GREENPARK-TRINAR HALL-GLENWAY 7A-ELEV 1-R1

## **Engineering Note Page (ENP-2)**

**REVISION 2018-10-17** 

## Please read all notes prior to installation of the component

## **DESIGN INFORMATION**

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is only limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with transfer blocks. Structural elements such as walls, posts, connectors, and transfer blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of floor joists is to be carried out in accordance with the current edition of the manufacturer's literature available at http://www.kottgroup.com.

## CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

## COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru transfer block framing is required at all point loads over bearings.

## HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the authorization.

These plans have been reviewed for use with the corrections as noted. No other changes may be Ontario Building Code, as amehded. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

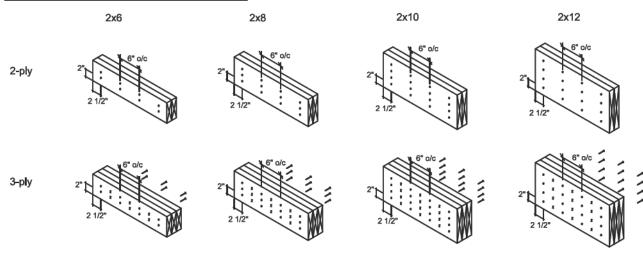
East Gwillimbury

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-03
Sewage System			
Zoning			



GREENPARK-TRINAR HALL-GLENWAY 7A-ELEV 1-R1

# **Conventional Connections**



Conventional connection notes:

- -Nails to be 3" long wire nails.
- -Nalls to be located 2" mln. from the top and bottom of the member. Start all nalls 2 1/2" mln. from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.

3 1/4" mlr

3 1/4" mlr

- "X" represents nall driven from the opposite side.

#### SIMPSON SDW SPACING REQUIREMENT





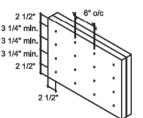
# LVL Connections

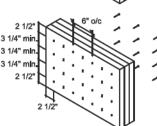
HEAD OF ALL SPECIFIED NAILS AND SCREWS MUST BE ON THE LOADED SIDE

9 1/2" LVL

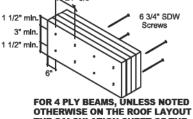
11 7/8" - 14" LVL

16"-18" LVL





4-ply LVL (Top load only)



OTHERWISE ON THE ROOF LAYOUT OR THE CALCULATION SHEET OF THE BEAM, USE MINIMUM 6-3/4" SDW SCREWS PLACED IN 2 ROWS AT 16" C/C



- -LVL ply width is 1-3/4"
- -Nalls to be 3 1/2" common wire nalls.
- -Nails to be located 2 1/2" min. from the top and bottom of the member.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nall or screw driven from the opposite side.



corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly

osted on site at all time

Reviewer BCIN Date
H. Authier 43236 2021-0 KOT1 Building Code 3228 Sewage System

Ottaw

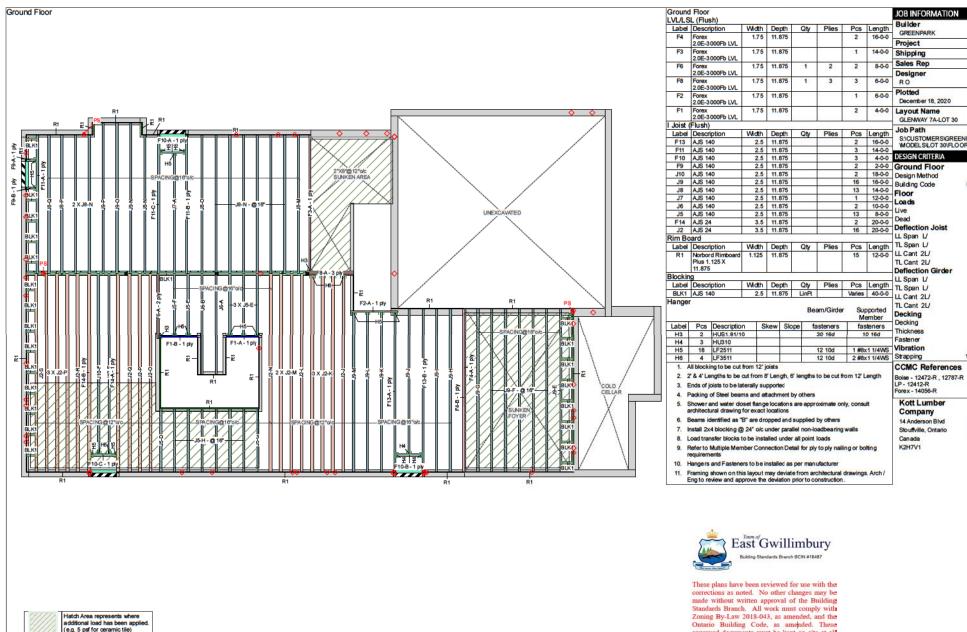
K2H /VT 613-838-2775

# **Multiple Member Connections**

All connections are for uniformly distributed loads.

For multi-ply connections of I-joists, refer to Manufacturer's Installation Guide





This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them

Version 20.20.002 Powered by iStruct\*\*

	l Floor								JOB INFORMATION	
	L (Flus								Builder	-
	Descri	ption	Width	Depth	Qty	Plies	Pcs	Length	GREENPARK	
F4	Forex 2.0E-3	000Fb LVL	1.75	11.875			2	16-0-0	Project	Ī
F3	Forex 2.0E-3	000Fb LVL	1.75	11.875			1	14-0-0	Shipping	_
F6	Forex		1.75	11.875	1	2	2	8-0-0	Sales Rep	
		000Fb LVL							Designer	
F8	Forex 2.0E-3	000Fb LVL	1.75	11.875	1	3	3	6-0-0	RO Plotted	_
F2	Forex 2.0E-3	000Fb LVL	1.75	11.875		es.	1	6-0-0	December 18, 2020	
F1	Forex	000Fb LVL	1.75	11.875		Sc.	2	4-0-0	Layout Name	3
ist (	(Flush)	JOOI D LVL					10		GLENWAY 7A-LOT 30	_
	Descri	ption	Width	Depth	Qty	Plies	Pcs	Length	Job Path	
13	AJS 14	0	2.5	11.875			2	16-0-0	S:\CUSTOMERS\GREENPARK\TRINAR HALL WODELS\LOT 30\FLO ORS\GLENWAY 7A-LOT 30	
11	AJS 14	0	2.5	11.875			3	14-0-0		4
10	AJS 14	0	2.5	11.875		à	3	4-0-0	DESIGN CRITERIA	
F9	AJS 14	0	2.5	11.875			2	2-0-0	Ground Floor	Т
10	AJS 14	0	2.5	11.875			2	18-0-0	Design Method LSD (Canada)	
J9	AJS 14	0	2.5	11.875			16	16-0-0		
J8	AJS 14	0	2.5	11.875			13	14-0-0	Floor	
J7	AJS 14	0	2.5	11.875		3	1	12-0-0	Loads	
J6	AJS 14	0	2.5	11.875			2	10-0-0	Live 40	
J5	AJS 14		2.5	11.875			13	8-0-0	77 IL TO 10 10 10 10 10 10 10 10 10 10 10 10 10	
14	AJS 24		3.5	11.875			2	20-0-0		
J2	AJS 24		3.5	11.875		i i	16	20-0-0	Deflection Joist	
n Bo	pard			91.595		SWITTERS >	227 2	68. 62	LL Span L/ 480	
bel	Descri	ption	Width	Depth	Qty	Plies	Pcs	Length	TL Span L/ 360	
₹1		d Rimboard	1.125	11.875	7.1		15	12-0-0	LL Cant 2L/ 480	
	Plus 1.	125 X							TL Cant 2L/ 360	
	11.875								Deflection Girder	
ckin abel		-4	Width	D#-	04.	Plies	Pcs	1 41-	LL Span L/ 360	
LK1	Descri AJS 14		2.5	Depth 11.875	Qty	Piles	Varies	Length 40-0-0	TL Span L/ 240	
ngei		0	2.5	11.0/3	Line		verires	40-0-0	LL Cant 2L/ 480	
iyei					Be	am/Girde		pported	TL Cant 2L/ 240 Decking	
h al	Des	Description	- 10	Cia.					Decking OSB	
bel I3	Pcs 2	Description		kew Slog	xe ta	asteners 30 16d		o 16d	Thickness 3/4"	
14	3	HUS1.81/1	-	_	+	30 160	,	U 160	Fastener Nailed & Glued	
15	18	LF2511			- 1	12 10d	1 #8	c1 1/4WS	Vibration	
16		LF3511	_		_	12 10d		(1 1/4WS	Strapping 1"X4", 1 Row at Midspan	
		na to be cut	from 12's	oists	8.8				CCMC References	3



Point Load Support Load from Above

Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Legend PS

Reviewer	BCIN	Date
H. Authier	43236	2021-02-03
	-	

NE1220-141

Client: Project: isDesign Address: **GREENPARK** 

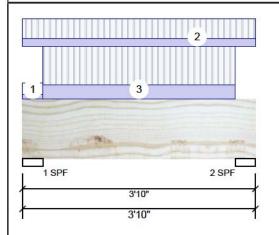
Date: 12/17/2020 Input by:

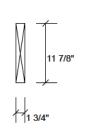
Job Name: GLENWAY 7A-ELEV. 1-DECK-R1

Project #:

#### 1.750" X 11.875" - PASSED Forex 2.0E-3000Fb LVL

Level: Ground Floor





Page 5 of 47

#### Member Information Application: Floor (Residential) Type: Plies: Design Method: Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Floor Live: 40 PSF

			(-	,
Brg	Live	Dead	Snow	Wind
1	644	251	0	0
2	620	242	0	0

Unfactored Reactions UNPATTERNED Ib (Uplift)

#### **Bearings and Factored Reactions** Cap. React D/L lb Bearing Length Total Ld. Case Ld. Comb. 1 - SPF 4.000" 30% 314 / 966 1280 L 1.25D+1.5L 2 - SPF 4.000" 29% 302 / 929 1232 L 1.25D+1.5L

#### Analysis Results

Dead:

15 PSF

Г	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	981 ft-lb	1'11"	17130 ft-lb	0.057 (6%)	1.25D+1.5L	L
	Unbraced	981 ft-lb	1'11"	13098 ft-lb	0.075 (7%)	1.25D+1.5L	L
	Shear	719 lb	2'6 7/8"	5798 lb	0.124 (12%)	1.25D+1.5L	L
	Perm Defl in.	0.002 (L/21591)	1'11"	0.110 (L/360)	0.020 (2%)	D	Uniform
	LL Defl inch	0.005 (L/8386)	1'11"	0.110 (L/360)	0.040 (4%)	L	L
	TL Defl inch	0.007 (L/6040)	1'11"	0.165 (L/240)	0.040 (4%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Wind

0 PSF

0 PLF

0 PLF

Comments

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

Snow

0 PSF

0 PLF

0 PLF

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Live

40 PSF

125 PLF

240 PLF



### **Design Notes**

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead
1	Tie-In	0-0-0 to 0-4-0	1-10-2	Тор	15 PSF
2	Part. Uniform	0-0-0 to 3-10-0		Far Face	47 PLF
3	Part. Uniform	0-4-0 to 3-6-0		Тор	90 PLF
	Self Weight				5 PLF



These plans have been reviewed for use with the These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Notes

structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corn
- **Handling & Installation**
- LVL beams must not be cut or drilled
   Refer to manufacturer's product regarding installation requirement
- Daniaged beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

Forex APA: PR-L318

Manufacturer Info

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-03
Sewage System			
Zoning			

NE1220-141

isDesign

Client: GREENPA

Project:
Address:

GREENPARK

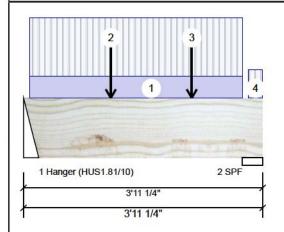
Date: 12/17/2020 Input by: R O

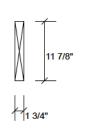
Job Name: GLENWAY 7A-ELEV. 1-DECK-R1

Project #:

## F1-B Forex 2.0E-3000Fb LVL 1.750" X 11.875" - PASSED

Level: Ground Floor





Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Page 6 of 47

Member Information						
Type:	Girder	Application:	F			
Plies:	1	Design Method:	L			
Moisture Condition	Dry	Building Code:	N			
Deflection LL:	360	Load Sharing:	N			
Deflection TL:	240	Deck:	N			
Importance:	Normal	Vibration:	N			
General Load		SA 44 1 (2.44 SA 2016 A				
Floor Live:	40 PSF					

Application:	Floor (Residential)
Design Method:	LSD
Building Code:	NBCC 2015 / OBC 2012
oad Sharing:	No
Deck:	Not Checked
/ibration:	Not Checked

	Unfactore	d Reactions	UNPATTERN	NED lb (Uplift	:)
	Brg	Live	Dead	Snow	Wind
	1	535	210	0	0
2	2	550	217	0	0

Cap. React D/L lb

263 / 803

271 / 826

Dead:	15 PSF								
Analysis Re	Analysis Results								
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case			
Moment	991 ft-lb	1'9 15/16"	17130 ft-lb	0.058 (6%)	1.25D+1.5L	L			
Unbraced	991 ft-lb	1'9 15/16"	12625 ft-lb	0.079 (8%)	1.25D+1.5L	L			

Moment	991 ft-lb	1'9 15/16"	17130 ft-lb	0.058 (6%)	1.25D+1.5L	L
Unbraced	991 ft-lb	1'9 15/16"	12625 ft-lb	0.079 (8%)	1.25D+1.5L	L
Shear	671 lb	2'8 1/8"	5798 lb	0.116 (12%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/21374)	1'10 11/16"	0.116 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.005 (L/8339)	1'10 5/8"	0.116 (L/360)	0.040 (4%)	L	L
TL Defl inch	0.007 (L/5999)	1'10 5/8"	0.174 (L/240)	0.040 (4%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

25%

**Bearings and Factored Reactions** 

Bearing Length

Hanger 2 - SPF 4.000"

3.000"

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



Total Ld. Case

1066 L

1096 L

## **Design Notes**

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-1-4 to 3-7-4		Тор	79 PLF	210 PLF	0 PLF	0 PLF	
2	Point	1-5-4		Far Face	65 lb	174 lb	0 lb	0 lb	J5
3	Point	2-9-4		Far Face	60 lb	159 lb	0 lb	0 lb	J5 (
4	Tie-In	3-8-6 to 3-11-4	1-10-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				5 PLF				The



These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orineria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

#### Lumber

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive.
- Handling & Installation
- 1. IVI, beams must not be cut or drilled
   2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
   3. Damaged Beams must not be used
- Design assumes top edge is laterally restrained
   Provide lateral support at bearing points to avoid lateral displacement and rotation

APA: PR-L318

Manufacturer Info

4	Discipline	Reviewer	BCIN	Date
l	Building Code	H. Authier	43236	2021-02-0
l	Sewage System			
ŀ	Zoning			
I		<b>(</b> I		



NE1220-141

Client: GREENPARK Date: 12/17/2020

isDesign Address:

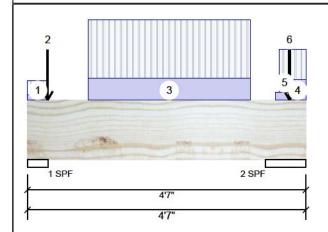
Date: 12/17/2020 Input by: R O

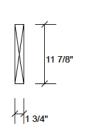
Job Name: GLENWAY 7A-ELEV. 1-DECK-R1

Project #:

## F2-A Forex 2.0E-3000Fb LVL 1.750" X 11.875" - PASSED

Level: Ground Floor





Wind

0

0

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

0

0

Total Ld. Case

1047 L

1413 L

Page 7 of 47

#### Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Conditi	ion: Dry	Building Code:	NBCC 2015 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load		2010/10/2010/06/20	
Floor Live:	40 PSF		
Dead:	15 PSF		

# Unfactored Reactions UNPATTERNED Ib (Uplift) Brg Live Dead Snow

206

301

Cap. React D/L lb

526

691

1

2

Bearing Length

1 - SPF 4.000"

2 - SPF 8.000"

Bearings and Factored Reactions	

258 / 789

376 / 1037

#### **Analysis Results**

Γ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	957 ft-lb	2'2 3/16"	17130 ft-lb	0.056 (6%)	1.25D+1.5L	L
	Unbraced	957 ft-lb	2'2 3/16"	12012 ft-lb	0.080 (8%)	1.25D+1.5L	L
	Shear	973 lb	1'3 1/8"	5798 lb	0.168 (17%)	1.25D+1.5L	L
	Perm Defl in.	0.002 (L/22494)	2'1 15/16"	0.124 (L/360)	0.020 (2%)	D	Uniform
	LL Defl inch	0.005 (L/8784)	2'1 15/16"	0.124 (L/360)	0.040 (4%)	L	L
	TL Defl inch	0.007 (L/6317)	2'1 15/16"	0.185 (L/240)	0.040 (4%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

24%

16%

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



## **Design Notes**

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-4-0	1-9-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-4-0		Near Face	51 lb	137 lb	0 lb	0 lb	J9
3	Part. Uniform	1-0-0 to 3-8-0		Near Face	115 PLF	308 PLF	0 PLF	0 PLF	
4	Part. Uniform	4-1-0 to 4-7-0		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self V
5	Part. Uniform	4-1-12 to 4-7-0		Тор	67 PLF	159 PLF	0 PLF	0 PLF	J9 🚽
6	Point	4-3-12		Near Face	69 lb	165 lb	0 lb	0 lb	F13 The
	Self Weight				5 PLF				con



These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

## Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orineria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

#### Lumber

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosin
- Handling & Installation
- LVL beams must not be cut or drilled
   Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals.
- Design assumes top edge is laterally restrained
   Provide lateral support at bearing points to avoid lateral displacement and rotation
- . .

Forex APA: PR-L318

Manufacturer Info

ı	Discipline	Reviewer	BUIN	TARC
	Building Code	H. Authier	43236	2021-02-03
ı	Sewage System			
	Zoning			
		$\P$		



NE1220-141 Page 8 of 47 Client: **GREENPARK** Date: 12/17/2020 Input by: Project: RO isDesign Address: Job Name: GLENWAY 7A-ELEV. 1-DECK-R1 Project #: F3-A Level: Ground Floor 1.750" X 11.875" - PASSED Forex 2.0E-3000Fb LVL 1 2 SPF 1 Hanger (HUS1.81/10) 13'1 1/4' 13'1 1/4' Member Information Unfactored Reactions UNPATTERNED lb (Uplift) Application: Brg Dead Wind Type: Floor (Residential) Live Snow Plies: 1 Design Method: 145 86 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 0 142 84 0 2 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 107 / 217 1.25D+1.5L 3.000" 8% 324 1 Hanger Analysis Results 2 - SPF 1.875" 16% 106 / 214 319 L 1.25D+1.5L Actual Comb Case **Analysis** Location Allowed Capacity 0.059 (6%) 1.25D+1.5L L Moment 1009 ft-lb 6'7 3/16" 17130 ft-lb 6'7 3/16" 3506 ft-lb Unbraced 1009 ft-lb 0.288 (29%) 1.25D+1.5L L READ ALL NOTES ON THIS PAGE AND ON PROFESSIONAL Shear 266 lb 0.046 (5%) 1.25D+1.5L L **ENGINEERING NOTE PAGE ENP-2. THIS** 1'2 1/8" 5798 lb NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.018 (L/8724) 6'7 3/16" 0.427 (L/360) 0.040 (4%) D Uniform CALCULATION SUMMARY PAGE AS IT LL Defl inch 0.030 (L/5169) 6'7 3/16" 0.427 (L/360) 0.070 (7%) L L CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. A. EL-MASRI TL Defl inch 0.047 (L/3246) 6'7 3/16" 0.641 (L/240) 0.070 (7%) D+L L REFER TO MULTIPLE MEMBER TO MEMBER Design Notes CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. 1 Fill all hanger nailing holes. PASS THRU FRAMING SQUASH 20, 2020 2 Girders are designed to be supported on the bottom edge only. **BLOCK IS REQUIRED AT ALL** 3 Top braced at bearings. POINT LOADS OVER BEARINGS 4 Bottom braced at bearings. ID Trib Width Side Dead Wind Comments Load Type Location I ive Snow 15 PSF 1 Tie-In 0-0-0 to 13-1-4 0-6-9 Top 40 PSF 0 PSF 0 PSF Self Weight 5 PLF East Gwillimbury These plans have been reviewed for use with the corrections as noted. No other changes may be

made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all time

Discipline Reviewer BCIN Date
Building Code H. Authier 43236 2021-02-03 Sewage System

Notes

structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corr
- **Handling & Installation**
- LVL beams must not be cut or drilled
   Refer to manufacturer's product regarding installation requirement aged Beams must not be us
- Daniaged beams must not be used

  Design assumes top edge is laterally restrained

  Provide lateral support at bearing points to avoid

  lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

This design is valid until 1/8/2023

Forex

APA: PR-L318

Manufacturer Info

NE1220-141 Page 9 of 47 Client: **GREENPARK** Date: 12/17/2020 Page 5 of 21 Input by: Project: RO isDesign Address: Job Name: GLENWAY 7A-ELEV. 1-DECK-R1 Project #: Level: Ground Floor 1.750" X 11.875" - PASSED Forex 2.0E-3000Fb LVL 2 1 1 SPF 2 SPF 15'8 3/4' 15'8 3/4' Member Information Unfactored Reactions UNPATTERNED lb (Uplift) Application: Brg Dead Wind Type: Floor (Residential) Live Snow Plies: 1 Design Method: 190 129 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 0 200 137 0 2 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Cap. React D/L lb Dead: Bearing Length Total Ld. Case Ld. Comb. 1 - SPF 1.875" 162 / 284 22% 446 1 1.25D+1.5L 2 - SPF 6.875" 6% 171 / 300 471 I 1.25D+1.5L Analysis Results Actual Comb. **Analysis** Location Allowed Capacity Case 7'7 7/8" 17130 ft-lb 0.098 (10%) 1.25D+1.5L L Moment 1672 ft-lb Unbraced 1672 ft-lb 7'7 7/8" 2973 ft-lb 0.562 (56%) 1.25D+1.5L L READ ALL NOTES ON THIS PAGE AND ON PROFESSIONAL Shear 384 lb 1'1" 5798 lb 0.066 (7%) 1.25D+1.5L L ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.044 (L/4144) 7'7 15/16" 0.504 (L/360) 0.090 (9%) D Uniform CALCULATION SUMMARY PAGE AS IT LL Defl inch 0.064 (L/2852) 7'7 15/16" 0.504 (L/360) 0.130 (13%) L L CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. A. EL-MASRI TL Defl inch 0.107 (L/1689) 7'7 15/16" 0.756 (L/240) 0.140 (14%) D+L L REFER TO MULTIPLE MEMBER TO MEMBER Design Notes CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. 1 Girders are designed to be supported on the bottom edge only. PASS THRU FRAMING SQUASH 20, 2020 2 Top braced at bearings. **BLOCK IS REQUIRED AT ALL** 3 Bottom braced at bearings. POINT LOADS OVER BEARING ID Load Type Location Trib Width Side Dead Snow Wind Comments 15 PSF 40 PSF 0 PSF 0 PSF 1 Tie-In 0-0-0 to 15-8-12 0-7-7 Top 2 Part. Uniform 0-4-6 to 15-4-6 3 PLF 0 PLF 0 PLF 0 PLF Top Self Weight 5 PLF East Gwillimbury These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all time Manufacturer Info Notes Discipline Reviewer BCIN Date
Building Code H. Authier 43236 2021-02-03 **Handling & Installation** 

This design is valid until 1/8/2023

Forex

APA: PR-L318

Sewage System

CSD DESIGN

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corr

structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended

LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requirement

aged Beams must not be us Daniaged beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid

lateral displacement and rotation

NE1220-141 Page 10 of 47 Client: **GREENPARK** Date: 12/17/2020 Input by: Project: RO isDesign Address: Job Name: GLENWAY 7A-ELEV. 1-DECK-R1 Project #: Level: Ground Floor 1.750" X 11.875" - PASSED Forex 2.0E-3000Fb LVL F4-B 1 1 SPF 2 SPF 15'8 3/4' 15'8 3/4' Member Information Unfactored Reactions UNPATTERNED lb (Uplift) Application: Brg Dead Wind Type: Floor (Residential) Live Snow Plies: 1 Design Method: 193 109 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 0 204 115 0 2 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Cap. React D/L lb Dead: Bearing Length Total Ld. Case Ld. Comb. 1 - SPF 1.875" 136 / 290 21% 426 I 1.25D+1.5L 2 - SPF 6.875" 6% 144 / 306 449 1 1.25D+1.5L Analysis Results Actual Comb **Analysis** Location Allowed Capacity Case 7'7 7/8" 17130 ft-lb 0.093 (9%) 1.25D+1.5L L Moment 1591 ft-lb Unbraced 1591 ft-lb 7'7 7/8" 2973 ft-lb 0.535 (54%) 1.25D+1.5L L READ ALL NOTES ON THIS PAGE AND ON PROFESSIONAL Shear 366 lb 1'1" 5798 lb 0.063 (6%) 1.25D+1.5L L ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.037 (L/4966) 7'7 15/16" 0.504 (L/360) 0.070 (7%) D Uniform CALCULATION SUMMARY PAGE AS IT LL Defl inch 0.065 (L/2799) 7'7 15/16" 0.504 (L/360) 0.130 (13%) L L CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. A. EL-MASRI TL Defl inch 0.101 (L/1790) 7'7 15/16" 0.756 (L/240) 0.130 (13%) D+L L REFER TO MULTIPLE MEMBER TO MEMBER Design Notes CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. 1 Girders are designed to be supported on the bottom edge only. PASS THRU FRAMING SQUASH 20, 2020 2 Top braced at bearings. **BLOCK IS REQUIRED AT ALL** 3 Bottom braced at bearings. POINT LOADS OVER BEARING ID Load Type Location Trib Width Side Dead Snow Wind Comments 15 PSF 40 PSF 0 PSF 1 Tie-In 0-0-0 to 15-8-12 0-7-9 Top 0 PSF Self Weight 5 PLF East Gwillimbury These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all time Manufacturer Info Notes Discipline Reviewer BCIN Date
Building Code H. Authier 43236 2021-02-03 **Handling & Installation** Forex structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requirement APA: PR-L318 Sewage System aged Beams must not be used

This design is valid until 1/8/2023

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corr

Daniaged beams must not be used

Possign assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid

lateral displacement and rotation

NE1220-141 Page 11 of 47 Client: **GREENPARK** Date: 12/17/2020

Project: Address:

isDesign

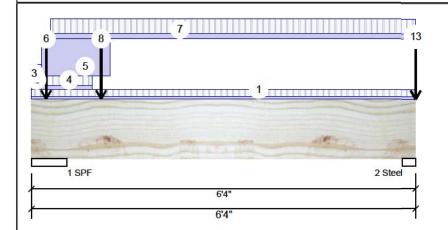
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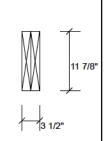
Job Name: GLENWAY 7A-ELEV. 1-DECK-R1

Project #:

1.750" X 11.875" Forex 2.0E-3000Fb LVL 2-Ply - PASSED

Level: Ground Floor





Wind

0

O

1.25D+1.5L

1.25D+1.5L

0

n

3241 L

700 1

#### Member Information Application: Floor (Residential) Type: Plies: Design Method: Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Floor Live: 40 PSF

Unfactored	Reactions	UNPATTERNE	D lb (Uplift)
Brg	Live	Dead	Snow

747

171

1538

324

1

2

1 - SPF 7.000"

2 - Steel 2.625"

Bearings and	Factored Reactions		
Bearing Leng	th Cap. React D/L	lb Total Ld. Case	Ld. Comb.

934 / 2307

214 / 486

**Analysis Results** 

Dead:

Ì	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
I	Moment	1509 ft-lb	1'1 3/4"	34261 ft-lb	0.044 (4%)	1.25D+1.5L	L
l	Unbraced	1509 ft-lb	1'1 3/4"	32772 ft-lb	0.046 (5%)	1.25D+1.5L	L
ı	Shear	1418 lb	1'6 1/8"	11596 lb	0.122 (12%)	1.25D+1.5L	L
	Perm Defl in.	0.003 (L/26040)	2'10 3/16"	0.189 (L/360)	0.010 (1%)	D	Uniform
	LL Defl inch	0.005 (L/12622)	2'9 1/2"	0.189 (L/360)	0.030 (3%)	L	L
I	TL Defl inch	0.008 (L/8502)	2'9 11/16"	0.283 (L/240)	0.030 (3%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY

24%

10%

NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



#### Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.

15 PSF

- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

7 Lateral slenderness ratio based on full section width.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Commen	East Gwillimbury
1	Tie-In	0-0-0 to 6-4-0	0-4-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF		Building Standards Branch BCIN #16487
2	Tapered Start	0-0-0		Тор	4 PLF	10 PLF	0 PLF	0 PLF		These plans have been reviewed for use with the
	End	0-2-0			4 PLF	10 PLF	0 PLF	0 PLF		corrections as noted. No other changes may be made without written approval of the Building
3	Part. Uniform	0-0-0 to 0-2-0		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self	Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the
Continued on pag	ge 2						82			Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.
							Manufactur	ror Info		



Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corn
- **Handling & Installation**
- LVL beams must not be cut or drilled
   Refer to manufacturer's product regarding installation requirement fastening details, beam strength value

naged Beams must not be used

Daniaged beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

This design is valid until 1/8/2023

Forex

Manufacturer Info APA: PR-L318

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-03
Sewage System			
Zoning			
	11		

CSD DESIGN

NE1220-141 Page 12 of 47 Client: GREENPARK Date: 12/17/2020 Page 8 of 21

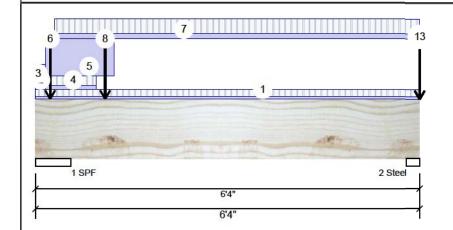
Project: isDesign Address: Input by:

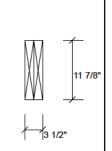
Job Name: GLENWAY 7A-ELEV. 1-DECK-R1

Project #:

1.750" X 11.875" Forex 2.0E-3000Fb LVL 2-Ply - PASSED

Level: Ground Floor





Continued fro	m page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
4	Tapered Start	0-2-0		Тор	8 PLF	21 PLF	0 PLF	0 PLF	
	End	1-0-0			8 PLF	21 PLF	0 PLF	0 PLF	
5	Part. Uniform	0-2-0 to 1-3-8		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
6	Point	0-2-14		Near Face	129 lb	329 lb	0 lb	0 lb	F1
7	Tie-In	0-3-12 to 6-4-0	0-9-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
8	Point	1-1-12		Тор	486 lb	1174 lb	0 lb	0 lb	F7 F7
11	Point	6-4-0		Тор	15 lb	40 lb	0 lb	0 lb	J8
12	Point	6-4-0		Тор	6 lb	15 lb	0 lb	0 lb	J4
13	Point	6-4-0		Тор	12 lb	0 lb	0 lb	0 lb	Wall Self Weight
	Self Weight				10 PLF				

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



These plans have been reviewed for use with the These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orineria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used

Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

4. 5.

For flat roofs provide proper drainage to prevent ponding

Forex

Manufacturer Info

APA: PR-L318

Discipline Reviewer BCIN Date
Building Code H. Authier 43236 2021-02-03 Sewage System

NE1220-141 Page 13 of 47 Client: **GREENPARK** Date: 12/17/2020

Project:

Input by:

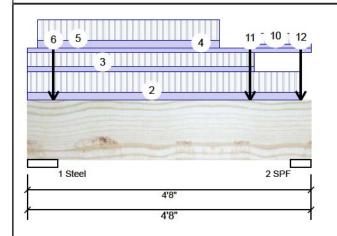
Job Name: GLENWAY 7A-ELEV. 1-DECK-R1

Project #:

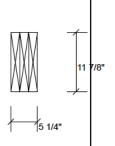
1.750" X 11.875" Forex 2.0E-3000Fb LVL 3-Ply - PASSED

Address:

Level: Ground Floor



isDesign



Member Inforn	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	3	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2015 / OBC 2012
Deflection LL:	360	Load Sharing:	Yes
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load		SA 4 4 4 1 10 A 4 5 4 5 4 5 4 5 4 5 4 5 4 5 4 5 4 5 4	
Floor Live:	40 DCE		

Unfacto	ored Reactions	UNPATTERN	NED lb (Uplift	)
Brg	Live	Dead	Snow	Wind
1	2551	1218	0	0
2	3239	1458	0	0

#### **Bearings and Factored Reactions** Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - Steel 6.000" 1522 / 3827 5349 I 1.25D+1.5L 2 - SPF 4.000" 52% 1823 / 4858 6681 I 1.25D+1.5L

**Analysis Results** 

Dead:

Γ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	4169 ft-lb	2'5"	53447 ft-lb	0.078 (8%)	1.25D+1.5L	L
	Unbraced	4169 ft-lb	2'5"	53447 ft-lb	0.078 (8%)	1.25D+1.5L	L
	Shear	3405 lb	3'4 7/8"	17394 lb	0.196 (20%)	1.25D+1.5L	L
	Perm Defl in.	0.004 (L/13447)	2'5 1/16"	0.132 (L/360)	0.030 (3%)	D	Uniform
	LL Defl inch	0.008 (L/6292)	2'5 1/16"	0.132 (L/360)	0.060 (6%)	L	L
	TL Defl inch	0.011 (L/4287)	2'5 1/16"	0.198 (L/240)	0.060 (6%)	D+L	L

**Design Notes** 

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.

15 PSF

- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- based on full section width

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY

NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



/ Latera	ii sienderness ratio based o	on tuli section wiath.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Commen
2	Part. Uniform	0-0-0 to 4-5-12		Тор	143 PLF	380 PLF	0 PLF	0 PLF	J2
3	Part. Uniform	0-0-0 to 3-8-12		Тор	99 PLF	263 PLF	0 PLF	0 PLF	J8
4	Part. Uniform	0-0-0 to 4-8-0		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self V
5	Part. Uniform	0-2-0 to 3-2-0		Near Face	142 PLF	379 PLF	0 PLF	0 PLF	n S
6	Point	0-5-2		Far Face	86 lb	145 lb	0 lb	0 lb	F3 o
Continued	on page 2								a <sub>l</sub>



hese plans have been reviewed for use with the These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Reviewer BCIN Date

Building Code	H. Authier	43236	2021-02-03
Sewage System			
Zoning			
	U		

Notes

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   LVL not to be treated with fire retardant or corro
- **Handling & Installation**
- LVL beams must not be cut or drilled
   Refer to manufacturer's product regarding installation requirement
- naged Beams must not be used
- Daniaged beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent

Forex APA: PR-L318

Manufacturer Info

This design is valid until 1/8/2023

Discipline

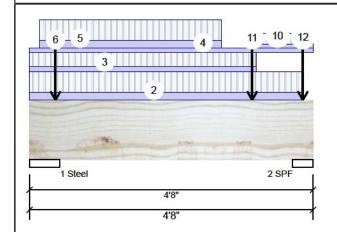
NE1220-141 Page 14 of 47 Client: GREENPARK Date: 12/17/2020 Page 10 of 21 Project: Input by: isDesign

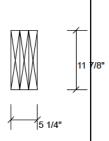
Address: Job Name: GLENWAY 7A-ELEV. 1-DECK-R1

Project #:

1.750" X 11.875" 3-Ply - PASSED Forex 2.0E-3000Fb LVL

Level: Ground Floor





Continued froi	n page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
10	Part. Uniform	3-6-12 to 4-8-0		Тор	73 PLF	196 PLF	0 PLF	0 PLF	J8
11	Point	3-8-0		Near Face	140 lb	374 lb	0 lb	0 lb	J2
12	Point	4-6-0		Тор	494 lb	1235 lb	0 lb	0 lb	F5 F5
	Self Weight				14 PLF				

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



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Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orineria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- 1. LVL beams must not be cut or drilled
  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
  3. Damaged Beams must not be used
- Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation 4. 5.
- For flat roofs provide proper drainage to prevent ponding

Forex

Manufacturer Info

APA: PR-L318

Discipline Reviewer BCIN Date
Building Code H. Authier 43236 2021-02-03 Sewage System



## F9-A

Dry | 1 span | No cant.

Page 15 of 47 **PASSED** 

**BC CALC® Member Report** 

**Build 7364** 

Code reports:

December 17, 2020 12:29:49

Job name:

Address:

City, Province, Postal Code: Customer:

CCMC 12787-R

File name:

S:\CUSTOMERS\GREENPA...OOD 4-ELEV. 2-R1.isl

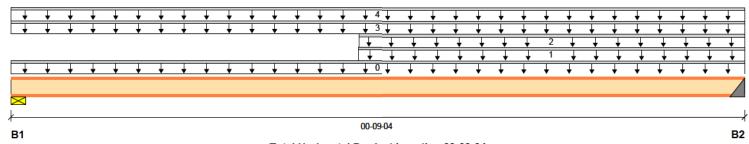
Description: Level - Ground Floor

Specifier:

Designer: R<sub>0</sub>

Company: **GREENPARK** 

Wind



Total Horizontal Product Length = 00-09-04

Snow

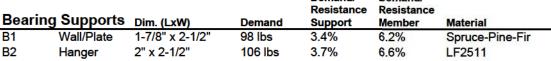
Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	
B1, 1-7/8"	49 / 0	20 / 0	
R2 2"	50 / 0	25 / 0	

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	00-09-04	Тор		3			00-00-00
1		Unf. Lin. (lb/ft)	L	00-04-06	00-09-04	Тор		7			n\a
2		Unf. Lin. (lb/ft)	L	00-04-06	00-09-04	Тор		9			n\a
3		Unf. Lin. (lb/ft)	L	00-00-00	00-09-04	Тор	54	20			n\a
4		Unf. Lin. (lb/ft)	L	00-00-00	00-09-04	Тор	74	28			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	11 ft-lbs	5305 ft-lbs	0.2%	1	00-04-10
End Reaction	106 lbs	1607 lbs	6.6%	1	00-09-04
End Shear	60 lbs	2350 lbs	2.6%	1	00-07-04
Total Load Deflection	L/999 (0")	n\a	n\a	4	00-04-10
Live Load Deflection	L/999 (0")	n\a	n\a	5	00-04-09
Max Defl.	0"	n\a	n\a	4	00-04-10
Span / Depth	0.6				

Bear	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	1-7/8" x 2-1/2"	98 lbs	3.4%	6.2%	Spruce-Pine-Fir
B2	Hanger	2" x 2-1/2"	106 lbs	3.7%	6.6%	LF2511



### Cautions

Hanger LF2511 requires (12) 10d face nails, (1) #8x1.25 joist nails. Header for the hanger LF2511 is a Single 3-1/2" x 11-7/8" I-joist



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Authier	43236	2021-02-03
		2021-02-03

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



## F9-A

Dry | 1 span | No cant.

Page 16 of 47 **PASSED** 

December 17, 2020 12:29:49

**BC CALC® Member Report** 

**Build 7364** 

Job name: S:\CUSTOMERS\GREENPA...OOD 4-ELEV. 2-R1.isl File name:

Address: Description: Level - Ground Floor

City, Province, Postal Code: Specifier:

Customer: Designer: R<sub>0</sub>

Code reports: CCMC 12787-R Company: **GREENPARK** 

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced. Hanger Manufacturer: Simpson Strong-Tie, Inc.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

## Disclosure

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CONTAINS SPECIFICATIONS AND CRITERIA

USED IN THE DESIGN OF THIS COMPONENT.

**ENGINEERING NOTE PAGE ENP-2. THIS** 

CALCULATION SUMMARY PAGE AS IT

obtain Instance | Reviewer | BCIN | Date | Discipline | Reviewer | BCIN | Date | Discipline | H. Authier | 43236 | 2021-02-03 BC CALCEL

ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



Job name:

Customer:

## Single 11-7/8" AJS® 140

### F9-B

Dry | 1 span | No cant. December 17, 2020 12:29:49

S:\CUSTOMERS\GREENPA...OOD 4-ELEV. 2-R1.isl

**BC CALC® Member Report Build 7364** 

File name:

Designer:

R<sub>0</sub>

Address: Description: Level - Ground Floor

City, Province, Postal Code: Specifier:

Code reports: CCMC 12787-R Company: **GREENPARK** 

00-09-04 B2

Total Horizontal Product Length = 00-09-04

Reaction Summary (Down / Uplift) (lbs)

mouotion out		, p, (1.20)			
Bearing	Live	Dead	Snow	Wind	
B1, 1-7/8"	21 / 0	9/0			
B2, 2"	21 / 0	11 / 0			

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	00-09-04	Тор		3			00-00-00
1		Unf. Lin. (lb/ft)	L	00-04-06	00-09-04	Top		7			n\a
2		Unf. Lin. (lb/ft)	L	00-00-00	00-09-04	Тор	54	20			n\a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	5 ft-lbs	5305 ft-lbs	n\a	1	00-04-10
End Reaction	46 lbs	1607 lbs	2.8%	1	00-09-04
End Shear	26 lbs	2350 lbs	1.1%	1	00-07-04
Total Load Deflection	L/999 (0")	n\a	n\a	4	00-04-10
Max Defl.	0"	n\a	n\a	4	00-04-10
Span / Depth	0.6				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	1-7/8" x 2-1/2"	42 lbs	1.5%	2.7%	Spruce-Pine-Fir
B2	Hanger	2" x 2-1/2"	46 lbs	1.6%	2.8%	LF2511

#### Cautions

Hanger LF2511 requires (12) 10d face nails, (1) #8x1.25 joist nails. Header for the hanger LF2511 is a Single 3-1/2" x 11-7/8" I-joist

## Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Simpson Strong-Tie, Inc.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

> READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Page 17 of 47

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obtain Instance | Reviewer | BCIN | Date | Discipline | Reviewer | BCIN | Date | Discipline | H. Authier | 43236 | 2021-02-03 before insta BC CALCEL

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F9-C

Page 18 of 47 **PASSED** 

**BC CALC® Member Report** 

Dry | 1 span | No cant.

December 17, 2020 12:29:49

S:\CUSTOMERS\GREENPA...OOD 4-ELEV. 2-R1.isl

**Build 7364** 

Address:

Customer:

Job name:

Description: Level - Ground Floor

City, Province, Postal Code:

Specifier:

File name:

Designer: R<sub>0</sub>

Code reports: CCMC 12787-R Company: **GREENPARK** 

00-11-06 B2

Total Horizontal Product Length = 00-11-06

Reaction Summary (Down / Uplift) (lbs)

ntouction out		P(1, (1,2,0)			
Bearing	Live	Dead	Snow	Wind	
B1, 1-7/8"	48 / 0	19 / 0			
B2, 2"	49 / 0	20 / 0			

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	00-11-06	Тор		3			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	00-11-06	Top	49	18			n\a
2		Unf. Lin. (lb/ft)	L	00-00-00	00-11-06	Top	54	20			n\a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	14 ft-lbs	5305 ft-lbs	0.3%	1	00-05-10
End Reaction	98 lbs	1607 lbs	6.1%	1	00-11-06
End Shear	64 lbs	2350 lbs	2.7%	1	00-01-14
Total Load Deflection	L/999 (0")	n\a	n\a	4	00-05-10
Live Load Deflection	L/999 (0")	n\a	n\a	5	00-05-10
Max Defl.	0"	n\a	n\a	4	00-05-10
Span / Depth	0.8				

Bearing	J Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	1-7/8" x 2-1/2"	96 lbs	3.3%	6.1%	Spruce-Pine-Fir
B2	Hanger	2" x 2-1/2"	98 lbs	4.4%	6.1%	LF2511

## Cautions

Hanger LF2511 requires (12) 10dx1.5 face nails, (1) #8x1.25 joist nails.

Header for the hanger LF2511 is a Single 2-1/2" x 11-7/8" I-joist

## Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Simpson Strong-Tie, Inc.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

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Publisher Contents engineered Ontario Building Code, as ameleded. These accordance times. The building permit must be kept on site at all Guide and posted on site at all times.

obtain Instance | Reviewer | BCIN | Date | Discipline | Reviewer | BCIN | Date | Discipline | H. Authier | 43236 | 2021-02-03 before insta BC CALCEL

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**BC CALC® Member Report** 

## Single 11-7/8" AJS® 140

### F9-D

Dry | 1 span | No cant.

Designer:

R<sub>0</sub>

Page 19 of 47 **PASSED** 

December 17, 2020 12:29:49

**Build 7364** 

Customer:

Job name: S:\CUSTOMERS\GREENPA...OOD 4-ELEV. 2-R1.isl File name:

Address: Description: Level - Ground Floor

City, Province, Postal Code: Specifier:

Code reports: CCMC 12787-R Company: **GREENPARK** 

00-11-06 **B1** B2

Total Horizontal Product Length = 00-11-06

Peaction Summary (Down / Unlift) (lbe)

Neaction Sui	ililialy (Dowll / C	pilit) (lba)			
Bearing	Live	Dead	Snow	Wind	
B1, 1-7/8"	25 / 0	11 / 0			
B2, 2"	26 / 0	11 / 0			

Lo	Load Summary							Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	00-11-06	Тор		3			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	00-11-06	Top	54	20			n\a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	8 ft-lbs	5305 ft-lbs	0.1%	1	00-05-10
End Reaction	53 lbs	1607 lbs	3.3%	1	00-11-06
End Shear	34 lbs	2350 lbs	1.5%	1	00-01-14
Total Load Deflection	L/999 (0")	n\a	n\a	4	00-05-10
Live Load Deflection	L/999 (0")	n\a	n\a	5	00-05-10
Max Defl.	0"	n\a	n\a	4	00-05-10
Span / Depth	0.8				

Bear	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	1-7/8" x 2-1/2"	51 lbs	1.8%	3.2%	Spruce-Pine-Fir
B2	Hanger	2" x 2-1/2"	53 lbs	2.4%	3.3%	LF2511

#### Cautions

Hanger LF2511 requires (12) 10dx1.5 face nails, (1) #8x1.25 joist nails.

Header for the hanger LF2511 is a Single 2-1/2" x 11-7/8" I-joist

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Simpson Strong-Tie, Inc.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

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## F10-A

Dry | 1 span | No cant.

Page 20 of 47 **PASSED** 

December 17, 2020 12:29:49

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**BC CALC® Member Report** 

Job name:

Customer:

**Build 7364** 

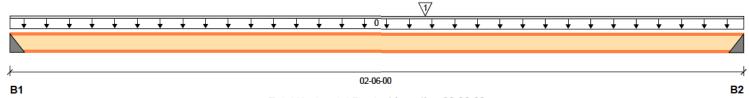
Address: Description: Level - Ground Floor City, Province, Postal Code:

Specifier:

File name:

Designer: R<sub>0</sub>

Code reports: CCMC 12787-R Company: **GREENPARK** 



Total Horizontal Product Length = 02-06-00

Reaction Summary (Down / Unlift) (lbs)

Neaction 0	unimary (Down / Op				
Bearing	Live	Dead	Snow	Wind	
B1, 2"	134 / 0	54 / 0			
B2, 2"	179 / 0	71 / 0			

Loa	Load Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start _	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	02-06-00	Тор		3			00-00-00
1	J7	Conc. Pt. (lbs)	L	01-05-00	01-05-00	Front	313	118			n\a

Controls Summary	Footoned Domand	Factored	Demand/ Resistance	Case	Location
	Factored Demand	Resistance		Case	
Pos. Moment	348 ft-lbs	5305 ft-lbs	6.6%	1	01-05-00
End Reaction	357 lbs	1607 lbs	22.2%	1	02-06-00
End Shear	357 lbs	2350 lbs	15.2%	1	02-04-00
Total Load Deflection	L/999 (0.004")	n\a	n\a	4	01-05-00
Live Load Deflection	L/999 (0.003")	n\a	n\a	5	01-05-00
Max Defl.	0.004"	n\a	n\a	4	01-05-00
Span / Depth	2.3				

Bear	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Hanger	2" x 2-1/2"	268 lbs	12.1%	16.6%	LF2511
B2	Hanger	2" x 2-1/2"	357 lbs	16.1%	22.2%	LF2511

## Cautions Disclosure Hanger LF2511 requires (12) 10dx1.5 face nails, (1) #8x1.25 joist nails.

READ ALL NOTES ON THIS PAGE AND ON

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CONTAINS SPECIFICATIONS AND CRITERIA

USED IN THE DESIGN OF THIS COMPONENT.

ENGINEERING NOTE PAGE ENP-2. THIS

CALCULATION SUMMARY PAGE AS IT

Header for the hanger LF2511 is a Single 2-1/2" x 11-7/8" I-joist

Header for the hanger LF2511 is a Single 2-1/2" x 11-7/8" I-joist

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Simpson Strong-Tie, Inc.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



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Authier	43236	2021-02-03
		2021-02-03

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## F10-B

Dry | 1 span | No cant.

Page 21 of 47 **PASSED** 

December 17, 2020 12:29:49

**BC CALC® Member Report** 

City, Province, Postal Code:

**Build 7364** 

Address:

Customer:

Job name: File name:

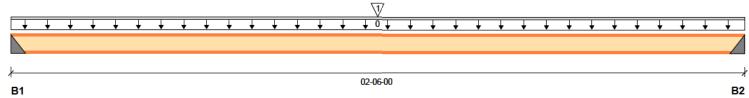
S:\CUSTOMERS\GREENPA...OOD 4-ELEV. 2-R1.isl

Description: Level - Ground Floor

Specifier:

Designer: R<sub>0</sub>

Code reports: CCMC 12787-R Company: **GREENPARK** 



Total Horizontal Product Length = 02-06-00

Reaction Summary (Down / Unlift) (lbs)

iteaction o	uniniary (Down / Op				
Bearing	Live	Dead	Snow	Wind	
B1, 2"	185 / 0	72 / 0			
B2, 2"	184 / 0	72 / 0			

Loa	Load Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start _	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	02-06-00	Тор		3			00-00-00
1	J9	Conc. Pt. (lbs)	L	01-03-00	01-03-00	Back	369	138			n\a

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USED IN THE DESIGN OF THIS COMPONENT.

ENGINEERING NOTE PAGE ENP-2. THIS

CALCULATION SUMMARY PAGE AS IT

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	418 ft-lbs	5305 ft-lbs	7.9%	1	01-03-00
End Reaction	367 lbs	1607 lbs	22.8%	1	00-00-00
End Shear	367 lbs	2350 lbs	15.6%	1	00-02-00
Total Load Deflection	L/999 (0.005")	n\a	n\a	4	01-03-00
Live Load Deflection	L/999 (0.003")	n\a	n\a	5	01-03-00
Max Defl.	0.005"	n\a	n\a	4	01-03-00
Span / Depth	2.3				

Bear	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Hanger	2" x 2-1/2"	367 lbs	16.6%	22.8%	LF2511
B2	Hanger	2" x 2-1/2"	367 lbs	16.6%	22.8%	LF2511

# Cautions

Hanger LF2511 requires (12) 10dx1.5 face nails, (1) #8x1.25 joist nails.

Header for the hanger LF2511 is a Single 2-1/2" x 11-7/8" I-joist

Header for the hanger LF2511 is a Single 2-1/2" x 11-7/8" I-joist

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Simpson Strong-Tie, Inc.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



## Disclosure

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qualified engineer or other approexpert to a: East Gwillimbury

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Zoning By-Law 2018-043, as amended, and the
engineered observed documents must be kept on site at all
times. The building permit must be clearly
posted on site at all times.

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Building Code			
Bulluing Code	H. Authier	43236	2021-02-03
Sewage System			
Zoning			

BC CALC® ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



#### F10-C

Dry | 1 span | No cant.

Page 22 of 47 **PASSED** 

December 17, 2020 12:29:49

**BC CALC® Member Report** 

City, Province, Postal Code:

**Build 7364** Job name:

Address:

Customer:

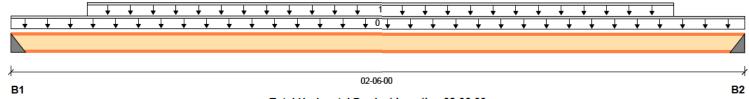
S:\CUSTOMERS\GREENPA...OOD 4-ELEV. 2-R1.isl File name:

Description: Level - Ground Floor

Specifier:

Designer: R<sub>0</sub>

Code reports: CCMC 12787-R Company: **GREENPARK** 



Total Horizontal Product Length = 02-06-00

Reaction Summary (Down / Unlift) (lbs)

redection of	anninary (Bown / Op				
Bearing	Live	Dead	Snow	Wind	
B1, 2"	328 / 0	148 / 0			
B2, 2"	333 / 0	150 / 0			

Loa	Load Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	02-06-00	Тор		3			00-00-00
1		Unf. Lin. (lb/ft)	L	00-03-02	02-03-02	Back	330	146			n\a

READ ALL NOTES ON THIS PAGE AND ON

NOTE PAGE IS AN INTEGRAL PART OF THIS

CONTAINS SPECIFICATIONS AND CRITERIA

USED IN THE DESIGN OF THIS COMPONENT.

ENGINEERING NOTE PAGE ENP-2. THIS

CALCULATION SUMMARY PAGE AS IT

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	440 ft-lbs	5305 ft-lbs	8.3%	1	01-03-00
End Reaction	687 lbs	1607 lbs	42.7%	1	02-06-00
End Shear	687 lbs	2350 lbs	29.2%	1	02-04-00
Total Load Deflection	L/999 (0.005")	n\a	n\a	4	01-03-00
Live Load Deflection	L/999 (0.004")	n\a	n\a	5	01-03-00
Max Defl.	0.005"	n\a	n\a	4	01-03-00
Span / Depth	2.3				

Bear	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Hanger	2" x 2-1/2"	677 lbs	23.5%	42.1%	LF2511
B2	Hanger	2" x 2-1/2"	687 lbs	23.9%	42.7%	LF2511

#### Cautions

Hanger LF2511 requires (12) 10d face nails, (1) #8x1.25 joist nails. Header for the hanger LF2511 is a Single 3-1/2" x 11-7/8" I-joist

Header for the hanger LF2511 is a Single 3-1/2" x 11-7/8" I-joist

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Simpson Strong-Tie, Inc.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



## Disclosure

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Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-03
Sewage System			
Zoning			

BC CALC® ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



#### F11-A

Designer:

R<sub>0</sub>

Dry | 1 span | No cant. December 17, 2020 12:29:49

**BC CALC® Member Report** 

**Build 7364** 

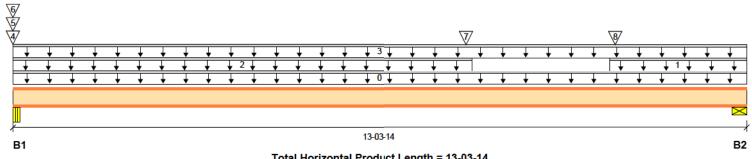
Customer:

Job name: S:\CUSTOMERS\GREENPA...OOD 4-ELEV. 2-R1.isl File name:

Address: Description: Level - Ground Floor

City, Province, Postal Code: Specifier:

Code reports: CCMC 12787-R Company: **GREENPARK** 



Total Horizontal Product Length = 13-03-14

## Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 2-5/8"	315 / 0	139 / 0		
B2, 1-7/8"	310 / 0	135 / 0		

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	13-03-14	Тор		3			00-00-00
1		Unf. Lin. (lb/ft)	L	10-10-00	13-03-14	Тор	22	8			n\a
2		Unf. Lin. (lb/ft)	L	00-00-00	08-04-00	Тор	22	8			n\a
3		Unf. Lin. (lb/ft)	L	00-00-00	13-03-14	Тор	22	8			n\a
4	J8	Conc. Pt. (lbs)	L	00-00-00	00-00-00	Top	7	3			n\a
5	J2	Conc. Pt. (lbs)	L	00-00-00	00-00-00	Top	9	3			n\a
6	Wall Self Weight	Conc. Pt. (lbs)	L	00-00-00	00-00-00	Top		3			n\a
7	F9	Conc. Pt. (lbs)	L	08-02-12	08-02-12	Back	26	11			n\a
8	F9	Conc. Pt. (lbs)	L	10-11-04	10-11-04	Back	49	20			n\a

<b>Controls Summary</b>	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1975 ft-lbs	5305 ft-lbs	37.2%	1	06-08-05
End Reaction	634 lbs	1582 lbs	40.1%	1	13-03-14
End Shear	620 lbs	2350 lbs	26.4%	1	13-02-00
Total Load Deflection	L/1000 (0.157")	n\a	24.0%	4	06-08-05
Live Load Deflection	L/999 (0.109")	n\a	n\a	5	06-08-05
Max Defl.	0.157"	n\a	15.7%	4	06-08-05
Span / Depth	13.2				

Bear	ring Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Beam	2-5/8" x 2-1/2"	646 lbs	0.2%	37.2%	Steel
B2	Wall/Plate	1-7/8" x 2-1/2"	634 lbs	22.0%	40.1%	Spruce-Pine-Fir



Page 23 of 47

**PASSED** 



These plans have been reviewed for use with the These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-03
Sewage System			
Zoning			



## F11-A

Dry | 1 span | No cant.

Page 24 of 47 **PASSED** 

December 17, 2020 12:29:49

**BC CALC® Member Report** 

City, Province, Postal Code:

**Build 7364** 

Job name:

S:\CUSTOMERS\GREENPA...OOD 4-ELEV. 2-R1.isl File name:

Address: Description: Level - Ground Floor

Specifier:

Customer: Designer: R<sub>0</sub>

Code reports: CCMC 12787-R Company: **GREENPARK** 

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

## Disclosure

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posted on site at all times.

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obtain Instance | Reviewer | BCIN | Date | Discipline | Reviewer | BCIN | Date | Discipline | H. Authier | 43236 | 2021-02-03 BC CALCEL

ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

USED IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON



## F11-B

Dry | 1 span | No cant.

Page 25 of 47 **PASSED** 

**BC CALC® Member Report** 

December 17, 2020 12:29:49

**Build 7364** Job name:

Address:

City, Province, Postal Code:

Customer:

Code reports: CCMC 12787-R

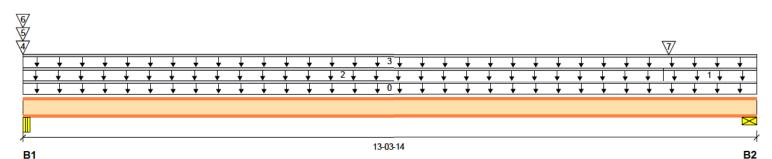
S:\CUSTOMERS\GREENPA...OOD 4-ELEV. 2-R1.isl File name:

Description: Level - Ground Floor

Wind

Specifier: Designer:

R<sub>0</sub> Company: **GREENPARK** 



Total Horizontal Product Length = 13-03-14

Snow

Reaction Summary (Down / Uplift) (lbs)

B1, 2-5/8" 488 / 0 224 / 0 B2, 1-7/8" 561/0 230 / 0

REFER TO MULTIPLE MEMBER TO MEM CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	13-03-14	Тор		3			00-00-00
1		Unf. Lin. (lb/ft)	L	11-07-08	13-03-14	Тор	54	20			n\a
2		Unf. Lin. (lb/ft)	L	00-00-00	11-07-08	Тор	24	9			n\a
3		Unf. Lin. (lb/ft)	L	00-00-00	13-03-14	Top	30	11			n\a
4	J8	Conc. Pt. (lbs)	L	00-00-00	00-00-00	Top	78	29			n\a
5	J4	Conc. Pt. (lbs)	L	00-00-00	00-00-00	Top	30	11			n\a
6	Wall Self Weight	Conc. Pt. (lbs)	L	00-00-00	00-00-00	Top		24			n\a
7	F10	Conc. Pt. (lbs)	L	11-08-12	11-08-12	Back	179	71			2/2

<b>Controls Summary</b>	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	2626 ft-lbs	5305 ft-lbs	49.5%	1	07-00-13
End Reaction	1129 lbs	1582 lbs	71.4%	1	13-03-14
End Shear	1103 lbs	2350 lbs	46.9%	1	13-02-00
Total Load Deflection	L/742 (0.211")	n\a	32.3%	4	06-10-14
Live Load Deflection	L/1053 (0.149")	n\a	34.2%	5	06-10-14
Max Defl.	0.211"	n\a	21.1%	4	06-10-14
Span / Depth	13.2				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Beam	2-5/8" x 2-1/2"	1013 lbs	0.4%	58.3%	Steel
B2	Wall/Plate	1-7/8" x 2-1/2"	1129 lbs	39.1%	71.4%	Spruce-Pine-Fir

## **Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

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## F11-C

Page 26 of 47 **PASSED** 

December 17, 2020 12:29:49

**BC CALC® Member Report** 

Dry | 1 span | No cant.

**Build 7364** 

Job name:

Address:

City, Province, Postal Code:

Customer:

Designer:

Code reports: CCMC 12787-R

S:\CUSTOMERS\GREENPA...OOD 4-ELEV. 2-R1.isl File name:

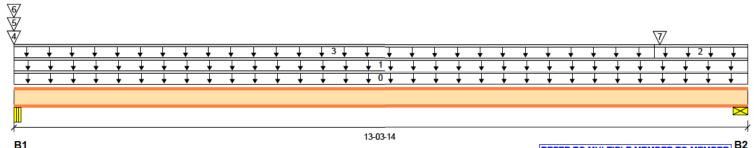
Description: Level - Ground Floor

Wind

Specifier:

R<sub>0</sub>

Company: **GREENPARK** 



Total Horizontal Product Length = 13-03-14

Snow

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Reaction Summary (Down / Uplift) (lbs)

B1, 2-5/8" 406 / 0 171 / 0 B2, 1-7/8" 535 / 0 221/0

Lo	ad Summary					Live	Dead	Snow	Wind	Tributary	
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	13-03-14	Тор		3			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	13-03-14	Тор	27	10			n\a
2		Unf. Lin. (lb/ft)	L	11-07-08	13-03-14	Тор	54	20			n\a
3		Unf. Lin. (lb/ft)	L	00-00-00	11-07-08	Тор	30	11			n\a
4	J8	Conc. Pt. (lbs)	L	00-00-00	00-00-00	Top	4	2			n\a
5	J4	Conc. Pt. (lbs)	L	00-00-00	00-00-00	Top	2	1			n\a
6	Wall Self Weight	Conc. Pt. (lbs)	L	00-00-00	00-00-00	Тор		1			n\a
7	F10	Conc. Pt. (lbs)	L	11-08-12	11-08-12	Front	134	54			2/2

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	2705 ft-lbs	5305 ft-lbs	51.0%	1	06-11-13
End Reaction	1078 lbs	1582 lbs	68.2%	1	13-03-14
End Shear	1053 lbs	2350 lbs	44.8%	1	13-02-00
Total Load Deflection	L/723 (0.217")	n\a	33.2%	4	06-08-14
Live Load Deflection	L/1025 (0.153")	n\a	35.1%	5	06-08-14
Max Defl.	0.217"	n\a	21.7%	4	06-08-14
Span / Depth	13.2				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Beam	2-5/8" x 2-1/2"	822 lbs	0.3%	47.4%	Steel
B2	Wall/Plate	1-7/8" x 2-1/2"	1078 lbs	37.4%	68.2%	Spruce-Pine-Fir

## **Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

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Job name:

Customer:

## Single 11-7/8" AJS® 140

### F13-A

File name:

Designer:

R<sub>0</sub>

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**BC CALC® Member Report** 

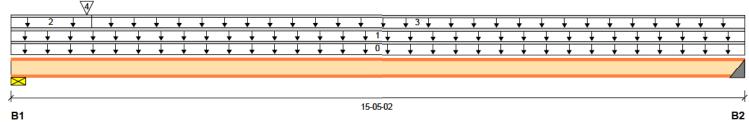
**Build 7364** 

Description: Level - Ground Floor

Address:

City, Province, Postal Code: Specifier:

Code reports: CCMC 12787-R Company: **GREENPARK** 



Total Horizontal Product Length = 15-05-02

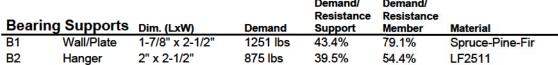
Reaction Summary (Down / Unlift) (lbs)

Neaction Out	illiary (Down / O				
Bearing	Live	Dead	Snow	Wind	
B1, 1-7/8"	622 / 0	255 / 0			
B2, 2"	432 / 0	182 / 0			

Load Summary							Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	15-05-02	Тор		3			00-00-00
1	_	Unf. Lin. (lb/ft)	L	00-00-00	15-05-02	Тор	26	10			n∖a
2		Unf. Lin. (lb/ft)	L	00-00-00	01-08-06	Тор	54	20			n∖a
3		Unf. Lin. (lb/ft)	L	01-08-06	15-05-02	Тор	27	10			n\a
4	F10	Conc. Pt. (lbs)	L	01-07-02	01-07-02	Front	185	72			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	3452 ft-lbs	5305 ft-lbs	65.1%	1	07-04-12
End Reaction	1251 lbs	1582 lbs	79.1%	1	00-00-00
End Shear	1226 lbs	2350 lbs	52.2%	1	00-01-14
Total Load Deflection	L/502 (0.364")	n\a	47.8%	4	07-07-02
Live Load Deflection	L/712 (0.257")	n\a	50.5%	5	07-07-02
Max Defl.	0.364"	n\a	36.4%	4	07-07-02
Span / Depth	15.4				

Bearin	ng Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	1-7/8" x 2-1/2"	1251 lbs	43.4%	79.1%	Spruce-Pine-Fir
B2	Hanger	2" x 2-1/2"	875 lbs	39.5%	54.4%	LF2511



## Cautions

Hanger LF2511 requires (12) 10dx1.5 face nails, (1) #8x1.25 joist nails. Header for the hanger LF2511 is a Single 1-3/4" x 11-7/8" LVL beam



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These plans have been reviewed for use with the These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-03
Sewage System			
Zoning			

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL

POINT LOADS OVER BEARINGS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



## F13-A

Dry | 1 span | No cant.

Page 28 of 47 **PASSED** 

December 17, 2020 12:29:49

**BC CALC® Member Report** 

City, Province, Postal Code:

**Build 7364** Job name:

Address:

S:\CUSTOMERS\GREENPA...OOD 4-ELEV. 2-R1.isl File name:

Description: Level - Ground Floor

Specifier:

Customer: Designer: R<sub>0</sub>

Code reports: CCMC 12787-R Company: **GREENPARK** 

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced. Hanger Manufacturer: Simpson Strong-Tie, Inc.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

## Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appre

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BC CALCEL

READ ALL NOTES ON THIS PAGE AND ON

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**ENGINEERING NOTE PAGE ENP-2. THIS** 

CALCULATION SUMMARY PAGE AS IT

obtain Instance | Reviewer | BCIN | Date | Discipline | Reviewer | BCIN | Date | Discipline | H. Authier | 43236 | 2021-02-03

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## F13-B

December 17, 2020 12:29:49 Dry | 1 span | No cant.

**BC CALC® Member Report** 

**Build 7364** 

Customer:

Job name: S:\CUSTOMERS\GREENPA...OOD 4-ELEV. 2-R1.isl File name:

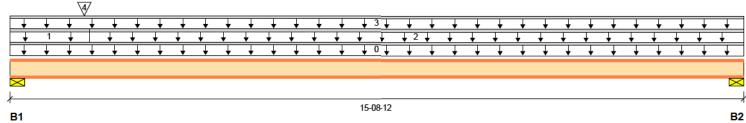
Designer:

R<sub>0</sub>

Address: Description: Level - Ground Floor

City, Province, Postal Code: Specifier:

Code reports: CCMC 12787-R Company: **GREENPARK** 



Total Horizontal Product Length = 15-08-12

## Reaction Summary (Down / Unlift) (lbs)

Reaction Outlina				
Bearing	Live	Dead	Snow	Wind
B1, 1-7/8"	618 / 0	253 / 0		
B2, 6-7/8"	451 / 0	190 / 0		

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	15-08-12	Тор		3			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	01-08-06	Тор	54	20			n\a
2		Unf. Lin. (lb/ft)	L	01-08-06	15-08-12	Тор	27	10			n\a
3		Unf. Lin. (lb/ft)	L	00-00-00	15-08-12	Тор	26	10			n\a
4	F10	Conc. Pt. (lbs)	L	01-07-02	01-07-02	Back	184	72			n\a

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ENGINEERING NOTE PAGE ENP-2. THIS

CALCULATION SUMMARY PAGE AS IT

<b>Controls Summary</b>	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	3408 ft-lbs	5305 ft-lbs	64.2%	1	07-04-03
End Reaction	1243 lbs	1582 lbs	78.6%	1	00-00-00
End Shear	1218 lbs	2350 lbs	51.8%	1	00-01-14
Total Load Deflection	L/512 (0.355")	n\a	46.9%	4	07-06-08
Live Load Deflection	L/726 (0.25")	n\a	49.6%	5	07-06-08
Max Defl.	0.355"	n\a	35.5%	4	07-06-08
Span / Depth	15.3				

Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material	
B1	Wall/Plate	1-7/8" x 2-1/2"	1243 lbs	43.1%	78.6%	Spruce-Pine-Fir	
B2	Wall/Plate	6-7/8" x 2-1/2"	914 lbs	8.6%	47.7%	Spruce-Pine-Fir	

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



Page 29 of 47

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#### Disclosure

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## F15-A

Dry | 1 span | No cant.

Page 30 of 47 **PASSED** 

December 17, 2020 12:29:49

**BC CALC® Member Report** 

Job name:

Customer:

**Build 7364** 

S:\CUSTOMERS\GREENPA...OOD 4-ELEV. 2-R1.isl File name:

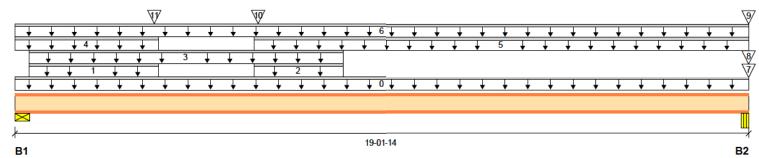
Designer:

R<sub>0</sub>

Address: Description: Level - Ground Floor

City, Province, Postal Code: Specifier:

Code reports: CCMC 12787-R Company: **GREENPARK** 



Total Horizontal Product Length = 19-01-14

Neaction Sui					
Bearing	Live	Dead	Snow	Wind	
B1, 1-7/8"	394 / 0	209 / 0			
B2, 2-5/8"	504 / 0	256 / 0			

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	19-01-14	Тор		3			00-00-00
1		Unf. Lin. (lb/ft)	L	00-04-06	03-08-14	Тор		2			n\a
2		Unf. Lin. (lb/ft)	L	06-02-14	08-06-11	Тор		2			n\a
3		Unf. Lin. (lb/ft)	L	00-04-06	08-06-12	Тор		2			n\a
4		Unf. Lin. (lb/ft)	L	00-00-00	03-08-14	Тор	19	7			n\a
5		Unf. Lin. (lb/ft)	L	06-02-14	19-01-14	Top	19	7			n\a
6		Unf. Lin. (lb/ft)	L	00-00-00	19-01-14	Тор	20	8			n\a
7	J8	Conc. Pt. (lbs)	L	19-01-14	19-01-14	Тор	51	23			n\a
8	J2	Conc. Pt. (lbs)	L	19-01-14	19-01-14	Тор	72	27			n\a
9	Wall Self Weight	Conc. Pt. (lbs)	L	19-01-14	19-01-14	Тор		22			n\a
10	F9	Conc. Pt. (lbs)	L	06-04-02	06-04-02	Back	21	11			n\a
11	F9	Conc. Pt. (lbs)	L	03-07-10	03-07-10	Back	50	25			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	3816 ft-lbs	8640 ft-lbs	44.2%	1	09-05-05
End Reaction	1076 lbs	1736 lbs	62.0%	1	19-01-14
End Shear	839 lbs	2350 lbs	35.7%	1	00-01-14
Total Load Deflection	L/543 (0.418")	n\a	44.2%	4	09-05-05
Live Load Deflection	L/818 (0.277")	n\a	44.0%	5	09-05-05
Max Defl.	0.418"	n\a	41.8%	4	09-05-05
Span / Depth	19.1				

Beari	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	1-7/8" x 3-1/2"	852 lbs	21.1%	53.8%	Spruce-Pine-Fir
B2	Beam	2-5/8" x 3-1/2"	1076 lbs	0.3%	62.0%	Steel



These plans have been reviewed for use with the These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-03
Sewage System			
Zoning			

READ ALL NOTES ON THIS PAGE AND ON



#### F15-A

Designer:

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ENGINEERING NOTE PAGE ENP-2. THIS

CALCULATION SUMMARY PAGE AS IT

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Dry | 1 span | No cant.

Page 31 of 47 **PASSED** 

December 17, 2020 12:29:49

**BC CALC® Member Report** 

**Build 7364** 

S:\CUSTOMERS\GREENPA...OOD 4-ELEV. 2-R1.isl

Job name: File name:

Address: Description: Level - Ground Floor

City, Province, Postal Code: Specifier: Customer:

Code reports: CCMC 12787-R Company: **GREENPARK** 

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Disclosure

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obtain Instance | Reviewer | BCIN | Date | Discipline | Reviewer | BCIN | Date | Discipline | H. Authier | 43236 | 2021-02-03 BC CALCEL

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## F15-B

Designer:

R<sub>0</sub>

Page 32 of 47 **PASSED** 

December 17, 2020 12:29:49

**BC CALC® Member Report Build 7364** 

Customer:

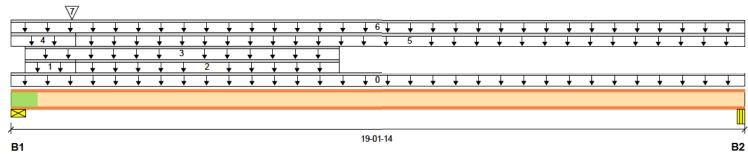
Dry | 1 span | No cant.

Job name: S:\CUSTOMERS\GREENPA...OOD 4-ELEV. 2-R1.isl File name:

Address: Description: Level - Ground Floor

City, Province, Postal Code: Specifier:

Code reports: CCMC 12787-R Company: **GREENPARK** 



Total Horizontal Product Length = 19-01-14

## Reaction Summary (Down / Unlift) (lbs)

reaction ou	innary (Down / O				
Bearing	Live	Dead	Snow	Wind	
B1, 1-7/8"	751 / 0	375 / 0			
B2, 2-5/8"	413 / 0	199 / 0			

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	19-01-14	Тор		3			00-00-00
1		Unf. Lin. (lb/ft)	L	00-04-06	01-08-06	Тор		7			n\a
2		Unf. Lin. (lb/ft)	L	01-08-06	08-06-13	Тор		2			n\a
3		Unf. Lin. (lb/ft)	L	00-04-06	08-06-13	Top		3			n\a
4		Unf. Lin. (lb/ft)	L	00-00-00	01-08-06	Top	56	21			n\a
5		Unf. Lin. (lb/ft)	L	01-08-06	19-01-14	Top	18	7			n\a
6		Unf. Lin. (lb/ft)	L	00-00-00	19-01-14	Тор	22	8			n\a
7	F10	Conc. Pt. (lbs)	L	01-07-02	01-07-02	Back	333	150			n\a

<b>Controls Summary</b>	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	4418 ft-lbs	8640 ft-lbs	51.1%	1	08-06-13
End Reaction	1594 lbs	2182 lbs	73.1%	1	00-00-00
End Shear	1570 lbs	2350 lbs	66.8%	1	00-01-14
Total Load Deflection	L/462 (0.491")	n\a	51.9%	4	09-01-14
Live Load Deflection	L/695 (0.326")	n\a	51.8%	5	09-05-06
Max Defl.	0.491"	n\a	49.1%	4	09-01-14
Span / Depth	19.1				

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Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	1-7/8" x 3-1/2"	1594 lbs	39.5%	73.1%	Spruce-Pine-Fir
B2	Beam	2-5/8" x 3-1/2"	869 lbs	0.2%	50.1%	Steel

## **Cautions**

Web stiffeners required at bearing B1.



These plans have been reviewed for use with the These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-03
Sewage System			
Zoning			



#### F15-B

Dry | 1 span | No cant. December 17, 2020 12:29:49

**BC CALC® Member Report** 

**Build 7364** 

Job name: S:\CUSTOMERS\GREENPA...OOD 4-ELEV. 2-R1.isl File name:

Address: Description: Level - Ground Floor

City, Province, Postal Code: Specifier:

Customer: Designer: R<sub>0</sub>

Code reports: CCMC 12787-R Company: **GREENPARK** 

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

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#### F15-C

December 17, 2020 12:29:49

**BC CALC® Member Report Build 7364** 

Dry | 1 span | No cant.

Page 34 of 47

**PASSED** 

Job name:

S:\CUSTOMERS\GREENPA...OOD 4-ELEV. 2-R1.isl File name:

Address:

Description: Level - Ground Floor

City, Province, Postal Code:

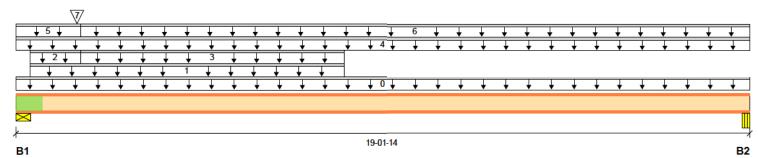
Specifier:

Customer:

Designer: R<sub>0</sub>

Code reports:

CCMC 12787-R Company: **GREENPARK** 



Total Horizontal Product Length = 19-01-14

## Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 1-7/8"	745 / 0	373 / 0		
B2, 2-5/8"	413 / 0	199 / 0		

Load Summary						Live	Dead	Snow	Wind	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0 Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	19-01-14	Тор		3			00-00-00
1	Unf. Lin. (lb/ft)	L	00-04-06	08-06-12	Top		3			n\a
2	Unf. Lin. (lb/ft)	L	00-04-06	01-08-06	Top		7			n\a
3	Unf. Lin. (lb/ft)	L	01-08-06	08-06-12	Top		2			n\a
4	Unf. Lin. (lb/ft)	L	00-00-00	19-01-14	Top	22	8			n\a
5	Unf. Lin. (lb/ft)	L	00-00-00	01-08-06	Top	56	21			n\a
6	Unf. Lin. (lb/ft)	L	01-08-06	19-01-14	Top	18	7			n\a
7 F10	Conc. Pt. (lbs)	L	01-07-02	01-07-02	Front	328	148			2/2
Controls Summary	Factored Demand	Factored Resistance	Dem Resi	and/ stance	Case	Location		/	& DPROFESS	NONAL CHO

<b>Controls Summary</b>	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	4409 ft-lbs	8640 ft-lbs	51.0%	1	08-06-13
End Reaction	1584 lbs	2182 lbs	72.6%	1	00-00-00
End Shear	1560 lbs	2350 lbs	66.4%	1	00-01-14
Total Load Deflection	L/463 (0.49")	n\a	51.8%	4	09-01-13
Live Load Deflection	L/697 (0.326")	n\a	51.7%	5	09-05-06
Max Defl.	0.49"	n\a	49.0%	4	09-01-13
Span / Depth	19.1				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	1-7/8" x 3-1/2"	1584 lbs	39.2%	72.6%	Spruce-Pine-Fir
B2	Beam	2-5/8" x 3-1/2"	869 lbs	0.2%	50.0%	Steel

## Cautions

Web stiffeners required at bearing B1.



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20, 2020

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Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-03
Sewage System			
Zoning			_

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



#### F15-C

Dry | 1 span | No cant.

Page 35 of 47 **PASSED** 

December 17, 2020 12:29:49

**BC CALC® Member Report** 

City, Province, Postal Code:

**Build 7364** 

Address:

Customer:

Code reports:

Job name:

S:\CUSTOMERS\GREENPA...OOD 4-ELEV. 2-R1.isl File name:

Description: Level - Ground Floor

Specifier:

Designer: R<sub>0</sub>

Company: **GREENPARK** 

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

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CCMC 12787-R

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Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

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POINT LOADS OVER BEARINGS

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obtain Instance | Reviewer | BCIN | Date | Discipline | Reviewer | BCIN | Date | Discipline | H. Authier | 43236 | 2021-02-03 BC CALCEL

ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

NE1220-141 Page 36 of 47 Client: GREENPARK Date: 12/17/2020 Input by: Project: R<sub>O</sub> isDesign Address: Job Name: GLENWAY 7A-ELEV. 1-DECK-R1 Project #: 2-Ply - PASSED Level: Ground Floor 1.750" X 11.875" Forex 2.0E-3000Fb LVL 3 10 4 2 8 T 5 6 1 11 7/8 1 SPF End Grain 2 SPF End Grain 3 3' Unfactored Reactions UNPATTERNED lb (Uplift) Member Information Brg Live Dead Wind Type: Application: Floor (Residential) Snow Plies Design Method: 80 318 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 52 307 n O 2 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 3.000" 398 / 120 10% 517 I 1 25D+1 5L Fnd Grain Analysis Results 1.25D+1.5L 2 - SPF 3.000" 384 / 78 461 L Actual Location Allowed Comb **Analysis** Capacity Case End Moment 108 ft-lb 1'6" 22269 ft-lb 0.005 (0%) 1.4D Uniform Grain Unbraced 108 ft-lb 22269 ft-lb 0.005 (0%) 1.4D Uniform PROFESSIONAL READ ALL NOTES ON THIS PAGE AND ON Shear 45 lb 7537 lb 0.006 (1%) 1.4D Uniform ENGINEERING NOTE PAGE ENP-2. THIS 1'9 7/8" NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) CALCULATION SUMMARY PAGE AS IT LL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. A. EL-MASRI TL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) REFER TO MULTIPLE MEMBER TO MEMBER Design Notes CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam PASS THRU FRAMING SQUASH 2020 width X 4.5 **BLOCK IS REQUIRED AT ALL** 2 Girders are designed to be supported on the bottom edge only. POINT LOADS OVER BEARINGS. 3 Multiple plies must be fastened together as per manufacturer's details. 4 Top loads must be supported equally by all plies. 5 Top braced at bearings. 6 Bottom braced at bearings. 7 Lateral slenderness ratio based on full section width. ID Trib Width Load Type Location Side Dead Live Snow Wind Comment Part Uniform 0-0-0 to 0-3-0 40 PLF 0 PLF 0 PLF 0 PLF Wall Self V East Gwillimbury 1 Top 2 Part Uniform 0-0-0 to 0-3-0 Near Face 40 PLF 0 PLF 0 PLF 0 PLF Wall Self V 3 19 PLF 0 PLF 0 PLF Tapered Start 0-0-0 Near Face 7 PLF These plans have been reviewed for use with the 7 PLF 19 PLF 0 PLF 0 PLF corrections as noted. No other changes may be Fnd 0-3-0 made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Point 0-0-10 182 lb 75 lb 0 lb Top Header Co Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly Continued on page 2... posted on site at all time Manufacturer Info Notes Reviewer BCIN Date
H. Authier 43236 2021-0 Discipline Building Code **Handling & Installation** Forex structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requiremen APA: PR-L318 Sewage System

This design is valid until 1/8/2023

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aged Beams m

Daniaged beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid

lateral displacement and rotation

NE1220-141 Page 37 of 47 Client: GREENPARK Date: 12/17/2020 Page 12 of 21

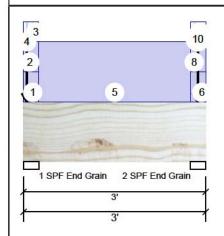
Project: Input by:

Job Name: GLENWAY 7A-ELEV. 1-DECK-R1

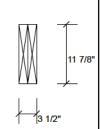
Project #:

2-Ply - PASSED Level: Ground Floor 1.750" X 11.875" FH5-A Forex 2.0E-3000Fb LVL

Address:



isDesign



Continued	from page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
5	Part. Uniform	0-3-0 to 2-9-0		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
6	Part. Uniform	2-9-0 to 3-0-0		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
8	Part. Uniform	2-9-0 to 3-0-0		Near Face	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
9	Tapered Start	2-9-0		Near Face	7 PLF	19 PLF	0 PLF	0 PLF	
	End	3-0-0			7 PLF	19 PLF	0 PLF	0 PLF	
10	Point	2-10-7		Тор	171 lb	47 lb	0 lb	0 lb	F9 Header Column Header Column
	Self Weight				10 PLF				

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER

CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



These plans have been reviewed for use with the These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orineria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used

Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

4. 5.

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 1/8/2023

Forex

Manufacturer Info

APA: PR-L318

Discipline Reviewer BCIN Date
Building Code H. Authier 43236 2021-02-03 Sewage System

NE1220-141 Page 38 of 47 Client: **GREENPARK** 12/17/2020 Date Project: Input by: R<sub>O</sub> isDesign Address: Job Name: GLENWAY 7A-ELEV. 1-DECK-R1 Project # 1.750" X 11.875" 2-Ply - PASSED Level: Ground Floor Forex 2.0E-3000Fb LVL FH5-B ~ 10 5 9 2 4 8 1 6 7 11 7/8 1 SPF End Grain 2 SPF End Grain 3 3' Unfactored Reactions UNPATTERNED lb (Uplift) Member Information Brg Dead Wind Type: Application: Floor (Residential) Live Snow Plies Design Method: 30 147 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 155 n O 2 54 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 3.000" 184 / 46 230 I 1 25D+1 5L Fnd Grain Analysis Results 274 L 1.25D+1.5L 2 - SPF 3.000" 194 / 80 Actual **Analysis** Location Allowed Capacity Comb Case End Moment 108 ft-lb 1'6" 22269 ft-lb 0.005 (0%) 1.4D Uniform Grain Unbraced 108 ft-lb 22269 ft-lb 0.005 (0%) 1.4D Uniform READ ALL NOTES ON THIS PAGE AND ON PROFESSIONAL Shear 45 lb 7537 lb Uniform ENGINEERING NOTE PAGE ENP-2. THIS 1'2 1/8" 0.006 (1%) 1.4D NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) CALCULATION SUMMARY PAGE AS IT LL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. A. EL-MASRI TL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) REFER TO MULTIPLE MEMBER TO MEMBER Design Notes CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. 1 Girders are designed to be supported on the bottom edge only. PASS THRU FRAMING SQUASH 2020 2 Multiple plies must be fastened together as per manufacturer's details. **BLOCK IS REQUIRED AT ALL** 3 Top loads must be supported equally by all plies. POINT LOADS OVER BEARINGS. 4 Top braced at bearings. 5 Bottom braced at bearings 6 Lateral slenderness ratio based on full section width. ID Load Type Location Trib Width Side Dead Live Wind Comments Snow 40 PLF 1 Part Uniform 0-0-0 to 0-3-0 Top 0 PLF 0 PLF 0 PLF Wall Self Weight 2 Part. Uniform 0-0-0 to 0-0-0 Near Face 80 PLF 0 PLF 0 PLF Wall Self V East Gwillimbury 3 Part. Uniform 0-0-0 to 0-3-0 Near Face 40 PLF 0 PLF 0 PLF 0 PLF Wall Self V Tapered Start 0-0-0 Near Face 8 PLF 22 PI F 0 PLF 0 PLF These plans have been reviewed for use with the End 0-3-0 8 PLF 22 PLF 0 PLF 0 PLF corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Point 0 - 1 - 12Near Face 11 lb 25 lb 0 lb 0 lb F9 Continued on page 2... Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all time Manufacturer Info Notes Reviewer BCIN Date H. Authier 43236 2021-0 Discipline Building Code **Handling & Installation** Forex structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requiremen APA: PR-L318 Sewage System Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corr This design is valid until 1/8/2023 Version 20.20.002 Powered by iStruct™ CSD DESIGN

NE1220-141 Page 39 of 47 Page 14 of 21 Client: **GREENPARK** Date: 12/17/2020 Project: Input by: isDesign Address: Job Name: GLENWAY 7A-ELEV. 1-DECK-R1 Project #: 2-Ply - PASSED Level: Ground Floor Forex 2.0E-3000Fb LVL 1.750" X 11.875" FH5-B 2 10 5 9 2 4 8 1 7 6 11 7/8 1 SPF End Grain 2 SPF End Grain 3' 3' .Continued from page 1 ID Location Trib Width Side Dead Live Snow Wind Comments Load Type 80 PLF 0 PLF 0 PLF 6 Part. Uniform 0-3-0 to 2-9-0 Top 0 PLF Wall Self Weight 0 PLF 7 Part. Uniform 2-9-0 to 3-0-0 Top 40 PLF 0 PLF 0 PLF Wall Self Weight Part. Uniform 2-9-0 to 3-0-0 Near Face 40 PLF 0 PLF 0 PLF 0 PLF Wall Self Weight 8 9 Tapered Start 2-9-0 Near Face 8 PLF 22 PLF 0 PLF 0 PLF End 3-0-0 8 PLF 22 PLF 0 PLF 0 PLF 10 Point 2-10-4 Near Face 19 lb 48 lb 0 lb 0 lb F9 Self Weight 10 PLF

> READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER

CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amehded. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times. posted on site at all time

Notes

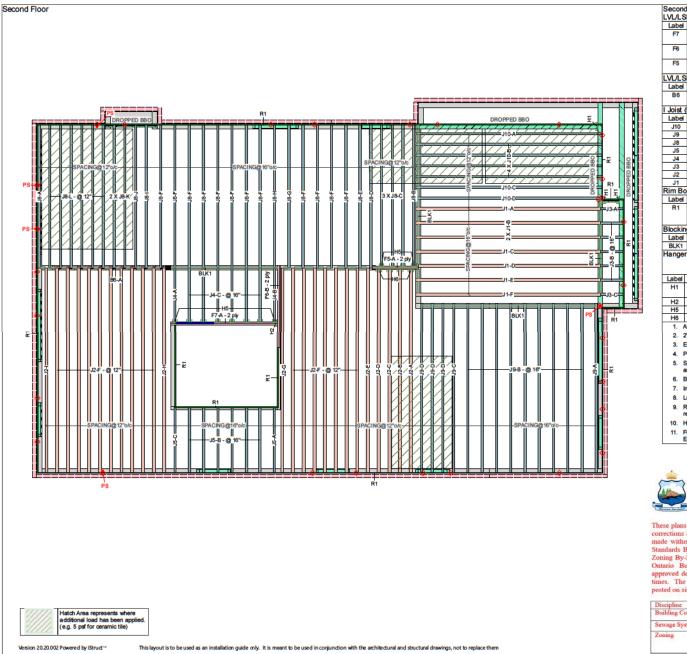
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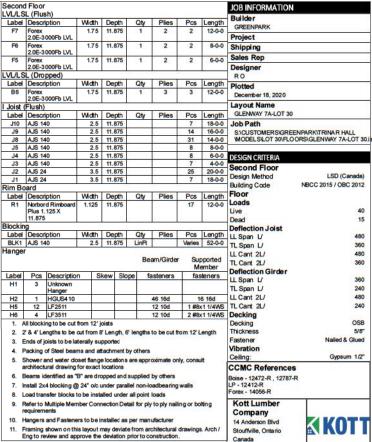
- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corre
- Handling & Installation
- LVL beams must not be cut or drilled
   Refer to manufacturer's product regarding installation requirement fastening details, beam strength value
- naged Beams must not be used
- Daniaged beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation 5.
- 6. For flat roofs provide proper drainage to prevent

Manufacturer Info Forex

APA: PR-L318

Discipline Building Code Sewage System







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Discipline	Reviewer	BCIN	Date
Building Code	H. Authier	43236	2021-02-03
Sewage System			
Zoning			

Legend

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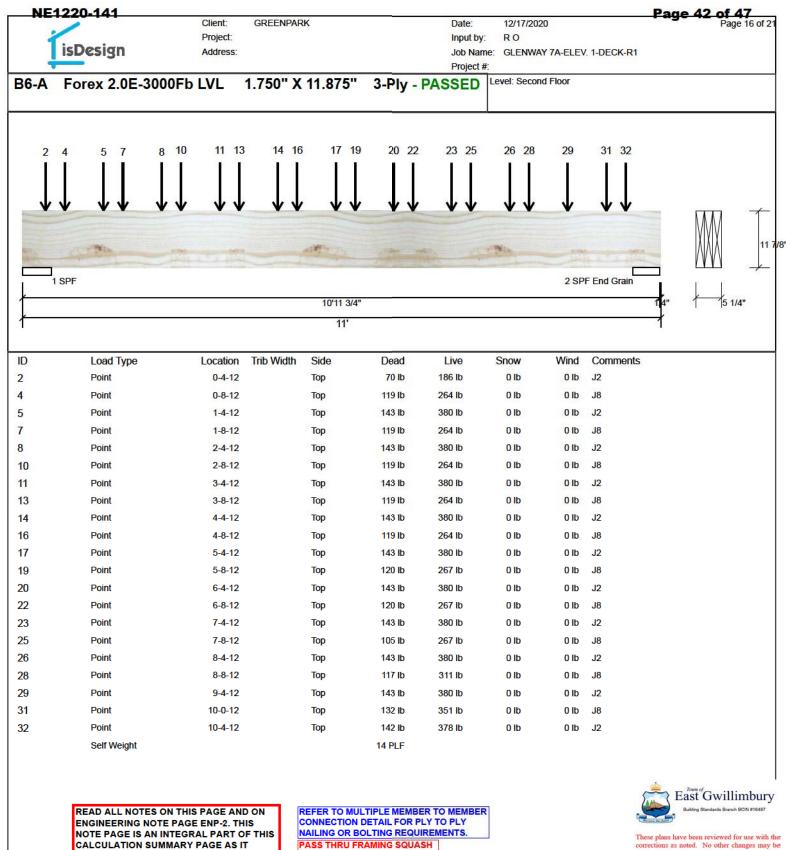
Point Load Support Load from Above

NE1220-141 Page 41 of 47 Client: **GREENPARK** Date: 12/17/2020 Page 15 of 21 Input by: Project: RO isDesign Address: Job Name: GLENWAY 7A-ELEV. 1-DECK-R1 Project #: Forex 2.0E-3000Fb LVL Level: Second Floor 1.750" X 11.875" 3-Ply - PASSED 10 14 16 17 19 20 23 25 26 28 29 31 32 2 8 11 7/8 1 SPF 2 SPF End Grain 10'11 3/4" 11' Unfactored Reactions UNPATTERNED lb (Uplift) Member Information Wind Type: Application: Floor (Residential) Brg Live Dead Snow Plies 3 Design Method: 3399 1454 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 0 3368 1391 n 2 Deflection LL: 360 Load Sharing: Yes Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 6.000" 1817 / 5098 6915 L 1.25D+1.5L 2 - SPF 5.500" 1739 / 5052 32% 6792 L 1.25D+1.5L Analysis Results End Grain Comb. **Analysis** Actual Location Allowed Capacity Case 17454 ft-lb Moment 5'4 3/4" 53447 ft-lb 0.327 (33%) 1.25D+1.5L L\_ Unbraced 17454 ft-lb 5'4 3/4" 51798 ft-lb 0.337 (34%) 1.25D+1.5L L\_ READ ALL NOTES ON THIS PAGE AND ON PROFESSIONAL Shear 6114 lb 0.351 (35%) 1.25D+1.5L L\_ ENGINEERING NOTE PAGE ENP-2. THIS 1'5 1/8" 17394 lb NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.054 (L/2288) 5'7" 0.344 (L/360) 0.160 (16%) D Uniform CALCULATION SUMMARY PAGE AS IT 5'7 1/8" 0.344 (L/360) 0.370 (37%) L LL Defl inch 0.128 (L/965) LL CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. A. EL-MASRI TL Defl inch 0.182 (L/679) 5'7 1/8" 0.516 (L/240) 0.350 (35%) D+L LL REFER TO MULTIPLE MEMBER TO MEMBER LL Cant -0.001 Rt Cant 0.200 0.004 (0%) L CONNECTION DETAIL FOR PLY TO PLY (2L/687) (2L/480) NAILING OR BOLTING REQUIREMENTS. TL Cant -0.001 Rt Cant 0.300 0.003 (0%) D+L ш PASS THRU FRAMING SQUASH 2020 (2L/484)(2L/360) **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS. **Design Notes** 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5. 2 Girders are designed to be supported on the bottom edge only. 3 Multiple plies must be fastened together as per manufacturer's details. 4 Top loads must be supported equally by all plies. 5 Top braced at bearings. East Gwillimbury 6 Bottom braced at bearings. 7 Lateral slenderness ratio based on full section width. These plans have been reviewed for use with the corrections as noted. No other changes may be corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended, These approved documents must be kept on site at all times. The building permit must be clearly protected on site at all times. posted on site at all time Manufacturer Info Notes Reviewer BCIN Date
H. Authier 43236 2021-0 Discipline Building Code **Handling & Installation** Forex structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requiremen APA: PR-L318 Sewage System aged Beams must not be used Daniaged beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid

lateral displacement and rotation Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corr This design is valid until 1/8/2023 Version 20.20.002 Powered by iStruct™ CSD DESIGN



CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended, and the proved documents must be kept on site at all times. The building permit must be clearly protected on site at all times. osted on site at all tim

#### Notes

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corre

## Handling & Installation

LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requirement fastening details, beam strength value

aged Beams must not be us

Daniaged beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

This design is valid until 1/8/2023

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APA: PR-L318

Manufacturer Info

Discipline Building Code H. Authier 43236 Sewage System

NE1220-141 Page 43 of 47 Client: **GREENPARK** Date: 12/17/2020 Project: Input by: R<sub>O</sub> isDesign Address: Job Name: GLENWAY 7A-ELEV. 1-DECK-R1 Project #: Level: Second Floor 1.750" X 11.875" Forex 2.0E-3000Fb LVL 2-Ply - PASSED 3 5 1 11 7/8 1 SPF 2 SPF 4'4' Member Information Unfactored Reactions UNPATTERNED lb (Uplift) Application: Brg Dead Snow Wind Type: Floor (Residential) Live Plies: Design Method: 1235 494 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 1187 484 0 O 2

#### Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 4.000" 618 / 1853 29% 2470 I 1.25D+1.5L 605 / 1781 2386 1 2 - SPF 3.000" 37% 1.25D+1.5L

Analysis Results

Г	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	2341 ft-lb	2'3 1/8"	34261 ft-lb	0.068 (7%)	1.25D+1.5L	L
	Unbraced	2341 ft-lb	2'3 1/8"	34261 ft-lb	0.068 (7%)	1.25D+1.5L	L
	Shear	3403 lb	3'1 7/8"	11596 lb	0.293 (29%)	1.25D+1.5L	L
	Perm Defl in.	0.003 (L/17823)	2'2 13/16"	0.129 (L/360)	0.020 (2%)	D	Uniform
	LL Defl inch	0.006 (L/7169)	2'2 11/16"	0.129 (L/360)	0.050 (5%)	L	L
	TL Defl inch	0.009 (L/5113)	2'2 11/16"	0.194 (L/240)	0.050 (5%)	D+L	L

## **Design Notes**

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER

Manufacturer Info

APA: PR-L318

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CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



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ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-1-12 to 2-1-12		Near Face	144 PLF	376 PLF	0 PLF	0 PLF	
_	Ded Heifers	0.4.40.1-0.4.40		F F	07.01.5	000 DLE	0.01.5	0.01.5	(

Part. Uniform 0-4-12 to 3-4-12 Far Face 97 PLF 260 PLF 0 PLF 0 PLF 3 Point 2-7-12 Near Face 153 lb 376 lb 0 lb 0 lb J2 Point 3-7-12 Near Face 141 lb 343 lb 0 lb 0 lb J2 5 **Point** 3-10-12 Far Face 59 lb 158 lb 0 lb 0 lb .18 Tie-In 15 PSF 40 PSF 0 PSF 0 PSF 4-1-0 to 4-4-0 0-9-2 Top

Continued on page 2...



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Discipline Reviewer BCIN Date

Notes

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- Dry service conditions, unless noted otherwise
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- **Handling & Installation**
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NE1220-141 Page 44 of 47 Client: **GREENPARK** Date: 12/17/2020 Page 18 of 21 Project: Input by: isDesign Address: Job Name: GLENWAY 7A-ELEV. 1-DECK-R1 Project #: Level: Second Floor 1.750" X 11.875" 2-Ply - PASSED Forex 2.0E-3000Fb LVL F5-A 3 5 1 11 7/8" 1 SPF 2 SPF 4'4" 4'4" .Continued from page 1 ID Load Type Location Trib Width Side Dead Live Snow Wind Comments 7 15 PSF 40 PSF 0 PSF 0 PSF Tie-In 4-1-0 to 4-4-0 0-6-14 Top Self Weight 10 PLF READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS. East Gwillimbury These plans have been reviewed for use with the

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Discipline Reviewer BCIN Date
Building Code H. Authier 43236 2021-02-03

Sewage System

Notes

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  2. Refer to manufacturer's product inforegarding installation requirements, in fastening details, beam strength values, an approvals
  3. Damaged Beams must not be used
- Daniaged beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation 4. 5.
- 6. For flat roofs provide proper drainage to prevent ponding

Forex APA: PR-L318

Manufacturer Info

NE1220-141 Page 45 of 47 Client: GREENPARK Date: 12/17/2020 Input by: Project: RO isDesign Address: Job Name: GLENWAY 7A-ELEV. 1-DECK-R1 Project #: Level: Second Floor Forex 2.0E-3000Fb LVL 1.750" X 11.875" 2-Ply - PASSED 2 5 3 11 7/8 1 SPF 2 SPI 7'8 3/4' 7'8 3/4' Unfactored Reactions UNPATTERNED lb (Uplift) Member Information Dead Snow Wind Type: Application: Floor (Residential) Brg Live Plies Design Method: 527 268 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 240 139 n O 2 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 335 / 791 1 - SPF 6.000" 9% 1126 I 1 25D+1 5L 2 - SPF 5.500" 5% 174 / 360 534 1 1.25D+1.5L Analysis Results Actual **Analysis** Location Allowed Capacity Comb Case Moment 2048 ft-lb 2'3 3/4" 34261 ft-lb 0.060 (6%) 1.25D+1.5L L 1.25D+1.5L L Unbraced 2048 ft-lb 2'3 3/4" 32048 ft-lb 0.064 (6%) READ ALL NOTES ON THIS PAGE AND ON PROFESSIONAL Shear 1091 lb 1'5 1/8" 11596 lb 0.094 (9%) 1.25D+1.5L L ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.005 3'3 3/4" 0.230 (L/360) 0.020 (2%) D Uniform CALCULATION SUMMARY PAGE AS IT (L/18271) CONTAINS SPECIFICATIONS AND CRITERIA LL Defl inch 0.009 (L/9264) 3'2 9/16" 0.230 (L/360) 0.040 (4%) L L USED IN THE DESIGN OF THIS COMPONENT. A. EL-MASRI TL Defl inch 0.013 (L/6148) 3'2 15/16" 0.345 (L/240) 0.040 (4%) D+L REFER TO MULTIPLE MEMBER TO MEMBER L CONNECTION DETAIL FOR PLY TO PLY **Design Notes** NAILING OR BOLTING REQUIREMENTS. 1 Girders are designed to be supported on the bottom edge only. PASS THRU FRAMING SQUASH 2020 **BLOCK IS REQUIRED AT ALL** 2 Multiple plies must be fastened together as per manufacturer's details. POINT LOADS OVER BEARINGS 3 Top loads must be supported equally by all plies. 4 Top braced at bearings. 5 Bottom braced at bearings. 6 Lateral slenderness ratio based on full section width ID Trib Width Side Dead Wind Comments Load Type Location Live Snow 1 Tie-In 0-0-0 to 7-6-0 0-1-14 Top 15 PSF 40 PSF 0 PSF 0 PSF East Gwillimbury 2 Point 2-3-12 Far Face 297 lb 670 lb 0 lb 0 lb F7 40 PSF 3 Tie-In 2-5-8 to 7-6-0 0-2-10 Top 15 PSF 0 PSF 0 PSF 15 PSF Tie-In 7-6-0 to 7-8-12 0-2-10 Top 40 PSF 0 PSF 0 PSF These plans have been reviewed for use with the corrections as noted. No other changes may be corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended, These approved documents must be kept on site at all times. The building permit must be clearly protected on site at all times. 7-6-0 to 7-8-12 0-5-6 15 PSF 40 PSF 0 PSF 5 0 PSF Tie-In Top Self Weight 10 PLF osted on site at all tim Manufacturer Info Notes Reviewer BCIN Date H. Authier 43236 2021-0 Discipline Building Code **Handling & Installation** Forex structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requiremen APA: PR-L318 Sewage System Daniaged beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid

lateral displacement and rotation Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or conditions. This design is valid until 1/8/2023 CSD DESIGN

NE1220-141 Page 46 of 47 Client: **GREENPARK** Date: 12/17/2020 Page 20 of 21 Input by: Project: RO isDesign Address: Job Name: GLENWAY 7A-ELEV. 1-DECK-R1 Project #: Level: Second Floor Forex 2.0E-3000Fb LVL 1.750" X 11.875" 2-Ply - PASSED 2 11 7/8 1 SPF 2 Hanger (HGUS410) 10' 1/4' Unfactored Reactions UNPATTERNED lb (Uplift) Member Information Brg Dead Wind Type: Application: Floor (Residential) Live Snow Plies Design Method: 1174 486 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 0 670 297 n 2 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 5.500" 20% 608 / 1761 2369 L 1.25D+1.5L 2 4.000" 13% 371 / 1005 1376 L 1.25D+1.5L Analysis Results Hanger Actual Comb. **Analysis** Location Allowed Capacity Case Moment 4203 ft-lb 3'9 9/16" 34261 ft-lb 0.123 (12%) 1.25D+1.5L \_L Unbraced 4203 ft-lb 3'9 9/16" 30062 ft-lb 0.140 (14%) 1.25D+1.5L \_L READ ALL NOTES ON THIS PAGE AND ON PROFESSIONAL Shear 1800 lb 0.155 (16%) 1.25D+1.5L \_L **ENGINEERING NOTE PAGE ENP-2. THIS** 1'2 7/8" 11596 lb NOTE PAGE IS AN INTEGRAL PART OF THIS Perm Defl in. 0.017 (L/6900) 4'8 7/16" 0.317 (L/360) 0.050 (5%) D Uniform CALCULATION SUMMARY PAGE AS IT 4'7 7/8" 0.317 (L/360) 0.120 (12%) L LL Defl inch 0.039 (L/2937) LL CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. A. EL-MASRI TL Defl inch 0.055 (L/2060) 4'8 1/16" 0.475 (L/240) 0.120 (12%) D+L LL REFER TO MULTIPLE MEMBER TO MEMBER LL Cant -0.000 Lt Cant 0.200 0.001 (0%) L CONNECTION DETAIL FOR PLY TO PLY (2L/1978) (2L/480) NAILING OR BOLTING REQUIREMENTS. TL Cant -0.000 Lt Cant 0.300 0.001 (0%) D+L ш PASS THRU FRAMING SQUASH 2020 (2L/1391) (2L/240) **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS. **Design Notes** 1 Fill all hanger nailing holes. 2 Girders are designed to be supported on the bottom edge only. 3 Multiple plies must be fastened together as per manufacturer's details. 4 Top loads must be supported equally by all plies. 5 Top braced at bearings. 6 Bottom braced at bearings. East Gwillimbury 7 Lateral slenderness ratio based on full section width. ID Load Type Location Trib Width Side Dead Live Snow Wind Comment 5 1 Point 0-4-12 Far Face 44 lb 116 lb 0 lb 0 lb .14 These plans have been reviewed for use with the corrections as noted. No other changes may be corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended, These approved documents must be kept on site at all times. The building permit must be clearly protected on site at all times. 90 PLF 240 PLF 0 PLF 0 PLF 2 Part Uniform 0-6-0 to 4-0-0 Top 3 Part Uniform 1-0-12 to 9-0-12 Far Face 37 PLF 100 PLF 0 PLF 0 PLF Continued on page 2... posted on site at all time Manufacturer Info Notes Discipline Building Code **Handling & Installation** Forex structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended LVL beams must not be cut or drilled
 Refer to manufacturer's product regarding installation requiremen APA: PR-L318 Sewage System aged Beams m Daniaged beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid

lateral displacement and rotation Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corn This design is valid until 1/8/2023 Version 20.20.002 Powered by iStruct™ CSD DESIGN

NE1220-141 Page 47 of 47 Client: **GREENPARK** Date: 12/17/2020 Page 21 of 21 Project: Input by: isDesign Address: Job Name: GLENWAY 7A-ELEV. 1-DECK-R1 Project #: Level: Second Floor 1.750" X 11.875" Forex 2.0E-3000Fb LVL 2-Ply - PASSED 4 2 11 7/8" 1 SPF 2 Hanger (HGUS410) 10 10' 1/4" .Continued from page 1 ID Location Trib Width Side Dead Live Snow Wind Comments Load Type 4 88 lb 0 lb 0 lb J4 **Point** 9-8-12 Far Face 33 lb Self Weight 10 PLF READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS. East Gwillimbury These plans have been reviewed for use with the corrections as noted. No other changes may be made without written approval of the Building Standards Branch. All work must comply with Zoning By-Law 2018-043, as amended, and the Ontario Building Code, as amended. These approved documents must be kept on site at all times. The building permit must be clearly posted on site at all times. posted on site at all times Manufacturer Info 6. For flat roofs provide proper drainage to prevent ponding Notes Discipline Reviewer BCIN Date
Building Code H. Authier 43236 2021-02-03 Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orineria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Forex Handling & Installation 1. IVI, beams must not be cut or drilled
2. Refer to manufacturer's product inforregarding installation requirements, in fastening details, beam strength values, an approvals
3. Damaged Beams must not be used APA: PR-L318 Sewage System Daniaged beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive 4. 5. This design is valid until 1/8/2023 Version 20.20.002 Powered by iStruct™