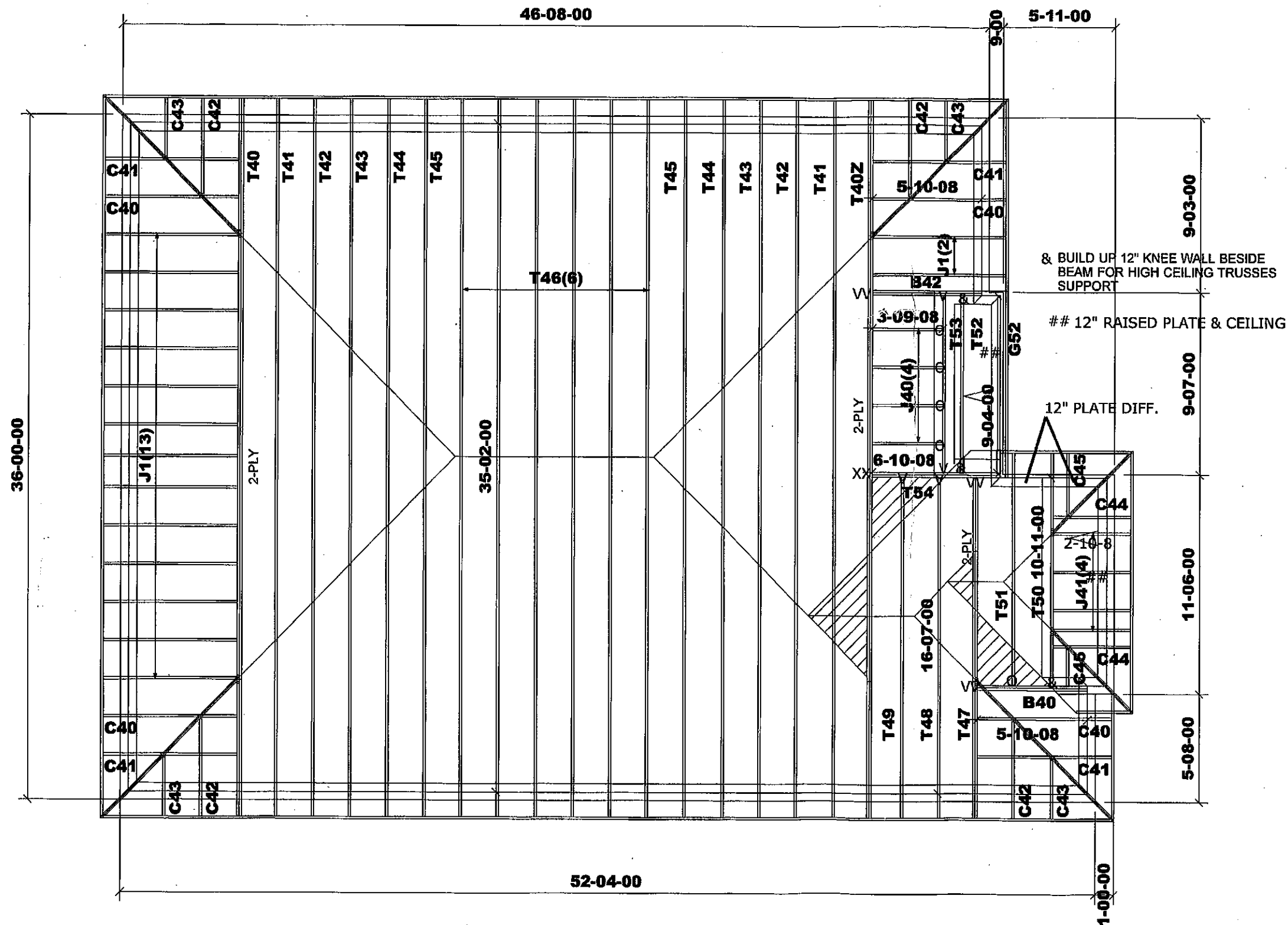


6/12 roof pitch unless noted



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
LUS26-2-(VV)

BEAMS:
B40= B41 = B42= 2 - 2x10 SPF #2

CITY OF HAMILTON
Building Division

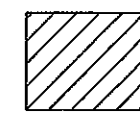
Permit No. 21-105913

THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE

THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW

These drawings and/or specifications have been reviewed by

FOR CHIEF BUILDING OFFICIAL DATE FEB. 23/21

 DENOTES:
CONVENTIONAL
FRAMING

M13125



Job Track: 51225

Plan Log: 202401

Layout ID: 408152

Builder / Location:

GREEN PARK HOMES / WATERDOWN

Project: RUSSELL GARDENS PH.3

Date: 2020-04-27

Sales: Mario DiCano

Designer: JG

Model / Elevation:

MOUNTAINASH 4/3- std. or opt.

Mike ver 8.3.1.215

LOT 232

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

DELIVERY SHIPLIST



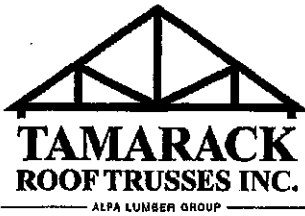
Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 4
 Lot #: 232
 Elevation: 3- std. or opt.

Job Track: 51225
 PlanLog: 202401
 Layout ID: 408152
 Ref #
 Page: 1 of 3
 Date: 04-27-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T40 Hip Girder	6 /12	35-02-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	340.74 212.00		
	1 2-ply	T40Z Hip Girder	6 /12	35-02-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	340.74 212.00		
	2	T41 Hip	6 /12	35-02-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	277.99 174.00		
	2	T42 Hip	6 /12	35-02-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	277.12 174.67		
	2	T43 Hip	6 /12	35-02-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	289.07 179.67		
	2	T44 Hip	6 /12	35-02-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	290.73 183.33		
	2	T45 Hip	6 /12	35-02-00	9-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	301.87 191.33		
	6	T46 Common	6 /12	35-02-00	9-11-08	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	910.08 558.00		
	1 2-ply	T47 Hip Girder	6 /12	16-07-00	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 2-02-00	156.02 100.67		
	1	T48 Hip	6 /12	16-07-00	5-01-04	2 x 4	1-03-08	1-02-00 2-02-00	68.15 43.33		
	1	T49 Common	6 /12	16-07-00	5-09-12	2 x 4	1-03-08	1-02-00 2-02-00	67.14 42.33		
	1	T50 Hip Girder	6 /12	10-11-00	2-07-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	42.73 28.33		
	1	T51 Hip	6 /12	10-11-00	4-07-04	2 x 4	1-03-08 1-03-08	2-02-00 2-02-00	51.54 34.50		
	1	T52 Flat	0 /12	9-04-00	2-10-00	2 x 4		2-10-00 2-10-00	36.11 23.83		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 4
 Lot #: 232
 Elevation: 3- std. or opt.

Job Track: 51225
 PlanLog: 202401
 Layout ID: 408152
 Ref #
 Page: 2 of 3
 Date: 04-27-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	G52 GABLE	0 /12	9-04-00	2-10-00	2 x 4		2-10-00 2-10-00	33.75 23.33		
	1	T53 Flat Girder	0 /12	9-04-00	3-10-00	2 x 6		3-10-00 3-10-00	50.9 31.50		
	1 2-ply	T54 Jack-Closed Girder	6 /12	6-10-08	4-01-04	2 x 4 2 x 6		8-00 4-01-04	59.61 37.33		
	15	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	251.92 160.00		
	4	J40 Jack-Open	6 /12	3-09-08	4-01-04	2 x 4		2-02-08 4-01-04	52.34 35.33		
	4	J41 Jack-Open	6 /12	2-10-08	2-07-04	2 x 4	1-03-08	1-02-00 2-07-04	38.2 24.00		
	4	C40 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	56.53 34.67		
	4	C41 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	46.33 29.33		
	4	C42 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	38.28 24.00		
	4	C43 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	28.08 18.67		
	2	C44 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 11-09	1-02-00 2-01-08	18.64 10.67		
	2	C45 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 1-01	1-02-00 2-01-08	14.65 9.33		

TOTAL # TRUSSES= 74

TOTAL BFT OF ALL TRUSSES= 2596.15

BFT.

TOTAL WEIGHT OF ALL TRUSSES 4137.05 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
4	Hardware	LJS26DS	

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
Builder: GREEN PARK HOMES
Project: RUSSELL GARDENS PH.3
Location: WATERDOWN
Model: MOUNTAINASH 4
Lot #: 232
Elevation: 3- std. or opt.

Job Track: 51225
PlanLog: 202401
Layout ID: 408152
Ref #
Page: 3 of 3
Date: 04-27-2020
Designer:
Sales Rep: Mario DiCano

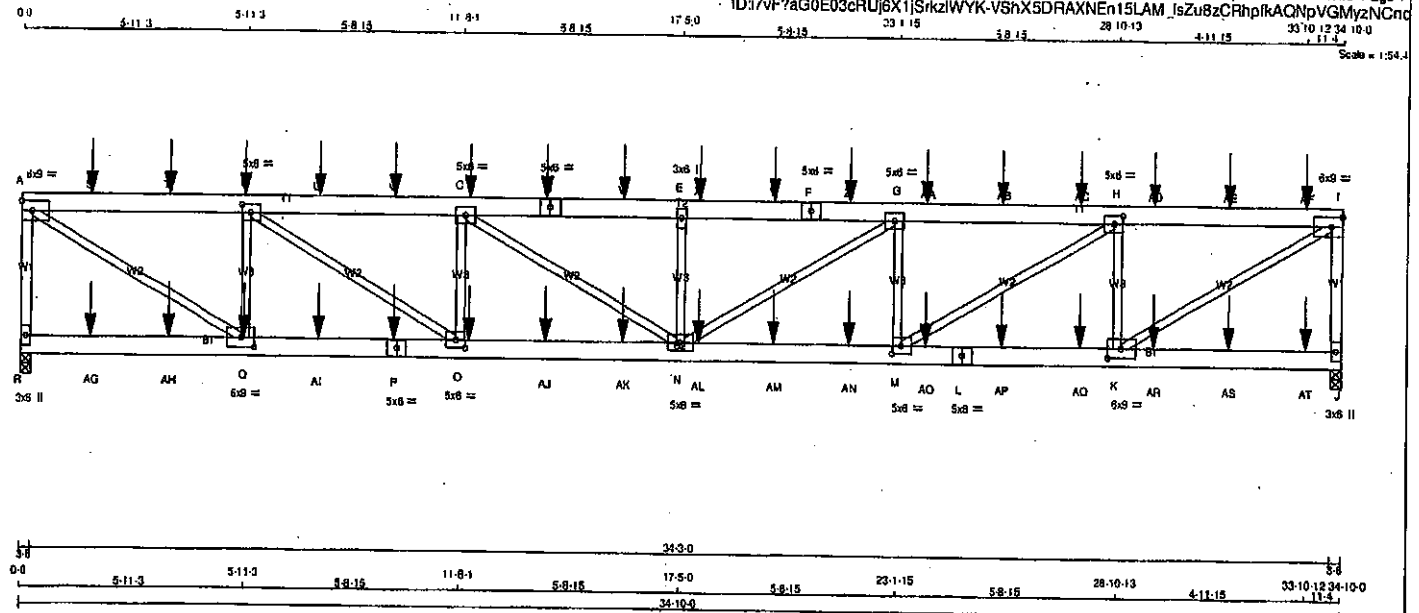
HARDWARE

QTY	TYPE	MODEL	LENGTH
5	Hardware	LUS24	
3	Hardware	LUS26-2	

TOTAL NUMBER OF
ITEMS= 13

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408150	T1	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
R - A	2x4	DRY	No.2
A - D	2x8	DRY	No.2
D - F	2x8	DRY	No.2
F - I	2x8	DRY	No.2
J - L	2x4	DRY	No.2
R - P	2x8	DRY	No.2
P - L	2x8	DRY	No.2
L - J	2x8	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
R - A	12	TOP
A - D	12	TOP
D - F	12	SIDE (183.1)
F - I	12	SIDE (0.0)
I - L	12	SIDE (0.0)
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
R - P	12	SIDE (183.1)
P - L	12	SIDE (183.1)
L - J	12	SIDE (0.0)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT RECORD
	VERT	HORZ	UP/LIFT
JT	3052	0	0
R	3118	0	0
J	3118	0	0

UNFACTORED REACTIONS			
	1ST LOASE	MAX./MIN. COMPONENT REACTIONS	
	COMBINED	SNOW	LIVE
JT	2181	1401 / 0	0 / 0
R	2209	1430 / 0	0 / 0
J	2209	1430 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)
FR-TO			
R-A	-2974 / 0	0.0	0.0
A-S	-4328 / 0	-91.8	0.20 (1)
S-T	-4328 / 0	-91.8	0.20 (1)
T-B	-4328 / 0	-91.8	0.20 (1)
B-U	-6696 / 0	-91.8	0.25 (1)
U-V	-6696 / 0	-91.8	0.25 (1)
V-C	-6696 / 0	-91.8	0.25 (1)
C-D	-7505 / 0	-91.8	0.23 (1)
D-W	-7505 / 0	-91.8	0.23 (1)
W-E	-7505 / 0	-91.8	0.23 (1)
E-X	-7505 / 0	-91.8	0.23 (1)
X-Y	-7505 / 0	-91.8	0.23 (1)
Y-F	-7505 / 0	-91.8	0.23 (1)
F-Z	-7505 / 0	-91.8	0.23 (1)
Z-G	-7505 / 0	-91.8	0.23 (1)
G-AA	-6696 / 0	-91.8	0.23 (1)
AA-AB	-6696 / 0	-91.8	0.26 (1)
AB-AC	-6696 / 0	-91.8	0.26 (1)
AC-H	-6696 / 0	-91.8	0.26 (1)
H-AD	-4330 / 0	-91.8	0.21 (1)
AD-AE	-4330 / 0	-91.8	0.21 (1)
AE-AF	-4330 / 0	-91.8	0.21 (1)
AF-I	-4330 / 0	-91.8	0.21 (1)
I-J	-3028 / 0	0.0	0.0
TO-TO			
R-AG	0 / 0	-18.5	0.06 (4)
AG-AH	0 / 0	-18.5	0.06 (4)
AH-Q	0 / 0	-18.5	0.06 (4)
Q-AI	0 / 4328	-18.5	0.33 (1)
AI-P	0 / 4328	-18.5	0.33 (1)
P-O	0 / 4328	-18.5	0.33 (1)
O-AJ	0 / 6696	-18.5	0.49 (1)
AJ-AK	0 / 6696	-18.5	0.49 (1)
AK-N	0 / 6696	-18.5	0.49 (1)
N-AL	0 / 6696	-18.5	0.49 (1)
AL-AM	0 / 6696	-18.5	0.49 (1)
AM-AN	0 / 6696	-18.5	0.49 (1)
AN-M	0 / 6696	-18.5	0.49 (1)
M-AO	0 / 4330	-18.5	0.33 (1)
AO-L	0 / 4330	-18.5	0.33 (1)
L-AP	0 / 4330	-18.5	0.33 (1)
AP-AQ	0 / 4330	-18.5	0.33 (1)
AQ-K	0 / 4330	-18.5	0.33 (1)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.16')
CALCULATED VERT. DEFL.(LL) = L/999 (0.20')
ALLOWABLE DEFL.(TL) = L/360 (1.16')
CALCULATED VERT. DEFL.(TL) = L/999 (0.38')

CSI: TC=0.35/1.00 (I-J-1), BC=0.49/1.00 (M-N-1),
WB=0.83/1.00 (K-1), SSI=0.17/1.00 (H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (K) (INPUT = 0.90)

JSI METAL= 0.42 (P) (INPUT = 1.00)



Structural component only
DWG# T-2007063 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408150	T1	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (Table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-I	MT20	6.0	9.0		Edge
B	TMVW-I	MT20	5.0	6.0	2.50	2.75
C	TMVW-I	MT20	5.0	6.0		
D	TS-I	MT20	5.0	6.0		
E	TMVW-I	MT20	3.0	6.0		
F	TS-I	MT20	5.0	6.0		
G	TMVW-I	MT20	5.0	6.0		
H	TMVW-I	MT20	5.0	6.0	2.50	2.75
I	TMVW-I	MT20	6.0	9.0		Edge
J	BMV1-p	MT20	3.0	6.0		
K	BMVW-I	MT20	6.0	9.0	3.00	4.25
L	BS-I	MT20	5.0	6.0		
M	BMVW-I	MT20	5.0	6.0	2.50	2.75
N	BMVW-I	MT20	5.0	6.0		
O	BMVW-I	MT20	5.0	6.0	2.50	2.75
P	BS-I	MT20	5.0	6.0		
Q	BMVW-I	MT20	6.0	9.0	3.00	4.25
R	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LCI MAX	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LCI)
FR-TO		FROM TO			FR-TO		
K-AR	0/0	-18.5	-18.5	0.08 (4)	10.00		
AR-AS	0/0	-18.5	-18.5	0.08 (4)	10.00		
AS-AT	0/0	-18.5	-18.5	0.08 (4)	10.00		
AT-J	0/0	-18.5	-18.5	0.08 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LCI	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	5-10-12	-110	-110		FRONT	VERT	TOTAL		C1
C	11-10-12	-110	-110		FRONT	VERT	TOTAL		C1
D	13-10-12	-110	-110		FRONT	VERT	TOTAL		C1
O	11-10-12	-26	-26		FRONT	VERT	TOTAL		C1
P	9-10-12	-26	-26		FRONT	VERT	TOTAL		C1
Q	5-10-12	-26	-26		FRONT	VERT	TOTAL		C1
S	1-10-12	-110	-110		FRONT	VERT	TOTAL		C1
T	3-10-12	-110	-110		FRONT	VERT	TOTAL		C1
U	7-10-12	-110	-110		FRONT	VERT	TOTAL		C1
V	9-10-12	-110	-110		FRONT	VERT	TOTAL		C1
W	15-10-12	-110	-110		FRONT	VERT	TOTAL		C1
X	17-10-12	-110	-110		FRONT	VERT	TOTAL		C1
Y	19-10-12	-110	-110		FRONT	VERT	TOTAL		C1
Z	21-10-12	-110	-110		FRONT	VERT	TOTAL		C1
AA	23-10-12	-110	-110		FRONT	VERT	TOTAL		C1
AB	25-10-12	-110	-110		FRONT	VERT	TOTAL		C1
AC	27-10-12	-110	-110		FRONT	VERT	TOTAL		C1
AD	29-10-12	-110	-110		FRONT	VERT	TOTAL		C1
AE	31-10-12	-110	-110		FRONT	VERT	TOTAL		C1
AF	33-10-12	-113	-113		FRONT	VERT	TOTAL		C1
AG	1-10-12	-26	-26		FRONT	VERT	TOTAL		C1
AH	3-10-12	-26	-26		FRONT	VERT	TOTAL		C1
AI	7-10-12	-26	-26		FRONT	VERT	TOTAL		C1
AJ	13-10-12	-26	-26		FRONT	VERT	TOTAL		C1
AK	15-10-12	-26	-26		FRONT	VERT	TOTAL		C1
AL	17-10-12	-26	-26		FRONT	VERT	TOTAL		C1
AM	19-10-12	-26	-26		FRONT	VERT	TOTAL		C1
AN	21-10-12	-26	-26		FRONT	VERT	TOTAL		C1
AO	23-10-12	-26	-26		FRONT	VERT	TOTAL		C1
AP	25-10-12	-26	-26		FRONT	VERT	TOTAL		C1
AQ	27-10-12	-26	-26		FRONT	VERT	TOTAL		C1
AR	29-10-12	-26	-26		FRONT	VERT	TOTAL		C1
AS	31-10-12	-26	-26		FRONT	VERT	TOTAL		C1
AT	33-10-12	-27	-27		FRONT	VERT	TOTAL		C1

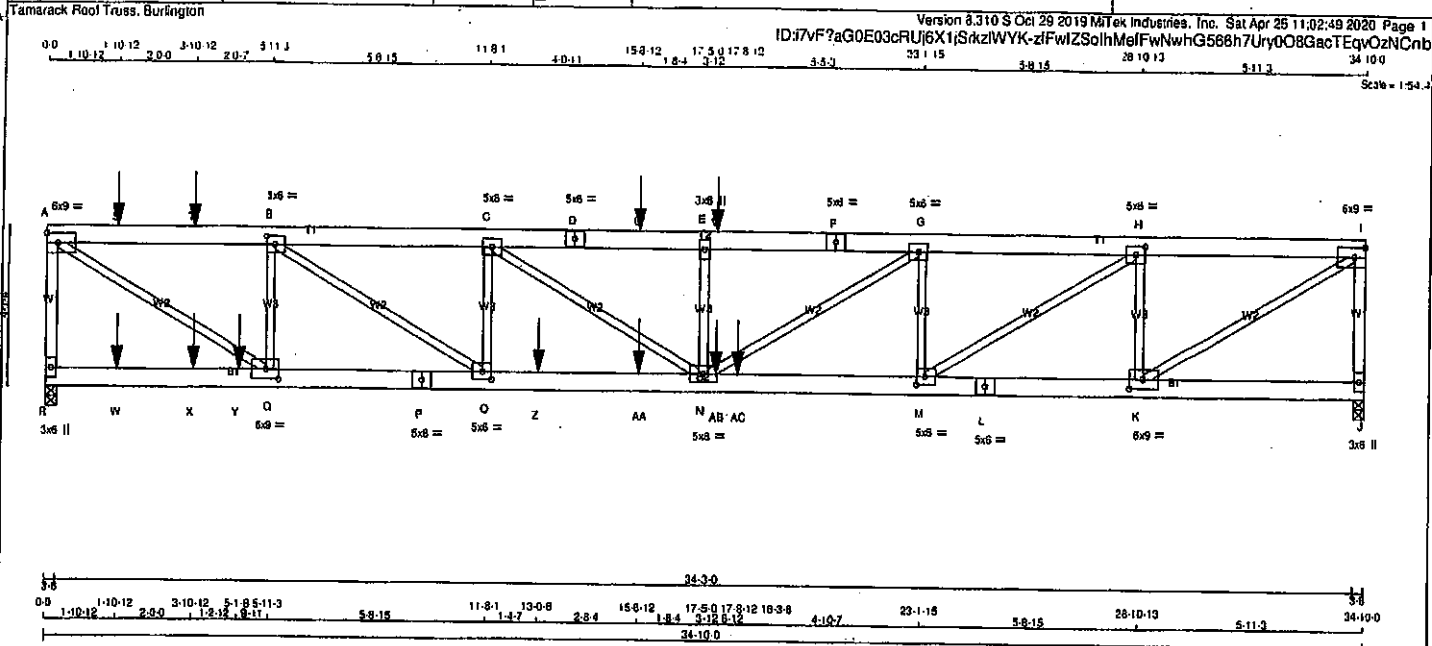
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007063 7/2

JOB NAME 408150	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.310 S Oct 29 2019 Mitex Industries, Inc. Sat Apr 25 11:02:49 2020 Page 1		ID:J7VF?aG0E03cRUJ6X1(SkziWYK-zifWIZSolhMefWnwhG568h7Ury008GactEqvOzNCnb	



LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES					
R - A	2x4	DRY	No.2	SPF	
A - D	2x6	DRY	No.2	SPF	
D - F	2x6	DRY	No.2	SPF	
F - I	2x6	DRY	No.2	SPF	
J - I	2x4	DRY	No.2	SPF	
R - P	2x6	DRY	No.2	SPF	
P - L	2x6	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'X3') SPIRAL NAILS		
R - A	12	TOP
I - J	12	TOP
A - D	12	SIDE(0.0)
D - F	12	SIDE(183.1)
F - I	12	TOP
BOTTOM CHORDS : (0.122'X3') SPIRAL NAILS		
R - P	12	SIDE(0.0)
P - L	12	SIDE(183.1)
L - J	12	TOP
WEBS : (0.122'X3') SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING DESIGNER									
<u>BEARINGS</u>									
		FACTORED		MAXIMUM FACTORED			INPUT	REQRD	
JT		GROSS REACTION		GROSS REACTION			BRG	BRG	
	VERT	HORZ		DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
R	4002	0		4002	0	0	3-B	3-B	
J	3132	0		3132	0	0	3-B	3-B	

UNFACTORED REACTIONS		1ST CASE		MAX/MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
R	2824	1886 / 0	0 / 0	0 / 0	0 / 0
J	2212	1470 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CS1 (LC)	
FR-TO		FROM TO			FR-TO			
R-A	-3851 / 0	0.0	0.0 0.44 (1)	6.01	K-I	0 / 5428	0.87 (1)	
A-S	-5722 / 0	-91.8	-91.8 0.25 (1)	4.72	A-Q	0 / 8717	0.83 (1)	
S-T	-5722 / 0	-91.8	-91.8 0.25 (1)	4.72	K-H	-2783 / 0	0.33 (1)	
T-B	-5722 / 0	-91.8	-91.8 0.25 (1)	4.72	Q-B	-2710 / 0	0.32 (1)	
B-C	-8848 / 0	-91.8	-91.8 0.27 (1)	3.93	M-H	0 / 4038	0.50 (1)	
C-D	-10219 / 0	-91.8	-91.8 0.34 (1)	3.84	B-O	0 / 3700	0.46 (1)	
D-U	-10219 / 0	-91.8	-91.8 0.34 (1)	3.84	M-G	-1901 / 0	0.23 (1)	
U-E	-10219 / 0	-91.8	-91.8 0.34 (1)	3.84	O-C	-1400 / 0	0.17 (1)	
E-V	-10219 / 0	-91.8	-91.8 0.31 (1)	3.67	N-G	0 / 2591	0.32 (1)	
V-F	-10219 / 0	-91.8	-91.8 0.31 (1)	3.67	C-N	0 / 1828	0.20 (1)	
F-G	-10219 / 0	-91.8	-91.8 0.31 (1)	3.67	N-E	-710 / 0	0.08 (1)	
G-H	-8031 / 0	-91.8	-91.8 0.23 (1)	4.14				
H-I	-4822 / 0	-91.8	-91.8 0.15 (1)	5.25				
I-J	-3071 / 0	0.0	0.0 0.35 (1)	8.60				

R-W	0 / 0	-18.5	-18.5	0.16 (1)	10.00
W-X	0 / 0	-18.5	-18.5	0.15 (1)	10.00
X-Y	0 / 0	-18.5	-18.5	0.15 (1)	10.00
Y-Q	0 / 0	-18.5	-18.5	0.15 (1)	10.00
Q-P	0 / 5722	-18.5	-18.5	0.44 (1)	10.00
P-O	0 / 5722	-18.5	-18.5	0.44 (1)	10.00
O-Z	0 / 8848	-18.5	-18.5	0.81 (1)	10.00
Z-AA	0 / 8848	-18.5	-18.5	0.81 (1)	10.00
AA-N	0 / 8848	-18.5	-18.5	0.81 (1)	10.00
N-AB	0 / 8031	-18.5	-18.5	0.74 (1)	10.00
AB-AC	0 / 8031	-18.5	-18.5	0.74 (1)	10.00
AC-M	0 / 8031	-18.5	-18.5	0.74 (1)	10.00
M-L	0 / 4822	-18.5	-18.5	0.33 (1)	10.00
L-K	0 / 4822	-18.5	-18.5	0.33 (1)	10.00
K-J	0 / 0	-18.5	-18.5	0.04 (4)	10.00

FACTORED CONCENTRATED LOADS (LBS)										
JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.	
S	1-10-12	-110	-110	---	BACK	VERT	TOTAL	---	C1	
T	3-10-12	-110	-110	---	BACK	VERT	TOTAL	---	C1	
U	15-8-12	-110	-110	---	BACK	VERT	TOTAL	---	C1	
V	17-8-12	-110	-110	---	BACK	VERT	TOTAL	---	C1	
W	1-10-12	-26	-26	---	BACK	VERT	TOTAL	---	C1	
X	3-10-12	-26	-26	---	BACK	VERT	TOTAL	---	C1	
Y	5-1-8	-727	-727	---	BACK	VERT	TOTAL	---	C1	
Z	13-0-8	-727	-727	---	BACK	VERT	TOTAL	---	C1	
AA	15-8-12	-26	-26	---	BACK	VERT	TOTAL	---	C1	
AB	17-8-12	-26	-26	---	BACK	VERT	TOTAL	---	C1	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

15% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.18")
CALCULATED VERT. DEFL.(LL) = L/999 (0.28")
ALLOWABLE DEFL.(TL) = L/360 (1.18")
CALCULATED VERT. DEFL.(TL) = L/828 (0.50")
CSI: TC=0.44/1.00 (A-R:1), BC=0.81/1.00 (N-O:1),
WB=0.89/1.00 (A-Q:1), SS=0.41/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1887 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (M) (INPUT = 0.90)
JSI METAL = 0.50 (P) (INPUT = 1.00)



Structural component only
DWG# T-2007064 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408150	T1Z	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-I	MT20	6.0	9.0		Edge
B	TMWW-I	MT20	5.0	6.0	2.50	2.75
C	TMWW-I	MT20	5.0	6.0		
D	TS-I	MT20	5.0	6.0		
E	TMWW-w	MT20	3.0	6.0		
F	TS-I	MT20	5.0	6.0		
G	TMWW-I	MT20	5.0	6.0		
H	TMWW-I	MT20	5.0	6.0	2.50	2.75
I	TMWW-I	MT20	6.0	9.0		Edge
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-I	MT20	6.0	9.0	3.00	4.25
L	BS-I	MT20	5.0	6.0		
M	BMWW-I	MT20	5.0	6.0	2.50	2.75
N	BMWW-I	MT20	5.0	6.0		
O	BMWW-I	MT20	5.0	6.0	2.50	2.75
P	BS-I	MT20	5.0	6.0		
Q	BMWW-I	MT20	6.0	9.0	3.00	4.25
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIF.	TYPE	HEEL	CONN.
AC	18-3-B	-1293	-1293	---	BACK	VERT	TOTAL	---	C1

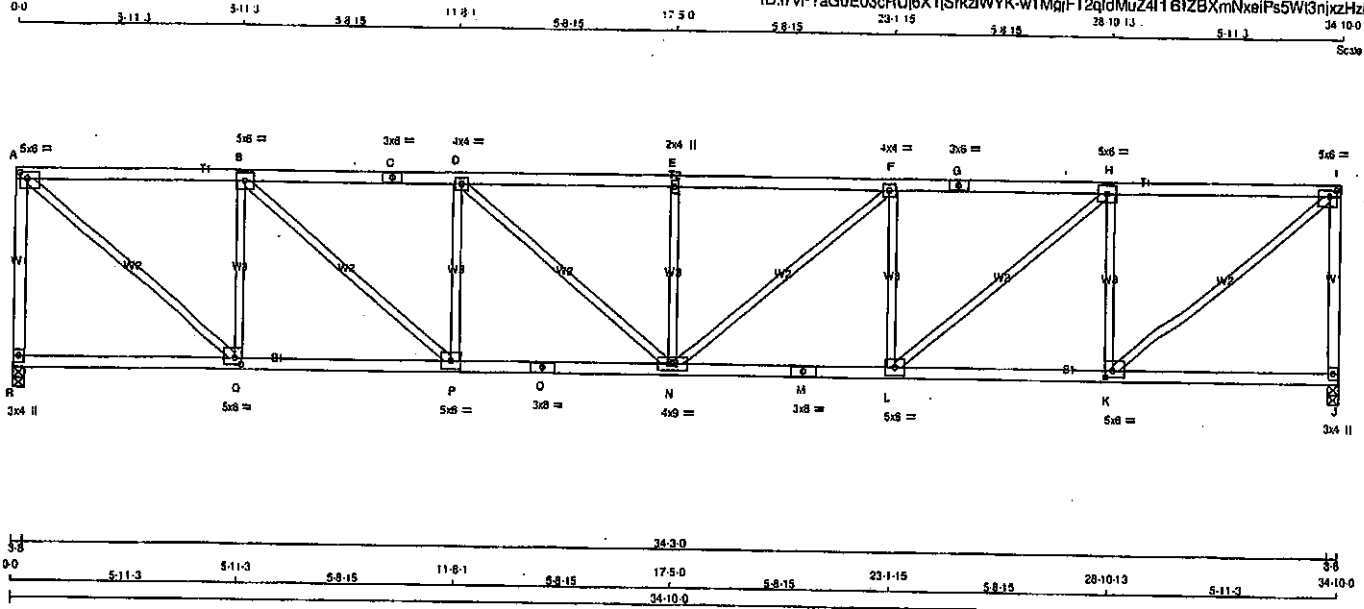
CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007064 72

JOB NAME 408150	TRUSS NAME T2	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						
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Scale = 1:54.4						



LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
R - A	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
C - G	2x4 DRY	No.2	SPF
G - I	2x4 DRY	No.2	SPF
J - I	2x4 DRY	No.2	SPF
R - O	2x4 DRY	No.2	SPF
O - M	2x4 DRY	No.2	SPF
M - J	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	X
A	TMVW-1	MT20	5.0	8.0
B	TMVW-1	MT20	5.0	8.0
C	TS-1	MT20	3.0	8.0
D	TMVW-1	MT20	4.0	4.0
E	TMVW-1	MT20	2.0	4.0
F	TMVW-1	MT20	4.0	4.0
G	TS-1	MT20	3.0	8.0
H	TMVW-1	MT20	5.0	8.0
I	TMVW-1	MT20	5.0	8.0
J	BMV1-p	MT20	3.0	4.0
K	BMVW-1	MT20	5.0	8.0
L	BMVW-1	MT20	5.0	8.0
M	BS-1	MT20	3.0	8.0
N	BMVW-1	MT20	4.0	9.0
O	BS-1	MT20	3.0	8.0
P	BMVW-1	MT20	5.0	8.0
Q	BMVW-1	MT20	5.0	8.0
R	BMV1-p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
R	1920	0	1920	0	0	3-8	3-8	3-8	3-8
J	1920	0	1920	0	0	3-8	3-8	3-8	3-8

UNFACTORED REACTIONS		1ST LOSE		MAX./MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
R	1358	891 / 0	0 / 0	0 / 0	0 / 0
J	1358	891 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MEMB.		MAX. FACTORED		FACTORED		MAX. FACTORED		WEBS		MEMB.		MAX. FACTORED	
FR-TO	MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1	MAX	LC1	MAX	LC1	MAX	FR-TO	MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1	MAX
R-A	-1875 / 0	0.0	0.0	0.83 (1)	6.13	K-I	0 / 2830	0.59 (1)							
A-B	-2042 / 0	-91.8	-91.8	0.59 (1)	4.10	A-Q	0 / 2830	0.59 (1)							
B-C	-3149 / 0	-91.8	-91.8	0.74 (1)	3.28	K-H	-1535 / 0	0.59 (1)							
C-D	-3149 / 0	-91.8	-91.8	0.74 (1)	3.28	Q-B	-1535 / 0	0.59 (1)							
D-E	-3533 / 0	-91.8	-91.8	0.88 (1)	3.23	L-H	0 / 1444	0.33 (1)							
E-F	-3533 / 0	-91.8	-91.8	0.68 (1)	3.23	B-P	0 / 1444	0.33 (1)							
F-G	-3149 / 0	-91.8	-91.8	0.74 (1)	3.28	L-F	-826 / 0	0.32 (1)							
G-H	-3149 / 0	-91.8	-91.8	0.74 (1)	3.28	P-D	-826 / 0	0.32 (1)							
H-I	-2042 / 0	-91.8	-91.8	0.59 (1)	4.10	N-F	0 / 502	0.11 (1)							
J-I	-1875 / 0	0.0	0.0	0.83 (1)	6.13	D-N	0 / 502	0.11 (1)							
R-Q	0 / 0	-18.5	-18.5	0.15 (4)	10.00	N-E	-537 / 0	0.21 (1)							
Q-P	0 / 2042	-18.5	-18.5	0.40 (1)	10.00										
P-O	0 / 3149	-18.5	-18.5	0.56 (1)	10.00										
O-N	0 / 3149	-18.5	-18.5	0.68 (1)	10.00										
N-M	0 / 3149	-18.5	-18.5	0.58 (1)	10.00										
M-L	0 / 3149	-18.5	-18.5	0.58 (1)	10.00										
L-K	0 / 2042	-18.5	-18.5	0.40 (1)	10.00										
K-J	0 / 0	-18.5	-18.5	0.15 (4)	10.00										

TOTAL WEIGHT = 2 X 143 = 286 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 0.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.16")
CALCULATED VERT. DEFL. (LL) = L/998 (0.23")
ALLOWABLE DEFL. (TL) = L/360 (1.16")
CALCULATED VERT. DEFL. (TL) = L/998 (0.42")

CSI: TC=0.83/1.00 (A-R:1), BC=0.58/1.00 (N-P:1), WB=0.69/1.00 (H-K:1), SS=0.25/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

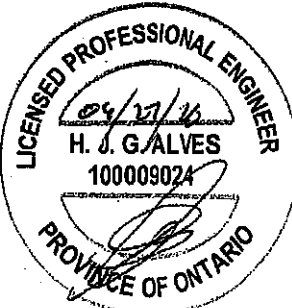
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP (DRY)	SHEAR	SECTION	
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (A) (INPUT = 0.90)
JSI METAL = 0.99 (M) (INPUT = 1.00)

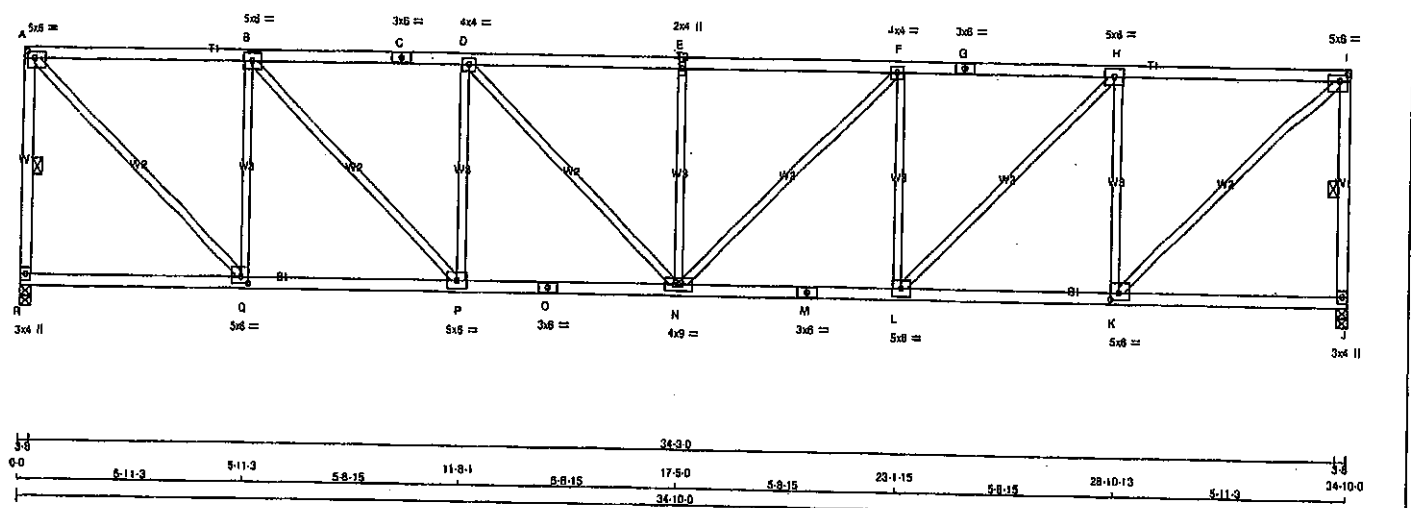
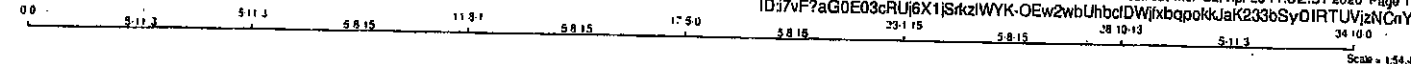


Structural component only
DWG# T-2007065

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408150	T3	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
TRUSS DESC.					

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
R - A	2x4	DRY No.2
A - C	2x4	DRY No.2
C - G	2x4	DRY No.2
G - I	2x4	DRY No.2
I - J	2x4	DRY No.2
J - O	2x4	DRY No.2
O - M	2x4	DRY No.2
M - J	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQ'D BRG
JT	VERT	DOWN	IN-SX	IN-SX
R	1820	0	0	3-8
J	1820	0	0	3-8

UNFACTORED REACTIONS

	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED							
R	1358	891 / 0	0 / 0	0 / 0	0 / 0	467 / 0	0 / 0	0 / 0
J	1358	891 / 0	0 / 0	0 / 0	0 / 0	467 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-R, I-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1	MAX	CS1 (LC)	UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX	CS1 (LC)
FR-TO												
R-A	-1875 / 0	0.0	0.0	0.33 (1)	4.90	K-I	0 / 2368	0.53 (1)				
A-B	-1891 / 0	-91.8	-91.8	0.55 (1)	4.46	A-Q	0 / 2368	0.53 (1)				
B-C	-2808 / 0	-91.8	-91.8	0.68 (1)	3.83	K-H	-1536 / 0	0.90 (1)				
C-D	-2808 / 0	-91.8	-91.8	0.68 (1)	3.83	Q-B	-1536 / 0	0.90 (1)				
D-E	-2827 / 0	-91.8	-91.8	0.58 (1)	3.59	L-H	0 / 1304	0.29 (1)				
E-F	-2827 / 0	-91.8	-91.8	0.58 (1)	3.59	B-P	0 / 1304	0.29 (1)				
F-G	-2808 / 0	-91.8	-91.8	0.68 (1)	3.83	L-F	-826 / 0	0.48 (1)				
G-H	-2808 / 0	-91.8	-91.8	0.68 (1)	3.83	P-O	-826 / 0	0.48 (1)				
H-I	-1691 / 0	-91.8	-91.8	0.55 (1)	4.46	N-F	0 / 453	0.10 (1)				
I-J	-1875 / 0	0.0	0.0	0.33 (1)	4.90	D-N	0 / 453	0.10 (1)				
						N-E	-537 / 0	0.31 (1)				
R-Q	0 / 0	-18.5	-18.5	0.15 (4)	10.00							
Q-P	0 / 1691	-18.5	-18.5	0.35 (1)	10.00							
P-O	0 / 2808	-18.5	-18.5	0.47 (1)	10.00							
O-N	0 / 2808	-18.5	-18.5	0.47 (1)	10.00							
N-M	0 / 2808	-18.5	-18.5	0.47 (1)	10.00							
M-L	0 / 2808	-18.5	-18.5	0.47 (1)	10.00							
L-K	0 / 1691	-18.5	-18.5	0.35 (1)	10.00							
K-J	0 / 0	-18.5	-18.5	0.15 (4)	10.00							

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. G/G

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2010
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.18")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (1.18")
CALCULATED VERT. DEFL.(TL) = L/999 (0.33")

CS1: TC=0.66/1.00 (F-H:1), BC=0.47/1.00 (N-P:1), WB=0.90/1.00 (H-K:1), SSI=0.25/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

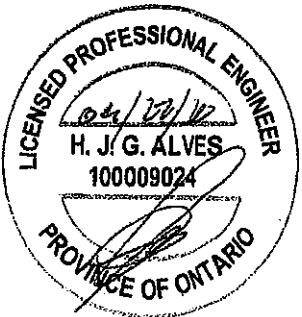
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1685

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

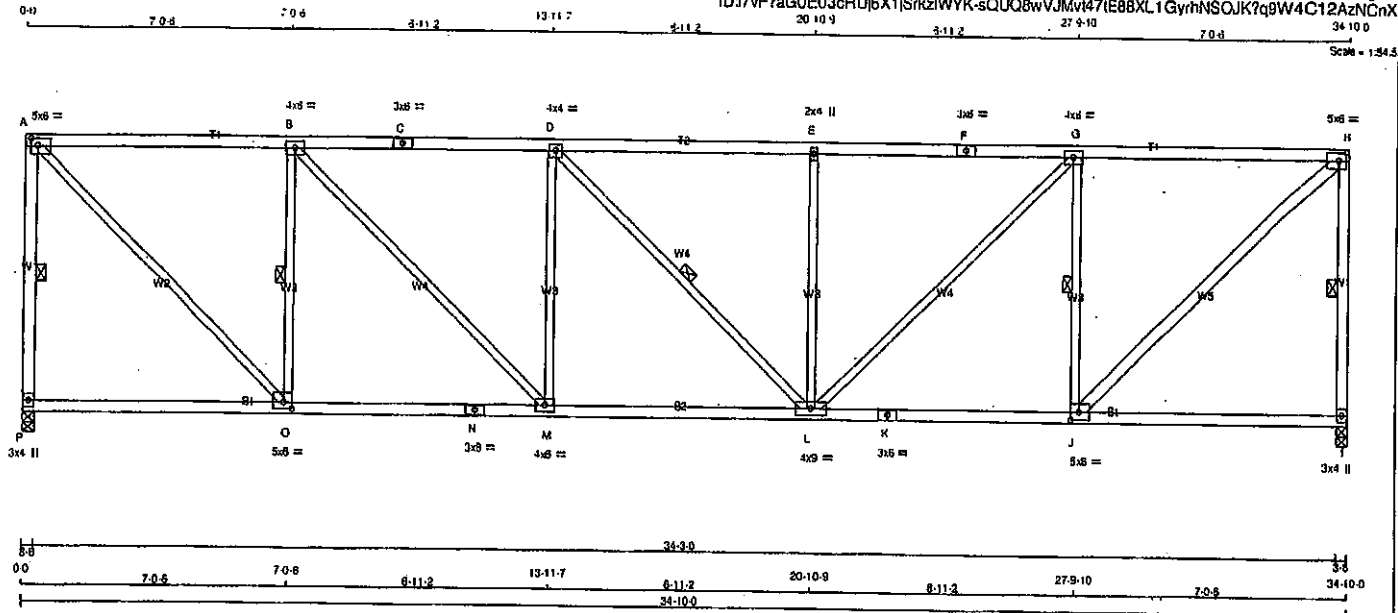
JSI GRIP = 0.88 (I) (INPUT = 0.96)
JSI METAL = 0.83 (M) (INPUT = 1.00)



Structural component only
DWG# T-2007086

JOB NAME 408150	TRUSS NAME T4	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:7vF7aG0E03cRUj6X1iSkziWYK-sQUQ8wVJMv47IE88XL1GymNSOJK7qBW4C12AzNCnX



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
P - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
I - H	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS EXCEPT J - H	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW-1	MT20	5.0	8.0 2.25 2.00
B	TMVW-1	MT20	4.0	8.0
C	TS-1	MT20	3.0	8.0
D	TMVW-1	MT20	4.0	4.0
E	TMVW-1	MT20	2.0	4.0
F	TS-1	MT20	3.0	8.0
G	TMVW-1	MT20	4.0	8.0
H	TMVW-1	MT20	5.0	8.0 2.50 2.50
I	BMV1+p	MT20	3.0	4.0
J	BMVW-1	MT20	5.0	8.0 2.50 2.50
K	BS-1	MT20	3.0	8.0
L	BMVW-1	MT20	4.0	8.0
M	BMVW-1	MT20	4.0	8.0
N	BS-1	MT20	3.0	8.0
O	BMVW-1	MT20	5.0	8.0 2.00 2.50
P	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT REQD BRG
P	1920 0	1920 0	3-8
I	1920 0	1920 0	3-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1358	891 / 0	0 / 0	0 / 0	0 / 0	467 / 0	0 / 0
I	1358	891 / 0	0 / 0	0 / 0	0 / 0	467 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-P, H-I, B-O, D-L, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM TO		FR-TO			
P-A	-1868 / 0	0.0	0.0 0.43 (1)	A-O	0 / 2315	0.52 (1)	
A-B	-1862 / 0	-91.8	-91.8 0.82 (1)	O-B	-1468 / 0	0.47 (1)	
B-C	-2416 / 0	-91.8	-91.8 0.95 (1)	B-M	0 / 1058	0.24 (1)	
C-D	-2416 / 0	-91.8	-91.8 0.95 (1)	M-D	-617 / 0	0.54 (1)	
D-E	-2415 / 0	-91.8	-91.8 0.74 (1)	D-L	-2 / 0	0.00 (1)	
E-F	-2415 / 0	-91.8	-91.8 0.98 (1)	L-E	-618 / 0	0.54 (1)	
F-G	-2415 / 0	-91.8	-91.8 0.98 (1)	L-G	0 / 1054	0.24 (1)	
G-H	-1863 / 0	-91.8	-91.8 0.82 (1)	J-G	-1465 / 0	0.47 (1)	
H-I	-1868 / 0	0.0	0.0 0.43 (1)	J-H	0 / 2316	0.37 (1)	
P-O	0 / 0	-18.5	-18.5 0.22 (4)				
O-N	0 / 1862	-18.5	-18.5 0.39 (1)				
N-M	0 / 1662	-18.5	-18.5 0.39 (1)				
M-L	0 / 2416	-18.5	-18.5 0.47 (1)				
L-K	0 / 1863	-18.5	-18.5 0.39 (1)				
K-J	0 / 1663	-18.5	-18.5 0.39 (1)				
J-I	0 / 0	-18.5	-18.5 0.22 (4)				

TOTAL WEIGHT = 2 X 157 = 313 lb

DESIGN CRITERIA:

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = U/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = U/999 (0.29")
CSI: TC=0.96/1.00 (E-G:1), BC=0.47/1.00 (L-M:1), WB=0.54/1.00 (D-M:1), SS=0.30/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	610 354	1667 788

PLATE PLACEMENT TOL. = 0.250 inches

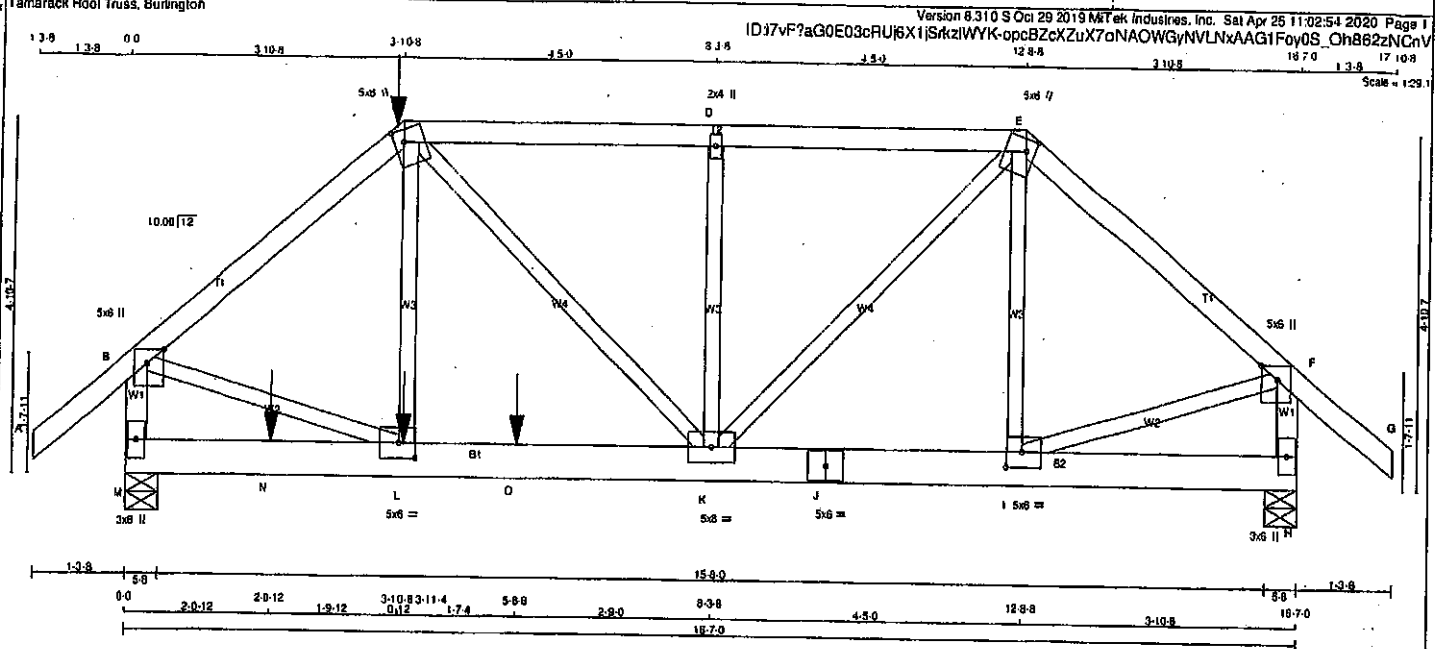
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (A) (INPUT = 0.90)
JSI METAL = 0.53 (K) (INPUT = 1.00)



Structural component only
DWG# T-2007067

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408150	T6	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
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Scale = 1/2" = 1'-0"					



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - J	2x6	DRY	No.2
J - H	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
B	TMVW+P	MT20	5.0	6.0	Edge 2.75
C	TTWW+M	MT20	5.0	8.0	2.00 1.50
D	TMVW+P	MT20	2.0	4.0	
E	TTWW+M	MT20	5.0	8.0	2.00 1.50
F	TMVW+P	MT20	5.0	8.0	Edge 2.75
H	BMVW+P	MT20	3.0	6.0	
I	BS-I	MT20	5.0	6.0	
K	BMVW+P	MT20	5.0	8.0	2.50 2.75
L	BMVW+P	MT20	5.0	8.0	2.50 2.75
M	BMVW+P	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT VERT	HORZ	JT VERT	HORZ	UPLFT	IN-SX	IN-SX	
M 1888	0	1888	0	5-8	5-8	5-8	
H 1422	0	1422	0	5-8	5-8	5-8	

UNFACTORED REACTIONS

1ST LCASE	MAX MIN. COMPONENT REACTIONS						
JT COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
M 1329	909 / 0	0 / 0	0 / 0	0 / 0	420 / 0	0 / 0	
H 1001	683 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (L)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (L)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8 -91.8 0.14 (1)	10.00	L-C	0 / 383	0.09 (1)	
B-C	-1847 / 0	-91.8 -91.8 0.34 (1)	4.55	C-K	0 / 310	0.08 (1)	
C-D	-1841 / 0	-91.8 -91.8 0.36 (1)	4.72	K-D	-481 / 0	0.16 (1)	
D-E	-1841 / 0	-91.8 -91.8 0.36 (1)	4.72	K-E	0 / 1010	0.25 (1)	
E-F	-1225 / 0	-91.8 -91.8 0.30 (1)	5.40	I-E	-274 / 0	0.09 (1)	
F-G	0 / 41	-91.8 -91.8 0.14 (1)	10.00	B-L	0 / 1480	0.37 (1)	
M-B	-1928 / 0	0.0 0.0 0.22 (1)	6.01	I-F	0 / 981	0.24 (1)	
H-F	-1380 / 0	0.0 0.0 0.16 (1)	8.88				
M-N	0 / 0	-18.5 -18.5 0.24 (1)	10.00				
N-L	0 / 0	-18.5 -18.5 0.24 (1)	10.00				
L-O	0 / 1424	-18.5 -18.5 0.70 (1)	10.00				
O-K	0 / 1424	-18.5 -18.5 0.70 (1)	10.00				
K-J	0 / 932	-18.5 -18.5 0.29 (1)	10.00				
J-I	0 / 932	-18.5 -18.5 0.29 (1)	10.00				
I-H	0 / 0	-18.5 -18.5 0.04 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-241	-241	---	FRONT	VERT	TOTAL	---	C1
L	3-11-4	-17	-17	---	FRONT	VERT	TOTAL	---	C1
N	2-0-12	-17	-17	---	FRONT	VERT	TOTAL	---	C1
O	5-6-8	-953	-953	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2010, NBCO 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 085-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.38/1.00 (C-D:1), BC=0.70/1.00 (K-L:1),
WB=0.37/1.00 (B-L:1), SS=0.45/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667
	788	1987	1668

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

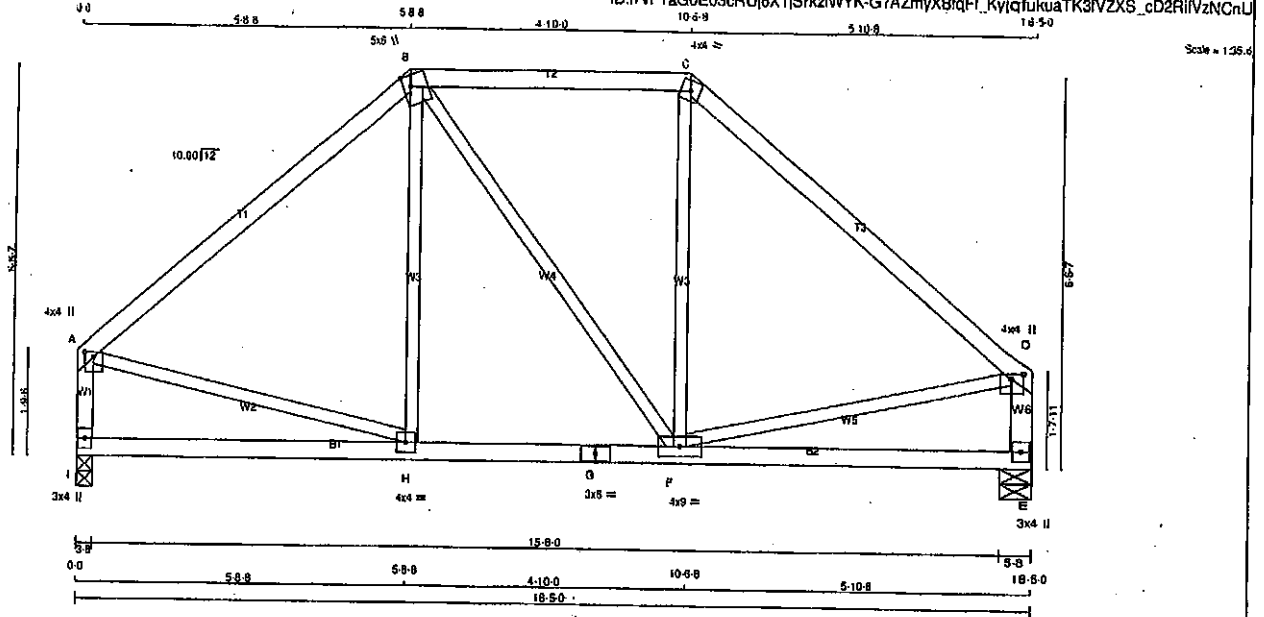
JSI GRIP = 0.89 (C) (INPUT = 0.90)
JSI METAL = 0.57 (J) (INPUT = 1.00)



Structural component only
DWG# T-2007069

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408150	T7	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

PLATES (tablets in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW+p	MT20	4.0	4.0 1.00 2.00
B	TTWW+m	MT20	5.0	6.0 2.25 1.50
C	TTW-m	MT20	4.0	4.0
D	TMVW+p	MT20	4.0	4.0 1.00 2.00
E	BMV1+p	MT20	3.0	4.0
F	BMVWW-t	MT20	4.0	9.0
G	BS-t	MT20	3.0	6.0
H	BMVW-t	MT20	4.0	4.0
I	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
I	905 0	905 0	0 3-8	0 3-8	
E	905 0	905 0	0 3-8	0 3-8	

UNFACTORED REACTIONS							
JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD
I	840	420 / 0	0 / 0	0 / 0	0 / 0	0 / 0	220 / 0
E	840	420 / 0	0 / 0	0 / 0	0 / 0	0 / 0	220 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (LC1)
FR-TO				FR-TO			
A-B	-718 / 0	-91.8	-91.8 0.39 (1)	H-B	-34 / 68	0.02 (4)	0.00 (1)
B-C	-558 / 0	-91.8	-91.8 0.28 (1)	B-F	0 / 14	0.00 (1)	0.00 (1)
C-D	-729 / 0	-91.8	-91.8 0.42 (1)	F-C	-23 / 74	0.03 (4)	0.00 (1)
I-A	-860 / 0	0.0	0.0 0.09 (1)	A-H	0 / 567	0.13 (1)	0.00 (1)
E-D	-858 / 0	0.0	0.0 0.09 (1)	F-D	0 / 572	0.13 (1)	0.00 (1)
I-H	0 / 0	-18.5	-18.5 0.13 (4)				
H-G	0 / 550	-18.5	-18.5 0.18 (4)				
G-F	0 / 550	-18.5	-18.5 0.18 (4)				
F-E	0 / 0	-18.5	-18.5 0.14 (4)				

TOTAL WEIGHT = 70 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIG 2011, TPIG 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.55")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.55")
CALCULATED VERT. DEFL. (TL) = L/999 (0.05")

CSI: TC=0.42/1.00 (C-D:1), BC=0.18/1.00 (F-H:4), WB=0.13/1.00 (D-F:1), SS=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

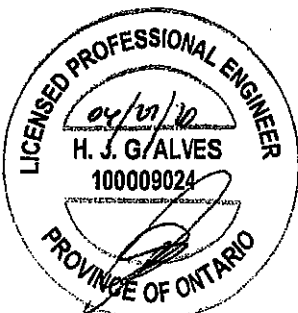
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (D) (INPUT = 0.90)
JSI METAL = 0.24 (D) (INPUT = 1.00)



Structural component only
DWG# T-2007070

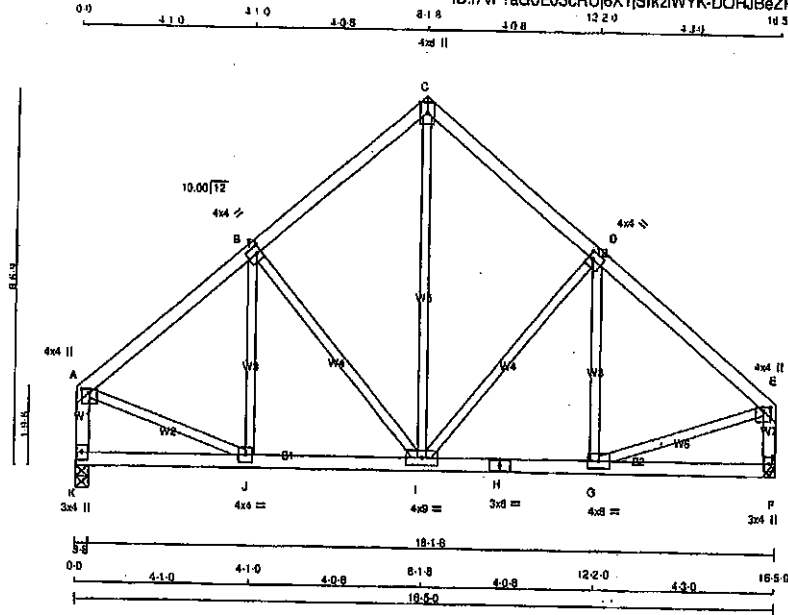
DRWG NO.



JSI GRIP= 0.75 (G) (INPUT = 0.80)
JSI METAL= 0.23 (G) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408150	T9	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.310 S Oct 29 2019 MTek Industries, Inc. Sat Apr 25 11:02:57 2020 Page 1					
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Scale = 1:48.5					



CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
K - A	2x4	DRY	No.2
F - E	2x4	DRY	No.2
K - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV-p	MT20	4.0	4.0	1.00	2.00
B	TMWV-1	MT20	4.0	4.0	2.00	1.25
C	TMWV-p	MT20	4.0	4.0	2.00	1.25
D	TMWV-1	MT20	4.0	4.0	2.00	1.25
E	TMWV-p	MT20	4.0	4.0	2.00	1.25
F	BMV1-p	MT20	3.0	4.0	1.00	2.00
G	BMWV-1	MT20	4.0	4.0	2.00	1.25
H	BS-1	MT20	3.0	4.0	1.00	2.00
I	BMWVW-1	MT20	4.0	4.0	2.00	1.25
J	BMWV-1	MT20	4.0	4.0	2.00	1.25
K	BMV1-p	MT20	3.0	4.0	1.00	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORIZ
K	905	0	905	0
F	905	0	905	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
K	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
F	840	420/0	0/0	0/0	220/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORZ. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (PLF)
FR-TO					FR-TO		
A-B	-772/0	-91.8	-91.8	0.20 (1)	J-B	-135/21	0.05 (1)
B-C	-620/0	-91.8	-91.8	0.19 (1)	B-I	-251/0	0.18 (1)
C-D	-622/0	-91.8	-91.8	0.20 (1)	I-C	0/478	0.11 (1)
D-E	-795/0	-91.8	-91.8	0.21 (1)	I-D	-278/0	0.20 (1)
K-A	-873/0	0.0	0.0	0.10 (1)	G-D	-110/31	0.04 (1)
F-E	-872/0	0.0	0.0	0.09 (1)	A-J	0/653	0.15 (1)
					G-E	0/682	0.15 (1)
K-J	0/0	-18.5	-18.5	0.07 (4)			
J-I	0/615	-18.5	-18.5	0.13 (1)			
I-H	0/633	-18.5	-18.5	0.14 (1)			
H-G	0/833	-18.5	-18.5	0.14 (1)			
G-F	0/0	-18.5	-18.5	0.07 (4)			

TOTAL WEIGHT = 77 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.21/1.00 (D-E:1), BC=0.14/1.00 (G-I:1), WB=0.20/1.00 (D-I:1), SS=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR	SECTION
(PSI)	(PL)
MAX MIN	MAX MIN
MT20	818 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

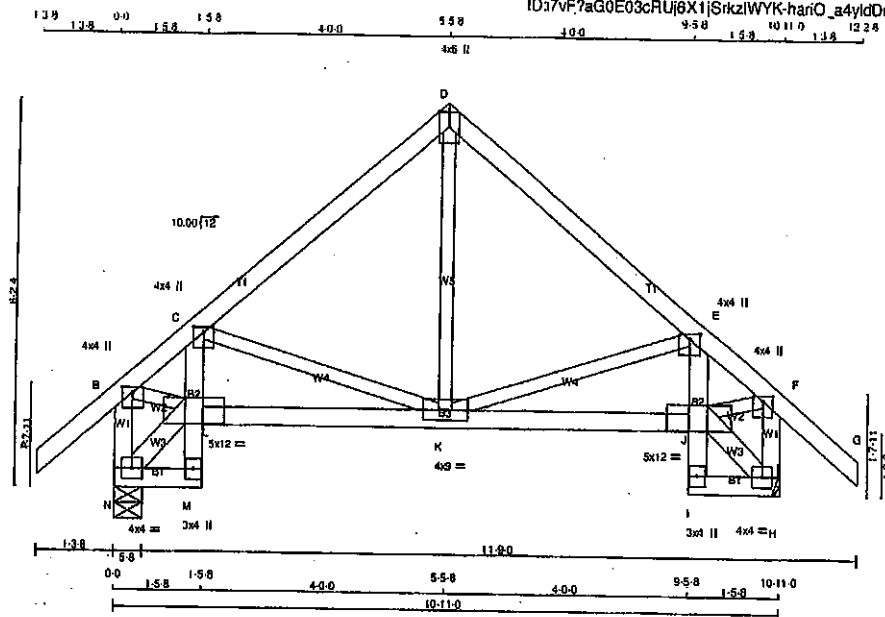
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.24 (E) (INPUT = 1.00)



Structural component only
DWG# T-2007072

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408150	T10S	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.310 8 Oct 2019 Mitek Industries, Inc. Sat Apr 25 11:02:58 2020 Page 1					
ID:7vF7aG0E03cRUJ8X1SskzIWK-harO_a4yldDrohlV6RRWD5vyIX_kqu2v0MFpzNCnR					
Scale = 1/32" = 1'-0"					



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2		
D - G	2x4	DRY	No.2		
N - B	2x4	DRY	No.2		
H - F	2x4	DRY	No.2		
N - M	2x4	DRY	No.2		
M - C	2x4	DRY	No.2		
L - J	2x4	DRY	No.2		
I - E	2x4	DRY	No.2		
I - H	2x4	DRY	No.2		
ALL WEBS	2x3	DRY	No.2		
EXCEPT					
N - L	2x4	DRY	No.2		
J - H	2x4	DRY	No.2		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, C, E, F						
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTW+p	MT20	4.0	8.0	Edge	
H	BMVW1-i	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BMVWW-i	MT20	5.0	12.0	5.00	7.75
K	BMVWW-i	MT20	4.0	9.0		
L	BMVWW-i	MT20	5.0	12.0	5.00	7.75
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-i	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORZ	DOWN	HORZ
N	729	0	729
H	729	0	729

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1'-8".

UNFACTORED REACTIONS

1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT COMBINED						
N	513	350 / 0	0 / 0	0 / 0	163 / 0	0 / 0
H	513	350 / 0	0 / 0	0 / 0	163 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)	UNBRAC	LENGTH	FR-TO	WEBS	MAX. FACTORED	MEMB.	FORCE	MAX	CS1 (LC)
MEMB.	(LBS)	(PLF)									(LBS)		(LBS)		
FR-TO		FROM	TO												
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	C-K	-211 / 0	0.07 (1)							
B-C	-647 / 0	-91.8	-91.8	0.14 (1)	8.25	K-D	0 / 229	0.05 (1)							
C-D	-447 / 0	-91.8	-91.8	0.10 (1)	6.25	K-E	-210 / 0	0.07 (1)							
D-E	-447 / 0	-91.8	-91.8	0.18 (1)	8.25	N-L	-25 / 0	0.00 (1)							
E-F	-646 / 0	-91.8	-91.8	0.14 (1)	8.25	B-L	0 / 527	0.12 (1)							
F-G	0 / 41	-91.8	-91.8	0.13 (1)	10.00	J-F	0 / 526	0.12 (1)							
N-B	-701 / 0	0.0	0.0	0.07 (1)	7.81	J-H	-26 / 0	0.00 (1)							
H-F	-701 / 0	0.0	0.0	0.07 (1)	7.81										
N-M	0 / 20	-18.5	-18.5	0.01 (4)	10.00										
M-L	0 / 14	0.0	0.0	0.03 (1)	10.00										
L-C	-92 / 2	0.0	0.0	0.02 (1)	7.81										
L-K	0 / 526	-18.5	-18.5	0.13 (1)	10.00										
K-J	0 / 627	-18.5	-18.5	0.13 (1)	10.00										
I-J	0 / 14	0.0	0.0	0.03 (1)	10.00										
J-E	-93 / 2	0.0	0.0	0.02 (1)	7.81										
I-H	0 / 20	-18.5	-18.5	0.01 (4)	10.00										

TOTAL WEIGHT = 2 X 58 = 112 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088.09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = $L/360$ (0.38")
CALCULATED VERT. DEF. (LL) = $L/999$ (0.01")
ALLOWABLE DEF. (TL) = $L/360$ (0.38")
CALCULATED VERT. DEF. (TL) = $L/999$ (0.02")

CS1: TC=0.19/1.00 (C-D:1), BC=0.13/1.00 (K-L:1),
WB=0.12/1.00 (B-L:1), SS1=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

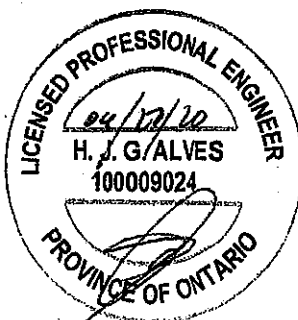
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX	MIN	MAX
MT20	516	354
	1667	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

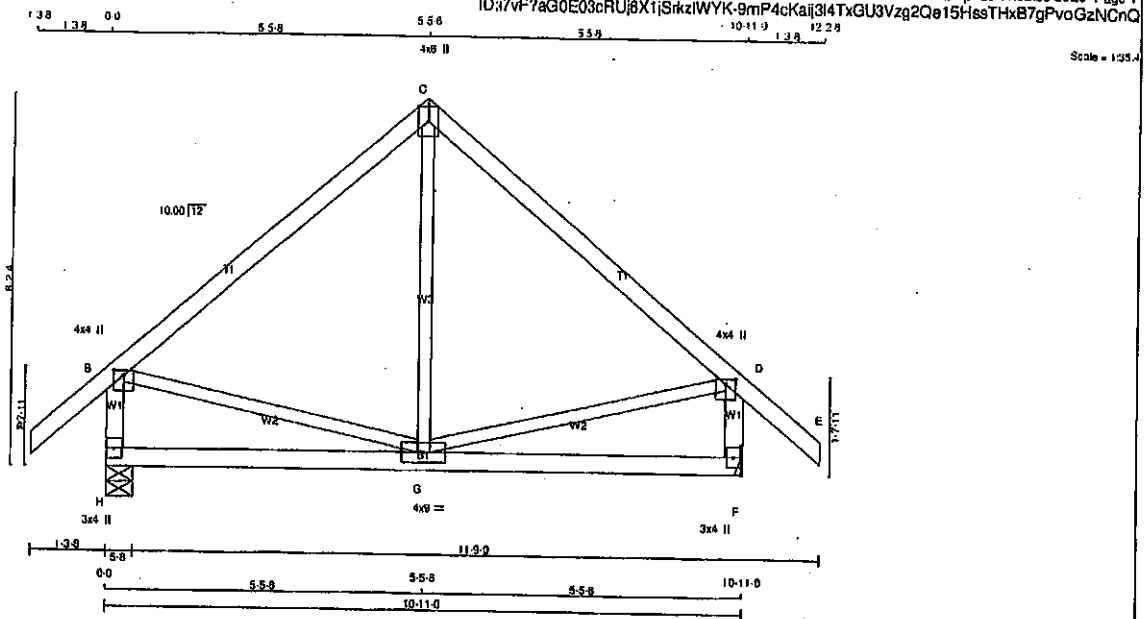
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.74 (F) (INPUT = 0.90)
JSI METAL = 0.20 (F) (INPUT = 1.00)



Structural component only
DWG# T-2007073

JOB NAME 408150	TRUSS NAME T11	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (Table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B - TMVW+p	MT20	4.0	4.0	1.00	2.00
C - TTW+p	MT20	4.0	6.0	Edge	
D - TMVW+p	MT20	4.0	4.0	1.00	2.00
F - BMV1+p	MT20	3.0	4.0		
G - BMVWW-I	MT20	4.0	9.0		
H - BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
H	729	0	0	0
F	729	0	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	513	350 / 0	0 / 0	0 / 0	0 / 0	0 / 0	163 / 0	0 / 0
F	513	350 / 0	0 / 0	0 / 0	0 / 0	0 / 0	163 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00	G-C	-15 / 89	0.03 (4)
B-C	-380 / 0	-91.8	-91.8 0.35 (1)	8.25	B-G	0 / 300	0.07 (1)
C-D	-380 / 0	-91.8	-91.8 0.35 (1)	8.25	G-D	0 / 300	0.07 (1)
D-E	0 / 41	-91.8	-91.8 0.13 (1)	10.00			
H-B	-890 / 0	0.0	0.0 0.07 (1)	7.81			
F-D	-890 / 0	0.0	0.0 0.07 (1)	7.81			
H-G	0 / 0	-18.5	-18.5 0.16 (4)	10.00			
G-F	0 / 0	-18.5	-18.5 0.16 (4)	10.00			

TOTAL WEIGHT = 49 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2018 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.36")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.36")
CALCULATED VERT. DEFL. (TL) = L/989 (0.02")

CSI: TC=0.35/1.00 (B-C:1), BC=0.18/1.00 (G-H:4),
WB=0.07/1.00 (D-G:1), SS=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

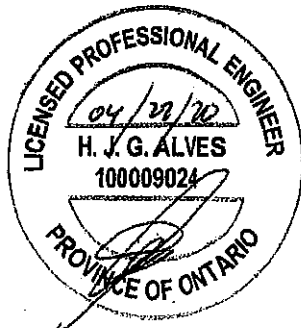
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.82 (D) (INPUT = 0.90)
JSI METAL = 0.17 (D) (INPUT = 1.00)

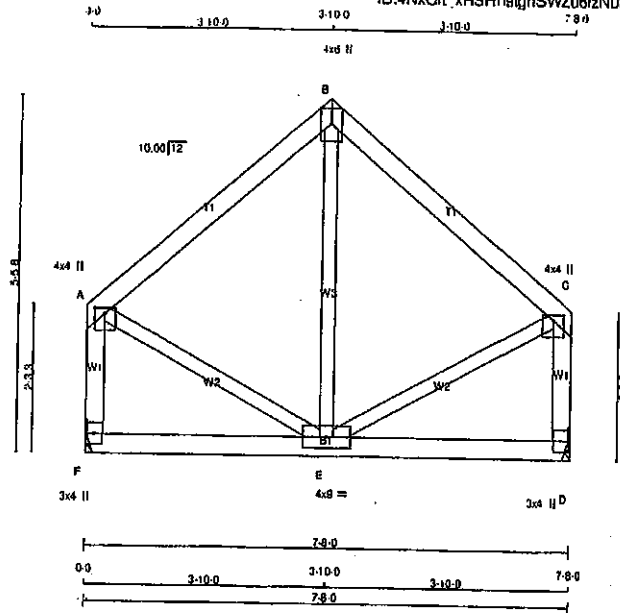


Structural component only
DWG# T-2007074

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408150	T12	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:4NkGft_xHSHnatgnSWZu6izNuP6-59Xq1?cyEg?oiFQsAw788rjPP4ZbxCqUb_u0s8zNCnO

Scale = 1:325



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)			
JT	TYPE	PLATES	W LEN Y X
A	TMVW+p	MT20	4.0 4.0 1.00 2.00
B	TTW+p	MT20	4.0 6.0 Edge
C	TMVW+p	MT20	4.0 4.0 1.00 2.00
D	BMV1+p	MT20	3.0 4.0
E	BMVWW-I	MT20	4.0 9.0
F	BMV1+p	MT20	3.0 4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
F	423	0	423	0
D	423	0	423	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	PERMA LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE			
F	299	196 / 0	0 / 0	0 / 0	103 / 0	0 / 0
D	299	196 / 0	0 / 0	0 / 0	103 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-214 / 0	-91.8 -91.8 0.17 (1)	8.25	E-B	-78 / 37	0.03 (1)	
B-C	-214 / 0	-91.8 -91.8 0.17 (1)	8.25	A-E	0 / 184	0.04 (1)	
F-A	-395 / 0	0.0 0.0 0.05 (1)	7.81	E-C	0 / 184	0.04 (1)	
D-C	-395 / 0	0.0 0.0 0.05 (1)	7.81				
F-E	0 / 0	-18.5 -18.5 0.08 (4)	10.00				
E-D	0 / 0	-18.5 -18.5 0.08 (4)	10.00				

TOTAL WEIGHT = 35 lb

[M/F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/380 (0.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.17/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.04/1.00 (A-E:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788

PLATE PLACEMENT TOL. = 0.250 inches

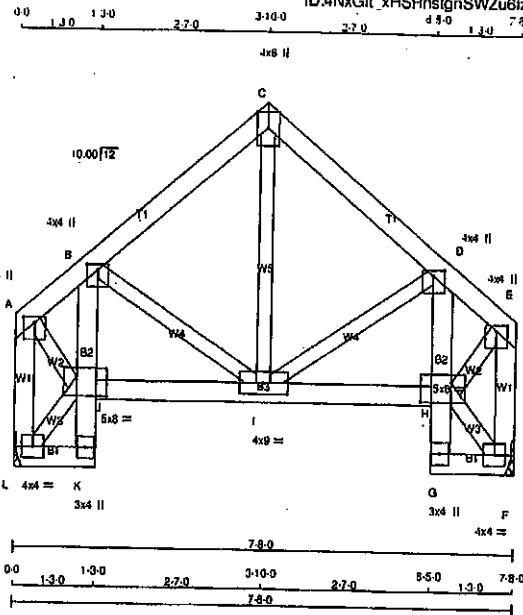
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.31 (C) (INPUT = 0.90)
JSI METAL=0.09 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007075

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408150	T12S	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
L - A	2x4 DRY	No.2	SPF
F - E	2x4 DRY	No.2	SPF
L - K	2x4 DRY	No.2	SPF
K - B	2x4 DRY	No.2	SPF
J - H	2x4 DRY	No.2	SPF
G - D	2x4 DRY	No.2	SPF
G - F	2x4 DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A, B, D, E				
A	TMVW+p	MT20	4.0	4.0 1.00 2.00
C	TTW+p	MT20	4.0	8.0 Edge
F	BMVW1-i	MT20	4.0	4.0
G	BMV+p	MT20	3.0	4.0
H	BMVWW-i	MT20	5.0	8.0 3.50 5.50
I	BMVWW-i	MT20	4.0	9.0
J	BMVWW-i	MT20	5.0	8.0 3.50 5.50
K	BMV+p	MT20	3.0	4.0
L	BMVW1-i	MT20	4.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	HORZ	UPLIFT
L 423	0	423	0
F 423	0	423	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L, F. MINIMUM BEARING LENGTH AT JOINT L = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS						
JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
L	299	196 / 0	0 / 0	0 / 0	0 / 0	103 / 0
F	299	196 / 0	0 / 0	0 / 0	0 / 0	103 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNSPACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRAC LENGTH (FT)
FR-TO			
A-B	-282 / 0	-91.8 -91.8 0.06 (1)	8.25
B-C	-274 / 0	-91.8 -91.8 0.07 (1)	8.25
C-D	-274 / 0	-91.8 -91.8 0.07 (1)	8.25
D-E	-282 / 0	-91.8 -91.8 0.06 (1)	8.25
L-A	-404 / 0	0.0 0.0 0.06 (1)	7.81
F-E	-404 / 0	0.0 0.0 0.06 (1)	7.81
L-K	0 / 9	-18.5 -18.5 0.01 (4)	10.00
K-J	0 / 11	0.0 0.0 0.01 (1)	10.00
J-B	-182 / 0	0.0 0.0 0.01 (1)	7.81
J-I	0 / 247	-18.5 -18.5 0.06 (1)	10.00
I-H	0 / 247	-18.5 -18.5 0.06 (1)	10.00
G-H	0 / 11	0.0 0.0 0.01 (1)	10.00
H-D	-182 / 0	0.0 0.0 0.01 (1)	7.81
G-F	0 / 9	-18.5 -18.5 0.01 (4)	10.00

WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH (FT)	CS (LC)
FR-TO			
I-C	0 / 125	0.03 (1)	
I-D	-57 / 0	0.01 (1)	
B-I	-57 / 0	0.01 (1)	
L-J	-13 / 0	0.00 (1)	
A-J	0 / 317	0.07 (1)	
H-F	-13 / 0	0.00 (1)	
H-E	0 / 317	0.07 (1)	

TOTAL WEIGHT = 2 X 43 = 86 lb (MIF)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/899 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/899 (0.01")

CSI: TC=0.07/1.00 (C-D:1), BC=0.06/1.00 (H-I:1), WB=0.07/1.00 (A-J:1), SS=0.09/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

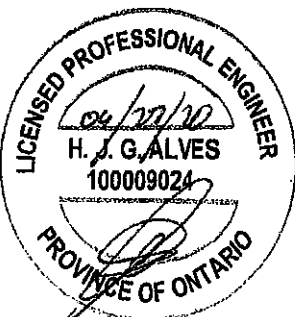
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR	SECTION	
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 768	1587 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

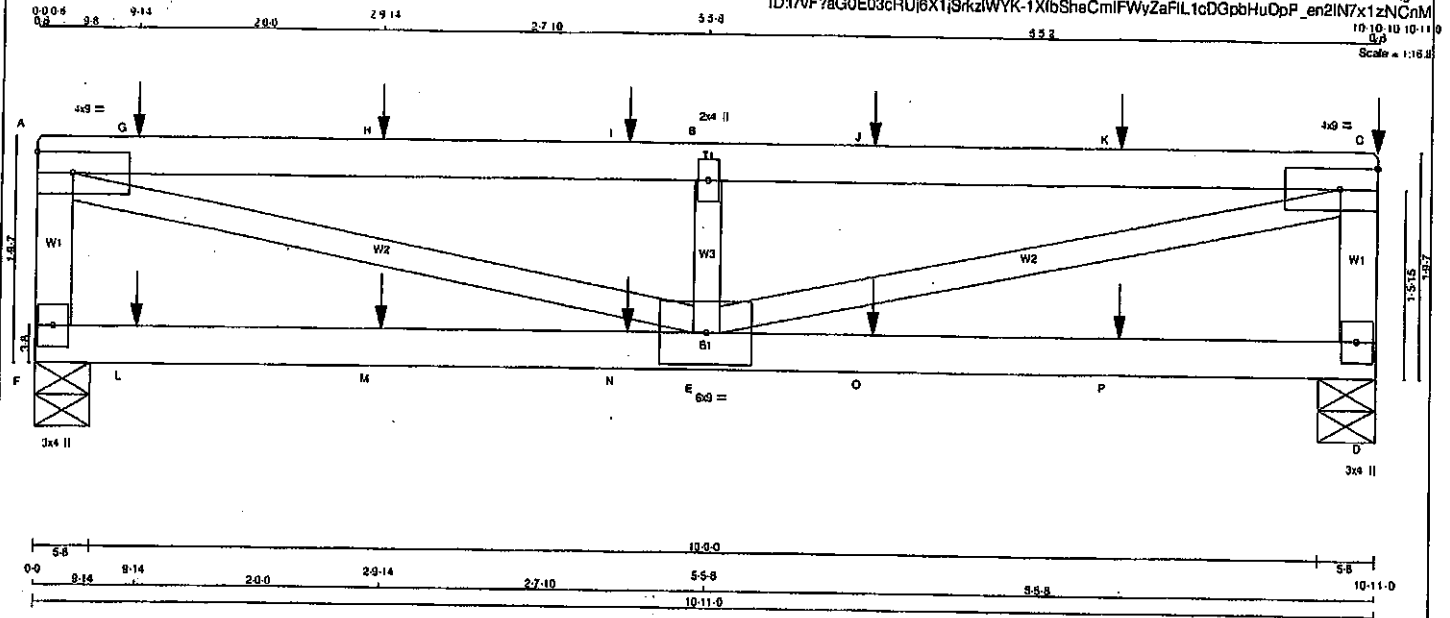
JSI GRIP= 0.30 (E) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)



Structural component only
DWG# T-2007076

JOB NAME 408150	TRUSS NAME T15	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2 EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-I	MT20	4.0	9.0		Edge
B	TMVW+V	MT20	2.0	4.0		
C	TMVW-I	MT20	4.0	9.0		Edge
D	BMV1+P	MT20	3.0	4.0		
E	BMVWW-I	MT20	6.0	9.0		
F	BMV1+P	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	976	0	976	0	0	5-8	5-8
D	1015	0	1015	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS	DEAD	SOIL
F	890	453 / 0	0 / 0	0 / 0	0 / 0	237 / 0
D	716	479 / 0	0 / 0	0 / 0	0 / 0	237 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	MAX	CSI (L.C)
FR-TO		FROM TO	LENGTH	FR-TO			
A-G	-2081 / 0	-91.8	-91.8 0.79 (1)	A-E	0 / 2157	0.53 (1)	
G-H	-2081 / 0	-91.8	-91.8 0.79 (1)	E-B	-976 / 0	0.14 (1)	
H-I	-2081 / 0	-91.8	-91.8 0.79 (1)	E-C	0 / 2157	0.53 (1)	
I-B	-2081 / 0	-91.8	-91.8 0.79 (1)				
B-J	-2081 / 0	-91.8	-91.8 0.79 (1)				
J-K	-2081 / 0	-91.8	-91.8 0.79 (1)				
K-C	-2081 / 0	-91.8	-91.8 0.79 (1)				
F-A	-869 / 0	0.0	0.0 0.10 (1)				
D-C	-938 / 0	0.0	0.0 0.11 (1)				
F-L	0 / 0	-18.5	-18.5 0.22 (4)				
L-M	0 / 0	-18.5	-18.5 0.22 (4)				
M-N	0 / 0	-18.5	-18.5 0.22 (4)				
N-E	0 / 0	-18.5	-18.5 0.22 (4)				
E-O	0 / 0	-18.5	-18.5 0.22 (4)				
O-P	0 / 0	-18.5	-18.5 0.22 (4)				
P-D	0 / 0	-18.5	-18.5 0.22 (4)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	10-11-0	-123	-123	---	BACK	VERT	TOTAL	C1
G	9-14	-90	-90	---	BACK	VERT	TOTAL	C1
H	2-9-14	-82	-82	---	BACK	VERT	TOTAL	C1
I	4-9-14	-82	-82	---	BACK	VERT	TOTAL	C1
J	6-9-14	-82	-82	---	BACK	VERT	TOTAL	C1
K	8-9-14	-82	-82	---	BACK	VERT	TOTAL	C1
L	9-14	-51	-51	---	BACK	VERT	TOTAL	C1
M	2-9-14	-49	-49	---	BACK	VERT	TOTAL	C1
N	4-9-14	-49	-49	---	BACK	VERT	TOTAL	C1
O	6-9-14	-49	-49	---	BACK	VERT	TOTAL	C1
P	8-9-14	-49	-49	---	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 37 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TRIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/817 (0.18")

CSI: TC=0.79/1.00 (A-B:1), BC=0.22/1.00 (E-F:4),
WB=0.53/1.00 (A-E:1), SS=0.38/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

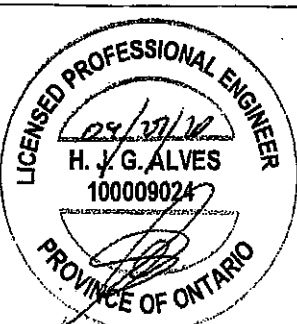
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.89 (A) (INPUT = 0.90)
JSI METAL=0.40 (E) (INPUT = 1.00)



Structural component only
DWG# T-2007077

DRWG NO.



JOB NAME 408151	TRUSS NAME T20	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310.5 Oct 29 2019 Mitek Industries, Inc. Sat Apr 26 11:19:44 2020 Page 2	
				ID:h2dflgC44wS E2zqPs atzIRw-E6e?Xz51rNUYObeR4VLHmmlFOqOSi27EXMbZWzNCX	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	7.0	8.0		
C	TTWW+m	MT20	8.0	9.0	3.50	1.75
D	TMWW-l	MT20	5.0	6.0	2.50	2.50
E	TMWW+w	MT20	3.0	6.0		
F	TS-l	MT20	5.0	6.0		
G	TMWW-l	MT20	5.0	6.0	2.50	2.50
H	TTWW+m	MT20	8.0	9.0	3.50	1.75
I	TMWW+p	MT20	7.0	8.0		
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-l	MT20	5.0	6.0	2.50	2.25
M	BMWW-l	MT20	5.0	6.0	2.50	3.00
N	BS-l	MT20	6.0	9.0		
O	BMWW-l	MT20	5.0	12.0		
P	BS-l	MT20	6.0	9.0		
Q	BMWW-l	MT20	5.0	6.0	2.50	3.00
R	BMWW-l	MT20	5.0	6.0	2.50	2.25
S	BMV1+p	MT20	3.0	6.0		

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH FR-TO
FR-TO		FROM TO					
M-AS	0/2579	-18.5	-18.5 0.16 (1)	10.00			
AS-AT	0/2579	-18.5	-18.5 0.16 (1)	10.00			
AT-AU	0/2579	-18.5	-18.5 0.16 (1)	10.00			
AU-L	0/2579	-18.5	-18.5 0.16 (1)	10.00			
L-AV	0/0	-18.5	-18.5 0.08 (4)	10.00			
AV-K	0/0	-18.5	-18.5 0.08 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-11-8	-39	-42	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-191	-191	---	FRONT	VERT	SNOW	---	C1
H	32-2-8	-39	-42	---	FRONT	VERT	DEAD	---	C1
H	32-2-8	-116	-116	---	FRONT	VERT	TOTAL	---	C1
H	32-2-8	-181	-191	---	FRONT	VERT	SNOW	---	C1
L	31-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	11-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
T	3-11-4	-111	-111	---	FRONT	VERT	TOTAL	---	C1
U	5-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	11-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	15-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	17-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	19-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	21-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	23-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AE	25-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AF	27-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AG	29-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AH	1-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AI	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AJ	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AK	7-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AL	9-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AM	13-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AN	15-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AO	17-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AP	19-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AQ	21-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AR	23-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AS	25-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AT	27-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AU	29-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AV	33-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1

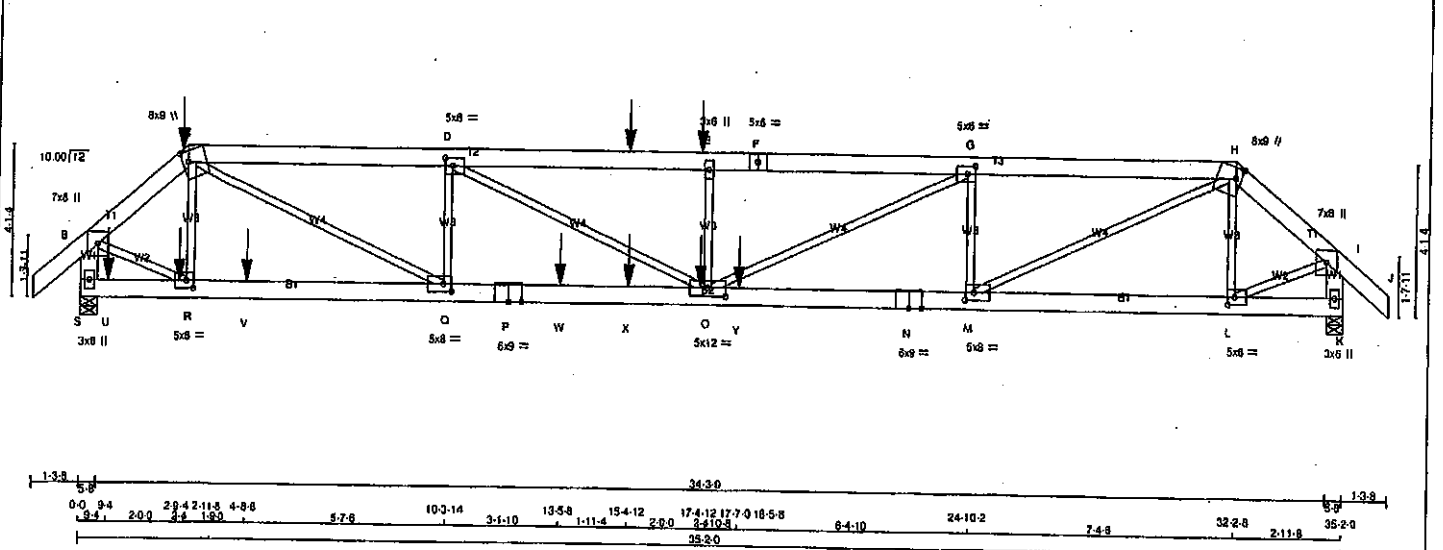
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007081 3/1

JOB NAME 408151	TRUSS NAME T20Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 11:19:45 2020 Page 1 ID:h2dflgC44ws E2t2qPs abzllRw-iJCOLmko9VL9YAq n0aqaJuZo4EB5KGTB585yzNCX	



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x8 DRY	No.2	SPF
C - F	2x6 DRY	No.2	SPF
F - H	2x6 DRY	No.2	SPF
H - J	2x6 DRY	No.2	SPF
S - B	2x8 DRY	No.2	SPF
K - I	2x6 DRY	No.2	SPF
S - P	2x6 DRY	1850F 1.5E	SPF
P - N	2x6 DRY	1850F 1.5E	SPF
N - K	2x6 DRY	1850F 1.5E	SPF
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF)

TOP CHORDS: (0.122"x3") SPIRAL NAILS

A-C 2 12 SIDE (122.0)

C-F 2 12 SIDE (183.1)

F-H 2 12 TOP

H-J 2 12 TOP

S-B 2 12 TOP

K-I 2 12 TOP

BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS

S-P 2 12 SIDE (183.1)

P-N 2 12 SIDE (183.1)

N-K 2 12 TOP

WEBS: (0.122"x3") SPIRAL NAILS

2x3 1 6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
S 4991	0	4991	0	0	5-8	5-8	
K 3694	0	3694	0	0	5-8	5-8	

UNFACTORED REACTIONS

1ST LOASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED SNOW	LIVE PERM. LIVE WIND DEAD SOIL
S 3528 2323 / 0	0 / 0 0 / 0 1208 / 0 0 / 0
K 2608 1737 / 0	0 / 0 0 / 0 871 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)	
CHORDS	FACTORED	WEBS	FACTORED
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 42	R-C	-253 / 44
B-C	-5342 / 0	C-D	0 / 8589
C-D	-9871 / 0	D-E	-1958 / 0
D-T	-12379 / 0	E-F	0 / 2694
T-E	-12379 / 0	F-G	-782 / 0
E-F	-12379 / 0	G-H	0 / 4229
F-G	-12379 / 0	H-I	-2619 / 0
G-H	-8598 / 0	I-J	0 / 8416
H-I	-3750 / 0	J-K	-864 / 0
I-J	0 / 42	K-L	0 / 4341
J-K	-5197 / 0	L-I	0 / 3047
K-L	-3727 / 0		
S-U	0 / 0		
U-R	0 / 0		
R-V	0 / 4073		
V-Q	0 / 4073		
Q-P	0 / 9971		
P-W	0 / 9971		
W-X	0 / 9971		
X-O	0 / 9971		
O-Y	0 / 8800		
Y-N	0 / 8800		
N-M	0 / 8800		
M-L	0 / 2839		
L-K	0 / 0		

FACTORED CONCENTRATED LOADS (LBS)		FACE		TYPE		HEEL		CONN.	
JT	LOC.	LOC1	MAX.	MAX+	FRONT	VERT	DEAD		
C	2-11-8	-38	-42		BACK	VERT	TOTAL		
C	2-11-8	-117	-117		BACK	VERT	TOTAL		
C	2-11-8	-191	-191		FRONT	VERT	TOTAL		
E	17-4-12	-110	-110		BACK	VERT	TOTAL		
O	17-4-12	-28	-28		BACK	VERT	TOTAL		
R	2-9-4	-28	-28		BACK	VERT	TOTAL		
T	15-4-12	-110	-110		BACK	VERT	TOTAL		
U	9-4	-29	-29		BACK	VERT	TOTAL		
V	4-8-8	-1068	-1068		BACK	VERT	TOTAL		
W	13-5-8	-1068	-1068		BACK	VERT	TOTAL		
X	15-4-12	-28	-28		BACK	VERT	TOTAL		
Y	18-5-8	-1738	-1738		BACK	VERT	TOTAL		

TOTAL WEIGHT = 2 X 183 = 365 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT

NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.0/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 088-09, CSA 088-14

- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")

CALCULATED VERT. DEFL.(LL) = L/998 (0.33")

ALLOWABLE DEFL.(TL) = L/360 (1.17")

CALCULATED VERT. DEFL.(TL) = L/685 (0.82")

CSI: TC=0.52/1.00 (D-E:1), BC=0.73/1.00 (D-Q:1), WB=0.81/1.00 (C-Q:1), SSI=0.57/1.00 (M-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

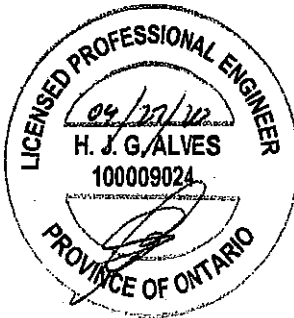
MT20 618 354 1887 708 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.88 (D) (INPUT = 0.90)

JSI METAL=0.84 (P) (INPUT = 1.00)



Structural component only
DWG# T-2007082 42

JOB NAME 408151	TRUSS NAME T20Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

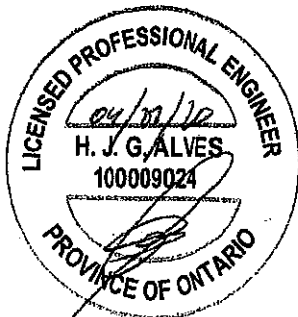
Version 8.310 S Oct 29 2019 Mitok Industries, Inc. Sat Apr 25 11:19:45 2020 Page 2
ID:h2dfiqC44wS E2lzoPa atzllRw-UCOUmko9VL9YAq n0agaJuZ04EB5KGT8585yzNCX

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	7.0	8.0		
C	TTWW+m	MT20	8.0	9.0	3.50	1.75
D	TMWW-i	MT20	5.0	6.0	2.50	2.50
E	TMW+w	MT20	3.0	6.0		
F	TS-i	MT20	5.0	6.0		
G	TMWW-i	MT20	5.0	6.0	2.50	2.50
H	TTWW+m	MT20	8.0	9.0	3.50	1.75
I	TMVW+p	MT20	7.0	8.0		
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-i	MT20	5.0	6.0	2.50	2.25
M	BMWW-i	MT20	5.0	6.0	2.50	2.75
N	BS-i	MT20	6.0	9.0		
O	BMWWW-i	MT20	5.0	12.0	2.75	6.00
P	BS-i	MT20	6.0	9.0		
Q	BMWW-i	MT20	5.0	6.0	2.50	2.75
R	BMWW-i	MT20	5.0	6.0	2.50	2.25
S	BMV1+p	MT20	3.0	6.0		

CONNECTION REQUIREMENTS

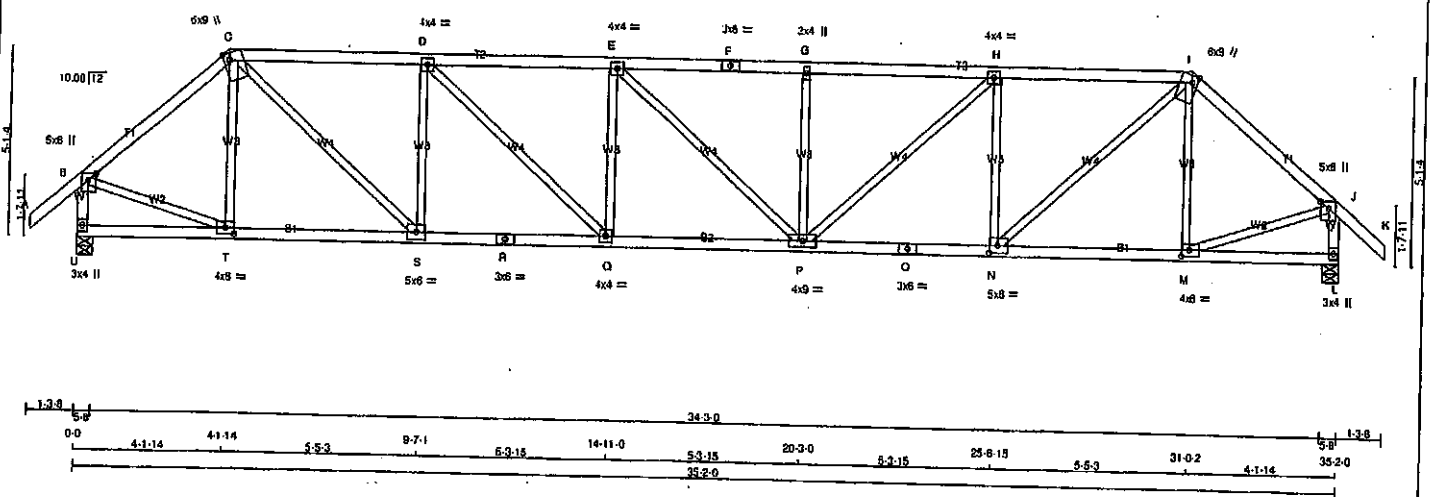
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007082 7/2

JOB NAME 408151	TRUSS NAME T21	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	ORWNGO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 ID:h2dlqC44wS_E2tqPs_atzIRw-AVImymMZTdCnliOYUXpMnr1ECRIweZQhridOzNCXh
 20-3-0 5-2-15 25-4-15 5-5-3 31-0-2 4-1-14 35-2-0
 Scale = 1:57.3



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - G	2x4	DRY No.2
C - F	2x4	DRY No.2
F - I	2x4	DRY No.2
I - K	2x4	DRY No.2
U - B	2x4	DRY No.2
L - J	2x4	DRY No.2
U - R	2x4	DRY No.2
R - O	2x4	DRY No.2
O - L	2x4	DRY No.2
ALL WEBS	2x3	DRY No.2
EXCEPT		

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWV+p	MT20	5.0	6.0	Edge	2.75
C TTWV+m	MT20	6.0	9.0	Edge	1.75
D, E, H					
D TMWV-i	MT20	4.0	4.0		
F TS-i	MT20	3.0	6.0		
G TMWV-w	MT20	2.0	4.0		
I TTWV+m	MT20	6.0	9.0	Edge	1.75
J TMWV+p	MT20	5.0	6.0	Edge	2.75
L BMV1+p	MT20	3.0	4.0		
M BMWV-i	MT20	4.0	6.0	2.00	2.75
N BMWV-i	MT20	5.0	6.0	2.50	2.75
O BS-i	MT20	3.0	6.0		
P BMWVW-i	MT20	4.0	9.0		
R BS-i	MT20	4.0	4.0		
S BMWV-i	MT20	3.0	6.0		
T BMWV-i	MT20	5.0	6.0		
U BMV1+p	MT20	4.0	6.0	2.00	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
U	2086	0	2086	0	5-8	5-8
L	2086	0	2086	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
U	1458	971/0	0/0	0/0	0/0	488/0	0/0
L	1458	971/0	0/0	0/0	0/0	488/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS		MAX. UNBRACED LENGTH
	MAX. FACTORED	FORCE (LBS)	VERT. LOAD	LC1 MAX			MAX. FACTORED	FORCE (LBS)	
FR-TO						FR-TO			
A-B	0/41	-91.8	-91.8	0.13 (1)	10.00	T-C	-358/0	0.14 (1)	
B-C	-1960/0	-91.8	-91.8	0.36 (1)	4.48	C-S	0/1836	0.41 (1)	
C-D	-2871/0	-91.8	-91.8	0.57 (1)	3.59	S-D	-1118/0	0.43 (1)	
D-E	-3506/0	-91.8	-91.8	0.85 (1)	3.21	D-Q	0/856	0.19 (1)	
E-F	-3505/0	-91.8	-91.8	0.53 (1)	3.36	Q-E	-475/0	0.18 (1)	
F-G	-3505/0	-91.8	-91.8	0.53 (1)	3.36	E-P	-2/0	0.00 (1)	
G-H	-3505/0	-91.8	-91.8	0.66 (1)	3.20	P-G	-474/0	0.18 (1)	
H-I	-2871/0	-91.8	-91.8	0.57 (1)	3.59	P-H	0/853	0.19 (1)	
I-J	-1960/0	-91.8	-91.8	0.36 (1)	4.48	H-I	-1117/0	0.43 (1)	
J-K	0/41	-91.8	-91.8	0.13 (1)	10.00	N-I	0/1837	0.41 (1)	
U-B	-2037/0	0.0	0.0	0.22 (1)	5.92	M-I	-358/0	0.14 (1)	
L-J	-2037/0	0.0	0.0	0.22 (1)	5.92	B-T	0/1571	0.35 (1)	
U-T	0/0	-18.5	-18.5	0.10 (4)	10.00	M-J	0/1571	0.35 (1)	
T-S	0/1494	-18.5	-18.5	0.30 (1)	10.00				
S-R	0/2871	-18.5	-18.5	0.51 (1)	10.00				
R-Q	0/2871	-18.5	-18.5	0.51 (1)	10.00				
Q-P	0/3506	-18.5	-18.5	0.82 (1)	10.00				
P-O	0/2872	-18.5	-18.5	0.51 (1)	10.00				
O-N	0/2872	-18.5	-18.5	0.51 (1)	10.00				
N-M	0/1494	-18.5	-18.5	0.30 (1)	10.00				
M-L	0/0	-18.5	-18.5	0.10 (4)	10.00				

TOTAL WEIGHT = 2 X 147 = 293 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2018 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")

CALCULATED VERT. DEFL.(LL) = L/999 (0.20")

ALLOWABLE DEFL.(TL) = L/360 (1.17")

CALCULATED VERT. DEFL.(TL) = L/999 (0.39")

CSI: TC=0.86/1.00 (G-H:1), BC=0.62/1.00 (P-Q:1), WB=0.43/1.00 (D-S:1), SS=0.23/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

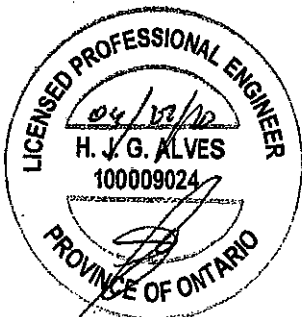
PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	364	1667	788	1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

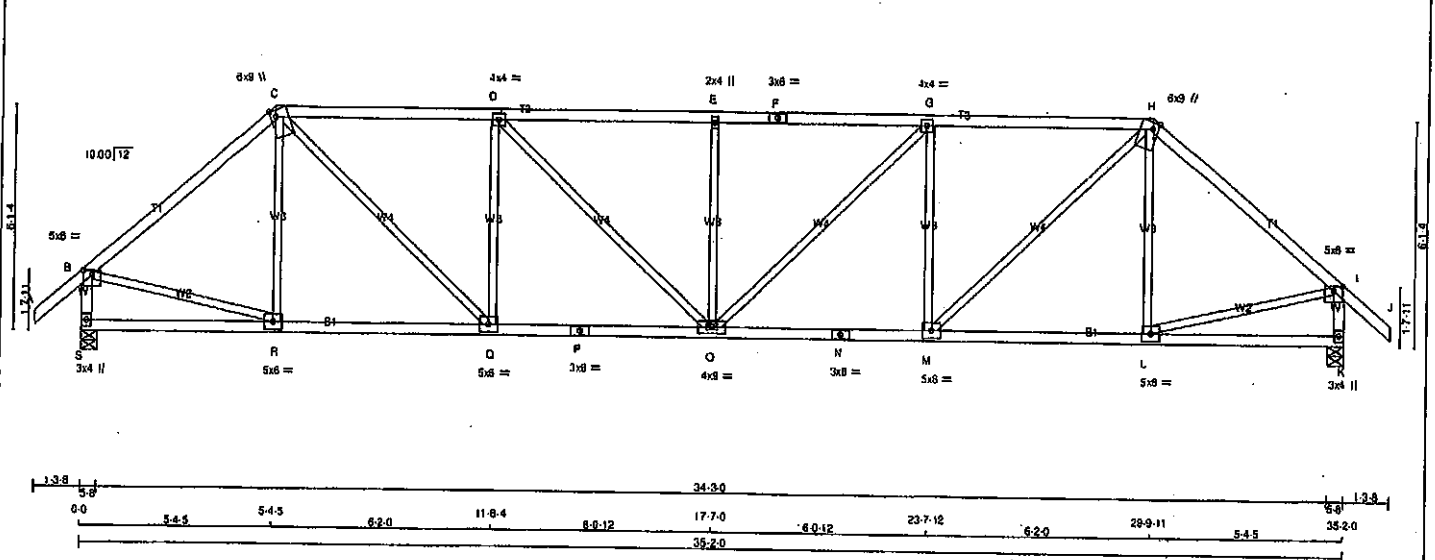
JSI GRIP = 0.90 (S) (INPUT = 0.90)

JSI METAL = 0.90 (Q) (INPUT = 1.00)



Structural component only
 DWG# T-2007083

JOB NAME 408151	TRUSS NAME T22	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 11:19:47 2020 Page 1 ID:h2dlqC44wS_E2tqPe_atzllRw-ehJ897n_Kml3PrKD6C22v?OAzcpWl3vZwVaFArZNCXq	



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
S - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
S - P	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMVW-p	MT20	5.0	8.0	1.50 3.00
C TTWW-m	MT20	8.0	9.0	Edge 1.75
D TMVW-t	MT20	4.0	4.0	
E TMVW-w	MT20	2.0	4.0	
F TS-t	MT20	3.0	8.0	
G TMVW-t	MT20	4.0	4.0	
H TTWW-m	MT20	8.0	9.0	Edge 1.75
I TMVW-p	MT20	5.0	8.0	1.50 3.00
K BMV1-p	MT20	3.0	4.0	
L, M, Q, R				
L BMVW-t	MT20	5.0	8.0	
N BS-t	MT20	3.0	8.0	
O BMVW-t	MT20	4.0	4.0	
P BS-t	MT20	3.0	8.0	
S BMV1-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORIZ	DOWN	HORIZ	BRG	IN-SX	BRG	IN-SX
S	2088	0	0	2088	0	0	5-8	5-8	5-8
K	2088	0	0	2088	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
S	1458	971/0	0/0	0/0	0/0
K	1458	971/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0/41	R-C	-246/7
B-C	-2004/0	C-Q	0/1538
C-D	-2853/0	Q-D	-943/0
D-E	-2978/0	D-O	0/450
E-F	-2978/0	O-E	-513/0
F-G	-2978/0	G-O	0/450
G-H	-2853/0	M-G	-943/0
H-I	-2004/0	M-H	0/1538
I-J	0/41	L-H	-246/7
S-B	-2028/0	B-R	0/1580
K-I	-2028/0	L-I	0/1580
S-R	0/0		
R-Q	0/1532		
Q-P	0/2853		
P-O	0/2853		
O-N	0/2853		
N-M	0/2853		
M-L	0/1532		
L-K	0/0		

TOTAL WEIGHT = 2 X 148 = 296 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/999 (0.16")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = L/999 (0.29")

CSL: TC=0.78/1.00 (D-E:1), BC=0.49/1.00 (O-Q:1), WS=0.55/1.00 (G-M:1), SS=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 -818 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (B) (INPUT = 0.90)
JSI METAL = 0.85 (N) (INPUT = 1.00)

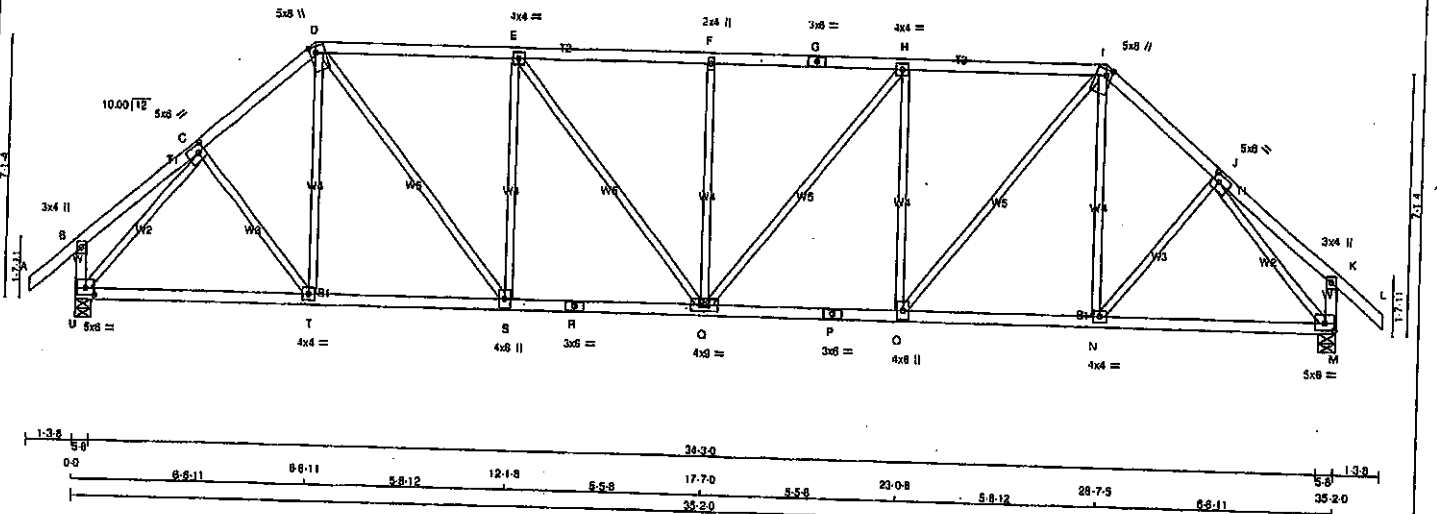


Structural component only
DWG# T-2007084

JOB NAME: 408151 TRUSS NAME: T23 QUANTITY: 2 PLY: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC: Tamarack Roof Truss, Burlington

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ID:h2dflgC44wS_E2t2qPs_atzIRw-6uWVKoc54w07vPgVZHSCwP40ArOPU189KoH2NCX

Scale: 1/8" = 1'-0"



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
H - I	2x4	DRY No.2	SPF
J - L	2x4	DRY No.2	SPF
M - K	2x4	DRY No.2	SPF
N - R	2x4	DRY No.2	SPF
S - P	2x4	DRY No.2	SPF
Q - M	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+1	MT20	5.0	8.0	2.50	2.25
D	TTVW+m	MT20	5.0	8.0	2.00	1.75
E	TMVW+1	MT20	4.0	4.0		
F	TMVW+w	MT20	2.0	4.0		
G	TS-1	MT20	3.0	6.0		
H	TMVW+1	MT20	4.0	4.0		
I	TTVW+m	MT20	5.0	8.0	2.00	1.75
J	TMVW+1	MT20	5.0	8.0	2.50	2.25
K	TMV+p	MT20	3.0	4.0		
M	BMVW+1	MT20	5.0	8.0	2.25	3.00
N	BMVW+1	MT20	4.0	4.0		
O	BMVW+1	MT20	4.0	8.0		
P	BS-1	MT20	3.0	6.0		
Q	BMVW+1	MT20	4.0	9.0		
R	BS-1	MT20	3.0	6.0		
S	BMVW+1	MT20	4.0	8.0		
T	BMVW+1	MT20	4.0	4.0		
U	BMVW+1	MT20	5.0	6.0	2.25	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	2086	2086	0	5-8
U HORZ	0	0	0	5-8
M HORZ	0	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
U	1458	971/0
M	1458	971/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0/141	-91.8	0.13 (1)	C-T	0/156	0.04 (1)	
B-C	0/119	-91.8	0.14 (1)	D-S	0/178	0.03 (4)	
C-D	2023/0	-91.8	0.20 (1)	E-Q	0/1220	0.27 (1)	
D-E	2308/0	-91.8	0.55 (1)	F-R	-850/0	0.74 (1)	
E-F	-2534/0	-91.8	0.57 (1)	G-H	0/361	0.08 (1)	
F-G	-2534/0	-91.8	0.57 (1)	I-J	-461/0	0.40 (1)	
G-H	-2534/0	-91.8	0.57 (1)	J-K	0/361	0.08 (1)	
H-I	-2308/0	-91.8	0.55 (1)	K-L	-850/0	0.74 (1)	
I-J	-2023/0	-91.8	0.20 (1)	L-M	0/1220	0.27 (1)	
J-K	0/119	-91.8	0.14 (1)	N-I	0/178	0.03 (4)	
K-L	0/141	-91.8	0.13 (1)	O-N	0/156	0.04 (1)	
L-M	-248/0	0.0	0.03 (1)	P-O	-2277/0	0.98 (1)	
M-N	-248/0	0.0	0.03 (1)	Q-M	-2277/0	0.98 (1)	
N-O	0/1433	-18.5	-18.5 0.33 (1)				
O-P	0/1535	-18.5	-18.5 0.35 (1)				
P-Q	0/2308	-18.5	-18.5 0.42 (1)				
Q-R	0/2308	-18.5	-18.5 0.42 (1)				
R-S	0/2308	-18.5	-18.5 0.42 (1)				
S-T	0/2308	-18.5	-18.5 0.42 (1)				
T-U	0/1535	-18.5	-18.5 0.35 (1)				
U-V	0/1433	-18.5	-18.5 0.33 (1)				

TOTAL WEIGHT = 2 X 162 = 323 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = U/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = U/999 (0.23")

CSI: TC=0.57/1.00 (E-F:1), BC=0.42/1.00 (O-Q:1),
WB=0.98/1.00 (J-K:1), SS=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

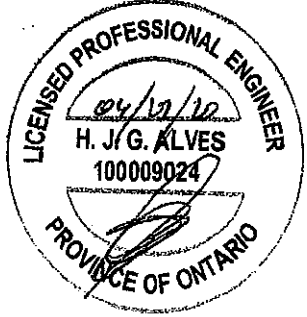
MT20 618 354 1667 788 1987 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (I) (INPUT = 0.90)

JSI METAL = 0.72 (P) (INPUT = 1.00)

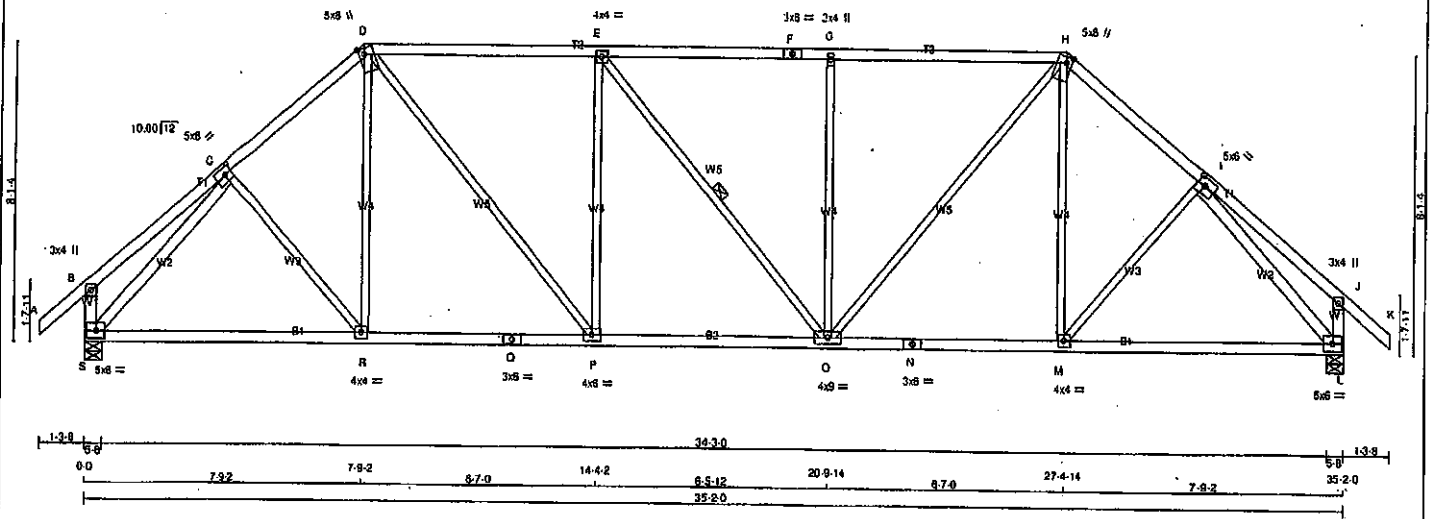


Structural component only
DWG# T-2007085

JOB NAME 408151	TRUSS NAME T24	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 ID:h2dlfgC44ws E2tqPs_atzllRw-a4RuagpEsO?ne9UbDd4W_QTXEPW57sssNp3MEzNCXk

Scale = 1/8" = 1'-0"



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHOROS			
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - K	2x4	DRY No.2	SPF
S - B	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
S - Q	2x4	DRY No.2	SPF
Q - N	2x4	DRY No.2	SPF
N - L	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
S - C	2x4	DRY No.2	SPF
I - L	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV-p	MT20	3.0	4.0	
C	TMWW-1	MT20	5.0	8.0	2.50 2.50
D	TTWW-m	MT20	5.0	8.0	2.00 1.75
E	TMWW-1	MT20	4.0	4.0	
F	TS-t	MT20	3.0	6.0	
G	TMWW-w	MT20	2.0	4.0	
H	TTWW-m	MT20	5.0	8.0	2.00 1.75
I	TMWW-1	MT20	5.0	8.0	2.50 2.50
J	TMV-p	MT20	3.0	4.0	
L	BMWW-1	MT20	5.0	8.0	
M	BMWW-1	MT20	4.0	4.0	
N	BS-1	MT20	3.0	8.0	
O	BMWW-1	MT20	4.0	8.0	
P	BMWW-1	MT20	4.0	8.0	
Q	BS-t	MT20	3.0	6.0	
R	BMWW-1	MT20	4.0	4.0	
S	BMWW-1	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
S 2066	0	2066	0	0	5-8	5-8	
L 2066	0	2066	0	0	5-8	5-8	

UNFACTORED REACTIONS

1ST CASE		MAX/MIN COMPONENT REACTIONS					
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
S 1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0	
L 1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0	

BEARING MATERIAL TO BE 6E SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LOI MAX	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX
FR-TO		FROM TO	CSI (LC)		FR-TO		CSI (LC)
A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00	C-R	0 / 64	0.02 (4)
B-C	0 / 26	-91.8	-91.8 0.21 (1)	10.00	R-D	0 / 126	0.04 (4)
C-D	-2006 / 0	-91.8	-91.8 0.29 (1)	4.50	D-P	0 / 998	0.22 (1)
D-E	-2163 / 0	-91.8	-91.8 0.73 (1)	3.81	P-E	-850 / 0	0.82 (1)
E-F	-2162 / 0	-91.8	-91.8 0.73 (1)	3.81	E-O	-2 / 0	0.00 (1)
F-G	-2162 / 0	-91.8	-91.8 0.73 (1)	3.81	O-G	-849 / 0	0.82 (1)
G-H	-2162 / 0	-91.8	-91.8 0.73 (1)	3.82	O-H	0 / 998	0.22 (1)
H-I	-2006 / 0	-91.8	-91.8 0.29 (1)	4.50	M-H	0 / 126	0.04 (4)
I-J	0 / 26	-91.8	-91.8 0.21 (1)	10.00	M-I	0 / 84	0.02 (4)
J-K	0 / 41	-91.8	-91.8 0.13 (1)	10.00	S-C	-2291 / 0	0.87 (1)
S-B	-264 / 0	0.0	0.0 0.03 (1)	7.81	I-L	-2291 / 0	0.87 (1)
L-J	-264 / 0	0.0	0.0 0.03 (1)	7.81			
S-R	0 / 1487	-18.5	-18.5 0.38 (1)	10.00			
R-Q	0 / 1519	-18.5	-18.5 0.39 (1)	10.00			
Q-P	0 / 1519	-18.5	-18.5 0.39 (1)	10.00			
P-O	0 / 2163	-18.5	-18.5 0.42 (1)	10.00			
O-N	0 / 1519	-18.5	-18.5 0.39 (1)	10.00			
N-M	0 / 1519	-18.5	-18.5 0.39 (1)	10.00			
M-L	0 / 1487	-18.5	-18.5 0.38 (1)	10.00			

TOTAL WEIGHT = 2 X 163 = 326 LB

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.8 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, CBC 2012, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 088-09, CSA 088-14
 - TPIC 2011, TPIC 2014

65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (1.17")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
 ALLOWABLE DEFL.(TL) = L/380 (1.17")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.73/1.00 (D-E:1), BC=0.42/1.00 (O-P:1), WB=0.97/1.00 (I-L:1), SS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

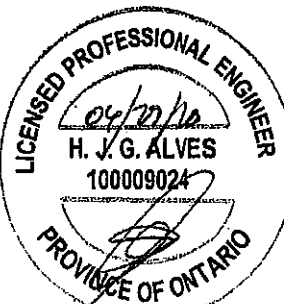
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

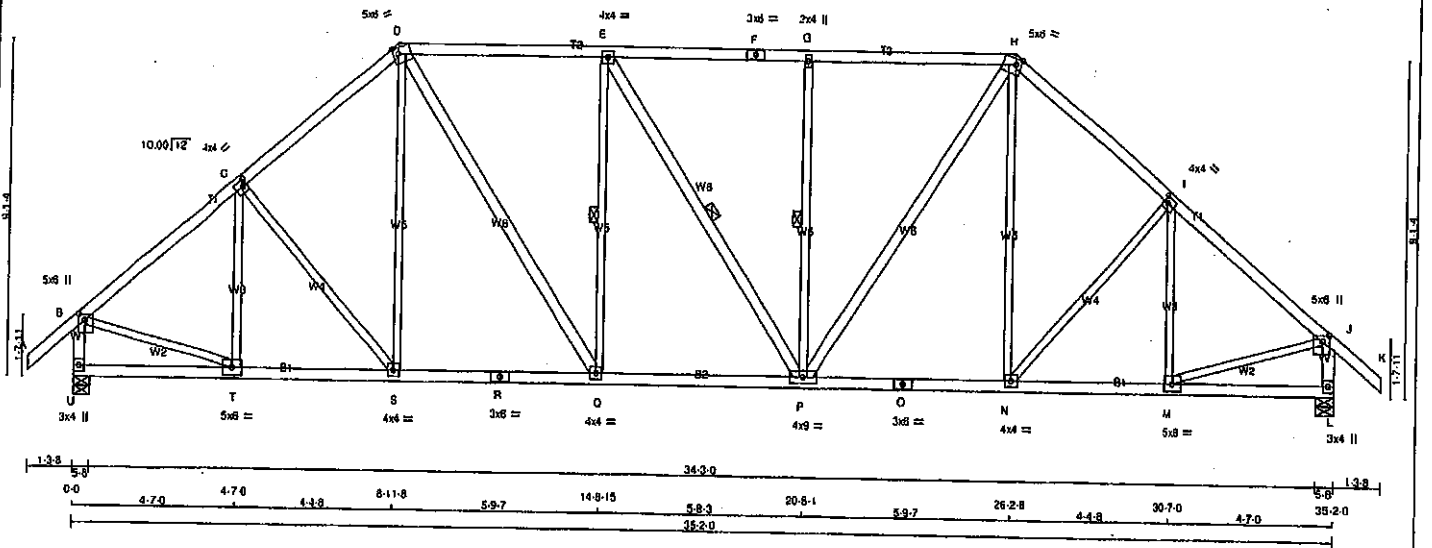
JSI GRIP=0.87 (I) (INPUT = 0.90)
 JSI METAL=0.56 (I) (INPUT = 1.00)



Structural component only
 DWG# T-2007086

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408151	T25	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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 ID:h2dflgC44ws_E2tqPs_atzllRw-2G7HoDpsdh7eGJ2nnKbXddn8pt_sTb7cTpvnm9zNCXc
 20.51 39.7 26.28 4.48 30.70 4.70 1.38
 Scale = 1:57.3



CHORDS	SIZE	LUMBER
A - D	2x4	DRY
D - F	2x4	DRY
F - H	2x4	DRY
H - K	2x4	DRY
U - B	2x4	DRY
L - J	2x4	DRY
U - R	2x4	DRY
R - O	2x4	DRY
O - L	2x4	DRY

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWW+p	MT20	5.0	8.0	2.00	2.00
C TMWW-I	MT20	4.0	4.0	2.00	1.25
D TTWW-m	MT20	5.0	6.0	2.00	1.75
E TMWW-I	MT20	4.0	4.0		
F TS-I	MT20	3.0	6.0		
G TMWW-w	MT20	2.0	4.0		
H TTWW-m	MT20	5.0	8.0	2.00	1.75
I TMWW-I	MT20	4.0	4.0	2.00	1.25
J TMWW+p	MT20	5.0	8.0	2.00	2.00
L BMW+p	MT20	3.0	4.0		
M BMWW-I	MT20	5.0	8.0		
N, Q, S					
N BMWW-I	MT20	4.0	4.0		
O BS-I	MT20	3.0	6.0		
P BMWW-I	MT20	4.0	4.0		
R BS-I	MT20	3.0	6.0		
T BMWW-I	MT20	5.0	8.0		
U BMW+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT VERT	2086	2086	0	5-8
U VERT	2086	2086	0	5-8
L VERT	2086	2086	0	5-8

UNFACTORED REACTIONS

1ST CASE	MAX/MIN COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM LIVE WIND DEAD SOIL
U	1458 971 / 0 0 / 0 0 / 0 488 / 0 0 / 0
L	1458 971 / 0 0 / 0 0 / 0 488 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.44 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, E-P, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 41	-91.8 -91.8	0.13 (1)	10.00	T-C	-357 / 0	0.18 (1)
B-C	-1994 / 0	-91.8 -91.8	0.30 (1)	4.83	C-S	-121 / 0	0.10 (1)
C-D	-1857 / 0	-91.8 -91.8	0.29 (1)	4.67	S-D	0 / 188	0.05 (4)
D-E	-1823 / 0	-91.8 -91.8	0.40 (1)	4.44	D-Q	0 / 810	0.13 (1)
E-F	-1921 / 0	-91.8 -91.8	0.37 (1)	4.49	Q-E	-569 / 0	0.31 (1)
F-G	-1921 / 0	-91.8 -91.8	0.37 (1)	4.49	E-P	-3 / 0	0.00 (1)
G-H	-1921 / 0	-91.8 -91.8	0.40 (1)	4.45	P-G	-568 / 0	0.31 (1)
H-I	-1957 / 0	-91.8 -91.8	0.29 (1)	4.57	P-H	0 / 807	0.13 (1)
I-J	-1894 / 0	-91.8 -91.8	0.30 (1)	4.53	N-H	0 / 189	0.05 (4)
J-K	0 / 41	-91.8 -91.8	0.13 (1)	10.00	N-I	-121 / 0	0.10 (1)
U-B	-2028 / 0	0.0 0.0	0.22 (1)	5.84	M-I	-358 / 0	0.18 (1)
L-J	-2028 / 0	0.0 0.0	0.22 (1)	5.84	G-T	0 / 1817	0.38 (1)
U-T	0 / 0	-18.5 -18.5	0.08 (4)	10.00	M-J	0 / 1817	0.38 (1)
T-S	0 / 1555	-18.5 -18.5	0.30 (1)	10.00			
S-R	0 / 1477	-18.5 -18.5	0.30 (1)	10.00			
R-Q	0 / 1477	-18.5 -18.5	0.30 (1)	10.00			
Q-P	0 / 1923	-18.5 -18.5	0.38 (1)	10.00			
P-O	0 / 1478	-18.5 -18.5	0.30 (1)	10.00			
O-N	0 / 1478	-18.5 -18.5	0.30 (1)	10.00			
N-M	0 / 1555	-18.5 -18.5	0.31 (1)	10.00			
M-L	0 / 0	-18.5 -18.5	0.08 (4)	10.00			

TOTAL WEIGHT = 2 X 182 = 363 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBC 2018, CBC 2012, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17)
 CALCULATED VERT. DEFL. (LL) = L/999 (0.08)
 ALLOWABLE DEFL. (TL) = L/360 (1.17)
 CALCULATED VERT. DEFL. (TL) = L/999 (0.18)
 CSI: TC=0.40/1.00 (D-E:1), BC=0.38/1.00 (P-Q:1), WB=0.38/1.00 (B-T:1), SS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 818 354 1867 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

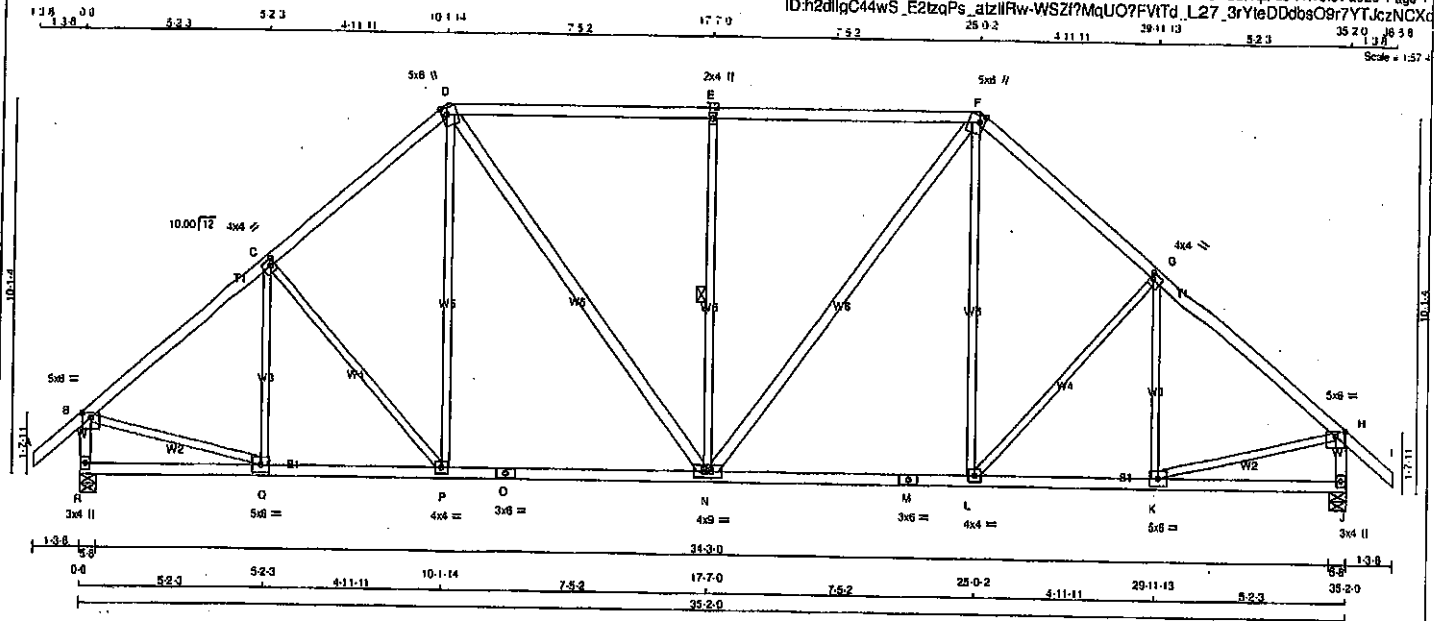
JSI GRIP = 0.89 (8) (INPUT = 0.80)
 JSI METAL = 0.50 (0) (INPUT = 1.00)



Structural component only
 DWG# T-2007087

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408151	T26	4	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:h2dlpC44wS_E2tqPs_atzllRw-WSZf?MqUO?FVtId_L27_3rYieDDdsO9r7YTJczNCXc



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
R - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
D - N	2x4	DRY	No.2	SPF	
N - F	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	6.0	1.50	3.00
C	TMWV-l	MT20	4.0	4.0	2.00	1.25
D	TMWV-m	MT20	5.0	6.0	2.25	1.50
E	TMWV-w	MT20	2.0	4.0		
F	TMWV-n	MT20	5.0	6.0	2.25	1.50
G	TMWV-l	MT20	4.0	4.0	2.00	1.25
H	TMWV-p	MT20	5.0	6.0	1.50	3.00
J	BMV-l-p	MT20	3.0	4.0		
K	BMWV-l	MT20	5.0	6.0		
L	BMWV-l	MT20	4.0	4.0		
M	BS-l	MT20	3.0	8.0		
N	BMWVW-l	MT20	4.0	9.0		
O	BS-l	MT20	3.0	6.0		
P	BMWV-l	MT20	4.0	4.0		
Q	BMWV-l	MT20	5.0	6.0		
R	BMV-l-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT	DOWN	HORZ	UPLIFT	IN-SX
R 2068	0	2068	0	5-8
J 2068	0	2068	0	5-8

UNFACTORED REACTIONS

1ST LOASE	MAX./MIN. COMPONENT REACTIONS						
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
R 1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0	
J 1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8 -91.8 0.13 (1)	10.00	Q-C	-302 / 0	0.17 (1)	
B-C	-2021 / 0	-91.8 -91.8 0.39 (1)	4.41	C-P	-217 / 0	0.25 (1)	
C-D	-1808 / 0	-91.8 -91.8 0.37 (1)	4.52	P-D	0 / 278	0.06 (1)	
D-E	-1808 / 0	-91.8 -91.8 0.74 (1)	3.88	D-N	0 / 815	0.10 (1)	
E-F	-1808 / 0	-91.8 -91.8 0.74 (1)	3.88	N-E	-838 / 0	0.58 (1)	
F-G	-1808 / 0	-91.8 -91.8 0.37 (1)	4.52	N-F	0 / 615	0.10 (1)	
G-H	-2021 / 0	-91.8 -91.8 0.39 (1)	4.41	L-F	0 / 278	0.06 (1)	
H-I	0 / 41	-91.8 -91.8 0.13 (1)	10.00	L-G	-217 / 0	0.25 (1)	
R-B	-2024 / 0	0.0 0.0 0.22 (1)	5.94	K-G	-302 / 0	0.17 (1)	
J-H	-2024 / 0	0.0 0.0 0.22 (1)	5.94	B-Q	0 / 1629	0.37 (1)	
R-Q	0 / 0	-18.5 -18.5 0.10 (4)	10.00	K-H	0 / 1629	0.37 (1)	
Q-P	0 / 1580	-18.5 -18.5 0.34 (1)	10.00				
P-O	0 / 1437	-18.5 -18.5 0.35 (1)	10.00				
O-N	0 / 1437	-18.5 -18.5 0.35 (1)	10.00				
N-M	0 / 1437	-18.5 -18.5 0.35 (1)	10.00				
M-L	0 / 1437	-18.5 -18.5 0.35 (1)	10.00				
L-K	0 / 1580	-18.5 -18.5 0.34 (1)	10.00				
K-J	0 / 0	-18.5 -18.5 0.10 (4)	10.00				

TOTAL WEIGHT = 4 X 173 = 693 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 8.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17)
CALCULATED VERT. DEFL.(LL) = L/999 (0.08)
ALLOWABLE DEFL.(TL) = L/360 (1.17)
CALCULATED VERT. DEFL.(TL) = L/999 (0.17)

CSI: TC=0.74/1.00 (D-E:1), BC=0.35/1.00 (L-N:1), WB=0.58/1.00 (E-N:1), SS=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

MAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1887 788	1887 1656

PLATE PLACEMENT TOL. = 0.250 inches

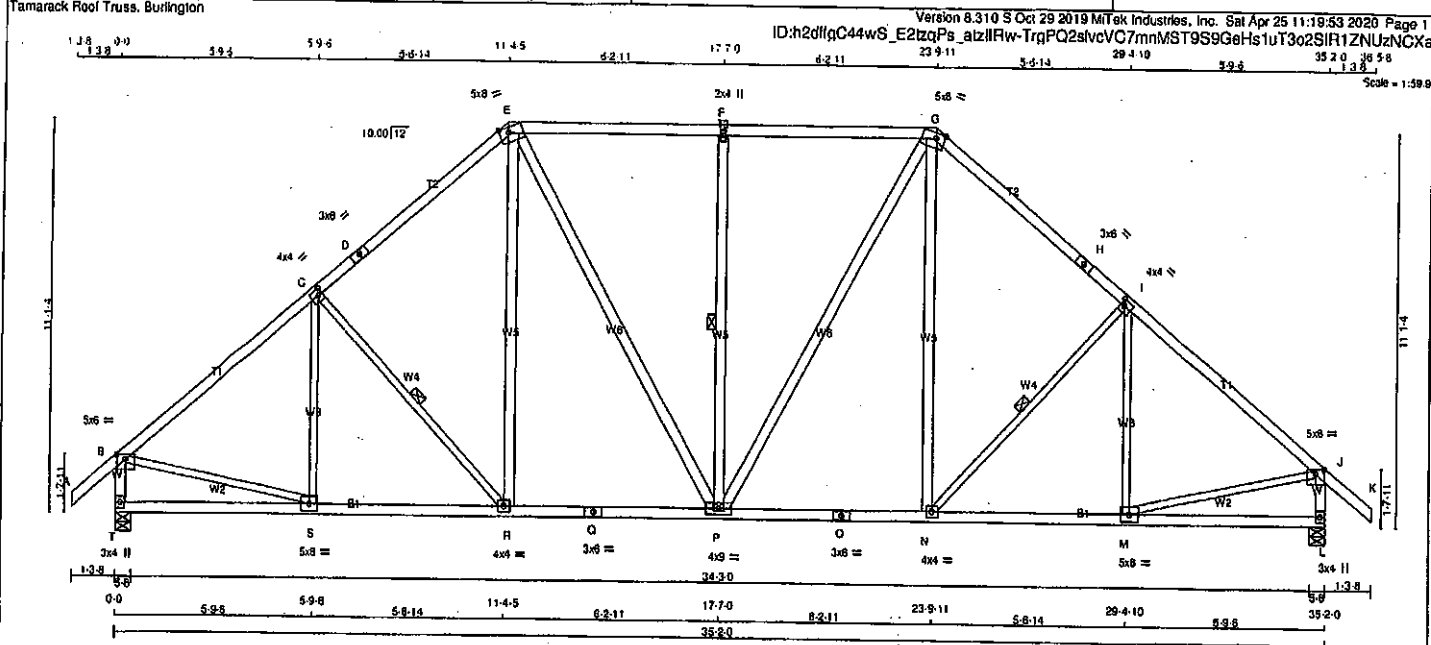
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.48 (M) (INPUT = 1.00)



Structural component only
DWG# T-2007088

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408151	T27	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
T - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
T - Q	2x4	DRY	No.2
Q - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS 2x4 DRY No.2			
EXCEPT			
S - C	2x3	DRY	No.2
C - R	2x3	DRY	No.2
N - I	2x3	DRY	No.2
M - I	2x3	DRY	No.2
B - S	2x3	DRY	No.2
M - J	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMWW-p	MT20	5.0	8.0 1.50 3.00
C	TMWW-p	MT20	4.0	4.0 2.00 1.25
D	TS-1	MT20	3.0	8.0
E	TTWW-m	MT20	5.0	8.0 Edge 3.00
F	TMWW-w	MT20	2.0	4.0
G	TTWW-m	MT20	5.0	8.0 Edge 3.00
H	TS-1	MT20	3.0	8.0
I	TMWW-1	MT20	4.0	4.0 2.00 1.25
J	TMWW-p	MT20	5.0	8.0 1.50 3.00
L	BMWW-1	MT20	3.0	4.0
M	BMWW-1	MT20	5.0	8.0
N	BMWW-1	MT20	4.0	4.0
O	BS-1	MT20	3.0	8.0
P	BMWWW-1	MT20	4.0	9.0
Q	BS-1	MT20	3.0	8.0
R	BMWW-1	MT20	4.0	4.0
S	BMWW-1	MT20	5.0	8.0
T	BMWW-1	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED	GROSS REACTION		MAXIMUM FACTORED		INPUT	REQD
		VERT	HORZ	DOWN	HORZ	UPLIFT	BRG
JT	2068	0	0	2068	0	0	5-8
T	2068	0	0	2068	0	0	5-8
L	2068	0	0	2068	0	0	5-8

UNFACTORED REACTIONS							
	1ST CASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
T	1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
L	1458	971 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-R, F-P, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8 -91.8 0.13 (1)	10.00	S-C	245 / 10	0.17 (1)	
B-C	-2039 / 0	-91.8 -91.8 0.50 (1)	4.27	C-H	-314 / 0	0.15 (1)	
C-D	-1844 / 0	-91.8 -91.8 0.48 (1)	4.47	R-E	0 / 338	0.05 (1)	
D-E	-1844 / 0	-91.8 -91.8 0.48 (1)	4.47	E-P	0 / 489	0.08 (1)	
E-F	-1821 / 0	-91.8 -91.8 0.50 (1)	4.80	P-F	-699 / 0	0.45 (1)	
F-G	-1821 / 0	-91.8 -91.8 0.50 (1)	4.80	P-G	0 / 489	0.08 (1)	
G-H	-1844 / 0	-91.8 -91.8 0.48 (1)	4.47	N-G	0 / 338	0.05 (1)	
H-I	-1844 / 0	-91.8 -91.8 0.48 (1)	4.47	M-I	-314 / 0	0.15 (1)	
I-J	-2039 / 0	-91.8 -91.8 0.50 (1)	4.27	M-I	-245 / 10	0.17 (1)	
J-K	0 / 41	-91.8 -91.8 0.13 (1)	10.00	B-S	0 / 1637	0.37 (1)	
T-B	-2021 / 0	0.0 0.0 0.21 (1)	5.94	M-J	0 / 1637	0.37 (1)	
L-J	-2021 / 0	0.0 0.0 0.21 (1)	5.94				
T-S	0 / 0	-18.5 -18.5 0.14 (4)	10.00				
S-R	0 / 1597	-18.5 -18.5 0.33 (1)	10.00				
R-O	0 / 1387	-18.5 -18.5 0.30 (1)	10.00				
O-P	0 / 1387	-18.5 -18.5 0.30 (1)	10.00				
P-O	0 / 1387	-18.5 -18.5 0.30 (1)	10.00				
O-N	0 / 1387	-18.5 -18.5 0.30 (1)	10.00				
N-M	0 / 1597	-18.5 -18.5 0.33 (1)	10.00				
M-L	0 / 0	-18.5 -18.5 0.14 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 8 OF NBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 080-09, CSA 086-14
- TPC 2011, TPC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = U/989 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = U/989 (0.14")

CSI: TC=0.50/1.00 (E-F:1), BC=0.33/1.00 (M-N:1), WB=0.45/1.00 (F-P:1), SS=0.28/1.00 (E-F:1)

OOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

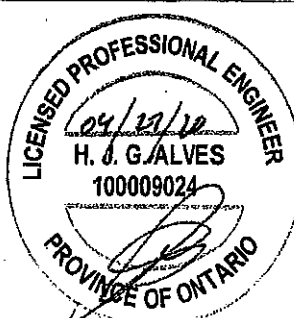
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.47 (O) (INPUT = 1.00)

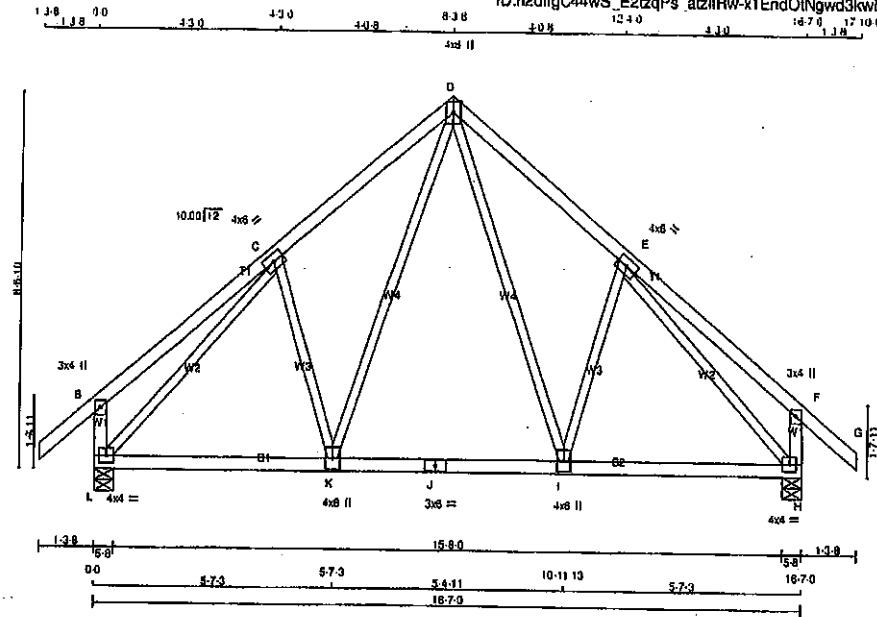


Structural component only
DWG# T-2007089

JOB NAME 408151	TRUSS NAME T28	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - G	2x4	DRY	No.2	SPF		
L - B	2x4	DRY	No.2	SPF		
H - F	2x4	DRY	No.2	SPF		
L - J	2x4	DRY	No.2	SPF		
J - H	2x4	DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	8.0	Edge	
E	TMWW-t	MT20	4.0	8.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMVW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMVW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
L	1041	0	1041	0
H	1041	0	1041	0

UNFACTORED REACTIONS	1ST LOASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
L	734	495 / 0
H	734	495 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO						FR-TO		
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	D-I	0 / 339	0.08 (1)
B-C	0 / 29	-91.8	-91.8	0.26 (1)	10.00	E-E	-222 / 0	0.09 (1)
C-D	-764 / 0	-91.8	-91.8	0.20 (1)	6.25	K-D	0 / 339	0.08 (1)
D-E	-764 / 0	-91.8	-91.8	0.20 (1)	6.25	C-K	-222 / 0	0.09 (1)
E-F	0 / 29	-91.8	-91.8	0.26 (1)	10.00	L-C	-964 / 0	0.68 (1)
F-G	0 / 41	-91.8	-91.8	0.13 (1)	10.00	E-H	-964 / 0	0.68 (1)
L-B	-270 / 0	0.0	0.0	0.03 (1)	7.81			
H-F	-270 / 0	0.0	0.0	0.03 (1)	7.81			
L-K	0 / 632	-18.5	-18.5	0.18 (4)	10.00			
K-J	0 / 459	-18.5	-18.5	0.17 (4)	10.00			
J-I	0 / 459	-18.5	-18.5	0.17 (4)	10.00			
I-H	0 / 632	-18.5	-18.5	0.18 (4)	10.00			

TOTAL WEIGHT = 2 X 82 = 163 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/899 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = L/899 (0.04")

CSI: TC=0.28/1.00 (B-C:1), BC=0.18/1.00 (H-I:4), WB=0.88/1.00 (E-H:1), SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

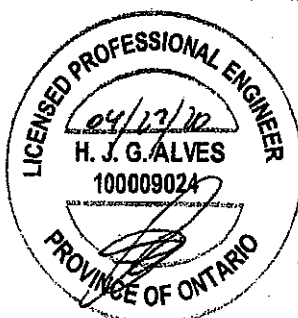
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

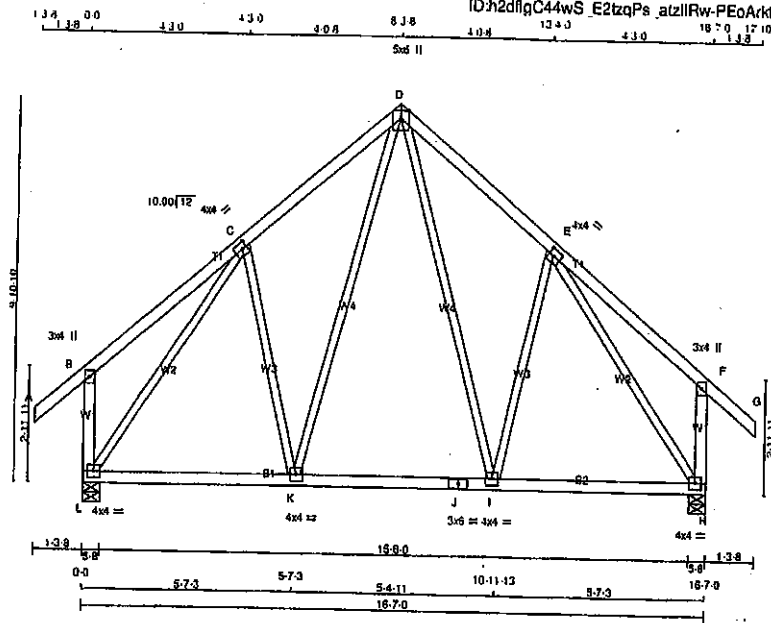
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (C) (INPUT = 0.90)
JSI METAL= 0.26 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007090

JOB NAME 408151	TRUSS NAME T29	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES	A - D	2x4	DRY No.2	SPF
	D - G	2x4	DRY No.2	SPF
	L - B	2x4	DRY No.2	SPF
	H - F	2x4	DRY No.2	SPF
	L - J	2x4	DRY No.2	SPF
	J - H	2x4	DRY No.2	SPF
	ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+1	MT20	4.0	4.0	2.00	1.50
D	TMVW+p	MT20	5.0	6.0	Edge	
E	TMVW+1	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW+1	MT20	4.0	4.0		
I	BMVW+1	MT20	4.0	4.0		
J	BS+1	MT20	3.0	6.0		
K	BMVW+1	MT20	4.0	4.0		
L	BMVW+1	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	1041	1041	0	5-8
SPF L	1041	0	0	5-8
SPF H	1041	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS					
JT COMBINED	SNOW	LIVE	PERM/LIVE	WIND	DEAD	SOIL
L	734	495 / 0	0 / 0	0 / 0	239 / 0	0 / 0
H	734	495 / 0	0 / 0	0 / 0	239 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (L)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (L)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8 -91.8	0.13 (1)	10.00	D-I	0 / 254	0.08 (1)
B-C	0 / 28	-91.8 -91.8	0.28 (1)	10.00	J-E	-138 / 14	0.10 (1)
C-D	-633 / 0	-91.8 -91.8	0.20 (1)	6.25	K-D	0 / 254	0.08 (1)
D-E	-633 / 0	-91.8 -91.8	0.20 (1)	6.25	C-K	-138 / 14	0.10 (1)
E-F	0 / 20	-91.8 -91.8	0.28 (1)	10.00	L-C	-875 / 0	0.90 (1)
F-G	0 / 41	-91.8 -91.8	0.13 (1)	10.00	E-H	-875 / 0	0.90 (1)
L-B	-276 / 0	0.0 0.0	0.04 (1)	7.81			
H-F	-276 / 0	0.0 0.0	0.04 (1)	7.81			
L-K	0 / 485	-18.5 -18.5	0.17 (4)	10.00			
K-J	0 / 395	-18.5 -18.5	0.16 (4)	10.00			
J-I	0 / 395	-18.5 -18.5	0.16 (4)	10.00			
I-H	0 / 485	-18.5 -18.5	0.17 (4)	10.00			

TOTAL WEIGHT = 91 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/980 (0.55")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.02")
ALLOWABLE DEFL. (TL) = 1/380 (0.55")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.04")

CSI: TC=0.28/1.00 (E-F:1), BC=0.17/1.00 (K-L:4), WB=0.80/1.00 (E-H:1), SS=0.14/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 768 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (E) (INPUT = 0.80)
JSI METAL = 0.34 (C) (INPUT = 1.00)

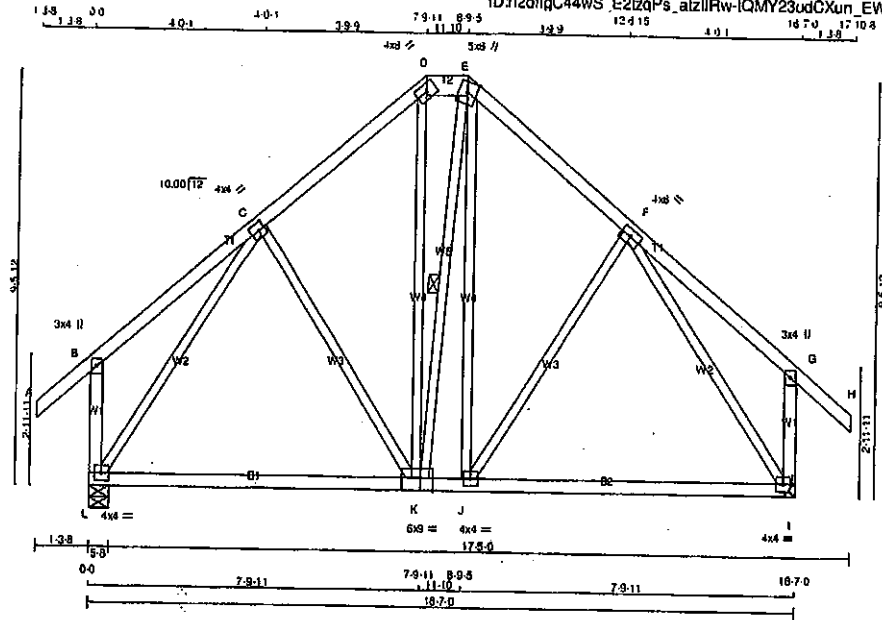


Structural component only
DWG# T-2007091

JOB NAME 408151	TRUSS NAME T30	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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CHORDS	SIZE	DRY	NO.2
A - D	2x4	DRY	No.2
D - E	2x6	DRY	No.2
E - H	2x4	DRY	No.2
L - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
L - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2

ALL WEBS 2x3 DRY NO.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW+1	MT20	4.0	4.0	2.00	1.75
D	TTW-h	MT20	4.0	6.0		
E	TTWW+m	MT20	5.0	6.0	Edge	
F	TMWW-1	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW+1	MT20	4.0	4.0		
J	BMVW+1	MT20	4.0	4.0		
K	BSWW-1	MT20	6.0	9.0	Edge	3.75
L	BMVW+1	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
L	VERT	DOWN	UPLIFT	IN-SX
I	1041	0	0	5-8
I	1041	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
L	734	495 / 0	0 / 0	0 / 0	0 / 0	239 / 0	0 / 0
I	734	495 / 0	0 / 0	0 / 0	0 / 0	239 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

I LATERAL BRACE(S) AT 1/2 LENGTH OF E-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO					FR-TO			
A-B	0 / 41	-91.8	-91.8	0.13 (1)	C-K	-98 / 9	0.09 (1)	10.00
B-C	0 / 27	-91.8	-91.8	0.23 (1)	K-D	0 / 164	0.04 (1)	10.00
C-D	-566 / 0	-91.8	-91.8	0.18 (1)	K-E	-109 / 0	0.07 (4)	8.25
D-E	-414 / 0	-91.8	-91.8	0.01 (1)	J-E	0 / 266	0.07 (4)	8.25
E-F	-582 / 0	-91.8	-91.8	0.18 (1)	J-F	-93 / 15	0.09 (1)	8.25
F-G	0 / 27	-91.8	-91.8	0.23 (1)	L-C	-848 / 0	0.78 (1)	10.00
G-H	0 / 41	-91.8	-91.8	0.13 (1)	F-I	-882 / 0	0.79 (1)	10.00
L-B	-265 / 0	0.0	0.0	0.04 (1)				
I-G	-265 / 0	0.0	0.0	0.04 (1)				
L-K	0 / 468	-18.5	-18.5	0.36 (4)				
K-J	0 / 424	-18.5	-18.5	0.30 (4)				
J-I	0 / 477	-18.5	-18.5	0.30 (4)				

TOTAL WEIGHT = 97 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, CBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.55')
CALCULATED VERT. DEFL.(LL) = $L/989$ (0.01')
ALLOWABLE DEFL.(TL) = $L/360$ (0.55')
CALCULATED VERT. DEFL.(TL) = $L/989$ (0.17')

CSI: TO=0.23/1.00 (B-C:1), BC=0.38/1.00 (K-L:4), WB=0.78/1.00 (F-I:1), SSI=0.14/1.00 (J-K:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	818	354	1667
	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

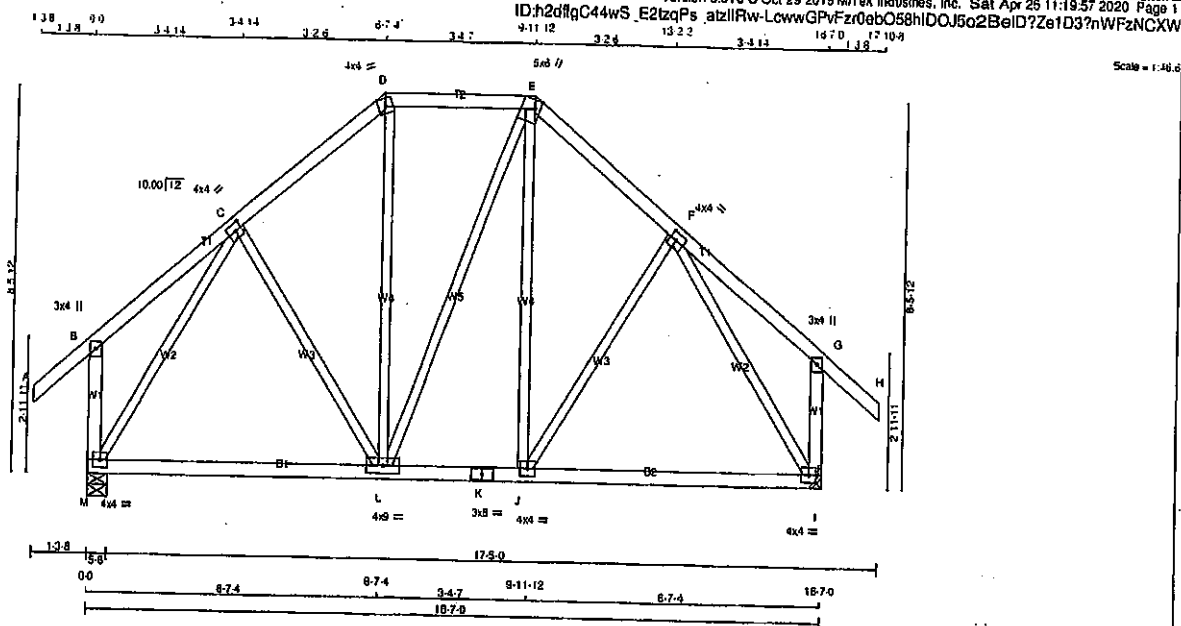
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007092

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408151	T31	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
M - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
M - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-1	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW-1	MT20	4.0	4.0	2.00	1.75
G	TMV+p	MT20	3.0	4.0		
J	BMVW-1	MT20	4.0	4.0		
K	BS-1	MT20	4.0	4.0		
L	BMVW-1	MT20	4.0	9.0		
M	BMVW-1	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
M	1041	0	1041	0	0
I	1041	0	1041	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	734	495/0	0/0	0/0	0/0	239/0	0/0
I	734	495/0	0/0	0/0	0/0	239/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO				FR-TO			
A-B	0/41	-91.8	0.13 (1)	C-L	-10/34	0.01 (4)	
B-C	0/22	-91.8	0.16 (1)	L-D	0/110	0.03 (4)	
C-D	-613/0	-91.8	0.13 (1)	D-E	0/0	0.00 (1)	
D-E	-483/0	-91.8	0.14 (1)	E-F	0/109	0.03 (4)	
E-F	-612/0	-91.8	0.13 (1)	F-G	-11/34	0.01 (4)	
F-G	0/22	-91.8	0.16 (1)	G-H	0/0	0.00 (1)	
G-H	0/41	-91.8	0.13 (1)	H-I	-873/0	0.60 (1)	
H-I	-245/0	0.0	0.04 (1)	I-J	-873/0	0.60 (1)	
I-J	-245/0	0.0	0.04 (1)				
M-L	0/457	-18.5	-18.5 0.21 (4)				
L-K	0/453	-18.5	-18.5 0.20 (4)				
K-J	0/453	-18.5	-18.5 0.20 (4)				
J-I	0/457	-18.5	-18.5 0.21 (4)				

TOTAL WEIGHT = 92 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2015, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.55")
CALCULATED VERT. DEFL. (LL) = U/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.55")
CALCULATED VERT. DEFL. (TL) = U/999 (0.07")

CSI: TC=0.18/1.00 (F-G:1), BC=0.21/1.00 (H-I:4), WB=0.60/1.00 (C-M:1), SS=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

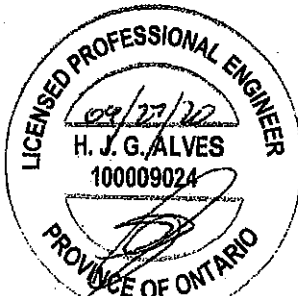
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (RY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 798 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

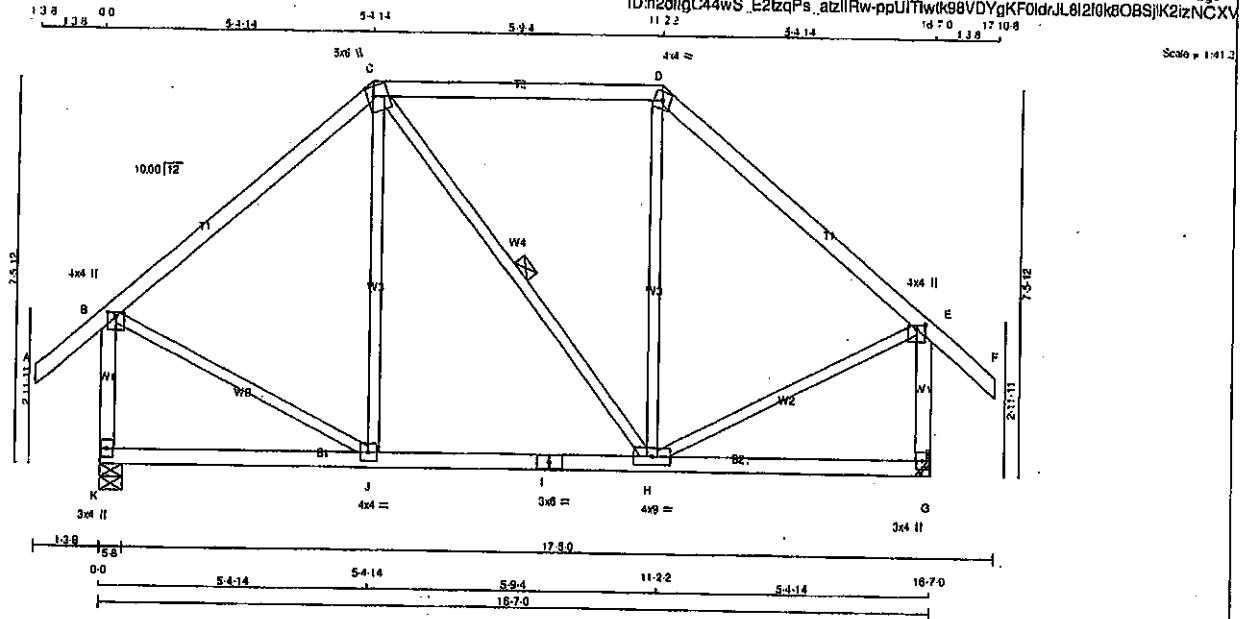
JSI GRIP = 0.87 (C) (INPUT = 0.90)
JSI METAL = 0.31 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007093

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408151	T32	1	1	GREEN PARK HOMES	
Tamarack Pool Truss, Burlington					

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TOTAL WEIGHT = 79 lb
(M/F)

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER		DESCR
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTWW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMVWW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		RECORD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG	IN-SX	IN-SX
K	1041	0	1041	0	0	5-8	5-8		
G	1041	0	1041	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	734	495/0	0/0	0/0	0/0	238/0	0/0
G	734	495/0	0/0	0/0	0/0	238/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. (LC)
FR-TO					FR-TO		
A-B	0/41	-91.8	-91.8 0.13 (1)	10.00	J-C	-115/48	0.12 (1)
B-C	-619/0	-91.8	-91.8 0.35 (1)	8.25	C-H	0/0	0.00 (1)
C-D	-473/0	-91.8	-91.8 0.39 (1)	8.25	H-D	-115/48	0.12 (1)
D-E	-619/0	-91.8	-91.8 0.35 (1)	8.25	B-J	0/526	0.12 (1)
E-F	0/41	-91.8	-91.8 0.13 (1)	10.00	H-E	0/526	0.12 (1)
K-B	-1000/0	0.0	0.0 0.18 (1)	7.81			
G-E	-1000/0	0.0	0.0 0.18 (1)	7.81			
K-J	0/0	-18.5	-18.5 0.13 (4)	10.00			
J-I	0/473	-18.5	-18.5 0.17 (4)	10.00			
I-H	0/473	-18.5	-18.5 0.17 (4)	10.00			
H-G	0/0	-18.5	-18.5 0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(56% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.39/1.00 (C-D:1), BC=0.17/1.00 (H-J:4), WB=0.12/1.00 (B-J:1), SS=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 518 354 1697 788 1987 1696

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

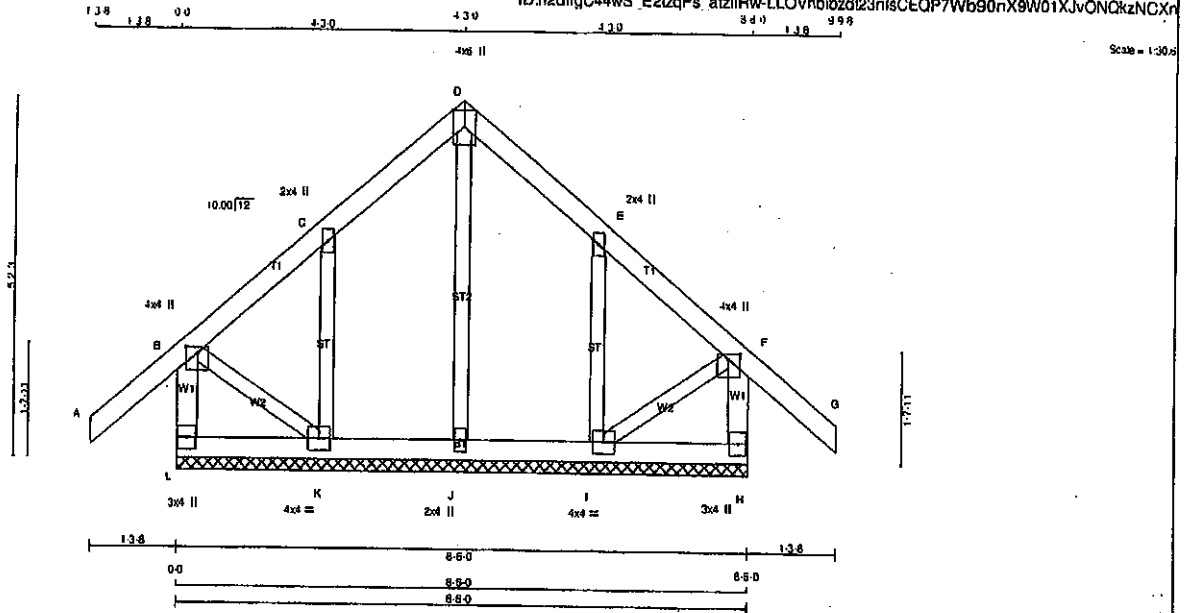
JSI GRIP = 0.82 (B) (INPUT = 0.90)
JSI METAL = 0.23 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007094

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408151	G33	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
L - B	2x4	DRY	No.2		
A - D	2x4	DRY	No.2		
D - G	2x4	DRY	No.2		
H - F	2x4	DRY	No.2		
L - H	2x4	DRY	No.2		

ALL WEBS 2x3 DRY No.2
ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C TMVW+w	MT20	2.0	4.0		
D TTW+p	MT20	4.0	6.0	Edge	
E TMVW+w	MT20	2.0	4.0		
F TMVW+p	MT20	4.0	4.0	1.00	2.00
H BMV1+p	MT20	3.0	4.0		
I BMVW1-t	MT20	4.0	4.0		
J BMV1+w	MT20	2.0	4.0		
K BMVW1-t	MT20	4.0	4.0		
L BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
L-B	-231 / 0	0.0	0.0	0.02 (1)	J-D	-123 / 0	0.05 (1)
A-B	0 / 41	-91.8	-91.8	0.13 (1)	K-C	-236 / 0	0.05 (1)
B-C	-11 / 0	-91.8	-91.8	0.07 (1)	I-E	-236 / 0	0.05 (1)
C-D	-32 / 0	-91.8	-91.8	0.07 (1)	B-K	0 / 23	0.01 (1)
D-E	-32 / 0	-91.8	-91.8	0.07 (1)	I-F	0 / 23	0.01 (1)
E-F	-11 / 0	-91.8	-91.8	0.07 (1)			
F-G	0 / 41	-91.8	-91.8	0.13 (1)			
H-F	-231 / 0	0.0	0.0	0.02 (1)			
L-K	0 / 0	-18.5	-18.5	0.02 (4)			
K-J	0 / 13	-18.5	-18.5	0.02 (4)			
J-I	0 / 13	-18.5	-18.5	0.02 (4)			
I-H	0 / 0	-18.5	-18.5	0.02 (4)			

TOTAL WEIGHT = 42 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	38.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (F-G:1), BC=0.02/1.00 (J-K:4), WB=0.05/1.00 (C-K:1), SS=0.08/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

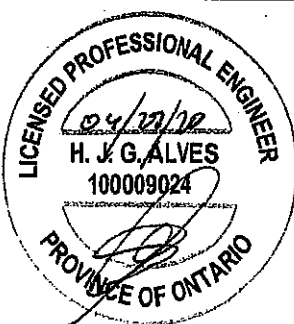
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PS)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1687
	768	1987	1655

PLATE PLACEMENT TOL. = 0.250 inches

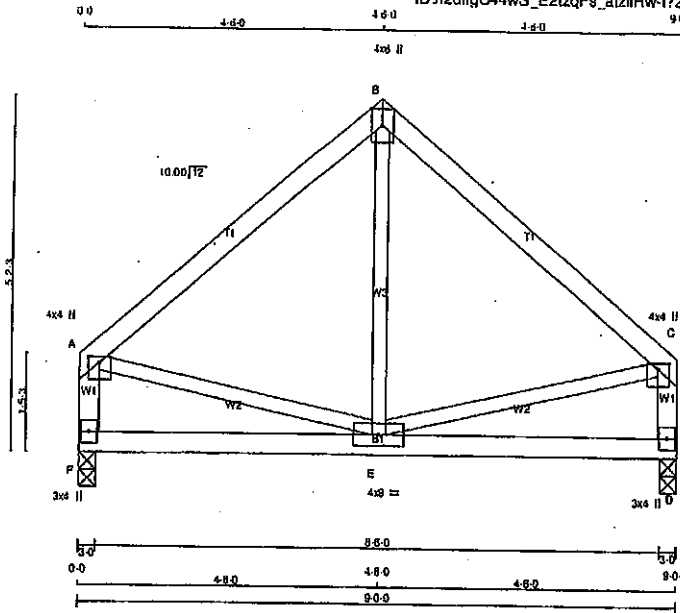
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (C) (INPUT = 0.80)
JSI METAL= 0.13 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007078

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408151	T33X	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 11:19:59 2020 Page 1	



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
A - B	2x4	DRY	No.2		
B - C	2x4	DRY	No.2		
F - A	2x4	DRY	No.2		
D - C	2x4	DRY	No.2		
F - D	2x4	DRY	No.2		

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW+p	MT20	4.0	4.0	1.00	2.00
B	TTW+p	MT20	4.0	8.0	Edge	
C	TMW+p	MT20	4.0	4.0	1.00	2.00
D	BMV+p	MT20	3.0	4.0		
E	BMVWW-t	MT20	4.0	9.0		
F	BMV+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG.	REQD BRG.
JT	VERT	HORZ	DOWN	HORZ
F	498	0	498	0
D	498	0	498	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	351	230 / 0	0 / 0	0 / 0	0 / 0	0 / 0	121 / 0	0 / 0
D	351	230 / 0	0 / 0	0 / 0	0 / 0	0 / 0	121 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC)	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD (LC)
FR-TO						FR-TO			
A-B	-311 / 0	-91.8	-91.8	0.24 (1)	8.25	E-B	-14 / 72	0.03 (4)	
B-C	-311 / 0	-91.8	-91.8	0.24 (1)	8.25	A-E	0 / 246	0.08 (1)	
F-A	-464 / 0	0.0	0.0	0.05 (1)	7.81	E-C	0 / 246	0.08 (1)	
D-C	-464 / 0	0.0	0.0	0.05 (1)	7.81				
F-E	0 / 0	-18.5	-18.5	0.11 (4)	10.00				
E-D	0 / 0	-18.5	-18.5	0.11 (4)	10.00				

TOTAL WEIGHT = 37 lb (M) F

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = $L/360$ (0.30")
CALCULATED VERT. DEF. (LL) = $L/999$ (0.00")
ALLOWABLE DEF. (TL) = $L/360$ (0.30")
CALCULATED VERT. DEF. (TL) = $L/999$ (0.01")

CSI: TC=0.24/1.00 (A-B:1), BC=0.11/1.00 (E-F:4), WB=0.06/1.00 (C-E:1), SI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

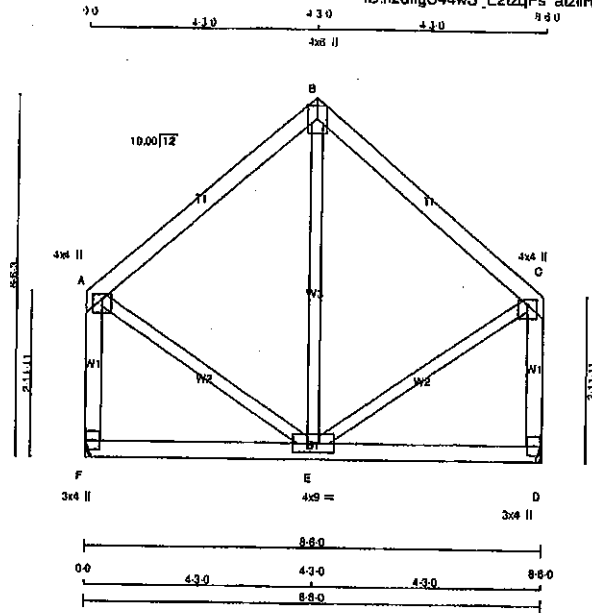
JSI GRIP = 0.42 (A) (INPUT = 0.90)
JSI METAL = 0.12 (A) (INPUT = 1.00)



Structural component only
DWG# T-2007095

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408151	T34	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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Scale = 1/8" = 1'-0"

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	X
A	TMVW+p	MT20	4.0	4.0	1.00 2.00
B	TTW+p	MT20	4.0	6.0	Edge
C	TMVW+p	MT20	4.0	4.0	1.00 2.00
D	BMV1+p	MT20	3.0	4.0	
E	BMVWW-t	MT20	4.0	9.0	
F	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG	BRG	BRG
F	469	0	469	0	0	MECHANICAL	MECHANICAL
D	469	0	469	0	0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	331	217/0	0/0	0/0	0/0	114/0	0/0
D	331	217/0	0/0	0/0	0/0	114/0	0/0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (PLF)	MEMB.	FORCE (LBS)	MAX. (PLF)	MAX. (PLF)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	-218/0	-91.8	-91.8 0.21 (1)	E-B	-111/32	0.07 (1)	
B-C	-218/0	-91.8	-91.8 0.21 (1)	A-E	0/197	0.04 (1)	
F-A	-438/0	0.0	0.0 0.07 (1)	E-C	0/197	0.04 (1)	
D-C	-438/0	0.0	0.0 0.07 (1)				
F-E	0/0	-18.5	-18.5 0.09 (4)				
E-D	0/0	-18.5	-18.5 0.09 (4)				

TOTAL WEIGHT = 2 X 41 = 82 lb

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(56% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.28")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.28")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.01")

CSI: TC=0.21/1.00 (A-B:1), BC=0.09/1.00 (E-F:4),
WB=0.07/1.00 (B-E:1), SS=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	
PLATE GRIP (DRY)	SHEAR SECTION (PL)
(PS)	(PL)
MAX MIN	MAX MIN
MT20 618 354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

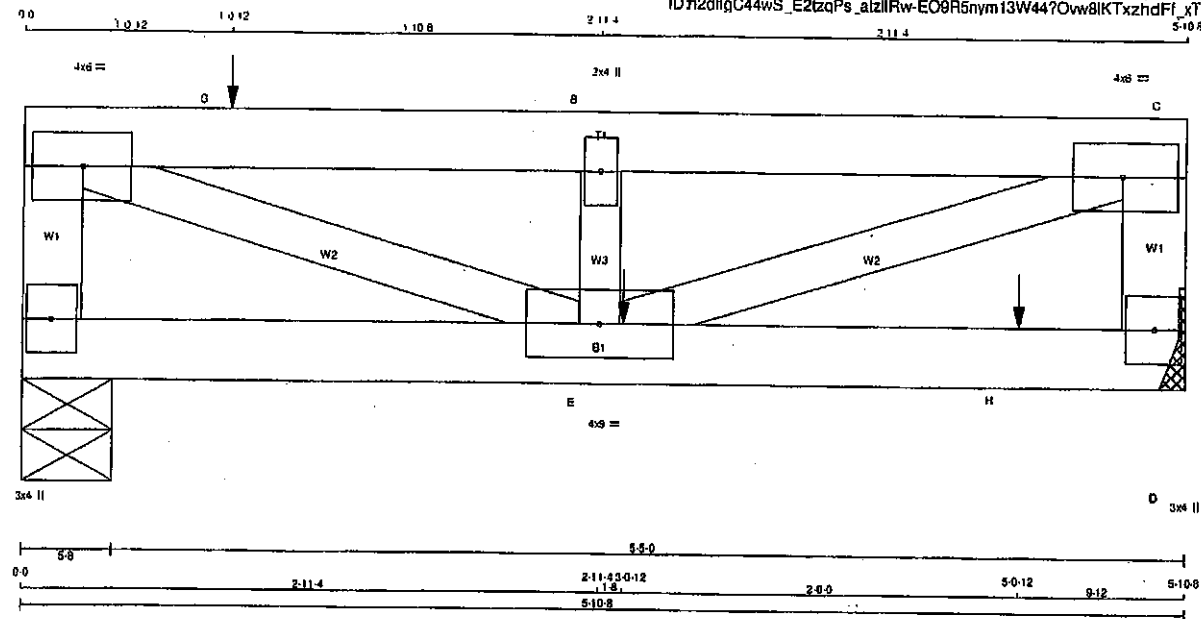
JSI GRIP = 0.33 (A) (INPUT = 0.90)
JSI METAL = 0.09 (A) (INPUT = 1.00)



Structural component only
DWG# T-2007096

JOB NAME 408151	TRUSS NAME T35	QUANTITY 2	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
F - A	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
D - C	2x4 DRY	No.2	SPF
F - D	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN.)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A	1 12	TOP
A - C	1 12	TOP
C - D	1 12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	1 11	SIDE (97.1)
WEBS : (0.122"x3") SPIRAL NAILS		
B - E	1 6	SIDE (20.3)
2x3	1 6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-1	MT20	4.0	6.0	
B	TMVW-1	MT20	2.0	4.0	
C	TMVW-1	MT20	4.0	6.0	
D	BMV1+3	MT20	3.0	4.0	
E	BMVWV-1	MT20	4.0	9.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT
F	1016	0	1016	0	5-8
D	1085	0	1085	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-6.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
F	786	438 / 0	0 / 0	0 / 0	290 / 0	0 / 0
D	772	479 / 0	0 / 0	0 / 0	293 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.23 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LBS)	MAX. VERT. LOAD (LBS)	MAX. VERT. LOAD (LBS)	MAX. VERT. LOAD (LBS)	MAX. VERT. LOAD (LBS)	MAX. VERT. LOAD (LBS)
MEMB.	FR-TO								
F-A	-965 / 0	0.0	0.0	0.05 (1)	7.81	A-E	0 / 1810	0.22 (1)	
A-G	-1706 / 0	-91.8	-91.8	0.28 (1)	6.23	E-B	-482 / 0	0.04 (1)	
G-B	-1706 / 0	-91.8	-91.8	0.28 (1)	6.23	E-C	0 / 1810	0.22 (1)	
B-C	-1706 / 0	-91.8	-91.8	0.08 (1)	6.25				
D-C	-707 / 0	0.0	0.0	0.04 (1)	7.81				
F-E	0 / 0	-43.5	-43.5	0.05 (4)	10.00				
E-H	0 / 0	-43.5	-43.5	0.22 (1)	10.00				
H-D	0 / 0	-43.5	-43.5	0.22 (1)	10.00				

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
E	3-0-12	-450	-450	---	FRONT	VERT	TOTAL	---	C1
G	1-8-12	-404	-404	---	TOP	VERT	TOTAL	---	C1
H	5-0-12	-452	-452	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 20 = 80 LB

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.28/1.00 (A-B:1), BC=0.22/1.00 (D-E:1), WB=0.22/1.00 (A-E:1), SS=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

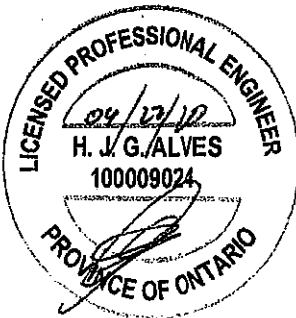
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES		PLATE GRIP(DRY) SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
		788	1987

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.59 (C) (INPUT = 0.90)
JSI METAL = 0.21 (C) (INPUT = 1.00)



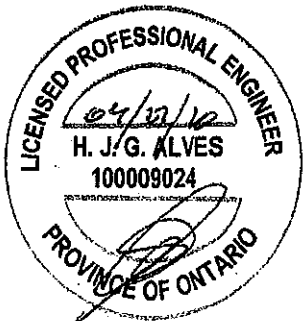
Structural component only
DWG# T-2007097 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408151	T35	2	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

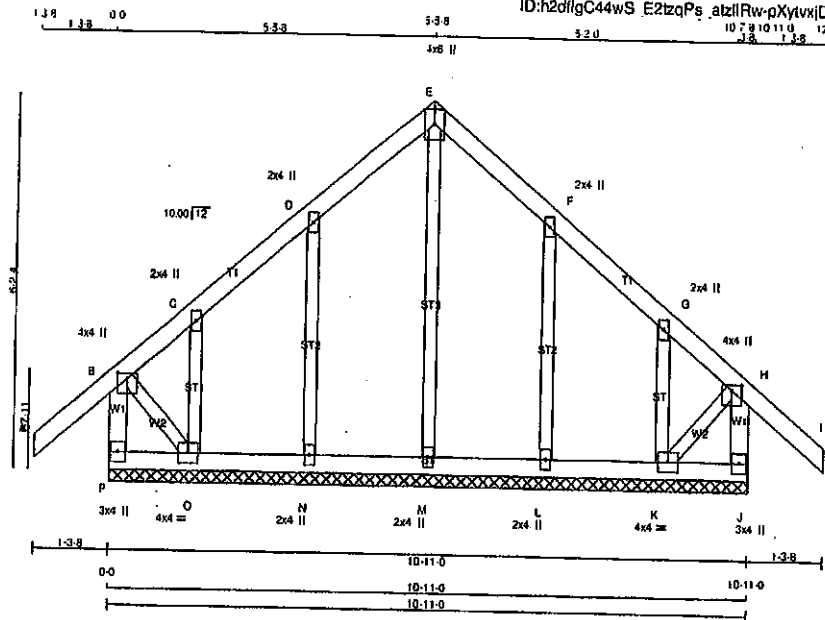
Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 11:20:01 2020 Page 2
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PLATES (table in inches)
 JT TYPE PLATES W LEN Y X
 F BMV1-p MT20 3.0 4.0



Structural component only
 DWG# T-2007097 32

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408151	G36	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 11:19:41 2020 Page 1					
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10 7 8 10 11 9 12 2 8					
Scale = 1/32"					



LUMBER		
N. L. G. A. RULES	CHORDS	SIZE
P - B	2x4	DRY
A - E	2x4	DRY
E - I	2x4	DRY
J - H	2x4	DRY
P - J	2x4	DRY

ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0\"/>

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMW+p	MT20	4.0	4.0	1.00 2.00
C, D, F, G					
C	TMW+w	MT20	2.0	4.0	
E	TTW+p	MT20	4.0	8.0	Edge
H	TMW+p	MT20	4.0	4.0	1.00 2.00
J	BMV1+p	MT20	3.0	4.0	
K	BMW1-t	MT20	4.0	4.0	
L, M, N					
L	BMV1+w	MT20	2.0	4.0	
O	BMW1-t	MT20	4.0	4.0	
P	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX. UNBRACED LENGTH (FT)
FR-TO			FR-TO		
P-B	-274/0	0.0	M-E	-139/0	0.08 (1)
A-B	0/41	-91.8	N-D	-223/0	0.07 (1)
B-C	-57/0	-91.8	O-C	-75/0	0.01 (1)
C-D	-4/0	-91.8	L-F	-223/0	0.07 (1)
D-E	-22/0	-91.8	K-G	-75/0	0.01 (1)
E-F	-22/0	-91.8	B-O	0/20	0.00 (1)
F-G	-4/0	-91.8	K-H	0/20	0.00 (1)
G-H	-57/0	-91.8			
H-I	0/41	-91.8			
J-H	-274/0	0.0			
P-O	0/0	-18.5			
O-N	0/12	-18.5			
N-M	0/7	-18.5			
M-L	0/7	-18.5			
L-K	0/12	-18.5			
K-J	0/0	-18.5			

TOTAL WEIGHT = 53 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (H-I:1), GC=0.02/1.00 (K-L:4), WB=0.08/1.00 (E-M:1), SS=0.08/1.00 (A-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618 354 1667 788 1987 1656	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.12 (F) (INPUT = 1.00)



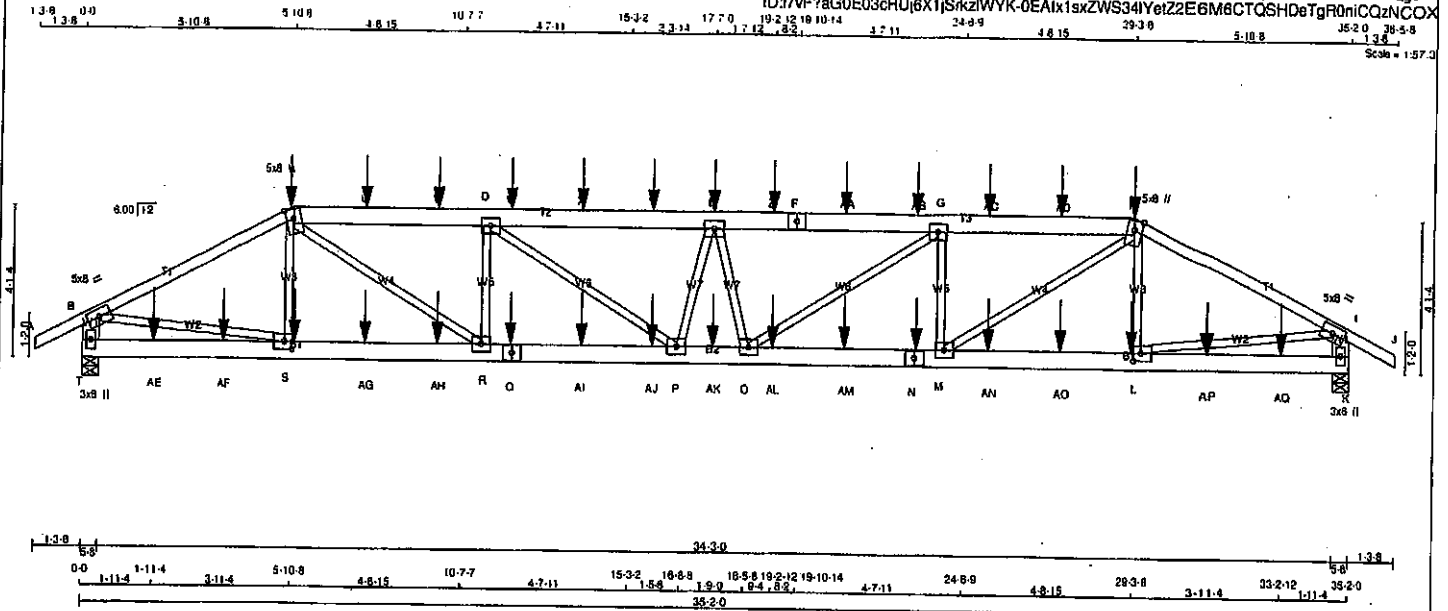
Structural component only
DWG# T-2007079

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408152	T40	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - H	2x8	DRY	No.2	SPF		
H - J	2x4	DRY	No.2	SPF		
T - B	2x8	DRY	No.2	SPF		
K - I	2x6	DRY	No.2	SPF		
T - Q	2x8	DRY	No.2	SPF		
Q - N	2x8	DRY	No.2	SPF		
N - K	2x8	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE(61.0)
H - J	12	SIDE(61.0)
C - F	12	SIDE(61.0)
F - H	12	SIDE(61.0)
T - B	12	TOP
K - I	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
T - Q	12	SIDE(183.1)
Q - N	12	SIDE(0.0)
N - K	12	SIDE(183.1)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
T	3331	0	5-8	5-8
K	3331	0	5-8	5-8

UNFACTORED REACTIONS

1ST LOASE	MAX/MIN COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2356	1544 / 0	0 / 0	0 / 0	0 / 0	812 / 0	0 / 0
K	2356	1544 / 0	0 / 0	0 / 0	0 / 0	812 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 28	-91.8 -91.8 0.07 (1)	10.00	S-C -427 / 8
B-C	-5077 / 0	-91.8 -91.8 0.52 (1)	3.87	C-H 0 / 3083
C-U	-7106 / 0	-91.8 -91.8 0.23 (1)	4.33	R-D -1565 / 0
U-V	-7106 / 0	-91.8 -91.8 0.23 (1)	4.33	O-G 0 / 1071
V-D	-7106 / 0	-91.8 -91.8 0.23 (1)	4.33	M-G -1565 / 0
D-W	-7994 / 0	-91.8 -91.8 0.27 (1)	4.10	L-H -427 / 8
W-X	-7994 / 0	-91.8 -91.8 0.27 (1)	4.10	B-S 0 / 4592
X-Y	-7994 / 0	-91.8 -91.8 0.27 (1)	4.10	L-I 0 / 4592
Y-E	-7994 / 0	-91.8 -91.8 0.27 (1)	4.10	D-P 0 / 1071
E-Z	-7994 / 0	-91.8 -91.8 0.27 (1)	4.10	M-H 0 / 3083
Z-F	-7994 / 0	-91.8 -91.8 0.27 (1)	4.10	P-E -505 / 0
F-AA	-7994 / 0	-91.8 -91.8 0.27 (1)	4.10	E-O -505 / 0
AA-AB	-7994 / 0	-91.8 -91.8 0.27 (1)	4.10	
AB-G	-7994 / 0	-91.8 -91.8 0.27 (1)	4.10	
G-AC	-7106 / 0	-91.8 -91.8 0.23 (1)	4.33	
AC-AD	-7106 / 0	-91.8 -91.8 0.23 (1)	4.33	
AD-H	-7106 / 0	-91.8 -91.8 0.23 (1)	4.33	
H-I	-5077 / 0	-91.8 -91.8 0.52 (1)	3.87	
I-J	0 / 28	-91.8 -91.8 0.07 (1)	10.00	
T-B	-3255 / 0	0.0 0.0 0.11 (1)	7.69	
K-I	-3255 / 0	0.0 0.0 0.11 (1)	7.69	
T-AE	0 / 0	-18.5 -18.5 0.06 (4)	10.00	
AE-AF	0 / 0	-18.5 -18.5 0.06 (4)	10.00	
AF-S	0 / 0	-18.5 -18.5 0.06 (4)	10.00	
S-AG	0 / 4563	-18.5 -18.5 0.34 (1)	10.00	
AG-AH	0 / 4563	-18.5 -18.5 0.34 (1)	10.00	
AH-R	0 / 4563	-18.5 -18.5 0.34 (1)	10.00	
R-Q	0 / 7105	-18.5 -18.5 0.52 (1)	10.00	
Q-AI	0 / 7105	-18.5 -18.5 0.52 (1)	10.00	
AI-AJ	0 / 7105	-18.5 -18.5 0.52 (1)	10.00	
AJ-P	0 / 7105	-18.5 -18.5 0.52 (1)	10.00	
P-AK	0 / 8119	-18.5 -18.5 0.56 (1)	10.00	
AK-O	0 / 8119	-18.5 -18.5 0.56 (1)	10.00	
O-AL	0 / 7105	-18.5 -18.5 0.52 (1)	10.00	
AL-AM	0 / 7105	-18.5 -18.5 0.52 (1)	10.00	
AM-N	0 / 7105	-18.5 -18.5 0.52 (1)	10.00	
N-M	0 / 7105	-18.5 -18.5 0.52 (1)	10.00	
M-AN	0 / 4563	-18.5 -18.5 0.34 (1)	10.00	
AN-AO	0 / 4563	-18.5 -18.5 0.34 (1)	10.00	
AO-L	0 / 4563	-18.5 -18.5 0.34 (1)	10.00	
L-AP	0 / 0	-18.5 -18.5 0.06 (4)	10.00	
AP-AQ	0 / 0	-18.5 -18.5 0.06 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/899 (0.21")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = L/899 (0.39")

CSI: TO=0.52/1.00 (H-I:1), BC=0.56/1.00 (O-P:1),
WB=0.57/1.00 (B-S:1), SS=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (S) (INPUT = 0.90)
JSI METAL= 0.84 (N) (INPUT = 1.00)



Structural component only
DWG# T-2007107 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408152	T40	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID: j7vF7aG0E03cRU6X11SkzWYK-0EA1x1sxZWS341YeiZ2E8M6CTQSHD8TgR0niCQzNCOX

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	5.0	8.0		
C TTWW+m	MT20	5.0	8.0	Edge	
D, E, G					
D TMVW-t	MT20	5.0	8.0		
F TS-t	MT20	5.0	8.0		
H TTWW+m	MT20	5.0	8.0	Edge	
I TMVW-t	MT20	5.0	8.0		
K BMV1+p	MT20	3.0	8.0		
L BMVW-t	MT20	5.0	8.0	2.50	2.50
M, O, P, R					
M BMVW-t	MT20	5.0	8.0		
N BS-t	MT20	5.0	8.0		
Q BS-t	MT20	5.0	8.0		
S BMVW-t	MT20	5.0	8.0	2.50	2.50
T BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: (4)

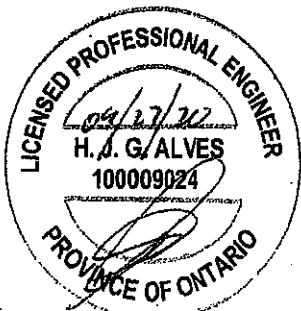
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
AQ-K	0.0	-18.5 -18.5	0.06 (4)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-438	-438		FRONT	VERT	TOTAL		C1
E	17-7-0	-110	-110		FRONT	VERT	TOTAL		C1
H	29-3-8	-438	-438		FRONT	VERT	TOTAL		C1
L	29-2-12	-28	-28		FRONT	VERT	TOTAL		C1
N	23-2-12	-28	-28		FRONT	VERT	TOTAL		C1
Q	11-11-4	-28	-28		FRONT	VERT	TOTAL		C1
S	5-11-4	-28	-28		FRONT	VERT	TOTAL		C1
U	7-11-4	-110	-110		FRONT	VERT	TOTAL		C1
V	9-11-4	-110	-110		FRONT	VERT	TOTAL		C1
W	11-11-4	-110	-110		FRONT	VERT	TOTAL		C1
X	13-11-4	-110	-110		FRONT	VERT	TOTAL		C1
Y	15-11-4	-110	-110		FRONT	VERT	TOTAL		C1
Z	19-2-12	-110	-110		FRONT	VERT	TOTAL		C1
AA	21-2-12	-110	-110		FRONT	VERT	TOTAL		C1
AB	23-2-12	-110	-110		FRONT	VERT	TOTAL		C1
AC	25-2-12	-110	-110		FRONT	VERT	TOTAL		C1
AD	27-2-12	-110	-110		FRONT	VERT	TOTAL		C1
AE	1-11-4	-28	-28		FRONT	VERT	TOTAL		C1
AF	3-11-4	-28	-28		FRONT	VERT	TOTAL		C1
AG	7-11-4	-28	-28		FRONT	VERT	TOTAL		C1
AH	9-11-4	-28	-28		FRONT	VERT	TOTAL		C1
AI	13-11-4	-28	-28		FRONT	VERT	TOTAL		C1
AJ	15-11-4	-28	-28		FRONT	VERT	TOTAL		C1
AK	17-7-0	-28	-28		FRONT	VERT	TOTAL		C1
AL	19-2-12	-28	-28		FRONT	VERT	TOTAL		C1
AM	21-2-12	-28	-28		FRONT	VERT	TOTAL		C1
AN	23-2-12	-28	-28		FRONT	VERT	TOTAL		C1
AO	25-2-12	-28	-28		FRONT	VERT	TOTAL		C1
AP	27-2-12	-28	-28		FRONT	VERT	TOTAL		C1
AQ	31-2-12	-28	-28		FRONT	VERT	TOTAL		C1
	33-2-12	-28	-28		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

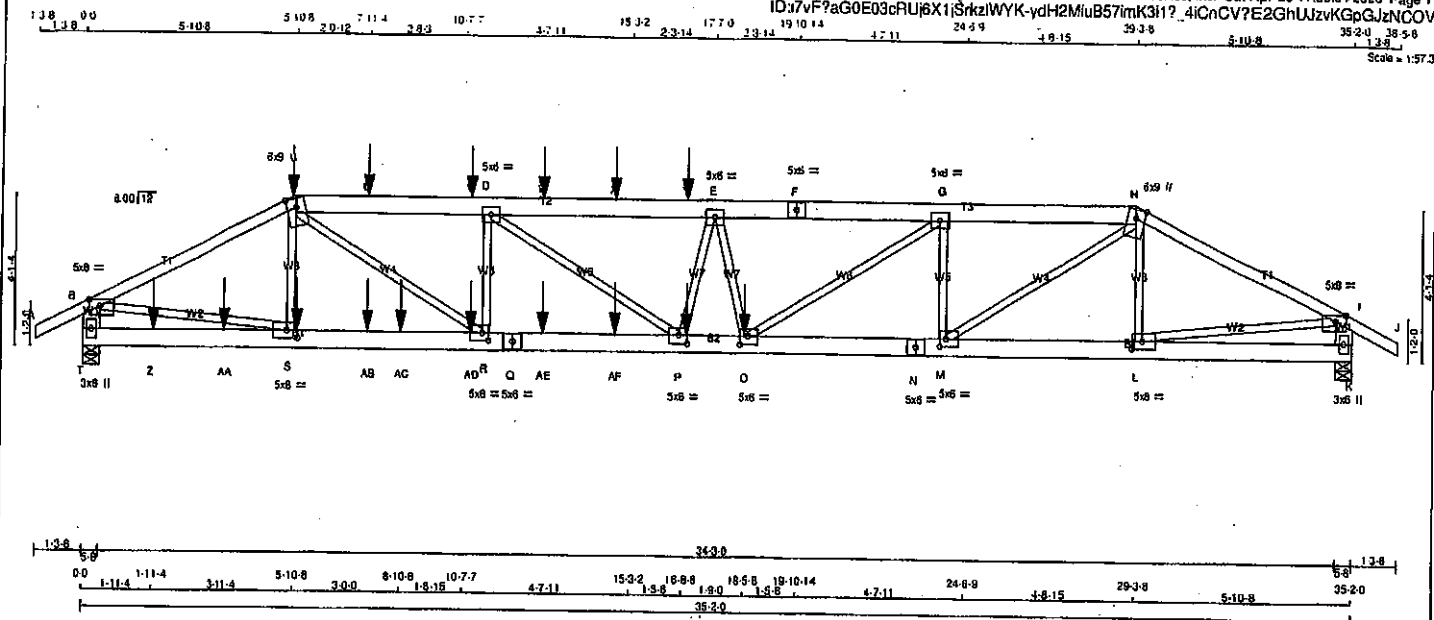
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007107 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408152	T40Z	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:7vF?aG0E03cRUj6X1(SrkzIWK-ydH2MiuB57mK317_4CnCV?E2GhUJzvKGpGJzNCOV



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - J	2x4	DRY	No.2
T - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
T - Q	2x6	DRY	No.2
Q - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

ORY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(61.0)
H-J	12	TOP
O-F	12	SIDE(61.0)
F-H	12	TOP
T-B	12	TOP
K-I	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
T-Q	12	SIDE(183.1)
Q-N	12	SIDE(0.0)
N-K	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
T	4474	0	0	4474	0	0	0	5-8	5-8
K	3775	0	0	3775	0	0	0	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
T	3182	2088 / 0	0 / 0	0 / 0	1073 / 0
K	2685	1772 / 0	0 / 0	0 / 0	893 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)	
CHORDS	MAX. FACTORED	FACTORED	WEBS
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. FACTORED
FR-TO	FROM TO	CS1 (LC)	UNBRAC
A-B	0 / 28	-91.8	-91.8
B-C	-7183 / 0	-91.8	-91.8
C-U	-10289 / 0	-91.8	-91.8
U-V	-10289 / 0	-91.8	-91.8
V-D	-10289 / 0	-91.8	-91.8
D-W	-12012 / 0	-91.8	-91.8
W-X	-12012 / 0	-91.8	-91.8
X-Y	-12012 / 0	-91.8	-91.8
Y-E	-12012 / 0	-91.8	-91.8
E-F	-12381 / 0	-91.8	-91.8
F-G	-12381 / 0	-91.8	-91.8
G-H	-9388 / 0	-91.8	-91.8
H-I	-5888 / 0	-91.8	-91.8
I-J	0 / 28	-91.8	-91.8
T-B	-4430 / 0	0.0	0.0
K-I	-3718 / 0	0.0	0.0

CHORDS	MAX. FACTORED	FACTORED	WEBS
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. FACTORED
FR-TO	FROM TO	CS1 (LC)	UNBRAC
A-B	0 / 28	-91.8	-91.8
B-C	-7183 / 0	-91.8	-91.8
C-U	-10289 / 0	-91.8	-91.8
U-V	-10289 / 0	-91.8	-91.8
V-D	-10289 / 0	-91.8	-91.8
D-W	-12012 / 0	-91.8	-91.8
W-X	-12012 / 0	-91.8	-91.8
X-Y	-12012 / 0	-91.8	-91.8
Y-E	-12012 / 0	-91.8	-91.8
E-F	-12381 / 0	-91.8	-91.8
F-G	-12381 / 0	-91.8	-91.8
G-H	-9388 / 0	-91.8	-91.8
H-I	-5888 / 0	-91.8	-91.8
I-J	0 / 28	-91.8	-91.8
T-B	-4430 / 0	0.0	0.0
K-I	-3718 / 0	0.0	0.0

FACTORED CONCENTRATED LOADS (LBS)										
	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.	
C	5-10-8	-438	-438	---	BACK	VERT	TOTAL	---	C1	
O	18-5-8	-2436	-2436	---	BACK	VERT	TOTAL	---	C1	
P	18-9-12	-17	-17	---	BACK	VERT	TOTAL	---	C1	
S	5-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1	
U	7-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1	
V	10-9-12	-82	-82	---	BACK	VERT	TOTAL	---	C1	
W	12-9-12	-82	-82	---	BACK	VERT	TOTAL	---	C1	

TOTAL WEIGHT = 2 X 170 = 341 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2010, NBC2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC2010, CBC2012, ABC2019
- PART 9 OF CBC2012 (2019 AMENDMENT)
- CSA 088-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.31")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = 1/740 (0.57")

CS1: TC=0.71/1.00 (B-C:1), BC=0.91/1.00 (O-P:1),
WB=0.80/1.00 (B-S:1), SS=0.15/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

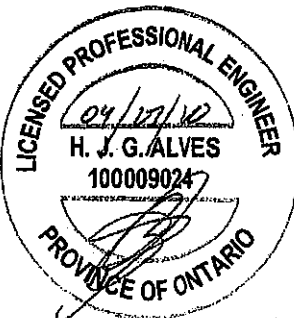
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1887 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.89 (S) (INPUT = 0.90)
JSI METAL=0.93 (N) (INPUT = 1.00)



Structural component only
DWG# T-2007108

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408152	T40Z	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:J7vF?AG0E03cRU8X1(S)kziWYK-ydH2MiuB57imK3i1? 4iCnCV?E2GhUzVKGpGJzNCOV

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	2.00	3.50
C	TTWW+m	MT20	8.0	9.0	Edge	
D, E, G						
D	TMVW-l	MT20	5.0	6.0		
F	TS-l	MT20	5.0	8.0		
H	TTWW+m	MT20	6.0	9.0	Edge	
I	TMVW-p	MT20	5.0	8.0	2.00	3.50
K	BMV1+p	MT20	3.0	6.0		
L	BMVW-l	MT20	5.0	8.0	2.50	3.50
M	BMVW-l	MT20	5.0	8.0	2.50	2.00
N	BS-l	MT20	5.0	6.0		
O	BMVW-l	MT20	5.0	8.0	2.75	2.75
P	BMVW-l	MT20	5.0	6.0	2.75	2.75
Q	BS-l	MT20	5.0	6.0		
R	BMVW-l	MT20	5.0	8.0	2.50	2.00
S	BMVW-l	MT20	5.0	8.0	2.50	3.50
T	BMV1+p	MT20	3.0	6.0		

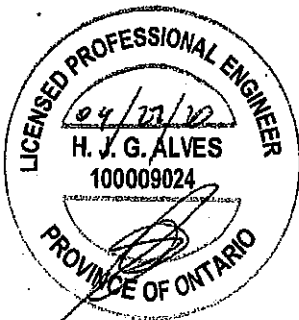
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
X	14-9-12	-82	-82	---	BACK	VERT	TOTAL	---	C1
Y	16-9-12	-82	-82	---	BACK	VERT	TOTAL	---	C1
Z	1-11-4	-25	-26	---	BACK	VERT	TOTAL	---	C1
AA	3-11-4	-28	-28	---	BACK	VERT	TOTAL	---	C1
AB	7-11-4	-28	-28	---	BACK	VERT	TOTAL	---	C1
AC	8-10-8	-839	-839	---	BACK	VERT	TOTAL	---	C1
AD	10-9-12	-17	-17	---	BACK	VERT	TOTAL	---	C1
AE	12-9-12	-17	-17	---	BACK	VERT	TOTAL	---	C1
AF	14-9-12	-17	-17	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

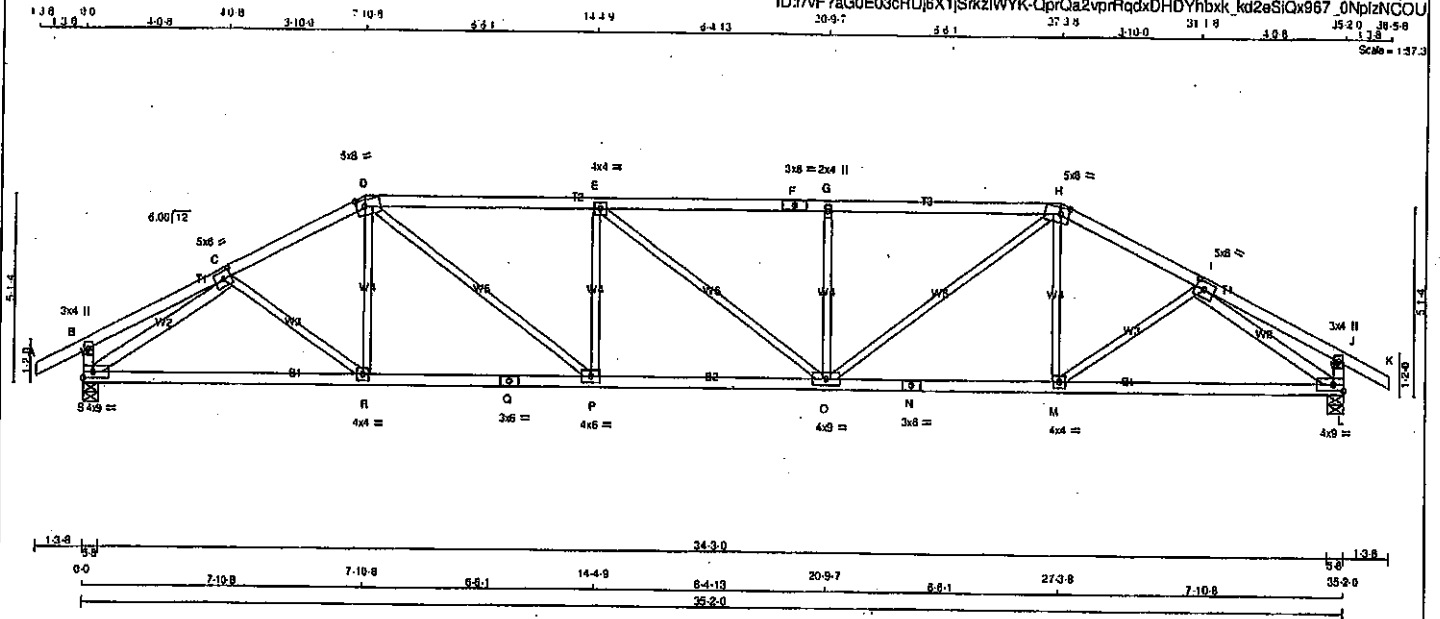


Structural component only
DWG# T-2007108

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408152	T41	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:7vF7aG0E03cRUj8X1jSrkzIWyK-QprQa2vprQdxDHOYhbxk_kd2eSiQx967_0NpizNCOU



CHORDS	SIZE	LU	MBR	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
S - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMWW-1	MT20	5.0	8.0	2.50	2.75
D TTWW-m	MT20	5.0	8.0	2.25	2.75
E TMWW-1	MT20	4.0	4.0		
F TS-1	MT20	3.0	8.0		
G TMV+w	MT20	2.0	4.0		
H TTWW-m	MT20	5.0	8.0	2.25	2.75
I TMWW-1	MT20	5.0	8.0	2.50	2.75
J TMV+p	MT20	3.0	4.0		
L BMVW-1	MT20	4.0	9.0		Edge
M BMVW-1	MT20	4.0	4.0		
N BS-1	MT20	3.0	8.0		
O BMVWW-1	MT20	4.0	9.0		
P BMVW-1	MT20	4.0	8.0		
Q BS-1	MT20	3.0	8.0		
R BMVW-1	MT20	4.0	4.0		
S BMVW-1	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
S	2063	0	5-8	5-8
L	2063	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX MIN COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1457	989 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0	0 / 0
L	1457	989 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO						FR-TO			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	C-R	0 / 93	0.03 (4)	
B-C	0 / 18	-91.8	-91.8	0.20 (1)	10.00	R-D	0 / 121	-0.04 (4)	
C-D	-2799 / 0	-91.8	-91.8	0.32 (1)	3.88	D-P	0 / 1267	0.29 (1)	
D-E	-3508 / 0	-91.8	-91.8	0.95 (1)	2.94	P-E	-643 / 0	0.25 (1)	
E-F	-3507 / 0	-91.8	-91.8	0.93 (1)	2.94	E-O	-2 / 0	0.00 (1)	
F-G	-3507 / 0	-91.8	-91.8	0.94 (1)	2.94	O-G	-643 / 0	0.25 (1)	
G-H	-3507 / 0	-91.8	-91.8	0.94 (1)	2.95	H-M	0 / 1268	0.28 (1)	
H-I	-2799 / 0	-91.8	-91.8	0.32 (1)	3.88	M-H	0 / 121	0.04 (4)	
I-J	0 / 18	-91.8	-91.8	0.20 (1)	10.00	M-I	0 / 93	0.03 (4)	
J-K	0 / 28	-91.8	-91.8	0.12 (1)	10.00	S-C	-2974 / 0	0.83 (1)	
S-B	-270 / 0	0.0	0.0	0.03 (1)	7.81	I-L	-2974 / 0	0.83 (1)	
L-J	-270 / 0	0.0	0.0	0.03 (1)	7.81				
S-R	0 / 2417	-18.5	-18.5	0.53 (1)	10.00				
R-Q	0 / 2489	-18.5	-18.5	0.54 (1)	10.00				
Q-P	0 / 2469	-18.5	-18.5	0.54 (1)	10.00				
P-O	0 / 3509	-18.5	-18.5	0.64 (1)	10.00				
O-N	0 / 2469	-18.5	-18.5	0.54 (1)	10.00				
N-M	0 / 2489	-18.5	-18.5	0.54 (1)	10.00				
M-L	0 / 2417	-18.5	-18.5	0.53 (1)	10.00				

TOTAL WEIGHT = 2 X 139 = 278 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.41")

CSI: TC=0.05/1.00 (D-E:1), BC=0.64/1.00 (O-P:1), WB=0.53/1.00 (L-I:1), SS=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618 354 1667 788 1987 1856	

PLATE PLACEMENT TOL. = 0.250 inches

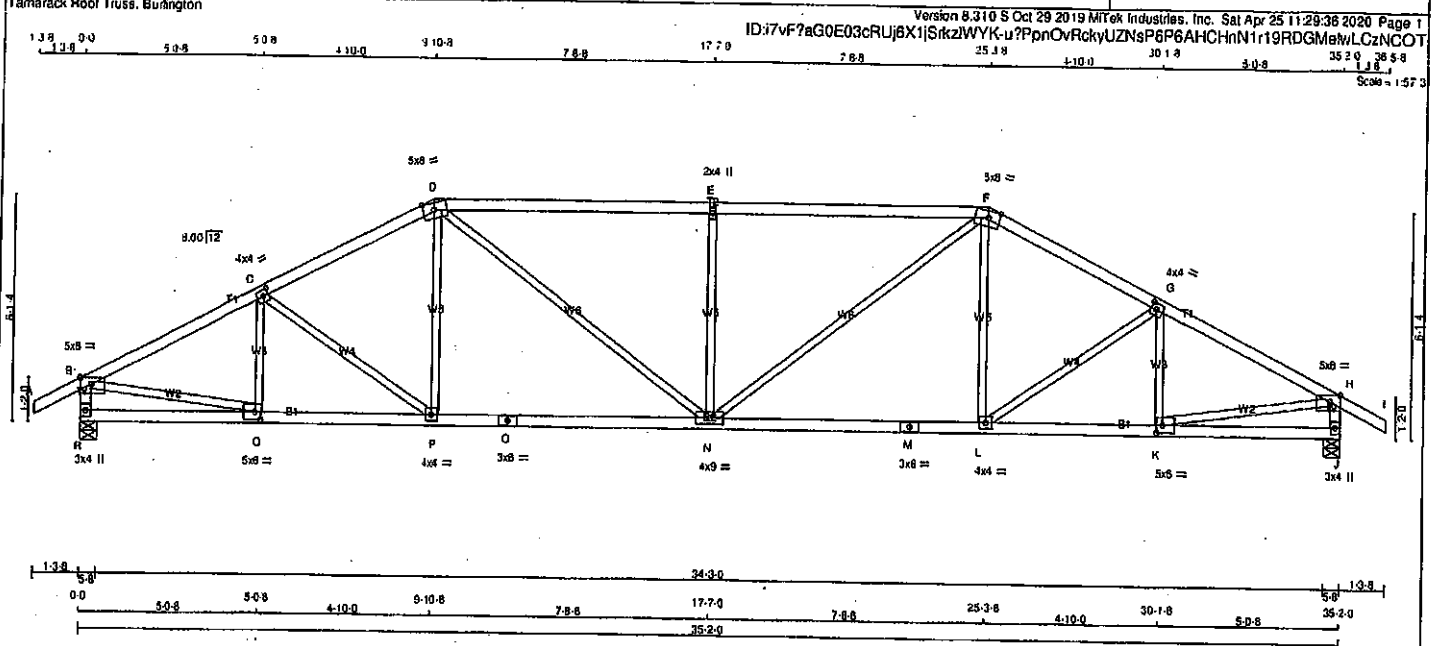
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (L) (INPUT = 0.89)
JSI METAL = 0.77 (N) (INPUT = 1.00)



Structural component only
DWG# T-2007109

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408152	T42	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 11:29:36 2020 Page 1					
ID:7vF7aG0E03cRUj6X1jSkzWYK-u?PpnOvRckyUZnsP6P6AHCHn1r19RDGMaWLCzNCOT					
Scale = 1:57.3					



LUMBER	SIZE	LUMBER	DESCR.
N. L. G. A. RULES			
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
H - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	8.0	Edge	3.50
C	TMWV-t	MT20	4.0	4.0	2.00	1.75
D	TTWV-m	MT20	5.0	8.0	2.25	3.75
E	TMWV-w	MT20	2.0	4.0		
F	TTWV-m	MT20	5.0	8.0	2.25	3.75
G	TMWV-t	MT20	4.0	4.0	2.00	1.75
H	TMWV-p	MT20	5.0	8.0	Edge	3.50
J	BMV1-p	MT20	3.0	4.0		
K	BMWV-t	MT20	5.0	8.0	2.50	2.00
L	BMWV-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	8.0		
N	BMWV-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	8.0		
P	BMWV-t	MT20	4.0	4.0		
Q	BMWV-t	MT20	5.0	8.0	2.50	2.00
R	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
R	2083	0	0	5-8
J	2083	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL
R	1457 988 / 0 0 / 0 0 / 0 0 488 / 0 0 / 0
J	1457 988 / 0 0 / 0 0 / 0 0 488 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LGI (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LGI (LC)
FR-TO						FR-TO			
A-B	0 / 28		-91.8	-91.8	0.12 (1)	10.00	Q-C	-353 / 0	0.08 (1)
B-C	-2834 / 0		-91.8	-91.8	0.39 (1)	3.81	C-P	-210 / 0	0.13 (1)
C-D	-2887 / 0		-91.8	-91.8	0.37 (1)	3.81	P-D	0 / 248	0.08 (4)
D-E	-3080 / 0		-91.8	-91.8	0.97 (1)	2.78	D-N	0 / 844	0.19 (1)
E-F	-3060 / 0		-91.8	-91.8	0.37 (1)	2.78	N-E	-872 / 0	0.51 (1)
F-G	-2887 / 0		-91.8	-91.8	0.37 (1)	3.91	N-F	0 / 844	0.19 (1)
G-H	-2834 / 0		-91.8	-91.8	0.39 (1)	3.81	L-F	0 / 248	0.08 (4)
H-I	0 / 28		-91.8	-91.8	0.12 (1)	10.00	L-G	-210 / 0	0.13 (1)
R-B	-2018 / 0		0.0	0.0	0.20 (1)	5.94	K-Q	-353 / 0	0.08 (1)
J-H	-2018 / 0		0.0	0.0	0.20 (1)	5.94	B-Q	0 / 2591	0.58 (1)
R-Q	0 / 0		-18.5	-18.5	0.10 (4)	10.00	K-H	0 / 2591	0.58 (1)
Q-P	0 / 2555		-18.5	-18.5	0.51 (1)	10.00			
P-O	0 / 2388		-18.5	-18.5	0.49 (1)	10.00			
O-N	0 / 2388		-18.5	-18.5	0.49 (1)	10.00			
N-M	0 / 2388		-18.5	-18.5	0.49 (1)	10.00			
M-L	0 / 2388		-18.5	-18.5	0.49 (1)	10.00			
L-K	0 / 2555		-18.5	-18.5	0.51 (1)	10.00			
K-J	0 / 0		-18.5	-18.5	0.10 (4)	10.00			

TOTAL WEIGHT = 2 X 139 = 277 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.17")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.17")
ALLOWABLE DEFL.(TL) = $L/360$ (1.17")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.34")

CS: TC=0.97/1.00 (D-E:1), BC=0.51/1.00 (K-L:1),
WB=0.58/1.00 (H-K:1), SS=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

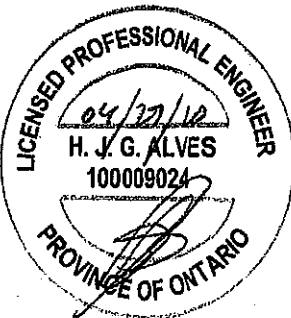
NAIL VALUES

PLATE GRIP(ORY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

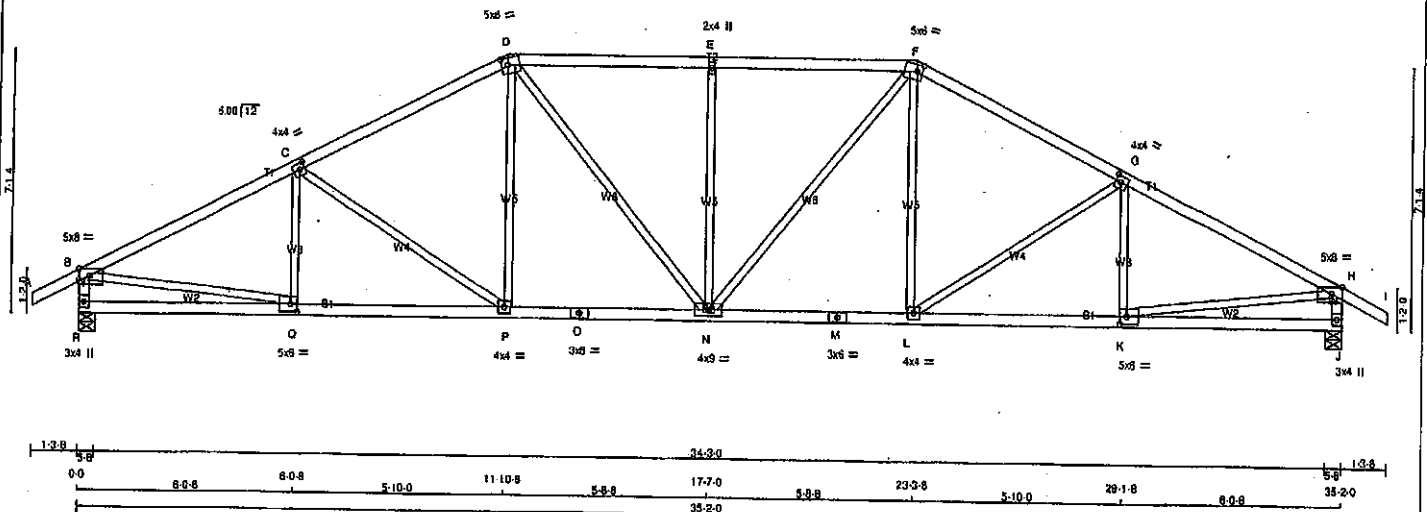
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.75 (O) (INPUT = 1.00)



Structural component only
DWG# T-2007110

JOB NAME 408152	TRUSS NAME T43	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Sat Apr 25 11:29:37 2020 Page 1 ID:17vF7aG0E03cRUj6X1SrkzIWYK-MCzB kw3N24LBWQcg8dPpQzFRBxutKPbiVTIezNCOS	



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - H	2x4	DRY	No.2
J - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - L	2x4	DRY	No.2
L - K	2x4	DRY	No.2
K - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMWW-p	MT20	5.0	8.0 Edge 3.50
C	TMWW-i	MT20	4.0	4.0 2.00 1.75
D	TTWW-m	MT20	5.0	8.0 2.25 2.00
E	TMWW-w	MT20	2.0	4.0
F	TTWW-m	MT20	5.0	8.0 2.25 2.00
G	TMWW-i	MT20	4.0	4.0 2.00 1.75
H	TMWW-p	MT20	5.0	8.0 Edge 3.50
J	BMV1-p	MT20	3.0	4.0
K	BMWW-i	MT20	5.0	8.0 2.50 2.25
L	BMWW-i	MT20	4.0	4.0
M	BS-i	MT20	3.0	8.0
N	BMWW-i	MT20	4.0	9.0
O	BS-i	MT20	3.0	8.0
P	BMWW-i	MT20	4.0	4.0
Q	BMWW-i	MT20	5.0	8.0 2.50 2.25
R	BMV1-p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT REQD BRG
JT	VERT	DOWN	BRG
R	2083	0	5-8
J	2083	0	5-8

UNFACTORED REACTIONS			
1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
R	1457	989 / 0	0 / 0
J	1457	989 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)
FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)
B-C	-2889 / 0	-91.8	-91.8 0.58 (1)
C-D	-2539 / 0	-91.8	-91.8 0.52 (1)
D-E	-2559 / 0	-91.8	-91.8 0.48 (1)
E-F	-2559 / 0	-91.8	-91.8 0.48 (1)
F-G	-2559 / 0	-91.8	-91.8 0.52 (1)
G-H	-2889 / 0	-91.8	-91.8 0.58 (1)
H-I	0 / 28	-91.8	-91.8 0.12 (1)
R-B	-2014 / 0	0.0	0.0 0.20 (1)
J-H	-2014 / 0	0.0	0.0 0.20 (1)
R-Q	0 / 0	-18.5	-18.5 0.15 (4)
Q-P	0 / 2609	-18.5	-18.5 0.49 (1)
P-O	0 / 2249	-18.5	-18.5 0.43 (1)
O-N	0 / 2249	-18.5	-18.5 0.43 (1)
N-M	0 / 2249	-18.5	-18.5 0.43 (1)
M-L	0 / 2249	-18.5	-18.5 0.43 (1)
L-K	0 / 2609	-18.5	-18.5 0.49 (1)
K-J	0 / 0	-18.5	-18.5 0.15 (4)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBCG 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSK TC=0.58/1.00 (G-H-1), BC=0.49/1.00 (K-L-1),
WB=0.59/1.00 (B-Q-1), SS=0.25/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

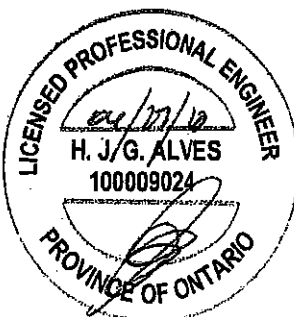
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PLI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (Q) (INPUT = 0.90)
JSI METAL = 0.69 (M) (INPUT = 1.00)



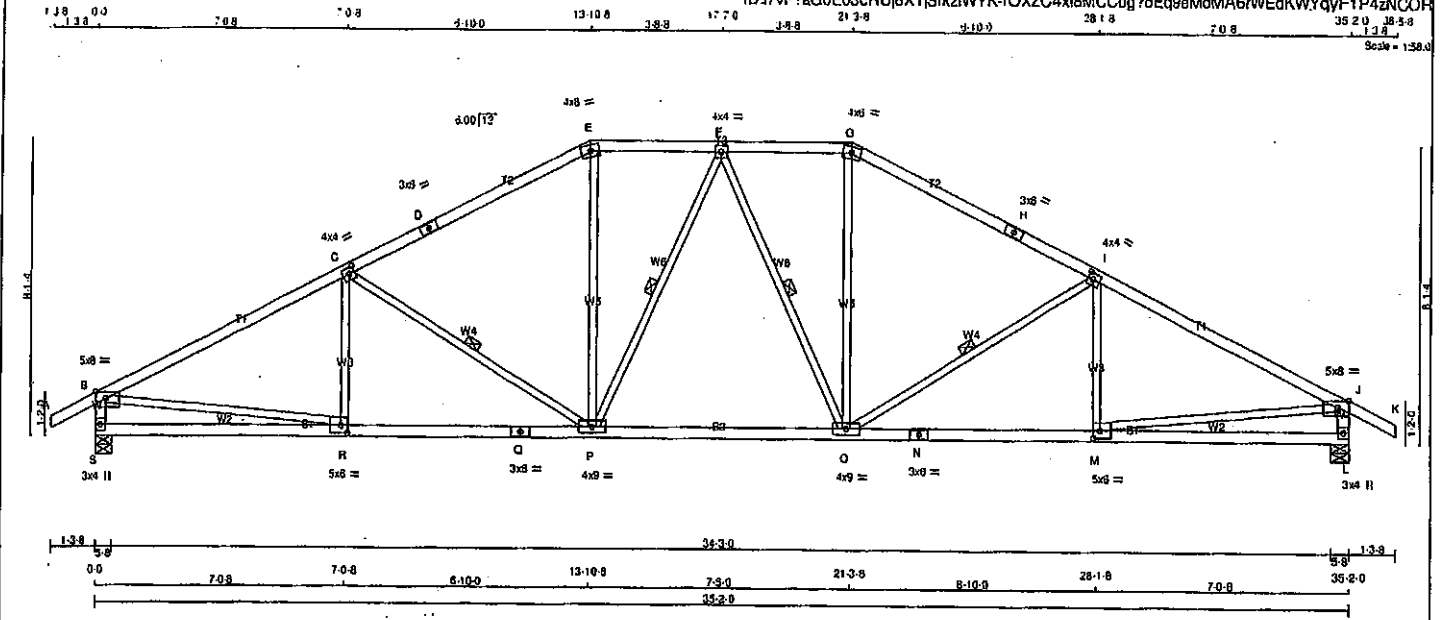
Structural component only
DWG# T-2007111

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408152	T44	2	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - K	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - B	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWV-p	MT20	5.0	8.0	Edge 3.50
C	TMWV-I	MT20	4.0	4.0	2.00 1.75
D	TS-I	MT20	3.0	6.0	
E	TTW-m	MT20	4.0	8.0	
F	TMWV-I	MT20	4.0	4.0	
G	TTW-m	MT20	4.0	8.0	
H	TS-I	MT20	3.0	6.0	
I	TMWV-I	MT20	4.0	4.0	2.00 1.75
J	TMWV-p	MT20	5.0	8.0	Edge 3.50
L	BMV-I-p	MT20	3.0	4.0	
M	BMWV-I	MT20	5.0	8.0	2.50 2.25
N	BS-I	MT20	3.0	6.0	
O	BMWV-I	MT20	4.0	9.0	
P	BMWV-I	MT20	4.0	9.0	
Q	BS-I	MT20	3.0	6.0	
R	BMWV-I	MT20	5.0	8.0	2.50 2.25
S	BMV-I-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
S	2063	0	2063	0	5-8	5-8	5-8	5-8	5-8
L	2063	0	2063	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1457	989 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
L	1457	989 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-P, F-Q, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. MAX. UNBRACED LENGTH (FT)	CSF (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8 0.12 (1)	10.00	R-C	-182 / 52	0.08 (1)	
B-C	-2906 / 0	-91.8 -91.8 0.83 (1)	3.25	C-P	-637 / 0	0.29 (1)	
C-D	-2373 / 0	-91.8 -91.8 0.72 (1)	3.66	P-E	0 / 674	0.15 (1)	
D-E	-2373 / 0	-91.8 -91.8 0.72 (1)	3.66	F-Q	-222 / 0	0.11 (1)	
E-F	-2100 / 0	-91.8 -91.8 0.19 (1)	4.53	O-I	-637 / 0	0.29 (1)	
F-G	-2100 / 0	-91.8 -91.8 0.19 (1)	4.53	G-H	0 / 674	0.15 (1)	
G-H	-2373 / 0	-91.8 -91.8 0.72 (1)	3.66	H-I	-182 / 52	0.08 (1)	
H-I	-2373 / 0	-91.8 -91.8 0.72 (1)	3.66	I-J	0 / 2849	0.60 (1)	
I-J	-2906 / 0	-91.8 -91.8 0.83 (1)	3.25	J-K	0 / 2849	0.60 (1)	
J-K	0 / 28	-91.8 -91.8 0.12 (1)	10.00	K-L	0 / 2849	0.60 (1)	
K-L	-2006 / 0	0.0 0.0 0.20 (1)	5.96				
L-M	-2006 / 0	0.0 0.0 0.20 (1)	5.96				
M-N	0 / 0	-18.5 -18.5 0.20 (4)	10.00				
N-O	0 / 2830	-18.5 -18.5 0.52 (1)	10.00				
O-P	0 / 2830	-18.5 -18.5 0.52 (1)	10.00				
P-Q	0 / 2193	-18.5 -18.5 0.45 (1)	10.00				
Q-R	0 / 2830	-18.5 -18.5 0.52 (1)	10.00				
R-S	0 / 2830	-18.5 -18.5 0.52 (1)	10.00				
S-T	0 / 0	-18.5 -18.5 0.20 (4)	10.00				

TOTAL WEIGHT = 2 X 145 = 291 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/999 (0.14")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = L/999 (0.30")

CSI: TC=0.83/1.00 (I-J), BC=0.52/1.00 (M-O), WB=0.60/1.00 (B-R), SSI=0.28/1.00 (H-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1997 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (M) (INPUT = 0.90)
JSI METAL = 0.79 (N) (INPUT = 1.00)



Structural component only
DWG# T-2007112

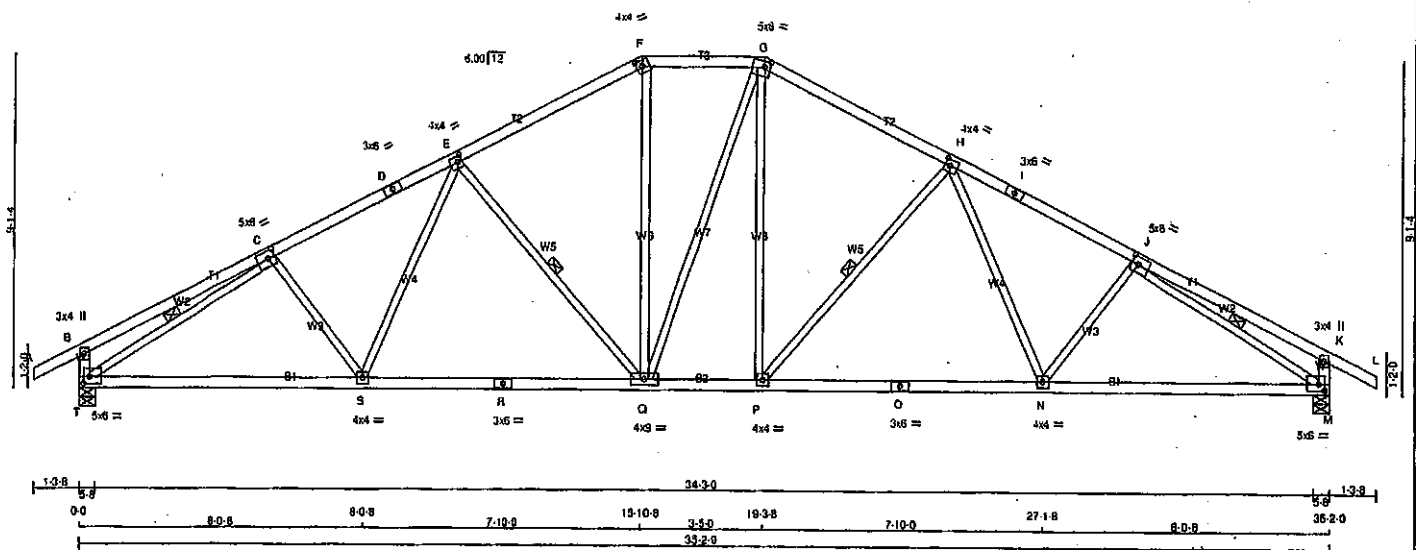
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC	TRUSS DESC.	DRWG NO.
408152	T45	2	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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1.38 3.0 3.3 3.3 10.7 13 3.2 11 15.10 3.50 19.38 5.2 11 24.9 3 5.2 11 29.9 13 35.2 0 38.5 8
1.38 5.5 3.3 3.3 10.7 13 3.2 11 15.10 3.50 19.38 5.2 11 24.9 3 5.2 11 29.9 13 35.2 0 38.5 8
Scale = 1/8" = 1'-0"



LUMBER	SIZE	DRY	No.2	DESCR.
N. L. G. A. RULES				
CHORDS				
A - D	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
L - R	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0	2.50	2.25
D	TS-1	MT20	3.0	6.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.50
F	TTW-t	MT20	4.0	4.0	2.00	1.75
G	TTWW-m	MT20	5.0	6.0	2.00	2.00
H	TMVW-t	MT20	4.0	4.0	2.00	1.50
I	TS-1	MT20	3.0	6.0		
J	TMVW-t	MT20	5.0	6.0	2.50	2.25
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	2.25	2.00
N, P, S						
N	BMVW-t	MT20	4.0	4.0		
O	BS-1	MT20	3.0	6.0		
Q	BMVW-t	MT20	4.0	4.0		
R	BS-1	MT20	3.0	6.0		
T	BMVW1-t	MT20	5.0	6.0	2.25	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD IN-SX
JT	VERT	HORZ	DOWN	HORZ
T	2063	0	2063	0
M	2063	0	2063	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1457	989 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
M	1457	989 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, H-P, C-T, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	C-S	-110 / 37	0.04 (1)	
B-C	0 / 19	-91.8	-91.8	0.32 (1)	10.00	S-E	0 / 276	0.06 (1)	
C-D	-2759 / 0	-91.8	-91.8	0.40 (1)	3.88	E-Q	-881 / 0	0.31 (1)	
D-E	-2759 / 0	-91.8	-91.8	0.40 (1)	3.88	Q-F	0 / 813	0.14 (1)	
E-F	-2167 / 0	-91.8	-91.8	0.38 (1)	4.30	Q-G	0 / 4	0.00 (1)	
F-G	-1925 / 0	-91.8	-91.8	0.18 (1)	4.71	P-H	0 / 609	0.14 (1)	
G-H	-2168 / 0	-91.8	-91.8	0.38 (1)	4.30	P-H	-682 / 0	0.31 (1)	
H-I	-2760 / 0	-91.8	-91.8	0.40 (1)	3.88	H-N	0 / 276	0.06 (1)	
I-J	-2760 / 0	-91.8	-91.8	0.40 (1)	3.88	N-J	-110 / 37	0.04 (1)	
J-K	0 / 19	-91.8	-91.8	0.32 (1)	10.00	T-C	-3040 / 0	0.85 (1)	
K-L	0 / 28	-91.8	-91.8	0.12 (1)	10.00	J-M	-3041 / 0	0.85 (1)	
T-B	-328 / 0	0.0	0.0	0.03 (1)	7.81				
M-K	-328 / 0	0.0	0.0	0.03 (1)	7.81				
T-S	0 / 2536	-18.5	-18.5	0.55 (1)	10.00				
S-R	0 / 2362	-18.5	-18.5	0.53 (1)	10.00				
R-Q	0 / 2362	-18.5	-18.5	0.53 (1)	10.00				
Q-P	0 / 1924	-18.5	-18.5	0.40 (1)	10.00				
P-O	0 / 2362	-18.5	-18.5	0.53 (1)	10.00				
O-N	0 / 2362	-18.5	-18.5	0.53 (1)	10.00				
N-M	0 / 2537	-18.5	-18.5	0.56 (1)	10.00				

TOTAL WEIGHT = 2 X 151 = 302 b

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.32")

CSI: TC=0.40/1.00 (H-J:1), BC=0.50/1.00 (M-N:1), WB=0.85/1.00 (J-M:1), SS=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1897 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

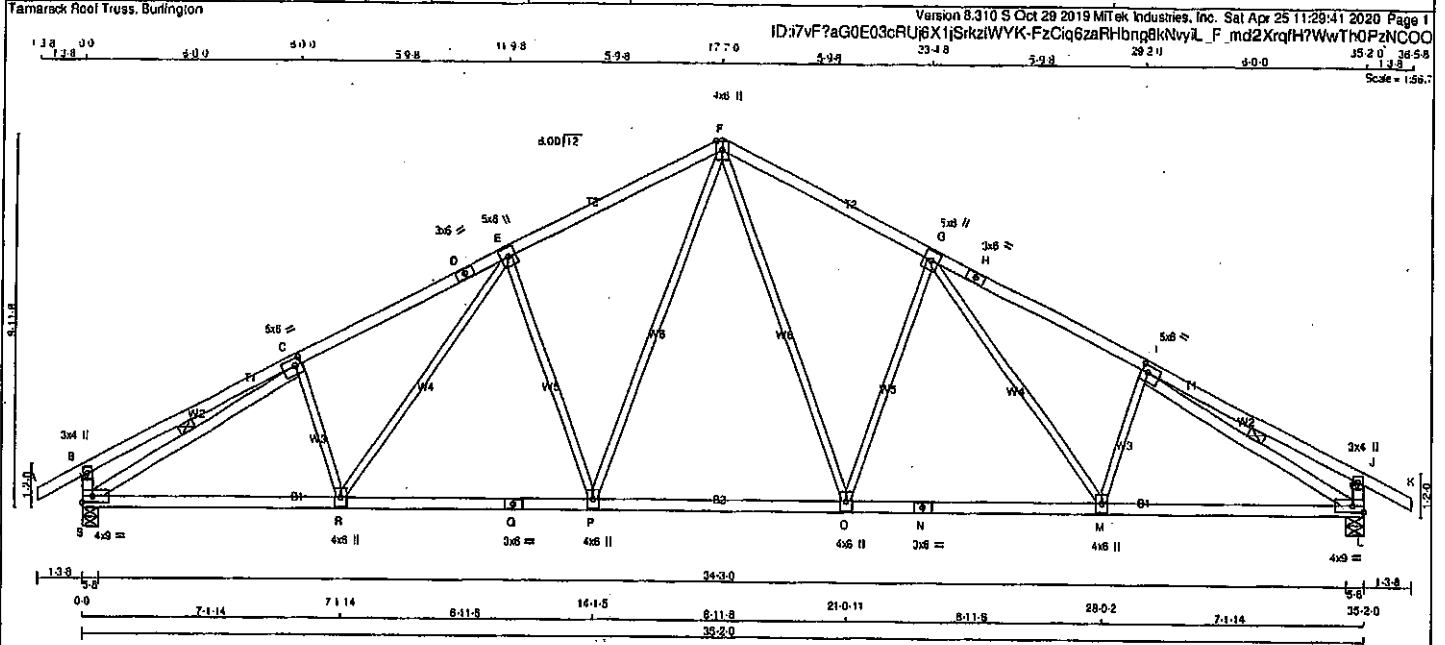
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.83 (O) (INPUT = 1.00)



Structural component only
DWG# T-2007113

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408152	T46	6	1	TRUSS DESC.		



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - K	2x4	DRY No.2	SPF
S - B	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
S - Q	2x4	DRY No.2	SPF
Q - N	2x4	DRY No.2	SPF
N - L	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
S - C	2x4	DRY No.2	SPF
I - L	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV+p	MT20	3.0	4.0	
C TMWW-i	MT20	5.0	8.0	2.25 2.00
D TS-1	MT20	3.0	6.0	
E TMWW+i	MT20	5.0	8.0	
F TTWW+p	MT20	4.0	6.0	Edge
G TMWW-i	MT20	5.0	8.0	
H TS-1	MT20	3.0	6.0	
I TMWW-i	MT20	5.0	8.0	2.25 2.00
J TMV+p	MT20	3.0	4.0	
L BMWW-i	MT20	4.0	9.0	Edge
M, O, P, R				
M BMWW+i	MT20	4.0	6.0	
N BS-1	MT20	3.0	6.0	
Q BS-1	MT20	3.0	6.0	
S BMWW-i	MT20	4.0	9.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
S	2063	0	2063	0	0	5-8	5-8
L	2063	0	2063	0	0	5-8	5-8

UNFACTORED REACTIONS		MAX/MIN COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	DEAD	SOIL
S	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0	488 / 0	0 / 0
L	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-S, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	F-O	0 / 866	0.19 (1)
B-G	0 / 22	-91.8	-91.8 0.40 (1)	10.00	O-G	-728 / 0	0.72 (1)
C-D	-2808 / 0	-91.8	-91.8 0.49 (1)	3.75	G-M	0 / 391	0.09 (1)
D-E	-2808 / 0	-91.8	-91.8 0.49 (1)	3.75	M-I	-192 / 18	0.05 (1)
E-F	-2322 / 0	-91.8	-91.8 0.46 (1)	4.07	P-F	0 / 866	0.19 (1)
F-G	-2322 / 0	-91.8	-91.8 0.46 (1)	4.07	E-P	-728 / 0	0.72 (1)
G-H	-2808 / 0	-91.8	-91.8 0.49 (1)	3.75	R-E	0 / 391	0.09 (1)
H-I	-2808 / 0	-91.8	-91.8 0.49 (1)	3.75	C-R	-192 / 18	0.05 (1)
I-J	0 / 22	-91.8	-91.8 0.40 (1)	10.00	S-C	-3064 / 0	0.72 (1)
J-K	0 / 28	-91.8	-91.8 0.12 (1)	10.00	I-L	-3064 / 0	0.72 (1)
S-B	-344 / 0	0.0	0.0 0.03 (1)	7.81			
L-J	-344 / 0	0.0	0.0 0.03 (1)	7.81			
S-R	0 / 2574	-18.5	-18.5 0.53 (1)	10.00			
R-O	0 / 2291	-18.5	-18.5 0.47 (1)	10.00			
O-P	0 / 2291	-18.5	-18.5 0.47 (1)	10.00			
P-O	0 / 1762	-18.5	-18.5 0.38 (1)	10.00			
O-N	0 / 2291	-18.5	-18.5 0.47 (1)	10.00			
N-M	0 / 2291	-18.5	-18.5 0.47 (1)	10.00			
M-L	0 / 2574	-18.5	-18.5 0.53 (1)	10.00			

TOTAL WEIGHT = 8 X 152 = 910 lb (M/F)

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TO=0.49/1.00 (C-E:1), BC=0.53/1.00 (L-M:1), WB=0.72/1.00 (E-P:1), SS=0.22/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

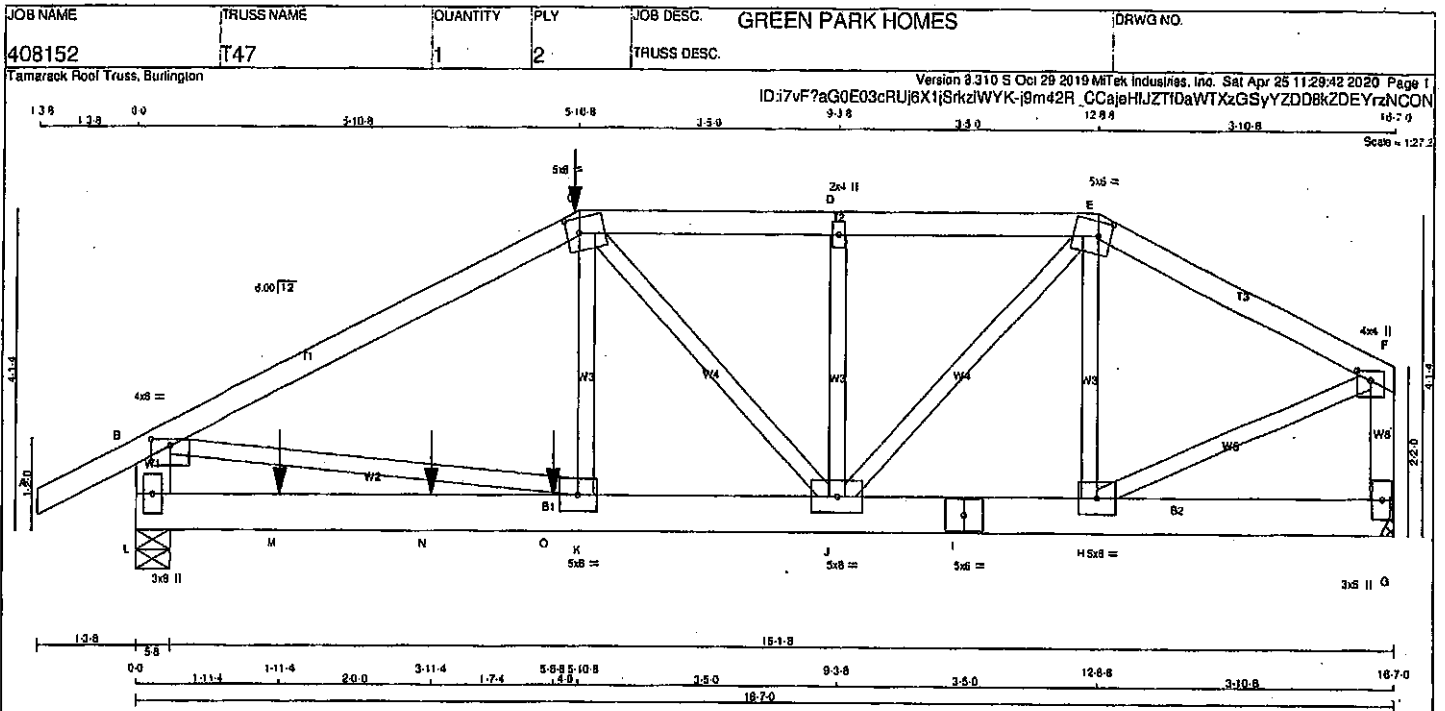
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)

JSI METAL= 0.77 (I) (INPUT = 1.00)



Structural component only
DWG# T-2007114



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - F	2x4	DRY No.2	SPF
L - B	2x6	DRY No.2	SPF
G - F	2x4	DRY No.2	SPF
L - I	2x6	DRY No.2	SPF
I - G	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE (61.0)
C - E	12	SIDE (61.0)
E - F	12	TOP
G - F	12	TOP
L - B	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L - I	12	SIDE (0.0)
I - G	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	BRG	IN-SX	IN-SX
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
L	2005	0	2005	0	5-8	5-8		
G	1397	0	1397	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 2-0.

UNFACTORED REACTIONS

1ST LC CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM LIVE WIND DEAD SOIL
L	1412 958 / 0 0 / 0 0 / 0 0 457 / 0 0 / 0
G	985 662 / 0 0 / 0 0 / 0 0 323 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.18 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8 0.07 (1)	10.00	K-C	0 / 943	0.12 (1)	
B-C	-2843 / 0	-91.8 -91.8 0.37 (1)	5.18	G-J	-848 / 0	0.13 (1)	
C-D	-1951 / 0	-91.8 -91.8 0.10 (1)	6.17	J-D	-371 / 0	0.08 (1)	
D-E	-1951 / 0	-91.8 -91.8 0.10 (1)	6.17	J-E	0 / 1047	0.13 (1)	
E-F	-1405 / 0	-91.8 -91.8 0.14 (1)	8.25	H-E	-446 / 0	0.08 (1)	
L-B	-1885 / 0	0.0 0.0 0.07 (1)	7.81	B-K	0 / 2382	0.29 (1)	
G-F	-1369 / 0	0.0 0.0 0.08 (1)	7.81	H-F	0 / 1375	0.17 (1)	
L-M	0 / 0	-18.5 -18.5 0.10 (1)	10.00				
M-N	0 / 0	-18.5 -18.5 0.10 (1)	10.00				
N-O	0 / 0	-18.5 -18.5 0.10 (1)	10.00				
O-K	0 / 0	-18.5 -18.5 0.10 (1)	10.00				
K-J	0 / 2389	-18.5 -18.5 0.24 (1)	10.00				
J-I	0 / 1243	-18.5 -18.5 0.11 (1)	10.00				
I-H	0 / 1243	-18.5 -18.5 0.11 (1)	10.00				
H-G	0 / 0	-18.5 -18.5 0.02 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIF.	TYPE	HEEL	CONN.
C	5-10-8	-320	-320	---	FRONT	VERT	TOTAL	---	C1
M	1-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
N	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	5-6-8	-1076	-1076	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 78 = 156 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
 OL = 8.0 PSF
 BOT CH. LL = 0.0 PSF
 OL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBCC 2010, OBC 2012, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 088-09, CSA 088-14
 - TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.55")
 CALCULATED VERT. DEFL. (LL) = U/999 (0.03")
 ALLOWABLE DEFL. (TL) = L/360 (0.55")
 CALCULATED VERT. DEFL. (TL) = U/999 (0.06")

CSI: TC=0.37/1.00 (B-C:1), BC=0.24/1.00 (J-K:1),
 WB=0.29/1.00 (B-K:1), SSI=0.37/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

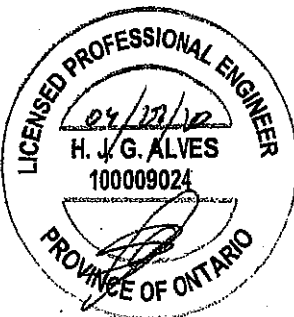
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354 1667 768 1987 1656		

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.80 (B) (INPUT = 0.90)
 JSI METAL = 0.32 (B) (INPUT = 1.00)



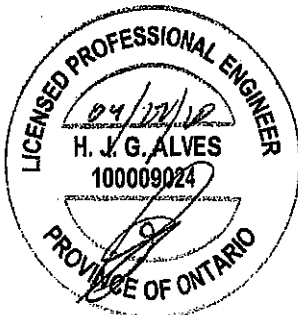
Structural component only
 DWG# T-2007115

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408152	T47	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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ID:17vF?aG0E03cRUI8X1ISkzIWK-19m42R Cc1eHLZTIDaWTXzGSyYZDD8KZDEYzNCON

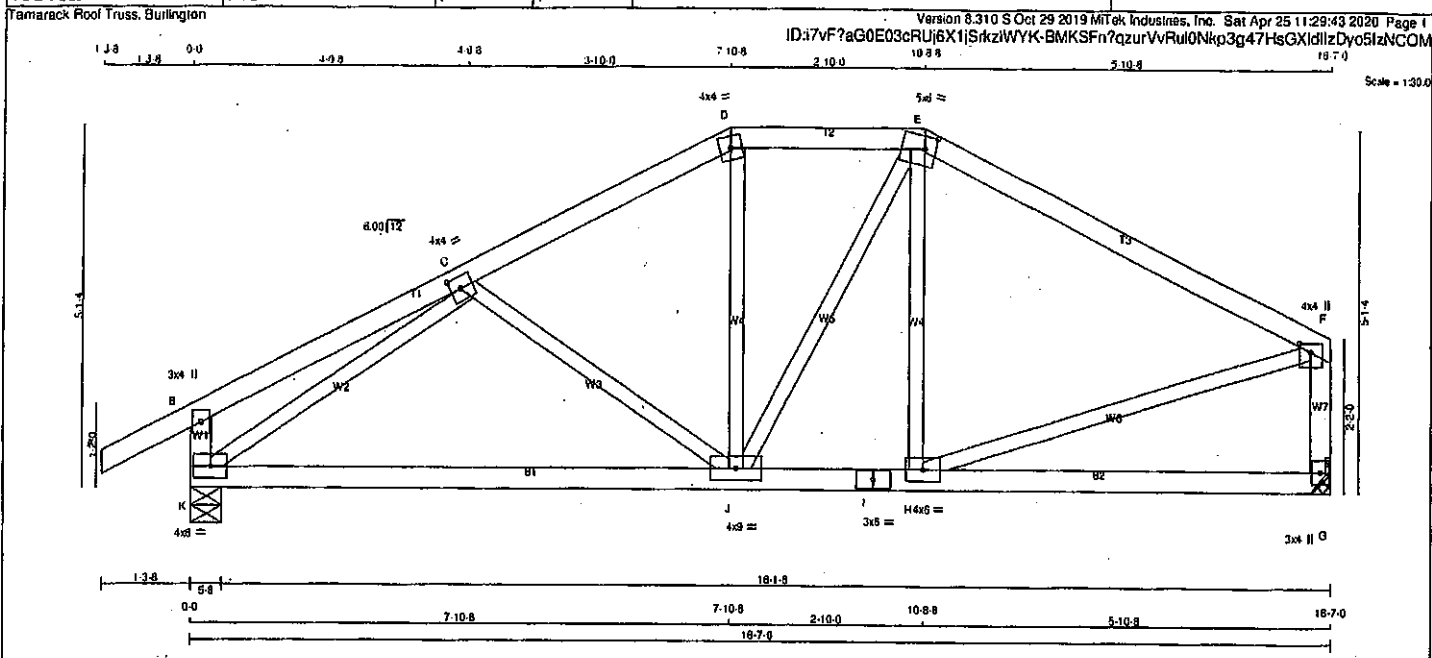
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.00
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMW-w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMVW-p	MT20	4.0	4.0	1.50	2.00
G	BMV1-p	MT20	3.0	6.0		
H	BMWW-i	MT20	5.0	6.0		
I	BS-i	MT20	6.0	6.0		
J	BMWWW-i	MT20	5.0	6.0		
K	BMWW-i	MT20	5.0	6.0		
L	BMV1-p	MT20	3.0	6.0		



Structural component only
DWG# T-2007115 *ML*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408152	T48	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMWW+1	MT20	4.0	4.0 2.00 1.75
D	TTW-m	MT20	4.0	4.0
E	TTWW-m	MT20	5.0	6.0 2.25 2.00
F	TMVW+p	MT20	4.0	4.0 1.50 2.00
G	BMV1+p	MT20	3.0	4.0
H	BMWW+1	MT20	4.0	6.0
I	BS-1	MT20	3.0	6.0
J	BMWWW+1	MT20	4.0	9.0
K	BMVW1+1	MT20	4.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX	IN-SX	REGRD
K	1039	0	1039	0	0	5-8	5-8		
G	914	0	914	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS							
JT	1ST CASE	2ND CASE	3RD CASE	4TH CASE	5TH CASE	6TH CASE	7TH CASE
K	732	494 / 0	0 / 0	0 / 0	0 / 0	238 / 0	0 / 0
G	646	424 / 0	0 / 0	0 / 0	0 / 0	222 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	FACTORED HORIZ. LOAD (PL)	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	FACTORED HORIZ. LOAD (PL)	MAX. FACTORED FORCE (LBS)
FR-TO					FR-TO				
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	C-J	-232 / 0	0.10 (1)		
B-C	0 / 18	-91.8	-91.8 0.22 (1)	10.00	J-D	0 / 134	0.03 (4)		
C-D	-918 / 0	-91.8	-91.8 0.18 (1)	8.24	J-E	0 / 152	0.03 (1)		
D-E	-808 / 0	-91.8	-91.8 0.10 (1)	8.25	H-E	-183 / 0	0.06 (1)		
E-F	-821 / 0	-91.8	-91.8 0.42 (1)	8.12	K-C	-1221 / 0	0.47 (1)		
K-B	-265 / 0	0.0	0.0 0.03 (1)	7.81	H-F	0 / 769	0.17 (1)		
G-F	-868 / 0	0.0	0.0 0.10 (1)	7.81					
K-J	0 / 993	-18.5	-18.5 0.32 (4)	10.00					
J-I	0 / 731	-18.5	-18.5 0.31 (4)	10.00					
I-H	0 / 731	-18.5	-18.5 0.31 (4)	10.00					
H-G	0 / 0	-18.5	-18.5 0.16 (4)	10.00					

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH. LL	=	25.6	PSF
DL	=	8.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF
SPACING	=	24.0	IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.55")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.55")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.13")

CS: TC=0.42/1.00 (E-F:1), BC=0.32/1.00 (J-K:4), WB=0.47/1.00 (C-K:1), SS=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP (DRY)	SHEAR	SECTION	
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 788	1887 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (C) (INPUT = 0.90)
JSI METAL = 0.41 (C) (INPUT = 1.00)

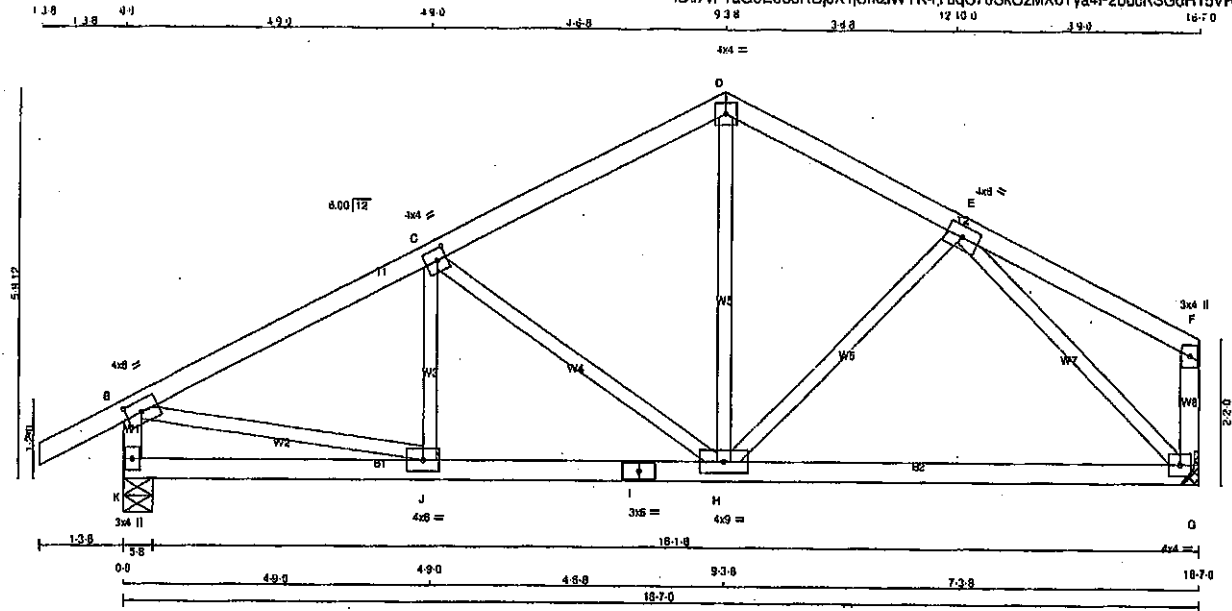


Structural component only
DWG# T-2007116

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408152	T49	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:7vF7aG0E03cRUj8X1jSkzWYK-YuqS70SkCzMXbTy4F2buoKSGdH15VRCILdkzNCO



TOTAL WEIGHT = 67 lb
(M/F)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	6.0	2.00	3.00
C	TMVW-1	MT20	4.0	4.0	2.00	1.75
D	TTW-p	MT20	4.0	4.0		
E	TMVW-1	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW-1	MT20	4.0	4.0		
H	BMVW-1	MT20	4.0	9.0		
I	BS-1	MT20	3.0	6.0		
J	BMVW-1	MT20	4.0	6.0		
K	BMV-1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
K	1039	0	1039	0
G	914	0	914	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	732	494/0	0/0	0/0	0/0	238/0	0/0
G	646	424/0	0/0	0/0	0/0	222/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	J-C	-114 / 23	0.02 (1)
B-C	-1135 / 0	-91.8	-91.8	0.26 (1)	5.68	G-H	-418 / 0	0.22 (1)
C-D	-799 / 0	-91.8	-91.8	0.25 (1)	8.25	H-D	0 / 407	0.09 (1)
D-E	-793 / 0	-91.8	-91.8	0.15 (1)	6.25	H-E	-39 / 37	0.02 (1)
E-F	0 / 17	-91.8	-91.8	0.19 (1)	10.00	B-J	0 / 1051	0.24 (1)
K-B	-998 / 0	0.0	0.0	0.10 (1)	7.81	E-G	-1026 / 0	0.44 (1)
G-F	-129 / 0	0.0	0.0	0.02 (1)	7.81			
K-J	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
J-I	0 / 1034	-18.5	-18.5	0.29 (4)	10.00			
I-H	0 / 1034	-18.5	-18.5	0.29 (4)	10.00			
H-G	0 / 722	-18.5	-18.5	0.28 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.28/1.00 (B-C:1), BC=0.29/1.00 (H-J:4),
WB=0.44/1.00 (E-G:1), SS=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

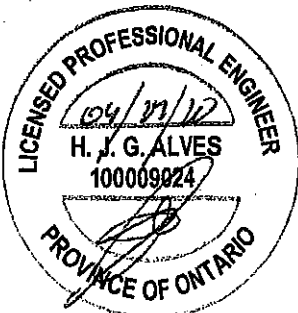
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1687
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (G) (INPUT = 0.90)
JSI METAL = 0.36 (H) (INPUT = 1.00)

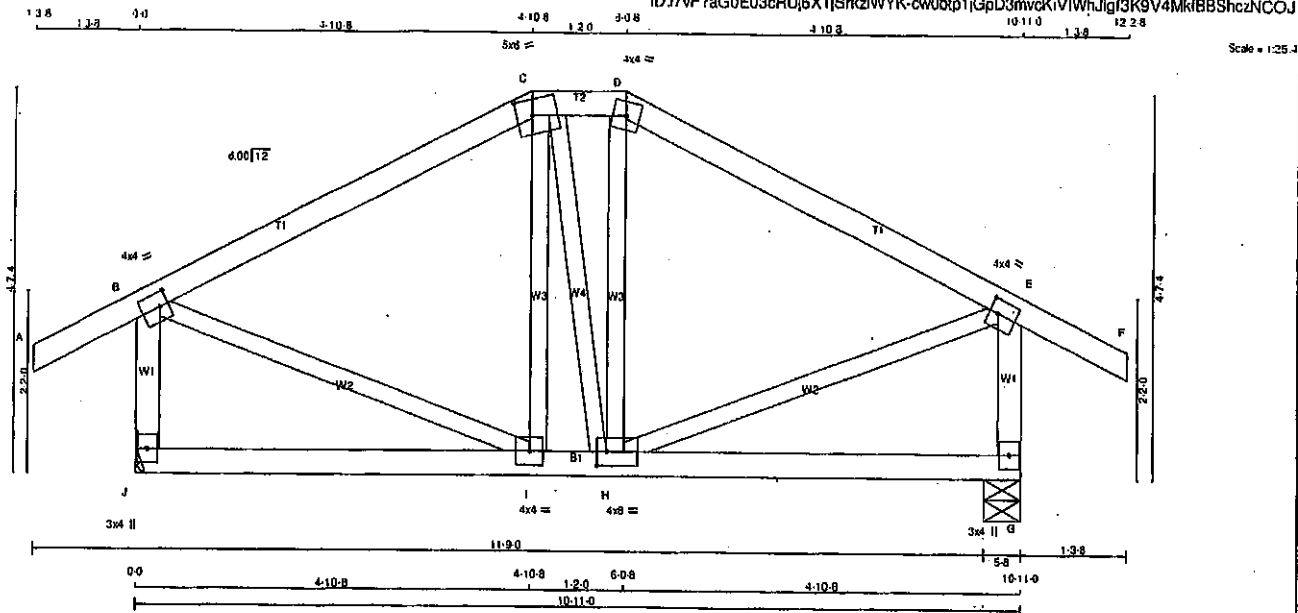


Structural component only
DWG# T-2007117

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408152	T51	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:7vF?ag0E03cRUj6X1iSrkzWYK-cw0bplGpD3mvckIvIWhJlgf3K9V4MkIBBSheNCOJ



TOTAL WEIGHT = 52 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-i	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	6.0	2.25	2.25
D	TTW-m	MT20	4.0	4.0		
E	TMVW-i	MT20	4.0	4.0	2.00	1.25
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-i	MT20	4.0	6.0	2.00	1.50
I	BMWW-i	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
J	726	0	726	0
G	726	0	726	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	DEAD	SOIL
J	511	349/0	0/0	0/0
G	511	349/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/28	-91.8	-91.8 0.12 (1)	10.00	I-C	-78/18	0.02 (1)
B-C	-435/0	-91.8	-91.8 0.28 (1)	6.25	C-H	-4/0	0.00 (1)
C-D	-368/0	-91.8	-91.8 0.02 (1)	6.25	H-D	-82/19	0.03 (1)
D-E	-434/0	-91.8	-91.8 0.28 (1)	6.25	B-I	0/415	0.09 (1)
E-F	0/28	-91.8	-91.8 0.12 (1)	10.00	H-E	0/414	0.09 (1)
J-B	-688/0	0.0	0.0 0.08 (1)	7.81			
G-E	-688/0	0.0	0.0 0.08 (1)	7.81			
J-I	0/0	-18.5	-18.5 0.10 (4)	10.00			
I-H	0/387	-18.5	-18.5 0.14 (4)	10.00			
H-G	0/0	-18.5	-18.5 0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.09/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014:

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.36")
CALCULATED VERT. DEFL. (LL) = U/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.36")
CALCULATED VERT. DEFL. (TL) = U/999 (0.02")

CS1: TC=0.28/1.00 (B-C:1), BC=0.14/1.00 (H-I:1), WB=0.09/1.00 (B-I:1), SS=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

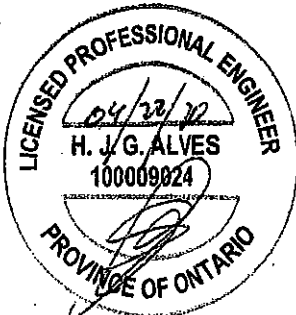
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX	MIN	MAX
MT20	618	354
	1687	788
	1887	1850

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

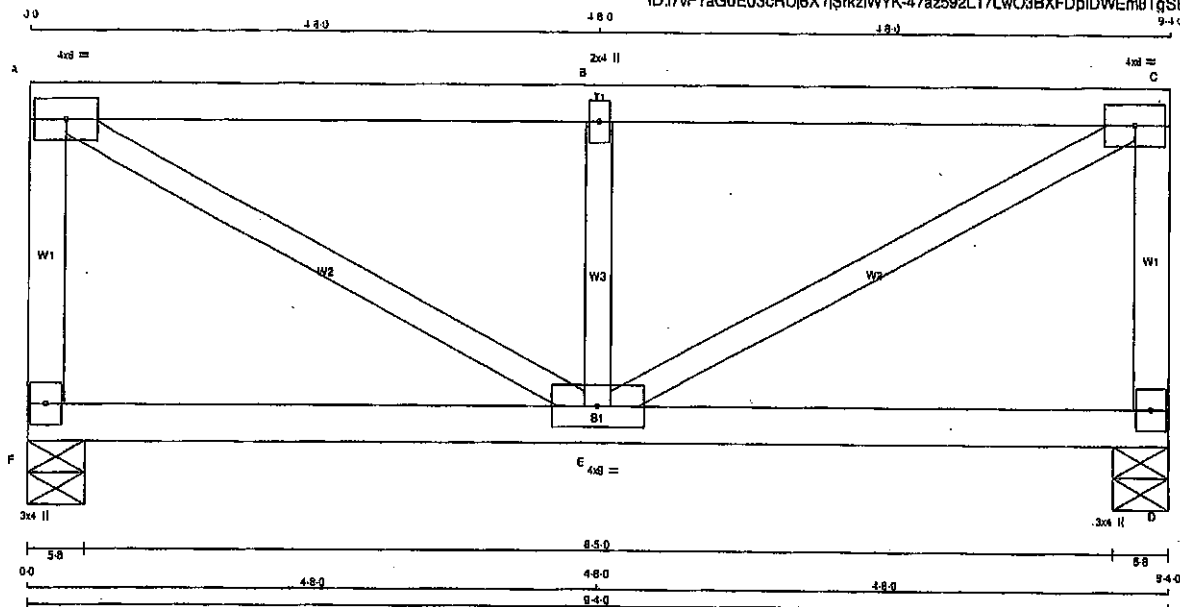
JSI GRIP = 0.68 (B) (INPUT = 0.90)
JSI METAL = 0.22 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007119

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408152	T52	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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ID: i7vF?aG0E03cRUj6X1j5kziWYK-47az592L17LwO3BXFDpIDWEb8TgSEVLturw?E3zNCOI



Scale = 1:16.0

TOTAL WEIGHT = 38 lb (MIR)

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY No.2	SPF
A - C	2x4	DRY No.2	SPF
D - C	2x4	DRY No.2	SPF
F - D	2x4	DRY No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF			
DRY: SEASONED LUMBER.			

PLATES (tablets in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW-I	MT20	4.0	8.0
B	TMVW-w	MT20	2.0	4.0
C	TMVW-I	MT20	4.0	8.0
D	BMV1-p	MT20	3.0	4.0
E	BMVWW-I	MT20	4.0	8.0
F	BMV1-p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	620	0	620	0
D	620	0	620	0

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL
F	448 239 / 0 0 / 0 0 / 0 209 / 0 0 / 0
D	448 239 / 0 0 / 0 0 / 0 209 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
F-A	-583 / 0	0.0	0.0 0.11 (1)	A-E	0 / 792	0.24 (1)	
A-B	-895 / 0	-114.3	-114.3 0.55 (1)	E-B	-658 / 0	0.15 (1)	
B-C	-895 / 0	-114.3	-114.3 0.55 (1)	E-C	0 / 792	0.24 (1)	
D-C	-583 / 0	0.0	0.0 0.11 (1)				
F-E	0 / 0	-18.5	-18.5 0.13 (4)				
E-D	0 / 0	-18.5	-18.5 0.13 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 15.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 48.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.31")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.31")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CSI: TC=0.55/1.00 (B-C:1), BC=0.13/1.00 (E-F:4),
WB=0.24/1.00 (A-E:1), SS=0.34/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

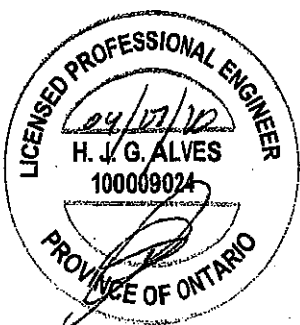
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

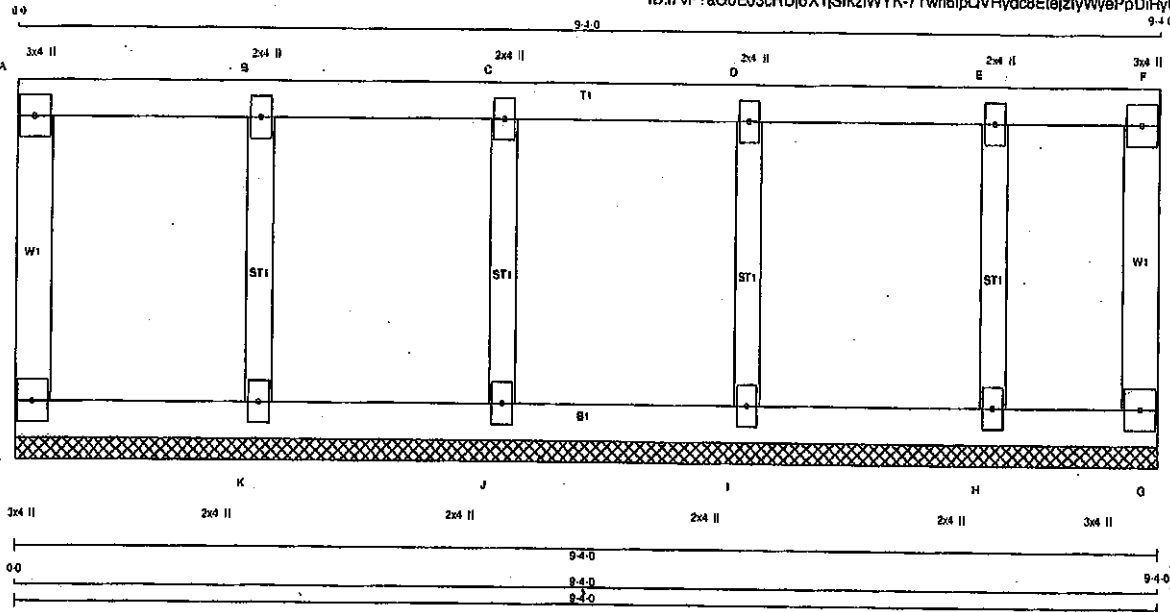
JSI GRIP= 0.61 (A) (INPUT = 0.90)
JSI METAL= 0.24 (A) (INPUT = 1.00)



Structural component only
DWG# T-2007120

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408152	G52	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
L - A	2x4	DRY No.2	SPF
A - F	2x4	DRY No.2	SPF
G - H	2x4	DRY No.2	SPF
L - G	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
ALL GABLE WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2'-0" OC.			

PLATES (table in inches)			
JT TYPE	PLATES	W	LEN Y X
A TMV+p	MT20	3.0	4.0
B, C, D, E			
F TMV+w	MT20	2.0	4.0
G TMV+p	MT20	3.0	4.0
H BMV1+p	MT20	3.0	4.0
I, J, K			
L BMV1+w	MT20	2.0	4.0
M BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING
BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO				FR-TO			
L-A	-101/0	0.0	0.03 (1)	7.81	K-B	-245/0	0.06 (1)
A-B	-87/0	-114.3	-114.3 0.08 (1)	10.00	J-C	-224/0	0.05 (1)
B-C	-87/0	-114.3	-114.3 0.08 (1)	10.00	I-D	-237/0	0.05 (1)
C-D	-87/0	-114.3	-114.3 0.07 (1)	10.00	H-E	-195/0	0.04 (1)
D-E	-87/0	-114.3	-114.3 0.07 (1)	10.00			
E-F	-87/0	-114.3	-114.3 0.05 (1)	10.00			
G-F	-847/0	0.0	0.0 0.02 (1)	7.81			
L-K	0/8	-18.5	-18.5 0.02 (4)	10.00			
K-J	0/8	-18.5	-18.5 0.02 (4)	10.00			
J-I	0/8	-18.5	-18.5 0.02 (4)	10.00			
I-H	0/8	-18.5	-18.5 0.02 (4)	10.00			
H-G	0/8	-18.5	-18.5 0.02 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:		
TOP CH.	LL = 25.6	PSF
	DL = 15.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 48.0 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (A-B:1), BC=0.02/1.00 (K-L:4), WB=0.09/1.00 (B-K:1), SS=0.13/1.00 (A-B:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.17 (K) (INPUT = 0.80)
JSI METAL = 0.07 (K) (INPUT = 1.00)



Structural component only
DWG# T-2007104

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Scale = 1:22.0

The structural drawing shows a symmetrical roof truss system. The main truss consists of two vertical webs (W1) connected by three diagonal members (W2, W3, W4). The top chord is composed of three segments (A-B, B-C, C-D) and the bottom chord is composed of four segments (F-G, G-H, H-I, I-J). The total width is 9.4.0 units. The height from the bottom chord to the peak is 3.4.0 units. The drawing includes various dimension lines and labels for components and joints.

LUMBER

N.L.G.A.	RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF		
A - C	2x6	DRY	No.2	SPF		
C - D	2x4	DRY	No.2	SPF		
F - D	2x6	DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	=	15.0
LL	=	15.0

BOT CH. LL = 0.0 PSF
DL = 0.0 PSF
TOTAL LOAD = 48.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIQ 2011, TPIC 2014

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

**CSI: TC=0.24/1.00 (B-C:1), BC=0.24/1.00 (D-E:1),
DB=0.35/1.00 (A-E:1), SS=0.29/1.00 (E-F:1)**

**DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00**

**COMPANION LIVE LOAD FACTOR = 1.00
PLAT ROOF FACTOR = 0.75**

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1887 788

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP = 0.76 (E) (INPUT = 0.90)
JSI METAL = 0.32 (E) (INPUT = 1.00)

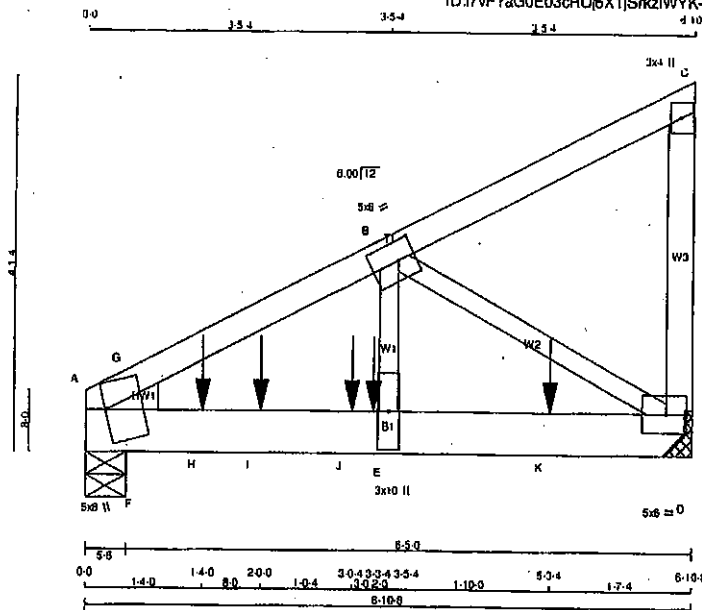
STRUCTURAL COMPONENT ONLY
DWG# T-2007121

LICENSED PROFESSIONAL ENGINEER
H.J.G. ALVES
100009024
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG# T-2007121

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408152	IT54	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/32"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
A - D	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS	SPACING (IN)	
A - C	12	TOP
C - D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
A - D	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	12	

NAIls TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TBMH1+m	MT20	5.0	6.0	3.50
B	TMWW-1	MT20	5.0	6.0	2.50 2.25
C	TMV+p	MT20	3.0	4.0	
D	BMVW1-1	MT20	5.0	6.0	2.50 2.75
E	BMV+w	MT20	3.0	10.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORZ	DOWN	HORZ
A	3467	0	3467
D	2454	0	2454

A SUITABLE HANGER MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4'-0".

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	2449	1622 / 0	0 / 0	0 / 0	0 / 0	828 / 0	0 / 0
D	1735	1144 / 0	0 / 0	0 / 0	0 / 0	591 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LCI	MAX	WEBS	MAX. FACTORED	FACTORED	VERT. LOAD	LCI	MAX
MEMB.	FORCE	(LBS)	FROM	TO	CSI (LC)	MEMB.	FORCE	(LBS)	FROM	TO	CSI (LC)
FR-TO						UNBRACED					
A-G	-4978 / 0	-91.8	-91.8	0.19 (1)	4.16	E-B	0 / 3365	0.42 (1)			
G-B	-3853 / 0	-91.8	-91.8	0.17 (1)	4.66	B-D	-4035 / 0	0.53 (1)			
B-C	-13 / 0	-91.8	-91.8	0.07 (1)	6.25	F-G	0 / 1740	0.00 (1)			
D-C	-129 / 0	0.0	0.0	0.02 (1)	7.81						
A-F	0 / 3490	-18.5	-18.5	0.25 (1)	10.00						
F-H	0 / 3490	-18.5	-18.5	0.86 (1)	10.00						
H-I	0 / 3490	-18.5	-18.5	0.68 (1)	10.00						
I-J	0 / 3490	-18.5	-18.5	0.68 (1)	10.00						
J-E	0 / 3490	-18.5	-18.5	0.86 (1)	10.00						
E-K	0 / 3490	-18.5	-18.5	0.49 (1)	10.00						
K-D	0 / 3490	-18.5	-18.5	0.49 (1)	10.00						

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	1-4-0	-1378	-1378	---	BACK	VERT	TOTAL	---	C1
I	2-0-0	-1000	-1000	---	FRONT	VERT	TOTAL	---	C1
J	3-0-4	-994	-994	---	FRONT	VERT	TOTAL	---	C1
J	3-3-4	-898	-898	---	BACK	VERT	TOTAL	---	C1
K	5-3-4	-898	-898	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 30 = 60 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	= 25.8	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF	

SPACING = 24.0 IN. G.C

*** NON STANDARD GIRDER ***

ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBCG 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(58 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.19/1.00 (A-G:1), BC=0.68/1.00 (E-F:1),
WB=0.53/1.00 (B-D:1), SS=0.90/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1687 788 1987 1656	

PLATE PLACEMENT TOL. = 0.250 inches

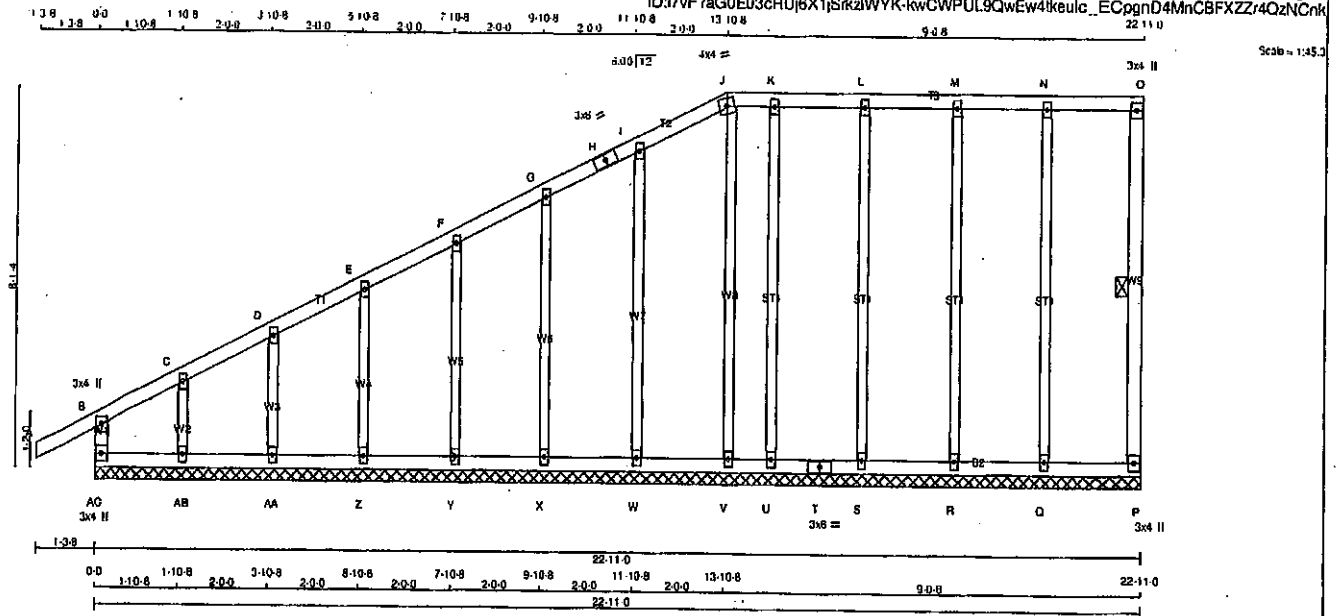
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.70 (A) (INPUT = 1.00)



Structural component only
DWG# T-2007122

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408150	G60	4	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 11:02:39 2020 Page 1	



TOTAL WEIGHT = 4 X 116 = 464 lb (M)

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
AC - B	2x4	DRY No.2	SPF
A - H	2x4	DRY No.2	SPF
H - J	2x4	DRY No.2	SPF
J - O	2x4	DRY No.2	SPF
P - O	2x4	DRY No.2	SPF
AC - T	2x4	DRY No.2	SPF
T - P	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
ALL GABLE WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table in inches)			
JT TYPE	PLATES	W	LEN Y X
B TMV+p	MT20	3.0	4.0
C, D, E, F, G, I, K, L, M, N			
C TMV+w	MT20	2.0	4.0
H TS-I	MT20	3.0	8.0
J TTW-m	MT20	4.0	4.0
O TMV+p	MT20	3.0	4.0
P BMV+p	MT20	3.0	4.0
Q, R, S, U, V, W, X, Y, Z, AA, AB			
Q BMV+w	MT20	2.0	4.0
T BS-I	MT20	3.0	6.0
AC BMV+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF O-P.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
AC-B	-224/0	0.0	0.0 0.02 (1)	Q-N	-195/0	0.25 (1)	
A-B	0/28	-91.8	-91.8 0.12 (1)	R-M	-183/0	0.23 (1)	
B-C	-28/0	-91.8	-91.8 0.04 (1)	S-L	-188/0	0.24 (1)	
C-D	-19/0	-91.8	-91.8 0.04 (1)	U-K	-136/0	0.17 (1)	
D-E	-15/0	-91.8	-91.8 0.04 (1)	V-J	-131/0	0.17 (1)	
E-F	-11/0	-91.8	-91.8 0.04 (1)	W-I	-185/0	0.17 (1)	
F-G	-8/0	-91.8	-91.8 0.04 (1)	X-G	-183/0	0.11 (1)	
G-H	-8/0	-91.8	-91.8 0.04 (1)	Y-F	-182/0	0.07 (1)	
H-I	-5/0	-91.8	-91.8 0.04 (1)	Z-E	-182/0	0.05 (1)	
I-J	-5/0	-91.8	-91.8 0.04 (1)	AA-D	-183/0	0.03 (1)	
J-K	-2/0	-91.8	-91.8 0.03 (1)	AB-C	-178/0	0.03 (1)	
K-L	-2/0	-91.8	-91.8 0.04 (1)				
L-M	-2/0	-91.8	-91.8 0.04 (1)				
M-N	-2/0	-91.8	-91.8 0.04 (1)				
N-O	-2/0	-91.8	-91.8 0.04 (1)				
P-Q	-81/0	0.0	0.0 0.01 (1)				
AC-AB	0/24	-18.5	-18.5 0.02 (4)				
AB-AA	0/17	-18.5	-18.5 0.02 (4)				
AA-Z	0/13	-18.5	-18.5 0.02 (4)				
Z-Y	0/10	-18.5	-18.5 0.02 (4)				
Y-X	0/7	-18.5	-18.5 0.01 (4)				
X-W	0/5	-18.5	-18.5 0.02 (4)				
W-V	0/4	-18.5	-18.5 0.02 (4)				
V-U	0/2	-18.5	-18.5 0.01 (4)				
U-T	0/2	-18.5	-18.5 0.02 (4)				
T-S	0/2	-18.5	-18.5 0.02 (4)				
S-R	0/2	-18.5	-18.5 0.02 (4)				
R-Q	0/2	-18.5	-18.5 0.02 (4)				
Q-P	0/2	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (Q-R:4), WB=0.25/1.00 (N-Q:1), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

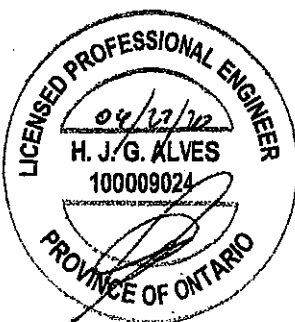
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.80 (J) (INPUT = 0.90)
JSI METAL=0.08 (I) (INPUT = 1.00)

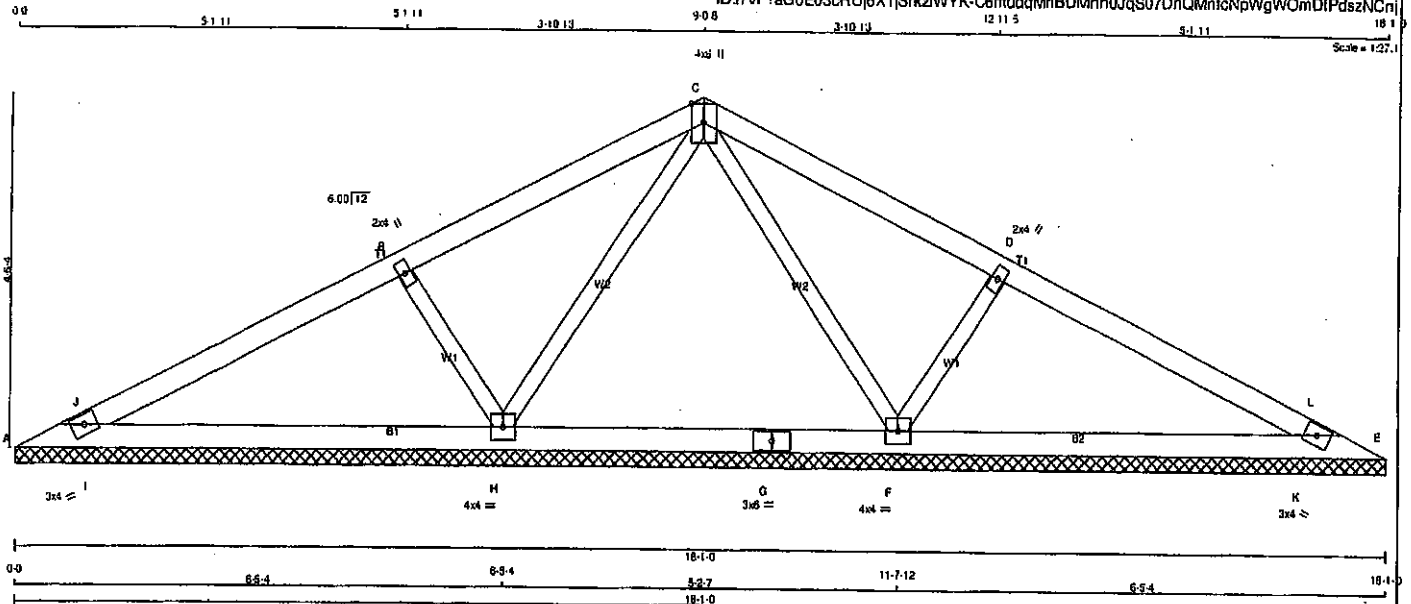


Structural component only
DWG# T-2007057

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408150	GP1	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 2 X 54 = 107 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
A - G	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW+w	MT20	2.0	4.0		
C	TTWW+p	MT20	4.0	6.0	Edge	
D	TMW+w	MT20	2.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F	BMWW1-l	MT20	4.0	4.0		
G	BS-l	MT20	3.0	6.0		
H	BMWW1-l	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
A	134	0	134	0	18-1-0 (9-11-12)1-0	
E	134	0	134	0	18-1-0 (8-11-12)1-0	
F	883	0	883	0	18-1-0 (9-11-12)1-0	
H	883	0	883	0	18-1-0 (9-11-12)1-0	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX/MIN COMPONENT REACTIONS				WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
A	94	88/0	0/0	0/0	26/0	0/0			
E	94	88/0	0/0	0/0	26/0	0/0			
F	811	385/0	0/0	0/0	216/0	0/0			
H	811	385/0	0/0	0/0	216/0	0/0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS A, E, F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-J	0/240	-91.8 -91.8	0.12 (1)	10.00	C-F	-428/0	0.18 (1)
J-B	0/301	-91.8 -91.8	0.34 (1)	10.00	F-D	-436/0	0.08 (1)
B-C	0/503	-91.8 -91.8	0.37 (1)	10.00	H-C	-428/0	0.18 (1)
C-D	0/503	-91.8 -91.8	0.37 (1)	10.00	G-H	-436/0	0.08 (1)
D-L	0/301	-91.8 -91.8	0.34 (1)	10.00	I-J	-76/35	0.00 (1)
L-E	0/240	-91.8 -91.8	0.12 (1)	10.00	K-L	-76/35	0.00 (1)
A-I	-255/0	-18.5 -18.5	0.11 (1)	8.25			
I-H	-243/0	-18.5 -18.5	0.13 (4)	8.25			
H-G	-246/0	-18.5 -18.5	0.13 (4)	8.25			
G-F	-246/0	-18.5 -18.5	0.13 (4)	8.25			
F-K	-243/0	-18.5 -18.5	0.13 (4)	8.25			
K-E	-255/0	-18.5 -18.5	0.11 (1)	8.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOY CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PARTS, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.37/1.00 (C-D:1), BC=0.13/1.00 (F-K:4), WB=0.18/1.00 (C-H:1), SS=0.17/1.00 (D-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.48 (H) (INPUT = 0.90)
JSI METAL= 0.15 (C) (INPUT = 1.00)

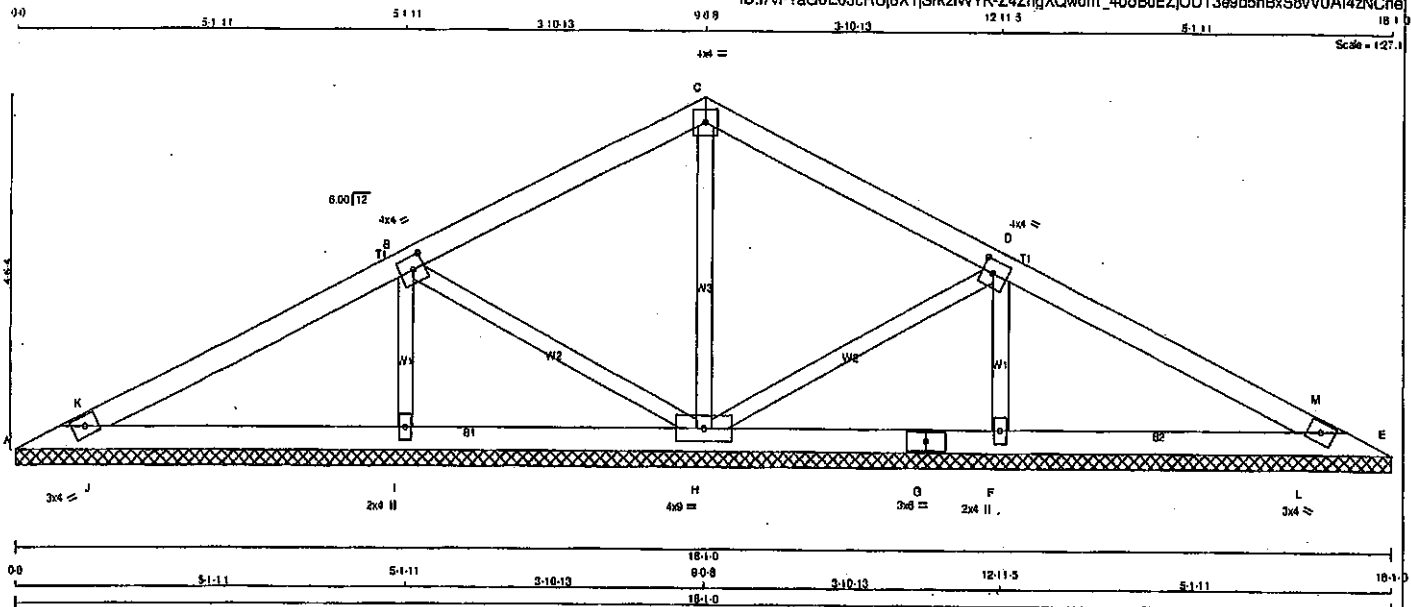


Structural component only
DWG# T-2007058

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408150	P1	17	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 17 X 55 = 943 lb

LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
A - G	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
A	124	0	124	0	0	18'-0"	(11-1142) 0	0	0
E	124	0	124	0	0	18'-0"	(11-1142) 0	0	0
H	415	0	415	0	0	18'-0"	(11-1142) 0	0	0
F	666	0	666	0	0	18'-0"	(11-1142) 0	0	0
I	666	0	666	0	0	18'-0"	(11-1142) 0	0	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	87	59/0	0/0	0/0	0/0	0/0	28/0	0/0
E	87	59/0	0/0	0/0	0/0	0/0	28/0	0/0
H	293	192/0	0/0	0/0	0/0	0/0	101/0	0/0
F	471	307/0	0/0	0/0	0/0	0/0	184/0	0/0
I	471	307/0	0/0	0/0	0/0	0/0	184/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, H, F, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (LBS)	MEMB.	FORCE (LBS)	MAX. (LBS)	CS (LC)
FR-TO		FROM	TO	FR-TO			
A-K	0/248	-91.8	-91.8 0.07 (1)	H-C	-445/0	0.13 (1)	
K-B	0/292	-91.8	-91.8 0.32 (1)	H-D	0/59	0.01 (1)	
B-C	0/188	-91.8	-91.8 0.30 (1)	F-D	-528/0	0.08 (1)	
C-D	0/188	-91.8	-91.8 0.30 (1)	B-H	0/59	0.01 (1)	
D-M	0/292	-91.8	-91.8 0.32 (1)	I-B	-528/0	0.08 (1)	
M-E	0/248	-91.8	-91.8 0.07 (1)	J-K	-128/8	0.00 (1)	
				L-M	-128/8	0.00 (1)	
A-J	-256/0	-18.5	-18.5 0.14 (1)				
J-I	-240/0	-18.5	-18.5 0.14 (1)				
I-H	-240/0	-18.5	-18.5 0.11 (1)				
H-G	-240/0	-18.5	-18.5 0.11 (1)				
G-F	-240/0	-18.5	-18.5 0.11 (1)				
F-L	-240/0	-18.5	-18.5 0.14 (1)				
L-E	-256/0	-18.5	-18.5 0.14 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. G/G

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (D-M:1), RC=0.14/1.00 (F-L:1),
WB=0.13/1.00 (C-H:1), SS=0.17/1.00 (D-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

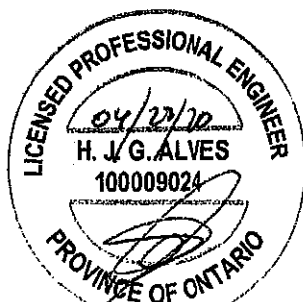
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1656

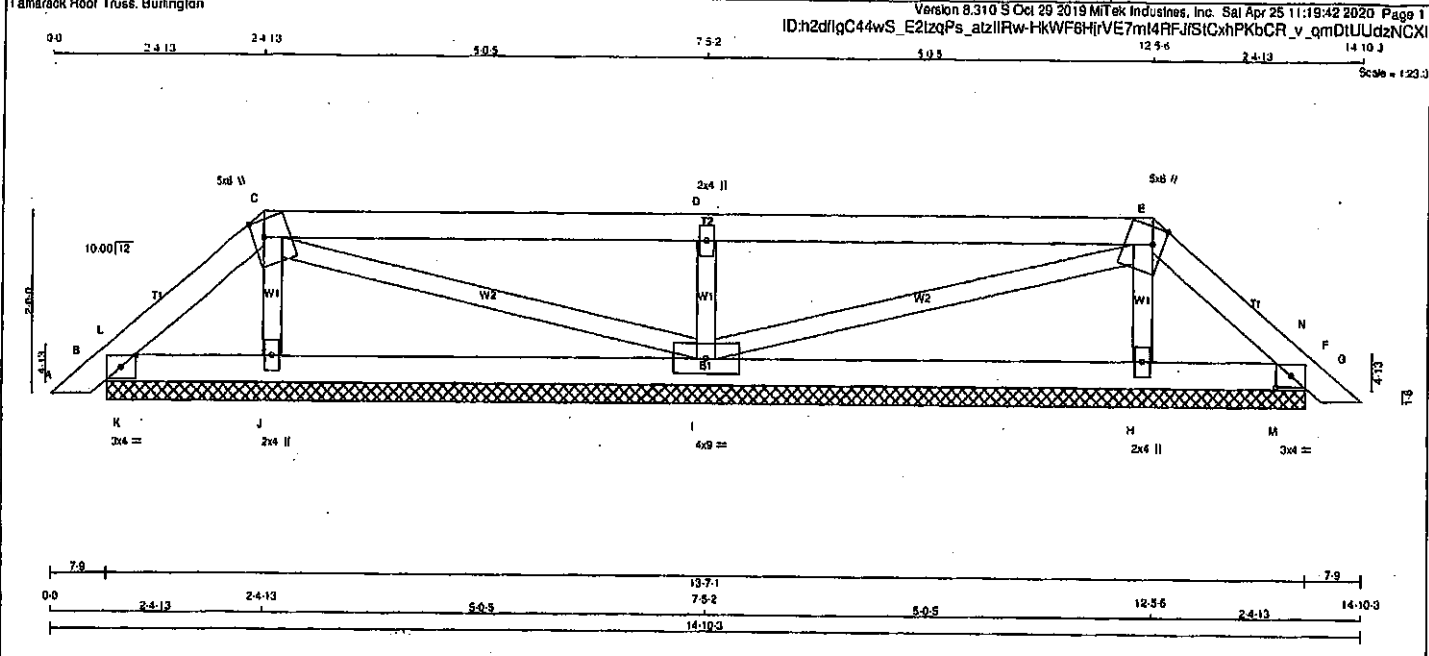
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.51 (B) (INPUT = 0.80)
JSI METAL = 0.13 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007062



TOTAL WEIGHT = 2 X 45 = 90 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4	DRY No.2
C - E	2x4	DRY No.2
E - G	2x4	DRY No.2
G - F	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H	BMW1+w	MT20	2.0	4.0		
I	BMWW1-I	MT20	4.0	9.0		
J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT DOWN	HORZ UPLIFT	IN-SX	IN-SX
JT	169	0	13-7-1	13-7-1
B	169	0	13-7-1	13-7-1
F	288	0	13-7-1	13-7-1
I	683	0	13-7-1	13-7-1
H	288	0	13-7-1	13-7-1

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
B	118	93/0	0/0	0/0	0/0	23/0	0/0
F	116	93/0	0/0	0/0	0/0	23/0	0/0
J	207	118/0	0/0	0/0	0/0	89/0	0/0
I	481	327/0	0/0	0/0	0/0	154/0	0/0
H	207	118/0	0/0	0/0	0/0	89/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSF (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSF (LC)
PR-TO				FR-TO		
A-B	0/14	-91.8	-91.8 0.02 (1)	10.00	J-C	-194/0 0.03 (1)
B-L	-31/4	-91.8	-91.8 0.01 (1)	6.25	C-I	-22/0 0.01 (1)
L-C	-60/0	-91.8	-91.8 0.03 (1)	6.25	I-D	-576/0 0.08 (1)
C-D	-8/0	-91.8	-91.8 0.39 (1)	10.00	D-E	-22/0 0.01 (1)
D-E	-8/0	-91.8	-91.8 0.39 (1)	10.00	E-H	-194/0 0.03 (1)
E-N	-60/0	-91.8	-91.8 0.03 (1)	6.25	K-L	-121/0 0.00 (1)
N-F	-31/4	-91.8	-91.8 0.01 (1)	6.25	M-N	-121/0 0.00 (1)
F-G	0/14	-91.8	-91.8 0.02 (1)	10.00		
B-K	0/42	-18.5	-18.5 0.04 (1)	10.00		
K-J	0/42	-18.5	-18.5 0.07 (4)	10.00		
J-I	0/29	-18.5	-18.5 0.10 (4)	10.00		
I-H	0/29	-18.5	-18.5 0.10 (4)	10.00		
H-M	0/42	-18.5	-18.5 0.07 (4)	10.00		
M-F	0/42	-18.5	-18.5 0.04 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCG 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TFC 2011, TRC 2014

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.39/1.00 (C-D:1), BC=0.10/1.00 (I-J:4), WB=0.08/1.00 (D-I:1), SS=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

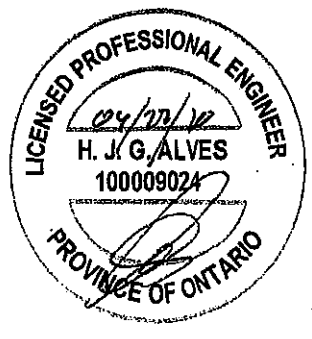
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.30 (D) (INPUT = 0.80)

JSI METAL = 0.12 (D) (INPUT = 1.00)



Structural component only
DWG# T-2007080



TOTAL WEIGHT = 21 X 17 = 353 lb

DRY: SEASONED LUMBER.

Q	DATE	TIME
1	10/10/2010	10:00
2	10/10/2010	10:00
3	10/10/2010	10:00
4	10/10/2010	10:00
5	10/10/2010	10:00
6	10/10/2010	10:00
7	10/10/2010	10:00
8	10/10/2010	10:00
9	10/10/2010	10:00
10	10/10/2010	10:00
11	10/10/2010	10:00
12	10/10/2010	10:00
13	10/10/2010	10:00
14	10/10/2010	10:00
15	10/10/2010	10:00
16	10/10/2010	10:00
17	10/10/2010	10:00
18	10/10/2010	10:00
19	10/10/2010	10:00
20	10/10/2010	10:00
21	10/10/2010	10:00
22	10/10/2010	10:00
23	10/10/2010	10:00
24	10/10/2010	10:00
25	10/10/2010	10:00
26	10/10/2010	10:00
27	10/10/2010	10:00
28	10/10/2010	10:00
29	10/10/2010	10:00
30	10/10/2010	10:00
31	10/10/2010	10:00
32	10/10/2010	10:00
33	10/10/2010	10:00
34	10/10/2010	10:00
35	10/10/2010	10:00
36	10/10/2010	10:00
37	10/10/2010	10:00
38	10/10/2010	10:00
39	10/10/2010	10:00
40	10/10/2010	10:00
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75	10/10/2010	10:00
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78	10/10/2010	10:00
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85	10/10/2010	10:00
86	10/10/2010	10:00
87	10/10/2010	10:00
88	10/10/2010	10:00
89	10/10/2010	10:00
90	10/10/2010	10:00
91	10/10/2010	10:00
92	10/10/2010	10:00
93	10/10/2010	10:00
94	10/10/2010	10:00
95	10/10/2010	10:00
96	10/10/2010	10:00
97	10/10/2010	

BEARINGS

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) IF

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL FITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

E-D	0/0	-18.5	-18.5	0.13 (4)	10.00
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DESIGN CRITERIA

SPECIFIED LOADS:

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 0.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = 1/998 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = 1/998 (0.03")

CSI: TC=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	818	354	1667	788	1987	1658

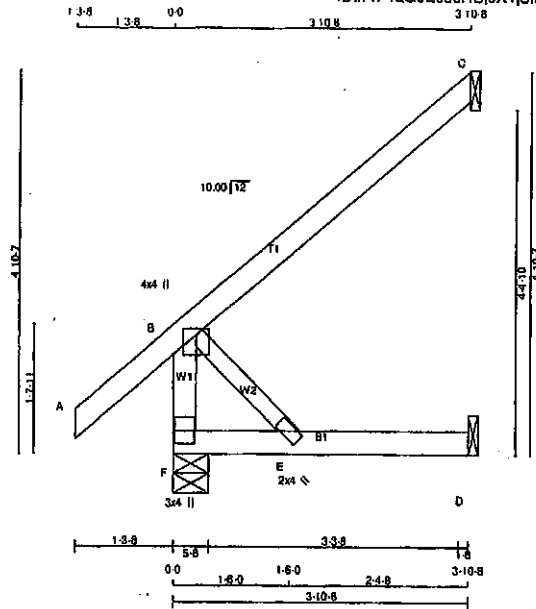
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408150	J2	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 11:02:43 2020 Page 1	



Scale = 1/2" = 1'-0"

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS, AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
F	341	0	341	0	5-8
C	178	0	178	0	1-8
D	38	0	40	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM.LIVE
F	239	170/0	0/0	0/0
C	122	99/0	0/0	0/0
D	29	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED	VERT. LOAD	MAX. FACTORED	MEMB.	MAX. FACTORED
FR-TO	FORCE (LBS)	LC1	LC1	FR-TO	FORCE (LBS)
F-B	-305/0	0.0	0.0	0.03 (1)	7.81
A-B	0/41	-91.8	-91.8	0.14 (5)	10.00
B-C	0/0	-91.8	-91.8	0.23 (1)	10.00
F-E	0/0	-18.5	-18.5	0.08 (4)	10.00
E-D	0/0	-18.5	-18.5	0.08 (4)	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 15 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.23/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(FSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1997 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.24 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



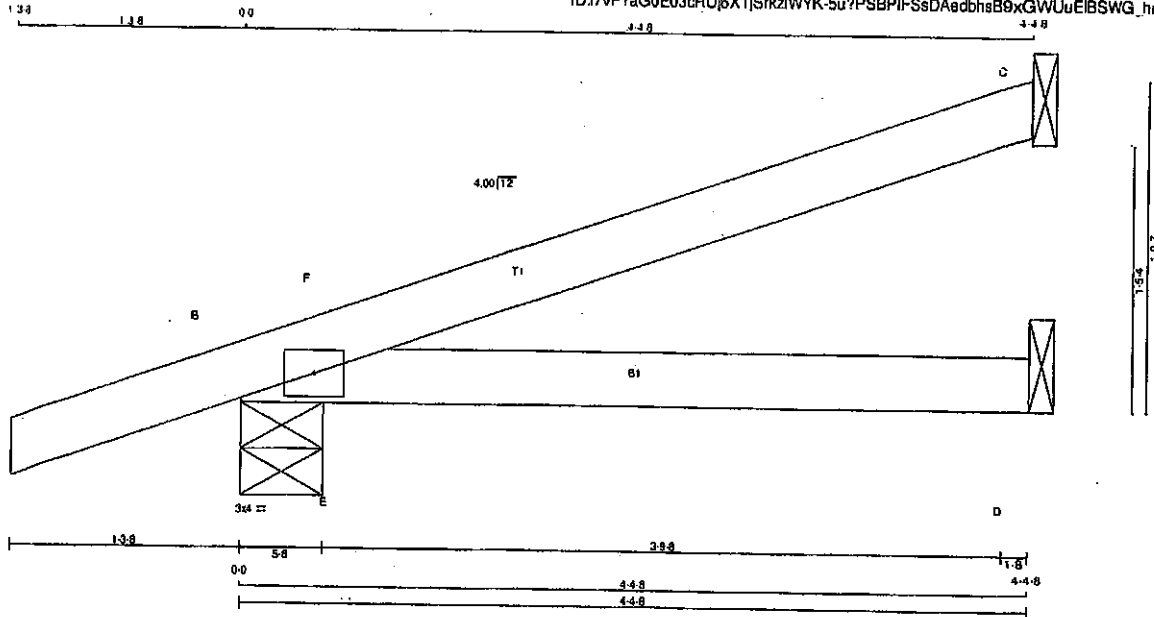
Structural component only
DWG# T-2007060

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408150	J3	6	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:11.5



TOTAL WEIGHT = 6 X 12 = 71 lb (M)

LUMBER			
N. L. G. A. RULES	SIZE	DRY	DESCR.
CHORDS	2x4	DRY	No.2
A - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMB1-1	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
C	174	174	0	1-8	1-8
B	384	384	0	5-8	5-8
D	68	68	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS						
JT	1ST LOASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD
C	120	93/0	0/0	0/0	0/0	27/0
B	255	180/0	0/0	0/0	0/0	75/0
D	50	19/0	0/0	0/0	0/0	32/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LG1 MAX. CSI (LC)	UNBRAC LENGTH FR-TO	MEMB. MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	UNBRAC LENGTH FR-TO	MEMB. MAX. FACTORED FORCE (LBS)
A-B	0/18	-91.8	-91.8	0.11 (1)	10.00		E-F	-194/7	0.00 (1)		
B-F	-14/0	-91.8	-91.8	0.05 (4)	6.25						
F-C	0/2	-91.8	-91.8	0.22 (1)	10.00						
B-E	0/0	-18.5	-18.5	0.17 (1)	10.00						
E-D	0/0	-18.5	-18.5	0.17 (1)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH. LL	= 25.8 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.05")

CSI: TC=0.22/1.00 (C-F:1), BC=0.17/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SSI=0.16/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

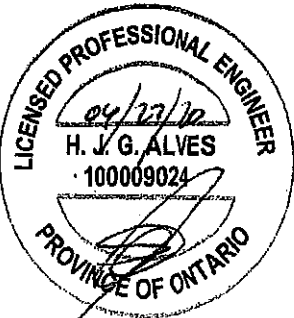
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRP(DRY)	SHEAR	SECTION	
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

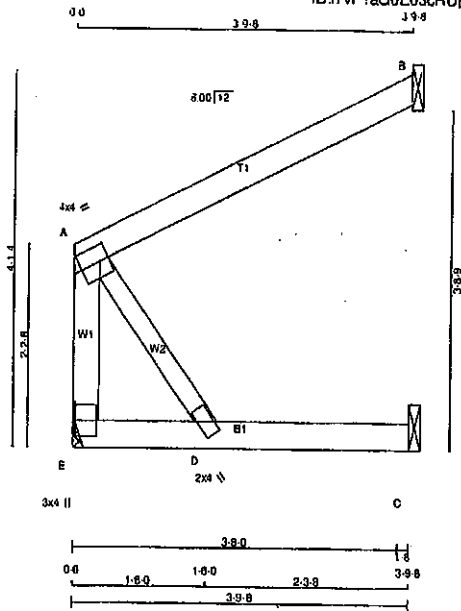
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007061

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408152	J40	4	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 11:28:29 2020 Page 1					
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Scale = 1:23.0					



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMWW-1	MT20	4.0	4.0	2.00 1.25
D	BMW+vw	MT20	2.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	UPLIFT	BRG	BRG	IN-SX
E	209	0	0	209	0	0	MECHANICAL	1-8	1-8
B	174	0	0	174	0	0	1-8	1-8	1-8
C	35	0	0	39	0	0	1-8	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL B97781H FOR CONNECTION TO JOINT(S) B, C.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	148	97.0	0.0	0.0	0.0	51.0	0.0
B	120	97.0	0.0	0.0	0.0	23.0	0.0
C	28	0.0	0.0	0.0	0.0	28.0	0.0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	FORCE (LBS)	MAX	LC1
E-A	-174.0	0.0	0.0 0.02 (1)	E-D	0.0	0.0	0.0 (1)
A-B	0.0	-91.8	-91.8 0.22 (1)	D-C	0.0	-18.5	-18.5 0.08 (4)
E-D	0.0	-18.5	-18.5 0.07 (4)				
D-C	0.0	-18.5	-18.5 0.08 (4)				

TOTAL WEIGHT = 4 X 13 = 52 lb

DESIGN CRITERIA

SPECIFIED LOADS:		
TOP CH.	LL	= 25.6 PSF
	DL	= 8.0 PSF
BOT CH.	LL	= 0.0 PSF
	DL	= 7.4 PSF
TOTAL LOAD = 39.0 PSF		

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/998 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/998 (0.01")

CSI: TC=0.22/1.00 (A-B:1), BC=0.08/1.00 (C-D:4),
WB=0.00/1.00 (A-D:1), SS=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	618	354
	1667	788
	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

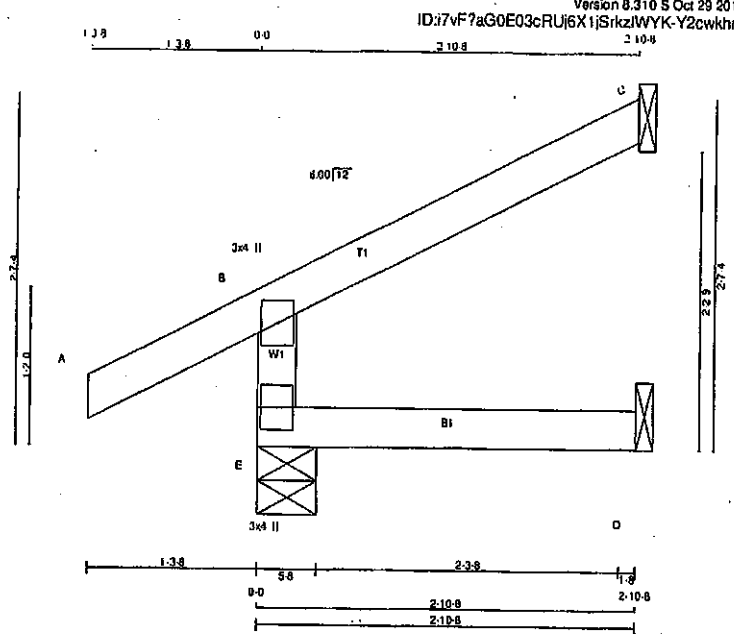
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.11 (A) (INPUT = 0.90)
JSI METAL = 0.03 (A) (INPUT = 1.00)



Structural component only
DWG# T-2007105

JOB NAME 408152	TRUSS NAME J4.1	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	ORWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



TOTAL WEIGHT = 4 X 10 = 38 lb

LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	310	0	310	0	5-8	5-8
C	99	0	99	0	1-8	1-8
D	23	0	23	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS		PERM.LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE				
E	223	161/0	0/0	0/0	0/0	0/0	62/0	0/0
C	68	55/0	0/0	0/0	0/0	0/0	13/0	0/0
D	18	0/0	0/0	0/0	0/0	0/0	18/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
E-B	-289/0	0.0	0.0	0.02 (4)			
A-B	0/28	-91.8	-91.8	0.13 (5)			
B-C	-15/0	-91.8	-91.8	0.13 (1)			
E-D	0/0	-18.5	-18.5	0.03 (4)			
				10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, CBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (B-C:1), BC=0.03/1.00 (D-E:4), WB=0.00/1.00 (vs:0), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1867 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.12 (E) (INPUT = 0.90)
JSI METAL = 0.08 (B) (INPUT = 1.00)

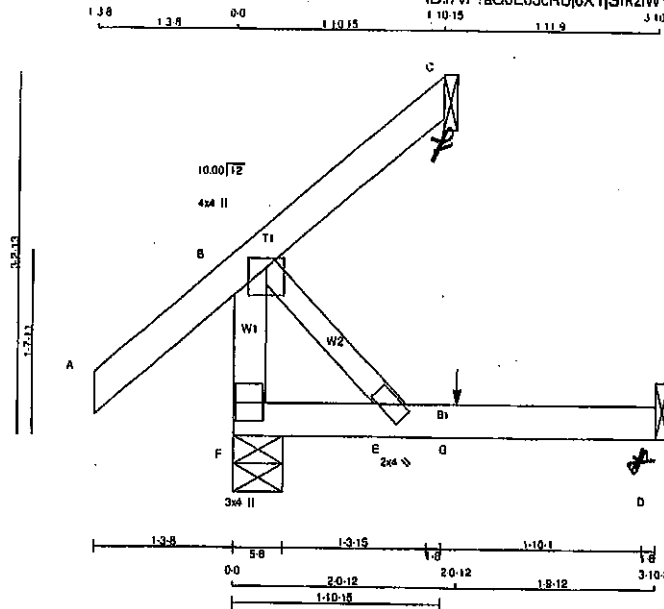


Structural component only
DWG# T-2007106

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ORWG NO.
408150	C1	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:19.0



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT
F	297	0	0
C	42	0	0
D	38	0	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS			
1ST LCASE	MAX/MIN COMPONENT REACTIONS		
JT	COMBINED	SNOW	LIVE
F	208	145/0	0/0
C	29	23/-26	0/0
D	29	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS			
MEMB.	FORCE	FACTORED	WEBS
	(LBS)	VERT. LOAD LC1	MAX. FACTORED
		(PLF)	CSI (LC)
FR-TO		FROM TO	LENGTH FR-TO
F-B	-281/0	0.0	0.0 0.03 (1)
A-B	0/41	-91.8	-91.8 0.14 (5)
B-C	-29/0	-91.8	-91.8 0.14 (5)
F-E	0/0	-18.5	-18.5 0.08 (4)
E-G	0/0	-18.5	-18.5 0.08 (4)
G-D	0/0	-18.5	-18.5 0.08 (4)

FACTORED CONCENTRATED LOADS (LBS)			
JT	LOC.	LC1	MAX- MAX+
G	2-0-12	1	1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.18")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.18")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.14/1.00 (A-B:5), BC=0.08/1.00 (D-E:4),
WB=0.00/1.00 (B-E:1), SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
	MAX MIN	MAX MIN	MAX MIN
MT20	818 354	1887 788	1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

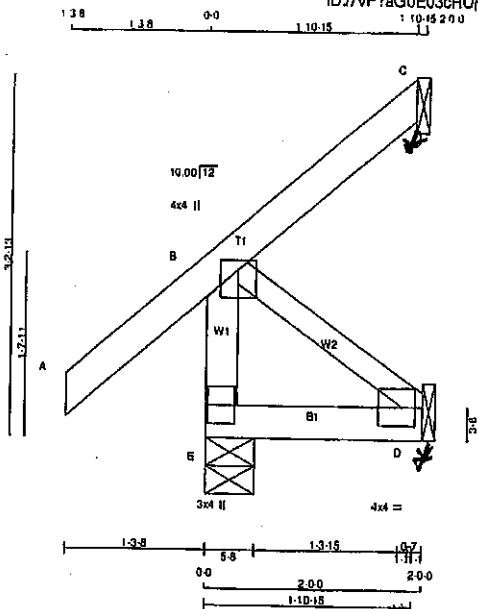


Structural component only
DWG# T-2007055

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DWG NO.
408150	C2	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:19.0

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESCR.	SPF
SPF	SPF
SPF	SPF
SPF	SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
VERT	HORZ	DOWN	HORZ	UPLIFT
E	278	0	278	0
C	42	0	42	0
D	18	0	20	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	194	145/0	0/0	0/0	0/0	0/0	48/0	0/0
C	28	23/-28	0/0	0/0	0/0	0/0	5/0	0/0
D	14	0/0	0/0	0/0	0/0	0/0	14/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
E-B	-261/0	0.0	0.03 (1)	7.81	B-D	0/0	0.00 (1)
A-B	0/41	-91.8	-91.8 0.13 (1)	10.00			
B-C	-29/0	-91.8	-91.8 0.12 (1)	6.25			
E-D	0/0	-18.5	-18.5 0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 10 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.18")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (B-D:1), SS=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	788	1987	1656

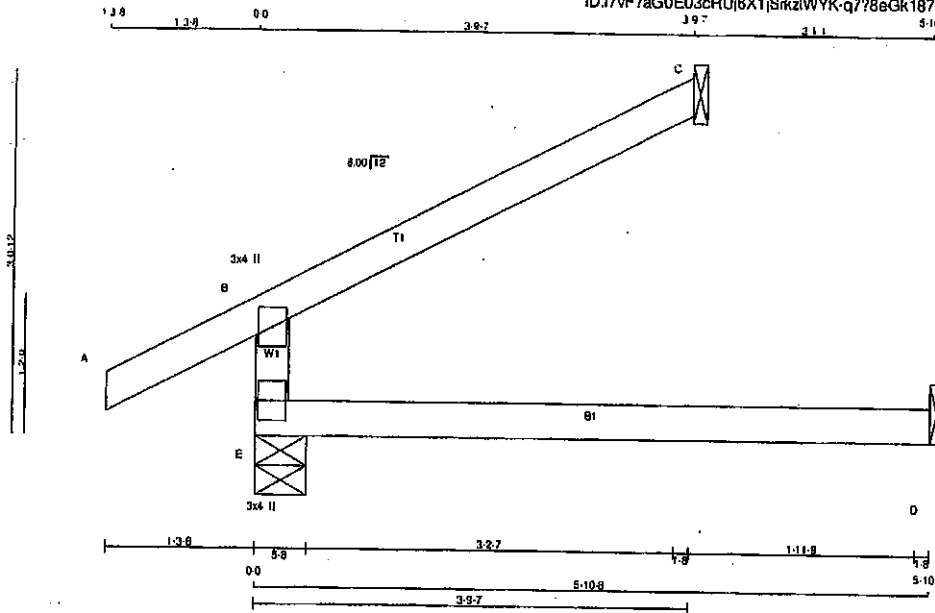
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007056



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
E	405	405	5-8	5-8
C	130	130	1-8	1-8
D	45	50	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
E	285	190 / 0	0 / 0	0 / 0	98 / 0	0 / 0
C	90	73 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	38	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	WEBS		MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)
				FROM	TO		
FR-TO							
E-B		-342 / 0	0.0	0.0	0.13 (4)	7.81	
A-B		0 / 28	-91.8	-91.8	0.12 (1)	10.00	
B-C		-19 / 0	-91.8	-91.8	0.22 (1)	6.25	
E-D		0 / 0	-18.5	-18.5	0.13 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
OL = 8.0 PSF
BOT CH. LL = 0.0 PSF
OL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CSI: TC=0.22/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (A-B:0), SI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

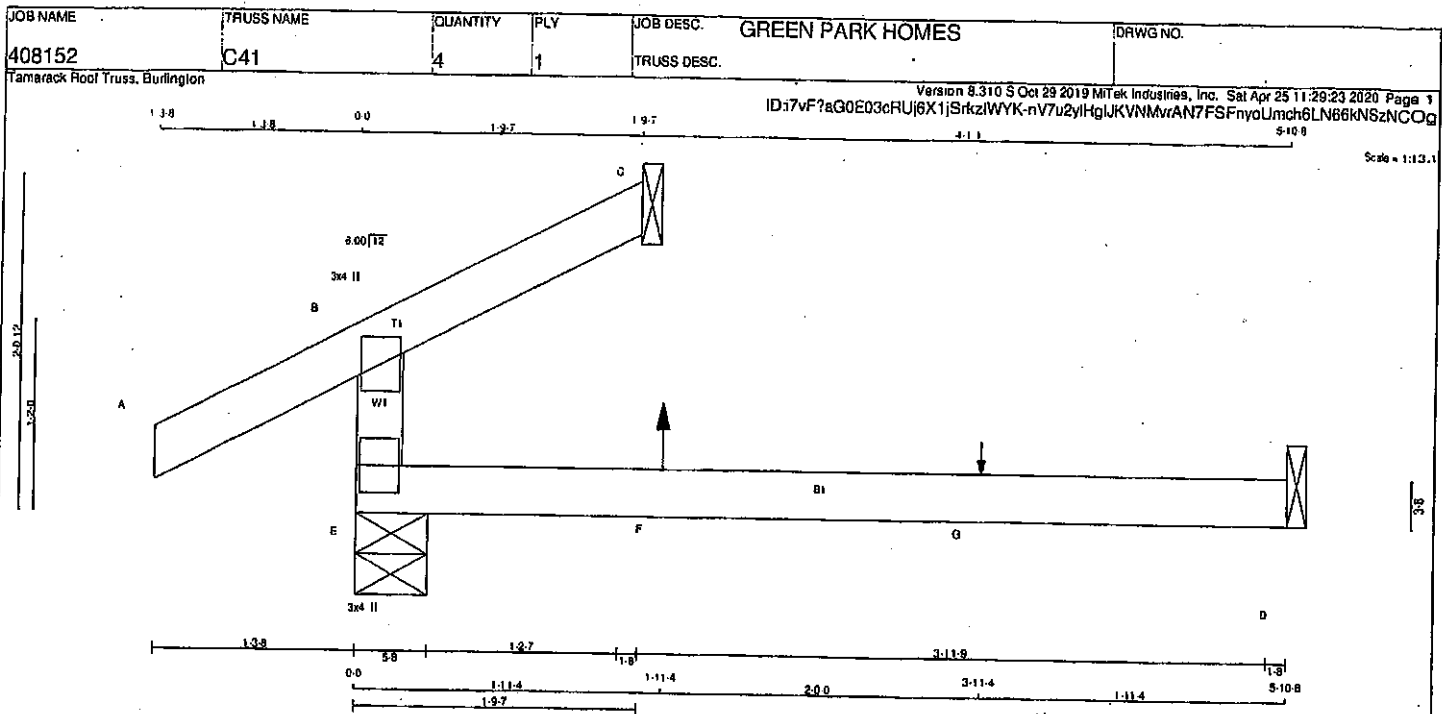
NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1867 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.14 (E) (INPUT = 0.90)
JSI METAL = 0.09 (B) (INPUT = 1.00)





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
E	284	0	284	0	5-8	5-8
C	63	0	63	0	1-8	1-8
D	44	0	52	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	200	137/0	0/0	0/0	0/0	62/0	0/0
C	46	21/0	0/0	0/0	0/0	25/0	0/0
D	35	0/-3	0/0	0/0	0/0	37/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNFACTORED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
E-B	-227/0	0.0	0.0	0.11 (4)	7.81		
A-B	0/28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-9/9	-91.8	-91.8	0.08 (4)	10.00		
E-F	0/0	-18.5	-18.5	0.14 (4)	10.00		
F-G	0/0	-18.5	-18.5	0.14 (4)	10.00		
G-D	0/0	-18.5	-18.5	0.14 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11.4	7	1	12	BACK	VERT	TOTAL	--	C1
G	3-11.4	1	1	--	BACK	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 12 = 48 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CSI: TC=0.12/1.00 (A-B:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.08/1.00 (A-B:1)

DOL LUMBER=0.98 NAIL=0.98 L.S BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

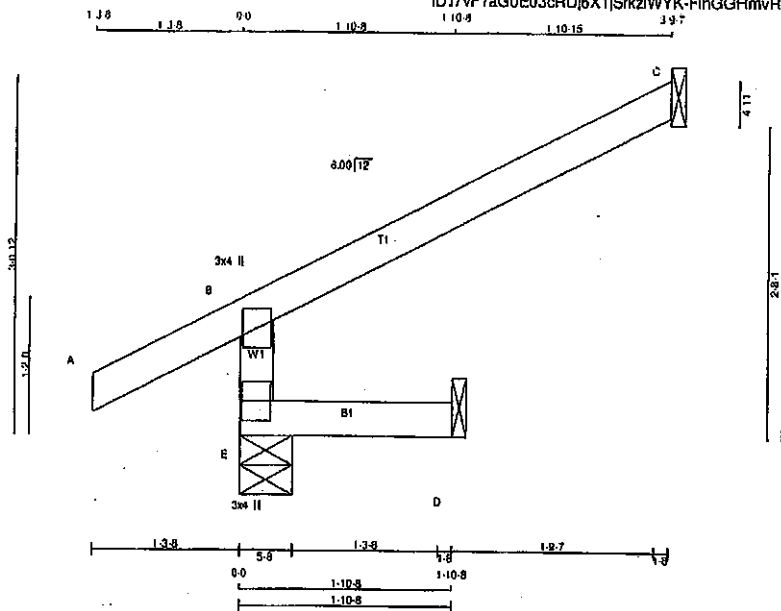
JSI GRIP = 0.10 (E) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only.
DWG# T-2007099

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408152	C42	4	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	SPF
E - B	2x4	DRY	No.2	SPF	SPF
A - C	2x4	DRY	No.2	SPF	SPF
E - D	2x4	DRY	No.2	SPF	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	GROSS REACTION	DOWN	HORIZ		
E	381	0	381	0	5-8	5-8
C	130	0	130	0	1-8	1-8
D	18	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

JT	UNFACTORED REACTIONS		MAX/MIN. COMPONENT REACTIONS		DEAD	SOIL
	1ST LCASE	SNOW	LIVE	PERM. LIVE		
E	250	190/0	0/0	0/0	60/0	0/0
C	90	73/0	0/0	0/0	17/0	0/0
D	12	0/0	0/0	0/0	12/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE	VERT. LOAD	LC1	MAX	MAX. MEMB.	FORCE	MAX	CS1 (LC)
FR-TO	(LBS)	(PLF)	CS1 (LC)	UNBRAC	LENGTH	FR-TO	(LBS)	CS1 (LC)
E-B	-342/0	0.0	0.0	0.01 (4)	7.81			
A-B	0/28	-91.8	-91.8	0.13 (5)	10.00			
B-C	-19/0	-91.8	-91.8	0.22 (1)	6.25			
E-D	0/0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 10 = 38 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, CBC 2012, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.22/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (N/A:0), SS=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

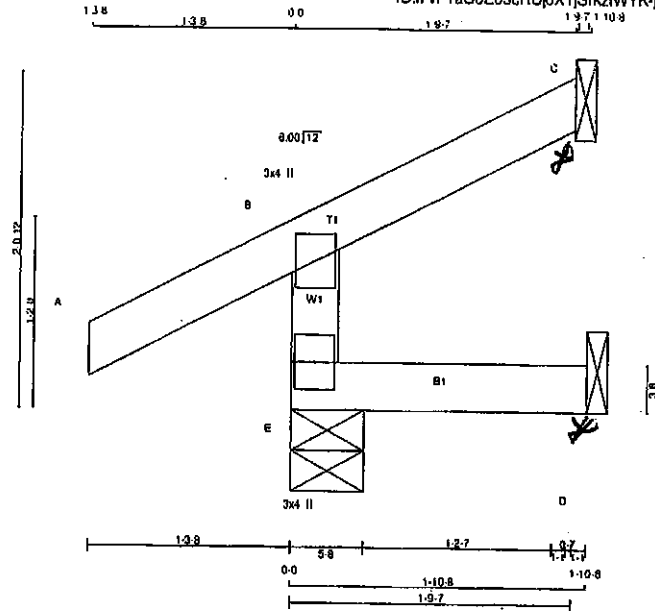
JSI GRIP= 0.14 (E) (INPUT = 0.90)
 JSI METAL= 0.09 (B) (INPUT = 1.00)



Structural component only
 DWG# T-2007100

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408152	C43	4	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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Scale = 1/12.1

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (tablets in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
E	BMV+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT IN-SX	IN-SX
E	271	0	271	0	5-8	5-8
C	45	0	45	0	1-8	1-8
D	8	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 180 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE				
E	188	141/0	0/0	0/0	0/0	0/0	47/0	0/0
C	31	24/-10	0/0	0/0	0/0	0/0	7/0	0/0
D	7	0/-8	0/0	0/0	0/0	0/0	12/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX. CSI (LC)	MAX. FORCE (LBS)	MAX. CSI (LC)
FR-TO								
E-B	-244/0	0.0	0.0	0.04 (5)	7.81			
A-B	0/28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-17/0	-91.8	-91.8	0.09 (1)	6.25			
E-D	0/0	-18.5	-18.5	0.04 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 7 = 28 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2018 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/360 (0.19")
CALCULATED VERT. DEF. (LL) = L/999 (0.00")
ALLOWABLE DEF. (TL) = L/360 (0.19")
CALCULATED VERT. DEF. (TL) = L/999 (0.00")

CSI: TO=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5),
WB=0.00/1.00 (N/A:0), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 L5 BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

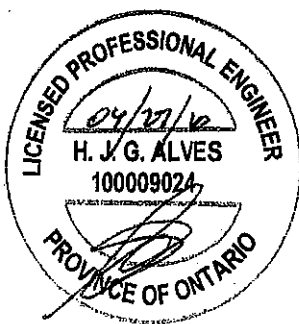
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.10 (E) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007101

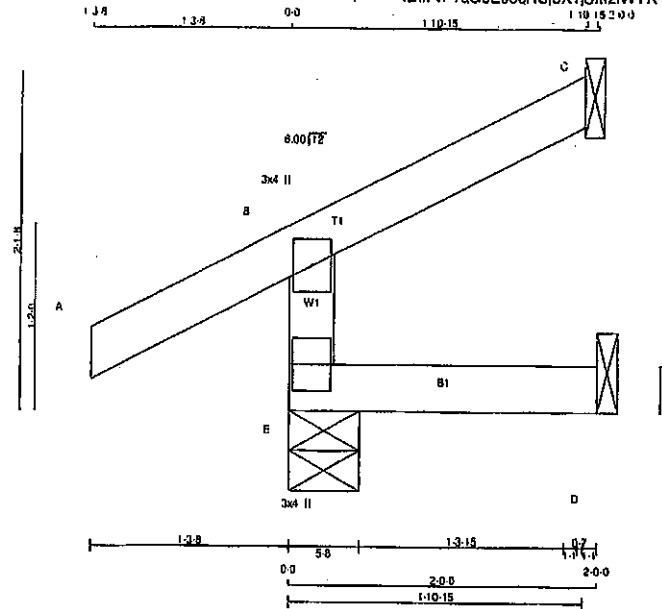
JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408152	C45	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:13.4



TOTAL WEIGHT = 2 X 7 = 14 lb

LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER		SPF	
CHORDS					
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
E	254	0	254	0	0	5.8	5.8	5.8	5.8
C	68	0	68	0	0	1.8	1.8	1.8	1.8
D	18	0	18	0	0	1.8	1.8	1.8	1.8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS		1ST CASE		MAX. MIN. COMPONENT REACTIONS		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	VERT.	VERT.	LC1	MAX	MAX.	MEMB.	MAX.	FORCE	MAX	CS1 (LC)	UNBRAC	LENGTH	FR-TO
E	177	130 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
C	46	37 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT.	LC1	MEMB.	FORCE (LBS)	CS1 (LC)	UNBRAC
FR-TO		FROM	TO	FR-TO		FROM	TO
E-B	-234 / 0	0.0	0.0	0.01 (4)	7.81		
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-10 / 0	-91.8	-91.8	0.06 (1)	10.00		
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2015
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CS1: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SS1=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.10 (E) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007103

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

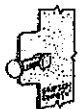
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options Information on p. 126.



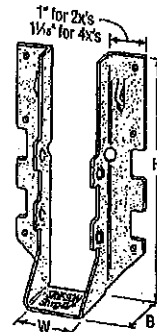
Double-Shear
Nailing
Top View



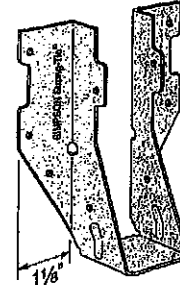
Double-Shear
Nailing
Side View;
Do not
bend tab



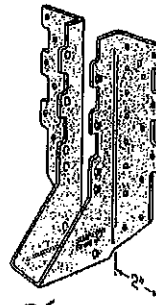
Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580



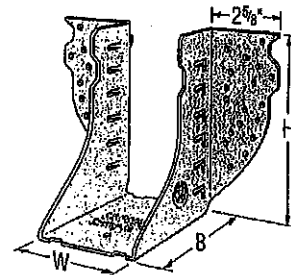
✓ LUS28



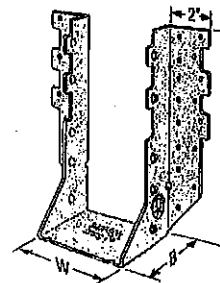
LU26L



✓ HUS210
(HUS26, HUS28,
and HHUS similar)

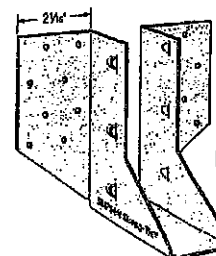
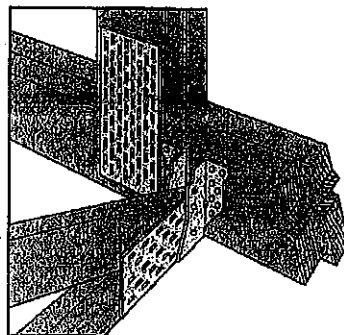


✓ HGUS28-2



✓ HHUS210-2

Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS

HHUS/HGUS

See Hanger Options information on pp. 125–127.

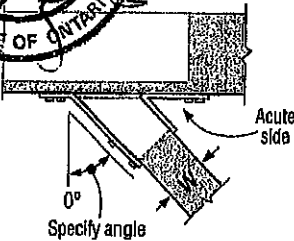
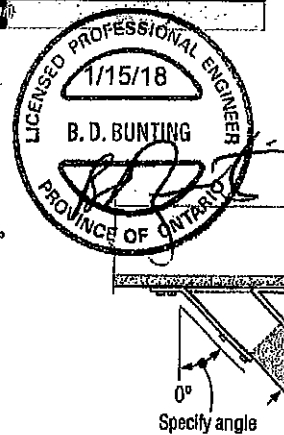
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
$W < 2"$	Bevel or square cut	0.62 of table value	0.46 of table value
$2" < W < 6"$	Bevel cut	0.67 of table value	0.41 of table value
$2" < W < 6"$	Square cut	0.46 of table value	0.41 of table value
$W > 6"$	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger
Skewed Right
(joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance			
		W	H	B	d _e ³	Header	Joist		D-Fir-L		S-P-F	
									Uplift	Normal	Uplift	Normal
									(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
									lb.	lb.	lb.	lb.
									kN	kN	kN	kN
Single 2x Sizes												
■	LUS24	18	1 1/8	3 1/4	1 3/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155
									3.16	7.23	2.87	5.14
■	LU24L	22	1 1/8	3	1 1/4	2 1/4	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725
									1.60	4.54	1.42	3.22
■	LU26L	22	1 1/8	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"	720	1605	645	1140
									3.20	7.14	2.87	5.07
SS	LUS26	18	1 1/8	4 1/4	1 1/4	3 3/4	(4) 10d	(4) 10d	1420	2170	1290	1630
									6.32	9.65	5.74	7.25
■	HUS26	18	1 1/8	5 1/4	3	3 3/4	(14) 10d	(6) 16d	2705	4940	2065	3875
									11.30	21.97	9.20	17.24
■	LJS26DS	18	1 1/8	5	3 1/4	4 1/4	(16) 16d	(6) 16d	2055	4265	1460	4115
									9.14	18.97	6.49	18.31
■	HGUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 16d	(8) 16d	2885	6625	2685	5700
									11.98	29.51	11.96	25.35
■	LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1550
									5.07	9.72	4.54	6.89
SS	LUS28	18	1 1/8	6 1/4	1 1/4	3 3/4	(6) 10d	(4) 10d	1420	2520	1290	1790
									6.32	11.21	5.74	7.96
■	HUS28	16	1 1/8	7 1/4	3	6 1/4	(22) 16d	(8) 16d	3805	5385	2675	4345
									16.04	23.86	11.90	19.33
■	HGUS28	12	1 1/8	7 1/4	5	6 1/4	(36) 16d	(12) 16d	3310	7675	3310	6900
									14.74	34.19	14.74	30.73
■	LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(8) 10d x 1 1/2"	1140	2495	1020	1770
									5.07	11.10	4.54	7.87
SS	LUS210	18	1 1/8	7 1/4	1 1/4	3 3/4	(8) 10d	(4) 10d	1420	2785	1290	2210
									6.32	12.39	5.74	9.83

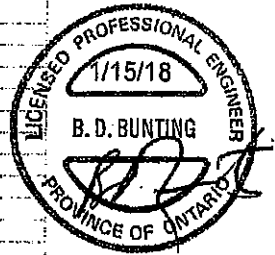
1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSON and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _a ^a	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K ₀ = 1.15)	(K ₀ = 1.00)	(K ₀ = 1.15)	(K ₀ = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
■ LUS24-2	18	3 3/8	3 3/8	2	1 1/2	(4) 16d	(2) 16d	835	2020	590	1435
SS LUS26-2	18	3 3/8	4 3/8	2	4	(4) 16d	(4) 16d	3.71	8.99	2.62	6.38
■ HHUS26-2	14	3 3/8	5 3/8	3	3 3/8	(14) 16d	(6) 16d	1720	2595	1545	1920
■ HGUS26-2	12	3 3/8	5 3/8	4	4 3/8	(20) 16d	(8) 16d	7.65	11.54	6.87	8.64
■ HHUS28-2	14	3 3/8	7 3/8	3	6 3/8	(22) 16d	(8) 16d	2850	7335	2065	5205
SS HHUS28-2	14	3 3/8	7 3/8	3	6 3/8	(22) 16d	(8) 16d	12.68	32.63	9.20	23.15
■ HGUS28-2	12	3 3/8	7 3/8	4	8 3/8	(36) 16d	(12) 16d	4385	8950	3110	6355
■ HHUS210-2	14	3 3/8	9 3/8	3	8 3/8	(30) 16d	(10) 16d	19.51	39.81	13.83	28.27
SS HHUS210-2	14	3 3/8	9 3/8	3	8 3/8	(30) 16d	(10) 16d	1720	3325	1545	2575
■ HGUS210-2	12	3 3/8	9 3/8	4	10 3/8	(56) 16d	(20) 16d	7.65	14.79	6.87	11.45
■ HHUS210-2	14	3 3/8	9 3/8	3	8 3/8	(30) 16d	(10) 16d	3785	8940	2675	6345
■ HGUS210-2	12	3 3/8	9 3/8	4	10 3/8	(56) 16d	(20) 16d	16.75	39.77	11.90	28.22
■ HHUS210-2	14	3 3/8	9 3/8	3	8 3/8	(30) 16d	(10) 16d	6070	12980	4310	9215
■ HGUS210-2	12	3 3/8	9 3/8	4	10 3/8	(56) 16d	(20) 16d	27.00	57.74	19.17	40.99
■ HHUS210-2	14	3 3/8	9 3/8	3	8 3/8	(30) 16d	(10) 16d	2580	4500	2320	3195
■ HGUS210-2	12	3 3/8	9 3/8	4	10 3/8	(56) 16d	(20) 16d	11.48	20.02	10.32	14.21
■ HHUS210-2	14	3 3/8	9 3/8	3	8 3/8	(30) 16d	(10) 16d	4670	9660	4235	7000
■ HGUS210-2	12	3 3/8	9 3/8	4	10 3/8	(56) 16d	(20) 16d	29.77	42.97	18.84	31.14
■ HHUS210-2	14	3 3/8	9 3/8	3	8 3/8	(30) 16d	(10) 16d	6840	14015	4855	10270
■ HGUS210-2	12	3 3/8	9 3/8	4	10 3/8	(56) 16d	(20) 16d	30.43	62.34	21.60	45.69
Triple 2x Sizes											
■ HGUS26-3	12	4 3/8	5 3/8	4	4 3/8	(20) 16d	(8) 16d	4385	8950	3110	6355
■ HGUS28-3	12	4 3/8	7 3/8	4	6 3/8	(36) 16d	(12) 16d	19.51	39.81	13.83	28.27
■ HHUS210-3	14	4 3/8	9 3/8	3	7 3/8	(30) 16d	(10) 16d	6070	12980	4310	9215
■ HGUS210-3	12	4 3/8	9 3/8	4	8 3/8	(46) 16d	(16) 16d	27.00	57.74	19.17	40.99
■ HHUS210-3	14	4 3/8	9 3/8	3	7 3/8	(30) 16d	(10) 16d	4670	9670	4235	6865
■ HGUS210-3	12	4 3/8	9 3/8	4	8 3/8	(46) 16d	(16) 16d	20.77	43.02	18.84	30.54
■ HHUS210-3	14	4 3/8	9 3/8	3	7 3/8	(30) 16d	(10) 16d	6840	14645	4855	10400
■ HGUS210-3	12	4 3/8	9 3/8	4	8 3/8	(46) 16d	(16) 16d	30.43	65.14	21.60	46.26
Quadruple 2x Sizes											
■ HGUS26-4	12	6 3/8	5 3/8	4	4 3/8	(20) 16d	(8) 16d	4385	8950	3110	6355
■ HGUS28-4	12	6 3/8	7 3/8	4	6 3/8	(36) 16d	(12) 16d	19.51	39.81	13.83	28.27
■ HHUS210-4	14	6 3/8	8 3/8	3	7 3/8	(30) 16d	(10) 16d	6070	12980	4310	9215
■ HGUS210-4	12	6 3/8	9 3/8	4	8 3/8	(46) 16d	(16) 16d	27.00	57.74	19.17	40.99
■ HHUS210-4	14	6 3/8	8 3/8	3	7 3/8	(30) 16d	(10) 16d	4670	10165	4235	7210
■ HGUS210-4	12	6 3/8	9 3/8	4	8 3/8	(46) 16d	(16) 16d	20.77	45.17	18.84	32.07
■ HHUS210-4	14	6 3/8	8 3/8	3	7 3/8	(30) 16d	(10) 16d	6840	14645	4855	10400
■ HGUS210-4	12	6 3/8	9 3/8	4	8 3/8	(46) 16d	(16) 16d	30.43	65.14	21.60	46.26
■ HHUS212-4	12	6 3/8	10 3/8	4	10 3/8	(56) 16d	(20) 16d	7640	14995	5425	10645
■ HGUS214-4	12	6 3/8	12 3/8	4	11 3/8	(66) 16d	(22) 16d	33.98	66.70	24.13	47.35
■ HHUS214-4	12	6 3/8	12 3/8	4	11 3/8	(66) 16d	(22) 16d	10130	16400	7195	11645
■ HGUS214-4	12	6 3/8	12 3/8	4	11 3/8	(66) 16d	(22) 16d	45.06	72.95	32.00	51.80
4x Sizes											
■ LUS46	18	3 3/8	4 3/8	2	3 3/8	(4) 16d	(4) 16d	1720	2595	1545	1920
■ HHUS46	14	3 3/8	5 3/8	3	3 3/8	(14) 16d	(6) 16d	7.65	11.54	6.87	8.64
■ HGUS46	12	3 3/8	5 3/8	4	4 3/8	(20) 16d	(8) 16d	2540	7335	2065	5205
■ LUS48	18	3 3/8	6 3/8	2	3 3/8	(6) 16d	(4) 16d	11.30	32.63	9.20	23.15
■ HHUS48	14	3 3/8	7 3/8	3	6 3/8	(22) 16d	(8) 16d	4385	8950	3110	6355
■ HGUS48	12	3 3/8	7 3/8	4	6 3/8	(36) 16d	(12) 16d	19.51	39.81	13.83	28.27
■ LUS410	18	3 3/8	8 3/8	2	5 3/8	(8) 16d	(6) 16d	1720	3325	1545	2575
■ HHUS410	14	3 3/8	9 3/8	3	8 3/8	(30) 16d	(10) 16d	7.65	14.79	6.87	11.45
■ HGUS410	12	3 3/8	9 3/8	4	8 3/8	(46) 16d	(16) 16d	3785	8940	2675	6345
■ LUS412	18	3 3/8	10 3/8	2	6 3/8	(12) 16d	(10) 16d	16.75	39.77	11.90	28.22
■ HHUS412	14	3 3/8	11 3/8	3	9 3/8	(36) 16d	(12) 16d	6070	12980	4310	9215
■ HGUS412	12	3 3/8	11 3/8	4	10 3/8	(56) 16d	(20) 16d	27.00	57.74	19.17	40.99
■ LUS414	18	3 3/8	12 3/8	2	7 3/8	(16) 16d	(14) 16d	2580	4500	2320	3195
■ HHUS414	14	3 3/8	13 3/8	3	10 3/8	(46) 16d	(16) 16d	11.48	20.02	10.32	14.21
■ HGUS414	12	3 3/8	13 3/8	4	11 3/8	(66) 16d	(22) 16d	4670	9660	4235	7000
■ LUS416	18	3 3/8	14 3/8	2	8 3/8	(20) 16d	(18) 16d	29.77	42.97	18.84	31.14
■ HHUS416	14	3 3/8	15 3/8	3	11 3/8	(56) 16d	(22) 16d	6840	14015	4855	10270
■ HGUS416	12	3 3/8	15 3/8	4	12 3/8	(76) 16d	(28) 16d	30.43	62.34	21.60	45.69
■ LUS418	18	3 3/8	16 3/8	2	9 3/8	(24) 16d	(22) 16d	7640	14995	5425	10645
■ HHUS418	14	3 3/8	17 3/8	3	12 3/8	(66) 16d	(24) 16d	33.98	66.70	24.13	47.35
■ HGUS418	12	3 3/8	17 3/8	4	13 3/8	(86) 16d	(30) 16d	10130	16400	7195	11645
■ LUS420	18	3 3/8	18 3/8	2	10 3/8	(28) 16d	(26) 16d	45.06	72.95	32.00	51.80



Plated Truss Connectors

See footnotes on p. 258.

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

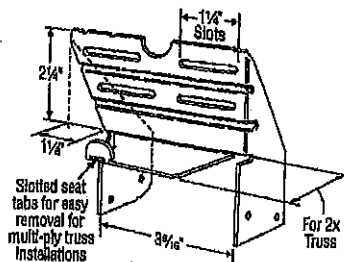
Design: Factored resistances are in accordance with CSA 086-14

Installation:

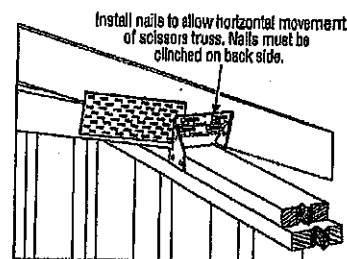
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

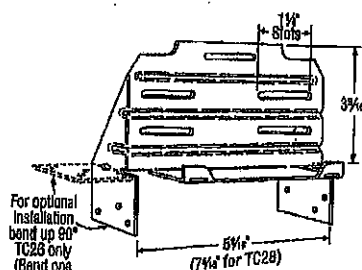
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



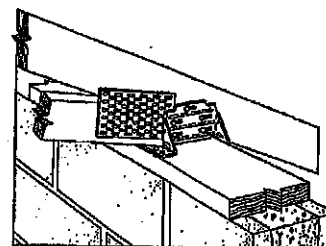
TC24
U.S. Patent 4,932,173



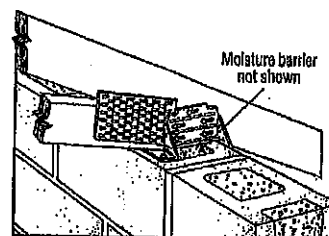
Typical TC24 Installation



TC26
(TC28 Similar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



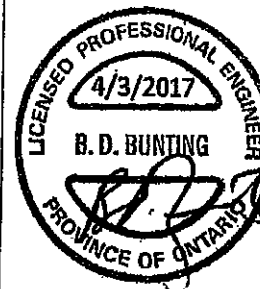
Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K ₀ =1.15)	Uplift (K ₀ =1.15)
			lb.	lb.
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(5) 10d	1015	720
TC28	(5) 10d	(5) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K ₀ =1.15)	Uplift (K ₀ =1.15)
			lb.	lb.
TC26	(5) 10d	(5) 10d x 1½"	810	660
	(5) 10d	(5) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (8) - ¾" x 2¼" Titen screws has a factored uplift resistance of 276 lb.



DESIGN

(800) 999-5099
strongtie.com

H/TSP

Seismic and Hurricane Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Fasteners			Factored Resistance ($K_p = 1.15$)					
					D.Fir-L			S-P-F		
		To Rafters/ Truss	To Plates	To Studs	Uplift	Lateral		Uplift	Lateral	
						F_1	F_2		F_1	F_2
					lb.	lb.	lb.	lb.	lb.	lb.
					kN	kN	kN	kN	kN	kN
H1	18	(6) 8d x 1 1/2"	(4) 8d	—	740	685	300	680	485	215
					3.29	3.05	1.33	3.02	2.16	0.96
SS H2A	18	(5) 8d x 1 1/2"	(2) 8d x 1 1/2"	(5) 8d x 1 1/2"	830	220	75	590	155	55
					3.68	0.98	0.33	2.62	0.69	0.24
SS H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
					3.58	0.71	0.71	3.36	0.71	0.71
H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
					3.71	0.78	0.93	3.29	0.71	0.93
SS H3	18	(4) 8d	(4) 8d	—	740	180	285	615	125	190
					3.29	0.80	1.18	2.74	0.56	0.85
H6	16	—	(8) 8d	(8) 8d	1585	1085	—	1125	770	—
					7.05	4.83	—	5.00	3.43	—
H7Z	16	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
					6.18	2.98	—	4.40	2.11	—
SS H8 ²	18	(5) 10d x 1 1/2"	(5) 10d x 1 1/2"	—	1120	—	—	1025	—	—
					4.98	—	—	4.66	—	—
SS H10A ³	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1735	795	410	1505	566	290
					7.72	3.54	1.82	6.69	2.51	1.29
H10AR	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1485	690	430	1220	570	305
					6.61	3.07	1.91	5.43	2.54	1.36
H10A-2	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1835	1275	430	1645	880	305
					8.16	5.67	1.91	7.32	3.91	1.36
H10S ^{2,3}	18	(8) 8d x 1 1/2"	(8) 8d x 1 1/2"	(8) 8d	1465	795	315	1040	565	225
					6.52	3.54	1.40	4.63	2.51	1.00
H11Z	18	(6) 16d x 2 1/2"	(6) 16d x 2 1/2"	—	1095	920	545	780	655	390
					4.87	4.09	2.42	3.47	2.91	1.73
H14	18	[1] (12) 8d x 1 1/2"	(13) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
		[2] (12) 8d x 1 1/2"	(15) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
TSP	16	(9) 10d x 1 1/2"	(6) 10d x 1 1/2"	—	1295	440	—	920	310	—
					5.76	1.98	—	4.09	1.38	—
		(9) 10d x 1 1/2"	(6) 10d	—	1560	440	—	1105	310	—
					6.94	1.98	—	4.92	1.38	—

- Factored resistances have been increased 15% for short term loading; no further increase is allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2 1/2" must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
- H8 factored uplift resistances for stud-to-bottom plate installations are 595 lb. (2.65 kN) for D.Fir-L and 390 lb. (1.74 kN) for S-P-F.
- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-HTIECONPATH).
- Factored resistances in the F_1 direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.

- H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.38 kN) D.Fir-L and 1015 lb. (4.51 kN) S-P-F.
- H10S nails to plates are optional for uplift but required for lateral loads.
- H10A may be field-bent up to a slope of 8/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
- The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SSNAILS at strongtie.com.
- D.Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1 1/2" nails into the top plates and (3) 8d x 1 1/2" nails into the lowest three flange holes into the truss bottom chord is 495 lb. (2.20 kN).
- Nails: 16d x 2 1/2" = 0.162" dia. x 2 1/2" long, 10d = 0.148" dia. x 3" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 8d = 0.131" dia. x 2 1/2" long, 8d x 1 1/2" = 0.131" dia. x 1 1/2" long. See pp. 27–28 for other nail sizes and information.

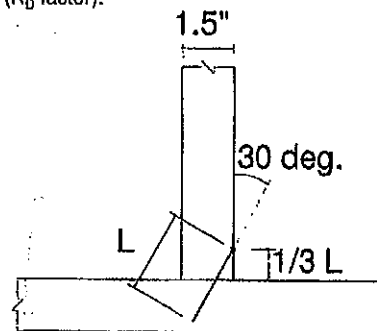
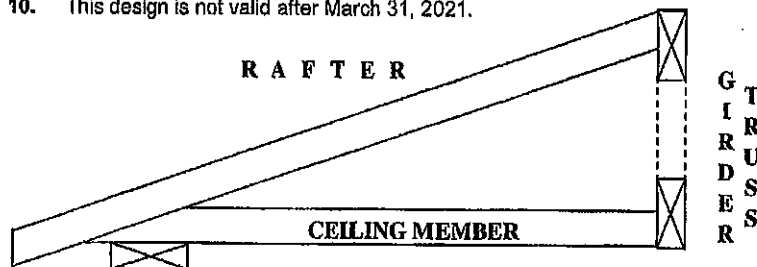
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

- Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
- Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
- For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
- Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
- Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
- Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
- For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
- Lumber must be dry (< 19% moisture content) at the time of nail installation.
- Nail values in this table comply with CSA O86-14, section 12.9.4
- This design is not valid after March 31, 2021.

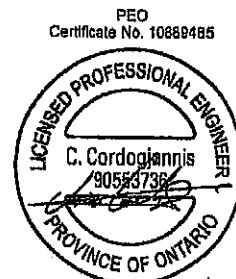


TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

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December 2, 2019



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

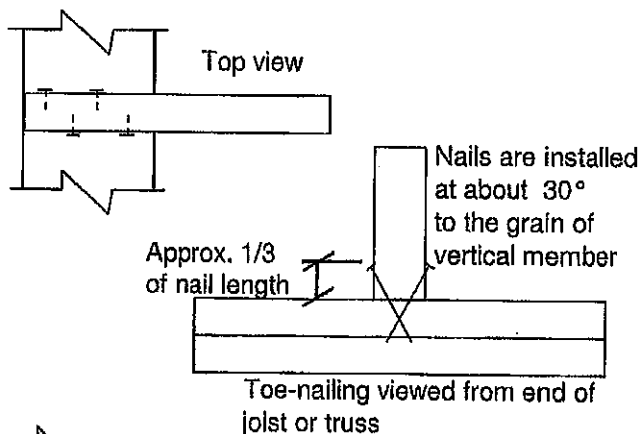
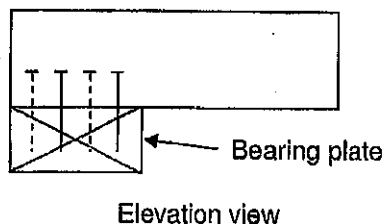
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

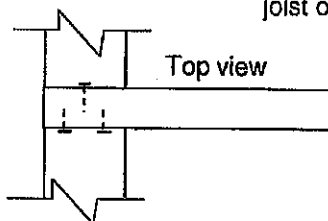
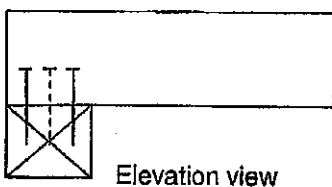
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



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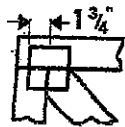
PEO
Certificate No. 10889485



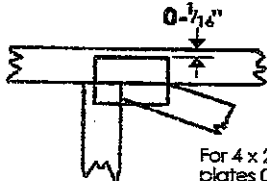
December 2, 2019

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint, unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/4\" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



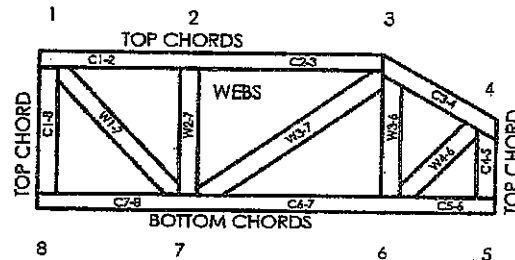
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)

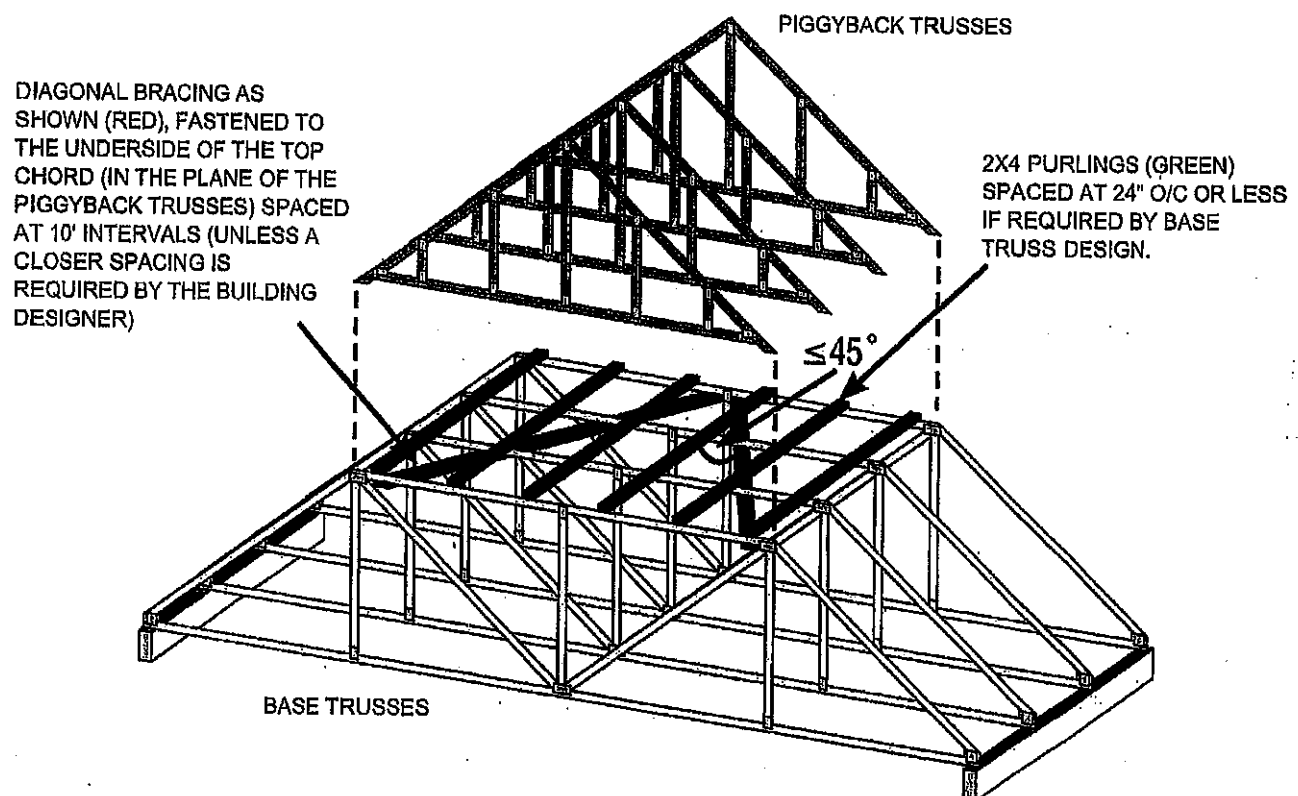


Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

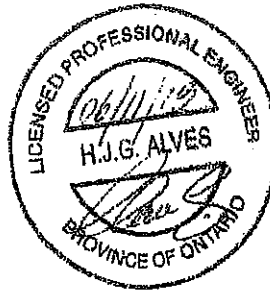


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet M117473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800219

Feb 09, 2018