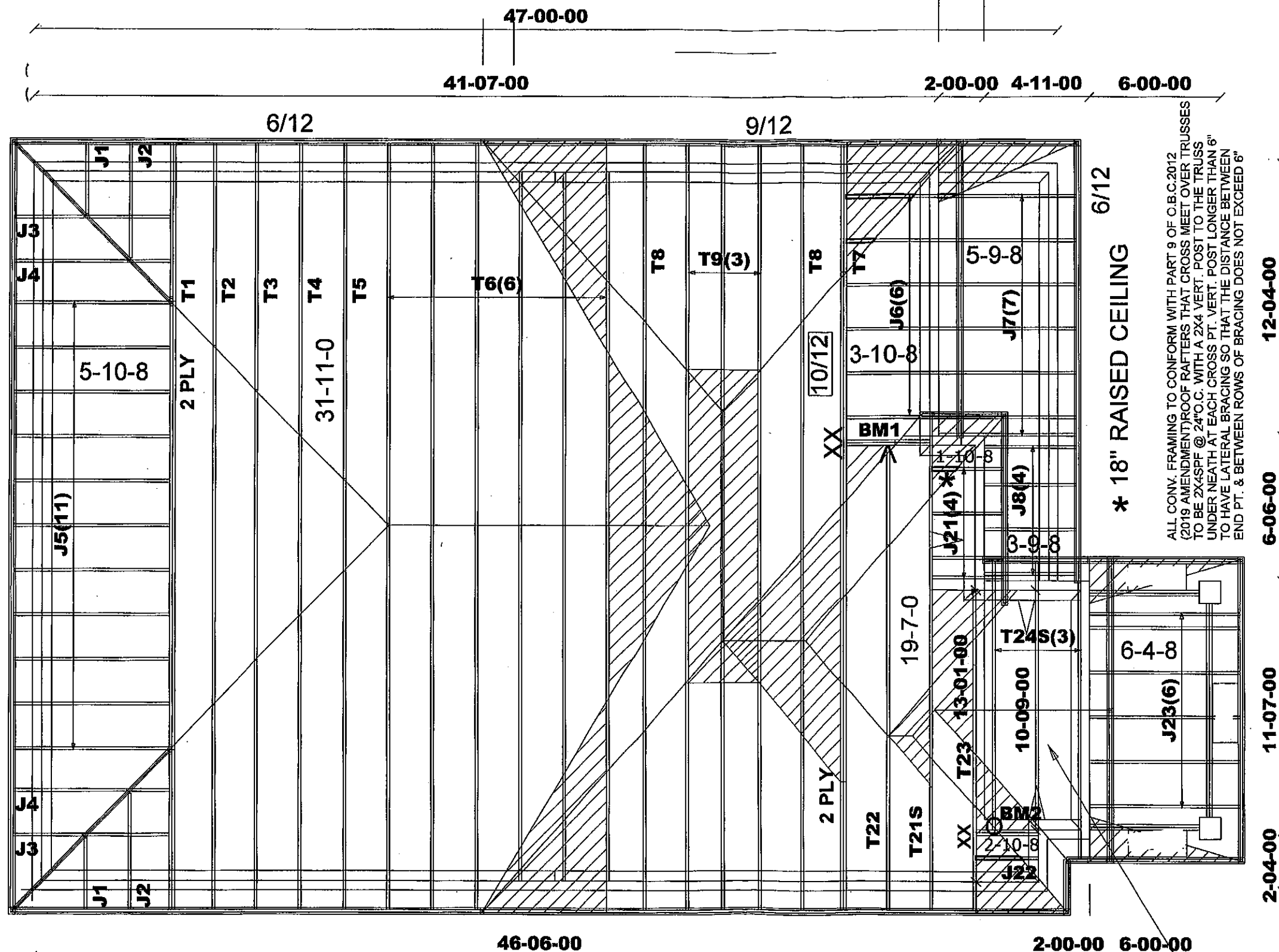


32-09-00



ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C.2012
(2019 AMENDMENT) ROOF RAFTERS THAT CROSS MEET OVER TRUSSES
TO BE 2X4SPF @ 24\"/>

ASPHALT SHINGLES
12\"/>

HARDWARE
LUS24 - (10)
LJS26DS - (12) - 1575
LUS26-2 - (10)
HGUS26-2 - (XX)
CITY OF HAMILTON
Building Division
Permit No. 18-1575
THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE
THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH
THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW
These drawings and/or specifications have been reviewed by
DATE 1/21

DENOTES
CONV.
FRAMING



DESIGN CONFORMS
WITH O.B.C.2012 PART 9
(2019 AMENDMENT)
DESIGN LOADS:
SNOW LOAD 25.6 PSF
TC DEAD 6 PSF
BC LIVE 0 PSF
BC DEAD 7.4 PSF

BM1, BM2 = 2-2X10

CATHEDRAL CEILING
INSIDE PITCH 6/12

M13108



Job Track: 51225

Plan Log: 202400

Layout ID: 408147

Builder / Location:

GREEN PARK HOMES / WATERDOWN

Project: RUSSELL GARDENS PH.3

Date: 2020-04-14

Sales: Mario DiCano

Designer: AC/JC

Model / Elevation:

VALLEYCREEK 2/2

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR
REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY
TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Mike ver 8.3.1.215

LOT 324

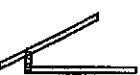
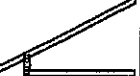
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST			
	Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: RUSSELL GARDENS PH.3 Location: WATERDOWN Model: VALLEYCREEK 2 Lot #: 324 Elevation: 2	Job Track: 51225 PlanLog: 202400 Layout ID: 408147 Ref #: Page: 1 of 2 Date: 04-14-2020 Designer: Andrew Conway Sales Rep: Mario DiCano		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	31-11-00	4-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	249.19 157.33		
	1	T2 Hip	6 /12	31-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	129.52 80.83		
	1	T3 Hip	6 /12	31-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	129.12 80.67		
	1	T4 Hip	6 /12	31-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	135.61 84.17		
	1	T5 Hip	6 /12	31-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	131.28 82.50		
	6	T6 Hip	6 /12	31-11-00	9-01-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	775.42 485.00		
	1 2-ply	T7 Hip Girder	9 /12	31-11-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	296.99 183.00		
	2	T8 Hip	9 /12	31-11-00	6-06-07	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	276.62 173.67		
	3	T9 Hip	9 /12	31-11-00	8-02-07	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	451.1 285.00		
	1	T21S Hip Girder	9 /12	19-07-00	4-08-07	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	89.33 59.83		
	1	T22 Hip	9 /12	19-07-00	6-04-07	2 x 4	1-03-08	1-06-04 3-00-04	91.27 57.83		
	1	T23 Hip Girder	9 /12	13-01-00	4-00-07	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	66.9 44.33		
	3	T24S Scissor	9 /12 6 /12	10-09-00	5-06-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	140.17 94.50		
	2	J1 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST				Job Track: 51225 PlanLog: 202400 Layout ID: 408147 Ref # Page: 2 of 2 Date: 04-14-2020 Designer: Andrew Conway Sales Rep: Mario DiCano	
	Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: RUSSELL GARDENS PH.3 Location: WATERDOWN Model: VALLEYCREEK 2 Lot #: 324 Elevation: 2					

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	J2 Jack-Open	6 / 12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J3 Jack-Open	6 / 12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J4 Jack-Open	6 / 12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	11	J5 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	184.74 117.33		
	6	J6 Jack-Open	10 / 12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	93.44 61.00		
	7	J7 Jack-Open	6 / 12	5-10-00	3-10-11	2 x 4	1-03-08	4-03 2-10-03	111.31 70.00		
	4	J8 Jack-Open	6 / 12	3-09-08	2-10-07	2 x 4	1-03-08	4-03 1-10-03	46.54 29.33		
	4	J21 Jack-Open	10 / 12	1-10-08	3-02-07	2 x 4	1-03-08	1-07-11 3-02-07	40.49 28.00		
	1	J22 Jack-Open	10 / 12	2-10-08	4-00-07	2 x 4	1-03-08	1-07-11 4-00-07	13.45 8.83		
	6	J23 Jack-Open	4 / 12	6-04-08	2-10-06	2 x 4	1-03-08	3-15 2-02-04	99.57 64.00		

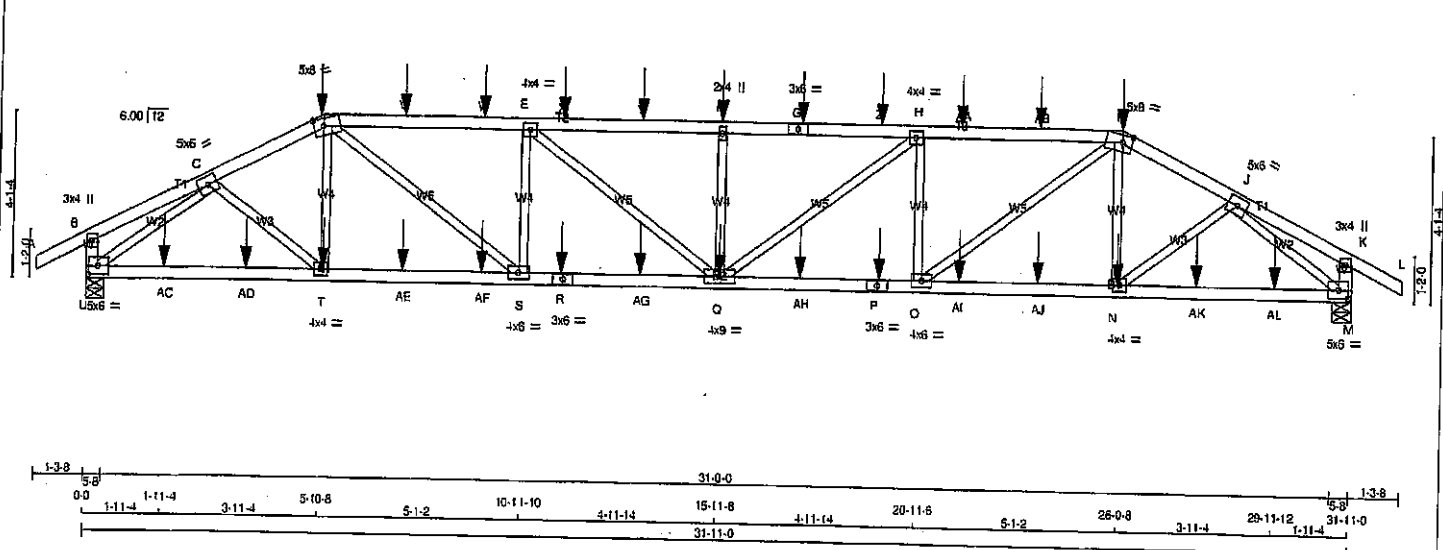
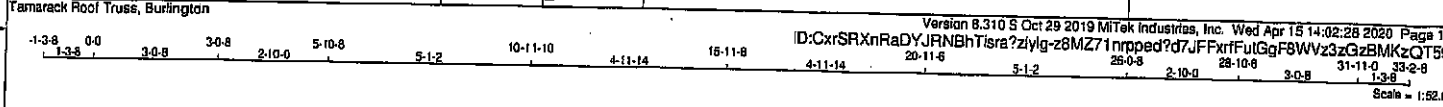
TOTAL # TRUSS= 72 TOTAL BFT OF ALL TRUSSES= 2300.48 BFT. TOTAL WEIGHT OF ALL TRSSES 3636.68 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	LJS28DS	
1	Hardware	LUS24	
2	Hardware	HGUS28-2	

TOTAL NUMBER OF
ITEMS= 4

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408146	T1	1	2	GREEN PARK HOMES		



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - G	2x4	DRY	No.2	SPF		
G - I	2x4	DRY	No.2	SPF		
I - L	2x4	DRY	No.2	SPF		
U - B	2x4	DRY	No.2	SPF		
M - K	2x4	DRY	No.2	SPF		
U - R	2x4	DRY	No.2	SPF		
R - P	2x4	DRY	No.2	SPF		
P - M	2x4	DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D	12	SIDE(61.0)
D - G	12	SIDE(61.0)
G - I	12	SIDE(61.0)
I - L	12	SIDE(61.0)
U - B	12	TOP
M - K	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
U - R	12	SIDE(61.0)
R - P	12	SIDE(61.0)
P - M	12	SIDE(61.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMW+p	MT20	3.0	4.0			
C TMW+W	MT20	5.0	6.0			
D TTWW-m	MT20	5.0	8.0	2.25	3.00	
E TMW+W	MT20	4.0	4.0			
F TMW+W	MT20	2.0	4.0			
G TS-I	MT20	3.0	6.0			
H TMW+W	MT20	4.0	4.0			
I TTWW-m	MT20	5.0	8.0	2.25	3.00	
J TMW+W	MT20	5.0	6.0			
K TMW+p	MT20	3.0	4.0			
M BMW+W	MT20	5.0	8.0	2.50	2.75	
N BMW+W	MT20	4.0	4.0			
O BMW+W	MT20	4.0	6.0			
P BS-I	MT20	3.0	6.0			
Q BMW+W	MT20	4.0	9.0			
R BS-I	MT20	3.0	6.0			
S BMW+W	MT20	4.0	6.0			
T BMW+W	MT20	4.0	4.0			
U BMW+W	MT20	5.0	8.0	2.50	2.75	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT	IN-SX
U	3035	0	0	5-8
M	3034	0	0	5-8

UNFACTORED REACTIONS	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERMLIVE WIND DEAD SOIL
U	2146	1410 0 0 0 0 0 736 0 0 0
M	2145	1409 0 0 0 0 0 736 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

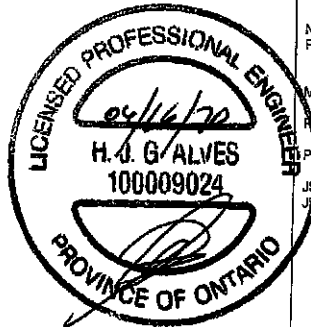
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. UNBRACED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LENGTH (FT)	MEMB.	FORCE (LBS)
FR-TO				FR-TO	
A-B	0.28	-91.8 -91.8 0.07 (1)	10.00	C-T	0.893 0.09 (1)
B-C	0.5	-91.8 -91.8 0.06 (1)	10.00	T-D	-223.55 0.03 (1)
C-D	-4467.0	-91.8 -91.8 0.13 (1)	4.44	D-S	0.2320 0.29 (1)
D-V	-5839.0	-91.8 -91.8 0.48 (1)	3.63	S-E	-1244.0 0.17 (1)
V-W	-5839.0	-91.8 -91.8 0.48 (1)	3.63	E-Q	0.893 0.09 (1)
W-E	-5839.0	-91.8 -91.8 0.48 (1)	3.63	Q-F	-892.0 0.09 (1)
E-X	-6390.0	-91.8 -91.8 0.51 (1)	3.45	C-H	0.897 0.09 (1)
X-Y	-6390.0	-91.8 -91.8 0.51 (1)	3.45	H-I	-1243.0 0.17 (1)
Y-F	-6390.0	-91.8 -91.8 0.51 (1)	3.45	O-I	0.2318 0.29 (1)
F-G	-6390.0	-91.8 -91.8 0.51 (1)	3.45	N-I	-223.55 0.03 (1)
G-Z	-6390.0	-91.8 -91.8 0.51 (1)	3.45	N-J	0.892 0.09 (1)
Z-H	-6390.0	-91.8 -91.8 0.51 (1)	3.45	U-C	-4383.0 0.56 (1)
H-AA	-5839.0	-91.8 -91.8 0.48 (1)	3.83	J-M	-4382.0 0.56 (1)
AA-AB	-5839.0	-91.8 -91.8 0.48 (1)	3.83		
AB-I	-5839.0	-91.8 -91.8 0.48 (1)	3.83		
I-J	-4466.0	-91.8 -91.8 0.13 (1)	4.44		
J-K	0.5	-91.8 -91.8 0.06 (1)	10.00		
K-L	0.28	-91.8 -91.8 0.07 (1)	10.00		
U-B	-249.0	0.0 0.0 0.01 (1)	7.81		
M-K	-249.0	0.0 0.0 0.01 (1)	7.81		

U-AC	0.3439	-18.5 -18.5 0.38 (1)	10.00
AC-AD	0.3438	-18.5 -18.5 0.38 (1)	10.00
AD-T	0.3438	-18.5 -18.5 0.38 (1)	10.00
T-AE	0.3881	-18.5 -18.5 0.42 (1)	10.00
AE-AF	0.3881	-18.5 -18.5 0.42 (1)	10.00
AF-S	0.3881	-18.5 -18.5 0.42 (1)	10.00
S-R	0.5840	-18.5 -18.5 0.57 (1)	10.00
R-AG	0.5840	-18.5 -18.5 0.57 (1)	10.00
AG-Q	0.5840	-18.5 -18.5 0.57 (1)	10.00
Q-AH	0.5837	-18.5 -18.5 0.57 (1)	10.00
AH-P	0.5837	-18.5 -18.5 0.57 (1)	10.00
P-O	0.5837	-18.5 -18.5 0.57 (1)	10.00
O-AI	0.3879	-18.5 -18.5 0.42 (1)	10.00
AI-AJ	0.3879	-18.5 -18.5 0.42 (1)	10.00
AJ-N	0.3878	-18.5 -18.5 0.42 (1)	10.00
N-AK	0.3437	-18.5 -18.5 0.38 (1)	10.00
AK-AL	0.3437	-18.5 -18.5 0.38 (1)	10.00
AL-M	0.3437	-18.5 -18.5 0.38 (1)	10.00



TOTAL WEIGHT = 2 X 125 = 249 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = L/999 (0.34")

CSI: TC=0.51/1.00 (E-F:1), BC=0.57/1.00 (Q-S:1), WB=0.56/1.00 (U-T:1), SS=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1967 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

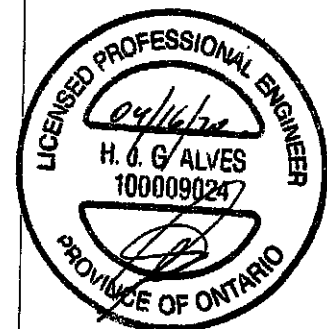
JS GRIP=0.85 (R) (INPUT = 0.80)
JT METAL=0.87 (R) (INPUT = 1.00)

JOB NAME 408146	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitak Industries, Inc. Wed Apr 15 14:02:28 2020 Page 2 ID:CxrSRXnRaDYJRNbHtira?zlvig-zBMZ71nnped?d7JFFxrfutGqF8WVz3zGzBMkzQT59	

FACTORED CONCENTRATED LOADS (LBS)									
	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	5-10-8	-457	-457	---	FRONT	VERT	TOTAL	---	C1
D	15-11-8	-110	-110	---	FRONT	VERT	TOTAL	---	C1
F	17-11-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
G	28-0-8	-457	-457	---	FRONT	VERT	TOTAL	---	C1
I	25-11-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
N	19-11-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	15-11-8	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Q	11-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
R	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
T	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	11-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	19-11-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	21-11-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	23-11-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	1-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AD	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AE	7-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AF	9-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AG	13-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AH	17-11-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AI	21-11-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AJ	23-11-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AK	27-11-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AL	29-11-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1

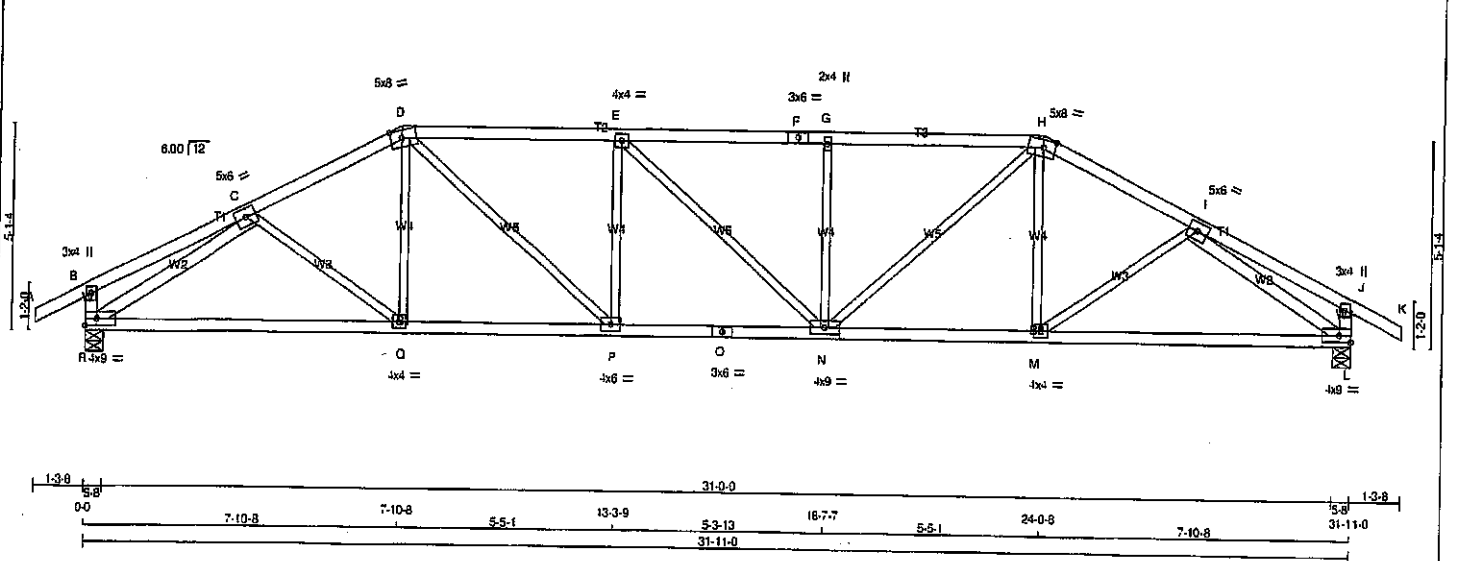
CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408146	T2	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Wed Apr 15 14:02:29 2020 Page 1
 ID: CxrSRXnRaDYJRNbHtIsra?zlylg-RKwxKnnTa6mUcniVpyS4CTR1X4b_FvRCBwkvzmzQT58
 1-3-8 0-0 4-0-8 4-0-8 3-10-0 7-10-8 5-5-1 13-3-9 5-3-13 18-7-7 5-5-1 24-0-8 3-10-0 27-10-8 4-0-8 31-11-0 33-2-8
 Scale = 1:52.8



LUMBER	CHORDS	SIZE	DRY	NO.2
A - D	2x4	DRY	No.2	
D - F	2x4	DRY	No.2	
F - H	2x4	DRY	No.2	
H - K	2x4	DRY	No.2	
K - L	2x4	DRY	No.2	
L - J	2x4	DRY	No.2	
J - O	2x4	DRY	No.2	
O - L	2x4	DRY	No.2	
ALL WEBS	2x3	DRY	No.2	
EXCEPT				
R - C	2x4	DRY	No.2	
I - L	2x4	DRY	No.2	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	8.0		
D	TTWW-m	MT20	5.0	8.0	2.25	3.50
E	TMV+p	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	6.0		
G	TTWW-m	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.25	3.50
I	TMV+p	MT20	5.0	6.0		
J	TMV+p	MT20	3.0	4.0		
L	BMVW-1	MT20	4.0	9.0		Edge
M	BMVW-1	MT20	4.0	4.0		
N	BMVW-1	MT20	4.0	9.0		
O	BS-1	MT20	3.0	6.0		
P	BMVW-1	MT20	4.0	6.0		
Q	BMVW-1	MT20	4.0	4.0		
R	BMVW-1	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
R	1894	0	1894	0
L	1894	0	1894	0

UNFACTORED REACTIONS	1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
R	1330	886 0
L	1330	886 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	UNBRAC LENGTH	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 28	-91.8 -91.8 0.12 (1)	10.00		C-D	0 74	0.03 (4)
B-C	0 17	-91.8 -91.8 0.20 (1)	10.00		D-E	0 133	0.04 (4)
C-D	-2469 0	-91.8 -91.8 0.30 (1)	4.12		E-F	0 993	0.21 (1)
D-E	-2693 0	-91.8 -91.8 0.54 (1)	3.61		F-G	-534 0	0.21 (1)
E-F	-2891 0	-91.8 -91.8 0.93 (1)	3.61		G-H	-2 0	0.00 (1)
F-G	-2891 0	-91.8 -91.8 0.53 (1)	3.61		H-I	-533 0	0.21 (1)
G-H	-2891 0	-91.8 -91.8 0.30 (1)	4.12		I-J	0 134	0.04 (4)
H-I	-2469 0	-91.8 -91.8 0.12 (1)	10.00		J-K	0 74	0.03 (4)
I-J	0 17	-91.8 -91.8 0.20 (1)	10.00		K-L	-2895 0	0.74 (1)
J-K	0 28	-91.8 -91.8 0.12 (1)	10.00		L-M	-2667 0	0.74 (1)
K-L	-269 0	0 0 0.03 (1)	7.81				
L-M	-269 0	0 0 0.03 (1)	7.81				
R-Q	0 2167	-18.5 -18.5 0.49 (1)	10.00				
Q-P	0 2194	-18.5 -18.5 0.48 (1)	10.00				
P-O	0 2893	-18.5 -18.5 0.53 (1)	10.00				
O-N	0 2893	-18.5 -18.5 0.53 (1)	10.00				
N-M	0 2194	-18.5 -18.5 0.49 (1)	10.00				
M-L	0 2168	-18.5 -18.5 0.49 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 8.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF
 SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, CBC 2012, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08")
 CALCULATED VERT. DEFL.(LL) = L/909 (0.15")
 ALLOWABLE DEFL.(TL) = L/360 (1.08")
 CALCULATED VERT. DEFL.(TL) = L/909 (0.29")

CSI: TC=0.54/1.00 (D-E:1), BC=0.53/1.00 (N-P:1), WB=0.74/1.00 (I-L:1), SS=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PS) (PL) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 818 354 1667 788 1987 1656

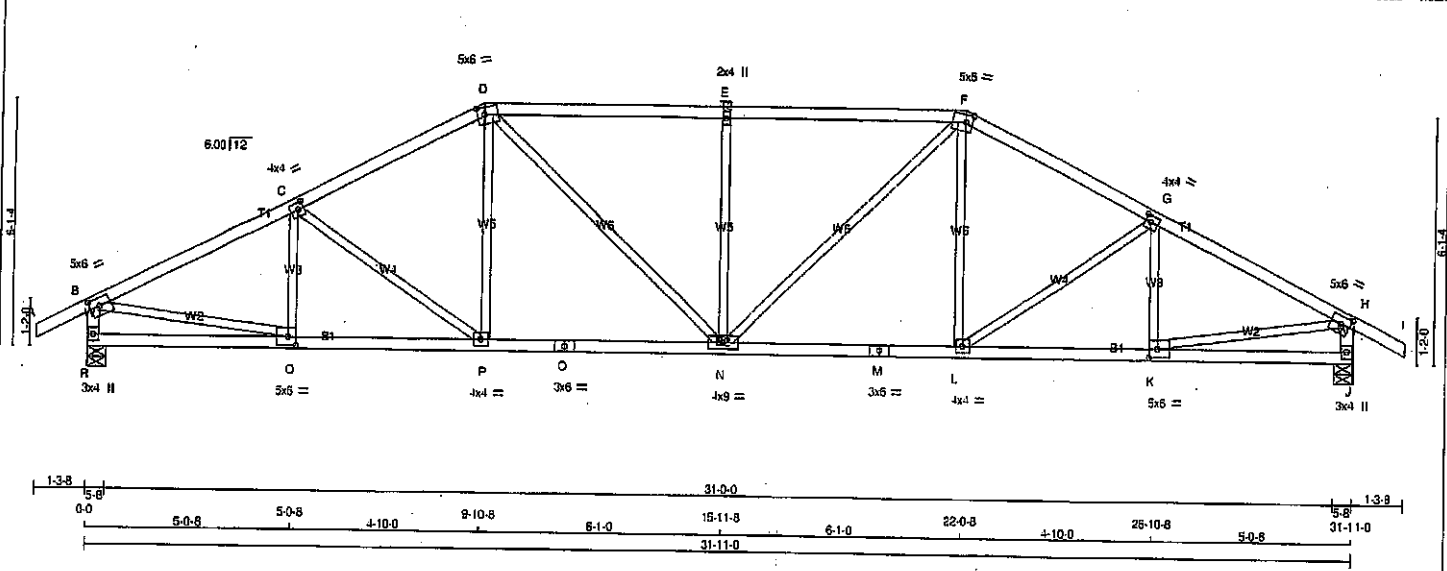
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (O) (INPUT = 0.90)
 JSI METAL = 0.94 (O) (INPUT = 1.00)



Structural component only
 DWG# T-2006232



LUMBER N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.
A - D 2x4 DRY No.2
D - F 2x4 DRY No.2
F - I 2x4 DRY No.2
R - B 2x4 DRY No.2
J - H 2x4 DRY No.2
R - O 2x4 DRY No.2
O - M 2x4 DRY No.2
M - J 2x4 DRY No.2
ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER		BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG	IN-SX	IN-SX	IN-SX
R	1884	0	1884	0	0	5-8	5-8				
J	1884	0	1884	0	0	5-8	5-8				

UNFACTORED REACTIONS		1ST LCASE		MAX. MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM	LIVE	WIND	WIND	DEAD	DEAD	SOIL	SOIL
R	1330	886 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	444 / 0	444 / 0	0 / 0	0 / 0
J	1330	886 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	444 / 0	444 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.0 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	MAX. FACTORED	MAX. FACTORED
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	Q-C	-301 / 0	0.07 (1)
B-C	-2538 / 0	-91.8	-91.8 0.37 (1)	4.02	C-P	-264 / 0	0.16 (1)
C-D	-2342 / 0	-91.8	-91.8 0.35 (1)	4.17	P-D	0 / 257	0.06 (1)
D-E	-2493 / 0	-91.8	-91.8 0.54 (1)	3.82	D-N	0 / 575	0.13 (1)
E-F	-2493 / 0	-91.8	-91.8 0.54 (1)	3.82	N-E	-685 / 0	0.40 (1)
F-G	-2342 / 0	-91.8	-91.8 0.35 (1)	4.17	N-F	0 / 575	0.13 (1)
G-H	-2538 / 0	-91.8	-91.8 0.37 (1)	4.02	L-F	0 / 257	0.06 (1)
H-I	0 / 28	-91.8	-91.8 0.12 (1)	10.00	L-G	-264 / 0	0.16 (1)
I-J	-1841 / 0	0.0	0.0 0.19 (1)	6.17	K-G	-301 / 0	0.07 (1)
J-K	-1841 / 0	0.0	0.0 0.19 (1)	6.17	B-O	0 / 2323	0.52 (1)
K-H					K-H	0 / 2323	0.52 (1)
R-Q	0 / 0	-18.5	-18.5 0.10 (4)	10.00			
Q-P	0 / 2290	-18.5	-18.5 0.43 (1)	10.00			
P-O	0 / 2077	-18.5	-18.5 0.40 (1)	10.00			
O-N	0 / 2077	-18.5	-18.5 0.40 (1)	10.00			
N-M	0 / 2077	-18.5	-18.5 0.40 (1)	10.00			
M-L	0 / 2077	-18.5	-18.5 0.40 (1)	10.00			
L-K	0 / 2290	-18.5	-18.5 0.43 (1)	10.00			
K-J	0 / 0	-18.5	-18.5 0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.06")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.13")
ALLOWABLE DEFL. (TL) = L/360 (1.06")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.24")

CSI: TC=0.54/1.00 (D-E:1), BC=0.43/1.00 (K-L:1), WB=0.52/1.00 (H-K:1), SS=0.27/1.00 (E-F:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1857 788 1887 1856

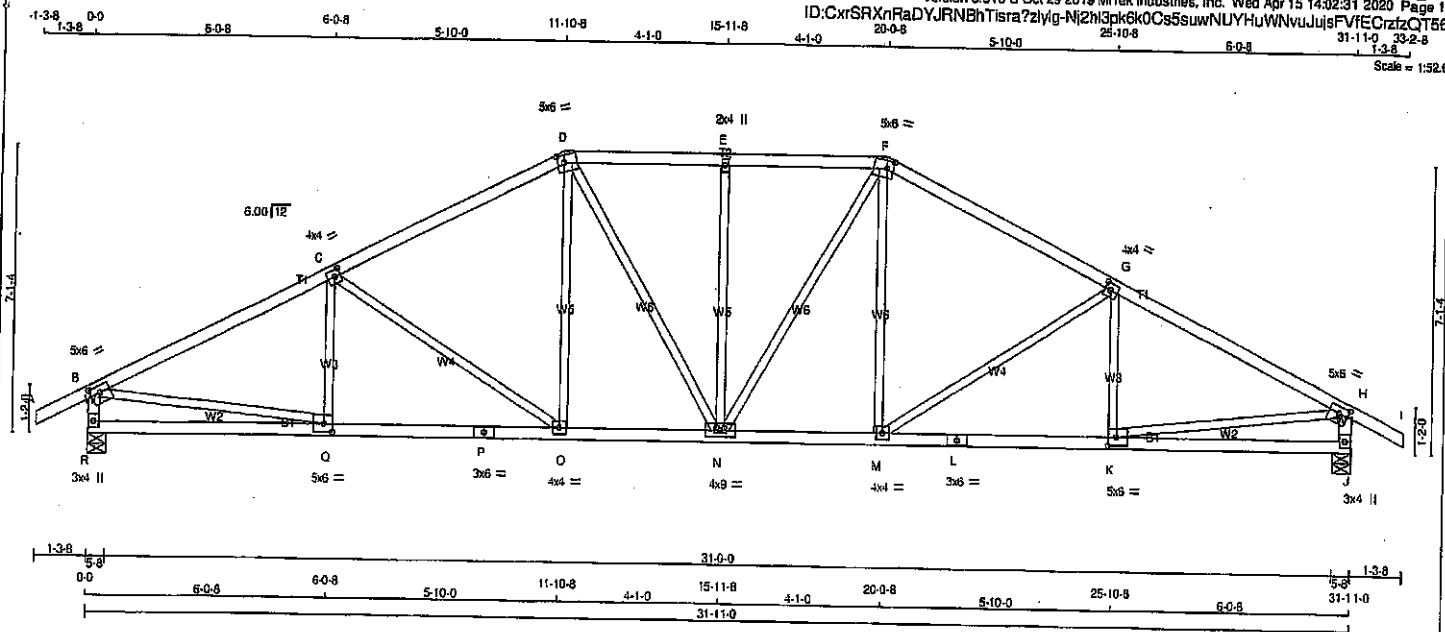
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.67 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100099024
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408146	T4	1	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Wed Apr 15 14:02:31 2020 Page 1
ID: CxrSRXnRaDYJRNbTisra?zlylg-NI2h3pk6K0Cs5suwNUYHuWVnuJulsFVfECrztzQT56



TOTAL WEIGHT = 138 lb (M/F)

LUMBER	SIZE	DRY	NO.2	DESCR.
N. L. G. A. RULES				
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-1	MT20	5.0	6.0	2.00	2.75
C TMVW-1	MT20	4.0	4.0	2.00	1.75
D TMVW-m	MT20	5.0	6.0	2.25	2.00
E TMVW-w	MT20	2.0	4.0		
F TMVW-m	MT20	5.0	6.0	2.25	2.00
G TMVW-1	MT20	4.0	4.0	2.00	1.75
H TMVW-1	MT20	5.0	6.0	2.00	2.75
J BMV1-p	MT20	3.0	4.0		
K BMVW-1	MT20	5.0	6.0	2.50	2.50
L BS-1	MT20	3.0	6.0		
M BMVW-1	MT20	4.0	4.0		
N BMVW-1	MT20	4.0	4.0		
O BMVW-1	MT20	4.0	4.0		
P BS-1	MT20	3.0	6.0		
Q BMVW-1	MT20	5.0	6.0	2.50	2.50
R BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UP	IN-SX
R	1884	0	1884	0
J	1884	0	1884	0

UNFACTORED REACTIONS	1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
R	1330	885.0 0.0 0.0 0.0 444.0 0.0
J	1330	885.0 0.0 0.0 0.0 444.0 0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MAX. FACTORED		FACTORED		MAX. UNBRAC	MEMB.		MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LO. CSI (LC)		MEMB.	FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	0.28	-91.8	-91.8	0.12 (1)	10.00	Q-C	-212 / 29	0.06 (1)	
B-C	-2579.0	-91.8	-91.8	0.55 (1)	3.79	C-O	-480.0	0.46 (1)	
C-D	-2188.0	-91.8	-91.8	0.49 (1)	4.11	O-D	0.362	0.08 (1)	
D-E	-2088.0	-91.8	-91.8	0.24 (1)	4.49	D-N	0.296	0.07 (1)	
E-F	-2088.0	-91.8	-91.8	0.24 (1)	4.49	N-E	-451.0	0.39 (1)	
F-G	-2188.0	-91.8	-91.8	0.49 (1)	4.11	E-F	0.296	0.07 (1)	
G-H	-2579.0	-91.8	-91.8	0.55 (1)	3.79	M-F	0.362	0.08 (1)	
H-I	0.28	-91.8	-91.8	0.12 (1)	10.00	M-G	-480.0	0.46 (1)	
I-J	-1836.0	0.0	0.0	0.19 (1)	8.18	K-Q	-212 / 29	0.06 (1)	
J-H	-1836.0	0.0	0.0	0.19 (1)	8.18	B-Q	0.2355	0.53 (1)	
						K-H	0.2355	0.53 (1)	
R-Q	0.0	-18.5	-18.5	0.15 (4)	10.00				
Q-P	0.2332	-18.5	-18.5	0.44 (1)	10.00				
P-O	0.2332	-18.5	-18.5	0.44 (1)	10.00				
O-N	0.1935	-18.5	-18.5	0.37 (1)	10.00				
N-M	0.1935	-18.5	-18.5	0.37 (1)	10.00				
M-L	0.2332	-18.5	-18.5	0.44 (1)	10.00				
L-K	0.2332	-18.5	-18.5	0.44 (1)	10.00				
K-J	0.0	-18.5	-18.5	0.15 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2018, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPC 2011, TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/360 (1.06")
CALCULATED VERT. DEF. (LL) = L/999 (0.12")
ALLOWABLE DEF. (TL) = L/360 (1.06")
CALCULATED VERT. DEF. (TL) = L/999 (0.21")

CSI: TC=0.55/1.00 (B-C:1), BC=0.44/1.00 (C-O:1), WB=0.53/1.00 (H-K:1), SS=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

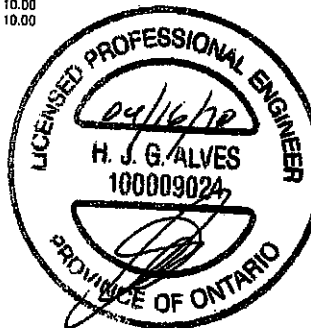
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (B) (INPUT = 0.90)
JSI METAL = 0.74 (P) (INPUT = 1.00)



Structural component only
DWG# T-2006234

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408146	T5	1	1	TRUSS DESC.		

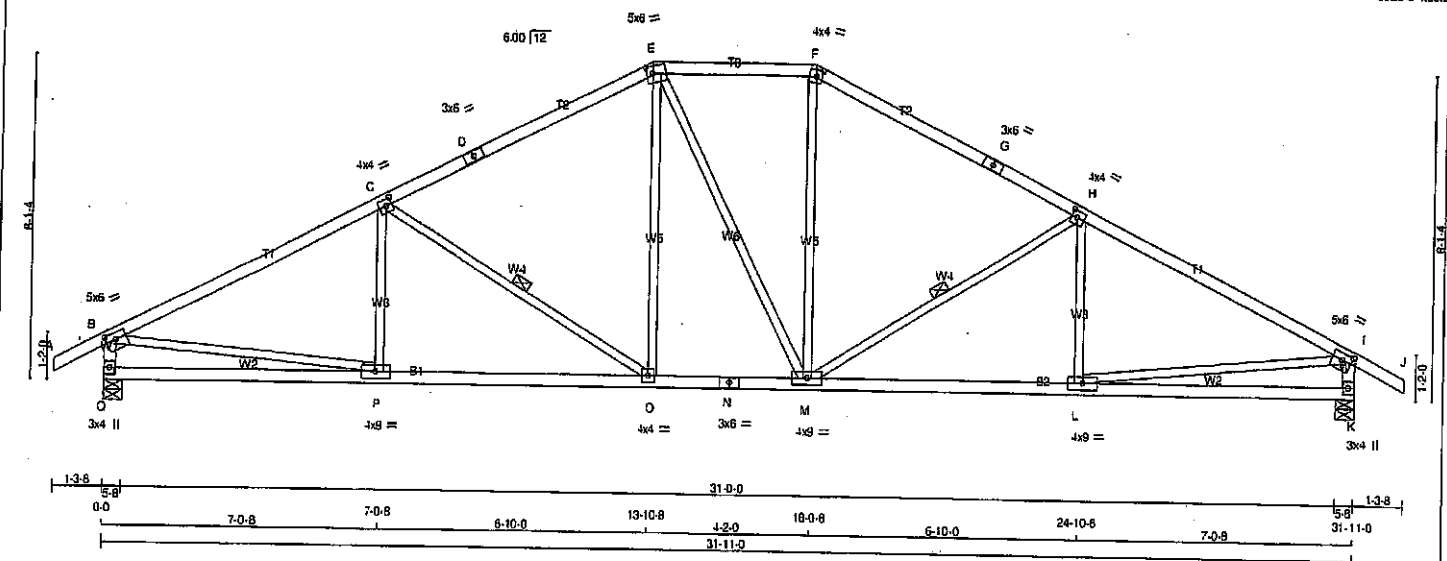
Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Wed Apr 15 14:02:32 2020 Page 1

ID: CxrSRXnRaDYJRNbHTisra?zlylg-rvc3zPgM183TER4U5?nq5U2HeQJSJTauuxOV5zQT55

31-11-0 33-2-8 1-3-3

Scale = 1:53.3



LUMBER				DESCH.			
N. L. G. A. RULES	SIZE	LUMBER		SPF			
CHORDS				SPF			
A - D	2x4	DRY	No.2	SPF			
D - E	2x4	DRY	No.2	SPF			
E - F	2x4	DRY	No.2	SPF			
F - G	2x4	DRY	No.2	SPF			
G - J	2x4	DRY	No.2	SPF			
Q - B	2x4	DRY	No.2	SPF			
K - I	2x4	DRY	No.2	SPF			
Q - N	2x4	DRY	No.2	SPF			
N - K	2x4	DRY	No.2	SPF			
ALL WEBS	2x3	DRY	No.2	SPF			
EXCEPT							

DRY: SEASONED LUMBER.

TOTAL WEIGHT = 131 lb

PLATES (table is in inches)				BEARING			
JT TYPE	PLATES	W	LEN Y X	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
B	TMVW-1	MT20	5.0 6.0 2.00 2.75	GROSS REACTION	GROSS REACTION	BRG	BRG
C	TMVW-1	MT20	4.0 4.0 2.00 1.75	VERT	DOWN	IN-SX	IN-SX
D	TS-1	MT20	3.0 6.0	Q	1884 0	1884 0	5-8
E	TTWW-m	MT20	5.0 6.0 2.25 2.00	K	1884 0	1884 0	5-8
F	TTW-m	MT20	4.0 4.0 2.00 1.75				
G	TS-1	MT20	3.0 6.0				
H	TMVW-1	MT20	4.0 4.0 2.00 1.75				
I	TMVW-1	MT20	5.0 6.0 2.00 2.75				
K	BMV1-p	MT20	3.0 4.0				
L	BMVW-1	MT20	4.0 9.0				
M	BMVW-1	MT20	4.0 9.0				
N	BS-1	MT20	3.0 6.0				
O	BMVW-1	MT20	4.0 4.0				
P	BMVW-1	MT20	4.0 9.0				
Q	BMV1-p	MT20	3.0 4.0				

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING				CHORDS			
TOTAL LOAD CASES: (4)				MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD LC1 MAX
				FR-TO	FORCE (LBS)	VERT. LOAD LC1	MAX (PLF)
				A-B	0 28	-91.8	-91.8 0.12 (1)
				B-C	-2587 0	-91.8	-91.8 0.78 (1)
				C-D	-2013 0	-91.8	-91.8 0.88 (1)
				D-E	-2013 0	-91.8	-91.8 0.88 (1)
				E-F	-1775 0	-91.8	-91.8 0.24 (1)
				F-G	-2013 0	-91.8	-91.8 0.68 (1)
				G-H	-2013 0	-91.8	-91.8 0.88 (1)
				H-I	-2587 0	-91.8	-91.8 0.78 (1)
				I-J	0 28	-91.8	-91.8 0.12 (1)
				J-K	-1830 0	0.0	0.0 0.18 (1)
				K-L	-1830 0	0.0	0.0 0.18 (1)
				Q-P	0 0	-18.5	-18.5 0.22 (4)
				P-O	0 2344	-18.5	-18.5 0.48 (1)
				O-N	0 1774	-18.5	-18.5 0.35 (1)
				N-M	0 1774	-18.5	-18.5 0.35 (1)
				M-L	0 2344	-18.5	-18.5 0.48 (1)
				L-K	0 0	-18.5	-18.5 0.22 (4)

WEBS				DESIGN CRITERIA			
MEMB.	MAX. FACTORED	MAX. FACTORED	UNBRAC	SPECIFIED LOADS:			
FR-TO	FORCE (LBS)	FORCE (LBS)	LENGTH	TOP CH.	LL = 25.8	PSF	
A-B	-140 71	0.05 (1)		BOT CH.	LL = 0.0	PSF	
B-C	-881 0	0.31 (1)		DL	DL = 6.0	PSF	
C-D	0 458	0.10 (1)		TOTAL LOAD	DL = 7.4	PSF	
D-E	0 2	0.00 (1)		SPACING	SPACING = 24.0	IN. C/C	
E-F	0 460	0.10 (1)					
F-G	-680 0	0.31 (1)					
G-H	-142 70	0.05 (1)					
H-I	0 2362	0.53 (1)					
I-J	0 2361	0.53 (1)					

ALLOWABLE DEFL.(LL) = L/360 (1.05")

CALCULATED VERT. DEFL.(LL) = L/999 (0.12")

ALLOWABLE DEFL.(TL) = L/360 (1.05")

CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.78/1.00 (B-C-1), BC=0.48/1.00 (O-P-1), WB=0.53/1.00 (B-P-1), SS=0.28/1.00 (H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

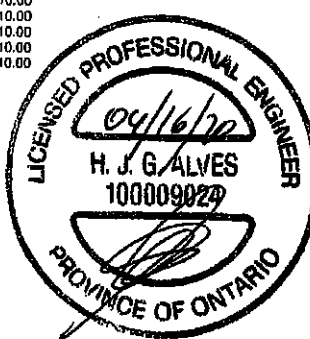
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (B) (INPUT = 0.90)

JSI METAL = 0.70 (B) (INPUT = 1.00)

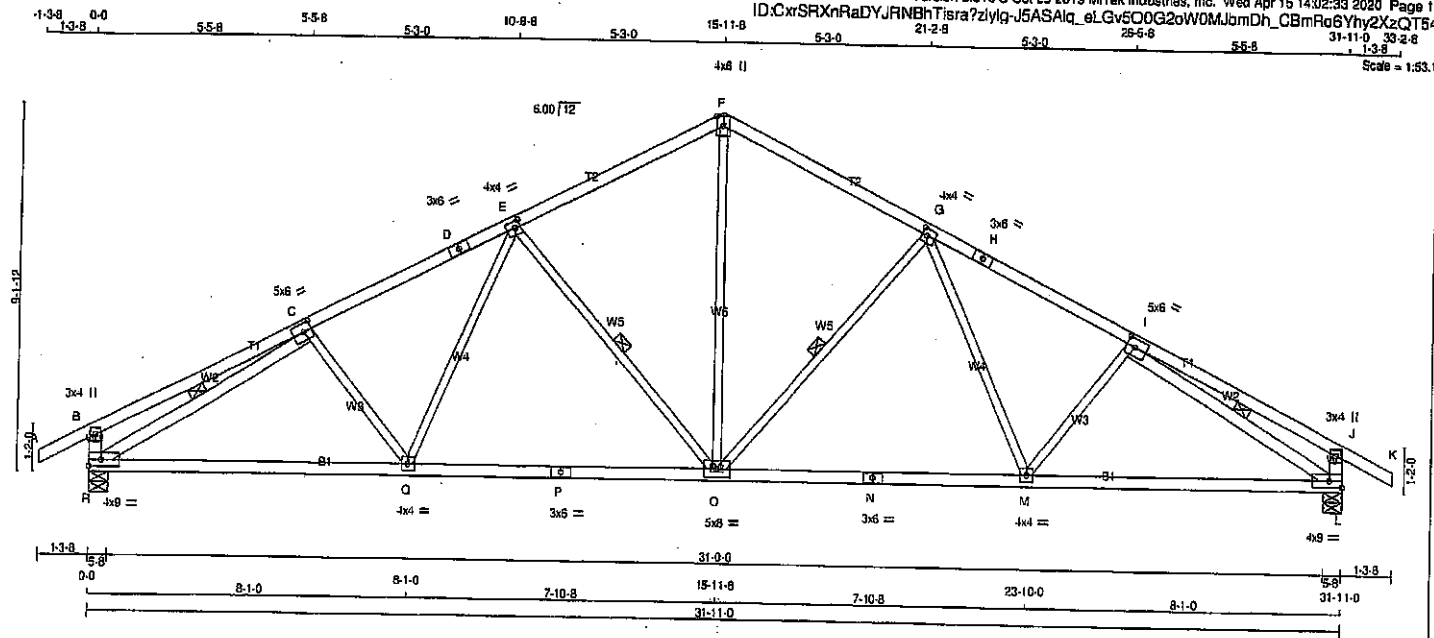
Structural component only

DWG# T-2006235



Tamarack Roof Truss, Burlington

Version B.310 S Oct 29 2019 MITek Industries, Inc. Wed Apr 15 14:02:33 2020 Page 1
ID: CxrSRXnRaDYJRNbH Tisra?zilylg-J5ASAlq_eLGv500G2oW0Mj0mDh_C8mRo6Yhy2XzQT54



LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
K - J	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - P	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
R - C	2x4	DRY	No.2
L - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0	
C	TMWV-1	MT20	5.0	8.0	2.50 2.50
D	TS-1	MT20	3.0	6.0	
E	TTWV-1	MT20	4.0	4.0	2.00 1.50
F	TTW-p	MT20	4.0	6.0	Edge
G	TMWV-1	MT20	4.0	4.0	2.00 1.50
H	TS-1	MT20	3.0	6.0	
I	TMWV-1	MT20	5.0	8.0	2.50 2.50
J	TMV-p	MT20	3.0	4.0	
L	BMWV-1	MT20	4.0	9.0	Edge
M	BMWV-1	MT20	4.0	4.0	
N	BS-1	MT20	3.0	6.0	
O	BMWV-1	MT20	5.0	8.0	
P	BS-1	MT20	3.0	6.0	
Q	BMWV-1	MT20	4.0	4.0	
R	BMWV-1	MT20	4.0	9.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		RECORD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
R	1884	0	1884	0	0	5-8	5-8	5-8	5-8
L	1884	0	1884	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1330	886	0	0	0	0	444	0
L	1330	886	0	0	0	0	444	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-O, E-O, C-R, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)
FR-TO					FR-TO		
A-B	0:28	-91.8	-91.8 0.12 (1)	10.00	O-F	0:1211	0.27 (1)
B-C	0:20	-91.8	-91.8 0.33 (1)	10.00	O-G	-701:0	0.32 (1)
C-D	-2428:0	-91.8	-91.8 0.37 (1)	4.11	G-M	0:312	0.07 (1)
D-E	-2428:0	-91.8	-91.8 0.37 (1)	4.11	M-I	-158:21	0.05 (1)
E-F	-1808:0	-91.8	-91.8 0.34 (1)	4.65	E-O	-701:0	0.32 (1)
F-G	-1808:0	-91.8	-91.8 0.34 (1)	4.65	Q-E	0:312	0.07 (1)
G-H	-2428:0	-91.8	-91.8 0.37 (1)	4.11	C-Q	-158:21	0.05 (1)
H-I	-2428:0	-91.8	-91.8 0.37 (1)	4.11	R-C	-2717:0	0.55 (1)
I-J	0:20	-91.8	-91.8 0.12 (1)	10.00	I-L	-2717:0	0.55 (1)
J-K	0:28	-91.8	-91.8 0.12 (1)	10.00			
R-B	-325:0	0.0	0.0 0.03 (1)	7.81			
L-J	-325:0	0.0	0.0 0.03 (1)	7.81			
R-Q	0:2268	-18.5	-18.5 0.51 (1)	10.00			
Q-P	0:2061	-18.5	-18.5 0.48 (1)	10.00			
P-O	0:2061	-18.5	-18.5 0.48 (1)	10.00			
O-N	0:2061	-18.5	-18.5 0.48 (1)	10.00			
N-M	0:2061	-18.5	-18.5 0.48 (1)	10.00			
M-L	0:2268	-18.5	-18.5 0.51 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/993 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/993 (0.24")

CSI: TC=0.37/1.00 (C-E:1), BC=0.51/1.00 (Q-R:1), WB=0.55/1.00 (C-R:1), SS=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

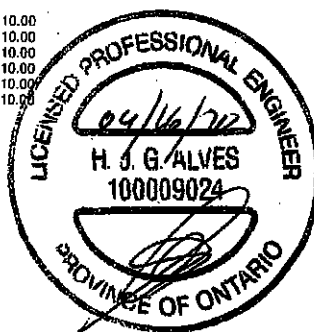
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (I) (INPUT = 0.90)
JSI METAL= 0.70 (P) (INPUT = 1.00)



JOB NAME 408146	TRUSS NAME T7	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Wed Apr 15 14:02:34 2020 Page 2
ID:QxrSRXnPaDYJRNbhTisra?zlylg-nlkgQ5icP!QmYbTcW2FvW8v25lgwG3xLCQVa_zQT53

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AA	7-0-12	-17	-17	---	BACK	VERT	TOTAL	---	C1
AB	9-0-12	-17	-17	---	BACK	VERT	TOTAL	---	C1
AC	11-0-12	-17	-17	---	BACK	VERT	TOTAL	---	C1

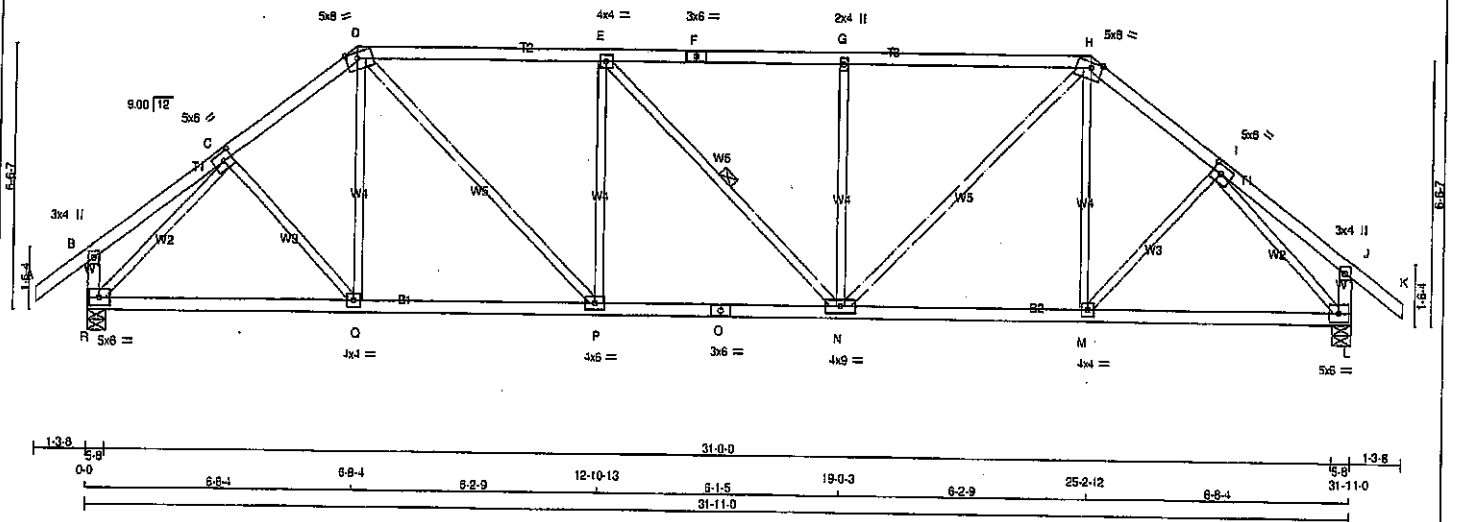
CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2006237

2/2



LUMBER

N. L. G. A. RULES	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
K - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0	
C	TMWW-t	MT20	5.0	6.0	2.50 2.75
D	TTWW-m	MT20	5.0	8.0	Edge 3.50
E	TMWW-t	MT20	4.0	4.0	
F	TS-t	MT20	3.0	6.0	
G	TMW-w	MT20	2.0	4.0	
H	TTWW-m	MT20	5.0	8.0	Edge 3.50
I	TMWW-t	MT20	5.0	6.0	2.50 2.75
J	TMV-p	MT20	3.0	4.0	
L	BMVW-t	MT20	5.0	6.0	
M	BMWW-t	MT20	4.0	4.0	
N	BMWW-t	MT20	4.0	9.0	
O	BS-t	MT20	3.0	6.0	
P	BMWW-t	MT20	4.0	6.0	
Q	BMWW-t	MT20	4.0	4.0	
R	BMVW-t	MT20	5.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
R	1885	0	1885	0	5-8	5-8
L	1886	0	1885	0	5-8	5-8

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1331	887.0	0.0	0.0	0.0	444.0	0.0
L	1331	887.0	0.0	0.0	0.0	444.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	FR-TO	CHORDS		WEBS	
		MAX. FACTORED FORCE (LBS)	VERT. LOAD (PL)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
A-B	0.138	-91.8	-91.8	10.00	0.110
B-C	0.19	-91.8	-91.8	10.00	0.95
C-D	-1907.0	-91.8	-91.8	4.69	0.1019
D-E	-2227.0	-91.8	-91.8	3.88	0.23 (1)
E-F	-2226.0	-91.8	-91.8	3.88	-0.13
F-G	-2226.0	-91.8	-91.8	3.88	-2.0
G-H	-2226.0	-91.8	-91.8	3.88	-0.12
H-I	-1907.0	-91.8	-91.8	4.69	0.1017
I-J	0.19	-91.8	-91.8	10.00	0.95
J-K	0.38	-91.8	-91.8	10.00	0.110
K-L	-248.0	0.0	0.0	7.81	-2137.0
L-J	-248.0	0.0	0.0	7.81	-2137.0
R-Q	0.1431	-18.5	-18.5	10.00	0.84 (1)
Q-P	0.1507	-18.5	-18.5	10.00	0.84 (1)
P-O	0.2227	-18.5	-18.5	10.00	0.84 (1)
O-N	0.2227	-18.5	-18.5	10.00	0.84 (1)
N-M	0.1508	-18.5	-18.5	10.00	0.84 (1)
M-L	0.1431	-18.5	-18.5	10.00	0.84 (1)

TOTAL WEIGHT = 2 X 138 = 277 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.8 PSF
 BOT CH. LL = 0.0 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBCC 2018, OBC 2012, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
 ALLOWABLE DEFL.(TL) = L/360 (1.05")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.65/1.00 (D-E:1), BC=0.42/1.00 (N-P:1), WB=0.84/1.00 (H-L:1), SS=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

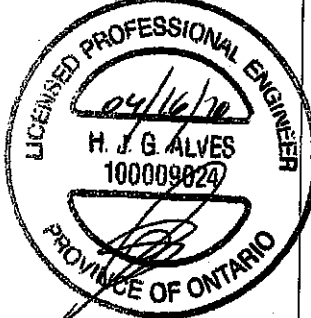
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1987 789 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

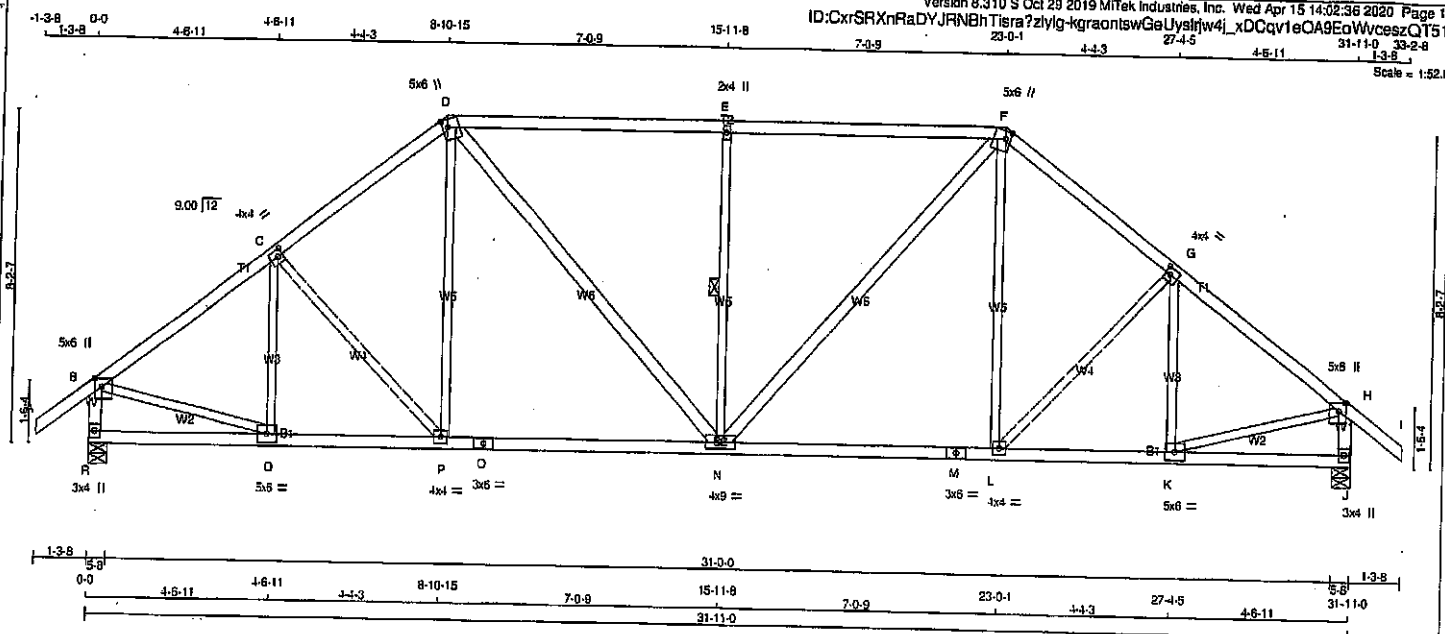
PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP = 0.87 (I) (INPUT = 0.90)
 SI METAL = 0.74 (O) (INPUT = 1.00)



JOB NAME 408146	TRUSS NAME T9	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
---------------------------	-------------------------	----------------------	-----------------	--------------------------------------	-------------	----------

Tamarack Roof Truss, Burlington



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.00
C	TMVW-i	MT20	4.0	4.0	2.00	1.58
D	TTVW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TTVW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW-i	MT20	4.0	4.0	2.00	1.80
H	TMVW+p	MT20	5.0	6.0	2.50	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMVW-i	MT20	5.0	6.0		
L	BMVW-i	MT20	4.0	4.0		
M	BS-i	MT20	3.0	6.0		
N	BMVW-i	MT20	4.0	9.0		
O	BS-i	MT20	3.0	6.0		
P	BMVW-i	MT20	4.0	4.0		
Q	BMVW-i	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	JT VERT	IN-SX	IN-SX
R 1886 0	1886 0	5-8	5-8
J 1886 0	1886 0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	JT COMBINED
R 1331 887.0 0.0	R 1331 887.0 0.0
J 1331 887.0 0.0	J 1331 887.0 0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. MAX. (LC)	
FR-TO				FR-TO			
A-B	0.38	-91.8	-91.8 0.12 (1)	Q-C	-324.0	0.12 (1)	
B-C	-1898.0	-91.8	-91.8 0.28 (1)	C-P	-150.0	0.11 (1)	
C-D	-1829.0	-91.8	-91.8 0.27 (1)	P-D	0.219	0.05 (4)	
D-E	-1854.0	-91.8	-91.8 0.66 (1)	D-N	0.623	0.10 (1)	
E-F	-1854.0	-91.8	-91.8 0.66 (1)	N-E	-795.0	0.34 (1)	
F-G	-1829.0	-91.8	-91.8 0.27 (1)	N-F	0.623	0.10 (1)	
G-H	-1898.0	-91.8	-91.8 0.26 (1)	F-G	0.219	0.05 (4)	
H-I	0.38	-91.8	-91.8 0.12 (1)	L-G	-150.0	0.11 (1)	
I-B	-1848.0	0.0	0.0 0.19 (1)	K-G	-324.0	0.12 (1)	
J-H	-1848.0	0.0	0.0 0.19 (1)	B-Q	0.1593	0.36 (1)	
R-Q	0.0	-18.5	-18.5 0.08 (4)	K-H	0.1593	0.36 (1)	
Q-P	0.1541	-18.5	-18.5 0.32 (1)				
P-O	0.1439	-18.5	-18.5 0.34 (1)				
O-N	0.1439	-18.5	-18.5 0.34 (1)				
N-M	0.1439	-18.5	-18.5 0.34 (1)				
M-L	0.1439	-18.5	-18.5 0.34 (1)				
L-K	0.1541	-18.5	-18.5 0.32 (1)				
K-J	0.0	-18.5	-18.5 0.08 (4)				

TOTAL WEIGHT = 3 X 150 = 451 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
OL = 6.0 PSF
BOT CH. LL = 0.0 PSF
OL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD
EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CS: TC=0.68/1.00 (D-E:1), BC=0.34/1.00 (N-P:1),
WB=0.36/1.00 (B-Q:1), SS=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

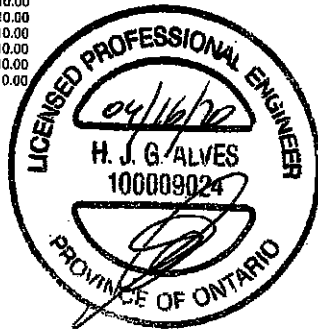
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1567 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.89 (B) (INPUT = 0.80)
JSI METAL=0.42 (C) (INPUT = 1.00)



Version 8.310 S Oct 29 2019 MITek Industries, Inc. Wed Apr 15 14:02:37 2020 Page 1

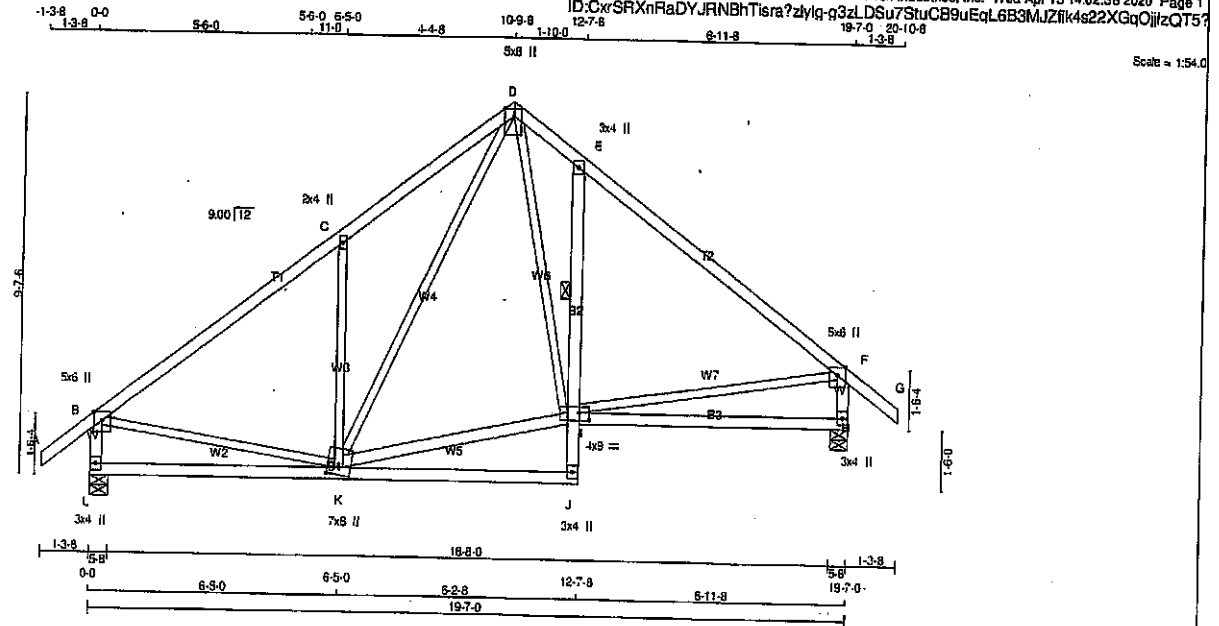


Structural component only
DWG# T-2006240

JOB NAME 408146	TRUSS NAME T10S	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
---------------------------	---------------------------	----------------------	-----------------	--------------------------------------	-------------	----------

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Wed Apr 15 14:02:38 2020 Page 1
ID: CxSRXnRadyJRNhTisra?zlylg-q3zLDSu7StuCB9uEqL6B3MJZtk4s22XGqOjizQT5?



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - E	2x4	DRY	No.2
I - H	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
EXCEPT			
K - I	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	2.0	4.0		
D	TTWV+p	MT20	5.0	8.0	Edge 2.75	
E	TMVW+p	MT20	3.0	4.0		
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BVMWWV-I	MT20	4.0	9.0	2.25	3.00
J	BMV+p	MT20	3.0	4.0		
K	BMVWWV+MT20	MT20	7.0	8.0	3.50	2.25
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
L	1206	0	1206	0
H	1206	0	1206	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE	PERM.LIVE
L	851	572	0	0
H	851	572	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO						FR-TO		
A-B	0 38	-91.8	-91.8	0.12 (1)	10.00	B-K	0 868	0.20 (1)
B-C	-1034 0	-91.8	-91.8	0.46 (1)	5.56	K-C	-617 0	0.40 (1)
C-D	-1094 0	-91.8	-91.8	0.38 (1)	5.55	K-I	0 540	0.10 (1)
D-E	-1158 0	-91.8	-91.8	0.47 (1)	5.28	D-I	0 844	0.19 (1)
E-F	-889 0	-91.8	-91.8	0.58 (1)	5.43	K-D	0 492	0.11 (1)
F-G	0 38	-91.8	-91.8	0.12 (1)	10.00	I-F	0 832	0.19 (1)
L-B	-1159 0	0.0	0.0	0.12 (1)	7.40			
H-F	-1140 0	0.0	0.0	0.12 (1)	7.45			
L-K	0 0	-18.5	-18.5	0.21 (4)	10.00			
K-J	0 10	-18.5	-18.5	0.21 (4)	10.00			
J-I	0 46	0.0	0.0	0.03 (1)	10.00			
I-E	-700 0	0.0	0.0	0.04 (1)	6.25			
I-H	0 0	-18.5	-18.5	0.27 (4)	10.00			

TOTAL WEIGHT = 2 X 102 = 204 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.65")
CALCULATED VERT. DEFL. (LL) = $L/998$ (0.03")
ALLOWABLE DEFL. (TL) = $L/360$ (0.65")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.14")

CSI: TC=0.58/1.00 (E-F:1), BC=0.27/1.00 (H-I:4), WB=0.40/1.00 (C-K:1), SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

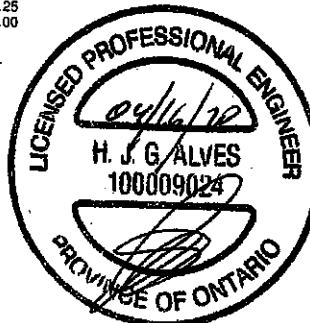
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
	1687	788
	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (I) (INPUT = 0.80)
JSI METAL= 0.45 (B) (INPUT = 1.00)



Structural component only
DWG# T-2006241

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Wed Apr 15 14:02:39 2020 Page 1
ID: CxrSRXnRaDYJRNbHtIsra?zlylg-8FXjRovIDB03pJTQ03dQcarr164SbX3qVU8GFBzQT5



DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S). H

BRACING

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.15/1.00 (E-F:1), BC=0.26/1.00 (I-J:4),
WB=0.28/1.00 (E-H:1), SSI=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

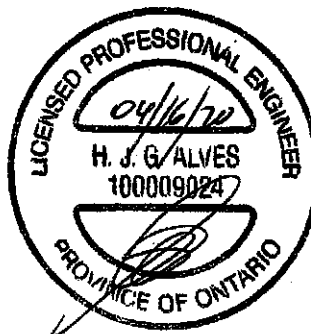
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP (DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX
MT20	618	354	1667	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (C) (INPUT = 0.90)
JSI METAL= 0.27 (C) (INPUT = 1.00)

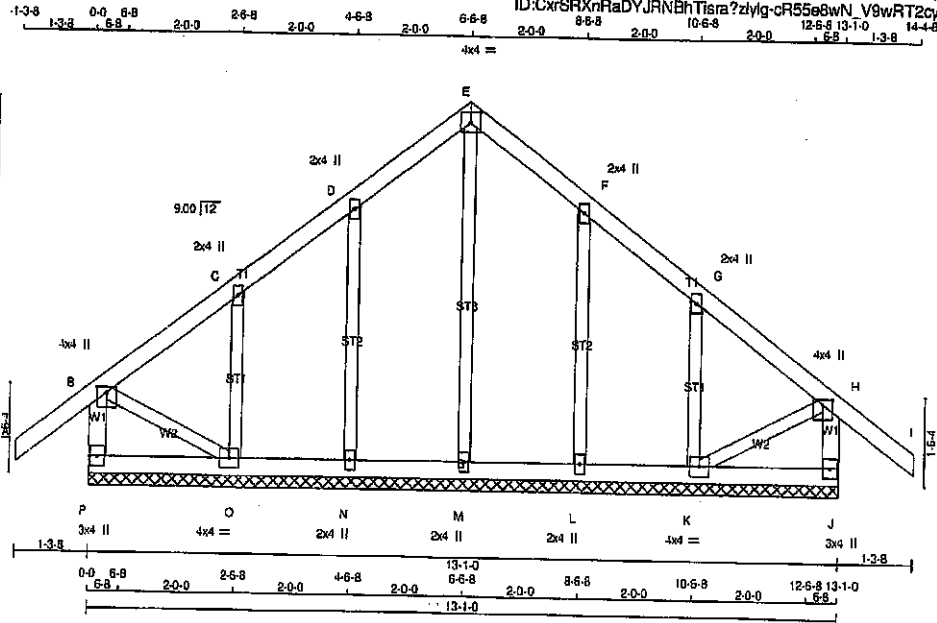


Structural component only
DWG# T-2006242

JOB NAME 408146	TRUSS NAME T11G	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
---------------------------	---------------------------	----------------------	-----------------	--------------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Wed Apr 15 14:02:40 2020 Page 1
ID: CxSRXnRaDYJRNhTisra?zylg-cR55e8wN V9wRT2cym8f9nO1EWTK1PqJ8tpmdzQT4z



Scale = 1:36.4

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
P - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 O.C.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G					
C TMVW+w	MT20	2.0	4.0		
E TTW+p	MT20	4.0	4.0	2.25	2.00
H TMVW+p	MT20	4.0	4.0	1.00	2.00
J BMV1+p	MT20	3.0	4.0		
K BMVW1-1	MT20	4.0	4.0		
L, M, N					
L BMV1+w	MT20	2.0	4.0		
O BMVW1-1	MT20	4.0	4.0		
P BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0:38	-91.8 -91.8 0.12 (1)	10.00	M-E	-133:0	0.09 (1)	
B-C	-22:0	-91.8 -91.8 0.07 (1)	8.25	N-D	-194:0	0.07 (1)	
C-D	-30:0	-91.8 -91.8 0.07 (1)	8.25	O-C	-228:0	0.05 (1)	
D-E	-31:0	-91.8 -91.8 0.05 (1)	6.25	L-F	-194:0	0.07 (1)	
E-F	-31:0	-91.8 -91.8 0.05 (1)	6.25	K-G	-228:0	0.05 (1)	
F-G	-30:0	-91.8 -91.8 0.07 (1)	8.25	B-O	0:30	0.01 (1)	
G-H	-22:0	-91.8 -91.8 0.07 (1)	8.25	K-H	0:30	0.01 (1)	
H-I	0:38	-91.8 -91.8 0.12 (1)	10.00				
P-B	-252:0	0.0 0.0 0.03 (1)	7.81				
J-H	-252:0	0.0 0.0 0.03 (1)	7.81				
P-O	0:0	-18.5 -18.5 0.03 (4)	10.00				
O-N	0:21	-18.5 -18.5 0.03 (4)	10.00				
N-M	0:17	-18.5 -18.5 0.02 (4)	10.00				
M-L	0:17	-18.5 -18.5 0.02 (4)	10.00				
L-K	0:21	-18.5 -18.5 0.03 (4)	10.00				
K-J	0:0	-18.5 -18.5 0.03 (4)	10.00				

TOTAL WEIGHT = 61 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 0.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TO=0.12/1.00 (H-H), BC=0.03/1.00 (N-C), WB=0.09/1.00 (E-M), SSI=0.08/1.00 (G-H)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1687
		788	1987
			1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.20 (H) (INPUT = 0.90)

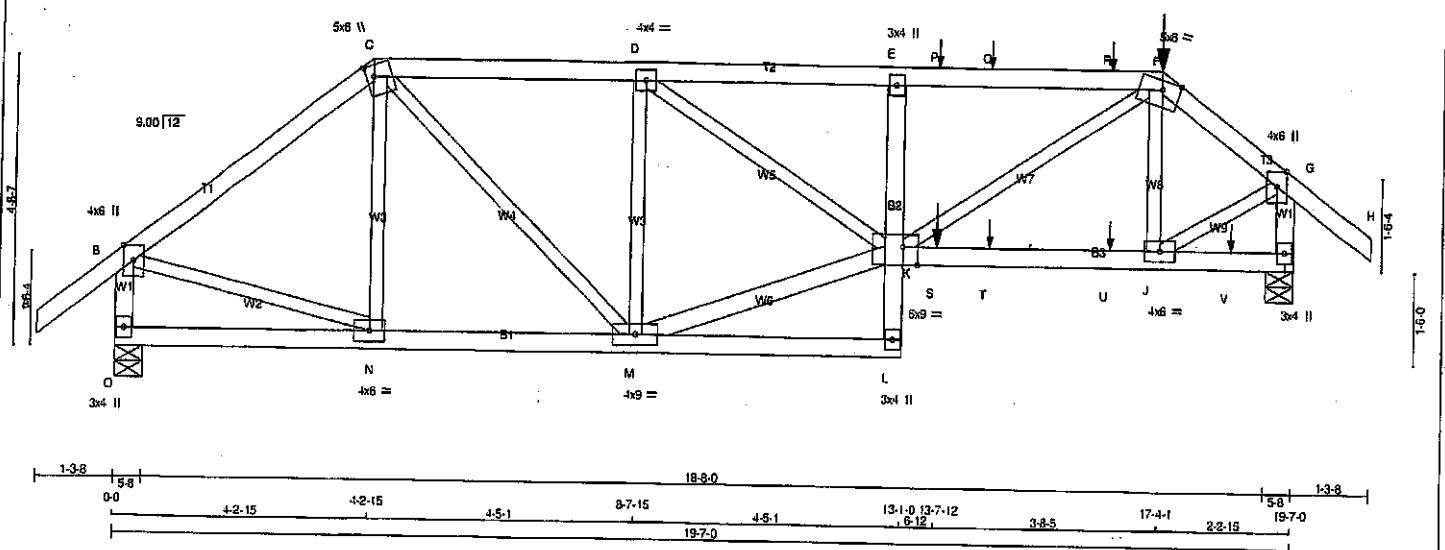
JSI METAL = 0.12 (C) (INPUT = 1.00)



Structural component only
DWG# T-2006243

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408147	T21S	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Wed Apr 15 15:52:28 2020 Page 1
 ID:jqP?amb3WFwXbCtUZtsE03zlxMq-jtAdJpfoVtA9ovhnuMLZ93FpCAv4vXQWd5IQ0nzQRU1
 13-1-0 13-7-12 14-6-1 16-6-4 17-4-1 19-7-0 20-10-8
 1-3-8 0-0 4-2-15 4-2-15 4-5-1 8-7-15 4-5-1 6-12 10-8 2-0-0 9-13 2-2-15 1-3-8 20-10-8
 Scale = 1:34.6



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - E	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
M - K	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	4.0	6.0 Edge
C	TTWW+m	MT20	5.0	6.0 2.25 1.50
D	TMWW-i	MT20	4.0	4.0
E	TMV+p	MT20	3.0	4.0
F	TTWW-m	MT20	5.0	8.0 Edge 3.50
G	TMVW+p	MT20	4.0	6.0 Edge
I	BMV1+p	MT20	3.0	4.0
J	BMVW-i	MT20	4.0	6.0
K	BVMWW-i	MT20	6.0	9.0 Edge 3.00
L	BMV+p	MT20	3.0	4.0
M	BMVW-i	MT20	4.0	9.0
N	BMVW-i	MT20	4.0	9.0
O	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORIZ	GROSS REACTION	HORIZ	BRG	IN-SX	BRG	IN-SX
O	1210	0	1210	0	0	5-8	5-8	5-8	5-8
I	1239	0	1239	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
O	854	574.0	0.0	0.0	0.0
I	873	589.0	0.0	0.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (7)

CHORDS		MEMB.		W E B S	
MAX. FACTORED	FORCE	MAX. FACTORED	FORCE	MAX. FACTORED	FORCE
FR-TO	(LBS)	FROM TO	(LBS)	FR-TO	(LBS)
A-B	0/38	-91.8	-91.8 0.14 (1)	N-C	-140/20
B-C	-1043/0	-91.8	-91.8 0.35 (1)	C-M	0/573
C-D	-1236/0	-91.8	-91.8 0.32 (5)	M-D	-742/0
D-E	-1683/0	-91.8	-91.8 0.33 (5)	K-F	0/1100
E-P	-1698/0	-91.8	-91.8 0.29 (1)	J-F	-319/0
P-Q	-1698/0	-91.8	-91.8 0.29 (1)	B-N	0/863
Q-R	-1698/0	-91.8	-91.8 0.29 (1)	J-G	0/887
R-F	-1698/0	-91.8	-91.8 0.29 (1)	M-K	0/1278
F-G	-992/0	-91.8	-91.8 0.10 (1)	D-K	0/541
G-H	0/38	-91.8	-91.8 0.14 (1)		
O-B	-1178/0	0.0	0.0 0.13 (1)		
I-G	-1231/0	0.0	0.0 0.14 (1)		

CHORDS		MEMB.		W E B S	
MAX. FACTORED	FORCE	MAX. FACTORED	FORCE	MAX. FACTORED	FORCE
FR-TO	(LBS)	FROM TO	(LBS)	FR-TO	(LBS)
A-B	0/38	-91.8	-91.8 0.14 (1)	N-C	-140/20
B-C	-1043/0	-91.8	-91.8 0.35 (1)	C-M	0/573
C-D	-1236/0	-91.8	-91.8 0.32 (5)	M-D	-742/0
D-E	-1683/0	-91.8	-91.8 0.33 (5)	K-F	0/1100
E-P	-1698/0	-91.8	-91.8 0.29 (1)	J-F	-319/0
P-Q	-1698/0	-91.8	-91.8 0.29 (1)	B-N	0/863
Q-R	-1698/0	-91.8	-91.8 0.29 (1)	J-G	0/887
R-F	-1698/0	-91.8	-91.8 0.29 (1)	M-K	0/1278
F-G	-992/0	-91.8	-91.8 0.10 (1)	D-K	0/541
G-H	0/38	-91.8	-91.8 0.14 (1)		
O-B	-1178/0	0.0	0.0 0.13 (1)		
I-G	-1231/0	0.0	0.0 0.14 (1)		

FACTORED CONCENTRATED LOADS (LBS)		FACE		DIR	
JT	LOC.	MAX.	MAX+	FRONT	VERT
F	17-4-1	-30	-30	FRONT	VERT
P	13-7-12	1	1	FRONT	VERT
Q	14-6-4	1	1	FRONT	VERT
R	16-6-4	1	1	FRONT	VERT
S	13-7-12	-1	-1	FRONT	VERT
T	14-6-4	1	1	FRONT	VERT
U	16-6-4	1	1	FRONT	VERT
V	18-6-4	1	1	FRONT	VERT

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
 TOP CH. LL = 25.5 PSF
 DL = 8.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
 ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, OBC 2012, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 088-09, CSA 088-14
 - TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
 ALLOWABLE DEFL.(TL) = L/360 (0.65")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.35/1.00 (B-C:1), BC=0.19/1.00 (M-N:1),
 WB=0.27/1.00 (F-K:1), SS=0.21/1.00 (C-D:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

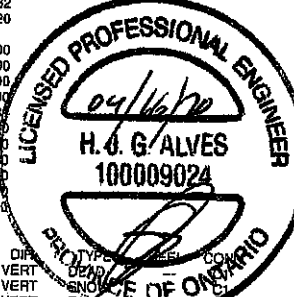
NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1659

PLATE PLACEMENT TOL. = 0.250 inches

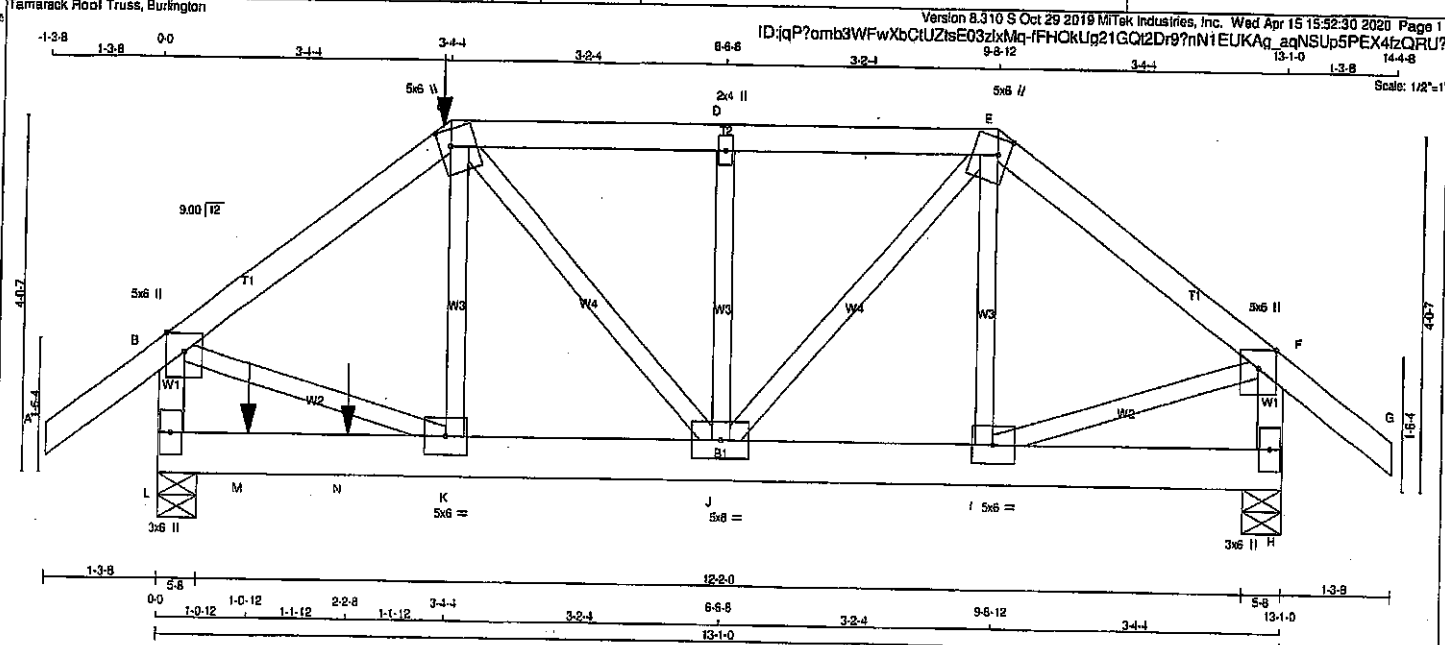
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (B) (INPUT = 0.90)
 JSI METAL = 0.55 (B) (INPUT = 1.00)

Structural component only
 DWG# T-2006247



JOB NAME 408147	TRUSS NAME T23	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER	SIZE	LUMBER	DESCR.
N. L. G. A. RULES			
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - H	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (tablets in inches)	W	LEN	Y	X
JT TYPE				
B TMVW+p	MT20	5.0	6.0	Edge
C TTWW+m	MT20	5.0	6.0	2.25 1.50
D TMVW+w	MT20	2.0	4.0	
E TTWW+m	MT20	5.0	6.0	2.25 1.50
F TMVW+p	MT20	5.0	6.0	Edge
H BMV1+p	MT20	3.0	6.0	
I BMVW+t	MT20	5.0	6.0	
J BMVW+t	MT20	5.0	6.0	
K BMVW+t	MT20	5.0	6.0	
L BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UPLIFT	IN-SX
L	1470	0	1470	5-8
H	985	0	985	5-8

UNFACTORED REACTIONS	1ST LCASE	MAX MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
L	1033	717 0 0 0 0 0 316 0 0 0
H	693	475 0 0 0 0 0 218 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO						FR-TO			
A-B	0.738	-91.8	-91.8	0.14 (1)	10.00	K-C	0.328	0.08 (1)	
B-C	-1140.0	-91.8	-91.8	0.22 (1)	5.65	C-J	-29.0	0.01 (1)	
C-D	-900.0	-91.8	-91.8	0.17 (1)	6.25	J-D	-347.0	0.08 (1)	
D-E	-900.0	-91.8	-91.8	0.17 (1)	6.25	E-E	0.412	0.10 (1)	
E-F	-791.0	-91.8	-91.8	0.21 (1)	10.00	B-K	0.955	0.24 (1)	
F-G	0.738	-91.8	-91.8	0.14 (1)	7.13	I-F	0.693	0.16 (1)	
L-H	-1259.0	0.0	0.0	0.14 (1)	7.13				
H-F	-960.0	0.0	0.0	0.11 (1)	7.81				
L-M	0.0	-18.5	-18.5	0.30 (1)	10.00				
M-N	0.0	-18.5	-18.5	0.30 (1)	10.00				
N-K	0.0	-18.5	-18.5	0.30 (1)	10.00				
K-J	0.919	-18.5	-18.5	0.27 (1)	10.00				
J-I	0.628	-18.5	-18.5	0.13 (1)	10.00				
I-H	0.0	-18.5	-18.5	0.03 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	3-4-4	-19	-21	---	FRONT	VERT	DEAD	C1
C	3-4-4	-60	-90	---	FRONT	VERT	SNOW	C1
M	1-0-12	-7	-7	---	FRONT	VERT	TOTAL	C1
N	2-2-8	-844	-644	---	FRONT	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 67 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF
SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.22/1.00 (B-C:1), BC=0.30/1.00 (K-L:1), WB=0.24/1.00 (B-K:1), SSI=0.33/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

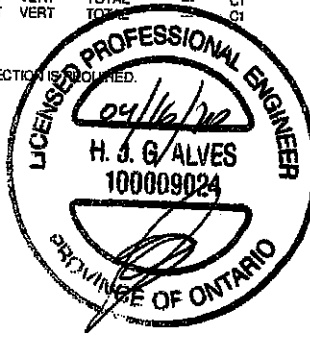
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1667 786 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

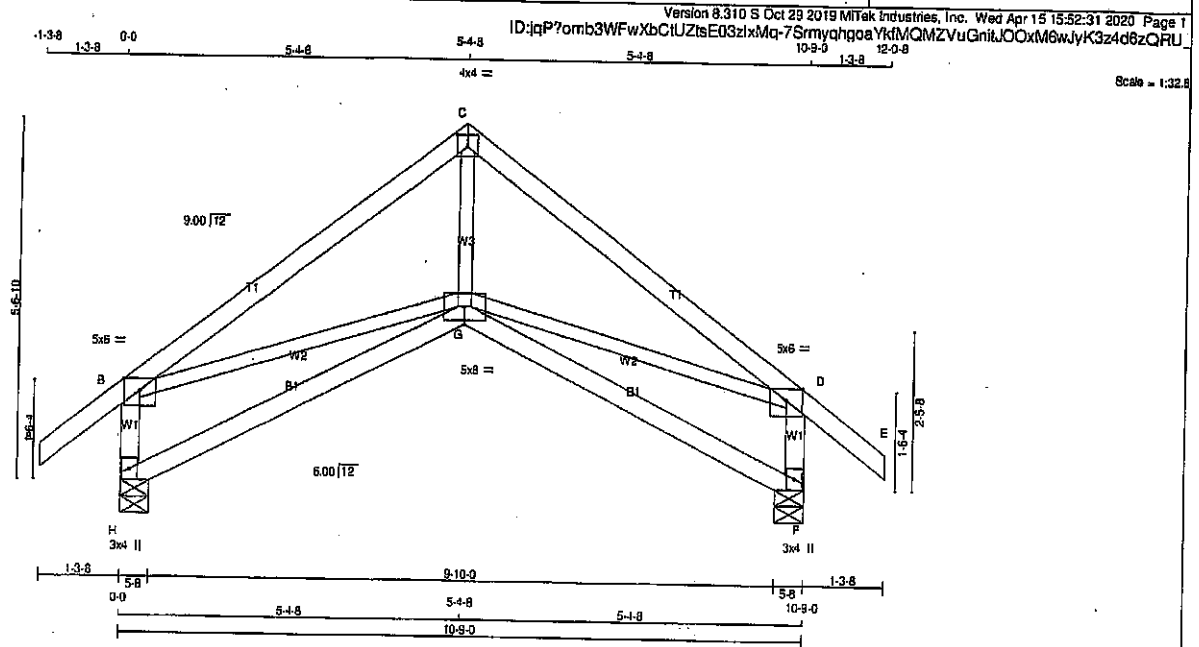
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

Structural component only
DWG# T-2006249



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408147	T24S	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4	DRY No.2
C - E	2x4	DRY No.2
H - B	2x4	DRY No.2
F - D	2x4	DRY No.2
H - G	2x4	DRY No.2
G - F	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW-p	MT20	5.0	8.0	Edge	
F	BMV1-p	MT20	3.0	4.0		
G	BMV1-p	MT20	5.0	8.0		
H	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
H	719	0	719	0
F	719	0	719	0

UNFACTORED REACTIONS

JT	1ST LOOSE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
H	506	346 / 0	0 / 0	0 / 0	161 / 0	0 / 0
F	506	346 / 0	0 / 0	0 / 0	161 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
A-B	0 / 38	-91.8	-91.8	0.12 (1)	10.00							
B-C	-724 / 0	-91.8	-91.8	0.35 (1)	6.25							
C-D	-724 / 0	-91.8	-91.8	0.35 (1)	6.25							
D-E	0 / 38	-91.8	-91.8	0.12 (1)	10.00							
H-B	-669 / 0	0.0	0.0	0.07 (1)	7.81							
F-D	-669 / 0	0.0	0.0	0.07 (1)	7.81							
H-G	0 / 0	-18.5	-18.5	0.16 (4)	10.00							
G-F	0 / 0	-18.5	-18.5	0.16 (4)	10.00							

TOTAL WEIGHT = 3 X 47 = 140 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.36")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.36")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.08")

CSI: $TC=0.35/1.00$ (B-C:1), $BC=0.18/1.00$ (G-H:4), $WB=0.13/1.00$ (B-G:1), $SSI=0.15/1.00$ (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

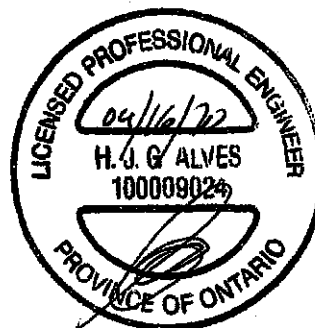
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PU)	
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.67 (C) (INPUT = 0.90)
JSI METAL = 0.19 (F) (INPUT = 1.00)

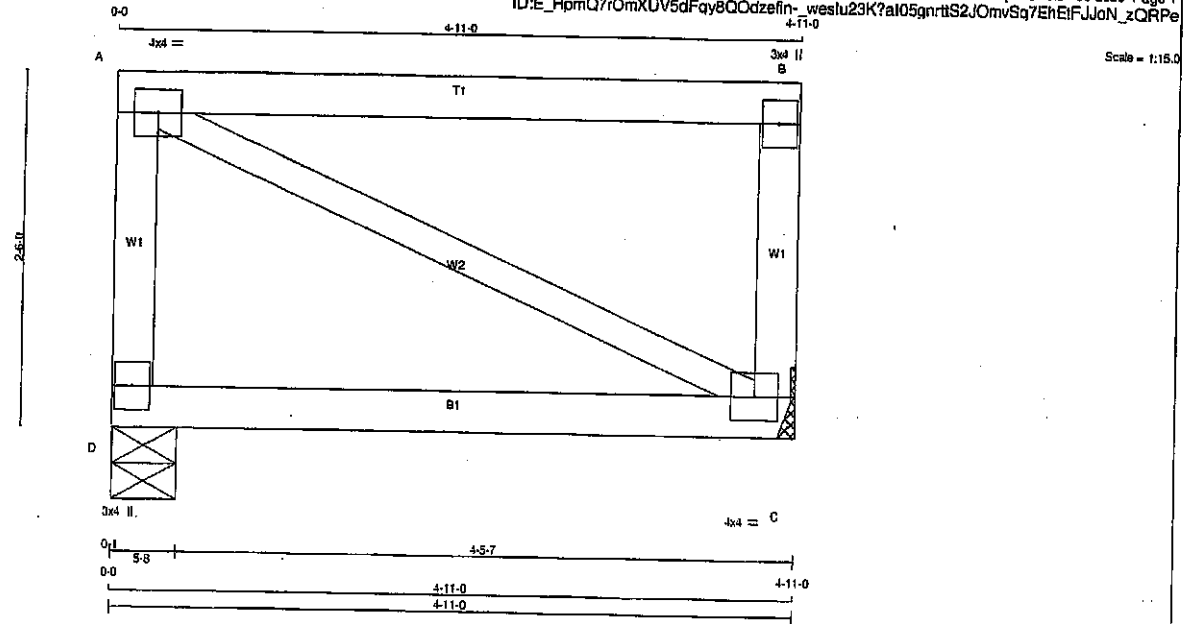


Structural component only
DWG# T-2006250

JOB NAME 408148	TRUSS NAME T32	QUANTITY 6	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
---------------------------	--------------------------	----------------------	-----------------	--------------------------------------	-------------	----------

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Wed Apr 15 15:57:09 2020 Page 1
ID: E_HpmQ7rOmXUV5dFqy8QOdzeFin- weslu29K?ai05gnrttS2JOmvSq7EhEIFJJoN_zQRPe



LUMBER

CHORDS	SIZE	LUMBER
D - A	2x4 DRY	No.2
A - B	2x4 DRY	No.2
C - B	2x4 DRY	No.2
D - C	2x4 DRY	No.2
ALL WEBS	2x3 DRY	No.2
DRY: SEASONED LUMBER.		

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0		
B	TMVW-p	MT20	3.0	4.0		
C	BMVW1-t	MT20	4.0	4.0		
D	BMVW1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	328	0	328	0	0	5-8	5-8
C	328	0	328	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

1ST LOASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	235	126 / 0	0 / 0	0 / 0	0 / 0	110 / 0	0 / 0
C	236	126 / 0	0 / 0	0 / 0	0 / 0	110 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO 2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM	TO			FR-TO		
D-A	-281 / 0	0.0	0.0	0.05 (1)	7.81	A-C	0 / 0	0.00 (1)
A-B	0 / 0	-114.3	-114.3	0.83 (1)	10.00			
C-B	-281 / 0	0.0	0.0	0.05 (1)	7.81			
D-C	0 / 0	-18.5	-18.5	0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 15.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 48.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.18")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.83/1.00 (A-B:1), BC=0.17/1.00 (C-D:4),
WB=0.00/1.00 (A-C:1), SSI=0.29/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

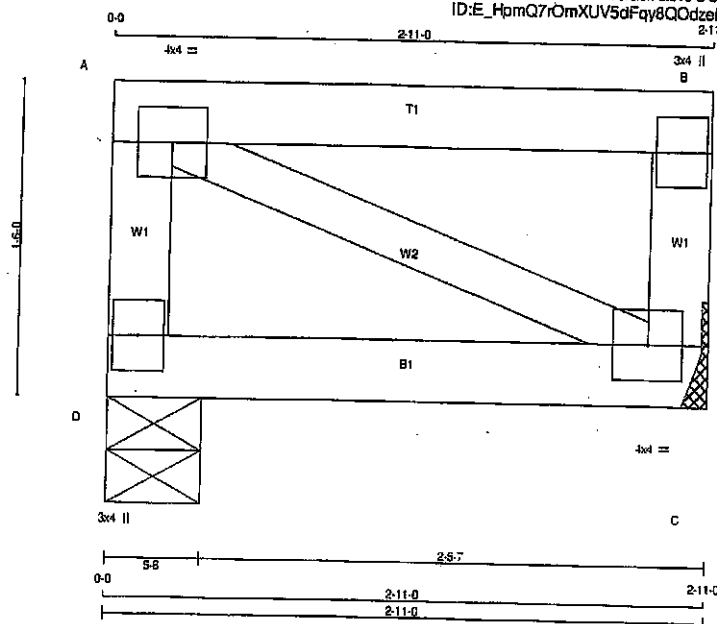
JSI GRIP= 0.22 (A) (INPUT = 0.30)
JSI METAL= 0.06 (A) (INPUT = 1.00)



Structural component only
DWG# T-2006251

JOB NAME 408148	TRUSS NAME T33	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.310 5 Oct 29 2019 MITek Industries, Inc. Wed Apr 15 15:57:10 2020 Page 1
ID:E_HomQ7rOmXUV5dFqy8QOdzefin-S7CEVE3h5i8eFFzPbPhbXw0isB1z8U1Uz3LvRzQRPd



Scale = 1:10.1

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
D - A	2x4 DRY	No.2
A - B	2x4 DRY	No.2
C - B	2x4 DRY	No.2
D - C	2x4 DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	4.0	4.0		
B	TMV+p	MT20	3.0	4.0		
C	BMVW1-1	MT20	4.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
D	226 0	226 0	5-8	5-8
C	226 0	226 0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	166	75/0	0/0	0/0	0/0	92/0	0/0
C	166	75/0	0/0	0/0	0/0	92/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (LC)	MAX. UNBRAC. LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (LC)		
FR-TO		FROM	TO		FR-TO				
D-A	-199.0	0.0	0.0	0.03 (1)	A-C	0.0	0.00 (1)		
A-B	0.0	-136.8	-136.8	0.30 (1)					
C-B	-199.0	0.0	0.0	0.03 (1)					
D-C	0.0	-18.5	-18.5	0.07 (4)					

TOTAL WEIGHT = 11 lb
(M/R)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 15.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 48.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.30/1.00 (A-B:1), BC=0.07/1.00 (C-D:4),
WB=0.00/1.00 (A-C:1), SS=0.24/1.00 (A-B:1)

DOL LUMBER=0.96 NAIL=0.96 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

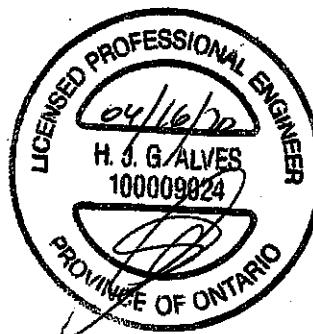
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 798 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

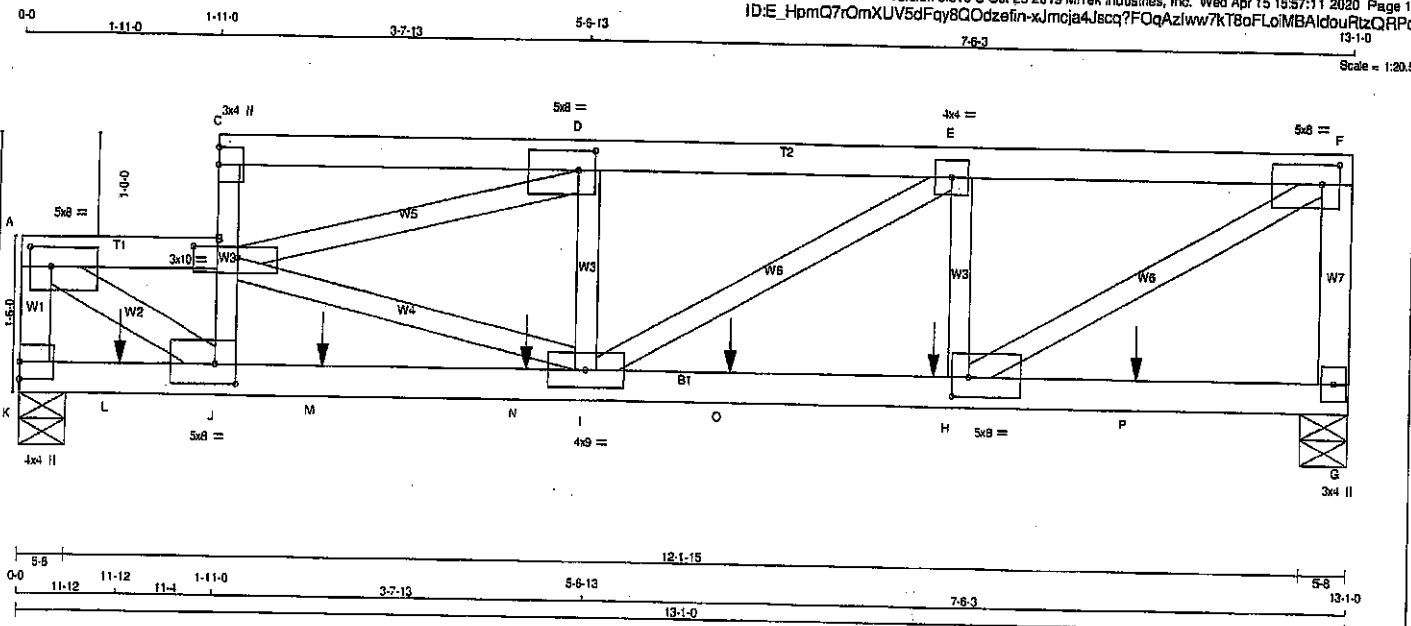
JSI GRIP= 0.17 (A) (INPUT = 0.90)
JSI METAL= 0.04 (A) (INPUT = 1.00)

Structural component only
DWG# T-2006252



JOB NAME 408148	TRUSS NAME T34	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Wed Apr 15 15:57:11 2020 Page 1
ID:E_HpmQ7rOmXUV5dFay8QOdzeIn-xJmcja4Jscq?FQqAzlww7kT8oFLoMBAldouRtzQRPC



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
K - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
J - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - G	2x4	DRY	1650F 1.5E
ALL WEBS	2x3	DRY	No.2
EXCEPT	A - J	2x4	DRY
DRY: SEASONED LUMBER.			

PLATES (tables in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-I	MT20	5.0	8.0	2.25	2.50
B	TMVWV-I	MT20	3.0	10.0	1.25	5.25
C	TMV-p	MT20	3.0	4.0		
D	TMVW-I	MT20	5.0	8.0	2.25	2.00
E	TMVWV-I	MT20	4.0	4.0		
F	TMVW-I	MT20	5.0	8.0	2.25	2.00
G	BMV1-p	MT20	3.0	4.0		
H	BMVWV-I	MT20	5.0	8.0	2.25	2.00
I	BMVWV-I	MT20	4.0	9.0		
J	BMVW-I	MT20	5.0	8.0	2.25	2.50
K	BMV1-p	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
K	1773	0	1773	0	0	5-8	5-8	5-8	5-8
G	1711	0	1711	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1280	895 / 0	0 / 0	0 / 0	0 / 0	585	0
G	1233	878 / 0	0 / 0	0 / 0	0 / 0	555	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (LBS)	MEMB.	FORCE (LBS)	MAX. (LBS)	MAX. (LBS)
FR-TO		FROM TO		FR-TO			
K-A	-1689.0	0.0	0.0 0.25 (1)	A-J	0	3405	0.80 (1)
A-B	-2929.0	-114.3	-114.3 0.31 (1)	B-D	-2838.0		1.00 (1)
J-B	-1293.0	0.0	0.0 0.39 (1)	B-I	-74.0		0.03 (1)
B-C	-170.0	0.0	0.0 0.32 (1)	I-D	0	412	0.14 (1)
C-D	-72.0	-114.3	-114.3 0.25 (1)	D-E	0	451	0.15 (1)
D-E	-2799.0	-114.3	-114.3 0.48 (1)	E-F	-702.0		0.16 (1)
E-F	-2416.0	-114.3	-114.3 0.47 (1)	F-H	0	2785	0.92 (1)
G-F	-1572.0	0.0	0.0 0.28 (1)				
K-L	0.0	-18.5	-18.5 0.14 (1)				
L-J	0.0	-18.5	-18.5 0.14 (1)				
J-M	0 2849	-18.5	-18.5 0.80 (1)				
M-N	0 2849	-18.5	-18.5 0.80 (1)				
N-I	0 2849	-18.5	-18.5 0.80 (1)				
I-O	0 2416	-18.5	-18.5 0.71 (1)				
O-H	0 2416	-18.5	-18.5 0.71 (1)				
H-P	0.0	-18.5	-18.5 0.42 (1)				
P-G	0.0	-18.5	-18.5 0.42 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
H	8-11-12	-308	-308	---	FRONT	VERT	TOTAL	---	C1
L	11-12	-208	-208	---	FRONT	VERT	TOTAL	---	C1
M	2-11-12	-308	-308	---	FRONT	VERT	TOTAL	---	C1
N	4-11-12	-308	-308	---	FRONT	VERT	TOTAL	---	C1
O	6-11-12	-308	-308	---	FRONT	VERT	TOTAL	---	C1
P	10-11-12	-308	-308	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 52 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
	DL = 15.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 48.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.44")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.07")
ALLOWABLE DEFL. (TL) = $L/360$ (0.44")
CALCULATED VERT. DEFL. (TL) = $L/994$ (0.16")

CSI: TG=0.47/1.00 (E-F:1), BC=0.80/1.00 (I-J:1), WB=1.00/1.00 (B-D:1), SSI=0.45/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

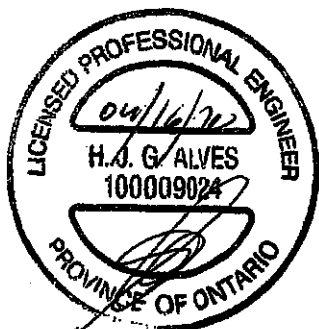
PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1687
	618	354	1687

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.77 (D) (INPUT = 1.00)

Structural component only
DWG# T-2006253



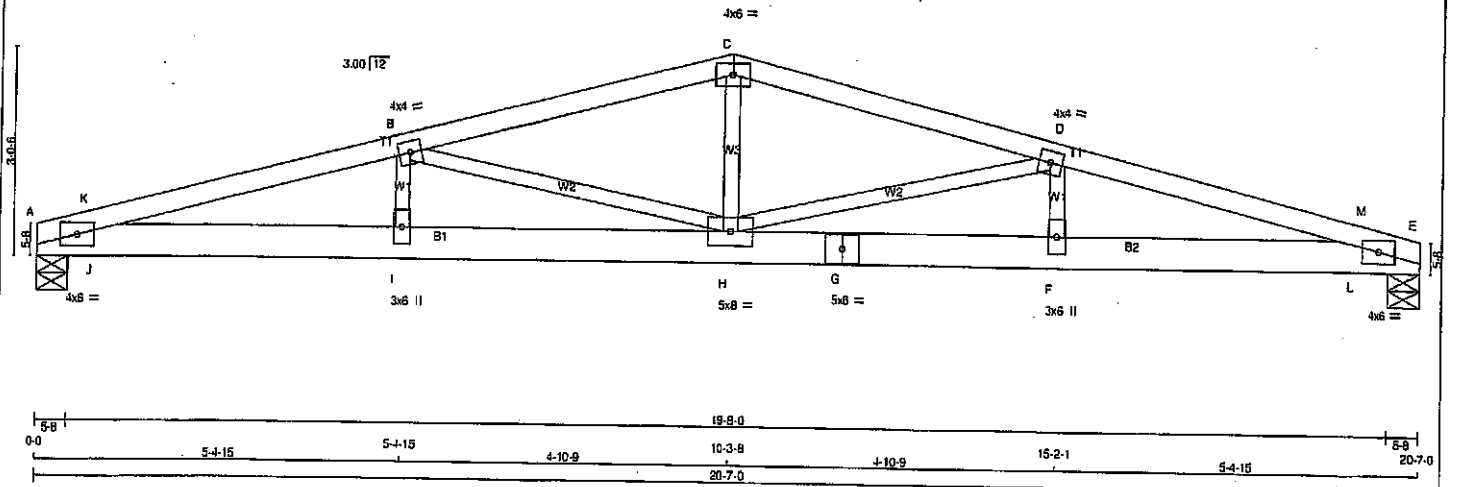
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408148	T35	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Wed Apr 15 15:57:12 2020 Page 1

ID:E_HpmQ7rOmXUV5dFqy8QOdzefin-PVK_ww4xdwystYOMW0R9gy0LnITRyoKxHYsZjzQRPt

0-0 5-4-15 5-4-15 4-10-9 10-3-8 4-10-9 15-2-1 5-4-15 20-7-0
Scale = 1:31.0



LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY No.2

C - E 2x4 DRY No.2

A - G 2x6 DRY No.2

G - E 2x6 DRY No.2

ALL WEBS 2x3 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMB1-I	MT20	4.0	6.0		
B	TWVW-I	MT20	4.0	4.0		
C	TTW-p	MT20	4.0	6.0		
D	TWVW-I	MT20	4.0	4.0		
E	TMB1-I	MT20	4.0	6.0		
F	BMVW-w	MT20	3.0	6.0		
G	BS-I	MT20	5.0	6.0		
H	BMVWVW-I	MT20	5.0	6.0		
I	BMVW-w	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
A	1135	0	1135	0	5-8	5-8
E	1135	0	1135	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	LIVE			
A	802	527.0	0.0	0.0	0.0	276.0	0.0
E	802	527.0	0.0	0.0	0.0	276.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				FACTORED				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	LC1 MAX CSI (LC)				
FR-TO		FROM	TO		FR-TO						
A-K	-3281.0	-91.8	-91.8	0.18 (1)	3.75	H-C	0.748	0.17 (1)			
K-B	-3223.0	-91.8	-91.8	0.32 (1)	3.67	H-D	-921.0	0.40 (1)			
B-C	-2326.0	-91.8	-91.8	0.33 (1)	4.20	F-D	-9.69	0.02 (4)			
C-D	-3226.0	-91.8	-91.8	0.33 (1)	4.20	B-H	-921.0	0.40 (1)			
D-M	-3223.0	-91.8	-91.8	0.32 (1)	3.67	I-B	-9.69	0.02 (4)			
M-E	-3281.0	-91.8	-91.8	0.18 (1)	3.75	J-K	-28.43	0.00 (1)			
						L-M	-28.43	0.00 (1)			
A-J	0.3132	-18.5	-18.5	0.43 (1)	10.00						
J-I	0.3132	-18.5	-18.5	0.52 (1)	10.00						
I-H	0.3132	-18.5	-18.5	0.51 (1)	10.00						
H-G	0.3132	-18.5	-18.5	0.51 (1)	10.00						
G-F	0.3132	-18.5	-18.5	0.51 (1)	10.00						
F-L	0.3132	-18.5	-18.5	0.52 (1)	10.00						
L-E	0.3132	-18.5	-18.5	0.43 (1)	10.00						

TOTAL WEIGHT = 73 b

(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.69")

CALCULATED VERT. DEFL.(LL) = L/999 (0.15")

ALLOWABLE DEFL.(TL) = L/360 (0.69")

CALCULATED VERT. DEFL.(TL) = L/873 (0.28")

CSI: TC=0.33/1.00 (B-C:1), BC=0.52/1.00 (I-J:1),

WB=0.40/1.00 (B-H:1), SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

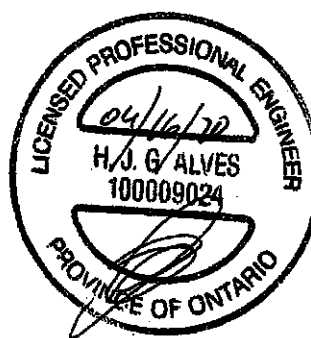
MT20 518 354 1667 788 1987 1668

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (E) (INPUT = 0.90)

JSI METAL= 0.73 (A) (INPUT = 1.00)

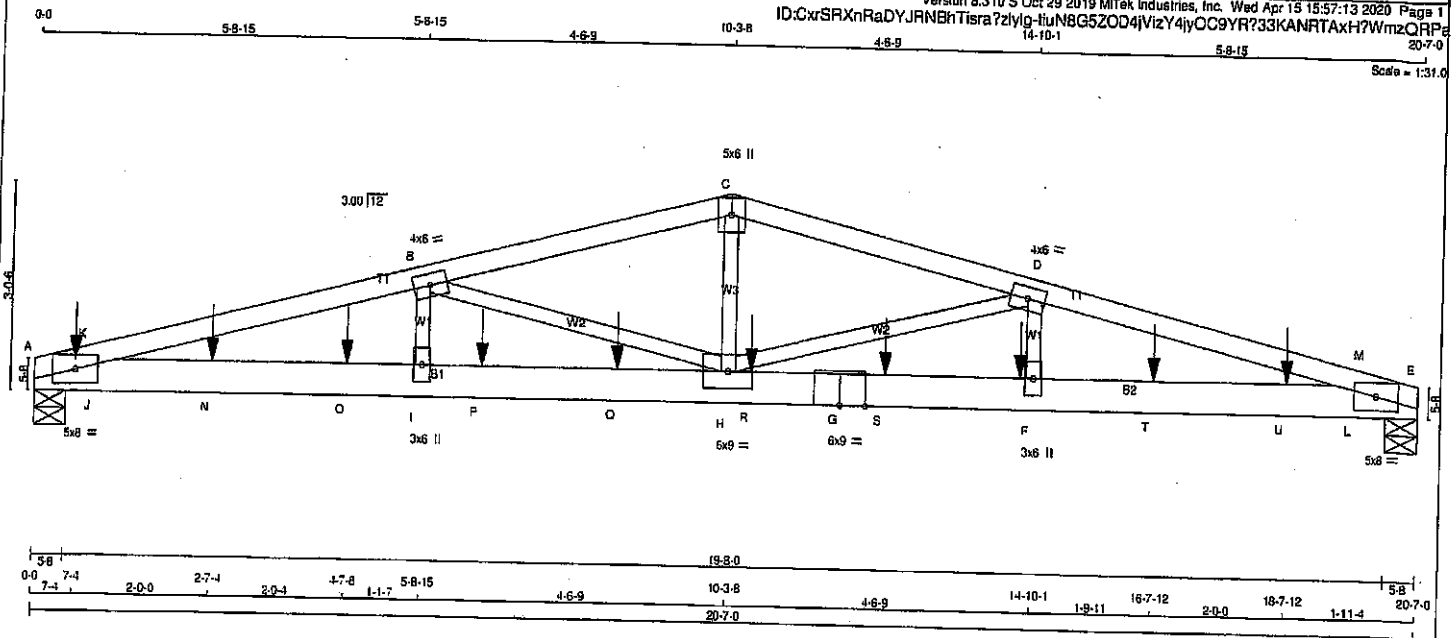


Structural component only
DWG# T-2006254

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408148	T35Z	1	2	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Wed Apr 15 15:57:13 2020 Page 1
ID:CxrSRXnRaDYJRNbTisra7zlylg-tluN8G5ZOD4jVizY4jyOC9YR733KANRTAxH7WmzQRP6
20-7-0
Scale = 1:31.0



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR
A - C	2x4	DRY	SPF
C - E	2x4	DRY	No.2
A - G	2x6	DRY	1650F 1.5E
G - E	2x6	DRY	1650F 1.5E
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			SPF

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1 12	TOP	
C-E 1 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
A-G 2 12	SIDE(0.0)	
G-E 2 12	SIDE(183.1)	
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMB1-I	MT20	5.0	8.0	
B	TMWV-I	MT20	4.0	8.0	
C	TTW+p	MT20	5.0	8.0	
D	TMWV-I	MT20	4.0	8.0	
E	TMB1-I	MT20	5.0	8.0	
F	BMW+w	MT20	3.0	6.0	
G	BS-I	MT20	6.0	9.0	
H	BMWVW-I	MT20	6.0	9.0	
I	BMW+w	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
A	3775	0	0	5-8
E	3203	0	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	2647	1865.0	0.0	0.0	0.0	782.0	0.0
E	2248	1575.0	0.0	0.0	0.0	673.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
A-K	-10604.0	-91.8	-91.8	0.36 (1)	2.75	H-C	0.3272	0.41 (1)	
K-B	-10343.0	-91.8	-91.8	0.68 (1)	2.58	H-D	-2394.0	0.47 (1)	
B-C	-7553.0	-91.8	-91.8	0.40 (1)	3.30	F-D	0.886	0.11 (1)	
C-D	-7552.0	-91.8	-91.8	0.41 (1)	3.28	B-H	-2862.0	0.56 (1)	
D-M	-9891.0	-91.8	-91.8	0.61 (1)	2.72	I-B	0.1117	0.14 (1)	
M-E	-10103.0	-91.8	-91.8	0.36 (1)	2.82	J-K	0.863	0.00 (1)	
						L-M	0.497	0.00 (1)	
A-J	0.10039	-18.5	-18.5	0.48 (1)	10.00				
J-N	0.10039	-18.5	-18.5	0.67 (1)	10.00				
N-O	0.10039	-18.5	-18.5	0.67 (1)	10.00				
O-I	0.10039	-18.5	-18.5	0.67 (1)	10.00				
I-P	0.10039	-18.5	-18.5	0.57 (1)	10.00				
P-Q	0.10039	-18.5	-18.5	0.57 (1)	10.00				
Q-H	0.10039	-18.5	-18.5	0.57 (1)	10.00				
H-R	0.8595	-18.5	-18.5	0.55 (1)	10.00				
R-G	0.8595	-18.5	-18.5	0.55 (1)	10.00				
G-S	0.8595	-18.5	-18.5	0.55 (1)	10.00				
S-F	0.8595	-18.5	-18.5	0.55 (1)	10.00				
F-T	0.8595	-18.5	-18.5	0.60 (1)	10.00				
T-U	0.8595	-18.5	-18.5	0.60 (1)	10.00				
U-L	0.8595	-18.5	-18.5	0.60 (1)	10.00				
L-E	0.8595	-18.5	-18.5	0.39 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	MAX.	MAX.	FACE	VERT.	TOTAL	REQD
F	14-7-12	-507	-507	FRONT	VERT	TOTAL	C1
J	7-4	-510	-510	FRONT	VERT	TOTAL	C1
N	2-7-4	-507	-507	FRONT	VERT	TOTAL	C1
O	4-7-8	-507	-507	FRONT	VERT	TOTAL	C1
P	8-7-12	-507	-507	FRONT	VERT	TOTAL	C1
Q	8-7-12	-507	-507	FRONT	VERT	TOTAL	C1
R	10-7-12	-507	-507	FRONT	VERT	TOTAL	C1
S	12-7-12	-507	-507	FRONT	VERT	TOTAL	C1
T	16-7-12	-387	-387	FRONT	VERT	TOTAL	C1
U	18-7-12	-285	-285	TOP	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 72 = 145 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.8	PSF
DL	= 8.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 21.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/981 (0.25")
ALLOWABLE DEFL.(TL) = L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/559 (0.44")

CSI: TC=0.88/1.00 (B-K:1), BC=0.67/1.00 (I-J:1),
WB=0.56/1.00 (B-H:1), SSI=0.33/1.00 (F-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

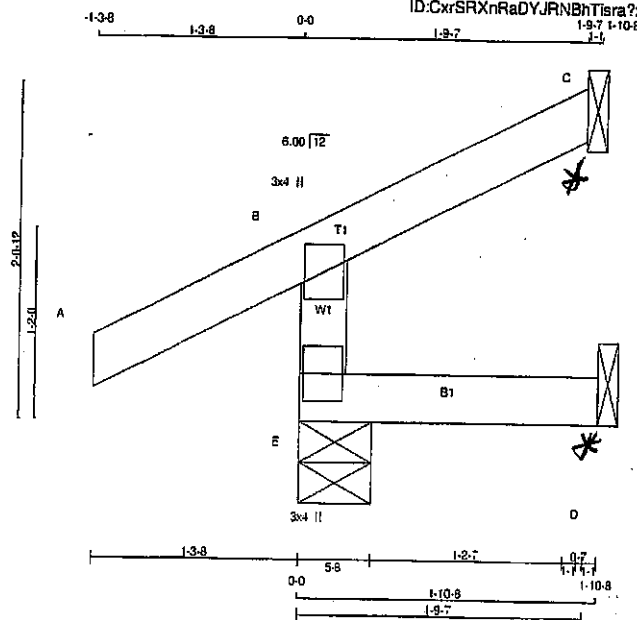
JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.67 (E) (INPUT = 1.08)

Structural component only
DWG# T-2006255

JOB NAME 408146	TRUSS NAME J1	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mittek Industries, Inc. Wed Apr 15 14:02:18 2020 Page 1
ID: CxrSRXnRaDYJRNbHtIsra?zlylg-FDmn1dlaAkN2o5MOg8lUG8UGse26A6lagiYf0vzQT5J

Scale = 1:13.1



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED		MAXIMUM FACTORED		INPUT	REQRD
	GROSS REACTION	DOWN	GROSS REACTION	DOWN		
E	271	0	271	0	5-8	5-8
C	45	0	45	0	1-8	1-8
D	8	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	188	141	0	0	0	47	0
C	31	24	-18	0	0	7	0
D	7	0	-8	0	0	12	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		W E B S	MAX. FACTORED	
	FORCE (LBS)	VERT. LOAD (LBS)	VERT. LOAD (LBS)	MAX. (LBS)		FORCE (LBS)	MAX. (LBS)
FR-TO					LENGTH FR-TO		
E-B	-244	0	0.0	0.0	0.04 (5)	7.81	
A-B	0	28	-91.8	-91.8	0.12 (1)	10.00	
B-C	-17	0	-91.8	-91.8	0.09 (1)	8.25	
E-D	0	0	-18.5	-18.5	0.04 (5)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.18")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.18")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5), WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	618	354
	1887	788
	1887	1656

PLATE PLACEMENT TOL. = 0.250 inches

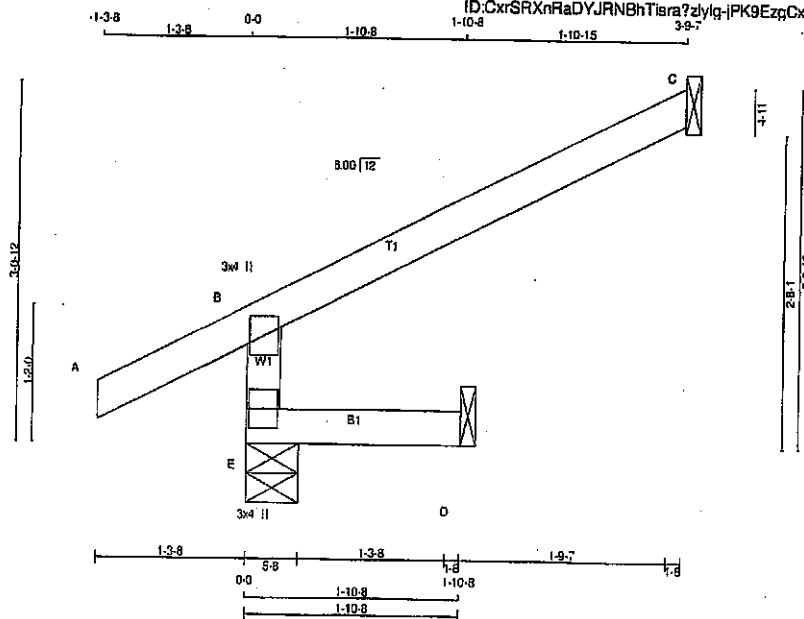
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) INPUT = 0.90
JSI METAL= 0.07 (B) INPUT = 1.00



Structural component only
DWG# T-2006223

JOB NAME 408146	TRUSS NAME J2	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	No.2
E - B	2x4	DRY	No.2		
A - C	2x4	DRY	No.2		
E - D	2x4	DRY	No.2		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
E	361	361	5-8	5-8
C	130	130	1-8	1-8
D	16	17	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
E	250	190 0	0 0	0 0	60 0	0 0
C	90	73 0	0 0	0 0	17 0	0 0
D	12	0 0	0 0	0 0	12 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
E-B	FR-TO	-342 0	0.0	0.0	0.01 (4)	7.81	FR-TO	-342 0	0.01 (4)
A-B		0 28	-91.8	-91.8	0.13 (5)	10.00		0 28	0.13 (5)
B-C		-19 0	-91.8	-91.8	0.22 (1)	6.25		-19 0	0.22 (1)
E-D		0 0	-18.5	-18.5	0.02 (4)	10.00		0 0	0.02 (4)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 10 = 19 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6 PSF
DL	= 6.0 PSF	
BOT CH.	LL	= 0.0 PSF
DL	= 7.4 PSF	
TOTAL LOAD	= 39.0 PSF	

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 089-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.22:1.00 (B-C:1), BC=0.02:1.00 (D-E:4), WB=0.00:1.00 (n/a:0), SSI=0.15:1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

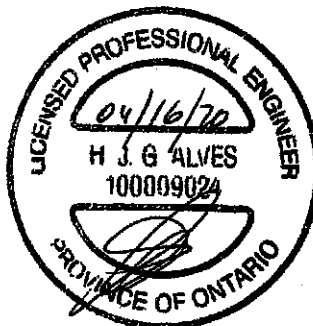
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX MIN
MT20	818	354 1687 788 1987 1856

PLATE PLACEMENT TOL = 0.250 inches

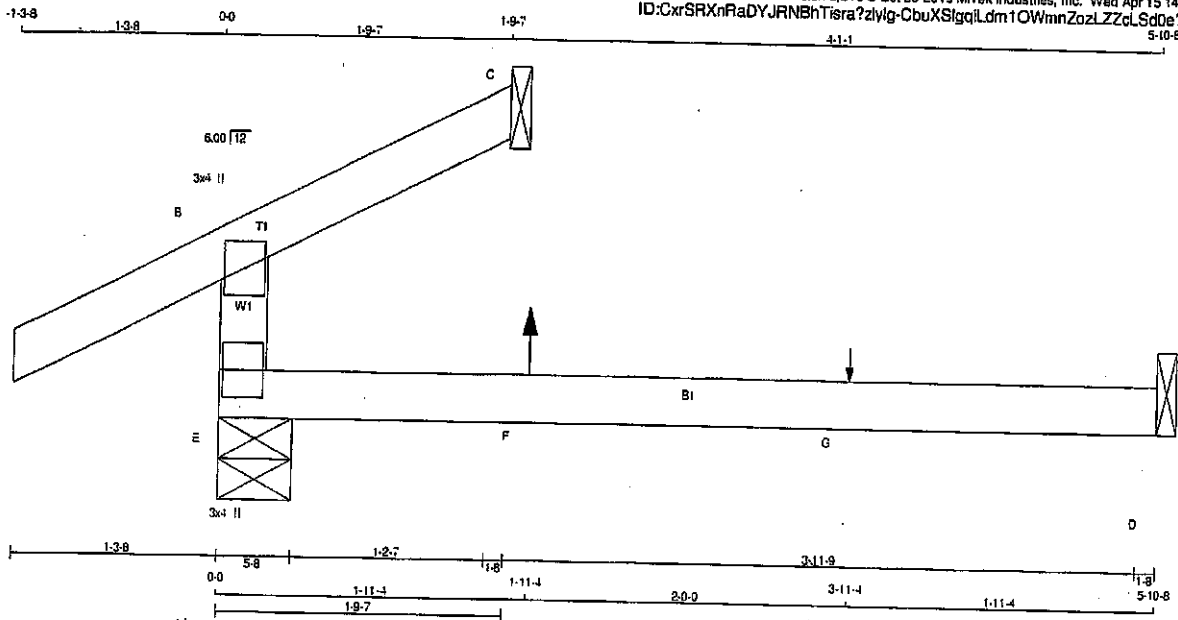
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2006224

JOB NAME 408146	TRUSS NAME J3	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8,310 S Oct 29 2019 MITek Industries, Inc. Wed Apr 15 14:02:20 2020 Page 1	



LUMBER

N. L. G. A. RULES
CHORDS SIZE
E - B 2x4 DRY
A - C 2x4 DRY
E - D 2x4 DRY

LUMBER
No.2
No.2
No.2

DESCR.
SPF
SPF
SPF

DRY, SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	284	0	284	0	5-8	5-8
C	83	0	83	0	1-8	1-8
D	44	0	52	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791 H FOR CONNECTION TO JOINT(S) E, C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX. MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	SNOW	LIVE	SNOW	LIVE				
E	200	137	0	0	0	0	82	0
C	46	21	0	0	0	0	25	0
D	35	0	-3	0	0	0	37	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FORCE	FACTORED	FACTORED	MAX.	MEMB.	MAX. FORCE	FACTORED
	(LBS)	VERT. LOAD	LOAD	MAX.		(LBS)	MAX.
		(PLF)	CSI (LC)	UNBRAC			CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
E-B	-227.8	0.0	0.0	0.11 (4)	7.81		
A-B	0.28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-9.9	-91.8	-91.8	0.08 (4)	10.00		
E-F	0.0	-18.5	-18.5	0.14 (4)	10.00		
F-G	0.0	-18.5	-18.5	0.14 (4)	10.00		
G-D	0.0	-18.5	-18.5	0.14 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	BACK	VERT	TOTAL	---	C1
G	3-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 12 = 23 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.12/1.00 (A-B:1), BC=0.14/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SS=0.09/1.00 (A-B:1)

DOL LUMBER=0.89 NAIL=0.89 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

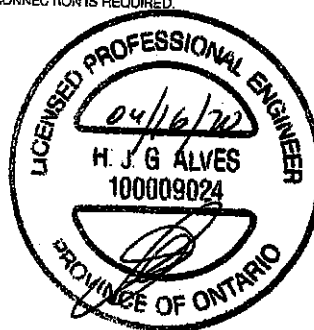
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 789 1987 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) INPUT = 0.90

JSI METAL= 0.08 (B) INPUT = 1.00

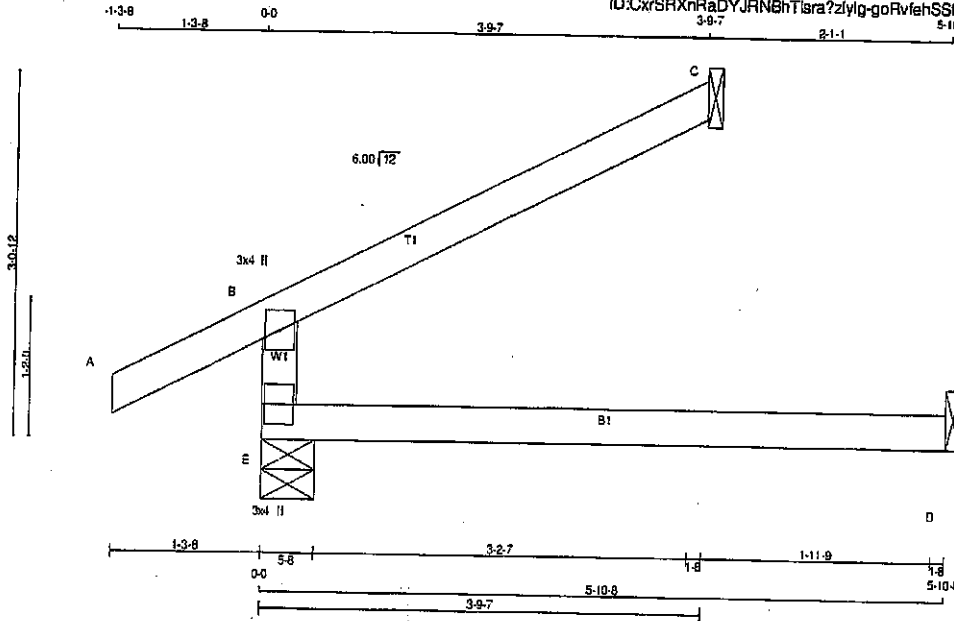


Structural component only
DWG# T-2006225

JOB NAME 408146	TRUSS NAME J4	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Wed Apr 15 14:02:22 2020 Page 1
ID: CxrSRXnRaDYJRNhTara?zlylg-goRvfhSSlcfY5zLHJCn6UusyLNS11MgnJcDzQT5G

Scale = 1:17.8



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	405	0	405	0	5-8	5-8
C	130	0	130	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS				WIND	DEAD	SOIL
		SNOW	LIVE	PERM. LIVE	WIND			
E	288	190	0	0	0	0	98	0
C	90	73	0	0	0	0	17	0
D	35	0	0	0	0	0	36	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		VERT. LOAD (PL)	LC1 MAX	MAX. MEMB. FORCE (LBS)	W E B S	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
	FR-TO	FROM TO						
E-B	-342	0	0.0	0.0	0.13 (4)	7.81		
A-B	0	28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-19	0	-91.8	-91.8	0.22 (1)	6.25		
E-D	0	0	-18.5	-18.5	0.13 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.22/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),

WB=0.00/1.00 (n/a:0), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

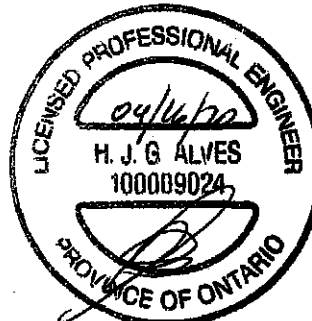
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 518 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

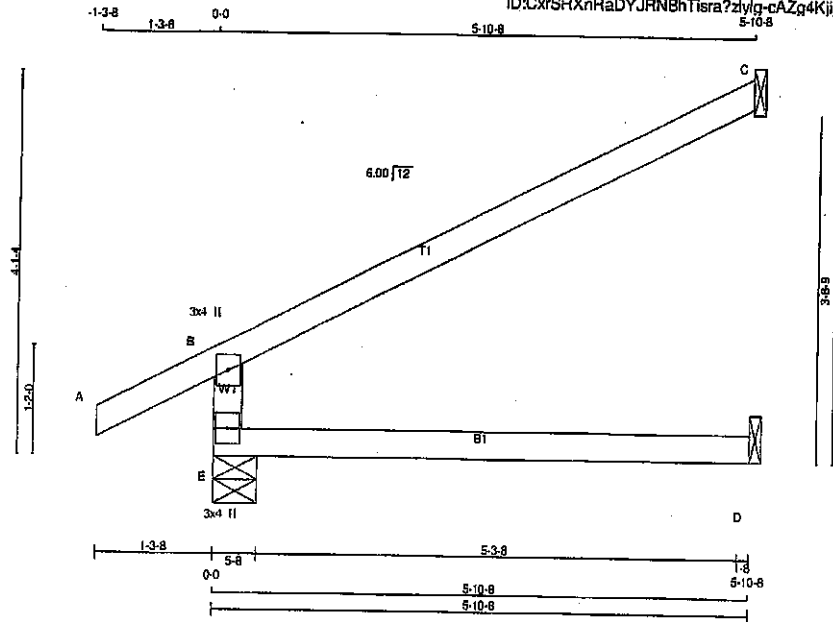
JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2006226

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408146	J5	11	1	GREEN PARK HOMES	
Famrack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Wed Apr 15 14:02:23 2020 Page 1
ID:CxrSRXnRaDYJRNhTisra?zylg-cAZg4Kjl_G7KusFLSLgzCB01tprMXKp_GQh8zQT5E



Scale = 1:22.9

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	No.2
E - B	2x4	DRY	No.2		
A - C	2x4	DRY	No.2		
E - D	2x4	DRY	No.2		

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
E	525	0	525	0
C	202	0	202	0
D	45	0	50	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	369	257	0	0	0	111	0
C	139	113	0	0	0	26	0
D	38	0	0	0	0	36	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORZ. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)
FR-TO					
E-B	-481	0	0.0	0.0	0.13 (4)
A-B	0	28	-91.8	-91.8	0.12 (1)
B-C	-30	0	-91.8	-91.8	0.54 (1)
E-D	0	0	-18.5	-18.5	0.13 (4)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 085-08, CSA 085-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CSI: TC=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS=0.24/1.00 (B-C:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

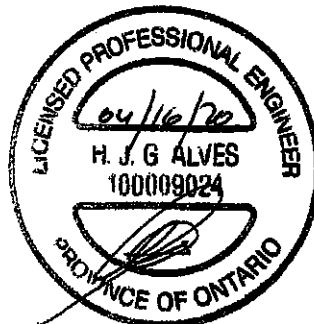
NAIL VALUES

PLATE GRIP (DRY)	SHEAR SECTION (PSI)	PLI	PLI	PLI
MAX	MIN	MAX	MIN	MAX
MT20	618	354	1667	788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E) (INPUT = 0.30)
JSI METAL= 0.13 (B) (INPUT = 1.00)

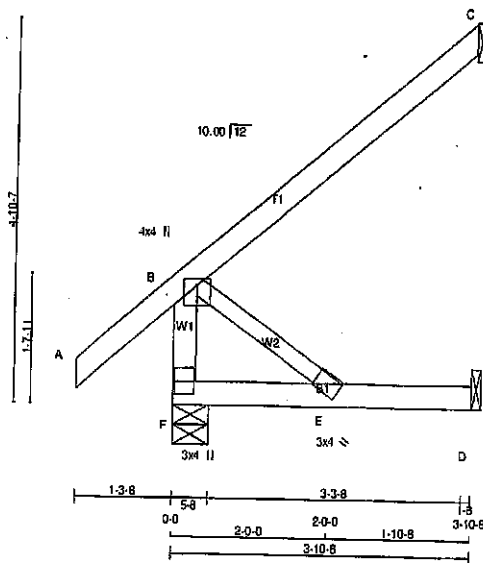


Structural component only
DWG# T-2006227

JOB NAME 408146	TRUSS NAME J6	QUANTITY 6	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Wed Apr 15 14:02:24 2020 Page 1
ID: CxrSRXnRaDYJRNhTIsra?ziylg-4M72HgkLa8BW0qY0PsvVPkGY3?papnT2e7zDZzQT5C

Scale = 1:27.0



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
F - D	2x4 DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMVW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
JT	VERT	HORZ	DOWN	HORZ
F	341	0	341	0
C	178	0	178	0
D	36	0	40	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	239	170	0	0	0	69	0
C	122	99	0	0	0	23	0
D	29	0	0	0	0	29	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (FR-TO)	LC1 MAX (LC)
FR-TO							
F-B	-305	0	0.0	0.03 (1)	7.81	0	0.00 (1)
A-B	0	-91.8	-91.8	0.14 (5)	10.00		
B-C	0	-91.8	-91.8	0.23 (1)	10.00		
F-E	0	-18.5	-18.5	0.08 (4)	10.00		
E-D	0	-18.5	-18.5	0.08 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 6 X 16 = 93 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL	= 6.0	PSF
BOT CH.	LL = 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.23/1.00 (B-C:1), BC=0.08/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 788

PLATE PLACEMENT TOL. = 0.250 inches

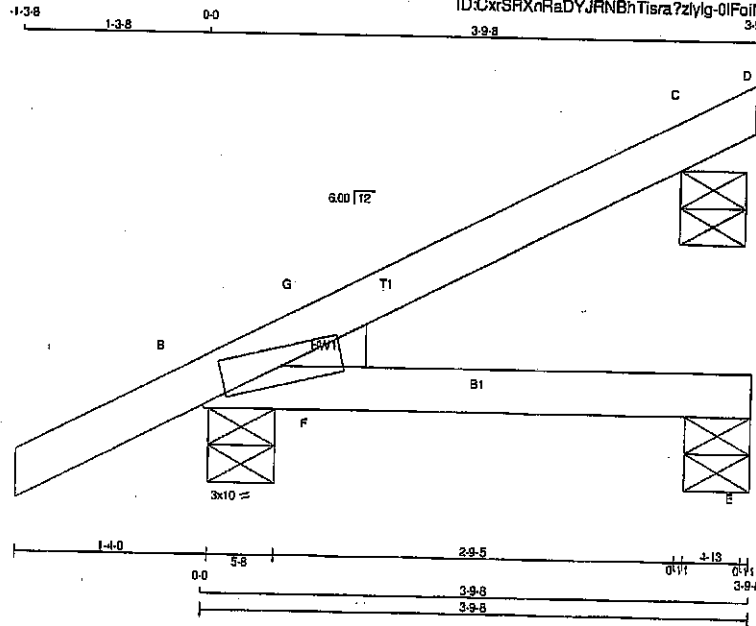
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.24 (B) (INPUT = 0.90)
JSI METAL=0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2006228

JOB NAME 408146	TRUSS NAME J8	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
B - E	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	10.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
B	322	0	322	0	5-8	5-8	2x4 L
E	57	0	57	0	5-8	5-8	
C	169	0	169	0	5-8	5-8	

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
		COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
B	225	160.0	0.0	0.0	0.0	85.0	0.0
E	42	16.0	0.0	0.0	0.0	27.0	0.0
C	117	90.0	0.0	0.0	0.0	27.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	WEBS	
			MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO				
A-B	0.26	-91.8	-91.8	0.14 (5)
B-G	-22.0	-91.8	-91.8	0.05 (1)
G-C	0.7	-91.8	-91.8	0.14 (1)
C-D	-7.0	-91.8	-91.8	0.01 (1)
B-F	0.0	-18.5	-18.5	0.12 (1)
F-E	0.0	-18.5	-18.5	0.12 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 12 = 47 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/398 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/398 (0.02")

CSI: TC=0.14/1.00 (C-G:1), BC=0.12/1.00 (B-F:1),
WB=0.00/1.00 (F-G:1), SSI=0.10/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

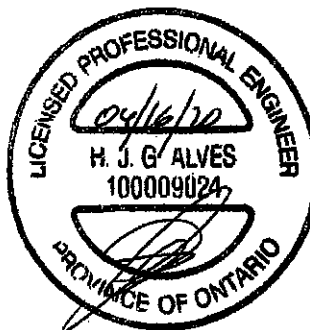
PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.08 (B) (INPUT = 0.90)

JSI METAL=0.02 (B) (INPUT = 1.00)



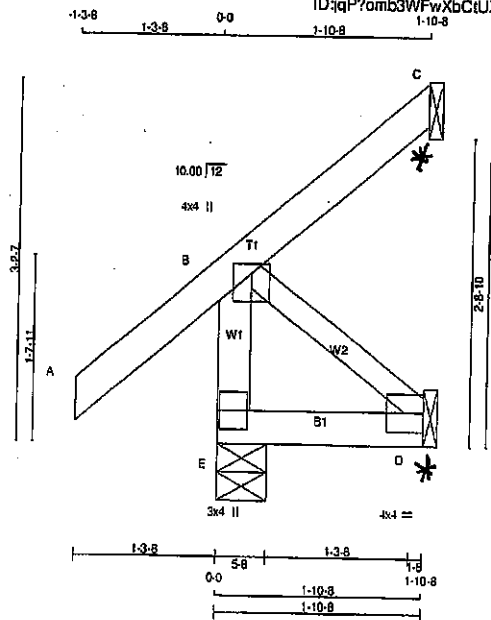
Structural component only
DWG# T-2006230

JOB NAME 408147	TRUSS NAME J21	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
--------------------	-------------------	---------------	----------	-------------------------------	-------------	----------

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Wed Apr 15 15:52:24 2020 Page 1
ID:jqP?omb3WfWxXbCtUzIsE03zixMq-q5w6URbHRQRKIN?XGd?D4BeZbuzohxjTnCl0zQRUS

Scale = 1:18.9



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
E - B	2x4	DRY No.2
A - C	2x4	DRY No.2
E - D	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (tables in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV+p	MT20	4.0	4.0	1.00	2.00
D	BMW1+1	MT20	4.0	4.0	2.00	Edge
E	BMW1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
JT	VERT	DOWN	UP	
E	277	0	0	5-8
C	39	0	-37	1-8
D	17	0	0	1-8

* SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE WIND	DEAD	SOIL
E	192	145 / 0	0 / 0	47	0
C	27	22 / -26	0 / 0	5	0
D	13	0 / 0	0 / 0	13	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORZ. LOAD (LC)	MAX. FACTORED LENGTH FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LENGTH FR-TO
E-B		-268	0	0.0	0.03 (1)			0	0.00 (1)
A-B		0 / 41	-91.8	-91.8	0.13 (1)			0	0.00 (1)
B-C		-30 / 0	-91.8	-91.8	0.12 (1)			0	0.00 (1)
E-D		0 / 0	-18.5	-18.5	0.02 (4)			0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 10 = 40 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN.C.C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (B-C:1), SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

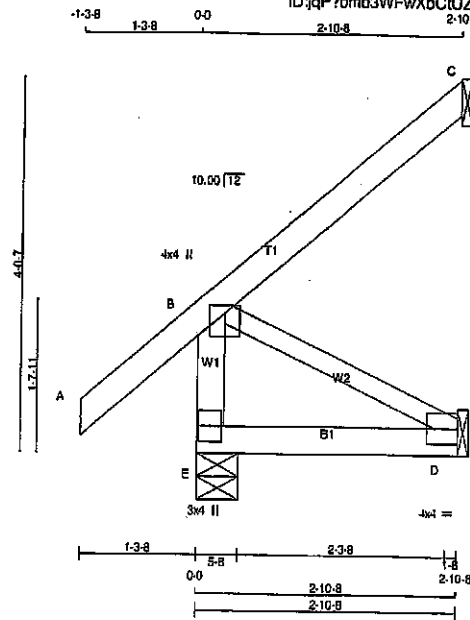
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2006244

JOB NAME 408147	TRUSS NAME J22	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
D BMV1-t	MT20	4.0	4.0	2.00	Edge
E BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORIZ	DOWN	UPLIFT
E	284	0	284	0
C	132	0	132	0
D	25	0	25	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERMILIVE WIND DEAD SOIL
E	199 144.0 0.0 0.0 0.0 54.0 0.0
C	91 74.0 0.0 0.0 0.0 17.0 0.0
D	20 0.0 0.0 0.0 0.0 20.0 0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO					FR-TO		
E-B	-259.0	0.0	0.0	0.03 (1)	7.61	0.0	0.00 (1)
A-B	0/41	-91.8	-91.8	0.13 (5)	10.00		
B-C	0.0	-91.8	-91.8	0.13 (1)	10.00		
E-D	0.0	-18.5	-18.5	0.04 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. G.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI TC=0.13/1.00 (A-B:5), BC=0.04/1.00 (D-E:4), WB=0.00/1.00 (B-D:1), SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

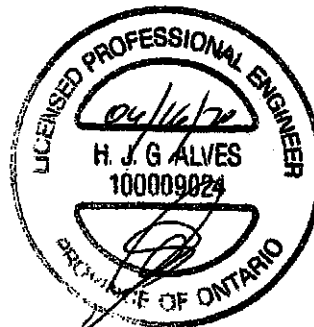
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

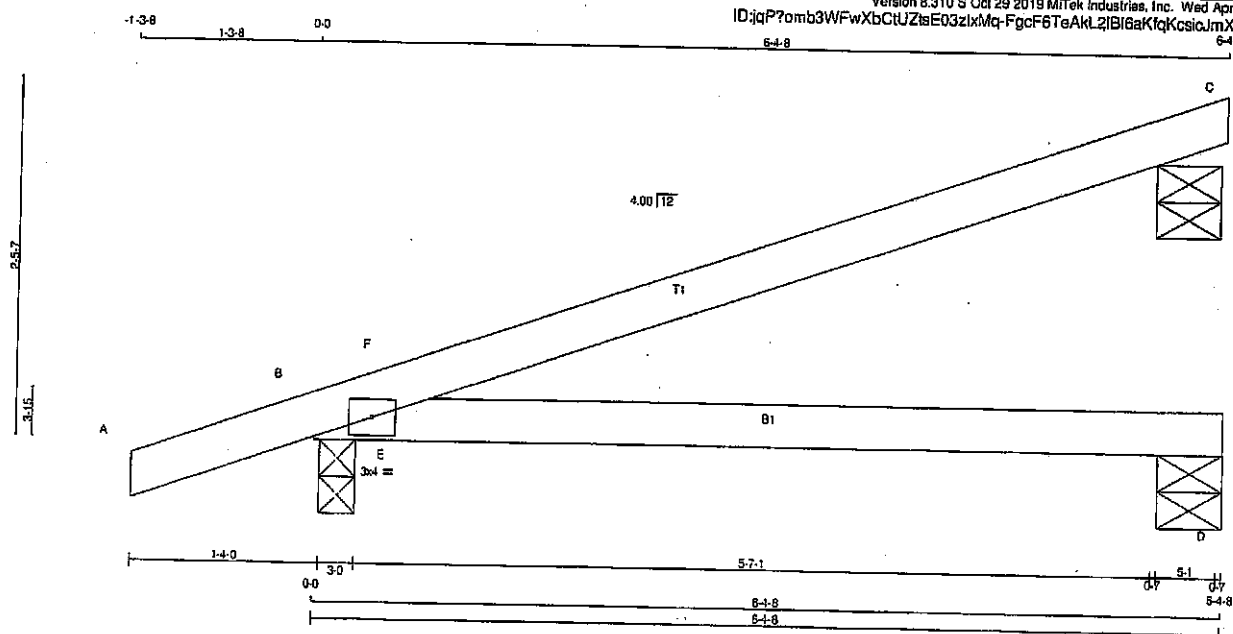


Structural component only
DWG# T-2006245

JOB NAME 408147	TRUSS NAME J23	QUANTITY 6	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Wed Apr 15 15:52:27 2020 Page 1
ID:jqp7omb3WFwXbCUZtsE03zlxMq-FgcF6TeAkl2lBlaKqKscicJmXeA8QNPR7lULzQRU2

Scale = 1:14.8



LUMBER
N. L. G. A. RULES
CHORDS SIZE
A - C 2x4 DRY
B - D 2x4 DRY
LUMBER No.2
DESCR. SPF
SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMB1-1 MT20 3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
C	256	0	256	0	5-8	5-8	5-8	5-8
B	474	0	474	0	3-0	3-0	3-0	3-0
D	95	0	95	0	5-8 1/2	5-8	5-8	5-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	COMBINED	MAX. MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	177	138 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0
B	333	232 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0
D	71	25 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S): C, B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED				WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (LBS)	VERT. (PLF)	LC1 MAX	LC1 MAX	UNBRAC LENGTH	MEMB. FR-TO	MAX. FORCE (LBS)	MAX. (LBS)	CSI (LC)
FR-TO										
A-B	0 18		-91.8	-91.8	0.11 (1)	10.00	E-F	-381 / 15	0.00 (1)	
B-F	-22 / 28		-91.8	-91.8	0.07 (1)	6.25				
F-C	-3 / 2		-91.8	-91.8	0.48 (1)	10.00				
B-E	0 0		-18.5	-18.5	0.33 (1)	10.00				
E-D	0 0		-18.5	-18.5	0.33 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PARTS. NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/795 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/387 (0.20")

CSI: TO=0.48/1.00 (C-F:1), BC=0.33/1.00 (D-E:1),
WB=0.00/1.00 (E-F:1), SSI=0.31/1.00 (B-E:1)

DDL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

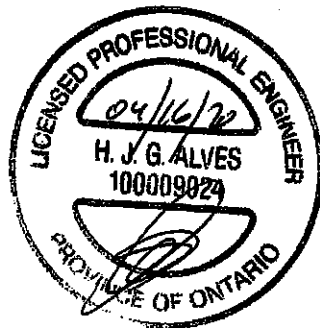
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1967 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.32 (B) (INPUT = 0.80)
JSI METAL = 0.09 (B) (INPUT = 1.00)



LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

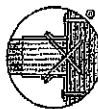
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

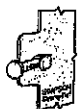
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/4" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

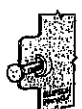
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options Information on p. 126.



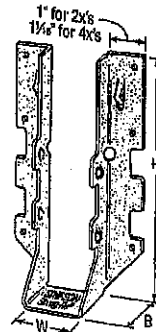
Double-Shear Nailing Top View



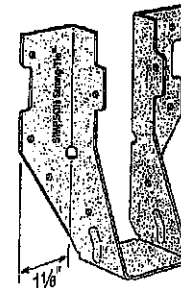
Double-Shear Nailing Side View; Do not bend tab



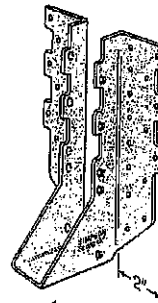
Dome Double-Shear Nailing Side View (available on some models) U.S. Patent 5,603,560



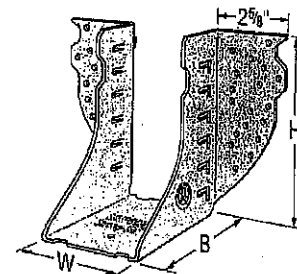
LUS28



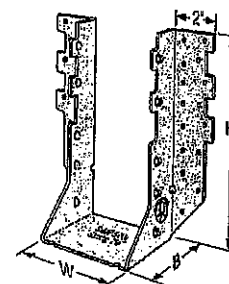
LU26L



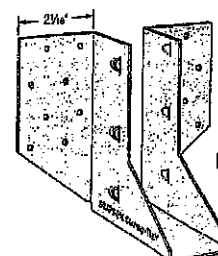
HUS210
(HUS26, HUS28, and HHUS similar)



HGUS28-2

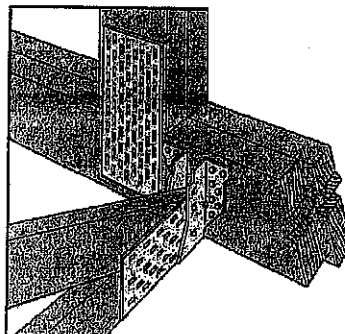


HHUS210-2



LJS26DS

Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)



Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS**HHUS/HGUS**

See Hanger Options Information on pp. 125–127.

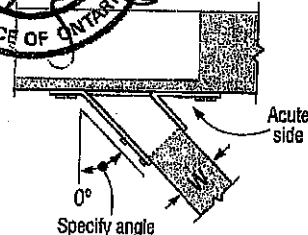
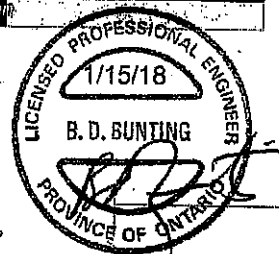
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
$W < 2"$	Bevel or square cut	0.62 of table value	0.46 of table value
$2" < W < 6"$	Bevel cut	0.67 of table value	0.41 of table value
$2" < W < 6"$	Square cut	0.46 of table value	0.41 of table value
$W > 6"$	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(Joist must be bevel cut)
All joist nails installed on the outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _g ³	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15) lb. kN	(K _D = 1.00) lb. kN	(K _D = 1.15) lb. kN	(K _D = 1.00) lb. kN
Single 2x Sizes											
LUS24	18	1 9/16	3 1/4	1 3/4	2 1/4	(4) 10d	(2) 10d	710 3.16	1625 7.23	645 2.87	1155 5.14
LU24L	22	1 9/16	3	1 3/4	2 1/4	(4) 10d	(2) 10d x 1 1/2"	360 1.60	1020 4.54	320 1.42	725 3.22
LU26L	22	1 9/16	5	1 3/4	4 3/4	(6) 10d	(4) 10d x 1 1/2"	720 3.20	1605 7.14	645 2.87	1140 5.07
LUS26	18	1 9/16	4 3/4	1 3/4	3 3/4	(4) 10d	(4) 10d	1420 6.32	2170 9.65	1290 5.74	1630 7.25
HUS26	16	1 1/2	5 1/4	3	3 3/4	(14) 16d	(6) 16d	2705 11.90	4940 21.97	2065 9.20	3875 17.24
LJS26DS	18	1 9/16	5	3 1/2	4 3/4	(16) 16d	(6) 16d	2055 9.14	4265 18.97	1460 6.49	4115 18.31
HGUS26	12	1 1/2	5 1/4	5	4 3/4	(20) 16d	(6) 16d	2685 11.96	6625 29.51	2685 11.96	5700 25.35
LU28L	20	1 9/16	6 3/4	1 3/4	5 3/4	(8) 10d	(6) 10d x 1 1/2"	1140 5.07	2185 9.72	1020 4.54	1550 6.89
LUS28	18	1 9/16	6 3/4	1 3/4	3 3/4	(6) 10d	(4) 10d	1420 6.32	2520 11.21	1290 5.74	1790 7.96
HUS28	16	1 1/2	7 1/4	3	6 3/4	(22) 16d	(8) 16d	3805 16.04	6365 23.88	2675 11.90	4345 19.33
HGUS28	12	1 1/2	7 1/4	5	6 3/4	(36) 16d	(12) 16d	3310 14.74	7675 34.19	3310 14.74	5900 30.73
LU210L	20	1 9/16	8	1 3/4	7 3/4	(10) 10d	(6) 10d x 1 1/2"	1140 5.07	2495 11.10	1020 4.54	1770 7.87
LUS210	18	1 9/16	7 1/4	1 3/4	3 3/4	(8) 10d	(4) 10d	1420 6.32	2785 12.39	1290 5.74	2210 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_g is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

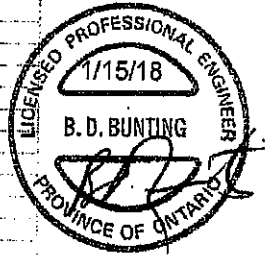
Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _g ¹	Header	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _g = 1.15)	(K _g = 1.00)	(K _g = 1.15)	(K _g = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
LUS24-2	18	3 1/8	3 1/8	2	1 1/4	(4) 16d	(2) 16d	835	2020	590	1435
SS LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	3.71	8.99	2.62	6.38
								1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS26-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2850	7335	2065	5205
								12.68	32.63	9.20	23.15
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
SS LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS28-2	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
SS LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
								20.77	42.07	18.84	31.14
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(48) 16d	(18) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
Triple 2x Sizes											
HGUS26-3	12	4 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d	4670	9670	4235	6865
								20.77	43.02	18.84	30.54
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(48) 16d	(18) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-4	14	6 1/8	8 1/8	3	7 1/8	(30) 16d	(10) 16d	4670	10155	4235	7210
								20.77	45.17	18.84	32.07
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(48) 16d	(18) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
LUS46	18	3 1/8	4 3/8	2	3 1/8	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS46	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2540	7335	2065	5205
								11.30	32.63	9.20	23.15
HGUS46	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS48	18	3 1/8	6 3/8	2	3 1/8	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS48	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS48	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS410	18	3 1/8	8 3/8	2	5 1/8	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HGUS410	12	3 1/8	9	4	8 1/8	(48) 16d	(18) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
HGUS412	12	3 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS414	12	3 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80



Plated Truss Connectors

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1/4". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

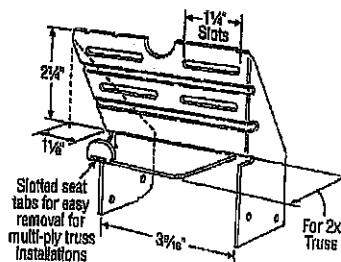
Design: Factored resistances are in accordance with CSA 086-14

Installation:

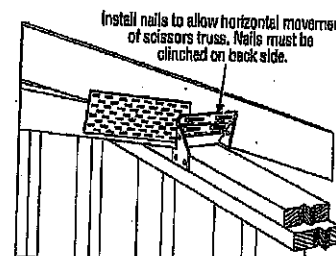
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

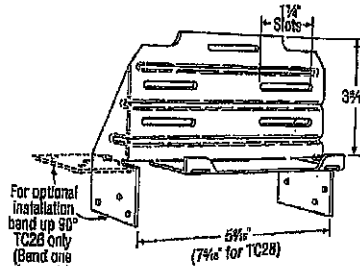
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



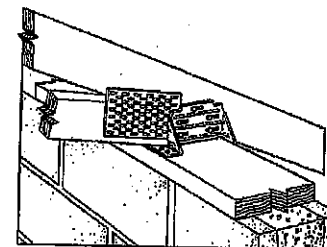
TC24
U.S. Patent 4,932,173



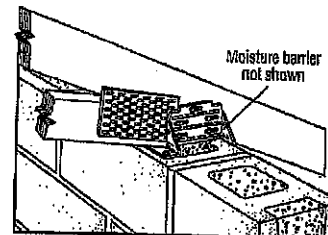
Typical TC24 Installation



TC26
(TC28 Similar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _o =1.15)	Uplift (K _o =1.15)
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _o =1.15)	Uplift (K _o =1.15)
TC26	(5) 10d	(6) 10d x 1 1/2"	810	680
	(5) 10d	(6) 10d	930	680

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) - 3/8" x 2 1/4" Titen screws has a factored uplift resistance of 275 lb.



LIMIT
STATES
DESIGN

© 2017 Simpson Strong-Tie Company
Simpson Strong-Tie is a registered trademark of Simpson Strong-Tie Company.
All other trademarks are the property of their respective owners.

(800) 999-5099
strongtie.com

H/TSP**SIMPSON**
Strong-Tie**Seismic and Hurricane Ties (cont.)**

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Fasteners			Factored Resistance ($K_D = 1.15$)					
		To Rafters/ Truss	To Plates	To Studs	D.Fir-L			S-P-F		
					Uplift	Lateral		Uplift	Lateral	
						F ₁	F ₂		F ₁	F ₂
					lb.	lb.	lb.	lb.	lb.	lb.
					kN	kN	kN	kN	kN	kN
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
					3.29	3.05	1.33	3.02	2.13	0.96
SS H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
					3.69	0.98	0.33	2.62	0.69	0.24
SS H2.5A	18	(5) 8d	(5) 8d	—	805	180	180	755	160	160
					3.58	0.71	0.71	3.36	0.71	0.71
H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
					3.71	0.78	0.93	3.29	0.71	0.93
SS H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
					3.29	0.80	1.18	2.74	0.56	0.85
H6	16	—	(8) 8d	(8) 8d	1585	1085	—	1125	770	—
					7.05	4.83	—	5.00	3.43	—
H7Z	16	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
					6.18	2.98	—	4.40	2.11	—
SS H8 ^a	18	(5) 10d x 1½"	(5) 10d x 1½"	—	1120	—	—	1025	—	—
					4.98	—	—	4.56	—	—
SS H10A ^a	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290
					7.72	3.54	1.82	6.69	2.51	1.29
H10AR	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1485	690	430	1220	570	305
					6.61	3.07	1.91	5.43	2.54	1.36
H10A-2	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1835	1275	430	1645	890	305
					8.18	5.67	1.91	7.32	3.91	1.36
H10S ^{a,b}	18	(8) 8d x 1½"	(8) 8d x 1½"	(8) 8d	1465	795	315	1040	565	225
					6.52	3.54	1.40	4.63	2.51	1.00
H11Z	18	(6) 16d x 2½"	(6) 16d x 2½"	—	1095	920	545	780	655	390
					4.87	4.09	2.42	3.47	2.91	1.73
H14	18	1 (12) 8d x 1½"	(13) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
		2 (12) 8d x 1½"	(15) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
TSP	18	(9) 10d x 1½"	(6) 10d x 1½"	—	1295	440	—	820	310	—
					5.76	1.98	—	4.09	1.38	—
		(9) 10d x 1½"	(6) 10d	—	1560	440	—	1105	310	—
					6.94	1.98	—	4.92	1.38	—

- Factored resistances have been increased 15% for short term loading; no further increase is allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
- H8 factored uplift resistances for stud-to-bottom plate installations are 595 lb. (2.65 kN) for D.Fir-L and 390 lb. (1.74 kN) for S-P-F.
- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-HIECONPATH).
- Factored resistances in the F₁ direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.

- H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.39 kN) D.Fir-L and 1015 lb. (4.51 kN) S-P-F.
- H10S nails to plates are optional for uplift but required for lateral loads.
- H10A may be field-bent up to a slope of 6/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
- The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SSNAILS at strongtie.com.
- D.Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1½" nails into the top plates and (3) 8d x 1½" nails into the lowest three flange holes into the truss bottom chord is 485 lb. (2.20 kN).
- Nails: 16d x 2½" = 0.162" dia., x 2½" long, 10d = 0.148" dia., x 3" long, 10d x 1½" = 0.148" dia., x 1½" long, 8d = 0.131" dia., x 2½" long, 8d x 1½" = 0.131" dia., x 1½" long. See pp. 27-28 for other nail sizes and information.

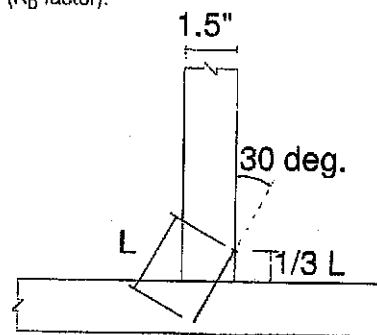
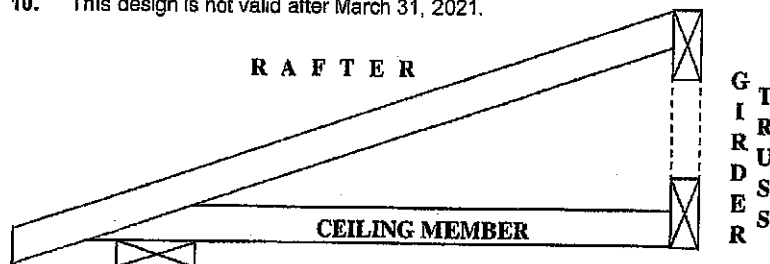
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.

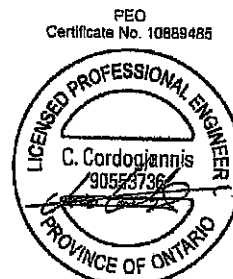


TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

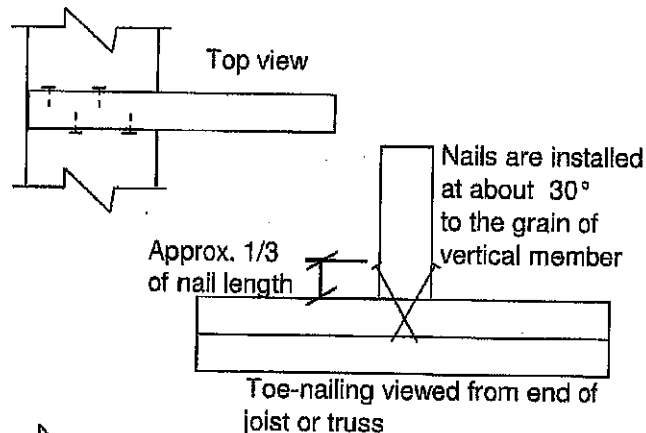
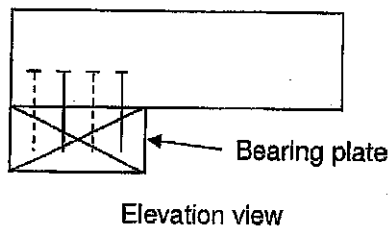
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

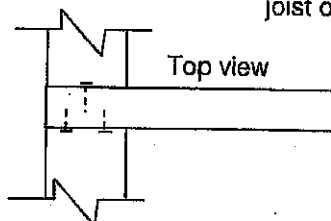
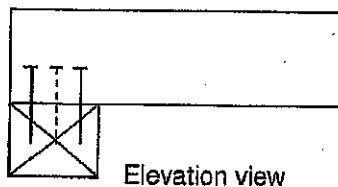
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to wind or earthquake load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



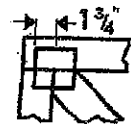
MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019

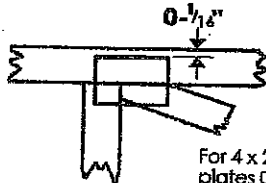


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

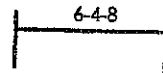


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

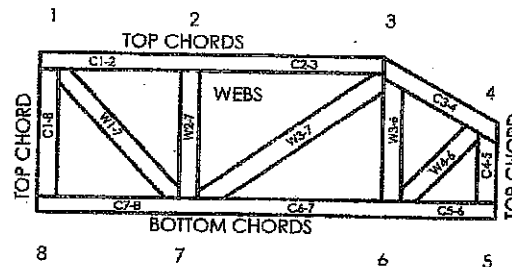
Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
- DSB-89: Design Standard for Bracing.
- BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

© 2007 MiTek® All Rights Reserved

MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

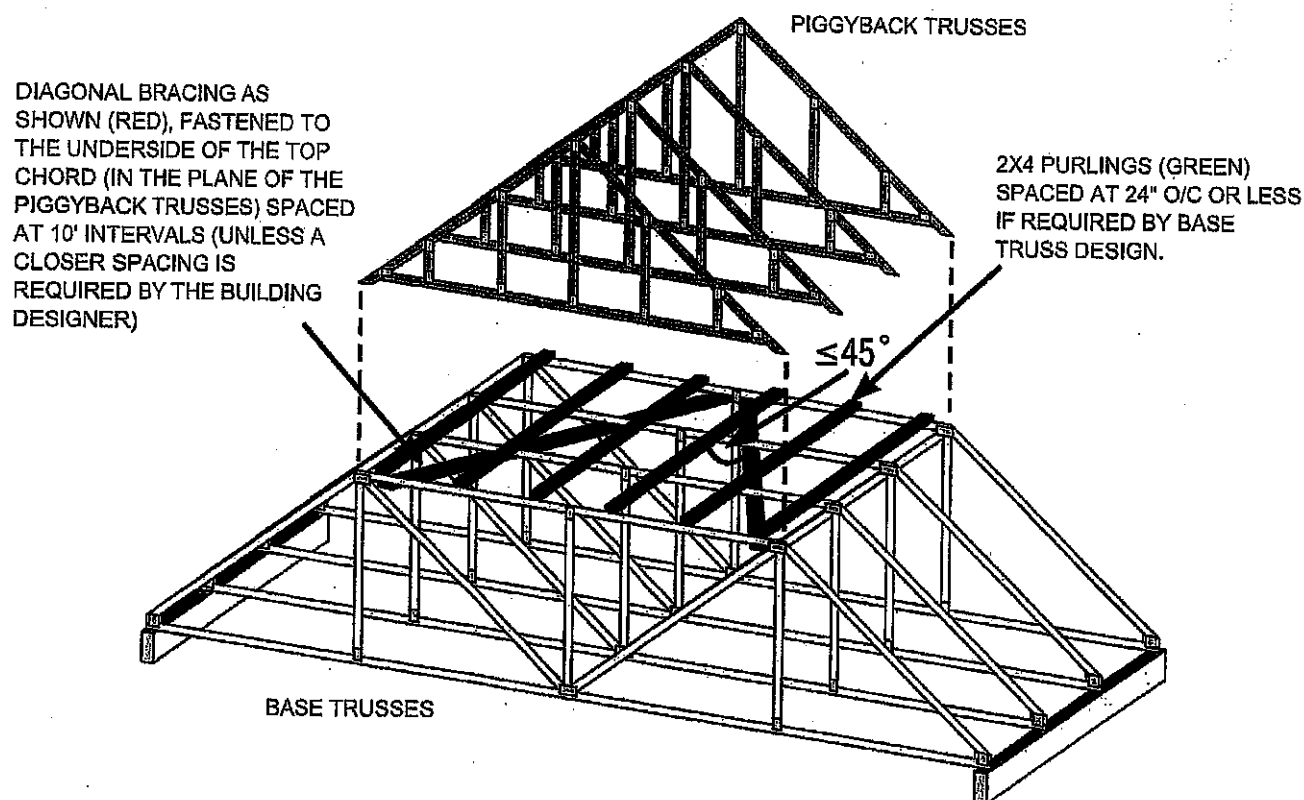
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane of joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

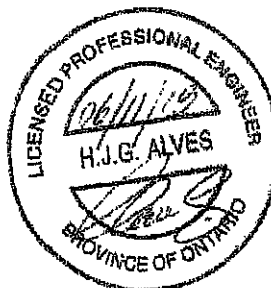


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet M117473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800018

Feb 09, 2018