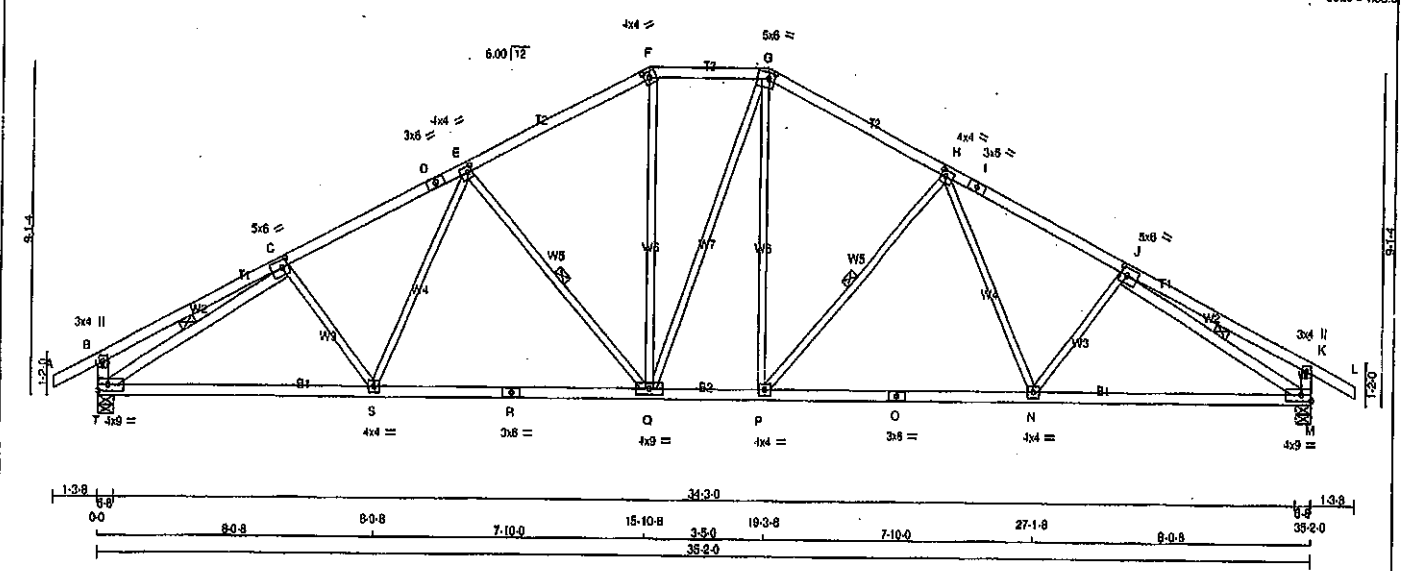
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (M) (INPUT = 0.80)
JSI METAL = 0.79 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007718



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV-p	MT20	3.0	4.0		
C TMWW-t	MT20	5.0	8.0	2.50	2.25
D TS-1	MT20	3.0	8.0		
E TMWW-t	MT20	4.0	4.0	2.00	1.50
F TTW-h	MT20	4.0	4.0	2.00	1.75
G TTWW-m	MT20	5.0	8.0	2.00	2.00
H TMWW-t	MT20	4.0	4.0	2.00	1.50
I TS-1	MT20	3.0	8.0		
J TMWW-t	MT20	5.0	8.0	2.50	2.25
K TMV-p	MT20	3.0	4.0		
M BMWW-t	MT20	4.0	9.0		Edge
N, P, S					
O BMWW-t	MT20	4.0	4.0		
Q SS-1	MT20	3.0	8.0		
R BMWW-t	MT20	4.0	9.0		
Q SS-1	MT20	3.0	8.0		
T BMWW-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	2063	0	0
T	2063	0	0
M	2063	0	0

UNFACTORED REACTIONS

1ST CASE	MAX	MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE	PERM.LIVE
T	1457	969	0
M	1457	969	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, H-P, C-T, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO	CSI (LC)
FR-TO				FR-TO			
A-B	0/28	-91.8	-91.8 0.12 (1)	C-S	-110/37	10.00	0.04 (1)
B-C	0/20	-91.8	-91.8 0.32 (1)	S-E	0/277	10.00	0.06 (1)
C-D	-2760/0	-91.8	-91.8 0.40 (1)	E-Q	-881/0	3.88	0.31 (1)
D-E	-2760/0	-91.8	-91.8 0.40 (1)	Q-F	0/813	3.88	0.14 (1)
E-F	-2167/0	-91.8	-91.8 0.36 (1)	F-G	0/4	4.30	0.00 (1)
F-G	-1925/0	-91.8	-91.8 0.18 (1)	G-H	0/809	4.71	0.14 (1)
G-H	-2185/0	-91.8	-91.8 0.36 (1)	H-I	-883/0	4.30	0.31 (1)
H-I	-2761/0	-91.8	-91.8 0.40 (1)	I-J	0/279	3.88	0.06 (1)
I-J	-2761/0	-91.8	-91.8 0.40 (1)	J-K	-110/37	10.00	0.04 (1)
J-K	0/20	-91.8	-91.8 0.32 (1)	K-L	-3042/0	10.00	0.81 (1)
K-L	0/28	-91.8	-91.8 0.12 (1)	L-M	-3043/0	10.00	0.81 (1)
T-B	-325/0	0.0	0.0 0.03 (1)				
M-K	-325/0	0.0	0.0 0.03 (1)				
T-S	0/2538	-18.5	-18.5 0.58 (1)				
S-R	0/2362	-18.5	-18.5 0.53 (1)				
R-Q	0/2362	-18.5	-18.5 0.53 (1)				
Q-P	0/1924	-18.5	-18.5 0.40 (1)				
P-O	0/2362	-18.5	-18.5 0.53 (1)				
O-N	0/2362	-18.5	-18.5 0.53 (1)				
N-M	0/2538	-18.5	-18.5 0.58 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2010
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")

CALCULATED VERT. DEFL.(LL) = L/989 (0.14")

ALLOWABLE DEFL.(TL) = L/360 (1.17")

CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CSI: TC=0.40/1.00 (H-J:1), BC=0.58/1.00 (M-N:1), WB=0.81/1.00 (J-M:1), SS=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 818 354 1887 788 1987 1650

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)

JSI METAL= 0.83 (C) (INPUT = 1.00)



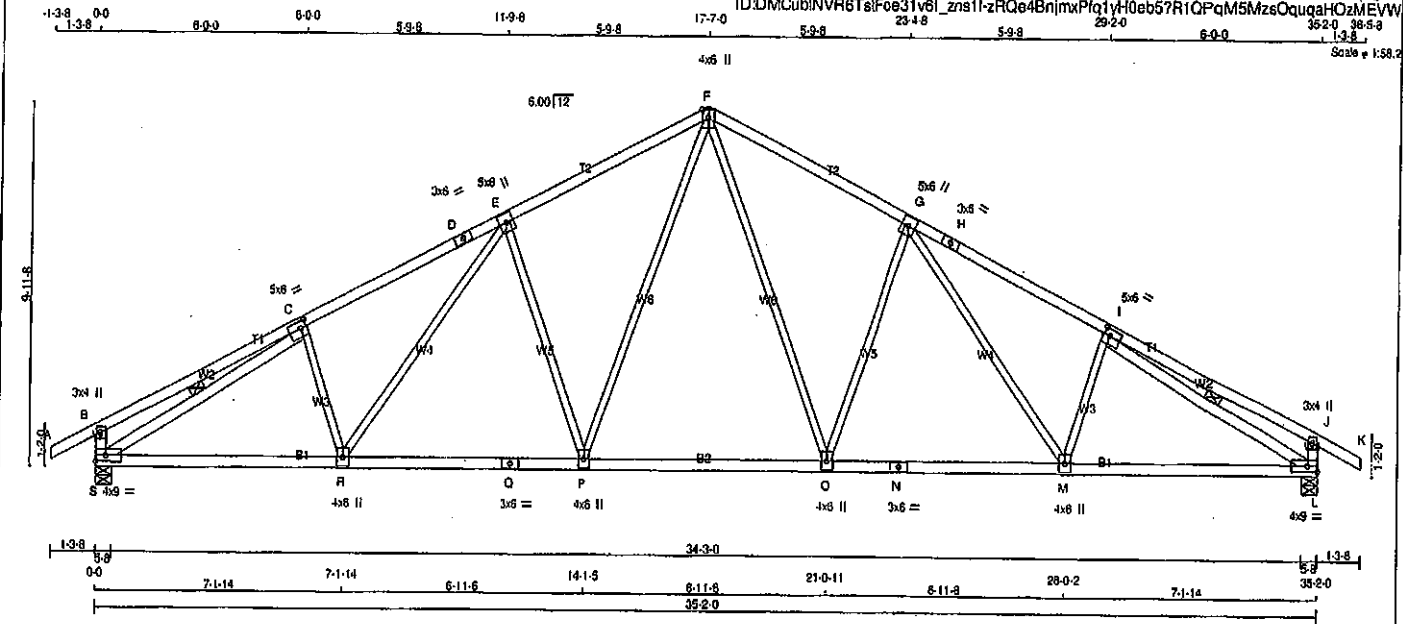
Structural component only
DWG# T-2007719

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408268	T36	5	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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ID:DMCubINVR6TsFoe31v6I_zns1I-zRQe4BnjmxPq1yH0eb57R1QPqM5MzOqugaH0zMEVW

Scale = 1:50.2



TOTAL WEIGHT = 5 X 152 = 758 (LBS)

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
B - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - O	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
S - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMWW-t	MT20	5.0	8.0 2.25 2.00
D	TS-t	MT20	3.0	8.0
E	TMWW-t	MT20	5.0	8.0
F	TTWW+p	MT20	4.0	6.0 Edge
G	TMWW-t	MT20	5.0	8.0
H	TS-t	MT20	3.0	8.0
I	TMWW-t	MT20	5.0	8.0 2.25 2.00
J	TMV+p	MT20	3.0	4.0
L	BMVW-t	MT20	4.0	9.0 Edge
M, O, P, R				
M	BMVW-t	MT20	4.0	6.0
N	BS-t	MT20	3.0	6.0
Q	BS-t	MT20	3.0	6.0
S	BMVW-t	MT20	4.0	9.0 Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		RECORD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
S	2063	0	2063	0	0	5.8	5.8		
L	2063	0	2063	0	0	5.8	5.8		

UNFACTORED REACTIONS

1ST LOASE		MAX. MIN. COMPONENT REACTIONS		1ST LOASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1457	999 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
L	1457	999 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-S, L-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	CS (LC)	MEMB.	FORCE (LBS)	VERT. LOAD	CS (LC)
FR-TO				FR-TO			
A-B	0.28	-91.8	-91.8 0.12 (1)	F-O	0.866	0.19 (1)	
B-C	0.22	-91.8	-91.8 0.40 (1)	O-G	-728.0	0.72 (1)	
C-D	-2808.0	-91.8	-91.8 0.49 (1)	G-M	0.391	0.09 (1)	
D-E	-2808.0	-91.8	-91.8 0.49 (1)	M-I	-192.18	0.05 (1)	
E-F	-2322.0	-91.8	-91.8 0.46 (1)	P-F	0.866	0.19 (1)	
F-G	-2322.0	-91.8	-91.8 0.46 (1)	E-P	-728.0	0.72 (1)	
G-H	-2808.0	-91.8	-91.8 0.49 (1)	R-E	0.391	0.09 (1)	
H-I	-2808.0	-91.8	-91.8 0.49 (1)	C-R	-192.18	0.05 (1)	
I-J	0.22	-91.8	-91.8 0.40 (1)	S-C	-3064.0	0.72 (1)	
J-K	0.28	-91.8	-91.8 0.12 (1)	I-L	-3064.0	0.72 (1)	
S-B	-344.0	0.0	0.0 0.03 (1)				
L-J	-344.0	0.0	0.0 0.03 (1)				
S-R	0.2574	-18.5	-18.5 0.53 (1)				
R-Q	0.2291	-18.5	-18.5 0.47 (1)				
Q-P	0.2291	-18.5	-18.5 0.47 (1)				
P-O	0.1762	-18.5	-18.5 0.38 (1)				
O-N	0.2291	-18.5	-18.5 0.47 (1)				
N-M	0.2291	-18.5	-18.5 0.47 (1)				
M-L	0.2574	-18.5	-18.5 0.53 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CS: TC=0.49/1.00 (C-E:1), BC=0.53/1.00 (L-M:1), WB=0.72/1.00 (E-P:1), SS=0.22/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COM P=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (C) (INPUT = 0.90)
JSI METAL = 0.77 (I) (INPUT = 1.00)



Structural component only
DWG# T-2007720

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 268–269.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

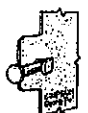
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

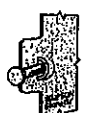
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options Information on p. 126.



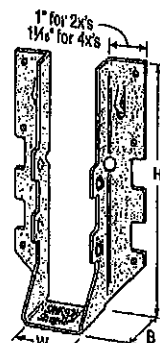
Double-Shear
Nailing
Top View



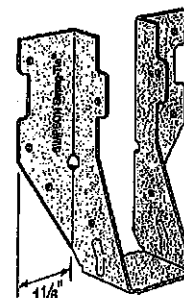
Double-Shear
Nailing
Side View;
Do not
bend tab



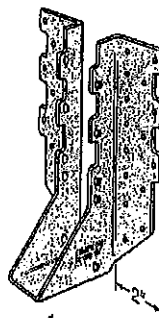
Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580



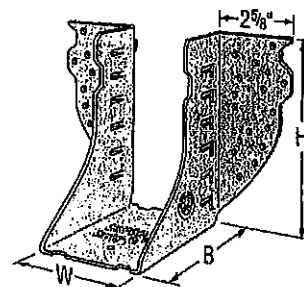
✓ LUS28



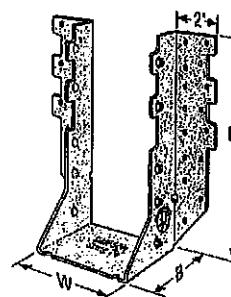
LU26L



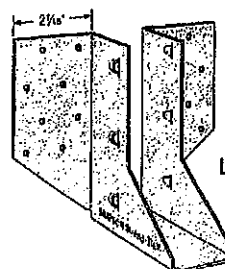
✓ HUS210
(HUS26, HUS28,
and HHUS similar)



✓ HGUS28-2

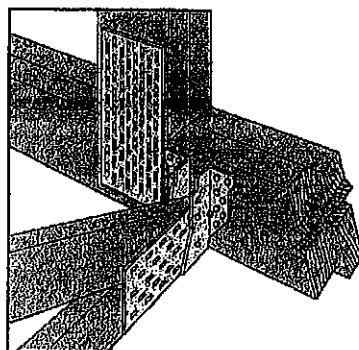


✓ HHUS210-2



LJS26DS

**Typical HUS26
Installation
with Reduced
Heel Height**
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LUL/LUS/LJS/HUS/HHUS/HGUS**HHUS/HGUS**

See Hanger Options Information on pp. 125-127.

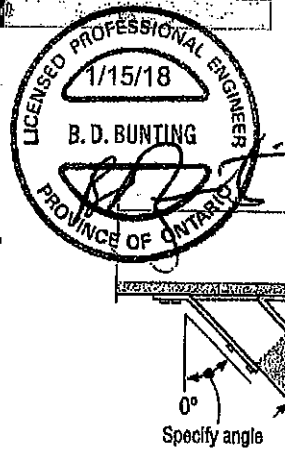
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



**Top View HHUS Hanger
Skewed Right**
(joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _a ³	Header	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
								lb.	kN	lb.	kN
Single 2x Sizes											
LUS24	18	1 1/8	3 1/4	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1626	645	1155
								316	7.23	2.87	5.14
								360	1020	320	725
LU24L	22	1 1/8	3	1 1/4	2 1/8	(4) 10d	(2) 10d x 1 1/2"	1.60	4.54	1.42	3.22
								720	1805	645	1140
								3.20	7.14	2.87	5.07
LUS26	18	1 1/8	4 1/4	1 1/4	3 1/4	(4) 10d	(4) 10d	1420	2170	1290	1630
								6.32	9.65	5.74	7.25
								2705	4940	2065	3875
HUS26	18	1 1/8	5 1/4	3	3 1/8	(14) 16d	(6) 16d	11.80	21.97	9.20	17.24
								2055	4265	1460	4115
								9.14	18.97	6.49	18.31
LJS26DS	18	1 1/8	5	3 1/4	4 1/4	(16) 16d	(6) 16d	2685	6625	2685	5700
								11.86	29.51	11.96	25.35
								1140	2185	1020	1550
HGUS28	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"	5.07	9.72	4.54	6.89
								1420	2520	1290	1790
								6.32	11.21	5.74	7.96
LUS28	18	1 1/8	6 1/4	1 1/4	3 1/4	(8) 10d	(4) 10d	3605	5365	2675	4345
								16.04	23.88	11.90	19.33
								3310	7675	3310	6900
HUS28	18	1 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	14.74	34.19	14.74	30.73
								1140	2495	1020	1770
								5.07	11.10	4.54	7.87
HGUS28	12	1 1/8	7 1/4	5	6 1/4	(36) 16d	(12) 16d	1420	2785	1290	2210
								6.32	12.39	5.74	9.83

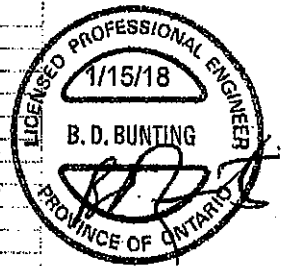
1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_a is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSON and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27-28 for other nail sizes and information.

Face-Mount Hangers

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance					
		W	H	B	d _o ³	Header	Joist	D-Fir-L		S-P-F			
								Uplift	Normal	Uplift	Normal		
								(K _D = 1.15) lb. kN	(K _D = 1.00) lb. kN	(K _D = 1.15) lb. kN	(K _D = 1.00) lb. kN		
Double 2x Sizes													
LUS24-2	18	3 1/8	3 1/8	2	1 1/8	(4) 16d	(2) 16d	835	2020	590	1435		
								3.71	8.99	2.62	6.38		
LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920		
								7.65	11.54	6.87	8.54		
HHUS26-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2650	7335	2085	5205		
								12.68	32.83	9.20	23.15		
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355		
								19.51	39.81	13.83	28.27		
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575		
								7.65	14.79	6.87	11.45		
HHUS28-2	14	3 1/8	7 1/8	3	8 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345		
								16.75	39.77	11.90	28.22		
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215		
								27.00	57.74	19.17	40.99		
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(8) 16d	2580	4500	2320	3195		
								11.48	20.02	10.32	14.21		
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000		
								20.77	42.97	18.84	31.14		
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270		
								30.43	62.34	21.60	45.69		
Triple 2x Sizes													
HGUS26-3	12	4 1/8	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355		
								19.51	39.81	13.83	28.27		
HGUS28-3	12	4 1/8	7 1/4	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215		
								27.00	57.74	19.17	40.99		
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d	4670	9670	4235	6965		
								20.77	43.02	18.84	30.54		
HGUS210-3	12	4 1/8	9 1/4	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400		
								30.43	65.14	21.60	46.26		
Quadruple 2x Sizes													
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355		
								19.51	39.81	13.83	28.27		
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215		
								27.00	57.74	19.17	40.99		
HHUS210-4	14	8 1/8	8 1/8	3	7 1/8	(30) 16d	(10) 16d	4670	10155	4235	7210		
								20.77	45.17	18.84	32.07		
HGUS210-4	12	6 1/8	8 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400		
								30.43	65.14	21.60	46.26		
HGUS212-4	12	8 1/8	10 1/8	4	10 1/8	(50) 16d	(20) 16d	7840	14995	5425	10645		
								33.98	66.70	24.13	47.35		
HGUS214-4	12	8 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645		
								45.06	72.95	32.00	51.80		
4x Sizes													
LUS46	18	3 1/8	4 1/8	2	3 1/8	(4) 16d	(4) 16d	1720	2595	1545	1920		
								7.65	11.54	6.87	8.54		
HHUS46	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2640	7335	2085	5205		
								11.30	32.83	9.20	23.15		
HGUS46	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355		
								19.51	39.81	13.83	28.27		
LUS48	18	3 1/8	6 1/8	2	3 1/8	(6) 16d	(4) 16d	1720	3325	1545	2575		
								7.65	14.79	6.87	11.45		
HHUS48	14	3 1/8	7 1/8	3	8 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345		
								16.75	39.77	11.90	28.22		
HGUS48	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215		
								27.00	57.74	19.17	40.99		
LUS410	18	3 1/8	8 1/8	2	5 1/8	(8) 16d	(6) 16d	2580	4500	2320	3195		
								11.48	20.02	10.32	14.21		
HGUS410	12	3 1/8	9	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270		
								30.43	62.34	21.60	45.69		
HGUS412	12	3 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645		
								33.98	66.70	24.13	47.35		
HGUS414	12	3 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645		
								45.06	72.95	32.00	51.80		



Plated Truss Connectors

See footnotes on p. 258.

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1/4". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

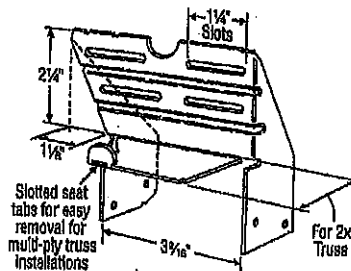
Design: Factored resistances are in accordance with CSA 086-14

Installation:

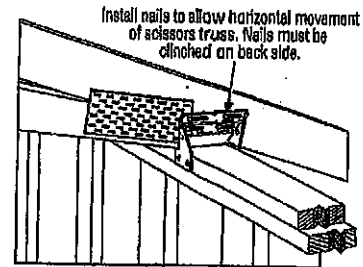
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

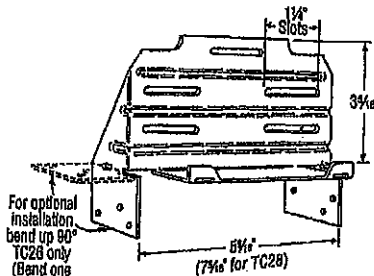
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



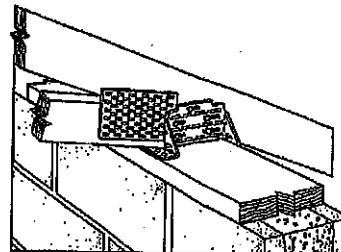
TC24
U.S. Patent 4,932,173



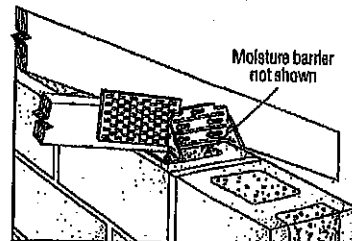
Typical TC24 Installation



TC28
(TC28 Similar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K ₀ =1.18)	Uplift (K ₀ =1.18)
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K ₀ =1.18)	Uplift (K ₀ =1.18)
TC26	(5) 10d	(6) 10d x 1 1/2"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) - 3/8" x 2 1/4" Titen screws has a factored uplift resistance of 275 lb.



U.S. PATENT
DESIGN

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15826101-1 (Rev. 2014)

(800) 999-5099
strongtie.com

H/TSP

Seismic and Hurricane Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Fasteners			Factored Resistance ($K_0 = 1.15$)					
					D.Fir-L			S-P-F		
		To Rafters/Truss	To Plates	To Studs	Uplift	Lateral		Uplift	Lateral	
						F ₁	F ₂		F ₁	F ₂
					lb.	lb.	lb.	lb.	lb.	lb.
					kN	kN	kN	kN	kN	kN
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
					3.29	3.05	1.33	3.02	2.16	0.96
SS H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	165	55
					3.69	0.98	0.33	2.62	0.69	0.24
SS H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
					3.58	0.71	0.71	3.36	0.71	0.71
H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
					3.71	0.78	0.93	3.29	0.71	0.93
SS H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
					3.29	0.80	1.18	2.74	0.56	0.85
H6	16	—	(8) 8d	(8) 8d	1585	1085	—	1125	770	—
					7.05	4.83	—	5.00	3.43	—
H7Z	16	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
					6.18	2.98	—	4.40	2.11	—
SS H8 ^a	18	(5) 10d x 1½"	(5) 10d x 1½"	—	1120	—	—	1025	—	—
					4.98	—	—	4.56	—	—
SS H10A ^a	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290
					7.72	3.54	1.82	6.69	2.51	1.29
H10AR	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1485	690	430	1220	570	305
					6.61	3.07	1.91	5.43	2.54	1.36
H10A-2	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1835	1275	430	1645	880	305
					8.16	5.67	1.91	7.32	3.91	1.36
H10S ^{a,b}	18	(8) 8d x 1½"	(8) 8d x 1½"	(8) 8d	1465	795	315	1040	565	225
					6.52	3.54	1.40	4.63	2.51	1.00
H11Z	18	(8) 16d x 2½"	(6) 16d x 2½"	—	1095	920	545	780	655	390
					4.87	4.09	2.42	3.47	2.91	1.73
H14	18	[1] (12) 8d x 1½"	(13) 8d	—	2390	855	320	1805	610	230
		[2] (12) 8d x 1½"	(15) 8d	—	10.63	3.80	1.42	8.03	2.71	1.02
					2390	855	320	1805	610	230
TSP	18	(9) 10d x 1½"	(6) 10d x 1½"	—	1295	440	—	920	310	—
					5.76	1.96	—	4.09	1.38	—
		(9) 10d x 1½"	(6) 10d	—	1560	440	—	1105	310	—
					6.94	1.96	—	4.92	1.38	—

- Factored resistances have been increased 15% for short term loading; no further increase is allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on the same side of the plate (exception: H2.6A).
- H8 factored uplift resistances for stud-to-bottom plate installations are 595 lb. (2.65 kN) for D.Fir-L and 390 lb. (1.74 kN) for S-P-F.
- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-HTIECONPATH).
- Factored resistances in the F₁ direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.

- H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.38 kN) D.Fir-L and 1015 lb. (4.51 kN) S-P-F.
- H10S nails to plates are optional for uplift but required for lateral loads.
- H10A may be field-bent up to a slope of 6/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
- The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SSNAILS at strongtie.com.
- D.Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (6) 8d x 1½" nails into the top plates and (3) 8d x 1½" nails into the lowest three flange holes into the truss bottom chord is 495 lb. (2.20 kN).
- Nails: 16d x 2½" = 0.162" dia. x 2½" long, 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long, 8d x 1½" = 0.131" dia. x 1½" long. See pp. 27–28 for other nail sizes and information.

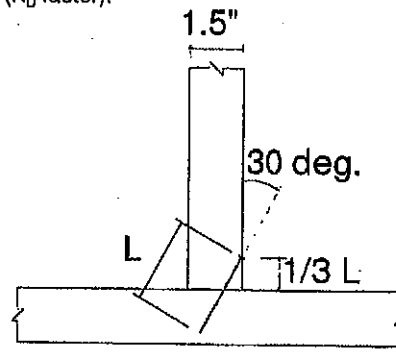
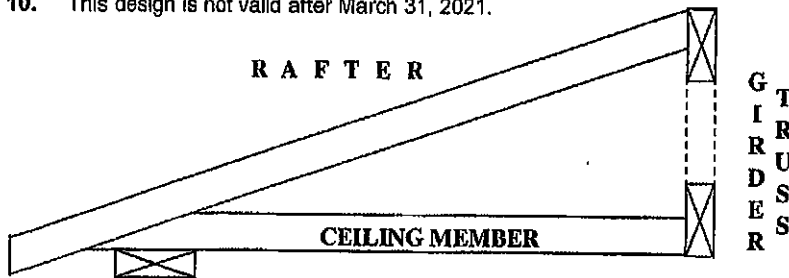
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

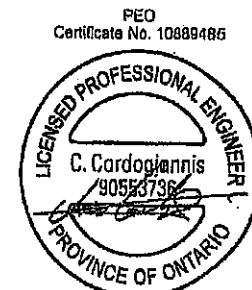
1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.



Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

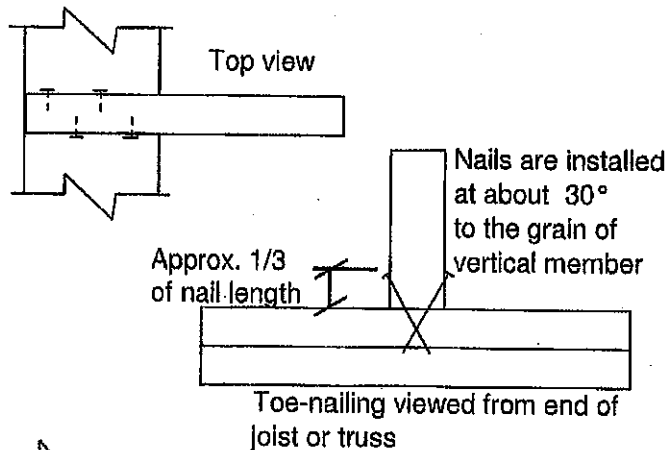
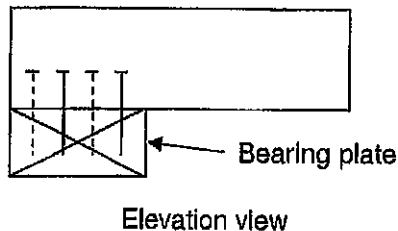
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

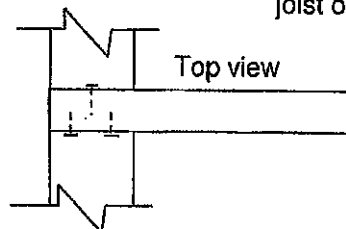
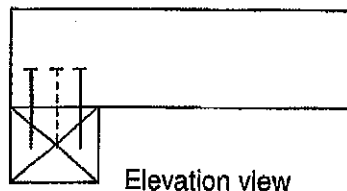
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to wind or earthquake load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate

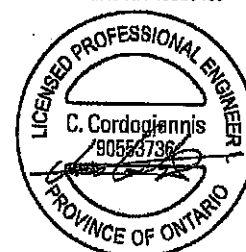


Toe-nailing on 2x4 Bearing Plate



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Bradford, Ontario L3Z 3G7

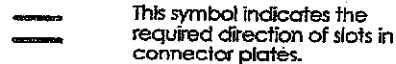
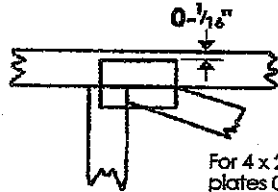
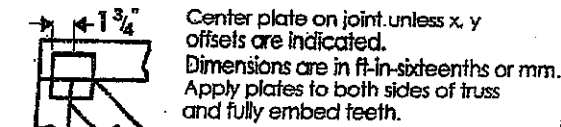
PEO
Certificate No. 10889485



December 2, 2019

Symbols

PLATE LOCATION AND ORIENTATION



*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

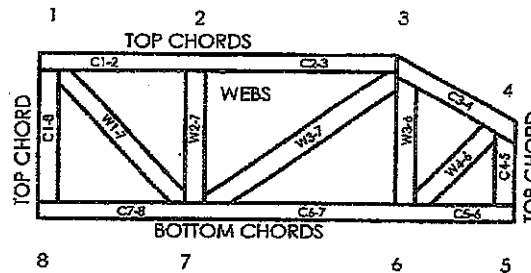


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L 10319-L 13270-L 12691-R

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MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing of 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

TECH-NOTES

ONTARIO WOOD TRUSS
FABRICATORS ASSOCIATION

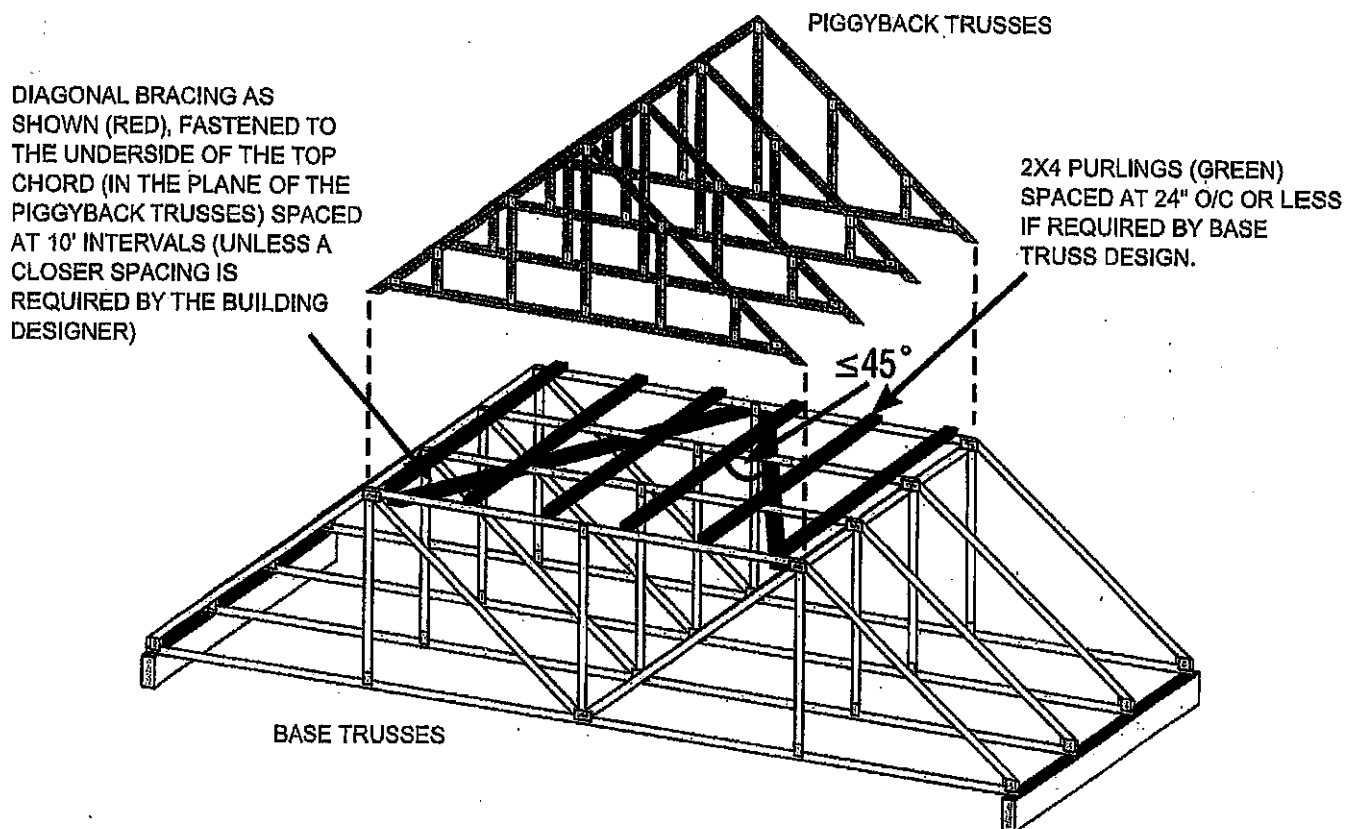
TN 15-001 Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

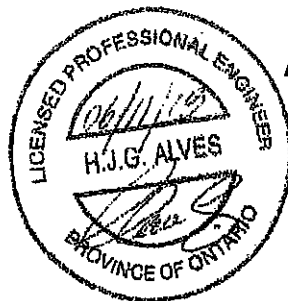


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.



Alves Engineering Services Inc.

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(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800213

Feb 09, 2018