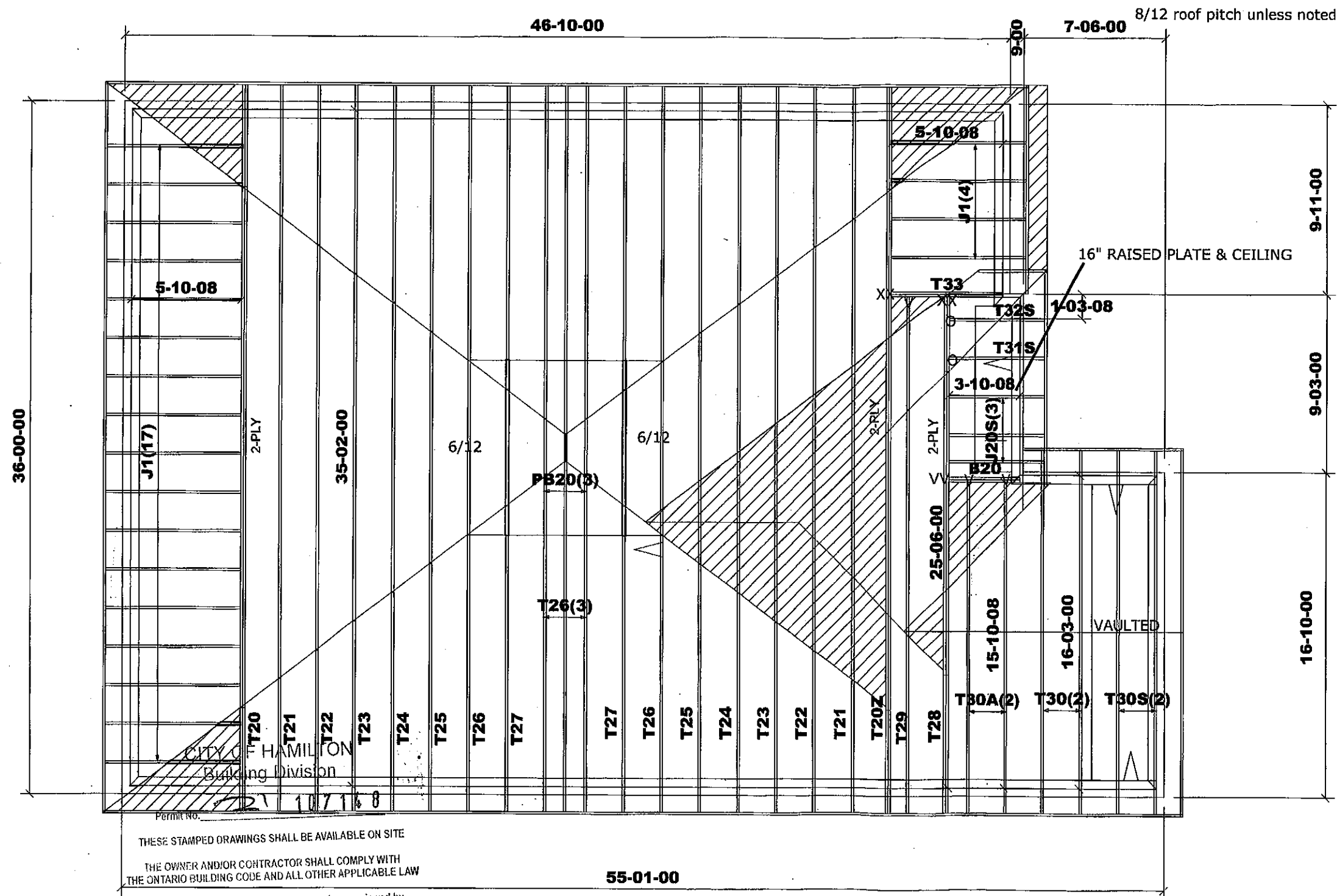




ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

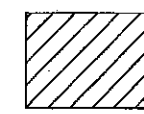
All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
LUS26-2-(VV)

BEAMS:
B20= B21 =
2 - 2x10 SPF #2

 DENOTES:
CONVENTIONAL
FRAMING

THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE
THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH
THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW

These drawings and/or specifications have been reviewed by

 FEB 23 2021
DATE



Job Track: 51225
Plan Log: 202406
Layout ID: 408169

Builder / Location:
GREEN PARK HOMES / WATERDOWN
Project: **RUSSELL GARDENS PH.3**
Date: 2020-04-27 Sales: Mario DiCano Designer: JG

Model / Elevation:
MOUNTAINASH 5/2- STD OR OPT.5 BR

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Lot 268



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 5
 Lot #: 268
 Elevation: 2- STD OR OPT.5 BR

Job Track: 51225
 PlanLog: 202406
 Layout ID: 408169
 Ref #
 Page: 1 of 2
 Date: 04-27-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T20 Hip Girder	8 / 12	35-02-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	320.72 200.67		
	1 2-ply	T20Z Hip Girder	8 / 12	35-02-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	320.72 200.67		
	2	T21 Hip	8 / 12	35-02-00	5-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	279.76 177.00		
	2	T22 Hip	8 / 12	35-02-00	6-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	302.77 191.33		
	2	T23 Hip	8 / 12	35-02-00	7-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	309.14 195.67		
	2	T24 Hip	8 / 12	35-02-00	8-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	316.79 196.67		
	2	T25 Hip	8 / 12	35-02-00	9-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	329.28 205.00		
	5	T26 Hip	8 / 12	35-02-00	10-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	857.95 538.33		
	2	T27 Hip	8 / 12	35-02-00	11-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	332.54 208.67		
	1 2-ply	T28 Hip Girder	8 / 12	25-06-00	5-03-13	2 x 4 2 x 6	1-03-08	1-04-13 2-08-13	244.99 160.00		
	1	T29 Hip	8 / 12	25-06-00	6-07-13	2 x 4	1-03-08	1-04-13 2-08-13	108.89 70.33		
	2	T30 Common	8 / 12	16-03-00	6-09-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	144.92 91.33		
	2	T30A Common	8 / 12	15-10-08	6-09-13	2 x 4	1-03-08	1-04-13 1-04-13	139.83 87.87		
	2	T30S Scissor	8 / 12 4 / 12	16-03-00	6-09-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	133.1 87.33		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: RUSSELL GARDENS PH.3 Location: WATERDOWN Model: MOUNTAINASH 5 Lot #: 268 Elevation: 2- STD OR OPT.5 BR	Job Track: 51225 PlanLog: 202406 Layout ID: 408169 Ref # Page: 2 of 2 Date: 04-27-2020 Designer: Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T31S Half Hip	8 /12	3-10-08	4-09-13	2 x 4	1-03-08	1-04-13 4-09-13	29.4 20.17		
	1	T32S Half Hip	8 /12	3-10-08	3-05-13	2 x 4	1-03-08	1-04-13 3-05-13	24.65 17.17		
	1 2-ply	T33 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	3	PB20 Piggyback	8 /12	9-00-12	2-00-00	2 x 4			74.54 50.50		
	21	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	352.88 224.00		
	3	J20S Jack-Open	8 /12	3-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	85.86 57.50		

TOTAL # TRUSS= 61 TOTAL BFT OF ALL TRUSSES= 3005.01 BFT. TOTAL WEIGHT OF ALL TRSSES 4765.74 LBS

HARDWARE

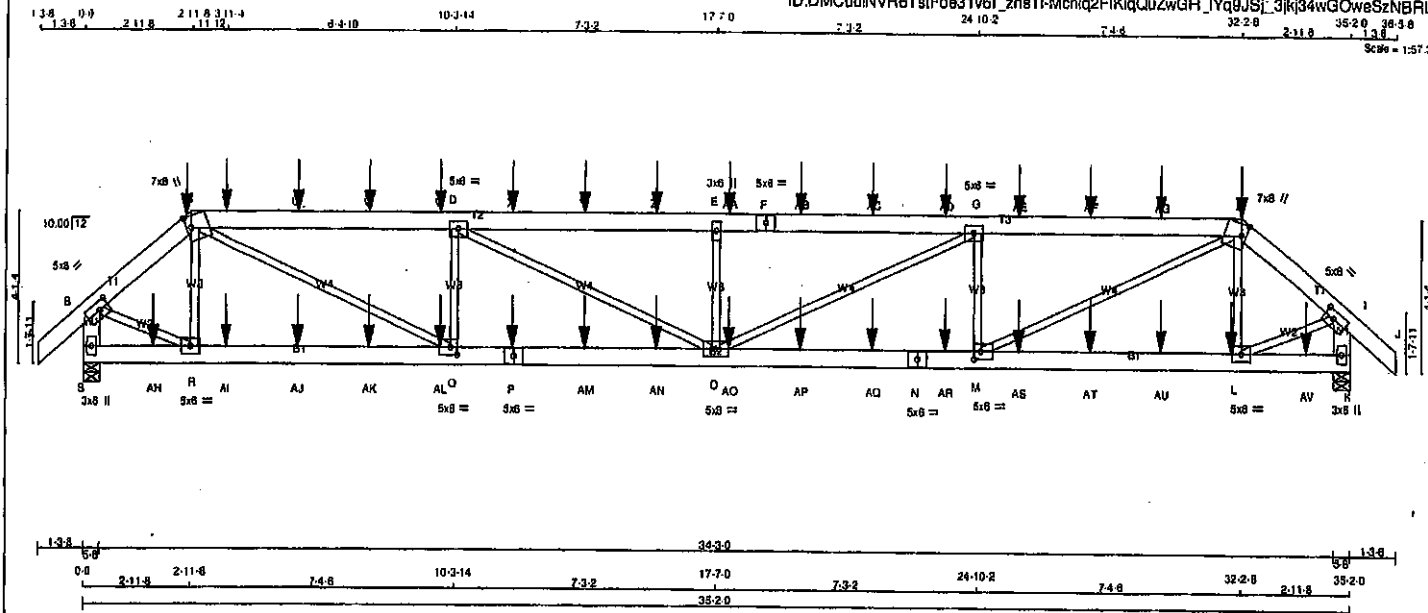
QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
3	Hardware	LJS26DS	
2	Hardware	LUS24	

TOTAL NUMBER OF
ITEMS= 7

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408168	T1	1	2	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6l_zne11-Meniq2FKiqQuZwGH_Yq9JSI_3kj34wGOWeSzNBRL



CHORDS	SIZE	DRY	LUMBER	DESCR.
A - O	2x8	DRY	No.2	SPF
C - F	2x8	DRY	No.2	SPF
F - H	2x8	DRY	No.2	SPF
H - J	2x8	DRY	No.2	SPF
S - B	2x8	DRY	No.2	SPF
K - I	2x8	DRY	No.2	SPF
S - P	2x8	DRY	2100F 1.8E	SPF
P - N	2x8	DRY	2100F 1.8E	SPF
N - K	2x8	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-G 2	12	SIDE(122.0)
C-F 2	12	SIDE(81.0)
F-H 2	12	SIDE(81.0)
H-J 2	12	SIDE(122.0)
S-B 2	12	TOP
K-I 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S-P 2	12	SIDE(0.0)
P-N 2	12	SIDE(0.0)
N-K 2	12	SIDE(197.8)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
S	3330	0	3330	0	0	0	5-8	5-8	5-8
K	3379	0	3379	0	0	0	5-8	5-8	5-8
UNFACTORED REACTIONS		1ST LOOSE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
S	2354	1553	0	0	0	0	0	0	0
K	2388	1572	0	0	0	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 42	-91.8	-91.8 0.04 (1)	R-C	-850 / 0
B-C	-3387 / 0	-91.8	-91.8 0.05 (1)	C-Q	0 / 4509
C-T	-8802 / 0	-91.8	-91.8 0.38 (1)	Q-D	-1799 / 0
T-U	-8802 / 0	-91.8	-91.8 0.38 (1)	D-O	0 / 1370
U-V	-8802 / 0	-91.8	-91.8 0.38 (1)	C-E	-887 / 0
V-W	-8802 / 0	-91.8	-91.8 0.38 (1)	C-Q	0 / 1382
W-D	-8802 / 0	-91.8	-91.8 0.38 (1)	M-H	-1787 / 0
D-X	-7828 / 0	-91.8	-91.8 0.42 (1)	B-R	0 / 4481
X-Y	-7828 / 0	-91.8	-91.8 0.42 (1)	L-H	-881 / 0
Y-Z	-7828 / 0	-91.8	-91.8 0.42 (1)	B-R	0 / 2736
Z-E	-7828 / 0	-91.8	-91.8 0.42 (1)	L-I	0 / 2771
E-A	-7828 / 0	-91.8	-91.8 0.42 (1)		
A-A'	-7828 / 0	-91.8	-91.8 0.42 (1)		
F-A	-7828 / 0	-91.8	-91.8 0.42 (1)		
AB-AC	-7828 / 0	-91.8	-91.8 0.42 (1)		
AC-AD	-7828 / 0	-91.8	-91.8 0.42 (1)		
AD-AG	-7828 / 0	-91.8	-91.8 0.42 (1)		
GA-E	-8808 / 0	-91.8	-91.8 0.38 (1)		
AE-AF	-8808 / 0	-91.8	-91.8 0.38 (1)		
AF-AG	-8808 / 0	-91.8	-91.8 0.38 (1)		
AG-H	-8808 / 0	-91.8	-91.8 0.38 (1)		
H-I	-3410 / 0	-91.8	-91.8 0.05 (1)		
I-J	0 / 42	-91.8	-91.8 0.04 (1)		
S-B	-3375 / 0	0.0	0.0 0.12 (1)		
K-I	-3414 / 0	0.0	0.0 0.12 (1)		
S-AH	0 / 0	-18.5	-18.5 0.05 (4)		
AH-R	0 / 0	-18.5	-18.5 0.05 (4)		
R-AI	0 / 2553	-18.5	-18.5 0.12 (1)		
AI-AJ	0 / 2553	-18.5	-18.5 0.12 (1)		
AJ-AK	0 / 2553	-18.5	-18.5 0.12 (1)		
AK-AL	0 / 2553	-18.5	-18.5 0.12 (1)		
AL-Q	0 / 2553	-18.5	-18.5 0.12 (1)		
Q-P	0 / 6802	-18.5	-18.5 0.22 (1)		
P-AM	0 / 6802	-18.5	-18.5 0.22 (1)		
AM-AN	0 / 6802	-18.5	-18.5 0.22 (1)		
AN-O	0 / 6802	-18.5	-18.5 0.22 (1)		
O-AO	0 / 6809	-18.5	-18.5 0.22 (1)		
AO-AP	0 / 6809	-18.5	-18.5 0.22 (1)		
AP-AQ	0 / 6809	-18.5	-18.5 0.22 (1)		
AQ-N	0 / 6809	-18.5	-18.5 0.22 (1)		
N-AR	0 / 6809	-18.5	-18.5 0.22 (1)		
AR-M	0 / 6809	-18.5	-18.5 0.22 (1)		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 0.0 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 2.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPC 2011, TPC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17)
CALCULATED VERT. DEFL.(LL) = L/899 (0.20)
ALLOWABLE DEFL.(TL) = L/360 (1.17)
CALCULATED VERT. DEFL.(TL) = L/899 (0.37)

CSI: TC=0.42/1.00 (D-E:1), BC=0.22/1.00 (C-Q:1), WB=0.58/1.00 (C-Q:1), SS=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 364 1887 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.85 (N) (INPUT = 1.00)



Structural component only
DWG# T-2007129 1/2

CONTINUED ON PAGE 2

JOB NAME

408168

TRUSS NAME

T1

QUANTITY

1

PLY

2

JOB DESC.

GREEN PARK HOMES

DRWG NO.

Tamarack Roof Truss, Burlington

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ID:DMCubiNVR6TsjFoe31v6I zng11-Mcnio2FikQQuZwGH IYq9JSI 3IK34wGOWsSzNBR

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-1	MT20	5.0	8.0	2.50	3.25
C	TTWW+m	MT20	7.0	8.0	3.75	1.50
D	TMWV-1	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-1	MT20	5.0	6.0		
G	TMWV-1	MT20	5.0	6.0		
H	TTWW+m	MT20	7.0	8.0	3.75	1.50
I	TMWV-1	MT20	5.0	8.0	2.50	3.25
K	BMV1+p	MT20	3.0	6.0		
L	BMWV-1	MT20	5.0	6.0		
M	BMWV-1	MT20	5.0	6.0	2.50	2.25
N	BS-1	MT20	5.0	6.0		
O	BMWV-1	MT20	5.0	6.0		
P	BS-1	MT20	5.0	6.0		
Q	BMWV-1	MT20	5.0	6.0	2.50	2.25
R	BMWV-1	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	6.0		

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1 (PLF)	MAX CS1 (LC)	MAX UNBRAC LENGTH	FR-TO
M-AS	0 / 2588	-18.5	-18.5	0.12 (1)	10.00	
AS-AT	0 / 2588	-18.5	-18.5	0.12 (1)	10.00	
AT-AU	0 / 2588	-18.5	-18.5	0.12 (1)	10.00	
AU-L	0 / 2588	-18.5	-18.5	0.12 (1)	10.00	
L-AV	0 / 0	-18.5	-18.5	0.05 (1)	10.00	
AV-K	0 / 0	-18.5	-18.5	0.05 (1)	10.00	

WEBS

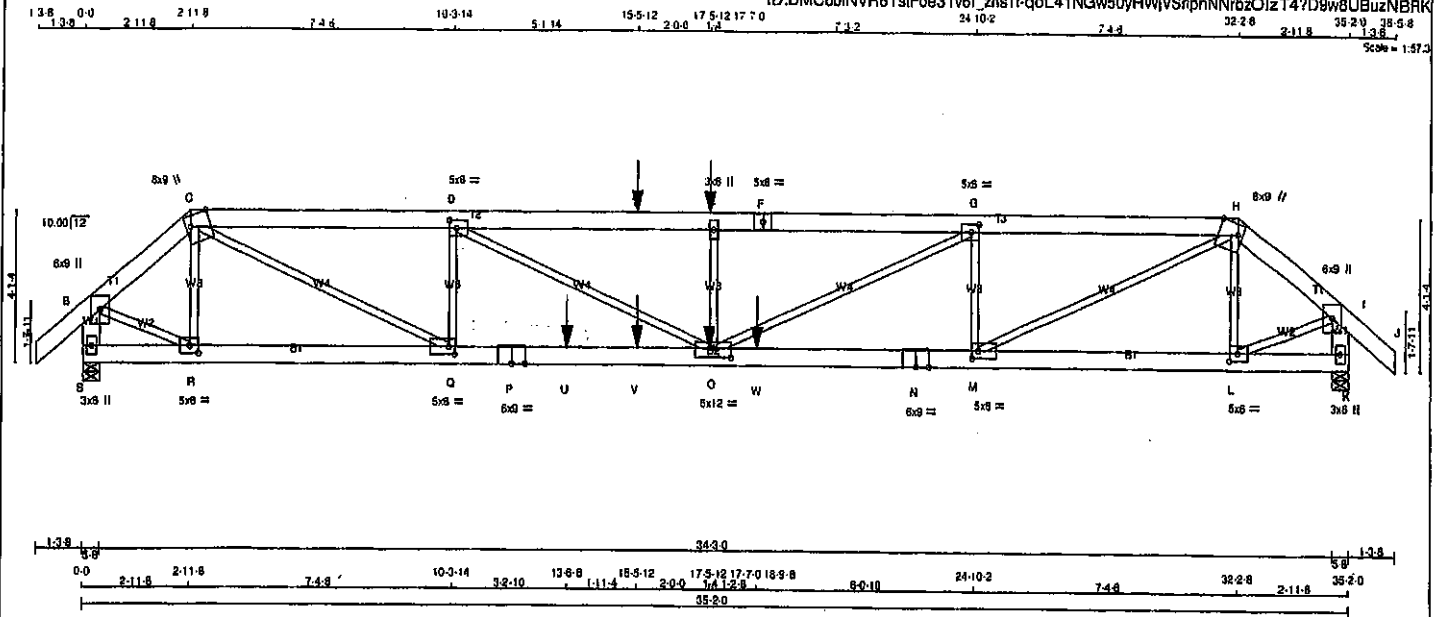
MEMB.	FORCE (LBS)	MAX CS1 (LC)	MAX UNBRAC LENGTH	FR-TO
M-AS	0 / 2588	-18.5	-18.5	0.12 (1)
AS-AT	0 / 2588	-18.5	-18.5	0.12 (1)
AT-AU	0 / 2588	-18.5	-18.5	0.12 (1)
AU-L	0 / 2588	-18.5	-18.5	0.12 (1)
L-AV	0 / 0	-18.5	-18.5	0.05 (1)
AV-K	0 / 0	-18.5	-18.5	0.05 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-38	-42	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-191	-191	---	FRONT	VERT	SNOW	---	C1
H	32-2-8	-38	-42	---	FRONT	VERT	DEAD	---	C1
H	32-2-8	-116	-116	---	FRONT	VERT	TOTAL	---	C1
H	32-2-8	-191	-191	---	FRONT	VERT	SNOW	---	C1
L	31-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
P	11-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
T	3-11-4	-111	-111	---	FRONT	VERT	TOTAL	---	C1
U	5-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	11-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	15-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	17-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	19-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	21-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	23-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408168	T1Z	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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ID:DMCubINVR6TslFoe31v6L_zns1f-qoL41NGw50yHWjVSdpnNNrbzOizT47D9w8UBuzNBRK



LUMBER				DESIGN CRITERIA			
N. L. G. A. RULES				SPECIFIED LOADS:			
CHORDS	SIZE	DRY	LUMBER	TOP CH.	LL = 25.6	PSF	
A - C	2x6	DRY	No.2	DL	= 6.0	PSF	
C - F	2x6	DRY	No.2	BOT CH.	LL = 0.0	PSF	
F - H	2x6	DRY	No.2	DL	= 7.4	PSF	
H - J	2x6	DRY	No.2	TOTAL LOAD	= 39.0	PSF	
J - L	2x6	DRY	No.2				
L - N	2x6	DRY	No.2				
N - K	2x6	DRY	No.2				
ALL WEBS	2x3	DRY	No.2				
EXCEPT							

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 2	12	TOP
C-F 2	12	SIDE(183.1)
F-H 2	12	TOP
H-J 2	12	TOP
J-L 2	12	TOP
L-N 2	12	TOP
N-K 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S-P 2	12	TOP
P-N 2	12	SIDE(187.8)
N-K 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
S	3883	0	0	3883	0	0	5-8	5-8	5-8
K	3887	0	0	3887	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LOASE		MAX/MIN. COMPONENT REACTIONS		COMBINED		SNOW		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	2783	1890 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
K	2599	1758 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE	MAX	MAX
	(LBS)	(PLF)	(LC)	(LBS)	(LBS)		(LBS)	(LBS)	(LBS)
FR-TO		FROM	TO	FROM	TO	FR-TO		FROM	TO
A-B	0 / 42	-91.8	-91.8	0.04 (1)	10.00	R-C	-1086 / 0	0.13 (1)	
B-C	-3988 / 0	-91.8	-91.8	0.08 (1)	5.85	C-Q	0 / 7283	0.90 (1)	
C-D	-9534 / 0	-91.8	-91.8	0.37 (1)	3.74	Q-D	-2180 / 0	0.28 (1)	
D-T	-12331 / 0	-91.8	-91.8	0.52 (1)	3.20	D-O	0 / 3130	0.39 (1)	
T-E	-12331 / 0	-91.8	-91.8	0.52 (1)	3.20	O-E	-776 / 0	0.09 (1)	
E-F	-12331 / 0	-91.8	-91.8	0.50 (1)	3.23	C-G	0 / 4203	0.52 (1)	
F-G	-12331 / 0	-91.8	-91.8	0.50 (1)	3.23	G-G	-2813 / 0	0.31 (1)	
G-H	-8575 / 0	-91.8	-91.8	0.33 (1)	3.94	M-H	0 / 6383	0.79 (1)	
H-I	-3758 / 0	-91.8	-91.8	0.08 (1)	5.78	L-H	-843 / 0	0.10 (1)	
I-J	0 / 42	-91.8	-91.8	0.04 (1)	10.00	B-R	0 / 3239	0.40 (1)	
S-B	-3945 / 0	0.0	0.0	0.14 (1)	7.14	L-I	0 / 3052	0.38 (1)	
K-I	-3733 / 0	0.0	0.0	0.13 (1)	7.29				
S-R	0 / 0	-18.5	-18.5	0.02 (4)	10.00				
R-Q	0 / 3013	-18.5	-18.5	0.21 (1)	10.00				
Q-P	0 / 8534	-18.5	-18.5	0.67 (1)	10.00				
P-U	0 / 8534	-18.5	-18.5	0.87 (1)	10.00				
U-V	0 / 8534	-18.5	-18.5	0.67 (1)	10.00				
V-O	0 / 8534	-18.5	-18.5	0.67 (1)	10.00				
O-W	0 / 8575	-18.5	-18.5	0.49 (1)	10.00				
W-N	0 / 8575	-18.5	-18.5	0.49 (1)	10.00				
N-M	0 / 8575	-18.5	-18.5	0.49 (1)	10.00				
M-L	0 / 2844	-18.5	-18.5	0.12 (1)	10.00				
L-K	0 / 0	-18.5	-18.5	0.05 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	17-5-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
O	17-5-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
T	15-5-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
U	13-5-8	-1615	-1615	---	BACK	VERT	TOTAL	---	C1
V	15-5-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
W	18-9-8	-1620	-1620	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



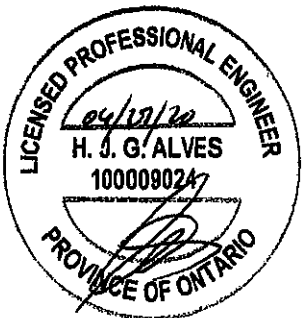
Structural component only
DWG# T-2007130 1/2

CONTINUED ON PAGE 2

JOB NAME 408168	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 12:34:49 2020 Page 2	
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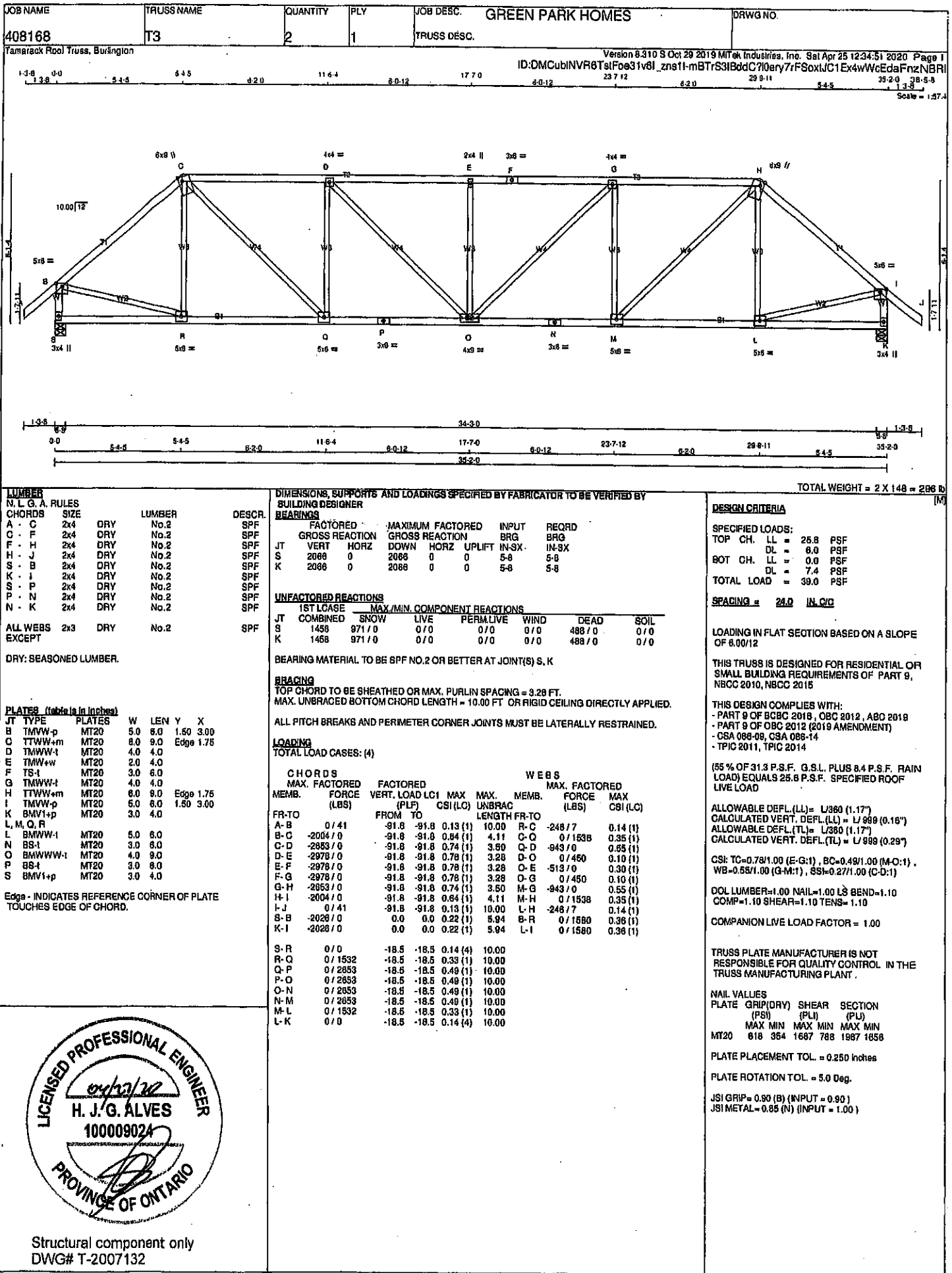
PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	6.0	9.0	
C	TTWW+m	MT20	8.0	8.0	Edge 8.50
D	TMVW-l	MT20	5.0	8.0	2.50 2.50
E	TMVW+w	MT20	3.0	8.0	
F	TS-l	MT20	5.0	8.0	
G	TMVW-l	MT20	5.0	8.0	2.50 2.50
H	TTWW+m	MT20	8.0	9.0	Edge 6.50
I	TMVW+p	MT20	6.0	9.0	
K	BMV-l+p	MT20	3.0	8.0	
L	BMVW-l	MT20	5.0	8.0	2.50 2.75
M	BMVW-l	MT20	5.0	8.0	2.50 2.00
N	BS-l	MT20	6.0	9.0	
O	BMVWVW-l	MT20	5.0	12.0	3.00 8.00
P	BS-l	MT20	6.0	9.0	
Q	BMVW-l	MT20	5.0	8.0	2.50 2.00
R	BMVW-l	MT20	5.0	8.0	2.50 2.75
S	BMV-l+p	MT20	3.0	8.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

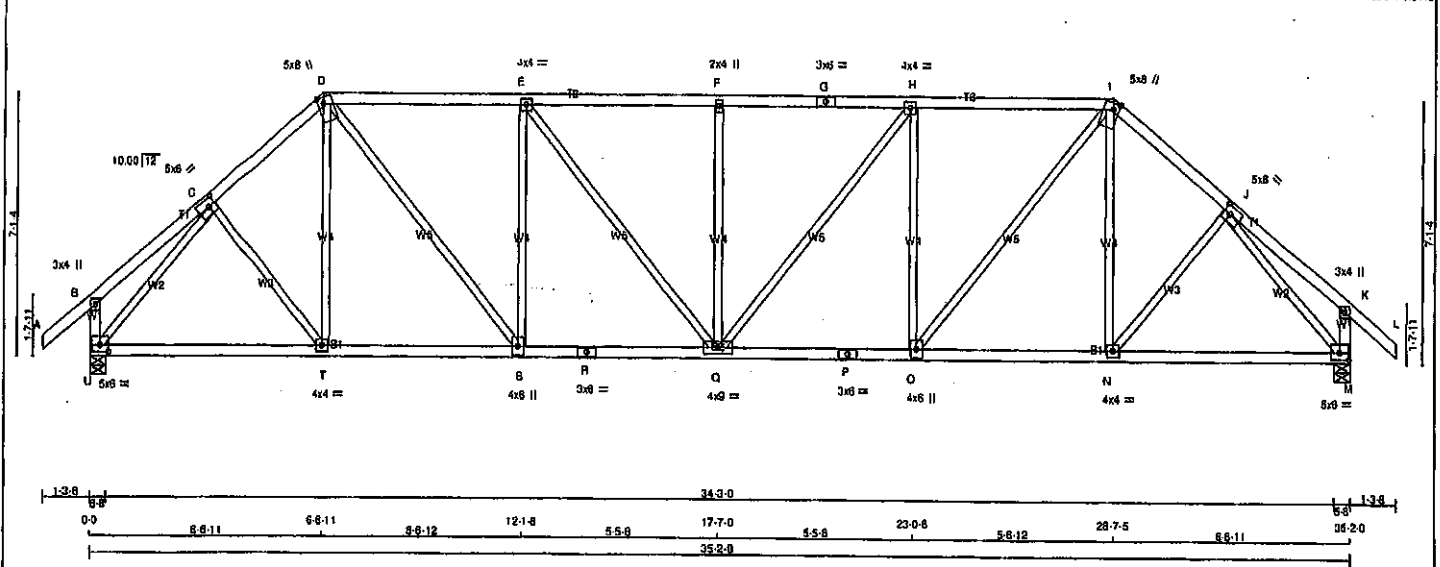


Structural component only
DWG# T-2007130 92

[illegible]



JOB NAME 408168	TRUSS NAME T4	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 8 Oct 2019 Mitek Industries, Inc. Sat Apr 25 12:34:52 2020 Page 1	
ID:DMCubINVR6TstFoe31v6I zns1I-EN1DIPpOxKsNAD1WqNU_1T6RbNZgQVfruM8oDzNBRIH				Scale = 1:57.5	



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
L - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
U - R	2x4	DRY	No.2
R - P	2x4	DRY	No.2
P - M	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF
DRY: SEASONED LUMBER.			

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV+p	MT20	3.0	4.0	
C TMWW-l	MT20	6.0	8.0	2.50 2.25
D TTWW+m	MT20	5.0	8.0	2.00 1.75
E TMWW-l	MT20	4.0	4.0	
F TMWW-w	MT20	2.0	4.0	
G TS-l	MT20	3.0	6.0	
H TMWW-l	MT20	4.0	4.0	
I TTWW+m	MT20	5.0	8.0	2.00 1.75
J TMWW-l	MT20	5.0	6.0	2.50 2.25
K TMV+p	MT20	3.0	4.0	
M BMVV1-l	MT20	6.0	8.0	2.25 3.00
N BMWW-l	MT20	4.0	4.0	
O BMWW-l	MT20	4.0	8.0	
P BS-l	MT20	3.0	6.0	
Q BMWW-l	MT20	4.0	9.0	
R BS-l	MT20	3.0	6.0	
S BMWW-l	MT20	4.0	8.0	
T BMWW-l	MT20	4.0	4.0	
U BMVV1-l	MT20	6.0	8.0	2.25 3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED	MAXIMUM FACTORED	INPUT	REQD	
	GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	DOWN	HORIZ	UPLIFT	IN-SX
U	2088	0	2088	0	5-8
M	2088	0	2088	0	5-8

UNFACTORED REACTIONS

1ST CASE MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
U	1458	971/0	0/0	0/0	488/0
M	1458	971/0	0/0	0/0	488/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED	FACTORED	
	FORCE	VERT. LOAD	LC1 MAX		FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)		(LBS)	CSI (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/141	-91.8	-91.8 0.13 (1)	10.00	C-T	0/156	0.04 (1)
B-C	0/119	-91.8	-91.8 0.14 (1)	10.00	T-D	0/76	0.03 (4)
C-D	-2023/0	-91.8	-91.8 0.20 (1)	4.59	D-S	0/1220	0.27 (1)
D-E	-2308/0	-91.8	-91.8 0.55 (1)	3.94	S-E	-850/0	0.74 (1)
E-F	-2534/0	-91.8	-91.8 0.67 (1)	3.77	E-Q	0/391	0.08 (1)
F-G	-2534/0	-91.8	-91.8 0.57 (1)	3.77	Q-F	-481/0	0.40 (1)
G-H	-2534/0	-91.8	-91.8 0.57 (1)	3.77	Q-H	0/391	0.08 (1)
H-I	-2308/0	-91.8	-91.8 0.55 (1)	3.94	C-H	-850/0	0.74 (1)
I-J	-2023/0	-91.8	-91.8 0.20 (1)	4.59	O-I	0/1220	0.27 (1)
J-K	0/119	-91.8	-91.8 0.14 (1)	10.00	N-I	0/76	0.03 (4)
K-L	0/141	-91.8	-91.8 0.13 (1)	10.00	N-J	0/156	0.04 (1)
U-B	-248/0	0.0	0.0 0.03 (1)	7.81	U-G	-2277/0	0.98 (1)
M-K	-248/0	0.0	0.0 0.03 (1)	7.81	J-M	-2277/0	0.98 (1)
U-T	0/1433	-18.5	-18.5 0.33 (1)	10.00			
T-S	0/1835	-18.5	-18.5 0.35 (1)	10.00			
S-R	0/2308	-18.5	-18.5 0.42 (1)	10.00			
R-Q	0/2308	-18.5	-18.5 0.42 (1)	10.00			
Q-P	0/2308	-18.5	-18.5 0.42 (1)	10.00			
P-O	0/2308	-18.5	-18.5 0.42 (1)	10.00			
O-N	0/1935	-18.5	-18.5 0.35 (1)	10.00			
N-M	0/1433	-18.5	-18.5 0.33 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 688-09, CSA 688-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = L/360 (1.17')
CALCULATED VERT. DEFL (LL) = L/999 (0.13')
ALLOWABLE VERT. DEFL (TL) = L/360 (1.17')
CALCULATED VERT. DEFL (TL) = L/999 (0.23')

CSI: TC=0.57/1.00 (E-F:1), BC=0.42/1.00 (O-Q:1),
WB=0.98/1.00 (J-M:1), SS=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

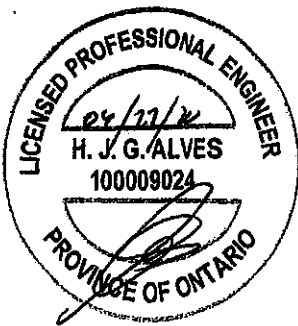
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1887 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL = 0.72 (P) (INPUT = 1.00)

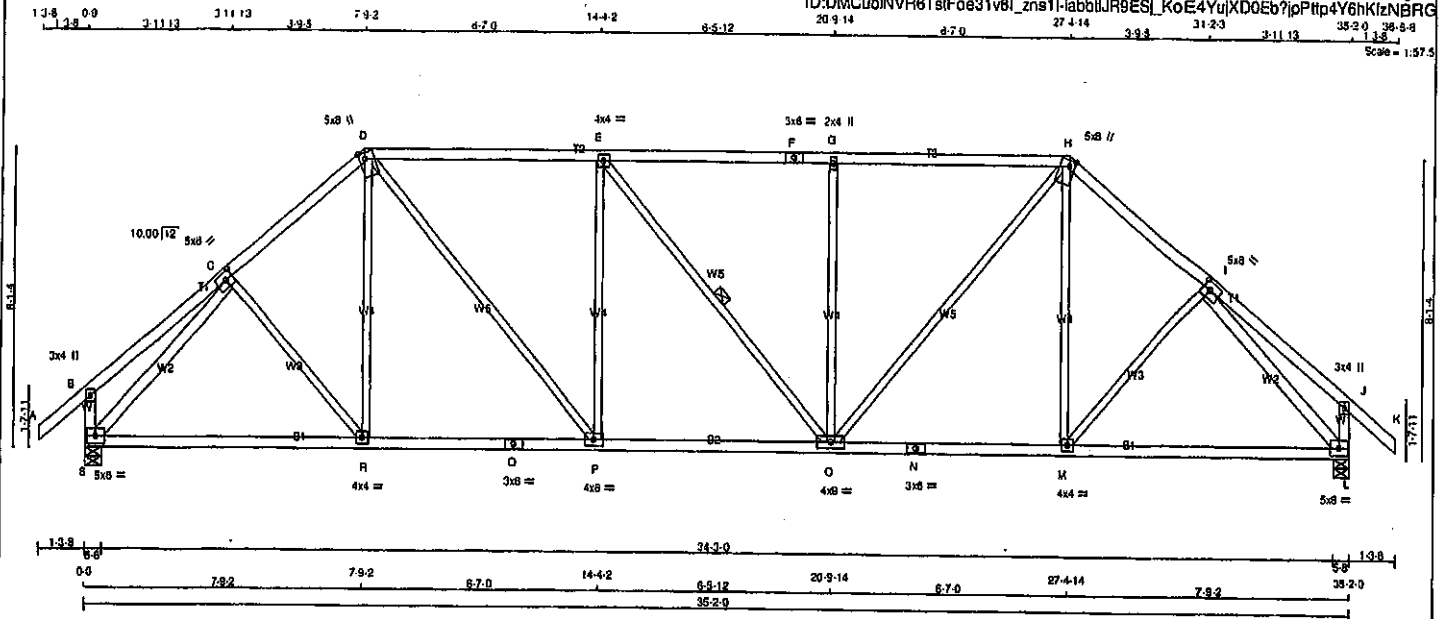


Structural component only
DWG# T-2007133

JOB NAME 408168	TRUSS NAME T5	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
TRUSS DESC.					

Temarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v8l_zns11-habbilJR9ESL_KoE4YUjXD0Eb7pPtp4Y6hKlzNBRC



TOTAL WEIGHT = 2 X 163 = 326 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
S - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV+p	MT20	3.0	4.0	
C TMVW-I	MT20	5.0	6.0	2.50 2.50
D TTWW+m	MT20	5.0	8.0	2.00 1.75
E TMVW-I	MT20	4.0	4.0	
F TS-I	MT20	3.0	6.0	
G TMVW+m	MT20	2.0	4.0	
H TTWW+m	MT20	5.0	8.0	2.00 1.75
I TMVW-I	MT20	5.0	6.0	2.50 2.50
J TMV+p	MT20	3.0	4.0	
K BMVW-I	MT20	5.0	6.0	
M BMVW-I	MT20	4.0	4.0	
N BS-I	MT20	3.0	6.0	
O BMVW-I	MT20	4.0	9.0	
P BMVW-I	MT20	4.0	6.0	
Q BS-I	MT20	3.0	6.0	
R BMVW-I	MT20	4.0	4.0	
S BMVW-I	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED	MAXIMUM FACTORED	INPUT	REQD	
	GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
S	2086	0	2086	0	0
L	2086	0	2086	0	0

UNFACTORED REACTIONS

1ST CASE MAX/MIN COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
S	1458	971/0	0/0	0/0	0/0	488/0
L	1458	971/0	0/0	0/0	0/0	488/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/41	-91.8 -91.8	0.13 (1)	10.00	C-R	0/64	0.02 (4)
B-C	0/28	-91.8 -91.8	0.21 (1)	10.00	R-D	0/126	0.04 (4)
C-D	-2008/0	-91.8 -91.8	0.29 (1)	4.50	D-P	0/988	0.22 (1)
D-E	-2163/0	-91.8 -91.8	0.73 (1)	3.81	P-E	-850/0	0.82 (1)
E-F	-2162/0	-91.8 -91.8	0.73 (1)	3.81	E-O	-2/0	0.00 (1)
F-G	-2162/0	-91.8 -91.8	0.73 (1)	3.81	O-G	-849/0	0.82 (1)
G-H	-2162/0	-91.8 -91.8	0.73 (1)	3.82	H-I	0/988	0.22 (1)
H-I	-2008/0	-91.8 -91.8	0.29 (1)	4.50	M-H	0/126	0.04 (4)
I-J	0/28	-91.8 -91.8	0.21 (1)	10.00	M-I	0/64	0.02 (4)
J-K	0/41	-91.8 -91.8	0.13 (1)	10.00	S-C	-2281/0	0.97 (1)
S-B	-284/0	0.0 0.0	0.03 (1)	7.81	I-L	-2281/0	0.97 (1)
L-J	-284/0	0.0 0.0	0.03 (1)	7.81			

S-R	0/1487	-18.5 -18.5	0.38 (1)	10.00
R-Q	0/1519	-18.5 -18.5	0.39 (1)	10.00
Q-P	0/1519	-18.5 -18.5	0.39 (1)	10.00
P-O	0/2183	-18.5 -18.5	0.42 (1)	10.00
O-N	0/1519	-18.5 -18.5	0.39 (1)	10.00
N-M	0/1519	-18.5 -18.5	0.39 (1)	10.00
M-L	0/1487	-18.5 -18.5	0.38 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019

- PART 9 OF OBC 2012 (2018 AMENDMENT)

- CSA 088-09, CSA 088-14

- TPIC 2011, TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (1.17')

CALCULATED VERT. DEFL.(LL) = L/999 (0.10')

ALLOWABLE DEFL.(TL) = L/380 (1.17')

CALCULATED VERT. DEFL.(TL) = L/999 (0.21')

CSF: TC=0.73/1.00 (D-E:1), BC=0.42/1.00 (O-P:1), WB=0.97/1.00 (I-L:1), SS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENB=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(ORY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1687 788 1887 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

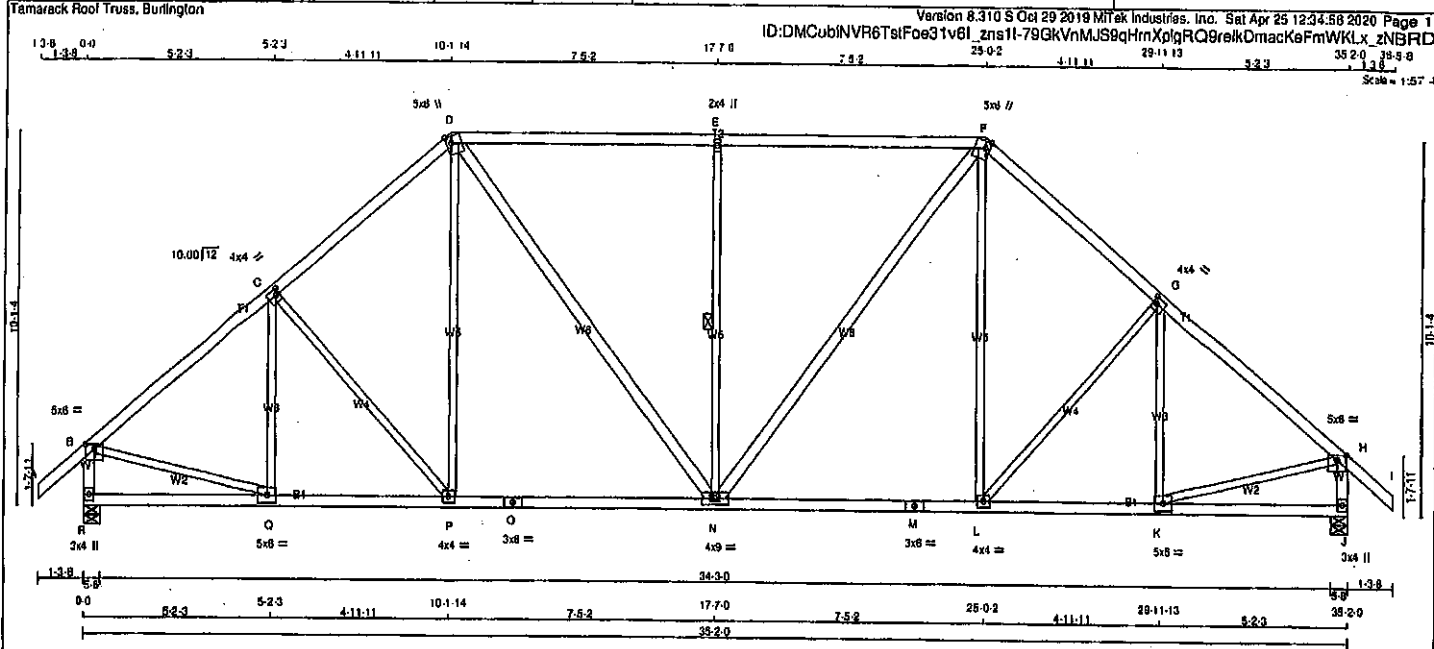
JSI GRIP = 0.87 (I) (INPUT = 0.90)

JSI METAL = 0.56 (I) (INPUT = 1.00)



Structural component only
DWG# T-2007134

JOB NAME 408168	TRUSS NAME T7	QUANTITY 5	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	8.0	2.25	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	2.25	1.50
G	TMVW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW-p	MT20	5.0	8.0	1.50	3.00
J	BMV1-p	MT20	3.0	4.0		
K	BMVW-t	MT20	5.0	8.0		
L	BMVW-t	MT20	4.0	4.0		
M	BS-1	MT20	3.0	6.0		
N	BMVW-w	MT20	4.0	9.0		
O	BS-1	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	5.0	8.0		
R	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
GROSS REACTION		GROSS REACTION		BRG		IN-SX	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	2068	0	2068	0	0	5-8	5-8
J	2068	0	2068	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX/MIN COMPONENT REACTIONS		PERM/LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM/LIVE	WIND	DEAD	SOIL	COMBINED	SNOW	LIVE	PERM/LIVE
R	1458	971/0	0/0	0/0	0/0	488/0	0/0	1458	971/0	0/0	0/0
J	1458	971/0	0/0	0/0	0/0	488/0	0/0	1458	971/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FACTORED	VERT. LOAD	LC1 MAX	MEMB.	MAX. FACTORED	VERT. LOAD	LC1 MAX
	FORCE (LBS)	(PLF)	CSI (LC)		FORCE (LBS)	(PLF)	CSI (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/41	-91.8	-91.8 0.13 (1)	Q-Q	-302/0	0.17 (1)	
B-C	-2021/0	-91.8	-91.8 0.39 (1)	C-P	-217/0	0.25 (1)	
C-D	-1908/0	-91.8	-91.8 0.37 (1)	P-D	0/278	0.08 (1)	
D-E	-1809/0	-91.8	-91.8 0.74 (1)	D-N	0/816	0.10 (1)	
E-F	-1809/0	-91.8	-91.8 0.74 (1)	N-E	-898/0	0.58 (1)	
F-G	-1808/0	-91.8	-91.8 0.37 (1)	N-F	0/615	0.10 (1)	
G-H	-2021/0	-91.8	-91.8 0.39 (1)	L-F	-0/278	0.08 (1)	
H-I	0/41	-91.8	-91.8 0.13 (1)	L-G	-217/0	0.25 (1)	
R-B	-2024/0	0.0	0.0 0.22 (1)	K-G	-302/0	0.17 (1)	
J-H	-2024/0	0.0	0.0 0.22 (1)	B-Q	0/1629	0.37 (1)	
				K-H	0/1629	0.37 (1)	
R-Q	0/0	-18.5	-18.5 0.10 (4)				
Q-P	0/1580	-18.5	-18.5 0.34 (1)				
P-O	0/1437	-18.5	-18.5 0.35 (1)				
O-N	0/1437	-18.5	-18.5 0.35 (1)				
N-M	0/1437	-18.5	-18.5 0.35 (1)				
M-L	0/1437	-18.5	-18.5 0.35 (1)				
L-K	0/1580	-18.5	-18.5 0.34 (1)				
K-J	0/0	-18.5	-18.5 0.10 (4)				

TOTAL WEIGHT = 5 X 173 = 867 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.0012

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17)
CALCULATED VERT. DEFL. (LL) = L/999 (0.08)
ALLOWABLE DEFL. (TL) = L/360 (1.17)
CALCULATED VERT. DEFL. (TL) = L/999 (0.17)

CSI: TC=0.74/1.00 (D-E:1), BC=0.35/1.00 (L-N:1), WB=0.58/1.00 (E-N:1), SSI=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1687 1656

PLATE PLACEMENT TOL. = 0.250 inches

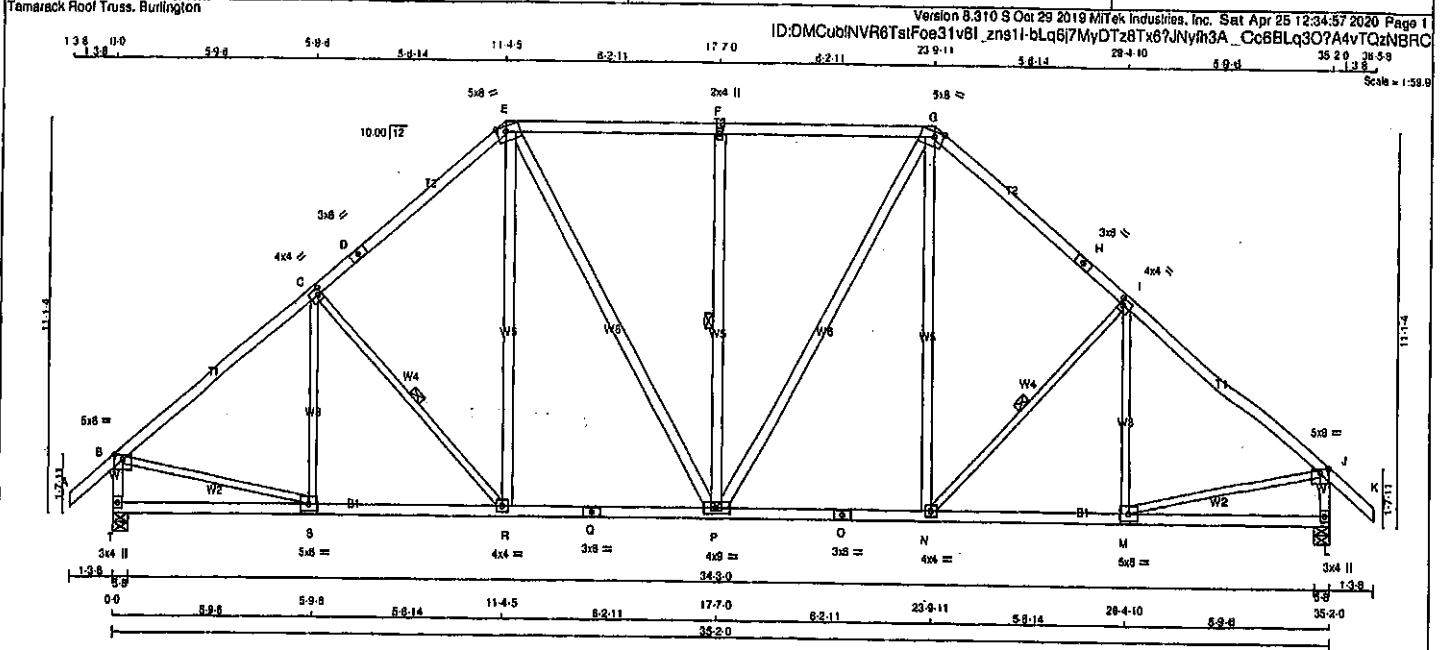
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (H) (INPUT = 0.90)
JSI METAL = 0.46 (O) (INPUT = 1.00)



Structural component only
DWG# T-2007136

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408168	T8	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 12:34:57 2020 Page 1					
ID:DMCubINVR6TstFoe31v8l_zns11-bLq6I7MyDTz8T67JNyth3A_Cc6BLq3O?A4vTQzNBRC					
23-9-11 28-4-10 5-9-4 35-2-0 38-5-9 1-3-8					
Scale = 1:50.0					



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
T - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
T - Q	2x4	DRY	No.2
Q - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
S - C	2x3	DRY	No.2
O - R	2x3	DRY	No.2
N - I	2x3	DRY	No.2
M - I	2x3	DRY	No.2
B - S	2x3	DRY	No.2
M - J	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMWV-p	MT20	5.0	8.0	1.50 3.00
C TMWV-l	MT20	4.0	4.0	2.00 1.25
D TS-l	MT20	3.0	8.0	
E TTWW-m	MT20	5.0	8.0	Edge 3.00
F TMW-w	MT20	2.0	4.0	
G TTWW-m	MT20	6.0	3.0	Edge 3.00
H TS-l	MT20	3.0	8.0	
I TMWV-l	MT20	4.0	4.0	2.00 1.25
J TMWV-p	MT20	5.0	8.0	1.50 3.00
L BMV-l-p	MT20	3.0	4.0	
M BMWV-l	MT20	5.0	8.0	
N BMWV-l	MT20	4.0	4.0	
O BS-l	MT20	3.0	8.0	
P BMWV-l	MT20	4.0	4.0	
Q BS-l	MT20	3.0	8.0	
R BMWV-l	MT20	4.0	4.0	
S BMWV-l	MT20	5.0	8.0	
T BMV-l-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
T	2066	0	2066	0	0	5-8	5-8
L	2066	0	2066	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LOOSE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1458	971/0	0/0	0/0	0/0	488/0	0/0
L	1458	971/0	0/0	0/0	0/0	488/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-R, F-P, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH (FT)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	MAX. UNBRACED LENGTH (FT)	MEMB.
FR-TO		FROM TO			FR-TO				
A-B	0/41	-91.8	-91.8 0.13 (1)	10.00	S-C	-245/10	0.17 (1)		
B-C	-2039/0	-91.8	-91.8 0.50 (1)	4.27	C-R	-314/0	0.18 (1)		
C-D	-1844/0	-91.8	-91.8 0.48 (1)	4.47	R-E	0/338	0.05 (1)		
D-E	-1844/0	-91.8	-91.8 0.48 (1)	4.47	E-P	0/489	0.08 (1)		
E-F	-1821/0	-91.8	-91.8 0.50 (1)	4.60	P-F	-699/0	0.45 (1)		
F-G	-1821/0	-91.8	-91.8 0.50 (1)	4.47	N-G	0/338	0.05 (1)		
G-H	-1844/0	-91.8	-91.8 0.48 (1)	4.47	N-I	-314/0	0.15 (1)		
H-I	-1844/0	-91.8	-91.8 0.48 (1)	4.47	I-J	-245/10	0.17 (1)		
I-J	-2039/0	-91.8	-91.8 0.50 (1)	4.27	M-I	0/1637	0.37 (1)		
J-K	0/41	-91.8	-91.8 0.13 (1)	10.00	B-S	0/1637	0.37 (1)		
T-B	-2021/0	0.0	0.0 0.21 (1)	5.94	M-J	0/1637	0.37 (1)		
L-J	-2021/0	0.0	0.0 0.21 (1)	5.94					
T-S	0/0	-18.5	-18.5 0.14 (4)	10.00					
S-R	0/1597	-18.5	-18.5 0.33 (1)	10.00					
R-Q	0/1387	-18.5	-18.5 0.30 (1)	10.00					
Q-P	0/1387	-18.5	-18.5 0.30 (1)	10.00					
P-O	0/1387	-18.5	-18.5 0.30 (1)	10.00					
O-N	0/1387	-18.5	-18.5 0.30 (1)	10.00					
N-M	0/1597	-18.5	-18.5 0.33 (1)	10.00					
M-L	0/0	-18.5	-18.5 0.14 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 8.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = U/989 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = U/989 (0.14")

CSI: TC=0.50/1.00 (E-F:1), BC=0.33/1.00 (M-N:1), WB=0.45/1.00 (F-P:1), SS1=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS SHEAR=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PS)	SECTION (PL)
MT20	818 354 1887 788 1987 1856	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (J) (INPUT = 0.90)
JSI METAL = 0.47 (O) (INPUT = 1.00)



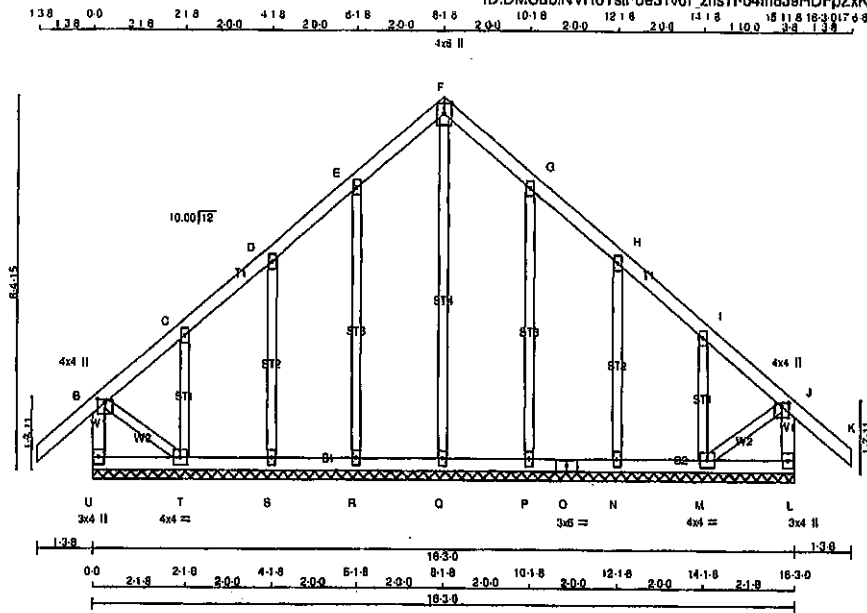
Structural component only
DWG# T-2007137

CONTINUED ON PAGE 2

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408168	G9	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR6TstFoe31v6I_zns11-b4lh8J6HDFpZxKJkpJ9gVTzANmNylCau40SVmwzNBRT



Scale: 1/4" = 1'

TOTAL WEIGHT = 81 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - F	2x4	DRY	No.2
F - K	2x4	DRY	No.2
U - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
U - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C, D, E, G, H, I					
F	TMVW+w	MT20	2.0	4.0	
J	TTW+p	MT20	4.0	8.0	Edge
L	TMVW+p	MT20	4.0	4.0	1.00 2.00
M	BMVW+p	MT20	3.0	4.0	
N, P, Q, R, S					
U	BMVW+p	MT20	2.0	4.0	
O	BS+	MT20	3.0	6.0	
T	BMVW+p	MT20	4.0	4.0	
U	BMVW+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/41	-91.8 -91.8 0.13 (1)	10.00	Q-F	-124/0	0.17 (1)	
B-C	-28/0	-91.8 -91.8 0.05 (1)	6.25	R-E	-210/0	0.16 (1)	
C-D	-38/0	-91.8 -91.8 0.05 (1)	6.25	S-D	-169/0	0.08 (1)	
D-E	-24/0	-91.8 -91.8 0.05 (1)	6.25	T-C	-205/0	0.04 (1)	
E-F	-38/0	-91.8 -91.8 0.05 (1)	6.25	P-G	-210/0	0.18 (1)	
F-G	-38/0	-91.8 -91.8 0.05 (1)	6.25	N-H	-169/0	0.08 (1)	
G-H	-24/0	-91.8 -91.8 0.05 (1)	6.25	M-I	-205/0	0.04 (1)	
H-I	-38/0	-91.8 -91.8 0.05 (1)	6.25	B-T	0/37	0.01 (1)	
I-J	-28/0	-91.8 -91.8 0.05 (1)	6.25	M-J	0/37	0.01 (1)	
J-K	0/41	-91.8 -91.8 0.13 (1)	10.00				
U-B	-248/0	0.0 0.0 0.03 (1)	7.81				
L-J	-248/0	0.0 0.0 0.03 (1)	7.81				
U-T	0/0	-18.5 -18.5 0.02 (4)	10.00				
T-S	0/25	-18.5 -18.5 0.02 (4)	10.00				
S-R	0/21	-18.5 -18.5 0.02 (4)	10.00				
R-Q	0/18	-18.5 -18.5 0.02 (4)	10.00				
Q-P	0/18	-18.5 -18.5 0.02 (4)	10.00				
P-O	0/21	-18.5 -18.5 0.02 (4)	10.00				
O-N	0/21	-18.5 -18.5 0.02 (4)	10.00				
N-M	0/25	-18.5 -18.5 0.02 (4)	10.00				
M-L	0/0	-18.5 -18.5 0.02 (4)	10.00				

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF
SPACING = 24.0 IN. C/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015
THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBO 2012 (2018 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014
(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD
CS: TO=0.13/1.00 (A-B:1), BC=0.02/1.00 (M-N:4), WB=0.17/1.00 (F-Q:1), SS=0.08/1.00 (A-B:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 789 1987 1855
PLATE PLACEMENT TOL = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP = 0.52 (1) (INPUT = 0.90)
JSI METAL = 0.11 (5) (INPUT = 1.00)

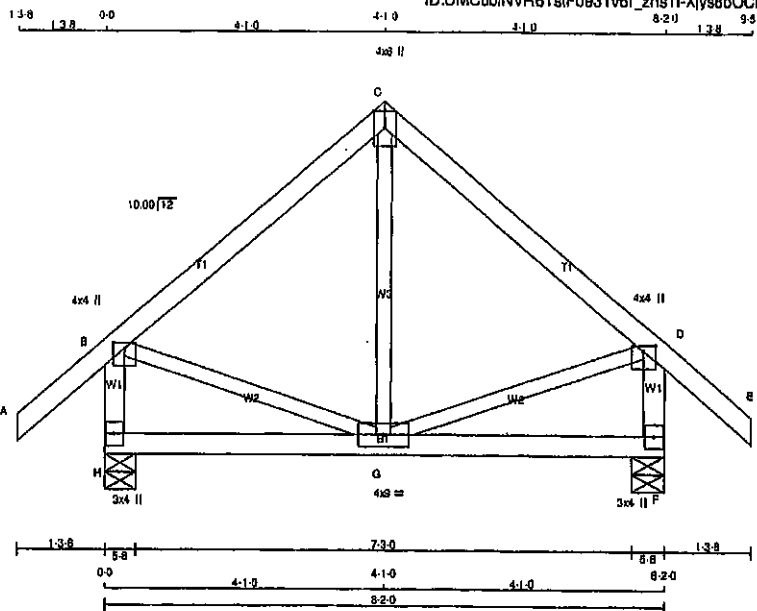


Structural component only
DWG# T-2007123

JOB NAME 408168	TRUSS NAME T10	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:DMCubiNVR6TsiFo831v6l_zns1I-XIys8oOCi4DaiFFNQo77nUGOUQrOpqnhSUZ7XJzNBRAA



Scale = 1/32

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	8.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV+p	MT20	3.0	4.0		
G	BMVWW+p	MT20	4.0	9.0		
H	BMV+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT	VERT. HORZ	DOWN	HORZ	BRG	BRG	IN-SX	IN-SX
H	577 0	577 0	0 0	5-8	5-8		
F	577 0	577 0	0 0	5-8	5-8		

UNFACTORED REACTIONS

JT	1ST LOSE	MAX/MIN	COMPONENT REACTIONS	DEAD	SOIL
H	408	280 / 0	0 / 0	0 / 0	128 / 0
F	408	280 / 0	0 / 0	0 / 0	128 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORZ. LOAD (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 41	-91.8 -91.8	0.13 (1) 10.00
B-C	-284 / 0	-91.8 -91.8	0.20 (1) 8.25
C-D	-284 / 0	-91.8 -91.8	0.20 (1) 8.25
D-E	0 / 41	-91.8 -91.8	0.13 (1) 10.00
E-F	-348 / 0	0.0 0.0	0.68 (1) 7.81
F-G	-348 / 0	0.0 0.0	0.68 (1) 7.81
H-G	0 / 0	-18.5 -18.5	0.09 (4) 10.00
G-F	0 / 0	-18.5 -18.5	0.09 (4) 10.00

TOTAL WEIGHT = 38 lb

(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 8.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPC 2011, TPC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.20/1.00 (B-C:1), BC=0.09/1.00 (F-G:4),
WB=0.06/1.00 (D-G:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 L8 BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

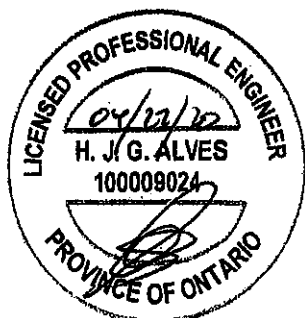
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1667 789 1987 1856	

PLATE PLACEMENT TOL. = 0.260 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.46 (D) (INPUT = 0.80)
JSI METAL = 0.12 (D) (INPUT = 1.00)



Structural component only
DWG# T-2007139

JOB NAME 408168	TRUSS NAME G10	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington
Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Sat Apr 25 12:34:41 2020 Page 1
ID:DMCubINVR6TsiFoe31v6f_zns1f-3Gs3MeAv_YyQYUwN0gv2hWL7A[CchL2JgC3vMzNBRS

Scale = 1/20'

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
O - G	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
L - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					
GABLE STUDS SPACED AT 2'-0" O.C.					

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C TMW+w	MT20	2.0	4.0		
D TTW+p	MT20	4.0	6.0	Edge	
E TMW+w	MT20	2.0	4.0		
F TMVW+p	MT20	4.0	4.0	1.00	2.00
H BMV1+p	MT20	3.0	4.0		
I BMW1-i	MT20	4.0	4.0		
J BMW1+w	MT20	2.0	4.0		
K BMW1-i	MT20	4.0	4.0		
L BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	FACTORED CS (LC)
L-B	-224 / 0	0.0	0.02 (1)	J-D	-126 / 0	0.05 (1)	0.05 (1)
A-B	0 / 41	-91.8	-91.8 0.13 (1)	K-C	-227 / 0	0.04 (1)	0.04 (1)
B-C	-11 / 0	-91.8	-91.8 0.08 (1)	I-E	-227 / 0	0.04 (1)	0.04 (1)
C-D	-31 / 0	-91.8	-91.8 0.08 (1)	B-K	0 / 24	0.01 (1)	0.01 (1)
D-E	-31 / 0	-91.8	-91.8 0.08 (1)	I-F	0 / 24	0.01 (1)	0.01 (1)
E-F	-11 / 0	-91.8	-91.8 0.08 (1)				
F-G	0 / 41	-91.8	-91.8 0.13 (1)				
H-F	-224 / 0	0.0	0.02 (1)				
L-K	0 / 0	-18.5	-18.5 0.02 (4)				
K-J	0 / 13	-18.5	-18.5 0.02 (4)				
J-I	0 / 13	-18.5	-18.5 0.02 (4)				
I-H	0 / 0	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2015 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (J-K:4), WB=0.05/1.00 (D-J:1), SS=0.08/1.00 (A-B:1)

OOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.17 (C) (INPUT = 0.80)

JSI METAL = 0.12 (C) (INPUT = 1.00)

TOTAL WEIGHT = 40 lb

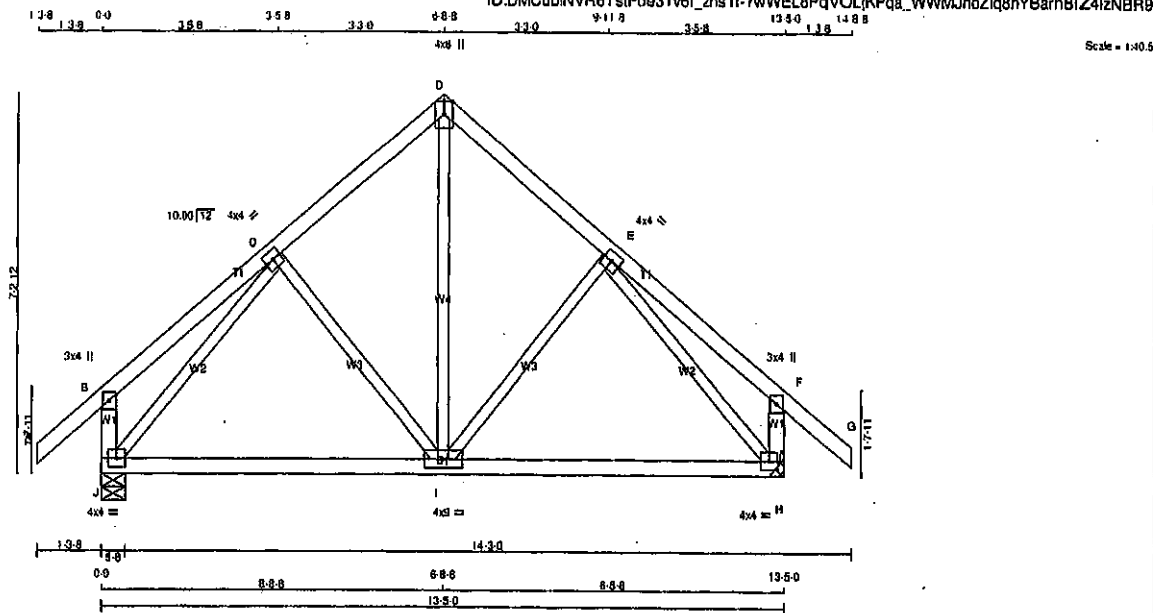
Structural component only

DWG# T-2007124

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408168	T11	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 64 = 191 Lb [MAX]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	FACTORED
D - G	2x4	DRY	No.2	SPF	GROSS REACTION
J - B	2x4	DRY	No.2	SPF	MAXIMUM FACTORED
H - F	2x4	DRY	No.2	SPF	INPUT
J - H	2x4	DRY	No.2	SPF	RECORD

ALL WEBS	2x3	DRY	No.2	SPF	A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.
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DRY: SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-1	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	8.0		Edge
E	TMWW-1	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-1	MT20	4.0	4.0		
I	BMWWW-1	MT20	4.0	8.0		
J	BMVW1-1	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
J	VERT	HORZ	DOWN	HORZ
J	887	0	887	0
H	887	0	887	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED							
J	811	414 / 0	0 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0
H	811	414 / 0	0 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MEMB.	FORCE	MAX
(LBS)	(LBS)	(PLP)	CSI (LC)	(LBS)	(LBS)	CSI (LC)
FR-TO		FROM	TO	FR-TO		
A-B	0 / 41	-81.8	-81.8 0.13 (1)	10.00	I-D	0 / 382
B-C	0 / 23	-81.8	-81.8 0.18 (1)	10.00	I-E	-149 / 0
C-D	-509 / 0	-81.8	-81.8 0.13 (1)	8.25	C-I	-149 / 0
D-E	-509 / 0	-81.8	-81.8 0.13 (1)	8.25	J-C	-742 / 0
E-F	0 / 23	-81.8	-81.8 0.18 (1)	10.00	E-H	-742 / 0
F-G	0 / 41	-81.8	-81.8 0.13 (1)	10.00		
J-B	-244 / 0	0.0	0.0 0.03 (1)	7.81		
H-F	-244 / 0	0.0	0.0 0.03 (1)	7.81		
J-I	0 / 469	-18.5	-18.5 0.27 (4)	10.00		
I-H	0 / 469	-18.5	-18.5 0.27 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.45")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.45")
CALCULATED VERT. DEFL. (TL) = L/999 (0.05")

CSI: TC=0.16/1.00 (E-F:1), GC=0.27/1.00 (I-J:4),
WB=0.33/1.00 (E-H:1), SSI=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

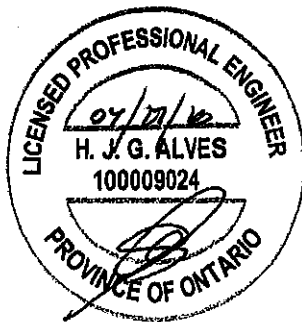
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1686

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

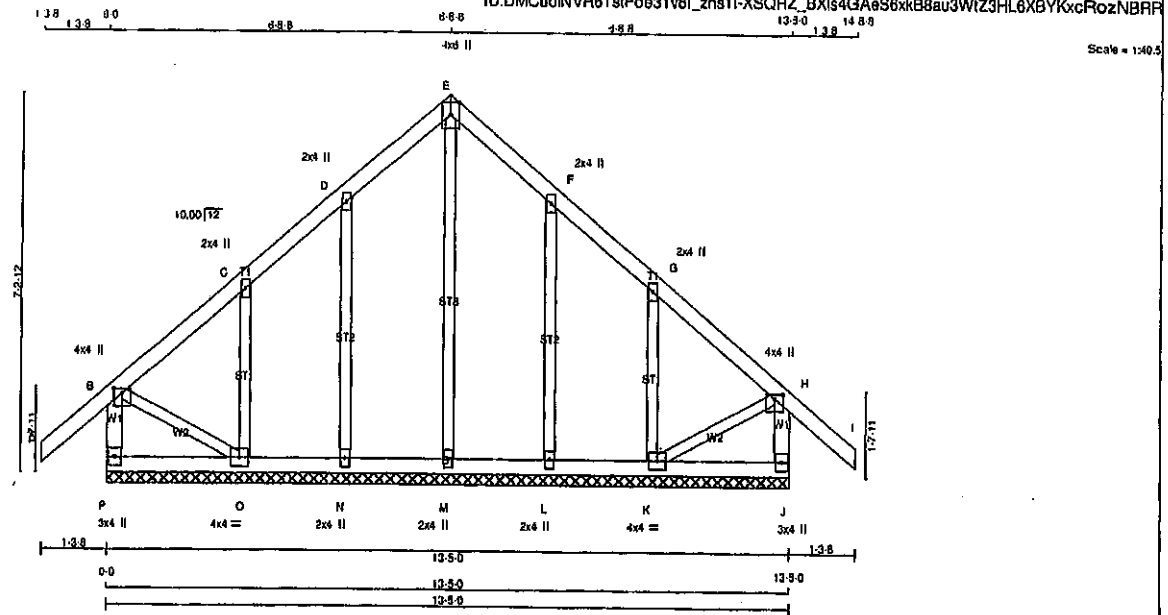
JSI GRIP = 0.84 (E) (INPUT = 0.90)
JSI METAL = 0.27 (E) (INPUT = 1.00)



Structural component only
DWG# T-2007140

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408168	G11	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

Version 8.310 S Oct 28 2019 MITek Industries, Inc. Sat Apr 25 12:34:42 2020 Page 1
ID:DMCubINVR6TstFoe31v6L_zns1I-XSQRZ_BXIs4GAeS6xk88au3WIZ3HL6XBYKxcRozNBRR



TOTAL WEIGHT = 66 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF	
A - E	2x4	DRY	No.2	SPF	
E - I	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
P - J	2x4	DRY	No.2	SPF	

ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMW+p	MT20	4.0	4.0	1.00	2.00	
C, D, F, G						
C TMW+w	MT20	2.0	4.0			
E TTW+p	MT20	4.0	6.0	Edge		
H TMW+p	MT20	4.0	4.0	1.00	2.00	
J BMV1+p	MT20	3.0	4.0			
K BMW1-i	MT20	4.0	4.0			
L, M, N						
L BMW1+w	MT20	2.0	4.0			
O BMW1-i	MT20	4.0	4.0			
P BMV1+p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	UNBRAC LENGTH
FR-TO				FR-TO			
P-B	-283 / 0	0.0	0.03 (1)	M-E	-128 / 0	0.11 (1)	7.81
A-B	0 / 41	-91.8	-91.8 0.13 (1)	N-D	-190 / 0	0.09 (1)	10.00
B-C	-24 / 0	-91.8	-91.8 0.08 (1)	O-C	-239 / 0	0.08 (1)	6.25
C-D	-38 / 0	-91.8	-91.8 0.08 (1)	L-F	-190 / 0	0.09 (1)	6.25
D-E	-35 / 0	-91.8	-91.8 0.08 (1)	K-G	-239 / 0	0.08 (1)	6.25
E-F	-35 / 0	-91.8	-91.8 0.08 (1)	B-O	0 / 32	0.01 (1)	6.25
F-G	-38 / 0	-91.8	-91.8 0.08 (1)	K-H	0 / 32	0.01 (1)	6.25
G-H	-24 / 0	-91.8	-91.8 0.08 (1)				
H-I	0 / 41	-91.8	-91.8 0.13 (1)				10.00
J-H	-283 / 0	0.0	0.03 (1)				7.81
P-O	0 / 0	-18.5	-18.5 0.03 (4)				10.00
O-N	0 / 22	-18.5	-18.5 0.03 (4)				10.00
N-M	0 / 19	-18.5	-18.5 0.02 (4)				10.00
M-L	0 / 19	-18.5	-18.5 0.02 (4)				10.00
L-K	0 / 22	-18.5	-18.5 0.03 (4)				10.00
K-J	0 / 0	-18.5	-18.5 0.03 (4)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.8	PSF
DL	=	8.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (H-1), BC=0.03/1.00 (K-L-4), WB=0.11/1.00 (E-M-1), SS=0.08/1.00 (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PLI)	SECTION (PLI)
MAX MIN MAX MIN MAX MIN		
MT20	618 354 1887 786 1987 1858	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

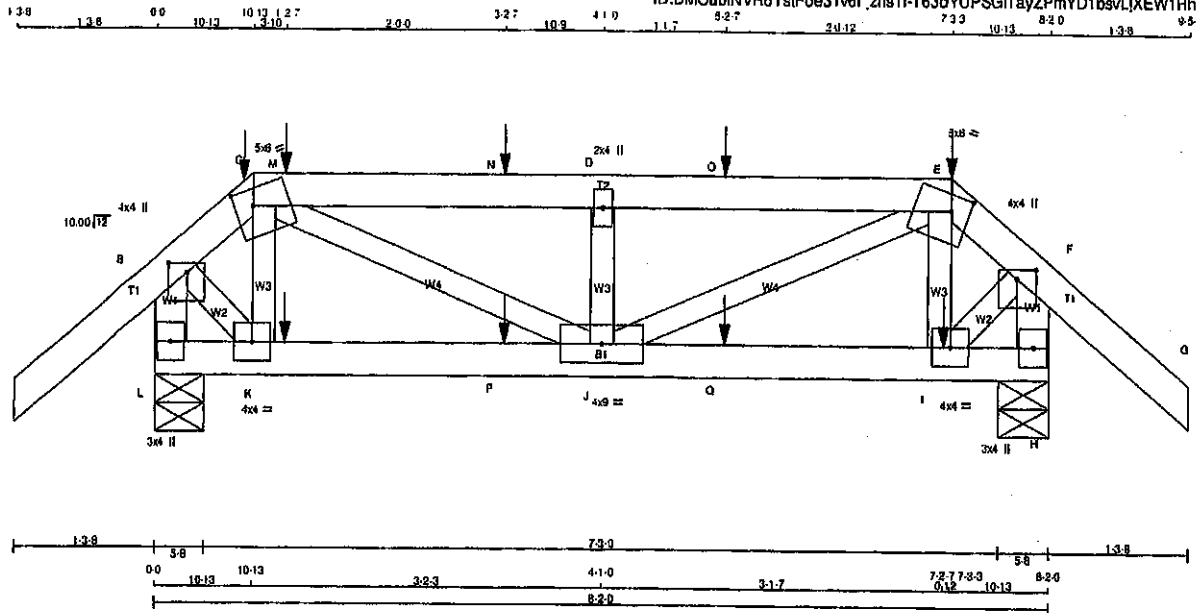
JSI GRIP = 0.20 (B) (INPUT = 0.90)
JSI METAL = 0.13 (G) (INPUT = 1.00)



Structural component only
DWG# T-2007125

JOB NAME 408168	TRUSS NAME T12	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Sat Apr 25 12:35:01 2020 Page 1
ID:DMCubINVR8Ts(Foe31v8l_zns1f-T63dYUPSGITayZpMYD1bsvLJXEW1Hh1_vn26cCzNBR8



Scale = 1/8" = 1'-0"

LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C	TTWW+m	MT20	6.0	8.0	1.75 2.00
D	TMVW+w	MT20	2.0	4.0	
E	TTWW-m	MT20	5.0	8.0	1.75 2.00
F	TMVW+p	MT20	4.0	4.0	1.00 2.00
H	BMV1+p	MT20	3.0	4.0	
I	BMVW-i	MT20	4.0	4.0	
J	BMVWW-i	MT20	4.0	9.0	
K	BMVW-i	MT20	4.0	4.0	
L	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
L	913	0	913	0	5-8
H	911	0	911	0	5-8

UNFACTORED REACTIONS		1ST CASE	MAX MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
L	642	442 / 0	0 / 0	0 / 0	0 / 0	199 / 0	0 / 0	
H	641	437 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO			
A-B	0 / 41	-91.8 -91.8 0.14 (1)	10.00
B-C	-631 / 0	-91.8 -91.8 0.14 (1)	6.25
C-M	-1054 / 0	-91.8 -91.8 0.23 (1)	5.82
M-N	-1054 / 0	-91.8 -91.8 0.23 (1)	5.82
N-D	-1054 / 0	-91.8 -91.8 0.23 (1)	5.82
D-O	-1054 / 0	-91.8 -91.8 0.23 (1)	5.82
O-E	-1054 / 0	-91.8 -91.8 0.23 (1)	5.82
E-F	-632 / 0	-91.8 -91.8 0.14 (1)	6.25
F-G	0 / 41	-91.8 -91.8 0.14 (1)	10.00
L-B	-938 / 0	0.0 0.0 0.10 (1)	7.81
H-F	-937 / 0	0.0 0.0 0.10 (1)	7.81
L-K	0 / 0	-18.5 -18.5 0.05 (1)	10.00
K-P	0 / 420	-18.5 -18.5 0.13 (1)	10.00
P-J	0 / 420	-18.5 -18.5 0.13 (1)	10.00
J-Q	0 / 421	-18.5 -18.5 0.14 (1)	10.00
Q-I	0 / 421	-18.5 -18.5 0.14 (1)	10.00
I-H	0 / 0	-18.5 -18.5 0.05 (1)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	10-13	-9	-10	---	FRONT	VERT	DEAD	---	C1
C	1-2-7	-114	-114	---	BACK	VERT	TOTAL	---	C1
C	10-13	-44	-44	---	FRONT	VERT	SNOW	---	C1
E	7-3-3	-9	-10	---	FRONT	VERT	DEAD	---	C1
E	7-3-3	-90	-90	---	BACK	VERT	TOTAL	---	C1
E	7-3-3	-44	-44	---	FRONT	VERT	SNOW	---	C1
I	7-2-7	-49	-49	---	BACK	VERT	TOTAL	---	C1
K	1-2-7	-49	-49	---	BACK	VERT	TOTAL	---	C1
N	3-2-7	-82	-82	---	BACK	VERT	TOTAL	---	C1
O	5-2-7	-82	-82	---	BACK	VERT	TOTAL	---	C1
P	3-2-7	-49	-49	---	BACK	VERT	TOTAL	---	C1
Q	5-2-7	-49	-49	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 34 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = L/360 (0.27")
CALCULATED VERT. DEFL (LL) = L/999 (0.02")
ALLOWABLE DEFL (TL) = L/360 (0.27")
CALCULATED VERT. DEFL (TL) = L/999 (0.03")

CSK: TC=0.23/1.00 (C-D:1), BC=0.14/1.00 (I-J:1),
WB=0.17/1.00 (C-J:1), SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1667 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

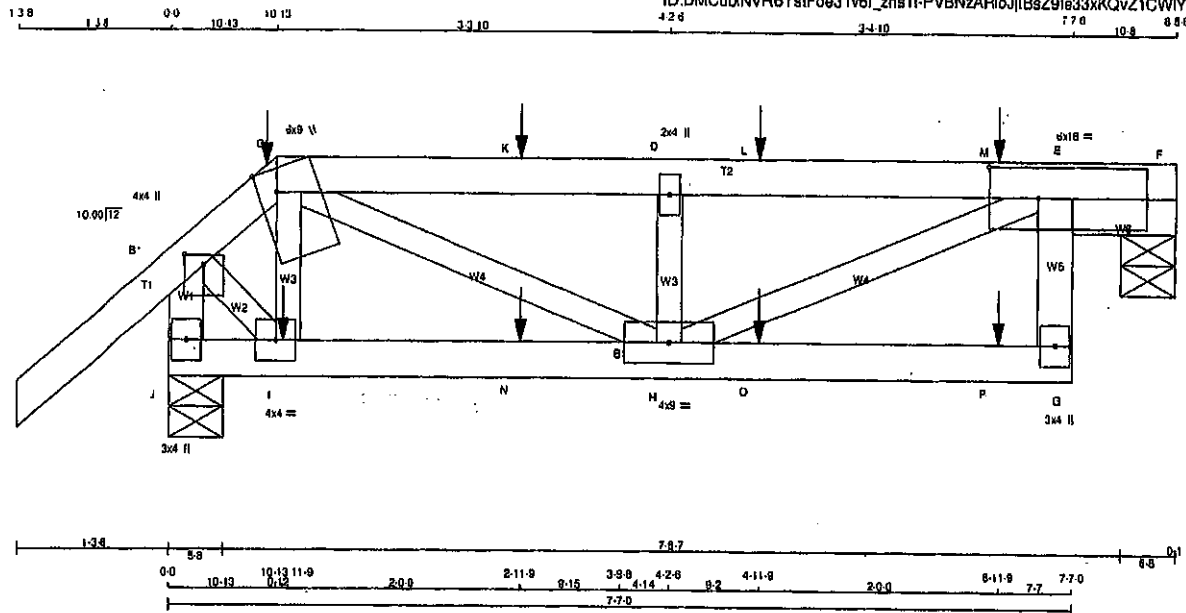
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Structural component only
DWG# T-2007141

JOB NAME 408168	TRUSS NAME T13	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.310 5 Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 12:35:03 2020 Page 1
ID:DMCubtNVR6TsIFo631v6I_zns11-PVBnzAFioJlIBsZ9ie33xKQvZ1CWYJHN5XDq4zNBR6



Scale = 1:17.3

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
E - F	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - B	2x4	DRY	No.2
J - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
SPF DRY: SEASONED LUMBER.

PLATES (tablets in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C	TTWW+m	MT20	6.0	8.0	Edge 1.75
D	TMVW+w	MT20	2.0	4.0	
E	TMVW-w	MT20	6.0	16.0	3.00 5.00
G	BMV+p	MT20	3.0	4.0	
H	BMVW-w	MT20	4.0	9.0	
I	BMVW-w	MT20	4.0	4.0	
J	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	N-SX	IN-SX	N-SX	IN-SX
F	695	0	695	0	0	5-8	5-8		
J	936	0	936	0	0	5-8	5-8		

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): F

UNFACTORED REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	490	327.0	0.0	0.0	0.0	163.0	0.0
J	659	449.0	0.0	0.0	0.0	210.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED (CSI (LC))	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (CSI (LC))	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0.141	-91.8	-91.8 0.14 (1)	10.00	I-C	-158.0	0.03 (1)
B-C	-558.0	-91.8	-91.8 0.14 (1)	8.25	B-I	0.548	0.14 (1)
C-K	-1230.0	-91.8	-91.8 0.48 (1)	5.09	H-D	-767.0	0.12 (1)
K-D	-1230.0	-91.8	-91.8 0.48 (1)	5.09	C-H	0.887	0.21 (1)
D-L	-1230.0	-91.8	-91.8 0.89 (1)	3.56	I-E	0.1345	0.33 (1)
L-M	-1230.0	-91.8	-91.8 0.89 (1)	3.56			
M-E	-1230.0	-91.8	-91.8 0.89 (1)	3.56			
E-F	0.0	-91.8	-91.8 0.35 (1)	10.00			
G-E	0.83	0.0	0.0 0.02 (4)	10.00			
J-B	-968.0	0.0	0.0 0.11 (1)	7.81			
J-I	0.0	-18.5	-18.5 0.03 (1)	10.00			
I-N	0.438	-18.5	-18.5 0.14 (1)	10.00			
N-H	0.438	-18.5	-18.5 0.14 (1)	10.00			
H-O	0.0	-18.5	-18.5 0.09 (1)	10.00			
O-P	0.0	-18.5	-18.5 0.09 (1)	10.00			
P-G	0.0	-18.5	-18.5 0.09 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
C	10-13	-	-10	-	FRONT	VERT	DEAD
C	10-13	-90	-90	-	FRONT	VERT	TOTAL
C	10-13	-44	-44	-	FRONT	VERT	SNOW
I	11-9	-49	-49	-	FRONT	VERT	TOTAL
K	2-11-9	-82	-82	-	FRONT	VERT	TOTAL
L	4-11-9	-82	-82	-	FRONT	VERT	TOTAL
M	6-11-9	-82	-82	-	FRONT	VERT	TOTAL
N	2-11-9	-49	-49	-	FRONT	VERT	TOTAL
O	4-11-9	-49	-49	-	FRONT	VERT	TOTAL
P	6-11-9	-53	-53	-	FRONT	VERT	TOTAL

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/688 (0.13")

CSI: TC=0.89/1.00 (D-E:1), BC=0.14/1.00 (H-I:1), WB=0.33/1.00 (E-H:1), SSI=0.42/1.00 (D-E:1)

DCL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

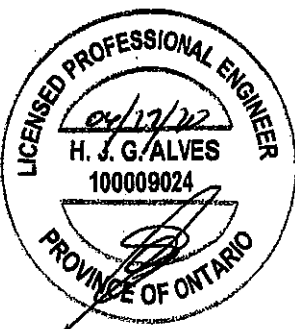
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

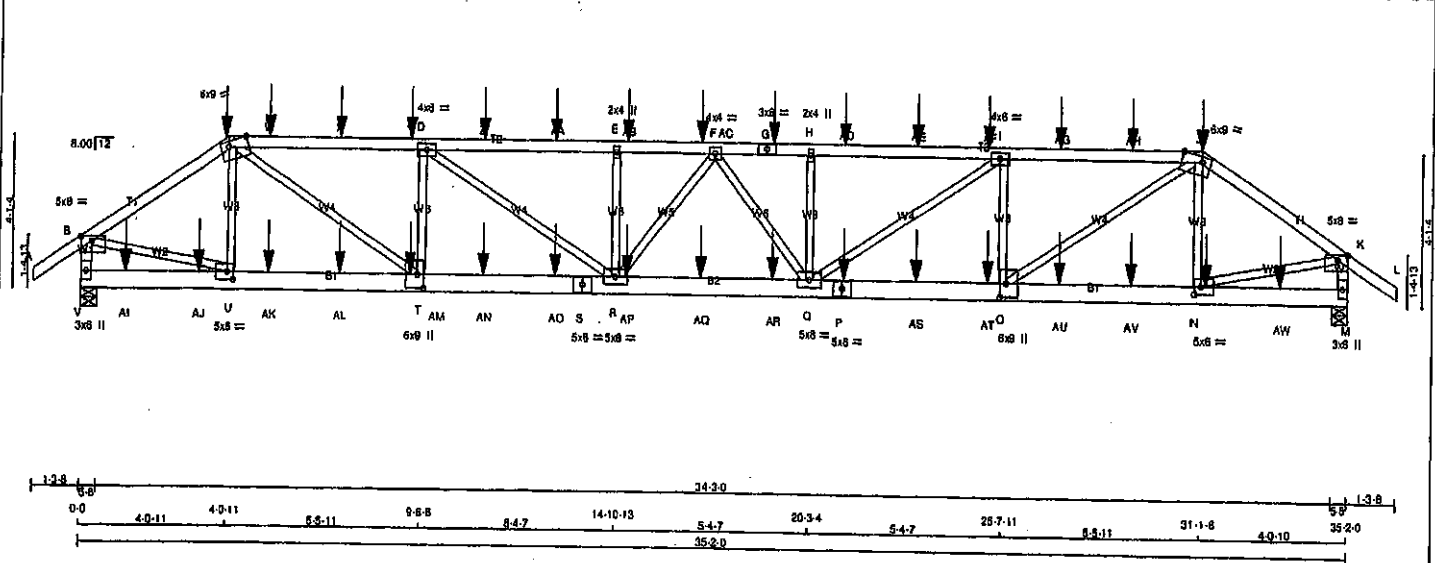
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.79 (C) (INPUT = 0.90)
JSI METAL = 0.30 (H) (INPUT = 1.00)

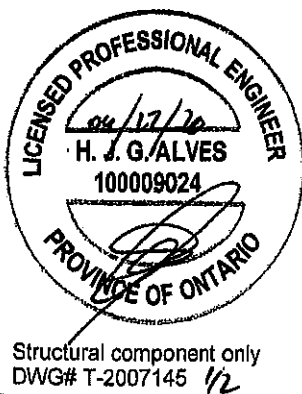


Structural component only
DWG# T-2007142

JOB NAME 408169	TRUSS NAME T20	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 12:49:24 2020 Page 1 ID:K7TPdghj0npl1qYdWIOFzIdeG-Hlgasnl8B7_Budbz0zKub4B8zN_xr0M2bHOgzNBD	



LUMBER N. L. G. A. RULES CHORDS <table> <tr> <th>SIZE</th> <th>DRY</th> <th>LUMBER</th> <th>DESCR.</th> </tr> <tr> <td>A - C</td> <td>2x4</td> <td>No.2</td> <td>SPF</td> </tr> <tr> <td>C - G</td> <td>2x4</td> <td>DRY</td> <td>SPF</td> </tr> <tr> <td>G - J</td> <td>2x4</td> <td>DRY</td> <td>SPF</td> </tr> <tr> <td>J - L</td> <td>2x4</td> <td>DRY</td> <td>SPF</td> </tr> <tr> <td>V - B</td> <td>2x4</td> <td>DRY</td> <td>SPF</td> </tr> <tr> <td>M - K</td> <td>2x4</td> <td>DRY</td> <td>SPF</td> </tr> <tr> <td>V - S</td> <td>2x8</td> <td>DRY</td> <td>SPF</td> </tr> <tr> <td>S - P</td> <td>2x8</td> <td>DRY</td> <td>SPF</td> </tr> <tr> <td>P - M</td> <td>2x8</td> <td>DRY</td> <td>SPF</td> </tr> </table> ALL WEBS EXCEPT 2x3 DRY No.2 SPF		SIZE	DRY	LUMBER	DESCR.	A - C	2x4	No.2	SPF	C - G	2x4	DRY	SPF	G - J	2x4	DRY	SPF	J - L	2x4	DRY	SPF	V - B	2x4	DRY	SPF	M - K	2x4	DRY	SPF	V - S	2x8	DRY	SPF	S - P	2x8	DRY	SPF	P - M	2x8	DRY	SPF	DESIGN CRITERIA *** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE SPECIFIED LOADS: TOP CH. LL = 25.6 PSF OL = 8.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 39.0 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12 *** NON STANDARD GIRDER *** ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, CBC 2012, ABC 2019 - PART 9 OF CBC 2012 (2019 AMENDMENT) - CSA 088-09, CSA 088-14 - TPIC 2011, TPIC 2014 (55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (1.17") CALCULATED VERT. DEFL.(LL) = L/999 (0.23") ALLOWABLE DEFL.(TL) = L/380 (1.17") CALCULATED VERT. DEFL.(TL) = L/984 (0.44") CSI: TC=0.71/1.00 (D-E:1), BC=0.59/1.00 (O-R:1), WB=0.44/1.00 (C-T:1), SS=0.23/1.00 (C-O:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 818 354 1667 768 1907 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP = 0.71 (G) (INPUT = 0.80) JSI METAL = 0.59 (S) (INPUT = 1.00)																																																																																																																																												
SIZE	DRY	LUMBER	DESCR.																																																																																																																																																																																			
A - C	2x4	No.2	SPF																																																																																																																																																																																			
C - G	2x4	DRY	SPF																																																																																																																																																																																			
G - J	2x4	DRY	SPF																																																																																																																																																																																			
J - L	2x4	DRY	SPF																																																																																																																																																																																			
V - B	2x4	DRY	SPF																																																																																																																																																																																			
M - K	2x4	DRY	SPF																																																																																																																																																																																			
V - S	2x8	DRY	SPF																																																																																																																																																																																			
S - P	2x8	DRY	SPF																																																																																																																																																																																			
P - M	2x8	DRY	SPF																																																																																																																																																																																			
ALL WEBS EXCEPT 2x3 DRY No.2 SPF DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF) TOP CHORDS: (0.122"x3") SPIRAL NAILS <table> <tr> <td>A - C</td> <td>1</td> <td>12</td> <td>SIDE(81.0)</td> </tr> <tr> <td>C - G</td> <td>1</td> <td>12</td> <td>SIDE(81.0)</td> </tr> <tr> <td>G - J</td> <td>1</td> <td>12</td> <td>SIDE(81.0)</td> </tr> <tr> <td>J - L</td> <td>1</td> <td>12</td> <td>SIDE(81.0)</td> </tr> <tr> <td>V - B</td> <td>1</td> <td>12</td> <td>TOP</td> </tr> <tr> <td>M - K</td> <td>1</td> <td>12</td> <td>TOP</td> </tr> </table> BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS <table> <tr> <td>V - S</td> <td>2</td> <td>12</td> <td>SIDE(183.1)</td> </tr> <tr> <td>S - P</td> <td>2</td> <td>12</td> <td>SIDE(183.1)</td> </tr> <tr> <td>P - M</td> <td>2</td> <td>12</td> <td>SIDE(183.1)</td> </tr> </table> WEBS: (0.122"x3") SPIRAL NAILS <table> <tr> <td>2x3</td> <td>1</td> <td>6</td> <td></td> </tr> </table> NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.		A - C	1	12	SIDE(81.0)	C - G	1	12	SIDE(81.0)	G - J	1	12	SIDE(81.0)	J - L	1	12	SIDE(81.0)	V - B	1	12	TOP	M - K	1	12	TOP	V - S	2	12	SIDE(183.1)	S - P	2	12	SIDE(183.1)	P - M	2	12	SIDE(183.1)	2x3	1	6		DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table> <tr> <th>FACTORED</th> <th>MAXIMUM FACTORED</th> <th>INPUT</th> <th>REQD</th> </tr> <tr> <th>GROSS REACTION</th> <th>GROSS REACTION</th> <th>BRG</th> <th>BRG</th> </tr> <tr> <td>JT VERT</td> <td>3342</td> <td>0</td> <td>5-8</td> </tr> <tr> <td>V</td> <td>3342</td> <td>0</td> <td>5-8</td> </tr> <tr> <td>M</td> <td>3393</td> <td>0</td> <td>5-8</td> </tr> </table> UNFACTORED REACTIONS <table> <tr> <th>1ST CASE</th> <th>MAX/MIN COMPONENT REACTIONS</th> </tr> <tr> <th>COMBINED</th> <th>LIVE PERMLIVE WIND DEAD SOIL</th> </tr> <tr> <td>JT</td> <td>2363 1855 / 0 0 / 0 0 / 0 808 / 0 0 / 0</td> </tr> <tr> <td>V</td> <td>2363 1855 / 0 0 / 0 0 / 0 808 / 0 0 / 0</td> </tr> <tr> <td>M</td> <td>2397 1885 / 0 0 / 0 0 / 0 812 / 0 0 / 0</td> </tr> </table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.03 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. LOADING TOTAL LOAD CASES: (4) <table> <tr> <th>CHORDS</th> <th>FACTORED</th> <th>WEBS</th> </tr> <tr> <th>MEMB.</th> <th>FORCE (LBS)</th> <th>FORCE (LBS)</th> </tr> <tr> <td>FR-TO</td> <td>FROM TO</td> <td>FR-TO</td> </tr> <tr> <td>A-B</td> <td>0 / 35</td> <td>U-C</td> <td>-598 / 0</td> </tr> <tr> <td>B-C</td> <td>-3851 / 0</td> <td>C-T</td> <td>0 / 3530</td> </tr> <tr> <td>C-W</td> <td>-6186 / 0</td> <td>T-D</td> <td>-1823 / 0</td> </tr> <tr> <td>W-X</td> <td>-6186 / 0</td> <td>D-R</td> <td>0 / 1802</td> </tr> <tr> <td>X-Y</td> <td>-6186 / 0</td> <td>R-E</td> <td>-602 / 0</td> </tr> <tr> <td>Y-D</td> <td>-6186 / 0</td> <td>E-H</td> <td>-598 / 0</td> </tr> <tr> <td>D-Z</td> <td>-7502 / 0</td> <td>O-I</td> <td>0 / 1573</td> </tr> <tr> <td>Z-AA</td> <td>-7502 / 0</td> <td>I-H</td> <td>-1608 / 0</td> </tr> <tr> <td>AA-E</td> <td>-7502 / 0</td> <td>C-J</td> <td>0 / 3493</td> </tr> <tr> <td>E-AB</td> <td>-7502 / 0</td> <td>N-J</td> <td>-618 / 0</td> </tr> <tr> <td>AB-AC</td> <td>-7502 / 0</td> <td>B-U</td> <td>0 / 3374</td> </tr> <tr> <td>AC-F</td> <td>-7502 / 0</td> <td>N-K</td> <td>0 / 3439</td> </tr> <tr> <td>F-G</td> <td>-7509 / 0</td> <td>R-F</td> <td>-179 / 0</td> </tr> <tr> <td>G-H</td> <td>-7509 / 0</td> <td>F-Q</td> <td>-166 / 0</td> </tr> <tr> <td>H-AD</td> <td>-7509 / 0</td> <td></td> <td></td> </tr> <tr> <td>AD-AE</td> <td>-7509 / 0</td> <td></td> <td></td> </tr> <tr> <td>AE-AF</td> <td>-7509 / 0</td> <td></td> <td></td> </tr> <tr> <td>AF-I</td> <td>-7509 / 0</td> <td></td> <td></td> </tr> <tr> <td>I-AG</td> <td>-8217 / 0</td> <td></td> <td></td> </tr> <tr> <td>AG-AH</td> <td>-8217 / 0</td> <td></td> <td></td> </tr> <tr> <td>AH-J</td> <td>-8217 / 0</td> <td></td> <td></td> </tr> <tr> <td>J-K</td> <td>-4025 / 0</td> <td></td> <td></td> </tr> <tr> <td>K-L</td> <td>0 / 35</td> <td></td> <td></td> </tr> <tr> <td>V-B</td> <td>-3288 / 0</td> <td></td> <td></td> </tr> <tr> <td>M-K</td> <td>-3356 / 0</td> <td></td> <td></td> </tr> </table>		FACTORED	MAXIMUM FACTORED	INPUT	REQD	GROSS REACTION	GROSS REACTION	BRG	BRG	JT VERT	3342	0	5-8	V	3342	0	5-8	M	3393	0	5-8	1ST CASE	MAX/MIN COMPONENT REACTIONS	COMBINED	LIVE PERMLIVE WIND DEAD SOIL	JT	2363 1855 / 0 0 / 0 0 / 0 808 / 0 0 / 0	V	2363 1855 / 0 0 / 0 0 / 0 808 / 0 0 / 0	M	2397 1885 / 0 0 / 0 0 / 0 812 / 0 0 / 0	CHORDS	FACTORED	WEBS	MEMB.	FORCE (LBS)	FORCE (LBS)	FR-TO	FROM TO	FR-TO	A-B	0 / 35	U-C	-598 / 0	B-C	-3851 / 0	C-T	0 / 3530	C-W	-6186 / 0	T-D	-1823 / 0	W-X	-6186 / 0	D-R	0 / 1802	X-Y	-6186 / 0	R-E	-602 / 0	Y-D	-6186 / 0	E-H	-598 / 0	D-Z	-7502 / 0	O-I	0 / 1573	Z-AA	-7502 / 0	I-H	-1608 / 0	AA-E	-7502 / 0	C-J	0 / 3493	E-AB	-7502 / 0	N-J	-618 / 0	AB-AC	-7502 / 0	B-U	0 / 3374	AC-F	-7502 / 0	N-K	0 / 3439	F-G	-7509 / 0	R-F	-179 / 0	G-H	-7509 / 0	F-Q	-166 / 0	H-AD	-7509 / 0			AD-AE	-7509 / 0			AE-AF	-7509 / 0			AF-I	-7509 / 0			I-AG	-8217 / 0			AG-AH	-8217 / 0			AH-J	-8217 / 0			J-K	-4025 / 0			K-L	0 / 35			V-B	-3288 / 0			M-K	-3356 / 0		
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Structural component only
DWG# T-2007145 1/2

JOB NAME 408169	TRUSS NAME T20	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TTWW-m	MT20	8.0	9.0	Edge	6.25
D	TMVW-l	MT20	4.0	6.0		
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-w	MT20	2.0	4.0		
I	TMVW-l	MT20	4.0	6.0		
J	TTWW-m	MT20	8.0	9.0	Edge	6.25
K	TMVW-p	MT20	5.0	8.0	1.50	3.50
M	BMV1-p	MT20	3.0	6.0		
N	BMVW-t	MT20	5.0	8.0	2.50	2.00
O	BMVW-t	MT20	8.0	9.0	4.50	2.00
P	BS-t	MT20	5.0	8.0		
Q	BMVW-w-t	MT20	5.0	8.0		
R	BMVW-w-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
T	BMVW-t	MT20	8.0	9.0	4.50	2.00
U	BMVW-t	MT20	5.0	8.0		
V	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC. LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
AS-AT	0/6217	-18.5	-18.5 0.48 (1)	10.00			
AT-O	0/6217	-18.5	-18.5 0.48 (1)	10.00			
O-AU	0/3328	-18.5	-18.5 0.28 (1)	10.00			
AU-AV	0/3328	-18.5	-18.5 0.28 (1)	10.00			
AV-N	0/3328	-18.5	-18.5 0.28 (1)	10.00			
N-AW	0/0	-18.5	-18.5 0.04 (4)	10.00			
AW-M	0/0	-18.5	-18.5 0.04 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-49	-55	---	FRONT	VERT	DEAD	---	C1
G	4-0-11	-264	-284	---	FRONT	VERT	SNOW	---	C1
G	19-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
J	31-1-8	-49	-55	---	FRONT	VERT	DEAD	---	C1
J	31-1-8	-117	-117	---	BACK	VERT	TOTAL	---	C1
J	31-1-8	-284	-284	---	FRONT	VERT	SNOW	---	C1
N	31-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
P	21-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
W	5-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
X	7-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
Y	9-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
Z	11-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
AA	13-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
AB	15-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
AO	17-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
AD	21-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
AE	23-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
AF	25-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
AG	27-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
AH	29-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
AI	1-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AJ	3-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AK	5-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AL	7-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AM	9-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AN	11-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AO	13-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AP	15-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AQ	17-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AR	19-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AS	23-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AT	25-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AU	27-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AV	29-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AW	33-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



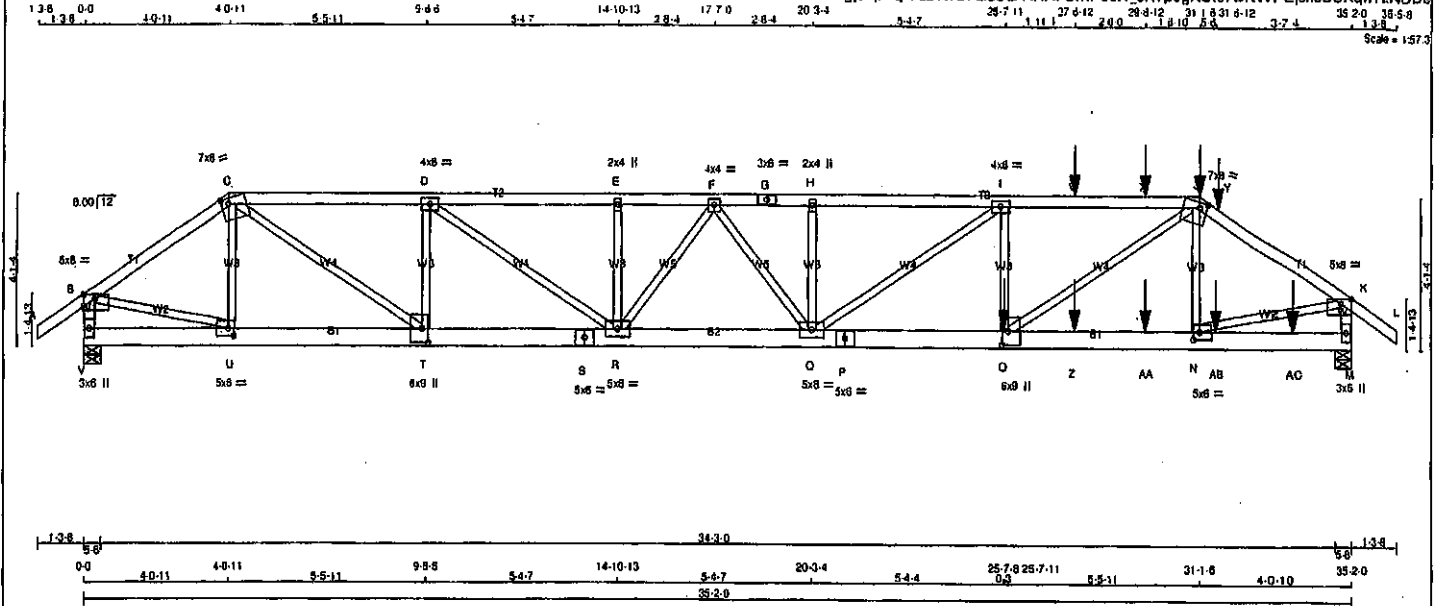
Structural component only
DWG# T-2007145

JOB NAME 408169	TRUSS NAME T20Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:K7TFdngj0npl1qYdbWIOFzIdG-AKHFWntPo9K_cKTP9gXC167DKWF2Jk9bCKqW7zNB0s



CHORDS	SIZE	LUMBER	DESOR.
A - C	2x4 DRY	No.2	SPF
C - G	2x4 DRY	No.2	SPF
G - J	2x4 DRY	No.2	SPF
J - L	2x4 DRY	No.2	SPF
L - V	2x4 DRY	No.2	SPF
V - B	2x4 DRY	No.2	SPF
B - K	2x4 DRY	No.2	SPF
K - S	2x8 DRY	No.2	SPF
S - P	2x8 DRY	No.2	SPF
P - M	2x8 DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
C-G	12	TOP
G-J	12	SIDE(81.0)
J-L	12	SIDE(81.0)
L-V	12	TOP
V-B	12	TOP
B-K	12	TOP
K-S	12	TOP
S-P	12	TOP
P-M	12	SIDE(183.1)
M-T	12	SIDE(183.1)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
V-B	12	TOP
S-P	12	TOP
P-M	12	SIDE(183.1)
M-T	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	8	SIDE(67.9)
1x3	3	SIDE(67.9)
D-T	3	SIDE(67.9)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS								
	FACTORED		MAXIMUM FACTORED			INPUT	REQD	
	GROSS REACTION		GROSS REACTION			BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
V	2838	0	2838	0	0	5-8	5-8	
M	4798	0	4798	0	0	5-8	5-8	

UNFACTORED REACTIONS		MAX/MIN. COMPONENT REACTIONS					
JT	1ST LOSE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2073	1888 / 0	0 / 0	0 / 0	0 / 0	684 / 0	0 / 0
M	3381	2278 / 0	0 / 0	0 / 0	0 / 0	1103 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0/35	-91.8	-91.8 0.07 (1)	10.00	U-C	-584/0	0.07 (1)
B-C	-3442/0	-91.8	-91.8 0.20 (1)	4.88	C-T	0/3700	0.46 (1)
C-D	-5905/0	-91.8	-91.8 0.43 (1)	3.70	T-D	-1983/0	0.25 (1)
D-E	-7947/0	-91.8	-91.8 0.68 (1)	3.11	D-R	0/2485	0.31 (1)
E-F	-7947/0	-91.8	-91.8 0.31 (1)	3.30	R-E	-368/0	0.05 (1)
F-G	-9222/0	-91.8	-91.8 0.38 (1)	3.01	Q-H	-348/0	0.04 (1)
G-H	-9222/0	-91.8	-91.8 0.38 (1)	3.01	H-I	-503/0	0.19 (1)
H-I	-9222/0	-91.8	-91.8 0.77 (1)	2.73	O-I	-397/0	0.05 (1)
I-W	-9835/0	-91.8	-91.8 0.88 (1)	2.57	O-J	0/5776	0.71 (1)
W-X	-9835/0	-91.8	-91.8 0.88 (1)	2.57	N-J	-915/0	0.12 (1)
X-J	-9835/0	-91.8	-91.8 0.88 (1)	2.57	B-U	0/2940	0.38 (1)
J-Y	-5847/0	-91.8	-91.8 0.33 (1)	3.78	N-K	0/5020	0.02 (1)
Y-K	-5847/0	-91.8	-91.8 0.33 (1)	3.78	R-F	-1209/0	0.21 (1)
K-L	0/35	-91.8	-91.8 0.07 (1)	10.00	F-Q	0/974	0.12 (1)
V-B	-2914/0	0.0	0.0 0.16 (1)	6.74			
M-K	-4771/0	0.0	0.0 0.27 (1)	5.48			

FACTORED CONCENTRATED LOADS (LBS)		MAX.		FACE		DIR.		TYPE		HEEL		CONN.	
JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.	HEEL	CONN.	HEEL	CONN.
J	31-1-8	-49	-55	-55	FRONT	VERT	DEAD	---	C1	---	C1	---	C1
J	31-1-8	-254	-254	-254	FRONT	VERT	SNOW	---	C1	---	C1	---	C1
O	25-7-8	-2840	-2840	-2840	FRONT	VERT	TOTAL	---	C1	---	C1	---	C1
W	27-8-12	-110	-110	-110	FRONT	VERT	TOTAL	---	C1	---	C1	---	C1
X	29-8-12	-110	-110	-110	FRONT	VERT	TOTAL	---	C1	---	C1	---	C1
Y	31-8-12	-138	-138	-138	FRONT	VERT	TOTAL	---	C1	---	C1	---	C1
Z	27-8-12	-28	-28	-28	FRONT	VERT	TOTAL	---	C1	---	C1	---	C1
AA	29-8-12	-28	-28	-28	FRONT	VERT	TOTAL	---	C1	---	C1	---	C1
AB	31-8-12	-28	-28	-28	FRONT	VERT	TOTAL	---	C1	---	C1	---	C1
AC	33-8-12	-28	-28	-28	FRONT	VERT	TOTAL	---	C1	---	C1	---	C1

TOTAL WEIGHT = 2 X 180 = 321 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2011, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 089-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/860 (1.17)
CALCULATED VERT. DEFL.(LL) = L/999 (0.28")
ALLOWABLE DEFL.(TL) = L/860 (1.17)
CALCULATED VERT. DEFL.(TL) = L/832 (0.51")

CSI: TO=0.88/1.00 (I-J:1), BO=0.72/1.00 (O-Q:1),
WB=0.71/1.00 (J-O:1), SSI=0.18/1.00 (J-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1997 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (C) (INPUT = 0.80)
JSI METAL = 0.87 (S) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2007146 1/2

JOB NAME 408169	TRUSS NAME T20Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Sat Apr 25 12:49:25 2020 Page 2
ID:K7TPdhaDnpl1aYdcWIOFzIdeG-AKHFWwIPgK qKTo9aXCi67DKWF2iJk9bCKgw7zNBDe

PLATES (table in inches)				
IT	TYPE	PLATES	W	LEN Y X
B	TMVW-p	MT20	5.0	8.0 1.50 3.50
C	TTWW-m	MT20	7.0	8.0 1.75 2.50
D	TMWW-t	MT20	4.0	8.0
E	TMW+w	MT20	2.0	4.0
F	TMWW-t	MT20	4.0	4.0
G	TS-t	MT20	3.0	8.0
H	TMW+w	MT20	2.0	4.0
I	TMWW-t	MT20	4.0	8.0
J	TTWW-m	MT20	7.0	8.0 1.75 2.50
K	TMVW-p	MT20	5.0	8.0 1.50 3.50
M	BMV1-p	MT20	3.0	6.0
N	BMWW-t	MT20	5.0	6.0 2.50 2.00
O	BMWW-t	MT20	6.0	9.0 4.50 2.00
P	BS-t	MT20	5.0	8.0
Q	BMWWW-t	MT20	5.0	8.0
R	BMWWW-t	MT20	5.0	8.0
S	BS-t	MT20	5.0	6.0
T	BMWW-t	MT20	6.0	9.0 4.50 2.00
U	BMWW-t	MT20	5.0	8.0 2.50 2.00
V	BMV1-p	MT20	3.0	6.0

CONNECTION REQUIREMENTS

II C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

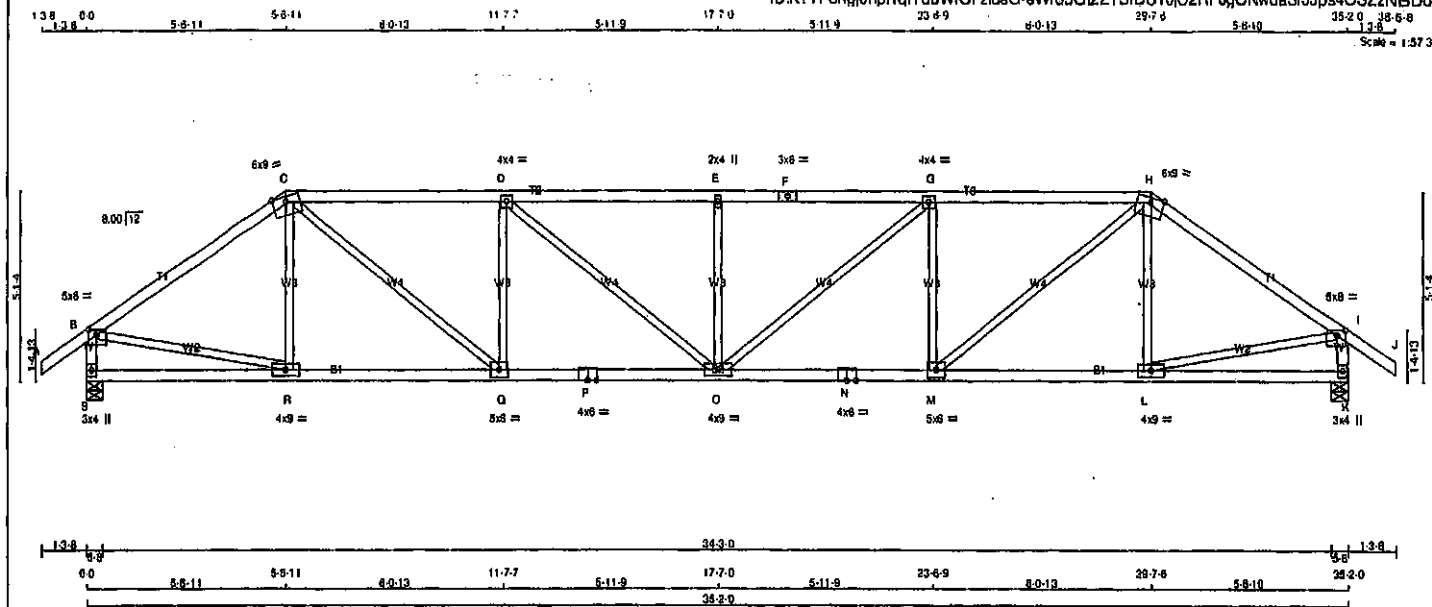
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2007146 42

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408169	T21	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mittek Industries, Inc. Sat Apr 25 12:49:26 2020 Page 1
ID:K2TPdngl0npt1qYdbWOFzldG-eWrd5G12ZTSrDU10O2RPJgONwdaSrJps4OSZzNBdD



TOTAL WEIGHT = 2 X 140 = 280 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
O - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
S - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
S - P	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
	FACTORED	MAXIMUM FACTORED		INPUT		REQD			
	GROSS REACTION	GROSS REACTION		BRG		BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
S	2085	0	2085	0	5-8	5-8	5-8		
K	2085	0	2085	0	5-8	5-8	5-8		
UNFACTORED REACTIONS									
JT	1ST LOASE	MAX/MIN. COMPONENT REACTIONS							
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
S	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0		
K	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0		
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (4)									

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	R-C	-243 / 10	0.09 (1)	
B-C	-2287 / 0	-91.8	-91.8	0.70 (1)	3.78	C-Q	0 / 1869	0.38 (1)	
C-D	-3213 / 0	-91.8	-91.8	0.60 (1)	3.19	D-O	-929 / 0	0.36 (1)	
D-E	-3594 / 0	-91.8	-91.8	0.68 (1)	2.97	O-Q	0 / 489	0.11 (1)	
E-F	-3594 / 0	-91.8	-91.8	0.68 (1)	2.97	C-E	-505 / 0	0.19 (1)	
F-G	-3594 / 0	-91.8	-91.8	0.68 (1)	2.97	O-G	0 / 490	0.11 (1)	
G-H	-3213 / 0	-91.8	-91.8	0.80 (1)	3.19	M-G	-929 / 0	0.36 (1)	
H-I	-2286 / 0	-91.8	-91.8	0.70 (1)	3.78	M-H	0 / 1870	0.38 (1)	
I-J	0 / 35	-91.8	-91.8	0.12 (1)	10.00	L-H	-243 / 10	0.09 (1)	
S-B	-2023 / 0	0.0	0.0	0.21 (1)	5.94	B-R	0 / 1943	0.44 (1)	
K-I	-2023 / 0	0.0	0.0	0.21 (1)	5.94	L-I	0 / 1943	0.44 (1)	
S-R	0 / 0	-18.5	-18.5	0.15 (4)	10.00				
R-Q	0 / 1904	-18.5	-18.5	0.39 (1)	10.00				
Q-P	0 / 3213	-18.5	-18.5	0.58 (1)	10.00				
P-O	0 / 3213	-18.5	-18.5	0.58 (1)	10.00				
O-N	0 / 3213	-18.5	-18.5	0.58 (1)	10.00				
N-M	0 / 3213	-18.5	-18.5	0.58 (1)	10.00				
M-L	0 / 1903	-18.5	-18.5	0.39 (1)	10.00				
L-K	0 / 0	-18.5	-18.5	0.15 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.8	PSF
DL	= 6.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BOB 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TRG 2011, TRG 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.17")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.21")
ALLOWABLE DEFL.(TL) = $L/360$ (1.17")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.39")

CS1: TC=0.88/1.00 (E-G:1), BC=0.58/1.00 (O-Q:1),
WB=0.44/1.00 (B-R:1), SS=0.28/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 788
	1887	1656

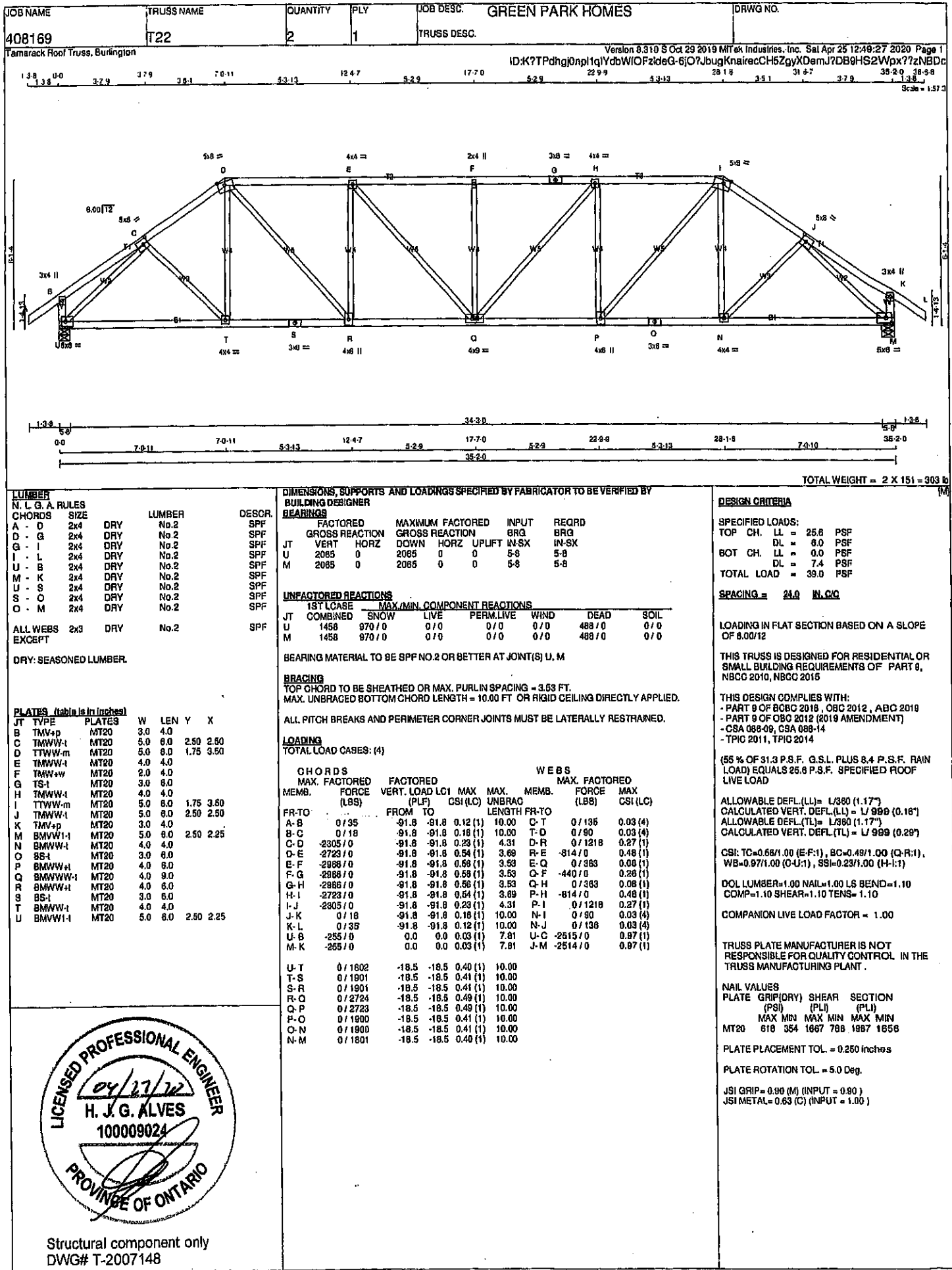
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.80)
JSI METAL= 0.74 (P) (INPUT = 1.00)

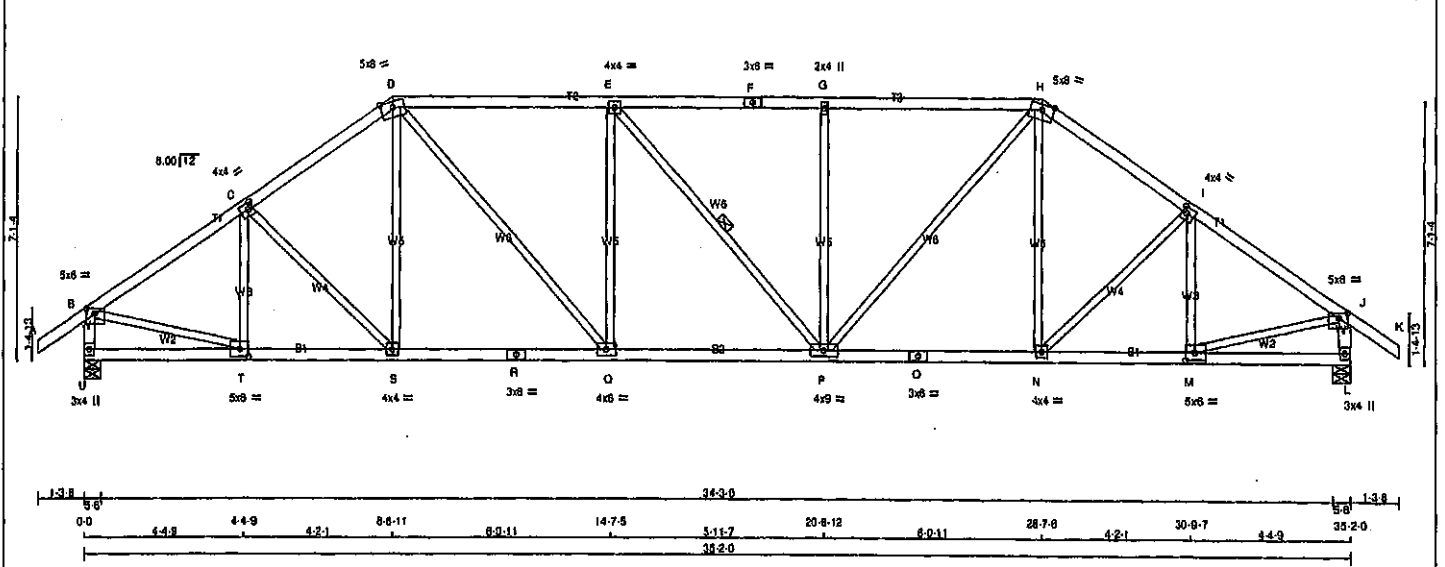


Structural component only
DWG# T-2007147



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	T23	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 12:49:28 2020 Page 1
ID:K7TPdghi0npt1qYdbWIOFzldcG-bvYNWkVl54IZt8Oqp4VWknEjLmwjVbHAZVXRzNBDb



LUMBER					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																			
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F - H					2x4					DRY					No.2					SPF					JT					VERT					HORZ					DOWN					HORZ					UPLIFT					IN-SX					IN-SX																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																						
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PLATES (table is in inches)	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	6.0	1.75	2.75
C TMVW-i	MT20	4.0	4.0	2.00	1.50
D TMVW-m	MT20	5.0	6.0	Edge	
E TMVW-i	MT20	4.0	4.0		
F TS-i	MT20	3.0	6.0		
G TMVW-w	MT20	2.0	4.0		
H TMVW-m	MT20	5.0	6.0	Edge	
I TMVW-i	MT20	4.0	4.0	2.00	1.50
J TMVW-p	MT20	5.0	6.0	1.75	2.75
L BMV-i-p	MT20	3.0	4.0		
M BMVW-i	MT20	5.0	6.0	2.50	2.75
N BMVW-i	MT20	4.0	4.0		
O BS-i	MT20	3.0	6.0		
P BMVW-i	MT20	4.0	4.0		
Q BMVW-i	MT20	4.0	4.0		
R BS-i	MT20	3.0	6.0		
S BMVW-i	MT20	4.0	4.0		
T BMVW-i	MT20	5.0	6.0	2.50	2.75
U BMV-i-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

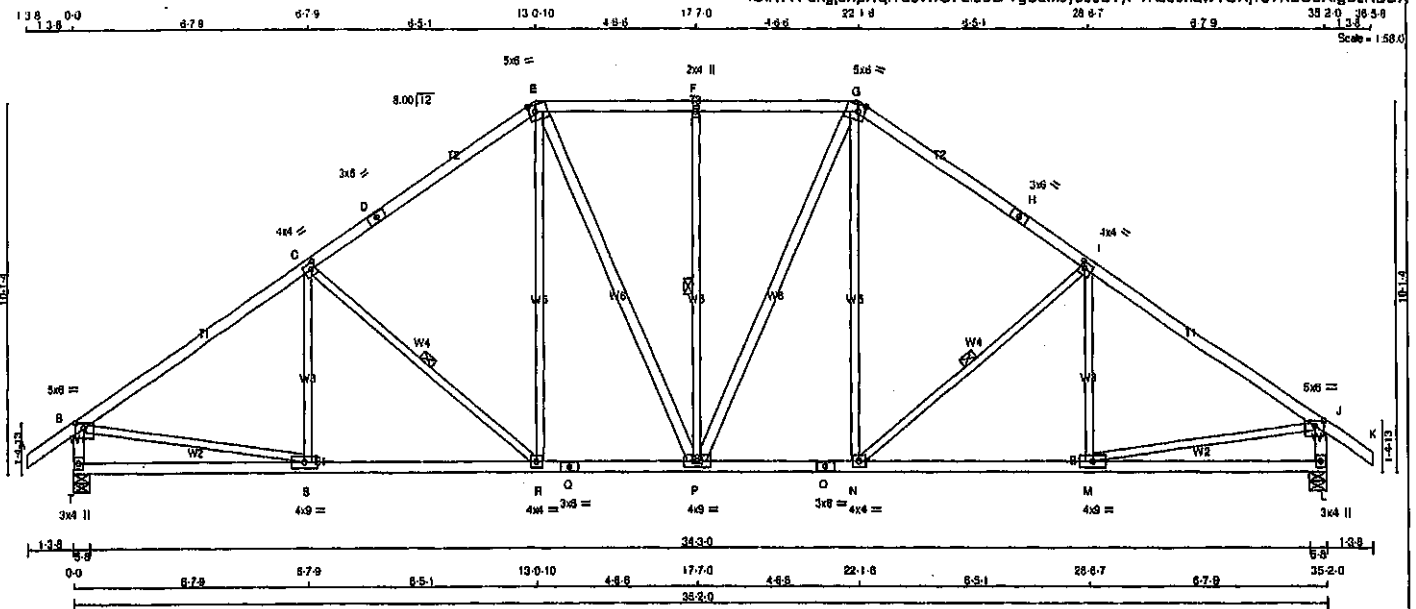


Structural component only
DWG# T-2007149

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	T26	5	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 5 X 172 = 850 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
L - B	2x4	DRY	No.2
T - J	2x4	DRY	No.2
Q - Q	2x4	DRY	No.2
T - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
E - P	2x4	DRY	No.2
P - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	8.0	1.75 2.75
C	TMVW-1	MT20	4.0	4.0	2.00 1.50
D	TS-1	MT20	3.0	8.0	
E	TTVW-m	MT20	5.0	8.0	2.25 2.00
F	TMVW-w	MT20	2.0	4.0	
G	TTVW-m	MT20	5.0	8.0	2.25 2.00
H	TS-1	MT20	3.0	8.0	
I	TMVW-1	MT20	4.0	4.0	2.00 1.50
J	TMVW-p	MT20	5.0	8.0	1.75 2.75
L	BMV1-p	MT20	3.0	4.0	
M	BMVW-1	MT20	4.0	9.0	
N	BMVW-1	MT20	4.0	4.0	
O	BS-1	MT20	3.0	6.0	
P	BMVW-1	MT20	4.0	9.0	
Q	BS-1	MT20	3.0	6.0	
R	BMVW-1	MT20	4.0	4.0	
S	BMVW-1	MT20	4.0	9.0	
T	BMV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
JT	VERT	DOWN	UP	IN-SX	IN-SX
T	2085	0	0	5-8	5-8
L	2085	0	0	5-8	5-8

UNFACTORED REACTIONS					
1ST CASE	MAX/MIN COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
T	1458	970 / 0	0 / 0	0 / 0	488 / 0
L	1458	970 / 0	0 / 0	0 / 0	488 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-R, F-P, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING					
TOTAL LOAD CASES: (4)					
CHORDS			WEBS		
MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	FACTORED FORCE (LBS)
FR-TO				FR-TO	
A-B	0 / 35	-91.8	-91.8 0.12 (1)	S-C	-189 / 46
B-C	-2359 / 0	-91.8	-91.8 0.69 (1)	C-R	-485 / 0
C-D	-1896 / 0	-91.8	-91.8 0.62 (1)	R-E	0 / 416
D-E	-1998 / 0	-91.8	-91.8 0.62 (1)	E-P	0 / 315
E-F	-1761 / 0	-91.8	-91.8 0.27 (1)	P-F	-502 / 0
F-G	-1761 / 0	-91.8	-91.8 0.27 (1)	P-G	0 / 315
G-H	-1896 / 0	-91.8	-91.8 0.62 (1)	N-G	0 / 415
H-I	-1896 / 0	-91.8	-91.8 0.62 (1)	N-I	-484 / 0
I-J	-2358 / 0	-91.8	-91.8 0.69 (1)	M-I	-189 / 46
J-K	0 / 35	-91.8	-91.8 0.12 (1)	S-S	0 / 2022
T-B	-2014 / 0	0.0	0.0 0.21 (1)	M-J	0 / 2021
L-J	-2014 / 0	0.0	0.0 0.21 (1)		
T-S	0 / 0	-18.5	-18.5 0.19 (4)		
S-R	0 / 1998	-18.5	-18.5 0.42 (1)		
R-Q	0 / 1829	-18.5	-18.5 0.32 (1)		
Q-P	0 / 1829	-18.5	-18.5 0.32 (1)		
P-O	0 / 1829	-18.5	-18.5 0.32 (1)		
O-N	0 / 1829	-18.5	-18.5 0.32 (1)		
N-M	0 / 1995	-18.5	-18.5 0.42 (1)		
M-L	0 / 0	-18.5	-18.5 0.19 (4)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. B.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17')
CALCULATED VERT. DEFL.(LL) = L/999 (0.09')
ALLOWABLE DEFL.(TL) = L/360 (1.17')
CALCULATED VERT. DEFL.(TL) = L/999 (0.16')

CSI: TC=0.89/1.00 (B-C:1), BC=0.42/1.00 (R-S:1), WB=0.45/1.00 (B-S:1), SS=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

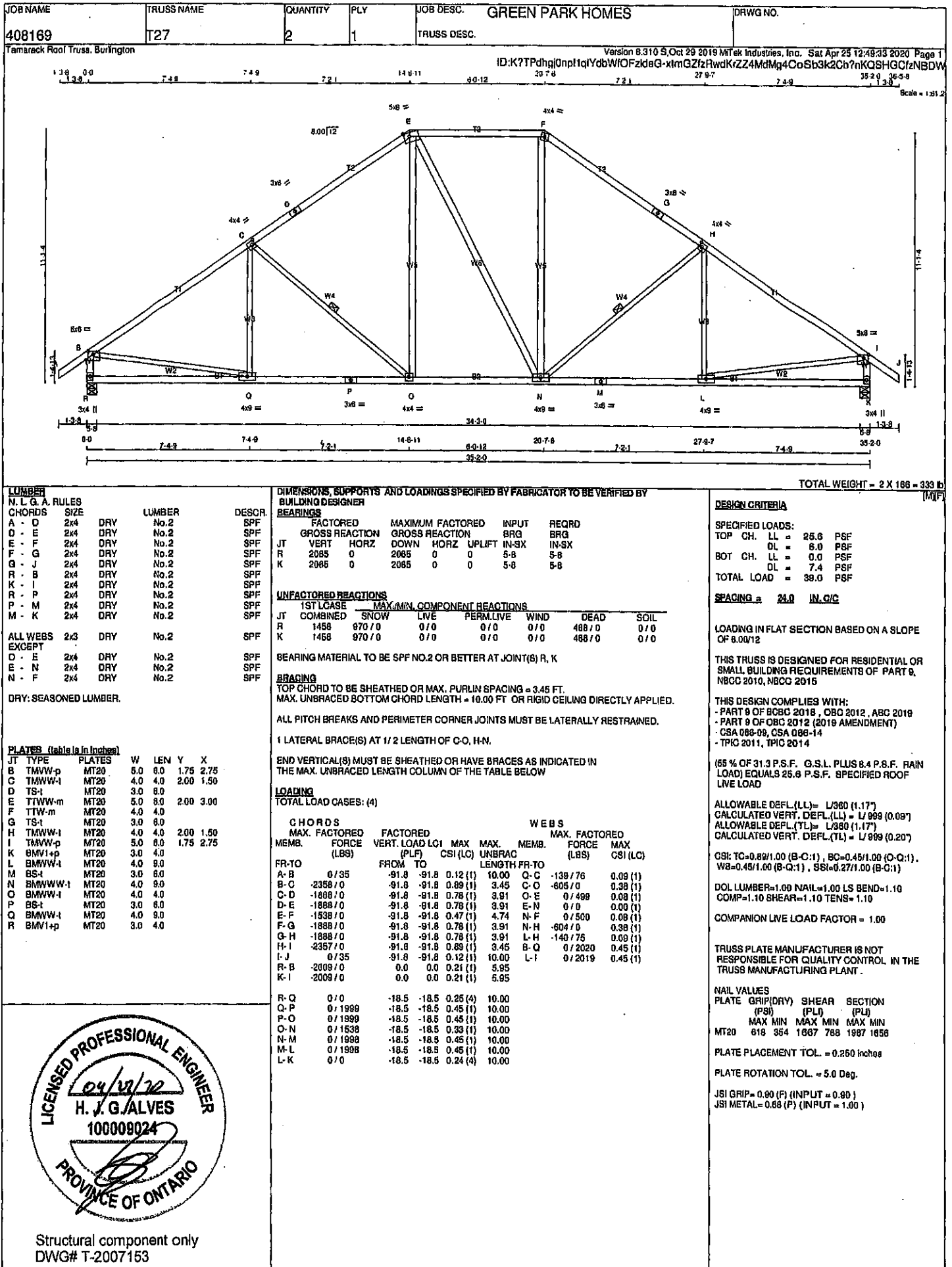
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.80)
JSI METAL= 0.50 (O) (INPUT = 1.00)

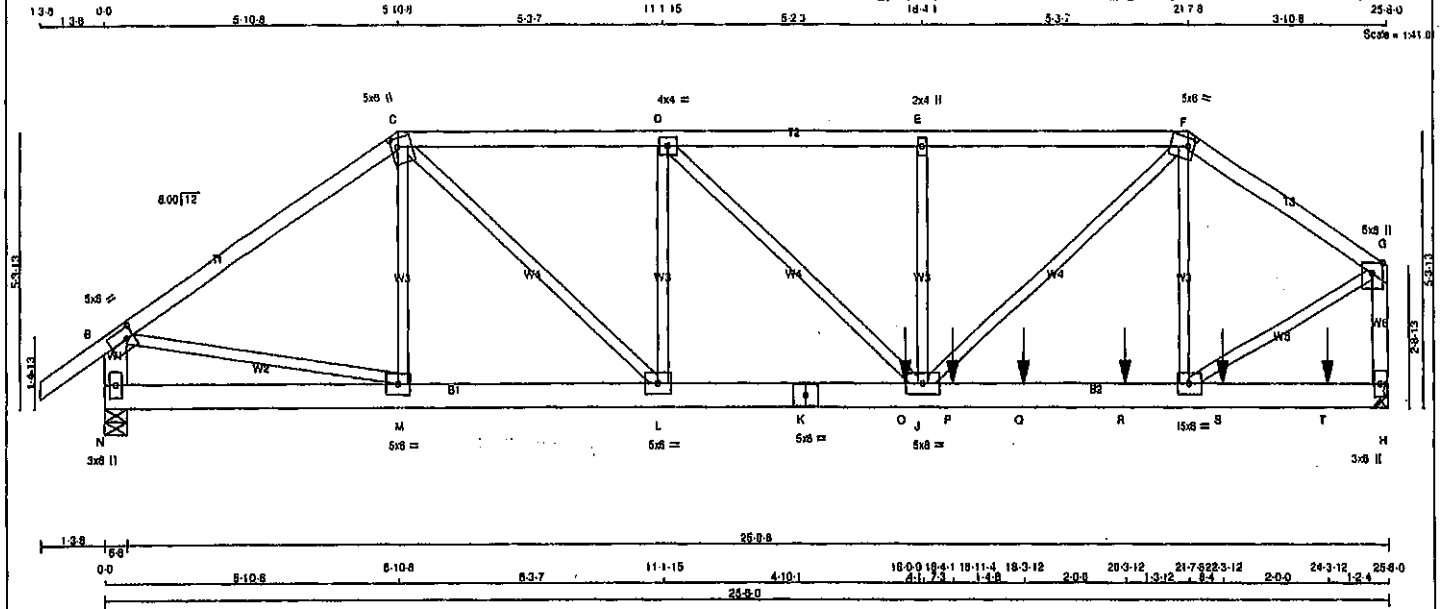


Structural component only
DWG# T-2007152



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408169	T28	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 12:49:34 2020 Page 1
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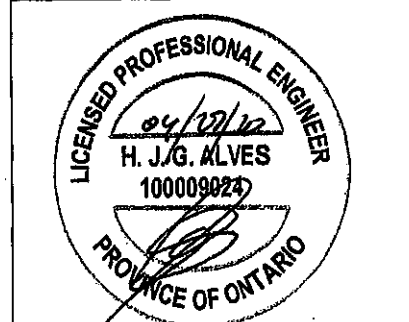


LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - F	2x4 DRY	No.2	SPF		
F - G	2x4 DRY	No.2	SPF		
H - B	2x6 DRY	No.2	SPF		
H - G	2x4 DRY	No.2	SPF		
N - K	2x6 DRY	No.2	SPF		
K - H	2x6 DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF		

DRY: SEASONED LUMBER.
 DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - F	12	TOP
F - G	12	TOP
H - G	12	TOP
N - B	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
N - K	12	TOP
K - H	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
E - J	12	SIDE(183.1)
2x3	8	SIDE(77.0)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.
 GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.
 TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.
 SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.



Structural component only
 DWG# T-2007154 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
N	2088	0	0	5-8
H	2756	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS	1ST CASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
N	1470	994 / 0
H	1941	1320 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

SPACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.78 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)
FR-TO					FR-TO			
A-B	0 / 35	-91.8	-91.8	0.07 (1)	M-O	-218 / 22	0.04 (1)	
B-C	-2383 / 0	-91.8	-91.8	0.37 (1)	C-L	0 / 1550	0.19 (1)	
C-D	-3093 / 0	-91.8	-91.8	0.24 (1)	L-D	-928 / 0	0.19 (1)	
D-E	-3519 / 0	-91.8	-91.8	0.25 (1)	D-J	0 / 589	0.07 (1)	
E-F	-3519 / 0	-91.8	-91.8	0.24 (1)	J-E	-508 / 0	0.10 (1)	
F-G	-2352 / 0	-91.8	-91.8	0.16 (1)	J-F	0 / 2156	0.27 (1)	
N-B	-2037 / 0	0.0	0.0	0.07 (1)	I-F	-548 / 0	0.11 (1)	
H-G	-2632 / 0	0.0	0.0	0.19 (1)	B-M	0 / 1991	0.25 (1)	
					I-G	0 / 2264	0.28 (1)	
N-M	0 / 0	-18.5	-18.5	0.04 (1)				
M-L	0 / 1959	-18.5	-18.5	0.14 (1)				
L-K	0 / 3093	-18.5	-18.5	0.27 (1)				
K-O	0 / 3093	-18.5	-18.5	0.27 (1)				
O-J	0 / 3093	-18.5	-18.5	0.27 (1)				
J-P	0 / 1942	-18.5	-18.5	0.23 (1)				
P-Q	0 / 1942	-18.5	-18.5	0.23 (1)				
Q-R	0 / 1942	-18.5	-18.5	0.23 (1)				
R-I	0 / 1942	-18.5	-18.5	0.23 (1)				
I-S	0 / 0	-18.5	-18.5	0.10 (1)				
S-T	0 / 0	-18.5	-18.5	0.10 (1)				
T-H	0 / 0	-18.5	-18.5	0.10 (1)				

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	16-0-0	-997	-997	---	---	FRONT	VERT	TOTAL	---	C1
P	16-11-4	-187	-187	---	---	FRONT	VERT	TOTAL	---	C1
Q	16-3-12	-187	-187	---	---	FRONT	VERT	TOTAL	---	C1
R	20-3-12	-187	-187	---	---	FRONT	VERT	TOTAL	---	C1
S	22-3-12	-174	-174	---	---	FRONT	VERT	TOTAL	---	C1
T	24-3-12	-173	-173	---	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 8.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBC 2010, NBC 2012, ABC 2019
 - PART 9 OF NBC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 086-14
 - TPIG 2011, TPIG 2014

(85 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/360 (0.85")
 CALCULATED VERT. DEF. (LL) = L/999 (0.06")
 ALLOWABLE DEF. (TL) = L/360 (0.85")
 CALCULATED VERT. DEF. (TL) = L/899 (0.11")

CSI: TC=0.37/1.00 (B-C-1), BC=0.27/1.00 (J-L-1), WB=0.22/1.00 (G-I-1), SSI=0.34/1.00 (J-L-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PS) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.80 (F) (INPUT = 0.80)
 JSI METAL = 0.45 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	T28	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:k?TPdhi0nc1glYdbWIOFzdeG-P3Jln7_3gwSIBIYB3Bjk??uz8RBKVLI/60p5zNBDV

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.75
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMWW-l	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	1.50
G	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I, L, M						
I	BMWW-l	MT20	5.0	6.0		
J	BMWWW-l	MT20	5.0	6.0		
K	BS-l	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



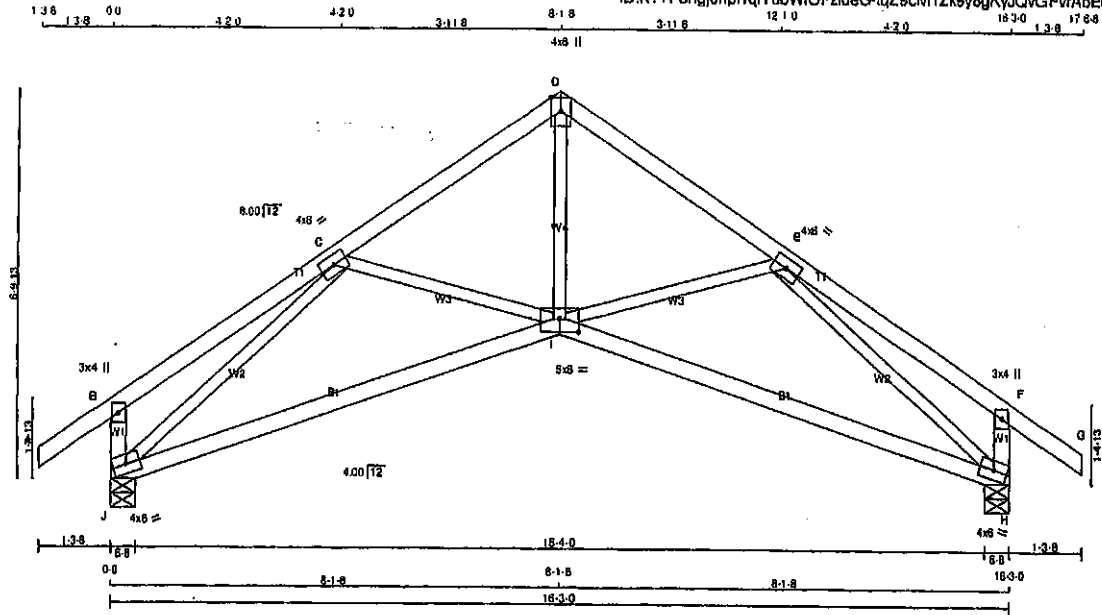
Structural component only
DWG# T-2007154 *mv*

JSI GRIP= 0.82 (D) {INPUT = 0.90}
JSI METAL= 0.51 (G) {INPUT = 1.00}

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408169	T30S	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/32"

TOTAL WEIGHT = 2 X 87 = 133 lb

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR.
A - D	2x4	DRY	No.2		SPF
D - G	2x4	DRY	No.2		SPF
J - B	2x4	DRY	No.2		SPF
H - F	2x4	DRY	No.2		SPF
J - I	2x4	DRY	No.2		SPF
I - H	2x4	DRY	No.2		SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATE	TYPE	W	LEN	Y	X
JT	TMV+p	MT20	3.0	4.0	
B	TMV+p	MT20	4.0	8.0	
C	TMV+p	MT20	4.0	8.0	
D	TMV+p	MT20	4.0	8.0	Edge
E	TMV+p	MT20	4.0	8.0	
F	TMV+p	MT20	3.0	4.0	
H	BMVW1-I	MT20	4.0	8.0	
J	BMVW1-p	MT20	5.0	8.0	2.75 4.00
I	BMVW1-I	MT20	4.0	8.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
J	1022	0	1022	0	5-8
H	1022	0	1022	0	5-8

UNFACTORED REACTIONS		MAX/MIN COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
J	720	488 / 0	0 / 0	0 / 0	0 / 0
H	720	488 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)		CHORDS		WEBS	
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 35	-91.8	-91.8	10.00	I-D	0 / 842	0.21 (1)
B-C	0 / 22	-91.8	-91.8	10.00	I-E	-143 / 11	0.05 (1)
C-D	-1134 / 0	-91.8	-91.8	10.00	C-I	-143 / 11	0.05 (1)
D-E	-1134 / 0	-91.8	-91.8	10.00	J-C	-1478 / 0	0.78 (1)
E-F	0 / 22	-91.8	-91.8	10.00	E-H	-1478 / 0	0.78 (1)
F-G	0 / 35	-91.8	-91.8	10.00			
J-B	-273 / 0	0.0	0.0	0.03 (1)			
H-F	-273 / 0	0.0	0.0	0.03 (1)			
J-I	0 / 1120	-18.5	-18.5	0.44 (4)			
I-H	0 / 1120	-18.5	-18.5	0.44 (4)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/789 (0.28")

CSI: TO=0.22/1.00 (E-F:1), BC=0.44/1.00 (H-I:1), WB=0.78/1.00 (C-J:1), SS=0.16/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.80)
JSI METAL= 0.35 (H) (INPUT = 1.00)



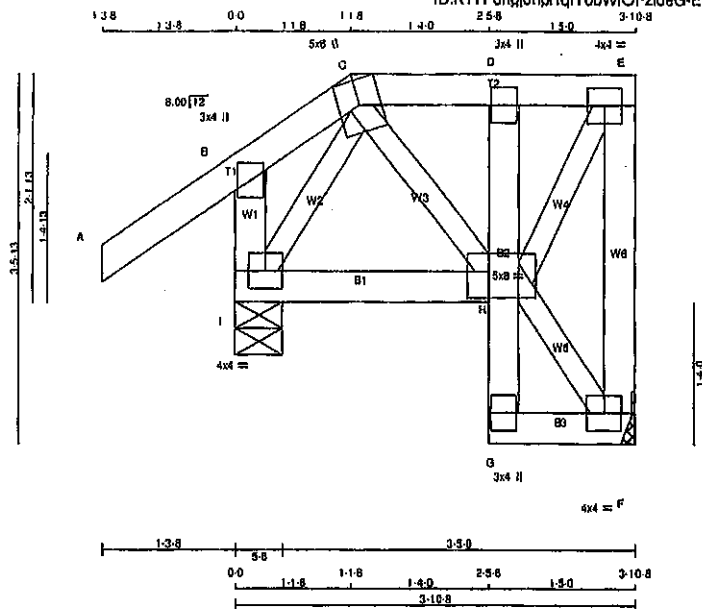
Structural component only
DWG# T-2007158

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408169	T32S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:K7TPdhpj0npl1qYobWlOFzIdeG-EDhw122qGmCsve6iXKlj_Gfy1ZYDkG4M227yzNBDF



Scale = 1/20.1

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
I - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
G - D	2x4	DRY	No.2
G - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TTWW+m	MT20	5.0	6.0 Edge
D	TMV+p	MT20	3.0	4.0
E	TMVW-1	MT20	4.0	4.0
F	BMVW-1	MT20	4.0	4.0
G	BMV+p	MT20	3.0	4.0
H	BMVWVW-1	MT20	5.0	8.0 3.00 2.60
I	BMVW-1	MT20	4.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
		GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
F	191	0	191	0	0
I	362	0	362	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	136	87 / 12	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
I	253	182 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		WEBS	
MEMB.	FACTORED	MEMB.	FACTORED
FORCE (LBS)	VERT. LOAD LC1 MAX (PLF) CSI (LC)	FORCE (LBS)	MAX. FACTORED (LBS) CSI (LC)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 35	-91.8 -91.8 0.14 (5)	10.00 C-H 0 / 60 0.01 (1)
B-C	-41 / 0	-91.8 -91.8 0.13 (5)	6.25 H-F -7 / 0 0.00 (1)
C-D	-89 / 0	-91.8 -91.8 0.02 (1)	6.25 H-E 0 / 153 0.03 (1)
D-E	-88 / 0	-91.8 -91.8 0.02 (1)	6.25 I-C -100 / 34 0.02 (1)
E-F	-175 / 0	0.0 0.0 0.03 (1)	7.81
F-G	-253 / 0	0.0 0.0 0.03 (1)	7.81
I-H	-18 / 53	-18.5 -18.5 0.04 (4)	6.25
G-H	0 / 13	0.0 0.0 0.01 (1)	10.00
H-D	-142 / 0	0.0 0.0 0.01 (1)	7.81
G-F	0 / 5	-18.5 -18.5 0.01 (4)	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 25 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2018 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (A-B-5), BC=0.04/1.00 (H-I-4), WB=0.03/1.00 (E-H-1), SS=0.09/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

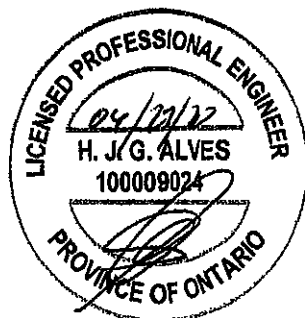
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX MIN MAX MIN MAX MIN		
MT20	818 354 1667 788 1987 1858	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007160

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408169	T33	1	2	TRUSS DESC.		

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PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



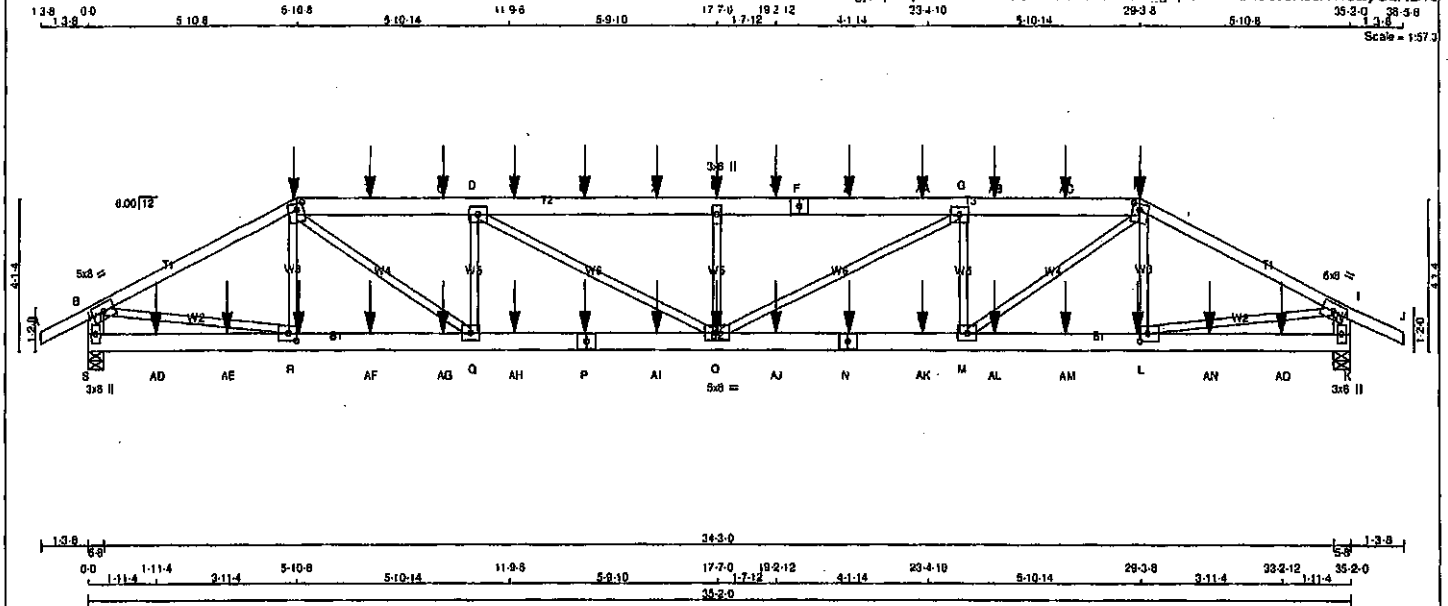
Structural component only
 DWG# T-2007161 32

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T40	1	2	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 169 = 338 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
CHORDS	SIZE	LUMBER	DESCR.	BEARINGS				SPECIFIED LOADS:			
A - C	2x4	DRY	No.2	FACTORED	MAXIMUM FACTORED	INPUT	REQD	TOP CH. LL =	25.8	PSF	
C - F	2x6	DRY	No.2	GROSS REACTION	GROSS REACTION	BRG	BRG	DL =	8.0	PSF	
F - H	2x6	DRY	No.2	JT VERT	HORZ	DOWN	HORZ	BOT CH. LL =	0.0	PSF	
H - J	2x4	DRY	No.2	S	3331	0	3331	DL =	7.4	PSF	
S - B	2x6	DRY	No.2	K	3331	0	3331	TOTAL LOAD =	39.0	PSF	
K - I	2x6	DRY	No.2	UNFACTORED REACTIONS				SPACING =			
S - P	2x6	DRY	No.2	1ST LCASE	MAX/MIN	COMPONENT REACTIONS		LOADING IN FLAT SECTION BASED ON A SLOPE			
P - N	2x6	DRY	No.2	JT COMBINED	SNOW	LIVE	PERM.LIVE	OF 8.00/12			
N - K	2x6	DRY	No.2	S	2356	1544	0	THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR			
				K	2356	1544	0	SMALL BUILDING REQUIREMENTS OF PART 9,			
								NBCC 2010, NBCC 2015			
								THIS DESIGN COMPLIES WITH:			
								- PART 9 OF BCBC 2018, OBC 2012, ABC 2019			
								- PART 9 OF OBC 2012 (2019 AMENDMENT)			
								- CSA 088-09, CSA 088-14			
								- TPIQ 2011, TPIQ 2014			
								(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN			
								LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF			
								LIVE LOAD			
								ALLOWABLE DEFL.(LL)= L/360 (1.17")			
								CALCULATED VERT. DEFL.(LL)= L/999 (0.21")			
								ALLOWABLE DEFL.(TL)= L/360 (1.17")			
								CALCULATED VERT. DEFL.(TL)= L/999 (0.39")			
								CSI: TO=0.52/1.00 (B-C:1), BC=0.52/1.00 (O-Q:1),			
								WB=0.57/1.00 (B-R:1), SSI=0.16/1.00 (D-E:1)			
								DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00			
								COMP=1.00 SHEAR=1.00 TENS=1.00			
								COMPANION LIVE LOAD FACTOR = 1.00			
								AUTOSOLVE HEELS OFF			
								TRUSS PLATE MANUFACTURER IS NOT			
								RESPONSIBLE FOR QUALITY CONTROL IN THE			
								TRUSS MANUFACTURING PLANT.			
								NAIL VALUES			
								PLATE GRIP(DRY) SHEAR SECTION			
								(PSI) (PLI) (PLI) (PLI)			
								MAX MIN MAX MIN MAX MIN			
								MT20 618 354 1867 788 1987 1856			
								PLATE PLACEMENT TOL. = 0.250 inches			
								PLATE ROTATION TOL. = 5.0 Deg.			
								JSI GRIP= 0.90 (H) (INPUT = 0.90)			
								JSI METAL= 0.87 (P) (INPUT = 1.00)			



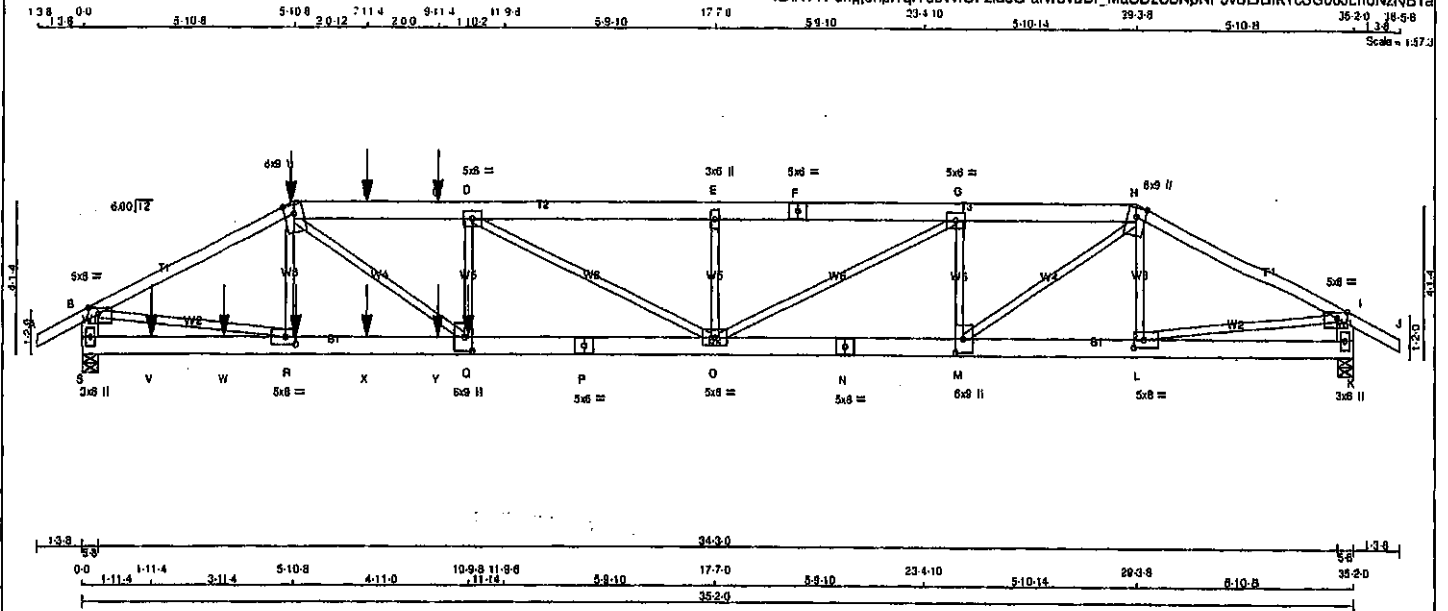
Structural component only
DWG# T-2007168 1/4

CONTINUED ON PAGE 2

JOB NAME 408170	TRUSS NAME T40Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - J	2x4	DRY	No.2
S - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-G 1	12	SIDE(81.0)
H-J 1	12	TOP
C-F 2	12	SIDE(81.0)
F-H 2	12	TOP
S-B 2	12	TOP
K-I 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S-P 2	12	SIDE(183.1)
P-N 2	12	TOP
N-K 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	
D-Q 1	3	SIDE(825.1)
G-M 1	3	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS	REACTION	DOWN	HORIZ	BRG	IN-SX	BRG	IN-SX
S	4582	0	4582	0	0	5-8	5-8	5-8	5-8
K	3047	0	3047	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	3236	2151 / 0	0 / 0	0 / 0	0 / 0	1084 / 0	0 / 0
K	2151	1438 / 0	0 / 0	0 / 0	0 / 0	715 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FORCE (LBS)
FR-TO				FR-TO	
A-B	0 / 28	-91.8	-91.8 0.07 (1)	R-C	-673 / 0
B-C	-7283 / 0	-91.8	-91.8 0.72 (1)	C-Q	0 / 4988
C-T	-10549 / 0	-91.8	-91.8 0.34 (1)	Q-D	-184 / 38
T-U	-10549 / 0	-91.8	-91.8 0.34 (1)	D-O	-1207 / 0
U-D	-10549 / 0	-91.8	-91.8 0.34 (1)	O-E	-948 / 0
D-E	-9485 / 0	-91.8	-91.8 0.31 (1)	E-G	0 / 2871
E-F	-9485 / 0	-91.8	-91.8 0.29 (1)	M-G	-1894 / 0
F-G	-9485 / 0	-91.8	-91.8 0.29 (1)	M-H	0 / 3512
G-H	-8958 / 0	-91.8	-91.8 0.17 (1)	L-H	-464 / 0
H-I	-4808 / 0	-91.8	-91.8 0.48 (1)	B-R	0 / 6598
I-J	0 / 28	-91.8	-91.8 0.07 (1)	L-I	0 / 4167
S-B	-4503 / 0	0.0	0.0 0.18 (1)		
K-I	-2990 / 0	0.0	0.0 0.11 (1)		

MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FORCE (LBS)
S-V	0 / 0	-18.5	-18.5 0.08 (4)		
V-W	0 / 0	-18.5	-18.5 0.08 (4)		
W-R	0 / 0	-18.5	-18.5 0.08 (4)		
R-X	0 / 8557	-18.5	-18.5 0.50 (1)		
X-Y	0 / 8557	-18.5	-18.5 0.50 (1)		
Y-Q	0 / 8557	-18.5	-18.5 0.50 (1)		
Q-P	0 / 10548	-18.5	-18.5 0.77 (1)		
P-O	0 / 10548	-18.5	-18.5 0.77 (1)		
O-N	0 / 8955	-18.5	-18.5 0.50 (1)		
N-M	0 / 8955	-18.5	-18.5 0.50 (1)		
M-L	0 / 4144	-18.5	-18.5 0.29 (1)		
L-K	0 / 0	-18.5	-18.5 0.04 (4)		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LG1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-437	-437	---	BACK	VERT	TOTAL	---	C1
Q	10-9-8	-2715	-2715	---	BACK	VERT	TOTAL	---	C1
R	5-11-4	-28	-28	---	BACK	VERT	TOTAL	---	C1
T	7-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
U	9-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
V	1-11-4	-25	-25	---	BACK	VERT	TOTAL	---	C1
W	3-11-4	-28	-28	---	BACK	VERT	TOTAL	---	C1
X	7-11-4	-28	-28	---	BACK	VERT	TOTAL	---	C1
Y	9-11-4	-28	-28	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPG 2011, TPG 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17)
CALCULATED VERT. DEFL.(LL) = L/999 (0.26")
ALLOWABLE DEFL.(TL) = L/360 (1.17)
CALCULATED VERT. DEFL.(TL) = L/881 (0.48")

CS1: TO=0.72/1.00 (B-C:1), BC=0.77/1.00 (O-Q:1),
WB=0.82/1.00 (B-R:1), SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	818 354	1667 789

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (B) (INPUT = 0.90)
JSI METAL = 0.99 (P) (INPUT = 1.00)



Structural component only
DWG# T-2007169

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T40Z	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	2.00	3.50
C	ITWW+m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMVW-w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	ITWW+m	MT20	6.0	9.0	Edge	
I	TMVW-p	MT20	5.0	8.0	2.00	3.50
K	BMVt+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	8.0	2.50	3.25
M	BMWW-t	MT20	6.0	9.0	4.60	2.50
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	6.0	9.0	4.60	2.50
R	BMWW-t	MT20	6.0	8.0	2.50	3.25
S	BMVt+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

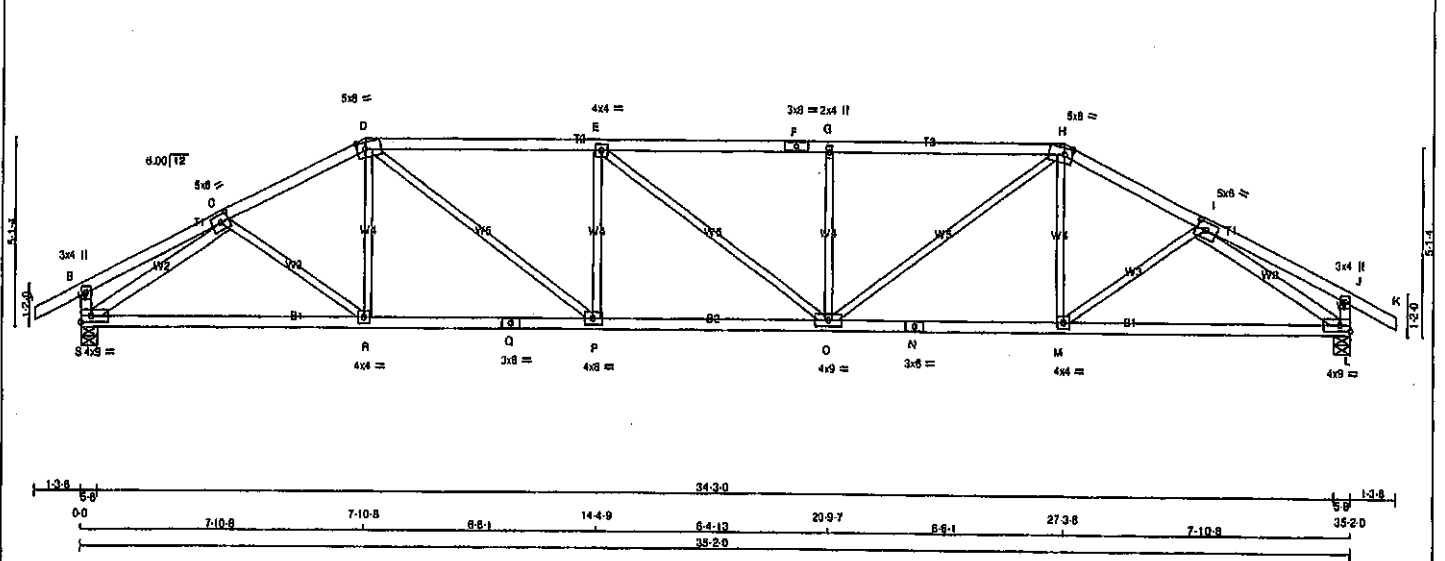
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007169 3/2

JOB NAME 408170	TRUSS NAME T41	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:57.3



TOTAL WEIGHT = 2 X 139 = 278 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES				BEARINGS			
CHORDS	SIZE	LUMBER	DESCR.	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
A - D	2x4	DRY	No.2	SPF	DOWN	HORIZ	UPLIFT
D - F	2x4	DRY	No.2	SPF	DOWN	HORIZ	UPLIFT
F - H	2x4	DRY	No.2	SPF	DOWN	HORIZ	UPLIFT
H - K	2x4	DRY	No.2	SPF	DOWN	HORIZ	UPLIFT
S - B	2x4	DRY	No.2	SPF	DOWN	HORIZ	UPLIFT
L - J	2x4	DRY	No.2	SPF	DOWN	HORIZ	UPLIFT
S - Q	2x4	DRY	No.2	SPF	DOWN	HORIZ	UPLIFT
Q - N	2x4	DRY	No.2	SPF	DOWN	HORIZ	UPLIFT
N - L	2x4	DRY	No.2	SPF	DOWN	HORIZ	UPLIFT
ALL WEBS	2x3	DRY	No.2	SPF	DOWN	HORIZ	UPLIFT
EXCEPT				SPF	DOWN	HORIZ	UPLIFT
S - C	2x4	DRY	No.2	SPF	DOWN	HORIZ	UPLIFT
I - L	2x4	DRY	No.2	SPF	DOWN	HORIZ	UPLIFT

DRY: SEASONED LUMBER.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
S	1457	989 / 0 0 / 0 0 / 0 0 / 0 489 / 0 0 / 0
L	1457	989 / 0 0 / 0 0 / 0 0 / 0 489 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.94 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD LC1	MAX	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	C-R	0 / 93	0.03 (4)
B-C	0 / 18	-91.8 -91.8	0.20 (1)	10.00	R-D	0 / 121	0.04 (4)
C-D	-2799 / 0	-91.8 -91.8	0.32 (1)	3.88	D-P	0 / 1267	0.29 (1)
D-E	-3508 / 0	-91.8 -91.8	0.95 (1)	2.94	P-E	-843 / 0	0.25 (1)
E-F	-3507 / 0	-91.8 -91.8	0.93 (1)	2.94	E-O	-2 / 0	0.00 (1)
F-G	-3507 / 0	-91.8 -91.8	0.93 (1)	2.94	O-G	-843 / 0	0.25 (1)
G-H	-3507 / 0	-91.8 -91.8	0.94 (1)	2.95	O-H	0 / 1268	0.29 (1)
H-I	-2799 / 0	-91.8 -91.8	0.32 (1)	3.88	H-M	0 / 121	0.04 (4)
I-J	0 / 16	-91.8 -91.8	0.20 (1)	10.00	M-I	0 / 93	0.03 (4)
J-K	0 / 28	-91.8 -91.8	0.12 (1)	10.00	S-C	-2974 / 0	0.83 (1)
S-B	-270 / 0	0.0 0.0	0.03 (1)	7.81	I-L	-2974 / 0	0.83 (1)
L-J	-270 / 0	0.0 0.0	0.03 (1)	7.81			

S-R	0 / 2417	-18.5 -18.5	0.53 (1)	10.00
R-Q	0 / 2489	-18.5 -18.5	0.54 (1)	10.00
Q-P	0 / 2489	-18.5 -18.5	0.54 (1)	10.00
P-O	0 / 3509	-18.5 -18.5	0.64 (1)	10.00
O-N	0 / 2489	-18.5 -18.5	0.54 (1)	10.00
N-M	0 / 2489	-18.5 -18.5	0.54 (1)	10.00
M-L	0 / 2417	-18.5 -18.5	0.53 (1)	10.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/380 (1.17")
CALCULATED VERT. DEFL. (LL) = L/999 (0.21")
ALLOWABLE DEFL. (TL) = L/380 (1.17")
CALCULATED VERT. DEFL. (TL) = L/899 (0.41")

CS1: TC=0.98/1.00 (D-E:1), BC=0.84/1.00 (O-P:1), WB=0.83/1.00 (I-L:1), SS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (FU)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 -763 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

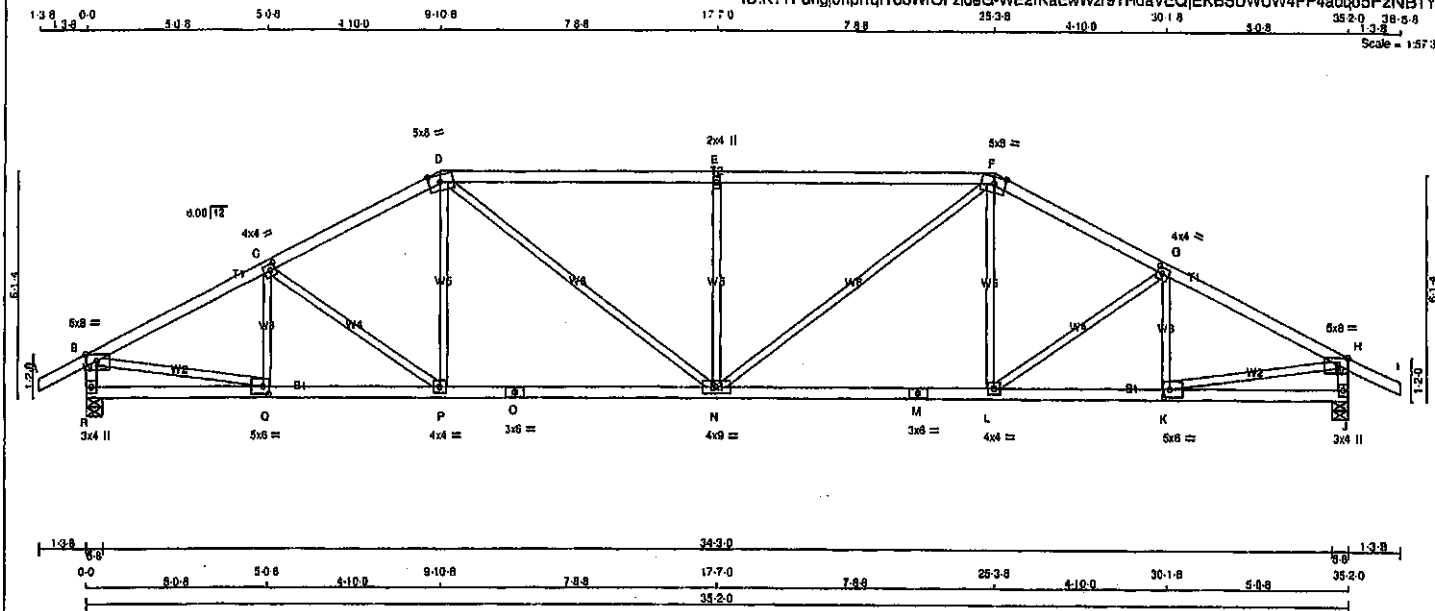
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (L) (INPUT = 0.90)
JSI METAL = 0.77 (N) (INPUT = 1.00)



Structural component only
DWG# T-2007170

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T42	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 26 13:02:19 2020 Page 1			
		ID:K?TPdngi0npl1qIYdbWlOFzIdG-WE2rKaEwWzr9THdaVEQIEKBSUWUW4PP4adpo5FzNB1Y			



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	8.0	Edge 3.50
C	TMVW-l	MT20	4.0	4.0	2.00 1.75
D	TTVW-m	MT20	5.0	8.0	2.25 3.75
E	TMVW-w	MT20	2.0	4.0	
F	TTVW-m	MT20	5.0	8.0	2.25 3.75
G	TMVW-l	MT20	4.0	4.0	2.00 1.75
H	TMVW-p	MT20	5.0	8.0	Edge 3.50
J	BMV1-p	MT20	3.0	4.0	
K	BMVW-l	MT20	5.0	8.0	2.50 2.00
L	BMVW-l	MT20	4.0	4.0	
M	BS-l	MT20	3.0	6.0	
N	BMVW-l	MT20	4.0	9.0	
O	BS-l	MT20	3.0	6.0	
P	BMVW-l	MT20	4.0	4.0	
Q	BMVW-l	MT20	5.0	6.0	2.50 2.00
R	BMV1-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
R	2063	0	2063	0	0	5-8	5-8	5-8	5-8
J	2063	0	2063	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LOSE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
R	1437	888 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
J	1437	888 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	Q-C	-353 / 0	0.08 (1)
B-C	-2834 / 0	-91.8	-91.8 0.39 (1)	3.81	C-P	-210 / 0	0.13 (1)
C-D	-2887 / 0	-91.8	-91.8 0.37 (1)	3.91	P-D	0 / 246	0.08 (4)
D-E	-3080 / 0	-91.8	-91.8 0.97 (1)	2.78	D-N	0 / 844	0.19 (1)
E-F	-3080 / 0	-91.8	-91.8 0.97 (1)	2.78	N-E	-872 / 0	0.51 (1)
F-G	-2887 / 0	-91.8	-91.8 0.37 (1)	3.91	N-F	0 / 844	0.19 (1)
G-H	-2834 / 0	-91.8	-91.8 0.39 (1)	3.81	L-F	0 / 246	0.06 (4)
H-I	0 / 28	-91.8	-91.8 0.12 (1)	10.00	L-G	-210 / 0	0.13 (1)
R-B	-2018 / 0	0.0	0.0 0.20 (1)	5.94	K-G	-353 / 0	0.08 (1)
J-H	-2018 / 0	0.0	0.0 0.20 (1)	5.94	B-Q	0 / 2591	0.58 (1)
R-Q	0 / 0	-18.5	-18.5 0.10 (4)	10.00	K-H	0 / 2591	0.58 (1)
Q-P	0 / 2555	-18.5	-18.5 0.51 (1)	10.00			
P-O	0 / 2388	-18.5	-18.5 0.49 (1)	10.00			
O-N	0 / 2388	-18.5	-18.5 0.49 (1)	10.00			
N-M	0 / 2388	-18.5	-18.5 0.49 (1)	10.00			
M-L	0 / 2388	-18.5	-18.5 0.49 (1)	10.00			
L-K	0 / 2555	-18.5	-18.5 0.51 (1)	10.00			
K-J	0 / 0	-18.5	-18.5 0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN PLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (1.17")
CALCULATED VERT. DEFL. (LL) = 1/989 (0.17")
ALLOWABLE DEFL. (TL) = 1/360 (1.17")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.34")

CSI: TC=0.97/1.00 (D-E:1), BC=0.51/1.00 (P-Q:1),
WB=0.59/1.00 (B-Q:1), SS=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1987 1693

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (F) (INPUT = 0.90)
JSI METAL = 0.75 (M) (INPUT = 1.00)

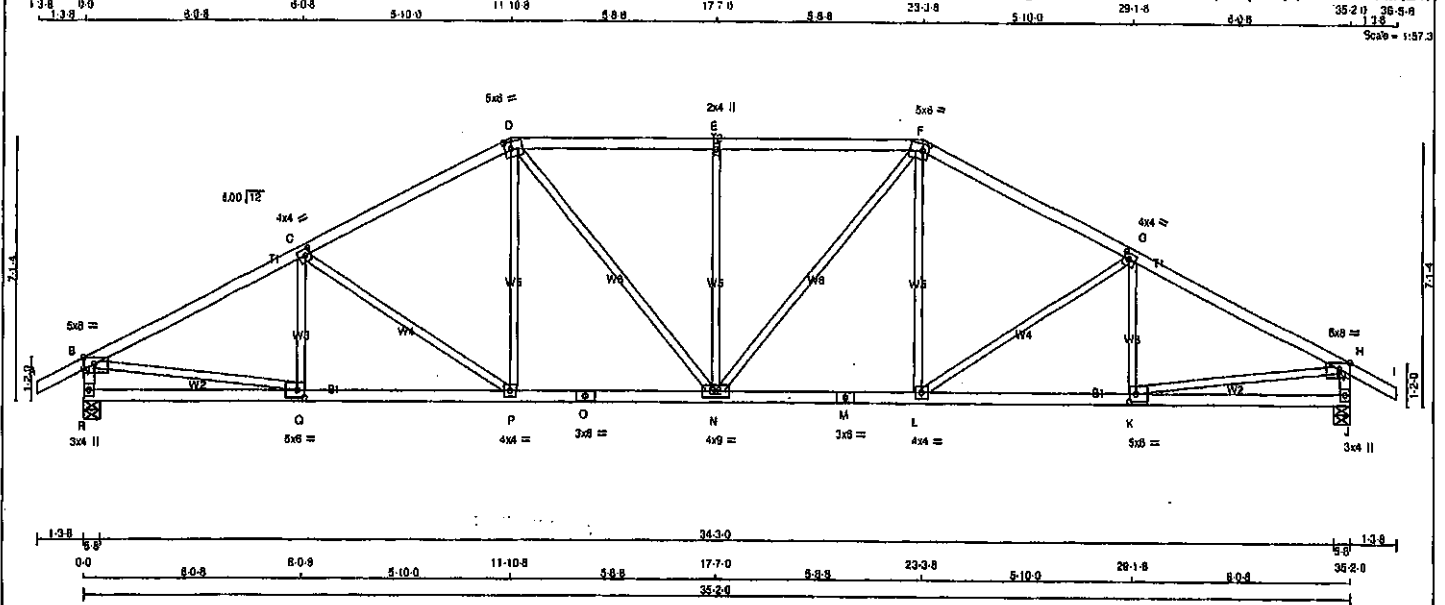


Structural component only
DWG# T-2007171

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T43	2	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (tablets in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMWV-p	MT20	5.0	8.0 Edge 3.50
C	TMWV-l	MT20	4.0	4.0 2.00 1.75
D	TTWV-m	MT20	5.0	8.0 2.25 2.00
E	TMWV-w	MT20	2.0	4.0
F	TTWV-m	MT20	5.0	8.0 2.25 2.00
G	TMWV-l	MT20	4.0	4.0 2.00 1.75
H	TMWV-p	MT20	5.0	8.0 Edge 3.50
J	BMV1-p	MT20	3.0	4.0
K	BMWV-l	MT20	5.0	8.0 2.50 2.25
L	BMWV-l	MT20	4.0	4.0
M	BS-l	MT20	3.0	8.0
N	BMWV-l	MT20	4.0	9.0
O	BS-l	MT20	3.0	8.0
P	BMWV-l	MT20	4.0	4.0
Q	BMWV-l	MT20	5.0	8.0 2.50 2.25
R	BMV1-p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED	MAXIMUM FACTORED	INPUT	RECORD			
	GROSS REACTION	GROSS REACTION	BRG	BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	2083	0	2083	0	0	5-8	5-8
J	2083	0	2083	0	0	5-8	5-8

UNFACTORED REACTIONS							
	1ST CASE	MAX. MIN.	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1457	989 / 0	0 / 0	0 / 0	0 / 0	489 / 0	0 / 0
J	1457	989 / 0	0 / 0	0 / 0	0 / 0	489 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING			
TOTAL LOAD CASES: (4)			
CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORZ. LOAD (LBS)
FR-TO			
A-B	0 / 28	-91.8 -91.8 0.12 (1)	10.00 Q-C -285 / 12 0.07 (1)
B-C	-2889 / 0	-91.8 -91.8 0.58 (1)	3.68 Q-P -435 / 0 0.42 (1)
C-D	-2539 / 0	-91.8 -91.8 0.52 (1)	3.83 P-D 0 / 360 0.08 (1)
D-E	-2559 / 0	-91.8 -91.8 0.48 (1)	3.85 D-N 0 / 482 0.11 (1)
E-F	-2559 / 0	-91.8 -91.8 0.48 (1)	3.85 N-E -841 / 0 0.58 (1)
F-G	-2559 / 0	-91.8 -91.8 0.52 (1)	3.83 N-F 0 / 482 0.11 (1)
G-H	-2889 / 0	-91.8 -91.8 0.58 (1)	3.68 L-F 0 / 360 0.08 (1)
H-I	0 / 28	-91.8 -91.8 0.12 (1)	10.00 L-G -435 / 0 0.42 (1)
I-J	-2014 / 0	0.0 0.0 0.20 (1)	6.95 K-G -255 / 12 0.07 (1)
J-H	-2014 / 0	0.0 0.0 0.20 (1)	6.95 B-Q 0 / 2835 0.59 (1)
K-H	0 / 0	-18.5 -18.5 0.15 (4)	10.00 K-H 0 / 2835 0.59 (1)
R-Q	0 / 0	-18.5 -18.5 0.15 (4)	10.00
Q-P	0 / 2609	-18.5 -18.5 0.49 (1)	10.00
P-O	0 / 2249	-18.5 -18.5 0.43 (1)	10.00
O-N	0 / 2249	-18.5 -18.5 0.43 (1)	10.00
N-M	0 / 2249	-18.5 -18.5 0.43 (1)	10.00
M-L	0 / 2249	-18.5 -18.5 0.43 (1)	10.00
L-K	0 / 2609	-18.5 -18.5 0.49 (1)	10.00
K-J	0 / 0	-18.5 -18.5 0.15 (4)	10.00

TOTAL WEIGHT = 2 X 145 = 289 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012, ABO 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.58/1.00 (B-C:1), BC=0.49/1.00 (P-Q:1),
WB=0.59/1.00 (B-Q:1), SS=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1087 768 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (Q) (INPUT = 0.90)
JSI METAL= 0.89 (M) (INPUT = 1.00)



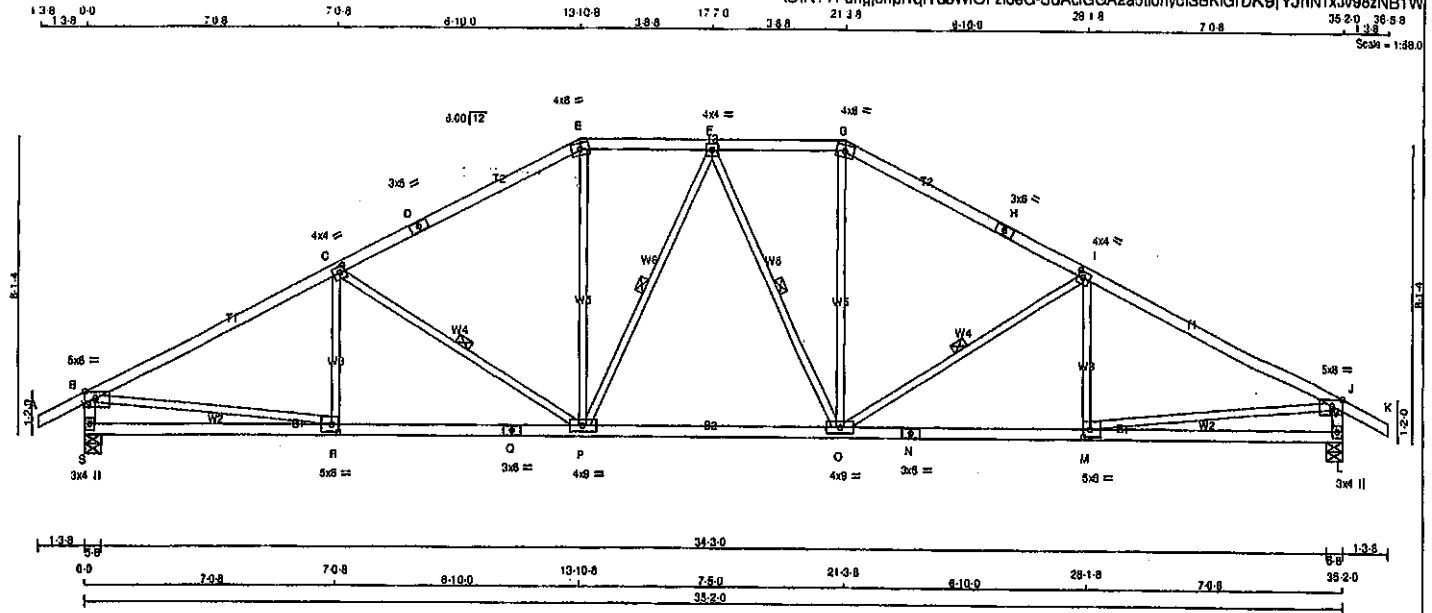
Structural component only
DWG# T-2007172

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408170	T44	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 145 = 291 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - E	2x4 DRY	No.2	SPF		
E - G	2x4 DRY	No.2	SPF		
G - H	2x4 DRY	No.2	SPF		
H - K	2x4 DRY	No.2	SPF		
S - B	2x4 DRY	No.2	SPF		
L - J	2x4 DRY	No.2	SPF		
S - Q	2x4 DRY	No.2	SPF		
Q - N	2x4 DRY	No.2	SPF		
N - L	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMWW-p	MT20	5.0	8.0	Edge 3.50
C TMWW-i	MT20	4.0	4.0	2.00 1.75
D TS-i	MT20	3.0	6.0	
E TTW-m	MT20	4.0	6.0	
F TMWW-i	MT20	4.0	4.0	
G TTW-m	MT20	4.0	6.0	
H TS-i	MT20	3.0	6.0	
I TMWW-i	MT20	4.0	4.0	2.00 1.75
J TMWW-p	MT20	5.0	8.0	Edge 3.50
L BMV-i	MT20	3.0	4.0	
M BMWW-i	MT20	5.0	6.0	2.50 2.25
N BS-i	MT20	3.0	6.0	
O BMWW-i	MT20	4.0	6.0	
P BMWW-i	MT20	4.0	6.0	
Q BS-i	MT20	3.0	6.0	
R BMWW-i	MT20	5.0	6.0	2.50 2.25
S BMV-i	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
S	2063	0	2063	0
L	2063	0	2063	0

UNFACTORED REACTIONS	1ST LGASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM.LIVE	
S	1457	989 / 0	0 / 0	0 / 0	488 / 0
L	1457	989 / 0	0 / 0	0 / 0	488 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-P, F-O, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING	TOTAL LOAD CASES: (4)	CHORDS	MAX. FACTORED	FACTORED	W E B S	MAX. FACTORED
		MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	CS1 (LC)	MEMB. FORCE (LBS)
FR-TO						
A-B	0 / 28			-91.8	-91.8 0.12 (1)	10.00
B-C	-2908 / 0			-91.8	-91.8 0.83 (1)	3.25
C-D	-2373 / 0			-91.8	-91.8 0.72 (1)	3.66
D-E	-2373 / 0			-91.8	-91.8 0.72 (1)	3.66
E-F	-2100 / 0			-91.8	-91.8 0.19 (1)	4.53
F-G	-2100 / 0			-91.8	-91.8 0.19 (1)	4.53
G-H	-2373 / 0			-91.8	-91.8 0.72 (1)	3.66
H-I	-2373 / 0			-91.8	-91.8 0.72 (1)	3.66
I-J	-2908 / 0			-91.8	-91.8 0.83 (1)	3.25
J-K	0 / 28			-91.8	-91.8 0.12 (1)	10.00
S-B	-2008 / 0			0.0	0.0 0.20 (1)	5.96
L-J	-2008 / 0			0.0	0.0 0.20 (1)	5.96
S-R	0 / 0			-18.5	-18.5 0.20 (4)	10.00
R-Q	0 / 2630			-18.5	-18.5 0.52 (1)	10.00
Q-P	0 / 2630			-18.5	-18.5 0.52 (1)	10.00
P-O	0 / 2193			-18.5	-18.5 0.45 (1)	10.00
O-N	0 / 2630			-18.5	-18.5 0.52 (1)	10.00
N-M	0 / 2630			-18.5	-18.5 0.52 (1)	10.00
M-L	0 / 0			-18.5	-18.5 0.20 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
OL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. G.C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CS1: TC=0.83/1.00 (B-C:1), BC=0.52/1.00 (P-R:1),
WB=0.80/1.00 (B-R:1), SS=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

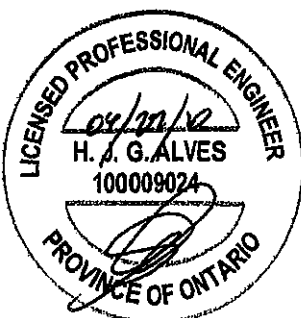
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1887 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

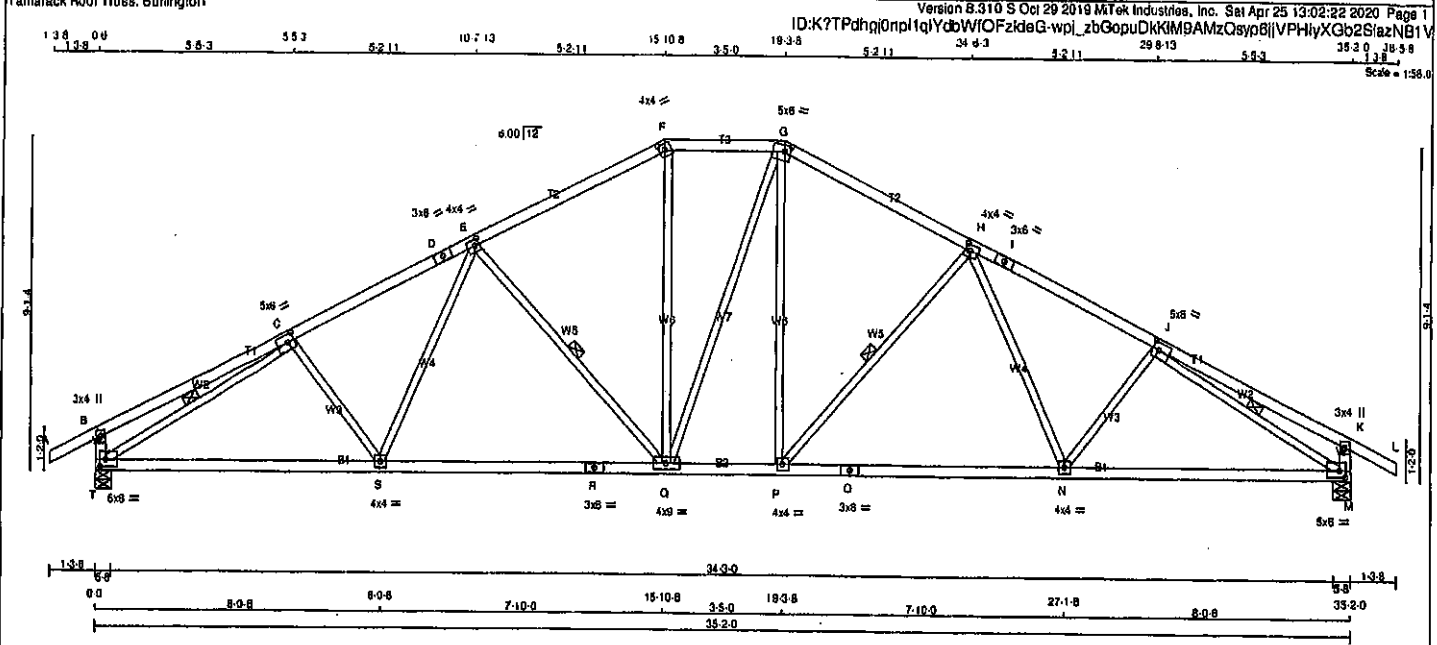
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.88 (M) (INPUT = 0.90)
JSI METAL=0.79 (Q) (INPUT = 1.00)



Structural component only
DWG# T-2007173

JOB NAME 408170	TRUSS NAME T45	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER

N. L. G. A. RULES

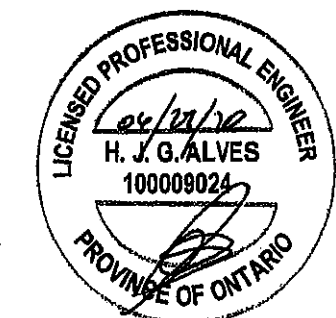
CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - K	2x4	DRY	No.2
K - R	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0	
C	TMWW-t	MT20	5.0	6.0	2.50 2.25
D	TS-l	MT20	3.0	6.0	
E	TMWW-t	MT20	4.0	4.0	2.00 1.50
F	TTW-h	MT20	4.0	4.0	2.00 1.75
G	TTWW-m	MT20	5.0	6.0	2.00 2.00
H	TMWW-t	MT20	4.0	4.0	2.00 1.50
I	TS-l	MT20	3.0	6.0	
J	TMWW-t	MT20	5.0	6.0	2.50 2.25
K	TMV-p	MT20	3.0	4.0	
M	BMVW-l	MT20	5.0	6.0	2.25 2.00
N, P, S					
N	BMWW-t	MT20	4.0	4.0	
O	BS-l	MT20	3.0	6.0	
Q	BMWW-t	MT20	4.0	6.0	
R	BS-l	MT20	3.0	6.0	
T	BMVW-l	MT20	5.0	6.0	2.25 2.00



Structural component only
DWG# T-2007174

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
T	2083	0	2083	0
M	2083	0	2083	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE PERM. LIVE WIND DEAD SOIL
T	1457	969 / 0	0 / 0 0 / 0 0 / 0 488 / 0 0 / 0
M	1457	969 / 0	0 / 0 0 / 0 0 / 0 488 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, H-P, C-T, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO				FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	C-S	-110 / 37	0.04 (1)
B-C	0 / 19	-91.8	-91.8 0.32 (1)	10.00	S-E	0 / 276	0.08 (1)
C-D	-2759 / 0	-91.8	-91.8 0.40 (1)	3.88	E-Q	-681 / 0	0.31 (1)
D-E	-2759 / 0	-91.8	-91.8 0.40 (1)	3.88	Q-F	0 / 613	0.14 (1)
E-F	-2187 / 0	-91.8	-91.8 0.38 (1)	4.30	F-G	0 / 4	0.00 (1)
F-G	-1926 / 0	-91.8	-91.8 0.18 (1)	4.71	P-G	0 / 609	0.14 (1)
G-H	-2186 / 0	-91.8	-91.8 0.38 (1)	4.30	P-H	-682 / 0	0.31 (1)
H-I	-2780 / 0	-91.8	-91.8 0.40 (1)	3.88	H-N	0 / 278	0.08 (1)
I-J	-2780 / 0	-91.8	-91.8 0.40 (1)	3.88	N-J	-110 / 37	0.04 (1)
J-K	0 / 19	-91.8	-91.8 0.32 (1)	10.00	T-C	-3040 / 0	0.85 (1)
K-L	0 / 28	-91.8	-91.8 0.12 (1)	10.00	J-M	-3041 / 0	0.85 (1)
T-B	-328 / 0	0.0	0.0 0.03 (1)	7.81			
M-K	-328 / 0	0.0	0.0 0.03 (1)	7.81			
T-S	0 / 2538	-18.5	-18.5 0.55 (1)	10.00			
S-R	0 / 2382	-18.5	-18.5 0.53 (1)	10.00			
R-Q	0 / 2382	-18.5	-18.5 0.53 (1)	10.00			
Q-P	0 / 1924	-18.5	-18.5 0.40 (1)	10.00			
P-O	0 / 2382	-18.5	-18.5 0.53 (1)	10.00			
O-N	0 / 2382	-18.5	-18.5 0.53 (1)	10.00			
N-M	0 / 2537	-18.5	-18.5 0.58 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(66 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.32")

CSI: TC=0.40/1.00 (H-J-I), BC=0.58/1.00 (M-N-I), WB=0.85/1.00 (J-M-I), SS=0.20/1.00 (B-C-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

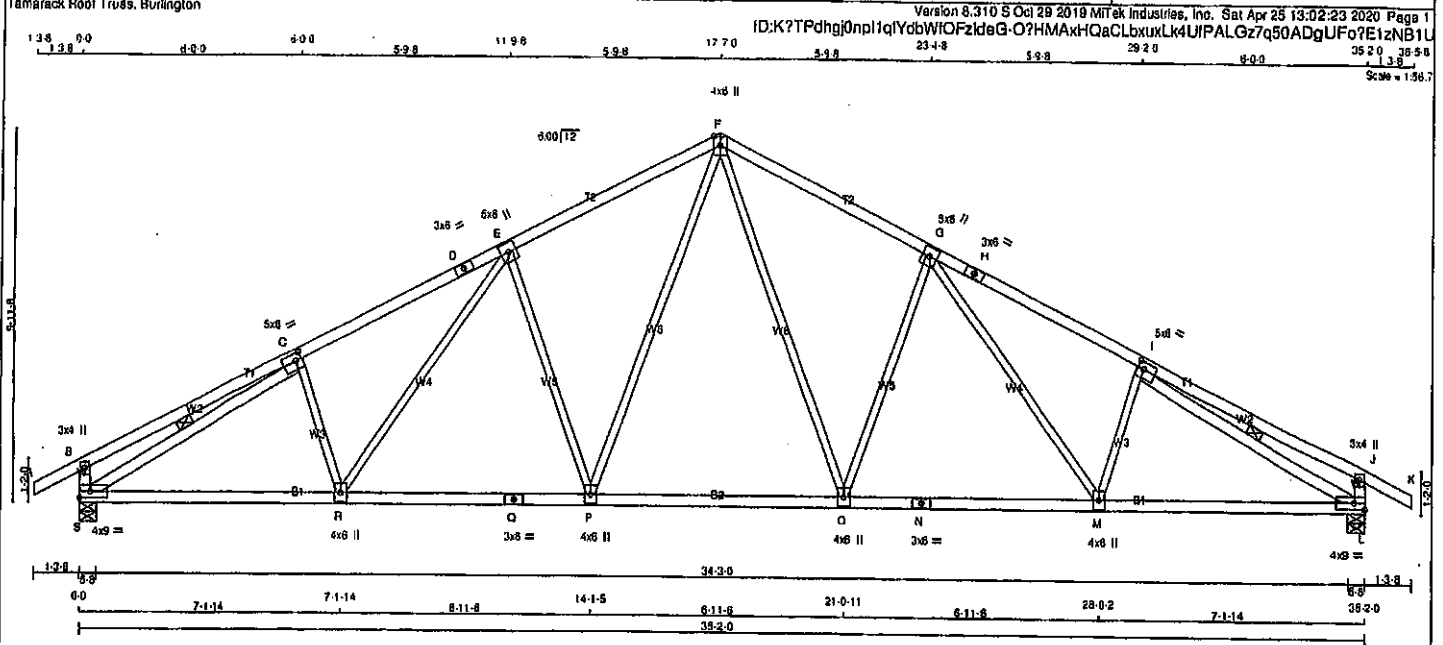
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (M) (INPUT = 0.90)
JSI METAL = 0.78 (O) (INPUT = 1.00)

TOTAL WEIGHT = 2 X 151 = 302 lb (M/F)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T46	6	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR. SPF
A - D 2x4 DRY No.2
D - F 2x4 DRY No.2
F - H 2x4 DRY No.2
H - K 2x4 DRY No.2
S - B 2x4 DRY No.2
L - J 2x4 DRY No.2
S - Q 2x4 DRY No.2
Q - N 2x4 DRY No.2
N - L 2x4 DRY No.2
ALL WEBS 2x3 DRY No.2
EXCEPT
S - C 2x4 DRY No.2
I - L 2x4 DRY No.2
DRY: SEASONED LUMBER.

FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
S	2083	0	2083	0	5-8	5-8	
L	2083	0	2083	0	5-8	5-8	

UNFACTORED REACTIONS
1ST LCASE MAX/MIN COMPONENT REACTIONS
JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL
S 1457 969 / 0 0 / 0 0 / 0 488 / 0 0 / 0
L 1457 969 / 0 0 / 0 0 / 0 488 / 0 0 / 0
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-S, I-L.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.		FORCE		MEMB.		FORCE	
FR-TO		FROM TO		FR-TO		FROM TO	
A-B		-91.8 -91.8 0.12 (1)		F-O		0 / 866	
B-C		-91.8 -91.8 0.40 (1)		O-G		-728 / 0	
C-D		-2808 / 0		G-M		0 / 391	
D-E		-2808 / 0		M-I		-192 / 18	
E-F		-2322 / 0		F-P		0 / 866	
F-G		-2322 / 0		P-R		-728 / 0	
G-H		-2808 / 0		R-E		0 / 391	
H-I		-2808 / 0		E-C		-192 / 18	
I-J		0 / 22		S-C		-3084 / 0	
J-K		0 / 28		C-L		-3084 / 0	
K-S		-344 / 0					
L-J		-344 / 0					

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.
S-R 0 / 2574 -18.5 -18.5 0.53 (1) 10.00
R-Q 0 / 2281 -18.5 -18.5 0.47 (1) 10.00
Q-P 0 / 2281 -18.5 -18.5 0.47 (1) 10.00
P-O 0 / 1762 -18.5 -18.5 0.38 (1) 10.00
O-N 0 / 2281 -18.5 -18.5 0.47 (1) 10.00
N-M 0 / 2281 -18.5 -18.5 0.47 (1) 10.00
M-L 0 / 2574 -18.5 -18.5 0.53 (1) 10.00

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF 8CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/989 (0.14")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = L/989 (0.28")

CSI: TO=0.49/1.00 (G-I:1), BC=0.53/1.00 (R-S:1),
WB=0.72/1.00 (E-P:1), SI=0.22/1.00 (B-C:1)
DOJ LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

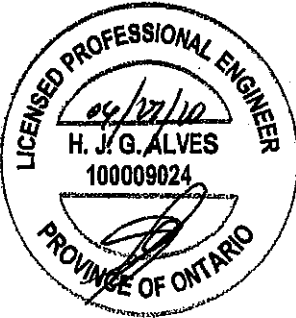
COMPANION LIVE LOAD FACTOR = 1.00
AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1887 1656

PLATE PLACEMENT TOL = 0.250 inches
PLATE ROTATION TOL = 5.0 Deg.

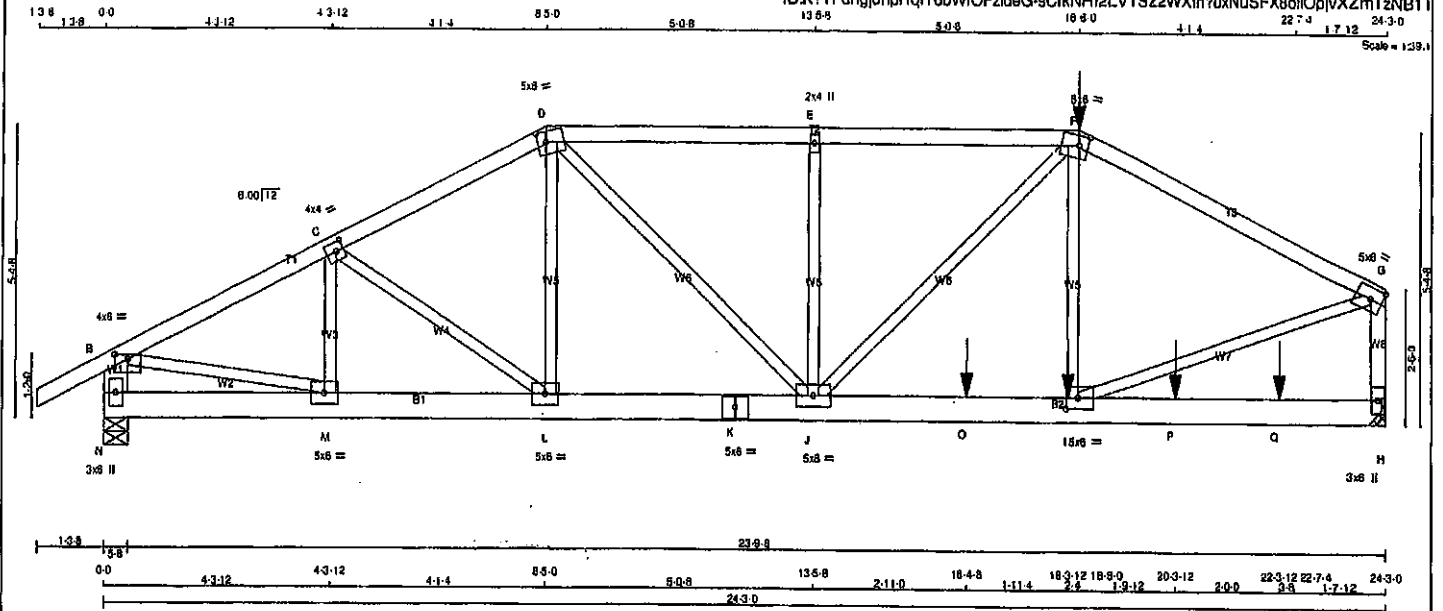
JSI GRIP = 0.97 (C) (INPUT = 0.90)
JSI METAL = 0.77 (I) (INPUT = 1.00)



Structural component only
DWG# T-2007175

JOB NAME 408170	TRUSS NAME T47	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamareck Roof Truss, Burlington					

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TOTAL WEIGHT = 2 X 114 = 228 lb

LUMBER			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
N - B	2x6	DRY	No.2
H - G	2x4	DRY	No.2
N - K	2x6	DRY	No.2
K - H	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D	12	TOP
D - F	12	SIDE(61.0)
F - G	12	SIDE(61.0)
H - G	12	TOP
N - B	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
N - K	12	TOP
K - H	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.
SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
FACTORED	MAXIMUM FACTORED	INPUT	RECORD		
GROSS REACTION	GROSS REACTION	BRG	BRG		
JT VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
N	2120	0	2120	0	5-8
H	3119	0	3119	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 4'-0."

UNFACTORED REACTIONS					
1ST CASE	MAX MIN.	COMPONENT REACTIONS			
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
N	1495	1003 / 0	0 / 0	0 / 0	492 / 0
H	2201	1472 / 0	0 / 0	0 / 0	729 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8 0.07 (1)	10.00	M-C	-468 / 0	0.04 (1)	
B-C	-2814 / 0	-91.8 -91.8 0.17 (1)	5.22	C-L	-69 / 0	0.01 (1)	
C-D	-2892 / 0	-91.8 -91.8 0.17 (1)	5.24	L-D	0 / 70	0.01 (4)	
D-E	-3517 / 0	-91.8 -91.8 0.27 (1)	4.74	D-J	0 / 1336	0.17 (1)	
E-F	-3517 / 0	-91.8 -91.8 0.27 (1)	4.74	J-E	-555 / 0	0.12 (1)	
F-G	-3466 / 0	-91.8 -91.8 0.39 (1)	4.64	J-F	0 / 575	0.07 (1)	
N-B	-2084 / 0	0.0 0.0 0.07 (1)	7.81	F-E	0 / 522	0.06 (1)	
H-G	-2833 / 0	0.0 0.0 0.20 (1)	6.72	I-G	0 / 3293	0.41 (1)	
				B-M	0 / 2864	0.33 (1)	
N-M	0 / 0	-18.5 -18.5 0.03 (1)	10.00				
M-L	0 / 2622	-18.5 -18.5 0.21 (1)	10.00				
L-K	0 / 2588	-18.5 -18.5 0.33 (1)	10.00				
K-J	0 / 2588	-18.5 -18.5 0.33 (1)	10.00				
J-I	0 / 3109	-18.5 -18.5 0.69 (1)	10.00				
I-P	0 / 3109	-18.5 -18.5 0.69 (1)	10.00				
P-Q	0 / 0	-18.5 -18.5 0.32 (1)	10.00				
Q-H	0 / 0	-18.5 -18.5 0.32 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)					
JT	LOC.	LC1	MAX.	MAX+	
F	18-8-0	-178	-178	---	FRONT VERT
I	18-3-12	-35	-35	---	FRONT VERT
O	18-4-8	-1661	-1661	---	FRONT VERT
P	20-3-12	-283	-283	---	FRONT VERT
Q	22-3-12	-284	-284	---	FRONT VERT

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.39/1.00 (F-G-1), BC=0.69/1.00 (I-J-1), WB=0.41/1.00 (G-I-1), SS=0.37/1.00 (I-J-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE GRP(DRY)	SHEAR	SECTION			
(PSI)	(PL)	(PL)			
MAX MIN	MAX MIN	MAX MIN			
MT20	618	354	1687	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (I) (INPUT = 0.90)
JSI METAL = 0.44 (G) (INPUT = 1.00)

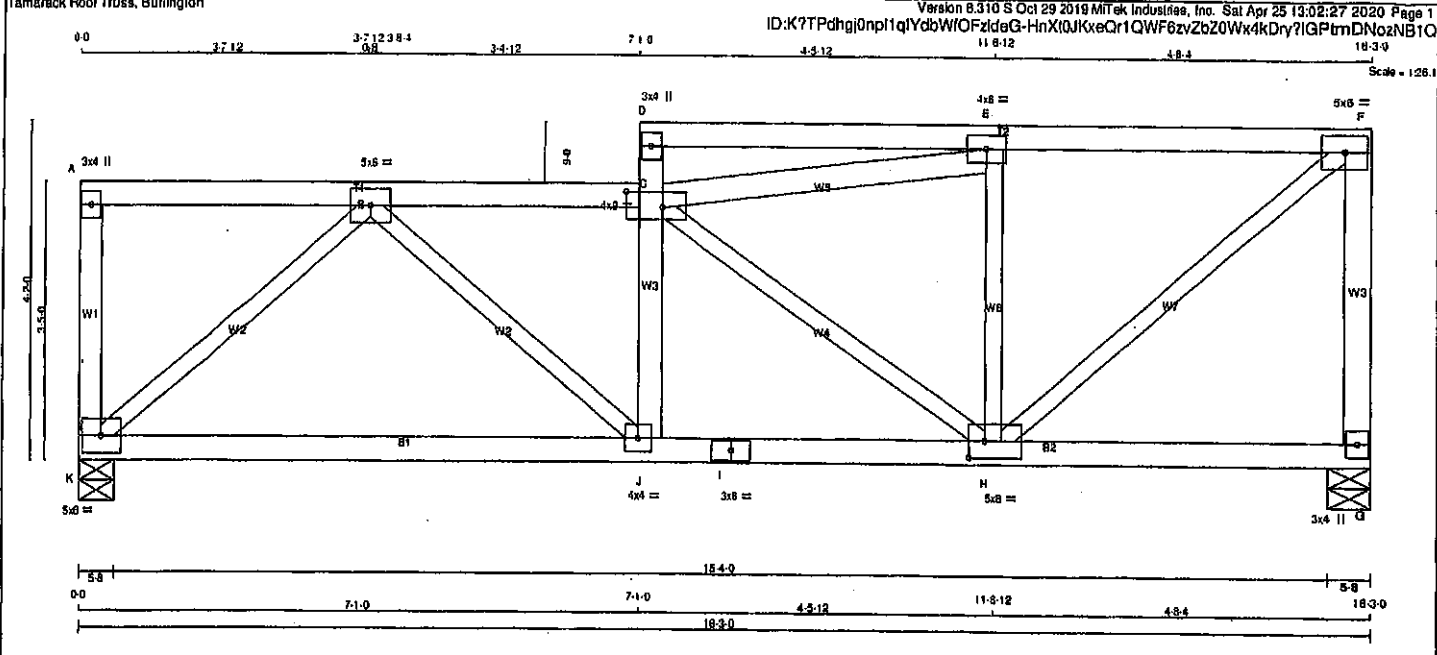


Structural component only
DWG# T-2007176 42



JSI METAL= 0.60 (B) (INPUT = 1.00)





TOTAL WEIGHT = 3 X 72 = 216 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
K - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
J - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
C - E	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMVW-1	MT20	5.0	8.0		
C	TMVW-1	MT20	4.0	9.0	2.25	5.60
D	TMV+p	MT20	3.0	4.0		
E	TMVW-1	MT20	4.0	8.0		
F	TMVW-1	MT20	5.0	8.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-1	MT20	5.0	8.0	2.50	2.50
I	BS-1	MT20	3.0	8.0		
J	BMVW-1	MT20	4.0	4.0		
K	BMVW-1	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORIZ		
K	1079	0	1079	0	5-8	5-8
G	1079	0	1079	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
K	780	418/0	0/0	0/0	0/0	384/0	0/0
G	780	418/0	0/0	0/0	0/0	384/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	UNBRACED LENGTH
K-A	-183/0	0.0	0.04 (1)	7.81	K-B	-1328/0	0.65 (1)
A-B	0/0	-114.3	-114.3	0.32 (1)	10.00	B-J	0/538
B-C	-1418/0	-114.3	-114.3	0.37 (1)	4.47	C-E	-981/0
J-C	-228/0	0.0	0.08 (1)	7.81	C-H	-489/0	0.30 (1)
C-D	-183/0	0.0	0.08 (1)	7.81	H-E	-471/0	0.18 (1)
D-E	-58/0	-114.3	-114.3	0.51 (1)	6.25	E-F	0/1300
E-F	-1004/0	-114.3	-114.3	0.55 (1)	4.81		
G-F	-1039/0	0.0	0.0	0.37 (1)	6.81		
K-J	0/1013	-18.5	-18.5	0.41 (4)	10.00		
J-I	0/1402	-18.5	-18.5	0.48 (4)	10.00		
I-H	0/1402	-18.5	-18.5	0.48 (4)	10.00		
H-G	0/0	-18.5	-18.5	0.13 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 15.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 48.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012, ABC 2010
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.55/1.00 (E-F:1), SC=0.48/1.00 (H-J:4),
WB=0.65/1.00 (B-K:1), SS=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	354	1687
	788	1687	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

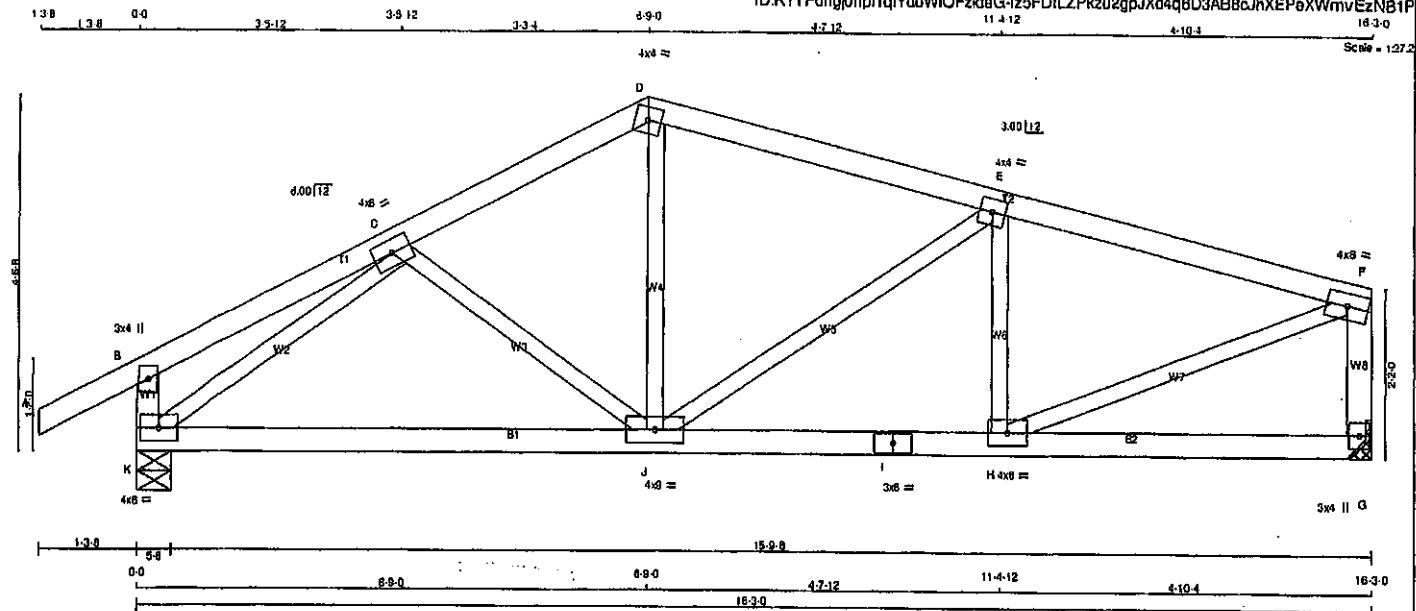
JSI GRIP= 0.85 (C) (INPUT = 0.80)
JSI METAL= 0.82 (I) (INPUT = 1.00)



Structural component only
DWG# T-2007178

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T50	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
K - B	2x4	DRY No.2	SPF
G - F	2x4	DRY No.2	SPF
K - I	2x4	DRY No.2	SPF
I - G	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMVW-1	MT20	4.0	8.0		
D TTW-1	MT20	4.0	4.0		
E TMVW-1	MT20	4.0	4.0		
F TMVW-1	MT20	4.0	8.0		
G BMV1+p	MT20	3.0	4.0		
H BMVW-1	MT20	4.0	6.0		
I BS-1	MT20	3.0	8.0		
J BMVW-1	MT20	4.0	9.0		
K BMVW-1	MT20	4.0	6.0		

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORZ	DOWN	HORZ
K 1020	0	1020	0
G 896	0	896	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3'-8".

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	719	485/0	0/0	0/0	0/0	234/0	0/0
G	633	418/0	0/0	0/0	0/0	218/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX	MAX	MAX	FACTORED	MAX
MEMB.	FORCE	VERT. LOAD	LC1	CS1	UNBRAC	MEMB.	FORCE
FR-TO	(LBS)	FROM TO		(LC)	LENGTH	FR-TO	(LBS)
A-B	0/28	-91.8	-91.8	0.12 (1)	10.00	C-J	-135/7
B-C	0/14	-91.8	-91.8	0.15 (1)	10.00	J-D	0/355
C-D	-982/0	-91.8	-91.8	0.13 (1)	8.20	J-E	-263/0
D-E	-894/0	-91.8	-91.8	0.26 (1)	6.18	H-E	-335/0
E-F	-1094/0	-91.8	-91.8	0.27 (1)	5.73	K-C	-1199/0
K-B	-248/0	0.0	0.0	0.03 (1)	7.81	H-F	0/1150
G-F	-855/0	0.0	0.0	0.10 (1)	7.81		
K-J	0/956	-18.5	-18.5	0.28 (1)	10.00		
J-I	0/1073	-18.5	-18.5	0.27 (1)	10.00		
I-H	0/1073	-18.5	-18.5	0.27 (1)	10.00		
H-G	0/0	-18.5	-18.5	0.10 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.54")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.54")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.09")

CS: TC=0.27/1.00 (E-F:1), BC=0.27/1.00 (H-J:1),
WB=0.35/1.00 (C-K:1), SS=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

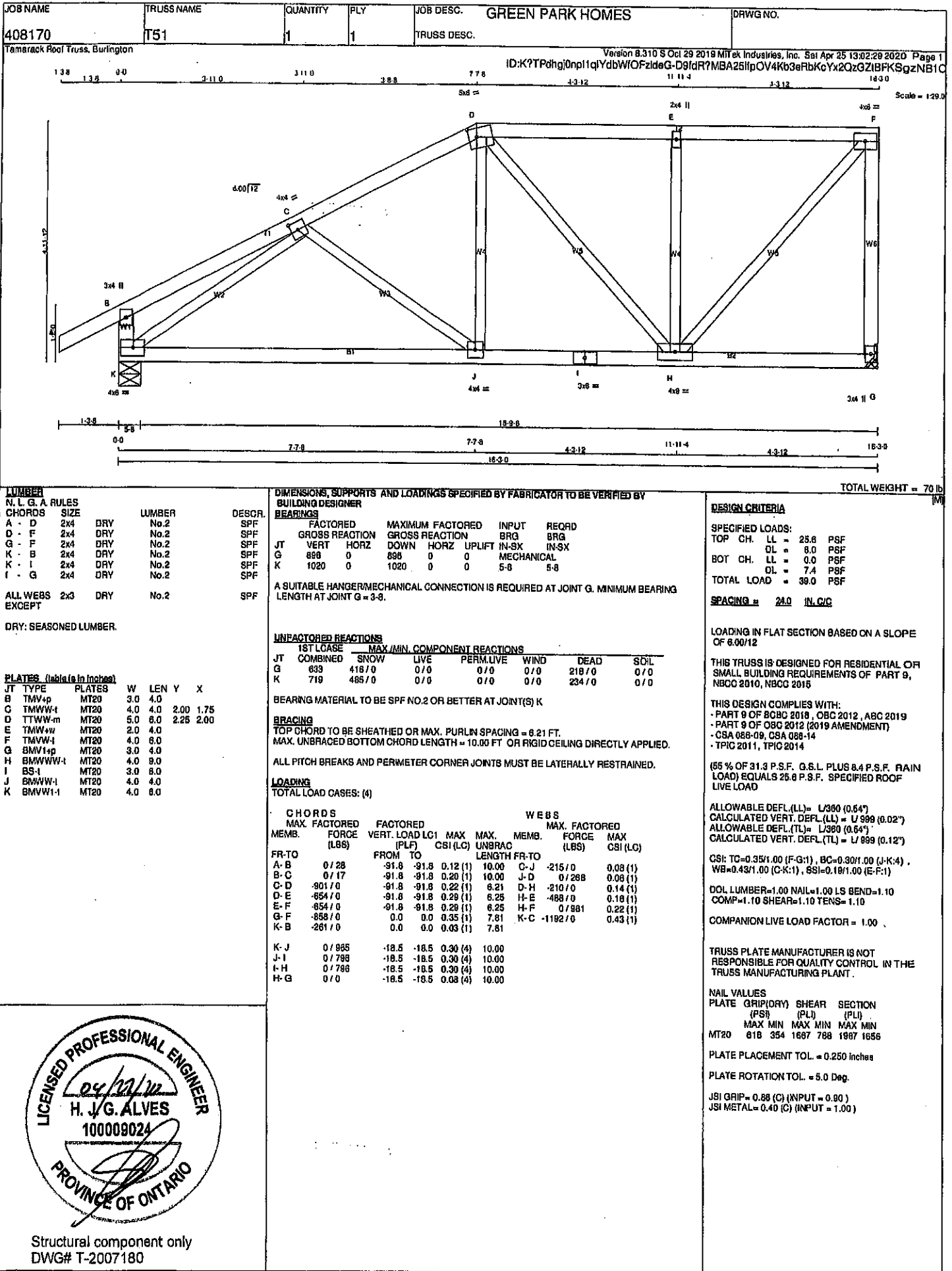
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (H) (INPUT = 0.90)
JSI METAL= 0.32 (H) (INPUT = 1.00)

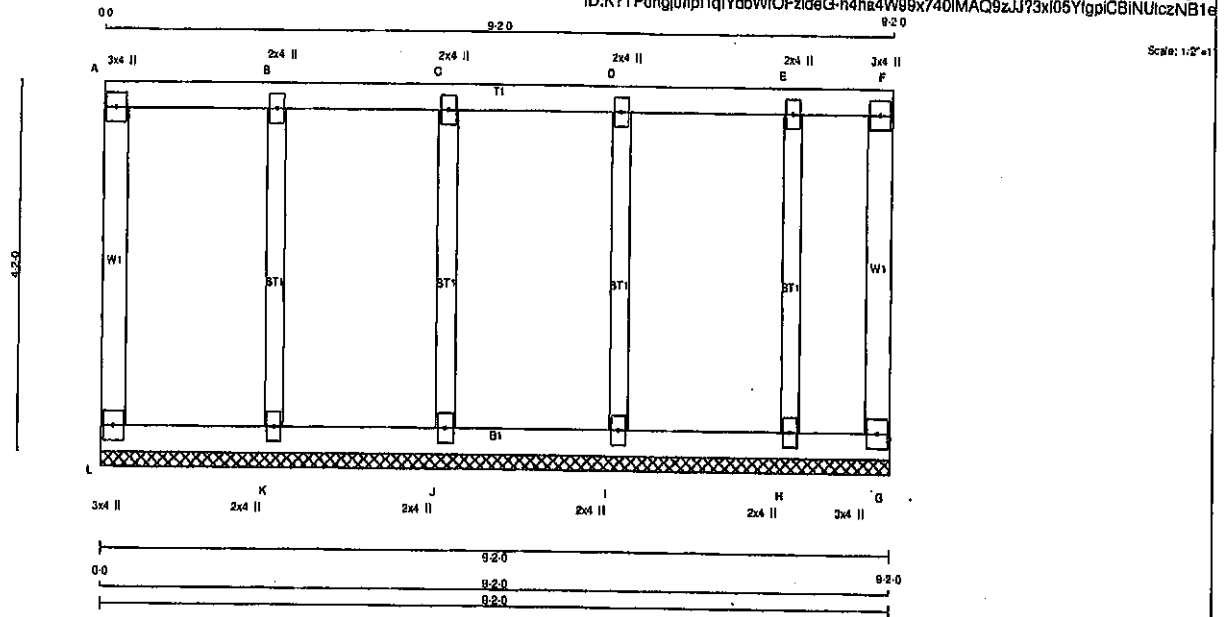


Structural component only
DWG# T-2007179



JOB NAME 408170	TRUSS NAME G52	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER			
N. L. Q. A. RULES	SIZE	DRY	LUMBER
CHORDS			
L - A	2x4	DRY	No.2
A - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (tablets in inches)					
JT	TYPE	PLATES	W	LEN	Y
A	TMV+p	MT20	3.0	4.0	
B, C, D, E					
B	TMV+w	MT20	2.0	4.0	
F	TMV+p	MT20	3.0	4.0	
G	BMV1+p	MT20	3.0	4.0	
H, I, J, K					
H	BMV1+w	MT20	2.0	4.0	
L	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
L-A	-100/0	0.0	0.02 (1)	7.81	K-B	-245/0	0.09 (1)
A-B	-4/0	-114.3 -114.3	0.08 (1)	10.00	J-C	-224/0	0.08 (1)
B-C	-4/0	-114.3 -114.3	0.08 (1)	10.00	I-D	-238/0	0.08 (1)
C-D	-4/0	-114.3 -114.3	0.07 (1)	10.00	H-E	-188/0	0.06 (1)
D-E	-4/0	-114.3 -114.3	0.07 (1)	10.00			
E-F	-4/0	-114.3 -114.3	0.05 (1)	10.00			
G-F	-54/0	0.0	0.02 (1)	7.81			
L-K	0/4	-18.5 -18.5	0.02 (4)	10.00			
K-J	0/4	-18.5 -18.5	0.02 (4)	10.00			
J-I	0/4	-18.5 -18.5	0.02 (4)	10.00			
I-H	0/4	-18.5 -18.5	0.02 (4)	10.00			
H-G	0/4	-18.5 -18.5	0.01 (4)	10.00			

TOTAL WEIGHT = 41 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
DL = 15.0	PSF	
ROT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 48.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPOC 2011, TPOC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (B-C:1), BC=0.02/1.00 (J-K:4), WB=0.09/1.00 (B-K:1), SS=0.13/1.00 (A-B:1)

OOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 768 1937 1636

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

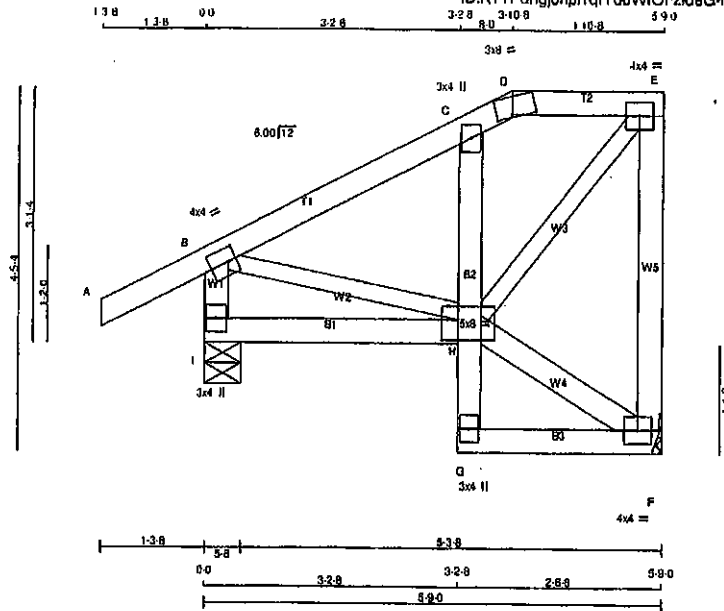
JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.07 (K) (INPUT = 1.00)



Structural component only
DWG# T-2007166

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T53S	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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Scale = 1/28.0

TOTAL WEIGHT = 33 b

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
I - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
G - C	2x4	DRY	No.2
G - F	2x4	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			
B - H	2x3	DRY	No.2
H - E	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWV-I	MT20	4.0	4.0	2.00	1.25
C TMWV-p	MT20	3.0	4.0		
D TT-m	MT20	3.0	6.0	0.50	2.50
E TMWV-I	MT20	4.0	4.0		
F BMWV-I	MT20	4.0	4.0		
G BMWV-p	MT20	3.0	4.0		
H BVMWV-I	MT20	5.0	8.0	3.00	2.50
I BVMV-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG	IN-SX	IN-SX
F	302	0	302	0	0	5-8	5-8		
I	457	0	457	0	0	5-8	5-8		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
F	214	138 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0
I	320	225 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED	MEMB.	MAX. FACTORED
FR-TO	FORCE (LBS)	FR-TO	FORCE (LBS)
A-B	0 / 28	H-F	-10 / 0
B-C	-217 / 0	B-H	0 / 153
C-D	-200 / 0	H-E	0 / 247
D-E	-167 / 0		
F-E	-273 / 0		
I-B	-420 / 0		
I-H	0 / 41		
G-H	0 / 25		
H-C	-168 / 0		
G-F	0 / 16		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/G

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(85 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (B-C:1), BC=0.05/1.00 (H-I:4), WB=0.06/1.00 (E-H:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

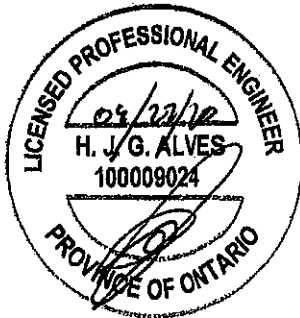
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PLU)
MAX MIN MAX MIN MAX MIN
MT20 618 364 1667 788 1967 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.36 (H) (INPUT = 0.90)
JSI METAL = 0.13 (B) (INPUT = 1.00)



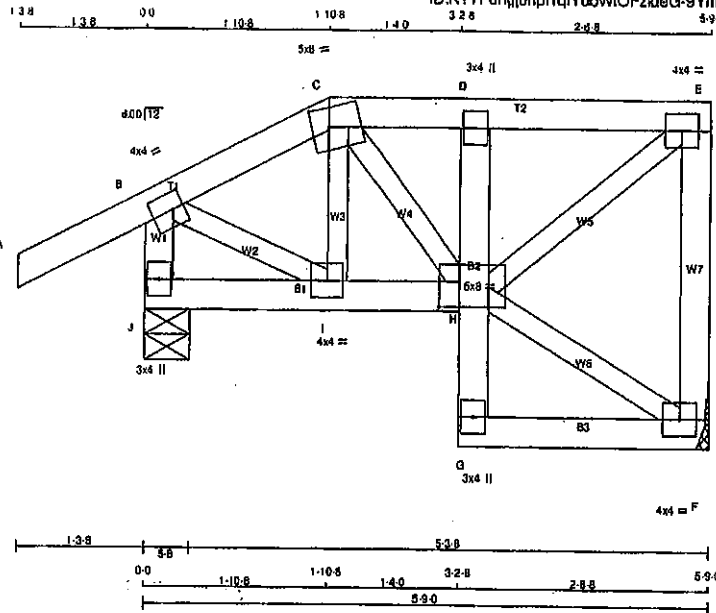
Structural component only
DWG# T-2007181

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408170	T54S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/2" = 1'-0"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - E	2x4	DRY	No.2	SPF		
F - H	2x4	DRY	No.2	SPF		
J - L	2x4	DRY	No.2	SPF		
M - O	2x4	DRY	No.2	SPF		
P - R	2x4	DRY	No.2	SPF		
S - T	2x4	DRY	No.2	SPF		
U - V	2x4	DRY	No.2	SPF		
W - X	2x4	DRY	No.2	SPF		
Y - Z	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMVW-p	MT20	3.0	4.0		
E	TMVW-1	MT20	4.0	4.0		
F	BMVW-1	MT20	4.0	4.0		
G	BMVW-p	MT20	3.0	4.0		
H	BMVW-1	MT20	5.0	6.0	3.00	2.50
I	BMVW-1	MT20	4.0	4.0		
J	BMVW-1	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED	MAXIMUM FACTORED	INPUT	RECORD			
	GROSS REACTION	GROSS REACTION	BRG	BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	302	0	302	0	0	MECHANICAL	
J	458	0	458	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS							
1ST CASE MAX. MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	214	189 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0
J	320	225 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	I-C	-50 / 10	0.01 (1)
B-C	-240 / 0	-91.8 -91.8	0.12 (1)	6.25	C-H	0 / 84	0.02 (1)
C-D	-248 / 0	-91.8 -91.8	0.05 (1)	8.25	H-F	-9 / 0	0.00 (1)
D-E	-241 / 0	-91.8 -91.8	0.07 (1)	8.25	H-E	0 / 302	0.07 (1)
E-F	-278 / 0	0.0 0.0	0.05 (1)	7.81	B-I	0 / 214	0.05 (1)
J-B	-439 / 0	0.0 0.0	0.04 (1)	7.81			
J-I	0 / 0	-18.5 -18.5	0.02 (4)	10.00			
I-H	0 / 193	-18.5 -18.5	0.04 (1)	10.00			
G-H	0 / 25	0.0 0.0	0.02 (1)	10.00			
H-D	-219 / 0	0.0 0.0	0.01 (1)	7.81			
G-F	0 / 8	-18.5 -18.5	0.03 (4)	10.00			

TOTAL WEIGHT = 31 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012, ABC 2010
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/898 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/898 (0.01")

CS1: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (H-I:1), WB=0.07/1.00 (E-H:1), SS=0.10/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

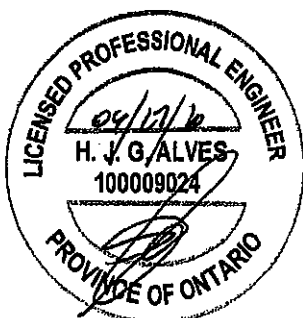
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.33 (B) (INPUT = 0.30)
JSI METAL = 0.13 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007182

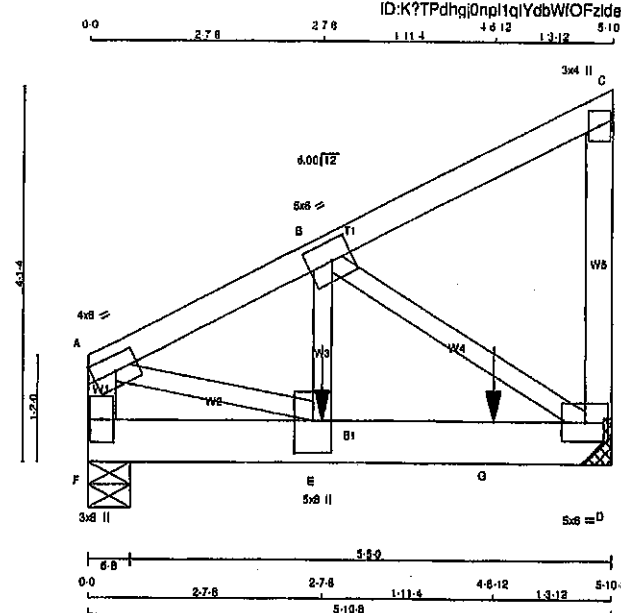
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T55	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 ID:K?TPdghi0npl1qIYdbWIOFzIdeG-dkKm30C3SzTJWH74mT9mG3DvHI 9dNX7Z9U 2?zNB1L

PLATES (table in inches)
 JT TYPE PLATES W LEN Y X
 F BMV1+p MT20 3.0 6.0



Structural component only
 DWG# T-2007183



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	No.2	SPF
A - C	2x4	No.2	SPF
D - C	2x4	No.2	SPF
F - D	2x6	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 1	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	5	SIDE (549.3)
WEBS : (0.122"x3") SPIRAL NAILS		
B-E 1	2	SIDE (259.5)
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	8.0		Edge
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW-t	MT20	5.0	6.0		
E	BMVW-t	MT20	5.0	8.0	4.25	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
F	2334	0	2334	0
D	2733	0	2733	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4'-0."

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	2ND CASE SNOW	3RD CASE LIVE	4TH CASE PERM. LIVE	5TH CASE WIND	6TH CASE DEAD	7TH CASE SOIL
F	1846	1109 / 0	0 / 0	0 / 0	0 / 0	543 / 0	0 / 0
D	1929	1290 / 0	0 / 0	0 / 0	0 / 0	639 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		FR-TO			
F-A	-2316 / 0	0.0	0.0 0.13 (1)	7.37	A-E	0 / 2888	0.38 (1)
A-B	-3101 / 0	-91.8	-91.8 0.08 (1)	5.20	E-B	0 / 2880	0.38 (1)
B-C	-5 / 0	-91.8	-91.8 0.08 (1)	10.00	B-D	-3303 / 0	0.42 (1)
D-C	-138 / 0	0.0	0.0 0.02 (1)	7.81			
F-E	0 / 0	-18.5	-18.5 0.01 (4)	10.00			
E-G	0 / 2779	-18.5	-18.5 0.57 (1)	10.00			
G-D	0 / 2779	-18.5	-18.5 0.57 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2'-7"	-3101	-3101	---	BACK	VERT	TOTAL	---	C1
G	4'-6-12	-1318	-1318	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 57 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	DL
	25.6	8.0
BOT CH.	LL	DL
	0.0	7.4
TOTAL LOAD	39.0	PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012, ABC 2010
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/989 (0.04")

CSI: TO=0.13/1.00 (A-F:1), BC=0.57/1.00 (D-E:1), WB=0.42/1.00 (B-D:1), SI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667	768	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (E) (INPUT = 0.90)
JSI METAL = 0.50 (E) (INPUT = 1.00)



Structural component only
DWG# T-2007184 1/2

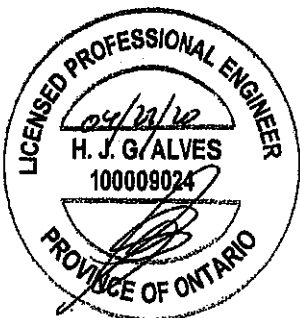
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T56	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
F	8MV1+p	MT20	3.0	6.0	

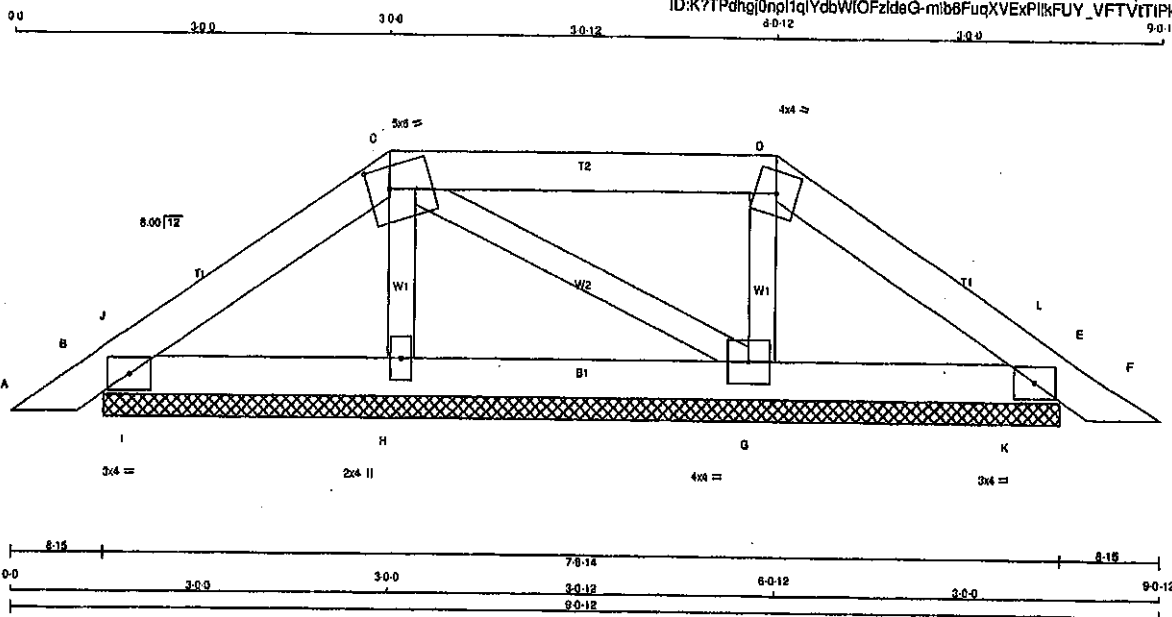
Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



Structural component only
DWG# T-2007184 3/2

[illegible]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	PB20	3	1	GREEN PARK HOMES	
Tamarack Roof Trusses, Burlington		TRUSS DESC.		Version 8.310 5 Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 12:49:22 2020 Page 1	
				ID:K7TPdhgJ0np1qIYdbWIOFzIdaG-mib8FuqXVEpIkFUY_VFTVTIPkW7jvF6AJozNBDh	



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	QESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB14	MT20	3.0	4.0	
C	TTWW-m	MT20	5.0	6.0	2.00 2.00
D	TTWW-m	MT20	4.0	4.0	
E	TMB14	MT20	3.0	4.0	
G	DMW1-w	MT20	4.0	4.0	
H	DMW1-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	JT	GROSS REACTION	BRG	BRG	BRG	BRG
B	221	0	221	0	0	7-6-14	7-6-14
E	209	0	209	0	0	7-6-14	7-6-14
H	248	0	248	0	0	7-6-14	7-6-14
G	280	0	280	0	0	7-6-14	7-6-14

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN. COMPONENT REACTIONS	JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	154	113 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	41 / 0	0 / 0	0 / 0
E	146	108 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0	0 / 0
H	178	109 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0	0 / 0
G	198	127 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (PLF)	MEMB.	FORCE (LBS)	MAX. (LBS)	MAX. (LBS)
FR-TO	0 / 15	-91.8	-91.8 0.03 (1)	FR-TO	0 / 15	-176 / 0	0.03 (1)
A-B	-63 / 0	-91.8	-91.8 0.01 (1)	H-C	-21 / 0	0.00 (1)	0.00 (1)
B-J	-74 / 0	-91.8	-91.8 0.05 (1)	C-G	-189 / 0	0.03 (1)	0.03 (1)
J-C	-30 / 0	-91.8	-91.8 0.16 (1)	G-D	-121 / 0	0.00 (1)	0.00 (1)
C-D	-54 / 0	-91.8	-91.8 0.05 (1)	I-J	-123 / 0	0.00 (1)	0.00 (1)
D-L	-41 / 0	-91.8	-91.8 0.01 (1)	K-L	-123 / 0	0.00 (1)	0.00 (1)
L-E	0 / 15	-91.8	-91.8 0.03 (1)				
E-F	0 / 15	-18.5	-18.5 0.08 (1)				
B-I	0 / 60	-18.5	-18.5 0.08 (1)				
I-H	0 / 60	-18.5	-18.5 0.08 (1)				
H-G	0 / 48	-18.5	-18.5 0.03 (1)				
G-K	0 / 43	-18.5	-18.5 0.08 (1)				
K-E	0 / 43	-18.5	-18.5 0.08 (1)				

TOTAL WEIGHT = 3 X 25 = 75 LB

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCB 2018, OBC 2012, ABC 2018
- PART 9 OF OBC 2012 (2018 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSK: TC=0.15/1.00 (C-D:1), BC=0.08/1.00 (B-I:1), WS=0.03/1.00 (D-G:1), SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

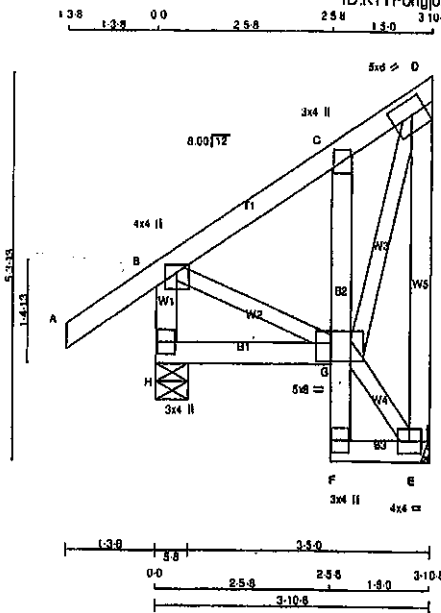
JSI GRIP = 0.18 (B) (INPUT = 0.90)
JSI METAL = 0.04 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007144

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	J20S	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/2" = 1'-0"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
F - C	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table in inches)	PLATES	W	LEN	Y	X
JT TYPE					
B TMVW+p	MT20	4.0	4.0	1.25	2.00
C TMVW+p	MT20	3.0	4.0		
D TMVW-1	MT20	5.0	8.0	2.50	2.50
E BMVW-1	MT20	4.0	4.0		
F BMVW-1	MT20	3.0	4.0		
G BMVW-1	MT20	5.0	8.0	3.00	2.50
H BMV-1	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
H	332	0	332	0
E	208	0	208	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1'-8".

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	
H	232 168 / 0 0 / 0 0 / 0 0 / 0 0 / 0 0
E	145 95 / 0 0 / 0 0 / 0 0 / 0 0 / 0 0 / 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CS1 (LC) UNBRAC LENGTH FR-TO	MEMB. MAX. FACTORED FORCE (LBS) MAX. CS1 (LC)
FR-TO				
H-B	-307 / 0	0.0	0.0 0.03 (1)	7.81 B-G 0 / 83 0.02 (1)
A-B	0 / 35	-91.8	-91.8 0.14 (5)	10.00 E-D -184 / 0 0.05 (1)
B-C	-83 / 0	-91.8	-91.8 0.07 (1)	8.25 G-E -14 / 0 0.00 (1)
C-D	-100 / 0	-91.8	-91.8 0.08 (1)	8.25 G-D 0 / 220 0.08 (1)
H-G	0 / 0	-18.5	-18.5 0.04 (4)	10.00
F-G	0 / 12	0.0	0.0 0.02 (1)	10.00
G-C	-218 / 0	0.0	0.0 0.02 (1)	7.81
F-E	0 / 9	-18.5	-18.5 0.01 (4)	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.18")
CALCULATED VERT. DEFL. (LL) = L/889 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.18")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (A-B-S), BC=0.04/1.00 (G-H-4), WB=0.05/1.00 (D-E-1), SS=0.09/1.00 (A-B-S)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

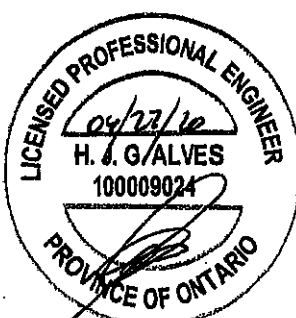
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.21 (B) (INPUT = 0.90)

JSI METAL = 0.07 (C) (INPUT = 1.00)



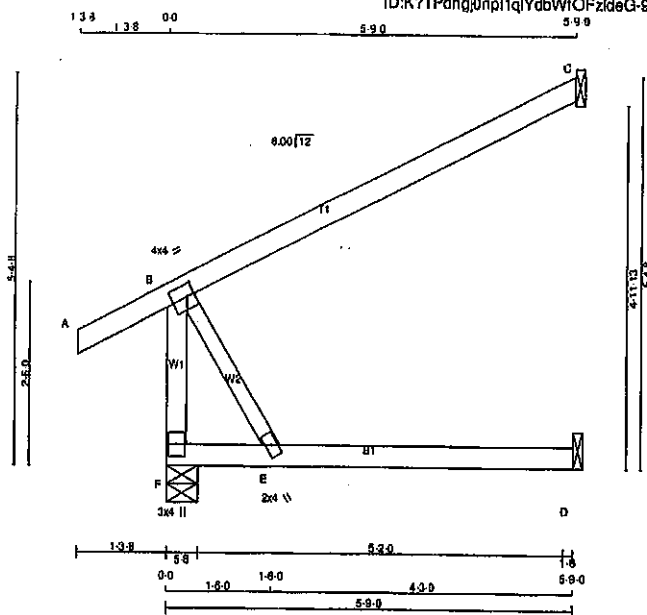
Structural component only
DWG# T-2007143

JOB NAME 408170	TRUSS NAME J40	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Trusses, Burlington

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Scale = 1:29.2



TOTAL WEIGHT = 20 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-1	MT20	4.0	4.0	2.00 1.25
E	BMVW-1	MT20	2.0	4.0	
F	BMV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	441	0	441	0	0	5-8	5-8
C	284	0	284	0	0	1-8	1-8
D	53	0	60	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	310	217 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0
C	182	147 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING							
TOTAL LOAD CASES: (4)							
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO				FR-TO			
F-B	-388 / 0	0.0	0.0 0.05 (1)	8-E	0 / 0	0.00 (1)	
A-B	0 / 28	-91.8	-91.8 0.12 (1)				
B-C	0 / 0	-91.8	-91.8 0.52 (1)				
F-E	0 / 0	-18.5	-18.5 0.14 (4)				
E-D	0 / 0	-18.5	-18.5 0.18 (4)				

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 25.8 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. CC
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/980$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/380$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.05")

OSI: TC=0.52/1.00 (B-C:1), BC=0.18/1.00 (D-E:4), WB=0.00/1.00 (8-E:1), SS=0.18/1.00 (8-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.24 (B) (INPUT = 0.80)
JSI METAL = 0.07 (B) (INPUT = 1.00)



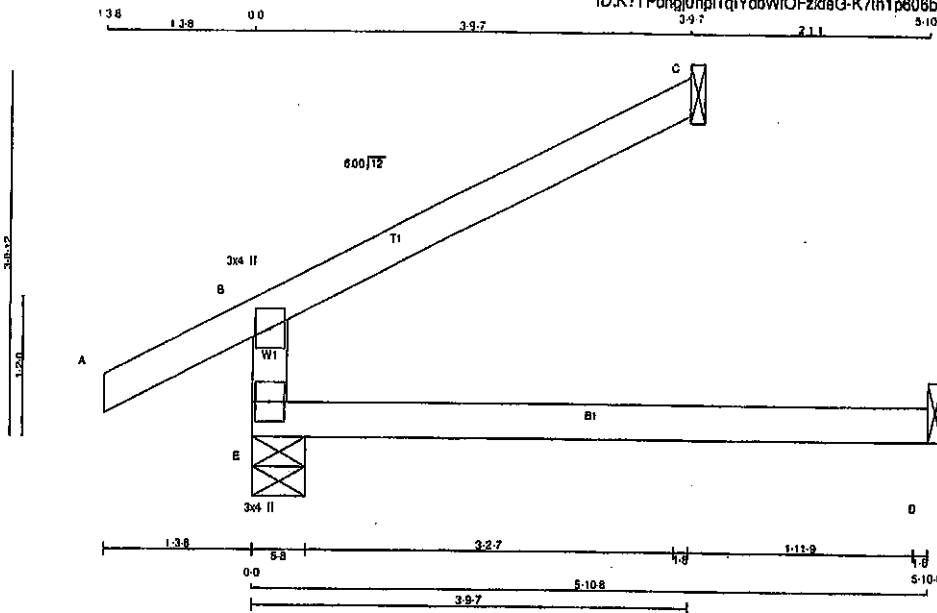
Structural component only
DWG# T-2007167

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408170	C40	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:17.9



LUMBER				
N. L. G. A. RULES	SIZE	DRY	LUMBER	DESCR.
CHORDS	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (tablets in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	HORZ	UP	IN-SX	BRG	BRG
E	405	0	405	0	0	0	0	5-8	5-8
C	130	0	130	0	0	0	0	1-8	1-8
D	45	0	50	0	0	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS		1ST CASE		MAX/MIN COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
E	288	180/0	0/0	0/0	0/0
C	90	73/0	0/0	0/0	0/0
D	36	0/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
E-B	-342/0	0.0	0.0 0.13 (4)	7.81	
A-B	0/28	-91.8	-91.8 0.12 (1)	10.00	
B-C	-19/0	-91.8	-91.8 0.22 (1)	8.25	
E-D	0/0	-18.5	-18.5 0.13 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2010, NBCO 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBCO 2018, CBCO 2012, ABC 2019
- PART 9 OF CBCO 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: $TG=0.22/1.00$ (B-C:1), $BC=0.13/1.00$ (D-E:4),
 $WB=0.00/1.00$ (N/A:0), $SSI=0.16/1.00$ (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

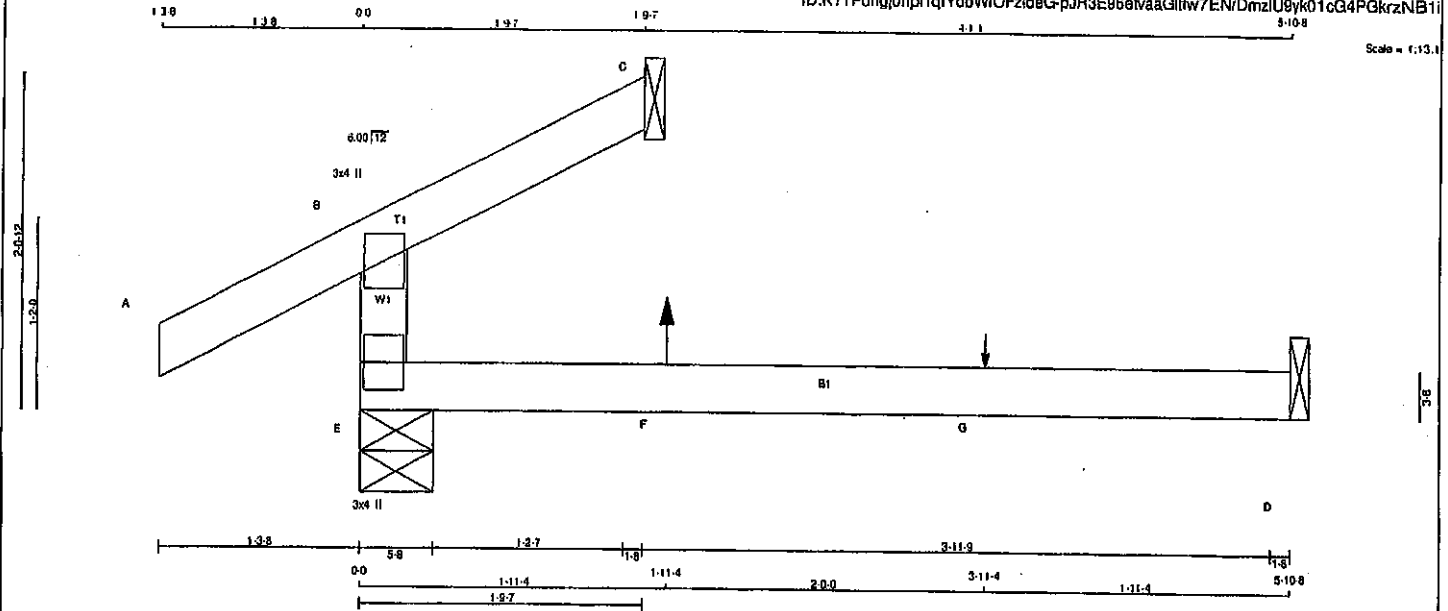
JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007162

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	C41	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS	2x4	DRY	No.2
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
E	284	0	284	0	5-8
C	63	0	63	0	1-8
D	44	0	52	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	200	137/0	0/0	0/0	0/0	82/0	0/0
C	46	21/0	0/0	0/0	0/0	25/0	0/0
D	35	0/3	0/0	0/0	0/0	37/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	MAX. CS1 (LC)	UNBRACED LENGTH
FR-TO				FR-TO			
E-B	-227/0	0.0	0.0	0.11 (4)	7.81		
A-B	0/28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-9/9	-91.8	-91.8	0.08 (4)	10.00		
E-F	0/0	-18.5	-18.5	0.14 (4)	10.00		
F-G	0/0	-18.5	-18.5	0.14 (4)	10.00		
G-D	0/0	-18.5	-18.5	0.14 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	BACK	VERT	TOTAL	--	C1
G	3-11-4	1	1	---	BACK	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/989 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/989 (0.04")

CS1: TC=0.12/1.00 (A-B:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS1=0.09/1.00 (A-B:1)

DOL LUMBER=0.98 NAIL=0.98 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.10 (E) (INPUT = 0.80)
JSI METAL = 0.06 (B) (INPUT = 1.00)

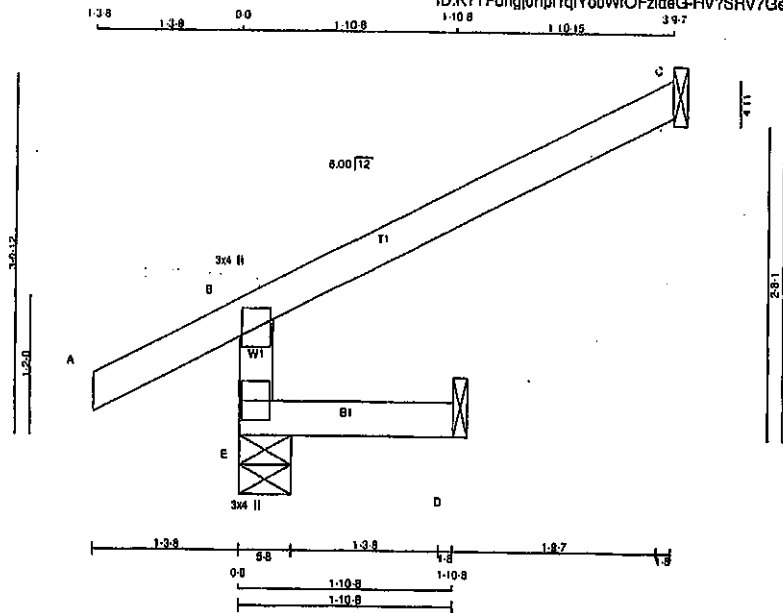


Structural component only
DWG# T-2007163

JOB NAME 408170	TRUSS NAME C42	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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Scale = 1:10.2



LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV+p	MT20	3.0	4.0	
E BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	381	0	381	0	5-8	5-8
C	130	0	130	0	1-8	1-8
D	16	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND		
E	250	180/0	0/0	0/0	60/0	0/0
C	90	73/0	0/0	0/0	17/0	0/0
D	12	0/0	0/0	0/0	12/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)
FR-TO		FROM TO		FROM TO
E-B	-342/0	0.0 0.0 0.01 (4)	7.81	
A-B	0/28	-91.8 -91.8 0.13 (5)	10.00	
B-C	-19/0	-91.8 -91.8 0.22 (1)	6.25	
E-D	0/0	-18.5 -18.5 0.02 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(56 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.10")
CALCULATED VERT. DEFL. (LL) = $L/889$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.10")
CALCULATED VERT. DEFL. (TL) = $L/889$ (0.00")

CSI: TC=0.22/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

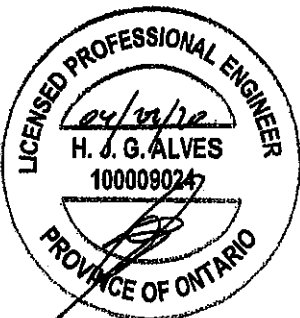
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 394	1687 789

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.14 (E) (INPUT = 0.90)
JSI METAL = 0.09 (E) (INPUT = 1.00)

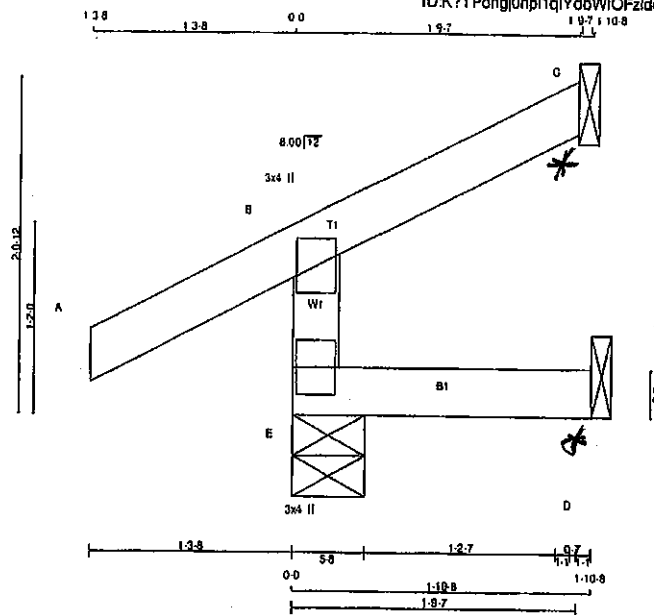


Structural component only
DWG# T-2007164

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	C43	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:12.1



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	271	0	271	0	0	5-8	5-8	
C	45	0	45	0	-23	1-8	1-8	
D	8	0	17	0	-2	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	188	141/0	0/0	0/0	0/0	47/0	0/0
C	31	24/-18	0/0	0/0	0/0	7/0	0/0
D	7	0/-8	0/0	0/0	0/0	12/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (LBS)	MAX. (PLF)	MAX. (LC)	MEMB. MAX. FORCE (LBS)	MAX. (LC)	MAX. (LC)	MAX. (LC)
FR-TO								
E-B	-244/0	0.0	0.0	0.04 (5)	7.81			
A-B	0/28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-17/0	-91.8	-91.8	0.08 (1)	8.25			
E-D	0/0	-18.5	-18.5	0.04 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/899$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/899$ (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5),
WB=0.00/1.00 (N/A:0), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1668

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.10 (E) (INPUT = 0.80)
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007165

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

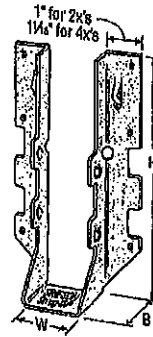
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

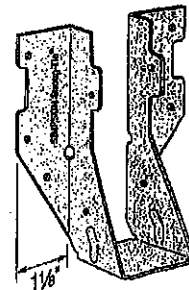
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

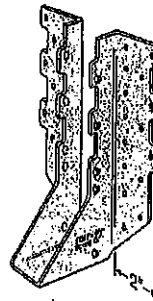
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options Information on p. 126.



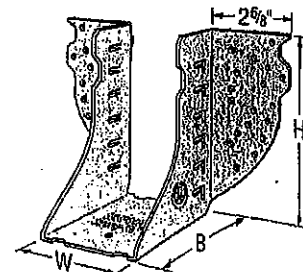
✓ LUS28



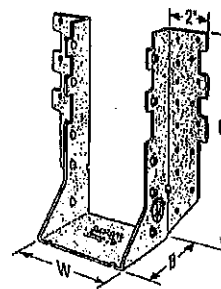
LU26L



✓ HUS210
(HUS26, HUS28,
and HHUS similar)



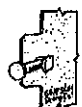
✓ HGUS28-2



✓ HHUS210-2



Double-Shear
Nailing
Top View

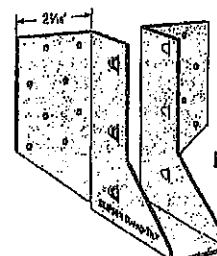
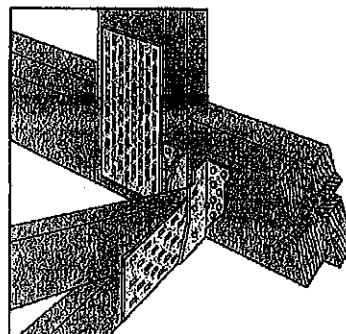


Double-Shear
Nailing
Side View;
Do not
bend tab



Dome Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,680

Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LJS26DS

Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS**HHUS/HGUS**

See Hanger Options information on pp. 125–127.

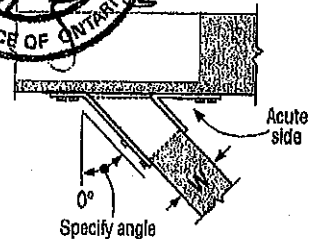
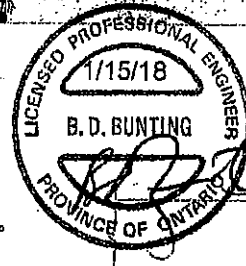
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.82 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.87 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



**Top View HHUS Hanger
Skewed Right**
(joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance				
		W	H	B	d _g ³	Header	Joist	D.Fir-L		S-P-F		
								Uplift	Normal	Uplift	Normal	
								(K _D = 1.15) lb.	(K _D = 1.00) lb.	(K _D = 1.15) lb.	(K _D = 1.00) lb.	
								kN	kN	kN	kN	
Single 2x Sizes												
■	LUS24	18	1 1/8	3 1/4	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1626	846	1156
									3.16	7.23	2.87	5.14
	LU24L	22	1 1/8	3	1 1/4	2 1/4	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725
									1.60	4.54	1.42	3.22
	LU26L	22	1 1/8	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"	720	1606	646	1140
									3.20	7.14	2.87	5.07
SS	LUS26	18	1 1/8	4 1/4	1 1/4	3 1/4	(4) 10d	(4) 10d	1420	2170	1290	1630
									6.32	8.85	5.74	7.25
■	HUS26	16	1 1/8	5 1/4	3	3 1/4	(14) 16d	(6) 16d	2705	4940	2065	3876
									11.30	21.97	9.20	17.24
	LJS26DS	18	1 1/8	5	3 1/2	4 1/4	(16) 16d	(6) 16d	2055	4265	1460	4115
									9.14	18.97	6.49	18.31
	HGUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 16d	(8) 16d	2885	6626	2685	5700
									11.98	29.51	11.98	25.35
	LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(6) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1550
									5.07	9.72	4.54	8.89
SS	LUS28	18	1 1/8	6 1/4	1 1/4	3 1/4	(6) 10d	(4) 10d	1420	2520	1290	1790
									6.32	11.21	5.74	7.96
■	HUS28	16	1 1/8	7 1/4	3	6 1/4	(22) 16d	(8) 16d	3805	5366	2675	4345
									16.04	23.88	11.90	19.33
	HGUS28	12	1 1/8	7 1/4	5	6 1/4	(36) 16d	(12) 16d	3310	7675	3310	6900
									14.74	34.19	14.74	30.73
	LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(6) 10d x 1 1/2"	1140	2495	1020	1770
									5.07	11.10	4.64	7.87
SS	LUS210	18	1 1/8	7 1/4	1 1/4	3 1/4	(8) 10d	(4) 10d	1420	2785	1290	2210
									6.32	12.39	5.74	9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_g is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-O-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _o ³	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _g = 1.15)	(K _g = 1.00)	(K _g = 1.15)	(K _g = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
LUS24-2	18	3 1/8	3 1/8	2	1 1/4	(4) 18d	(2) 18d	835	2020	590	1435
								3.71	8.89	2.62	6.38
SS LUS26-2	18	3 1/8	4 1/8	2	4	(4) 18d	(4) 18d	1720	2505	1545	1920
								7.65	11.54	6.87	8.54
HHUS26-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2850	7335	2065	5205
								12.68	32.83	9.20	23.15
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
SS LUS28-2	18	3 1/8	7	2	4	(6) 18d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS28-2	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3785	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS28-2	12	3 1/8	7 1/8	4	8 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
SS LUS210-2	18	3 1/8	9	2	6	(8) 18d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	4670	9680	4235	7000
								20.77	42.97	18.84	31.14
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.80	45.69
Triple 2x Sizes											
HGUS26-3	12	4 1/8	5 1/8	4	4 1/8	(20) 18d	(8) 18d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 18d	(12) 18d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 18d	(10) 18d	4670	9670	4235	6865
								20.77	43.02	18.84	30.54
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 18d	(16) 18d	6840	14645	4855	10400
								30.43	65.14	21.80	46.28
Quadruple 2x Sizes											
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 18d	(8) 18d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 18d	(12) 18d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-4	14	8 1/8	8 1/8	3	7 1/8	(30) 18d	(10) 18d	4670	10155	4235	7210
								20.77	45.17	18.84	32.07
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 18d	(16) 18d	6840	14645	4855	10400
								30.43	65.14	21.80	46.28
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 18d	(20) 18d	7640	14995	5425	10845
								33.98	66.70	24.13	47.35
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 18d	(22) 18d	10130	16400	7195	11645
								45.08	72.95	32.00	51.80
4x Sizes											
LUS46	18	3 1/8	4 1/8	2	3 1/8	(4) 18d	(4) 18d	1720	2505	1545	1920
								7.65	11.54	6.87	8.54
HHUS46	14	3 1/8	5 1/8	3	3 1/8	(14) 18d	(6) 18d	2540	7335	2065	5205
								11.30	32.83	9.20	23.15
HGUS46	12	3 1/8	5 1/8	4	4 1/8	(20) 18d	(8) 18d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS48	18	3 1/8	6 1/8	2	3 1/8	(6) 18d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS48	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3785	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS48	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS410	18	3 1/8	8 1/8	2	5 1/8	(8) 18d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HGUS410	12	3 1/8	9	4	8 1/8	(46) 18d	(16) 18d	6840	14015	4855	10270
								30.43	62.34	21.80	45.69
HGUS412	12	3 1/8	10 1/8	4	10 1/8	(56) 18d	(20) 18d	7640	14995	5425	10845
								33.98	66.70	24.13	47.35
HGUS414	12	3 1/8	12 1/8	4	11 1/8	(66) 18d	(22) 18d	10130	16400	7195	11645
								45.08	72.95	32.00	51.80



Plated Truss Connectors

See footnotes
on p. 258.

TC – Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1/4". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

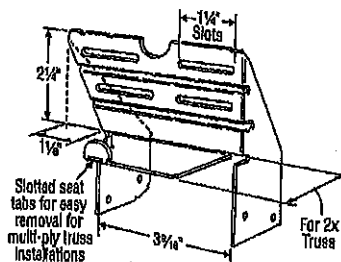
Design: Factored resistances are in accordance with CSA 086-14

Installation:

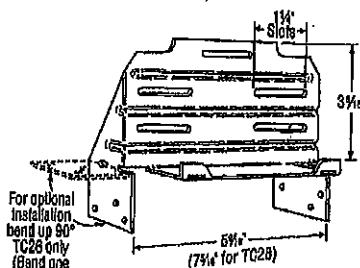
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

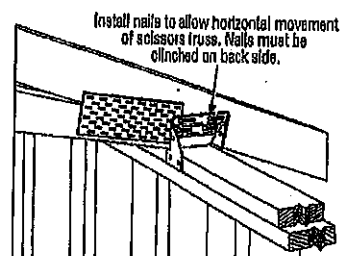
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



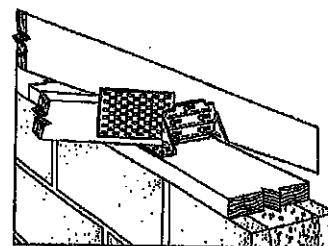
TC24
U.S. Patent 4,932,173



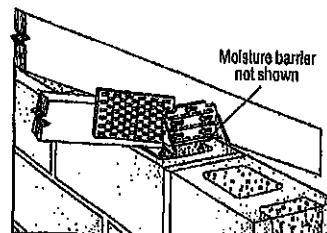
TC28
(TC28 Similar)



Typical TC24 Installation



Optional TC28 Installation for Grouted Concrete Block using a Wood Nailer (8", 10", 12" Wall Installation Similar)



Optional TC28 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _p =1.15)	Uplift (K _p =1.15)
TC24	(4) 10d	(4) 10d	605	430
TC28	(5) 10d	(5) 10d	1015	720
TC28	(5) 10d	(5) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _p =1.15)	Uplift (K _p =1.15)
TC28	(5) 10d	(5) 10d x 1 1/2"	810	660
	(5) 10d	(5) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC28 installation with 10d nails requires minimum 3" top plate thickness.
4. TC28 fastened to grouted concrete block with (5) - 5/16" x 2 1/4" Titen screws has a factored uplift resistance of 275 lb.



(800) 999-5099
strongtie.com

H/TSP**SIMPSON**
Strong-Tie**Seismic and Hurricane Ties (cont.)**

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Fasteners			Factored Resistance ($K_F = 1.16$)					
					D.Fir-L			S-P-F		
		To Rafters/ Truss	To Plates	To Studs	Uplift	Lateral		Uplift	Lateral	
						F ₁	F ₂		F ₁	F ₂
					lb.	lb.	lb.	lb.	lb.	lb.
					kN	kN	kN	kN	kN	kN
H1	18	(8) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
					3.29	3.05	1.33	3.02	2.16	0.98
SS H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	65
					3.68	0.98	0.33	2.62	0.69	0.24
SS H2.5A	18	(5) 8d	(5) 8d	—	805	180	160	755	160	160
					3.58	0.71	0.71	3.38	0.71	0.71
H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
					3.71	0.78	0.93	3.29	0.71	0.93
SS H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
					3.29	0.80	1.18	2.74	0.56	0.85
H6	16	—	(8) 8d	(8) 8d	1585	1085	—	1125	770	—
					7.05	4.83	—	5.00	3.43	—
H7Z	16	(4) 8d	(2) 8d	(8) 8d	1390	670	—	980	475	—
					6.18	2.99	—	4.40	2.11	—
SS H8 ³	18	(5) 10d x 1½"	(5) 10d x 1½"	—	1120	—	—	1025	—	—
					4.98	—	—	4.58	—	—
SS H10A ³	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	585	290
					7.72	3.54	1.82	6.89	2.51	1.29
H10AR	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1485	690	430	1220	670	305
					6.61	3.07	1.91	5.43	2.54	1.36
H10A-2	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1835	1275	430	1645	880	305
					8.16	5.67	1.91	7.32	3.91	1.36
H10S ^{2,3}	18	(8) 8d x 1½"	(8) 8d x 1½"	(8) 8d	1465	795	315	1040	565	225
					6.52	3.54	1.40	4.63	2.51	1.00
H11Z	18	(6) 16d x 2½"	(6) 16d x 2½"	—	1085	920	545	780	655	390
					4.87	4.09	2.42	3.47	2.91	1.73
H14	18	[1] (12) 8d x 1½"	(13) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
		[2] (12) 8d x 1½"	(15) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
TSP	16	(9) 10d x 1½"	(6) 10d x 1½"	—	1295	440	—	920	310	—
					5.76	1.96	—	4.09	1.38	—
		(9) 10d x 1½"	(6) 10d	—	1560	440	—	1105	310	—
					6.94	1.96	—	4.92	1.38	—

- Factored resistances have been increased 16% for short term loading; no further increase is allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
- H8 factored uplift resistances for stud-to-bottom plate installations are 595 lb. (2.65 kN) for D.Fir-L and 390 lb. (1.74 kN) for S-P-F.
- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-HTIECONPATH).
- Factored resistances in the F₁ direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.

- H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.38 kN) D.Fir-L and 1015 lb. (4.51 kN) S-P-F.
- H10S nails to plates are optional for uplift but required for lateral loads.
- H10A may be field-bent up to a slope of 8/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
- The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SNAILS at strongtie.com.
- D.Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1½" nails into the top plates and (3) 8d x 1½" nails into the lowest three flange holes into the truss bottom chord is 485 lb. (2.20 kN).
- Nails: 16d x 2½" = 0.182" dia. x 2½" long., 10d = 0.148" dia. x 3" long., 10d x 1½" = 0.148" dia. x 1½" long., 8d = 0.131" dia. x 2½" long., 8d x 1½" = 0.131" dia. x 1½" long. See pp. 27–28 for other nail sizes and information.

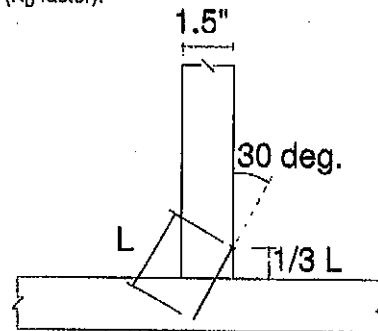
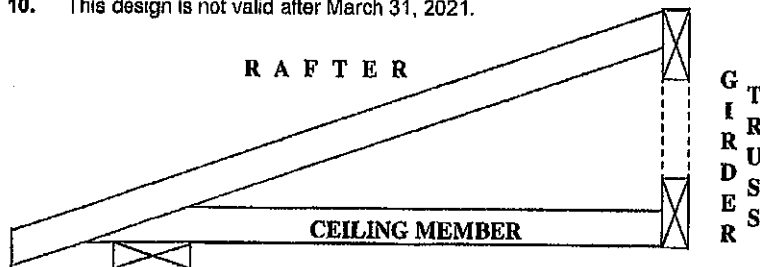
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

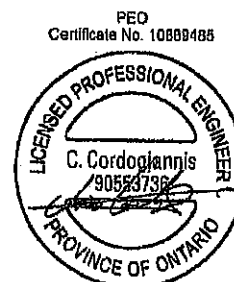
- Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
- Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
- For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
- Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
- Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
- Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
- For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
- Lumber must be dry (< 19% moisture content) at the time of nail installation.
- Nail values in this table comply with CSA O86-14, section 12.9.4
- This design is not valid after March 31, 2021.



Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (In)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

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December 2, 2019



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

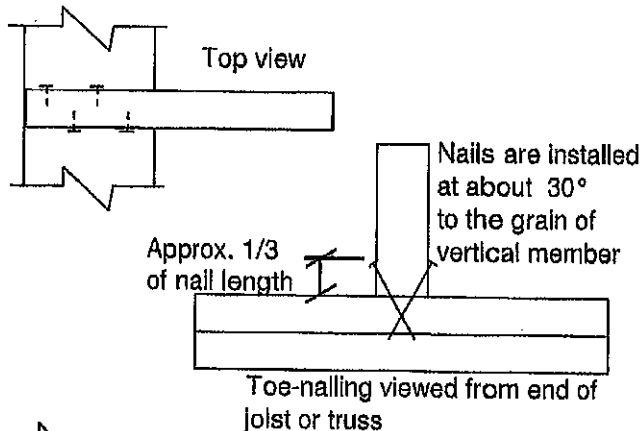
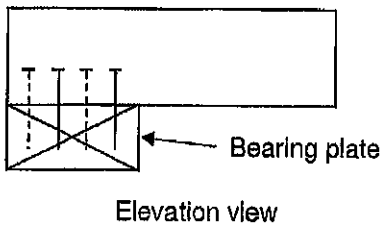
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

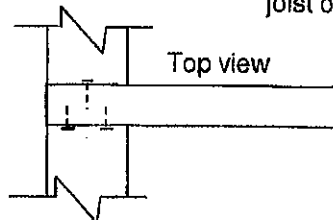
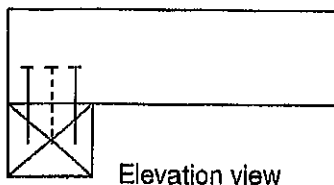
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



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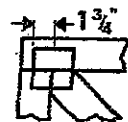
December 2, 2019

PEO
Certificate No. 10889485.

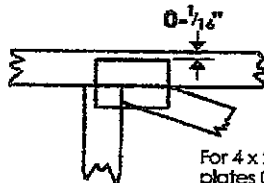


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

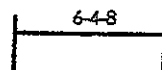


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

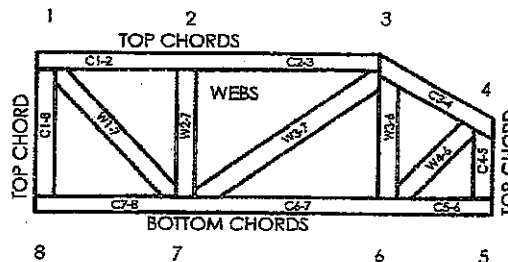
Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MIT-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

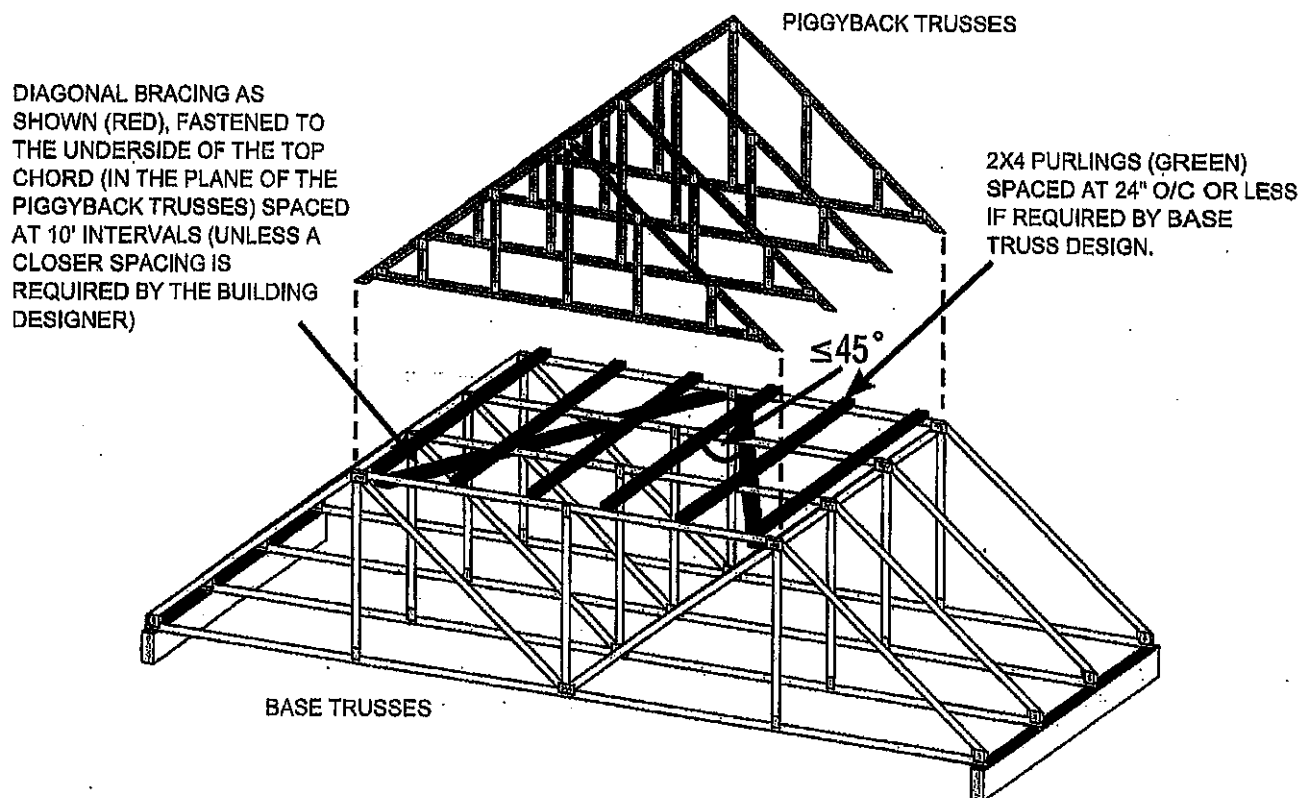
- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
- Design assumes trusses will be suitably protected from the environment in accord with TPIC.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with TPIC Quality Criteria.

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

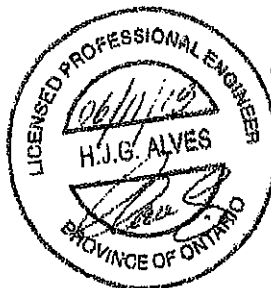


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufacturer's responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate Institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet M117473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218

Feb 09, 2018