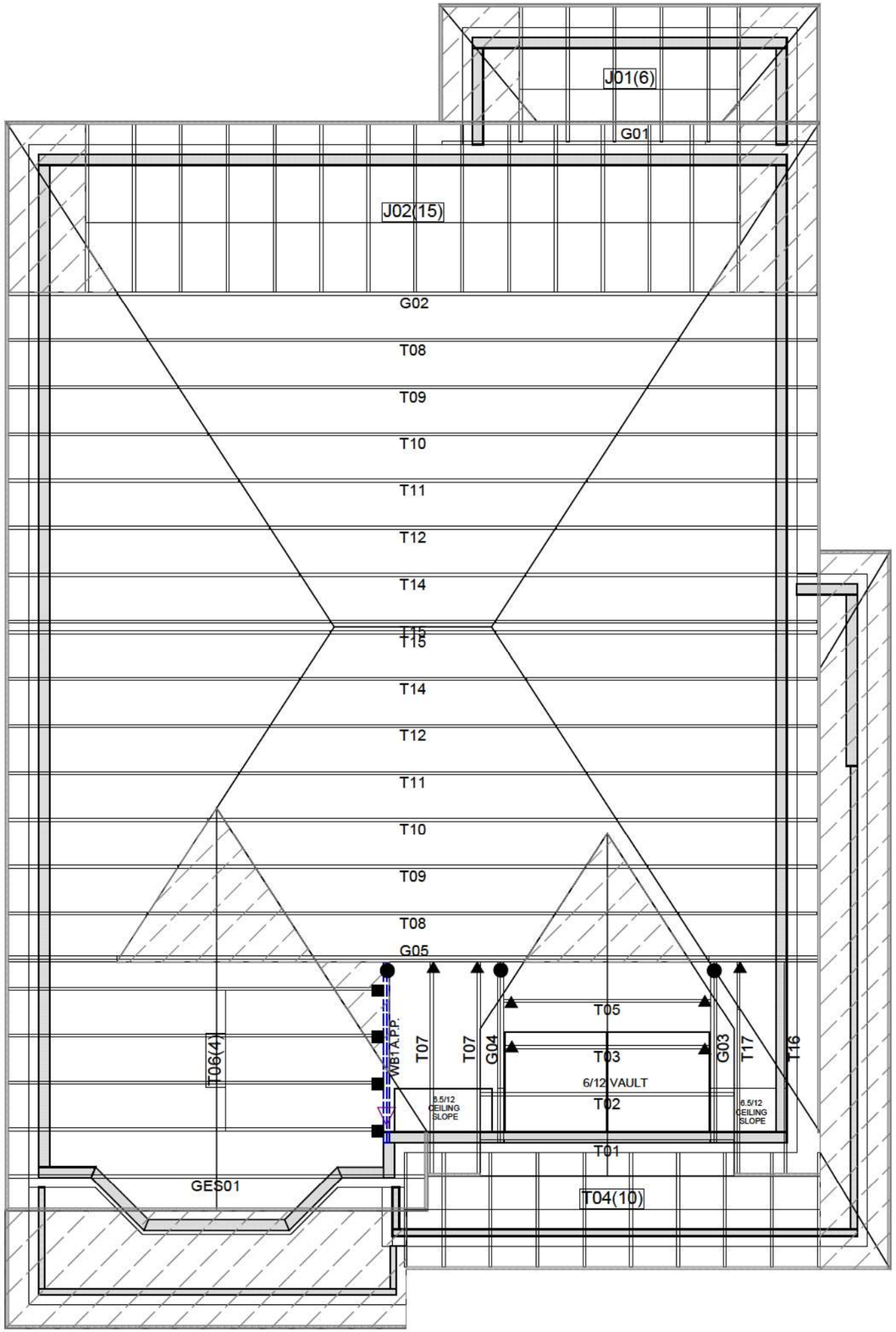

Hanger Name	Symbol	QTY
LUS24	▲	7
LJS26DS	■	4
HGUS26-2	●	3

Additional 5 psf dead load has been added to Top Chord loading to account for weight of solar panels. Solar panels must be installed as per Detail in Engineering Package

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2021-02-19
TOWN OF EAST GWILLIMBURY
Building Standards Branch

Town of East Gwillimbury
Building Standards Branch BCIN #16487

These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

CONVENTIONAL FRAMING BY OTHERS

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE OBC. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE MIN. 2x4 SPF @ 24" C/C WITH A 2x4 VERTICAL POST TO THE TRUSS BELOW. VERTICAL POSTS TO BE LATERALLY BRACED SO THAT UNBRACED LENGTH DOES NOT EXCEED 6'. DESIGN OF CONVENTIONAL FRAMING IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.

JOB INFORMATION

Customer	GREENPARK HOMES
Job #	T20-00761
Address	TRINAR HALL EAST GWILLIMBURY,ON
Model	GLENWAY 2A ELEV 2
Sales Rep	RALPH MIRIGELLO
Designer	DM
Date	12/16/2020
Path	C:\DATA\JOBS\GREENPARK\TRINAR HALL\MODELS\GLENWAY 2A\ELEV 2\T-GLENWAY2A-2\

DESIGN INFORMATION

Code	NBCC 2015
Bldg	Residential - HSB (NBCC Part 9)
TC LL	34.8 lb/ft²
TC DL	8.0 lb/ft²
BC LL	10.5 lb/ft²
BC DL	7.3 lb/ft²
Deflection	LL=L/360 TL=L/360
Spacing	24" O/C unless otherwise noted
Complies With	OBC 2012 (2019 Amendment) CSA O86-14 and TPIC 2014

IMPORTANT INFORMATION

Refer to truss drawings in the Truss Engineering Package for ply-to-ply attachment notes

For site-framed valleys: top chords of all roof trusses must be laterally supported using 2x4 continuous bracing @24 O/C - all bracing must be anchored at ends as per TPIC Installation Guidelines

Read all notes on this page in addition to those shown on the KOTT Truss Engineering package

Field erection, handling and bracing are not the responsibility of KOTT, or KOTT Engineering

Unless noted otherwise, hurricane ties are to be installed at the bearings of all trusses > 40 ft clear span, and any girder or beam supporting trusses with a clear span >40 ft. See hanger legend for type.

Unless noted otherwise, for Part 9 bldgs, all trusses are to be anchored to the top of supporting walls as follows: trusses with a clear span <40 ft use 3-1/4" nails @ each bearing; trusses with a clear span >40 ft use 3-1/4" nails @ each bearing in addition to the appropriate hurricane tie.

KOTT Inc.
14 Anderson Blvd.
Uxbridge, ON
905.642.4400

KOTT

NE1220-107
GREENPARK - TRINAR
HALL - GLENWAY 2A ELE 2

ENGINEERING NOTE PAGE (ENP-1)

PLEASE READ PRIOR TO INSTALLATION

RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY KOTT DESIGN. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- **It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	AWG NO.
NE1220-107	G01	1	1	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	
				TRUSS DESC.	**Page 2 of 31**

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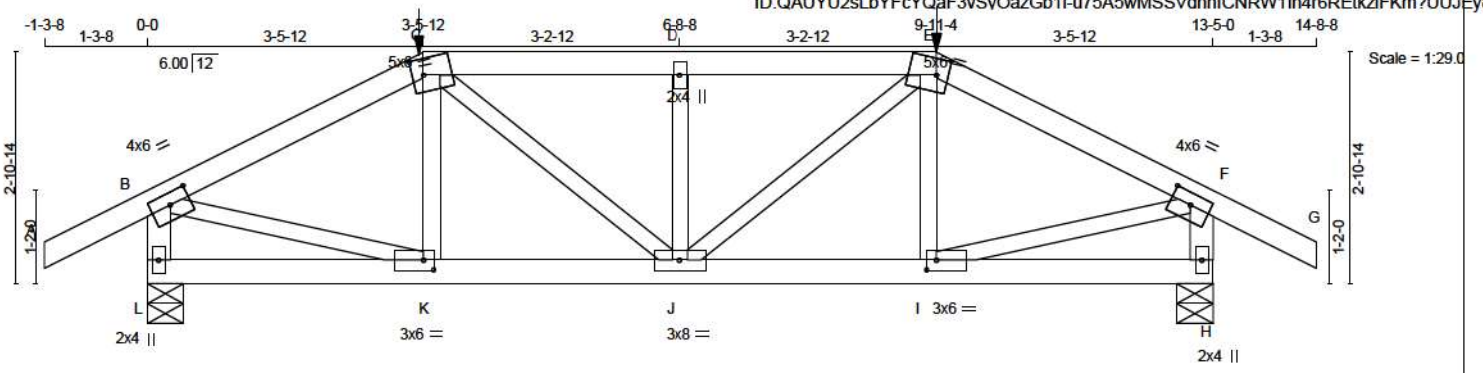

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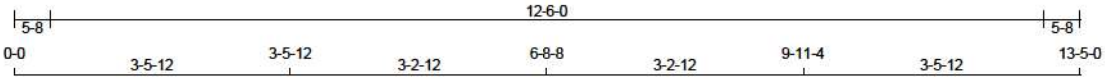

TOTAL WEIGHT = 54 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TTWW-m	MT20	5.0	6.0	2.50	1.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.50	1.75
F	TMVW-t	MT20	4.0	6.0	1.75	3.00
H	BMV1+p	MT20	2.0	4.0		
I	BMWW-t	MT20	3.0	6.0	1.50	1.50
J	BMWWW-t	MT20	3.0	8.0		
K	BMWW-t	MT20	3.0	6.0	1.50	1.50
L	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1813	0	1813	0	0	5-8	2-6
H	1813	0	1813	0	0	5-8	2-6

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1334	793 / 0	211 / 0	0 / 0	0 / 0	329 / 0	0 / 0
H	1334	793 / 0	211 / 0	0 / 0	0 / 0	329 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.01 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A- B	0 / 38	-124.4 -124.4	0.18 (1)	10.00	K- C	-201 / 112	0.04 (1)
B- C	-2021 / 0	-124.4 -124.4	0.36 (1)	4.34	C- J	0 / 892	0.17 (1)
C- D	-2336 / 0	-186.6 -186.6	0.39 (1)	4.01	J- D	-718 / 0	0.13 (1)
D- E	-2336 / 0	-186.6 -186.6	0.39 (1)	4.01	J- E	0 / 892	0.17 (1)
E- F	-2021 / 0	-124.4 -124.4	0.36 (1)	4.34	I- E	-201 / 112	0.04 (1)
F- G	0 / 38	-124.4 -124.4	0.18 (1)	10.00	B- K	0 / 1856	0.46 (1)
L- B	-1724 / 0	0.0 0.0	0.19 (1)	6.29	I- F	0 / 1856	0.46 (1)
H- F	-1724 / 0	0.0 0.0	0.19 (1)	6.29			
L- K	0 / 0	-58.9 -58.9	0.14 (3)	10.00			
K- J	0 / 1798	-58.9 -58.9	0.41 (1)	10.00			
J- I	0 / 1798	-58.9 -58.9	0.41 (1)	10.00			
I- H	0 / 0	-58.9 -58.9	0.14 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-5-12	-213	-213	---	FRONT	VERT	TOTAL	---	C1
E	9-11-4	-213	-213	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 34.8	PSF
	DL	= 8.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.3	PSF
TOTAL LOAD		= 60.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 3-5-12
END SETBACK = 4-5-8
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.39/1.00 (C-D:1) , BC=0.41/1.00 (J-K:1) , WB=0.46/1.00 (B-K:1) , SSI=0.31/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Town of East Gwillimbury
Building Standards Branch BCIN #16487

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Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

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2021-02-19
TOWN OF EAST GWILLIMBURY
Building Standards Branch

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DWG NO.
NE1220-107	G02	1	1	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	
				TRUSS DESC.	**Page 3 of 31**

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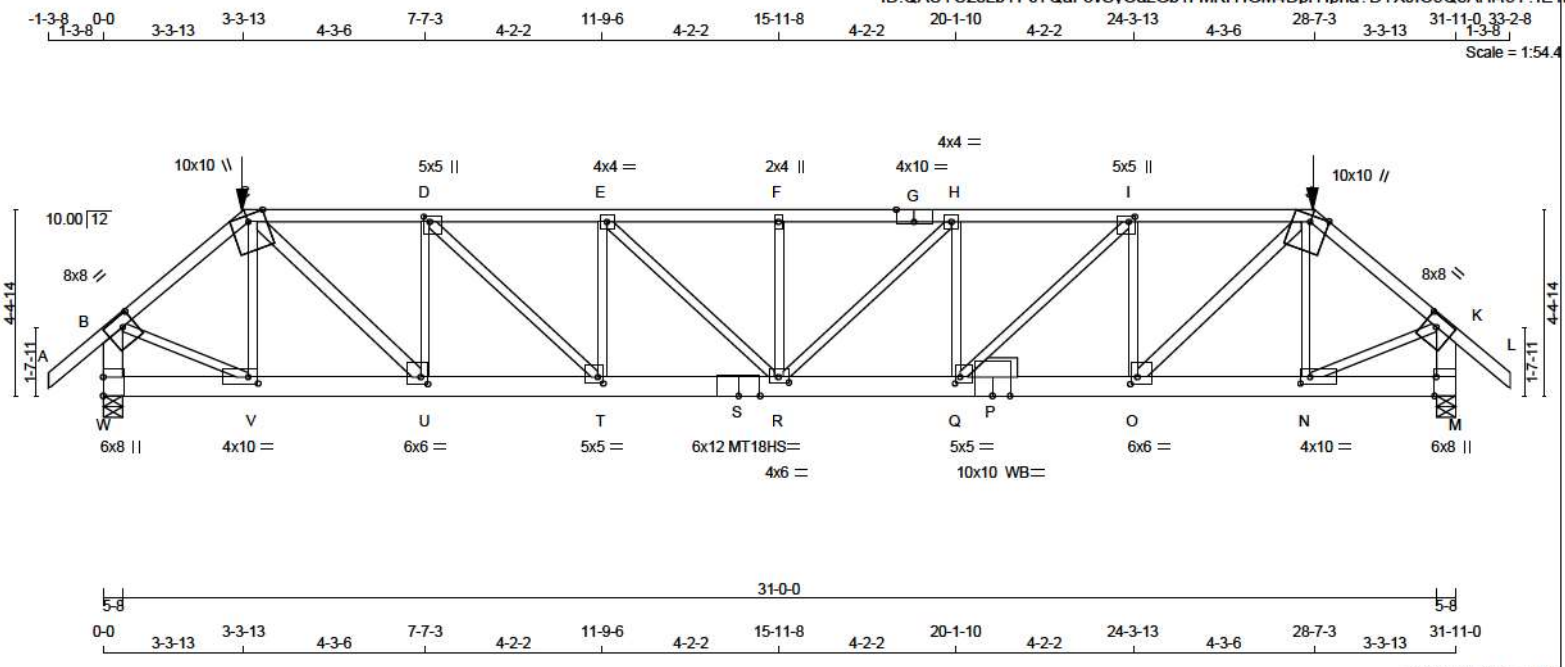

TOTAL WEIGHT = 162 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	2100F 1.8E	SPF
G - J	2x4	DRY	2100F 1.8E	SPF
J - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - S	2x6	DRY	2100F 1.8E	SPF
S - P	2x6	DRY	2100F 1.8E	SPF
P - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - U	2x4	DRY	No.2	SPF
O - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	8.0	8.0	3.00	3.25
C	TTWW+m	MT20	10.0	10.0	Edge	
D	TMWW+t	MT20	5.0	5.0	1.50	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	4.0	10.0	Edge	5.00
H	TMWW-t	MT20	4.0	4.0		
I	TMWW+t	MT20	5.0	5.0	1.50	1.75
J	TTWW+m	MT20	10.0	10.0	Edge	
K	TMVW-t	MT20	8.0	8.0	3.00	3.25
M	BMV1+t	MT20	6.0	8.0	Edge	0.50
N	BMWW-t	MT20	4.0	10.0	1.75	2.75
O	BMWW-t	MT20	6.0	6.0	2.00	2.00
P	BS-t	MT20	10.0	10.0		
Q	BMWW-t	MT20	5.0	5.0	1.75	1.50
R	BMWWW-t	MT20	4.0	6.0	1.50	3.00
S	BS-t	MT18HS	6.0	12.0		
T	BMWW-t	MT20	5.0	5.0	1.75	1.50
U	BMWW-t	MT20	6.0	6.0	2.00	2.00
V	BMWW-t	MT20	4.0	10.0	1.75	2.75
W	BMV1+t	MT20	6.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	4958	0	4958	0	0	5-8	5-8
M	4958	0	4958	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	3666	2124 / 0	621 / 0	0 / 0	0 / 0	920 / 0	0 / 0
M	3666	2124 / 0	621 / 0	0 / 0	0 / 0	920 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 55	-124.4	-124.4	0.19 (1)	10.00	V- C	-999 / 0		0.28 (1)
B- C	-4935 / 0	-124.4	-124.4	0.69 (1)	2.55	C- U	0 / 4715		0.83 (1)
C- D	-7165 / 0	-230.7	-230.7	0.76 (1)	2.62	U- D	-2946 / 0		0.83 (1)
D- E	-9085 / 0	-230.7	-230.7	0.96 (1)	2.06	D- T	0 / 2667		0.66 (1)
E- F	-9728 / 0	-230.7	-230.7	0.90 (1)	2.11	T- E	-1550 / 0		0.44 (1)
F- G	-9728 / 0	-230.7	-230.7	0.90 (1)	2.11	E- R	0 / 894		0.22 (1)
G- H	-9728 / 0	-230.7	-230.7	0.90 (1)	2.11	R- F	-972 / 0		0.28 (1)
H- I	-9085 / 0	-230.7	-230.7	0.96 (1)	2.06	R- H	0 / 894		0.22 (1)
I- J	-7165 / 0	-230.7	-230.7	0.76 (1)	2.62	Q- H	-1550 / 0		0.44 (1)
J- K	-4935 / 0	-124.4	-124.4	0.69 (1)	2.55	Q- I	0 / 2667		0.66 (1)
K- L	0 / 55	-124.4	-124.4	0.19 (1)	10.00	O- I	-2946 / 0		0.83 (1)
W- B	-4854 / 0	0.0	0.0	0.36 (1)	4.84	O- J	0 / 4715		0.83 (1)
M- K	-4854 / 0	0.0	0.0	0.36 (1)	4.84	N- J	-999 / 0		0.28 (1)
						B- V	0 / 4012		0.99 (1)
W- V	0 / 0	-72.8	-72.8	0.05 (3)	10.00	N- K	0 / 4012		0.99 (1)
V- U	0 / 3752	-72.8	-72.8	0.23 (1)	10.00				
U- T	0 / 7165	-72.8	-72.8	0.45 (1)	10.00				
T- S	0 / 9085	-72.8	-72.8	0.58 (1)	10.00				
S- R	0 / 9085	-72.8	-72.8	0.58 (1)	10.00				
R- Q	0 / 9085	-72.8	-72.8	0.58 (1)	10.00				
Q- P	0 / 7165	-72.8	-72.8	0.45 (1)	10.00				
P- O	0 / 7165	-72.8	-72.8	0.45 (1)	10.00				
O- N	0 / 3752	-72.8	-72.8	0.23 (1)	10.00				
N- M	0 / 0	-72.8	-72.8	0.05 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-4-7	-301	-301	---	FRONT	VERT	TOTAL	---	C1
J	28-6-9	-301	-301	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Town of East Gwillimbury
Building Standards Branch BCIN #16487

These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

RECEIVED
2021-02-19
TOWN OF EAST GWILLIMBURY
Building Standards Branch

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 34.8	PSF	
	DL = 8.0	PSF	
BOT CH.	LL = 10.5	PSF	
	DL = 7.3	PSF	
TOTAL LOAD	= 60.6	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 3-3-13
END SETBACK = 5-10-8
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 948 (0.40")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 569 (0.67")

CSI: TC=0.96/1.00 (D-E:1) , BC=0.58/1.00 (R-T:1) , WB=0.99/1.00 (K-N:1) , SSI=0.50/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873
MT18HS	586	403	2455	1382	3163	3004

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.96 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE1220-107	G03	1	2	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	
				TRUSS DESC.	

Version 8.420 S Oct 9 2020 MiTek Industries, Inc. Thu Dec 17 14:46:15 2020 Page 1
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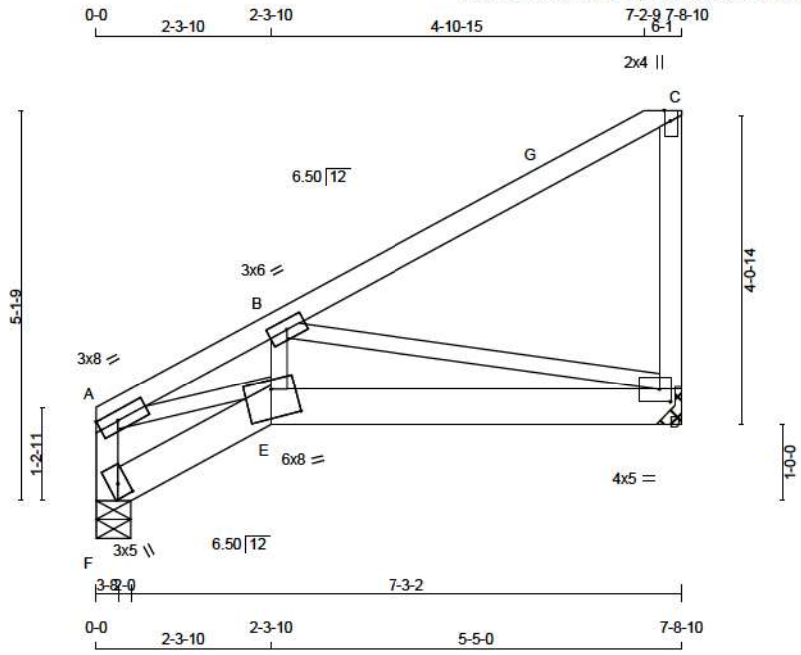

Scale = 1:30.3

TOTAL WEIGHT = 2 X 36 = 72 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS#ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	SIDE(24.6)
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- E	2	12	TOP
E- D	2	12	SIDE(139.8)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	8.0		Edge
B	TMWW-t	MT20	3.0	6.0		
C	TMV+p	MT20	2.0	4.0	Edge	
D	BMVW1-t	MT20	4.0	5.0	2.00	1.75
E	BBWW-m	MT20	6.0	8.0	4.50	3.75
F	BVM1+I	MT20	3.0	5.0	2.25	1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1858	0	1858	0	0	5-8	1-8
D	1761	0	1761	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1382	743 / 0	224 / 0	0 / 0	0 / 0	415 / 0	0 / 0
D	1307	733 / 0	221 / 0	0 / 0	0 / 0	353 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1813 / 0	0.0	0.0	0.10 (1)	7.81		A- E	0 / 3041	0.38 (1)
A- B	-3386 / 0	-461.7	-441.6	0.28 (1)	4.76		E- B	0 / 1168	0.14 (1)
B- G	-35 / 0	-154.4	-124.4	0.24 (1)	6.25		B- D	-2988 / 0	0.72 (1)
G- C	-35 / 0	-124.4	-124.4	0.24 (1)	6.25				
D- C	-283 / 0	0.0	0.0	0.04 (1)	7.81				
F- E	0 / 0	-39.2	-39.2	0.01 (3)	10.00				
E- D	0 / 2922	-318.9	-318.9	0.62 (1)	10.00				

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.3 PSF
TOTAL LOAD = 60.6 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 9-4-4
END DISTANCE = 2-3-10
END SPAN CARRIED = 9-4-4
END WALL WIDTH = 4-0
APPLIED TO TOP EDGE OF TOP CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 2-3-10
START SPAN CARRIED = 8-10-0
END DISTANCE = 7-8-10
END SPAN CARRIED = 8-10-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 803 (0.12")

CSI: TC=0.28/1.00 (A-B:1) , BC=0.62/1.00 (D-E:1) , WB=0.72/1.00 (B-D:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Town of East Gwillimbury
Building Standards Branch BCIN #16487

These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

RECEIVED
2021-02-19
TOWN OF EAST GWILLIMBURY
Building Standards Branch

CONTINUED ON PAGE 2

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DWG NO.
NE1220-107	G04	1	2	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	
				TRUSS DESC.	**Page 5 of 31**

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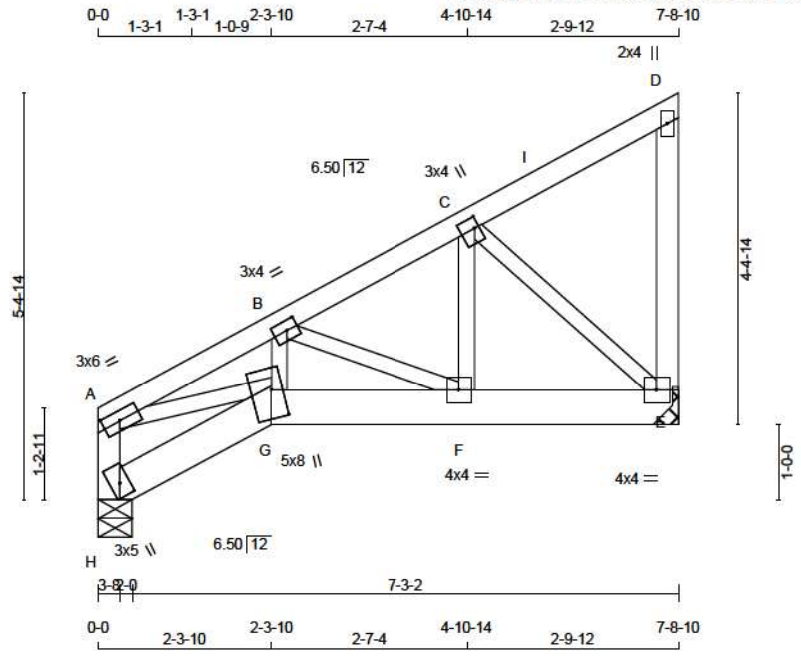

Scale = 1:30.6

TOTAL WEIGHT = 2 X 38 = 77 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS#ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
H- A	1	12	TOP
A- D	1	12	SIDE(24.6)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- G	2	12	TOP
G- E	2	12	SIDE(139.8)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	6.0	1.50	2.75
B	TMWW-t	MT20	3.0	4.0	1.50	1.75
C	TMWW+t	MT20	3.0	4.0	1.75	0.75
D	TMW+w	MT20	2.0	4.0		
E	BMWW1-t	MT20	4.0	4.0		
F	BMWW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1824	0	1824	0	0	5-8	1-8
E	1722	0	1722	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1358	729 / 0	220 / 0	0 / 0	0 / 0	409 / 0	0 / 0
E	1278	714 / 0	217 / 0	0 / 0	0 / 0	347 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- A	-1779 / 0	0.0	0.0	0.10 (1)	7.81		A- G	0 / 2766	0.34 (1)
A- B	-3119 / 0	-461.7	-441.6	0.25 (1)	4.97		G- B	0 / 647	0.08 (1)
B- C	-1493 / 0	-154.4	-133.5	0.08 (1)	6.25		C- E	-1786 / 0	0.22 (1)
C- I	-17 / 0	-133.5	-124.4	0.06 (1)	6.25		F- C	0 / 1563	0.19 (1)
I- D	-17 / 0	-124.4	-124.4	0.06 (1)	6.25		E- D	-132 / 0	0.01 (1)
							B- F	-1525 / 0	0.15 (1)
H- G	0 / 0	-39.2	-39.2	0.01 (3)	10.00				
G- F	0 / 2696	-318.9	-318.9	0.25 (1)	10.00				
F- E	0 / 1308	-318.9	-318.9	0.16 (1)	10.00				

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.3 PSF
TOTAL LOAD = 60.6 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 9-4-4
END DISTANCE = 2-3-10
END SPAN CARRIED = 9-4-4
END WALL WIDTH = 4-0
APPLIED TO TOP EDGE OF TOP CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 2-3-10
START SPAN CARRIED = 8-10-0
END DISTANCE = 7-8-10
END SPAN CARRIED = 8-10-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.25/1.00 (A-B:1) , BC=0.25/1.00 (F-G:1) , WB=0.34/1.00 (A-G:1) , SSI=0.22/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

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RECEIVED
2021-02-19
TOWN OF EAST GWILLIMBURY
Building Standards Branch

Town of East Gwillimbury
Building Standards Branch BCIN #16487

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Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

KOTT

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
G	BBWW+m	MT20	5.0	8.0	4.50	1.75
H	BVM1+I	MT20	3.0	5.0	2.25	1.50

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.46 (G) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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Building Standards Branch BCIN #16487

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Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE1220-107	G05	1	2	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	
				TRUSS DESC.	

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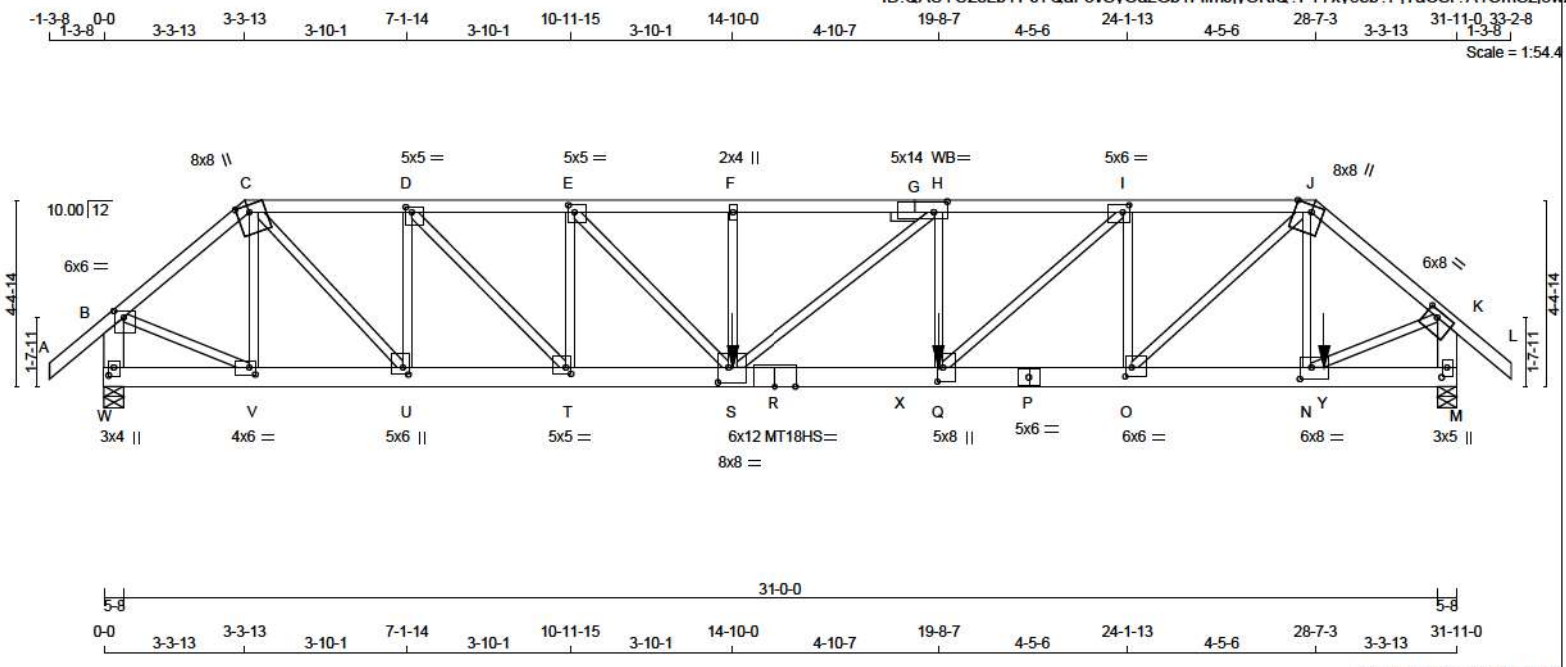

TOTAL WEIGHT = 2 X 158 = 315 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	2100F 1.8E	SPF
G - J	2x4	DRY	2100F 1.8E	SPF
J - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - R	2x6	DRY	2100F 1.8E	SPF
R - P	2x6	DRY	2100F 1.8E	SPF
P - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- G	1	12	TOP
G- J	1	12	TOP
J- L	1	12	TOP
W- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- R	2	12	SIDE(107.6)
R- P	2	12	SIDE(215.3)
P- M	2	12	SIDE(107.6)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
W	5695	0	5695	0	0	5-8	3-1
M	8111	0	8111	0	0	5-8	5-4

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	4209	2445 / 0	709 / 0	0 / 0	0 / 0	1055 / 0	0 / 0
M	5998	3473 / 0	1019 / 0	0 / 0	0 / 0	1506 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM TO	LC	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX FR-TO CSI (LC)
A- B	0 / 55	-124.4 -124.4	0.09 (1)	10.00		V- C	-1319 / 0	0.19 (1)
B- C	-5831 / 0	-124.4 -124.4	0.27 (1)	3.83		C- U	0 / 6375	0.79 (1)
C- D	-8793 / 0	-124.4 -124.4	0.19 (1)	4.03		U- D	-4515 / 0	0.64 (1)
D- E	-12586 / 0	-124.4 -124.4	0.30 (1)	3.35		D- T	0 / 5499	0.68 (1)
E- F	-15912 / 0	-124.4 -124.4	0.42 (1)	2.91		T- E	-3945 / 0	0.56 (1)
F- G	-15912 / 0	-124.4 -124.4	0.55 (1)	2.84		E- S	0 / 4823	0.60 (1)
G- H	-15912 / 0	-124.4 -124.4	0.55 (1)	2.84		S- F	-539 / 0	0.08 (1)
H- I	-15377 / 0	-124.4 -124.4	0.47 (1)	2.94		S- H	0 / 695	0.09 (1)
I- J	-10996 / 0	-124.4 -124.4	0.30 (1)	3.58		Q- H	-1022 / 0	0.14 (1)
J- K	-7964 / 0	-124.4 -124.4	0.39 (1)	3.21		Q- I	0 / 5912	0.73 (1)
K- L	0 / 55	-124.4 -124.4	0.09 (1)	10.00		O- I	-4574 / 0	0.65 (1)
W- B	-5667 / 0	0.0 0.0	0.21 (1)	6.18		O- J	0 / 6608	0.82 (1)
M- K	-7599 / 0	0.0 0.0	0.28 (1)	5.45		N- J	0 / 233	0.03 (3)
						B- V	0 / 4741	0.59 (1)
W- V	0 / 0	-39.2 -39.2	0.03 (1)	10.00		N- K	0 / 6475	0.80 (1)
V- U	0 / 4428	-39.2 -39.2	0.15 (1)	10.00				
U- T	0 / 8793	-39.2 -39.2	0.27 (1)	10.00				
T- S	0 / 12586	-39.2 -39.2	0.37 (1)	10.00				
S- R	0 / 15377	-254.5 -254.5	0.63 (1)	10.00				
R- X	0 / 15377	-469.8 -469.8	0.63 (1)	10.00				
X- Q	0 / 15377	-254.5 -254.5	0.63 (1)	10.00				
Q- P	0 / 10996	-39.2 -39.2	0.34 (1)	10.00				
P- O	0 / 10996	-39.2 -39.2	0.34 (1)	10.00				
O- N	0 / 6125	-39.2 -39.2	0.26 (1)	10.00				
N- Y	0 / 0	-39.2 -39.2	0.10 (1)	10.00				
Y- M	0 / 0	-254.5 -254.5	0.10 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	19-8-7	-1765	-1765	---	FRONT	VERT	TOTAL	---	C1
S	14-10-0	-2343	-2343	---	FRONT	VERT	TOTAL	---	C1
Y	28-9-9	-1765	-1765	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.3 PSF
TOTAL LOAD = 60.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 14-10-0
START SPAN CARRIED = 7-8-10
END DISTANCE = 19-8-7
END SPAN CARRIED = 7-8-10
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 28-9-9
START SPAN CARRIED = 7-8-10
END DISTANCE = 31-11-0
END SPAN CARRIED = 7-8-10
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 15-8-8
START SPAN CARRIED = 7-8-10
END DISTANCE = 18-10-0
END SPAN CARRIED = 7-8-10
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.35")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 663 (0.58")

CSI: TC=0.55/1.00 (F-H:1) , BC=0.63/1.00 (Q-S:1) , WB=0.82/1.00 (J-O:1) , SSI=0.68/1.00 (M-N:1)

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

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RECEIVED
2021-02-19
TOWN OF EAST GWILLIMBURY
Building Standards Branch

Town of East Gwillimbury
Building Standards Branch BCIN #16487

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Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

KOTT

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	6.0	1.75	2.75
C	TTWW+m	MT20	8.0	8.0	2.00	3.50
D	TMWW-t	MT20	5.0	5.0	1.50	1.75
E	TMWW-t	MT20	5.0	5.0	2.00	1.75
F	TMW+w	MT20	2.0	4.0		
G						
G	TSWW-l	MT20	5.0	14.0	3.00	3.75
H						
I	TMWW-t	MT20	5.0	6.0	2.00	1.75
J	TTWW+m	MT20	8.0	8.0	Edge	4.75
K	TMVW-t	MT20	6.0	8.0	1.75	3.25
M	BMV1+p	MT20	3.0	5.0	2.75	1.50
N	BMWW-t	MT20	6.0	8.0	3.25	3.25
O	BMWW-t	MT20	6.0	6.0	2.50	1.75
P	BS-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	5.0	8.0	4.00	1.50
R	BS-t	MT18HS	6.0	12.0		
S	BMWWW-t	MT20	8.0	8.0	4.25	3.00
T	BMWW-t	MT20	5.0	5.0	1.75	1.50
U	BMWW+t	MT20	5.0	6.0	2.00	1.50
V	BMWW-t	MT20	4.0	6.0	2.00	1.75
W	BMV1+p	MT20	3.0	4.0	2.25	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873
MT18HS	586	403	2455	1382	3163	3004

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (T) (INPUT = 0.90)
JSI METAL= 0.96 (P) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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Town of East Gwillimbury
Building Standards Branch BCIN #16487

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Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DWG NO.
NE1220-107	GES01	1	1	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	
				TRUSS DESC.	**Page 9 of 31**

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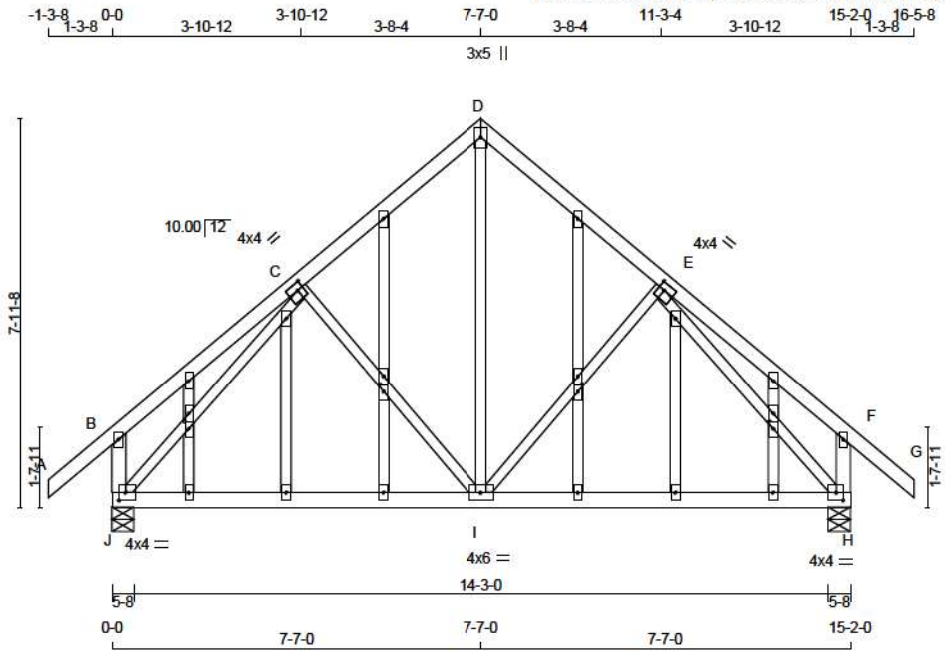
ID:QAUYU2sLbYFcYQaF3vSvOazGb1I-mvKhxHPvWk76AHW9qL6Exw0p7sBqvVPwhdSiSDy8



TOTAL WEIGHT = 90 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0	1.75	1.50
D	TTW+p	MT20	3.0	5.0		
E	TMWW-t	MT20	4.0	4.0	1.75	1.50
F	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	4.0	4.0	2.00	1.75
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0	2.00	1.75
K	NP+w	MT20	2.0	4.0	2.00	1.00
K, L, M, N, O, P, Q, R, S, T, U, V, W, X, Y, Z						
K	NP+w	MT20	2.0	4.0		
P	NP+w	MT20	2.0	4.0	1.75	1.00
U	NP+w	MT20	2.0	4.0	2.00	1.00
Z	NP+w	MT20	2.0	4.0	1.75	1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER [M][F]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1414	0	1414	0	0	5-8	1-9
H	1414	0	1414	0	0	5-8	1-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1038	624 / 0	159 / 0	0 / 0	0 / 0	254 / 0	0 / 0
H	1038	624 / 0	159 / 0	0 / 0	0 / 0	254 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 55	-124.4	-124.4	0.17 (1)	10.00	I- D	0 / 720	0.16 (1)	
B- C	0 / 35	-124.4	-124.4	0.29 (1)	10.00	I- E	-232 / 11	0.14 (1)	
C- D	-884 / 0	-124.4	-124.4	0.23 (1)	6.25	C- I	-232 / 11	0.14 (1)	
D- E	-884 / 0	-124.4	-124.4	0.23 (1)	6.25	J- C	-1244 / 0	0.70 (1)	
E- F	0 / 35	-124.4	-124.4	0.29 (1)	10.00	E- H	-1244 / 0	0.70 (1)	
F- G	0 / 55	-124.4	-124.4	0.17 (1)	10.00				
J- B	-352 / 0	0.0	0.0	0.04 (1)	7.81				
H- F	-352 / 0	0.0	0.0	0.04 (1)	7.81				
J- I	0 / 804	-39.2	-39.2	0.59 (3)	10.00				
I- H	0 / 804	-39.2	-39.2	0.59 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	60.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.59/1.00 (H-I:3) , WB=0.70/1.00 (C-J:1) , SSI=0.20/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (H) (INPUT = 0.90)
JSI METAL= 0.42 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

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Building Standards Branch

Town of East Gwillimbury
Building Standards Branch BCIN #16487

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Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE1220-107	J01	6	1	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	
				TRUSS DESC.	

Version 8.420 S Oct 9 2020 MiTek Industries, Inc. Thu Dec 17 14:46:17 2020 Page 1
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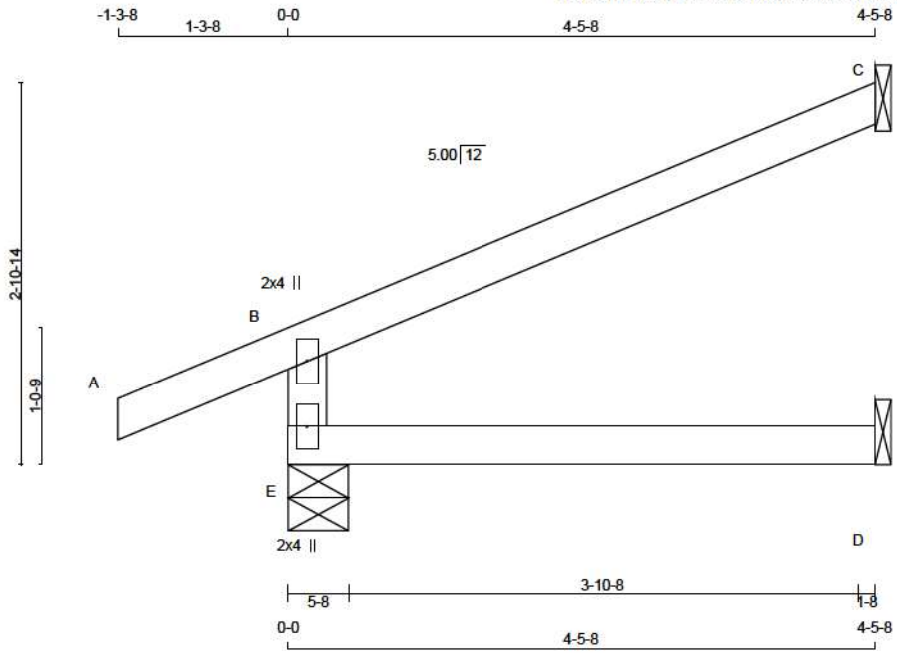

TOTAL WEIGHT = 6 X 13 = 78 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	616	0	616	0	0	5-8	1-8
C	208	0	208	0	0	1-8	1-8
D	73	0	92	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

	1ST LCASE	MAX /MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	446	288 / 0	55 / 0	0 / 0	0 / 0	104 / 0	0 / 0
C	143	116 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
D	66	0 / 0	39 / 0	0 / 0	0 / 0	27 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. MEMB. UNBRAC LENGTH	MAX. FACTORED FORCE (LBS) FR-TO	MAX CSI (LC)	
E- B	-514 / 0	0.0	0.0	0.13 (3)	7.81			
A- B	0 / 32	-124.4	-124.4	0.16 (1)	10.00			
B- C	-27 / 0	-124.4	-124.4	0.42 (1)	6.25			
E- D	0 / 0	-39.2	-39.2	0.13 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.3 PSF
TOTAL LOAD = 60.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.42/1.00 (B-C:1) , BC=0.13/1.00 (D-E:3) , WB=0.00/1.00 (n/a:0) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

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Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DWG NO.
NE1220-107	J02	15	1	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	

TRUSS DESC.

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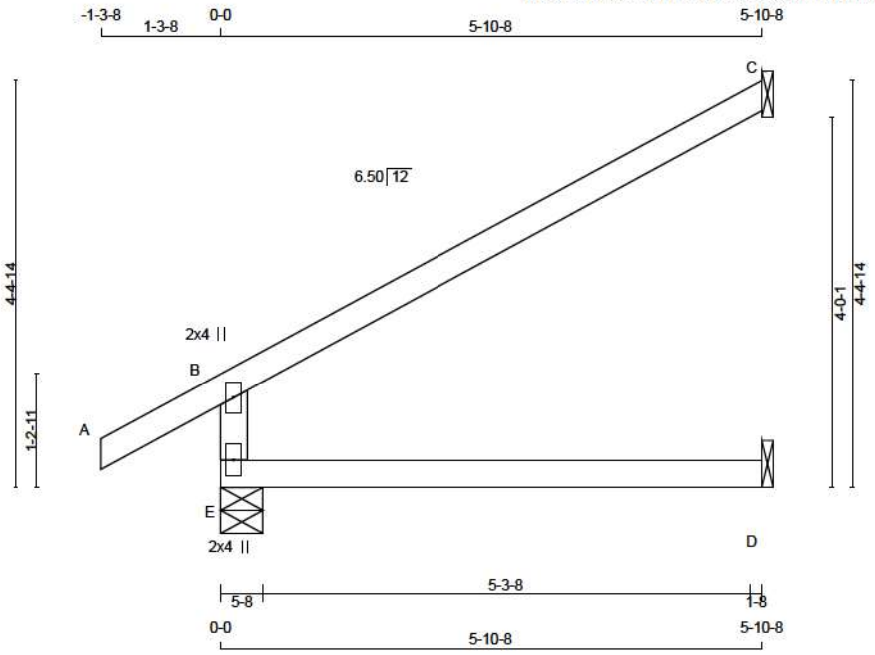

TOTAL WEIGHT = 15 X 17 = 256 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER [M]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	761	0	761	0	0	5-8	1-8
C	274	0	274	0	0	1-8	1-8
D	96	0	121	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	553	350 / 0	72 / 0	0 / 0	0 / 0	131 / 0	0 / 0
C	189	153 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	87	0 / 0	51 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-626 / 0	0.0	0.0	0.22 (3)		7.81			
A- B	0 / 40	-124.4	-124.4	0.16 (1)		10.00			
B- C	-43 / 0	-124.4	-124.4	0.73 (1)		6.25			
E- D	0 / 0	-39.2	-39.2	0.23 (3)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.3 PSF
TOTAL LOAD = 60.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 915 (0.08")

CSI: TC=0.73/1.00 (B-C:1) , BC=0.23/1.00 (D-E:3) , WB=0.00/1.00 (n/a:0) , SSI=0.31/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (B) (INPUT = 0.90)
JSI METAL= 0.28 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
2021-02-19
TOWN OF EAST GWILLIMBURY
Building Standards Branch

Town of East Gwillimbury
Building Standards Branch BCIN #16487

These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE1220-107	T01	1	1	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	
				TRUSS DESC.	

Version 8.420 S Oct 9 2020 MiTek Industries, Inc. Thu Dec 17 14:46:19 2020 Page 1
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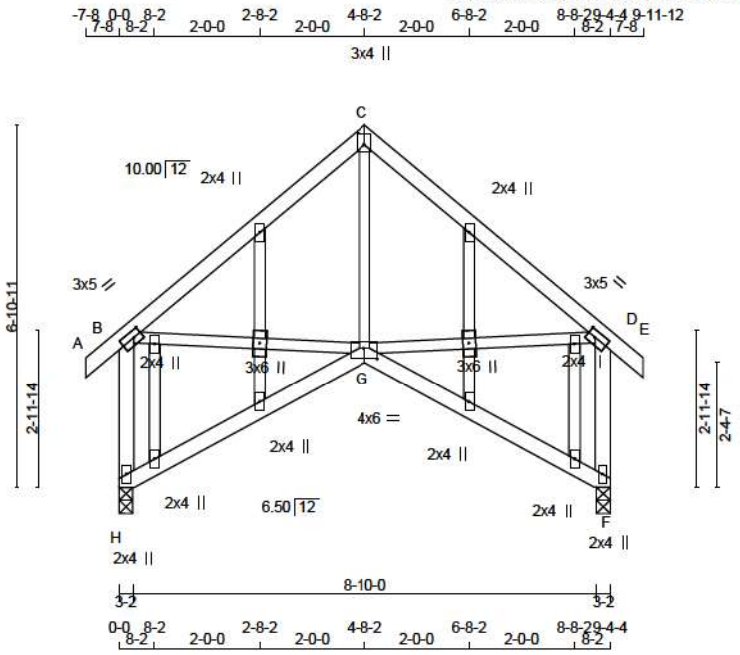

TOTAL WEIGHT = 54 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	1.75
C	TTW+p	MT20	3.0	4.0	2.50	1.50
D	TMVW-t	MT20	3.0	5.0	1.50	1.75
F	BMV1+p	MT20	2.0	4.0		
G	BBWWW-p	MT20	4.0	6.0	2.50	3.00
H	BMV1+p	MT20	2.0	4.0		
I, I, P, P						
I	WMWW+t	MT20	3.0	6.0		
J, K, L, M, N, O, Q, R						
J	NP+w	MT20	2.0	4.0		
P	WMWW+t	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER [M][F]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	855	0	855	0	0	3-2	1-8
F	855	0	855	0	0	3-2	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	628	376 / 0	98 / 0	0 / 0	0 / 0	155 / 0	0 / 0
F	628	376 / 0	98 / 0	0 / 0	0 / 0	155 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 29	-124.4	-124.4	0.05 (1)	10.00	G- C	0 / 212	0.05 (3)
B- C	-560 / 0	-124.4	-124.4	0.35 (1)	6.25	B- G	0 / 431	0.10 (1)
C- D	-560 / 0	-124.4	-124.4	0.35 (1)	6.25	G- D	0 / 431	0.10 (1)
D- E	0 / 29	-124.4	-124.4	0.05 (1)	10.00			
H- B	-763 / 0	0.0	0.0	0.13 (1)	7.81			
F- D	-763 / 0	0.0	0.0	0.13 (1)	7.81			
H- G	0 / 0	-39.2	-39.2	0.21 (3)	10.00			
G- F	0 / 0	-39.2	-39.2	0.21 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	60.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.35/1.00 (C-D:1) , BC=0.21/1.00 (F-G:3) , WB=0.10/1.00 (B-G:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (D) (INPUT = 0.90)
JSI METAL= 0.34 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

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Town of East Gwillimbury
Building Standards Branch BCIN #16487

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Sewage System			
Zoning			

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DWG NO.
NE1220-107	T02	1	1	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	
				TRUSS DESC.	**Page 13 of 31**

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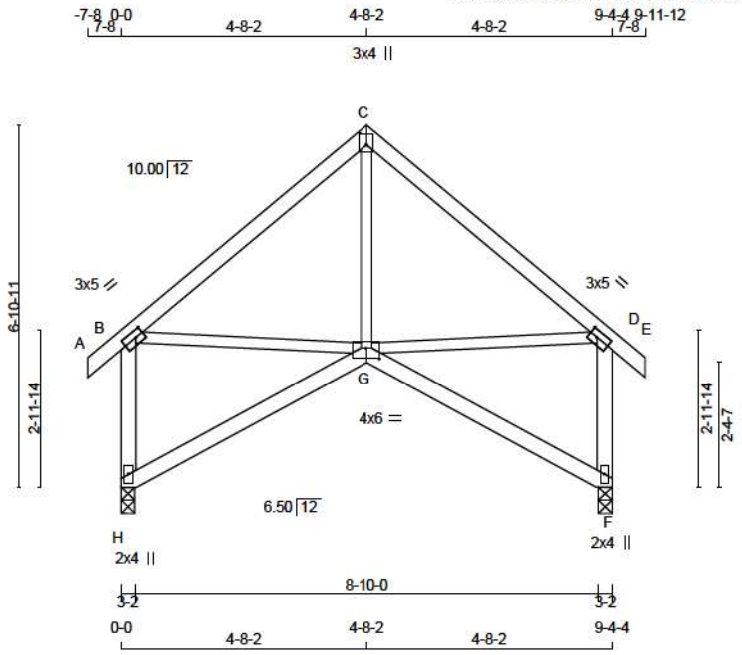

TOTAL WEIGHT = 45 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	1.75
C	TTW+p	MT20	3.0	4.0	2.50	1.50
D	TMVW-t	MT20	3.0	5.0	1.50	1.75
F	BMV1+p	MT20	2.0	4.0		
G	BBWWW-p	MT20	4.0	6.0	2.50	3.00
H	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	855	0	855	0	0	3-2	1-8
F	855	0	855	0	0	3-2	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	628	376 / 0	98 / 0	0 / 0	0 / 0	155 / 0	0 / 0
F	628	376 / 0	98 / 0	0 / 0	0 / 0	155 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO									
A- B	0 / 29	-124.4	-124.4	0.05 (1)	10.00		G- C	0 / 212	0.05 (3)
B- C	-560 / 0	-124.4	-124.4	0.35 (1)	6.25		B- G	0 / 431	0.10 (1)
C- D	-560 / 0	-124.4	-124.4	0.35 (1)	6.25		G- D	0 / 431	0.10 (1)
D- E	0 / 29	-124.4	-124.4	0.05 (1)	10.00				
H- B	-763 / 0	0.0	0.0	0.13 (1)	7.81				
F- D	-763 / 0	0.0	0.0	0.13 (1)	7.81				
H- G	0 / 0	-39.2	-39.2	0.21 (3)	10.00				
G- F	0 / 0	-39.2	-39.2	0.21 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	= 34.8	PSF
		DL	= 8.0	PSF
BOT	CH.	LL	= 10.5	PSF
		DL	= 7.3	PSF
TOTAL LOAD			= 60.6	PSF

SPACING = 24.0 IN. C/C

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- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.35/1.00 (C-D:1) , BC=0.21/1.00 (F-G:3) , WB=0.10/1.00 (B-G:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (D) (INPUT = 0.90)
JSI METAL= 0.34 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

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Building Standards Branch

Town of East Gwillimbury
Building Standards Branch BCIN #16487

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Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRAWG NO.
NE1220-107	T03	1	1	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	
				TRUSS DESC.	**Page 14 of 31**

Version 8.420 S Oct 9 2020 MiTek Industries, Inc. Thu Dec 17 14:46:20 2020 Page 1
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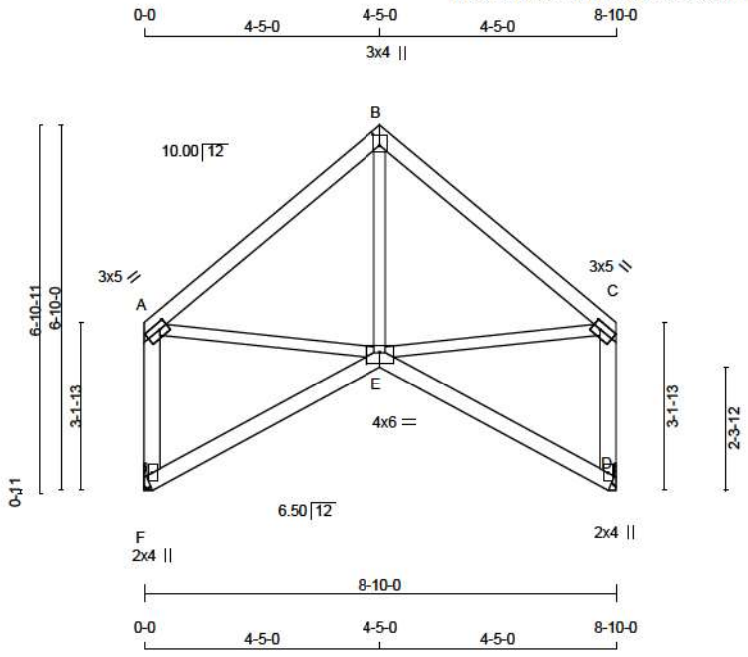

TOTAL WEIGHT = 41 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	1.75
B	TTW+p	MT20	3.0	4.0	2.50	1.50
C	TMVW-t	MT20	3.0	5.0	1.50	1.75
D	BMV1+p	MT20	2.0	4.0		
E	BBWWW-p	MT20	4.0	6.0	2.50	3.00
F	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER [M][F]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	723	0	723	0	0	MECHANICAL	
D	723	0	723	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-12, JOINT D = 1-12.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	535	307 / 0	93 / 0	0 / 0	0 / 0	135 / 0	0 / 0
D	535	307 / 0	93 / 0	0 / 0	0 / 0	135 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-499 / 0	-124.4	-124.4	0.31 (1)	6.25		E- B	0 / 184	0.05 (3)
B- C	-499 / 0	-124.4	-124.4	0.31 (1)	6.25		A- E	0 / 386	0.09 (1)
F- A	-636 / 0	0.0	0.0	0.11 (1)	7.81		E- C	0 / 386	0.09 (1)
D- C	-636 / 0	0.0	0.0	0.11 (1)	7.81				
F- E	0 / 0	-39.2	-39.2	0.18 (3)	10.00				
E- D	0 / 0	-39.2	-39.2	0.18 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.3 PSF
TOTAL LOAD = 60.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.18/1.00 (D-E:3) , WB=0.09/1.00 (A-E:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (C) (INPUT = 0.90)
JSI METAL= 0.28 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
2021-02-19
TOWN OF EAST GWILLIMBURY
Building Standards Branch

Town of East Gwillimbury
Building Standards Branch BCIN #16487

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Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRAWG NO.
NE1220-107	T04	10	1	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	
				TRUSS DESC.	**Page 15 of 31**

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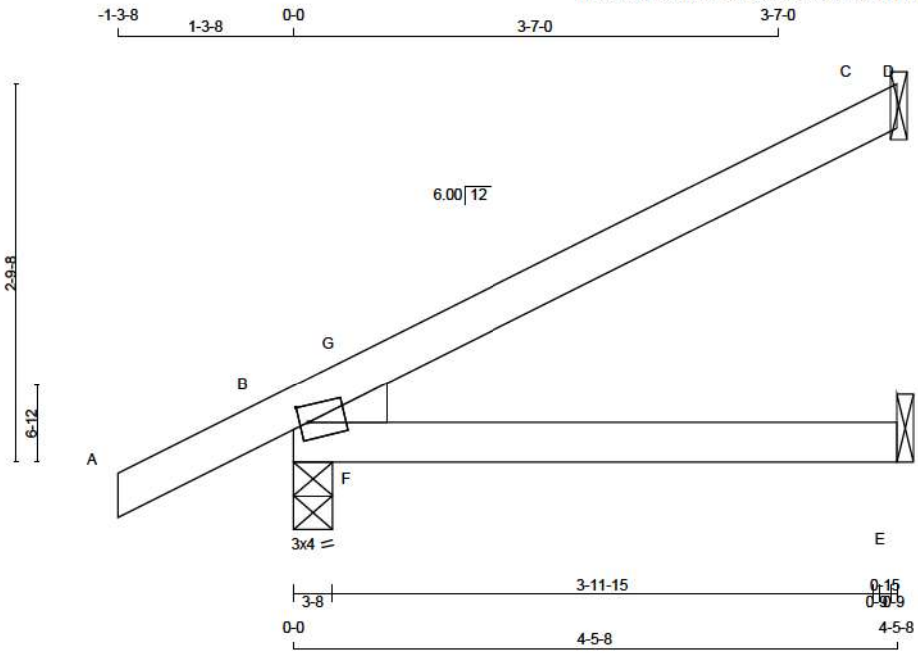

TOTAL WEIGHT = 10 X 13 = 132 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	4.0	1.50	0.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	528	0	528	0	0	3-8	1-8	2x4 L
E	117	0	125	0	0	1-8	1-8	
C	243	0	243	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) E , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	382	246 / 0	47 / 0	0 / 0	0 / 0	89 / 0	0 / 0
E	97	24 / 0	40 / 0	0 / 0	0 / 0	33 / 0	0 / 0
C	171	127 / 0	7 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM TO	LC	MAX. CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 25	-124.4	-124.4	0.16 (1)	10.00	F- G	-256 / 80	0.00 (1)
B- G	-77 / 0	-124.4	-124.4	0.09 (3)	6.25			
G- C	0 / 7	-124.4	-124.4	0.31 (1)	10.00			
C- D	0 / 0	-20.0	-20.0	0.00 (4)	10.00			
B- F	0 / 0	-39.2	-39.2	0.21 (1)	10.00			
F- E	0 / 0	-39.2	-39.2	0.24 (1)	10.00			

[M]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.3 PSF
TOTAL LOAD = 60.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 790 (0.07")

CSI: TC=0.31/1.00 (C-G:1) , BC=0.24/1.00 (E-F:1) , WB=0.00/1.00 (F-G:1) , SSI=0.24/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
2021-02-19
TOWN OF EAST GWILLIMBURY
Building Standards Branch

Town of East Gwillimbury
Building Standards Branch BCIN #16487

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Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRAWG NO.
NE1220-107	T05	1	1	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	
				TRUSS DESC.	

Version 8.420 S Oct 9 2020 MiTek Industries, Inc. Thu Dec 17 14:46:21 2020 Page 1
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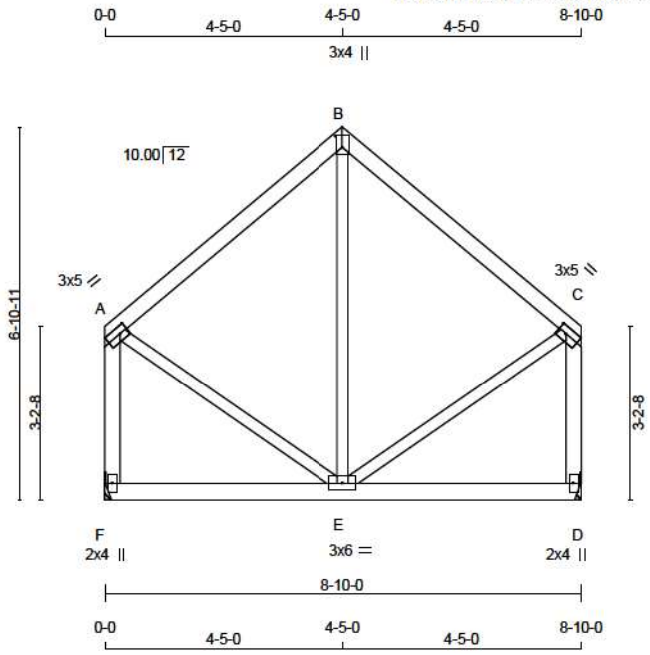

TOTAL WEIGHT = 43 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	1.75
B	TTW+p	MT20	3.0	4.0	2.50	1.50
C	TMVW-t	MT20	3.0	5.0	1.50	1.75
D	BMV1+p	MT20	2.0	4.0		
E	BMWWW-t	MT20	3.0	6.0		
F	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER [M][F]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	723	0	723	0	0	MECHANICAL	
D	723	0	723	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	535	307 / 0	93 / 0	0 / 0	0 / 0	135 / 0	0 / 0
D	535	307 / 0	93 / 0	0 / 0	0 / 0	135 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B		-335 / 0	-124.4	-124.4	0.31 (1)	6.25	E- B		-121 / 115	0.09 (1)
B- C		-335 / 0	-124.4	-124.4	0.31 (1)	6.25	A- E		0 / 307	0.07 (1)
F- A		-657 / 0	0.0	0.0	0.12 (1)	7.81	E- C		0 / 307	0.07 (1)
D- C		-657 / 0	0.0	0.0	0.12 (1)	7.81				
F- E		0 / 0	-39.2	-39.2	0.17 (3)	10.00				
E- D		0 / 0	-39.2	-39.2	0.17 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.3 PSF
TOTAL LOAD = 60.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.17/1.00 (D-E:3) , WB=0.09/1.00 (B-E:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (C) (INPUT = 0.90)
JSI METAL= 0.15 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
2021-02-19
TOWN OF EAST GWILLIMBURY
Building Standards Branch

Town of East Gwillimbury
Building Standards Branch BCIN #16487

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Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	AWG NO.
NE1220-107	T06	4	1	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	
				TRUSS DESC.	

Version 8.420 S Oct 9 2020 MiTek Industries, Inc. Thu Dec 17 14:46:21 2020 Page 1
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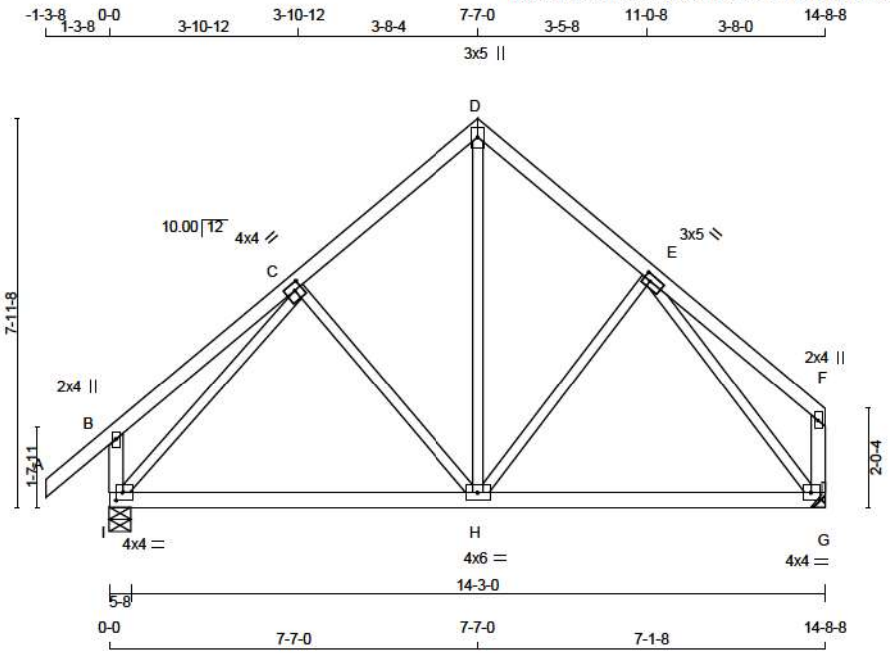

TOTAL WEIGHT = 4 X 68 = 272 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0	1.75	1.75
D	TTW+p	MT20	3.0	5.0		
E	TMWW-t	MT20	3.0	5.0	1.50	1.75
F	TMV+p	MT20	2.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0	2.00	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1376	0	1376	0	0	5-8	1-8
G	1204	0	1204	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1010	608 / 0	154 / 0	0 / 0	0 / 0	247 / 0	0 / 0
G	891	512 / 0	154 / 0	0 / 0	0 / 0	225 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 55	-124.4	-124.4	0.17 (1)	10.00		C- H	-241 / 6	0.14 (1)
B- C	0 / 35	-124.4	-124.4	0.29 (1)	10.00		H- D	0 / 669	0.15 (1)
C- D	-833 / 0	-124.4	-124.4	0.23 (1)	6.25		H- E	-161 / 33	0.10 (1)
D- E	-831 / 0	-124.4	-124.4	0.20 (1)	6.25		I- C	-1192 / 0	0.68 (1)
E- F	0 / 32	-124.4	-124.4	0.25 (1)	10.00		E- G	-1168 / 0	0.66 (1)
I- B	-352 / 0	0.0	0.0	0.04 (1)	7.81				
G- F	-172 / 0	0.0	0.0	0.02 (1)	7.81				
I- H	0 / 771	-39.2	-39.2	0.55 (2)	10.00				
H- G	0 / 711	-39.2	-39.2	0.54 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.3 PSF
TOTAL LOAD = 60.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/ 993 (0.18")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.55/1.00 (H-I:2) , WB=0.68/1.00 (C-I:1) , SSI=0.20/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

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Building Standards Branch

Town of East Gwillimbury
Building Standards Branch BCIN #16487

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Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE1220-107	T07	2	1	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	
				TRUSS DESC.	**Page 18 of 31**

Version 8.420 S Oct 9 2020 MiTek Industries, Inc. Thu Dec 17 14:46:22 2020 Page 1

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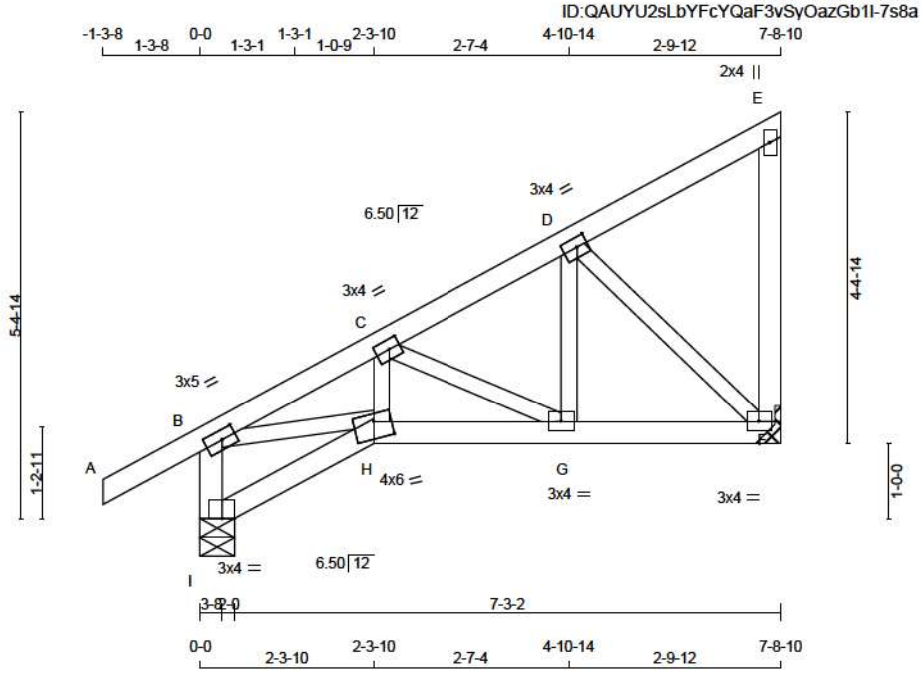

TOTAL WEIGHT = 2 X 35 = 71 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	2.25
C	TMWW-t	MT20	3.0	4.0	1.50	1.75
D	TMWW-t	MT20	3.0	4.0	1.50	1.75
E	TMW+w	MT20	2.0	4.0		
F	BMWW1-t	MT20	3.0	4.0		
G	BMWW-t	MT20	3.0	4.0		
H	BBWW-m	MT20	4.0	6.0	2.75	2.75
I	BVM1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	789	0	789	0	0	5-8	1-8
F	611	0	611	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	575	358 / 0	80 / 0	0 / 0	0 / 0	138 / 0	0 / 0
F	453	259 / 0	80 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	-744 / 0	0.0	0.0	0.08 (1)		7.81	B- H	0 / 884	0.20 (1)
A- B	0 / 40	-124.4	-124.4	0.16 (1)		10.00	H- C	0 / 144	0.03 (2)
B- C	-996 / 0	-124.4	-124.4	0.10 (1)		6.16	D- F	-634 / 0	0.15 (1)
C- D	-513 / 0	-124.4	-124.4	0.09 (1)		6.25	G- D	0 / 320	0.07 (2)
D- E	-18 / 0	-124.4	-124.4	0.09 (1)		6.25	F- E	-127 / 0	0.03 (1)
							C- G	-459 / 0	0.08 (1)
I- H	0 / 0	-39.2	-39.2	0.05 (3)		10.00			
H- G	0 / 872	-39.2	-39.2	0.17 (1)		10.00			
G- F	0 / 459	-39.2	-39.2	0.11 (2)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 34.8	PSF
	DL	= 8.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.3	PSF
TOTAL LOAD		= 60.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.17/1.00 (G-H:1) , WB=0.20/1.00 (B-H:1) , SSI=0.13/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.44 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
2021-02-19
TOWN OF EAST GWILLIMBURY
Building Standards Branch

Town of East Gwillimbury
Building Standards Branch BCIN #16487

These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	AWG NO.
NE1220-107	T08	2	1	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	

TRUSS DESC.

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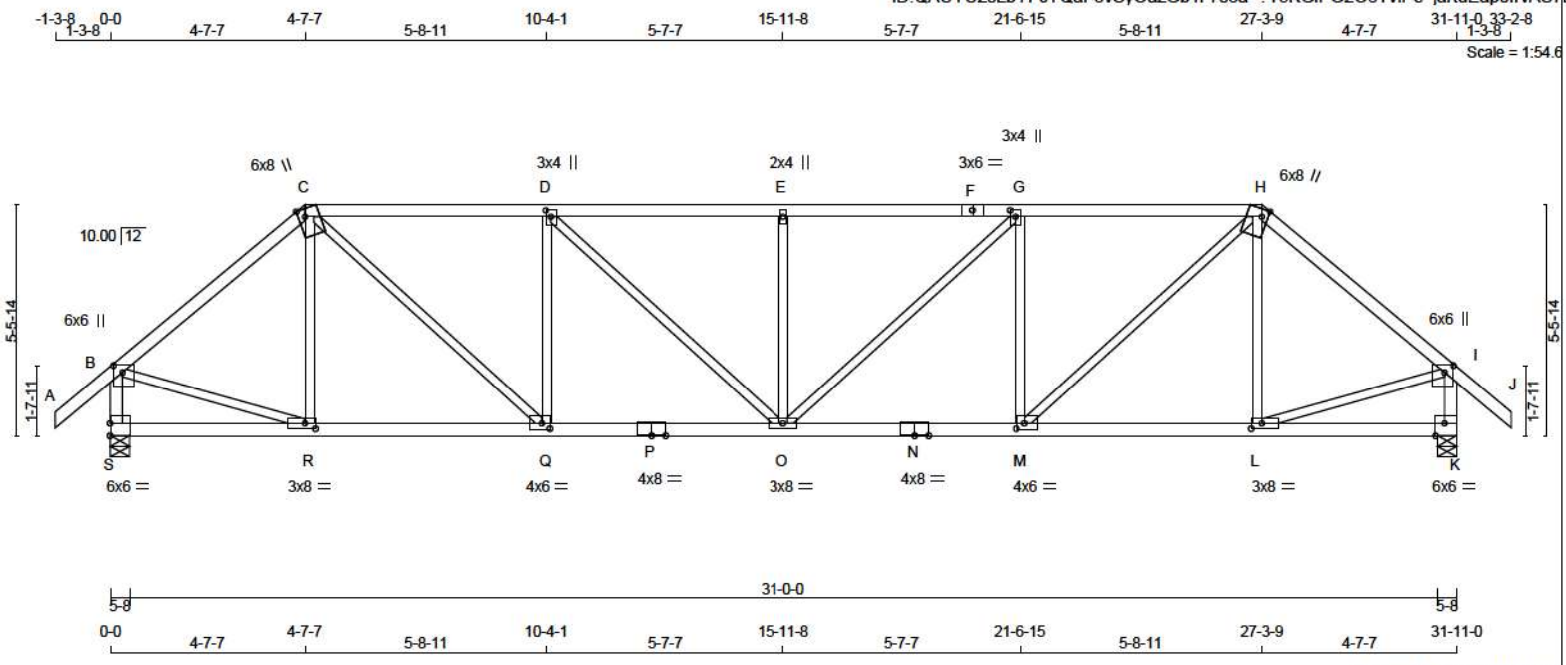


TOTAL WEIGHT = 2 X 134 = 268 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	2100F 1.8E	SPF
F - H	2x4	DRY	2100F 1.8E	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	6.0	2.00	2.50
C	TTWW+m	MT20	6.0	8.0	Edge	1.75
D	TMWW+t	MT20	3.0	4.0	1.75	1.50
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW+t	MT20	3.0	4.0	1.75	1.50
H	TTWW+m	MT20	6.0	8.0	Edge	1.75
I	TMV+p	MT20	6.0	6.0	2.00	2.50
K	BMV1-t	MT20	6.0	6.0	Edge	2.50
L	BMWW-t	MT20	3.0	8.0	1.50	3.00
M	BMWW-t	MT20	4.0	6.0	1.50	2.25
N	BS-t	MT20	4.0	8.0		
O	BMWWW-t	MT20	3.0	8.0		
P	BS-t	MT20	4.0	8.0		
Q	BMWW-t	MT20	4.0	6.0	1.50	2.25
R	BMWW-t	MT20	3.0	8.0	1.50	3.00
S	BMV1-t	MT20	6.0	6.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2784	0	2784	0	0	5-8	5-8
K	2784	0	2784	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2053	1207 / 0	335 / 0	0 / 0	0 / 0	510 / 0	0 / 0
K	2053	1207 / 0	335 / 0	0 / 0	0 / 0	510 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 55	-124.4	-124.4	0.17 (1)	10.00	R- C	-331 / 44		0.15 (1)
B- C	-2639 / 0	-124.4	-124.4	0.67 (1)	3.58	C- Q	0 / 2147		0.48 (1)
C- D	-3608 / 0	-124.4	-124.4	0.51 (1)	4.05	Q- D	-1222 / 0		0.55 (1)
D- E	-4076 / 0	-124.4	-124.4	0.53 (1)	3.82	D- O	0 / 637		0.14 (1)
E- F	-4076 / 0	-124.4	-124.4	0.53 (1)	3.82	O- E	-646 / 0		0.29 (1)
F- G	-4076 / 0	-124.4	-124.4	0.53 (1)	3.82	O- G	0 / 637		0.14 (1)
G- H	-3608 / 0	-124.4	-124.4	0.51 (1)	4.05	M- G	-1222 / 0		0.55 (1)
H- I	-2639 / 0	-124.4	-124.4	0.67 (1)	3.58	M- H	0 / 2147		0.48 (1)
I- J	0 / 55	-124.4	-124.4	0.17 (1)	10.00	L- H	-331 / 44		0.15 (1)
S- B	-2714 / 0	0.0	0.0	0.29 (1)	5.23	B- R	0 / 2098		0.47 (1)
K- I	-2714 / 0	0.0	0.0	0.29 (1)	5.23	L- I	0 / 2098		0.47 (1)
S- R	0 / 0	-39.2	-39.2	0.20 (3)	10.00				
R- Q	0 / 2016	-39.2	-39.2	0.47 (1)	10.00				
Q- P	0 / 3608	-39.2	-39.2	0.70 (1)	10.00				
P- O	0 / 3608	-39.2	-39.2	0.70 (1)	10.00				
O- N	0 / 3608	-39.2	-39.2	0.70 (1)	10.00				
N- M	0 / 3608	-39.2	-39.2	0.70 (1)	10.00				
M- L	0 / 2016	-39.2	-39.2	0.47 (1)	10.00				
L- K	0 / 0	-39.2	-39.2	0.20 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL = 34.8	PSF
		DL = 8.0	PSF
BOT	CH.	LL = 10.5	PSF
		DL = 7.3	PSF
TOTAL	LOAD	= 60.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.67/1.00 (B-C:1) , BC=0.70/1.00 (O-Q:1) , WB=0.55/1.00 (D-Q:1) , SSI=0.33/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

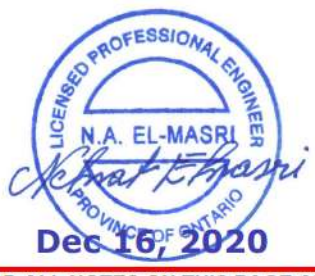
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.87 (P) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Town of East Gwillimbury
Building Standards Branch BCIN #16487

These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	AWG NO.
NE1220-107	T09	2	1	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	**Page 20 of 31**
				TRUSS DESC.	

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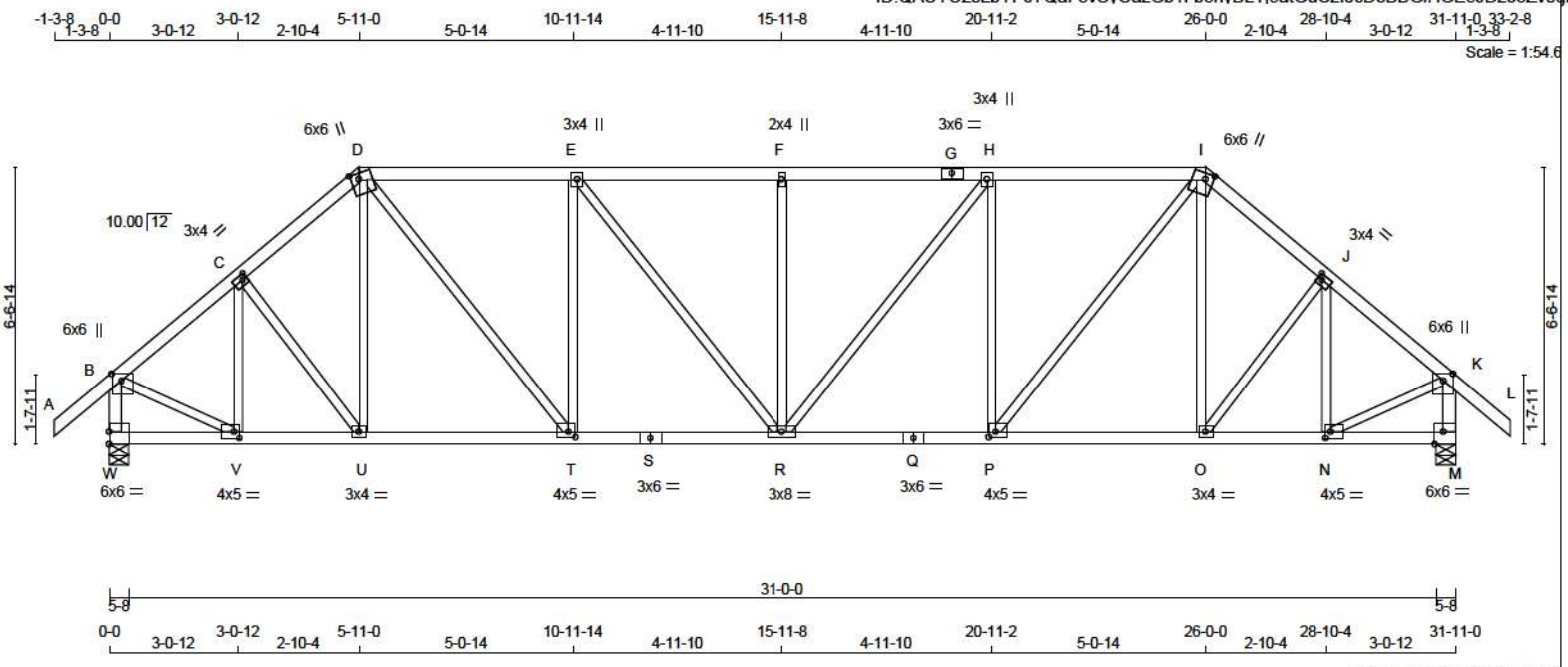

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	6.0	2.00	2.75
C	TMWW-t	MT20	3.0	4.0	1.50	1.25
D	TTWW+m	MT20	6.0	6.0	1.75	2.25
E	TMWW+t	MT20	3.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW+t	MT20	3.0	4.0		
I	TTWW+m	MT20	6.0	6.0	1.75	2.25
J	TMWW-t	MT20	3.0	4.0	1.50	1.25
K	TMVW+p	MT20	6.0	6.0	2.00	2.75
M	BMV1-t	MT20	6.0	6.0	Edge	2.50
N	BMWW-t	MT20	4.0	5.0	1.75	1.50
O	BMWW-t	MT20	3.0	4.0		
P	BMWW-t	MT20	4.0	5.0	1.50	2.00
Q	BS-t	MT20	3.0	6.0		
R	BMWWW-t	MT20	3.0	8.0		
S	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	4.0	5.0	1.50	2.00
U	BMWW-t	MT20	3.0	4.0		
V	BMWW-t	MT20	4.0	5.0	1.75	1.50
W	BMV1-t	MT20	6.0	6.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2784	0	2784	0	0	5-8	5-8
M	2784	0	2784	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2053	1207 / 0	335 / 0	0 / 0	0 / 0	510 / 0	0 / 0
M	2053	1207 / 0	335 / 0	0 / 0	0 / 0	510 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
A- B		0 / 55	-124.4	-124.4	0.17 (1)	10.00		V- C		-711 / 0	0.19 (1)
B- C		-2458 / 0	-124.4	-124.4	0.24 (1)	4.18		C- U		0 / 191	0.04 (1)
C- D		-2670 / 0	-124.4	-124.4	0.25 (1)	4.02		U- D		0 / 130	0.03 (3)
D- E		-3060 / 0	-124.4	-124.4	0.66 (1)	3.33		D- T		0 / 1646	0.37 (1)
E- F		-3359 / 0	-124.4	-124.4	0.69 (1)	3.14		T- E		-1078 / 0	0.76 (1)
F- G		-3359 / 0	-124.4	-124.4	0.69 (1)	3.14		E- R		0 / 482	0.11 (1)
G- H		-3359 / 0	-124.4	-124.4	0.69 (1)	3.14		R- F		-569 / 0	0.40 (1)
H- I		-3060 / 0	-124.4	-124.4	0.66 (1)	3.33		R- H		0 / 482	0.11 (1)
I- J		-2670 / 0	-124.4	-124.4	0.25 (1)	4.02		P- H		-1078 / 0	0.76 (1)
J- K		-2458 / 0	-124.4	-124.4	0.24 (1)	4.18		P- I		0 / 1646	0.37 (1)
K- L		0 / 55	-124.4	-124.4	0.17 (1)	10.00		O- I		0 / 130	0.03 (3)
W- B		-2727 / 0	0.0	0.0	0.29 (1)	5.21		O- J		0 / 191	0.04 (1)
M- K		-2727 / 0	0.0	0.0	0.29 (1)	5.21		N- J		-711 / 0	0.19 (1)
								B- V		0 / 2073	0.47 (1)
W- V		0 / 0	-39.2	-39.2	0.07 (3)	10.00		N- K		0 / 2073	0.47 (1)
V- U		0 / 1907	-39.2	-39.2	0.39 (1)	10.00					
U- T		0 / 2024	-39.2	-39.2	0.43 (1)	10.00					
T- S		0 / 3060	-39.2	-39.2	0.60 (1)	10.00					
S- R		0 / 3060	-39.2	-39.2	0.60 (1)	10.00					
R- Q		0 / 3060	-39.2	-39.2	0.60 (1)	10.00					
Q- P		0 / 3060	-39.2	-39.2	0.60 (1)	10.00					
P- O		0 / 2024	-39.2	-39.2	0.43 (1)	10.00					
O- N		0 / 1907	-39.2	-39.2	0.39 (1)	10.00					
N- M		0 / 0	-39.2	-39.2	0.07 (3)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.3 PSF
TOTAL LOAD = 60.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.69/1.00 (F-H:1) , BC=0.60/1.00 (R-T:1) , WB=0.76/1.00 (E-T:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.97 (S) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
2021-02-19
TOWN OF EAST GWILLIMBURY
Building Standards Branch

Town of East Gwillimbury
Building Standards Branch BCIN #16487

These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	AWG NO.
NE1220-107	T10	2	1	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	
				TRUSS DESC.	**Page 21 of 31**

Version 8.420 S Oct 9 2020 MiTek Industries, Inc. Thu Dec 17 14:46:24 2020 Page 1
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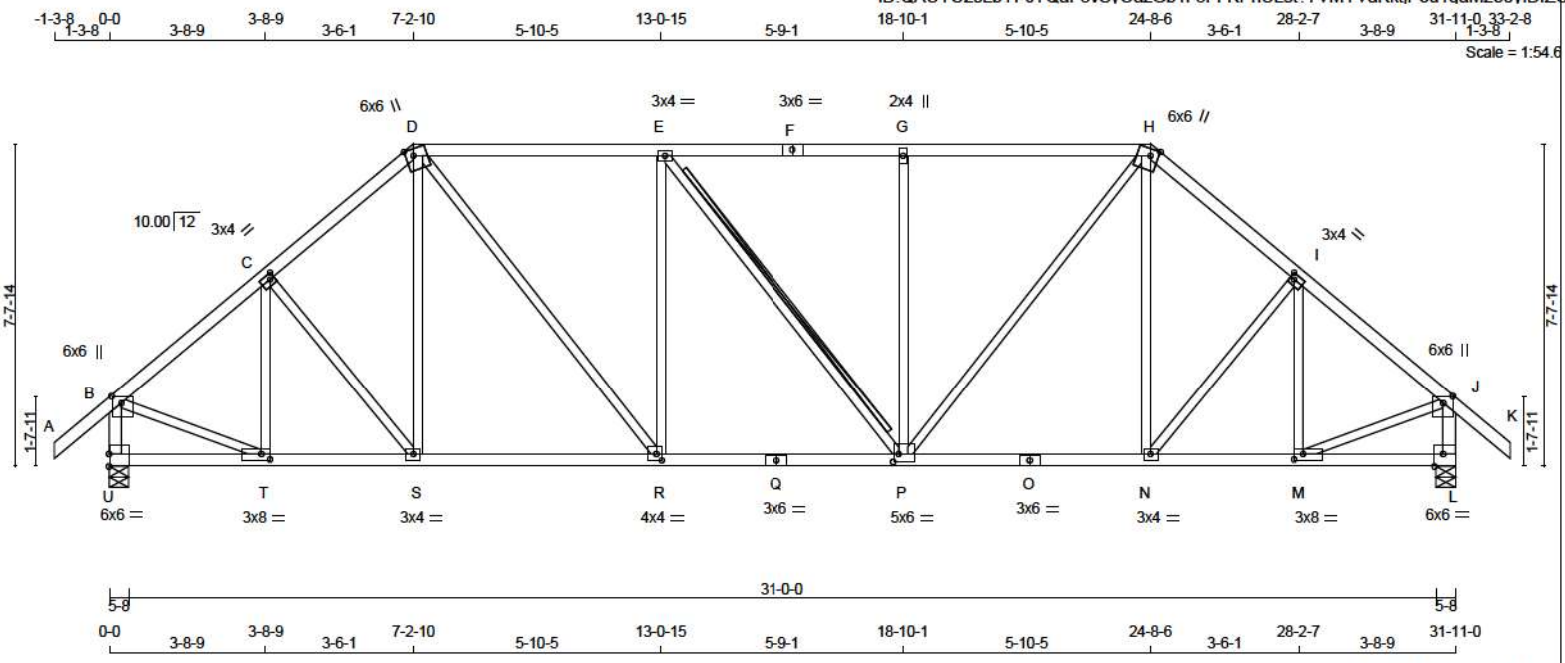

TOTAL WEIGHT = 2 X 151 = 302 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	6.0	2.00	2.75
C	TMWW-t	MT20	3.0	4.0	1.50	1.25
D	TTWW+m	MT20	6.0	6.0	2.00	2.25
E	TMWW-t	MT20	3.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	6.0	2.00	2.25
I	TMWW-t	MT20	3.0	4.0	1.50	1.25
J	TMVW+p	MT20	6.0	6.0	2.00	2.75
L	BMV1-t	MT20	6.0	6.0	Edge	2.50
M	BMWW-t	MT20	3.0	8.0	1.50	2.50
N	BMWW-t	MT20	3.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	5.0	6.0	2.25	1.50
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0	1.75	1.50
S	BMWW-t	MT20	3.0	4.0		
T	BMWW-t	MT20	3.0	8.0	1.50	2.50
U	BMV1-t	MT20	6.0	6.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2784	0	2784	0	0	5-8	5-8
L	2784	0	2784	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2053	1207 / 0	335 / 0	0 / 0	0 / 0	510 / 0	0 / 0
L	2053	1207 / 0	335 / 0	0 / 0	0 / 0	510 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 55	-124.4	-124.4	1	0.17 (1)	10.00
B- C	-2566 / 0	-124.4	-124.4	1	0.37 (1)	3.97
C- D	-2642 / 0	-124.4	-124.4	1	0.37 (1)	3.92
D- E	-2808 / 0	-124.4	-124.4	1	0.82 (1)	3.23
E- F	-2806 / 0	-124.4	-124.4	1	0.81 (1)	3.23
F- G	-2806 / 0	-124.4	-124.4	1	0.81 (1)	3.23
G- H	-2806 / 0	-124.4	-124.4	1	0.81 (1)	3.25
H- I	-2642 / 0	-124.4	-124.4	1	0.37 (1)	3.92
I- J	-2565 / 0	-124.4	-124.4	1	0.37 (1)	3.97
J- K	0 / 55	-124.4	-124.4	1	0.17 (1)	10.00
U- B	-2718 / 0	0.0	0.0	1	0.29 (1)	5.22
L- J	-2718 / 0	0.0	0.0	1	0.29 (1)	5.22
U- T	0 / 0	-39.2	-39.2	1	0.09 (3)	10.00
T- S	0 / 1995	-39.2	-39.2	1	0.44 (1)	10.00
S- R	0 / 2000	-39.2	-39.2	1	0.46 (1)	10.00
R- Q	0 / 2808	-39.2	-39.2	1	0.59 (1)	10.00
Q- P	0 / 2808	-39.2	-39.2	1	0.59 (1)	10.00
P- O	0 / 2001	-39.2	-39.2	1	0.45 (1)	10.00
O- N	0 / 2001	-39.2	-39.2	1	0.45 (1)	10.00
N- M	0 / 1995	-39.2	-39.2	1	0.44 (1)	10.00
M- L	0 / 0	-39.2	-39.2	1	0.09 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-568 / 0	0.19 (1)
C- S	0 / 21	0.01 (3)
S- D	0 / 236	0.06 (3)
D- R	0 / 1295	0.29 (1)
R- E	-781 / 0	0.84 (1)
E- P	-3 / 0	0.00 (1)
P- G	-780 / 0	0.84 (1)
P- H	0 / 1291	0.29 (1)
N- H	0 / 238	0.06 (3)
N- I	0 / 21	0.01 (3)
M- I	-569 / 0	0.19 (1)
B- T	0 / 2115	0.48 (1)
M- J	0 / 2115	0.48 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 34.8	PSF
	DL	= 8.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.3	PSF
TOTAL LOAD		= 60.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.82/1.00 (D-E:1) , BC=0.59/1.00 (P-R:1) , WB=0.84/1.00 (E-R:1) , SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.95 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
2021-02-19
TOWN OF EAST GWILLIMBURY
Building Standards Branch

Town of East Gwillimbury
Building Standards Branch BCIN #16487

These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	AWG NO.
NE1220-107	T11	2	1	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	
				TRUSS DESC.	**Page 22 of 31**

Version 8.420 S Oct 9 2020 MiTek Industries, Inc. Thu Dec 17 14:46:25 2020 Page 1
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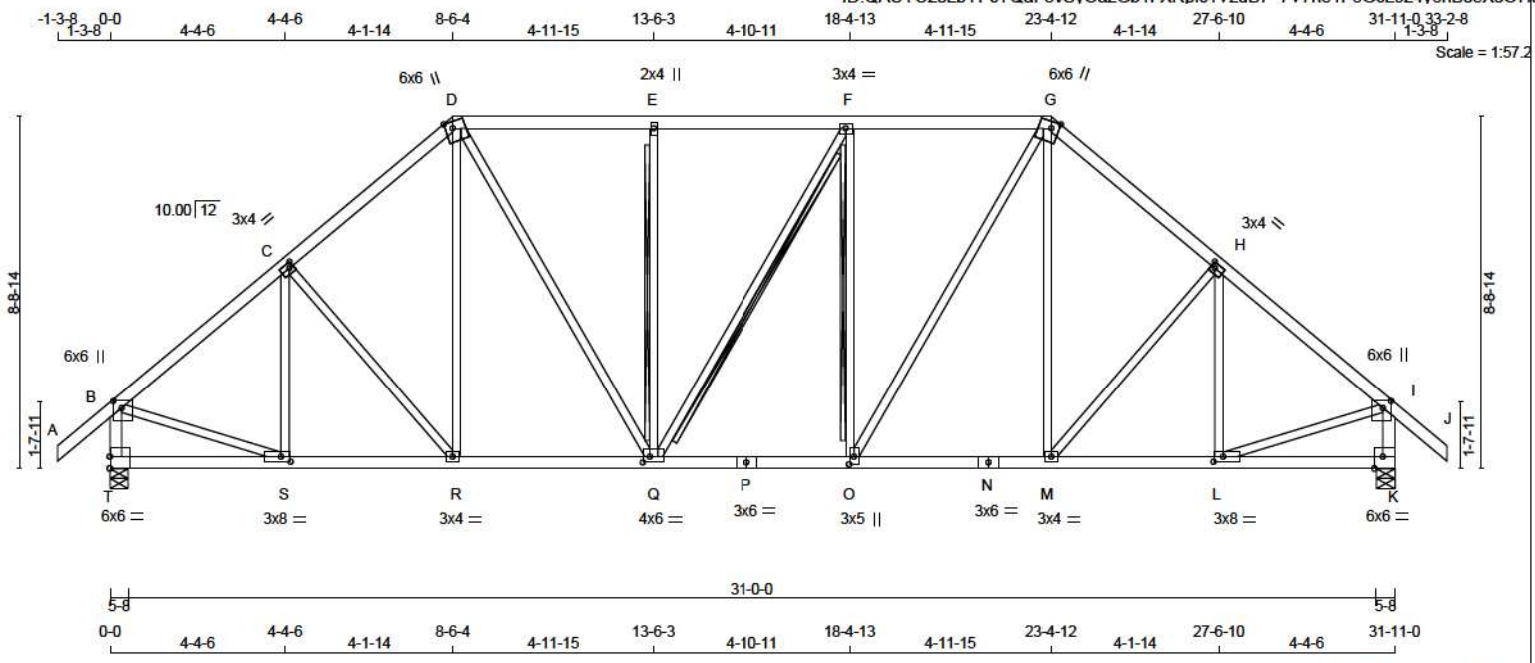

TOTAL WEIGHT = 2 X 160 = 320 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	6.0	2.00	2.50
C	TMWW-t	MT20	3.0	4.0	1.50	1.25
D	TTWW+m	MT20	6.0	6.0	2.00	2.25
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	3.0	4.0		
G	TTWW+m	MT20	6.0	6.0	2.00	2.25
H	TMWW-t	MT20	3.0	4.0	1.50	1.25
I	TMVW+p	MT20	6.0	6.0	2.00	2.50
K	BMV1-t	MT20	6.0	6.0	Edge	2.50
L	BMWW-t	MT20	3.0	8.0	1.50	2.75
M	BMWW-t	MT20	3.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW+t	MT20	3.0	5.0	2.25	1.50
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	6.0	1.75	2.00
R	BMWW-t	MT20	3.0	4.0		
S	BMWW-t	MT20	3.0	8.0	1.50	2.75
T	BMV1-t	MT20	6.0	6.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2784	0	2784	0	0	5-8	5-8
K	2784	0	2784	0	0	5-8	5-8

UNFACTORED REACTIONS
1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2053	1207 / 0	335 / 0	0 / 0	0 / 0	510 / 0	0 / 0
K	2053	1207 / 0	335 / 0	0 / 0	0 / 0	510 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-Q, F-Q, F-O

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 55	-124.4	-124.4	0.17 (1)	10.00	
B- C	-2637 / 0	-124.4	-124.4	0.39 (1)	3.93	
C- D	-2575 / 0	-124.4	-124.4	0.38 (1)	3.97	
D- E	-2451 / 0	-124.4	-124.4	0.42 (1)	3.99	
E- F	-2451 / 0	-124.4	-124.4	0.39 (1)	4.02	
F- G	-2453 / 0	-124.4	-124.4	0.42 (1)	3.99	
G- H	-2574 / 0	-124.4	-124.4	0.38 (1)	3.97	
H- I	-2637 / 0	-124.4	-124.4	0.39 (1)	3.93	
I- J	0 / 55	-124.4	-124.4	0.17 (1)	10.00	
T- B	-2711 / 0	0.0	0.0	0.29 (1)	5.23	
K- I	-2711 / 0	0.0	0.0	0.29 (1)	5.23	
T- S	0 / 0	-39.2	-39.2	0.13 (3)	10.00	
S- R	0 / 2055	-39.2	-39.2	0.43 (1)	10.00	
R- Q	0 / 1946	-39.2	-39.2	0.41 (1)	10.00	
Q- P	0 / 2453	-39.2	-39.2	0.50 (1)	10.00	
P- O	0 / 2453	-39.2	-39.2	0.50 (1)	10.00	
O- N	0 / 1945	-39.2	-39.2	0.42 (1)	10.00	
N- M	0 / 1945	-39.2	-39.2	0.42 (1)	10.00	
M- L	0 / 2055	-39.2	-39.2	0.42 (1)	10.00	
L- K	0 / 0	-39.2	-39.2	0.13 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-438 / 0	0.18 (1)
C- R	-174 / 0	0.13 (1)
R- D	0 / 337	0.08 (2)
D- Q	0 / 991	0.22 (1)
Q- E	-660 / 0	0.36 (1)
Q- F	-4 / 0	0.00 (1)
O- F	-661 / 0	0.37 (1)
O- G	0 / 996	0.22 (1)
M- G	0 / 335	0.08 (2)
M- H	-175 / 0	0.13 (1)
L- H	-437 / 0	0.18 (1)
B- S	0 / 2145	0.48 (1)
L- I	0 / 2145	0.48 (1)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.3 PSF
TOTAL LOAD = 60.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
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(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.42/1.00 (D-E:1) , BC=0.50/1.00 (O-Q:1) , WB=0.48/1.00 (I-L:1) , SSI=0.29/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (O) (INPUT = 0.90)
JSI METAL= 0.81 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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TOWN OF EAST GWILLIMBURY
Building Standards Branch

Town of East Gwillimbury
Building Standards Branch BCIN #16487

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Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	AWG NO.
NE1220-107	T12	2	1	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	
				TRUSS DESC.	**Page 23 of 31**

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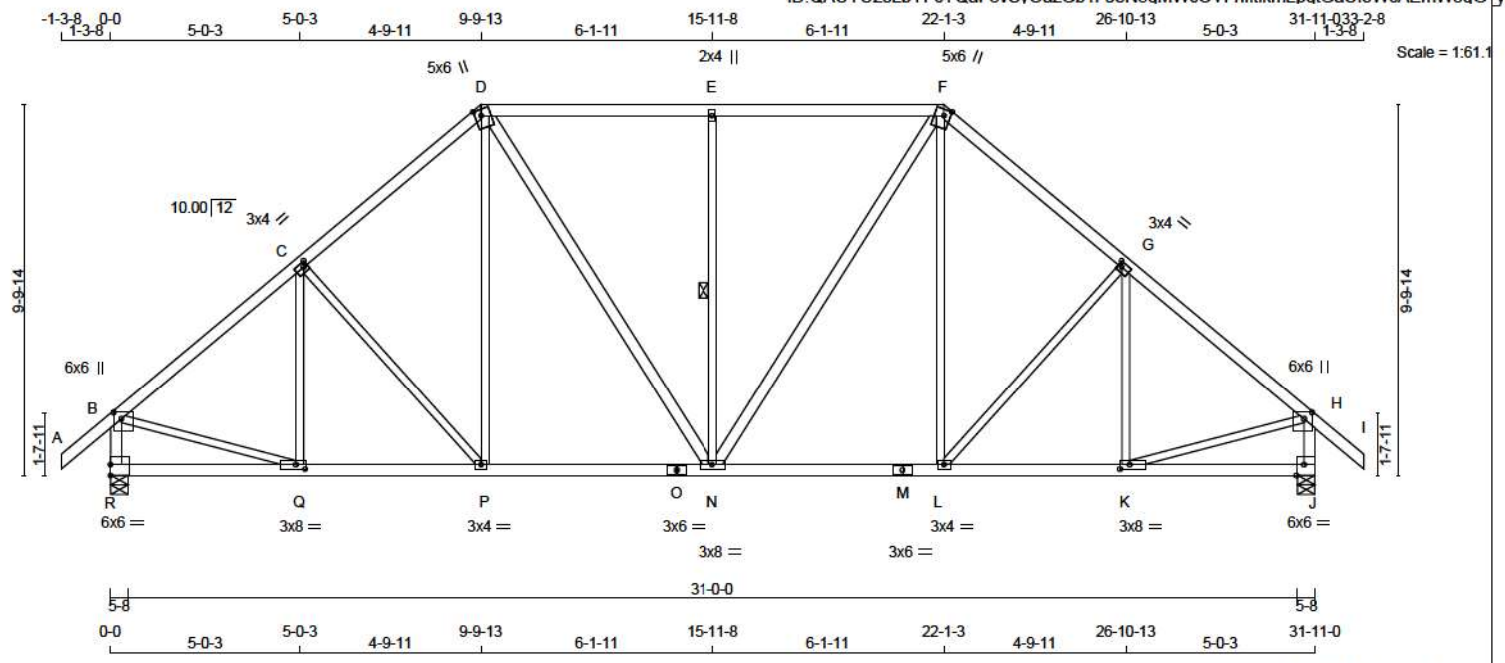

TOTAL WEIGHT = 2 X 162 = 324 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	6.0	2.00	2.50
C	TMWW-t	MT20	3.0	4.0	1.50	1.25
D	TTWW+m	MT20	5.0	6.0	2.00	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	2.00
G	TMWW-t	MT20	3.0	4.0	1.50	1.25
H	TMVW+p	MT20	6.0	6.0	2.00	2.50
J	BMV1-t	MT20	6.0	6.0	Edge	2.50
K	BMWW-t	MT20	3.0	8.0	1.50	3.00
L	BMWW-t	MT20	3.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	3.0	8.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	3.0	4.0		
Q	BMWW-t	MT20	3.0	8.0	1.50	3.00
R	BMV1-t	MT20	6.0	6.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2784	0	2784	0	0	5-8	5-8
J	2784	0	2784	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2053	1207 / 0	335 / 0	0 / 0	0 / 0	510 / 0	0 / 0
J	2053	1207 / 0	335 / 0	0 / 0	0 / 0	510 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N. DBS = 20-0-0 . CBF = 117 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 55	-124.4	-124.4	0.17 (1)	10.00	
B- C	-2678 / 0	-124.4	-124.4	0.52 (1)	3.76	
C- D	-2492 / 0	-124.4	-124.4	0.49 (1)	3.90	
D- E	-2259 / 0	-124.4	-124.4	0.69 (1)	3.68	
E- F	-2259 / 0	-124.4	-124.4	0.69 (1)	3.68	
F- G	-2492 / 0	-124.4	-124.4	0.49 (1)	3.90	
G- H	-2678 / 0	-124.4	-124.4	0.52 (1)	3.76	
H- I	0 / 55	-124.4	-124.4	0.17 (1)	10.00	
R- B	-2701 / 0	0.0	0.0	0.29 (1)	5.23	
J- H	-2701 / 0	0.0	0.0	0.29 (1)	5.23	
R- Q	0 / 0	-39.2	-39.2	0.17 (3)	10.00	
Q- P	0 / 2092	-39.2	-39.2	0.46 (1)	10.00	
P- O	0 / 1878	-39.2	-39.2	0.48 (2)	10.00	
O- N	0 / 1878	-39.2	-39.2	0.48 (2)	10.00	
N- M	0 / 1878	-39.2	-39.2	0.48 (2)	10.00	
M- L	0 / 1878	-39.2	-39.2	0.48 (2)	10.00	
L- K	0 / 2092	-39.2	-39.2	0.46 (1)	10.00	
K- J	0 / 0	-39.2	-39.2	0.17 (3)	10.00	

WEBS MEMB.	FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		
Q- C	-338 / 38	0.18 (1)
C- P	-328 / 0	0.35 (1)
P- D	0 / 470	0.11 (2)
D- N	0 / 703	0.11 (1)
N- E	-933 / 0	0.61 (1)
N- F	0 / 703	0.11 (1)
L- F	0 / 470	0.11 (2)
L- G	-328 / 0	0.35 (1)
K- G	-338 / 38	0.18 (1)
B- Q	0 / 2162	0.49 (1)
K- H	0 / 2162	0.49 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 34.8	PSF
	DL	= 8.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.3	PSF
TOTAL LOAD		= 60.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.69/1.00 (D-E:1) , BC=0.48/1.00 (N-P:2) , WB=0.61/1.00 (E-N:1) , SSI=0.37/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Q) (INPUT = 0.90)
JSI METAL= 0.59 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
2021-02-19
TOWN OF EAST GWILLIMBURY
Building Standards Branch

Town of East Gwillimbury
Building Standards Branch BCIN #16487

These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DWG NO.
NE1220-107	T14	2	1	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	
				TRUSS DESC.	**Page 24 of 31**

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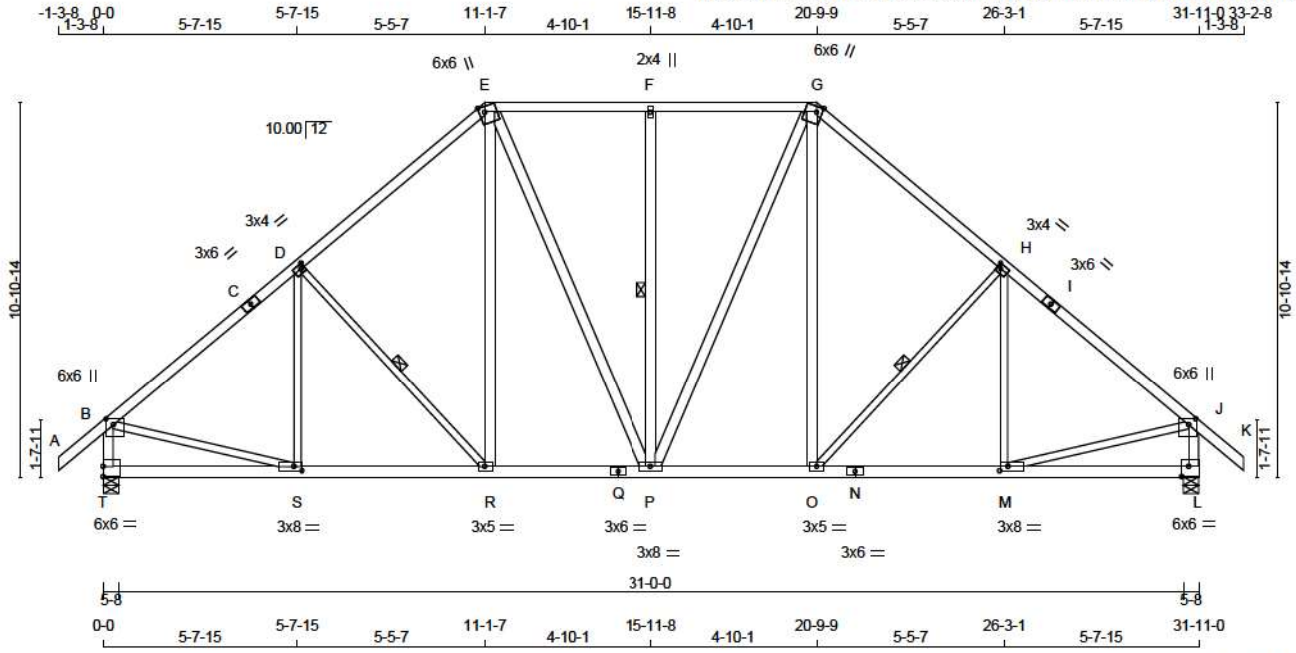

TOTAL WEIGHT = 2 X 180 = 359 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
S - D	2x3	DRY	No.2	SPF
D - R	2x3	DRY	No.2	SPF
O - H	2x3	DRY	No.2	SPF
M - H	2x3	DRY	No.2	SPF
B - S	2x3	DRY	No.2	SPF
M - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	6.0	2.00	2.50
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	3.0	4.0	1.50	1.25
E	TTWW+m	MT20	6.0	6.0	2.00	2.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	6.0	6.0	2.00	2.00
H	TMWW-t	MT20	3.0	4.0	1.50	1.25
I	TS-t	MT20	3.0	6.0		
J	TMVW+p	MT20	6.0	6.0	2.00	2.50
L	BMV1-t	MT20	6.0	6.0	Edge	2.50
M	BMWW-t	MT20	3.0	8.0	1.50	2.75
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	3.0	5.0		
P	BMWWW-t	MT20	3.0	8.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	3.0	5.0		
S	BMWW-t	MT20	3.0	8.0	1.50	2.75
T	BMV1-t	MT20	6.0	6.0	3.50	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER [M][F]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2784	0	2784	0	0	5-8	5-8
L	2784	0	2784	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2053	1207 / 0	335 / 0	0 / 0	0 / 0	510 / 0	0 / 0
L	2053	1207 / 0	335 / 0	0 / 0	0 / 0	510 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-R, F-P, H-O. DBS = 20-0-0 . CBF = 91 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B		0 / 55	-124.4	-124.4	0.17 (1)	10.00		S- D		-240 / 113	0.16 (1)
B- C		-2702 / 0	-124.4	-124.4	0.69 (1)	3.54		D- R		-482 / 0	0.22 (1)
C- D		-2702 / 0	-124.4	-124.4	0.69 (1)	3.54		R- E		0 / 553	0.09 (1)
D- E		-2387 / 0	-124.4	-124.4	0.62 (1)	3.80		E- P		0 / 497	0.08 (1)
E- F		-2002 / 0	-124.4	-124.4	0.42 (1)	4.33		P- F		-728 / 0	0.44 (1)
F- G		-2002 / 0	-124.4	-124.4	0.42 (1)	4.33		P- G		0 / 497	0.08 (1)
G- H		-2387 / 0	-124.4	-124.4	0.62 (1)	3.80		O- G		0 / 553	0.09 (1)
H- I		-2702 / 0	-124.4	-124.4	0.69 (1)	3.54		O- H		-482 / 0	0.22 (1)
I- J		-2702 / 0	-124.4	-124.4	0.69 (1)	3.54		M- H		-240 / 113	0.16 (1)
J- K		0 / 55	-124.4	-124.4	0.17 (1)	10.00		B- S		0 / 2171	0.49 (1)
T- B		-2694 / 0	0.0	0.0	0.29 (1)	5.24		M- J		0 / 2171	0.49 (1)
L- J		-2694 / 0	0.0	0.0	0.29 (1)	5.24					
T- S		0 / 0	-39.2	-39.2	0.24 (3)	10.00					
S- R		0 / 2116	-39.2	-39.2	0.50 (2)	10.00					
R- Q		0 / 1796	-39.2	-39.2	0.40 (1)	10.00					
Q- P		0 / 1796	-39.2	-39.2	0.40 (1)	10.00					
P- O		0 / 1796	-39.2	-39.2	0.40 (1)	10.00					
O- N		0 / 2116	-39.2	-39.2	0.50 (2)	10.00					
N- M		0 / 2116	-39.2	-39.2	0.50 (2)	10.00					
M- L		0 / 0	-39.2	-39.2	0.24 (3)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	34.8	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	60.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.69/1.00 (B-D:1) , BC=0.50/1.00 (R-S:2) , WB=0.49/1.00 (B-S:1) , SSI=0.29/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.64 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
2021-02-19
TOWN OF EAST GWILLIMBURY
Building Standards Branch

Town of East Gwillimbury
Building Standards Branch BCIN #16487

These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

CONTINUED ON PAGE 2

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DWG NO.
NE1220-107	T15	2	1	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	
				TRUSS DESC.	

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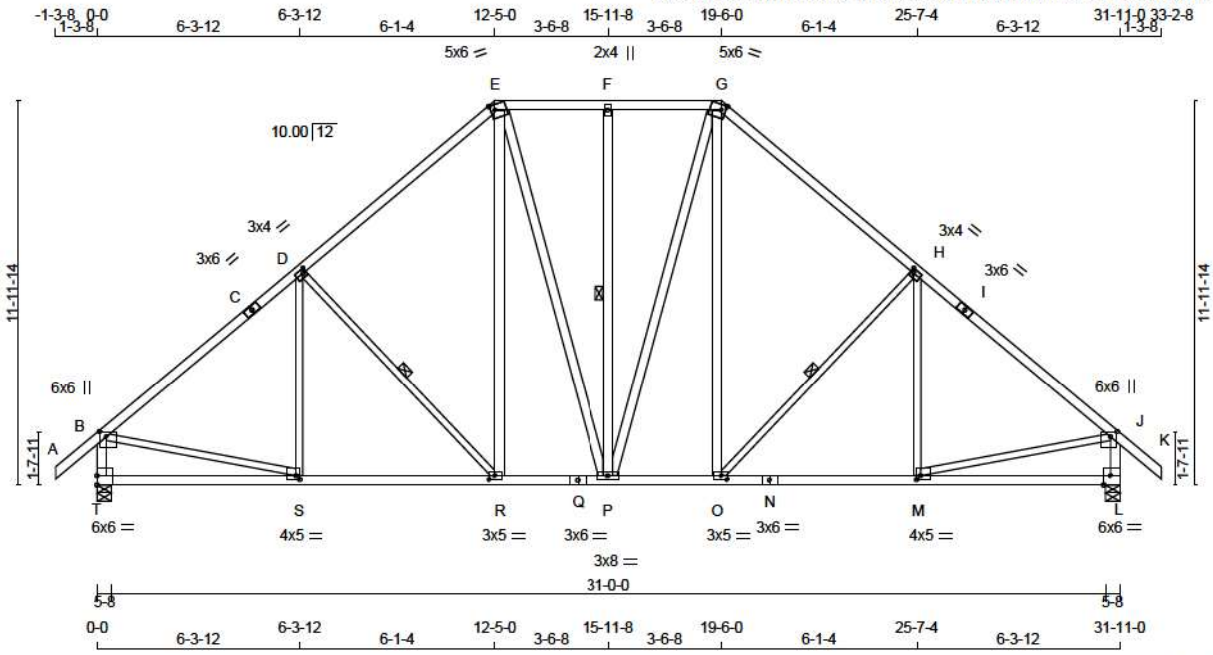

TOTAL WEIGHT = 2 X 189 = 378 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
S - D	2x3	DRY	No.2	SPF
D - R	2x3	DRY	No.2	SPF
O - H	2x3	DRY	No.2	SPF
M - H	2x3	DRY	No.2	SPF
B - S	2x3	DRY	No.2	SPF
M - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	6.0	2.00	2.50
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	3.0	4.0	1.50	1.25
E	TTWW-m	MT20	5.0	6.0	2.00	1.75
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.00	1.75
H	TMWW-t	MT20	3.0	4.0	1.50	1.25
I	TS-t	MT20	3.0	6.0		
J	TMVW+p	MT20	6.0	6.0	2.00	2.50
L	BMV1-t	MT20	6.0	6.0	Edge	2.50
M	BMWW-t	MT20	4.0	5.0	1.50	1.50
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	3.0	5.0	1.50	2.25
P	BMWWW-t	MT20	3.0	8.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	3.0	5.0	1.50	2.25
S	BMWW-t	MT20	4.0	5.0	1.50	1.50
T	BMV1-t	MT20	6.0	6.0	3.50	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2784	0	2784	0	0	5-8	5-8
L	2784	0	2784	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2053	1207 / 0	335 / 0	0 / 0	0 / 0	510 / 0	0 / 0
L	2053	1207 / 0	335 / 0	0 / 0	0 / 0	510 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-R, F-P, H-O. DBS = 20-0-0 . CBF = 78 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
FR-TO									
A- B	0 / 55	-124.4	-124.4	0.17 (1)	10.00	S- D	-158 / 175	0.13 (1)	
B- C	-2710 / 0	-124.4	-124.4	0.89 (1)	3.23	D- R	-623 / 0	0.35 (1)	
C- D	-2710 / 0	-124.4	-124.4	0.89 (1)	3.23	R- E	0 / 649	0.10 (1)	
D- E	-2275 / 0	-124.4	-124.4	0.78 (1)	3.61	E- P	0 / 319	0.05 (1)	
E- F	-1798 / 0	-124.4	-124.4	0.22 (1)	4.79	P- F	-517 / 0	0.41 (1)	
F- G	-1798 / 0	-124.4	-124.4	0.22 (1)	4.79	P- G	0 / 319	0.05 (1)	
G- H	-2275 / 0	-124.4	-124.4	0.78 (1)	3.61	O- G	0 / 649	0.10 (1)	
H- I	-2710 / 0	-124.4	-124.4	0.89 (1)	3.23	O- H	-623 / 0	0.35 (1)	
I- J	-2710 / 0	-124.4	-124.4	0.89 (1)	3.23	M- H	-158 / 175	0.13 (1)	
J- K	0 / 55	-124.4	-124.4	0.17 (1)	10.00	B- S	0 / 2172	0.49 (1)	
T- B	-2685 / 0	0.0	0.0	0.29 (1)	5.25	M- J	0 / 2172	0.49 (1)	
L- J	-2685 / 0	0.0	0.0	0.29 (1)	5.25				
T- S	0 / 0	-39.2	-39.2	0.31 (3)	10.00				
S- R	0 / 2127	-39.2	-39.2	0.57 (2)	10.00				
R- Q	0 / 1705	-39.2	-39.2	0.39 (1)	10.00				
Q- P	0 / 1705	-39.2	-39.2	0.39 (1)	10.00				
P- O	0 / 1705	-39.2	-39.2	0.39 (1)	10.00				
O- N	0 / 2127	-39.2	-39.2	0.57 (2)	10.00				
N- M	0 / 2127	-39.2	-39.2	0.57 (2)	10.00				
M- L	0 / 0	-39.2	-39.2	0.31 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	= 34.8	PSF
		DL	= 8.0	PSF
BOT	CH.	LL	= 10.5	PSF
		DL	= 7.3	PSF
TOTAL	LOAD		= 60.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.89/1.00 (B-D:1) , BC=0.57/1.00 (R-S:2) , WB=0.49/1.00 (B-S:1) , SSI=0.29/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.72 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
2021-02-19
TOWN OF EAST GWILLIMBURY
Building Standards Branch

Town of East Gwillimbury
Building Standards Branch BCIN #16487

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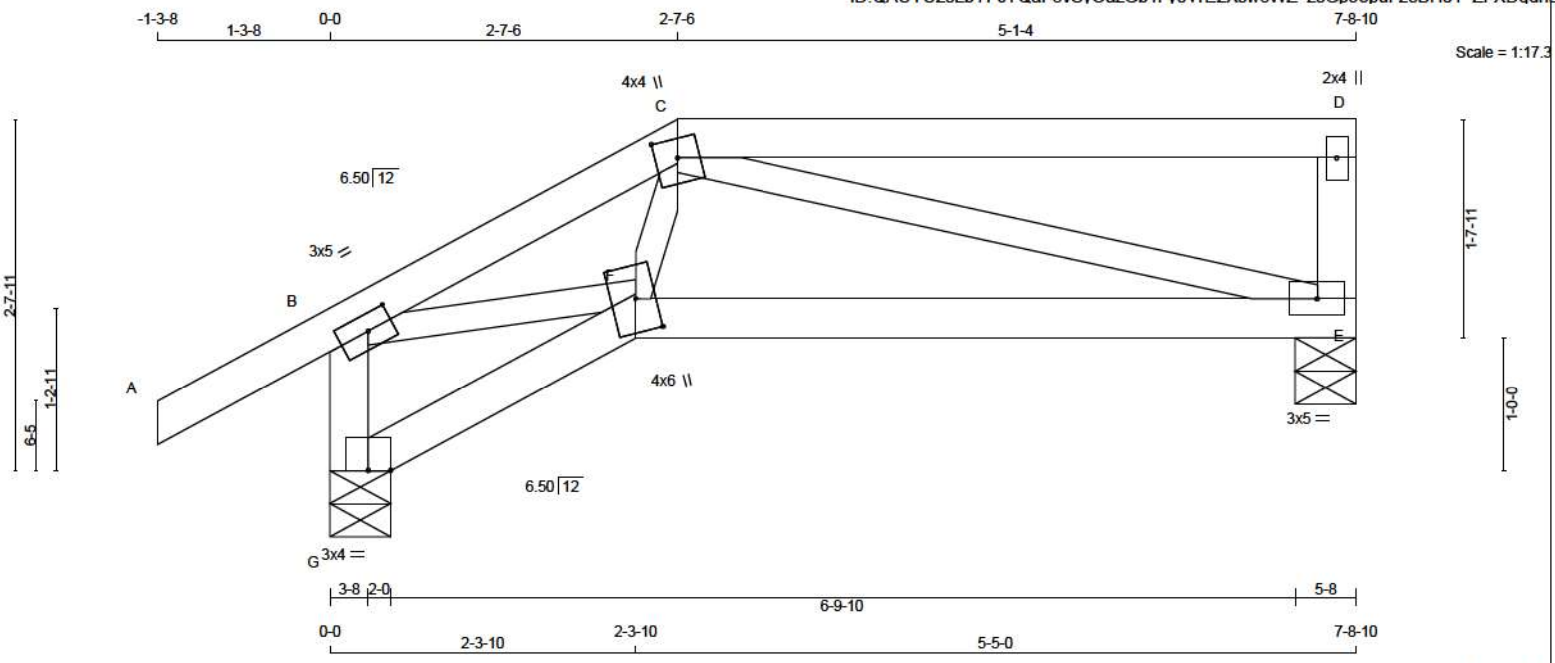
Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

CONTINUED ON PAGE 2

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE1220-107	T16	1	1	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	
				TRUSS DESC.	**Page 26 of 31**

Version 8.420 S Oct 9 2020 MiTek Industries, Inc. Thu Dec 17 14:46:28 2020 Page 1
ID:QAUYU2sLbYFcYQaF3vSyOazGb1I-y0VrE2Xsw6WZ_zsGp9opuFzeBH01_ZFXDqdn_ty8



TOTAL WEIGHT = 29 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
G - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	2.25
C	TTWW+m	MT20	4.0	4.0	1.75	2.00
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	5.0		
F	BBWW+m	MT20	4.0	6.0	3.00	1.75
G	BVM1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	801	0	801	0	0	5-8	5-8
E	632	0	632	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	584	363 / 0	81 / 0	0 / 0	0 / 0	140 / 0	0 / 0
E	468	269 / 0	81 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- B	-756 / 0	0.0	0.0	0.08 (1)	7.81	B- F	0 / 843	0.19 (1)	
A- B	0 / 40	-124.4	-124.4	0.16 (1)	10.00	F- C	0 / 230	0.05 (3)	
B- C	-959 / 0	-124.4	-124.4	0.16 (1)	6.15	C- E	-825 / 0	0.38 (1)	
C- D	0 / 0	-124.4	-124.4	0.55 (1)	10.00				
E- D	-317 / 0	0.0	0.0	0.03 (1)	7.81				
G- F	0 / 0	-39.2	-39.2	0.05 (3)	10.00				
F- E	0 / 798	-39.2	-39.2	0.36 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.3 PSF
TOTAL LOAD = 60.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
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- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 889 (0.10")

CSI: TC=0.55/1.00 (C-D:1) , BC=0.36/1.00 (E-F:2) , WB=0.38/1.00 (C-E:1) , SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.44 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

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RECEIVED
2021-02-19
TOWN OF EAST GWILLIMBURY
Building Standards Branch

Town of East Gwillimbury
Building Standards Branch BCIN #16487

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Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE1220-107	T17	1	1	GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2	
				TRUSS DESC.	

Version 8.420 S Oct 9 2020 MiTek Industries, Inc. Thu Dec 17 14:46:29 2020 Page 1
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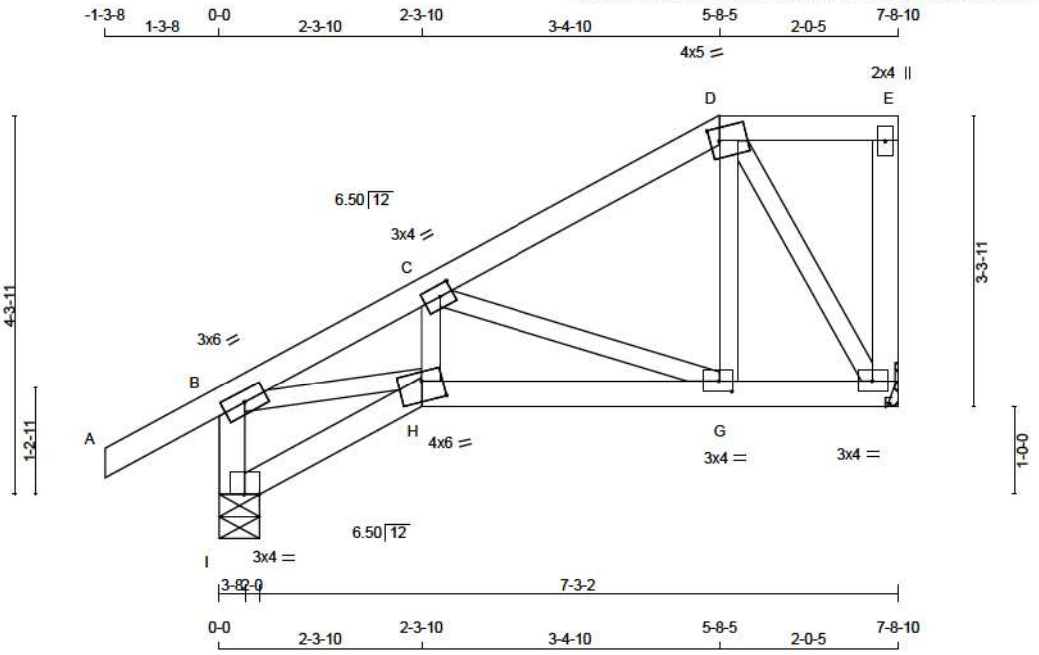

TOTAL WEIGHT = 35 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	6.0		
C	TMWW-t	MT20	3.0	4.0	1.50	1.75
D	TTWW-m	MT20	4.0	5.0	1.75	1.25
E	TMV+p	MT20	2.0	4.0		
F	BMVW1-t	MT20	3.0	4.0		
G	BMWW-t	MT20	3.0	4.0	1.50	1.75
H	BBWW-m	MT20	4.0	6.0	2.75	2.75
I	BVM1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER [M][F]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	FACTORED UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	801	0	801	0	0	5-8	1-8
F	632	0	632	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	584	363 / 0	81 / 0	0 / 0	0 / 0	140 / 0	0 / 0
F	468	269 / 0	81 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	-756 / 0	0.0	0.0	0.08 (1)	7.81		B- H	0 / 957	0.22 (1)
A- B	0 / 40	-124.4	-124.4	0.16 (1)	10.00		H- C	0 / 167	0.04 (2)
B- C	-1068 / 0	-124.4	-124.4	0.12 (1)	5.98		C- G	-668 / 0	0.16 (1)
C- D	-373 / 0	-124.4	-124.4	0.15 (1)	6.25		G- D	0 / 358	0.08 (1)
D- E	0 / 0	-124.4	-124.4	0.09 (1)	10.00		D- F	-586 / 0	0.13 (1)
F- E	-126 / 0	0.0	0.0	0.02 (1)	7.81				
I- H	0 / 0	-39.2	-39.2	0.05 (3)	10.00				
H- G	0 / 944	-39.2	-39.2	0.21 (1)	10.00				
G- F	0 / 328	-39.2	-39.2	0.11 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.3 PSF
TOTAL LOAD = 60.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

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(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.21/1.00 (G-H:1) , WB=0.22/1.00 (B-H:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (H) (INPUT = 0.90)
JSI METAL= 0.44 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

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Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

KOTT

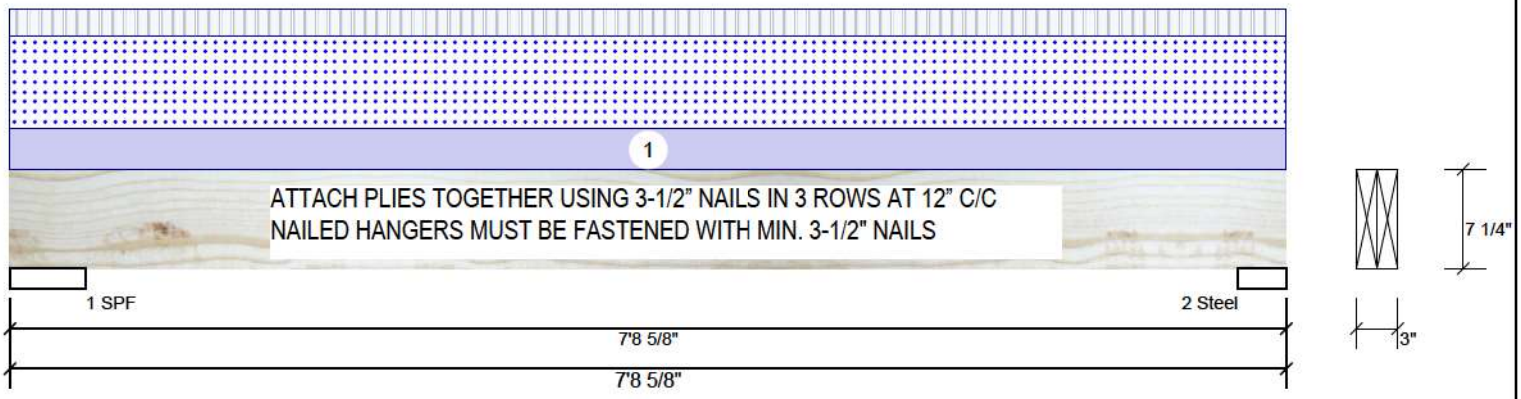
iDesign

Client: NE1220-107
Project: GREENPARK - TRINAR
Address: HALL - GLENWAY 2A ELE 2

Date: 12/16/2020
Input by: DM
Job Name: B1
Project #: GLENWAY 2A EL 2

B1 S-P-F #2 2.000" X 8.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015 / OBC 2012
Deflection TL:	360	Load Sharing:	No
Importance:	Normal - II	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	314	457	1040	0
2	301	438	996	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	24%	572 / 1875	2446	L	1.25D+1.5S +L
2 - Steel	3.500"	36%	548 / 1795	2343	L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3903 ft-lb	3'11 5/16"	4047 ft-lb	0.964 (96%)	1.25D+1.5S +L	L
Unbraced	3903 ft-lb	3'11 5/16"	3906 ft-lb	0.999 (100%)	1.25D+1.5S +L	L
Shear	2201 lb	1'	3406 lb	0.646 (65%)	1.25D+1.5S +L	L
LL Defl inch	0.132 (L/646)	3'11 3/8"	0.236 (L/360)	0.560 (56%)	S+0.5L	L
TL Defl inch	0.182 (L/467)	3'11 3/8"	0.236 (L/360)	0.770 (77%)	D+S+0.5L	L

Design Notes

1 Girders are designed to be supported on the bottom edge only.
2 Multiple plies must be fastened together as per manufacturer's details.
3 Top must be laterally braced at a maximum of 3'4 7/8" o.c.
4 Bottom braced at bearings.
5 Lateral slenderness ratio based on single ply width.

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 16, 2020

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		7-7-0	Far Face	15.3 PSF	10.5 PSF	34.8 PSF	0 PSF	

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Town of East Gwillimbury
Building Standards Branch BCIN #16487

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Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

RECEIVED
2021-02-19
TOWN OF EAST GWILLIMBURY
Building Standards Branch

This design is valid until 10/10/2023

Kott Group
14 Anderson Blvd., On
L4A7X4
905-642-4400

KOTT

CSD DRAW DESIGN BUILD

1) ENSURE TRUSS HAS BEEN DESIGNED WITH ADDITIONAL TOP CHORD DEAD LOAD EQUAL TO OR GREATER THAN WEIGHT OF SOLAR PANELS BEING INSTALLED

2) ATTACH SOLAR PANELS TO EACH TRUSS LOCATED UNDER THE SOLAR PANELS (I.E. @ 24" C/C PERPENDICULAR TO THE TRUSS DIRECTION)

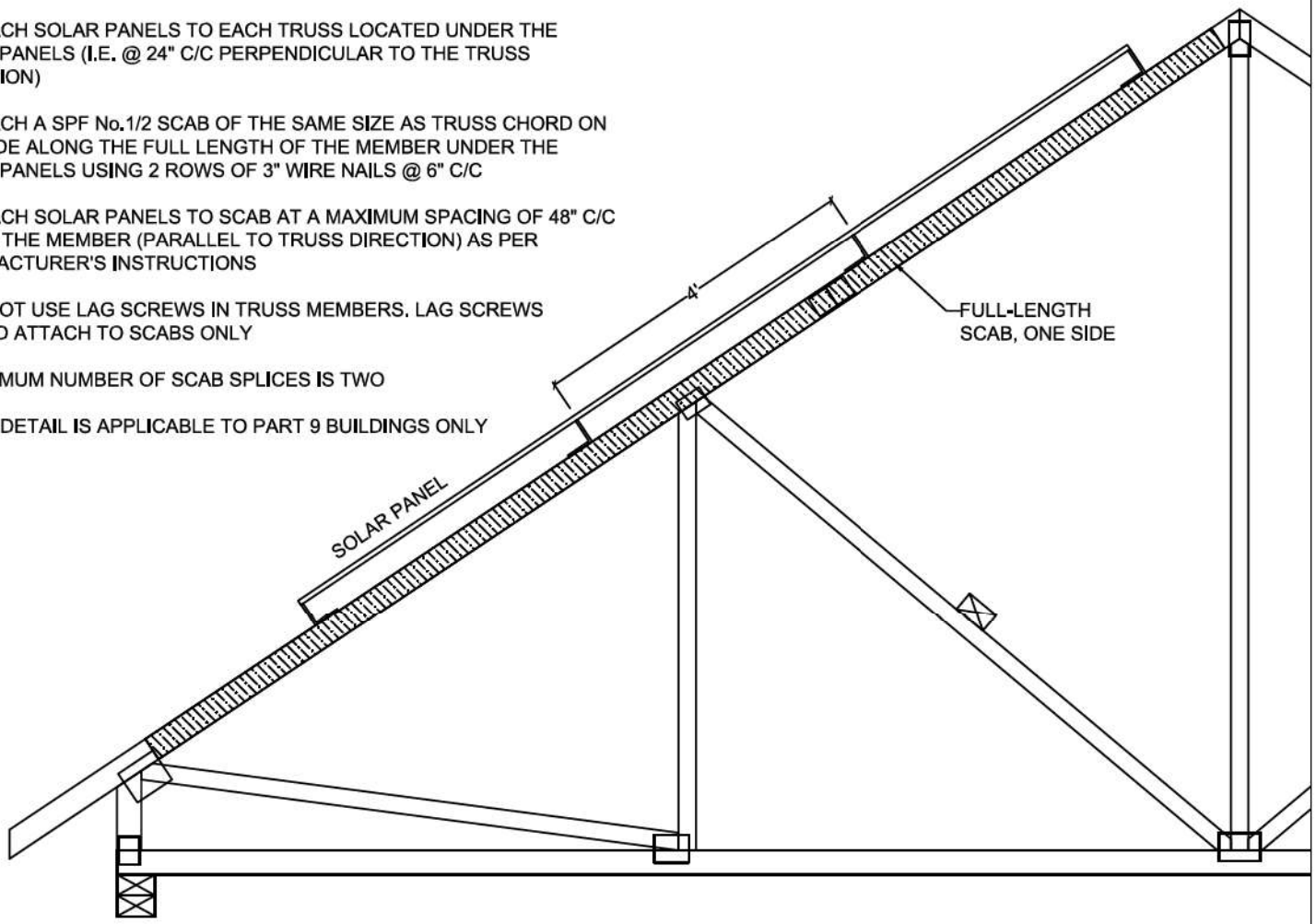
3) ATTACH A SPF No.1/2 SCAB OF THE SAME SIZE AS TRUSS CHORD ON ONE SIDE ALONG THE FULL LENGTH OF THE MEMBER UNDER THE SOLAR PANELS USING 2 ROWS OF 3" WIRE NAILS @ 6" C/C

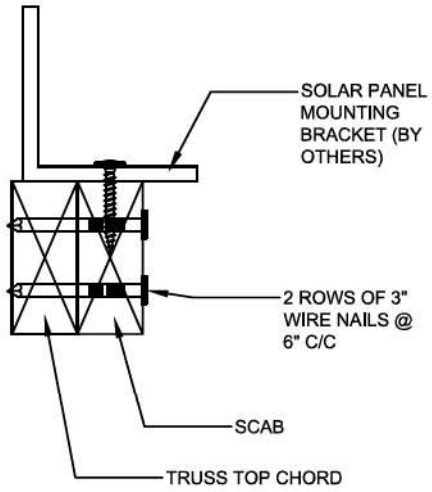
4) ATTACH SOLAR PANELS TO SCAB AT A MAXIMUM SPACING OF 48" C/C ALONG THE MEMBER (PARALLEL TO TRUSS DIRECTION) AS PER MANUFACTURER'S INSTRUCTIONS

5) DO NOT USE LAG SCREWS IN TRUSS MEMBERS. LAG SCREWS SHOULD ATTACH TO SCABS ONLY

6) MAXIMUM NUMBER OF SCAB SPLICES IS TWO

7) THIS DETAIL IS APPLICABLE TO PART 9 BUILDINGS ONLY





NE1220-107
GREENPARK - TRINAR HALL - GLENWAY 2A ELE 2

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Dec 17, 2020

Detail for Installation of Solar Panels - Scab Method

KOTT

NE1220-107
GREENPARK - TRINAR
HALL - GLENWAY 2A ELE 2

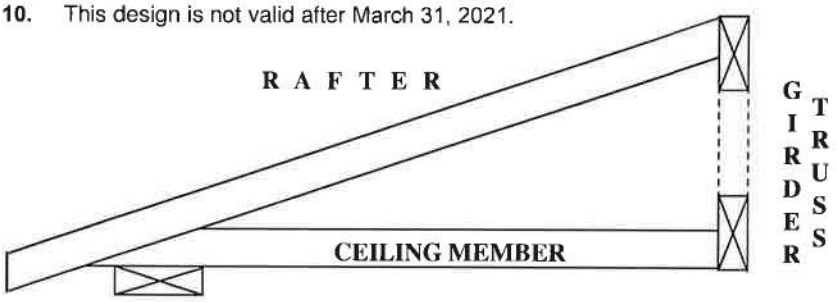
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

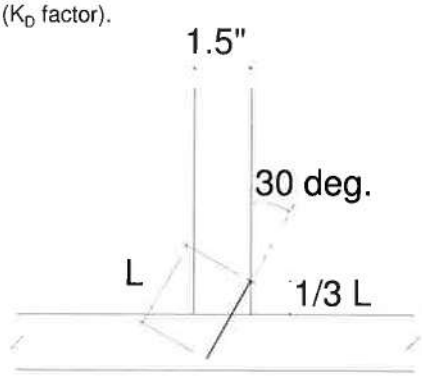
B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.





TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

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Town of East Gwillimbury
Building Standards Branch BCIN #16487

These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-19
Sewage System			
Zoning			

December

RECEIVED
2021-02-19
TOWN OF EAST GWILLIMBURY
Building Standards Branch

NE1220-107
GREENPARK - TRINAR
HALL - GLENWAY 2A ELE 2

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

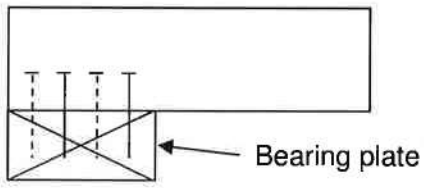
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

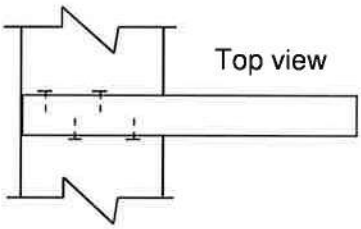
NOTES:

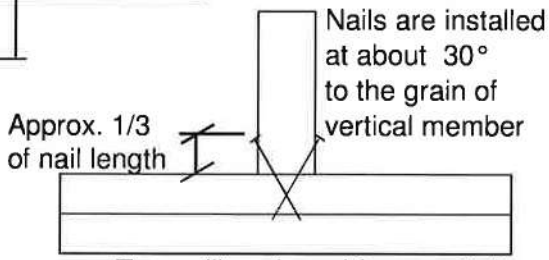
1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



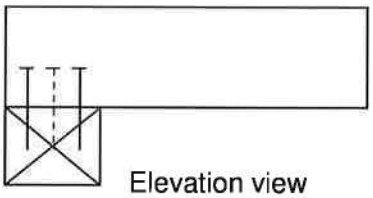
Elevation view



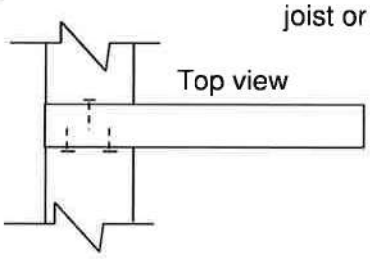


Toe-nailing viewed from end of joist or truss

Toe-nailing on 2x4 Bearing Plate



Elevation view



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