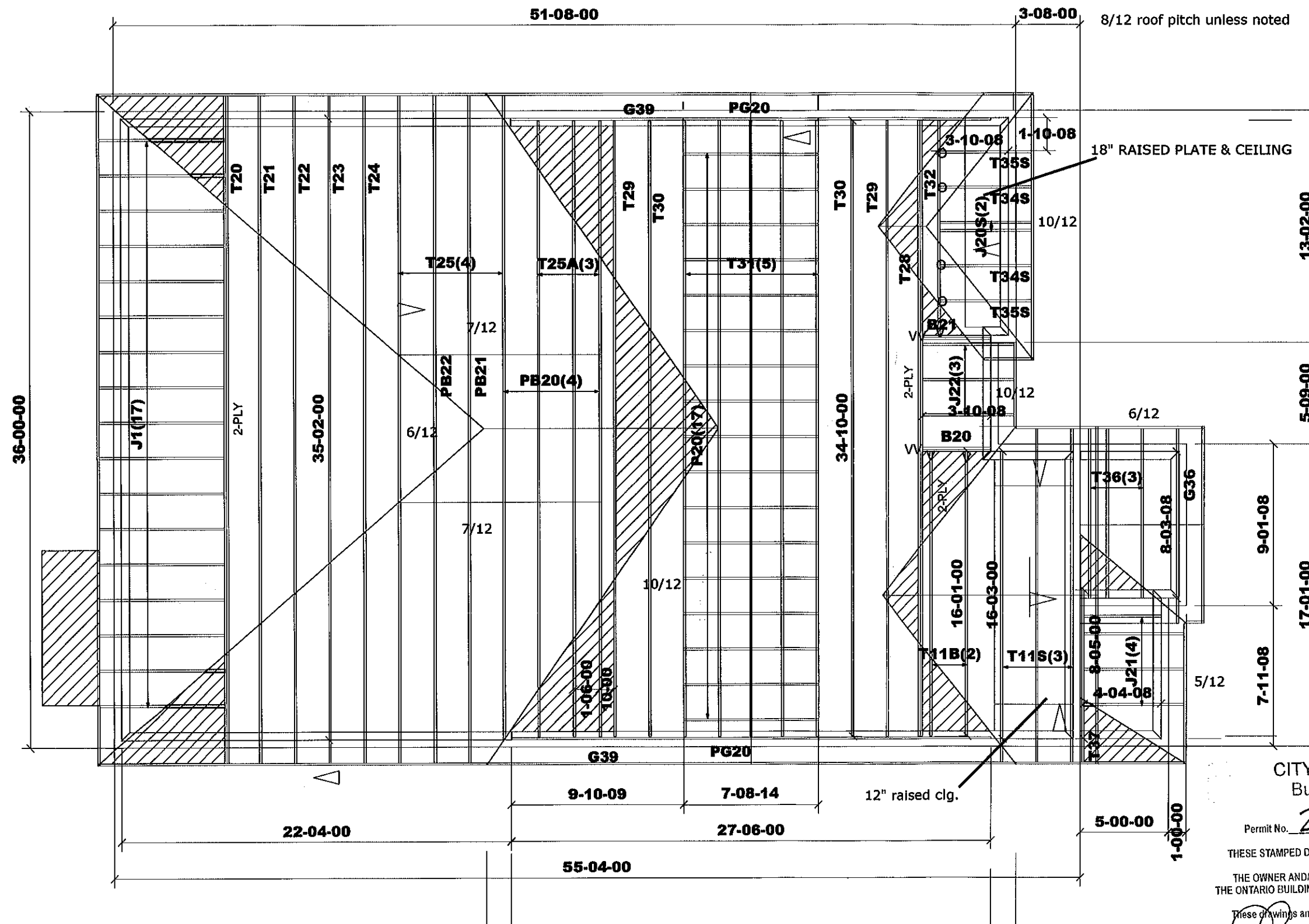




ASPHALT SHINGLES
 FINISHED OVERHANG: 12"
 2x6 EXTERIOR WALLS
 2x6 FASCIA BOARD
 HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
 Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
 TCSL = 25.6 psf
 TCDL = 6.0 psf
 BCLL = 0.0 psf
 BCDL = 7.4 psf

HARDWARE:
 LUS24 - (O)
 LJS26DS - (V)
 HGUS26-2 - (XX)
 LUS26-2-(VV)

BEAMS:
 B20= B21 =
 2 - 2x10 SPF #2

NOTES:
 CITY OF HAMILTON CONVENTIONAL Building Division FRAMING

Permit No. **21-105895**

THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE
 THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW

These drawings and/or specifications have been reviewed by
 [Signature] **FEB. 23/21**
 FOR CHIEF BUILDING OFFICIAL DATE



Job Track: **51225**
 Plan Log: **202419**
 Layout ID: **408223**

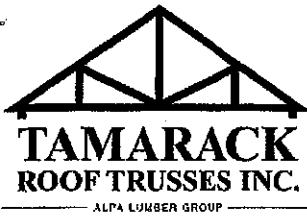
Builder / Location: **GREEN PARK HOMES / WATERDOWN**
 Project: **RUSSELL GARDENS PH.3**
 Date: 2020-12-08 Sales: Mario DiCano Designer: JG

Model / Elevation: **MOUNTAINASH 6 182/2-STD OR OPT.5 BR**

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Milek ver 8.3.3.247

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 6
 Lot #: 182
 Elevation: 2-STD OR OPT.5 BR

Job Track: 51225
 PlanLog: 202419
 Layout ID: 408223
 Ref #
 Page: 1 of 3
 Date: 04-28-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T11B Common	8/12	16-01-00	6-09-13	2 x 4		1-06-02 1-04-13	137.47 88.33		
	3	T11S Roof Special	8/12 8/12	16-03-00	6-09-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	201.4 133.00		
	1 2-ply	T20 Hip Girder	7/12	35-02-00	4-01-04	2 x 6	1-03-08 1-03-08	1-03-07 1-03-07	359.46 218.00		
	1	T21 Hip	7/12	35-02-00	5-01-04	2 x 4	1-03-08 1-03-08	1-03-07 1-03-07	142.49 89.50		
	1	T22 Hip	7/12	35-02-00	6-01-04	2 x 4	1-03-08 1-03-08	1-03-07 1-03-07	146.47 93.50		
	1	T23 Hip	7/12	35-02-00	7-01-04	2 x 4	1-03-08 1-03-08	1-03-07 1-03-07	144.95 91.17		
	1	T24 Hip	7/12	35-02-00	8-01-04	2 x 4	1-03-08 1-03-08	1-03-07 1-03-07	151.12 95.67		
	4	T25 Hip	7/12	35-02-00	9-01-04	2 x 4	1-03-08 1-03-08	1-03-07 1-03-07	631.9 399.33		
	3	T25A Piggyback Base	7/12	34-10-00	9-01-04	2 x 4		1-04-09 1-04-09	461.61 286.50		
	1 2-ply	T28 Flat Girder	0/12	34-10-00	4-10-07	2 x 6		4-10-07 4-10-07	366.38 214.67		
	2	T29 Flat	0/12	34-10-00	6-06-07	2 x 4		6-06-07 6-06-07	313.97 193.33		
	2	T30 Flat	0/12	34-10-00	8-02-07	2 x 4		8-02-07 8-02-07	358.85 220.00		
	5	T31 Flat	0/12	34-10-00	9-10-07	2 x 4		9-10-07 9-10-07	980.54 600.00		
	1	T32 Hip Girder	8/12	12-02-00	6-04-07	2 x 4 2 x 6		3-00-02 2-10-13	66.45 43.67		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 6
 Lot #: 182
 Elevation: 2-STD OR OPT.5 BR

Job Track: 51225
 PlanLog: 202419
 Layout ID: 408223
 Ref #:
 Page: 2 of 3
 Date: 04-28-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T34S Half Hip	10 /12	3-10-08	5-05-13	2 x 4	1-03-08	1-07-11 5-05-13	63.75 44.33		
	2	T35S Half Hip	10 /12	3-10-08	4-01-13	2 x 4	1-03-08	1-07-11 4-01-13	57.2 41.00		
	3	T36 Common	6 /12	8-03-08	3-00-09	2 x 4	1-03-08 1-03-08	5-07 5-07	79.61 53.00		
	1	G36 GABLE	6 /12	8-03-08	3-01-05	2 x 4	1-03-08 1-05-00	5-07 5-07	26.93 17.33		
	1	T37 Half Hip Girder	8 /12	8-05-00	2-08-03	2 x 4 2 x 6	1-03-08	8-05 2-01-15	36.62 26.00		
	2	G39 GABLE	10 /12	27-06-00	9-10-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	303.3 193.33		
	17	P20 Common	10 /12	7-08-14	3-02-12	2 x 4		0-01 0-01	363.26 229.50		
	4	PB20 Piggyback	7 /12	8-04-05	2-05-04	2 x 4			80.17 49.33		
	1	PB22 Piggyback	7 /12	8-04-05	1-00-00	2 x 4			18.26 11.67		
	1	PB21 Piggyback	7 /12	8-04-05	2-00-00	2 x 4			21.88 14.33		
	2	PG20 GABLE	10 /12	7-09-00	3-02-12	2 x 4			44.53 30.33		
	17	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	285.51 181.33		
	2	J20S Jack-Open	10 /12	3-10-08	6-04-07	2 x 4	1-03-08	1-07-11 6-04-07	65.13 43.33		
	4	J21 Jack-Open	5 /12	4-04-08	2-08-02	2 x 4	1-03-08	4-01 2-01-15	48.51 32.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 6
 Lot #: 182
 Elevation: 2-STD OR OPT.5 BR

Job Track: 51225
 PlanLog: 202419
 Layout ID: 408223
 Ref #
 Page: 3 of 3
 Date: 04-28-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	3	J22 Jack-Open	10 / 12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	45.88 29.00		

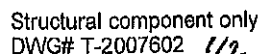
TOTAL # TRUSS= 92 TOTAL BFT OF ALL TRUSSES= 3762.48 BFT. TOTAL WEIGHT OF ALL TRSSES 6003.41 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LJS26DS	
4	Hardware	LUS24	
2	Hardware	LUS26-2	

TOTAL NUMBER OF
ITEMS= 9

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	T1	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR6TsIFoe31v6I zns11-Jm9J91AhZl22a2EEemNZsEz3HThi7mEdCAUuvcozMFH

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	7.0	8.0	Edge	2.50
D, F, I						
D	TMWV-i	MT20	4.0	4.0		
E	TMW-w	MT20	2.0	4.0		
G	TS-i	MT20	3.0	6.0		
H	TMW-w	MT20	2.0	4.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.50
K	TMWV-p	MT20	5.0	6.0	1.50	3.00
M	BMV-i+p	MT20	3.0	6.0		
N, O, T, U						
N	BMWV-i	MT20	5.0	6.0		
P	BS-i	MT20	5.0	6.0		
Q	BMWVW-i	MT20	5.0	8.0		
R	BMWVW-i	MT20	5.0	8.0		
S	BS-i	MT20	5.0	6.0		
V	BMV-i+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: 14)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 (PLF)	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB. MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM	TO				
AR-AS	0 3261	-18.5	-18.5	0.25 (1)	10.00		
AS-N	0 3261	-18.5	-18.5	0.25 (1)	10.00		
N-AT	0 0	-18.5	-18.5	0.64 (4)	10.00		
AT-AU	0 0	-18.5	-18.5	0.64 (4)	10.00		
AU-M	0 0	-18.5	-18.5	0.64 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-10	-49	-55	---	FRONT	VERT	DEAD	---	C1
C	4-0-10	-117	-117	---	FRONT	VERT	TOTAL	---	C1
C	4-0-10	-254	-254	---	FRONT	VERT	SNOW	---	C1
G	17-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
I	25-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
J	31-1-6	-49	-55	---	FRONT	VERT	DEAD	---	C1
J	31-1-6	-254	-254	---	FRONT	VERT	SNOW	---	C1
O	25-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
S	11-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
U	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
W	5-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	11-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	15-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	19-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	21-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AE	23-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AF	27-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AG	29-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AH	1-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AI	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AJ	7-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AK	9-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AL	13-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AM	15-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AN	17-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AO	19-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AP	21-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AQ	23-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AR	27-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AS	29-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AT	31-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AU	33-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007602 72

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	T1Z	1	2	GREEN PARK HOMES	
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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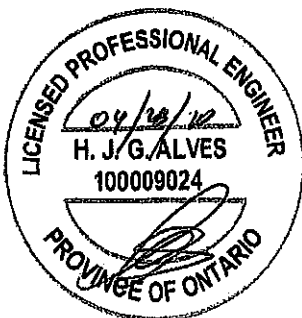
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	8.0	9.0	1.75	3.50
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW-m	MT20	8.0	9.0	1.75	3.50
K	TMVW-p	MT20	5.0	8.0	Edge	
M	BMVt+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.25
O	BMWW+t	MT20	6.0	9.0	4.50	2.50
P	BS-t	MT20	5.0	8.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	8.0		
T	BMWW+t	MT20	6.0	9.0	4.50	2.50
U	BMWW-t	MT20	5.0	6.0	2.50	2.25
V	BMVt+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007603 *7/2*

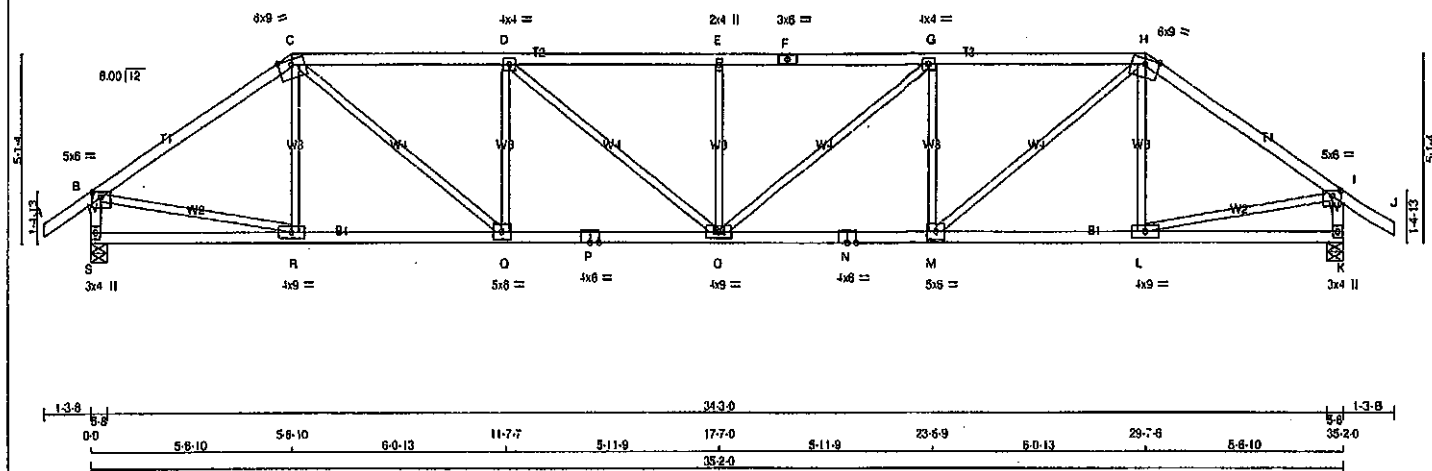
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO
40B222	T2	2	1	GREEN PARK HOMES	
				TRUSS DESC.	

Tamrack Roof Truss, Burlington

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ID:DMCubINVR6TstF0e31v6I zns11-F8H4ajCx5wlmvMOcuAP1x2LCCy5TgYwUz? UzMFld

1-3.8 0.0 5-8-10 5-8-10 5-8-10 11-7.7 5-11-9 17-7.0 5-11-9 23-6.9 5-8-10 29-7.8 5-8-10 35-2.0 1-3.8
Scale = 1:57.6



TOTAL WEIGHT = 2 X 140 = 280 lb

LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
J - K	2x4	DRY	No.2	SPF	
K - L	2x4	DRY	No.2	SPF	
L - M	2x4	DRY	No.2	SPF	
M - N	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	8.0	1.75 2.75
C	TTWW-m	MT20	6.0	9.0	Edge
D	TMVW-l	MT20	4.0	4.0	
E	TMVW-w	MT20	2.0	4.0	
F	TS-l	MT20	3.0	6.0	
G	TMVW-l	MT20	4.0	4.0	
H	TTWW-m	MT20	6.0	9.0	Edge
I	TMVW-p	MT20	5.0	8.0	1.75 2.75
K	BMV1-p	MT20	3.0	4.0	
L	BMVW-l	MT20	4.0	9.0	
M	BMVW-l	MT20	5.0	6.0	
N	BS-l	MT20	4.0	6.0	
O	BMVW-w-l	MT20	4.0	9.0	
P	BS-l	MT20	4.0	6.0	
Q	BMVW-l	MT20	5.0	6.0	
R	BMVW-l	MT20	4.0	9.0	
S	BMV1-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
S	2065	0	2065	0	0	5-8	5-8	5-8	5-8
K	2065	0	2065	0	0	5-8	5-8	5-8	5-8
UNFACTORED REACTIONS									
		1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
S	1458	970.0	0.0	0.0	0.0	488	0	0	0
K	1458	970.0	0.0	0.0	0.0	488	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 35	-91.8	-91.8 0.12 (1)	10.00	R-C	-243 10	0.09 (1)
B-C	-2298 0	-91.8	-91.8 0.70 (1)	3.78	C-Q	0 1670	0.38 (1)
C-D	-3213 0	-91.8	-91.8 0.80 (1)	3.19	Q-D	-929 0	0.38 (1)
D-E	-3594 0	-91.8	-91.8 0.88 (1)	2.97	D-Q	0 490	0.11 (1)
E-F	-3594 0	-91.8	-91.8 0.88 (1)	2.97	Q-E	-505 0	0.19 (1)
F-G	-3594 0	-91.8	-91.8 0.88 (1)	2.97	Q-G	0 490	0.11 (1)
G-H	-3213 0	-91.8	-91.8 0.80 (1)	3.19	M-H	-929 0	0.38 (1)
H-I	-2298 0	-91.8	-91.8 0.70 (1)	3.78	M-H	0 1670	0.38 (1)
I-J	0 35	-91.8	-91.8 0.12 (1)	10.00	L-H	-243 10	0.09 (1)
S-B	-2023 0	0.0	0.0 0.21 (1)	5.94	B-R	0 1943	0.44 (1)
K-I	-2023 0	0.0	0.0 0.21 (1)	5.94	L-I	0 1943	0.44 (1)
S-R	0 0	-18.5	-18.5 0.15 (4)	10.00			
R-Q	0 1903	-18.5	-18.5 0.39 (1)	10.00			
Q-P	0 3213	-18.5	-18.5 0.58 (1)	10.00			
P-O	0 3213	-18.5	-18.5 0.58 (1)	10.00			
O-N	0 3213	-18.5	-18.5 0.58 (1)	10.00			
N-M	0 3213	-18.5	-18.5 0.58 (1)	10.00			
M-L	0 1903	-18.5	-18.5 0.39 (1)	10.00			
L-K	0 0	-18.5	-18.5 0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/989 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/989 (0.39")

CSI: TC=0.88/1.00 (D-E:1), BC=0.58/1.00 (M-O:1), WB=0.44/1.00 (I-L:1), SSI=0.28/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(FSD) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

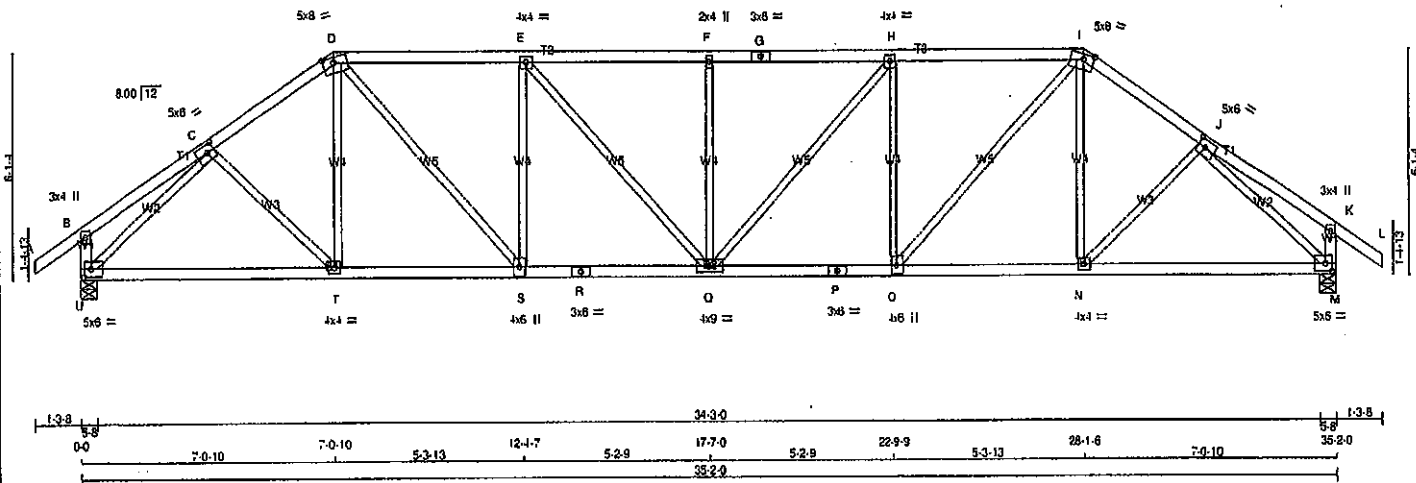
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (I) (INPUT = 0.90)
JSI METAL = 0.74 (I) (INPUT = 1.00)



Structural component only
DWG# T-2007604

JOB NAME 408222	TRUSS NAME T3	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 5 Oct 2019 M&T Industries, Inc. Tue Apr 28 09:00:06 2020 Page 1	
IO:DMCubINVR6TstFoe31v8l_zns11-F8H4ajCx5wlmvMOcuAP1x42QrG_VTXHwUz7_UzMfId				28-1-6 31-9-7 35-2-0 36-5-8	
1-3-8 0-0 3-7-9 3-7-9 3-5-1 7-0-10 5-3-13 12-4-7 5-2-9 17-7-0 5-2-9 22-9-9 5-3-13 28-1-6 31-9-7 35-2-0 36-5-8				Scale = 1:57.6	



TOTAL WEIGHT = 2 X 151 = 303 lb (M)

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
U - R	2x4	DRY	No.2
R - P	2x4	DRY	No.2
P - M	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV-p	MT20	3.0	4.0	
C TMVW-i	MT20	5.0	6.0	2.50 2.50
D TTWW-m	MT20	5.0	6.0	1.75 3.50
E TMVW-i	MT20	4.0	4.0	
F TMVW-w	MT20	2.0	4.0	
G TS-t	MT20	3.0	6.0	
H TMVW-i	MT20	4.0	4.0	
I TTWW-m	MT20	5.0	6.0	1.75 3.50
J TMVW-i	MT20	5.0	6.0	2.50 2.50
K TMV-p	MT20	3.0	4.0	
M BMVW-i	MT20	5.0	6.0	2.50 2.25
N BMVW-i	MT20	4.0	4.0	
O BMVW-i	MT20	4.0	6.0	
P BS-t	MT20	3.0	6.0	
Q BMVWW-i	MT20	4.0	9.0	
R BS-t	MT20	3.0	6.0	
S BMVW-i	MT20	4.0	6.0	
T BMVW-i	MT20	4.0	4.0	
U BMVW-i	MT20	5.0	6.0	2.50 2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED	MAXIMUM FACTORED	INPUT	RECORD	
	GROSS REACTION	GROSS REACTION	BRG	BRG	
	VERT	HORZ	DOWN	UPLIFT	IN-SX
U	2065	0	2065	0	5-8
M	2065	0	2065	0	5-8
UNFACTORED REACTIONS					
JT	COMBINED	SNOW	LIVE	PERALIVE	WIND
U	1458	970.0	0.0	0.0	488.0
M	1458	970.0	0.0	0.0	488.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSF (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSF (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 35	-91.8 -91.8 0.12 (1)	10.00	C-T	0 135	0.03 (4)	
B-C	0 18	-91.8 -91.8 0.16 (1)	10.00	T-D	0 90	0.03 (4)	
C-D	-2305 0	-91.8 -91.8 0.23 (1)	4.31	D-S	0 1218	0.27 (1)	
D-E	-2723 0	-91.8 -91.8 0.34 (1)	3.89	S-E	-814 0	0.48 (1)	
E-F	-2868 0	-91.8 -91.8 0.58 (1)	3.53	E-Q	0 363	0.08 (1)	
F-G	-2868 0	-91.8 -91.8 0.58 (1)	3.53	Q-F	-440 0	0.28 (1)	
G-H	-2868 0	-91.8 -91.8 0.58 (1)	3.53	H-Q	0 383	0.08 (1)	
H-I	-2723 0	-91.8 -91.8 0.54 (1)	3.69	Q-H	-814 0	0.48 (1)	
I-J	-2305 0	-91.8 -91.8 0.23 (1)	4.31	O-I	0 1218	0.27 (1)	
J-K	0 18	-91.8 -91.8 0.16 (1)	10.00	N-I	0 90	0.03 (4)	
K-L	0 35	-91.8 -91.8 0.12 (1)	10.00	N-J	0 135	0.03 (4)	
U-B	-255 0	0.0 0.0 0.03 (1)	7.81	U-C	-2514 0	0.87 (1)	
M-K	-255 0	0.0 0.0 0.03 (1)	7.81	J-M	-2514 0	0.87 (1)	
U-T	0 1801	-18.5 -18.5 0.40 (1)	10.00				
T-S	0 1900	-18.5 -18.5 0.41 (1)	10.00				
S-R	0 2723	-18.5 -18.5 0.49 (1)	10.00				
R-Q	0 2723	-18.5 -18.5 0.49 (1)	10.00				
Q-P	0 2723	-18.5 -18.5 0.49 (1)	10.00				
P-O	0 2723	-18.5 -18.5 0.49 (1)	10.00				
O-N	0 1800	-18.5 -18.5 0.41 (1)	10.00				
N-M	0 1801	-18.5 -18.5 0.40 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CSF: TC=0.58/1.00 (E-F:1), BC=0.49/1.00 (O-Q:1), WB=0.97/1.00 (J-M:1), SS=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

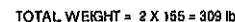
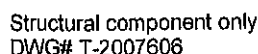
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.99 (M) INPUT = 0.90
JSI METAL= 0.84 (P) INPUT = 1.00



Structural component only
DWG# T-2007605

JSI GRIP = 0.90 (D) (INPUT = 0.90)
JSI METAL = 0.62 (O) (INPUT = 1.00)

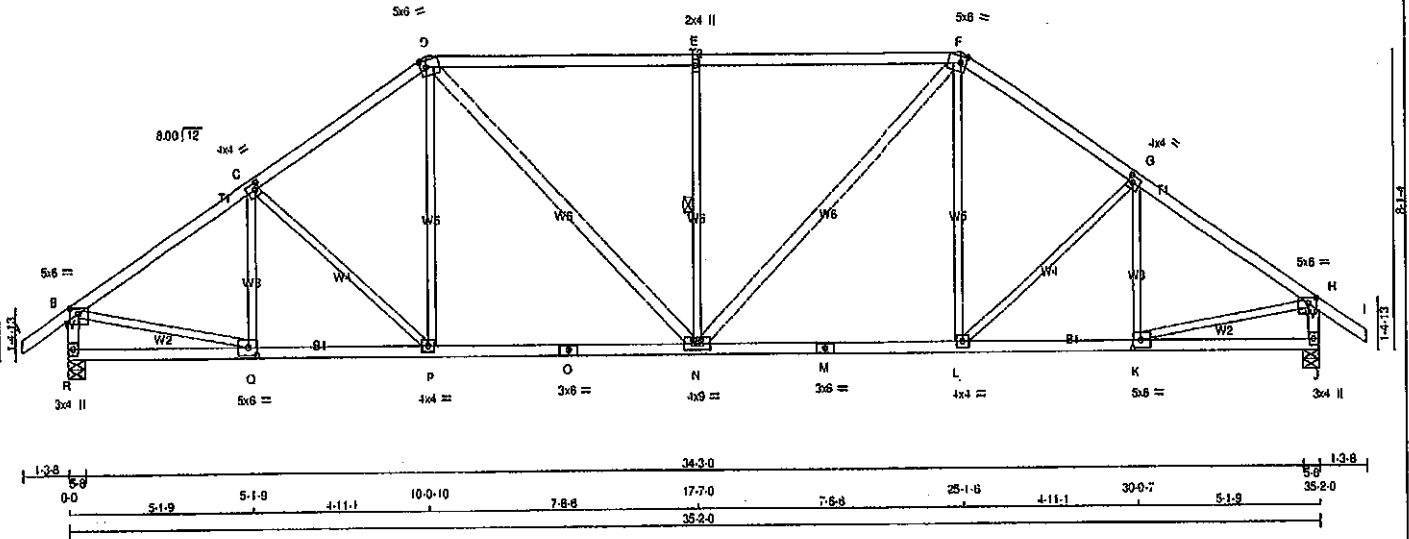
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408222	T5	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe3tvl z1st1-BXPq ODBdXYU8fY??bRV048M4gExZrD7oS52NzMFib

1-3.8 0.0 5-1.9 5-1.9 4-11.1 10-0-10 7-6.6 17-7-0 7-6.6 25-1-6 4-11.1 30-0-7 5-1-9 1-3-8
1-3.8 5-1-9 5-1-9 4-11.1 10-0-10 7-6.6 17-7-0 7-6.6 25-1-6 4-11.1 30-0-7 5-1-9 1-3-8
Scale = 1:57.6



TOTAL WEIGHT = 2 X 158 = 317 lb (M/F)

LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF
A - D	2x4	DRY	No.2	SPF	SPF
D - F	2x4	DRY	No.2	SPF	SPF
F - I	2x4	DRY	No.2	SPF	SPF
I - B	2x4	DRY	No.2	SPF	SPF
B - H	2x4	DRY	No.2	SPF	SPF
H - O	2x4	DRY	No.2	SPF	SPF
O - M	2x4	DRY	No.2	SPF	SPF
M - J	2x4	DRY	No.2	SPF	SPF
ALL WEBS				2x3	DRY
EXCEPT				No.2	SPF
D - N	2x4	DRY	No.2	SPF	SPF
N - F	2x4	DRY	No.2	SPF	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	6.0	1.75 2.75
C	TMVW-l	MT20	4.0	4.0	2.00 1.50
D	TTWW-m	MT20	5.0	6.0	2.25 1.75
E	TMVW-w	MT20	2.0	4.0	
F	TTWW-m	MT20	5.0	6.0	2.25 1.75
G	TMVW-l	MT20	4.0	4.0	2.00 1.50
H	TMVW-p	MT20	5.0	6.0	1.75 2.75
J	BMV1-p	MT20	3.0	4.0	
K	BMVW-l	MT20	5.0	6.0	2.50 2.75
L	BMVW-l	MT20	4.0	4.0	
M	BS-1	MT20	3.0	6.0	
N	BMVW-l	MT20	4.0	4.0	
O	BS-1	MT20	3.0	6.0	
P	BMVW-l	MT20	4.0	4.0	
Q	BMVW-l	MT20	5.0	6.0	2.50 2.75
R	BMV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT	RECRD
	VERT	HORZ	GROSS REACTION	GROSS REACTION	SRG	SRG
R	2065	0	2065	0	5-8	5-8
J	2065	0	2065	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1458	970 0	0 0	0 0	0 0	488 0	0 0
J	1458	970 0	0 0	0 0	0 0	488 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FORCE	VERT. LOAD	MEMB.	MAX. FACTORED	FORCE	MAX. FACTORED
		(LBS)	(PLF)			(LBS)	(LBS)
FR-TO				FR-TO			
A-B	0	35	-91.8 -91.8 0.12 (1)	Q-C	-321	0	0.11 (1)
B-C	-2317	0	-91.8 -91.8 0.38 (1)	C-P	-218	0	0.18 (1)
C-D	-2185	0	-91.8 -91.8 0.37 (1)	P-D	0	268	0.08 (1)
D-E	-2274	0	-91.8 -91.8 0.81 (1)	D-N	0	891	0.11 (1)
E-F	-2274	0	-91.8 -91.8 0.81 (1)	N-E	-851	0	0.35 (1)
F-G	-2185	0	-91.8 -91.8 0.37 (1)	N-F	0	891	0.11 (1)
G-H	-2317	0	-91.8 -91.8 0.38 (1)	L-F	0	268	0.08 (1)
H-I	0	35	-91.8 -91.8 0.12 (1)	L-G	-218	0	0.18 (1)
I-R	-2022	0	0.0 0.0 0.21 (1)	K-G	-321	0	0.11 (1)
J-H	-2022	0	0.0 0.0 0.21 (1)	B-Q	0	1995	0.45 (1)
				K-H	0	1995	0.45 (1)
R-Q	0	0	-18.5 -18.5 0.10 (4)				
Q-P	0	1953	-18.5 -18.5 0.40 (1)				
P-O	0	1794	-18.5 -18.5 0.41 (1)				
O-N	0	1794	-18.5 -18.5 0.41 (1)				
N-M	0	1794	-18.5 -18.5 0.41 (1)				
M-L	0	1794	-18.5 -18.5 0.41 (1)				
L-K	0	1953	-18.5 -18.5 0.40 (1)				
K-J	0	0	-18.5 -18.5 0.10 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.81/1.00 (E/F:1), BC=0.41/1.00 (L/N:1), WB=0.45/1.00 (H/K:1), SS=0.34/1.00 (D/E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

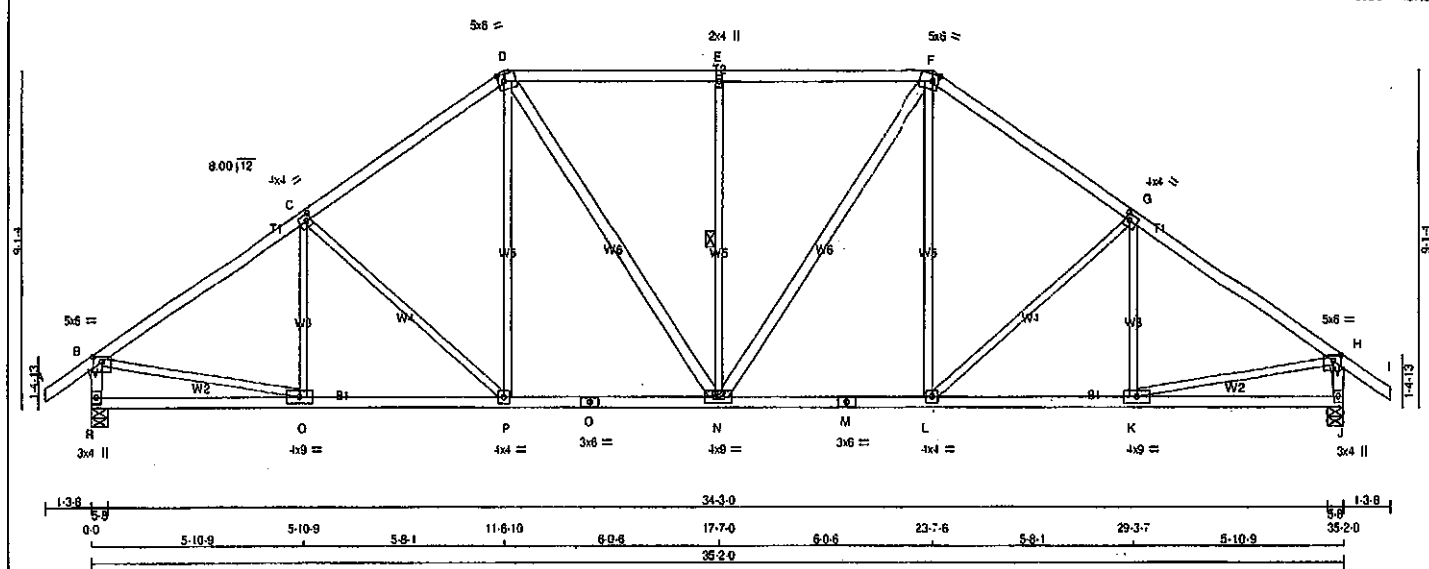
JSI GRIP = 0.87 (H) (INPUT = 0.90)
JSI METAL = 0.64 (M) (INPUT = 1.00)



Structural component only
DWG# T-2007607

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	T6	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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ID:DMCubINVR6TstFoe31v6l_zns1l-fjyCCKEpOrgLmp6BZlykZHqykU0ngD1MLSBapzMFla
Scale = 1:57.6



LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES	A - D	2x4	DRY	No.2	SPF
	D - F	2x4	DRY	No.2	SPF
	F - I	2x4	DRY	No.2	SPF
	R - B	2x4	DRY	No.2	SPF
	J - H	2x4	DRY	No.2	SPF
	R - O	2x4	DRY	No.2	SPF
	O - M	2x4	DRY	No.2	SPF
	M - J	2x4	DRY	No.2	SPF
	ALL WEBS	2x3	DRY	No.2	SPF
	EXCEPT				
	D - N	2x4	DRY	No.2	SPF
	N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W.	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	2.75
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTVW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTVW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.75	2.75
J	BMV1-p	MT20	3.0	4.0		
K	BMVW-t	MT20	4.0	9.0		
L	BMVW-t	MT20	4.0	4.0		
M	BS-1	MT20	3.0	6.0		
N	BMVW-w	MT20	4.0	9.0		
O	BS-1	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	4.0	9.0		
R	BMV1-p	MT20	3.0	4.0		



Structural component only
DWG# T-2007608

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UP	IN-SX
R	2065	0	0	5-8
J	2065	0	0	5-8

UNFACTORED REACTIONS	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
R	1458	970
J	1458	970

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATEROALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
A-B	0	35	-81.8	10.00	Q-C	-249	11	10.00
B-C	-2348	0	-81.8	10.00	P-D	-368	0	10.00
C-D	-2095	0	-81.8	10.00	D-N	0	341	10.00
D-E	-1985	0	-81.8	10.00	N-F	0	477	10.00
E-F	-1985	0	-81.8	10.00	F-G	-678	0	10.00
F-G	-2095	0	-81.8	10.00	G-H	0	341	10.00
G-H	-2348	0	-81.8	10.00	H-I	0	35	10.00
H-I	0	35	-81.8	10.00	I-J	-2018	0	10.00
I-J	-2018	0	-81.8	10.00	J-K	-2018	0	10.00
J-K	-2018	0	-81.8	10.00	K-L	0	0	10.00
K-L	0	0	-81.8	10.00	L-M	0	0	10.00
L-M	0	0	-81.8	10.00	M-N	0	0	10.00
M-N	0	0	-81.8	10.00	N-O	0	0	10.00
N-O	0	0	-81.8	10.00	O-P	0	0	10.00
O-P	0	0	-81.8	10.00	P-Q	0	0	10.00
P-Q	0	0	-81.8	10.00	Q-R	0	0	10.00
Q-R	0	0	-81.8	10.00	R-S	0	0	10.00
R-S	0	0	-81.8	10.00	S-T	0	0	10.00
S-T	0	0	-81.8	10.00	T-U	0	0	10.00
T-U	0	0	-81.8	10.00	U-V	0	0	10.00
U-V	0	0	-81.8	10.00	V-W	0	0	10.00
V-W	0	0	-81.8	10.00	W-X	0	0	10.00
W-X	0	0	-81.8	10.00	X-Y	0	0	10.00
X-Y	0	0	-81.8	10.00	Y-Z	0	0	10.00
Y-Z	0	0	-81.8	10.00	Z-A	0	0	10.00
Z-A	0	0	-81.8	10.00	A-B	0	0	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17')
CALCULATED VERT. DEFL.(LL) = L/999 (0.10')
ALLOWABLE DEFL.(TL) = L/360 (1.17')
CALCULATED VERT. DEFL.(TL) = L/999 (0.19')

CS: TC=0.52/1.00 (G-H:1), BC=0.39/1.00 (K-L:1), WB=0.45/1.00 (H-K:1), SS=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 799 1997 1656

PLATE PLACEMENT TOL. = 0.250 inches

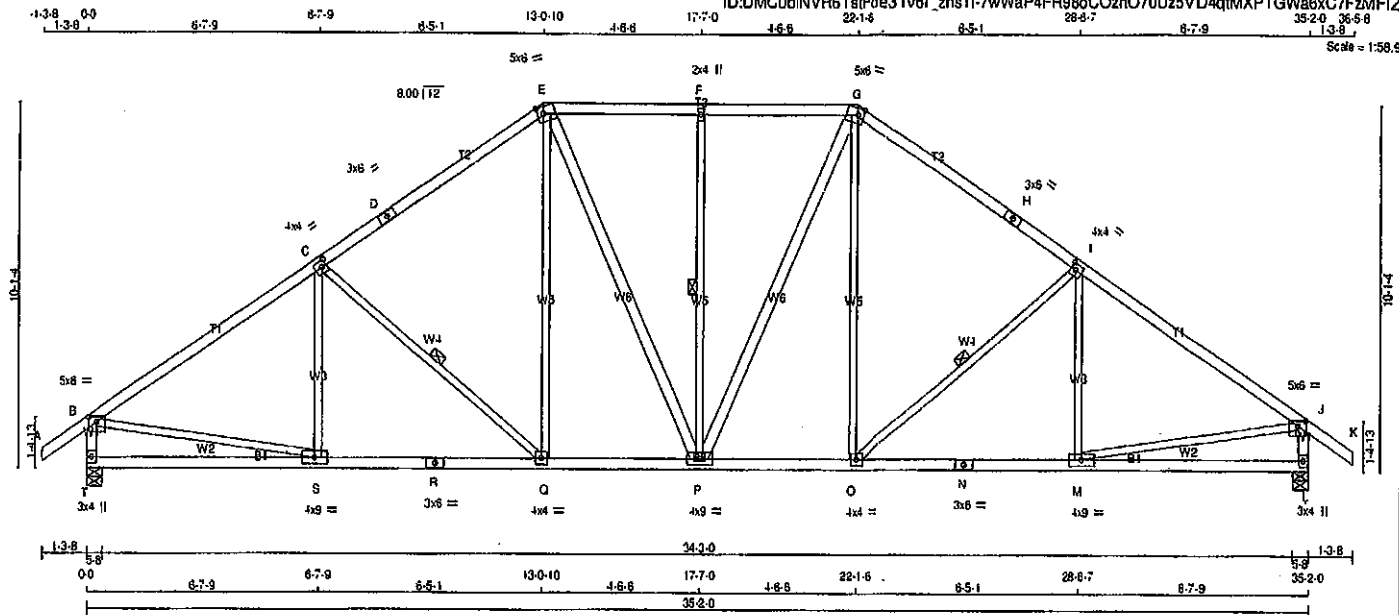
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (H) (INPUT = 0.90)
JSI METAL = 0.56 (M) (INPUT = 1.00)

JOB NAME 408222	TRUSS NAME T7	QUANTITY 6	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:DMCubINVR6TslFoe31v6l_zns11-7wWp4FR980C0zhO70Uz5VD4qtMXPTGWa6xC7FzMFIZ



TOTAL WEIGHT = 6 X 172 = 1030 lb

LUMBER	SIZE	DESCR.
N. L. G. A. RULES		
CHORDS		
A - D	2x4 DRY	No.2
D - E	2x4 DRY	No.2
E - G	2x4 DRY	No.2
G - H	2x4 DRY	No.2
H - K	2x4 DRY	No.2
T - B	2x4 DRY	No.2
L - J	2x4 DRY	No.2
T - R	2x4 DRY	No.2
R - N	2x4 DRY	No.2
N - L	2x4 DRY	No.2
ALL WEBS	2x3 DRY	No.2
EXCEPT		
E - P	2x4 DRY	No.2
P - G	2x4 DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	6.0	1.75	2.75
C TMVW-i	MT20	4.0	4.0	2.00	1.50
D TS-i	MT20	3.0	6.0		
E TTWW-m	MT20	5.0	6.0	2.25	2.00
F TMVW-w	MT20	2.0	4.0		
G TTWW-m	MT20	5.0	6.0	2.25	2.00
H TS-i	MT20	3.0	6.0		
I TMVW-i	MT20	4.0	4.0	2.00	1.50
J TMVW-p	MT20	5.0	6.0	1.75	2.75
L BMV1-p	MT20	3.0	4.0		
M BMVW-i	MT20	4.0	9.0		
N BS-i	MT20	3.0	6.0		
O BMVW-i	MT20	4.0	4.0		
P BMVW-i	MT20	4.0	9.0		
Q BMVW-i	MT20	4.0	4.0		
R BS-i	MT20	3.0	6.0		
S BMVW-i	MT20	4.0	9.0		
T BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
T	2065	0	2065	0
L	2065	0	2065	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN.	COMPONENT REACTIONS	PERM.	LIVE	WIND	DEAD	SOIL
T	1458	970	0	0	0	0	488	0
L	1458	970	0	0	0	0	488	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1:2 LENGTH OF C-Q, F-P, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 35	-91.8	-91.8	0.12 (1)	10.00	S-C	-189 46 0.10 (1)
B-C	-2358 0	-91.8	-91.8	0.89 (1)	3.77	C-Q	-484 0 0.24 (1)
C-D	-1996 0	-91.8	-91.8	0.82 (1)	4.11	E-O	0 415 0.09 (1)
D-E	-1996 0	-91.8	-91.8	0.82 (1)	4.11	E-P	0 315 0.05 (1)
E-F	-1761 0	-91.8	-91.8	0.27 (1)	4.77	P-F	-502 0 0.38 (1)
F-G	-1761 0	-91.8	-91.8	0.27 (1)	4.77	P-G	0 315 0.05 (1)
G-H	-1996 0	-91.8	-91.8	0.82 (1)	4.11	O-G	0 415 0.09 (1)
H-I	-1996 0	-91.8	-91.8	0.82 (1)	4.11	O-I	-484 0 0.24 (1)
I-J	-2358 0	-91.8	-91.8	0.89 (1)	3.77	M-I	-189 48 0.10 (1)
J-K	0 35	-91.8	-91.8	0.12 (1)	10.00	B-S	0 2021 0.45 (1)
T-B	-2014 0	0.0	0.0	0.21 (1)	5.95	M-J	0 2021 0.45 (1)
L-J	-2014 0	0.0	0.0	0.21 (1)	5.95		
T-S	0 0	-18.5	-18.5	0.19 (4)	10.00		
S-R	0 1995	-18.5	-18.5	0.42 (1)	10.00		
R-Q	0 1995	-18.5	-18.5	0.42 (1)	10.00		
Q-P	0 1829	-18.5	-18.5	0.32 (1)	10.00		
P-O	0 1829	-18.5	-18.5	0.32 (1)	10.00		
O-N	0 1995	-18.5	-18.5	0.42 (1)	10.00		
N-M	0 1995	-18.5	-18.5	0.42 (1)	10.00		
M-L	0 0	-18.5	-18.5	0.19 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
DL	= 6.0	PSF	
BOT CH.	LL	= 0.0	PSF
DL	= 7.4	PSF	
TOTAL LOAD	= 39.0	PSF	

SPACING = 24.0 IN. CIO

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/399 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/399 (0.18")

CSI: TC=0.69/1.00 (J-I), BC=0.42/1.00 (M-O),
WB=0.45/1.00 (J-M), SI=0.24/1.00 (J-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX	MIN	MAX
618	384	1987
788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.67 (IN) (INPUT = 1.00)



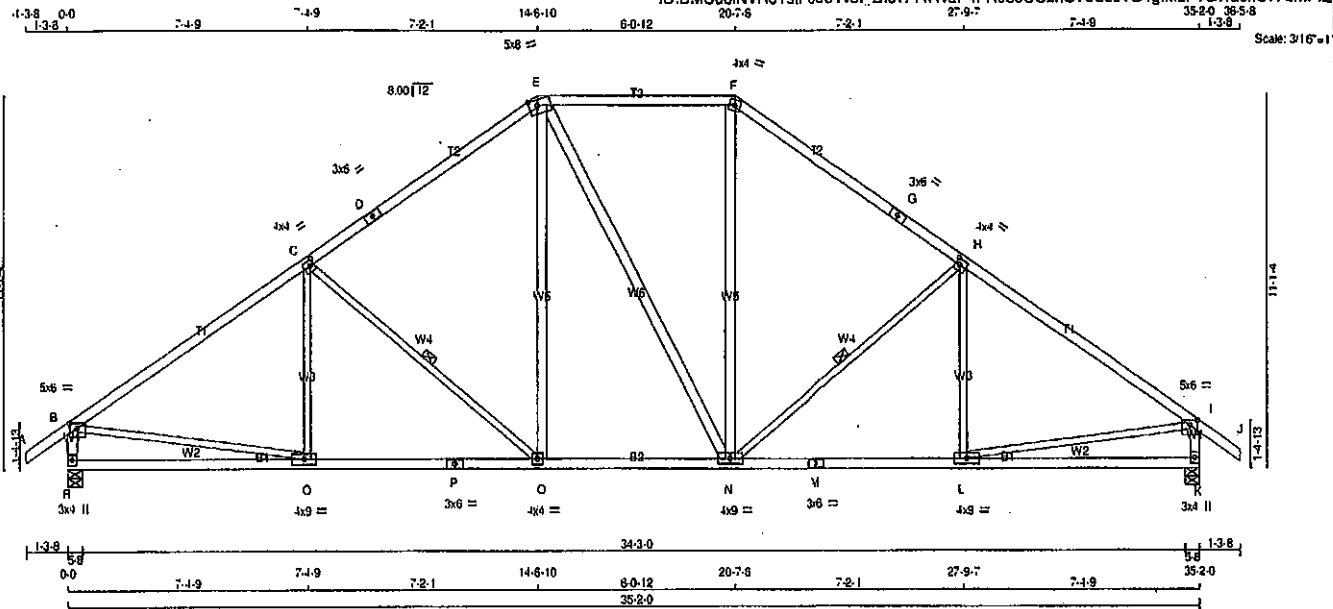
Structural component only
DWG# T-2007609

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408222	T8	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TsiFoe31v6l, zns11-7wWaP4FR98oCOzhO70Uz5VD1gIM2PTGWa8xC7FzMFIZ



TOTAL WEIGHT = 2 X 166 = 333 lb

(M/F)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - E	2x4	DRY	No.2	SPF		
E - F	2x4	DRY	No.2	SPF		
F - G	2x4	DRY	No.2	SPF		
G - J	2x4	DRY	No.2	SPF		
J - B	2x4	DRY	No.2	SPF		
K - I	2x4	DRY	No.2	SPF		
R - P	2x4	DRY	No.2	SPF		
P - M	2x4	DRY	No.2	SPF		
M - K	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						
O - E	2x4	DRY	No.2	SPF		
E - N	2x4	DRY	No.2	SPF		
N - F	2x4	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
O	TMVW-i	MT20	4.0	4.0	2.00	1.50
D	TS-i	MT20	3.0	6.0		
E	TTWV-m	MT20	5.0	8.0	2.00	3.00
F	TTW-m	MT20	4.0	4.0		
G	TS-i	MT20	3.0	6.0		
H	TMVW-i	MT20	4.0	4.0	2.00	1.50
I	TMVW-p	MT20	5.0	6.0	1.75	2.75
K	BMVt+p	MT20	3.0	4.0		
L	BMVW-i	MT20	4.0	9.0		
M	BS-i	MT20	3.0	6.0		
N	BMVW-i	MT20	4.0	9.0		
O	BMVW-i	MT20	4.0	4.0		
P	BS-i	MT20	3.0	6.0		
Q	BMVW-i	MT20	4.0	9.0		
R	BMVt+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
R	2065	0	0	5-8	5-8
K	2065	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
R	1458	970	0	0	0	488	0
K	1458	970	0	0	0	488	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 35	-91.8	-91.8 0.12 (1)	10.00	Q-C	-140 76	0.09 (1)
B-C	-2357 0	-91.8	-91.8 0.89 (1)	3.45	C-O	-604 0	0.38 (1)
C-D	-1888 0	-91.8	-91.8 0.78 (1)	3.91	O-E	0 499	0.08 (1)
D-E	-1888 0	-91.8	-91.8 0.78 (1)	3.91	E-N	0 0	0.00 (1)
E-F	-1538 0	-91.8	-91.8 0.47 (1)	4.74	N-F	0 500	0.08 (1)
F-G	-1888 0	-91.8	-91.8 0.78 (1)	3.91	N-H	-604 0	0.38 (1)
G-H	-1888 0	-91.8	-91.8 0.78 (1)	3.91	L-H	-140 75	0.09 (1)
H-I	-2357 0	-91.8	-91.8 0.89 (1)	3.45	B-Q	0 2020	0.45 (1)
I-J	0 35	-91.8	-91.8 0.12 (1)	10.00	L-I	0 2019	0.45 (1)
R-B	-2009 0	0.0	0.0 0.21 (1)	5.95			
K-I	-2009 0	0.0	0.0 0.21 (1)	5.95			
R-Q	0 0	-18.5	-18.5 0.25 (4)	10.00			
Q-P	0 1998	-18.5	-18.5 0.45 (1)	10.00			
P-O	0 1998	-18.5	-18.5 0.45 (1)	10.00			
O-N	0 1538	-18.5	-18.5 0.33 (1)	10.00			
N-M	0 1998	-18.5	-18.5 0.45 (1)	10.00			
M-L	0 1998	-18.5	-18.5 0.45 (1)	10.00			
L-K	0 0	-18.5	-18.5 0.24 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

ISS 5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17')
CALCULATED VERT. DEFL.(LL) = L/999 (0.09')
ALLOWABLE DEFL.(TL) = L/360 (1.17')
CALCULATED VERT. DEFL.(TL) = L/999 (0.20')

CSI: TC=0.89/1.00 (B-C:1), 8C=0.45/1.00 (O-Q:1), WB=0.45/1.00 (B-Q:1), SSI=0.27/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

MAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (F) INPUT = 0.90
JSI METAL= 0.68 (P) INPUT = 1.00



Structural component only
DWG# T-2007610

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408222	T9	1	2	TRUSS DESC.		

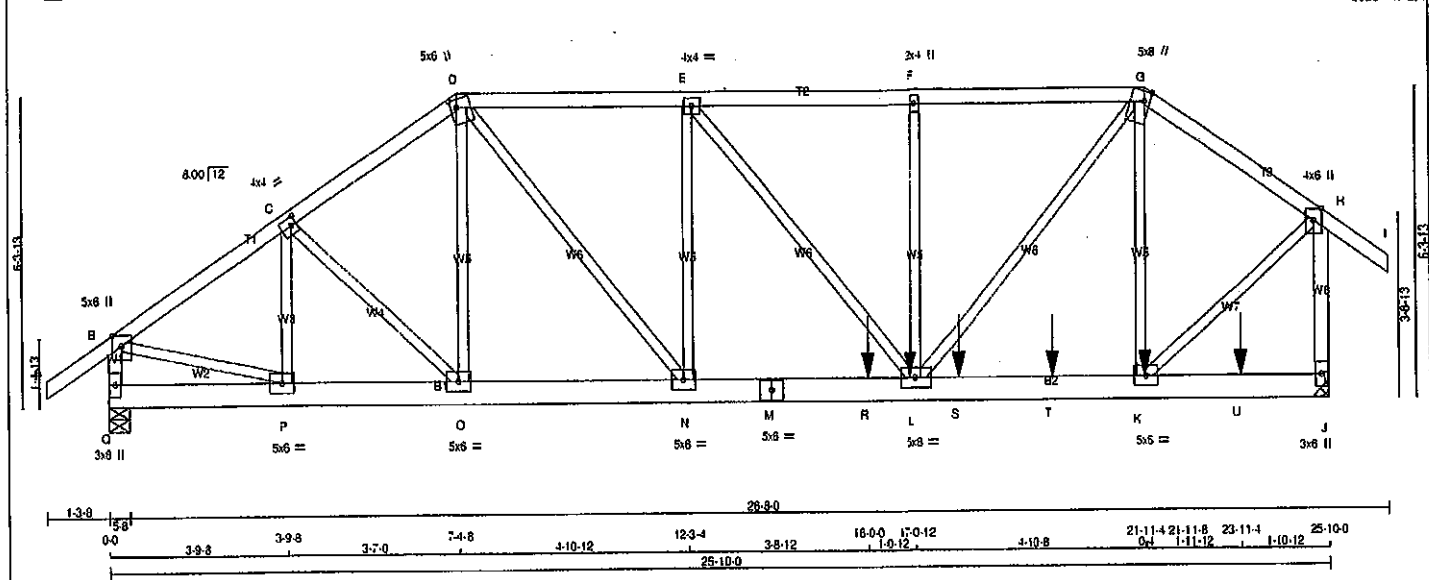
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ID:DMCubINVR6TslFoe31v6L_zns1l-b64ydQG4wSx377Gahj7CeimKBHb88zlpmgmlizMFTY

1-3-8 0-0 3-9-8 3-9-8 3-7-0 7-4-8 4-10-12 12-3-4 4-9-8 17-0-12 4-10-12 21-11-8 3-10-8 25-10-0 1-3-8

Scale = 1:10.4



TOTAL WEIGHT = 2 X 136 = 271 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES					
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - B	2x4	DRY	No.2	SPF	
B - H	2x4	DRY	No.2	SPF	
H - M	2x6	DRY	No.2	SPF	
M - J	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1 12	TOP	
D-G 1 12	TOP	
G-I 1 12	TOP	
I-B 1 12	TOP	
B-H 1 12	TOP	
H-M 2 12	TOP	
M-J 2 12	TOP	
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6	TOP	
	SIDE 183.11	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	UPLIFT
Q	2236	0	2236	0
J	3067	0	3067	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 4-0.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	1576	1085.0	0.0	0.0	0.0	510.0	0.0
J	2157	1479.0	0.0	0.0	0.0	678.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD (LBS)
FR-TO						FR-TO			
A-B	0	35	-91.8	-91.8	0.07 (1)	10.00	P-C	-503	0
B-C	2484	0	-91.8	-91.8	0.13 (1)	5.62	C-O	0	70
C-D	2571	0	-91.8	-91.8	0.13 (1)	5.53	O-D	-2	80
D-E	3030	0	-91.8	-91.8	0.20 (1)	5.11	D-N	0	1433
E-F	3235	0	-91.8	-91.8	0.21 (1)	4.97	N-E	-738	0
F-G	3235	0	-91.8	-91.8	0.20 (1)	4.99	E-L	0	327
G-H	2123	0	-91.8	-91.8	0.15 (1)	5.91	L-F	-469	0
H-I	0	35	-91.8	-91.8	0.07 (1)	10.00	I-G	0	2339
I-B	2180	0	0.0	0.0	0.12 (1)	7.53	G-K	-995	0
J-H	-2982	0	0.0	0.0	0.33 (1)	6.70	B-P	0	2137
							K-H	0	2299
Q-P	0	0	-18.5	-18.5	0.02 (1)	10.00			
P-O	0	2067	-18.5	-18.5	0.16 (1)	10.00			
O-N	0	2118	-18.5	-18.5	0.22 (1)	10.00			
N-M	0	3030	-18.5	-18.5	0.45 (1)	10.00			
M-R	0	3030	-18.5	-18.5	0.45 (1)	10.00			
R-L	0	3030	-18.5	-18.5	0.45 (1)	10.00			
L-S	0	1748	-18.5	-18.5	0.27 (1)	10.00			
S-T	0	1748	-18.5	-18.5	0.27 (1)	10.00			
T-K	0	1748	-18.5	-18.5	0.27 (1)	10.00			
K-U	0	0	-18.5	-18.5	0.08 (1)	10.00			
U-J	0	0	-18.5	-18.5	0.06 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
K	21-11-4	-187	-187	---	FRONT	VERT	TOTAL	---	C1
L	16-11-4	-187	-187	---	FRONT	VERT	TOTAL	---	C1
R	16-0-0	-1259	-1259	---	FRONT	VERT	TOTAL	---	C1
S	17-11-4	-187	-187	---	FRONT	VERT	TOTAL	---	C1
T	19-11-4	-187	-187	---	FRONT	VERT	TOTAL	---	C1
U	23-11-4	-195	-195	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TO=0.33/1.00 (H-J:1), BC=0.45/1.00 (L-N:1),
WB=0.30/1.00 (G-K:1), SSI=0.35/1.00 (L-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (H) (INPUT = 0.90)
JSI METAL = 0.53 (H) (INPUT = 1.00)



Structural component only
DWG# T-2007611 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ORWG NO.
408222	T9	1	2	GREEN PARK HOMES	
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TslFoe31v6l zns11-b64ydQG4wSx377Qah?CelmKBHhB8ztbmamfzMF1Y

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-I	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMWW-I	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.50	1.50
H	TMVW+p	MT20	4.0	6.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K, N, O, P						
K	BMWW-I	MT20	5.0	6.0		
L	BMWWW-I	MT20	5.0	8.0		
M	BS-I	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	6.0		

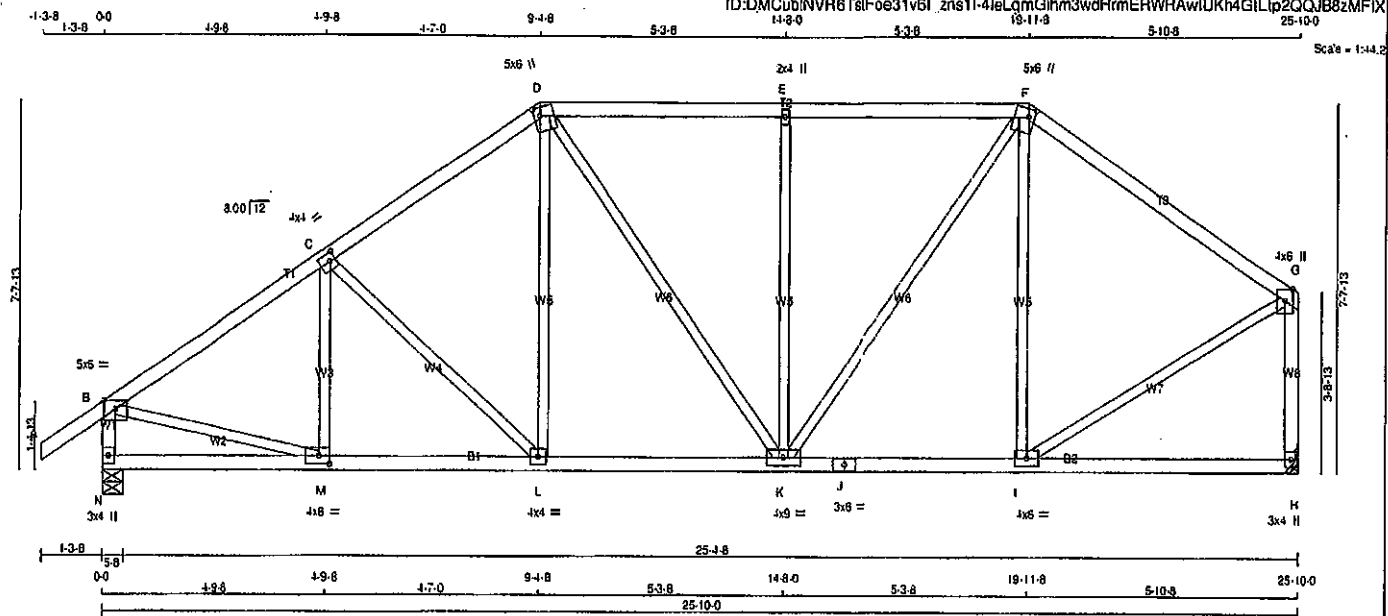
Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



Structural component only
DWG# T-2007611

JOB NAME 408222	TRUSS NAME T10	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:DMCubINVR6TsF0e31v6l_zns1l-4leLqmGihm3wdHrmERWRawIUKh4GILip2QQJB8zMFIX



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
N - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	6.0	Edge	
C TMVW-l	MT20	4.0	4.0	2.00	1.50
D TTWW-m	MT20	5.0	6.0	2.00	1.50
E TMVW-w	MT20	2.0	4.0		
F TTWW-m	MT20	5.0	6.0	2.00	1.50
G TMVW-p	MT20	4.0	6.0	Edge	
H BMVl+p	MT20	3.0	4.0		
I BMVW-l	MT20	4.0	6.0		
J BS-l	MT20	3.0	6.0		
K BMVWW-l	MT20	4.0	9.0		
L BMVW-l	MT20	4.0	4.0		
M BMVW-l	MT20	4.0	6.0	2.00	2.50
N BMVl+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
N	1550	0	1550	0
H	1424	0	1424	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3'-8".

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	1094	731	0	0	0	363	0
H	1007	661	0	0	0	346	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 35	-91.8 -91.8	0.12 (1)	10.00	M-C	-217 3	0.07 (1)
B-C	-1609 0	-91.8 -91.8	0.29 (1)	4.93	C-L	-273 0	0.20 (1)
C-D	-1425 0	-91.8 -91.8	0.28 (1)	5.17	L-D	0 280	0.06 (1)
D-E	-1272 0	-91.8 -91.8	0.35 (1)	5.30	D-K	0 185	0.04 (1)
E-F	-1272 0	-91.8 -91.8	0.35 (1)	5.30	K-E	-595 0	0.64 (1)
F-G	-1077 0	-91.8 -91.8	0.43 (1)	5.52	K-F	0 655	0.15 (1)
N-B	-1511 0	0.0 0.0	0.16 (1)	6.68	I-F	-390 0	0.42 (1)
H-G	-1380 0	0.0 0.0	0.32 (1)	6.92	B-M	0 1396	0.31 (1)
					I-G	0 1031	0.23 (1)
N-M	0 0	-18.5 -18.5	0.09 (4)	10.00			
M-L	0 1362	-18.5 -18.5	0.27 (1)	10.00			
L-K	0 1164	-18.5 -18.5	0.24 (1)	10.00			
K-J	0 890	-18.5 -18.5	0.23 (1)	10.00			
J-I	0 890	-18.5 -18.5	0.23 (1)	10.00			
I-H	0 0	-18.5 -18.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TRC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.43/1.00 (F-G:1), BC=0.27/1.00 (L-M:1),
WB=0.84/1.00 (E-K:1), CSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1 10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

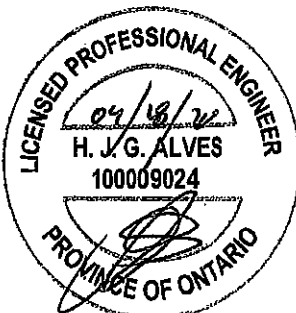
NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	616	354	1867	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.80)
JSI METAL= 0.56 (G) (INPUT = 1.00)



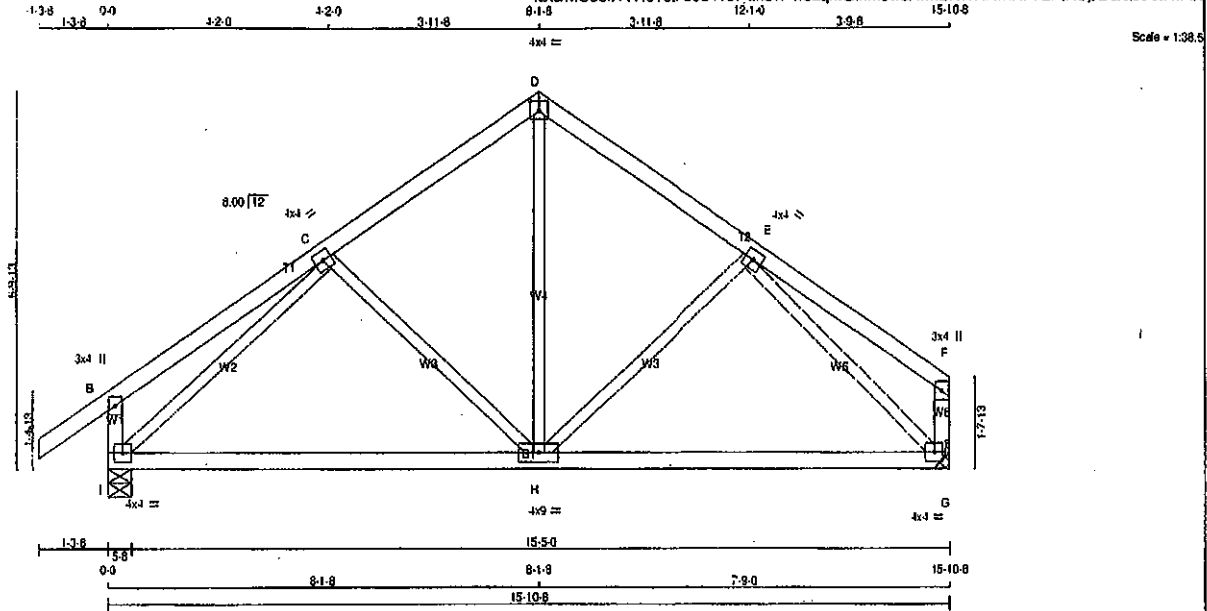
Structural component only
DWG# T-2007612.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408222	T11A	2	1	TRUSS DESC.		

Tamarrack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 68 = 133 lb (M/F)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
I - B	2x4 DRY	No.2	SPF		
G - F	2x4 DRY	No.2	SPF		
I - G	2x4 DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
G	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-i	MT20	4.0	4.0		
H	BMVW1-i	MT20	4.0	9.0		
I	BMVW1-i	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
I	1001	0	1001	0
G	875	0	875	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
I	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
I	706	476	0	0	0	0
G	619	406	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. MAX. (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 35	-91.8	-91.8 0.12 (1)	10.00	C-D	-228 0	0.12 (1)
B-C	0 24	-91.8	-91.8 0.24 (1)	10.00	H-D	0 449	0.10 (1)
C-D	-691 0	-91.8	-91.8 0.19 (1)	6.25	H-E	-188 0	0.09 (1)
D-E	-691 0	-91.8	-91.8 0.17 (1)	6.25	I-C	-988 0	0.50 (1)
E-F	0 24	-91.8	-91.8 0.22 (1)	10.00	E-G	-971 0	0.44 (1)
F-B	-289 0	0.0	0.0 0.03 (1)	7.81			
G-F	-125 0	0.0	0.0 0.01 (1)	7.81			
I-H	0 722	-18.5	-18.5 0.39 (4)	10.00			
H-G	0 677	-18.5	-18.5 0.39 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

15% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.24/1.00 (B-C:1), BC=0.39/1.00 (H-I:4), WB=0.50/1.00 (C-I:1), SS=0.15/1.00 (C-D:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (RY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.84 (E) (INPUT = 0.90)
JSI METAL=0.35 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007613

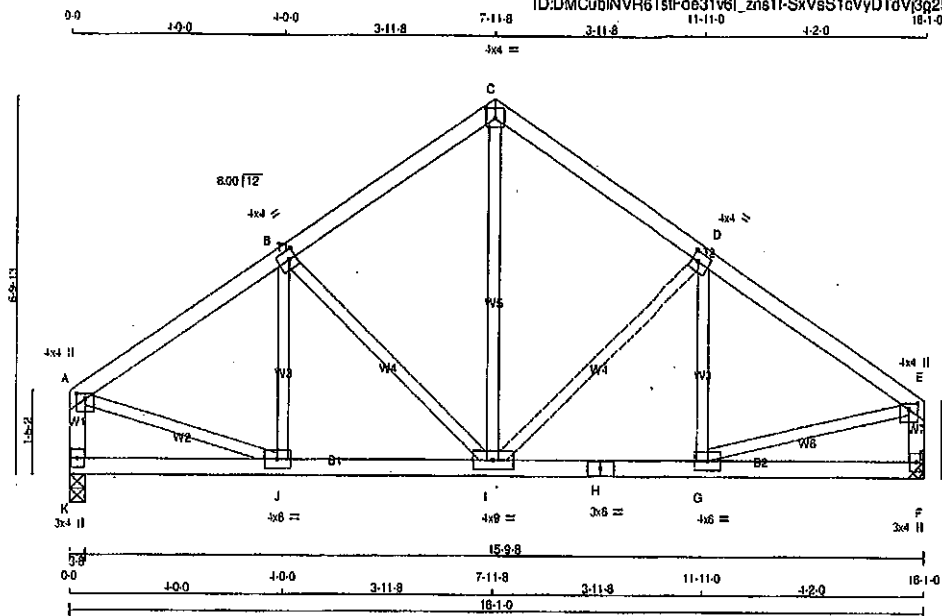
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T11B	2	1	GREEN PARK HOMES	
Tallharack Roof Truss, Burlington				TRUSS DESC.	

Tallharack Roof Truss, Burlington

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ID:DMCubINVR6TstF0e31v6I_zns1I-SxVsStcvDTdV3q25MsNothBxxWB7PW6SQ4izMEMI

Scale = 1:38.5



TOTAL WEIGHT = 2 X 69 = 137 lb
(M)(F)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
K - A	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
K - H	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMVW-I	MT20	4.0	4.0	2.00	1.50
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW-I	MT20	4.0	4.0	2.00	1.50
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVW-I	MT20	4.0	6.0		
H	BS-I	MT20	3.0	6.0		
I	BMVW-I	MT20	4.0	9.0		
J	BMVW-I	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UP/LT	IN-SX
K	887	0	887	0
F	887	0	887	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	827	411 0	0 0	0 0	0 0	216 0	0 0
F	827	411 0	0 0	0 0	0 0	216 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LG1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX FACTORED CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	-868 0	-91.8	-91.8	0.19 (1)	6.25	J-B	-142 18 0.04 (1)
B-C	-897 0	-91.8	-91.8	0.18 (1)	6.25	B-I	-263 0 0.13 (1)
C-D	-868 0	-91.8	-91.8	0.19 (1)	6.25	I-C	0 461 0.10 (1)
D-E	-891 0	-91.8	-91.8	0.20 (1)	6.25	I-D	-292 0 0.15 (1)
K-A	-855 0	0.0	0.0	0.09 (1)	7.81	G-D	-117 28 0.03 (1)
F-E	-853 0	0.0	0.0	0.09 (1)	7.81	A-J	0 772 0.17 (1)
						G-E	0 787 0.18 (1)
K-J	0 0	-18.5	-18.5	0.07 (4)	10.00		
J-I	0 740	-18.5	-18.5	0.15 (1)	10.00		
I-H	0 781	-18.5	-18.5	0.15 (1)	10.00		
H-G	0 781	-18.5	-18.5	0.15 (1)	10.00		
G-F	0 0	-18.5	-18.5	0.07 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.20/1.00 (D-E:1), BC=0.15/1.00 (G-I:1), WB=0.18/1.00 (E-G:1), SS=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

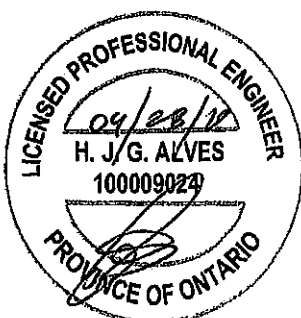
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1656

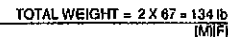
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (E) (INPUT = 0.80)
JSI METAL = 0.25 (H) (INPUT = 1.00)



Structural component only
DWG# T-2007631



ALL WEBS 2x3 DRY
 DRY: SEASONED LUMBER.

SPF

BEARINGS

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L. H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: 14)

L-K	0 0	-18.5	-18.5	0.02 (4)	10.00
K-J	0 1319	-18.5	-18.5	0.33 (1)	10.00
J-I	0 1320	-18.5	-18.5	0.33 (1)	10.00
I-H	0 0	-18.5	-18.5	0.02 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9.
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN
LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF
LIVE LOAD

ALLOWABLE DEFL.(LL)= L380 (0.54")
CALCULATED VERT. DEFL.(LL) = L 899 (0.03")
ALLOWABLE DEFL.(TL)= L360 (0.54")
CALCULATED VERT. DEFL.(TL) = L 999 (0.08")

CS: TC=0.42/1.00 (D-E:1) , BC=0.33/1.00 (I-J:1) ,
WB=0.47/1.00 (E-J:1) , SS=-0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	818	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

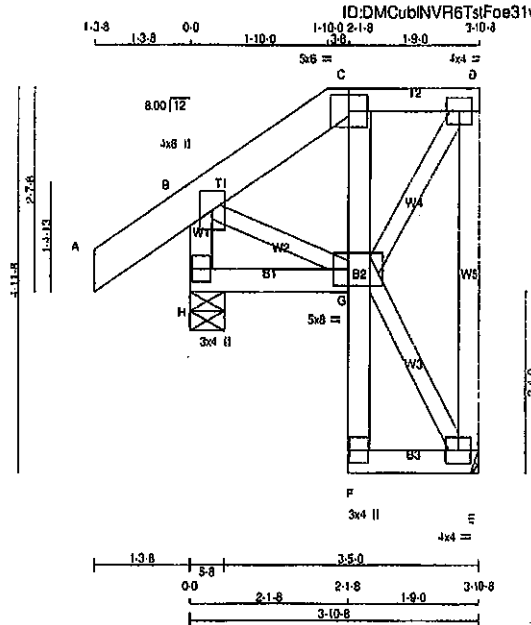
JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.47 (K) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	T12S	1	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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Scale = 1/2" = 1'-0"

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR
A - C	2x6	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
F - C	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMVW+p	MT20	4.0	6.0	
C TTV-m	MT20	5.0	6.0	
D TMVW-l	MT20	4.0	4.0	
E BMVW-l	MT20	4.0	4.0	
F BMV+p	MT20	3.0	4.0	
G BMVWW-l	MT20	5.0	8.0	2.50 2.50
H BMV-l+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	GROSS REACTION	DOWN	HORZ	BRG	BRG	IN-SX	IN-SX
E	214	0	214	0	0	0	0	5-8	5-8
H	344	0	344	0	0	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS		1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	COMBINED	SNOW	LIVE	PERM. LIVE
E	151	99	0	0	0	0	0	52	0	0	0
H	241	172	0	0	0	0	0	69	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.61 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (5)		CHORDS		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. CS (LBS)	MEMB.	FORCE (LBS)	MAX. CS (LBS)	MEMB.
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 36	-91.8	-91.8 0.07 (5)	10.00	G-D	0 139	0.03 (1)
B-C	-105 0	-91.8	-91.8 0.03 (1)	6.25	B-G	0 98	0.02 (1)
C-D	-79 0	-91.8	-91.8 0.05 (1)	6.25	G-E	-3 0	0.00 (1)
E-D	-198 0	0.0	0.0 0.08 (1)	7.81			
H-B	-323 0	0.0	0.0 0.03 (1)	7.81			
H-G	0 0	-18.5	-18.5 0.03 (4)	10.00			
F-G	0 17	0.0	0.0 0.01 (1)	10.00			
G-C	-119 0	0.0	0.0 0.01 (1)	7.81			
F-E	0 2	-18.5	-18.5 0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.08/1.00 (D-E:1), BC=0.03/1.00 (G-H:4), WB=0.03/1.00 (D-G:1), SS=0.07/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	354	1687
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.19 (B) INPUT = 0.90
JSI METAL = 0.11 (B) INPUT = 1.00



Structural component only
DWG# T-2007615

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T20	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 M/Tek Industries, Inc. Tue Apr 28 10:04:02 2020 Page 2	
				ID:DMCubINVR6TsF0g31v6I zns1I-x73EqNd7IXbU8sascoDbPaw0PaBNFZ7YIIB c8zMEMh	

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
B	TMVW-p	MT20	5.0	6.0	1.75 2.75
C	TTWW-m	MT20	6.0	9.0	2.75 3.50
D	TMWW-l	MT20	5.0	8.0	
E	TMW-w	MT20	3.0	6.0	
F	TS-l	MT20	5.0	6.0	
G	TMWW-l	MT20	5.0	6.0	
H	TTWW-m	MT20	6.0	9.0	2.75 3.50
I	TMVW-p	MT20	5.0	6.0	1.75 2.75
K	BMVl-p	MT20	3.0	6.0	
L	BMWW-l	MT20	5.0	6.0	2.50 2.75
M	BMWW-l	MT20	5.0	6.0	
N	BS-l	MT20	5.0	6.0	
O	BMWW-l	MT20	5.0	6.0	
P	BS-l	MT20	5.0	6.0	
Q	BMWW-l	MT20	5.0	6.0	
R	BMWW-l	MT20	5.0	6.0	2.50 2.75
S	BMVl-p	MT20	3.0	6.0	

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED UNBRAC LENGTH FR-TO	MAX. CS1 (LC)	
FR-TO					FR-TO				
L-AR	0 0		-18.5	-18.5 0.08 (4)	10.00				
AR-AS	0 0		-18.5	-18.5 0.08 (4)	10.00				
AS-K	0 0		-18.5	-18.5 0.08 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-10-0	-60	-67	---	FRONT	VERT	DEAD	---	C1
C	4-10-0	-311	-311	---	FRONT	VERT	SNOW	---	C1
D	11-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
H	30-4-0	-60	-67	---	FRONT	VERT	DEAD	---	C1
H	30-4-0	-311	-311	---	FRONT	VERT	SNOW	---	C1
N	21-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	11-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
R	5-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
T	5-2-12	-138	-138	---	FRONT	VERT	TOTAL	---	C1
U	7-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	9-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	13-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	15-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	17-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	19-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	21-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	23-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	25-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	27-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AE	29-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AF	1-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AG	3-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AH	7-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AI	9-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AJ	13-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AK	15-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AL	17-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AM	19-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AN	23-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AO	25-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AP	27-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AQ	29-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AR	31-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AS	33-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) G1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

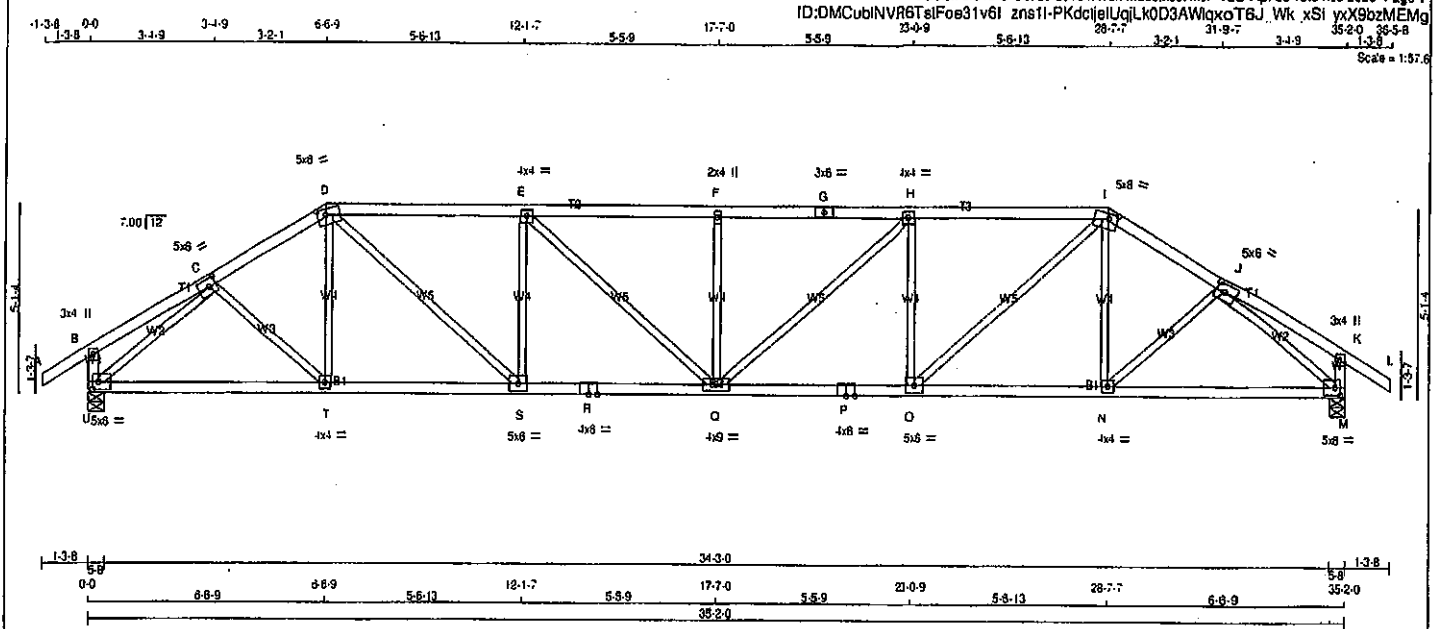


Structural component only
DWG# T-2007632

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T21	1	1	GREEN PARK HOMES	
TRUSS DESC.					

Tagarack Roof Truss, Burlington

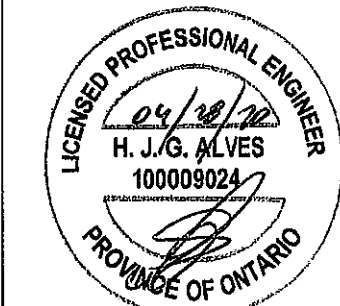
Version 8.310 S Oct 29 2019 Mtek Industries, Inc. Tue Apr 28 10:04:03 2020 Page 1
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352.0 365.8
Scale = 1:97.6



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - G	2x4	DRY	No.2	SPF		
G - I	2x4	DRY	No.2	SPF		
I - L	2x4	DRY	No.2	SPF		
U - B	2x4	DRY	No.2	SPF		
M - K	2x4	DRY	No.2	SPF		
U - R	2x4	DRY	No.2	SPF		
R - P	2x4	DRY	No.2	SPF		
P - M	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVap	MT20	3.0	4.0		
C	TMWW-I	MT20	5.0	6.0	2.50	2.50
D	TTWW-m	MT20	5.0	6.0	1.75	3.00
E	TMWW-I	MT20	4.0	4.0		
F	TMWW-m	MT20	2.0	4.0		
G	TS-I	MT20	3.0	6.0		
H	TMWW-I	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	6.0	1.75	3.00
J	TMWW-I	MT20	5.0	6.0	2.50	2.50
K	TMVap	MT20	3.0	4.0		
M	BMVW-I	MT20	5.0	6.0	2.50	2.00
N	BMVW-I	MT20	4.0	4.0		
O	BMVW-I	MT20	5.0	6.0		
P	BS-I	MT20	4.0	6.0		
Q	BMVW-I	MT20	4.0	9.0		
R	BS-I	MT20	4.0	6.0		
S	BMVW-I	MT20	5.0	6.0		
T	BMVW-I	MT20	3.0	4.0		
U	BMVW-I	MT20	5.0	6.0	2.50	2.00



Structural component only
DWG# T-2007633

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORZ	UPLIFT
U	2084	0	2084	0
M	2084	0	2084	0

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED
U	1457
M	1457

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LOI	MAX	CS1 (LC)	UNBRAC	MEMB.	MAX. FACTORED
FR-TO									
A-B	0	32	-91.8	-91.8	0.12 (1)	10.00	C-T	0	213
B-C	0	13	-91.8	-91.8	0.12 (1)	10.00	T-D	-8	69
C-D	-2527	0	-91.8	-91.8	0.20 (1)	4.18	D-S	0	1447
D-E	-3284	0	-91.8	-91.8	0.68 (1)	3.30	S-E	-853	0
E-F	-3685	0	-91.8	-91.8	0.70 (1)	3.12	E-O	0	428
F-G	-3685	0	-91.8	-91.8	0.70 (1)	3.12	Q-F	-482	0
G-H	-3685	0	-91.8	-91.8	0.70 (1)	3.12	O-H	0	428
H-I	-3284	0	-91.8	-91.8	0.66 (1)	3.30	O-H	-853	0
I-J	-2527	0	-91.8	-91.8	0.20 (1)	4.18	O-I	0	1447
J-K	0	13	-91.8	-91.8	0.12 (1)	10.00	N-I	-8	69
K-L	0	32	-91.8	-91.8	0.12 (1)	10.00	N-J	0	213
U-B	-249	0	0.0	0.0	0.03 (1)	7.81	U-C	-2671	0
M-K	-249	0	0.0	0.0	0.03 (1)	7.81	J-M	-2671	0
U-T	0	2005	-18.5	-18.5	0.42 (1)	10.00			
T-S	0	2167	-18.5	-18.5	0.44 (1)	10.00			
S-R	0	3264	-18.5	-18.5	0.58 (1)	10.00			
R-O	0	3264	-18.5	-18.5	0.58 (1)	10.00			
Q-P	0	3264	-18.5	-18.5	0.58 (1)	10.00			
P-O	0	3264	-18.5	-18.5	0.58 (1)	10.00			
O-N	0	2167	-18.5	-18.5	0.44 (1)	10.00			
N-M	0	2005	-18.5	-18.5	0.42 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

165% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.39")

CS1: TC=0.70/1.00 (E/F:1), BC=0.58/1.00 (G/S:1), WB=0.82/1.00 (J/M:1), SS1=0.24/1.00 (D/E:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.74 (R) (INPUT = 1.00)

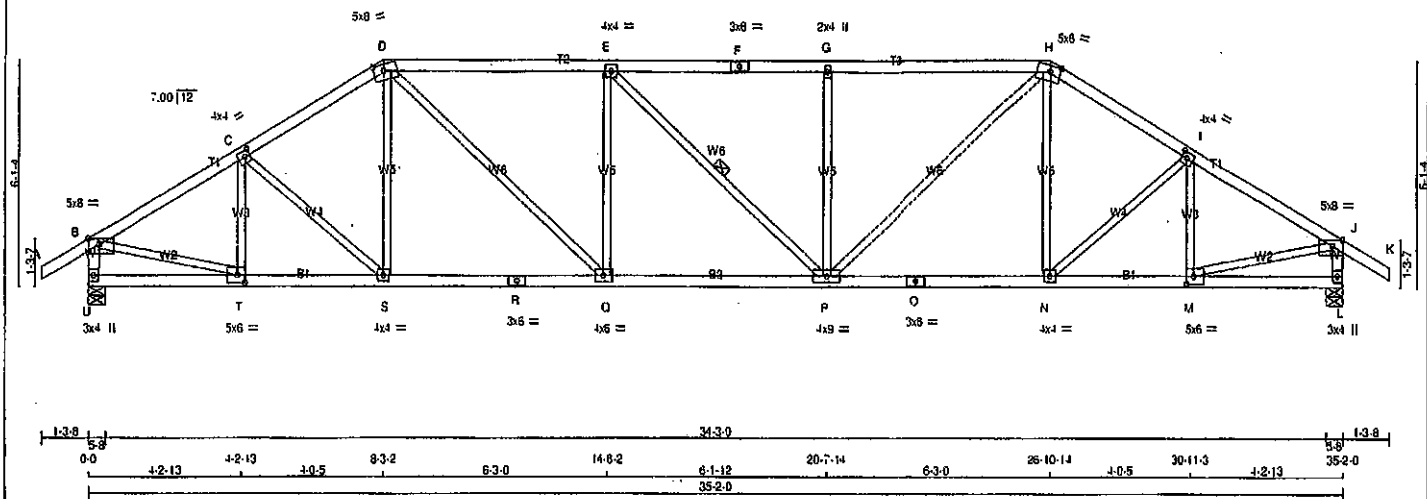
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408223	T22	1	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFos31v8l zns11-IWB743eNF8CMAoFjDG3U70GF0dJTrCcg4h1zMEMI

1-3.8 0-0 1-2.13 1-2.13 1-0.5 3-3.2 6-3.0 14-6.2 8-1.12 20-7.14 6-3.0 26-10.14 1-0.5 30-11.3 1-2.13 1-3.8 35-2.0 36-5.8 Scale = 1:57.6



TOTAL WEIGHT = 146 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
U	2064	0	2064	0
L	2064	0	2064	0

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1457	970	0	0	0	488	0
L	1457	970	0	0	0	488	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE	VERT. LOAD	LOI	MAX.	MAX.	UNBRACED	FR-TO	MEMB.	MAX. FACTORED	FORCE	MAX.
			(LBS)			(PLF)	CSI (LC)	LENGTH				(LBS)	CSI (LC)
A-B	0	32	-91.8	-91.8	0.12 (1)	10.00	T-C	-428	0	0.10 (1)			
B-C	-2468	0	-91.8	-91.8	0.35 (1)	4.07	C-S	-39	0	0.02 (1)			
C-D	-2478	0	-91.8	-91.8	0.34 (1)	4.06	S-D	0	131	0.04 (4)			
D-E	-2910	0	-91.8	-91.8	0.75 (1)	3.37	D-Q	0	1076	0.24 (1)			
E-F	-2909	0	-91.8	-91.8	0.74 (1)	3.38	Q-E	-617	0	0.36 (1)			
F-G	-2909	0	-91.8	-91.8	0.74 (1)	3.38	E-P	-2	0	0.00 (1)			
G-H	-2909	0	-91.8	-91.8	0.75 (1)	3.38	P-G	-617	0	0.36 (1)			
H-I	-2478	0	-91.8	-91.8	0.34 (1)	4.06	P-H	0	1073	0.24 (1)			
I-J	-2468	0	-91.8	-91.8	0.35 (1)	4.07	N-H	0	132	0.04 (4)			
J-K	0	32	-91.8	-91.8	0.12 (1)	10.00	N-I	-39	0	0.02 (1)			
U-B	-2025	0	0.0	0.0	0.21 (1)	5.94	M-I	-426	0	0.10 (1)			
L-J	-2025	0	0.0	0.0	0.21 (1)	5.94	B-T	0	2205	0.50 (1)			
							M-J	0	2205	0.50 (1)			
U-T	0	0	-18.5	-18.5	0.07 (4)	10.00							
T-S	0	2149	-18.5	-18.5	0.41 (1)	10.00							
S-R	0	2122	-18.5	-18.5	0.41 (1)	10.00							
R-Q	0	2122	-18.5	-18.5	0.41 (1)	10.00							
Q-P	0	2910	-18.5	-18.5	0.54 (1)	10.00							
P-O	0	2122	-18.5	-18.5	0.41 (1)	10.00							
O-N	0	2122	-18.5	-18.5	0.41 (1)	10.00							
N-M	0	2149	-18.5	-18.5	0.41 (1)	10.00							
M-L	0	0	-18.5	-18.5	0.07 (4)	10.00							

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.75(1.00 (D-E)), BC=0.54(1.00 (P-Q)), WB=0.50(1.00 (B-T)), SSI=0.27(1.00 (D-E))

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1997 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

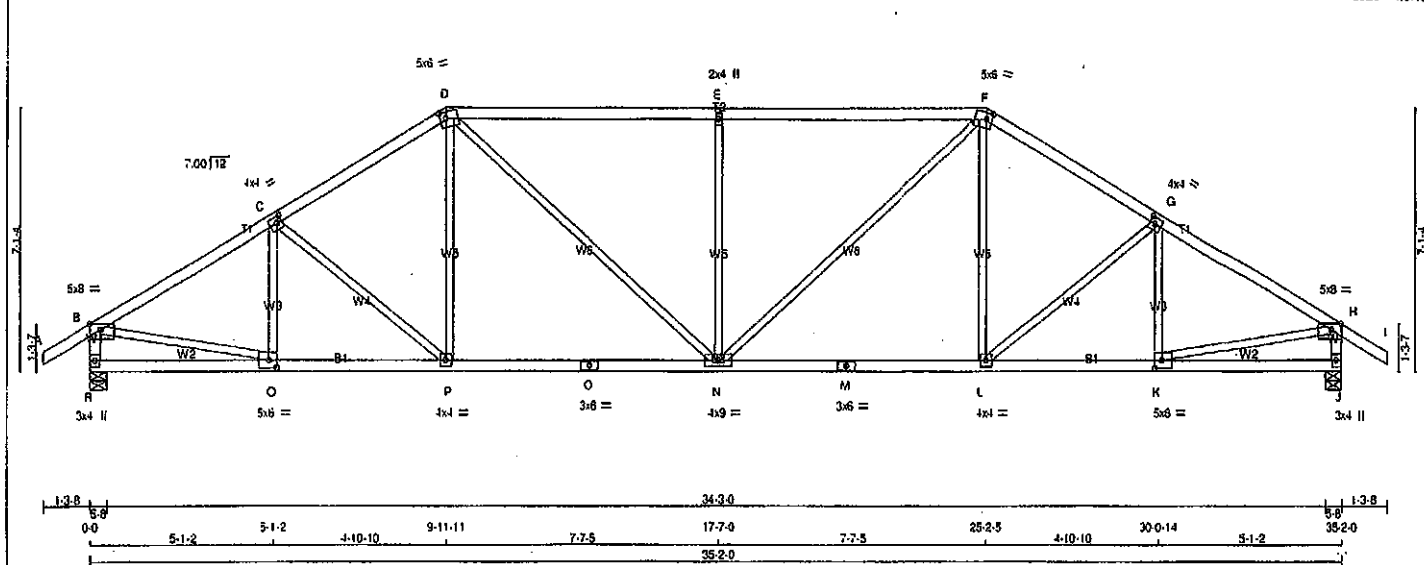
JSI GRIP = 0.89 (T) INPUT = 0.90
JSI METAL = 0.70 (O) INPUT = 1.00



Structural component only
DWG# T-2007634

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T23	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 1-3-8 0-0 5-1-2 5-1-2 4-10-10 9-11-11 7-7-5 17-7-0 7-7-5 25-2-5 4-10-10 30-0-14 5-1-2 35-2-0 1-3-8
 Scale = 1:57.6

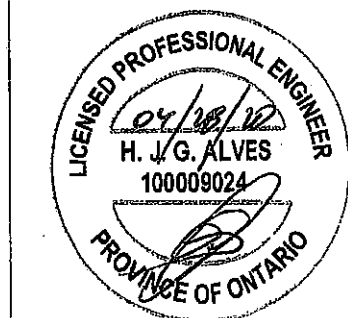


LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	No.2	SPF		
F - I	2x4	DRY	No.2	SPF		
I - B	2x4	DRY	No.2	SPF		
B - H	2x4	DRY	No.2	SPF		
H - O	2x4	DRY	No.2	SPF		
O - M	2x4	DRY	No.2	SPF		
M - J	2x4	DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT
 DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	8.0	Edge	3.50
C	TMWV-l	MT20	4.0	4.0	2.00	1.75
D	TTWV-m	MT20	5.0	6.0	2.00	2.00
E	TMWV-w	MT20	2.0	4.0		
F	TTWV-m	MT20	5.0	6.0	2.00	2.00
G	TMWV-l	MT20	4.0	4.0	2.00	1.75
H	TMWV-p	MT20	5.0	8.0	Edge	3.50
J	BMV1-p	MT20	3.0	4.0		
K	BMWV-l	MT20	5.0	6.0	2.50	2.50
L	BMWV-l	MT20	4.0	4.0		
M	BS-l	MT20	3.0	6.0		
N	BMWVW-l	MT20	4.0	4.0	9.0	
O	BS-l	MT20	3.0	6.0		
P	BMWV-l	MT20	4.0	4.0		
Q	BMWV-l	MT20	5.0	6.0	2.50	2.50
R	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



Structural component only
 DWG# T-2007635

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING DESIGNER								
<u>BEARINGS</u>								
FACTORED			MAXIMUM FACTORED			INPUT	REQRD	
JT	VERT	HORZ	GROSS REACTION	GROSS REACTION	DOWN	UP/LIFT	BRG	BRG
R	2064	0	2064	0	0	5-8	IN-SX	IN-SX
J	2064	0	2064	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LOASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1457	970 0	0 0	0 0	0 0	488 0	0 0
J	1457	970 0	0 0	0 0	0 0	488 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.18 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE	MEMB.	FORCE
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 32	D-C	-335 0
B-C	-2537 0	C-P	-215 0
C-D	-2388 0	P-D	0 258
D-E	-2809 0	D-N	0 752
E-F	-2809 0	N-E	-880 0
F-G	-2395 0	N-F	0 752
G-H	-2537 0	L-F	0 258
H-I	0 32	L-G	-215 0
I-H	-2020 0	K-G	-335 0
J-H	-2020 0	B-Q	0 2253
		K-H	0 2253
R-O	0 0		
Q-P	0 2213		
P-O	0 2048		
O-N	0 2048		
N-M	0 2048		
M-L	0 2048		
L-K	0 2213		
K-J	0 0		

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

ISS % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L 360 (1.17")
 CALCULATED VERT. DEFL.(LL) = L 999 (0.13")
 ALLOWABLE DEFL.(TL) = L 360 (1.17")
 CALCULATED VERT. DEFL.(TL) = L 999 (0.27")

CSI: TC=0.88/1.00 (E-F:1) , BC=0.45/1.00 (K-L:1)
 WB=0.75/1.00 (E-N:1) , SS=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 788 1987 1656

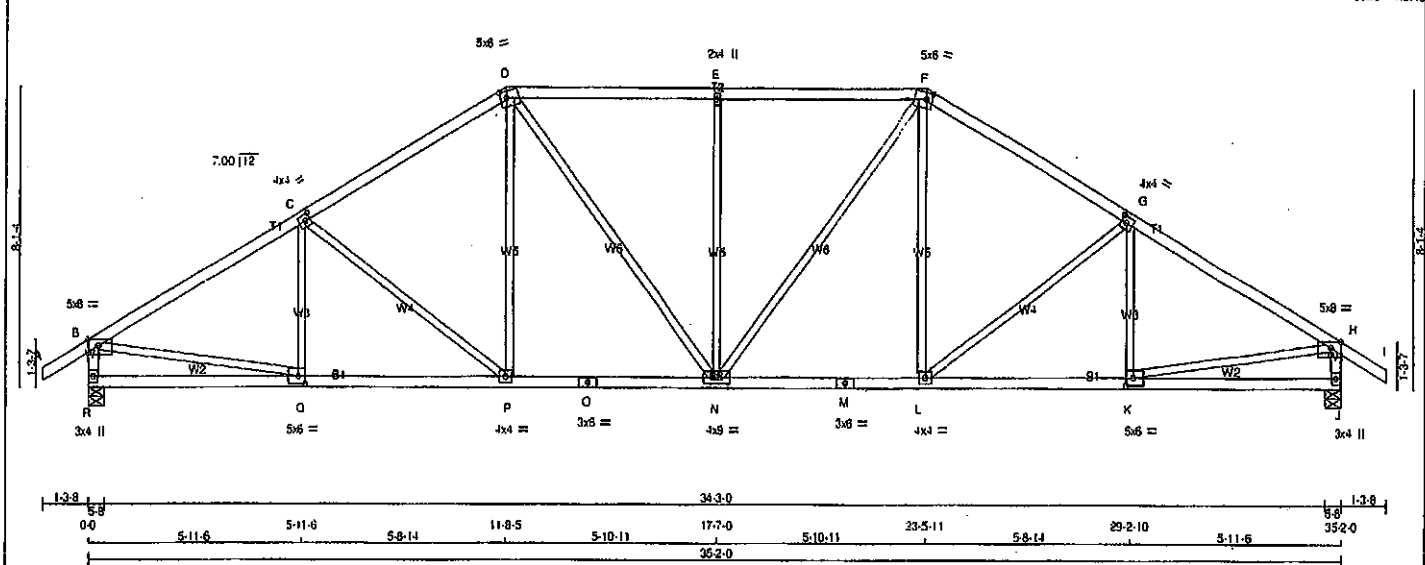
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
 JSI METAL= 0.72 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T24	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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 ID:DMCubINVR6TsFoe31v6L_zns11-H5s74hGX3DnDeWqPLpm8eapobw_wk8HuaVkiMzMEMC
 352-0 36-58 1-3-8
 Scale = 1:57.6



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
R - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATE TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge 3.50
C	TMVW-t	MT20	4.0	4.0	2.00 1.75
D	TTWW-m	MT20	5.0	6.0	2.25 2.00
E	TMVW-w	MT20	2.0	4.0	
F	TTWW-m	MT20	5.0	6.0	2.25 2.00
G	TMVW-t	MT20	4.0	4.0	2.00 1.75
H	TMVW-p	MT20	5.0	8.0	Edge 3.50
J	BMVt-p	MT20	3.0	4.0	
K	BMVW-t	MT20	5.0	6.0	2.50 2.50
L	BMVW-t	MT20	4.0	4.0	
M	BS-t	MT20	3.0	8.0	
N	BMVW-t	MT20	4.0	9.0	
O	BS-t	MT20	3.0	8.0	
P	BMVW-t	MT20	4.0	4.0	
Q	BMVW-t	MT20	5.0	6.0	2.50 2.50
R	BMVt-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
R	2064	0	2064	0
J	2064	0	2064	0

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
R	1457	970	0	0	0
J	1457	970	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	MAX
MEMB.	FORCE	VERT.	LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX
(LBS)	(LBS)	(PLF)	(LC)	(LC)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)
FR-TO	FROM	TO	LENGTH	FR-TO	FROM	TO	LENGTH	FR-TO	FROM
A-B	0	32	-91.8	-91.8	0.12 (1)	10.00	Q-C	-252	12
B-C	-2575	0	-91.8	-91.8	0.54 (1)	3.81	C-P	-388	0
C-D	-2283	0	-91.8	-91.8	0.50 (1)	4.05	P-D	0	345
D-E	-2236	0	-91.8	-91.8	0.49 (1)	4.08	D-N	0	480
E-F	-2236	0	-91.8	-91.8	0.49 (1)	4.08	N-E	-662	0
F-G	-2283	0	-91.8	-91.8	0.50 (1)	4.05	N-F	0	480
G-H	-2575	0	-91.8	-91.8	0.54 (1)	3.81	L-F	0	345
H-I	0	32	-91.8	-91.8	0.12 (1)	10.00	L-G	-388	0
R-B	-2016	0	0.0	0.0	0.21 (1)	5.94	K-G	-252	12
J-H	-2016	0	0.0	0.0	0.21 (1)	5.94	B-Q	0	2281
R-Q	0	0	-18.5	-18.5	0.14 (4)	10.00	K-H	0	2281
Q-P	0	2251	-18.5	-18.5	0.43 (1)	10.00			
P-O	0	1947	-18.5	-18.5	0.38 (1)	10.00			
O-N	0	1947	-18.5	-18.5	0.38 (1)	10.00			
N-M	0	1947	-18.5	-18.5	0.38 (1)	10.00			
M-L	0	1947	-18.5	-18.5	0.38 (1)	10.00			
L-K	0	2251	-18.5	-18.5	0.43 (1)	10.00			
K-J	0	0	-18.5	-18.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 8.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L 380 (1.17")
 CALCULATED VERT. DEFL.(LL) = L 999 (0.12")
 ALLOWABLE DEFL.(TL) = L 360 (1.17")
 CALCULATED VERT. DEFL.(TL) = L 999 (0.22")

CSI: TC=0.54, 1.00 (B-C:1), BC=0.43, 1.00 (P-Q:1), WS=0.84, 1.00 (E-N:1), SS=0.26, 1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1887

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (Q) (INPUT = 0.90)
 JSI METAL= 0.61 (M) (INPUT = 1.00)



Structural component only
 DWG# T-2007636

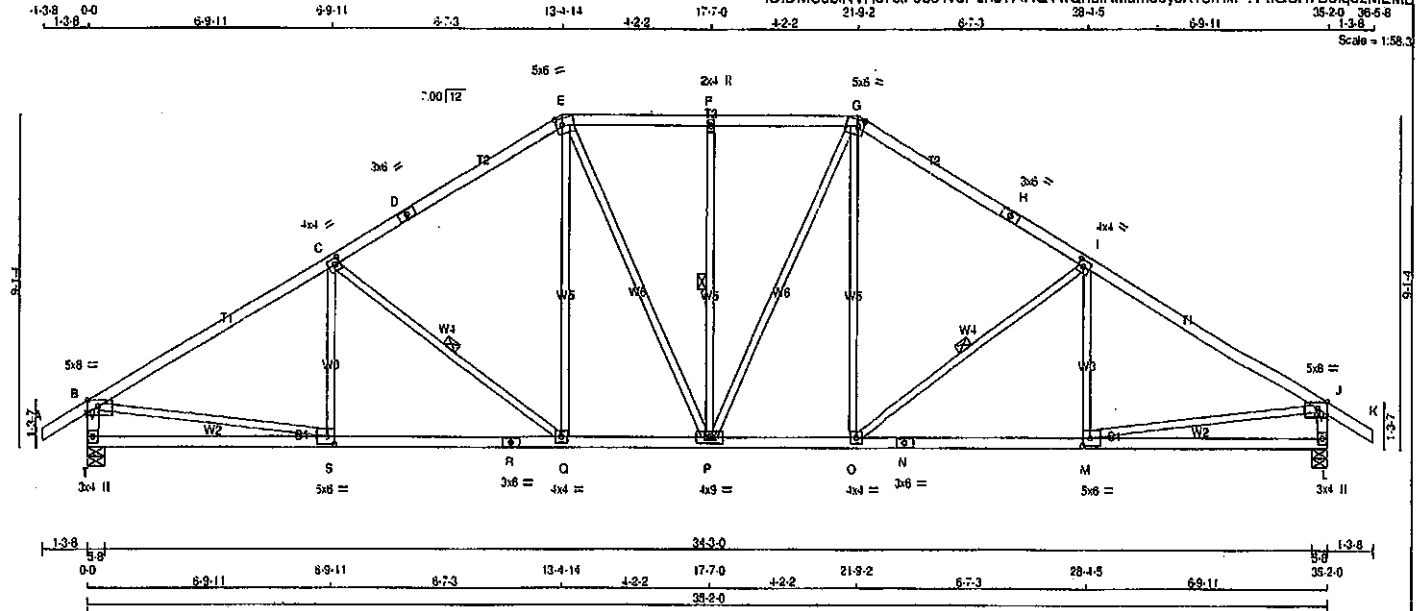
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408223	T25	4	1	GREEN PARK HOMES		

Tantarak Roof Truss, Burlington

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ID:DMCubINVR6TsIFos31v6I zns1IHQVwQhulNMdm50y3K7erAxP?FIKSR7DelqozMEMb

Scale = 1:58.3



TOTAL WEIGHT = 4 X 158 = 632 lb
(MJP)

LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE	MT20	5.0	8.0	Edge 3.50
B TMWW-p	MT20	4.0	4.0	2.00 1.75
C TMWW-t	MT20	3.0	6.0	
D TS-t	MT20	5.0	6.0	2.25 2.00
E TTWW-m	MT20	2.0	4.0	
F TMWW-w	MT20	5.0	6.0	2.25 2.00
G TTWW-m	MT20	3.0	6.0	
H TS-t	MT20	4.0	4.0	2.00 1.75
I TMWW-t	MT20	5.0	8.0	Edge 3.50
J TMWW-p	MT20	3.0	4.0	
M BMWW-t	MT20	5.0	6.0	2.50 2.50
N BS-t	MT20	3.0	6.0	
O BMWW-t	MT20	4.0	4.0	
P BMWW-t	MT20	4.0	4.0	
Q BMWW-t	MT20	4.0	4.0	
R BS-t	MT20	3.0	6.0	
S BMWW-t	MT20	5.0	6.0	2.50 2.50
T BMV-t	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
	FACTORED	MAXIMUM FACTORED	INPUT	RECORD			
	GROSS REACTION	GROSS REACTION	BRG	BRG			
JT	VERT	DOWN	UP/LIFT	IN-SX	IN-SX		
T	2084	0	2084	0	5-8	5-8	
L	2084	0	2084	0	5-8	5-8	

UNFACTORED REACTIONS							
	1ST CASE	MAX. MIN.	COMPONENT	REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1457	970	0	0	0	488	0
L	1457	970	0	0	0	488	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-Q, F-P, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING				TOTAL LOAD CASES: (4)			
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 32	-91.8 -91.8 0.12 (1)	10.00	S-C	-185 51	0.08 (1)	
B-C	-2590 0	-91.8 -91.8 0.74 (1)	3.54	Q-E	-548 0	0.28 (1)	
C-D	-2158 0	-91.8 -91.8 0.86 (1)	3.91	E-P	0 431	0.10 (1)	
D-E	-2158 0	-91.8 -91.8 0.86 (1)	3.91	P-F	0 288	0.08 (1)	
E-F	-1958 0	-91.8 -91.8 0.24 (1)	4.61	F-G	-460 0	0.25 (1)	
F-G	-1958 0	-91.8 -91.8 0.24 (1)	4.61	G-H	0 288	0.08 (1)	
G-H	-2158 0	-91.8 -91.8 0.86 (1)	3.91	H-I	0 431	0.10 (1)	
H-I	-2158 0	-91.8 -91.8 0.86 (1)	3.91	I-J	-548 0	0.28 (1)	
I-J	-2590 0	-91.8 -91.8 0.74 (1)	3.54	M-I	-185 51	0.08 (1)	
J-K	0 32	-91.8 -91.8 0.12 (1)	10.00	B-S	0 2282	0.52 (1)	
T-B	-2011 0	0.0 0.0 0.20 (1)	5.95	M-J	0 2282	0.52 (1)	
L-J	-2011 0	0.0 0.0 0.20 (1)	5.95				
T-S	0 0	-18.5 -18.5 0.20 (4)	10.00				
S-R	0 2269	-18.5 -18.5 0.48 (1)	10.00				
R-Q	0 2269	-18.5 -18.5 0.48 (1)	10.00				
Q-P	0 1833	-18.5 -18.5 0.38 (1)	10.00				
P-O	0 1833	-18.5 -18.5 0.38 (1)	10.00				
O-N	0 2269	-18.5 -18.5 0.48 (1)	10.00				
N-M	0 2269	-18.5 -18.5 0.48 (1)	10.00				
M-L	0 0	-18.5 -18.5 0.20 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.0012

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.74/1.00 (I-J:1), BC=0.46/1.00 (M-O:1), WB=0.52/1.00 (J-M:1), SSI=0.28/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (S) INPUT = 0.90 I
JSI METAL= 0.69 (N) INPUT = 1.00 I



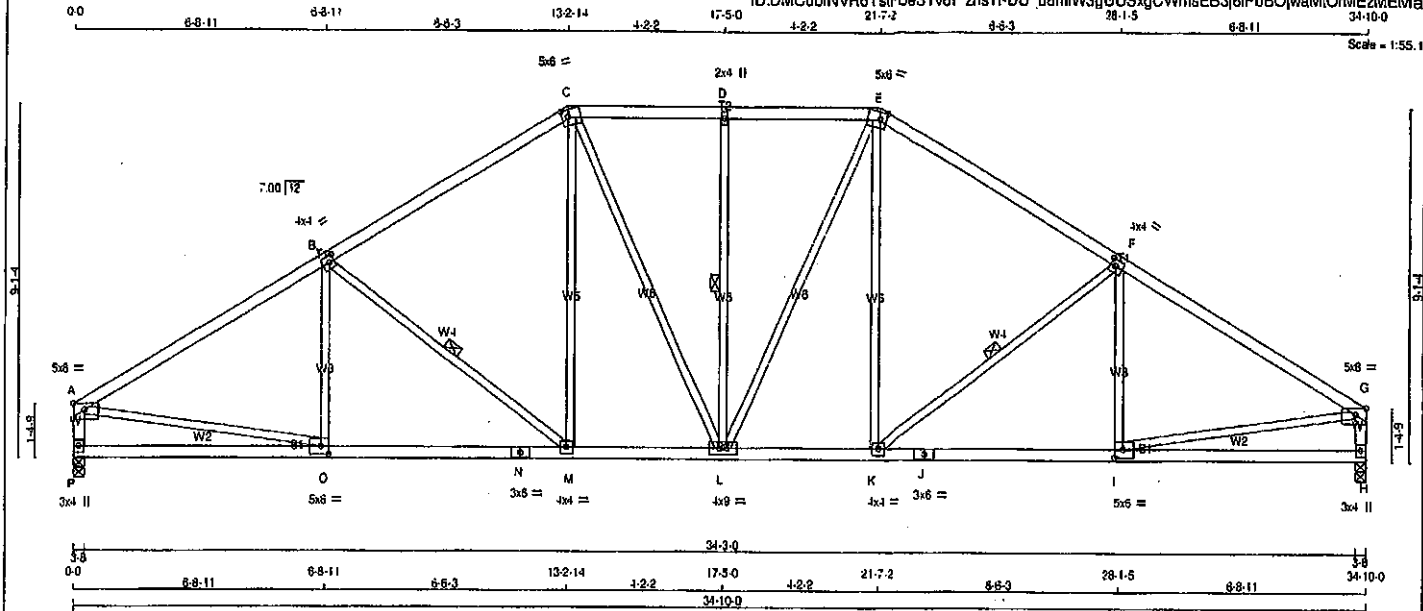
Structural component only
DWG# T-2007637

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408223	T25A	3	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TslFoe31v6l zns11-DU u8miW3gUJUSgCWmsEB3j6lPbBOIwamIOMeMEMa



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
P - A	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
P - N	2x4	DRY	No.2	SPF	
N - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	5.0	8.0	2.00	Edge
B	TMWW-i	MT20	4.0	4.0	2.00	1.75
C	TTWW-m	MT20	5.0	8.0	2.25	2.00
D	TMWW-w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	2.25	2.00
F	TMWW-i	MT20	4.0	4.0	2.00	1.75
G	TMWW-p	MT20	5.0	8.0	Edge	3.50
H	BMV1-p	MT20	3.0	4.0		
I	BMV1-i	MT20	5.0	6.0	2.50	2.50
J	BS-i	MT20	3.0	6.0		
K	BMVW-i	MT20	4.0	4.0		
L	BMVW-w	MT20	4.0	4.0		
M	BMVW-i	MT20	4.0	4.0		
N	BS-i	MT20	3.0	8.0		
O	BMVW-i	MT20	5.0	8.0	2.50	2.50
P	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
P	1920	0	1920	0
H	1920	0	1920	0

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1358	891	0	0	0	0	0
H	1358	891	0	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF 8-M, D-L, F-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. CSF (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSF (LC)
FR-TO						FR-TO			
A-B	-2510	0	-91.8	-91.8	0.71 (1)	3.84	O-B	-212	40
B-C	-2113	0	-91.8	-91.8	0.63 (1)	3.89	B-M	-510	0
C-D	-1919	0	-91.8	-91.8	0.24 (1)	4.84	M-C	0	412
D-E	-1919	0	-91.8	-91.8	0.24 (1)	4.84	C-L	0	287
E-F	-2113	0	-91.8	-91.8	0.63 (1)	3.99	L-D	-461	0
F-G	-2510	0	-91.8	-91.8	0.71 (1)	3.84	L-E	0	287
P-A	-1859	0	0.0	0.0	0.19 (1)	6.13	K-E	0	412
H-G	-1859	0	0.0	0.0	0.19 (1)	6.13	K-F	-510	0
P-O	0	0	-18.5	-18.5	0.20 (4)	10.00	I-F	-212	40
O-N	0	2199	-18.5	-18.5	0.45 (1)	10.00	A-O	0	2227
N-M	0	2199	-18.5	-18.5	0.45 (1)	10.00	I-G	0	2227
M-L	0	1797	-18.5	-18.5	0.35 (1)	10.00			
L-K	0	1797	-18.5	-18.5	0.35 (1)	10.00			
K-J	0	2199	-18.5	-18.5	0.45 (1)	10.00			
J-I	0	2199	-18.5	-18.5	0.45 (1)	10.00			
I-H	0	0	-18.5	-18.5	0.20 (4)	10.00			

TOTAL WEIGHT = 3 X 154 = 462 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.18")
CALCULATED VERT. DEFL. (LL) = L/999 (0.11")
ALLOWABLE DEFL. (TL) = L/360 (1.18")
CALCULATED VERT. DEFL. (TL) = L/999 (0.21")

CSF: TC=0.71 (A-B:1), BC=0.45 (1.00 (M-O:1), WB=0.50 (1.00 (A-O:1), SS=0.26 (1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1657 788 1987 1656

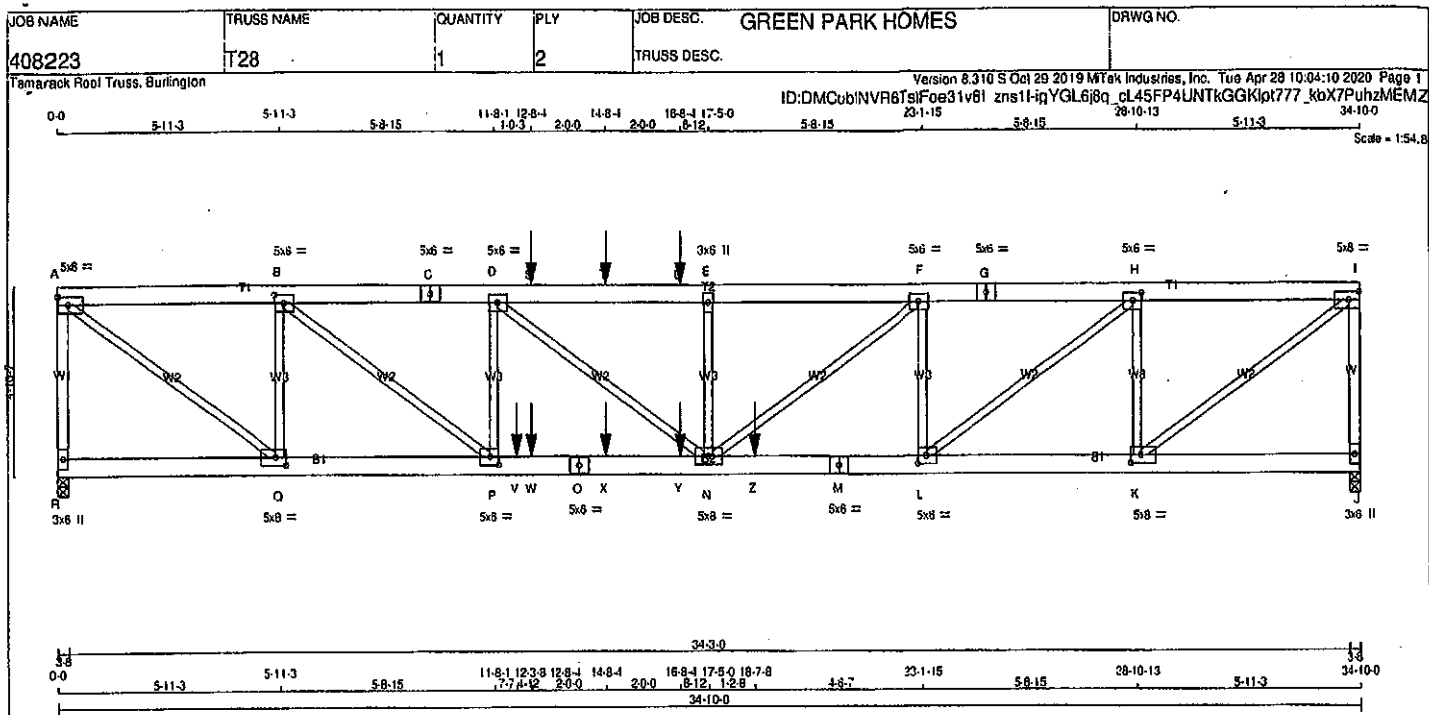
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (1) (INPUT = 0.90)
JSI METAL= 0.66 (N) (INPUT = 1.00)



Structural component only
DWG# T-2007638



TOTAL WEIGHT = 2 X 183 = 366 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
R - A	2x4	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - O	2x4	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS			
R-A	1	12	TOP
I-J	1	12	TOP
A-C	2	12	TOP
C-G	2	12	SIDE(0.0)
G-I	2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS			
R-O	2	12	SIDE(0.0)
O-M	2	12	SIDE(0.0)
M-J	2	12	TOP
WEBS: (0.122"x3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
R	3373	0	0	3-8
J	3103	0	0	3-8

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
R	2380	1591	0	0	0	0	789	0
J	2190	1459	0	0	0	0	731	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS		FACTORED		WEBS	
	MAX. FACTORED	VERT. LOAD	MAX. FACTORED	VERT. LOAD	MAX. FACTORED	VERT. LOAD
R-A	-3316	0	0.0	0.0	0.58	11
A-B	-4149	0	-91.8	-91.8	0.14	11
B-C	-7288	0	-91.8	-91.8	0.20	11
C-D	-7288	0	-91.8	-91.8	0.20	11
D-E	-8278	0	-91.8	-91.8	0.28	11
E-F	-8278	0	-91.8	-91.8	0.28	11
F-G	-8278	0	-91.8	-91.8	0.28	11
G-H	-8278	0	-91.8	-91.8	0.28	11
H-I	-3787	0	-91.8	-91.8	0.13	11
I-J	-3042	0	0.0	0.0	0.51	11
R-Q	0	0	-18.5	-18.5	0.04	11
Q-P	0	-4149	-18.5	-18.5	0.30	11
P-V	0	7288	-18.5	-18.5	0.67	11
V-W	0	7288	-18.5	-18.5	0.67	11
W-O	0	7288	-18.5	-18.5	0.67	11
O-X	0	7288	-18.5	-18.5	0.67	11
X-Y	0	7288	-18.5	-18.5	0.67	11
Y-N	0	7288	-18.5	-18.5	0.67	11
N-Z	0	6586	-18.5	-18.5	0.72	11
Z-M	0	6586	-18.5	-18.5	0.72	11
M-L	0	6586	-18.5	-18.5	0.72	11
L-K	0	3787	-18.5	-18.5	0.32	11
K-J	0	0	-18.5	-18.5	0.04	11

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
S	12-8-4	-88	-88	---	BACK	VERT	TOTAL	C1
T	14-8-4	-88	-88	---	BACK	VERT	TOTAL	C1
U	16-8-4	-88	-88	---	BACK	VERT	TOTAL	C1
V	12-3-8	-1052	-1052	---	BACK	VERT	TOTAL	C1
W	12-8-4	-17	-17	---	BACK	VERT	TOTAL	C1
X	14-8-4	-17	-17	---	BACK	VERT	TOTAL	C1
Y	16-8-4	-17	-17	---	BACK	VERT	TOTAL	C1
Z	18-7-8	-1273	-1273	---	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/999 (0.39")

CSI: TC=0.56/1.00 (A/R:1), GC=0.72/1.00 (L/N:1), WB=0.64/1.00 (A/Q:1), SSI=0.37/1.00 (L/N:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLB)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.73 (M) (INPUT = 1.00)



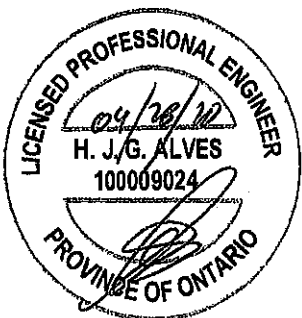
Structural component only
DWG# T-2007639 1/2

CONTINUED ON PAGE 2

JOB NAME 408223	TRUSS NAME T28	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.370 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 10:04:10 2020 Page 2 ID:DMCub\NVR6Ts\Foe3Iv6I zns1I-igYGL68g cl45FP4UNTkGGKPl777 KbX7PuhzMEMZ	

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMWW-I	MT20	5.0	8.0	2.50 3.25
B	TMWW-I	MT20	5.0	6.0	2.50 2.75
C	TS-I	MT20	5.0	6.0	
D	TMWW-I	MT20	5.0	6.0	
E	TMWW-w	MT20	3.0	8.0	
F	TMWW-I	MT20	5.0	6.0	
G	TS-I	MT20	5.0	8.0	
H	TMWW-I	MT20	5.0	6.0	2.50 2.75
I	TMWW-I	MT20	5.0	8.0	2.50 3.25
J	BMV1-p	MT20	3.0	6.0	
K	BMWW-I	MT20	5.0	8.0	2.50 3.25
L	BMWW-I	MT20	5.0	8.0	2.50 2.75
M	BS-I	MT20	5.0	6.0	
N	BMWW-I	MT20	5.0	8.0	
O	BS-I	MT20	5.0	6.0	
P	BMWW-I	MT20	5.0	8.0	2.50 2.75
Q	BMWW-I	MT20	5.0	8.0	2.50 3.25
R	BMV1-p	MT20	3.0	6.0	

CONNECTION REQUIREMENTS	
11	C1: A SUITABLE HANGER MECHANICAL CONNECTION IS REQUIRED.



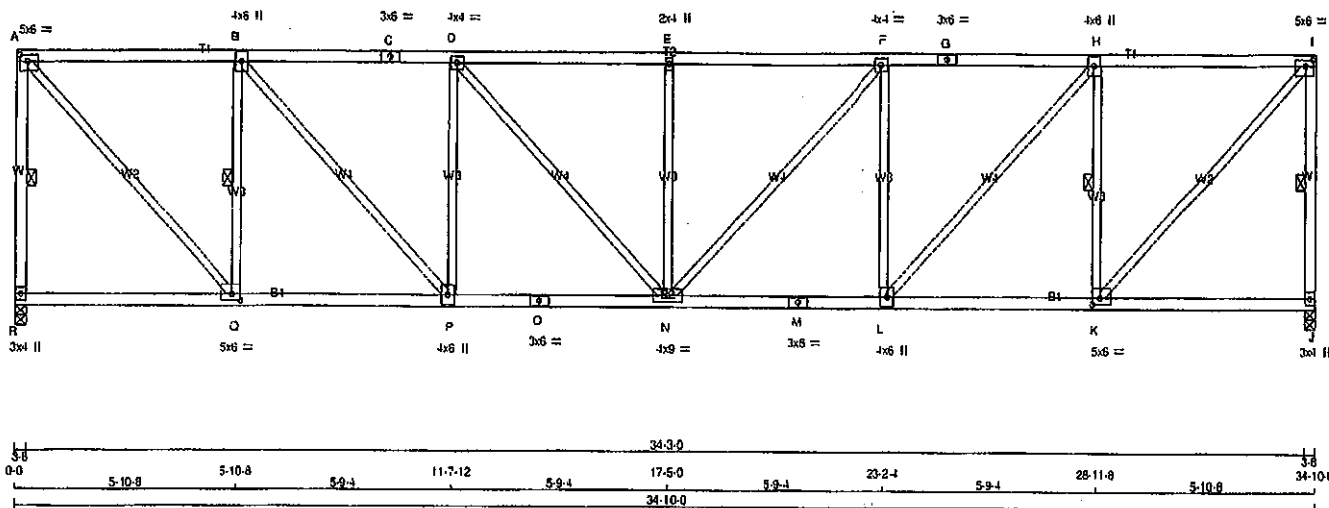
04/28/17
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2007639 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T29	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:04:11 2020 Page 1
ID:DMCUBINVR6TatF0e31v8l zns11-As6eYSkmbkCfQbeBuiGuoUPDIjsbAtpBtyR7zMEMY

0-0 5-10-8 5-10-8 5-9-4 11-7-12 5-9-4 17-5-0 5-9-4 23-2-4 5-9-4 28-11-8 5-10-8 34-10-0
Scale = 1:54.0



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
R - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
J - I	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-1	MT20	5.0	6.0	2.00 2.50
B	TMVW-1	MT20	4.0	8.0	
C	TS-1	MT20	3.0	6.0	
D	TMVW-1	MT20	4.0	4.0	
E	TMVW-1	MT20	2.0	4.0	
F	TMVW-1	MT20	4.0	4.0	
G	TS-1	MT20	3.0	6.0	
H	TMVW-1	MT20	4.0	6.0	
I	TMVW-1	MT20	5.0	6.0	2.00 2.50
J	BMV1+p	MT20	3.0	4.0	
K	BMV1+p	MT20	5.0	6.0	2.00 2.50
L	BMV1+p	MT20	4.0	6.0	
M	BS-1	MT20	3.0	6.0	
N	BMVW-1	MT20	4.0	9.0	
O	BS-1	MT20	3.0	6.0	
P	BMVW-1	MT20	4.0	6.0	
Q	BMVW-1	MT20	5.0	6.0	2.00 2.50
R	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED	MAXIMUM FACTORED	INPUT	REQD	
	GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
R	1920	0	1920	0	0
J	1920	0	1920	0	0

UNFACTORED REACTIONS

1ST LOASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1358	891	0	0	0	0	0
J	1358	891	0	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-R, I-J, B-Q, H-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSH (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	-1878	0.0	0.0 0.37 (1)	A-Q	0	2281	0.51 (1)
B-C	-1583	-91.8	-91.8 0.53 (1)	B-P	-1540	0	0.43 (1)
C-D	-2428	-91.8	-91.8 0.63 (1)	C-R	0	1272	0.29 (1)
D-E	-2725	-91.8	-91.8 0.63 (1)	D-N	0	440	0.10 (1)
E-F	-2725	-91.8	-91.8 0.58 (1)	E-M	-539	0	0.38 (1)
F-G	-2428	-91.8	-91.8 0.63 (1)	F-L	0	440	0.10 (1)
G-H	-2428	-91.8	-91.8 0.63 (1)	G-K	-831	0	0.58 (1)
H-I	-1583	-91.8	-91.8 0.53 (1)	H-J	0	1272	0.29 (1)
I-J	-1878	0.0	0.0 0.37 (1)	I-K	-1540	0	0.43 (1)
R-Q	0	-18.5	-18.5 0.15 (4)	K-L	0	2281	0.51 (1)
Q-P	0	-18.5	-18.5 0.33 (1)				
P-O	0	-18.5	-18.5 0.44 (1)				
O-N	0	-18.5	-18.5 0.44 (1)				
N-M	0	-18.5	-18.5 0.44 (1)				
M-L	0	-18.5	-18.5 0.44 (1)				
L-K	0	-18.5	-18.5 0.33 (1)				
K-J	0	-18.5	-18.5 0.15 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPC 2011, TPC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/989 (0.31")

CS: TC=0.63/1.00 (F-H:1), BC=0.44/1.00 (N-P:1), WB=0.58/1.00 (F-L:1), SS=0.25/1.00 (A-B:1)

COL LUMBER=1.00 NAIL=1.00 LS 6END=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (A) (INPUT = 0.90)
JSI METAL = 0.78 (M) (INPUT = 1.00)



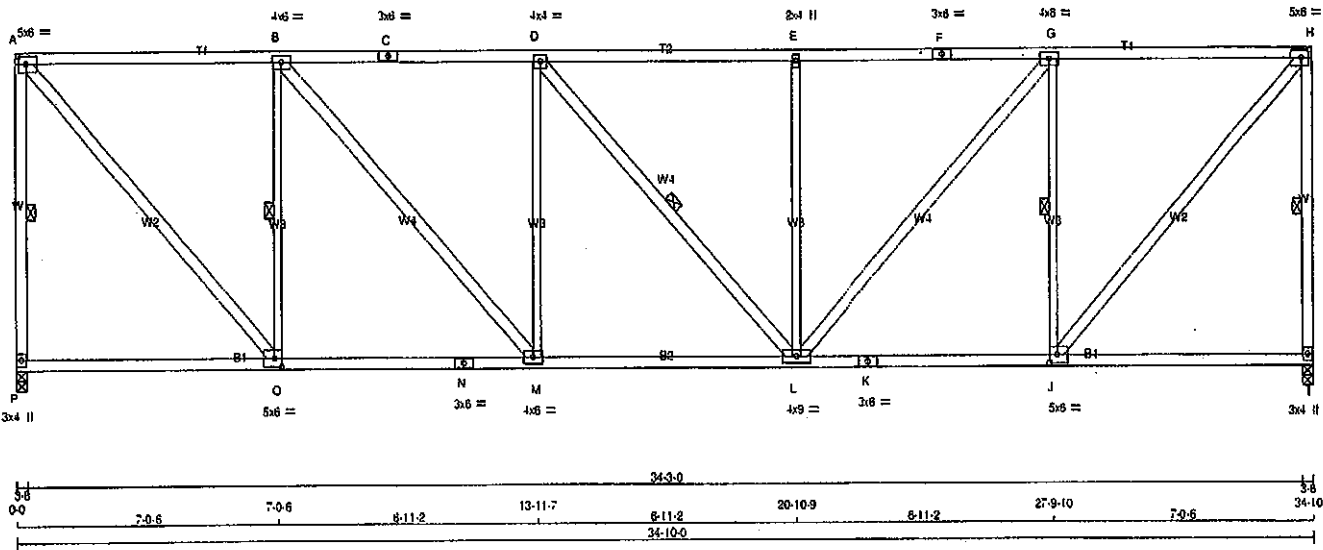
Structural component only
DWG# T-2007640

JOB NAME 408223	TRUSS NAME T30	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR6TstFoe31v6l_zns11-e3p0m0k0Mbs3JPPnBvPxpHb5cdJb7s02rcvZzMEMX

0+0 7-0-6 7-0-6 8-11-2 13-11-7 8-11-2 20-10-9 8-11-2 27-9-10 7-9-6 34-10-0
Scale = 1:55.0



TOTAL WEIGHT = 2 X 179 = 359 lb

LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
P-A	2x4	DRY	No.2
A-C	2x4	DRY	No.2
C-F	2x4	DRY	No.2
F-H	2x4	DRY	No.2
I-H	2x4	DRY	No.2
P-N	2x4	DRY	No.2
N-K	2x4	DRY	No.2
K-I	2x4	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			
O-B	2x3	DRY	No.2
M-D	2x3	DRY	No.2
L-E	2x3	DRY	No.2
J-G	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
A-TMWW-1	MT20	5.0	6.0	2.50	2.50
B-TMWW-1	MT20	4.0	6.0		
C-TS-1	MT20	3.0	6.0		
D-TMWW-1	MT20	4.0	4.0		
E-TMWW-1	MT20	2.0	4.0		
F-TS-1	MT20	3.0	6.0		
G-TMWW-1	MT20	4.0	6.0		
H-TMWW-1	MT20	5.0	6.0	2.50	2.50
I-BMW1-p	MT20	3.0	4.0		
J-BMW1-p	MT20	5.0	6.0	2.50	2.50
K-BS-1	MT20	3.0	6.0		
L-BMW1-p	MT20	4.0	6.0		
M-BMW1-p	MT20	4.0	6.0		
N-BS-1	MT20	3.0	6.0		
O-BMW1-p	MT20	5.0	6.0	2.50	2.50
P-BMW1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ
P	1920	0	1920	0
I	1920	0	1920	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
P	1358	891	0	0	0	0
I	1358	891	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1:2 LENGTH OF A-P, H-I, B-O, D-L, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN

THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
P-A	-1868	0	0.0	A-O	0	2158	0.35 (1)
A-B	-1432	0	-91.8	O-B	-1466	0	0.62 (1)
B-C	-2081	0	-91.8	B-M	0	986	0.16 (1)
C-D	-2081	0	-91.8	M-O	-517	0	0.81 (1)
D-E	-2080	0	-91.8	O-L	-2	0	0.00 (1)
E-F	-2080	0	-91.8	L-E	-616	0	0.81 (1)
F-G	-2080	0	-91.8	G-L	0	984	0.16 (1)
G-H	-1432	0	-91.8	L-G	-1465	0	0.82 (1)
H-I	-1868	0	0.0	I-H	0	2158	0.35 (1)

P-O	0	0	-18.5	-18.5	0.22 (4)	10.00
O-N	0	1432	-18.5	-18.5	0.35 (1)	10.00
N-M	0	1432	-18.5	-18.5	0.35 (1)	10.00
M-L	0	2081	-18.5	-18.5	0.42 (1)	10.00
L-K	0	1432	-18.5	-18.5	0.35 (1)	10.00
K-J	0	1432	-18.5	-18.5	0.35 (1)	10.00
J-I	0	0	-18.5	-18.5	0.22 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 6.0	PSF
BOY CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1/16")
CALCULATED VERT. DEFL. (LL) = L/999 (0.12")
ALLOWABLE DEFL. (TL) = L/360 (1/16")
CALCULATED VERT. DEFL. (TL) = L/999 (0.24")

CS: TC=0.89/1.00 (E-G:1), BC=0.42/1.00 (L-M:1), WB=0.81/1.00 (D-M:1), SS=0.30/1.00 (G-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618	354
	1687	788
	1957	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (H) (INPUT = 0.90)
JSI METAL = 0.46 (K) (INPUT = 1.00)



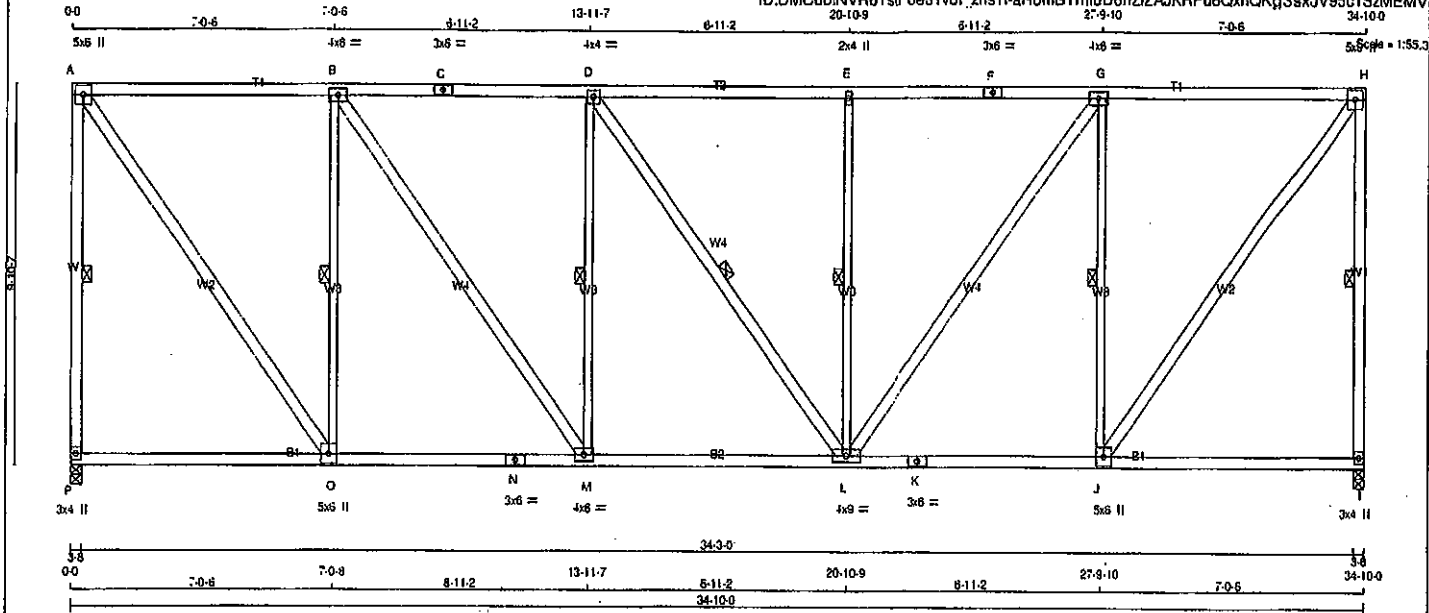
Structural component only
DWG# T-2007641

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T31	5	1	GREEN PARK HOMES	
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TsF0e31v8I_zns11aRomBTmJfD6nZiAJKRPu6QxnQKq3sxJV95c1SzMEMV



CHORDS	SIZE	DRY	LUMBER	DESCR.
P - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

ALL WEBS	SIZE	DRY	LUMBER	DESCR.
EXCEPT				
O - B	2x3	DRY	No.2	SPF
M - D	2x3	DRY	No.2	SPF
L - E	2x3	DRY	No.2	SPF
J - G	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	TYPE	PLATES	W	LEN	Y	X
A	TMVW+P	MT20	5.0	6.0		
B	TMVW+I	MT20	4.0	6.0		
C	TS-I	MT20	3.0	6.0		
D	TMVW+I	MT20	4.0	4.0		
E	TMVW+I	MT20	2.0	4.0		
F	TS-I	MT20	3.0	6.0		
G	TMVW+I	MT20	4.0	6.0		
H	TMVW+P	MT20	5.0	6.0		
I	BMV1+P	MT20	3.0	4.0		
J	BMVW+I	MT20	5.0	6.0		
K	BS-I	MT20	3.0	6.0		
L	BMVW+I	MT20	4.0	9.0		
M	BMVW+I	MT20	4.0	6.0		
N	BS-I	MT20	3.0	6.0		
O	BMVW+I	MT20	5.0	6.0		
P	BMV1+P	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	UPLIFT
P	1920	0	0	3-8
I	1920	0	0	3-8

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED
P	1358 891 0 0 0 0 0 0 467 0 0 0
I	1358 891 0 0 0 0 0 0 467 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-P, H-I, B-O, D-M, O-L, E-L, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX
(LBS)	(LBS)	(PLF)	CS1(LC)	UNBRAC	LENGTH	FR-TO	(LBS)	CS1(LC)
P-A	-1888	0	0.0	0.0	0.88 (1)	4.91	A-O	0 1993 0.32 (1)
A-B	-1183	0	-91.8	-91.8	0.75 (1)	4.69	O-B	-1466 0 0.96 (1)
B-C	-1719	0	-91.8	-91.8	0.82 (1)	3.98	B-M	0 916 0.15 (1)
C-D	-1719	0	-91.8	-91.8	0.82 (1)	3.98	M-D	-817 0 0.41 (1)
D-E	-1718	0	-91.8	-91.8	0.83 (1)	4.37	D-L	-2 0 0.00 (1)
E-F	-1718	0	-91.8	-91.8	0.83 (1)	3.98	L-E	-816 0 0.41 (1)
F-G	-1718	0	-91.8	-91.8	0.83 (1)	3.98	G-L	0 913 0.15 (1)
G-H	-1183	0	-91.8	-91.8	0.75 (1)	4.69	J-G	-1466 0 0.96 (1)
H-I	-1888	0	0.0	0.0	0.88 (1)	4.91	J-H	0 2000 0.32 (1)

P-O	0 0	-18.5	-18.5	0.22 (4)	10.00
O-N	0 1183	-18.5	-18.5	0.32 (4)	10.00
N-M	0 1183	-18.5	-18.5	0.32 (4)	10.00
M-L	0 1719	-18.5	-18.5	0.36 (1)	10.00
L-K	0 1183	-18.5	-18.5	0.32 (4)	10.00
K-J	0 1183	-18.5	-18.5	0.32 (4)	10.00
J-I	0 0	-18.5	-18.5	0.22 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2018 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.18")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.18")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CS1: TC=0.88/1.00 (H+I), BC=0.36/1.00 (L-M), WS=0.98/1.00 (B-O), SS=0.30/1.00 (G-H)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.85 (H) (INPUT=0.90)
JSI METAL=0.43 (H) (INPUT=1.00)



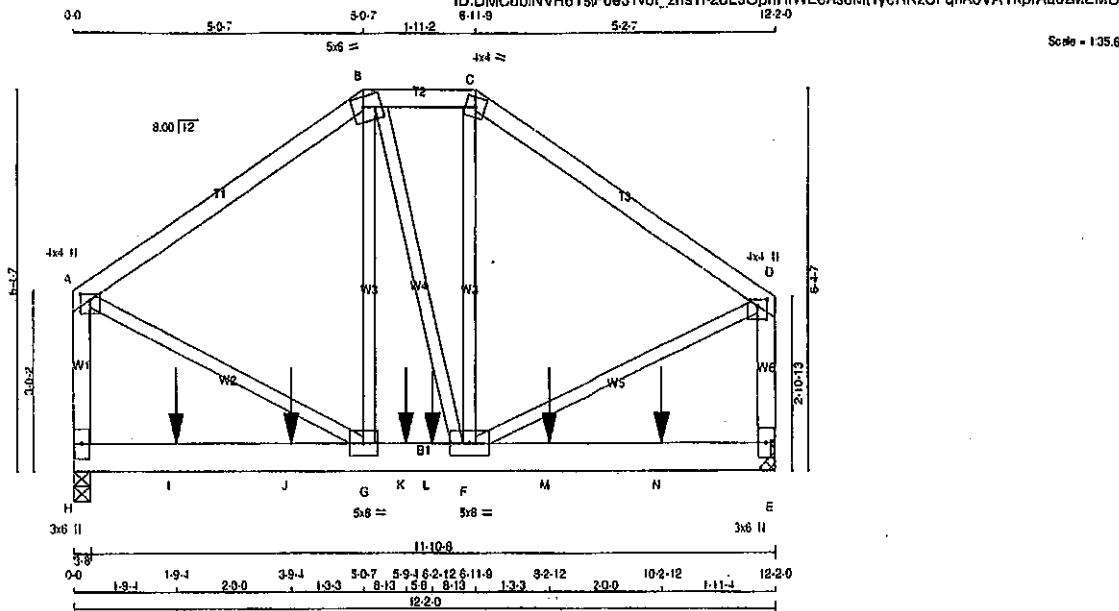
Structural component only
DWG# T-2007642

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408223	T32	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TsFoe31v6t_2ns11-2dL9OpnHlWEeAs8Mt1yeRKzCFqAovATkprAauzMEMU



TOTAL WEIGHT = 68 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
A - B	2x4	DRY	No.2		
B - C	2x4	DRY	No.2		
C - D	2x4	DRY	No.2		
D - E	2x4	DRY	No.2		
E - F	2x4	DRY	No.2		
F - G	2x4	DRY	No.2		
G - H	2x4	DRY	No.2		
H - I	2x4	DRY	No.2		
I - J	2x4	DRY	No.2		
J - K	2x4	DRY	No.2		
K - L	2x4	DRY	No.2		
L - M	2x4	DRY	No.2		
M - N	2x4	DRY	No.2		
N - O	2x4	DRY	No.2		
O - P	2x4	DRY	No.2		
P - Q	2x4	DRY	No.2		
Q - R	2x4	DRY	No.2		
R - S	2x4	DRY	No.2		
S - T	2x4	DRY	No.2		
T - U	2x4	DRY	No.2		
U - V	2x4	DRY	No.2		
V - W	2x4	DRY	No.2		
W - X	2x4	DRY	No.2		
X - Y	2x4	DRY	No.2		
Y - Z	2x4	DRY	No.2		
Z - A	2x4	DRY	No.2		

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW+p	MT20	4.0	4.0	1.25	2.00
B	TTWW-m	MT20	5.0	6.0	2.00	2.25
C	TTW-m	MT20	4.0	4.0		
D	TMW+p	MT20	4.0	4.0	1.25	2.00
E	BMV1+p	MT20	3.0	6.0		
F	BMWWW-1	MT20	5.0	8.0		
G	BMWW-1	MT20	5.0	8.0		
H	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
			IN-SX	IN-SX
H	1233	0	1233	0
E	1218	0	1218	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	867	596	0	0	0	0	271	0
E	856	588	0	0	0	0	268	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	-853	0	-91.8	-91.8
B-C	-721	0	-91.8	-91.8
C-D	-864	0	-91.8	-91.8
D-E	-1085	0	-91.8	-91.8
E-F	-1057	0	-91.8	-91.8
F-G	0	0	-18.5	-18.5
G-H	0	0	-18.5	-18.5
H-I	0	0	-18.5	-18.5
I-J	0	0	-18.5	-18.5
J-K	0	0	-18.5	-18.5
K-L	0	0	-18.5	-18.5
L-M	0	0	-18.5	-18.5
M-N	0	0	-18.5	-18.5
N-O	0	0	-18.5	-18.5

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC	LOC	MAX	MAX	FACE	DIR.	TYPE	HEEL	CONN.
I	1-9-4	-172	-172	---	BACK	VERT	TOTAL	---	C1
J	3-9-4	-195	-195	---	BACK	VERT	TOTAL	---	C1
K	5-9-4	-187	-187	---	BACK	VERT	TOTAL	---	C1
L	6-2-12	-187	-187	---	BACK	VERT	TOTAL	---	C1
M	8-2-12	-195	-195	---	BACK	VERT	TOTAL	---	C1
N	10-2-12	-172	-172	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
OL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2018 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.51 (C-D:1), BC=0.28 (F-G:1), WB=0.20 (A-G:1), SS=0.20 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.78 (D) (INPUT = 0.90)
JSI METAL = 0.23 (D) (INPUT = 1.00)

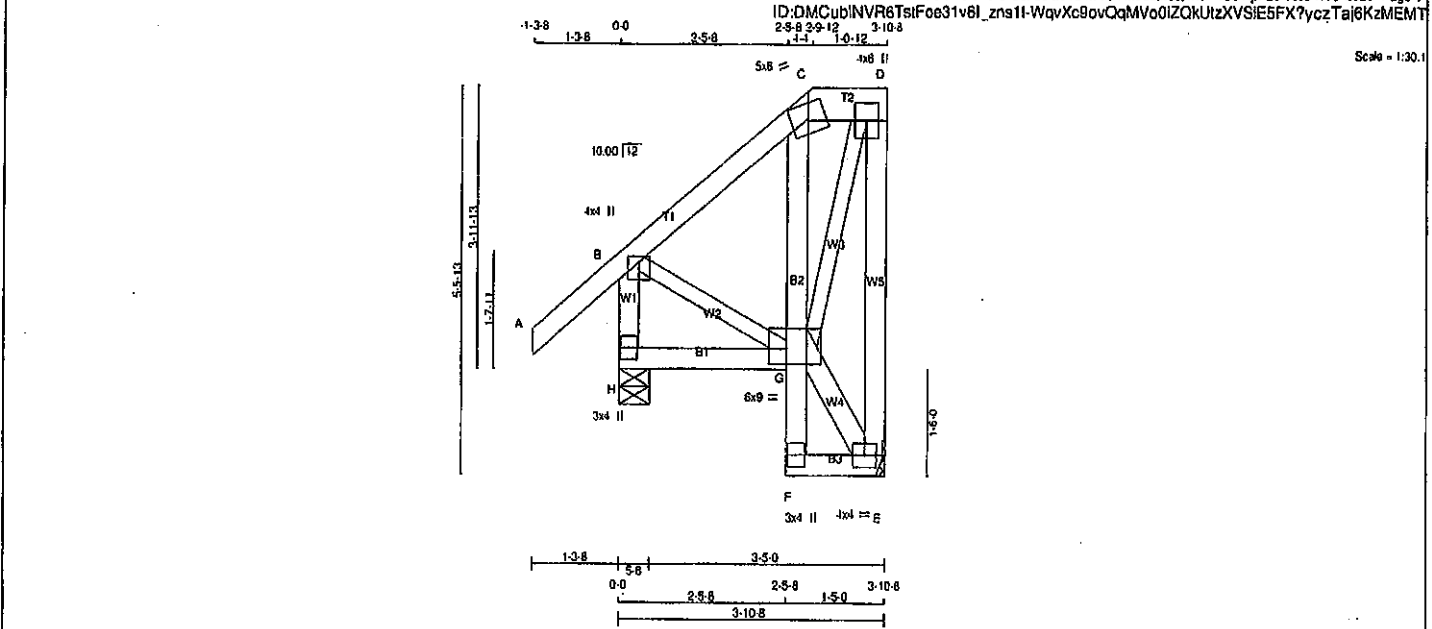


Structural component only
DWG# T-2007643

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T34S	2	1	GREEN PARK HOMES	

Tagmarack Roof Truss, Burlington

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Scale = 1:30.1

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
F - C	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF	
B - G	2x3	DRY	No.2	SPF	
G - D	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (tablets in inches)	JT TYPE	PLATES	W	LEN	Y	X
B - TMVW+p	MT20	4.0	4.0	1.00	2.00	
C - ITV-m	MT20	5.0	6.0			
D - TMVW+p	MT20	4.0	6.0			
E - BMVW-i	MT20	4.0	4.0			
F - BMV-p	MT20	3.0	4.0			
G - BMVWW-i	MT20	6.0	9.0	2.50	3.00	
H - BMV-i-p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
E	214 0	214 0	0	0
H	341 0	341 0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
E 151	99 0 0 0 0 0 62 0 0 0
H 239	170 0 0 0 0 0 89 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

SPACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MEMB.	MAX. FACTORED	FORCE (LBS)	VERT. LOAD	LC1	MAX	MAX	UNBRAC	LENGTH	FR-TO	MEMB.	MAX. FACTORED	FORCE (LBS)	MAX	CS1 (LC)
A-B	0	41	-91.8	-91.8	0.14	(5)	10.00	G-E	-6	0	0.00	(11)			
B-C	-62	0	-91.8	-91.8	0.12	(1)	6.25	B-G	0	53	0.01	(11)			
C-D	-51	0	-91.8	-91.8	0.01	(1)	6.25	G-D	0	154	0.03	(11)			
E-D	-197	0	0.0	0.0	0.10	(1)	7.81								
H-B	-317	0	0.0	0.0	0.03	(1)	7.81								
H-G	0	0	-18.5	-18.5	0.04	(4)	10.00								
F-G	0	13	0.0	0.0	0.01	(1)	10.00								
G-C	-138	0	0.0	0.0	0.01	(1)	7.81								
F-E	0	4	-18.5	-18.5	0.01	(4)	10.00								

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CS1: TC=0.14:1.00 (A-B:5), BC=0.04:1.00 (G-H:4), WB=0.03:1.00 (D-G:1), SS1=0.09:1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

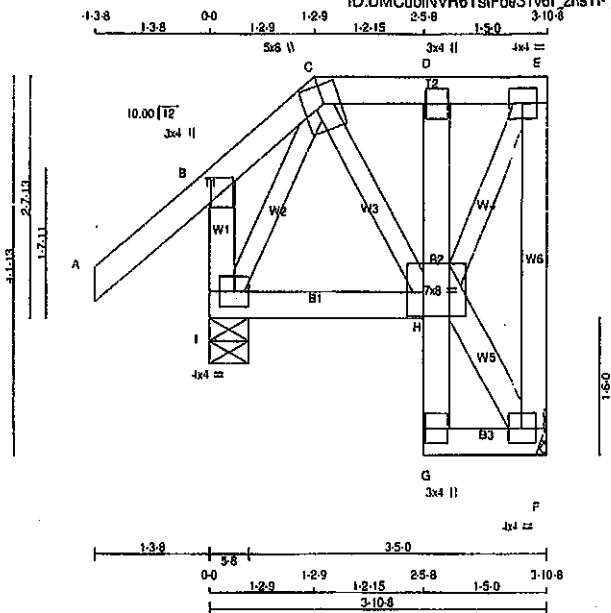
JS1 GRIP = 0.24 (8) (INPUT = 0.90)
JS1 METAL = 0.06 (8) (INPUT = 1.00)



Structural component only
DWG# T-2007644

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T35S	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR6TstFoe31v6l_zns1f_0TvpVoXBBLUQAHL_S76WidVdRQGSemC7KGenzMEMS



TOTAL WEIGHT = 2 X 29 = 57 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
I - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
G - D	2x4	DRY	No.2
G - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
H - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TTWW+m	MT20	6.0	6.0	2.50 2.25
D	TMV+p	MT20	3.0	4.0	
E	TMVW-i	MT20	4.0	4.0	
F	BMVW-i	MT20	4.0	4.0	
G	BMV+p	MT20	3.0	4.0	
H	BMVWW-i	MT20	7.0	8.0	3.25 2.25
I	BMVW-i	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG			
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	191	0	191	0	0	MECHANICAL	
I	363	0	363	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS							
JT	1ST LCASE COMBINED	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD
F	135	88	-13	0.0	0.0	0.0	49.0
I	254	163	0	0.0	0.0	0.0	72.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 41	-91.8 -91.8	0.14 (5)	10.00	H-F	-6 0	0.00 (1)
B-C	-45 0	-91.8 -91.8	0.14 (5)	6.25	H-E	0 141	0.03 (1)
C-D	-69 0	-91.8 -91.8	0.02 (1)	6.25	C-H	0 48	0.01 (1)
D-E	-87 0	-91.8 -91.8	0.02 (1)	6.25	I-C	-98 28	0.02 (1)
F-E	-175 0	0.0 0.0	0.05 (1)	7.81			
I-B	-255 0	0.0 0.0	0.03 (1)	7.81			
I-H	-13 45	-18.5 -18.5	0.04 (4)	6.25			
G-H	0 13	0.0 0.0	0.01 (1)	10.00			
H-D	-135 0	0.0 0.0	0.01 (1)	7.81			
G-F	0 4	-18.5 -18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (A-B:5), BC=0.04/1.00 (H-I:4),
WB=0.03/1.00 (E-H:1), SS=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1937 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

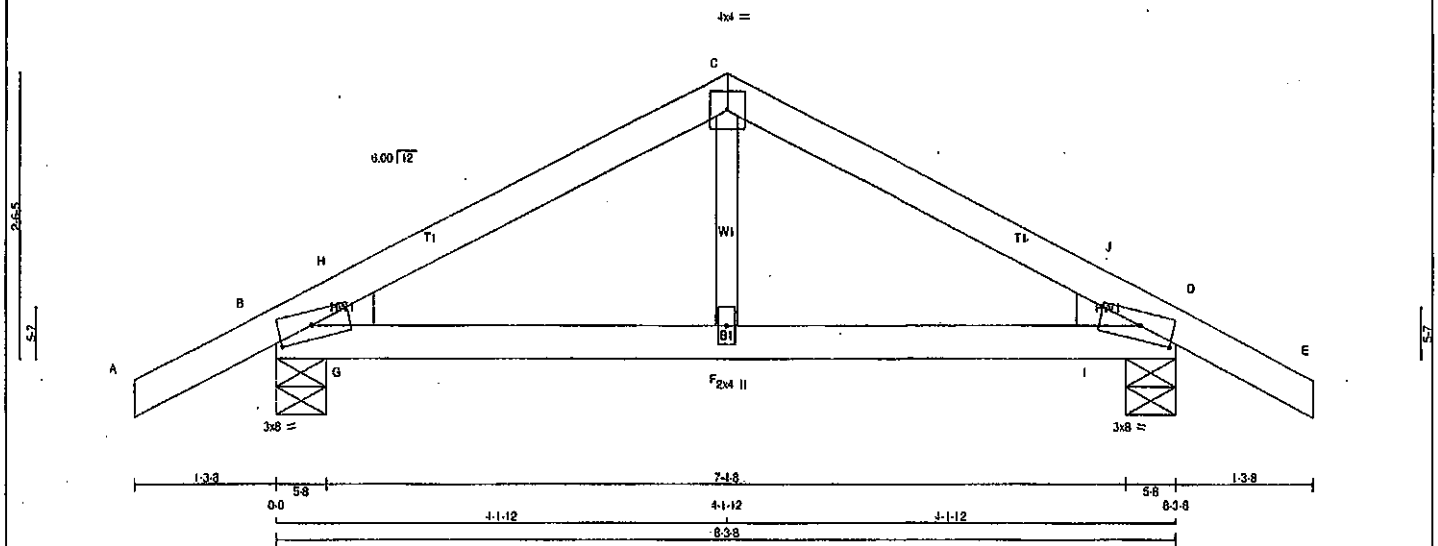
JSI GRIP= 0.18 (E) INPUT = 0.80
JSI METAL= 0.09 (B) INPUT = 1.00



Structural component only
DWG# T-2007645

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T36	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 ID:DMCubiNVR6T5Foe31v6J zns11-TC1H0rp9yRcD1KexY8WL2yba51j27vNvQn3q3DzMEMR
 Scale = 1:18.9



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - E	2x4	DRY	No.2	SPF		
B - D	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

PLATES (tablets in inches)	W	LEN	Y	X
JT TYPE PLATES				
g TMBH1-m	MT20	3.0	8.0	1.50 3.75
C TTW-p	MT20	4.0	4.0	
D TMBH1-m	MT20	3.0	8.0	1.50 3.75
F BMW-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT	RECORD	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
B	582	0	582	0	5-8	5-8
D	582	0	582	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX-MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
B	409 282 0 0 0 0 0 0 127 0 0 0
D	409 282 0 0 0 0 0 0 127 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: 14)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE
FR-TO	(LBS)	(PLF)	CSI (LC)	UNBRAC	LENGTH	FR-TO	(LBS)	(PLF)	CSI (LC)	UNBRAC	LENGTH	FR-TO	(LBS)
A-B	0 23	-91.8	-91.8	0.12 (1)	10.00	F-C	0 170	0.04 (1)					
B-H	-539 0	-91.8	-91.8	0.04 (1)	8.25	G-H	-158 3	0.00 (1)					
H-C	-525 0	-91.8	-91.8	0.15 (1)	6.25	I-J	-158 3	0.00 (1)					
C-J	-525 0	-91.8	-91.8	0.15 (1)	8.25								
J-D	-539 0	-91.8	-91.8	0.04 (1)	6.25								
D-E	0 23	-91.8	-91.8	0.12 (1)	10.00								
B-G	0 485	-18.5	-18.5	0.21 (1)	10.00								
G-F	0 485	-18.5	-18.5	0.21 (1)	10.00								
F-I	0 485	-18.5	-18.5	0.21 (1)	10.00								
I-D	0 485	-18.5	-18.5	0.21 (1)	10.00								

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. GC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, OBC 2012, ABC 2019
 - PART 9 OF OBC 2012 (2013 AMENDMENT)
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFLECTION (LL) = L/360 (0.28")
 CALCULATED VERT. DEFLECTION (LL) = L/999 (0.01")
 ALLOWABLE DEFLECTION (TL) = L/360 (0.28")
 CALCULATED VERT. DEFLECTION (TL) = L/999 (0.02")

CSI: TC=0.15/1.00 (C-J:1), BC=0.21/1.00 (F-I:1), WB=0.04/1.00 (C-F:1), SS=0.12/1.00 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

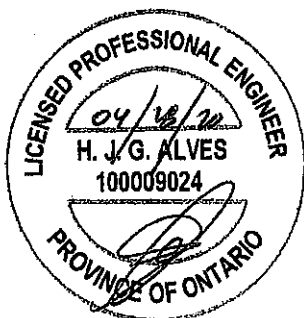
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION	(PSI)	(PLI)	(PLI)
MAX MIN MAX MIN MAX MIN			
MT20	618	354	1887 788 1987 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.25 (B) (INPUT = 0.90)
 JSI METAL = 0.11 (B) (INPUT = 1.00)

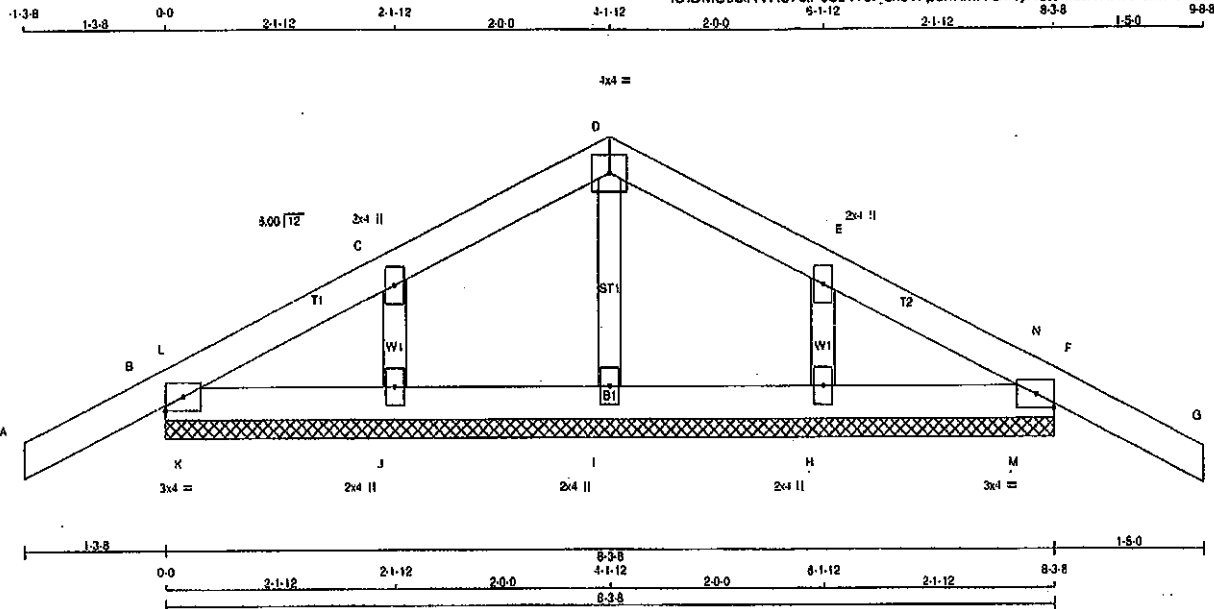


Structural component only
 DWG# T-2007646

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	G36	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:19.1

TOTAL WEIGHT = 27 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
B - F	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
ALL GABLE WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2'-0" OC.			

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y
B	TMG1-H	MT20	3.0	4.0	Edge
C	TMW-w	MT20	2.0	4.0	
D	TTW-w	MT20	4.0	4.0	
E	TMW-w	MT20	2.0	4.0	
F	TMG1-H	MT20	3.0	4.0	Edge
H, I, J	BMW1-w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
A-B	0 23	-91.8 -91.8 0.12 (1)	10.00	I-D	-115 0	0.02 (1)	
B-L	-61 0	-91.8 -91.8 0.01 (4)	6.25	J-C	-218 0	0.03 (1)	
L-C	-39 0	-91.8 -91.8 0.06 (1)	6.25	K-E	-218 0	0.03 (1)	
C-D	-45 0	-91.8 -91.8 0.06 (1)	6.25	K-L	-52.7	0.00 (1)	
D-E	-45 0	-91.8 -91.8 0.06 (1)	6.25	M-N	-52.7	0.00 (1)	
E-N	-39 0	-91.8 -91.8 0.06 (1)	6.25				
N-F	-61 0	-91.8 -91.8 0.01 (4)	6.25				
F-G	0 25	-91.8 -91.8 0.14 (1)	10.00				
B-K	0 44	-18.5 -18.5 0.03 (1)	10.00				
K-J	0 44	-18.5 -18.5 0.03 (1)	10.00				
J-I	0 32	-18.5 -18.5 0.02 (1)	10.00				
I-H	0 32	-18.5 -18.5 0.02 (1)	10.00				
H-M	0 44	-18.5 -18.5 0.03 (1)	10.00				
M-F	0 44	-18.5 -18.5 0.03 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2018, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14(1.00) (F-G:1) . SC=0.03(1.00) (H-M:1) . WB=0.03(1.00) (E-H:1) . SS=0.10(1.00) (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1967 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

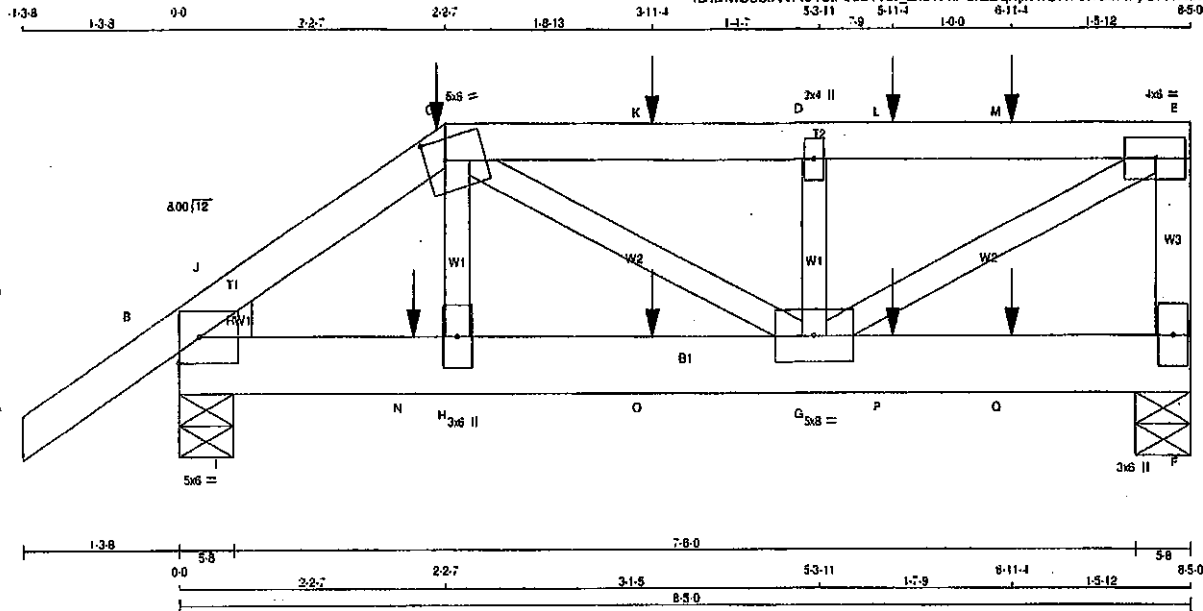
JSI GRIP = 0.21 (B) (INPUT = 0.90)
JSI METAL = 0.09 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007621

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T37	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR6TstFoe31v6l_zns1f-xPbfEBqnlk4fUR76t1abA7yUR47k172RpNjzMEMQ



Scale = 1:17.0

TOTAL WEIGHT = 37.0 (M)

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (Tables in inches)	W	LEN	Y	X
JT TYPE				
B TMSH1-1	MT20	5.0	6.0	Edge
C TTMW-m	MT20	5.0	6.0	2.00 2.00
D TTMW-w	MT20	2.0	4.0	
E TTMW-w	MT20	4.0	6.0	
F BMV1-p	MT20	3.0	6.0	
G BMVWW-1	MT20	5.0	6.0	
H BMV-w	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	760	0	760	0
B	823	0	823	0

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM LIVE WIND DEAD SOIL
F	537 352 0 0 0 0 0 0 185 0 0 0
B	580 392 0 0 0 0 0 0 188 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX.	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO		FR-TO	
A-B	0 26	-91.8 -91.8 0.13 (1)	10.00	H-C	0 73
B-J	-1068 0	-91.8 -91.8 0.08 (1)	5.99	C-G	0 252
J-C	-860 0	-91.8 -91.8 0.08 (1)	6.25	G-D	-528 0
C-K	-923 0	-91.8 -91.8 0.24 (1)	6.09	D-E	0 1062
K-D	-923 0	-91.8 -91.8 0.24 (1)	6.09	E-F	0 207
D-L	-924 0	-91.8 -91.8 0.24 (1)	6.09	F-J	0 207
L-M	-924 0	-91.8 -91.8 0.24 (1)	6.09		
M-E	-924 0	-91.8 -91.8 0.24 (1)	6.09		
F-E	-882 0	0.0 0.0 0.08 (1)	7.81		
B-I	0 701	-18.5 -18.5 0.14 (1)	10.00		
I-N	0 701	-18.5 -18.5 0.14 (1)	10.00		
N-H	0 701	-18.5 -18.5 0.14 (1)	10.00		
H-O	0 705	-18.5 -18.5 0.15 (1)	10.00		
O-G	0 705	-18.5 -18.5 0.15 (1)	10.00		
G-P	0 0	-18.5 -18.5 0.07 (1)	10.00		
P-Q	0 0	-18.5 -18.5 0.07 (1)	10.00		
Q-F	0 0	-18.5 -18.5 0.07 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	2-2.7	-88	-88	---	FRONT	VERT	TOTAL	---	C1
K	3-11-4	-83	-83	---	FRONT	VERT	TOTAL	---	C1
L	5-11-4	-83	-83	---	FRONT	VERT	TOTAL	---	C1
M	6-11-4	-83	-83	---	FRONT	VERT	TOTAL	---	C1
N	1-11-4	-48	-48	---	FRONT	VERT	TOTAL	---	C1
O	3-11-4	-48	-48	---	FRONT	VERT	TOTAL	---	C1
P	5-11-4	-48	-48	---	FRONT	VERT	TOTAL	---	C1
Q	6-11-4	-48	-48	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CS1: TC=0.24/1.00 (D-E:1), BC=0.15/1.00 (G-H:1),
WB=0.26/1.00 (E-G:1), SS=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1867 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

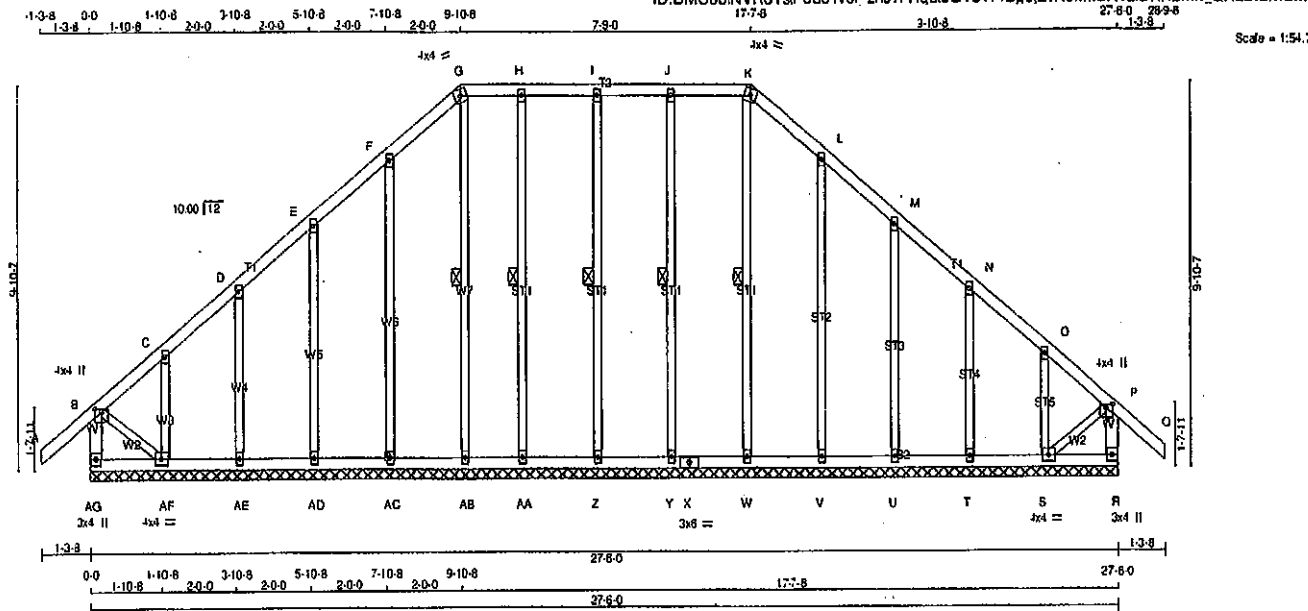
JSI GRIP = 0.61 (E) (INPUT = 0.90)
JSI METAL = 0.24 (E) (INPUT = 1.00)



Structural component only
DWG# T-2007647

JOB NAME 408223	TRUSS NAME G39	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR6TsfFos31v6l zns11-HqLi8GTbYr4Bq0iZvH0mw2R1aIDRRBmn_OHLBrzMEM
27-8-0 2998
Scale = 1:54.7



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
AG - B	2x4	DRY	No.2
A - G	2x4	DRY	No.2
G - K	2x4	DRY	No.2
K - Q	2x4	DRY	No.2
R - P	2x4	DRY	No.2
AG - X	2x4	DRY	No.2
X - R	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMW+p	MT20	4.0	4.0	1.00 2.00
C, D, E, F, H, I, J, L, M, N, O				
C TMW+w	MT20	2.0	4.0	
G TTW-m	MT20	4.0	4.0	
K TTW-m	MT20	4.0	4.0	
P TMW+p	MT20	4.0	4.0	1.00 2.00
R BMV1+p	MT20	3.0	4.0	
S BMW1+1	MT20	4.0	4.0	
T, U, V, W, Y, Z, AA, AB, AC, AD, AE				
T BMW1+w	MT20	2.0	4.0	
X BS+1	MT20	3.0	6.0	
AF BMW1+1	MT20	4.0	4.0	
AG BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF K-W, J-Y, I-Z, H-AA, G-AB.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO			FR-TO		
AG-B	-292 0	0.0 0.0 0.03 (1)	7.81	W-K	-140 0	0.09 (1)	
A-B	0 41	-91.8 -91.8 0.13 (1)	10.00	Y-J	-210 0	0.14 (1)	
B-C	-60 0	-91.8 -91.8 0.12 (1)	6.25	Z-I	-181 0	0.12 (1)	
C-D	-23 0	-91.8 -91.8 0.05 (1)	6.25	AA-H	-182 0	0.12 (1)	
D-E	-28 0	-91.8 -91.8 0.05 (1)	6.25	V-L	-203 0	0.27 (1)	
E-F	-19 0	-91.8 -91.8 0.05 (1)	6.25	U-M	-175 0	0.13 (1)	
F-G	-29 0	-91.8 -91.8 0.05 (1)	6.25	T-N	-192 0	0.07 (1)	
G-H	-12 0	-91.8 -91.8 0.04 (1)	6.25	S-O	-126 0	0.02 (1)	
H-I	-12 0	-91.8 -91.8 0.04 (1)	6.25	AB-G	-121 0	0.08 (1)	
I-J	-12 0	-91.8 -91.8 0.05 (1)	6.25	AC-F	-202 0	0.27 (1)	
J-K	-12 0	-91.8 -91.8 0.05 (1)	6.25	AD-E	-178 0	0.13 (1)	
K-L	-30 0	-91.8 -91.8 0.05 (1)	6.25	AE-D	-192 0	0.07 (1)	
L-M	-19 0	-91.8 -91.8 0.05 (1)	6.25	AF-C	-126 0	0.02 (1)	
M-N	-28 0	-91.8 -91.8 0.05 (1)	6.25	BAF	0 32	0.01 (1)	
N-O	-24 0	-91.8 -91.8 0.05 (1)	6.25	S-P	0 32	0.01 (1)	
O-P	-60 0	-91.8 -91.8 0.12 (1)	6.25				
P-Q	0 41	-91.8 -91.8 0.13 (1)	10.00				
R-P	-293 0	0.0 0.0 0.03 (1)	7.81				
AG-AF	0 0	-18.5 -18.5 0.02 (4)	10.00				
AF-AE	0 23	-18.5 -18.5 0.02 (4)	10.00				
AE-AD	0 19	-18.5 -18.5 0.02 (4)	10.00				
AD-AC	0 16	-18.5 -18.5 0.02 (4)	10.00				
AC-AB	0 14	-18.5 -18.5 0.02 (4)	10.00				
AB-AA	0 12	-18.5 -18.5 0.01 (4)	10.00				
AA-Z	0 12	-18.5 -18.5 0.02 (4)	10.00				
Z-Y	0 12	-18.5 -18.5 0.02 (4)	10.00				
Y-X	0 12	-18.5 -18.5 0.02 (4)	10.00				
X-W	0 12	-18.5 -18.5 0.02 (4)	10.00				
W-V	0 14	-18.5 -18.5 0.02 (4)	10.00				
V-U	0 16	-18.5 -18.5 0.02 (4)	10.00				
U-T	0 19	-18.5 -18.5 0.02 (4)	10.00				
T-S	0 23	-18.5 -18.5 0.02 (4)	10.00				
S-R	0 0	-18.5 -18.5 0.02 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 6.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.13/1.00 (P-O:1), BC=0.02/1.00 (S-T:4), WB=0.27 1.00 (L-V:1), SS1=0.09/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1887 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

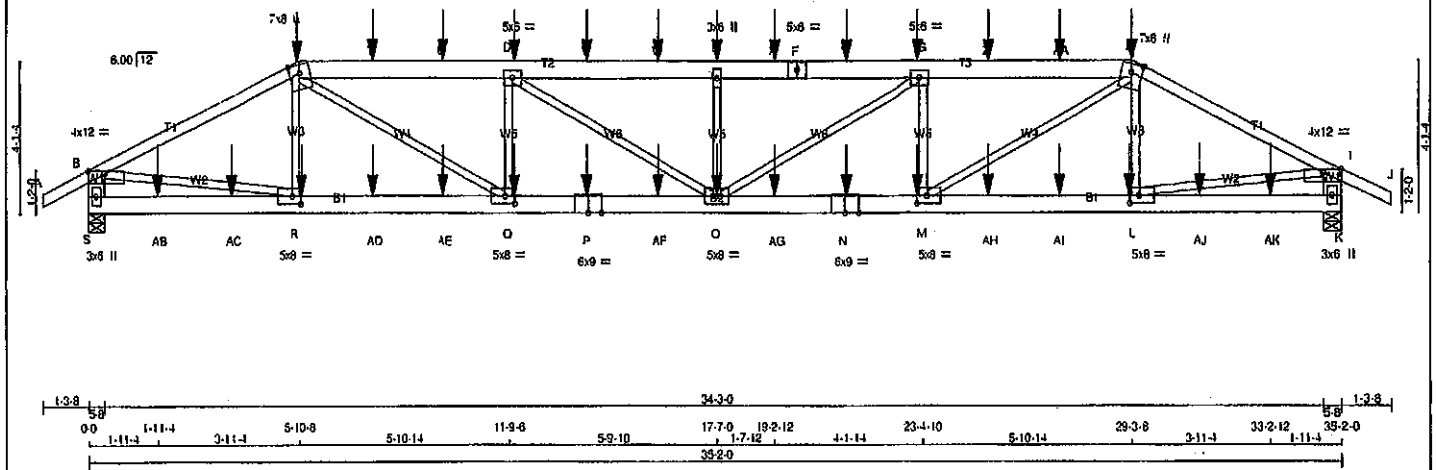
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.62 (K) INPUT = 0.80
JSI METAL = 0.11 (L) INPUT = 1.00



Structural component only
DWG# T-2007622

JOB NAME 408224	TRUSS NAME T40	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
Tsharak Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mtek Industries, Inc. Tue Apr 28 10:19:57 2020 Page 1		
ID:DMCubINVR6TstFoe3iv6l_zns11-TsGG5wA9pyw_Bd8P2isXcz_APYAIVa7LN10dlzME7m				35-2-0 38-5-8 1-3-8 1-3-8		
1-3-8 0-0 5-10-8 5-10-8 5-10-14 11-9-8 5-9-10 17-7-0 19-2-12 4-1-14 23-4-10 5-10-14 29-3-8 5-10-8				Scale = 1:57.6		



LUMBER

N. L. G. A. RULES

CHORDS SIZE LUMBER DESCR.

A - C 2x4 DRY No.2 SPF

C - F 2x6 DRY No.2 SPF

F - H 2x6 DRY No.2 SPF

H - J 2x4 DRY No.2 SPF

S - B 2x6 DRY No.2 SPF

K - I 2x6 DRY No.2 SPF

S - P 2x6 DRY 1650F 1.5E SPF

P - N 2x6 DRY 1650F 1.5E SPF

N - K 2x6 DRY 1650F 1.5E SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY; SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN)

LOAD(PLF)

TOP CHORDS : (0.122"x3") SPIRAL NAILS

A-C 1 12 SIDE(61.0)

H-J 1 12 SIDE(61.0)

C-F 2 12 SIDE(183.1)

F-H 2 12 SIDE(183.1)

S-B 2 12 TOP

K-I 2 12 TOP

BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS

S-P 2 12 SIDE(183.1)

P-N 2 12 SIDE(183.1)

N-K 2 12 SIDE(183.1)

WEBS : (0.122"x3") SPIRAL NAILS

2x3 1 6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2007660 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION GROSS REACTION INPUT REQD

JT VERT HORZ DOWN HORZ UPLIFT IN-SX BRG IN-SX BRG

S 3323 0 3323 0 0 5-8 5-8

K 3323 0 3323 0 0 5-8 5-8

UNFACTORED REACTIONS

1ST LCASE MAX. MIN. COMPONENT REACTIONS

JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL

S 2350 1539 0 0-0 0 0 0 811 0 0 0

K 2350 1539 0 0-0 0 0 0 811 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.

MAX. FACTORED FORCE (LBS)

FACTORED VERT. LOAD (PLF)

MAX. LIVE (LC1)

MAX. UNBRACED LENGTH (FT)

WEBS

MEMB.

MAX. FACTORED FORCE (LBS)

MAX. LIVE (LC1)

FR-TO

A-B 0 28 -91.8 -91.8 0.07 (1) 10.00

R-C -413 20 0.05 (1)

B-C -5066 0 -91.8 -91.8 0.52 (1) 3.87

C-Q 0 3193 0.40 (1)

C-T -7284 0 -91.8 -91.8 0.27 (1) 4.26

Q-D -1500 0 0.18 (1)

T-U -7284 0 -91.8 -91.8 0.27 (1) 4.26

D-O 0 946 0.12 (1)

U-D -7264 0 -91.8 -91.8 0.27 (1) 4.26

O-E -830 0 0.10 (1)

D-V -8084 0 -91.8 -91.8 0.27 (1) 4.08

E-G 0 948 0.12 (1)

V-W -8064 0 -91.8 -91.8 0.27 (1) 4.08

M-G -1500 0 0.18 (1)

W-E -8064 0 -91.8 -91.8 0.27 (1) 4.08

M-H 0 3193 0.40 (1)

E-X -8064 0 -91.8 -91.8 0.27 (1) 4.08

L-H -413 20 0.05 (1)

X-F -8084 0 -91.8 -91.8 0.27 (1) 4.08

B-R 0 4582 0.57 (1)

F-Y -8084 0 -91.8 -91.8 0.27 (1) 4.08

L-I 0 4582 0.57 (1)

Y-G -8084 0 -91.8 -91.8 0.27 (1) 4.08

G-Z -7284 0 -91.8 -91.8 0.27 (1) 4.26

Z-AA -7284 0 -91.8 -91.8 0.27 (1) 4.26

AA-H -7284 0 -91.8 -91.8 0.27 (1) 4.26

H-I -5066 0 -91.8 -91.8 0.52 (1) 3.87

I-J 0 28 -91.8 -91.8 0.07 (1) 10.00

S-B -3248 0 0.0 0.0 0.11 (1) 7.69

K-I -3248 0 0.0 0.0 0.11 (1) 7.69

S-AB 0 0 -18.5 -18.5 0.04 (4) 10.00

AB-AC 0 0 -18.5 -18.5 0.04 (4) 10.00

AC-R 0 0 -18.5 -18.5 0.04 (4) 10.00

R-AD 0 4553 -18.5 -18.5 0.22 (1) 10.00

AD-AE 0 4553 -18.5 -18.5 0.22 (1) 10.00

AE-O 0 4553 -18.5 -18.5 0.22 (1) 10.00

O-P 0 7283 -18.5 -18.5 0.34 (1) 10.00

P-AF 0 7283 -18.5 -18.5 0.34 (1) 10.00

AF-O 0 7283 -18.5 -18.5 0.34 (1) 10.00

O-AG 0 7283 -18.5 -18.5 0.34 (1) 10.00

AG-N 0 7283 -18.5 -18.5 0.34 (1) 10.00

N-M 0 7283 -18.5 -18.5 0.34 (1) 10.00

M-AH 0 4553 -18.5 -18.5 0.22 (1) 10.00

AH-AI 0 4553 -18.5 -18.5 0.22 (1) 10.00

AI-L 0 4553 -18.5 -18.5 0.22 (1) 10.00

L-AJ 0 0 -18.5 -18.5 0.04 (4) 10.00

AJ-AK 0 0 -18.5 -18.5 0.04 (4) 10.00

AK-K 0 0 -18.5 -18.5 0.04 (4) 10.00

FACTORED CONCENTRATED LOADS (LBS)

JT LOC. LC1 MAX. MAX+ FACE DIR. TYPE HEEL CONN.

C 5-10-8 -429 -429 --- FRONT VERT TOTAL --- C1

D 11-11-4 -110 -110 --- FRONT VERT TOTAL --- C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF

DL = 8.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

PART 9 OF NBCC 2010, OBC 2012, ABC 2019

PART 9 OF OBC 2012 (2019 AMENDMENT)

CSA 086-09, CSA 086-14

TPIC 2011, TPIC 2014

65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")

CALCULATED VERT. DEFL.(LL)= L/999 (0.21")

ALLOWABLE DEFL.(TL)= L/380 (1.17")

CALCULATED VERT. DEFL.(TL)= L/999 (0.39")

CSI: TC=0.52(1.00) (B-C:1), BC=0.34(1.00) (C-Q:1), WB=0.57(1.00) (B-R:1), SS=0.15(1.00) (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 818 354 1867 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (R) (INPUT = 0.90)

JSI METAL= 0.56 (P) (INPUT = 1.00)

CONTINUED ON PAGE



Structural component only
DWG# T-2007660 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	T40	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mtek Industries, Inc. Tue Apr 28 10:19:57 2020 Page 2	
				ID:DMCubINVR6tsiFoe31v6I_zns1I-TsGGSwA9pyw_Bu8P2lsXcz_APYAIVTa7LN10dIzME7m	

PLATES (table in inches)					FACTORED CONCENTRATED LOADS (LBS)											
JT	TYPE	PLATES	W	LEN	Y	X	JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	TMWV-p	MT20	4.0	12.0	1.00	5.50	E	17-7-0	-110	-110	---	FRONT	VERT	TOTAL	---	C1
C	TTWW+m	MT20	7.0	8.0	Edge		G	23-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
D	TMWV-t	MT20	5.0	6.0			H	29-3-8	-429	-429	---	FRONT	VERT	TOTAL	---	C1
E	TMW+u	MT20	3.0	6.0			L	29-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
F	TS-t	MT20	5.0	6.0			M	23-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
G	TMWV-t	MT20	5.0	6.0			N	21-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
H	TTWW+m	MT20	7.0	8.0	Edge		O	17-7-0	-28	-28	---	FRONT	VERT	TOTAL	---	C1
I	TMWV-p	MT20	4.0	12.0	1.00	5.50	P	13-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
K	BMV1+p	MT20	3.0	6.0			Q	11-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
L	BMWV-t	MT20	5.0	8.0	2.50	3.00	R	5-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
M	BMWV-t	MT20	5.0	8.0	2.50	3.25	T	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
N	BS-t	MT20	6.0	9.0			U	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
O	BMWV-t	MT20	5.0	8.0			V	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
P	BS-t	MT20	6.0	9.0			W	15-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Q	BMWV-t	MT20	5.0	8.0	2.50	3.00	X	19-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
R	BMWV-t	MT20	5.0	8.0	2.50	3.00	Y	21-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
S	BMV1+p	MT20	3.0	6.0			Z	25-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.							AA	27-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
							AB	1-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
							AC	3-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
							AD	7-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
							AE	9-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
							AF	15-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
							AG	19-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
							AH	25-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
							AI	27-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
							AJ	31-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
							AK	33-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

(1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2007660

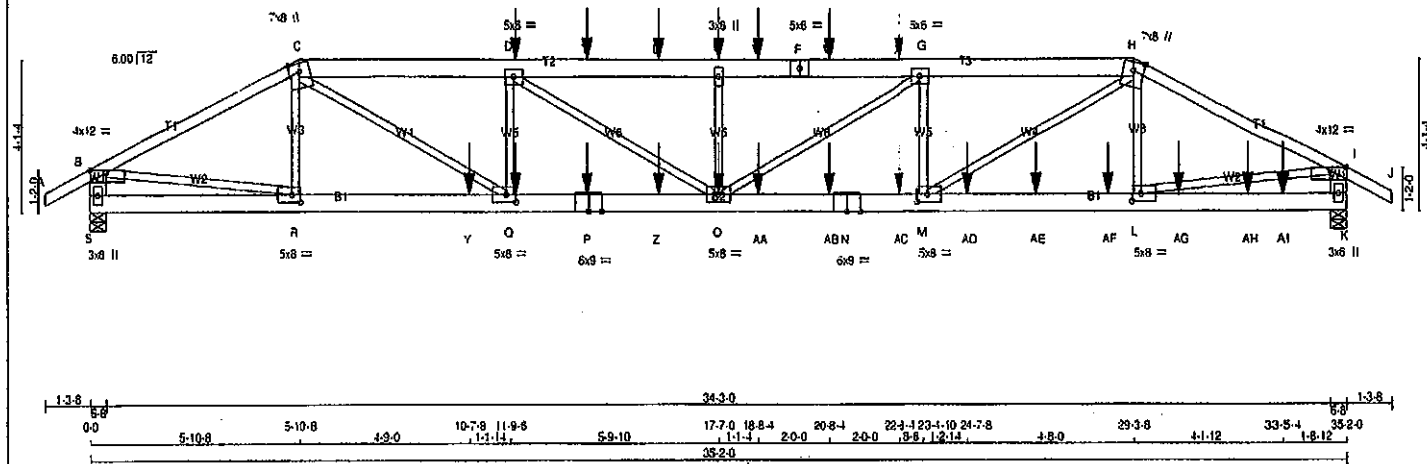
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	T40Z	1	2	GREEN PARK HOMES	
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TslFos31v6l zns1l-x2qeJG8oaF2rpnjbcam9BWHayQ8EpcGaImZakzME7l

1-3.8 0.0 5-10.8 5-10.8 5-10.14 11-9.4 5-9.10 17-7.0 18.8-4 20-8-4 22-4-123-4-10 29-3-8 35-2-0 36-5-8 1-3.8 35-2-0 1-3.8 Scale = 1:57.6



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x6	DRY	No.2	SPF	
F - H	2x6	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
S - B	2x6	DRY	No.2	SPF	
K - I	2x6	DRY	No.2	SPF	
S - P	2x8	DRY	1650F 1.5E	SPF	
P - N	2x8	DRY	1650F 1.5E	SPF	
N - K	2x8	DRY	1650F 1.5E	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
H-J	12	TOP
C-F	12	SIDE(183.1)
F-H	12	SIDE(0.0)
S-B	12	TOP
K-I	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S-P	12	SIDE(183.1)
P-N	12	SIDE(183.1)
N-K	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAIL HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
S	4161	0	4161	0
K	5042	0	5042	0

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL
S	2336 1965 0 0 0 0 0 970 0 0 0
K	3552 2409 0 0 0 0 0 1143 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS) S. K.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.95 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX (PLF)	MAX. UNBRAC (LC)	MEMB.	FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 28	-91.8 -91.8	0.07 (1)	10.00	R-C	-470 0	0.06 (1)
B-C	-8650 0	-91.8 -91.8	0.66 (1)	3.34	C-Q	0 5008	0.89 (1)
C-D	-10735 0	-91.8 -91.8	0.33 (1)	3.55	Q-D	-1678 0	0.20 (1)
D-T	-12119 0	-91.8 -91.8	0.40 (1)	3.30	D-O	0 1836	0.20 (1)
T-U	-12119 0	-91.8 -91.8	0.40 (1)	3.30	O-E	-1004 0	0.12 (1)
U-E	-12119 0	-91.8 -91.8	0.40 (1)	3.30	C-G	-46 33	0.02 (1)
E-V	-12119 0	-91.8 -91.8	0.45 (1)	3.23	M-H	0 5832	0.10 (1)
V-F	-12119 0	-91.8 -91.8	0.45 (1)	3.23	M-H	0 5832	0.10 (1)
F-W	-12119 0	-91.8 -91.8	0.45 (1)	3.23	L-H	0 57	0.01 (1)
W-X	-12119 0	-91.8 -91.8	0.45 (1)	3.23	B-R	0 8014	0.74 (1)
X-G	-12119 0	-91.8 -91.8	0.45 (1)	3.23	L-I	0 7274	0.90 (1)
G-H	-12159 0	-91.8 -91.8	0.39 (1)	3.30			
H-I	-8042 0	-91.8 -91.8	0.81 (1)	2.95			
I-J	0 28	-91.8 -91.8	0.07 (1)	10.00			
S-B	-4141 0	0.0 0.0	0.15 (1)	7.00			
K-I	-4926 0	0.0 0.0	0.17 (1)	8.55			

S-R	0 0	-18.5 -18.5 0.05 (1)	10.00
R-Y	0 5974	-18.5 -18.5 0.44 (1)	10.00
Y-Q	0 5974	-18.5 -18.5 0.44 (1)	10.00
Q-P	0 10734	-18.5 -18.5 0.54 (1)	10.00
P-Z	0 10734	-18.5 -18.5 0.54 (1)	10.00
Z-O	0 10734	-18.5 -18.5 0.54 (1)	10.00
O-AA	0 12159	-18.5 -18.5 0.71 (1)	10.00
AA-AB	0 12159	-18.5 -18.5 0.71 (1)	10.00
AB-N	0 12159	-18.5 -18.5 0.71 (1)	10.00
N-AC	0 12159	-18.5 -18.5 0.71 (1)	10.00
AC-M	0 12159	-18.5 -18.5 0.71 (1)	10.00
M-AD	0 7208	-18.5 -18.5 0.69 (1)	10.00
AD-AE	0 7208	-18.5 -18.5 0.69 (1)	10.00
AE-AF	0 7208	-18.5 -18.5 0.69 (1)	10.00
AF-L	0 7208	-18.5 -18.5 0.69 (1)	10.00
L-AG	0 0	-18.5 -18.5 0.16 (1)	10.00
AG-AH	0 0	-18.5 -18.5 0.16 (1)	10.00
AH-AI	0 0	-18.5 -18.5 0.16 (1)	10.00
AI-K	0 0	-18.5 -18.5 0.16 (1)	10.00

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
D	11-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
E	17-7-0	-110	-110	---	BACK	VERT	TOTAL	---	C1
O	17-7-0	-26	-26	---	BACK	VERT	TOTAL	---	C1
P	13-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-09, CSA 088-14
- TPIC 2011, TPIC 2014

55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.32")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/718 (0.59")

CSI: TC=0.61-1.00 (H=1.1), BC=0.71-1.00 (M=0.1), WB=0.90-1.00 (L=1.1), SSI=0.64-1.00 (L=M=1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE KEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (R) INPUT = 0.90)
JSI METAL = 0.93 (P) INPUT = 1.00)



Structural component only
DWG# T-2007661

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	T40Z	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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ID:DMCubINVR6TstFoa31v6l_zns11-x2qeJGBoaF2rpnbcNm9BWHeyO8EpcGa1mZakzME7

PLATES (table is in inches)	W	LEN	Y	X
B TMW-p MT20	4.0	12.0	1.00	5.50
C TTWW-m MT20	7.0	8.0	Edge	
D TMWW-l MT20	5.0	6.0		
E TMW-w MT20	3.0	6.0		
F TS-l MT20	5.0	6.0		
G TMWW-l MT20	5.0	6.0		
H TTWW-m MT20	7.0	8.0	Edge	
I TMW-p MT20	4.0	12.0	1.00	5.50
K BMV1-p MT20	3.0	6.0		
L BMWW-l MT20	5.0	6.0	2.50	3.00
M BMWW-l MT20	5.0	6.0	2.50	3.25
N BS-l MT20	6.0	9.0		
O BMWW-l MT20	5.0	6.0		
P BS-l MT20	6.0	9.0		
Q BMWW-l MT20	5.0	6.0	2.50	3.25
R BMWW-l MT20	5.0	6.0	2.50	3.00
S BMV1-p MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)	FACE	DIR.	TYPE	HEEL	CONN.
JT LOC. LG1 MAX+ MAX-					
Q 11-11-4 -28 -28	BACK	VERT	TOTAL	---	C1
T 13-11-4 -110 -110	BACK	VERT	TOTAL	---	C1
U 15-11-4 -110 -110	BACK	VERT	TOTAL	---	C1
V 18-8-4 -110 -110	BACK	VERT	TOTAL	---	C1
W 20-8-4 -159 -159	BACK	VERT	TOTAL	---	C1
X 22-8-4 -159 -159	BACK	VERT	TOTAL	---	C1
Y 10-7-8 -1171 -1171	BACK	VERT	TOTAL	---	C1
Z 15-11-4 -28 -28	BACK	VERT	TOTAL	---	C1
AA 18-8-4 -26 -26	BACK	VERT	TOTAL	---	C1
AB 20-8-4 -32 -32	BACK	VERT	TOTAL	---	C1
AC 22-8-4 -32 -32	BACK	VERT	TOTAL	---	C1
AD 24-7-8 -2237 -2237	BACK	VERT	TOTAL	---	C1
AE 28-6-12 -121 -121	BACK	VERT	TOTAL	---	C1
AF 28-6-12 -121 -121	BACK	VERT	TOTAL	---	C1
AG 30-6-12 -121 -121	BACK	VERT	TOTAL	---	C1
AH 32-6-12 -121 -121	BACK	VERT	TOTAL	---	C1
AI 33-5-4 -121 -121	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

11 C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



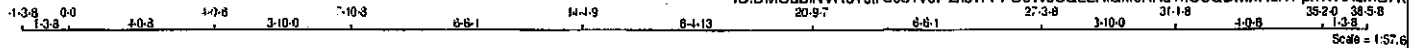
Structural component only
DWG# T-2007661

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408224	T41	2	1	GREEN PARK HOMES		

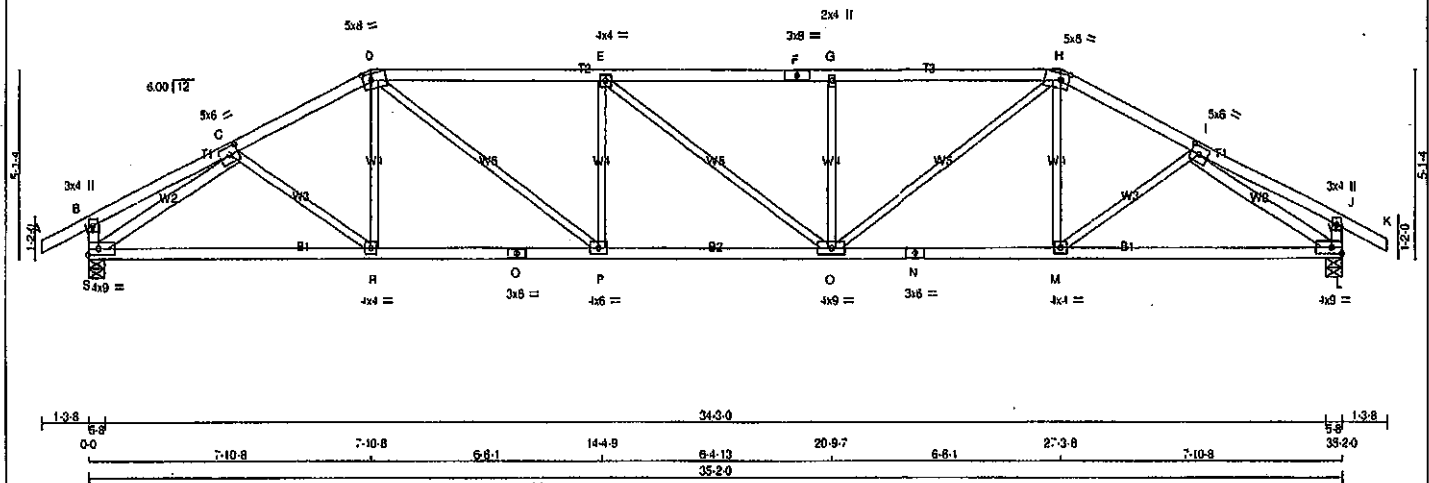
Tagharack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6I zns11-PFOOWCQLZAIQXloAHu7hO3QDMnRzi7PphW7AzME7k



Scale = 1:57.6



TOTAL WEIGHT = 2 X 139 = 278 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - O	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
S - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
	FACTORED	MAXIMUM FACTORED	INPUT	REQD					
	GROSS REACTION	GROSS REACTION	BRG	BRG					
JT	VERT	DOWN	HORIZ	UPLIFT	IN-SX				
S	2063	0	2063	0	5-8				
L	2063	0	2063	0	5-8				
UNFACTORED REACTIONS									
1ST CASE	MAX	MIN	COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
S	1457	989	0	0	0	0	0		
L	1457	989	0	0	0	0	0		
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS) S, L									

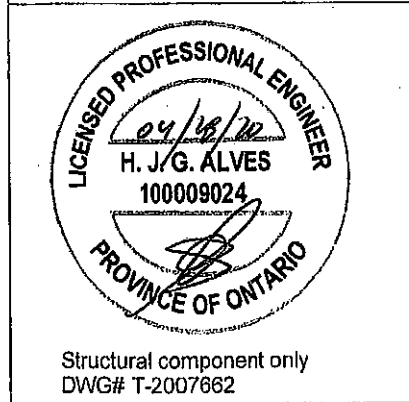
DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH.	LL = 25.6 PSF
DL	= 8.0 PSF
BOT CH.	LL = 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF
SPACING = 24.0 IN./C/C	
LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12	
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015	
THIS DESIGN COMPLIES WITH:	
- PART 9 OF CBC 2010, CBC 2012, ABC 2019	
- PART 9 OF CBC 2012 (2019 AMENDMENT)	
- CSA 086-09, CSA 086-14	
- TPIC 2011, TPIC 2014	

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y
B	TMV-p	MT20	3.0	4.0
C	TMWW-l	MT20	5.0	6.0
D	TTWW-m	MT20	5.0	8.0
E	TMWW-l	MT20	4.0	4.0
F	TS-l	MT20	3.0	8.0
G	TMWW-w	MT20	2.0	4.0
H	TTWW-m	MT20	5.0	8.0
I	TMWW-l	MT20	5.0	6.0
J	TMV-p	MT20	3.0	4.0
L	BMVW-l	MT20	4.0	9.0
M	BMVW-l	MT20	4.0	4.0
N	BS-l	MT20	3.0	6.0
O	BMWW-w	MT20	4.0	9.0
P	BMWW-l	MT20	4.0	6.0
Q	BS-l	MT20	3.0	6.0
R	BMWW-l	MT20	4.0	4.0
S	BMVW-l	MT20	4.0	9.0

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. CSI (LC)	UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO					FR-TO				
A-B	0 28	-91.8	-91.8	0.12 (1)	10.00	C-R	0 93	0.03 (4)	
B-C	0 16	-91.8	-91.8	0.20 (1)	10.00	R-D	0 121	0.04 (4)	
C-D	-2799 0	-91.8	-91.8	0.32 (1)	3.88	D-P	0 1267	0.29 (1)	
D-E	-3508 0	-91.8	-91.8	0.95 (1)	2.94	P-E	-643 0	0.25 (1)	
E-F	-3507 0	-91.8	-91.8	0.93 (1)	2.94	E-O	-2 0	0.00 (1)	
F-G	-3507 0	-91.8	-91.8	0.93 (1)	2.94	O-G	-643 0	0.25 (1)	
G-H	-3507 0	-91.8	-91.8	0.94 (1)	2.95	O-H	0 1268	0.28 (1)	
H-I	-2799 0	-91.8	-91.8	0.32 (1)	3.88	M-I	0 121	0.04 (4)	
I-J	0 16	-91.8	-91.8	0.20 (1)	10.00	M-I	0 93	0.03 (4)	
J-K	0 28	-91.8	-91.8	0.12 (1)	10.00	S-C	-2974 0	0.83 (1)	
S-B	-270 0	0.0	0.0	0.03 (1)	7.81	I-L	-2974 0	0.83 (1)	
L-J	-270 0	0.0	0.0	0.03 (1)	7.81				
S-R	0 2417	-18.5	-18.5	0.53 (1)	10.00				
R-O	0 2489	-18.5	-18.5	0.54 (1)	10.00				
Q-P	0 2489	-18.5	-18.5	0.54 (1)	10.00				
P-O	0 3509	-18.5	-18.5	0.84 (1)	10.00				
O-N	0 2489	-18.5	-18.5	0.54 (1)	10.00				
N-M	0 2489	-18.5	-18.5	0.54 (1)	10.00				
M-L	0 2417	-18.5	-18.5	0.53 (1)	10.00				

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD
ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.41")
CSI: TC=0.95, 1.00 (D-E:1), BC=0.64, 1.00 (O-P:1), WB=0.83, 1.00 (I-L:1), SSI=0.28, 1.00 (D-E:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00
AUTOSOLVE HEELS OFF

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 768 1987 1656
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP = 0.89 (I) (INPUT = 0.90)
JSI METAL = 0.77 (N) (INPUT = 1.00)

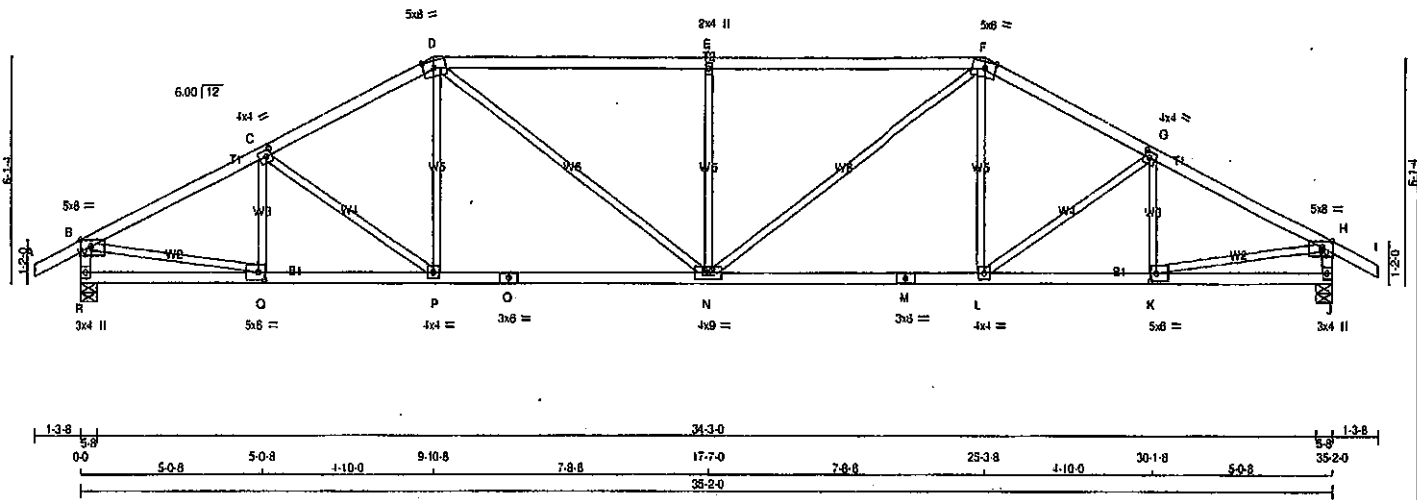
Structural component only
DWG# T-2007662

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ORWG NO.
408224	T42	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR8tSf0e31v6l_zns1f-LdWnxHEgIAQpGSAHixTm8U9V7RFJIG??En3zME7I

1-3-8 0-0 5-0-8 5-0-8 4-10-0 9-10-8 7-8-8 17-7-0 7-8-8 25-3-8 30-1-8 35-2-0 38-6-8 1-3-8
Scale = 1:57.6



TOTAL WEIGHT = 2 X 139 = 277 lb (M/F)

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
F - I	2x4 DRY	No.2	SPF		
R - B	2x4 DRY	No.2	SPF		
J - H	2x4 DRY	No.2	SPF		
R - O	2x4 DRY	No.2	SPF		
O - M	2x4 DRY	No.2	SPF		
M - J	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMVW-l	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	2.25	3.75
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	2.25	3.75
G	TMVW-l	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	5.0	8.0	Edge	3.50
J	BMV1-p	MT20	3.0	4.0		
K	BMVW-l	MT20	5.0	8.0	2.50	2.00
L	BMVW-l	MT20	4.0	4.0		
M	BS-l	MT20	3.0	8.0		
N	BMVW-l	MT20	4.0	4.0		
O	BS-l	MT20	3.0	8.0		
P	BMVW-l	MT20	4.0	4.0		
Q	BMVW-l	MT20	5.0	8.0	2.50	2.00
R	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT	DOWN	HORIZ	UPLIFT	IN-SX
R	2063	0	0	5-8
J	2063	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1457	969 0	0 0	0 0	0 0	488 0	0 0
J	1457	969 0	0 0	0 0	0 0	488 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 28	-91.8 -91.8 0.12 (1)	O-C	-353 0
B-C	-2834 0	-91.8 -91.8 0.39 (1)	C-P	210 0
C-D	-2687 0	-91.8 -91.8 0.37 (1)	P-D	0 246
D-E	-3060 0	-91.8 -91.8 0.37 (1)	D-N	0 844
E-F	-3060 0	-91.8 -91.8 0.37 (1)	N-E	-872 0
F-G	-2687 0	-91.8 -91.8 0.37 (1)	N-F	0 844
G-H	-2634 0	-91.8 -91.8 0.39 (1)	L-F	0 246
H-I	0 28	-91.8 -91.8 0.12 (1)	L-G	-210 0
I-B	-2018 0	0.0 0.0 0.20 (1)	K-G	-353 0
J-H	-2018 0	0.0 0.0 0.20 (1)	B-Q	0 2591
R-Q	0 0	-18.5 -18.5 0.10 (4)	K-H	0 2591
O-P	0 2555	-18.5 -18.5 0.51 (1)		
P-O	0 2388	-18.5 -18.5 0.49 (1)		
O-N	0 2388	-18.5 -18.5 0.49 (1)		
N-M	0 2388	-18.5 -18.5 0.49 (1)		
M-L	0 2386	-18.5 -18.5 0.49 (1)		
L-K	0 2555	-18.5 -18.5 0.51 (1)		
K-J	0 0	-18.5 -18.5 0.10 (4)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.34")

CSI: TC=0.97 1.00 (D-E:1) .8C=0.51 1.00 (P-Q:1).
WB=0.58/1.00 (B-Q:1) .SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1887 788 1887 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (F) INPUT = 0.90
JSI METAL = 0.75 (M) INPUT = 1.00



Structural component only
DWG# T-2007663

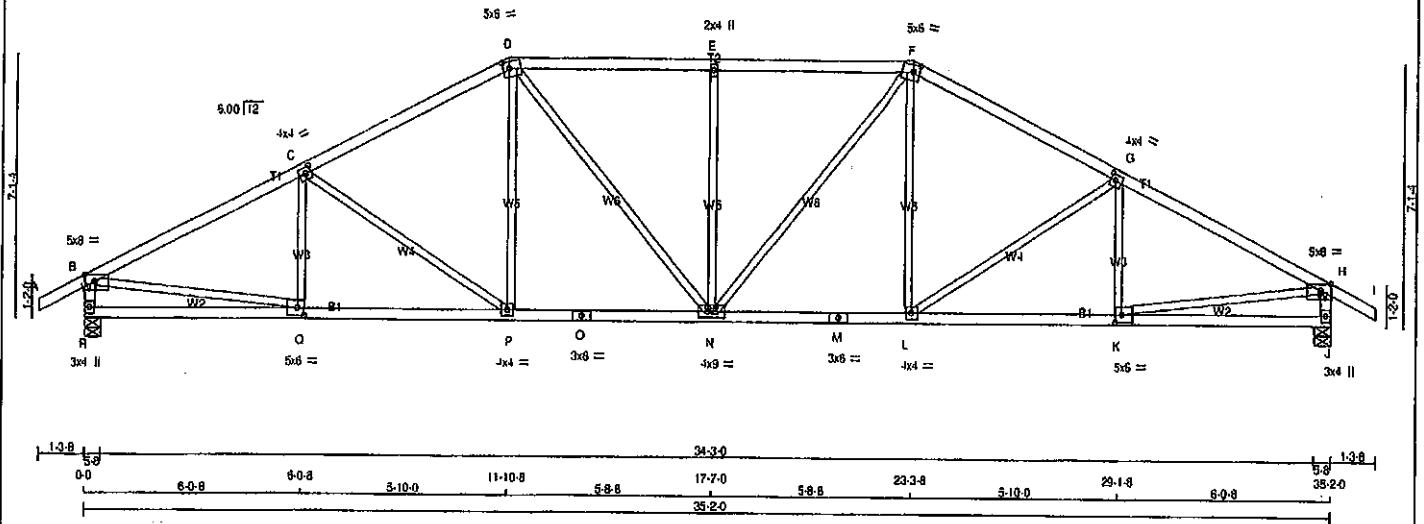
JOB NAME 408224	TRUSS NAME T43	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRAWG NO.
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Tamarack Roof Truss, Burlington

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ID:DMCubINVR6tstFoe31v6l_zns11-qp498dEleUYGHO1NqSIJh7BZrVAiQsV/knJVzME7h

1-3-8 0-0 1-3-8 6-0-8 6-0-8 5-10-0 11-10-8 5-8-8 17-7-0 5-8-8 23-3-8 5-10-0 29-1-8 6-0-8 35-2-0 1-3-8
Scale = 1:57.6



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2	
E - F	2x4	DRY	No.2	
G - H	2x4	DRY	No.2	
I - J	2x4	DRY	No.2	
K - L	2x4	DRY	No.2	
M - N	2x4	DRY	No.2	
O - P	2x4	DRY	No.2	
Q - R	2x4	DRY	No.2	
S - T	2x4	DRY	No.2	

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
A - B	TMWW-p	MT20	5.0	8.0	Edge	3.50
C - D	TMWW-i	MT20	4.0	4.0	2.00	1.75
E - F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G - H	TMWW-i	MT20	2.0	4.0		
I - J	TTWW-m	MT20	5.0	6.0	2.25	2.00
K - L	TMWW-i	MT20	4.0	4.0	2.00	1.75
M - N	TMWW-p	MT20	5.0	8.0	Edge	3.50
O - P	BMV1-p	MT20	3.0	4.0		
Q - R	BMV1-p	MT20	5.0	8.0	2.50	2.25
S - T	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
R	VERT	DOWN	UP	IN-SX
J	2063 0	2063 0	0 0	5-8 5-8

UNFACTORED REACTIONS

1ST CASE	MAX MIN COMPONENT REACTIONS	DEAD	SOIL
JT	COMBINED	SNOW	LIVE
R	1457 968 0	0 0	0 0
J	1457 968 0	0 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE (LBS)	VERT. LOAD	LC1	MAX	UNBRAC	LENGTH	FR-TO	MEMB.	MAX. FACTORED	FORCE (LBS)	MAX	UNBRAC	LENGTH	FR-TO
A-B	0	28	-91.8	-91.8	0.12 (1)	10.00	Q-C	255	12	0.07 (1)						
B-C	-2889	0	-91.8	-91.8	0.58 (1)	3.58	C-P	-435	0	0.42 (1)						
C-D	-2539	0	-91.8	-91.8	0.52 (1)	3.83	P-D	0	350	0.08 (1)						
D-E	-2559	0	-91.8	-91.8	0.48 (1)	3.85	D-N	0	482	0.11 (1)						
E-F	-2559	0	-91.8	-91.8	0.48 (1)	3.85	N-E	-641	0	0.56 (1)						
F-G	-2539	0	-91.8	-91.8	0.52 (1)	3.83	N-F	0	482	0.11 (1)						
G-H	-2889	0	-91.8	-91.8	0.58 (1)	3.58	L-F	0	350	0.08 (1)						
H-I	0	28	-91.8	-91.8	0.12 (1)	10.00	L-G	-435	0	0.42 (1)						
I-J	-2014	0	0.0	0.0	0.20 (1)	5.95	K-G	-265	12	0.07 (1)						
J-H	-2014	0	0.0	0.0	0.20 (1)	5.95	B-Q	0	2835	0.59 (1)						
R-Q	0	0	-18.5	-18.5	0.15 (4)	10.00	K-H	0	2835	0.59 (1)						
Q-P	0	2809	-18.5	-18.5	0.49 (1)	10.00										
P-O	0	2249	-18.5	-18.5	0.43 (1)	10.00										
O-N	0	2249	-18.5	-18.5	0.43 (1)	10.00										
N-M	0	2249	-18.5	-18.5	0.43 (1)	10.00										
M-L	0	2249	-18.5	-18.5	0.43 (1)	10.00										
L-K	0	2809	-18.5	-18.5	0.49 (1)	10.00										
K-J	0	0	-18.5	-18.5	0.15 (4)	10.00										

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BDT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2015
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

65% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/999 (0.15")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = L/999 (0.28")

CSI: TC=0.58/1.00 (G-H:1), BC=0.49/1.00 (K-L:1),
WB=0.59/1.00 (H-K:1), SS=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (C) (INPUT = 0.90)
JSI METAL = 0.89 (M) (INPUT = 1.00)



Structural component only
DWG# T-2007664

Document ID: A66091

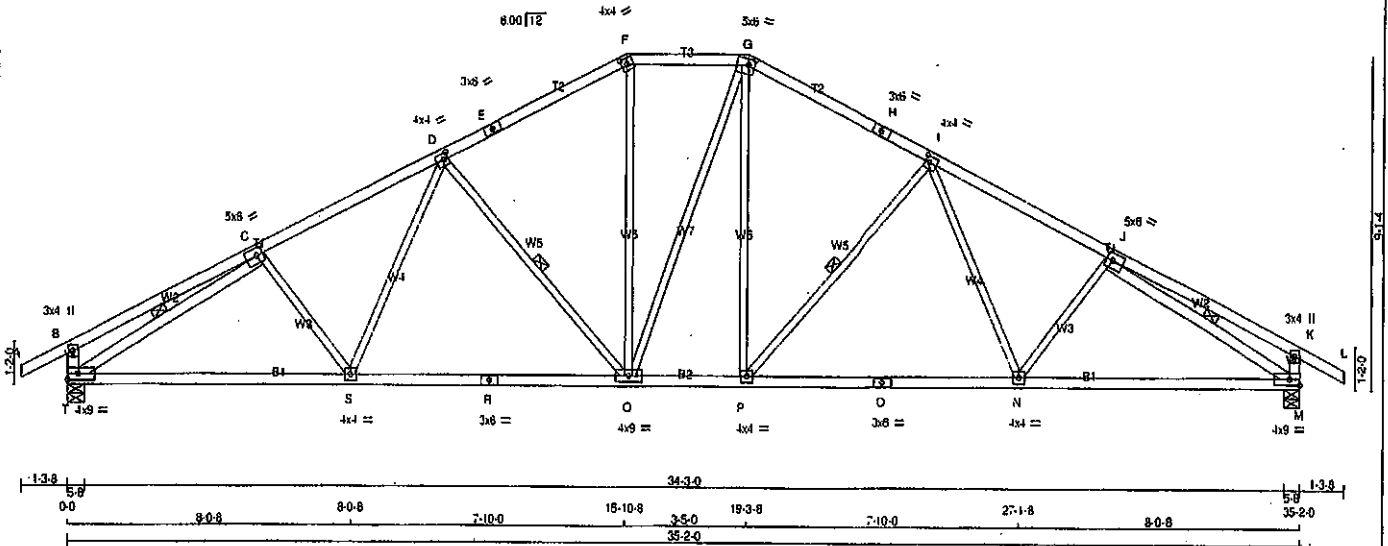
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	T45	2	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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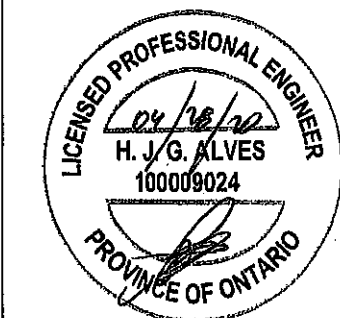
1-3.8 0-0 5-5.3 5-5.3 5-2.11 10-7.13 5-2.11 15-10.8 3-5.0 19-3.8 5-2.11 5-2.11 29-0.13 35-2.0 38-5.8 1-3.8
Scale = 1:58.3



TOTAL WEIGHT = 2 X 155 = 310 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - E	2x4	DRY	No.2
F - G	2x4	DRY	No.2
H - I	2x4	DRY	No.2
J - L	2x4	DRY	No.2
M - K	2x4	DRY	No.2
N - R	2x4	DRY	No.2
O - M	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
T - C	2x4	DRY	No.2
J - M	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN	Y
B TMV+p	MT20	3.0	4.0	
C TMWW-1	MT20	5.0	6.0	2.50
D TMWW-1	MT20	4.0	4.0	2.00
E TS-1	MT20	3.0	6.0	
F TTW-h	MT20	4.0	4.0	2.00
G TTWW-m	MT20	5.0	6.0	2.00
H TS-1	MT20	3.0	6.0	
I TMWW-1	MT20	4.0	4.0	2.00
J TMWW-1	MT20	5.0	6.0	2.50
K TMV+p	MT20	3.0	4.0	
M BMVW-1	MT20	4.0	9.0	Edge
N, P, S				
N BMWW-1	MT20	4.0	4.0	
O BS-1	MT20	3.0	6.0	
Q BMWW-1	MT20	4.0	9.0	
R BS-1	MT20	3.0	6.0	
T BMVW-1	MT20	4.0	9.0	Edge
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.				



Structural component only
DWG# T-2007666

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
T	2063	0	0	2063	0	0	0	5-8	5-8
M	2063	0	0	2063	0	0	0	5-8	5-8

UNFACTORED REACTIONS		1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
T	1457	989	0	0	0	0	0	488	0
M	1457	989	0	0	0	0	0	488	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D.Q. I.P. C.T. J.M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS (ILC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. CS (ILC)		
FR-TO					FR-TO				
A-B	0 28	-91.8	-91.8	0.12 (1)	10.00	C-S	-110 37	0.04 (1)	
B-C	0 20	-91.8	-91.8	0.32 (1)	10.00	S-D	0 278	0.08 (1)	
C-D	-2760 0	-91.8	-91.8	0.40 (1)	3.88	D-Q	-692 0	0.31 (1)	
D-E	-2167 0	-91.8	-91.8	0.36 (1)	4.30	Q-F	0 614	0.14 (1)	
E-F	-2167 0	-91.8	-91.8	0.36 (1)	4.30	Q-G	0 4	0.00 (1)	
F-G	-1925 0	-91.8	-91.8	0.18 (1)	4.71	P-G	0 609	0.14 (1)	
G-H	-2165 0	-91.8	-91.8	0.36 (1)	4.30	P-I	-683 0	0.31 (1)	
H-I	-2165 0	-91.8	-91.8	0.36 (1)	4.30	I-N	0 279	0.08 (1)	
I-J	-2761 0	-91.8	-91.8	0.40 (1)	3.88	N-J	-110 37	0.04 (1)	
J-K	0 20	-91.8	-91.8	0.32 (1)	10.00	T-C	-3042 0	0.81 (1)	
K-L	0 28	-91.8	-91.8	0.12 (1)	10.00	J-M	-3043 0	0.81 (1)	
T-B	-325 0	0.0	0.0	0.03 (1)	7.81				
M-K	-325 0	0.0	0.0	0.03 (1)	7.81				
T-S	0 2937	-18.5	-18.5	0.58 (1)	10.00				
S-R	0 2383	-18.5	-18.5	0.53 (1)	10.00				
R-O	0 2363	-18.5	-18.5	0.53 (1)	10.00				
Q-P	0 1924	-18.5	-18.5	0.40 (1)	10.00				
P-O	0 2362	-18.5	-18.5	0.53 (1)	10.00				
O-N	0 2392	-18.5	-18.5	0.53 (1)	10.00				
N-M	0 2538	-18.5	-18.5	0.58 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17')
CALCULATED VERT. DEFL.(LL) = L/999 (0.14')
ALLOWABLE DEFL.(TL) = L/360 (1.17')
CALCULATED VERT. DEFL.(TL) = L/999 (0.29')

CSI: TC=0.40/1.00 (I-J:1), BC=0.58/1.00 (M-N:1), WB=0.61/1.00 (J-M:1), SS=0.20/1.00 (G-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

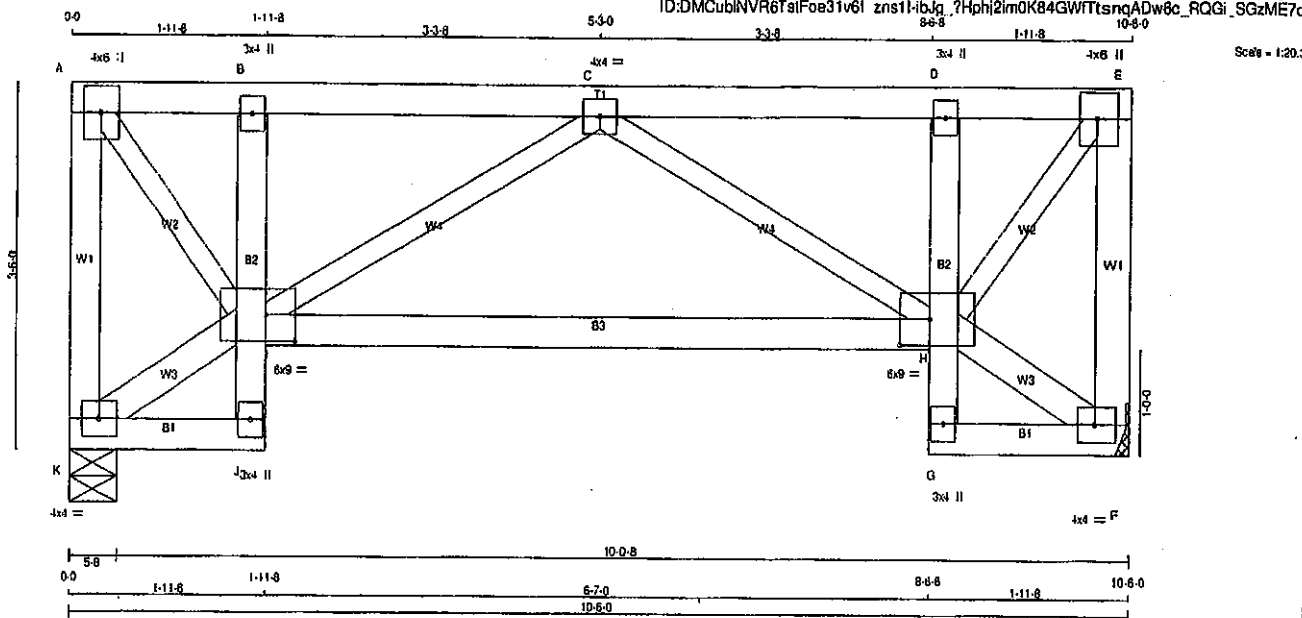
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (F) INPUT = 0.50)
JSI METAL = 0.83 (O) INPUT = 1.00)

JOB NAME 408224	TRUSS NAME T47S	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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Scale = 1:20.3

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
K - A	2x4	DRY	No.2
A - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
K - J	2x4	DRY	No.2
J - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
G - D	2x4	DRY	No.2
G - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
K - I	2x4	DRY	No.2
H - F	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (Table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW+p	MT20	4.0	6.0
B	TMV+p	MT20	3.0	4.0
C	TMWW-I	MT20	4.0	4.0
D	TMV+p	MT20	3.0	4.0
E	TMVW+p	MT20	4.0	6.0
F	BMVW1-I	MT20	4.0	4.0
G	BMV+p	MT20	3.0	4.0
H	BVMWW1-I	MT20	6.0	9.0 3.00 3.50
I	BVMWW1-I	MT20	6.0	9.0 3.00 3.50
J	BMV+p	MT20	3.0	4.0
K	BMVW1-I	MT20	4.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG	BRG
K	697	0	697	0	0	5.8	5.8		
F	697	0	697	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LOASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	504	289 0	0 0	0 0	0 0	235 0	0 0
F	504	289 0	0 0	0 0	0 0	235 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
K-A	-667 0	0.0 0.0 0.17 (1)	7.81	K-I	-28 0	0.00 (1)	
A-B	-497 0	-114.3 -114.3 0.14 (1)	8.25	A-I	0 783	0.24 (1)	
B-C	-508 0	-114.3 -114.3 0.22 (1)	8.25	H-F	-28 0	0.00 (1)	
C-D	-508 0	-114.3 -114.3 0.22 (1)	8.25	H-E	0 783	0.24 (1)	
D-E	-497 0	-114.3 -114.3 0.14 (1)	8.25	I-C	-384 0	0.15 (1)	
E-F	-667 0	0.0 0.0 0.17 (1)	7.81	C-H	-384 0	0.15 (1)	
K-J	0 25	-18.5 -18.5 0.03 (4)	10.00				
J-I	0 19	0.0 0.0 0.05 (1)	10.00				
I-B	-331 0	0.0 0.0 0.05 (1)	7.81				
H-H	0 892	-18.5 -18.5 0.48 (4)	10.00				
G-H	0 19	0.0 0.0 0.05 (1)	10.00				
H-D	-331 0	0.0 0.0 0.05 (1)	7.81				
G-F	0 25	-18.5 -18.5 0.03 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	48.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.35")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.35")
CALCULATED VERT. DEFL. (TL) = L/999 (0.12")

CSI: TC=0.22/1.00 (B-C:1), BC=0.46/1.00 (H-I:4), WB=0.24/1.00 (E-H:1), SS=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

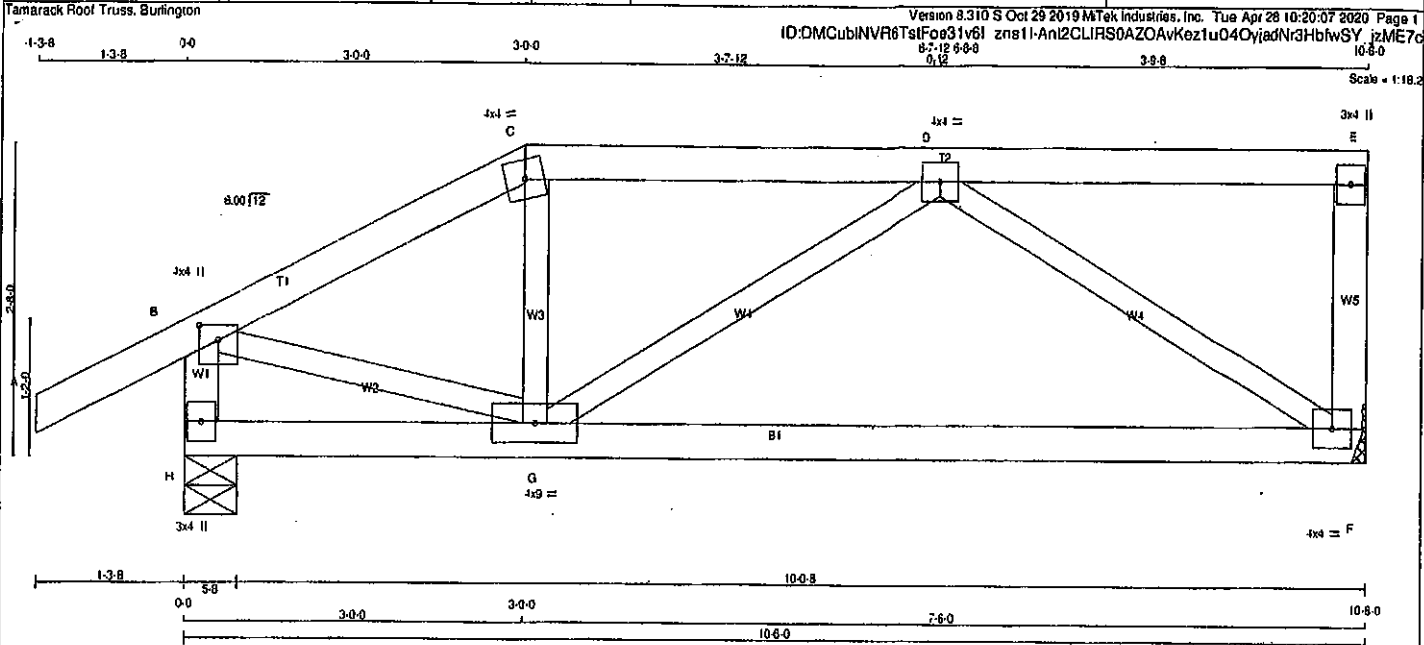
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (H) (INPUT = 0.90)
JSI METAL= 0.30 (E) (INPUT = 1.00)



Structural component only
DWG# T-2007668

JOB NAME 408224	TRUSS NAME T48	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.50 2.00
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	TMV+p	MT20	3.0	4.0	
F	BMVW1-t	MT20	4.0	4.0	
G	BMVWW-t	MT20	4.0	9.0	
H	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
F	579	0	579	0	0
H	703	0	703	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOL
F	409	289	0	0	0	141	0
H	495	338	0	0	0	157	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 28	-91.8 -91.8	0.12 (1)	10.00	G-C	0 61	0.02 (4)
B-C	-835 0	-91.8 -91.8	0.15 (1)	8.25	G-D	-81 60	0.02 (4)
C-D	-569 0	-91.8 -91.8	0.21 (1)	8.25	D-F	-731 0	0.23 (1)
D-E	0 0	-91.8 -91.8	0.20 (1)	10.00	B-G	0 587	0.13 (1)
F-E	-135 0	0.0 0.0	0.02 (1)	7.81			
H-B	-699 0	0.0 0.0	0.07 (1)	7.81			
H-G	0 0	-18.5 -18.5	0.21 (4)	10.00			
G-F	0 819	-18.5 -18.5	0.25 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.21-1.00 (C-D:1), BC=0.25-1.00 (F-G:4), WB=0.23-1.00 (D-F:1), SS=0.17-1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	818	354	1667
		788	1987
			1656

PLATE PLACEMENT TOL. = 0.250 inches

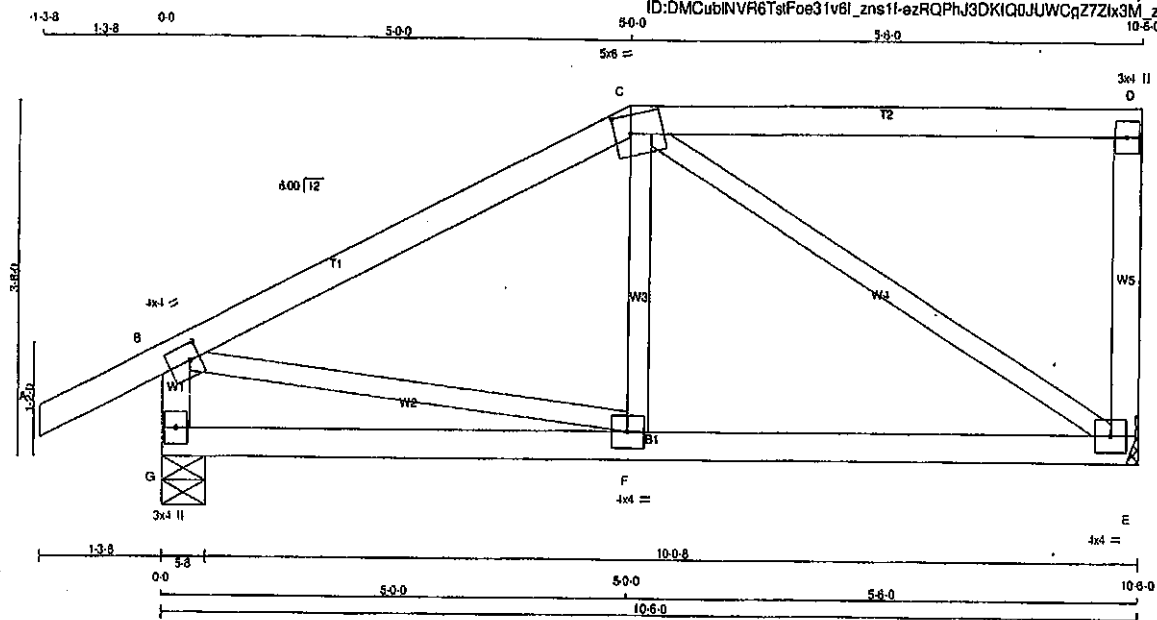
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.88 (B) INPUT = 0.90
JSI METAL=0.23 (F) INPUT = 1.00



Structural component only
DWG# T-2007669

JOB NAME 408224	TRUSS NAME T49	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



Scale = 1/2\"/>

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (tablets in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-1	MT20	4.0	4.0	2.00 1.25
C	TTWW-m	MT20	5.0	6.0	2.25 2.00
D	TMVW-p	MT20	3.0	4.0	
E	BMVW1-1	MT20	4.0	4.0	
F	BMVW1-1	MT20	4.0	4.0	
G	BMV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER					
BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
E	VERT 579	DOWN 579	0	0	MECHANICAL
G	HORIZ 703	UP 703	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS					
JT	1ST LOASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD
E	COMBINED 409	SNOW 269	0	0	0
G	495	338	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 28	-91.8	-91.8	0.12 (1)	10.00	F-C	0 106	0.04 (4)	
B-C	-523 0	-91.8	-91.8	0.41 (1)	6.25	C-E	-549 0	0.37 (1)	
C-D	0 0	-91.8	-91.8	0.47 (1)	10.00	B-F	0 474	0.11 (1)	
E-D	-252 0	0.0	0.0	0.05 (1)	7.81				
G-B	-667 0	0.0	0.0	0.07 (1)	7.81				
G-F	0 0	-18.5	-18.5	0.14 (4)	10.00				
F-E	0 468	-18.5	-18.5	0.19 (4)	10.00				

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH.	LL = 25.6 PSF
DL =	6.0 PSF
BOT CH.	LL = 0.0 PSF
DL =	7.4 PSF
TOTAL LOAD	= 39.0 PSF
SPACING	= 24.0 IN. CC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.35")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.35")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CSI: TC=0.47 1.00 (C-D:1), BC=0.19/1.00 (E-F:4), WB=0.37 1.00 (C-E:1), SS=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)
	MAX	MIN	MAX	MIN	MAX
MT20	818	354	1667	786	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.80)
JSI METAL= 0.28 (B) (INPUT = 1.00)

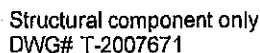


Structural component only
DWG# T-2007670

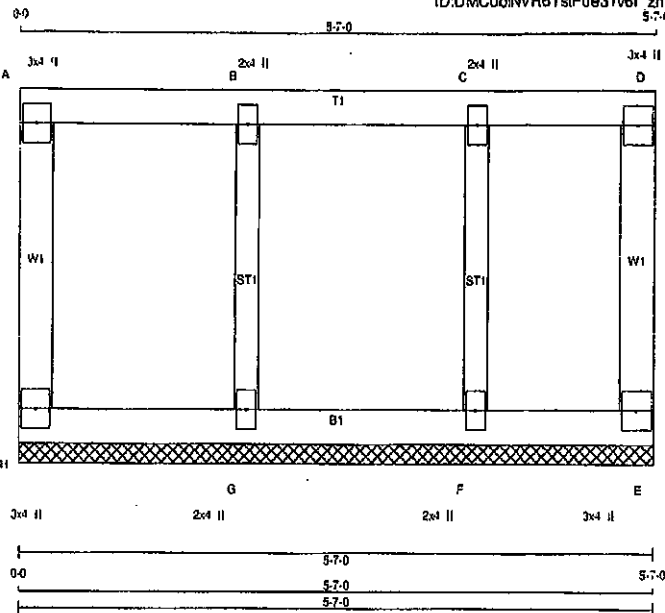
Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 10:20:10 2020 Page 1
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JSI GRIP = 0.25 (A) (INPUT = 0.90)
JSI METAL = 0.07 (A) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO
408224	G50	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 3.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:19:51 2020 Page 1	



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
H - A	2x4	DRY	No.2
A - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
H - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2'-0" O.C.			

PLATES (plates in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMV+p	MT20	3.0	4.0
B	TMW+w	MT20	2.0	4.0
C	TMV+p	MT20	2.0	4.0
D	TMW+w	MT20	2.0	4.0
E	BMV1+p	MT20	3.0	4.0
F	BMW1+w	MT20	2.0	4.0
G	BMV1+p	MT20	2.0	4.0
H	BMW1+w	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LG1 (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO				FR-TO			
H-A	-101 0	0.0	0.03 (1)	G-B	-248 0	0.08 (1)	
A-B	-8 0	-114.3	-114.3 0.08 (1)	F-C	-209 0	0.05 (1)	
B-C	-8 0	-114.3	-114.3 0.08 (1)				
C-D	-8 0	-114.3	-114.3 0.08 (1)				
E-D	-80 0	0.0	0.03 (1)				
H-G	0 8	-18.5	-18.5 0.02 (4)				
G-F	0 8	-18.5	-18.5 0.02 (4)				
F-E	0 8	-18.5	-18.5 0.02 (4)				

TOTAL WEIGHT = 22 k (MTR)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	15.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	48.0	PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (A-B:1), BC=0.02/1.00 (G-H:1), WB=0.08/1.00 (B-G:1), SSI=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1657 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.17 (G) INPUT = 0.90
JSI METAL = 0.07 (G) INPUT = 1.00



Structural component only
DWG# T-2007656



Structural component only
DWG# T-2007672



JSI GRIP= 0.65 (B) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

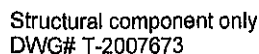
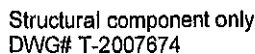




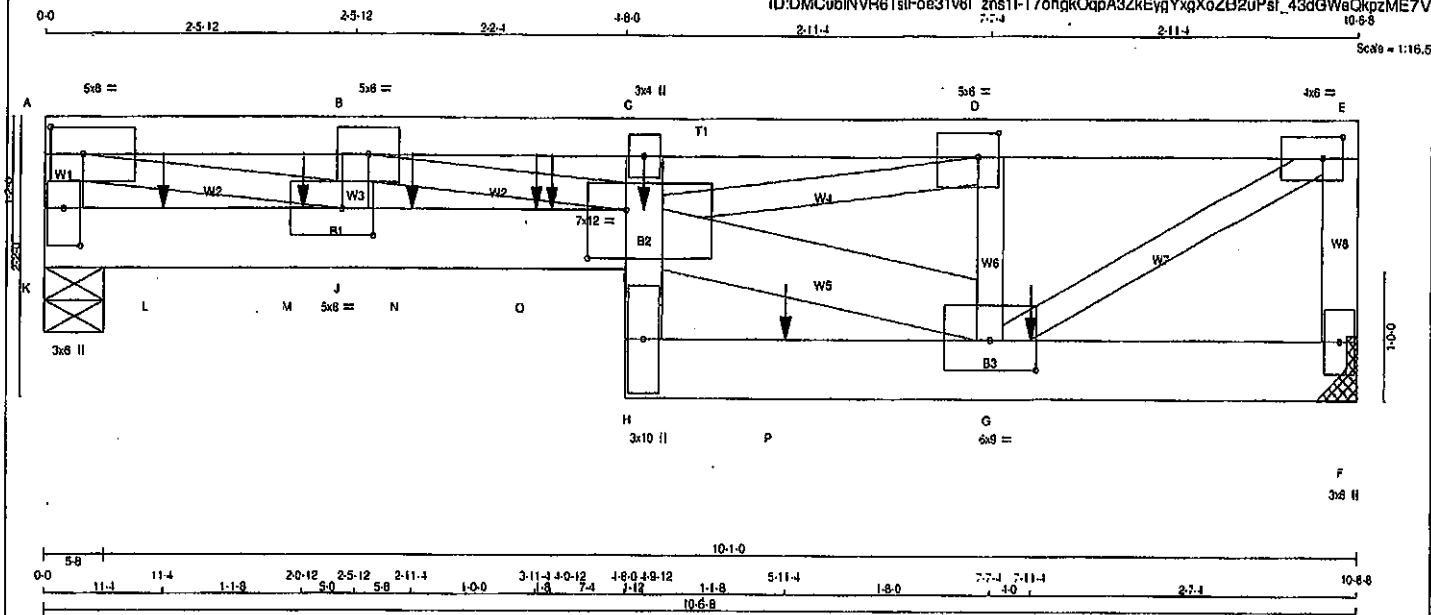
PLATE ROTATION TOL. = 5.0 Deg.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408224	T54S	1	2	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TsiFoe31v6I zns11-T7ohgkOqpA3ZkEygYxgXoZB2uPsf_43dGWaOkpzME7V



TOTAL WEIGHT = 2 X 49 = 98 lb (M)

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
K - A	2x4	DRY	No.2 SPF
A - E	2x4	DRY	No.2 SPF
F - E	2x4	DRY	No.2 SPF
K - I	2x6	DRY	No.2 SPF
H - C	2x4	DRY	No.2 SPF
H - F	2x6	DRY	No.2 SPF
ALL WEBS	2x3	DRY	No.2 SPF
EXCEPT			
I - G	2x6	DRY	No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

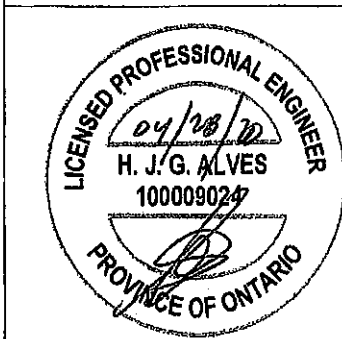
CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'X3') SPIRAL NAILS		
K-A	12	TOP
A-E	12	TOP
E-F	12	TOP
BOTTOM CHORDS : (0.122'X3') SPIRAL NAILS		
K-I	12	SIDE (81.0)
H-F	12	SIDE (189.1)
C-H	12	SIDE (89.1)
WEBS : (0.122'X3') SPIRAL NAILS		
2x3	6	
2x6	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.



Structural component only
DWG# T-2007675 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
K	2804	0	2804	0
F	2255	0	2255	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F MINIMUM BEARING LENGTH AT JOINT F = 4-0.

UNFACTORED REACTIONS

1ST LOASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED
K	1841 1212' 0
F	1592 1081' 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO	FROM	FR-TO	FROM
K-A	-2274 0	I-G	0 2898
A-B	-6894 0	I-D	0 5928
B-C	-10298 0	G-D	-2148 0
C-D	-9247 0	G-E	0 4043
D-E	-3418 0	J-B	-1577 0
F-E	-2242 0	A-J	0 7027
		B-I	0 3808
K-L	0 0		
L-M	0 0		
M-J	0 0		
J-N	0 6894		
N-O	0 6894		
O-I	0 6894		
H-I	0 354		
I-C	0 128		
H-P	0 832		
P-G	0 832		
G-F	0 0		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
G	7-11-4	-861	-661	---	BACK	VERT	TOTAL	C1
I	4-9-12	-577	-577	---	FRONT	VERT	TOTAL	C1
L	11-4	-72	-72	---	BACK	VERT	TOTAL	C1
M	2-0-12	-352	-352	---	FRONT	VERT	TOTAL	C1
N	2-11-4	-142	-142	---	BACK	VERT	TOTAL	C1
O	3-11-4	-883	-883	---	BACK	VERT	TOTAL	C1
P	4-0-12	-354	-354	---	FRONT	VERT	TOTAL	C1
O	5-11-4	-556	-556	---	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.35')
CALCULATED DEFL.(LL) = L/ 882 (0.15')
ALLOWABLE DEFL.(TL) = L/360 (0.35')
CALCULATED VERT. DEFL.(TL) = L/ 469 (0.27')

CS1: TC=0.66/1.00 (B-C:1), BC=0.79/1.00 (J-I:1), WB=0.87/1.00 (A-J:1), SS1=0.45/1.00 (C-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX	MIN	MAX
MT20	618	354
	1887	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (E) INPUT = 0.90
JSI METAL = 0.73 (D) INPUT = 1.00

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	T54S	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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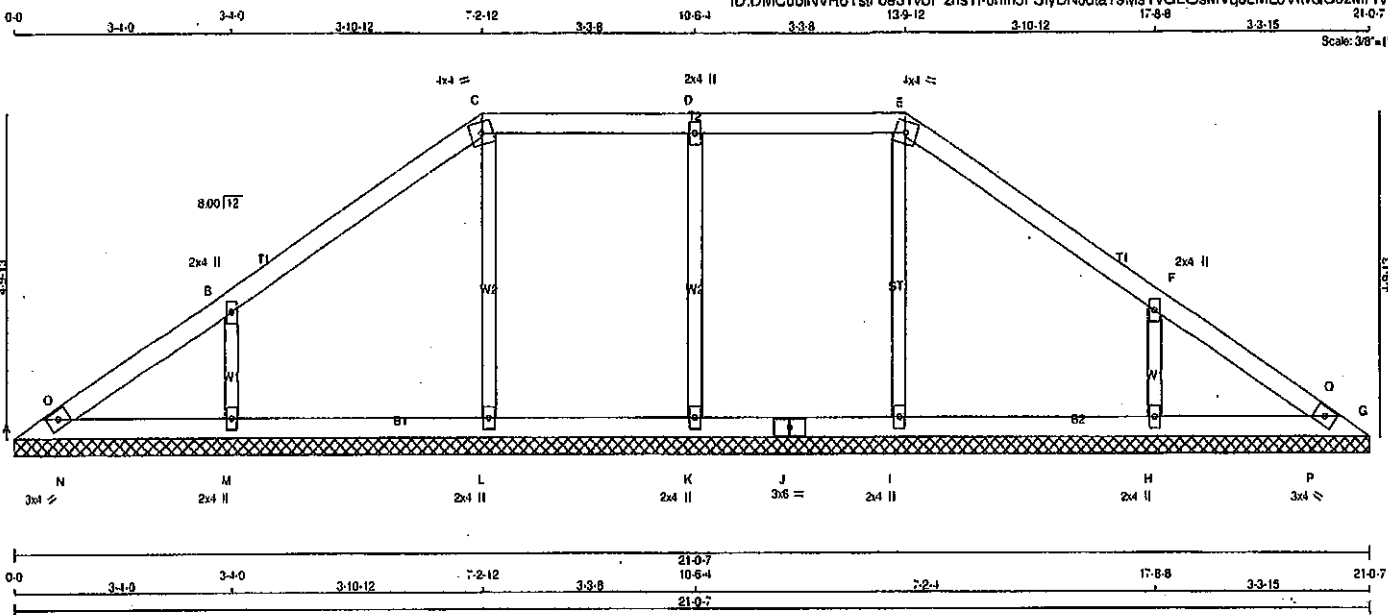
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-I	MT20	5.0	8.0	2.50	3.00
B	TMVW-I	MT20	5.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	TMVW-I	MT20	5.0	6.0	2.25	2.00
E	TMVW-I	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	3.0	6.0		
G	BMVWW-I	MT20	6.0	9.0	2.75	4.50
H	BMV+p	MT20	3.0	10.0		
I	BVMWWW-I	MT20	7.0	12.0	4.50	3.75
J	BMVW-I	MT20	5.0	8.0	2.50	3.00
K	BMV1+p	MT20	3.0	6.0	3.50	1.50



Structural component only
DWG# T-2007675 *HL*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	V1	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER		DESCR.	
N. L. G. A. RULES		SPF	
CHORDS			
A - C	2x4 DRY No.2	SPF	
C - E	2x4 DRY No.2	SPF	
E - G	2x4 DRY No.2	SPF	
A - J	2x4 DRY No.2	SPF	
J - G	2x4 DRY No.2	SPF	
ALL WEBS	2x3 DRY No.2	SPF	
DRY: SEASONED LUMBER.			

PLATES (table is in inches)			
JT TYPE	PLATES	W	LEN Y X
A TBM1-h	MT20	3.0	4.0
B, D, F			
B TMW+w	MT20	2.0	4.0
C TTW-m	MT20	4.0	4.0
E TTW-m	MT20	4.0	4.0
G TBM1-h	MT20	3.0	4.0
H, I, K, L, M			
H BMW1+w	MT20	2.0	4.0
J BS-I	MT20	3.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORIZ	GROSS REACTION	HORIZ	BRG	IN-SX	BRG	IN-SX
A	128	0	128	0	0	0	21-0-7 (11-11200-7	0	0
G	128	0	128	0	0	0	21-0-7 (11-11200-7	0	0
I	330	0	330	0	0	0	21-0-7 (11-11200-7	0	0
L	330	0	330	0	0	0	21-0-7 (11-11200-7	0	0
K	433	0	433	0	0	0	21-0-7 (11-11200-7	0	0
H	485	0	485	0	0	0	21-0-7 (11-11200-7	0	0
M	485	0	485	0	0	0	21-0-7 (11-11200-7	0	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
A	90	61	0
G	90	61	0
I	235	143	0
L	235	143	0
K	304	228	0
H	342	228	0
M	342	228	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G, I, L, K, H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-O	-23 2	I-E	-266 0
O-B	0 33	L-C	-266 0
B-C	-8 8	K-D	-377 0
C-D	0 17	H-F	-393 0
D-E	0 17	M-B	-393 0
E-F	-8 8	N-O	-39 5
F-G	0 33	P-Q	-39 5
Q-G	-23 2		
A-N	-9 0		
N-M	-7 7		
M-L	-11 0		
L-K	-17 0		
K-J	-17 0		
J-I	-17 0		
I-H	-11 0		
H-P	-7 7		
P-G	-9 0		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21 1.00 (B-O:1), BC=0.06 1.00 (L-M:4), WB=0.13 1.00 (D-K:1), SSI=0.15 1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

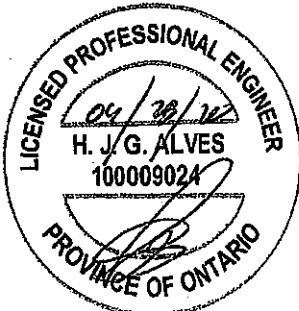
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) INPUT = 0.90
JSI METAL= 0.20 (F) INPUT = 1.00



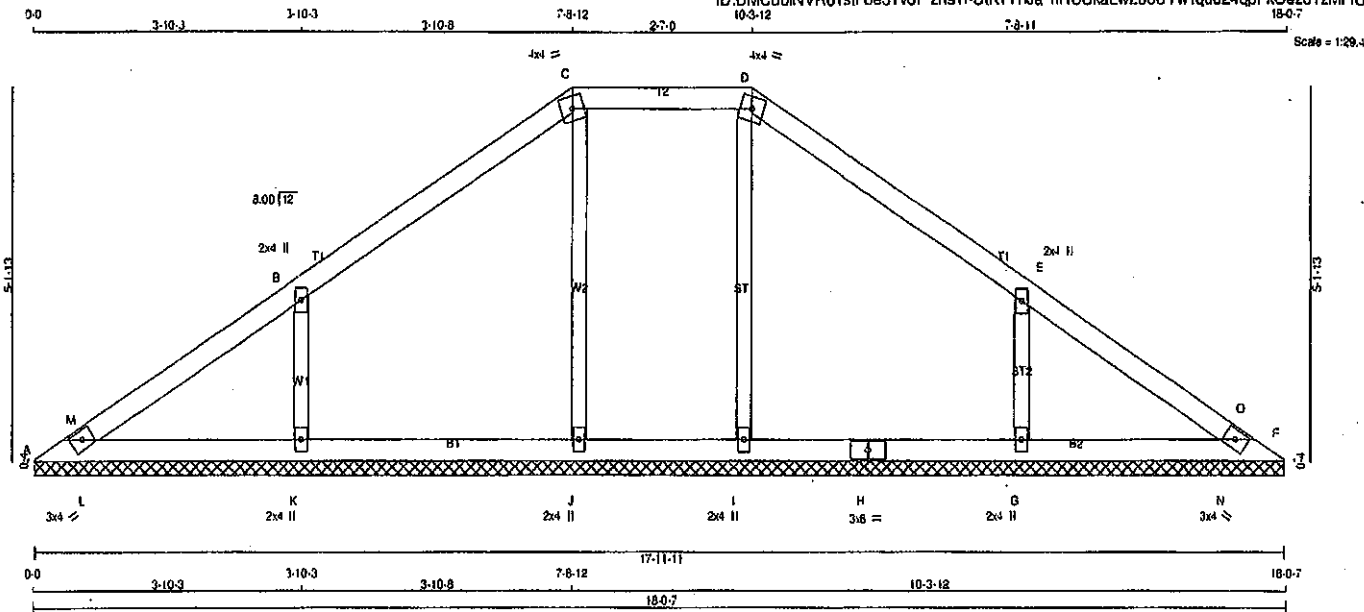
Structural component only
DWG# T-2007616

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408222	V2	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TsFoe31v6I zns1I-UUKTTnJa hRUUKaLwZ38oYw1qu824qpFkOzoTzMFU



TOTAL WEIGHT = 54 lb

LUMBER	SIZE	LUMBER	DESCR.
N. L. G. A. RULES			
CHORDS			
A - C	2x4 DRY	No.2	SPF
C - D	2x4 DRY	No.2	SPF
D - F	2x4 DRY	No.2	SPF
A - H	2x4 DRY	No.2	SPF
H - F	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (tablets in inches)	W	LEN	Y	X
JT TYPE				
A TBM1-h	MT20	3.0	4.0	
B TMW-w	MT20	2.0	4.0	
C TTW-m	MT20	4.0	4.0	
D TTW-m	MT20	4.0	4.0	
E TMW-w	MT20	2.0	4.0	
F TBM1-h	MT20	3.0	4.0	
G, I, J, K				
G BMW1-w	MT20	2.0	4.0	
H BS-t	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
A	136	0	136	0	17-11-11 (11-17-12-11)	
F	135	0	135	0	17-11-11 (11-17-12-11)	
I	344	0	344	0	17-11-11 (11-17-12-11)	
G	513	0	513	0	17-11-11 (11-17-12-11)	
J	342	0	342	0	17-11-11 (11-17-12-11)	
K	513	0	513	0	17-11-11 (11-17-12-11)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
A	98	65	0.0	0	0	30	0	
F	95	65	0.0	0	0	30	0	
I	244	155	0	0	0	89	0	
G	362	240	0.0	0	0	122	0	
J	242	154	0	0	0	88	0	
K	362	240	0	0	0	122	0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS A, F, I, G, J, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX CS (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
A-M	0 20	-91.8	-91.8 0.04 (1)	10.00	I-D	-292 0	0.11 (1)		
M-B	0 71	-91.8	-91.8 0.22 (1)	10.00	G-E	-408 0	0.07 (1)		
B-C	0 33	-91.8	-91.8 0.22 (1)	10.00	J-C	-291 0	0.11 (1)		
C-D	0 52	-91.8	-91.8 0.11 (1)	10.00	K-B	-408 0	0.07 (1)		
D-E	0 33	-91.8	-91.8 0.22 (1)	10.00	L-M	-71 4	0.00 (1)		
E-O	0 71	-91.8	-91.8 0.22 (1)	10.00	N-O	-69 5	0.00 (1)		
O-F	0 19	-91.8	-91.8 0.04 (1)	10.00					
A-L	-43 0	-18.5	-18.5 0.07 (1)	6.25					
L-K	-30 0	-18.5	-18.5 0.07 (1)	6.25					
K-J	-45 0	-18.5	-18.5 0.07 (4)	6.25					
J-I	-52 0	-18.5	-18.5 0.04 (4)	6.25					
I-H	-45 0	-18.5	-18.5 0.07 (4)	6.25					
H-G	-45 0	-18.5	-18.5 0.07 (4)	6.25					
G-N	-30 0	-18.5	-18.5 0.07 (1)	6.25					
N-F	-43 0	-18.5	-18.5 0.07 (1)	6.25					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22/1.00 (E-O:1), BC=0.07/1.00 (K-L:1), WB=0.11/1.00 (D-I:1), SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

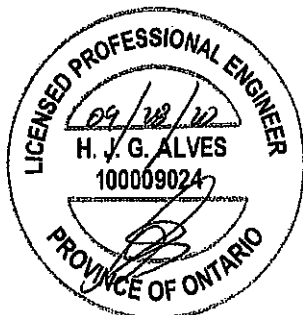
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

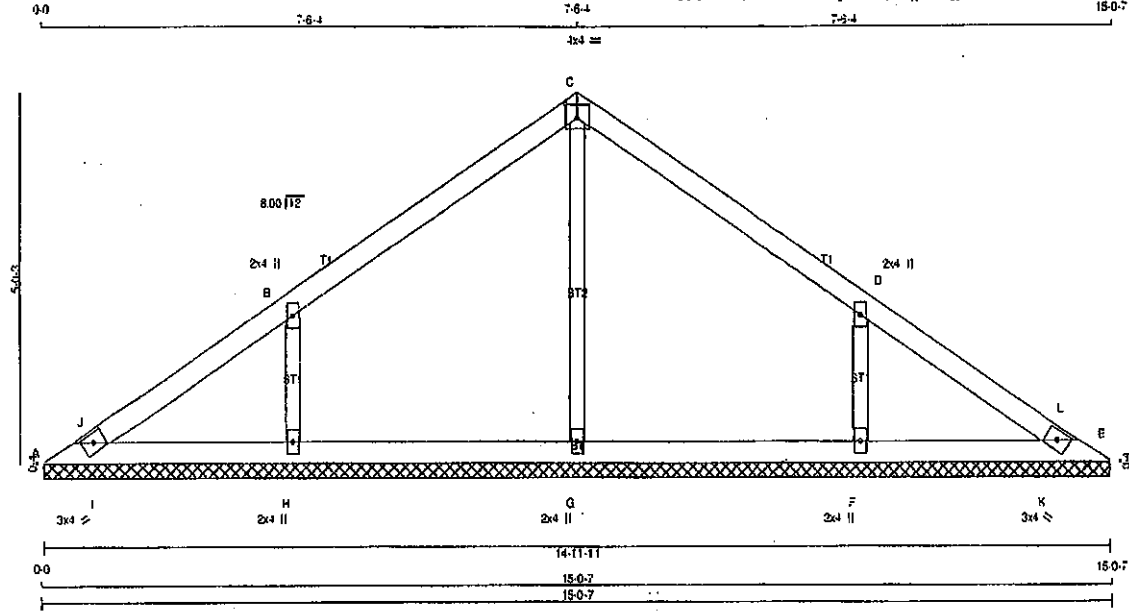
JSI GRIP = 0.81 (C) (INPUT = 0.90)
JSI METAL = 0.21 (E) (INPUT = 1.00)



Structural component only
DWG# T-2007617

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	V3	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/28.8

TOTAL WEIGHT = 43 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
A - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW+w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMW+w	MT20	2.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F, G, H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
A	134	0	134	0
E	134	0	134	0
G	381	0	381	0
H	501	0	501	0
F	501	0	501	0

UNFACTORED REACTIONS	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED						
A	95	65	0	0	0	30	0
E	95	65	0	0	0	30	0
G	272	162	0	0	0	110	0
H	353	237	0	0	0	118	0
F	353	237	0	0	0	118	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS) A, E, G, H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO		
A-J	-20 4	-91.8 -91.8	0.04 (1)	6.25	G-O	-316 0	0.11 (1)
J-B	0 38	-91.8 -91.8	0.22 (1)	10.00	H-B	-405 0	0.08 (1)
B-C	-5 12	-91.8 -91.8	0.21 (1)	10.00	F-D	-408 0	0.08 (1)
C-D	-5 12	-91.8 -91.8	0.21 (1)	10.00	I-J	-46 8	0.00 (1)
D-L	0 36	-91.8 -91.8	0.22 (1)	10.00	K-L	-46 8	0.00 (1)
L-E	-20 4	-91.8 -91.8	0.04 (1)	6.25			
A-I	-13 0	-18.5 -18.5	0.06 (1)	6.25			
I-H	-10 3	-18.5 -18.5	0.06 (4)	6.25			
H-G	-13 0	-18.5 -18.5	0.06 (4)	6.25			
G-F	-13 0	-18.5 -18.5	0.06 (4)	6.25			
F-K	-10 3	-18.5 -18.5	0.06 (4)	6.25			
K-E	-13 0	-18.5 -18.5	0.06 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
OL = 6.0 PSF
BOT CH. LL = 0.0 PSF
OL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22/1.00 (B-J:1), BC=0.09/1.00 (G-H:4), WB=0.11/1.00 (C-G:1), SS=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

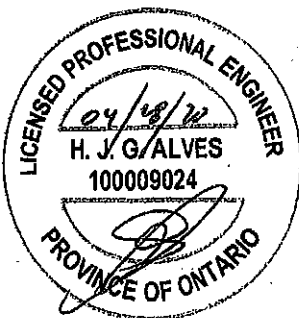
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1867 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (D) (INPUT = 0.90)
JSI METAL= 0.21 (D) (INPUT = 1.00)



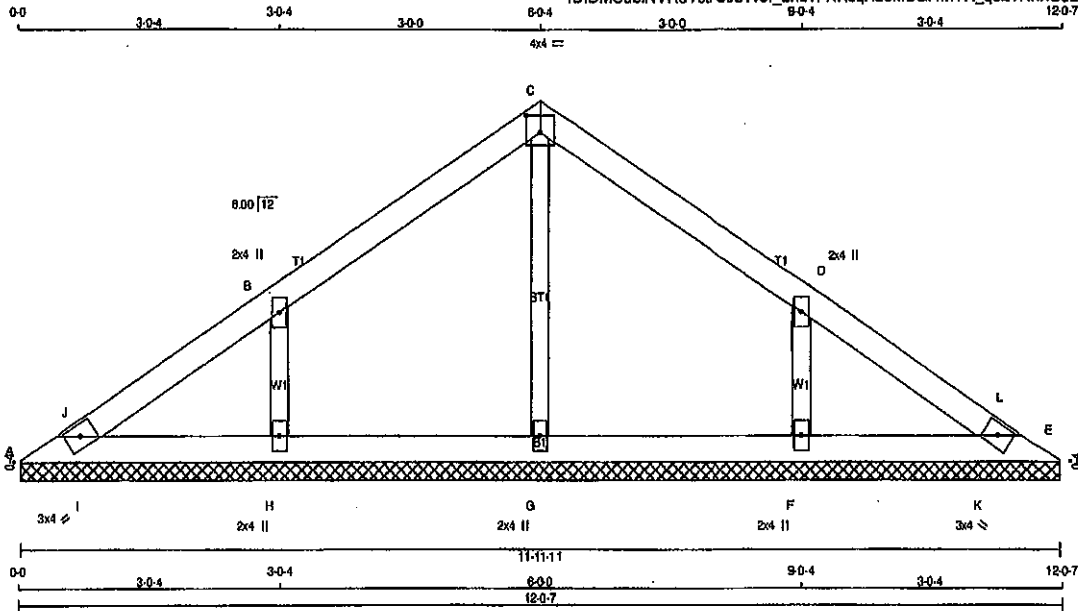
Structural component only
DWG# T-2007618

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408222	V4	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:23.7

TOTAL WEIGHT = 34 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - E	2x4	DRY	No.2	SPF		
A - E	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

PLATES (table is in inches)	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TBMW-w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TBMW-w	MT20	2.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F, G, H						
F	BMW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRO
	VERT	HORZ	DOWN	HORZ		
A	117	0	117	0	11-11-11	11-11-11
E	117	0	117	0	11-11-11	11-11-11
G	280	0	280	0	11-11-11	11-11-11
F	398	0	398	0	11-11-11	11-11-11
H	398	0	398	0	11-11-11	11-11-11

UNFACTORED REACTIONS

JT	COMBINED	SNOW		LIVE		PERM. LIVE	WIND	DEAD	SOIL
		1ST CASE	MAX	MIN	COMPONENT REACTIONS				
A	83	58/0	0/0	0/0	0/0	28/0	0/0	0/0	0/0
E	83	58/0	0/0	0/0	0/0	28/0	0/0	0/0	0/0
G	207	125/0	0/0	0/0	0/0	82/0	0/0	0/0	0/0
F	281	188/0	0/0	0/0	0/0	93/0	0/0	0/0	0/0
H	281	188/0	0/0	0/0	0/0	93/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. FACTORED	WEBS		MAX. FACTORED
	MAX. FACTORED	FORCE	VERT. LOAD	LC1 MAX		MAX. FACTORED	FORCE	
FR-TO	FROM	TO	FROM	TO	LENGTH	FR-TO	FROM	TO
A-J	-8/5	-91.8	-91.8	0.03 (1)	10.00	G-C	-248/0	0.08 (1)
J-B	0/36	-91.8	-91.8	0.13 (1)	10.00	F-D	-314/0	0.05 (1)
B-C	0/12	-91.8	-91.8	0.13 (1)	10.00	H-B	-314/0	0.05 (1)
C-D	0/12	-91.8	-91.8	0.13 (1)	10.00	I-J	-47/4	0.00 (1)
D-L	0/36	-91.8	-91.8	0.13 (1)	10.00	K-L	-47/4	0.00 (1)
L-E	-8/5	-91.8	-91.8	0.03 (1)	10.00			
A-I	-13/0	-18.5	-18.5	0.05 (1)	6.25			
I-H	-10/0	-18.5	-18.5	0.05 (1)	10.00			
H-G	-20/0	-18.5	-18.5	0.04 (4)	6.25			
G-F	-20/0	-18.5	-18.5	0.04 (4)	6.25			
F-K	-10/0	-18.5	-18.5	0.05 (1)	10.00			
K-E	-13/0	-18.5	-18.5	0.05 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (B-J:1), BC=0.05/1.00 (A-K:1), WB=0.08/1.00 (C-G:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

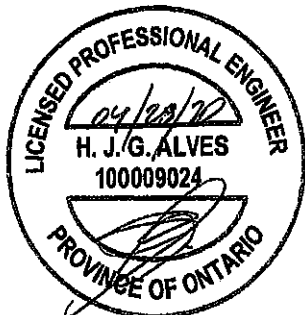
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 768 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (D) (INPUT = 0.99)
JSI METAL= 0.16 (B) (INPUT = 1.00)

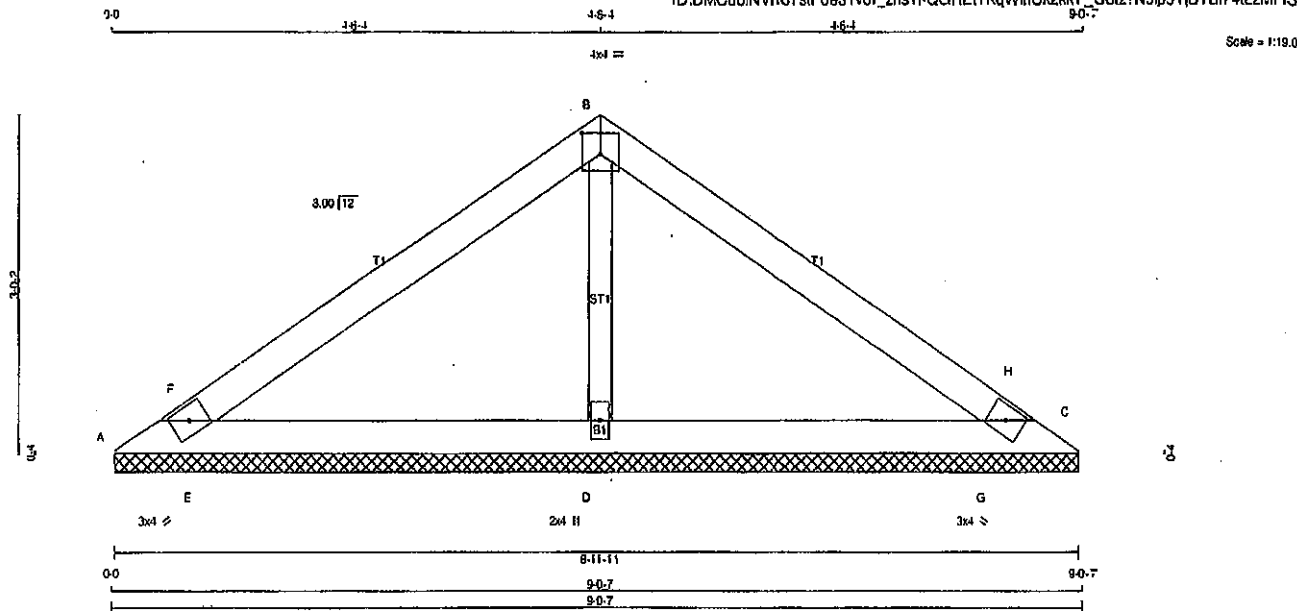


Structural component only
DWG# T-2007619

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408222	V5	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 23 b (M)

LUMBER	DESCR.
N. L. G. A. RULES	
CHORDS	
A - B 2x4 DRY No.2	SPF
B - C 2x4 DRY No.2	SPF
A - C 2x4 DRY No.2	SPF
ALL WEBS 2x3 DRY No.2	SPF
DRY: SEASONED LUMBER.	

PLATES (tablets in inches)	W	LEN	Y	X
JT TYPE PLATES				
A TBMi-h MT20	3.0	4.0		
B TTW-p MT20	4.0	4.0	2.25	2.00
C TBMi-h MT20	3.0	4.0		
D BMWt-w MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BRACKINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
A	49	0	49	0
C	49	0	49	0
D	893	0	893	0

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	34	22.0	0.0	0.0	0.0	12.0	0.0
C	34	22.0	0.0	0.0	0.0	12.0	0.0
D	631	415.0	0.0	0.0	0.0	216.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	CS1 (LC)	MEMB.	FORCE
FR-TO	(LBS)	FROM TO	CS1 (LC)	FR-TO	(LBS)
A-F	0 347	-91.8 -91.8 0.12 (1)	10.00	D-B	-698 0
F-B	0 341	-91.8 -91.8 0.23 (1)	10.00	E-F	-225 0
B-H	0 341	-91.8 -91.8 0.23 (1)	10.00	G-H	-225 0
H-C	0 347	-91.8 -91.8 0.12 (1)	10.00		
A-E	-321 0	-18.5 -18.5 0.16 (1)	6.25		
E-D	-290 0	-18.5 -18.5 0.17 (1)	6.25		
D-G	-290 0	-18.5 -18.5 0.17 (1)	6.25		
G-C	-321 0	-18.5 -18.5 0.16 (1)	6.25		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBCG 2018, CBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

ISS % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.23/1.00 (B-F:1), BC=0.17/1.00 (D-E:1), WB=0.12/1.00 (B-D:1), SS1=0.12/1.00 (A-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20 618 354	1867 788	1887 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

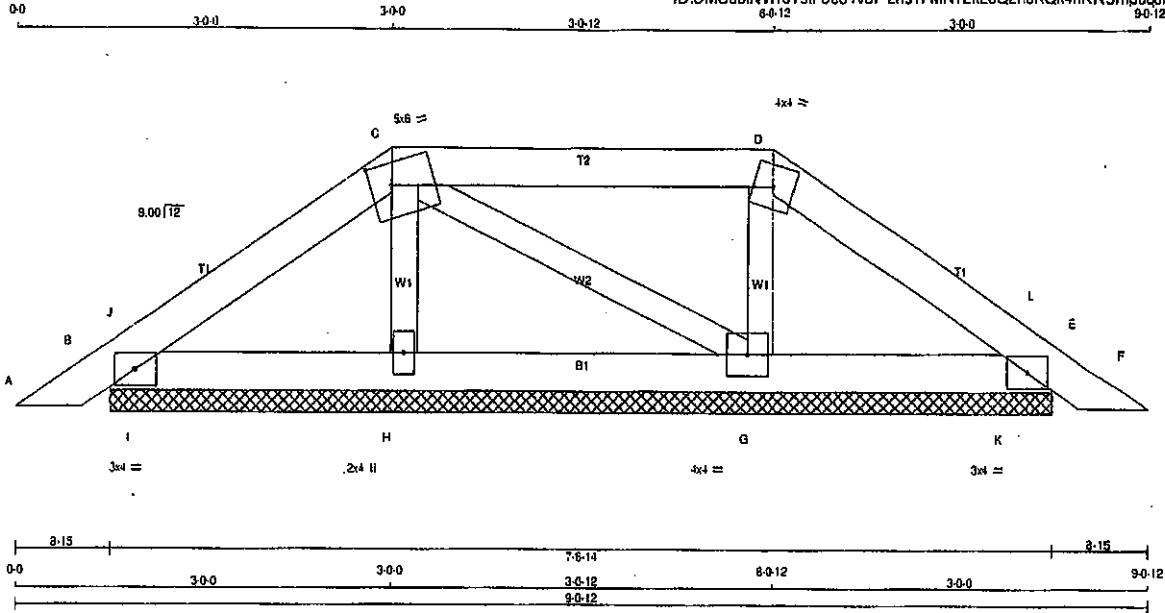
JSI GRIP= 0.59 (B) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007620

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	PB1	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:16.4

LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMB1-I	MT20	3.0	4.0	
C ITTW-m	MT20	5.0	6.0	2.00 2.00
D ITTW-m	MT20	4.0	4.0	
E TMB1-I	MT20	3.0	4.0	
G BMWW1-I	MT20	4.0	4.0	
H BMW1-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
B	221	0	221	0	0	7-6-14	7-6-14	7-6-14	7-6-14
E	209	0	209	0	0	7-6-14	7-6-14	7-6-14	7-6-14
H	248	0	248	0	0	7-6-14	7-6-14	7-6-14	7-6-14
G	280	0	280	0	0	7-6-14	7-6-14	7-6-14	7-6-14

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
B	154	113	0	0	0	0	41	0	0
E	148	106	0	0	0	0	39	0	0
H	178	109	0	0	0	0	67	0	0
G	198	127	0	0	0	0	71	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT.	LC1 MAX	MEMB.	FORCE (LBS)	VERT.	LC1 MAX
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 15	-91.8	-91.8 0.03 (1)	10.00	H-C	-176	0 0.03 (1)
B-J	-83 0	-91.8	-91.8 0.01 (1)	6.25	C-G	-21	0 0.00 (1)
J-C	-75 0	-91.8	-91.8 0.05 (1)	8.25	G-D	-198	0 0.03 (1)
C-D	-30 0	-91.8	-91.8 0.15 (1)	8.25	I-J	-121	0 0.00 (1)
D-L	-54 0	-91.8	-91.8 0.05 (1)	8.25	K-L	-123	0 0.00 (1)
L-E	-41 0	-91.8	-91.8 0.01 (1)	8.25			
E-F	0 15	-91.8	-91.8 0.03 (1)	10.00			
B-I	0 60	-18.5	-18.5 0.06 (1)	10.00			
I-H	0 60	-18.5	-18.5 0.06 (1)	10.00			
H-G	0 48	-18.5	-18.5 0.03 (1)	10.00			
G-K	0 43	-18.5	-18.5 0.06 (1)	10.00			
K-E	0 43	-18.5	-18.5 0.06 (1)	10.00			

TOTAL WEIGHT = 2 X 25 = 50 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 8.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15 (C-D:1), BC=0.08 (B-I:1), WB=0.03 (I-D:1), SSI=0.11 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	616 354	1667 788	1887 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.18 (B) INPUT = 0.90
JSI METAL = 0.04 (B) INPUT = 1.00

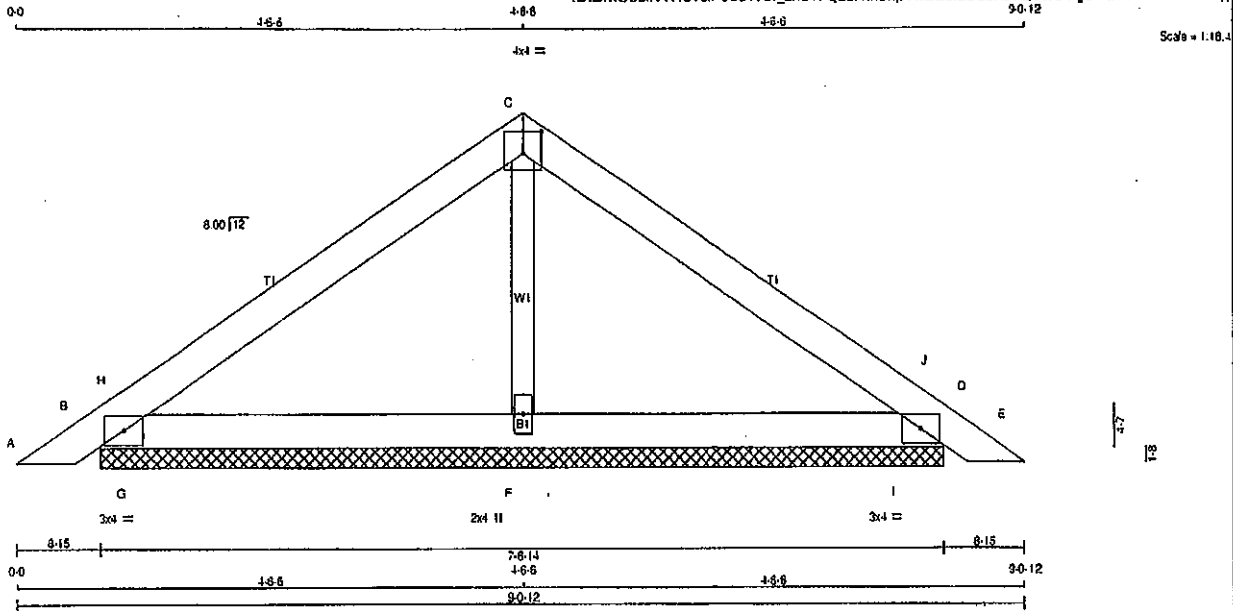


Structural component only
DWG# T-2007600

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	PB2	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 2 X 23 = 46 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - E	2x4	DRY	No.2	SPF		
B - D	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

PLATES (table in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMB1-I	MT20	3.0	4.0	
C ITW-p	MT20	4.0	4.0	2.25 2.00
D TMB1-I	MT20	3.0	4.0	
F BMW1-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	HORZ	DOWN	UPLIFT
B	318	0	0	7-8-14
D	318	0	0	7-8-14
F	320	0	0	7-8-14

UNFACTORED REACTIONS

1ST LOSE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
B	223	159	0	0	0	0	0
D	223	159	0	0	0	0	0
F	229	137	0	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	CS (LC)	FR-TO	
A-B	0 15	-91.8 -91.8	0.03 (1)	F-C	-148 0
B-H	-31 0	-91.8 -91.8	0.07 (1)	G-H	-349 0
H-C	-135 0	-91.8 -91.8	0.16 (1)	I-J	-349 0
C-J	-135 0	-91.8 -91.8	0.16 (1)		
J-D	-31 0	-91.8 -91.8	0.07 (1)		
D-E	0 15	-91.8 -91.8	0.03 (1)		
B-G	0 105	-18.5 -18.5	0.15 (1)		
G-F	0 105	-18.5 -18.5	0.15 (1)		
F-I	0 105	-18.5 -18.5	0.15 (1)		
I-D	0 105	-18.5 -18.5	0.15 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/1.00 (C-J:1), BC=0.15/1.00 (F-I:1), WB=0.03/1.00 (C-F:1), SS=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.28 (D) (INPUT = 0.90)
JSI METAL = 0.07 (D) (INPUT = 1.00)



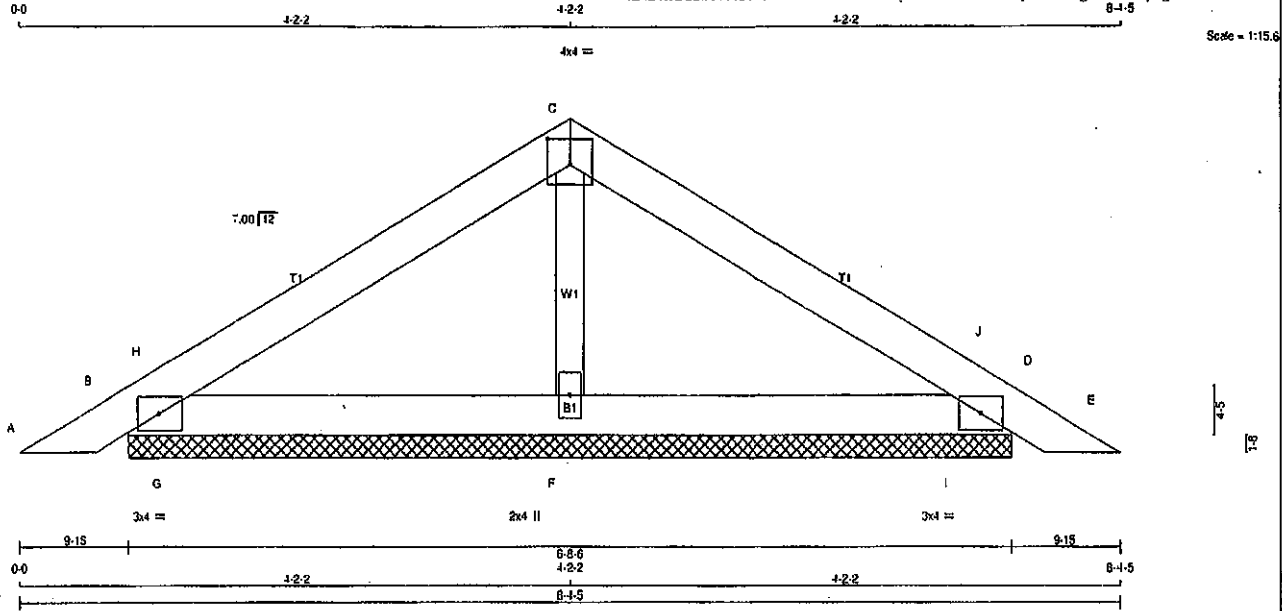
Structural component only
DWG# T-2007601

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408223	PB20	4	1	TRUSS DESC.		

Parasack Roof Truss, Burlington

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TOTAL WEIGHT = 4 X 20 = 80 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - E	2x4 DRY	No.2	SPF		
B - D	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
DRY: SEASONED LUMBER.					

PLATES (table in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMBI-1	MT20	3.0	4.0			
C	TTW-p	MT20	4.0	4.0	2.25	2.00	
D	TMBI-1	MT20	3.0	4.0			
F	BMW1+w	MT20	2.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT	REORD
	VERT	HORZ	DOWN	HORZ	UPLIFT
JT	281	0	281	0	0
B	281	0	281	0	0
D	281	0	281	0	0
F	314	0	314	0	0

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	197	141	0	0	0	55	0
D	197	141	0	0	0	55	0
F	224	137	0	0	0	87	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE	VERT. LOAD	LC1	MAX.	MAX.	UNBRAC	WEBS	MEMB.	MAX. FACTORED	FORCE	MAX.
			(LBS)	(PLF)		(LBS)	(LBS)					(LBS)	(LBS)
FR-TO													
A-B	0	10	-91.8	-91.8	0.04	(1)	10.00		F-C	-159	0	0.02	(1)
B-H	-44	0	-91.8	-91.8	0.05	(1)	8.25		G-H	-250	0	0.00	(1)
H-C	-99	0	-91.8	-91.8	0.12	(1)	8.25		I-J	-250	0	0.00	(1)
C-J	-99	0	-91.8	-91.8	0.12	(1)	8.25						
J-D	-44	0	-91.8	-91.8	0.05	(1)	8.25						
D-E	0	10	-91.8	-91.8	0.04	(1)	10.00						
B-G	0	81	-18.5	-18.5	0.12	(1)	10.00						
G-F	0	81	-18.5	-18.5	0.12	(1)	10.00						
F-I	0	81	-18.5	-18.5	0.12	(1)	10.00						
I-D	0	81	-18.5	-18.5	0.12	(1)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (CH:1), BC=0.12/1.00 (B-G:1), WB=0.02/1.00 (C-F:1), SS=0.18/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

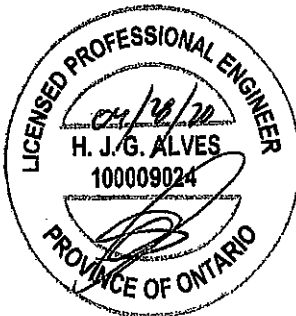
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	518	354	1667
	788	1907	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

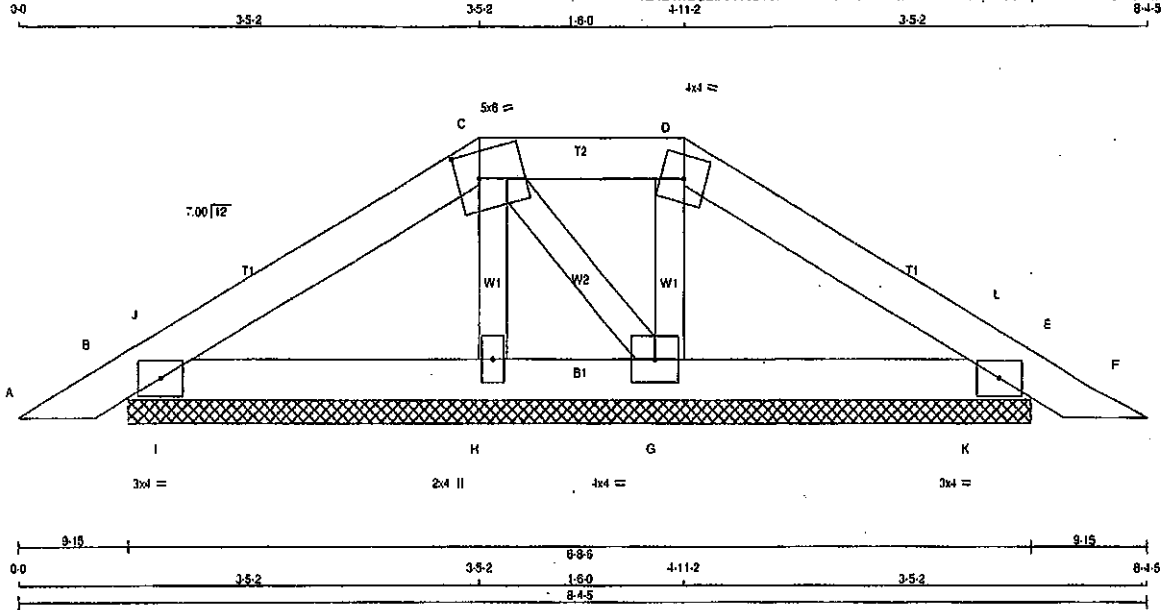
JSI GRIP = 0.24 (D) INPUT = 0.90
JSI METAL = 0.06 (B) INPUT = 1.00



Structural component only
DWG# T-2007627

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	PB21	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:15.2

TOTAL WEIGHT = 22 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF		
C - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	No.2	SPF		
B - E	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTWW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMWW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	DOWN	HORZ	DOWN	HORZ	UP	IN-SX	BRG	IN-SX
B	241	0	0	241	0	0	6-8-6	6-8-6	6-8-6
E	228	0	0	228	0	0	6-8-6	6-8-6	6-8-6
H	170	0	0	170	0	0	6-8-6	6-8-6	6-8-6
G	238	0	0	238	0	0	6-8-6	6-8-6	6-8-6

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
B	189	121.0	0.0	0.0	0.0	48.0	0.0
E	189	114.0	0.0	0.0	0.0	48.0	0.0
H	122	74.0	0.0	0.0	0.0	48.0	0.0
G	168	111.0	0.0	0.0	0.0	57.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	CS1 (LC)	UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO		FROM	TO			FR-TO			
A-B	0 16	-91.8	-91.8 0.04 (1)	10.00		H-C	-107 0	0.02 (1)	
B-J	-59 0	-91.8	-91.8 0.01 (1)	6.25		C-G	-36 0	0.01 (1)	
J-C	-71 0	-91.8	-91.8 0.07 (1)	6.25		G-D	-147 0	0.02 (1)	
C-D	-28 0	-91.8	-91.8 0.04 (1)	6.25		I-J	-139 0	0.00 (1)	
D-L	-46 0	-91.8	-91.8 0.07 (1)	6.25		K-L	-141 0	0.00 (1)	
L-E	-33 0	-91.8	-91.8 0.01 (1)	6.25					
E-F	0 16	-91.8	-91.8 0.04 (1)	10.00					
B-I	0 59	-18.5	-18.5 0.07 (1)	10.00					
I-H	0 59	-18.5	-18.5 0.07 (1)	10.00					
H-G	0 52	-18.5	-18.5 0.04 (1)	10.00					
G-K	0 38	-18.5	-18.5 0.07 (1)	10.00					
K-E	0 38	-18.5	-18.5 0.07 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012: ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN (LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07 (D-L:1), BC=0.07 (B-I:1), WB=0.02 (D-G:1), SSI=0.11 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

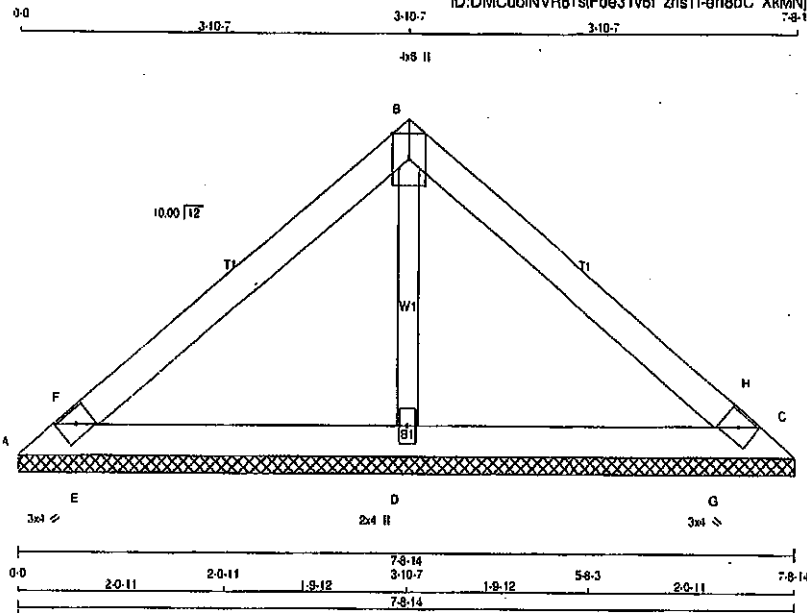
JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)



Structural component only
DWG# T-2007628

JOB NAME 408223	TRUSS NAME P20	QUANTITY 17	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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ID:DMCubINVR6TstFoe31v6l zns11-en8bC XKMNIUnocWqbyd68sgmus6Uhw8i76i2zMEMo



Scale = 1/20"

LUMBER		N. L. G. A. RULES		CHORDS - SIZE		LUMBER		DESCR.	
A - B	2x4	DRY	No.2	SPF				SPF	
B - C	2x4	DRY	No.2	SPF				SPF	
A - C	2x4	DRY	No.2	SPF				SPF	
ALL WEBS		2x3	DRY	No.2	SPF			SPF	
DRY: SEASONED LUMBER.									

TOTAL WEIGHT = 17 X 21 = 357 lb

PLATES (table is in inches)		JT TYPE		PLATES		W		LEN		Y		X	
A	TBM1-h	MT20	3.0	4.0									
B	TTW+p	MT20	4.0	6.0	Edge								
C	TBM1-h	MT20	3.0	4.0									
D	BMW1+w	MT20	2.0	4.0									

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT		GROSS REACTION		GROSS REACTION		BRG		BRG	
A		VERT		DOWN		HORZ		UPLIFT	
B		0		0		0		0	
C		0		0		0		0	
D		0		0		0		0	

UNFACTORED REACTIONS		1ST CASE		MAX. MIN. COMPONENT REACTIONS		SNOW		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT		COMBINED		0		0		0		0		0		0		0	
A		43		28		0		0		0		0		14		0	
B		42		28		0		0		0		0		14		0	
C		518		339		0		0		0		0		179		0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		MEMB.		WEBS		MAX. FACTORED		MEMB.		MAX. FACTORED		MEMB.	
FR-TO		FORCE (LBS)		VERT. LOAD (PLF)		MAX. (LBS)		FORCE (LBS)		MAX. (LBS)		FORCE (LBS)		MAX. (LBS)		FORCE (LBS)	
A-F		0 227		-91.8 -91.8 0.09 (1)		10.00		D-B		-558 0		0.10 (1)		0.10 (1)		0.10 (1)	
F-B		0 219		-91.8 -91.8 0.17 (1)		10.00		E-F		-199 0		0.00 (1)		0.00 (1)		0.00 (1)	
B-H		0 219		-91.8 -91.8 0.17 (1)		10.00		G-H		-199 0		0.00 (1)		0.00 (1)		0.00 (1)	
H-C		0 228		-91.8 -91.8 0.09 (1)		10.00											
A-E		-207 0		-18.5 -18.5 0.13 (1)		6.25											
E-D		-174 0		-18.5 -18.5 0.13 (1)		6.25											
D-G		-174 0		-18.5 -18.5 0.13 (1)		6.25											
G-C		-207 0		-18.5 -18.5 0.13 (1)		6.25											

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 1.00 (B-F:1), BC=0.13 1.00 (D-E:1), WS=0.10 1.00 (B-D:1), SS=0.11 1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PU)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

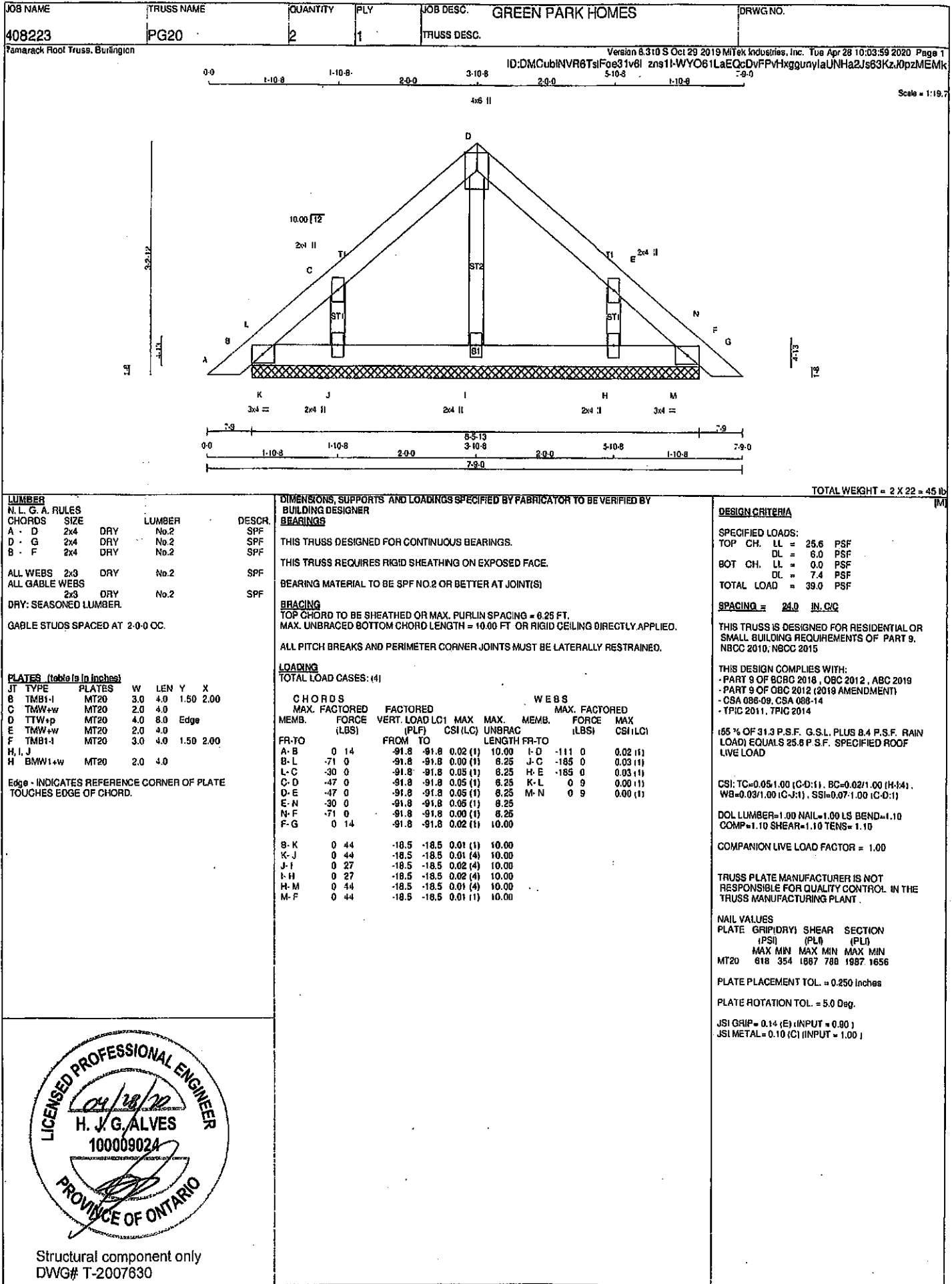
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.29 (D) INPUT = 0.90)
JSI METAL = 0.12 (D) INPUT = 1.00)

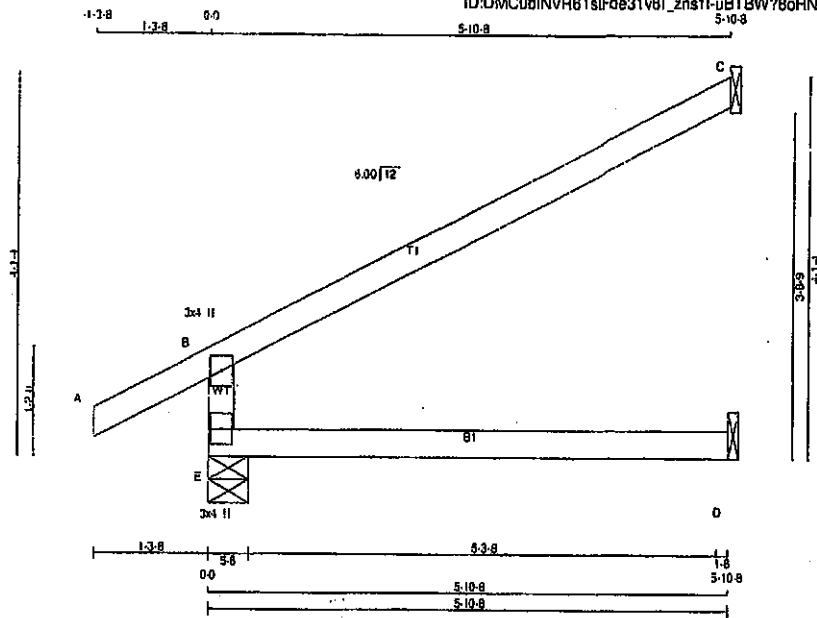


Structural component only
DWG# T-2007626



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	J1	21	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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Scale = 1/2" = 1'-0"

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN	Y
B	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
E	525	0	525	0	0	5-8	5-8		
C	202	0	202	0	0	1-8	1-8		
D	45	0	50	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
E	389	257	0
C	139	113	0
D	36	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1 MAX (PLF)	MEMB.	FORCE (LBS)	MAX. (LBS)	MAX. (LBS)
FR-TO				FR-TO			
E-B	-161	0	0.0	0.0	0.13	7.81	
A-B	0	28	-91.8	-91.8	0.12	10.00	
B-C	-30	0	-91.8	-91.8	0.54	6.25	
E-D	0	0	-18.5	-18.5	0.13	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. GIC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF.

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CS: TC=0.54, 1.00 (B-C:1), BC=0.13, 1.00 (D-E:4), WB=0.00, 1.00 (A-B:0), SSI=0.24, 1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.19 (E) INPUT = 0.90
JSI METAL = 0.13 (B) INPUT = 1.00



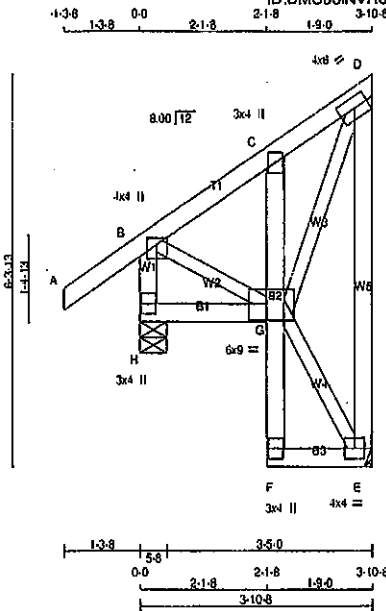
Structural component only
DWG# T-2007598

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	J2S	4	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1/32

TOTAL WEIGHT = 4 X 31 = 125 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
F - C	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMVW+p MT20	4.0	4.0	1.25	2.00
C TMV+p MT20	3.0	4.0		
D TMVW-1 MT20	4.0	8.0	2.00	2.75
E BMVW-1 MT20	4.0	4.0		
F BMV+p MT20	3.0	4.0		
G BVWVW-1 MT20	6.0	9.0	3.25	3.50
H BMV-1+p MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	HORIZ	UPLIFT
H 332	0	332	0
E 206	0	206	0
			MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1'-8".

UNFACTORED REACTIONS

1ST LC CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM LIVE WIND DEAD SOIL
H 232	168 0 0 0 0 0 68 0 0 0
E 145	95 0 0 0 0 0 50 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED	FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MEMB.	FORCE
FR-TO	(LBS)	(PLF)	CS (LC)	FR-TO	(LBS)
H-B	-311 0	0.0	0.0 0.03 (1)	8-G	0 97
A-B	0 35	-91.8	-91.8 0.14 (5)	E-D	-185 0
B-C	-96 0	-91.8	-91.8 0.05 (1)	G-E	-7 0
C-D	-105 0	-91.8	-91.8 0.05 (1)	G-D	0 204
H-G	0 0	-18.5	-18.5 0.03 (4)		
F-G	0 15	0.0	0.0 0.01 (1)		
G-C	-202 0	0.0	0.0 0.01 (1)		
F-E	0 4	-18.5	-18.5 0.01 (4)		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. GC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19')
CALCULATED VERT. DEFL. (LL) = L/999 (0.00')
ALLOWABLE DEFL. (TL) = L/360 (0.19')
CALCULATED VERT. DEFL. (TL) = L/999 (0.00')

CSI: TC=0.14 (1.00 (A-B:5), 8C=0.03 (1.00 (G-H:4), WB=0.08 (1.00 (D-E:1), SS)=0.09 (1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

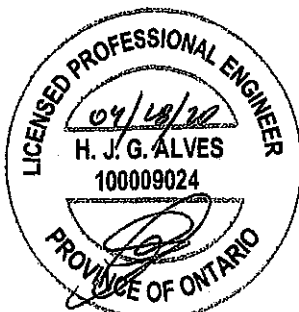
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.22 (8) INPUT = 0.90
JSI METAL = 0.07 (C) INPUT = 1.00

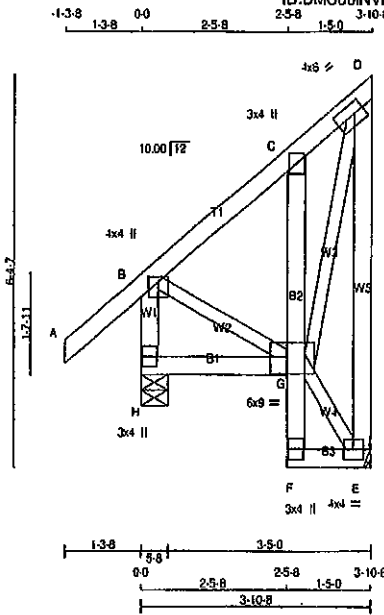


Structural component only
DWG# T-2007599

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	J20S	2	1	GREEN PARK HOMES	
Famarack Roof Truss, Burlington					

Famarack Roof Truss, Burlington

ID:DMCubINVR6TstF0e31v6I_zns11-10v4McUDJ8C2IAIT_X7SGZa59ZPAq_xD41uH2MEMS



TOTAL WEIGHT = 2 X 33 = 66 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES					
H - B	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
F - C	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMV+p	MT20	3.0	4.0		
D	TMVW-1	MT20	4.0	6.0	2.00	2.00
E	BMVW-1	MT20	4.0	4.0		
F	BMV+p	MT20	3.0	4.0		
G	BMVW-1	MT20	6.0	9.0	3.25	3.50
H	BMV-1	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
H	333	0	333	0
E	206	0	206	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	233	166	0	0	0	67	0
E	145	95	0	0	0	50	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)
FR-TO								
H-B	-309	0	0.0	0.0	0.03 (1)	7.81	B-G	0 70
A-B	0	41	-91.8	-91.8	0.14 (5)	10.00	E-D	-185 0
B-C	-70	0	-91.8	-91.8	0.07 (1)	8.25	G-E	-13 0
C-D	-92	0	-91.8	-91.8	0.08 (1)	6.25	G-D	0 215
H-G	0	0	-18.5	-18.5	0.04 (4)	10.00		
F-G	0	12	0.0	0.0	0.02 (1)	10.00		
G-E	-216	0	0.0	0.0	0.02 (1)	7.81		
F-E	0	8	-18.5	-18.5	0.01 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2010. NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (A-B:5), BC=0.04/1.00 (G-H:4),
WB=0.08/1.00 (D-E:1), SS=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

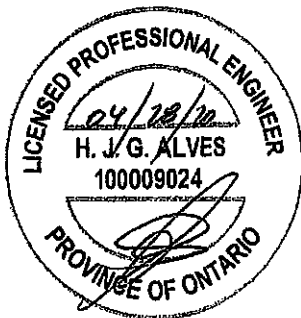
NAIL VALUES

PLATE	GRIP (PLY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.24 (B) (INPUT = 0.90)
JSI METAL = 0.08 (C) (INPUT = 1.00)



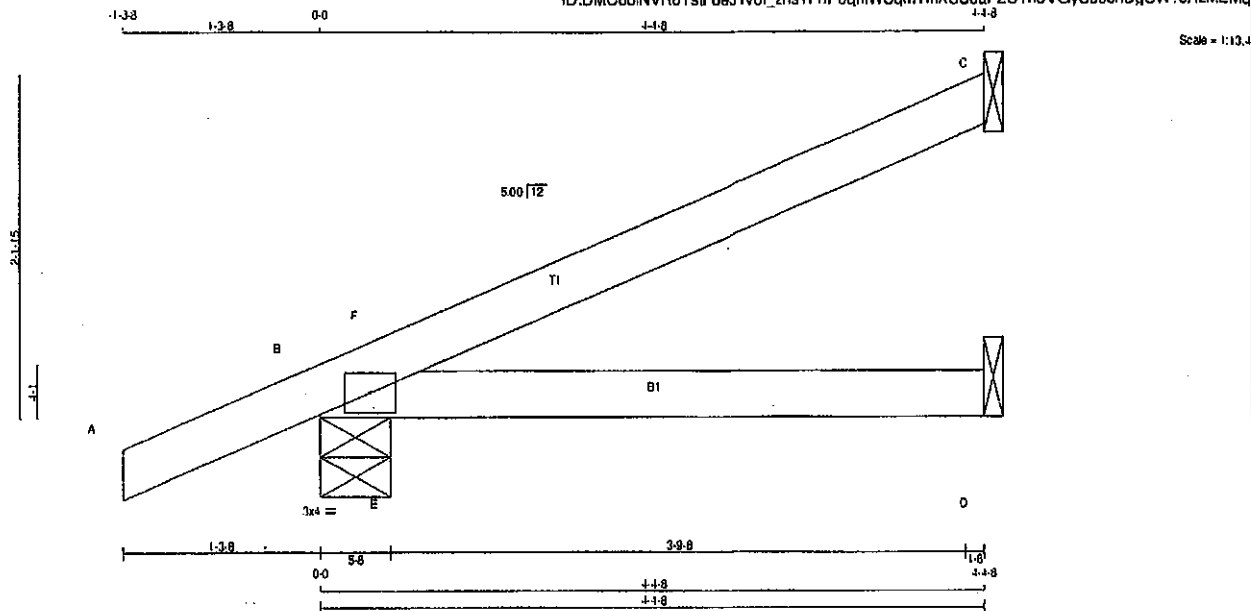
Structural component only
DWG# T-2007623

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408223	J21	4	1	TRUSS DESC.		

Paradeck Roof Truss, Burlington

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ID:DMCubINVR6TstFoa31v6l_zns1FhP0qnlWUqmTmXUS8aPZUyh3VGyCuebnDgOW?oAzMEMQ



TOTAL WEIGHT = 4 X 12 = 49 lb

LUMBER				DESCR.	
N.L.G.A. RULES	CHORDS	SIZE	DRY	SPF	SPF
A - C	2x4	DRY	No.2		
B - D	2x4	DRY	No.2		

DRY: SEASONED LUMBER.

PLATES (tablets in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TM81-I	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG			
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
C	175	0	175	0	0	1-8	1-8
B	365	0	365	0	0	5-8	5-8
D	67	0	67	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	121	94	0	0	0	27	0
B	256	181	0	0	0	75	0
D	50	18	0	0	0	32	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 23	-91.8 -91.8	0.12 (1)	10.00	E-F	-235 7	0.00 (1)
B-F	-17 13	-91.8 -91.8	0.04 (4)	8.25			
F-C	-1 2	-91.8 -91.8	0.23 (1)	10.00			
B-E	0 0	-18.5 -18.5	0.16 (1)	10.00			
E-D	0 0	-18.5 -18.5	0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./O.C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.23/1.00 (C-F:1), BC=0.16/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.19/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1937 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.26 (B) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)

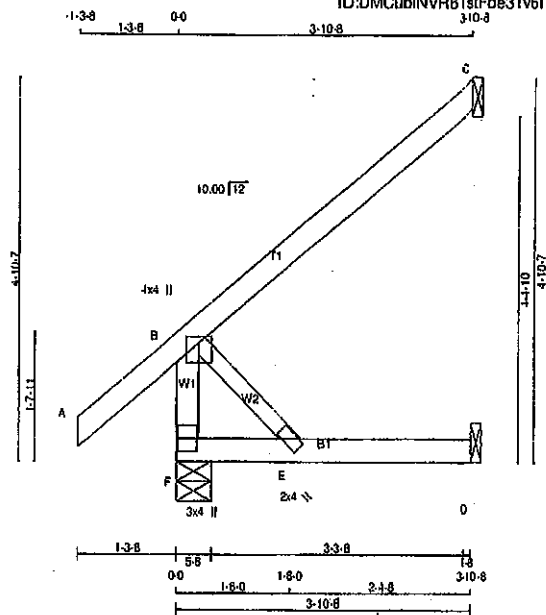


Structural component only
DWG# T-2007624

Ramarack Roof Truss, Burlington

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Scale = 1:27.0



TOTAL WEIGHT = 3 X 15 = 46 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY No.2	SPF
A - C	2x4	DRY No.2	SPF
F - D	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (tablets in inches)		W.	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	VERT	HORZ	DOWN	HORZ	BRG	BRG
JT	341	0	341	0	5-8	5-8
F	178	0	178	0	1-8	1-8
C	36	0	40	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	239	170 0	0 0	0 0	0 0	89 0	0 0
C	122	99 0	0 0	0 0	0 0	23 0	0 0
D	29	0 0	0 0	0 0	0 0	29 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LO)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LO)
FR-TO		FROM TO			FR-TO		
F-B	-305 0	0.0 0.0	0.03 (1)	7.81	B-E	0 0	0.00 (1)
A-B	0 41	-91.8 -91.8	0.14 (5)	10.00			
B-C	0 0	-91.8 -91.8	0.23 (1)	10.00			
F-E	0 0	-18.5 -18.5	0.08 (4)	10.00			
E-D	0 0	-18.5 -18.5	0.08 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:				
TOP	CH.	LL	=	25.6 PSF
		DL	=	6.0 PSF
BOT	CH.	LL	=	0.0 PSF
		DL	=	7.4 PSF
TOTAL LOAD			=	39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 088-09, CSA 088-14
 - TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN
LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF
LIVE LOAD

ALLOWABLE DEFL.(LL) = L 360 (0.19")
CALCULATED VERT. DEFL.(LL) = L 999 (0.00")
ALLOWABLE DEFL.(TL) = L 360 (0.19")
CALCULATED VERT. DEFL.(TL) = L 999 (0.01")

CSI: TC=0.23/1.00 (B-C:1), BC=0.08/1.00 (D-E:4),
WB=0.00/1.00 (B-E:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.24 (B) INPUT = 0.90)
JSI METAL = 0.08 (B) INPUT = 1.00)

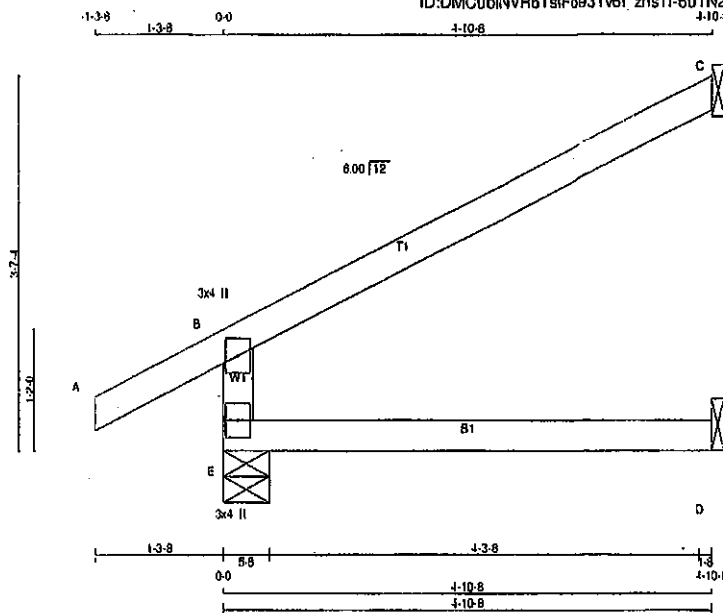


Structural component only
DWG# T-2007625

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	J40	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Tamarack Roof Truss, Burlington

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Scale = 1:20.5

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4 DRY No.2			
A - C	2x4 DRY No.2			
E - D	2x4 DRY No.2			

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UP
E	456	0	456	0
C	168	0	168	0
D	38	0	42	0

SEE MTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) G, D

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED						
E	320	225	0	0	0	95	0
C	116	94	0	0	0	22	0
D	30	0	0	0	0	30	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX
FR-TO	(LBS)	(PLF)	CS1 (LC)	UNBRAC	LENGTH	FR-TO	(LBS)	CS1 (LC)
E-B	-404	0	0.0	0.0	0.08 (4)	7.81		
A-B	0	28	-81.8	-81.8	0.12 (1)	10.00		
B-C	-25	0	-81.8	-81.8	0.37 (1)	6.25		
E-D	0	0	-18.5	-18.5	0.09 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPC 2011, TPC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CS1: TC=0.37 (1.00) (B-C:1), BC=0.09 (1.00) (D-E:4), WB=0.00 (1.00) (A-B:0), SS=0.20 (1.00) (B-C:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.17 (E) (INPUT = 0.80)
JSI METAL = 0.11 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007657

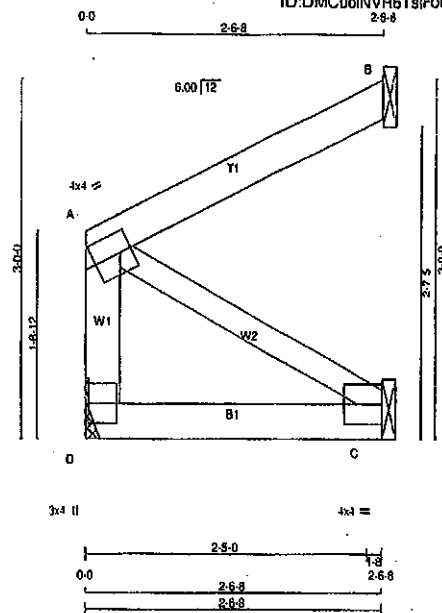
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	J41	5	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TsFos31v6l zns11-3Hb7Tu8HW1XPKAQqNkQ_LMmiLEntEng/PoM1zzME7p

Scale = 1:17.6



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
D - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
D - C	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW	MT20	4.0	4.0	2.00 1.25
C	BMV1	MT20	4.0	4.0	2.00 Edge
D	BMV1	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
D	139	0	139	0	0	0	0	MECHANICAL	
B	117	0	117	0	0	1-8	1-8		
C	23	0	25	0	0	1-8	1-8		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS		1ST CASE		MAX. MIN. COMPONENT REACTIONS		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW													
D	98	65	0	0.0	0	0	0	0	0	0	0	33	0	0	0
B	80	65	0	0.0	0	0	0	0	0	0	0	15	0	0	0
C	18	0	0	0.0	0	0	0	0	0	0	0	18	0	0	0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	LC1 MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
D-A	-117 0	0.0	0.0 0.01 (1)	7.81	A-C	0 0	0.00 (1)
A-B	0 0	-91.8 -91.8	0.10 (1)	10.00			
D-C	0 0	-18.5 -18.5	0.03 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 096-09, CSA 096-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CS: TC=0.10/1.00 (A-B:1), BC=0.09/1.00 (C-D:4), WB=0.09/1.00 (A-C:1), BS1=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.07 (A) (INPUT = 0.80)
JSI METAL = 0.02 (A) (INPUT = 1.00)



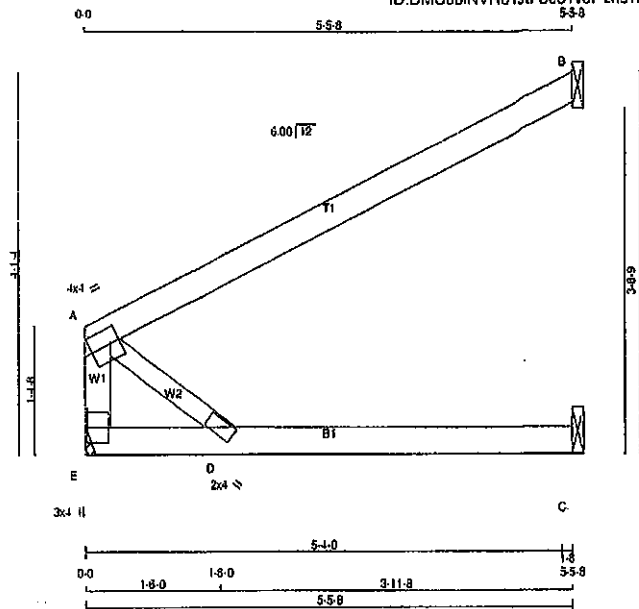
Structural component only
DWG# T-2007658

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	J42	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC	

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ID:DMCubINVR6t9IF0a31v6I zns11-XT8WgE9vHKIGyK71xSq3XYumIX 1hxqu3YvZPzME7c

Scale = 1:22.5



TOTAL WEIGHT = 2 X 18 = 31 lb

(M/F)

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES	E - A	2x4	DRY	No.2	SPF
	A - B	2x4	DRY	No.2	SPF
	E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	4.0	4.0	2.00	1.25
D	BMVW-1	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
E	301	0	0	0
B	250	0	1-8	1-8
C	50	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	213	140	0	0	0	0	73	0
B	172	140	0	0	0	0	33	0
C	40	0	0	0	0	0	40	0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LG1 MAX. CSI (LC)	UNBRACED LENGTH	FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX. CSI (LC)
E-A	250	0	0.0	0.0	0.03 (1)	7.81	A-D	0	0	0.00 (1)
A-B	0	0	-91.8	-91.8	0.48 (1)	10.00				
E-D	0	0	-18.5	-18.5	0.13 (4)	10.00				
D-C	0	0	-18.5	-18.5	0.16 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.04")

CSF: TC=0.46/1.00 (A-B:1), BC=0.16/1.00 (C-D:4), WB=0.00/1.00 (A-D:1), SSI=0.17/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (PSI) SHEAR SECTION (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.15 (A) (INPUT = 0.90)
JSI METAL = 0.04 (A) (INPUT = 1.00)



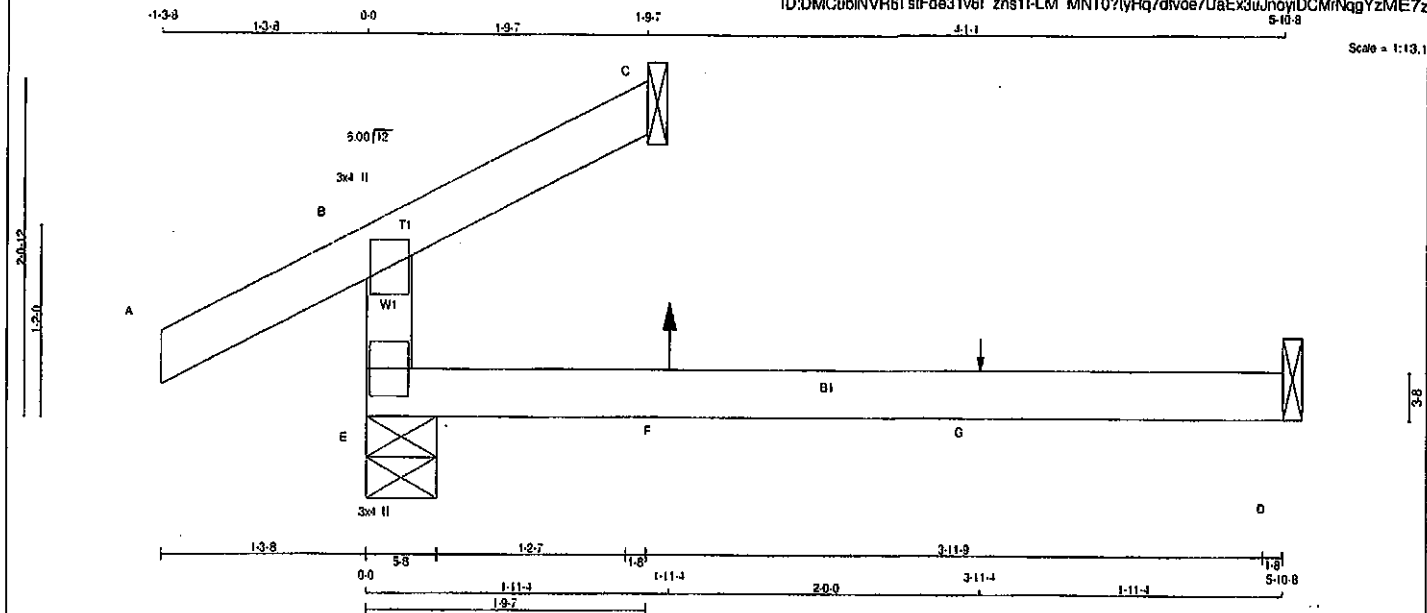
Structural component only
DWG# T-2007659

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	C41	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:13.1



TOTAL WEIGHT = 2 X 12 = 23 lb (M)

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
FACTORED				MAXIMUM FACTORED		INPUT	
GROSS REACTION				DOWN		BRG	
JT	VERT	HORZ		HORZ	UPLIFT	IN-SX	IN-SX
E	284	0		284	0	5-8	5-8
C	63	0		63	0	1-8	1-8
D	44	0		52	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
1ST LOASE				MAX. MIN. COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	200	137	0	0	0	62	0
C	46	21	0	0	0	25	0
D	35	0	3	0	0	37	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MEMB.		MAX. FACTORED FORCE (LBS)	FACTORED MAX. CSI (LC)
FR-TO				FR-TO			
E-B	-227	0	0.0	0.0	0.11 (4)	7.31	
A-B	0	28	-91.8	-91.8	0.12 (1)	10.00	
B-C	-9	9	-91.8	-91.8	0.08 (4)	10.00	
E-F	0	0	-18.5	-18.5	0.14 (4)	10.00	
F-G	0	0	-18.5	-18.5	0.14 (4)	10.00	
G-D	0	0	-18.5	-18.5	0.14 (4)	10.00	

FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX-	MAX+	FACE	D/R.	TYPE
F	1-11-4	7	1	12	BACK	VERT	TOTAL
G	3-11-4	1	1	---	BACK	VERT	TOTAL

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.12:1.00 (A-B:1), BC=0.14:1.00 (D-E:4), WB=0.09:1.00 (A-B:1), SSI=0.09:1.00 (A-B:1)

DOL LUMBER=0.98 NAIL=0.98 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1997 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) INPUT = 0.90 I
JSI METAL= 0.06 (8) INPUT = 1.00 I



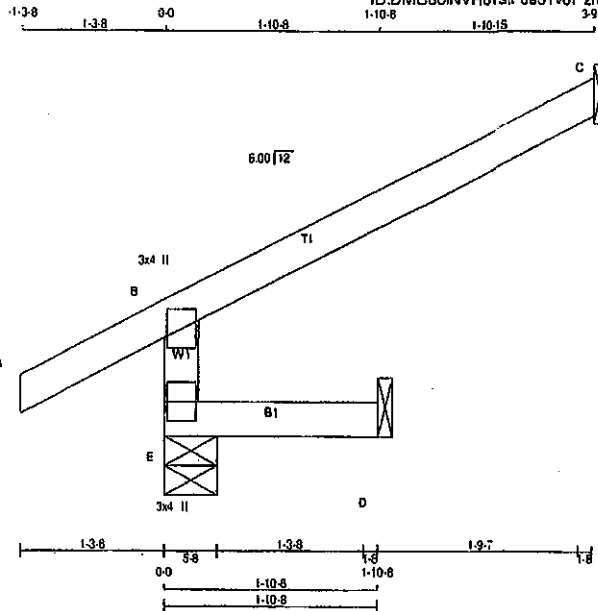
Structural component only
DWG# T-2007649

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	C42	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:18.2

TOTAL WEIGHT = 3 X 10 = 28 lb

LUMBER	N. L. G. A. RULES	SIZE	LUMBER
CHORDS	SIZE		
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE	PLATES			
B TMV-p	MT20	3.0	4.0	
E BMV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
E	361	361	0	5-6
C	130	130	0	1-6
D	16	17	0	1-6

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
E	250	190	0	0	0	60	0
C	90	73	0	0	0	17	0
D	12	0	0	0	0	12	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (6)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (L)	MAX. CS1 (L)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (L)
FR-TO		FROM TO			LENGTH	FR-TO	
E-B	-342	0	0.0	0.0	0.01 (4)	7.81	
A-B	0	28	-91.8	-91.8	0.13 (5)	10.00	
B-C	-19	0	-91.8	-91.8	0.22 (1)	6.25	
E-D	0	0	-18.5	-18.5	0.02 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/380 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.22(1.00 (B-C:1), 0C=0.02(1.00 (D-E:4), WB=0.00(1.00 (rva:0), SSt=0.15(1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRV)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	818	354 1887 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.14 (E) (INPUT = 0.99)
JSI METAL = 0.09 (B) (INPUT = 1.00)



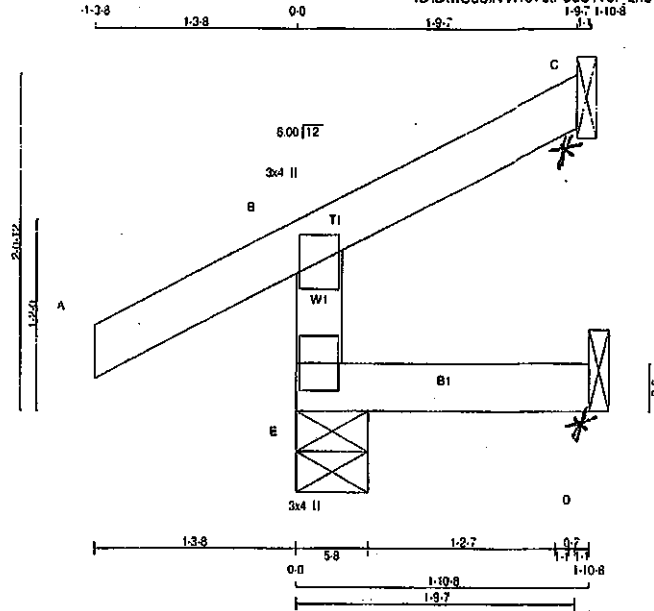
Structural component only
DWG# T-2007650

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	C43	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1/10.1



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT HORZ	DOWN HORZ UPLIFT	IN-SX	IN-SX	
E	271 0	271 0	5-8	5-8
C	45 0	45 0	-23	1-8
D	8 0	17 0	-2	1-8

SEE MITEK STANDARD DETAIL 697791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 160 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	188	141	0	0.0	0.0	47	0
C	31	24	-18	0	0	7	0
D	7	0	-8	0	0	12	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED			MEMB.	MAX. FACTORED	
	FORCE	VERT. LOAD	LC1	MAX.		FORCE	MAX.
	(LBS)	(PLF)	CS1 (LBS)	UNBRAC		(LBS)	CS1 (LBS)
FR-TO		FROM TO		LENGTH	FR-TO		
E-B	-244 0	0.0 0.0	0.04 (5)	7.81			
A-B	0 28	-91.8 -91.8	0.12 (1)	10.00			
B-C	-17 0	-91.8 -91.8	0.09 (1)	6.25			
E-D	0 0	-18.5 -18.5	0.04 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = L/360 (0.19")
CALCULATED VERT. DEFL (LL) = L/999 (0.00")
ALLOWABLE DEFL (TL) = L/360 (0.19")
CALCULATED VERT. DEFL (TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), GC=0.04/1.00 (D-E:8),
WB=0.00/1.00 (truss), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1687 788 1987 1658	

PLATE PLACEMENT TOL. = 0.250 inches

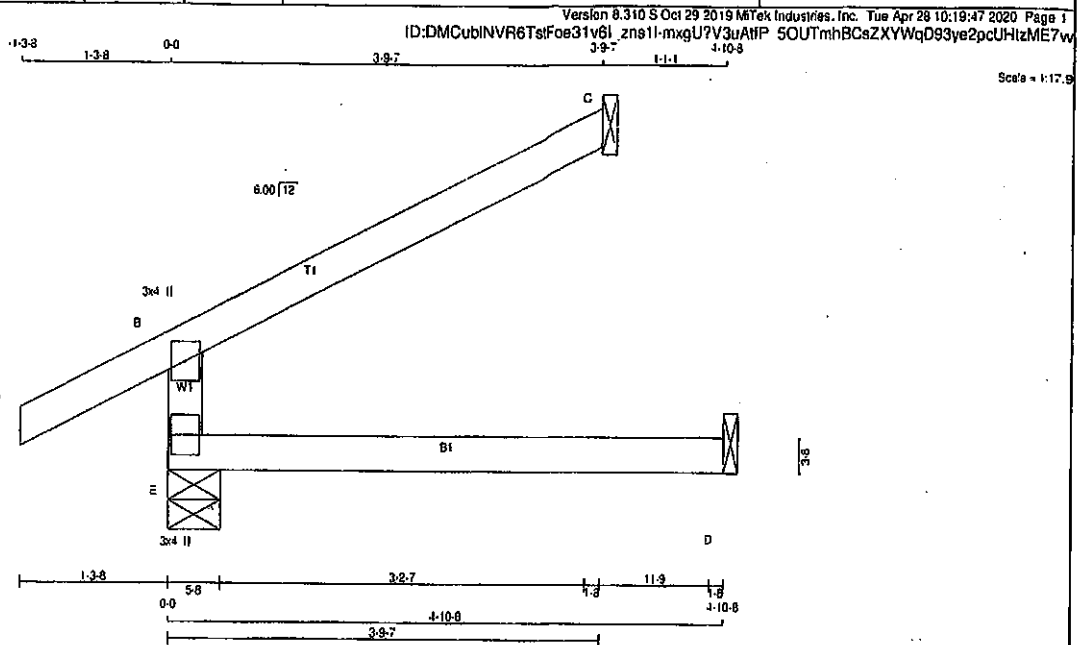
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.10 (E) INPUT = 0.90
JSI METAL = 0.07 (B) INPUT = 1.00



Structural component only
DWG# T-2007651

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	C44	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			



TOTAL WEIGHT = 2 X 13 = 26 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
E	394	0	0	394	0	0	0	5-8	5-8
C	130	0	0	130	0	0	0	1-8	1-8
D	38	0	0	42	0	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL 097791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS		1ST CASE		MAX. MIN. COMPONENT REACTIONS		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW
E	277	190	0	0	0	0	0	0	0	0	0	87	0	0	0
C	90	73	0	0	0	0	0	0	0	0	0	17	0	0	0
D	30	0	0	0	0	0	0	0	0	0	0	30	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (LBS)	MAX. (PL)	MEMB.	MAX. FORCE (LBS)	MAX. (PL)	MAX. (PL)
FR-TO				FR-TO			
E-B	-342	0	0.0	0.0	0.0	0.08	7.81
A-B	0	28	-91.8	-91.8	0.12	(1)	10.00
B-C	-19	0	-91.8	-91.8	0.22	(1)	6.25
E-D	0	0	-18.5	-18.5	0.09	(4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	8.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.22(1.00 (B-C)), BC=0.09(1.00 (D-E)), WB=0.00(1.00 (m.s.)), SSI=0.15(1.00 (B-C))

DOE LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

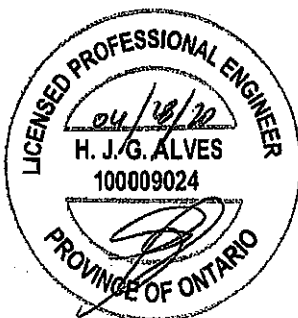
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	818	354	1867
			788
			1987
			1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.14 (E) INPUT = 0.90
JSI METAL = 0.09 (B) INPUT = 1.00



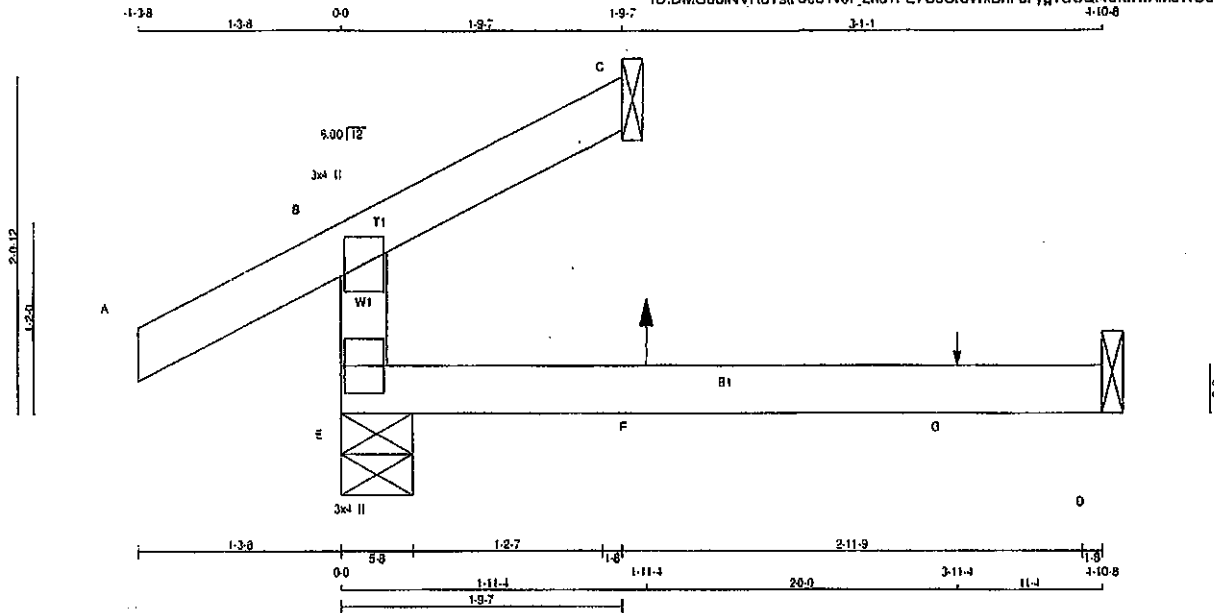
Structural component only
DWG# T-2007652

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	C45	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR6TsFoe31v61_zns1l-E7DsCr3WxBnFcFyq1UCQ146kwwAMuWC0HTL1pJzME7v

Scale = 1:13.1



TOTAL WEIGHT = 2 X 10 = 21 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
E	BMV+p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
E	282	0	282	0	0	5-8	5-8		
C	55	0	55	0	0	1-8	1-8		
D	35	0	44	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOSE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	198	137	0	0	0	81	0
C	39	21	0	0	0	18	0
D	29	0	4	0	0	31	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (LBS)	MEMB.	FORCE (LBS)	MAX. (LBS)	CSH (LBS)
FR-TO		FROM	TO	FR-TO			
E-B	-235	0	0.0	0.0	0.07 (4)	7.81	
A-B	0	28	-91.8	-91.8	0.12 (11)	10.00	
B-C	-13	5	-91.8	-91.8	0.07 (5)	6.25	
E-F	0	0	-18.5	-18.5	0.10 (4)	10.00	
F-G	0	0	-18.5	-18.5	0.10 (4)	10.00	
G-O	0	0	-18.5	-18.5	0.10 (4)	10.00	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11.4	7	1	12	BACK	VERT	TOTAL	---	C1
G	3-11.4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

155 % OF 31.3 P.S.F. G.S.L. PLUS 9.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TC=0.12/1.00 (A-B-1), BC=0.10/1.00 (D-E-4), WB=0.00/1.00 (A-B-2), SSI=0.09/1.00 (A-B-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 768 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

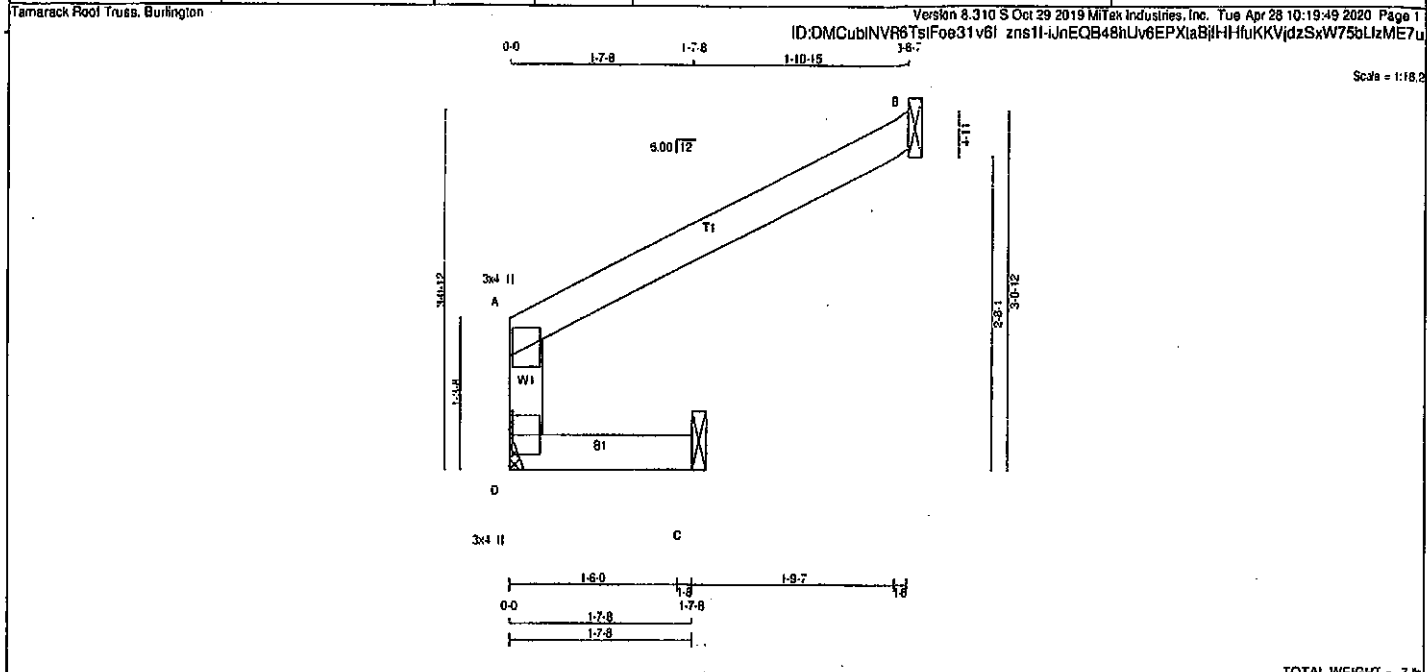
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.10 (E) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007653

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	C46	1	1	GREEN PARK HOMES	
TRUSS DESC.					



LUMBER				DESCR.	
N. L. G. A. RULES				SPF	SPF
CHORDS	SIZE	LUMBER			
O - A	2x4	DRY	No.2		
A - B	2x4	DRY	No.2		
D - C	2x4	DRY	No.2		

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMV+p	MT20	3.0	4.0	
D	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BEVERIFIED BY BUILDING DESIGNER					
BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
	VERT	DOWN	HORZ	UPLIFT	IN-SX IN-SX
D	155 0	155 0	0 0	MECHANICAL	
B	144 0	144 0	0 0	1-8 1-8	
C	55 0	55 0	0 0	1-8 1-8	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS							
JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	109	78 0	0 0	0 0	0 0	31 0	0 0
B	99	80 0	0 0	0 0	0 0	19 0	0 0
C	40	23 0	0 0	0 0	0 0	17 0	0 0

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: 14)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
D-A	-181 0	0.0 0.0 0.08 (1)	7.81				
A-B	-8 0	-91.8 -91.8 0.14 (1)	10.00				
D-C	0 0	-18.5 -18.5 0.09 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, OBC 2012, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 088-09, CSA 088-14
 - TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19')
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00')
 ALLOWABLE DEFL.(TL)= L/380 (0.19')
 CALCULATED VERT. DEFL.(TL) = L/999 (0.00')

CSI: TC=0.14/1.00 (A-B:1), BC=0.09/1.00 (C-D:1), WB=0.00/1.00 (W:0), SS1=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.99 (D) (INPUT = 0.99)
 JSI METAL = 0.05 (A) (INPUT = 1.00)

Structural component only
 DWG# T-2007654

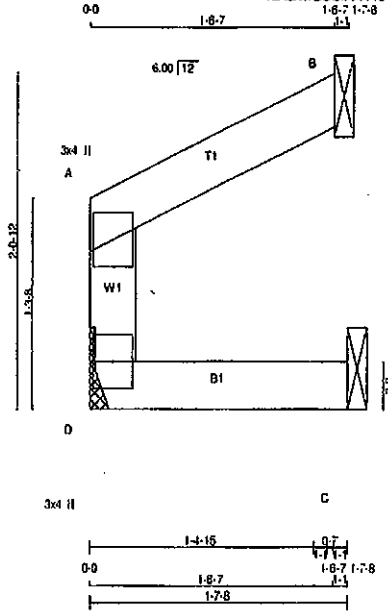


Structural component only
DWG# T-2007654

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408224	C47	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR8TslFoe31v8l zns1l-AWLddX5mSotzrY638uEuqVB5ske7MQl5knq8uCzME7t



LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 268–259.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

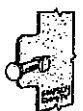
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

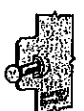
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options Information on p. 126.



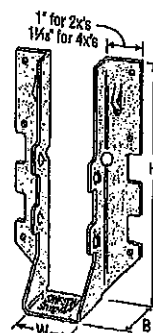
Double-Shear
Nailing
Top View



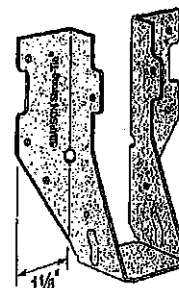
Double-Shear
Nailing
Side View;
Do not
bend tab



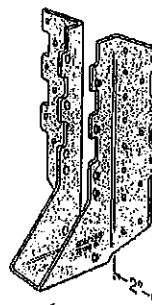
Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580



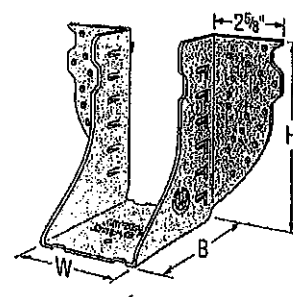
✓ LUS28



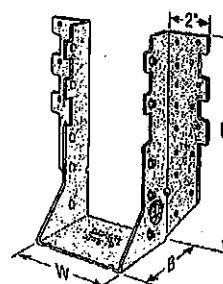
LU28L



✓ HUS210
(HUS26, HUS28,
and HHUS similar)

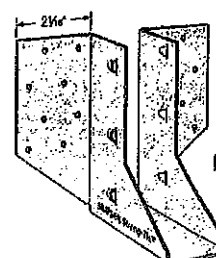
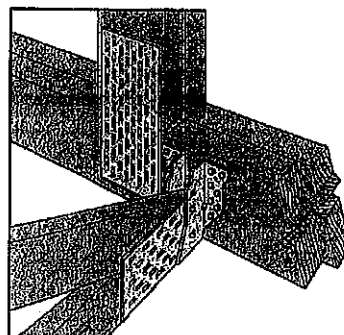


✓ HGUS28-2



✓ HHUS210-2

Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS**HHUS/HGUS**

See Hanger Options Information on pp. 125–127.

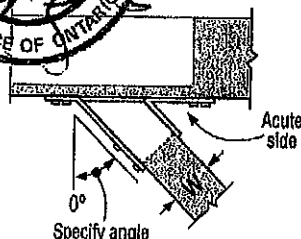
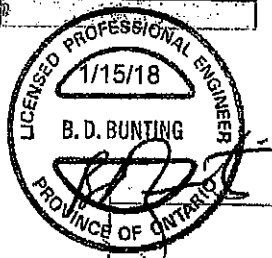
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



**Top View HHUS Hanger
Skewed Right**
(joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance				
		W	H	B	d _g ³	Header	Joist	D.Fir-L		S-P-F		
								Uplift	Normal	Uplift	Normal	
								(K _D = 1.15) lb.	(K _D = 1.00) lb.	(K _D = 1.15) lb.	(K _D = 1.00) lb.	
Single 2x Sizes												
■	LUS24	18	1 9/16	3 1/8	1 3/4	2 1/4	(4) 10d	(2) 10d	710	1626	645	1155
									3.16	7.23	2.87	5.14
■	LU24L	22	1 9/16	3	1 1/8	2 1/8	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	726
									1.60	4.54	1.42	3.22
■	LU26L	22	1 9/16	5	1 1/8	4 1/8	(6) 10d	(4) 10d x 1 1/2"	720	1606	645	1140
									3.20	7.14	2.87	5.07
SS	LUS26	18	1 9/16	4 1/4	1 3/4	3 3/4	(4) 10d	(4) 10d	1420	2170	1290	1630
									6.32	9.65	5.74	7.25
■	HUS26	18	1 9/16	5 1/8	3	3 3/8	(14) 16d	(8) 16d	2705	4940	2065	3875
									11.30	21.97	9.20	17.24
■	LJS26DS	18	1 9/16	5	3 1/2	4 1/8	(16) 16d	(6) 16d	2055	4265	1460	4115
									9.14	18.97	6.49	18.31
■	HGUS26	12	1 9/16	5 1/8	5	4 1/8	(20) 16d	(8) 16d	2885	6625	2685	5700
									11.96	29.51	11.96	25.35
■	LU28L	20	1 9/16	6 1/4	1 1/8	5 1/8	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1550
									5.07	9.72	4.54	6.89
SS	LUS28	18	1 9/16	6 1/8	1 3/4	3 3/4	(6) 10d	(4) 10d	1420	2520	1290	1790
									6.32	11.21	5.74	7.96
■	HUS28	16	1 9/16	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3605	5365	2675	4345
									16.04	23.86	11.90	19.33
■	HGUS28	12	1 9/16	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3310	6900
									14.74	34.19	14.74	30.73
■	LU210L	20	1 9/16	8	1 1/8	7 1/8	(10) 10d	(6) 10d x 1 1/2"	1140	2495	1020	1770
									5.07	11.10	4.54	7.87
SS	LUS210	18	1 9/16	7 1/8	1 3/4	3 3/4	(8) 10d	(4) 10d	1420	2785	1290	2210
									6.32	12.39	5.74	9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_g is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

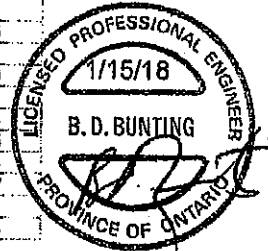
Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _a ¹	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
SS LUS24-2	18	3 3/8	3 3/8	2	1 1/2	(4) 18d	(2) 18d	835	2020	580	1435
								3.71	8.89	2.62	6.38
SS LUS26-2	18	3 3/8	4 3/8	2	4	(4) 18d	(4) 18d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS26-2	14	3 3/8	5 3/8	3	3 3/8	(14) 18d	(6) 18d	2850	7335	2085	5205
								12.68	32.63	9.20	23.15
HGUS26-2	12	3 3/8	5 3/8	4	4 3/8	(20) 18d	(8) 18d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
SS LUS28-2	18	3 3/8	7	2	4	(8) 18d	(4) 18d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS28-2	14	3 3/8	7 3/8	3	6 3/8	(22) 18d	(8) 18d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS28-2	12	3 3/8	7 3/8	4	8 3/8	(36) 18d	(12) 18d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
SS LUS210-2	18	3 3/8	9	2	6	(8) 18d	(6) 18d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HHUS210-2	14	3 3/8	9 3/8	3	8	(30) 18d	(10) 18d	4670	9660	4235	7000
								20.77	42.97	18.84	31.14
HGUS210-2	12	3 3/8	9 3/8	4	8 3/8	(48) 18d	(16) 18d	6840	14015	4855	10270
								30.43	62.34	21.60	45.60
Triple 2x Sizes											
HGUS26-3	12	4 3/8	5 1/2	4	4 3/8	(20) 18d	(8) 18d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-3	12	4 3/8	7 1/4	4	6 3/8	(36) 18d	(12) 18d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-3	14	4 3/8	9	3	7 3/8	(30) 18d	(10) 18d	4670	9670	4235	6865
								20.77	43.02	18.84	30.54
HGUS210-3	12	4 3/8	9 1/4	4	8 3/8	(48) 18d	(16) 18d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	6 3/8	5 3/8	4	4 3/8	(20) 18d	(8) 18d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-4	12	6 3/8	7 3/8	4	6 3/8	(36) 18d	(12) 18d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-4	14	8 3/8	8 3/8	3	7 3/8	(30) 18d	(10) 18d	4670	10155	4235	7210
								20.77	45.17	18.84	32.07
HGUS210-4	12	6 3/8	9 3/8	4	8 3/8	(48) 18d	(16) 18d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
HGUS212-4	12	6 3/8	10 3/8	4	10 3/8	(56) 18d	(20) 18d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS214-4	12	6 3/8	12 3/8	4	11 3/8	(66) 18d	(22) 18d	10130	16400	7195	11645
								45.06	72.95	32.00	61.80
4x Sizes											
LUS46	18	3 3/8	4 3/8	2	3 3/8	(4) 18d	(4) 18d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS46	14	3 3/8	5 3/8	3	3 3/8	(14) 18d	(6) 18d	2540	7335	2085	5205
								11.30	32.63	9.20	23.15
HGUS46	12	3 3/8	5 3/8	4	4 3/8	(20) 18d	(8) 18d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS48	18	3 3/8	6 3/8	2	3 3/8	(6) 18d	(4) 18d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS48	14	3 3/8	7 3/8	3	6 3/8	(22) 18d	(8) 18d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS48	12	3 3/8	7 3/8	4	6 3/8	(36) 18d	(12) 18d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS410	18	3 3/8	8 3/8	2	5 3/8	(8) 18d	(6) 18d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HGUS410	12	3 3/8	9	4	8 3/8	(48) 18d	(16) 18d	6840	14015	4855	10270
								30.43	62.34	21.60	45.60
HGUS412	12	3 3/8	10 3/8	4	10 3/8	(56) 18d	(20) 18d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS414	12	3 3/8	12 3/8	4	11 3/8	(66) 18d	(22) 18d	10130	16400	7195	11645
								45.06	72.95	32.00	61.80



Plated Truss Connectors

See footnotes
on p. 258.

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1 1/4". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

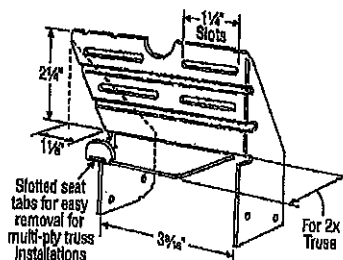
Design: Factored resistances are in accordance with CSA 086-14

Installation:

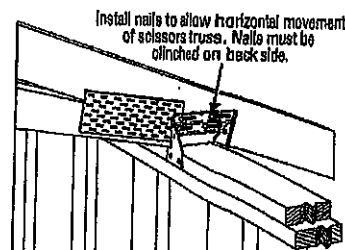
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

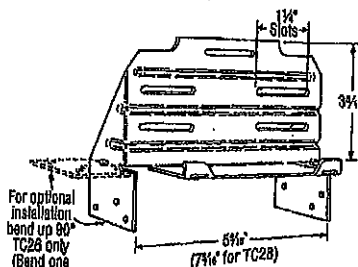
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



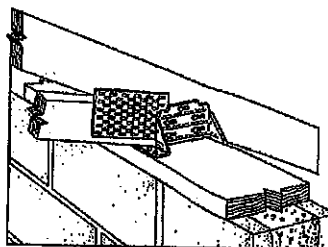
TC24
U.S. Patent 4,952,173



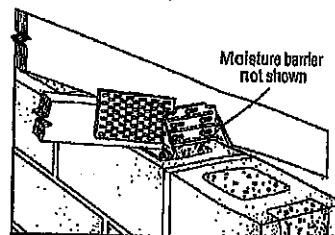
Typical TC24 Installation



TC26
(TC28 Similar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer (8", 10", 12" Wall Installation Similar)



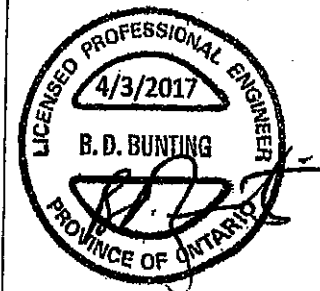
Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fk-L	S-P-F
			Uplift (K _c =1.15)	Uplift (K _c =1.15)
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fk-L	S-P-F
			Uplift (K _c =1.15)	Uplift (K _c =1.15)
TC26	(5) 10d	(8) 10d x 1 1/2"	810	680
	(5) 10d	(8) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC28 fastened to grouted concrete block with (8) - 9/16" x 2 1/4" Titen screws has a factored uplift resistance of 275 lb.



UNITED STATES
DESIGN

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SECTION 05120 - ROOFING

(800) 999-5099
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Seismic and Hurricane Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Fasteners			Factored Resistance ($K_p = 1.15$)					
					D.Fir-L			S-P-F		
		To Rafters/Truss	To Plates	To Studs	Uplift	Lateral		Uplift	Lateral	
						F ₁	F ₂		F ₁	F ₂
					lb.	lb.	lb.	lb.	lb.	lb.
					kN	kN	kN	kN	kN	kN
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
					3.29	3.05	1.33	3.02	2.16	0.96
SS H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
					3.69	0.98	0.33	2.62	0.69	0.24
SS H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
					3.58	0.71	0.71	3.36	0.71	0.71
H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
					3.71	0.78	0.93	3.29	0.71	0.93
SS H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	180
					3.29	0.80	1.18	2.74	0.66	0.85
H8	16	—	(3) 8d	(8) 8d	1585	1085	—	1125	770	—
					7.05	4.83	—	5.00	3.43	—
H7Z	16	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
					6.18	2.98	—	4.40	2.11	—
SS H8 ³	18	(5) 10d x 1½"	(5) 10d x 1½"	—	1120	—	—	1025	—	—
					4.98	—	—	4.56	—	—
SS H10A ³	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290
					7.72	3.54	1.82	6.69	2.51	1.29
H10AR	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1485	690	430	1220	570	305
					6.61	3.07	1.91	5.43	2.54	1.36
H10A-2	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1835	1275	430	1645	880	305
					8.16	5.67	1.91	7.32	3.91	1.36
H10S ^{3,4}	18	(8) 8d x 1½"	(8) 8d x 1½"	(8) 8d	1465	795	315	1040	565	225
					6.52	3.54	1.40	4.63	2.51	1.00
H11Z	18	(6) 16d x 2½"	(6) 16d x 2½"	—	1095	920	545	780	655	390
					4.87	4.09	2.42	3.47	2.91	1.73
H14	18	1 (12) 8d x 1½"	(13) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
		2 (12) 8d x 1½"	(15) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
TSP	18	(9) 10d x 1½"	(6) 10d x 1½"	—	1295	440	—	920	310	—
					5.76	1.98	—	4.09	1.38	—
		(9) 10d x 1½"	(6) 10d	—	1560	440	—	1105	310	—
					6.94	1.98	—	4.92	1.38	—

- Factored resistances have been increased 16% for short term loading; no further increase is allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
- H8 factored uplift resistances for stud-to-bottom plate installations are 585 lb. (2.65 kN) for D.Fir-L and 390 lb. (1.74 kN) for S-P-F.
- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-H-TIECONPATH).
- Factored resistances in the F₁ direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.

- H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.38 kN) D.Fir-L and 1015 lb. (4.51 kN) S-P-F.
- H10S nails to plates are optional for uplift but required for lateral loads.
- H10A may be field-bent up to a slope of 6/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
- The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SSNAILS at strongtie.com.
- D.Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1½" nails into the top plates and (3) 8d x 1½" nails into the lowest three flange holes into the truss bottom chord is 495 lb. (2.20 kN).
- Nails: 16d x 2½" = 0.162" dia. x 2½" long, 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long, 8d x 1½" = 0.131" dia. x 1½" long. See pp. 27-28 for other nail sizes and information.

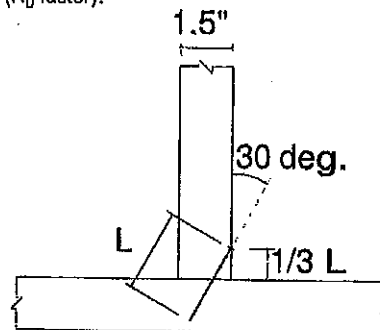
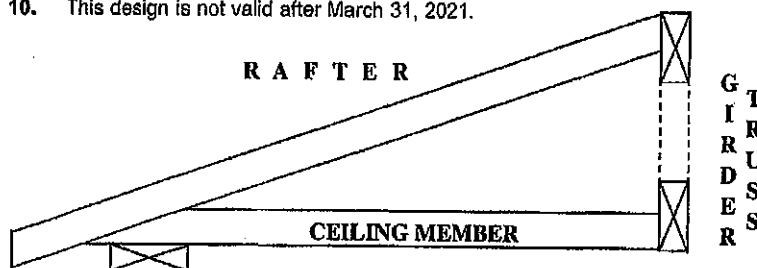
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.



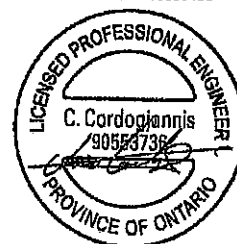
TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (In)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019

PEO
Certificate No. 10889485



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

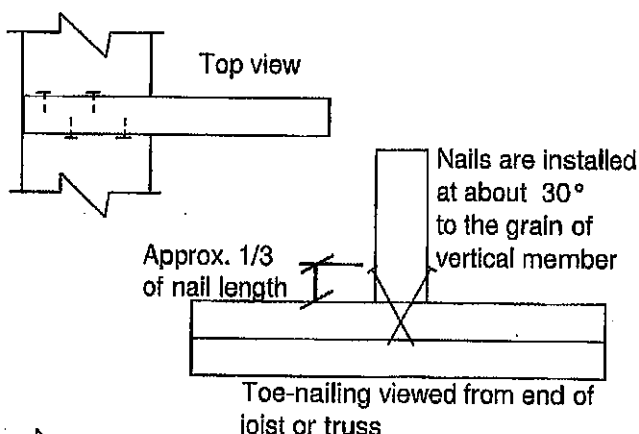
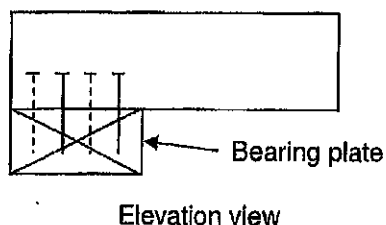
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

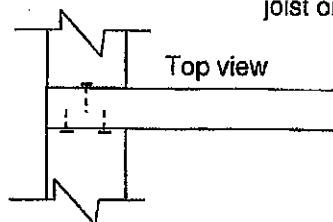
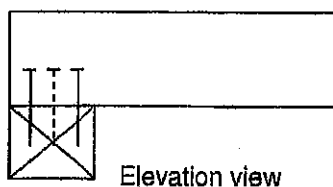
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37679H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



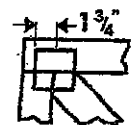
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Bradford, Ontario L3Z 3G7



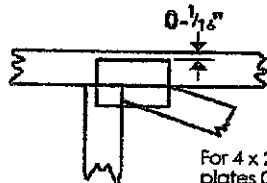
December 2, 2019

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/4" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



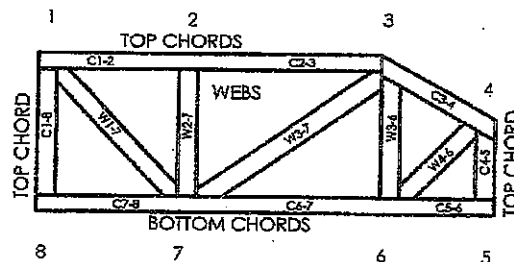
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MIT-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

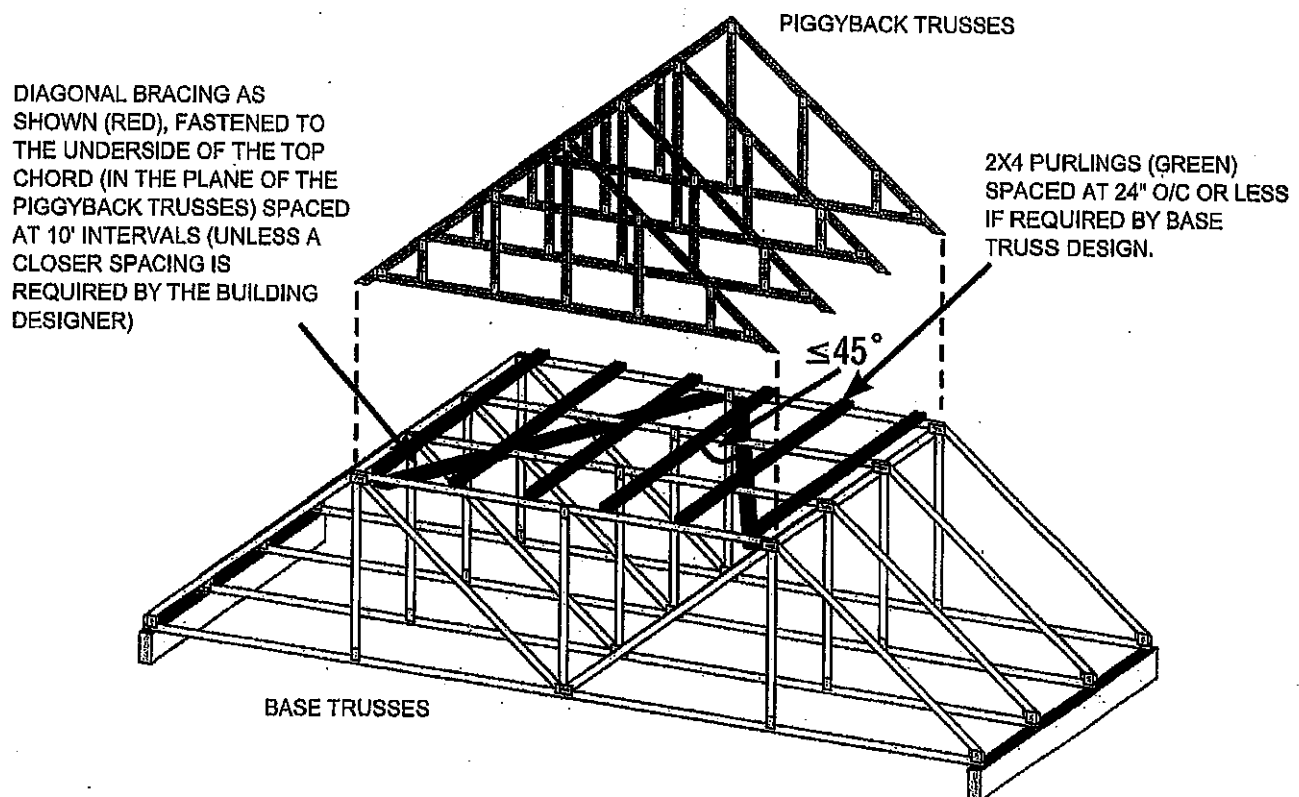
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane of joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

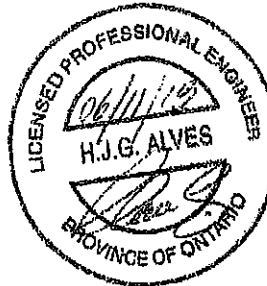


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.



Alves Engineering Services Inc.

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RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufacturer's responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1300213

Feb 09, 2018