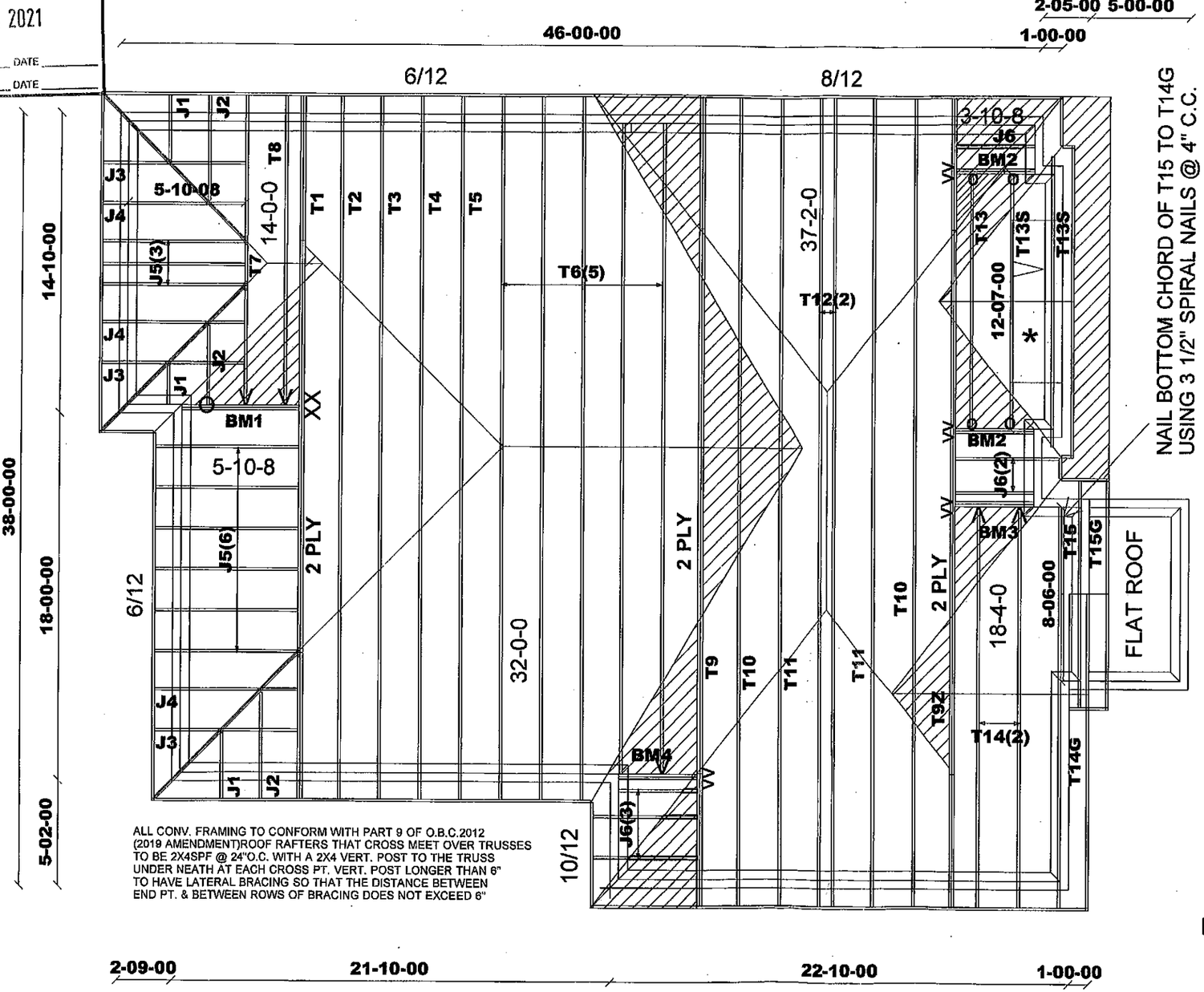
CITY OF HAMILTON
BUILDING DIVISION
Planning & Development Department
FEB 04 2021
RECD BY _____ DATE _____
EXAM'D _____ DATE _____



ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C.2012 (2019 AMENDMENT)ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4SPF @ 24"O.C. WITH A 2X4 VERT. POST TO THE TRUSS UNDER NEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6" TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6"

ASPHALT SHINGLES
12"FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
LUS26-2- (VV)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C.2012 PART 9 (2019 AMENDMENT)
DESIGN LOADS:
SNOW LOAD 25.6 PSF
TC DEAD 6 PSF
BC LIVE 0 PSF
BC DEAD 7.4 PSF

* 16" RAISED CEILING

BM1, BM2, BM3, BM4 = 2-2X10

CITY OF HAMILTON
Building Division
Permit No. 21-105908
THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE
THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW
These drawings and/or specifications have been reviewed by
2/19/21
FOR CHIEF BUILDING OFFICIAL DATE

M13112

TAMARACK ROOF TRUSSES INC.

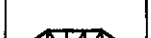
Job Track: 51225	Builder / Location: GREEN PARK HOMES / WATERDOWN	Model / Elevation: VALLEYCREEK 6/1	Mitek ver 8.3.1.215
Plan Log: 202415			LOT 248
Layout ID: 408204	Project: RUSSELL GARDENS PH.3		
	Date: 2020-04-16 Sales: Mario DiCano Designer: AC/JG		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51225
Builder:	GREEN PARK HOMES	PlanLog:	202415
Project:	RUSSELL GARDENS PH.3	Layout ID:	408204
Location:	WATERDOWN	Ref #	
Model:	VALLEYCREEK 6	Page:	1 of 2
Lot #:	248	Date:	04-16-2020
Elevation:	1	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

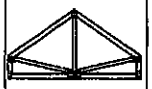
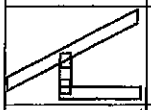
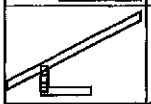
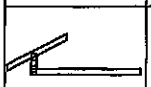
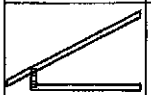
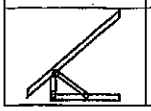
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1 Hip Girder	6 /12	32-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	288.29 184.00		
	1	T2 Hip	6 /12	32-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	129.76 81.50		
	1	T3 Hip	6 /12	32-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	129.36 82.33		
	1	T4 Hip	6 /12	32-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	135.83 84.83		
	1	T5 Hip	6 /12	32-00-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	131.5 83.17		
	5	T6 Hip	6 /12	32-00-00	9-02-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	647.77 407.50		
	1	T7 Hip Girder	6 /12	14-00-00	4-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	59.02 38.67		
	1	T8 Common	6 /12	14-00-00	4-08-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	55.49 35.50		
	1 2-ply	T9 Hip Girder	8 /12	37-02-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	351.67 215.33		
	1 2-ply	T9Z Hip Girder	8 /12	37-02-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	351.67 215.33		
	2	T10 Hip	8 /12	37-02-00	6-06-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	327.85 202.00		
	2	T11 Hip	8 /12	37-02-00	8-02-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	337.47 212.00		
	2	T12 Hip	8 /12	37-02-00	9-10-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	351.09 220.67		
	1	T13 Common	8 /12	12-07-00	5-07-02	2 x 4		1-04-13 1-04-13	51.65 33.17		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51225
Builder:	GREEN PARK HOMES	PlanLog:	202415
Project:	RUSSELL GARDENS PH.3	Layout ID:	408204
Location:	WATERDOWN	Ref #	
Model:	VALLEYCREEK 6	Page:	2 of 2
Lot #:	248	Date:	04-16-2020
Elevation:	1	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	T13S Roof Special	8 /12	12-07-00	5-07-02	2 x 4		1-04-13 1-04-13	113.8 76.67		
	2	T14 Common	8 /12	18-04-00	7-06-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	161.18 100.67		
	1	T14G GABLE	8 /12	18-04-00	7-06-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	83.28 52.83		
	1	T15 Common	8 /12	8-06-00	4-02-13	2 x 4		1-04-13 1-04-13	33.35 21.33		
	1	T15G GABLE	8 /12	8-06-00	4-02-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	37.61 24.67		
	3	J1 Jack-Open	6 /12	2-00-00	2-01-08	2 x 4	1-03-08	1-02-00 2-02-00	21.97 14.00		
	3	J2 Jack-Open	6 /12	2-00-00	3-01-08	2 x 4	1-03-08 1-10-15	1-02-00 2-02-00	29.62 18.00		
	3	J3 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 3-11-09	1-02-00 2-01-08	35.22 22.00		
	3	J4 Jack-Open	6 /12	3-10-15	3-01-08	2 x 4	1-03-08 1-11-09	1-02-00 3-01-08	42.87 26.00		
	9	J5 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	151.15 96.00		
	6	J6 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	93.44 61.00		

TOTAL # TRUSS= **58** TOTAL BFT OF ALL TRUSSES= **2609.17** BFT. TOTAL WEIGHT OF ALL TRSSES **4151.89** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
5	Hardware	LJS26DS	
5	Hardware	LUS24	
4	Hardware	LUS26-2	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= **15**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	T1	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Fri Apr 17 07:47:44 2020 Page 1
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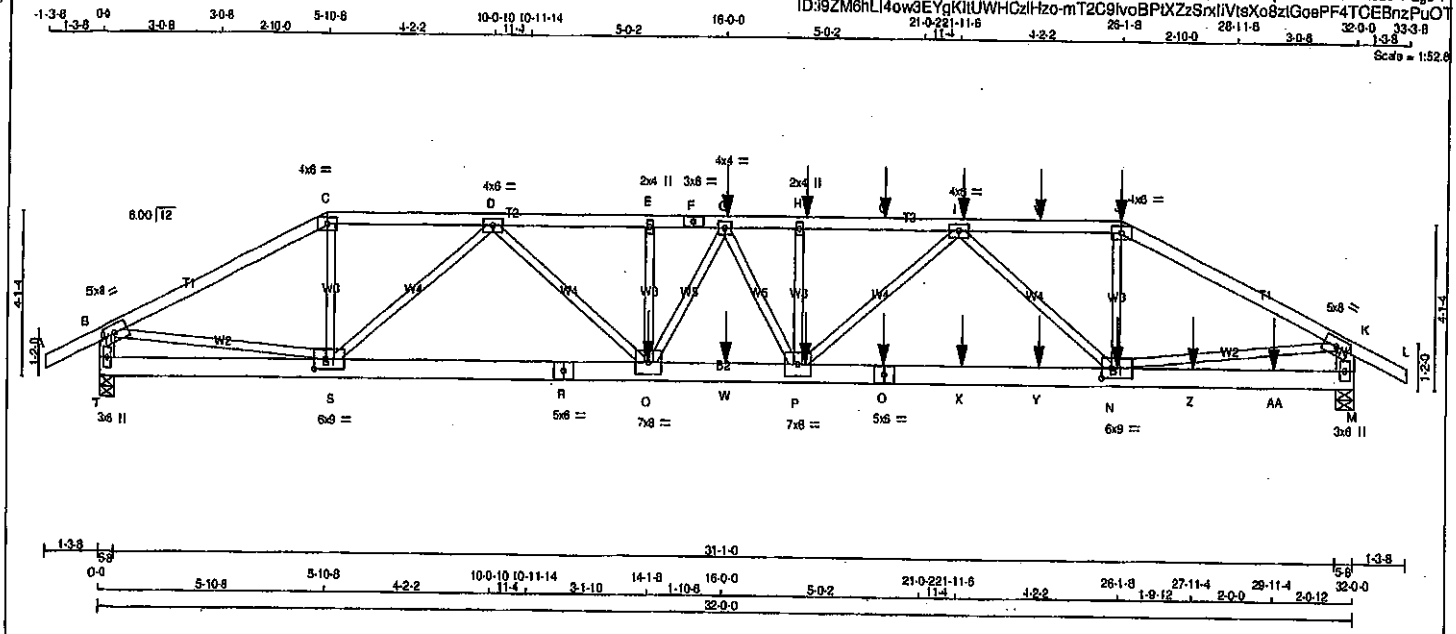


TOTAL WEIGHT = 2 X 144 = 288 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
T - R	2x8	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-F	1	12	TOP
F-J	1	12	SIDE(61.0)
J-L	1	12	SIDE(61.0)
T-B	2	12	TOP
M-K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T-R	2	12	TOP
R-O	2	12	SIDE(183.1)
O-M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	8	
E-Q	1	5	SIDE(313.7)
H-P	1	5	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TTW-t	MT20	4.0	8.0		
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMW+w	MT20	2.0	4.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTW-t	MT20	4.0	6.0		
K	TMVW-t	MT20	5.0	8.0		
M	BMV1+p	MT20	3.0	6.0		
N	BMWWW-t	MT20	8.0	9.0	3.00	3.00
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	7.0	8.0		
Q	BMWWW-t	MT20	7.0	8.0		
R	BS-t	MT20	5.0	6.0		
S	BMWWW-t	MT20	6.0	9.0	3.00	3.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	3303	0	3303	0	0	5-8	5-8
M	3600	0	3600	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2329	1568 / 0	0 / 0	0 / 0	0 / 0	761 / 0	0 / 0
M	2541	1695 / 0	0 / 0	0 / 0	0 / 0	846 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
A-B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	S-C	0 / 1846	0.23 (1)
B-C	-5078 / 0	-91.8	-91.8	0.52 (1)	3.86	Q-E	-234 / 0	0.03 (1)
C-D	-4631 / 0	-91.8	-91.8	0.22 (1)	4.30	P-H	-421 / 0	0.05 (1)
D-E	-9059 / 0	-91.8	-91.8	0.46 (1)	2.97	N-J	0 / 1566	0.19 (1)
E-F	-9059 / 0	-91.8	-91.8	0.31 (1)	3.08	N-K	0 / 5016	0.62 (1)
F-G	-9059 / 0	-91.8	-91.8	0.31 (1)	3.08	Q-G	0 / 429	0.05 (1)
G-H	-8547 / 0	-91.8	-91.8	0.28 (1)	3.19	G-P	-711 / 0	0.11 (1)
H-U	-8547 / 0	-91.8	-91.8	0.49 (1)	3.01	D-Q	0 / 2705	0.33 (1)
U-I	-8547 / 0	-91.8	-91.8	0.49 (1)	3.01	P-I	0 / 1886	0.23 (1)
I-V	-5053 / 0	-91.8	-91.8	0.29 (1)	4.05	B-S	0 / 4576	0.57 (1)
V-J	-5053 / 0	-91.8	-91.8	0.29 (1)	4.05	S-D	-3330 / 0	0.87 (1)
J-K	-5568 / 0	-91.8	-91.8	0.58 (1)	3.69	I-N	-2878 / 0	0.76 (1)
K-L	0 / 28	-91.8	-91.8	0.07 (1)	10.00			
T-B	-3250 / 0	0.0	0.0	0.11 (1)	7.69			
M-K	-3525 / 0	0.0	0.0	0.12 (1)	7.48			
T-S	0 / 0	-18.5	-18.5	0.05 (4)	10.00			
S-R	0 / 7064	-18.5	-18.5	0.53 (1)	10.00			
R-Q	0 / 7064	-18.5	-18.5	0.53 (1)	10.00			
Q-W	0 / 8866	-18.5	-18.5	0.62 (1)	10.00			
W-P	0 / 8866	-18.5	-18.5	0.62 (1)	10.00			
P-O	0 / 7156	-18.5	-18.5	0.55 (1)	10.00			
O-X	0 / 7156	-18.5	-18.5	0.55 (1)	10.00			
X-Y	0 / 7156	-18.5	-18.5	0.55 (1)	10.00			
Y-N	0 / 7156	-18.5	-18.5	0.55 (1)	10.00			
N-Z	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
Z-AA	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
AA-M	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	LOAD
G	16-0-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
H	18-0-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
I	22-0-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
J	26-1-8	-459	-459	---	FRONT	VERT	TOTAL	---	C1
N	26-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	20-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	18-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Q	14-1-8	-1904	-1904	---	FRONT	VERT	TOTAL	---	C1
U	20-0-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	24-0-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	16-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
X	22-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 934 (0.41")

CSI: TC=0.56/1.00 (J-K:1) , BC=0.62/1.00 (P-Q:1) , WB=0.87/1.00 (D-S:1) , SSI=0.13/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.69 (R) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006376

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408204	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:i9ZM6hLj4ow3EYgKltUWHCzlHzo-mT2C9lvaBPlXZzSrxliVtsXo6ztGoePF4TCEBnzPuOT

PLATES (table is in inches)

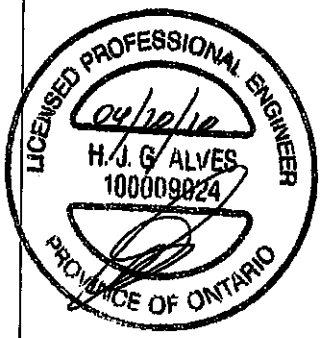
JT	TYPE	PLATES	W	LEN	Y	X
T	BMV1+p	MT20	3.0	6.0		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Y	24-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Z	27-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AA	29-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2006376

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	T2	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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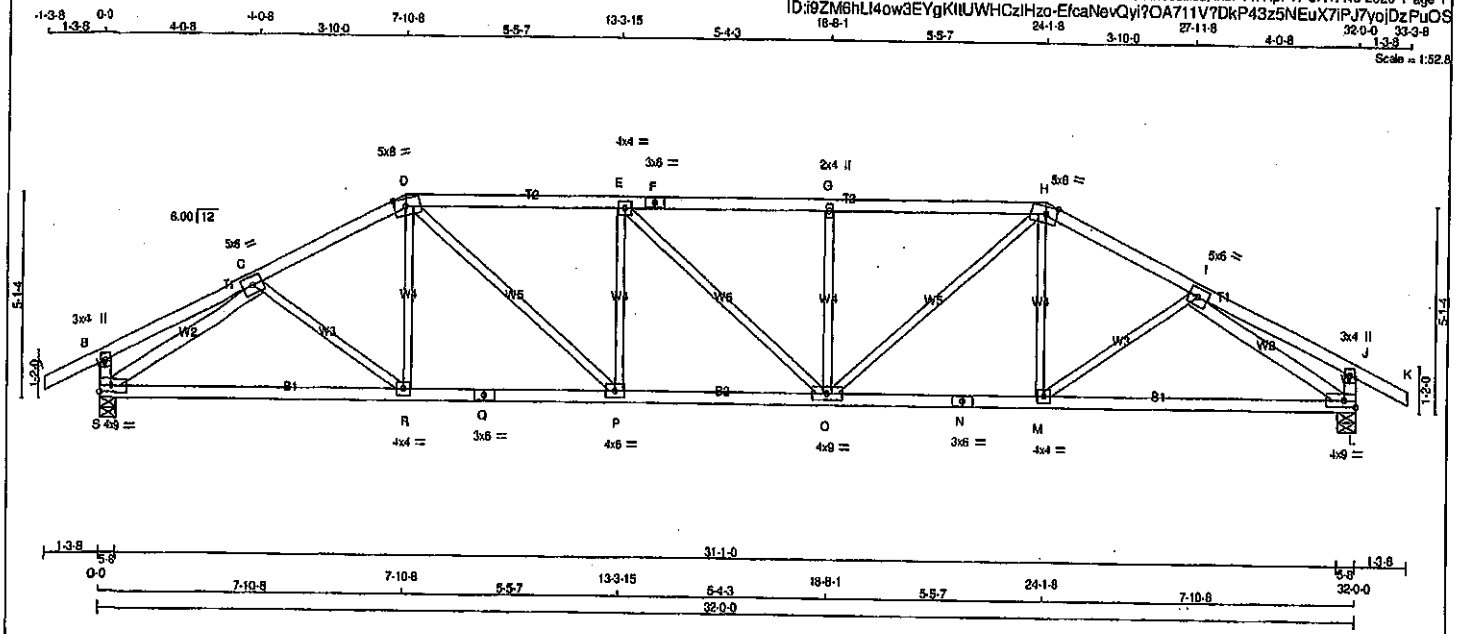


TOTAL WEIGHT = 130 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	8.0		
D	TTWW-m	MT20	5.0	8.0	2.25	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.25	3.50
I	TMWW-t	MT20	5.0	8.0		
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	9.0		Edge
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	RECRD BRG IN-SX
S	1889	0	1889	0	0	5-8	5-8
L	1889	0	1889	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1333	888 / 0	0 / 0	0 / 0	0 / 0	445 / 0	0 / 0
L	1333	888 / 0	0 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

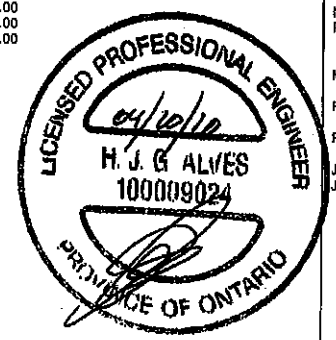
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B-C	0 / 17	-91.8	-91.8	0.20 (1)	10.00
C-D	-2477 / 0	-91.8	-91.8	0.30 (1)	4.11
D-E	-2906 / 0	-91.8	-91.8	0.55 (1)	3.59
E-F	-2906 / 0	-91.8	-91.8	0.54 (1)	3.59
F-G	-2906 / 0	-91.8	-91.8	0.54 (1)	3.59
G-H	-2906 / 0	-91.8	-91.8	0.54 (1)	3.59
H-I	-2478 / 0	-91.8	-91.8	0.30 (1)	4.11
I-J	0 / 17	-91.8	-91.8	0.20 (1)	10.00
J-K	0 / 28	-91.8	-91.8	0.12 (1)	10.00
S-B	-269 / 0	0.0	0.0	0.03 (1)	7.81
L-J	-269 / 0	0.0	0.0	0.03 (1)	7.81
S-R	0 / 2174	-18.5	-18.5	0.49 (1)	10.00
R-Q	0 / 2202	-18.5	-18.5	0.49 (1)	10.00
Q-P	0 / 2202	-18.5	-18.5	0.49 (1)	10.00
P-O	0 / 2906	-18.5	-18.5	0.53 (1)	10.00
O-N	0 / 2202	-18.5	-18.5	0.49 (1)	10.00
N-M	0 / 2202	-18.5	-18.5	0.49 (1)	10.00
M-L	0 / 2174	-18.5	-18.5	0.49 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-R	0 / 74	0.03 (4)
R-D	0 / 132	0.04 (4)
D-P	0 / 941	0.21 (1)
P-E	-537 / 0	0.21 (1)
E-O	-2 / 0	0.00 (1)
O-G	-536 / 0	0.21 (1)
O-H	0 / 939	0.21 (1)
M-H	0 / 133	0.04 (4)
M-J	0 / 74	0.03 (4)
S-C	-2674 / 0	0.74 (1)
I-L	-2674 / 0	0.74 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.56/1.00 (D-E:1) , BC=0.53/1.00 (O-P:1) , WB=0.74/1.00 (I-L:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (D) (INPUT = 0.90)
JSI METAL= 0.66 (I) (INPUT = 1.00)

Structural component only
DWG# T-2006377

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	T3	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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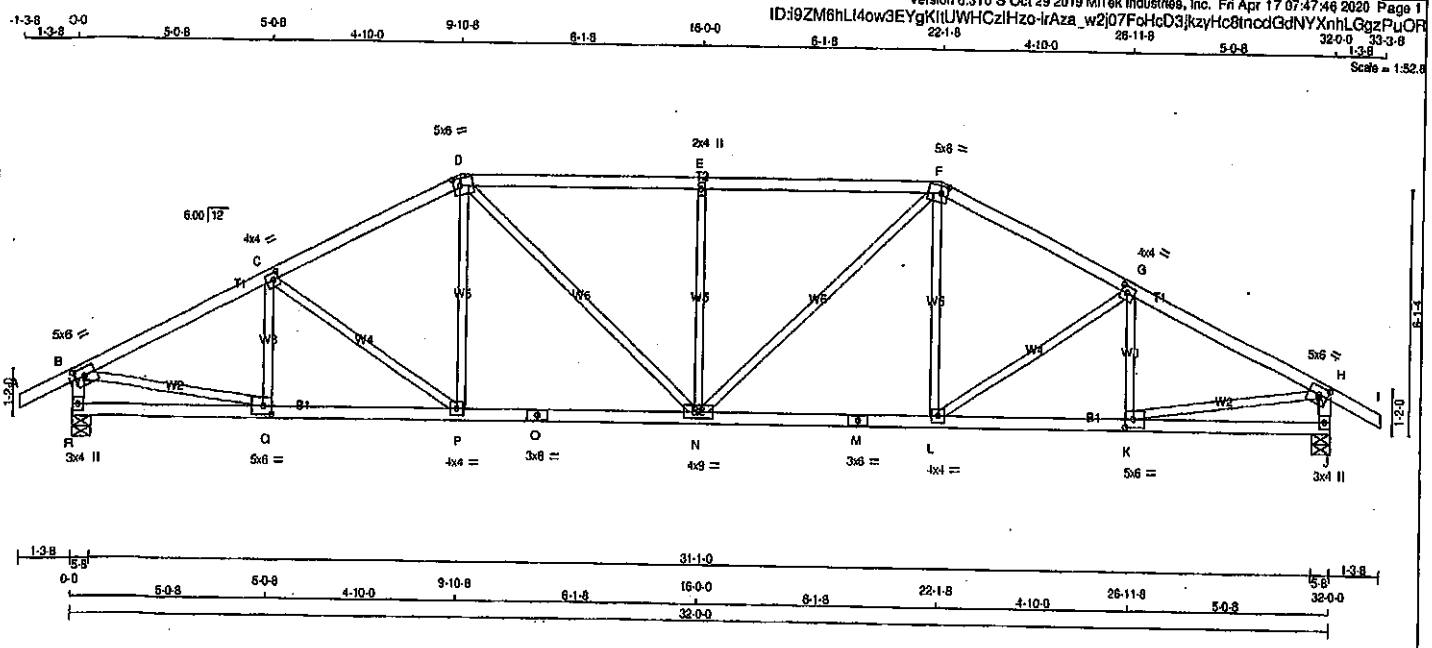

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.25	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	6.0	2.25	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.50
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 129 lb (M)(F)

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1889	0	1889	0	0	5-8	5-8
J	1889	0	1889	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1333	888 / 0	0 / 0	0 / 0	0 / 0	445 / 0	0 / 0
J	1333	888 / 0	0 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B-C	-2546 / 0	-91.8	-91.8	0.37 (1)	4.01
C-D	-2351 / 0	-91.8	-91.8	0.35 (1)	4.16
D-E	-2607 / 0	-91.8	-91.8	0.54 (1)	3.80
E-F	-2507 / 0	-91.8	-91.8	0.54 (1)	3.80
F-G	-2351 / 0	-91.8	-91.8	0.35 (1)	4.16
G-H	-2546 / 0	-91.8	-91.8	0.37 (1)	4.01
H-I	0 / 28	-91.8	-91.8	0.12 (1)	10.00
R-B	-1845 / 0	0.0	0.0	0.19 (1)	6.16
J-H	-1845 / 0	0.0	0.0	0.19 (1)	6.16
R-Q	0 / 0	-18.5	-18.5	0.10 (4)	10.00
Q-P	0 / 2297	-18.5	-18.5	0.44 (1)	10.00
P-O	0 / 2085	-18.5	-18.5	0.40 (1)	10.00
O-N	0 / 2085	-18.5	-18.5	0.40 (1)	10.00
N-M	0 / 2085	-18.5	-18.5	0.40 (1)	10.00
M-L	0 / 2085	-18.5	-18.5	0.40 (1)	10.00
L-K	0 / 2297	-18.5	-18.5	0.44 (1)	10.00
K-J	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-C	-302 / 0	0.07 (1)
C-P	-263 / 0	0.16 (1)
P-D	0 / 257	0.08 (1)
D-N	0 / 581	0.13 (1)
N-E	-690 / 0	0.40 (1)
N-F	0 / 581	0.13 (1)
L-F	0 / 257	0.08 (1)
L-G	-263 / 0	0.16 (1)
K-G	-302 / 0	0.07 (1)
B-Q	0 / 2330	0.52 (1)
K-H	0 / 2330	0.52 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.54/1.00 (D-E:1) , BC=0.44/1.00 (P-Q:1) , WB=0.52/1.00 (B-Q:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.68 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/16/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006378

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	T4	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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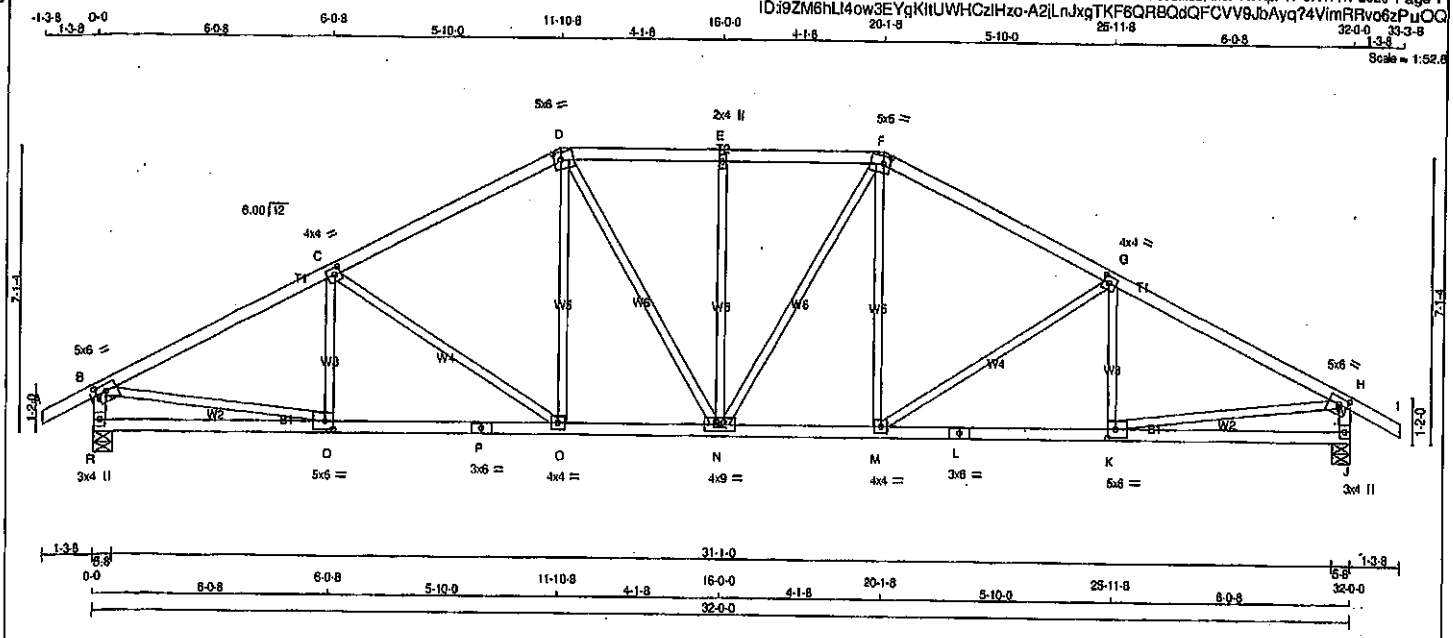


TOTAL WEIGHT = 136 lb (M)[F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	5.0	6.0	2.00	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMW-t	MT20	5.0	6.0	2.00	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.50
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1889	0	1889	0	0	5-8	5-8
J	1889	0	1889	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1333	888 / 0	0 / 0	0 / 0	0 / 0	445 / 0	0 0
J	1333	888 / 0	0 / 0	0 / 0	0 / 0	445 / 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

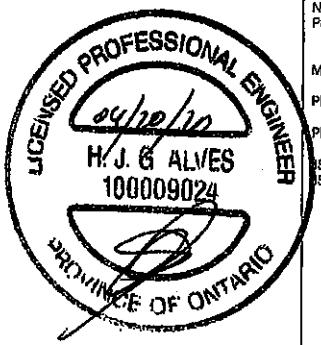
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B-C	-2587 / 0	-91.8	-91.8	0.55 (1)	3.78
C-D	-2197 / 0	-91.8	-91.8	0.49 (1)	4.11
D-E	-2099 / 0	-91.8	-91.8	0.24 (1)	4.48
E-F	-2099 / 0	-91.8	-91.8	0.24 (1)	4.48
F-G	-2197 / 0	-91.8	-91.8	0.49 (1)	4.11
G-H	-2587 / 0	-91.8	-91.8	0.55 (1)	3.78
H-I	0 / 28	-91.8	-91.8	0.12 (1)	10.00
R-B	-1840 / 0	0.0	0.0	0.19 (1)	6.17
J-H	-1840 / 0	0.0	0.0	0.19 (1)	6.17
R-Q	0 / 0	-18.5	-18.5	0.15 (4)	10.00
Q-P	0 / 2339	-18.5	-18.5	0.44 (1)	10.00
P-O	0 / 2339	-18.5	-18.5	0.44 (1)	10.00
O-N	0 / 1943	-18.5	-18.5	0.37 (1)	10.00
N-M	0 / 1943	-18.5	-18.5	0.37 (1)	10.00
M-L	0 / 2339	-18.5	-18.5	0.44 (1)	10.00
L-K	0 / 2339	-18.5	-18.5	0.44 (1)	10.00
K-J	0 / 0	-18.5	-18.5	0.15 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-C	-213 / 28	0.08 (1)
C-O	-479 / 0	0.46 (1)
O-D	0 / 362	0.08 (1)
D-N	0 / 301	0.07 (1)
N-E	-456 / 0	0.40 (1)
N-F	0 / 301	0.07 (1)
M-F	0 / 362	0.08 (1)
M-G	-479 / 0	0.46 (1)
K-G	-213 / 28	0.08 (1)
B-Q	0 / 2362	0.53 (1)
K-H	0 / 2362	0.53 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.55/1.00 (B-C:1) , BC=0.44/1.00 (O-Q:1) , WB=0.53/1.00 (H-K:1) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.88 (B) (INPUT = 0.90)
SSI METAL= 0.74 (P) (INPUT = 1.00)

Structural component only
DWG# T-2006379

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	T5	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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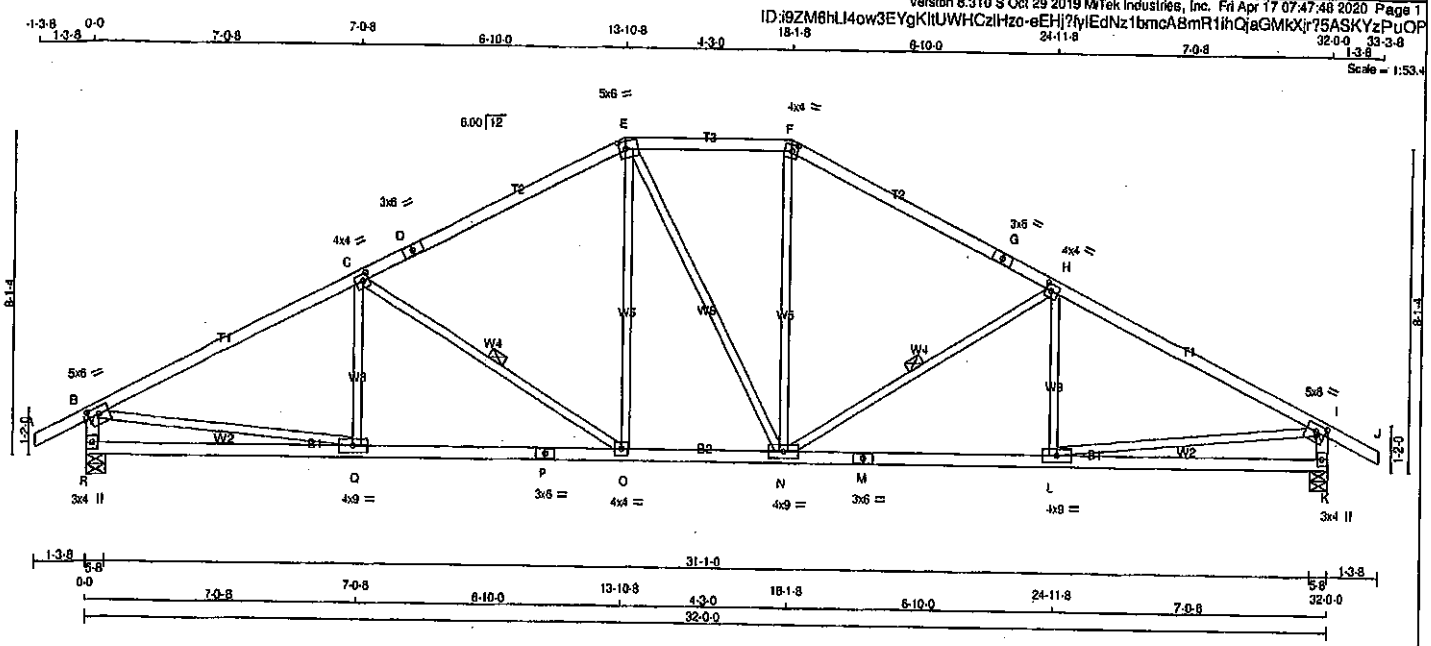


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0	2.00	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TTW-m	MT20	4.0	4.0	2.00	1.75
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.75
I	TMW-t	MT20	5.0	6.0	2.00	2.75
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	9.0		
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1889	0	1889	0	0	5-8	5-8
K	1889	0	1889	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1333	888 / 0	0 / 0	0 / 0	0 / 0	445 / 0	0 / 0
K	1333	888 / 0	0 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

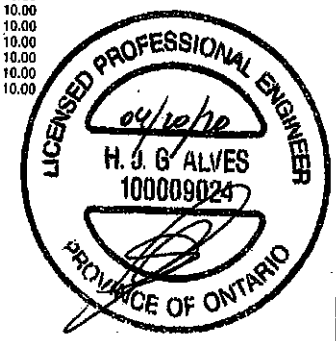
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B-C	-2595 / 0	-91.8	-91.8	0.78 (1)	3.47
C-D	-2022 / 0	-91.8	-91.8	0.68 (1)	3.97
D-E	-2022 / 0	-91.8	-91.8	0.68 (1)	3.97
E-F	-1783 / 0	-91.8	-91.8	0.25 (1)	4.77
F-G	-2023 / 0	-91.8	-91.8	0.68 (1)	3.97
G-H	-2023 / 0	-91.8	-91.8	0.68 (1)	3.97
H-I	-2595 / 0	-91.8	-91.8	0.78 (1)	3.47
I-J	0 / 28	-91.8	-91.8	0.12 (1)	10.00
R-B	-1834 0	0.0	0.0	0.19 (1)	6.18
K-I	-1834 0	0.0	0.0	0.19 (1)	6.18
R-Q	0 0	-18.5	-18.5	0.22 (4)	10.00
Q-P	0 / 2351	-18.5	-18.5	0.48 (1)	10.00
P-O	0 / 2351	-18.5	-18.5	0.48 (1)	10.00
O-N	0 / 1783	-18.5	-18.5	0.35 (1)	10.00
N-M	0 / 2351	-18.5	-18.5	0.48 (1)	10.00
M-L	0 / 2351	-18.5	-18.5	0.48 (1)	10.00
L-K	0 / 0	-18.5	-18.5	0.22 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-C	-141 / 71	0.05 (1)
C-O	-680 / 0	0.31 (1)
O-E	0 / 458	0.10 (1)
E-N	0 / 2	0.00 (1)
N-F	0 / 460	0.10 (1)
N-H	-679 . 0	0.31 (1)
L-H	-143 / 70	0.05 (1)
B-Q	0 / 2369	0.53 (1)
L-I	0 . 2368	0.53 (1)



TOTAL WEIGHT = 132 lb
(M)(F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.78/1.00 (B-C:1) , BC=0.48/1.00 (O-Q:1) , WB=0.53/1.00 (B-Q:1) , SSI=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.77 (P) (INPUT = 1.00)

Structural component only
DWG# T-2006380

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408204	T6	5	1	TRUSS DESC.	

Tamarack Road Truss, Burlington

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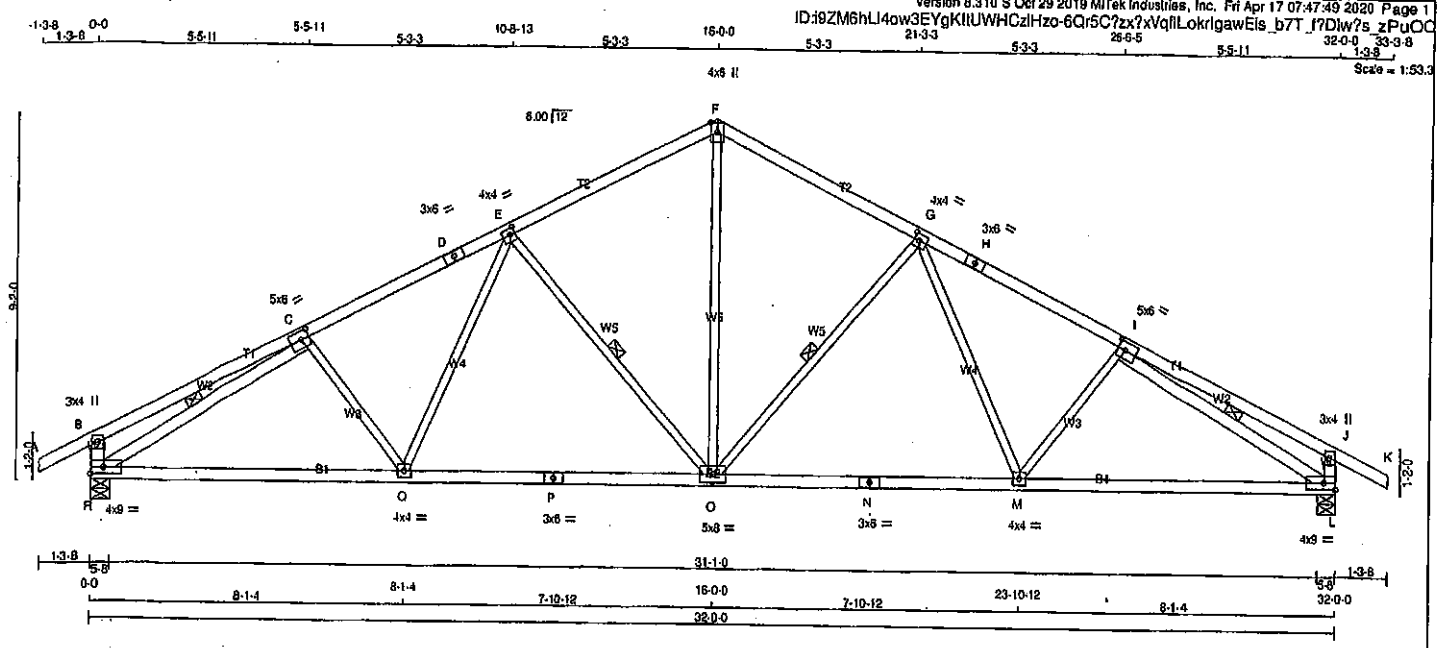


TOTAL WEIGHT = 5 X 134 = 668 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TTW+p	MT20	4.0	8.0	Edge	
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.50
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	9.0		Edge
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	1889	0	1889	0	0	IN-SX	IN-SX
L	1889	0	1889	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1333	888 / 0	0 / 0	0 / 0	0 / 0	445 / 0	0 / 0
L	1333	888 / 0	0 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

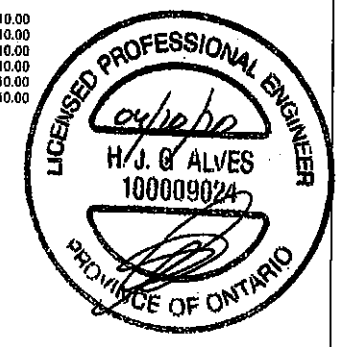
1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-O, E-O, C-R, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LOI	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
B-C	0 / 20	-91.8	-91.8	0.33 (1)	10.00	
C-D	-2435 / 0	-91.8	-91.8	0.37 (1)	4.10	
D-E	-2435 / 0	-91.8	-91.8	0.37 (1)	4.10	
E-F	-1813 / 0	-91.8	-91.8	0.34 (1)	4.64	
F-G	-1813 / 0	-91.8	-91.8	0.34 (1)	4.64	
G-H	-2435 / 0	-91.8	-91.8	0.37 (1)	4.10	
H-I	-2435 / 0	-91.8	-91.8	0.37 (1)	4.10	
I-J	0 / 20	-91.8	-91.8	0.33 (1)	10.00	
J-K	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
R-B	-325 / 0	0.0	0.0	0.03 (1)	7.81	
L-J	-325 / 0	0.0	0.0	0.03 (1)	7.81	
R-Q	0 / 2275	-18.5	-18.5	0.51 (1)	10.00	
Q-P	0 / 2057	-18.5	-18.5	0.48 (1)	10.00	
P-O	0 / 2057	-18.5	-18.5	0.48 (1)	10.00	
O-N	0 / 2057	-18.5	-18.5	0.48 (1)	10.00	
N-M	0 / 2057	-18.5	-18.5	0.48 (1)	10.00	
M-L	0 / 2275	-18.5	-18.5	0.51 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O-F	0 / 1214	0.27 (1)
O-G	-703 / 0	0.32 (1)
G-M	0 / 313	0.07 (1)
M-I	-157 / 21	0.05 (1)
E-O	-703 / 0	0.32 (1)
Q-E	0 / 313	0.07 (1)
C-Q	-157 / 21	0.05 (1)
R-C	-2726 / 0	0.55 (1)
I-L	-2726 / 0	0.55 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.37/1.00 (C-E:1) , BC=0.51/1.00 (Q-R:1) , WB=0.55/1.00 (C-R:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.70 (P) (INPUT = 1.00)

Structural component only
DWG# T-2006381

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	T7	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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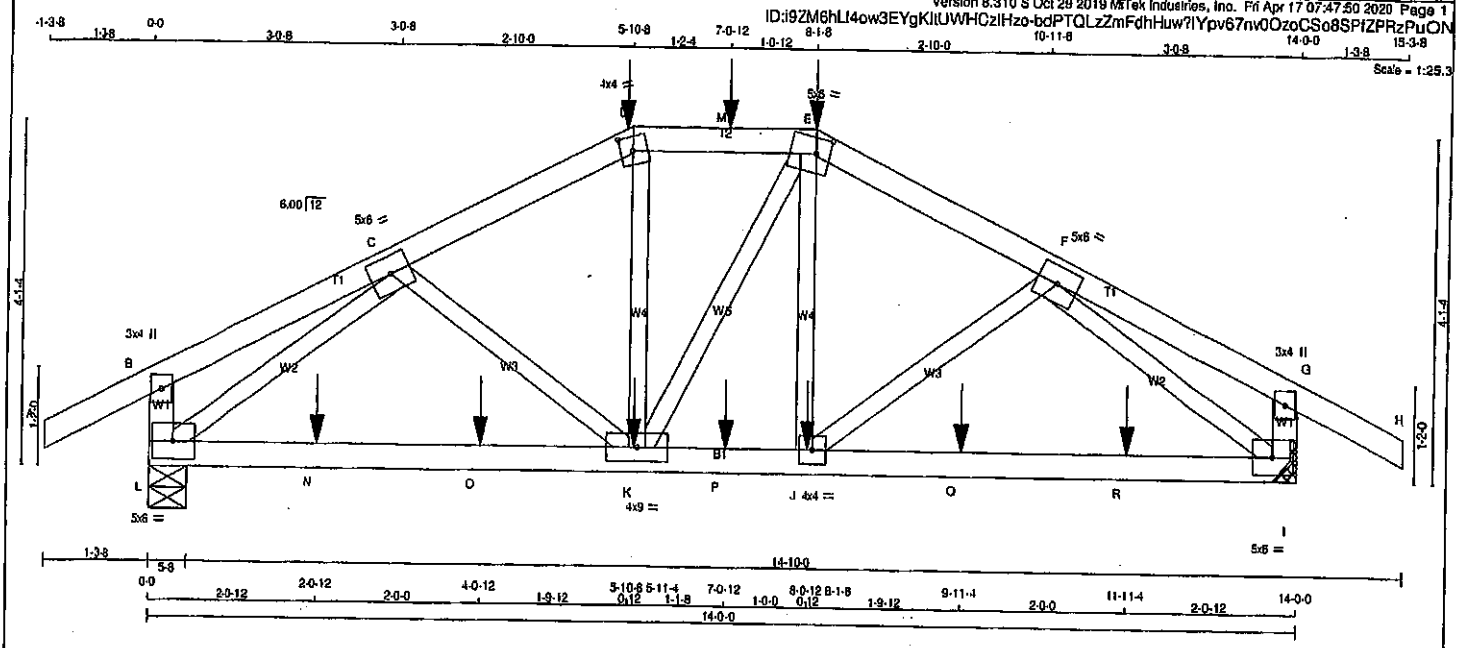

TOTAL WEIGHT = 59 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW-m	MT20	4.0	4.0	2.00	1.75
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMWW-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	5.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1502	0	1502	0	0	5-8	5-8
I	1503	0	1503	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1060	710 / 0	0 / 0	0 / 0	0 / 0	350 / 0	0 / 0
I	1060	711 / 0	0 / 0	0 / 0	0 / 0	350 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)
A-B	0 / 28	-91.8	-91.8	0.13 (1)
B-C	0 / 10	-91.8	-91.8	0.11 (1)
C-D	-1610 / 0	-91.8	-91.8	0.15 (1)
D-M	-1610 / 0	-91.8	-91.8	0.21 (1)
M-E	-1610 / 0	-91.8	-91.8	0.21 (1)
E-F	-1612 / 0	-91.8	-91.8	0.15 (1)
F-G	0 / 10	-91.8	-91.8	0.11 (1)
G-H	0 / 28	-91.8	-91.8	0.13 (1)
L-B	-237 / 0	0.0	0.0	0.03 (1)
I-G	-237 / 0	0.0	0.0	0.03 (1)
L-N	0 1513	-18.5	-18.5	0.42 (1)
N-O	0 / 1513	-18.5	-18.5	0.42 (1)
O-K	0 / 1513	-18.5	-18.5	0.42 (1)
K-P	0 / 1612	-18.5	-18.5	0.40 (1)
P-J	0 / 1612	-18.5	-18.5	0.40 (1)
J-Q	0 / 1515	-18.5	-18.5	0.42 (1)
Q-R	0 / 1515	-18.5	-18.5	0.42 (1)
R-I	0 / 1515	-18.5	-18.5	0.42 (1)

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	MAX. UNBRAC LENGTH
C-K	0 / 120	0.03 (4)	10.00
K-D	0 / 126	0.05 (4)	10.00
K-E	-3 / 0	0.00 (1)	4.81
J-E	0 / 126	0.05 (4)	4.94
J-F	0 / 120	0.03 (4)	4.94
L-C	-1929 / 0	0.50 (1)	4.81
F-I	-1931 / 0	0.50 (1)	4.81

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-459	-459	---	FRONT	VERT	TOTAL	---	C1
E	8-1-8	-459	-459	---	FRONT	VERT	TOTAL	---	C1
J	8-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
K	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
M	7-0-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
N	2-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	4-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	7-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Q	9-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
R	11-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.21/1.00 (D-E:1) , BC=0.42/1.00 (I-J:1) , WB=0.50/1.00 (F-I:1) , SSI=0.14/1.00 (K-L:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (D) (INPUT = 0.90)
JSI METAL= 0.49 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H.J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006382

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	T8	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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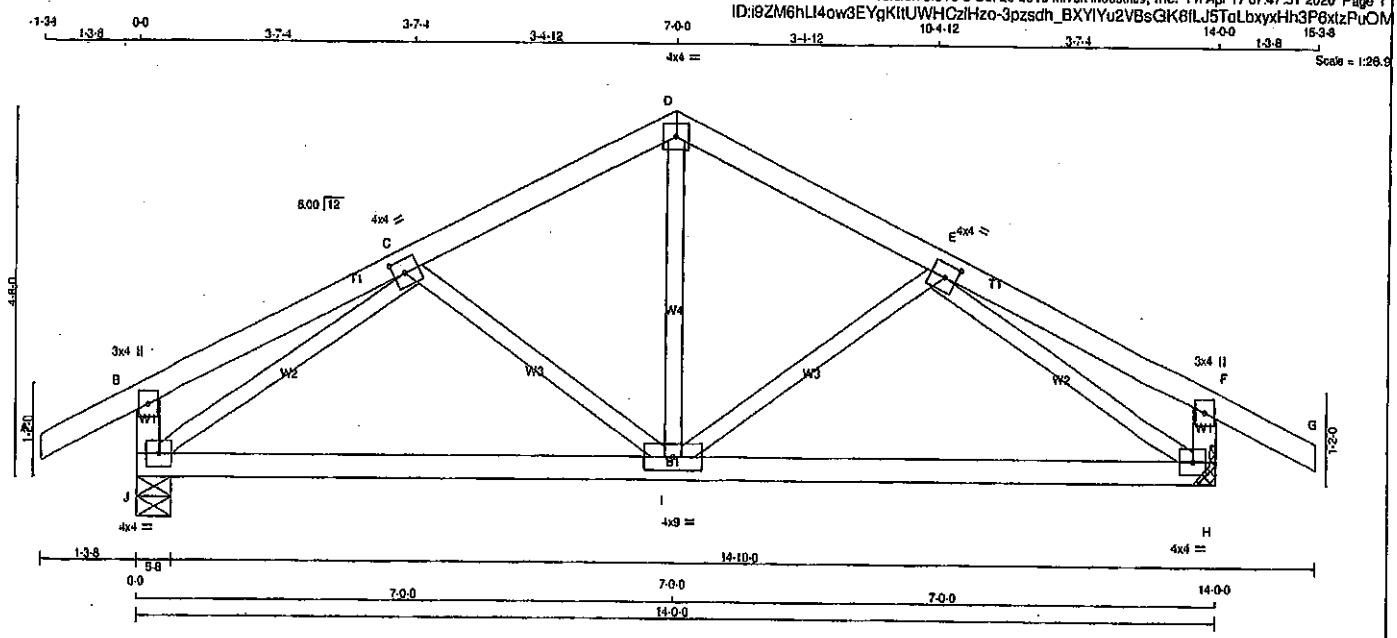


TOTAL WEIGHT = 58 lb (M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-p	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	896	0	896	0	0	5-8	5-8
H	896	0	896	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	831	428 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0
H	831	428 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

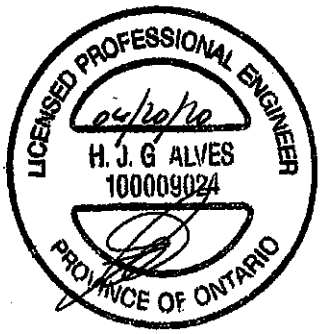
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	I-D	0 / 394	0.09 (1)	
B-C	0 / 16	-91.8	-91.8	0.17 (1)	10.00	I-E	-196 / 0	0.06 (1)	
C-D	-735 / 0	-91.8	-91.8	0.13 (1)	6.25	C-I	-196 / 0	0.06 (1)	
D-E	-735 / 0	-91.8	-91.8	0.13 (1)	6.25	J-C	-997 / 0	0.31 (1)	
E-F	0 / 16	-91.8	-91.8	0.17 (1)	10.00	E-H	-997 / 0	0.31 (1)	
F-G	0 / 28	-91.8	-91.8	0.12 (1)	10.00				
J-B	-251 / 0	0.0	0.0	0.03 (1)	7.81				
H-F	-251 / 0	0.0	0.0	0.03 (1)	7.81				
J-I	0 800	-18.5	-18.5	0.32 (4)	10.00				
I-H	0 800	-18.5	-18.5	0.32 (4)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.17/1.00 (E-F:1) , BC=0.32/1.00 (H-I:4) , WB=0.31/1.00 (E-H:1) , SSI=0.14/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.34 (E) (INPUT = 1.00)

Structural component only
DWG# T-2006383

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	T9	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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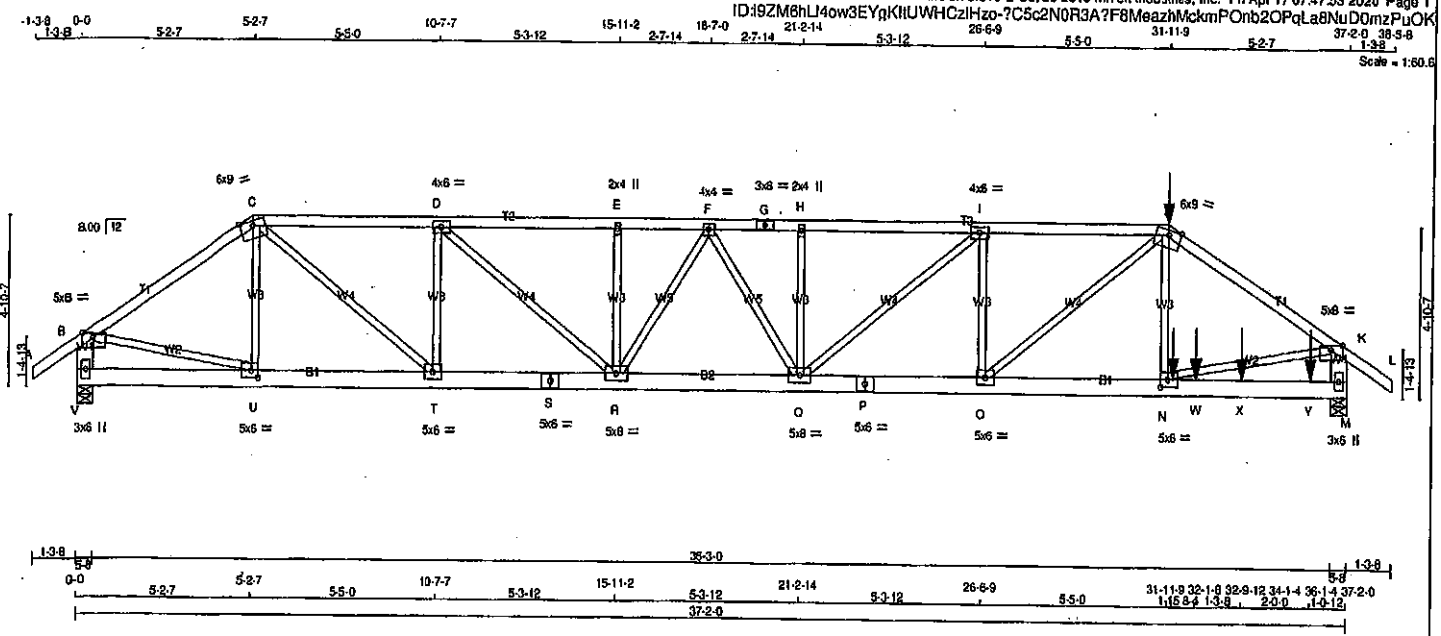

TOTAL WEIGHT = 2 X 176 = 352 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	1650F 1.5E	SPF
S - P	2x6	DRY	1650F 1.5E	SPF
P - M	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-G	1	12	TOP
G-J	1	12	SIDE(61.0)
J-L	1	12	SIDE(61.0)
V-B	2	12	TOP
M-K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V-S	2	12	TOP
S-P	2	12	TOP
P-M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
N-J	1	8	SIDE(132.7)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW-m	MT20	6.0	9.0	Edge	
K	TMW-p	MT20	5.0	8.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2385	0	2385	0	0	5-8	5-8
M	3466	0	3466	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	MAX./MIN. COMPONENT REACTIONS PERM.LIVE	WIND	DEAD	SOIL
V	1684	1122 / 0	0 / 0	0 / 0	0 / 0	561 / 0	0 / 0
M	2446	1631 / 0	0 / 0	0 / 0	0 / 0	815 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	U-C	-318 / 0	0.05 (1)
B-C	-2778 / 0	-91.8	-91.8	0.30 (1)	5.16	C-T	0 / 2222	0.27 (1)
C-D	-4013 / 0	-91.8	-91.8	0.32 (1)	4.46	T-D	-1323 / 0	0.22 (1)
D-E	-4910 / 0	-91.8	-91.8	0.36 (1)	4.06	D-R	0 / 1175	0.15 (1)
E-F	-4910 / 0	-91.8	-91.8	0.18 (1)	4.22	R-E	-373 / 0	0.06 (1)
F-G	-5158 / 0	-91.8	-91.8	0.19 (1)	4.13	Q-H	-373 / 0	0.06 (1)
G-H	-5158 / 0	-91.8	-91.8	0.19 (1)	4.13	Q-I	0 / 523	0.06 (1)
H-I	-5158 / 0	-91.8	-91.8	0.37 (1)	3.97	O-I	-898 / 0	0.15 (1)
I-J	-4759 / 0	-91.8	-91.8	0.35 (1)	4.13	O-J	0 / 1580	0.20 (1)
J-K	-4244 / 0	-91.8	-91.8	0.37 (1)	4.32	N-J	0 / 667	0.08 (1)
K-L	0 / 35	-91.8	-91.8	0.07 (1)	10.00	B-U	0 / 2349	0.29 (1)
V-B	-2344 / 0	0.0	0.0	0.08 (1)	7.81	N-K	0 / 3589	0.44 (1)
M-K	-3388 / 0	0.0	0.0	0.12 (1)	7.57	R-F	-342 / 0	0.08 (1)
						F-Q	0 / 147	0.02 (1)
V-U	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
U-T	0 / 2301	-18.5	-18.5	0.11 (1)	10.00			
T-S	0 / 4013	-18.5	-18.5	0.19 (1)	10.00			
S-R	0 / 4013	-18.5	-18.5	0.19 (1)	10.00			
R-Q	0 / 5084	-18.5	-18.5	0.23 (1)	10.00			
Q-P	0 / 4759	-18.5	-18.5	0.22 (1)	10.00			
P-O	0 / 4759	-18.5	-18.5	0.22 (1)	10.00			
O-N	0 / 3542	-18.5	-18.5	0.16 (1)	10.00			
N-W	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
W-X	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
X-Y	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
Y-M	0 / 0	-18.5	-18.5	0.03 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE	HEEL	CONN
J	31-11-9	-44	-49	---	FRONT	VERT	SNOW	---	C1
J	31-11-9	-195	-195	---	FRONT	VERT	TOTAL	---	C1
N	32-1-8	-1210	-1210	---	FRONT	VERT	TOTAL	---	C1
W	32-9-12	-17	-17	---	FRONT	VERT	TOTAL	---	C1
X	34-1-4	-17	-17	---	FRONT	VERT	TOTAL	---	C1
Y	36-1-4	-17	-17	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.37/1.00 (H-I:1) , BC=0.23/1.00 (Q-R:1) , WB=0.44/1.00 (K-N:1) , SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (U) (INPUT = 0.90)
JSI METAL= 0.44 (S) (INPUT = 1.00)

Structural component only
DWG# T-2006384

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
04/12/22
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408204	T9	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:i9ZM6hLi4ow3EYgKIIUWHCzIHzo-?C5c2N0R3A?F8MeazhMckmPOnb2OPgLa8NuD0mzPuOK

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
U	BMWW+	MT20	5.0	6.0	2.50	2.50
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/17/20
H. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006384

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	T9Z	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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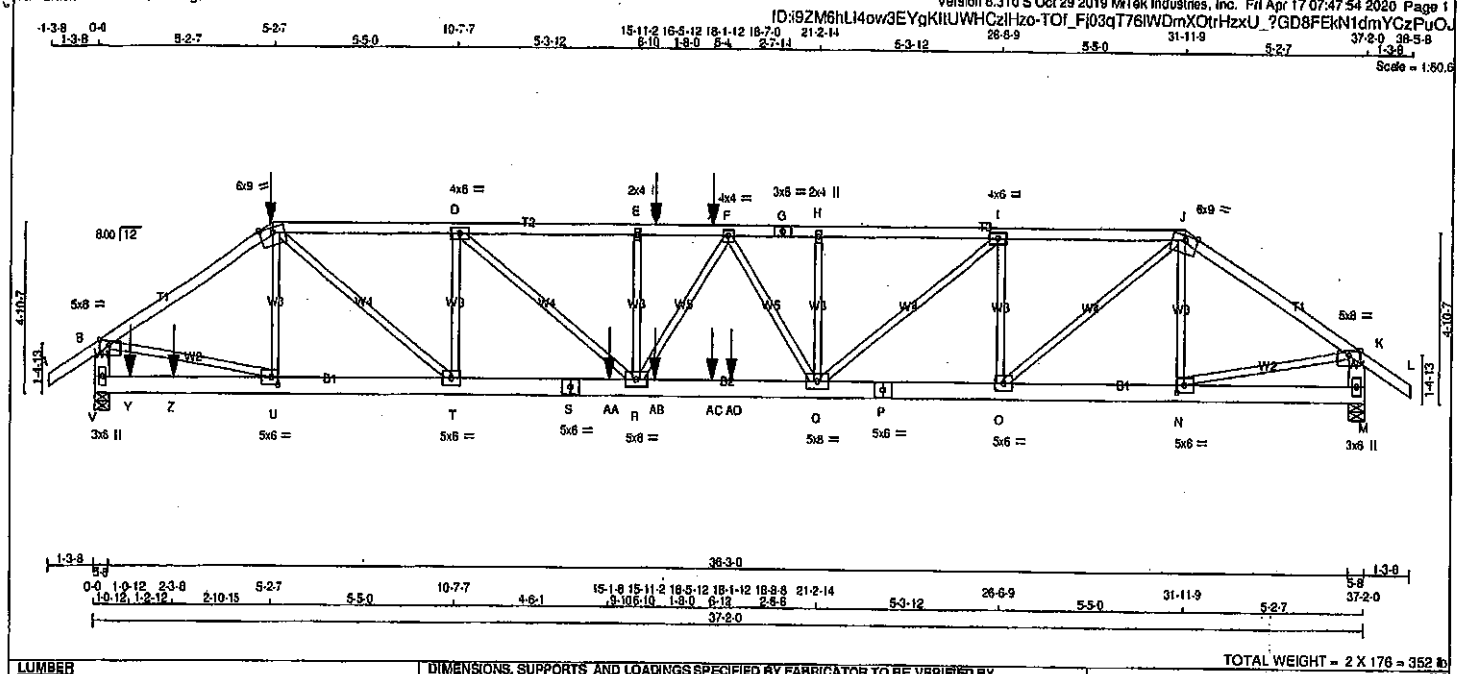
ID:i9ZM6hLI4ow3EYgKltUWHCzIHzo-TOf_Fj03qT76IWDmXOtrHzxU_?GD8FEkN1dmYCzPuOJ



TOTAL WEIGHT = 2 X 176 = 352 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	1650F 1.5E	SPF
S - P	2x6	DRY	1650F 1.5E	SPF
P - M	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-G	1	12	SIDE(61.0)
G-J	1	12	TOP
J-L	1	12	TOP
V-B	2	12	TOP
M-K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V-S	2	12	SIDE(0.0)
S-P	2	12	SIDE(0.0)
P-M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	4.0	8.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TMWW-t	MT20	4.0	8.0		
J	TTWW-m	MT20	6.0	9.0	Edge	
K	TMVW-p	MT20	5.0	8.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMWW-t	MT20	5.0	6.0	2.50	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
V	4478	0	4478	0	0	5-8	5-8
M	3310	0	3310	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	3155	2137 / 0	0 / 0	0 / 0	0 / 0	1017 / 0	0 / 0
M	2334	1572 / 0	0 / 0	0 / 0	0 / 0	762 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.07 (1)	10.00
B-C	-5076 / 0	-91.8	-91.8	0.42 (1)	3.96
C-D	-6908 / 0	-91.8	-91.8	0.50 (1)	3.40
D-E	-8902 / 0	-91.8	-91.8	0.66 (1)	2.88
E-W	-8902 / 0	-91.8	-91.8	0.37 (1)	3.06
W-X	-8902 / 0	-91.8	-91.8	0.37 (1)	3.06
X-F	-8902 / 0	-91.8	-91.8	0.37 (1)	3.06
F-G	-8262 / 0	-91.8	-91.8	0.33 (1)	3.22
G-H	-8262 / 0	-91.8	-91.8	0.33 (1)	3.22
H-I	-8262 / 0	-91.8	-91.8	0.61 (1)	3.03
I-J	-6172 / 0	-91.8	-91.8	0.44 (1)	3.62
J-K	-4084 / 0	-91.8	-91.8	0.36 (1)	4.39
K-L	0 / 35	-91.8	-91.8	0.07 (1)	10.00
V-B	-3981 / 0	0.0	0.0	0.14 (1)	7.12
M-K	-3274 / 0	0.0	0.0	0.12 (1)	7.67
V-Y	0 / 0	-18.5	-18.5	0.27 (1)	10.00
Y-Z	0 / 0	-18.5	-18.5	0.27 (1)	10.00
Z-U	0 / 0	-18.5	-18.5	0.27 (1)	10.00
U-T	0 / 4216	-18.5	-18.5	0.28 (1)	10.00
T-S	0 / 6908	-18.5	-18.5	0.44 (1)	10.00
S-AA	0 / 6908	-18.5	-18.5	0.44 (1)	10.00
AA-R	0 / 6908	-18.5	-18.5	0.44 (1)	10.00
R-AB	0 / 8669	-18.5	-18.5	0.64 (1)	10.00
AB-AC	0 / 8669	-18.5	-18.5	0.64 (1)	10.00
AC-AD	0 / 8669	-18.5	-18.5	0.64 (1)	10.00
AD-Q	0 / 8669	-18.5	-18.5	0.64 (1)	10.00
Q-P	0 / 6172	-18.5	-18.5	0.33 (1)	10.00
P-O	0 / 6172	-18.5	-18.5	0.33 (1)	10.00
O-N	0 / 3382	-18.5	-18.5	0.17 (1)	10.00
N-M	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U-C	-98 / 55	0.02 (1)
C-T	0 / 3484	0.43 (1)
T-D	-2245 / 0	0.38 (1)
D-R	0 / 2613	0.32 (1)
R-E	-434 / 0	0.07 (1)
Q-H	-348 / 0	0.06 (1)
Q-I	0 / 2738	0.34 (1)
O-I	-2330 / 0	0.39 (1)
O-J	0 / 3621	0.45 (1)
N-J	-490 / 0	0.08 (1)
B-U	0 / 4292	0.53 (1)
N-K	0 / 3453	0.43 (1)
R-F	0 / 458	0.06 (1)
F-Q	-801 / [illegible]	[illegible]

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	TREE	CONN.
C	5-2-7	-44	-49	---	FRONT	VERT	DEAD	---	C1
C	5-2-7	-195	-195	---	FRONT	VERT	SNOW	---	C1
W	16-5-12	-86	-86	---	BACK	VERT	TOTAL	---	C1
X	18-1-12	-86	-86	---	BACK	VERT	TOTAL	---	C1
Y	1-0-12	-17	-17	---	BACK	VERT	TOTAL	---	C1
Z	2-3-8	-915	-915	---	BACK	VERT	TOTAL	---	C1
AA	15-1-8	-915	-915	---	BACK	VERT	TOTAL	---	C1
AB	16-5-12	-17	-17	---	BACK	VERT	TOTAL	---	C1
AC	18-1-12	-17	-17	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.27")
ALLOWABLE DEFL.(TL)= L/360 (1.24")
CALCULATED VERT. DEFL.(TL) = L/ 896 (0.50")

CSI: TC=0.66/1.00 (D-E:1) , BC=0.64/1.00 (Q-R:1) , WB=0.53/1.00 (B-U:1) , SSI=0.32/1.00 (R-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.67 (S) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H.J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006385

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408204	T9Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
V	BMV1+p	MT20	3.0	6.0		

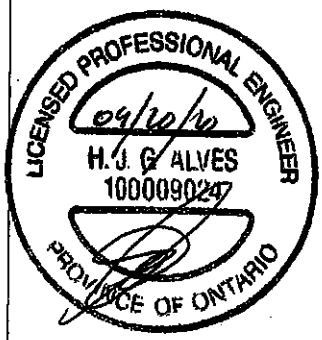
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AD	18-8-8	-1145	-1145	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2006385

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	T10	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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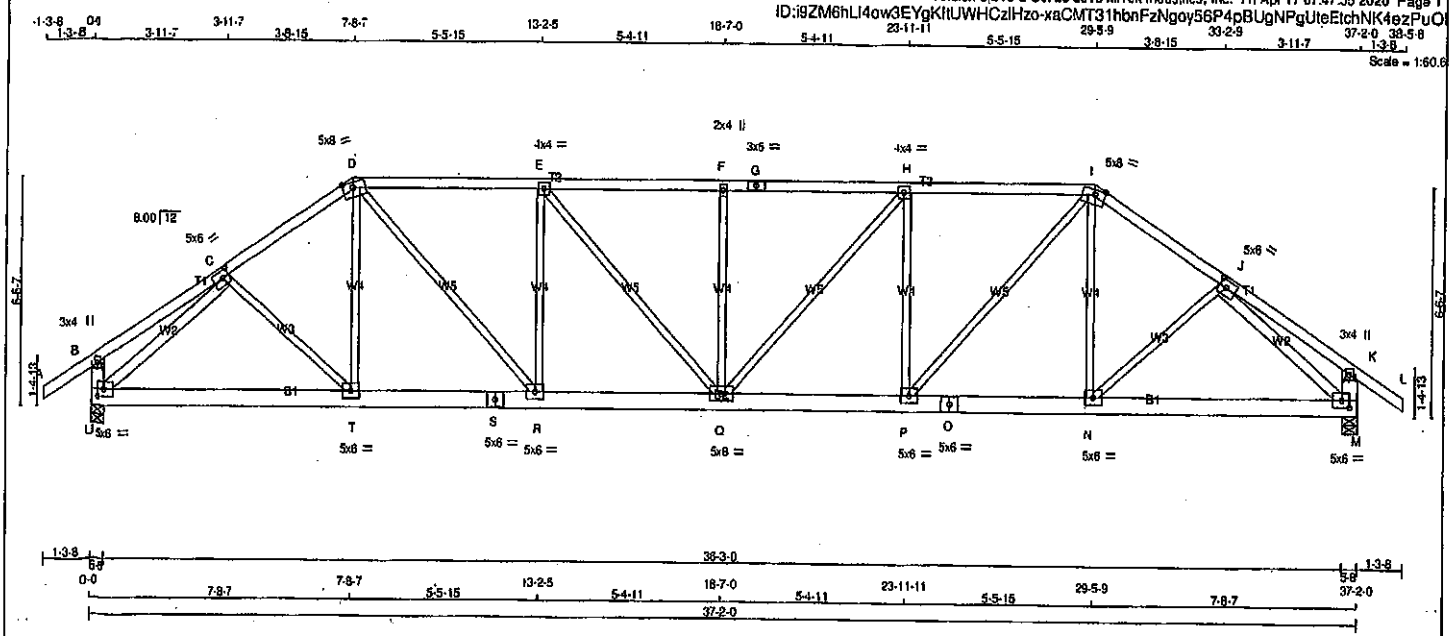


TOTAL WEIGHT = 2 X 187 = 375 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW-m	MT20	5.0	8.0	1.75	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	8.0	1.75	3.50
J	TMWW-t	MT20	5.0	6.0	2.50	2.50
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	2.50	2.75
N, P, R, T						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	6.0		
S	BS-t	MT20	5.0	6.0		
U	BMVW1-t	MT20	5.0	6.0	2.50	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2175	0	2175	0	0	5-8	5-8
M	2175	0	2175	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	MAX./MIN. COMPONENT REACTIONS PERM.LIVE	WIND	DEAD	SOIL
U	1538	1021 / 0	0 / 0	0 / 0	0 0	515 0	0 / 0
M	1536	1021 / 0	0 / 0	0 / 0	0 0	515 0	0 0

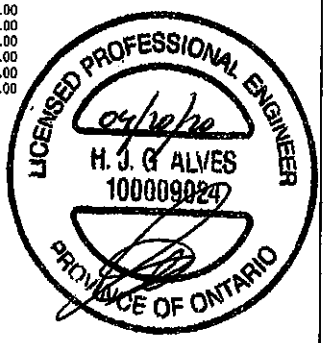
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	C- T	0 / 77	0.03 (4)
B- C	0 / 21	-91.8	-91.8	0.20 (1)	10.00	T- D	0 / 115	0.04 (4)
C- D	-2487 / 0	-91.8	-91.8	0.30 (1)	4.10	N- I	0 / 115	0.04 (4)
D- E	-2877 / 0	-91.8	-91.8	0.60 (1)	3.55	N- J	0 / 77	0.03 (4)
E- F	-3115 / 0	-91.8	-91.8	0.62 (1)	3.39	U- C	-2722 / 0	0.80 (1)
F- G	-3115 / 0	-91.8	-91.8	0.62 (1)	3.39	J- M	-2722 / 0	0.80 (1)
G- H	-3115 / 0	-91.8	-91.8	0.62 (1)	3.39	P- I	0 / 1242	0.28 (1)
H- I	-2877 / 0	-91.8	-91.8	0.60 (1)	3.55	D- R	0 / 1242	0.28 (1)
I- J	-2487 / 0	-91.8	-91.8	0.30 (1)	4.10	P- H	-835 / 0	0.55 (1)
J- K	0 / 21	-91.8	-91.8	0.20 (1)	10.00	R- E	-835 / 0	0.55 (1)
K- L	0 / 35	-91.8	-91.8	0.12 (1)	10.00	Q- H	0 / 361	0.08 (1)
U- B	-264 / 0	0.0	0.0	0.02 (1)	7.81	E- Q	0 / 361	0.08 (1)
M- K	-264 / 0	0.0	0.0	0.02 (1)	7.81	Q- F	-454 / 0	0.30 (1)
U- T	0 / 1997	-18.5	-18.5	0.30 (1)	10.00			
T- S	0 / 2050	-18.5	-18.5	0.30 (1)	10.00			
S- R	0 / 2050	-18.5	-18.5	0.30 (1)	10.00			
R- Q	0 / 2878	-18.5	-18.5	0.39 (1)	10.00			
Q- P	0 / 2878	-18.5	-18.5	0.39 (1)	10.00			
P- O	0 / 2050	-18.5	-18.5	0.30 (1)	10.00			
O- N	0 / 2050	-18.5	-18.5	0.30 (1)	10.00			
N- M	0 / 1997	-18.5	-18.5	0.30 (1)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.62/1.00 (E-F;1) , BC=0.39/1.00 (Q-R;1) , WB=0.80/1.00 (J-M;1) , SSI=0.24/1.00 (D-E;1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (M) (INPUT = 0.90)
JSI METAL= 0.68 (C) (INPUT = 1.00)

Structural component only
DWG# T-2006386

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	T11	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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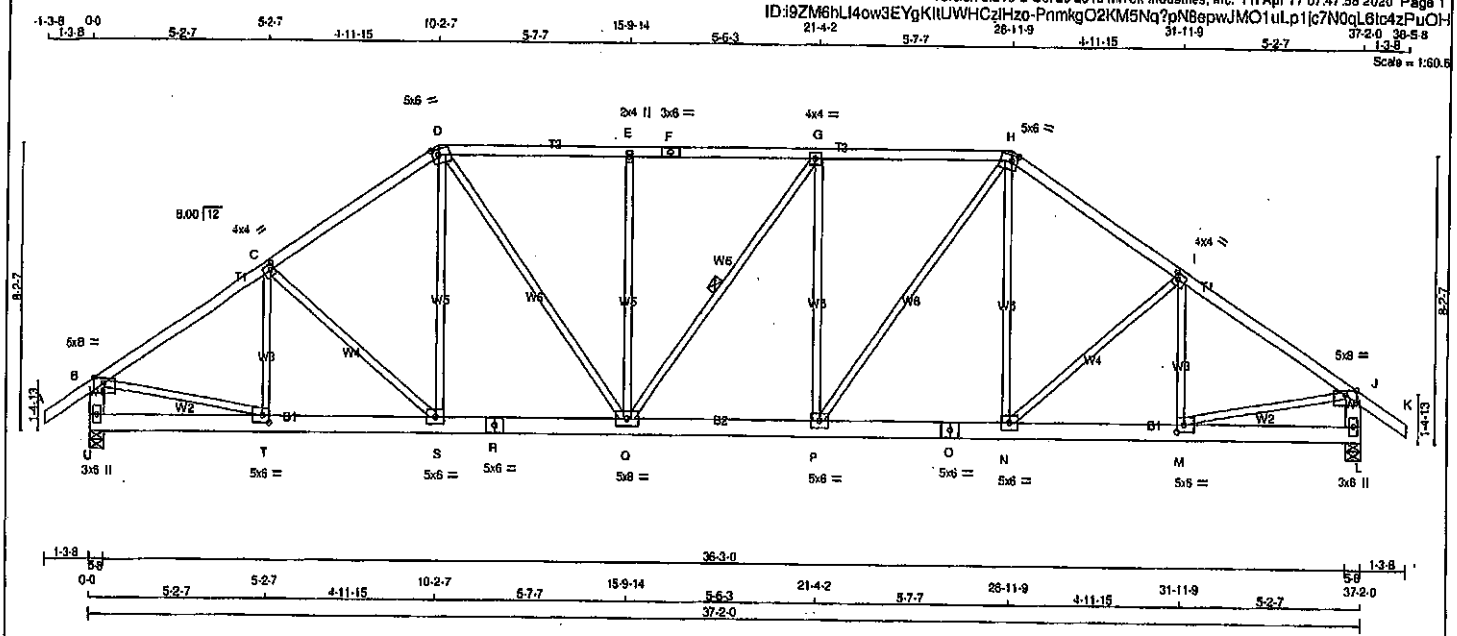


TOTAL WEIGHT = 2 X 192 = 385 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.00	2.00
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.00	2.00
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N, P, S						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	8.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.50
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
U	2175	0	2175	0	0	5-8	5-8
L	2175	0	2175	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1536	1021 / 0	0 / 0	0 / 0	0 / 0	515 0	0 0
L	1536	1021 / 0	0 / 0	0 / 0	0 / 0	515 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

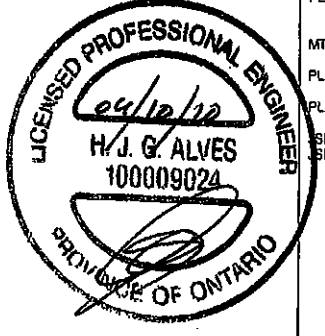
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-2505 / 0	-91.8	-91.8	0.41 (1)	4.00
C- D	-2373 / 0	-91.8	-91.8	0.39 (1)	4.11
D- E	-2413 / 0	-91.8	-91.8	0.42 (1)	4.04
E- F	-2413 / 0	-91.8	-91.8	0.39 (1)	4.08
F- G	-2413 / 0	-91.8	-91.8	0.39 (1)	4.08
G- H	-2418 / 0	-91.8	-91.8	0.42 (1)	4.03
H- I	-2372 / 0	-91.8	-91.8	0.39 (1)	4.11
I- J	-2506 / 0	-91.8	-91.8	0.41 (1)	4.00
J- K	0 / 35	-91.8	-91.8	0.12 (1)	10.00
U- B	-2121 / 0	0.0	0.0	0.14 (1)	6.98
L- J	-2121 / 0	0.0	0.0	0.14 (1)	6.98
U- T	0 / 0	-18.5	-18.5	0.06 (4)	10.00
T- S	0 / 2109	-18.5	-18.5	0.29 (1)	10.00
S- R	0 / 1950	-18.5	-18.5	0.27 (1)	10.00
R- Q	0 / 1950	-18.5	-18.5	0.27 (1)	10.00
Q- P	0 / 2418	-18.5	-18.5	0.32 (1)	10.00
P- O	0 / 1949	-18.5	-18.5	0.27 (1)	10.00
O- N	0 / 1949	-18.5	-18.5	0.27 (1)	10.00
N- M	0 / 2110	-18.5	-18.5	0.29 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.06 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
T- C	-332 / 0	0.11 (1)
C- S	-217 / 0	0.18 (1)
S- D	0 / 270	0.08 (1)
D- Q	0 794	0.18 (1)
Q- E	-552 / 0	0.68 (1)
Q- G	-8 / 0	0.01 (1)
P- G	-549 0	0.68 (1)
P- H	0 803	0.18 (1)
N- H	0 263	0.08 (1)
N- I	-219 0	0.18 (1)
M- I	-330 0	0.11 (1)
B- T	0 2148	0.48 (1)
M- J	0 2148	0.48 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.24")
CALCULATED VERT. DEFL.(LL) = U 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.24")
CALCULATED VERT. DEFL.(TL) = U 999 (0.22")

CSI: TC=0.42/1.00 (G-H:1) , BC=0.32/1.00 (P-Q:1) , WB=0.68/1.00 (E-Q:1) , SSI=0.24/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.85 (H) (INPUT = 0.90)
SI METAL= 0.53 (M) (INPUT = 1.00)

Structural component only
DWG# T-2006387

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	T12	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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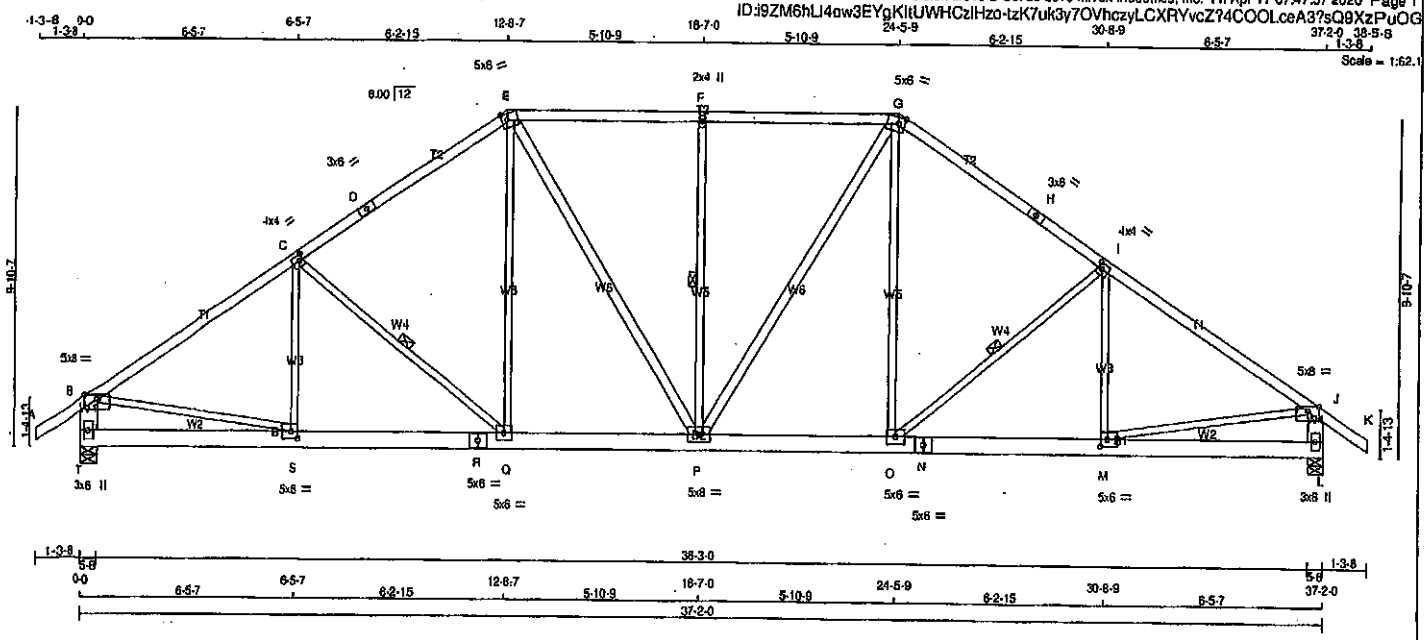


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x8	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMWW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N	BS-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.50
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2175	0	2175	0	0	5-8	5-8
L	2175	0	2175	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1538	1021 / 0	0 / 0	0 / 0	0 / 0	515 / 0	0 0
L	1536	1021 / 0	0 / 0	0 / 0	0 / 0	515 / 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-Q, F-P, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW.

LOADING

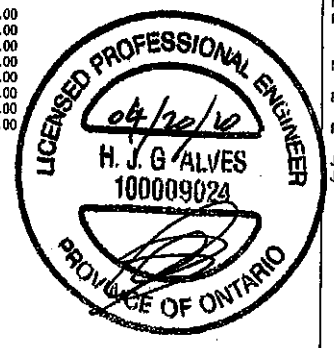
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B-C	-2544 / 0	-91.8	-91.8	0.67 (1)	3.68
C-D	-2217 / 0	-91.8	-91.8	0.61 (1)	3.96
D-E	-2217 / 0	-91.8	-91.8	0.61 (1)	3.96
E-F	-2048 / 0	-91.8	-91.8	0.47 (1)	4.24
F-G	-2048 / 0	-91.8	-91.8	0.47 (1)	4.24
G-H	-2217 / 0	-91.8	-91.8	0.61 (1)	3.96
H-I	-2217 / 0	-91.8	-91.8	0.61 (1)	3.96
I-J	-2544 0	-91.8	-91.8	0.67 (1)	3.68
J-K	0 35	-91.8	-91.8	0.12 (1)	10.00
T-B	-2117 0	0.0	0.0	0.14 (1)	6.99
L-J	-2117 0	0.0	0.0	0.14 (1)	6.99
T-S	0 / 0	-18.5	-18.5	0.08 (4)	10.00
S-R	0 / 2149	-18.5	-18.5	0.29 (1)	10.00
R-Q	0 / 2149	-18.5	-18.5	0.29 (1)	10.00
Q-P	0 / 1814	-18.5	-18.5	0.25 (1)	10.00
P-O	0 / 1814	-18.5	-18.5	0.25 (1)	10.00
O-N	0 / 2149	-18.5	-18.5	0.29 (1)	10.00
N-M	0 / 2149	-18.5	-18.5	0.29 (1)	10.00
M-L	0 0	-18.5	-18.5	0.08 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S-C	-219 / 32	0.10 (1)
C-Q	-442 / 0	0.20 (1)
Q-E	0 / 408	0.09 (1)
E-P	0 / 445	0.07 (1)
P-F	-660 / 0	0.42 (1)
P-G	0 / 445	0.07 (1)
Q-G	0 / 408	0.09 (1)
O-I	-442 / 0	0.20 (1)
M-I	-219 / 32	0.10 (1)
B-S	0 2174	0.49 (1)
M-J	0 2174	0.49 (1)



TOTAL WEIGHT = 2 X 199 = 399 lb (M)[F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.67/1.00 (I-J:1) , BC=0.29/1.00 (M-O:1) , WB=0.49/1.00 (J-M:1) , SSI=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (M) (INPUT = 0.90)
JSI METAL= 0.54 (M) (INPUT = 1.00)

Structural component only
DWG# T-2006388

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	T13	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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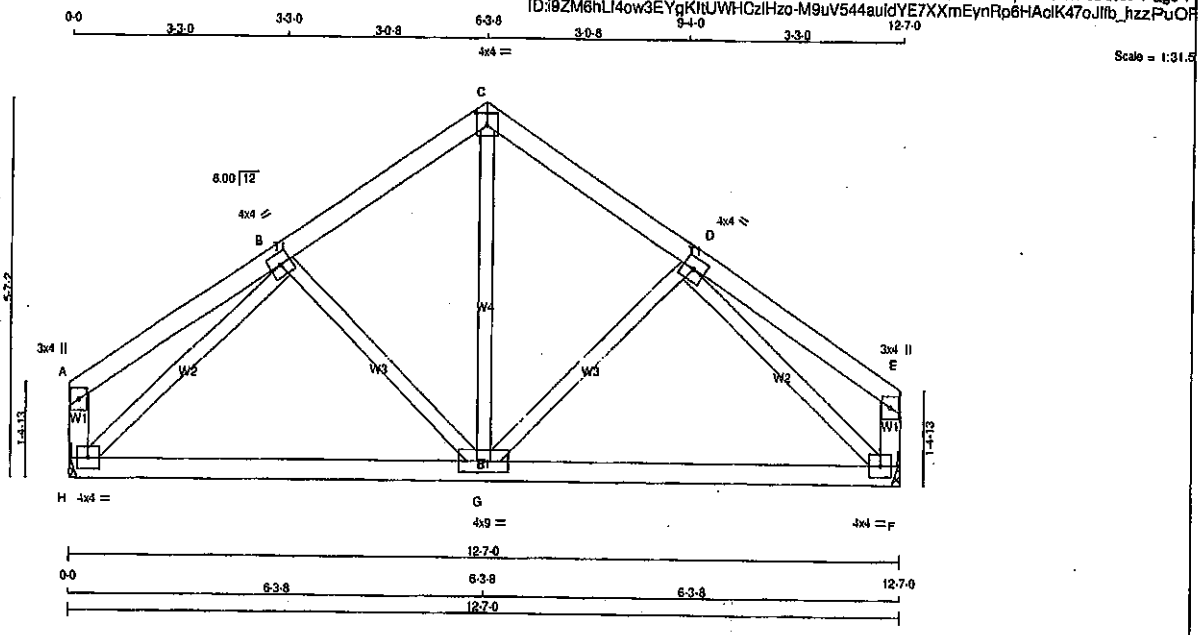


TOTAL WEIGHT = 52 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	694	0	694	0	0	MECHANICAL	
F	694	0	694	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, F. MINIMUM BEARING LENGTH AT JOINT H = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	491	322 / 0	0 / 0	0 / 0	0 : 0	169 : 0	0 0
F	491	322 : 0	0 / 0	0 : 0	0 . 0	169 : 0	0 . 0

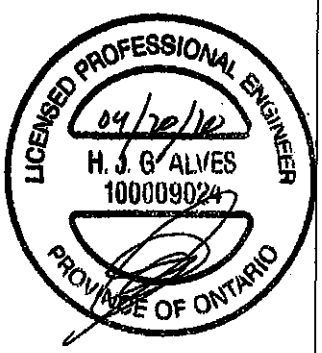
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 18	-91.8	-91.8	0.14 (1)	10.00	
B- C	-534 / 0	-91.8	-91.8	0.11 (1)	6.25	
C- D	-534 / 0	-91.8	-91.8	0.11 (1)	6.25	
D- E	0 / 18	-91.8	-91.8	0.14 (1)	10.00	
H- A	-112 / 0	0.0	0.0	0.01 (1)	7.81	
F- E	-112 / 0	0.0	0.0	0.01 (1)	7.81	
H- G	0 : 531	-18.5	-18.5	0.25 (4)	10.00	
G- F	0 : 531	-18.5	-18.5	0.25 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 347	0.08 (1)
G- D	-144 / 0	0.05 (1)
B- G	-144 / 0	0.05 (1)
H- B	-755 / 0	0.24 (1)
D- F	-755 / 0	0.24 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.25/1.00 (F-G:4) , WB=0.24/1.00 (B-H:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1887	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (D) (INPUT = 0.90)
JSI METAL= 0.27 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006389

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	T13S	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Trusses, Burlington

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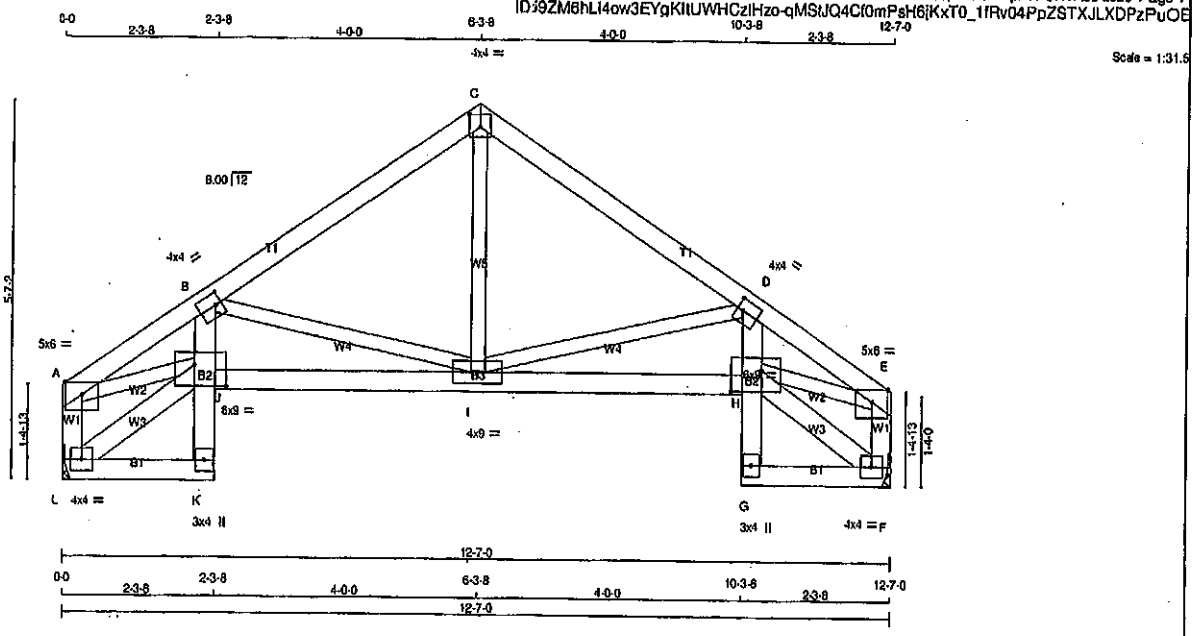


TOTAL WEIGHT = 2 X 57 = 114 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (tabloids in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	TMVW-p	MT20	5.0	6.0	Edge	
F	BMVW1-t	MT20	4.0	4.0		
G	BMV+p	MT20	3.0	4.0		
H	BVMWW-t	MT20	6.0	9.0	3.75	5.50
I	BMWWW-t	MT20	4.0	9.0		
J	BVMWW-t	MT20	6.0	9.0	3.75	5.50
K	BMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	694	0	894	0	0	MECHANICAL	
F	694	0	894	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L, F. MINIMUM BEARING LENGTH AT JOINT L = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	491	322 / 0	0 / 0	0 / 0	0 / 0	169 / 0	0 / 0
F	491	322 / 0	0 / 0	0 / 0	0 / 0	169 / 0	0 / 0

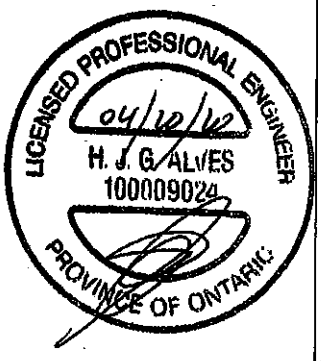
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A-B	-1434 / 0	-91.8	-91.8	0.12 (1)	5.34
B-C	-891 / 0	-91.8	-91.8	0.20 (1)	6.25
C-D	-891 / 0	-91.8	-91.8	0.20 (1)	6.25
D-E	-1434 / 0	-91.8	-91.8	0.12 (1)	5.34
L-A	-641 / 0	0.0	0.0	0.07 (1)	7.81
F-E	-641 / 0	0.0	0.0	0.07 (1)	7.81
L-K	0 / 48	-18.5	-18.5	0.03 (4)	10.00
K-J	0 / 22	0.0	0.0	0.10 (1)	10.00
J-B	0 / 280	0.0	0.0	0.14 (1)	10.00
J-I	0 / 1282	-18.5	-18.5	0.28 (1)	10.00
I-H	0 1283	-18.5	-18.5	0.28 (1)	10.00
G-H	0 - 22	0.0	0.0	0.10 (1)	10.00
H-D	0 - 280	0.0	0.0	0.14 (1)	10.00
G-F	0 - 48	-18.5	-18.5	0.03 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-C	0 / 418	0.09 (1)
I-D	-747 / 0	0.23 (1)
B-I	-747 / 0	0.23 (1)
L-J	-58 / 0	0.01 (1)
A-J	0 / 1235	0.28 (1)
H-F	-58 / 0	0.01 (1)
H-E	0 / 1235	0.28 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.20/1.00 (C-D:1) , BC=0.28/1.00 (H-I:1) , WB=0.28/1.00 (E-H:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (B) (INPUT = 0.90)
JSI METAL= 0.28 (E) (INPUT = 1.00)

Structural component only
DWG# T-2006390

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	T14	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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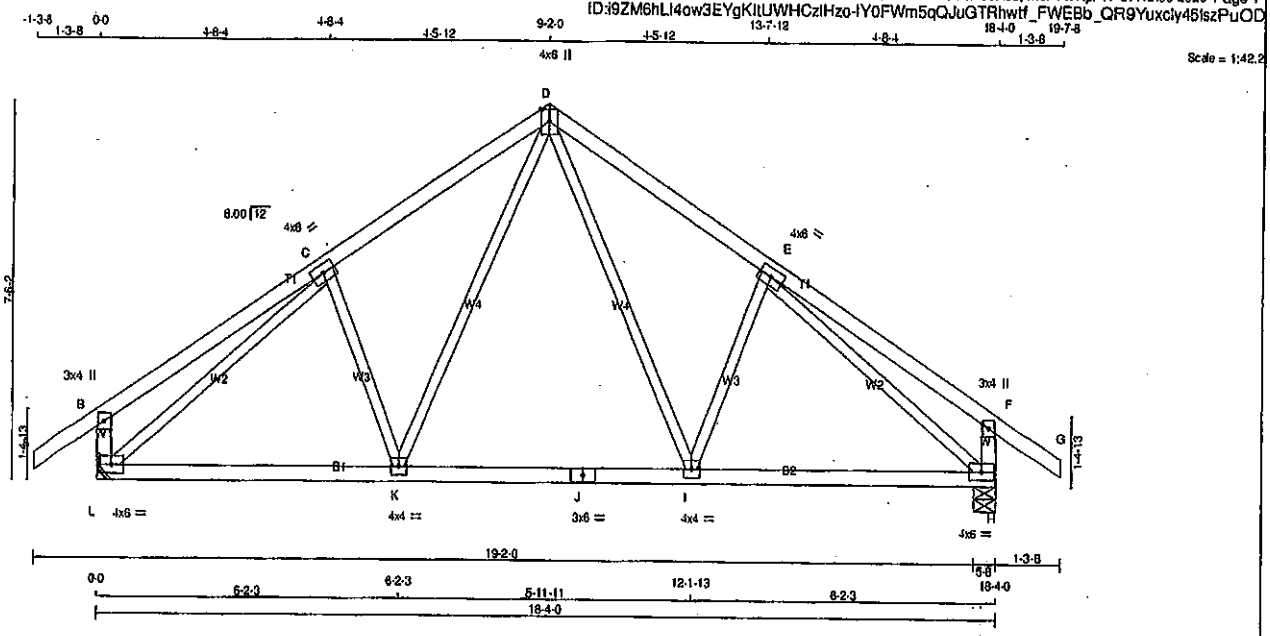

TOTAL WEIGHT = 2 X 81 = 161 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	RECMD BRG
L	1137	0	1137	0	0	MECHANICAL	
H	1137	0	1137	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	801	539 / 0	0 / 0	0 / 0	0 : 0	262 : 0	0 : 0
H	801	539 / 0	0 / 0	0 / 0	0 : 0	262 : 0	0 : 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B-C	0 / 27	-91.8	-91.8	0.31 (1)	10.00
C-D	-981 / 0	-91.8	-91.8	0.25 (1)	5.99
D-E	-981 / 0	-91.8	-91.8	0.25 (1)	5.99
E-F	0 / 27	-91.8	-91.8	0.31 (1)	10.00
F-G	0 / 35	-91.8	-91.8	0.12 (1)	10.00
L-B	-286 / 0	0.0	0.0	0.03 (1)	7.81
H-F	-286 / 0	0.0	0.0	0.03 (1)	7.81
L-K	0 889	-18.5	-18.5	0.23 (4)	10.00
K-J	0 641	-18.5	-18.5	0.21 (4)	10.00
J-I	0 641	-18.5	-18.5	0.21 (4)	10.00
I-H	0 889	-18.5	-18.5	0.23 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D-I	0 : 397	0.09 (1)
I-E	-262 / 0	0.09 (1)
K-D	0 : 397	0.09 (1)
C-K	-262 / 0	0.09 (1)
L-C	-1199 / 0	0.77 (1)
E-H	-1199 / 0	0.77 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.23/1.00 (K-L:4) , WB=0.77/1.00 (C-L:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.31 (E) (INPUT = 1.00)

Structural component only
DWG# T-2006391

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	T14G	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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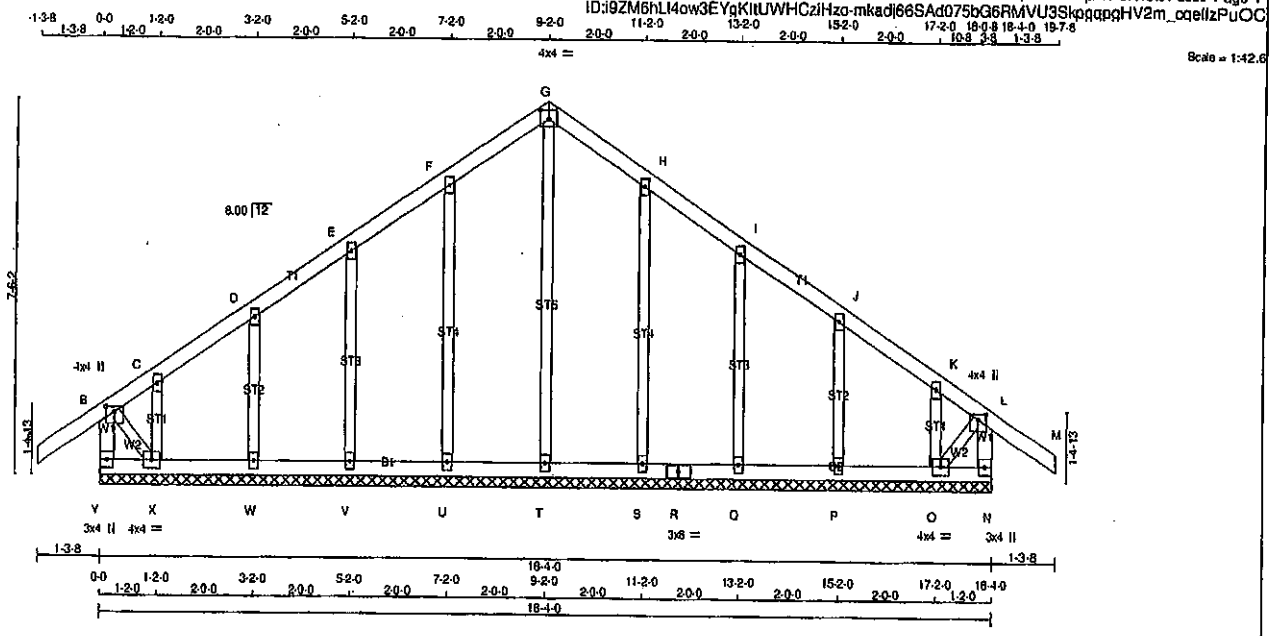


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
Y - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - R	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW-p	MT20	4.0	4.0	2.25	2.00
L	TMVW+p	MT20	4.0	4.0	1.25	2.00
N	BMV1+p	MT20	3.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P, Q, S, T, U, V, W						
P	BMW1+w	MT20	2.0	4.0		
R	BS-t	MT20	3.0	6.0		
X	BMWW1-t	MT20	4.0	4.0		
Y	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

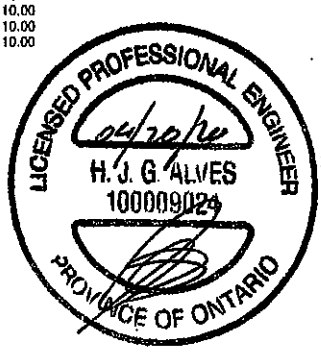
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.12	(1)		10.00
B-C	-64 / 0	-91.8	-91.8	0.12	(1)		6.25
C-D	-16 / 0	-91.8	-91.8	0.05	(1)		6.25
D-E	-16 / 0	-91.8	-91.8	0.05	(1)		6.25
E-F	-10 / 0	-91.8	-91.8	0.05	(1)		10.00
F-G	-20 / 0	-91.8	-91.8	0.05	(1)		6.25
G-H	-20 / 0	-91.8	-91.8	0.05	(1)		6.25
H-I	-10 / 0	-91.8	-91.8	0.05	(1)		10.00
I-J	-16 / 0	-91.8	-91.8	0.05	(1)		6.25
J-K	-16 / 0	-91.8	-91.8	0.05	(1)		6.25
K-L	-64 / 0	-91.8	-91.8	0.12	(1)		6.25
L-M	0 / 35	-91.8	-91.8	0.12	(1)		10.00
Y-B	-280 / 0	0.0	0.0	0.03	(1)		7.81
N-L	-280 / 0	0.0	0.0	0.03	(1)		7.81
Y-X	0 / 0	-18.5	-18.5	0.01	(4)		10.00
X-W	0 / 19	-18.5	-18.5	0.02	(4)		10.00
W-V	0 / 14	-18.5	-18.5	0.02	(4)		10.00
V-U	0 / 11	-18.5	-18.5	0.02	(4)		10.00
U-T	0 / 8	-18.5	-18.5	0.01	(4)		10.00
T-S	0 / 8	-18.5	-18.5	0.01	(4)		10.00
S-R	0 / 11	-18.5	-18.5	0.02	(4)		10.00
R-Q	0 / 11	-18.5	-18.5	0.02	(4)		10.00
Q-P	0 / 14	-18.5	-18.5	0.02	(4)		10.00
P-O	0 / 19	-18.5	-18.5	0.02	(4)		10.00
O-N	0 / 0	-18.5	-18.5	0.01	(4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
T-G	-143 / 0	0.14	(1)
U-F	-209 / 0	0.13	(1)
V-E	-173 / 0	0.08	(1)
W-D	-198 / 0	0.04	(1)
X-C	-56 / 0	0.01	(1)
S-H	-209 / 0	0.13	(1)
Q-I	-173 / 0	0.08	(1)
P-J	-198 / 0	0.04	(1)
O-K	-56 / 0	0.01	(1)
B-X	0 / 29	0.01	(1)
O-L	0 / 29	0.01	(1)



TOTAL WEIGHT = 83 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (W-X:4) , WB=0.14/1.00 (G-T:1) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.48 (O) (INPUT = 0.90)
JSI METAL= 0.11 (H) (INPUT = 1.00)

Structural component only
DWG# T-2006392

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408204	T15	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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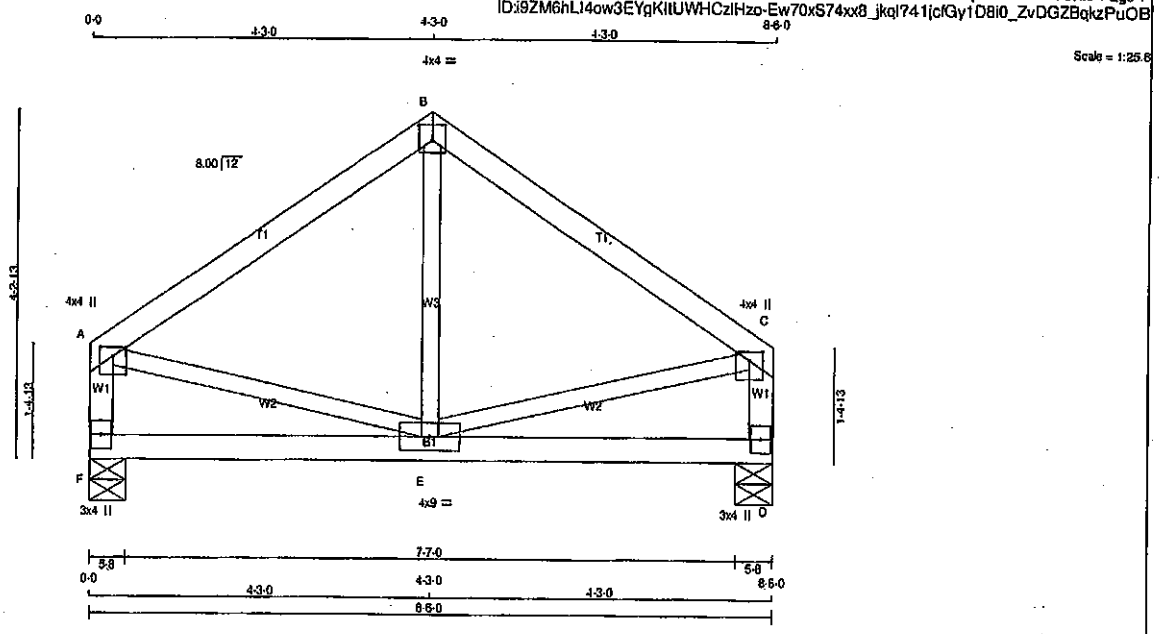

TOTAL WEIGHT = 33 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTW-p	MT20	4.0	4.0	2.25	2.00
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	469	0	469	0	0	IN-SX	IN-SX
D	469	0	469	0	0	5-8	5-8
						5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	331	217 / 0	0 / 0	0 / 0	0 / 0	114 / 0	0 0
D	331	217 / 0	0 / 0	0 / 0	0 / 0	114 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	-317 / 0	-91.8	-91.8	0.21 (1)	6.25		E-B	-38 / 59	0.02 (4)
B-C	-317 / 0	-91.8	-91.8	0.21 (1)	6.25		A-E	0 / 272	0.06 (1)
F-A	-438 / 0	0.0	0.0	0.05 (1)	7.81		E-C	0 / 272	0.08 (1)
D-C	-438 / 0	0.0	0.0	0.05 (1)	7.81				
F-E	0 / 0	-18.5	-18.5	0.09 (4)	10.00				
E-D	0 / 0	-18.5	-18.5	0.09 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.21/1.00 (A-B:1) , BC=0.09/1.00 (E-F:4) , WB=0.06/1.00 (C-E:1) , SSI=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (C) (INPUT = 0.90)
JSI METAL= 0.10 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/20/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006393

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	T15G	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Rod Truss, Burlington

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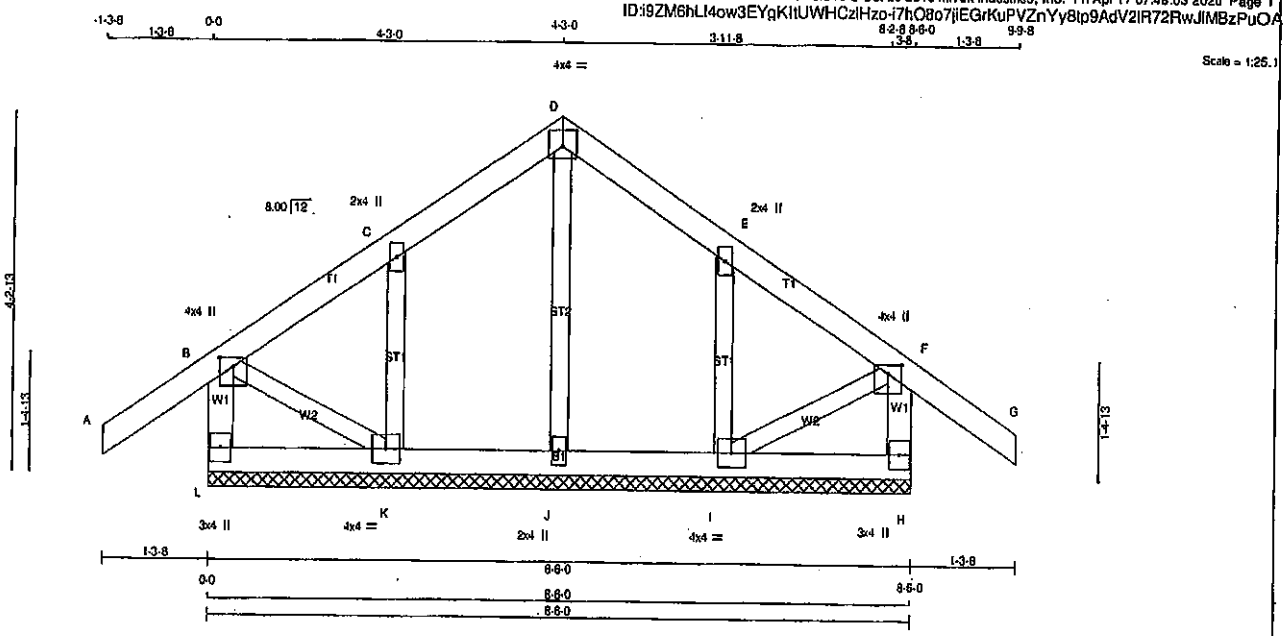


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0	1.25	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1+t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1+t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
L-B	-222 / 0	0.0	0.0	0.02 (1)		7.81
A-B	0 / 35	-91.8	-91.8	0.12 (1)		10.00
B-C	-8 / 0	-91.8	-91.8	0.07 (1)		10.00
C-D	-25 / 0	-91.8	-91.8	0.07 (1)		6.25
D-E	-25 / 0	-91.8	-91.8	0.07 (1)		6.25
E-F	-8 / 0	-91.8	-91.8	0.07 (1)		10.00
F-G	0 / 35	-91.8	-91.8	0.12 (1)		10.00
H-F	-222 / 0	0.0	0.0	0.02 (1)		7.81
L-K	0 / 0	-18.5	-18.5	0.02 (4)		10.00
K-J	0 / 10	-18.5	-18.5	0.02 (4)		10.00
J-I	0 / 10	-18.5	-18.5	0.02 (4)		10.00
I-H	0 / 0	-18.5	-18.5	0.02 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-D	-129 / 0	0.03 (1)
K-C	-237 / 0	0.04 (1)
I-E	-237 / 0	0.04 (1)
B-K	0 / 19	0.00 (1)
I-F	0 / 19	0.00 (1)

TOTAL WEIGHT = 38 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (F-G:1) , BC=0.02/1.00 (J-K:4) , WB=0.04/1.00 (C-K:1) , SSI=0.08/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

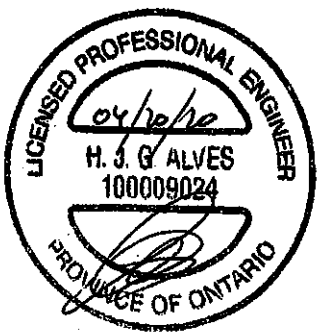
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (E) (INPUT = 0.90)
JSI METAL= 0.12 (E) (INPUT = 1.00)



Structural component only
DWG# T-2006394

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408205	T21	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Rod Truss, Burlington

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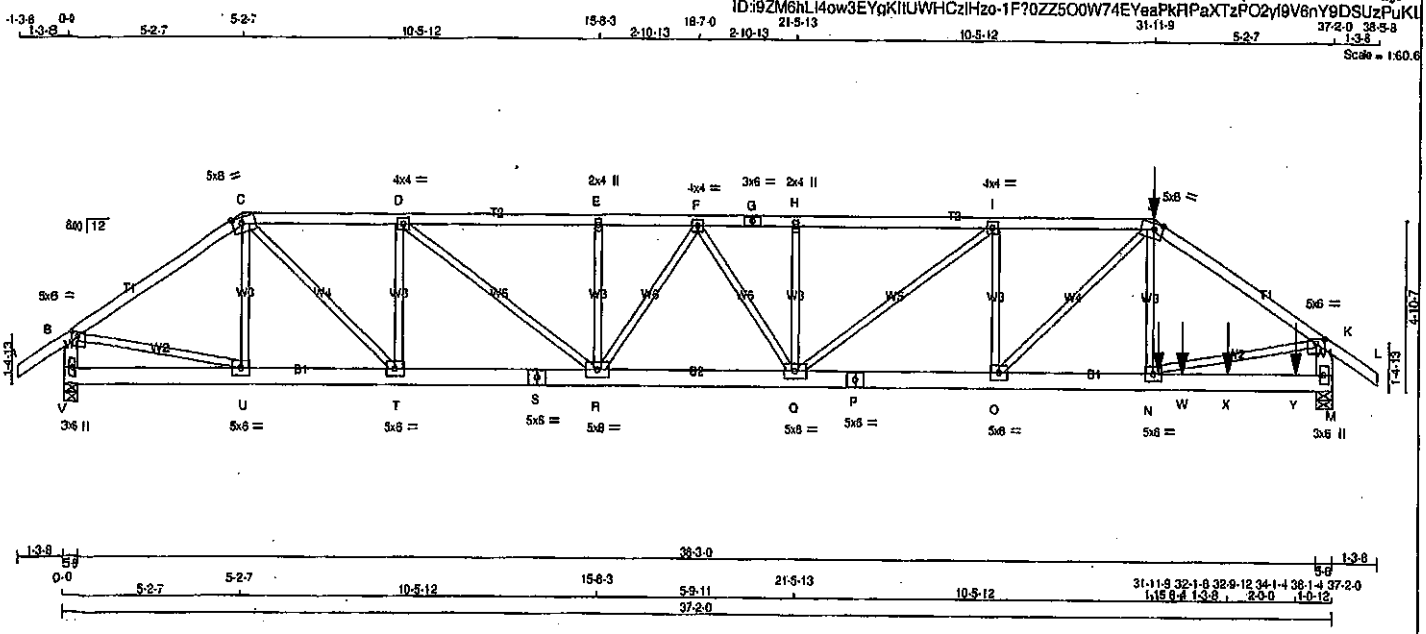

TOTAL WEIGHT = 2 X 176 = 352 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-G	1	12	TOP
G-J	1	12	SIDE(61.0)
J-L	1	12	SIDE(61.0)
V-B	2	12	TOP
M-K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V-S	2	12	TOP
S-P	2	12	TOP
P-M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
N-J	1	6	SIDE(132.7)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-m	MT20	5.0	8.0	2.00	3.25
D, F, I						
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
J	TTWW-m	MT20	5.0	8.0	2.00	3.25
K	TMVW-p	MT20	5.0	6.0	1.50	3.00
M	BMV1+p	MT20	3.0	6.0		
N, O, T, U						
N	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
V	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
V	2385	0	2385	0	0	5-8	5-8
M	3466	0	3466	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1684	1122 / 0	0 / 0	0 0	0 0	561 0	0 0
M	2446	1631 / 0	0 / 0	0 0	0 0	815 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	U-C	-331 / 0		0.06 (1)
B-C	-2774 / 0	-91.8	-91.8	0.30 (1)	5.18	C-T	0 / 2116		0.28 (1)
C-D	-3825 / 0	-91.8	-91.8	0.28 (1)	4.58	T-D	-1368 / 0		0.23 (1)
D-E	-4897 / 0	-91.8	-91.8	0.36 (1)	4.09	O-I	-944 / 0		0.16 (1)
E-F	-4897 / 0	-91.8	-91.8	0.22 (1)	4.17	O-J	0 / 1524		0.19 (1)
F-G	-5169 / 0	-91.8	-91.8	0.23 (1)	4.07	N-J	0 / 653		0.08 (1)
G-H	-5169 / 0	-91.8	-91.8	0.23 (1)	4.07	B-U	0 / 2346		0.29 (1)
H-I	-5169 / 0	-91.8	-91.8	0.37 (1)	3.99	N-K	0 / 3587		0.44 (1)
I-J	-4640 . 0	-91.8	-91.8	0.31 (1)	4.20	O-I	0 / 688		0.08 (1)
J-K	-4242 / 0	-91.8	-91.8	0.37 (1)	4.32	D-R	0 / 1355		0.17 (1)
K-L	0 35	-91.8	-91.8	0.07 (1)	10.00	O-H	-444 / 0		0.08 (1)
V-B	-2341 / 0	0.0	0.0	0.08 (1)	7.81	R-E	-443 / 0		0.08 (1)
M-K	-3388 / 0	0.0	0.0	0.12 (1)	7.57	R-F	-340 / 0		0.08 (1)
						F-Q	0 - 162		0.02 (1)
V-U	0 0	-18.5	-18.5	0.03 (4)	10.00				
U-T	0 2298	-18.5	-18.5	0.18 (1)	10.00				
T-S	0 3825	-18.5	-18.5	0.28 (1)	10.00				
S-R	0 3825	-18.5	-18.5	0.28 (1)	10.00				
R-Q	0 / 5081	-18.5	-18.5	0.37 (1)	10.00				
Q-P	0 / 4640	-18.5	-18.5	0.33 (1)	10.00				
P-O	0 . 4640	-18.5	-18.5	0.33 (1)	10.00				
O-N	0 . 3540	-18.5	-18.5	0.26 (1)	10.00				
N-W	0 0	-18.5	-18.5	0.05 (4)	10.00				
W-X	0 0	-18.5	-18.5	0.05 (4)	10.00				
X-Y	0 0	-18.5	-18.5	0.05 (4)	10.00				
Y-M	0 0	-18.5	-18.5	0.05 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE	HREQ	CONN.
J	31-11-9	-44	-49	---	FRONT	VERT	DEAD	---	C1
J	31-11-9	-195	-195	---	FRONT	VERT	SNOW	---	C1
N	32-1-8	-1211	-1211	---	FRONT	VERT	TOTAL	---	C1
W	32-9-12	-17	-17	---	FRONT	VERT	TOTAL	---	C1
X	34-1-4	-17	-17	---	FRONT	VERT	TOTAL	---	C1
Y	36-1-4	-17	-17	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.37/1.00 (H-I:1) , BC=0.37/1.00 (Q-R:1) , WB=0.44/1.00 (K-N:1) , SSI=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (U) (INPUT = 0.90)
JSI METAL= 0.44 (U) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/20/20
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006398

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408205	T21Z	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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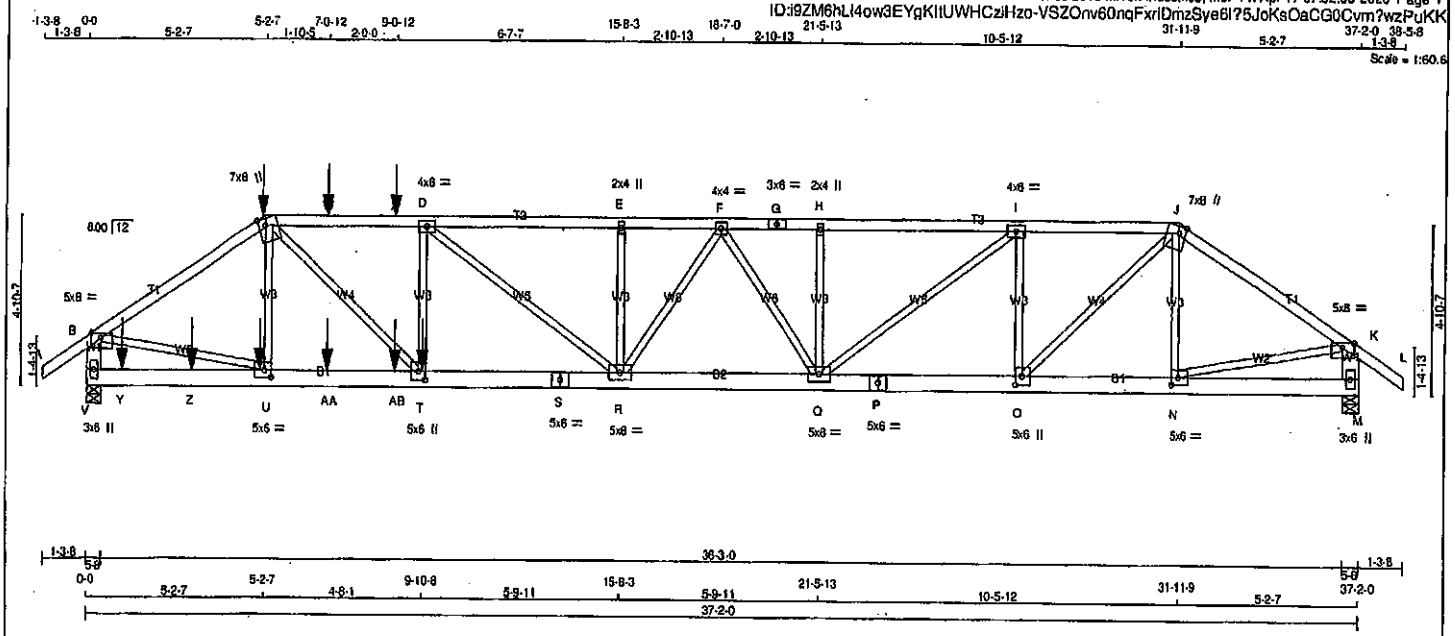


TOTAL WEIGHT = 2 X 176 = 352 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-G	1	12	SIDE(61.0)
G-J	1	12	TOP
J-L	1	12	TOP
V-B	2	12	TOP
M-K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V-S	2	12	SIDE(183.1)
S-P	2	12	TOP
P-M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
O-T	1	6	SIDE(329.1)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	2.50
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.50
K	TMW-p	MT20	5.0	8.0	Edge	
M	BMW+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMWW+l	MT20	5.0	6.0	3.00	2.25
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW+t	MT20	5.0	6.0	3.00	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	4111	0	4111	0	0	5-8	5-8
M	2792	0	2792	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2900	1941 / 0	0 / 0	0 / 0	0 0	959 0	0 0
M	1970	1317 / 0	0 / 0	0 0	0 0	653 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	U-C	-622 / 0	0.11 (1)
B-C	-5175 / 0	-91.8	-91.8	0.43 (1)	3.92	C-T	0 / 3885	0.48 (1)
C-W	-7089 / 0	-91.8	-91.8	0.48 (1)	3.33	T-O	-667 / 0	0.11 (1)
W-X	-7089 / 0	-91.8	-91.8	0.48 (1)	3.33	O-I	-1766 / 0	0.30 (1)
X-D	-7089 / 0	-91.8	-91.8	0.48 (1)	3.33	O-J	0 / 2700	0.33 (1)
D-E	-7113 / 0	-91.8	-91.8	0.58 (1)	3.33	N-J	-421 / 0	0.07 (1)
E-F	-7113 / 0	-91.8	-91.8	0.31 (1)	3.48	B-U	0 / 4375	0.54 (1)
F-G	-6307 / 0	-91.8	-91.8	0.28 (1)	3.60	N-K	0 / 2828	0.35 (1)
G-H	-6307 / 0	-91.8	-91.8	0.28 (1)	3.89	Q-I	0 / 2009	0.25 (1)
H-I	-6307 / 0	-91.8	-91.8	0.46 (1)	3.59	D-R	0 / 44	0.01 (4)
I-J	-4719 / 0	-91.8	-91.8	0.31 (1)	4.16	Q-H	-439 / 0	0.07 (1)
J-K	-3345 / 0	-91.8	-91.8	0.32 (1)	4.79	R-E	-427 / 0	0.07 (1)
K-L	0 / 35	-91.8	-91.8	0.07 (1)	10.00	R-F	0 / 648	0.09 (1)
V-B	-4051 / 0	0.0	0.0	0.14 (1)	7.06	F-Q	-838 / 0	0.21 (1)
M-K	-2748 / 0	0.0	0.0	0.10 (1)	7.81			
V-Y	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
Y-Z	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
Z-U	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
U-AA	0 / 4286	-18.5	-18.5	0.33 (1)	10.00			
AA-AB	0 / 4286	-18.5	-18.5	0.33 (1)	10.00			
AB-T	0 / 4286	-18.5	-18.5	0.33 (1)	10.00			
T-S	0 / 7089	-18.5	-18.5	0.52 (1)	10.00			
S-R	0 / 7089	-18.5	-18.5	0.52 (1)	10.00			
R-Q	0 / 6762	-18.5	-18.5	0.48 (1)	10.00			
Q-P	0 / 4719	-18.5	-18.5	0.35 (1)	10.00			
P-O	0 / 4719	-18.5	-18.5	0.35 (1)	10.00			
O-N	0 / 2770	-18.5	-18.5	0.19 (1)	10.00			
N-M	0 / 0	-18.5	-18.5	0.03 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	LC
C	5-2-7	-44	-49	---	FRONT	VERT	DEAD	C1
C	5-2-7	-92	-92	---	BACK	VERT	TOTAL	C1
C	5-2-7	-195	-195	---	FRONT	VERT	SNOW	C1
T	9-10-8	-1983	-1983	---	BACK	VERT	TOTAL	C1
U	5-0-12	-17	-17	---	BACK	VERT	TOTAL	C1
W	7-0-12	-86	-86	---	BACK	VERT	TOTAL	C1
X	9-0-12	-86	-86	---	BACK	VERT	TOTAL	C1
Y	1-0-12	-17	-17	---	BACK	VERT	TOTAL	C1
Z	3-0-12	-17	-17	---	BACK	VERT	TOTAL	C1
AA	7-0-12	-17	-17	---	BACK	VERT	TOTAL	C1
AB	9-0-12	-17	-17	---	BACK	VERT	TOTAL	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.38")

CSI: TC=0.56/1.00 (D-E:1) , BC=0.52/1.00 (R-T:1) , WB=0.54/1.00 (B-U:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.65 (S) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/20/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only 1/2
DWG# T-2006399

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408205	T21Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

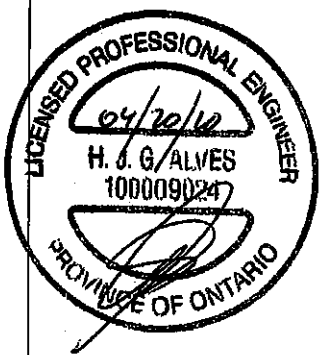
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
U	BMWW+	MT20	5.0	6.0	2.50	2.50
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2006399

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408205	T22	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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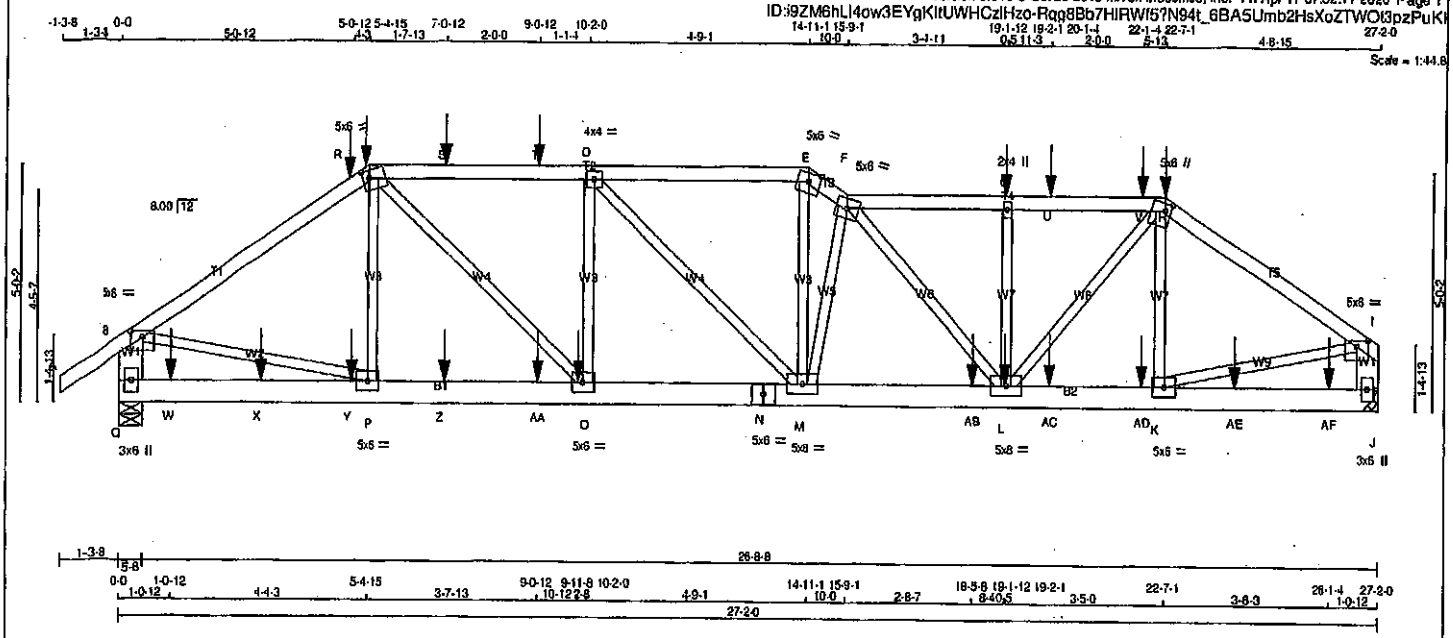


TOTAL WEIGHT = 2 X 133 = 266 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-E	1	12	SIDE(61.0)
E-F	1	12	TOP
F-H	1	12	SIDE(61.0)
H-I	1	12	SIDE(61.0)
Q-B	2	12	TOP
J-I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q-N	2	12	SIDE(183.1)
N-J	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-m	MT20	5.0	6.0	2.00	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	5.0	6.0	Edge	
F	TTWW-m	MT20	5.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	5.0	6.0	2.00	1.50
I	TMVW-p	MT20	5.0	6.0	1.50	3.00
J	BMV1+p	MT20	3.0	6.0		
K, O, P						
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BS-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	2768	0	2768	0	0	5-8	5-8
J	2551	0	2551	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1950	1318 / 0	0 / 0	0 / 0	0 0	633 / 0	0 0
J	1799	1208 / 0	0 / 0	0 / 0	0 / 0	592 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

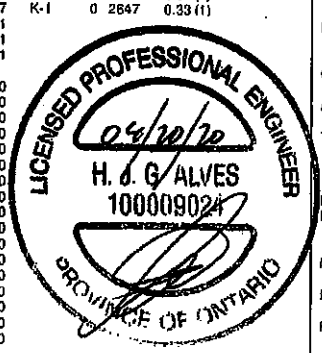
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	
B-R	-3253 / 0	-91.8	-91.8	0.37 (1)	4.79	
R-C	-3253 / 0	-91.8	-91.8	0.37 (1)	4.79	
C-S	-4025 / 0	-91.8	-91.8	0.30 (1)	4.44	
S-T	-4025 / 0	-91.8	-91.8	0.30 (1)	4.44	
T-D	-4025 / 0	-91.8	-91.8	0.30 (1)	4.44	
D-E	-3973 / 0	-91.8	-91.8	0.30 (1)	4.47	
E-F	-4710 / 0	-91.8	-91.8	0.07 (1)	4.40	
F-G	-3896 / 0	-91.8	-91.8	0.15 (1)	4.67	
G-U	-3896 / 0	-91.8	-91.8	0.15 (1)	4.67	
U-V	-3896 / 0	-91.8	-91.8	0.15 (1)	4.67	
V-H	-3896 / 0	-91.8	-91.8	0.15 (1)	4.67	
H-I	-3117 / 0	-91.8	-91.8	0.24 (1)	5.01	
Q-B	-2701 / 0	0.0	0.0	0.10 (1)	7.81	
J-I	-2496 / 0	0.0	0.0	0.09 (1)	7.81	
Q-W	0 / 0	-18.5	-18.5	0.05 (4)	10.00	
W-X	0 / 0	-18.5	-18.5	0.05 (4)	10.00	
X-Y	0 / 0	-18.5	-18.5	0.05 (4)	10.00	
Y-P	0 / 0	-18.5	-18.5	0.05 (4)	10.00	
P-Z	0 / 2721	-18.5	-18.5	0.21 (1)	10.00	
Z-AA	0 / 2721	-18.5	-18.5	0.21 (1)	10.00	
AA-O	0 / 2721	-18.5	-18.5	0.21 (1)	10.00	
O-N	0 / 4025	-18.5	-18.5	0.29 (1)	10.00	
N-M	0 / 4025	-18.5	-18.5	0.29 (1)	10.00	
M-AB	0 / 4418	-18.5	-18.5	0.39 (1)	10.00	
AB-L	0 / 4418	-18.5	-18.5	0.39 (1)	10.00	
L-AC	0 / 2578	-18.5	-18.5	0.18 (1)	10.00	
AC-AD	0 / 2578	-18.5	-18.5	0.18 (1)	10.00	
AD-K	0 / 2578	-18.5	-18.5	0.18 (1)	10.00	
K-AE	0 0	-18.5	-18.5	0.03 (4)	10.00	
AE-AF	0 / 0	-18.5	-18.5	0.03 (4)	10.00	
AF-J	0 / 0	-18.5	-18.5	0.03 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-C	-315 / 32	0.06 (1)
C-O	0 / 1817	0.22 (1)
O-D	-587 / 0	0.11 (1)
D-M	-73 / 0	0.03 (1)
M-E	0 / 2406	0.30 (1)
M-F	-2223 / 0	0.34 (1)
F-L	-811 / 0	0.19 (1)
L-G	-475 / 0	0.07 (1)
L-H	0 / 2048	0.25 (1)
K-H	-447 / 0	0.06 (1)
B-P	0 2773	0.34 (1)
K-I	0 2647	0.33 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-4-15	-41	-46	---	FRONT	VERT	DEAD	---	C1
C	5-4-15	-218	-218	---	FRONT	VERT	SNOW	---	C1
G	19-1-12	-63	-63	---	TOP	VERT	TOTAL	---	C1



DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015.

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.91")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.91")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.37/1.00 (B-C:1) , BC=0.39/1.00 (L-M:1) , WB=0.34/1.00 (B-P:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1655

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.45 (E) (INPUT = 1.00)

Structural component only
DWG# T-2006400

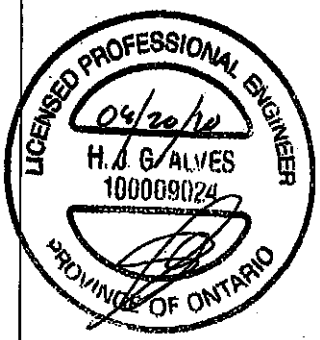
CONTINUED ON PAGE 2

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	22-7-1	-29	-32	---	FRONT	VERT	DEAD	---	C1
H	22-7-1	-147	-147	---	FRONT	VERT	SNOW	---	C1
L	19-1-12	-13	-13	---	FRONT	VERT	TOTAL	---	C1
O	9-11-8	-620	-620	---	FRONT	VERT	TOTAL	---	C1
R	5-0-12	-124	-124	---	FRONT	VERT	TOTAL	---	C1
S	7-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	C1
T	9-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	C1
U	20-1-4	-63	-63	---	TOP	VERT	TOTAL	---	C1
V	22-1-4	-87	-87	---	TOP	VERT	TOTAL	---	C1
W	1-0-12	-19	-19	---	FRONT	VERT	TOTAL	---	C1
X	3-0-12	-19	-19	---	FRONT	VERT	TOTAL	---	C1
Y	5-0-12	-19	-19	---	FRONT	VERT	TOTAL	---	C1
Z	7-0-12	-19	-19	---	FRONT	VERT	TOTAL	---	C1
AA	9-0-12	-19	-19	---	FRONT	VERT	TOTAL	---	C1
AB	18-5-8	-461	-461	---	FRONT	VERT	TOTAL	---	C1
AC	20-1-4	-13	-13	---	FRONT	VERT	TOTAL	---	C1
AD	22-1-4	-13	-13	---	FRONT	VERT	TOTAL	---	C1
AE	24-1-4	-13	-13	---	FRONT	VERT	TOTAL	---	C1
AF	26-1-4	-13	-13	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2006400

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408205	T24	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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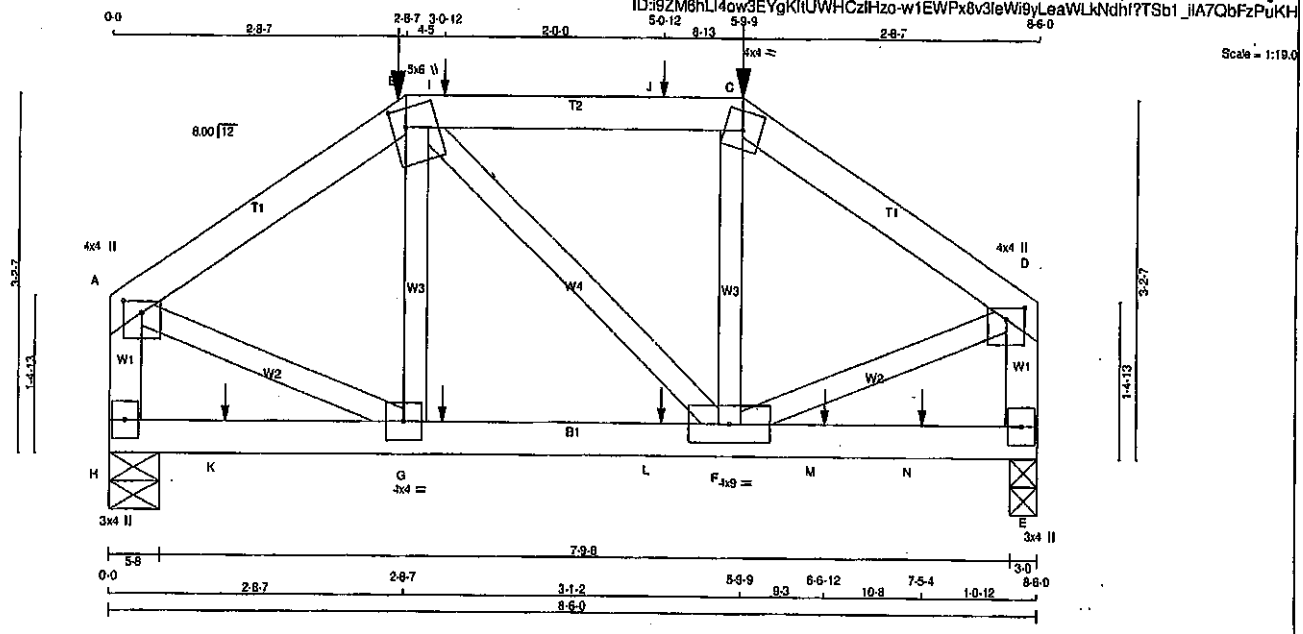

TOTAL WEIGHT = 35 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTWW+m	MT20	5.0	8.0	2.00	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	512	0	512	0	0	5-8	5-8
E	512	0	512	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	361	242 / 0	0 / 0	0 / 0	0 0	119 0	0 0
E	361	242 / 0	0 / 0	0 / 0	0 0	119 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	-417 / 0	-91.8	-91.8	0.13 (1)	6.25	G-B	-76 / 21	0.02 (1)
B-I	-343 / 0	-91.8	-91.8	0.17 (1)	6.25	B-F	0 / 6	0.00 (5)
I-J	-343 / 0	-91.8	-91.8	0.17 (1)	6.25	F-C	-77 / 21	0.02 (1)
J-C	-343 / 0	-91.8	-91.8	0.17 (1)	6.25	F-D	0 / 371	0.09 (1)
C-D	-417 / 0	-91.8	-91.8	0.13 (1)	6.25	A-G	0 / 371	0.09 (1)
H-A	-490 / 0	0.0	0.0	0.06 (1)	7.81			
E-D	-490 / 0	0.0	0.0	0.06 (1)	7.81			
H-K	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
K-G	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
G-L	0 / 344	-18.5	-18.5	0.08 (1)	10.00			
L-F	0 / 344	-18.5	-18.5	0.08 (1)	10.00			
F-M	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
M-N	0 0	-18.5	-18.5	0.04 (4)	10.00			
N-E	0 0	-18.5	-18.5	0.04 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	2-8-7	-6	-7	---	FRONT	VERT	DEAD	---	C1
B	2-8-7	-38	-38	---	FRONT	VERT	SNOW	---	C1
C	5-9-9	-6	-7	---	FRONT	VERT	DEAD	---	C1
C	5-9-9	-38	-38	---	FRONT	VERT	SNOW	---	C1
G	3-0-12	1	1	---	BACK	VERT	TOTAL	---	C1
I	3-0-12	1	1	91	BACK	VERT	TOTAL	---	C1
J	5-0-12	1	1	106	BACK	VERT	TOTAL	---	C1
K	1-0-12	1	1	---	BACK	VERT	TOTAL	---	C1
L	5-0-12	1	1	---	BACK	VERT	TOTAL	---	C1
M	6-6-12	1	1	---	BACK	VERT	TOTAL	---	C1
N	7-5-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019 .
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.17/1.00 (B-C:1) , BC=0.08/1.00 (F-G:1) , WB=0.09/1.00 (A-G:1) , SS=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (G) (INPUT = 0.90)
JSI METAL= 0.13 (G) (INPUT = 1.00)

Structural component only
DWG# T-2006401

LICENSED PROFESSIONAL ENGINEER
04/17/20
H.J. G. ALVES
100009024
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408205	T25A	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack RoofTruss, Burlington

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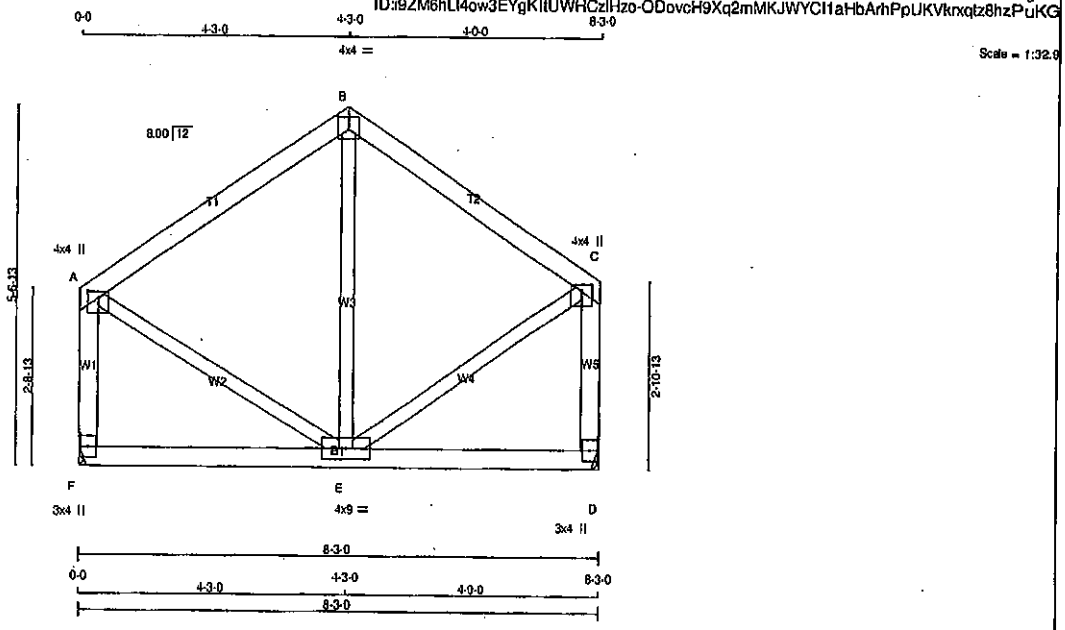

TOTAL WEIGHT = 38 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWHp	MT20	4.0	4.0	1.25	2.00
B	TTW-p	MT20	4.0	4.0	2.25	2.00
C	TMWHp	MT20	4.0	4.0	1.25	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	455	0	455	0	0	MECHANICAL	
D	455	0	455	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	322	211 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
D	322	211 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	-223 / 0	-91.8	-91.8	21 (1)	0.21 (1)	6.25	E-B	-131 / 23	0.06 (1)
B-C	-223 / 0	-91.8	-91.8	19 (1)	0.19 (1)	6.25	A-E	0 / 213	0.05 (1)
F-A	-424 / 0	0.0	0.0	06 (1)	0.06 (1)	7.81	E-C	0 / 221	0.05 (1)
D-C	-427 / 0	0.0	0.0	07 (1)	0.07 (1)	7.81			
F-E	0 / 0	-18.5	-18.5	09 (4)	0.09 (4)	10.00			
E-D	0 / 0	-18.5	-18.5	09 (4)	0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.21/1.00 (A-B:1) , BC=0.09/1.00 (E-F:4) , WB=0.06/1.00 (B-E:1) , SSI=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.30 (C) (INPUT = 0.90)
JSI METAL = 0.09 (C) (INPUT = 1.00)

Structural component only
DWG# T-2006402

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408205	T26	1	2	GREEN PARK HOMES TRUSS DESC.	

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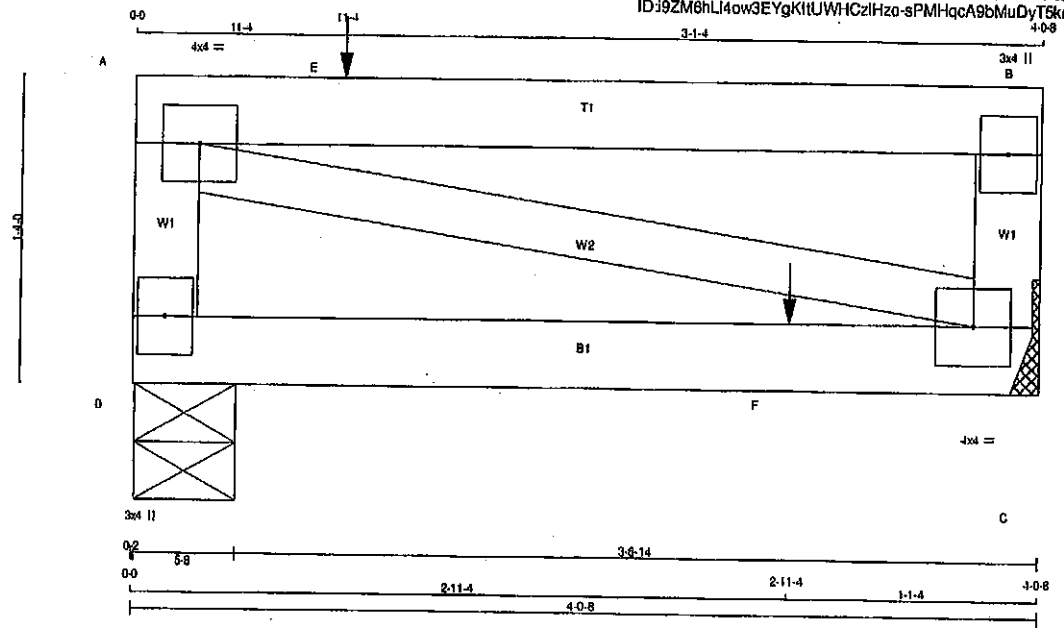


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
D- A 1	12	TOP
A- B 1	12	TOP
B- C 1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
D- C 1	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0		
B	TMV+p	MT20	3.0	4.0		
C	BMVW1-t	MT20	4.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	667	0	667	0	0	5-8	5-8
C	638	0	638	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	472	309 / 0	0 / 0	0 / 0	0 / 0	163 / 0	0 / 0
C	450	302 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
D- A	-511 / 0	0.0	0.0	0.03 (1)		7.81
A- E	0 / 0	-91.8	-91.8	0.33 (1)		10.00
E- B	0 / 0	-91.8	-91.8	0.33 (1)		10.00
C- B	-284 / 0	0.0	0.0	0.02 (1)		7.81
D- F	0 / 0	-18.5	-18.5	0.28 (1)		10.00
F- C	0 / 0	-18.5	-18.5	0.28 (1)		10.00

WEBS

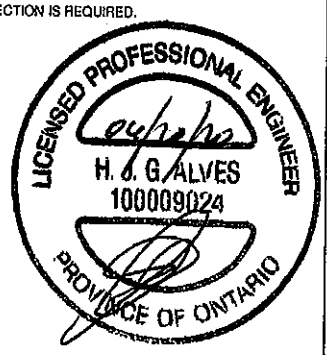
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- C	0 / 0	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	11-4	-423	-423	---	TOP	VERT	TOTAL	---	C1
F	2-11-4	-436	-436	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



TOTAL WEIGHT = 2 X 14 = 28 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 944 (0.05")

CSI: TC=0.33/1.00 (A-B:1) , BC=0.28/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.22/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.15 (A) (INPUT = 0.90)
JSI METAL = 0.04 (A) (INPUT = 1.00)

Structural component only
DWG# T-2006403

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408205	T27	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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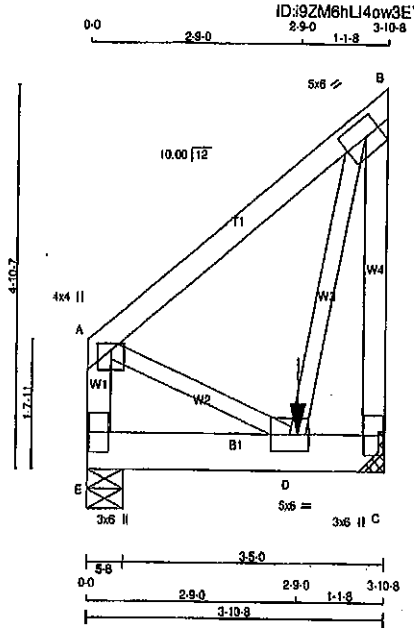


TOTAL WEIGHT = 2 X 24 = 48 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
E - A	2x4	DRY	No.2	SPF
E - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-B	1	12	TOP
B-C	1	12	TOP
E-A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
E-C	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMVW-t	MT20	5.0	8.0	2.50	2.50
C	BMV1+p	MT20	3.0	6.0		
D	BMWW-t	MT20	5.0	6.0		
E	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
C	1982	0	1982	0	0	MECHANICAL	
E	978	0	978	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	1397	942 / 0	0 / 0	0 / 0	0 / 0	455 0	0 / 0
E	689	463 / 0	0 / 0	0 / 0	0 / 0	226 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

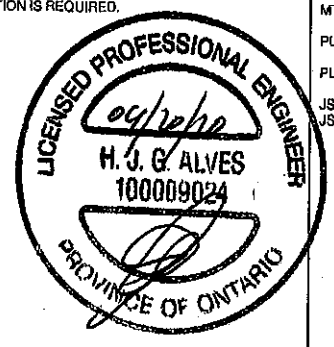
CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	-818 / 0	-91.8	-91.8	0.13 (1)	6.25	A-D	0 / 519	0.06 (1)
C-B	-1585 / 0	0.0	0.0	0.28 (1)	7.81	D-B	0 / 1862	0.23 (1)
E-A	-788 / 0	0.0	0.0	0.04 (1)	7.81			
E-D	0 / 0	-18.5	-18.5	0.17 (1)	10.00			
D-C	0 / 0	-18.5	-18.5	0.17 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	2-9-0	-2532	-2532	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.28/1.00 (B-C:1) , BC=0.17/1.00 (D-E:1) , WB=0.23/1.00 (B-D:1) , SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (B) (INPUT = 0.90)
JSI METAL= 0.20 (D) (INPUT = 1.00)

Structural component only
DWG# T-2006404

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408206	T31	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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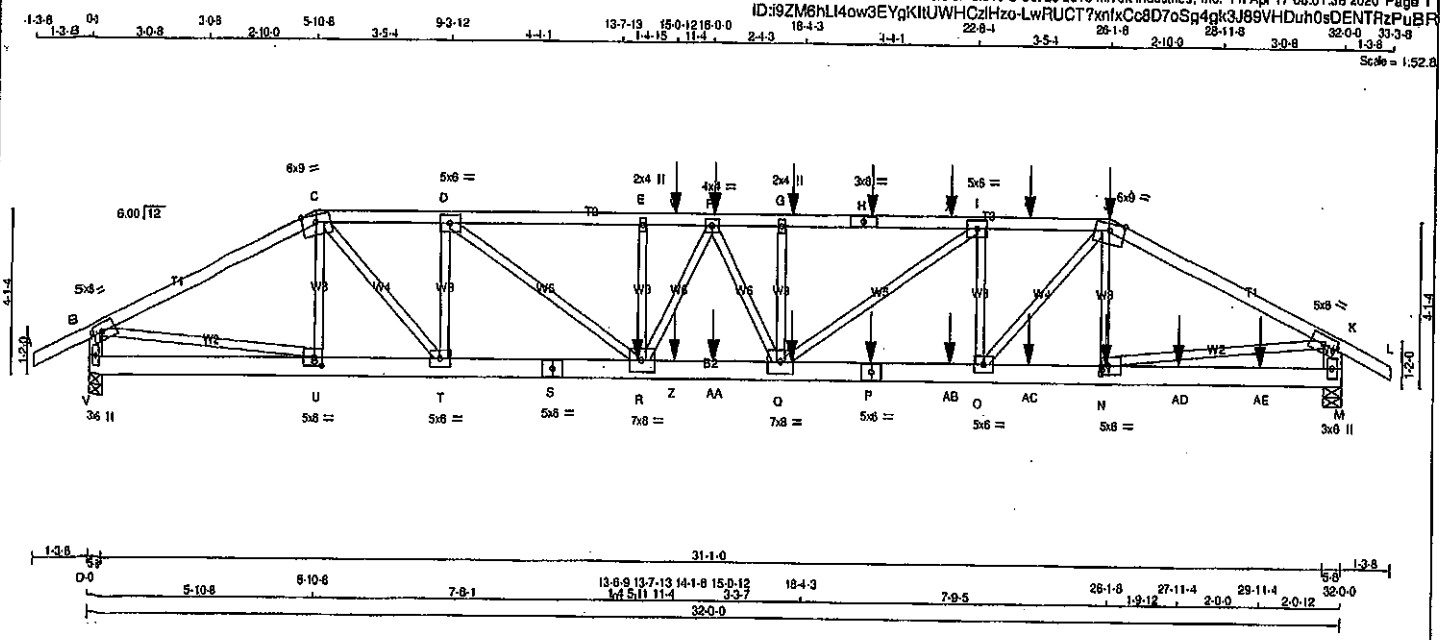


TOTAL WEIGHT = 2 X 150 = 299 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR
A - C	2x4	DRY	No.2	SPF
C - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-H	1	12	SIDE(61.0)
H-J	1	12	SIDE(61.0)
J-L	1	12	SIDE(61.0)
V-B	2	12	TOP
M-K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V-S	2	12	TOP
S-P	2	12	SIDE(183.1)
P-M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
Q-G	1	5	
R-E	1	5	SIDE(313.6)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (tabs is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TTWW-m	MT20	8.0	9.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	8.0		
I	TMWW-t	MT20	5.0	6.0		
J	TTWW-m	MT20	6.0	9.0	Edge	
K	TMVW-t	MT20	5.0	8.0		
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.25
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	7.0	8.0		
R	BMWWW-t	MT20	7.0	8.0		
S	BS-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
V	3370	0	3370	0	0	5-8	5-8
M	3670	0	3670	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2376	1597 / 0	0 0	0 0	0 0	779 0	0 0
M	2591	1727 / 0	0 / 0	0 0	0 0	864 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	
B-C	-5162 / 0	-91.8	-91.8	0.53 (1)	3.83	
C-D	-8810 / 0	-91.8	-91.8	0.30 (1)	3.54	
D-E	-9348 / 0	-91.8	-91.8	0.55 (1)	2.89	
E-W	-9348 / 0	-91.8	-91.8	0.36 (1)	2.97	
W-F	-9348 / 0	-91.8	-91.8	0.36 (1)	2.97	
F-G	-8840 / 0	-91.8	-91.8	0.39 (1)	3.01	
G-H	-8840 / 0	-91.8	-91.8	0.59 (1)	3.92	
H-X	-8840 / 0	-91.8	-91.8	0.59 (1)	2.92	
X-I	-8840 / 0	-91.8	-91.8	0.59 (1)	2.92	
I-Y	-7001 / 0	-91.8	-91.8	0.39 (1)	3.39	
Y-J	-7001 / 0	-91.8	-91.8	0.39 (1)	3.39	
J-K	-5653 / 0	-91.8	-91.8	0.57 (1)	3.66	
K-L	0 28	-91.8	-91.8	0.07 (1)	10.00	
V-B	-3305 0	0.0	0.0	0.12 (1)	7.64	
M-K	-3582 / 0	0.0	0.0	0.13 (1)	7.42	
V-U	0 / 0	-18.5	-18.5	0.05 (4)	10.00	
U-T	0 / 4599	-18.5	-18.5	0.32 (1)	10.00	
T-S	0 / 6810	-18.5	-18.5	0.53 (1)	10.00	
S-R	0 / 6810	-18.5	-18.5	0.53 (1)	10.00	
R-Z	0 9129	-18.5	-18.5	0.69 (1)	10.00	
Z-AA	0 / 9129	-18.5	-18.5	0.69 (1)	10.00	
AA-Q	0 / 9129	-18.5	-18.5	0.69 (1)	10.00	
Q-P	0 / 7001	-18.5	-18.5	0.52 (1)	10.00	
P-AB	0 7001	-18.5	-18.5	0.52 (1)	10.00	
AB-O	0 / 7001	-18.5	-18.5	0.52 (1)	10.00	
O-AC	0 5038	-18.5	-18.5	0.35 (1)	10.00	
AC-N	0 / 5038	-18.5	-18.5	0.35 (1)	10.00	
N-AD	0 0	-18.5	-18.5	0.07 (4)	10.00	
AD-AE	0 / 0	-18.5	-18.5	0.07 (4)	10.00	
AE-M	0 / 0	-18.5	-18.5	0.07 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-U	0 / 4653	0.58 (1)
U-C	-546 / 0	0.07 (1)
C-T	0 / 3314	0.41 (1)
T-D	-2313 0	0.29 (1)
D-I	-2033 / 0	0.26 (1)
O-J	0 2941	0.36 (1)
N-J	-522 / 0	0.07 (1)
N-K	0 / 5095	0.63 (1)
O-I	0 / 2290	0.28 (1)
D-R	0 / 3160	0.39 (1)
Q-G	-597 / 0	0.08 (1)
R-E	-418 / 0	0.05 (1)
R-F	0 / 507	0.06 (1)
F-Q	-674 0	0.10 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HGR	CONN.
F	16-0-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
G	18-0-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
H	20-0-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
J	26-1-8	-459	-459	---	FRONT	VERT	TOTAL	---	C1
N	26-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	20-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Q	18-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
R	14-1-8	-1904	-1904	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 856 (0.45")

CSI: TC=0.59/1.00 (G-I:1) , BC=0.69/1.00 (Q-R:1) , WB=0.63/1.00 (K-N:1) , SSI=0.17/1.00 (G-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (T) (INPUT = 0.90)
JSI METAL= 0.67 (S) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only 1/2
DWG# T-2006409

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408206	T31	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
T	BMWW-t	MT20	5.0	8.0		
U	BMWW-t	MT20	5.0	8.0	2.50	2.25
V	BMV1+p	MT20	3.0	8.0		

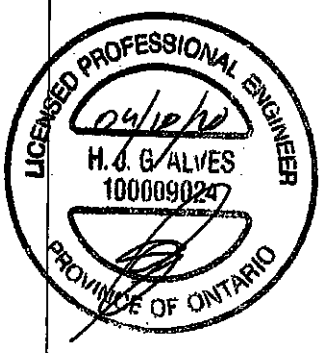
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
W	15-0-12	-110	-110	---	FRONT	VERT	TOTAL	--	C1
X	22-0-12	-110	-110	---	FRONT	VERT	TOTAL	--	C1
Y	24-0-12	-110	-110	---	FRONT	VERT	TOTAL	--	C1
Z	15-0-12	-26	-26	---	FRONT	VERT	TOTAL	--	C1
AA	16-0-12	-26	-26	---	FRONT	VERT	TOTAL	--	C1
AB	22-0-12	-26	-26	---	FRONT	VERT	TOTAL	--	C1
AC	24-0-12	-26	-26	---	FRONT	VERT	TOTAL	--	C1
AD	27-11-4	-26	-26	---	FRONT	VERT	TOTAL	--	C1
AE	29-11-4	-26	-26	---	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2006409

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408206	T31Z	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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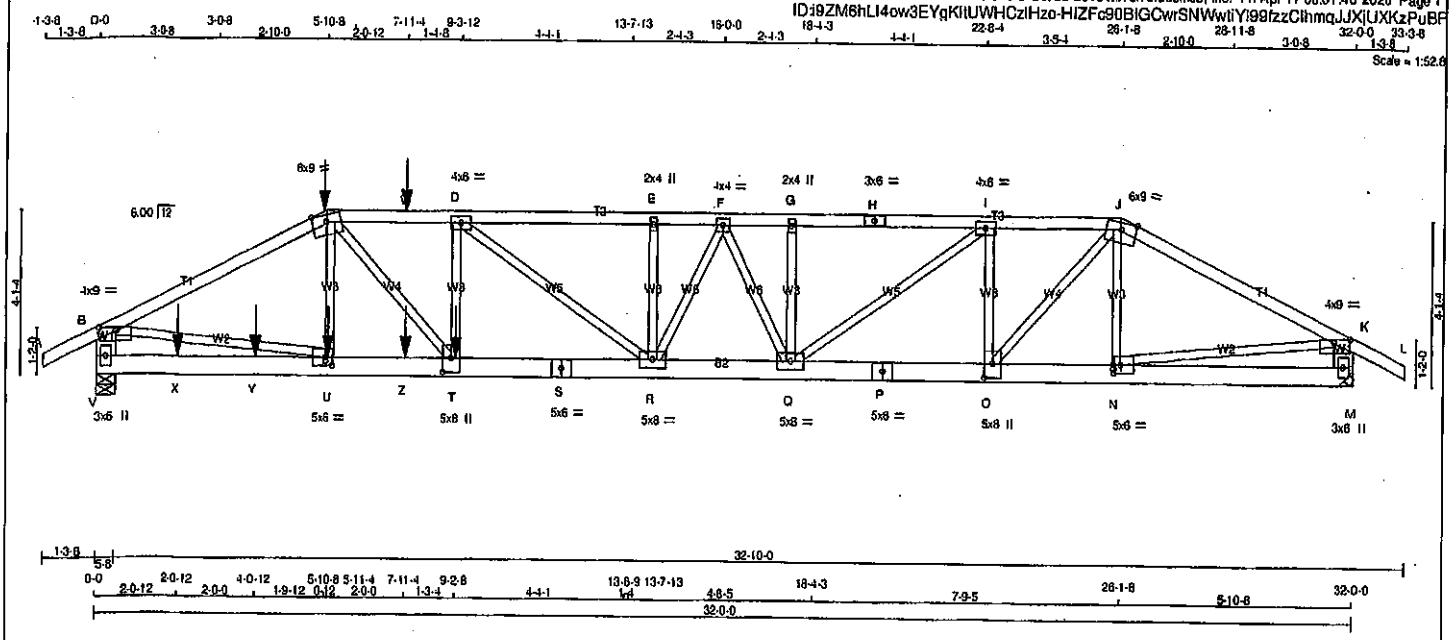


TOTAL WEIGHT = 2 X 150 = 299 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x8	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-H	1	12	SIDE(61.0)
H-J	1	12	TOP
J-L	1	12	TOP
V-B	2	12	TOP
M-K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V-S	2	12	SIDE(183.1)
S-P	2	12	TOP
P-M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
D-T	1	4	SIDE(426.9)
I-O	1	4	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	9.0	1.00	4.75
C	TTWW-m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW-m	MT20	6.0	9.0	Edge	
K	TMVW-p	MT20	4.0	9.0	1.00	4.75
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.00
O	BMWW+t	MT20	5.0	6.0	4.25	2.50
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
V	4101	0	4101	0	0	5-8	5-8
M	2690	0	2690	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2893	1938 / 0	0 / 0	0 / 0	0 / 0	954 / 0	0 / 0
M	1897	1272 / 0	0 / 0	0 / 0	0 / 0	625 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	
B-C	-6420 / 0	-91.8	-91.8	0.64 (1)	3.41	
C-W	-7989 / 0	-91.8	-91.8	0.36 (1)	3.23	
W-D	-7989 / 0	-91.8	-91.8	0.36 (1)	3.23	
D-E	-7575 / 0	-91.8	-91.8	0.40 (1)	3.34	
E-F	-7575 / 0	-91.8	-91.8	0.26 (1)	3.37	
F-G	-8791 / 0	-91.8	-91.8	0.24 (1)	3.59	
G-H	-8791 / 0	-91.8	-91.8	0.35 (1)	3.55	
H-I	-8791 / 0	-91.8	-91.8	0.35 (1)	3.65	
I-J	-5124 / 0	-91.8	-91.8	0.22 (1)	4.10	
J-K	-3963 / 0	-91.8	-91.8	0.44 (1)	4.35	
K-L	0 / 28	-91.8	-91.8	0.07 (1)	10.00	
V-B	-4015 / 0	0.0	0.0	0.14 (1)	7.10	
M-K	-2629 / 0	0.0	0.0	0.09 (1)	7.81	
V-X	0 / 0	-18.5	-18.5	0.07 (4)	10.00	
X-Y	0 / 0	-18.5	-18.5	0.07 (4)	10.00	
Y-U	0 / 0	-18.5	-18.5	0.07 (4)	10.00	
U-Z	0 / 5723	-18.5	-18.5	0.44 (1)	10.00	
Z-T	0 / 5723	-18.5	-18.5	0.44 (1)	10.00	
T-S	0 / 7989	-18.5	-18.5	0.59 (1)	10.00	
S-R	0 / 7989	-18.5	-18.5	0.59 (1)	10.00	
R-Q	0 / 7181	-18.5	-18.5	0.51 (1)	10.00	
Q-P	0 / 5124	-18.5	-18.5	0.38 (1)	10.00	
P-O	0 / 5124	-18.5	-18.5	0.38 (1)	10.00	
O-N	0 / 3532	-18.5	-18.5	0.24 (1)	10.00	
N-M	0 / 0	-18.5	-18.5	0.04 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-U	0 / 5787	0.72 (1)
U-C	-549 / 0	0.07 (1)
C-T	0 / 3395	0.42 (1)
T-D	-177 / 0	0.02 (1)
O-I	-1665 / 0	0.21 (1)
O-J	0 / 2387	0.30 (1)
N-J	-389 / 0	0.05 (1)
N-K	0 / 3572	0.44 (1)
Q-I	0 / 2076	0.26 (1)
D-R	-516 / 0	0.18 (1)
Q-G	-388 / 0	0.05 (1)
R-E	-390 / 0	0.05 (1)
R-F	0 / 918	0.11 (1)
F-Q	-909 / 0	0.13 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR	TYPE	REF	CONN
C	5-10-8	-459	-459	---	BACK	VERT	TOTAL	---	C1
T	9-2-8	-2338	-2338	---	BACK	VERT	TOTAL	---	C1
U	5-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
W	7-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
X	2-0-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
Y	4-0-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
Z	7-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.37")

CSI: TC=0.64/1.00 (B-C:1) , BC=0.59/1.00 (R-T:1) , WB=0.72/1.00 (B-U:1) , SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (N) (INPUT = 0.90)
SI METAL= 0.75 (S) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/10/20
H. O. GALVES
100090926
PROVINCE OF ONTARIO

Structural component only 1/2
DWG# T-2006410

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
408206	T31Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

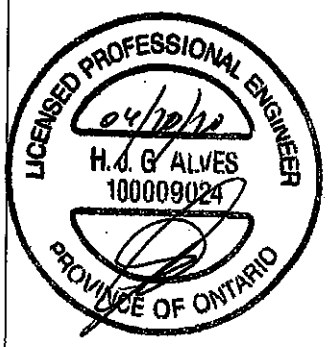
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
T	BMWW+t	MT20	5.0	6.0	4.25	2.50
U	BMWW-t	MT20	5.0	6.0	2.50	2.00
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2006410

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408206	T32	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Flod Truss, Burlington

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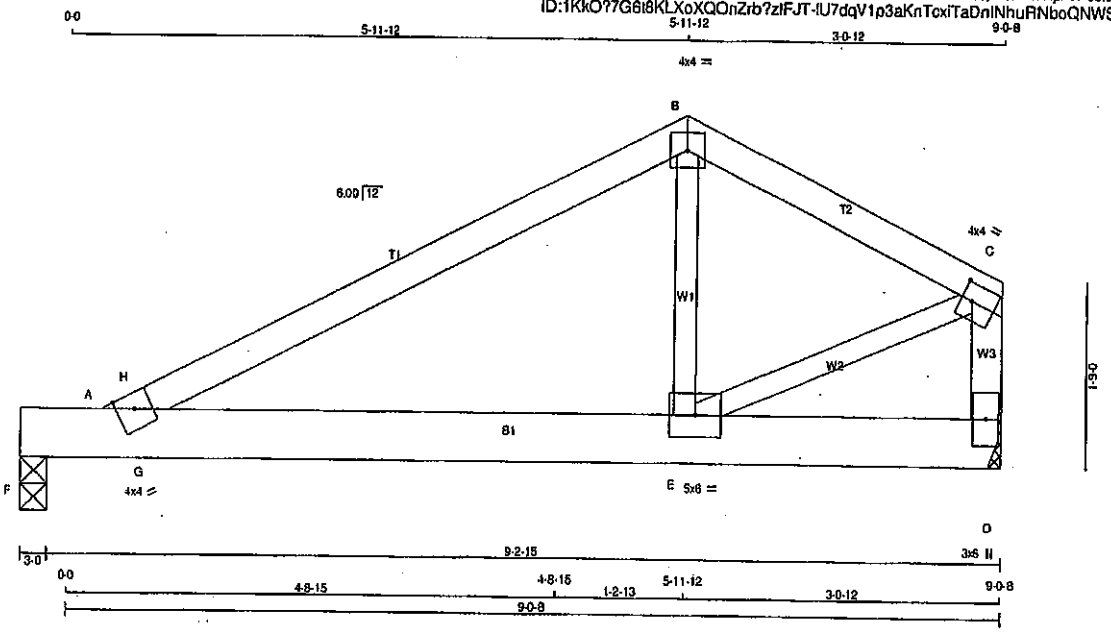


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM-h	MT20	4.0	4.0	Edge	
B	TTW-p	MT20	4.0	4.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	BMV1+p	MT20	3.0	6.0		
E	BMWW-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	524	0	524	0	0	3-0	3-0
D	524	0	524	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	370	243 / 0	0 0	0 0	0 0	127 0	0 0
D	370	243 / 0	0 0	0 0	0 0	127 0	0 0

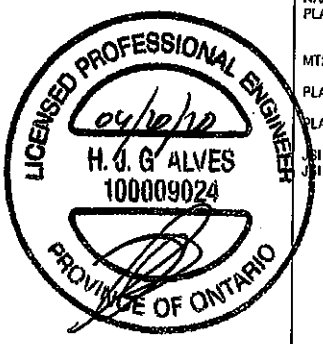
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- H	-378 / 0	-91.8	-91.8	1	0.20 (1)	6.25	E- B	0 / 109	0.03 (4)
H- B	-524 / 0	-91.8	-91.8	1	0.33 (1)	6.25	E- C	0 / 500	0.11 (1)
B- C	-511 / 0	-91.8	-91.8	1	0.15 (1)	6.25	G- H	-531 / 0	0.00 (1)
D- C	-572 / 0	0.0	0.0	1	0.06 (1)	7.81			
F- A	0 / 0	-110.3	-110.3	1	0.25 (1)	10.00			
A- G	0 / 457	-18.5	-18.5	1	0.45 (1)	10.00			
G- E	0 457	-18.5	-18.5	1	0.45 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	1	0.16 (1)	10.00			



TOTAL WEIGHT = 2 X 34 = 68 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.33/1.00 (B-H:1) , BC=0.45/1.00 (A-G:1) , WB=0.11/1.00 (C-E:1) , SSI=0.32/1.00 (A-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP = 0.79 (C) (INPUT = 0.90)
SI METAL = 0.22 (C) (INPUT = 1.00)

Structural component only
DWG# T-2006411

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408206	T33	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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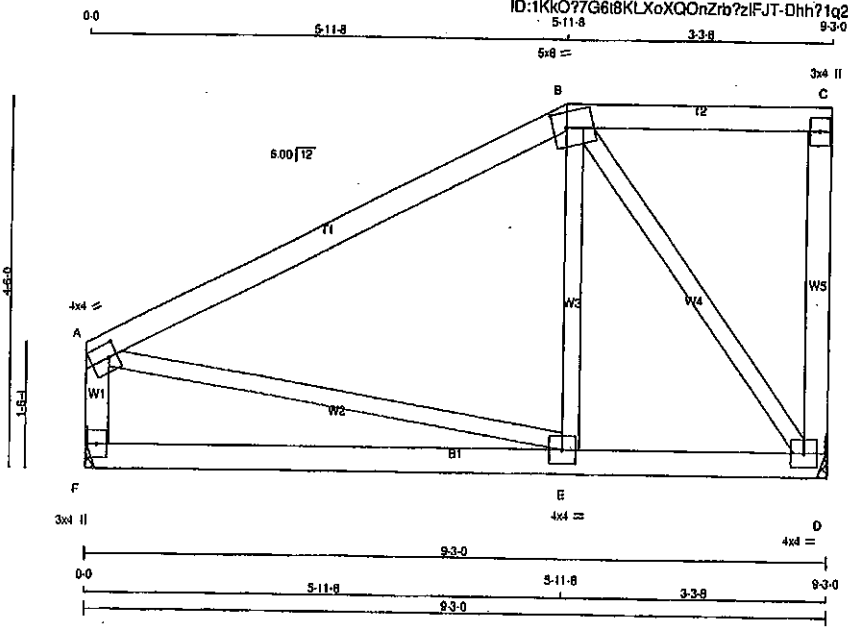

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TTWW-m	MT20	5.0	6.0	2.25	2.00
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	4.0	4.0		
E	BMWW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	510	0	510	0	0	IN-SX	MECHANICAL
F	510	0	510	0	0		MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D, F. MINIMUM BEARING LENGTH AT JOINT D = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	361	237 / 0	0 / 0	0 / 0	0 / 0	124 0	0 / 0
F	361	237 0	0 / 0	0 / 0	0 0	124 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (1)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	-303 / 0	-91.8	-91.8	0.42 (1)	6.25	E-B	0 / 108	0.04 (4)
B-C	0 / 0	-91.8	-91.8	0.17 (1)	10.00	B-D	-441 / 0	0.19 (1)
D-C	-151 / 0	0.0	0.0	0.05 (1)	7.81	A-E	0 / 276	0.06 (1)
F-A	-463 / 0	0.0	0.0	0.05 (1)	7.81			
F-E	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
E-D	0 272	-18.5	-18.5	0.17 (4)	10.00			

TOTAL WEIGHT = 40 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.42/1.00 (A-B:1) , BC=0.17/1.00 (D-E:4) , WB=0.19/1.00 (B-D:1) , SSI=0.19/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.48 (A) (INPUT = 0.90)
SI METAL= 0.18 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/20/20
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006412

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408206	T34	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack RoofTruss, Burlington

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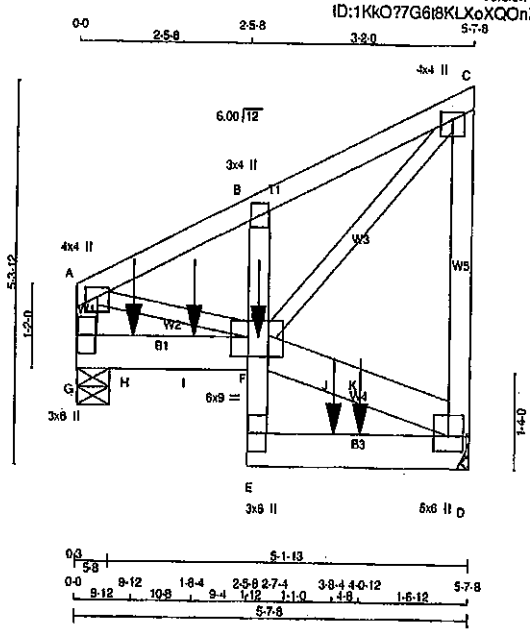

TOTAL WEIGHT = 2 X 37 = 74 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-D	1	12	TOP
G-A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
G-F	2	12	SIDE(61.0)
E-D	2	12	SIDE(0.0)
B-E	1	12	SIDE(72.6)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x6	2	8	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (tablals in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B	TMV+p	MT20	3.0	4.0		
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	BMVW1+p	MT20	5.0	8.0	2.50	2.50
E	BMV+p	MT20	3.0	6.0		
F	BVMWWW-I	MT20	8.0	9.0	3.25	3.00
G	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	1481	0	1481	0	0	MECHANICAL	
G	1941	0	1941	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1043	709 / 0	0 / 0	0 / 0	0 / 0	333 / 0	0 / 0
G	1365	941 / 0	0 / 0	0 / 0	0 / 0	423 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A-B	-1513 / 0	-91.8	-91.8	0.05 (1)	6.25
B-C	-1486 / 0	-91.8	-91.8	0.08 (1)	6.25
D-C	-1029 / 0	0.0	0.0	0.23 (1)	7.81
G-A	-1187 / 0	0.0	0.0	0.07 (1)	7.81
G-H	0 / 0	-18.5	-18.5	0.22 (1)	10.00
H-I	0 / 0	-18.5	-18.5	0.22 (1)	10.00
I-F	0 / 0	-18.5	-18.5	0.22 (1)	10.00
E-F	0 / 562	0.0	0.0	0.12 (1)	10.00
F-B	-257 / 0	0.0	0.0	0.07 (1)	7.81
E-J	0 / 67	-18.5	-18.5	0.24 (1)	10.00
J-K	0 / 67	-18.5	-18.5	0.24 (1)	10.00
K-D	0 / 67	-18.5	-18.5	0.24 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-D	-74 / 0	0.00 (1)
F-C	0 / 2029	0.25 (1)
A-F	0 / 1418	0.18 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE	HEEL	COMM
F	2-7-4	-512	-512	---	FRONT	VERT	TOTAL		C1
H	9-12	-507	-507	---	FRONT	VERT	TOTAL		C1
H	9-12	-240	-240	---	TOP	VERT	TOTAL		C1
I	1-8-4	-615	-615	---	BACK	VERT	TOTAL		C1
J	3-8-4	-436	-436	---	BACK	VERT	TOTAL		C1
K	4-0-12	-491	-491	---	FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.23/1.00 (C-D:1) , BC=0.24/1.00 (D-E:1) , WB=0.25/1.00 (C-F:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.88 (C) (INPUT = 0.90)
SI METAL= 0.33 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/20/20
H. J. C. ALVES
100090024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006413

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408206	T35	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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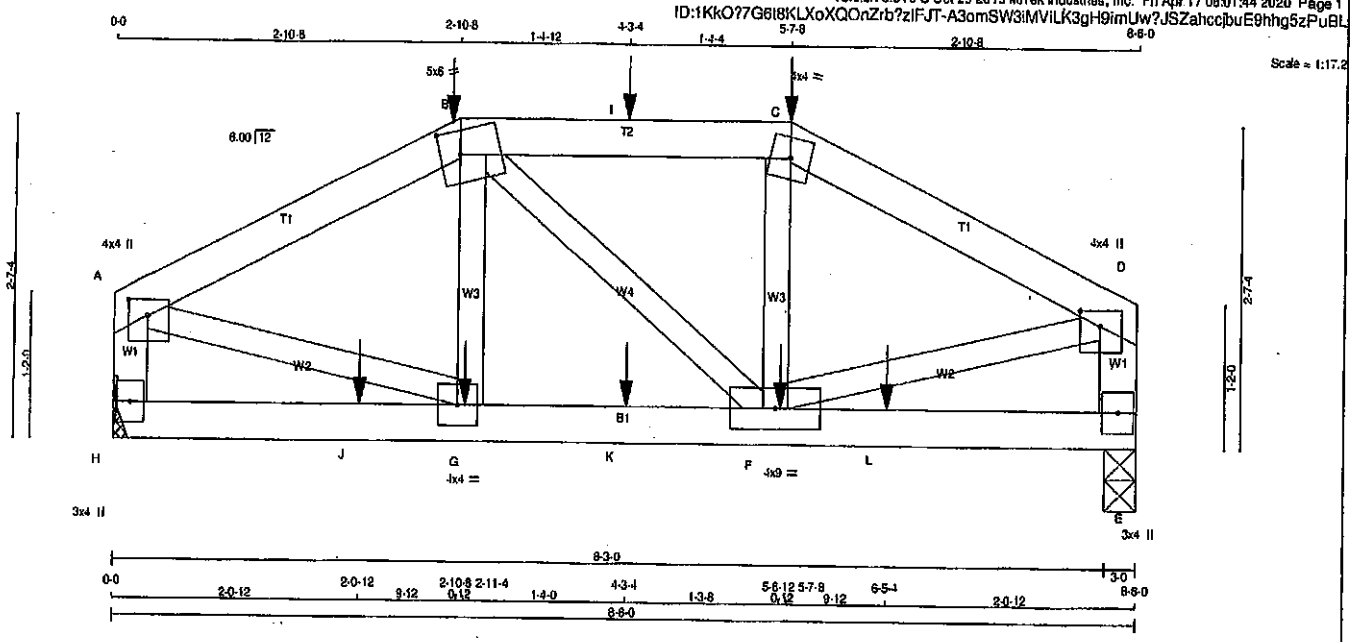

TOTAL WEIGHT = 32 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B	TTWW-m	MT20	5.0	6.0	2.25	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.50	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	633	0	633	0	0	MECHANICAL	
E	633	0	633	0	0	3-0	3-0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	447	299 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0
E	447	299 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-672 / 0	-91.8	-91.8	0.15 (1)		6.25
B-I	-596 / 0	-91.8	-91.8	0.14 (1)		6.25
I-C	-596 / 0	-91.8	-91.8	0.14 (1)		6.25
C-D	-677 / 0	-91.8	-91.8	0.15 (1)		6.25
H-A	-606 / 0	0.0	0.0	0.07 (1)		7.81
E-D	-605 / 0	0.0	0.0	0.07 (1)		7.81
H-J	0 / 0	-18.5	-18.5	0.04 (4)		10.00
J-G	0 / 0	-18.5	-18.5	0.04 (4)		10.00
G-K	0 595	-18.5	-18.5	0.12 (1)		10.00
K-F	0 595	-18.5	-18.5	0.12 (1)		10.00
F-L	0 0	-18.5	-18.5	0.04 (4)		10.00
L-E	0 0	-18.5	-18.5	0.04 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-B	-110 / 15	0.02 (1)
B-F	0 / 2	0.00 (4)
F-C	-107 / 17	0.02 (1)
A-G	0 / 624	0.15 (1)
F-D	0 / 628	0.16 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
B	2-10-8	-149	-149	---	BACK	VERT	TOTAL
C	5-7-8	-149	-149	---	BACK	VERT	TOTAL
F	5-8-12	-5	-5	---	BACK	VERT	TOTAL
G	2-11-4	-5	-5	---	BACK	VERT	TOTAL
I	4-3-4	-7	-7	---	BACK	VERT	TOTAL
J	2-0-12	-5	-5	---	BACK	VERT	TOTAL
K	4-3-4	-5	-5	---	BACK	VERT	TOTAL
L	6-5-4	-5	-5	---	BACK	VERT	TOTAL

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.15/1.00 (C-D:1) , BC=0.12/1.00 (F-G:1) , WB=0.16/1.00 (D-F:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PSI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	786	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (G) (INPUT = 0.90)
JSI METAL= 0.22 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/20/20
H. G. ALLVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006414

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408206	T36A	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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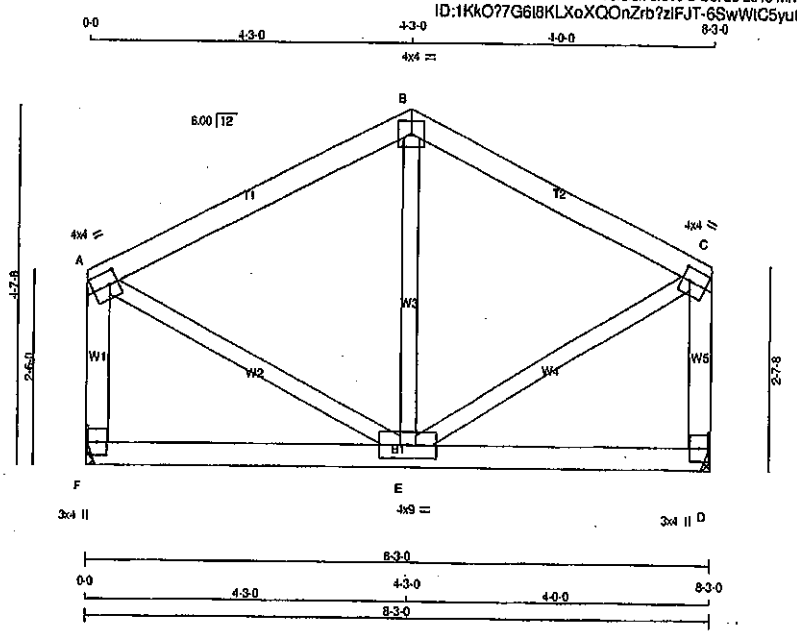


TOTAL WEIGHT = 35 lb
(M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TTW-p	MT20	4.0	4.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	455	0	455	0	0	MECHANICAL	
D	455	0	455	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	322	211 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
D	322	211 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A-B	-252 / 0	-91.8	-91.8	0.21 (1)		6.25	E-B	-153 / 14	0.05 (1)
B-C	-252 / 0	-91.8	-91.8	0.19 (1)		6.25	A-E	0 / 253	0.08 (1)
F-A	-424 / 0	0.0	0.0	0.06 (1)		7.81	E-C	0 / 260	0.06 (1)
D-C	-427 / 0	0.0	0.0	0.06 (1)		7.81			
F-E	0 / 0	-18.5	-18.5	0.09 (4)		10.00			
E-D	0 / 0	-18.5	-18.5	0.09 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.21/1.00 (A-B:1) , BC=0.09/1.00 (E-F:4) , WB=0.08/1.00 (C-E:1) , SSI=0.14/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

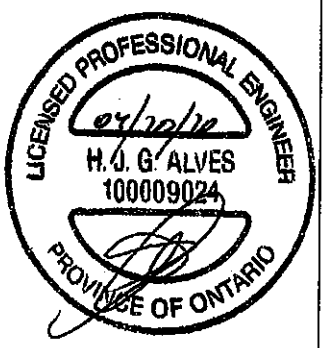
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (C) (INPUT = 0.90)
JSI METAL= 0.13 (A) (INPUT = 1.00)



Structural component only
DWG# T-2006415

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408206	T37	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Pool Truss, Burlington

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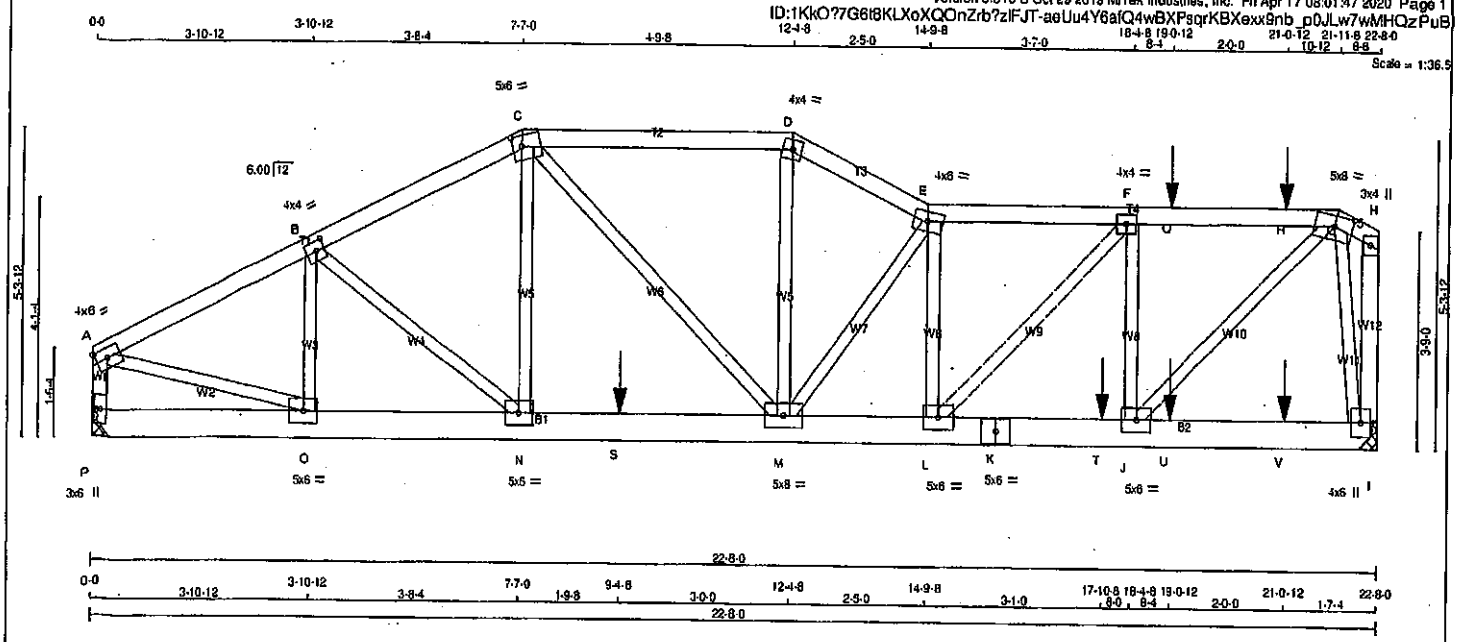

TOTAL WEIGHT = 2 X 114 = 227 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESGR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
P - A	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
P - K	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-D	1	12	TOP
D-E	1	12	TOP
E-G	1	12	SIDE(0.0)
G-H	1	12	TOP
P-A	1	12	TOP
I-H	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
P-K	2	12	SIDE(0.0)
K-I	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	8.0		Edge
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TTWW-m	MT20	4.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	8.0	2.25	3.75
H	TMV+p	MT20	3.0	4.0		
I	BMW1+p	MT20	4.0	6.0		
J, L, N, O						
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
P	2310	0	2310	0	0	MECHANICAL	
I	2740	0	2740	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT P. I. MINIMUM BEARING LENGTH AT JOINT P = 4-0, JOINT I = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1629	1093 / 0	0 / 0	0 / 0	0 / 0	537 / 0	0 / 0
I	1933	1292 / 0	0 / 0	0 / 0	0 / 0	641 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A-B	-2924 / 0	-91.8	-91.8	0.14 (1)	5.26
B-C	-3405 / 0	-91.8	-91.8	0.15 (1)	4.94
C-D	-3302 / 0	-91.8	-91.8	0.26 (1)	4.87
D-E	-3661 / 0	-91.8	-91.8	0.09 (1)	4.85
E-F	-3742 / 0	-91.8	-91.8	0.18 (1)	4.72
F-Q	-2832 / 0	-91.8	-91.8	0.16 (1)	5.29
Q-R	-2832 / 0	-91.8	-91.8	0.16 (1)	5.29
R-G	-2832 / 0	-91.8	-91.8	0.16 (1)	5.29
G-H	0 / 0	-91.8	-91.8	0.00 (1)	10.00
P-A	-2229 / 0	0.0	0.0	0.12 (1)	7.48
I-H	-34 / 0	0.0	0.0	0.00 (1)	7.81
P-O	0 / 0	-18.5	-18.5	0.08 (1)	10.00
O-N	0 / 2629	-18.5	-18.5	0.38 (1)	10.00
N-S	0 / 3047	-18.5	-18.5	0.59 (1)	10.00
S-M	0 / 3047	-18.5	-18.5	0.59 (1)	10.00
M-L	0 / 3774	-18.5	-18.5	0.42 (1)	10.00
L-K	0 / 2832	-18.5	-18.5	0.28 (1)	10.00
K-T	0 / 2832	-18.5	-18.5	0.28 (1)	10.00
T-J	0 / 2832	-18.5	-18.5	0.28 (1)	10.00
J-U	0 / 533	-18.5	-18.5	0.08 (1)	10.00
U-V	0 / 533	-18.5	-18.5	0.08 (1)	10.00
V-I	0 / 533	-18.5	-18.5	0.08 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
O-B	-950 / 0	0.10 (1)
B-N	0 / 515	0.06 (1)
N-C	0 / 901	0.11 (1)
C-M	0 / 369	0.05 (1)
M-D	0 / 1307	0.16 (1)
M-E	-892 / 0	0.15 (1)
L-E	-1137 / 0	0.14 (1)
L-F	0 / 1335	0.17 (1)
J-F	-1504 / 0	0.19 (1)
J-G	0 / 3335	0.41 (1)
A-O	0 / 2737	0.34 (1)
G-I	-2728 / 0	0.35 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
Q	19-0-12	-110	-110	---	BACK	VERT	TOTAL
R	21-0-12	-117	-117	---	BACK	VERT	TOTAL
S	9-4-8	-1462	-1462	---	BACK	VERT	TOTAL
T	17-10-8	-808	-808	---	BACK	VERT	TOTAL
U	19-0-12	-26	-26	---	BACK	VERT	TOTAL
V	21-0-12	-26	-26	---	BACK	VERT	TOTAL

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.28/1.00 (C-D:1) , BC=0.59/1.00 (M-N:1) , WB=0.41/1.00 (G-J:1) , SSI=0.33/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.45 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H.J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006416

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408206	T38	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Flod Truss, Burlington

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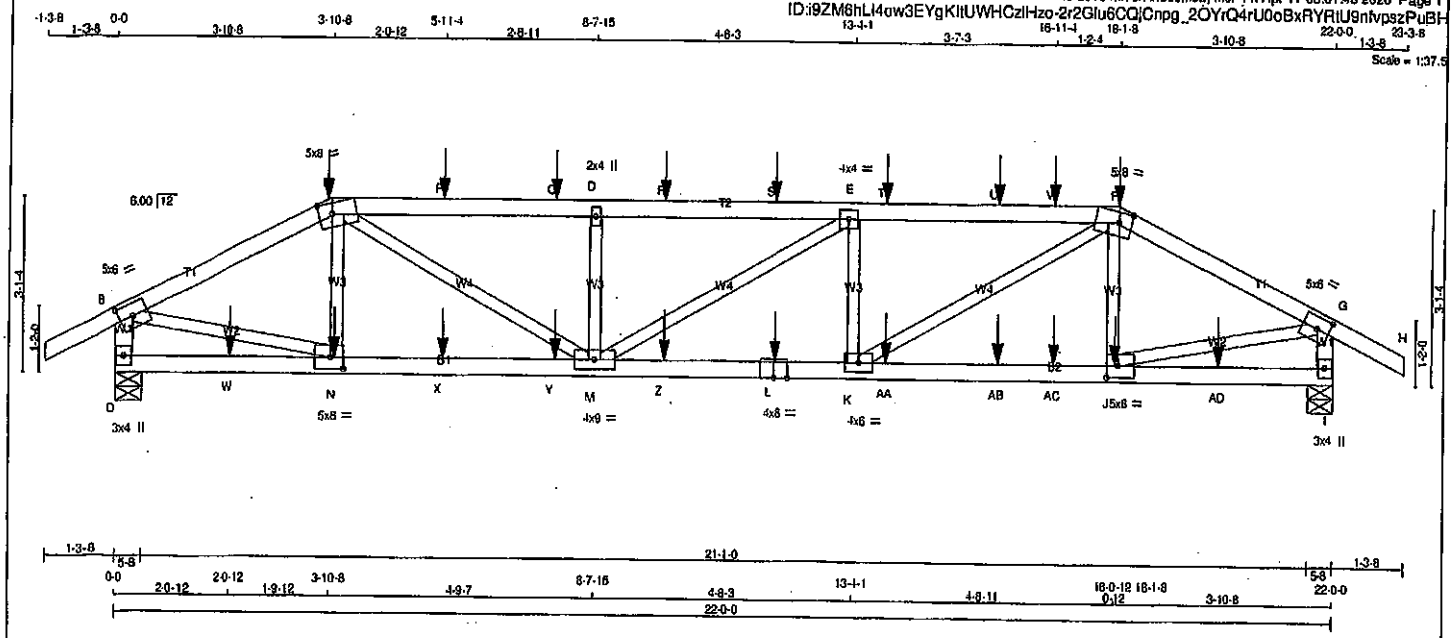


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TTWW-m	MT20	5.0	8.0	2.25	2.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	5.0	8.0	2.25	2.75
G	TMVW-t	MT20	5.0	6.0	2.50	2.75
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0	2.50	2.50
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	4.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.75
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1784	0	1784	0	0	5-8	5-8
I	1811	0	1811	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1259	842 / 0	0 / 0	0 / 0	0 / 0	416 / 0	0 / 0
I	1278	854 / 0	0 / 0	0 / 0	0 / 0	424 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.13 (1)		10.00
B-C	-2257 / 0	-91.8	-91.8	0.36 (1)		4.16
C-P	-3188 / 0	-91.8	-91.8	0.55 (1)		3.36
P-Q	-3188 / 0	-91.8	-91.8	0.55 (1)		3.36
Q-D	-3188 / 0	-91.8	-91.8	0.55 (1)		3.36
D-R	-3188 / 0	-91.8	-91.8	0.57 (1)		3.32
R-S	-3188 / 0	-91.8	-91.8	0.57 (1)		3.32
S-E	-3188 / 0	-91.8	-91.8	0.57 (1)		3.32
E-T	-3215 / 0	-91.8	-91.8	0.58 (1)		3.30
T-U	-3215 / 0	-91.8	-91.8	0.58 (1)		3.30
U-V	-3215 / 0	-91.8	-91.8	0.58 (1)		3.30
V-F	-3215 / 0	-91.8	-91.8	0.58 (1)		3.30
F-G	-2300 / 0	-91.8	-91.8	0.36 (1)		4.12
G-H	0 / 28	-91.8	-91.8	0.13 (1)		10.00
O-B	-1760 / 0	0.0	0.0	0.19 (1)		6.26
I-G	-1778 / 0	0.0	0.0	0.20 (1)		6.22
O-W	0 / 0	-18.5	-18.5	0.11 (4)		10.00
W-N	0 / 0	-18.5	-18.5	0.11 (4)		10.00
N-X	0 / 2005	-18.5	-18.5	0.42 (1)		10.00
X-Y	0 / 2005	-18.5	-18.5	0.42 (1)		10.00
Y-M	0 / 2005	-18.5	-18.5	0.42 (1)		10.00
M-Z	0 / 3215	-18.5	-18.5	0.64 (1)		10.00
Z-L	0 / 3215	-18.5	-18.5	0.64 (1)		10.00
L-K	0 / 3215	-18.5	-18.5	0.64 (1)		10.00
K-AA	0 / 2044	-18.5	-18.5	0.43 (1)		10.00
AA-AB	0 / 2044	-18.5	-18.5	0.43 (1)		10.00
AB-AC	0 / 2044	-18.5	-18.5	0.43 (1)		10.00
AC-J	0 / 2044	-18.5	-18.5	0.43 (1)		10.00
J-AD	0 / 0	-18.5	-18.5	0.11 (4)		10.00
AD-I	0 / 0	-18.5	-18.5	0.11 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	-315 / 0	0.06 (1)
C-M	0 / 1371	0.34 (1)
M-D	-589 / 0	0.11 (1)
M-E	-32 / 0	0.02 (1)
K-E	-572 / 0	0.11 (1)
K-F	0 / 1357	0.34 (1)
J-F	-315 / 0	0.06 (1)
B-N	0 / 2063	0.51 (1)
J-G	0 / 2103	0.52 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-248	-248	---	FRONT	VERT	TOTAL	--	C1
F	18-1-8	-248	-248	---	FRONT	VERT	TOTAL	--	C1
J	18-0-12	-12	-12	---	FRONT	VERT	TOTAL	--	C1
L	11-11-4	-12	-12	---	FRONT	VERT	TOTAL	--	C1
N	3-11-4	-12	-12	---	FRONT	VERT	TOTAL	--	C1
P	5-11-4	-42	-42	---	FRONT	VERT	TOTAL	--	C1
Q	7-11-4	-42	-42	---	FRONT	VERT	TOTAL	--	C1
R	9-11-4	-42	-42	---	FRONT	VERT	TOTAL	--	C1
S	11-11-4	-42	-42	---	FRONT	VERT	TOTAL	--	C1

TOTAL WEIGHT = 83 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.58/1.00 (E-F:1) , BC=0.64/1.00 (K-M:1) , WB=0.52/1.00 (G-J:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.74 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/17/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only 1/2
DWG# T-2006417

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408206	T38	1	1	GREEN PARK HOMES TRUSS DESC.	

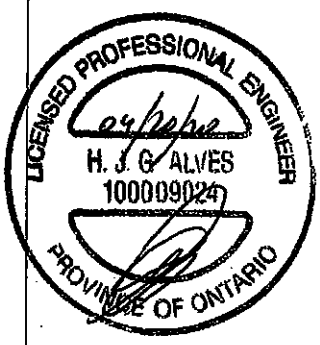
Tamarack Roel Truss, Burlington

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	13-11-4	-42	-42	---	FRONT	VERT	TOTAL	---	C1
U	15-11-4	-42	-42	---	FRONT	VERT	TOTAL	---	C1
V	16-11-4	-42	-42	---	FRONT	VERT	TOTAL	---	C1
W	2-0-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1
X	5-11-4	-12	-12	---	FRONT	VERT	TOTAL	---	C1
Y	7-11-4	-12	-12	---	FRONT	VERT	TOTAL	---	C1
Z	9-11-4	-12	-12	---	FRONT	VERT	TOTAL	---	C1
AA	13-11-4	-12	-12	---	FRONT	VERT	TOTAL	---	C1
AB	15-11-4	-12	-12	---	FRONT	VERT	TOTAL	---	C1
AC	16-11-4	-12	-12	---	FRONT	VERT	TOTAL	---	C1
AD	19-11-4	-12	-12	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2006417

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408206	T39	1	3	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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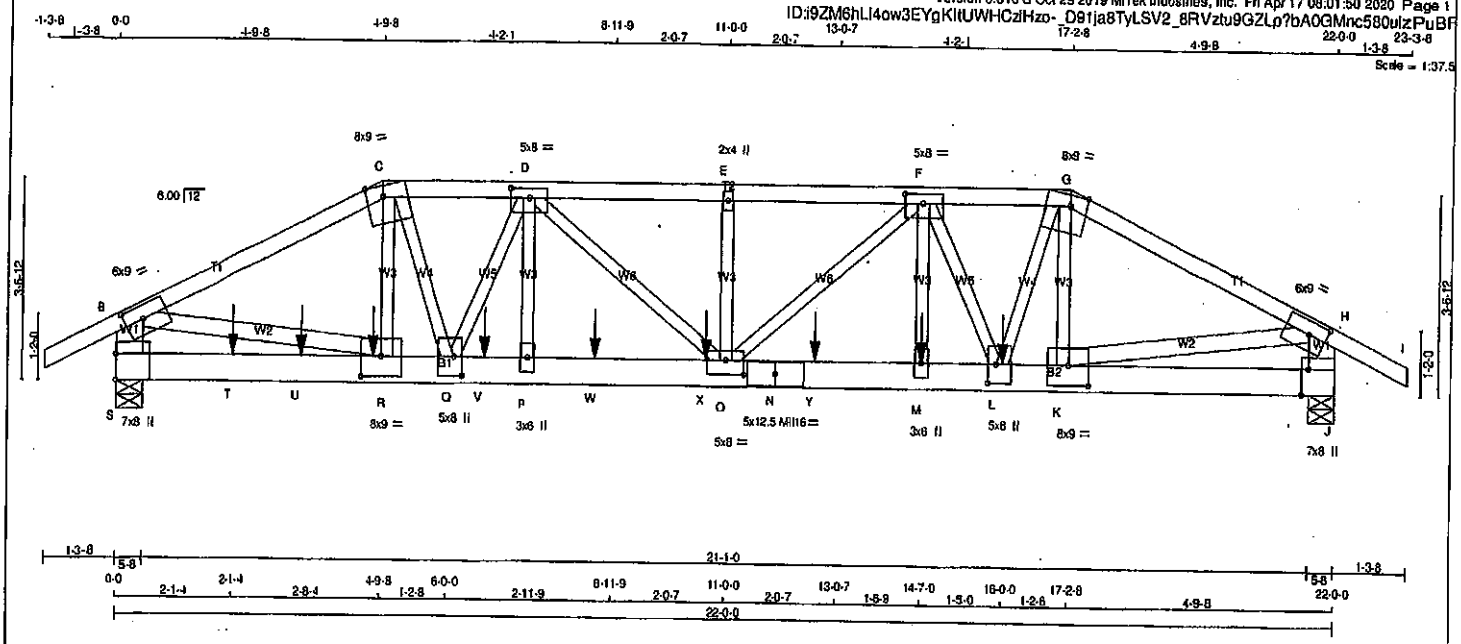

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
S - N	2x6	DRY	1650F 1.5E	SPF
N - J	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - R	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A-C 1	12	TOP
C-G 1	12	TOP
G-I 1	12	TOP
S-B 2	12	TOP
J-H 2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
S-N 2	5	SIDE(1246.0)
N-J 2	5	SIDE(1098.1)
WEBS : (0.122"X3") SPIRAL NAILS		
2x3 1	6	
2x4 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	3.75
C	TTWW-m	MT20	8.0	9.0	Edge	3.75
D	TMWWW-t	MT20	5.0	8.0	2.00	4.00
E	TMW+w	MT20	2.0	4.0		
F	TMWWW-t	MT20	5.0	8.0	2.00	4.00
G	TTWW-m	MT20	8.0	9.0	Edge	3.75
H	TMVW-t	MT20	6.0	9.0	2.75	3.75
J	BMV1+t	MT20	7.0	8.0	Edge	1.50
K	BMWW-t	MT20	8.0	9.0	4.25	4.50
L	BMWW+t	MT20	5.0	8.0	4.00	1.75
M	BMW+w	MT20	3.0	6.0		
N	BS-t	MII16	5.0	12.5		
O	BMWWW-t	MT20	5.0	8.0	3.00	4.00
P	BMW+w	MT20	3.0	6.0		
Q	BMWW-t	MT20	5.0	8.0	4.00	1.75
R	BMWW-t	MT20	8.0	9.0	4.25	4.50
S	BMV1+t	MT20	7.0	8.0	5.50	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	11923	0	11923	0	0	5-8	5-8
J	9234	0	9234	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE — MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	8406	5661 / 0	0 / 0	0 / 0	0 0	2745 / 0	0 0
J	6510	4385 / 0	0 / 0	0 / 0	0 0	2125 / 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.04 (1)	10.00	
B-C	-17353 / 0	-91.8	-91.8	0.68 (1)	2.56	
C-D	-17212 / 0	-91.8	-91.8	0.38 (1)	2.69	
D-E	-21376 / 0	-91.8	-91.8	0.67 (1)	2.12	
E-F	-21376 / 0	-91.8	-91.8	0.68 (1)	2.11	
F-G	-16518 / 0	-91.8	-91.8	0.36 (1)	2.76	
G-H	-14988 / 0	-91.8	-91.8	0.54 (1)	2.87	
H-I	0 / 28	-91.8	-91.8	0.04 (1)	10.00	
S-B	-10581 / 0	0.0	0.0	0.23 (1)	5.68	
J-H	-9184 / 0	0.0	0.0	0.20 (1)	6.03	
S-T	0 / 0	-18.5	-18.5	0.43 (1)	10.00	
T-U	0 / 0	-18.5	-18.5	0.43 (1)	10.00	
U-R	0 / 0	-18.5	-18.5	0.43 (1)	10.00	
R-Q	0 15823	-18.5	-18.5	0.67 (1)	10.00	
Q-V	0 19150	-18.5	-18.5	0.54 (1)	10.00	
V-P	0 / 19150	-18.5	-18.5	0.54 (1)	10.00	
P-W	0 / 19150	-18.5	-18.5	0.68 (1)	10.00	
W-X	0 19150	-18.5	-18.5	0.68 (1)	10.00	
X-O	0 / 19150	-18.5	-18.5	0.68 (1)	10.00	
O-N	0 / 19034	-18.5	-18.5	0.69 (1)	10.00	
N-Y	0 / 19034	-18.5	-18.5	0.69 (1)	10.00	
Y-M	0 / 19034	-18.5	-18.5	0.69 (1)	10.00	
M-L	0 / 19034	-18.5	-18.5	0.57 (1)	10.00	
L-K	0 / 13343	-18.5	-18.5	0.42 (1)	10.00	
K-J	0 0	-18.5	-18.5	0.01 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-C	0 / 3451	0.26 (1)
P-D	0 / 2362	0.18 (1)
M-F	0 / 3653	0.27 (1)
K-G	-1560 / 0	0.10 (1)
B-R	0 / 15703	0.84 (1)
K-H	0 / 13561	0.73 (1)
D-O	0 / 2980	0.22 (1)
O-E	-303 / 0	0.02 (1)
O-F	0 / 3134	0.24 (1)
C-Q	0 / 4251	0.32 (1)
L-G	0 / 8494	0.64 (1)
Q-D	-4987 / 0	0.38 (1)
F-L	-6478 / 0	0.47 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.		
L	16-0-0	-2721	-2721	---	BACK	VERT	TOTAL	--- C1
M	14-7-0	-2671	-2671	---	BACK	VERT	TOTAL	--- C1
R	4-7-12	-1870	-1870	---	BACK	VERT	TOTAL	--- C1
T	2-1-4	-1870	-1870	---	BACK	VERT	TOTAL	--- C1
U	3-3-12	-1870	-1870	---	BACK	VERT	TOTAL	--- C1
V	6-7-12	-1870	-1870	---	TOP	VERT	TOTAL	--- C1
W	8-7-12	-1870	-1870	---	TOP	VERT	TOTAL	--- C1
X	10-7-12	-1870	-1870	---	TOP	VERT	TOTAL	--- C1
Y	12-7-12	-1870	-1870	---	TOP	VERT	TOTAL	--- C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 3 X 110 = 330 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 649 (0.41")

CSI: TC=0.68/1.00 (E-F:1) , BC=0.69/1.00 (M-O:1) , WB=0.84/1.00 (B-R:1) , SSI=0.50/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656
MII16	438	276	2341	1245	4283	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.97 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/14/20
H. J. G. ALVES
100090924
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006418

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408206	T40	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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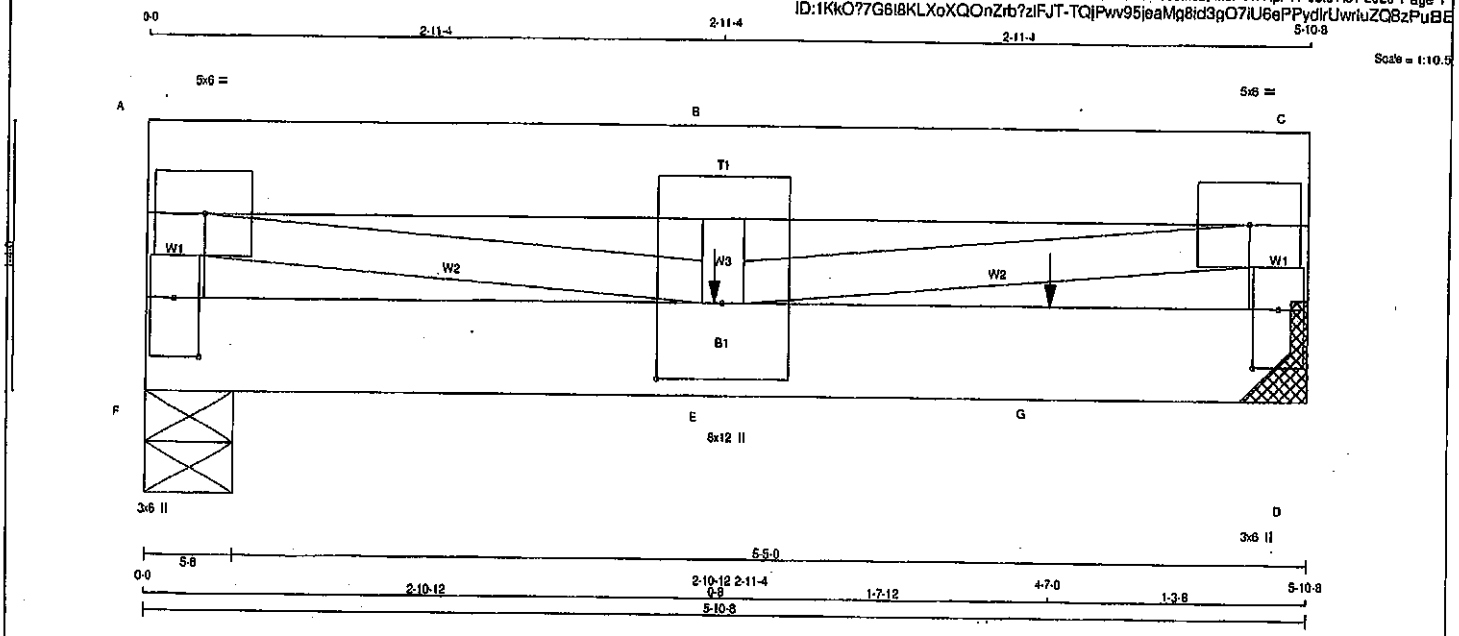

TOTAL WEIGHT = 2 X 27 = 53 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
C- D	1	12	TOP
A- C	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW-I	MT20	5.0	6.0		
B						
C	TMW-I	MT20	5.0	6.0		
D	BMV1+p	MT20	3.0	6.0	3.50	1.50
E						
E	TMGMWWW+HMT20		8.0	12.0	4.50	4.00
F	BMV1+p	MT20	3.0	6.0	3.50	1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1073	0	1073	0	0	5-8	5-8
D	2357	0	2357	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	757	509 / 0	0 / 0	0 / 0	0 / 0	248 0	0 0
D	1661	1121 / 0	0 / 0	0 0	0 0	540 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1086 / 0	0.0	0.0	0.08 (1)	7.81	A- E	0 / 2727	0.34 (1)
A- B	-2614 / 0	-91.8	-91.8	0.17 (1)	6.25	E- B	0 / 58	0.01 (4)
B- C	-2614 / 0	-91.8	-91.8	0.17 (1)	6.25	E- C	0 / 2727	0.34 (1)
D- C	-1086 / 0	0.0	0.0	0.08 (1)	7.81			
F- E	0 / 0	-18.5	-18.5	0.02 (1)	10.00			
E- G	0 / 0	-18.5	-18.5	0.61 (1)	10.00			
G- D	0 / 0	-18.5	-18.5	0.61 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-10-12	-491	-491	---	BACK	VERT	TOTAL	---	C1
G	4-7-0	-2291	-2291	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIR

LICENSED PROFESSIONAL ENGINEER
04/10/12
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.61/1.00 (D-E:1) , WB=0.34/1.00 (A-E:1) , SSI=0.43/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

MSI GRIP= 0.75 (E) (INPUT = 0.90)
MSI METAL= 0.33 (C) (INPUT = 1.00)

Structural component only
DWG# T-2006419

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408206	T41	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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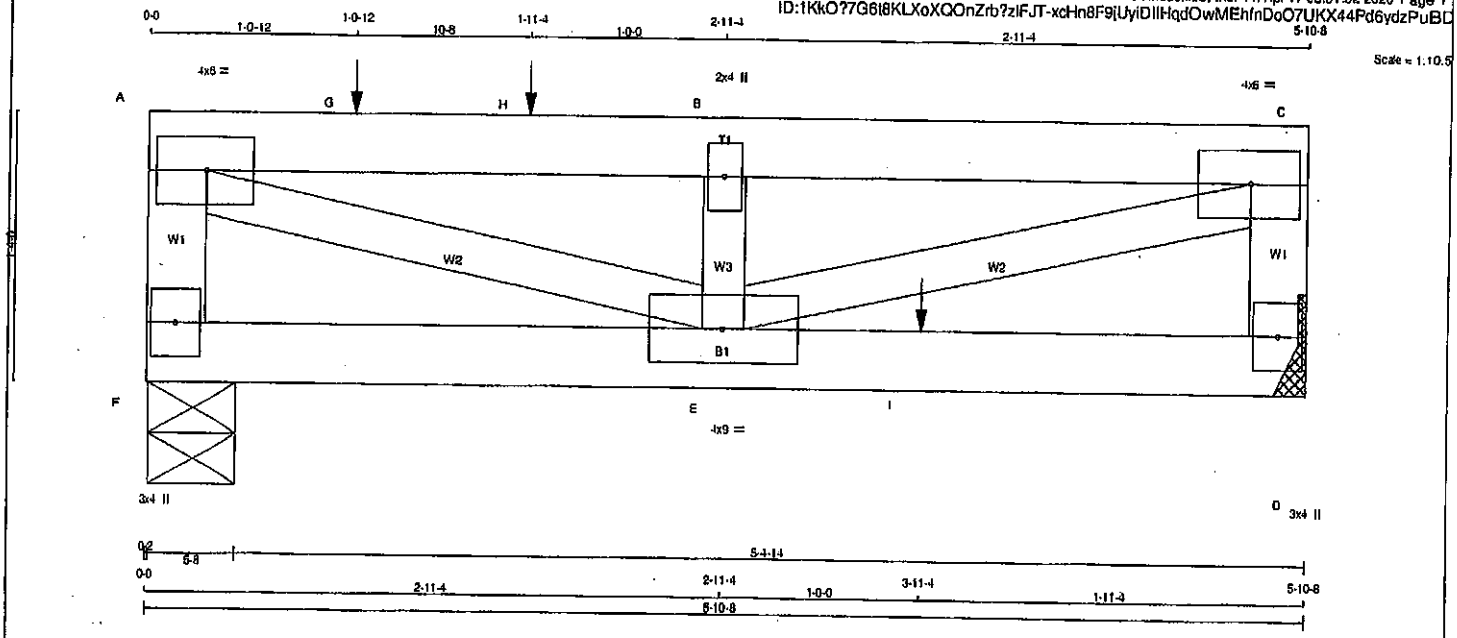


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F-A	1	12	TOP
A-C	1	12	TOP
C-D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F-D	1	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		
B	TMW+w	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	974	0	974	0	0	5-8	5-8
D	827	0	827	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	688	457 / 0	0 / 0	0 / 0	0 / 0	231 0	0 / 0
D	583	389 / 0	0 / 0	0 / 0	0 / 0	194 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
F-A	-963 / 0	0.0	0.0	0.05 (1)	7.81
A-G	-1684 / 0	-91.8	-91.8	0.30 (1)	6.20
G-H	-1684 / 0	-91.8	-91.8	0.30 (1)	6.20
H-B	-1684 / 0	-91.8	-91.8	0.30 (1)	6.20
B-C	-1684 / 0	-91.8	-91.8	0.16 (1)	6.25
D-C	-666 / 0	0.0	0.0	0.04 (1)	7.81
F-E	0 / 0	-18.5	-18.5	0.03 (1)	10.00
E-I	0 / 0	-18.5	-18.5	0.21 (1)	10.00
I-D	0 / 0	-18.5	-18.5	0.21 (1)	10.00

WEBS

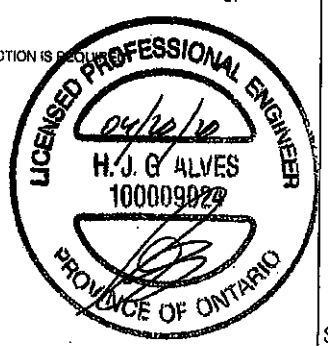
MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-E	0 / 787	0.22 (1)
E-B	-821 0	0.06 (1)
E-C	0 / 787	0.22 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-0-12	-176	-176	---	TOP	VERT	TOTAL	---	C1
H	1-11-4	-541	-541	---	TOP	VERT	TOTAL	---	C1
I	3-11-4	-436	-436	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED



TOTAL WEIGHT = 2 X 20 = 40 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.30/1.00 (A-B:1) , BC=0.21/1.00 (D-E:1) , WB=0.22/1.00 (A-E:1) , SSI=0.27/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (A) (INPUT = 0.90)
JSI METAL= 0.21 (C) (INPUT = 1.00)

Structural component only
DWG# T-2006420

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	J1	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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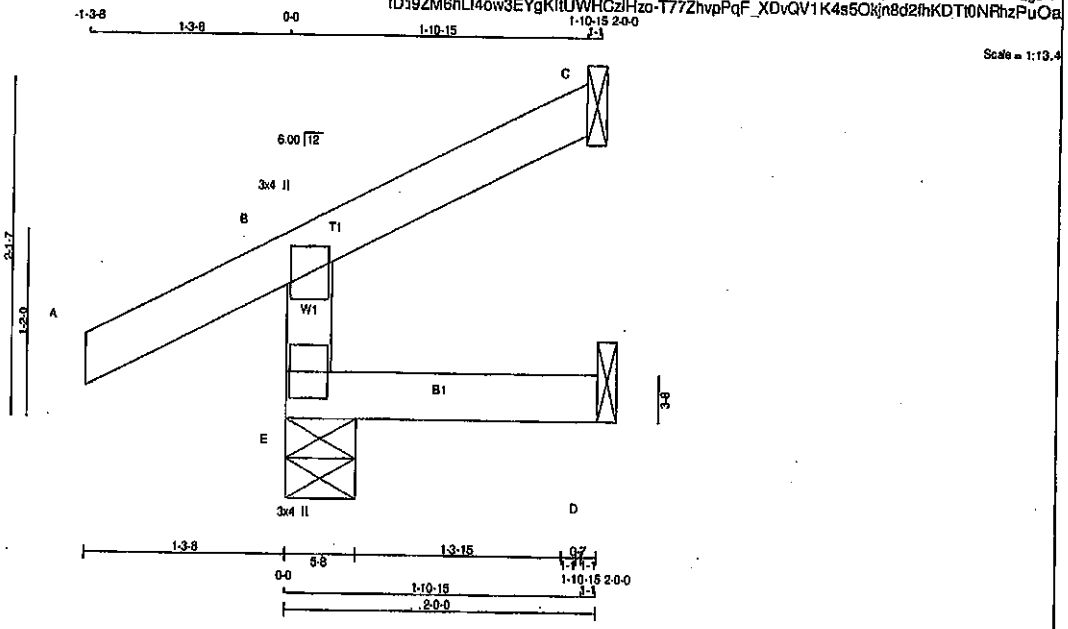


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	7MV+p	MT20	3.0	4.0		
E	8MV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG - IN-SX	REQRD BRG IN-SX
E	254	0	254	0	0	5-8	5-8
C	66	0	66	0	0	1-8	1-8
D	16	0	16	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	177	130 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	46	37 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

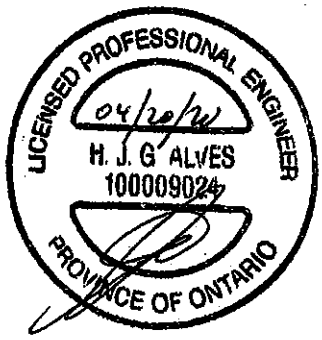
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-234 / 0	0.0	0.0	0	0.01 (4)	7.81			
A- B	0 / 26	-91.8	-91.8	0	0.12 (1)	10.00			
B- C	-10 / 0	-91.8	-91.8	0	0.06 (1)	10.00			
E- D	0 / 0	-16.5	-16.5	0	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



TOTAL WEIGHT = 3 X 7 = 22 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1997	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006370

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	J2	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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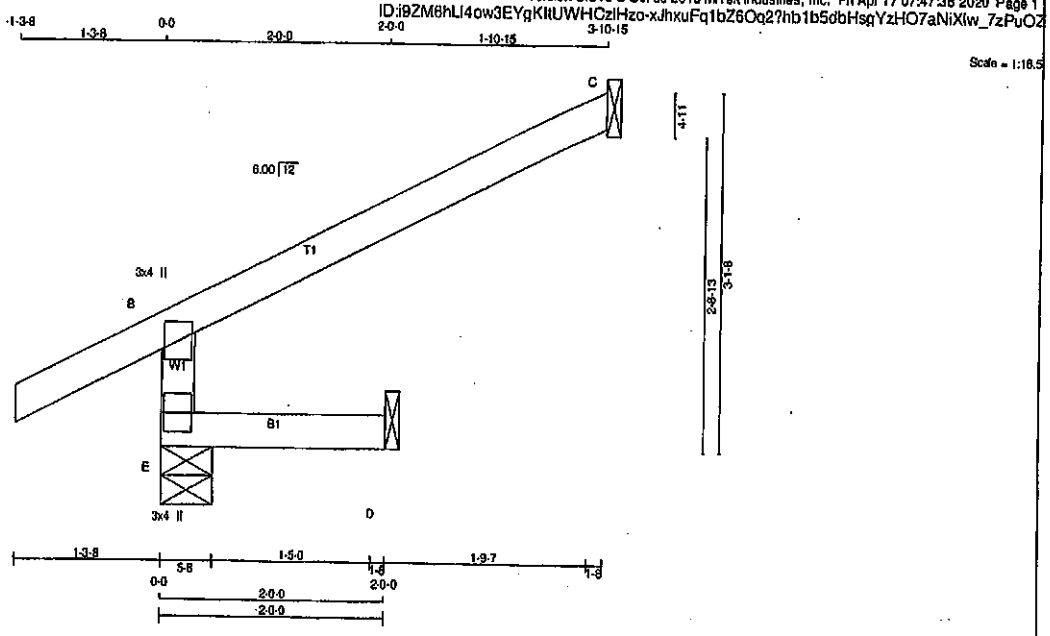

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	369	0	369	0	0	5-8	5-8
C	135	0	135	0	0	1-8	1-8
D	16	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	256	194 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	93	75 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS FR-TO	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-349 / 0	0.0	0.0	0.01 (4)		7.81			
A-B	0 / 28	-91.8	-91.8	0.13 (5)		10.00			
B-C	-20 / 0	-91.8	-91.8	0.24 (1)		6.25			
E-D	0 / 0	-18.5	-18.5	0.02 (4)		10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 10 = 30 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006371

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	J3	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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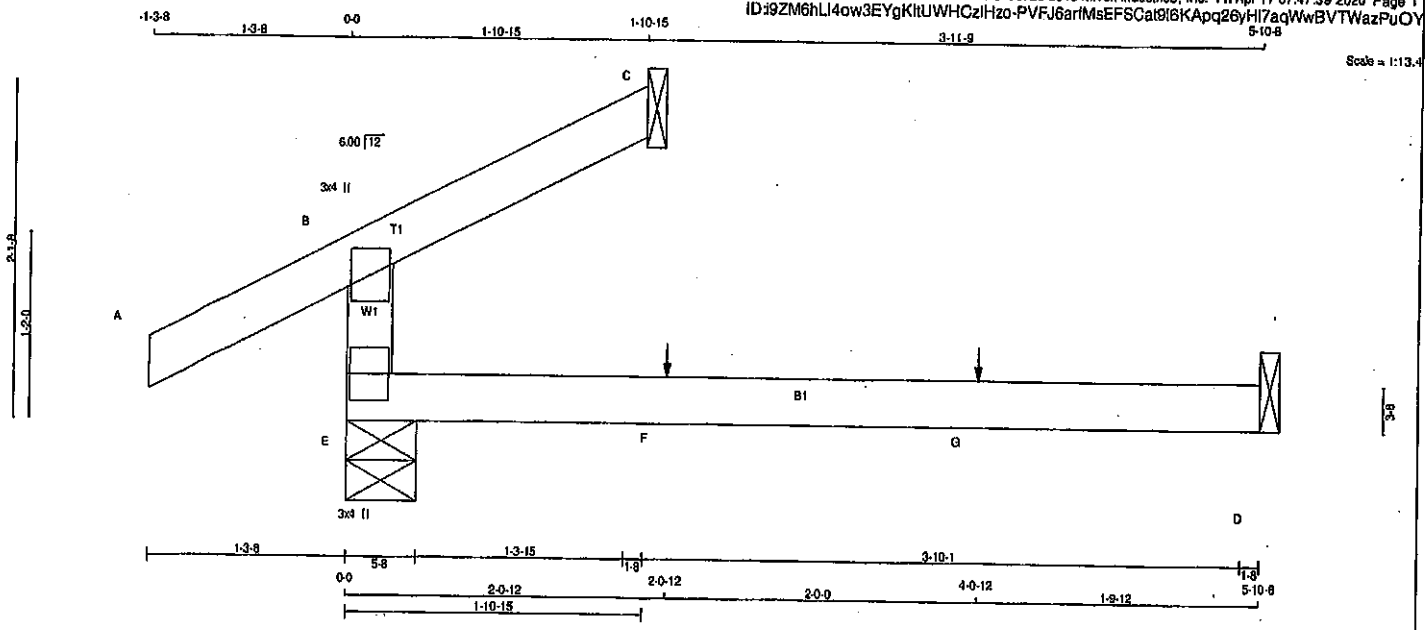


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	297	0	297	0	0	5-8	5-8
C	68	0	68	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	212	130 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
C	46	37 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

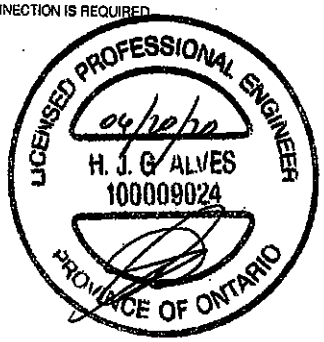
CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-234 / 0	0.0	0.0	0.13	(4)	7.81			
A-B	0 / 26	-91.8	-91.8	0.12	(1)	10.00			
B-C	-10 / 0	-91.8	-91.8	0.06	(1)	10.00			
E-F	0 / 0	-18.5	-18.5	0.13	(4)	10.00			
F-G	0 / 0	-18.5	-18.5	0.13	(4)	10.00			
G-D	0 / 0	-18.5	-18.5	0.13	(4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	1	1	---	BACK	VERT	TOTAL	---	C1
G	4-0-12	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



TOTAL WEIGHT = 3 X 12 = 35 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.13/1.00 (B-E:4) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006372

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	J4	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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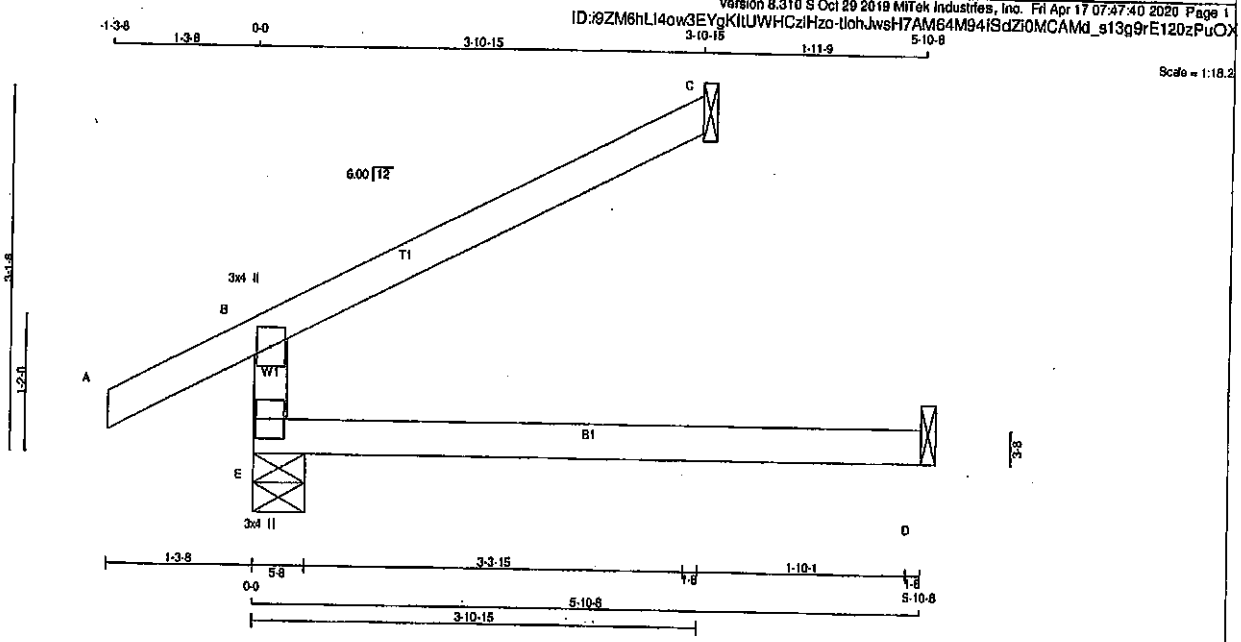

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	412	0	412	0	0	5-8	5-8
C	135	0	135	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	291	194 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
C	93	75 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-349 / 0	0.0	0.0	0.13 (4)		7.81			
A-B	0 / 28	-91.8	-91.8	0.12 (1)		10.00			
B-C	-20 / 0	-91.8	-91.8	0.24 (1)		6.25			
E-D	0 / 0	-18.5	-18.5	0.13 (4)		10.00			

TOTAL WEIGHT = 3 X 14 = 43 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/20/20
H.J. G ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2006373

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	J5	9	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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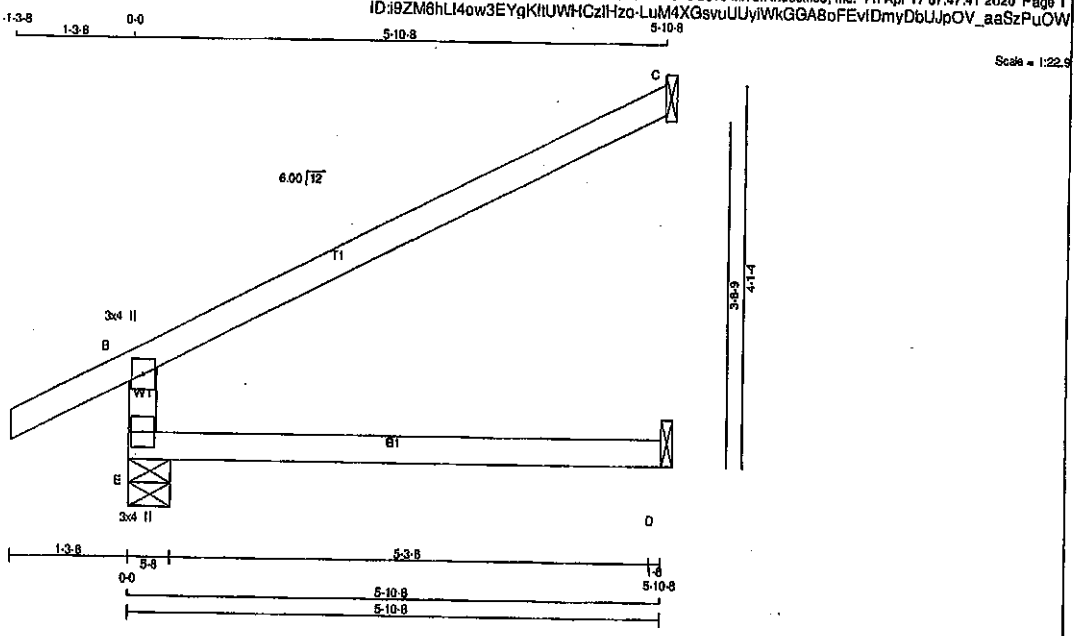


TOTAL WEIGHT = 9 X 17 = 151 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	525	0	525	0	0	5-8	5-8
C	202	0	202	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	369	257 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

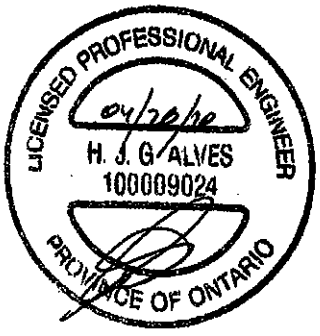
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-461 / 0	0.0	0.0	0.13 (4)	7.81			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-30 / 0	-91.8	-91.8	0.54 (1)	6.25			
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006374

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408204	U6	6	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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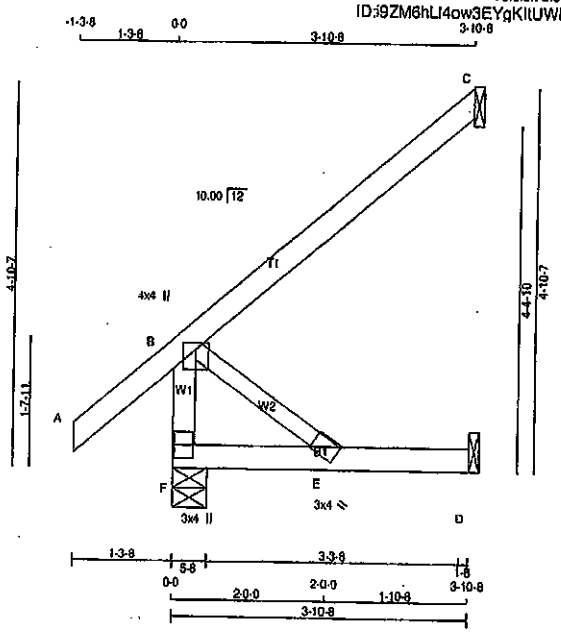


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	341	0	341	0	0	5-8	5-8
C	178	0	178	0	0	1-8	1-8
D	38	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	239	170 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 0
C	122	99 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

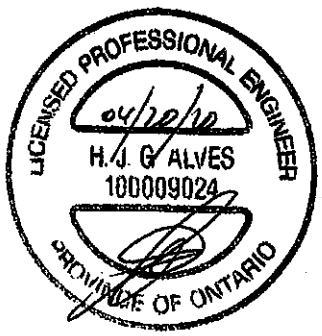
CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
F-B	-305 / 0	0.0	0.0	0.03	(1)		7.81
A-B	0 / 41	-91.8	-91.8	0.14	(5)		10.00
B-C	0 / 0	-91.8	-91.8	0.23	(1)		10.00
F-E	0 / 0	-18.5	-18.5	0.08	(4)		10.00
E-D	0 / 0	-18.5	-18.5	0.08	(4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
B-E	0 / 0	0.00	(1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



TOTAL WEIGHT = 6 X 16 = 93 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.08/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006375

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408205	J21	5	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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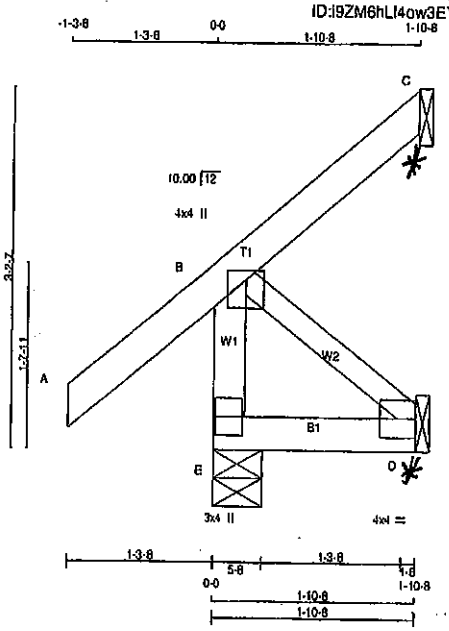


TOTAL WEIGHT = 5 X 10 = 51 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMW1-t	MT20	4.0	4.0	2.00	Edge
E	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	277	0	277	0	0	5-8	5-8
C	39	0	39	0	-37 ✓	1-8	1-8
D	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897781H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS
1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	192	145 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	27	22 / -28	0 / 0	0 . 0	0 . 0	5 . 0	0 . 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

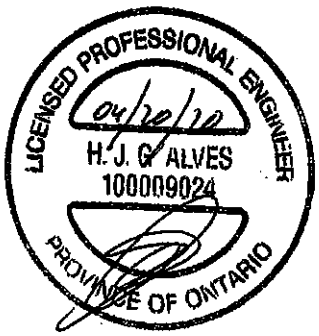
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-260 / 0	0.0	0.0	0.03 (1)	7.81	B-D	0 / 0	0.00 (1)
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00			
B-C	-30 / 0	-91.8	-91.8	0.12 (1)	6.25			
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.06/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006395

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408205	J22	5	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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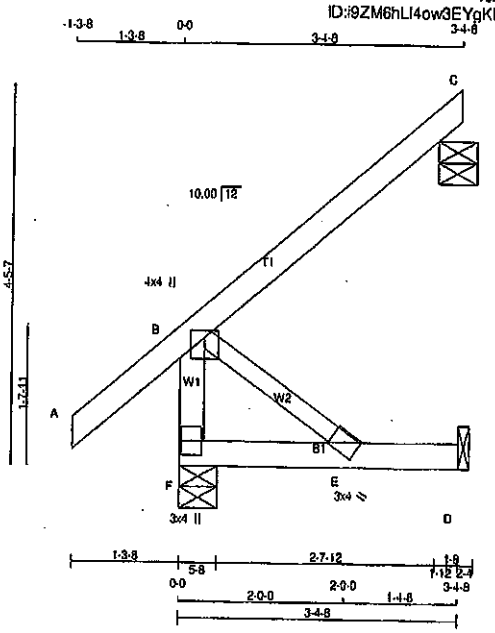


TOTAL WEIGHT = 5 X 14 = 71 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (tablels in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	313	0	313	0	0	5-8	5-8
C	155	0	155	0	0	5-8 (3-4)	5-8
D	31	0	35	0	0	1-8	1-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	219	157 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	107	86 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0
D	25	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

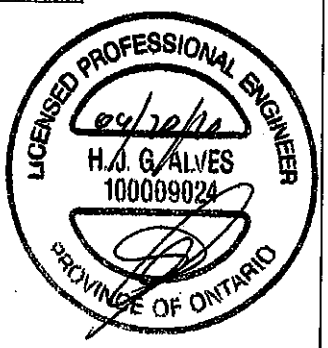
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-B	-282 / 0	0.0	0.0	0	0.03 (1)	7.81	B-E	0 0	0.00 (1)
A-B	0 / 41	-91.8	-91.8	0	0.14 (5)	10.00			
B-C	0 0	-91.8	-91.8	0	0.18 (1)	10.00			
F-E	0 0	-18.5	-18.5	0	0.06 (4)	10.00			
E-D	0 0	-18.5	-18.5	0	0.06 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.08/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL = 0.08 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006396

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408205	J23	5	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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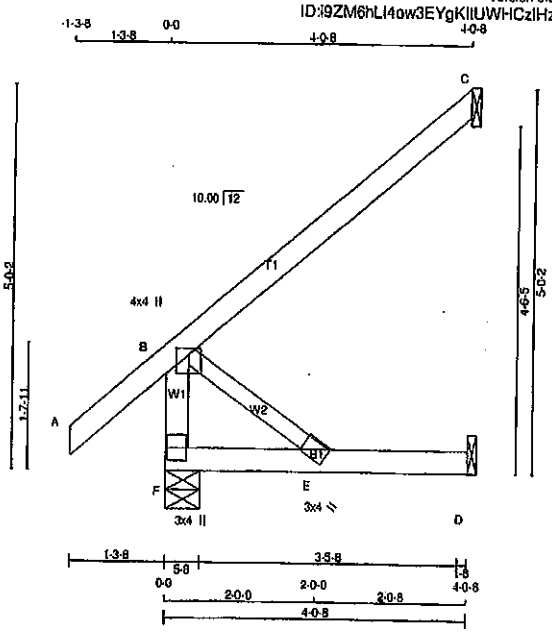


TOTAL WEIGHT = 5 X 16 = 80 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	350	0	350	0	0	5-8	5-8
C	185	0	185	0	0	1-8	1-8
D	37	0	42	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	245	174 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 0
C	128	103 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0
D	30	0 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

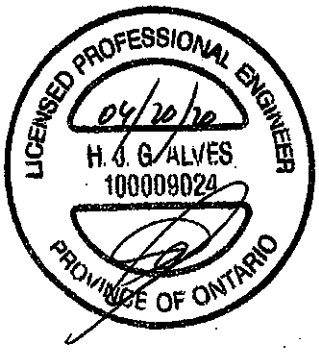
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-312 / 0	0.0	0.0	0.03 (1)	7.81	
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	
B- C	0 / 0	-91.8	-91.8	0.25 (1)	10.00	
F- E	0 / 0	-18.5	-18.5	0.09 (4)	10.00	
E- D	0 / 0	-18.5	-18.5	0.09 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.25/1.00 (B-C:1) , BC=0.09/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006397

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408206	J31	2	1	GREEN PARK HOMES TRUSS DESC.	

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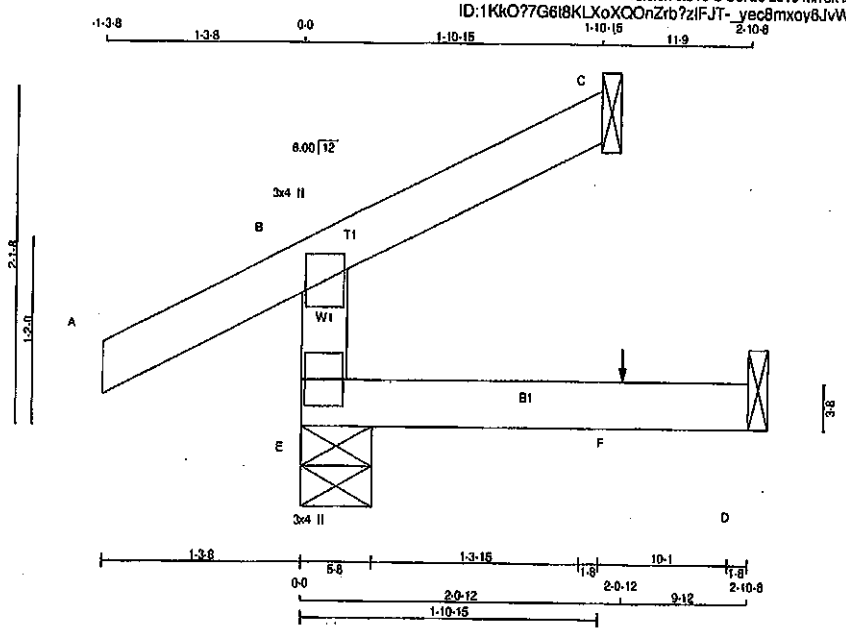


TOTAL WEIGHT = 2 X 8 = 17 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	264	0	264	0	0	5-8	5-8
C	66	0	66	0	0	1-8	1-8
D	23	0	26	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	185	130 / 0	0 / 0	0 / 0	0 / 0	55 0	0 0
C	46	37 / 0	0 / 0	0 / 0	0 / 0	9 0	0 0
D	18	0 / 0	0 / 0	0 / 0	0 / 0	18 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-234 / 0	0.0	0.0	4	0.02	7.81			
A-B	0 / 28	-91.8	-91.8	5	0.13	10.00			
B-C	-10 / 0	-91.8	-91.8	1	0.06	10.00			
E-F	0 / 0	-18.5	-18.5	4	0.03	10.00			
F-D	0 / 0	-18.5	-18.5	4	0.03	10.00			

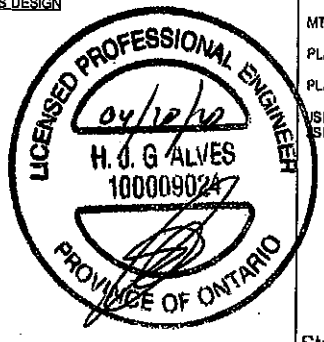
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.03/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.10/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.10 (E) (INPUT = 0.90)
USI METAL= 0.06 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006405

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408206	J32	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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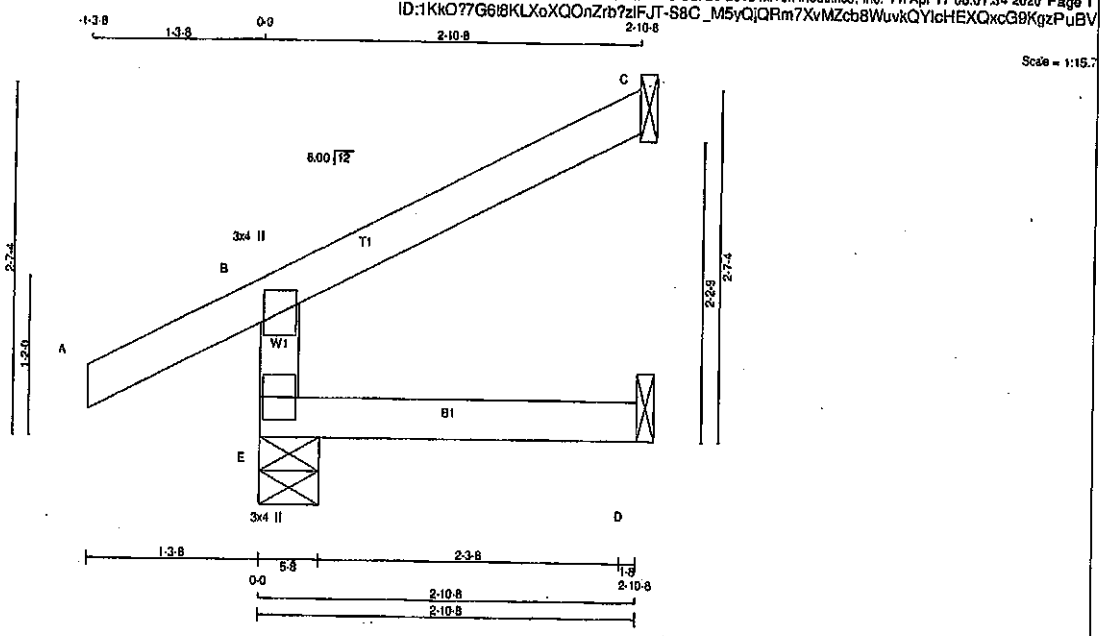


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	319	0	319	0	0	5-8	5-8
C	99	0	99	0	0	1-8	1-8
D	23	0	26	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	223	161 / 0	0 / 0	0 / 0	0 0	62 . 0	0 0
C	68	55 / 0	0 . 0	0 / 0	0 0	13 0	0 0
D	18	0 . 0	0 / 0	0 / 0	0 . 0	18 . 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

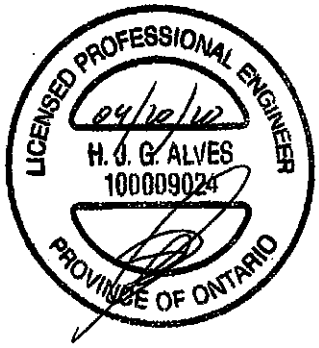
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-289 / 0	0.0	0.0	0.02 (4)		7.81			
A-B	0 / 28	-91.8	-91.8	0.13 (5)		10.00			
B-C	-15 / 0	-91.8	-91.8	0.13 (1)		6.25			
E-D	0 / 0	-18.5	-18.6	0.03 (4)		10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



TOTAL WEIGHT = 3 X 10 = 29 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (B-C:1) , BC=0.03/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (E) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006406

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408206	J33	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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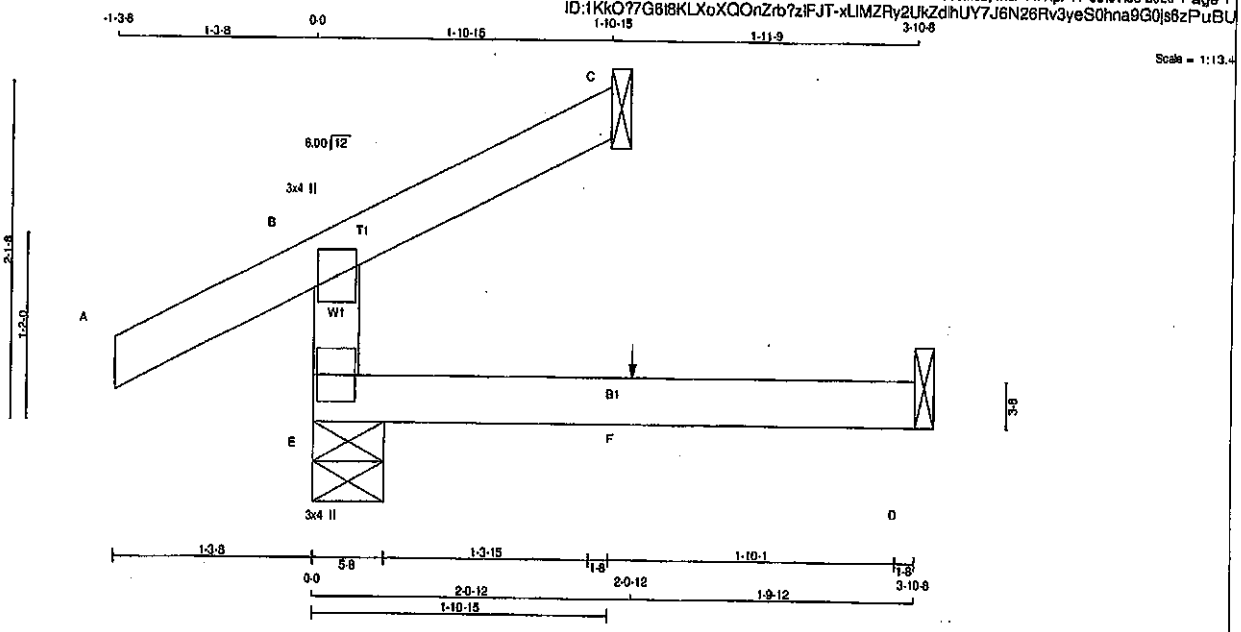


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	275	0	275	0	0	5-8	5-8
C	66	0	66	0	0	1-8	1-8
D	30	0	34	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	194	130 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
C	46	37 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	24	0 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-234 / 0	0.0	0.0	0.05 (4)	7.81			
A-B	0 / 28	-91.8	-91.8	0.14 (5)	10.00			
B-C	-10 / 0	-91.8	-91.8	0.06 (1)	10.00			
E-F	0 / 0	-18.5	-18.5	0.06 (4)	10.00			
F-D	0 / 0	-18.5	-18.5	0.06 (4)	10.00			

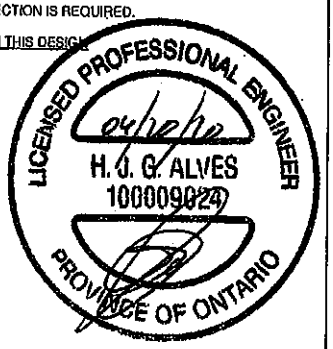
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



TOTAL WEIGHT = 2 X 9 = 19 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14/1.00 (A-B:5) , BC=0.06/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.10/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

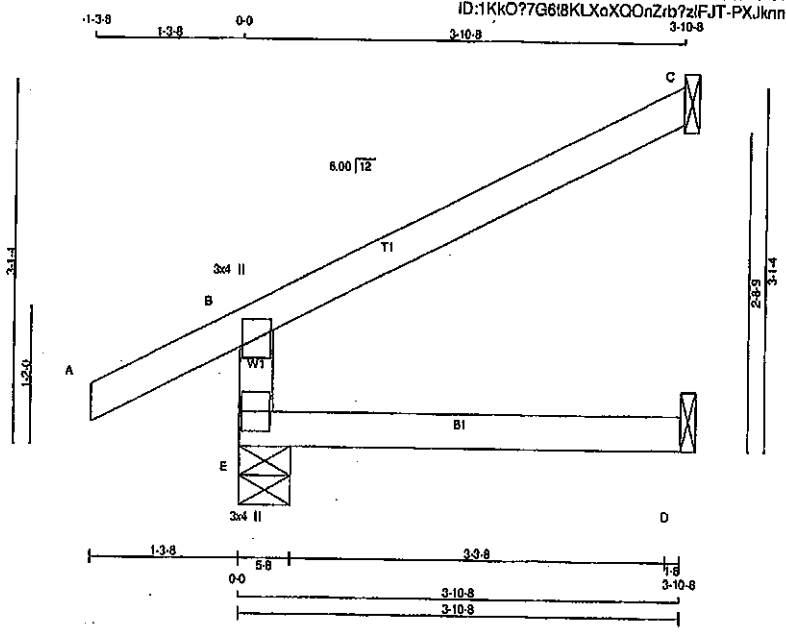
JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006407

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408206	J34	9	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roll Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Fri Apr 17 08:01:36 2020 Page 1
ID:1KkO?7G6!8KLXoXQOnZrb?zlFJT-PXJknnzgF1hUNq3lh1dcbJ_3HMzhl81lOwlGOZzPuBT



TOTAL WEIGHT = 9 X 12 = 108 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	388	0	388	0	0	5-8	5-8
C	133	0	133	0	0	1-8	1-8
D	30	0	34	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	272	193 / 0	0 : 0	0 : 0	0 0	78 0	0 0
C	92	74 / 0	0 : 0	0 : 0	0 0	17 0	0 : 0
D	24	0 / 0	0 : 0	0 : 0	0 : 0	24 0	0 : 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

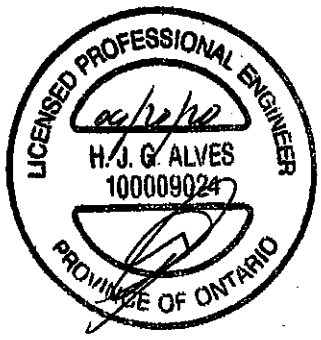
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-347 / 0	0.0	0.0		0.05 (4)	7.81			
A-B	0 / 28	-91.8	-91.8		0.14 (5)	10.00			
B-C	-20 / 0	-91.8	-91.8		0.23 (1)	6.25			
E-D	0 0	-18.5	-18.5		0.08 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
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- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.06/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006408

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

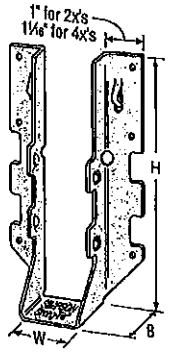
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

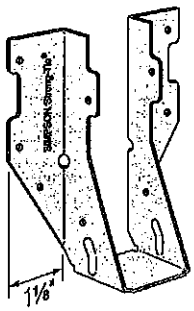
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

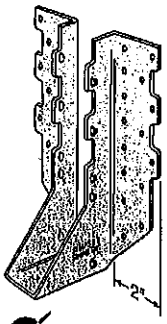
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



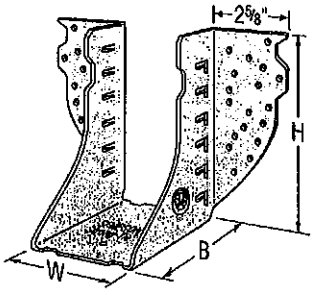
LUS28



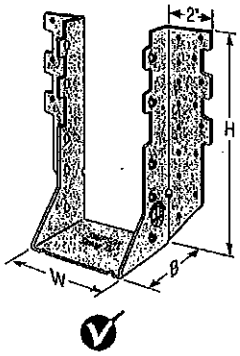
LU26L



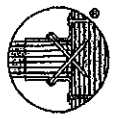
HUS210
(HUS26, HUS28, and HHUS similar)



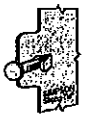
HGUS28-2



HHUS210-2



Double-Shear Nailing Top View



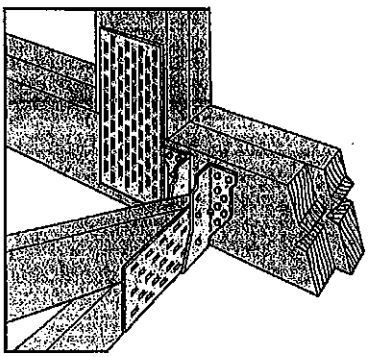
Double-Shear Nailing Side View; Do not bend tab

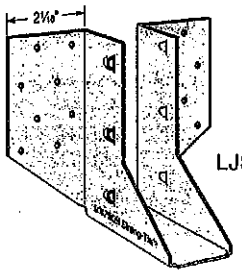
Dome Double-Shear Nailing Side View *(available on some models)*

U.S. Patent 5,603,580

Typical HUS26 Installation with Reduced Heel Height

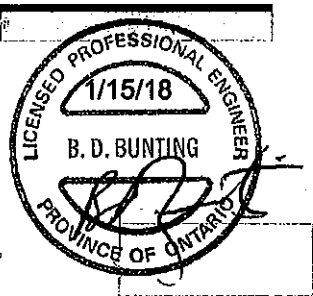
(Truss Designer to provide fastener quantity for connecting multiple members together)





LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS



HHUS/HGUS

See Hanger Options information on pp. 125–127.

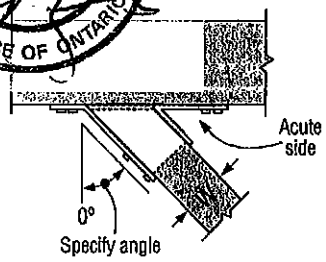
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Single 2x Sizes											
LUS24	18	1⅜	3⅛	1¾	2¼	(4) 10d	(2) 10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
LU24L	22	1⅜	3	1⅝	2⅛	(4) 10d	(2) 10d x 1½"	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
LU26L	22	1⅜	5	1⅝	4⅝	(6) 10d	(4) 10d x 1½"	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
LUS26	18	1⅜	4¾	1¾	3¾	(4) 10d	(4) 10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
HUS26	16	1⅝	5⅜	3	3⅛	(14) 16d	(6) 16d	2705 / 11.30	4940 / 21.97	2065 / 9.20	3875 / 17.24
LJS26DS	18	1⅜	5	3½	4⅝	(16) 16d	(6) 16d	2055 / 9.14	4265 / 18.97	1460 / 6.49	4115 / 18.31
HGUS26	12	1⅝	5⅜	5	4⅛	(20) 16d	(8) 16d	2685 / 11.96	6625 / 29.51	2685 / 11.96	5700 / 25.35
LU28L	20	1⅜	6¾	1⅝	5⅞	(8) 10d	(6) 10d x 1½"	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
LUS28	18	1⅜	6⅝	1¾	3¾	(6) 10d	(4) 10d	1420 / 6.32	2520 / 11.21	1290 / 5.74	1790 / 7.96
HUS28	16	1⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3605 / 16.04	5365 / 23.86	2875 / 11.90	4345 / 19.33
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310 / 14.74	7675 / 34.19	3310 / 14.74	6900 / 30.73
LU210L	20	1⅜	8	1⅝	7⅝	(10) 10d	(6) 10d x 1½"	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
LUS210	18	1⅜	7¹³⁄₁₆	1¾	3⅛	(8) 10d	(4) 10d	1420 / 6.32	2785 / 12.39	1290 / 5.74	2210 / 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3½" long. See pp. 27–28 for other nail sizes and information.

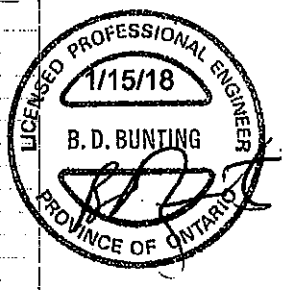
SIMPSON Strong-Tie®

Face-Mount Hangers

■ These products are available with additional corrosion protection. For more information, see p. 24.

◤ These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
								D.Fir-L		S-P-F	
		W	H	B	d_e[3]	Header	Joist	Uplift (K_D = 1.15) lb. kN	Normal (K_D = 1.00) lb. kN	Uplift (K_D = 1.15) lb. kN	Normal (K_D = 1.00) lb. kN
Double 2x Sizes											
■ LUS24-2	18	3⅛	3⅛	2	1½	(4) 16d	(2) 16d	835 3.71	2020 8.99	590 2.62	1435 6.38
SS LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720 7.65	2595 11.54	1545 6.87	1920 8.54
■ HHUS26-2	14	3⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2850 12.68	7335 32.63	2065 9.20	5205 23.15
HGUS26-2	12	3⅝	5⅞	4	4⅛	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
SS LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720 7.65	3325 14.79	1545 6.87	2575 11.45
■ HHUS28-2	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 16.75	8940 39.77	2675 11.90	6345 28.22
HGUS28-2	12	3⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
SS LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580 11.48	4500 20.02	2320 10.32	3195 14.21
■ HHUS210-2	14	3⅝	9⅛	3	8	(30) 16d	(10) 16d	4670 20.77	9660 42.97	4235 18.84	7000 31.14
HGUS210-2	12	3⅝	9⅛	4	8⅛	(46) 16d	(16) 16d	6840 30.43	14015 62.34	4855 21.60	10270 45.69
Triple 2x Sizes											
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
HGUS28-3	12	4 15/16	7¼	4	6⅛	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
■ HHUS210-3	14	4 11/16	9	3	7 15/16	(30) 16d	(10) 16d	4670 20.77	9670 43.02	4235 18.84	6865 30.54
HGUS210-3	12	4 15/16	9¼	4	8⅛	(46) 16d	(16) 16d	6840 30.43	14645 65.14	4855 21.60	10400 46.26
Quadruple 2x Sizes											
HGUS26-4	12	6 9/16	5⅞	4	4⅛	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
HGUS28-4	12	6 9/16	7 9/16	4	6⅛	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
■ HHUS210-4	14	6⅛	8⅞	3	7 13/16	(30) 16d	(10) 16d	4670 20.77	10155 45.17	4235 18.84	7210 32.07
HGUS210-4	12	6 9/16	9 9/16	4	8⅛	(46) 16d	(16) 16d	6840 30.43	14645 65.14	4855 21.60	10400 46.26
HGUS212-4	12	6 9/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640 33.98	14995 66.70	5425 24.13	10645 47.35
HGUS214-4	12	6 9/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130 45.06	16400 72.95	7195 32.00	11645 51.80
4x Sizes											
■ LUS46	18	3 9/16	4¾	2	3 7/16	(4) 16d	(4) 16d	1720 7.65	2595 11.54	1545 6.87	1920 8.54
■ HHUS46	14	3⅝	5¼	3	3 15/16	(14) 16d	(6) 16d	2540 11.30	7335 32.63	2065 9.20	5205 23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
■ LUS48	18	3 9/16	6¾	2	3 7/16	(6) 16d	(4) 16d	1720 7.65	3325 14.79	1545 6.87	2575 11.45
■ HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 16.75	8940 39.77	2675 11.90	6345 28.22
HGUS48	12	3⅝	7 1/16	4	6 1/16	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
■ LUS410	18	3 9/16	8¾	2	5 9/16	(8) 16d	(6) 16d	2580 11.48	4500 20.02	2320 10.32	3195 14.21
HGUS410	12	3⅝	9	4	8⅛	(46) 16d	(16) 16d	6840 30.43	14015 62.34	4855 21.60	10270 45.69
HGUS412	12	3⅝	10 7/16	4	10⅛	(56) 16d	(20) 16d	7640 33.98	14995 66.70	5425 24.13	10645 47.35
HGUS414	12	3⅝	12 7/16	4	11⅛	(66) 16d	(22) 16d	10130 45.06	16400 72.95	7195 32.00	11645 51.80



See footnotes on p. 258.

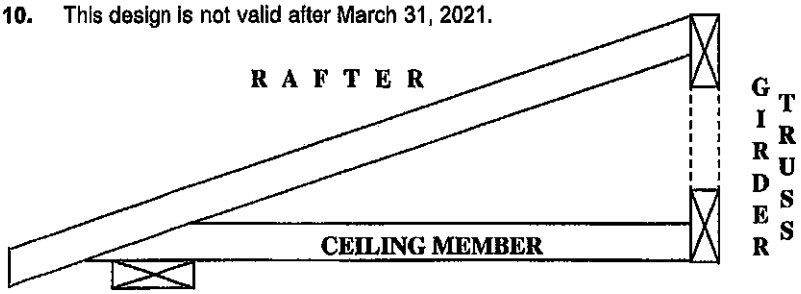
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

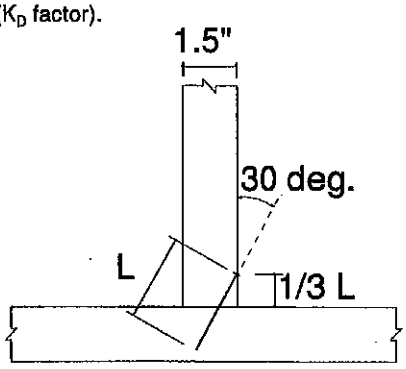
B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.





TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek® MiTek Canada Inc
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Bradford, Ontario L3Z 3G7

December 2, 2019

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING B97791H2

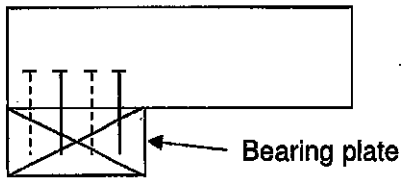
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

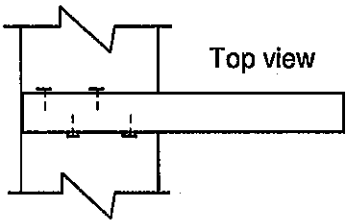
NOTES:

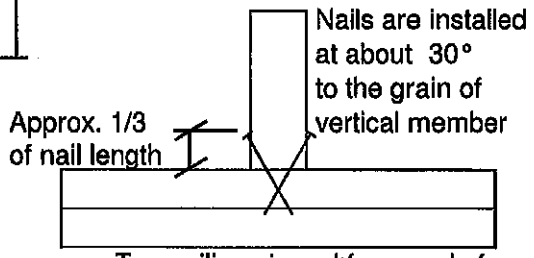
1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



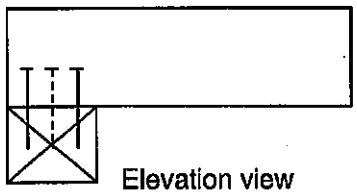
Elevation view



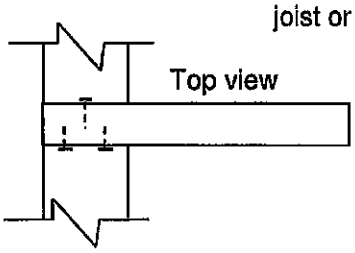


Toe-nailing viewed from end of joist or truss

Toe-nailing on 2x4 Bearing Plate



Elevation view



MiTek® MiTek Canada Inc
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December 2, 2019

ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

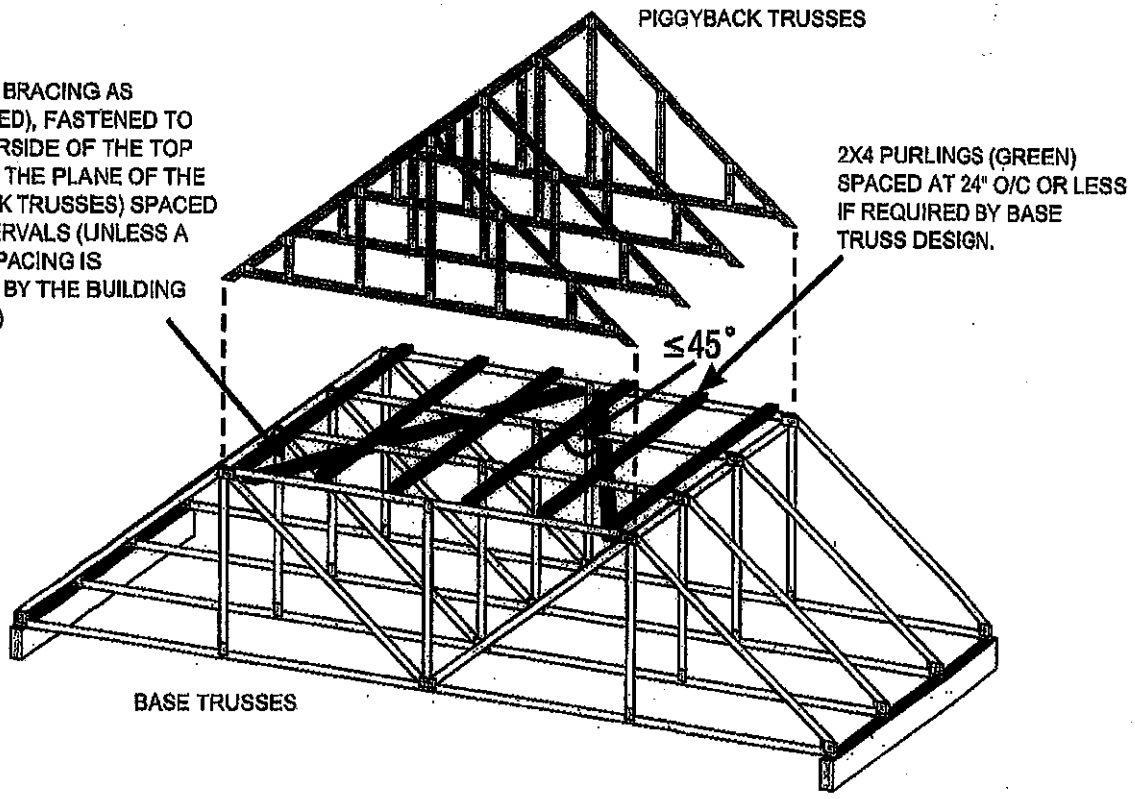
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

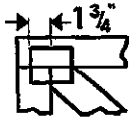


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

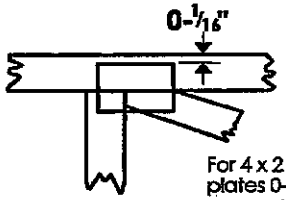
SKETCH FROM BCSI-CANADA 2013

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

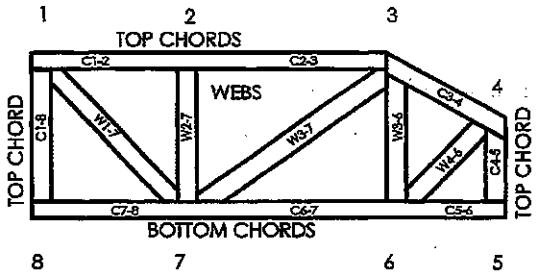
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

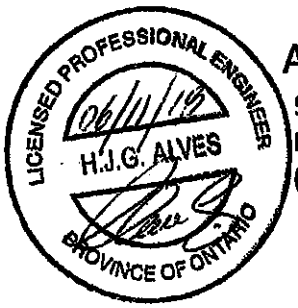
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900218 Feb 09, 2018