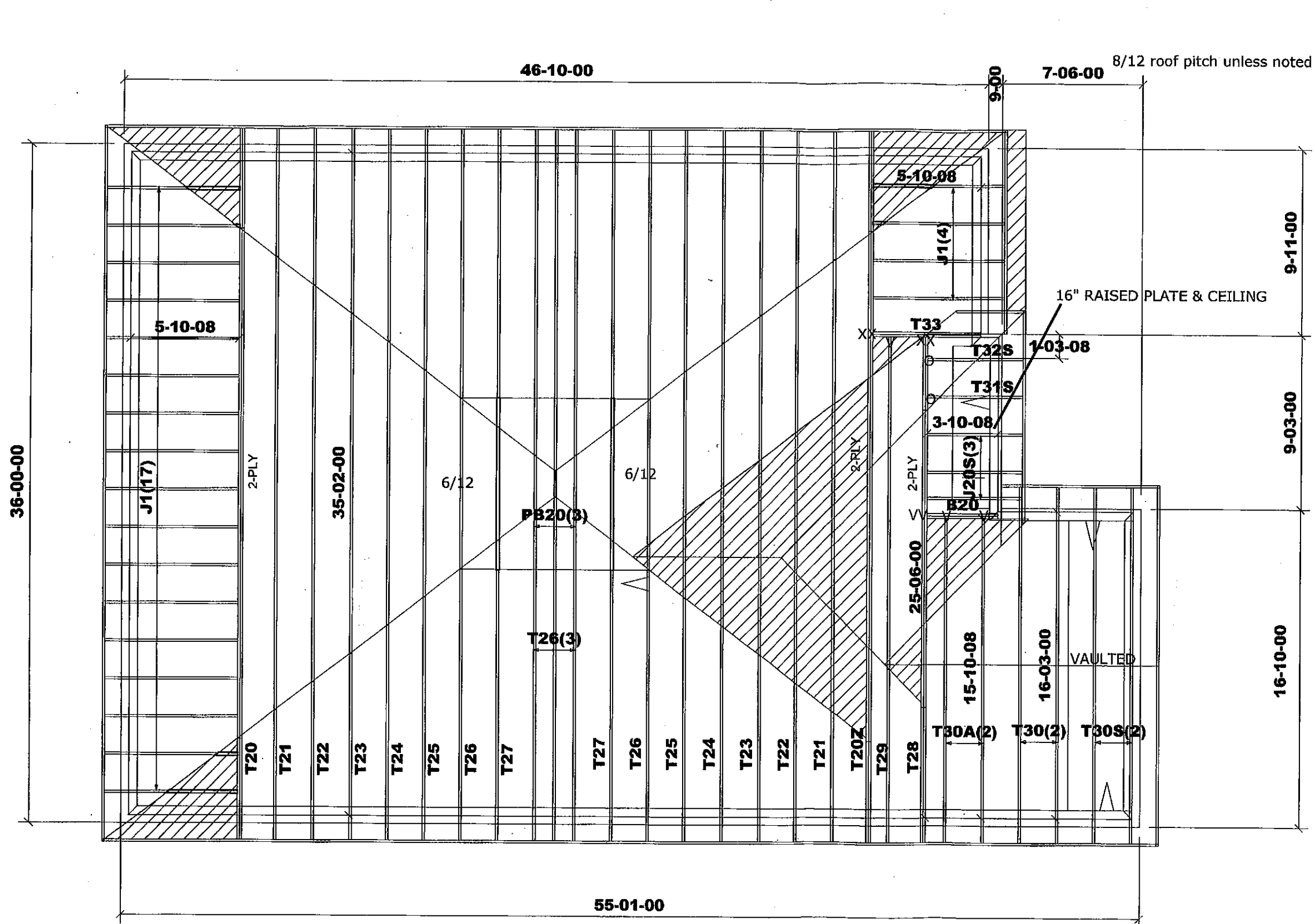




ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

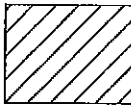
All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

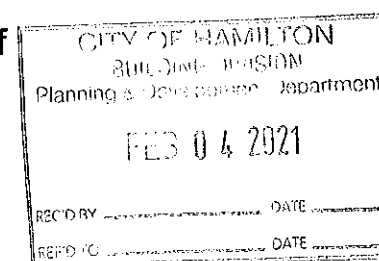
DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

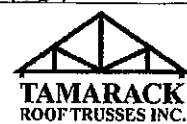
HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
LUS26-2-(VV)

BEAMS:
B20= B21 =
2 - 2x10 SPF #2

 DENOTES:
CONVENTIONAL
FRAMING



M13126



Job Track: 51225
Plan Log: 202406
Layout ID: 408169

Builder / Location:
GREEN PARK HOMES / WATERDOWN

Project: RUSSELL GARDENS PH.3

Date: 2020-04-27 Sales: Mario DiCano Designer: JG

Model / Elevation:
MOUNTAINASH 5/2- STD OR OPT.5 BR

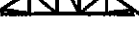
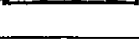
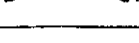
Model ver 8.3.1.215

Lot 294

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

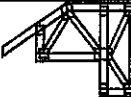
 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	DELIVERY SHIPLIST							
	Lumber Yard: TAMARACK LUMBER						Job Track: 51225	
	Builder: GREEN PARK HOMES						Plan Log: 202406	
	Project: RUSSELL GARDENS PH.3						Layout ID: 408169	
	Location: WATERDOWN						Ref #	
	Model: MOUNTAINASH 5						Page: 1 of 2	
Lot #: 294						Date: 04-27-2020		
Elevation: 2- STD OR OPT.5 BR						Designer:		
						Sales Rep: Mario DiCano		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T20 Hip Girder	8 /12	35-02-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	320.72 200.67		
	1 2-ply	T20Z Hip Girder	8 /12	35-02-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	320.72 200.67		
	2	T21 Hip	8 /12	35-02-00	5-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	279.76 177.00		
	2	T22 Hip	8 /12	35-02-00	6-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	302.77 191.33		
	2	T23 Hip	8 /12	35-02-00	7-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	309.14 195.67		
	2	T24 Hip	8 /12	35-02-00	8-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	316.79 196.67		
	2	T25 Hip	8 /12	35-02-00	9-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	329.26 205.00		
	5	T26 Hip	8 /12	35-02-00	10-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	857.95 538.33		
	2	T27 Hip	8 /12	35-02-00	11-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	332.54 206.67		
	1 2-ply	T28 Hip Girder	8 /12	25-06-00	5-03-13	2 x 4 2 x 6	1-03-08	1-04-13 2-08-13	244.99 150.00		
	1	T29 Hip	8 /12	25-06-00	6-07-13	2 x 4	1-03-08	1-04-13 2-08-13	108.69 70.33		
	2	T30 Common	8 /12	16-03-00	6-09-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	144.92 91.33		
	2	T30A Common	8 /12	15-10-08	6-09-13	2 x 4	1-03-08	1-04-13 1-04-13	139.63 87.67		
	2	T30S Scissor	8 /12 4 /12	16-03-00	6-09-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	133.1 87.33		

 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	DELIVERY SHIPLIST			
	Lumber Yard:	TAMARACK LUMBER	Job Track:	51225
	Builder:	GREEN PARK HOMES	PlanLog:	202406
	Project:	RUSSELL GARDENS PH.3	Layout ID:	408169
	Location:	WATERDOWN	Ref #	
	Model:	MOUNTAINASH 5	Page:	2 of 2
Lot #:	294	Date:	04-27-2020	
Elevation:	2- STD OR OPT.5 BR	Designer:		
		Sales Rep:	Mario DiCano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T31S Half Hip	8 /12	3-10-08	4-09-13	2 x 4	1-03-08	1-04-13 4-09-13	29.4 20.17		
	1	T32S Half Hip	8 /12	3-10-08	3-05-13	2 x 4	1-03-08	1-04-13 3-05-13	24.65 17.17		
	1 2-ply	T33 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	3	PB20 Piggyback	8 /12	9-00-12	2-00-00	2 x 4			74.54 50.50		
	21	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	352.68 224.00		
	3	J20S Jack-Open	8 /12	3-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	85.88 57.50		

TOTAL # TRUSS= 61

TOTAL BFT OF ALL TRUSSES= 3005.01 BFT.

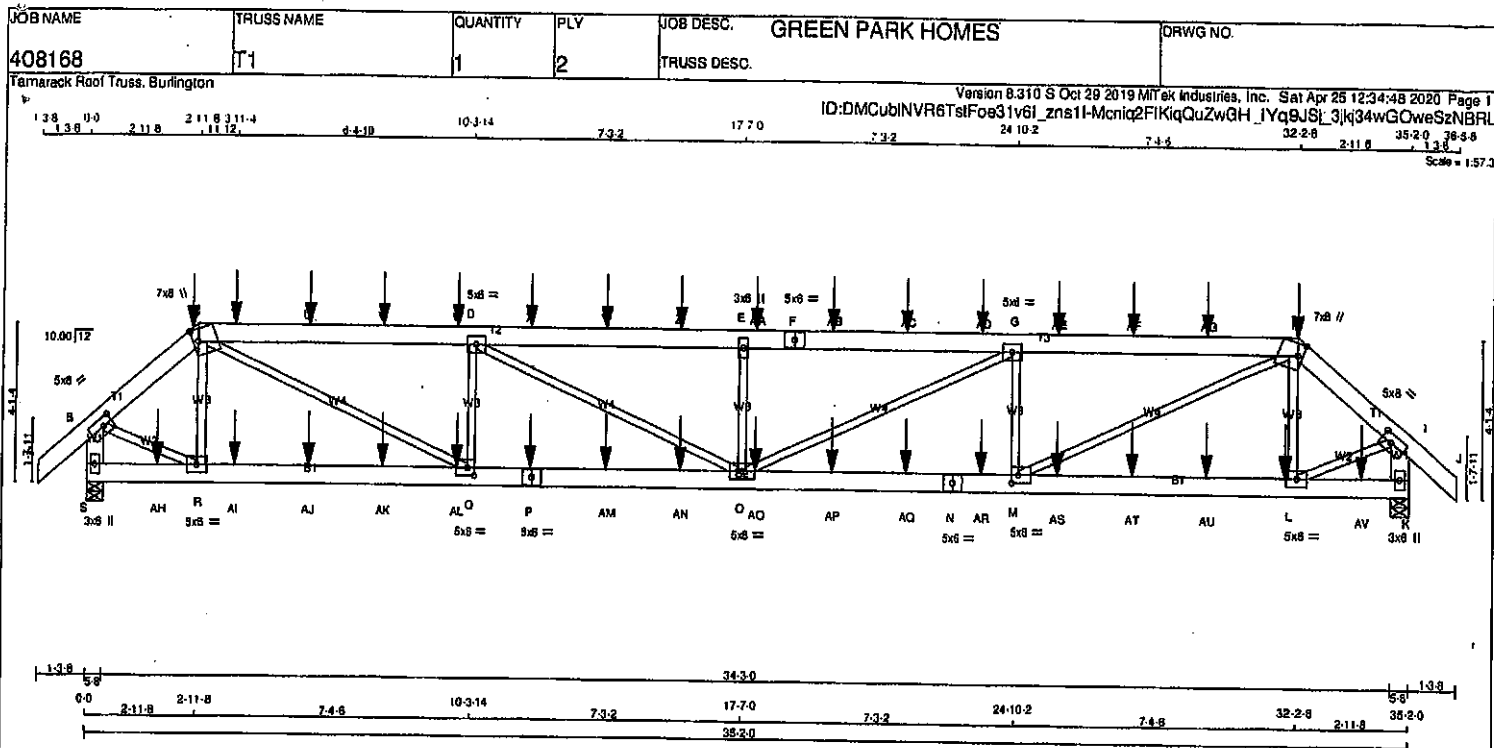
TOTAL WEIGHT OF ALL TRSSES 4765.74 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
3	Hardware	LJS26DS	
2	Hardware	LUS24	

TOTAL NUMBER OF
ITEMS=

7



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	2100F 1.8E	SPF
P - N	2x6	DRY	2100F 1.8E	SPF
N - K	2x6	DRY	2100F 1.8E	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 2	12	SIDE(122.0)
C - F 2	12	SIDE(81.0)
F - H 2	12	SIDE(81.0)
H - J 2	12	SIDE(122.0)
S - B 2	12	TOP
K - I 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S - P 2	12	SIDE(0.0)
P - N 2	12	SIDE(0.0)
N - K 2	12	SIDE(197.8)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE PLACED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT VERT	3330	3330	5-8	5-8
S	3330	3330	5-8	5-8
K	3379	3379	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2354	1553 / 0	0 / 0	0 / 0	0 / 0	801 / 0	0 / 0
K	2388	1572 / 0	0 / 0	0 / 0	0 / 0	818 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO						FR-TO		
A-B	0 / 42	-91.8	-91.8	0.04 (1)	10.00	R-C	-850 / 0	0.08 (1)
B-C	-3387 / 0	-91.8	-91.8	0.05 (1)	6.02	C-Q	0 / 4509	0.58 (1)
C-T	-8802 / 0	-91.8	-91.8	0.38 (1)	4.31	Q-D	-1798 / 0	0.22 (1)
T-U	-8802 / 0	-91.8	-91.8	0.38 (1)	4.31	D-O	0 / 1370	0.17 (1)
U-V	-8802 / 0	-91.8	-91.8	0.38 (1)	4.31	O-E	-987 / 0	0.12 (1)
V-W	-8802 / 0	-91.8	-91.8	0.38 (1)	4.31	E-G	0 / 1382	0.17 (1)
W-O	-8802 / 0	-91.8	-91.8	0.38 (1)	4.31	M-G	-1787 / 0	0.21 (1)
D-X	-7828 / 0	-91.8	-91.8	0.42 (1)	3.98	M-H	0 / 4481	0.55 (1)
X-Y	-7828 / 0	-91.8	-91.8	0.42 (1)	3.98	L-H	-861 / 0	0.08 (1)
Y-Z	-7828 / 0	-91.8	-91.8	0.42 (1)	3.98	B-R	0 / 2738	0.34 (1)
Z-E	-7828 / 0	-91.8	-91.8	0.42 (1)	3.98	L-I	0 / 2771	0.34 (1)
E-AA	-7828 / 0	-91.8	-91.8	0.42 (1)	3.98			
AA-F	-7828 / 0	-91.8	-91.8	0.42 (1)	3.98			
F-AB	-7828 / 0	-91.8	-91.8	0.42 (1)	3.98			
AB-AC	-7828 / 0	-91.8	-91.8	0.42 (1)	3.98			
AC-AD	-7828 / 0	-91.8	-91.8	0.42 (1)	3.98			
AD-G	-7828 / 0	-91.8	-91.8	0.42 (1)	3.98			
G-AE	-8808 / 0	-91.8	-91.8	0.38 (1)	4.31			
AE-AF	-8808 / 0	-91.8	-91.8	0.38 (1)	4.31			
AF-AG	-8808 / 0	-91.8	-91.8	0.38 (1)	4.31			
AG-H	-8808 / 0	-91.8	-91.8	0.38 (1)	4.31			
H-I	-3410 / 0	-91.8	-91.8	0.05 (1)	6.00			
I-J	0 / 42	-91.8	-91.8	0.04 (1)	10.00			
S-B	-3375 / 0	0.0	0.0	0.12 (1)	7.58			
K-I	-3414 / 0	0.0	0.0	0.12 (1)	7.55			

S-AH	0 / 0	-18.5	-18.5	0.05 (4)	10.00
AH-R	0 / 0	-18.5	-18.5	0.05 (4)	10.00
R-AI	0 / 2553	-18.5	-18.5	0.12 (1)	10.00
AI-AJ	0 / 2553	-18.5	-18.5	0.12 (1)	10.00
AJ-AK	0 / 2553	-18.5	-18.5	0.12 (1)	10.00
AK-AL	0 / 2553	-18.5	-18.5	0.12 (1)	10.00
AL-Q	0 / 2553	-18.5	-18.5	0.12 (1)	10.00
Q-P	0 / 8802	-18.5	-18.5	0.22 (1)	10.00
P-AM	0 / 8802	-18.5	-18.5	0.22 (1)	10.00
AM-AN	0 / 8802	-18.5	-18.5	0.22 (1)	10.00
AN-O	0 / 8802	-18.5	-18.5	0.22 (1)	10.00
O-AO	0 / 8809	-18.5	-18.5	0.22 (1)	10.00
AO-AP	0 / 8809	-18.5	-18.5	0.22 (1)	10.00
AP-AQ	0 / 8809	-18.5	-18.5	0.22 (1)	10.00
AQ-N	0 / 8809	-18.5	-18.5	0.22 (1)	10.00
N-AR	0 / 8809	-18.5	-18.5	0.22 (1)	10.00
AR-M	0 / 8809	-18.5	-18.5	0.22 (1)	10.00

TOTAL WEIGHT = 2 X 183 = 365 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPC 2011, TPC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/899 (0.20")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/899 (0.37")

CSI: TC=0.42/1.00 (D-E:1), BC=0.22/1.00 (O-Q:1),
WB=0.58/1.00 (C-Q:1), SS=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

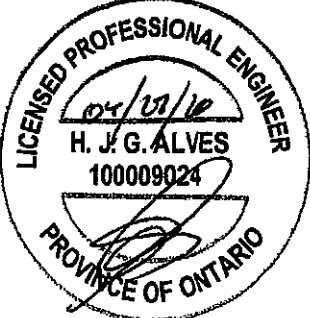
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.85 (N) (INPUT = 1.00)



Structural component only
DWG# T-2007129 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408168	T1	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 12:34:48 2020 Page 2
ID:DMCubINVR6TsFoe31v6I zns11-Monic2FIKiaQuZwGH 1Yq9J5I 3Ik34wGOWsSzNBRL

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	5.0	8.0	2.50 3.25
C	TTWW+m	MT20	7.0	8.0	3.75 1.50
D	TMVW-t	MT20	5.0	6.0	
E	TMVW-w	MT20	3.0	6.0	
F	TS-t	MT20	5.0	6.0	
G	TMVW-t	MT20	5.0	6.0	
H	TTWW+m	MT20	7.0	8.0	3.75 1.50
I	TMVW-t	MT20	5.0	8.0	2.50 3.25
K	BMV1+p	MT20	3.0	8.0	
L	BMVW-t	MT20	5.0	8.0	
M	BMVW-t	MT20	5.0	6.0	2.50 2.25
N	BS-t	MT20	5.0	6.0	
O	BMVW-t	MT20	5.0	8.0	
P	BS-t	MT20	5.0	6.0	
Q	BMVW-t	MT20	5.0	6.0	2.50 2.25
R	BMVW-t	MT20	5.0	8.0	
S	BMV1+p	MT20	3.0	8.0	

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	MAX. FACTORED FORCE (LBS)	LC1 MAX	UNBRAG
FR-TO		FROM	TO	FR-TO		FROM	TO
MAS	0 / 2588	-18.5	-18.5	0.12 (1)	10.00		
AS-AT	0 / 2588	-18.5	-18.5	0.12 (1)	10.00		
AT-AU	0 / 2588	-18.5	-18.5	0.12 (1)	10.00		
AU-L	0 / 2588	-18.5	-18.5	0.12 (1)	10.00		
L-AV	0 / 0	-18.5	-18.5	0.05 (1)	10.00		
AV-K	0 / 0	-18.5	-18.5	0.05 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-38	-42	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-191	-191	---	FRONT	VERT	SNOW	---	C1
H	32-2-8	-38	-42	---	FRONT	VERT	DEAD	---	C1
H	32-2-8	-116	-118	---	FRONT	VERT	TOTAL	---	C1
H	32-2-8	-191	-191	---	FRONT	VERT	SNOW	---	C1
L	31-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
P	11-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
T	3-11-4	-111	-111	---	FRONT	VERT	TOTAL	---	C1
U	5-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	11-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	15-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	17-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	19-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	21-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	23-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AE	25-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AF	27-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AG	29-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AH	1-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AI	3-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AJ	5-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AK	7-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AL	9-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AM	13-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AN	15-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AO	17-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AP	19-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AQ	21-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AR	23-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AS	25-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AT	27-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AU	29-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AV	33-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

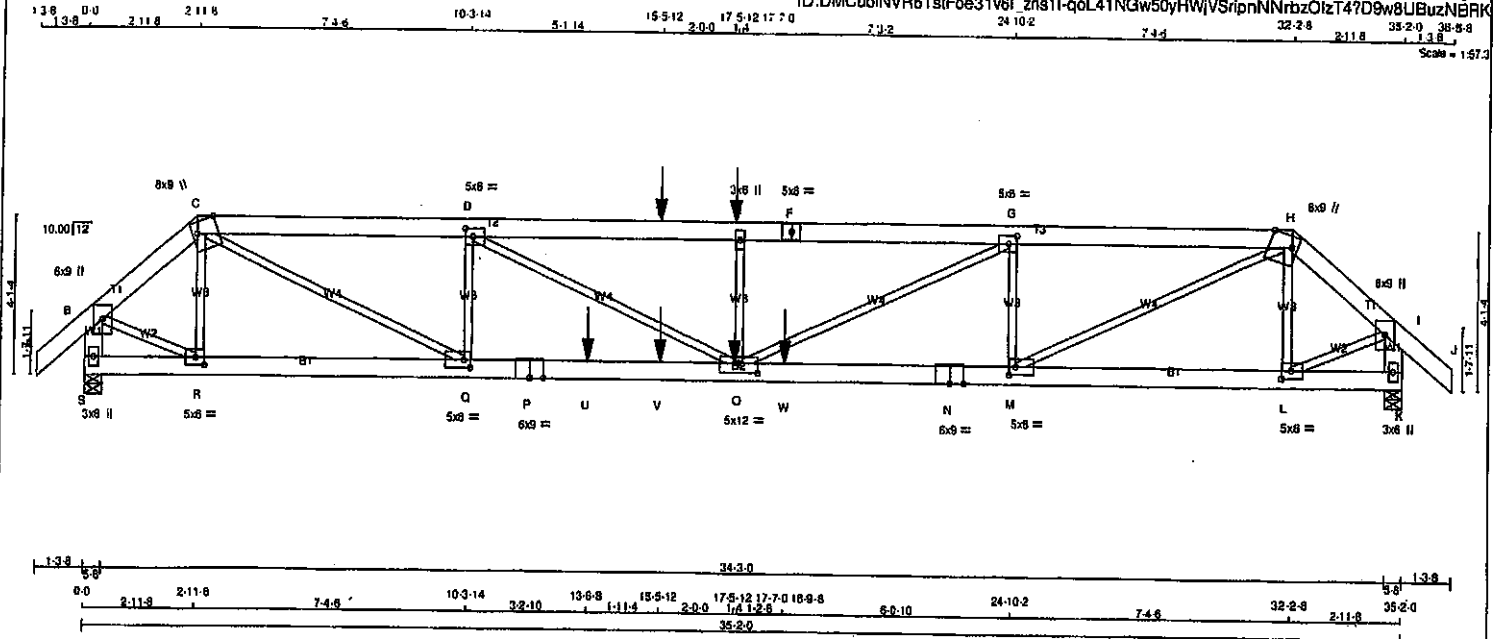
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408168	T1Z	1	2	GREEN PARK HOMES	

Trussack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6l_zns1l-qL41NGw50yHWjVSripnNNrbzOizT47D9w8UBuzNBK



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2
C - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - J	2x6	DRY	No.2
J - K	2x6	DRY	No.2
K - I	2x6	DRY	No.2
I - P	2x6	DRY	2100F 1.8E
P - N	2x6	DRY	2100F 1.8E
N - K	2x6	DRY	2100F 1.8E

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C 2	12	TOP
C-F 2	12	SIDE(183.1)
F-H 2	12	TOP
H-J 2	12	TOP
J-K 2	12	TOP
K-I 2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
S-P 2	12	TOP
P-N 2	12	SIDE(197.8)
N-K 2	12	TOP
WEBS: (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	GROSS REACTION	DOWN	HORIZ	UPLIFT	IN-SX	IN-SX
S	3963	0	3963	0	0	5-8	5-8
K	3887	0	3887	0	0	5-8	5-8

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2793	1890 / 0	0 / 0	0 / 0	0 / 0	902 / 0	0 / 0
K	2599	1758 / 0	0 / 0	0 / 0	0 / 0	843 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED			WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 42	-91.8	-91.8	0.04 (1)	10.00	R-C	-1088 / 0	0.13 (1)
B-C	-3988 / 0	-91.8	-91.8	0.06 (1)	5.95	C-Q	0 / 7283	0.80 (1)
C-D	-9534 / 0	-91.8	-91.8	0.37 (1)	3.74	Q-D	-2160 / 0	0.28 (1)
D-T	-12331 / 0	-91.8	-91.8	0.52 (1)	3.20	D-O	0 / 3130	0.39 (1)
T-E	-12331 / 0	-91.8	-91.8	0.52 (1)	3.20	O-E	-778 / 0	0.09 (1)
E-F	-12331 / 0	-91.8	-91.8	0.50 (1)	3.23	O-G	0 / 4203	0.52 (1)
F-G	-12331 / 0	-91.8	-91.8	0.50 (1)	3.23	M-G	-2813 / 0	0.31 (1)
G-H	-8575 / 0	-91.8	-91.8	0.33 (1)	3.94	M-H	0 / 6383	0.79 (1)
H-I	-3758 / 0	-91.8	-91.8	0.08 (1)	5.78	L-H	-843 / 0	0.10 (1)
I-J	0 / 42	-91.8	-91.8	0.04 (1)	7.14	B-R	0 / 3239	0.40 (1)
S-B	-3945 / 0	0.0	0.0	0.14 (1)	7.14	L-I	0 / 3052	0.38 (1)
K-I	-3733 / 0	0.0	0.0	0.13 (1)	7.28			

S-R	0 / 0	-18.5	-18.5	0.02 (4)	10.00
R-Q	0 / 3013	-18.5	-18.5	0.21 (1)	10.00
Q-P	0 / 9534	-18.5	-18.5	0.67 (1)	10.00
P-U	0 / 9534	-18.5	-18.5	0.67 (1)	10.00
U-V	0 / 9534	-18.5	-18.5	0.67 (1)	10.00
V-O	0 / 9534	-18.5	-18.5	0.67 (1)	10.00
C-W	0 / 8575	-18.5	-18.5	0.49 (1)	10.00
W-N	0 / 8575	-18.5	-18.5	0.49 (1)	10.00
N-M	0 / 8575	-18.5	-18.5	0.49 (1)	10.00
M-L	0 / 2844	-18.5	-18.5	0.12 (1)	10.00
L-K	0 / 0	-18.5	-18.5	0.05 (1)	10.00

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC	LOC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	17-5-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
O	17-5-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
T	15-5-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
U	13-8-8	-1615	-1615	---	BACK	VERT	TOTAL	---	C1
V	15-5-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
W	18-9-8	-1620	-1620	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 183 = 365 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.35")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/657 (0.64")

CSI: TC=0.52/1.00 (D-E:1), BC=0.67/1.00 (O-Q:1), WB=0.90/1.00 (C-Q:1), SS=0.53/1.00 (M-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

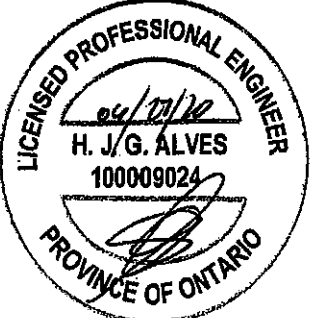
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (C) (INPUT = 0.90)
JSI METAL = 0.95 (P) (INPUT = 1.00)



Structural component only
DWG# T-2007130 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408168	T1Z	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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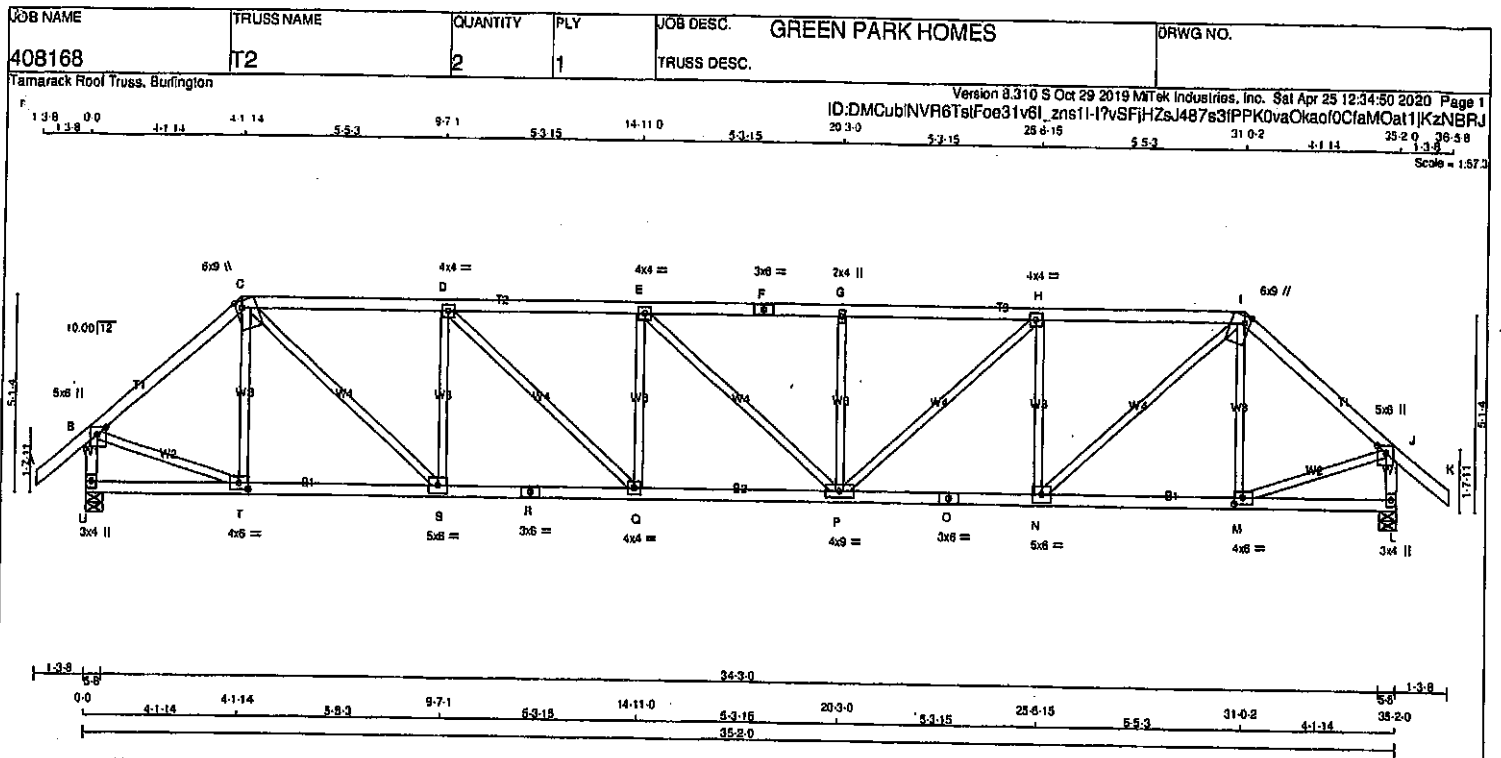
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	9.0		
C	TTWW+m	MT20	8.0	9.0	Edge	6.50
D	TMWW-l	MT20	5.0	6.0	2.50	2.50
E	TMW+w	MT20	3.0	6.0		
F	TS-l	MT20	5.0	6.0		
G	TMWW-l	MT20	5.0	6.0	2.50	2.50
H	TTWW+m	MT20	8.0	9.0	Edge	6.50
I	TMW+p	MT20	6.0	9.0		
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-l	MT20	6.0	8.0	2.50	2.75
M	BMWW-l	MT20	5.0	8.0	2.50	2.00
N	BS-l	MT20	6.0	9.0		
O	BMWWW-l	MT20	5.0	12.0	3.00	6.00
P	BS-l	MT20	6.0	9.0		
Q	BMWW-l	MT20	6.0	8.0	2.50	2.00
R	BMWW-l	MT20	5.0	6.0	2.50	2.75
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



Structural component only
DWG# T-2007130 47



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
I - K	2x4	DRY No.2	SPF
U - B	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
U - R	2x4	DRY No.2	SPF
R - O	2x4	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	5.0	6.0	Edge	2.75
C TTWW+m	MT20	6.0	9.0	Edge	1.75
D, E, H					
D TMVW-i	MT20	4.0	4.0		
F TS-i	MT20	3.0	6.0		
G TMVW+w	MT20	2.0	4.0		
I TTWW+m	MT20	6.0	9.0	Edge	1.75
J TMVW+p	MT20	5.0	6.0	Edge	2.75
L BMV1+p	MT20	3.0	4.0		
M BMVW-i	MT20	4.0	6.0	2.00	2.75
N BMVW-i	MT20	5.0	6.0		
O BS-i	MT20	3.0	6.0		
P BMVWVW-i	MT20	4.0	9.0		
Q BMVW-i	MT20	4.0	4.0		
R BS-i	MT20	3.0	6.0		
S BMVW-i	MT20	5.0	6.0		
T BMVW-i	MT20	4.0	6.0	2.00	2.75
U BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT VERT	2066	2066	0	5-8
U	2066	2066	0	5-8
L	2066	2066	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN. COMPONENT REACTIONS
JT COMBINED	1458	971 / 0
U	1458	971 / 0
L	1458	971 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	MEMB.
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	T-C	-358 / 0
B-C	-1960 / 0	-91.8	-91.8	0.38 (1)	4.46	C-S	0 / 1836
C-D	-2871 / 0	-91.8	-91.8	0.57 (1)	3.59	S-D	-1118 / 0
D-E	-3506 / 0	-91.8	-91.8	0.65 (1)	3.21	D-Q	0 / 856
E-F	-3505 / 0	-91.8	-91.8	0.53 (1)	3.36	Q-E	-475 / 0
F-G	-3505 / 0	-91.8	-91.8	0.53 (1)	3.36	E-P	-2 / 0
G-H	-3505 / 0	-91.8	-91.8	0.66 (1)	3.20	P-G	-474 / 0
H-I	-2871 / 0	-91.8	-91.8	0.57 (1)	3.59	P-H	0 / 853
I-J	-1960 / 0	-91.8	-91.8	0.38 (1)	4.46	N-I	-1117 / 0
J-K	0 / 41	-91.8	-91.8	0.13 (1)	10.00	H-I	0 / 1837
U-B	-2037 / 0	0.0	0.0	0.22 (1)	5.92	M-I	-358 / 0
L-J	-2037 / 0	0.0	0.0	0.22 (1)	5.92	B-T	0 / 1571
U-T	0 / 0	-18.5	-18.5	0.10 (4)	10.00	M-J	0 / 1571
T-S	0 / 1494	-18.5	-18.5	0.30 (1)	10.00		
S-R	0 / 2871	-18.5	-18.5	0.51 (1)	10.00		
R-Q	0 / 2871	-18.5	-18.5	0.51 (1)	10.00		
Q-P	0 / 3508	-18.5	-18.5	0.62 (1)	10.00		
P-O	0 / 2872	-18.5	-18.5	0.51 (1)	10.00		
O-N	0 / 2872	-18.5	-18.5	0.51 (1)	10.00		
N-M	0 / 1494	-18.5	-18.5	0.30 (1)	10.00		
M-L	0 / 0	-18.5	-18.5	0.10 (4)	10.00		

TOTAL WEIGHT = 2 X 147 = 293 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2010, NBC2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC2010, NBC2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/988 (0.20")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.39")

CSI: TC=0.66/1.00 (G-H:1), BC=0.82/1.00 (P-Q:1),
WB=0.43/1.00 (D-S:1), SS=0.23/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (N) (INPUT = 0.90)
JSI METAL=0.90 (O) (INPUT = 1.00)

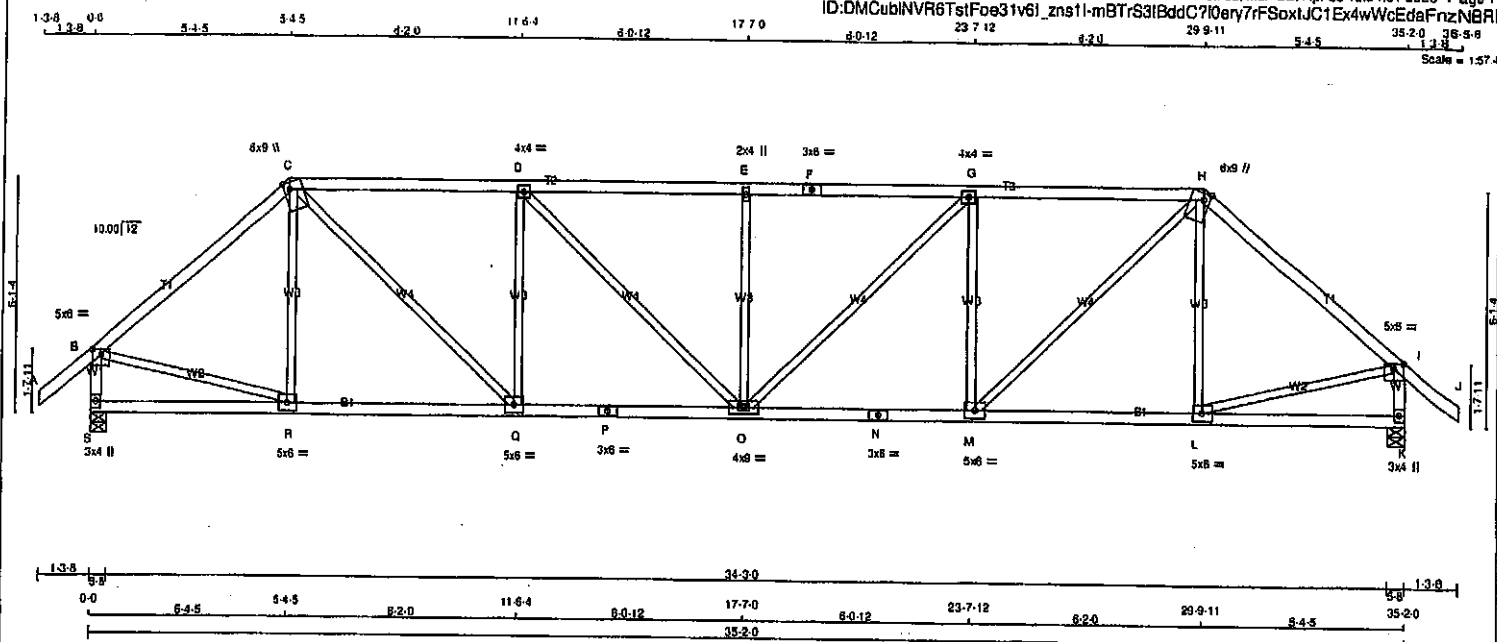


Structural component only
DWG# T-2007131

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408168	T3	2	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
S - I	2x4	DRY	No.2	SPF	
P - N	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMVW-t	MT20	4.0	4.0		
E	TMVW-w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	8.0	9.0	Edge	1.75
I	TMVW-p	MT20	5.0	6.0	1.50	3.00
K	BMV1+p	MT20	3.0	4.0		
L, M, Q, R						
L	BMVW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	9.0		
P	BS-t	MT20	3.0	6.0		
S	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
S	2086	0	2086	0	0	5-8	5-8
K	2086	0	2086	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1458	971 / 0	0 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
K	1458	971 / 0	0 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	MAX. FACTORED	VERT. LOAD	LC1	MAX	UNBRAC	MEMB.	MAX. FACTORED
	(LBS)	(PLP)	CS1 (LC)	LENGTH	FR-TO		(LBS)
FR-TO							
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	R-C	-246 / 7
B-C	-2004 / 0	-91.8	-91.8	0.84 (1)	4.11	C-Q	0 / 1532
C-D	-2653 / 0	-91.8	-91.8	0.74 (1)	3.50	Q-D	-943 / 0
D-E	-2978 / 0	-91.8	-91.8	0.78 (1)	3.28	D-O	0 / 450
E-F	-2978 / 0	-91.8	-91.8	0.78 (1)	3.28	O-E	-513 / 0
F-G	-2978 / 0	-91.8	-91.8	0.78 (1)	3.28	G-G	0 / 450
G-H	-2653 / 0	-91.8	-91.8	0.74 (1)	4.11	M-G	-943 / 0
H-I	-2004 / 0	-91.8	-91.8	0.64 (1)	4.11	M-H	0 / 1532
I-J	0 / 41	-91.8	-91.8	0.13 (1)	10.00	L-H	-246 / 7
S-B	-2026 / 0	0.0	0.0	0.22 (1)	5.94	B-R	0 / 1580
K-I	-2026 / 0	0.0	0.0	0.22 (1)	5.94	L-I	0 / 1580
S-R	0 / 0	-18.5	-18.5	0.14 (4)	10.00		
R-Q	0 / 1532	-18.5	-18.5	0.33 (1)	10.00		
Q-P	0 / 2653	-18.5	-18.5	0.49 (1)	10.00		
P-O	0 / 2653	-18.5	-18.5	0.49 (1)	10.00		
O-N	0 / 2653	-18.5	-18.5	0.49 (1)	10.00		
N-M	0 / 2653	-18.5	-18.5	0.49 (1)	10.00		
M-L	0 / 1532	-18.5	-18.5	0.33 (1)	10.00		
L-K	0 / 0	-18.5	-18.5	0.14 (4)	10.00		

TOTAL WEIGHT = 2 X 148 = 296 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/999 (0.16")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = L/999 (0.29")

CSI: TC=0.78/1.00 (E-G:1), BC=0.48/1.00 (M-O:1), WB=0.55/1.00 (G-M:1), SS=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (B) (INPUT = 0.90)
JSI METAL = 0.85 (N) (INPUT = 1.00)



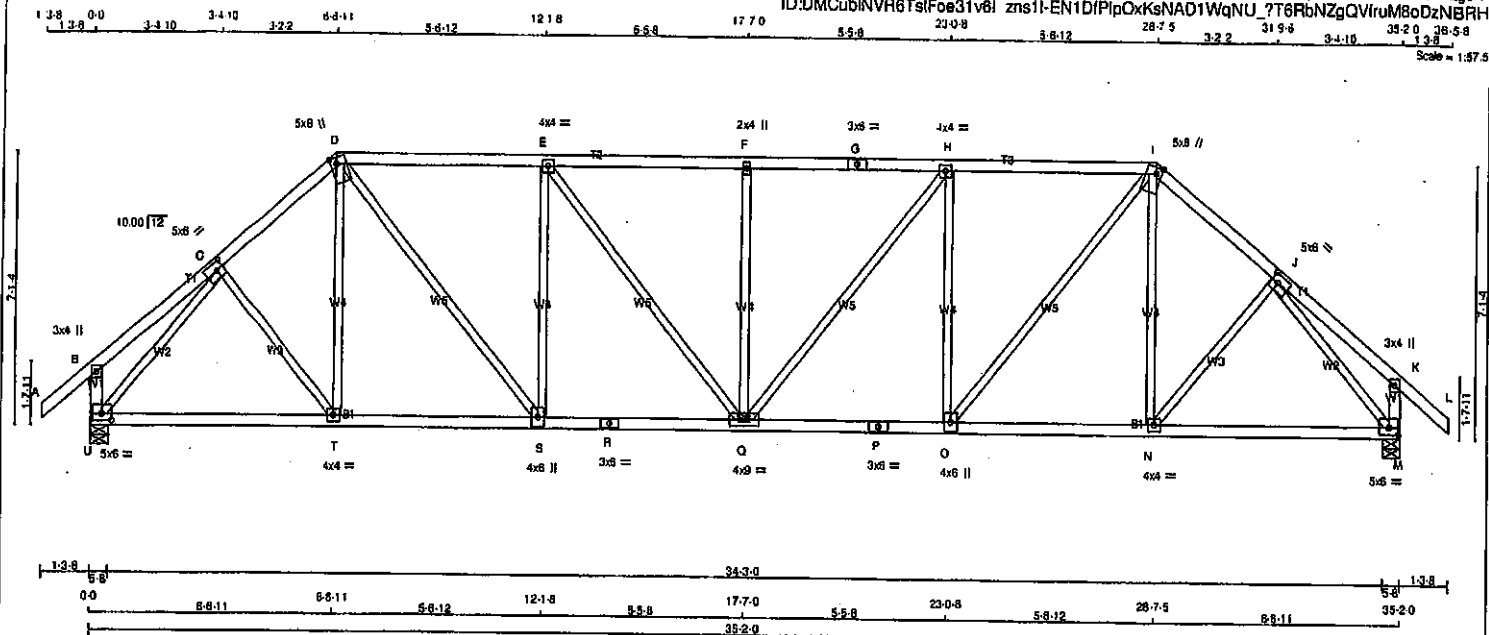
Structural component only
DWG# T-2007132

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408168	T4	2	1	GREEN PARK HOMES		

Taharack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6l zns11-EN1DIPloXsNAD1WqNU_?T6RbNZgQVruM8oDzNBRH



CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMV+p MT20	3.0	4.0		
C TMWW-t MT20	5.0	8.0	2.50	2.25
D TTWW+m MT20	5.0	8.0	2.00	1.75
E TMWW-t MT20	4.0	4.0		
F TMW+w MT20	2.0	4.0		
G TS-t MT20	3.0	6.0		
H TMWW-t MT20	4.0	4.0		
I TTWW+m MT20	5.0	8.0	2.00	1.75
J TMWW-t MT20	5.0	8.0	2.50	2.25
K TMV+p MT20	3.0	4.0		
M BMVW-t MT20	5.0	8.0	2.25	3.00
N BMWW-t MT20	4.0	4.0		
O BMWW-t MT20	4.0	6.0		
P BS-t MT20	3.0	6.0		
Q BMWW-t MT20	4.0	6.0		
R BS-t MT20	3.0	6.0		
S BMWW-t MT20	4.0	6.0		
T BMWW-t MT20	4.0	4.0		
U BMVW-t MT20	5.0	6.0	2.25	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX	IN-SX
U	2066	0	2066	0	0	5-8	5-8		
M	2066	0	2066	0	0	5-8	5-8		

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1458	971 / 0	0 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
M	1458	971 / 0	0 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX (PL)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX (LC)
FR-TO					FR-TO		
A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00	C-T	0 / 156	0.04 (1)
B-C	0 / 19	-91.8	-91.8 0.14 (1)	10.00	T-D	0 / 76	0.03 (4)
C-D	-2023 / 0	-91.8	-91.8 0.20 (1)	4.59	D-S	0 / 1220	0.27 (1)
D-E	-2308 / 0	-91.8	-91.8 0.55 (1)	3.94	S-E	-850 / 0	0.74 (1)
E-F	-2534 / 0	-91.8	-91.8 0.57 (1)	3.77	E-Q	0 / 361	0.08 (1)
F-G	-2534 / 0	-91.8	-91.8 0.57 (1)	3.77	Q-F	-461 / 0	0.40 (1)
G-H	-2534 / 0	-91.8	-91.8 0.57 (1)	3.77	Q-H	0 / 361	0.08 (1)
H-I	-2308 / 0	-91.8	-91.8 0.55 (1)	3.94	O-H	-850 / 0	0.74 (1)
I-J	-2023 / 0	-91.8	-91.8 0.20 (1)	4.59	O-I	0 / 1220	0.27 (1)
J-K	0 / 19	-91.8	-91.8 0.14 (1)	10.00	N-I	0 / 76	0.03 (4)
K-L	0 / 41	-91.8	-91.8 0.13 (1)	10.00	N-J	0 / 156	0.04 (1)
U-B	-246 / 0	0.0	0.0 0.03 (1)	7.81	U-C	-2277 / 0	0.98 (1)
M-K	-246 / 0	0.0	0.0 0.03 (1)	7.81	J-M	-2277 / 0	0.98 (1)
U-T	0 / 1433	-18.5	-18.5 0.33 (1)	10.00			
T-S	0 / 1635	-18.5	-18.5 0.35 (1)	10.00			
S-R	0 / 2308	-18.5	-18.5 0.42 (1)	10.00			
R-Q	0 / 2308	-18.5	-18.5 0.42 (1)	10.00			
Q-P	0 / 2308	-18.5	-18.5 0.42 (1)	10.00			
P-O	0 / 2308	-18.5	-18.5 0.42 (1)	10.00			
O-N	0 / 1535	-18.5	-18.5 0.35 (1)	10.00			
N-M	0 / 1433	-18.5	-18.5 0.33 (1)	10.00			

TOTAL WEIGHT = 2 X 162 = 323 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")

CALCULATED VERT. DEFL.(LL) = L/999 (0.13")

ALLOWABLE DEFL.(TL) = L/360 (1.17")

CALCULATED VERT. DEFL.(TL) = L/999 (0.23")

CSI: TC=0.57/1.00 (E-F:1), BC=0.42/1.00 (O-Q:1), WB=0.98/1.00 (J-M:1), SS=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

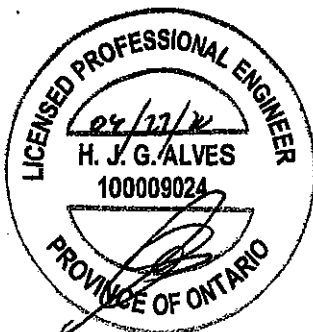
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1887 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (I) (INPUT = 0.90)
JSI METAL = 0.72 (P) (INPUT = 1.00)

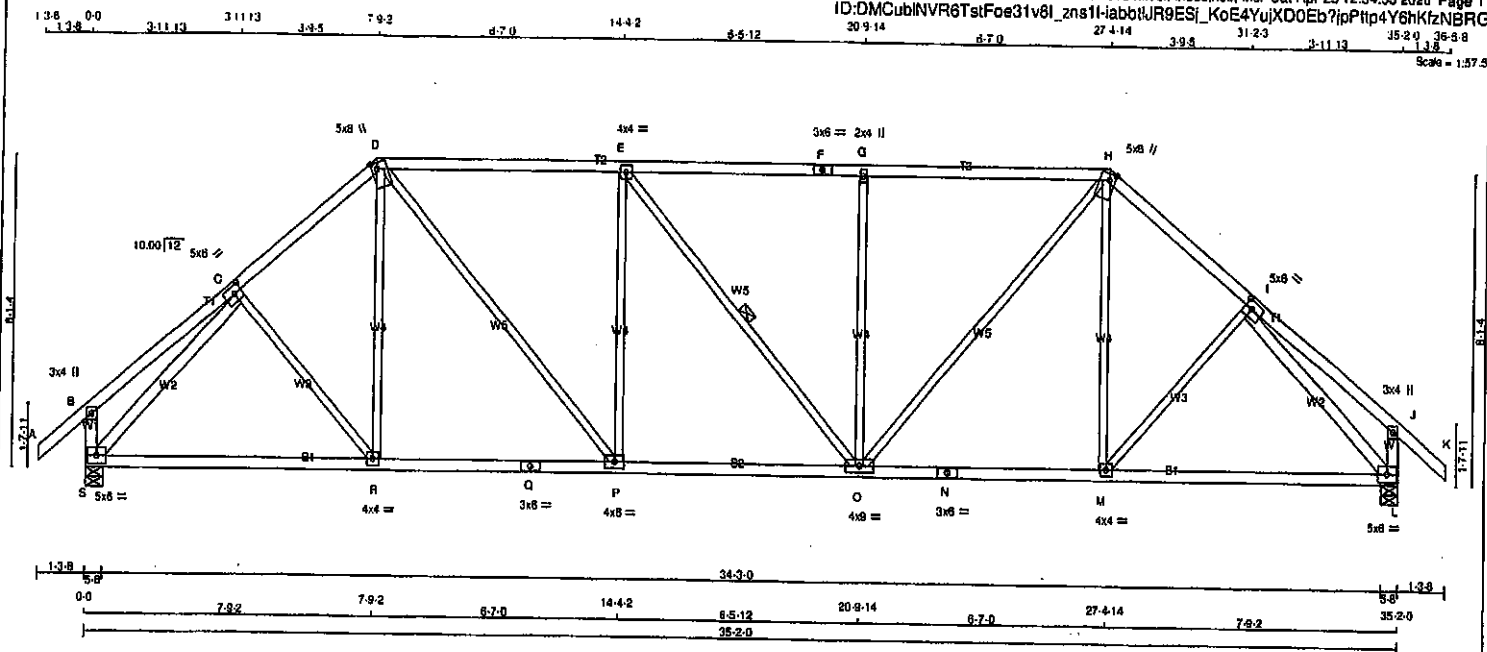


Structural component only
DWG# T-2007133

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408168	T5	2	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v8l_zns1habb1UR9ES1_KoE4YujXD0Eb7ipPtp4Y6hKfzNBRC



LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
S - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+P	MT20	3.0	4.0		
C TMWW-1	MT20	5.0	6.0	2.50	2.50
D TTWW+m	MT20	5.0	8.0	2.00	1.75
E TMWW-1	MT20	4.0	4.0		
F TS-1	MT20	3.0	6.0		
G TMW+w	MT20	2.0	4.0		
H TTWW+m	MT20	5.0	8.0	2.00	1.75
I TMWW-1	MT20	5.0	6.0	2.50	2.50
J TMV+P	MT20	3.0	4.0		
L BMVW-1	MT20	5.0	6.0		
M BMWW-1	MT20	4.0	4.0		
N BS-1	MT20	3.0	6.0		
O BMWW-1	MT20	4.0	9.0		
P BMWW-1	MT20	4.0	6.0		
Q BS-1	MT20	3.0	6.0		
R BMWW-1	MT20	4.0	4.0		
S BMVW-1	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX	BRG	IN-SX
S	2068	0	2068	0	0	5-8	5-8	5-8	5-8
L	2068	0	2068	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1458	871 / 0	0 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
L	1458	871 / 0	0 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00	C-R	0 / 64	0.02 (4)
B-C	0 / 26	-91.8	-91.8 0.21 (1)	10.00	R-D	0 / 126	0.04 (4)
C-D	-2068 / 0	-91.8	-91.8 0.29 (1)	4.50	D-P	0 / 998	0.22 (1)
D-E	-2163 / 0	-91.8	-91.8 0.73 (1)	3.81	P-E	-850 / 0	0.82 (1)
E-F	-2162 / 0	-91.8	-91.8 0.73 (1)	3.81	E-O	-2 / 0	0.00 (1)
F-G	-2162 / 0	-91.8	-91.8 0.73 (1)	3.81	O-H	0 / 998	0.22 (1)
G-H	-2162 / 0	-91.8	-91.8 0.73 (1)	3.81	H-I	0 / 126	0.04 (4)
H-I	-2068 / 0	-91.8	-91.8 0.29 (1)	4.50	I-J	0 / 64	0.02 (4)
I-J	0 / 26	-91.8	-91.8 0.21 (1)	10.00	J-K	0 / 41	0.02 (4)
J-K	0 / 41	-91.8	-91.8 0.13 (1)	10.00	K-L	-2291 / 0	0.97 (1)
S-B	-284 / 0	0.0	0.0 0.03 (1)	7.81	L-I	-2291 / 0	0.97 (1)
L-J	-284 / 0	0.0	0.0 0.03 (1)	7.81			
S-R	0 / 1487	-18.5	-18.5 0.38 (1)	10.00			
R-O	0 / 1519	-18.5	-18.5 0.39 (1)	10.00			
O-P	0 / 1519	-18.5	-18.5 0.39 (1)	10.00			
P-Q	0 / 2183	-18.5	-18.5 0.42 (1)	10.00			
Q-N	0 / 1519	-18.5	-18.5 0.39 (1)	10.00			
N-M	0 / 1519	-18.5	-18.5 0.39 (1)	10.00			
M-L	0 / 1487	-18.5	-18.5 0.38 (1)	10.00			

TOTAL WEIGHT = 2 X 163 = 326 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ORC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/998 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/998 (0.21")

CS1: TG=0.73/1.00 (D-E:1), BC=0.42/1.00 (O-P:1), WB=0.97/1.00 (L-I:1), SS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

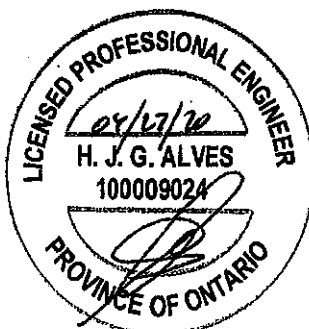
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1668

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

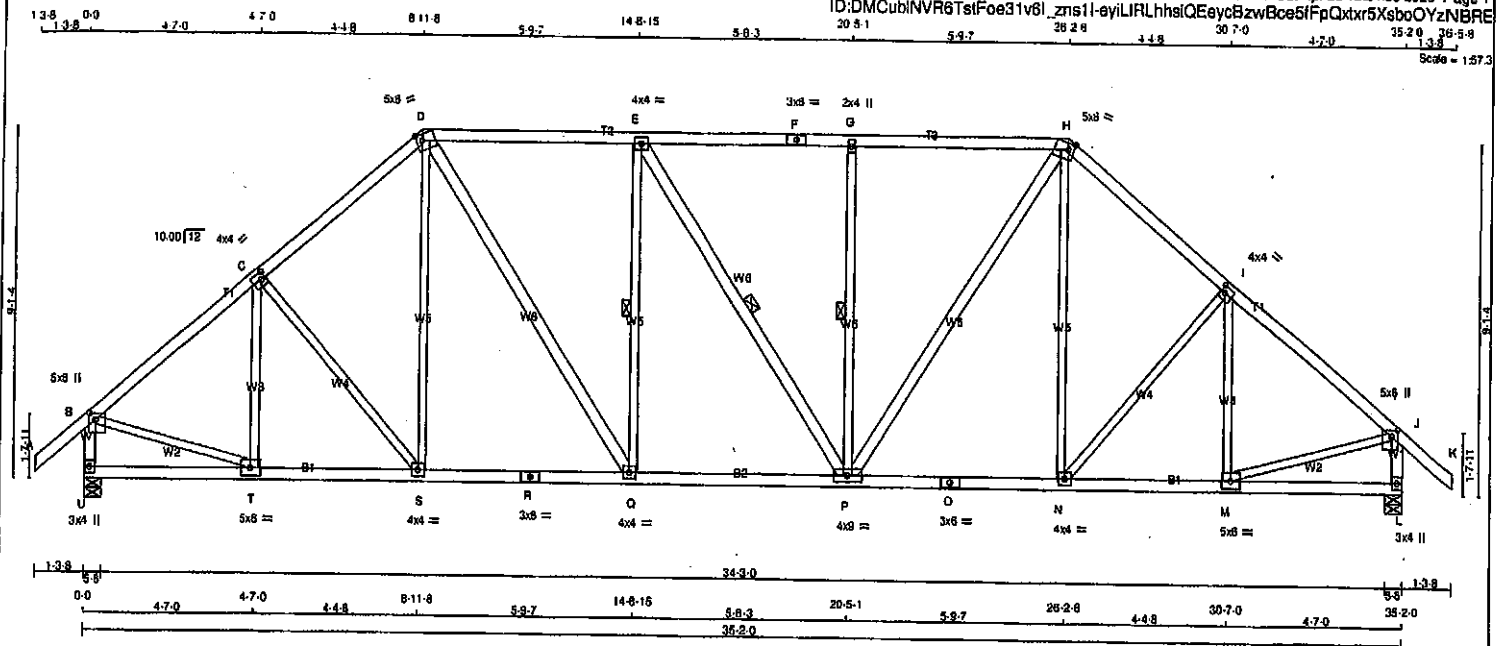
JSI GRIP = 0.87 (1) (INPUT = 0.90)
JSI METAL = 0.56 (1) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408168	T6	2	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6l_zns1l-eylLRLhshiQEaycBzwBoe5fPpCxtxr5XsboOYzNBRE



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
F - H	2x4 DRY	No.2	SPF		
H - K	2x4 DRY	No.2	SPF		
U - B	2x4 DRY	No.2	SPF		
L - J	2x4 DRY	No.2	SPF		
U - R	2x4 DRY	No.2	SPF		
R - O	2x4 DRY	No.2	SPF		
O - L	2x4 DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF		
D - Q	2x4 DRY	No.2	SPF		
E - P	2x4 DRY	No.2	SPF		
P - H	2x4 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	5.0	6.0	2.00	2.00
C TMVW-i	MT20	4.0	4.0	2.00	1.25
D TTWW-m	MT20	5.0	6.0	2.00	1.75
E TMVW-i	MT20	4.0	4.0		
F TS-i	MT20	3.0	6.0		
G TMVW-w	MT20	2.0	4.0		
H TTWW-m	MT20	5.0	6.0	2.00	1.75
I TMVW-i	MT20	4.0	4.0	2.00	1.25
J TMVW+p	MT20	5.0	6.0	2.00	2.00
L BMV-i+p	MT20	3.0	4.0		
M BMVW-i	MT20	5.0	6.0		
N, Q, S					
N BMVW-i	MT20	4.0	4.0		
O BS-i	MT20	3.0	6.0		
P BMVW-w-i	MT20	4.0	9.0		
R BS-i	MT20	3.0	6.0		
T BMVW-i	MT20	5.0	6.0		
U BMV-i+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	2088	0	0	5-8
U	2088	0	0	5-8
L	2088	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS	DEAD	SOIL
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND
U	1458	971 / 0	0 / 0	0 / 0
L	1458	971 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, E-P, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	T-C	-357 / 0	0.16 (1)
B-C	-1894 / 0	-91.8	-91.8	0.30 (1)	4.53	C-S	-121 / 0	0.10 (1)
C-D	-1957 / 0	-91.8	-91.8	0.29 (1)	4.57	S-D	0 / 188	0.05 (4)
D-E	-1923 / 0	-91.8	-91.8	0.40 (1)	4.44	D-Q	0 / 810	0.13 (1)
E-F	-1921 / 0	-91.8	-91.8	0.37 (1)	4.49	Q-E	-569 / 0	0.31 (1)
F-G	-1921 / 0	-91.8	-91.8	0.40 (1)	4.45	E-P	-3 / 0	0.00 (1)
G-H	-1921 / 0	-91.8	-91.8	0.40 (1)	4.45	P-G	-568 / 0	0.31 (1)
H-I	-1957 / 0	-91.8	-91.8	0.29 (1)	4.57	P-H	0 / 807	0.13 (1)
I-J	-1994 / 0	-91.8	-91.8	0.30 (1)	4.53	N-H	0 / 169	0.05 (4)
J-K	0 / 41	-91.8	-91.8	0.13 (1)	10.00	N-I	-121 / 0	0.10 (1)
U-B	-2028 / 0	0.0	0.0	0.22 (1)	5.94	M-I	-358 / 0	0.16 (1)
L-J	-2028 / 0	0.0	0.0	0.22 (1)	5.94	B-T	0 / 1617	0.36 (1)
U-T	0 / 0	-18.5	-18.5	0.08 (4)	10.00	M-J	0 / 1617	0.36 (1)
T-S	0 / 1555	-18.5	-18.5	0.30 (1)	10.00			
S-R	0 / 1477	-18.5	-18.5	0.30 (1)	10.00			
R-Q	0 / 1477	-18.5	-18.5	0.30 (1)	10.00			
Q-P	0 / 1923	-18.5	-18.5	0.38 (1)	10.00			
P-O	0 / 1478	-18.5	-18.5	0.30 (1)	10.00			
O-N	0 / 1478	-18.5	-18.5	0.30 (1)	10.00			
N-M	0 / 1555	-18.5	-18.5	0.31 (1)	10.00			
M-L	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

TOTAL WEIGHT = 2 X 182 = 363 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, OBC 2012, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.40/1.00 (D-E:1), BC=0.38/1.00 (P-Q:1), WB=0.38/1.00 (B-T:1), SS=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (B) (INPUT = 0.80)
JSI METAL = 0.50 (C) (INPUT = 1.00)

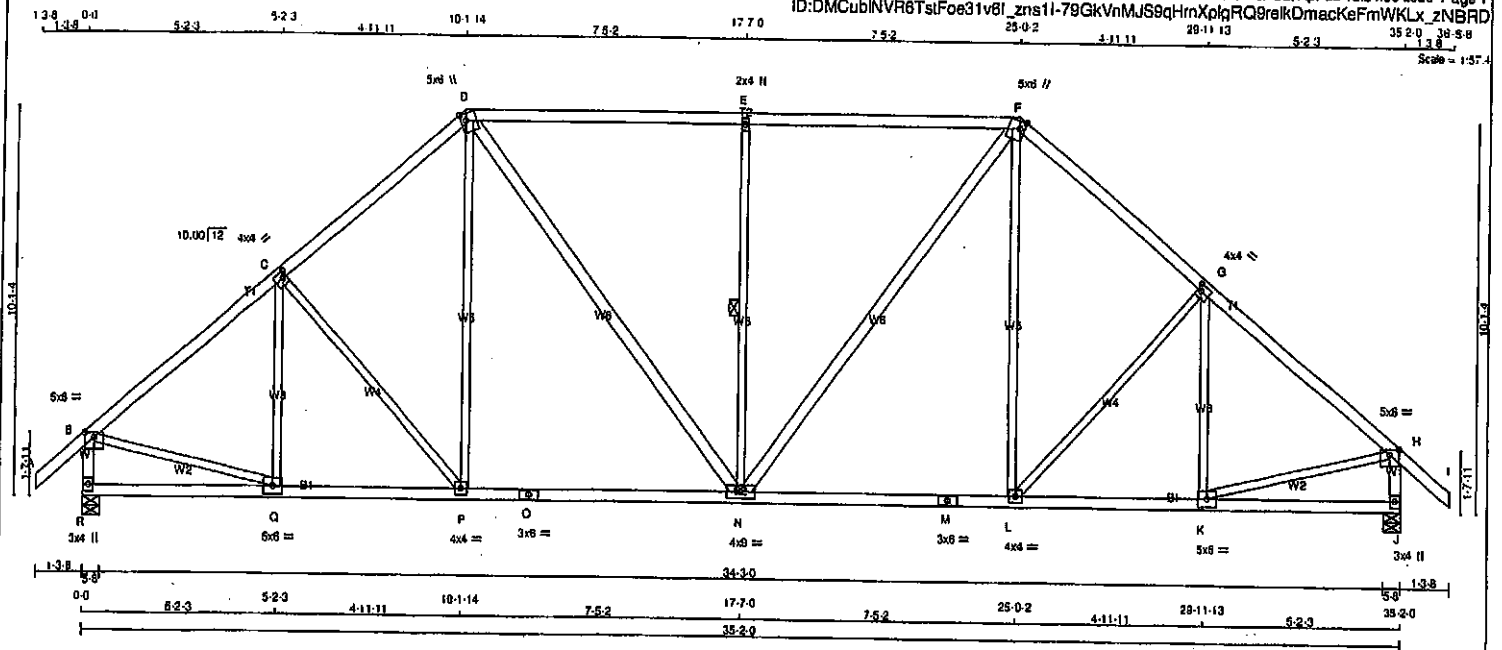


Structural component only
DWG# T-2007135

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408168	T7	5	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR8TslFoe31v8l_zns11-79GkVnMJS9qHnXpQ9reikDmacKeFmWKLx_zNBRD



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	6.0 1.50 3.00
C	TMVW-1	MT20	4.0	4.0 2.00 1.25
D	TTWV-m	MT20	5.0	6.0 2.25 1.50
E	TMVW-w	MT20	2.0	4.0
F	TTWV-m	MT20	5.0	6.0 2.25 1.50
G	TMVW-1	MT20	4.0	4.0 2.00 1.25
H	TMVW-p	MT20	5.0	6.0 1.50 3.00
J	BMV1-p	MT20	3.0	4.0
K	BMVW-1	MT20	5.0	6.0
L	BMVW-1	MT20	4.0	4.0
M	BS-1	MT20	3.0	6.0
N	BMVWV-1	MT20	4.0	9.0
O	BS-1	MT20	3.0	6.0
P	BMVW-1	MT20	4.0	4.0
Q	BMVW-1	MT20	5.0	6.0
R	BMV1-p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
JT VERT	2066	0	2066	0	5-8
R	2066	0	2066	0	5-8
J	2066	0	2066	0	5-8

UNFACTORED REACTIONS					
1ST LCASE	MAX/MIN. COMPONENT REACTIONS				
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
R	1458	971/0	0/0	0/0	488/0
J	1458	971/0	0/0	0/0	488/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING					
TOTAL LOAD CASES: (4)					
CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO	
A-B	0/41	-91.8	-91.8 0.13 (1)	Q-C	-302/0
B-C	-2021/0	-91.8	-91.8 0.39 (1)	C-P	-217/0
C-D	-1908/0	-91.8	-91.8 0.37 (1)	P-D	0/278
D-E	-1809/0	-91.8	-91.8 0.74 (1)	D-N	0/815
E-F	-1809/0	-91.8	-91.8 0.74 (1)	N-E	-838/0
F-G	-1908/0	-91.8	-91.8 0.37 (1)	N-F	0/815
G-H	-2021/0	-91.8	-91.8 0.39 (1)	L-F	0/278
H-I	0/41	-91.8	-91.8 0.13 (1)	L-G	-217/0
R-B	-2024/0	0.0	0.0 0.22 (1)	K-G	-302/0
J-H	-2024/0	0.0	0.0 0.22 (1)	B-Q	0/1829
R-Q	0/0	-18.5	-18.5 0.10 (4)	K-H	0/1829
Q-P	0/1580	-18.5	-18.5 0.34 (1)		
P-O	0/1437	-18.5	-18.5 0.35 (1)		
O-N	0/1437	-18.5	-18.5 0.35 (1)		
N-M	0/1437	-18.5	-18.5 0.35 (1)		
M-L	0/1437	-18.5	-18.5 0.35 (1)		
L-K	0/1580	-18.5	-18.5 0.34 (1)		
K-J	0/0	-18.5	-18.5 0.10 (4)		

TOTAL WEIGHT = 5 X 173 = 867 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OSC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TO=0.74/1.00 (D-E:1), BC=0.35/1.00 (L-N:1), WB=0.59/1.00 (E-N:1), SS=0.33/1.00 (D-E:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1856

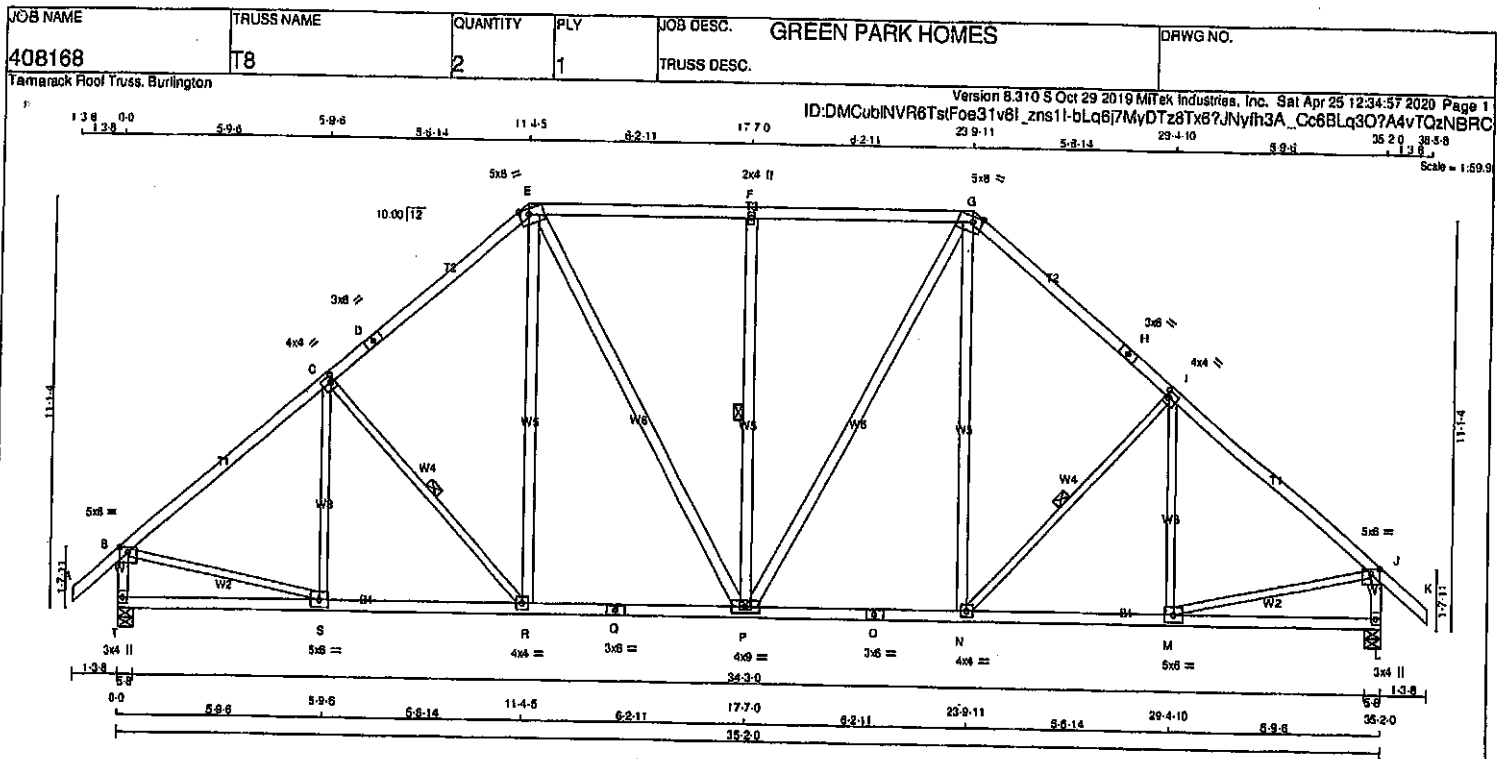
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.69 (H) (INPUT = 0.90)
JSI METAL = 0.46 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007136



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
S - C	2x3	DRY	No.2	SPF
C - R	2x3	DRY	No.2	SPF
N - I	2x3	DRY	No.2	SPF
M - I	2x3	DRY	No.2	SPF
B - S	2x3	DRY	No.2	SPF
M - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.00
C	TMVW-l	MT20	4.0	4.0	2.00	1.25
D	TS-l	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	Edge	3.00
F	TMVW-w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	3.00
H	TS-l	MT20	3.0	6.0		
I	TMVW-l	MT20	4.0	4.0	2.00	1.25
J	TMVW-p	MT20	5.0	8.0	1.50	3.00
L	BMV1-p	MT20	3.0	4.0		
M	BMVW-l	MT20	5.0	8.0		
N	BMVW-l	MT20	4.0	4.0		
O	BS-l	MT20	3.0	6.0		
P	BMVW-l	MT20	4.0	4.0		
Q	BS-l	MT20	3.0	6.0		
R	BMVW-l	MT20	4.0	4.0		
S	BMVW-l	MT20	5.0	8.0		
T	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
T	2088	0	2088	0	0	5-8
L	2088	0	2088	0	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX. MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	SNOW	LIVE	SNOW	LIVE				
T	1458	971 / 0	0 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
L	1458	971 / 0	0 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-R, F-P, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LCI MAX (PLF)	LCI MAX (PLF)			MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO						FR-TO		
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	S-C	-245 / 10	0.17 (1)
B-C	-2039 / 0	-91.8	-91.8	0.50 (1)	4.27	C-R	-314 / 0	0.15 (1)
C-D	-1844 / 0	-91.8	-91.8	0.46 (1)	4.47	R-E	0 / 338	0.05 (1)
D-E	-1844 / 0	-91.8	-91.8	0.46 (1)	4.47	E-P	0 / 489	0.08 (1)
E-F	-1821 / 0	-91.8	-91.8	0.50 (1)	4.60	P-F	-699 / 0	0.45 (1)
F-G	-1821 / 0	-91.8	-91.8	0.50 (1)	4.60	P-G	0 / 489	0.08 (1)
G-H	-1844 / 0	-91.8	-91.8	0.46 (1)	4.47	N-G	0 / 338	0.05 (1)
H-I	-1844 / 0	-91.8	-91.8	0.46 (1)	4.47	I-N	-314 / 0	0.15 (1)
I-J	-2039 / 0	-91.8	-91.8	0.50 (1)	4.27	M-I	-245 / 10	0.17 (1)
J-K	0 / 41	-91.8	-91.8	0.13 (1)	10.00	B-S	0 / 1637	0.37 (1)
T-B	-2021 / 0	0.0	0.0	0.21 (1)	5.84	M-J	0 / 1637	0.37 (1)
L-J	-2021 / 0	0.0	0.0	0.21 (1)	5.84			
T-S	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
S-R	0 / 1597	-18.5	-18.5	0.33 (1)	10.00			
R-Q	0 / 1387	-18.5	-18.5	0.30 (1)	10.00			
Q-P	0 / 1387	-18.5	-18.5	0.30 (1)	10.00			
P-O	0 / 1387	-18.5	-18.5	0.30 (1)	10.00			
O-N	0 / 1387	-18.5	-18.5	0.30 (1)	10.00			
N-M	0 / 1597	-18.5	-18.5	0.33 (1)	10.00			
M-L	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

TOTAL WEIGHT = 2 X 190 = 380 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17)
CALCULATED VERT. DEFL.(LL) = L/999 (0.07)
ALLOWABLE DEFL.(TL) = L/360 (1.17)
CALCULATED VERT. DEFL.(TL) = L/999 (0.14)

CSI: TC=0.50/1.00 (E-F:1), BC=0.33/1.00 (M-N:1), WB=0.45/1.00 (F-P:1), SS=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.47 (O) (INPUT = 1.00)



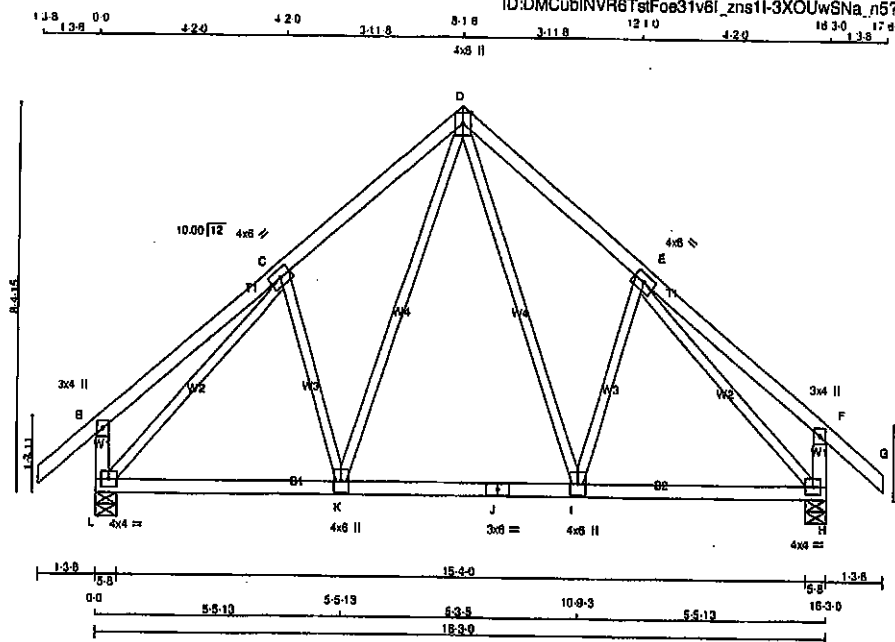
Structural component only
DWG# T-2007137

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408168	T9	6	1	TRUSS DESC.		

Tariarack Roof Truss, Burlington

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ID:DMCubINVR6TstFos31v6I_zns11-3XOUwSNa_n5755hBt5UuEGJD_0Uq4EhYDqps7tznBRB



Scale: 1/4"=1'

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2		SPF
D - G	2x4	DRY	No.2		SPF
L - B	2x4	DRY	No.2		SPF
H - F	2x4	DRY	No.2		SPF
L - J	2x4	DRY	No.2		SPF
J - H	2x4	DRY	No.2		SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+H	MT20	4.0	8.0		
D	TMVW+p	MT20	4.0	8.0		Edge
E	TMVW+H	MT20	4.0	8.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW+H	MT20	4.0	4.0		
I	BMVW+H	MT20	4.0	8.0		
J	BS+I	MT20	3.0	6.0		
K	BMVW+H	MT20	4.0	8.0		
L	BMVW+H	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT	IN-SX
L	1023	0	1023	0
H	1023	0	1023	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
L	721	487 / 0	0 / 0	234 / 0	0 / 0
H	721	487 / 0	0 / 0	234 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FR-TO
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	D-I	0 / 330	0.07 (1)			
B-C	0 / 28	-91.8	-91.8	0.24 (1)	10.00	I-E	-218 / 0	0.09 (1)			
C-D	-745 / 0	-91.8	-91.8	0.19 (1)	6.25	K-D	0 / 330	0.07 (1)			
D-E	-745 / 0	-91.8	-91.8	0.19 (1)	6.25	C-K	-218 / 0	0.09 (1)			
E-F	0 / 28	-91.8	-91.8	0.24 (1)	10.00	L-C	-943 / 0	0.61 (1)			
F-G	0 / 41	-91.8	-91.8	0.13 (1)	10.00	E-H	-943 / 0	0.61 (1)			
L-B	-267 / 0	0.0	0.0	0.03 (1)	7.81						
H-F	-267 / 0	0.0	0.0	0.03 (1)	7.81						
L-K	0 / 616	-18.5	-18.5	0.17 (4)	10.00						
K-J	0 / 449	-18.5	-18.5	0.16 (4)	10.00						
J-I	0 / 449	-18.5	-18.5	0.16 (4)	10.00						
I-H	0 / 616	-18.5	-18.5	0.17 (4)	10.00						

TOTAL WEIGHT = 6 X 80 = 481 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 8.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.24/1.00 (B-C:1), BC=0.17/1.00 (H-I:4), WB=0.61/1.00 (C-L:1), SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
		788	1987
			1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.76 (C) (INPUT = 0.90)
JSI METAL = 0.25 (C) (INPUT = 1.00)

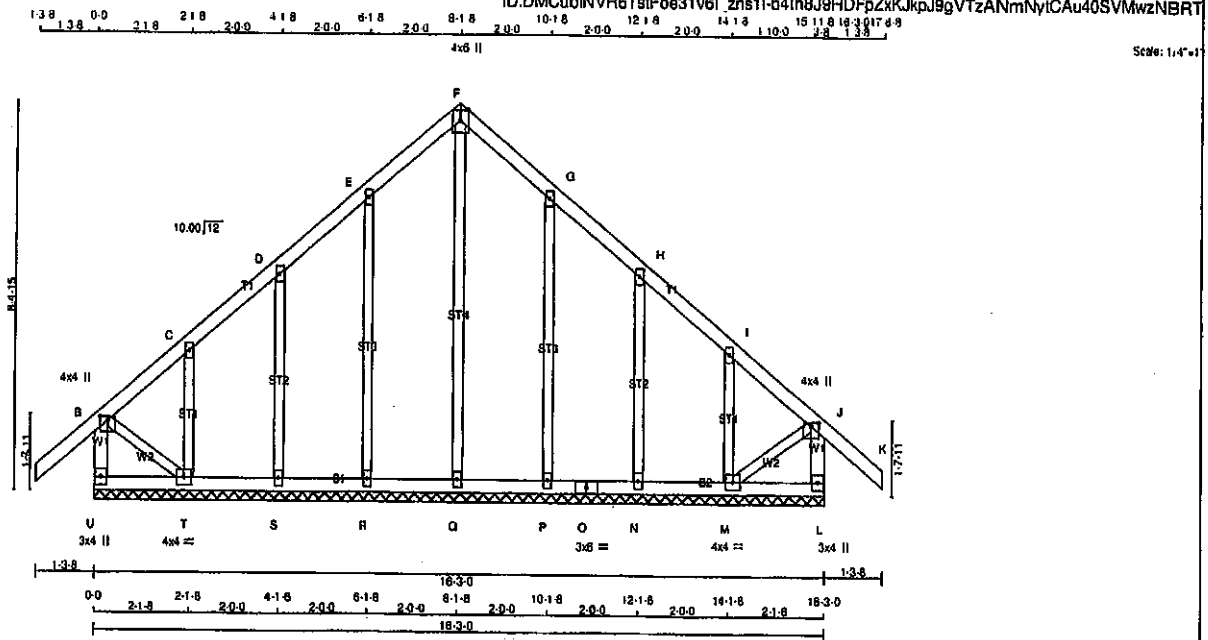


Structural component only
DWG# T-2007138

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408168	G9	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 81 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - F	2x4 DRY	No.2	SPF
F - K	2x4 DRY	No.2	SPF
U - B	2x4 DRY	No.2	SPF
L - J	2x4 DRY	No.2	SPF
U - O	2x4 DRY	No.2	SPF
O - L	2x4 DRY	No.2	SPF

ALL WEBS	2x3 DRY	No.2	SPF
ALL GABLE WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I					
C TMW+w	MT20	2.0	4.0		
F TTW+p	MT20	4.0	8.0	Edge	
J TMW+p	MT20	4.0	4.0	1.00	2.00
L BMV1+p	MT20	3.0	4.0		
M BMWV1-t	MT20	4.0	4.0		
N, P, Q, R, S					
N BMV1+w	MT20	2.0	4.0		
O BS4	MT20	3.0	8.0		
T BMWV1-t	MT20	4.0	4.0		
U BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO				FR-TO			
A-B	0/41	-91.8	-91.8 0.13 (1)	10.00	Q-F	-124/0	0.17 (1)
B-C	-28/0	-91.8	-91.8 0.05 (1)	8.25	R-E	-210/0	0.16 (1)
C-D	-39/0	-91.8	-91.8 0.05 (1)	6.25	S-D	-169/0	0.06 (1)
D-E	-24/0	-91.8	-91.8 0.05 (1)	6.25	T-C	-205/0	0.04 (1)
E-F	-38/0	-91.8	-91.8 0.05 (1)	6.25	P-G	-210/0	0.16 (1)
F-G	-38/0	-91.8	-91.8 0.05 (1)	6.25	N-H	-169/0	0.06 (1)
G-H	-24/0	-91.8	-91.8 0.05 (1)	6.25	M-I	-205/0	0.04 (1)
H-I	-38/0	-91.8	-91.8 0.05 (1)	6.25	B-T	0/37	0.01 (1)
I-J	-28/0	-91.8	-91.8 0.05 (1)	6.25	M-J	0/37	0.01 (1)
J-K	0/41	-91.8	-91.8 0.13 (1)	10.00			
U-B	-248/0	0.0	0.0 0.03 (1)	7.81			
L-J	-248/0	0.0	0.0 0.03 (1)	7.81			
U-T	0/0	-18.5	-18.5 0.02 (4)	10.00			
T-S	0/25	-18.5	-18.5 0.02 (4)	10.00			
S-R	0/21	-18.5	-18.5 0.02 (4)	10.00			
R-Q	0/18	-18.5	-18.5 0.02 (4)	10.00			
Q-P	0/18	-18.5	-18.5 0.02 (4)	10.00			
P-O	0/21	-18.5	-18.5 0.02 (4)	10.00			
O-N	0/21	-18.5	-18.5 0.02 (4)	10.00			
N-M	0/25	-18.5	-18.5 0.02 (4)	10.00			
M-L	0/0	-18.5	-18.5 0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (M-N:4), WB=0.17/1.00 (F-G:1), SI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	616	354
	1567	788
	1987	1556

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.52 (T) (INPUT = 0.90)

JSI METAL = 0.11 (G) (INPUT = 1.00)



Structural component only
DWG# T-2007123

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ID:DMCubiNVR6TsiFoe31v6l_zns1l-Xjys8cOCi4DsiFFNQo?7nUGOUQrOpqnhSUZ?XJzNBRA



**Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.**

CHORDS		WEBS
MAX FACTORED	FACTORED	
1	1	1
2	2	2
3	3	3
4	4	4
5	5	5
6	6	6
7	7	7
8	8	8
9	9	9
10	10	10
11	11	11
12	12	12
13	13	13
14	14	14
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91	91	91
92	92	92
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95	95	95
96	96	96
97	97	97
98	98	98
99	99	99
100	100	100

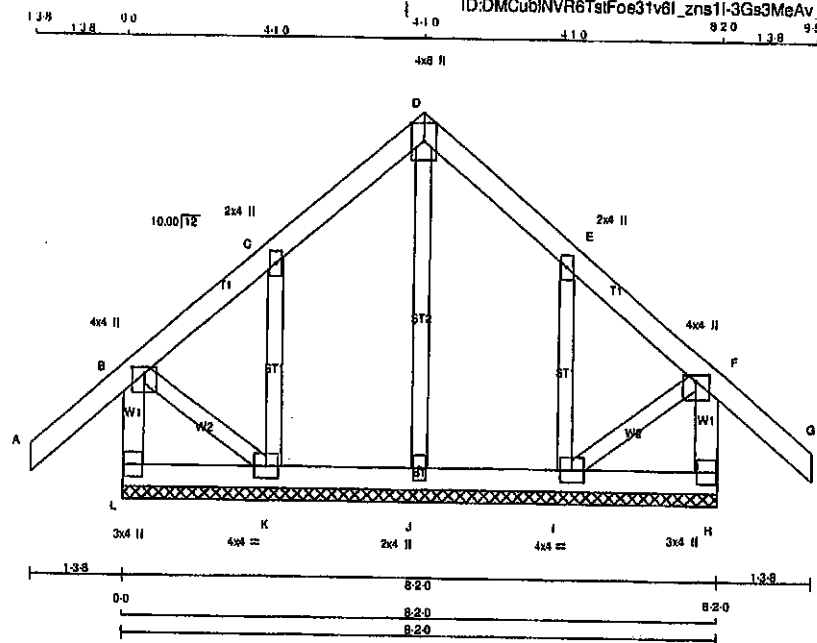
JSI GRIP= 0.46 (D) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408168	G10	1	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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Scale = 1/29.6

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
L - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - H	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C TMW+w	MT20	2.0	4.0		
D TTW+p	MT20	4.0	6.0	Edge	
E TMW+w	MT20	2.0	4.0		
F TMVW+p	MT20	4.0	4.0	1.00	2.00
H BMV1+p	MT20	3.0	4.0		
I BMW1-w	MT20	4.0	4.0		
J BMW1+w	MT20	2.0	4.0		
K BMW1-w	MT20	4.0	4.0		
L BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
L-B	-224 / 0	0.0 0.0 0.02 (1)	7.81	J-D	-128 / 0	0.05 (1)	
A-B	0 / 41	-91.8 -91.8 0.13 (1)	10.00	K-C	-227 / 0	0.04 (1)	
B-C	-11 / 0	-91.8 -91.8 0.08 (1)	8.25	I-E	-227 / 0	0.04 (1)	
C-D	-31 / 0	-91.8 -91.8 0.08 (1)	8.25	B-K	0 / 24	0.01 (1)	
D-E	-31 / 0	-91.8 -91.8 0.08 (1)	8.25	I-F	0 / 24	0.01 (1)	
E-F	-11 / 0	-91.8 -91.8 0.08 (1)	8.25				
F-G	0 / 41	-91.8 -91.8 0.13 (1)	10.00				
H-F	-224 / 0	0.0 0.0 0.02 (1)	7.81				
L-K	0 / 0	-18.5 -18.5 0.02 (4)	10.00				
K-J	0 / 13	-18.5 -18.5 0.02 (4)	10.00				
J-I	0 / 13	-18.5 -18.5 0.02 (4)	10.00				
I-H	0 / 0	-18.5 -18.5 0.02 (4)	10.00				

TOTAL WEIGHT = 40 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. G.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (J-K:4), WB=0.05/1.00 (D-J:1), SS=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

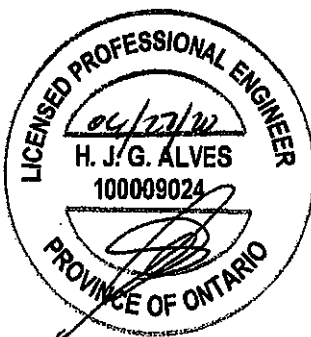
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20 618 354	1667 788	1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.17 (C) (INPUT = 0.90)
JSI METAL = 0.12 (C) (INPUT = 1.00)

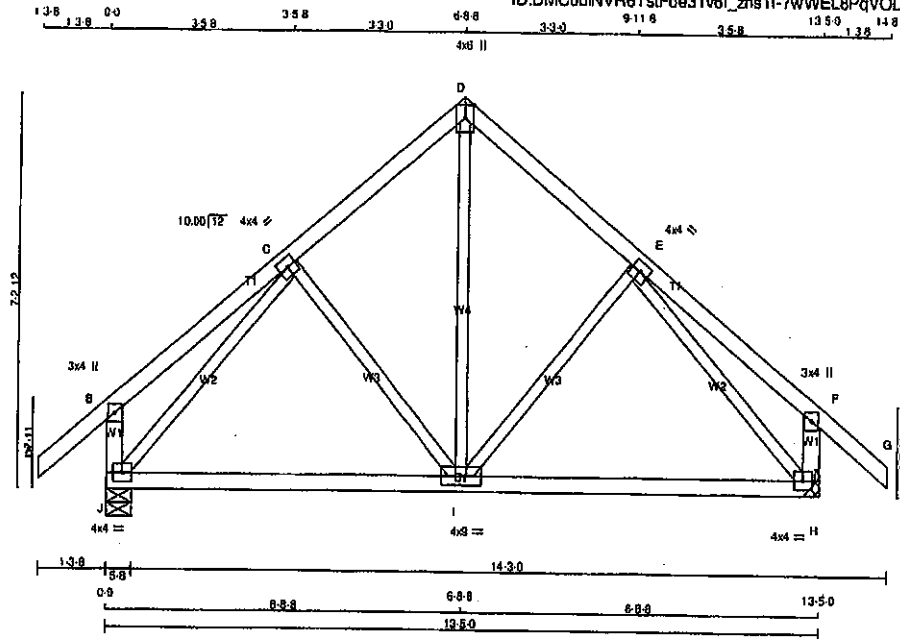


Structural component only
DWG# T-2007124

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408168	T11	3	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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Scale = 1:10.5

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
C	TMWW-t	MT20	4.0	4.0			
D	TTW+p	MT20	4.0	6.0			Edge
E	TMWW-t	MT20	4.0	4.0			
F	TMV+p	MT20	3.0	4.0			
H	BMWW1-t	MT20	4.0	4.0			
I	BMWW1-t	MT20	4.0	9.0			
J	BMWW1-t	MT20	4.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQD
		GROSS REACTION		GROSS REACTION		BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	867	0	867	0	0	5-8	5-8
H	867	0	867	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	611	414 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0	0 / 0
H	611	414 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED VERT. LOAD (PL)	MAX. FACTORED VERT. LOAD (PL)	MAX. FACTORED VERT. LOAD (PL)	MAX. FACTORED VERT. LOAD (PL)	MAX. FACTORED VERT. LOAD (PL)	MAX. FACTORED VERT. LOAD (PL)
FR-TO									
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	I-D	0 / 382	0.09 (1)	
B-C	0 / 23	-91.8	-91.8	0.16 (1)	10.00	I-E	-149 / 0	0.07 (1)	
C-D	-509 / 0	-91.8	-91.8	0.13 (1)	8.25	C-I	-149 / 0	0.07 (1)	
D-E	-509 / 0	-91.8	-91.8	0.13 (1)	8.25	J-C	-742 / 0	0.33 (1)	
E-F	0 / 23	-91.8	-91.8	0.16 (1)	10.00	E-H	-742 / 0	0.33 (1)	
F-G	0 / 41	-91.8	-91.8	0.13 (1)	10.00				
J-B	-244 / 0	0.0	0.0	0.03 (1)	7.81				
H-F	-244 / 0	0.0	0.0	0.03 (1)	7.81				
J-I	0 / 469	-18.5	-18.5	0.27 (4)	10.00				
I-H	0 / 469	-18.5	-18.5	0.27 (4)	10.00				

TOTAL WEIGHT = 3 X 64 = 191 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2016, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.45")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.45")
CALCULATED VERT. DEFL. (TL) = L/999 (0.05")

CSI: TC=0.18/1.00 (E-F:1), BC=0.27/1.00 (I-J:4),
WB=0.39/1.00 (E-H:1), SSI=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

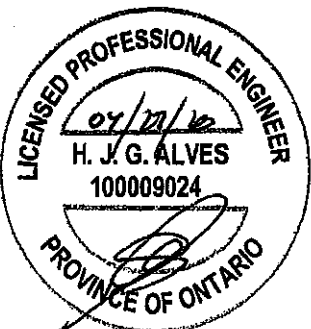
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLJ) (PLJ)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

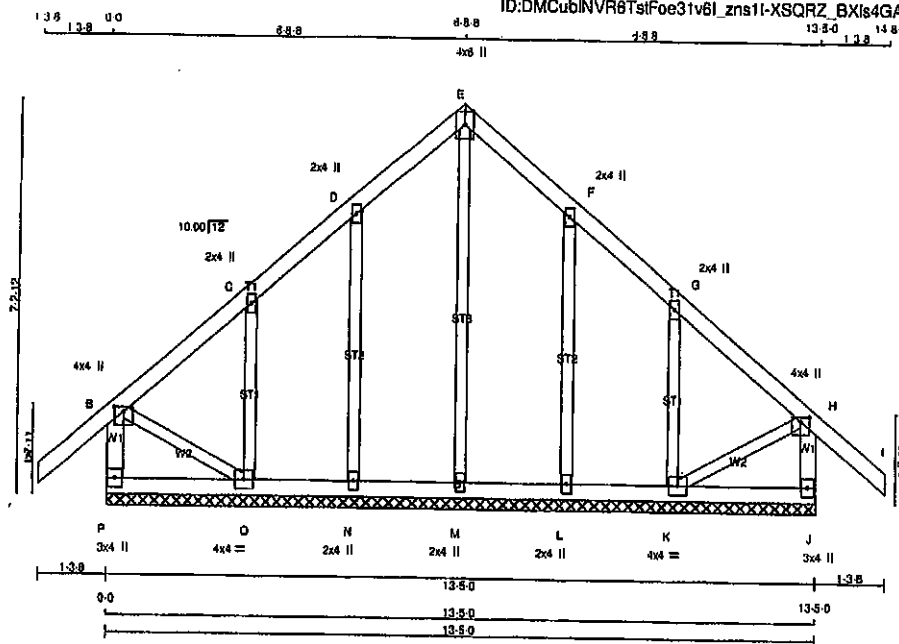
JSI GRIP = 0.84 (E) (INPUT = 0.80)
JSI METAL = 0.27 (E) (INPUT = 1.00)



Structural component only
DWG# T-2007140

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408168	G11	1	1	GREEN PARK HOMES	
Taharack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:40.5

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMVW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	8.0	Edge	
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMVW1-i	MT20	4.0	4.0		
L, M, N						
L	BMV1+w	MT20	2.0	4.0		
O	BMVW1-i	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
P-B	-263 / 0	0.0 0.0 0.03 (1)	7.81	M-E	-126 / 0	0.11 (1)	
A-B	0 / 41	-91.8 -91.8 0.13 (1)	10.00	N-D	-190 / 0	0.09 (1)	
B-C	-24 / 0	-91.8 -91.8 0.08 (1)	6.25	O-C	-239 / 0	0.05 (1)	
C-D	-36 / 0	-91.8 -91.8 0.08 (1)	6.25	L-F	-190 / 0	0.09 (1)	
D-E	-35 / 0	-91.8 -91.8 0.05 (1)	8.25	K-G	-239 / 0	0.08 (1)	
E-F	-35 / 0	-91.8 -91.8 0.05 (1)	8.25	B-O	0 / 32	0.01 (1)	
F-G	-38 / 0	-91.8 -91.8 0.08 (1)	8.25	K-H	0 / 32	0.01 (1)	
G-H	-24 / 0	-91.8 -91.8 0.08 (1)	6.25				
H-I	0 / 41	-91.8 -91.8 0.13 (1)	10.00				
J-H	-263 / 0	0.0 0.0 0.03 (1)	7.81				
P-O	0 / 0	-18.5 -18.5 0.03 (4)	10.00				
O-N	0 / 22	-18.5 -18.5 0.03 (4)	10.00				
N-M	0 / 19	-18.5 -18.5 0.02 (4)	10.00				
M-L	0 / 19	-18.5 -18.5 0.02 (4)	10.00				
L-K	0 / 22	-18.5 -18.5 0.03 (4)	10.00				
K-J	0 / 0	-18.5 -18.5 0.03 (4)	10.00				

TOTAL WEIGHT = 85 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (H-L:1), BC=0.03/1.00 (K-L:4),
WB=0.11/1.00 (E-M:1), SS=0.08/1.00 (G-H:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.20 (B) (INPUT = 0.90)
JSI METAL = 0.13 (G) (INPUT = 1.00)

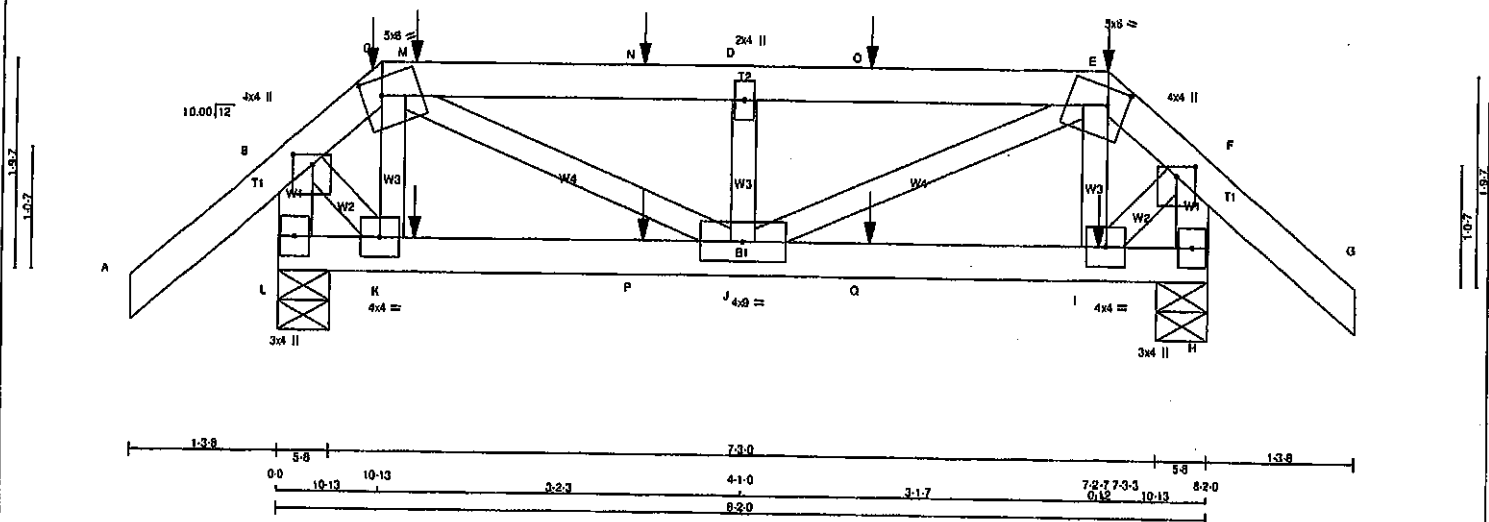


Structural component only
DWG# T-2007125

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408168	T12	1	1	GREEN PARK HOMES	
Targarak Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR6TstFoe31v6l_zns1I-T63dYUPSGITayZPmYD1bsvLjXEW1HhI_vn26cCzNBR8

1.38 1.38 0.0 10.13 10.13 1.27 2.00 3.27 4.10 5.27 6.42 7.33 10.13 8.20 1.38 9.50
Scale = 1:18.8



LUMBER

N. L. & A. RULES

CHORDS SIZE

A - C 2x4 DRY No.2

C - E 2x4 DRY No.2

E - G 2x4 DRY No.2

L - B 2x4 DRY No.2

H - F 2x4 DRY No.2

L - H 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE PLATES W LEN Y X

B TMVW-p MT20 4.0 4.0 1.00 2.00

C TTWW-m MT20 5.0 8.0 1.75 2.00

D TMVW-w MT20 2.0 4.0

E TTWW-m MT20 5.0 8.0 1.75 2.00

F TMVW-p MT20 4.0 4.0 1.00 2.00

H BMV1-p MT20 3.0 4.0

I BMWW-1 MT20 4.0 4.0

J BMWW-1 MT20 4.0 9.0

K BMWW-1 MT20 4.0 4.0

L BMV1-p MT20 3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
L	913	0	913	0
H	911	0	911	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	842	442 / 0	0 / 0	0 / 0	0 / 0	189 / 0	0 / 0
H	841	437 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8 -91.8	0.14 (1)	10.00	J-D	-487 / 0	0.08 (1)
B-C	-831 / 0	-91.8 -91.8	0.14 (1)	8.25	K-C	-156 / 0	0.02 (1)
C-M	-1054 / 0	-91.8 -91.8	0.23 (1)	5.82	C-J	0 / 700	0.17 (1)
M-N	-1054 / 0	-91.8 -91.8	0.23 (1)	5.82	B-K	0 / 525	0.13 (1)
N-D	-1054 / 0	-91.8 -91.8	0.23 (1)	5.82	I-E	-149 / 0	0.02 (1)
D-O	-1054 / 0	-91.8 -91.8	0.23 (1)	5.82	J-E	0 / 698	0.17 (1)
O-E	-1054 / 0	-91.8 -91.8	0.23 (1)	5.82	I-F	0 / 526	0.13 (1)
E-F	-832 / 0	-91.8 -91.8	0.14 (1)	6.25			
F-G	0 / 41	-91.8 -91.8	0.14 (1)	10.00			
L-B	-938 / 0	0.0 0.0	0.10 (1)	7.81			
H-F	-937 / 0	0.0 0.0	0.10 (1)	7.81			
L-K	0 / 0	-18.5 -18.5	0.05 (1)	10.00			
K-P	0 / 420	-18.5 -18.5	0.13 (1)	10.00			
P-J	0 / 420	-18.5 -18.5	0.13 (1)	10.00			
J-Q	0 / 421	-18.5 -18.5	0.14 (1)	10.00			
Q-I	0 / 421	-18.5 -18.5	0.14 (1)	10.00			
I-H	0 / 0	-18.5 -18.5	0.05 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	10-13	-9	-10	---	FRONT	VERT	DEAD	C1
C	1-2-7	-114	-114	---	BACK	VERT	TOTAL	C1
C	10-13	-44	-44	---	FRONT	VERT	SNOW	C1
E	7-3-3	-9	-10	---	FRONT	VERT	DEAD	C1
E	7-3-3	-90	-90	---	BACK	VERT	TOTAL	C1
E	7-3-3	-44	-44	---	FRONT	VERT	SNOW	C1
I	7-2-7	-49	-49	---	BACK	VERT	TOTAL	C1
K	1-2-7	-49	-49	---	BACK	VERT	TOTAL	C1
N	3-2-7	-82	-82	---	BACK	VERT	TOTAL	C1
O	5-2-7	-82	-82	---	BACK	VERT	TOTAL	C1
P	3-2-7	-49	-49	---	BACK	VERT	TOTAL	C1
Q	5-2-7	-49	-49	---	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 34 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.27")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.27")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CSI: TC=0.23/1.00 (C-D:1), BC=0.14/1.00 (I-J:1), WB=0.17/1.00 (C-J:1), SS=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

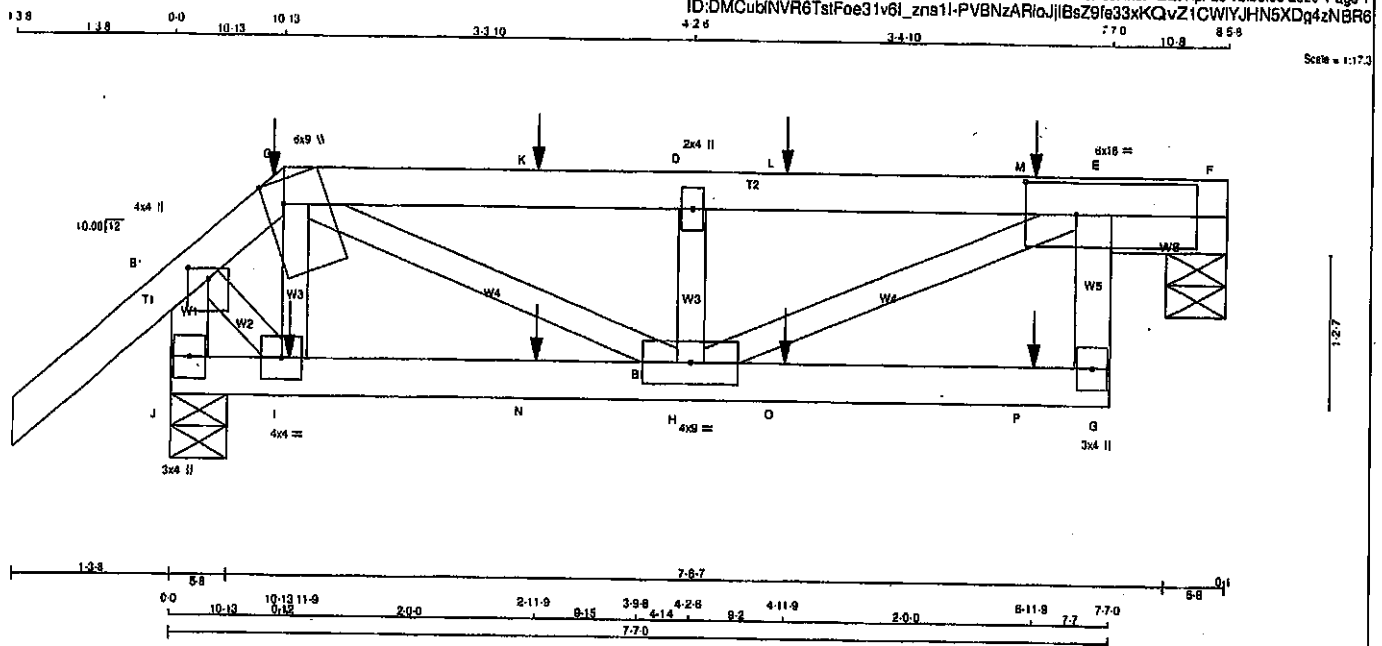
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.



Structural component only
DWG# T-2007141

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408168	T13	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
E - F	2x4	DRY	No.2
G - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
J - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
ORY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW+m	MT20	8.0	9.0	Edge	1.75
D	TMVW+w	MT20	2.0	4.0		
E	TMVW-w	MT20	8.0	16.0	3.00	5.00
G	BMV+p	MT20	3.0	4.0		
H	BMVW-w	MT20	4.0	9.0		
I	BMVW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	BRG	IN-SX	IN-SX	
F	895	0	895	0	5-8	5-8	
J	936	0	936	0	5-8	5-8	

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): F

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	490	327 / 0	0 / 0	0 / 0	0 / 0	163 / 0	0 / 0
J	859	449 / 0	0 / 0	0 / 0	0 / 0	210 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 41	I-C	-158 / 0
B-C	-658 / 0	B-I	0 / 548
C-K	-1230 / 0	H-D	-767 / 0
K-D	-1230 / 0	C-H	0 / 887
D-L	-1230 / 0	H-E	0 / 1345
L-M	-1230 / 0		
M-E	-1230 / 0		
E-F	0 / 0		
G-E	0 / 83		
J-B	-968 / 0		
J-I	0 / 0		
I-N	0 / 438		
N-H	0 / 438		
H-O	0 / 0		
O-P	0 / 0		
P-G	0 / 0		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	10-13	-9	-10	---	FRONT	VERT	DEAD	---	C1
C	10-13	-80	-80	---	FRONT	VERT	TOTAL	---	C1
C	10-13	-44	-44	---	FRONT	VERT	SNOW	---	C1
I	11-9	-49	-49	---	FRONT	VERT	TOTAL	---	C1
K	2-11-9	-82	-82	---	FRONT	VERT	TOTAL	---	C1
L	4-11-9	-82	-82	---	FRONT	VERT	TOTAL	---	C1
M	6-11-9	-82	-82	---	FRONT	VERT	TOTAL	---	C1
N	2-11-9	-49	-49	---	FRONT	VERT	TOTAL	---	C1
O	4-11-9	-48	-48	---	FRONT	VERT	TOTAL	---	C1
P	6-11-9	-53	-53	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 32 b

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BC80 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/899 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/888 (0.13")

CSI: TC=0.89/1.00 (D-E:1), BC=0.14/1.00 (H-I:1),
WB=0.33/1.00 (E-H:1), SSI=0.42/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.78 (C) (INPUT = 0.90)
JSI METAL = 0.30 (H) (INPUT = 1.00)

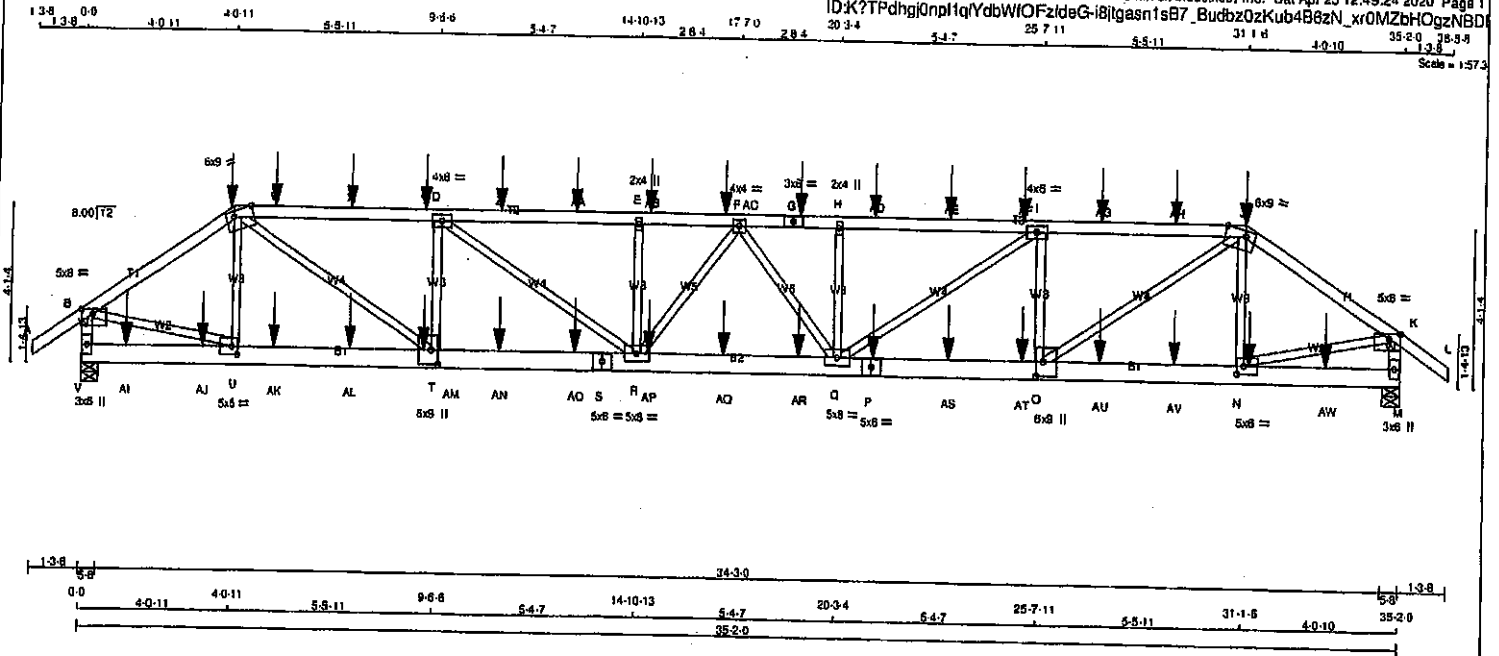


Structural component only
DWG# T-2007142

Tamarack Roof Truss, Burlington

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ID:K?TPdnglOnp1qYdbWOFzdeG-i8lGasntS7_Budbz0zKub4B6zN_xr0MZbHOgzNBD



CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
V - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
V - S	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS			
A - C	1	12	SIDE(61.0)
C - G	1	12	SIDE(61.0)
G - J	1	12	SIDE(61.0)
J - L	1	12	SIDE(61.0)
V - B	1	12	TOP
M - K	1	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS			
V - S	2	12	SIDE(183.1)
S - P	2	12	SIDE(183.1)
P - M	2	12	SIDE(183.1)
WEBS: (0.122"x3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT 3342	DOWN 3342	0	5-8
V	0	0	0	5-8
M	0	0	0	5-8

UNFACTORED REACTIONS	1ST CASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
V	2363	1555 / 0 0 / 0 0 / 0 608 / 0 0 / 0
M	2397	1585 / 0 0 / 0 0 / 0 812 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED GSI (LO)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO					FR-TO		
A-B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	U-C	-586 / 0
B-C	-3851 / 0	-91.8	-91.8	0.22 (1)	4.58	C-T	0 / 3530
C-W	-6188 / 0	-91.8	-91.8	0.61 (1)	3.41	T-D	-1829 / 0
W-X	-6188 / 0	-91.8	-91.8	0.61 (1)	3.41	D-R	0 / 1602
X-Y	-6188 / 0	-91.8	-91.8	0.61 (1)	3.41	R-E	-802 / 0
Y-D	-6188 / 0	-91.8	-91.8	0.61 (1)	3.41	Q-H	-586 / 0
D-Z	-7502 / 0	-91.8	-91.8	0.71 (1)	3.03	Q-I	0 / 1573
Z-AA	-7502 / 0	-91.8	-91.8	0.71 (1)	3.03	O-I	-1808 / 0
AA-E	-7502 / 0	-91.8	-91.8	0.71 (1)	3.03	O-J	0 / 3493
E-AB	-7502 / 0	-91.8	-91.8	0.35 (1)	3.33	N-J	-816 / 0
AB-AC	-7502 / 0	-91.8	-91.8	0.35 (1)	3.33	B-U	0 / 3374
AC-F	-7502 / 0	-91.8	-91.8	0.35 (1)	3.33	N-K	0 / 3439
F-G	-7509 / 0	-91.8	-91.8	0.37 (1)	3.31	R-F	-179 / 0
G-H	-7509 / 0	-91.8	-91.8	0.37 (1)	3.31	F-Q	-166 / 0
H-AD	-7509 / 0	-91.8	-91.8	0.71 (1)	3.03		
AD-AE	-7509 / 0	-91.8	-91.8	0.71 (1)	3.03		
AE-AF	-7509 / 0	-91.8	-91.8	0.71 (1)	3.03		
AF-I	-7509 / 0	-91.8	-91.8	0.71 (1)	3.03		
I-AG	-8217 / 0	-91.8	-91.8	0.61 (1)	3.41		
AG-AH	-8217 / 0	-91.8	-91.8	0.61 (1)	3.41		
AH-J	-8217 / 0	-91.8	-91.8	0.61 (1)	3.41		
J-K	-4025 / 0	-91.8	-91.8	0.22 (1)	4.55		
K-L	0 / 35	-91.8	-91.8	0.07 (1)	10.00		
V-B	-3288 / 0	0.0	0.0	0.16 (1)	8.41		
M-K	-3356 / 0	0.0	0.0	0.19 (1)	6.36		
V-AI	0 / 0	-18.5	-18.5	0.05 (4)	10.00		
AI-AJ	0 / 0	-18.5	-18.5	0.05 (4)	10.00		
AJ-U	0 / 0	-18.5	-18.5	0.05 (4)	10.00		
U-AK	0 / 3265	-18.5	-18.5	0.25 (1)	10.00		
AK-AL	0 / 3265	-18.5	-18.5	0.25 (1)	10.00		
AL-AM	0 / 3265	-18.5	-18.5	0.25 (1)	10.00		
AM-T	0 / 3265	-18.5	-18.5	0.25 (1)	10.00		
T-AN	0 / 6188	-18.5	-18.5	0.46 (1)	10.00		
AN-AO	0 / 6188	-18.5	-18.5	0.46 (1)	10.00		
AO-S	0 / 6188	-18.5	-18.5	0.46 (1)	10.00		
S-R	0 / 6188	-18.5	-18.5	0.46 (1)	10.00		
R-AP	0 / 7608	-18.5	-18.5	0.56 (1)	10.00		
AP-AQ	0 / 7608	-18.5	-18.5	0.56 (1)	10.00		
AQ-AR	0 / 7608	-18.5	-18.5	0.56 (1)	10.00		
AR-Q	0 / 7608	-18.5	-18.5	0.56 (1)	10.00		
Q-P	0 / 8217	-18.5	-18.5	0.46 (1)	10.00		
P-AS	0 / 8217	-18.5	-18.5	0.46 (1)	10.00		

TOTAL WEIGHT = 2 X 180 = 321 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:	
TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPC 2011, TPC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/984 (0.44")

CSI: TC=0.71/1.00 (D-E:1), BC=0.56/1.00 (Q-R:1), WB=0.44/1.00 (C-T:1), SS=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)	
MAX MIN MAX MIN MAX MIN	
MT20 818 354 1687 788 1987 1658	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRP= 0.71 (G) (INPUT = 0.90)
JSI METAL= 0.59 (S) (INPUT = 1.00)



Structural component only
DWG# T-2007145 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	T20	1	2	GREEN PARK HOMES	
Tetrahack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	8.0	1.50	3.50
C	TTWW-m	MT20	6.0	9.0	Edge 8.25	
D	TMWV-t	MT20	4.0	6.0		
E	TMWV-w	MT20	2.0	4.0		
F	TMWV-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWV-w	MT20	2.0	4.0		
I	TMWV-t	MT20	4.0	6.0		
J	TTWW-m	MT20	6.0	9.0	Edge 8.25	
K	TMWV-p	MT20	5.0	8.0	1.50	3.50
M	BMV1+p	MT20	3.0	6.0		
N	BMWV-t	MT20	5.0	6.0	2.50	2.00
O	BMWV-t	MT20	6.0	9.0	4.50	2.00
P	BS-t	MT20	5.0	6.0		
Q	BMWVW-t	MT20	5.0	8.0		
R	BMWVW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	8.0		
T	BMWVW-t	MT20	6.0	9.0	4.50	2.00
U	BMWVW-t	MT20	6.0	9.0	2.50	2.00
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	MAX. FACTORED FORCE (LBS)	CS1 (LC)	UNBRAC
FR-TO		FROM TO		LENGTH FR-TO			
AS-AT	0 / 8217	-18.5	-18.5	0.48 (1)	10.00		
AT-O	0 / 8217	-18.5	-18.5	0.48 (1)	10.00		
O-AU	0 / 3328	-18.5	-18.5	0.28 (1)	10.00		
AU-AV	0 / 3328	-18.5	-18.5	0.26 (1)	10.00		
AV-N	0 / 3328	-18.5	-18.5	0.26 (1)	10.00		
N-AW	0 / 0	-18.5	-18.5	0.04 (4)	10.00		
AW-M	0 / 0	-18.5	-18.5	0.04 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-49	-55	---	FRONT	VERT	DEAD	---	C1
C	4-0-11	-254	-254	---	FRONT	VERT	SNOW	---	C1
G	19-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
J	31-1-8	-49	-55	---	FRONT	VERT	DEAD	---	C1
J	31-1-8	-117	-117	---	BACK	VERT	TOTAL	---	C1
J	31-1-8	-254	-254	---	FRONT	VERT	SNOW	---	C1
N	31-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
P	21-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
W	5-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
X	7-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
Y	9-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
Z	11-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
AA	13-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
AB	15-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
AC	17-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
AD	21-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
AE	23-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
AF	25-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
AG	27-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
AH	29-2-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
AI	1-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AJ	3-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AK	5-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AL	7-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AM	9-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AN	11-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AO	13-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AP	15-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AQ	17-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AR	19-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AS	23-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AT	25-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AU	27-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AV	29-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AW	33-2-12	-28	-28	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



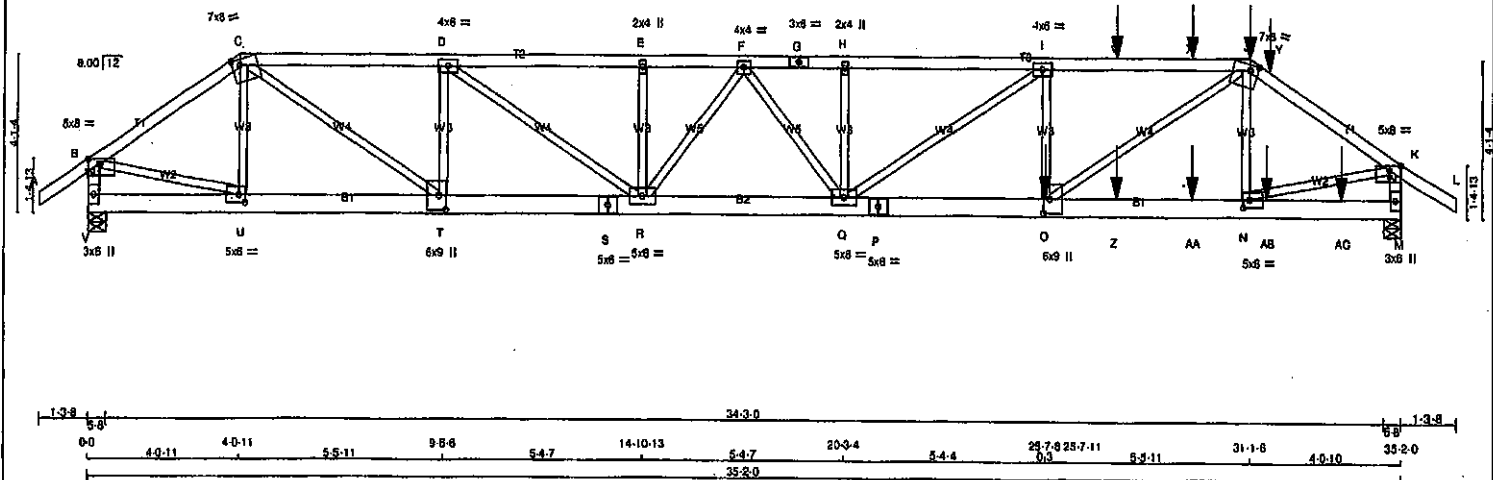
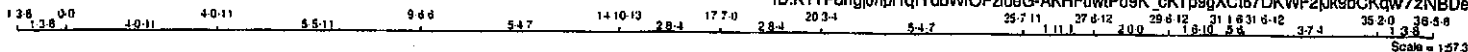
Structural component only
DWG# T-2007145

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408169	T20Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 160 = 321 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - G	2x4	DRY	No.2	SPF		
G - J	2x4	DRY	No.2	SPF		
J - L	2x4	DRY	No.2	SPF		
V - B	2x4	DRY	No.2	SPF		
M - K	2x4	DRY	No.2	SPF		
V - S	2x4	DRY	No.2	SPF		
S - P	2x4	DRY	No.2	SPF		
P - M	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - G	12	TOP
G - J	12	SIDE(61.0)
J - L	12	SIDE(61.0)
V - B	12	TOP
M - K	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
V - S	12	TOP
S - P	12	TOP
P - M	12	SIDE(163.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	
I - O	3	SIDE(557.9)
D - T	3	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION				
JT VERT	2938	0	2938	0
V	2938	0	2938	0
M	4796	0	4796	0

UNFACTORED REACTIONS	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
V	2073	1388 / 0 0 / 0 0 / 0 0 / 0 884 / 0 0 / 0
M	3381	2278 / 0 0 / 0 0 / 0 0 / 0 1103 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING	CHORDS	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED
TOTAL LOAD CASES: (4)					
FR-TO	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MAX. FORCE (LBS)
A-B	0 / 35	-91.8	-91.8	0.07 (1)	10.00
B-C	-3442 / 0	-91.8	-91.8	0.20 (1)	4.88
C-D	-5905 / 0	-91.8	-91.8	0.43 (1)	3.70
D-E	-7947 / 0	-91.8	-91.8	0.58 (1)	3.11
E-F	-7947 / 0	-91.8	-91.8	0.31 (1)	3.30
F-G	-9222 / 0	-91.8	-91.8	0.38 (1)	3.01
G-H	-9222 / 0	-91.8	-91.8	0.38 (1)	3.01
H-I	-9222 / 0	-91.8	-91.8	0.77 (1)	2.73
I-W	-9835 / 0	-91.8	-91.8	0.88 (1)	2.57
W-X	-9835 / 0	-91.8	-91.8	0.88 (1)	2.57
X-J	-9835 / 0	-91.8	-91.8	0.88 (1)	2.57
J-Y	-5847 / 0	-91.8	-91.8	0.33 (1)	3.78
Y-K	-5847 / 0	-91.8	-91.8	0.33 (1)	3.78
K-L	0 / 35	-91.8	-91.8	0.07 (1)	10.00
V-B	-2814 / 0	0.0	0.0	0.18 (1)	6.74
M-K	-4771 / 0	0.0	0.0	0.27 (1)	5.48

FACTORED CONCENTRATED LOADS (LBS)	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
JT	31-1-6	-49	-59	---	FRONT	VERT	DEAD	---	C1
J	31-1-6	-254	-254	---	FRONT	VERT	SNOW	---	C1
O	25-7-8	-2840	-2840	---	FRONT	VERT	TOTAL	---	C1
W	27-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	28-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	31-6-12	-136	-136	---	FRONT	VERT	TOTAL	---	C1
Z	27-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AA	29-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AB	31-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AC	33-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.0/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(85 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.28")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/832 (0.51")

CSI: TC=0.88/1.00 (I-J:1), BC=0.72/1.00 (O-Q:1), WB=0.71/1.00 (J-O:1), SS=0.18/1.00 (J-I:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.89 (C) (INPUT = 0.90)

JSI METAL=0.87 (S) (INPUT = 1.00)



Structural component only
DWG# T-2007146 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	T20Z	1	2	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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ID:K?TPdhai0nol1alYdbWIOFzIdaG-AKHFWuPc9K cKTo9aXCi67DKWF2iJk9bCKaw7zNBDe

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	1.50	3.50
C	TTWW-m	MT20	7.0	8.0	1.75	2.50
D	TMWW-l	MT20	4.0	6.0		
E	TMWW-w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-l	MT20	3.0	6.0		
H	TMWW-w	MT20	2.0	4.0		
I	TMWW-t	MT20	4.0	8.0		
J	TTWW-m	MT20	7.0	8.0	1.75	2.50
K	TMWW-p	MT20	5.0	8.0	1.50	3.50
M	BMV1-p	MT20	3.0	6.0		
N	BMWW-l	MT20	5.0	8.0	2.50	2.00
O	BMWW-t	MT20	6.0	9.0	4.50	2.00
P	BS-l	MT20	5.0	6.0		
Q	BMWWW-l	MT20	5.0	8.0		
R	BMWWW-l	MT20	5.0	8.0		
S	BS-l	MT20	5.0	6.0		
T	BMWW-t	MT20	6.0	9.0	4.50	2.00
U	BMWW-l	MT20	5.0	8.0	2.50	2.00
V	BMV1-p	MT20	3.0	6.0		

CONNECTION REQUIREMENTS

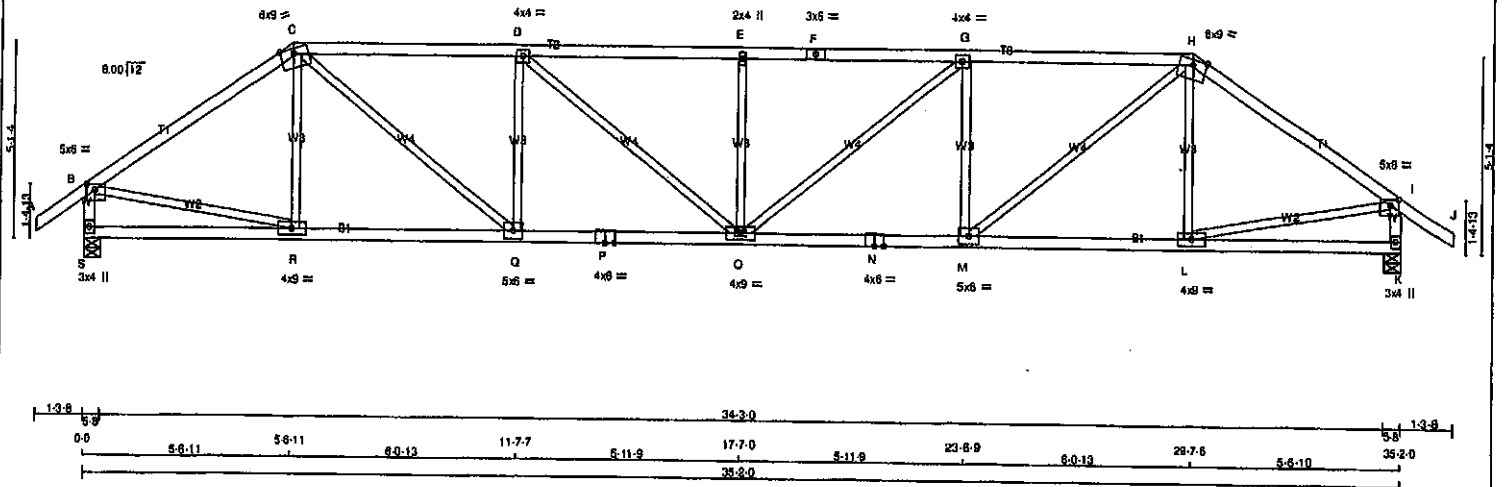
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007146

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JOB NAME 408169	TRUSS NAME T21	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 5 Oct 2019 Mitek Industries, Inc. Sat Apr 25 12:49:28 2020 Page 1	
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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
L - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - P	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (tables in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMWW-p	MT20	5.0	6.0 1.75 2.75
C	TTWW-m	MT20	6.0	9.0 Edge
D	TMWW-l	MT20	4.0	4.0
E	TMWW-w	MT20	2.0	4.0
F	TS-l	MT20	3.0	6.0
G	TMWW-l	MT20	4.0	4.0
H	TTWW-m	MT20	6.0	9.0 Edge
I	TMWW-p	MT20	5.0	6.0 1.75 2.75
K	BMV1+sp	MT20	3.0	4.0
L	BMWW-l	MT20	4.0	9.0
M	BMWW-l	MT20	5.0	6.0
N	BS-l	MT20	4.0	6.0
O	BMWWW-l	MT20	4.0	9.0
P	BS-l	MT20	4.0	6.0
Q	BMWW-l	MT20	6.0	6.0
R	BMWW-l	MT20	4.0	9.0
S	BMV1+sp	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ		DOWN	HORZ	IN-SX		IN-SX	
S	2065	0		2065	0	0	5-8	5-8	
K	2065	0		2065	0	0	5-8	5-8	
UNFACTORED REACTIONS									
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
S	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0		
K	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
		CHORDS			WEBS				
		MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED	MAX.		
		FORCE	VERT. LOAD	LO1 MAX	MEMB.	FORCE	FORCE		
		(LBS)	(PLF)	CSI (LC)	UNBRAC	(LBS)	MAX		
			FROM TO		LENGTH		CSI (LC)		
FR-TO									
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	R-C	-243 / 10	0.09 (1)	
B-C	-2297 / 0	-91.8	-91.8	0.70 (1)	3.78	C-Q	0 / 1669	0.38 (1)	
C-D	-3213 / 0	-91.8	-91.8	0.80 (1)	3.19	D-O	-929 / 0	0.38 (1)	
D-E	-3594 / 0	-91.8	-91.8	0.86 (1)	2.97	O-G	0 / 489	0.11 (1)	
E-F	-3594 / 0	-91.8	-91.8	0.86 (1)	2.97	G-Q	-505 / 0	0.19 (1)	
F-G	-3594 / 0	-91.8	-91.8	0.86 (1)	2.97	O-G	0 / 490	0.11 (1)	
G-H	-3213 / 0	-91.8	-91.8	0.80 (1)	3.19	M-Q	-929 / 0	0.38 (1)	
H-I	-2297 / 0	-91.8	-91.8	0.70 (1)	3.78	M-H	0 / 1670	0.38 (1)	
I-J	0 / 35	-91.8	-91.8	0.12 (1)	10.00	L-H	-243 / 10	0.09 (1)	
S-B	-2023 / 0	0.0	0.0	0.21 (1)	5.94	B-R	0 / 1943	0.44 (1)	
K-I	-2023 / 0	0.0	0.0	0.21 (1)	5.94	L-I	0 / 1943	0.44 (1)	
S-R	0 / 0	-18.5	-18.5	0.15 (4)	10.00				
R-Q	0 / 1904	-18.5	-18.5	0.39 (1)	10.00				
Q-P	0 / 3213	-18.5	-18.5	0.58 (1)	10.00				
P-O	0 / 3213	-18.5	-18.5	0.58 (1)	10.00				
O-N	0 / 3213	-18.5	-18.5	0.58 (1)	10.00				
N-M	0 / 3213	-18.5	-18.5	0.58 (1)	10.00				
M-L	0 / 1903	-18.5	-18.5	0.39 (1)	10.00				
L-K	0 / 0	-18.5	-18.5	0.15 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/899 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.39")

CSI: TC=0.88/1.00 (E-G:1), BC=0.58/1.00 (O-Q:1), WB=0.44/1.00 (B-R:1), SSI=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1887 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (B) (INPUT = 0.80)
JSI METAL = 0.74 (P) (INPUT = 1.00)

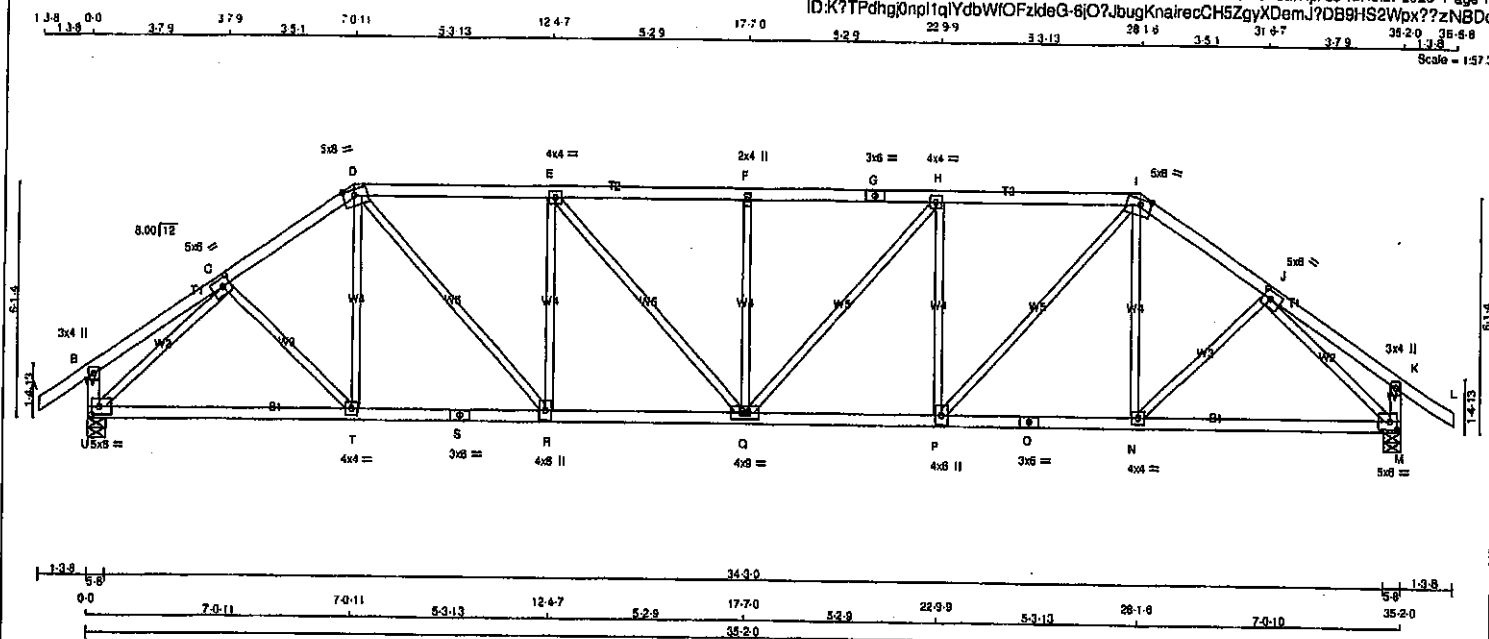


Structural component only
DWG# T-2007147

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408169	T22	2	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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ID:K7TPdng0npl1qYdbWioFzIdaG-8jO7JbugKnairecCH5ZgyXDemJ?DB9HS2Wpx??zNBDC



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
U - S	2x4	DRY	No.2
S - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMWW-t	MT20	5.0	6.0 2.50 2.50
D	TTWW-m	MT20	5.0	6.0 1.75 3.50
E	TMWW-t	MT20	4.0	4.0
F	TMWW-w	MT20	2.0	4.0
G	TS-t	MT20	3.0	6.0
H	TMWW-t	MT20	4.0	4.0
I	TTWW-m	MT20	5.0	6.0 1.75 3.50
J	TMWW-t	MT20	5.0	6.0 2.50 2.50
K	TMV+p	MT20	3.0	4.0
M	BMVW-t	MT20	5.0	6.0 2.50 2.25
N	BMWW-t	MT20	4.0	4.0
O	BS-t	MT20	3.0	6.0
P	BMWW-t	MT20	4.0	6.0
Q	BMWW-t	MT20	4.0	9.0
R	BMWW-t	MT20	4.0	6.0
S	BS-t	MT20	3.0	6.0
T	BMWW-t	MT20	4.0	4.0
U	BMVW-t	MT20	5.0	6.0 2.50 2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
	FACTORED	MAXIMUM FACTORED	INPUT	RECORD					
JT	GROSS REACTION	GROSS REACTION	BRG	BRG					
U	2065	0	2065	0	0	5-8	5-8		
M	2065	0	2065	0	0	5-8	5-8		
UNFACTORED REACTIONS									
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
U	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0		
M	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0		
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED LGT (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 35	-81.8	-81.8	0.12 (1)	10.00	C-T	0 / 136	0.03 (4)	
B-C	0 / 18	-81.8	-81.8	0.16 (1)	10.00	T-D	0 / 80	0.03 (4)	
C-D	-2305 / 0	-81.8	-81.8	0.23 (1)	4.31	D-R	0 / 1218	0.27 (1)	
D-E	-2723 / 0	-81.8	-81.8	0.54 (1)	3.89	R-E	-814 / 0	0.48 (1)	
E-F	-2968 / 0	-81.8	-81.8	0.58 (1)	3.53	E-Q	0 / 383	0.08 (1)	
F-G	-2968 / 0	-81.8	-81.8	0.58 (1)	3.53	Q-F	-440 / 0	0.26 (1)	
G-H	-2968 / 0	-81.8	-81.8	0.58 (1)	3.53	Q-H	0 / 383	0.08 (1)	
H-I	-2723 / 0	-81.8	-81.8	0.54 (1)	3.89	P-H	-814 / 0	0.48 (1)	
I-J	-2305 / 0	-81.8	-81.8	0.23 (1)	4.31	P-I	0 / 1218	0.27 (1)	
J-K	0 / 18	-81.8	-81.8	0.16 (1)	10.00	N-I	0 / 90	0.03 (4)	
K-L	0 / 35	-81.8	-81.8	0.12 (1)	10.00	N-J	0 / 136	0.03 (4)	
L-M	-255 / 0	0.0	0.0	0.03 (1)	7.81	U-C	-2515 / 0	0.97 (1)	
M-K	-255 / 0	0.0	0.0	0.03 (1)	7.81	J-M	-2514 / 0	0.97 (1)	
U-T	0 / 1802	-18.5	-18.5	0.40 (1)	10.00				
T-S	0 / 1801	-18.5	-18.5	0.41 (1)	10.00				
S-R	0 / 1801	-18.5	-18.5	0.41 (1)	10.00				
R-Q	0 / 2724	-18.5	-18.5	0.49 (1)	10.00				
Q-P	0 / 2723	-18.5	-18.5	0.49 (1)	10.00				
P-O	0 / 1800	-18.5	-18.5	0.41 (1)	10.00				
O-N	0 / 1800	-18.5	-18.5	0.41 (1)	10.00				
N-M	0 / 1801	-18.5	-18.5	0.40 (1)	10.00				

TOTAL WEIGHT = 2 X 151 = 303 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCG 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CSI: TC=0.58/1.00 (E-F:1), BC=0.49/1.00 (Q-R:1),
WB=0.97/1.00 (G-U:1), SSI=0.23/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

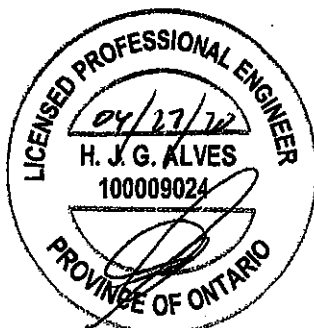
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)

JSI METAL= 0.93 (C) (INPUT = 1.00)

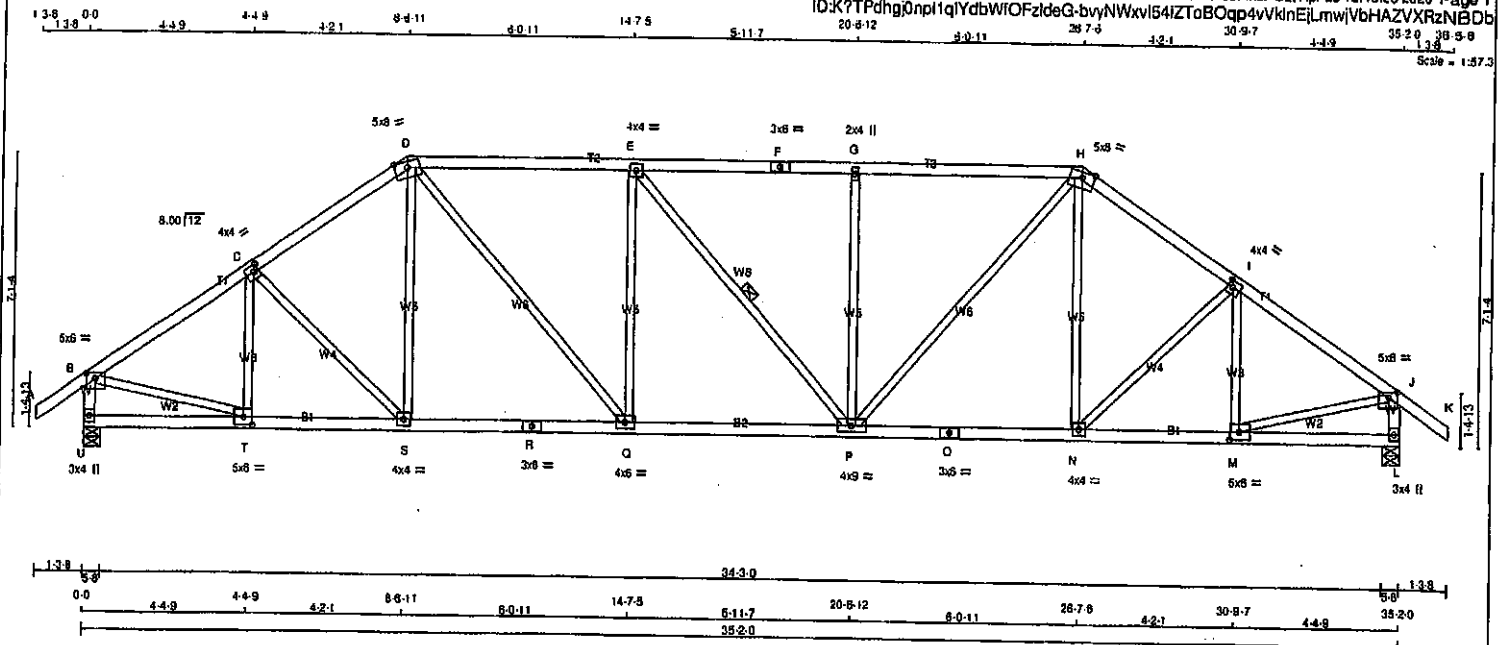


Structural component only
DWG# T-2007148

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	T23	2	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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ID:K7TPdhg0npl1q1YdbWIOFzdeG-bvYvNWxv154IZToBOqp4vVknEjLmwjVbHAZVXRzNBDB



LUMBER	SIZE	DRY	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
U - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
U - R	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TMVW-i	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	8.0	Edge	
E	TMVW-i	MT20	4.0	4.0		
F	TS-i		3.0	6.0		
G	TMVW-w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge	
I	TMVW-i	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	1.75	2.75
L	BMV1-p	MT20	3.0	4.0		
M	BMVW-i	MT20	5.0	6.0	2.50	2.75
N	BMVW-i	MT20	4.0	4.0		
O	BS-i	MT20	3.0	6.0		
P	BMVW-i	MT20	4.0	4.0		
Q	BMVW-i	MT20	4.0	4.0		
R	BS-i	MT20	3.0	6.0		
S	BMVW-i	MT20	4.0	4.0		
T	BMVW-i	MT20	5.0	8.0	2.50	2.75
U	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	GROSS REACTION	DOWN	HORIZ		
JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT	BRG
U	2085	0	2085	0	0	5-8
L	2085	0	2085	0	0	5-8

UNFACTORED REACTIONS

JT	1ST LOASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
U	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
L	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	T-C	-388 / 0	0.11 (1)	
B-C	-2270 / 0	-91.8	-91.8	0.37 (1)	4.19	C-S	-79 / 0	0.05 (1)	
C-D	-2254 / 0	-91.8	-91.8	0.36 (1)	4.21	S-D	0 / 156	0.04 (4)	
D-E	-2485 / 0	-91.8	-91.8	0.64 (1)	3.72	D-Q	0 / 949	0.21 (1)	
E-F	-2483 / 0	-91.8	-91.8	0.64 (1)	3.72	Q-E	-597 / 0	0.52 (1)	
F-G	-2483 / 0	-91.8	-91.8	0.64 (1)	3.72	E-P	-2 / 0	0.00 (1)	
G-H	-2483 / 0	-91.8	-91.8	0.63 (1)	3.74	P-G	-588 / 0	0.52 (1)	
H-I	-2254 / 0	-91.8	-91.8	0.36 (1)	4.21	P-H	0 / 947	0.21 (1)	
I-J	-2289 / 0	-91.8	-91.8	0.37 (1)	4.19	N-H	0 / 157	0.04 (4)	
J-K	0 / 35	-91.8	-91.8	0.12 (1)	10.00	N-I	-78 / 0	0.05 (1)	
U-B	-2027 / 0	0.0	0.0	0.21 (1)	5.94	M-I	-397 / 0	0.11 (1)	
L-J	-2028 / 0	0.0	0.0	0.21 (1)	5.94	B-T	0 / 1966	0.44 (1)	
						M-J	0 / 1965	0.44 (1)	
U-T	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
T-S	0 / 1909	-18.5	-18.5	0.37 (1)	10.00				
S-R	0 / 1854	-18.5	-18.5	0.36 (1)	10.00				
R-Q	0 / 1854	-18.5	-18.5	0.36 (1)	10.00				
Q-P	0 / 2485	-18.5	-18.5	0.47 (1)	10.00				
P-O	0 / 1854	-18.5	-18.5	0.36 (1)	10.00				
O-N	0 / 1854	-18.5	-18.5	0.36 (1)	10.00				
N-M	0 / 1908	-18.5	-18.5	0.37 (1)	10.00				
M-L	0 / 0	-18.5	-18.5	0.08 (4)	10.00				

TOTAL WEIGHT = 2 X 155 = 309 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 8.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.64/1.00 (D-E:1), BC=0.47/1.00 (P-Q:1),
WB=0.52/1.00 (E-Q:1), SS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

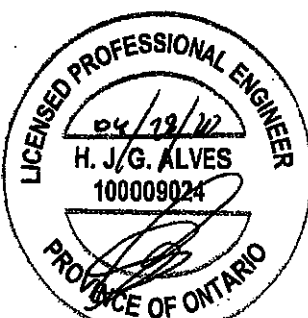
NAIL VALUES

PLATE GRP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618	354
	1697	788
	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)
JSI METAL= 0.62 (O) (INPUT = 1.00)

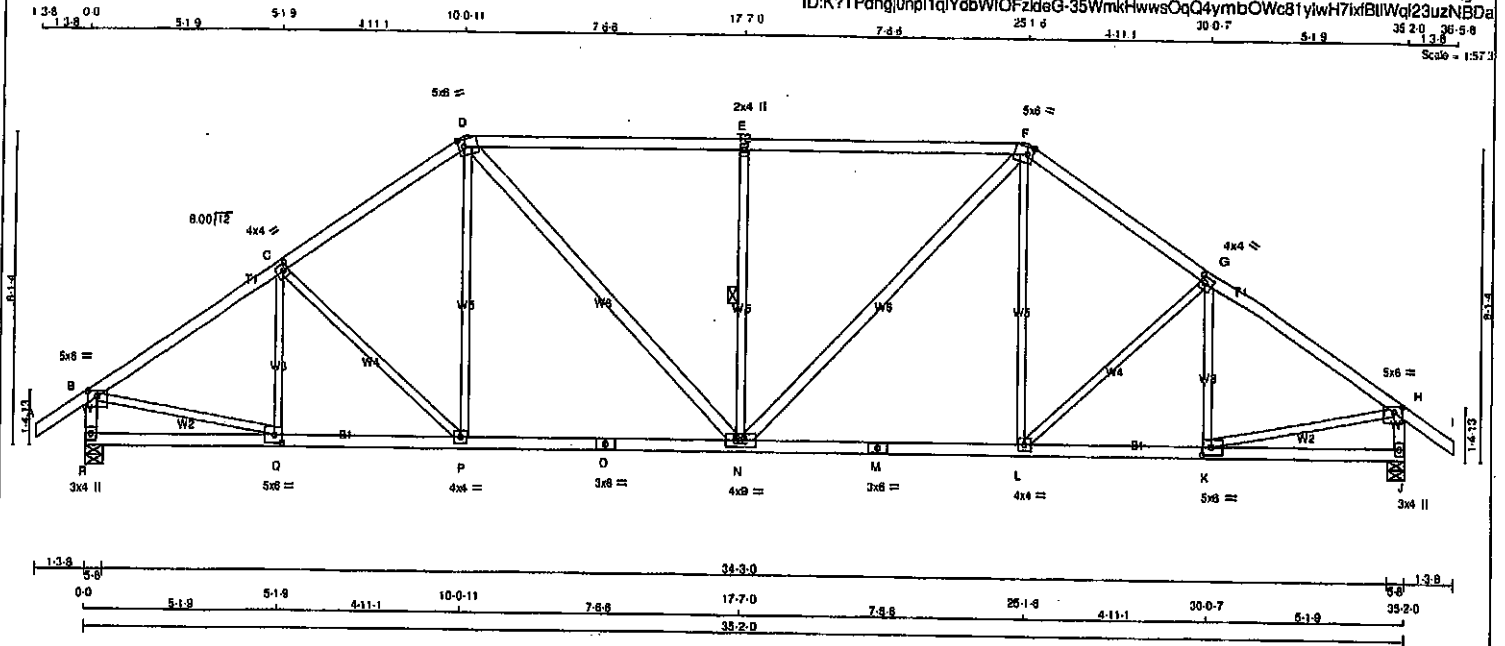


Structural component only
DWG# T-2007149

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408169	T24	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:K?TPdhgJnpl1qYdbWIOFzIdeG-35WmkHwswCqQ4ymbOWc81yW7H7dBIWqI23uzNBDe



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWW-p	MT20	5.0	6.0	1.75 2.75
C	TMWW-i	MT20	4.0	4.0	2.00 1.50
D	TTWW-m	MT20	5.0	6.0	2.25 1.75
E	TMWW-w	MT20	2.0	4.0	
F	TTWW-m	MT20	5.0	6.0	2.25 1.75
G	TMWW-i	MT20	4.0	4.0	2.00 1.50
H	TMWW-p	MT20	5.0	6.0	1.75 2.75
J	BMV1-p	MT20	3.0	4.0	
K	BMWW-i	MT20	5.0	6.0	2.50 2.75
L	BMWW-i	MT20	4.0	4.0	
M	BS-1	MT20	3.0	6.0	
N	BMWWWW-i	MT20	4.0	9.0	
O	BS-1	MT20	3.0	6.0	
P	BMWW-i	MT20	4.0	4.0	
Q	BMWW-i	MT20	5.0	6.0	2.50 2.75
R	BMV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
R	2085	0	2085	0	5-8
J	2085	0	2085	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
J	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	MAX	LC1 (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 35	-91.8	-91.8 0.12 (1)	Q-C	-321 / 0	0.11 (1)	
B-C	-2318 / 0	-91.8	-91.8 0.38 (1)	C-P	-218 / 0	0.18 (1)	
C-D	-2188 / 0	-91.8	-91.8 0.37 (1)	P-D	0 / 268	0.08 (1)	
D-E	-2274 / 0	-91.8	-91.8 0.81 (1)	D-N	0 / 891	0.11 (1)	
E-F	-2274 / 0	-91.8	-91.8 0.81 (1)	N-E	-851 / 0	0.35 (1)	
F-G	-2185 / 0	-91.8	-91.8 0.37 (1)	N-F	0 / 891	0.11 (1)	
G-H	-2317 / 0	-91.8	-91.8 0.38 (1)	L-F	0 / 268	0.08 (1)	
H-I	0 / 35	-91.8	-91.8 0.12 (1)	L-G	-218 / 0	0.18 (1)	
R-B	-2022 / 0	0.0	0.0 0.21 (1)	K-G	-321 / 0	0.11 (1)	
J-H	-2022 / 0	0.0	0.0 0.21 (1)	B-Q	0 / 1995	0.45 (1)	
				K-H	0 / 1995	0.45 (1)	
R-Q	0 / 0	-18.5	-18.5 0.10 (4)				
Q-P	0 / 1953	-18.5	-18.5 0.40 (1)				
P-O	0 / 1794	-18.5	-18.5 0.41 (1)				
O-N	0 / 1794	-18.5	-18.5 0.41 (1)				
N-M	0 / 1794	-18.5	-18.5 0.41 (1)				
M-L	0 / 1794	-18.5	-18.5 0.41 (1)				
L-K	0 / 1953	-18.5	-18.5 0.40 (1)				
K-J	0 / 0	-18.5	-18.5 0.10 (4)				

TOTAL WEIGHT = 2 X 158 = 317 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.8 PSF
DL	= 8.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2010, NBCO 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN. LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL)	= L/360 (1.17")
CALCULATED VERT. DEFL. (LL)	= L/999 (0.11")
ALLOWABLE DEFL. (TL)	= L/360 (1.17")
CALCULATED VERT. DEFL. (TL)	= L/999 (0.22")

CSI: TC=0.81/1.00 (D-E-I), EC=0.41/1.00 (N-P-I), WB=0.48/1.00 (B-Q-I), SS=0.34/1.00 (D-E-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

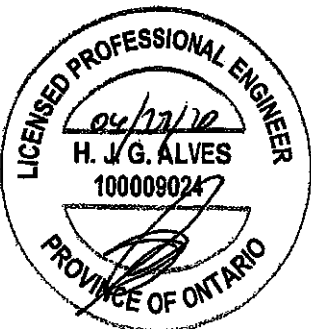
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	818	354	1867 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.80)
JSI METAL= 0.84 (M) (INPUT = 1.00)



Structural component only
DWG# T-2007150

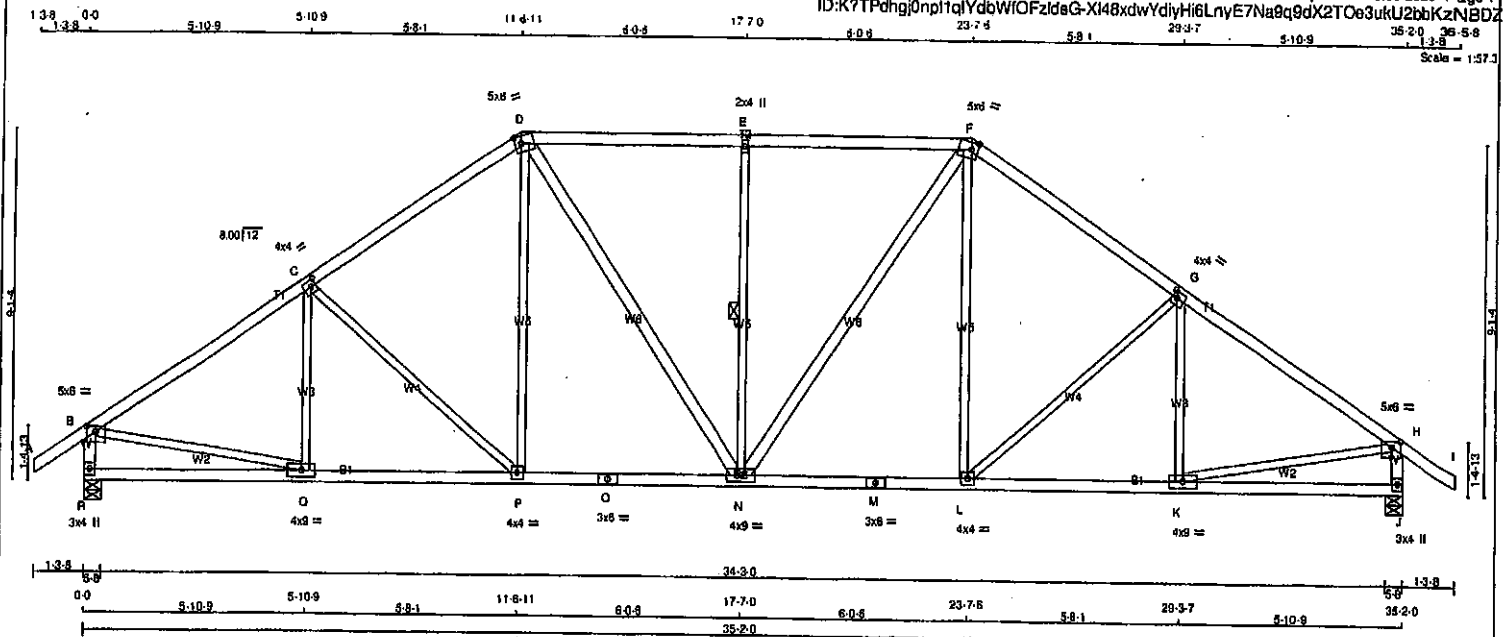
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408169	T25	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:K7TPdhgi0npl1q1Yd0WIOF2IdaG-Xi48xdwYdiYHi6LnY7Na9q9dX2TOe3utU2bbKzNBDZ

Scale = 1:27.2



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES	A - D	2x4	DRY	No.2
	D - F	2x4	DRY	No.2
	F - I	2x4	DRY	No.2
	R - B	2x4	DRY	No.2
	J - H	2x4	DRY	No.2
	R - O	2x4	DRY	No.2
	O - M	2x4	DRY	No.2
	M - J	2x4	DRY	No.2
	ALL WEBS	2x3	DRY	No.2
	EXCEPT			
	D - N	2x4	DRY	No.2
	N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	2.75
C	TMVW-i	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-i	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	1.75	2.75
J	BMV1-p	MT20	3.0	4.0		
K	BMVW-i	MT20	4.0	9.0		
L	BMVW-i	MT20	4.0	4.0		
M	BS-i	MT20	3.0	6.0		
N	BMVWW-i	MT20	4.0	9.0		
O	BS-i	MT20	3.0	6.0		
P	BMVW-i	MT20	4.0	4.0		
Q	BMVW-i	MT20	4.0	9.0		
R	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UPLIFT	IN-SX
R	2065	0	0	5-8
J	2065	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
J	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 35	-91.8 -91.8 0.12 (1)	Q-C	-248 / 11
B-C	-2347 / 0	-91.8 -91.8 0.52 (1)	C-P	-358 / 0
C-D	-2098 / 0	-91.8 -91.8 0.48 (1)	P-D	0 / 342
D-E	-1985 / 0	-91.8 -91.8 0.49 (1)	D-N	0 / 477
E-F	-1985 / 0	-91.8 -91.8 0.49 (1)	N-E	-677 / 0
F-G	-2098 / 0	-91.8 -91.8 0.48 (1)	N-F	0 / 477
G-H	-2348 / 0	-91.8 -91.8 0.52 (1)	L-F	0 / 341
H-I	0 / 35	-91.8 -91.8 0.12 (1)	L-G	-358 / 0
R-B	-2018 / 0	0.0 0.0 0.21 (1)	K-G	-249 / 11
J-H	-2018 / 0	0.0 0.0 0.21 (1)	B-Q	0 / 2014
R-Q	0 / 0	-18.5 -18.6 0.14 (4)	K-H	0 / 2014
Q-P	0 / 1981	-18.5 -18.6 0.39 (1)		
P-O	0 / 1716	-18.5 -18.6 0.34 (1)		
O-N	0 / 1716	-18.5 -18.6 0.34 (1)		
N-M	0 / 1716	-18.5 -18.6 0.34 (1)		
M-L	0 / 1716	-18.5 -18.6 0.34 (1)		
L-K	0 / 1980	-18.5 -18.6 0.39 (1)		
K-J	0 / 0	-18.5 -18.6 0.14 (4)		

TOTAL WEIGHT = 2 X 165 = 329 lb

[M][P]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.52/1.00 (B-C:1), BC=0.39/1.00 (P-Q:1),
WB=0.45/1.00 (B-Q:1), SS=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSD) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.80)
JSI METAL= 0.56 (M) (INPUT = 1.00)



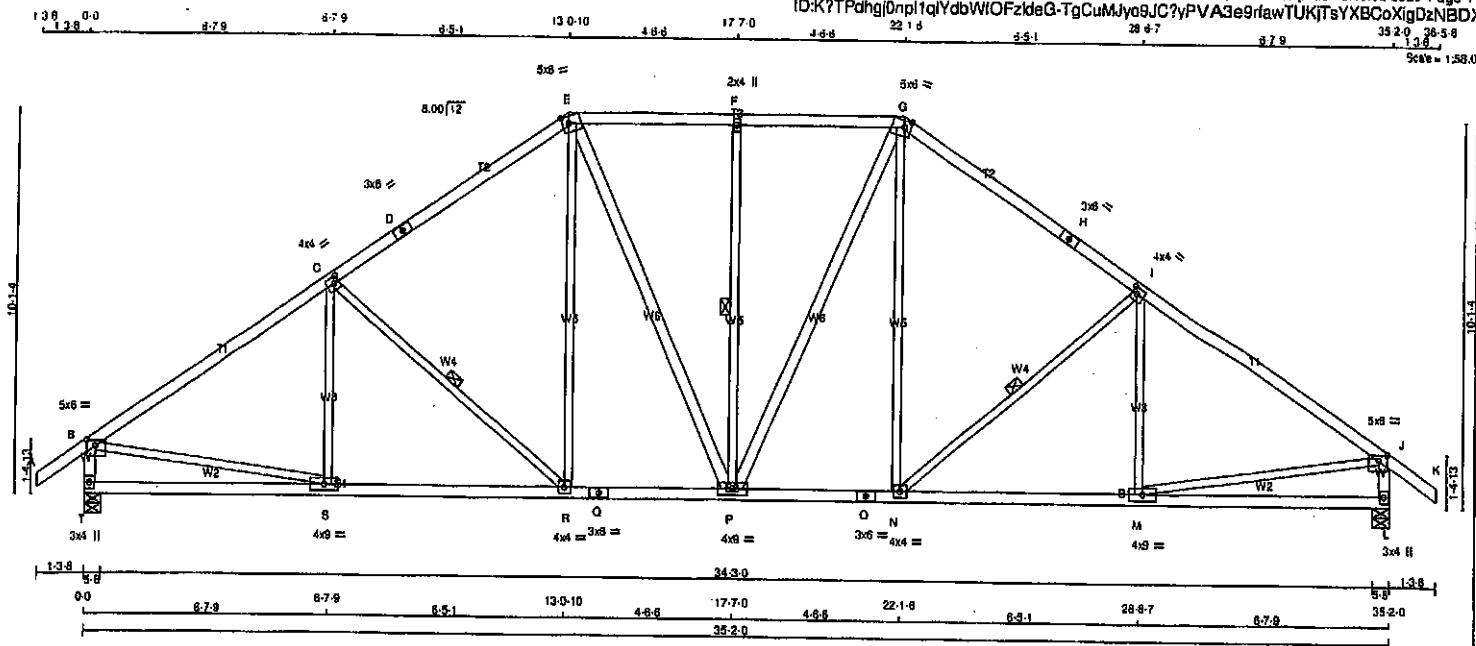
Structural component only
DWG# T-2007151

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	T26	5	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:500.0



TOTAL WEIGHT = 5 X 172 = 858 lb

N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
T - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
T - Q	2x4	DRY	No.2
Q - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
E - P	2x4	DRY	No.2
P - G	2x4	DRY	No.2

DRY, SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	8.0 1.75 2.75
C	TMVW-t	MT20	4.0	4.0 2.00 1.50
D	TS-t	MT20	3.0	8.0
E	TTVW-m	MT20	5.0	8.0 2.25 2.00
F	TMVW-w	MT20	2.0	4.0
G	TTVW-n	MT20	5.0	8.0 2.25 2.00
H	TS-t	MT20	3.0	8.0
I	TMVW-t	MT20	4.0	4.0 2.00 1.50
J	TMVW-p	MT20	5.0	8.0 1.75 2.75
L	BMV1-p	MT20	3.0	4.0
M	BMVW-t	MT20	4.0	9.0
N	BMVW-t	MT20	4.0	4.0
O	BS-t	MT20	3.0	6.0
P	BMVW-t	MT20	4.0	9.0
Q	BS-t	MT20	3.0	6.0
R	BMVW-t	MT20	4.0	4.0
S	BMVW-t	MT20	4.0	9.0
T	BMV1-p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
T	2065	0	2065	0	0	5-8	5-8	5-8	5-8
L	2065	0	2065	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	DEAD	SOIL
T	1458	970/0	0/0	0/0	0/0	488/0	0/0	488/0	0/0
L	1458	970/0	0/0	0/0	0/0	488/0	0/0	488/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-R, F-P, IN.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LCI MAX	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0/35	-91.8 -91.8 0.12 (1)	10.00	S-C	-169/46	0.10 (1)	
B-C	-2359/0	-91.8 -91.8 0.69 (1)	3.77	C-R	-485/0	0.24 (1)	
C-D	-1996/0	-91.8 -91.8 0.62 (1)	4.11	R-E	0/416	0.09 (1)	
D-E	-1996/0	-91.8 -91.8 0.62 (1)	4.11	E-P	0/315	0.06 (1)	
E-F	-1781/0	-91.8 -91.8 0.27 (1)	4.77	P-G	-502/0	0.35 (1)	
F-G	-1781/0	-91.8 -91.8 0.27 (1)	4.77	G-H	0/315	0.06 (1)	
G-H	-1996/0	-91.8 -91.8 0.62 (1)	4.11	H-I	0/415	0.09 (1)	
H-I	-1996/0	-91.8 -91.8 0.62 (1)	4.11	I-J	-494/0	0.24 (1)	
I-J	-2358/0	-91.8 -91.8 0.69 (1)	3.77	J-K	-189/46	0.10 (1)	
J-K	0/35	-91.8 -91.8 0.12 (1)	10.00	K-L	0/2022	0.45 (1)	
T-B	-2014/0	0.0 0.0 0.21 (1)	5.95	L-J	0/2021	0.45 (1)	
L-J	-2014/0	0.0 0.0 0.21 (1)	5.95				

T-S	0/0	-18.5 -18.5 0.19 (4)	10.00
S-R	0/1996	-18.5 -18.5 0.42 (1)	10.00
R-Q	0/1629	-18.5 -18.5 0.32 (1)	10.00
Q-P	0/1629	-18.5 -18.5 0.32 (1)	10.00
P-O	0/1629	-18.5 -18.5 0.32 (1)	10.00
O-N	0/1629	-18.5 -18.5 0.32 (1)	10.00
N-M	0/1996	-18.5 -18.5 0.42 (1)	10.00
M-L	0/0	-18.5 -18.5 0.19 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.17')
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.09')
ALLOWABLE DEFL. (TL) = $L/360$ (1.17')
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.18')

CSI: TC=0.89/1.00 (B-C:1), BC=0.42/1.00 (R-S:1), WB=0.45/1.00 (B-S:1), SS=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

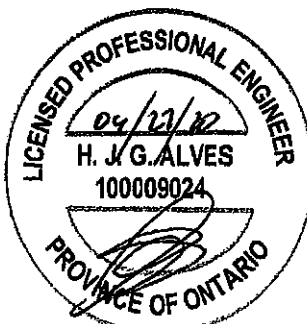
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687 788 1967 1858

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

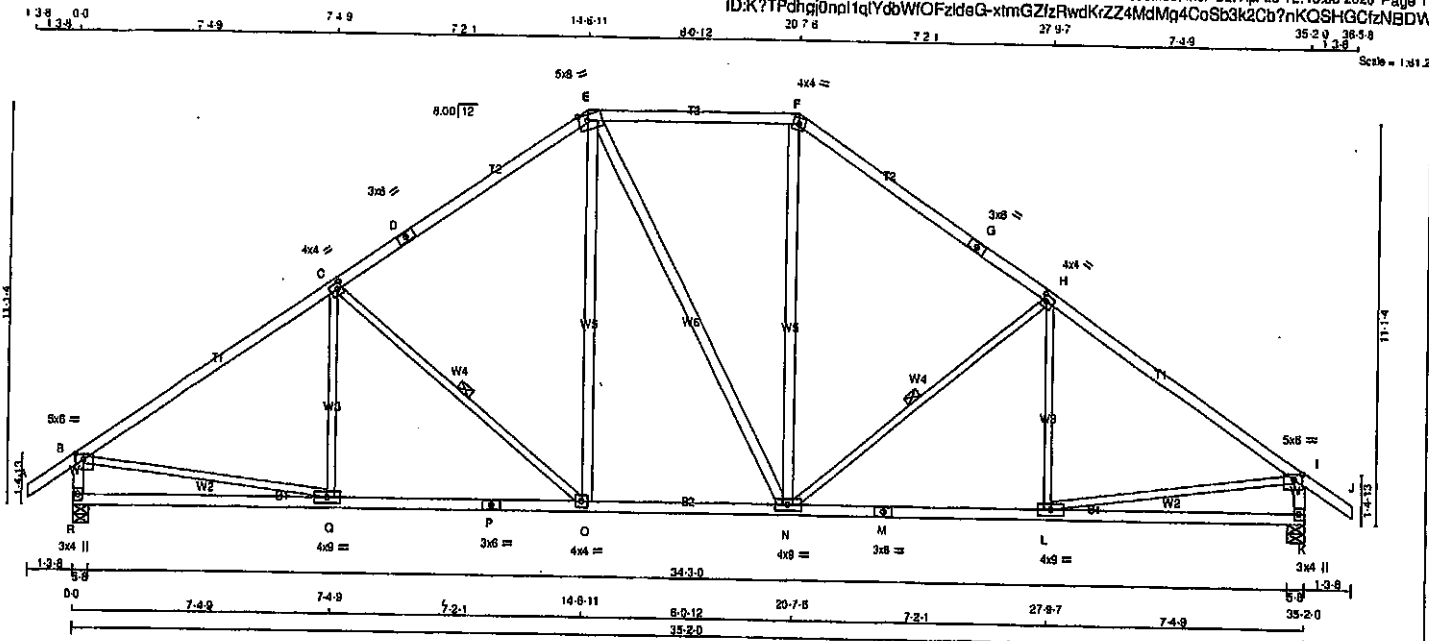
JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.50 (O) (INPUT = 1.00)



Structural component only
DWG# T-2007152

JOB NAME 408169	TRUSS NAME T27	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - E	2x4	DRY	No.2	SPF
E - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TMVW-i	MT20	4.0	4.0	2.00	1.50
D	TS-i	MT20	3.0	8.0		
E	TTVW-m	MT20	5.0	8.0	2.00	3.00
F	TTVW-m	MT20	4.0	4.0		
G	TS-i	MT20	3.0	8.0		
H	TMVW-i	MT20	4.0	4.0	2.00	1.50
I	TMVW-p	MT20	5.0	6.0	1.75	2.75
K	BMV1-p	MT20	3.0	4.0		
L	BMVW-i	MT20	4.0	9.0		
M	BS-i	MT20	3.0	6.0		
N	BMVW-i	MT20	4.0	9.0		
O	BMVW-i	MT20	4.0	4.0		
P	BS-i	MT20	3.0	6.0		
Q	BMVW-i	MT20	4.0	9.0		
R	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UPLIFT		
R	2065	0	2065	0	5-8	5-8
K	2065	0	2065	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
K	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED				MEMB.	MAX. FACTORED FORCE (LBS)	MAX	
		VERT.	LOAD	LC1	MAX.			CS1 (LC)	
		(PLF)			UNBRAC				
FR-TO		FROM	TO	CS1 (LC)	LENGTH	FR-TO			
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	Q-C	-139 / 78	0.09 (1)	
B-C	-2359 / 0	-91.8	-91.8	0.89 (1)	3.45	C-O	-805 / 0	0.38 (1)	
C-D	-1888 / 0	-91.8	-91.8	0.78 (1)	3.91	O-E	0 / 499	0.08 (1)	
D-E	-1888 / 0	-91.8	-91.8	0.78 (1)	3.91	E-N	0 / 0	0.00 (1)	
E-F	-1538 / 0	-91.8	-91.8	0.47 (1)	4.74	N-F	0 / 500	0.08 (1)	
F-G	-1888 / 0	-91.8	-91.8	0.78 (1)	3.91	N-H	-804 / 0	0.38 (1)	
G-H	-1888 / 0	-91.8	-91.8	0.78 (1)	3.91	L-H	-140 / 75	0.09 (1)	
H-I	-2357 / 0	-91.8	-91.8	0.89 (1)	3.45	B-Q	0 / 2020	0.45 (1)	
I-J	0 / 35	-91.8	-91.8	0.12 (1)	10.00	L-I	0 / 2019	0.45 (1)	
R-B	-2009 / 0	0.0	0.0	0.21 (1)	5.95				
K-I	-2009 / 0	0.0	0.0	0.21 (1)	5.95				
R-Q	0 / 0	-18.5	-18.5	0.25 (4)	10.00				
Q-P	0 / 1999	-18.5	-18.5	0.45 (1)	10.00				
P-O	0 / 1999	-18.5	-18.5	0.45 (1)	10.00				
O-N	0 / 1538	-18.5	-18.5	0.33 (1)	10.00				
N-M	0 / 1998	-18.5	-18.5	0.45 (1)	10.00				
M-L	0 / 1998	-18.5	-18.5	0.45 (1)	10.00				
L-K	0 / 0	-18.5	-18.5	0.24 (4)	10.00				

TOTAL WEIGHT = 2 X 166 = 333 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CS1: TC=0.89/1.00 (B-C:1), BC=0.45/1.00 (O-Q:1), WB=0.45/1.00 (B-Q:1), SS=0.27/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MAX	MIN	MAX MIN
MT20	618	354 1987 788 1987 1655

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.80 (F) (INPUT = 0.80)
JSI METAL = 0.68 (P) (INPUT = 1.00)

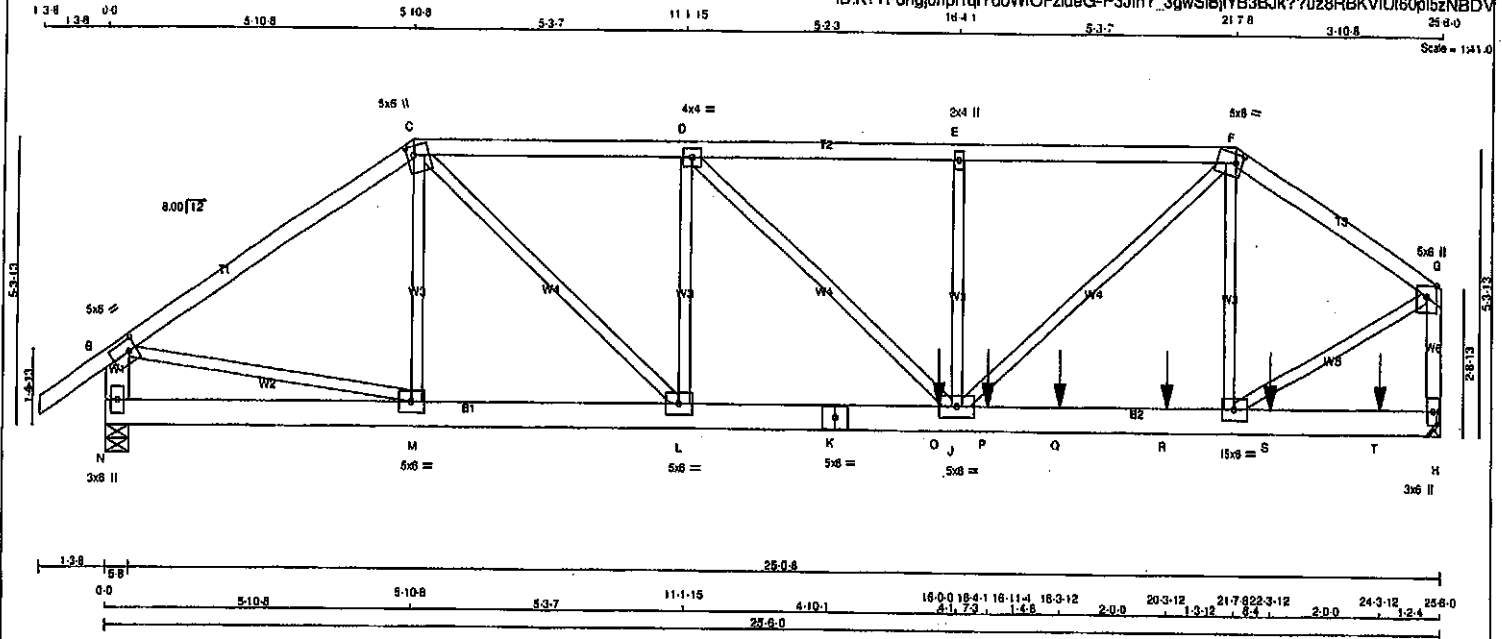


Structural component only
DWG# T-2007153

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408169	T28	1	2	TRUSS DESC.		

Terriarack Roof Truss, Burlington

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CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
N - B	2x6	DRY	No.2
H - G	2x4	DRY	No.2
N - K	2x6	DRY	No.2
K - H	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (#)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - G	1	12
C - F	1	12
F - G	1	12
H - G	1	12
N - B	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
N - K	2	12
K - H	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
E - J	1	6
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
N	2086	0	0	5-8
H	2756	0	2756	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN.	COMPONENT REACTIONS
JT	COMBINED	SNOW
N	1470	894 / 0
H	1941	1320 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.78 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)
FR-TO									
A-B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	M-C	-218 / 22	0.04 (1)	
B-C	-2363 / 0	-91.8	-91.8	0.37 (1)	5.41	C-L	0 / 1550	0.19 (1)	
C-D	-3093 / 0	-91.8	-91.8	0.24 (1)	5.03	L-D	-928 / 0	0.19 (1)	
D-E	-3519 / 0	-91.8	-91.8	0.25 (1)	4.78	D-J	0 / 589	0.07 (1)	
E-F	-3519 / 0	-91.8	-91.8	0.24 (1)	4.78	J-E	-508 / 0	0.10 (1)	
F-G	-2352 / 0	-91.8	-91.8	0.18 (1)	5.69	J-F	0 / 2156	0.27 (1)	
N-B	-2037 / 0	0.0	0.0	0.07 (1)	7.81	I-F	-548 / 0	0.11 (1)	
H-G	-2632 / 0	0.0	0.0	0.19 (1)	7.01	B-M	0 / 1991	0.25 (1)	
						I-G	0 / 2284	0.28 (1)	
N-M	0 / 0	-18.5	-18.5	0.04 (4)	10.00				
M-L	0 / 1959	-18.5	-18.5	0.14 (1)	10.00				
L-K	0 / 3093	-18.5	-18.5	0.27 (1)	10.00				
K-O	0 / 3093	-18.5	-18.5	0.27 (1)	10.00				
O-J	0 / 3093	-18.5	-18.5	0.27 (1)	10.00				
J-P	0 / 1842	-18.5	-18.5	0.23 (1)	10.00				
P-Q	0 / 1842	-18.5	-18.5	0.23 (1)	10.00				
Q-R	0 / 1842	-18.5	-18.5	0.23 (1)	10.00				
R-I	0 / 1842	-18.5	-18.5	0.23 (1)	10.00				
I-S	0 / 0	-18.5	-18.5	0.10 (1)	10.00				
S-T	0 / 0	-18.5	-18.5	0.10 (1)	10.00				
T-H	0 / 0	-18.5	-18.5	0.10 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	16-0-0	-997	-997	---	FRONT	VERT	TOTAL	---	C1
O	16-11-4	-187	-187	---	FRONT	VERT	TOTAL	---	C1
Q	18-3-12	-187	-187	---	FRONT	VERT	TOTAL	---	C1
R	20-3-12	-187	-187	---	FRONT	VERT	TOTAL	---	C1
S	22-3-12	-174	-174	---	FRONT	VERT	TOTAL	---	C1
T	24-3-12	-173	-173	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 122 = 245 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.37/1.00 (B-C:1), BC=0.27/1.00 (J-L:1), WB=0.28/1.00 (G-I:1), SS=0.34/1.00 (J-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1857	788 1887 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.45 (G) (INPUT = 1.00)



Structural component only
DWG# T-2007154 1/2

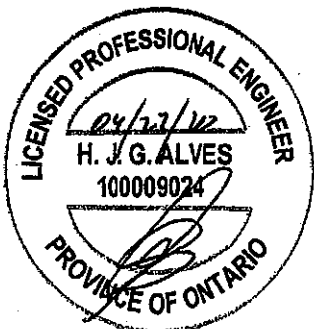
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	T28	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Sat Apr 26 12:49:34 2020 Page 2
ID:K?TPdhaQnpl1qYdbWfOFzdeG-P3J/n? 3qwSiBiyB3Bjk??uz8RBKVIU60pl5zNEDV

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.75
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	1.50
G	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I, L, M						
I	BMWW-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	8.0	8.0		
K	BS-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



amarack Roof Truss, Burlington

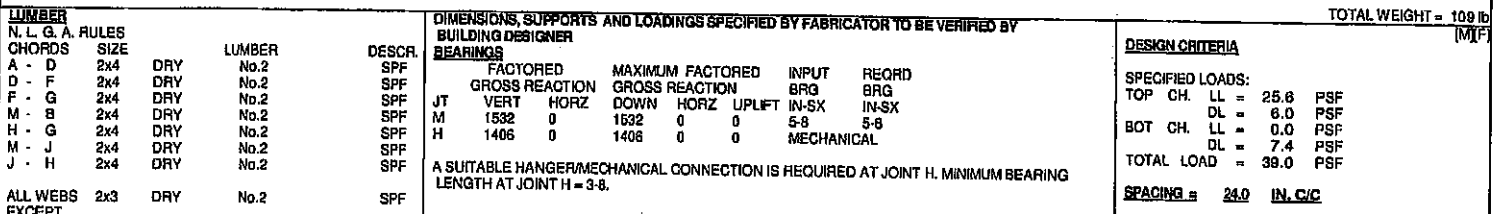
Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Sat Apr 25 12:49:35 2020 Page 1

ID:K?TPdhgJ0np1qYdbWOFzideG-iF11_L_hReAZpIdKlnYHDY1dYm23pNduummMHYZNBDU

13.8 0.0 4.0 7.10.8 5.10.8 13.9 5.10.8 19.7 5.10.8 25.4

13.8 13.8 19.8 3.10.8 5.10.8 19.7 5.10.8 25.4

Scale = 1/4" = 1'-0"



PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV-p	MT20	3.0	4.0	
C	TMWW-I	MT20	5.0	6.0	
D	TMWW-m	MT20	5.0	6.0	2.00 1.50
E	TMWW-s	MT20	2.0	4.0	
F	TMWW-m	MT20	5.0	6.0	2.00 1.50
G	TMWW-p	MT20	5.0	6.0	Edge
H	BMV-I	MT20	3.0	4.0	
I	BMWW-I	MT20	4.0	6.0	
J	BS-I	MT20	3.0	6.0	
K	BMWWWW-I	MT20	4.0	9.0	
L	BMWW-I	MT20	4.0	4.0	
M	BMWW-I	MT20	5.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1532	0	1532	0	0	5-8	5-8
H	1406	0	1408	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3'-8".

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	1081	723 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0
H	994	852 / 0	0 / 0	0 / 0	0 / 0	342 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MEMB.		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	C-L	-80 / 27 0.04 (1)
B-C	0 / 22	-91.8	-91.8	0.22 (1)	10.00	L-D	0 / 209 0.05 (4)
C-D	-1491 / 0	-91.8	-91.8	0.19 (1)	5.19	D-K	0 / 354 0.08 (1)
D-E	-1464 / 0	-91.8	-91.8	0.44 (1)	4.89	K-E	-662 / 0 0.48 (1)
E-F	-1464 / 0	-91.8	-91.8	0.44 (1)	4.89	K-F	0 / 663 0.15 (1)
F-G	-1228 / 0	-91.8	-91.8	0.44 (1)	5.25	I-F	-282 / 0 0.21 (1)
M-B	-268 / 0	0.0	0.0	0.03 (1)	7.81	M-C	-1757 / 0 0.83 (1)
H-G	-1382 / 0	0.0	0.0	0.20 (1)	6.98	I-G	0 / 1098 0.25 (1)
M-L	0 / 1279	-18.5	-18.5	0.36 (4)	10.00		
L-K	0 / 1223	-18.5	-18.5	0.36 (4)	10.00		
K-J	0 / 1014	-18.5	-18.5	0.28 (1)	10.00		
J-I	0 / 1014	-18.5	-18.5	0.28 (1)	10.00		
I-H	0 / 0	-18.5	-18.5	0.17 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:				
TOP CH.	LL =	25.6	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.4	PSF	
TOTAL LOAD =		39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.85")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.85")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.15")

CSI: TC=0.44/1.00 (F-G:1), BC=0.36/1.00 (K-L:4),
WB=0.83/1.00 (C-M:1), SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	818	354	1687	788	1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (D) (INPUT = 0.90)
JSI METAL= 0.51 (G) (INPUT = 1.00)



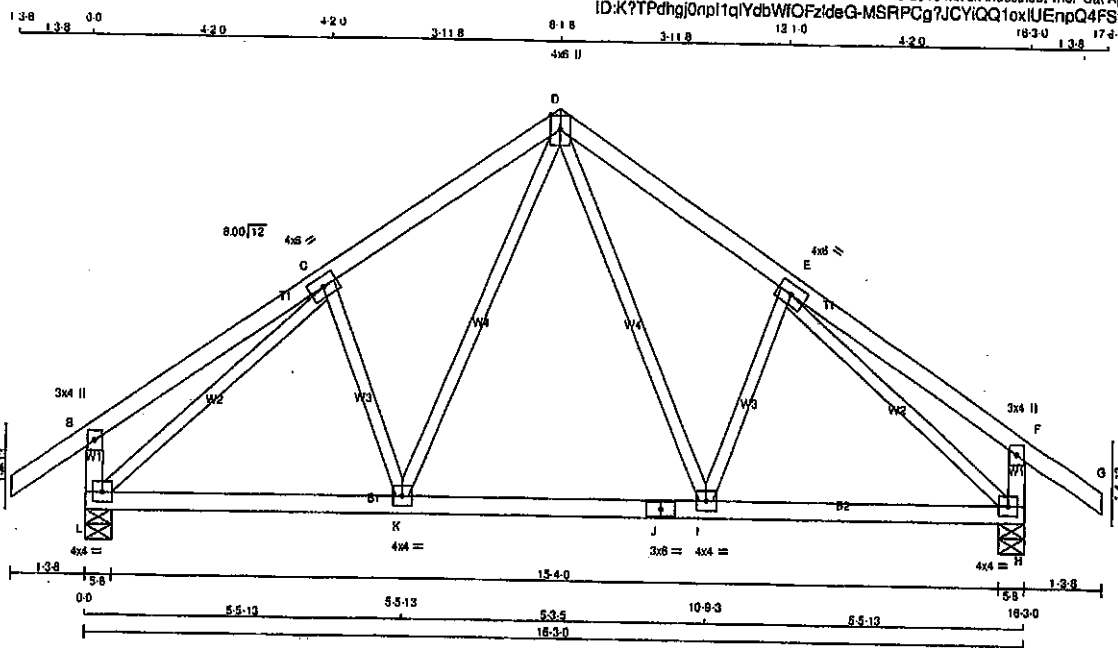
Structural component only
DWG# T-2007155

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408169	T30	2	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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Scale = 1/32.0

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV-p	MT20	3.0	4.0
C	TMVW-1	MT20	4.0	8.0
D	TTWV-p	MT20	4.0	8.0
E	TMVW-1	MT20	4.0	8.0
F	TMV-p	MT20	3.0	4.0
H	BMVW-1	MT20	4.0	4.0
I	BMVW-1	MT20	4.0	4.0
J	BS-1	MT20	3.0	8.0
K	BMVW-1	MT20	4.0	4.0
L	BMVW-1	MT20	4.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	IN-SX	IN-SX
L	1022	0	5-8	5-8
H	1022	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	720	488 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0	0 / 0
H	720	488 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-91.8 -91.8	10.00	D-I	0 / 338	0.08 (1)	
B-C	0 / 24	-91.8 -91.8	10.00	I-E	-217 / 0	0.08 (1)	
C-D	-845 / 0	-91.8 -91.8	8.25	K-D	0 / 338	0.08 (1)	
D-E	-845 / 0	-91.8 -91.8	8.25	C-K	-217 / 0	0.08 (1)	
E-F	0 / 24	-91.8 -91.8	10.00	L-C	-1042 / 0	0.52 (1)	
F-G	0 / 35	-91.8 -91.8	10.00	E-H	-1042 / 0	0.52 (1)	
L-B	-288 / 0	0.0 0.0	7.81				
H-F	-288 / 0	0.0 0.0	7.81				
L-K	0 / 762	-18.5 -18.5	10.00				
K-J	0 / 557	-18.5 -18.5	10.00				
J-I	0 / 557	-18.5 -18.5	10.00				
I-H	0 / 762	-18.5 -18.5	10.00				

TOTAL WEIGHT = 2 X 72 = 144 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.54")
CALCULATED VERT. DEFL. (LL) = L/998 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.54")
CALCULATED VERT. DEFL. (TL) = L/998 (0.04")

OSI: TC=0.24/1.00 (B-C:1), BC=0.19/1.00 (H-I:1),
WB=0.52/1.00 (C-L:1), SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PS)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 788	1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.68 (H) (INPUT = 0.90)
JSI METAL = 0.29 (L) (INPUT = 1.00)



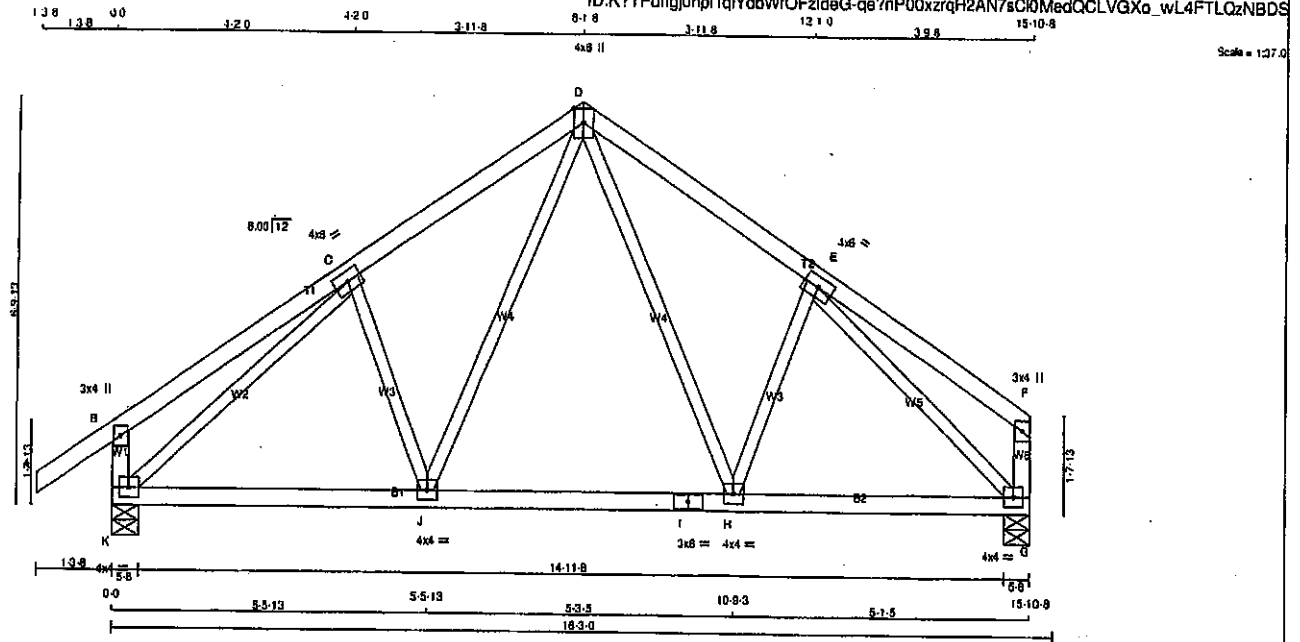
Structural component only
DWG# T-2007156

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408169	T30A	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 70 = 140 lb
(M/F)

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (tablets in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMWW-1	MT20	4.0	6.0
D	TTWW+p	MT20	4.0	6.0
E	TMWW-1	MT20	4.0	6.0
F	TMV+p	MT20	3.0	4.0
G	BMVW-1	MT20	4.0	4.0
H	BMVW-1	MT20	4.0	4.0
I	BS-1	MT20	3.0	6.0
J	BMVW-1	MT20	4.0	4.0
K	BMVW-1	MT20	4.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		RECORD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
K	1001	0	1001	0	0	5-8	5-8	5-8	5-8
G	875	0	875	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL
K	708	476 / 0	0 / 0	0 / 0	0 / 0	0 / 0	229 / 0	0 / 0
G	619	408 / 0	0 / 0	0 / 0	0 / 0	0 / 0	213 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED VERT. LOAD (PL)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PL)	MAX. FACTORED VERT. LOAD (PL)
FR-TO				FR-TO			
A-B	0 / 35	-91.8	-91.8	D-H	0 / 272	0.08 (1)	0.08 (1)
B-C	0 / 24	-91.8	-91.8	H-E	-180 / 2	0.05 (1)	0.05 (1)
C-D	-817 / 0	-91.8	-91.8	J-D	0 / 342	0.08 (1)	0.08 (1)
D-E	-783 / 0	-91.8	-91.8	C-J	-222 / 0	0.08 (1)	0.08 (1)
E-F	0 / 24	-91.8	-91.8	K-C	-1012 / 0	0.51 (1)	0.51 (1)
K-B	-288 / 0	0.0	0.0	E-G	-992 / 0	0.44 (1)	0.44 (1)
G-F	-124 / 0	0.0	0.0				
K-J	0 / 740	-18.5	-18.5				
J-I	0 / 531	-18.5	-18.5				
I-H	0 / 531	-18.5	-18.5				
H-G	0 / 691	-18.5	-18.5				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2018 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.24/1.00 (B-C:1), BC=0.18/1.00 (J-K:1), WB=0.51/1.00 (C-K:1), SS=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

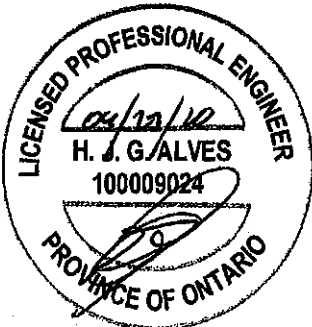
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.28 (K) (INPUT = 1.00)



Structural component only
DWG# T-2007157



JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.35 (H) (INPUT = 1.00)



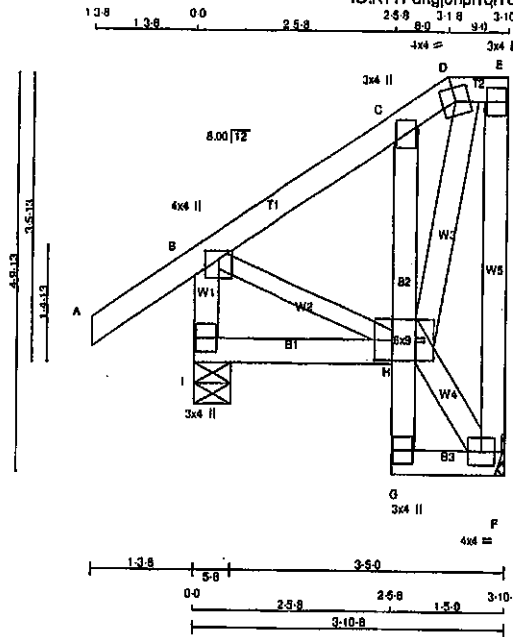
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408169	T31S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:25.8



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
I - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
G - C	2x4	DRY	No.2
G - F	2x4	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			
B - H	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)			
JT	TYPE	PLATES	W LEN Y X
B	TMVW+p	MT20	4.0 4.0 1.25 2.00
C	TMV+p	MT20	3.0 4.0
D	TTW+m	MT20	4.0 4.0
E	TMV+p	MT20	3.0 4.0
F	BMVW1-i	MT20	4.0 4.0
G	BMV+p	MT20	3.0 4.0
H	BVMWVW4	MT20	6.0 6.0 3.00 2.75
I	BMV1+p	MT20	3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UP
F	193	0	193	0
I	361	0	361	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	136	88/0	0/0	0/0	0/0	49/0	0/0
I	252	181/0	0/0	0/0	0/0	72/0	0/0

BEARING MATERIAL TO BE 8PF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.
FR-TO				FR-TO			
A-B	0/35	-91.8	-91.8 0.14 (5)	10.00	H-F	-27/0	0.00 (1)
B-C	-87/0	-91.8	-91.8 0.09 (1)	6.25	B-H	0/68	0.02 (1)
C-D	-89/0	-91.8	-91.8 0.07 (1)	6.25	H-D	0/173	0.03 (1)
D-E	-11/0	-91.8	-91.8 0.07 (1)	6.25			
F-E	-148/0	0.0	0.0 0.04 (1)	7.81			
I-B	-327/0	0.0	0.0 0.02 (4)	7.81			
I-H	0/8	-18.5	-18.5 0.03 (4)	10.00			
G-H	-2/8	0.0	0.0 0.05 (1)	10.00			
H-C	-203/0	0.0	0.0 0.04 (1)	7.81			
G-F	0/30	-18.5	-18.5 0.03 (1)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 29 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.8 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.14/1.00 (A-B:5), BC=0.05/1.00 (G-H:1), WB=0.03/1.00 (D-H:1), SS=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1867 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

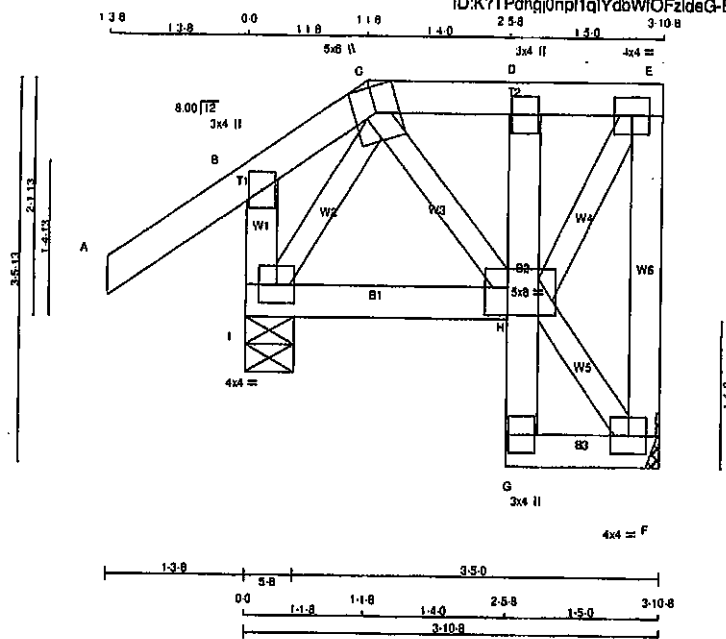


Structural component only
DWG# T-2007159

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408169	T32S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:20.1

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
I - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
G - D	2x4	DRY	No.2
G - F	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMV+p	MT20	3.0	4.0		
E	TMVW-i	MT20	4.0	4.0		
F	BMVW-i	MT20	4.0	4.0		
G	BMV+p	MT20	3.0	4.0		
H	BMVWW-i	MT20	5.0	8.0	3.00	2.50
I	BMVW-i	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
F	191 0	191 0	0 0	MECHANICAL	IN-SX
I	382 0	382 0	0 0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	136	87 / -12	0 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
I	253	182 / 0	0 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 35	-91.8	-91.8	0.14 (5)	10.00	C-H	0 / 80 0.01 (1)
B-C	-41 / 0	-91.8	-91.8	0.13 (5)	6.25	H-F	-7 / 0 0.00 (1)
C-D	-89 / 0	-91.8	-91.8	0.02 (1)	6.25	H-E	0 / 153 0.03 (1)
D-E	-86 / 0	-91.8	-91.8	0.02 (1)	6.25	I-C	-100 / 34 0.02 (1)
F-E	-175 / 0	0.0	0.0	0.03 (1)	7.81		
I-B	-253 / 0	0.0	0.0	0.03 (1)	7.81		
I-H	-18 / 53	-18.5	-18.5	0.04 (4)	6.25		
G-H	0 / 13	0.0	0.0	0.01 (1)	10.00		
H-D	-142 / 0	0.0	0.0	0.01 (1)	7.81		
G-F	0 / 5	-18.5	-18.5	0.01 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 25 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/899 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/899 (0.00")

CSI: TC=0.14/1.00 (A-B:5), BC=0.04/1.00 (H-I:4), WB=0.03/1.00 (E-H:1), SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1687 1688

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

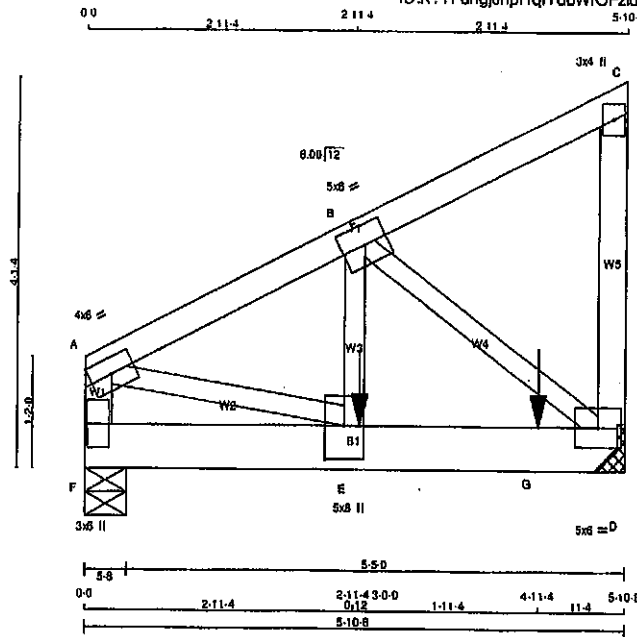


Structural component only
DWG# T-2007160

JOB NAME 408169	TRUSS NAME T33	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
TRUSS DESC.					

Tantarak Roof Truss, Burlington

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Scale = 1:23.3

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x8	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A	12	TOP
A - C	12	TOP
C - D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	9	SIDE(305.2)
WEBS : (0.122"x3") SPIRAL NAILS		
B - E	2	SIDE(408.9)
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	4.0	6.0		Edge
B	TMVW-1	MT20	5.0	6.0		
C	TMV-p	MT20	3.0	4.0		
D	BMVW-1	MT20	5.0	6.0		
E	BMVW-1	MT20	5.0	8.0	4.25	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
F	1914	0	1914	0
D	2859	0	2859	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4'-0.

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1348	915 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0
D	2015	1359 / 0	0 / 0	0 / 0	0 / 0	657 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
F - A	-1877 / 0	0.0	0.0 0.10 (1)	A - E	0 / 2380	0.29 (1)	
A - B	-2688 / 0	-91.8	-91.8 0.08 (1)	E - B	0 / 2608	0.32 (1)	
B - C	-7 / 0	-91.8	-91.8 0.08 (1)	B - D	-2895 / 0	0.35 (1)	
D - C	-120 / 0	0.0	0.0 0.01 (1)				
F - E	0 / 0	-18.5	-18.5 0.01 (1)				
E - G	0 / 2301	-18.5	-18.5 0.49 (1)				
G - D	0 / 2301	-18.5	-18.5 0.49 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-0-0	-2737	-2737	---	BACK	VERT	TOTAL	---	C1
G	4-11-4	-1388	-1388	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.10/1.00 (A-F:1), BC=0.49/1.00 (D-E:1), WB=0.35/1.00 (B-D:1), SS=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (B) (INPUT = 0.80)
JSI METAL = 0.44 (E) (INPUT = 1.00)



Structural component only
DWG# T-2007161 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	T33	1	2	GREEN PARK HOMES	

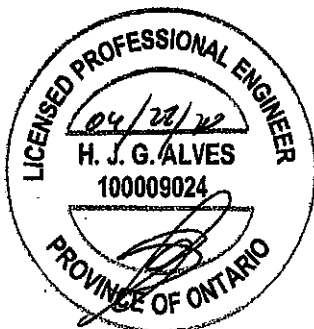
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PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



Structural component only
DWG# T-2007161 42

DWG# T-2007168 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T40	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+	MT20	5.0	8.0		
C	TTWW+m	MT20	5.0	8.0	2.00	2.25
D	TMW+	MT20	5.0	8.0		
E	TMW+	MT20	3.0	8.0		
F	TS+	MT20	5.0	8.0		
G	TMW+	MT20	5.0	8.0		
H	TTWW+m	MT20	5.0	8.0	2.00	2.25
I	TMW+	MT20	5.0	8.0		
K	BMV+	MT20	3.0	8.0		
L	BMW+	MT20	5.0	8.0	2.50	2.50
M	BMW+	MT20	5.0	8.0		
N	BS+	MT20	5.0	8.0		
O	BMW+	MT20	5.0	8.0		
P	BS+	MT20	5.0	8.0		
Q	BMW+	MT20	5.0	8.0		
R	BMW+	MT20	5.0	8.0	2.50	2.50
S	BMV+	MT20	3.0	8.0		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-437	-437	---	FRONT	VERT	TOTAL	---	C1
E	17-7-0	-110	-110	---	FRONT	VERT	TOTAL	---	C1
H	29-3-8	-437	-437	---	FRONT	VERT	TOTAL	---	C1
L	29-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
N	21-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
O	17-7-0	-28	-28	---	FRONT	VERT	TOTAL	---	C1
P	13-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
R	5-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
T	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
U	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	11-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	15-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	19-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	21-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	23-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	25-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	27-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	1-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AE	3-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AF	7-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AG	9-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AH	11-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AI	15-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AJ	19-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AK	23-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AL	25-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AM	27-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AN	31-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AO	33-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

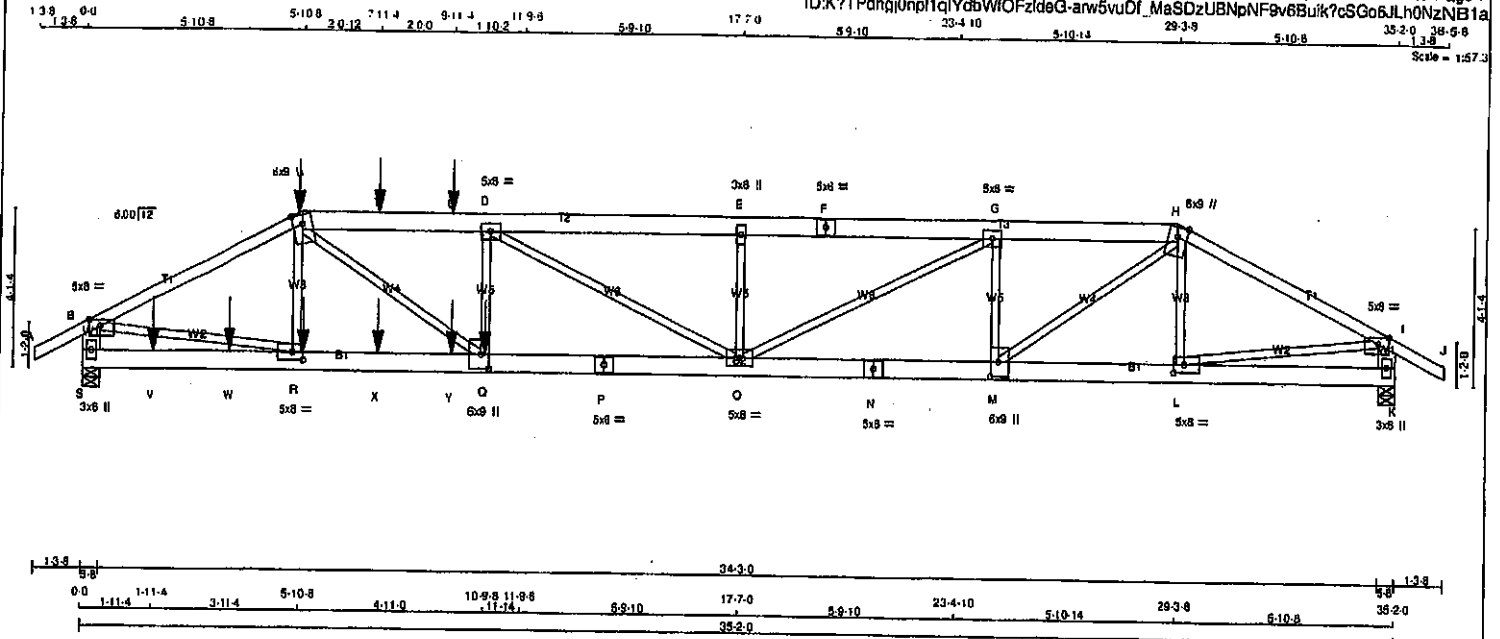
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007168

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T40Z	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x8	DRY	No.2	SPF
F - H	2x8	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x8	DRY	No.2	SPF
K - I	2x8	DRY	No.2	SPF
S - P	2x8	DRY	No.2	SPF
P - N	2x8	DRY	No.2	SPF
N - K	2x8	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE(81.0)
H - J	12	TOP
C - F	12	SIDE(81.0)
F - H	12	TOP
S - B	12	TOP
K - I	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S - P	12	SIDE(183.1)
P - N	12	TOP
N - K	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	SIDE(525.1)
D - Q	3	
G - M	3	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	4582	4582	0	0
S	3047	3047	0	0
K	3047	3047	0	0

UNFACTORED REACTIONS

1ST LOASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	3236	2151 / 0	0 / 0	0 / 0	0 / 0	1084 / 0	0 / 0
K	2151	1438 / 0	0 / 0	0 / 0	0 / 0	715 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
A-B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	R-C	-673 / 0	0.08 (1)	
B-C	-7283 / 0	-91.8	-91.8	0.72 (1)	3.15	C-Q	0 / 4986	0.82 (1)	
C-T	-10549 / 0	-91.8	-91.8	0.34 (1)	3.55	Q-D	-184 / 38	0.02 (1)	
T-U	-10549 / 0	-91.8	-91.8	0.34 (1)	3.55	D-O	-1207 / 0	0.72 (1)	
U-D	-10549 / 0	-91.8	-91.8	0.34 (1)	3.55	O-E	-646 / 0	0.08 (1)	
D-E	-9485 / 0	-91.8	-91.8	0.29 (1)	3.81	E-G	0 / 2871	0.38 (1)	
E-F	-9485 / 0	-91.8	-91.8	0.29 (1)	3.81	M-G	-1984 / 0	0.24 (1)	
F-G	-9485 / 0	-91.8	-91.8	0.17 (1)	4.44	G-H	0 / 3512	0.43 (1)	
G-H	-6958 / 0	-91.8	-91.8	0.48 (1)	4.08	H-I	-464 / 0	0.08 (1)	
H-I	-4608 / 0	-91.8	-91.8	0.16 (1)	6.76	I-J	0 / 8596	0.82 (1)	
I-J	0 / 28	-91.8	-91.8	0.07 (1)	10.00	L-I	0 / 4187	0.52 (1)	
S-B	-4503 / 0	0.0	0.0	0.16 (1)	6.76				
K-I	-2980 / 0	0.0	0.0	0.11 (1)	7.81				
S-V	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
V-W	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
W-R	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
R-X	0 / 8557	-18.5	-18.5	0.50 (1)	10.00				
X-Y	0 / 8557	-18.5	-18.5	0.50 (1)	10.00				
Y-Q	0 / 8557	-18.5	-18.5	0.50 (1)	10.00				
Q-P	0 / 10548	-18.5	-18.5	0.77 (1)	10.00				
P-O	0 / 10548	-18.5	-18.5	0.77 (1)	10.00				
O-N	0 / 8955	-18.5	-18.5	0.50 (1)	10.00				
N-M	0 / 8955	-18.5	-18.5	0.50 (1)	10.00				
M-L	0 / 4144	-18.5	-18.5	0.29 (1)	10.00				
L-K	0 / 0	-18.5	-18.5	0.04 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-437	-437	---	BACK	VERT	TOTAL	C1
Q	10-9-8	-2715	-2715	---	BACK	VERT	TOTAL	C1
R	5-11-4	-26	-26	---	BACK	VERT	TOTAL	C1
T	7-11-4	-110	-110	---	BACK	VERT	TOTAL	C1
U	9-11-4	-110	-110	---	BACK	VERT	TOTAL	C1
V	1-11-4	-25	-25	---	BACK	VERT	TOTAL	C1
W	3-11-4	-26	-26	---	BACK	VERT	TOTAL	C1
X	7-11-4	-26	-26	---	BACK	VERT	TOTAL	C1
Y	9-11-4	-26	-26	---	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 169 = 338 b

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.28")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/881 (0.48")

CSI: TC=0.72/1.00 (B-C:1), BC=0.77/1.00 (O-Q:1),
WB=0.82/1.00 (B-R:1), SS=0.13/1.00 (C-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.99 (F) (INPUT = 1.00)



Structural component only
DWG# T-2007169

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T40Z	1	2	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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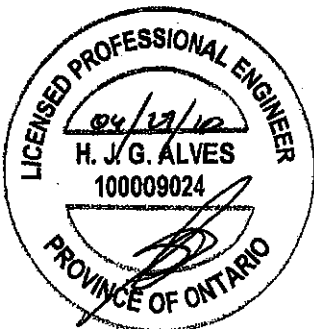
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	2.00	3.50
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMWW-l	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-l	MT20	5.0	6.0		
G	TMWW-l	MT20	5.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	
I	TMVW-p	MT20	5.0	8.0	2.00	3.50
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-l	MT20	6.0	6.0	2.50	3.25
M	BMWW+l	MT20	6.0	6.0	4.50	2.50
N	BS-l	MT20	5.0	6.0		
O	BMWWW-l	MT20	5.0	6.0		
P	BS-l	MT20	5.0	6.0		
Q	BMWW+l	MT20	6.0	6.0	4.50	2.50
R	BMWW-l	MT20	5.0	6.0	2.50	3.25
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



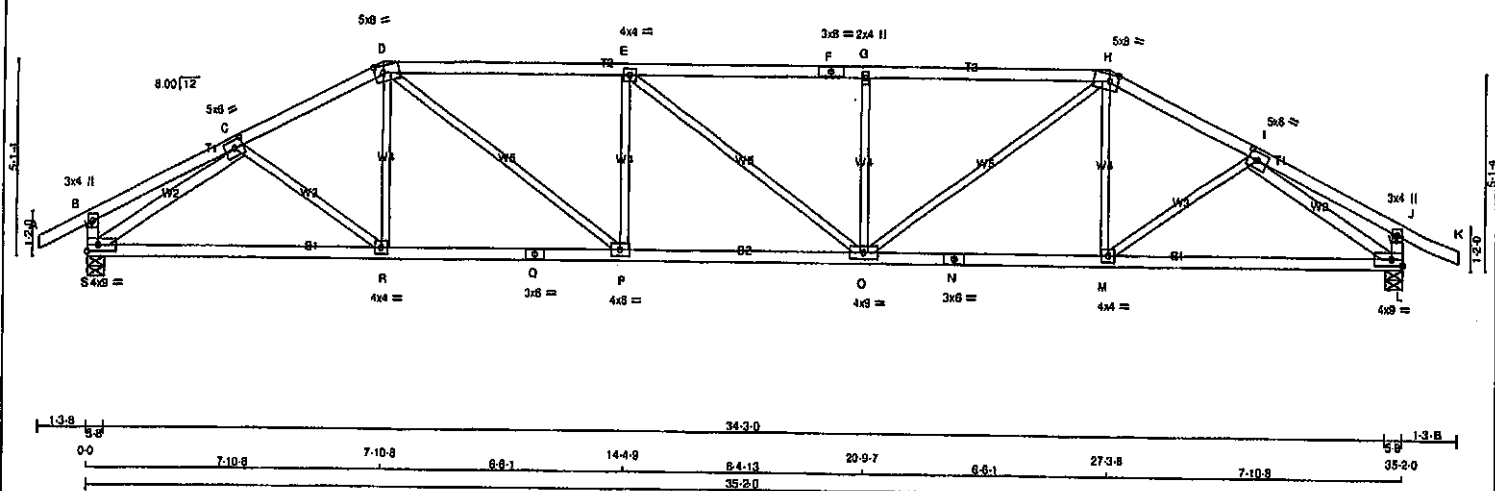
Structural component only
DWG# T-2007169 3/2

Tamarack Roof Truss, Burlington

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Version 8.3.10 © Oct 29 2019 Miller Industries, Inc. Sat Apr 26 13:02:18 2020 Page 1
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Scale = 1:57.2



TOTAL WEIGHT = 2 X 139 = 278 lb

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT SRG	REQ'D BRG		
JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT	IN-SX	IN-SX
S	2083	0	2083	0	0	5-8	5-8
L	2083	0	2083	0	0	5-8	5-8

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1457	989 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
L	1457	989 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S. L.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	C-R	0 / 93	0.03 (4)
B-C	0 / 16	-91.8	-91.8 0.20 (1)	10.00	R-D	0 / 121	0.04 (4)
C-D	-2759 / 0	-91.8	-91.8 0.32 (1)	3.88	D-P	0 / 1267	0.29 (1)
D-E	-3508 / 0	-91.8	-91.8 0.85 (1)	2.94	P-E	-543 / 0	0.25 (1)
E-F	-3507 / 0	-91.8	-91.8 0.93 (1)	2.94	O-E	-2 / 0	0.00 (1)
F-G	-3507 / 0	-91.8	-91.8 0.93 (1)	2.94	O-G	-543 / 0	0.25 (1)
G-H	-3507 / 0	-91.8	-91.8 0.94 (1)	2.95	O-H	0 / 1266	0.28 (1)
H-I	-2997 / 0	-91.8	-91.8 0.32 (1)	3.88	M-H	0 / 121	0.04 (4)
I-J	0 / 16	-91.8	-91.8 0.20 (1)	10.00	M-I	0 / 93	0.03 (4)
J-K	0 / 28	-91.8	-91.8 0.12 (1)	10.00	S-C	-2974 / 0	0.83 (1)
S-B	-270 / 0	0.0	0.0 0.03 (1)	7.81	I-L	-2974 / 0	0.83 (1)
L-J	-270 / 0	0.0	0.0 0.03 (1)	7.81			
S-R	0 / 2417	-18.5	-18.5 0.53 (1)	10.00			
R-Q	0 / 2489	-18.5	-18.5 0.54 (1)	10.00			
Q-P	0 / 2489	-18.5	-18.5 0.54 (1)	10.00			
P-O	0 / 3509	-18.5	-18.5 0.64 (1)	10.00			
O-N	0 / 2489	-18.5	-18.5 0.54 (1)	10.00			
N-M	0 / 2489	-18.5	-18.5 0.54 (1)	10.00			
M-L	0 / 2417	-18.5	-18.5 0.53 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD =		39.0	PSF

SPACING = 24.0 N.C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.41")

CSI: TC=0.95/1.00 (D-E:1), BC=0.64/1.00 (O-P:1),
WB=0.83/1.00 (I-L:1), SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (L) (INPUT = 0.90)
JSI METAL = 0.77 (N) (INPUT = 1.00)

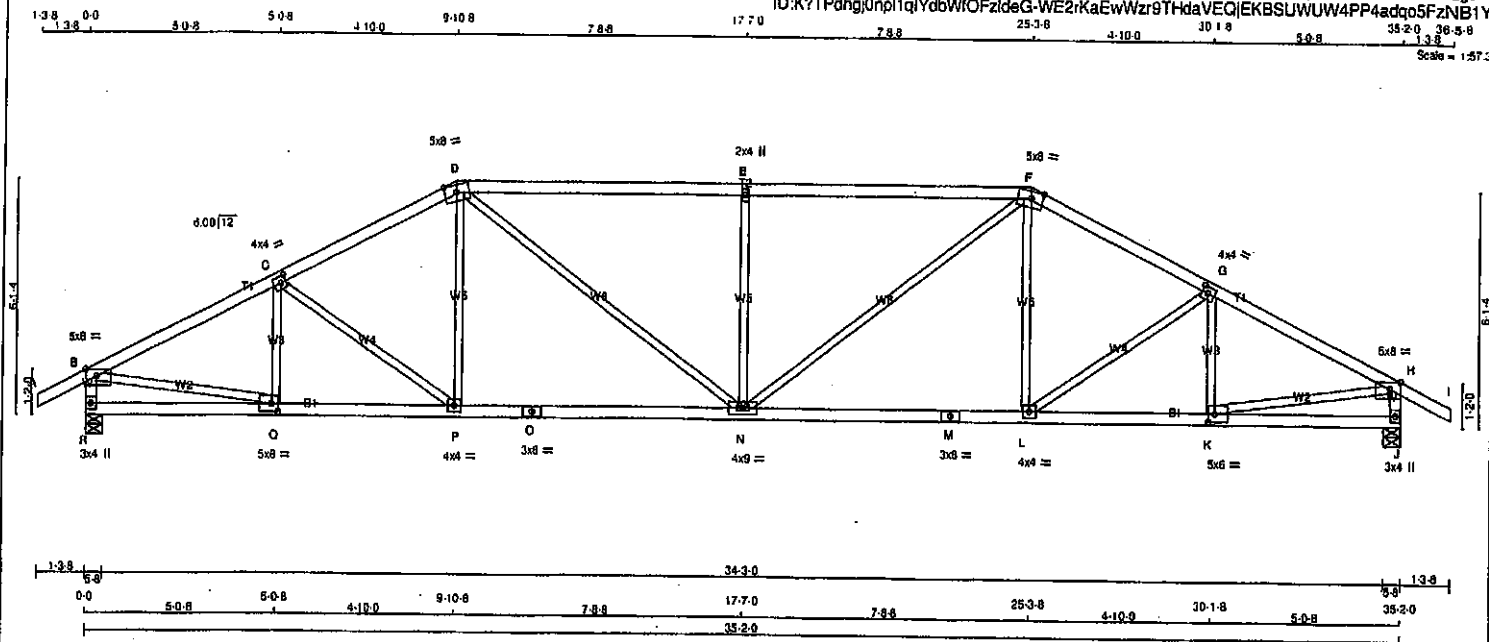


Structural component only
DWG# T-2007170

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408170	T42	2	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR
A - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
F - I	2x4 DRY	No.2	SPF		
R - 8	2x4 DRY	No.2	SPF		
J - H	2x4 DRY	No.2	SPF		
R - O	2x4 DRY	No.2	SPF		
O - M	2x4 DRY	No.2	SPF		
M - J	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50	
C	TMVW-t	MT20	4.0	4.0	2.00	1.75	
D	TTVW-m	MT20	5.0	8.0	2.25	3.75	
E	TMVW-w	MT20	2.0	4.0			
F	TTVW-m	MT20	5.0	8.0	2.25	3.75	
G	TMVW-t	MT20	4.0	4.0	2.00	1.75	
H	TMVW-p	MT20	5.0	8.0	Edge	3.50	
J	BMV1-p	MT20	3.0	4.0			
K	BMVW-t	MT20	5.0	8.0	2.50	2.00	
L	BMVW-t	MT20	4.0	4.0			
M	BS-t	MT20	3.0	6.0			
N	BMVW-t	MT20	4.0	9.0			
O	BS-t	MT20	3.0	6.0			
P	BMVW-t	MT20	4.0	4.0			
Q	BMVW-t	MT20	5.0	6.0	2.50	2.00	
R	BMV1-p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT	HORZ	DOWN	HORZ	UPLIFT
R	2063	0	2063	0
J	2063	0	2063	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1457	889 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
J	1457	889 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	WELLS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO					FR-TO			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	Q-C	-353 / 0	0.08 (1)
B-C	-2834 / 0	-91.8	-91.8	0.39 (1)	3.81	C-P	-210 / 0	0.13 (1)
C-D	-2887 / 0	-91.8	-91.8	0.37 (1)	3.51	P-D	0 / 246	0.08 (4)
D-E	-3080 / 0	-91.8	-91.8	0.97 (1)	2.78	D-N	0 / 844	0.19 (1)
E-F	-3080 / 0	-91.8	-91.8	0.97 (1)	2.78	N-E	-872 / 0	0.51 (1)
F-G	-2887 / 0	-91.8	-91.8	0.37 (1)	3.91	N-F	0 / 844	0.19 (1)
G-H	-2834 / 0	-91.8	-91.8	0.39 (1)	3.81	L-F	0 / 246	0.08 (4)
H-I	0 / 28	-91.8	-91.8	0.12 (1)	10.00	L-G	-210 / 0	0.13 (1)
R-H	-2018 / 0	0.0	0.0	0.20 (1)	5.94	K-G	-353 / 0	0.08 (1)
J-H	-2018 / 0	0.0	0.0	0.20 (1)	5.94	B-Q	0 / 2581	0.58 (1)
R-Q	0 / 0	-18.5	-18.5	0.10 (4)	10.00	K-H	0 / 2581	0.58 (1)
Q-P	0 / 2555	-18.5	-18.5	0.51 (1)	10.00			
P-O	0 / 2388	-18.5	-18.5	0.49 (1)	10.00			
O-N	0 / 2388	-18.5	-18.5	0.49 (1)	10.00			
N-M	0 / 2388	-18.5	-18.5	0.49 (1)	10.00			
M-L	0 / 2388	-18.5	-18.5	0.49 (1)	10.00			
L-K	0 / 2555	-18.5	-18.5	0.51 (1)	10.00			
K-J	0 / 0	-18.5	-18.5	0.10 (4)	10.00			

TOTAL WEIGHT = 2 X 139 = 277 b

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, OBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.34")

CSI: TC=0.97/1.00 (D-E:1), BC=0.51/1.00 (P-Q:1), WB=0.58/1.00 (B-Q:1), SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

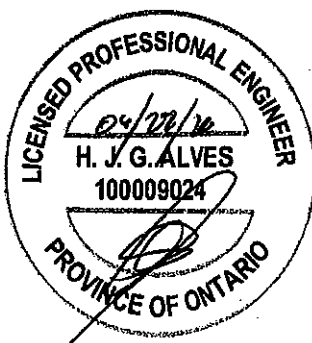
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)	
MAX	MIN	MAX	MIN	MAX
MT20	818	354	1687	788
				1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (F) (INPUT = 0.90)
JSI METAL = 0.75 (M) (INPUT = 1.00)



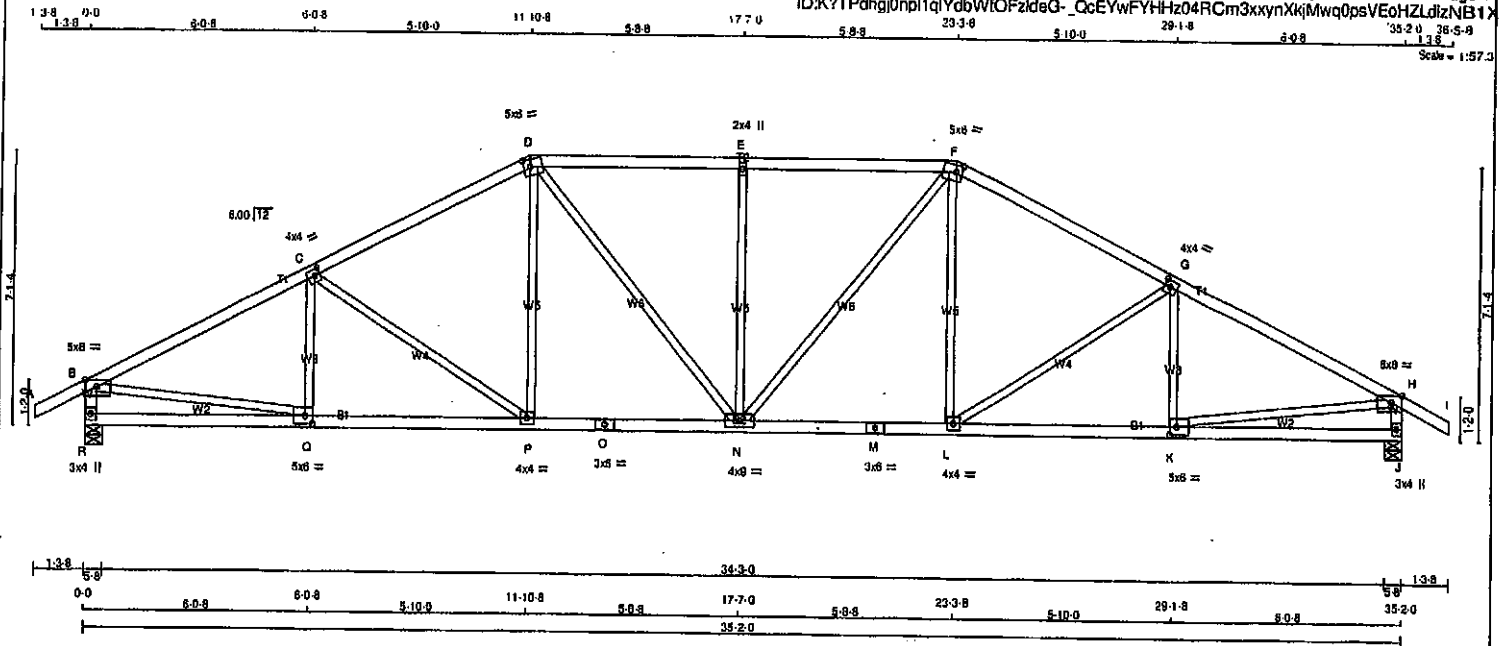
Structural component only
DWG# T-2007171

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	ORWG NO.
408170	T43	2	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	8.0	Edge 3.50
C	TMVW-t	MT20	4.0	4.0	2.00 1.75
D	TTVW-m	MT20	5.0	6.0	2.25 2.00
E	TMVW-w	MT20	2.0	4.0	
F	TTVW-m	MT20	5.0	6.0	2.25 2.00
G	TMVW-t	MT20	4.0	4.0	2.00 1.75
H	TMVW-p	MT20	5.0	8.0	Edge 3.50
J	BMVW-t	MT20	3.0	4.0	
K	BMVW-t	MT20	5.0	6.0	2.50 2.25
L	BMVW-t	MT20	4.0	4.0	
M	BS-t	MT20	3.0	6.0	
N	BMVWW-t	MT20	4.0	9.0	
O	BS-t	MT20	3.0	6.0	
P	BMVW-t	MT20	4.0	4.0	
Q	BMVW-t	MT20	5.0	6.0	2.50 2.25
R	BMVW-t	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
R	2083	0	2083	0	0
J	2083	0	2083	0	0

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1457	989 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
J	1457	989 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 28	Q-C	-255 / 12
B-C	-2889 / 0	C-P	-435 / 0
C-D	-2539 / 0	P-D	0 / 350
D-E	-2559 / 0	D-N	0 / 482
E-F	-2559 / 0	N-E	-841 / 0
F-G	-2539 / 0	N-F	0 / 482
G-H	-2889 / 0	L-F	0 / 350
H-I	0 / 28	L-G	-435 / 0
R-B	-2014 / 0	K-G	-255 / 12
J-H	-2014 / 0	B-Q	0 / 2635
		K-H	0 / 2635
R-Q	0 / 0		
Q-P	0 / 2609		
P-O	0 / 2249		
O-N	0 / 2249		
N-M	0 / 2249		
M-L	0 / 2249		
L-K	0 / 2609		
K-J	0 / 0		

TOTAL WEIGHT = 2 X 145 = 289 lb
(M)(F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

GSI: TC=0.58/1.00 (B-C:1), BC=0.48/1.00 (P-Q:1), WB=0.59/1.00 (B-Q:1), SG=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

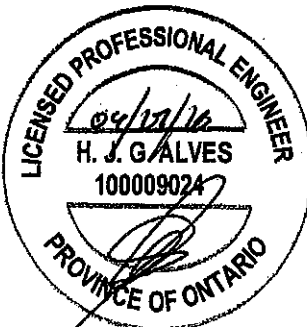
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1687 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (Q) (INPUT = 0.90)
JSI METAL = 0.89 (M) (INPUT = 1.00)



Structural component only
DWG# T-2007172

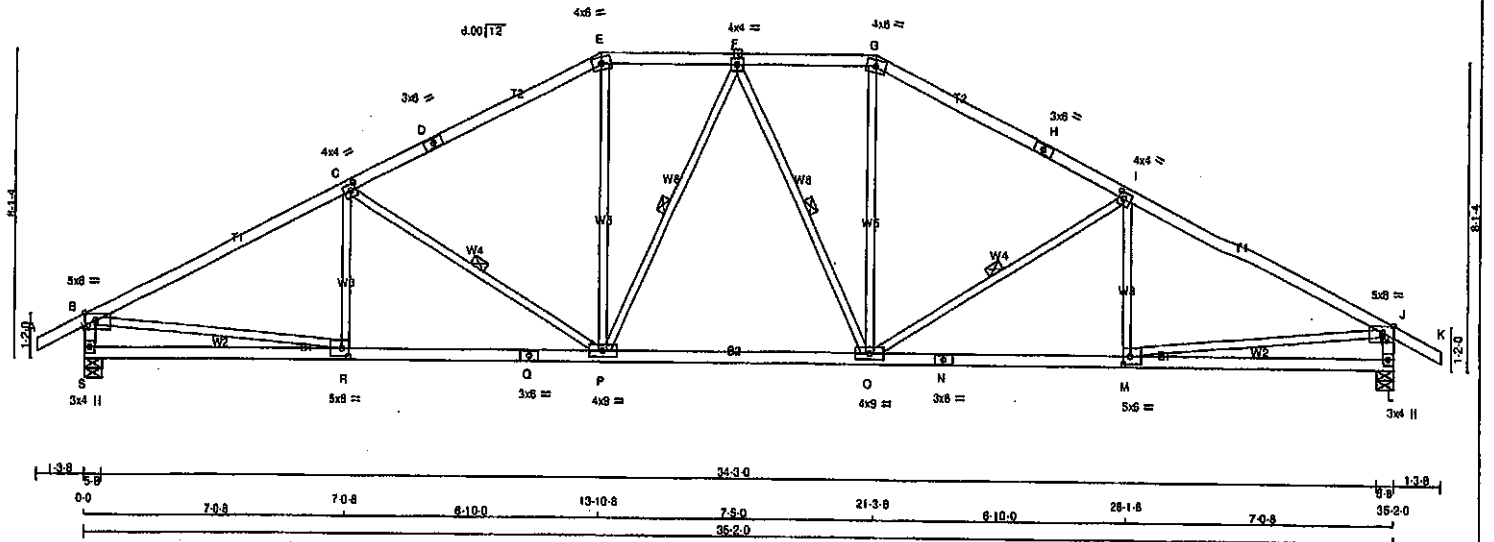
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408170	T44	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:50.0



TOTAL WEIGHT = 2 X 145 = 291 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
S - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
S - Q	2x4	DRY	No.2	SPF	
Q - N	2x4	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY, SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50	
C	TMVW-t	MT20	4.0	4.0	2.00	1.75	
D	TS-t	MT20	3.0	8.0			
E	TTW-m	MT20	4.0	8.0			
F	TMVW-t	MT20	4.0	4.0			
G	TTW-m	MT20	4.0	8.0			
H	TS-t	MT20	3.0	8.0			
I	TMVW-t	MT20	4.0	4.0	2.00	1.75	
J	TMVW-p	MT20	5.0	8.0	Edge	3.50	
L	BMV1-p	MT20	3.0	4.0			
M	BMVW-t	MT20	5.0	6.0	2.50	2.25	
N	BS-t	MT20	3.0	8.0			
O	BMVW-t	MT20	4.0	9.0			
P	BMVW-t	MT20	4.0	9.0			
Q	BS-t	MT20	3.0	6.0			
R	BMVW-t	MT20	5.0	6.0	2.50	2.25	
S	BMV1-p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS						
FACTORED			MAXIMUM FACTORED		INPUT	REQD
GROSS REACTION			GROSS REACTION		BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
S	2063	0	2063	0	0	5-8
L	2063	0	2063	0	0	5-8

UNFACTORED REACTIONS	1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED						
S	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
L	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L
 BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-P, F-Q, I-O.
 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING		TOTAL LOAD CASES: (4)							
CHORDS					W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO		FROM TO	CS1 (LC)	FR-TO			FR-TO		
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	R-C	-182 / 52	0.08 (1)		
B-C	-2906 / 0	-91.8 -91.8	0.83 (1)	3.25	C-P	-637 / 0	0.29 (1)		
C-D	-2373 / 0	-91.8 -91.8	0.72 (1)	3.68	P-E	0 / 674	0.15 (1)		
D-E	-2373 / 0	-91.8 -91.8	0.72 (1)	3.66	F-F	-222 / 0	0.11 (1)		
E-F	-2100 / 0	-91.8 -91.8	0.19 (1)	4.53	F-O	-222 / 0	0.11 (1)		
F-G	-2100 / 0	-91.8 -91.8	0.19 (1)	4.53	O-G	0 / 674	0.15 (1)		
G-H	-2373 / 0	-91.8 -91.8	0.72 (1)	3.66	O-I	-637 / 0	0.29 (1)		
H-I	-2373 / 0	-91.8 -91.8	0.72 (1)	3.66	M-I	-182 / 52	0.08 (1)		
I-J	-2906 / 0	-91.8 -91.8	0.83 (1)	3.25	B-R	0 / 2849	0.80 (1)		
J-K	0 / 28	-91.8 -91.8	0.12 (1)	10.00	M-J	0 / 2849	0.80 (1)		
S-B	-2008 / 0	0.0 0.0	0.20 (1)	5.98					
L-J	-2008 / 0	0.0 0.0	0.20 (1)	5.96					
S-R	0 / 0	-18.5 -18.5	0.20 (4)	10.00					
R-Q	0 / 2830	-18.5 -18.5	0.52 (1)	10.00					
Q-P	0 / 2830	-18.5 -18.5	0.52 (1)	10.00					
P-O	0 / 2193	-18.5 -18.5	0.45 (1)	10.00					
O-N	0 / 2830	-18.5 -18.5	0.52 (1)	10.00					
N-M	0 / 2830	-18.5 -18.5	0.52 (1)	10.00					
M-L	0 / 0	-18.5 -18.5	0.20 (4)	10.00					

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF
SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12
 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, CBC 2012, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 088-09, CSA 088-14
 - TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/380 (1.17")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.14")
 ALLOWABLE DEFL. (TL) = L/360 (1.17")
 CALCULATED VERT. DEFL. (TL) = L/999 (0.30")

CSI: TC=0.83/1.00 (B-C:1), BC=0.52/1.00 (P-R:1), WB=0.60/1.00 (B-R:1), SSI=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

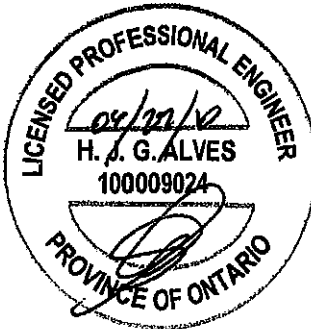
NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1687	788	1987 1858

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (M) (INPUT = 0.80)

JSI METAL = 0.79 (Q) (INPUT = 1.00)



Structural component only
 DWG# T-2007173

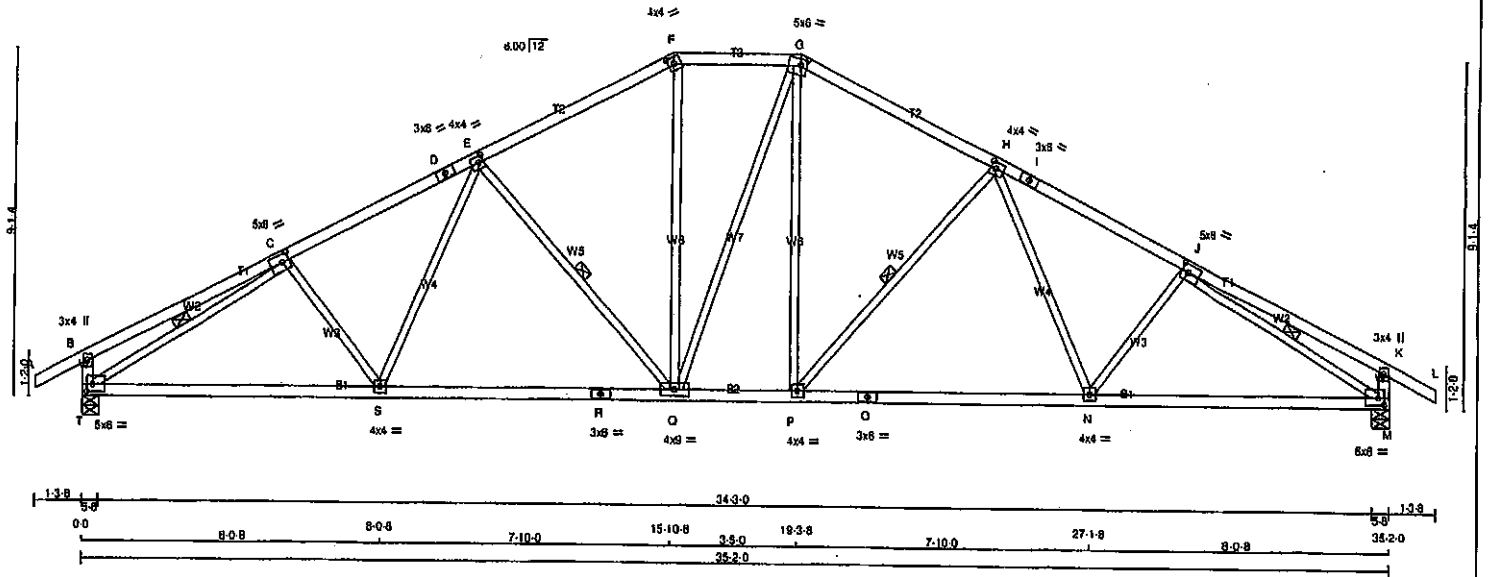
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T45	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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13.8 0.0 8.0-3 5.5-3 3.2-11 10.7-13 8.2-11 15.10-8 3.5-0 19.3-8 5.2-11 24.4-3 5.2-11 29.6-13 5.5-3 35.2-0 35.5-6 13.8 13.8

Scale = 1:58.0



TOTAL WEIGHT = 2 X 181 = 362 lbs

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
T - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
T - R	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (tables in inches)			
JT TYPE	PLATES	W	LEN Y X
B TMV+p	MT20	3.0	4.0
C TMVW-i	MT20	5.0	8.0 2.50 2.25
D TS-i	MT20	3.0	8.0
E TMVW-i	MT20	4.0	4.0 2.00 1.50
F TTW-h	MT20	4.0	4.0 2.00 1.75
G TTWV-m	MT20	5.0	8.0 2.00 2.00
H TMVW-i	MT20	4.0	4.0 2.00 1.50
I TS-i	MT20	3.0	8.0
J TMVW-i	MT20	5.0	8.0 2.50 2.25
K TMV+p	MT20	3.0	4.0
M BMVW-i	MT20	5.0	8.0 2.25 2.00
N, P, S			
N BMVW-i	MT20	4.0	4.0
O BS-i	MT20	3.0	8.0
Q BMVWVW-i	MT20	4.0	9.0
R BS-i	MT20	3.0	8.0
T BMVW-i	MT20	5.0	8.0 2.25 2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT RECD BRG
T	2063 0	2063 0	5-8
M	2063 0	2063 0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1457	989 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
M	1457	989 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, H-P, C-T, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)
FR-TO			
A-B	0 / 28	-91.8 -91.8 0.12 (1)	10.00
B-C	0 / 19	-91.8 -91.8 0.32 (1)	10.00
C-D	-2759 / 0	-91.8 -91.8 0.40 (1)	3.88
D-E	-2759 / 0	-91.8 -91.8 0.40 (1)	3.88
E-F	-2167 / 0	-91.8 -91.8 0.36 (1)	4.30
F-G	-1925 / 0	-91.8 -91.8 0.18 (1)	4.71
G-H	-2166 / 0	-91.8 -91.8 0.36 (1)	4.30
H-I	-2760 / 0	-91.8 -91.8 0.40 (1)	3.88
I-J	-2760 / 0	-91.8 -91.8 0.40 (1)	3.88
J-K	0 / 19	-91.8 -91.8 0.32 (1)	10.00
K-L	0 / 28	-91.8 -91.8 0.12 (1)	10.00
T-B	-328 / 0	0.0 0.0 0.03 (1)	7.81
M-K	-328 / 0	0.0 0.0 0.03 (1)	7.81
WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. UNBRACED LENGTH (FT)
FR-TO			
C-S	-110 / 37	0.04 (1)	
S-E	0 / 276	0.06 (1)	
E-Q	-881 / 0	0.31 (1)	
Q-F	0 / 613	0.14 (1)	
F-G	0 / 4	0.00 (1)	
P-G	0 / 609	0.14 (1)	
P-H	-882 / 0	0.31 (1)	
H-N	0 / 276	0.06 (1)	
N-J	-110 / 37	0.04 (1)	
T-C	-3040 / 0	0.85 (1)	
J-M	-3041 / 0	0.85 (1)	
T-S	0 / 2536	-18.5 -18.5 0.55 (1)	10.00
S-R	0 / 2362	-18.5 -18.5 0.53 (1)	10.00
R-Q	0 / 2362	-18.5 -18.5 0.53 (1)	10.00
Q-P	0 / 1924	-18.5 -18.5 0.40 (1)	10.00
P-O	0 / 2362	-18.5 -18.5 0.53 (1)	10.00
O-N	0 / 2362	-18.5 -18.5 0.53 (1)	10.00
N-M	0 / 2537	-18.5 -18.5 0.56 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.32")

CSI: TC=0.40/1.00 (H-J:1), BC=0.50/1.00 (M-N:1), WB=0.65/1.00 (J-M:1), SS=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1887 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (M) (INPUT = 0.90)
JSI METAL = 0.78 (O) (INPUT = 1.00)



Structural component only
DWG# T-2007174

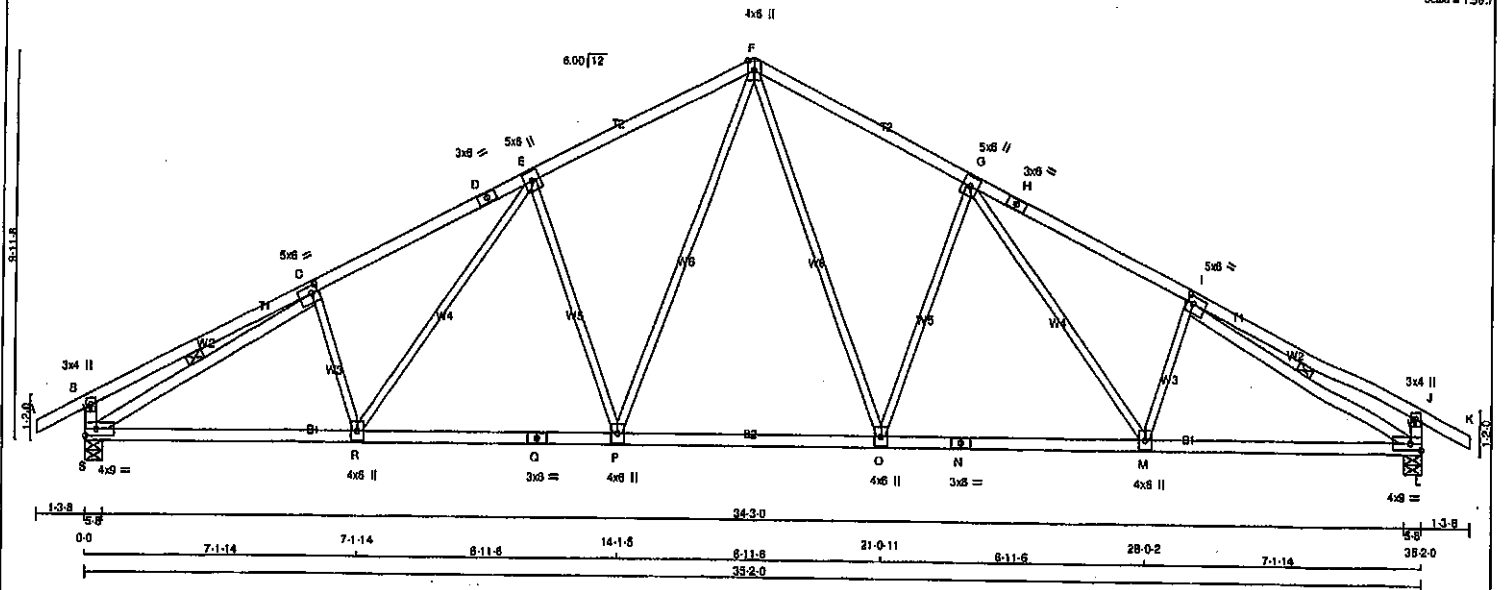
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408170	T46	6	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:K7TPchqjDnpl1qYdbWfOfzIdG-O?HMaxHGaCLbxuxLk4UfPALGz7q50ADgUfO7E1zNB1U

Scale = 1/8" = 1'-0"



TOTAL WEIGHT = 6 X 152 = 910 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	No.2	SPF		
F - H	2x4	DRY	No.2	SPF		
H - K	2x4	DRY	No.2	SPF		
S - B	2x4	DRY	No.2	SPF		
L - J	2x4	DRY	No.2	SPF		
S - Q	2x4	DRY	No.2	SPF		
Q - N	2x4	DRY	No.2	SPF		
N - L	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						
S - C	2x4	DRY	No.2	SPF		
I - L	2x4	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
C	TMVW+1	MT20	5.0	8.0	2.25	2.00	
D	TS-1	MT20	3.0	8.0			
E	TMVW+1	MT20	5.0	8.0			
F	TMVW+p	MT20	4.0	8.0	Edge		
G	TMVW+1	MT20	5.0	8.0			
H	TS-1	MT20	3.0	8.0			
I	TMVW+1	MT20	5.0	8.0	2.25	2.00	
J	TMV+p	MT20	3.0	4.0			
L	BMVW+1	MT20	4.0	9.0	Edge		
M, O, P, R							
N	BS-1	MT20	3.0	8.0			
Q	BS-1	MT20	3.0	8.0			
S	BMVW+1	MT20	4.0	9.0	Edge		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED	MAXIMUM FACTORED	INPUT	REQD			
	GROSS REACTION	GROSS REACTION	BRG	BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
S	2063	0	2063	0	0	5-8	5-8
L	2063	0	2063	0	0	5-8	5-8
UNFACTORED REACTIONS							
	1ST LCASE	MAX/MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
L	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-S, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	F-O	0 / 866	0.19 (1)
B-C	0 / 22	-91.8	-91.8 0.40 (1)	10.00	O-G	-726 / 0	0.72 (1)
C-D	-2808 / 0	-91.8	-91.8 0.49 (1)	3.75	G-M	0 / 391	0.09 (1)
D-E	-2808 / 0	-91.8	-91.8 0.49 (1)	3.75	M-I	-192 / 18	0.05 (1)
E-F	-2322 / 0	-91.8	-91.8 0.46 (1)	4.07	P-F	0 / 866	0.19 (1)
F-G	-2322 / 0	-91.8	-91.8 0.46 (1)	4.07	E-P	-726 / 0	0.72 (1)
G-H	-2808 / 0	-91.8	-91.8 0.49 (1)	3.75	R-E	0 / 391	0.09 (1)
H-I	-2808 / 0	-91.8	-91.8 0.49 (1)	3.75	C-R	-192 / 18	0.05 (1)
I-J	0 / 22	-91.8	-91.8 0.40 (1)	10.00	S-C	-3084 / 0	0.72 (1)
J-K	0 / 28	-91.8	-91.8 0.12 (1)	10.00	I-L	-3084 / 0	0.72 (1)
S-B	-344 / 0	0.0	0.0 0.03 (1)	7.81			
L-J	-344 / 0	0.0	0.0 0.03 (1)	7.81			
S-R	0 / 2574	-18.5	-18.5 0.53 (1)	10.00			
R-Q	0 / 2291	-18.5	-18.5 0.47 (1)	10.00			
Q-P	0 / 2291	-18.5	-18.5 0.47 (1)	10.00			
P-O	0 / 1762	-18.5	-18.5 0.38 (1)	10.00			
O-N	0 / 2291	-18.5	-18.5 0.47 (1)	10.00			
N-M	0 / 2291	-18.5	-18.5 0.47 (1)	10.00			
M-L	0 / 2574	-18.5	-18.5 0.53 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/380 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.48/1.00 (G-I), BC=0.53/1.00 (R-S), WB=0.72/1.00 (E-P), SI=0.22/1.00 (B-C)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.77 (I) (INPUT = 1.00)

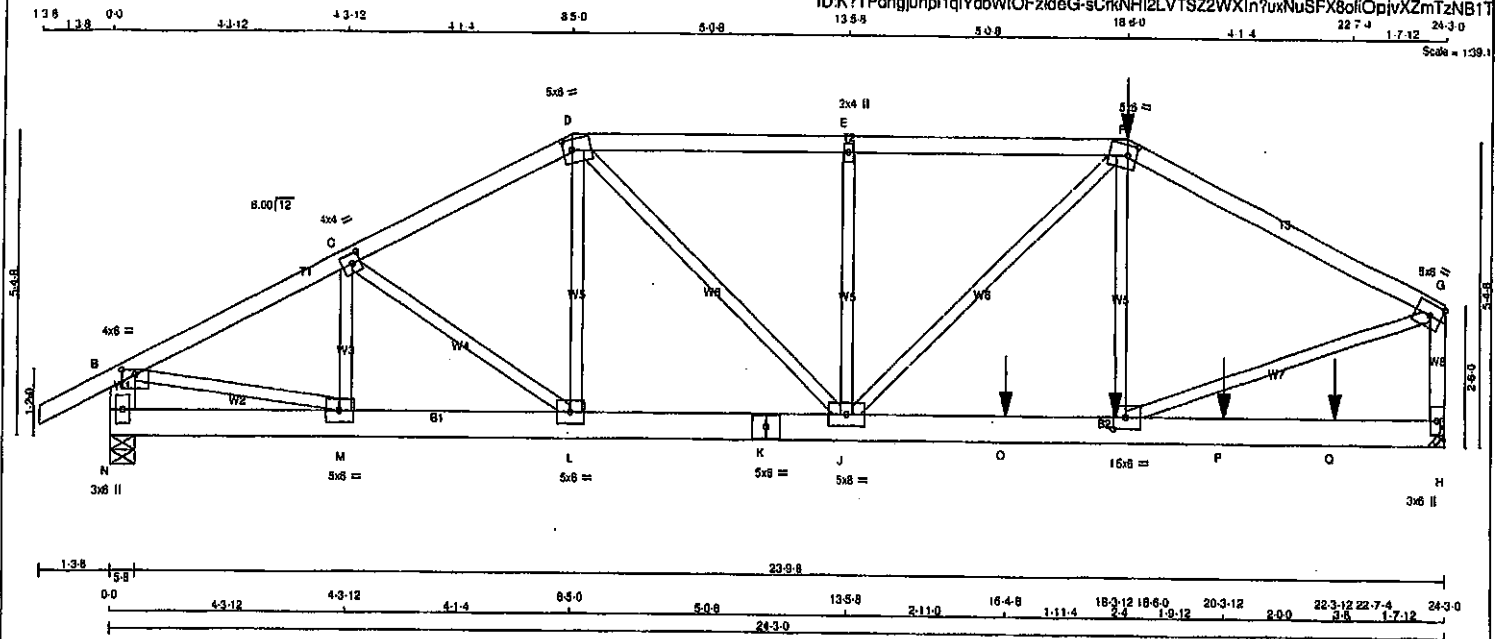


Structural component only
DWG# T-2007175

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408170	T47	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2 SPF
D - F	2x4	DRY	No.2 SPF
F - G	2x4	DRY	No.2 SPF
N - B	2x6	DRY	No.2 SPF
H - G	2x4	DRY	No.2 SPF
N - K	2x6	DRY	No.2 SPF
K - H	2x6	DRY	No.2 SPF
ALL WEBS	2x3	DRY	No.2 SPF
EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS			
A-D	1	12	TOP
D-F	1	12	SIDE(81.0)
F-G	1	12	SIDE(81.0)
H-G	1	12	TOP
N-B	2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS			
N-K	2	12	TOP
K-H	2	12	SIDE(183.1)
WEBS: (0.122"x3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	UP	IN-SX
N	2120	0	2120	0	5-8
H	3119	0	3119	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 4-0.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1495	1003 / 0	0 / 0	0 / 0	0 / 0	492 / 0	0 / 0
H	2201	1472 / 0	0 / 0	0 / 0	0 / 0	729 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)
MEMB.	FR-TO								
A-B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	M-C	-408 / 0	0.04 (1)	
B-C	-2914 / 0	-91.8	-91.8	0.17 (1)	5.22	C-L	-68 / 0	0.01 (1)	
C-D	-2892 / 0	-91.8	-91.8	0.17 (1)	5.24	L-D	0 / 70	0.01 (4)	
D-E	-3517 / 0	-91.8	-91.8	0.27 (1)	4.74	D-J	0 / 1338	0.17 (1)	
E-F	-3517 / 0	-91.8	-91.8	0.27 (1)	4.74	J-E	-555 / 0	0.12 (1)	
F-G	-3468 / 0	-91.8	-91.8	0.39 (1)	4.64	J-F	0 / 575	0.07 (1)	
N-B	-2084 / 0	0.0	0.0	0.07 (1)	7.81	I-F	0 / 522	0.08 (1)	
H-G	-2933 / 0	0.0	0.0	0.20 (1)	6.72	I-G	0 / 3293	0.41 (1)	
						B-M	0 / 2664	0.33 (1)	
N-M	0 / 0	-18.5	-18.5	0.03 (1)	10.00				
M-L	0 / 2622	-18.5	-18.5	0.21 (1)	10.00				
L-K	0 / 2568	-18.5	-18.5	0.33 (1)	10.00				
K-J	0 / 2568	-18.5	-18.5	0.33 (1)	10.00				
J-I	0 / 3109	-18.5	-18.5	0.69 (1)	10.00				
O-I	0 / 3109	-18.5	-18.5	0.69 (1)	10.00				
I-P	0 / 0	-18.5	-18.5	0.32 (1)	10.00				
P-Q	0 / 0	-18.5	-18.5	0.32 (1)	10.00				
Q-H	0 / 0	-18.5	-18.5	0.32 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
F	18-8-0	-178	-178	---	FRONT	VERY	TOTAL	C1
I	18-3-12	-35	-35	---	FRONT	VERY	TOTAL	C1
O	18-4-8	-1661	-1661	---	FRONT	VERY	TOTAL	C1
P	20-3-12	-283	-283	---	FRONT	VERY	TOTAL	C1
Q	22-3-12	-284	-284	---	FRONT	VERY	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 114 = 228 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN CC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = U/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = U/999 (0.15")

CSI: TC=0.39/1.00 (F-G:1), BC=0.69/1.00 (I-J:1), WB=0.41/1.00 (G-I:1), SS=0.37/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)	FLU
MT20	518	354	1687
	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (I) (INPUT = 0.90)
JSI METAL = 0.44 (G) (INPUT = 1.00)



Structural component only
DWG# T-2007176 92

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T47	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
S	TMWV-p	MT20	4.0	8.0	1.00	3.00
C	TMWV-t	MT20	4.0	4.0	2.00	1.75
D	TTWV-m	MT20	5.0	8.0	2.25	2.00
E	TMW-v	MT20	2.0	4.0		
F	TTWV-m	MT20	5.0	8.0	2.25	2.00
G	TMWV-t	MT20	5.0	8.0		Edge
H	BMV1-p	MT20	3.0	8.0		
I	BMWV-t	MT20	5.0	8.0	2.50	2.75
J	BMWVW-t	MT20	5.0	8.0		
K	BS-t	MT20	5.0	8.0		
L	BMWV-t	MT20	5.0	8.0		
M	BMWV-t	MT20	5.0	8.0		
N	BMV1-p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

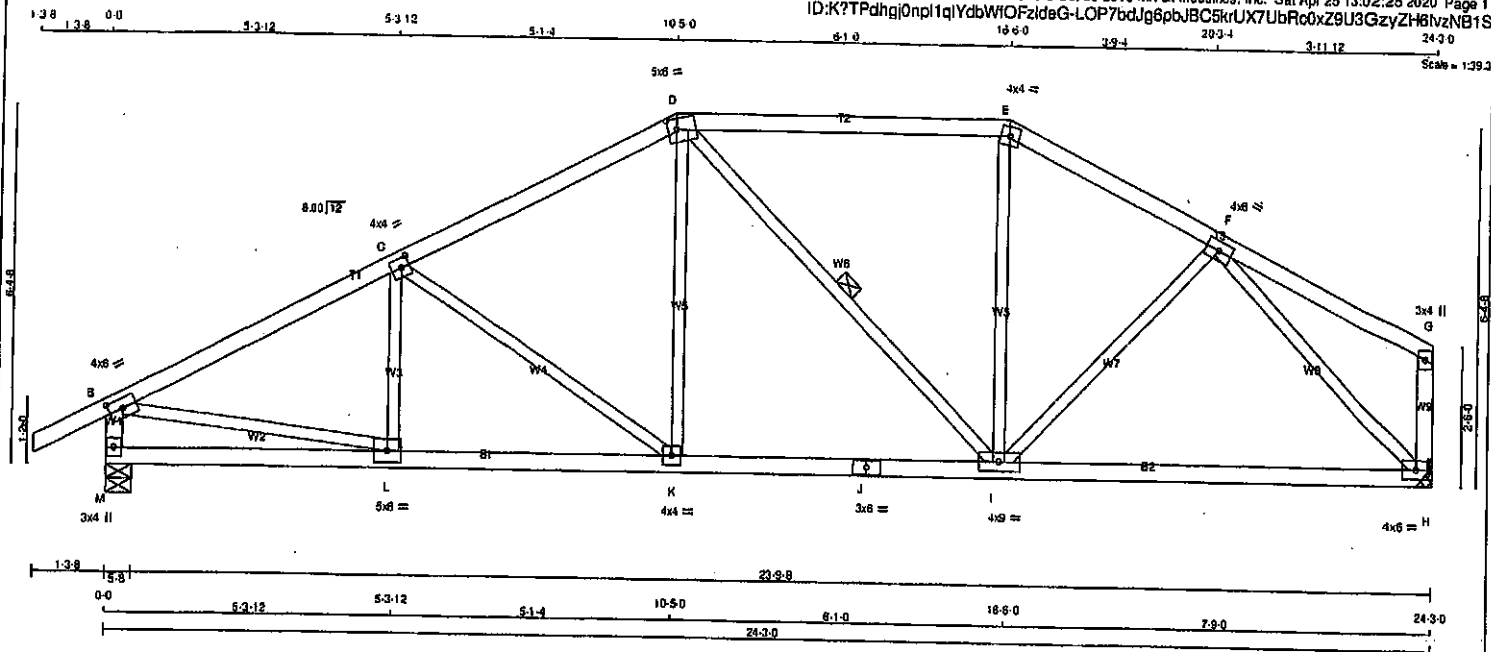


Structural component only
DWG# T-2007176 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T48	1	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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ID:K7TPdhgJ0npl1qIYdbWIOFzIdeG-LOP7bdJg6pbJBC5krUX7UbRc0xZ9U3GzyZH6NzNB1S



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-1	MT20	4.0	6.0	2.00	3.00
C TMVW-1	MT20	4.0	4.0	2.00	1.75
D TTWW-m	MT20	5.0	6.0	2.25	2.00
E TTW-m	MT20	4.0	4.0		
F TMVW-1	MT20	4.0	6.0		
G TMV-p	MT20	3.0	4.0		
H BMVW-1	MT20	4.0	6.0		
I BMVW-1	MT20	4.0	9.0		
J BS-1	MT20	3.0	6.0		
K BMVW-1	MT20	4.0	4.0		
L BMVW-1	MT20	5.0	6.0		
M BMV-1p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	1461	1461	0	0
M VERT	1337	1337	0	0
H VERT	1337	1337	0	0
			MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX MIN. COMPONENT REACTIONS					
M	1031	690 / 0	0 / 0	0 / 0	0 / 0	341 / 0	0 / 0
H	945	620 / 0	0 / 0	0 / 0	0 / 0	325 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	L-C	-165 / 32	0.04 (1)
B-C	-1844 / 0	-91.8 -91.8	0.36 (1)	4.58	C-K	-438 / 0	0.31 (1)
C-D	-1488 / 0	-91.8 -91.8	0.34 (1)	5.00	K-D	0 / 334	0.08 (1)
D-E	-1181 / 0	-91.8 -91.8	0.46 (1)	5.27	D-I	-189 / 0	0.09 (1)
E-F	-1336 / 0	-91.8 -91.8	0.18 (1)	5.42	I-E	0 / 175	0.05 (4)
F-G	0 / 18	-91.8 -91.8	0.21 (1)	10.00	I-F	0 / 158	0.04 (4)
M-B	-1419 / 0	0.0 0.0	0.14 (1)	6.85	B-L	0 / 1692	0.38 (1)
H-G	-138 / 0	0.0 0.0	0.02 (1)	7.81	F-H	-1593 / 0	0.82 (1)
M-L	0 / 0	-18.5 -18.5	0.12 (4)	10.00			
L-K	0 / 1671	-18.5 -18.5	0.32 (1)	10.00			
K-J	0 / 1313	-18.5 -18.5	0.36 (4)	10.00			
J-I	0 / 1313	-18.5 -18.5	0.36 (4)	10.00			
I-H	0 / 1069	-18.5 -18.5	0.34 (4)	10.00			

TOTAL WEIGHT = 99 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.0/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OSC 2012, ABC 2019
- PART 9 OF OSC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) =	L/360 (0.81")
CALCULATED VERT. DEFL.(LL) =	U/889 (0.05")
ALLOWABLE DEFL.(TL) =	U/360 (0.81")
CALCULATED VERT. DEFL.(TL) =	U/889 (0.14")

CSI: TC=0.48/1.00 (D-E:1), BC=0.38/1.00 (I-K:4), WB=0.82/1.00 (F-H:1), SS=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR (PSI)	SECTION (PLI)	MAX MIN MAX MIN
MT20	618 384 1667 788 1897 1658	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 1.00)

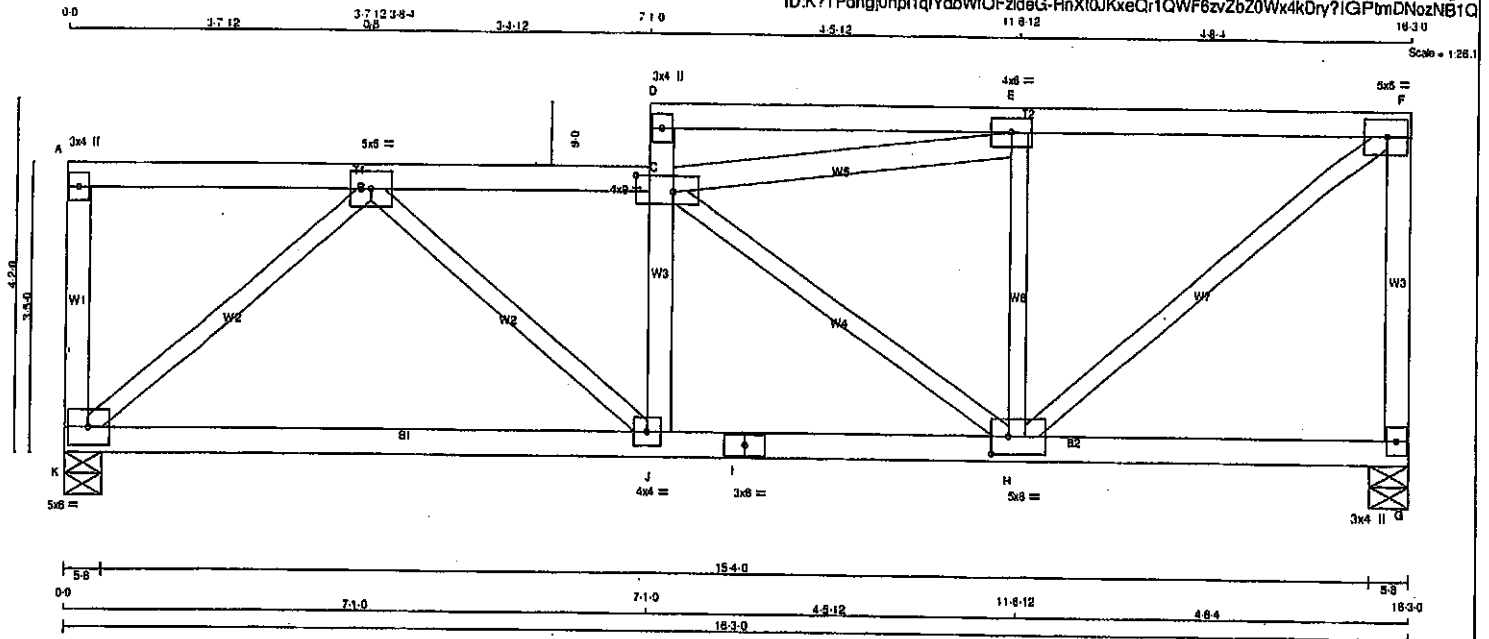


Structural component only
DWG# T-2007177

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408170	T49	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 72 = 216 lb
(M/R)

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
K - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
J - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
C - E	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMV+p	MT20	3.0	4.0
B	TMVW-1	MT20	5.0	6.0
C	TMVW-1	MT20	4.0	9.0 2.25 5.50
D	TMV+p	MT20	3.0	4.0
E	TMVW-1	MT20	4.0	6.0
F	TMVW-1	MT20	5.0	6.0
G	BMV1+p	MT20	3.0	4.0
H	BMVW-1	MT20	5.0	6.0 2.50 2.50
I	BS4	MT20	3.0	6.0
J	BMVW-1	MT20	4.0	4.0
K	BMVW-1	MT20	5.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
K	1079	0	1079	0
G	1079	0	1079	0

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
K	780	416 / 0	0 / 0	0 / 0	0 / 0	384 / 0	0 / 0
G	780	416 / 0	0 / 0	0 / 0	0 / 0	384 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED	FACTORED	
	FORCE	VERT. LOAD	LC1 MAX		FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)		(LBS)	CSI (LC)	
FR-TO		FROM TO		FR-TO			
K-A	-163 / 0	0.0	0.0 0.04 (1)	7.81	K-B	-1328 / 0	0.65 (1)
A-B	0 / 0	-114.3 -114.3	0.32 (1)	10.00	B-J	0 / 538	0.16 (1)
B-C	-1416 / 0	-114.3 -114.3	0.37 (1)	4.47	C-E	-961 / 0	0.30 (1)
J-C	-228 / 0	0.0	0.0 0.08 (1)	7.81	C-H	-489 / 0	0.30 (1)
C-D	-183 / 0	0.0	0.0 0.08 (1)	7.81	H-E	-471 / 0	0.16 (1)
D-E	-58 / 0	-114.3 -114.3	0.51 (1)	6.25	H-F	0 / 1303	0.39 (1)
E-F	-1004 / 0	-114.3 -114.3	0.55 (1)	4.81			
G-F	-1039 / 0	0.0	0.0 0.37 (1)	8.91			
K-J	0 / 1013	-18.5 -18.5	0.41 (4)	10.00			
J-I	0 / 1402	-18.5 -18.5	0.48 (4)	10.00			
I-H	0 / 1402	-18.5 -18.5	0.48 (4)	10.00			
H-G	0 / 0	-18.5 -18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 15.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 48.0	PSF

SPACING = 24.0 IN. CG

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.54")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.54")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.11")

CSI: TC=0.65/1.00 (E-F:1), BC=0.48/1.00 (H-J:4),
WB=0.65/1.00 (B-K:1), SS=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX MIN
MT20	618	354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.62 (I) (INPUT = 1.00)

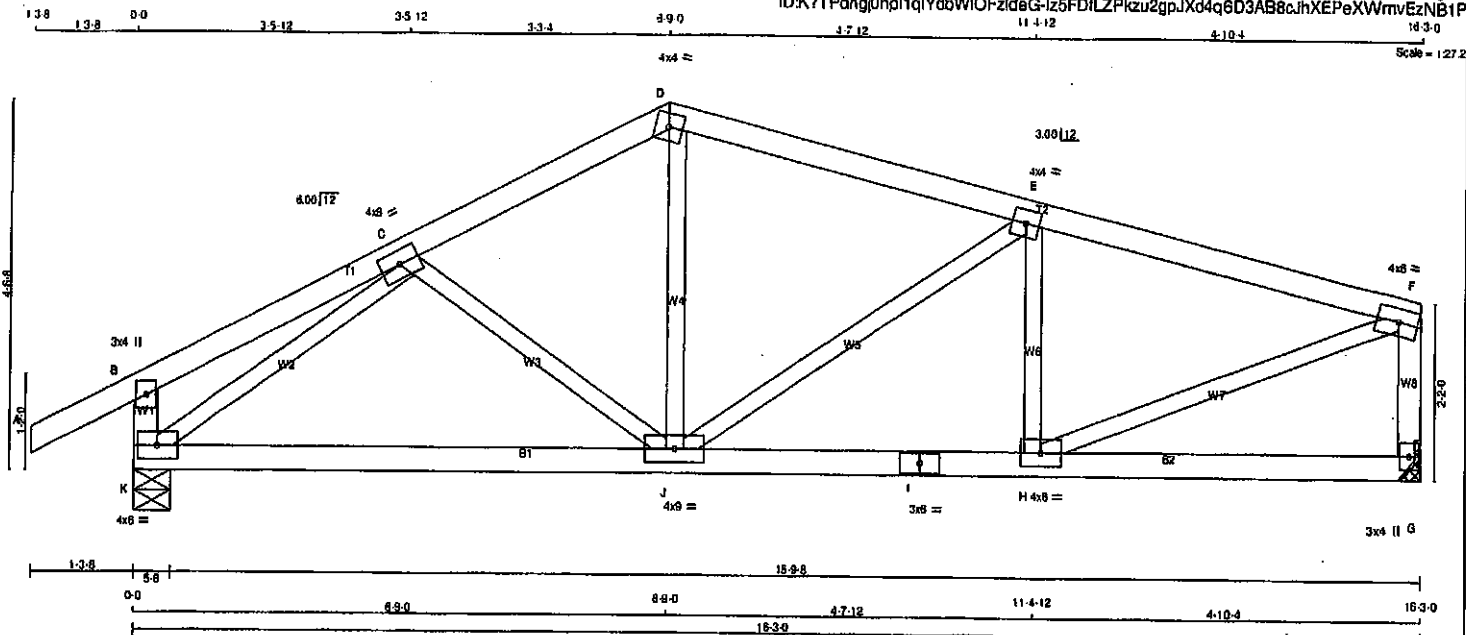


Structural component only
DWG# T-2007178

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408170	T50	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 63 = 126 lb
(M/F)

N. L. G. A. RULES	SIZE	LUMBER
CHORDS		
A - D	2x4	DRY No.2
D - F	2x4	DRY No.2
K - B	2x4	DRY No.2
G - F	2x4	DRY No.2
K - I	2x4	DRY No.2
I - G	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)	W	LEN	Y	X
JT TYPE				
B TMV-p	MT20	3.0	4.0	
C TMWV-t	MT20	4.0	6.0	
D TTW-J	MT20	4.0	4.0	
E TMWV-t	MT20	4.0	4.0	
F TMWV-t	MT20	4.0	6.0	
G BMV1-p	MT20	3.0	4.0	
H BMWV-t	MT20	4.0	6.0	
I BS-1	MT20	3.0	6.0	
J BMWVW-t	MT20	4.0	9.0	
K BMWV1-t	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORZ	UPLIFT
K	1020	0	0	5-8
G	696	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING
LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS
1ST CASE MAX/MIN COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	719	485/0	0/0	0/0	0/0	234/0	0/0
G	633	418/0	0/0	0/0	0/0	218/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED		
MEMB.	FORCE	VERT. LOAD	LC1	MAX.	MEMB.	FORCE	MAX.	
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO			
A-B	0/28	-91.8	-91.8	0.12 (1)	10.00	C-J	-135/7	0.04 (1)
B-C	0/14	-91.8	-91.8	0.15 (1)	10.00	J-D	0/355	0.08 (1)
C-D	-962/0	-91.8	-91.8	0.13 (1)	6.20	J-E	-263/0	0.13 (1)
D-E	-894/0	-91.8	-91.8	0.28 (1)	6.18	H-E	-335/0	0.07 (1)
E-F	-1094/0	-91.8	-91.8	0.27 (1)	5.73	K-C	-1196/0	0.35 (1)
K-B	-248/0	0.0	0.0	0.03 (1)	7.61	H-F	0/1150	0.28 (1)
G-F	-855/0	0.0	0.0	0.10 (1)	7.81			
K-J	0/956	-18.5	-18.5	0.26 (1)	10.00			
J-I	0/1073	-18.5	-18.5	0.27 (1)	10.00			
I-H	0/1073	-18.5	-18.5	0.27 (1)	10.00			
H-G	0/0	-18.5	-18.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.I. PLUS 9.4 P.S.F. RAIN
LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF
LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.54")
CALCULATED VERT. DEFL. (LL) = L/999 (0.03")
ALLOWABLE DEFL. (TL) = L/360 (0.54")
CALCULATED VERT. DEFL. (TL) = L/999 (0.09")

CSI: TC=0.27/1.00 (E-F:1), BC=0.27/1.00 (H-J:1),
WB=0.35/1.00 (C-K:1), SS=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

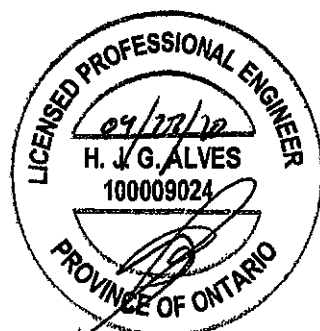
TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.71 (H) (INPUT = 0.90)
JSI METAL = 0.32 (I) (INPUT = 1.00)

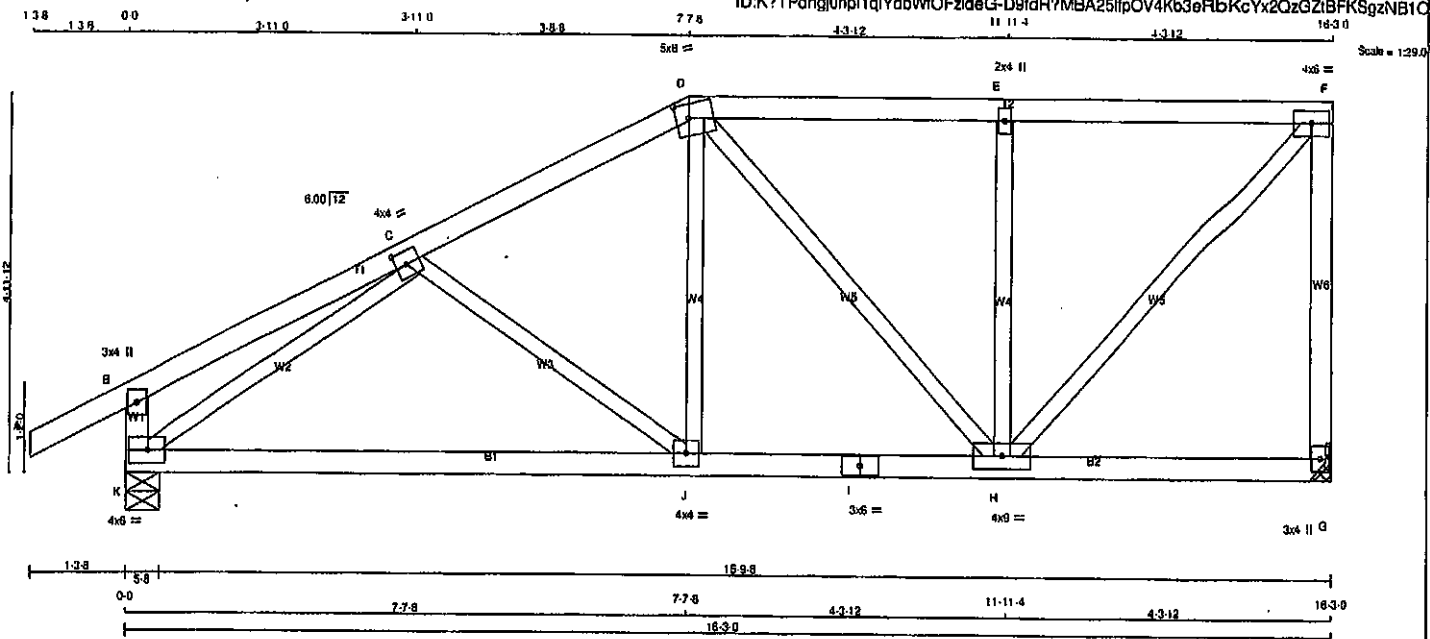


Structural component only
DWG# T-2007179

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T51	1	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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LUMBER	N, L, G, A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

PLATES (Table in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
C	TMWW-l	MT20	4.0	4.0	2.00	1.75	
D	TTWW-m	MT20	5.0	8.0	2.25	2.00	
E	TMW-w	MT20	2.0	4.0			
F	TMW-l	MT20	4.0	8.0			
G	BMV1+p	MT20	3.0	4.0			
H	BMWW-l	MT20	4.0	8.0			
I	BS-l	MT20	3.0	6.0			
J	BMWW-l	MT20	4.0	4.0			
K	BMV1-l	MT20	4.0	6.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG	BRG	BRG	IN-SX	IN-SX
G	888	0	888	0	0	MECHANICAL			
K	1020	0	1020	0	0	5-8	5-8		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS							
	1ST LC CASE	MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	633	416 / 0	0 / 0	0 / 0	0 / 0	218 / 0	0 / 0
K	719	485 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE (LBS)	MAX	MAX
FR-TO		FROM	TO	CS1 (LC)	UNBRAC	FR-TO		CS1 (LC)	
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	C-J	-215 / 0	0.08 (1)	
B-C	0 / 17	-91.8	-91.8	0.20 (1)	10.00	J-D	0 / 288	0.08 (1)	
C-D	-901 / 0	-91.8	-91.8	0.22 (1)	8.21	D-H	-210 / 0	0.14 (1)	
D-E	-854 / 0	-91.8	-91.8	0.29 (1)	8.25	H-E	-488 / 0	0.18 (1)	
E-F	-854 / 0	-91.8	-91.8	0.29 (1)	8.25	H-F	0 / 961	0.22 (1)	
G-F	-858 / 0	0.0	0.0	0.35 (1)	7.81	K-C	-1192 / 0	0.43 (1)	
K-B	-261 / 0	0.0	0.0	0.03 (1)	7.81				
K-J	0 / 965	-18.5	-18.5	0.30 (4)	10.00				
J-I	0 / 796	-18.5	-18.5	0.30 (4)	10.00				
I-H	0 / 796	-18.5	-18.5	0.30 (4)	10.00				
H-G	0 / 0	-18.5	-18.5	0.08 (4)	10.00				

TOTAL WEIGHT = 70 LB

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
OL = 8.0 PSF
BOT CH. LL = 0.0 PSF
OL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.35/1.00 (F-G:1), BC=0.30/1.00 (J-K:4),
WB=0.43/1.00 (C-K:1), SSI=0.18/1.00 (E-F:1)

DDL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

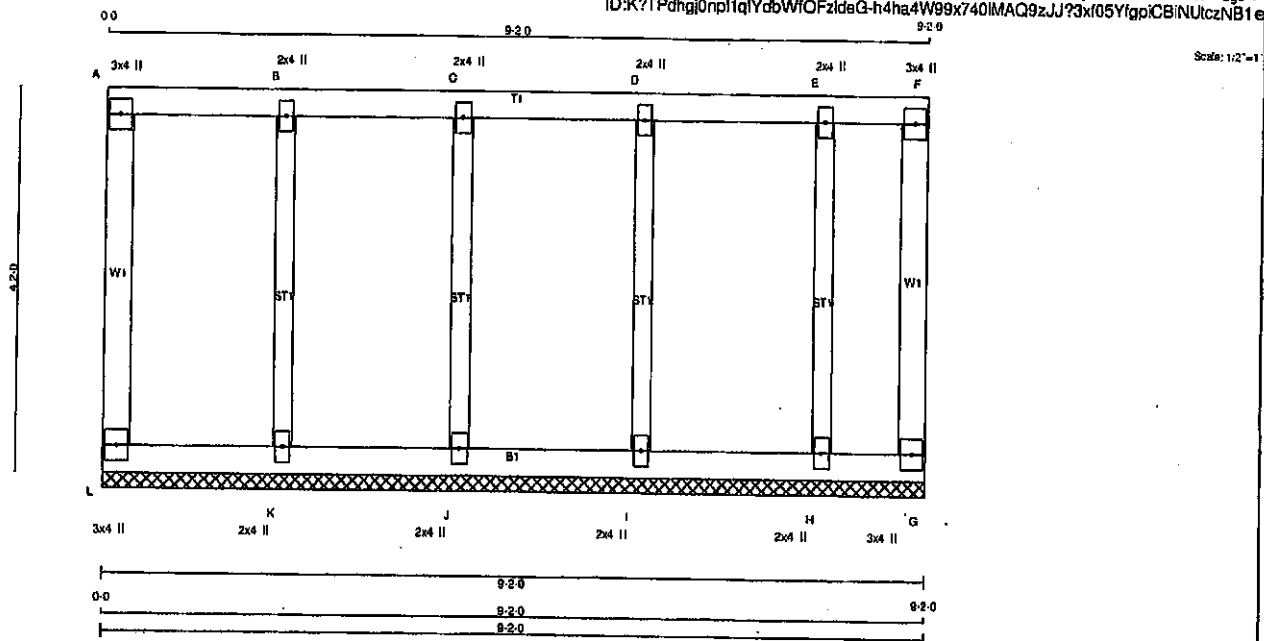
JSI GRIP=0.88 (C) (INPUT = 0.90)
JSI METAL=0.40 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007180

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	G52	1	1	GREEN PARK HOMES	
Tagharack Roof Truss, Burlington					

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
L - A	2x4	DRY	No.2	SPF	
A - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
L - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (tablets in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B, C, D, E						
B	TMV+w	MT20	2.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMV1+p	MT20	3.0	4.0		
H, I, J, K						
H	BMV1+w	MT20	2.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.
BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO			
L-A	-100 / 0	0.0	0.02 (1)	K-B	-245 / 0	0.09 (1)	7.81
A-B	-4 / 0	-114.3	-114.3 0.08 (1)	J-C	-224 / 0	0.08 (1)	10.00
B-C	-4 / 0	-114.3	-114.3 0.08 (1)	I-D	-238 / 0	0.08 (1)	10.00
C-D	-4 / 0	-114.3	-114.3 0.07 (1)	H-E	-188 / 0	0.08 (1)	10.00
D-E	-4 / 0	-114.3	-114.3 0.07 (1)				
E-F	-4 / 0	-114.3	-114.3 0.05 (1)				
G-F	-54 / 0	0.0	0.02 (1)				
L-K	0 / 4	-18.5	-18.5 0.02 (4)				
K-J	0 / 4	-18.5	-18.5 0.02 (4)				
J-I	0 / 4	-18.5	-18.5 0.02 (4)				
I-H	0 / 4	-18.5	-18.5 0.02 (4)				
H-G	0 / 4	-18.5	-18.5 0.01 (4)				

TOTAL WEIGHT = 41 lb (M/R)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 15.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 48.0 PSF
SPACING = 24.0 IN. C/C
LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015
THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014
(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.8 P.S.F. SPECIFIED ROOF LIVE LOAD
CSI: TC=0.08/1.00 (B-C:1), BC=0.02/1.00 (J-K:4), WB=0.09/1.00 (B-K:1), SS=0.13/1.00 (A-B:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	818 354	1087 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.17 (B) (INPUT = 0.90)
JSI METAL = 0.07 (K) (INPUT = 1.00)



Structural component only
DWG# T-2007166

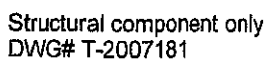
Tamarack Roof Truss, Burlington

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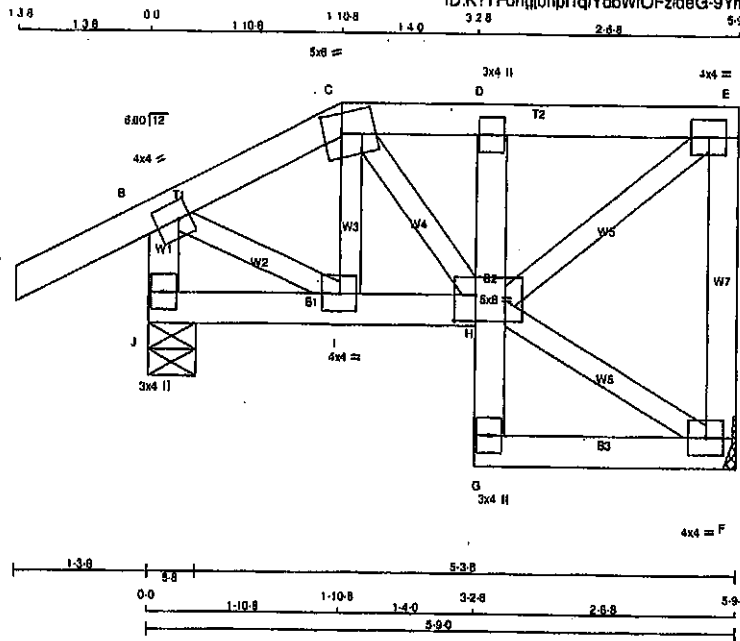
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T54S	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.310 S Oct 29 2019 Mtek Industries, Inc. Sat Apr 25 13:02:31 2020 Page 1	



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - E	2x4	DRY	No.2	SPF		
F - E	2x4	DRY	No.2	SPF		
J - B	2x4	DRY	No.2	SPF		
J - H	2x4	DRY	No.2	SPF		
G - D	2x4	DRY	No.2	SPF		
Q - F	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	4.0	2.00	1.25
C	TTVW-m	MT20	5.0	6.0	2.25	2.00
D	TMV+p	MT20	3.0	4.0		
E	TMVW-1	MT20	4.0	4.0		
F	BMVW-1	MT20	4.0	4.0		
G	BMV+p	MT20	3.0	4.0		
H	BMVW-1	MT20	5.0	8.0	3.00	2.50
I	BMVW-1	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	302	0	302	0
J	456	0	456	0
			MECHANICAL	IN-SX
			5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	214	139 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0
J	320	225 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1	MAX. CS1 (LC)	MAX. CS1 (LC)
FR-TO		FROM	TO	FR-TO
A-B	0 / 28	-91.8	-91.8	10.00
B-C	-240 / 0	-91.8	-91.8	6.25
C-D	-246 / 0	-91.8	-91.8	6.25
D-E	-241 / 0	-91.8	-91.8	6.25
E-F	-276 / 0	0.0	0.0	7.81
F-G	-439 / 0	0.0	0.0	7.81
J-I	0 / 0	-18.5	-18.5	10.00
I-H	0 / 193	-18.5	-18.5	10.00
H-G	0 / 25	0.0	0.0	10.00
G-F	-219 / 0	0.0	0.0	7.81
F-E	0 / 8	-18.5	-18.5	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/899 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/899 (0.01")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (H-I:1), WB=0.07/1.00 (E-H:1), SSI=0.10/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

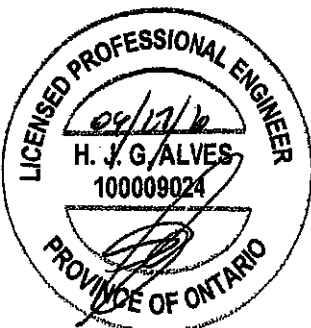
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1907 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

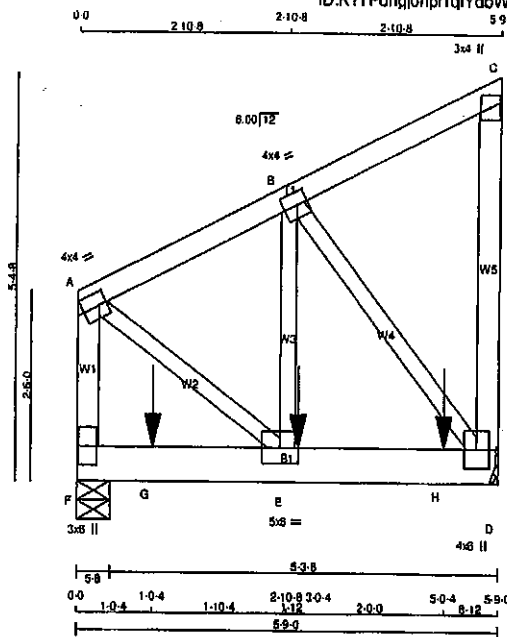


Structural component only
DWG# T-2007182

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408170	T55	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

ID:K?TPdhgJ0np1qIYdbWIOFzIdG-dkKm30Q3SztJWH74mT9mG3DvHl_9dNX?Z9U_2?zNB1L



Scale = 1/2" = 1'-0"

CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
D - O	2x4	DRY	No.2
F - D	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A	12	TOP
A - C	12	TOP
C - D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
B - E	6	SIDE(45.8)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00 1.25
B	TMVW-t	MT20	4.0	4.0	2.00 1.75
C	TMV-p	MT20	3.0	4.0	
D	BMVW1-p	MT20	4.0	6.0	
E	BMVW-t	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
F	1589 0	1589 0	0 0	5-8 5-8
D	1880 0	1880 0	0 0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
F	1121	750 / 0	0 / 0	0 / 0	371 / 0	0 / 0
D	1185	792 / 0	0 / 0	0 / 0	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
F - A	-1074 / 0	0.0	0.0	0.07 (1)	7.81	A - E	0 / 975	0.12 (1)	
A - B	-887 / 0	-91.8	-91.8	0.08 (1)	6.25	E - B	0 / 1044	0.13 (1)	
B - C	-12 / 0	-91.8	-91.8	0.08 (1)	6.25	B - D	-1248 / 0	0.21 (1)	
D - C	-106 / 0	0.0	0.0	0.02 (1)	7.81				
F - G	0 / 0	-18.5	-18.5	0.19 (1)	10.00				
G - E	0 / 0	-18.5	-18.5	0.19 (1)	10.00				
E - H	0 / 788	-18.5	-18.5	0.21 (1)	10.00				
H - D	0 / 788	-18.5	-18.5	0.21 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-0-4	-877	-877	---	BACK	VERT	TOTAL	---	C1
G	1-0-4	-877	-877	---	BACK	VERT	TOTAL	---	C1
H	5-0-4	-880	-880	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 34 = 67 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.8	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.18")
CALCULATED VERT. DEFL.(LL) = L/989 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.18")
CALCULATED VERT. DEFL.(TL) = L/989 (0.01")

CSI: TO=0.07/1.00 (A-F:1) , BO=0.21/1.00 (D-E:1) , WB=0.21/1.00 (B-D:1) , SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.72 (A) (INPUT = 0.90)
JSI METAL = 0.18 (D) (INPUT = 1.00)



Structural component only
DWG# T-20071831/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T55	1	2	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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ID:K7TPdhai0npl1qlYdbWIOFzIdeG-dkKm30O3SzTJWH74mT9mG3DvHI 9dNX?Z9U 27zNB1L

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

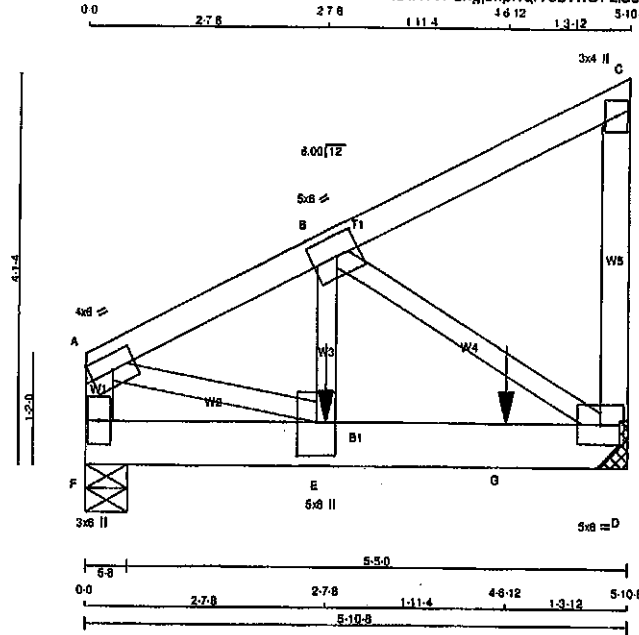


Structural component only
DWG# T-2007183

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408170	T56	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 13:02:33 2020 Page 1
ID:K?TPdngj0np1qYdbWIOFzIdeG-6wu8GMPIDGbA8RIGJAg?pHm498FsMnS8opDXbRzNB1K



Scale = 1/2" = 1'-0"

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
F - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A	12	TOP
A-C	12	TOP
C-D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D	5	SIDE(549.3)
WEBS : (0.122"x3") SPIRAL NAILS		
B-E	2	SIDE(259.5)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UOL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW-1	MT20	4.0	8.0		Edge
B TMVW-1	MT20	5.0	8.0	2.50	2.75
C TMV+p	MT20	3.0	4.0		
D BMVW-1	MT20	5.0	8.0		
E BMVW-1	MT20	5.0	8.0	4.25	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
F	2334	0	2334	0	0	0	5-8	5-8	
D	2733	0	2733	0	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4'-0".

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	1846	1103 / 0	0 / 0	0 / 0	0 / 0	0 / 0	543 / 0	0 / 0
D	1929	1290 / 0	0 / 0	0 / 0	0 / 0	0 / 0	639 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. UNBRAC
FR-TO		FROM	TO	LENGTH	FR-TO		
F-A	-2318 / 0	0.0	0.0	0.13 (1)	7.37	A-E	0 / 2898
A-B	-3101 / 0	-91.8	-91.8	0.08 (1)	5.20	E-B	0 / 2880
B-C	-5 / 0	-91.8	-91.8	0.08 (1)	10.00	B-D	-3303 / 0
D-C	-138 / 0	0.0	0.0	0.02 (1)	7.81		
F-E	0 / 0	-18.5	-18.5	0.01 (4)	10.00		
E-G	0 / 2779	-18.5	-18.5	0.57 (1)	10.00		
G-D	0 / 2779	-18.5	-18.5	0.57 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-7-8	-3101	-3101	---	BACK	VERT	TOTAL	---	C1
G	4-6-12	-1318	-1318	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH.	LL = 25.6 PSF
	DL = 8.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CSI: TC=0.13/1.00 (A-F:1), BC=0.57/1.00 (D-E:1),
WB=0.42/1.00 (B-D:1), SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	518	354	1697
	788	1987	1686

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.88 (E) (INPUT = 0.90)

JSI METAL=0.50 (E) (INPUT = 1.00)



Structural component only
DWG# T-2007184 1/2

CONTINUED ON PAGE 21

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	T56	1	2	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

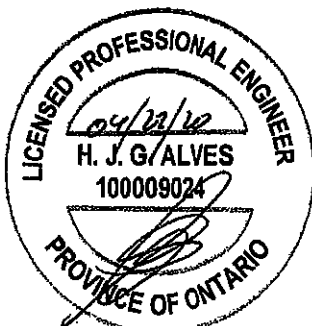
TRUSS DESC.

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ID:K?TPdhai0npl1aiYdbWIOFzldaG-6wu8GMPiDQbA8RiGJAa?plHm499FsmnS8ooDXbRzNB1k

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

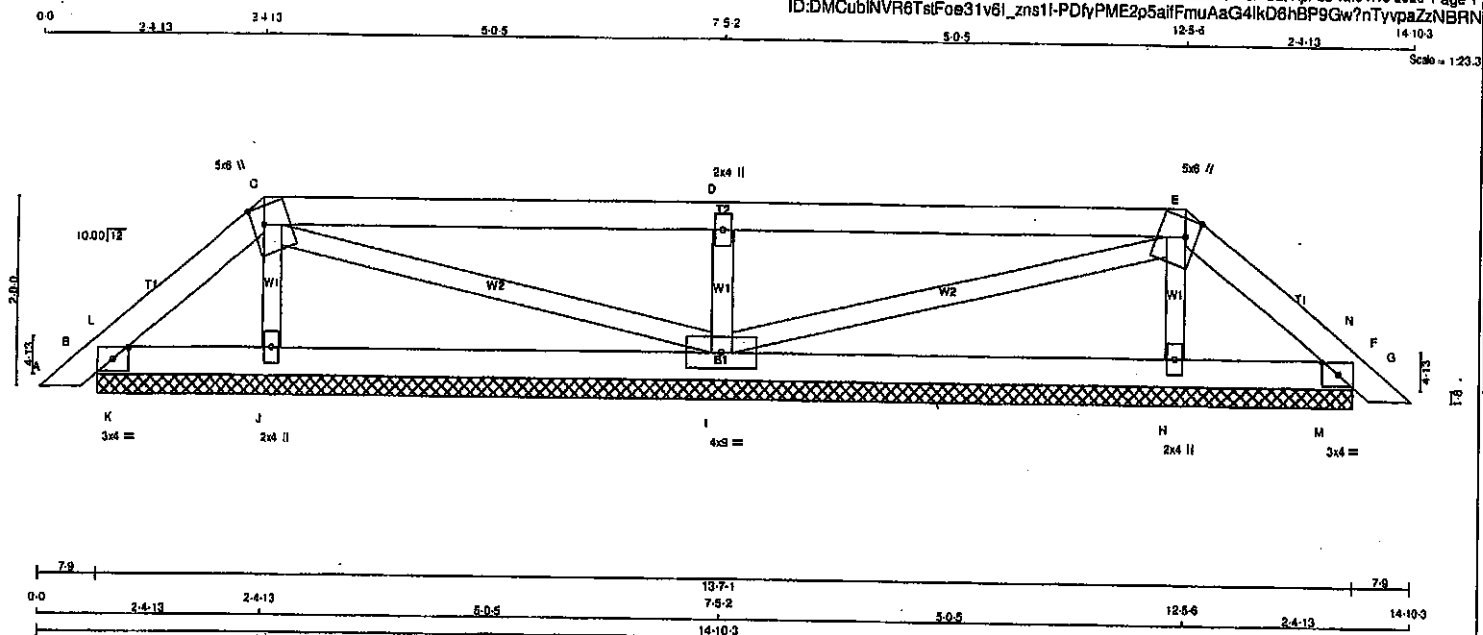
Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



Structural component only
DWG# T-2007184 3/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408168	PB1	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR8tstFoe31v6l_zns11-PDfyPME2p5aifFmuAaG4lkD8hBP9Gw7nTyvpaZzNBRN
12-5-4 2-4-13 14-10-3
Scale = 1:23.3



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES	A - C	2x4	DRY	No.2
	C - E	2x4	DRY	No.2
	E - G	2x4	DRY	No.2
	G - I	2x4	DRY	No.2
	I - M	2x4	DRY	No.2
	M - N	2x4	DRY	No.2
	ALL WEBS	2x3	DRY	No.2
	DRY: SEASONED LUMBER.			

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
	B	TMB1-I	MT20	3.0	4.0	1.50	2.00
	C	TTWW+m	MT20	5.0	6.0	2.25	1.50
	D	TMW+w	MT20	2.0	4.0		
	E	TTWW+m	MT20	5.0	6.0	2.25	1.50
	F	TMB1-I	MT20	3.0	4.0	1.50	2.00
	H	BMW1+w	MT20	2.0	4.0		
	I	BMWW1-t	MT20	4.0	9.0		
	J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	UP/LIFT	IN-SX	IN-SX	BRG
B	169	0	169	0	0	13-7-1	13-7-1	13-7-1	
F	169	0	169	0	0	13-7-1	13-7-1	13-7-1	
J	288	0	288	0	0	13-7-1	13-7-1	13-7-1	
I	683	0	683	0	0	13-7-1	13-7-1	13-7-1	
H	288	0	288	0	0	13-7-1	13-7-1	13-7-1	

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	118	93/0	0/0	0/0	0/0	23/0	0/0
F	118	93/0	0/0	0/0	0/0	23/0	0/0
J	207	118/0	0/0	0/0	0/0	89/0	0/0
I	461	327/0	0/0	0/0	0/0	154/0	0/0
H	207	118/0	0/0	0/0	0/0	89/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD	CS1 (LC)	MEMB.	MAX. FORCE (LBS)	CS1 (LC)	MAX. FORCE (LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/14	-91.8	-91.8 0.02 (1)	10.00	J-C	-194/0	0.03 (1)
B-L	-31/4	-91.8	-91.8 0.01 (1)	6.25	C-I	-22/0	0.01 (1)
L-C	-80/0	-91.8	-91.8 0.03 (1)	6.25	I-D	-576/0	0.08 (1)
C-D	-8/0	-91.8	-91.8 0.39 (1)	10.00	I-E	-22/0	0.01 (1)
D-E	-8/0	-91.8	-91.8 0.39 (1)	10.00	H-E	-184/0	0.03 (1)
E-N	-80/0	-91.8	-91.8 0.03 (1)	6.25	K-L	-121/0	0.00 (1)
N-F	-31/4	-91.8	-91.8 0.01 (1)	6.25	M-N	-121/0	0.00 (1)
F-G	0/14	-91.8	-91.8 0.02 (1)	10.00			
B-K	0/42	-18.5	-18.5 0.04 (1)	10.00			
K-J	0/42	-18.5	-18.5 0.07 (4)	10.00			
J-I	0/29	-18.5	-18.5 0.10 (4)	10.00			
I-H	0/29	-18.5	-18.5 0.10 (4)	10.00			
H-M	0/42	-18.5	-18.5 0.07 (4)	10.00			
M-F	0/42	-18.5	-18.5 0.04 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(\$5 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.39/1.00 (C-D:1), BC=0.10/1.00 (H-K), WB=0.09/1.00 (D-I:1), SS=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

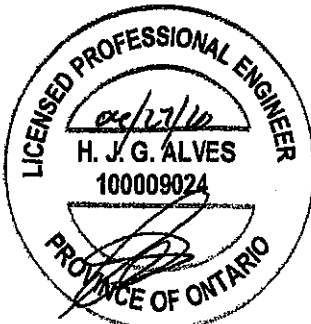
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(P.S.) (P.L.) (P.L.)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

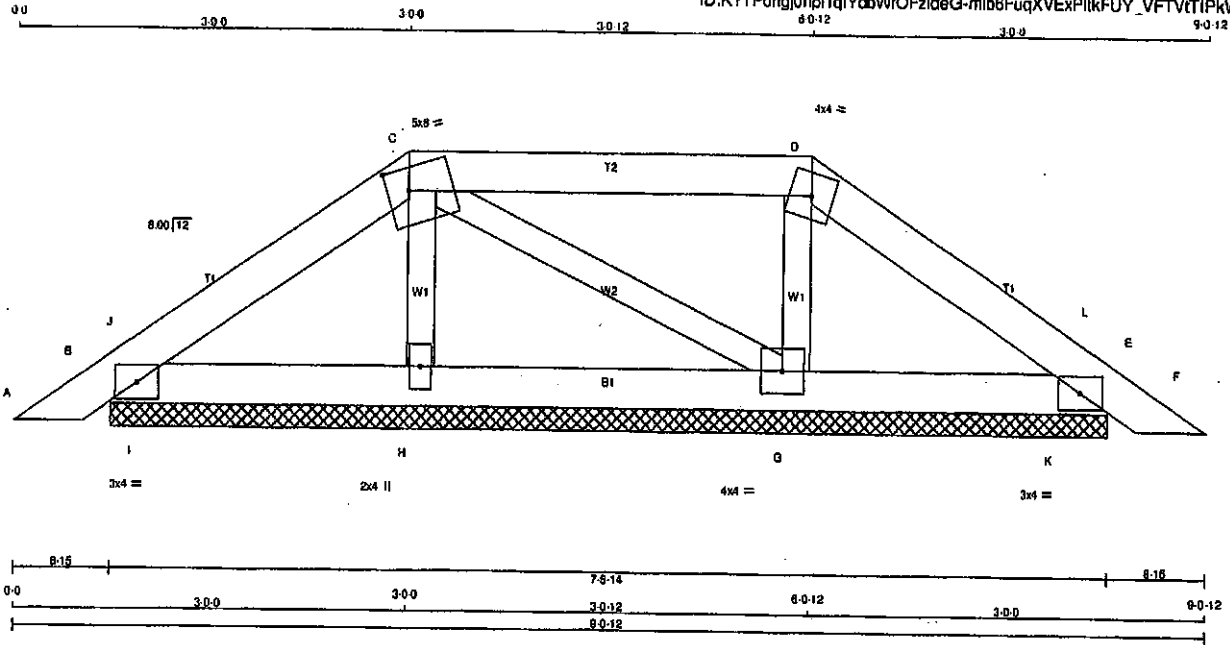
JSI GRIP= 0.30 (D) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408169	PB20	3	1	GREEN PARK HOMES	
TRUSS DESC.					

Tamrack Roof Truss, Burlington

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Scale = 1/8" = 1'-0"

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-1	MT20	3.0	4.0
C	TTWW-m	MT20	5.0	8.0 2.00 2.00
D	TTW-m	MT20	4.0	4.0
E	TMB1-1	MT20	3.0	4.0
G	BMWW1-1	MT20	4.0	4.0
H	BMW1-w	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
B	221	0	221	0	0	7-6-14	7-6-14	7-6-14	7-6-14
E	209	0	209	0	0	7-6-14	7-6-14	7-6-14	7-6-14
H	248	0	248	0	0	7-6-14	7-6-14	7-6-14	7-6-14
G	280	0	280	0	0	7-6-14	7-6-14	7-6-14	7-6-14

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	154	113 / 0	0 / 0	0 / 0	0 / 0	0 / 0	41 / 0	0 / 0
E	148	108 / 0	0 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0
H	178	109 / 0	0 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
G	198	127 / 0	0 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX (LBS)	MEMB.	FORCE (LBS)	MAX (LBS)	MAX (LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 15	-91.8	-91.8 0.03 (1)	H-C	-178 / 0	0.03 (1)	10.00
B-J	-83 / 0	-91.8	-91.8 0.01 (1)	C-G	-21 / 0	0.00 (1)	6.25
J-C	-74 / 0	-91.8	-91.8 0.05 (1)	G-D	-188 / 0	0.03 (1)	6.25
C-D	-30 / 0	-91.8	-91.8 0.15 (1)	I-J	-121 / 0	0.00 (1)	6.25
D-L	-54 / 0	-91.8	-91.8 0.05 (1)	K-L	-123 / 0	0.00 (1)	6.25
L-E	-41 / 0	-91.8	-91.8 0.01 (1)				6.25
E-F	0 / 15	-91.8	-91.8 0.03 (1)				10.00
B-I	0 / 60	-18.5	-18.5 0.08 (1)				10.00
I-H	0 / 60	-18.5	-18.5 0.08 (1)				10.00
H-G	0 / 48	-18.5	-18.5 0.03 (1)				10.00
G-K	0 / 43	-18.5	-18.5 0.08 (1)				10.00
K-E	0 / 43	-18.5	-18.5 0.06 (1)				10.00

TOTAL WEIGHT = 3 X 25 = 75 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (C-D:1), BC=0.06/1.00 (B-I:1), WB=0.03/1.00 (D-G:1), SSI=0.11/1.00 (C-D:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788 1997 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.18 (B) (INPUT = 0.90)
JSI METAL = 0.04 (B) (INPUT = 1.00)



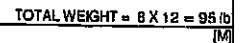
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DWG# T-2007144

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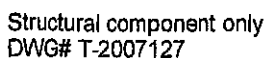
Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Sat Apr 25 12:34:44 2020 Page 1

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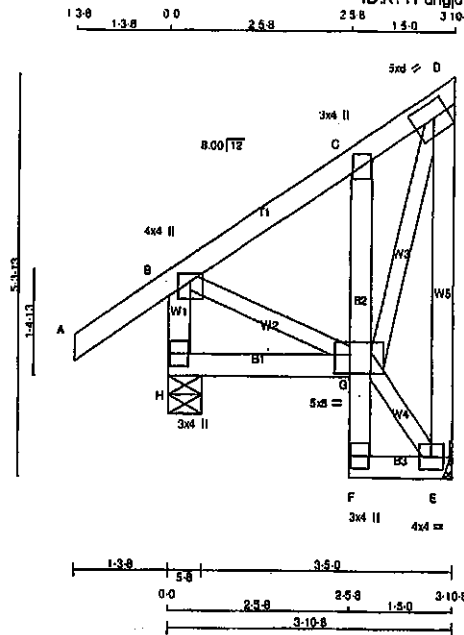
JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408169	J20S	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

IDK?TPdngl0npl1qYdbWfOFzIdG-I21k2YpVoxpY7j92wrSGiGzIsV3mg3ZgbMdnLzNBd



Scale = 1/2" = 1'-0"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
F - C	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT E - D	2x3	DRY	No.2	SPF	
	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMV+p	MT20	3.0	4.0		
D	TMVW-1	MT20	5.0	6.0	2.50	2.50
E	BMVW-1	MT20	4.0	4.0		
F	BMV+p	MT20	3.0	4.0		
G	BMVW-1	MT20	5.0	6.0	3.00	2.50
H	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
H	332	0	332	0
E	206	0	206	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1'-8".

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	232	166 / 0	0 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0
E	145	95 / 0	0 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	WEBS	MAX. FACTORED	MAX
		FORCE	VERT.	(PLF)	CSI (LC)	UNBRAC	LENGTH			FORCE	MAX
FR-TO								FR-TO			
H-B	-307 / 0	0.0	0.0	0.03 (1)	7.81	B-G	0 / 83	0.02 (1)			
A-B	0 / 35	-91.8	-91.8	0.14 (5)	10.00	E-D	-184 / 0	0.05 (1)			
B-C	-83 / 0	-91.8	-91.8	0.07 (1)	6.25	G-E	-14 / 0	0.00 (1)			
C-D	-100 / 0	-91.8	-91.8	0.06 (1)	6.25	G-D	0 / 220	0.05 (1)			
H-G	0 / 0	-18.5	-18.5	0.04 (4)	10.00						
F-G	0 / 12	0.0	0.0	0.02 (1)	10.00						
G-C	-218 / 0	0.0	0.0	0.02 (1)	7.81						
F-E	0 / 9	-18.5	-18.5	0.01 (4)	10.00						

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 29 = 86 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (A-B-5), BC=0.04/1.00 (G-H-4), WB=0.05/1.00 (D-E-1), SS=0.09/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

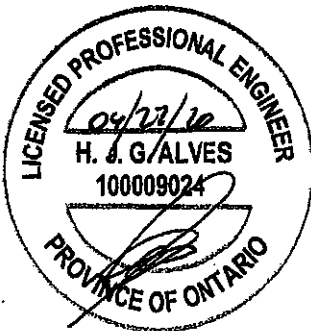
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1687
	788	1687	1688

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.07 (C) (INPUT = 1.00)



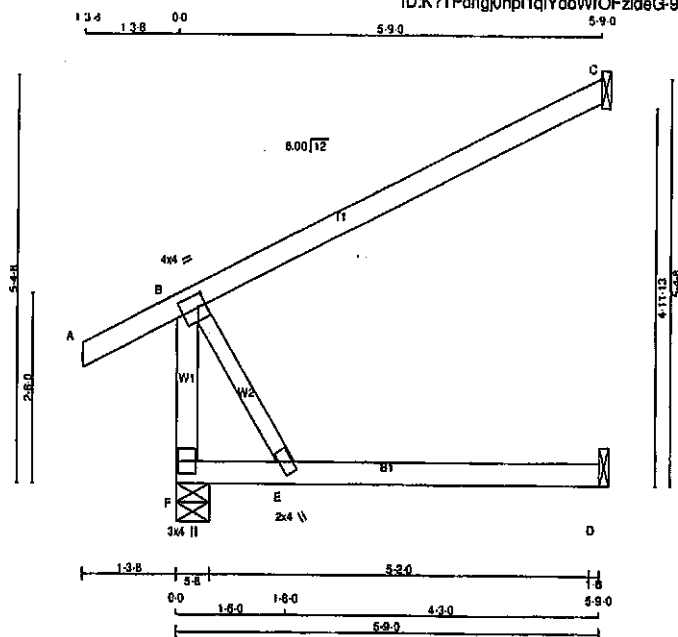
Structural component only
DWG# T-2007143

JOB NAME 408170	TRUSS NAME J40	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tangerack Roof Truss, Burlington

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Scale = 1:20.2



LUMBER	N. L. G. A. RULES	CHOROS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
F	441	0	441	0	0	5-8	5-8		
C	284	0	284	0	0	1-8	1-8		
D	53	0	60	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	310	217/0	0/0	0/0	0/0	0/0	93/0	0/0
C	182	147/0	0/0	0/0	0/0	0/0	35/0	0/0
D	43	0/0	0/0	0/0	0/0	0/0	43/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	MAX	LC1
FR-TO				FR-TO			
F-B	-388/0	0.0	0.0	0.05 (1)	7.81	0/0	0.00 (1)
A-B	0/28	-91.8	-91.8	0.12 (1)	10.00		
B-C	0/0	-91.8	-91.8	0.52 (1)	10.00		
F-E	0/0	-18.5	-18.5	0.14 (4)	10.00		
E-D	0/0	-18.5	-18.5	0.18 (4)	10.00		

TOTAL WEIGHT = 20 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(58 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.05")

CSI: TC=0.52/1.00 (B-C:1), BC=0.18/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.24 (B) (INPUT = 0.80)
JSI METAL = 0.07 (B) (INPUT = 1.00)



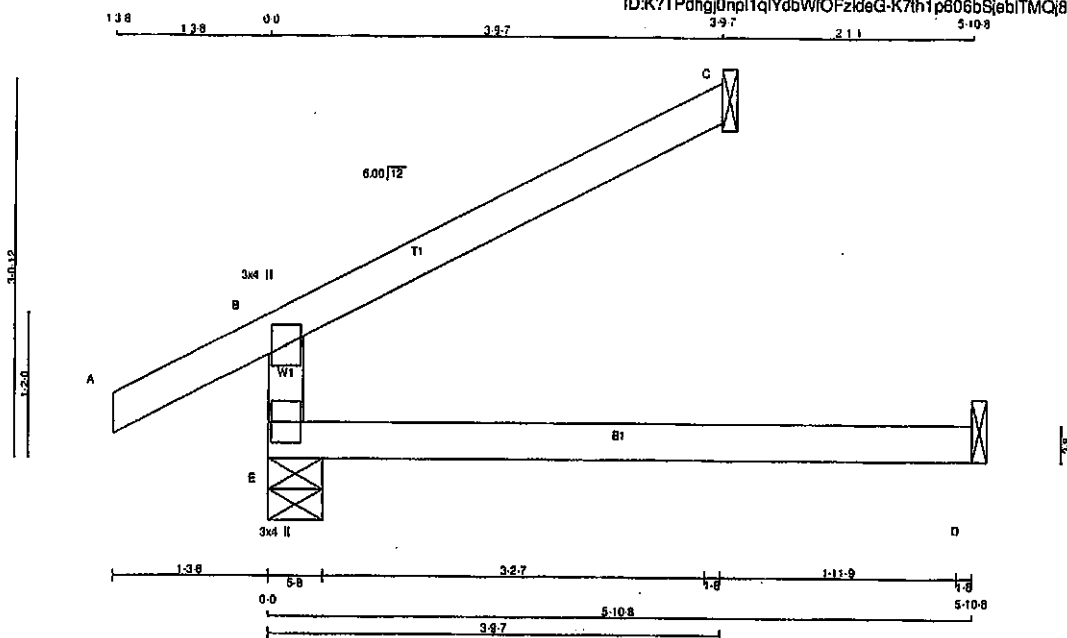
Structural component only
DWG# T-2007167

JOB NAME 408170	TRUSS NAME C40	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Targarak Roof Truss, Burlington

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Scale = 1:17.9



LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	No.2	SPF		
A - C	2x4 DRY	No.2	SPF		
E - D	2x4 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	405	0	405	0	0	5-8	5-8
C	130	0	130	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	288	190 / 0	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	38	0 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX. CS1 (LC)
FR-TO						
E-B	-342/0	0.0	0.0	0.13 (4)	7.81	
A-B	0/28	-91.8	-91.8	0.12 (1)	10.00	
B-C	-19/0	-91.8	-91.8	0.22 (1)	6.25	
E-D	0/0	-18.5	-18.5	0.13 (4)	10.00	

TOTAL WEIGHT = 3 X 14 = 42 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 8.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2019, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.08")

CS1: TC=0.22/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS1=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLJ)	(PLJ)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 788	1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.80)
JSI METAL= 0.09 (B) (INPUT = 1.00)

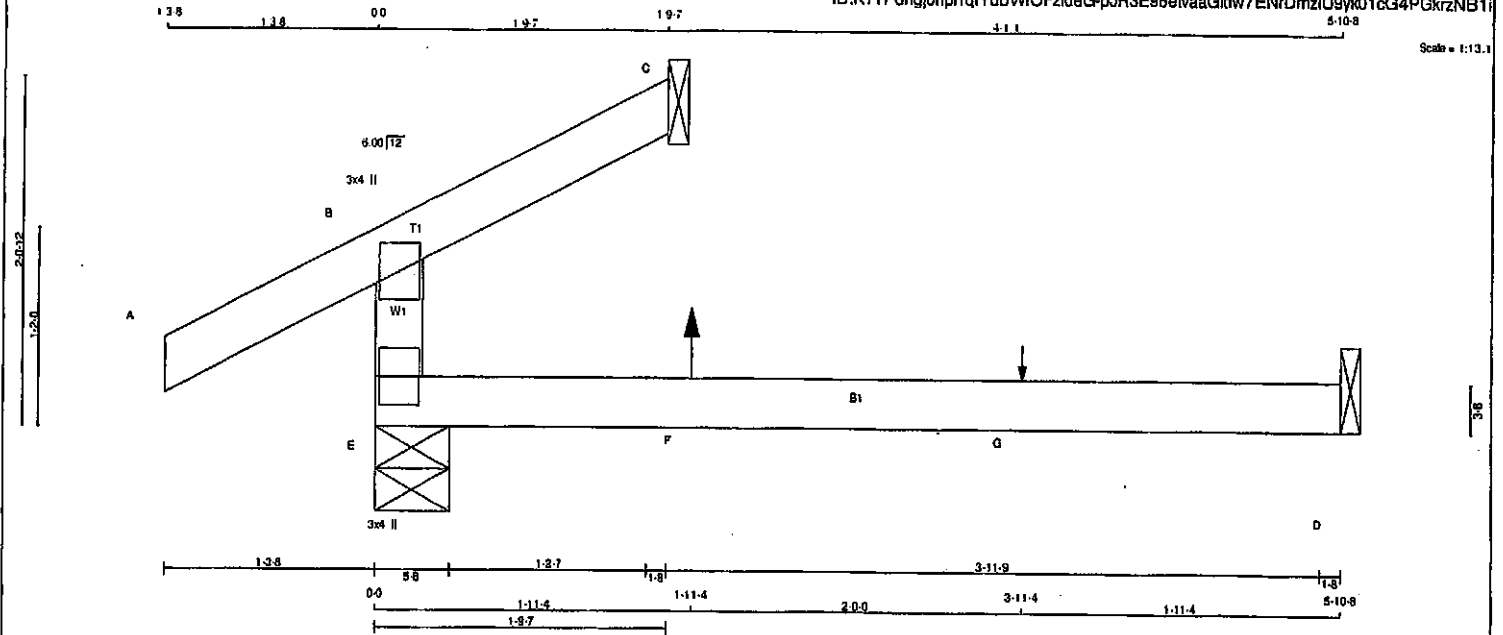


Structural component only
DWG# T-2007162

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408170	C41	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
E	BMV1+p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT	HORZ	DOWN	HORZ	UPLIFT
E	284	0	284	0
C	63	0	63	0
D	44	0	52	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS	1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
E	200	137 / 0
C	48	21 / 0
D	35	0 / 3

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS	MAX. FACTORED	FACTORED	W E B S	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	UNBRAC LENGTH
FR-TO		FROM	TO	FR-TO
E-B	-227 / 0	0.0	0.0	0.11 (4)
A-B	0 / 28	-91.8	-91.8	0.12 (1)
B-C	-9 / 9	-91.8	-91.8	0.08 (4)
E-F	0 / 0	-18.5	-18.5	0.14 (4)
F-G	0 / 0	-18.5	-18.5	0.14 (4)
G-D	0 / 0	-18.5	-18.5	0.14 (4)

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11.4	7	1	12	BACK	VERT	TOTAL	--	--	C1
G	3-11.4	1	1	---	BACK	VERT	TOTAL	--	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 3 X 12 = 35 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/899 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/899 (0.04")

CS1: TC=0.12/1.00 (A-B:1), EC=0.14/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS=0.09/1.00 (A-B:1)

DOL LUMBER=0.98 NAIL=0.98 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1887 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.10 (E) (INPUT = 0.90)
JSI METAL = 0.08 (B) (INPUT = 1.00)



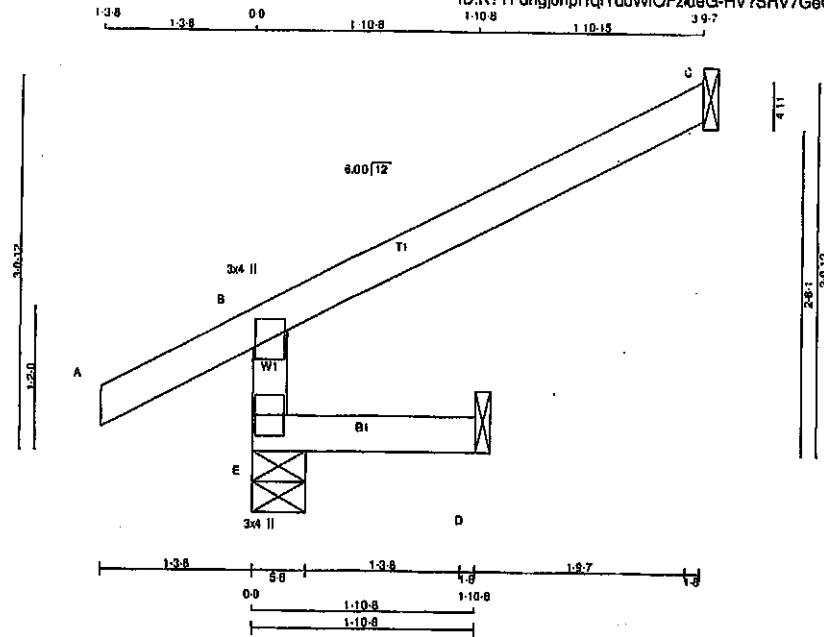
Structural component only
DWG# T-2007163

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	C42	3	1	GREEN PARK HOMES	

Tarapack Roof Truss, Burlington

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ID:K?TPdhgJ0npl1qYdbWOFzdeG-HV?SRV7GeCIRuvRrTricNQJ6SuW4TTHmVksPHHzNB1h

Scale = 1:16.2



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	ORY	No.2	SPF	
A - C	2x4	ORY	No.2	SPF	
E - D	2x4	ORY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	361	0	361	0	5-8	5-8
C	130	0	130	0	1-8	1-8
D	16	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX/MIN COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	250	190 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (6)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		FR-TO			
E-B	-342/0	0.0	0.0	0.01 (4)			
A-B	0/28	-91.8	-91.8	0.13 (5)			
B-C	-19/0	-91.8	-91.8	0.22 (1)			
E-D	0/0	-18.5	-18.5	0.02 (4)			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 10 = 29 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.0	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TG=0.22/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1887 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.14 (E) (INPUT = 0.90)
JSI METAL = 0.09 (B) (INPUT = 1.00)



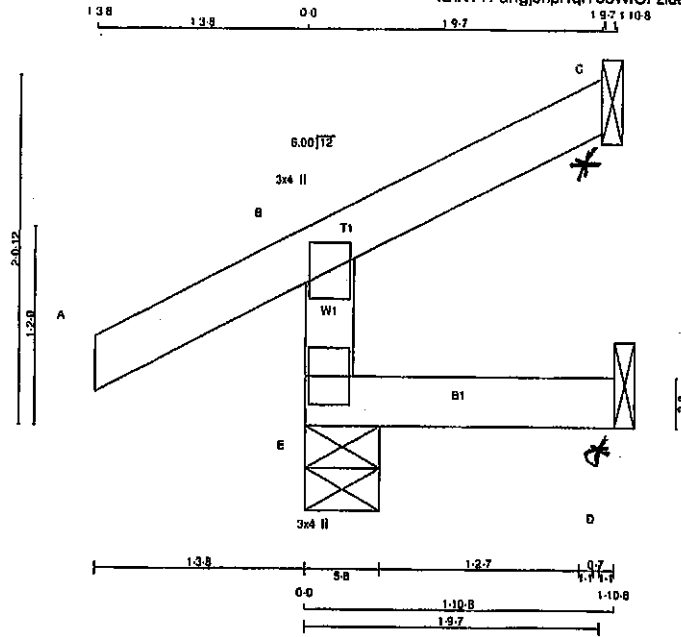
Structural component only
DWG# T-2007164

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408170	C43	3	1	GREEN PARK HOMES	
Tangarak Roof Truss, Burlington				TRUSS DESC.	

Tangarak Roof Truss, Burlington

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ID:K?TPdhgJ0npl1qYdbWlOFzIdeG-lZqr8uPWqIV2011YHrweJqls_CwXvkOuNpJzNB1g

Scale = 1:12.1



LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	SPF
CHORDS	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y
B	TMVap	MT20	3.0	4.0	X
E	BMV1+p	MT20	3.0	4.0	X

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INFLIT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	271	0	271	0	5-8	5-8
C	45	0	45	0	1-8	1-8
D	8	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM	LIVE			
E	188	141/0	0/0	0/0	0/0	47/0	0/0
C	31	24/-18	0/0	0/0	0/0	7/0	0/0
D	7	0/-8	0/0	0/0	0/0	12/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED UNBRAC LENGTH FR-TO	MAX. FACTORED CSI (LC)
FR-TO				
E-B	-244/0	0.0	0.0	0.04 (5)
A-B	0/28	-91.8	-91.8	0.12 (1)
B-C	-17/0	-91.8	-91.8	0.09 (1)
E-D	0/0	-18.5	-18.5	0.04 (5)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 7 = 21 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5),
WB=0.00/1.00 (n/a:0), SS=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 788	1987 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007165

LUL/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

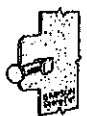
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



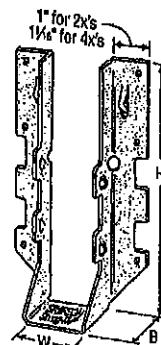
Double-Shear
Nailing
Top View



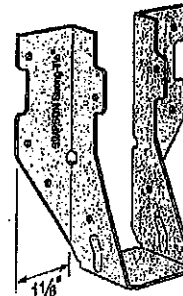
Double-Shear
Nailing
Side View;
Do not
bend tab



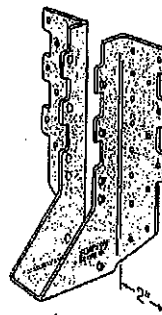
Dome Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,803,580



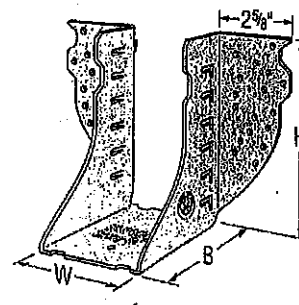
LUS28



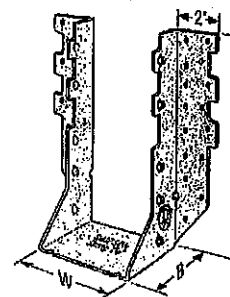
LU26L



HUS210
(HUS26, HUS28,
and HHUS similar)

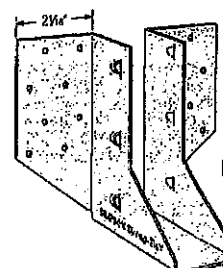
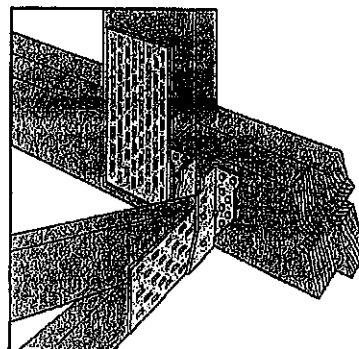


HGUS28-2



HHUS210-2

Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LJS26DS

Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS**HHUS/HGUS**

See Hanger Options information on pp. 125-127.

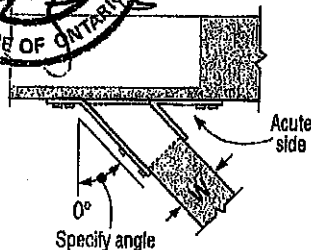
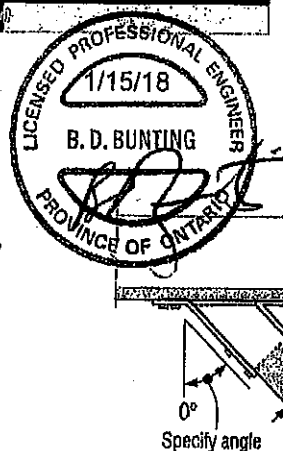
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.82 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



**Top View HHUS Hanger
Skewed Right**
(joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _g ³	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15) lb. kN	(K _D = 1.00) lb. kN	(K _D = 1.15) lb. kN	(K _D = 1.00) lb. kN
Single 2x Sizes											
LUS24	18	1 1/8	3 1/4	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1626	645	1155
								3.16	7.23	2.87	5.14
								360	1020	320	725
LU24L	22	1 1/8	3	1 1/4	2 1/8	(4) 10d	(2) 10d x 1 1/2"	1.60	4.54	1.42	3.22
								720	1806	645	1140
								3.20	7.14	2.87	5.07
LUS26	18	1 1/8	4 1/4	1 1/4	3 1/4	(4) 10d	(4) 10d	1420	2170	1290	1630
								6.32	9.85	5.74	7.25
								2705	4940	2065	3875
HUS28	16	1 1/4	5 1/4	3	3 3/8	(14) 16d	(6) 16d	11.80	21.97	9.20	17.24
								2055	4265	1460	4115
								9.14	18.97	6.49	18.31
LJS26DS	18	1 1/8	5	3 1/4	4 1/4	(16) 16d	(6) 16d	2885	6826	2685	5700
								11.96	29.51	11.96	25.35
								1140	2185	1020	1550
LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"	5.07	9.72	4.54	6.89
								1420	2520	1290	1790
								6.32	11.21	5.74	7.96
LUS28	18	1 1/8	6 1/4	1 1/4	3 3/4	(6) 10d	(4) 10d	3805	5365	2675	4345
								16.04	23.86	11.90	19.33
								3310	7675	3310	6800
HGUS28	12	1 1/4	7 1/4	5	6 1/4	(36) 16d	(12) 16d	14.74	34.19	14.74	30.73
								1140	2495	1020	1770
								5.07	11.10	4.54	7.87
LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(6) 10d x 1 1/2"	1420	2785	1290	2210
								6.32	12.39	5.74	9.83

1. Factored uplift resistances have been increased 16% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_g is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/4" long. See pp. 27-28 for other nail sizes and information.

Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _a	Header	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _p = 1.15) lb.	(K _p = 1.00) lb.	(K _p = 1.15) lb.	(K _p = 1.00) lb.
Double 2x Sizes											
LUS24-2	18	3 1/8	3 1/8	2	1 1/2	(4) 16d	(2) 16d	835	2020	590	1435
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	3.71	8.99	2.62	6.38
								1720	2595	1545	1920
HHUS26-2	14	3 1/8	5 1/4	3	3 1/8	(14) 16d	(6) 16d	7.65	11.54	6.87	8.54
								2850	7335	2085	5205
HGUS26-2	12	3 1/8	5 1/8	4	4 1/4	(20) 16d	(8) 16d	12.68	32.63	9.20	23.15
								4385	8950	3110	6355
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	19.51	39.81	13.83	28.27
								1720	3325	1545	2575
HHUS28-2	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	7.65	14.79	6.87	11.45
								3765	8940	2675	6345
HGUS28-2	12	3 1/8	7 1/8	4	8 1/8	(36) 16d	(12) 16d	16.75	39.77	11.90	28.22
								6070	12980	4310	9215
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	27.00	57.74	19.17	40.99
								2580	4500	2320	3195
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	11.48	20.02	10.32	14.21
								4670	9660	4235	7000
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	28.77	42.97	18.84	31.14
								6840	14015	4855	10270
								30.43	62.34	21.60	45.60
Triple 2x Sizes											
HGUS26-3	12	4 1/8	5 1/2	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-3	12	4 1/8	7 1/4	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d	4670	9670	4235	6865
								20.77	43.02	18.84	30.54
HGUS210-3	12	4 1/8	9 1/4	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	6 1/8	5 1/8	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-4	12	6 1/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-4	14	6 1/8	8 1/8	3	7 1/8	(30) 16d	(10) 16d	4670	10155	4235	7210
								20.77	45.17	18.84	32.07
HGUS210-4	12	6 1/8	9 3/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.08	72.95	32.00	51.80
4x Sizes											
LUS46	18	3 1/8	4 1/4	2	3 1/8	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS46	14	3 1/8	5 1/4	3	3 1/8	(14) 16d	(6) 16d	2540	7335	2085	5205
								11.30	32.63	9.20	23.15
HGUS46	12	3 1/8	5 1/4	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS48	18	3 1/8	6 1/4	2	3 1/8	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS48	14	3 1/8	7 1/4	3	6 1/4	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS48	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS410	18	3 1/8	8 1/4	2	5 1/8	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HGUS410	12	3 1/8	9	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.60
HGUS412	12	3 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS414	12	3 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.08	72.95	32.00	51.80



Plated Truss Connectors

See footnotes
on p. 258.

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1 1/4". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G80 galvanized

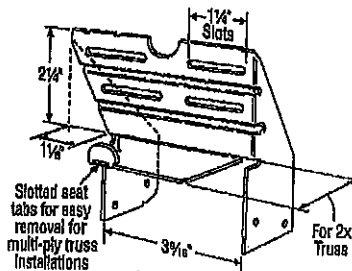
Design: Factored resistances are in accordance with CSA 086-14

Installation:

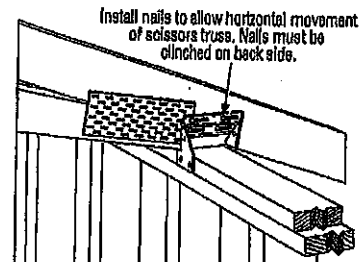
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

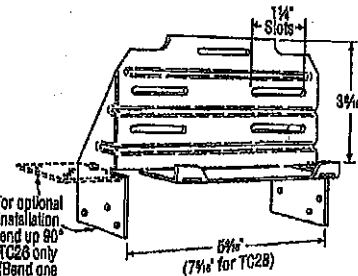
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



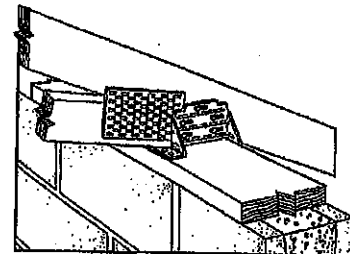
TC24
U.S. Patent 4,932,173



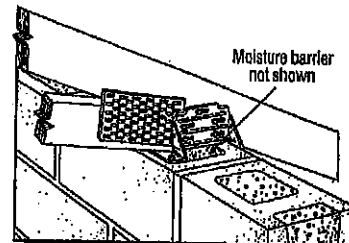
Typical TC24 Installation



TC26
(TC28 Similar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift ($K_p=1.15$) lb.	Uplift ($K_p=1.15$) lb.
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift ($K_p=1.15$) lb.	Uplift ($K_p=1.15$) lb.
TC26	(5) 10d	(6) 10d x 1 1/2"	810	680
	(5) 10d	(6) 10d	930	680

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC28 installation with 10d nails requires minimum 3" top plate thickness.
4. TC28 fastened to grouted concrete block with (6) - 3/8" x 2 1/4" Titen screws has a factored uplift resistance of 276 lb.



DESIGN

(800) 999-5099
strongtie.com

H/TSP

Seismic and Hurricane Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Fasteners			Factored Resistance ($K_D = 1.15$)					
					D-Fir-L			S-P-F		
		To Rafter/Truss	To Plates	To Studs	Uplift	Lateral		Uplift	Lateral	
						F ₁	F ₂		F ₁	F ₂
					lb.	lb.	lb.	lb.	lb.	lb.
					kN	kN	kN	kN	kN	kN
H1	18	(6) 8d x 1 1/2"	(4) 8d	—	740	685	300	680	485	215
					3.29	3.05	1.33	3.02	2.16	0.96
SS H2A	18	(5) 8d x 1 1/2"	(2) 8d x 1 1/2"	(5) 8d x 1 1/2"	830	220	75	590	155	55
					3.69	0.98	0.33	2.62	0.69	0.24
SS H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
					3.58	0.71	0.71	3.36	0.71	0.71
H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
					3.71	0.78	0.93	3.29	0.71	0.93
SS H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
					3.29	0.80	1.18	2.74	0.56	0.85
H6	16	—	(8) 8d	(8) 8d	1585	1085	—	1125	770	—
					7.05	4.83	—	5.00	3.43	—
H7Z	16	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
					6.18	2.98	—	4.40	2.11	—
SS H8 ³	18	(6) 10d x 1 1/2"	(5) 10d x 1 1/2"	—	1120	—	—	1025	—	—
					4.98	—	—	4.56	—	—
SS H10A ⁹	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1735	795	410	1505	565	290
					7.72	3.54	1.82	6.69	2.51	1.29
H10AR	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1485	690	430	1220	570	305
					6.61	3.07	1.91	5.43	2.54	1.36
H10A-2	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1835	1275	430	1645	880	305
					8.16	5.67	1.91	7.32	3.91	1.36
H10S ^{7,10}	18	(8) 8d x 1 1/2"	(8) 8d x 1 1/2"	(8) 8d	1465	795	315	1040	565	225
					6.52	3.54	1.40	4.63	2.51	1.00
H11Z	18	(6) 16d x 2 1/4"	(6) 16d x 2 1/4"	—	1095	920	545	780	655	390
					4.87	4.09	2.42	3.47	2.91	1.73
H14	18	[1] (12) 8d x 1 1/2"	(13) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
		[2] (12) 8d x 1 1/2"	(15) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
TSP	16	(9) 10d x 1 1/2"	(6) 10d x 1 1/2"	—	1295	440	—	920	310	—
					5.76	1.98	—	4.09	1.38	—
		(9) 10d x 1 1/2"	(6) 10d	—	1560	440	—	1105	310	—
					6.94	1.98	—	4.92	1.38	—

- Factored resistances have been increased 15% for short term loading; no further increase is allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2 1/2" must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
- H8 factored uplift resistances for stud-to-bottom plate installations are 595 lb. (2.65 kN) for D-Fir-L and 390 lb. (1.74 kN) for S-P-F.
- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-HTIECONPATH).
- Factored resistances in the F₁ direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.

- H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.38 kN) D-Fir-L and 1015 lb. (4.51 kN) S-P-F.
- H10S nails to plates are optional for uplift but required for lateral loads.
- H10A may be field-bent up to a slope of 8/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
- The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SNAILS at strongtie.com.
- D-Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1 1/2" nails into the top plates and (3) 8d x 1 1/2" nails into the lowest three flange holes into the truss bottom chord is 495 lb. (2.20 kN).
- Nails: 16d x 2 1/4" = 0.182" dia. x 2 1/4" long, 10d = 0.148" dia. x 3" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 8d = 0.131" dia. x 2 1/2" long, 8d x 1 1/2" = 0.131" dia. x 1 1/2" long. See pp. 27-28 for other nail sizes and information.

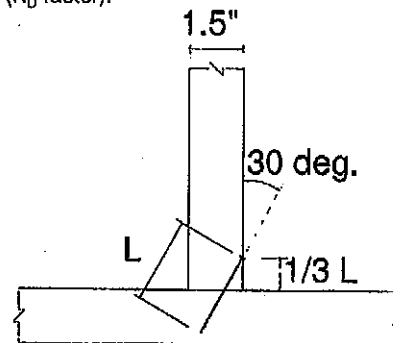
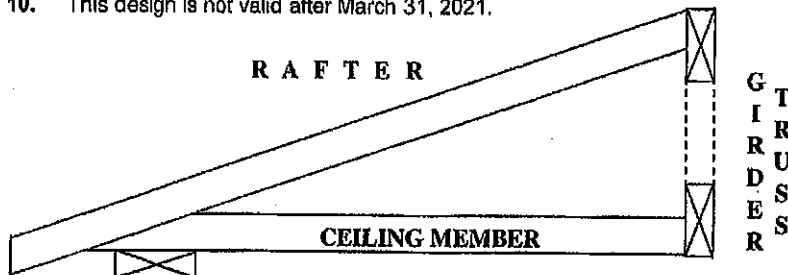
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.



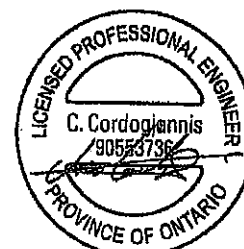
TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019

PEO
Certificate No. 10889485



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

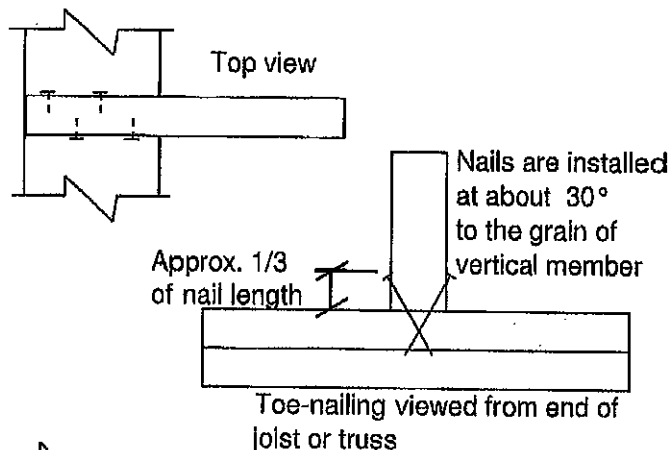
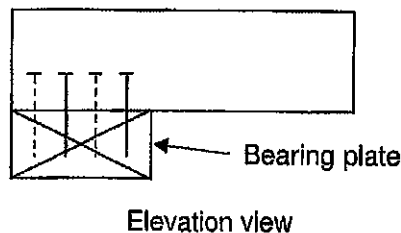
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

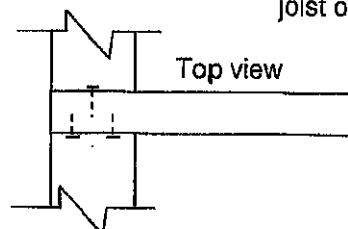
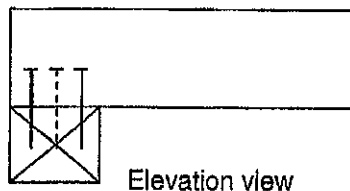
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to wind or earthquake load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



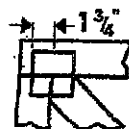
MiTek® MiTek Canada Inc
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Bradford, Ontario L3Z 3G7



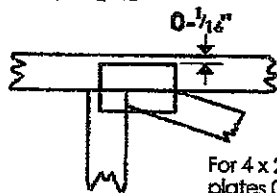
December 2, 2019

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint, unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

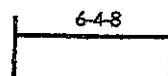


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

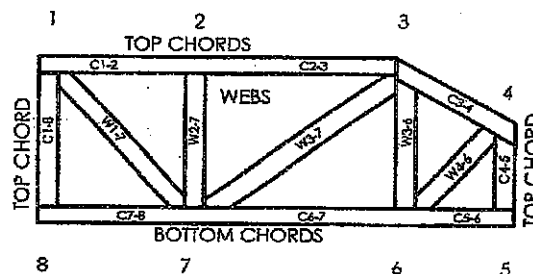
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-08



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See 8CSL.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

TECH-NOTES

ONTARIO WOOD TRUSS
FABRICATORS ASSOCIATION

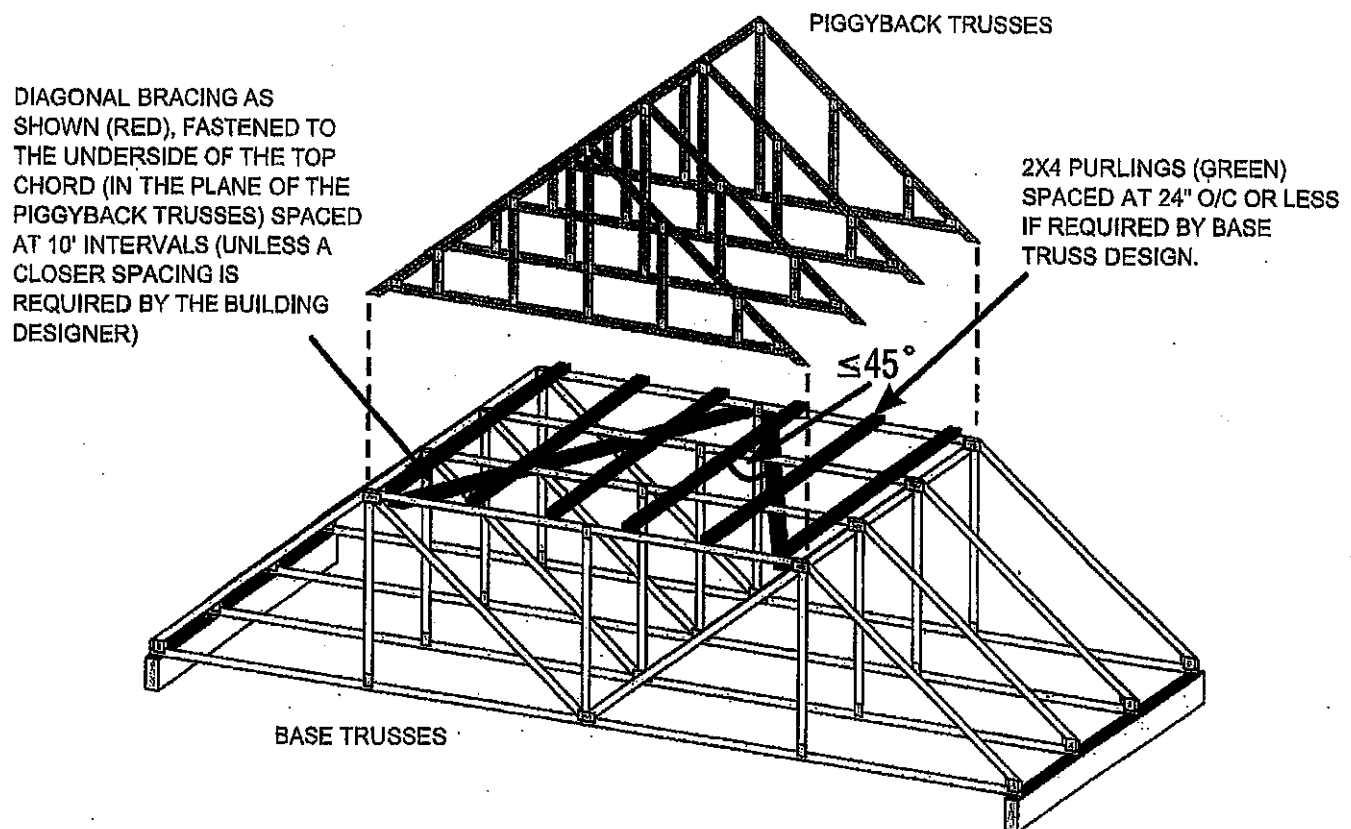
TN 15-001 Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

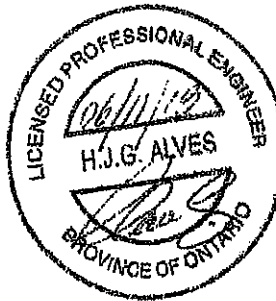


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.



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RESPONSABILITIES

- 1-Alves Engineering Services Inc. Is responsible for the design of trusses as individual components
- 2-It Is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

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