

DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 6
 Lot #: 244
 Elevation: 2-STD OR OPT.5 BR

Job Track: 51225
 PlanLog: 202419
 Layout ID: 408223
 Ref #
 Page: 1 of 3
 Date: 04-28-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T11B Common	8/12	16-01-00	6-09-13	2 x 4		1-06-02 1-04-13	137.47 88.33		
	3	T11S Roof Special	8/12 8/12	16-03-00	6-09-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	201.4 133.00		
	1 2-ply	T20 Hip Girder	7/12	35-02-00	4-01-04	2 x 6	1-03-08 1-03-08	1-03-07 1-03-07	359.46 218.00		
	1	T21 Hip	7/12	35-02-00	5-01-04	2 x 4	1-03-08 1-03-08	1-03-07 1-03-07	142.49 89.50		
	1	T22 Hip	7/12	35-02-00	6-01-04	2 x 4	1-03-08 1-03-08	1-03-07 1-03-07	148.47 93.50		
	1	T23 Hip	7/12	35-02-00	7-01-04	2 x 4	1-03-08 1-03-08	1-03-07 1-03-07	144.95 91.17		
	1	T24 Hip	7/12	35-02-00	8-01-04	2 x 4	1-03-08 1-03-08	1-03-07 1-03-07	151.12 95.67		
	4	T25 Hip	7/12	35-02-00	9-01-04	2 x 4	1-03-08 1-03-08	1-03-07 1-03-07	631.9 399.33		
	3	T25A Piggyback Base	7/12	34-10-00	9-01-04	2 x 4		1-04-09 1-04-09	481.61 286.50		
	1 2-ply	T28 Flat Girder	0/12	34-10-00	4-10-07	2 x 6		4-10-07 4-10-07	366.38 214.67		
	2	T29 Flat	0/12	34-10-00	6-06-07	2 x 4		6-06-07 6-06-07	313.97 193.33		
	2	T30 Flat	0/12	34-10-00	8-02-07	2 x 4		8-02-07 8-02-07	358.85 220.00		
	5	T31 Flat	0/12	34-10-00	9-10-07	2 x 4		9-10-07 9-10-07	980.54 600.00		
	1	T32 Hip Girder	8/12	12-02-00	6-04-07	2 x 4 2 x 6		3-00-02 2-10-13	66.45 43.67		

DELIVERY SHIPLIST

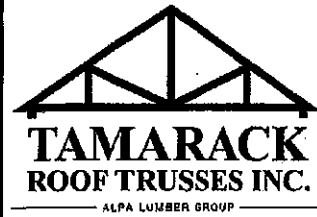


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Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T34S Half Hip	10 /12	3-10-08	5-05-13	2 x 4	1-03-08	1-07-11 5-05-13	63.75 44.33		
	2	T35S Half Hip	10 /12	3-10-08	4-01-13	2 x 4	1-03-08	1-07-11 4-01-13	57.2 41.00		
	3	T36 Common	6 /12	8-03-08	3-00-09	2 x 4	1-03-08 1-03-08	5-07 5-07	79.61 53.00		
	1	G36 GABLE	6 /12	8-03-08	3-01-05	2 x 4	1-03-08 1-05-00	5-07 5-07	26.93 17.33		
	1	T37 Half Hip Girder	8 /12	8-05-00	2-08-03	2 x 4 2 x 6	1-03-08	8-05 2-01-15	36.62 26.00		
	2	G39 GABLE	10 /12	27-06-00	9-10-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	303.3 193.33		
	17	P20 Common	10 /12	7-08-14	3-02-12	2 x 4		0-01 0-01	363.26 229.50		
	4	PB20 Piggyback	7 /12	8-04-05	2-05-04	2 x 4			80.17 49.33		
	1	PB22 Piggyback	7 /12	8-04-05	1-00-00	2 x 4			18.26 11.67		
	1	PB21 Piggyback	7 /12	8-04-05	2-00-00	2 x 4			21.88 14.33		
	2	PG20 GABLE	10 /12	7-09-00	3-02-12	2 x 4			44.53 30.33		
	17	J1 Jack-Open	8 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	285.51 181.33		
	2	J20S Jack-Open	10 /12	3-10-08	6-04-07	2 x 4	1-03-08	1-07-11 6-04-07	65.13 43.33		
	4	J21 Jack-Open	5 /12	4-04-08	2-08-02	2 x 4	1-03-08	4-01 2-01-15	48.51 32.00		



DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51225
Builder:	GREEN PARK HOMES	PlanLog:	202419
Project:	RUSSELL GARDENS PH.3	Layout ID:	408223
Location:	WATERDOWN	Ref #	
Model:	MOUNTAINASH 6	Page:	3 of 3
Lot #:	244	Date:	04-28-2020
Elevation:	2-STD OR OPT.5 BR	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	3	J22 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	45.68 29.00		

TOTAL # TRUSS= 92 TOTAL BFT OF ALL TRUSSES= 3762.48 BFT. TOTAL WEIGHT OF ALL TRSSES 6003.41 LBS

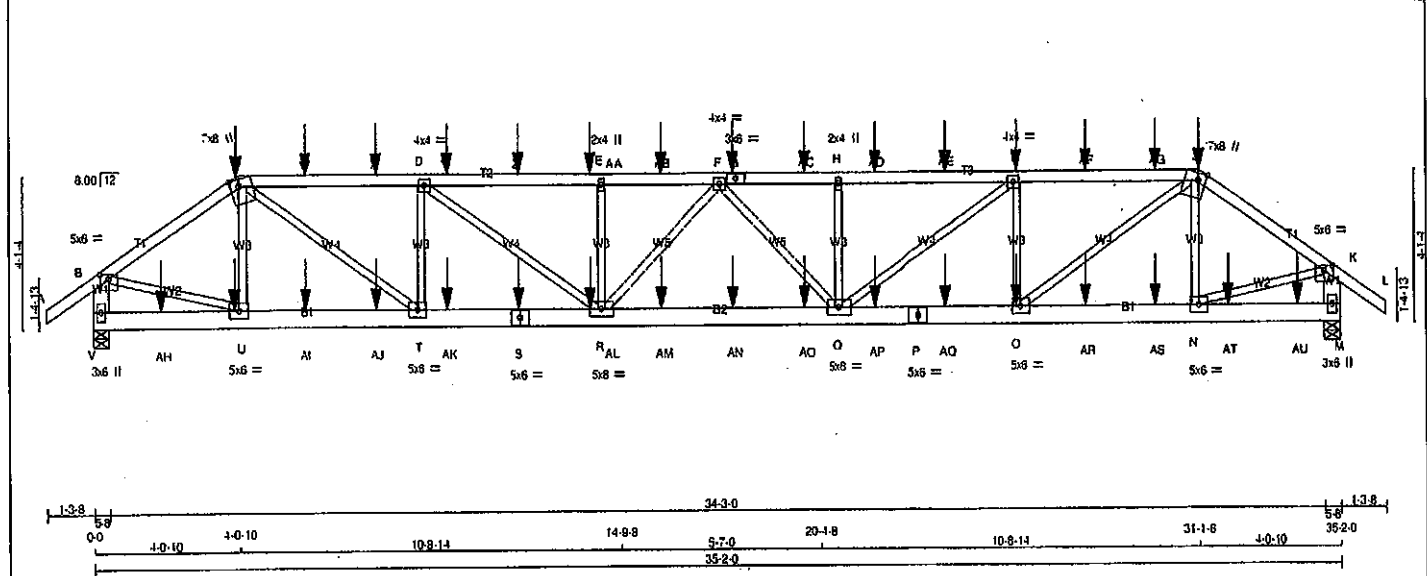
HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LJS28DS	
4	Hardware	LUS24	
2	Hardware	LUS26-2	

TOTAL NUMBER OF
ITEMS= 9

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	T1	1	2	GREEN PARK HOMES	

TRUSS DESC.	Version	310 S Oct 29 2019	MTex Industries, Inc.	Tue Apr 28 09:00:04 2020	Page 1
Tamarack Roof Truss, Burlington	ID:DMCubINVR6TstFoe31v6l	zns1l-Jm9J81AhZl22g2EEminZsEz3HTHl?mEdCAUuvczMFif			



TOTAL WEIGHT = 2 X 161 = 323 lb	
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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - G	2x4	DRY	No.2	SPF	
G - J	2x4	DRY	No.2	SPF	
J - L	2x4	DRY	No.2	SPF	
V - B	2x6	DRY	No.2	SPF	
M - K	2x6	DRY	No.2	SPF	
V - S	2x6	DRY	No.2	SPF	
S - P	2x6	DRY	No.2	SPF	
P - M	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
A - C	1	12	SIDE(183.0)
C - G	1	12	SIDE(183.0)
G - J	1	12	SIDE(183.0)
J - L	1	12	SIDE(183.0)
V - B	2	12	TOP
M - K	2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
V - S	2	12	SIDE(183.1)
S - P	2	12	SIDE(183.1)
P - M	2	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER	BEARINGS
	FACTORED GROSS REACTION
JT	VERT 3393 0
V	3393 0
M	3342 0

UNFACTORED REACTIONS	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
V	2397 1585 0	0 0 0 0 812 0 0 0
M	2363 1555 0	0 0 0 0 808 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.11 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD (LBS)	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	CSI (LBS)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)
FR-TO	0 35	-91.8 -91.8 0.07 (1)	10.00	U-C	-620 0	0.08 (1)
B-C	-4024 0	-91.8 -91.8 0.22 (1)	4.55	C-T	0 3393	0.42 (1)
C-W	-6074 0	-91.8 -91.8 0.53 (1)	3.51	T-D	-1846 0	0.23 (1)
W-X	-6074 0	-91.8 -91.8 0.53 (1)	3.51	O-I	-1869 0	0.24 (1)
X-D	-6074 0	-91.8 -91.8 0.53 (1)	3.51	O-J	0 3425	0.42 (1)
D-Y	-7423 0	-91.8 -91.8 0.63 (1)	3.11	N-U	-601 0	0.08 (1)
Y-Z	-7423 0	-91.8 -91.8 0.63 (1)	3.11	B-U	0 3437	0.43 (1)
Z-AA	-7423 0	-91.8 -91.8 0.63 (1)	3.11	N-K	0 3371	0.42 (1)
AA-E	-7423 0	-91.8 -91.8 0.63 (1)	3.11	O-I	0 1704	0.21 (1)
E-AB	-7423 0	-91.8 -91.8 0.35 (1)	3.36	D-R	0 1677	0.21 (1)
AB-F	-7423 0	-91.8 -91.8 0.35 (1)	3.36	Q-H	-595 0	0.08 (1)
F-G	-7410 0	-91.8 -91.8 0.36 (1)	3.36	R-E	-594 0	0.08 (1)
G-AC	-7410 0	-91.8 -91.8 0.36 (1)	3.36	R-F	-285 0	0.06 (1)
AC-H	-7410 0	-91.8 -91.8 0.36 (1)	3.36	F-Q	-305 0	0.06 (1)
H-AD	-7410 0	-91.8 -91.8 0.61 (1)	3.14			
AD-AE	-7410 0	-91.8 -91.8 0.61 (1)	3.14			
AE-I	-7410 0	-91.8 -91.8 0.61 (1)	3.14			
I-AF	-6040 0	-91.8 -91.8 0.51 (1)	3.55			
AF-AG	-6040 0	-91.8 -91.8 0.51 (1)	3.55			
AG-J	-6040 0	-91.8 -91.8 0.51 (1)	3.55			
J-K	-3948 0	-91.8 -91.8 0.22 (1)	4.58			
K-L	0 35	-91.8 -91.8 0.07 (1)	10.00			
V-B	-3355 0	0.0 0.0 0.12 (1)	7.60			
M-K	-3298 0	0.0 0.0 0.12 (1)	7.64			
V-AH	0 0	-18.5 -18.5 0.04 (1)	10.00			
AH-U	0 0	-18.5 -18.5 0.04 (1)	10.00			
U-AI	0 3325	-18.5 -18.5 0.25 (1)	10.00			
AI-AJ	0 3325	-18.5 -18.5 0.25 (1)	10.00			
AJ-T	0 3325	-18.5 -18.5 0.25 (1)	10.00			
T-AK	0 6074	-18.5 -18.5 0.45 (1)	10.00			
AK-S	0 6074	-18.5 -18.5 0.45 (1)	10.00			
S-AL	0 6074	-18.5 -18.5 0.45 (1)	10.00			
AL-R	0 6074	-18.5 -18.5 0.57 (1)	10.00			
R-AM	0 7613	-18.5 -18.5 0.57 (1)	10.00			
AM-AN	0 7613	-18.5 -18.5 0.57 (1)	10.00			
AN-AO	0 7613	-18.5 -18.5 0.57 (1)	10.00			
AO-O	0 7613	-18.5 -18.5 0.46 (1)	10.00			
O-AP	0 6040	-18.5 -18.5 0.45 (1)	10.00			
AP-P	0 6040	-18.5 -18.5 0.45 (1)	10.00			
P-AQ	0 6040	-18.5 -18.5 0.45 (1)	10.00			
AQ-Q	0 6040	-18.5 -18.5 0.45 (1)	10.00			
Q-AR	0 3261	-18.5 -18.5 0.25 (1)	10.00			

CONTINUED ON PAGE 2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17')
CALCULATED VERT. DEFL.(LL) = L/989 (0.23')
ALLOWABLE DEFL.(TL) = L/360 (1.17')
CALCULATED VERT. DEFL.(TL) = L/970 (0.44')

CSI: TC=0.63/1.00 (D-E:1), BC=0.57/1.00 (Q-R:1), WB=0.43/1.00 (B-U:1), SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1957 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.86 (i) (INPUT = 0.80)
JSI METAL = 0.58 (i) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	T1	1	2	GREEN PARK HOMES	
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TsIFoe31v6I zns1I-Jm9J91AhZl22o2EEmlNZsEz3HTH7mEdCAUuvczMFI

PLATES (Table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	7.0	6.0	Edge	2.50
D, F, I						
D	TMW+I	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
G	TS-I	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
J	TTWW+m	MT20	7.0	6.0	Edge	2.50
K	TMW+p	MT20	5.0	6.0	1.50	3.00
M	BMV+p	MT20	3.0	6.0		
N, O, T, U						
N	BMW+I	MT20	5.0	6.0		
P	BS-I	MT20	5.0	6.0		
Q	BMWW+I	MT20	5.0	6.0		
R	BMWW+I	MT20	5.0	6.0		
S	BS-I	MT20	5.0	6.0		
V	BMV+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: 14)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 MAX (LBS)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LBS)
FR-TO		FROM	TO		FR-TO		
AR-AS	0 3281	-18.5	-18.5 0.25 (1)	10.00			
AS-N	0 3281	-18.5	-18.5 0.25 (1)	10.00			
N-AT	0 0	-18.5	-18.5 0.04 (4)	10.00			
AT-AU	0 0	-18.5	-18.5 0.04 (4)	10.00			
AU-M	0 0	-18.5	-18.5 0.04 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	4-0-10	-49	-55	---	FRONT	VERT	DEAD	---	C1
C	4-0-10	-117	-117	---	FRONT	VERT	TOTAL	---	C1
C	4-0-10	-254	-254	---	FRONT	VERT	SNOW	---	C1
G	17-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
I	25-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
J	31-1-6	-49	-55	---	FRONT	VERT	DEAD	---	C1
J	31-1-6	-254	-254	---	FRONT	VERT	SNOW	---	C1
O	25-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
S	11-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
U	3-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
W	5-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	11-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	15-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	19-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	21-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AE	23-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AF	27-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AG	29-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AH	1-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AI	5-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AJ	7-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AK	9-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AL	13-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AM	15-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AN	17-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AO	19-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AP	21-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AQ	23-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AR	27-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AS	29-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AT	31-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AU	33-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

11 C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007602 1/2

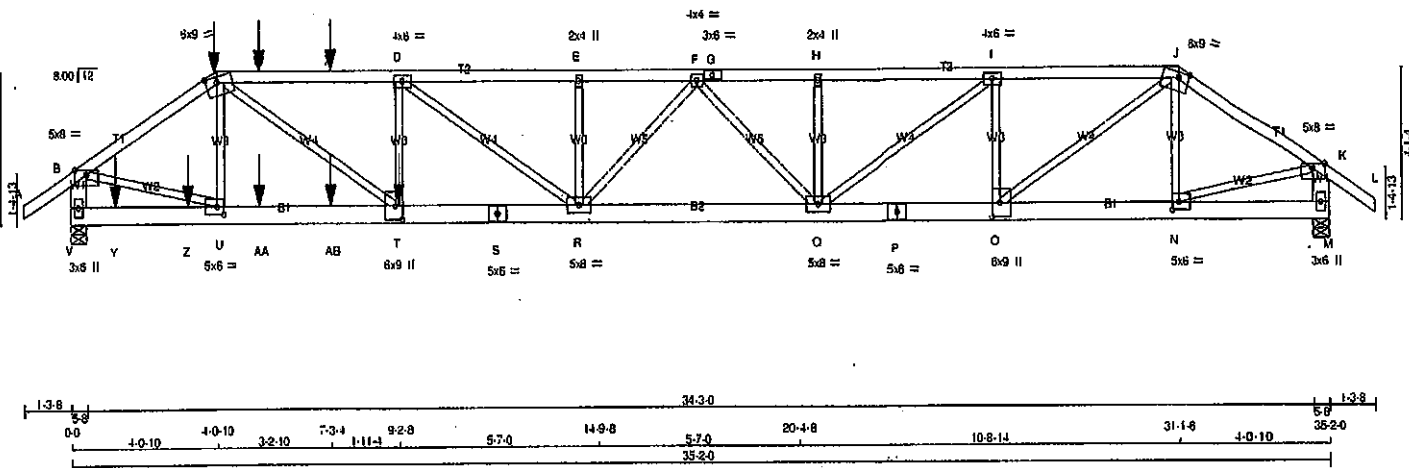
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408222	T1Z	1	2	TRUSS DESC.		

Temarack Roof Truss, Burlington

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ID:DMCubINVR8TstFoe31v6l zns1l-nyihMNBjKcAvHcPQKTuoORWdoscNkApnRqDRR2zMFie

1-3-8 0-0 4-0-10 4-0-10 5-3-4 7-3-4 9-3-12 14-9-8 17-7-0 20-4-6 25-10-4 31-1-6 35-2-0 38-5-8
1-3-8 4-0-10 4-0-10 5-3-4 7-3-4 9-3-12 14-9-8 17-7-0 20-4-6 25-10-4 31-1-6 35-2-0 38-5-8
Scale = 1:57.6



TOTAL WEIGHT = 2 X 161 = 323 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - G	2x4	DRY	No.2	SPF	
G - J	2x4	DRY	No.2	SPF	
J - L	2x4	DRY	No.2	SPF	
V - B	2x6	DRY	No.2	SPF	
M - K	2x6	DRY	No.2	SPF	
V - S	2x6	DRY	No.2	SPF	
S - P	2x6	DRY	No.2	SPF	
P - M	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-G	12	SIDE(61.0)
C-G	12	SIDE(61.0)
G-J	12	TOP
J-L	12	TOP
V-B	12	TOP
M-K	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
V-S	12	SIDE(183.1)
S-P	12	TOP
P-M	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	SIDE(385.4)
D-T	1	
I-O	4	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	IN-SX	IN-SX
V	4215	0	5-8	5-8
M	2722	0	5-8	5-8

UNFACTORED REACTIONS

1ST LOOSE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
V	2974	1987	0	0	0	987	0
M	1921	1283	0	0	0	638	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED	FACTORED	FACTORED	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1 (LBS)	MAX. (LBS)	MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1 (LBS)	MAX. (LBS)
FR-TO		FROM TO			FR-TO		FROM TO		
A-B	0 35	-91.8 -91.8 0.07 (1)	10.00	U-C	-783 0	0.10 (1)			
B-C	-5115 0	-91.8 -91.8 0.26 (1)	4.07	C-T	0 4817	0.60 (1)			
C-W	-8135 0	-91.8 -91.8 0.64 (1)	3.00	T-D	-622 0	0.08 (1)			
W-X	-8135 0	-91.8 -91.8 0.64 (1)	3.00	O-I	-1797 0	0.23 (1)			
X-D	-8135 0	-91.8 -91.8 0.64 (1)	3.00	O-J	0 3211	0.40 (1)			
D-E	-8108 0	-91.8 -91.8 0.59 (1)	3.05	N-J	-512 0	0.06 (1)			
E-F	-8108 0	-91.8 -91.8 0.32 (1)	3.28	B-U	0 4370	0.54 (1)			
F-G	-6928 0	-91.8 -91.8 0.26 (1)	3.57	N-K	0 2693	0.33 (1)			
G-H	-6928 0	-91.8 -91.8 0.26 (1)	3.57	Q-I	0 2133	0.26 (1)			
H-I	-6928 0	-91.8 -91.8 0.45 (1)	3.48	O-R	-37 24	0.01 (1)			
I-J	-5210 0	-91.8 -91.8 0.34 (1)	3.99	Q-H	-365 0	0.05 (1)			
J-K	-3153 0	-91.8 -91.8 0.19 (1)	5.04	R-E	-343 0	0.04 (1)			
K-L	0 35	-91.8 -91.8 0.07 (1)	10.00	R-F	0 692	0.05 (1)			
V-B	-4180 0	0.0 0.0 0.15 (1)	8.99	F-Q	-1091 0	0.23 (1)			
M-K	-2696 0	0.0 0.0 0.10 (1)	7.81						
V-Y	0 0	-18.5 -18.5 0.05 (4)	10.00						
Y-Z	0 0	-18.5 -18.5 0.05 (4)	10.00						
Z-U	0 0	-18.5 -18.5 0.05 (4)	10.00						
U-AA	0 4227	-18.5 -18.5 0.35 (1)	10.00						
AA-AB	0 4227	-18.5 -18.5 0.35 (1)	10.00						
AB-T	0 4227	-18.5 -18.5 0.35 (1)	10.00						
T-S	0 8135	-18.5 -18.5 0.81 (1)	10.00						
S-R	0 8135	-18.5 -18.5 0.81 (1)	10.00						
R-Q	0 7645	-18.5 -18.5 0.55 (1)	10.00						
Q-P	0 5210	-18.5 -18.5 0.38 (1)	10.00						
P-O	0 5210	-18.5 -18.5 0.38 (1)	10.00						
O-N	0 2604	-18.5 -18.5 0.19 (1)	10.00						
N-M	0 0	-18.5 -18.5 0.03 (4)	10.00						

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-10	-49	-55	---	FRONT	VERT	DEAD	---	C1
T	4-0-10	-254	-254	---	FRONT	VERT	SNOW	---	C1
T	9-2-8	-2179	-2179	---	BACK	VERT	TOTAL	---	C1
W	5-3-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
X	7-3-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
Y	1-3-4	-28	-28	---	BACK	VERT	TOTAL	---	C1
Z	3-3-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AA	5-3-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AB	7-3-4	-26	-26	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17')

CALCULATED VERT. DEFL.(LL) = L/999 (0.24')

ALLOWABLE DEFL.(TL) = L/360 (1.17')

CALCULATED VERT. DEFL.(TL) = L/954 (0.44')

CS: IC=0.84/1.00 (C-D:1), BC=0.61 1.00 (R-T:1), WB=0.60/1.00 (C-T:1), SS=0.17 1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

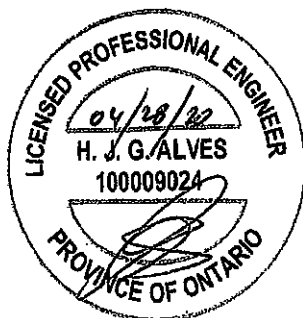
PLATE GRIP/RYI	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1807	788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) INPUT = 0.90
JSI METAL= 0.77 (P) INPUT = 1.00

CONTINUED ON PAGE 2



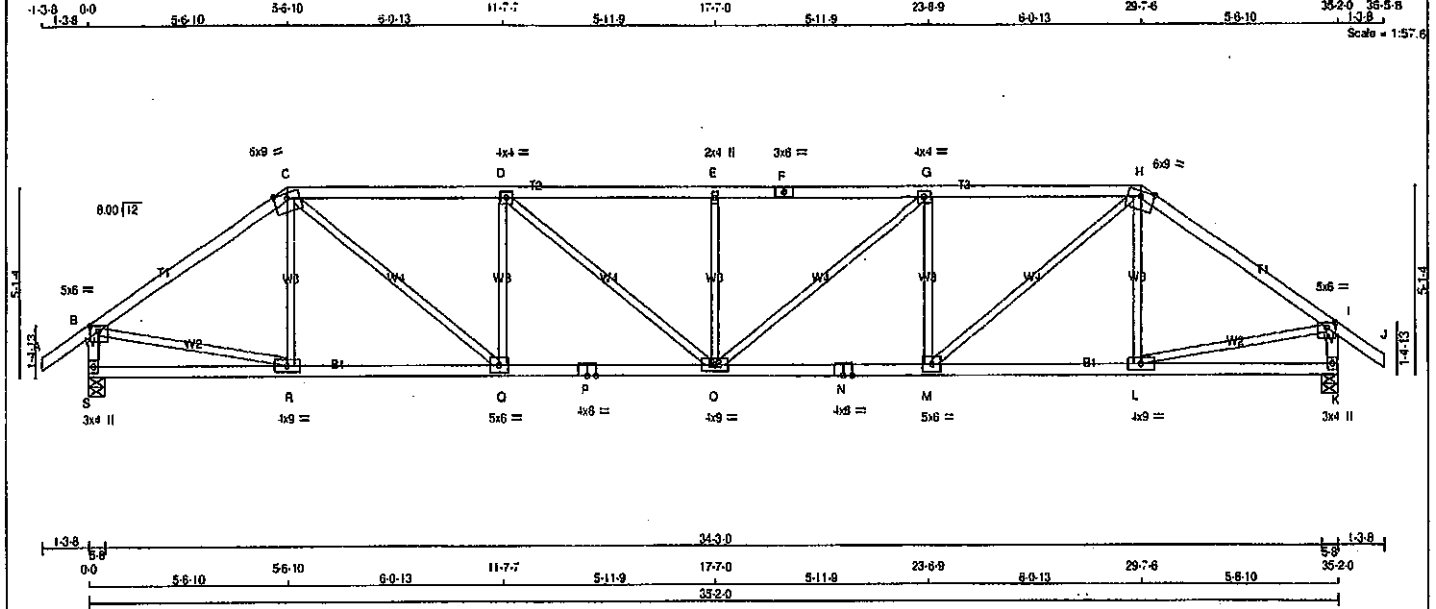
Structural component only
DWG# T-2007603 1/6

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	T2	2	1	GREEN PARK HOMES	
Truss Desc.					

Lamarack Roof Truss, Burlington

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ID:DMCubINVR6TsIFoe31v6I zns1l-F8H4ajCx5wlmvMOCuAP1x2LCCy5TgYwUz? UzMFId



TOTAL WEIGHT = 2 X 140 = 280 lb (M)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
J - K	2x4	DRY	No.2	SPF	
K - L	2x4	DRY	No.2	SPF	
L - M	2x4	DRY	No.2	SPF	
M - N	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	2.75
C	TTWW-m	MT20	6.0	9.0	Edge	
D	TMVW-t	MT20	4.0	4.0		
E	TMVW-w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	9.0	Edge	
I	TMVW-p	MT20	5.0	8.0	1.75	2.75
K	BMV1-p	MT20	3.0	4.0		
L	BMVW-t	MT20	4.0	9.0		
M	BMVW-t	MT20	5.0	6.0		
N	BS-t	MT20	4.0	6.0		
O	BMVW-w	MT20	4.0	9.0		
P	BS-t	MT20	4.0	6.0		
Q	BMVW-t	MT20	5.0	6.0		
R	BMVW-t	MT20	4.0	9.0		
S	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT 8RG	REQD 8RG
JT	VERT	DOWN	UPLIFT	IN-SX
S	2065	0	0	5-8
K	2065	0	0	5-8
UNFACTORED REACTIONS	1ST LCASE	MAX. MIN. COMPONENT REACTIONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE
S	1458	970 : 0	0 : 0	0 : 0
K	1458	970 : 0	0 : 0	0 : 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)
FR-TO						FR-TO			
A-B	0	35	-91.8	-91.8	0.12 (1)	10.00	R-C	-243	10
B-C	-2298	0	-91.8	-91.8	0.70 (1)	3.78	C-D	0	1070
C-D	-3213	0	-91.8	-91.8	0.80 (1)	3.19	D-E	-929	0
D-E	-3594	0	-91.8	-91.8	0.88 (1)	2.97	E-F	0	490
E-F	-3594	0	-91.8	-91.8	0.88 (1)	2.97	F-G	-505	0
F-G	-3594	0	-91.8	-91.8	0.88 (1)	2.97	G-H	0	480
G-H	-3213	0	-91.8	-91.8	0.80 (1)	3.19	H-I	-929	0
H-I	-2298	0	-91.8	-91.8	0.70 (1)	3.78	I-J	0	1070
I-J	0	35	-91.8	-91.8	0.12 (1)	10.00	J-K	-243	10
J-K	-2023	0	0.0	0.0	0.21 (1)	5.94	K-L	0	1943
K-L	-2023	0	0.0	0.0	0.21 (1)	5.94	L-M	0	1943
S-H	0	0	-18.5	-18.5	0.15 (4)	10.00			
R-Q	0	1903	-18.5	-18.5	0.33 (1)	10.00			
Q-P	0	3213	-18.5	-18.5	0.58 (1)	10.00			
P-O	0	3213	-18.5	-18.5	0.58 (1)	10.00			
O-N	0	3213	-18.5	-18.5	0.58 (1)	10.00			
N-M	0	3213	-18.5	-18.5	0.58 (1)	10.00			
M-L	0	1903	-18.5	-18.5	0.33 (1)	10.00			
L-K	0	0	-18.5	-18.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
OL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.39")

CSI: TC=0.88/1.00 (D-E:1), BC=0.58/1.00 (M-O:1), WB=0.44/1.00 (I-L:1), SS=0.28/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

MAIL VALUES
PLATE GRIP(OR)Y SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

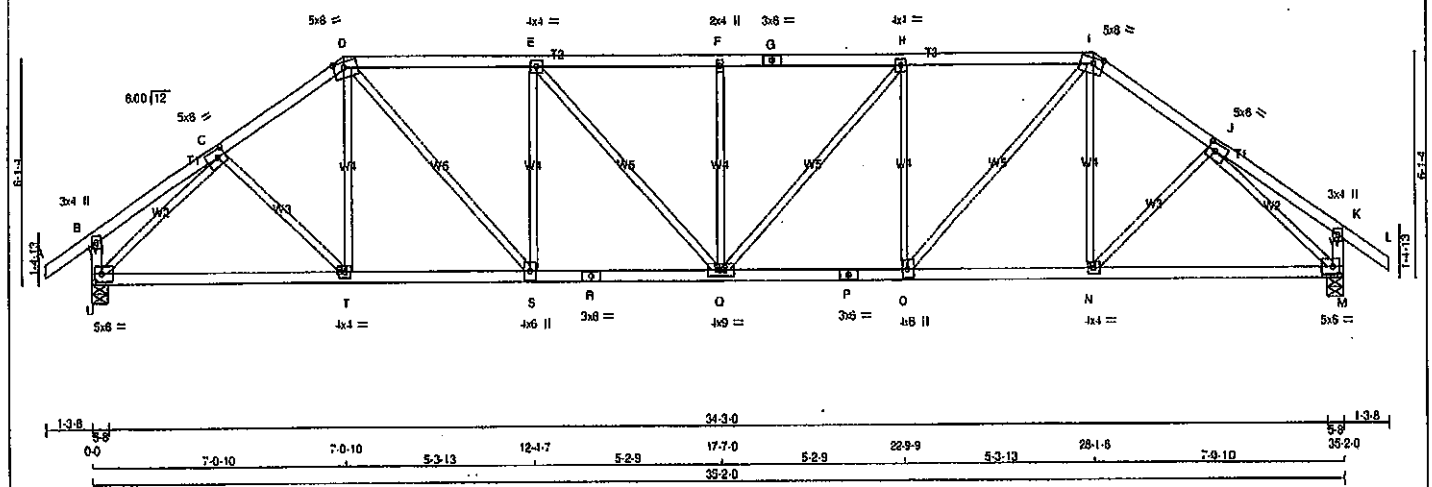
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (I) (INPUT = 0.90)
JSI METAL = 0.74 (I) (INPUT = 1.00)



Structural component only
DWG# T-2007604



TOTAL WEIGHT = 2 X 151 = 303 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
I - L	2x4	DRY No.2	SPF
U - B	2x4	DRY No.2	SPF
M - K	2x4	DRY No.2	SPF
U - R	2x4	DRY No.2	SPF
R - P	2x4	DRY No.2	SPF
P - M	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	2065	2065	0	0
U	2065	2065	0	0
M	2065	2065	0	0

UNFACTORED REACTIONS

	1ST LOSE	MAX. MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT COMBINED	1458	970.0	0.0	0.0	0.0
U	1458	970.0	0.0	0.0	0.0
M	1458	970.0	0.0	0.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-1	MT20	5.0	6.0	2.50	2.50
D	TTWW-m	MT20	5.0	6.0	1.75	3.50
E	TMVW-1	MT20	4.0	4.0		
F	TMVW-w	MT20	2.0	4.0		
G	TS-1	MT20	3.0	6.0		
H	TMVW-1	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	6.0	1.75	3.50
J	TMVW-1	MT20	5.0	6.0	2.50	2.50
K	TMV+p	MT20	3.0	4.0		
M	BMVW-1	MT20	5.0	6.0	2.50	2.25
N	BMVW-1	MT20	4.0	4.0		
O	BMVW-1	MT20	4.0	6.0		
P	BS-1	MT20	3.0	6.0		
Q	BMVW-1	MT20	4.0	9.0		
R	BS-1	MT20	3.0	6.0		
S	BMVW-1	MT20	4.0	6.0		
T	BMVW-1	MT20	4.0	4.0		
U	BMVW-1	MT20	5.0	6.0	2.50	2.25

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 35	-91.8 -91.8 0.12 (1)	10.00	C-T	0 135	0.03 (4)	
B-C	0 18	-91.8 -91.8 0.16 (1)	10.00	T-D	0 90	0.03 (4)	
C-D	-2305 0	-91.8 -91.8 0.23 (1)	4.31	D-S	0 1218	0.27 (1)	
D-E	-2723 0	-91.8 -91.8 0.54 (1)	3.69	S-E	-814 0	0.48 (1)	
E-F	-2986 0	-91.8 -91.8 0.56 (1)	3.53	E-Q	0 363	0.08 (1)	
F-G	-2986 0	-91.8 -91.8 0.56 (1)	3.53	Q-F	-440 0	0.28 (1)	
G-H	-2986 0	-91.8 -91.8 0.56 (1)	3.53	C-H	0 363	0.08 (1)	
H-I	-2723 0	-91.8 -91.8 0.54 (1)	3.69	C-H	-814 0	0.48 (1)	
I-J	-2305 0	-91.8 -91.8 0.23 (1)	4.31	O-I	0 1218	0.27 (1)	
J-K	0 18	-91.8 -91.8 0.16 (1)	10.00	N-I	0 90	0.03 (4)	
K-L	0 35	-91.8 -91.8 0.12 (1)	10.00	N-J	0 135	0.03 (4)	
U-B	-255 0	0.0 0.0 0.03 (1)	7.81	U-C	-2514 0	0.97 (1)	
M-K	-255 0	0.0 0.0 0.03 (1)	7.81	J-M	-2514 0	0.97 (1)	
U-T	0 1801	-18.5 -18.5 0.40 (1)	10.00				
T-S	0 1900	-18.5 -18.5 0.41 (1)	10.00				
S-R	0 2723	-18.5 -18.5 0.49 (1)	10.00				
R-Q	0 2723	-18.5 -18.5 0.49 (1)	10.00				
Q-P	0 2723	-18.5 -18.5 0.49 (1)	10.00				
P-O	0 2723	-18.5 -18.5 0.49 (1)	10.00				
O-N	0 1900	-18.5 -18.5 0.41 (1)	10.00				
N-M	0 1801	-18.5 -18.5 0.40 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
 ALLOWABLE DEFL.(TL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CSI: TC=0.58/1.00 (E-F:1), BC=0.49/1.00 (O-Q:1), WB=0.97/1.00 (J-M:1), SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)

	MAX	MIN	MAX	MIN
MT20	618	354	1867	788
	1987	1656		

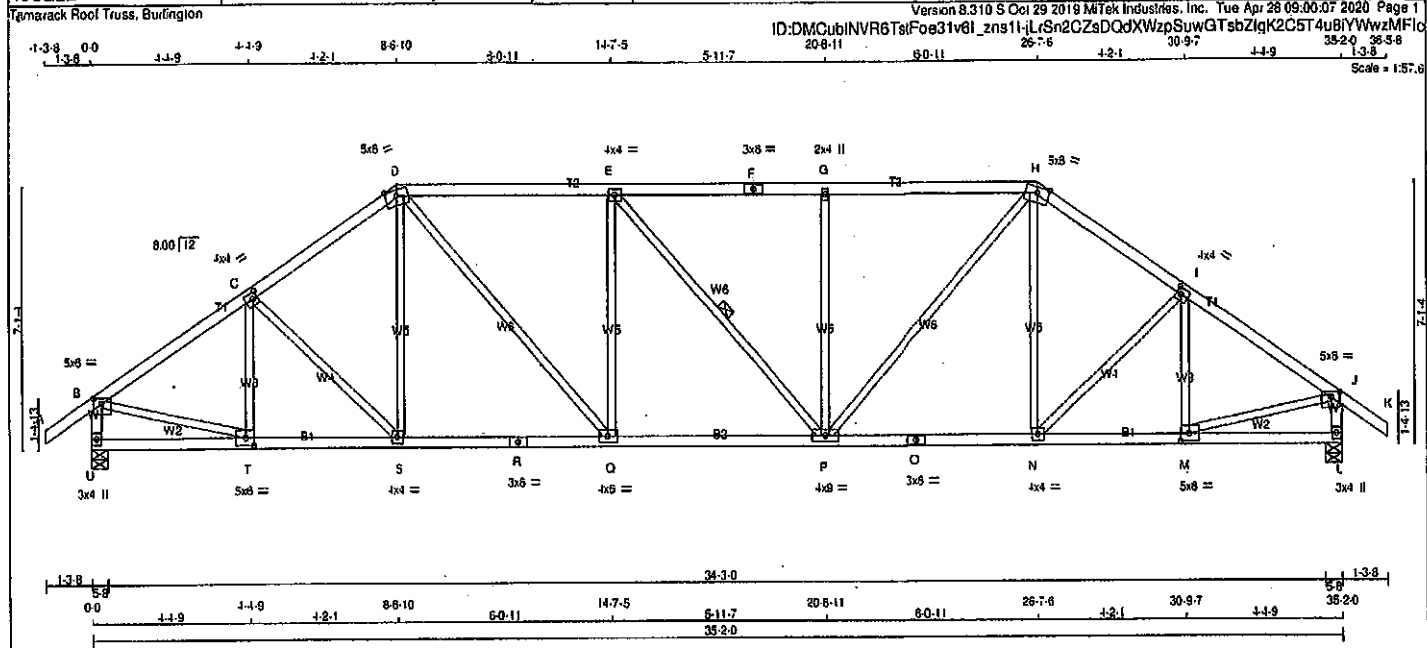
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) INPUT = 0.90 (1)
 JSI METAL= 0.64 (P) INPUT = 1.00 (1)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	T4	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		Version 8.310 S Oct 29 2018 M&T Tek Industries, Inc. Tue Apr 28 09:00:07 2020 Page 1			
		ID:DMCubINVR6TstF0e31v6l_zns1t-1LrSn2CZsDQdXWzpsuWGTsbZigK2C5T4u8iYVWwzMf1c			
		20-8-11 26-7-6 30-9-7 35-2-0 38-5-6 1-3-8			
		Scale = 1:57.6			



TOTAL WEIGHT = 2 X 156 = 309 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
O - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
U - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMVW-p	MT20	5.0	6.0	1.75 2.75
C TMVW-i	MT20	4.0	4.0	2.00 1.50
D TMVW-m	MT20	5.0	6.0	Edge
E TMVW-l	MT20	4.0	4.0	
F TS-1	MT20	3.0	6.0	
G TMVW-w	MT20	2.0	4.0	
H TMVW-m	MT20	5.0	6.0	Edge
I TMVW-l	MT20	4.0	4.0	2.00 1.50
J TMVW-p	MT20	5.0	6.0	1.75 2.75
L BMV1-p	MT20	3.0	4.0	
M BMVW-i	MT20	5.0	6.0	2.50 2.75
N BMVW-l	MT20	4.0	4.0	
O BS-1	MT20	3.0	6.0	
P BMVW-w	MT20	4.0	9.0	
Q BMVW-i	MT20	4.0	6.0	
R BS-1	MT20	3.0	6.0	
S BMVW-l	MT20	4.0	4.0	
T BMVW-i	MT20	5.0	6.0	2.50 2.75
U BMV1-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UP	BRG	BRG	BRG
U	2065	0	2065	0	0	5-8	5-8	5-8	5-8
L	2065	0	2065	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST CASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
U	1458	970	0	0	0
L	1458	970	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U. L.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1	MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	VERT. LOAD	LC1	MAX	MAX. UNBRAC
FR-TO						FR-TO					
A-B	0 35	-91.8	-91.8	0.12 (1)	10.00	T-C	-397 0	0.11 (1)			
B-C	-2289 0	-91.8	-91.8	0.37 (1)	4.19	C-S	-78 0	0.05 (1)			
C-D	-2254 0	-91.8	-91.8	0.36 (1)	4.21	S-D	0 155	0.04 (4)			
D-E	-2485 0	-91.8	-91.8	0.64 (1)	3.72	D-Q	0 949	0.21 (1)			
E-F	-2483 0	-91.8	-91.8	0.64 (1)	3.72	Q-E	-597 0	0.52 (1)			
F-G	-2483 0	-91.8	-91.8	0.64 (1)	3.72	E-P	-2 0	0.00 (1)			
G-H	-2483 0	-91.8	-91.8	0.63 (1)	3.74	P-G	-597 0	0.52 (1)			
H-I	-2254 0	-91.8	-91.8	0.36 (1)	4.21	P-H	0 947	0.21 (1)			
I-J	-2269 0	-91.8	-91.8	0.37 (1)	4.19	N-H	0 157	0.04 (4)			
J-K	0 35	-91.8	-91.8	0.12 (1)	10.00	N-I	-78 0	0.05 (1)			
U-B	-2027 0	0.0	0.0	0.21 (1)	5.94	M-I	-397 0	0.11 (1)			
L-J	-2026 0	0.0	0.0	0.21 (1)	5.94	B-T	0 1955	0.44 (1)			
U-T	0 0	-18.5	-18.5	0.08 (4)	10.00	M-J	0 1955	0.44 (1)			
T-S	0 1908	-18.5	-18.5	0.37 (1)	10.00						
S-R	0 1853	-18.5	-18.5	0.36 (1)	10.00						
R-Q	0 1853	-18.5	-18.5	0.36 (1)	10.00						
Q-P	0 2485	-18.5	-18.5	0.47 (1)	10.00						
P-O	0 1854	-18.5	-18.5	0.36 (1)	10.00						
O-N	0 1854	-18.5	-18.5	0.36 (1)	10.00						
N-M	0 1908	-18.5	-18.5	0.37 (1)	10.00						
M-L	0 0	-18.5	-18.5	0.08 (4)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17')
CALCULATED VERT. DEFL.(LL) = L/999 (0.12')
ALLOWABLE DEFL.(TL) = L/360 (1.17')
CALCULATED VERT. DEFL.(TL) = L/999 (0.24')

CSI: TC=0.84/1.00 (D-E:1), BC=0.47/1.00 (P-Q:1),
WB=0.52/1.00 (E-Q:1), SS=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

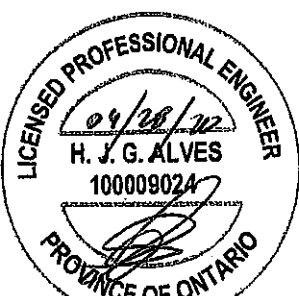
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.62 (O) (INPUT = 1.00)



Structural component only
DWG# T-2007606

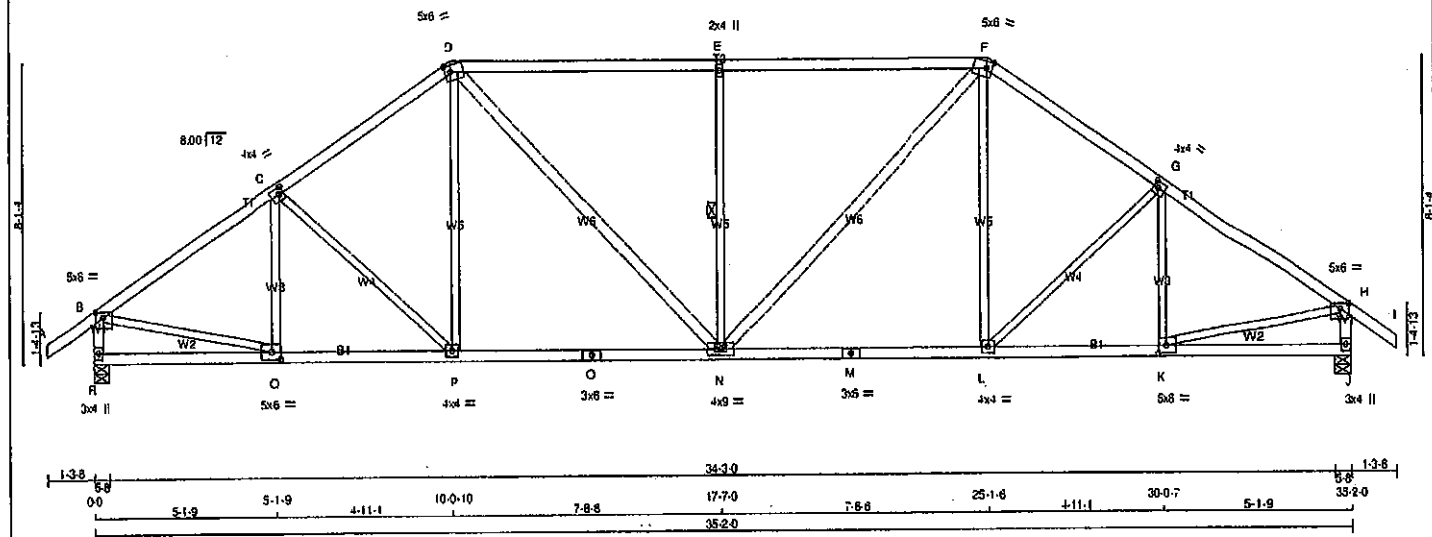
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408222	T5	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TslFoe31v6l znsII-BXPq ODBdXYU8Y7?bRV048IM4GExZrD7oSS2NzMFk

1-3.8 0.0 5-1.9 5-1.9 4-11.1 10-0-10 7-6.6 17-7-0 7-6.6 25-1-6 4-11.1 30-0-7 5-1-9 35-2-0 1-3.8 Scale = 1:57.6



TOTAL WEIGHT = 2 X 158 = 317 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
R - B	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
R - O	2x4	DRY No.2	SPF
O - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
D - N	2x4	DRY No.2	SPF
N - F	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
R	2065	0	2065	0	0	5-8	5-8	5-8	5-8
J	2065	0	2065	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LOSE		MAX. MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	DEAD	SOIL
R	1458	970	0	0	0	488	0	0	0
J	1458	970	0	0	0	488	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. CS (LC)	MAX. UNBRACED LENGTH	MEMB.
FR-TO					FR-TO				
A-B	0 35	-91.8	-91.8 0.12 (1)	10.00	C-C	-321 0	0.11 (1)		
B-C	-2317 0	-91.8	-91.8 0.38 (1)	4.16	C-P	-218 0	0.18 (1)		
C-D	-2185 0	-91.8	-91.8 0.37 (1)	4.28	P-D	0 268	0.06 (1)		
D-E	-2274 0	-91.8	-91.8 0.81 (1)	3.47	D-N	0 691	0.11 (1)		
E-F	-2274 0	-91.8	-91.8 0.81 (1)	3.47	N-E	-651 0	0.35 (1)		
F-G	-2185 0	-91.8	-91.8 0.37 (1)	4.28	N-F	0 691	0.11 (1)		
G-H	-2317 0	-91.8	-91.8 0.38 (1)	4.16	L-F	0 268	0.06 (1)		
H-I	0 35	-91.8	-91.8 0.12 (1)	10.00	L-G	-218 0	0.18 (1)		
R-B	-2022 0	0.0	0.0 0.21 (1)	5.94	K-G	-321 0	0.11 (1)		
J-H	-2022 0	0.0	0.0 0.21 (1)	5.94	B-Q	0 1995	0.45 (1)		
					K-H	0 1995	0.45 (1)		
R-Q	0 0	-18.5	-18.5 0.10 (4)	10.00					
Q-P	0 1953	-18.5	-18.5 0.40 (1)	10.00					
P-O	0 1794	-18.5	-18.5 0.41 (1)	10.00					
O-N	0 1794	-18.5	-18.5 0.41 (1)	10.00					
N-M	0 1794	-18.5	-18.5 0.41 (1)	10.00					
M-L	0 1794	-18.5	-18.5 0.41 (1)	10.00					
L-K	0 1953	-18.5	-18.5 0.40 (1)	10.00					
K-J	0 0	-18.5	-18.5 0.10 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.81:1.00 (E:F:1), BC=0.41:1.00 (L:N:1), WB=0.45:1.00 (H:K:1), SSI=0.34:1.00 (D:E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	(IPL)
MAX MIN MAX MIN MAX MIN			
MT20	618	354	1687
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (H) (INPUT = 0.90)
JSI METAL = 0.84 (M) (INPUT = 1.00)

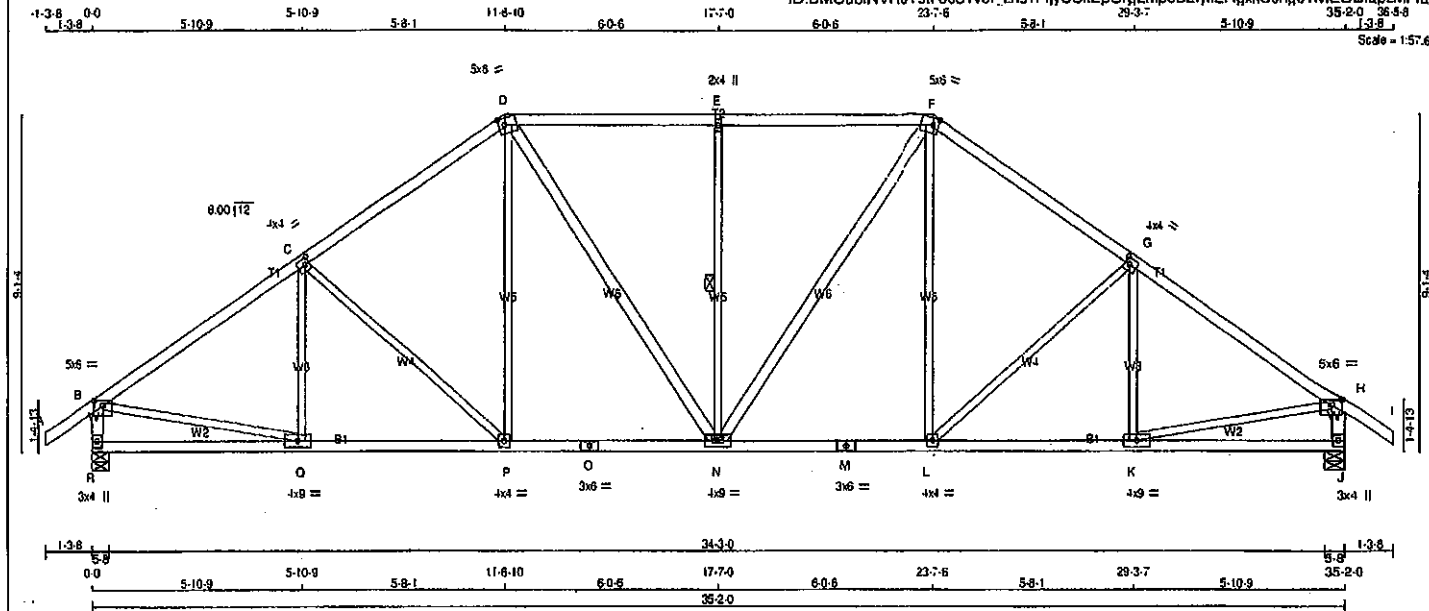


Structural component only
DWG# T-2007607

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	ORWG NO.
408222	T6	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 165 = 329 lb
[M/F]

LUMBER			
N, L, G, A	RULES		
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - J	2x4	DRY	No.2
J - B	2x4	DRY	No.2
B - H	2x4	DRY	No.2
H - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W.	LEN Y X
B	TMWW-p	MT20	5.0	6.0 1.75 2.75
C	TMWW-l	MT20	4.0	4.0 2.00 1.50
D	TTWW-m	MT20	5.0	6.0 2.25 2.00
E	TMWW-w	MT20	2.0	4.0
F	TTWW-m	MT20	5.0	6.0 2.25 2.00
G	TMWW-l	MT20	4.0	4.0 2.00 1.50
H	TMWW-p	MT20	5.0	6.0 1.75 2.75
J	BMV-l-p	MT20	3.0	4.0
K	BMWW-l	MT20	4.0	9.0
L	BMWW-l	MT20	4.0	4.0
M	BS-l	MT20	3.0	6.0
N	BMWW-l	MT20	4.0	9.0
O	BS-l	MT20	3.0	6.0
P	BMWW-l	MT20	4.0	4.0
Q	BMWW-l	MT20	4.0	9.0
R	BMV-l-p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
R	2065	0	2065	0	5-8
J	2065	0	2065	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
R	1458	970 0	0 0	0 0	468 0	0 0
J	1458	970 0	0 0	0 0	468 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	MAX.	MEMB.	MAX. FACTORED	MAX.	
	FORCE	VERT. LOAD	LC1		FORCE	MAX.	
	(LBS)	(PLF)	CSI (LC)		(LBS)	CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 35	-91.8 -91.8	0.12 (1)	10.00	Q-C	-249 11	0.11 (1)
B-C	-2346 0	-91.8 -91.8	0.52 (1)	3.99	C-P	-356 0	0.42 (1)
C-D	-2085 0	-91.8 -91.8	0.48 (1)	4.22	P-D	0 341	0.08 (1)
D-E	-1985 0	-91.8 -91.8	0.49 (1)	4.26	D-N	0 477	0.08 (1)
E-F	-1985 0	-91.8 -91.8	0.49 (1)	4.26	N-E	-678 0	0.37 (1)
F-G	-2095 0	-91.8 -91.8	0.48 (1)	4.22	N-F	0 477	0.08 (1)
G-H	-2346 0	-91.8 -91.8	0.52 (1)	3.99	L-F	0 341	0.08 (1)
H-I	0 35	-91.8 -91.8	0.12 (1)	10.00	L-G	-356 0	0.42 (1)
I-B	-2018 0	0.0 0.0	0.21 (1)	5.84	K-G	-249 11	0.11 (1)
J-H	-2018 0	0.0 0.0	0.21 (1)	5.84	B-Q	0 2014	0.45 (1)
					K-H	0 2014	0.45 (1)
R-Q	0 0	-18.5 -18.5	0.14 (4)	10.00			
Q-P	0 1980	-18.5 -18.5	0.39 (1)	10.00			
P-O	0 1716	-18.5 -18.5	0.34 (1)	10.00			
O-N	0 1716	-18.5 -18.5	0.34 (1)	10.00			
N-M	0 1716	-18.5 -18.5	0.34 (1)	10.00			
M-L	0 1716	-18.5 -18.5	0.34 (1)	10.00			
L-K	0 1981	-18.5 -18.5	0.39 (1)	10.00			
K-J	0 0	-18.5 -18.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.52/1.00 (G-H:1), BC=0.39/1.00 (K-L:1), WB=0.45/1.00 (H-K:1), SS=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1867 788 1887 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (H) (INPUT = 0.90)
JSI METAL = 0.56 (M) (INPUT = 1.00)

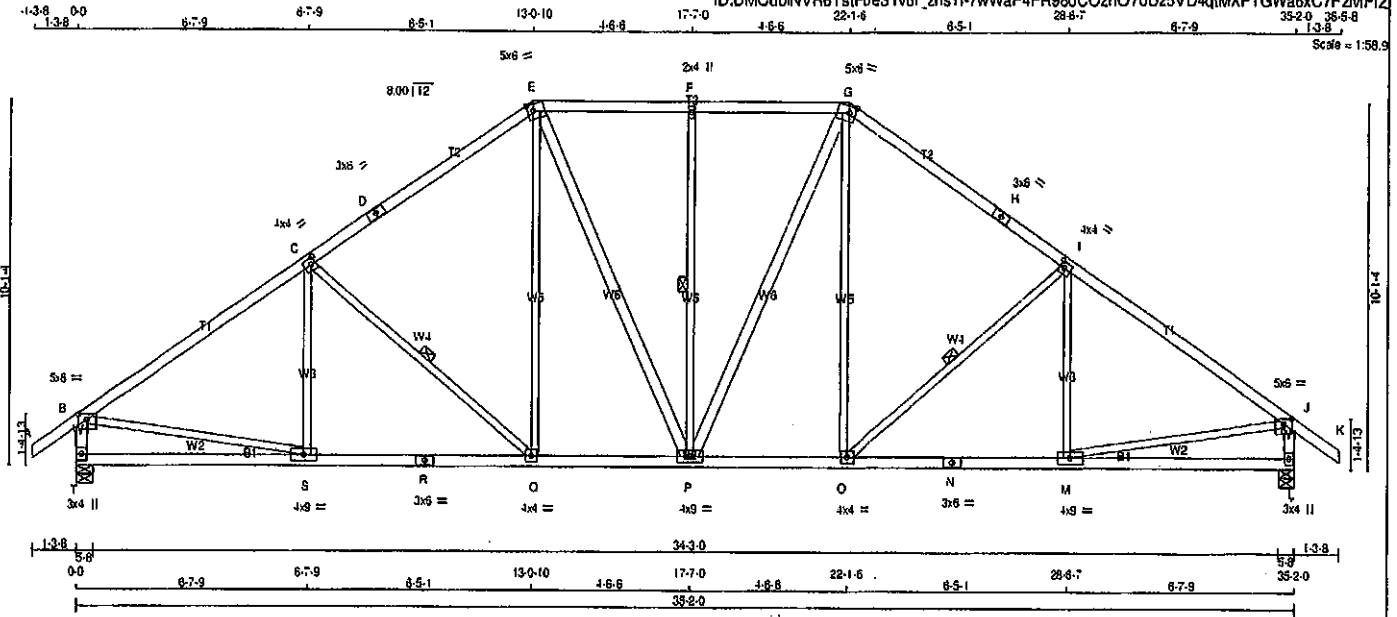


Structural component only
DWG# T-2007608

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	T7	6	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6l_zns11-7wWaP4FR98oCOzhO70Uz5VD4qIMXPTGwa8xC7FzMFIZ



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - N	2x4	DRY	No.2
N - O	2x4	DRY	No.2
O - P	2x4	DRY	No.2
P - Q	2x4	DRY	No.2
Q - R	2x4	DRY	No.2
R - S	2x4	DRY	No.2
S - T	2x4	DRY	No.2
T - U	2x4	DRY	No.2
U - V	2x4	DRY	No.2
V - W	2x4	DRY	No.2
W - X	2x4	DRY	No.2
X - Y	2x4	DRY	No.2
Y - Z	2x4	DRY	No.2
Z - A	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMWV-p	MT20	5.0	6.0 1.75 2.75
C	TMWV-t	MT20	4.0	4.0 2.00 1.50
D	TS-t	MT20	3.0	6.0
E	TTWV-m	MT20	5.0	6.0 2.25 2.00
F	TMW-t	MT20	2.0	4.0
G	TTWV-m	MT20	5.0	6.0 2.25 2.00
H	TS-t	MT20	3.0	6.0
I	TMWV-p	MT20	4.0	4.0 2.00 1.50
J	TMWV-t	MT20	5.0	6.0 1.75 2.75
L	BMV1-p	MT20	3.0	4.0
M	BMWV-t	MT20	4.0	9.0
N	BS-t	MT20	3.0	6.0
O	BMWV-t	MT20	4.0	4.0
P	BMWV-t	MT20	4.0	9.0
Q	BMWV-t	MT20	4.0	4.0
R	BS-t	MT20	3.0	6.0
S	BMWV-t	MT20	4.0	9.0
T	BMV1-p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	GROSS REACTION	DOWN	HORIZ	UPLIFT	BRG	BRG	IN-SX
T	2085	0	2085	0	0	0	5-8	5-8	5-8
L	2085	0	2085	0	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
T	1458	970	0	0	0
L	1458	970	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1:2 LENGTH OF C-Q, F-P, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	MAX. CSI (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 35	-91.8 -91.8 0.12 (1)	10.00	S-C	-189 46	0.10 (1)	
B-C	-2358 0	-91.8 -91.8 0.09 (1)	3.77	C-Q	-484 0	0.24 (1)	
C-D	-1996 0	-91.8 -91.8 0.02 (1)	4.11	Q-E	0 415	0.09 (1)	
D-E	-1996 0	-91.8 -91.8 0.02 (1)	4.11	E-P	0 315	0.05 (1)	
E-F	-1781 0	-91.8 -91.8 0.27 (1)	4.77	P-F	-502 0	0.35 (1)	
F-G	-1781 0	-91.8 -91.8 0.27 (1)	4.77	P-G	0 315	0.05 (1)	
G-H	-1996 0	-91.8 -91.8 0.02 (1)	4.11	O-G	0 415	0.09 (1)	
H-I	-1996 0	-91.8 -91.8 0.02 (1)	4.11	O-I	-484 0	0.24 (1)	
I-J	-2358 0	-91.8 -91.8 0.09 (1)	3.77	M-I	-189 46	0.10 (1)	
J-K	0 35	-91.8 -91.8 0.12 (1)	10.00	S-S	0 2021	0.45 (1)	
T-B	-2014 0	0.0 0.0 0.21 (1)	5.95	M-J	0 2021	0.45 (1)	
L-J	-2014 0	0.0 0.0 0.21 (1)	5.95				
T-S	0 0	-18.5 -18.5 0.19 (4)	10.00				
S-R	0 1995	-18.5 -18.5 0.42 (1)	10.00				
R-Q	0 1995	-18.5 -18.5 0.42 (1)	10.00				
Q-P	0 1629	-18.5 -18.5 0.32 (1)	10.00				
P-O	0 1629	-18.5 -18.5 0.32 (1)	10.00				
O-N	0 1995	-18.5 -18.5 0.42 (1)	10.00				
N-M	0 1995	-18.5 -18.5 0.42 (1)	10.00				
M-L	0 0	-18.5 -18.5 0.19 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.69/1.00 (I-J:1), BC=0.42/1.00 (M-O:1), WB=0.45/1.00 (J-M:1), SS=0.24/1.00 (J-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1087 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.87 (N) (INPUT = 1.00)



Structural component only
DWG# T-2007609

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408222	T8	2	1	TRUSS DESC.		

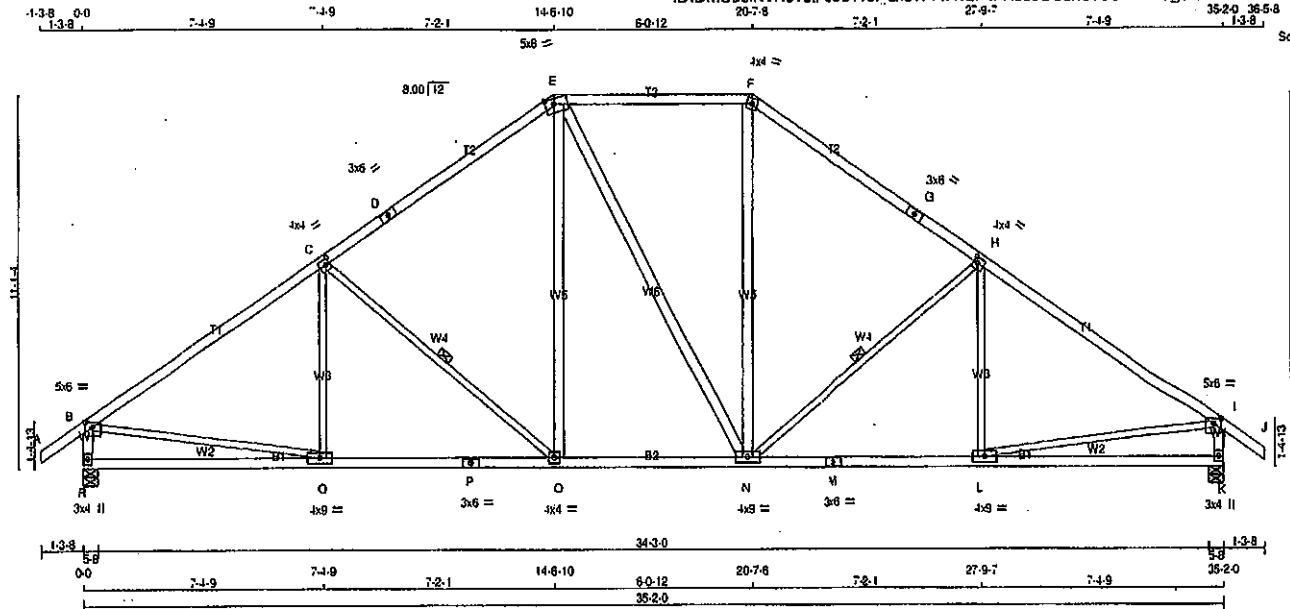
Tamarack Roof Truss, Burlington

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35-2-0 36-5-8

Scale: 3/16"=1'



TOTAL WEIGHT = 2 X 166 = 333 lb

LUMBER	CHORDS	SIZE	DRY	No.2
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	
D - E	2x4	DRY	No.2	
E - F	2x4	DRY	No.2	
F - G	2x4	DRY	No.2	
G - J	2x4	DRY	No.2	
J - B	2x4	DRY	No.2	
K - I	2x4	DRY	No.2	
R - P	2x4	DRY	No.2	
P - M	2x4	DRY	No.2	
M - K	2x4	DRY	No.2	
ALL WEBS	2x3	DRY	No.2	
EXCEPT				
O - E	2x4	DRY	No.2	
E - N	2x4	DRY	No.2	
N - F	2x4	DRY	No.2	

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
R	2085	0	2085	0
K	2085	0	2085	0

UNFACTORED REACTIONS

1ST LOASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1458	970	0	0	0	488	0
K	1458	970	0	0	0	488	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS				
MAX. FACTORED		FACTORED		VERT. LOAD		MAX. FACTORED		MAX. FACTORED		
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX	MAX	
	(LBS)	(PLF)	CS1 (LC)	UNBRAC			(LBS)	CS1 (LC)		
FR-TO		FROM	TO	LENGTH	FR-TO					
A-B	0 35	-91.8	-91.8	0.12 (1)	10.00	Q-C	-140 76	0.09 (1)		
B-C	-2357 0	-91.8	-91.8	0.89 (1)	3.45	C-O	-604 0	0.38 (1)		
C-D	-1888 0	-91.8	-91.8	0.78 (1)	3.91	O-E	0 499	0.09 (1)		
D-E	-1888 0	-91.8	-91.8	0.78 (1)	3.91	E-N	0 0	0.00 (1)		
E-F	-1538 0	-91.8	-91.8	0.47 (1)	4.74	N-F	0 500	0.08 (1)		
F-G	-1888 0	-91.8	-91.8	0.78 (1)	3.91	N-H	-604 0	0.38 (1)		
G-H	-1888 0	-91.8	-91.8	0.78 (1)	3.91	L-H	-140 75	0.09 (1)		
H-I	-2357 0	-91.8	-91.8	0.89 (1)	3.45	B-O	0 2020	0.45 (1)		
I-J	0 35	-91.8	-91.8	0.12 (1)	10.00	L-I	0 2019	0.45 (1)		
R-B	-2009 0	0.0	0.0	0.21 (1)	5.95					
K-I	-2009 0	0.0	0.0	0.21 (1)	5.95					
R-Q	0 0	-18.5	-18.5	0.25 (4)	10.00					
Q-P	0 1998	-18.5	-18.5	0.45 (1)	10.00					
P-O	0 1998	-18.5	-18.5	0.45 (1)	10.00					
O-N	0 1538	-18.5	-18.5	0.33 (1)	10.00					
N-M	0 1998	-18.5	-18.5	0.45 (1)	10.00					
M-L	0 1998	-18.5	-18.5	0.45 (1)	10.00					
L-K	0 0	-18.5	-18.5	0.24 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17)
CALCULATED VERT. DEFL.(LL) = L/999 (0.09)
ALLOWABLE DEFL.(TL) = L/360 (1.17)
CALCULATED VERT. DEFL.(TL) = L/999 (0.20)

CS1: TC=0.89(1.00) (B-C:1), BC=0.45(1.00) (O-Q:1), WB=0.45(1.00) (B-Q:1), SS1=0.27(1.00) (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1957 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (F) INPUT = 0.90
JSI METAL=0.68 (F) INPUT = 1.00



Structural component only
DWG# T-2007610

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	T9	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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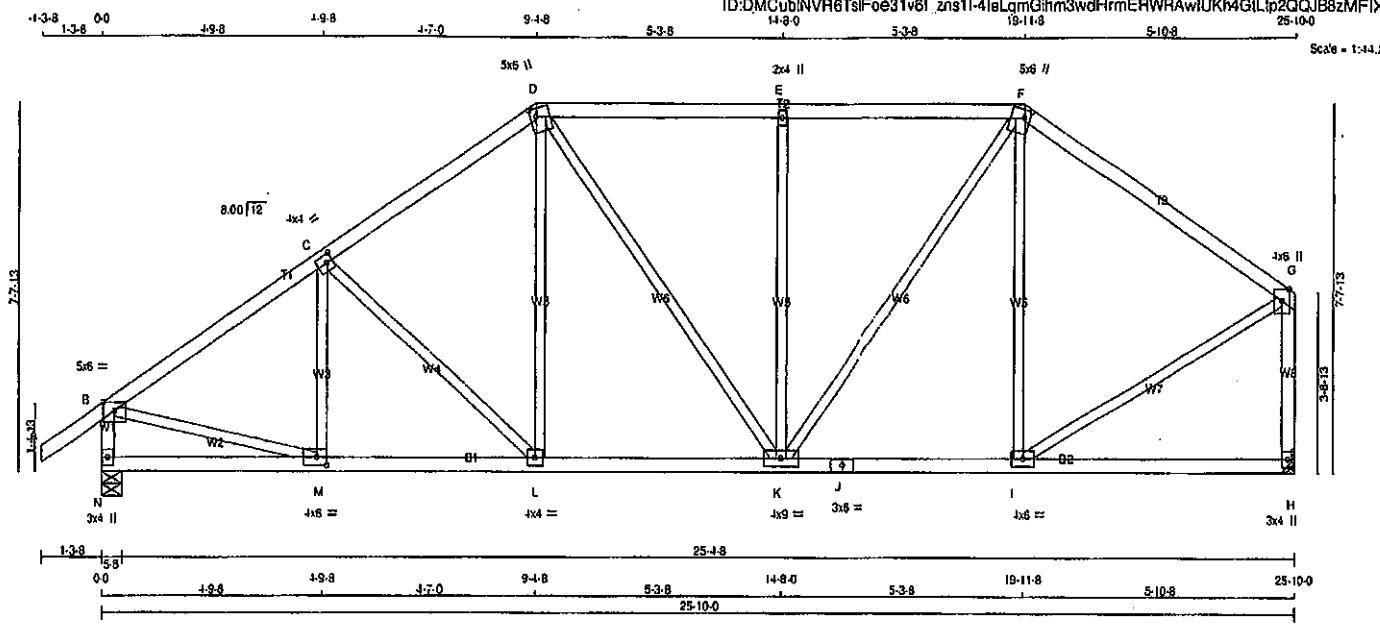
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-l	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMWW-l	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	8.0	2.50	1.50
H	TMVW+p	MT20	4.0	6.0	Edge	
J	BMVl+p	MT20	3.0	6.0		
K, N, O, P						
K	BMWW-l	MT20	5.0	6.0		
L	BMWWW-l	MT20	5.0	8.0		
M	BS-l	MT20	5.0	6.0		
Q	BMVl+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



Structural component only
DWG# T-2007611



TOTAL WEIGHT = 118 lb (M/F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - G	2x4	DRY No.2	SPF
N - B	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
N - J	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMVW-l	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	1.50
G	TMVW+p	MT20	4.0	8.0	Edge	
H	BMVl+p	MT20	3.0	4.0		
I	BMVW-l	MT20	4.0	8.0		
J	BS-l	MT20	3.0	6.0		
K	BMVW-w-l	MT20	4.0	9.0		
L	BMVW-w-l	MT20	4.0	4.0		
M	BMVW-l	MT20	4.0	8.0	2.00	2.50
N	BMVl+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	1550	1550	0	0
N	1650	1650	0	0
H	1424	1424	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	DEAD	SOIL
N	1094	731.0	0.0	0.0
H	1007	881.0	0.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LCI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LCI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 35	-91.8 -91.8	0.12 (1)	10.00	M-C	-217 3	0.07 (1)
B-C	-1609 0	-91.8 -91.8	0.29 (1)	4.93	C-L	-273 0	0.20 (1)
C-D	-1425 0	-91.8 -91.8	0.28 (1)	5.17	L-D	0 280	0.08 (1)
D-E	-1272 0	-91.8 -91.8	0.35 (1)	5.30	D-K	0 185	0.04 (1)
E-F	-1272 0	-91.8 -91.8	0.35 (1)	5.30	K-E	-595 0	0.84 (1)
F-G	-1077 0	-91.8 -91.8	0.43 (1)	5.52	K-F	0 655	0.15 (1)
N-B	-1511 0	0.0 0.0	0.18 (1)	6.88	F-M	-380 0	0.42 (1)
H-G	-1380 0	0.0 0.0	0.32 (1)	6.92	B-M	0 1396	0.31 (1)
					I-G	0 1031	0.23 (1)
N-M	0 0	-18.5 -18.5	0.09 (4)	10.00			
M-L	0 1362	-18.5 -18.5	0.27 (1)	10.00			
L-K	0 1164	-18.5 -18.5	0.24 (1)	10.00			
K-J	0 890	-18.5 -18.5	0.23 (1)	10.00			
J-I	0 890	-18.5 -18.5	0.23 (1)	10.00			
I-H	0 0	-18.5 -18.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.43/1.00 (F-G:1), BC=0.27/1.00 (L-M:1), WB=0.84/1.00 (E-K:1), SS=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

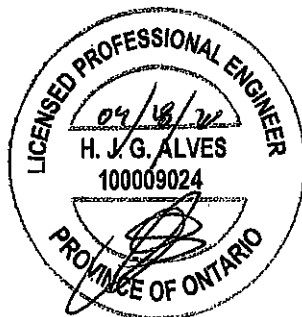
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 789 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (B) (INPUT = 0.90)
JSI METAL = 0.56 (G) (INPUT = 1.00)



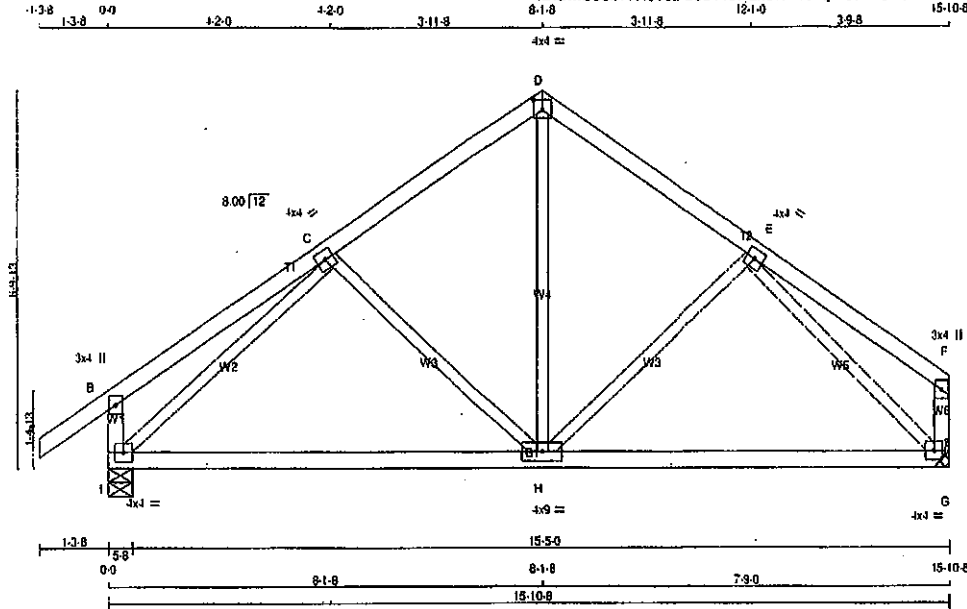
Structural component only
DWG# T-2007612.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408222	T11A	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TsiFoe31v6I zns11-4leLqmGihm3wdHmERWRAwIXHh2PIN5p2QQJB8zMFIX



Scale = 1:38.6

TOTAL WEIGHT = 2 X 60 = 133 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
I - B	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMV+p MT20	3.0	4.0		
C YMWV-i MT20	4.0	4.0		
D TTW-p MT20	4.0	4.0	2.25	2.00
E TMWV-i MT20	4.0	4.0		
F TMV+p MT20	3.0	4.0		
G BMWV-i MT20	4.0	4.0		
H BMWV-i MT20	4.0	4.0		
I BMWV-i MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
I	1001	0	1001	0
G	875	0	875	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	706	476	0	0	0	0	229	0
G	619	406	0	0	0	0	213	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 35	-91.8 -91.8 0.12 (1)	C-H	-228 0 0.12 (1)
B-C	0 24	-91.8 -91.8 0.24 (1)	H-D	0 449 0.10 (1)
C-D	-891 0	-91.8 -91.8 0.19 (1)	E-E	-166 0 0.09 (1)
D-E	-889 0	-91.8 -91.8 0.17 (1)	I-C	-988 0 0.50 (1)
E-F	0 24	-91.8 -91.8 0.22 (1)	E-G	-971 0 0.44 (1)
I-B	-289 0	0.0 0.0 0.03 (1)		
G-F	-125 0	0.0 0.0 0.01 (1)		
I-H	0 722	-18.5 -18.5 0.39 (4)		
H-G	0 677	-18.5 -18.5 0.39 (4)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.53")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.53")
CALCULATED VERT. DEFL. (TL) = L/999 (0.11")

CSI: TC=0.24/1.00 (B-C:1), BC=0.30/1.00 (H-I:4), WB=0.50/1.00 (C-I:1), SS=0.15/1.00 (C-D:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

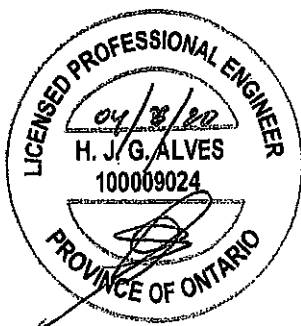
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1567 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (E) (INPUT = 0.90)
JSI METAL = 0.35 (C) (INPUT = 1.00)

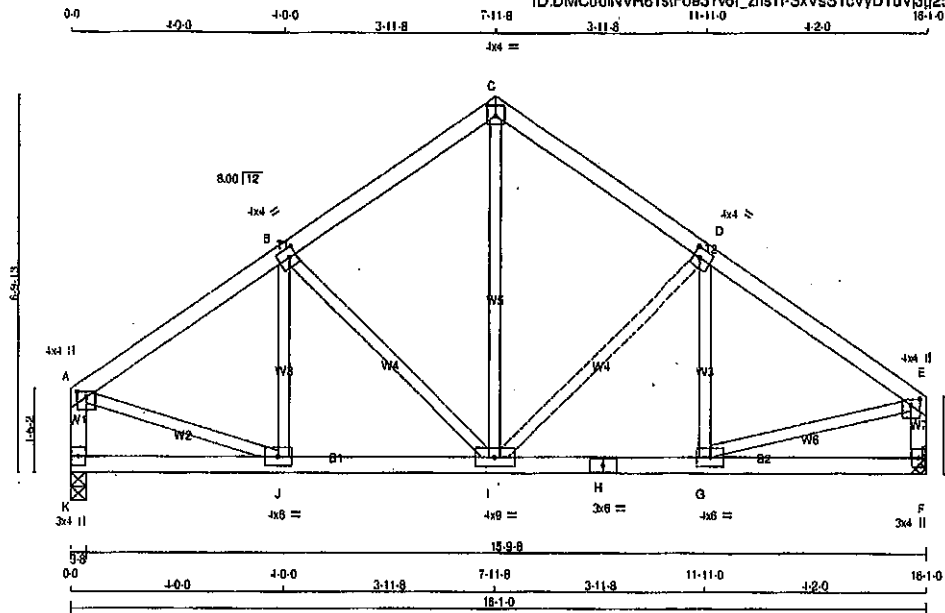


Structural component only
DWG# T-2007613

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T11B	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR6TstFoa31v6I_zns11-SxVsS1cVvDTdV3g25MsN0ihBxxWB?PWsQ4izMEMi



Scale = 1:38.5

TOTAL WEIGHT = 2 X 69 = 137 lb (M/F)

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
K - A	2x4	DRY	No.2
F - E	2x4	DRY	No.2
K - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
A	TMVW-p	MT20	4.0	4.0	1.25 2.00
B	TMVW-l	MT20	4.0	4.0	2.00 1.50
C	TTW-p	MT20	4.0	4.0	2.25 2.00
D	TMVW-l	MT20	4.0	4.0	2.00 1.50
E	TMVW-p	MT20	4.0	4.0	1.25 2.00
F	BMV1-p	MT20	3.0	4.0	
G	BMVW-l	MT20	4.0	6.0	
H	BS-l	MT20	3.0	6.0	
I	BMVW-l	MT20	4.0	9.0	
J	BMVW-l	MT20	4.0	6.0	
K	BMV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	VERT	HORZ	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
K	887	0	887	0	3-8
F	887	0	887	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS						
JT	1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD
K	627	411	0	0	0	216
F	627	411	0	0	0	216

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO				FR-TO			
A-B	-868	0	-91.8	J-B	-142	18	0.04
B-C	-697	0	-91.8	B-I	-263	0	0.13
C-D	-686	0	-91.8	I-C	0	481	0.10
D-E	-891	0	-91.8	I-D	292	0	0.15
K-A	-855	0	0.0	G-D	-117	28	0.03
F-E	-853	0	0.0	A-J	0	772	0.17
				G-E	0	787	0.18
K-J	0	0	-18.5				
J-I	0	740	-18.5				
I-H	0	781	-18.5				
H-G	0	781	-18.5				
G-F	0	0	-18.5				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2018 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.20/1.00 (D-E:1), BC=0.15/1.00 (G-I:1), WB=0.18/1.00 (E-G:1), SS=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

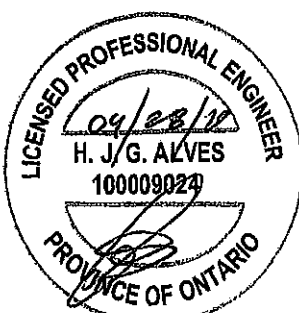
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 518 354 1687 768 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (E) (INPUT = 0.80)
JSI METAL = 0.25 (H) (INPUT = 1.00)



Structural component only
DWG# T-2007631

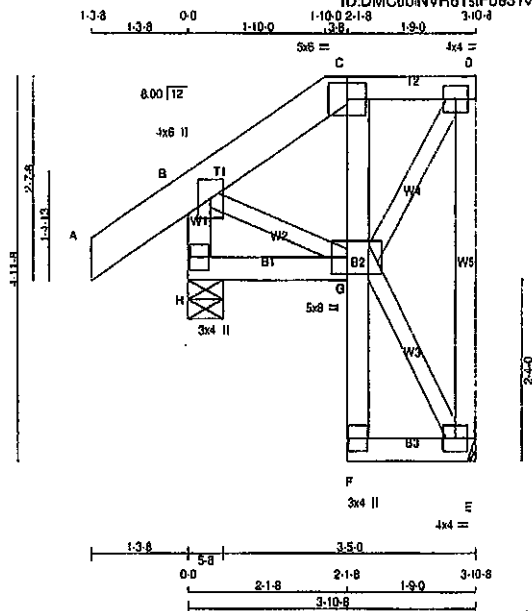
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408222	T12S	1	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstF0e31v6l zns11-0hm5FSlyDNJdla?9MsYvGLQuJvqYLOs6VkvQQ0zMFIV

Scale = 1:27.5



TOTAL WEIGHT = 31 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DRY
A - C	2x6	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
H - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
F - C	2x4	DRY	No.2
F - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWV-p	MT20	4.0	6.0	
C	TTV-m	MT20	5.0	6.0	
D	TMWV-l	MT20	4.0	4.0	
E	BMVW-l	MT20	4.0	4.0	
F	BMV-p	MT20	3.0	4.0	
G	BVMWVW-l	MT20	5.0	8.0	2.50 2.50
H	BMV-l-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
E	214	0	214	0	0
H	344	0	344	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	161	99	0	0	0	0	52	0
H	241	172	0	0	0	0	89	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		WEBS	
MEMB.	MAX. FACTORED	MEMB.	MAX. FACTORED
FR-TO	FORCE (LBS)	FR-TO	FORCE (LBS)
A-B	0 36	G-D	0 139
B-C	-105 0	B-G	0 96
C-D	-79 0	G-E	-3 0
E-D	-198 0		
H-B	-323 0		
H-G	0 0		
F-G	0 17		
G-C	-119 0		
F-E	0 2		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.08:1.00 (D:E=1), BC=0.03:1.00 (G:H=1), WB=0.03:1.00 (D:G=1), SSI=0.07:1.00 (A:B=5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

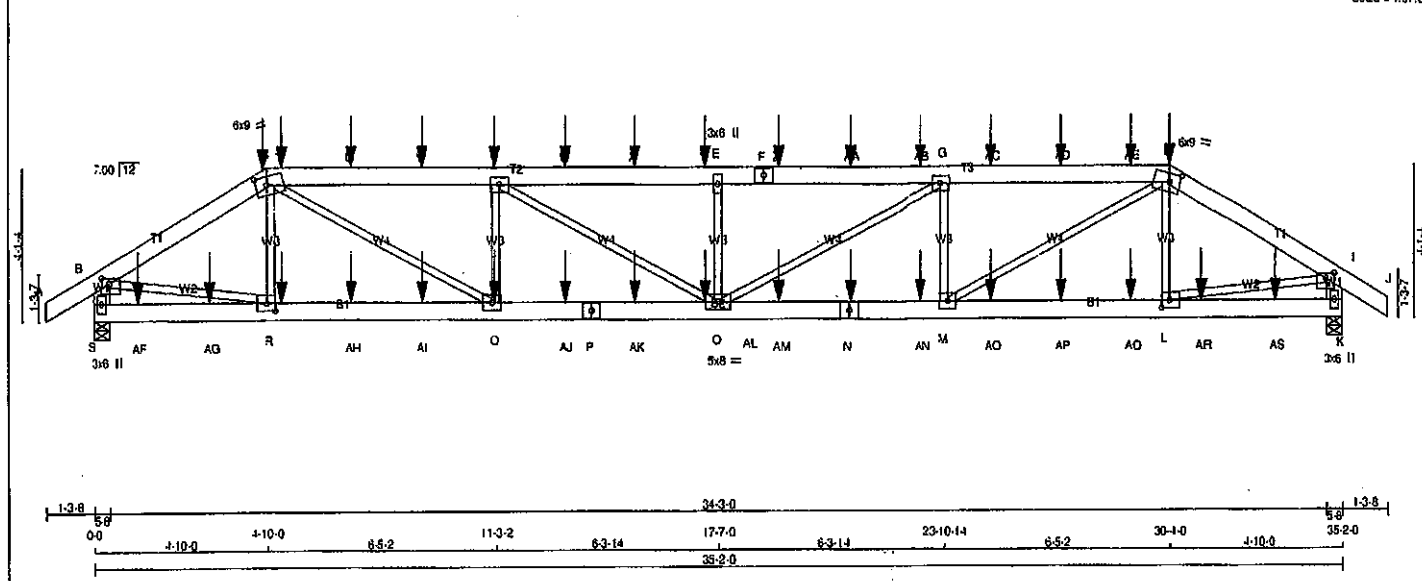
JSI GRIP = 0.19 (B) INPUT = 0.90 1
JSI METAL = 0.11 (B) INPUT = 1.00 1



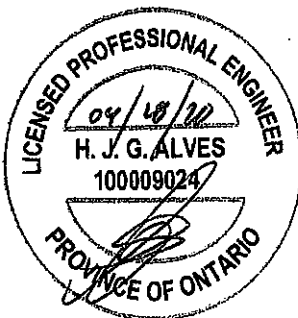
Structural component only
DWG# T-2007615

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T20	1	2	GREEN PARK HOMES	

Yamarack Roof Truss, Burlington	Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 16:04:02 2020 Page 1	ID:DMCubINVR6TsIFoe31v6I zns11-x73EgNd7jXbU6ssscDdbPaw0PaBNFZ7Y1IB c8ZMEMH	352.0 18.5 8
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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x6 DRY No.2 SPF C - F 2x6 DRY No.2 SPF F - H 2x6 DRY No.2 SPF H - J 2x6 DRY No.2 SPF S - B 2x6 DRY No.2 SPF K - I 2x6 DRY No.2 SPF S - P 2x6 DRY No.2 SPF P - N 2x6 DRY No.2 SPF N - K 2x6 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REGR GROSS REACTION GROSS REACTION BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX S 3423 0 3423 0 0 5-8 5-8 K 3365 0 3365 0 0 5-8 5-8 UNFACTORED REACTIONS 1ST LCASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL S 2416 1600 0 0 0 0 0 818 0 0 0 K 2376 1573 0 0 0 0 0 804 0 0 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MAX. FACTORED FORCE VERT. LOAD LC1 MAX. MAX. MEMB. FORCE MAX. FORCE (LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC) FR-TO A-B 0 33 -91.8 -91.8 0.04 (1) 10.00 R-C -431 0 0.05 (1) B-C -4681 0 -91.8 -91.8 0.13 (1) 5.23 C-Q 0 3534 0.44 (1) C-T -7102 0 -91.8 -91.8 0.31 (1) 4.26 Q-D -1565 0 0.19 (1) T-U -7102 0 -91.8 -91.8 0.31 (1) 4.26 D-O 0 1048 0.13 (1) U-V -7102 0 -91.8 -91.8 0.31 (1) 4.26 O-E -884 0 0.10 (1) V-D -7102 0 -91.8 -91.8 0.31 (1) 4.26 E-G 0 1065 0.13 (1) D-W -8010 0 -91.8 -91.8 0.32 (1) 4.05 M-H -1573 0 0.19 (1) W-X -8010 0 -91.8 -91.8 0.32 (1) 4.05 M-H 0 3572 0.44 (1) X-Y -8010 0 -91.8 -91.8 0.32 (1) 4.05 L-H -421 10 0.05 (1) Y-E -8010 0 -91.8 -91.8 0.32 (1) 4.05 R-R 0 4088 0.51 (1) E-F -8010 0 -91.8 -91.8 0.33 (1) 4.03 L-I 0 4022 0.50 (1) F-Z -8010 0 -91.8 -91.8 0.33 (1) 4.03 Z-AA -8010 0 -91.8 -91.8 0.33 (1) 4.03 AA-AB -8010 0 -91.8 -91.8 0.33 (1) 4.03 AB-G -8010 0 -91.8 -91.8 0.33 (1) 4.03 G-AC -7070 0 -91.8 -91.8 0.30 (1) 4.27 AC-AD -7070 0 -91.8 -91.8 0.30 (1) 4.27 AD-AE -7070 0 -91.8 -91.8 0.30 (1) 4.27 AE-H -7070 0 -91.8 -91.8 0.30 (1) 4.27 H-I -4808 0 -91.8 -91.8 0.13 (1) 5.28 I-J 0 33 -91.8 -91.8 0.04 (1) 10.00 S-B -3367 0 0.0 0.0 0.12 (1) 7.58 K-I -3318 0 0.0 0.0 0.12 (1) 7.83 S-AF 0 0 -18.5 -18.5 0.08 (4) 10.00 AF-AG 0 0 -18.5 -18.5 0.08 (4) 10.00 AG-R 0 0 -18.5 -18.5 0.08 (4) 10.00 R-AH 0 4024 -18.5 -18.5 0.31 (1) 10.00 AH-AI 0 4024 -18.5 -18.5 0.31 (1) 10.00 AI-O 0 4024 -18.5 -18.5 0.31 (1) 10.00 O-AJ 0 7102 -18.5 -18.5 0.52 (1) 10.00 AJ-P 0 7102 -18.5 -18.5 0.52 (1) 10.00 P-AK 0 7102 -18.5 -18.5 0.52 (1) 10.00 AK-AL 0 7102 -18.5 -18.5 0.52 (1) 10.00 AL-O 0 7102 -18.5 -18.5 0.52 (1) 10.00 O-AM 0 7070 -18.5 -18.5 0.52 (1) 10.00 AM-N 0 7070 -18.5 -18.5 0.52 (1) 10.00 N-AN 0 7070 -18.5 -18.5 0.52 (1) 10.00 AN-M 0 7070 -18.5 -18.5 0.52 (1) 10.00 M-AO 0 3960 -18.5 -18.5 0.30 (1) 10.00 AO-AP 0 3960 -18.5 -18.5 0.30 (1) 10.00 AP-AQ 0 3960 -18.5 -18.5 0.30 (1) 10.00 AQ-L 0 3960 -18.5 -18.5 0.30 (1) 10.00				DESIGN CRITERIA *** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE SPECIFIED LOADS: TOP CH. LL = 25.8 PSF OL = 6.0 PSF BOT CH. LL = 0.0 PSF OL = 7.4 PSF TOTAL LOAD = 39.0 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 *** NON STANDARD GIRDER *** ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, CBC 2012, ABC 2019 - PART 9 OF CBC 2012 (2019 AMENDMENT) - CSA 088-09, CSA 086-14 - TPIC 2011, TPIC 2014 55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (1.17") CALCULATED VERT. DEFL.(LL) = L/999 (0.20") ALLOWABLE DEFL.(TL) = L/360 (1.17") CALCULATED VERT. DEFL.(TL) = L/999 (0.37") CSI: TC=0.33/1.00 (E-G:1), BC=0.52/1.00 (O-Q:1), WB=0.51/1.00 (R-R:1), SSI=0.18/1.00 (G-H:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 788 1987 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.89 (C) (INPUT = 0.90) JSI METAL= 0.68 (P) (INPUT = 1.00)			
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Structural component only
DWG# T-2007632 1/2

CONTINUED ON PAGE 2

JOB NAME 408223	TRUSS NAME T20	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:04:02 2020 Page 2	
				ID:DMCubINVR6TslF0a31v6I zns1I-x73EqNd7IXbU6988ccD0Paw0PaBNFZ7YIIB c8zMEMh	

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TTWW-m	MT20	6.0	9.0	2.75	3.50
D	TMVW-l	MT20	5.0	6.0		
E	TMVW-w	MT20	3.0	6.0		
F	TS-l	MT20	5.0	6.0		
G	TMVW-l	MT20	5.0	6.0		
H	TTWW-m	MT20	6.0	9.0	2.75	3.50
I	TMVW-p	MT20	5.0	6.0	1.75	2.75
K	BMVl+p	MT20	3.0	6.0		
L	BMVW-l	MT20	5.0	6.0	2.50	2.75
M	BMVW-l	MT20	5.0	6.0		
N	BS-l	MT20	5.0	6.0		
O	BMVW-l	MT20	5.0	6.0		
P	BS-l	MT20	5.0	6.0		
Q	BMVW-l	MT20	5.0	6.0		
R	BMVW-l	MT20	5.0	6.0	2.50	2.75
S	BMVl+p	MT20	3.0	6.0		

LOADING

TOTAL LOAD CASES: (4)

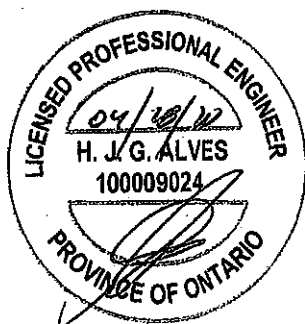
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED UNBRAC. LENGTH FR-TO	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)
FR-TO		FROM TO					
L-AR	0 0	-18.5 -18.5	0.06 (4)		10.00		
AR-AS	0 0	-18.5 -18.5	0.06 (4)		10.00		
AS-K	0 0	-18.5 -18.5	0.06 (4)		10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN
C	4-10-0	-60	-67	---	FRONT	VERT	DEAD	---	C1
C	4-10-0	-311	-311	---	FRONT	VERT	SNOW	---	C1
D	11-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
H	30-4-0	-60	-67	---	FRONT	VERT	DEAD	---	C1
H	30-4-0	-311	-311	---	FRONT	VERT	SNOW	---	C1
N	21-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
O	11-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
R	5-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
T	5-2-12	-138	-138	---	FRONT	VERT	TOTAL	---	C1
U	7-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	9-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	13-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	15-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	17-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	19-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	21-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	23-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	25-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	27-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AE	29-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AF	1-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AG	3-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AH	7-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AI	9-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AJ	13-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AK	15-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AL	17-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AM	19-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AN	23-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AO	25-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AP	27-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AQ	29-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AR	31-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AS	33-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



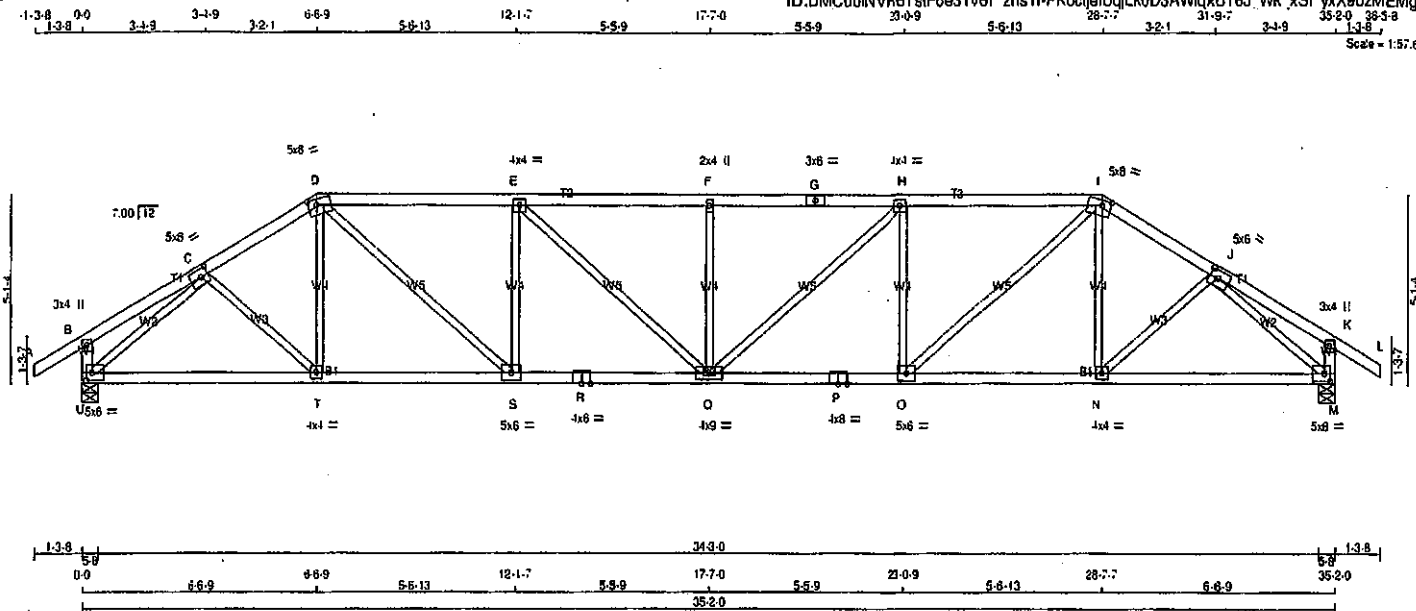
Structural component only
DWG# T-2007632

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T21	1	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 10:04:03 2020 Page 1

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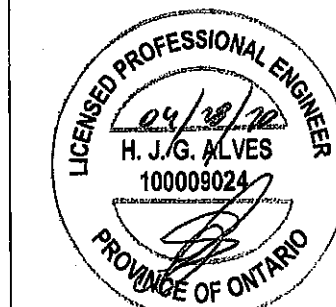


LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
U - R	2x4	DRY	No.2
P - P	2x4	DRY	No.2
R - M	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (Tables in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMVW-i	MT20	5.0	6.0	2.50	2.50
D TTWW-m	MT20	5.0	8.0	1.75	3.00
E TMVW-i	MT20	4.0	4.0		
F TMVW-w	MT20	2.0	4.0		
G TS-i	MT20	3.0	6.0		
H TMVW-i	MT20	4.0	4.0		
I TTWW-m	MT20	5.0	8.0	1.75	3.00
J TMVW-i	MT20	5.0	6.0	2.50	2.50
K TMV+p	MT20	3.0	4.0		
M BMVW-i	MT20	5.0	6.0	2.50	2.00
N BMVW-i	MT20	4.0	4.0		
O BMVW-i	MT20	5.0	6.0		
P BS-i	MT20	4.0	6.0		
Q BMVW-i	MT20	4.0	6.0		
R BS-i	MT20	4.0	6.0		
S BMVW-i	MT20	5.0	6.0		
T BMVW-i	MT20	4.0	4.0		
U BMVW-i	MT20	5.0	6.0	2.50	2.00



Structural component only
DWG# T-2007633

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQD
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG
U	2084	0	2084	0	0	5-8	5-8
M	2084	0	2084	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1457	970	0	0:0	0	0	488	0
M	1457	970	0	0:0	0	0	488	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 32	-91.8 -91.8	0.12 (1)	10.00	C-T	0 213 0.05 (1)	
B-C	0 13	-91.8 -91.8	0.12 (1)	10.00	T-D	-8 89 0.02 (4)	
C-D	-2527 0	-91.8 -91.8	0.20 (1)	4.18	D-S	0 1447 0.33 (1)	
D-E	-3284 0	-91.8 -91.8	0.66 (1)	3.30	S-E	-853 0 0.33 (1)	
E-F	-3585 0	-91.8 -91.8	0.70 (1)	3.12	E-Q	0 428 0.10 (1)	
F-G	-3585 -0	-91.8 -91.8	0.70 (1)	3.12	Q-F	-482 0 0.18 (1)	
G-H	-3585 0	-91.8 -91.8	0.70 (1)	3.12	H-Q	0 428 0.10 (1)	
H-I	-3264 0	-91.8 -91.8	0.66 (1)	3.30	Q-H	-853 0 0.33 (1)	
I-J	-2527 0	-91.8 -91.8	0.20 (1)	4.18	O-I	0 1447 0.33 (1)	
J-K	0 13	-91.8 -91.8	0.12 (1)	10.00	N-J	-8 89 0.02 (4)	
K-L	0 32	-91.8 -91.8	0.12 (1)	10.00	N-J	0 213 0.05 (1)	
U-B	-249 -0	0.0 0.0	0.03 (1)	7.81	U-C	-2871 0 0.82 (1)	
M-K	-249 0	0.0 0.0	0.03 (1)	7.81	J-M	-2871 0 0.82 (1)	
U-T	0 2005	-18.5 -18.5	0.42 (1)	10.00			
T-S	0 2187	-18.5 -18.5	0.44 (1)	10.00			
S-R	0 3284	-18.5 -18.5	0.58 (1)	10.00			
R-Q	0 3284	-18.5 -18.5	0.68 (1)	10.00			
Q-P	0 3284	-18.5 -18.5	0.58 (1)	10.00			
P-O	0 3284	-18.5 -18.5	0.58 (1)	10.00			
O-N	0 2187	-18.5 -18.5	0.44 (1)	10.00			
N-M	0 2005	-18.5 -18.5	0.42 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 886-09, CSA 886-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.39")

CS: TC=0.70/1.00 (E-F:1), BC=0.58/1.00 (Q-S:1), WB=0.82/1.00 (J-M:1), SS=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX	MIN	MAX
MT20	618	354
	1667	788
	1987	1658

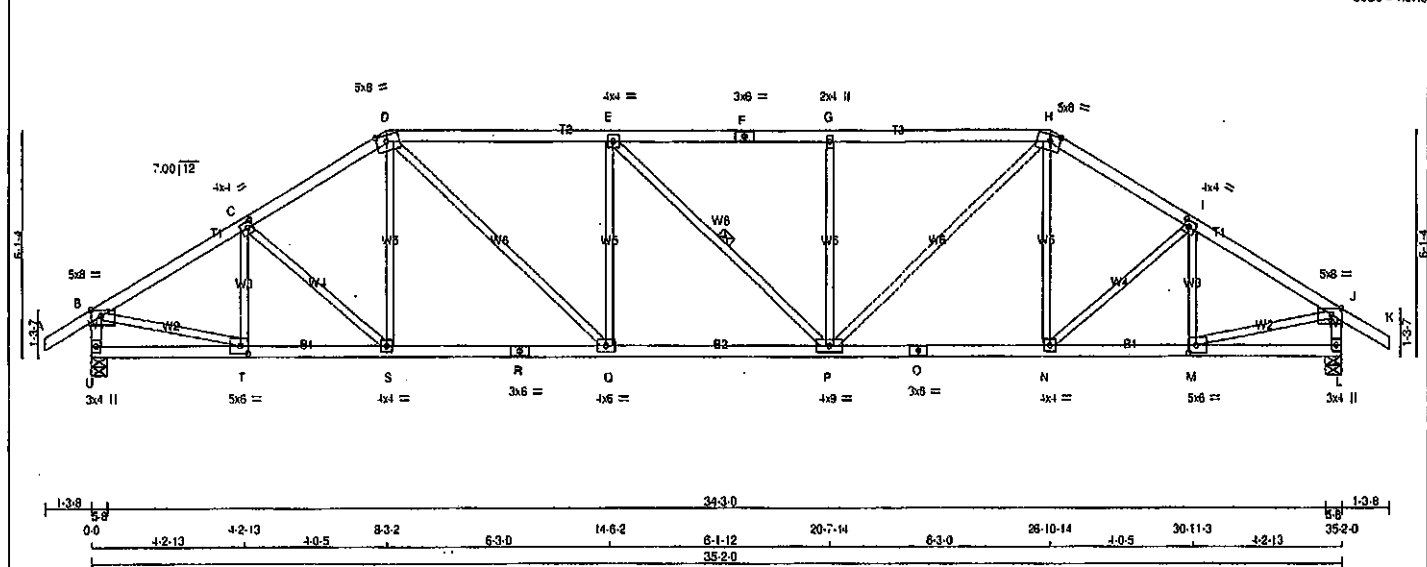
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRF = 0.90 (M) (INPUT = 0.90)
JSI METAL = 0.74 (R) (INPUT = 1.00)

JOB NAME 408223	TRUSS NAME T22	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWGNO.
Famarack Roof Truss, Burlington				TRUSS DESC.	

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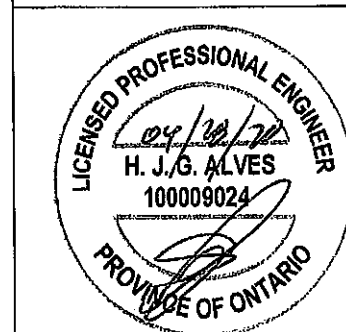


LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
E - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - K	2x4	DRY No.2	SPF
U - B	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
U - R	2x4	DRY No.2	SPF
R - O	2x4	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2
 EXCEPT
 DRY: SEASONED LUMBER.

PLATES (flats in inches)			
JT	TYPE	PLATES	W LEN Y X
B	TMVW-p	MT20	5.0 8.0 Edge 3.50
C	TMVW-t	MT20	4.0 4.0 2.00 1.75
D	TTWV-m	MT20	5.0 8.0 2.00 3.50
E	TMVW-t	MT20	4.0 4.0
F	TS-t	MT20	3.0 6.0
G	TMVW-w	MT20	2.0 4.0
H	TTWV-m	MT20	5.0 8.0 2.00 3.50
I	TMVW-t	MT20	4.0 4.0 2.00 1.75
J	TMVW-p	MT20	5.0 8.0 Edge 3.50
L	BMV-t-p	MT20	3.0 4.0
M	BMVW-t	MT20	5.0 6.0 2.50 2.50
N	BMVW-t	MT20	4.0 4.0
O	BS-t	MT20	3.0 6.0
P	BMVW-t	MT20	4.0 9.0
Q	BMVW-t	MT20	4.0 6.0
R	BS-t	MT20	3.0 6.0
S	BMVW-t	MT20	4.0 4.0
T	BMVW-t	MT20	5.0 6.0 2.50 2.50
U	BMV-t-p	MT20	3.0 4.0

Edge - INDICATES REFERENCE CORNER OF PLATE
 TOUCHES EDGE OF CHORD.



Structural component only
 DWG# T-2007634

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
U	2064	0	2064	0
L	2064	0	2064	0

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1457	970	0	0	0	488	0
L	1457	970	0	0	0	488	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.37 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
 THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	MAX.	MEMB.	MAX. FACTORED	FACTORED	MAX.
	FORCE	VERT. LOAD	CS1 (LC)		FORCE	CS1 (LC)	
	(LBS)	(PLF)			(LBS)		
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 32	-91.8	-91.8 0.12 (1)	10.00	T-C	-428	0 0.10 (1)
B-C	-2468	0	-91.8 -91.8 0.35 (1)	4.07	C-S	-39	0 0.02 (1)
C-D	-2478	0	-91.8 -91.8 0.34 (1)	4.08	S-D	0 131	0.04 (4)
D-E	-2910	0	-91.8 -91.8 0.75 (1)	3.37	D-Q	0 1076	0.24 (1)
E-F	-2809	0	-91.8 -91.8 0.74 (1)	3.38	Q-E	-817	0 0.36 (1)
F-G	-2809	0	-91.8 -91.8 0.74 (1)	3.38	E-P	-2	0 0.00 (1)
G-H	-2809	0	-91.8 -91.8 0.75 (1)	3.38	P-G	-817	0 0.36 (1)
H-I	-2478	0	-91.8 -91.8 0.34 (1)	4.08	P-H	0 1073	0.24 (1)
I-J	-2468	0	-91.8 -91.8 0.35 (1)	4.07	N-H	0 132	0.04 (4)
J-K	0 32	-91.8	-91.8 0.12 (1)	10.00	N-I	-39	0 0.02 (1)
U-B	-2025	0	0.0 0.0 0.21 (1)	5.94	M-I	-428	0 0.10 (1)
L-J	-2025	0	0.0 0.0 0.21 (1)	5.94	B-T	0 2205	0.50 (1)
					M-J	0 2205	0.50 (1)
U-T	0 0	-18.5	-18.5 0.07 (4)	10.00			
T-S	0 2149	-18.5	-18.5 0.41 (1)	10.00			
S-R	0 2122	-18.5	-18.5 0.41 (1)	10.00			
R-O	0 2122	-18.5	-18.5 0.41 (1)	10.00			
Q-P	0 2910	-18.5	-18.5 0.54 (1)	10.00			
P-O	0 2122	-18.5	-18.5 0.41 (1)	10.00			
O-N	0 2122	-18.5	-18.5 0.41 (1)	10.00			
N-M	0 2149	-18.5	-18.5 0.41 (1)	10.00			
M-L	0 0	-18.5	-18.5 0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.8 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE
 OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
 SMALL BUILDING REQUIREMENTS OF PART 9,
 NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, CBC 2012, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 08-09, CSA 08-14
 - TFC 2011, TFC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN
 LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF
 LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
 ALLOWABLE DEFL.(TL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CS1: TC=0.75/1.00 (D-E:1), BC=0.54/1.00 (P-Q:1),
 WB=0.50/1.00 (B-T:1), SS=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
 RESPONSIBLE FOR QUALITY CONTROL IN THE
 TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1867 768 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

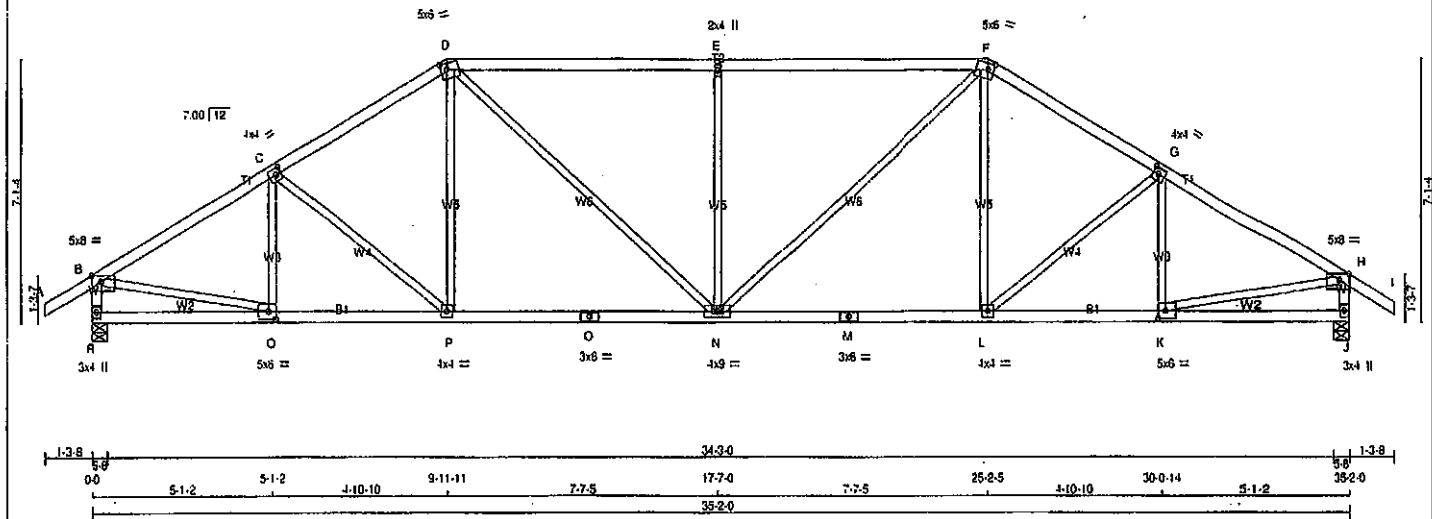
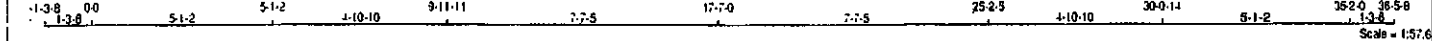
JSI GRIP = 0.89 (T) (INPUT = 0.90)
 JSI METAL = 0.70 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T23	1	1	GREEN PARK HOMES	
				TRUSS DESC.	

Lamarack Roof Truss, Burlington

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ID:DMCubINVR6TsiFoe31v6l_zns1l-pvJivkgeml5wbUyrelXZQ5aqCaVBjG8gw9BlvzMEMd



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
I - B	2x4	DRY	No.2	SPF	
B - H	2x4	DRY	No.2	SPF	
H - O	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY; SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMVW-l	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	2.00	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	2.00	2.00
G	TMVW-l	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	5.0	8.0	Edge	3.50
J	BMV1-p	MT20	3.0	4.0		
K	BMVW-l	MT20	5.0	8.0	2.50	2.50
L	BMVW-l	MT20	4.0	4.0		
M	BS-l	MT20	3.0	6.0		
N	BMVW-w	MT20	4.0	9.0		
O	BS-l	MT20	3.0	6.0		
P	BMVW-l	MT20	4.0	4.0		
Q	BMVW-l	MT20	5.0	8.0	2.50	2.50
R	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
R	2064	0	2064	0
J	2064	0	2064	0

UNFACTORED REACTIONS	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
R	1457	970 0
J	1457	970 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)									
CHORDS					WEBS						
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (PLF)	MAX. CS (I/C)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (I/C)			
FR-TO		FROM TO				FR-TO					
A-B	0 32	-91.8	-91.8	0.12 (1)	10.00	Q-C	-335 0	0.09 (1)			
B-C	-2537 0	-91.8	-91.8	0.39 (1)	4.00	C-P	-216 0	0.16 (1)			
C-D	-2398 0	-91.8	-91.8	0.37 (1)	4.11	P-D	0 258	0.06 (4)			
D-E	-2809 0	-91.8	-91.8	0.88 (1)	3.16	D-N	0 -752	0.17 (1)			
E-F	-2809 0	-91.8	-91.8	0.89 (1)	3.16	N-E	-880 0	0.75 (1)			
F-G	-2398 0	-91.8	-91.8	0.37 (1)	4.11	N-F	0 752	0.17 (1)			
G-H	-2537 0	-91.8	-91.8	0.39 (1)	4.00	L-F	0 258	0.06 (4)			
H-I	0 32	-91.8	-91.8	0.12 (1)	10.00	L-G	-216 0	0.16 (1)			
I-B	-2020 0	0.0	0.0	0.21 (1)	5.94	K-G	-335 0	0.09 (1)			
J-H	-2020 0	0.0	0.0	0.21 (1)	5.94	B-Q	0 2253	0.51 (1)			
						K-H	0 2253	0.51 (1)			
R-Q	0 0	-18.5	-18.5	0.10 (4)	10.00						
Q-P	0 2213	-18.5	-18.5	0.45 (1)	10.00						
P-O	0 2048	-18.5	-18.5	0.44 (1)	10.00						
O-N	0 2048	-18.5	-18.5	0.44 (1)	10.00						
N-M	0 2048	-18.5	-18.5	0.44 (1)	10.00						
M-L	0 2048	-18.5	-18.5	0.44 (1)	10.00						
L-K	0 2213	-18.5	-18.5	0.45 (1)	10.00						
K-J	0 0	-18.5	-18.5	0.10 (4)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.08:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL)= L/999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL)= L/999 (0.27")

CSI: TC=0.88:1.00 (E-F:1), BC=0.45:1.00 (K-L:1),
WB=0.75:1.00 (E-N:1), SS=0.34:1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.72 (M) (INPUT = 1.00)

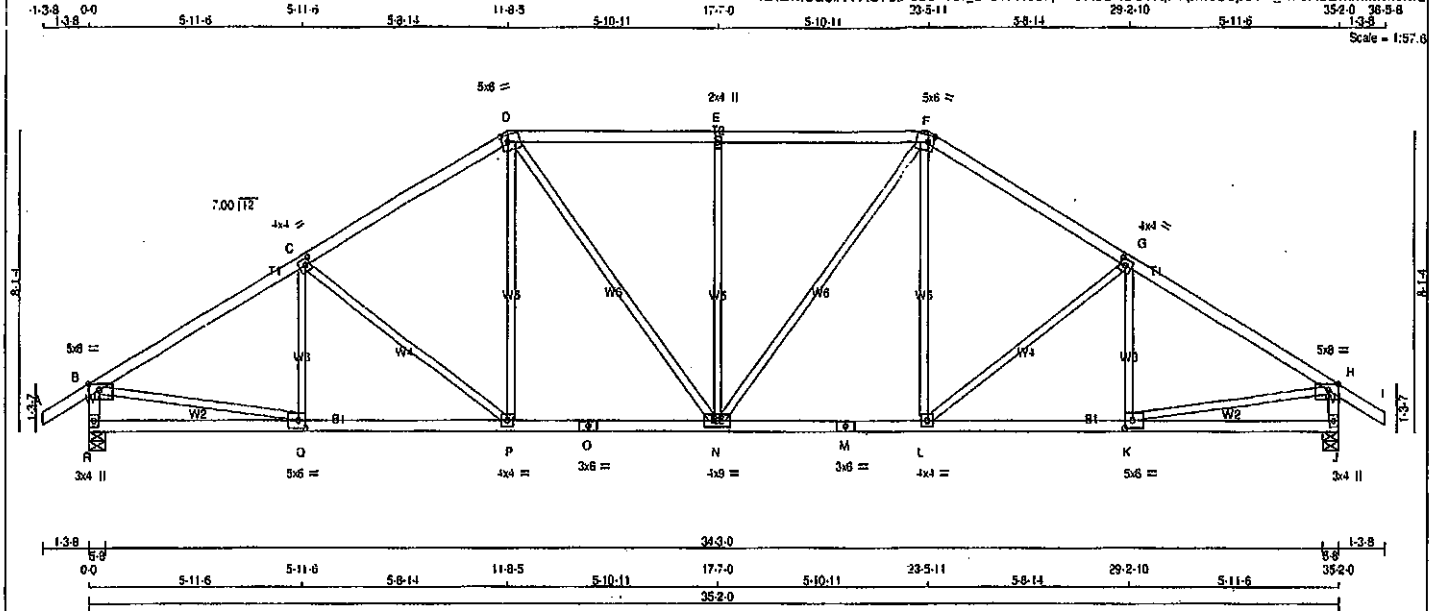


Structural component only
DWG# T-2007635

JOB NAME 408223	TRUSS NAME T24	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Jamarack Roof Truss, Burlington

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352.0 36.8-8
1.38



TOTAL WEIGHT = 151 LB
[MIP]

LUMBER

N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	Edge	3.50
C TMVW-t	MT20	4.0	4.0	2.00	1.75
D TTWW-m	MT20	5.0	6.0	2.25	2.00
E TMVW-w	MT20	2.0	4.0		
F TTWW-m	MT20	5.0	6.0	2.25	2.00
G TMVW-t	MT20	4.0	4.0	2.00	1.75
H TMVW-p	MT20	5.0	8.0	Edge	3.50
J BMV1-p	MT20	3.0	4.0		
K BMVW-t	MT20	5.0	6.0	2.50	2.50
L BMVW-t	MT20	4.0	4.0		
M BS-t	MT20	3.0	6.0		
N BMVW-t	MT20	4.0	4.0	9.0	
O BS-t	MT20	3.0	6.0		
P BMVW-t	MT20	4.0	4.0		
Q BMVW-t	MT20	5.0	6.0	2.50	2.50
R BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING DESIGNER									
BEARINGS									
		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
R		2084	0	2084	0	0	5-8	5-8	
J		2084	0	2084	0	0	5-8	5-8	

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERMALIVE	WIND	DEAD	SOIL
R	1457	970	0	0	0	488	0
J	1457	970	0	0	0	488	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. UNBRACED LENGTH (FT)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 32	-91.8 -91.8 0.12 (1)	10.00	Q-C	-252 12	0.08 (1)	
B-C	-2575 0	-91.8 -91.8 0.54 (1)	3.81	C-P	-388 0	0.41 (1)	
C-D	-2283 0	-91.8 -91.8 0.50 (1)	4.05	P-D	0 345	0.08 (1)	
D-E	-2238 0	-91.8 -91.8 0.49 (1)	4.08	D-N	0 480	0.11 (1)	
E-F	-2236 0	-91.8 -91.8 0.49 (1)	4.08	N-E	-682 0	0.84 (1)	
F-G	-2283 0	-91.8 -91.8 0.50 (1)	4.05	N-F	0 480	0.11 (1)	
G-H	-2575 0	-91.8 -91.8 0.54 (1)	3.81	L-F	0 345	0.08 (1)	
H-I	0 32	-91.8 -91.8 0.12 (1)	10.00	L-G	-388 0	0.41 (1)	
R-B	-2016 0	0.0 0.0 0.21 (1)	5.94	K-G	-252 12	0.08 (1)	
J-H	-2016 0	0.0 0.0 0.21 (1)	5.94	B-Q	0 2281	0.51 (1)	
R-Q	0 0	-18.5 -18.5 0.14 (4)	10.00	K-H	0 2281	0.51 (1)	
Q-P	0 2251	-18.5 -18.5 0.43 (1)	10.00				
P-O	0 1947	-18.5 -18.5 0.38 (1)	10.00				
O-N	0 1947	-18.5 -18.5 0.38 (1)	10.00				
N-M	0 1947	-18.5 -18.5 0.38 (1)	10.00				
M-L	0 1947	-18.5 -18.5 0.38 (1)	10.00				
L-K	0 2251	-18.5 -18.5 0.43 (1)	10.00				
K-J	0 0	-18.5 -18.5 0.14 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF.(LL) = L/360 (1.17")
CALCULATED VERT. DEF.(LL) = L/999 (0.12")
ALLOWABLE DEF.(TL) = L/360 (1.17")
CALCULATED VERT. DEF.(TL) = L/999 (0.22")
CSI: TC=0.54/1.00 (B-C:1), BC=0.43/1.00 (P-Q:1),
WB=0.84/1.00 (E-N:1), SS=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (Q) (INPUT = 0.90)
JSI METAL = 0.61 (M) (INPUT = 1.00)

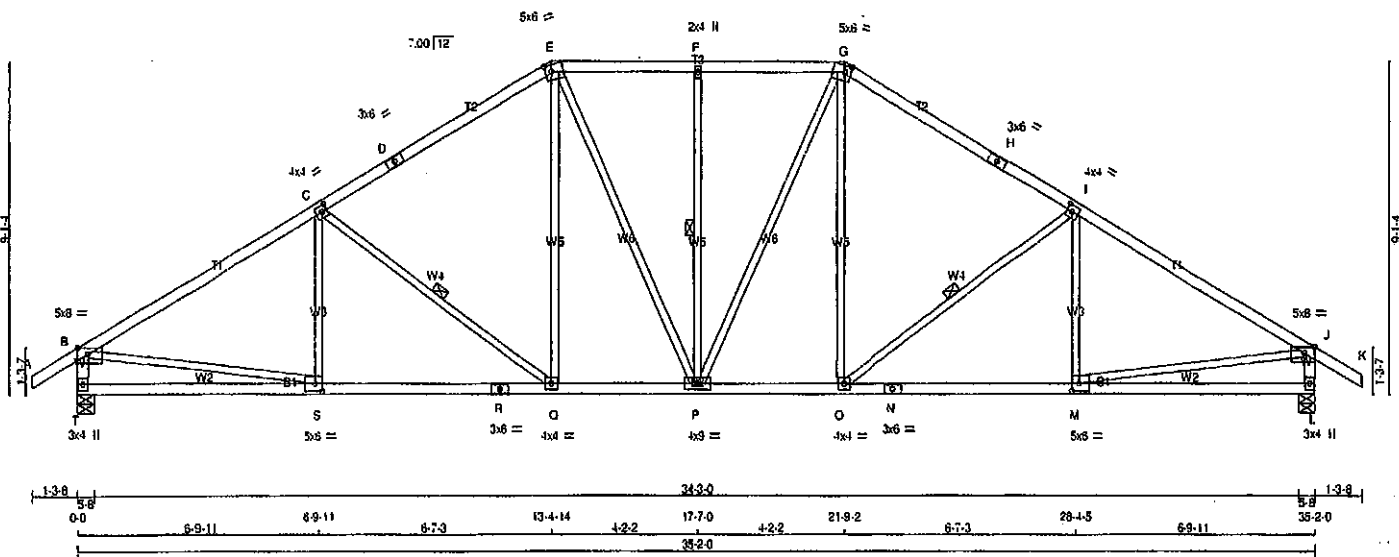


Structural component only
DWG# T-2007636

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T25	4	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR6TstFoe31v6l zns11-HQVwChulNMdm50y3K?erAxP?FtGSR7DeIqozMEMb
35-2-0 35-5-8
Scale = 1:50.3



TOTAL WEIGHT = 4 X 158 = 632 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
T - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
R - R	2x4	DRY	No.2
R - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	Edge	3.50
C TMVW-l	MT20	4.0	4.0	2.00	1.75
D TS-l	MT20	3.0	6.0		
E TTWW-m	MT20	5.0	6.0	2.25	2.00
F TMVW-w	MT20	2.0	4.0		
G TTWW-m	MT20	5.0	6.0	2.25	2.00
H TS-l	MT20	3.0	6.0		
I TMVW-l	MT20	4.0	4.0	2.00	1.75
J TMVW-p	MT20	5.0	8.0	Edge	3.50
L BMVW-l-p	MT20	3.0	4.0		
M BMVW-l	MT20	5.0	8.0	2.50	2.50
N BS-l	MT20	3.0	6.0		
O BMVW-l	MT20	4.0	4.0		
P BMVW-w-l	MT20	4.0	4.0		
Q BMVW-l	MT20	4.0	4.0		
R BS-l	MT20	3.0	6.0		
S BMVW-l	MT20	5.0	8.0	2.50	2.50
T BMVW-l-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
		GROSS REACTION		GROSS REACTION		BHQ		BRG	
JT	VERT	HORZ		DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
T	2064	0		2064	0	0	5-8	5-8	
L	2064	0		2064	0	0	5-8	5-8	

UNFACTORED REACTIONS

1ST LOSE		MAX. MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
T	1457	970	0	0	0	0	0	488	0
L	1457	970	0	0	0	0	0	488	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1:2 LENGTH OF C-Q, F-P, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (PLF)	MAX. (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	FORCE (LBS)	MAX. (LBS)	MAX. (LBS)
FR-TO						FR-TO			
A-B	0 32	-91.8	-91.8	0.12 (1)	10.00	S-C	-165	51	0.08 (1)
B-C	-2590 0	-91.8	-91.8	0.74 (1)	3.54	C-Q	-548	0	0.26 (1)
C-D	-2158 0	-91.8	-91.8	0.58 (1)	3.91	Q-E	0	431	0.10 (1)
D-E	-2158 0	-91.8	-91.8	0.58 (1)	3.91	E-P	0	288	0.08 (1)
E-F	-1958 0	-91.8	-91.8	0.24 (1)	4.81	P-F	-460	0	0.25 (1)
F-G	-1958 0	-91.8	-91.8	0.24 (1)	4.81	P-G	0	288	0.08 (1)
G-H	-2158 0	-91.8	-91.8	0.58 (1)	3.91	Q-G	0	431	0.10 (1)
H-I	-2158 0	-91.8	-91.8	0.58 (1)	3.91	O-I	-548	0	0.26 (1)
I-J	-2590 0	-91.8	-91.8	0.74 (1)	3.54	M-I	-185	51	0.08 (1)
J-K	0 32	-91.8	-91.8	0.12 (1)	10.00	S-S	0	2292	0.52 (1)
T-B	-2011 0	0.0	0.0	0.20 (1)	5.95	M-J	0	2292	0.52 (1)
L-J	-2011 0	0.0	0.0	0.20 (1)	5.95				

T-S	0 0	-18.5	-18.5	0.20 (4)	10.00
S-R	0 2269	-18.5	-18.5	0.46 (1)	10.00
R-Q	0 2269	-18.5	-18.5	0.46 (1)	10.00
Q-P	0 1833	-18.5	-18.5	0.36 (1)	10.00
P-O	0 1833	-18.5	-18.5	0.36 (1)	10.00
O-N	0 2269	-18.5	-18.5	0.46 (1)	10.00
N-M	0 2269	-18.5	-18.5	0.46 (1)	10.00
M-L	0 0	-18.5	-18.5	0.20 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.74 (1.00 (K-J)), BC=0.46 (1.00 (M-O)), WB=0.52 (1.00 (J-M)), SS=0.28 (1.00 (I-J))

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP/DRY/ SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1687 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (S) (INPUT = 0.90)
JSI METAL= 0.69 (M) (INPUT = 1.00)



Structural component only
DWG# T-2007637

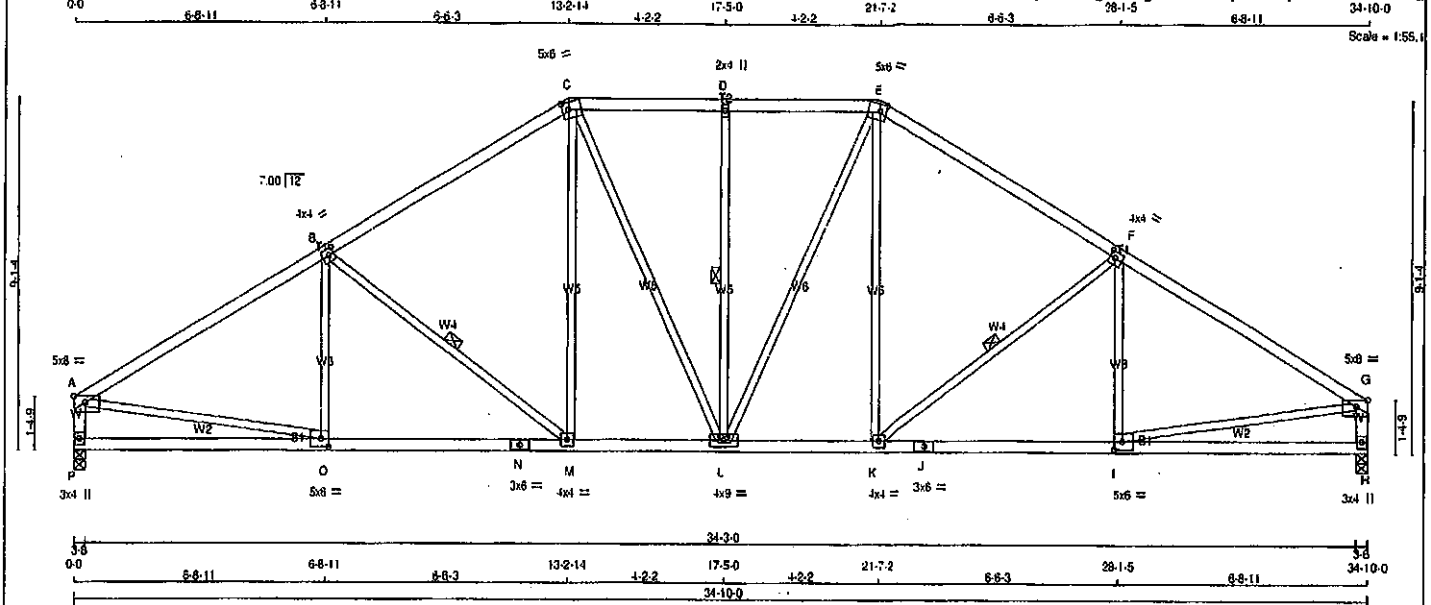
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408223	T25A	3	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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Scale = 1/55.1



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
P - A	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
P - N	2x4	DRY	No.2	SPF	
N - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	2.00	Edge
B	TMVW-w	MT20	4.0	4.0	2.00	1.75
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMVW-w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMVW-w	MT20	4.0	4.0	2.00	1.75
G	TMVW-p	MT20	5.0	8.0	Edge	3.50
H	BMV1-p	MT20	3.0	4.0		
I	BMVW-t	MT20	5.0	6.0	2.50	2.50
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMVW-w	MT20	4.0	4.0		
M	BMVW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMVW-t	MT20	5.0	6.0	2.50	2.50
P	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
P	1920	0	1920	0
H	1920	0	1920	0

UNFACTORED REACTIONS							
	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
P	1358	891 0	0 0	0 0	0 0	467 0	0 0
H	1358	891 0	0 0	0 0	0 0	467 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-M, D-L, F-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING	CHORDS	WEBS
TOTAL LOAD CASES: (4)	MAX. FACTORED	MAX. FACTORED
MEMB.	FORCE (LBS)	FORCE (LBS)
FR-TO	FROM TO	FR-TO
A-B	-2510 0	-91.8 -91.8 0.71 (1)
B-C	-2113 0	-91.8 -91.8 0.63 (1)
C-D	-1919 0	-91.8 -91.8 0.24 (1)
D-E	-1919 0	-91.8 -91.8 0.24 (1)
E-F	-2113 0	-91.8 -91.8 0.63 (1)
F-G	-2510 0	-91.8 -91.8 0.71 (1)
P-A	-1868 0	0.0 0.0 0.19 (1)
H-G	-1868 0	0.0 0.0 0.19 (1)
P-O	0 0	-18.5 -18.5 0.20 (4)
O-N	0 2199	-18.5 -18.5 0.45 (1)
N-M	0 2199	-18.5 -18.5 0.45 (1)
M-L	0 1797	-18.5 -18.5 0.35 (1)
L-K	0 1797	-18.5 -18.5 0.35 (1)
K-J	0 2199	-18.5 -18.5 0.45 (1)
J-I	0 2199	-18.5 -18.5 0.45 (1)
I-H	0 0	-18.5 -18.5 0.20 (4)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")
CSI: TC=0.71 (A-B:1), BC=0.45 (M-O:1), WB=0.50 (A-O:1), SS=0.26 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

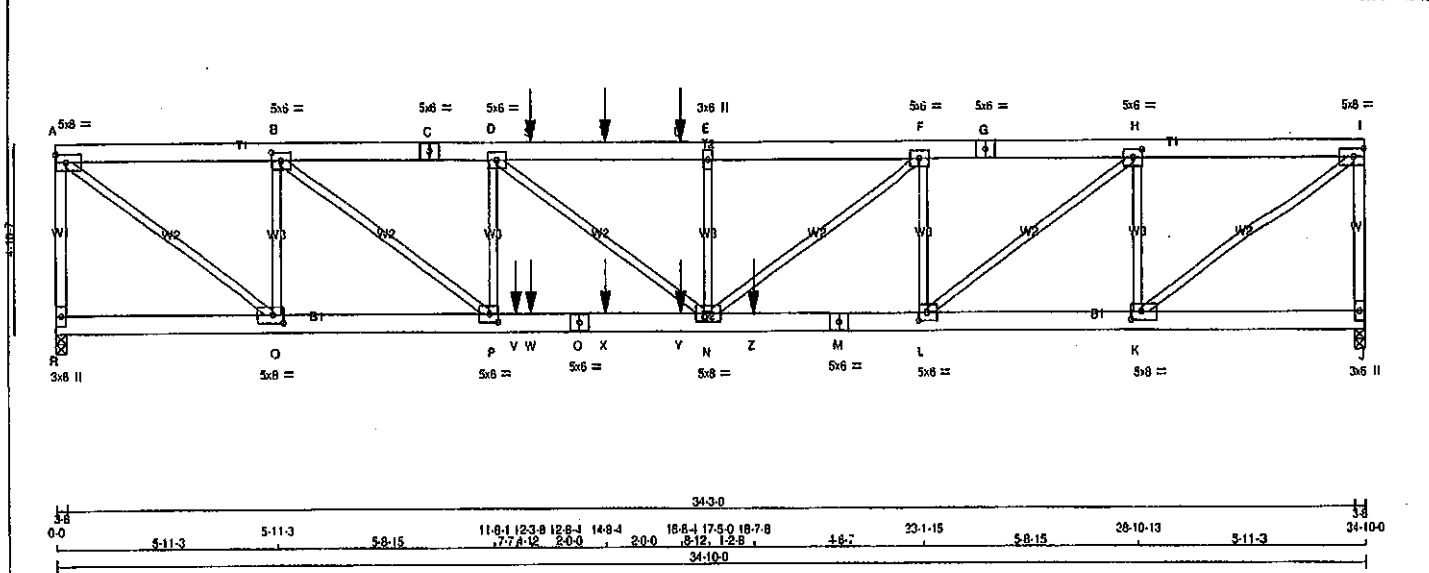
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (I) (INPUT = 0.90)
JSI METAL = 0.65 (N) (INPUT = 1.00)



Structural component only
DWG# T-2007638

JOB NAME 408223	TRUSS NAME T28	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 10:04:10 2020 Page 1	
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TOTAL WEIGHT = 2 X 183 = 366 lb

LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF R-A 2x4 DRY No.2 A-C 2x4 DRY No.2 C-G 2x4 DRY No.2 G-I 2x4 DRY No.2 I-K 2x4 DRY No.2 K-M 2x4 DRY No.2 M-J 2x4 DRY No.2 ALL WEBS 2x3 DRY No.2 DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF) TOP CHORDS: (0.122"x3") SPIRAL NAILS R-A 1 12 TOP I-J 1 12 TOP A-C 2 12 TOP C-G 2 12 SIDE(0.01) G-I 2 12 TOP BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS R-O 2 12 SIDE(0.01) O-M 2 12 SIDE(0.01) M-J 2 12 TOP WEBS: (0.122"x3") SPIRAL NAILS 2x3 1 6 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY. SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.		DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REQD BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX R 3373 0 3373 0 0 3-8 3-8 J 3103 0 3103 0 0 3-8 3-8 UNFACTORED REACTIONS 1ST CASE MAX. MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL R 2380 1591 0 0 0 0 0 789 0 0 0 J 2190 1459 0 0 0 0 0 731 0 0 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS WEBS MAX. FACTORED FORCE MAX. FACTORED FORCE MEMB. (LBS) VERT. LOAD (PLF) CSI (LC) UNBRACED LENGTH FR-TO MEMB. (LBS) MAX. FACTORED FORCE MAX. FACTORED FORCE FR-TO FROM TO FR-TO R-A -3316 0 0.0 0.0 0.58 (1) 8.39 K-I 0 4719 0.58 (1) A-B -4149 0 -91.8 -91.8 0.14 (1) 5.47 A-Q 0 5171 0.84 (1) B-C -7288 0 -91.8 -91.8 0.20 (1) 4.33 K-H -2757 0 0.44 (1) C-D -7288 0 -91.8 -91.8 0.20 (1) 4.33 Q-B -2997 0 0.48 (1) D-S -8278 0 -91.8 -91.8 0.28 (1) 4.03 L-H 0 3530 0.44 (1) S-T -8278 0 -91.8 -91.8 0.28 (1) 4.03 L-P 0 3959 0.48 (1) T-U -8278 0 -91.8 -91.8 0.28 (1) 4.03 P-D -1791 0 0.28 (1) U-E -8278 0 -91.8 -91.8 0.21 (1) 4.11 N-F 0 2134 0.28 (1) E-F -6586 0 -91.8 -91.8 0.19 (1) 4.52 D-N 0 1249 0.15 (1) F-G -6586 0 -91.8 -91.8 0.19 (1) 4.52 N-E -673 0 0.11 (1) G-H -6586 0 -91.8 -91.8 0.13 (1) 5.88 H-I -3787 0 0.0 0.0 0.51 (1) 6.02 I-J -3042 0 0.0 0.0 0.51 (1) 6.02 R-Q 0 0 -18.5 -18.5 0.04 (4) 10.00 Q-P 0 4149 -18.5 -18.5 0.30 (1) 10.00 P-V 0 7288 -18.5 -18.5 0.67 (1) 10.00 V-W 0 7288 -18.5 -18.5 0.67 (1) 10.00 W-O 0 7288 -18.5 -18.5 0.67 (1) 10.00 O-X 0 7288 -18.5 -18.5 0.67 (1) 10.00 X-Y 0 7288 -18.5 -18.5 0.67 (1) 10.00 Y-N 0 7288 -18.5 -18.5 0.67 (1) 10.00 N-Z 0 6586 -18.5 -18.5 0.72 (1) 10.00 Z-M 0 6586 -18.5 -18.5 0.72 (1) 10.00 M-L 0 6586 -18.5 -18.5 0.72 (1) 10.00 L-K 0 3787 -18.5 -18.5 0.32 (1) 10.00 K-J 0 0 -18.5 -18.5 0.04 (4) 10.00 FACTORED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX. FACE DIR. TYPE HEEL CONN. S 12-8-4 -86 -86 --- BACK VERT TOTAL --- C1 T 14-8-4 -86 -86 --- BACK VERT TOTAL --- C1 U 16-8-4 -86 -86 --- BACK VERT TOTAL --- C1 V 12-3-8 -1052 -1052 --- BACK VERT TOTAL --- C1 W 12-8-4 -17 -17 --- BACK VERT TOTAL --- C1 X 14-8-4 -17 -17 --- BACK VERT TOTAL --- C1 Y 16-8-4 -17 -17 --- BACK VERT TOTAL --- C1 Z 18-7-8 -1273 -1273 --- BACK VERT TOTAL --- C1 CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED		DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 25.6 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 39.0 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, OBC 2012, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 088-09, CSA 088-14 - TPIC 2011, TPIC 2014 155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL. (LL) = L/360 (1.16") CALCULATED VERT. DEFL. (LL) = L/999 (0.21") ALLOWABLE DEFL. (TL) = L/360 (1.16") CALCULATED VERT. DEFL. (TL) = L/999 (0.39") CSI: TC=0.56/1.00 (A-R:1), BC=0.72/1.00 (L-N:1), WB=0.64/1.00 (A-Q:1), SS=0.37/1.00 (L-N:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 788 1987 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.89 (I) INPUT = 0.90 JSI METAL= 0.73 (M) INPUT = 1.00	
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Structural component only
DWG# T-2007639 1/2

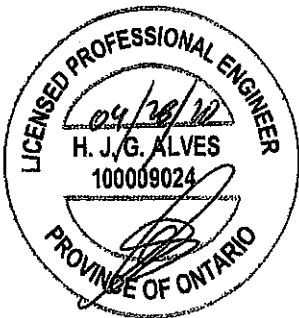
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JOB NAME 408223	TRUSS NAME T28	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:04:10 2020 Page 2 ID:DMCubINVR6TstF0e31v6l zns11-lqYGL68g cL45FP4UNtkGGKpt777 kbX7PuhzMEMZ	

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-1	MT20	5.0	8.0	2.50 3.25
B	TMVW-1	MT20	5.0	8.0	2.50 2.75
C	TS-1	MT20	5.0	6.0	
D	TMVW-1	MT20	5.0	6.0	
E	TMVW-1	MT20	3.0	6.0	
F	TMVW-1	MT20	5.0	6.0	
G	TS-1	MT20	5.0	6.0	
H	TMVW-1	MT20	5.0	8.0	2.50 2.75
I	TMVW-1	MT20	5.0	8.0	2.50 3.25
J	BMV1+p	MT20	3.0	6.0	
K	BMVW-1	MT20	5.0	8.0	2.50 3.25
L	BMVW-1	MT20	5.0	8.0	2.50 2.75
M	BS-1	MT20	5.0	6.0	
N	BMVW-1	MT20	5.0	8.0	
O	BS-1	MT20	5.0	6.0	
P	BMVW-1	MT20	5.0	8.0	2.50 2.75
Q	BMVW-1	MT20	5.0	8.0	2.50 3.25
R	BMV1+p	MT20	3.0	6.0	

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

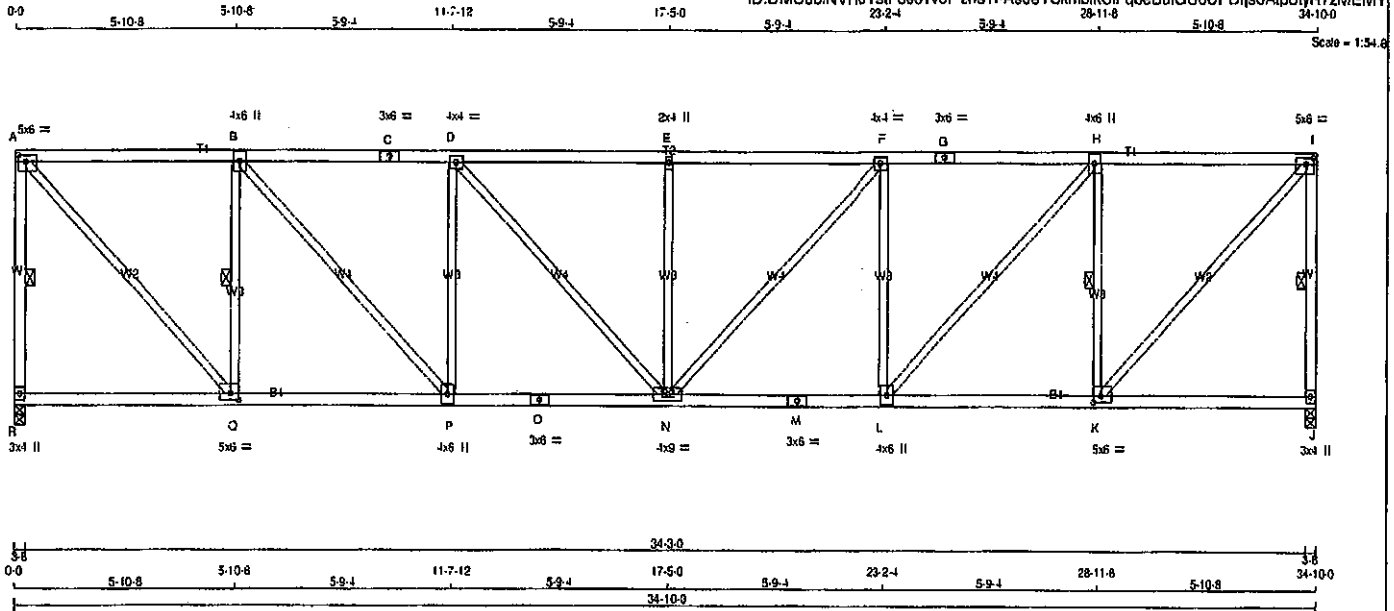


Structural component only
DWG# T-2007639 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T29	2	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstF0e31v6l zns11-As6eYSkmbktCiFqbeBuIGUoUPDljsbAlpBtyR7zMEMY
23-2-4 28-11-8 34-10-0



TOTAL WEIGHT = 2 X 157 = 314 lb (M)

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
R - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
J - I	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
SPF
DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-I	MT20	5.0	6.0	2.00 2.50
B	TMWW-I	MT20	4.0	6.0	
C	TS-I	MT20	3.0	6.0	
D	TMWW-I	MT20	4.0	4.0	
E	TMWW-I	MT20	2.0	4.0	
F	TMWW-I	MT20	4.0	4.0	
G	TS-I	MT20	3.0	6.0	
H	TMWW-I	MT20	4.0	6.0	
I	TMVW-I	MT20	5.0	6.0	2.00 2.50
J	BMV-I+p	MT20	3.0	4.0	
K	BMWW-I	MT20	5.0	6.0	2.00 2.50
L	BMWW-I	MT20	4.0	6.0	
M	BS-I	MT20	3.0	6.0	
N	BMWW-I	MT20	4.0	6.0	
O	BS-I	MT20	3.0	6.0	
P	BMWW-I	MT20	4.0	6.0	
Q	BMWW-I	MT20	5.0	6.0	2.00 2.50
R	BMV-I+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED	MAXIMUM FACTORED	INPUT	REORD			
	GROSS REACTION	GROSS REACTION	BRG	BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	1920	0	1920	0	0	3-8	3-8
J	1920	0	1920	0	0	3-8	3-8

UNFACTORED REACTIONS							
1ST CASE	MAX. MIN.	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1358	891	0	0	0	467	0
J	1358	891	0	0	0	467	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-R, I-J, B-Q, H-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (ILC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (ILC)	
FR-TO		FROM TO		FR-TO			
R-A	-1876 0	0.0	0.0 0.37 (1)	A-C	0 2281	0.51 (1)	
A-B	-1563 0	-18.5	-18.5 0.53 (1)	Q-B	-1540 0	0.43 (1)	
B-C	-2426 0	-18.5	-18.5 0.63 (1)	B-P	0 1272	0.29 (1)	
C-D	-2426 0	-18.5	-18.5 0.63 (1)	P-D	-831 0	0.58 (1)	
D-E	-2725 0	-18.5	-18.5 0.58 (1)	D-N	0 440	0.10 (1)	
E-F	-2725 0	-18.5	-18.5 0.58 (1)	N-E	-539 0	0.38 (1)	
F-G	-2426 0	-18.5	-18.5 0.63 (1)	N-F	0 440	0.10 (1)	
G-H	-2426 0	-18.5	-18.5 0.63 (1)	L-F	-831 0	0.58 (1)	
H-I	-1563 0	-18.5	-18.5 0.53 (1)	L-H	0 1272	0.29 (1)	
I-J	-1876 0	0.0	0.0 0.37 (1)	4.90	K-H	-1540 0	0.43 (1)
					K-I	0 2281	0.51 (1)
R-Q	0 0	-18.5	-18.5 0.15 (4)	10.00			
Q-P	0 1563	-18.5	-18.5 0.33 (1)	10.00			
P-O	0 2426	-18.5	-18.5 0.44 (1)	10.00			
O-N	0 2426	-18.5	-18.5 0.44 (1)	10.00			
N-M	0 2426	-18.5	-18.5 0.44 (1)	10.00			
M-L	0 2426	-18.5	-18.5 0.44 (1)	10.00			
L-K	0 1563	-18.5	-18.5 0.33 (1)	10.00			
K-J	0 0	-18.5	-18.5 0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.0/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1/16")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1/16")
CALCULATED VERT. DEFL.(TL) = L/999 (0.31")

CS: TC=0.63/1.00 (F-H:1), BC=0.44/1.00 (N-P:1), WB=0.58/1.00 (F-L:1), SS=0.25/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (A) INPUT = 0.90
JSI METAL= 0.78 (M) INPUT = 1.00

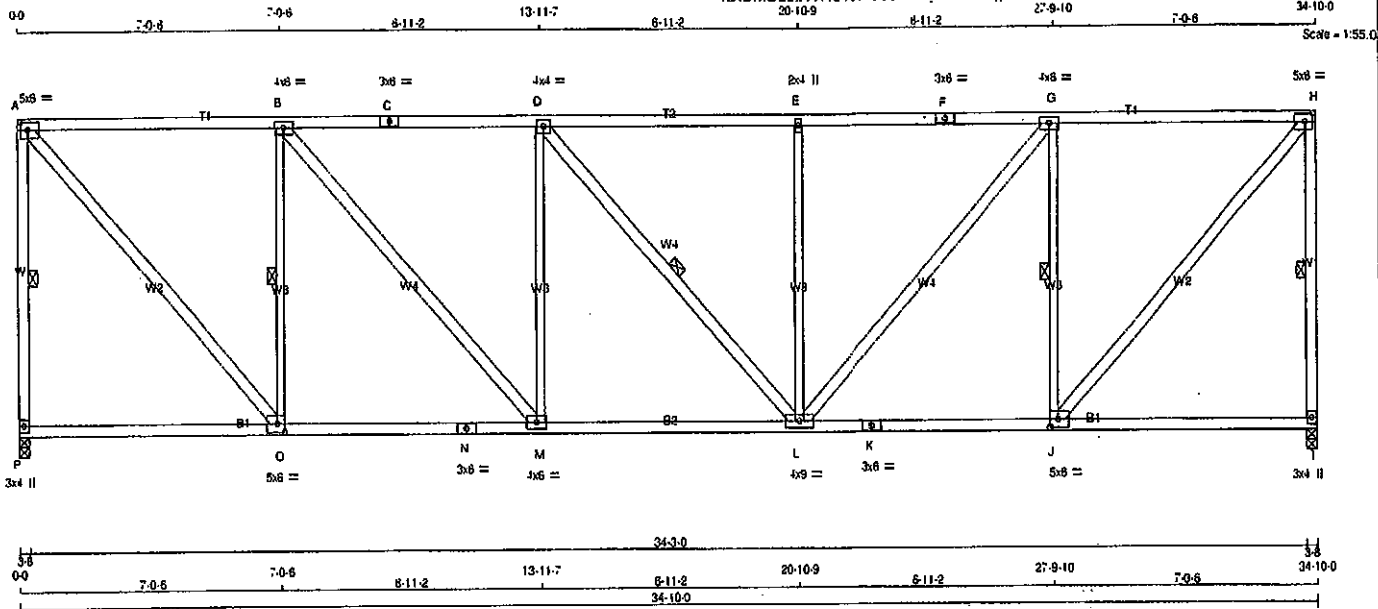


Structural component only
DWG# T-2007640

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408223	T30	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6i_zns11-e3g0mokCMbs3JPPnBvPxpHb5cdJb7s02rcvZzMEMX



TOTAL WEIGHT = 2 X 179 = 359 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
P - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
I - N	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			
O - B	2x3	DRY	No.2
M - D	2x3	DRY	No.2
L - E	2x3	DRY	No.2
J - G	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV-1	MT20	5.0	6.0	2.50	2.50
B	TMWV-1	MT20	4.0	6.0		
C	TS-1	MT20	3.0	6.0		
D	TMWV-1	MT20	4.0	4.0		
E	TMWV-1	MT20	2.0	4.0		
F	TS-1	MT20	3.0	6.0		
G	TMWV-1	MT20	4.0	6.0		
H	TMWV-1	MT20	5.0	6.0	2.50	2.50
I	BMV1-p	MT20	3.0	4.0		
J	BMWV-1	MT20	5.0	6.0	2.50	2.50
K	BS-1	MT20	3.0	6.0		
L	BMWV-1	MT20	4.0	6.0		
M	BMWV-1	MT20	3.0	6.0		
N	BS-1	MT20	3.0	6.0		
O	BMWV-1	MT20	5.0	6.0	2.50	2.50
P	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UP/LY IN-SX	IN-SX
P	1920	0	1920	0	3-8	3-8
I	1920	0	1920	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERMALIVE			
P	1358	891	0	0	0	467	0
I	1358	891	0	0	0	467	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-P, H-I, B-O, D-L, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO				FR-TO			
P-A	-1868	0	0.0	0.0	0.57	(1)	4.91
A-B	-1432	0	-91.8	-91.8	0.78	(1)	4.31
B-C	-2081	0	-91.8	-91.8	0.89	(1)	3.60
C-D	-2081	0	-91.8	-91.8	0.89	(1)	3.60
D-E	-2080	0	-91.8	-91.8	0.68	(1)	4.00
E-F	-2080	0	-91.8	-91.8	0.89	(1)	3.60
F-G	-2080	0	-91.8	-91.8	0.89	(1)	3.60
G-H	-1432	0	-91.8	-91.8	0.78	(1)	4.31
H-I	-1868	0	0.0	0.0	0.57	(1)	4.91
P-O	0	0	-18.5	-18.5	0.22	(4)	10.00
O-N	0	1432	-18.5	-18.5	0.35	(1)	10.00
N-M	0	1432	-18.5	-18.5	0.35	(1)	10.00
M-L	0	2081	-18.5	-18.5	0.42	(1)	10.00
L-K	0	1432	-18.5	-18.5	0.35	(1)	10.00
K-J	0	1432	-18.5	-18.5	0.35	(1)	10.00
J-I	0	0	-18.5	-18.5	0.22	(4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD			=	39.0 PSF

SPACING = 24.0 IN. GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.18")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSL: TC=0.89(1.00) (E-G-1), BC=0.42(1.00) (L-M-1), WB=0.81(1.00) (D-M-1), SS=0.30(1.00) (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

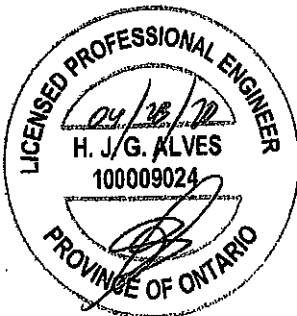
NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (H) (INPUT = 0.90)
JSI METAL = 0.46 (K) (INPUT = 1.00)

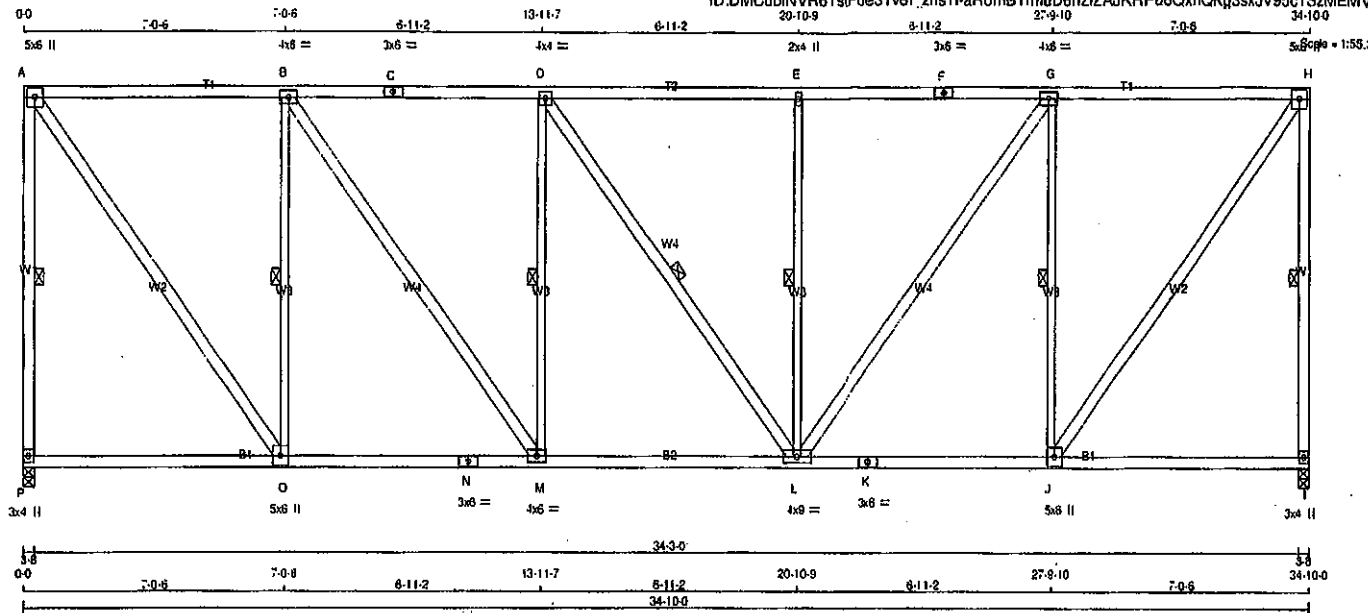


Structural component only
DWG# T-2007641

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408223	T31	5	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TslFoe31v6l_zns11-aRomBTmU6nZiZAJKRPu6QxnQKq3sxJV95c1SzMEMV
20-10-9 8-11-2 27-9-10 34-10-0 5.8000 = 1:55.3



TOTAL WEIGHT = 5 X 198 = 991 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
P - A	2x4 DRY	No.2	SPF		
A - C	2x4 DRY	No.2	SPF		
C - F	2x4 DRY	No.2	SPF		
F - H	2x4 DRY	No.2	SPF		
H - I	2x4 DRY	No.2	SPF		
I - K	2x4 DRY	No.2	SPF		
K - L	2x4 DRY	No.2	SPF		
L - M	2x4 DRY	No.2	SPF		
M - N	2x4 DRY	No.2	SPF		
N - O	2x4 DRY	No.2	SPF		
O - P	2x4 DRY	No.2	SPF		
P - Q	2x4 DRY	No.2	SPF		
Q - R	2x4 DRY	No.2	SPF		
R - S	2x4 DRY	No.2	SPF		
S - T	2x4 DRY	No.2	SPF		
T - U	2x4 DRY	No.2	SPF		
U - V	2x4 DRY	No.2	SPF		
V - W	2x4 DRY	No.2	SPF		
W - X	2x4 DRY	No.2	SPF		
X - Y	2x4 DRY	No.2	SPF		
Y - Z	2x4 DRY	No.2	SPF		
Z - A	2x4 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0		
B	TMVW+p	MT20	4.0	6.0		
C	TS-1	MT20	3.0	6.0		
D	TMVW+p	MT20	4.0	6.0		
E	TMVW+p	MT20	2.0	6.0		
F	TS-1	MT20	3.0	6.0		
G	TMVW+p	MT20	4.0	6.0		
H	TMVW+p	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMV1+p	MT20	5.0	6.0		
K	BS-1	MT20	3.0	6.0		
L	BMVW+p	MT20	4.0	9.0		
M	BMVW+p	MT20	4.0	6.0		
N	BS-1	MT20	3.0	6.0		
O	BMVW+p	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG	BRG	BRG
P	1920	0	1920	0	0	3-8	3-8	3-8	3-8
I	1920	0	1920	0	0	3-8	3-8	3-8	3-8

UNFACTORED REACTIONS

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.95 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-P, H-I, B-O, D-M, Q-L, E-L, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

A-B	-1183	0	-91.8	-91.8	0.75 (1)	4.89	O-B	-1465	0	0.96 (1)
B-C	-1719	0	-91.8	-91.8	0.82 (1)	3.98	B-M	0	916	0.15 (1)
C-D	-1719	0	-91.8	-91.8	0.82 (1)	3.98	M-D	-817	0	0.41 (1)
D-E	-1718	0	-91.8	-91.8	0.83 (1)	4.37	D-L	-2	0	0.00 (1)
E-F	-1718	0	-91.8	-91.8	0.83 (1)	3.96	L-E	-616	0	0.41 (1)
F-G	-1718	0	-91.8	-91.8	0.83 (1)	3.96	L-G	0	913	0.15 (1)
G-H	-1183	0	-91.8	-91.8	0.75 (1)	4.89	J-G	-1465	0	0.96 (1)
H-I	-1688	0	0.0	0.0	0.88 (1)	4.91	J-H	0	2008	0.32 (1)
P-O	0	0	-18.5	-18.5	0.22 (4)	10.00				
O-N	0	1183	-18.5	-18.5	0.32 (4)	10.00				
N-M	0	1183	-18.5	-18.5	0.32 (4)	10.00				
M-L	0	1719	-18.5	-18.5	0.36 (1)	10.00				
L-K	0	1183	-18.5	-18.5	0.32 (4)	10.00				
K-J	0	1183	-18.5	-18.5	0.32 (4)	10.00				
J-I	0	0	-18.5	-18.5	0.22 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSK TC=0.88/1.00 (H/H:1), BC=0.38/1.00 (L/M:1), WB=0.98/1.00 (B/O:1), SSI=0.30/1.00 (G/H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 610 354 1687 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (H) (INPUT = 0.90)
JSI METAL = 0.43 (H) (INPUT = 1.00)



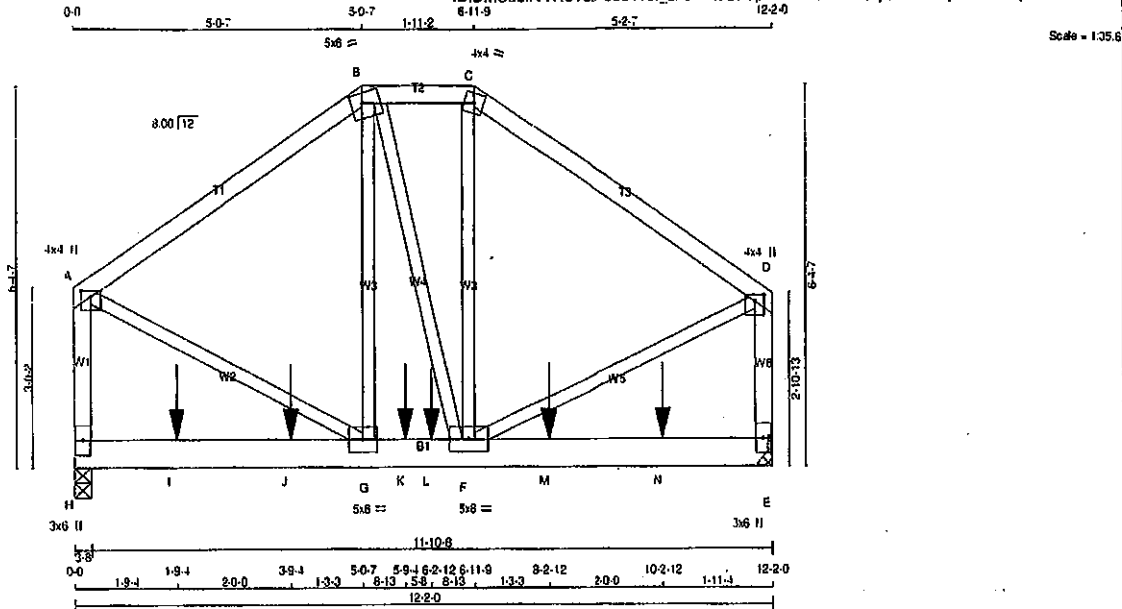
Structural component only
DWG# T-2007642

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408223	T32	1	1	TRUSS DESC		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TsfFoe31v6I_zns11-2dL9OpnHWEeAs8M1yeRKzCFqhAoVATkp7AauzMEMU



TOTAL WEIGHT = 68 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2	
B - C	2x4	DRY	No.2	
C - D	2x4	DRY	No.2	
H - A	2x4	DRY	No.2	
E - D	2x4	DRY	No.2	
H - E	2x6	DRY	No.2	
ALL WEBS	2x3	DRY	No.2	
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTWW-m	MT20	5.0	6.0	2.00	2.25
C	TTW-m	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMV1+p	MT20	3.0	6.0		
F	BMVW-1	MT20	5.0	6.0		
G	BMVW-1	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
H	1233	0	1233	0
E	1218	0	1218	0
			MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E MINIMUM BEARING LENGTH AT JOINT E = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE	
H	867	596	0	0	0
E	858	586	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.77 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO									
A-B	-853	0	-91.8	-91.8	0.48	(1)	5.86	Q-B	0
B-C	-721	0	-91.8	-91.8	0.07	(1)	6.25	B-F	0
C-D	-884	0	-91.8	-91.8	0.51	(1)	5.77	F-C	0
H-A	-1065	0	0.0	0.0	0.17	(1)	7.60	A-G	0
E-D	-1057	0	0.0	0.0	0.16	(1)	7.63	F-D	0
H-I	0	0	-18.5	-18.5	0.20	(1)	10.00		
I-J	0	0	-18.5	-18.5	0.20	(1)	10.00		
J-G	0	0	-18.5	-18.5	0.20	(1)	10.00		
G-K	0	711	-18.5	-18.5	0.28	(1)	10.00		
K-L	0	711	-18.5	-18.5	0.28	(1)	10.00		
L-F	0	711	-18.5	-18.5	0.28	(1)	10.00		
F-M	0	0	-18.5	-18.5	0.20	(1)	10.00		
M-N	0	0	-18.5	-18.5	0.20	(1)	10.00		
N-E	0	0	-18.5	-18.5	0.20	(1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
J	1-9-4	-172	-172	---	BACK	VERT	TOTAL	---	C1
I	3-9-4	-195	-195	---	BACK	VERT	TOTAL	---	C1
K	5-9-4	-187	-187	---	BACK	VERT	TOTAL	---	C1
L	6-2-12	-187	-187	---	BACK	VERT	TOTAL	---	C1
M	8-2-12	-195	-195	---	BACK	VERT	TOTAL	---	C1
N	10-2-12	-172	-172	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) O1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2018 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.51 (1.00 (C-D:1), BC=0.28 (1.00 (F-G:1), WB=0.20 (1.00 (A-G:1), SS=0.20 (1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 783 1967 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.78 (D) (INPUT = 0.90)
JSI METAL = 0.23 (D) (INPUT = 1.00)



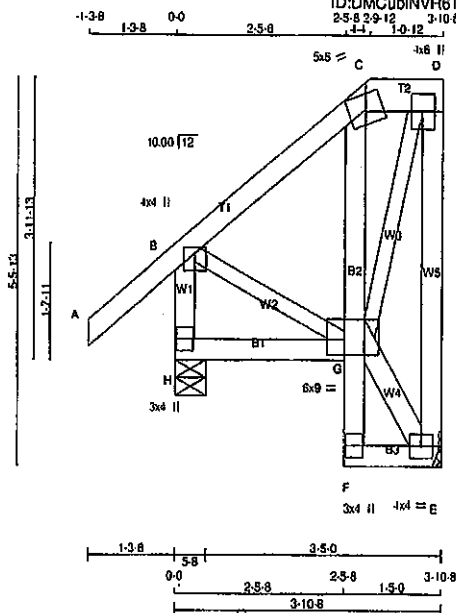
Structural component only
DWG# T-2007643

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T34S	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR6TsFoe31v6l_zns1l-WqvXc9ovQqMVo0IZQkUtzXVSIE5FX7ycZTaj6KzMEMT

Scale = 1:30.1



TOTAL WEIGHT = 2 X 32 = 64 lb (M)

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4 DRY	No.2	SPF
C - D	2x6 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF
H - B	2x4 DRY	No.2	SPF
H - G	2x4 DRY	No.2	SPF
F - C	2x4 DRY	No.2	SPF
F - E	2x4 DRY	No.2	SPF
ALL WEBS	2x4 DRY	No.2	SPF
EXCEPT			
B - G	2x3 DRY	No.2	SPF
G - D	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C	ITV-m	MT20	5.0	6.0	
D	TMVW+p	MT20	4.0	6.0	
E	BMVW1-i	MT20	4.0	4.0	
F	BMV+p	MT20	3.0	4.0	
G	BMVWW1-i	MT20	6.0	9.0	2.50 3.00
H	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
E	214	0	214	0	0
H	341	0	341	0	0
				MECHANICAL	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LOASE	MAX. MIN. COMPONENT REACTIONS					
E	151	99.0	0.0	0.0	0.0	62.0	0.0
H	239	170.0	0.0	0.0	0.0	89.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0.41	-91.8 -91.8 0.14 (5)	10.00	G-E	-8.0	0.00 (11)	
B-C	-62.0	-91.8 -91.8 0.12 (1)	8.25	B-G	0.53	0.01 (11)	
C-D	-51.0	-91.8 -91.8 0.01 (1)	8.25	G-D	0.154	0.03 (11)	
E-D	-197.0	0.0 0.0 0.10 (1)	7.81				
H-B	-317.0	0.0 0.0 0.03 (1)	7.81				
H-G	0.0	-18.5 -18.5 0.04 (4)	10.00				
F-G	0.13	0.0 0.0 0.01 (1)	10.00				
G-C	-138.0	0.0 0.0 0.01 (1)	7.81				
F-E	0.4	-18.5 -18.5 0.01 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TG=0.14:1.00 (A-B:5), BC=0.04:1.00 (G-H:4), WB=0.03:1.00 (D-G:1), SSI=0.09:1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.24 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007644



TOTAL WEIGHT = 2 X 29 = 57 lb

DESCR	QTY	UNIT	PRICE	TOTAL
SPF				
SPF				
SPF				
SPF				
SPF				
SPF				
SPF				
SPF				

DRY: SEASONED LUMBER

<u>PLATES (table in inches)</u>					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TTWW+m	MT20	5.0	6.0	2.50 2.25
D	TMV+p	MT20	3.0	4.0	
E	TMVW-i	MT20	4.0	4.0	
F	BMVW-i	MT20	4.0	4.0	
G	BMV+p	MT20	3.0	4.0	
H	BVMWVW-i	MT20	7.0	8.0	3.25 2.25
I	BMVW-i	MT20	4.0	4.0	

BOARDING BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	191	0	191	0	0	MECHANICAL	
I	363	0	363	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1.8.

1ST LCASE		MAX., MIN. COMPONENT REACTIONS	
1	1	1	1
2	2	2	2
3	3	3	3
4	4	4	4
5	5	5	5
6	6	6	6
7	7	7	7
8	8	8	8
9	9	9	9
10	10	10	10
11	11	11	11
12	12	12	12
13	13	13	13
14	14	14	14
15	15	15	15
16	16	16	16
17	17	17	17
18	18	18	18
19	19	19	19
20	20	20	20
21	21	21	21
22	22	22	22
23	23	23	23
24	24	24	24
25	25	25	25
26	26	26	26
27	27	27	27
28	28	28	28
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31	31	31	31
32	32	32	32
33	33	33	33
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36	36	36	36
37	37	37	37
38	38	38	38
39	39	39	39
40	40	40	40
41	41	41	41
42	42	42	42
43	43	43	43
44	44	44	44
45	45	45	45
46	46	46	46
47	47	47	47
48	48	48	48
49	49	49	49
50	50	50	50
51	51	51	51
52	52	52	52
53	53	53	53
54	54	54	54
55	55	55	55
56	56	56	56
57	57	57	57
58	58	58	58
59	59	59	59
60	60	60	60
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85	85	85	85
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87	87	87	87
88	88	88	88
89	89	89	89
90	90	90	90
91	91	91	91
92	92	92	92
93	93	93	93
94	94	94	94
95	95	95	95
96	96	96	96
97	97	97	97
98	98	98	98
99	99	99	99
100	100	100	100

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	135	88 13	0 0	0 0	0 0	49 0	0 0
I	254	183 0	0 0	0 0	0 0	72 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: 15)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO							
A-B	0 41	-91.8	-91.8	0.14 (5)	10.00	H-F	-6 0 0.00 (1)		
B-C	-45 0	-91.8	-91.8	0.14 (5)	6.25	H-E	0 141 0.03 (1)		
C-D	-69 0	-91.8	-91.8	0.02 (1)	8.25	C-H	0 48 0.01 (1)		
D-E	-67 0	-91.8	-91.8	0.02 (1)	6.25	I-C	-96 28 0.02 (1)		
F-E	-175 0	0.0	0.0	0.05 (1)	7.81				
I-B	-255 0	0.0	0.0	0.03 (1)	7.81				
I-H	-13 45	-18.5	-18.5	0.04 (4)	6.25				
G-H	0 13	0.0	0.0	0.01 (1)	10.00				
H-D	-135 0	0.0	0.0	0.01 (1)	7.81				
G-F	0 4	-18.5	-18.5	0.01 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

SPECIFIED LOADS:

SPECIFIED LOADS:				
TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD			=	39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9.
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN
LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF
LIVE LOAD

ALLOWABLE DEFL.(LL)= L 360 (0.19")
CALCULATED VERT. DEFL.(LL) = L 999 (0.00")
ALLOWABLE DEFL.(TL)= L 360 (0.19")
CALCULATED VERT. DEFL.(TL) = L 999 (0.00")

CSI: TC=0.14, 1.00 (A-B:5), BC=0.04, 1.00 (H-I:4),
WB=0.03, 1.00 (E-H:1), SS=0.09, 1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT KEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT
'RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1867	788	1987	1650

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.09 (8) (INPUT = 1.00)

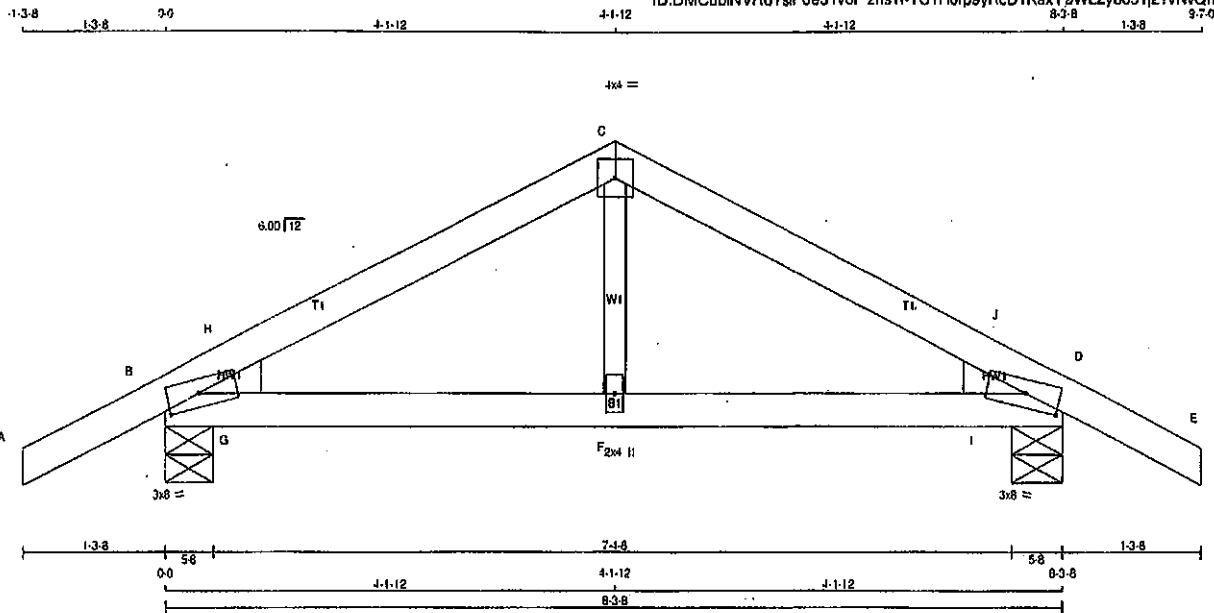


Structural component only
DWG# T-2007645

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408223	T36	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6I zns11-TC1H0p9yRcD1KexY9WL2ybc51j27vNvQn3qBDzMEMR



Scale = 1/16" = 1'-0"

TOTAL WEIGHT = 3 X 27 = 81 lb (M/F)

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	8.0	1.50	3.75
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-m	MT20	3.0	8.0	1.50	3.75
F	BMW-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	HORIZ	BRG	BRG	HEEL	WEDGE
B	582	0	582	0	0	5-8	5-8	2x4 L	
D	582	0	582	0	0	5-8	5-8	2x4 R	

UNFACTORED REACTIONS		1ST LCASE		MAX-MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	COMBINED	SNOW	LIVE	PERM. LIVE
B	409	282	0	0	0	0	0	127	0	0	0
D	409	282	0	0	0	0	0	127	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)		CHORDS		WEBS	
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 23	-91.8	-91.8 0.12 (1)	10.00	F-C	0 170	0.04 (1)
B-H	-539 0	-91.8	-91.8 0.04 (1)	6.25	G-H	-158 3	0.00 (1)
C-E	-525 0	-91.8	-91.8 0.15 (1)	6.25	I-J	-158 3	0.00 (1)
C-J	-525 0	-91.8	-91.8 0.15 (1)	6.25			
D-E	-539 0	-91.8	-91.8 0.04 (1)	6.25			
D-E	0 23	-91.8	-91.8 0.12 (1)	10.00			
B-G	0 465	-18.5	-18.5 0.21 (1)	10.00			
G-F	0 465	-18.5	-18.5 0.21 (1)	10.00			
F-I	0 465	-18.5	-18.5 0.21 (1)	10.00			
I-O	0 465	-18.5	-18.5 0.21 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.28")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.28")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TC=0.15/1.00 (C-J:1), BC=0.21/1.00 (F-I:1), WB=0.04/1.00 (C-F:1), SS=0.12/1.00 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

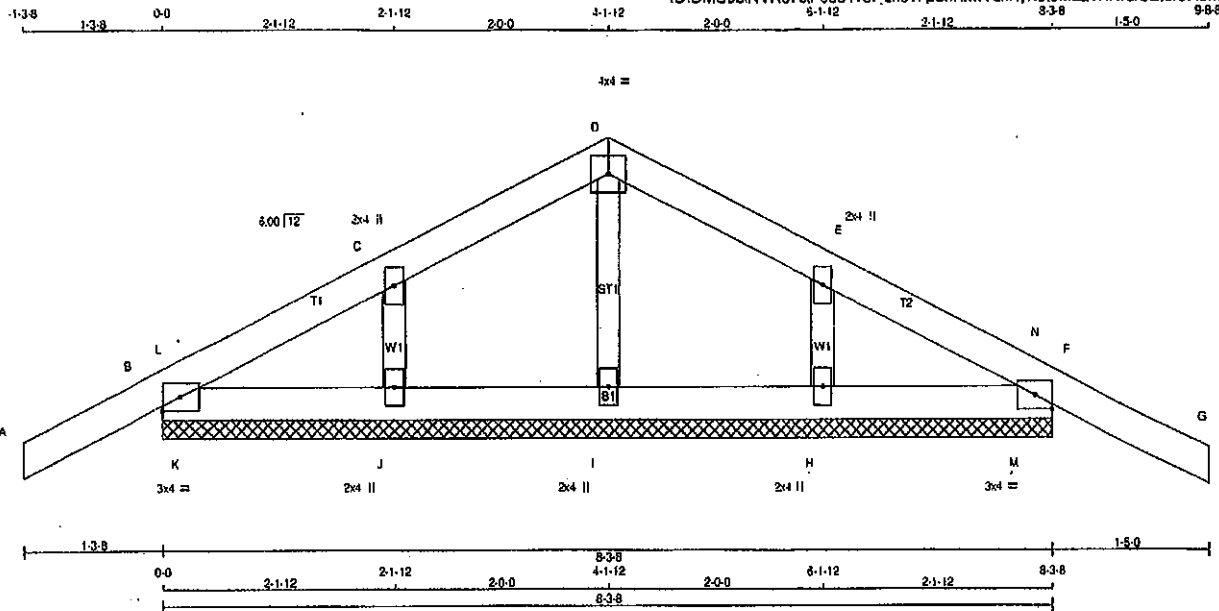
JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007646

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	G36	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR6TstFoe31v6I_zns1I-pdnKwTznXyK3t8MLaVXNrbL5ioKemmYnIzMEMu



TOTAL WEIGHT = 27 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
B - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	Edge
C	TMW+w	MT20	2.0	4.0	
D	TTW-p	MT20	4.0	4.0	
E	TMW+w	MT20	2.0	4.0	
F	TMB1-I	MT20	3.0	4.0	Edge
H, I, J					
H	BMW1+w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 23	-91.8 -91.8	0.12 (1)	10.00	F-D	-115 0	0.02 (1)
B-L	-61 0	-91.8 -91.8	0.01 (4)	6.25	J-C	-218 0	0.03 (1)
L-C	-39 0	-91.8 -91.8	0.06 (1)	6.25	H-E	-218 0	0.03 (1)
C-D	-45 0	-91.8 -91.8	0.08 (1)	6.25	K-L	-52.7	0.00 (1)
D-E	-45 0	-91.8 -91.8	0.08 (1)	6.25	M-N	-52.7	0.00 (1)
E-N	-39 0	-91.8 -91.8	0.08 (1)	6.25			
N-F	-61 0	-91.8 -91.8	0.01 (4)	6.25			
F-G	0 25	-91.8 -91.8	0.14 (1)	10.00			
B-K	0 44	-18.5 -18.5	0.03 (1)	10.00			
K-J	0 44	-18.5 -18.5	0.03 (1)	10.00			
J-I	0 32	-18.5 -18.5	0.02 (1)	10.00			
I-H	0 32	-18.5 -18.5	0.02 (1)	10.00			
H-M	0 44	-18.5 -18.5	0.03 (1)	10.00			
M-F	0 44	-18.5 -18.5	0.03 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (F-G:1), BC=0.03/1.00 (H-M:1), WB=0.03/1.00 (E-H:1), SS=0.10/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE GRIP (DRY) SHEAR	SECTION				
(PSI)	(PLI)	(PLI)			
MAX MIN	MAX MIN	MAX MIN			
MT20	818	354	1667	788	1987 1658

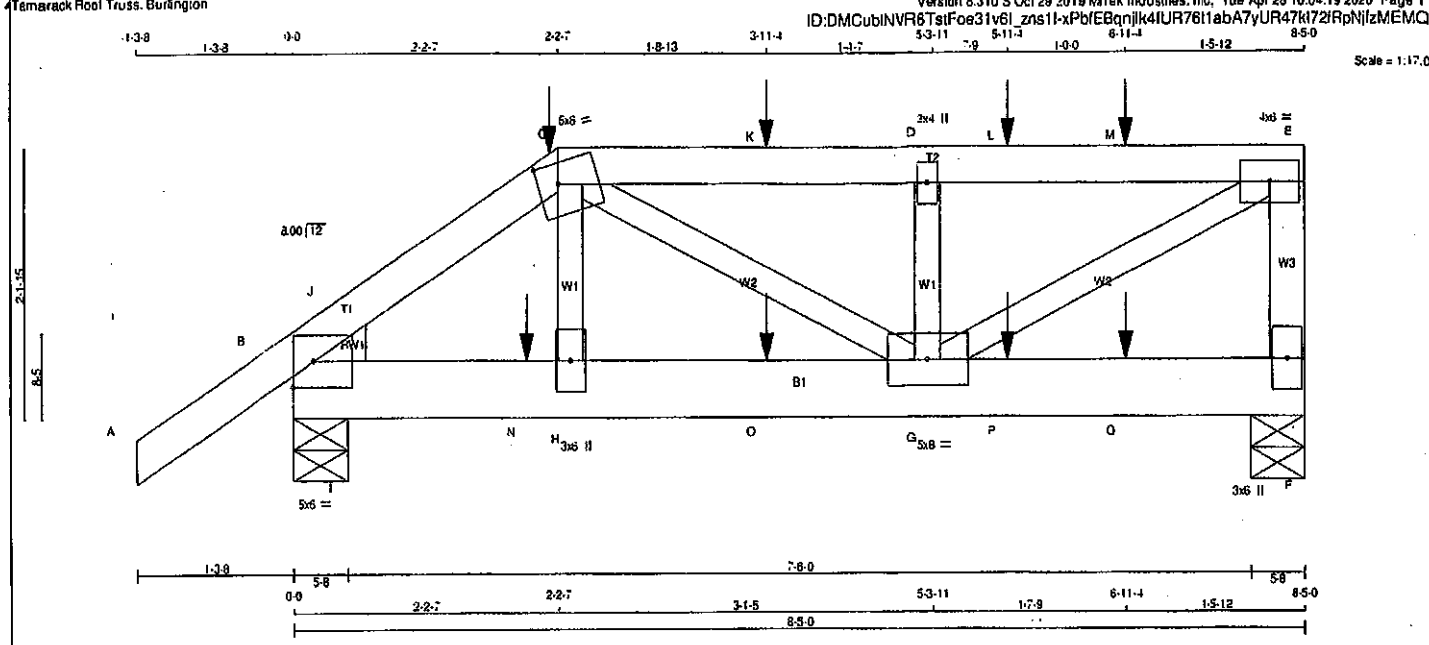
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007621



Scale = 1:12.0 TOTAL WEIGHT = 37 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
F - E	2x4	DRY No.2	SPF
B - F	2x6	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TM8H1-1	MT20	5.0	6.0		Edge
C	TTWW-m	MT20	5.0	6.0	2.00	2.00
D	TMW-w	MT20	2.0	4.0		
E	TMW-w	MT20	4.0	6.0		
F	BMV1-p	MT20	3.0	6.0		
G	BMWW-w	MT20	5.0	6.0		
H	BMW-w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
F	760	0	760	0	5-8	5-8	
B	823	0	823	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

JT	1ST LCASE		MAX. MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	537	352	0	0	0	185	0
B	580	392	0	0	0	188	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	FACTORED		FACTORED		MEMB.	MAX. FACTORED	
	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	UNBRAC LENGTH		FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		FROM TO			
A-B	0 26	-91.8	-91.8 0.13 (1)	10.00	H-C	0 73	0.02 (4)
B-J	-1068 0	-91.8	-91.8 0.08 (1)	5.99	C-G	0 252	0.06 (1)
J-C	-880 0	-91.8	-91.8 0.08 (1)	6.25	G-D	-528 0	0.09 (1)
C-K	-923 0	-91.8	-91.8 0.24 (1)	6.09	G-E	0 1062	0.26 (1)
K-D	-923 0	-91.8	-91.8 0.24 (1)	6.09	I-J	0 207	0.00 (1)
D-L	-924 0	-91.8	-91.8 0.24 (1)	6.09			
L-M	-924 0	-91.8	-91.8 0.24 (1)	6.09			
M-E	-924 0	-91.8	-91.8 0.24 (1)	6.09			
F-E	-682 0	0.0	0.0 0.08 (1)	7.81			
B-I	0 701	-18.5	-18.5 0.14 (1)	10.00			
I-N	0 701	-18.5	-18.5 0.14 (1)	10.00			
N-H	0 701	-18.5	-18.5 0.14 (1)	10.00			
H-O	0 705	-18.5	-18.5 0.15 (1)	10.00			
O-G	0 705	-18.5	-18.5 0.15 (1)	10.00			
G-P	0 0	-18.5	-18.5 0.07 (1)	10.00			
P-Q	0 0	-18.5	-18.5 0.07 (1)	10.00			
Q-F	0 0	-18.5	-18.5 0.07 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-2-7	-88	-88	---	FRONT	VERT	TOTAL	---	C1
K	3-11-4	-83	-83	---	FRONT	VERT	TOTAL	---	C1
L	5-11-4	-83	-83	---	FRONT	VERT	TOTAL	---	C1
M	8-11-4	-83	-83	---	FRONT	VERT	TOTAL	---	C1
N	1-11-4	-48	-48	---	FRONT	VERT	TOTAL	---	C1
O	3-11-4	-48	-48	---	FRONT	VERT	TOTAL	---	C1
P	5-11-4	-48	-48	---	FRONT	VERT	TOTAL	---	C1
Q	6-11-4	-48	-48	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/699 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.24/1.00 (D-E:1), BC=0.15/1.00 (G-H:1), WB=0.26/1.00 (E-G:1), SS=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

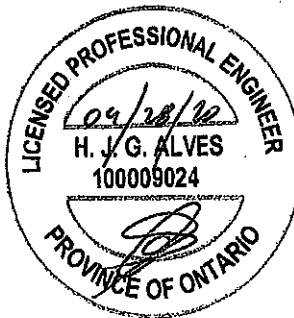
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (E) (INPUT = 0.80)
JSI METAL= 0.24 (E) (INPUT = 1.00)



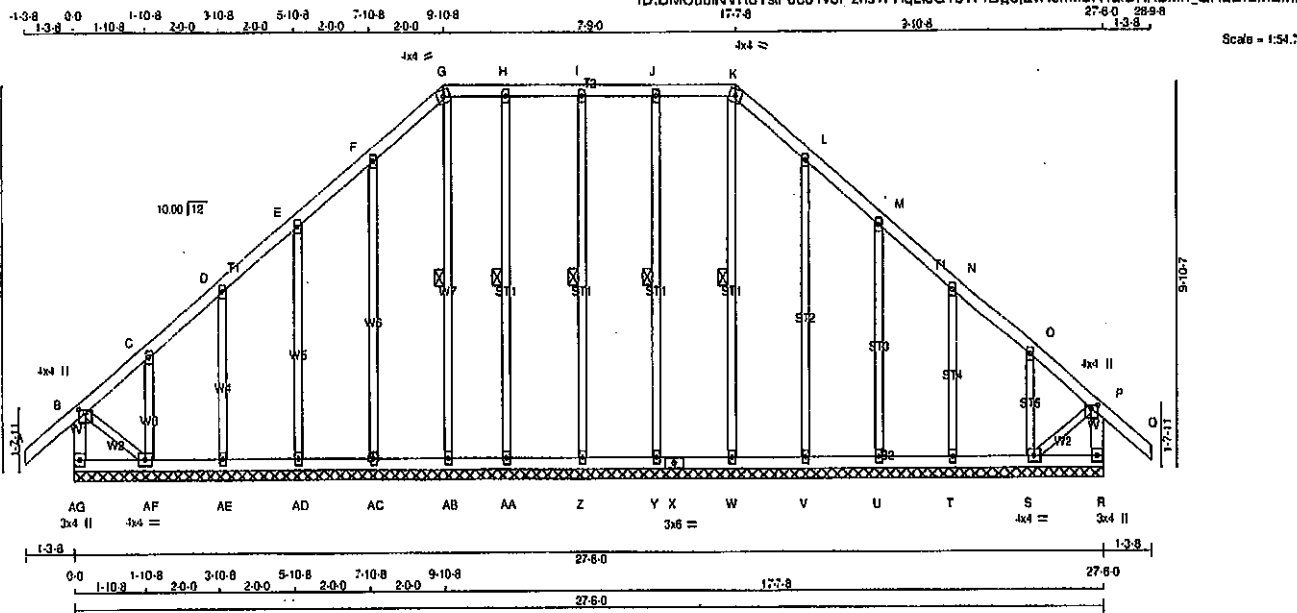
Structural component only
DWG# T-2007647

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408223	G39	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6l zns11-HqL8GTbYr4Bq0JZvH0mw2R1afDRRBmn_QHLBrzMEM



TOTAL WEIGHT = 2 X 152 = 303 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
AG - B	2x4	DRY	No.2	SPF	
A - G	2x4	DRY	No.2	SPF	
G - K	2x4	DRY	No.2	SPF	
K - Q	2x4	DRY	No.2	SPF	
R - P	2x4	DRY	No.2	SPF	
AG - X	2x4	DRY	No.2	SPF	
X - R	2x4	DRY	No.2	SPF	

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE	PLATES			
B TMW+p	MT20	4.0	4.0	1.00 2.00
C, D, E, F, H, I, J, L, M, N, O				
G TMW+w	MT20	2.0	4.0	
C TTW-m	MT20	4.0	4.0	
K TTW-m	MT20	4.0	4.0	
P TMW+p	MT20	4.0	4.0	1.00 2.00
R BMV1+p	MT20	3.0	4.0	
S BMWV1-t	MT20	4.0	4.0	
T, U, V, W, Y, Z, AA, AB, AC, AD, AE				
T BMW1+w	MT20	2.0	4.0	
X BS-t	MT20	3.0	6.0	
AF BMWV1-t	MT20	4.0	4.0	
AG BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF K-W, J-Y, I-Z, H-AA, G-AB.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO			
AG-B	-292 0	0.0	0.0 0.03 (1)	W-K	-140 0	0.09 (1)	
A-B	0 41	-91.8	-91.8 0.13 (1)	Y-J	-210 0	0.14 (1)	
B-C	-80 0	-91.8	-91.8 0.12 (1)	Z-I	-181 0	0.12 (1)	
C-D	-23 0	-91.8	-91.8 0.05 (1)	AA-H	-182 0	0.12 (1)	
D-E	-28 0	-91.8	-91.8 0.05 (1)	V-L	-203 0	0.27 (1)	
E-F	-19 0	-91.8	-91.8 0.05 (1)	U-M	-175 0	0.13 (1)	
F-G	-29 0	-91.8	-91.8 0.05 (1)	T-N	-192 0	0.07 (1)	
G-H	-12 0	-91.8	-91.8 0.04 (1)	S-O	-125 0	0.02 (1)	
H-I	-12 0	-91.8	-91.8 0.04 (1)	AB-G	-121 0	0.06 (1)	
I-J	-12 0	-91.8	-91.8 0.05 (1)	AC-F	-202 0	0.27 (1)	
J-K	-12 0	-91.8	-91.8 0.05 (1)	AD-E	-178 0	0.13 (1)	
K-L	-30 0	-91.8	-91.8 0.05 (1)	AE-D	-192 0	0.07 (1)	
L-M	-19 0	-91.8	-91.8 0.05 (1)	AF-C	-126 0	0.02 (1)	
M-N	-26 0	-91.8	-91.8 0.05 (1)	B-AF	0 32	0.01 (1)	
N-O	-24 0	-91.8	-91.8 0.05 (1)	S-P	0 32	0.01 (1)	
O-P	-60 0	-91.8	-91.8 0.12 (1)				
P-Q	0 41	-91.8	-91.8 0.13 (1)				
Q-R	-293 0	0.0	0.0 0.03 (1)				
AG-AF	0 0	-18.5	-18.5 0.02 (4)				
AF-AE	0 23	-18.5	-18.5 0.02 (4)				
AE-AD	0 19	-18.5	-18.5 0.02 (4)				
AD-AC	0 16	-18.5	-18.5 0.02 (4)				
AC-AB	0 14	-18.5	-18.5 0.02 (4)				
AB-AA	0 12	-18.5	-18.5 0.01 (4)				
AA-Z	0 12	-18.5	-18.5 0.02 (4)				
Z-Y	0 12	-18.5	-18.5 0.02 (4)				
Y-X	0 12	-18.5	-18.5 0.02 (4)				
X-W	0 12	-18.5	-18.5 0.02 (4)				
W-V	0 14	-18.5	-18.5 0.02 (4)				
V-U	0 16	-18.5	-18.5 0.02 (4)				
U-T	0 19	-18.5	-18.5 0.02 (4)				
T-S	0 23	-18.5	-18.5 0.02 (4)				
S-R	0 0	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (P-Q:1), BC=0.02/1.00 (S-T:4), WB=0.27 1.00 (L-V:1), SSI=0.09/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	818 354	1687 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (K) INPUT = 0.80
JSI METAL= 0.11 (L) INPUT = 1.00



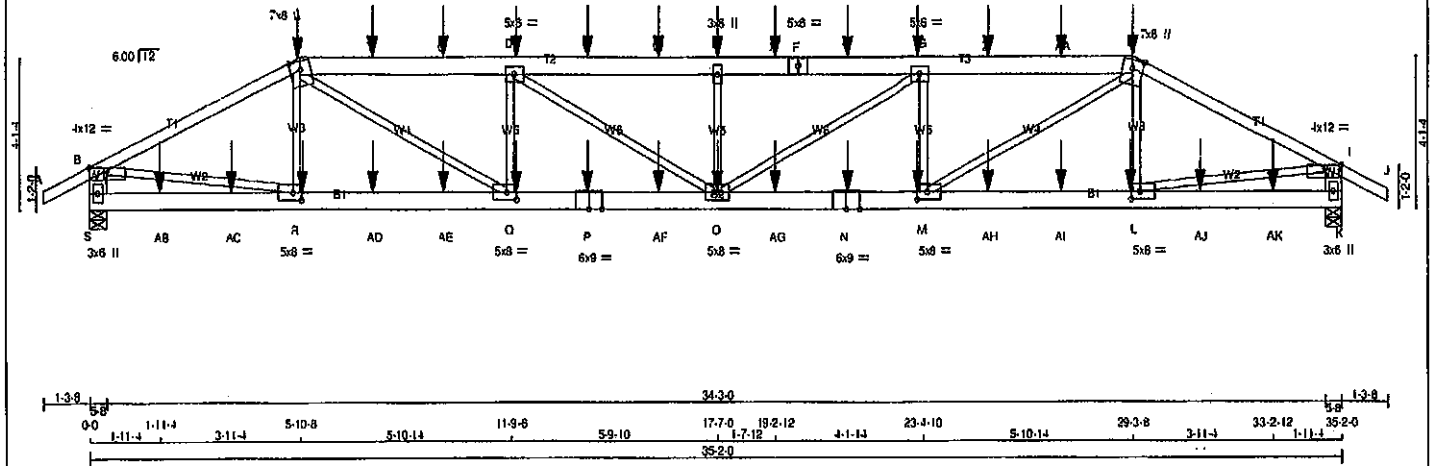
Structural component only
DWG# T-2007622

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	T40	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR6TstFoe31v6l_zns11-TaGG5wA9pyw_Bd8P2IsXcz_APYArTa7LN10JzME7m

1-3-8 0.0 5-10-8 5-10-8 5-10-14 11-9-8 5-9-10 17-7-0 19-2-12 3-1-14 23-4-10 5-10-14 29-3-8 35-2-0 1-3-8
1-3-8 0.0 5-10-8 5-10-8 5-10-14 11-9-8 5-9-10 17-7-0 19-2-12 3-1-14 23-4-10 5-10-14 29-3-8 35-2-0 1-3-8
Scale = 1:57.6



TOTAL WEIGHT = 2 X 169 = 338 lb

LUMBER	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES				
CHORDS	SIZE	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	1650F 1.5E	SPF
P - N	2x6	DRY	1650F 1.5E	SPF
N - K	2x6	DRY	1650F 1.5E	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(81.0)
H-J	12	SIDE(81.0)
C-F	12	SIDE(183.1)
F-H	12	SIDE(183.1)
S-B	12	TOP
K-I	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S-P	12	SIDE(183.1)
P-N	12	SIDE(183.1)
N-K	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORZ	HORZ	UPLIFT	IN-SX
S	3323	0	0	5-8
K	3323	0	0	5-8

UNFACTORED REACTIONS	1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
S	2350	1539 0
K	2350	1539 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1	MAX. MAX. UNBRAC	MEMB.	FORCE (LBS)
FR-TO		FROM TO	CS1 (LC)	FR-TO	
A-B	0 28	-91.8 -91.8 0.07 (1)	10.00	R-C	-413 20
B-C	-5086 0	-91.8 -91.8 0.52 (1)	3.87	C-Q	0 3193
C-T	-7284 0	-91.8 -91.8 0.27 (1)	4.26	Q-D	-1500 0
T-U	-7284 0	-91.8 -91.8 0.27 (1)	4.26	D-O	0 946
U-D	-7284 0	-91.8 -91.8 0.27 (1)	4.26	O-E	-830 0
D-V	-8084 0	-91.8 -91.8 0.27 (1)	4.08	E-G	0 -946
V-W	-8084 0	-91.8 -91.8 0.27 (1)	4.08	M-H	-1500 0
W-E	-8084 0	-91.8 -91.8 0.27 (1)	4.08	H-G	0 3193
E-X	-8084 0	-91.8 -91.8 0.27 (1)	4.08	L-H	-413 20
X-F	-8084 0	-91.8 -91.8 0.27 (1)	4.08	B-R	0 4582
F-Y	-8084 0	-91.8 -91.8 0.27 (1)	4.08	L-I	0 4582
Y-G	-8084 0	-91.8 -91.8 0.27 (1)	4.08		
G-Z	-7284 0	-91.8 -91.8 0.27 (1)	4.26		
Z-AA	-7284 0	-91.8 -91.8 0.27 (1)	4.26		
AA-H	-7284 0	-91.8 -91.8 0.27 (1)	4.26		
H-I	-5086 0	-91.8 -91.8 0.52 (1)	3.87		
I-J	0 28	-91.8 -91.8 0.07 (1)	10.00		
S-B	-3248 0	0.0 0.0 0.11 (1)	7.69		
K-I	-3248 0	0.0 0.0 0.11 (1)	7.69		
S-AB	0 0	-18.5 -18.5 0.04 (4)	10.00		
AB-AC	0 0	-18.5 -18.5 0.04 (4)	10.00		
AC-F	0 0	-18.5 -18.5 0.04 (4)	10.00		
R-AD	0 4553	-18.5 -18.5 0.22 (1)	10.00		
AD-AE	0 4553	-18.5 -18.5 0.22 (1)	10.00		
AE-O	0 4553	-18.5 -18.5 0.22 (1)	10.00		
O-P	0 7283	-18.5 -18.5 0.34 (1)	10.00		
P-AF	0 7283	-18.5 -18.5 0.34 (1)	10.00		
AF-O	0 7283	-18.5 -18.5 0.34 (1)	10.00		
O-AG	0 7283	-18.5 -18.5 0.34 (1)	10.00		
AG-N	0 7283	-18.5 -18.5 0.34 (1)	10.00		
N-M	0 7283	-18.5 -18.5 0.34 (1)	10.00		
M-AH	0 4553	-18.5 -18.5 0.22 (1)	10.00		
AH-AI	0 4553	-18.5 -18.5 0.22 (1)	10.00		
AI-L	0 4553	-18.5 -18.5 0.22 (1)	10.00		
L-AJ	0 0	-18.5 -18.5 0.04 (4)	10.00		
AJ-AK	0 0	-18.5 -18.5 0.04 (4)	10.00		
AK-K	0 0	-18.5 -18.5 0.04 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-429	-429	---	FRONT	VERT	TOTAL	---	C1
D	11-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, OBC 2012, ABC 2019
- PART 8 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1/17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1/17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.38")

CS1: TC=0.52/1.00 (B-C:1), BC=0.34/1.00 (O-Q:1), WB=0.57/1.00 (B-R:1), SS1=0.15/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (R) (INPUT = 0.90)
JSI METAL= 0.56 (P) (INPUT = 1.00)



Structural component only
DWG# T-2007660 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	T40	1	2	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	12.0	1.00	5.50
C	TTWW+m	MT20	7.0	8.0	Edge	
D	TMVW-l	MT20	5.0	8.0		
E	TMVW+w	MT20	3.0	8.0		
F	TS-l	MT20	5.0	8.0		
G	TMVW-l	MT20	5.0	8.0		
H	TTWW+m	MT20	7.0	8.0	Edge	
I	TMVW-p	MT20	4.0	12.0	1.00	5.50
K	BMV-l-p	MT20	3.0	5.0		
L	BMVW-l	MT20	5.0	8.0	2.50	3.00
M	BMVW-l	MT20	5.0	8.0	2.50	3.25
N	BS-l	MT20	8.0	9.0		
O	BMVWW-l	MT20	5.0	8.0		
P	BS-l	MT20	6.0	9.0		
Q	BMVW-l	MT20	5.0	8.0	2.50	3.25
R	BMVW-l	MT20	5.0	8.0	2.50	3.00
S	BMV-l-p	MT20	3.0	5.0		

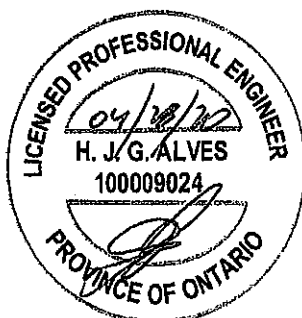
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	17-7-0	-110	-110	---	FRONT	VERT	TOTAL	---	C1
G	23-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
H	29-3-8	-429	-429	---	FRONT	VERT	TOTAL	---	C1
L	29-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
M	23-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
N	21-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	17-7-0	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	13-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Q	11-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
R	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
T	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
U	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	15-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	19-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	21-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	25-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	27-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	1-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AC	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AD	7-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AE	9-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AF	15-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AG	19-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AH	25-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AI	27-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AJ	31-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AK	33-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



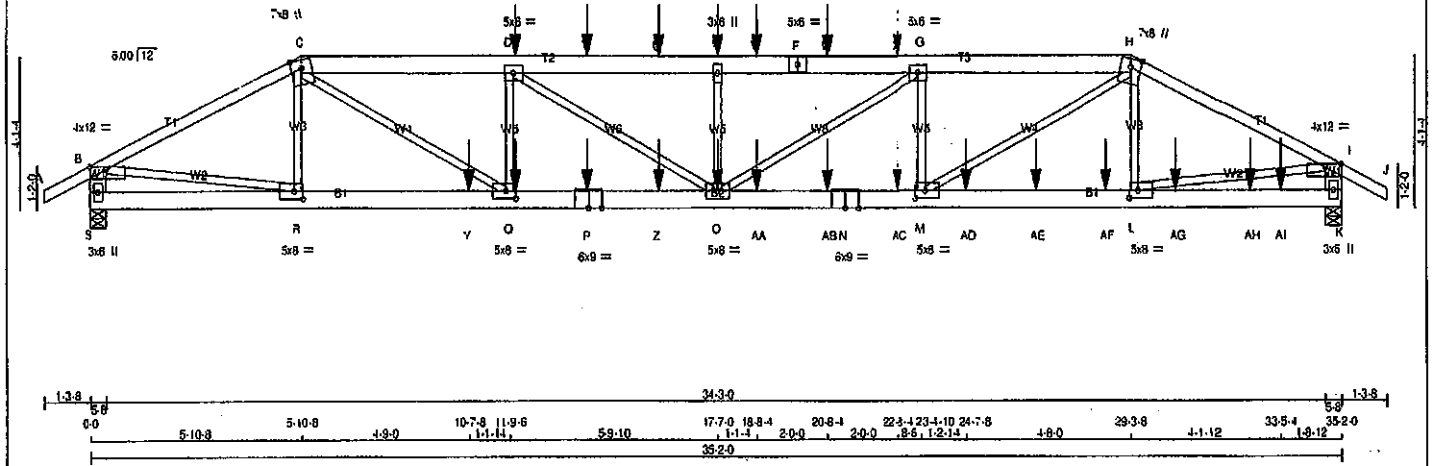
Structural component only
DWG# T-2007660

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408224	T40Z	1	2	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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1-3.8 0.0 5-10.8 5-10.8 5-10.14 11-9.6 59-10 17-7.0 18.9-4 20.8-4 22.3-4 23-4 10 5-10.14 29-3.8 5-10.8 35-2.0 36.6-8 1-3.8 1-3.8 Scale = 1:57.6



TOTAL WEIGHT = 2 X 169 = 338 lb (M)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - F	2x6	DRY	No.2	SPF		
F - H	2x6	DRY	No.2	SPF		
H - J	2x4	DRY	No.2	SPF		
S - B	2x6	DRY	No.2	SPF		
K - I	2x6	DRY	No.2	SPF		
S - P	2x6	DRY	1650 F 1.5E	SPF		
P - N	2x6	DRY	1650 F 1.5E	SPF		
N - K	2x6	DRY	1650 F 1.5E	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1 12	TOP	
H - J 1 12	TOP	
C - F 2 12	SIDE (183.1)	
F - H 2 12	SIDE (0.0)	
S - B 2 12	TOP	
K - I 2 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S - P 2 12	SIDE (183.1)	
P - N 2 12	SIDE (183.1)	
N - K 2 12	SIDE (0.0)	
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
S	4161	0	0	5-8
K	5042	0	0	5-8

UNFACTORED REACTIONS	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
S	2936	1985 0
K	3552	2409 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO					FR-TO		
A-B	0 28	-91.8	-91.8	0.07 (1)	R-C	-470 0	0.08 (1)
B-C	-6650 0	-91.8	-91.8	0.06 (1)	C-Q	0 5608	0.09 (1)
C-D	-10735 0	-91.8	-91.8	0.33 (1)	Q-D	-1678 0	0.20 (1)
D-T	-12118 0	-91.8	-91.8	0.40 (1)	D-O	0 1636	0.20 (1)
T-U	-12119 0	-91.8	-91.8	0.40 (1)	O-E	-1004 0	0.12 (1)
U-E	-12119 0	-91.8	-91.8	0.40 (1)	E-G	-46 33	0.02 (1)
E-V	-12119 0	-91.8	-91.8	0.45 (1)	M-G	-626 0	0.10 (1)
V-F	-12119 0	-91.8	-91.8	0.45 (1)	F-H	0 5832	0.72 (1)
F-W	-12119 0	-91.8	-91.8	0.45 (1)	L-H	0 57	0.01 (1)
W-X	-12119 0	-91.8	-91.8	0.45 (1)	X-G	0 8014	0.74 (1)
X-G	-12119 0	-91.8	-91.8	0.45 (1)	L-I	0 7274	0.90 (1)
G-H	-12159 0	-91.8	-91.8	0.39 (1)			
H-I	-8042 0	-91.8	-91.8	0.81 (1)			
I-J	0 28	-91.8	-91.8	0.07 (1)			
S-B	-4141 0	0.0	0.0	0.15 (1)			
K-I	-4926 0	0.0	0.0	0.17 (1)			

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
S-R	0 0	-18.5	-18.5	0.05 (1)
R-Y	0 5974	-18.5	-18.5	0.44 (1)
Y-Q	0 5974	-18.5	-18.5	0.44 (1)
Q-P	0 10734	-18.5	-18.5	0.54 (1)
P-Z	0 10734	-18.5	-18.5	0.54 (1)
Z-O	0 10734	-18.5	-18.5	0.54 (1)
O-AA	0 12159	-18.5	-18.5	0.71 (1)
AA-AB	0 12159	-18.5	-18.5	0.71 (1)
AB-N	0 12159	-18.5	-18.5	0.71 (1)
N-AC	0 12159	-18.5	-18.5	0.71 (1)
AC-M	0 12159	-18.5	-18.5	0.71 (1)
M-AD	0 7208	-18.5	-18.5	0.69 (1)
AD-AE	0 7208	-18.5	-18.5	0.69 (1)
AE-AF	0 7208	-18.5	-18.5	0.69 (1)
AF-L	0 7208	-18.5	-18.5	0.69 (1)
L-AG	0 0	-18.5	-18.5	0.16 (1)
AG-AH	0 0	-18.5	-18.5	0.16 (1)
AH-AI	0 0	-18.5	-18.5	0.16 (1)
AI-K	0 0	-18.5	-18.5	0.16 (1)

FACTORED CONCENTRATED LOADS (LBS)	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
JT	11-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
D	17-7-0	-110	-110	---	BACK	VERT	TOTAL	---	C1
O	17-7-0	-28	-28	---	BACK	VERT	TOTAL	---	C1
P	13-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, CBC 2012, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/999 (0.32")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = L/718 (0.59")

CSI: TC=0.81, 1.00 (H-1:1), BC=0.71, 1.00 (M-O:1), WB=0.90, 1.00 (L-1:1), SSI=0.84, 1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (R) (INPUT = 0.90)
JSI METAL= 0.93 (P) (INPUT = 1.00)



Structural component only
DWG# T-2007661

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	T40Z	1	2	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:19:58 2020 Page 2
ID:DMCubINVR6TstFoe31v6I zns11-x2qeJGBoaF2rpnibcaNm9BWHeyQ8EpcGa1mZakzME7I

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	12.0	1.00	5.50
C	TTWW+m	MT20	7.0	8.0	Edge	
D	TMWW-l	MT20	5.0	6.0		
E	TMWW+w	MT20	3.0	6.0		
F	TS-l	MT20	5.0	6.0		
G	TMWW-l	MT20	5.0	6.0		
H	TTWW+m	MT20	7.0	8.0	Edge	
I	TMVW-p	MT20	4.0	12.0	1.00	5.50
K	BMVl+p	MT20	3.0	6.0		
L	BMWW-l	MT20	5.0	8.0	2.50	3.00
M	BMWW-l	MT20	5.0	8.0	2.50	3.25
N	BS-l	MT20	6.0	9.0		
O	BMWW-l	MT20	5.0	8.0		
P	BS-l	MT20	6.0	9.0		
Q	BMWW-l	MT20	5.0	8.0	2.50	3.25
R	BMWW-l	MT20	5.0	8.0	2.50	3.00
S	BMVl+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	11-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
T	13-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
U	15-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
V	18-8-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
W	20-8-4	-159	-159	---	BACK	VERT	TOTAL	---	C1
X	22-8-4	-159	-159	---	BACK	VERT	TOTAL	---	C1
Y	10-7-8	-1171	-1171	---	BACK	VERT	TOTAL	---	C1
Z	15-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AA	18-8-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AB	20-8-4	-32	-32	---	BACK	VERT	TOTAL	---	C1
AC	22-8-4	-32	-32	---	BACK	VERT	TOTAL	---	C1
AD	24-7-8	-2237	-2237	---	BACK	VERT	TOTAL	---	C1
AE	26-6-12	-121	-121	---	BACK	VERT	TOTAL	---	C1
AF	28-6-12	-121	-121	---	BACK	VERT	TOTAL	---	C1
AG	30-6-12	-121	-121	---	BACK	VERT	TOTAL	---	C1
AH	32-6-12	-121	-121	---	BACK	VERT	TOTAL	---	C1
AI	33-5-4	-121	-121	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

11 C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007661

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	OL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	OL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00-12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR

CSA 086-09, CSA 008-14
-TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G S.L. PLUS 8.4 P.S.F. RAIN
LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF
LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17')
CALCULATED VERT. DEFL.(LL) = L/999 (0.21')
ALLOWABLE DEFL.(TL)= L/360 (1.17')
CALCULATED VERT. DEFL.(TL) = L/999 (0.41')

CSI: TC=0.96/1.00 (D-E:1), BC=0.84/1.00 (O-P:1),
WB=0.83/1.00 (L-L:1), SS=1.00/281.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP (DRY)	SHEAR	SECTION	
(PSI)	(PLI)	(PLI)	

	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	.354	1867	.708	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

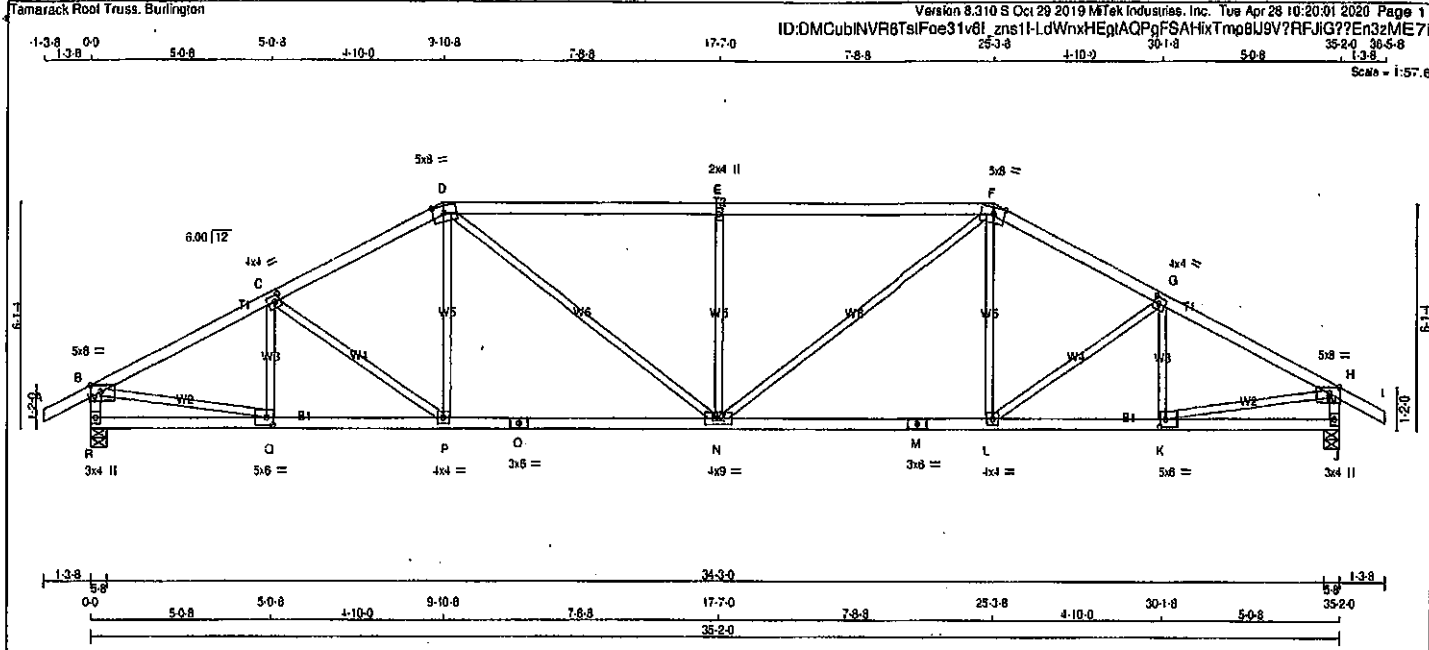
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP = 0.89 (L) (INPUT = 0.90)

```
JSIMETAL=0.77 (N) INPUT = (0.00 )
```

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	T42	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:20:01 2020 Page 1			
		ID:DMCubINVR6TsiFoe31vdl_zns1L-LdWnxHegAQpFSaHixTnp8U9V7RFJIG??En3ME7			
		35-24 30-5-8 13-8			
		Scale = 1:57.6			



LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No 2	SPF		
D - F	2x4 DRY	No 2	SPF		
F - I	2x4 DRY	No 2	SPF		
R - B	2x4 DRY	No 2	SPF		
J - H	2x4 DRY	No 2	SPF		
R - O	2x4 DRY	No 2	SPF		
O - M	2x4 DRY	No 2	SPF		
M - J	2x4 DRY	No 2	SPF		
ALL WEBS	2x3 DRY	No 2	SPF		
EXCEPT					

DAY: SEASONED LUMBER.

PLATE TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge 3.50
C	TMWW-l	MT20	4.0	4.0	2.00 1.75
D	TTWW-m	MT20	5.0	8.0	2.25 3.75
E	TMW-w	MT20	2.0	4.0	
F	TTWW-m	MT20	5.0	8.0	2.25 3.75
G	TMWW-l	MT20	4.0	4.0	2.00 1.75
H	TMVW-p	MT20	5.0	8.0	Edge 3.50
J	BMV1-p	MT20	3.0	4.0	
K	BMWW-l	MT20	5.0	8.0	2.50 2.00
L	BMWW-l	MT20	4.0	4.0	
M	BS-l	MT20	3.0	8.0	
N	BMWW-l	MT20	4.0	4.0	
O	BS-l	MT20	3.0	8.0	
P	BMWW-l	MT20	4.0	4.0	
Q	BMWW-l	MT20	5.0	8.0	2.50 2.00
R	BMV1-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ		DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
R	2063	0	2063	0	0	5-8	5-8	5-8	
J	2063	0	2063	0	0	5-8	5-8	5-8	

UNFACTORED REACTIONS		1ST LOOSE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
R	1457	969	0	0	0
J	1457	969	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 28	-91.8 -91.8 0.12 (1)	10.00	C-Q	-353 0	0.08 (1)	
B-C	-2634 0	-91.8 -91.8 0.39 (1)	3.81	C-P	-210 0	0.13 (1)	
C-D	-2667 0	-91.8 -91.8 0.37 (1)	3.91	P-D	0 246	0.08 (4)	
D-E	-3080 0	-91.8 -91.8 0.97 (1)	2.78	D-N	0 844	0.19 (1)	
E-F	-3080 0	-91.8 -91.8 0.97 (1)	2.78	N-E	-872 0	0.51 (1)	
F-G	-2667 0	-91.8 -91.8 0.37 (1)	3.91	N-F	0 844	0.19 (1)	
G-H	-2634 0	-91.8 -91.8 0.39 (1)	3.81	L-F	0 246	0.08 (4)	
H-I	0 28	-91.8 -91.8 0.12 (1)	10.00	L-G	-210 0	0.13 (1)	
I-B	-2018 0	0.0 0.0 0.20 (1)	5.94	K-G	-353 0	0.08 (1)	
J-H	-2018 0	0.0 0.0 0.20 (1)	5.94	B-Q	0 2591	0.58 (1)	
				K-H	0 2591	0.58 (1)	
R-Q	0 0	-18.5 -18.5 0.10 (4)	10.00				
Q-P	0 2555	-18.5 -18.5 0.51 (1)	10.00				
P-O	0 2388	-18.5 -18.5 0.49 (1)	10.00				
O-N	0 2388	-18.5 -18.5 0.49 (1)	10.00				
N-M	0 2388	-18.5 -18.5 0.49 (1)	10.00				
M-L	0 2388	-18.5 -18.5 0.49 (1)	10.00				
L-K	0 2555	-18.5 -18.5 0.51 (1)	10.00				
K-J	0 0	-18.5 -18.5 0.10 (4)	10.00				

TOTAL WEIGHT = 2 X 139 = 277 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.34")

CSI: TC=0.97 1.00 (D-E:1) . BC=0.51 1.00 (P-Q:1) .

WB=0.58 1.00 (B-Q:1) . SS=0.34 1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

MAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 789 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (F) INPUT = 0.90
JSI METAL= 0.75 (M) INPUT = 1.00



Structural component only
DWG# T-2007663

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	T43	2	1	GREEN PARK HOMES	

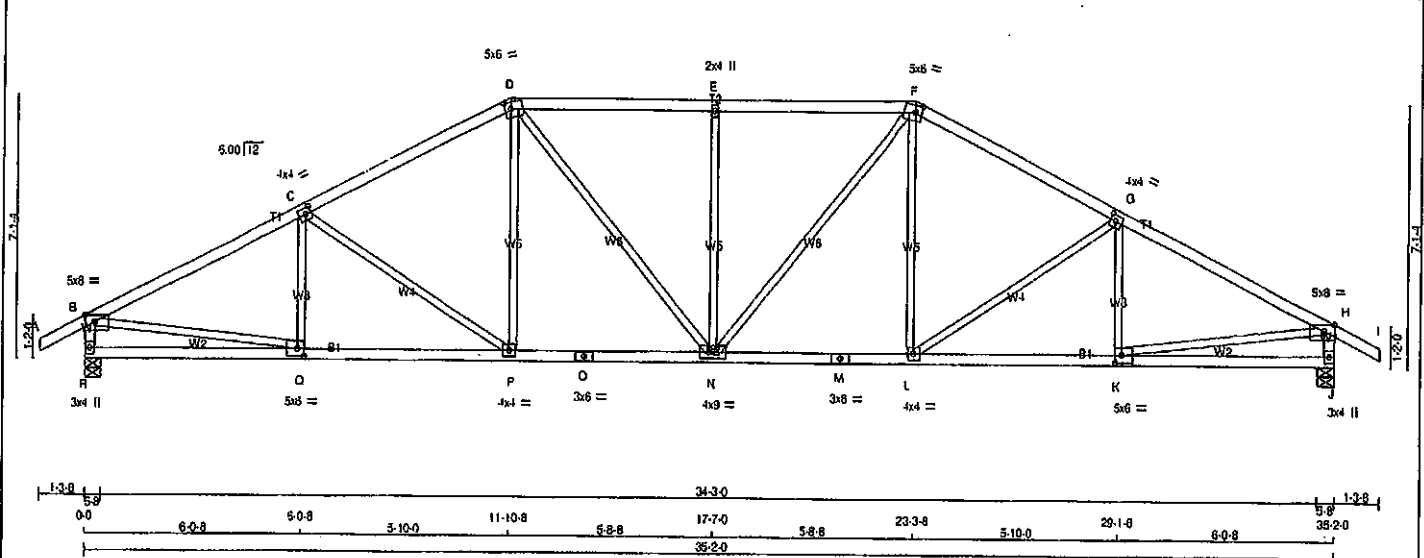
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23-3-8 5-10-0 29-1-4 35-2-0 36-5-8

Scale = 1:57.6



CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
C - H	2x4	DRY	No.2
H - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (tablets in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMVW-p	MT20	5.0	8.0	Edge 3.50
C TMVW-l	MT20	4.0	4.0	2.00 1.75
D TTWW-m	MT20	5.0	6.0	2.25 2.00
E TMVW-w	MT20	2.0	4.0	
F TTWW-m	MT20	5.0	6.0	2.25 2.00
G TMVW-l	MT20	4.0	4.0	2.00 1.75
H TMVW-p	MT20	5.0	8.0	Edge 3.50
J BMVl+p	MT20	3.0	4.0	
K BMVW-l	MT20	5.0	6.0	2.50 2.25
L BMVW-l	MT20	4.0	4.0	
M BS-l	MT20	3.0	6.0	
N BMVW-l	MT20	4.0	9.0	
O BS-l	MT20	3.0	6.0	
P BMVW-l	MT20	4.0	4.0	
Q BMVW-l	MT20	5.0	6.0	2.50 2.25
R BMVl+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORZ	UPLIFT
R	2063	0	0	5-8
J	2063	0	0	5-8

UNFACTORED REACTIONS	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
R	1457	989 0
J	1457	989 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING	CHORDS	WEBS
TOTAL LOAD CASES: (4)	MAX. FACTORED	MAX. FACTORED
	MEMB. FORCE (LBS)	MEMB. FORCE (LBS)
	FR-TO	FR-TO
	A-B	0 28
	B-C	-2689 0
	C-D	-2539 0
	D-E	-2559 0
	E-F	-2559 0
	F-G	-2539 0
	G-H	-2689 0
	H-I	0 28
	I-J	-2014 0
	J-K	-2014 0
	K-L	0 0
	L-M	0 2809
	M-N	0 2249
	N-O	0 2249
	O-P	0 2249
	P-Q	0 2249
	Q-R	0 2249
	R-S	0 2609
	S-T	0 0

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")

CALCULATED VERT. DEFL.(LL) = L/999 (0.15")

ALLOWABLE DEFL.(TL) = L/360 (1.17")

CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.59/1.00 (G-H:1), SC=0.49/1.00 (K-L:1), WB=0.59/1.00 (H-K:1), SI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

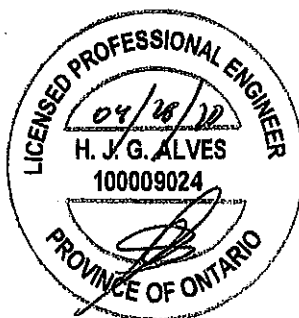
MT20 618 354 1867 788 1997 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)

JSI METAL= 0.89 (M) (INPUT = 1.00)



Structural component only

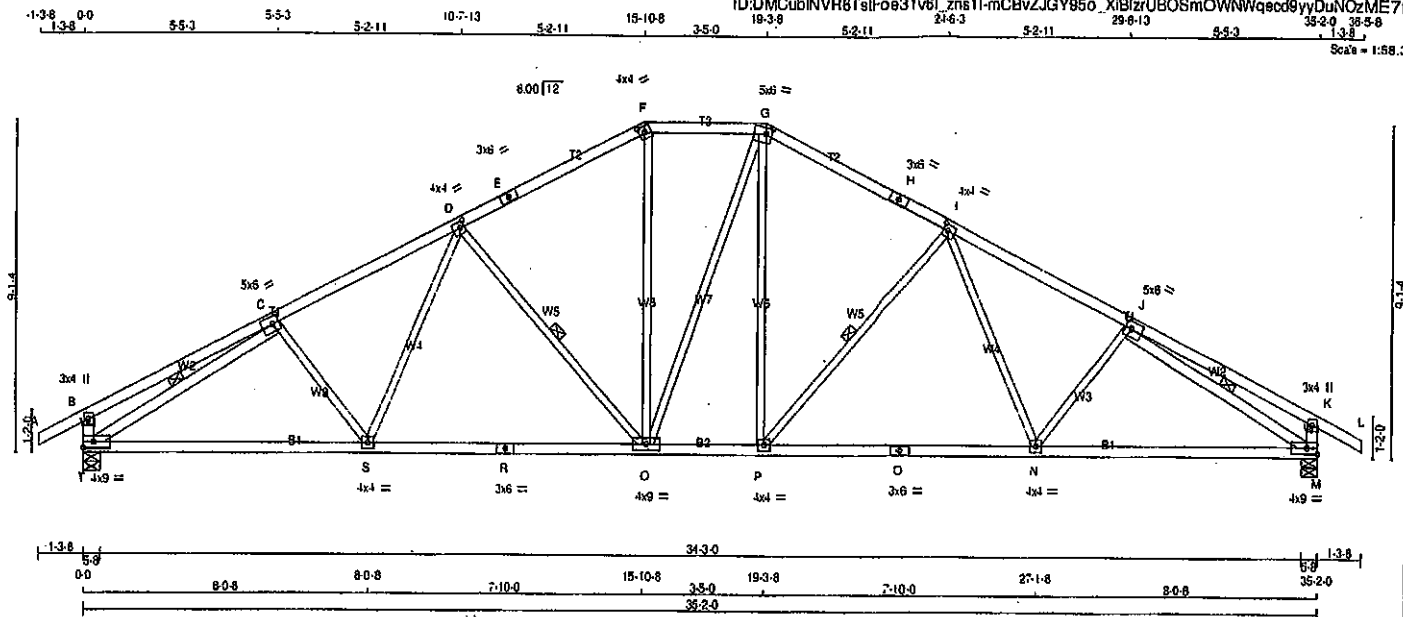
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	T45	2	1	GREEN PARK HOMES	
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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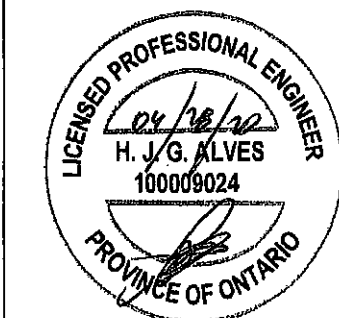
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TOTAL WEIGHT = 2 X 155 = 310 lb (M/F)

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
I - J	2x4	DRY	No.2
J - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - N	2x4	DRY	No.2
N - O	2x4	DRY	No.2
O - P	2x4	DRY	No.2
P - Q	2x4	DRY	No.2
Q - R	2x4	DRY	No.2
R - S	2x4	DRY	No.2
S - T	2x4	DRY	No.2
T - U	2x4	DRY	No.2
U - V	2x4	DRY	No.2
V - W	2x4	DRY	No.2
W - X	2x4	DRY	No.2
X - Y	2x4	DRY	No.2
Y - Z	2x4	DRY	No.2
Z - A	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
T - C	2x4	DRY	No.2
J - M	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table in inches)			
JT TYPE	PLATES	W	LEN Y X
B TMV+p	MT20	3.0	4.0
C TMWW-1	MT20	5.0	6.0 2.50 2.25
D TMWW-1	MT20	4.0	4.0 2.00 1.50
E TS-1	MT20	3.0	6.0
F TTW-h	MT20	4.0	4.0 2.00 1.75
G TTWW-m	MT20	5.0	6.0 2.00 2.00
H TS-1	MT20	3.0	6.0
I TMWW-1	MT20	4.0	4.0 2.00 1.50
J TMWW-1	MT20	5.0	6.0 2.50 2.25
K TMV+p	MT20	3.0	4.0
M BMVW1-1	MT20	4.0	9.0 Edge
N, P, S			
N BMVW-1	MT20	4.0	4.0
O BS-1	MT20	3.0	6.0
Q BMVW-1	MT20	4.0	9.0
R BS-1	MT20	3.0	6.0
T BMVW1-1	MT20	4.0	9.0 Edge
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.			



Structural component only
DWG# T-2007666

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT VERT	0	2063	0	0	5-8	5-8	
M	2063	0	2063	0	5-8	5-8	

UNFACTORED REACTIONS		MAX. MIN. COMPONENT REACTIONS	
1ST LOOSE		SNOW	LIVE
JT COMBINED		1457	989
T	1457	989	0
M	1457	989	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-Q, I-P, C-T, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO	
A-B	0 28	-91.8 -91.8 0.12 (1)	10.00	C-S	-110 37
B-C	0 20	-91.8 -91.8 0.32 (1)	10.00	S-D	0 278
C-D	-2760 0	-91.8 -91.8 0.40 (1)	3.88	D-Q	-682 0
D-E	-2167 0	-91.8 -91.8 0.38 (1)	4.30	Q-F	0 614
E-F	-2167 0	-91.8 -91.8 0.38 (1)	4.30	F-G	0 4
F-G	-1925 0	-91.8 -91.8 0.18 (1)	4.71	G-H	0 609
G-H	-2165 0	-91.8 -91.8 0.38 (1)	4.30	H-I	-683 0
H-I	-2165 0	-91.8 -91.8 0.38 (1)	4.30	I-N	0 279
I-J	-2781 0	-91.8 -91.8 0.40 (1)	3.88	N-J	-110 37
J-K	0 20	-91.8 -91.8 0.32 (1)	10.00	T-C	-3042 0
K-L	0 28	-91.8 -91.8 0.12 (1)	10.00	J-M	-3043 0
T-B	-325 0	0.0 0.0 0.03 (1)	7.61		
M-K	-325 0	0.0 0.0 0.03 (1)	7.61		
T-S	0 2537	-18.5 -18.5 0.58 (1)	10.00		
S-R	0 2363	-18.5 -18.5 0.53 (1)	10.00		
R-Q	0 2363	-18.5 -18.5 0.53 (1)	10.00		
Q-P	0 1924	-18.5 -18.5 0.40 (1)	10.00		
P-O	0 2362	-18.5 -18.5 0.53 (1)	10.00		
O-N	0 2362	-18.5 -18.5 0.53 (1)	10.00		
N-M	0 2538	-18.5 -18.5 0.58 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17)
CALCULATED VERT. DEFL.(LL) = L/999 (0.14)
ALLOWABLE DEFL.(TL) = L/360 (1.17)
CALCULATED VERT. DEFL.(TL) = L/999 (0.29)

CS1: TC=0.40-1.00 (J-I), BC=0.58-1.00 (M-N), WB=0.61-1.00 (J-M), SS=0.20-1.00 (G-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

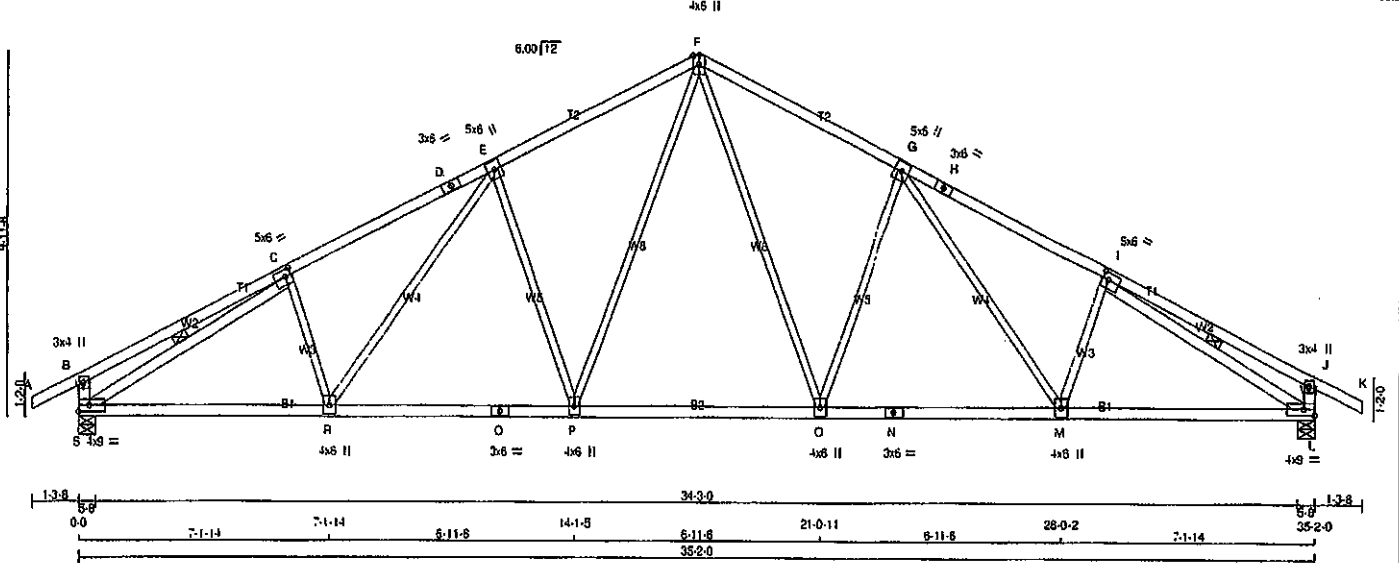
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (F) (INPUT = 0.90)
JSI METAL = 0.83 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	T46	8	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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1-3-8 0-0 3-0-0 6-0-0 9-0-8 11-9-8 14-0-8 17-0-0 19-9-8 23-0-0 25-9-8 29-2-0 32-0-0 35-5-8
1-3-8 0-0 3-0-0 6-0-0 9-0-8 11-9-8 14-0-8 17-0-0 19-9-8 23-0-0 25-9-8 29-2-0 32-0-0 35-5-8
Scale = 1:50.2



TOTAL WEIGHT = 8 X 152 = 1213 lb
(M/F)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No. 2	SPF		
D - F	2x4 DRY	No. 2	SPF		
F - H	2x4 DRY	No. 2	SPF		
H - K	2x4 DRY	No. 2	SPF		
S - B	2x4 DRY	No. 2	SPF		
L - J	2x4 DRY	No. 2	SPF		
S - Q	2x4 DRY	No. 2	SPF		
Q - N	2x4 DRY	No. 2	SPF		
N - L	2x4 DRY	No. 2	SPF		
ALL WEBS	2x3 DRY	No. 2	SPF		
EXCEPT					
S - C	2x4 DRY	No. 2	SPF		
I - L	2x4 DRY	No. 2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-1	MT20	5.0	6.0	2.25	2.00
D	TS-1	MT20	3.0	6.0		
E	TMWW+1	MT20	5.0	6.0		
F	TTWW+p	MT20	4.0	6.0	Edge	
G	TMWW+1	MT20	5.0	6.0		
H	TS-1	MT20	3.0	6.0		
I	TMWW-1	MT20	5.0	6.0	2.25	2.00
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-1	MT20	4.0	9.0	Edge	
M, O, P, R						
M	BMVW+1	MT20	4.0	6.0		
N	BS-1	MT20	3.0	6.0		
Q	BS-1	MT20	3.0	6.0		
S	BMVW1-1	MT20	4.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

READINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
S	2083	0	2083	0
L	2083	0	2083	0

UNFACTORED REACTIONS	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED							
S	1457	989	0	0	0	0	488	0
L	1457	989	0	0	0	0	488	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-S, L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING	CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORZ. LOAD (LBS)	MAX. FACTORED UPLIFT (LBS)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LBS)	MAX. FACTORED UPLIFT (LBS)
TOTAL LOAD CASES: (4)											
	PR-TO						FR-TO				
	A-B	0	28	-91.8	-91.8	0.12 (1)	10.00	F-O	0	886	0.19 (1)
	B-C	0	22	-91.8	-91.8	0.40 (1)	10.00	O-G	-726	0	0.72 (1)
	C-D	-2808	0	-91.8	-91.8	0.49 (1)	3.75	G-M	0	391	0.09 (1)
	D-E	-2808	0	-91.8	-91.8	0.49 (1)	3.75	M-I	-192	18	0.05 (1)
	E-F	-2322	0	-91.8	-91.8	0.46 (1)	4.07	P-F	0	866	0.19 (1)
	F-G	-2322	0	-91.8	-91.8	0.46 (1)	4.07	E-P	-726	0	0.72 (1)
	G-H	-2808	0	-91.8	-91.8	0.49 (1)	3.75	R-E	0	391	0.09 (1)
	H-I	-2808	0	-91.8	-91.8	0.49 (1)	3.75	C-R	-192	18	0.05 (1)
	I-J	0	22	-91.8	-91.8	0.40 (1)	10.00	S-C	-3084	0	0.72 (1)
	J-K	0	28	-91.8	-91.8	0.12 (1)	10.00	I-L	-3084	0	0.72 (1)
	S-B	-344	0	0.0	0.0	0.03 (1)	7.81				
	L-J	-344	0	0.0	0.0	0.03 (1)	7.81				
	S-R	0	2574	-18.5	-18.5	0.53 (1)	10.00				
	R-O	0	2291	-18.5	-18.5	0.47 (1)	10.00				
	O-P	0	2291	-18.5	-18.5	0.47 (1)	10.00				
	P-O	0	1762	-18.5	-18.5	0.39 (1)	10.00				
	O-N	0	2291	-18.5	-18.5	0.47 (1)	10.00				
	N-M	0	2291	-18.5	-18.5	0.47 (1)	10.00				
	M-L	0	2574	-18.5	-18.5	0.53 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17')
CALCULATED VERT. DEFL.(LL) = L/999 (0.14')
ALLOWABLE DEFL.(TL) = L/360 (1.17')
CALCULATED VERT. DEFL.(TL) = L/999 (0.28')

CSI: TC=0.49/1.00 (C-E:1), BC=0.53/1.00 (L-M:1), WB=0.72/1.00 (E-P:1), SSI=0.22/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

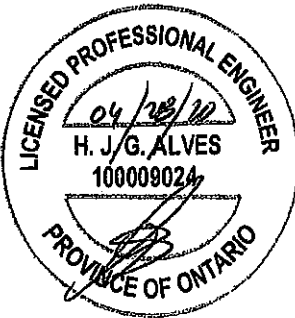
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

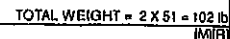
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.77 (I) (INPUT = 1.00)



Structural component only
DWG# T-2007667

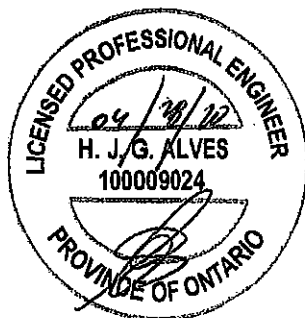


PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	6.0	
B	TMV+p	MT20	3.0	3.0	
C	TMVWV-l	MT20	4.0	4.0	
D	TMV+p	MT20	3.0	4.0	
E	TMVW+p	MT20	4.0	6.0	
F	8MVWV-l	MT20	4.0	4.0	
G	8MV+p	MT20	3.0	4.0	
H	8MVWVWV-l	MT20	6.0	9.0	3.00 3.50
I	8MVWVWV-l	MT20	6.0	9.0	3.00 3.50
J	8MV+p	MT20	3.0	4.0	
K	8MVWV-l	MT20	4.0	4.0	

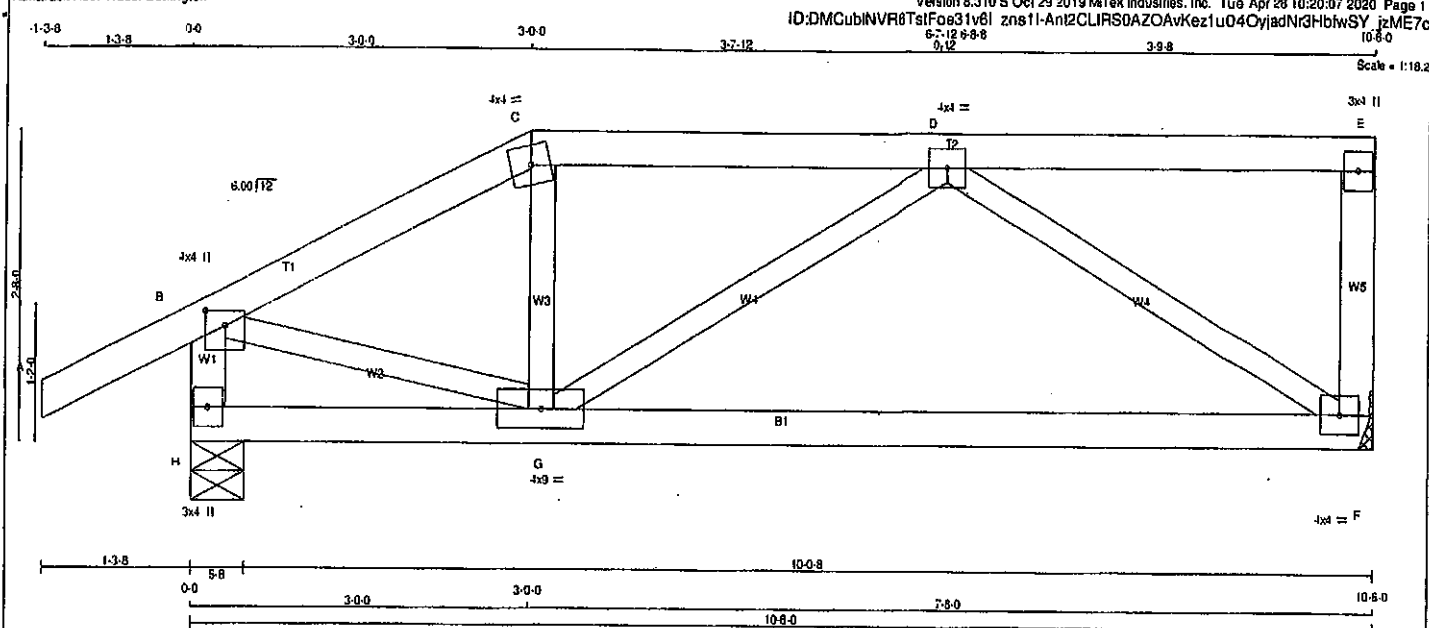
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
K-A	-687 0	0.0	0.0 0.17 (1)	7.81	K-I	-28 0	0.00 (11)
A-B	-497 0	-114.3	-114.3 0.14 (1)	8.25	A-J	0 783	0.24 (1)
B-C	-508 0	-114.3	-114.3 0.22 (1)	8.25	N-F	-28 0	0.00 (11)
C-D	-508 0	-114.3	-114.3 0.22 (1)	8.25	H-E	0 783	0.24 (1)
D-E	-497 0	-114.3	-114.3 0.14 (1)	8.25	I-C	-384 0	0.15 (11)
E-F	-687 0	0.0	0.0 0.17 (1)	7.81	C-H	-384 0	0.15 (11)
K-J	0 25	-18.5	-18.5 0.03 (4)	10.00			
J-I	0 19	0.0	0.0 0.05 (1)	10.00			
I-B	-331 0	0.0	0.0 0.05 (1)	7.81			
I-H	0 832	-18.5	-18.5 0.48 (4)	10.00			
G-H	0 19	0.0	0.0 0.05 (1)	10.00			
H-O	-331 0	0.0	0.0 0.05 (1)	7.81			
G-F	0 25	-18.5	-18.5 0.03 (4)	10.00			

JSI GRIP= 0.77 (H) ; INPUT = 0.90 ;
JSI METAL= 0.30 (E) ; INPUT = 1.00 ;



Structural component only
DWG# T-2007668



TOTAL WEIGHT = 40 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (tablets in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.50 2.00
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	TMV+p	MT20	3.0	4.0	
F	8MVW1-t	MT20	4.0	4.0	
G	8MVWVW-t	MT20	4.0	9.0	
H	8MV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

REARINGS						
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
F	579	0	679	0	0	MECHANICAL
H	703	0	703	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	409	269 0	0 0	0 0	0 0	141 0	0 0
H	495	338 0	0 0	0 0	0 0	157 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS			FACTORED			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	VERY. LOAD LG1 (PLF)	MAX CSl (L/G)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSl (L/G)	
FR-TO		FROM TO			FR-TO			
A-B	0 28	-91.8	-91.8 0.12 (1)	10.00	G-C	0 61	0.02 (4)	
B-C	-835 0	-91.8	-91.8 0.15 (1)	6.25	G-D	-60 60	0.02 (4)	
C-D	-589 0	-91.8	-91.8 0.21 (1)	8.25	D-E	-731 0	0.23 (1)	
D-E	0 0	-91.8	-91.8 0.20 (1)	10.00	B-G	0 597	0.19 (1)	
F-E	-138 0	0.0	0.0 0.02 (1)	7.81				
H-B	-699 0	0.0	0.0 0.07 (1)	7.81				
H-G	0 0	-18.5	-18.5 0.21 (4)	10.00				
G-F	0 819	-18.5	-18.5 0.28 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:				
TOP	CH.	LL	=	25.6 PSF
		OL	=	8.0 PSF
BOT	CH.	LL	=	0.0 PSF
		DL	=	7.4 PSF
TOTAL LOAD			=	39.0 PSF

SPACING is **24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9.
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN
LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF
LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CS1: TC=0.21/1.00 (C-D:1) , BC=0.25/1.00 (F-G:4) ,
WB=0.23/1.00 (D-F:1) , SS1=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

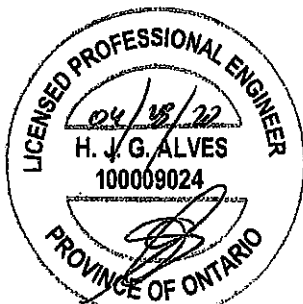
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	{PSI}		{PLI}		{PLI}
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	708	1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

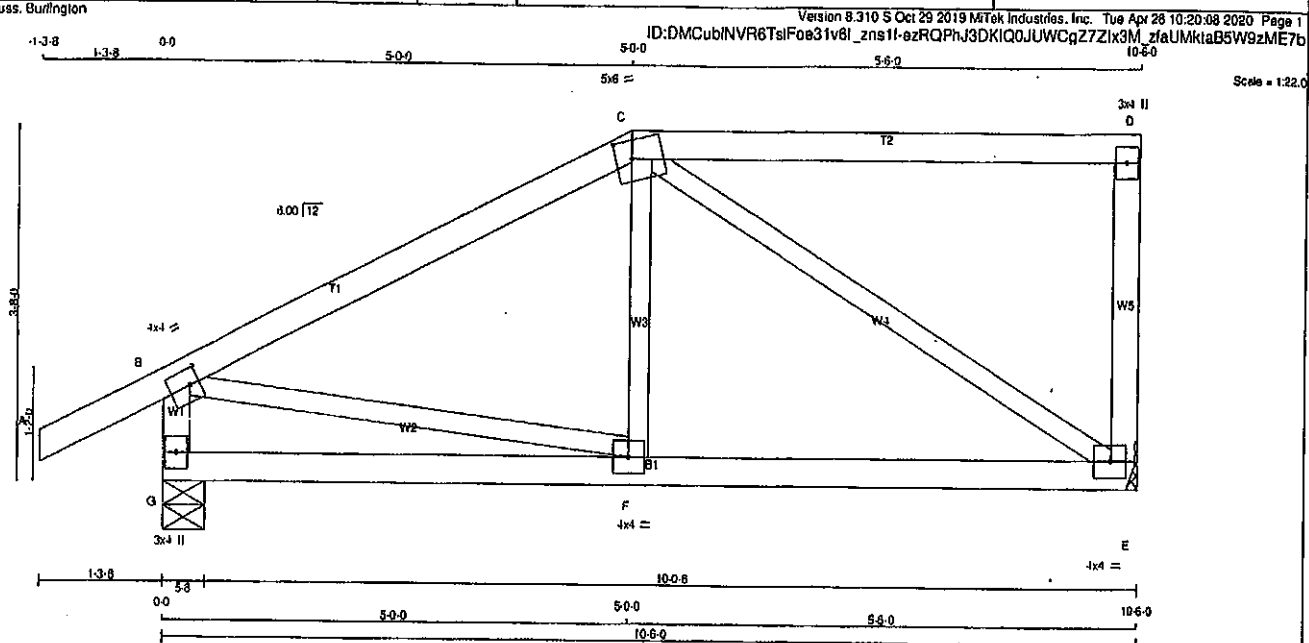
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.68 (B) (INPUT = 0.90)
JSI METAL = 0.23 (F) (INPUT = 1.00)



Structural component only
DWG# T-2007669

JOB NAME 408224	TRUSS NAME T49	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (tablets/inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	4.0	4.0	2.00	1.25
C TTWW-m	MT20	5.0	6.0	2.25	2.00
D TMVW-p	MT20	3.0	4.0		
E BMVW1-t	MT20	4.0	4.0		
F BMVW1-t	MT20	4.0	4.0		
G BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
E	579 0	579 0	0 0	0 0	MECHANICAL
G	703 0	703 0	0 0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL
E	409 289 0 0 0 0 141 0 0 0
G	495 338 0 0 0 0 157 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 28	F-C	0 108
B-C	-523 0	C-E	-549 0
C-D	0 0	E-F	0 474
E-D	-252 0		
G-B	-667 0		
G-F	0 0		
F-E	0 468		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.47 1.00 (C-D:1), BC=0.19 1.00 (E-F:4), WB=0.37 1.00 (C-E:1), SS=0.20 1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSD) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.85 (B) (INPUT = 0.90)
JSI METAL=0.28 (B) (INPUT = 1.00)

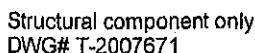


Structural component only
DWG# T-2007670

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 10:20:10 2020 Page 1
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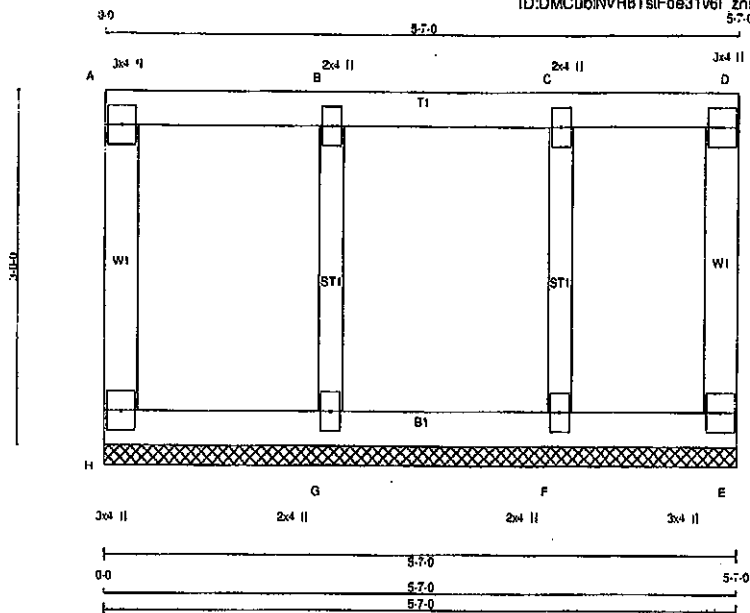
JSI GRIP = 0.25 (A) (INPUT = 0.90)
JSI METAL = 0.07 (A) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	G50	1	1	GREEN PARK HOMES	
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:DMCubINVR8TsiFoe31v6f zns11-eiv?it6OD89qThFic17MikFo7CC5i2EzRaiQezME7e



Scale = 1/16"

LUMBER	N. L. G. A. RULES
CHORDS	SIZE
H - A	2x4 DRY No.2
A - D	2x4 DRY No.2
E - D	2x4 DRY No.2
H - E	2x4 DRY No.2

ALL WEBS	2x3 DRY No.2
ALL GABLE WEBS	2x3 DRY No.2
DRY: SEASONED LUMBER.	

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMW+vv	MT20	2.0	4.0		
C	TMW+vv	MT20	2.0	4.0		
D	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMW1+vv	MT20	2.0	4.0		
G	BMW1+vv	MT20	2.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.
BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
H-A	-101 0	0.0 0.0	0.03 (1)	7.81	G-B	-248 0	0.08 (1)
A-B	-8 0	-114.3 -114.3	0.08 (1)	10.00	F-C	-209 0	0.05 (1)
B-C	-8 0	-114.3 -114.3	0.08 (1)	10.00			
C-D	-8 0	-114.3 -114.3	0.08 (1)	10.00			
E-D	-80 0	0.0 0.0	0.03 (1)	7.81			
H-G	0 8	-18.5 -18.5	0.02 (4)	10.00			
G-F	0 8	-18.5 -18.5	0.02 (4)	10.00			
F-E	0 8	-18.5 -18.5	0.02 (4)	10.00			

TOTAL WEIGHT = 22 lb (M/R)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 15.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 48.0 PSF	

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08:1.00 (A-B:1), SC=0.02:1.00 (G-H:4), WB=0.08:1.00 (B-G:1), SSI=0.13:1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1687 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.17 (G) INPUT = 0.90
JSI METAL = 0.07 (G) INPUT = 1.00

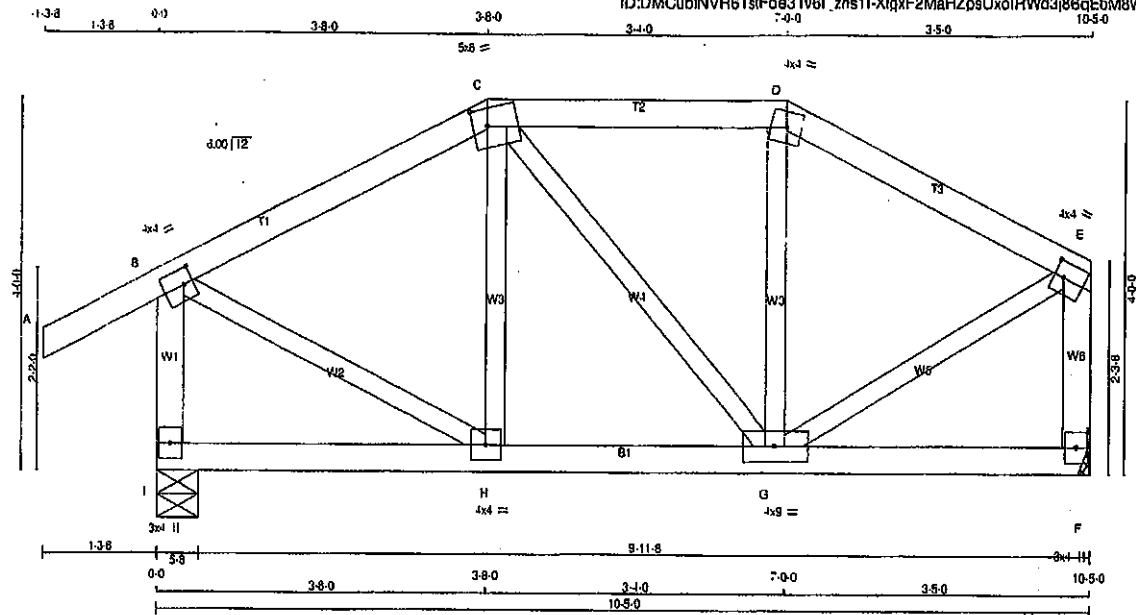


Structural component only
DWG# T-2007656

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	T52	1	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 26 10:20:12 2020 Page 1
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Scale = 1:22.8

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES					
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
G - H	2x4	DRY	No.2	SPF	
H - I	2x4	DRY	No.2	SPF	
I - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-I	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMW-I	MT20	4.0	4.0	2.00	1.25
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-I	MT20	4.0	9.0		
H	BMWW-I	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
I	699	0	699	0
F	574	0	574	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE	PERM. LIVE
I	492	338	0	0
F	406	287	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX
FR-TO	(LBS)	(PLF)	CS1 (LC)	UNBRAC	LENGTH	FR-TO	(LBS)	CS1 (LC)
A-B	0	28	-91.8	-91.8	0.12 (1)	10.00	H-C	-118
B-C	-423	0	-91.8	-91.8	0.16 (1)	6.25	C-D	-20
C-D	-362	0	-91.8	-91.8	0.13 (1)	6.25	D-E	-128
D-E	-409	0	-91.8	-91.8	0.14 (1)	6.25	E-F	0
E-F	-869	0	0.0	0.0	0.08 (1)	7.81	F-G	0
F-G	-546	0	0.0	0.0	0.07 (1)	7.81	G-H	0
H-I	0	0	-18.5	-18.5	0.05 (4)	10.00	I-H	0
I-H	0	375	-18.5	-18.5	0.09 (1)	10.00	H-G	0
G-F	0	0	-18.5	-18.5	0.05 (4)	10.00	F-E	0

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.35")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.35")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CS1: TC=0.16/1.00 (B-C:1), BC=0.09/1.00 (G-H:1), WB=0.09/1.00 (B-H:1), SS1=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

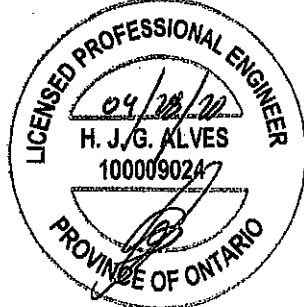
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	818	354	1687

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

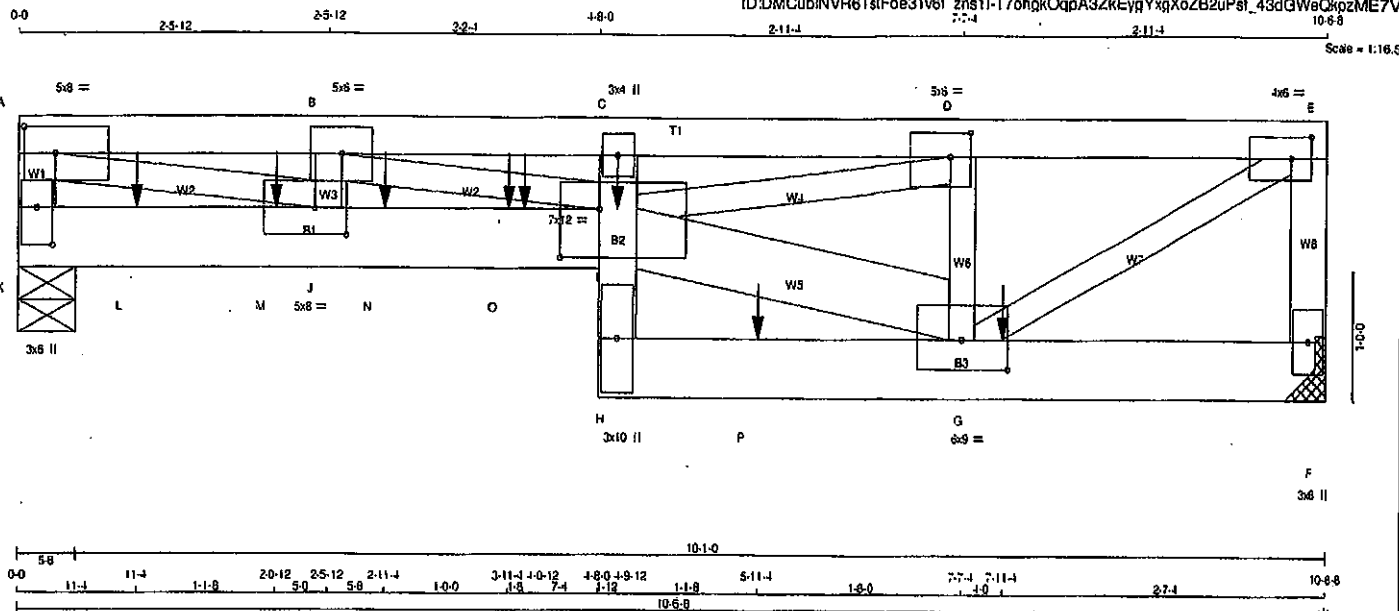
JSI GRIP = 0.65 (B) INPUT = 0.90
JSI METAL = 0.21 (B) INPUT = 1.00



Structural component only
DWG# T-2007673

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	T54S	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR6TstFoe31v6l zns1l-T7ohgkOqpA3ZkEypYxgXoZB2uPsf_43dQW6ChpzME7V



TOTAL WEIGHT = 2 X 49 = 98 lb (M)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
K - A	2x4	DRY	No.2	SPF		
A - E	2x4	DRY	No.2	SPF		
F - E	2x4	DRY	No.2	SPF		
K - I	2x6	DRY	No.2	SPF		
H - C	2x4	DRY	No.2	SPF		
H - F	2x6	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						
I - G	2x6	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'X3") SPIRAL NAILS		
K-A	12	TOP
A-E	12	TOP
E-F	12	TOP
BOTTOM CHORDS : (0.122'X3") SPIRAL NAILS		
K-I	12	SIDE (61.0)
H-F	12	SIDE (183.1)
C-H	12	SIDE (89.8)
WEBS : (0.122'X3") SPIRAL NAILS		
2x3	6	
2x6	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	UPLIFT
K	2804	0	0	5-8
F	2255	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 4'-0".

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL
K	1841 1212 0 0 0 0 628 0 0 0
F	1592 1081 0 0 0 0 531 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LO1 MAX (PLF)	CS1 (LC)	UNBRAC LENGTH FR-TO (LBS)
FR-TO				
K-A	-2274	0	0.0	0.13 (1)
A-B	-8594	0	-91.8	0.22 (1)
B-C	-10298	0	-91.8	0.88 (1)
C-D	-9247	0	-91.8	0.81 (1)
D-E	-3416	0	-91.8	0.16 (1)
F-E	-2242	0	0.0	0.13 (1)
K-L	0	0	-18.5	-18.5 0.21 (1)
L-M	0	0	-18.5	-18.5 0.21 (1)
M-J	0	0	-18.5	-18.5 0.21 (1)
J-N	0	6894	-18.5	-18.5 0.79 (1)
N-O	0	6894	-18.5	-18.5 0.79 (1)
O-I	0	6894	-18.5	-18.5 0.79 (1)
H-I	0	354	0.0	0.0 0.65 (1)
I-C	0	128	0.0	0.0 0.63 (1)
H-P	0	832	-18.5	-18.5 0.20 (1)
P-G	0	832	-18.5	-18.5 0.20 (1)
G-F	0	0	-18.5	-18.5 0.01 (4)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
G	7-11-4	-881	-881	---	BACK	VERT	TOTAL	C1
I	4-9-12	-577	-577	---	FRONT	VERT	TOTAL	C1
L	11-4	-72	-72	---	BACK	VERT	TOTAL	C1
M	2-0-12	-352	-352	---	FRONT	VERT	TOTAL	C1
N	2-11-4	-142	-142	---	BACK	VERT	TOTAL	C1
O	3-11-4	-983	-983	---	BACK	VERT	TOTAL	C1
P	4-0-12	-354	-354	---	FRONT	VERT	TOTAL	C1
O	5-11-4	-556	-556	---	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/G

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.35')
CALCULATED VERT. DEFL.(LL) = L/882 (0.15')
ALLOWABLE DEFL.(TL) = L/360 (0.35')
CALCULATED VERT. DEFL.(TL) = L/469 (0.27')

CS1: TC=0.66/1.00 (B-C:1), BC=0.79/1.00 (I-J:1), WE=0.87/1.00 (A-J:1), SS=0.45/1.00 (C-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (E) (INPUT = 0.99)
JSI METAL = 0.73 (D) (INPUT = 1.00)



Structural component only
DWG# T-2007675 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	T54S	1	2	GREEN PARK HOMES	

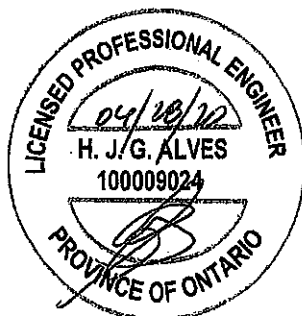
Famarack Roof Truss, Burlington

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PLATES (table in inches)

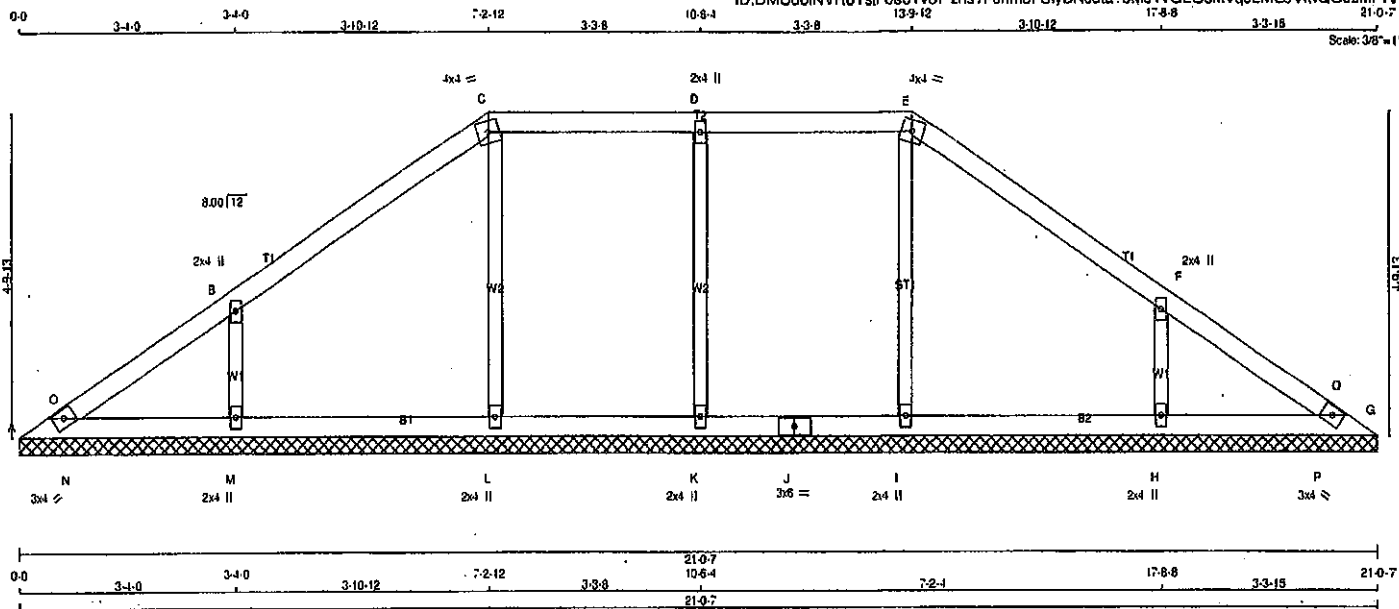
JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-I	MT20	5.0	8.0	2.50	3.00
B	TMWW-I	MT20	5.0	8.0		
C	TMV+p	MT20	3.0	4.0		
D	TMWW-I	MT20	5.0	6.0	2.25	2.00
E	TMWW-I	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	3.0	8.0		
G	BMWWW-I	MT20	6.0	9.0	2.75	4.50
H	BMV+p	MT20	3.0	10.0		
I	BVMWWW-I	MT20	7.0	12.0	4.50	3.75
J	BMWW-I	MT20	5.0	8.0	2.50	3.00
K	BMV1+p	MT20	3.0	6.0	3.50	1.50



Structural component only
DWG# T-2007675

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ORWG NO.
408222	V1	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 ID:DMCubINVR6TsF0e31v6l zns1l-0hm5FSlyDNJdla79MsYvGLCsMVq3LML6VkvQG0zMFIV 21-0-7



TOTAL WEIGHT = 83 lb (M)

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
A - J	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (feet/inches)			
JT	TYPE	PLATES	W LEN Y X
A	TBM1-h	MT20	3.0 4.0
B, D, F			
B	TMW+w	MT20	2.0 4.0
C	TTW-m	MT20	4.0 4.0
E	TTW-m	MT20	4.0 4.0
G	TBM1-h	MT20	3.0 4.0
H, I, K, L, M			
H	BMV1+w	MT20	2.0 4.0
J	BS-1	MT20	3.0 6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS	REACTION	DOWN	HORIZ	BRG	IN-SX	BRG	IN-SX
A	128	0	128	0	0	21-0-7 (11-11)220-7			
G	128	0	128	0	0	21-0-7 (11-11)220-7			
I	330	0	330	0	0	21-0-7 (11-11)220-7			
L	330	0	330	0	0	21-0-7 (11-11)220-7			
K	433	0	433	0	0	21-0-7 (11-11)220-7			
H	485	0	485	0	0	21-0-7 (11-11)220-7			
M	485	0	485	0	0	21-0-7 (11-11)220-7			

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	MAX. MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	90	61 0	0 0	0 0	0 0	29 0	0 0	0 0
G	90	61 0	0 0	0 0	0 0	29 0	0 0	0 0
I	235	143 0	0 0	0 0	0 0	93 0	0 0	0 0
L	235	143 0	0 0	0 0	0 0	93 0	0 0	0 0
K	304	212 0	0 0	0 0	0 0	92 0	0 0	0 0
H	342	228 0	0 0	0 0	0 0	114 0	0 0	0 0
M	342	228 0	0 0	0 0	0 0	114 0	0 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G, I, L, K, H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLINE SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS) MAX. (PLF) CSI (LC)	MAX. FACTORED FORCE (LBS) MAX. (PLF) CSI (LC)
FR-TO		FROM TO	FR-TO
A-O	-23 2	-91.8 -91.8 0.04 (1)	6.25 I-E -266 0 0.09 (1)
O-B	0 33	-91.8 -91.8 0.21 (1)	10.00 L-C -266 0 0.09 (1)
B-C	-8 8	-91.8 -91.8 0.20 (1)	10.00 K-D -377 0 0.13 (1)
C-D	0 17	-91.8 -91.8 0.17 (1)	10.00 H-F -393 0 0.06 (1)
D-E	0 17	-91.8 -91.8 0.17 (1)	10.00 M-B -393 0 0.06 (1)
E-F	-8 8	-91.8 -91.8 0.20 (1)	10.00 N-O -39 5 0.00 (1)
F-O	0 33	-91.8 -91.8 0.21 (1)	10.00 P-Q -39 5 0.00 (1)
O-G	-23 2	-91.8 -91.8 0.04 (1)	6.25
A-N	-9 0	-18.5 -18.5 0.05 (1)	10.00
N-M	-7 7	-18.5 -18.5 0.06 (1)	10.00
M-L	-11 0	-18.5 -18.5 0.06 (1)	6.25
L-K	-17 0	-18.5 -18.5 0.05 (1)	6.25
K-J	-17 0	-18.5 -18.5 0.05 (1)	6.25
J-I	-17 0	-18.5 -18.5 0.05 (1)	6.25
I-H	-11 0	-18.5 -18.5 0.06 (1)	6.25
H-P	-7 7	-18.5 -18.5 0.05 (1)	10.00
P-G	-9 0	-18.5 -18.5 0.05 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
 DL = 8.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBCC 2010, NBCC 2015
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

ISS 1/2 OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21 1.00 (B-O:1) , BC=0.06 1.00 (L-M:4) , WB=0.13 1.00 (D-K:1) , SSI=0.15 1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

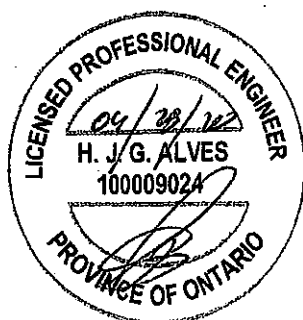
NAIL VALUES

PLATE (GRIP/DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 818 354 1067 788 1987 1656

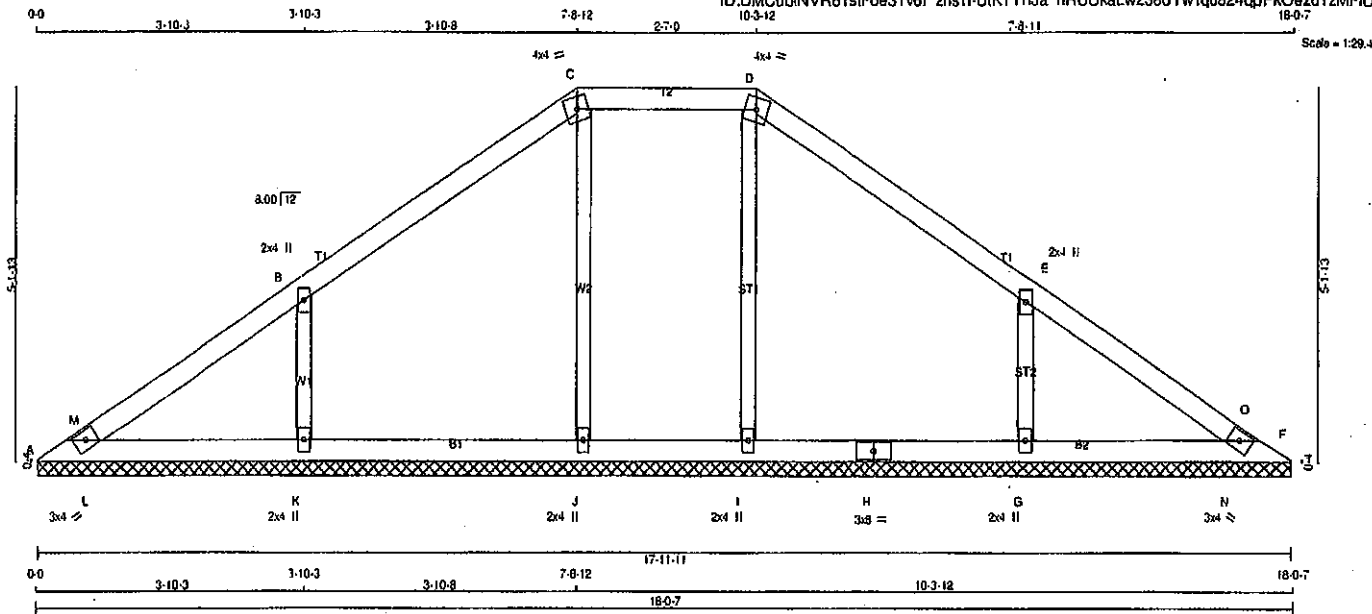
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (C) INPUT = 0.90
 JSI METAL = 0.20 (F) INPUT = 1.00



Structural component only
 DWG# T-2007616



TOTAL WEIGHT = 54 lb

LUMBER
 N. L. G. A. RULES

CHORDS	SIZE	DRY	No.2	DESCR.	SPF
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
A - H	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TBM1-h	MT20	3.0	4.0		
B TMW1-w	MT20	2.0	4.0		
C TTW-m	MT20	4.0	4.0		
D TTW-m	MT20	4.0	4.0		
E TMW1-w	MT20	2.0	4.0		
F TBM1-h	MT20	3.0	4.0		
G, I, J, K					
G BMW1-w	MT20	2.0	4.0		
H BS-1	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
A	136	0	136	0	17-11-11	11-17-12-11
F	135	0	135	0	17-11-11	11-17-12-11
I	344	0	344	0	17-11-11	11-17-12-11
G	513	0	513	0	17-11-11	11-17-12-11
J	342	0	342	0	17-11-11	11-17-12-11
K	513	0	513	0	17-11-11	11-17-12-11

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	95	85	0	0	0	0	0
F	95	85	0	0	0	0	0
I	244	155	0	0	0	0	0
G	362	240	0	0	0	0	0
J	242	154	0	0	0	0	0
K	362	240	0	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS) A, F, I, G, J, K

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. UNBRAC LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED LENGTH (LBS)	
FR-TO		FROM TO	CSI (LC)		FR-TO				
A-M	0 20	-91.8	-91.8	0.04 (1)	10.00	I-D	-292	0	0.11 (1)
M-B	0 71	-91.8	-91.8	0.22 (1)	10.00	G-E	-408	0	0.07 (1)
B-C	0 33	-91.8	-91.8	0.22 (1)	10.00	J-C	-291	0	0.11 (1)
C-D	0 52	-91.8	-91.8	0.11 (1)	10.00	K-B	-408	0	0.07 (1)
D-E	0 33	-91.8	-91.8	0.22 (1)	10.00	L-M	-71	4	0.00 (1)
E-O	0 71	-91.8	-91.8	0.22 (1)	10.00	N-O	-69	5	0.00 (1)
O-F	0 19	-91.8	-91.8	0.04 (1)	10.00				
A-L	-43 0	-18.5	-18.5	0.07 (1)	6.25				
L-K	-30 0	-18.5	-18.5	0.07 (1)	6.25				
K-J	-45 0	-18.5	-18.5	0.07 (4)	6.25				
J-I	-52 0	-18.5	-18.5	0.04 (4)	6.25				
I-H	-45 0	-18.5	-18.5	0.07 (4)	6.25				
H-G	-45 0	-18.5	-18.5	0.07 (4)	6.25				
G-N	-30 0	-18.5	-18.5	0.07 (1)	6.25				
N-F	-43 0	-18.5	-18.5	0.07 (1)	6.25				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.8 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, OBC 2012, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-03, CSA 086-14
 - TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22(1.00 (E-O:1), 8C=0.07(1.00 (K-L:1), WB=0.11 1.00 (D-I:1), SSI=0.14(1.00 (D-E:1)

DLX LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

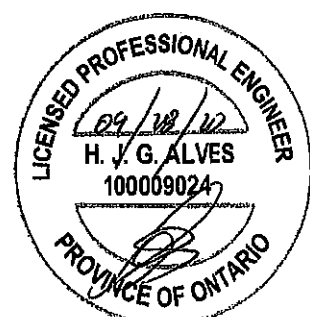
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (C) INPUT = 0.90
 JSI METAL= 0.21 (E) INPUT = 1.00

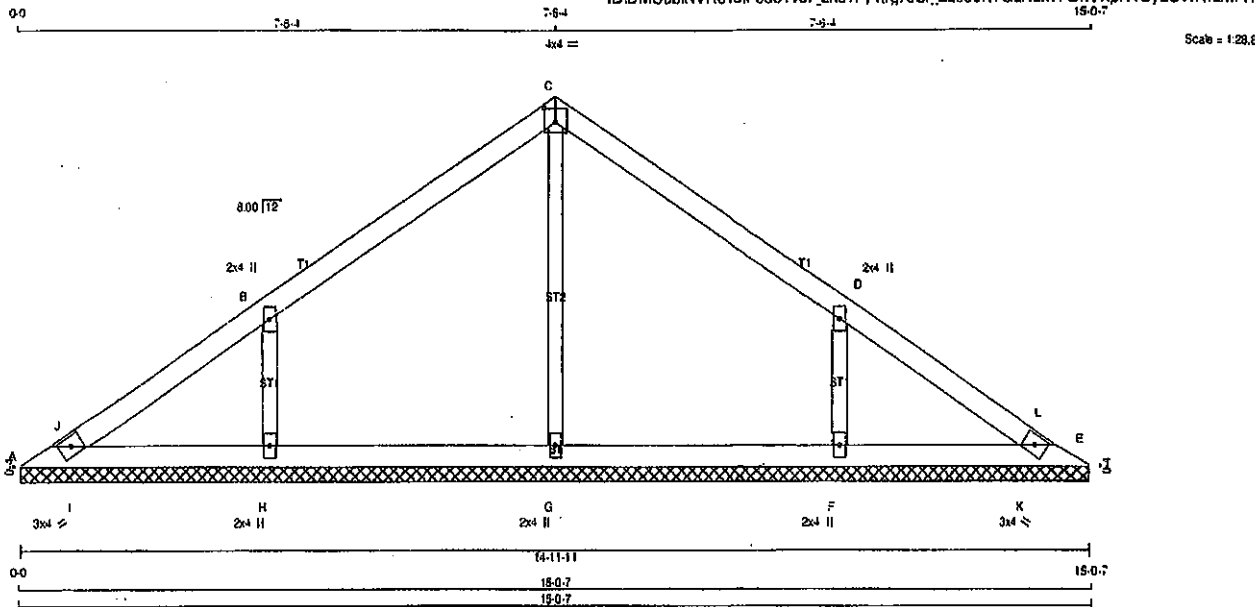


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408222	V3	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR8TslFoe31v6l_zns1l-y4trg7JCl_ZL6u9XTGaNLmTCiIVXoH4Oy2OWKvzMfIT



TOTAL WEIGHT = 43 lb (M)

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	No.2
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
A - E	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER.			

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
A	134	0	134	0	14-11-11	14-11-11
E	134	0	134	0	14-11-11	14-11-11
G	381	0	381	0	14-11-11	14-11-11
H	501	0	501	0	14-11-11	14-11-11
F	501	0	501	0	14-11-11	14-11-11

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX. MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	95	65	0	0	0	39	0
E	95	65	0	0	0	39	0
G	272	162	0	0	0	110	0
H	353	237	0	0	0	116	0
F	353	237	0	0	0	116	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1	MAX. FACTORED (PLF)	CS1 (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1	MAX. FACTORED (PLF)	CS1 (LC1)
FR-TO				FROM TO		FR-TO				FROM TO	
A-J	-20	4		-91.8	-91.8	0.04 (1)	6.25	G-C	-316	0	0.11 (1)
J-B	0	36		-91.8	-91.8	0.22 (1)	10.00	H-B	-406	0	0.08 (1)
B-C	-5	12		-91.8	-91.8	0.21 (1)	10.00	F-D	-406	0	0.08 (1)
C-D	-5	12		-91.8	-91.8	0.21 (1)	10.00	I-J	-46	6	0.00 (1)
D-L	0	36		-91.8	-91.8	0.22 (1)	10.00	K-L	-46	6	0.00 (1)
L-E	-20	4		-91.8	-91.8	0.04 (1)	6.25				
A-I	-13	0		-18.5	-18.5	0.08 (1)	6.25				
I-H	-10	3		-18.5	-18.5	0.08 (4)	6.25				
H-G	-13	0		-18.5	-18.5	0.08 (4)	6.25				
G-F	-13	0		-18.5	-18.5	0.08 (4)	6.25				
F-K	-10	3		-18.5	-18.5	0.08 (4)	6.25				
K-E	-13	0		-18.5	-18.5	0.08 (1)	6.25				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22(1.00 (B-J)), BC=0.06(1.00 (G-H)), WB=0.11(1.00 (C-G)), SS1=0.15(1.00 (B-C))

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

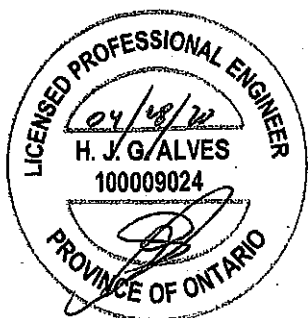
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE (GRIP/DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1856

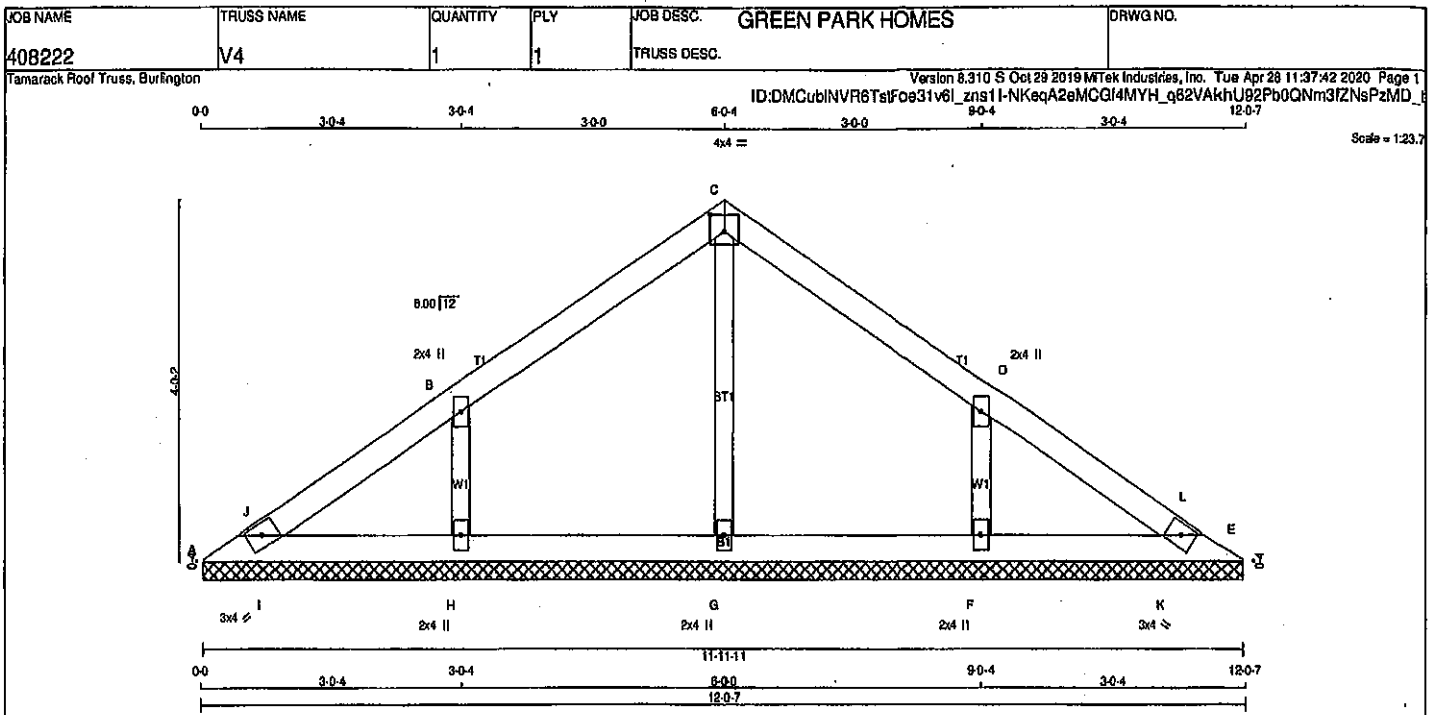
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRP = 0.28 (D) (INPUT = 0.80)
JSI METAL = 0.21 (D) (INPUT = 1.00)



Structural component only
DWG# T-2007618



LUMBER
N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
A - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TBM1-h	MT20	3.0	4.0		
B TMW+w	MT20	2.0	4.0		
C TTW-p	MT20	4.0	4.0	2.25	2.00
D TMW+w	MT20	2.0	4.0		
E TBM1-h	MT20	3.0	4.0		
F, G, H					
F BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
A	117	0	117	0	0	11-11-11	11-11-11
E	117	0	117	0	0	11-11-11	11-11-11
G	280	0	280	0	0	11-11-11	11-11-11
F	398	0	398	0	0	11-11-11	11-11-11
H	398	0	398	0	0	11-11-11	11-11-11

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	83	58/0	0/0	0/0	0/0	26/0	0/0
E	83	58/0	0/0	0/0	0/0	26/0	0/0
G	207	125/0	0/0	0/0	0/0	82/0	0/0
F	281	188/0	0/0	0/0	0/0	93/0	0/0
H	281	188/0	0/0	0/0	0/0	93/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-J	-8/5	-91.8 -91.8 0.03 (1)	10.00	G-C	-248/0	0.88 (1)	
J-B	0/36	-91.8 -91.8 0.13 (1)	10.00	F-D	-314/0	0.05 (1)	
B-C	0/12	-91.8 -91.8 0.13 (1)	10.00	H-B	-314/0	0.05 (1)	
C-D	0/12	-91.8 -91.8 0.13 (1)	10.00	I-J	-47/4	0.00 (1)	
D-L	0/36	-91.8 -91.8 0.13 (1)	10.00	K-L	-47/4	0.00 (1)	
L-E	-8/5	-91.8 -91.8 0.03 (1)	10.00				
A-I	-13/0	-18.5 -18.5 0.05 (1)	6.25				
I-H	-10/0	-18.5 -18.5 0.05 (1)	10.00				
H-G	-20/0	-18.5 -18.5 0.04 (4)	6.25				
G-F	-20/0	-18.5 -18.5 0.04 (4)	6.25				
F-K	-10/0	-18.5 -18.5 0.05 (1)	10.00				
K-E	-13/0	-18.5 -18.5 0.05 (1)	6.25				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

OSI: TC=0.13/1.00 (B-J:1), BC=0.05/1.00 (A-I:1), WB=0.08/1.00 (G-G:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

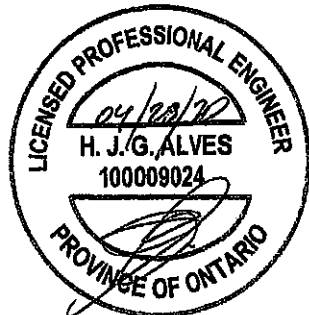
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1687 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

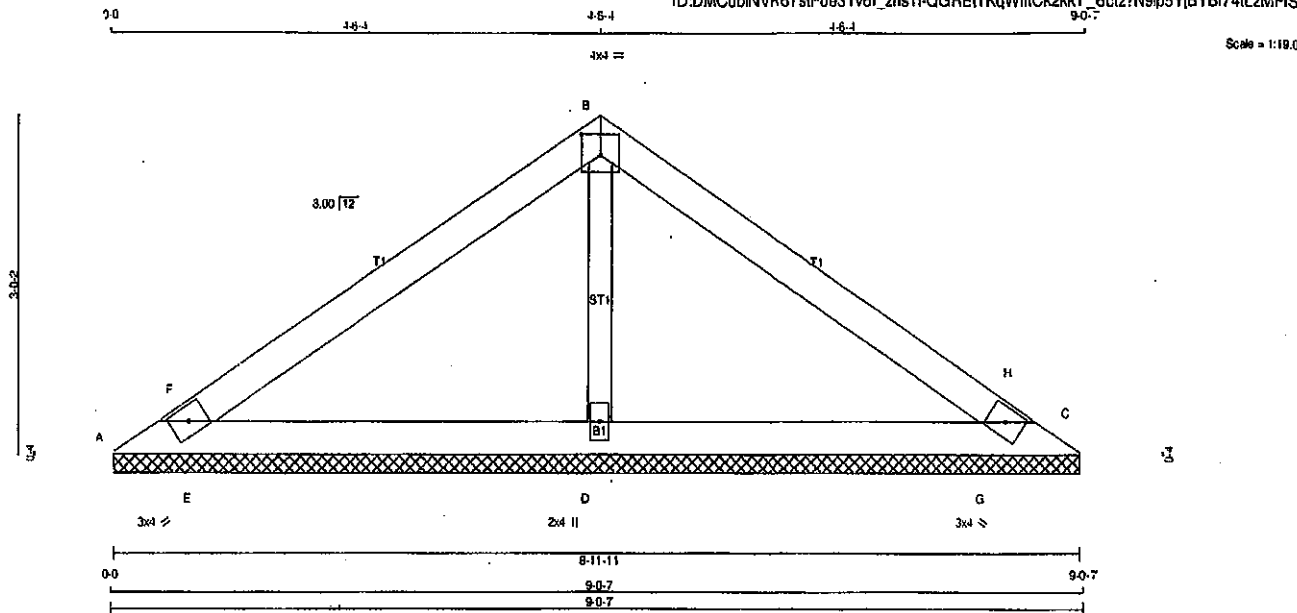
JSI GRIP=0.22 (D) (INPUT = 0.80)
JSI METAL=0.16 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007619

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	V5	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 23 k

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (tablets in inches)	W	LEN	Y	X
JT TYPE PLATES				
A - TBM1-h	MT20	3.0	4.0	
B - TTW-p	MT20	4.0	4.0	2.25 2.00
C - TBM1-h	MT20	3.0	4.0	
D - BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	HORZ	DOWN	UPLIFT
			IN-SX	IN-SX
A	49	0	49	0
C	49	0	49	0
D	893	0	893	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	34	22.0	0.0	0.0	0.0	0.0	12.0	0.0
C	34	22.0	0.0	0.0	0.0	0.0	12.0	0.0
D	631	415.0	0.0	0.0	0.0	0.0	216.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX CS (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS (LC)
FR-TO		FROM TO			FR-TO		
A-F	0 347	-91.8 -91.8	0.12 (1)	10.00	D-B	-698 0	0.12 (1)
F-B	0 341	-91.8 -91.8	0.23 (1)	10.00	E-F	-225 0	0.00 (1)
B-H	0 341	-91.8 -91.8	0.23 (1)	10.00	G-H	-225 0	0.00 (1)
H-C	0 347	-91.8 -91.8	0.12 (1)	10.00			
A-E	-321 0	-18.5 -18.5	0.16 (1)	6.25			
E-D	-290 0	-18.5 -18.5	0.17 (1)	6.25			
D-G	-290 0	-18.5 -18.5	0.17 (1)	6.25			
G-C	-321 0	-18.5 -18.5	0.16 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (B-F:1), BC=0.17/1.00 (D-E:1), WB=0.12/1.00 (B-D:1), SSI=0.12/1.00 (A-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

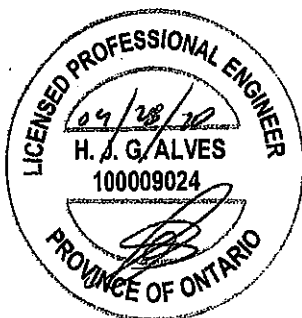
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (B), INPUT = 0.90
JSI METAL= 0.20 (B), INPUT = 1.00



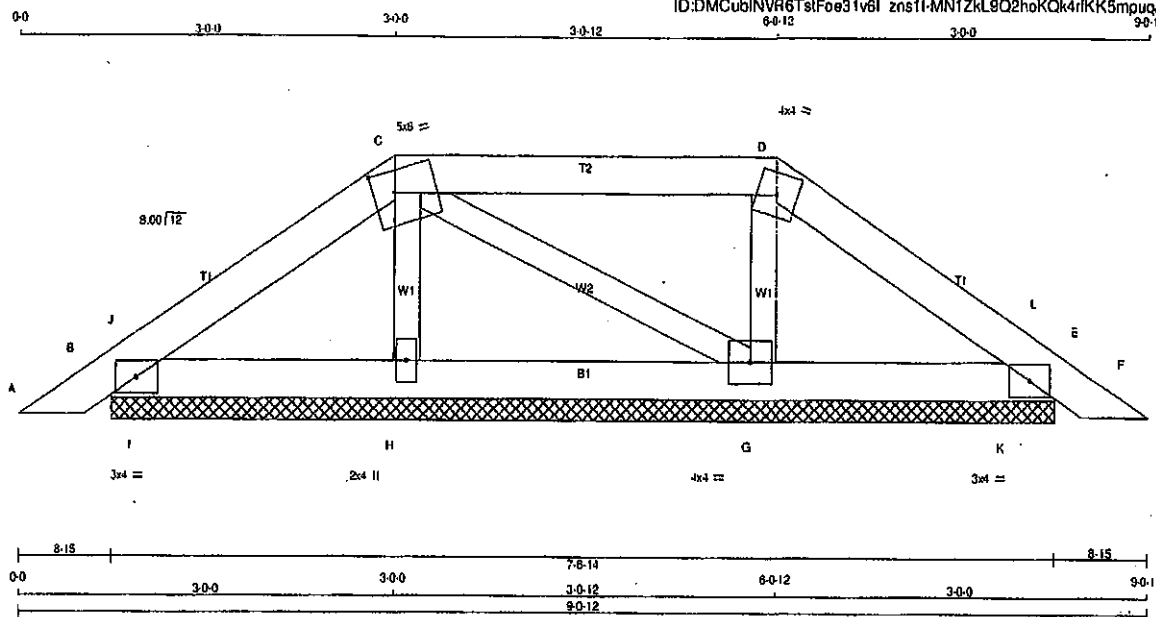
Structural component only
DWG# T-2007620

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	PB1	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR6TslFoe31v6I zns1I-MN1ZkL8Q2hoKQk4rlKK5mpuqJfFXyxLks7nrjzMFh

Scale = 1:16.4



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	3.0	4.0
C	TTWW-m	MT20	5.0	6.0 2.00 2.00
D	TTWW-m	MT20	4.0	4.0
E	TMB1-I	MT20	3.0	4.0
G	BMWW1-I	MT20	4.0	4.0
H	BMWW1-w	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	UPLIFT	IN-SX
B	221	0	221	0	7-6-14
E	209	0	209	0	7-6-14
H	248	0	248	0	7-6-14
G	280	0	280	0	7-6-14

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	154	113	0	0	0	41	0
E	148	106	0	0	0	39	0
H	178	109	0	0	0	67	0
G	198	127	0	0	0	71	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 15	-91.8	-91.8 0.03 (1)	10.00	H-C	-176 0	0.03 (1)
B-J	-63 0	-91.8	-91.8 0.01 (1)	8.25	C-G	-21 0	0.03 (1)
J-C	-75 0	-91.8	-91.8 0.05 (1)	8.25	G-D	-198 0	0.03 (1)
C-D	-30 0	-91.8	-91.8 0.15 (1)	8.25	I-J	-121 0	0.03 (1)
D-L	-54 0	-91.8	-91.8 0.05 (1)	8.25	K-L	-123 0	0.03 (1)
L-E	-41 0	-91.8	-91.8 0.01 (1)	8.25			
E-F	0 15	-91.8	-91.8 0.03 (1)	10.00			
B-I	0 80	-18.5	-18.5 0.08 (1)	10.00			
I-H	0 60	-18.5	-18.5 0.06 (1)	10.00			
H-G	0 48	-18.5	-18.5 0.03 (1)	10.00			
G-K	0 43	-18.5	-18.5 0.08 (1)	10.00			
K-E	0 43	-18.5	-18.5 0.06 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
OL = 6.0 PSF
BOT CH. LL = 0.0 PSF
OL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.15/1.00 (C-D:1), BC=0.08/1.00 (B-I:1), WB=0.03/1.00 (D-G:1), SS1=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (D)RY SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) INPUT = 0.90
JSI METAL= 0.04 (B) INPUT = 1.00



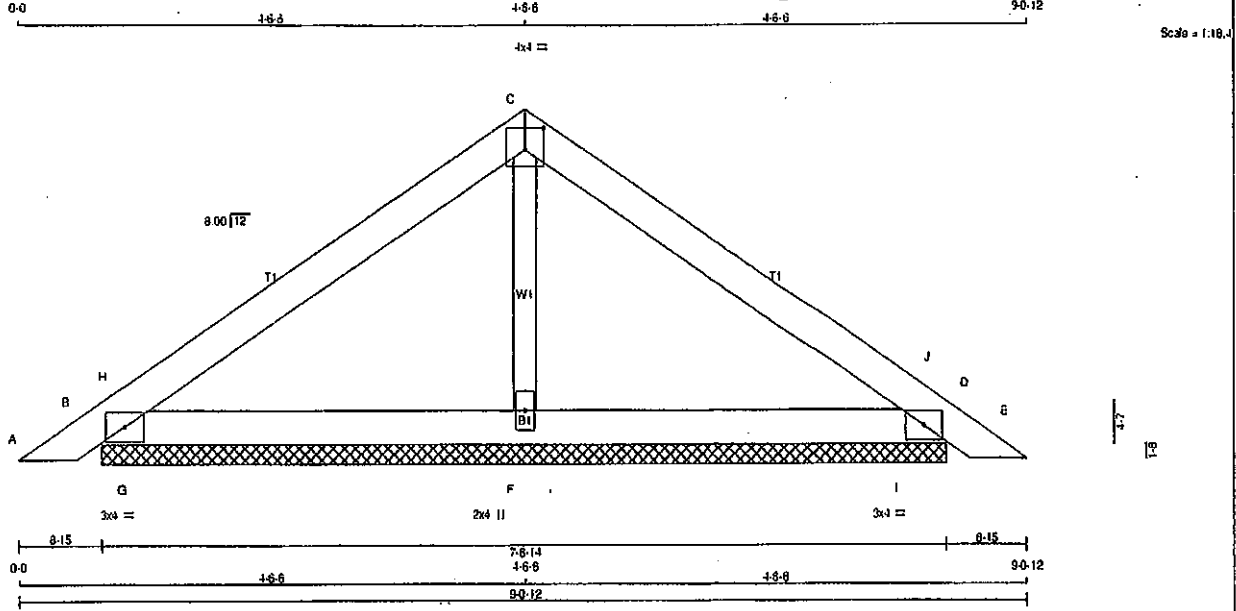
Structural component only
DWG# T-2007600

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408222	PB2	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6l_zris1l-qabxth92p?wB2uf2D2sKJ0Q?w31_GPEUzWkKN9zMF1q



TOTAL WEIGHT = 2 X 23 = 46 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - E	2x4 DRY	No.2	SPF		
B - D	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE	PLATES			
B	TM81-1	MT20	3.0	4.0
C	TTW-p	MT20	4.0	4.0
D	TM81-1	MT20	3.0	4.0
F	BMW1-w	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
B	318	0	318	0	0	7-8-14	7-8-14	7-8-14	7-8-14
D	318	0	318	0	0	7-8-14	7-8-14	7-8-14	7-8-14
F	320	0	320	0	0	7-8-14	7-8-14	7-8-14	7-8-14

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	PERM. LIVE	WIND	DEAD	SOIL
B	223	159	0	0	0	0	0	63	0	0	0
D	223	159	0	0	0	0	0	63	0	0	0
F	229	137	0	0	0	0	0	92	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE (LBS)	MAX	MAX	MEMB.	FORCE (LBS)	MAX	MAX
FR-TO		FROM	TO	CS1 (LC)	UNBRAC	LENGTH	FR-TO			FR-TO			
A-B	0 15	-91.8	-91.8	0.03 (1)	10.00	F-C	-148	0	0.03 (1)				
B-H	-31	0	-91.8	-91.8	0.07 (1)	8.25	G-H	-349	0	0.00 (1)			
H-C	-135	0	-91.8	-91.8	0.16 (1)	8.25	I-J	-349	0	0.00 (1)			
C-J	-135	0	-91.8	-91.8	0.16 (1)	8.25							
J-D	-31	0	-91.8	-91.8	0.07 (1)	8.25							
D-E	0 15	-91.8	-91.8	0.03 (1)	10.00								
B-G	0 105	-18.5	-18.5	0.15 (1)	10.00								
G-F	0 105	-18.5	-18.5	0.15 (1)	10.00								
F-I	0 105	-18.5	-18.5	0.15 (1)	10.00								
I-D	0 105	-18.5	-18.5	0.15 (1)	10.00								

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.16 (C-J), BC=0.15 (F-I), WB=0.03 (C-F), SSI=0.26 (I-D)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

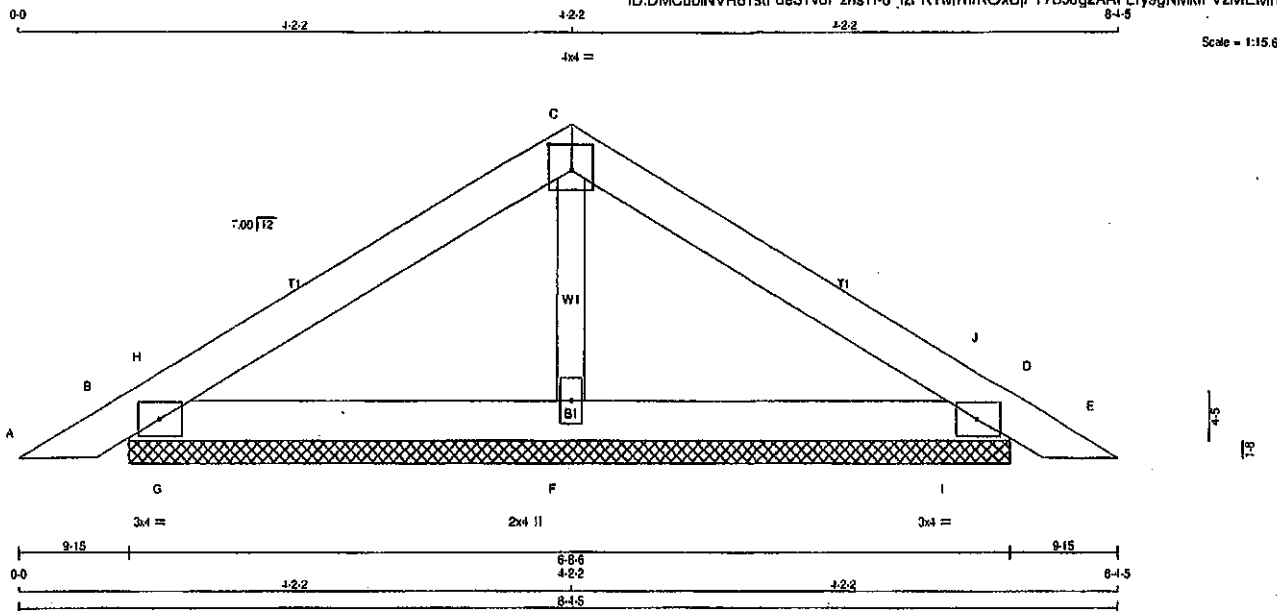
JSI GRIP = 0.28 (D) INPUT = 0.90
JSI METAL = 0.07 (D) INPUT = 1.00



Structural component only
DWG# T-2007601

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	PB20	4	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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ID:DMCubINVR6TstFas31v6I zns11-6 jzPKYM7hrKOxBjFY7B9Jg2AAFEry9gNMkIPVzMEMr



TOTAL WEIGHT = 4 X 20 = 80 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-p	MT20	4.0	3.0	2.25 2.00
D	TMB1-I	MT20	3.0	4.0	
F	BMW1-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS									
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG	VERT	HORZ	DOWN	HORZ	UPLIFT
B	281	0	281	0	0	0	0	0	0
D	281	0	281	0	0	0	0	0	0
F	314	0	314	0	0	0	0	0	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	197	141	0	0	0	0	55	0
D	197	141	0	0	0	0	55	0
F	224	137	0	0	0	0	87	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: 14)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 18	-91.8 -91.8	0.04 (1)	10.00	F-C	-159 0	0.02 (1)
B-H	-44 0	-91.8 -91.8	0.05 (1)	6.25	G-H	-250 0	0.00 (1)
H-C	-99 0	-91.8 -91.8	0.12 (1)	6.25	I-J	-250 0	0.00 (1)
C-J	-99 0	-91.8 -91.8	0.12 (1)	6.25			
J-D	-44 0	-91.8 -91.8	0.05 (1)	6.25			
D-E	0 18	-91.8 -91.8	0.04 (1)	10.00			
B-G	0 81	-18.5 -18.5	0.12 (1)	10.00			
G-F	0 81	-18.5 -18.5	0.12 (1)	10.00			
F-I	0 81	-18.5 -18.5	0.12 (1)	10.00			
I-D	0 81	-18.5 -18.5	0.12 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TO=0.12/1.00 (C-H:1), BC=0.12/1.00 (B-G:1), WB=0.02/1.00 (C-F:1), SSI=0.18/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1650

PLATE PLACEMENT TOL. = 0.250 inches

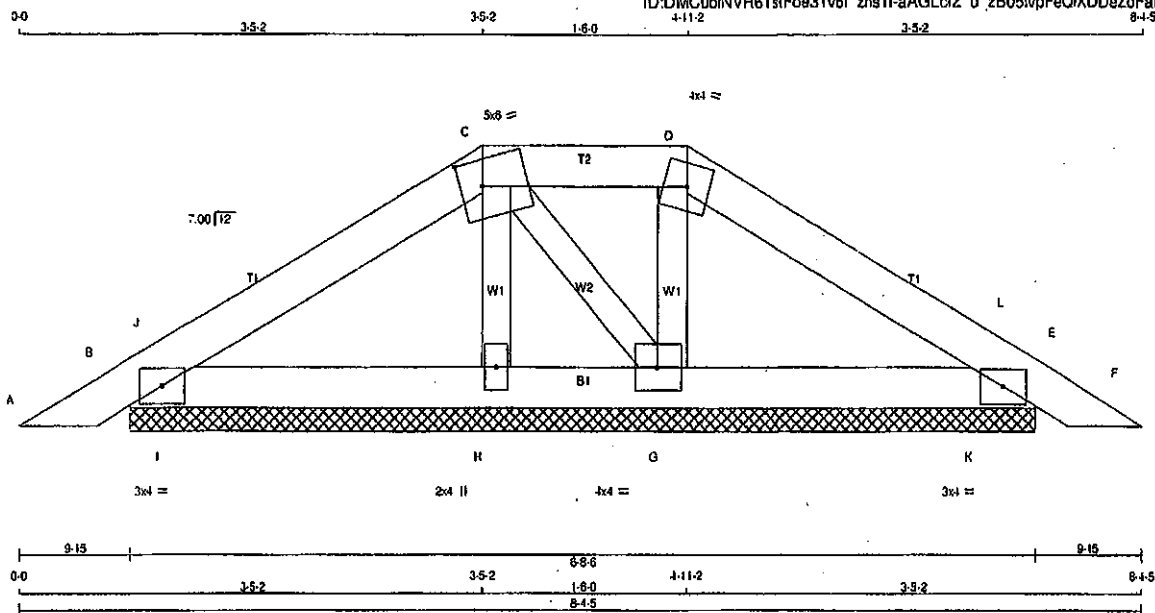
PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP = 0.24 (D) INPUT = 0.90
SSI METAL = 0.06 (B) INPUT = 1.00



Structural component only
DWG# T-2007627

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	PB21	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
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Scale = 1:15.2

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	No.2	SPF		
B - E	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
B	241	0	241	0
E	228	0	228	0
H	170	0	170	0
G	238	0	238	0

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 8.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, OBC 2012 : ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 088-09, CSA 088-14
 - TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07(1.00(D-L:1), 8C=0.07(1.00(B-L:1), WB=0.02(1.00(D-G:1), SSI=0.11(1.00(E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) INPUT = 0.90)
 JSI METAL= 0.05 (D) INPUT = 1.00)

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMB1-I	MT20	3.0	4.0	
C TTWW-m	MT20	5.0	8.0	2.25 2.00
D TTWW-m	MT20	4.0	4.0	
E TMB1-I	MT20	3.0	4.0	
G BMW1-I	MT20	4.0	4.0	
H BMW1-W	MT20	2.0	4.0	

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	SNOW LIVE PERM. LIVE WIND DEAD SOIL
B	169 121 0 0 0 0 48 0 0 0
E	160 114 0 0 0 0 46 0 0 0
H	122 74 0 0 0 0 48 0 0 0
G	168 111 0 0 0 0 57 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	(PLF)	CS1 (LC)	UNBRAC	LENGTH	FR-TO	MEMB.	FORCE (LBS)
A-B	0 16	-91.8	-91.8	0.04 (1)	10.00	H-C	-107 0	0.02 (1)
B-C	-59 0	-91.8	-91.8	0.01 (1)	6.25	C-G	-36 0	0.01 (1)
J-C	-71 0	-91.8	-91.8	0.07 (1)	6.25	G-D	-147 0	0.02 (1)
C-D	-28 0	-91.8	-91.8	0.04 (1)	6.25	I-J	-139 0	0.00 (1)
D-L	-46 0	-91.8	-91.8	0.07 (1)	6.25	K-L	-141 0	0.00 (1)
L-E	-33 0	-91.8	-91.8	0.01 (1)	6.25			
E-F	0 16	-91.8	-91.8	0.04 (1)	10.00			
B-I	0 59	-18.5	-18.5	0.07 (1)	10.00			
I-H	0 59	-18.5	-18.5	0.07 (1)	10.00			
H-G	0 52	-18.5	-18.5	0.04 (1)	10.00			
G-K	0 38	-18.5	-18.5	0.07 (1)	10.00			
K-E	0 38	-18.5	-18.5	0.07 (1)	10.00			



Structural component only
 DWG# T-2007628

JOB NAME 408223	TRUSS NAME PB22	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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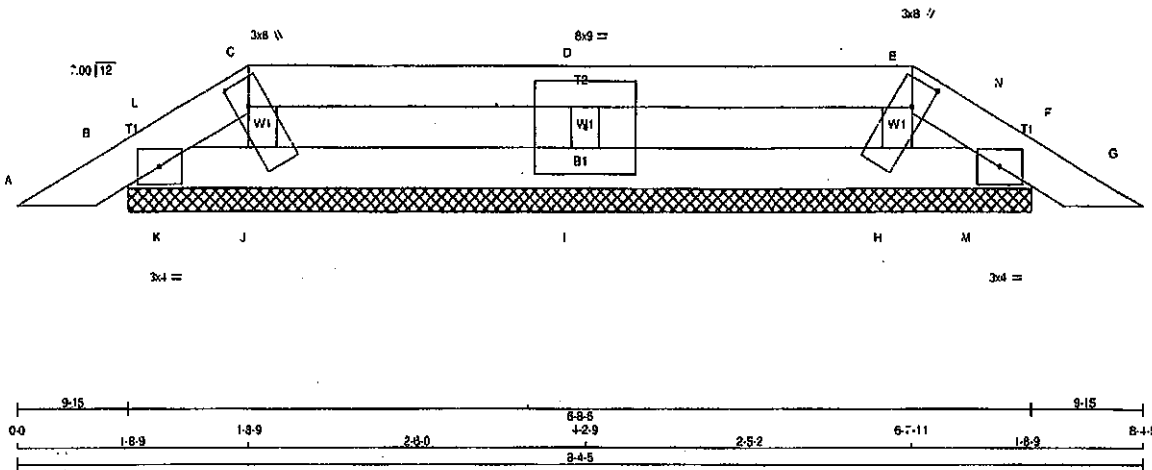
Tamarack Roof Truss, Burlington

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0.0 1-8.9 1-3.9 2-8.0 4-2.9 2-5.2 6-7.11 1-8.9 8-4.5

Scale = 1:15.2



LUMBER	CHORDS	SIZE	DRY	No.2	DESCR.	SPF
A - C	2x4	DRY	No.2	SPF		
C - E	2x4	DRY	No.2	SPF		
E - G	2x4	DRY	No.2	SPF		
B - F	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

PLATES (table in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0			
C	D, E, H, I, J						
C	TTBW1+H	MT20	3.0	8.0	2.25	1.25	
D	TMBMW1-I	MT20	8.0	9.0			
E	TTBW1+H	MT20	3.0	8.0	2.25	1.25	
F	TMB1-I	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
B	151	0	151	0
F	152	0	152	0
J	125	0	125	0
H	120	0	120	0
I	330	0	330	0

UNFACTORED REACTIONS

1ST LOOSE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE		
B	105	83	0	0	21
F	105	83	0	0	22
J	90	49	0	0	41
H	86	47	0	0	39
I	232	158	0	0	74

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, H, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MEMB.	MAX. FACTORED	FACTORED
		FORCE	VERT. LOAD	LC1 MAX			FORCE	LC1 MAX
		(LBS)	(PLF)	CS1 (LC)			(LBS)	CS1 (LC)
FR-TO			FROM	TO	LENGTH	FR-TO		
A-B	0	16	-91.8	-91.8	0.04 (1)	J-C	-95	0
B-L	-53	0	-91.8	-91.8	0.04 (1)	H-E	-92	0
L-C	-22	0	-91.8	-91.8	0.01 (1)	I-D	-282	0
C-D	0	3	-91.8	-91.8	0.09 (1)	K-L	-1	12
D-E	0	3	-91.8	-91.8	0.09 (1)	M-N	0	13
E-N	-21	0	-91.8	-91.8	0.01 (1)			
N-F	-52	0	-91.8	-91.8	0.04 (1)			
F-G	0	16	-91.8	-91.8	0.04 (1)			
B-K	0	14	-18.5	-18.5	0.01 (1)			
K-J	0	14	-18.5	-18.5	0.02 (4)			
J-I	-3	0	-18.5	-18.5	0.02 (4)			
I-H	-3	0	-18.5	-18.5	0.02 (4)			
H-M	0	14	-18.5	-18.5	0.01 (4)			
M-F	0	14	-18.5	-18.5	0.01 (1)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

.55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.09:1.00 (D-E:1), BC=0.02:1.00 (I-J:4),
WB=0.04:1.00 (D-I:1), SS1=0.11:1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

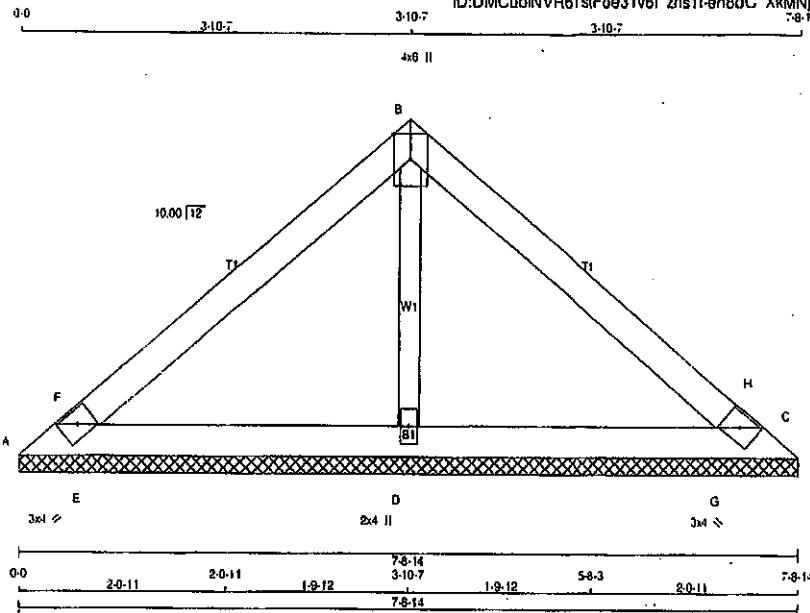
JSI GRIP= 0.13 (C) INPUT = 0.90)
JSI METAL= 0.04 (C) INPUT = 1.00)



Structural component only
DWG# T-2007629

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	P20	17	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR6TsFoe31v6l zns11-en8bC XkMNUnocWqbyd68sgmus6Uhw8i76i2zMEMo



Scale = 1/20.4

TOTAL WEIGHT = 17 X 21 = 363 lb

LUMBER	N. L. G. A. RULES	CHORDS - SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
A - TBM1-h	MT20	3.0	4.0			
B - TTW+p	MT20	4.0	6.0	Edge		
C - TBM1-h	MT20	3.0	4.0			
D - BMW1+ew	MT20	2.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORIZ	DOWN	UPLIFT
A	80	0	80	0
C	80	0	80	0
D	733	0	733	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	43	28.0	0.0	0.0	0.0	0.0	14.0	0.0
C	42	28.0	0.0	0.0	0.0	0.0	14.0	0.0
D	518	339.0	0.0	0.0	0.0	0.0	179.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	WESS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO						FR-TO		
A-F	0 227	-91.8	-91.8	0.09 (1)	10.00	D-B	-558 0	0.10 (1)
F-B	0 219	-91.8	-91.8	0.17 (1)	10.00	E-F	-189 0	0.00 (1)
B-H	0 219	-91.8	-91.8	0.17 (1)	10.00	G-H	-189 0	0.00 (1)
H-C	0 228	-91.8	-91.8	0.09 (1)	10.00			
A-E	-207 0	-18.5	-18.5	0.13 (1)	8.25			
E-D	-174 0	-18.5	-18.5	0.13 (1)	8.25			
D-G	-174 0	-18.5	-18.5	0.13 (1)	8.25			
G-C	-207 0	-18.5	-18.5	0.13 (1)	8.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (B-F:1), BC=0.13 (D-E:1), WB=0.10 (B-D:1), SSI=0.11 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PU)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

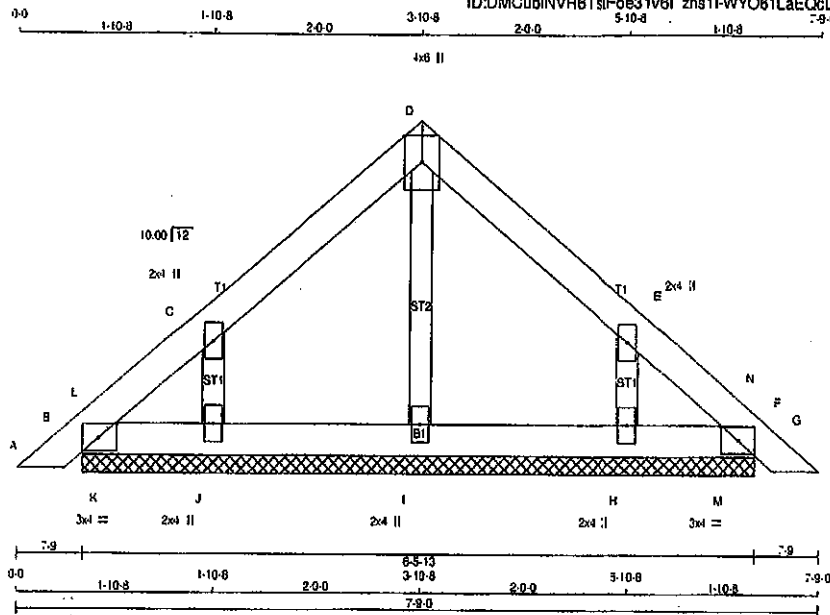
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.29 (D) (INPUT = 0.90)
JSI METAL = 0.12 (D) (INPUT = 1.00)



Structural component only
DWG# T-2007626

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	PG20	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



TOTAL WEIGHT = 2 X 22 = 45 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES	A - D	2x4	DRY	No.2	SPF
	D - G	2x4	DRY	No.2	SPF
	B - F	2x4	DRY	No.2	SPF
	ALL WEBS	2x3	DRY	No.2	SPF
	ALL GABLE WEBS	2x3	DRY	No.2	SPF
	DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
8	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	4.0	8.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H, I, J	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	FACTORED MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO				FR-TO			
A-B	0 14	-91.8	-91.8 0.02 (1)	I-D	-111 0	0.02 (1)	
B-L	-71 0	-91.8	-91.8 0.00 (1)	J-C	-185 0	0.03 (1)	
L-C	-30 0	-91.8	-91.8 0.05 (1)	H-E	-185 0	0.03 (1)	
C-D	-47 0	-91.8	-91.8 0.05 (1)	K-L	0 9	0.00 (1)	
D-E	-47 0	-91.8	-91.8 0.05 (1)	M-N	0 9	0.00 (1)	
E-N	-30 0	-91.8	-91.8 0.05 (1)				
N-F	-71 0	-91.8	-91.8 0.00 (1)				
F-G	0 14	-91.8	-91.8 0.02 (1)				
B-K	0 44	-18.5	-18.5 0.01 (1)				
K-J	0 44	-18.5	-18.5 0.01 (4)				
J-I	0 27	-18.5	-18.5 0.02 (4)				
I-H	0 27	-18.5	-18.5 0.02 (4)				
H-M	0 44	-18.5	-18.5 0.01 (4)				
M-F	0 44	-18.5	-18.5 0.01 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05(1.00 (C-D:1), BC=0.02(1.00 (H-I:4), WB=0.03(1.00 (C-J:1), BS=0.07(1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI)	(PLI)	(PLI)
MAX MIN MAX MIN MAX MIN		
MT20	618	354 1667 768 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)

JSI METAL= 0.10 (C) (INPUT = 1.00)

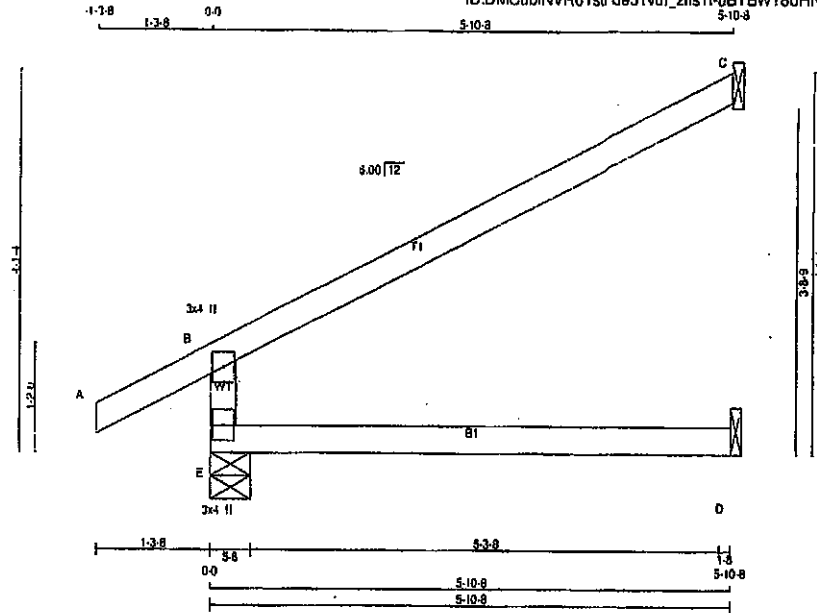


Structural component only
DWG# T-2007630

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408222	U1	21	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TstF0a31v6l_zns1f-u8TBW78oHNqUpbVidspEblZQFMuoV7BWCFEHzMFH



TOTAL WEIGHT = 21 X 17 = 353 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV+p	MT20	3.0	4.0	
E BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG
E	525	0	525	0	0	5-8	5-8	5-8	
C	202	0	202	0	0	1-8	1-8	1-8	
D	45	0	50	0	0	1-8	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	389	257	0	0	0	111	0
C	139	113	0	0	0	26	0
D	38	0	0	0	0	36	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: 14)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO		FROM	TO	FR-TO			
E-B	-161	0	0.0	0.0	0.13 (4)	7.81	
A-B	0	28	-91.8	-91.8	0.12 (1)	10.00	
B-C	-30	0	-91.8	-91.8	0.54 (1)	6.25	
E-D	0	0	-18.5	-18.5	0.13 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF.

65 % OF 31.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CS1: TC=0.54 (1.00 (B-C:1), BC=0.13 (1.00 (D-E:4), WB=0.00 (1.00 (m-a:0), SS1=0.24 (1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSD) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 789 1967 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E), INPUT = 0.90
JSI METAL= 0.13 (B), INPUT = 1.00

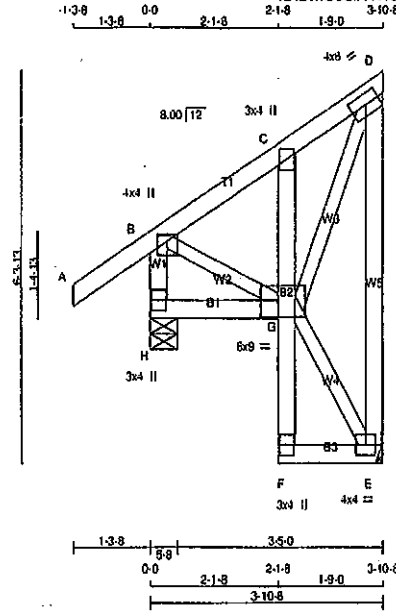


Structural component only
DWG# T-2007598

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	J2S	4	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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ID:DMCubINVR6TslFoe31v6l_zns1l-uBTBW78oHNgUpbVf5dpsEblhFOVoUjBWCFEHzMFli



TOTAL WEIGHT = 4 X 31 = 125 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
H - B	2x4	DRY	No. 2	SPF	
A - D	2x4	DRY	No. 2	SPF	
H - G	2x4	DRY	No. 2	SPF	
F - C	2x4	DRY	No. 2	SPF	
F - E	2x4	DRY	No. 2	SPF	
ALL WEBS	2x3	DRY	No. 2	SPF	
EXCEPT					
E - D	2x4	DRY	No. 2	SPF	

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMVW+p	MT20	3.0	4.0		
D	TMVW-1	MT20	4.0	6.0	2.00	2.75
E	BMVW-1	MT20	4.0	4.0		
F	BMVW+p	MT20	3.0	4.0		
G	BMVW-1	MT20	6.0	9.0	3.25	3.50
H	BMVW+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
H	332	0	332	0
E	208	0	208	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

1ST LOASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED
H	232
E	145

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	MEMB.	FORCE
	(LBS)	LO1		(LBS)
FR-TO		FROM TO	FR-TO	
H-B	-311	0.0	H-G	0
A-B	0	-91.8	E-D	-188
B-C	-96	-91.8	G-E	-7
C-D	-105	-91.8	G-D	0
H-G	0	-18.5		
F-G	0	0.0		
G-C	-202	0.0		
F-E	0	-18.5		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

.55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.14 (1.00 (A-B-5)), SC=0.03 (1.00 (G-H-4)), WB=0.08 (1.00 (D-E-1)), SSI=0.09 (1.00 (A-B-5))

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP/DRY SHEAR SECTION
(PSI) (PL) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP = 0.22 (B) INPUT = 0.90
JSI METAL = 0.07 (C) INPUT = 1.00

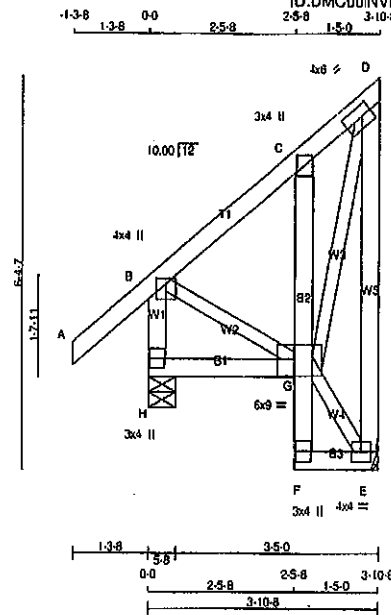


Structural component only
DWG# T-2007599

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408223	J20S	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:34.8

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
H - B	2x4	DRY	No.2	SPF	
H - D	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
F - C	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (Tables in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMV+p	MT20	3.0	4.0		
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
E	BMVW+p	MT20	4.0	4.0		
F	BMV+p	MT20	3.0	4.0		
G	BMVW+p	MT20	6.0	9.0	3.25	3.50
H	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
H	333	0	333	0	5-8	5-8
E	208	0	208	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	LIVE	PERM. LIVE			
H	233	168	0	0	0	67	0
E	145	95	0	0	0	50	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FORCE CS1 (LC)	
FR-TO		FROM TO		FR-TO			
H-B	-309	0	0.0	0.0	0.03	(1)	7.81
A-B	0	41	-91.8	-91.8	0.14	(5)	10.00
B-C	-70	0	-91.8	-91.8	0.07	(1)	6.25
C-D	-92	0	-91.8	-91.8	0.06	(1)	6.25
H-G	0	0	-18.5	-18.5	0.04	(4)	10.00
F-G	0	12	0.0	0.0	0.02	(1)	10.00
G-C	-216	0	0.0	0.0	0.02	(1)	7.81
F-E	0	8	-18.5	-18.5	0.01	(4)	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 33 = 65 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (A-B-5), BC=0.04/1.00 (G-H-4),
WB=0.08/1.00 (D-E-1), SS1=0.09/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	788	1997	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

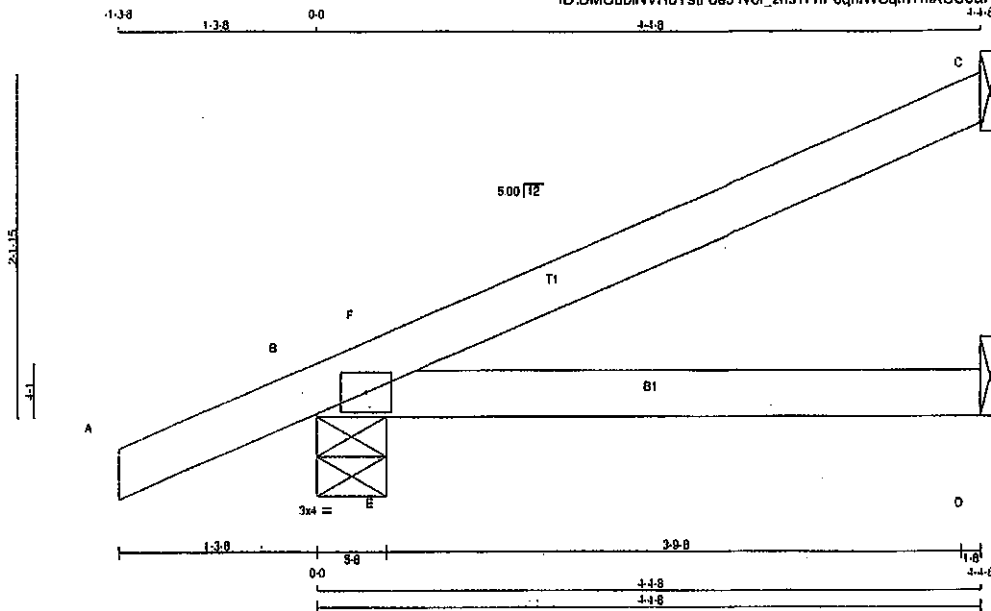
JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.08 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007623

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	J21	4	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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Scale = 1:12.4

TOTAL WEIGHT = 4 X 12 = 49 lb

LUMBER
N. L. G. A. RULES
CHORDS SIZE
A - C 2x4 DRY No.2
B - D 2x4 DRY No.2
LUMBER
DESCR. SPF
SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-1	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
C	175	0	175	0
B	365	0	365	0
D	67	0	67	0

SEE MTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	121	94	0	0	0	27	0
B	256	181	0	0	0	75	0
D	50	18	0	0	0	32	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 23	-91.8 -91.8	0.12 (1)	E-F	235 7	0.00 (1)	
B-F	-17 13	-91.8 -91.8	0.04 (4)				
F-C	-1 2	-91.8 -91.8	0.23 (1)				
B-E	0 0	-18.5 -18.5	0.16 (1)				
E-D	0 0	-18.5 -18.5	0.16 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.05")

DSK TO = 0.23/1.00 (C-F:1), BC = 0.16/1.00 (D-E:1), WB = 0.00/1.00 (E-F:1), SS = 0.19/1.00 (B-E:1)

DOL LUMBER = 1.00 NAIL = 1.00 LS BEND = 1.10
COMP = 1.10 SHEAR = 1.10 TENS = 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.26 (B) (INPUT = 0.80)
JSI METAL = 0.07 (B) (INPUT = 1.00)



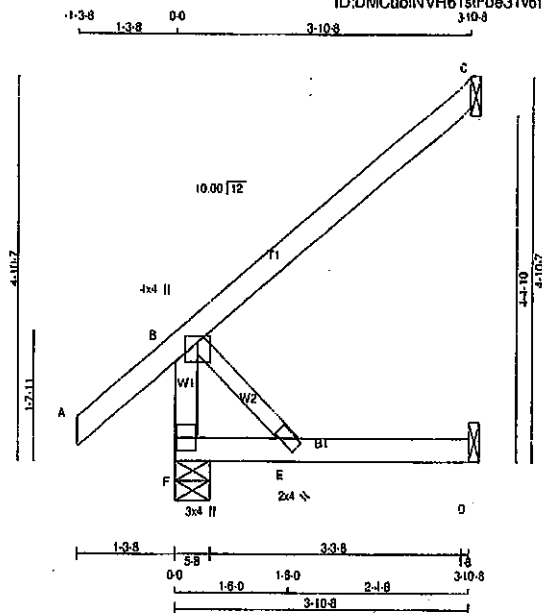
Structural component only
DWG# T-2007624

JOB NAME 408223	TRUSS NAME J22	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:27.0



TOTAL WEIGHT = 3 X 15 = 45 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER.			

PLATES (table in inches)					
JT	TYPE	PLATES	W.	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	IN-SX	IN-SX	
F	341	0	341	0	5-8	5-8	
C	178	0	178	0	1-8	1-8	
D	36	0	40	0	1-8	1-8	

SEE MITEK STANDARD DETAIL 697791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOOSE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
F	239	170	0	0	0	89	0
C	122	99	0	0	0	23	0
D	29	0	0	0	0	29	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
F-B	-305	0	0.0	0.0	0.03 (1)	7.81	B-E
A-B	0	41	-91.8	-91.8	0.14 (5)	10.00	
B-C	0	0	-91.8	-91.8	0.23 (1)	10.00	
F-E	0	0	-18.5	-18.5	0.06 (4)	10.00	
E-D	0	0	-18.5	-18.5	0.08 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
80T CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.23/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN MAX MIN MAX MIN		
MT20	618 354 1687 788 1987 1656	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

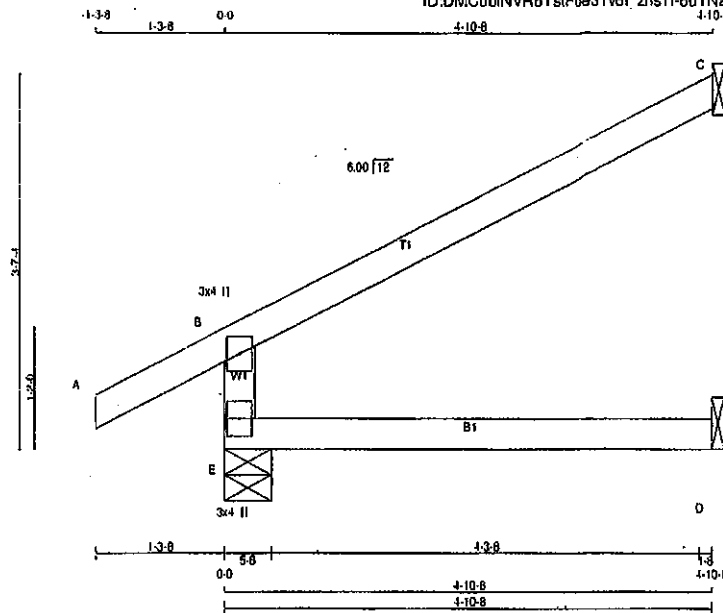


Structural component only
DWG# T-2007625

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	J40	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 14 = 28 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No. 2	
A - C	2x4	DRY	No. 2	
E - D	2x4	DRY	No. 2	

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
E	BMV1+p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ UPLIFT
E	456	0	456	0
C	168	0	168	0
D	38	0	42	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE	MAX. MN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	320	225	0	0	0	0	95	0
C	116	94	0	0	0	0	22	0
D	30	0	0	0	0	0	30	0

BEARING MATERIAL TO BE SPF NO. 2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED MOMENT (FT-LB)	MAX. FACTORED SHEAR (LBS)
FR-TO							
E-B		-404	0	0	0	0.08 (4)	7.81
A-B		0	28	-91.8	-91.8	0.12 (1)	10.00
B-C		-25	0	-91.8	-91.8	0.37 (1)	6.25
E-D		0	0	-18.5	-18.5	0.09 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 8.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TC=0.37 1.00 (B-C:1), BC=0.09/1.00 (D-E:4),
WB=0.00/1.00 (A-B:0), SS=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

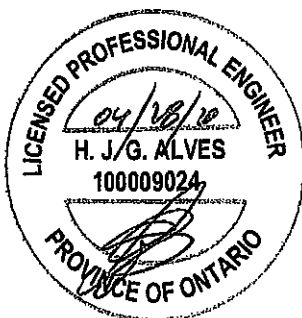
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	616 354	1667 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.17 (E) (INPUT = 0.90)
JSI METAL = 0.11 (B) (INPUT = 1.00)



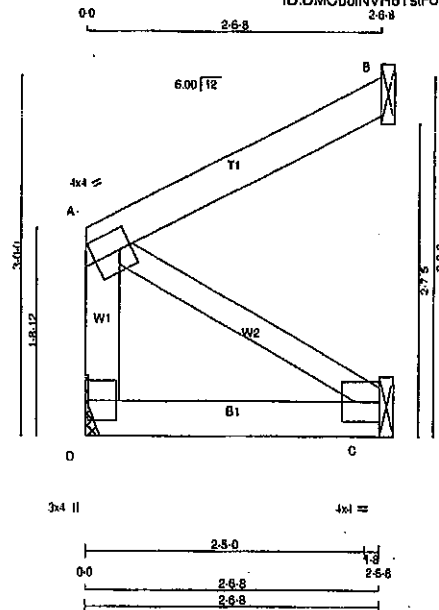
Structural component only
DWG# T-2007657

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	J41	5	1	GREEN PARK HOMES	
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:17.8



LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	4.0	4.0	2.00	1.25
C	BMV1-1	MT20	4.0	4.0	2.00	Edge
D	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
D	139	0	139	0
B	117	0	117	0
C	23	0	25	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED						
D	98	85	0	0.0	0.0	33	0
B	80	65	0	0.0	0.0	15	0
C	18	0	0	0.0	0.0	18	0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX CS1 (LC)	MAX. UNBRAC LENGTH	MEMB. MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM TO		FR-TO		
D-A	-117 0	0.0	0.0 0.01 (1)	7.81	0 0	0.00 (1)
A-B	0 0	-91.8 -91.8	0.10 (1)	10.00		
D-C	0 0	-18.5 -18.5	0.03 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOY CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.10/1.00 (A-B:1), BC=0.03/1.00 (C-D:4),
WB=0.00/1.00 (A-C:1), SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1907 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.07 (A) (INPUT = 0.90)
JSI METAL = 0.02 (A) (INPUT = 1.00)

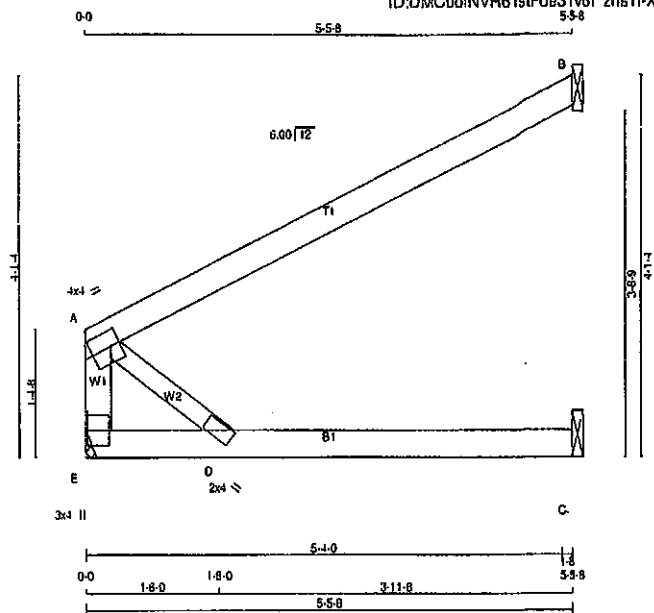


Structural component only
DWG# T-2007658

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408224	J42	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 16 = 31 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
E - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
E - C	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER			

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	4.0	4.0	2.00	1.25
D	BMW+w	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQD
JT	GROSS REACTION		GROSS REACTION		UPLIFT	BRG	BRG
	VERT	HORZ	DOWN	HORZ		IN-SX	IN-SX
E	301	0	301	0	0	MECHANICAL	
B	250	0	250	0	0	1-8	1-8
C	50	0	57	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1'-8"

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	213	140.0	0.0	0.0	0.0	73.0	0.0
B	172	140.0	0.0	0.0	0.0	33.0	0.0
C	40	0.0	0.0	0.0	0.0	40.0	0.0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

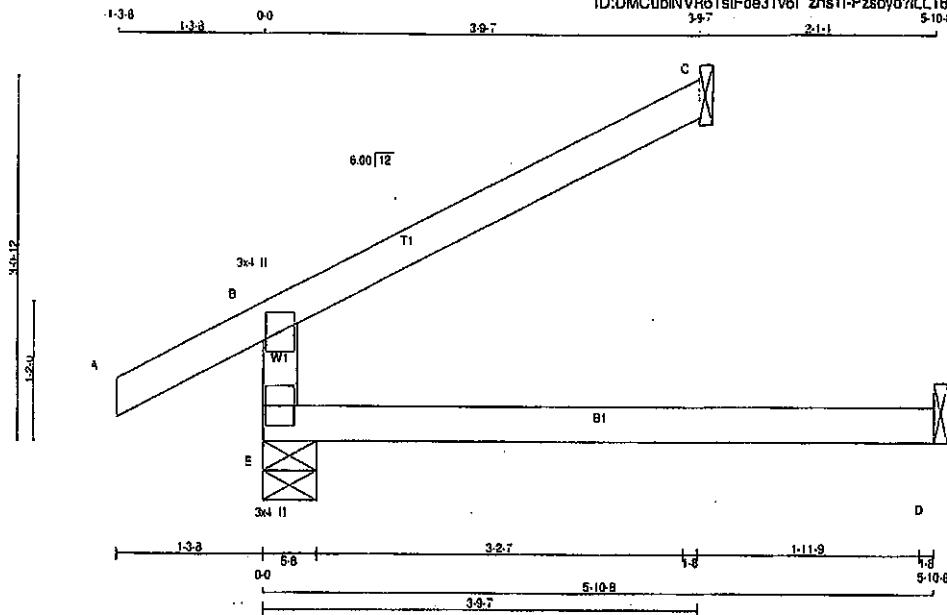
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 CSl (LX)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSl (LX)		
FR-TO		FROM TO			FR-TO				
E-A	-250 0	0.0 0.0	0.03 (1)	7.81	A-D	0 0	0.00 (1)		
A-B	0 0	-91.8 -91.8	0.46 (1)	10.00					
E-D	0 0	-18.5 -18.5	0.13 (4)	10.00					
D-C	0 0	-18.5 -18.5	0.16 (4)	10.00					



Structural component only
DWG# T-2007659

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	C40	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:19:42 2020 Page 1					
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Scale = 1:17.8					



TOTAL WEIGHT = 2 X 14 = 28 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	No.2
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
E	405	0	405	0	0	5-8	5-8		
C	130	0	130	0	0	1-8	1-8		
D	45	0	50	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
E	266	190	0	0	0	96	0	0
C	90	73	0	0	0	17	0	0
D	36	0	0	0	0	36	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED	
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MEMB.	FORCE	MAX.	FACTORED
	(LBS)	(PLF)	CSI (LC)		(LBS)	UNBRAC	CSI (LC)
FR-TO		FROM TO	LENGTH	FR-TO			
E-B	-342 0	0.0 0.0	0.13 (4)	7.81			
A-B	0 28	-91.8 -91.8	0.12 (1)	10.00			
B-C	-19 0	-91.8 -91.8	0.22 (1)	6.25			
E-D	0 0	-18.5 -18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.22/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (A-B:0), SS=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	815	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

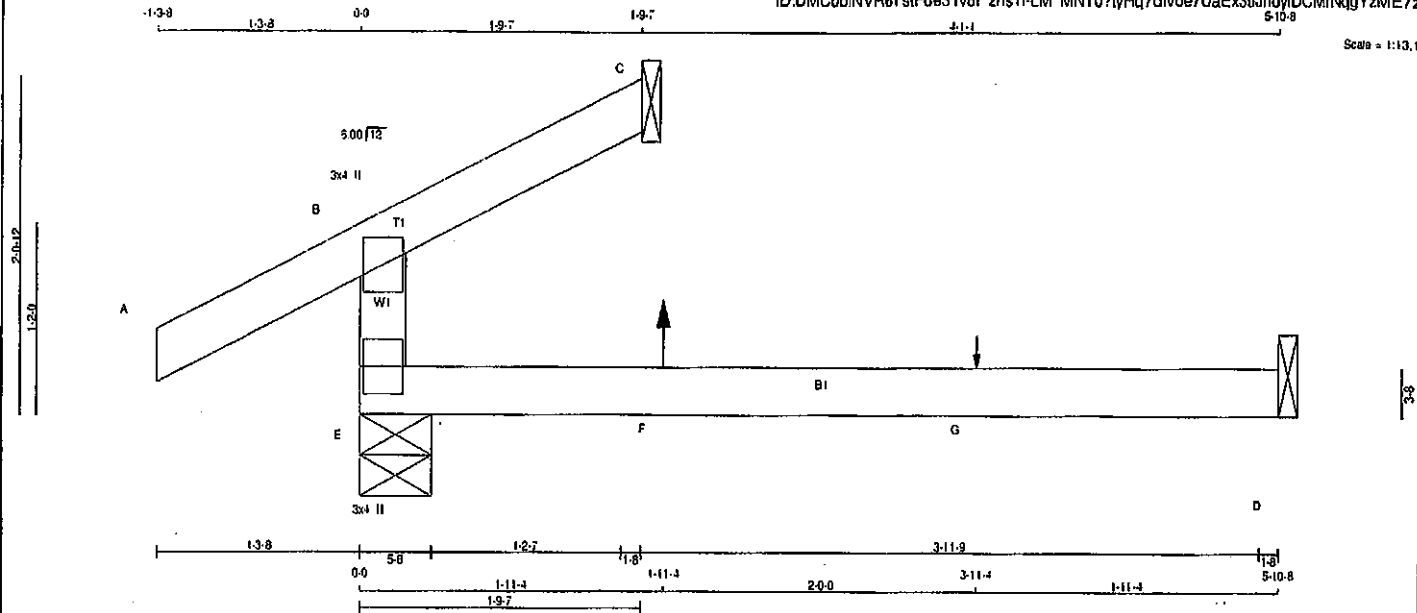
JSI GRIP = 0.14 (E); INPUT = 0.90 ;
JSI METAL = 0.09 (B); INPUT = 1.00 ;



Structural component only
DWG# T-2007648

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	C41	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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5-10-8



TOTAL WEIGHT = 2 X 12 = 23 lb (M)

LUMBER	N. L. C. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (tablets in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HORZ
E	284	0	0	5-8
C	63	0	0	1-8
D	44	0	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	MAX. MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	200	137	0	0	0	62	0
C	46	21	0	0	0	25	0
D	35	0	0	0	0	37	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO							
E-B	-227	0	0.0	0.0	0.11	4	7.81
A-B	0	28	-91.8	-91.8	0.12	11	10.00
B-C	-9	9	-91.8	-91.8	0.08	4	10.00
E-F	0	0	-18.5	-18.5	0.14	4	10.00
F-G	0	0	-18.5	-18.5	0.14	4	10.00
G-D	0	0	-18.5	-18.5	0.14	4	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	BACK	VERT	TOTAL	---	C1
G	3-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TO=0.12/1.00 (A-B:1), BC=0.14/1.00 (D-E:4),
WB=0.00/1.00 (F-G:0), SS=0.09/1.00 (A-B:1)

DOL LUMBER=0.90 NAIL=0.98 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)	MAX MIN	MAX MIN
MT20	618	354	1687	788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

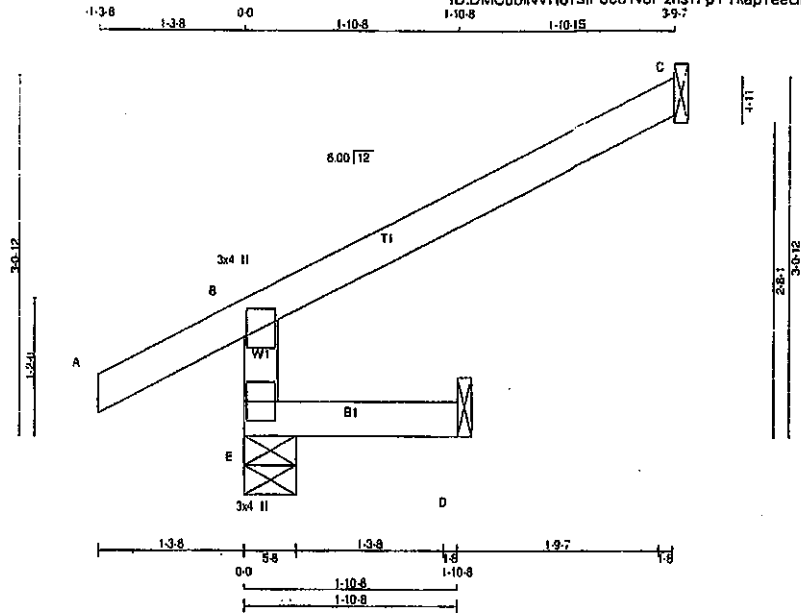
JSI GRIP= 0.10 (E) INPUT = 0.80
JSI METAL= 0.06 (B) INPUT = 1.00



Structural component only
DWG# T-2007649

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	C42	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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Scale = 1/18.2

LUMBER N. L. G. A. RULES CHORDS SIZE DRY LUMBER No.2

CHORDS	SIZE	DRY	LUMBER	No.2
E - B	2x4	DRY	No.2	
A - C	2x4	DRY	No.2	
E - D	2x4	DRY	No.2	

DRY: SEASONED LUMBER.

PLATES (table in inches)	W	LEN	Y	X
JT TYPE PLATES	3.0	4.0		
B TMV+p MT20	3.0	4.0		
E BMV1+p MT20	3.0	4.0		

UNFACTORED REACTIONS	1ST LOOSE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED							
E	250	190	0	0	0	0	60	0
C	90	73	0	0	0	0	17	0
D	12	0	0	0	0	0	12	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS							WEBS							CALCULATED VERT. DEFL.(LL) = L 999 (0.00")		
MAX. FACTORED							MAX. FACTORED							ALLOWABLE DEFL.(TL)= L360 (0.19")		
MEMB. FORCE (LBS)							MEMB. FORCE (LBS)							CALCULATED VERT. DEFL.(TL) = L 999 (0.00")		
VERT. LOAD (PLF)							VERT. LOAD (PLF)									
LC1 MAX. CSI (LC)							LC1 MAX. CSI (LC)									
FROM TO							FROM TO									
E-B	-342	0	0.0	0.0	0.01 (4)	7.81										
A-B	0	28	-91.8	-91.8	0.13 (5)	10.00										
B-C	-19	0	-91.8	-91.8	0.22 (1)	8.25										
E-D	0	0	-18.5	-18.5	0.02 (4)	10.00										

CSI: TC=0.22:1.00 (B-C:1) . BC=0.02:1.00 (D-E:4) .	
WB=0.0:1.00 (A-B:1) . SS=0.15:1.00 (B-C:1)	
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10	
COMP=1.10 SHEAR=1.10 TENS=1.10	
COMPANION LIVE LOAD FACTOR = 1.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

15% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TO=0.22(1.00) (B-C:1), BC=0.02(1.00) (D-E:4), WB=0.00(1.00) (A-B:0), SSI=0.15(1.00) (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PL)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.14 (E) (INPUT = 0.90)
JSI METAL = 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007650



TOTAL WEIGHT = $3 \times 7 = 21$ lb

DESCA
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REORD
GROSS REACTION		GROSS REACTION		GROSS REACTION		BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
JT	271	0	271	0	0	5-8	5-8
E	45	0	45	0	-23	1-8	1-8
C	8	0	17	0	-2	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	188	141 0	0 0	0 0	0 0	47 0	0 0
C	31	24 -18	0 0	0 0	0 0	7 0	0 0
D	7	0 -8	0 0	0 0	0 0	12 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E. C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	MAX. UNBRAG LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO		FR-TO			
E-B	-244 0	0.0 0.0	0.04 (5)	7.81			
A-B	0 28	-91.8 -91.8	0.12 (1)	10.00			
B-C	-17 0	-91.8 -91.8	0.09 (1)	6.25			
E-D	0 0	-18.5 -18.5	0.04 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBCG 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L360 (0.19")
CALCULATED VERT. DEFL.(LL) = L 999 (0.00")
ALLOWABLE DEFL.(TL)= L360 (0.19")
CALCULATED VERT. DEFL.(TL) = L 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , SC=0.04/1.00 (D-E:5) ,
WB=0.00/1.00 (n/a:0) , SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

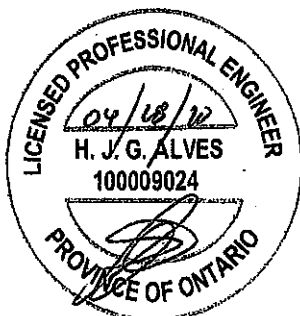
NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PSI)	(PLI)	(PLI)	(PLI)	(PLI)
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	818	354	1687	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) INPUT = 0.90 1
JSI METAL= 0.07 (B) INPUT = 1.00 1



Structural component only
DWG# T-2007651

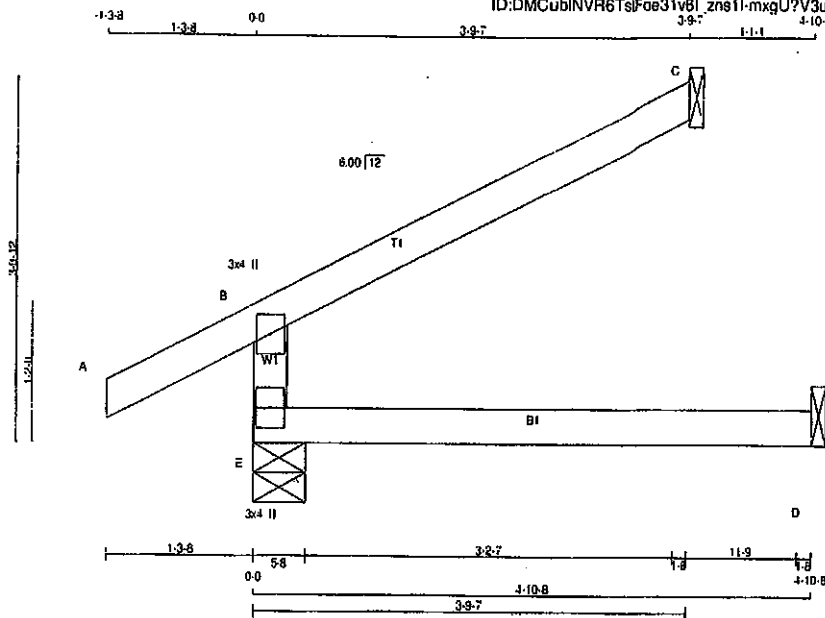
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408224	C44	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:17.9



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (tablets in inches)	JT TYPE	PLATES	W	LEN	V	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	UPLIFT
E	394	0	394	0
C	130	0	130	0
D	38	0	42	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED					
E	277	190	0	0	0	0
C	90	73	0	0	0	0
D	30	0	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	MEMB.	FORCE
	(LBS)	(PLF)		(LBS)
FR-TO		FROM TO	FR-TO	
E-B	-342	0	0.0	0.09 (4)
A-B	0	28	-91.8	-91.8 0.12 (1)
B-C	-19	0	-91.8	-91.8 0.22 (1)
E-D	0	0	-18.5	-18.5 0.09 (4)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. GC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2016, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.22/1.00 (B-C:1), 8C=0.09/1.00 (D-E:4),
WB=0.00/1.00 (D-E:0), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP/DRY	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	818	354

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.50)
JSI METAL= 0.09 (B) (INPUT = 1.00)

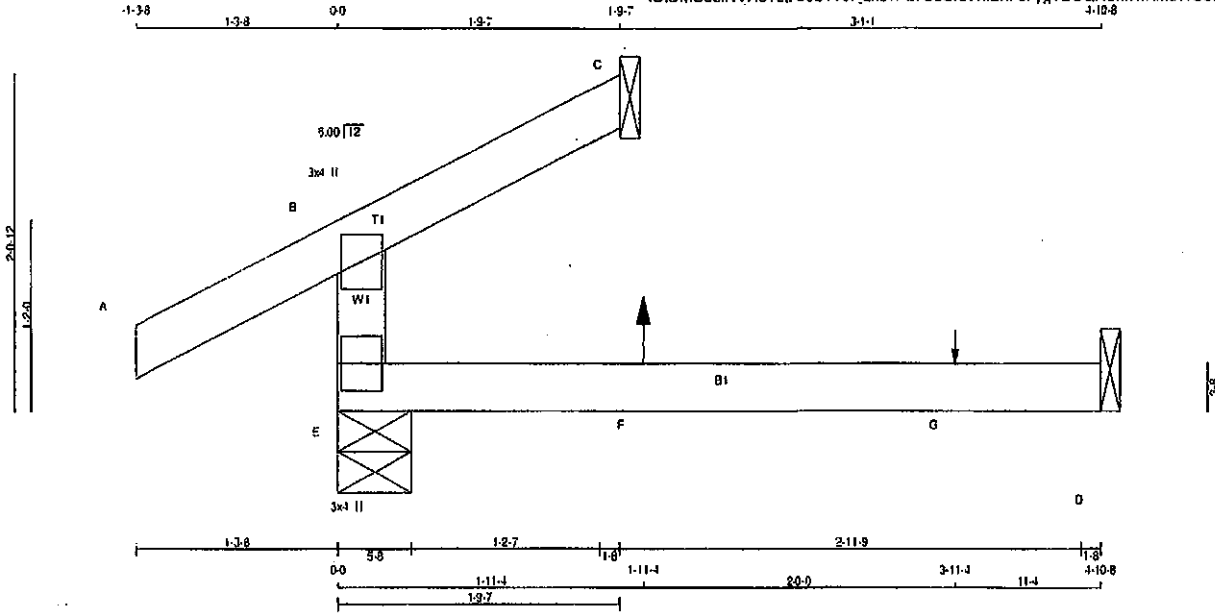


Structural component only
DWG# T-2007652

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	C45	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:1.1



TOTAL WEIGHT = 2 X 10 = 21 lb (M)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
E	BMV+p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING DESIGNER							
BEARINGS							
	FACTORED		MAXIMUM FACTORED		INPUT	REQRD	
	GROSS REACTION		GROSS REACTION		BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	282	0	282	0	0	5-8	5-8
C	55	0	55	0	0	1-8	1-8
D	35	0	44	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	198	137	0	0	0	0	0
C	39	21	0	0	0	0	0
D	29	0	-4	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

MEMB.	CHORDS		WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO				
E-B	-235	0	0.0	0.07 (4)	7.81	
A-B	0	28	-91.8	-91.8 0.12 (1)	10.00	
B-C	-13	5	-91.8	-91.8 0.07 (5)	6.25	
E-F	0	0	-18.5	-18.5 0.10 (4)	10.00	
F-G	0	0	-18.5	-18.5 0.10 (4)	10.00	
G-D	0	0	-18.5	-18.5 0.10 (4)	10.00	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	BACK	VERT	TOTAL	---	C1
G	3-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TC=0.12/1.00 (A-B:1), BC=0.10/1.00 (D-E:4), WB=0.00/1.00 (A-B:3), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.10 (E) / INPUT = 0.90
JSI METAL = 0.07 (B) / INPUT = 1.00

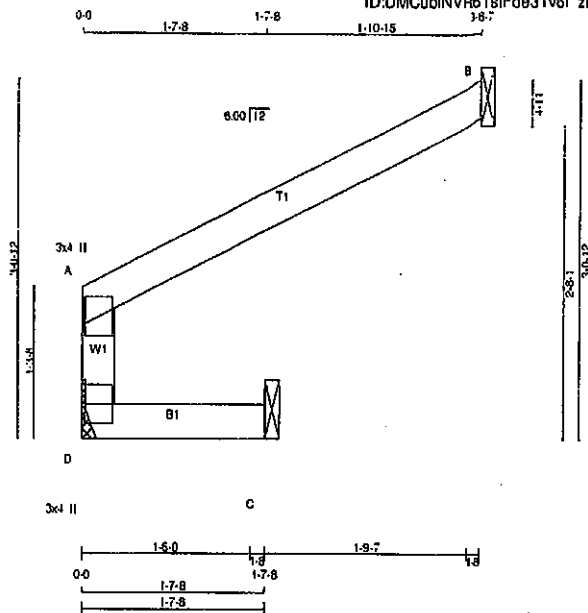


Structural component only
DWG# T-2007653

JOB NAME 408224	TRUSS NAME C46	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:18.2



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
D - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
D - C	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMV+p	MT20	3.0	4.0	
D	BMV+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
D	156	0	156	0	0	MECHANICAL			
B	144	0	144	0	0	1-8	1-8		
C	55	0	55	0	0	1-8	1-8		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	109	78	0	0	0	0	31	0
B	99	80	0	0	0	0	19	0
C	40	23	0	0	0	0	17	0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: 14

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1 (PLF)	MAX. CS1 (LC)	UNBRAC LENGTH	MEMB.	FORCE (LBS)
FR-TO						FR-TO	
D-A	-181	0	0.0	0.0	0.08 (1)	7.81	
A-B	-8	0	-91.8	-91.8	0.14 (1)	10.00	
D-C	0	0	-18.5	-18.5	0.09 (1)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
OL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (A-B:1), BC=0.09/1.00 (C-D:1), WB=0.00/1.00 (A-B:0), SS1=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.08 (D) INPUT = 0.50
JSI METAL = 0.05 (A) INPUT = 1.00



Structural component only
DWG# T-2007654

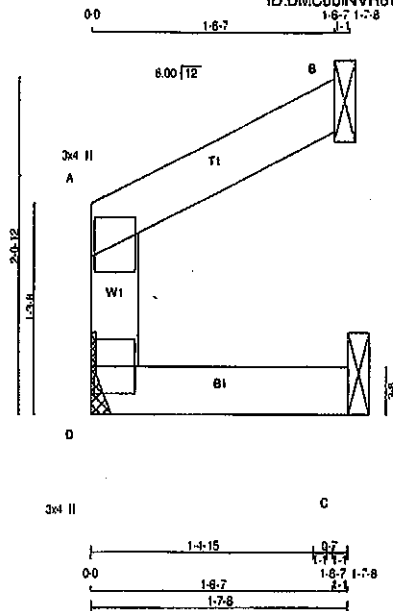
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	C47	1	1	GREEN PARK HOMES	
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:13.1



TOTAL WEIGHT = 5 lb (M/F)

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
A - TMV+p	MT20	3.0	4.0	
D - BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	BRG	BRG	BRG
D	88	0	88	0	0	MECHANICAL	MECHANICAL	1-8	1-8
B	67	0	67	0	0	1-8	1-8	1-8	1-8
C	19	0	19	0	0	1-8	1-8	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	1ST LOOSE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	81	39 0	0 0	0 0	0 0	0 0	21 0	0 0
B	46	37 0	0 0	0 0	0 0	0 0	9 0	0 0
C	14	2 0	0 0	0 0	0 0	0 0	12 0	0 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LC1)	MAX. (LC)	MEMB.	FORCE (LBS)	MAX. (LC)	UNBRACED LENGTH (FR-TO)
FR-TO							
D-A	-74 0	0.0	0.0 0.01 (1)	7.81			
A-B	-2 0	-91.8	-91.8 0.03 (1)	10.00			
D-C	0 0	-18.5	-18.5 0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TO=0.02, 1.00 (A-B:1), BC=0.01, 1.00 (C-D:4), WB=0.00, 1.00 (1/10:3), 9SI=0.05, 1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.03 (D) (INPUT = 0.90)
JSI METAL = 0.02 (A) (INPUT = 1.00)



Structural component only
DWG# T-2007655

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

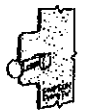
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

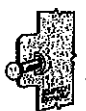
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



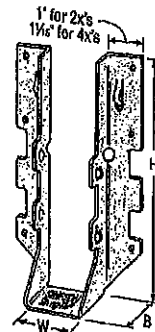
Double-Shear
Nailing
Top View



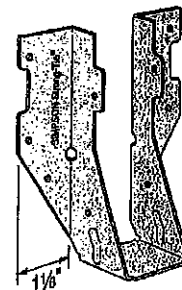
Double-Shear
Nailing
Side View;
Do not
bend tab



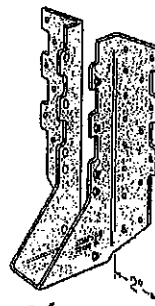
Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580



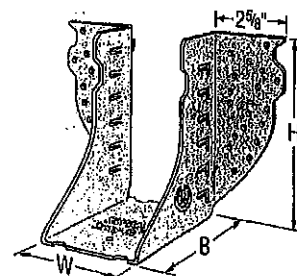
LUS26



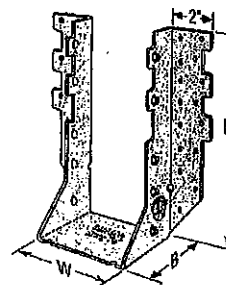
LU26L



HUS210
(HUS26, HUS28,
and HHUS similar)

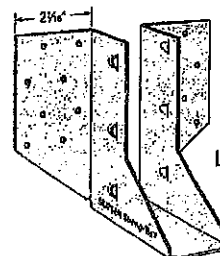
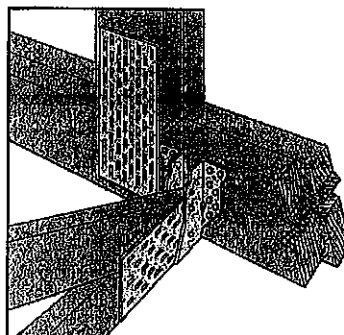


HGUS28-2



HHUS210-2

Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS**HHUS/HGUS**

See Hanger Options Information on pp. 125–127.

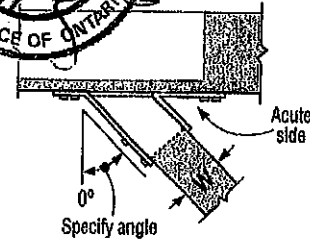
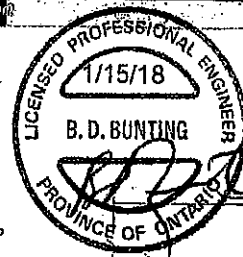
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.82 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



**Top View HHUS Hanger
Skewed Right**
(joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _g ³	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Single 2x Sizes											
LUS24	18	1 1/8	3 1/4	1 3/4	2 1/4	(4) 10d	(2) 10d	710	1625	845	1155
								3.16	7.23	2.87	5.14
LU24L	22	1 1/8	3	1 1/4	2 1/8	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725
								1.60	4.54	1.42	3.22
LU26L	22	1 1/8	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"	720	1605	645	1140
								3.20	7.14	2.87	5.07
LUS26	18	1 1/8	4 1/4	1 3/4	3 1/4	(4) 10d	(4) 10d	1420	2170	1290	1630
								6.32	9.85	5.74	7.25
HUS26	18	1 1/8	5 1/4	3	3 1/8	(14) 10d	(8) 10d	2705	4940	2065	3875
								11.30	21.97	9.20	17.24
LJS26DS	18	1 1/8	5	3 1/2	4 1/4	(16) 10d	(6) 10d	2055	4265	1480	4115
								9.14	18.97	6.49	18.31
HGUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 10d	(8) 10d	2885	6625	2685	5700
								11.96	29.51	11.98	25.35
LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1550
								5.07	9.72	4.54	6.89
LUS28	18	1 1/8	6 1/4	1 3/4	3 1/4	(6) 10d	(4) 10d	1420	2520	1290	1790
								6.32	11.21	5.74	7.96
HUS28	16	1 1/8	7 1/8	3	6 1/8	(22) 10d	(8) 10d	3605	5365	2675	4345
								16.04	23.86	11.90	19.33
HGUS28	12	1 1/8	7 1/8	5	6 1/4	(36) 10d	(12) 10d	3310	7675	3310	6900
								14.74	34.19	14.74	30.73
LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(8) 10d x 1 1/2"	1140	2495	1020	1770
								5.07	11.10	4.54	7.87
LUS210	18	1 1/8	7 1/8	1 3/4	3 1/4	(8) 10d	(4) 10d	1420	2785	1290	2210
								6.32	12.39	5.74	9.83

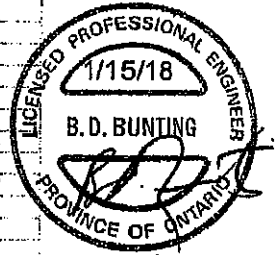
1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_g is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/4" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _o ^a	Header	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
LUS24-2	18	3 1/8	3 1/8	2	1 1/2	(4) 16d	(2) 16d	835	2020	580	1435
SS LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	3.71	8.89	2.62	6.38
HHUS28-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	1720	2595	1545	1920
HGUS28-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	7.65	11.54	6.87	8.64
SS LUS28-2	18	3 1/8	7	2	4	(8) 16d	(4) 16d	2850	7335	2085	5205
HHUS28-2	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	12.68	32.63	9.20	23.15
HGUS28-2	12	3 1/8	7 1/8	4	8 1/8	(36) 16d	(12) 16d	4385	8950	3110	6355
SS LUS210-2	18	3 1/8	9	2	6	(8) 16d	(8) 16d	19.51	39.81	13.83	28.27
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	1720	3325	1545	2675
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	7.65	14.79	6.87	11.46
								3765	8940	2675	6345
								16.75	39.77	11.90	28.22
								6070	12980	4310	9215
								27.00	57.74	19.17	40.99
SS LUS210-2	18	3 1/8	9	2	6	(8) 16d	(8) 16d	2580	4500	2320	3195
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	11.48	20.02	10.32	14.21
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	4670	9580	4235	7000
								20.77	42.97	18.04	31.14
								6840	14015	4855	10270
								30.43	62.34	21.60	45.69
Triple 2x Sizes											
HGUS26-3	12	4 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	19.51	39.81	13.83	28.27
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d	6070	12980	4310	9215
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	27.00	57.74	19.17	40.99
								4670	9580	4235	6865
								20.77	42.97	18.04	30.54
								6840	14645	4855	10400
								30.43	65.14	21.60	46.26
Quadruple 2x Sizes											
HGUS28-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	19.51	39.81	13.83	28.27
HHUS210-4	14	6 1/8	8 1/8	3	7 1/8	(30) 16d	(10) 16d	6070	12980	4310	9215
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	27.00	57.74	19.17	40.99
								4670	10155	4235	7210
								20.77	45.17	18.84	32.07
								6840	14645	4855	10400
								30.43	65.14	21.60	46.26
HGUS212-4	12	8 1/8	10 1/8	4	10 1/8	(50) 16d	(20) 16d	7640	14995	5425	10845
HGUS214-4	12	8 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	33.98	66.70	24.13	47.35
								10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
LUS48	18	3 1/8	4 1/8	2	3 1/8	(4) 16d	(4) 16d	1720	2595	1545	1920
HHUS48	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	7.65	11.54	6.87	8.64
HGUS48	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	2540	7335	2085	5205
SS LUS48	18	3 1/8	6 1/8	2	3 1/8	(8) 16d	(4) 16d	11.90	32.63	9.20	23.15
HHUS48	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	4385	8950	3110	6355
HGUS48	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	19.51	39.81	13.83	28.27
								1720	3325	1545	2675
								7.65	14.79	6.87	11.46
								3765	8940	2675	6345
								16.75	39.77	11.90	28.22
								6070	12980	4310	9215
								27.00	57.74	19.17	40.99
SS LUS410	18	3 1/8	8 1/8	2	5 1/8	(8) 16d	(8) 16d	2580	4500	2320	3195
HGUS410	12	3 1/8	9	4	8 1/8	(46) 16d	(16) 16d	11.48	20.02	10.32	14.21
								6840	14015	4855	10270
								30.43	62.34	21.60	45.69
								7640	14995	5425	10845
								33.98	66.70	24.13	47.35
								10130	16400	7195	11645
								45.06	72.95	32.00	61.80



Plated Truss Connectors

See footnotes on p. 258.

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1 1/4". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

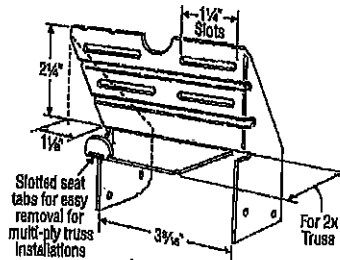
Design: Factored resistances are in accordance with CSA 086-14

Installation:

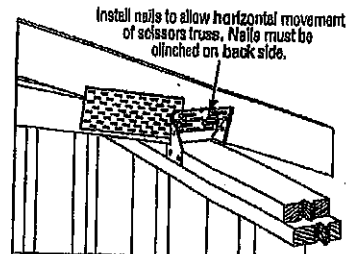
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

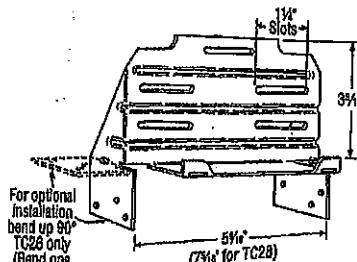
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titan® screws into the top and face of masonry wall. See optional load tables and installation details.



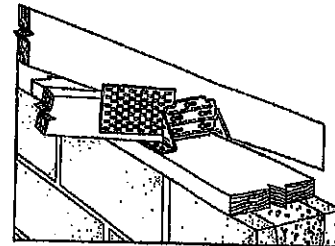
TC24
U.S. Patent 4,932,173



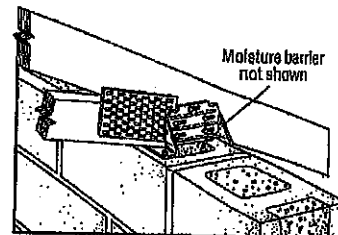
Typical TC24 Installation



TC26
(TC28 Similar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titan Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _o =1.15)	Uplift (K _o =1.15)
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(5) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _o =1.15)	Uplift (K _o =1.15)
TC26	(5) 10d	(6) 10d x 1 1/2"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.

2. Grout strength is 15 MPa minimum.

3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.

4. TC26 fastened to grouted concrete block with (6) - 9/16" x 2 1/4" Titan screws has a factored uplift resistance of 276 lb.



UNITED STATES DESIGN

2017 Simpson Strong-Tie Company

SECTION

(800) 999-5099
strongtie.com

H/TSP

Seismic and Hurricane Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Fasteners			Factored Resistance ($K_0 = 1.15$)					
					D.Fir-L			S-P-F		
		To Rafters/Truss	To Plates	To Studs	Uplift	Lateral		Uplift	Lateral	
						F ₁	F ₂		F ₁	F ₂
					lb.	lb.	lb.	lb.	lb.	lb.
					kN	kN	kN	kN	kN	kN
H1	18	(8) 8d x 1 1/2"	(4) 8d	—	740	685	300	680	485	216
					3.29	3.05	1.33	3.02	2.16	0.98
SS H2A	18	(5) 8d x 1 1/2"	(2) 8d x 1 1/2"	(5) 8d x 1 1/2"	830	220	75	590	155	55
					3.69	0.98	0.33	2.62	0.69	0.24
SS H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
					3.58	0.71	0.71	3.36	0.71	0.71
H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
					3.71	0.78	0.93	3.29	0.71	0.93
SS H3	18	(4) 8d	(4) 8d	—	740	180	285	615	125	190
					3.29	0.80	1.18	2.74	0.56	0.85
H6	16	—	(0) 8d	(8) 8d	1585	1085	—	1125	770	—
					7.05	4.83	—	5.00	3.43	—
H7Z	18	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
					6.18	2.98	—	4.40	2.11	—
SS H8 ³	18	(5) 10d x 1 1/2"	(5) 10d x 1 1/2"	—	1120	—	—	1025	—	—
					4.98	—	—	4.56	—	—
SS H10A ³	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1735	795	410	1605	565	290
					7.72	3.54	1.82	6.69	2.51	1.29
H10AR	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1485	690	430	1220	570	305
					6.61	3.07	1.91	5.43	2.54	1.36
H10A-2	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1835	1275	430	1645	880	305
					8.16	5.67	1.91	7.32	3.91	1.36
H10S ^{7,8}	18	(8) 8d x 1 1/2"	(8) 8d x 1 1/2"	(8) 8d	1465	795	315	1040	565	225
					6.52	3.54	1.40	4.63	2.51	1.00
H11Z	18	(6) 16d x 2 1/2"	(6) 16d x 2 1/2"	—	1095	920	545	780	655	390
					4.87	4.09	2.42	3.47	2.91	1.73
H14	18	[1] (12) 8d x 1 1/2"	(13) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
		[2] (12) 8d x 1 1/2"	(15) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
TSP	18	(9) 10d x 1 1/2"	(6) 10d x 1 1/2"	—	1285	440	—	920	310	—
					5.76	1.98	—	4.09	1.38	—
		(9) 10d x 1 1/2"	(6) 10d	—	1580	440	—	1105	310	—
					6.94	1.98	—	4.92	1.38	—

- Factored resistances have been increased 15% for short term loading; no further increase is allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2 1/2" must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
- H8 factored uplift resistances for stud-to-bottom plate installations are 695 lb. (2.65 kN) for D.Fir-L and 390 lb. (1.74 kN) for S-P-F.
- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-HIECONPATH).
- Factored resistances in the F₁ direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.

- H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.38 kN) D.Fir-L and 1015 lb. (4.51 kN) S-P-F.
- H10S nails to plates are optional for uplift but required for lateral loads.
- H10A may be field-bent up to a slope of 8/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
- The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SSNAILS at strongtie.com.
- D.Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1 1/2" nails into the top plates and (3) 8d x 1 1/2" nails into the lowest three flange holes into the truss bottom chord is 485 lb. (2.20 kN).
- Nails: 16d x 2 1/2" = 0.162" dia. x 2 1/2" long, 10d = 0.148" dia. x 3" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 8d = 0.131" dia. x 2 1/2" long, 8d x 1 1/2" = 0.131" dia. x 1 1/2" long. See pp. 27-28 for other nail sizes and information.

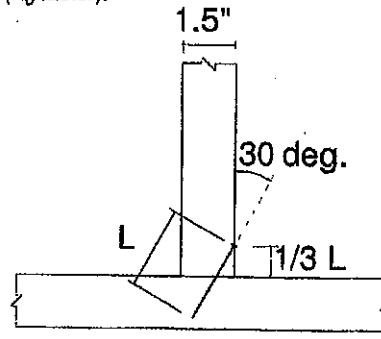
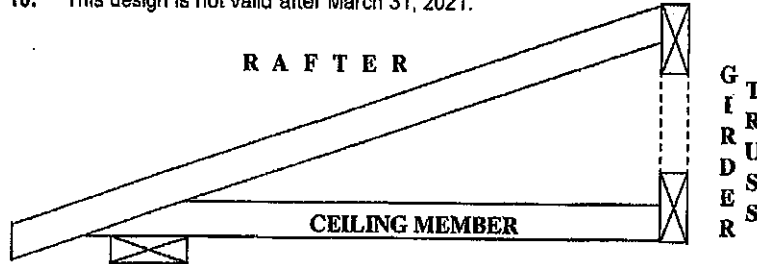
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.

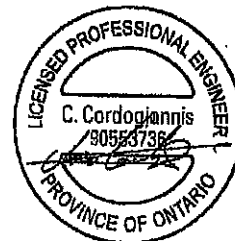


Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (In)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

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December 2, 2019

PEO
Certificate No. 10889485



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

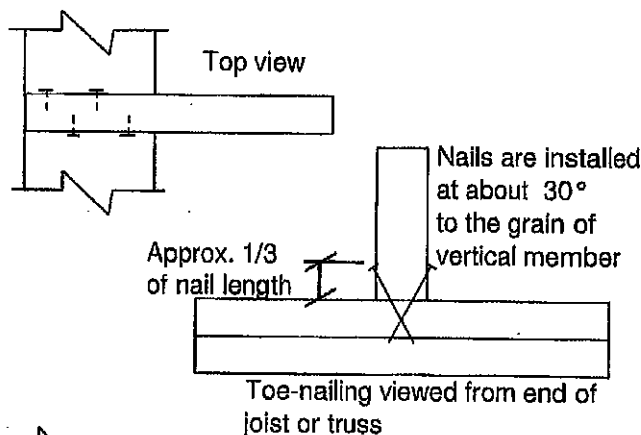
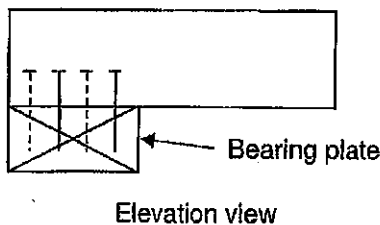
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

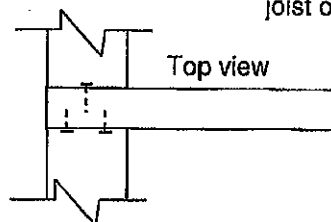
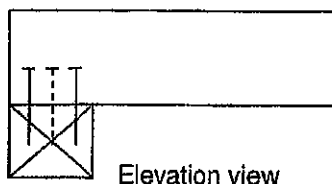
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to wind or earthquake load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



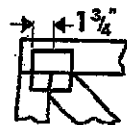
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Bradford, Ontario L3Z 3G7



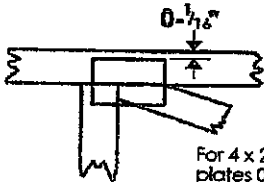
December 2, 2019

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint, unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/4" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

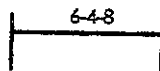


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

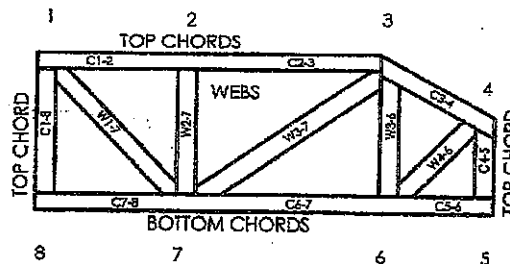
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MII-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

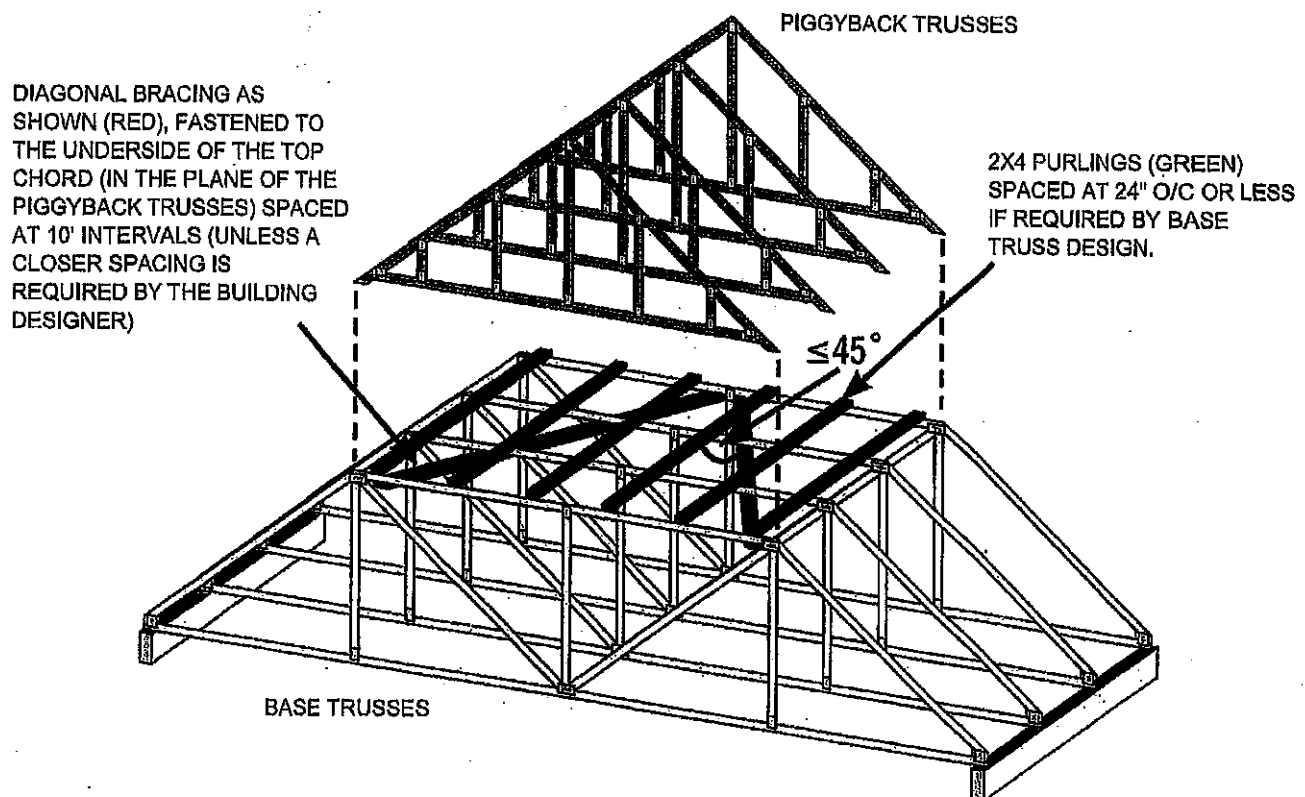
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane of joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

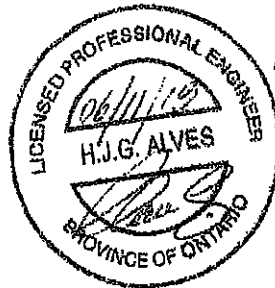
Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufacturer's responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet M117473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900213

Feb 09, 2018