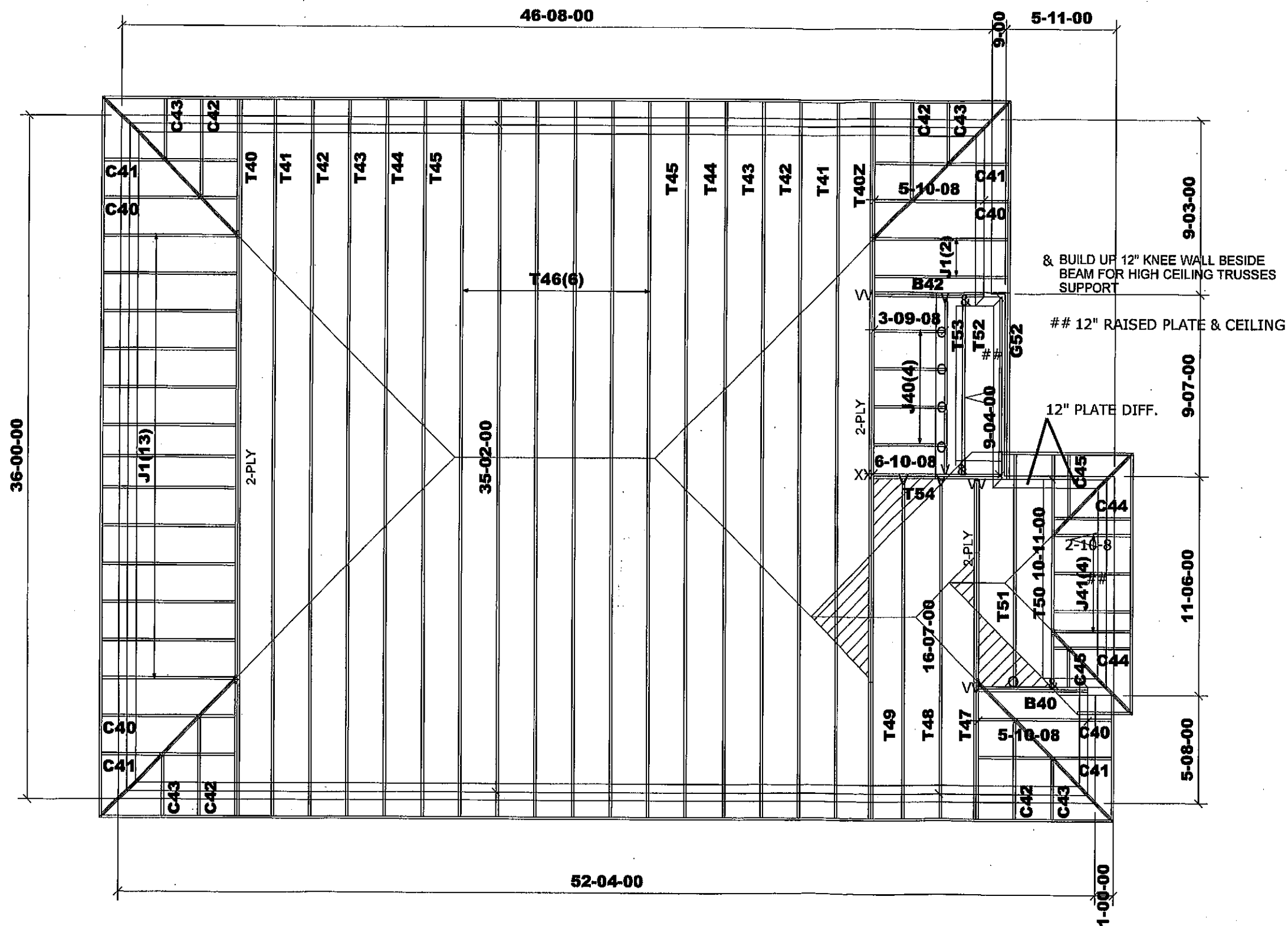


6/12 roof pitch unless noted



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment).
Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

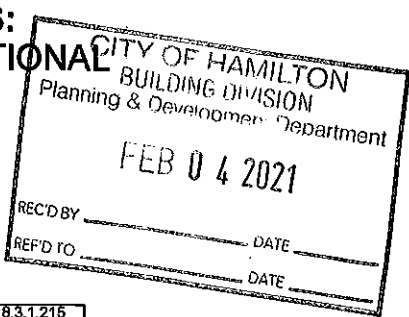
DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
LUS26-2-(VV)

BEAMS:
B40= B41 = B42=
2 - 2x10 SPF #2

 DENOTES:
CONVENTIONAL
FRAMING



M13125



Job Track: 51225
Plan Log: 202401
Layout ID: 408152

Builder / Location: GREEN PARK HOMES / WATERDOWN
Project: RUSSELL GARDENS PH.3
Date: 2020-04-27 Sales: Mario DiCano Designer: JG

Model / Elevation: MOUNTAINASH 4/3- std. or opt. LOT 217

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DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 4
 Lot #: 217
 Elevation: 3- std. or opt.

Job Track: 51225
 PlanLog: 202401
 Layout ID: 408152
 Ref #
 Page: 1 of 3
 Date: 04-27-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T40 Hip Girder	6/12	35-02-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	340.74 212.00		
	1 2-ply	T40Z Hip Girder	6/12	35-02-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	340.74 212.00		
	2	T41 Hip	6/12	35-02-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	277.99 174.00		
	2	T42 Hip	6/12	35-02-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	277.12 174.67		
	2	T43 Hip	6/12	35-02-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	289.07 179.67		
	2	T44 Hip	6/12	35-02-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	290.73 183.33		
	2	T45 Hip	6/12	35-02-00	9-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	301.67 191.33		
	6	T46 Common	6/12	35-02-00	9-11-08	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	910.08 558.00		
	1 2-ply	T47 Hip Girder	6/12	16-07-00	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 2-02-00	158.02 100.67		
	1	T48 Hip	6/12	16-07-00	5-01-04	2 x 4	1-03-08	1-02-00 2-02-00	68.15 43.33		
	1	T49 Common	6/12	16-07-00	5-09-12	2 x 4	1-03-08	1-02-00 2-02-00	67.14 42.33		
	1	T50 Hip Girder	6/12	10-11-00	2-07-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	42.73 28.33		
	1	T51 Hip	6/12	10-11-00	4-07-04	2 x 4	1-03-08 1-03-08	2-02-00 2-02-00	51.54 34.50		
	1	T52 Flat	0/12	9-04-00	2-10-00	2 x 4		2-10-00 2-10-00	38.11 23.83		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 4
 Lot #: 217
 Elevation: 3- std. or opt.

Job Track: 51225
 PlanLog: 202401
 Layout ID: 408152
 Ref #
 Page: 2 of 3
 Date: 04-27-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	G52 GABLE	0 /12	9-04-00	2-10-00	2 x 4		2-10-00 2-10-00	33.75 23.33		
	1	T53 Flat Girder	0 /12	9-04-00	3-10-00	2 x 6		3-10-00 3-10-00	50.9 31.50		
	1 2-ply	T54 Jack-Closed Girder	6 /12	6-10-08	4-01-04	2 x 4 2 x 6		8-00 4-01-04	59.61 37.33		
	15	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	251.92 160.00		
	4	J40 Jack-Open	6 /12	3-09-08	4-01-04	2 x 4		2-02-08 4-01-04	52.34 35.33		
	4	J41 Jack-Open	6 /12	2-10-08	2-07-04	2 x 4	1-03-08	1-02-00 2-07-04	38.2 24.00		
	4	C40 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	56.53 34.67		
	4	C41 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	46.33 29.33		
	4	C42 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	38.28 24.00		
	4	C43 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	28.08 18.67		
	2	C44 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 11-09	1-02-00 2-01-08	16.64 10.67		
	2	C45 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 1-01	1-02-00 2-01-08	14.65 9.33		

TOTAL # TRUSS= 74

TOTAL BFT OF ALL TRUSSES= 2596.15

BFT.

TOTAL WEIGHT OF ALL TRSSES 4137.05 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
4	Hardware	LJS26DS	

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
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 Location: WATERDOWN
 Model: MOUNTAINASH 4
 Lot #: 217
 Elevation: 3- std. or opt.

Job Track: 51225
 PlanLog: 202401
 Layout ID: 408152
 Ref #
 Page: 3 of 3
 Date: 04-27-2020
 Designer:
 Sales Rep: Mario DiCano

HARDWARE

QTY	TYPE	MODEL	LENGTH
5	Hardware	LUS24	
3	Hardware	LUS26-2	

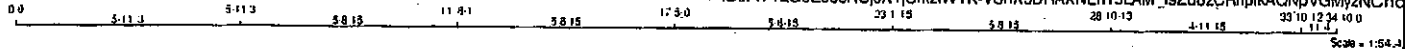
TOTAL NUMBER OF
 ITEMS= 13

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408150	T1	1	2	TRUSS DESC.		

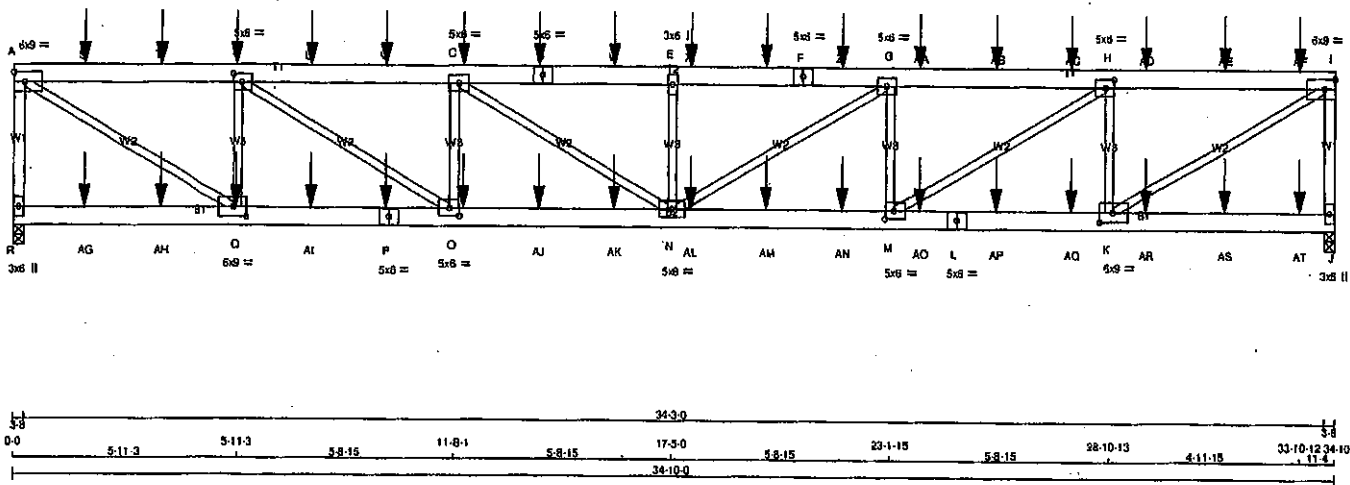
Tamarack Roof Truss, Burlington

Version 8.310 9 Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 11:02:47 2020 Page 1

ID:VF7aG0E03cRUj6X1[SrkziWYK-VShX5DRAXNEN1SLAM_IsZu8zCRhpKtAQNPVGMzNCnc



Scale = 1/8" = 1'-0"



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408150	T1	1	2	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitak Industries, Inc. Sat Apr 25 11:02:47 2020 Page 2
ID:7vF7aG0E03cRU6X1iSkzWYK-VShX5DRAXNen15LAM IsZu8zGRhoItAQNpVGMvzNCnc

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	6.0	9.0		Edge
B	TMWW-t	MT20	5.0	8.0	2.50	2.75
C	TMWW-t	MT20	5.0	8.0		
D	TS-t	MT20	5.0	8.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	8.0		
G	TMWW-t	MT20	5.0	8.0		
H	TMWW-t	MT20	5.0	8.0	2.50	2.75
I	TMWW-t	MT20	8.0	9.0		Edge
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	8.0	9.0	3.00	4.25
L	BS-t	MT20	5.0	8.0		
M	BMWW-t	MT20	5.0	8.0	2.50	2.75
N	BMWW-t	MT20	5.0	8.0		
O	BMWW-t	MT20	5.0	8.0	2.50	2.75
P	BS-t	MT20	5.0	8.0		
Q	BMWW-t	MT20	8.0	9.0	3.00	4.25
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. FACTORED UNBRAC. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)
FR-TO		FROM	TO		LENGTH	FR-TO	
K-AR	0/0	-18.5	-18.5	0.08 (4)	10.00		
AR-AS	0/0	-18.5	-18.5	0.08 (4)	10.00		
AS-AT	0/0	-18.5	-18.5	0.08 (4)	10.00		
AT-J	0/0	-18.5	-18.5	0.08 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

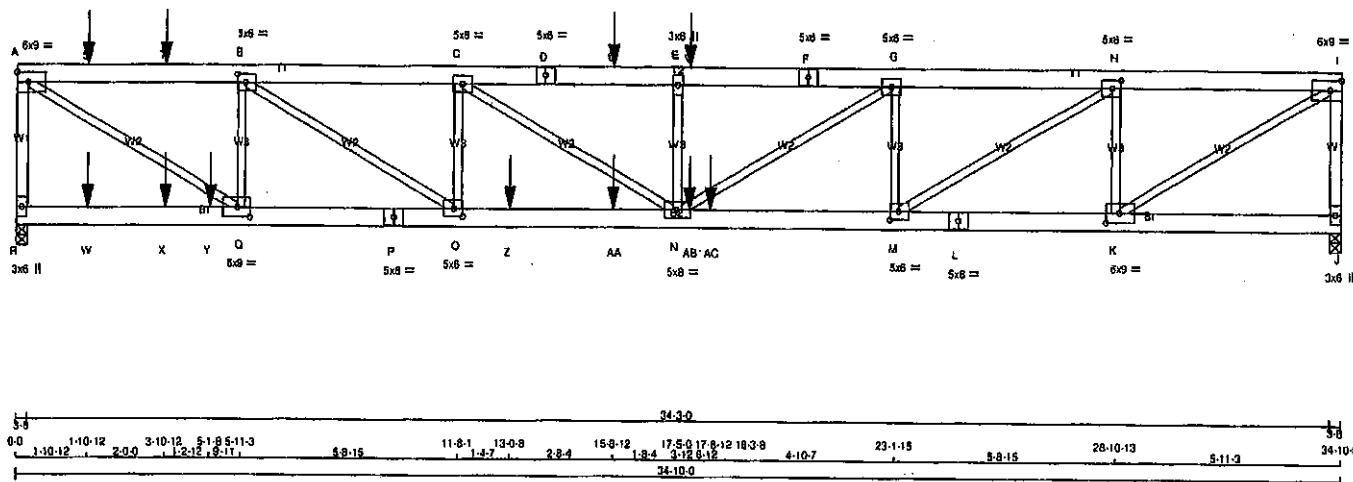
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	5-10-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
C	11-10-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
D	13-10-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
O	11-10-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	9-10-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Q	5-10-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
S	1-10-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
T	3-10-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
U	7-10-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	9-10-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	15-10-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	17-10-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	19-10-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	21-10-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	23-10-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	25-10-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	27-10-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	29-10-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AE	31-10-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AF	33-10-12	-113	-113	---	FRONT	VERT	TOTAL	---	C1
AG	1-10-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AH	3-10-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AI	7-10-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AJ	13-10-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AK	15-10-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AL	17-10-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AM	19-10-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AN	21-10-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AO	23-10-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AP	25-10-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AQ	27-10-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AR	29-10-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AS	31-10-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AT	33-10-12	-27	-27	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007063 3/2



LUMBER	SIZE	DRY	NO.2	DESCR.
N. L. G. A. RULES				
CHORDS				
R - A	2x4	DRY	No.2	SPF
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
L - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS: (0.122'X3") SPIRAL NAILS		
R - A	12	TOP
I - J	12	TOP
A - D	12	SIDE(0.0)
D - F	12	SIDE(183.1)
F - I	12	TOP
BOTTOM CHORDS: (0.122'X3") SPIRAL NAILS		
R - P	12	SIDE(0.0)
P - L	12	SIDE(183.1)
L - J	12	TOP
WEBS: (0.122'X3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG	IN-SX	IN-SX		
R	4002	0	4002	0	0	3-8	3-8		
J	3132	0	3132	0	0	3-8	3-8		

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
R	2824	1886 / 0	0 / 0	0 / 0	0 / 0	938 / 0	0 / 0		
J	2212	1470 / 0	0 / 0	0 / 0	0 / 0	742 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.84 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FORCE (LBS)	MAX. CS (LC)	
FR-TO				FR-TO			
R-A	-3951 / 0	0.0	0.0 0.44 (1)	6-01	K-I	0 / 5428	0.87 (1)
A-S	-5722 / 0	-91.8	-91.8 0.25 (1)	4-72	A-Q	0 / 6717	0.83 (1)
S-T	-5722 / 0	-91.8	-91.8 0.25 (1)	4-72	K-H	-2703 / 0	0.33 (1)
T-B	-5722 / 0	-91.8	-91.8 0.25 (1)	4-72	Q-B	-2710 / 0	0.32 (1)
B-C	-8846 / 0	-91.8	-91.8 0.27 (1)	3-93	M-H	0 / 4038	0.50 (1)
C-D	-10219 / 0	-91.8	-91.8 0.34 (1)	3-84	B-C	0 / 3700	0.46 (1)
D-U	-10219 / 0	-91.8	-91.8 0.34 (1)	3-84	M-G	-1801 / 0	0.23 (1)
U-E	-10219 / 0	-91.8	-91.8 0.34 (1)	3-84	C-C	-1400 / 0	0.17 (1)
E-V	-10219 / 0	-91.8	-91.8 0.31 (1)	3-87	N-G	0 / 2591	0.32 (1)
V-F	-10219 / 0	-91.8	-91.8 0.31 (1)	3-67	C-N	0 / 1828	0.20 (1)
F-G	-10219 / 0	-91.8	-91.8 0.31 (1)	3-67	N-E	-710 / 0	0.08 (1)
G-H	-8031 / 0	-91.8	-91.8 0.23 (1)	4-14			
H-I	-4622 / 0	-91.8	-91.8 0.15 (1)	6-25			
I-J	-3071 / 0	0.0	0.0 0.35 (1)	6-80			

R-W	0 / 0	-18.5	-18.5 0.15 (1)	10-00
W-X	0 / 0	-18.5	-18.5 0.15 (1)	10-00
X-Y	0 / 0	-18.5	-18.5 0.15 (1)	10-00
Y-Q	0 / 0	-18.5	-18.5 0.15 (1)	10-00
Q-P	0 / 5722	-18.5	-18.5 0.44 (1)	10-00
P-O	0 / 5722	-18.5	-18.5 0.44 (1)	10-00
O-Z	0 / 8846	-18.5	-18.5 0.81 (1)	10-00
Z-AA	0 / 8846	-18.5	-18.5 0.81 (1)	10-00
AA-N	0 / 8846	-18.5	-18.5 0.81 (1)	10-00
N-AB	0 / 8031	-18.5	-18.5 0.74 (1)	10-00
AB-AC	0 / 8031	-18.5	-18.5 0.74 (1)	10-00
AC-M	0 / 8031	-18.5	-18.5 0.74 (1)	10-00
M-L	0 / 4622	-18.5	-18.5 0.33 (1)	10-00
L-K	0 / 4622	-18.5	-18.5 0.33 (1)	10-00
K-J	0 / 0	-18.5	-18.5 0.04 (4)	10-00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
S	1-10-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
T	3-10-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
U	15-8-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
V	17-8-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
W	1-10-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
X	3-10-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
Y	5-4-8	-727	-727	---	BACK	VERT	TOTAL	---	C1
Z	13-0-8	-727	-727	---	BACK	VERT	TOTAL	---	C1
AA	15-8-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
AB	17-8-12	-26	-26	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.18")
CALCULATED VERT. DEFL.(LL) = L/999 (0.28")
ALLOWABLE DEFL.(TL) = L/360 (1.18")
CALCULATED VERT. DEFL.(TL) = L/828 (0.50")

CSI: TC=0.44/1.00 (A-R:1), SC=0.81/1.00 (N-O:1),
WB=0.83/1.00 (A-Q:1), SS=0.41/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1997 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (M) (INPUT = 0.90)
JSI METAL = 0.50 (P) (INPUT = 1.00)



Structural component only
DWG# T-2007064 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408150	T1Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: i7vF7aG0E03cRUj6X1ISkzWYK-SrplVvtQ3 UVGPVZTPnKeJDIEFIF7bWigZ NRrzNCna

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	6.0	9.0	Edge
B	TMWW-t	MT20	5.0	6.0	2.50 2.75
C	TMWW-t	MT20	5.0	6.0	
D	TS-t	MT20	5.0	6.0	
E	TMW+w	MT20	3.0	6.0	
F	TS-t	MT20	5.0	6.0	
G	TMWW-t	MT20	5.0	6.0	
H	TMWW-t	MT20	5.0	6.0	2.50 2.75
I	TMVW-t	MT20	6.0	9.0	Edge
J	BMV1+p	MT20	3.0	6.0	
K	BMWW-t	MT20	6.0	9.0	3.00 4.25
L	BS-t	MT20	6.0	6.0	
M	BMWW-t	MT20	5.0	6.0	2.50 2.75
N	BMWW-t	MT20	5.0	6.0	
O	BMWW-t	MT20	5.0	6.0	2.50 2.75
P	BS-t	MT20	5.0	6.0	
Q	BMWW-t	MT20	6.0	9.0	3.00 4.25
R	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AC	18-3-8	-1293	-1293		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

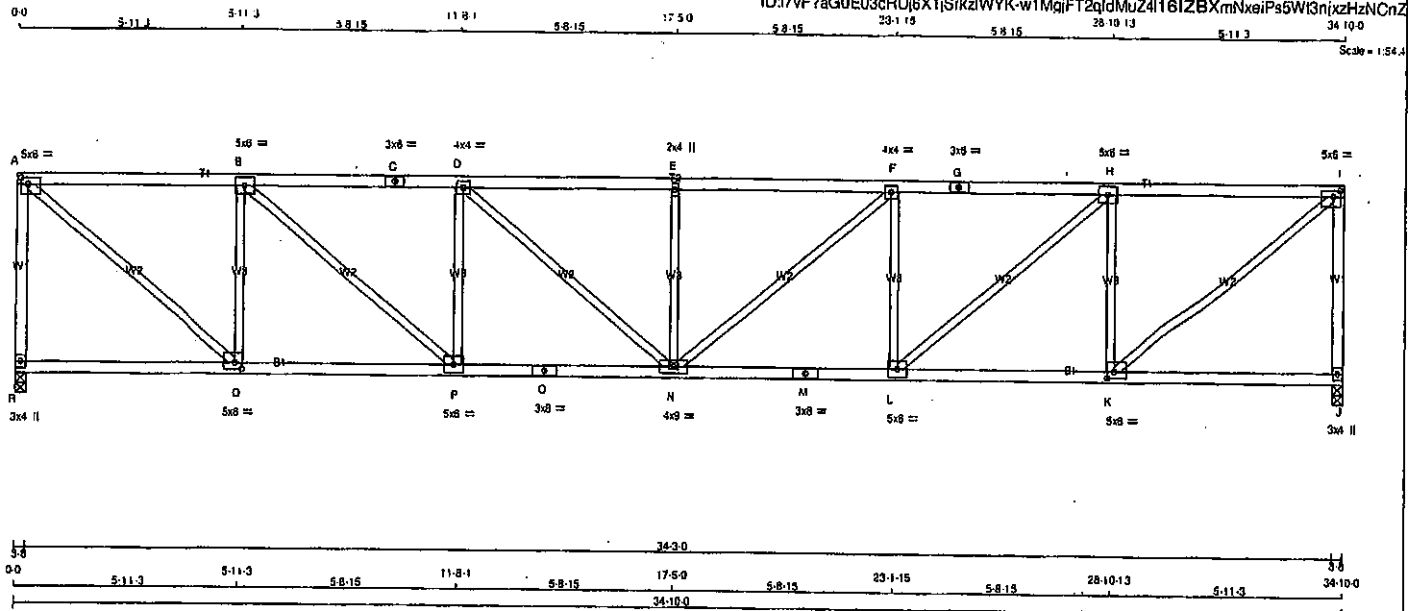
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007064 72

JOB NAME 408150	TRUSS NAME T2	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Famarack Roof Truss, Burlington				TRUSS DESC.	

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ID:7vF7aG0E03cRUj6X1jSrkzWYK-w1MqjFT2qldMuZ416IZBXmNxeiPs5W3n(xzHzNcNz



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
R - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
J - I	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
A TMVW-I	MT20	5.0	8.0	2.00 2.25
B TMVW-I	MT20	5.0	8.0	
C TS-I	MT20	3.0	8.0	
D TMVW-I	MT20	4.0	4.0	
E TMVW-I	MT20	2.0	4.0	
F TMVW-I	MT20	4.0	4.0	
G TS-I	MT20	3.0	8.0	
H TMVW-I	MT20	5.0	8.0	
I TMVW-I	MT20	5.0	8.0	2.00 2.25
J BMVW-I	MT20	3.0	4.0	
K BMVW-I	MT20	5.0	8.0	2.00 2.25
L BMVW-I	MT20	5.0	8.0	
M BS-I	MT20	3.0	8.0	
N BMVW-I	MT20	4.0	9.0	
O BS-I	MT20	3.0	8.0	
P BMVW-I	MT20	5.0	8.0	
Q BMVW-I	MT20	5.0	8.0	2.00 2.25
R BMVW-I	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
R	1920	0	1920	0	0	3-8	3-8		
J	1920	0	1920	0	0	3-8	3-8		

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
R	1358	891 / 0	0 / 0	0 / 0	0 / 0	467 / 0	0 / 0		
J	1358	891 / 0	0 / 0	0 / 0	0 / 0	467 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD I.C.I	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD I.C.I
	FORCE (LBS)	FROM TO	PLF		FORCE (LBS)	FROM TO	PLF
R-A	-1675 / 0	0.0	0.0 0.83 (1)	K-I	0 / 2630	0.59 (1)	
A-B	-2042 / 0	-91.8	-91.8 0.59 (1)	A-Q	0 / 2630	0.59 (1)	
B-C	-3149 / 0	-91.8	-91.8 0.74 (1)	K-H	-1535 / 0	0.59 (1)	
C-D	-3149 / 0	-91.8	-91.8 0.74 (1)	Q-B	-1535 / 0	0.59 (1)	
D-E	-3533 / 0	-91.8	-91.8 0.88 (1)	L-H	0 / 1444	0.33 (1)	
E-F	-3533 / 0	-91.8	-91.8 0.88 (1)	B-P	0 / 1444	0.33 (1)	
F-G	-3149 / 0	-91.8	-91.8 0.74 (1)	L-F	-826 / 0	0.32 (1)	
G-H	-3149 / 0	-91.8	-91.8 0.74 (1)	P-D	-826 / 0	0.32 (1)	
H-I	-2042 / 0	-91.8	-91.8 0.59 (1)	N-F	0 / 502	0.11 (1)	
I-J	-1875 / 0	0.0	0.0 0.83 (1)	D-N	0 / 502	0.11 (1)	
				N-E	-537 / 0	0.21 (1)	
R-Q	0 / 0	-18.5	-18.5 0.15 (4)				
Q-P	0 / 2042	-18.5	-18.5 0.40 (1)				
P-O	0 / 3149	-18.5	-18.5 0.58 (1)				
O-N	0 / 3149	-18.5	-18.5 0.58 (1)				
N-M	0 / 3149	-18.5	-18.5 0.58 (1)				
M-L	0 / 3149	-18.5	-18.5 0.58 (1)				
L-K	0 / 2042	-18.5	-18.5 0.40 (1)				
K-J	0 / 0	-18.5	-18.5 0.15 (4)				

TOTAL WEIGHT = 2 X 143 = 286 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, OBC 2012, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.16")
CALCULATED VERT. DEFL.(LL) = $L/998$ (0.23")
ALLOWABLE DEFL.(TL) = $L/360$ (1.16")
CALCULATED VERT. DEFL.(TL) = $L/998$ (0.42")

CSI: TC=0.83/1.00 (A-R:1), BC=0.58/1.00 (N-P:1),
WB=0.69/1.00 (I-K:1), SS=0.25/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

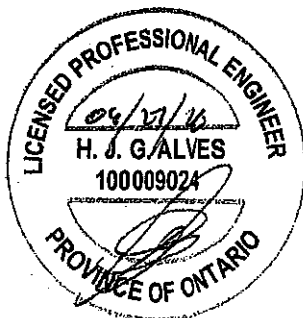
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (A) (INPUT = 0.90)
JSI METAL = 0.99 (M) (INPUT = 1.00)

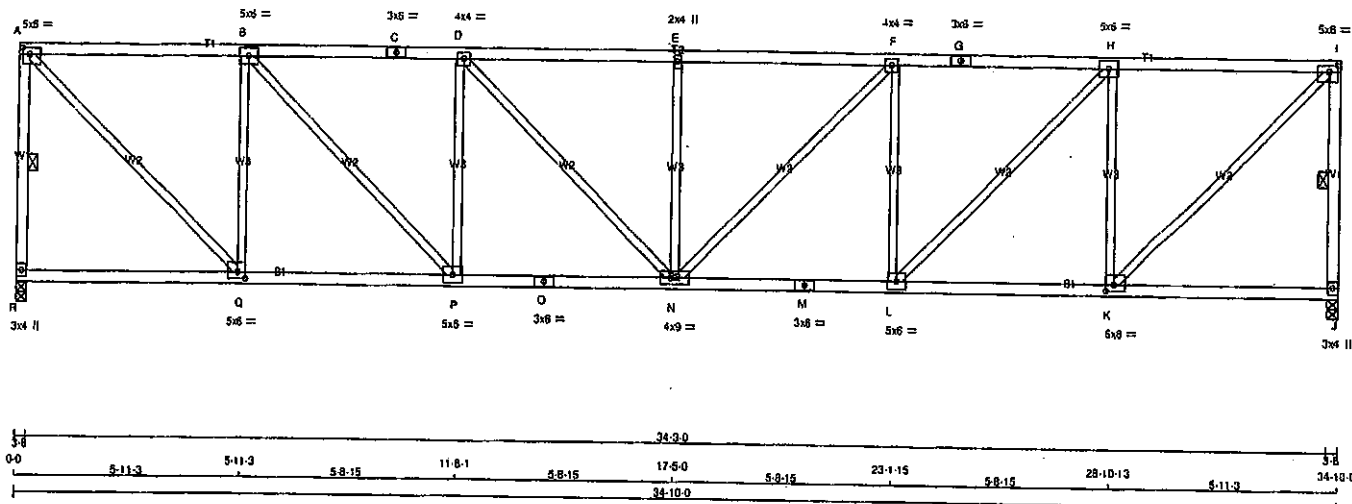
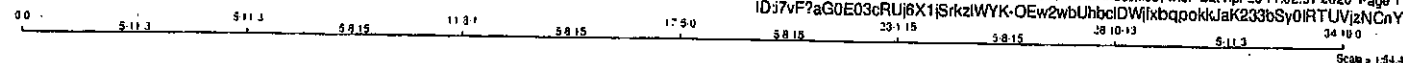


Structural component only
DWG# T-2007065

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408150	T3	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
R - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
C - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - J	2x4	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG
R	1920	0	1920	0	0	3-8	IN-SX
J	1920	0	1920	0	0	3-8	3-8

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED						
R	1358	891 / 0	0 / 0	0 / 0	0 / 0	467 / 0	0 / 0
J	1358	891 / 0	0 / 0	0 / 0	0 / 0	467 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-R, I-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LCI MAX (CSI (LC))	MAX. UNBRACED LENGTH FR-TO	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH FR-TO
R-A	-1875 / 0	0.0	0.0 0.33 (1)	4.90	K-I	0 / 2388	0.53 (1)	
A-B	-1891 / 0	-91.8	-91.8 0.55 (1)	4.46	A-Q	0 / 2388	0.53 (1)	
B-C	-2808 / 0	-91.8	-91.8 0.66 (1)	3.63	K-H	-1536 / 0	0.90 (1)	
C-D	-2808 / 0	-91.8	-91.8 0.66 (1)	3.63	Q-B	-1536 / 0	0.90 (1)	
D-E	-2927 / 0	-91.8	-91.8 0.58 (1)	3.59	L-H	0 / 1304	0.29 (1)	
E-F	-2927 / 0	-91.8	-91.8 0.58 (1)	3.59	B-P	0 / 1304	0.29 (1)	
F-G	-2608 / 0	-91.8	-91.8 0.66 (1)	3.63	L-F	-828 / 0	0.48 (1)	
G-H	-2608 / 0	-91.8	-91.8 0.66 (1)	3.63	P-D	-828 / 0	0.48 (1)	
H-I	-1891 / 0	-91.8	-91.8 0.55 (1)	4.46	N-F	0 / 453	0.10 (1)	
I-J	-1875 / 0	0.0	0.0 0.33 (1)	4.90	D-N	0 / 453	0.10 (1)	
					N-E	-537 / 0	0.31 (1)	
R-Q	0 / 0	-18.5	-18.5 0.15 (4)	10.00				
Q-P	0 / 1691	-18.5	-18.5 0.35 (1)	10.00				
P-O	0 / 2608	-18.5	-18.5 0.47 (1)	10.00				
O-N	0 / 2608	-18.5	-18.5 0.47 (1)	10.00				
N-M	0 / 2608	-18.5	-18.5 0.47 (1)	10.00				
M-L	0 / 2608	-18.5	-18.5 0.47 (1)	10.00				
L-K	0 / 1691	-18.5	-18.5 0.35 (1)	10.00				
K-J	0 / 0	-18.5	-18.5 0.15 (4)	10.00				

TOTAL WEIGHT = 2 X 153 = 305 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2010, NBCO 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCO 2010, NBCO 2015
- PART 9 OF NBCO 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.16")
CALCULATED VERT. DEFL. (LL) = L/999 (0.16")
ALLOWABLE DEFL. (TL) = L/360 (1.16")
CALCULATED VERT. DEFL. (TL) = L/999 (0.33")

CSI: TC=0.68/1.00 (F-H:1), BC=0.47/1.00 (N-P:1), WB=0.90/1.00 (H-K:1), SI=0.25/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20, 618 354 1887 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

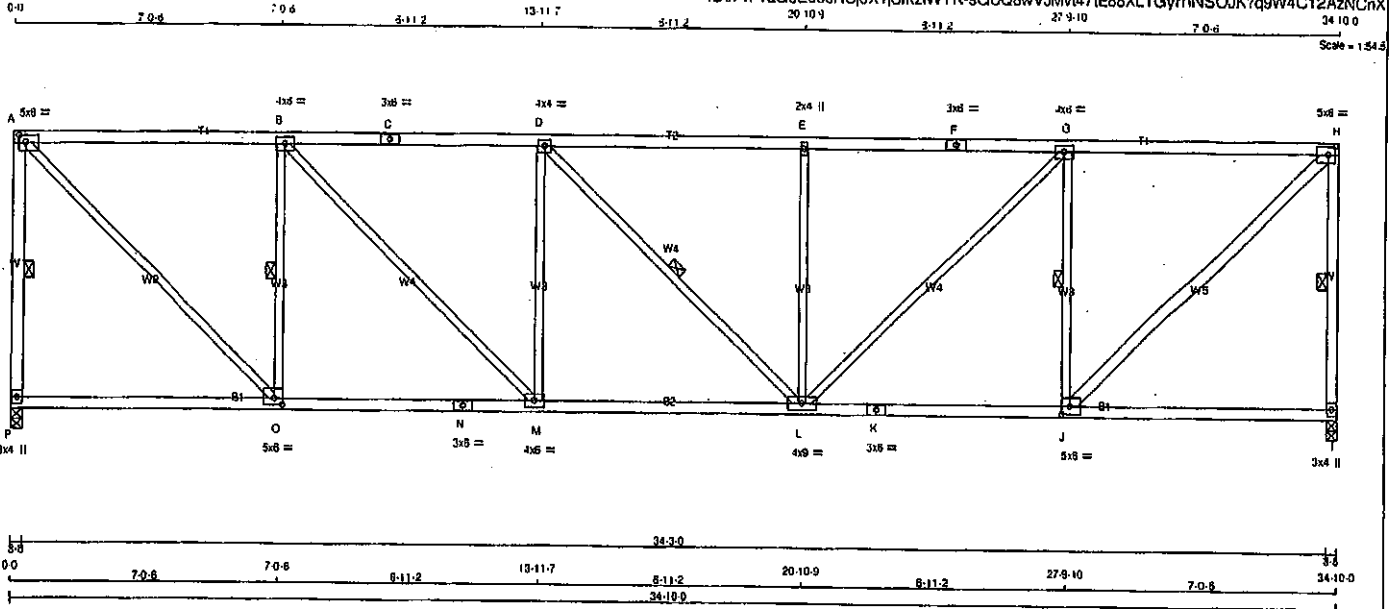
JSI GRIP = 0.88 (I) (INPUT = 0.90)
JSI METAL = 0.83 (M) (INPUT = 1.00)



Structural component only
DWG# T-2007066

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408150	T4	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:7vF?ag0E03cRUj6X1jSkzWYK-sQUQ8wVJMv47iE88XL1GyrhNSQJK7q9W4C12AZnCNX



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
P - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
P - N	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
J - H	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	5.0	8.0	2.25	2.00	
B	TMVW-1	MT20	4.0	8.0			
C	TS-1	MT20	3.0	8.0			
D	TMVW-1	MT20	4.0	8.0			
E	TMVW-1	MT20	2.0	8.0			
F	TS-1	MT20	3.0	8.0			
G	TMVW-1	MT20	4.0	8.0			
H	TMVW-1	MT20	5.0	8.0	2.50	2.50	
I	BMV1+p	MT20	3.0	4.0			
J	BMVW-1	MT20	5.0	8.0	2.50	2.50	
K	BS-1	MT20	3.0	8.0			
L	BMVW-1	MT20	4.0	8.0			
M	BMVW-1	MT20	4.0	8.0			
N	BS-1	MT20	3.0	8.0			
O	BMVW-1	MT20	5.0	8.0	2.00	2.50	
P	BMV1+p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UP/FT	IN-SX
P	1920	0	0	3-8
I	1920	0	0	3-8

UNFACTORED REACTIONS	1ST CASE	MAX / MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	0/0	0/0	467/0	0/0
P	1358	891/0	0/0	0/0	467/0	0/0
I	1358	891/0	0/0	0/0	467/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-P, H-I, B-O, D-L, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING	CHORDS	FACTORED	WELLS	FACTORED
TOTAL LOAD CASES: (4)	MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (LBS)
	FR-TO	FROM TO	CS1 (LC)	CS1 (LC)
	P-A	-1868/0	0.0	0.0
	A-B	-1862/0	-91.8	-91.8
	B-C	-2416/0	-91.8	-91.8
	C-D	-2416/0	-91.8	-91.8
	D-E	-2416/0	-91.8	-91.8
	E-F	-2416/0	-91.8	-91.8
	F-G	-2416/0	-91.8	-91.8
	G-H	-1663/0	-91.8	-91.8
	H-I	-1868/0	0.0	0.0
	P-O	0/0	-18.5	-18.5
	O-N	0/1662	-18.5	-18.5
	N-M	0/1662	-18.5	-18.5
	M-L	0/2416	-18.5	-18.5
	L-K	0/1663	-18.5	-18.5
	K-J	0/1663	-18.5	-18.5
	J-I	0/0	-18.5	-18.5

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CS1: TC=0.98/1.00 (E-G:1), BC=0.47/1.00 (L-M:1), WB=0.54/1.00 (D-M:1), SS=0.30/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1687 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

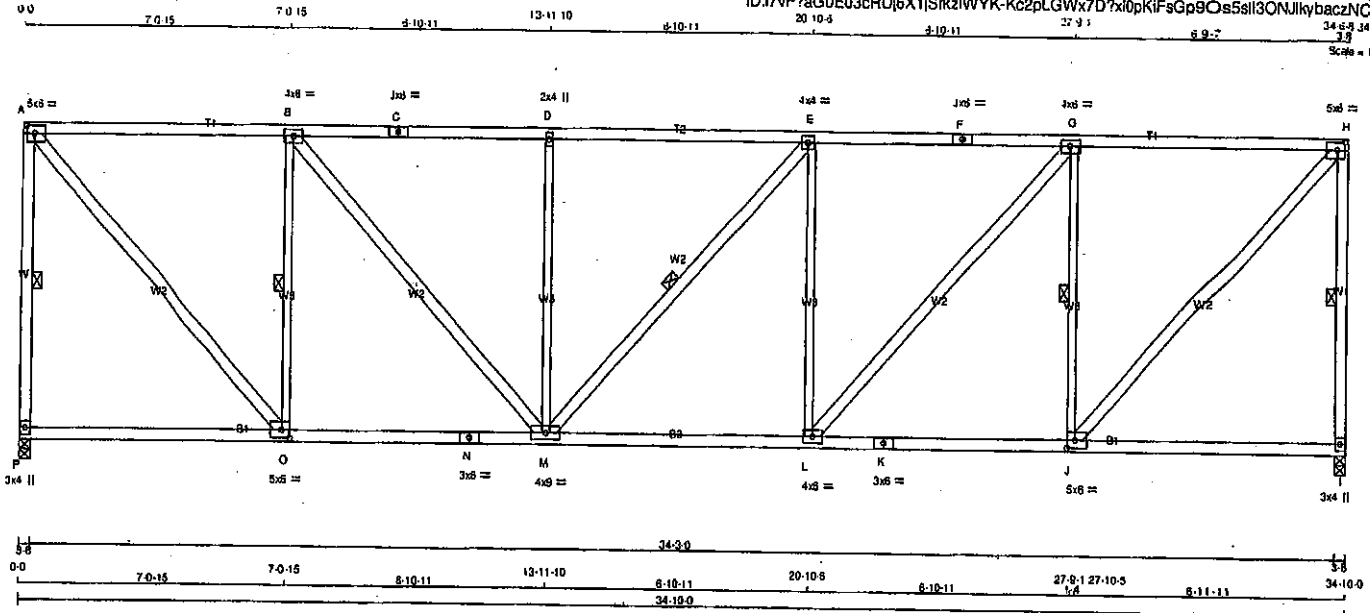
JSI GRIP= 0.90 (A) (INPUT = 0.90)
JSI METAL= 0.53 (K) (INPUT = 1.00)



Structural component only
DWG# T-2007067

JOB NAME 408150	TRUSS NAME T5	QUANTITY 10	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:7vF?aG0E03cRUj6X1iSkzIWK-Kc2pLWx7D7x10pKfSgP9Os5sll3ONJkybacznCnW
34.6-9 34.10-9
Scale = 1:54.5



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
P - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
I - H	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
J - G	2x3	DRY	No.2
O - B	2x3	DRY	No.2
L - E	2x3	DRY	No.2
M - D	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
A	TMVW-1	MT20	5.0	6.0 2.50 2.50
B	TMVW-1	MT20	4.0	6.0
C	TS-1	MT20	3.0	8.0
D	TMVW-1	MT20	2.0	4.0
E	TMVW-1	MT20	4.0	4.0
F	TS-1	MT20	3.0	8.0
G	TMVW-1	MT20	4.0	8.0
H	TMVW-1	MT20	5.0	8.0 2.50 2.50
I	BMV1+p	MT20	3.0	4.0
J	BMVW-1	MT20	5.0	8.0 2.50 2.50
K	BS-1	MT20	3.0	8.0
L	BMVW-1	MT20	4.0	8.0
M	BMVW-1	MT20	4.0	8.0
N	BS-1	MT20	3.0	8.0
O	BMVW-1	MT20	5.0	8.0 2.50 2.50
P	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG
P	1920 0	1920 0	3-8
I	1920 0	1920 0	3-8

UNFACTORED REACTIONS							
JT	COMBINED	1ST LCASE SNOW	MAX. MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD
P	1358	891 / 0	0 / 0	0 / 0	0 / 0	0 / 0	467 / 0
I	1358	891 / 0	0 / 0	0 / 0	0 / 0	0 / 0	467 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-P, H-I, G-J, B-O, E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING			
TOTAL LOAD CASES: (4)			
CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
P-A	-1888 / 0	0.0	0.0 0.55 (1)
A-B	-1458 / 0	-91.8	-91.8 0.80 (1)
B-C	-2108 / 0	-91.8	-91.8 0.90 (1)
C-D	-2108 / 0	-91.8	-91.8 0.90 (1)
D-E	-2108 / 0	-91.8	-91.8 0.68 (1)
E-F	-2108 / 0	-91.8	-91.8 0.90 (1)
F-G	-2108 / 0	-91.8	-91.8 0.90 (1)
G-H	-1458 / 0	-91.8	-91.8 0.79 (1)
H-I	-1888 / 0	0.0	0.0 0.55 (1)
P-O	0 / 0	-18.5	-18.5 0.22 (4)
O-N	0 / 1458	-18.5	-18.5 0.38 (1)
N-M	0 / 1458	-18.5	-18.5 0.38 (1)
M-L	0 / 2108	-18.5	-18.5 0.42 (1)
L-K	0 / 1458	-18.5	-18.5 0.38 (1)
K-J	0 / 1458	-18.5	-18.5 0.38 (1)
J-I	0 / 0	-18.5	-18.5 0.22 (4)

TOTAL WEIGHT = 10 X 178 = 1785 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.90/1.00 (B-D:1), BC=0.42/1.00 (L-M:1), WB=0.78/1.00 (E-L:1), SS=0.31/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 (S BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

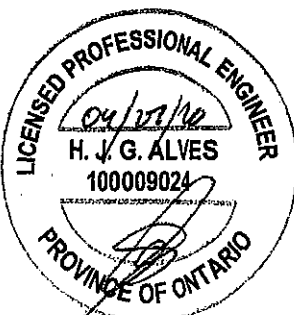
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

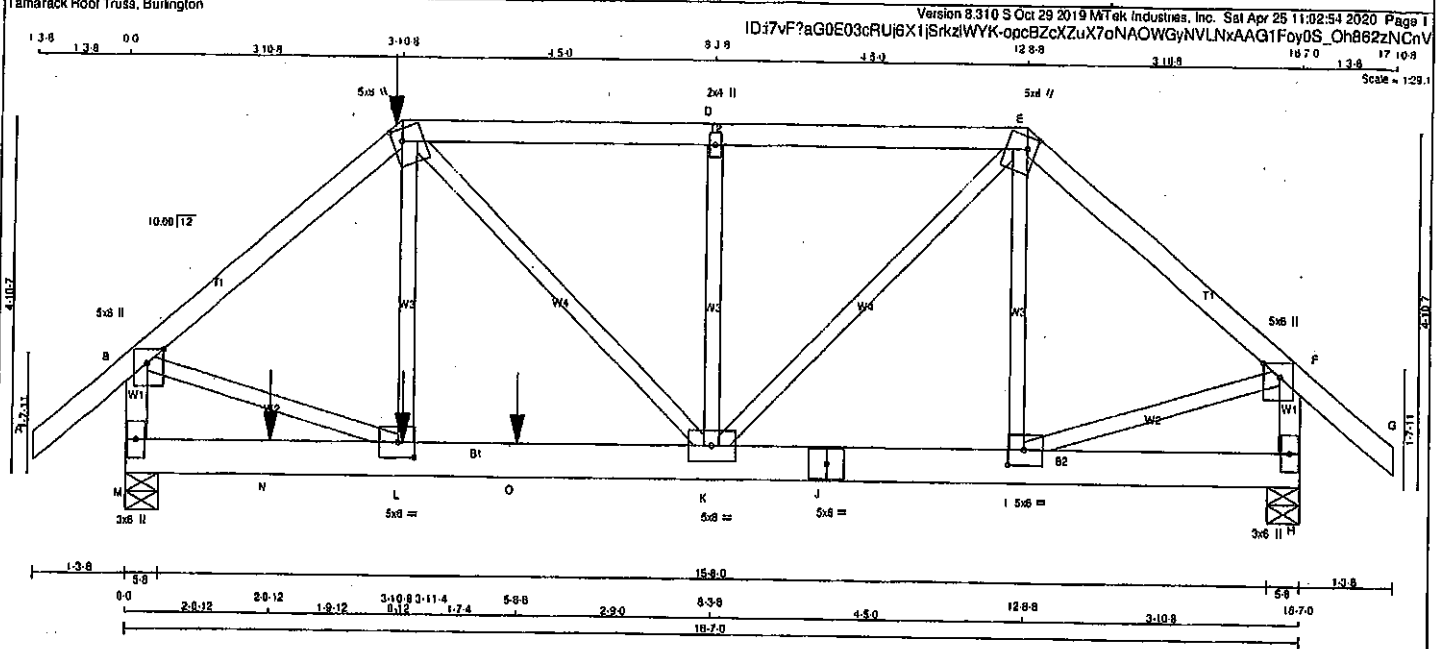
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (A) (INPUT = 0.90)
JSI METAL= 0.47 (N) (INPUT = 1.00)



Structural component only
DWG# T-2007068

JOB NAME 408150	TRUSS NAME T6	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No. 2	SPF	
C - E	2x4	DRY	No. 2	SPF	
E - G	2x4	DRY	No. 2	SPF	
M - B	2x4	DRY	No. 2	SPF	
B - F	2x4	DRY	No. 2	SPF	
M - J	2x4	DRY	No. 2	SPF	
J - H	2x4	DRY	No. 2	SPF	
ALL WEBS	2x3	DRY	No. 2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A - B	MT20	5.0	6.0	Edge	2.75		
C - D	MT20	5.0	6.0	2.00	1.50		
D - E	MT20	2.0	4.0				
E - F	MT20	5.0	6.0	2.00	1.50		
F - G	MT20	5.0	6.0	Edge	2.75		
H - I	MT20	3.0	6.0				
I - J	MT20	5.0	6.0	2.50	2.75		
J - K	MT20	5.0	6.0				
K - L	MT20	5.0	6.0				
L - M	MT20	5.0	6.0	2.50	2.75		
M - N	MT20	3.0	6.0				

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG
M	1888	0	1888	0	0	5-8	5-8	5-8	
H	1422	0	1422	0	0	5-8	5-8	5-8	

UNFACTORED REACTIONS		1ST LOSE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
M	1329	909 / 0	0 / 0	0 / 0	0 / 0	420 / 0	0 / 0		
H	1001	683 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8	-91.8 0.14 (1)	10.00	L-C	0 / 383	0.09 (1)
B-C	-1847 / 0	-91.8	-91.8 0.34 (1)	4.55	C-K	0 / 310	0.08 (1)
C-D	-1841 / 0	-91.8	-91.8 0.38 (1)	4.72	K-D	-481 / 0	0.16 (1)
D-E	-1841 / 0	-91.8	-91.8 0.36 (1)	4.72	K-E	0 / 1010	0.25 (1)
E-F	-1225 / 0	-91.8	-91.8 0.30 (1)	5.40	I-E	-274 / 0	0.09 (1)
F-G	0 / 41	-91.8	-91.8 0.14 (1)	10.00	G-L	0 / 1480	0.37 (1)
M-B	-1926 / 0	0.0	0.0 0.22 (1)	6.01	I-F	0 / 981	0.24 (1)
H-F	-1380 / 0	0.0	0.0 0.16 (1)	6.88			
M-N	0 / 0	-18.5	-18.5 0.24 (1)	10.00			
N-L	0 / 0	-18.5	-18.5 0.24 (1)	10.00			
L-O	0 / 1424	-18.5	-18.5 0.70 (1)	10.00			
O-K	0 / 1424	-18.5	-18.5 0.70 (1)	10.00			
K-J	0 / 932	-18.5	-18.5 0.29 (1)	10.00			
J-I	0 / 932	-18.5	-18.5 0.29 (1)	10.00			
I-H	0 / 0	-18.5	-18.5 0.04 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)		LOC.		MAX.		FACE		DIR.		TYPE		HEEL		CONN.	
JT	LOC.	LC1	MAX.	MAX.	FRONT	VERT	TOTAL	---	C1						
C	3-10-8	-241	-241	---	FRONT	VERT	TOTAL	---	C1						
L	3-11-4	-17	-17	---	FRONT	VERT	TOTAL	---	C1						
N	2-0-12	-17	-17	---	FRONT	VERT	TOTAL	---	C1						
O	5-6-8	-953	-953	---	FRONT	VERT	TOTAL	---	C1						

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.55")
CALCULATED VERT. DEFL. (LL) = L/999 (0.08")
ALLOWABLE DEFL. (TL) = L/360 (0.55")
CALCULATED VERT. DEFL. (TL) = L/999 (0.10")

CSI: TC=0.36/1.00 (C-D:1), BC=0.70/1.00 (K-L:1), WB=0.37/1.00 (B-L:1), SS=0.45/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

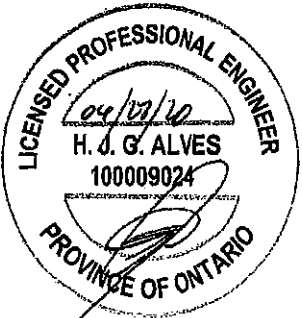
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLD)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.57 (J) (INPUT = 1.00)

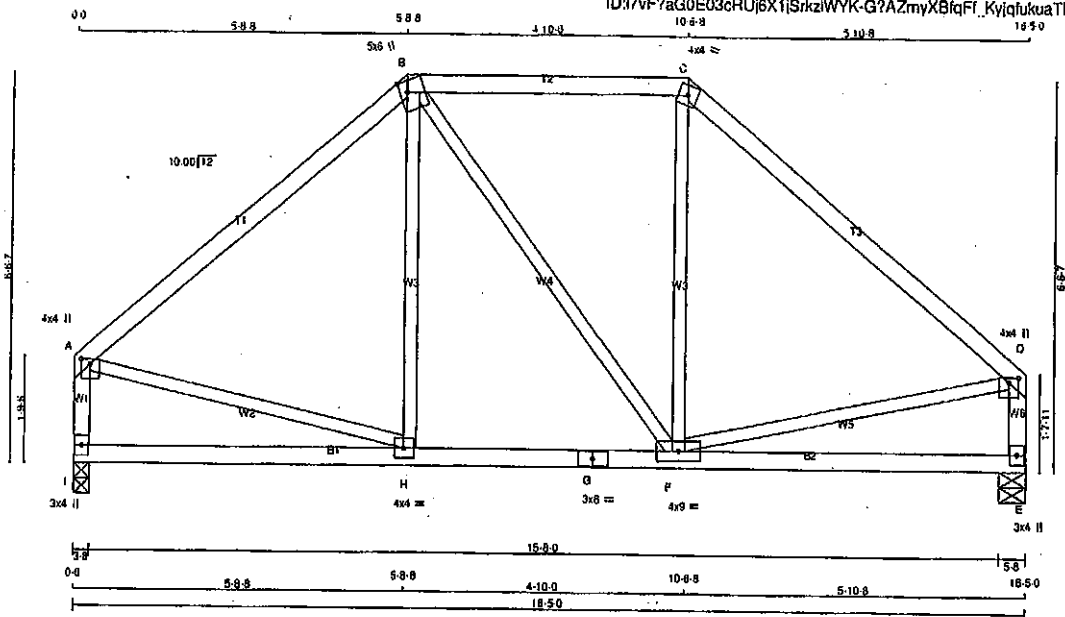


Structural component only
DWG# T-2007069

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408150	T7	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/32"

TOTAL WEIGHT = 70 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4 DRY	No.2	SPF		
B - C	2x4 DRY	No.2	SPF		
C - D	2x4 DRY	No.2	SPF		
I - A	2x4 DRY	No.2	SPF		
E - D	2x4 DRY	No.2	SPF		
I - G	2x4 DRY	No.2	SPF		
G - E	2x4 DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMVWW-l	MT20	4.0	9.0		
G	BS-t	MT20	3.0	6.0		
H	BMVWW-l	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
I	905	0	905	0	0	3-8	3-8		
E	905	0	905	0	0	5-8	5-8		

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	640	420 / 0	0 / 0	0 / 0	0 / 0	220 / 0	0 / 0	0 / 0
E	640	420 / 0	0 / 0	0 / 0	0 / 0	220 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	
FR-TO		FROM TO			FR-TO				
A-B	-718 / 0	-91.8	-91.8	0.39 (1)	6.25	H-B	-34 / 68	0.02 (4)	
B-C	-558 / 0	-91.8	-91.8	0.28 (1)	6.25	B-F	0 / 14	0.00 (1)	
C-D	-729 / 0	-91.8	-91.8	0.42 (1)	6.25	F-C	-23 / 74	0.03 (4)	
I-A	-860 / 0	0.0	0.0	0.09 (1)	7.81	A-H	0 / 567	0.13 (1)	
E-D	-858 / 0	0.0	0.0	0.09 (1)	7.81	F-D	0 / 572	0.13 (1)	
I-H	0 / 0	-18.5	-18.5	0.13 (4)	10.00				
H-G	0 / 550	-18.5	-18.5	0.18 (4)	10.00				
G-F	0 / 550	-18.5	-18.5	0.18 (4)	10.00				
F-E	0 / 0	-18.5	-18.5	0.14 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
	DL = 8.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 21.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.42/1.00 (C-D:1), BC=0.18/1.00 (F-H:4), WB=0.13/1.00 (D-F:1), SI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618	354
	1667	788
	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

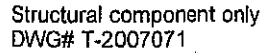
JSI GRIP= 0.85 (D) (INPUT = 0.90)
JSI METAL= 0.24 (D) (INPUT = 1.00)



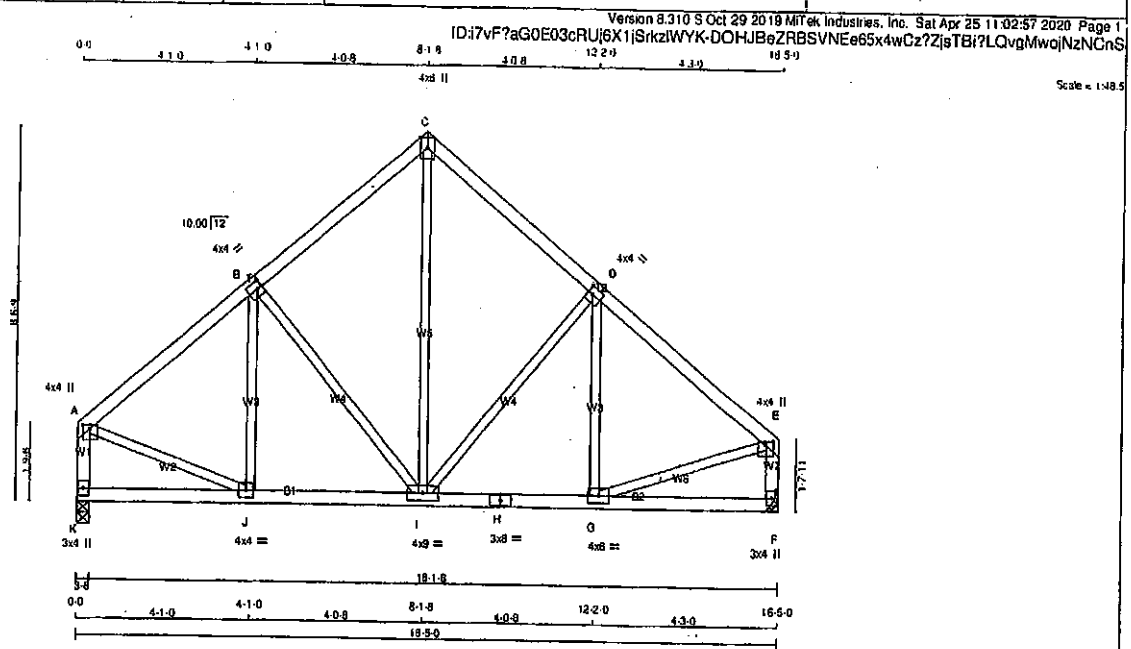
Structural component only
DWG# T-2007070

Scale = 1:43.9

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 27	-91.8	-91.8 0.22 (1)	10.00	B-I	-183 / 0	0.11 (1)
B-C	-849 / 0	-91.8	-91.8 0.17 (1)	6.25	I-E	-190 / 0	0.13 (1)
C-D	-489 / 0	-91.8	-91.8 0.1 (1)	6.25	J-B	-930 / 0	0.54 (1)
D-E	-650 / 0	-91.8	-91.8 0.18 (1)	6.25	E-G	-935 / 0	0.57 (1)
E-F	0 / 27	-91.8	-91.8 0.23 (1)	10.00	C-I	0 / 222	0.05 (1)
J-A	-120 / 0	0.0	0.0 0.01 (1)	7.81	I-D	0 / 222	0.05 (1)
G-F	-137 / 0	0.0	0.0 0.01 (1)	7.81			
J-I	0 / 590	-18.5	-18.5 0.41 (4)	10.00			
I-H	0 / 608	-18.5	-18.5 0.41 (4)	10.00			
H-G	0 / 608	-18.5	-18.5 0.41 (4)	10.00			



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408150	T9	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
K - A	2x4	DRY	No.2
F - E	2x4	DRY	No.2
K - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW+p	MT20	4.0	4.0	1.00	2.00
B	TMW-w	MT20	4.0	4.0	2.00	1.25
C	TTW+p	MT20	4.0	4.0	Edge	
D	TMW-w	MT20	4.0	4.0	2.00	1.25
E	TMW+p	MT20	4.0	4.0	1.00	2.00
F	BMV+p	MT20	3.0	4.0		
G	BMW-w	MT20	4.0	4.0		
H	BS-l	MT20	3.0	6.0		
I	BMW-w	MT20	4.0	4.0		
J	BMW-w	MT20	4.0	4.0		
K	BMV+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	VERT	GROSS REACTION	DOWN	BRG	BRG
K	905	0	905	0	0	3-8
F	905	0	905	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
K	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	840	420/0	0/0	0/0	0/0	220/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
A-B	-772/0	-91.8 -91.8 0.20 (1)	6.25	J-B	-135/21	0.05 (1)	
B-C	-620/0	-91.8 -91.8 0.19 (1)	6.25	B-I	-251/0	0.18 (1)	
C-D	-622/0	-91.8 -91.8 0.20 (1)	6.25	I-C	0/478	0.11 (1)	
D-E	-795/0	-91.8 -91.8 0.21 (1)	6.25	I-D	-278/0	0.20 (1)	
K-A	-873/0	0.0 0.0 0.10 (1)	7.81	G-D	-110/31	0.04 (1)	
F-E	-872/0	0.0 0.0 0.09 (1)	7.81	A-J	0/653	0.15 (1)	
				G-E	0/662	0.15 (1)	
K-J	0/0	-18.5 -18.5 0.07 (4)	10.00				
J-I	0/815	-18.5 -18.5 0.13 (1)	10.00				
I-H	0/633	-18.5 -18.5 0.14 (1)	10.00				
H-G	0/633	-18.5 -18.5 0.14 (1)	10.00				
G-F	0/0	-18.5 -18.5 0.07 (4)	10.00				

TOTAL WEIGHT = 77 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.55")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.55")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CS: TC=0.21/1.00 (D-E:1), BC=0.14/1.00 (G-I:1),
WB=0.20/1.00 (D-I:1), SS=0.14/1.00 (D-E:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PL) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.24 (E) (INPUT = 1.00)

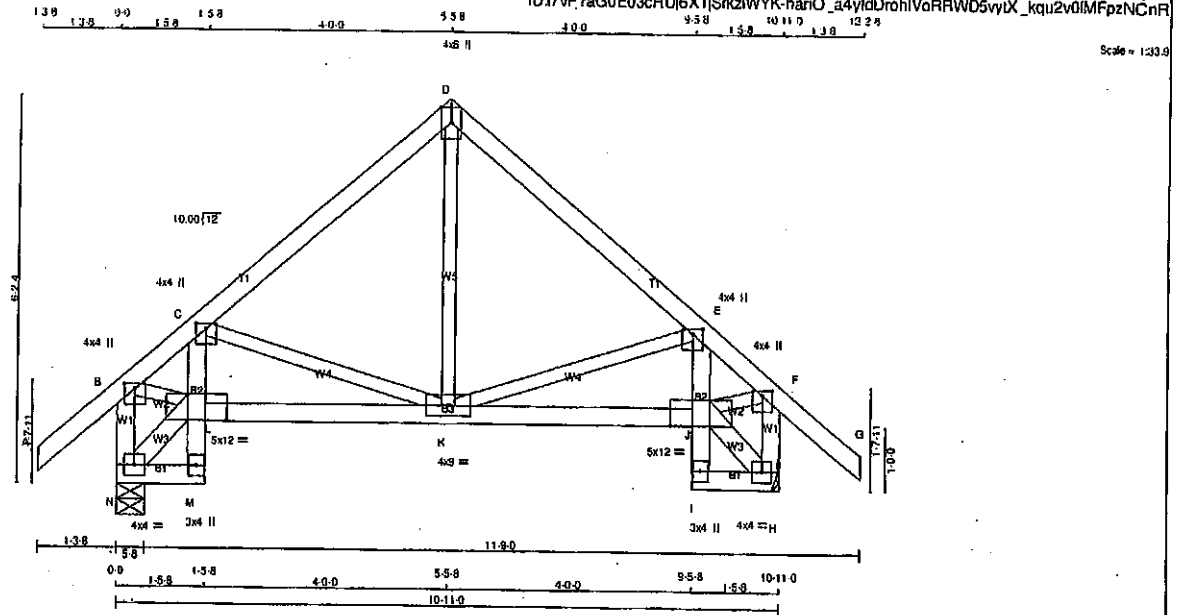


Structural component only
DWG# T-2007072

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408150	T10S	2	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 56 = 112 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2	
D - G	2x4	DRY	No.2	
N - B	2x4	DRY	No.2	
H - F	2x4	DRY	No.2	
N - M	2x4	DRY	No.2	
M - C	2x4	DRY	No.2	
L - J	2x4	DRY	No.2	
J - E	2x4	DRY	No.2	
I - H	2x4	DRY	No.2	
ALL WEBS	2x3	DRY	No.2	
EXCEPT				
N - L	2x4	DRY	No.2	
J - H	2x4	DRY	No.2	

DRY: SEASONED LUMBER.

PLATES (tablets in inches)	JT TYPE	PLATES	W	LEN	Y	X
B, C, E, F						
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTW+p	MT20	4.0	8.0	Edge	
H	BMVW1-i	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BMVWW-i	MT20	5.0	12.0	5.00	7.75
K	BMVWW-i	MT20	4.0	9.0		
L	BMVWW-i	MT20	5.0	12.0	5.00	7.75
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-i	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
N	729	0	729	0
H	729	0	729	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED					
N	513	350 / 0	0 / 0	0 / 0	163 / 0	0 / 0
H	513	350 / 0	0 / 0	0 / 0	163 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	FACTORED	FACTORED	WEBS	MAX. FACTORED	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1 MAX	LC2 MAX	MEMB.	FORCE (LBS)	MAX. FACTORED
FR-TO		FROM TO			FR-TO		
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	C-K	-211 / 0
B-C	-647 / 0	-91.8	-91.8	0.14 (1)	6.25	K-D	0 / 229
C-D	-447 / 0	-91.8	-91.8	0.19 (1)	6.25	K-E	-210 / 0
D-E	-447 / 0	-91.8	-91.8	0.18 (1)	6.25	N-L	-26 / 0
E-F	-645 / 0	-91.8	-91.8	0.14 (1)	6.25	B-L	0 / 527
F-G	0 / 41	-91.8	-91.8	0.13 (1)	10.00	J-F	0 / 526
N-B	-701 / 0	0.0	0.0	0.07 (1)	7.81	J-H	-26 / 0
H-F	-701 / 0	0.0	0.0	0.07 (1)	7.81		
N-M	0 / 20	-18.5	-18.5	0.01 (4)	10.00		
M-L	0 / 14	0.0	0.0	0.03 (1)	10.00		
L-C	-92 / 2	0.0	0.0	0.02 (1)	7.81		
L-K	0 / 528	-18.5	-18.5	0.13 (1)	10.00		
K-J	0 / 527	-18.5	-18.5	0.13 (1)	10.00		
I-J	0 / 14	0.0	0.0	0.03 (1)	10.00		
J-E	-93 / 2	0.0	0.0	0.02 (1)	7.81		
I-H	0 / 20	-18.5	-18.5	0.01 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.36")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.36")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.19/1.00 (C-D:1), BC=0.13/1.00 (K-L:1), WB=0.12/1.00 (B-L:1), CSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

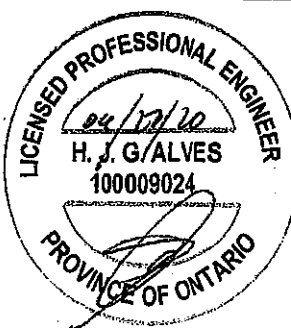
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.74 (F) (INPUT = 0.80)
JSI METAL = 0.20 (F) (INPUT = 1.00)

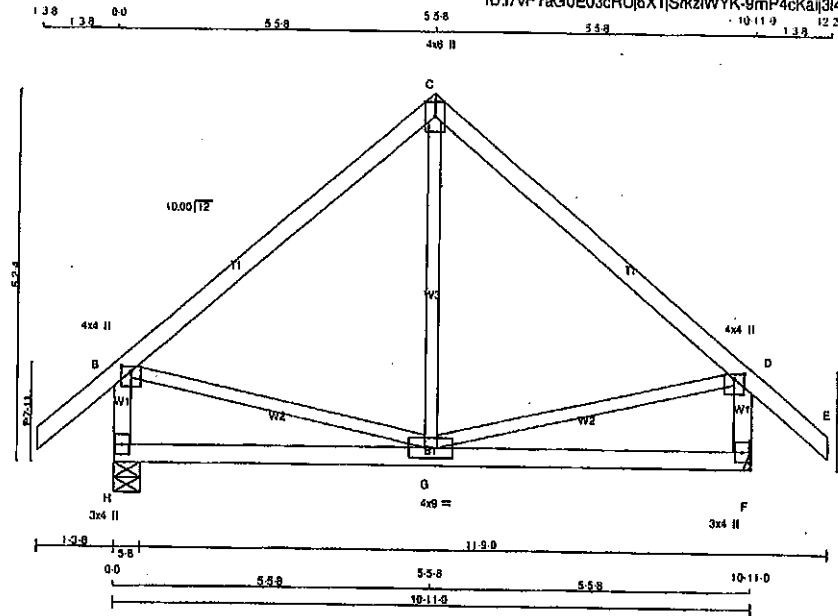


Structural component only
DWG# T-2007073

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408150	T11	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/32"

LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	
A - C	2x4	DRY	No.2	
C - E	2x4	DRY	No.2	
H - B	2x4	DRY	No.2	
F - D	2x4	DRY	No.2	
H - F	2x4	DRY	No.2	

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C TTW+p	MT20	4.0	6.0	Edge	
D TMVW+p	MT20	4.0	4.0	1.00	2.00
F BMV1+p	MT20	3.0	4.0		
G BMVWW-1	MT20	4.0	9.0		
H BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
H	729	0	0	5-8
F	729	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	513	350 / 0	0 / 0	0 / 0	0 / 0	163 / 0	0 / 0
F	513	350 / 0	0 / 0	0 / 0	0 / 0	163 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LOAD LC1 (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LOAD LC1 (LC)
FR-TO						FR-TO			
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	G-C	-15 / 89	0.03 (4)	
B-C	-380 / 0	-91.8	-91.8	0.35 (1)	6.25	B-G	0 / 300	0.07 (1)	
C-D	-380 / 0	-91.8	-91.8	0.35 (1)	6.25	G-D	0 / 300	0.07 (1)	
D-E	0 / 41	-91.8	-91.8	0.13 (1)	10.00				
H-B	-890 / 0	0.0	0.0	0.07 (1)	7.81				
F-D	-890 / 0	0.0	0.0	0.07 (1)	7.81				
H-G	0 / 0	-18.5	-18.5	0.16 (4)	10.00				
G-F	0 / 0	-18.5	-18.5	0.16 (4)	10.00				

TOTAL WEIGHT = 49 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.I. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.36")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.36")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TC=0.35/1.00 (B-C:1), BC=0.16/1.00 (G-H:4), WB=0.07/1.00 (D-G:1), SS=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

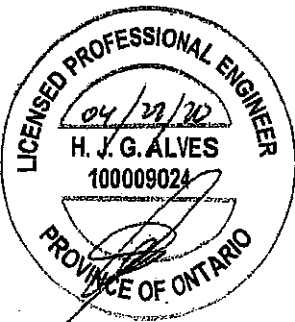
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLJ)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

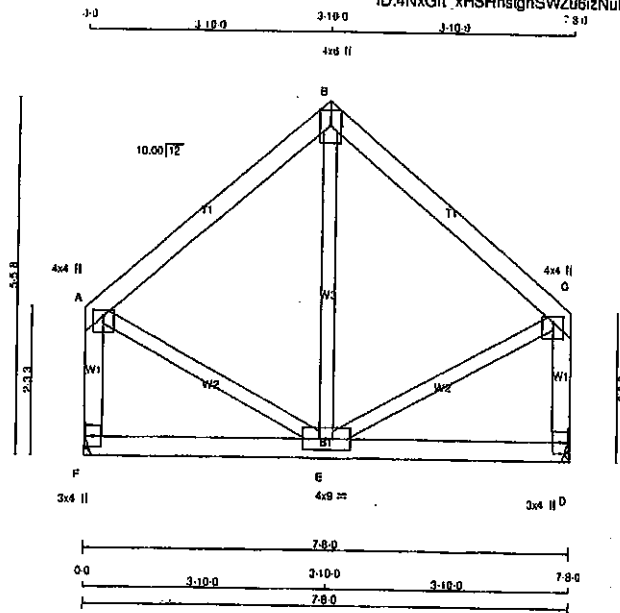
JSI GRIP= 0.82 (D) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)



Structural component only
DWG# T-2007074

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408150	T12	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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Scale = 1/32.5

LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (Table in inches)					
JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00 2.00
B	TTW+p	MT20	4.0	6.0	Edge
C	TMVW+p	MT20	4.0	4.0	1.00 2.00
D	BMV1+p	MT20	3.0	4.0	
E	BMVWV-l	MT20	4.0	9.0	
F	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
F	423	0	423	0	0
D	423	0	423	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	299	196 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
D	299	196 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	-214 / 0	E-B	-78 / 37
B-C	-214 / 0	A-E	0 / 184
F-A	-395 / 0	E-C	0 / 184
D-C	-395 / 0		
F-E	0 / 0		
E-D	0 / 0		

TOTAL WEIGHT = 35 lb (M)(F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 6.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.26")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL) = 1/360 (0.26")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.01")

CSI: TC=0.17/1.00 (B-C:1), BC=0.08/1.00 (D-E:1), WB=0.04/1.00 (A-E:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

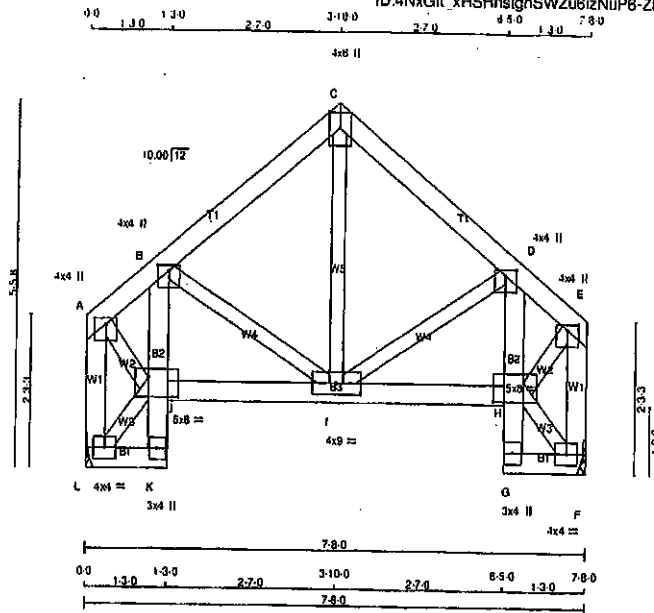
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.31 (C) (INPUT = 0.90)
JSI METAL = 0.09 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007075

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408150	T12S	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
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Scale = 1/31.0					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
L - A	2x4	DRY	No.2
F - E	2x4	DRY	No.2
L - K	2x4	DRY	No.2
K - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
G - D	2x4	DRY	No.2
G - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A, B, D, E					
A	TMVW+p	MT20	4.0	4.0	1.00 2.00
C	TTW+p	MT20	4.0	8.0	Edge
F	BMVW1-t	MT20	4.0	4.0	
G	BMV+p	MT20	3.0	4.0	
H	BMVWW-I	MT20	5.0	8.0	3.50 5.50
I	BMVWW-I	MT20	4.0	9.0	
J	BMVWW-I	MT20	5.0	8.0	3.50 5.50
K	BMV+p	MT20	3.0	4.0	
L	BMVW1-t	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	UPLIFT	IN-SX
L 423 0	423 0	0	MECHANICAL
F 423 0	423 0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L, F. MINIMUM BEARING LENGTH AT JOINT L = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LC CASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
L	299	196 / 0	0 / 0	0 / 0	0 / 0	103 / 0
F	299	196 / 0	0 / 0	0 / 0	0 / 0	103 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED	FACTORED	WEBS
FR-TO	FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED
A-B	-282 / 0	-91.8	0.06 (1)
B-C	-274 / 0	-91.8	0.07 (1)
C-D	-274 / 0	-91.8	0.07 (1)
D-E	-282 / 0	-91.8	0.06 (1)
L-A	-404 / 0	0.0	0.05 (1)
F-E	-404 / 0	0.0	0.05 (1)
L-K	0 / 9	-18.5	0.01 (4)
K-J	0 / 11	0.0	0.01 (1)
J-B	-182 / 0	0.0	0.01 (1)
J-I	0 / 247	-18.5	0.09 (1)
I-H	0 / 247	-18.5	0.06 (1)
G-H	0 / 11	0.0	0.01 (1)
H-D	-182 / 0	0.0	0.01 (1)
G-F	0 / 9	-18.5	0.01 (4)

TOTAL WEIGHT = 2 X 43 = 86 lb

(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.28")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.28")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.01")

CSI: TC=0.07/1.00 (C-D:1), BC=0.08/1.00 (H-I:1), WB=0.07/1.00 (A-J:1), SS=0.09/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (E) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)



Structural component only
DWG# T-2007076

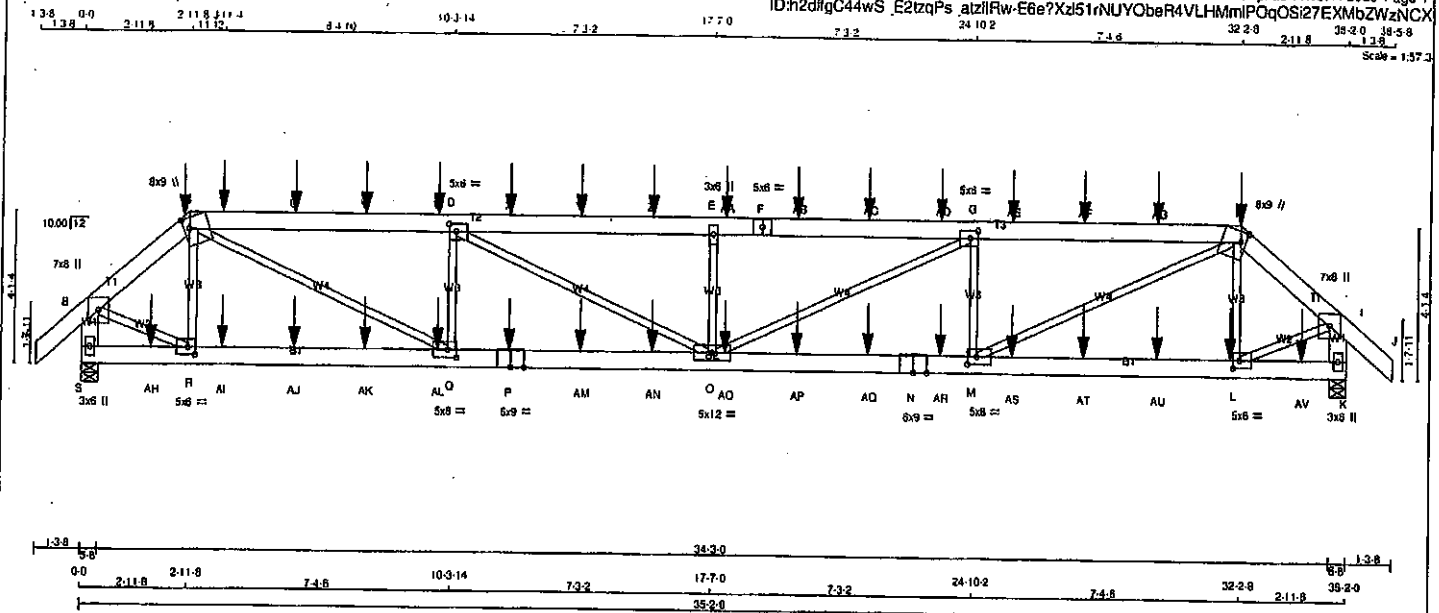
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JSI GRIP= 0.09 (A) (INPUT = 0.90)
JSI METAL= 0.40 (E) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408151	T20	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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LUMBER	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES				
CHORDS				
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	1650F 1.5E	SPF
P - N	2x6	DRY	1650F 1.5E	SPF
N - K	2x6	DRY	1650F 1.5E	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE(122.0)
C - F	12	SIDE(61.0)
F - H	12	SIDE(61.0)
H - J	12	SIDE(122.0)
S - B	12	TOP
K - I	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
S - P	12	SIDE(0.0)
P - N	12	SIDE(0.0)
N - K	12	SIDE(183.1)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UP	IN-SX
S	3330	0	0	5-8
K	3379	0	0	5-8

UNFACTORED REACTIONS	1ST LCASE	MAX MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
S	2354	1553 / 0
K	2388	1572 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO				FR-TO		
A-B	0 / 42	-91.8	-91.8 0.04 (1)	R-C	-662 / 0	0.08 (1)
B-C	-3359 / 0	-91.8	-91.8 0.05 (1)	C-D	0 / 4522	0.56 (1)
C-T	-8608 / 0	-91.8	-91.8 0.38 (1)	D-E	-1799 / 0	0.22 (1)
T-U	-8608 / 0	-91.8	-91.8 0.38 (1)	E-F	0 / 1375	0.17 (1)
U-V	-8608 / 0	-91.8	-91.8 0.38 (1)	F-G	-990 / 0	0.12 (1)
V-W	-8608 / 0	-91.8	-91.8 0.38 (1)	G-H	0 / 1368	0.17 (1)
W-D	-8608 / 0	-91.8	-91.8 0.38 (1)	H-I	-1766 / 0	0.21 (1)
D-X	-7835 / 0	-91.8	-91.8 0.42 (1)	I-J	0 / 4493	0.58 (1)
X-Y	-7835 / 0	-91.8	-91.8 0.42 (1)	J-K	-873 / 0	0.08 (1)
Y-Z	-7835 / 0	-91.8	-91.8 0.42 (1)	K-L	0 / 2729	0.34 (1)
Z-E	-7835 / 0	-91.8	-91.8 0.42 (1)	L-I	0 / 2764	0.34 (1)
E-AA	-7835 / 0	-91.8	-91.8 0.42 (1)			
AA-F	-7835 / 0	-91.8	-91.8 0.42 (1)			
F-AB	-7835 / 0	-91.8	-91.8 0.42 (1)			
AB-AC	-7835 / 0	-91.8	-91.8 0.42 (1)			
AC-AD	-7835 / 0	-91.8	-91.8 0.42 (1)			
AD-G	-7835 / 0	-91.8	-91.8 0.42 (1)			
G-AE	-6612 / 0	-91.8	-91.8 0.36 (1)			
AE-AF	-6612 / 0	-91.8	-91.8 0.38 (1)			
AF-AG	-8812 / 0	-91.8	-91.8 0.38 (1)			
AG-H	-6612 / 0	-91.8	-91.8 0.38 (1)			
H-I	-3401 / 0	-91.8	-91.8 0.05 (1)			
I-J	0 / 42	-91.8	-91.8 0.04 (1)			
S-B	-3368 / 0	0.0	0.0 0.12 (1)			
K-I	-3406 / 0	0.0	0.0 0.12 (1)			
S-AH	0 / 0	-18.5	-18.5 0.06 (4)			
AH-R	0 / 0	-18.5	-18.5 0.06 (4)			
R-AI	0 / 2548	-18.5	-18.5 0.16 (1)			
AI-AJ	0 / 2548	-18.5	-18.5 0.16 (1)			
AJ-AK	0 / 2548	-18.5	-18.5 0.16 (1)			
AK-AL	0 / 2548	-18.5	-18.5 0.16 (1)			
AL-O	0 / 2548	-18.5	-18.5 0.16 (1)			
O-P	0 / 6806	-18.5	-18.5 0.32 (1)			
P-AM	0 / 6806	-18.5	-18.5 0.32 (1)			
AM-AN	0 / 6806	-18.5	-18.5 0.32 (1)			
AN-O	0 / 6806	-18.5	-18.5 0.32 (1)			
O-AQ	0 / 6813	-18.5	-18.5 0.32 (1)			
AQ-AP	0 / 6813	-18.5	-18.5 0.32 (1)			
AP-AQ	0 / 6813	-18.5	-18.5 0.32 (1)			
AQ-N	0 / 6813	-18.5	-18.5 0.32 (1)			
N-AR	0 / 6813	-18.5	-18.5 0.32 (1)			
AR-M	0 / 6813	-18.5	-18.5 0.32 (1)			

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIAL LOADS:	TOP CH.	LL	PSF
	DL	8.0	PSF
	LL	0.0	PSF
	DL	7.4	PSF
	TOTAL LOAD	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17')
CALCULATED VERT. DEFL.(LL) = L/999 (0.20')
ALLOWABLE DEFL.(TL) = L/360 (1.17')
CALCULATED VERT. DEFL.(TL) = L/999 (0.38')

CSI: TC=0.42/1.00 (D-E:1), BC=0.32/1.00 (O-Q:1), WB=0.56/1.00 (C-Q:1), SSI=0.22/1.00 (G-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

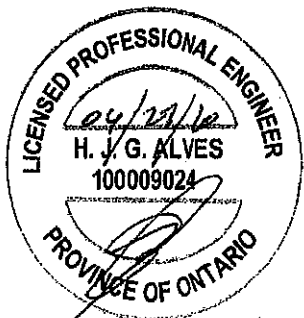
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	818 354	1667 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (IN) (INPUT = 0.90)
JSI METAL= 0.52 (P) (INPUT = 1.00)



Structural component only
DWG# T-2007081 1/2

JOB NAME 408151	TRUSS NAME T20	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+ip	MT20	7.0	8.0		
C	TTWW+m	MT20	8.0	9.0	3.50	1.75
D	TMWW-t	MT20	5.0	6.0	2.50	2.50
E	TMWW-w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TTWW+m	MT20	8.0	9.0	3.50	1.75
I	TMWW+ip	MT20	7.0	8.0		
K	BMV1+ip	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.25
M	BMWW-t	MT20	5.0	6.0	2.50	3.00
N	BS-t	MT20	6.0	9.0		
O	BMWWVW-t	MT20	5.0	12.0		
P	BS-t	MT20	6.0	9.0		
Q	BMWW-t	MT20	5.0	8.0	2.50	3.00
R	BMWW-t	MT20	5.0	6.0	2.50	2.25
S	BMV1+ip	MT20	3.0	6.0		

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. MEMB. FORCE (LBS)	MAX. FACTORED UNBRAC. LENGTH	FR-TO	TO
FR-TO							
M-AS	0 / 2579	-18.5	-18.5	0.16 (1)	10.00		
AS-AT	0 / 2579	-18.5	-18.5	0.16 (1)	10.00		
AT-AU	0 / 2579	-18.5	-18.5	0.16 (1)	10.00		
AU-L	0 / 2579	-18.5	-18.5	0.16 (1)	10.00		
L-AV	0 / 0	-18.5	-18.5	0.08 (4)	10.00		
AV-K	0 / 0	-18.5	-18.5	0.08 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-38	-42		FRONT	VERT	DEAD		C1
C	2-11-8	-191	-191		FRONT	VERT	SNOW		C1
H	32-2-8	-38	-42		FRONT	VERT	DEAD		C1
H	32-2-8	-116	-118		FRONT	VERT	TOTAL		C1
H	32-2-8	-191	-191		FRONT	VERT	SNOW		C1
L	31-11-4	-26	-28		FRONT	VERT	TOTAL		C1
P	11-11-4	-26	-28		FRONT	VERT	TOTAL		C1
T	3-11-4	-111	-111		FRONT	VERT	TOTAL		C1
U	5-11-4	-110	-110		FRONT	VERT	TOTAL		C1
V	7-11-4	-110	-110		FRONT	VERT	TOTAL		C1
W	9-11-4	-110	-110		FRONT	VERT	TOTAL		C1
X	11-11-4	-110	-110		FRONT	VERT	TOTAL		C1
Y	13-11-4	-110	-110		FRONT	VERT	TOTAL		C1
Z	15-11-4	-110	-110		FRONT	VERT	TOTAL		C1
AA	17-11-4	-110	-110		FRONT	VERT	TOTAL		C1
AB	19-11-4	-110	-110		FRONT	VERT	TOTAL		C1
AC	21-11-4	-110	-110		FRONT	VERT	TOTAL		C1
AD	23-11-4	-110	-110		FRONT	VERT	TOTAL		C1
AE	25-11-4	-110	-110		FRONT	VERT	TOTAL		C1
AF	27-11-4	-110	-110		FRONT	VERT	TOTAL		C1
AG	29-11-4	-110	-110		FRONT	VERT	TOTAL		C1
AH	1-11-4	-26	-28		FRONT	VERT	TOTAL		C1
AI	3-11-4	-26	-28		FRONT	VERT	TOTAL		C1
AJ	5-11-4	-26	-28		FRONT	VERT	TOTAL		C1
AK	7-11-4	-26	-28		FRONT	VERT	TOTAL		C1
AL	9-11-4	-26	-28		FRONT	VERT	TOTAL		C1
AM	13-11-4	-26	-28		FRONT	VERT	TOTAL		C1
AN	15-11-4	-26	-28		FRONT	VERT	TOTAL		C1
AO	17-11-4	-26	-28		FRONT	VERT	TOTAL		C1
AP	19-11-4	-26	-28		FRONT	VERT	TOTAL		C1
AQ	21-11-4	-26	-28		FRONT	VERT	TOTAL		C1
AR	23-11-4	-26	-28		FRONT	VERT	TOTAL		C1
AS	25-11-4	-26	-28		FRONT	VERT	TOTAL		C1
AT	27-11-4	-26	-28		FRONT	VERT	TOTAL		C1
AU	29-11-4	-26	-28		FRONT	VERT	TOTAL		C1
AV	33-11-4	-26	-28		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

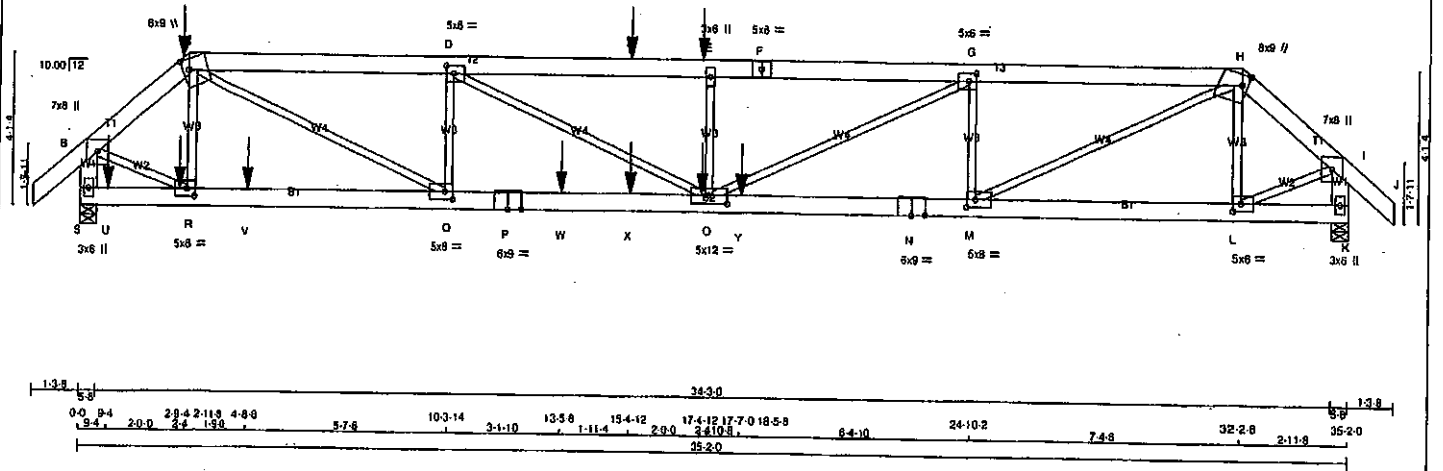
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007081 3/2

JOB NAME 408151	TRUSS NAME T20Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 13.8 0.0 2.118 2.118 7.48 10.314 59.14 15.412 2.00 17.412 17.70 7.32 24.102 7.48 32.28 2.118 35.20 1.38
 Scale = 1:57.3



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x8	DRY No.2	SPF
C - F	2x8	DRY No.2	SPF
F - H	2x8	DRY No.2	SPF
H - J	2x8	DRY No.2	SPF
S - B	2x8	DRY No.2	SPF
K - I	2x8	DRY No.2	SPF
S - P	2x8	DRY 1850F 1.5E	SPF
P - N	2x8	DRY 1850F 1.5E	SPF
N - K	2x8	DRY 1850F 1.5E	SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT

DRY: SEASONED LUMBER.
 DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (#)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 2	12	SIDE(122.0)
C-F 2	12	SIDE(183.1)
F-H 2	12	TOP
H-J 2	12	TOP
S-B 2	12	TOP
K-I 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S-P 2	12	SIDE(183.1)
P-N 2	12	SIDE(183.1)
N-K 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.
 GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.
 TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
S	4991	0	4991	0	5-8
K	3694	0	3694	0	5-8

UNFACTORED REACTIONS		1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
S	3528	2323 / 0	0 / 0
K	2608	1737 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	MAX.	MAX.	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT.	LOAD	LC1	MAX	MEMB.	FORCE
FR-TO		FROM	TO			FR-TO	
A-B	0 / 42	-91.8	-91.8	0.04 (1)	10.00	R-C	-253 / 44
B-C	-5342 / 0	-91.8	-91.8	0.07 (1)	5.02	C-D	0 / 6589
C-D	-8971 / 0	-91.8	-91.8	0.39 (1)	3.64	D-E	-1958 / 0
D-E	-12379 / 0	-91.8	-91.8	0.52 (1)	3.20	E-F	0 / 2694
E-F	-12379 / 0	-91.8	-91.8	0.52 (1)	3.20	F-G	-782 / 0
F-G	-12379 / 0	-91.8	-91.8	0.49 (1)	3.23	G-H	0 / 4229
G-H	-12379 / 0	-91.8	-91.8	0.49 (1)	3.23	H-I	-2619 / 0
H-I	-12379 / 0	-91.8	-91.8	0.33 (1)	3.93	I-J	0 / 8416
I-J	-3750 / 0	-91.8	-91.8	0.06 (1)	5.79	J-K	-864 / 0
J-K	-3750 / 0	-91.8	-91.8	0.04 (1)	10.00	K-L	0 / 4341
K-L	-3750 / 0	-91.8	-91.8	0.19 (1)	8.41	L-M	0 / 3047
L-M	-3750 / 0	-91.8	-91.8	0.13 (1)	7.30	M-N	0 / 4341
M-N	-3750 / 0	-91.8	-91.8	0.13 (1)	7.30	N-O	0 / 4341
N-O	-3750 / 0	-91.8	-91.8	0.13 (1)	7.30	O-P	0 / 4341
O-P	-3750 / 0	-91.8	-91.8	0.13 (1)	7.30	P-Q	0 / 4341
P-Q	-3750 / 0	-91.8	-91.8	0.13 (1)	7.30	Q-R	0 / 4341
Q-R	-3750 / 0	-91.8	-91.8	0.13 (1)	7.30	R-S	0 / 4341
R-S	-3750 / 0	-91.8	-91.8	0.13 (1)	7.30	S-T	0 / 4341
S-T	-3750 / 0	-91.8	-91.8	0.13 (1)	7.30	T-U	0 / 4341
T-U	-3750 / 0	-91.8	-91.8	0.13 (1)	7.30	U-V	0 / 4341
U-V	-3750 / 0	-91.8	-91.8	0.13 (1)	7.30	V-W	0 / 4341
V-W	-3750 / 0	-91.8	-91.8	0.13 (1)	7.30	W-X	0 / 4341
W-X	-3750 / 0	-91.8	-91.8	0.13 (1)	7.30	X-Y	0 / 4341
X-Y	-3750 / 0	-91.8	-91.8	0.13 (1)	7.30	Y-Z	0 / 4341
Y-Z	-3750 / 0	-91.8	-91.8	0.13 (1)	7.30	Z-A	0 / 4341
Z-A	-3750 / 0	-91.8	-91.8	0.13 (1)	7.30	A-B	0 / 4341

FACTORED CONCENTRATED LOADS (LBS)		LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
JT										
C	2-11-8	-38	-42	---	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-117	-117	---	---	BACK	VERT	TOTAL	---	C1
C	2-11-8	-191	-191	---	---	FRONT	VERT	SNOW	---	C1
E	17-4-12	-110	-110	---	---	BACK	VERT	TOTAL	---	C1
O	17-4-12	-26	-26	---	---	BACK	VERT	TOTAL	---	C1
R	2-9-4	-26	-26	---	---	BACK	VERT	TOTAL	---	C1
T	15-4-12	-110	-110	---	---	BACK	VERT	TOTAL	---	C1
U	9-4	-29	-29	---	---	BACK	VERT	TOTAL	---	C1
V	4-8-8	-1066	-1066	---	---	BACK	VERT	TOTAL	---	C1
W	13-5-8	-1066	-1066	---	---	BACK	VERT	TOTAL	---	C1
X	15-4-12	-26	-26	---	---	BACK	VERT	TOTAL	---	C1
Y	18-5-8	-1738	-1738	---	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
 ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC0 2010, NBC0 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, CBC 2012, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 088-09, CSA 088-14
 - TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L / 360 (1.17")
 CALCULATED VERT. DEFL.(LL) = L / 989 (0.33")
 ALLOWABLE DEFL.(TL) = L / 360 (1.17")
 CALCULATED VERT. DEFL.(TL) = L / 685 (0.82")

CSI: TC=0.52/1.00 (D-E:1) , BC=0.73/1.00 (O-Q:1) , WB=0.81/1.00 (C-Q:1) , SS=0.57/1.00 (M-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	818 354 1687 788 1987 1856		

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (D) (INPUT = 0.80)
 JSI METAL = 0.84 (P) (INPUT = 1.00)



Structural component only
 DWG# T-2007082 1/2

JOB NAME 408151	TRUSS NAME T20Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

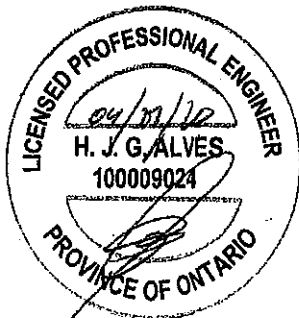
Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Sat Apr 25 11:19:45 2020 Page 2
ID:h2dflgC44wS E2tzoPs atzllRw-UCOUmko9VL9YAq nQagaJuZo4EB5KGTB585v2NCX

PLATES (table is in inches)

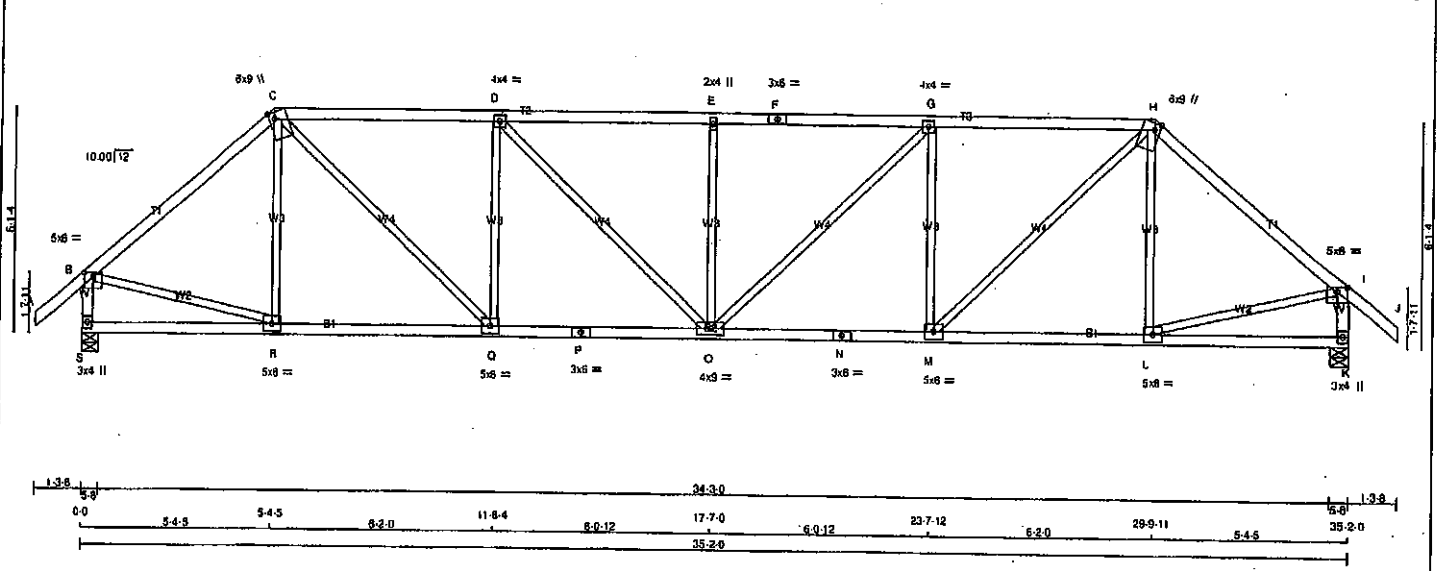
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	7.0	8.0		
C	TTWW+m	MT20	8.0	9.0	3.50	1.75
O	TMVW-l	MT20	5.0	8.0	2.50	2.50
E	TMVW+w	MT20	3.0	8.0		
F	TS-l	MT20	5.0	8.0		
G	TMVW-l	MT20	5.0	8.0	2.50	2.50
H	TTWW+m	MT20	8.0	9.0	3.50	1.75
I	TMVW+p	MT20	7.0	8.0		
K	BMV-l+p	MT20	3.0	8.0		
L	BMVW-l	MT20	5.0	8.0	2.50	2.25
M	BMVW-l	MT20	5.0	8.0	2.50	2.75
N	BS-l	MT20	8.0	9.0		
O	BMVWW-l	MT20	5.0	12.0	2.75	6.00
P	BS-l	MT20	8.0	9.0		
Q	BMVW-l	MT20	5.0	8.0	2.50	2.75
R	BMVW-l	MT20	5.0	8.0	2.50	2.25
S	BMV-l+p	MT20	3.0	8.0		

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007082 7/2



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - J	2x4	DRY No.2	SPF
J - L	2x4	DRY No.2	SPF
L - M	2x4	DRY No.2	SPF
M - N	2x4	DRY No.2	SPF
N - K	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-m	MT20	6.0	9.0	Edge	1.75
D	TMWW-l	MT20	4.0	4.0		
E	TMW-w	MT20	2.0	4.0		
F	TS-l	MT20	3.0	6.0		
G	TMWW-l	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	9.0	Edge	1.75
I	TMWW-p	MT20	5.0	6.0	1.50	3.00
K	BMV-lp	MT20	3.0	4.0		
L, M, O, R						
L	BMWW-l	MT20	5.0	6.0		
N	BS-l	MT20	3.0	6.0		
O	BMWW-l	MT20	4.0	9.0		
P	BS-l	MT20	3.0	6.0		
S	BMV-lp	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
S	2068	0	2068	0	5-8	5-8
K	2068	0	2068	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
S	1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
K	1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
FR-TO					FR-TO		
A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00	R-C	-248 / 7	0.14 (1)
B-C	-2004 / 0	-91.8	-91.8 0.84 (1)	4.11	C-Q	0 / 1538	0.35 (1)
C-D	-2653 / 0	-91.8	-91.8 0.74 (1)	3.50	Q-D	-943 / 0	0.55 (1)
D-E	-2878 / 0	-91.8	-91.8 0.78 (1)	3.28	D-O	0 / 450	0.10 (1)
E-F	-2978 / 0	-91.8	-91.8 0.78 (1)	3.28	O-E	-513 / 0	0.30 (1)
F-G	-2678 / 0	-91.8	-91.8 0.74 (1)	3.50	G-G	0 / 450	0.10 (1)
G-H	-2653 / 0	-91.8	-91.8 0.78 (1)	3.28	M-G	-943 / 0	0.55 (1)
H-I	-2004 / 0	-91.8	-91.8 0.84 (1)	4.11	M-H	0 / 1538	0.35 (1)
I-J	0 / 41	-91.8	-91.8 0.13 (1)	10.00	L-H	-248 / 7	0.14 (1)
S-B	-2026 / 0	0.0	0.0 0.22 (1)	5.94	B-R	0 / 1580	0.36 (1)
K-I	-2026 / 0	0.0	0.0 0.22 (1)	5.94	L-I	0 / 1580	0.38 (1)
S-R	0 / 0	-18.5	-18.5 0.14 (4)	10.00			
R-Q	0 / 1532	-18.5	-18.5 0.33 (1)	10.00			
Q-P	0 / 2653	-18.5	-18.5 0.49 (1)	10.00			
P-O	0 / 2653	-18.5	-18.5 0.49 (1)	10.00			
O-N	0 / 2653	-18.5	-18.5 0.49 (1)	10.00			
N-M	0 / 2653	-18.5	-18.5 0.49 (1)	10.00			
M-L	0 / 1532	-18.5	-18.5 0.33 (1)	10.00			
L-K	0 / 0	-18.5	-18.5 0.14 (4)	10.00			

TOTAL WEIGHT = 2 X 148 = 286 lb

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF
SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/980 (1.17")
CALCULATED VERT. DEFL.(LL) = L/989 (0.16")
ALLOWABLE DEFL.(TL) = L/380 (1.17")
CALCULATED VERT. DEFL.(TL) = L/989 (0.29")

CSI: TO=0.78/1.00 (D-E:1), BC=0.49/1.00 (O-Q:1), WB=0.55/1.00 (G-M:1), SS=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 -618 354 1887 788 1987 1656

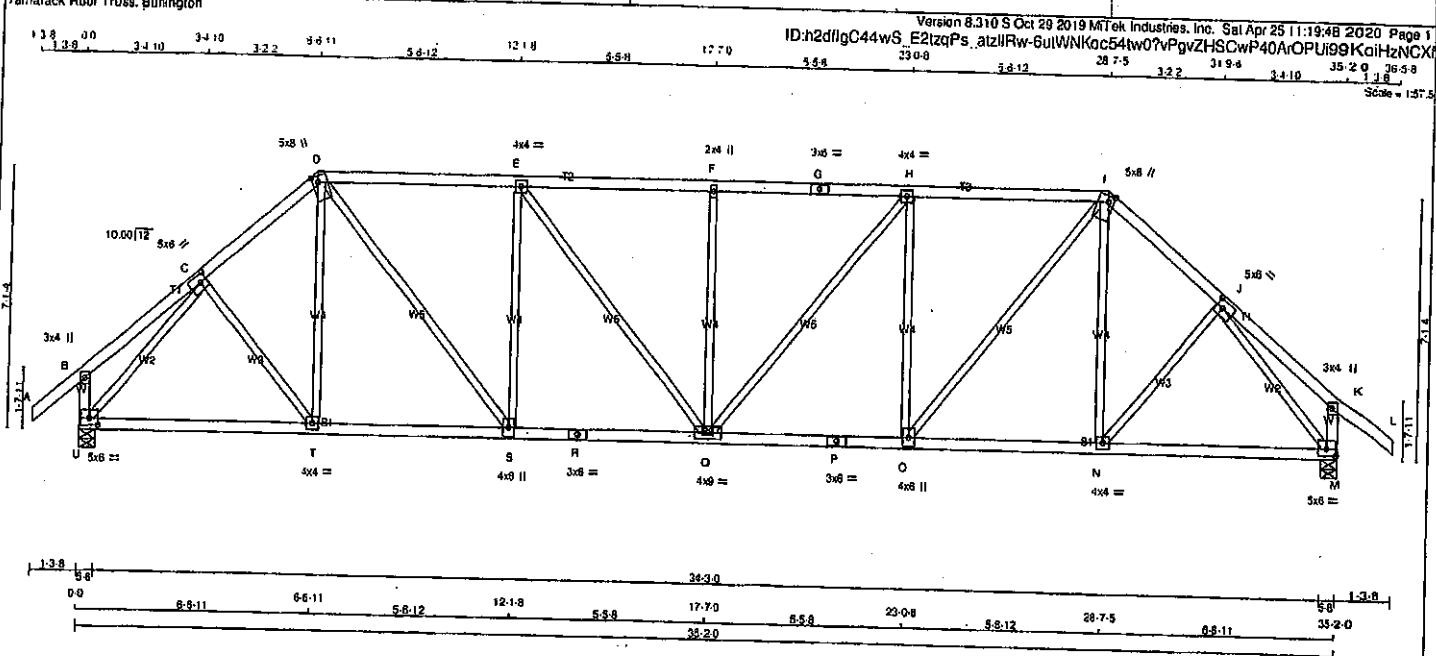
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.85 (N) (INPUT = 1.00)



JOB NAME 408151	TRUSS NAME T23	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - G	2x4	DRY No.2
G - I	2x4	DRY No.2
I - L	2x4	DRY No.2
U - B	2x4	DRY No.2
M - K	2x4	DRY No.2
U - R	2x4	DRY No.2
R - P	2x4	DRY No.2
P - M	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in to inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	5.0	8.0	2.50 2.25
D	TTWW+m	MT20	5.0	8.0	2.00 1.75
E	TMWW-t	MT20	4.0	4.0	
F	TMW-w	MT20	2.0	4.0	
G	TS-t	MT20	3.0	6.0	
H	TMWW-t	MT20	4.0	4.0	
I	TTWW+m	MT20	5.0	8.0	2.00 1.75
J	TMWW-t	MT20	5.0	8.0	2.50 2.25
K	TMV+p	MT20	3.0	4.0	
M	BMVW-t	MT20	5.0	8.0	2.25 3.00
N	BMWW-t	MT20	4.0	4.0	
O	BMWW-t	MT20	4.0	8.0	
P	BS-t	MT20	3.0	6.0	
Q	BMVW-t	MT20	4.0	8.0	
R	BS-t	MT20	3.0	6.0	
S	BMWW-t	MT20	4.0	8.0	
T	BMWW-t	MT20	4.0	4.0	
U	BMVW-t	MT20	5.0	8.0	2.25 3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
U	2088	0	2088	0	5-8	5-8
M	2088	0	2088	0	5-8	5-8

UNFACTORED REACTIONS

JT	MAX/MIN COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
U	1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0
M	1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)
FR-TO						
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	0 / 158
B-C	0 / 10	-91.8	-91.8	0.14 (1)	10.00	0 / 78
C-D	-2023 / 0	-91.8	-91.8	0.20 (1)	4.59	0 / 1220
D-E	-2308 / 0	-91.8	-91.8	0.55 (1)	3.94	0 / 1220
E-F	-2534 / 0	-91.8	-91.8	0.57 (1)	3.77	-850 / 0
F-G	-2534 / 0	-91.8	-91.8	0.57 (1)	3.77	0 / 361
G-H	-2534 / 0	-91.8	-91.8	0.57 (1)	3.77	-481 / 0
H-I	-2308 / 0	-91.8	-91.8	0.55 (1)	3.94	0 / 361
I-J	-2023 / 0	-91.8	-91.8	0.20 (1)	4.59	-850 / 0
J-K	0 / 19	-91.8	-91.8	0.14 (1)	10.00	0 / 1220
K-L	0 / 41	-91.8	-91.8	0.13 (1)	10.00	0 / 78
U-B	-248 / 0	0.0	0.0	0.03 (1)	7.81	0 / 158
M-K	-248 / 0	0.0	0.0	0.03 (1)	7.81	-2277 / 0
U-T	0 / 1433	-18.5	-18.5	0.33 (1)	10.00	0.98 (1)
T-S	0 / 1535	-18.5	-18.5	0.35 (1)	10.00	
S-R	0 / 2308	-18.5	-18.5	0.42 (1)	10.00	
R-Q	0 / 2308	-18.5	-18.5	0.42 (1)	10.00	
Q-P	0 / 2308	-18.5	-18.5	0.42 (1)	10.00	
P-O	0 / 2308	-18.5	-18.5	0.42 (1)	10.00	
O-N	0 / 1535	-18.5	-18.5	0.35 (1)	10.00	
N-M	0 / 1433	-18.5	-18.5	0.33 (1)	10.00	

TOTAL WEIGHT = 2 X 162 = 323 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2010, NBCO 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCO 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/999 (0.13")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = L/999 (0.23")

CSI: TO=0.57/1.00 (E-F:1), BC=0.42/1.00 (O-Q:1), WB=0.88/1.00 (J-K:1), SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

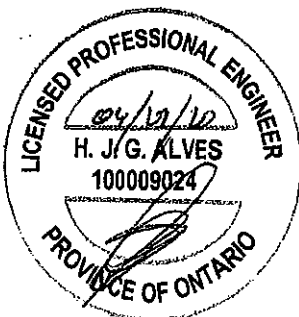
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1887 1656

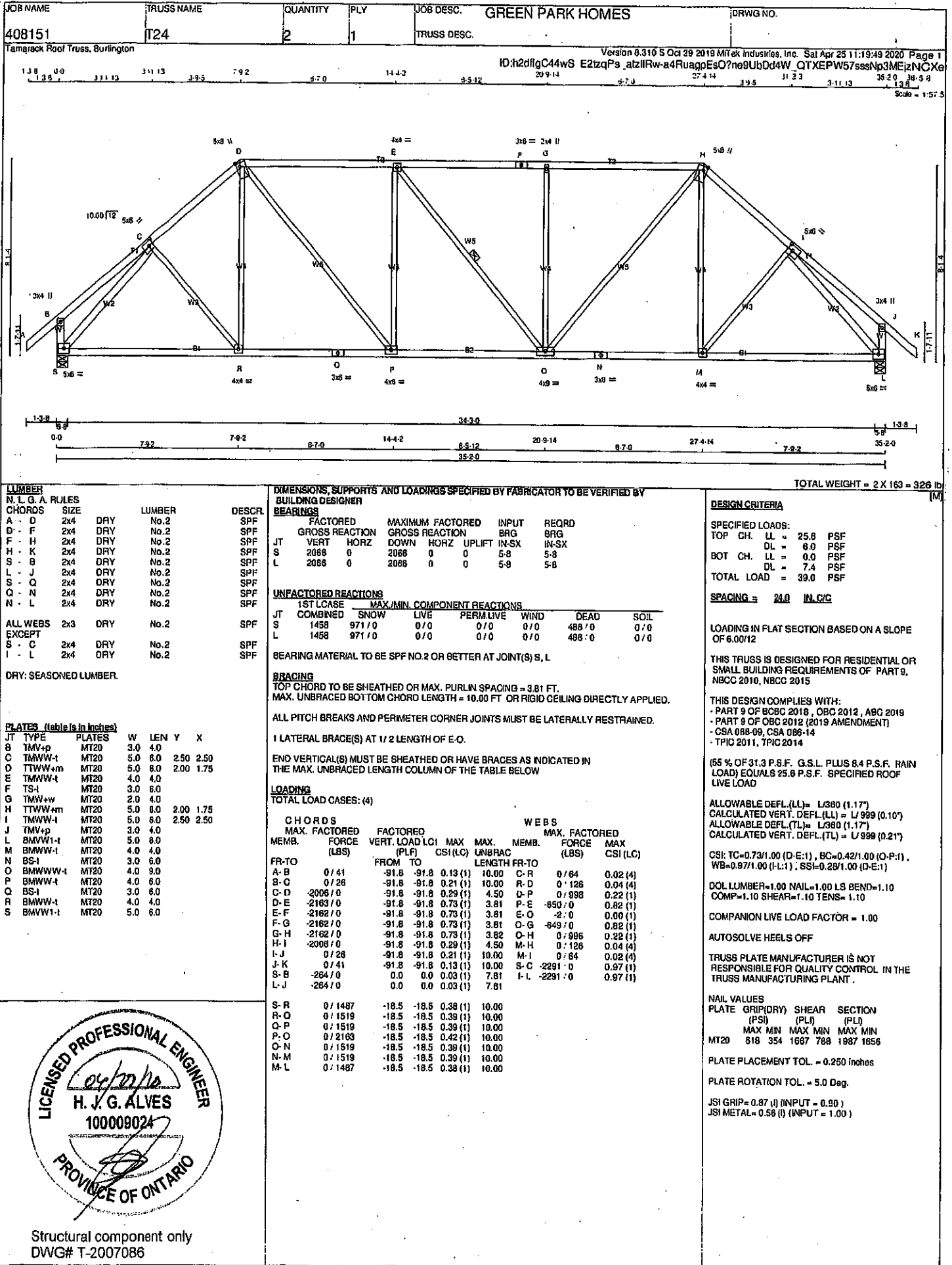
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (I) (INPUT = 0.90)
JSI METAL = 0.72 (P) (INPUT = 1.00)



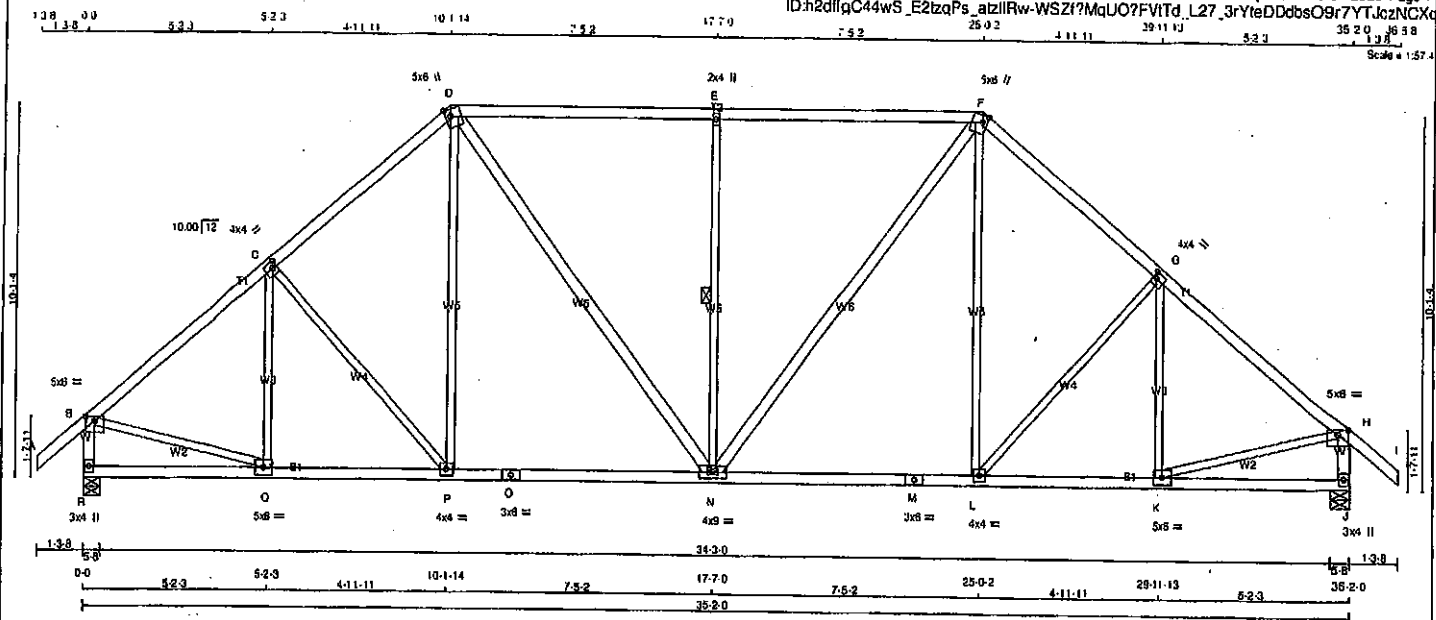
Structural component only
DWG# T-2007085



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408151	T26	4	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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ID:h2dlqC44wS_E2tqPs_alzIRw-WSZl?MqUO7FVtId_L27_3rYteDDbsO9r7YTJc2NCXc



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMVW-i	MT20	4.0	4.0	2.00	1.25
D	TTWW-am	MT20	5.0	6.0	2.25	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-am	MT20	5.0	6.0	2.25	1.50
G	TMVW-i	MT20	4.0	4.0	2.00	1.25
H	TMVW-p	MT20	5.0	6.0	1.50	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMVW-i	MT20	5.0	6.0		
L	BMVW-i	MT20	4.0	4.0		
M	BS-i	MT20	3.0	6.0		
N	BMVWW-i	MT20	4.0	4.0		
O	BS-i	MT20	3.0	6.0		
P	BMVW-i	MT20	4.0	4.0		
Q	BMVW-i	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
R	2068	0	2068	0
J	2068	0	2068	0

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
R	1458	971/0	0/0	0/0	0/0	488/0	0/0	
J	1458	971/0	0/0	0/0	0/0	488/0	0/0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/41	-91.8	-91.8 0.13 (1)	10.00	Q-C	-302/0	0.17 (1)
B-C	-2021/0	-91.8	-91.8 0.39 (1)	4.41	P-D	-217/0	0.25 (1)
C-D	-1908/0	-91.8	-91.8 0.37 (1)	4.52	D-E	0/278	0.08 (1)
D-E	-1809/0	-91.8	-91.8 0.74 (1)	3.98	E-F	0/615	0.10 (1)
E-F	-1809/0	-91.8	-91.8 0.74 (1)	3.98	F-G	-888/0	0.58 (1)
F-G	-1908/0	-91.8	-91.8 0.37 (1)	4.52	G-H	0/615	0.10 (1)
G-H	-2021/0	-91.8	-91.8 0.39 (1)	4.41	H-I	0/278	0.08 (1)
H-I	0/41	-91.8	-91.8 0.13 (1)	10.00	I-J	-217/0	0.25 (1)
J-K	-2021/0	0.0	0.0 0.22 (1)	5.94	K-L	-302/0	0.17 (1)
K-L	-2021/0	0.0	0.0 0.22 (1)	5.94	L-M	0/1829	0.37 (1)
L-M	0/0	-18.5	-18.5 0.10 (4)	10.00	M-N	0/1829	0.37 (1)
M-N	0/1580	-18.5	-18.5 0.34 (1)	10.00	N-O	0/1437	0.10 (1)
N-O	0/1437	-18.5	-18.5 0.35 (1)	10.00	O-P	0/1437	0.10 (1)
O-P	0/1437	-18.5	-18.5 0.35 (1)	10.00	P-Q	0/1437	0.10 (1)
P-Q	0/1437	-18.5	-18.5 0.35 (1)	10.00	Q-R	0/1437	0.10 (1)
Q-R	0/1437	-18.5	-18.5 0.35 (1)	10.00	R-S	0/1437	0.10 (1)
R-S	0/1437	-18.5	-18.5 0.35 (1)	10.00	S-T	0/1437	0.10 (1)
S-T	0/1437	-18.5	-18.5 0.35 (1)	10.00	T-U	0/1437	0.10 (1)
T-U	0/1437	-18.5	-18.5 0.35 (1)	10.00	U-V	0/1437	0.10 (1)
U-V	0/1437	-18.5	-18.5 0.35 (1)	10.00	V-W	0/1437	0.10 (1)
V-W	0/1437	-18.5	-18.5 0.35 (1)	10.00	W-X	0/1437	0.10 (1)
W-X	0/1437	-18.5	-18.5 0.35 (1)	10.00	X-Y	0/1437	0.10 (1)
X-Y	0/1437	-18.5	-18.5 0.35 (1)	10.00	Y-Z	0/1437	0.10 (1)
Y-Z	0/1437	-18.5	-18.5 0.35 (1)	10.00	Z-A	0/1437	0.10 (1)

TOTAL WEIGHT = 4 X 173 = 692 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 8.0 PSF
BOY CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. CC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, NBCC 2015, ABC 2019
- PART 9 OF NBCC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.74/1.00 (D-E:1), BC=0.35/1.00 (L-N:1), WB=0.58/1.00 (E-N:1), SB=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	818	354
	1687	788
	1987	1658

PLATE PLACEMENT TOL. = 0.260 inches

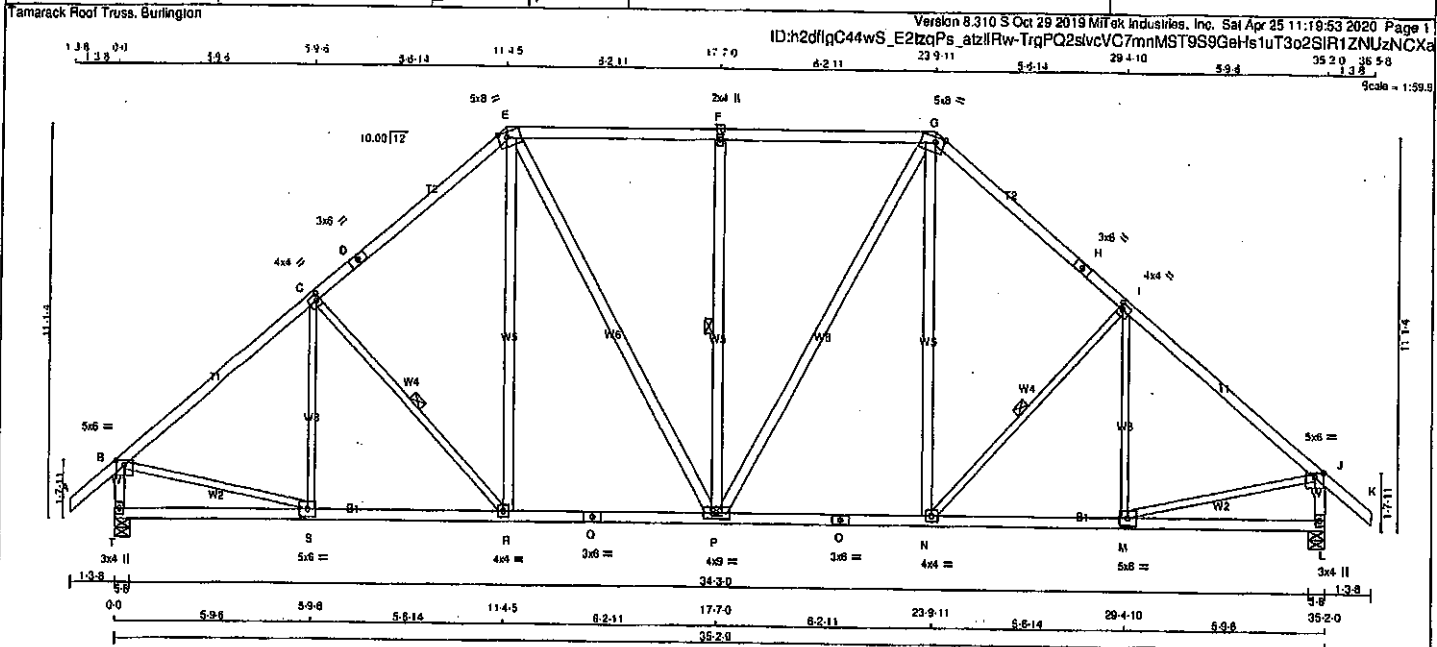
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.46 (M) (INPUT = 1.00)



Structural component only
DWG# T-2007088

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408151	T27	2	1	GREEN PARK HOMES	



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCH.
CHORDS			
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
T - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
T - Q	2x4	DRY	No.2
Q - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
S - C	2x3	DRY	No.2
C - R	2x3	DRY	No.2
N - I	2x3	DRY	No.2
M - I	2x3	DRY	No.2
B - S	2x3	DRY	No.2
M - J	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.00
C	TMVW-1	MT20	4.0	4.0	2.00	1.25
D	TS-1	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	Edge	3.00
F	TMVW-w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	3.00
H	TS-1	MT20	3.0	6.0		
I	TMVW-1	MT20	4.0	4.0	2.00	1.25
J	TMVW-p	MT20	5.0	8.0	1.50	3.00
L	BMV1-p	MT20	3.0	4.0		
M	BMVW-1	MT20	5.0	6.0		
N	BMVW-1	MT20	4.0	4.0		
O	BS-1	MT20	3.0	6.0		
P	BMVW-1	MT20	4.0	6.0		
Q	BS-1	MT20	3.0	6.0		
R	BMVW-1	MT20	4.0	6.0		
S	BMVW-1	MT20	5.0	6.0		
T	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	DOWN	HORIZ	UPLIFT	IN-SX	IN-SX	
T	2068	0	2068	0	5-8	5-8	
L	2068	0	2068	0	5-8	5-8	

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
T	1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0	0 / 0
L	1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-R, F-P, M-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00	S-C	-245 / 10	0.17 (1)
B-C	-2039 / 0	-91.8	-91.8 0.50 (1)	4.27	C-R	-314 / 0	0.15 (1)
C-D	-1844 / 0	-91.8	-91.8 0.48 (1)	4.47	R-E	0 / 338	0.05 (1)
D-E	-1844 / 0	-91.8	-91.8 0.48 (1)	4.47	E-P	0 / 489	0.08 (1)
E-F	-1821 / 0	-91.8	-91.8 0.50 (1)	4.60	P-F	-699 / 0	0.45 (1)
F-G	-1821 / 0	-91.8	-91.8 0.50 (1)	4.80	P-G	0 / 469	0.08 (1)
G-H	-1844 / 0	-91.8	-91.8 0.48 (1)	4.47	N-G	0 / 338	0.05 (1)
H-I	-1844 / 0	-91.8	-91.8 0.48 (1)	4.47	N-I	-314 / 0	0.15 (1)
I-J	-2039 / 0	-91.8	-91.8 0.50 (1)	4.27	M-I	-245 / 10	0.17 (1)
J-K	0 / 41	-91.8	-91.8 0.13 (1)	10.00	B-S	0 / 1637	0.37 (1)
T-B	-2021 / 0	0.0	0.0 0.21 (1)	5.94	M-J	0 / 1637	0.37 (1)
L-J	-2021 / 0	0.0	0.0 0.21 (1)	5.94			

T-S	0 / 0	-18.5	-18.5 0.14 (4)	10.00
S-R	0 / 1597	-18.5	-18.5 0.33 (1)	10.00
R-Q	0 / 1597	-18.5	-18.5 0.30 (1)	10.00
Q-P	0 / 1597	-18.5	-18.5 0.30 (1)	10.00
P-O	0 / 1597	-18.5	-18.5 0.30 (1)	10.00
O-N	0 / 1597	-18.5	-18.5 0.30 (1)	10.00
N-M	0 / 1597	-18.5	-18.5 0.33 (1)	10.00
M-L	0 / 0	-18.5	-18.5 0.14 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17')
CALCULATED VERT. DEFL.(LL) = L/999 (0.07')
ALLOWABLE DEFL.(TL) = L/360 (1.17')
CALCULATED VERT. DEFL.(TL) = L/999 (0.14')

CSI: TC=0.50/1.00 (E-F:1), BC=0.33/1.00 (M-N:1), WB=0.45/1.00 (F-P:1), SS=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX
MT20	618	354
	1667	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

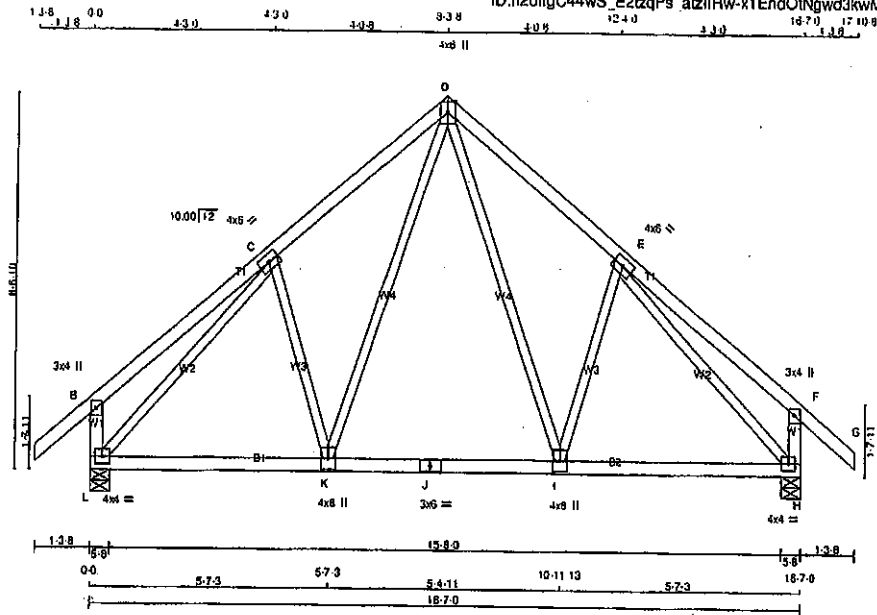
JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.47 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007089

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408151	T28	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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ID:h2dfgC44wS_E2zqPs_atzIRw-x1EndOIngwd3kwMZ0AghhTAWTQH00B7bX5n7vxzNCXZ
Scale: 1/4"=1'



TOTAL WEIGHT = 2 X 82 = 163 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2			SPF
D - G	2x4	DRY	No.2			SPF
L - B	2x4	DRY	No.2			SPF
H - F	2x4	DRY	No.2			SPF
L - J	2x4	DRY	No.2			SPF
J - H	2x4	DRY	No.2			SPF
ALL WEBS	2x3	DRY	No.2			SPF
EXCEPT						

DRY: SEASONED LUMBER.

PLATES (table is in inches)	PLATES	W	LEN	Y	X
JT TYPE					
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
D	TTWW+p	MT20	4.0	6.0	Edge
E	TMWW-t	MT20	4.0	6.0	
F	TMV+p	MT20	3.0	4.0	
H	BMWW-t	MT20	4.0	4.0	
I	BMWW-t	MT20	4.0	6.0	
J	BS-t	MT20	3.0	6.0	
K	BMWW-t	MT20	4.0	6.0	
L	BMWW-t	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
L	1041	0	5-8	5-8
H	1041	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED						
L	734	495/0	0/0	0/0	0/0	239/0	0/0
H	734	495/0	0/0	0/0	0/0	239/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LC)	UNSHAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO						FR-TO		
A-B	0/41	-91.8	-91.8	0.13 (1)	10.00	D-I	0/339	0.08 (1)
B-C	0/29	-91.8	-91.8	0.28 (1)	10.00	E	-222/0	0.09 (1)
C-D	-764/0	-91.8	-91.8	0.20 (1)	6.25	K-D	0/339	0.08 (1)
D-E	-764/0	-91.8	-91.8	0.20 (1)	6.25	C-K	-222/0	0.09 (1)
E-F	0/29	-91.8	-91.8	0.28 (1)	10.00	L-C	-964/0	0.66 (1)
F-G	0/41	-91.8	-91.8	0.13 (1)	10.00	E-H	-964/0	0.66 (1)
L-B	-270/0	0.0	0.0	0.03 (1)	7.81			
H-F	-270/0	0.0	0.0	0.03 (1)	7.81			
L-K	0/832	-18.5	-18.5	0.18 (4)	10.00			
K-J	0/459	-18.5	-18.5	0.17 (4)	10.00			
J-I	0/459	-18.5	-18.5	0.17 (4)	10.00			
I-H	0/832	-18.5	-18.5	0.18 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
OL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/969$ (0.55")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.02")
ALLOWABLE DEFL. (TL) = $L/969$ (0.55")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.04")

CSI: TC=0.28/1.00 (B-C:1), BC=0.18/1.00 (H-I:4),
WB=0.88/1.00 (E-H:1), SS=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1666

PLATE PLACEMENT TOL. = 0.250 inches

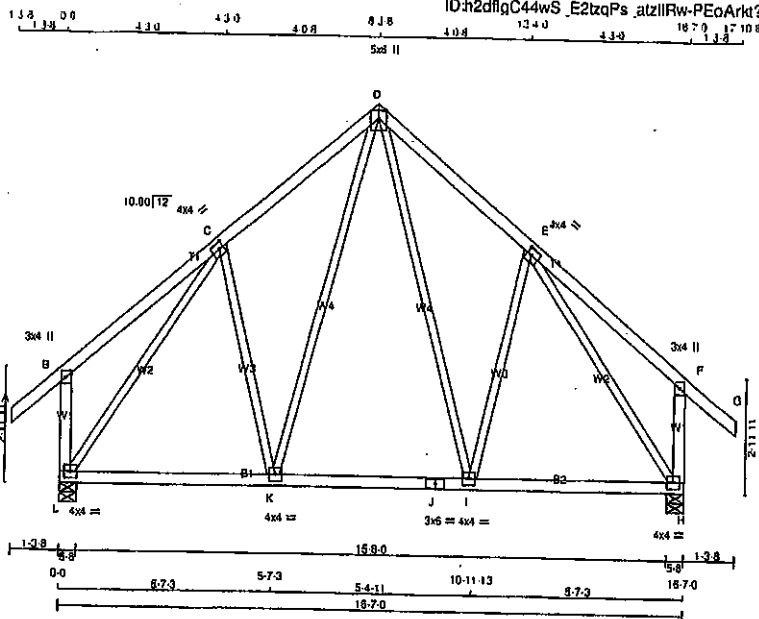
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (C) (INPUT = 0.90)
JSI METAL= 0.25 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007090

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408151	T29	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+p	MT20	5.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	8.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	RECORD
JT	VERT	DOWN	BRG	BRG
L	1041	0	0/0	0/0
H	1041	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
L	734	495/0	0/0	0/0	0/0	0/0	239/0	0/0
H	734	495/0	0/0	0/0	0/0	0/0	239/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LCI)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LCI)	MAX. FACTORED VERT. LOAD (LCI)
FR-TO				FR-TO			
A-B	0/41	-91.8	-91.8 0.13 (1)	D-I	0/254	0.08 (1)	0.08 (1)
B-C	0/28	-91.8	-91.8 0.26 (1)	J-E	-138/14	0.10 (1)	0.10 (1)
C-D	-633/0	-91.8	-91.8 0.20 (1)	K-D	0/254	0.08 (1)	0.08 (1)
D-E	-633/0	-91.8	-91.8 0.20 (1)	L-C	-138/14	0.10 (1)	0.10 (1)
E-F	0/28	-91.8	-91.8 0.26 (1)	M-C	-875/0	0.90 (1)	0.90 (1)
F-G	0/41	-91.8	-91.8 0.13 (1)	E-H	-875/0	0.90 (1)	0.90 (1)
L-B	-278/0	0.0	0.0 0.04 (1)				
H-F	-278/0	0.0	0.0 0.04 (1)				
L-K	0/495	-18.5	-18.5 0.17 (4)				
K-J	0/395	-18.5	-18.5 0.16 (4)				
J-I	0/395	-18.5	-18.5 0.16 (4)				
I-H	0/495	-18.5	-18.5 0.17 (4)				

TOTAL WEIGHT = 91 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.55")
CALCULATED VERT. DEFL.(LL) = $L/899$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.55")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CS: TC=0.28/1.00 (E-F:1), BC=0.17/1.00 (K-L:4), WB=0.90/1.00 (E-H:1), SS=0.14/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

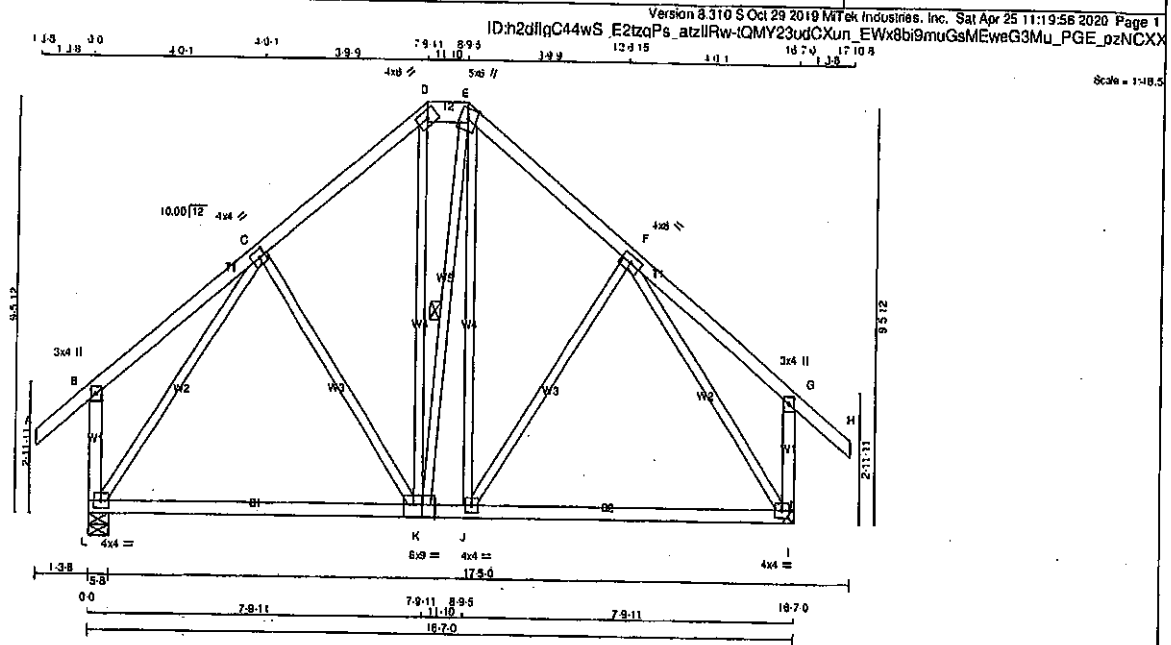
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.34 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007091

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408151	T30	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER			
CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
E - H	2x6	DRY	No.2
I - J	2x4	DRY	No.2
K - L	2x4	DRY	No.2
M - N	2x4	DRY	No.2
O - P	2x4	DRY	No.2
Q - R	2x4	DRY	No.2
S - T	2x4	DRY	No.2
U - V	2x4	DRY	No.2
W - X	2x4	DRY	No.2
Y - Z	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-h	MT20	4.0	8.0		
E	TTWw-m	MT20	5.0	8.0	Edge	
F	TMVW-t	MT20	4.0	6.0		
G	TMV-p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	4.0		
J	BMVW-t	MT20	4.0	4.0		
K	BSVW-t	MT20	6.0	9.0	Edge	3.75
L	BMVW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REORD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
L	1041	0	1041	0	5-8
I	1041	0	1041	0	5-8
				MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	734	485/0	0/0	0/0	0/0	239/0	0/0
I	734	485/0	0/0	0/0	0/0	239/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED			MEMB.	MAX. FACTORED	
	FORCE	VERT. LOAD	LC1	MAX.		FORCE	MAX.
	(LBS)	(PLF)	MAX	UNBRAC		(LBS)	CS1 (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0/41	-91.8	-91.8	0.13 (1)	10.00	C-K	-98/9
B-C	0/27	-91.8	-91.8	0.23 (1)	10.00	K-D	0/164
C-D	-566/0	-91.8	-91.8	0.18 (1)	8.25	K-E	-109/0
D-E	-414/0	-91.8	-91.8	0.01 (1)	8.25	J-F	0/266
E-F	-582/0	-91.8	-91.8	0.18 (1)	8.25	J-F	-93/15
F-G	0/27	-91.8	-91.8	0.23 (1)	10.00	L-C	-846/0
G-H	0/41	-91.8	-91.8	0.13 (1)	10.00	F-I	-582/0
L-B	-265/0	0.0	0.0	0.04 (1)	7.81		
I-G	-265/0	0.0	0.0	0.04 (1)	7.81		
L-K	0/468	-18.5	-18.5	0.38 (4)	10.00		
K-J	0/424	-18.5	-18.5	0.30 (4)	10.00		
J-I	0/477	-18.5	-18.5	0.30 (4)	10.00		

TOTAL WEIGHT = 97 lb (MIF)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.23/1.00 (B-C:1), BC=0.38/1.00 (K-L:4), WB=0.79/1.00 (F-I:1), SS=0.14/1.00 (J-K:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

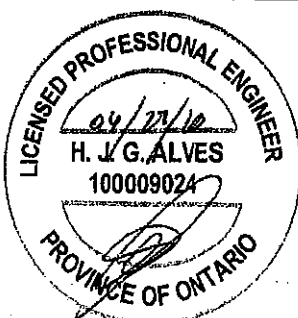
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	818	354	1667
		788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (C) (INPUT = 0.90)
JSI METAL = 0.31 (C) (INPUT = 1.00)

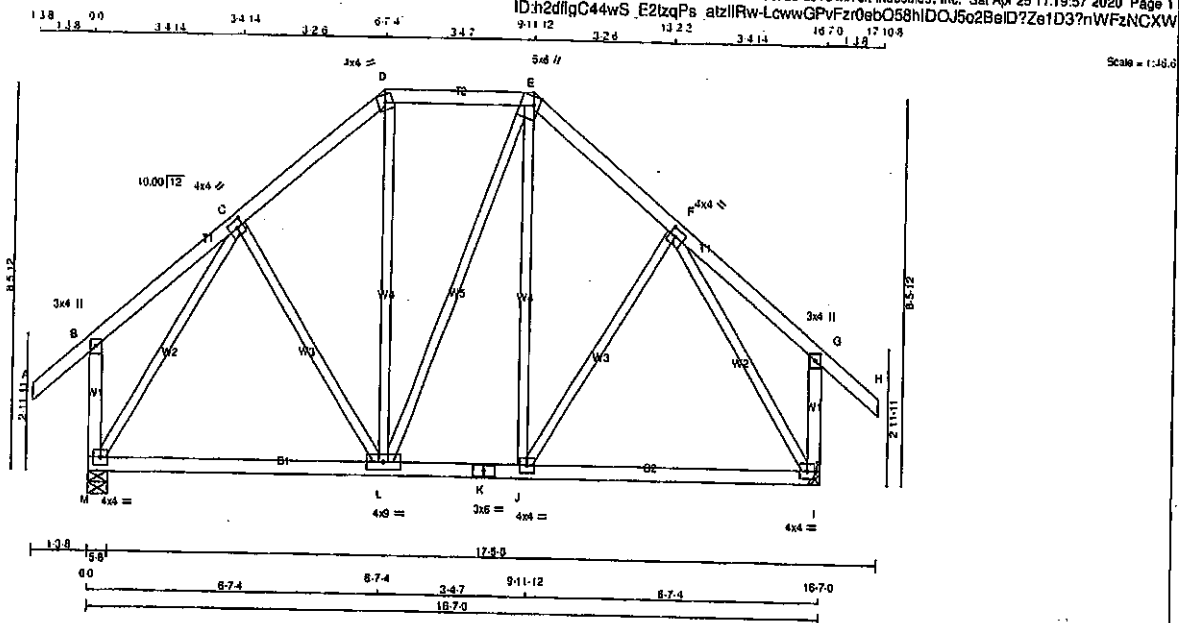


Structural component only
DWG# T-2007092

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408151	T31	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS			
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
M - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
M - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-I	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TTWV+m	MT20	5.0	6.0	2.25	1.50
F	TMVW-I	MT20	4.0	4.0	2.00	1.75
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-I	MT20	4.0	4.0		
J	BMVW-I	MT20	4.0	4.0		
K	BS-I	MT20	3.0	8.0		
L	BMVWVW-I	MT20	4.0	8.0		
M	BMVW1-I	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
M	1041	0	1041	0
I	1041	0	1041	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN.	COMPONENT REACTIONS			
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
M	734	495 / 0	0 / 0	0 / 0	0 / 0	239 / 0
I	734	495 / 0	0 / 0	0 / 0	0 / 0	239 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED		
	FORCE	VERT. LOAD LC1	MAX.		FORCE	MAX.	
	(LBS)	(PLF)	CSI (LC)		(LBS)	CSI (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00	C-L	-10 / 34	0.01 (4)
B-C	0 / 22	-91.8	-91.8 0.16 (1)	10.00	L-D	0 / 110	0.03 (4)
C-D	-513 / 0	-91.8	-91.8 0.13 (1)	6.25	L-E	0 / 0	0.00 (1)
D-E	-453 / 0	-91.8	-91.8 0.14 (1)	6.25	J-E	0 / 109	0.03 (4)
E-F	-612 / 0	-91.8	-91.8 0.13 (1)	6.25	J-F	-11 / 34	0.01 (4)
F-G	0 / 22	-91.8	-91.8 0.16 (1)	10.00	M-C	-873 / 0	0.60 (1)
G-H	0 / 41	-91.8	-91.8 0.13 (1)	10.00	F-I	-873 / 0	0.60 (1)
M-B	-245 / 0	0.0	0.0 0.04 (1)	7.81			
I-G	-245 / 0	0.0	0.0 0.04 (1)	7.81			
M-L	0 / 457	-18.5	-18.5 0.21 (4)	10.00			
L-K	0 / 453	-18.5	-18.5 0.20 (4)	10.00			
K-J	0 / 453	-18.5	-18.5 0.20 (4)	10.00			
J-I	0 / 457	-18.5	-18.5 0.21 (4)	10.00			

TOTAL WEIGHT = 92 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 21.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.07")

CSI: TC=0.18/1.00 (F-G:1), BC=0.21/1.00 (I-J:4),
WB=0.60/1.00 (C-M:1), SS=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (FLI)
MAX MIN MAX MIN MAX MIN
MT20 610 354 1687 788 1987 1686

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

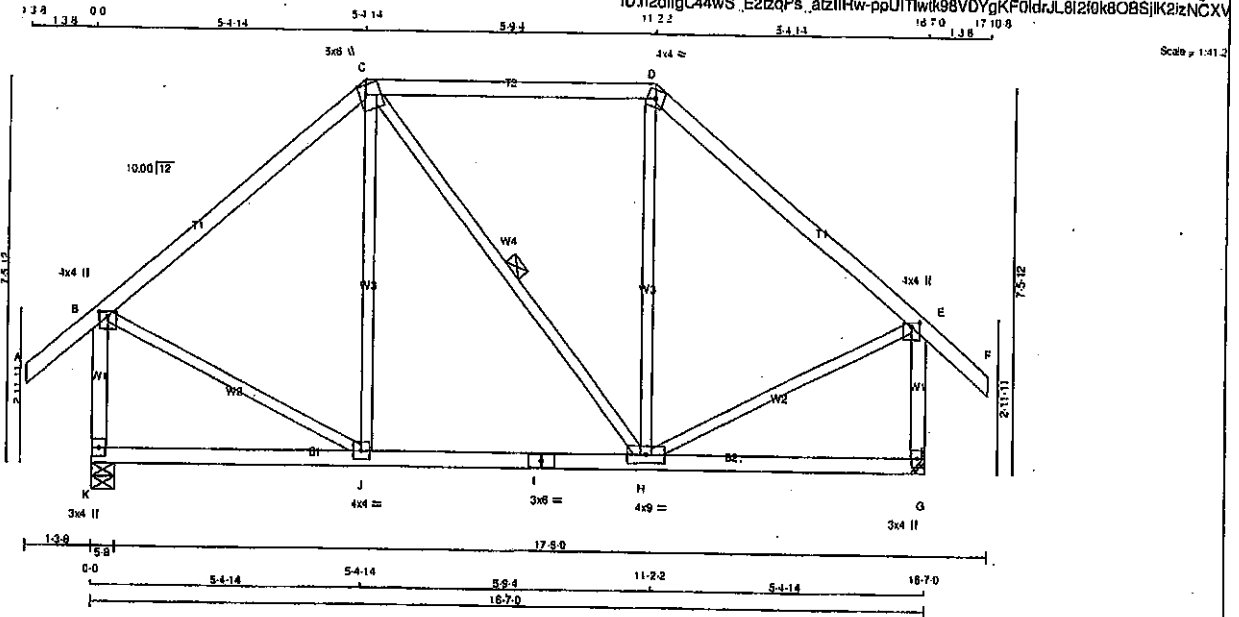
JSI GRIP = 0.87 (C) (INPUT = 0.90)
JSI METAL = 0.31 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007093

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408151	T32	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 79 lb (M/F)

LUMBER	N. L. G. A. RULES	SIZE	DRY	LUMBER	DESCR.
CHORDS					
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWV+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMVWV-t	MT20	4.0	9.0		
I	BS-1	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
K	1041	0	1041	0
G	1041	0	1041	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	734	495 / 0	0 / 0	0 / 0	0 / 0	239 / 0	0 / 0
G	734	495 / 0	0 / 0	0 / 0	0 / 0	239 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
FR-TO												
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	J-C	-115 / 46	0.12 (1)				
B-C	-619 / 0	-91.8	-91.8	0.35 (1)	6.25	C-H	0 / 0	0.00 (1)				
C-D	-473 / 0	-91.8	-91.8	0.39 (1)	6.25	H-D	-115 / 46	0.12 (1)				
D-E	-619 / 0	-91.8	-91.8	0.35 (1)	6.25	B-J	0 / 526	0.12 (1)				
E-F	0 / 41	-91.8	-91.8	0.13 (1)	10.00	H-E	0 / 526	0.12 (1)				
K-B	-1000 / 0	0.0	0.0	0.16 (1)	7.81							
G-E	-1000 / 0	0.0	0.0	0.16 (1)	7.81							
K-J	0 / 0	-18.5	-18.5	0.13 (4)	10.00							
J-I	0 / 473	-18.5	-18.5	0.17 (4)	10.00							
I-H	0 / 473	-18.5	-18.5	0.17 (4)	10.00							
H-G	0 / 0	-18.5	-18.5	0.13 (4)	10.00							

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.39/1.00 (C-D:1), BC=0.17/1.00 (H-J:4), WB=0.12/1.00 (B-J:1), SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

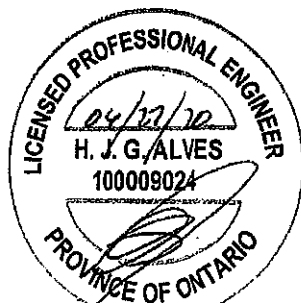
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PU)
MAX MIN MAX MIN MAX MIN
MT20 518 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

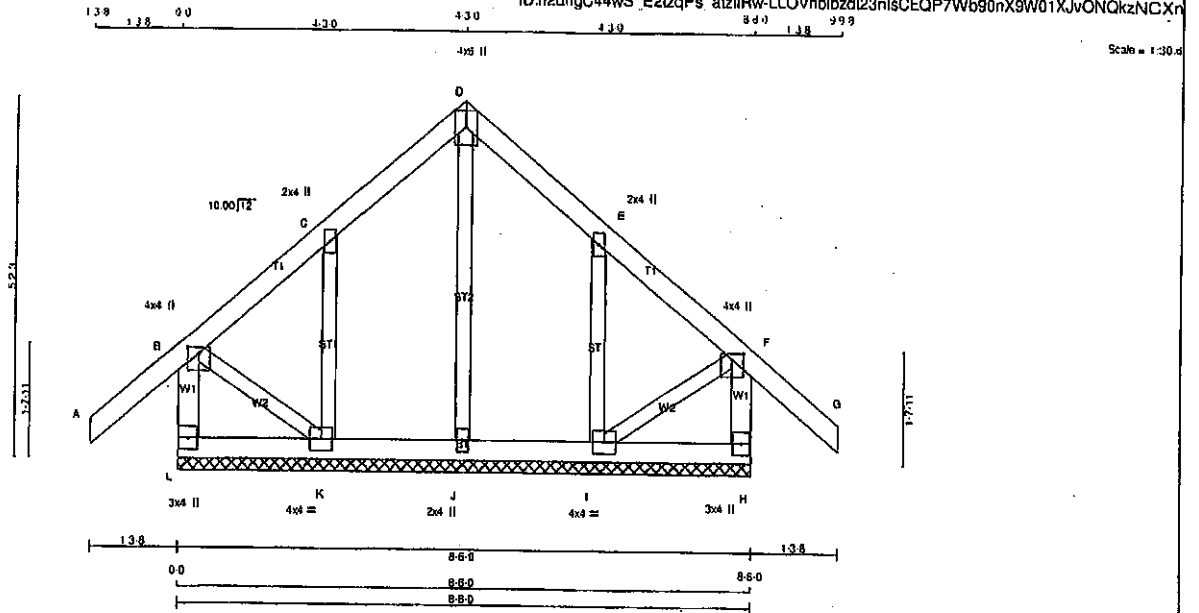
JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007094

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408151	G33	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
L - B	2x4 DRY	No.2	SPF
A - D	2x4 DRY	No.2	SPF
D - G	2x4 DRY	No.2	SPF
H - F	2x4 DRY	No.2	SPF
L - H	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
ALL GABLE WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMW+p	MT20	4.0	4.0	1.00 2.00
C	TMW+w	MT20	2.0	4.0	
D	TTW+p	MT20	4.0	6.0	Edge
E	TMW+w	MT20	2.0	4.0	
F	TMW+p	MT20	4.0	4.0	1.00 2.00
H	BMV1+p	MT20	3.0	4.0	
I	BMW1-t	MT20	4.0	4.0	
J	BMW1+w	MT20	2.0	4.0	
K	BMW1-t	MT20	4.0	4.0	
L	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLW SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	FR-TO
L-B	-231/0	0.0	0.0 0.02 (1)	J-D	-123/0	0.05 (1)	
A-B	0/41	-91.8	-91.8 0.13 (1)	K-C	-236/0	0.05 (1)	
B-C	-11/0	-91.8	-91.8 0.07 (1)	I-E	-236/0	0.05 (1)	
C-D	-32/0	-91.8	-91.8 0.07 (1)	B-K	0/23	0.01 (1)	
D-E	-32/0	-91.8	-91.8 0.07 (1)	I-F	0/23	0.01 (1)	
E-F	-11/0	-91.8	-91.8 0.07 (1)				
F-G	0/41	-91.8	-91.8 0.13 (1)				
H-F	-231/0	0.0	0.0 0.02 (1)				
L-K	0/0	-18.5	-18.5 0.02 (4)				
K-J	0/13	-18.5	-18.5 0.02 (4)				
J-I	0/13	-18.5	-18.5 0.02 (4)				
I-H	0/0	-18.5	-18.5 0.02 (4)				

TOTAL WEIGHT = 42 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NSCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (F-G:1), BC=0.02/1.00 (J-K:4), WB=0.05/1.00 (C-K:1), SS=0.08/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

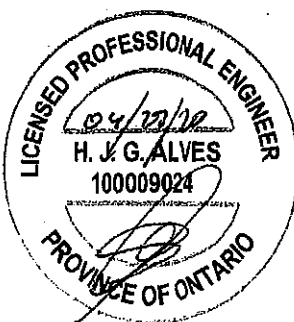
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.18 (C) (INPUT = 0.90)
JSI METAL = 0.13 (C) (INPUT = 1.00)

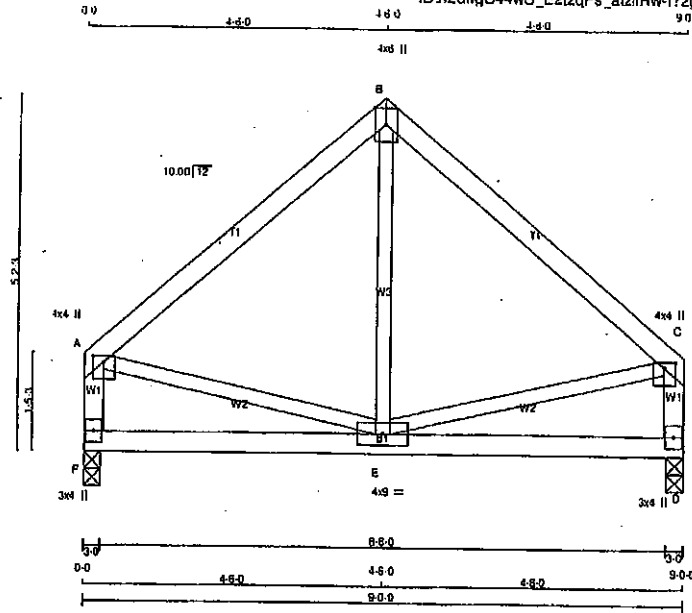


Structural component only
DWG# T-2007078

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408151	T33X	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/2" = 1'-0"

TOTAL WEIGHT = 37 lb (MIF)

LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW+p	MT20	4.0	4.0	1.00	2.00
B	TTW+p	MT20	4.0	5.0	Edge	
C	TMW+p	MT20	4.0	4.0	1.00	2.00
D	BMV+p	MT20	3.0	4.0		
E	BMW+p	MT20	4.0	3.0		
F	BMV+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	496	0	496	0
D	496	0	496	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	351	230 / 0	0 / 0	0 / 0	0 / 0	121 / 0	0 / 0
D	351	230 / 0	0 / 0	0 / 0	0 / 0	121 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-311 / 0	-91.8 -91.8	0.24 (1)	8.25	E-B	-14 / 72	0.03 (4)
B-C	-311 / 0	-91.8 -91.8	0.24 (1)	8.25	A-E	0 / 246	0.06 (1)
F-A	-464 / 0	0.0 0.0	0.05 (1)	7.81	E-C	0 / 246	0.06 (1)
D-C	-464 / 0	0.0 0.0	0.05 (1)	7.81			
F-E	0 / 0	-18.5 -18.5	0.11 (4)	10.00			
E-D	0 / 0	-18.5 -18.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.24/1.00 (A-B:1), BC=0.11/1.00 (E-F:4), WB=0.06/1.00 (C-E:1), SE=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (A) (INPUT = 0.90)
JSI METAL= 0.12 (A) (INPUT = 1.00)



Structural component only
DWG# T-2007095

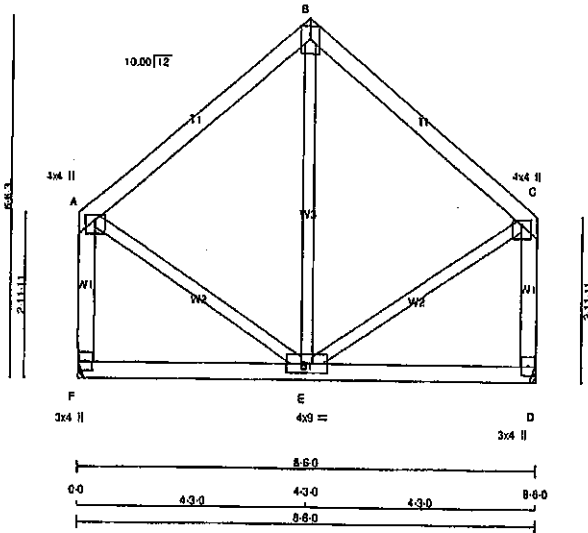
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408151	T34	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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10 4.30 4.30 4.30 9.40
3x8 II

Scale = 1/8" = 1'-0"



LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
A - B	2x4	DRY	No.2		SPF
B - C	2x4	DRY	No.2		SPF
F - A	2x4	DRY	No.2		SPF
D - C	2x4	DRY	No.2		SPF
F - D	2x4	DRY	No.2		SPF
ALL WEBS		2x3	DRY	No.2	SPF
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTW+p	MT20	4.0	8.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMVWW-l	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	469	0	469	0
D	469	0	469	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	331	217/0	0/0	0/0	0/0	114/0	0/0
D	331	217/0	0/0	0/0	0/0	114/0	0/0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-218/0	-91.8	-91.8 0.21 (1)	8.25	E-B	-111/32	0.07 (1)
B-C	-218/0	-91.8	-91.8 0.21 (1)	8.25	A-E	0/197	0.04 (1)
F-A	-438/0	0.0	0.0 0.07 (1)	7.81	E-C	0/197	0.04 (1)
D-C	-438/0	0.0	0.0 0.07 (1)	7.81			
F-E	0/0	-18.5	-18.5 0.09 (4)	10.00			
E-D	0/0	-18.5	-18.5 0.09 (4)	10.00			

TOTAL WEIGHT = 2 X 41 = 82 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 8.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.28")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.28")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.01")

CSI: TC=0.21/1.00 (A-B:1), BC=0.09/1.00 (E-F:4),
WB=0.07/1.00 (B-E:1), SSI=0.12/1.00 (A-B:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	788	1987	1656

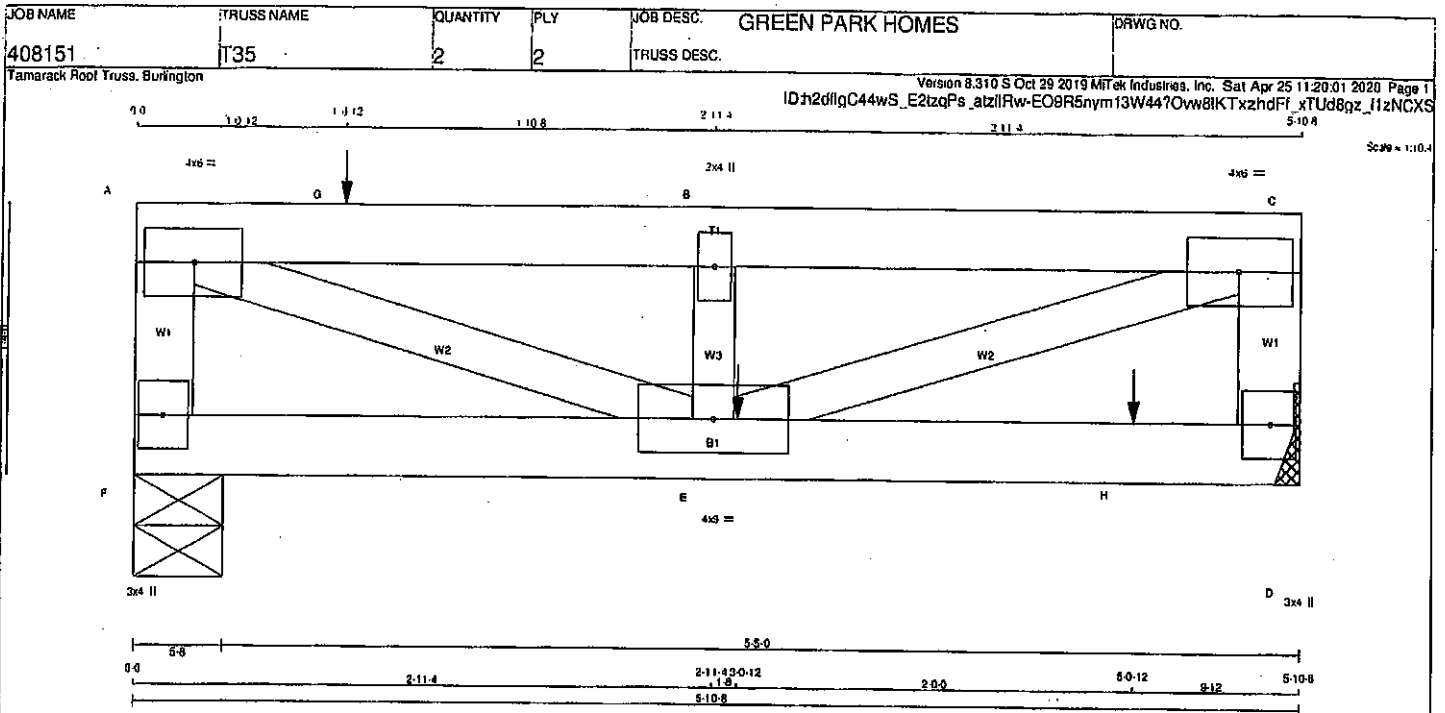
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.33 (A) (INPUT = 0.90)
JSI METAL = 0.09 (A) (INPUT = 1.00)



Structural component only
DWG# T-2007096



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A	12	TOP
A-C	12	TOP
C-D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D	11	SIDE(97.1)
WEBS : (0.122"x3") SPIRAL NAILS		
B-E	6	SIDE(20.3)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (Table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	2.0	4.0		
B	TMVW-1	MT20	2.0	4.0		
C	TMVW-1	MT20	4.0	4.0		
D	BMVW-1	MT20	3.0	4.0		
E	BMVW-1	MT20	4.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQ'D BRG
JT	VERT	HORZ	UPLIFT	IN-SX
F	1016	0	0	5-8
D	1085	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	728	436/0	0/0	0/0	0/0	290/0	0/0
D	772	479/0	0/0	0/0	0/0	293/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS				WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1	MAX. LC2	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1	MAX. LC2
FR-TO		FROM TO			FR-TO			
F-A	-985/0	0.0	0.0	0.05 (1)	7.81	A-E	0/1810	0.22 (1)
A-G	-1706/0	-91.8	-91.8	0.28 (1)	8.23	E-B	-482/0	0.04 (1)
G-B	-1706/0	-91.8	-91.8	0.28 (1)	8.23	E-C	0/1810	0.22 (1)
B-C	-1706/0	-91.8	-91.8	0.08 (1)	8.25			
D-C	-707/0	0.0	0.0	0.04 (1)	7.81			
F-E	0/0	-43.5	-43.5	0.05 (4)	10.00			
E-H	0/0	-43.5	-43.5	0.22 (1)	10.00			
H-D	0/0	-43.5	-43.5	0.22 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-0-12	-450	-450	---	FRONT	VERT	TOTAL	---	C1
G	1-0-12	-404	-404	---	TOP	VERT	TOTAL	---	C1
H	5-0-12	-452	-452	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	= 25.8	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.03")

CSI: TC=0.28/1.00 (A-B:1), BC=0.22/1.00 (D-E:1).
WB=0.22/1.00 (A-E:1), SS=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEAD=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL, IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	818	354
	1667	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (C) (INPUT = 0.90)
JSI METAL= 0.21 (C) (INPUT = 1.00)

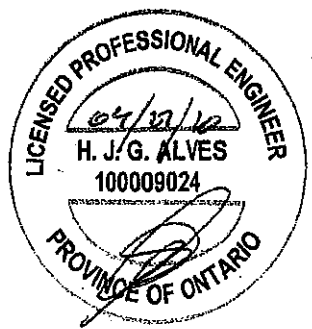


Structural component only
DWG# T-2007097 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408151	T35	2	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		Version 8.310 5 Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 11:20:01 2020 Page 2			
		ID:h2dl1qC44wS E2tzcPs atzllRw-EO9R5nym13W44?Ovw8lKTxzhdfF xTUd8gz l1zNCXS			
		TRUSS DESC.			

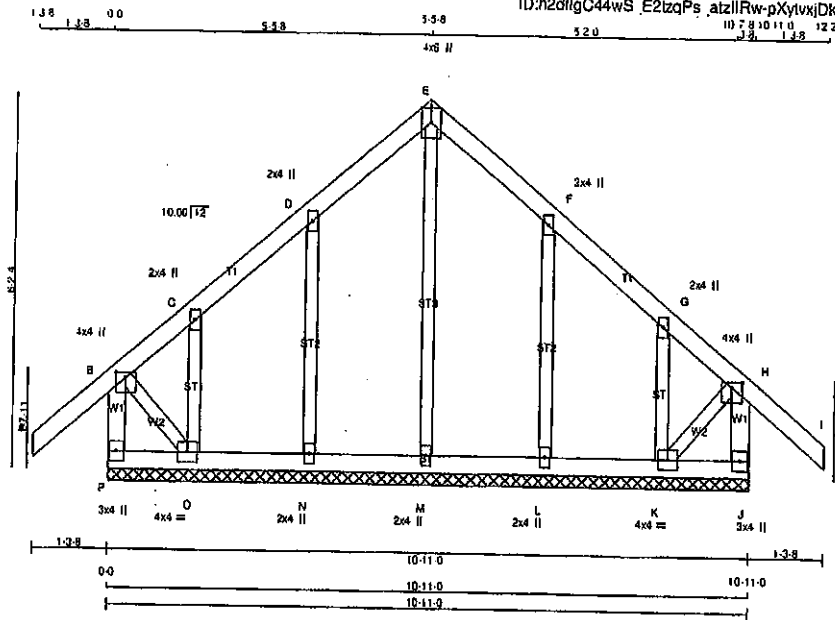
Version 8.310 5 Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 11:20:01 2020 Page 2
ID:h2dl1qC44wS E2tzcPs atzllRw-EO9R5nym13W44?Ovw8lKTxzhdfF xTUd8gz l1zNCXS

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
F	BMV1+p	MT20	3.0	4.0	



Structural component only
DWG# T-2007097 3/2

JOB NAME	TRUSS NAME	QUANTITY	PLV	JOB DESC.	DRWG NO.
408151	G36	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Sat Apr 25 11:19:41 2020 Page 1					
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10 7 9 10 11 0 12 2 8					
Scale = 1:35.2					



CHORDS	SIZE	DRY	No.2
P - B	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0\"/>

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G					
C TMVW+w	MT20	2.0	4.0		
E TTW+p	MT20	4.0	8.0	Edge	
H TMVW+p	MT20	4.0	4.0	1.00	2.00
J BMV1+p	MT20	3.0	4.0		
K BMVW1-i	MT20	4.0	4.0		
L, M, N					
L BMV1+w	MT20	2.0	4.0		
O BMVW1-i	MT20	4.0	4.0		
P BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO				FR-TO			
P-B	-274 / 0	0.0	0.0 (1)	M-E	-139 / 0	0.08 (1)	
A-B	0 / 41	-91.8	0.13 (1)	N-D	-223 / 0	0.07 (1)	
B-C	-57 / 0	-91.8	0.12 (1)	O-C	-75 / 0	0.01 (1)	
C-D	-4 / 0	-91.8	0.08 (1)	L-F	-223 / 0	0.07 (1)	
D-E	-22 / 0	-91.8	0.08 (1)	K-G	-75 / 0	0.01 (1)	
E-F	-22 / 0	-91.8	0.08 (1)	B-O	0 / 20	0.00 (1)	
F-G	-4 / 0	-91.8	0.08 (1)	K-H	0 / 20	0.00 (1)	
G-H	-57 / 0	-91.8	0.12 (1)				
H-I	0 / 41	-91.8	0.13 (1)				
J-H	-274 / 0	0.0	0.0 (1)				
P-O	0 / 0	-18.5	-18.5 0.01 (4)				
O-N	0 / 12	-18.5	-18.5 0.02 (4)				
N-M	0 / 7	-18.5	-18.5 0.02 (4)				
M-L	0 / 7	-18.5	-18.5 0.02 (4)				
L-K	0 / 12	-18.5	-18.5 0.02 (4)				
K-J	0 / 0	-18.5	-18.5 0.01 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 8.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2018 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (H-I:1), BC=0.02/1.00 (K-L:4), WB=0.08/1.00 (E-M:1), SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

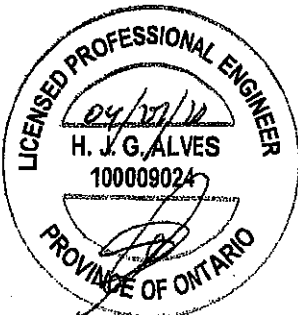
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 394	1867 788	1887 1656

PLATE PLACEMENT TOL. = 0.250 inches

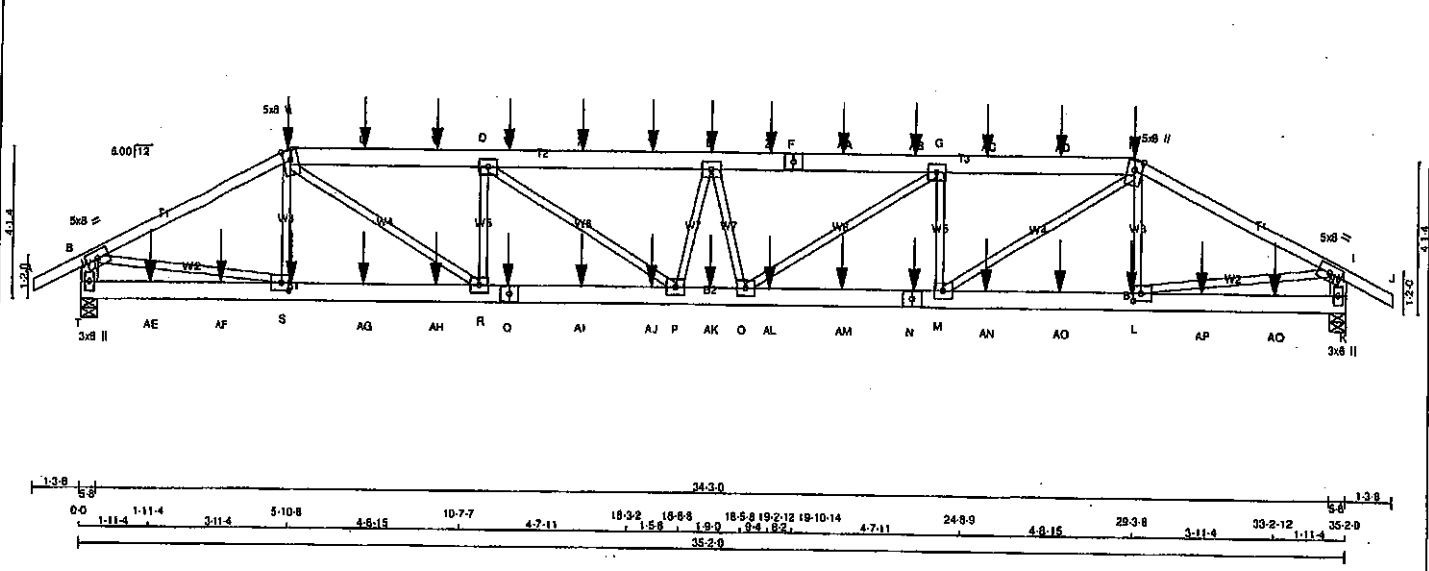
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.12 (F) (INPUT = 1.00)



Structural component only
DWG# T-2007079

ID: 37vF7aG0E03cRUJ8X1jSkzIWYK-0EA1x1sxZWS34YeiZ2E6M6CTQSHDeTgR0niCQzNCOX
 Scale = 1:57.2



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - F	2x6	DRY No.2	SPF
F - H	2x6	DRY No.2	SPF
H - J	2x4	DRY No.2	SPF
T - B	2x6	DRY No.2	SPF
K - I	2x6	DRY No.2	SPF
T - Q	2x6	DRY No.2	SPF
Q - N	2x6	DRY No.2	SPF
N - K	2x6	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE (81.0)
H - J	12	SIDE (81.0)
C - F	12	SIDE (81.0)
F - H	12	SIDE (81.0)
T - B	12	TOP
K - I	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
T - Q	12	SIDE (183.1)
Q - N	12	SIDE (0.0)
N - K	12	SIDE (183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
T	3331	0	0	3331	0	0	5-8	5-8	5-8
K	3331	0	0	3331	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
T	2355	1544 / 0	0 / 0	0 / 0	0 / 0	812 / 0	0 / 0
K	2355	1544 / 0	0 / 0	0 / 0	0 / 0	812 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. (PLF)	UNBRAC. LENGTH (FT)	MEMB.	FORCE (LBS)	MAX. (LBS)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO		FR-TO				
A-B	0 / 28	-91.8	-91.8 0.07 (1)	10.00	S-C	-427 / 8	0.05 (1)		
B-C	-5077 / 0	-91.8	-91.8 0.52 (1)	3.87	C-R	0 / 3083	0.38 (1)		
C-U	-7106 / 0	-91.8	-91.8 0.23 (1)	4.33	R-D	-1585 / 0	0.19 (1)		
U-V	-7106 / 0	-91.8	-91.8 0.23 (1)	4.33	O-G	0 / 1071	0.13 (1)		
V-D	-7106 / 0	-91.8	-91.8 0.23 (1)	4.33	M-G	-1585 / 0	0.19 (1)		
D-W	-7994 / 0	-91.8	-91.8 0.27 (1)	4.10	L-H	-427 / 8	0.05 (1)		
W-X	-7994 / 0	-91.8	-91.8 0.27 (1)	4.10	B-S	0 / 4592	0.57 (1)		
X-Y	-7994 / 0	-91.8	-91.8 0.27 (1)	4.10	I-L	0 / 4592	0.57 (1)		
Y-E	-7994 / 0	-91.8	-91.8 0.27 (1)	4.10	D-P	0 / 1071	0.13 (1)		
E-Z	-7994 / 0	-91.8	-91.8 0.27 (1)	4.10	M-H	0 / 3083	0.38 (1)		
Z-F	-7994 / 0	-91.8	-91.8 0.27 (1)	4.10	P-E	-505 / 0	0.06 (1)		
F-AA	-7994 / 0	-91.8	-91.8 0.27 (1)	4.10	E-O	-505 / 0	0.06 (1)		
AA-AB	-7994 / 0	-91.8	-91.8 0.27 (1)	4.10					
AB-G	-7994 / 0	-91.8	-91.8 0.27 (1)	4.10					
G-AC	-7106 / 0	-91.8	-91.8 0.23 (1)	4.33					
AC-AD	-7106 / 0	-91.8	-91.8 0.23 (1)	4.33					
AD-H	-7106 / 0	-91.8	-91.8 0.23 (1)	4.33					
H-I	-5077 / 0	-91.8	-91.8 0.52 (1)	3.87					
I-J	0 / 28	-91.8	-91.8 0.07 (1)	10.00					
T-B	-3255 / 0	0.0	0.0 0.11 (1)	7.89					
K-I	-3255 / 0	0.0	0.0 0.11 (1)	7.89					

TOTAL WEIGHT = 2 X 170 = 341 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, CBC 2012, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 088-08, CSA 088-14
 - TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/380 (1.17")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.21")
 ALLOWABLE DEFL. (TL) = L/380 (1.17")
 CALCULATED VERT. DEFL. (TL) = L/999 (0.39")

CSI: TC=0.521/0.0 (H-E), BC=0.56/1.00 (O-P), I, WB=0.571/0.0 (B-S), SSI=0.16/1.00 (D-E)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HELLS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 818 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.260 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (S) (INPUT = 0.80)
 JSI METAL = 0.84 (N) (INPUT = 1.00)



Structural component only
 DWG# T-2007107 1/2

JOB NAME 408152	TRUSS NAME T40	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Sat Apr 25 11:29:32 2020 Page 2 ID:17vF?aG0E03cRU6X1ISdkzWYK-0EAlk1sxZWS34IYeZ2E8M6CTQSHDgTgR0niCQzNCOX	

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMVW-t MT20	5.0	8.0		
C TTWW-m MT20	5.0	8.0	Edge	
D, E, G				
D TMVW-t MT20	5.0	8.0		
F TS-t MT20	5.0	6.0		
H TTWW-m MT20	5.0	8.0	Edge	
I TMVW-t MT20	5.0	8.0		
K BMV1-p MT20	3.0	6.0		
L BMVW-t MT20	5.0	6.0	2.50	2.50
M, O, P, R				
M BMVW-t MT20	5.0	6.0		
N BS-t MT20	5.0	6.0		
Q BS-t MT20	5.0	6.0		
S BMVW-t MT20	5.0	6.0	2.50	2.50
T BMV1-p MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: (4)

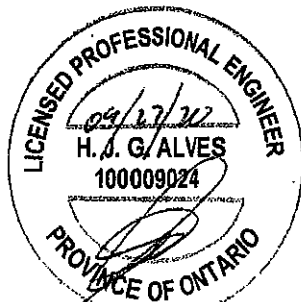
CHORDS		WEBS	
MAX. FACTORED	FACTORED	MAX. FACTORED	FACTORED
MEMB. FORCE (LBS)	VERT. LOAD LC1 (PLF)	MEMB. FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO	FROM TO	LENGTH	FR-TO
AQ-K	0.0 -18.5 -18.5 0.06 (4)	10.00	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-438	-438	---	FRONT	VERT	TOTAL	---	C1
E	17-7-0	-110	-110	---	FRONT	VERT	TOTAL	---	C1
H	29-3-8	-438	-438	---	FRONT	VERT	TOTAL	---	C1
L	29-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
N	23-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Q	11-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
S	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
U	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	11-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	15-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	19-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	21-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	23-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	25-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	27-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AE	1-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AF	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AG	7-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AH	9-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AI	13-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AJ	15-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AK	17-7-0	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AL	19-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AM	21-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AN	25-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AO	27-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AP	31-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AQ	33-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

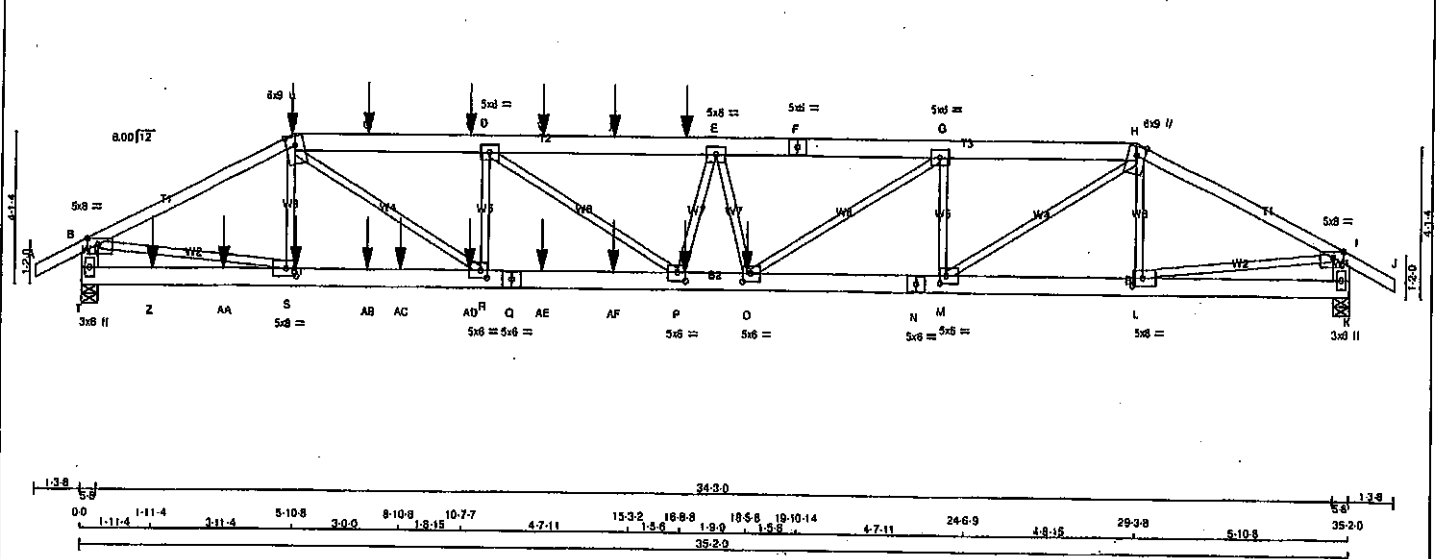
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007107 2/2

JOB NAME 408152	TRUSS NAME T40Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 11:29:34 2020 Page 1
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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - F	2x6	DRY	No.2	SPF		
F - H	2x6	DRY	No.2	SPF		
H - J	2x4	DRY	No.2	SPF		
T - B	2x6	DRY	No.2	SPF		
K - I	2x6	DRY	No.2	SPF		
T - Q	2x6	DRY	No.2	SPF		
Q - N	2x6	DRY	No.2	SPF		
N - K	2x6	DRY	No.2	SPF		

ALL WEBS	2x3	DRY	No.2	SPF
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DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
A - C	1	12	SIDE (61.0)
H - J	1	12	TOP
C - F	2	12	SIDE (61.0)
F - H	2	12	TOP
T - B	2	12	TOP
K - I	2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
T - Q	2	12	SIDE (183.1)
Q - N	2	12	SIDE (183.1)
N - K	2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS			
2x3	1	8	TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	4474	0	0	5-8
K VERT	3775	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
T	3182 2088 / 0 0 / 0 0 / 0 1073 / 0 0 / 0
K	2665 1772 / 0 0 / 0 0 / 0 893 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO			
A-B	0 / 28	-91.8	-91.8	0.07 (1)	S-C	-384 / 25	0.05 (1)	10.00
B-C	-7183 / 0	-91.8	-91.8	0.71 (1)	C-R	0 / 4087	0.50 (1)	3.19
C-U	-10289 / 0	-91.8	-91.8	0.32 (1)	R-D	-2057 / 0	0.25 (1)	3.62
U-V	-10289 / 0	-91.8	-91.8	0.32 (1)	D-Q	0 / 3611	0.45 (1)	3.62
V-D	-10289 / 0	-91.8	-91.8	0.32 (1)	Q-G	0 / 3611	0.45 (1)	3.62
D-W	-12012 / 0	-91.8	-91.8	0.45 (1)	W-H	-615 / 0	0.32 (1)	3.27
W-X	-12012 / 0	-91.8	-91.8	0.45 (1)	H-S	0 / 6479	0.50 (1)	3.27
X-Y	-12012 / 0	-91.8	-91.8	0.45 (1)	Y-L	0 / 5333	0.50 (1)	3.27
Y-E	-12012 / 0	-91.8	-91.8	0.45 (1)	E-P	0 / 2086	0.28 (1)	3.27
E-F	-12381 / 0	-91.8	-91.8	0.40 (1)	P-M	0 / 4950	0.61 (1)	3.29
F-G	-12381 / 0	-91.8	-91.8	0.40 (1)	G-O	-1110 / 0	0.14 (1)	3.29
G-H	-9385 / 0	-91.8	-91.8	0.25 (1)	O-E	0 / 383	0.05 (1)	3.84
H-I	-5898 / 0	-91.8	-91.8	0.59 (1)				3.58
I-J	0 / 28	-91.8	-91.8	0.07 (1)				10.00
T-B	-4430 / 0	0.0	0.0	0.16 (1)				6.82
K-I	-3716 / 0	0.0	0.0	0.13 (1)				7.31

T-Z	0 / 0	-18.5	-18.5	0.08 (4)	10.00
Z-AA	0 / 0	-18.5	-18.5	0.08 (4)	10.00
AA-S	0 / 0	-18.5	-18.5	0.08 (4)	10.00
S-AB	0 / 6432	-18.5	-18.5	0.69 (1)	10.00
AB-AC	0 / 6432	-18.5	-18.5	0.69 (1)	10.00
AC-AD	0 / 6432	-18.5	-18.5	0.69 (1)	10.00
AD-R	0 / 6432	-18.5	-18.5	0.69 (1)	10.00
R-Q	0 / 10298	-18.5	-18.5	0.76 (1)	10.00
Q-AE	0 / 10298	-18.5	-18.5	0.76 (1)	10.00
AE-AF	0 / 10298	-18.5	-18.5	0.76 (1)	10.00
AF-P	0 / 10298	-18.5	-18.5	0.76 (1)	10.00
P-O	0 / 12286	-18.5	-18.5	0.91 (1)	10.00
O-N	0 / 9385	-18.5	-18.5	0.72 (1)	10.00
N-M	0 / 9385	-18.5	-18.5	0.72 (1)	10.00
M-L	0 / 5303	-18.5	-18.5	0.37 (1)	10.00
L-K	0 / 0	-18.5	-18.5	0.04 (4)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-438	-438	---	BACK	VERT	TOTAL	---	C1
O	18-5-8	-2436	-2436	---	BACK	VERT	TOTAL	---	C1
P	16-9-12	-17	-17	---	BACK	VERT	TOTAL	---	C1
S	5-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
U	7-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
V	10-9-12	-82	-82	---	BACK	VERT	TOTAL	---	C1
W	12-9-12	-82	-82	---	BACK	VERT	TOTAL	---	C1

TOTAL WEIGHT = 2 X 170 = 341 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 8.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = 1/989 (0.31")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = 1/740 (0.57")

CSI: TC=0.71/1.00 (B-C:1), BC=0.91/1.00 (C-P:1), WB=0.80/1.00 (B-S:1), SS=0.15/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR SECTION (PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN

MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (S) (INPUT = 0.90)

JSI METAL = 0.93 (N) (INPUT = 1.00)

Structural component only
DWG# T-2007108

JOB NAME 408152	TRUSS NAME T40Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Metek Industries, Inc. Sat Apr 25 11:29:34 2020 Page 2 ID: i7vF?aG0E03cRU6X1/SrkziWYK-ydH2MiuB57imK311? 4iCnCV?E2GhUJzyKGnGJzNCOV	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	2.00	3.50
C	TTWW-m	MT20	8.0	9.0	Edge	
D	TS-l	MT20	5.0	6.0		
F	TS-l	MT20	5.0	6.0		
H	TTWW-m	MT20	8.0	9.0	Edge	
I	TMVW-p	MT20	5.0	8.0	2.00	3.50
K	BMV1-p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	8.0	2.50	3.50
M	BMWW-t	MT20	5.0	6.0	2.50	2.00
N	BS-l	MT20	8.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.75	2.75
P	BMWW-l	MT20	5.0	8.0	2.75	2.75
Q	BS-l	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.00
S	BMWW-l	MT20	5.0	8.0	2.50	3.50
T	BMV1-p	MT20	3.0	6.0		

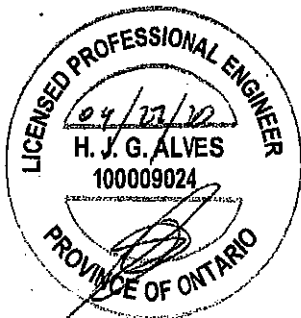
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
X	14-9-12	-82	-82	---	BACK	VERT	TOTAL	---	C1
Y	16-9-12	-82	-82	---	BACK	VERT	TOTAL	---	C1
Z	1-11-4	-25	-25	---	BACK	VERT	TOTAL	---	C1
AA	3-11-4	-28	-28	---	BACK	VERT	TOTAL	---	C1
AB	7-11-4	-28	-28	---	BACK	VERT	TOTAL	---	C1
AC	8-10-8	-839	-839	---	BACK	VERT	TOTAL	---	C1
AD	10-9-12	-17	-17	---	BACK	VERT	TOTAL	---	C1
AE	12-9-12	-17	-17	---	BACK	VERT	TOTAL	---	C1
AF	14-9-12	-17	-17	---	BACK	VERT	TOTAL	---	C1

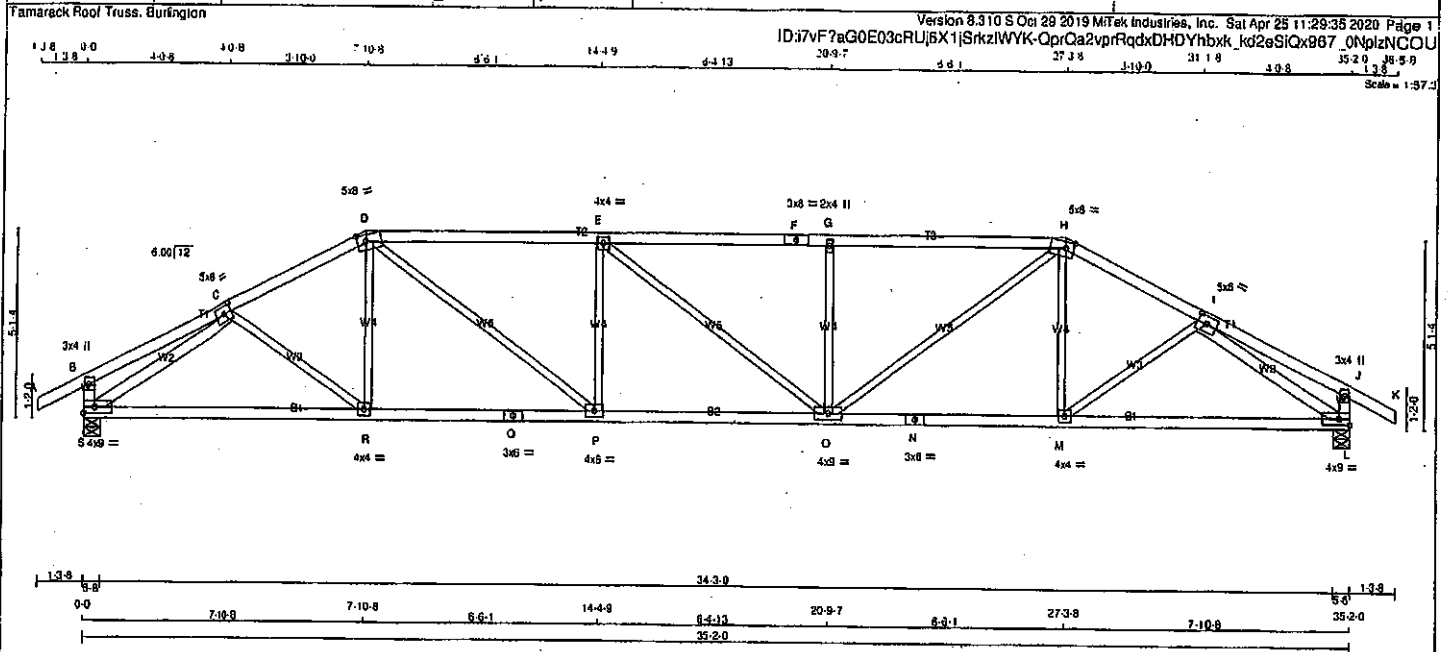
CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007108

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408152	T41	2	1	GREEN PARK HOMES	



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
S - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMV+P	MT20	3.0	4.0	
C	TMVW+I	MT20	5.0	8.0	2.50 2.75
D	TTWW-m	MT20	5.0	8.0	2.25 2.75
E	TMVW+I	MT20	4.0	4.0	
F	TS-I	MT20	3.0	8.0	
G	TMVW+P	MT20	2.0	4.0	
H	TTWW-m	MT20	5.0	8.0	2.25 2.75
I	TMVW+I	MT20	5.0	8.0	2.50 2.75
J	TMV+P	MT20	3.0	4.0	
L	BMVW+I	MT20	4.0	9.0	Edge
M	BMVW+I	MT20	4.0	4.0	
N	BS-I	MT20	3.0	8.0	
O	BMVW+I	MT20	4.0	9.0	
P	BMVW+I	MT20	4.0	6.0	
Q	BS-I	MT20	3.0	8.0	
R	BMVW+I	MT20	4.0	4.0	
S	BMVW+I	MT20	4.0	9.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
	GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
S	2063	0	2063	0	0	5-8	5-8	
L	2063	0	2063	0	0	5-8	5-8	

UNFACTORED REACTIONS

	MAX/MIN. COMPONENT REACTIONS						
	1ST LCASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
JT	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
L	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	MAX. FORCE	VERT. LOAD	LC1	MAX	MAX. UNBRAC	MEMB.	MAX. FORCE	MAX
FR-TO	(LBS)	(PLF)	CS1 (LC)	LENGTH	FR-TO	(LBS)	CS1 (LC)	
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	C-R	0 / 93	0.03 (4)	
B-C	0 / 18	-91.8	-91.8 0.20 (1)	10.00	R-D	0 / 121	0.04 (4)	
C-D	-2799 / 0	-91.8	-91.8 0.32 (1)	3.88	D-P	0 / 1267	0.29 (1)	
D-E	-3508 / 0	-91.8	-91.8 0.95 (1)	2.94	P-E	-843 / 0	0.25 (1)	
E-F	-3507 / 0	-91.8	-91.8 0.93 (1)	2.94	E-O	-2 / 0	0.00 (1)	
F-G	-3507 / 0	-91.8	-91.8 0.93 (1)	2.94	O-G	-843 / 0	0.25 (1)	
G-H	-3507 / 0	-91.8	-91.8 0.94 (1)	2.95	O-H	0 / 1266	0.28 (1)	
H-I	-2799 / 0	-91.8	-91.8 0.32 (1)	3.88	M-H	0 / 121	0.04 (4)	
I-J	0 / 18	-91.8	-91.8 0.20 (1)	10.00	M-I	0 / 93	0.03 (4)	
J-K	0 / 28	-91.8	-91.8 0.12 (1)	10.00	S-C	-2974 / 0	0.83 (1)	
S-B	-270 / 0	0.0	0.0 0.03 (1)	7.81	I-L	-2974 / 0	0.83 (1)	
L-J	-270 / 0	0.0	0.0 0.03 (1)	7.81				
S-R	0 / 2417	-18.5	-18.5 0.53 (1)	10.00				
R-Q	0 / 2489	-18.5	-18.5 0.54 (1)	10.00				
Q-P	0 / 2489	-18.5	-18.5 0.54 (1)	10.00				
P-O	0 / 3509	-18.5	-18.5 0.84 (1)	10.00				
O-N	0 / 2489	-18.5	-18.5 0.54 (1)	10.00				
N-M	0 / 2489	-18.5	-18.5 0.54 (1)	10.00				
M-L	0 / 2417	-18.5	-18.5 0.53 (1)	10.00				

TOTAL WEIGHT = 2 X 139 = 278 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.41")

CS1: TC=0.95/1.00 (D-E:1), BC=0.84/1.00 (O-P:1), WB=0.83/1.00 (I-L:1), SS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HELLS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.89)
JSI METAL= 0.77 (N) (INPUT = 1.00)

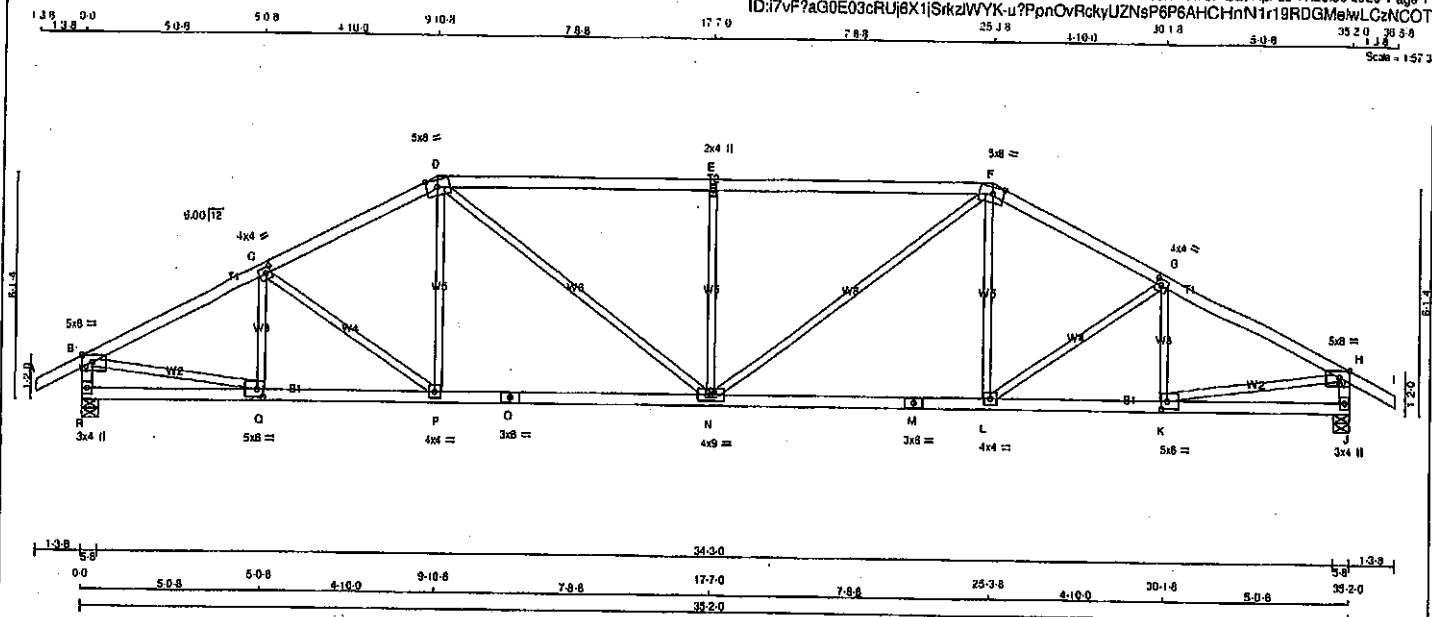


Structural component only
DWG# T-2007109

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408152	T42	2	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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ID: i7v7aG0E03cRUj6X1SkzVWYK-u?PpnOvRckyUZNsP6P6AHCHnN1r19RDGMewLCzNCOT



LUMBER			
N. L. G. A. RULES	SIZE	DRY	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF
DRY: SEASONED LUMBER.			

PLATES (tablets in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	8.0	Edge 3.50
C	TMVW-t	MT20	4.0	4.0	2.00 1.75
D	TTWW-m	MT20	5.0	8.0	2.25 3.75
E	TMVW-w	MT20	2.0	4.0	
F	TTWW-m	MT20	5.0	8.0	2.25 3.75
G	TMVW-t	MT20	4.0	4.0	2.00 1.75
H	TMVW-p	MT20	5.0	8.0	Edge 3.50
J	BMVt-p	MT20	3.0	4.0	
K	BMVW-t	MT20	5.0	8.0	2.50 2.00
L	BMVW-t	MT20	4.0	4.0	
M	BS-t	MT20	3.0	8.0	
N	BMVW-t	MT20	4.0	4.0	
O	BS-t	MT20	3.0	8.0	
P	BMVW-t	MT20	4.0	4.0	
Q	BMVW-t	MT20	5.0	8.0	2.50 2.00
R	BMVt-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
R	2063	0	2063	0	0	5.8	5.8	5.8	5.8
J	2063	0	2063	0	0	5.8	5.8	5.8	5.8

UNFACTORED REACTIONS

1ST LOOSE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1457	989 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
J	1457	989 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. GSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. GSI (LC)	
FR-TO				FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.08	Q-C	-353 / 0	0.08 (1)
B-C	-2834 / 0	-91.8	-91.8 0.39 (1)	3.81	C-P	-210 / 0	0.13 (1)
C-D	-2897 / 0	-91.8	-91.8 0.37 (1)	3.91	P-D	0 / 246	0.06 (4)
D-E	-3080 / 0	-91.8	-91.8 0.97 (1)	2.78	D-N	0 / 844	0.19 (1)
E-F	-3090 / 0	-91.8	-91.8 0.97 (1)	2.78	N-F	-872 / 0	0.51 (1)
F-G	-2897 / 0	-91.8	-91.8 0.37 (1)	3.91	L-F	0 / 844	0.19 (1)
G-H	-2834 / 0	-91.8	-91.8 0.39 (1)	3.81	L-G	0 / 246	0.06 (4)
H-I	0 / 28	-91.8	-91.8 0.12 (1)	10.08	K-G	-210 / 0	0.13 (1)
R-B	-2018 / 0	0.0	0.0 0.20 (1)	5.94	K-Q	-353 / 0	0.08 (1)
J-H	-2018 / 0	0.0	0.0 0.20 (1)	5.94	B-Q	0 / 2591	0.58 (1)
R-Q	0 / 0	-18.5	-18.5 0.10 (4)	10.00	K-H	0 / 2591	0.58 (1)
Q-P	0 / 2555	-18.5	-18.5 0.51 (1)	10.00			
P-O	0 / 2386	-18.5	-18.5 0.49 (1)	10.00			
O-N	0 / 2386	-18.5	-18.5 0.49 (1)	10.00			
N-M	0 / 2386	-18.5	-18.5 0.49 (1)	10.00			
M-L	0 / 2386	-18.5	-18.5 0.49 (1)	10.00			
L-K	0 / 2555	-18.5	-18.5 0.51 (1)	10.00			
K-J	0 / 0	-18.5	-18.5 0.10 (4)	10.00			

TOTAL WEIGHT = 2 X 139 = 277 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.34")

GSI: TC=0.97/1.00 (D-E:1), BC=0.51/1.00 (K-L:1), WB=0.58/1.00 (H-K:1), SS=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

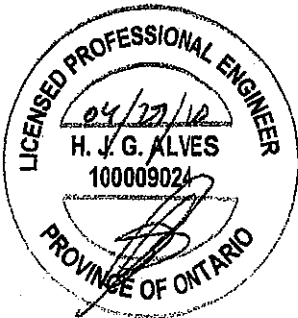
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 364 1687 799 1997 1658

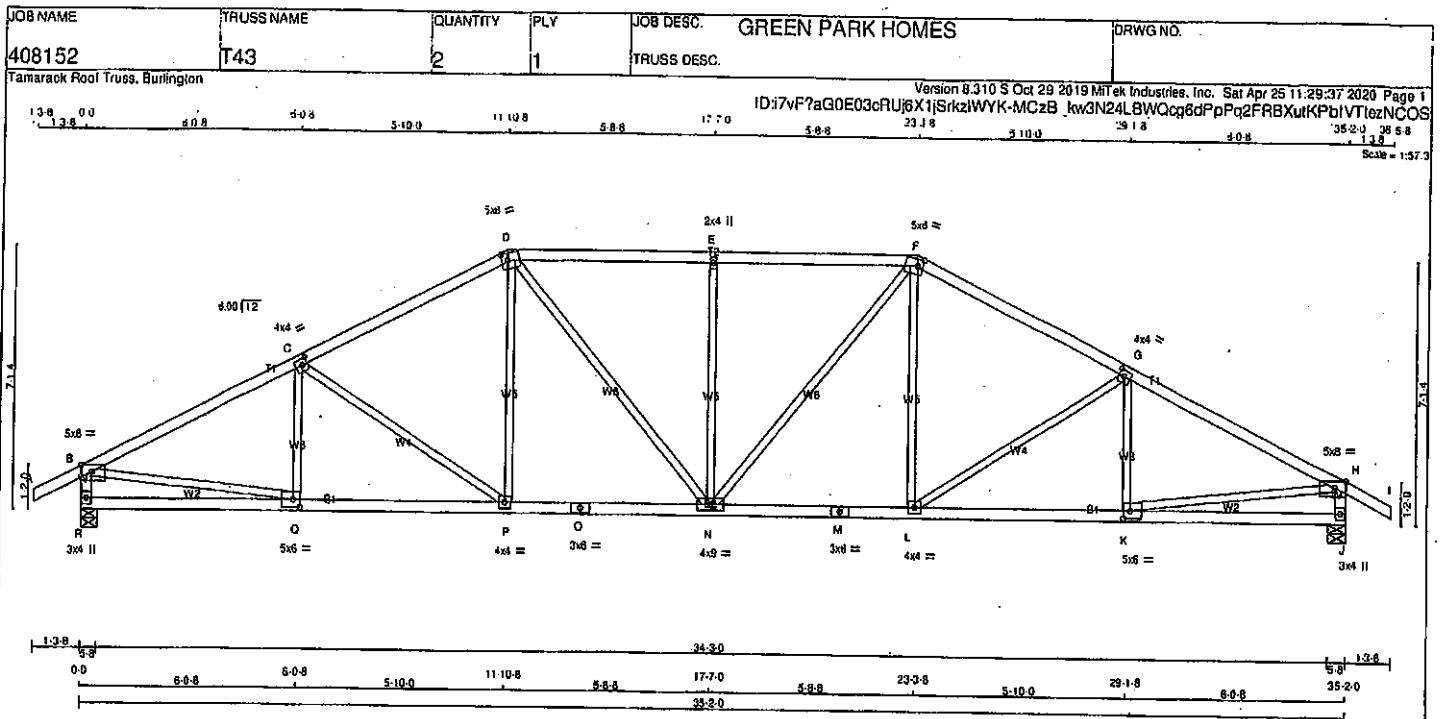
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.87 (D) (INPUT = 0.90)
JSI METAL=0.75 (O) (INPUT = 1.00)



Structural component only
DWG# T-2007110



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

EXCEPT
DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMVW-l	MT20	4.0	4.0	2.00	1.75
D	TTVW-m	MT20	5.0	8.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTVW-m	MT20	5.0	8.0	2.25	2.00
G	TMVW-l	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	5.0	8.0	Edge	3.50
J	BMV1-p	MT20	3.0	4.0		
K	BMVW-l	MT20	5.0	8.0	2.50	2.25
L	BMVW-l	MT20	4.0	4.0		
M	BS-l	MT20	3.0	8.0		
N	BMVW-w-l	MT20	4.0	9.0		
O	BS-l	MT20	3.0	6.0		
P	BMVW-l	MT20	4.0	4.0		
Q	BMVW-l	MT20	5.0	8.0	2.50	2.25
R	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQ'D BRG
	VERT	HORZ	DOWN	HORZ		
R	2063	0	2063	0	5-8	5-8
J	2063	0	2063	0	5-8	5-8

UNFACTORED REACTIONS

JT	MAX. MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
J	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	FACTORED (PLF)	MAX. FACTORED LCI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LCI (LC)	MAX. UNBRAC LENGTH
FR-TO		FROM	TO			FR-TO			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	Q-C	-255 / 12	0.07 (1)	
B-C	-2869 / 0	-91.8	-91.8	0.58 (1)	3.58	C-P	-435 / 0	0.42 (1)	
C-D	-2539 / 0	-91.8	-91.8	0.52 (1)	3.83	P-D	0 / 350	0.08 (1)	
D-E	-2559 / 0	-91.8	-91.8	0.48 (1)	3.85	D-N	0 / 482	0.11 (1)	
E-F	-2559 / 0	-91.8	-91.8	0.48 (1)	3.85	N-E	-541 / 0	0.56 (1)	
F-G	-2539 / 0	-91.8	-91.8	0.52 (1)	3.83	N-F	0 / 482	0.11 (1)	
G-H	-2869 / 0	-91.8	-91.8	0.58 (1)	3.58	L-F	0 / 350	0.08 (1)	
H-I	0 / 28	-91.8	-91.8	0.12 (1)	10.00	L-G	-435 / 0	0.42 (1)	
R-B	-2014 / 0	0.0	0.0	0.20 (1)	5.95	K-G	-255 / 12	0.07 (1)	
J-H	-2014 / 0	0.0	0.0	0.20 (1)	5.95	B-Q	0 / 2635	0.59 (1)	
						K-H	0 / 2635	0.59 (1)	
R-Q	0 / 0	-18.5	-18.5	0.15 (4)	10.00				
Q-P	0 / 2609	-18.5	-18.5	0.49 (1)	10.00				
P-O	0 / 2249	-18.5	-18.5	0.43 (1)	10.00				
O-N	0 / 2249	-18.5	-18.5	0.43 (1)	10.00				
N-M	0 / 2249	-18.5	-18.5	0.43 (1)	10.00				
M-L	0 / 2249	-18.5	-18.5	0.43 (1)	10.00				
L-K	0 / 2809	-18.5	-18.5	0.49 (1)	10.00				
K-J	0 / 0	-18.5	-18.5	0.15 (4)	10.00				

TOTAL WEIGHT = 2 X 145 = 289 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/899 (0.15")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = L/899 (0.28")

CSI: TC=0.58/1.00 (G-H:1), BC=0.49/1.00 (K-L:1), WB=0.59/1.00 (B-Q:1), SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (Q) (INPUT = 0.90)
JSI METAL = 0.89 (M) (INPUT = 1.00)



Structural component only
DWG# T-2007111

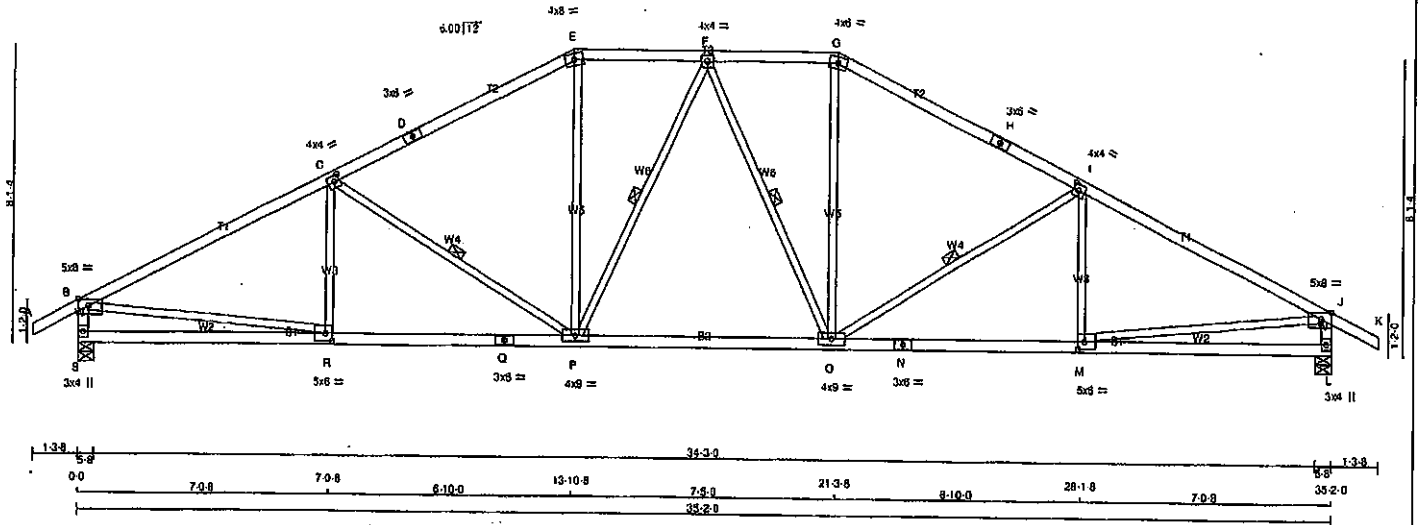
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC	GREEN PARK HOMES	DRWG NO.
408152	T44	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/8" = 1'-0"



TOTAL WEIGHT = 2 X 145 = 291 lb [MRF]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	Edge	3.50
C	TMWW-l	MT20	4.0	4.0	2.00	1.75
D	TS-l	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	8.0		
F	TMWW-l	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	8.0		
H	TS-l	MT20	3.0	6.0		
I	TMWW-l	MT20	4.0	4.0	2.00	1.75
J	TMWW-p	MT20	5.0	8.0	Edge	3.50
L	BMVt-p	MT20	3.0	4.0		
M	BMWW-l	MT20	5.0	8.0	2.50	2.25
N	BS-l	MT20	3.0	6.0		
O	BMWW-l	MT20	4.0	9.0		
P	BMWW-l	MT20	4.0	9.0		
Q	BS-l	MT20	3.0	6.0		
R	BMWW-l	MT20	5.0	8.0	2.50	2.25
S	BMVt-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

<u>BEARINGS</u>		FACTORED		MAXIMUM FACTORED		INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG		
	VERT. HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
S	2063 0	2063 0	0	5-8	5-8	5-8	
L	2063 0	2063 0	0	5-8	5-8	5-8	

<u>UNFACTORED REACTIONS</u>		1ST LCASE		MAX/MIN. COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1457	869 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
L	1457	968 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
L	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-P, Q-O, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	R-C	-182 / 52	0.08 (1)
B-C	-2906 / 0	-91.8	-91.8 0.83 (1)	3.25	C-P	-637 / 0	0.29 (1)
C-D	-2373 / 0	-91.8	-91.8 0.72 (1)	3.66	P-E	0 / 674	0.15 (1)
D-E	-2373 / 0	-91.8	-91.8 0.72 (1)	3.66	P-F	-222 / 0	0.11 (1)
E-F	-2100 / 0	-91.8	-91.8 0.19 (1)	4.53	F-O	-222 / 0	0.11 (1)
F-G	-2100 / 0	-91.8	-91.8 0.19 (1)	4.53	O-G	0 / 674	0.15 (1)
G-H	-2373 / 0	-91.8	-91.8 0.72 (1)	3.66	O-I	-637 / 0	0.29 (1)
H-I	-2373 / 0	-91.8	-91.8 0.72 (1)	3.66	M-I	-182 / 52	0.08 (1)
I-J	-2906 / 0	-91.8	-91.8 0.83 (1)	3.25	B-R	0 / 2649	0.60 (1)
J-K	0 / 28	-91.8	-91.8 0.12 (1)	10.00	M-J	0 / 2649	0.60 (1)
S-B	-2008 / 0	0.0	0.0 0.20 (1)	5.98			
L-J	-2008 / 0	0.0	0.0 0.20 (1)	5.98			
S-R	0 / 0	-18.5	-18.5 0.20 (4)	10.00			
R-Q	0 / 2630	-18.5	-18.5 0.52 (1)	10.00			
Q-P	0 / 2630	-18.5	-18.5 0.52 (1)	10.00			
P-O	0 / 2193	-18.5	-18.5 0.45 (1)	10.00			
O-N	0 / 2630	-18.5	-18.5 0.52 (1)	10.00			
N-M	0 / 2630	-18.5	-18.5 0.52 (1)	10.00			
M-L	0 / 0	-18.5	-18.5 0.20 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17)
CALCULATED VERT. DEFL.(LL) = L/999 (0.14)
ALLOWABLE DEFL.(TL) = L/360 (1.17)
CALCULATED VERT. DEFL.(TL) = L/999 (0.30)

CSI: TC=0.83/1.00 (I-J:1), BC=0.52/1.00 (M-O:1), WB=0.80/1.00 (B-R:1), SI=0.28/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

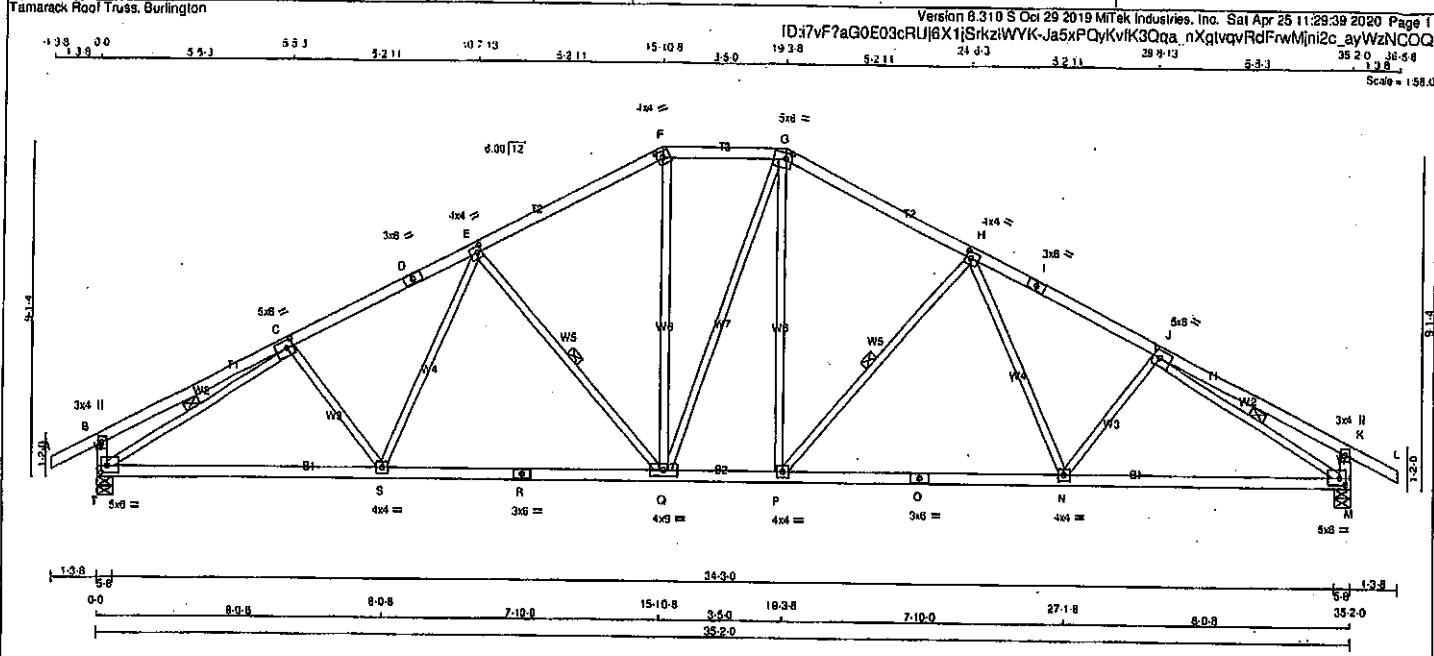
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (M) (INPUT = 0.80)
JSI METAL = 0.79 (N) (INPUT = 1.00)



Structural component only
DWG# T-2007-112

JOB NAME 408152	TRUSS NAME T45	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
C	TMWW-1	MT20	5.0	6.0	2.50	2.25
D	TS-1	MT20	3.0	6.0		
E	TMWW-1	MT20	4.0	4.0	2.00	1.50
F	TTW-h	MT20	4.0	4.0	2.00	1.75
G	TTWW-1	MT20	5.0	6.0	2.00	2.00
H	TMWW-1	MT20	4.0	4.0	2.00	1.50
I	TS-1	MT20	3.0	6.0		
J	TMWW-1	MT20	5.0	6.0	2.50	2.25
K	TMV-p	MT20	3.0	4.0		
M	BMVW-1	MT20	5.0	6.0	2.25	2.00
N, P, S						
N	BMVW-1	MT20	4.0	4.0		
O	BS-1	MT20	3.0	6.0		
Q	BMVWW-1	MT20	4.0	9.0		
R	BS-1	MT20	3.0	6.0		
T	BMVW-1	MT20	5.0	6.0	2.25	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
T	2083	0	0	0
M	2083	0	0	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED
T	1457 969 / 0
M	1457 969 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, H-P, C-T, J-M

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	C-S	-110 / 37	0.04 (1)	
B-C	0 / 19	-91.8 -91.8	0.32 (1)	S-E	0 / 278	0.06 (1)	
C-D	-2759 / 0	-91.8 -91.8	0.40 (1)	E-Q	-981 / 0	0.31 (1)	
D-E	-2759 / 0	-91.8 -91.8	0.40 (1)	Q-F	0 / 813	0.14 (1)	
E-F	-2167 / 0	-91.8 -91.8	0.38 (1)	F-G	0 / 4	0.00 (1)	
F-G	-1925 / 0	-91.8 -91.8	0.18 (1)	G-H	0 / 609	0.14 (1)	
G-H	-2168 / 0	-91.8 -91.8	0.36 (1)	H-I	-682 / 0	0.31 (1)	
H-I	-2760 / 0	-91.8 -91.8	0.40 (1)	I-J	0 / 278	0.06 (1)	
I-J	-2760 / 0	-91.8 -91.8	0.40 (1)	J-K	-110 / 37	0.04 (1)	
J-K	0 / 19	-91.8 -91.8	0.32 (1)	K-L	0 / 28	0.06 (1)	
K-L	0 / 28	-91.8 -91.8	0.12 (1)	L-M	-3040 / 0	0.85 (1)	
L-M	-326 / 0	0.0 0.0	0.03 (1)	M-K	-326 / 0	0.85 (1)	
M-K	-326 / 0	0.0 0.0	0.03 (1)				
T-S	0 / 2536	-18.5 -18.5	0.55 (1)				
S-R	0 / 2362	-18.5 -18.5	0.53 (1)				
R-Q	0 / 2362	-18.5 -18.5	0.53 (1)				
Q-P	0 / 1924	-18.5 -18.5	0.40 (1)				
P-O	0 / 2362	-18.5 -18.5	0.53 (1)				
O-N	0 / 2362	-18.5 -18.5	0.53 (1)				
N-M	0 / 2537	-18.5 -18.5	0.56 (1)				

TOTAL WEIGHT = 2 X 151 = 302 lb

(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(85% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")

CALCULATED VERT. DEFL. (LL) = L/999 (0.15")

ALLOWABLE DEFL. (TL) = L/360 (1.17")

CALCULATED VERT. DEFL. (TL) = L/999 (0.32")

CSI: TC=0.40/1.00 (H-J:1), BC=0.58/1.00 (M-N:1), WB=0.85/1.00 (J-M:1), SS=0.20/1.00 (B-C:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)

MAX MIN MAX MIN MAX MIN

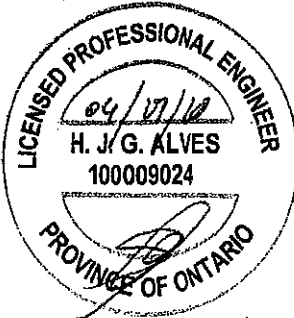
MT20 818 354 1667 788 1887 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (M) (INPUT = 0.90)

JSI METAL = 0.83 (O) (INPUT = 1.00)

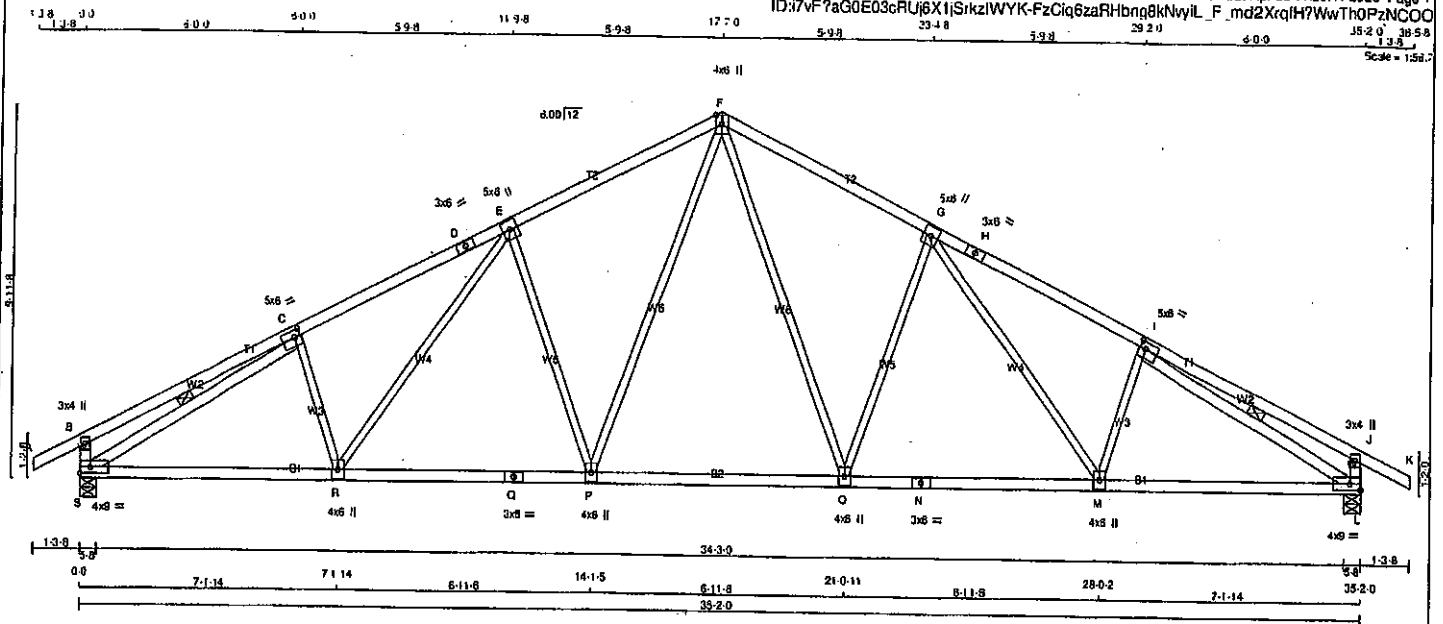


Structural component only
DWG# T-2007113

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408152	T46	6	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Matek Industries, Inc. Sat Apr 25 11:29:41 2020 Page 1
ID:7vF7aG0E03cRUj6X1jSkzIwYK-FzCiq6zaRHbng8kNvyIL_F_md2XrqIH7WwTh0PzNCOO
23-48 29-20 4-0-0
Scale = 1/8" = 1'-0"



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
S - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tab/s in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV+p	MT20	3.0	4.0	
C TMWW-1	MT20	5.0	8.0	225 2.00
D TS-1	MT20	3.0	8.0	
E TMWW-1	MT20	5.0	8.0	Edge
F TTWW+p	MT20	4.0	8.0	
G TMWW-1	MT20	5.0	8.0	
H TS-1	MT20	3.0	8.0	
I TMWW-1	MT20	5.0	8.0	225 2.00
J TMV+p	MT20	3.0	4.0	
L BMVW1-1	MT20	4.0	9.0	Edge
M, O, P, R				
M BMWW-1	MT20	4.0	8.0	
N BS-1	MT20	3.0	8.0	
Q BS-1	MT20	3.0	8.0	
S BMVW1-1	MT20	4.0	9.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	BRG	BRG
S	2063	0	2063	0	0	0	5-8	5-8	5-8
L	2063	0	2063	0	0	0	5-8	5-8	5-8

<u>UNFACTORED REACTIONS</u>							
1ST CASE		<u>MAX./MIN. COMPONENT REACTIONS</u>					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
L	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-S, I-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING		TOTAL LOAD CASES: (4)	
CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 28	F-O	0 / 866
B-C	0 / 22	G-M	0 / 391
C-D	-2808 / 0	M-I	-182 / 18
D-E	-2808 / 0	P-F	0 / 866
E-F	-2322 / 0	E-P	-726 / 0
F-G	-2322 / 0	R-E	0 / 391
G-H	-2808 / 0	C-R	-182 / 18
H-I	-2808 / 0	S-C	-3064 / 0
I-J	0 / 22	I-L	-3064 / 0
J-K	0 / 28		
S-B	-344 / 0		
L-J	-344 / 0		
S-R	0 / 2574		
R-O	0 / 2291		
Q-P	0 / 2291		
P-O	0 / 1762		
O-N	0 / 2291		
N-M	0 / 2291		
M-L	0 / 2574		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/999 (0.14")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = L/999 (0.28")

CSI: TC=0.49/1.00 (C-E:1), BC=0.63/1.00 (L-M:1), WB=0.72/1.00 (E-P:1), SS=0.22/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (ORY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

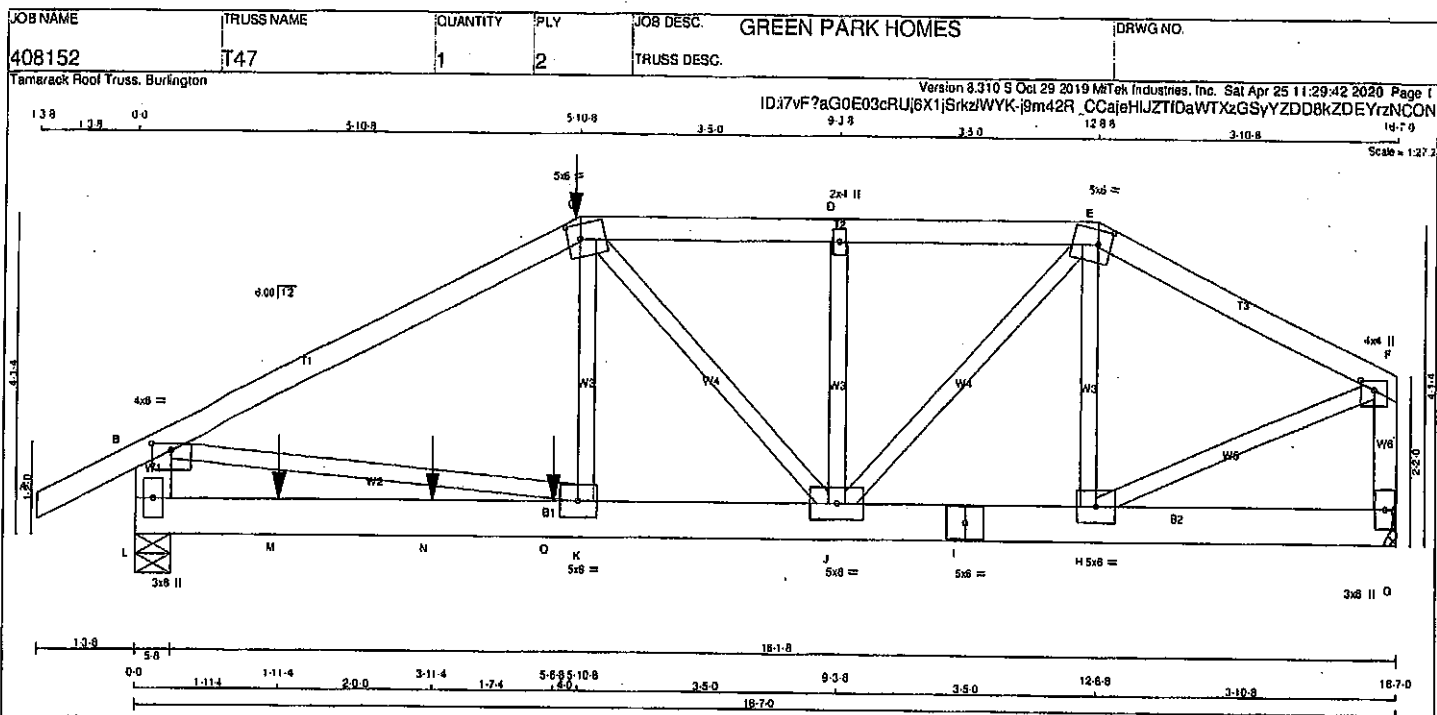
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.87 (C) (INPUT = 0.80)
JSI METAL = 0.77 (I) (INPUT = 1.00)



Structural component only
DWG# T-2007114



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - F	2x4	DRY No.2	SPF
L - B	2x6	DRY No.2	SPF
G - F	2x4	DRY No.2	SPF
L - I	2x6	DRY No.2	SPF
I - G	2x6	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF

EXCEPT

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE (61.0)
C - E	12	SIDE (61.0)
E - F	12	TOP
G - F	12	TOP
L - B	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L - I	12	SIDE (0.0)
I - G	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	IN-SX	IN-SX
L	2005	0	5-8	5-8
G	1397	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 2-0.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT COMBINED	
L	1412 958 / 0
G	985 662 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.18 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.07 (1)	10.00	K-C	0 / 543	
B-C	-2843 / 0	-91.8 -91.8	0.37 (1)	5.18	C-J	-648 / 0	
C-D	-1951 / 0	-91.8 -91.8	0.10 (1)	6.17	J-D	-371 / 0	
D-E	-1951 / 0	-91.8 -91.8	0.10 (1)	6.17	J-E	0 / 1047	
E-F	-1405 / 0	-91.8 -91.8	0.14 (1)	6.25	H-E	-446 / 0	
L-B	-1685 / 0	0.0 0.0	0.07 (1)	7.81	B-K	0 / 2382	
G-F	-1369 / 0	0.0 0.0	0.08 (1)	7.81	H-F	0 / 1375	
L-M	0 / 0	-18.5 -18.5	0.10 (1)	10.00			
M-N	0 / 0	-18.5 -18.5	0.10 (1)	10.00			
N-O	0 / 0	-18.5 -18.5	0.10 (1)	10.00			
O-K	0 / 0	-18.5 -18.5	0.10 (1)	10.00			
K-J	0 / 2389	-18.5 -18.5	0.24 (1)	10.00			
J-I	0 / 1243	-18.5 -18.5	0.11 (1)	10.00			
I-H	0 / 1243	-18.5 -18.5	0.11 (1)	10.00			
H-G	0 / 0	-18.5 -18.5	0.02 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-320	-320	---	FRONT	VERT	TOTAL	---	C1
M	1-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
N	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	5-6-8	-1076	-1076	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 78 = 156 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
BOT CH. LL = 8.0 PSF
DL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.55")
CALCULATED VERT. DEFL. (LL) = L/999 (0.03")
ALLOWABLE DEFL. (TL) = L/360 (0.55")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CS1: TC=0.37/1.00 (B-C:1), BC=0.24/1.00 (J-K:1), WB=0.29/1.00 (B-K:1), SS1=0.37/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 L/S BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSF) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN
MT20 616 354 1607 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.80 (B) (INPUT = 0.90)
JSI METAL = 0.32 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007115 1/1

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408152	T47	1	2	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	8.0	1.00	3.00
C	TTWW-m	MT20	5.0	8.0	2.25	2.00
D	TMVW-w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	2.25	2.00
F	TMVW-p	MT20	4.0	4.0	1.50	2.00
G	BMV1-p	MT20	3.0	8.0		
H	BMWW-1	MT20	5.0	8.0		
I	BS-1	MT20	5.0	8.0		
J	BMWWWW-1	MT20	5.0	8.0		
K	BMWW-1	MT20	5.0	8.0		
L	BMV1-p	MT20	3.0	8.0		

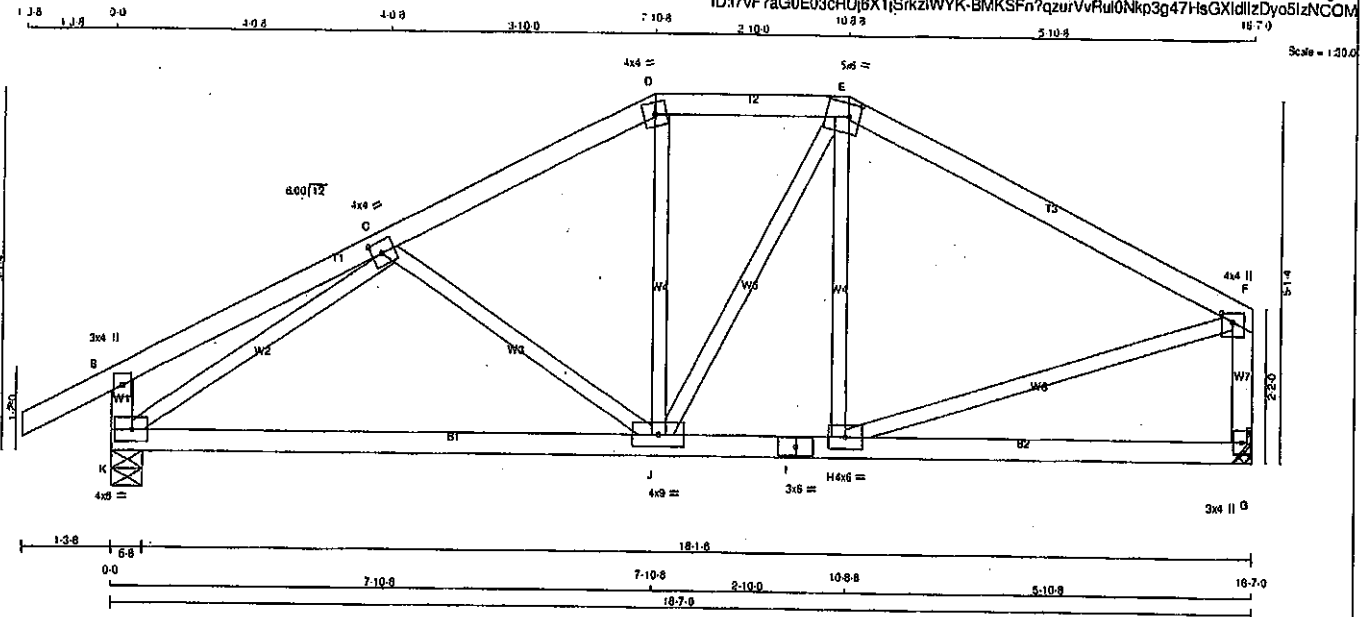


Structural component only
DWG# T-2007115 *ML*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408152	T48	1	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMWW+p	MT20	4.0	4.0	1.50	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UPLIFT	IN-SX
K	1039	0	0	5-8
G	914	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3'-8".

UNFACTORED REACTIONS

1ST LOASE	MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
K	732	494 / 0	0 / 0	0 / 0	0 / 0	238 / 0
G	646	424 / 0	0 / 0	0 / 0	0 / 0	222 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 28	-91.8	-91.8 0.12 (1)	C-J	-232 / 0
B-C	0 / 18	-91.8	-91.8 0.22 (1)	J-D	0 / 134
C-D	-916 / 0	-91.8	-91.8 0.18 (1)	J-E	0 / 152
D-E	-808 / 0	-91.8	-91.8 0.10 (1)	H-E	-163 / 0
E-F	-821 / 0	-91.8	-91.8 0.42 (1)	K-C	-1221 / 0
K-B	-265 / 0	0.0	0.0 0.03 (1)	H-F	0 / 769
G-F	-866 / 0	0.0	0.0 0.10 (1)		
K-J	0 / 993	-18.5	-18.5 0.32 (4)		
J-I	0 / 731	-18.5	-18.5 0.31 (4)		
I-H	0 / 731	-18.5	-18.5 0.31 (4)		
H-G	0 / 0	-18.5	-18.5 0.15 (4)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.13")

CSI: TC=0.42/1.00 (E-F:1), BC=0.32/1.00 (J-K:4), WB=0.47/1.00 (C-K:1), SS=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 795 1667 1658

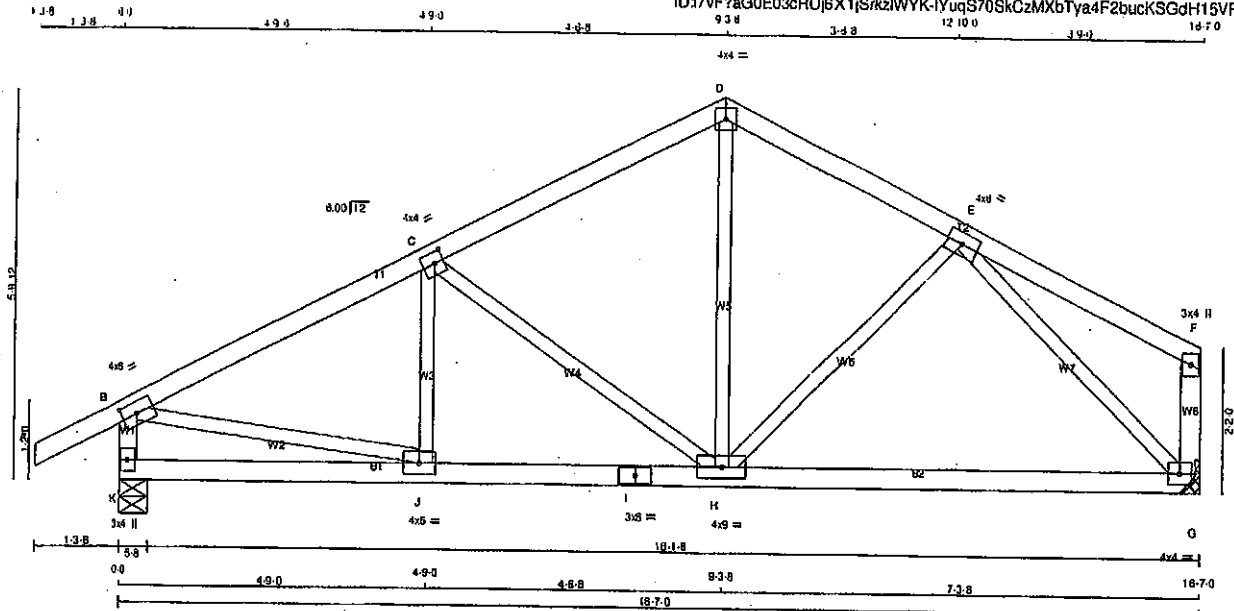
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (C) (INPUT = 0.90)
JSI METAL = 0.41 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007116



Scale: 3/8"=1'

TOTAL WEIGHT = 67 lb (M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
K - F	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	8.0	2.00	3.00
C	TMVW-1	MT20	4.0	4.0	2.00	1.75
D	TTW-p	MT20	4.0	4.0		
E	TMVW-1	MT20	4.0	6.0		
F	TMV-p	MT20	3.0	4.0		
G	BMVW1-1	MT20	4.0	4.0		
H	BMVW1-1	MT20	4.0	9.0		
I	BS-1	MT20	3.0	6.0		
J	BMVW-1	MT20	4.0	6.0		
K	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT		REQD	
	VERY	HORZ	GROSS REACTION	GROSS REACTION	BRG	BRG	IN-SX	IN-SX
K	1039	0	1039	0	0	5-8	5-8	
G	914	0	914	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

1ST LGASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	732	494 / 0	0 / 0	0 / 0	0 / 0	238 / 0	0 / 0
G	646	424 / 0	0 / 0	0 / 0	0 / 0	222 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
FR-TO					FR-TO		
A-B	0 / 28	-91.8	-91.8	0.12 (1)	J-C	-114 / 23	0.02 (1)
B-C	-1135 / 0	-91.8	-91.8	0.28 (1)	C-H	-418 / 0	0.22 (1)
C-D	-798 / 0	-91.8	-91.8	0.25 (1)	H-D	0 / 407	0.09 (1)
D-E	-793 / 0	-91.8	-91.8	0.15 (1)	H-E	-39 / 37	0.02 (1)
E-F	0 / 17	-91.8	-91.8	0.19 (1)	B-J	0 - 1051	0.24 (1)
K-B	-998 / 0	0.0	0.0	0.10 (1)	E-G	-1026 / 0	0.44 (1)
G-F	-129 / 0	0.0	0.0	0.02 (1)			
K-J	0 / 0	-18.5	-18.5	0.10 (4)			
J-I	0 / 1034	-18.5	-18.5	0.29 (4)			
I-H	0 / 1034	-18.5	-18.5	0.29 (4)			
H-G	0 / 722	-18.5	-18.5	0.25 (4)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ORC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.28/1.00 (B-C:1), BC=0.29/1.00 (H-J:4), WB=0.44/1.00 (E-G:1), SS=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

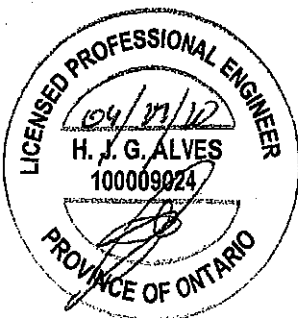
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1668

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (G) (INPUT = 0.90)
JSI METAL = 0.38 (I) (INPUT = 1.00)

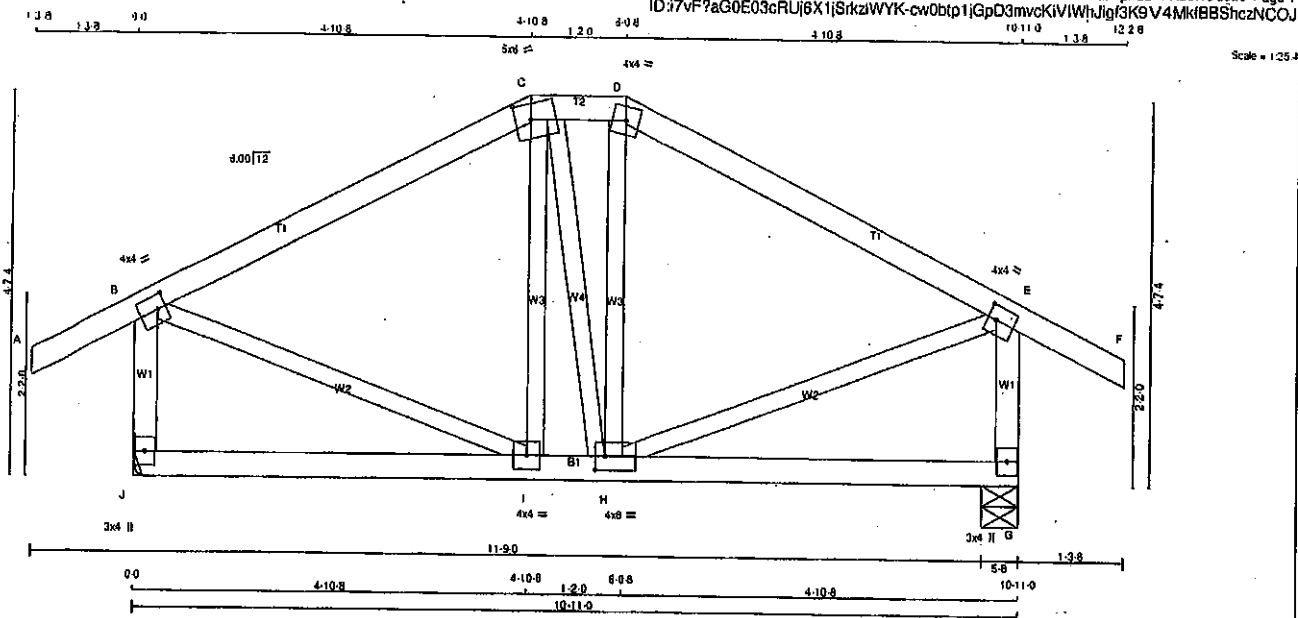


Structural component only
DWG# T-2007117

 Structural component only DWG# T-2007118	<u>CONNECTION REQUIREMENTS</u> 1) G1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.	
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408152	T51	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
A - C	2x4	DRY	No.2		SPF
C - D	2x4	DRY	No.2		SPF
D - F	2x4	DRY	No.2		SPF
J - B	2x4	DRY	No.2		SPF
G - E	2x4	DRY	No.2		SPF
J - G	2x4	DRY	No.2		SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (tables in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	4.0	2.00	1.25
C	TTWV-m	MT20	5.0	6.0	2.25	2.25
D	TTW-m	MT20	4.0	4.0		
E	TMVW-1	MT20	4.0	4.0	2.00	1.25
G	BMV1+P	MT20	3.0	4.0		
H	BMVWW-1	MT20	4.0	6.0	2.00	1.50
I	BMVWW-1	MT20	4.0	4.0		
J	BMV1+P	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
J	726	0	0	MECHANICAL
G	726	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED							
J	511	349 / 0	0 / 0	0 / 0	0 / 0	0 / 0	163 / 0	0 / 0
G	511	349 / 0	0 / 0	0 / 0	0 / 0	0 / 0	163 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED MAX. CS (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX. CS (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	I-C	-78 / 18
B-C	-435 / 0	-91.8	-91.8	0.28 (1)	8.25	C-H	-4 / 0
C-D	-386 / 0	-91.8	-91.8	0.02 (1)	6.25	H-D	-82 / 19
D-E	-434 / 0	-91.8	-91.8	0.28 (1)	8.25	B-I	0 / 415
E-F	0 / 28	-91.8	-91.8	0.12 (1)	10.00	H-E	0 / 414
J-B	-688 / 0	0.0	0.0	0.08 (1)	7.81		
G-E	-688 / 0	0.0	0.0	0.08 (1)	7.81		
J-I	0 / 0	-18.5	-18.5	0.10 (4)	10.00		
I-H	0 / 387	-18.5	-18.5	0.14 (4)	10.00		
H-G	0 / 0	-18.5	-18.5	0.10 (4)	10.00		

TOTAL WEIGHT = 52 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(56 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.28/1.00 (B-C:1), BC=0.14/1.00 (H-I:4), WB=0.09/1.00 (B-I:1), SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

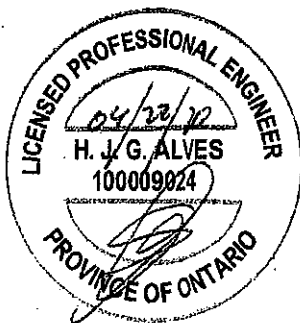
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (B) (INPUT = 0.80)
JSI METAL = 0.22 (B) (INPUT = 1.00)

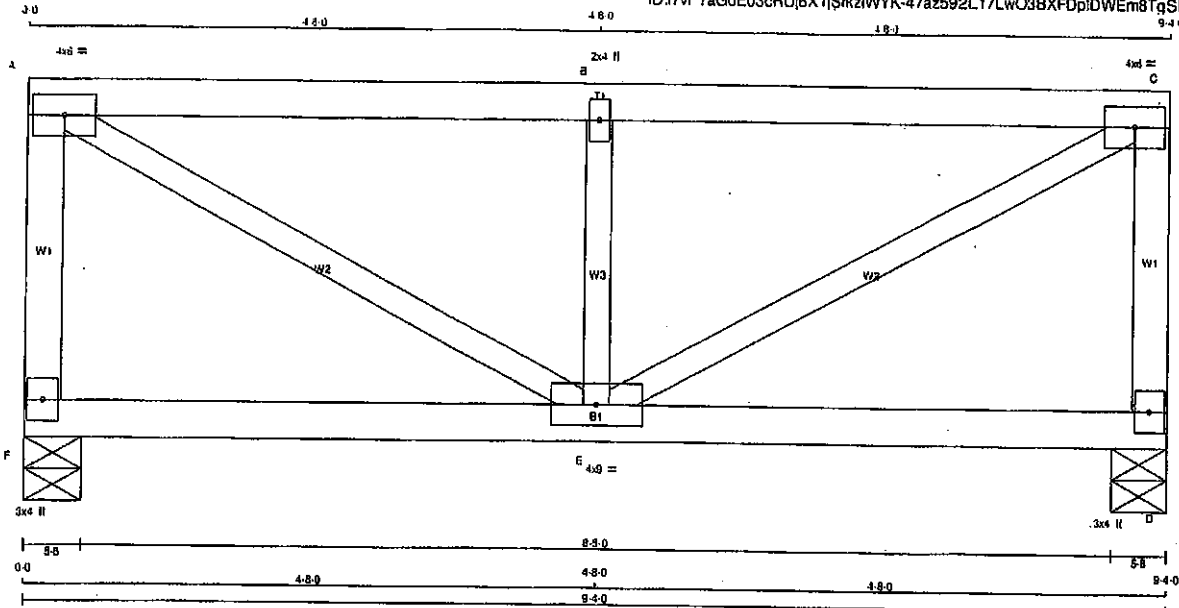


Structural component only
DWG# T-2007119

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408152	T52	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/16" = 1'-0"

TOTAL WEIGHT = 36 lb (M/R)

LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER		SPF	
F - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS 2x3 DRY				SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	4.0	6.0	
B	TMVW-w	MT20	2.0	4.0	
C	TMVW-l	MT20	4.0	6.0	
D	BMV-l-p	MT20	3.0	4.0	
E	BMVWW-t	MT20	4.0	6.0	
F	BMV-l-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
F	620	0	820	0	0	5-8	5-8		
D	620	0	820	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST CASE		MAX / MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	448	239 / 0	0 / 0	0 / 0	0 / 0	209 / 0	0 / 0
D	448	239 / 0	0 / 0	0 / 0	0 / 0	209 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.52 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FROM TO				FR-TO			
F-A	-583 / 0	0.0	0.0 0.11 (1)	7.81	A-E	0 / 792	0.24 (1)
A-B	-895 / 0	-114.3	-114.3 0.55 (1)	5.52	E-B	-858 / 0	0.15 (1)
B-C	-895 / 0	-114.3	-114.3 0.55 (1)	5.52	E-C	0 / 792	0.24 (1)
D-C	-583 / 0	0.0	0.0 0.11 (1)	7.81			
F-E	0 / 0	-18.5	-18.5 0.13 (4)	10.00			
E-D	0 / 0	-18.5	-18.5 0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
OL = 15.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 48.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 8 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/380 (0.31")
CALCULATED VERT. DEF. (LL) = L/999 (0.01")
ALLOWABLE DEF. (TL) = L/380 (0.31")
CALCULATED VERT. DEF. (TL) = L/999 (0.03")

CSI: TC=0.55/1.00 (B-C:1), BC=0.13/1.00 (E-F:4), WB=0.24/1.00 (A-E:1), SI=0.34/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

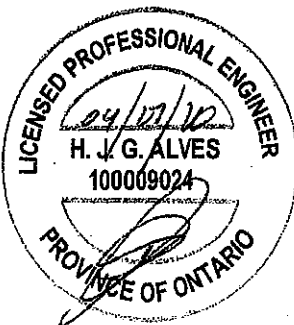
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MIN	MAX	MIN
MT20	616	354
	1667	788
	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

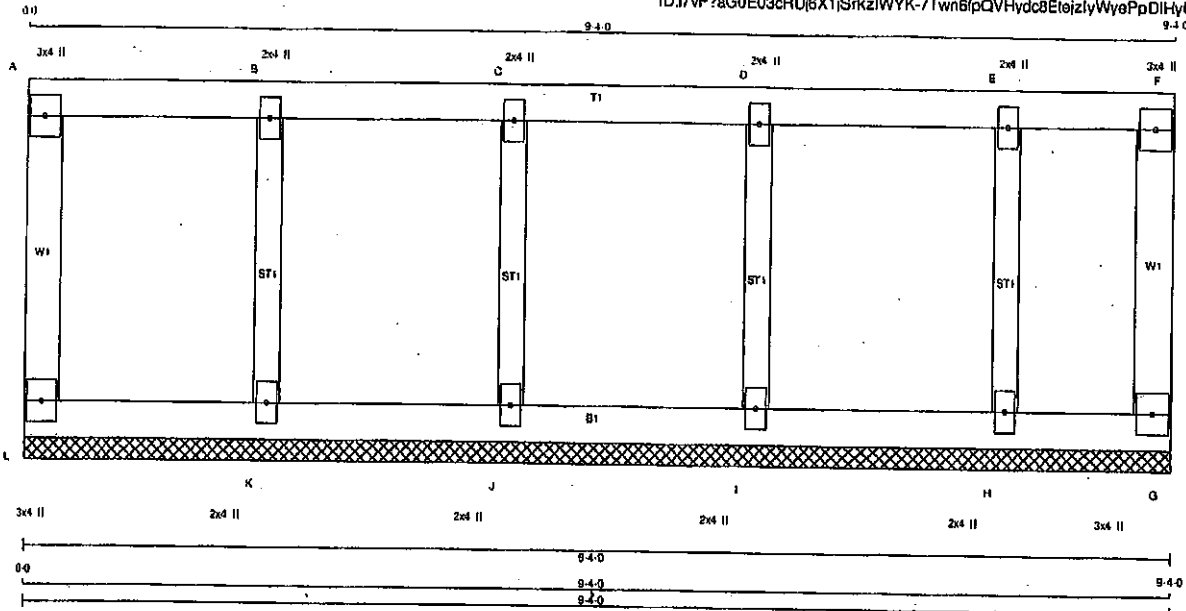
JSI GRIP=0.81 (A) (INPUT = 0.90)
JSI METAL=0.24 (A) (INPUT = 1.00)



Structural component only
DWG# T-2007120

JOB NAME 408152	TRUSS NAME G52	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER			
CHORDS					
L - A	2x4	DRY	No.2	SPF	
A - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
L - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					
GABLE STUDS SPACED AT 2'-0" O.C.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING
BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM TO			FR-TO		
L-A	-101 / 0	0.0	0.0	0.03 (1)	7.81	K-B	-245 / 0
A-B	-8 / 0	-114.3	-114.3	0.08 (1)	10.00	J-C	-224 / 0
B-C	-8 / 0	-114.3	-114.3	0.08 (1)	10.00	I-D	-237 / 0
C-D	-8 / 0	-114.3	-114.3	0.07 (1)	10.00	H-E	-196 / 0
D-E	-8 / 0	-114.3	-114.3	0.07 (1)	10.00		
E-F	-8 / 0	-114.3	-114.3	0.05 (1)	10.00		
G-F	-84 / 0	0.0	0.0	0.02 (1)	7.81		
L-K	0 / 8	-18.5	-18.5	0.02 (4)	10.00		
K-J	0 / 8	-18.5	-18.5	0.02 (4)	10.00		
J-I	0 / 8	-18.5	-18.5	0.02 (4)	10.00		
I-H	0 / 8	-18.5	-18.5	0.02 (4)	10.00		
H-G	0 / 8	-18.5	-18.5	0.02 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 15.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 48.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (A-B:1), BC=0.02/1.00 (K-L:4), WB=0.08/1.00 (B-K:1), SL=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1659

PLATE PLACEMENT TOL. = 0.250 inches

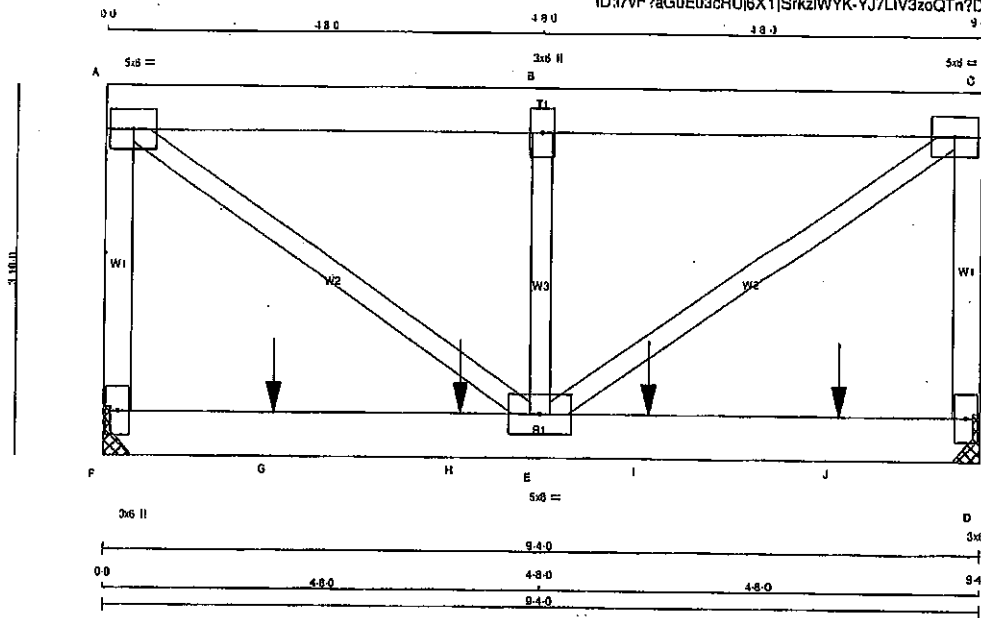
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.17 (K) (INPUT = 0.90)
JSI METAL = 0.07 (K) (INPUT = 1.00)



Structural component only
DWG# T-2007104

JOB NAME 408152	TRUSS NAME T53	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.310 S Oct 29 2019 MITek Industries, Inc. Sat Apr 25 11:29:48 2020 Page 1			
		ID: i7vF7aG0E03cRUj6X1jSrkzWYK-YJ7LIV3zoQTn?DmKpwK_mkn0k?2zwn17VgZmVzNCOH			



TOTAL WEIGHT = 51 lb
(M/R)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	REQRD
F	988	0	988	0	0	MECHANICAL		
D	1012	0	1012	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 3-8, JOINT D = 3-8.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	15.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	48.0	PSF	

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.04")

CSI: TC=0.24/1.00 (B-C:1), BC=0.24/1.00 (D-E:1), WB=0.35/1.00 (A-E:1), SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1697 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (E) (INPUT = 0.90)
JSI METAL= 0.32 (E) (INPUT = 1.00)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW-1	MT20	5.0	6.0		
B	TMW-w	MT20	3.0	6.0		
C	TMW-1	MT20	5.0	6.0		
D	BMV1+p	MT20	3.0	6.0		
E	BMWVW-1	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

UNFACTORED REACTIONS

1ST CASE MAX/MIN COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	705	427/0	0/0	0/0	0/0	279/0	0/0
D	722	439/0	0/0	0/0	0/0	283/0	0/0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
F-A	-846/0	0.0	0.0 0.23 (1)	A-E	0/1072	0.35 (1)	
A-B	-869/0	-114.3	-114.3 0.24 (1)	E-B	-832/0	0.18 (1)	
B-C	-869/0	-114.3	-114.3 0.24 (1)	E-C	0/1072	0.35 (1)	
D-C	-846/0	0.0	0.0 0.23 (1)				
F-G	0/0	-18.5	-18.5 0.24 (1)				10.00
G-H	0/0	-18.5	-18.5 0.24 (1)				10.00
H-E	0/0	-18.5	-18.5 0.24 (1)				10.00
E-I	0/0	-18.5	-18.5 0.24 (1)				10.00
I-J	0/0	-18.5	-18.5 0.24 (1)				10.00
J-D	0/0	-18.5	-18.5 0.24 (1)				10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-9-12	-190	-190	---	FRONT	VERT	TOTAL	---	C1
H	3-9-12	-190	-190	---	FRONT	VERT	TOTAL	---	C1
I	5-9-12	-190	-190	---	FRONT	VERT	TOTAL	---	C1
J	7-9-12	-190	-190	---	FRONT	VERT	TOTAL	---	C1

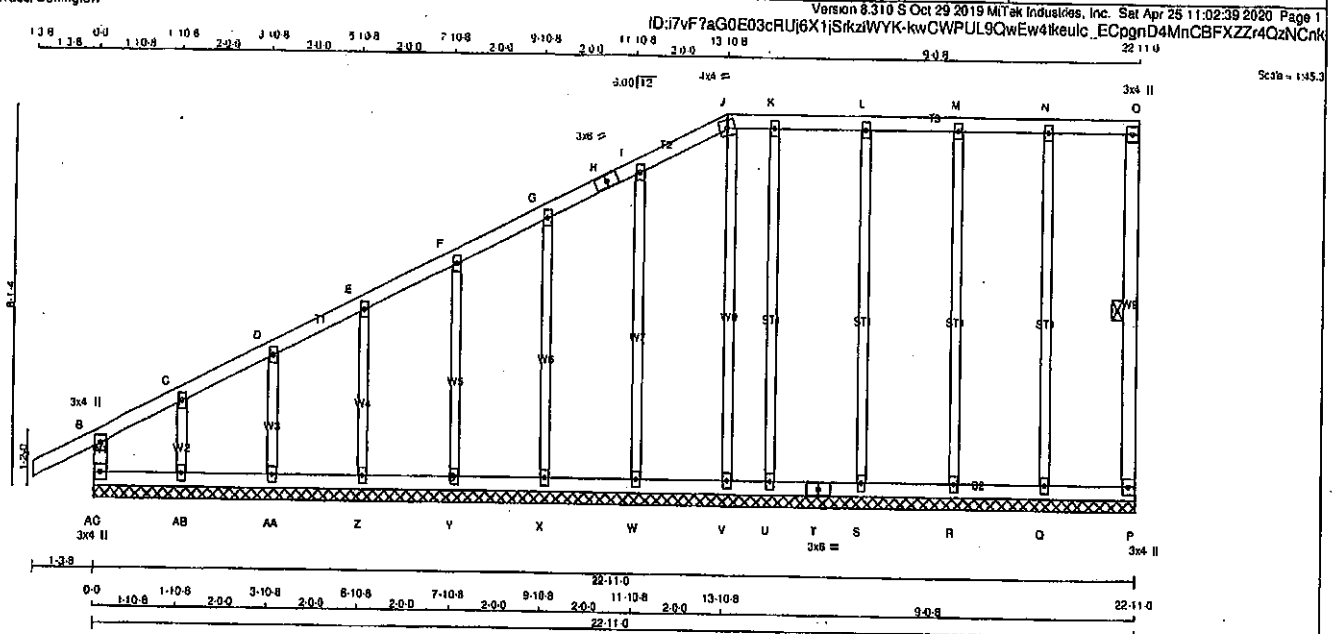
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007121

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.70 (A) (INPUT = 1.00)



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
AC - B	2x4	DRY	No.2
A - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - O	2x4	DRY	No.2
P - O	2x4	DRY	No.2
AC - T	2x4	DRY	No.2
T - P	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

ALL GABLE WEBS 2x3 DRY No.2

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C, D, E, F, G, I, K, L, M, N					
C	TMW+u	MT20	2.0	4.0	
H	TS-l	MT20	3.0	6.0	
J	TTW-m	MT20	4.0	4.0	
O	TMV+p	MT20	3.0	4.0	
P	BMV1+p	MT20	3.0	4.0	
Q, R, S, U, V, W, X, Y, Z, AA, AB					
Q	BMW1+u	MT20	2.0	4.0	
T	BS-l	MT20	3.0	6.0	
AC	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF O-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. UNBRACED LENGTH	FR-TO	WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)			MEMB.	MAX. FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO						
AC-B	-224/0	0.0	0.0	0.02 (1)	7.81	Q-N	-195/0	0.25 (1)	
A-B	0/28	-91.8	-91.8	0.12 (1)	10.00	R-M	-183/0	0.23 (1)	
B-C	-26/0	-91.8	-91.8	0.04 (1)	8.25	S-L	-188/0	0.24 (1)	
C-D	-19/0	-91.8	-91.8	0.04 (1)	8.25	U-K	-136/0	0.17 (1)	
D-E	-15/0	-91.8	-91.8	0.04 (1)	8.25	V-J	-131/0	0.17 (1)	
E-F	-11/0	-91.8	-91.8	0.04 (1)	8.25	W-I	-185/0	0.17 (1)	
F-G	-8/0	-91.8	-91.8	0.04 (1)	10.00	X-G	-183/0	0.11 (1)	
G-H	-6/0	-91.8	-91.8	0.04 (1)	10.00	Y-F	-182/0	0.07 (1)	
H-I	-6/0	-91.8	-91.8	0.04 (1)	10.00	Z-E	-182/0	0.05 (1)	
I-J	-8/0	-91.8	-91.8	0.04 (1)	10.00	AA-D	-183/0	0.03 (1)	
J-K	-2/0	-91.8	-91.8	0.03 (1)	10.00	AB-C	-176/0	0.03 (1)	
K-L	-2/0	-91.8	-91.8	0.04 (1)	10.00				
L-M	-2/0	-91.8	-91.8	0.04 (1)	10.00				
M-N	-2/0	-91.8	-91.8	0.04 (1)	10.00				
N-O	-2/0	-91.8	-91.8	0.04 (1)	10.00				
O-P	-81/0	0.0	0.0	0.01 (1)	8.25				
AC-AB	0/24	-18.5	-18.5	0.02 (4)	10.00				
AB-AA	0/17	-18.5	-18.5	0.02 (4)	10.00				
AA-Z	0/13	-18.5	-18.5	0.02 (4)	10.00				
Z-Y	0/10	-18.5	-18.5	0.02 (4)	10.00				
Y-X	0/7	-18.5	-18.5	0.01 (4)	10.00				
X-W	0/5	-18.5	-18.5	0.02 (4)	10.00				
W-V	0/4	-18.5	-18.5	0.02 (4)	10.00				
V-U	0/2	-18.5	-18.5	0.01 (4)	10.00				
U-T	0/2	-18.5	-18.5	0.02 (4)	10.00				
T-S	0/2	-18.5	-18.5	0.02 (4)	10.00				
S-R	0/2	-18.5	-18.5	0.02 (4)	10.00				
R-Q	0/2	-18.5	-18.5	0.02 (4)	10.00				
Q-P	0/2	-18.5	-18.5	0.02 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (Q-R:4), WB=0.25/1.00 (N-Q:1), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI)

MAX MIN MAX MIN MAX MIN

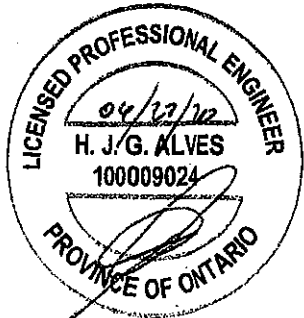
MT20 618 354 1667 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.80 (J) (INPUT = 0.90)

JSI METAL = 0.08 (I) (INPUT = 1.00)



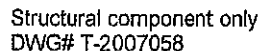


Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

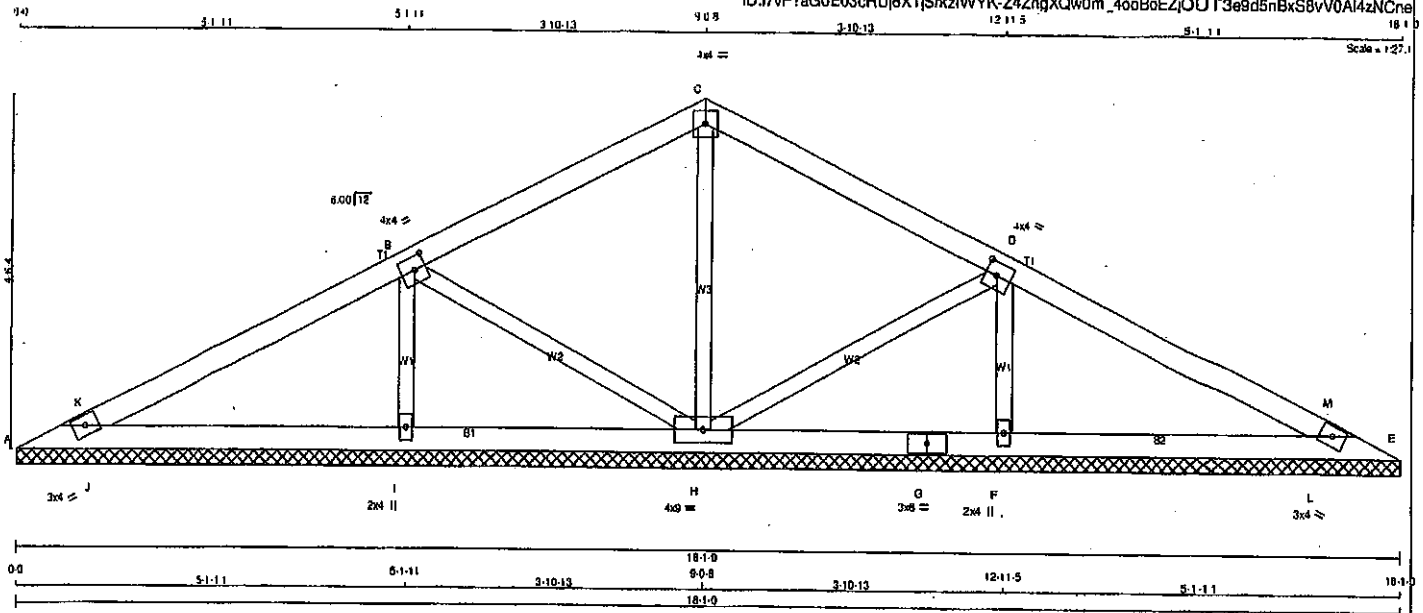
CHORDS				WEBB			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LCI (LC)	MAX. UNBRAC (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LCI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-J	0/240	-91.8	-91.8	0.12 (1)	10.00	C-F	-428 / 0
J-B	0/301	-91.8	-91.8	0.34 (1)	10.00	F-D	-436 / 0
B-C	0/503	-91.8	-91.8	0.37 (1)	10.00	H-C	-428 / 0
C-D	0/503	-91.8	-91.8	0.37 (1)	10.00	B-H	-436 / 0
D-L	0/301	-91.8	-91.8	0.34 (1)	10.00	I-J	-78 / 35
L-E	0/240	-91.8	-91.8	0.12 (1)	10.00	K-L	-76 / 35
							0.00 (1)
A-I	-255 / 0	-18.5	-18.5	0.11 (1)	6.25		
I-H	-243 / 0	-18.5	-18.5	0.13 (4)	8.25		
H-G	-246 / 0	-18.5	-18.5	0.13 (4)	8.25		
G-F	-246 / 0	-18.5	-18.5	0.13 (4)	6.25		
F-K	-243 / 0	-18.5	-18.5	0.13 (4)	6.25		
K-E	-255 / 0	-18.5	-18.5	0.11 (1)	8.25		

JSI GRIP= 0.48 (H) (INPUT = 0.90)
JSI METAL= 0.15 (C) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408150	P1	17	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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ID:7vF7aG0E03cRUj8X1jSkzIWK-Z4ZngXQw0m_400BoEZ/OUT3e9d5nBx58vV0AI4zNChe



TOTAL WEIGHT = 17 X 55 = 935 LB

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
A - G	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
A	124	0	124	0	18-1-0 (11-114)0	0
E	124	0	124	0	18-1-0 (11-114)0	0
H	415	0	415	0	18-1-0 (11-114)0	0
F	666	0	666	0	18-1-0 (11-114)0	0
I	666	0	666	0	18-1-0 (11-114)0	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
A	87	59 / 0	0 / 0	0 / 0	28 / 0	0 / 0	0 / 0
E	87	59 / 0	0 / 0	0 / 0	28 / 0	0 / 0	0 / 0
H	293	192 / 0	0 / 0	0 / 0	101 / 0	0 / 0	0 / 0
F	471	307 / 0	0 / 0	0 / 0	164 / 0	0 / 0	0 / 0
I	471	307 / 0	0 / 0	0 / 0	164 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, H, F, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-K	0 / 248	-91.8 -91.8	0.07 (1)	H-C	-445 / 0	0.13 (1)	
K-B	0 / 292	-91.8 -91.8	0.32 (1)	H-D	0 / 59	0.01 (1)	
B-C	0 / 188	-91.8 -91.8	0.30 (1)	F-D	-528 / 0	0.08 (1)	
C-D	0 / 188	-91.8 -91.8	0.30 (1)	B-H	0 / 59	0.01 (1)	
D-M	0 / 292	-91.8 -91.8	0.32 (1)	I-B	-528 / 0	0.08 (1)	
M-E	0 / 248	-91.8 -91.8	0.07 (1)	J-K	-128 / 8	0.00 (1)	
				L-M	-128 / 8	0.00 (1)	
A-J	-256 / 0	-18.5 -18.5	0.14 (1)				6.25
J-I	-240 / 0	-18.5 -18.5	0.14 (1)				6.25
I-H	-240 / 0	-18.5 -18.5	0.11 (1)				6.25
H-G	-240 / 0	-18.5 -18.5	0.11 (1)				6.25
G-F	-240 / 0	-18.5 -18.5	0.11 (1)				6.25
F-L	-240 / 0	-18.5 -18.5	0.14 (1)				6.25
L-E	-256 / 0	-18.5 -18.5	0.14 (1)				6.25

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012, ABC 2018
- PART 9 OF NBC 2012 (2018 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (D-M:1), BC=0.14/1.00 (F-L:1),
WB=0.13/1.00 (C-H:1), SS=0.17/1.00 (D-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

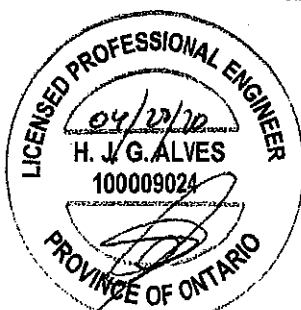
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

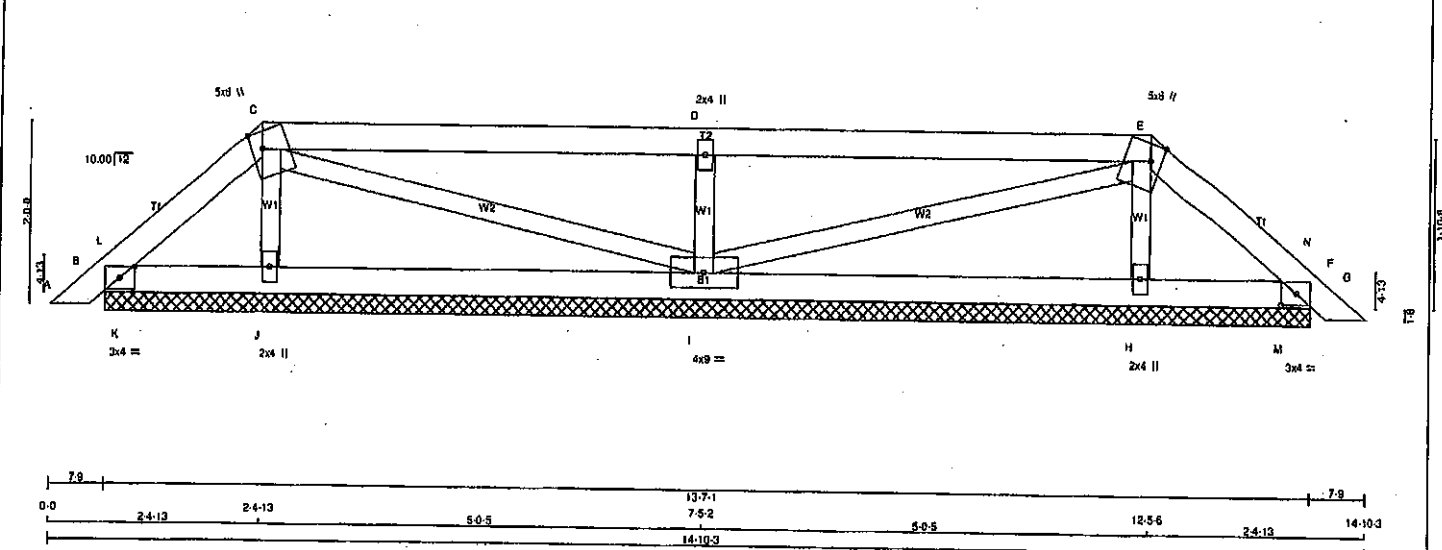
JSI GRIP= 0.51 (B) (INPUT = 0.90)
JSI METAL= 0.13 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007062

JOB NAME 408151	TRUSS NAME PB20	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:h2dfqC44wS_E2l2qPs_atzllRw-HkWF6Hjv7E7m14RFJfStCxbPKbCR_v_qmDlUdZNCXl



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				TOTAL WEIGHT = 2 X 45 = 90 lb	
N. L. G. A. RULES				BEARINGS					
CHORDS	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT	REQD		
A - C	2x4	DRY	No.2	GROSS REACTION	GROSS REACTION	BRG	BRG	DESIGN CRITERIA	
C - E	2x4	DRY	No.2	VERT	DOWN	UPLIFT	IN-SX	SPECIFIED LOADS:	
E - G	2x4	DRY	No.2	HORZ			IN-SX	TOP CH. LL = 25.6 PSF	
B - F	2x4	DRY	No.2					DL = 8.0 PSF	
ALL WEBS 2x3 DRY No.2 SPF				JT				BOT CH. LL = 0.0 PSF	
DRY: SEASONED LUMBER.				B	169	0	13-7-1	DL = 7.4 PSF	
				F	169	0	13-7-1	TOTAL LOAD = 39.0 PSF	
				J	288	0	13-7-1	SPACING = 24.0 IN. C/C	
				I	883	0	13-7-1		
				H	288	0	13-7-1		

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	1.50 2.00
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TTWW+w	MT20	2.0	4.0	
E	TTWW+m	MT20	5.0	6.0	2.25 1.50
F	TMB1-I	MT20	3.0	4.0	1.50 2.00
H	BMW1+w	MT20	2.0	4.0	
I	BMWW1-I	MT20	4.0	9.0	
J	BMW1+w	MT20	2.0	4.0	

UNFACTORED REACTIONS							
JT	1ST CASE	MAX	MIN	COMPONENT	REACTIONS		
B	116	93/0	0/0	PERM	LIVE	WIND	DEAD
F	116	93/0	0/0	PERM	LIVE	WIND	DEAD
J	207	118/0	0/0	PERM	LIVE	WIND	DEAD
I	481	327/0	0/0	PERM	LIVE	WIND	DEAD
H	207	118/0	0/0	PERM	LIVE	WIND	DEAD

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING							
TOTAL LOAD CASES: (4)							
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0/14	-91.8 -91.8	0.02 (1)	J-C	-194/0	0.03 (1)	
B-L	-31/4	-91.8 -91.8	0.01 (1)	C-I	-22/0	0.01 (1)	
L-C	-80/0	-91.8 -91.8	0.03 (1)	I-D	-576/0	0.08 (1)	
C-D	-8/0	-91.8 -91.8	0.39 (1)	D-E	-22/0	0.01 (1)	
D-E	-8/0	-91.8 -91.8	0.39 (1)	E-N	-194/0	0.03 (1)	
E-N	-80/0	-91.8 -91.8	0.03 (1)	N-F	-121/0	0.00 (1)	
N-F	-31/4	-91.8 -91.8	0.01 (1)	F-G	-121/0	0.00 (1)	
F-G	0/14	-91.8 -91.8	0.02 (1)				
B-K	0/42	-18.5 -18.5	0.04 (1)				
K-J	0/42	-18.5 -18.5	0.07 (4)				
J-I	0/29	-18.5 -18.5	0.10 (4)				
I-H	0/29	-18.5 -18.5	0.10 (4)				
H-M	0/42	-18.5 -18.5	0.07 (4)				
M-F	0/42	-18.5 -18.5	0.04 (1)				

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCG 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIG 2011, TPIG 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.39/1.00 (C-D:1), BC=0.10/1.00 (I-J:4), WB=0.08/1.00 (D-I:1), SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

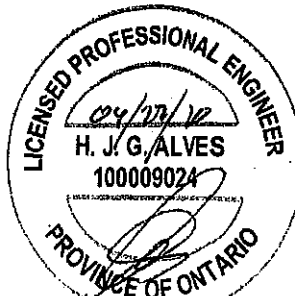
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.30 (D) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)

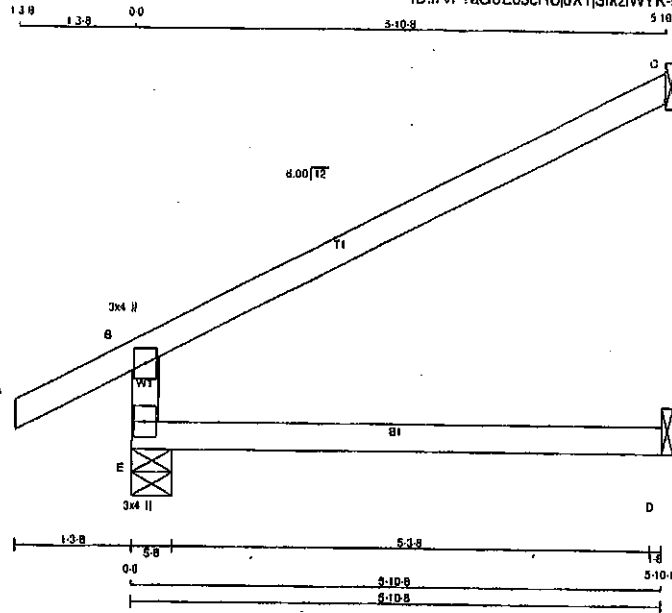


Structural component only
DWG# T-2007080

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408150	U1	21	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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Scale = 1/2" = 1'-0"

LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN
B	TMV+p	MT20	3.0	4.0
E	BMV+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UP/LIFT		
E	525	0	525	0	5-8	5-8
C	202	0	202	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	389	257/0	0/0	0/0	0/0	111/0	0/0
C	139	113/0	0/0	0/0	0/0	26/0	0/0
D	38	0/0	0/0	0/0	0/0	36/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD			MAX. UNBRACED LENGTH	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)
		FROM	TO	CSI (LC)			
FR-TO							
E-B	-481/0	0.0	0.0	0.13 (4)	7.81		
A-B	0/28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-30/0	-91.8	-91.8	0.54 (1)	6.25		
E-D	0/0	-18.5	-18.5	0.13 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
	DL = 8.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CSI: TC=0.54/1.00 (B-C:1), EC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

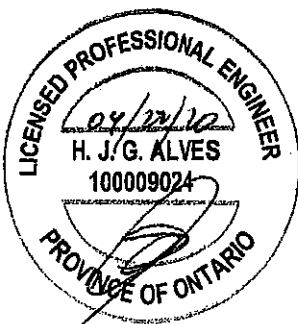
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.19 (E) (INPUT = 0.90)
JSI METAL = 0.13 (B) (INPUT = 1.00)

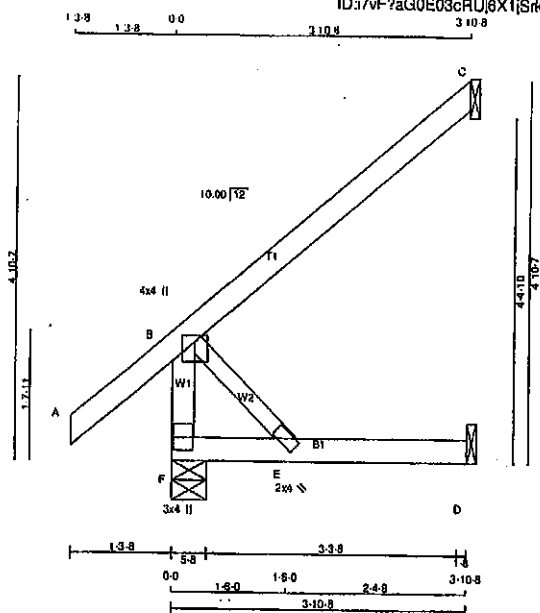


Structural component only
DWG# T-2007059

JOB NAME 408150	TRUSS NAME J2	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/27.0

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMVW+p	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	341	0	341	0	5-8	5-8
C	178	0	178	0	1-8	1-8
D	38	0	40	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
1ST CASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	239	170/0	0/0	0/0	0/0	89/0	0/0
C	122	99/0	0/0	0/0	0/0	23/0	0/0
D	29	0/0	0/0	0/0	0/0	29/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1(LC)	MAX. UNBRAC LENGTH FR-TO	MEMB. FORCE (LBS)	MAX. FACTORED CS1(LC)
F-B	-305/0	0.0	0.0	0.03 (1)	7.81	
A-B	0/41	-91.8	-91.8	0.14 (5)	10.00	
B-C	0/0	-91.8	-91.8	0.23 (1)	10.00	
F-E	0/0	-18.5	-18.5	0.08 (4)	10.00	
E-D	0/0	-18.5	-18.5	0.08 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 15 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.23/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SS1=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(ORY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

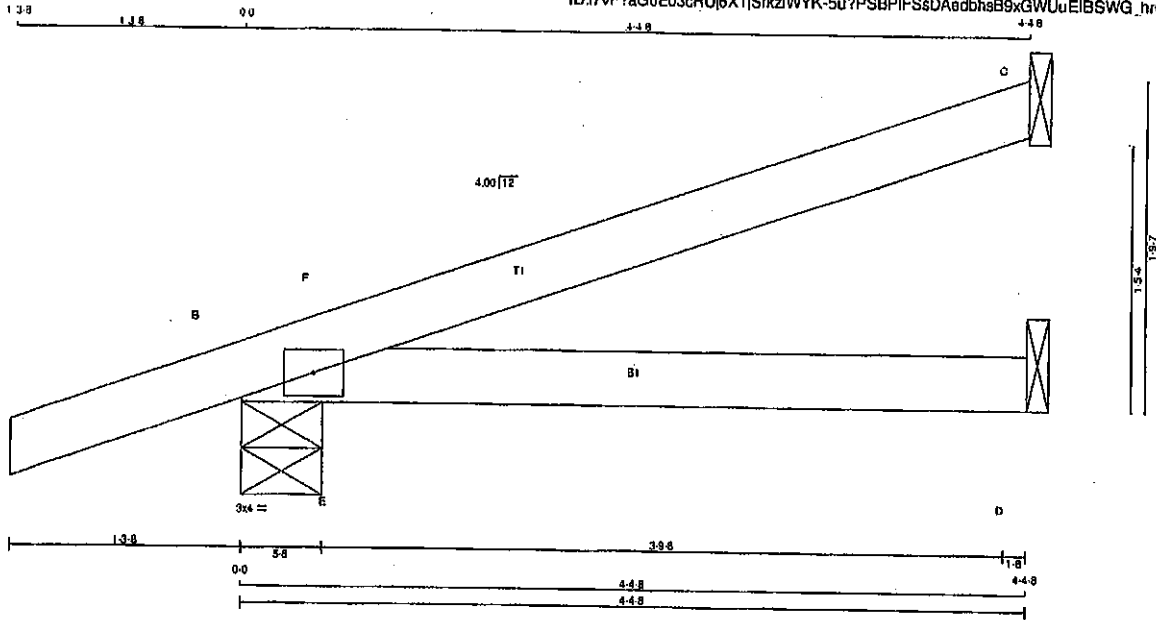
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.24 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007060

JOB NAME 408150	TRUSS NAME U3	QUANTITY 6	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 8 Oct 2019 Mitek Industries, Inc. Sat Apr 25 11:02:44 2020 Page 1 ID:17vF7aG0E03cRUj6X1jSrkzjWYK-5u?PSBPIFSsDAedbsB9xGWuEIBSWG_hrGcmdzNCnf	



TOTAL WEIGHT = 6 X 12 = 71 lb

LUMBER
N. L. G. A. RULES
CHORDS SIZE DRY LUMBER
A - C 2x4 DRY No.2
B - D 2x4 DRY No.2
DESCR. SPF
SPF

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TM814 MT20 3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		RECORD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX
C	174	0	174	0	0	1-8	1-8	1-8
B	384	0	384	0	0	5-8	5-8	5-8
D	68	0	68	0	0	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B97781H FOR CONNECTION TO JOINT(S) C, D

JT	UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS				
	1ST CASE	2ND CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD
C	120	93/0	0/0	0/0	0/0	0/0	27/0
B	255	180/0	0/0	0/0	0/0	0/0	75/0
D	50	19/0	0/0	0/0	0/0	0/0	32/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (LBS)	MAX. (PLF)	MAX. (LBS)	MAX. MEMB. FORCE (LBS)	MAX. (PLF)	MAX. (LBS)	MAX. (LBS)
A-B	0/18	-91.8	-91.8	0.11 (1)	10.00	E-F	-184/7	0.00 (1)
B-F	-14/0	-91.8	-91.8	0.05 (4)	6.25			
F-C	0/2	-91.8	-91.8	0.22 (1)	10.00			
B-E	0/0	-18.5	-18.5	0.17 (1)	10.00			
E-D	0/0	-18.5	-18.5	0.17 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.22/1.00 (C-F:1), BC=0.17/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.10/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 516 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 6.0 Deg.

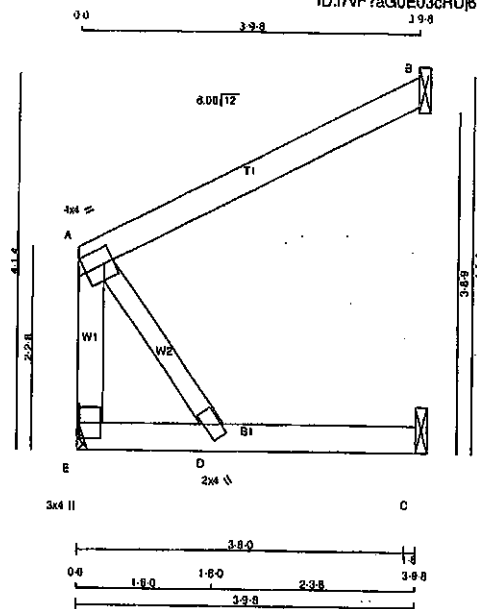
JSI GRIP= 0.24 (8) (INPUT = 0.90)
JSI METAL= 0.07 (8) (INPUT = 1.00)



Structural component only
DWG# T-2007061

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408152	J40	4	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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Scale = 1/2" = 1'-0"

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
E - A	2x4 DRY	No.2	SPF
A - B	2x4 DRY	No.2	SPF
E - C	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-1	MT20	4.0	4.0	2.00 1.25
D	BMW+w	MT20	2.0	4.0	
E	BMV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
E	209	0	209	0	0
B	174	0	174	0	0
C	35	0	39	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1'-8".

SEE MITEK STANDARD DETAIL 697791H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS		1ST LCASE	MAX /MIN COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	148	97/0	0/0	0/0	0/0	51/0	0/0
B	120	97/0	0/0	0/0	0/0	23/0	0/0
C	28	0/0	0/0	0/0	0/0	28/0	0/0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	(LBS)	(PLF)	CS1 (LC)	UNBRAC	LENGTH	FR-TO	MEMB.	FORCE	MAX	CS1 (LC)
FR-TO											
E-A	-174/0	0.0	0.0	0.02 (1)	7.81	A-D	0/0	0.00 (1)			
A-B	0/0	-91.8	-91.8	0.22 (1)	10.00						
E-D	0/0	-18.5	-18.5	0.07 (4)	10.00						
D-C	0/0	-18.5	-18.5	0.08 (4)	10.00						

TOTAL WEIGHT = 4 X 13 = 52 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = L/360 (0.18")
CALCULATED VERT. DEFL (LL) = L/999 (0.00")
ALLOWABLE DEFL (TL) = L/360 (0.18")
CALCULATED VERT. DEFL (TL) = L/999 (0.01")

CS1: TC=0.22/1.00 (A-B:1), BC=0.08/1.00 (C-D:4),

WB=0.00/1.00 (A-D:1), SS1=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.11 (A) (INPUT = 0.90)
JSI METAL=0.03 (A) (INPUT = 1.00)

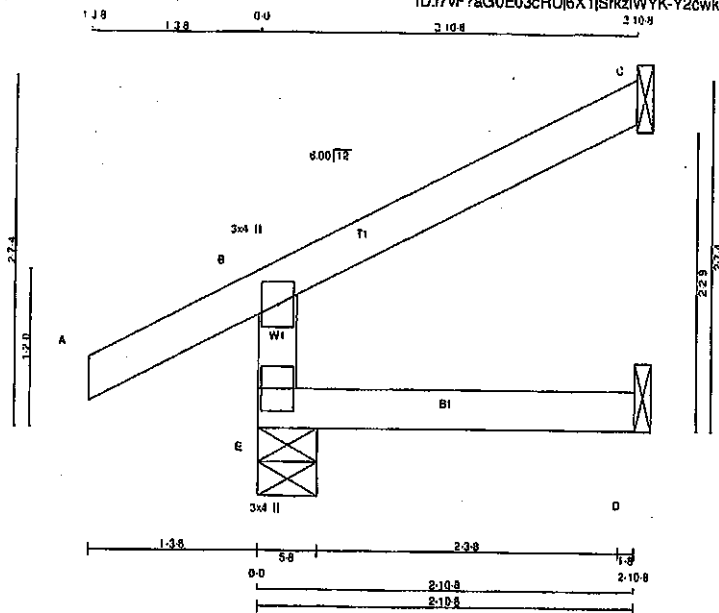


Structural component only
DWG# T-2007105

JOB NAME 408152	TRUSS NAME J41	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/8" = 1'-0"

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG			
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	319	0	319	0	0	5-8	5-8
C	99	0	99	0	0	1-8	1-8
D	23	0	23	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	223	181/0	0/0	0/0	0/0	62/0	0/0
C	68	55/0	0/0	0/0	0/0	13/0	0/0
D	18	0/0	0/0	0/0	0/0	18/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
E-B	-289/0	0.0	0.02 (4)	7.81			
A-B	0/28	-91.8	-91.8 0.13 (5)	10.00			
B-C	-15/0	-91.8	-91.8 0.13 (1)	6.25			
E-D	0/0	-18.5	-18.5 0.03 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 10 = 38 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/360 (0.19")
CALCULATED VERT. DEF. (LL) = L/999 (0.00")
ALLOWABLE DEF. (TL) = L/360 (0.19")
CALCULATED VERT. DEF. (TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (B-C:1), BC=0.03/1.00 (D-E:4), WB=0.00/1.00 (W-B:0), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

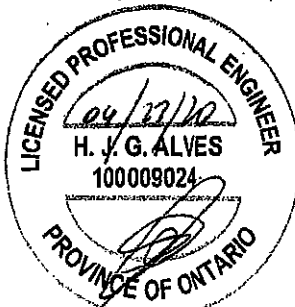
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667	788	1987

PLATE PLACEMENT TOL = 0.250 Inches

PLATE ROTATION TOL = 5.0 Deg.

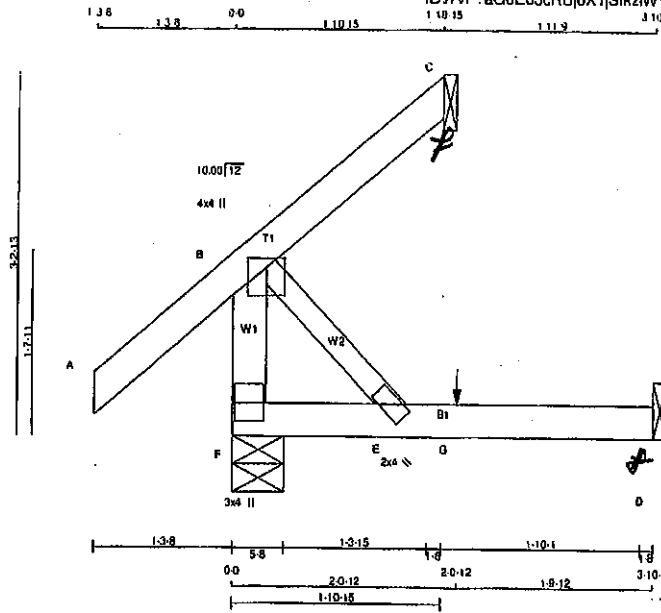
JSI GRIP = 0.12 (E) (INPUT = 0.90)
JSI METAL = 0.08 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007106

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408150	C1	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/16" = 1'-0"

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
F	297	0	297	0	5-8
C	42	0	42	0	5-8
D	38	0	40	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
F	208	145/0	0/0
C	29	23/-26	0/0
D	29	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		MAX FACTORED	FACTORED	WEBS	MAX FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)
F-B	-281/0	0.0	0.0	0.03 (1)	7.81
A-B	0/41	-91.8	-91.8	0.14 (5)	10.00
B-C	-29/0	-91.8	-91.8	0.14 (5)	6.25
F-E	0/0	-18.5	-18.5	0.08 (4)	10.00
E-G	0/0	-18.5	-18.5	0.08 (4)	10.00
G-D	0/0	-18.5	-18.5	0.08 (4)	10.00

FACTORED CONCENTRATED LOADS (LBS)		LOC	LC1	MAX	MAX	FACE	DIR	TYPE	HEEL	CONN.
JT	G	2-0-12	1	1	1	BACK	VERT	TOTAL		CI

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.14/1.00 (A-B:5), BC=0.08/1.00 (D-E:4),
WB=0.00/1.00 (B-E:1), SSI=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

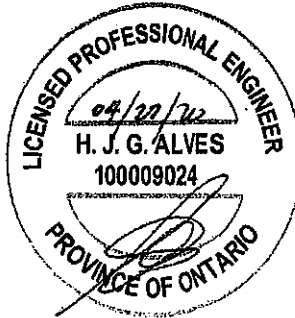
NAIL VALUES		PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN	MAX
MT20	818	354	1887	788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)

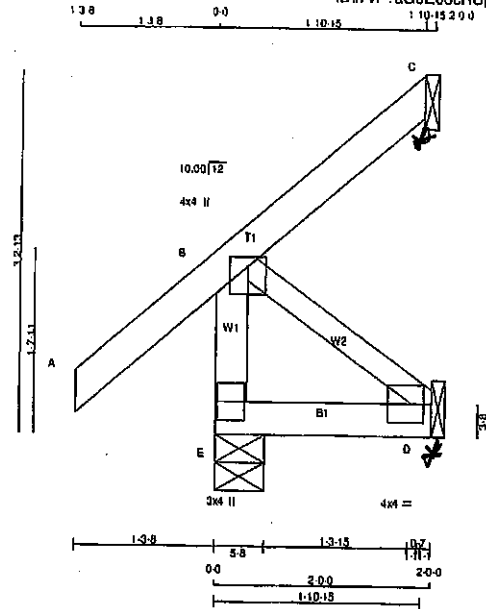
JSI METAL= 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007055

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408150	C2	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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Scale = 1:19.0

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG			
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	278	0	278	0	0	5-8	5-8
C	42	0	42	0	-36	1-8	1-8
D	18	0	20	0	0	1-8	1-8

TOTAL WEIGHT = 10 lb

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
D	BMV1-i	MT20	4.0	4.0	2.00 1.25
E	BMV1-p	MT20	3.0	4.0	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	194	145/0	0/0	0/0	0/0	48/0	0/0
C	29	23/-26	0/0	0/0	0/0	5/0	0/0
D	14	0/0	0/0	0/0	0/0	14/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM TO	LENGTH FR-TO				
E-B	-281/0	0.0	0.0 0.03 (1)	7.81	B-D	0/0	0.00 (1)
A-B	0/41	-91.8	-91.8 0.13 (1)	10.00			
B-C	-29/0	-91.8	-91.8 0.12 (1)	6.25			
E-D	0/0	-18.5	-18.5 0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (B-C:1), SS=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 798 1987 1656

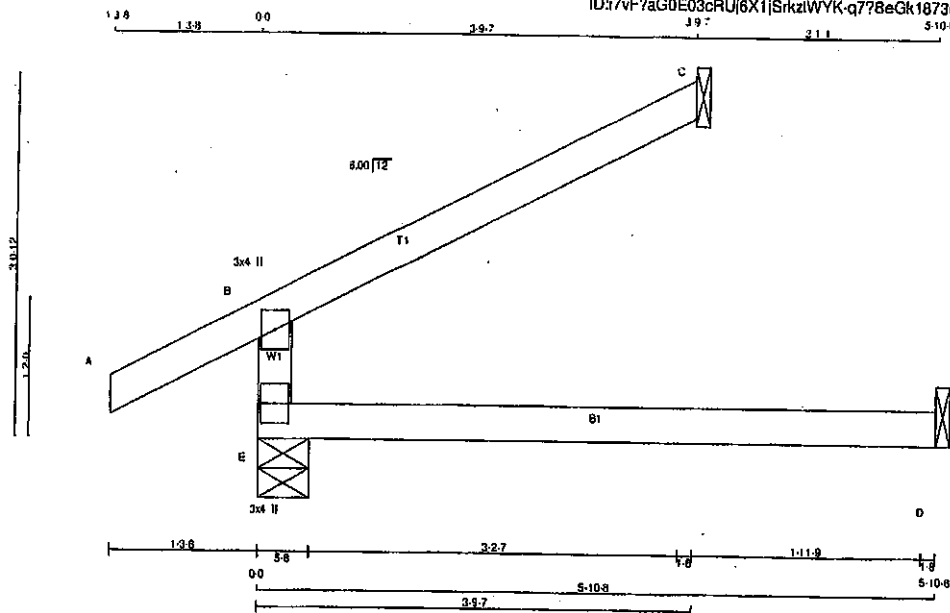
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.20 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007056



TOTAL WEIGHT = $4 \times 14 = 57 \text{ lb}$

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

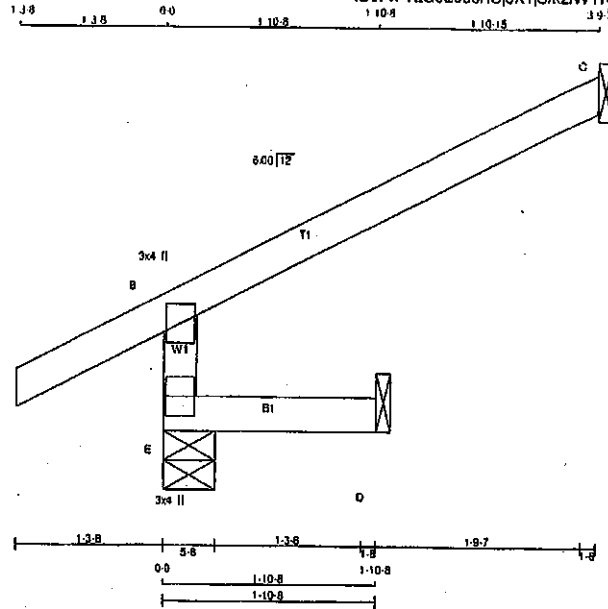


Structural component only
DWG# T-2007098

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408152	C42	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/16\"/>

LUMBER				DESCR.	SPF
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	UPLIFT	IN-SX	BRG	IN-SX
E	361	0	361	0	0	5-8	5-8	5-8	5-8
C	130	0	130	0	0	1-8	1-8	1-8	1-8
D	18	0	17	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS		1ST CASE		MAX/MIN COMPONENT REACTIONS		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	VERT.	LOAD	LC1	MAX	MIN	MAX	MIN	MAX	MIN	MAX	MIN	MAX	MIN
E	250	190/0	0/0	0/0	0/0	0/0	0/0	60/0	0/0	0/0	0/0	60/0	0/0	0/0	0/0
C	90	73/0	0/0	0/0	0/0	0/0	0/0	17/0	0/0	0/0	0/0	17/0	0/0	0/0	0/0
D	12	0/0	0/0	0/0	0/0	0/0	0/0	12/0	0/0	0/0	0/0	12/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT.	LOAD (PLF)	CS1 (LC)	UNBRAC LENGTH	MEMB.	FORCE (LBS)
FR-TO						FR-TO	
E-B	-342/0	0.0	0.0	0.01 (4)	7.81		
A-B	0/28	-91.8	-91.8	0.13 (5)	10.00		
B-C	-19/0	-91.8	-91.8	0.22 (1)	6.25		
E-D	0/0	-18.5	-18.5	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 10 = 38 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
OL = 6.0 PSF
BOT CH. LL = 0.0 PSF
OL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF B08C 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.19")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL) = 1/360 (0.19")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.00")

CS1: TC=0.22/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (A:0), SS1=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 788	1967 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

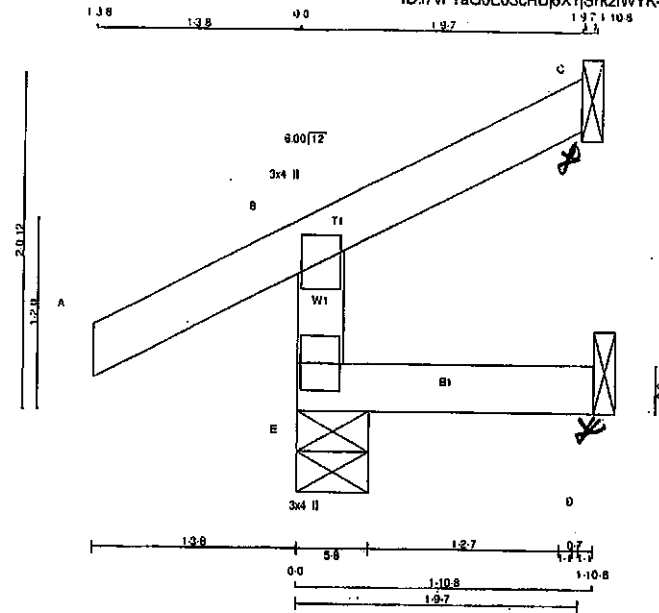


Structural component only
DWG# T-2007100

JOB NAME	TRUSS NAME	QUANTITY	PLV	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408152	C43	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/4" = 1'-0"

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
E - B 2x4 DRY No. 2
A - C 2x4 DRY No. 2
E - D 2x4 DRY No. 2
DRY: SEASONED LUMBER.

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UP/LIFT
JT	271	0	0	5-8
E	45	0	45	5-8
C	45	0	45	5-8
D	8	0	17	5-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS					
	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	188	141/0	0/0	0/0	47/0	0/0
E	31	24/-18	0/0	0/0	7/0	0/0
C	7	0/8	0/0	0/0	12/0	0/0

BEARING MATERIAL TO BE SPF NO. 2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. MEMB. FORCE (LBS)	MAX. MEMB. FORCE (LBS)
FR-TO				
E-B	-244/0	0.0	0.0	0.04 (5)
A-B	0/28	-91.8	-91.8	0.12 (1)
B-C	-17/0	-91.8	-91.8	0.09 (1)
E-D	0/0	-18.5	-18.5	0.04 (5)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 7 = 28 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIG 2011, TPIG 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5), WB=0.00/1.00 (N/A:0), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

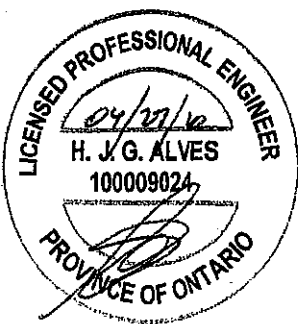
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1666

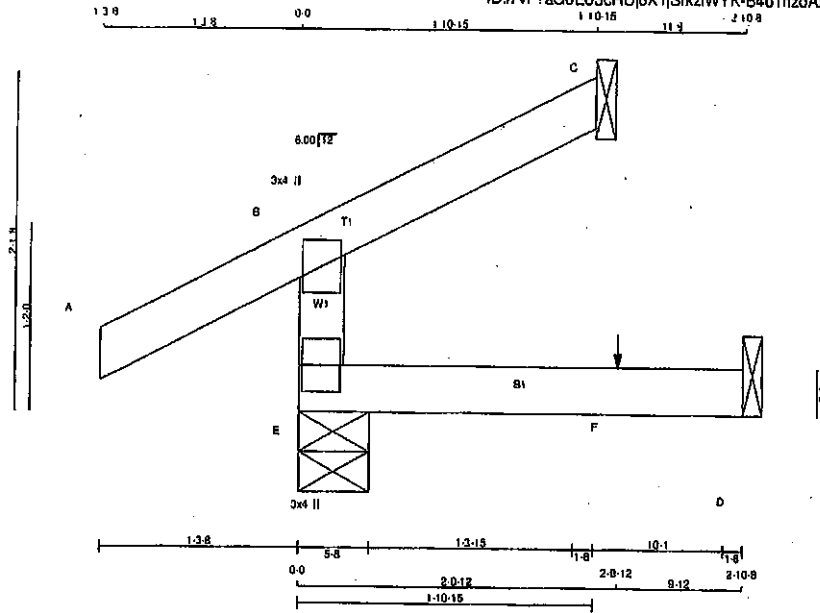
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.10 (E) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007101



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	8MV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
E	264	264	5-8	5-8
C	66	66	1-8	1-8
D	23	23	1-8	1-8

SEE MITEK STANDARD DETAIL 997791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	185	130/0	0/0	0/0	0/0	55/0	0/0	0/0
C	46	37/0	0/0	0/0	0/0	9/0	0/0	0/0
D	18	0/0	0/0	0/0	0/0	18/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

MEMB.	CHORDS				WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO	MAX. FACTORED FORCE (LBS)
FR-TO								
E-B	-234/0	0.0	0.0	0.02 (4)	7.81			
A-B	0/28	-91.8	-91.8	0.13 (5)	10.00			
B-C	-10/0	-91.8	-91.8	0.08 (1)	10.00			
E-F	0/0	-18.5	-18.5	0.03 (4)	10.00			
F-D	0/0	-18.5	-18.5	0.03 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE FRONT	DIR. VERT	TYPE TOTAL	HEEL	CONN. C1
F	2-0-12	1	1	1					

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 8 = 17 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/380 (0.19")
CALCULATED VERT. DEFL.(LL) = 1/989 (0.00")
ALLOWABLE DEFL.(TL) = 1/360 (0.19")
CALCULATED VERT. DEFL.(TL) = 1/989 (0.00")

CSI: TC=0.13/1.00 (A-B:5), BC=0.03/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.10/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

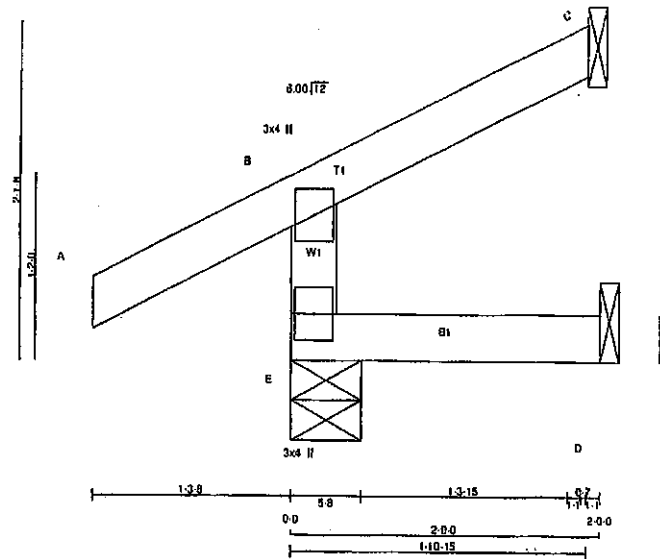


Structural component only
DWG# T-2007102

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408152	C45	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/16"



TOTAL WEIGHT = 2 X 7 = 16 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS	2x4	No.2	SPF
E - B	2x4	DRY	SPF
A - C	2x4	DRY	SPF
E - D	2x4	DRY	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)			
JT	TYPE	PLATES	W LEN V X
B	TMV+p	MT20	3.0 4.0
E	BMV+p	MT20	3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG
E	254 0	254 0	5-8
C	66 0	66 0	1-8
D	18 0	18 0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) G, D

UNFACTORED REACTIONS			
JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	
E	177	130 / 0	0 / 0
C	46	37 / 0	0 / 0
D	13	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)
E-B	-234 / 0	0.0	0.0
A-B	0 / 28	-91.8	-91.8
B-C	-10 / 0	-91.8	-91.8
E-D	0 / 0	-18.5	-18.5

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NRCO 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/380 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (A-B:0), SS=0.08/1.00 (A-B:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1887 788 1887 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.10 (E) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007103

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258-259.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20-24.

Installation:

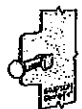
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options Information on p. 126.



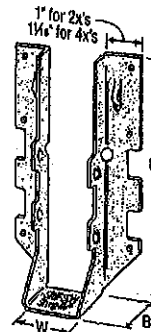
Double-Shear Nailing Top View



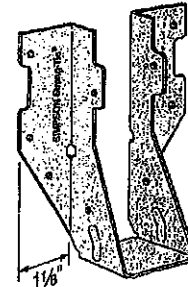
Double-Shear Nailing Side View; Do not bend tab



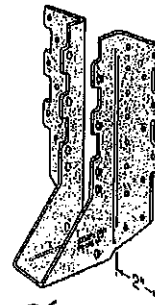
Dome Double-Shear Nailing Side View (available on some models)
U.S. Patent 5,603,580



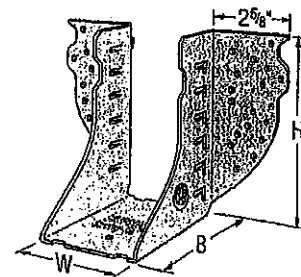
LUS28



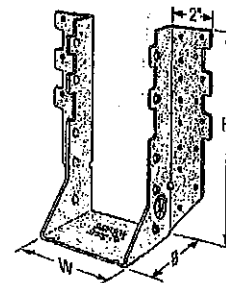
LU26L



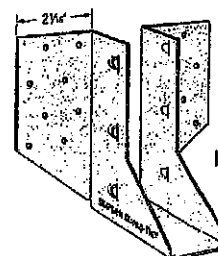
HUS210
(HUS26, HUS28, and HHUS similar)



HGUS28-2

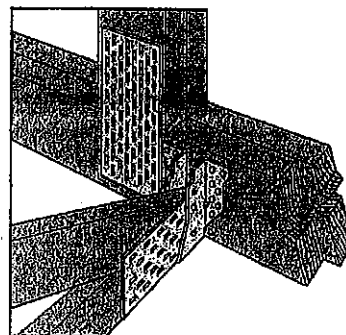


HHUS210-2



LJS26DS

Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)



LUL/LUS/LJS/HUS/HHUS/HGUS**SIMPSON**
Strong-Tie**HHUS/HGUS**

See Hanger Options information on pp. 125–127.

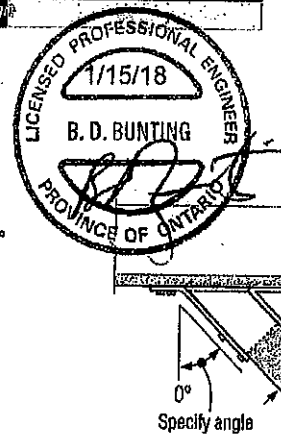
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



**Top View HHUS Hanger
Skewed Right**
(Joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _g ³	Header	Joist	D-Fir-L		S-P-F	
								Uplift (K _D = 1.15)	Normal (K _D = 1.00)	Uplift (K _D = 1.15)	Normal (K _D = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Single 2x Sizes											
LUS24	18	1 1/8	3 1/4	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155
								3.16	7.23	2.87	5.14
LU24L	22	1 1/8	3	1 1/4	2 1/4	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725
								1.60	4.54	1.42	3.22
LU26L	22	1 1/8	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"	720	1605	645	1140
								3.20	7.14	2.87	5.07
SS LUS26	18	1 1/8	4 1/4	1 1/4	3 1/4	(4) 10d	(4) 10d	1420	2170	1290	1630
								6.32	9.65	5.74	7.25
HUS26	18	1 1/8	5 1/4	3	3 1/4	(14) 16d	(8) 16d	2705	4940	2065	3875
								11.80	21.97	9.20	17.24
LJS26DS	18	1 1/8	5	3 1/2	4 1/4	(16) 16d	(8) 16d	2055	4265	1460	4115
								9.14	18.97	6.49	18.31
HGUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 16d	(8) 16d	2685	6625	2685	6700
								11.96	29.51	11.96	25.35
LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(8) 10d x 1 1/2"	1140	2185	1020	1550
								5.07	9.72	4.54	6.89
SS LUS28	18	1 1/8	6 1/4	1 1/4	3 1/4	(6) 10d	(4) 10d	1420	2520	1290	1790
								6.32	11.21	5.74	7.96
HUS28	16	1 1/8	7 1/4	3	6 1/4	(22) 16d	(8) 16d	3805	5365	2675	4345
								16.04	23.86	11.90	19.33
HGUS28	12	1 1/8	7 1/4	5	6 1/4	(36) 16d	(12) 16d	3310	7675	3310	6900
								14.74	34.19	14.74	30.73
LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(8) 10d x 1 1/2"	1140	2495	1020	1770
								5.07	11.10	4.54	7.87
SS LUS210	18	1 1/8	7 1/4	1 1/4	3 1/4	(8) 10d	(4) 10d	1420	2785	1290	2210
								6.32	12.39	5.74	9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.

2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.

3. d_g is the distance from the bearing seat to the top joist nail.

4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.

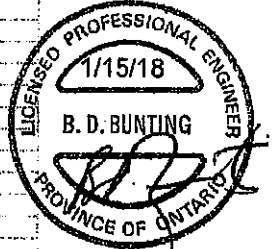
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _a ³	Header	Jolst	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
LUS24-2	18	3 1/8	3 1/8	2	1 1/2	(4) 16d	(2) 18d	835	2020	590	1435
SS LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 18d	3.71	8.99	2.62	6.38
HHUS26-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(8) 18d	1720	2595	1545	1920
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 18d	7.65	11.54	6.87	8.64
SS LUS28-2	18	3 1/8	7	2	4	(8) 16d	(4) 18d	2850	7335	2065	5205
HHUS28-2	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 18d	12.68	32.83	9.20	23.15
HGUS28-2	12	3 1/8	7 1/8	4	8 1/8	(36) 16d	(12) 18d	4385	8950	3110	6355
SS LUS210-2	18	3 1/8	9	2	6	(8) 16d	(8) 18d	19.51	39.81	13.83	28.27
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 18d	1720	3325	1645	2575
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 18d	7.65	14.79	8.87	11.45
								3765	8940	2675	6345
								16.75	39.77	11.90	28.22
								6070	12980	4310	9215
								27.00	57.74	19.17	40.99
								2680	4500	2320	3195
								11.48	20.02	10.32	14.21
								4670	9660	4235	7000
								20.77	42.97	18.84	31.14
								6840	14015	4855	10270
								30.43	62.34	21.60	45.60
Triple 2x Sizes											
HGUS28-3	12	4 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 18d	4385	8950	3110	6355
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 18d	19.51	39.81	13.83	28.27
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 18d	6070	12980	4310	9215
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 18d	27.00	57.74	19.17	40.99
								4670	9670	4235	6865
								20.77	43.02	18.84	30.54
								6840	14645	4855	10400
								30.43	65.14	21.60	46.26
Quadruple 2x Sizes											
HGUS28-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 18d	4385	8950	3110	6355
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 18d	19.51	39.81	13.83	28.27
HHUS210-4	14	6 1/8	8 1/8	3	7 1/8	(30) 16d	(10) 18d	6070	12980	4310	9215
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 18d	27.00	57.74	19.17	40.99
								4670	10155	4235	7210
								20.77	46.17	18.84	32.07
								6840	14645	4855	10400
								30.43	65.14	21.60	46.26
								7640	14995	5425	10645
								33.98	68.70	24.13	47.35
								10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
LUS46	18	3 1/8	4 1/8	2	3 1/8	(4) 16d	(4) 18d	1720	2595	1545	1920
HHUS46	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(8) 18d	7.65	11.54	6.87	8.64
HGUS46	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 18d	2840	7335	2065	5205
LUS48	18	3 1/8	6 1/8	2	3 1/8	(8) 16d	(4) 18d	11.30	32.83	9.20	23.15
HHUS48	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 18d	4385	8950	3110	6355
HGUS48	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 18d	19.51	39.81	13.83	28.27
LUS410	18	3 1/8	8 1/8	2	5 1/8	(8) 16d	(8) 18d	1720	3325	1545	2575
HHUS410	14	3 1/8	9 1/8	3	8 1/8	(30) 16d	(10) 18d	7.65	14.79	8.87	11.45
HGUS410	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 18d	3765	8940	2675	6345
HGUS412	12	3 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 18d	16.75	39.77	11.90	28.22
HGUS414	12	3 1/8	12 1/8	4	11 1/8	(68) 16d	(22) 18d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
								2580	4500	2320	3195
								11.48	20.02	10.32	14.21
								6840	14015	4855	10270
								30.43	62.34	21.60	45.60
								7640	14995	5425	10645
								33.98	68.70	24.13	47.35
								10130	16400	7195	11645
								45.06	72.95	32.00	51.80



Plated Truss Connectors

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

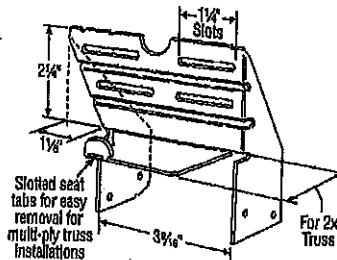
Design: Factored resistances are in accordance with CSA 086-14

Installation:

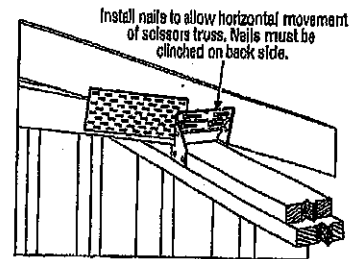
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

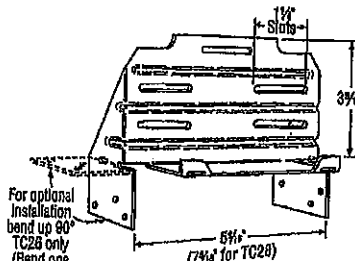
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



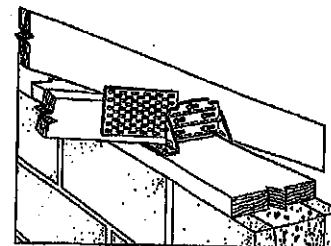
TC24
U.S. Patent 4,832,173



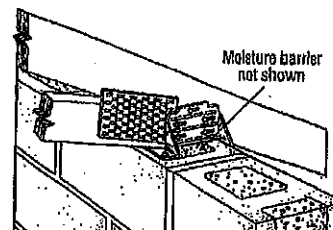
Typical TC24 Installation



TC26
(TC28 Similar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer (8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K ₀ =1.75)	Uplift (K ₀ =1.75)
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K ₀ =1.75)	Uplift (K ₀ =1.75)
TC28	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (8) - ¾" x 2¼" Titen screws has a factored uplift resistance of 275 lb.



(800) 999-5099
strongtie.com

Seismic and Hurricane Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Fasteners			Factored Resistance ($K_D = 1.16$)					
					D.Fir-L			S-P-F		
		To Rafters/ Truss	To Plates	To Studs	Uplift	Lateral		Uplift	Lateral	
						F ₁	F ₂		F ₁	F ₂
					lb.	lb.	lb.	lb.	lb.	lb.
					kN	kN	kN	kN	kN	kN
H1	18	(8) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
					3.29	3.05	1.33	3.02	2.16	0.96
SS H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
					3.69	0.98	0.33	2.62	0.69	0.24
SS H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
					3.58	0.71	0.71	3.36	0.71	0.71
H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
					3.71	0.78	0.93	3.29	0.71	0.93
SS H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
					3.29	0.80	1.18	2.74	0.56	0.85
H6	16	—	(8) 8d	(8) 8d	1585	1085	—	1125	770	—
					7.05	4.83	—	5.00	3.43	—
H7Z	18	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
					6.18	2.98	—	4.40	2.11	—
SS H8 ^a	18	(5) 10d x 1½"	(5) 10d x 1½"	—	1120	—	—	1025	—	—
					4.98	—	—	4.56	—	—
SS H10A ^a	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290
					7.72	3.54	1.82	6.69	2.51	1.29
H10AR	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1485	690	430	1220	570	305
					6.61	3.07	1.91	5.43	2.54	1.36
H10A-2	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1835	1275	430	1645	880	305
					8.16	5.67	1.91	7.32	3.91	1.36
H10S ^a	18	(8) 8d x 1½"	(8) 8d x 1½"	(8) 8d	1465	795	315	1040	565	225
					6.52	3.54	1.40	4.63	2.51	1.00
H11Z	18	(6) 16d x 2½"	(6) 16d x 2½"	—	1095	920	545	780	655	390
					4.87	4.09	2.42	3.47	2.91	1.73
H14	18	[1] (12) 8d x 1½"	(13) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
		[2] (12) 8d x 1½"	(15) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
TSP	16	(9) 10d x 1½"	(6) 10d x 1½"	—	1295	440	—	920	310	—
					5.76	1.96	—	4.09	1.38	—
		(9) 10d x 1½"	(6) 10d	—	1560	440	—	1105	310	—
					6.94	1.96	—	4.92	1.38	—

- Factored resistances have been increased 16% for short term loading; no further increase is allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
- H8 factored uplift resistances for stud-to-bottom plate installations are 595 lb. (2.65 kN) for D.Fir-L and 390 lb. (1.74 kN) for S-P-F.
- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-HTECONPATH).
- Factored resistances in the F₁ direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.

- H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.38 kN) D.Fir-L and 1015 lb. (4.51 kN) S-P-F.
- H10S nails to plates are optional for uplift but required for lateral loads.
- H10A may be field-bent up to a slope of 8/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
- The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SSNAILS at strongtie.com.
- D.Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1½" nails into the top plates and (3) 8d x 1½" nails into the lowest three flange holes into the truss bottom chord is 495 lb. (2.20 kN).
- Nails: 16d x 2½" = 0.162" dia., x 2½" long, 10d = 0.148" dia., x 3" long, 10d x 1½" = 0.148" dia., x 1½" long, 8d = 0.131" dia., x 2½" long, 8d x 1½" = 0.131" dia., x 1½" long. See pp. 27–28 for other nail sizes and information.

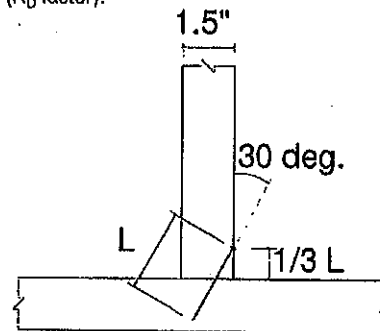
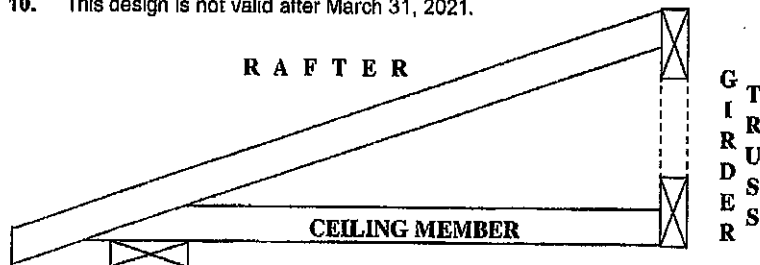
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4.
10. This design is not valid after March 31, 2021.

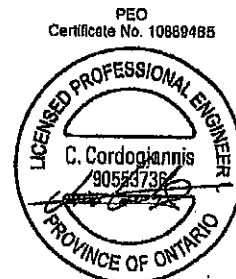


TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (In)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

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100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

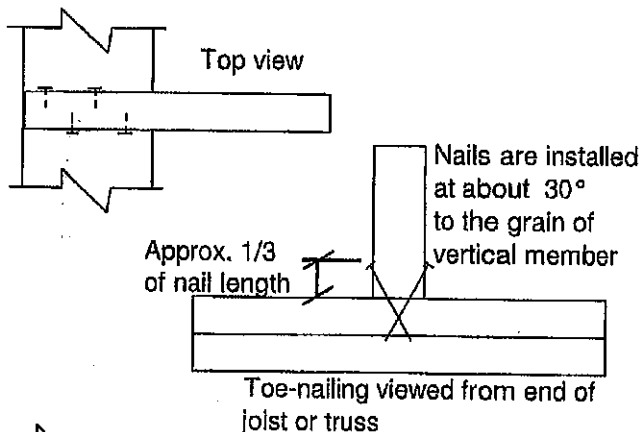
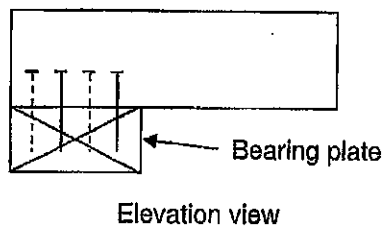
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

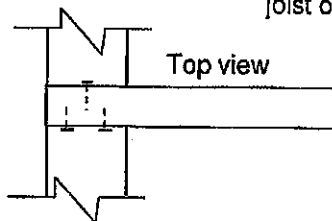
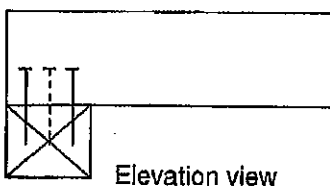
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



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Bradford, Ontario L3Z 3G7

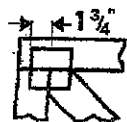
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Certificate No. 10889485



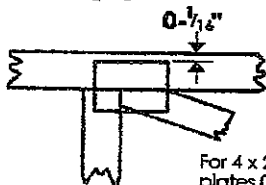
December 2, 2019

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint, unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/4" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

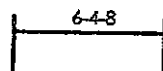


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

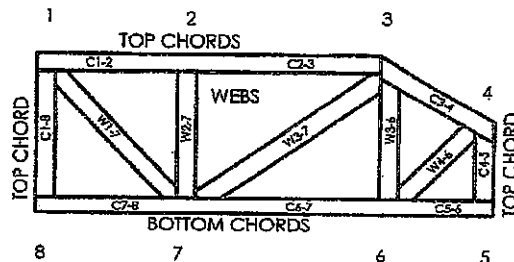
Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MII-7473C rev. 10-08



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

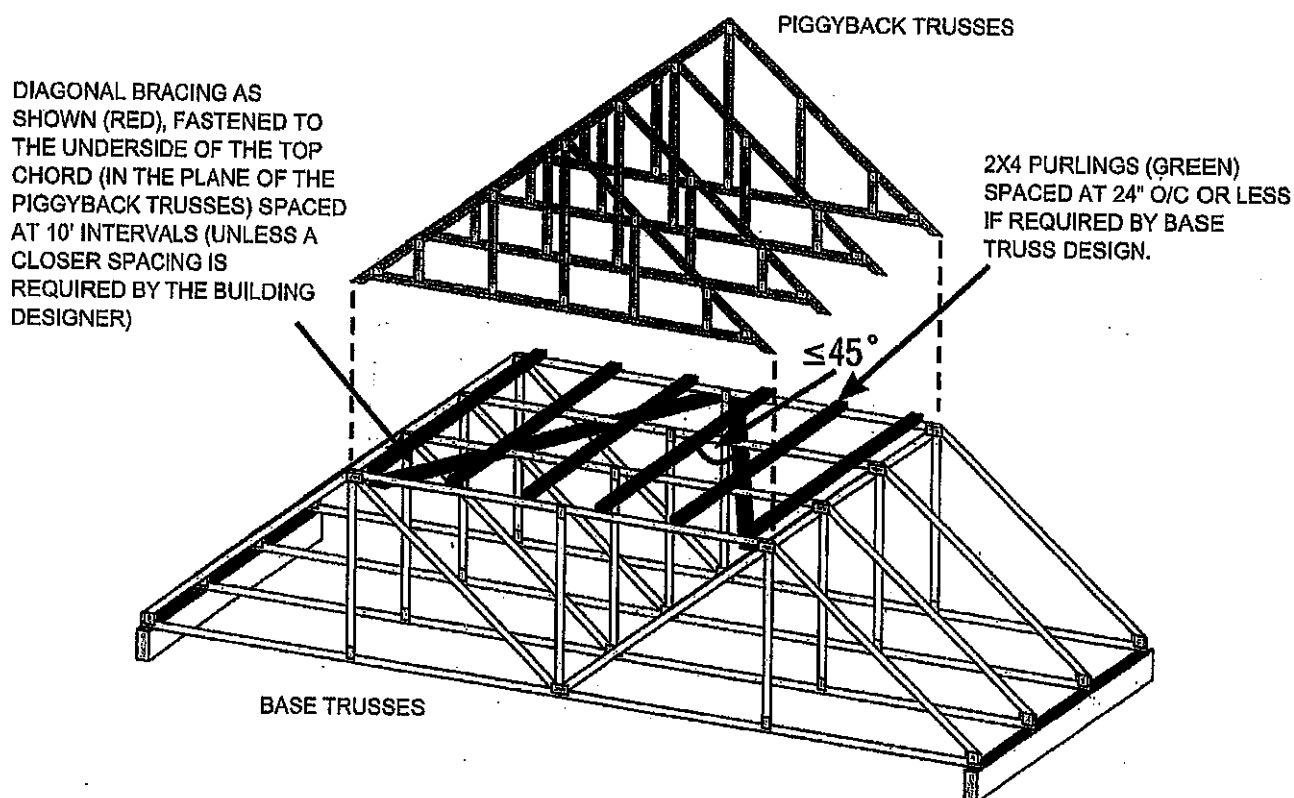
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

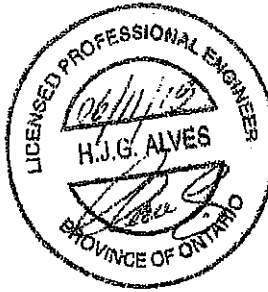


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet M117473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

F-1900213

Feb 09, 2018