GREENPARK-TRINAR HALL-BRENTWOOD 3-EL.1&2

Engineering Note Page (ENP-2)

REVISION 2018-10-17

<u>Please read all notes prior to installation of the component</u>

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is only limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with transfer blocks. Structural elements such as walls, posts, connectors, and transfer blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of floor joists is to be carried out in accordance with the current edition of the manufacturer's literature available at http://www.kottgroup.com.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details". ₹ East Gwillimbury arings.
- 4. Pass-thru transfer block framing is required at a 🛴

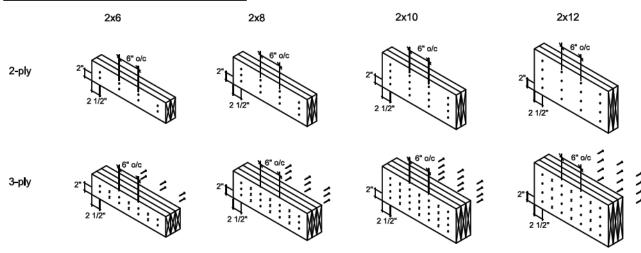
HANDLING AND IN
These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amepided. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

| Discipline | Reviewer | BCIN | Date |
|---------------|------------|-------|------------|
| Building Code | H. Authier | 43236 | 2021-02-05 |
| Sewage System | | | |
| Zoning | | | |
| | | | |



GREENPARK-TRINAR HALL-BRENTWOOD 3-EL.1&2

Conventional Connections



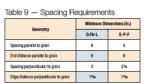
Conventional connection notes:

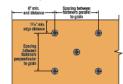
- -Nails to be 3" long wire nails.
- -Nalls to be located 2" mln. from the top and bottom of the member. Start all nalls 2 1/2" mln. from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.

3 1/4" mlr

- "X" represents nall driven from the opposite side.

SIMPSON SDW SPACING REQUIREMENT





Spacing Requirements

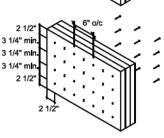
LVL Connections

9 1/2" LVL

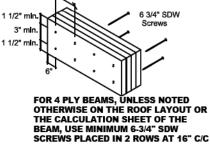
HEAD OF ALL SPECIFIED NAILS AND SCREWS MUST BE ON THE LOADED SIDE

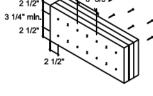
11 7/8" - 14" LVL 16"-18" LVL

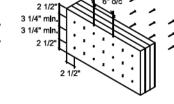
2 1/2" 3 1/4" mln. 3 1/4" mln. 3 1/4" min. 2 1/2"



4-ply LVL (Top load only)







LVL connection notes:

- -LVL ply width is 1-3/4"
- -Nalls to be 3 1/2" common wire nalls.
- -Nails to be located 2 1/2" min. from the top and bottom of the member.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nall or screw driven from the opposite side.



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Multiple Member Connections

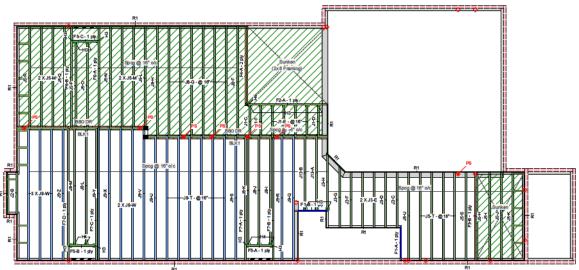
All connections are for uniformly distributed loads.

For multi-ply connections of I-joists, refer to Manufacturer's Installation Guide



KOTT Inc. 3228 Moodie Drive Ottawa, ON K2H 7V1 613-838-2775 NE1220-144

Ground Floor



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBE CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL

| Ground | | | | | | | | | | JOB INFORMATION | | |
|----------|---|-------------------------|----------|-----------|---------|-------|-------------|----------------|------------------|-------------------------------------|---|--|
| LVL/LS | | | | | _ | - | Dr | | | Builder | | |
| F3 | Descri | ption | Width | Depth | | Qty | Plies | Pcs | Length | GREENPARK | | |
| F3 | Forex 2.0E-3 | 000Fb LVL | 1.75 | 9.5 | ' | | | 1 | 12-0-0 | Project | | |
| F4 | Forex | 000Fb LVL | 1.75 | 9.5 | , | 1 | 2 | 2 | 10-0-0 | Shipping | | |
| F2 | Forex | DOOFD LVL | 1.75 | 9.5 | + | | | 1 | 10-0-0 | Sales Rep | | |
| 12 | | 000Fb LVL | 1.75 | 0.0 | 1 | | | | 1000 | RM | | |
| F1 | Forex | | 1.75 | 9.5 | , | | | 2 | 4-0-0 | Designer | | |
| | | 000Fb LVL | | | | | | | | RCO | | |
| | (Flush) | -41 | 145-44 | D | | 04. | Direc | D | 1 | Plotted | | |
| Label | Descri | | Width | Depth | | Qty | Plies | Pcs | Length 16-0-0 | December 17, 2020 | | |
| F7 F6 | AJS 14 | | 2.5 | 9.5 | | | | 2 | 12-0-0 | Layout Name | | |
| F5 | AJS 14 | | 2.5 | 9.5 | | | | 3 | 4-0-0 | BRENTWOOD 3 (ELE) | / 12) | |
| J8 | AJS 14 | | 2.5 | 9.5 | | | | 13 | 14-0-0 | Job Path | v. 1,2) | |
| J5 | AJS 14 | | 2.5 | 9.5 | | | | 20 | 12-0-0 | | | |
| J13 | AJS 14 | | 2.5 | 9.5 | | | | 2 | 10-0-0 | O GREENPARK(TRINAR HALL\BRENTWOOD 3 | | |
| J3 | AJS 14 | 0 | 2.5 | 9.5 | | | | 7 | 8-0-0 | | | |
| J2 | AJS 14 | 0 | 2.5 | 9.5 | , | | | 1 | 6-0-0 | | ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,, | |
| J1 | AJS 14 | 0 | 2.5 | 9.5 | , | | | 7 | 4-0-0 | DESIGN CRITERIA | | |
| J9 | AJS 20 |) | 2.5 | 9.5 | 5 | | | 19 | 16-0-0 | Ground Floor | | |
| Blockin | ng | | | | | | | | | Design Method | LSD (Canada) | |
| Label | Descri | | Width | Depth | | Qty | Plies | Pcs | Length | Building Code | NBCC 2015 / OBC 2012 | |
| | AJS 14 | 0 | 2.5 | 9.5 | L | LinFt | | Varies | 48-0-0 | Floor | | |
| Rim Bo | | | | | | | | | | Loads | | |
| Label | Descri | | Width | Depth | | Qty | Plies | Pcs | Length | Live | 40 | |
| R1 | | d Rimboard 125 X 9.5 | 1.125 | 9.5 | i | | | 14 | 12-0-0 | Dead | 15 | |
| Hanger | | 125 A 9.5 | | 1 | | | | | | Deflection Joist | | |
| nangei | | | | | | Da | am/Girde | | ported | LL Span L/ | 480 | |
| | | | | | | De | arni/Gilde | | ember | TL Span L/ | 360 | |
| Label | Pcs | Descriptio | n 5 | kew S | lope | fs | steners | | teners | LL Cant 2L/ | 480 | |
| H3 | 17 | LF259 | " | MON C | юро | - 10 | 13101013 | 100 | , abiliors | TL Cant 2L/ | 360 | |
| H3 | 1 | LF259 | | | | | | 1 | | Deflection Girder | | |
| H4 | 5 | LF259 | | | | | 10 10d | 1 #8 | c1 1/4WS | LLSpan L/ | 360 | |
| H6 | 1 | HUS1.81/1 | 0 | | | | | | | TL Span L/ | 240 | |
| 1. A | All blocki | ng to be cut | from 12' | ioists | | | | | | LL Cant 2L/ | 480 | |
| 2. 2 | 2 & 4' Le | ngths to be | out from | B' Length | , 6' le | naths | to be cut f | rom 12' L | enath | TL Cant 2L/ | 360 | |
| | | oists to be la | | | , | | | | | Decking | | |
| | | of Steel bear | , | | t by o | thers | | | | Decking | OSE | |
| | | | | | | | ovimata or | dy cone | | Thickness | 3/4" | |
| | Shower and water doset flange locations are approximate only, consult architectural drawing for exact locations | | | | | | Fastener | Nailed & Glued | | | | |
| • | and intestitutal disaming for exact locations | | | | | | Vibration | | | | | |

Vibration **CCMC References** LP - 12412-R Forex - 14056-R

Boise - 12472-R , 12787-R

Canada

L4A 7X4

905-642-4400

JOB INFORMATION

8. Load transfer blocks to be installed under all point loads 9. Refer to Multiple Member Connection Detail for ply to ply nailing or bolting

10. Hangers and Fasteners to be installed as per manufacturer

6. Beams identified as "B" are dropped and supplied by others

7. Install 2x4 blocking @ 24" o/c under parallel non-loadbearing walls

Framing shown on this layout may deviate from architectural drawings. Arch / Eng to review and approve the deviation prior to construction.

Legend

Ground Floor

Point Load Support Load from Above Wall Wall Opening Norbord Rimboard Plus 1.125 X AJS 140 9.5 AJS 20 9.5 Forex 2.0E-3000Fb LVL 1.75 X 9.5

Kott Lumber Company KOTT 14 Anderson Blvd Stouffville, Ontario

Hatch Area represents where additional load has been applied (e.g. 5 psf for ceramic tile)

Version 19.60.173 Powered by iStruct** This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them

East Gwillimbury

Building Standards Branch BCIN #16487

Reviewer BCIN Date H. Authier 43236 2021-02-05

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corrections as noted. No other changes may be

made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly

posted on site at all times. Discipline Building Code

Sewage System Zoning



Single 9-1/2" AJS® 140

F5-A

Dry | 1 span | No cant.

PASSED

December 17, 2020 13:18:12

Page 4 of 30

BC CALC® Member Report

City, Province, Postal Code:

Build 7364

Address:

Customer:

Job name:

C:\Users\rochavillo\Docu...ENTWOOD 3 (ELEV. 1).isl

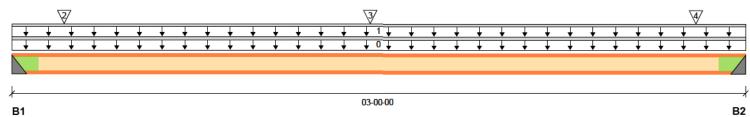
Description: Level - Ground Floor

Specifier:

File name:

Designer: **RCO**

Code reports: CCMC 12787-R Company: **GREENPARK**



Total Horizontal Product Length = 03-00-00

Reaction Summary (Down / Uplift) (lbs)

| Troublion C | | P() (120) | | | |
|-------------|---------|-----------|------|------|--|
| Bearing | Live | Dead | Snow | Wind | |
| B1, 2" | 410 / 0 | 157 / 0 | | | |
| B2, 2" | 412 / 0 | 157 / 0 | | | |

| Lo | ad Summary | | | | | | Live | Dead | Snow | Wind | Tributary |
|-----|-------------|-------------------|------|----------|----------|------|------|------|------|------|-----------|
| Tag | Description | Load Type | Ref. | Start | End | Loc. | 1.00 | 0.65 | 1.00 | 1.15 | |
| 0 | Self-Weight | Unf. Lin. (lb/ft) | L | 00-00-00 | 03-00-00 | Тор | | 2 | | | 00-00-00 |
| 1 | | Unf. Lin. (lb/ft) | L | 00-00-00 | 03-00-00 | Тор | 34 | 13 | | | n\a |
| 2 | J8 | Conc. Pt. (lbs) | L | 00-02-09 | 00-02-09 | Back | 194 | 73 | | | n\a |
| 3 | J8 | Conc. Pt. (lbs) | L | 01-05-09 | 01-05-09 | Back | 320 | 120 | | | n\a |
| 4 | J8 | Conc. Pt. (lbs) | L | 02-09-09 | 02-09-09 | Back | 204 | 76 | | | n\a |

| Controls Summary | Factored Demand | Factored Resistance | Demand/ Resistance | Case | Location |
|-------------------------|-----------------|------------------------|-----------------------|------|----------|
| Pos. Moment | 548 ft-lbs | 4095 ft-lbs | 13.4% | 1 | 01-05-09 |
| End Reaction | 814 lbs | 2050 lbs | 39.7% | 1 | 03-00-00 |
| End Shear | 802 lbs | 1830 lbs | 43.8% | 1 | 02-10-00 |
| Total Load Deflection | L/999 (0.009") | n\a | n\a | 4 | 01-05-09 |
| Live Load Deflection | L/999 (0.007") | n\a | n\a | 5 | 01-05-09 |
| Max Defl. | 0.009" | n\a | n\a | 4 | 01-05-09 |
| Span / Depth | 3.5 | | | | |

| Bear | ing Supports | Dim. (LxW) | Demand | Demand/ Resistance Support | Demand/ Resistance Member | Material |
|------|--------------|-------------|---------|----------------------------------|---------------------------------|----------|
| B1 | Hanger | 2" x 2-1/2" | 811 lbs | 41.7% | 39.6% | LF259 |
| B2 | Hanger | 2" x 2-1/2" | 814 lbs | 41.9% | 39.7% | LF259 |



Cautions

Web stiffeners required at bearing B1.

Hanger LF259 requires (10) 10dx1.5 face nails, (2) 10dx1.5 joist nails. Web stiffeners required at bearing B2.

Header for the hanger LF259 is a Single 2-1/2" x 9-1/2" I-joist Header for the hanger LF259 is a Single 2-1/2" x 9-1/2" I-joist



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| H. Authier | 43236 | 2021-02-05 |
|------------|-------|------------|
| | | |
| | | |
| | | |
| | | |

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS. READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Single 9-1/2" AJS® 140

F5-A

Dry | 1 span | No cant.

Page 5 of 30 **PASSED**

December 17, 2020 13:18:12

BC CALC® Member Report

City, Province, Postal Code:

Build 7364

Address:

Customer:

Code reports:

Job name: C:\Users\rochavillo\Docu...ENTWOOD 3 (ELEV. 1).isl File name:

> Description: Level - Ground Floor

Specifier:

Designer: **RCO**

Company: **GREENPARK**

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced. Hanger Manufacturer: Simpson Strong-Tie, Inc.

Resistance Factor phi has been applied to all presented results per CSA O86.

CCMC 12787-R

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9



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| Discipline | Reviewer | BCIN | Date |
|---------------|------------|-------|------------|
| Building Code | H. Authier | 43236 | 2021-02-05 |
| Sewage System | | | |
| Zoning | | | |
| | | | |

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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

BLOCK IS REQUIRED AT ALL

POINT LOADS OVER BEARINGS.



Single 9-1/2" AJS® 140

F5-B

Dry | 1 span | No cant.

December 17, 2020 13:18:12

Build 7364

Address:

Customer:

Job name: C:\Users\rochavillo\Docu...ENTWOOD 3 (ELEV. 1).isl File name:

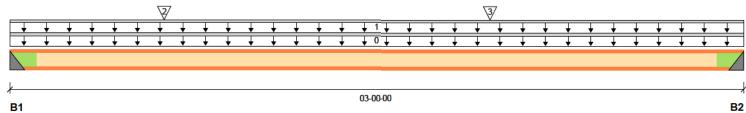
> Description: Level - Ground Floor

City, Province, Postal Code:

Specifier:

Designer: **RCO**

Code reports: CCMC 12787-R Company: **GREENPARK**



Total Horizontal Product Length = 03-00-00

Reaction Summary (Down / Uplift) (lbs)

| | - a \ \ | , () | | | |
|---------|---------|---------|------|------|--|
| Bearing | Live | Dead | Snow | Wind | |
| B1, 2" | 387 / 0 | 149 / 0 | | | |
| B2, 2" | 325 / 0 | 125 / 0 | | | |

| Lo | ad Summary | | | | | | Live | Dead | Snow | Wind | Tributary |
|----|-------------|-------------------|------|----------|----------|------|------|------|------|------|-----------|
| | Description | Load Type | Ref. | Start | End | Loc. | 1.00 | 0.65 | 1.00 | 1.15 | |
| 0 | Self-Weight | Unf. Lin. (lb/ft) | L | 00-00-00 | 03-00-00 | Тор | | 2 | | | 00-00-00 |
| 1 | | Unf. Lin. (lb/ft) | L | 00-00-00 | 03-00-00 | Тор | 34 | 13 | | | n\a |
| 2 | J8 | Conc. Pt. (lbs) | L | 00-07-09 | 00-07-09 | Back | 277 | 104 | | | n\a |
| 3 | J8 | Conc. Pt. (lbs) | L | 01-11-09 | 01-11-09 | Back | 332 | 124 | | | n\a |

| Camtuala Sumanama | | Factored | Demand/ | | |
|-----------------------|-----------------|-------------|------------|------|----------|
| Controls Summary | Factored Demand | Resistance | Resistance | Case | Location |
| Pos. Moment | 562 ft-lbs | 4095 ft-lbs | 13.7% | 1 | 01-11-09 |
| End Reaction | 767 lbs | 2050 lbs | 37.4% | 1 | 00-00-00 |
| End Shear | 755 lbs | 1830 lbs | 41.3% | 1 | 00-02-00 |
| Total Load Deflection | L/999 (0.01") | n\a | n\a | 4 | 01-10-01 |
| Live Load Deflection | L/999 (0.007") | n\a | n\a | 5 | 01-10-01 |
| Max Defl. | 0.01" | n\a | n\a | 4 | 01-10-01 |
| Span / Depth | 3.5 | | | | |

| Bearing | g Supports | Dim. (LxW) | Demand | Demand/ Resistance Support | Demand/ Resistance Member | Material |
|---------|------------|-------------|---------|----------------------------------|---------------------------------|----------|
| B1 | Hanger | 2" x 2-1/2" | 767 lbs | 39.4% | 37.4% | LF259 |
| B2 | Hanger | 2" x 2-1/2" | 644 lbs | 33.1% | 31.4% | LF259 |



Web stiffeners required at bearing B1.

Hanger LF259 requires (10) 10dx1.5 face nails, (2) 10dx1.5 joist nails. Web stiffeners required at bearing B2.

Header for the hanger LF259 is a Single 2-1/2" x 9-1/2" I-joist Header for the hanger LF259 is a Single 2-1/2" x 9-1/2" I-joist

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| Discipline | Reviewer | BCIN | Date |
|---------------|------------|-------|------------|
| Building Code | H. Authier | 43236 | 2021-02-05 |
| Sewage System | | | |
| Zoning | | | |
| | | | |

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PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Cautions

PROFESSIONAL ENO



Single 9-1/2" AJS® 140

F5-B

December 17, 2020 13:18:12

Dry | 1 span | No cant.

Job name: C:\Users\rochavillo\Docu...ENTWOOD 3 (ELEV. 1).isl File name:

Address: Description: Level - Ground Floor

City, Province, Postal Code: Specifier:

Customer: Designer: **RCO** Code reports: CCMC 12787-R Company: **GREENPARK**

Notes

Build 7364

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced. Hanger Manufacturer: Simpson Strong-Tie, Inc.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9



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| Discipline | Reviewer | BCIN | Date |
|---------------|------------|-------|------------|
| Building Code | H. Authier | 43236 | 2021-02-05 |
| Sewage System | | | |
| Zoning | | | |

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Page 7 of 30

PASSED



Customer:

Single 9-1/2" AJS® 140

F5-C

Designer:

December 17, 2020 13:18:12 Dry | 1 span | No cant.

RCO

Page 8 of 30

PROFESSIONALERO

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PASSED

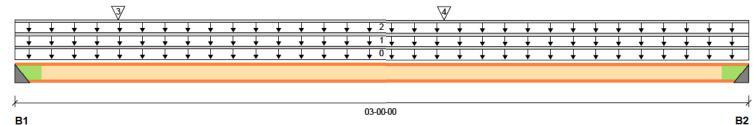
BC CALC® Member Report

Build 7364 Job name: File name:

Address: Description: Level - Ground Floor

City, Province, Postal Code: Specifier:

Code reports: CCMC 12787-R Company: **GREENPARK**



Total Horizontal Product Length = 03-00-00

Reaction Summary (Down / Unlift) (lbs)

| redetion of | | | | | |
|-------------|---------|---------|------|------|--|
| Bearing | Live | Dead | Snow | Wind | |
| B1, 2" | 324 / 0 | 160 / 0 | | | |
| B2, 2" | 230 / 0 | 115 / 0 | | | |

| Lo | ad Summary | | | | | | Live | Dead | Snow | Wind | Tributary |
|----|-------------|-------------------|------|----------|----------|-------|------|------|------|------|-----------|
| | Description | Load Type | Ref. | Start | End | Loc. | 1.00 | 0.65 | 1.00 | 1.15 | |
| 0 | Self-Weight | Unf. Lin. (lb/ft) | L | 00-00-00 | 03-00-00 | Тор | | 2 | | | 00-00-00 |
| 1 | _ | Unf. Lin. (lb/ft) | L | 00-00-00 | 03-00-00 | Тор | | 4 | | | n\a |
| 2 | | Unf. Lin. (lb/ft) | L | 00-00-00 | 03-00-00 | Тор | 34 | 13 | | | n\a |
| 3 | J5 | Conc. Pt. (lbs) | L | 00-05-01 | 00-05-01 | Front | 184 | 89 | | | n∖a |
| 4 | J5 | Conc. Pt. (lbs) | L | 01-09-01 | 01-09-01 | Front | 266 | 129 | | | n\a |

| Controls Summary | Factored Demand | Factored Resistance | Demand/ Resistance | Case | Location |
|-----------------------|-----------------|------------------------|-----------------------|------|----------|
| Pos. Moment | 499 ft-lbs | 4095 ft-lbs | 12.2% | 1 | 01-09-01 |
| End Reaction | 686 lbs | 2050 lbs | 33.4% | 1 | 00-00-00 |
| End Shear | 673 lbs | 1830 lbs | 36.8% | 1 | 00-02-00 |
| Total Load Deflection | L/999 (0.009") | n\a | n\a | 4 | 01-09-01 |
| Live Load Deflection | L/999 (0.006") | n\a | n\a | 5 | 01-09-01 |
| Max Defl. | 0.009" | n\a | n\a | 4 | 01-09-01 |
| Span / Depth | 3.5 | | | | |

| Bearin | ıg Supports | Dim. (LxW) | Demand | Demand/ Resistance Support | Demand/ Resistance Member | Material |
|--------|-------------|-------------|---------|----------------------------------|---------------------------------|----------|
| B1 | Hanger | 2" x 2-1/2" | 686 lbs | 35.3% | 33.4% | LF259 |
| B2 | Hanger | 2" x 2-1/2" | 489 lbs | 25.1% | 23.8% | LF259 |

Cautions

Web stiffeners required at bearing B1.

Hanger LF259 requires (10) 10dx1.5 face nails, (2) 10dx1.5 joist nails. Web stiffeners required at bearing B2.

Header for the hanger LF259 is a Single 2-1/2" x 9-1/2" I-joist Header for the hanger LF259 is a Single 2-1/2" x 9-1/2" l-joist



These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amehded. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

| I. Authier | 43236 | 2021-02-05 |
|------------|-------|------------|
| | | |
| | | |
| | | |
| | | |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Single 9-1/2" AJS® 140

F5-C

Dry | 1 span | No cant.

Page 9 of 30 **PASSED**

December 17, 2020 13:18:12

BC CALC® Member Report

Build 7364

Job name: C:\Users\rochavillo\Docu...ENTWOOD 3 (ELEV. 1).isl File name:

Address: Description: Level - Ground Floor

City, Province, Postal Code: Specifier:

Customer: Designer: **RCO**

Code reports: CCMC 12787-R Company: **GREENPARK**

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced. Hanger Manufacturer: Simpson Strong-Tie, Inc.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9



These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amehded. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

| Discipline | Reviewer | BCIN | Date |
|---------------|------------|-------|------------|
| Building Code | H. Authier | 43236 | 2021-02-05 |
| Sewage System | | | |
| Zoning | | | |
| | | | |

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Disclosure

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Single 9-1/2" AJS® 140

F6-A

Dry | 1 span | No cant.

Page 10 of 30 **PASSED**

December 17, 2020 13:18:12

BC CALC® Member Report

Build 7364

Job name:

Address: City, Province, Postal Code:

Customer:

Code reports:

CCMC 12787-R

C:\Users\rochavillo\Docu...ENTWOOD 3 (ELEV. 1).isl File name:

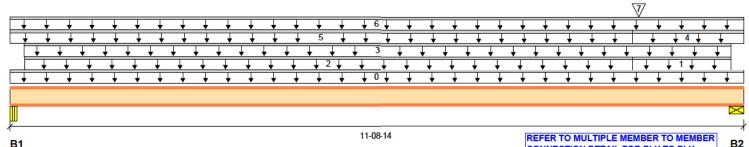
Description: Level - Ground Floor

Wind

Specifier:

RCO Designer:

Company: **GREENPARK**



Total Horizontal Product Length = 11-08-14

Snow

CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARING

Reaction Summary (Down / Uplift) (lbs)

B1, 2-5/8" 349 / 0 182 / 0 B2, 2-3/8" 573 / 0 294 / 0

| Loa | ad Summary | | | | | | Live | Dead | Snow | Wind | Tributary |
|-----|-------------|-------------------|------|----------|----------|------|------|------|------|-------|-----------|
| Tag | Description | Load Type | Ref. | Start | End | Loc. | 1.00 | 0.65 | 1.00 | 1.15 | |
| 0 | Self-Weight | Unf. Lin. (lb/ft) | L | 00-00-00 | 11-08-14 | Тор | | 2 | | | 00-00-00 |
| 1 | | Unf. Lin. (lb/ft) | L | 09-11-08 | 11-06-08 | Тор | | 8 | | | n\a |
| 2 | | Unf. Lin. (lb/ft) | L | 00-02-13 | 09-11-08 | Тор | | 3 | | | n\a |
| 3 | | Unf. Lin. (lb/ft) | L | 00-02-13 | 11-06-08 | Тор | | 3 | | | n\a |
| 4 | | Unf. Lin. (lb/ft) | L | 09-11-08 | 11-08-14 | Тор | 64 | 24 | | | n\a |
| 5 | | Unf. Lin. (lb/ft) | L | 00-00-00 | 09-11-08 | Тор | 27 | 10 | | | n\a |
| 6 | | Unf. Lin. (lb/ft) | L | 00-00-00 | 11-08-14 | Тор | 26 | 10 | | | n\a |
| 7 | F5 | Conc. Pt. (lbs) | L | 10-00-12 | 10-00-12 | Back | 230 | 115 | | ROFES | SIONA |

| Controls Summary | Factored Demand | Factored Resistance | Demand/ Resistance | Case | Location |
|-----------------------|-----------------|------------------------|-----------------------|------|----------|
| Pos. Moment | 2343 ft-lbs | 4095 ft-lbs | 57.2% | 1 | 06-07-04 |
| End Reaction | 1228 lbs | 1653 lbs | 74.3% | 1 | 11-08-14 |
| End Shear | 1192 lbs | 1830 lbs | 65.1% | 1 | 11-06-08 |
| Total Load Deflection | L/551 (0.249") | n\a | 43.5% | 4 | 06-00-08 |
| Live Load Deflection | L/840 (0.164") | n\a | 42.9% | 5 | 06-00-08 |
| Max Defl. | 0.249" | n\a | 24.9% | 4 | 06-00-08 |
| Span / Depth | 14.5 | | | | |

| D: | C | | | Demand/ Resistance | Demand/ Resistance | |
|-------|-------------|-----------------|----------|-----------------------|----------------------------|--------------------|
| Beari | ng Supports | Dim. (LxW) | Demand | Support | Memb East Gw | illimbury |
| B1 | Beam | 2-5/8" x 2-1/2" | 751 lbs | 18.6% | 44.5% | Branch BCIN #16487 |
| B2 | Wall/Plate | 2-3/8" x 2-1/2" | 1228 lbs | 33.6% | 74.3% Our trans, Our plant | |

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria. Design meets User specified (1") Maximum Total load deflection criteria. Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86 BC CALC® analysis is based on Canadian Limit States Design, as per NBC0

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

These plans have been reviewed for use with the corrections as noted. No other changes may be

made without written approval of the Building

| П | Discipinic | Reviewei | DCII | Date |
|----|---------------|------------|-------|------------|
| .[| Building Code | H. Authier | 43236 | 2021-02-05 |
| | Sewage System | | | |
| • | Zoning | | | |
| ١ | | | | |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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Customer:

Single 9-1/2" AJS® 140

F6-B

Designer:

RCO

Wind

Dry | 1 span | No cant.

Page 11 of 30 **PASSED**

December 17, 2020 13:18:12

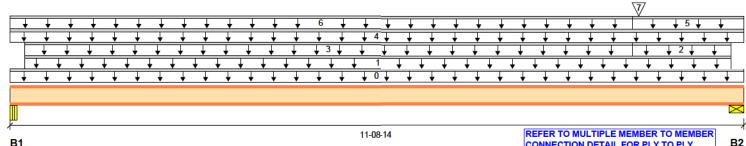
Build 7364

Job name:

C:\Users\rochavillo\Docu...ENTWOOD 3 (ELEV. 1).isl File name: Address: Description: Level - Ground Floor

City, Province, Postal Code: Specifier:

Code reports: CCMC 12787-R Company: **GREENPARK**



Total Horizontal Product Length = 11-08-14

Snow

CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH

BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

Reaction Summary (Down / Uplift) (lbs)

Live B1, 2-5/8" 207 / 0 113 / 0 B2, 2-3/8" 526 / 0 271 / 0

| Loa | ad Summary | | | | | | Live | Dead | Snow | Wind | Tributary |
|-----|-------------|-------------------|------|----------|----------|-------|------|------|------|------|-----------|
| Tag | Description | Load Type | Ref. | Start | End | Loc. | 1.00 | 0.65 | 1.00 | 1.15 | |
| 0 | Self-Weight | Unf. Lin. (lb/ft) | L | 00-00-00 | 11-08-14 | Тор | | 2 | | | 00-00-00 |
| 1 | | Unf. Lin. (lb/ft) | L | 00-02-14 | 11-06-08 | Тор | | 2 | | | n\a |
| 2 | | Unf. Lin. (lb/ft) | L | 09-11-08 | 11-06-08 | Тор | | 8 | | | n\a |
| 3 | | Unf. Lin. (lb/ft) | L | 00-02-14 | 09-11-08 | Тор | | 1 | | | n\a |
| 4 | | Unf. Lin. (lb/ft) | L | 00-00-00 | 11-08-14 | Top | 16 | 6 | | | n\a |
| 5 | | Unf. Lin. (lb/ft) | L | 09-11-08 | 11-08-14 | Тор | 64 | 24 | | | n\a |
| 6 | | Unf. Lin. (lb/ft) | L | 00-00-00 | 09-11-08 | Тор | 11 | 4 | | | n\a |
| 7 | F5 | Conc. Pt. (lbs) | L | 10-00-12 | 10-00-12 | Front | 324 | 160 | | OFES | SIONA |

| Controls Summary | Factored Demand | Factored Resistance | Demand/ Resistance | Case | Location |
|-----------------------|-----------------|------------------------|-----------------------|------|----------|
| Pos. Moment | 1668 ft-lbs | 4095 ft-lbs | 40.7% | 1 | 07-08-10 |
| End Reaction | 1127 lbs | 1653 lbs | 68.2% | 1 | 11-08-14 |
| End Shear | 1095 lbs | 1830 lbs | 59.8% | 1 | 11-06-08 |
| Total Load Deflection | L/785 (0.175") | n\a | 30.6% | 4 | 06-03-14 |
| Live Load Deflection | L/999 (0.114") | n\a | n\a | 5 | 06-03-14 |
| Max Defl. | 0.175" | n\a | 17.5% | 4 | 06-03-14 |
| Span / Depth | 14.5 | | | | |

| Poor | ina Cunnarta | . | | Demand/ Resistance | Demand/ Resistance |
|------|---------------|-----------------|----------|-----------------------|---------------------------------------|
| bear | ring Supports | Dim. (LxW) | Demand | Support | Memb East Gwillimbury |
| B1 | Beam | 2-5/8" x 2-1/2" | 452 lbs | 11.2% | Building Standards Branch BCIN #16487 |
| R2 | Wall/Plate | 2-3/8" x 2-1/2" | 1127 lbs | 30.8% | 68 2% Torong Our June |

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria. Design meets User specified (1") Maximum Total load deflection criteria. Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86 BC CALC® analysis is based on Canadian Limit States Design, as per NBC0

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

| Discipline | Reviewer | BCIN | Date |
|---------------|------------|-------|------------|
| Building Code | H. Authier | 43236 | 2021-02-05 |
| Sewage System | | | |
| Zoning | | | |
| | | | |

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City, Province, Postal Code:

Single 9-1/2" AJS® 140

F7-A

Dry | 1 span | No cant.

Page 12 of 30 **PASSED**

December 17, 2020 13:18:12

C:\Users\rochavillo\Docu...ENTWOOD 3 (ELEV. 1).isl

Build 7364 Job name:

Address:

Customer:

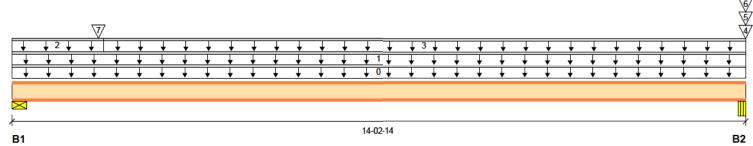
Description: Level - Ground Floor

Specifier: Designer:

File name:

RCO

Code reports: CCMC 12787-R Company: **GREENPARK**



Total Horizontal Product Length = 14-02-14

| Reaction Sui | | | | | |
|--------------|---------|---------|------|------|--|
| Bearing | Live | Dead | Snow | Wind | |
| B1, 2-3/8" | 664 / 0 | 267 / 0 | | | |
| B2, 2-5/8" | 442 / 0 | 209 / 0 | | | |

| Lo | Load Summary | | | | | | Live | Dead | Snow | Wind | Tributary |
|-----|------------------|-------------------|------|----------|----------|-------|------|------|------|------|-----------|
| Tag | Description | Load Type | Ref. | Start | End | Loc. | 1.00 | 0.65 | 1.00 | 1.15 | |
| 0 | Self-Weight | Unf. Lin. (lb/ft) | L | 00-00-00 | 14-02-14 | Тор | | 2 | | | 00-00-00 |
| 1 | | Unf. Lin. (lb/ft) | L | 00-00-00 | 14-02-14 | Тор | 22 | 8 | | | n\a |
| 2 | | Unf. Lin. (lb/ft) | L | 00-00-00 | 01-09-06 | Тор | 64 | 24 | | | n\a |
| 3 | | Unf. Lin. (lb/ft) | L | 01-09-06 | 14-02-14 | Тор | 6 | 2 | | | n\a |
| 4 | J9 | Conc. Pt. (lbs) | L | 14-02-14 | 14-02-14 | Тор | 100 | 37 | | | n\a |
| 5 | J8 | Conc. Pt. (lbs) | L | 14-02-14 | 14-02-14 | Тор | 89 | 33 | | | n\a |
| 6 | Wall Self Weight | Conc. Pt. (lbs) | L | 14-02-14 | 14-02-14 | Тор | | 28 | | | n\a |
| 7 | F5 | Conc. Pt. (lbs) | L | 01-08-02 | 01-08-02 | Front | 410 | 157 | | OFES | SIONA |

| Controls Summary | Factored Demand | Factored Resistance | Demand/ Resistance | Case | Location |
|-----------------------|-----------------|------------------------|-----------------------|------|----------|
| Pos. Moment | 2213 ft-lbs | 4095 ft-lbs | 54.0% | 1 | 05-04-05 |
| End Reaction | 1329 lbs | 1653 lbs | 80.4% | 1 | 00-00-00 |
| End Shear | 1295 lbs | 1830 lbs | 70.8% | 1 | 00-02-06 |
| Total Load Deflection | L/504 (0.332") | n\a | 47.6% | 4 | 06-07-05 |
| Live Load Deflection | L/718 (0.233") | n\a | 50.1% | 5 | 06-07-05 |
| Max Defl. | 0.332" | n\a | 33.2% | 4 | 06-07-05 |
| Span / Depth | 17.6 | | | | |

| _ | | | | Demand/ Resistance | Demand/ Resistance |
|------|--------------|-----------------|----------|-----------------------|---|
| Bear | ing Supports | Dim. (LxW) | Demand | Support | Memb East Gwillimbury |
| B1 | Wall/Plate | 2-3/8" x 2-1/2" | 1329 lbs | 36.4% | 8U.49 Building Standards Branch BCIN #16487 |
| B2 | Beam | 2-5/8" x 2-1/2" | 924 lbs | 22 9% | 54.5% Tour cours, Our factor |

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria. Design meets User specified (1") Maximum Total load deflection criteria. Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86 BC CALC® analysis is based on Canadian Limit States Design, as per NBC0

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9
REFER TO MULTIPLE MEMBER TO MEM
CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

ASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

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Discipline Building Code

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43236

H. Authier



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Code reports:

Single 9-1/2" AJS® 140

F7-B

Dry | 1 span | No cant.

GREENPARK

Build 7364 Job name: C:\Users\rochavillo\Docu...ENTWOOD 3 (ELEV. 1).isl

File name: Address: Description: Level - Ground Floor

City, Province, Postal Code: Specifier:

CCMC 12787-R

Customer: Designer: **RCO**

14-02-14 **B2**

Company:

Total Horizontal Product Length = 14-02-14

Reaction Summary (Down / Unlift) (lbs)

| redetion duminary (Bown / Opine) (186) | | | | | | | |
|--|---------|---------|------|------|--|--|--|
| Bearing | Live | Dead | Snow | Wind | | | |
| B1, 2-3/8" | 654 / 0 | 263 / 0 | | | | | |
| B2, 2-5/8" | 241 / 0 | 106 / 0 | | | | | |

| Lo | ad Summary | | | | | | Live | Dead | Snow | Wind | Tributary |
|----|-------------|-------------------|------|----------|----------|------|------|------|------|------|-----------|
| | Description | Load Type | Ref. | Start | End | Loc. | 1.00 | 0.65 | 1.00 | 1.15 | |
| 0 | Self-Weight | Unf. Lin. (lb/ft) | L | 00-00-00 | 14-02-14 | Тор | | 2 | | | 00-00-00 |
| 1 | | Unf. Lin. (lb/ft) | L | 00-00-00 | 01-09-06 | Тор | 64 | 24 | | | n\a |
| 2 | | Unf. Lin. (lb/ft) | L | 01-09-06 | 14-02-14 | Тор | 6 | 2 | | | n\a |
| 3 | | Unf. Lin. (lb/ft) | L | 00-00-00 | 14-02-14 | Тор | 21 | 8 | | | n\a |
| 4 | F5 | Conc. Pt. (lbs) | L | 01-08-02 | 01-08-02 | Back | 412 | 157 | | | n\a |

| Controls Summary | Factored Demand | Factored Resistance | Demand/ Resistance | Case | Location |
|-------------------------|-----------------|------------------------|-----------------------|------|----------|
| Pos. Moment | 2141 ft-lbs | 4095 ft-lbs | 52.3% | 1 | 05-04-05 |
| End Reaction | 1310 lbs | 1653 lbs | 79.2% | 1 | 00-00-00 |
| End Shear | 1276 lbs | 1830 lbs | 69.7% | 1 | 00-02-06 |
| Total Load Deflection | L/521 (0.321") | n\a | 46.0% | 4 | 06-07-05 |
| Live Load Deflection | L/744 (0.225") | n\a | 48.4% | 5 | 06-07-05 |
| Max Defl. | 0.321" | n\a | 32.1% | 4 | 06-07-05 |
| Span / Depth | 17.6 | | | | |

| Bearing Supports | | յ Supports | Dim. (LxW) | LxW) Demand | | Demand/ Resistance Member | Material | |
|------------------|----|------------|-----------------|-------------|-------|---------------------------------|-----------------|--|
| | B1 | Wall/Plate | 2-3/8" x 2-1/2" | 1310 lbs | 35.9% | 79.2% | Spruce-Pine-Fir | |
| | B2 | Beam | 2-5/8" x 2-1/2" | 495 lbs | 12.3% | 29.2% | Spruce-Pine-Fir | |

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria. Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86 BC CALC® analysis is based on Canadian Limit States Design, as per NBC(Ontario Building Code, as amended. These Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



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| Discipline | Keviewei | BCIN | Date |
|---------------|------------|-------|------------|
| Building Code | H. Authier | 43236 | 2021-02-05 |
| Sewage System | | | |
| Zoning | | | |
| | | | |

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Page 13 of 30

December 17, 2020 13:18:12

PASSED



Single 9-1/2" AJS® 140

F7-C

Dry | 1 span | No cant.

Page 14 of 30 **PASSED**

December 17, 2020 13:18:12

BC CALC® Member Report

City, Province, Postal Code:

Build 7364

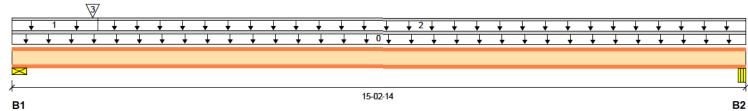
Job name: C:\Users\rochavillo\Docu...ENTWOOD 3 (ELEV. 1).isl File name:

Address: Description: Level - Ground Floor

Specifier:

Customer: Designer: **RCO**

Code reports: CCMC 12787-R Company: **GREENPARK**



Total Horizontal Product Length = 15-02-14

Reaction Summary (Down / Unlift) (lbs)

| ixeaction our | | | | | |
|---------------|---------|---------|------|------|--|
| Bearing | Live | Dead | Snow | Wind | |
| B1, 2-3/8" | 535 / 0 | 220 / 0 | | | |
| B2, 2-5/8" | 211 / 0 | 96 / 0 | | | |

| Lo | ad Summary | | | | | | Live | Dead | Snow | Wind | Tributary |
|-----|-------------|-------------------|------|----------|----------|------|------|------|------|------|-----------|
| Tag | Description | Load Type | Ref. | Start | End | Loc. | 1.00 | 0.65 | 1.00 | 1.15 | |
| 0 | Self-Weight | Unf. Lin. (lb/ft) | L | 00-00-00 | 15-02-14 | Тор | | 2 | | | 00-00-00 |
| 1 | | Unf. Lin. (lb/ft) | L | 00-00-00 | 01-09-06 | Тор | 64 | 24 | | | n\a |
| 2 | | Unf. Lin. (lb/ft) | L | 01-09-06 | 15-02-14 | Тор | 23 | 9 | | | n\a |
| 3 | F5 | Conc. Pt. (lbs) | L | 01-08-02 | 01-08-02 | Back | 325 | 125 | | | n\a |

| 0 1 1 0 | | Factored | Demand/ | | |
|-----------------------|-----------------|-------------|------------|------|----------|
| Controls Summary | Factored Demand | Resistance | Resistance | Case | Location |
| Pos. Moment | 1940 ft-lbs | 4095 ft-lbs | 47.4% | 1 | 06-00-12 |
| End Reaction | 1078 lbs | 1653 lbs | 65.2% | 1 | 00-00-00 |
| End Shear | 1052 lbs | 1830 lbs | 57.5% | 1 | 00-02-06 |
| Total Load Deflection | L/539 (0.333") | n\a | 44.5% | 4 | 07-02-12 |
| Live Load Deflection | L/777 (0.231") | n\a | 46.3% | 5 | 07-02-12 |
| Max Defl. | 0.333" | n\a | 33.3% | 4 | 07-02-12 |
| Span / Depth | 18.9 | | | | |

| Bearing | յ Supports | Dim. (LxW) | Demand | Demand/ Resistance Support | Demand/ Resistance Member | Material |
|---------|------------|-----------------|----------|----------------------------------|---------------------------------|-----------------|
| B1 | Wall/Plate | 2-3/8" x 2-1/2" | 1078 lbs | 29.5% | 65.2% | Spruce-Pine-Fir |
| B2 | Beam | 2-5/8" x 2-1/2" | 437 lbs | 10.8% | 25.8% | Spruce-Pine-Fir |

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O8(These plans have been reviewed for use with the BC CALC® analysis is based on Canadian Limit States Design, as per NBC(made without written approval of the Building Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

| Discipline | Reviewer | BCIN | Date |
|---------------|------------|-------|------------|
| Building Code | H. Authier | 43236 | 2021-02-05 |
| Sewage System | | | |
| Zoning | | | |
| | | | |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.



Job name:

Single 9-1/2" AJS® 140

F7-D

File name:

Dry | 1 span | No cant. December 17, 2020 13:18:12

C:\Users\rochavillo\Docu...ENTWOOD 3 (ELEV. 1).isl

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PASSED

BC CALC® Member Report

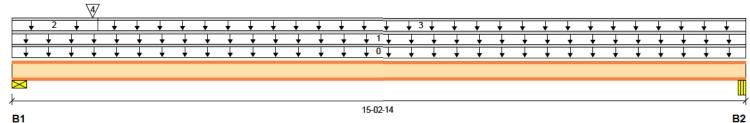
Build 7364

Address: Description: Level - Ground Floor

City, Province, Postal Code: Specifier:

Customer: Designer: **RCO**

Code reports: CCMC 12787-R Company: **GREENPARK**



Total Horizontal Product Length = 15-02-14

Reaction Summary (Down / Unlift) (lbs)

| reduction Cultimary (Bown / Opinit) (188) | | | | | | | | |
|---|-------------|---------|------|------|--|--|--|--|
| Bearing | Live | Dead | Snow | Wind | | | | |
| B1, 2-3 | /8" 634 / 0 | 258 / 0 | | | | | | |
| B2, 2-5 | /8" 248 / 0 | 110 / 0 | | | | | | |

| Lo | ad Summary | | | | | | Live | Dead | Snow | Wind | Tributary |
|----|-------------|-------------------|------|----------|----------|-------|------|------|------|------|-----------|
| | Description | Load Type | Ref. | Start | End | Loc. | 1.00 | 0.65 | 1.00 | 1.15 | |
| 0 | Self-Weight | Unf. Lin. (lb/ft) | L | 00-00-00 | 15-02-14 | Тор | | 2 | | | 00-00-00 |
| 1 | _ | Unf. Lin. (lb/ft) | L | 00-00-00 | 15-02-14 | Тор | 12 | 4 | | | n\a |
| 2 | | Unf. Lin. (lb/ft) | L | 00-00-00 | 01-09-06 | Тор | 64 | 24 | | | n∖a |
| 3 | | Unf. Lin. (lb/ft) | L | 01-09-06 | 15-02-14 | Тор | 15 | 6 | | | n∖a |
| 4 | F5 | Conc. Pt. (lbs) | L | 01-08-02 | 01-08-02 | Front | 387 | 149 | | | n\a |

| Controls Summary | Factored Demand | Factored Resistance | Demand/ Resistance | Case | Location |
|-----------------------|-----------------|------------------------|-----------------------|------|----------|
| Pos. Moment | 2270 ft-lbs | 4095 ft-lbs | 55.4% | 1 | 06-00-12 |
| End Reaction | 1273 lbs | 1653 lbs | 77.0% | 1 | 00-00-00 |
| End Shear | 1243 lbs | 1830 lbs | 67.9% | 1 | 00-02-06 |
| Total Load Deflection | L/461 (0.389") | n\a | 52.1% | 4 | 07-02-12 |
| Live Load Deflection | L/660 (0.272") | n\a | 54.5% | 5 | 07-02-12 |
| Max Defl. | 0.389" | n\a | 38.9% | 4 | 07-02-12 |
| Span / Depth | 18.9 | | | | |

| Bearing | յ Supports | Dim. (LxW) | Demand | Demand/ Resistance Support | Demand/ Resistance Member | Material |
|---------|------------|-----------------|----------|----------------------------------|---------------------------------|-----------------|
| B1 | Wall/Plate | 2-3/8" x 2-1/2" | 1273 lbs | 34.8% | 77.0% | Spruce-Pine-Fir |
| B2 | Beam | 2-5/8" x 2-1/2" | 510 lbs | 12.6% | 30.0% | Spruce-Pine-Fir |

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86 BC CALC® analysis is based on Canadian Limit States Design, as per NBC(Ontario Building Code, as amended. These Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



These plans have been reviewed for use with the corrections as noted. No other changes may be ade without written approval of the Building Standards Branch. All work must comply Coning By-Law 2018-043, as amended, and pproved documents must be kept on site at all The building permit must be clearly posted on site at all times.

| Discipline | Keviewei | BCIN | Date |
|---------------|------------|-------|------------|
| Building Code | H. Authier | 43236 | 2021-02-05 |
| Sewage System | | | |
| Zoning | | | |
| | | | |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



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PROFESSIONAL

NE1220-144 Page 16 of 30 Client: Date: 12/17/2020

Brg

isDesign

Project: Address: **GREENPARK**

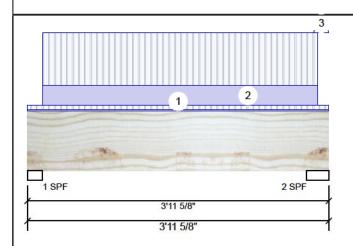
Input by: RCO

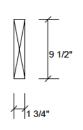
Job Name: BRENTWOOD 3 (ELEV. 1)

Project #:

1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL

Level: Ground Floor





Wind

Member Information

| Type: | Girder | Application: | Floor (Residential) |
|------------------|----------|--|----------------------|
| Plies: | 1 | Design Method: | LSD |
| Moisture Conditi | ion: Dry | Building Code: | NBCC 2015 / OBC 2012 |
| Deflection LL: | 360 | Load Sharing: | No |
| Deflection TL: | 240 | Deck: | Not Checked |
| Importance: | Normal | Vibration: | Not Checked |
| General Load | | 200 CAN 18 LEC 18 (18 CAN 18 C | |
| Floor Live: | 40 PSF | | |
| Dead: | 15 PSF | | |

Unfactored Reactions UNPATTERNED lb (Uplift)

| 1 | 455 | 178 | 0 | 0 |
|---|-----|-----|---|---|
| 2 | 493 | 193 | 0 | 0 |
| | | | | |
| | | | | |
| | | | | |

Dead

Bearings and Factored Reactions

Live

| Bearing Length | Cap. R | eact D/L lb | Total | Ld. Case | Ld. Comb. | |
|----------------|--------|-------------|-------|----------|------------|--|
| 1 - SPF 2.375" | 35% | 222 / 682 | 905 | L | 1.25D+1.5L | |
| 2 - SPF 3.500" | 26% | 241 / 739 | 980 | L | 1.25D+1.5L | |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|--------------------|------------|---------------|-------------|------------|---------|
| Moment | 836 ft-lb | 1'11 1/4" | 11362 ft-lb | 0.074 (7%) | 1.25D+1.5L | L |
| Unbraced | 836 ft-lb | 1'11 1/4" | 9310 ft-lb | 0.090 (9%) | 1.25D+1.5L | L |
| Shear | 521 lb | 11 1/8" | 4638 lb | 0.112 (11%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.003 (L/16203) | 1'11 5/16" | 0.120 (L/360) | 0.020 (2%) | D | Uniform |
| LL Defl inch | 0.007 (L/6315) | 1'11 5/16" | 0.120 (L/360) | 0.060 (6%) | L | L |
| TL Defl inch | 0.010 (L/4544) | 1'11 5/16" | 0.180 (L/240) | 0.050 (5%) | D+L | L |

ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

| ID | Load Type | Location | Trib Width | Side | Dead |
|----|---------------|------------------|------------|------|--------|
| 1 | Tie-In | 0-0-0 to 3-11-10 | 0-5-12 | Тор | 15 PSF |
| 2 | Part. Uniform | 0-2-6 to 3-9-14 | | Тор | 90 PLF |
| 3 | Tie-In | 3-9-4 to 3-11-10 | 0-2-4 | Тор | 15 PSF |
| | Self Weight | | | | 4 PLF |

Comments Wind Live Snow 40 PSF 0 PSF 0 PSF 0 PLF 240 PLF 0 PLF



These plans have been reviewed for use with the corrections as noted. No other changes may be corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orineria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requirement fastening details, beam strength value approvals approvals

 Damaged Beams must not be used
- Daniaged beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation
- For flat roofs provide prop ponding

| | Discipline | Reviewer | BCIN | Date |
|---|---------------|------------|-------|------------|
| • | Building Code | H. Authier | 43236 | 2021-02-05 |
| Ì | Sewage System | | | |
| Ì | Zoning | | | |
| | | | | |

Kott Lumber Company 14 Anderson Blvd, Ontario 905-642-4400



NE1220-144 Page 17 of 30 Client: **GREENPARK**

Project: isDesign Address: Date: 12/17/2020 Input by: RCO

Brg

1

2

Bearing Length

1 - SPF 4.938"

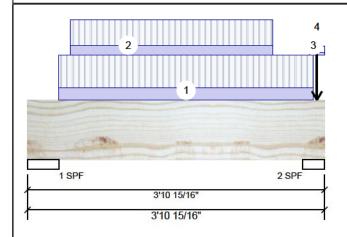
2 - SPF 3.500"

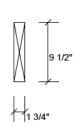
Job Name: BRENTWOOD 3 (ELEV. 1)

Project #:

1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL F1-B

Level: Ground Floor





Wind

0

0

Ld. Comb.

1.25D+1.5L 1.25D+1.5L

0

0

Total Ld. Case

1132 L

1449 I

Member Information

| Type: | Girder | Application: | Floor (Residential) |
|--------------------|--------|-------------------------|----------------------|
| Plies: | 1 | Design Method: | LSD |
| Moisture Condition | n: Dry | Building Code: | NBCC 2015 / OBC 2012 |
| Deflection LL: | 360 | Load Sharing: | No |
| Deflection TL: | 240 | Deck: | Not Checked |
| Importance: | Normal | Vibration: | Not Checked |
| General Load | | 25-12-1-2-1-2-1-2-1-2-1 | |
| Floor Live: | 40 PSF | | |
| Dead: | 15 DQE | | |

Unfactored Reactions UNPATTERNED lb (Uplift)

Dead

222

282

Cap. React D/L lb

Live

570

731

| Bearings and Factored Rea | actions | |
|---------------------------|---------|--|
| | | |
| | | |

277 / 855

352 / 1097

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|--------------------|-------------|---------------|-------------|------------|---------|
| Moment | 1006 ft-lb | 2' 1/16" | 11362 ft-lb | 0.089 (9%) | 1.25D+1.5L | L |
| Unbraced | 1006 ft-lb | 2' 1/16" | 9606 ft-lb | 0.105 (10%) | 1.25D+1.5L | L |
| Shear | 840 lb | 2'10 11/16" | 4638 lb | 0.181 (18%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.003 (L/13711) | 2' 1/8" | 0.111 (L/360) | 0.030 (3%) | D | Uniform |
| LL Defl inch | 0.008 (L/5290) | 2' 1/8" | 0.111 (L/360) | 0.070 (7%) | L | L |
| TL Defl inch | 0.010 (L/3817) | 2' 1/8" | 0.167 (L/240) | 0.060 (6%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

21%

38%

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

| ID | Load Type | Location | Trib Width | Side | Dead |
|----|---------------|------------------|------------|----------|--------|
| 1 | Part. Uniform | 0-4-15 to 3-9-3 | | Тор | 79 PLF |
| 2 | Part. Uniform | 0-6-12 to 3-2-12 | | Far Face | 61 PLF |
| 3 | Tie-In | 3-8-9 to 3-10-15 | 1-0-1 | Тор | 15 PSF |
| 4 | Point | 3-9-12 | | Far Face | 58 lb |
| | Self Weight | | | | 4 PLF |

Comments Live Snow Wind 210 PLF 0 PLF 0 PLF 0 PLF 163 PLF 0 PLF



These plans have been reviewed for use with the corrections as noted. No other changes may be corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amelyded. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orineria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corro
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requirement fastening details, beam strength value maged Beams must not be used
- Daniaged beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation
- For flat roofs provide prop ponding

| Discipline | Reviewer | BCIN | Date |
|---------------|------------|-------|------------|
| Building Code | H. Authier | 43236 | 2021-02-05 |
| Sewage System | | | |
| Zoning | | | |
| | | | |

Kott Lumber Company 14 Anderson Blvd, Ontario 905-642-4400



NE1220-144 Page 18 of 30 Client: GREENPARK Date: 12/17/2020 Input by: Project: RCO isDesign Address: Job Name: BRENTWOOD 3 (ELEV. 1) Project #: 1.750" X 9.500" - PASSED Level: Ground Floor Forex 2.0E-3000Fb LVL 2 3 5 4 2 SPE 1 Hanger 9'2 1/8' 9'2 1/8" Member Information Unfactored Reactions UNPATTERNED lb (Uplift) Application: Brg Dead Wind Type: Floor (Residential) Live Snow Plies: 1 Design Method: 682 354 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 0 649 333 0 2 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 442 / 1023 1.25D+1.5L 3.000" 38% 1465 I Hanger Analysis Results 2 - SPF 6.875" 19% 416 / 973 1389 L 1.25D+1.5L Actual Comb. Case **Analysis** Location Allowed Capacity 0.246 (25%) 1.25D+1.5L L Moment 2795 ft-lb 4'5 1/16" 11362 ft-lb 4'5 1/16" 4396 ft-lb Unbraced 2795 ft-lb 0.636 (64%) 1.25D+1.5L L READ ALL NOTES ON THIS PAGE AND ON AL PROFESSIONAL Shear 1287 lb 11 3/4" 4638 lb 0.277 (28%) 1.25D+1.5L L ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.040 (L/2574) 4'5 1/16" 0.283 (L/360) 0.140 (14%) D Uniform CALCULATION SUMMARY PAGE AS IT LL Defl inch 0.076 (L/1334) 4'5 1/16" 0.283 (L/360) 0.270 (27%) L L CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. A. EL-MASRI TL Defl inch 0.116 (L/879) 4'5 1/16" 0.424 (L/240) 0.270 (27%) D+L L REFER TO MULTIPLE MEMBER TO MEMBER Design Notes CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. 1 Fill all hanger nailing holes. PASS THRU FRAMING SQUASH **20, 202**0 2 Girders are designed to be supported on the bottom edge only. **BLOCK IS REQUIRED AT ALL** 3 Top braced at bearings. POINT LOADS OVER BEARINGS 4 Bottom braced at bearings. ID Location Trib Width Side Dead Comments Load Type Live Snow Wind 15 PSF 40 PSF 1 Tie-In 0-0-0 to 9-2-2 1-10-8 to Top 0 PSF 0 PSF 1-9-12 2 Part Uniform 0-0-0 to 8-7-7 Top 9 PLF 2I F East Gwillimbury 0 lb 3 Point 0 - 3 - 5Near Face 49 lb J1 Part. Uniform 0-10-5 to 7-6-5 Near Face 35 PLF PLF These plans have been reviewed for use with the **0 lb** corrections as noted. No other changes may be Point 8-2-5 Near Face 38 lb Self Weight 4 PLF nade without written approval of the Building Standards Branch. All work must comply Zoning By-Law 2018-043, as amended, an Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all tim Kott Lumber Company Discipline Building Code Reviewer BCIN Notes H. Authier 43236 2021-02-14 Anderson Blvd, Ontario **Handling & Installation** ewage System structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requiremen 905-642-4400

This design is valid until 10/15/2022

Damaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

NE1220-144 Page 19 of 30 Client: GREENPARK Date: 12/17/2020 Project: RCO Input by: isDesign Address: Job Name: BRENTWOOD 3 (ELEV. 1) Project #: 1.750" X 9.500" - PASSED Level: Ground Floor **F3-B** Forex 2.0E-3000Fb LVL 1 1 SPF 2 SPF 10'1 3/4' 10'1 3/4' Unfactored Reactions UNPATTERNED lb (Uplift) Member Information Brg Dead Wind Type: Application: Floor (Residential) Live Snow Plies 1 Design Method: 101 57 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 0 559 812 2 646 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.375" 9% 71 / 152 223 1 1 25D+1 5L 1.25D+1.5S 2 - SPF 6.875" 54% 807 / 1777 2584 I Analysis Results +L Comb **Analysis** Actual Location Allowed Capacity Case 0.046 (5%) 1.25D+1.5L L Moment 514 ft-lb 4'10 5/8" 11248 ft-lb Unbraced 514 ft-lb 4'10 5/8" 3924 ft-lb 0.131 (13%) 1.25D+1.5L L READ ALL NOTES ON THIS PAGE AND ON PROFESSIONAL Shear 180 lb 11 1/8" 4592 lb 0.039 (4%) 1.25D+1.5L L **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.009 4'10 5/8" 0.317 (L/360) 0.030 (3%) D Uniform CALCULATION SUMMARY PAGE AS IT (L/12135) CONTAINS SPECIFICATIONS AND CRITERIA LL Defl inch 0.017 (L/6781) 4'10 5/8" 0.317 (L/360) 0.050 (5%) L+0.5S L USED IN THE DESIGN OF THIS COMPONENT. A. EL-MASRI TL Defl inch 0.026 (L/4350) 4'10 5/8" 0.475 (L/240) 0.060 (6%) D+L+0.5S L REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY **Design Notes** NAILING OR BOLTING REQUIREMENTS. 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam PASS THRU FRAMING SQUASH 2020 **BLOCK IS REQUIRED AT ALL** width X 4.5. POINT LOADS OVER BEARINGS 2 Girders are designed to be supported on the bottom edge only. 3 Top braced at bearings. 4 Bottom braced at bearings. Trib Width ID Load Type Location Side Dead Live Snow Wind Comments 15 PSF AN PSE 0-0-0 to 10-1-12 0 PSF 0 PSF 1 Tie-In 0-6-3Top 2 F11 F11 Point 9-11-6 Top 462 lb East Gwillimbury Olb 3 Point 9-11-6 0 lb Wall Self Weight Top 24 lb Point 9-11-6 4 lb 0 lb Top These plans have been reviewed for use with the corrections as noted. No other changes may be 0 | |b| **Point** 9-11-6 Top 46 lb J5 nade without written approval of the Buildi Standards Branch. All work must comply with bound by Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These 8 Point 9-11-6 49 lb Wall Self Weight Top 4 PLF Self Weight approved documents must be kept on site at all times. The building permit must be clearly posted on site at all tim

> Discipline Building Code

ewage System

This design is valid until 10/15/2022

Reviewer BCIN

H. Authier 43236

2021-02-

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended

Handling & Installation

LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requirement

Damaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

Notes

905-642-4400

Kott Lumber Company

14 Anderson Blvd, Ontario

NE1220-144 Page 20 of 30 Client: GREENPARK Date: 12/17/2020 Input by: Project: RCO isDesign Address: Job Name: BRENTWOOD 3 (ELEV. 1) Project #: 1.750" X 9.500" Level: Ground Floor Forex 2.0E-3000Fb LVL 2-Ply - PASSED 2 3 1 2 SPF 1 SPF End Grain 9'1 5/8' 9'1 5/8" Member Information Unfactored Reactions UNPATTERNED lb (Uplift) Application: Brg Live Dead Snow Wind Type: Floor (Residential) Plies Design Method: 774 433 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 0 90 77 n 2 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 3.500" 541 / 1161 1702 I 1 25D+1 5L Fnd Grain Analysis Results 96 / 135 1.25D+1.5L 2 - SPF 2.375" 5% 231 L Actual Location Allowed Comb Case **Analysis** Capacity 4'7 3/8" 22724 ft-lb Moment 492 ft-lb 0.022 (2%) 1.25D+1.5L L Unbraced 492 ft-lb 4'7 3/8" 21205 ft-lb 0.023 (2%) 1.25D+1.5L L READ ALL NOTES ON THIS PAGE AND ON ALD PROFESSIONAL Shear 8'2 1/2" 9277 lb 0.020 (2%) 1.25D+1.5L L **ENGINEERING NOTE PAGE ENP-2. THIS** 184 lb NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.005 4'7 3/8" 0.292 (L/360) 0.020 (2%) D Uniform CALCULATION SUMMARY PAGE AS IT (L/20571) CONTAINS SPECIFICATIONS AND CRITERIA LL Defl inch 0.006 4'7 3/8" 0.292 (L/360) 0.020 (2%) L USED IN THE DESIGN OF THIS COMPONENT. .A. EL-MASRI (L/17660) REFER TO MULTIPLE MEMBER TO MEMBER TL Defl inch 0.011 (L/9502) 4'7 3/8" 0.439 (L/240) 0.030 (3%) D+L CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. Design Notes PASS THRU FRAMING SQUASH **20, 2020** 1 Girders are designed to be supported on the bottom edge only. **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS 2 Multiple plies must be fastened together as per manufacturer's details. 3 Top loads must be supported equally by all plies. 4 Top braced at bearings. 5 Bottom braced at bearings. 6 Lateral slenderness ratio based on full section width. ID Trib Width Comments Load Type Location Side Dead ind East Gwillimbury SF Tie-In 0-0-0 to 9-1-10 Top 15 PSF 1 2 Part. Uniform 0-0-0 to 8-11-4 Top 2 PLF 3 Point 0-1-12 Near Face These plans have been reviewed for use with the D lb corrections as noted. No other changes may be 354 lb Self Weight 8 PLF made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times. Kott Lumber Company Discipline Building Code Reviewer BCIN Notes H. Authier 43236 2021-02-14 Anderson Blvd, Ontario **Handling & Installation** ewage System structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requirement 905-642-4400 aged Beams must not be used Daniaged beams must not be used

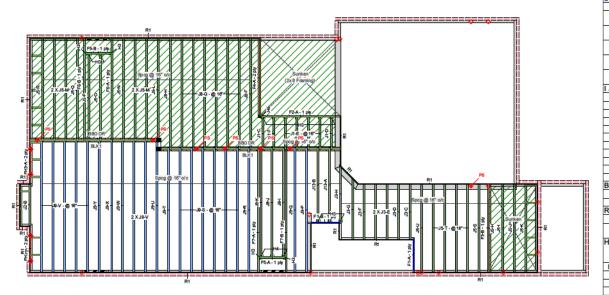
Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid

lateral displacement and rotation Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corr This design is valid until 10/15/2022 Version 19.60.173 Powered by iStruct™ CSD DESIGN

NE1220-144

Ground Floor



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

East Gwillimbury Building Standards Branch BCIN #16487

These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

| Discipline | Reviewer | BCIN | Date |
|---------------|------------|-------|------------|
| Building Code | H. Authier | 43236 | 2021-02-05 |
| Sewage System | 1 | | |
| Zoning | | | |

CONNECTION DETAIL FOR PLY TO PLY
NAILING OR BOLTING REQUIREMENTS. ASS THRU FRAMING SQUAS BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARING

REFER TO MULTIPLE MEMBER TO MEMBE

JOB INFORMATION Ground Floor LVL/LSL (Flush) Builder Label Description Width Depth Qty Plies Pcs Length GREENPARK F3 Forex 1.75 9.5 1 12-0-0 2.0E-3000Fb LVL Project F4 1.75 9.5 10-0-0 Shipping 2.0E-3000Fb LVL Sales Rep 10-0-0 2.0E-3000Fb LVL FH3 Forex 175 9.5 2 2 4 4-0-0 Designer 2.0E-3000Fb LVL RCO F1 4-0-0 Forex 2.0E-3000Fb LVL 1.75 2 Plotted December 17, 2020 I Joist (Flush) Label Description Width Depth Qty Plies Pcs Length Layout Name F7 A.IS 140 2.5 9.5 2 16-0-0 BRENTWOOD 3 (ELEV. 1,2) DECK CONDITION F6 AJS 140 2.5 9.5 2 12-0-0 Job Path F5 AJS 140 2.5 9.5 2 4-0-0 C:\Users\rochavillo\Documents\WORK FROM HOME \GREENPARK\TRINAR HALL\BRENTWOOD 3\ELEV J8 AJS 140 J5 AJS 140 2.5 9.5 11 14-0-0 9.5 20 12-0-0 1/FLOOR/DECK CONDITION/BRENTWOOD 3 (ELEV 2 10-0-0 7 8-0-0 J13 AJS 140 J3 AJS 140 2.5 9.5 1) isl 9.5 **DESIGN CRITERIA** J2 AJS 140 J1 AJS 140 2.5 9.5 1 6-0-0 7 4-0-0 Ground Floor 2.5 9.5 21 16-0-0 Design Method LSD (Canada) J9 AJS 20 2.5 9.5 **Building Code** NBCC 2015 / OBC 2012 Blocking
 Width
 Depth
 Qty
 Plies
 Pcs
 Length
 Floor

 2.5
 9.5
 LinRt
 Varies
 49-0-0
 Loads
 Label Description BLK1 AJS 140 Varies 49-0-0 Loads Rim Board Live Width Depth Qty Plies Pcs Length Dead Label Description 15 R1 Norbord Rimboard 1.125 9.5 14 12-0-0 Deflection Joist Plus 1.125 X 9.5 LL Span L/ 480 Hanger 360 TL Span L/ Beam/Girder Supported 480 LL Cant 2L/ Member TL Cant 2L/ 360 Label Pcs Description Skew Slope fasteners fasteners Deflection Girder 15 LF259 H3 LL Span L/ 360 H3 1 #8x1 1/4WS TL Span L/ LL Cant 2L/ 480 H6 1 HUS1.81/10 360 TL Cant 2L/

Decking

Decking

Thickness

Fastener

Vibration

Canada

14A7X4

905-642-4400

CCMC References

Boise - 12472-R , 12787-R LP - 12412-R

orex - 14056-R

1. All blocking to be cut from 12' joists

2. 2' & 4' Lengths to be cut from 8' Length, 6' lengths to be cut from 12' Length

3. Ends of joists to be laterally supported

4. Packing of Steel beams and attachment by others

5. Shower and water closet flange locations are approximate only, consult architectural drawing for exact locations

6. Beams identified as "B" are dropped and supplied by others

7. Install 2x4 blocking @ 24" o/c under parallel non-loadbearing walls 8. Load transfer blocks to be installed under all point loads

9. Refer to Multiple Member Connection Detail for ply to ply nailing or bolting

requirements 10. Hangers and Fasteners to be installed as per manufacturer

Framing shown on this layout may deviate from architectural drawings. Architeng to review and approve the deviation prior to construction.

Legend



Point Load Support Load from Above Wall Opening Norbord Rimboard Plus 1,125 X AJS 140 9.5 AJS 20 9.5 Forex 2.0E-3000Fb LVL 1.75 X

Kott Lumber Company 14 Anderson Blvd KOTT Stouffville, Ontario

OSB

3/4"

Nailed & Glued

Hatch Area represents where al load has been app e.g. 5 psf for ceramic tile)

Version 19.60.173 Powered by iStruct**

This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them

NE1220-144 Page 22 of 30 Client: **GREENPARK** 12/17/2020

isDesign Address:

Project:

Date: Input by: RCO

BRENTWOOD 3 (ELEV. 1) DECK CONDITION Job Name:

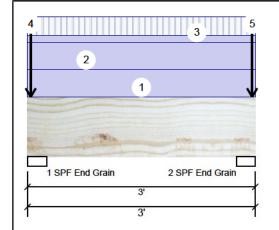
Project #:

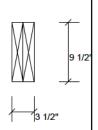
Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor





Wind

Member Information

| Type: | Girder | Application: | Floor (Residential) |
|---------------------|--------|--------------------------------|----------------------|
| Plies: | 2 | Design Method: | LSD |
| Moisture Condition: | Dry | Building Code: | NBCC 2015 / OBC 2012 |
| Deflection LL: | 360 | Load Sharing: | No |
| Deflection TL: | 240 | Deck: | Not Checked |
| Importance: | Normal | Vibration: | Not Checked |
| General Load | | 52.45.47.95.045.000000.000.000 | |
| Floor Live: | 40 PSF | | |

| | Reactions | UNPATTERI | NED lb (Uplift) |
|-----|-----------|-----------|-----------------|
| Brg | Live | Dead | Snow |

| 1 | 147 | 418 | 245 | 0 |
|---|-----|-----|-----|---|
| 2 | 147 | 418 | 245 | 0 |
| | | | | |
| | | | | |
| | | | | |

Analysis Results

Dead:

| _ | | | | | | |
|---------------|--------------------|----------|---------------|------------|------------|---------|
| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
| Moment | 140 ft-lb | 1'6" | 17497 ft-lb | 0.008 (1%) | 1.25D+1.5L | L |
| Unbraced | 140 ft-lb | 1'6" | 17497 ft-lb | 0.008 (1%) | 1.25D+1.5L | L |
| Shear | 166 lb | 2' 1/4" | 7143 lb | 0.023 (2%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.000 (L/63020) | 1'6" | 0.088 (L/360) | 0.010 (1%) | D | Uniform |
| LL Defl inch | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |
| TL Defl inch | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |

Bearings and Factored Reactions

| Bearing Length | Cap. Re | eact D/L lb Tota | Ld. Case | Ld. Comb. |
|--------------------------------|---------|------------------|----------|------------------|
| 1 - SPF 3.000" End Grain | 16% | 523 / 514 1037 | L | 1.25D+1.5S +L |
| 2 - SPF 3.000" End Grain | 16% | 523 / 514 1037 | L | 1.25D+1.5S +L |

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.

15 PSF

- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

| | ID Load Type | | Location | Trib Width | Side | Dead |
|--|-------------------|-----------------|----------------|----------------|-----------|--------|
| | 1 | Part. Uniform | 0-0-0 to 3-0-0 | | Тор | 40 PLF |
| | 2 | Part. Uniform | 0-0-0 to 3-0-0 | 0-0-0 to 3-0-0 | | 40 PLF |
| | 3 | 3 Tapered Start | | | Near Face | 10 PLF |
| | | End | 3-0-0 | | | 10 PLF |
| | Continued on page | 2 | | | | |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER

CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL**



East Gwillimbury ind

POINT LOADS OVER BEARINGS.

Comments ?LF Wall Self Weight

These plans have been reviewed for use with the corrections as noted. No other changes may be and without written approval of the Building corrections as noted. No other changes may be LF made without written approval of the Building Standards Branch. All work must comply with LF Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times. Wall Self Weight

Discipline Reviewer BCIN
Building Code H. Authier 43236 Reviewer BCIN Date Notes 2021-02-05 Handling & Installation Sewage System

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orienta and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requirements fastening details, beam strength value
- approvals

 Damaged Beams must not be used
- Daniaged beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation
 - This design is valid until 10/15/2022

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



NE1220-144 Page 23 of 30 Client: 12/17/2020

isDesign Address:

Project:

GREENPARK

Date: Input by: RCO

BRENTWOOD 3 (ELEV. 1) DECK CONDITION Job Name:

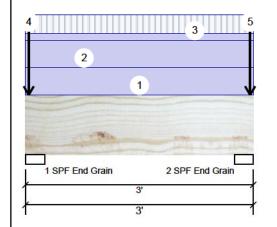
Project #:

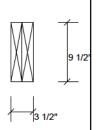
Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor





| Contin | Continued from page 1 | | | | | | | | | | |
|--------|-----------------------|----------|------------|------|--------|--------|--------|------|--------------------------------|--|--|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments | | |
| 4 | Point | 0-0-8 | | Тор | 272 lb | 106 lb | 245 lb | 0 lb | Header Column Header Column | | |
| 5 | Point | 2-11-8 | | Тор | 272 lb | 106 lb | 245 lb | 0 lb | Header Column Header Column | | |
| | Self Weight | | | | 8 PLF | | | | | | |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER

CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amehded. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orineria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

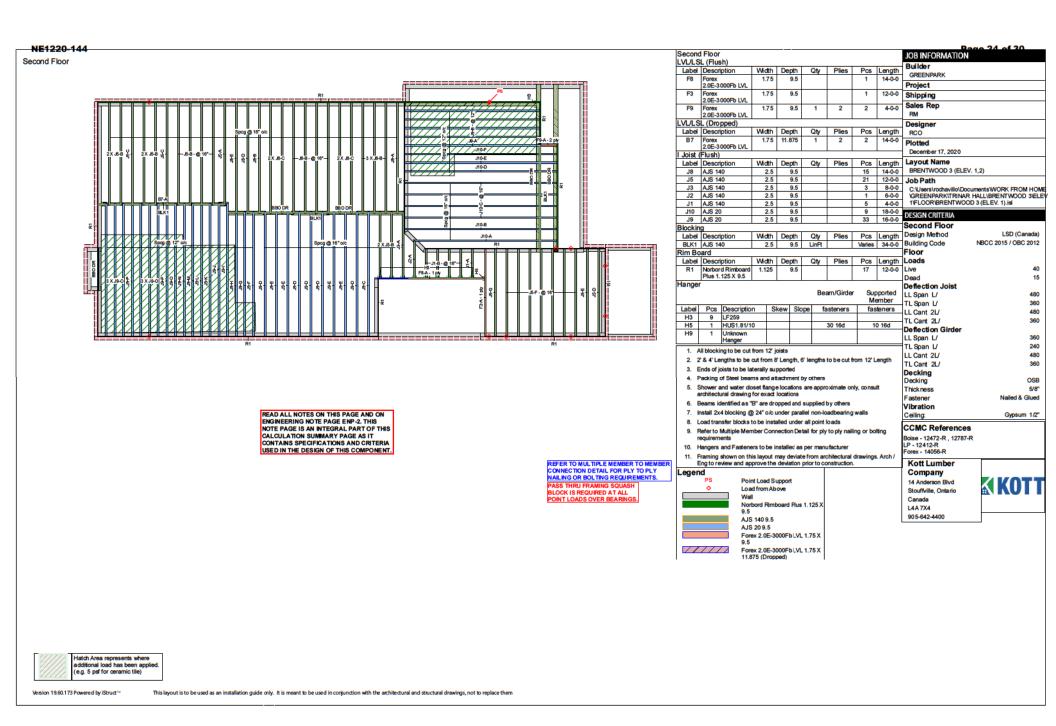
- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- 1. LVL beams must not be out or drilled
 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals
 3. Damaged Beams must not be used
 4. Design assumes top edge is laterally restrained
 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- For flat roofs provide proporting

| | Discipline | Reviewer | BCIN | Date | ı |
|-----|---------------|------------|-------|------------|---|
| ope | Building Code | H. Authier | 43236 | 2021-02-05 | |
| | Sewage System | | | | |
| | Zoning | | | | |

Kott Lumber Company 14 Anderson Blvd, Ontario 905-642-4400







NE1220-144 Page 25 of 30 Client: **GREENPARK** Date: 12/17/2020

isDesign

Project: Address:

Input by: RCO

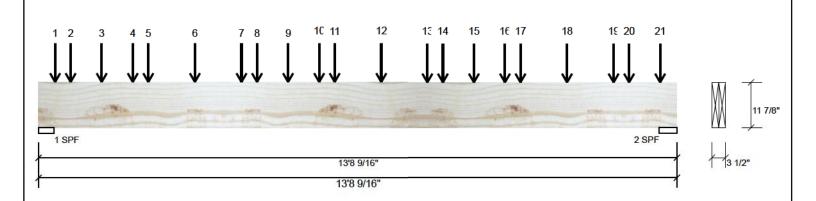
Job Name: BRENTWOOD 3 (ELEV. 1)

Project #:

B7-A Forex 2.0E-3000Fb LVL

1.750" X 11.875" 2-Ply - PASSED

Level: Second Floor



| Member Intorn | nation | | | Unfactored Reactions UNPAI TERNED ID (Upilit) | | | | | | |
|---------------------|--------|----------------|----------------------|---|--|------------------------------|----------------|------------|--|--|
| Type: | Girder | Application: | Floor (Residential) | Brg | Live | Dead | Snow | Wind | | |
| Plies: | 2 | Design Method: | LSD | 1 | 3681 | 1568 | 0 | 0 | | |
| Moisture Condition: | Dry | Building Code: | NBCC 2015 / OBC 2012 | 2 | 3461 | 1466 | 0 | 0 | | |
| Deflection LL: | 360 | Load Sharing: | No | 334 | | | | | | |
| Deflection TL: | 240 | Deck: | Not Checked | | | | | | | |
| Importance: | Normal | Vibration: | Not Checked | | | | | | | |
| General Load | | | | | F F 100 - 100 TO | an expense of a stage of the | | | | |
| Floor Live: | 40 PSF | | | Bearings a | and Facto | red Reactions | | | | |
| Dead: | 15 PSF | | | Bearing L | ength | Cap. React D/L lb | Total Ld. Case | Ld. Comb. | | |
| | | | | 1 - SPF 4 | .000" | 87% 1960 / 5521 | 7481 L | 1.25D+1.5L | | |
| | | | | 2 - SPF 4 | .575" | 71% 1832 / 5192 | 7024 L | 1.25D+1.5L | | |

Analysis Results

Mambay Information

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|---------------|-----------|---------------|-------------|------------|---------|
| Moment | 23291 ft-lb | 7'4 1/4" | 34261 ft-lb | 0.680 (68%) | 1.25D+1.5L | L |
| Unbraced | 23291 ft-lb | 7'4 1/4" | 26246 ft-lb | 0.887 (89%) | 1.25D+1.5L | L |
| Shear | 6570 lb | 12'4 7/8" | 11596 lb | 0.567 (57%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.168 (L/936) | 6'10" | 0.438 (L/360) | 0.380 (38%) | D | Uniform |
| LL Defl inch | 0.396 (L/398) | 6'10" | 0.438 (L/360) | 0.910 (91%) | L | L |
| TL Defl inch | 0.564 (L/279) | 6'10" | 0.656 (L/240) | 0.860 (86%) | D+L | L |

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width

| | / Lateral Sicilu | erriess ratio based orr ruit se | CUOIT WIGHT. | | | | |
|---------------------|------------------|---------------------------------|--------------|------------|------|--------|--|
| | ID | Load Type | Location | Trib Width | Side | Dead | |
| | 1 | Point | 0-4-4 | | Тор | 131 lb | |
| | 2 | Point | 0-8-4 | | Тор | 115 lb | |
| | 3 | Point | 1-4-4 | | Тор | 131 lb | |
| | 4 | Point | 2-0-4 | | Тор | 115 lb | |
| | 5 | Point | 2-4-4 | | Тор | 131 lb | |
| Continued on page 2 | | | | | | | |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

Unfactored Deactions UNIDATTEDNED Ib (Unlift)

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



East Gwillimbury

Comments 0 lb .19 0 lb J5

These plans have been reviewed for use with the ${\bf 0}$ lb corrections as noted. No other changes may be J9 corrections as noted. No other changes may be made without written approval of the Building of Ib Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times. J5 J9

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orineria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- 1. IVI, beams must not be out or drilled
 2. Refer to manufacturer's product inforegarding installation requirements, in fastening details, beam strength values, an approvals
 3. Damaged Beams must not be used
- Daniaged beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide prop ponding

| | Discipline | ICCVICACI | Den | Date | |
|---|---------------|------------|-------|------------|--|
| • | Building Code | H. Authier | 43236 | 2021-02-05 | |
| | Sewage System | | | | |
| | Zoning | | | | |
| | | | | | |

Kott Lumber Company 14 Anderson Blvd, Ontario 905-642-4400



NE1220-144 Page 26 of 30 Client: Page 7 of 11

isDesign Address:

Project:

GREENPARK

Date: 12/17/2020 Input by: RCO

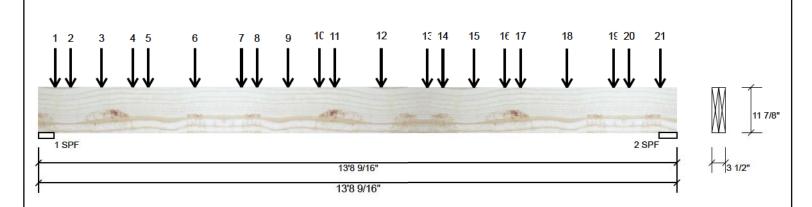
Job Name: BRENTWOOD 3 (ELEV. 1)

Project #:

B7-A Forex 2.0E-3000Fb LVL

1.750" X 11.875" 2-Ply - PASSED

Level: Second Floor



| Continued | from page 1 | | | | | | | |
|-----------|-------------|--------------------|---------|--------|--------|------|------|----------|
| ID | Load Type | Location Trib Widt | th Side | Dead | Live | Snow | Wind | Comments |
| 6 | Point | 3-4-4 | Тор | 249 lb | 616 lb | 0 lb | 0 lb | J5 J9 |
| 7 | Point | 4-4-4 | Тор | 131 lb | 302 lb | 0 lb | 0 lb | J9 |
| 8 | Point | 4-8-4 | Тор | 115 lb | 306 lb | 0 lb | 0 lb | J5 |
| 9 | Point | 5-4-4 | Тор | 131 lb | 302 lb | 0 lb | 0 lb | J9 |
| 10 | Point | 6-0-4 | Тор | 115 lb | 306 lb | 0 lb | 0 lb | J5 |
| 11 | Point | 6-4-4 | Тор | 131 lb | 302 lb | 0 lb | 0 lb | J9 |
| 12 | Point | 7-4-4 | Тор | 249 lb | 616 lb | 0 lb | 0 lb | J9 J5 |
| 13 | Point | 8-4-4 | Тор | 131 lb | 302 lb | 0 lb | 0 lb | J9 |
| 14 | Point | 8-8-4 | Тор | 115 lb | 306 lb | 0 lb | 0 lb | J5 |
| 15 | Point | 9-4-4 | Тор | 129 lb | 302 lb | 0 lb | 0 lb | J9 |
| 16 | Point | 10-0-4 | Тор | 115 lb | 306 lb | 0 lb | 0 lb | J5 |
| 17 | Point | 10-4-4 | Тор | 127 lb | 302 lb | 0 lb | 0 lb | J9 |
| 18 | Point | 11-4-4 | Тор | 245 lb | 616 lb | 0 lb | 0 lb | J9 J5 |
| 19 | Point | 12-4-4 | Тор | 127 lb | 302 lb | 0 lb | 0 lb | J9 |
| 20 | Point | 12-8-4 | Тор | 115 lb | 306 lb | 0 lb | 0 lb | J5 |
| 21 | Point | 13-4-4 | Тор | 55 lb | 132 lb | 0 lb | 0 lb | J9 |
| | Self Weight | | | 10 PLF | | | | |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



PASS 1H

BLOCK II These plans have been reviewed for use with the

POINT LC

corrections as noted. No other changes may be
made without written approval of the Building

Standards Branch. All work must comply with

Zoning By-Law 2018-043, as amended, and the
Ontario Building Code, as amehded. These
approved documents must be kept on site at all

times. The building permit must be clearly
posted on site at all times.

Notes

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- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- 1. LVL beams must not be cut or drilled
 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 3. Damaged Beams must not be used
- Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation 4. 5.
- For flat roofs provide proper ponding Sewage System

Discipline Reviewer BCIN Date
Building Code H. Authier 43236 2021-02-05

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



NE1220-144 Page 27 of 30 Client: GREENPARK Date: 12/17/2020 Project: Input by: RCO isDesign Address: Job Name: BRENTWOOD 3 (ELEV. 1) Project #: 1.750" X 9.500" - PASSED Level: Second Floor Forex 2.0E-3000Fb LVL 2 3 1 SPF 2 SPF 10'1 10'1 Member Information Unfactored Reactions UNPATTERNED lb (Uplift) Application: Brg Live Dead Wind Type: Floor (Residential) Snow Plies: 1 Design Method: 332 152 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 0 615 267 0 2 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Cap. React D/L lb Dead: Bearing Length Total Ld. Case Ld. Comb. 1 - SPF 4.375" 15% 190 / 498 688 I 1.25D+1.5L 2 - SPF 4.375" 27% 333 / 922 1255 I 1.25D+1.5L Analysis Results Actual Location Allowed Comb. Case **Analysis** Capacity 0.289 (29%) 1.25D+1.5L L Moment 3284 ft-lb 6'7 7/8" 11362 ft-lb Unbraced 3284 ft-lb 6'7 7/8" 3933 ft-lb 0.835 (84%) 1.25D+1.5L L READ ALL NOTES ON THIS PAGE AND ON &D PROFESSIONAL ENGINEERING NOTE PAGE ENP-2. THIS Shear 1135 lb 8'11 7/8" 4638 lb 0.245 (24%) 1.25D+1.5L L NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.042 (L/2721) 5'6 1/4" 0.316 (L/360) 0.130 (13%) D Uniform CALCULATION SUMMARY PAGE AS IT LL Defl inch 0.096 (L/1190) 5'6 5/8" 0.316 (L/360) 0.300 (30%) L L CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. A. EL-MASRI TL Defl inch 0.137 (L/828) 5'6 1/2" 0.474 (L/240) 0.290 (29%) D+L L REFER TO MULTIPLE MEMBER TO MEMBER Design Notes CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. 1 Girders are designed to be supported on the bottom edge only. PASS THRU FRAMING SQUASH Dec 20, 2020 2 Top braced at bearings. **BLOCK IS REQUIRED AT ALL** 3 Bottom braced at bearings. POINT LOADS OVER BEARING ID Load Type Location Trib Width Side Dead Snow Wind Comments 15 PSF 40 PSF 0 PSF 1 Tie-In 0-0-0 to 10-1-0 0-6-12 Top 0 PSF 2 6-7-14 Far Face 256 lb Point 616 lb 0 lb 0 lb F8 15 PSF n ner n DSF 3 Tie-In 6-8-12 to 10-1-0 0-9-4 Top ? East Gwillimbury Self Weight 4 PI F These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building

Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Notes

structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corr

Handling & Installation

 LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requiremen aged Beams must not be used

Daniaged beams must not be used

Possign assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid

lateral displacement and rotation

6. For flat roofs provide pro

| Discipline | Reviewer | BCIN | Date |
|---------------|------------|-------|------------|
| Building Code | H. Authier | 43236 | 2021-02-05 |
| Sewage System | | | |
| Zoning | | | |
| | | | |

Kott Lumber Company 14 Anderson Blvd, Ontario 905-642-4400



NE1220-144 Page 28 of 30 Client: GREENPARK Date: 12/17/2020 Project: Input by: RCO isDesign Address: Job Name: BRENTWOOD 3 (ELEV. 1) Project #: 1.750" X 9.500" - PASSED Level: Second Floor Forex 2.0E-3000Fb LVL 3 7 2 6 1 SPF 2 Hanger (HUS1.81/10) 12'8 7/8' 12'8 7/8" Unfactored Reactions UNPATTERNED lb (Uplift) Member Information Brg Dead Wind Type: Application: Floor (Residential) Plies: 1 Design Method: 1449 569 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 0 616 256 0 2 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 5.500" 49% 711 / 2174 2884 I 1.25D+1.5L 2 3 000" 32% 320 / 924 1244 I 1.25D+1.5L Analysis Results Hanger Actual Capacity Comb **Analysis** Location Allowed Case Moment 4839 ft-lb 4'7 5/16" 11362 ft-lb 0.426 (43%) 1.25D+1.5L L Unbraced 4839 ft-lb 4'7 5/16" 4849 ft-lb 0.998 1.25D+1.5L L READ ALL NOTES ON THIS PAGE AND ON PROFESSIONAL (100%)**ENGINEERING NOTE PAGE ENP-2. THIS** Shear 2110 lb 1'2 1/4" 4638 lb 0.455 (45%) 1.25D+1.5L L NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT Perm Defl in. 0.108 (L/1349) 6'1" 0.405 (L/360) 0.270 (27%) D Uniform CONTAINS SPECIFICATIONS AND CRITERIA LL Defl inch 0.266 (L/548) 6' 1/2" 0.405 (L/360) 0.660 (66%) L L USED IN THE DESIGN OF THIS COMPONENT. A. EL-MASRI TL Defl inch 0.375 (L/389) 6' 11/16" 0.608 (L/240) 0.620 (62%) D+L L REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY **Design Notes** NAILING OR BOLTING REQUIREMENTS. 1 Fill all hanger nailing holes. PASS THRU FRAMING SQUASH 2020 **BLOCK IS REQUIRED AT ALL** 2 Girders are designed to be supported on the bottom edge only. POINT LOADS OVER BEARINGS 3 Top must be laterally braced at a maximum of 7'8 1/4" o.c. 4 Bottom braced at bearings. Wind Comments Live Snow 152 PLF 0 PLF 0 PLF PLF East Gwillimbury 0 lb .13 0 lb J2

| ID | Load Type | Location | Trib Width | Side | Dead |
|----|---------------|-------------------|------------|----------|--------|
| 1 | Part. Uniform | 0-0-0 to 2-7-5 | | Far Face | 57 PLF |
| 2 | Part. Uniform | 0-3-12 to 3-9-12 | | Тор | 90 PLF |
| 3 | Point | 3-3-5 | | Far Face | 71 lb |
| 4 | Point | 4-7-5 | | Far Face | 41 lb |
| 5 | Part. Uniform | 5-3-5 to 10-7-5 | | Far Face | 24 PLF |
| 6 | Tie-In | 11-3-5 to 12-8-14 | 1-8-14 | Тор | 15 PSF |
| 7 | Point | 11-3-5 | | Far Face | 34 lb |
| | Self Weight | | | | 4 PLF |
| | | | | | |

These plans have been reviewed for use with the LF corrections as noted. No other changes may be made without written approval of the Building'SF Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These the 0 lb approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Notes

structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corn

Handling & Installation

LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requirement

Daniaged beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid

lateral displacement and rotation

aged Beams must not be used

| 6. | For | flat | roofs | provide | prope |
|----|-----|------|-------|---------|-------|
| | pon | ding | | | |

| | Discipline | Reviewer | BCIN | Date |
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| ₽€ | Building Code | H. Authier | 43236 | 2021-02-05 |
| | Sewage System | | | |
| | Zoning | | | |

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NE1220-144 Page 29 of 30 Client: 12/17/2020

isDesign Address:

Project:

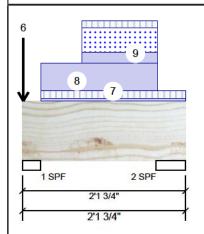
GREENPARK

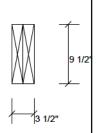
Date: Input by: RCO

Job Name: BRENTWOOD 3 (ELEV. 1)

Project #:

1.750" X 9.500" Forex 2.0E-3000Fb LVL 2-Ply - PASSED Level: Second Floor





Wind

| Mem | ber | Info | rma | tion |
|-----|-----|------|-----|------|
| | | | | |

| Type: | Girder | Application: | Floor (Residential) |
|--------------------|---------|----------------------------|----------------------|
| Plies: | 2 | Design Method: | LSD |
| Moisture Condition | on: Dry | Building Code: | NBCC 2015 / OBC 2012 |
| Deflection LL: | 360 | Load Sharing: | No |
| Deflection TL: | 240 | Deck: | Not Checked |
| Importance: | Normal | Vibration: | Not Checked |
| General Load | | 02/02/20/02/03/04/04/02/05 | |
| Floor Live: | 40 PSF | | |
| Dead: | 15 PSF | | |

Unfactored Reactions UNPATTERNED lb (Uplift) Brg Live Dead

| 1 | 29 | 103 | 41 | 0 |
|-----|----|-----|----|---|
| 1 2 | 39 | 101 | 47 | 0 |
| | | | | |
| | | | | |
| | | | | |

Analysis Results

| L | • | | | | | | |
|---|---------------|---------------|-----------|---------------|------------|------------------|------|
| Γ | Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
| | Moment | 89 ft-lb | 1' 11/16" | 20224 ft-lb | 0.004 (0%) | 1.25D+1.5S +L | L |
| | Unbraced | 89 ft-lb | 1' 11/16" | 20224 ft-lb | 0.004 (0%) | 1.25D+1.5S +L | L |
| | Shear | 28 lb | 11 5/8" | 8256 lb | 0.003 (0%) | 1.25D+1.5S +L | L |
| | Perm Defl in. | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |
| | LL Defl inch | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |
| l | TL Defl inch | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |

Bearings and Factored Reactions

| Bearing Length | Cap. R | eact D/L lb | Total | Ld. Case | Ld. Comb. | |
|----------------|--------|-------------|-------|----------|------------------|--|
| 1 - SPF 2.875" | 4% | 129 / 91 | 220 | L | 1.25D+1.5S +L | |
| 2 - SPF 4.625" | 3% | 126 / 105 | 231 | L | 1.25D+1.5L +S | |

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

| ID | Load Type | Location | Trib Width | Side | Dead |
|---------------------|-----------|----------|------------|------|------|
| 1 | Point | 0-0-2 | | Тор | 5 lb |
| 2 | Point | 0-0-2 | | Тор | 6 lb |
| 3 | Point | 0-0-2 | | Тор | 2 lb |
| 4 | Point | 0-0-2 | | Тор | 3 lb |
| Continued on page 2 | | | | | |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



East Gwillimbury

| Our town, Our future | |
|--|------|
| These plans have been reviewed for use with the | lb |
| corrections as noted. No other changes may be made without written approval of the Building | lb V |
| Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the 0 | lb |
| Ontario Building Code, as amended. These approved documents must be kept on site at all 0 times. The building permit must be clearly | lb V |
| posted on site at all times. | |

Comments

Wall Self Weight Wall Self Weight

ind

Notes

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| | Discipinic | Reviewei | DCII | Date |
|---|---------------|------------|-------|------------|
| × | Building Code | H. Authier | 43236 | 2021-02-05 |
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| | Zoning | | | |
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NE1220-144 Page 30 of 30 Client: **GREENPARK** Date: 12/17/2020 Page 11 of 11

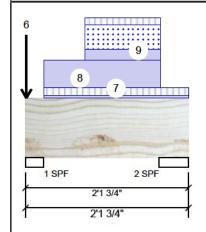
Project: isDesign Address:

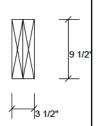
Input by: RCO

Job Name: BRENTWOOD 3 (ELEV. 1)

Project #:

1.750" X 9.500" F9-A Forex 2.0E-3000Fb LVL 2-Ply - PASSED Level: Second Floor





| Continued from page 1 | | | | | | | | | | |
|-----------------------|---------------|------------------|------------|------|--------|--------|--------|-------|------------------|--|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments | |
| 5 | Point | 0-0-2 | | Тор | 1 lb | 1 lb | 2 lb | 0 lb | | |
| 6 | Point | 0-0-2 | | Тор | 1 lb | 0 lb | 0 lb | 0 lb | Wall Self Weight | |
| 7 | Tie-In | 0-2-14 to 2-1-12 | 0-6-6 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | | |
| 8 | Part. Uniform | 0-2-14 to 1-9-6 | | Тор | 80 PLF | 0 PLF | 0 PLF | 0 PLF | Wall Self Weight | |
| 9 | Part. Uniform | 0-9-6 to 1-9-6 | | Тор | 31 PLF | 21 PLF | 70 PLF | 0 PLF | | |
| | Self Weight | | | | 8 PLF | | | | | |

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PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



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 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proporting

| | Discipline | Reviewer | BCIN | Date | ш |
|-----|---------------|------------|-------|------------|---|
| ope | Building Code | H. Authier | 43236 | 2021-02-05 |] |
| | Sewage System | | | | |
| | Zoning | | | | |

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