GREENPARK-TRINAR HALL-EAST
GWILLIMBURY-ON-GLENWAY 2A-EL-1-2-3

Engineering Note Page (ENP-2)

REVISION 2018-10-17

Please read all notes prior to installation of the component

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with transfer blocks. Structural elements such as walls, posts, connectors, and transfer blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of floor joists is to be carried out in accordance with the current edition of the manufacturer's literature available at http://www.kottgroup.com.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru transfer block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written standards Branch. All work must comply with authorization.

made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended, and made and the Standards Branch.

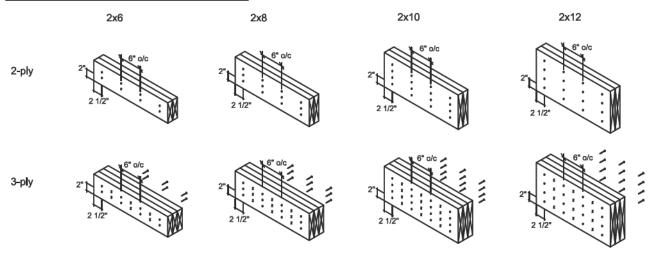


These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-08
Sewage System			
Zoning			



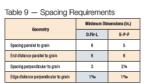
Conventional Connections



Conventional connection notes:

- -Nails to be 3" long wire nails.
- -Nalls to be located 2" mln. from the top and bottom of the member. Start all nalls 2 1/2" mln. from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nall driven from the opposite side.

SIMPSON SDW SPACING REQUIREMENT





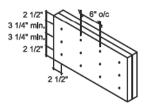
Spacing Requirements

LVL Connections

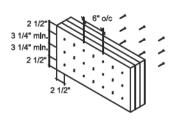
9 1/2" LVL

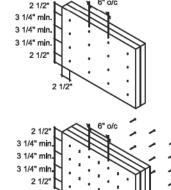
HEAD OF ALL SPECIFIED NAILS AND SCREWS MUST BE ON THE LOADED SIDE

2 1/2" 3 1/4" min. 2 1/2" 2 1/2"



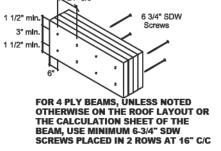
11 7/8" - 14" LVL





16"-18" LVL

4-ply LVL (Top load only)





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Disciplin	С	Reviewer	BCIN	Date
Building	Code	H. Authier	43236	2021-02-08
Sewage S	ystem			
Zoning				

LVL connection notes:

- -LVL ply width is 1-3/4"
- -Nalls to be 3 1/2" common wire nalls.
- -Nails to be located 2 1/2" min. from the top and bottom of the member.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nall or screw driven from the opposite side.

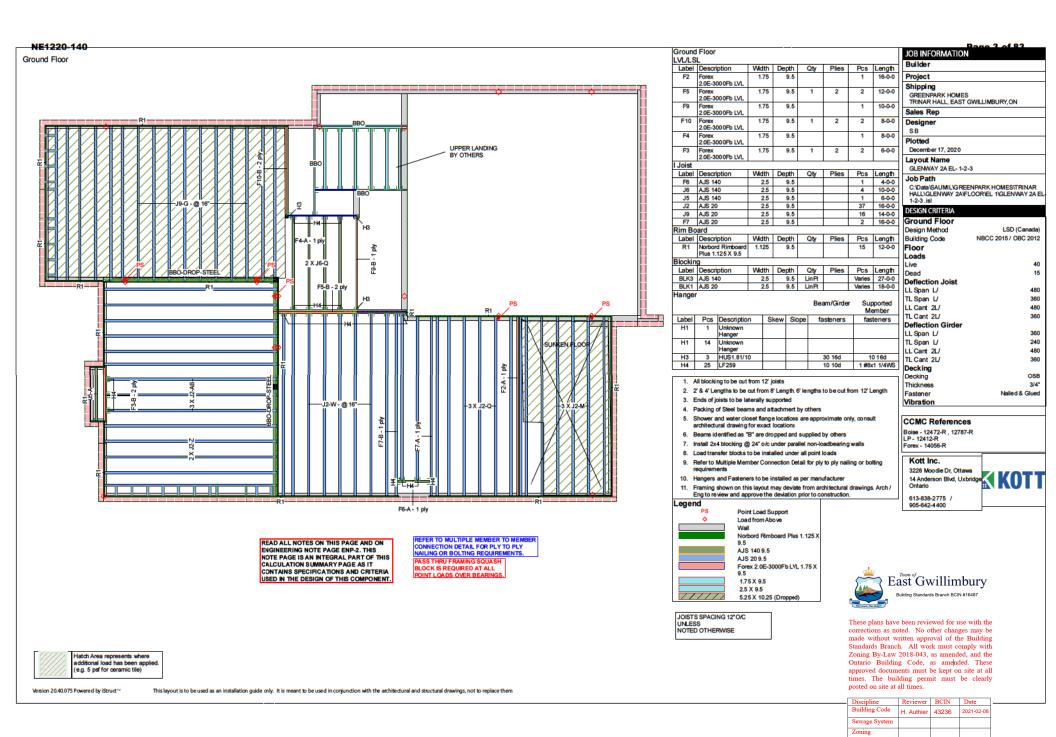
Multiple Member Connections

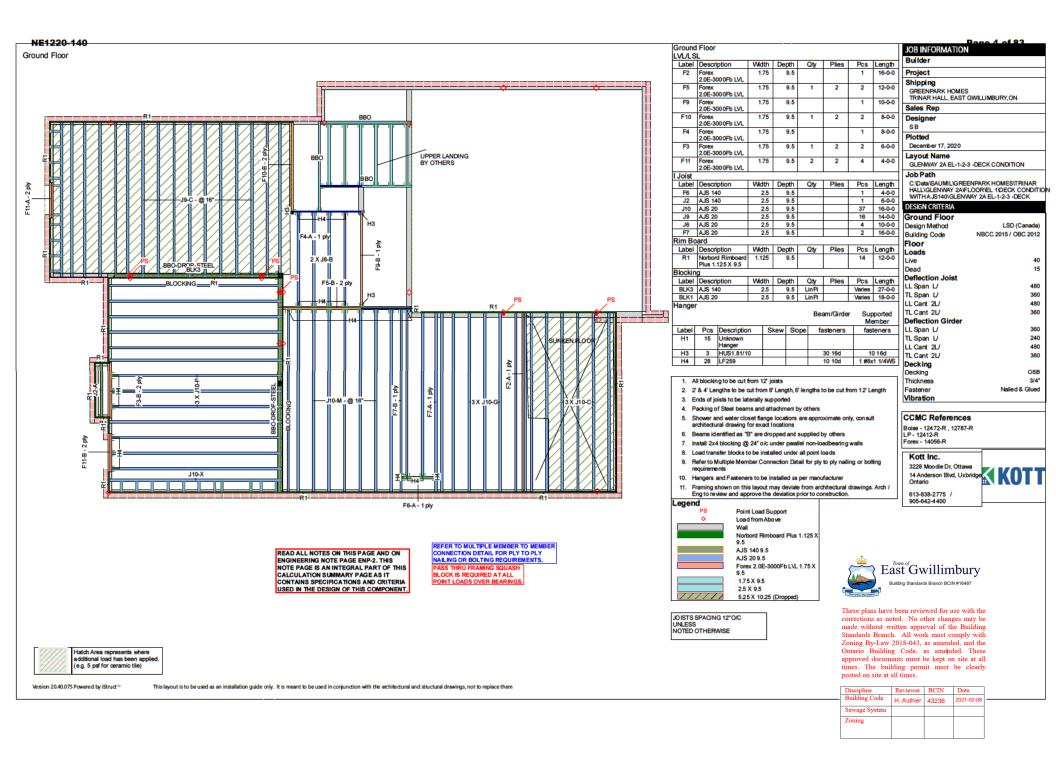
All connections are for uniformly distributed loads.

For multi-ply connections of I-joists, refer to Manufacturer's Installation Guide



KOTT Inc. 3228 Moodie Drive Ottawa, ON K2H 7V1 613-838-2775





NE1220-140 Page 5 of 83 Client: 12/17/2020 Date Input by: Project: SB isDesign Address: **GREENPARK HOMES** GLENWAY 2A EL-1-2-3 -DECK CONDITION Job Name: TRINAR HALL, EAST GWILLIMBURY, ON Project #: Level: Ground Floor Forex 2.0E-3000Fb LVL 1.750" X 9.500" F10-B 2-Ply - PASSED 6 2 3 11 1 SPF End Grain 2 SPE 7'11 3/4" 7'11 3/4' Unfactored Reactions UNPATTERNED lb (Uplift) Member Information Wind Type: Application: Floor (Residential) Brg Live Dead Plies Design Method: 2225 1275 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 556 n O 2 494 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1594 / 3338 1 - SPF 3.500" 4931 I 1 25D+1 5L Fnd Grain Analysis Results 1.25D+1.5L 2 - SPF 3.500" 19% 696 / 741 1437 L Comb. Actual Case **Analysis** Location Allowed Capacity 2'1 7/8" 22724 ft-lb 0.171 (17%) 1.25D+1.5L L Moment 3884 ft-lb Unbraced 3884 ft-lb 2'1 7/8" 21607 ft-lb 0.180 (18%) 1.25D+1.5L L READ ALL NOTES ON THIS PAGE AND ON AD PROFESSIONAL Shear 2149 lb 0.232 (23%) 1.25D+1.5L L **ENGINEERING NOTE PAGE ENP-2. THIS** 1' 1/4" 9277 lb NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.029 (L/3115) 3'9 7/16" 0.251 (L/360) 0.120 (12%) D Uniform CALCULATION SUMMARY PAGE AS IT LL Defl inch 0.034 (L/2662) 3'6 5/8" 0.251 (L/360) 0.140 (14%) L L CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. TL Defl inch 0.063 (L/1437) 3'7 15/16" 0.376 (L/240) 0.170 (17%) D+L L EL-MASRI REFER TO MULTIPLE MEMBER TO MEMBER Design Notes CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. 1 Girders are designed to be supported on the bottom edge only. PASS THRU FRAMING SQUASH 2 Multiple plies must be fastened together as per manufacturer's details. **BLOCK IS REQUIRED AT ALL** 3 Top loads must be supported equally by all plies. POINT LOADS OVER BEARINGS 4 Top braced at bearings. 5 Bottom braced at bearings 6 Lateral slenderness ratio based on full section width. ID Load Type Location Trib Width Side Dead Live Wind Comments Snow 0-0-0 to 7-10-10 15 PSF 40 PSF 0 PSF 1 0-6-0 Top 0 PSF 2 Part. Uniform 0-0-0 to 7-8-13 Top 2 PLF 0 PLF 0 PLF 0 PLF East Gwillimbury 3 Part. Uniform 0-0-0 to 7-10-3 82 PLF 0 PLF 0 PLF Wall Self We Top 0 PLF **Point** 0-2-12 Near Face 522 lb 1270 lb 0 lb 0 lb F4 Tie-In 0-3-10 to 7-11-12 15 PSF 40 PSF 0 PSF 0 PSF 1-2-6 Top These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the **Point** 2-0-12 385 lb 924 lb 0 lb 0 lb F10 F10 Top Continued on page 2... Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly osted on site at all tim Manufacturer Info Notes **Handling & Installation** Reviewer BCIN Date Forex Building Code structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended . LVL beams must not be cut or drilled . Refer to manufacturer's product regarding installation requiremen APA: PR-L318 H. Authier 43236 ewage System aged Beams r Daniaged beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corr This design is valid until 4/24/2023 Version 20.40.075 Powered by iStruct™ CSD DESIGN

NE1220-140 Page 6 of 83 Client: Date: 12/17/2020 Page 2 of 13 Project: Input by: isDesign Address: **GREENPARK HOMES** GLENWAY 2A EL-1-2-3 -DECK CONDITION Job Name: TRINAR HALL, EAST GWILLIMBURY, ON Project #: 1.750" X 9.500" Level: Ground Floor Forex 2.0E-3000Fb LVL F10-B 2-Ply - PASSED 6 5 2 3 11 1 1 SPF End Grain 2 SPF 7'11 3/4" 7'11 3/4' .Continued from page 1 ID **Load Type** Location Trib Width Side Dead Live Snow Wind Comments 41 PLF 0 PLF 0 PLF 0 PLF Wall Self Weight 7 Part. Uniform 7-10-3 to 7-11-3 Top 8 Part. Uniform 7-11-3 to 7-11-3 Top 9 PLF 6 PLF 21 PLF 0 PLF Part. Uniform 7-11-3 to 7-11-3 20 PLF 0 PLF 0 PLF 0 PLF Wall Self Weight Top Part. Uniform 7-11-3 to 7-11-12 82 PLF 0 PLF 0 PLF 0 PLF Wall Self Weight 11 Top Self Weight 8 PLF READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS. East Gwillimbury These plans have been reviewed for use with the corrections as noted. No other changes may be corrections as noted. No other changes may ob-made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amehded. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times. Manufacturer Info Notes Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design oriteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Handling & Installation Reviewer BCIN Date Forex Discipline Building Code LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requirement fastening details, beam strength value APA: PR-L318 H. Authier 43236 ewage System naged Beams must not be used Daniaged beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corre This design is valid until 4/24/2023

NE1220-140 Page 7 of 83 Page 3 of 13 Client: 12/17/2020 Date

Brg

1

2

isDesign Address:

Project:

GREENPARK HOMES

TRINAR HALL, EAST GWILLIMBURY, ON

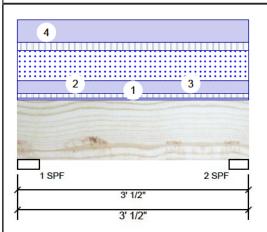
Input by: SB

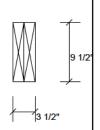
GLENWAY 2A EL-1-2-3 -DECK CONDITION Job Name:

Level: Ground Floor

Project #:

1.750" X 9.500" Forex 2.0E-3000Fb LVL 2-Ply - PASSED





Wind

0

0

Member Inforn	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2015 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load		22-12-17 (022-1-280-10-0-22)	
Floor Live:	40 PSF		
Dead:	15 DSF		

Bearings	s and Fact	ored Re	actions				
Bearing	Length	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.	_
1 - SPF	3.438"	8%	276 / 330	607	L	1.25D+1.5S +L	
2 - SPF	3.090"	9%	271 / 324	595	L	1.25D+1.5S	

Unfactored Reactions UNPATTERNED lb (Uplift)

Dead

221

217

Snow

169

166

Live

77

75

Analysis Results Capacity **Analysis** Actual Location Allowed Comb Case 1'6 7/16" 22497 ft-lb Moment 340 ft-lb 0.015 (2%) 1.25D+1.5S L +L Unbraced 340 ft-lb 1'6 7/16" 22497 ft-lb 0.015 (2%) 1.25D+1.5S L +L Shear 205 lb 1' 3/16" 9184 lb 0.022 (2%) 1.25D+1.5S L Perm Defl in. 0.001 1'6 7/16" 0.087 (L/360) 0.010 (1%) D Uniform (L/42770) LL Defl inch 0.001 1'6 7/16" 0.087 (L/360) 0.010 (1%) S+0.5L (L/45616) TL Defl inch 0.001 1'6 7/16" 0.131 (L/240) 0.010 (1%) D+S+0.5L L (L/22074)

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY

NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL**

POINT LOADS OVER BEARINGS.



+L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-0-8	0-5-2 to 0-5-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 3-0-8		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
3	Part. Uniform	0-0-0 to 3-0-8		Тор	47 PLF	33 PLF	110 PLF	0 PLF	



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Continued on page 2...

Notes Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corro
- **Handling & Installation**
- LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requirement naged Beams must not be used
- Daniaged beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

Forex APA: PR-L318

Manufacturer Info

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-08
Sewage System			
Zoning			

osted on site at all times.

This design is valid until 4/24/2023

NE1220-140 Page 8 of 83 Page 4 of 13 Client: Date: 12/17/2020 Project: Input by: isDesign Address: **GREENPARK HOMES** GLENWAY 2A EL-1-2-3 -DECK CONDITION Job Name: TRINAR HALL, EAST GWILLIMBURY, ON Project #: Level: Ground Floor 1.750" X 9.500" Forex 2.0E-3000Fb LVL 2-Ply - PASSED 4 2 3 1 1 SPF 2 SPF 3' 1/2' 3' 1/2" .Continued from page 1 ID **Load Type** Location Trib Width Side Dead Live Snow Wind Comments 82 PLF 0 PLF 0 PLF 0 PLF Wall Self Weight 4 Part. Uniform 0-0-0 to 3-0-8 Top Self Weight 8 PLF READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS. East Gwillimbury These plans have been reviewed for use with the corrections as noted. No other changes may be corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times. Manufacturer Info Notes Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orineria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Forex Handling & Installation Reviewer BCIN Date Discipline Building Code 1. IVI, beams must not be cut or drilled
2. Refer to manufacturer's product inforregarding installation requirements, in fastening details, beam strength values, an approvals
3. Damaged Beams must not be used APA: PR-L318 H. Authier 43236 ewage System

This design is valid until 4/24/2023

CSD DESIGN

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Daniaged beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

4. 5.

NE1220-140 Page 9 of 83 Page 5 of 13 Client: 12/17/2020 Date Project: Input by: SB isDesign Address: **GREENPARK HOMES** GLENWAY 2A EL-1-2-3 -DECK CONDITION Job Name: TRINAR HALL, EAST GWILLIMBURY, ON Project # Level: Ground Floor Forex 2.0E-3000Fb LVL 1.750" X 9.500" 2-Ply - PASSED 6 23 16 1 20 T 11 12 14 22 4 3 18 8 2 17 1 SPF 2 SPI 3'3 3/4' 3'3 3/4' Unfactored Reactions UNPATTERNED lb (Uplift) Member Information Wind Type: Application: Floor (Residential) Brg Live Dead Snow Plies Design Method: 1425 1278 1312 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 1488 O 2 1632 1542 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 3.375" 1598 / 3449 5047 I 1.25D+1.5L +S 2 - SPF 6.375" 43% 1860 / 3989 5848 L 1.25D+1.5L Analysis Results +S Actual Location Allowed Comb Case **Analysis** Capacity 2576 ft-lb Moment 1'7 5/16" 22724 ft-lb 0.113 (11%) 1.25D+1.5L L +S READ ALL NOTES ON THIS PAGE AND ON Unbraced 2576 ft-lb 1'7 5/16" 22724 ft-lb 0.113 (11%) 1.25D+1.5L L AD PROFESSIONAL **ENGINEERING NOTE PAGE ENP-2. THIS** +S NOTE PAGE IS AN INTEGRAL PART OF THIS Shear 1664 lb 2' 5/8" 9277 lb 0.179 (18%) 1.25D+1.5L L CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA Perm Defl in. 0.004 (L/8176) 1'7" 0.087 (L/360) 0.040 (4%) D Uniform USED IN THE DESIGN OF THIS COMPONENT. EL-MASRI REFER TO MULTIPLE MEMBER TO MEMBER LL Defl inch 0.006 (L/5118) 1'7 5/16" 0.087 (L/360) 0.070 (7%) L+0.5S ī CONNECTION DETAIL FOR PLY TO PLY TL Defl inch 0.010 (L/3148) 1'7 5/16" 0.131 (L/240) 0.080 (8%) D+L+0.5S NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH **Design Notes BLOCK IS REQUIRED AT ALL** 1 Girders are designed to be supported on the bottom edge only. POINT LOADS OVER BEARINGS 2 Multiple plies must be fastened together as per manufacturer's details. 3 Top loads must be supported equally by all plies. 4 Top braced at bearings. 5 Bottom braced at bearings. 6 Lateral slenderness ratio based on full section width. Dead ID Load Type Location Trib Width Side Live Snow Wind Comments Part, Uniform 0-0-0 to 0-11-5 Top 344 PLF 241 PLF 799 PLF 0 PLF East Gwillimbury 2 Part. Uniform 0-0-0 to 0-11-5 80 PI F 0 PLF 0 PLF 0 PLF Wall Self We Top 3 Tapered Start 0-0-0 Top 0 PLF 1 PLF 0 PLF 0 PLF These plans have been reviewed for use with the corrections as noted. No other changes may be End 0-4-5 0 PLF 1 PLF 0 PLF 0 PLF made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Part Uniform 116 PLF 236 PLF 0 PLF 0 PI F 0-0-0 to 0-0-5 Top Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times. Continued on page 2... Manufacturer Info Notes **Handling & Installation** Reviewer BCIN Date Forex Building Code structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended LVL beams must not be cut or drilled Refer to manufacturer's product regarding installation requiremen APA: PR-L318 H. Authier 43236

This design is valid until 4/24/2023

aged Beams m

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NE1220-140 Page 10 of 83 Page 6 of 13 Client: 12/17/2020 Date Project: Input by: SB isDesign **GREENPARK HOMES** GLENWAY 2A EL-1-2-3 -DECK CONDITION Address Job Name: TRINAR HALL, EAST GWILLIMBURY, ON Project # 1.750" X 9.500" Level: Ground Floor Forex 2.0E-3000Fb LVL 2-Ply - PASSED 6 23 READ ALL NOTES ON THIS PAGE AND ON 16 1 20 T 11 12 14 22 18 4 3 8 **ENGINEERING NOTE PAGE ENP-2. THIS** 2 NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. 17 9 REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. 9 1/2 PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL 1 SPF 2 SPI POINT LOADS OVER BEARINGS 3'3 3/4' 3'3 3/4' .Continued from page 1 Location Trib Width Side Dead Snow Wind Comments ID Load Type Live 297 PLF 0 PLF 5 Part. Uniform 0-0-0 to 0-11-5 Top 140 PLF 0 PLF 6 Part. Uniform 0-0-0 to 0-11-5 Top 82 PLF 0 PLF 0 PLF 0 PLF Wall Self Weight **Point** 0-3-5 206 lb 412 lb 89 lb 0 lb J10 Near Face 0 PLF 1 PLF 0 PLF 0 PLF 8 Tapered Start 0-4-5 Top 0-11-5 0 PLF 1 PLF 0 PLF 0 PLF 9 Part, Uniform 0-11-5 to 2-3-5 Top 344 PLF 241 PLF 799 PLF 0 PLF 80 PLF 10 Part. Uniform 0-11-5 to 2-3-5 Top 0 PI F 0 PI F 0 PLF Wall Self Weight 0-11-5 0 PLF 1 PLF 0 PLF 0 PLF 11 Tapered Start Top 0 PLF 0 PLF Fnd 1-4-5 1 PLF 0 PI F 12 Part. Uniform 0-11-5 to 1-10-5 Top 140 PLF 297 PLF 0 PLF 0 PLF 13 Part. Uniform 0-11-5 to 2-3-5 82 PLF 0 PLF 0 PLF 0 PLF Wall Self Weight Top 14 Tapered Start 1-4-5 Top 0 PLF 1 PLF 0 PLF 0 PLF 0 PLF **Fnd** 2-3-5 1 PLF 0 PI F 0 PLF 15 **Point** 1-7-5 Near Face 145 lb 386 lb 0 lb 0 lb J10 16 Part. Uniform 1-10-5 to 2-3-5 Top 144 PLF 298 PLF 0 PLF 0 PLF J4 17 Part. Uniform 2-3-5 to 3-3-12 Top 344 PLF 241 PLF 799 PLF 0 PLF 18 Part. Uniform 2-3-5 to 3-3-12 Top 80 PLF 0 PLF 0 PLF 0 PLF Wall Self Weight Tapered Start 0 PLF 19 2-3-5 Top 1 PLF 0 PLF 0 PLF End 2-4-5 0 PLF 1 PLF 0 PLF 0 PLF Part. Uniform 2-3-5 to 3-3-12 144 PLF 298 PLF 0 PLF 0 PLF 20 Top J4 21 Part. Uniform 2-3-5 to 3-3-12 Top 82 PLF 0 PLF 0 PLF 0 PLF Wall Self Weight 22 Tapered Start 0 PLF 0 PLF 0 PLF 2-4-5 1 PLF Top End 3-3-12 0 PLF 1 PLF 0 PLF 0 PLF 23 Point 2-11-5 Near Face 242 lb 466 lb 119 lb 0 lb J10 East Gwillimbury Self Weight 8 PLF These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly osted on site at all tin Manufacturer Info Notes **Handling & Installation** Reviewer BCIN Forex H. Authier 43236

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LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requirement fastening details, beam strength value.

Daniaged beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

This design is valid until 4/24/2023

APA: PR-L318

Building Code ewage System

NE1220-140 Page 11 of 83 Client: 12/17/2020 Page 7 of 13 Date Project: Input by: SB isDesign Address: **GREENPARK HOMES** GLENWAY 2A EL-1-2-3 -DECK CONDITION Job Name: TRINAR HALL, EAST GWILLIMBURY, ON Project # 1.750" X 9.500" - PASSED Level: Ground Floor Forex 2.0E-3000Fb LVL 1 1 SPF 2 SPF 15'4 3/4' 15'4 3/4' Unfactored Reactions UNPATTERNED lb (Uplift) Member Information Brg Live Dead Wind Type: Application: Floor (Residential) Snow Plies: 1 Design Method: 87 62 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 0 89 63 0 2 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Cap. React D/L lb Dead: Bearing Length Total Ld. Case Ld. Comb. 1 - SPF 2.375" 77 / 131 8% 208 1 1.25D+1.5L 2 - SPF 4.375" 5% 79 / 134 213 I 1.25D+1.5L Analysis Results Capacity **Analysis** Actual Location Allowed Comb. Case 0.067 (7%) 1.25D+1.5L L Moment 764 ft-lb 7'7 3/8" 11362 ft-lb Unbraced 764 ft-lb 7'7 3/8" 2492 ft-lb 0.307 (31%) 1.25D+1.5L L READ ALL NOTES ON THIS PAGE AND ON AD PROFESSIONAL Shear 183 lb 11 1/8" 4638 lb 0.039 (4%) 1.25D+1.5L L **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.038 (L/4712) 7'7 7/16" 0.499 (L/360) 0.080 (8%) D Uniform CALCULATION SUMMARY PAGE AS IT LL Defl inch 0.054 (L/3333) 7'7 7/16" 0.499 (L/360) 0.110 (11%) L L CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. TL Defl inch 0.092 (L/1952) 7'7 7/16" 0.748 (L/240) 0.120 (12%) D+L L EL-MASRI REFER TO MULTIPLE MEMBER TO MEMBER Design Notes CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. 1 Girders are designed to be supported on the bottom edge only. PASS THRU FRAMING SQUASH 2 Top braced at bearings. **BLOCK IS REQUIRED AT ALL** 3 Bottom braced at bearings. POINT LOADS OVER BEARING ID Load Type Location Trib Width Side Dead Snow Wind Comments 15 PSF 40 PSF 0 PSF 1 Tie-In 0-0-0 to 15-4-12 0-3-7 Top 0 PSF Self Weight 4 PLF East Gwillimbury These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all tim Manufacturer Info Notes **Handling & Installation** Reviewer BCIN Date Forex Building Code structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requirement APA: PR-L318 H. Authier 43236 ewage System

This design is valid until 4/24/2023

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naged Beams m

Daniaged beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

NE1220-140 Page 12 of 83 Client: 12/17/2020 Date Input by: Project: SB isDesign Address: **GREENPARK HOMES** GLENWAY 2A EL-1-2-3 -DECK CONDITION Job Name: TRINAR HALL, EAST GWILLIMBURY, ON Project #: Level: Ground Floor Forex 2.0E-3000Fb LVL 1.750" X 9.500" 2-Ply - PASSED 11 12 20 10 13 6 1 SPF 2 SPE 4'10 4'10' Unfactored Reactions UNPATTERNED lb (Uplift) Member Information Wind Type: Application: Floor (Residential) Brg Live Dead Snow Plies Design Method: 1904 1666 1910 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 1585 1750 O 2 1867 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 3.500" 2083 / 4769 6852 I 1.25D+1.5S +L 2 - SPF 3.500" 92% 1982 / 4550 6532 L 1.25D+1.5L Analysis Results +S Actual Location Allowed Comb Case **Analysis** Capacity Moment 1459 ft-lb 2'4 13/16" 22724 ft-lb 0.064 (6%) 1.25D+1.5L L +S READ ALL NOTES ON THIS PAGE AND ON Unbraced 1459 ft-lb 2'4 13/16" 22724 ft-lb 0.064 (6%) 1.25D+1.5L L AD PROFESSIONAL **ENGINEERING NOTE PAGE ENP-2. THIS** +S NOTE PAGE IS AN INTEGRAL PART OF THIS Shear 1255 lb 3'9 3/4" 9277 lb 0.135 (14%) 1.25D+1.5L L CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA Perm Defl in. 0.003 2'4 7/8" 0.146 (L/360) 0.020 (2%) D Uniform USED IN THE DESIGN OF THIS COMPONENT. EL-MASRI (L/17256) REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY LL Defl inch 0.008 (L/6980) 2'4 15/16" 0.146 (L/360) 0.050 (5%) L+0.5S NAILING OR BOLTING REQUIREMENTS. TL Defl inch 0.011 (L/4970) 2'4 7/8" 0.219 (L/240) 0.050 (5%) D+L+0.5S PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL Design Notes** POINT LOADS OVER BEARINGS. 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam 2 Girders are designed to be supported on the bottom edge only. 3 Multiple plies must be fastened together as per manufacturer's details. 4 Top loads must be supported equally by all plies. 5 Top braced at bearings. 6 Bottom braced at bearings 7 Lateral slenderness ratio based on full section width. East Gwillimbury Load Type Location Trib Width Side Dead Live Snow Wind Comments Part. Uniform 0-0-0 to 0-1-2 Top 86 PLF 60 PLF 200 PLF 0 PLF These plans have been reviewed for use with the Wall Self Wt corrections as noted. No other changes may be 2 Part. Uniform 0-0-0 to 0-1-2 Top 20 PLF 0 PLF 0 PLF 0 PLF made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Part. Uniform 0-0-0 to 0-1-2 72 PLF 149 PLF 0 PLF 0 PLF J4 Top Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly Continued on page 2... posted on site at all tim Manufacturer Info Notes Reviewer BCIN Date **Handling & Installation** Forex Building Code structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended . LVL beams must not be cut or drilled . Refer to manufacturer's product regarding installation requiremen APA: PR-L318 H. Authier 43236 ewage System aged Beams m Daniaged beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corr This design is valid until 4/24/2023 Version 20.40.075 Powered by iStruct™ CSD DESIGN

NE1220-140 Page 13 of 83 Page 9 of 13 Client: Date: 12/17/2020 Project: Input by: SB isDesign Address: **GREENPARK HOMES** GLENWAY 2A EL-1-2-3 -DECK CONDITION Job Name: TRINAR HALL, EAST GWILLIMBURY, ON Project # Level: Ground Floor Forex 2.0E-3000Fb LVL 1.750" X 9.500" F3-B 2-Ply - PASSED 11 12 20 10 13 5 4 6 1 SPF 2 SPF 4'10 4'10' .Continued from page 1 Location Trib Width Side Dead Live Snow Wind Comments ID Load Type 41 PLF 0 PLF 0 PLF Wall Self Weight 5 Part. Uniform 0-0-0 to 0-1-2 Top 0 PLF 6 Tie-In 0-1-2 to 4-8-14 Top 15 PSF 40 PSF 0 PSF 0 PSF Part. Uniform 0-1-2 to 0-5-8 172 PLF 120 PLF 400 PLF 0 PLF Top 0 PLF Part. Uniform 0-1-2 to 0-5-8 40 PLF 0 PLF 0 PLF Wall Self Weight 8 Top Part. Uniform 0-1-2 to 0-2-15 144 PLF 298 PLF 0 PLF 0 PLF J4 10 Top 82 PLF 0 PLF 0 PLF 0 PLF Wall Self Weight 11 Part. Uniform 0-1-2 to 0-5-8 Top 12 Point 0-2-12 Top 1261 lb 1160 lb 1748 lb 0 lb B3 B3 Part. Uniform 0-3-15 to 4-3-15 109 PLF 290 PLF 0 PLF 0 PLF 13 Near Face 14 **Point** 4-7-4 Top 11 lb 27 lb 0 lb 0 lb 15 **Point** 4-7-4 Top 13 lb 0 lb 0 lb 0 lb Wall Self Weight 16 Point 4-7-4 Top 1264 lb 1184 lb 1748 lb 0 lb B3 B3 17 **Point** 4-7-4 Top 9 lb 22 lb 0 lb 0 lb 18 **Point** 4-7-4 Top 10 lb 0 lb 0 lb 0 lb Wall Self Weight 19 Point 4-7-4 Top 16 lb 42 lb 0 lb 0 lb 20 **Point** 4-7-4 Top 20 lb 0 lb 0 lb 0 lb Wall Self Weight Self Weight 8 PLF READ ALL NOTES ON THIS PAGE AND ON REFER TO MULTIPLE MEMBER TO MEMBER ENGINEERING NOTE PAGE ENP-2. THIS CONNECTION DETAIL FOR PLY TO PLY NOTE PAGE IS AN INTEGRAL PART OF THIS NAILING OR BOLTING REQUIREMENTS. CALCULATION SUMMARY PAGE AS IT PASS THRU FRAMING SQUASH CONTAINS SPECIFICATIONS AND CRITERIA **BLOCK IS REQUIRED AT ALL** USED IN THE DESIGN OF THIS COMPONENT. POINT LOADS OVER BEARINGS. East Gwillimbury These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all tim Manufacturer Info Notes **Handling & Installation** Reviewer BCIN Date Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended Forex Building Code LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requirement fastening details, beam strength value APA: PR-L318 H. Authier 43236 ewage System aged Beams m Daniaged beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corr This design is valid until 4/24/2023

NE1220-140 Page 14 of 83 Client: 12/17/2020 Date Project: Input by: SB isDesign Address: **GREENPARK HOMES** GLENWAY 2A EL-1-2-3 -DECK CONDITION Job Name: TRINAR HALL, EAST GWILLIMBURY, ON Project # Level: Ground Floor Forex 2.0E-3000Fb LVL 1.750" X 9.500" - PASSED 6 2 3 1 Hanger (HUS1.81/10) 2 SPE 6' 15/16 6' 15/16' Unfactored Reactions UNPATTERNED lb (Uplift) Member Information Dead Wind Type: Application: Floor (Residential) Brg Live Snow Plies: Design Method: 1270 522 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 1055 437 n O 2 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 3.000" 66% 652 / 1905 2557 I 1.25D+1.5L Hanger Analysis Results 2 - SPF 5.500" 36% 546 / 1583 2129 L 1.25D+1.5L Comb. Case **Analysis** Actual Location Allowed Capacity 2'3" 11362 ft-lb 0.334 (33%) 1.25D+1.5L L Moment 3794 ft-lb Unbraced 3794 ft-lb 2'3" 6782 ft-lb 0.559 (56%) 1.25D+1.5L L READ ALL NOTES ON THIS PAGE AND ON ED PROFESSIONAL 2398 lb 4638 lb 0.517 (52%) 1.25D+1.5L L **ENGINEERING NOTE PAGE ENP-2. THIS** Shear 11 3/4" NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.021 (L/3160) 2'8 3/4" 0.183 (L/360) 0.110 (11%) D Uniform CALCULATION SUMMARY PAGE AS IT LL Defl inch 0.050 (L/1308) 2'9" 0.183 (L/360) 0.280 (28%) L L CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. TL Defl inch 0.071 (L/925) 2'8 7/8" 0.275 (L/240) 0.260 (26%) D+L L REFER TO MULTIPLE MEMBER TO MEMBER Design Notes CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS 1 Fill all hanger nailing holes. PASS THRU FRAMING SQUASH 2 Girders are designed to be supported on the bottom edge only. **BLOCK IS REQUIRED AT ALL** 3 Top braced at bearings. POINT LOADS OVER BEARINGS 4 Bottom braced at bearings ID Trib Width Side Dead Load Type Location Live Wind Comments Snow 1 Part. Uniform 0-0-0 to 5-10-3 Top 30 PLF 80 PLF 0 PLF 0 PLF 2 Point 0-5-4Near Face 92 lb 241 lb 0 lb 0 lb J6 Part. Uniform 1-1-4 to 3-9-4 Near Face 60 PLF 161 PLF 0 PLF 0 PLF 3 730 lb 0 lb F13 Point 2-3-0 327 lb 0 lb East Gwillimbury Tie-In 2-3-14 to 6-0-15 15 PSF 40 PSF 0 PSF 0 PSF Top 6 **Point** 4-5-4 Near Face 83 lb 222 lb 0 lb 0 lb J6 These plans have been reviewed for use with the corrections as noted. No other changes may be 7 **Point** 5-10-5 Near Face 36 lb 70 lb 0 lb 0 lb F9 made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Self Weight 4 PLF Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly osted on site at all tim Manufacturer Info Notes Reviewer BCIN Date **Handling & Installation** Forex Building Code structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended LVL beams must not be cut or drilled Refer to manufacturer's prod regarding installation requirem APA: PR-L318 H. Authier 43236 ewage System

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or cor

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This design is valid until 4/24/2023

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NE1220-140 Page 15 of 83 Client: 12/17/2020 Date Input by: Project: SB isDesign Address: **GREENPARK HOMES** GLENWAY 2A EL-1-2-3 -DECK CONDITION Job Name: TRINAR HALL, EAST GWILLIMBURY, ON Project #: Level: Ground Floor Forex 2.0E-3000Fb LVL 1.750" X 9.500" 2-Ply - PASSED F5-B 10 11 12 13 3 8 7 1 SPF 2 SPF 11'4 1/2' 11'4 1/2' Unfactored Reactions UNPATTERNED lb (Uplift) Member Information Brg Dead Wind Type: Application: Floor (Residential) Live Snow Plies Design Method: 2424 958 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 2653 1066 0 0 2 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 5.250" 1197 / 3636 4833 I 1 25D+1 5L 2 - SPF 4.375" 56% 1333 / 3980 5312 I 1.25D+1.5L Analysis Results Comb. Location Allowed **Analysis** Actual Capacity Case 5'8 5/8" 22724 ft-lb 0.600 (60%) 1.25D+1.5L L Moment 13624 ft-lb Unbraced 13624 ft-lb 5'8 5/8" 20464 ft-lb 0.666 (67%) 1.25D+1.5L L READ ALL NOTES ON THIS PAGE AND ON AD PROFESSIONAL Shear 4813 lb 0.519 (52%) 1.25D+1.5L L **ENGINEERING NOTE PAGE ENP-2. THIS** 10'3 3/8" 9277 lb NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.120 (L/1066) 5'8 11/16" 0.357 (L/360) 0.340 (34%) D Uniform CALCULATION SUMMARY PAGE AS IT LL Defl inch 0.304 (L/422) 5'8 11/16" 0.357 (L/360) 0.850 (85%) L L CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. TL Defl inch 0.425 (L/302) 5'8 11/16" 0.535 (L/240) 0.790 (79%) D+L L EL-MASRI REFER TO MULTIPLE MEMBER TO MEMBER Design Notes CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. 1 Girders are designed to be supported on the bottom edge only. PASS THRU FRAMING SQUASH 2 Multiple plies must be fastened together as per manufacturer's details. **BLOCK IS REQUIRED AT ALL** 3 Top loads must be supported equally by all plies. POINT LOADS OVER BEARINGS 4 Top braced at bearings 5 Bottom braced at bearings 6 Lateral slenderness ratio based on full section width. ID Load Type Location Trib Width Side Dead Live Wind Comments Snow 0 PSF 15 PSF 40 PSF 1 Tie-In 0-0-0 to 0-4-2 0 - 3 - 8Top 0 PSF 2 Tie-In 0-0-0 to 0-4-2 Top 15 PSF 40 PSF 0 PSF 0 PSF East Gwillimbury 3 **Point** 0-10-10 78 lb 208 lb 0 lb 0 lb J10 Near Face **Point** 1-8-10 Far Face 105 lb 278 lb 0 lb 0 lb J6 **Point** 127 lb 339 lb 0 lb J10 These plans have been reviewed for use with the corrections as noted. No other changes may be 1-8-10 Near Face 0 lb made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Part. Uniform 2-4-10 to 5-0-10 60 PLF 161 PLF 0 PLF 0 PLF Far Face Continued on page 2... Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly osted on site at all tim Manufacturer Info Notes **Handling & Installation** Reviewer BCIN Date Forex Building Code structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended . LVL beams must not be cut or drilled . Refer to manufacturer's product regarding installation requiremen APA: PR-L318 H. Authier 43236 ewage System aged Beams m Daniaged beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or conditions. This design is valid until 4/24/2023

NE1220-140 Page 16 of 83 Client: Date: 12/17/2020 Project: Input by: isDesign Address: **GREENPARK HOMES** GLENWAY 2A EL-1-2-3 -DECK CONDITION Job Name: TRINAR HALL, EAST GWILLIMBURY, ON Project #: Forex 2.0E-3000Fb LVL 1.750" X 9.500" Level: Ground Floor F5-B 2-Ply - PASSED 8 10 11 12 13 3 5 7 1 SPF 2 SPF 11'4 1/2' 11'4 1/2" .Continued from page 1 ID Location Trib Width Side Dead Live Snow Wind Comments Load Type 117 PLF 313 PLF 0 PLF 0 PLF 7 Part. Uniform 2-4-10 to 9-0-10 Near Face 222 lb 8 Point 5-8-10 Far Face 83 lb 0 lb 0 lb Part. Uniform 7-1-11 to 11-4-8 68 PLF 180 PLF 0 PLF 0 PLF Top 7-1-11 114 lb 0 lb F9 10 Point Far Face 58 lb 0 lb Point 9-8-10 Near Face 127 lb 339 lb 0 lb 0 lb J10 11 109 lb 239 lb 0 lb 0 lb 12 **Point** 10-6-10 Near Face 13 Point 11-0-10 Near Face 19 lb 51 lb 0 lb 0 lb J10 Self Weight 8 PLF READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS. East Gwillimbury These plans have been reviewed for use with the corrections as noted. No other changes may be corrections as noted. No other changes may ob-made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amehded. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times. Manufacturer Info Notes Forex Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Handling & Installation Reviewer BCIN Date Discipline Building Code LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requirement fastening details, beam strength value APA: PR-L318 H. Authier 43236 ewage System naged Beams must not be used Daniaged beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corre This design is valid until 4/24/2023

NE1220-140 Page 17 of 83 Client: 12/17/2020 Date Project: Input by: SB isDesign Address: **GREENPARK HOMES** GLENWAY 2A EL-1-2-3 -DECK CONDITION Job Name: TRINAR HALL, EAST GWILLIMBURY, ON Project #: 1.750" X 9.500" - PASSED Level: Ground Floor F9-B Forex 2.0E-3000Fb LVL 1 1 Hanger (HUS1.81/10) 2 Hanger (HUS1.81/10) 8' 7/16' 8' 7/16" Unfactored Reactions UNPATTERNED lb (Uplift) Member Information Brg Dead Wind Type: Application: Floor (Residential) Live Snow Plies: 1 Design Method: 58 0 0 114 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 58 0 0 2 114 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1_ 3.000" 6% 73 / 172 244 L 1.25D+1.5L Hanger Analysis Results 2 -3.000" 6% 73 / 172 244 L 1.25D+1.5L Hanger **Analysis** Actual Location Allowed Comb Case Capacity 446 ft-lb Moment 4' 3/16" 11362 ft-lb 0.039 (4%) 1.25D+1.5L L Unbraced 446 ft-lb 4' 3/16" 4867 ft-lb 0.092 (9%) 1.25D+1.5L L READ ALL NOTES ON THIS PAGE AND ON AD PROFESSIONAL Shear 185 lb 11 3/4" 4638 lb 0.040 (4%) 1.25D+1.5L L ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.005 4' 1/4" 0.255 (L/360) 0.020 (2%) D Uniform CALCULATION SUMMARY PAGE AS IT (L/17595) CONTAINS SPECIFICATIONS AND CRITERIA LL Defl inch 0.010 (L/8952) 4' 1/4" 0.255 (L/360) 0.040 (4%) L L USED IN THE DESIGN OF THIS COMPONENT. TL Defl inch 0.015 (L/5933) 4' 1/4" 0.383 (L/240) 0.040 (4%) D+L REFER TO MULTIPLE MEMBER TO MEMBER L CONNECTION DETAIL FOR PLY TO PLY **Design Notes** NAILING OR BOLTING REQUIREMENTS. 1 Fill all hanger nailing holes. PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** 2 Girders are designed to be supported on the bottom edge only. POINT LOADS OVER BEARINGS 3 Top braced at bearings. 4 Bottom braced at bearings. ID Load Type Location Trib Width Side Dead Live Wind Comments Snow 1 Tie-In 0-0-0 to 8-0-7 0-8-9 Top 15 PSF 40 PSF 0 PSF 0 PSF Self Weight 4 PLF East Gwillimbury These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all tim Manufacturer Info Notes **Handling & Installation** Reviewer BCIN Date Forex Building Code

This design is valid until 4/24/2023



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structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended

LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requiremen

Daniaged beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid

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BC CALC® Member Report

City, Province, Postal Code:

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Build 7364 Job name:

Address:

Customer:

GREENPARK HOMES

Description: Level - Ground Floor

TRINA...L, EAST GWILLIMBURY,ON

Specifier: SB

Code reports: CCMC 12787-R Designer: Company:

File name:

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/																														 }
B1															02-09-08															B2

Total Horizontal Product Length = 02-09-08

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 2"	392 / 0	150 / 0		
B2. 2"	278 / 0	107 / 0		

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	02-09-08	Тор		2			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	02-09-08	Тор	22	8			n\a
2	J10	Conc. Pt. (lbs)	L	00-04-12	00-04-12	Back	262	98			n\a
3	J10	Conc. Pt. (lbs)	L	01-08-12	01-08-12	Back	346	130			n\a

0 1 1 0		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	503 ft-lbs	4095 ft-lbs	12.3%	1	01-08-12
End Reaction	775 lbs	1588 lbs	48.8%	1	00-00-00
End Shear	767 lbs	1830 lbs	41.9%	1	00-02-00
Total Load Deflection	L/999 (0.008")	n\a	n\a	4	01-08-12
Live Load Deflection	L/999 (0.006")	n\a	n\a	5	01-08-12
Max Defl.	0.008"	n\a	n\a	4	01-08-12
Span / Depth	3.3				

Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Hanger	2" x 2-1/2"	775 lbs	39.8%	48.8%	LF259
B2	Hanger	2" x 2-1/2"	551 lbs	28.3%	34.7%	LF259



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PASSED

Cautions

Hanger LF259 requires (10) 10dx1.5 face nails, (1) #8x1.25 joist nails.

Header for the hanger LF259 is a Single 2-1/2" x 9-1/2" I-joist

Header for the hanger LF259 is a Single 2-1/2" x 9-1/2" I-joist



These plans have been reviewed for use with the corrections as noted. No other changes may be corrections as noted. No other changes may ob-made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amehded. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-08
Sewage System			
Zoning			

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS. READ ALL NOTES ON THIS PAGE AND ON



F6-A

Dry | 1 span | No cant.

BC CALC® Member Report Build 7364

C:\Data\SAUMIL\GREENPA... -DECK CONDITION .isl

December 17, 2020 12:23:48

Page 19 of 83

PASSED

Job name: Address:

GREENPARK HOMES

Description: Level - Ground Floor

TRINA...L, EAST GWILLIMBURY, ON

Specifier:

File name:

City, Province, Postal Code: Customer:

Designer: SB

READ ALL NOTES ON THIS PAGE AND ON

NOTE PAGE IS AN INTEGRAL PART OF THIS

USED IN THE DESIGN OF THIS COMPONENT.

ENGINEERING NOTE PAGE ENP-2. THIS

CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA

Code reports: CCMC 12787-R Company:

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Design meets User specified (0.72") Maximum live load deflection criteria.

Calculations assume member is fully braced. Hanger Manufacturer: Simpson Strong-Tie, Inc.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate

expert to ass anyone relyi evidence of

application.

building cod These plans have been reviewed for use with the Installation of Standards Brunch. All work must comply with engineered 1 Zoning By-Law 2018-043, as amended, and the accordance Guide and a times. The building posted on site at all times.

East Gwillimbury

questions, p Discipline Reviewer BCIN Date Building Code H. Authier 43236 2021-1 before instal-Sewage System

BC CALC®,

ALLJOIST®, BC KIM BOAKD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.



F7-A

Dry | 1 span | No cant.

Page 20 of 83 **PASSED**

December 17, 2020 12:23:48

BC CALC® Member Report

Build 7364 Job name:

Address:

GREENPARK HOMES

File name: Description: C:\Data\SAUMIL\GREENPA... -DECK CONDITION .isl

TRINA...L, EAST GWILLIMBURY, ON

Level - Ground Floor

City, Province, Postal Code:

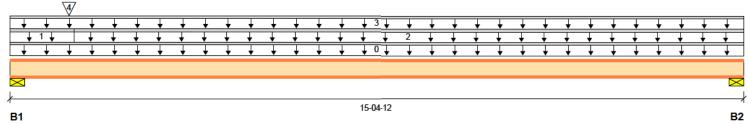
Specifier:

Customer:

Designer: SB

Code reports:

CCMC 12787-R Company:



Total Horizontal Product Length = 15-04-12

Reaction Summary (Down / Unlift) (lbs)

itcuction ou	reduction Summary (Bown r Spine) (185)										
Bearing	Live	Dead	Snow	Wind							
B1, 2-3/8"	559 / 0	231 / 0									
B2, 4-3/8"	282 / 0	125 / 0									

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	15-04-12	Тор		3			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	01-04-02	Тор	60	23			n\a
2		Unf. Lin. (lb/ft)	L	01-04-02	15-04-12	Тор	23	9			n\a
3		Unf. Lin. (lb/ft)	L	00-00-00	15-04-12	Тор	10	4			n\a
4	F6	Conc. Pt. (lbs)	L	01-02-14	01-02-14	Back	278	107			n∖a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	2268 ft-lbs	5675 ft-lbs	40.0%	1	06-11-13
End Reaction	1127 lbs	1653 lbs	68.2%	1	00-00-00
End Shear	1099 lbs	1830 lbs	60.1%	1	00-02-06
Total Load Deflection	L/577 (0.311")	n\a	41.6%	4	07-04-11
Live Load Deflection	L/830 (0.216")	n\a	43.4%	5	07-04-11
Max Defl.	0.311"	n\a	31.1%	4	07-04-11
Span / Depth	18.9				

Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	2-3/8" x 2-1/2"	1127 lbs	30.9%	68.2%	Spruce-Pine-Fir
B2	Wall/Plate	4-3/8" x 2-1/2"	579 lbs	8.6%	31.3%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Design meets User specified (0.72") Maximum live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate

expert to ass anyone relvi evidence of application.

East Gwillimbury

building cod These plans have been reviewed for use with the obtain Instal questions, p

before instal

properties at corrections as noted. No other changes may be Installation c made without written approval of the Building engineered \(\text{Zoning By-Law 2018-043}, \) as amended, and the accordance
Outaio Building Code, as amelided. These approved documents must be kept on site at all Guide and a times. The building permit must be clearly posted on site at all times.

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-08
iewage System			
Coning			

BC CALC®, ALLJOIST®, BURINI BUARD™, BUI®, BOISE GLULAM™, BC FloorValue®,

VERSA-LAM®, VERSA-RIM PLUS®,

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON



F7-B

Dry | 1 span | No cant.

Page 21 of 83 **PASSED**

BC CALC® Member Report

December 17, 2020 12:23:48

Build 7364 Job name:

Customer:

Address: **GREENPARK HOMES**

TRINA...L, EAST GWILLIMBURY,ON

C:\Data\SAUMIL\GREENPA... -DECK CONDITION .isl File name:

Description: Level - Ground Floor

City, Province, Postal Code:

Specifier:

SB Designer:

Code reports: CCMC 12787-R Company:

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\times																															
/																															
B1	15-08-01 B:								B2																						

Total Horizontal Product Length = 15-08-01

Reaction Summary (Down / Uplift) (lbs)

rtouotion ou	Redection Summary (Bown / Spinty (186)										
Bearing	Live	Dead	Snow	Wind							
B1, 2-3/8"	638 / 0	262 / 0									
B2, 2"	239 / 0	109 / 0									

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	15-08-01	Тор		3			00-00-00
1	_	Unf. Lin. (lb/ft)	L	00-00-00	15-08-01	Тор	17	6			n∖a
2		Unf. Lin. (lb/ft)	L	00-00-00	01-04-02	Тор	60	23			n∖a
3		Unf. Lin. (lb/ft)	L	01-04-02	15-08-01	Тор	10	4			n∖a
4	F6	Conc. Pt. (lbs)	L	01-02-14	01-02-14	Front	392	150			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	2153 ft-lbs	5675 ft-lbs	37.9%	1	06-09-05
End Reaction	1284 lbs	1653 lbs	77.7%	1	00-00-00
End Shear	1254 lbs	1830 lbs	68.5%	1	00-02-06
Total Load Deflection	L/589 (0.314")	n\a	40.7%	4	07-07-05
Live Load Deflection	L/852 (0.217")	n\a	42.2%	5	07-07-05
Max Defl.	0.314"	n\a	31.4%	4	07-07-05
Span / Depth	19.5				

Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	2-3/8" x 2-1/2"	1284 lbs	35.2%	77.7%	Spruce-Pine-Fir
B2	Hanger	2" x 2-1/2"	495 lbs	19.6%	31.2%	LF259



Cautions

Hanger LF259 requires (10) 10d face nails, (1) #8x1.25 joist nails. Header for the hanger LF259 is a Double 1-3/4" x 9-1/2" LVL beam



These plans have been reviewed for use with the corrections as noted. No other changes may be corrections as noted. No other changes may ob-made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amehded. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-08
Sewage System			
Zoning			

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



City, Province, Postal Code:

Single 9-1/2" AJS® 20

F7-B

December 17, 2020 12:23:48 Dry | 1 span | No cant.

Page 22 of 83

PASSED

BC CALC® Member Report Build 7364

Job name: Address:

C:\Data\SAUMIL\GREENPA... -DECK CONDITION .isl File name: Description: Level - Ground Floor

SB

GREENPARK HOMES

TRINA...L, EAST GWILLIMBURY, ON

Specifier:

Designer:

Code reports: CCMC 12787-R Company:

Notes

Customer:

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Design meets User specified (0.72") Maximum live load deflection criteria.

Calculations assume member is fully braced. Hanger Manufacturer: Simpson Strong-Tie, Inc.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a

qualified engineer or other appropriate expert to ass anyone relyi East Gwillimbury

evidence of application.

before instal-

building cod These plans have been reviewed for use with the Installation of Standards Brunch. All work must comply with engineered 1 Zoning By-Law 2018-043, as amended, and the accordance Guide and a times. The building posted on site at all times.

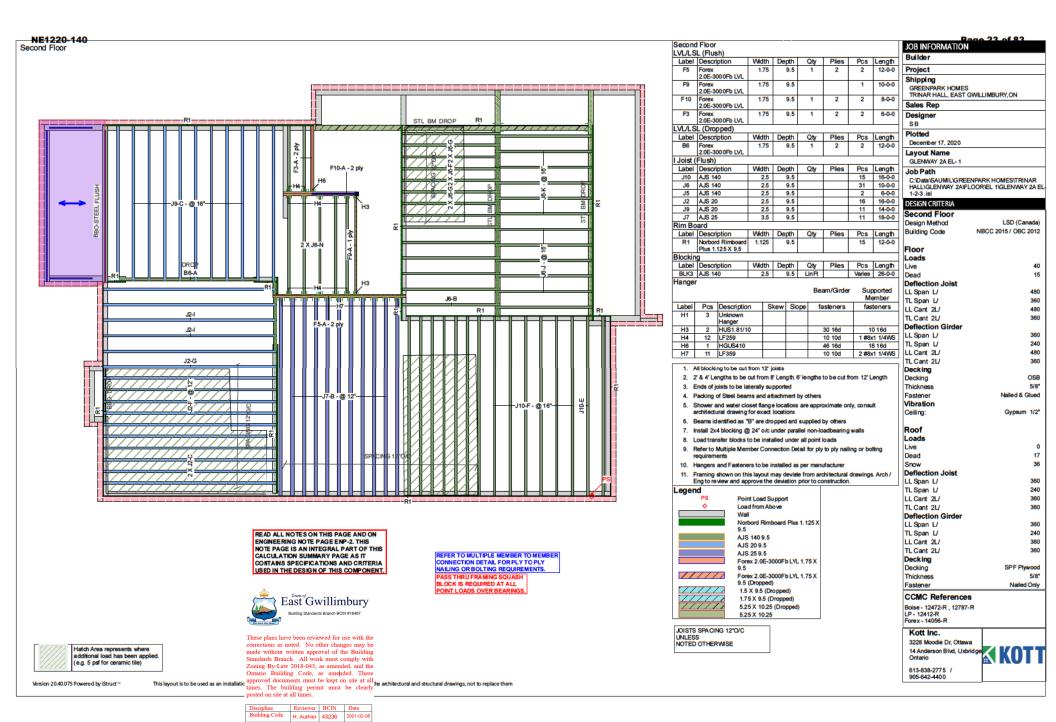
questions, p Discipline Reviewer BCIN Date Building Code H. Authier 43236 2021-1 Sewage System

BC CALC®, ALLJOIST®, BC KIM BOAKD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Sewage System

NE1220-140 Page 24 of 83 Client: 12/17/2020 Date Input by: Project: SB isDesign Address: **GREENPARK HOMES** Job Name: GLENWAY 2A EL- 1 TRINAR HALL, EAST GWILLIMBURY, ON Project #: Level: Second Floor Forex 2.0E-3000Fb LVL 1.750" X 9.500" **B6-A** 2-Ply - PASSED 5 4 3 6 1 SPF 2 SPF 11'6 5/16' 11'6 5/16" Unfactored Reactions UNPATTERNED lb (Uplift) Member Information Brg Dead Wind Type: Application: Floor (Residential) Live Snow Plies Design Method: 1561 625 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 590 0 0 2 1466 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.583" 56% 781 / 2341 3122 I 1 25D+1 5L 2 - SPF 3.750" 36% 737 / 2198 2936 I 1.25D+1.5L Analysis Results Comb. Actual Location Allowed **Analysis** Capacity Case 0.383 (38%) 1.25D+1.5L L Moment 8704 ft-lb 5'8 1/2" 22724 ft-lb Unbraced 8704 ft-lb 5'8 1/2" 20280 ft-lb 0.429 (43%) 1.25D+1.5L L READ ALL NOTES ON THIS PAGE AND ON AD PROFESSIONAL Shear 2694 lb 0.290 (29%) 1.25D+1.5L L ENGINEERING NOTE PAGE ENP-2. THIS 10'5 13/16" 9277 lb NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.083 (L/1600) 5'8 9/16" 0.371 (L/360) 0.230 (23%) D Uniform CALCULATION SUMMARY PAGE AS IT LL Defl inch 0.209 (L/639) 5'8 9/16" 0.371 (L/360) 0.560 (56%) L L CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. TL Defl inch 0.292 (L/457) 5'8 9/16" 0.556 (L/240) 0.530 (53%) D+L L EL-MASRI REFER TO MULTIPLE MEMBER TO MEMBER Design Notes CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. 1 Girders are designed to be supported on the bottom edge only. PASS THRU FRAMING SQUASH 2 Multiple plies must be fastened together as per manufacturer's details. **BLOCK IS REQUIRED AT ALL** 3 Top loads must be supported equally by all plies. POINT LOADS OVER BEARINGS 4 Top braced at bearings. 5 Bottom braced at bearings 6 Lateral slenderness ratio based on full section width. ID Load Type Location Trib Width Side Dead Live Wind Comments Snow Tapered Start 0 PLF 1 0-0-0 Top 1 PLF 2 PLF 0 PLF End 0-2-9 1 PLF 2 PLF 0 PLF 0 PLF East Gwillimbury 2 PLF 0 PLF 0 PLF 2 Tapered Start 0-0-0 Top 1 PLF End 0-2-9 1 PLF 2 PLF 0 PLF 0 PLF Tapered Start 0-2-9 6 PLF 17 PLF 0 PLF 0 PLF 3 Top These plans have been reviewed for use with the corrections as noted. No other changes may be End 11-2-9 6 PLF 17 PLF 0 PLF 0 PLF made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Tapered Start 0-2-9 Top 6 PLF **17 PLF** 0 PLF 0 PLF Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly Continued on page 2... posted on site at all tim Manufacturer Info Notes **Handling & Installation** Reviewer BCIN Date Forex Building Code APA: PR-L318 structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended . LVL beams must not be cut or drilled . Refer to manufacturer's product regarding installation requiremen H. Authier 43236 ewage System aged Beams n Daniaged beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid

lateral displacement and rotation Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or con This design is valid until 4/24/2023

CSD DESIGN

Page 25 of 83 NE1220-140 Client: Date: 12/17/2020 Project: Input by: isDesign Address: **GREENPARK HOMES** Job Name: GLENWAY 2A EL- 1 TRINAR HALL, EAST GWILLIMBURY, ON Project #: 1.750" X 9.500" Level: Second Floor Forex 2.0E-3000Fb LVL B6-A 2-Ply - PASSED 5 4 3 6 _____1 SPF 2 SPF 11'6 5/16" 11'6 5/16" .Continued from page 1 ID Location Trib Width Side Dead Live Snow Wind Comments Load Type 6 PLF 17 PLF 0 PLF 0 PLF End 0-10-9 265 PLF 0 PLF 0 PLF 5 Part. Uniform 0-2-9 to 10-10-9 Top 99 PLF 6 0 PLF Tapered Start 11-2-9 Top 1 PLF 2 PLF 0 PLF 0 PLF End 11-6-5 1 PLF 2 PLF 0 PLF Self Weight 8 PLF READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS. East Gwillimbury These plans have been reviewed for use with the corrections as noted. No other changes may be corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times. Manufacturer Info Notes Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orineria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Forex Handling & Installation Reviewer BCIN Date Discipline Building Code LVL beams must not be cut or drilled
 Refer to manufacturer's product info regarding installation requirements, r fastening details, beam strength values, an approvals APA: PR-L318 H. Authier 43236 ewage System maged Beams must not be used Daniaged beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corro

This design is valid until 4/24/2023

NE1220-140 Page 26 of 83 Client: 12/17/2020 Date Project: Input by: SB isDesign Address: **GREENPARK HOMES** Job Name: GLENWAY 2A EL- 1 TRINAR HALL, EAST GWILLIMBURY, ON Project #: Level: Second Floor Forex 2.0E-3000Fb LVL 1.750" X 9.500" 2-Ply - PASSED 2 5 6 8 1 1 SPF 2 SPF End Grain 6'5 7/16' 6'5 7/16' Unfactored Reactions UNPATTERNED lb (Uplift) Member Information Dead Wind Type: Application: Floor (Residential) Brg Live Snow Plies Design Method: 924 385 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 1089 455 n O 2 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 5.500" 16% 481 / 1386 1867 I 1 25D+1 5L 2 - SPF 3.465" 24% 569 / 1633 2202 I 1.25D+1.5L Analysis Results End Grain Comb. Actual Location Allowed Case **Analysis** Capacity 0.098 (10%) 1.25D+1.5L L Moment 2234 ft-lb 3'2 7/16" 22724 ft-lb Unbraced 2234 ft-lb 3'2 7/16" 22052 ft-lb 0.101 (10%) 1.25D+1.5L L READ ALL NOTES ON THIS PAGE AND ON AD PROFESSIONAL Shear 1538 lb 0.166 (17%) 1.25D+1.5L L **ENGINEERING NOTE PAGE ENP-2. THIS** 5'5 1/4" 9277 lb NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.007 (L/9538) 3'3 3/16" 0.194 (L/360) 0.040 (4%) D Uniform CALCULATION SUMMARY PAGE AS IT LL Defl inch 0.017 (L/4042) 3'3 7/16" 0.194 (L/360) 0.090 (9%) L L CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. TL Defl inch 0.025 (L/2839) 3'3 5/16" 0.292 (L/240) 0.080 (8%) D+L L EL-MASRI REFER TO MULTIPLE MEMBER TO MEMBER Design Notes CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. 1 Girders are designed to be supported on the bottom edge only. PASS THRU FRAMING SQUASH 2 Multiple plies must be fastened together as per manufacturer's details. **BLOCK IS REQUIRED AT ALL** 3 Top loads must be supported equally by all plies. POINT LOADS OVER BEARINGS 4 Top braced at bearings 5 Bottom braced at bearings 6 Lateral slenderness ratio based on full section width. ID Load Type Location Trib Width Side Dead Live Wind Comments Snow 0-0-0 to 3-9-12 66 PLF 177 PLF 1 Part Uniform Near Face 0 PLF 0 PLF 2 Point 0-5-12 Far Face 55 lb 146 lb 0 lb 0 lb East Gwillimbury 3 1-9-12 Far Face 41 lb 110 lb 0 lb 0 lb J5 Point Part. Uniform 2-5-12 to 6-3-12 Top 30 PLF 80 PLF 0 PLF 0 PLF **Point** 2-5-12 Far Face 36 lb 38 lb These plans have been reviewed for use with the corrections as noted. No other changes may be 0 lb 0 lb F3 made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the **Point** 4-5-12 Near Face 74 lb 197 lb 0 lb 0 lb J6 Continued on page 2... Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly osted on site at all tim Manufacturer Info Notes **Handling & Installation** Reviewer BCIN Date Forex Building Code structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended . LVL beams must not be cut or drilled . Refer to manufacturer's product regarding installation requiremen APA: PR-L318 H. Authier 43236 ewage System Daniaged beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid

lateral displacement and rotation Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or con This design is valid until 4/24/2023 Version 20.40.075 Powered by iStruct™

NE1220-140 Page 27 of 83 Client: Date: 12/17/2020 Project: Input by: isDesign Address: **GREENPARK HOMES** Job Name: GLENWAY 2A EL- 1 TRINAR HALL, EAST GWILLIMBURY, ON Project #: Level: Second Floor 1.750" X 9.500" Forex 2.0E-3000Fb LVL 2-Ply - PASSED 2 3 5 6 8 1 1 SPF 2 SPF End Grain 6'5 7/16" 6'5 7/16" .Continued from page 1 ID Location Trib Width Side Dead Live Snow Wind Comments Load Type 148 lb 0 lb 7 **Point** 5-5-12 Near Face 55 lb 0 lb J6 8 **Point** 6-2-11 Near Face 163 lb 392 lb 0 lb 0 lb F9 Self Weight 8 PLF READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS. East Gwillimbury These plans have been reviewed for use with the corrections as noted. No other changes may be corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times. Manufacturer Info Notes Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orineria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Forex Handling & Installation Reviewer BCIN Date Discipline Building Code LVL beams must not be cut or drilled
 Refer to manufacturer's product info regarding installation requirements, r fastening details, beam strength values, an approvals APA: PR-L318 H. Authier 43236 ewage System approvals

Damaged Beams must not be used Daniaged beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive This design is valid until 4/24/2023 Version 20.40.075 Powered by iStruct™

NE1220-140 Page 28 of 83 Client: Date: 12/17/2020 Project: Input by: SB isDesign Address: **GREENPARK HOMES** Job Name: GLENWAY 2A EL- 1 TRINAR HALL, EAST GWILLIMBURY, ON Project #: Level: Second Floor Forex 2.0E-3000Fb LVL 1.750" X 9.500" 2-Ply - PASSED 1 1 Hanger (HGUS410) 2 SPE 5'8 1/8' 5'8 1/8' Member Information Unfactored Reactions UNPATTERNED lb (Uplift) Dead Snow Wind Type: Application: Floor (Residential) Brg Live Plies Design Method: 38 36 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 38 36 n O 2 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Cap. React D/L lb Dead: Bearing Length Total Ld. Case Ld. Comb. 1.25D+1.5L 4.000" 1% 45 / 56 101 I Hanger Analysis Results 2 - SPF 4.375" 1% 45 / 57 102 L 1.25D+1.5L Actual Location Allowed **Analysis** Capacity Comb Case 2'9 7/8" 22724 ft-lb Moment 117 ft-lb 0.005 (1%) 1.25D+1.5L L Unbraced 117 ft-lb 2'9 7/8" 22724 ft-lb 0.005 (1%) 1.25D+1.5L L READ ALL NOTES ON THIS PAGE AND ON Shear 63 lb 4'7" 9277 lb 0.007 (1%) 1.25D+1.5L L ENGINEERING NOTE PAGE ENP-2. THIS PROFESSIONA NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) CALCULATION SUMMARY PAGE AS IT LL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. TL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) REFER TO MULTIPLE MEMBER TO MEMBER Design Notes CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. 1 Fill all hanger nailing holes. PASS THRU FRAMING SQUASH 2 Girders are designed to be supported on the bottom edge only. **BLOCK IS REQUIRED AT ALL** 3 Multiple plies must be fastened together as per manufacturer's details. POINT LOADS OVER BEARINGS 4 Top loads must be supported equally by all plies. 5 Top braced at bearings. 6 Bottom braced at bearings. 7 Lateral slenderness ratio based on full section width. ID Load Type Location Trib Width Side Dead Live Snow Wind Comments 1 0-0-0 to 5-8-2 0-4-0 15 PSF 40 PSF 0 PSF 0 PSF Tie-In Top East Gwillimbury Self Weight 8 PLF These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times. Manufacturer Info Notes **Handling & Installation** Reviewer BCIN Date Forex Building Code structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended . LVL beams must not be cut or drilled . Refer to manufacturer's product regarding installation requiremen APA: PR-L318 H. Authier 43236 ewage System

This design is valid until 4/24/2023

CSD DESIGN

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corr

aged Beams m

Daniaged beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid

lateral displacement and rotation

NE1220-140 Page 29 of 83 Client: Date: 12/17/2020 Input by: Project: SB isDesign Address: **GREENPARK HOMES** Job Name: GLENWAY 2A EL- 1 TRINAR HALL, EAST GWILLIMBURY, ON Project #: 1.750" X 9.500" Level: Second Floor Forex 2.0E-3000Fb LVL 2-Ply - PASSED 11 6 13 5 1 SPF 2 SPF 11'4 3/4' 11'5 5/8' Member Information Unfactored Reactions UNPATTERNED lb (Uplift) Brg Dead Wind Type: Application: Floor (Residential) Live Snow Plies Design Method: 2556 1007 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 2427 n O 2 964 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 4.500" 53% 1259 / 3833 5092 II 1 25D+1 5L 2 - SPF 5.500" 41% 1205 / 3640 4845 1.25D+1.5L Analysis Results Actual Comb. **Analysis** Location Allowed Capacity Case 14605 ft-lb Moment 5'6 7/8" 22724 ft-lb 0.643 (64%) 1.25D+1.5L _L Unbraced 14605 ft-lb 5'6 7/8" 20415 ft-lb 0.715 (72%) 1.25D+1.5L READ ALL NOTES ON THIS PAGE AND ON AD PROFESSIONAL Shear 5063 lb 0.546 (55%) 1.25D+1.5L LL ENGINEERING NOTE PAGE ENP-2. THIS 1' 5/8" 9277 lb NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.130 (L/996) 5'7 7/8" 0.360 (L/360) 0.360 (36%) D Uniform CALCULATION SUMMARY PAGE AS IT LL Defl inch 0.330 (L/393) 5'7 3/4" 0.360 (L/360) 0.920 (92%) L _L CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. TL Defl inch 0.461 (L/282) 5'7 13/16" 0.541 (L/240) 0.850 (85%) D+L _L EL-MASRI REFER TO MULTIPLE MEMBER TO MEMBER LL Cant -0.007 Lt Cant 0.200 0.033 (3%) L _L CONNECTION DETAIL FOR PLY TO PLY (2L/266)(2L/480) NAILING OR BOLTING REQUIREMENTS. TL Cant -0.009 Lt Cant 0.300 0.031 (3%) D+L _L PASS THRU FRAMING SQUASH (2L/191) (2L/360) **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS. **Design Notes** 1 Girders are designed to be supported on the bottom edge only. 2 Multiple plies must be fastened together as per manufacturer's details. 3 Top loads must be supported equally by all plies. 4 Top braced at bearings. 5 Bottom braced at bearings. 6 Lateral slenderness ratio based on full section width. 7 Cantilever Upward Deflection Live Load 0.0065849 greater than recommended 0.004 East Gwillimbury 8 Cantilever Upward Deflection Total Load 0.009178 greater than recommended 0.005 These plans have been reviewed for use with the corrections as noted. No other changes may be

made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly osted on site at all tim

Reviewer BCIN Date

H. Authier 43236 ewage System

Notes

structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or con
- **Handling & Installation**
- . LVL beams must not be cut or drilled . Refer to manufacturer's product regarding installation requiremen aged Beams m
- Daniaged beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid

 lateral displacement and rotation

Forex APA: PR-L318

Manufacturer Info

Building Code

Page 30 of 83 NE1220-140 Client: Date: 12/17/2020 Project: Input by: SB isDesign Address: **GREENPARK HOMES** Job Name: GLENWAY 2A EL- 1 TRINAR HALL, EAST GWILLIMBURY, ON Project #: Level: Second Floor Forex 2.0E-3000Fb LVL 1.750" X 9.500" F5-A 2-Ply - PASSED 11 6 8 13 5 1 SPF 2 SPF 11'4 3/4' 11'5 5/8' ID Load Type Location Trib Width Side Dead Live Snow Wind Comments 40 PSF 15 PSF 0 PSF 0 PSF 1 Tie-In 0-0-0 to 0-2-10 0 - 3 - 8Top 2 Tie-In 0-0-0 to 0-4-6 0 - 4 - 8Top 15 PSF 40 PSF 0 PSF 0 PSF 40 PSF 0 PSF 3 0-2-10 to 0-4-13 15 PSF 0 PSF Tie-In 0-4-8 to 0-1-2 Top 4 **Point** 0-8-10 Near Face 84 lb 224 lh 0 lb 0 lb J7 336 PLF 0 PI F 0 PLF 5 Part. Uniform 1-2-10 to 10-2-10 Near Face 126 PLF 6 Point 1-4-10 Far Face 81 lb 215 lb 0 lb 0 lb J6 7 0 PLF 0 PLF 2-0-10 to 4-8-10 63 PLF 169 PLF Part. Uniform Far Face 8 Point 5-4-10 Far Face 74 lb 197 lb 0 lb 0 lb J6 q Point 6-4-10 Far Face 55 lb 148 lb 0 lb 0 lb J6 10 Point 7-1-9 Far Face 163 lb 392 lb 0 lb 0 lb F9 11 117 lb 311 lb 0 lb Point 10-8-10 Near Face 0 lb J7 11-0-2 to 11-5-10 0-3-8 15 PSF 40 PSF 0 PSF 0 PSF 12 Tie-In Top 13 11-1-5 to 11-5-10 0-4-8 15 PSF 40 PSF 0 PSF 0 PSF Tie-In Top Self Weight 8 PLF REFER TO MULTIPLE MEMBER TO MEMBER READ ALL NOTES ON THIS PAGE AND ON CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** CALCULATION SUMMARY PAGE AS IT POINT LOADS OVER BEARINGS CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. East Gwillimbury



These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Reviewer BCIN Discipline Building Code H. Authier 43236 ewage System

Notes

structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corre

Handling & Installation

aged Beams m

LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requirement fastening details, beam strength value

Daniaged beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

This design is valid until 4/24/2023

Forex APA: PR-L318

Manufacturer Info

This design is valid until 4/24/2023

APA: PR-L318

H. Authier 43236

ewage System

CSD DESIGN

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corr

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Daniaged beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid

lateral displacement and rotation

aged Beams n

NE1220-140 Page 32 of 83 Client: Date: 12/17/2020 Project: Input by: isDesign Address: **GREENPARK HOMES** Job Name: GLENWAY 2A EL- 1 TRINAR HALL, EAST GWILLIMBURY, ON Project #: Level: Second Floor 1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL F9-A 2 3 4 5 6 7 8 9 1 1 Hanger (HUS1.81/10) 2 Hanger (HUS1.81/10) 8'5 1/2' 8'5 1/2" .Continued from page 1 ID Location Trib Width Side Dead Live Wind Comments Load Type Snow 9 15 PSF 40 PSF 0 PSF 0 PSF Tie-In 7-9-3 to 8-5-8 1-11-9 Top Self Weight 4 PLF READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS. East Gwillimbury These plans have been reviewed for use with the corrections as noted. No other changes may be corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

This design is valid until 4/24/2023

CSD DESIGN

Sewage System

Discipline Reviewer BCIN Date
Building Code H. Authier 42226 2224

Manufacturer Info

APA: PR-L318

Forex

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orineria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

4. 5.

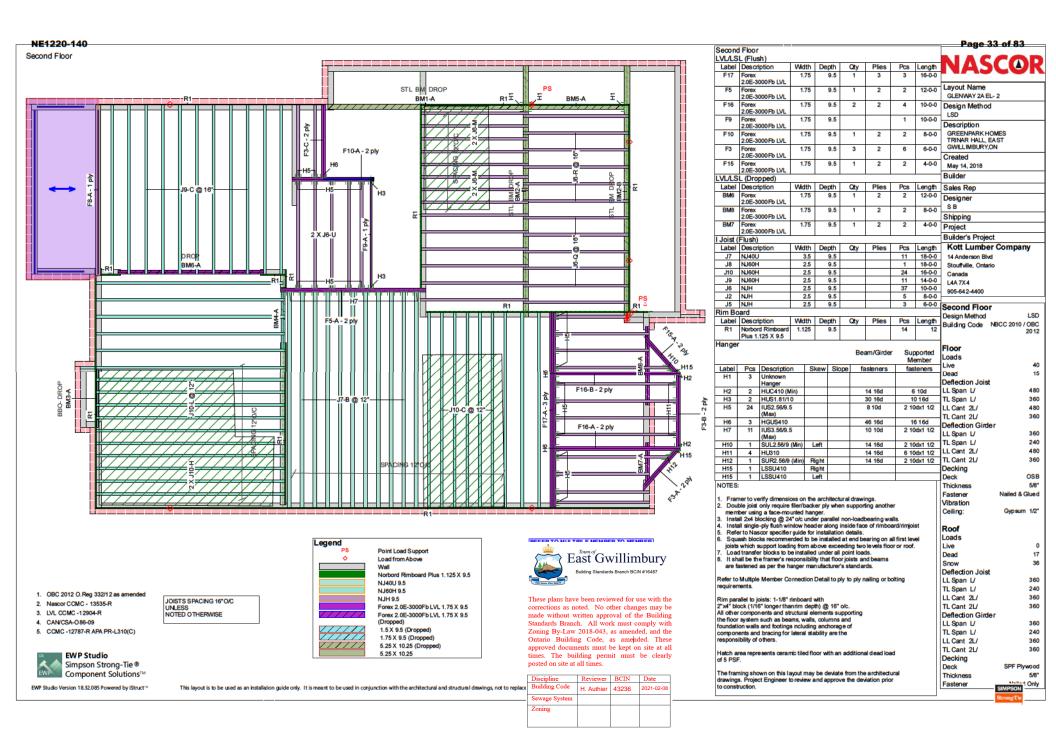
LVL beams must not be cut or drilled
 Refer to manufacturer's product infor regarding installation requirements, in fastening details, beam strength values, an

Daniaged beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

approvals

Damaged Beams must not be used

Notes



NE1220-140 Page 34 of 83 Client: Date: 5/15/2018 **EWP Studio** Project: Designer: SB Simpson Strong-Tie® Address Job Name: GLENWAY 2A EL- 2 Component Solutions™ Project #: Level: Second Floor 1.750" X 9.500" BM6-A Forex 2.0E-3000Fb LVL 2-Ply - PASSED 1 1 SPF 2 SPF 11'6 5/16' 11'6 5/16" Member Information Unfactored Reactions UNPATTERNED lb (Uplift) Application: Brg Dead Wind Type: Floor (Residential) Live Snow Plies: Design Method: 1456 587 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 0 1371 556 0 2 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.583" 52% 734 / 2183 2918 I 1.25D+1.5L 2 - SPF 3.750" 34% 696 / 2057 2752 I 1.25D+1.5L Analysis Results Actual Location Allowed Comb. **Analysis** Capacity Case 5'8 1/2" 22724 ft-lb 0.360 (36%) 1.25D+1.5L L Moment 8190 ft-lb 20280 ft-lb Unbraced 8190 ft-lb 5'8 1/2" 0.404 (40%) 1.25D+1.5L L READ ALL NOTES ON THIS PAGE AND ON Shear 2536 lb 0.273 (27%) 1.25D+1.5L L **ENGINEERING NOTE PAGE ENP-2. THIS** 10'5 13/16" 9277 lb ROFESSIONA NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.079 (L/1691) 5'8 9/16" 0.371 (L/360) 0.210 (21%) D Uniform CALCULATION SUMMARY PAGE AS IT LL Defl inch 0.196 (L/680) 5'8 9/16" 0.371 (L/360) 0.530 (53%) L L CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. TL Defl inch 0.275 (L/485) 5'8 9/16" 0.556 (L/240) 0.490 (49%) D+L L REFER TO MULTIPLE MEMBER TO MEMBER Design Notes CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. 1 Girders are designed to be supported on the bottom edge only. PASS THRU FRAMING SQUASH 2 Multiple plies must be fastened together as per manufacturer's details. **BLOCK IS REQUIRED AT ALL** 3 Top loads must be supported equally by all plies. POINT LOADS OVER BEARINGS 4 Top braced at bearings 5 Bottom braced at bearings 6 Lateral slenderness ratio based on full section width. ID Load Type Location Trib Width Side Dead Live Wind Comments Snow 0-2-9 to 10-10-9 99 PLF 265 PLF 0 PLF 0 PLF 1 Part. Uniform Top Self Weight 8 PLF East Gwillimbury These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all tim Manufacturer Info Notes Reviewer BCIN Date **Handling & Installation** Forex Building Code structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended LVL beams must not be cut or drilled.
 Refer to manufacturer's prod-regarding installation requirem APA: PR-L318 H. Authier 43236 ewage System Daniaged beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid

lateral displacement and rotation Dry service conditions, unless noted otherwise.
 LVL not to be treated with fire retardant or conditions.

Simpson Strong-Tie® Component Solutions™

when Information

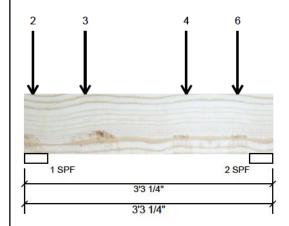
Project: Address Designer: SB

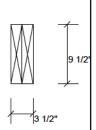
Job Name: GLENWAY 2A EL- 2

Project #

1.750" X 9.500" 2-Ply - PASSED BM7-A Forex 2.0E-3000Fb LVL

Level: Second Floor





Member Intorn	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load		SAN ANY LINCOLOGIC SAN DE	
Floor Live:	40 PSF		

Brg	Live	Dead	Snow	Wind
1	333 (-18)	309	162	0
2	549 (-75)	839	327	0
	1	1 333 (-18)	1 333 (-18) 309	1 333 (-18) 309 162

Unfactored Reactions UNPATTERNED lb (Uplift)

Analysis Results **Analysis** Actual Location Allowed Comb Case Capacity 2'1 9/16" 21360 ft-lb 0.030 (3%) 1.25D+1.5L L Moment 636 ft-lb 2'1 9/16" 21360 ft-lb Unbraced 636 ft-lb 0.030 (3%) 1.25D+1.5L L Shear 0.087 (9%) 1.25D+1.5L L 757 lb 2'2 7/8" 8720 lb Perm Defl in. 0.001 2'1 5/8" 0.093 (L/360) 0.010 (1%) D Uniform (L/28176) LL Defl inch 0.002 2' 11/16" 0.093 (L/360) 0.020 (2%) L+0.5S (L/20553)

Bearings and Factored Reactions Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 3.625" 1.25D+1.5L 386 / 581 966 I +0.5S 1.25D+1.5L 2 - SPF 3.625" 30% 1049 / 987 2036 L +0.5S

Design Notes

TL Defl inch 0.003

Dead:

1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.

2'1 5/16" 0.139 (L/240) 0.020 (2%) D+L+0.5S

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.

(L/11892)

15 PSF

- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

APA: PR-L318

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



/ Lateral	sienderness ratio based on	full section width.								_
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Commen	ts 🚡
1	Point	0-1-5		Тор	139 lb	57 lb	134 lb	0 lb	F3	
2	Point	0-1-5		Тор	0 lb	-12 lb	0 lb	0 lb	F3	Thes
3	Point	0-9-9		Тор	75 lb	201 lb	0 lb	0 lb	J2	mad Stan
4	Point	2-1-9		Тор	101 lb	270 lb	0 lb	0 lb	J6	Zoni
Continued or	n page 2									appr



nese plans have been reviewed for use with the rrections as noted. No other changes may be corrections as notes. No other canages may ob-made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as ameladed. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times. ontinued on page 2... Manufacturer Info

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-08
Sewage System			
Zoning			
M LA	76	LAJ	

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to warfity the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corre

Handling & Installation

LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requirement

naged Beams must not be used

Daniaged beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

NE1220-140 Page 36 of 83 Client: Date: 5/15/2018 **EWP Studio** Project: Designer: SB Simpson Strong-Tie® Address: Job Name: GLENWAY 2A EL- 2 Component Solutions™ Project # Forex 2.0E-3000Fb LVL 1.750" X 9.500" 2-Ply - PASSED Level: Second Floor 2 3 6 1 SPF 2 SPF 3'3 1/4" 3'3 1/4" .Continued from page 1 ID Location Trib Width Side Dead Live Snow Wind Comments Load Type Top 808 lb 354 lb 355 lb F16 5 **Point** 2-9-11 0 lb 6 **Point** 2-9-11 Top 0 lb -81 lb 0 lb 0 lb F16 Self Weight 8 PLF

> READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

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Reviewer BCIN Date Discipline Building Code H. Authier 43236 Sewage System

Notes

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Dry service conditions, unless noted otherwise
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Handling & Installation

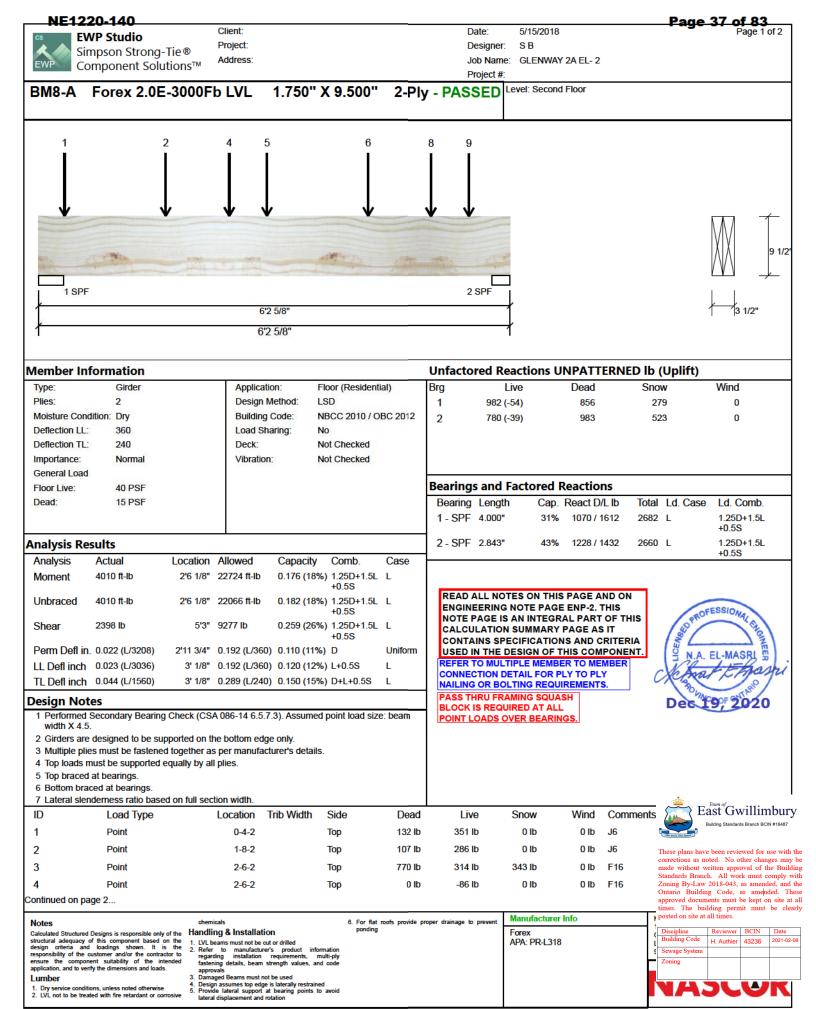
LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requirement fastening details, beam strength value approvals

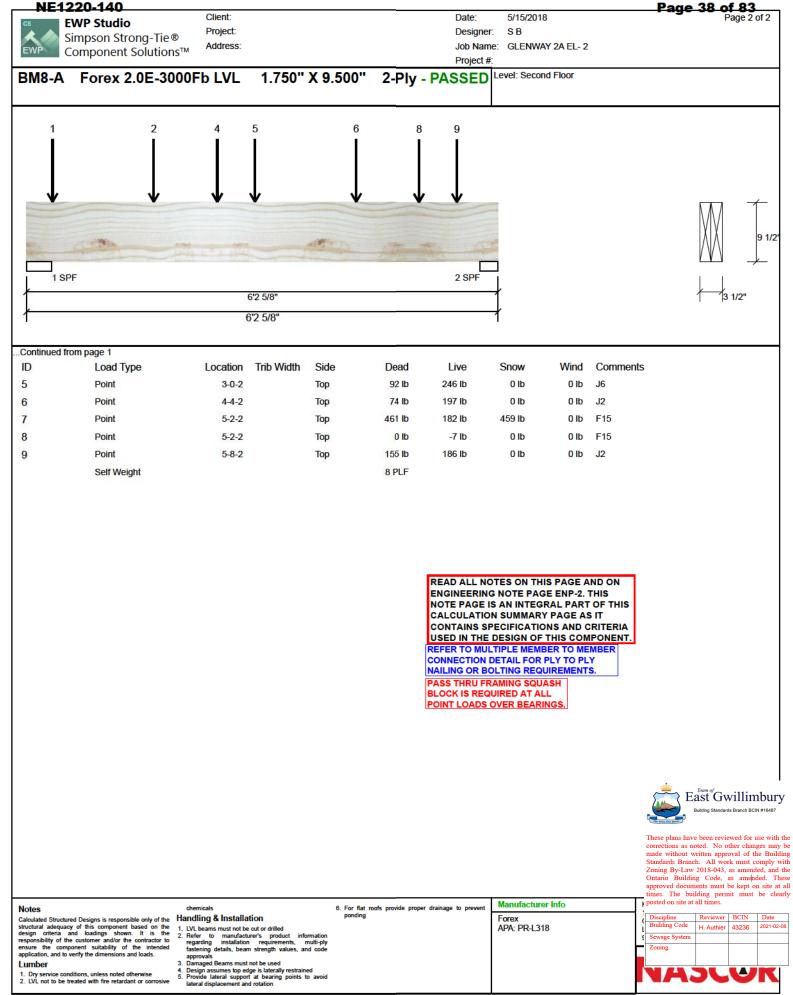
approvals

Damaged Beams must not be used Daniaged beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
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Manufacturer Info

APA: PR-L318





Dry service conditions, unless more conditions.
 LVL not to be treated with fire retardant or conditions.

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NE1220-140 Page 40 of 83 Client: Date: 5/15/2018 **EWP Studio** Project: Designer: SB Simpson Strong-Tie® Address: Job Name: GLENWAY 2A EL- 2 Component Solutions™ Project #: Level: Second Floor 1.750" X 9.500" F10-A Forex 2.0E-3000Fb LVL 2-Ply - PASSED 2 3 5 6 8 1 1 SPF 2 SPF End Grain 6'5 7/16" 6'5 7/16" .Continued from page 1 ID Location Trib Width Side Dead Live Snow Wind Comments Load Type 148 lb 0 lb 7 **Point** 5-5-12 Near Face 55 lb 0 lb J6 8 **Point** 6-2-11 Near Face 163 lb 392 lb 0 lb 0 lb F9 Self Weight 8 PLF READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS. East Gwillimbury These plans have been reviewed for use with the corrections as noted. No other changes may be corrections as noted. No other changes may obtained made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times. Manufacturer Info Notes Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orineria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Forex Handling & Installation Reviewer BCIN Date Discipline Building Code LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requirement fastening details, beam strength value APA: PR-L318 H. Authier 43236 Sewage System maged Beams must not be used Daniaged beams must not be used
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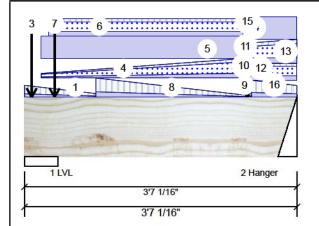
EWP Studio Simpson Strong-Tie® Component Solutions™ Project: Address: Designer: SB

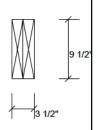
Job Name: GLENWAY 2A EL- 2

Project #:

1.750" X 9.500" F15-A Forex 2.0E-3000Fb LVL 2-Ply - PASSED

Level: Second Floor





Wind

0

0

Member Inform	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load		and the state of t	
Floor Live:	40 PSF		

Bearings and Factored Reactions									
	Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.		
	1 - LVL	5.268"	11%	576 / 779	1355	L	1.25D+1.5S +0.5L		
	2 - Hanger	3.000"	6%	253 / 221	474	L	1.25D+1.5S +0.5L		

Unfactored Reactions UNPATTERNED lb (Uplift)

182 (-7)

79 (-37)

Dead

461

203

Snow

459

121

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	251 ft-lb	1'10 3/16"	18179 ft-lb	0.014 (1%)	1.25D+1.5L	L
Unbraced	251 ft-lb	1'10 3/16"	18179 ft-lb	0.014 (1%)	1.25D+1.5L	L
Shear	162 lb	1'2"	7421 lb	0.022 (2%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/38858)	1'10 1/2"	0.101 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.001 (L/54658)	1'10 7/8"	0.101 (L/360)	0.010 (1%)	S+0.5L	L
TL Defl inch	0.002 (L/22714)	1'10 5/8"	0.151 (L/240)	0.010 (1%)	D+S+0.5L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Fill all hanger nailing holes.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.

15 PSF

- 6 Top braced at bearings.
- 7 Bottom braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

Manufacturer Info

APA: PR-L318

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



8 Latera	Il slenderness ratio based	on full section width.								_ 5
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Commen	ıts 🕌
1	Tie-In	0-0-0 to 0-11-4	(Span)1-6-0 to 0-5-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF		The
2 Continued	Point on page 2	0-1-2		Тор	26 lb	11 lb	39 lb	0 lb	F11 F11	mad Stan Zon Onta appr



nese plans have been reviewed for use with the rrections as noted. No other changes may be corrections as noted. No other changes may ob-made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amehded. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Reviewer BCIN Date Building Code H. Authier 43236 ewage System

Notes

structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corre

Handling & Installation

LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requirement

naged Beams must not be used

Daniaged beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation