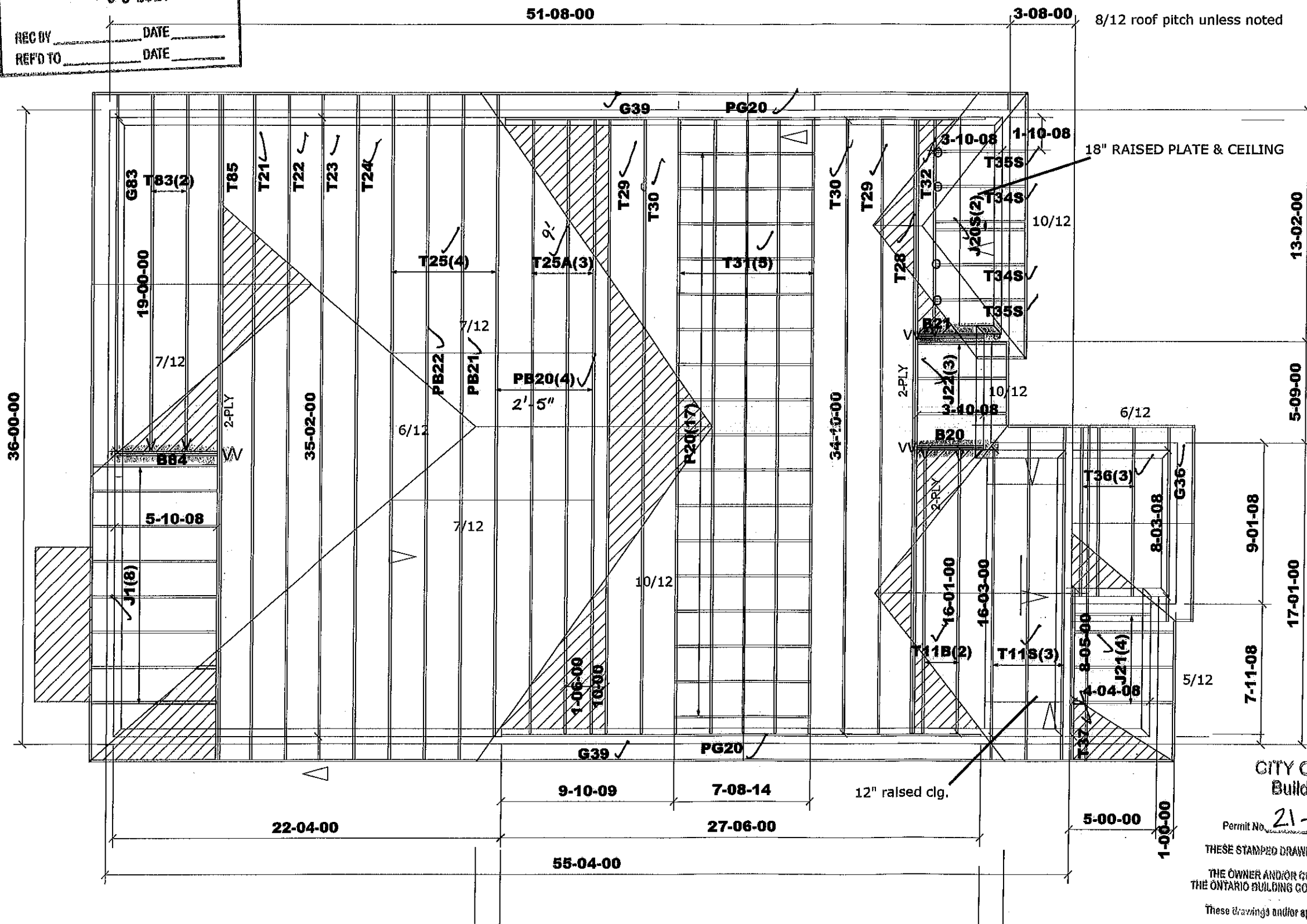


CITY OF HAMILTON
BUILDING DIVISION
Planning & Development Department

APR 09 2021

REC BY _____ DATE _____
REFD TO _____ DATE _____



ASPHALT SHINGLES
 FINISHED OVERHANG: 12"
 2x6 EXTERIOR WALLS
 2x6 FASCIA BOARD
 HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
 Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
 TCSL = 25.6 psf
 TCDL = 6.0 psf
 BCLL = 0.0 psf
 BCDL = 7.4 psf

HARDWARE:
 LUS24 - (O)
 LJS26DS - (V)
 HGUS26-2 - (XX)
 LUS26-2-(VV)

BEAMS:
 B20 = B21 = B84 =
 2 - 2x10 SPF #2

DENOTES:
 CONVENTIONAL FRAMING

CITY OF HAMILTON Building Division
 Permit No. 21-105991
 THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE
 THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW
 These drawings and/or specifications have been reviewed by
 JDL FOR CHIEF BUILDING OFFICIAL
 DATE Apr 20/21
 Mitek ver 8.3.3.247

 TAMARACK ROOF TRUSSES INC. ESTABLISHED 1987	Job Track: 51225	Builder / Location: GREEN PARK HOMES / WATERDOWN			Model / Elevation: MOUNTAINASH 6 - 154 / 2-STD OR OPT.5 BR			Mitek ver 8.3.3.247
	Plan Log: 202419	Project: RUSSELL GARDENS PH.3			THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.			
	Layout ID: 415090	Date: 2020-12-08	Sales: Mario DiCano	Designer: JG / ND				

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 6
 Lot #: 154
 Elevation: 2-STD OR OPT.5 BR

Job Track: 51225
 PlanLog: 202419
 Layout ID: 408223
 Ref #
 Page: 1 of 3
 Date: 04-28-2020
 Designer:
 Sales Rep: Mario DiCano

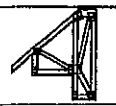
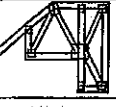
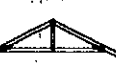
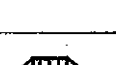
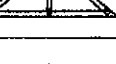
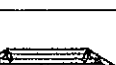
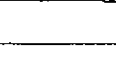
Roof Trusses BEAM (B20)(B21) + (B84) 2 Ply 2"X.10" S.P.F. #2.

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T11B Common	8 / 12	16-01-00	6-09-13	2 x 4		1-06-02 1-04-13	137.47 86.33		
	3	T11S Roof Special	8 / 12 8 / 12	16-03-00	6-09-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	201.4 133.00		
	1 2-ply	T20 Hip Girder	7 / 12	35-02-00	4-01-04	2 x 6	1-03-08 1-03-08	1-03-07 1-03-07	359.46 218.00		
	1	T21 Hip	7 / 12	35-02-00	5-01-04	2 x 4	1-03-08 1-03-08	1-03-07 1-03-07	142.49 89.50		
	1	T22 Hip	7 / 12	35-02-00	6-01-04	2 x 4	1-03-08 1-03-08	1-03-07 1-03-07	146.47 93.50		
	1	T23 Hip	7 / 12	35-02-00	7-01-04	2 x 4	1-03-08 1-03-08	1-03-07 1-03-07	144.95 91.17		
	1	T24 Hip	7 / 12	35-02-00	8-01-04	2 x 4	1-03-08 1-03-08	1-03-07 1-03-07	151.12 95.87		
	4	T25 Hip	7 / 12	35-02-00	9-01-04	2 x 4	1-03-08 1-03-08	1-03-07 1-03-07	631.9 399.33		
	3	T25A Piggyback Base	7 / 12	34-10-00	9-01-04	2 x 4		1-04-09 1-04-09	461.61 286.50		
	1 2-ply	T28 Flat Girder	0 / 12	34-10-00	4-10-07	2 x 6		4-10-07 4-10-07	366.38 214.67		
	2	T29 Flat	0 / 12	34-10-00	6-06-07	2 x 4		6-06-07 6-06-07	313.97 193.33		
	2	T30 Flat	0 / 12	34-10-00	8-02-07	2 x 4		8-02-07 8-02-07	358.85 220.00		
	5	T31 Flat	0 / 12	34-10-00	9-10-07	2 x 4		9-10-07 9-10-07	980.54 600.00		
	1	T32 Hip Girder	8 / 12	12-02-00	6-04-07	2 x 4 2 x 6		3-00-02 2-10-13	66.45 43.67		

T83 To be supplied ON SITE

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: RUSSELL GARDENS PH.3 Location: WATERDOWN Model: MOUNTAINASH 6 Lot #: 154 Elevation: 2-STD OR OPT.5 BR	Job Track: 51225 PlanLog: 202419 Layout ID: 408223 Ref # Page: 2 of 3 Date: 04-28-2020 Designer: Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T34S Half Hip	10 / 12	3-10-08	5-05-13	2 x 4	1-03-08	1-07-11 5-05-13	63.75 44.33		
	2	T35S Half Hip	10 / 12	3-10-08	4-01-13	2 x 4	1-03-08	1-07-11 4-01-13	67.2 41.00		
	3	T36 Common	6 / 12	8-03-08	3-00-09	2 x 4	1-03-08 1-03-08	5-07 5-07	79.61 53.00		
	1	G36 GABLE	6 / 12	8-03-08	3-01-05	2 x 4	1-03-08 1-05-00	5-07 5-07	28.93 17.33		
	1	T37 Half Hip Girder	8 / 12	8-05-00	2-08-03	2 x 4 2 x 6	1-03-08	8-05 2-01-15	38.62 26.00		
	2	G39 GABLE	10 / 12	27-06-00	9-10-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	303.8 193.33		
	17	P20 Common	10 / 12	7-08-14	3-02-12	2 x 4		0-01 0-01	363.26 229.50		
	4	PB20 Piggyback	7 / 12	8-04-05	2-05-04	2 x 4			80.17 49.33		
	1	PB22 Piggyback	7 / 12	8-04-05	1-00-00	2 x 4			18.26 11.67		
	1	PB21 Piggyback	7 / 12	8-04-05	2-00-00	2 x 4			21.88 14.33		
	2	PG20 GABLE	10 / 12	7-09-00	3-02-12	2 x 4			44.53 30.33		
	17	J1 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	285.51 181.33		
	2	J20S Jack-Open	10 / 12	3-10-08	6-04-07	2 x 4	1-03-08	1-07-11 6-04-07	65.13 43.33		
	4	J21 Jack-Open	5 / 12	4-04-08	2-08-02	2 x 4	1-03-08	4-01 2-01-15	48.51 32.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 6
 Lot #: 154
 Elevation: 2-STD OR OPT.5 BR

Job Track: 51225
 PlanLog: 202419
 Layout ID: 408223
 Ref #
 Page: 3 of 3
 Date: 04-28-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	3	J22 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	45.68 29.00		

TOTAL # TRUSS= 92

TOTAL BFT OF ALL TRUSSES= 3762.48

BFT.

TOTAL WEIGHT OF ALL TRSSES 6003.41 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LJS26DS	
4	Hardware	LUS24	
2	Hardware	LUS26-2	

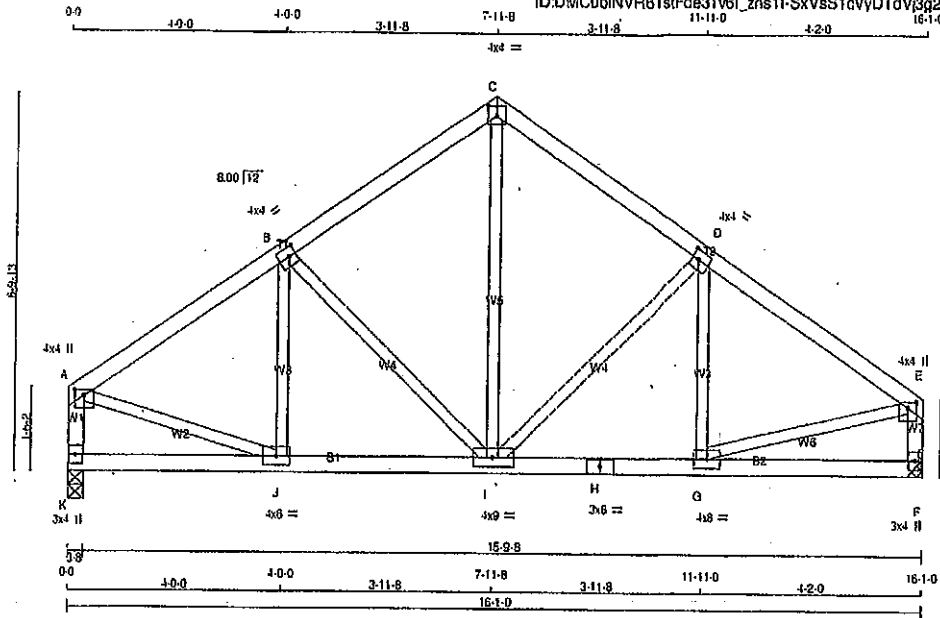
TOTAL NUMBER OF
ITEMS= 9

JOB NAME 408223	TRUSS NAME T11B ✓	QUANTITY 2 ✓	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 MitTek Industries, Inc. Tue Apr 28 10:04:01 2020 Page 1

ID:DMCubINVR6TstFoe31v6L_zns1I-SxVsS1cVvDTdV3g25MsNoIhBxxWB7PW6SQ4izMEMI

Scale = 1:38.5



TOTAL WEIGHT = 2 X 69 = 137 lb (M/F)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2		
C - E	2x4	DRY	No.2		
K - A	2x4	DRY	No.2		
F - E	2x4	DRY	No.2		
K - H	2x4	DRY	No.2		
H - F	2x4	DRY	No.2		

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (tablets in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00	
B	TMVW+p	MT20	4.0	4.0	2.00	1.50	
C	TMVW+p	MT20	4.0	4.0	2.25	2.00	
D	TMVW+p	MT20	4.0	4.0	2.00	1.50	
E	TMVW+p	MT20	4.0	4.0	1.25	2.00	
F	BMV+p	MT20	3.0	4.0			
G	BMVW+p	MT20	4.0	6.0			
H	BS-L	MT20	3.0	6.0			
I	BMVW+p	MT20	4.0	9.0			
J	BMVW+p	MT20	4.0	6.0			
K	BMV+p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
K	887	0	887	0
F	887	0	887	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

1ST LOSE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	627	411 0	0 0	0 0	0 0	216 0	0 0
F	627	411 0	0 0	0 0	0 0	216 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	VERT. LOAD	LC1	MAX	CSH(LC)	UNBRAC	LENGTH	FR-TO	WEBS	MEMB.	MAX. FACTORED	FORCE	MAX	CSH(LC)
A-B	-868	0	-91.8	-91.8	0.19 (1)	6.25	J-B	-142	18	0.04 (1)					
B-C	-897	0	-91.8	-91.8	0.18 (1)	6.25	B-I	-263	0	0.13 (1)					
C-D	-888	0	-91.8	-91.8	0.19 (1)	8.25	I-C	0	481	0.10 (1)					
D-E	-891	0	-91.8	-91.8	0.20 (1)	8.25	I-D	-292	0	0.15 (1)					
K-A	-855	0	0.0	0.0	0.09 (1)	7.81	G-D	-117	28	0.03 (1)					
F-E	-853	0	0.0	0.0	0.09 (1)	7.81	A-J	0	772	0.17 (1)					
							G-E	0	767	0.18 (1)					
K-J	0	0	-18.5	-18.5	0.07 (4)	10.00									
J-I	0	740	-18.5	-18.5	0.15 (1)	10.00									
I-H	0	781	-18.5	-18.5	0.15 (1)	10.00									
H-G	0	761	-18.5	-18.5	0.15 (1)	10.00									
G-F	0	0	-18.5	-18.5	0.07 (4)	10.00									

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
OL = 8.0 PSF
BOT CH. LL = 0.0 PSF
OL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBCQ 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

45% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1.360 (0.54")
CALCULATED VERT. DEFL.(LL) = L 999 (0.02")
ALLOWABLE DEFL.(TL) = 1.360 (0.54")
CALCULATED VERT. DEFL.(TL) = L 999 (0.03")

CSH: TC=0.20/1.00 (D-E:1), BC=0.15/1.00 (G-I:1), WB=0.18/1.00 (E-G:1), SS=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

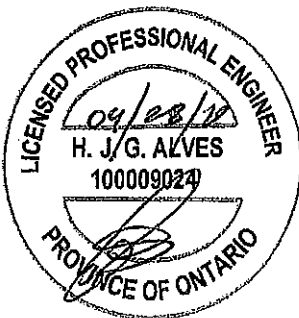
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 768 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.25 (H) (INPUT = 1.00)

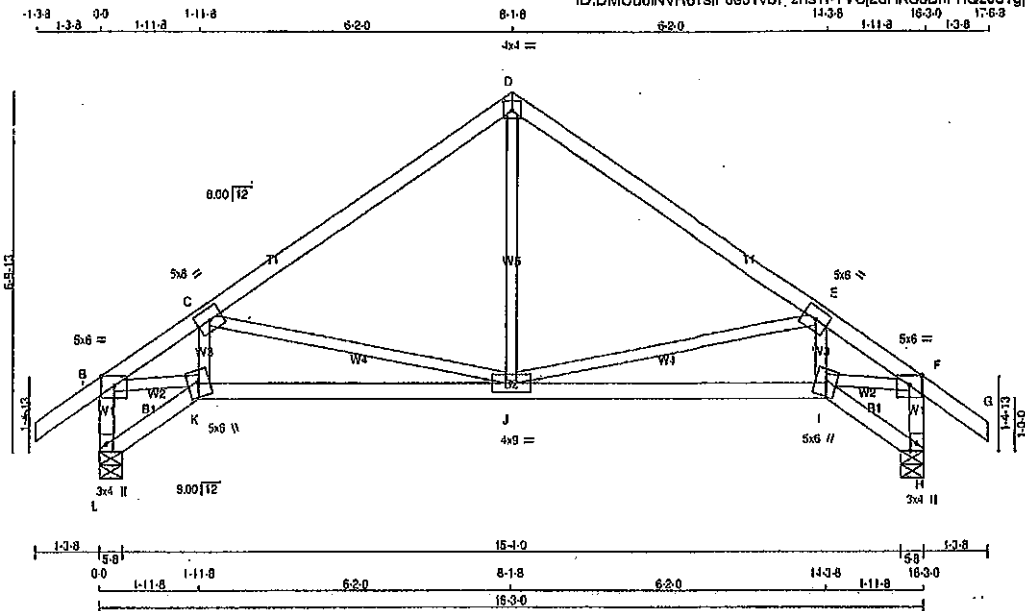


Structural component only
DWG# T-2007631

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	T11S	23	1	GREEN PARK HOMES	
Tampack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 09:00:13 2020 Page 1

ID:DMCubINVR6TsJFoe31v6I, zns11-YVCj2dHKS3BnFRQzo81gJ7reK5PdcqyG49sJazMF1W



TOTAL WEIGHT = 2 X 67 = 134 lb

LUMBER						
N. L. G. A. RULES						
CHORDS		SIZE	LUMBER		DESCR	
L - B	2x4	DRY	No.2			SPF
A - D	2x4	DRY	No.2			SPF
D - G	2x4	DRY	No.2			SPF
H - F	2x4	DRY	No.2			SPF
L - K	2x4	DRY	No.2			SPF
K - I	2x4	DRY	No.2			SPF
I - H	2x4	DRY	No.2			SPF
ALL WEBS		2x3	DRY	No.2		SPF
DRY: SEASONED LUMBER.						

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT	RECORD
		VERT	HORZ	BRG	BRG
JT					
L	1022	0	1022	0	5-8
H	1022	0	1022	0	5-8

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED					
L	720	486	0	0	0	0
H	720	486	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
L-B	-1004 0	0.0	0.0 0.11 (1)	7.80	B-K	0 1321	0.30 (1)
A-B	0 35	-91.8	-91.8 0.12 (1)	10.00	K-C	0 55	0.02 (4)
B-C	-1508 0	-91.8	-91.8 0.33 (1)	4.97	C-J	-659 0	0.48 (1)
C-D	-841 0	-91.8	-91.8 0.41 (1)	8.08	J-D	0 444	0.10 (1)
D-E	-841 0	-91.8	-91.8 0.42 (1)	8.07	J-E	-661 0	0.47 (1)
E-F	-1510 0	-91.8	-91.8 0.33 (1)	4.97	I-E	0 56	0.02 (4)
F-G	0 35	-91.8	-91.8 0.12 (1)	10.00	I-F	0 1323	0.30 (1)
H-F	-1004 0	0.0	0.0 0.11 (1)	7.80			
L-K	0 0	-18.5	-18.5 0.02 (4)	10.00			
K-J	0 1319	-18.5	-18.5 0.33 (1)	10.00			
J-I	0 1320	-18.5	-18.5 0.33 (1)	10.00			
I-H	0 0	-18.5	-18.5 0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	33.0	PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.42/1.00 (D-E:1), BC=0.33/1.00 (J-I:1), WB=0.47/1.00 (E-J:1), SS=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(FSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1887 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

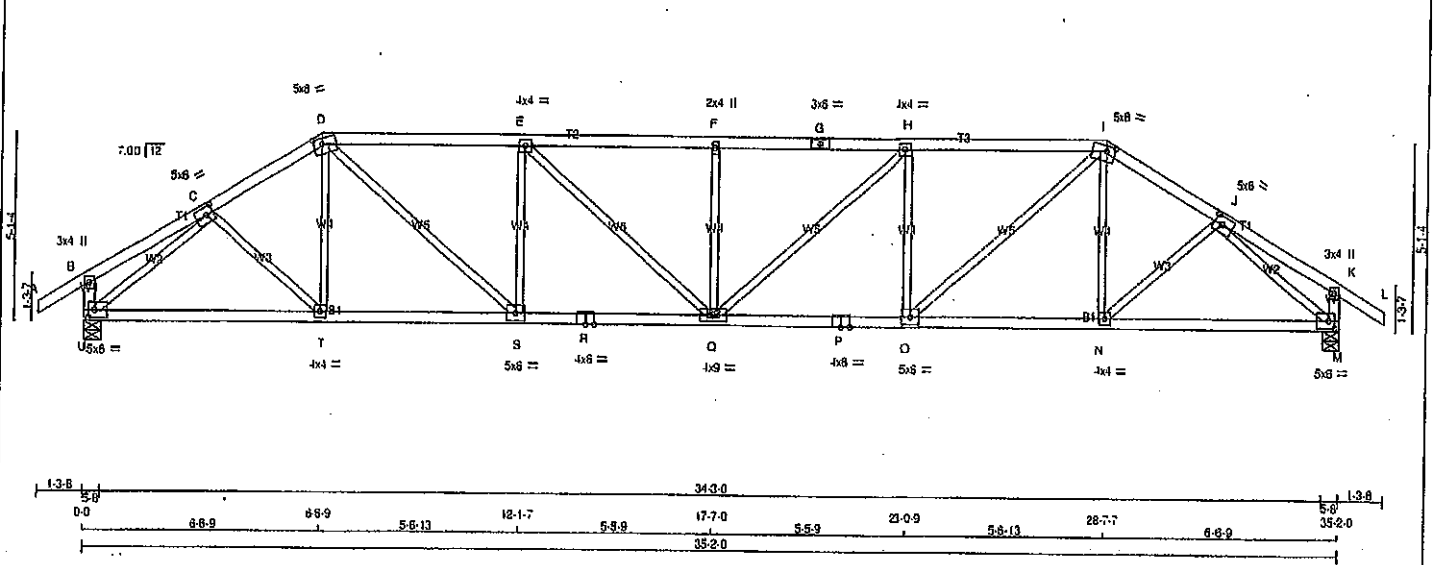
JSI GRIP = 0.89 (I) (INPUT = 0.89)
JSI METAL = 0.47 (K) (INPUT = 1.00)



Structural component only
DWG# T-2007614

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T21	1	1	GREEN PARK HOMES	
TAMARACK ROOF TRUSS, Burlington					

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 26 10:04:03 2020 Page 1
ID:DMCubINVR6TstFos31v6I zns11-PKdctelUgILk0D3AWiqxoT6J_Wk_xSI_yxX9bzMEMg
Scale = 1:57.6



CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
O - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
U - R	2x4	DRY	No.2
R - P	2x4	DRY	No.2
P - M	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY, SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE	MT20	3.0	4.0	
B TMV4p	MT20	5.0	6.0	2.50 2.50
C TMWW-t	MT20	5.0	8.0	1.75 3.00
D TTWW-m	MT20	4.0	4.0	
E TMWW-t	MT20	2.0	4.0	
F TMW-w	MT20	3.0	6.0	
G TS-t	MT20	4.0	4.0	
H TMWW-t	MT20	5.0	8.0	1.75 3.00
I TTWW-m	MT20	5.0	6.0	2.50 2.50
J TMWW-t	MT20	3.0	4.0	
K TMV4p	MT20	5.0	6.0	2.50 2.00
M BMVW-t	MT20	4.0	4.0	
N BMWW-t	MT20	5.0	8.0	
O BMWW-t	MT20	4.0	4.0	
P BS-t	MT20	4.0	8.0	
Q BMWW-t	MT20	5.0	6.0	
R BS-t	MT20	4.0	4.0	
S BMWW-t	MT20	5.0	6.0	
T BMWW-t	MT20	4.0	4.0	
U BMVW-t	MT20	5.0	6.0	2.50 2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
U	VERT	DOWN	IN-SX	IN-SX
M	2084 0	2084 0	5-8	5-8

UNFACTORED REACTIONS	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
U	1457 970 0	0 0 0 0 0 0
M	1457 970 0	0 0 0 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO	A-B	0 32	-91.8	-91.8 0.12 (1)	10.00	C-T	0 213	0.05 (1)
	B-C	0 13	-91.8	-91.8 0.12 (1)	10.00	T-D	-8 69	0.02 (4)
	C-D	2527 0	-91.8	-91.8 0.20 (1)	4.18	D-S	0 1447	0.33 (1)
	D-E	3264 0	-91.8	-91.8 0.68 (1)	3.30	S-E	-853 0	0.33 (1)
	E-F	3585 0	-91.8	-91.8 0.70 (1)	3.12	E-Q	0 428	0.10 (1)
	F-G	3585 0	-91.8	-91.8 0.70 (1)	3.12	Q-F	-462 0	0.18 (1)
	G-H	3585 0	-91.8	-91.8 0.70 (1)	3.12	H-Q	0 428	0.10 (1)
	H-I	3264 0	-91.8	-91.8 0.68 (1)	3.30	Q-H	-853 0	0.33 (1)
	I-J	2527 0	-91.8	-91.8 0.20 (1)	4.18	O-I	0 1447	0.33 (1)
	J-K	0 13	-91.8	-91.8 0.12 (1)	10.00	N-I	-8 69	0.02 (4)
	K-L	0 32	-91.8	-91.8 0.12 (1)	10.00	N-J	0 213	0.05 (1)
	U-B	-249 0	0.0	0.0 0.03 (1)	7.81	U-C	-2671 0	0.82 (1)
	M-K	-249 0	0.0	0.0 0.03 (1)	7.81	J-M	-2671 0	0.82 (1)
	U-T	0 2005	-18.5	-18.5 0.42 (1)	10.00			
	T-S	0 2167	-18.5	-18.5 0.44 (1)	10.00			
	S-R	0 3264	-18.5	-18.5 0.58 (1)	10.00			
	R-Q	0 3264	-18.5	-18.5 0.58 (1)	10.00			
	Q-P	0 3264	-18.5	-18.5 0.58 (1)	10.00			
	P-O	0 3264	-18.5	-18.5 0.58 (1)	10.00			
	O-N	0 2167	-18.5	-18.5 0.44 (1)	10.00			
	N-M	0 2005	-18.5	-18.5 0.42 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-03, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 91.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.39")

CSI: TC=0.70/1.00 (E-F:1), BC=0.58/1.00 (Q-S:1), WB=0.82/1.00 (J-M:1), SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

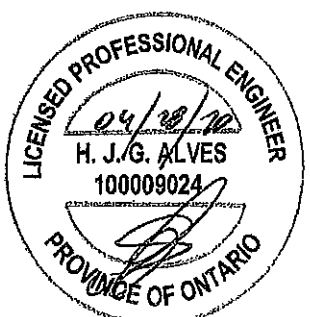
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY)	SHEAR (PLI)	SECTION (PLI)
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

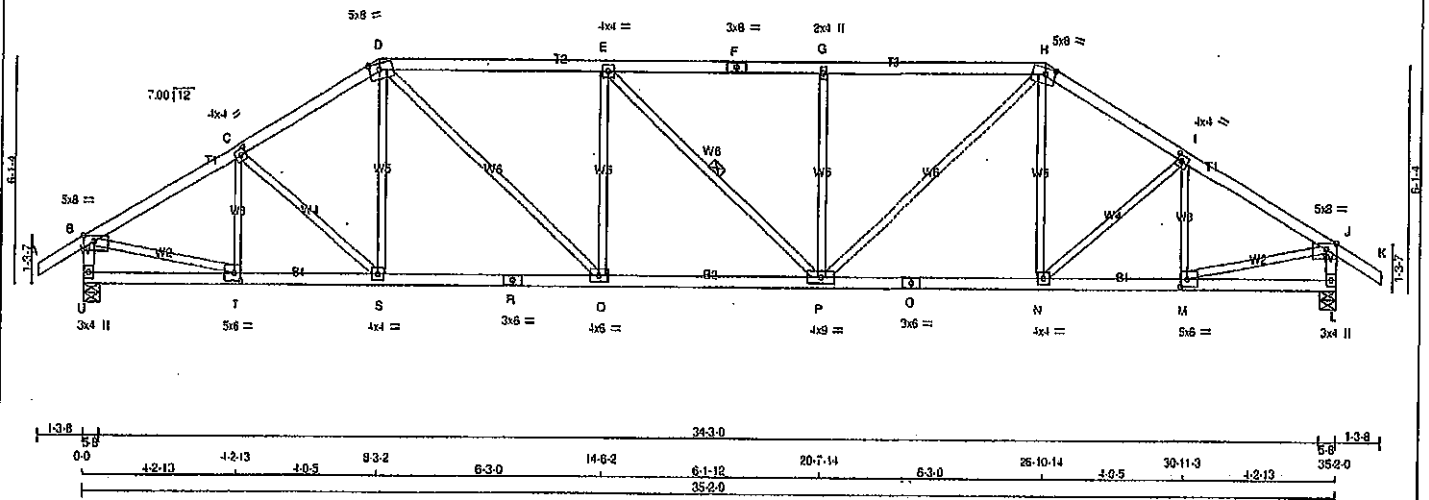
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (M) (INPUT = 0.90)
JSI METAL = 0.74 (R) (INPUT = 1.00)



Structural component only
DWG# T-2007633

JOB NAME 408223	TRUSS NAME T22	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Farside Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:04:04 2020 Page 1	
ID:DMCubINVR8TstFoe31v6l zns1f-WB743eNF8rCMAofJG3U?0GFOhJTrCog4h1zMEMI				35-2-0 35-5-0 1-3-8	
-1-3-8 0-0 4-2-13 4-5-13 4-0-5 3-3-2 6-3-0 14-6-2 6-1-12 20-7-14 6-3-0 26-10-14 4-0-5 30-11-3 4-2-13 35-2-0				Scale = 1:57.6	



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMWW-l	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	2.00	3.50
E	TMWW-l	MT20	4.0	4.0		
F	TS-l	MT20	3.0	6.0		
G	TMW-w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.00	3.50
I	TMWW-l	MT20	4.0	4.0	2.00	1.75
J	TMVW-p	MT20	5.0	8.0	Edge	3.50
L	BMV-l	MT20	3.0	4.0		
M	BMWW-l	MT20	5.0	6.0	2.50	2.50
N	BMWW-l	MT20	4.0	4.0		
O	BS-l	MT20	3.0	6.0		
P	BMWW-l	MT20	4.0	4.0		
Q	BMWW-l	MT20	4.0	6.0		
R	BS-l	MT20	3.0	6.0		
S	BMWW-l	MT20	4.0	4.0		
T	BMWW-l	MT20	5.0	6.0	2.50	2.50
U	BMV-l	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	RECORD
JT	VERT	DOWN	BRG	BRG
U	2064	0	0	5-8
L	2064	0	0	5-8

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED
U	1457 970 0 0 0 0 0 0 488 0 0 0
L	1457 970 0 0 0 0 0 0 488 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.37 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO				FR-TO			
A-B	0 32	-91.8 -91.8 0.12 (1)	10.00	T-C	-426 0	0.10 (1)	
B-C	-2468 0	-91.8 -91.8 0.35 (1)	4.07	C-S	-39 0	0.02 (1)	
C-D	-2478 0	-91.8 -91.8 0.34 (1)	4.08	S-D	0 131	0.04 (4)	
D-E	-2910 0	-91.8 -91.8 0.75 (1)	3.37	D-Q	0 1076	0.24 (1)	
E-F	-2909 0	-91.8 -91.8 0.74 (1)	3.38	Q-E	-617 0	0.38 (1)	
F-G	-2909 0	-91.8 -91.8 0.74 (1)	3.38	E-P	-2 0	0.00 (1)	
G-H	-2909 0	-91.8 -91.8 0.75 (1)	3.38	P-G	-617 0	0.36 (1)	
H-I	-2478 0	-91.8 -91.8 0.34 (1)	4.05	P-H	0 1073	0.24 (1)	
I-J	-2468 0	-91.8 -91.8 0.36 (1)	4.07	H-N	0 132	0.04 (4)	
J-K	0 32	-91.8 -91.8 0.12 (1)	10.00	N-I	-39 0	0.02 (1)	
U-B	-2025 0	0.0 0.0 0.21 (1)	5.94	M-I	-426 0	0.10 (1)	
L-J	-2025 0	0.0 0.0 0.21 (1)	5.94	B-T	0 2205	0.50 (1)	
U-T	0 0	-18.5 -18.5 0.07 (4)	10.00	M-J	0 2205	0.50 (1)	
T-S	0 2149	-18.5 -18.5 0.41 (1)	10.00				
S-R	0 2122	-18.5 -18.5 0.41 (1)	10.00				
R-Q	0 2122	-18.5 -18.5 0.41 (1)	10.00				
Q-P	0 2910	-18.5 -18.5 0.54 (1)	10.00				
P-O	0 2122	-18.5 -18.5 0.41 (1)	10.00				
O-N	0 2122	-18.5 -18.5 0.41 (1)	10.00				
N-M	0 2149	-18.5 -18.5 0.41 (1)	10.00				
M-L	0 0	-18.5 -18.5 0.07 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD = 39.0 PSF				

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 066-09, CSA 066-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")

CALCULATED VERT. DEFL.(LL) = L/999 (0.16")

ALLOWABLE DEFL.(TL) = L/360 (1.17")

CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.75(1.00 (D-E:1), BC=0.54(1.00 (P-Q:1), WB=0.50(1.00 (B-T:1), SS=0.27(1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1687 788 1987 1656	

PLATE PLACEMENT TOL. = 0.250 inches

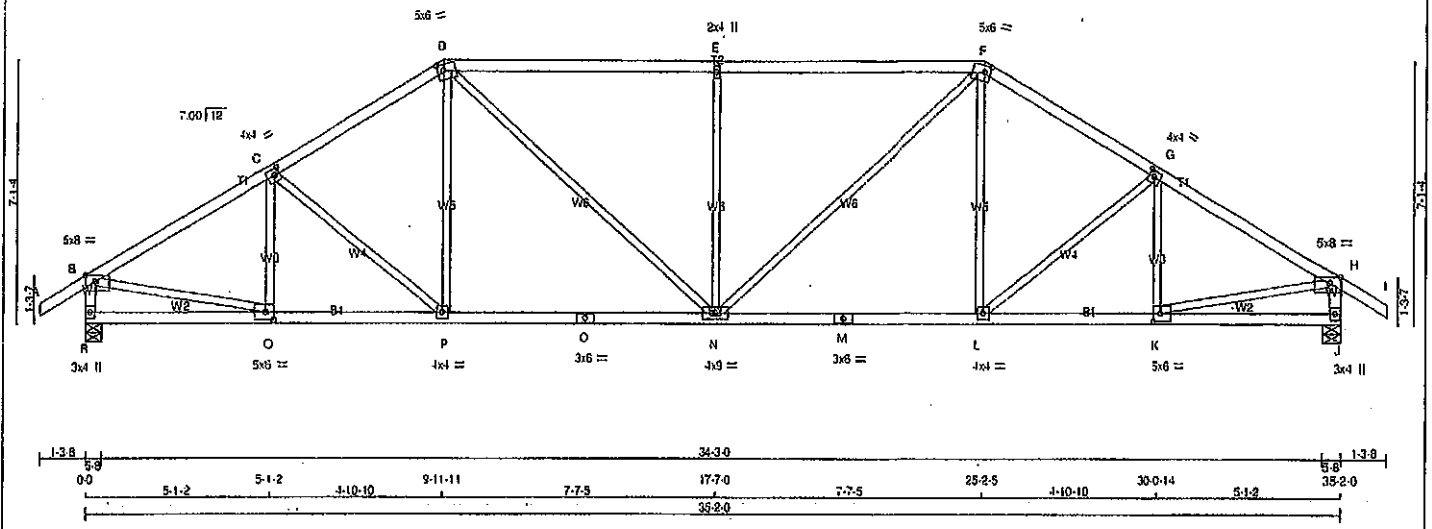
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (T) INPUT = 0.90

JSI METAL= 0.70 (C) INPUT = 1.00



Structural component only
DWG# T-2007634



LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	Edge	3.50
C TMVW-l	MT20	4.0	4.0	2.00	1.75
D TTWW-m	MT20	5.0	6.0	2.00	2.00
E TMVW-w	MT20	2.0	4.0		
F TTWW-m	MT20	5.0	6.0	2.00	2.00
G TMVW-l	MT20	4.0	4.0	2.00	1.75
H TMVW-p	MT20	5.0	8.0	Edge	3.50
J BMV1-p	MT20	3.0	4.0		
K BMVW-l	MT20	5.0	6.0	2.50	2.50
L BMVW-l	MT20	4.0	4.0		
M BS-l	MT20	3.0	6.0		
N BMVW-w-l	MT20	4.0	9.0		
O BS-l	MT20	3.0	6.0		
P BMVW-l	MT20	4.0	4.0		
Q BMVW-l	MT20	5.0	6.0	2.50	2.50
R BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT VERT	2064	2064	0	5-8
R	2064	2064	0	5-8
J	2064	2064	0	5-8

UNFACTORED REACTIONS

	1ST LOASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
R	1457	970.0
J	1457	970.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.16 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERY. LOAD (PLF)	MAX. MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (LC)	MAX. (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 32	-91.8	-91.8	0.12 (1)	10.00	Q-C	-335 0
B-C	-2537 0	-91.8	-91.8	0.39 (1)	4.00	C-P	-216 0
C-D	-2396 0	-91.8	-91.8	0.37 (1)	4.11	P-D	0 298
D-E	-2609 0	-91.8	-91.8	0.58 (1)	3.16	D-N	0 752
E-F	-2609 0	-91.8	-91.8	0.59 (1)	3.16	N-E	-860 0
F-G	-2396 0	-91.8	-91.8	0.37 (1)	4.11	N-F	0 752
G-H	-2537 0	-91.8	-91.8	0.39 (1)	4.00	L-F	0 258
H-I	0 32	-91.8	-91.8	0.12 (1)	10.00	L-G	-216 0
I-J	-2020 0	0.0	0.0	0.21 (1)	5.94	K-G	-335 0
J-H	-2020 0	0.0	0.0	0.21 (1)	5.94	Q-K	0 2253
R-O	0 0	-18.5	-18.5	0.10 (4)	10.00	K-H	0 2253
Q-P	0 2213	-18.5	-18.5	0.45 (1)	10.00		
P-O	0 2048	-18.5	-18.5	0.44 (1)	10.00		
O-N	0 2048	-18.5	-18.5	0.44 (1)	10.00		
N-M	0 2048	-18.5	-18.5	0.44 (1)	10.00		
M-L	0 2048	-18.5	-18.5	0.44 (1)	10.00		
L-K	0 2213	-18.5	-18.5	0.45 (1)	10.00		
K-J	0 0	-18.5	-18.5	0.10 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
 DL = 8.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
 ALLOWABLE DEFL.(TL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.27")

CSI: TC=0.88/1.00 (E-F:1), BC=0.45/1.00 (K-L:1), WB=0.75/1.00 (E-N:1), SSI=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS 6END=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

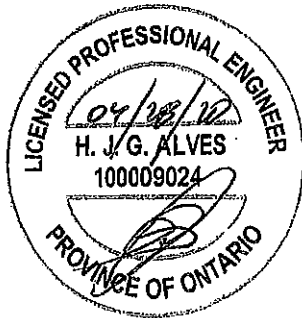
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	616 354	1667 789

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.88 (K) (INPUT = 0.90)
 JSI METAL=0.72 (M) (INPUT = 1.00)



Structural component only
 DWG# T-2007635

amarack Roof Truss, Burlington

Version 6.310 S Oct 29 2019 MTEK Industries, Inc. Tue Apr 23 10:04:07 2020 Page 1

ID:DMCubNVR6T9Foe31v6l_zns1l-H5s74hGX3DnDeWqPLm6eepobw_yk9HuavklMzMEMc

1-3.8 0-0 9-11-6 5-11-6 5-8-14 11-8-5 5-10-11 17-7-0 23-5-11 29-2-10 5-8-14 29-2-10 5-11-6 35-0-0 36-5-8 1-3-8

Scale = 1:57.6



BUILDING DESIGNER									
BEARINGS									
FACTORED			MAXIMUM FACTORED		INPUT		RECORD		
GROSS REACTION			GROSS REACTION		BRG		BRG		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
R	2054	0	2054	0	0	5-8	5-8		
J	2054	0	2054	0	0	5-8	5-8		

UNFACTORED REACTIONS									
1ST CASE		MAX. MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
R	1457	970	0	0	0	0	488	0	0
J	1457	970	0	0	0	0	488	0	0

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. CSI (LBS)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LBS)		
FR-TO		FROM TO	(PLF)		FR-TO				
A-B	0 32	-91.8	-91.8 0.12 (1)	10.00	Q-C	-252 12	0.08 (1)		
B-C	-2575 0	-91.8	-91.8 0.54 (1)	3.81	G-P	-388 0	0.41 (1)		
C-D	-2283 0	-91.8	-91.8 0.50 (1)	4.05	P-D	0 345	0.08 (1)		
D-E	-2236 0	-91.8	-91.8 0.49 (1)	4.08	D-N	0 480	0.11 (1)		
E-F	-2236 0	-87.8	-91.8 0.49 (1)	4.08	N-E	-662 0	0.84 (1)		
F-G	-2283 0	-91.8	-91.8 0.50 (1)	4.05	N-F	0 480	0.11 (1)		
G-H	-2575 0	-91.8	-91.8 0.54 (1)	3.81	L-F	0 345	0.08 (1)		
H-I	0 32	-91.8	-91.8 0.12 (1)	10.00	L-G	-388 0	0.41 (1)		
R-B	-2016 0	0.0	0.0 0.21 (1)	5.94	K-G	-252 12	0.08 (1)		
J-H	-2016 0	0.0	0.0 0.21 (1)	5.94	B-Q	0 2281	0.51 (1)		
					K-H	0 2281	0.51 (1)		
R-Q	0 0	-18.5	-18.5 0.14 (1)	10.00					
Q-P	0 2251	-18.5	-18.5 0.43 (1)	10.00					
P-Q	0 1947	-18.5	-18.5 0.38 (1)	10.00					
Q-N	0 1947	-18.5	-18.5 0.38 (1)	10.00					
N-M	0 1947	-18.5	-18.5 0.38 (1)	10.00					
M-L	0 1947	-18.5	-18.5 0.38 (1)	10.00					
L-K	0 2251	-18.5	-18.5 0.43 (1)	10.00					
K-J	0 0	-18.5	-18.5 0.14 (1)	10.00					

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018 , CBC 2012 , ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 085-14
- TFC 2011, TFC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN
LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF
LIVE LOAD

ALLOWABLE DEF.(LL) = L 360 (1.17")
CALCULATED VERT. DEF.(LL) = L 999 (0.12")
ALLOWABLE DEF.(TL) = L 360 (1.17")
CALCULATED VERT. DEF.(TL) = L 999 (0.22")

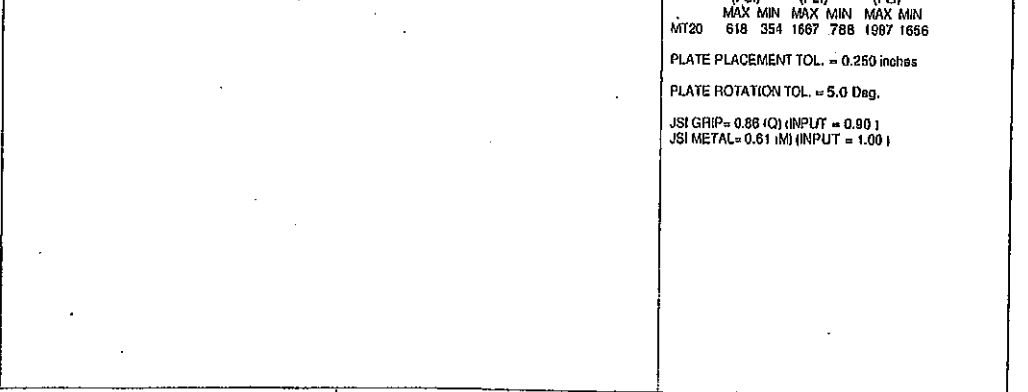
CSI: TC=0.54/1.00 (B-C:1) , BC=0.43/1.00 (P-Q:1) ,
WB=0.84/1.00 (E-N:1) , SS=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR DESIGN
(PSI) (PLI) (PLI) (PLI)

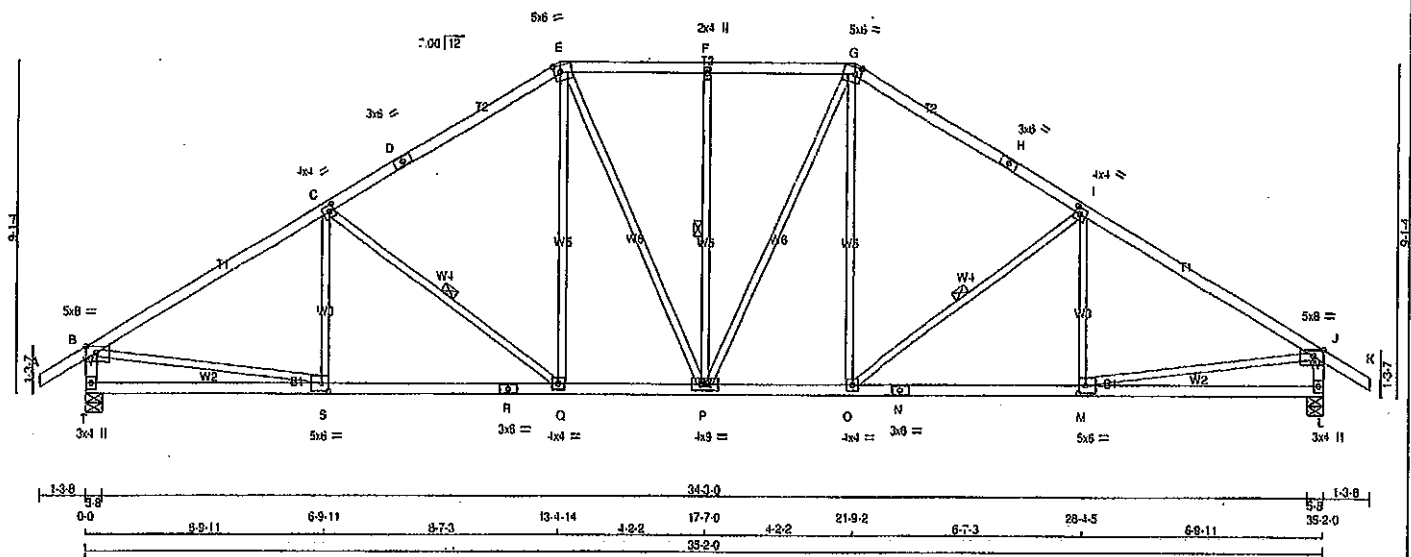


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T25	4	1	GREEN PARK HOMES	
Tamarack Pool Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 MTak Industries, Inc. Tue Apr 28 10:04:08 2020 Page 1

ID:DMCubINVRBTstFoe31v6l zns1HHQVwQhJINMdm50y3K7erAXP7FIIGSR7DqlqzMEMb

1-3.8 0.0 6.9-11 9.9-11 6.7-3 13-4-14 4-2.2 17-7-0 21-9-2 28-4-5 6.9-11 35-2-0 1-3.8
Scale = 1:50.0



TOTAL WEIGHT = 4 X 158 = 632 lb
[M/F]

LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE	PLATES			
B TMVW-p	MT20	5.0	8.0	Edge 3.50
C TMWW-I	MT20	4.0	4.0	2.00 1.75
D TS-I	MT20	3.0	6.0	
E TTWW-m	MT20	5.0	6.0	2.25 2.00
F TMW-w	MT20	2.0	4.0	
G TTWW-m	MT20	5.0	6.0	2.25 2.00
H TS-I	MT20	3.0	8.0	
I TMWW-I	MT20	4.0	4.0	2.00 1.75
J TMVW-p	MT20	5.0	8.0	Edge 3.50
L BMV1-p	MT20	3.0	4.0	
M BMWW-I	MT20	5.0	6.0	2.50 2.50
N BS-I	MT20	3.0	6.0	
O BMWW-I	MT20	4.0	4.0	
P BMWW-I	MT20	4.0	4.0	
R BS-I	MT20	3.0	8.0	
S BMWW-I	MT20	5.0	6.0	2.50 2.50
T BMV1-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
T	2084	0	0	5-8
L	2084	0	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN.	COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE
T	1457	970	0	0
L	1457	970	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-Q, F-P, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE	VERT. LOAD	TO	CS (LC)	UNBRAC	MEMB.	MAX. FACTORED	FORCE	TO	CS (LC)
FR-TO												
A-B	0	32	-91.8	-91.8	0.12 (1)	10.00	S-O	-185	51	0.08 (1)		
B-C	-2590	0	-91.8	-91.8	0.74 (1)	3.54	C-Q	-548	0	0.26 (1)		
C-D	-2158	0	-91.8	-91.8	0.68 (1)	3.91	O-E	0	431	0.10 (1)		
D-E	-2158	0	-91.8	-91.8	0.68 (1)	3.91	E-F	0	286	0.08 (1)		
E-F	-1958	0	-91.8	-91.8	0.24 (1)	4.61	F-G	-460	0	0.25 (1)		
F-G	-1958	0	-91.8	-91.8	0.24 (1)	4.61	G-H	0	208	0.08 (1)		
G-H	-2158	0	-91.8	-91.8	0.68 (1)	3.91	H-I	0	431	0.10 (1)		
H-I	-2158	0	-91.8	-91.8	0.68 (1)	3.91	I-J	-548	0	0.26 (1)		
I-J	-2690	0	-91.8	-91.8	0.74 (1)	3.54	J-K	-185	51	0.08 (1)		
J-K	0	32	-91.8	-91.8	0.12 (1)	10.00	K-L	0	2292	0.52 (1)		
L-B	-2011	0	0.0	0.0	0.20 (1)	5.95	M-J	0	2292	0.52 (1)		
L-J	-2011	0	0.0	0.0	0.20 (1)	5.95						
T-S	0	0	-18.5	-18.5	0.20 (4)	10.00						
S-R	0	2269	-18.5	-18.5	0.46 (1)	10.00						
R-Q	0	2269	-18.5	-18.5	0.46 (1)	10.00						
O-P	0	1833	-18.5	-18.5	0.36 (1)	10.00						
P-O	0	1833	-18.5	-18.5	0.36 (1)	10.00						
N-M	0	2269	-18.5	-18.5	0.46 (1)	10.00						
M-L	0	0	-18.5	-18.5	0.20 (4)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.8	PSF
DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF
DL =	7.4	PSF	
TOTAL LOAD =	39.0	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CS: TC=0.74/1.00 (I-J); BC=0.46/1.00 (M-O); WB=0.52/1.00 (J-M); SS=0.26/1.00 (I-J);

DO: LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

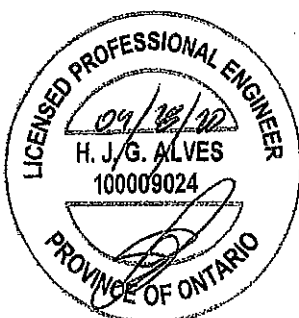
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
618	354	1667	788
1667	1587	1587	1587

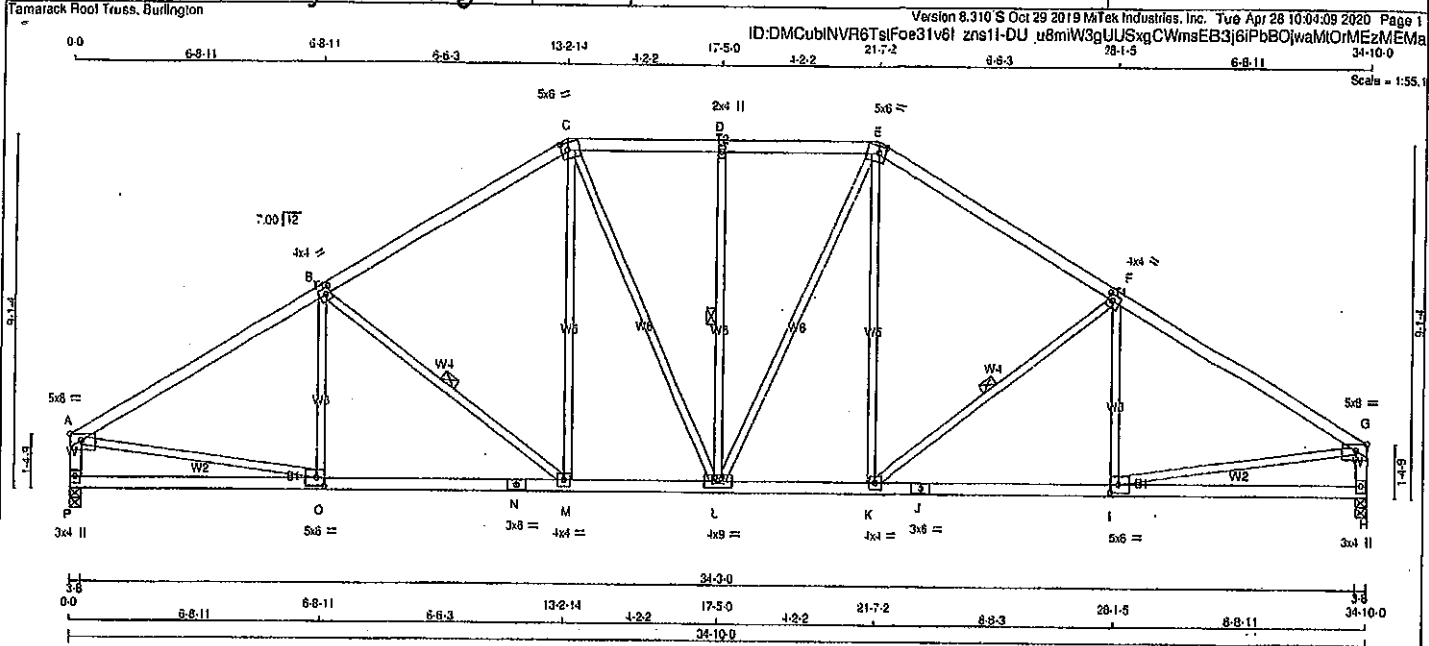
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (S) (INPUT = 0.90)
JSI METAL = 0.89 (N) (INPUT = 1.00)



Structural component only
DWG# T-2007637



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
K - L	2x4	DRY	No.2	SPF
L - M	2x4	DRY	No.2	SPF
M - N	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	2.00	Edge
B	TMVW-1	MT20	4.0	4.0	2.00	1.75
C	TTWW-m	MT20	5.0	8.0	2.25	2.00
D	TMVW-w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	2.25	2.00
F	TMVW-1	MT20	4.0	4.0	2.00	1.75
G	TMVW-p	MT20	5.0	8.0	Edge	3.50
H	BMVW-1	MT20	3.0	4.0		
I	BMVW-1	MT20	5.0	8.0	2.50	2.50
J	BS-1	MT20	3.0	6.0		
K	BMVW-1	MT20	4.0	4.0		
L	BMVW-1	MT20	4.0	9.0		
M	BMVW-1	MT20	4.0	4.0		
N	BS-1	MT20	3.0	6.0		
O	BMVW-1	MT20	5.0	8.0	2.50	2.50
P	BMVW-1	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
P	1920	0	1920	0	3-8	3-8
H	1920	0	1920	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
P	1358	891	0	0	0	0	0
H	1358	891	0	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-M, D-L, F-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

MEMB.	FORCE (LBS)	CHORDS		WEBS	
		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)
A-B	-2510	0	-91.8	-91.8	0.71 (1)
B-C	-2113	0	-91.8	-91.8	0.63 (1)
C-D	-1918	0	-91.8	-91.8	0.24 (1)
D-E	-1918	0	-91.8	-91.8	0.24 (1)
E-F	-2113	0	-91.8	-91.8	0.63 (1)
F-G	-2510	0	-91.8	-91.8	0.71 (1)
G-H	-1868	0	0.0	0.0	0.19 (1)
H-I	-1868	0	0.0	0.0	0.19 (1)
I-J	0	0	-18.5	-18.5	0.20 (4)
J-K	0	2199	-18.5	-18.5	0.45 (1)
K-L	0	2199	-18.5	-18.5	0.45 (1)
L-M	0	1797	-18.5	-18.5	0.35 (1)
M-N	0	1797	-18.5	-18.5	0.35 (1)
N-O	0	2199	-18.5	-18.5	0.45 (1)
O-P	0	2199	-18.5	-18.5	0.45 (1)
P-Q	0	0	-18.5	-18.5	0.20 (4)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 0.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.16')
CALCULATED VERT. DEFL.(LL) = L/999 (0.11')
ALLOWABLE DEFL.(TL) = L/360 (1.16')
CALCULATED VERT. DEFL.(TL) = L/999 (0.21')

CSI: TC=0.71 (A-B:1), BC=0.45 (M-O:1), WB=0.50 (I-O:1), SS=0.28 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 798 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

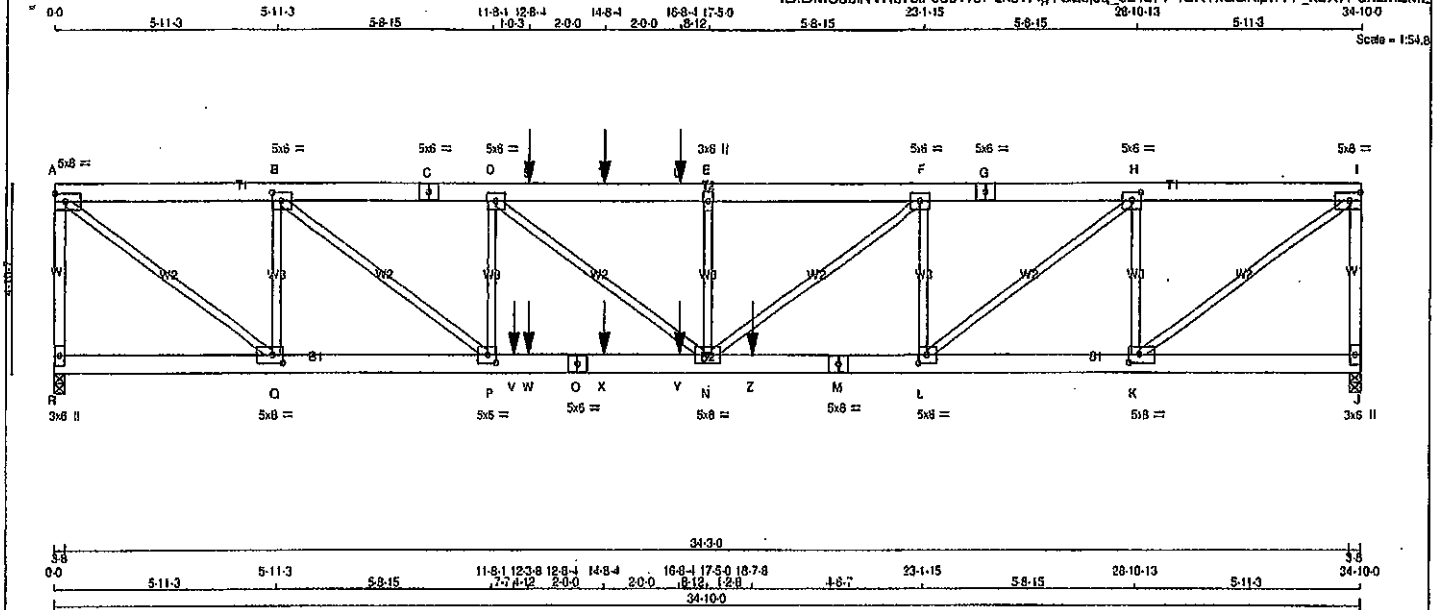
JSI GRIP = 0.64 (I) (INPUT = 0.50)
JSI METAL = 0.66 (N) (INPUT = 1.00)



JOB NAME 408223	TRUSS NAME T28	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
---------------------------	--------------------------	----------------------	-----------------	--------------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mtek Industries, Inc. Tue Apr 28 10:04:10 2020 Page 1
ID:DMCubINVR6tsIFos31v6l zns11-igYGL6j8q_cL45FP4UNTkGGKpt777_kbX7PuhzMEMZ



TOTAL WEIGHT = 2 X 183 = 366 lb

LUMBER			
N. L. G. A. RULE	CHORDS	SIZE	LUMBER
R - A	2x4	DRY	No. 2
A - C	2x6	DRY	No. 2
C - G	2x6	DRY	No. 2
G - I	2x6	DRY	No. 2
J - I	2x4	DRY	No. 2
R - O	2x6	DRY	No. 2
O - M	2x6	DRY	No. 2
M - J	2x6	DRY	No. 2

ALL WEBS 2x3 DRY No. 2
SPF DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
R - A	12	TOP
I - J	12	TOP
A - G	12	TOP
C - G	12	TOP
G - I	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
R - O	12	SIDE (0.0)
O - M	12	SIDE (0.0)
M - J	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
R	3373	0	3373	0	3-8
J	3103	0	3103	0	3-8

UNFACTORED REACTIONS

JT	1ST LOSE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
R	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
R	2380	1591	0	0	0	789
J	2190	1459	0	0	0	731

BEARING MATERIAL TO BE SPF NO. 2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX CSI (LC)	MEMB.	FORCE (LBS)	MAX CSI (LC)	MAX CSI (LC)
FR-TO		FROM TO		FR-TO			
R-A	-3316	0.0	0.0 0.58 (1)	K-I	0 4719	0.58 (1)	
A-B	-4149	0	-91.8 -91.8 0.14 (1)	A-Q	0 5171	0.64 (1)	
B-C	-7288	0	-91.8 -91.8 0.20 (1)	K-H	-2757	0 0.44 (1)	
C-D	-7288	0	-91.8 -91.8 0.20 (1)	Q-B	-2997	0 0.48 (1)	
D-S	-8278	0	-91.8 -91.8 0.28 (1)	L-H	0 3530	0.44 (1)	
S-T	-8278	0	-91.8 -91.8 0.28 (1)	B-P	0 3958	0.49 (1)	
T-U	-8278	0	-91.8 -91.8 0.28 (1)	L-F	-1791	0 0.28 (1)	
U-E	-8278	0	-91.8 -91.8 0.28 (1)	P-D	-1403	0 0.22 (1)	
E-F	-8278	0	-91.8 -91.8 0.21 (1)	N-F	0 2134	0.26 (1)	
F-G	-6588	0	-91.8 -91.8 0.19 (1)	D-N	0 1249	0.15 (1)	
G-H	-6588	0	-91.8 -91.8 0.19 (1)	N-E	-673	0 0.11 (1)	
H-I	-3787	0	-91.8 -91.8 0.13 (1)				
J-I	-3042	0.0	0.0 0.51 (1)				

R - Q	0	0	-18.5	-18.5	0.04 (4)	10.00
Q - P	0	4149	-18.5	-18.5	0.30 (1)	10.00
P - V	0	7288	-18.5	-18.5	0.67 (1)	10.00
V - W	0	7288	-18.5	-18.5	0.67 (1)	10.00
W - O	0	7288	-18.5	-18.5	0.67 (1)	10.00
O - X	0	7288	-18.5	-18.5	0.67 (1)	10.00
X - Y	0	7288	-18.5	-18.5	0.67 (1)	10.00
Y - N	0	7288	-18.5	-18.5	0.67 (1)	10.00
N - Z	0	6588	-18.5	-18.5	0.72 (1)	10.00
Z - M	0	6588	-18.5	-18.5	0.72 (1)	10.00
M - L	0	6588	-18.5	-18.5	0.72 (1)	10.00
L - K	0	3787	-18.5	-18.5	0.32 (1)	10.00
K - J	0	0	-18.5	-18.5	0.04 (4)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
S	12-8-4	-86	-86	---	BACK	VERT	TOTAL	C1
T	14-8-4	-86	-86	---	BACK	VERT	TOTAL	C1
U	16-8-4	-86	-86	---	BACK	VERT	TOTAL	C1
V	12-3-8	-1052	-1052	---	BACK	VERT	TOTAL	C1
W	12-8-4	-17	-17	---	BACK	VERT	TOTAL	C1
X	14-8-4	-17	-17	---	BACK	VERT	TOTAL	C1
Y	16-8-4	-17	-17	---	BACK	VERT	TOTAL	C1
Z	18-7-8	-1273	-1273	---	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPO 2011, TPO 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.16")

CALCULATED VERT. DEFL. (LL) = L/999 (0.21")

ALLOWABLE DEFL. (TL) = L/360 (1.16")

CALCULATED VERT. DEFL. (TL) = L/999 (0.39")

CS: TC=0.56/1.00 (A-R:1), SC=0.72/1.00 (L-N:1), WB=0.64/1.00 (A-Q:1), SS=0.37/1.00 (L-N:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618	354
	1667	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (I) INPUT = 0.90 I

JSI METAL = 0.73 (M) INPUT = 1.00 I



Structural component only
DWG# T-2007639 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T28	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 M Tek Industries, Inc. Tue Apr 28 10:04:10 2020 Page 2

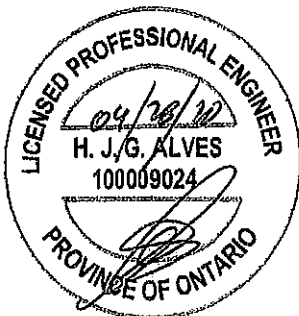
ID:DMCubINVR6TstF0e31v6I zns1I-gYGL6i8a cl45FP4UNTKGGKlp1777 kbX7PuhzMEM2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-I	MT20	5.0	8.0	2.50	3.25
B	TMWW-I	MT20	5.0	6.0	2.50	2.75
C	TS-I	MT20	5.0	6.0		
D	TMWW-I	MT20	5.0	6.0		
E	TMWW-I	MT20	3.0	6.0		
F	TMWW-I	MT20	5.0	6.0		
G	TS-I	MT20	5.0	6.0		
H	TMWW-I	MT20	5.0	6.0	2.50	2.75
I	TMWW-I	MT20	5.0	8.0	2.50	3.25
J	BMV1+p	MT20	3.0	8.0		
K	BMWW-I	MT20	5.0	8.0	2.50	3.25
L	BMWW-I	MT20	5.0	6.0	2.50	2.75
M	BS-I	MT20	5.0	6.0		
N	BMWW-I	MT20	5.0	6.0		
O	BS-I	MT20	5.0	6.0		
P	BMWW-I	MT20	5.0	6.0	2.50	2.75
Q	BMWW-I	MT20	5.0	8.0	2.50	3.25
R	BMV1+p	MT20	3.0	6.0		

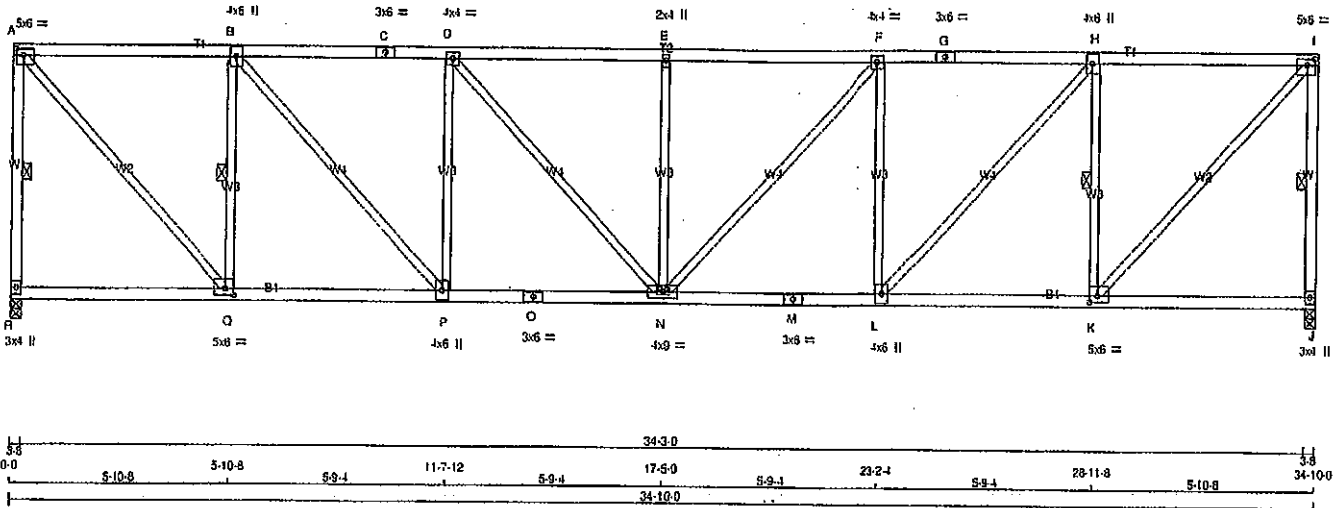
CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007639 2/2

JOB NAME 408223	TRUSS NAME T29	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Metek Industries, Inc. Tue Apr 28 10:04:11 2020 Page 1	
ID:DMCubINVR6TslF0a31v6l zns1l-As6eYSkmbIkCfQbeBuIGUoUPDljsbAlpBlyR7zMEMY				23-2-4	34-100
0-0 5-10-8 5-10-8 5-9-4 11-7-12 5-9-4 17-5-0 5-9-4 5-9-4 5-10-8 5-10-8				Scale = 1:54.6	



TOTAL WEIGHT = 2 X 157 = 314 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR
CHORDS			
R - A	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
C - G	2x4 DRY	No.2	SPF
G - I	2x4 DRY	No.2	SPF
I - J	2x4 DRY	No.2	SPF
J - O	2x4 DRY	No.2	SPF
O - M	2x4 DRY	No.2	SPF
M - J	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
R	1920	0	0	1920	0	3-8	3-8	3-8	3-8
J	1920	0	0	1920	0	3-8	3-8	3-8	3-8

UNFACTORED REACTIONS

1ST LGASE	MAX.	MIN.	COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
R	1358	891	0	0	0	0	0	0	0
J	1358	891	0	0	0	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-R, I-J, B-Q, H-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
R-A	-1876	0	0.0	A-Q	0	2281	0.51 (1)
A-B	-1583	0	-18.5	Q-B	-1540	0	0.43 (1)
B-C	-2426	0	-18.5	B-P	0	1272	0.29 (1)
C-D	-2426	0	-18.5	P-D	-931	0	0.58 (1)
D-E	-2725	0	-18.5	D-N	0	440	0.10 (1)
E-F	-2725	0	-18.5	N-E	-539	0	0.38 (1)
F-G	-2426	0	-18.5	G-F	0	440	0.10 (1)
G-H	-2426	0	-18.5	H-F	-831	0	0.59 (1)
H-I	-1583	0	-18.5	I-H	0	1272	0.29 (1)
I-J	-1876	0	0.0	J-K	-1540	0	0.43 (1)
R-Q	0	0	-18.5	K-I	0	2281	0.51 (1)
Q-P	0	1583	-18.5				
P-O	0	2426	-18.5				
O-N	0	2426	-18.5				
N-M	0	2426	-18.5				
M-L	0	2426	-18.5				
L-K	0	1583	-18.5				
K-J	0	0	-18.5				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/999 (0.31")

CSI: TC=0.83/1.00 (F-H), BC=0.44/1.00 (N-P), WB=0.58/1.00 (F-L), SSI=0.25/1.00 (A-B)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

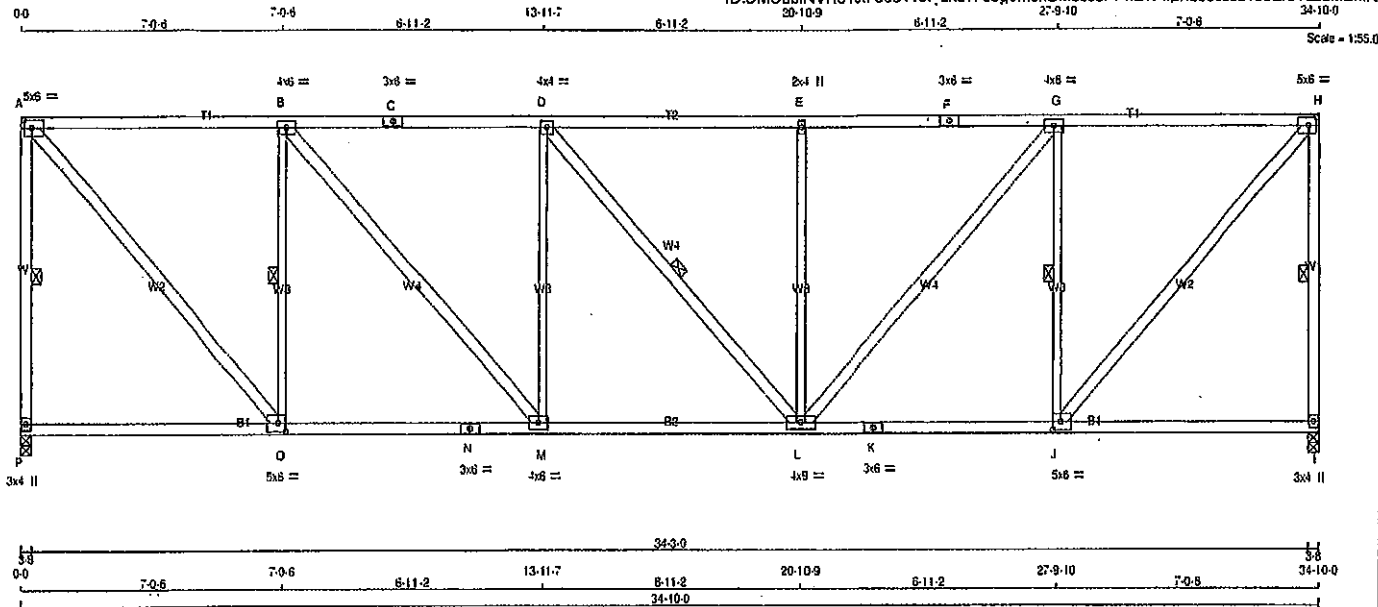
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (A) (INPUT = 0.90)
JSI METAL= 0.78 (M) (INPUT = 1.00)



Structural component only
DWG# T-2007640

JOB NAME 408223	TRUSS NAME T30	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRAWG NO.
Tandem Rack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:04:12 2020 Page 1		
ID:DMCubINVR6TstFoe31v61,zns11-e3g0mokOMbs3JPPnBvPxpHb5cdJb7s02rcVzZzMEMMX				34-10-0		



TOTAL WEIGHT = 2 X 179 = 358 lb (M)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	NO.2
P - A	2x4	DRY	No.2		
A - C	2x4	DRY	No.2		
C - F	2x4	DRY	No.2		
F - H	2x4	DRY	No.2		
I - M	2x4	DRY	No.2		
M - N	2x4	DRY	No.2		
N - K	2x4	DRY	No.2		
K - I	2x4	DRY	No.2		
ALL WEBS EXCEPT	2x4	DRY	No.2		
O - B	2x3	DRY	No.2		
M - D	2x3	DRY	No.2		
L - E	2x3	DRY	No.2		
J - G	2x3	DRY	No.2		

DRY: SEASONED LUMBER.

PLATES (Table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMWW-I	MT20	5.0	6.0	2.50	2.50
B	TMWW-I	MT20	4.0	6.0		
C	TS-I	MT20	3.0	6.0		
D	TMWW-I	MT20	4.0	4.0		
E	TMWW-I	MT20	2.0	4.0		
F	TS-I	MT20	3.0	6.0		
G	TMWW-I	MT20	4.0	6.0		
H	TMWW-I	MT20	5.0	6.0	2.50	2.50
I	BMV1+p	MT20	3.0	4.0		
J	BMV1+p	MT20	5.0	6.0	2.50	2.50
K	BS-I	MT20	3.0	6.0		
L	BMV1+p	MT20	4.0	9.0		
M	BMV1+p	MT20	4.0	6.0		
N	BS-I	MT20	3.0	6.0		
O	BMV1+p	MT20	5.0	6.0	2.50	2.50
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	GROSS REACTION	DOWN	HORIZ	UP/LIFT	IN-SX	IN-SX	IN-SX
P	1920	0	1920	0	0	3-8	3-8	3-8	3-8
I	1920	0	1920	0	0	3-8	3-8	3-8	3-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERMALIVE	WIND	DEAD	SOIL
P	1358	891	0	0	0	487	0
I	1358	891	0	0	0	487	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-P, H-I, B-O, D-L, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO			FR-TO		
P-A	-1888 0	0.0	0.0 0.57 (1)	4.91	A-O	0 2158	0.35 (1)
A-B	-1432 0	-91.8	-91.8 0.78 (1)	4.31	O-B	-1466 0	0.62 (1)
B-C	-2081 0	-91.8	-91.8 0.89 (1)	3.60	B-M	0 988	0.18 (1)
C-D	-2081 0	-91.8	-91.8 0.89 (1)	3.60	M-D	-817 0	0.81 (1)
D-E	-2080 0	-91.8	-91.8 0.88 (1)	4.00	D-L	-2 0	0.00 (1)
E-F	-2080 0	-91.8	-91.8 0.89 (1)	3.60	L-E	-818 0	0.81 (1)
F-G	-2080 0	-91.8	-91.8 0.89 (1)	3.60	L-G	0 984	0.16 (1)
G-H	-1432 0	-91.8	-91.8 0.78 (1)	4.31	J-G	-1465 0	0.62 (1)
H-I	-1888 0	0.0	0.0 0.57 (1)	4.91	J-H	0 2158	0.35 (1)
P-O	0 0	-18.5	-18.5 0.22 (4)	10.60			
O-N	0 1432	-18.5	-18.5 0.35 (1)	10.00			
N-M	0 1432	-18.5	-18.5 0.35 (1)	10.00			
M-L	0 2081	-18.5	-18.5 0.42 (1)	10.00			
L-K	0 1432	-18.5	-18.5 0.35 (1)	10.00			
K-J	0 1432	-18.5	-18.5 0.35 (1)	10.00			
J-I	0 0	-18.5	-18.5 0.22 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPC 2011, TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.16")
CALCULATED VERT. DEFL. (LL) = L/999 (0.12")
ALLOWABLE DEFL. (TL) = L/360 (1.16")
CALCULATED VERT. DEFL. (TL) = L/999 (0.24")

CS1: TC=0.89/1.00 (E-G:1), SC=0.42/1.00 (L-M:1), WB=0.81/1.00 (D-M:1), SS=0.30/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

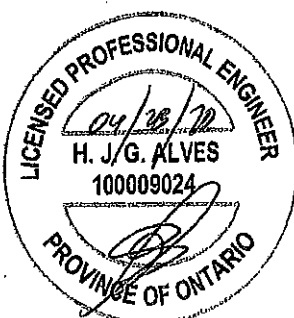
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLF) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 768 1987 1658

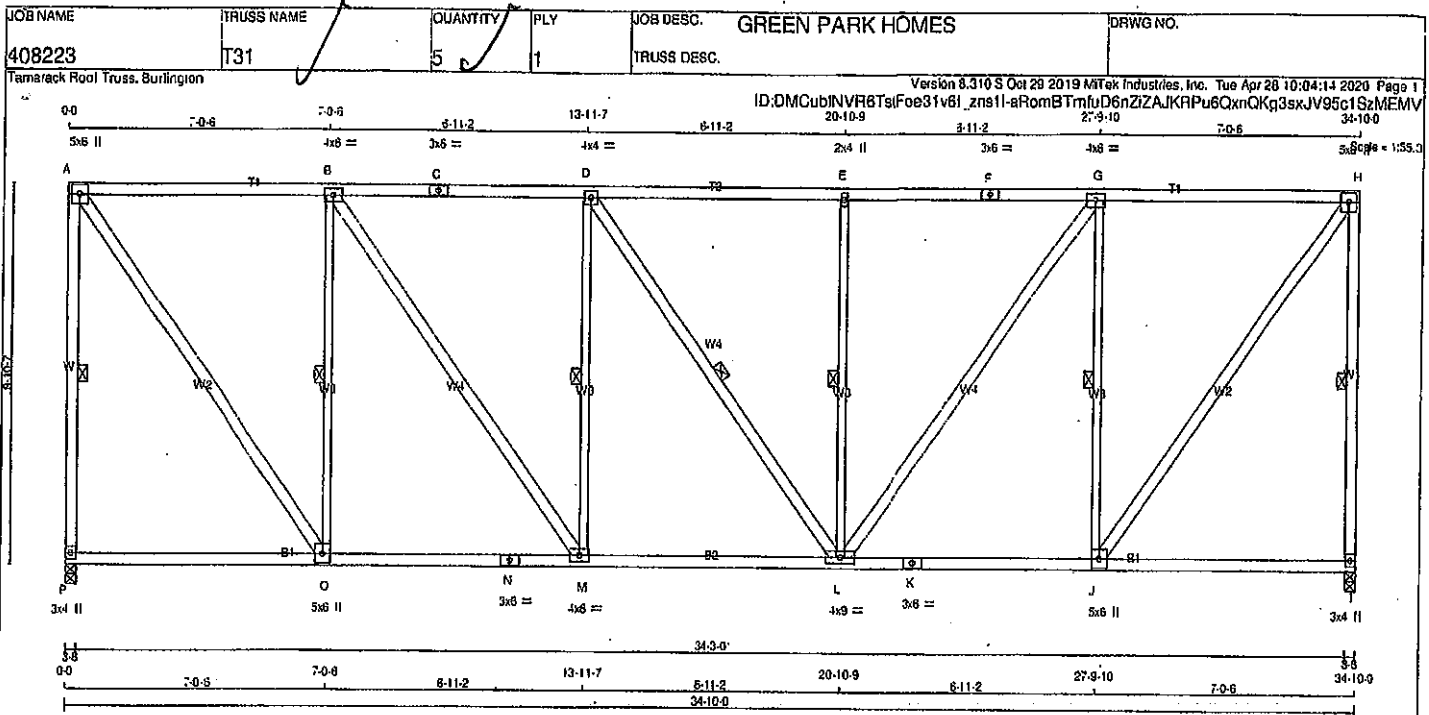
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (H) (INPUT = 0.99)
JSI METAL = 0.46 (K) (INPUT = 1.00)



Structural component only
DWG# T-2007641



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
P - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
I - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			
O - B	2x3	DRY	No.2
M - D	2x3	DRY	No.2
L - E	2x3	DRY	No.2
J - G	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tab's in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0		
B	TMVW-t	MT20	4.0	6.0		
C	TS-t	MT20	3.0	6.0		
D	TMVW-t	MT20	4.0	4.0		
E	TMVW-w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	6.0		
H	TMVW+p	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMVW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMVW-w	MT20	4.0	6.0		
M	BMVW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMVW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
P	1920	0	1920	0	3-8	3-8
I	1920	0	1920	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1358	891 0	0.0	0.0	0 0	467 0	0 0
I	1358	891 0	0.0	0 0	0 0	467 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-P, H-I, B-O, D-M, O-L, E-L, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS					
MEMB.	MAX. FACTORED	FORCE	FACTORED	MAX.	MAX.	MEMB.	MAX. FACTORED	FORCE	MAX.		
		(LBS)	VERT. LOAD	LC1	CS1 (LC)			(LBS)	CS1 (LC)		
FR-TO			FROM TO	PLF	LENGTH	FR-TO					
P-A	-1868	0	0.0	0.0	0.88 (1)	4.91	A-O	0	1939	0.32 (1)	
A-B	-1183	0	-91.8	-91.8	0.75 (1)	4.69	O-B	-1468	0	0.96 (1)	
B-C	-1719	0	-91.8	-91.8	0.82 (1)	3.98	B-M	0	916	0.15 (1)	
C-D	-1719	0	-91.8	-91.8	0.82 (1)	3.98	M-D	-617	0	0.41 (1)	
D-E	-1718	0	-91.8	-91.8	0.83 (1)	4.37	D-L	-2	0	0.00 (1)	
E-F	-1718	0	-91.8	-91.8	0.83 (1)	3.98	L-E	-616	0	0.41 (1)	
F-G	-1718	0	-91.8	-91.8	0.83 (1)	3.98	G-L	0	913	0.15 (1)	
G-H	-1183	0	-91.8	-91.8	0.75 (1)	4.69	J-G	-1465	0	0.96 (1)	
H-I	-1868	0	0.0	0.0	0.88 (1)	4.91	J-H	0	2000	0.32 (1)	
P-O	0	0	-18.5	-18.5	0.22 (4)	10.00					
O-N	0	1183	-18.5	-18.5	0.32 (4)	10.00					
N-M	0	1183	-18.5	-18.5	0.32 (4)	10.00					
M-L	0	1719	-18.5	-18.5	0.36 (1)	10.00					
L-K	0	1183	-18.5	-18.5	0.32 (4)	10.00					
K-J	0	1183	-18.5	-18.5	0.32 (4)	10.00					
J-I	0	0	-18.5	-18.5	0.22 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF OSC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPC 2011, TPC 2014

155 % OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CS: TC=0.88/1.00 (H:I), BC=0.38/1.00 (L:M:1), WB=0.88/1.00 (B:C:1), SS=0.30/1.00 (G:H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (H) (INPUT = 0.90)
JSI METAL = 0.43 (H) (INPUT = 1.00)



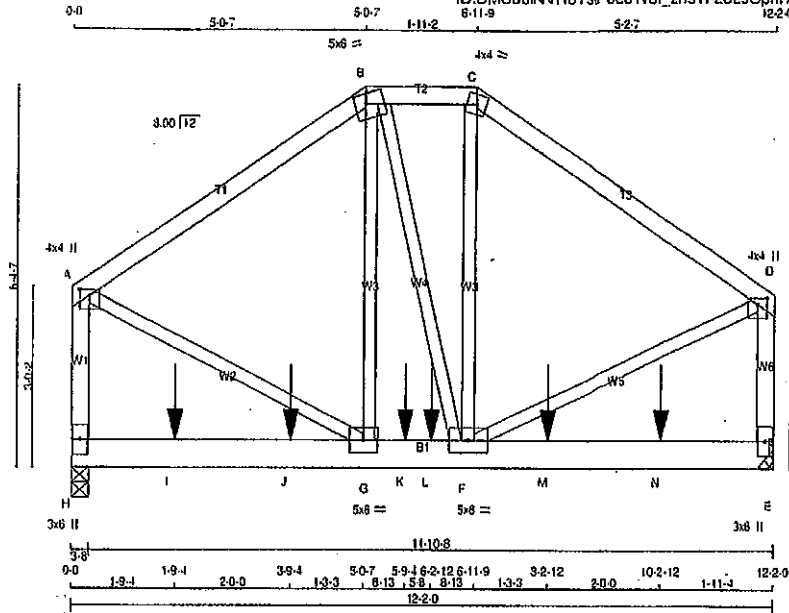
Structural component only
DWG# T-2007642

JOB NAME 408223	TRUSS NAME T32	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
TRUSS DESC.				Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Tue Apr 26 10:04:15 2020 Page 1	

Farmrack Roof Truss, Burlington

ID:DMCubINVR6T9Foe31v6I_zns11-2dL9OgnHWEeAs8M11y8KzCFqAovATkp/AauzMEMU
6-11-9 12-2-0

Scale = 1/32.0



TOTAL WEIGHT = 68 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2	
B - C	2x4	DRY	No.2	
C - D	2x4	DRY	No.2	
H - A	2x4	DRY	No.2	
E - D	2x4	DRY	No.2	
H - E	2x6	DRY	No.2	

ALL WEBS 2x3 DRY EXCEPT

DRY: SEASONED LUMBER.

PLATES (Table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTWW-m	MT20	5.0	6.0	2.00	2.25
C	TTW-m	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMV+p	MT20	3.0	6.0		
F	BMVWW-1	MT20	5.0	8.0		
G	BMVWW-1	MT20	5.0	8.0		
H	BMV+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQD
SPF	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
H	1233	0	1233	0
E	1218	0	1218	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E MINIMUM BEARING LENGTH AT JOINT E = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	887	596	0	0	0	271	0
E	856	588	0	0	0	288	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORZ. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LBS)
FR-TO					FR-TO		
A-B	-853	0	-91.8	91.8 0.48 (1)	G-B	0	122
B-C	-721	0	-91.8	91.8 0.07 (1)	H-F	0	34
C-D	-884	0	-91.8	91.8 0.51 (1)	F-C	0	154
H-A	-1065	0	0	0.17 (1)	A-G	0	795
E-D	-1057	0	0	0.18 (1)	F-D	0	794
H-I	0	0	-18.5	18.5 0.20 (1)			
I-J	0	0	-18.5	18.5 0.20 (1)			
J-G	0	0	-18.5	18.5 0.20 (1)			
G-K	0	711	-18.5	18.5 0.23 (1)			
K-L	0	711	-18.5	18.5 0.28 (1)			
L-F	0	711	-18.5	18.5 0.28 (1)			
F-M	0	0	-18.5	18.5 0.20 (1)			
M-N	0	0	-18.5	18.5 0.20 (1)			
N-E	0	0	-18.5	18.5 0.20 (1)			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	1-9-4	-172	-172	---	BACK	VERT	TOTAL	---	C1
J	3-9-4	-195	-195	---	BACK	VERT	TOTAL	---	C1
K	5-9-4	-187	-187	---	BACK	VERT	TOTAL	---	C1
L	6-2-12	-187	-187	---	BACK	VERT	TOTAL	---	C1
M	6-2-12	-195	-195	---	BACK	VERT	TOTAL	---	C1
N	10-2-12	-172	-172	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.51 1.00 (C-D:1), BC=0.28 1.00 (F-G:1),
WB=0.20 1.00 (A-G:1), SS=0.20 1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1987 768 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (D) (INPUT = 0.90)
JSI METAL= 0.23 (D) (INPUT = 1.00)

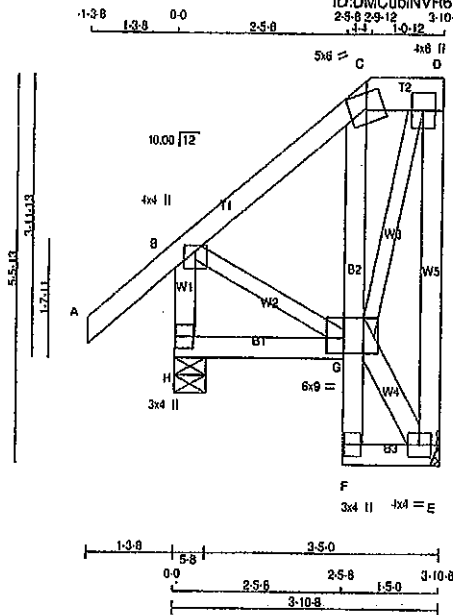


Structural component only
DWG# T-2007643

JOB NAME 408223	TRUSS NAME T34S	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
--------------------	--------------------	---------------	----------	-------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Tue Apr 26 10:04:16 2020 Page 1



Scale = 1:30.1

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x6	DRY	No.2
E - D	2x4	DRY	No.2
H - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
F - C	2x4	DRY	No.2
F - E	2x4	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
B - G	2x3	DRY	No.2
G - D	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (tablets in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	1.0	1.00 2.00
C	TTV-m	MT20	5.0	6.0	
D	TMVW+p	MT20	4.0	6.0	
E	BMVW-1	MT20	4.0	4.0	
F	BMV+p	MT20	3.0	4.0	
G	BMVWVW-1	MT20	6.0	9.0	2.50 3.00
H	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	BRG	IN-SX	IN-SX
E	214	0	214	0	0	MECHANICAL			
H	341	0	341	0	0	5-8	5-8		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	151	90.0	0.0	0.0	0.0	52.0	0.0
H	239	170.0	0.0	0.0	0.0	69.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. MAX. (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. MAX. (LBS)	MAX. (LBS)
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 41	-91.8 -91.8 0.14 (5)	10.00	G-E	-6 0	0.00 (1)	
B-C	-62 0	-91.8 -91.8 0.12 (1)	6.25	B-G	0 53	0.01 (1)	
C-D	-51 0	-91.8 -91.8 0.01 (1)	6.25	G-D	0 154	0.03 (1)	
E-D	-197 0	0.0 0.0 0.10 (1)	7.81				
H-B	-317 0	0.0 0.0 0.03 (1)	7.81				
H-G	0 0	-18.5 -18.5 0.04 (4)	10.00				
F-G	0 13	0.0 0.0 0.01 (1)	10.00				
G-C	-138 0	0.0 0.0 0.01 (1)	7.81				
F-E	0 4	-18.5 -18.5 0.01 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBCG 2018, CBC 2012, ABC 2018
- PART 9 OF CBCG 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (A-B:5), BC=0.04/1.00 (B-H:4), WB=0.03/1.00 (D-G:1), SS=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1887 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.24 (B) INPUT = 0.90
JSI METAL = 0.08 (B) INPUT = 1.00

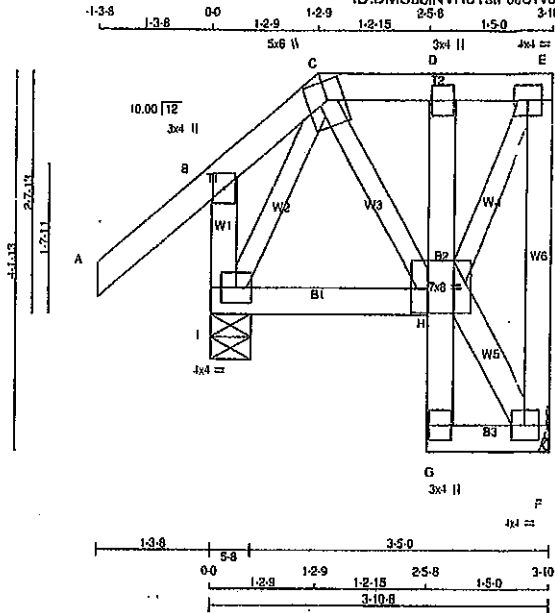


Structural component only
DWG# T-2007644

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408223	T35S	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:04:17 2020 Page 1
ID:DMCubINVR6TstFoe31v6I_zns1I-0TvpVoXB8UMQAH1_S78W12dVdRQGSEmC7KGenzMEMS



Scale = 1/2" = 1'-0"

TOTAL WEIGHT = 2 X 29 = 57 lb (M)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	QESCR.
A - C	2x4 DRY	No. 2	SPF		
C - E	2x4 DRY	No. 2	SPF		
F - E	2x4 DRY	No. 2	SPF		
I - B	2x4 DRY	No. 2	SPF		
I - H	2x4 DRY	No. 2	SPF		
G - D	2x4 DRY	No. 2	SPF		
G - F	2x4 DRY	No. 2	SPF		
ALL WEBS EXCEPT H - F	2x3 DRY	No. 2	SPF		
H - F	2x4 DRY	No. 2	SPF		

DRY: SEASONED LUMBER.

PLATES (Tables in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
C	TTWW+m	MT20	5.0	6.0	2.50	2.25	
D	TMV+p	MT20	3.0	4.0			
E	TMVW-t	MT20	4.0	4.0			
F	BMVW1-t	MT20	4.0	4.0			
G	BMV+p	MT20	3.0	4.0			
H	BVMWVW-1	MT20	7.0	8.0	3.25	2.25	
I	BMVW1-t	MT20	4.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORZ	UPLIFT
F	191 0	191 0	0 0	MECHANICAL
I	363 0	363 0	0 0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
F	135 86 -13 0 0 0 0 0 49 0 0 0
I	254 183 0 0 0 0 0 0 72 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 41	-91.8 -91.8	0.14 (5)	10.00	H-F	-5 0	0.00 (1)
B-C	-45 0	-91.8 -91.8	0.14 (5)	6.25	H-E	0 141	0.03 (1)
C-D	-59 0	-91.8 -91.8	0.02 (1)	6.25	C-H	0 48	0.01 (1)
D-E	-57 0	-91.8 -91.8	0.02 (1)	6.25	I-C	-98 28	0.02 (1)
F-E	-175 0	0.0 0.0	0.05 (1)	7.81			
I-B	-255 0	0.0 0.0	0.03 (1)	7.81			
I-H	-13 45	-18.5 -18.5	0.04 (4)	6.25			
G-H	0 13	0.0 0.0	0.01 (1)	10.00			
H-D	-135 0	0.0 0.0	0.01 (1)	7.81			
G-F	0 4	-18.5 -18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
OL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.0/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.14-1.00 (A-B-5), BC=0.04-1.00 (H-I-4), WB=0.03-1.00 (E-H-1), SSI=0.09-1.00 (A-B-5)

DDL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 758 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

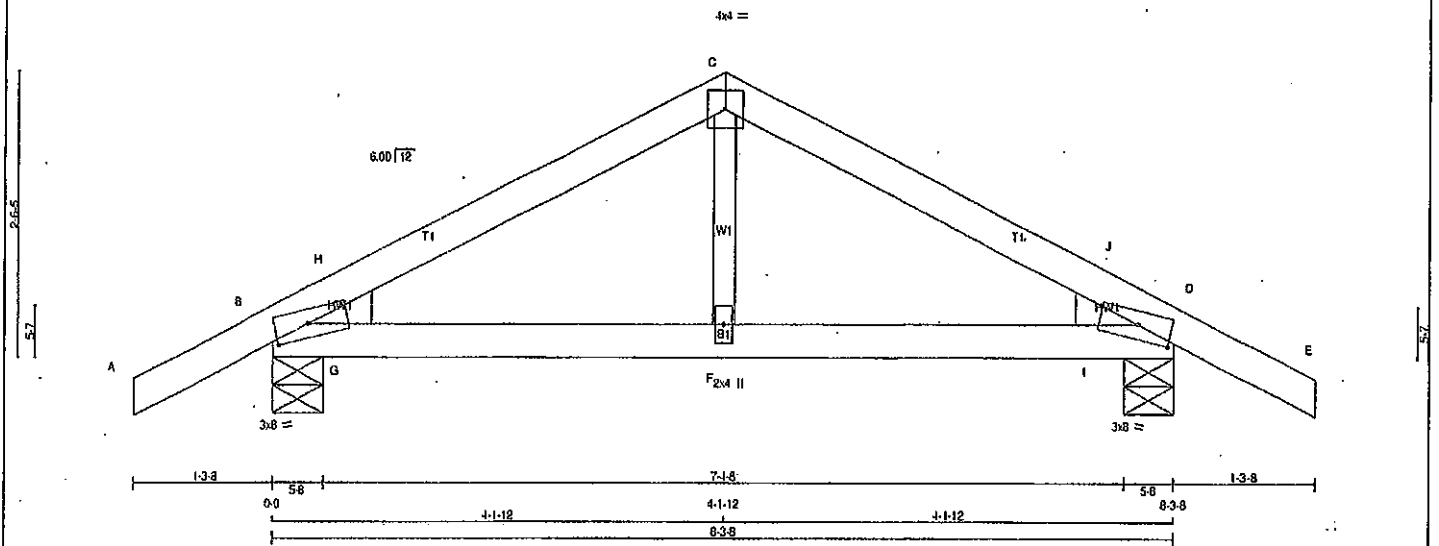
JSI GRIP=0.18 (E) INPUT = 0.50)
JSI METAL=0.09 (B) INPUT = 1.00)



Structural component only
DWG# T-2007645

JOB NAME 408223	TRUSS NAME T36	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 MTak Industries, Inc. Tue Apr 28 10:04:18 2020 Page 1	
ID:DMCubiNVR6TslFoe31v6l zns1i-TC1H0ip9yRcD1KsxY9WL2ybo5i127vNvQn3qBDzMEMR					

Scale = 1:18.9



LUMBER	N. L. G. A. RULES	SIZE	DRY	LUMBER	DESCR.
CHORDS					
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (tablets in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMBH1-m	MT20	3.0	8.0	1.50 3.75
C TTW-p	MT20	4.0	4.0	
D TMBH1-m	MT20	3.0	8.0	1.50 3.75
F 8MW-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
		GROSS REACTION		GROSS REACTION		BRG		HEEL	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE	
B	582	0	582	0	0	5-8	5-8	2x4 L	
D	582	0	582	0	0	5-8	5-8	2x4 R	

UNFACTORED REACTIONS

JT	1ST CASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	409	282	0	0	0	0	127	0
D	409	282	0	0	0	0	127	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 23	-91.8	-91.8	0.12 (1)	10.00	F-C	0 170 0.04 (1)
B-H	-539 0	-91.8	-91.8	0.04 (1)	6.25	G-H	-158 3 0.00 (1)
H-C	-525 0	-91.8	-91.8	0.15 (1)	6.25	I-J	-158 3 0.00 (1)
C-J	-525 0	-91.8	-91.8	0.15 (1)	8.25		
J-D	-539 0	-91.8	-91.8	0.04 (1)	6.25		
D-E	0 23	-91.8	-91.8	0.12 (1)	10.00		
B-G	0 465	-18.5	-18.5	0.21 (1)	10.00		
G-F	0 465	-18.5	-18.5	0.21 (1)	10.00		
F-I	0 465	-18.5	-18.5	0.21 (1)	10.00		
I-D	0 465	-18.5	-18.5	0.21 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.15/1.00 (C-J:1), BC=0.21/1.00 (F-I:1),
WB=0.04/1.00 (C-F:1), SS=0.12/1.00 (C-J:1)

DOL LUMBER=1.00 NAL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

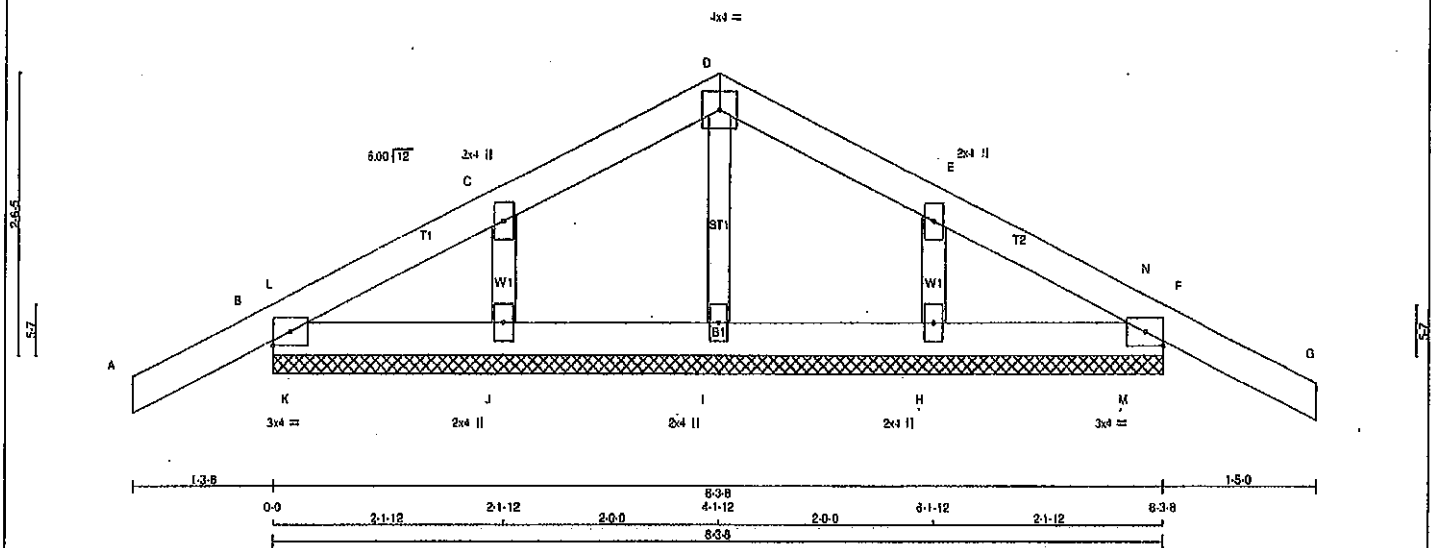
JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007646

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	G36	1	1	GREEN PARK HOMES	
Paradeck Roof Truss, Burlington		TRUSS DESC.		Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:33:49 2020 Page 1	

ID:DMCubiNVR6TstFoe31v61_zns11-pdnKxwTznXyK318MLaVXNrurbl161oKemYn(PzMEMu
5-1-12 8-3-8 9-8-8
Scale = 1:18.1



TOTAL WEIGHT = 27 lb (M)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - G	2x4	DRY	No.2	SPF		
B - F	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
ALL GABLE WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						
GABLE STUDS SPACED AT 2-0-0 OC.						

PLATES (tablets in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-1	MT20	3.0	4.0		Edge
C	TMW-w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TMW-w	MT20	2.0	4.0		
F	TMB1-1	MT20	3.0	4.0		Edge
H, I, J	BMW1-w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

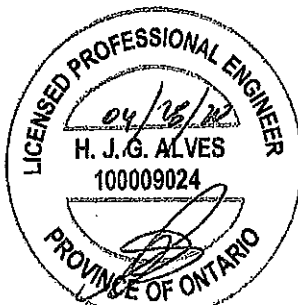
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

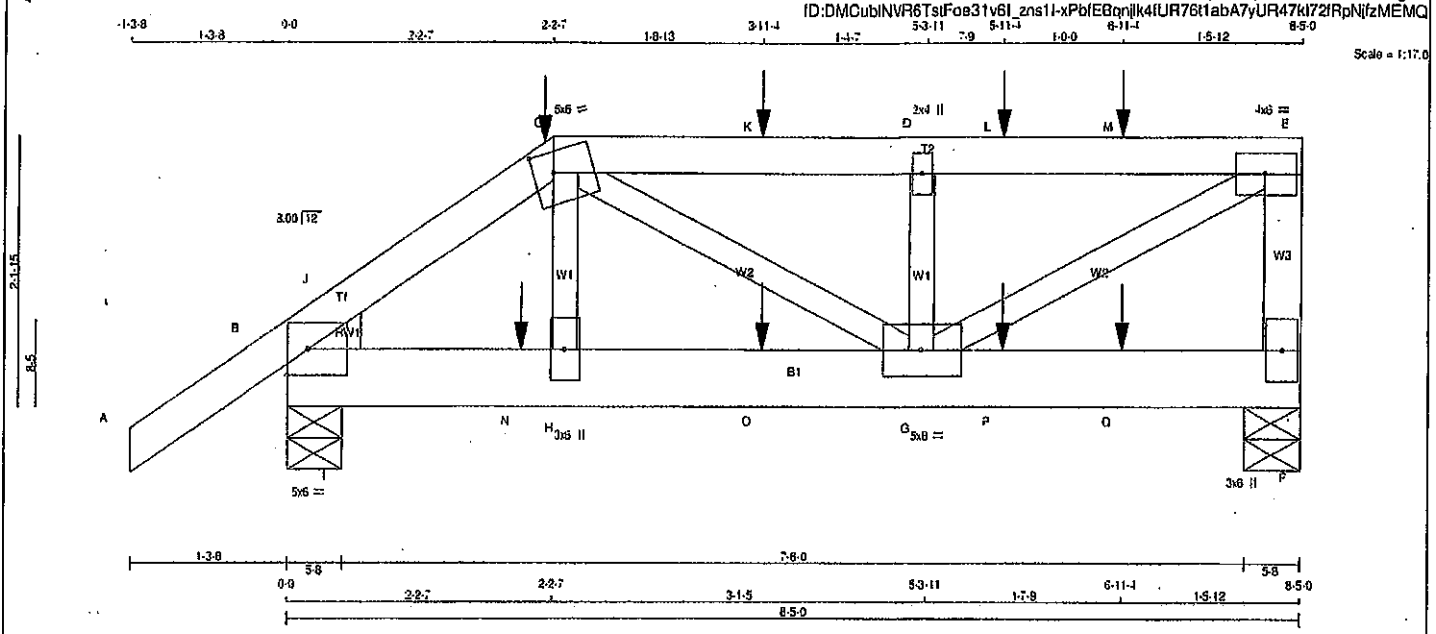
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)
FR-TO		FROM TO			FR-TO		FROM TO		
A-B	0 23	-91.8 -91.8 0.12 (1)	10.00		I-D	-115 0	0.02 (1)		
B-L	-61 0	-91.8 -91.8 0.01 (4)	6.25	J-C	-218 0	0.03 (1)			
L-C	-39 0	-91.8 -91.8 0.06 (1)	6.25	K-E	-218 0	0.03 (1)			
C-D	-45 0	-91.8 -91.8 0.06 (1)	6.25	K-L	-52 7	0.00 (1)			
D-E	-45 0	-91.8 -91.8 0.06 (1)	6.25	M-N	-52 7	0.00 (1)			
E-N	-39 0	-91.8 -91.8 0.06 (1)	6.25						
N-F	-61 0	-91.8 -91.8 0.01 (4)	6.25						
F-G	0 25	-91.8 -91.8 0.14 (1)	10.00						
B-K	0 44	-18.5 -18.5 0.03 (1)	10.00						
K-J	0 44	-18.5 -18.5 0.03 (1)	10.00						
J-I	0 32	-18.5 -18.5 0.02 (1)	10.00						
I-H	0 32	-18.5 -18.5 0.02 (1)	10.00						
H-M	0 44	-18.5 -18.5 0.03 (1)	10.00						
M-F	0 44	-18.5 -18.5 0.03 (1)	10.00						

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF
SPACING = 24.0 IN. C/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015
THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 888-09, CSA 888-14
- TPIC 2011, TPIC 2014
55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD
CSI: TC=0.14/1.00 (F-G:1), BC=0.03/1.00 (H-M:1), WB=0.03/1.00 (E-H:1), SSI=0.10/1.00 (F-G:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1907 1656
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007621

JOB NAME 408223	TRUSS NAME IT37	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tagarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 M/Tex Industries, Inc. Tue Apr 28 10:04:19 2020 Page 1 ID:DMCubINVR6TstFos31v6L_zns11-xPb(E8QnjK4fUR76t1abA7yUR47ki72RpnJfzMEMQ	



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
B - F	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0	Edge
C	TTWW-m	MT20	5.0	6.0	2.00 2.00
D	TMW-w	MT20	2.0	4.0	
E	TMW-I	MT20	4.0	6.0	
F	BMV-I	MT20	3.0	6.0	
G	BMVWW-I	MT20	5.0	6.0	
H	BMV-w	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG	HEEL WEDGE
JT	VERT	HORZ	UP/LIFT	IN-SX	
F	760	0	0	5-8	5-8
B	823	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL
F	537 352 0 0 0 0 0 0 185 0 0 0
B	580 392 0 0 0 0 0 0 188 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO				FR-TO			
A-B	0 26	-91.8	-91.8 0.13 (1)	H-C	0 73	0.02 (4)	0.02 (4)
B-J	-1088 0	-91.8	-91.8 0.08 (1)	C-G	0 252	0.06 (1)	0.06 (1)
J-C	-860 0	-91.8	-91.8 0.08 (1)	G-D	-528 0	0.09 (1)	0.09 (1)
C-K	-923 0	-91.8	-91.8 0.24 (1)	E-G	0 1082	0.28 (1)	0.28 (1)
K-D	-923 0	-91.8	-91.8 0.24 (1)	I-J	0 207	0.09 (1)	0.09 (1)
D-L	-924 0	-91.8	-91.8 0.24 (1)				
L-M	-924 0	-91.8	-91.8 0.24 (1)				
M-E	-924 0	-91.8	-91.8 0.24 (1)				
F-E	-582 0	0.0	0.0 0.08 (1)				
B-I	0 701	-18.5	-18.5 0.14 (1)				
I-N	0 701	-18.5	-18.5 0.14 (1)				
N-H	0 701	-18.5	-18.5 0.14 (1)				
H-O	0 705	-18.5	-18.5 0.15 (1)				
O-G	0 705	-18.5	-18.5 0.15 (1)				
G-P	0 0	-18.5	-18.5 0.07 (1)				
P-Q	0 0	-18.5	-18.5 0.07 (1)				
Q-F	0 0	-18.5	-18.5 0.07 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	2-2-7	-88	-88	---	FRONT	VERT	TOTAL	C1
K	3-11-4	-83	-83	---	FRONT	VERT	TOTAL	C1
L	5-11-4	-83	-83	---	FRONT	VERT	TOTAL	C1
M	6-11-4	-83	-83	---	FRONT	VERT	TOTAL	C1
N	1-11-4	-48	-48	---	FRONT	VERT	TOTAL	C1
O	3-11-4	-48	-48	---	FRONT	VERT	TOTAL	C1
P	5-11-4	-48	-48	---	FRONT	VERT	TOTAL	C1
Q	6-11-4	-48	-48	---	FRONT	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 085-09, CSA 088-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/989 (0.03")

CSI: TC=0.24,1.00 (D-E:1), BC=0.15,1.00 (G-H:1),
WB=0.26,1.00 (E-G:1), SS=0.25,1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

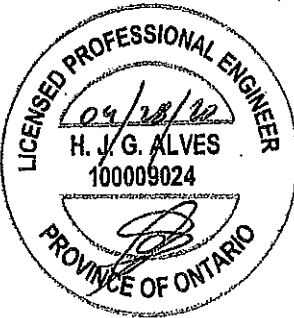
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

MAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1697 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (E) (INPUT = 0.90)
JSI METAL= 0.24 (E) (INPUT = 1.00)



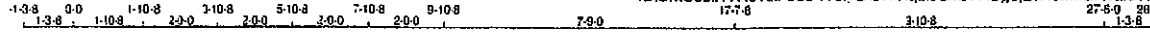
Structural component only
DWG# T-2007647

JOB NAME 408223	TRUSS NAME G39	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
TRUSS DESC.					

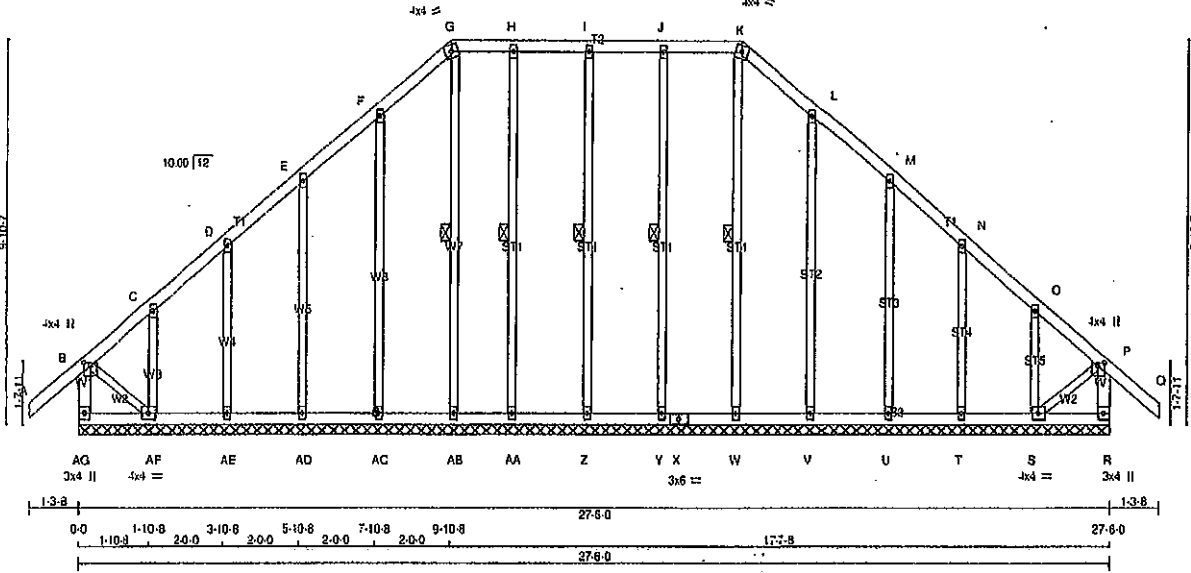
Tagharack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:03:50 2020 Page 1

ID:DMCubINVR6TslFoe31v6l_zns1l-HoL8GToYr4Bq0ZvH0mW2R1aIDRRBmn_QHLBrzMEM



Scale = 1:54.7



TOTAL WEIGHT = 2 X 152 = 303 lb (M)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
AG - B	2x4	DRY	No.2	SPF		
A - G	2x4	DRY	No.2	SPF		
G - K	2x4	DRY	No.2	SPF		
K - Q	2x4	DRY	No.2	SPF		
R - P	2x4	DRY	No.2	SPF		
AG - X	2x4	DRY	No.2	SPF		
X - R	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
ALL GABLE WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						
GABLE STUDS SPACED AT	2-0-0	OC.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF K-W, J-Y, I-Z, H-AA, G-AB.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM TO		FR-TO			
AG-B	-292 0	0.0 0.0 0.03 (1)	7.81	W-K	-140 0	0.09 (1)	
A-B	0 41	-91.8 -91.8 0.13 (1)	10.00	Y-J	-210 0	0.14 (1)	
B-C	-80 0	-91.8 -91.8 0.12 (1)	8.25	Z-I	-181 0	0.12 (1)	
C-D	-23 0	-91.8 -91.8 0.05 (1)	6.25	AA-H	-182 0	0.12 (1)	
D-E	-26 0	-91.8 -91.8 0.05 (1)	6.25	V-L	-203 0	0.27 (1)	
E-F	-19 0	-91.8 -91.8 0.05 (1)	6.25	U-M	-175 0	0.13 (1)	
F-G	-29 0	-91.8 -91.8 0.05 (1)	6.25	T-N	-192 0	0.07 (1)	
G-H	-12 0	-91.8 -91.8 0.04 (1)	5.25	S-O	-126 0	0.02 (1)	
H-I	-12 0	-91.8 -91.8 0.04 (1)	6.25	AB-G	-121 0	0.08 (1)	
I-J	-12 0	-91.8 -91.8 0.05 (1)	6.25	AC-F	-202 0	0.27 (1)	
J-K	-12 0	-91.8 -91.8 0.05 (1)	6.25	AD-E	-176 0	0.13 (1)	
K-L	-30 0	-91.8 -91.8 0.05 (1)	6.25	AE-D	-192 0	0.07 (1)	
L-M	-19 0	-91.8 -91.8 0.05 (1)	6.25	AF-C	-126 0	0.02 (1)	
M-N	-28 0	-91.8 -91.8 0.05 (1)	6.25	BA-F	0 32	0.01 (1)	
N-O	-24 0	-91.8 -91.8 0.05 (1)	6.25	SA-P	0 32	0.01 (1)	
O-P	-60 0	-91.8 -91.8 0.12 (1)	8.25				
P-Q	0 41	-91.8 -91.8 0.13 (1)	10.00				
R-P	-293 0	0.0 0.0 0.03 (1)	7.81				
AG-AF	0 0	-18.5 -18.5 0.02 (4)	10.00				
AF-AE	0 23	-18.5 -18.5 0.02 (4)	10.00				
AE-AD	0 19	-18.5 -18.5 0.02 (4)	10.00				
AD-AC	0 16	-18.5 -18.5 0.02 (4)	10.00				
AC-AB	0 14	-18.5 -18.5 0.02 (4)	10.00				
AB-AA	0 12	-18.5 -18.5 0.01 (4)	10.00				
AA-Z	0 12	-18.5 -18.5 0.02 (4)	10.00				
Z-Y	0 12	-18.5 -18.5 0.02 (4)	10.00				
Y-X	0 12	-18.5 -18.5 0.02 (4)	10.00				
X-W	0 12	-18.5 -18.5 0.02 (4)	10.00				
W-V	0 14	-18.5 -18.5 0.02 (4)	10.00				
V-U	0 16	-18.5 -18.5 0.02 (4)	10.00				
U-T	0 19	-18.5 -18.5 0.02 (4)	10.00				
T-S	0 23	-18.5 -18.5 0.02 (4)	10.00				
S-R	0 0	-18.5 -18.5 0.02 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. GIC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (P-Q:1), BC=0.02/1.00 (S-T:4), WB=0.27/1.00 (L-V:1), SSI=0.09/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLF) (PLF)

MAX MIN MAX MIN MAX MIN

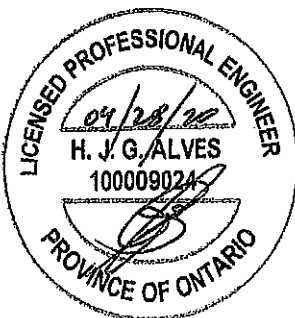
MT20 818 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.62 (K) INPUT=0.90

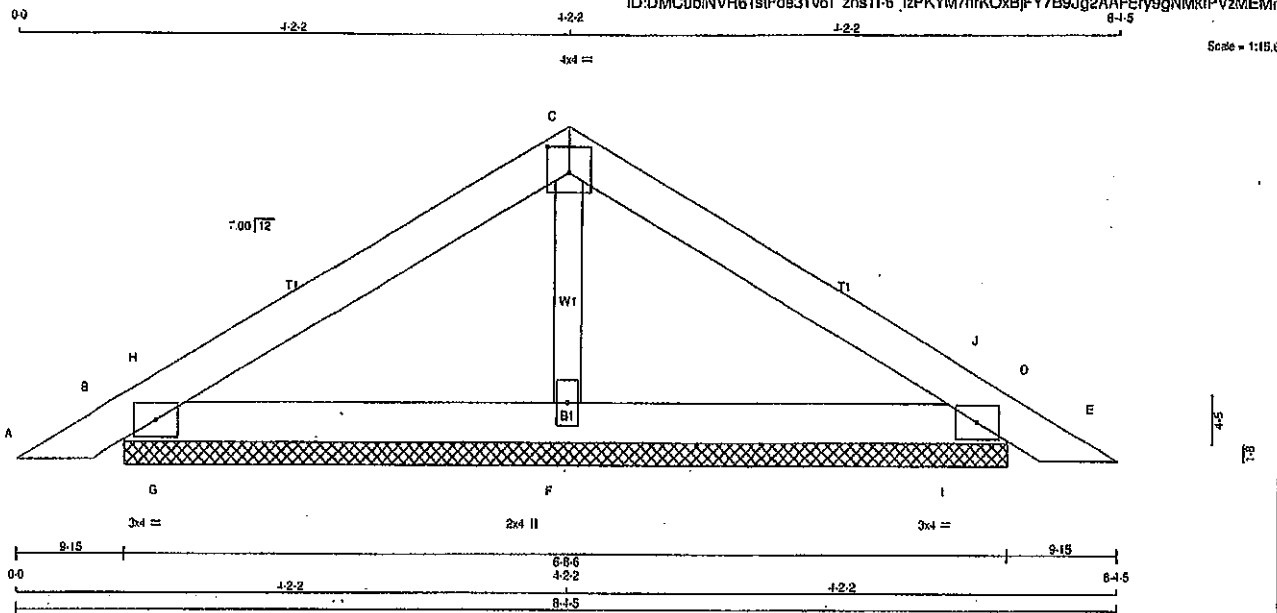
JSI METAL=0.11 (L) INPUT=1.00



Structural component only
DWG# T-2007622

JOB NAME 408223	TRUSS NAME PB20	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 20 2019 Mytek Industries, Inc. Tue Apr 28 10:03:56 2020 Page 1
ID:DMCubINVR6TstFos31v6l zns11-6 jzPKYM7hrKOxBjFY7B9Jg2AAFEry9gNMkPVzMEMr



TOTAL WEIGHT = 4 X 20 = 80 lb

LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-p	MT20	4.0	4.0	2.25 2.00
D	TMB1-I	MT20	3.0	4.0	
F	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
B	281	0	0	6-8-6
D	281	0	0	6-8-6
F	314	0	0	6-8-6

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	197	141	0	0	0	55	0
D	197	141	0	0	0	55	0
F	224	137	0	0	0	87	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED		
	FORCE	VERT. LOAD	MAX. LC1		FORCE	MAX. LC1	
FR-TO	(LBS)	(PLF)	CS1 (LC)	FR-TO	(LBS)	CS1 (LC)	
A-B	0 16	-91.8	-91.8 0.04 (1)	10.00	F-G	-159 0	0.02 (1)
B-H	-44 0	-91.8	-91.8 0.05 (1)	6.25	G-H	-250 0	0.00 (1)
H-C	-89 0	-91.8	-91.8 0.12 (1)	6.25	I-J	-250 0	0.00 (1)
C-J	-99 0	-91.8	-91.8 0.12 (1)	6.25			
J-D	-44 0	-91.8	-91.8 0.05 (1)	6.25			
D-E	0 16	-91.8	-91.8 0.04 (1)	10.00			
B-G	0 81	-18.5	-18.5 0.12 (1)	10.00			
G-F	0 81	-18.5	-18.5 0.12 (1)	10.00			
F-I	0 81	-18.5	-18.5 0.12 (1)	10.00			
I-D	0 81	-18.5	-18.5 0.12 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.12/1.00 (G-H:1), BC=0.12/1.00 (B-G:1), WB=0.02/1.00 (C-F:1), SS=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP/DRY	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	618	354
	1667	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

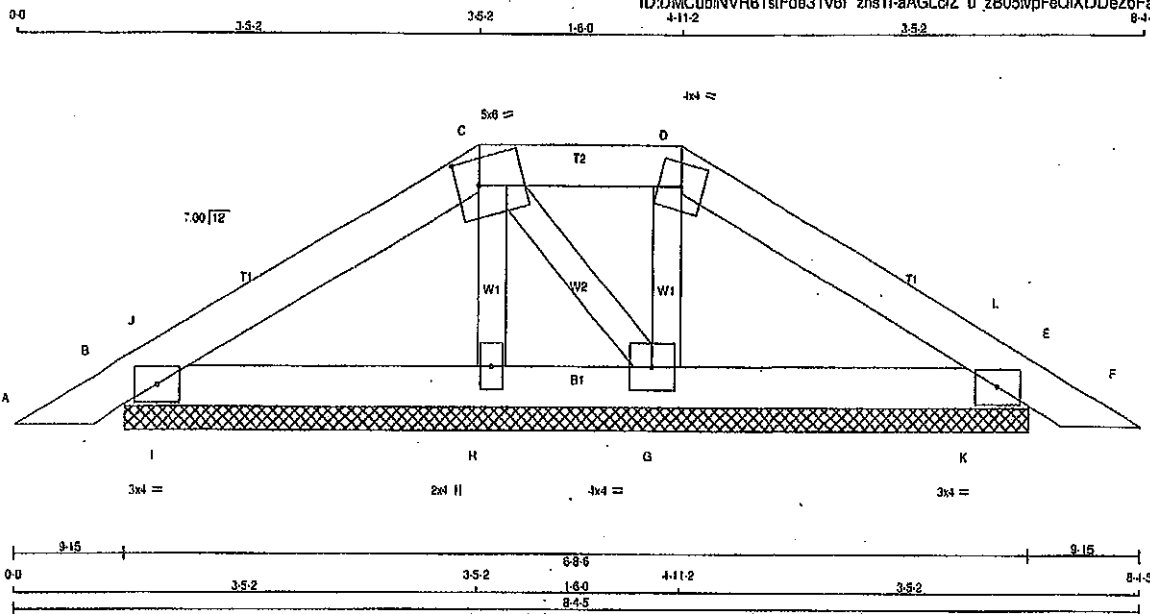
JSI GRIP = 0.24 (D) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007627

JOB NAME 408223	TRUSS NAME PB21	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 5 Oct 2019 Mitek Industries, Inc. Tue Apr 28 10:03:57 2020 Page 1
ID:DMCubINVR6TsiFoe31v6I znsII-aAGLdZ u zB05IvpFeQIXDDeZbFaPRpb0UDxzMEMm



Scale = 1:15.2

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	BRG	BRG	IN-SX	IN-SX
B	241	0	241	0	0	6-8-6	6-8-6	6-8-6	6-8-6
E	228	0	228	0	0	6-8-6	6-8-6	6-8-6	6-8-6
H	170	0	170	0	0	6-8-6	6-8-6	6-8-6	6-8-6
G	238	0	238	0	0	6-8-6	6-8-6	6-8-6	6-8-6

UNFACTORED REACTIONS

JT	1ST LOSE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	169	121.0	0.0	0.0	0.0	48.0	0.0
E	160	114.0	0.0	0.0	0.0	46.0	0.0
H	122	74.0	0.0	0.0	0.0	40.0	0.0
G	168	111.0	0.0	0.0	0.0	57.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
A-B	0 18	H-C	-107 0
B-J	-59 0	C-G	-36 0
J-C	-71 0	G-D	-147 0
C-D	-28 0	I-J	-139 0
D-L	-46 0	K-L	-141 0
L-E	-33 0		
E-F	0 16		
B-I	0 59		
I-H	0 59		
H-G	0 52		
G-K	0 38		
K-E	0 38		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. OC.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.0/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012: ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07(1.00 (D-L:1) , BC=0.07(1.00 (B-L:1) , WB=0.02(1.00 (D-G:1) , SSI=0.11(1.00 (E-K:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 768 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.19 (B) INPUT = 0.90
JSI METAL = 0.05 (D) INPUT = 1.00

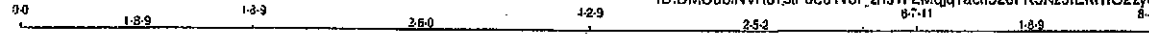


Structural component only
DWG# T-2007628

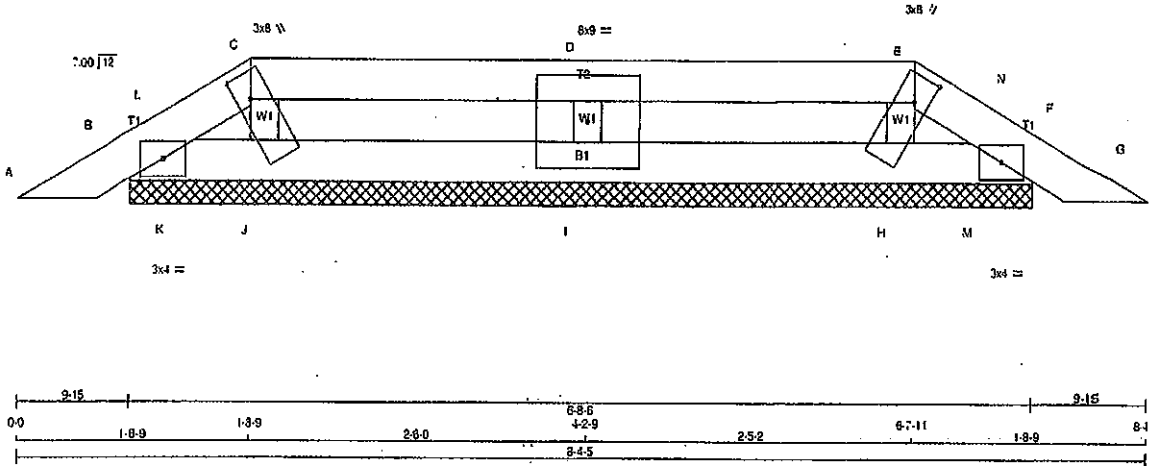
JOB NAME 408223	TRUSS NAME PB22	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
--------------------	--------------------	---------------	----------	-------------------------------	----------

Tamarack Roof Truss, Burlington

Version 9.310 S Oct 29 2019 MiTek Industries, Inc. Tue Apr 28 10:03:58 2020 Page 1
ID:DMCubINVR6TslFoe3fv6l_zns11-2Mqjq7actl52eFK5Nz9IEkmO2zyGJsPzqqDmTNzMEMI



Scale = 1:15.2



TOTAL WEIGHT = 18 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - E	2x4	DRY	No.2	SPF		
E - G	2x4	DRY	No.2	SPF		
G - F	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

PLATES (table(s) in notes)	JT TYPE	PLATES	W	LEN	Y	X
B - TMB1-1	MT20	3.0	4.0			
C, D, E, H, I, J						
C						
C	TTBW1-h	MT20	3.0	8.0	2.25	1.25
D	TMBW1-1	MT20	8.0	9.0		
E	TTBW1-h	MT20	3.0	8.0	2.25	1.25
F	TMB1-1	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	151	0	151	0	6-8-6	6-8-6
F	152	0	152	0	6-8-6	6-8-6
J	125	0	125	0	6-8-6	6-8-6
H	120	0	120	0	6-8-6	6-8-6
I	330	0	330	0	6-8-6	6-8-6

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	105	83.0	0.0	0.0	0.0	21.0	0.0
F	105	83.0	0.0	0.0	0.0	22.0	0.0
J	90	49.0	0.0	0.0	0.0	41.0	0.0
H	86	47.0	0.0	0.0	0.0	39.0	0.0
I	232	158.0	0.0	0.0	0.0	74.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, H, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLP)	MAX. FACTORED CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 16	-91.8	-91.8 0.04 (1)	10.00	J-C	-95 0	0.01 (1)		
B-L	-53.0	-91.8	-91.8 0.04 (1)	6.25	H-E	-92 0	0.01 (1)		
L-C	-22 0	-91.8	-91.8 0.01 (1)	6.25	I-D	-282 0	0.04 (1)		
C-D	0 3	-91.8	-91.8 0.09 (1)	10.00	K-L	-1 12	0.00 (1)		
D-E	0 3	-91.8	-91.8 0.09 (1)	10.00	M-N	0 13	0.00 (1)		
E-N	-21 0	-91.8	-91.8 0.01 (1)	6.25					
N-F	-52 0	-91.8	-91.8 0.04 (1)	6.25					
F-G	0 16	-91.8	-91.8 0.04 (1)	10.00					
B-K	0 14	-18.5	-18.5 0.01 (1)	10.00					
K-J	0 14	-18.5	-18.5 0.02 (1)	10.00					
J-I	-3 0	-18.5	-18.5 0.02 (1)	10.00					
I-H	-3 0	-18.5	-18.5 0.02 (1)	10.00					
H-M	0 14	-18.5	-18.5 0.01 (1)	10.00					
M-F	0 14	-18.5	-18.5 0.01 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 008-09, CSA 008-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (D-E:1.1, BC=0.02/1.00 (I-J:4), WB=0.04/1.00 (D-E:1), SS=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

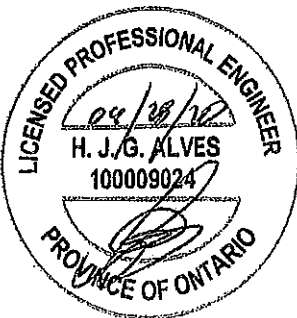
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLP) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.13 (C) INPUT = 0.30
JSI METAL = 0.04 (C) INPUT = 1.00



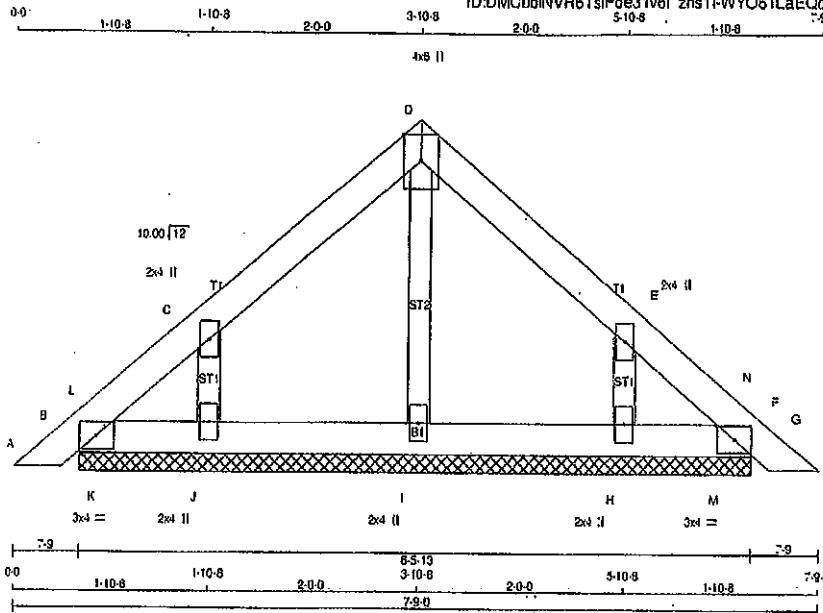
Structural component only
DWG# T-2007629

JOB NAME 408223	TRUSS NAME PG20	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
--------------------	--------------------	---------------	----------	-------------------------------	----------

Paradeck Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:03:59 2020 Page 1

ID:DMCubINVR6TslFoe31v6l zns11-WYQ61LaEQcDvFPVhXggunylaUNHa2Js63KzJ0pzMEMk



Scale = 1:19.7

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
B - G	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TMB1-w	MT20	2.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMB1-w	MT20	2.0	4.0		
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H, I, J						
H	BMW1-w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	UNBRACED LENGTH (FT)
FR-TO				FR-TO			
A-B	0 14	-91.8	-91.8 0.02 (1)	I-D	-111 0	0.02 (1)	10.00
B-L	-71 0	-91.8	-91.8 0.00 (1)	J-C	-185 0	0.03 (1)	6.25
L-C	-30 0	-91.8	-91.8 0.05 (1)	H-E	-185 0	0.03 (1)	6.25
C-D	-47 0	-91.8	-91.8 0.05 (1)	K-L	0 9	0.00 (1)	6.25
D-E	-47 0	-91.8	-91.8 0.05 (1)	M-N	0 9	0.00 (1)	6.25
E-N	-30 0	-91.8	-91.8 0.05 (1)				
N-F	-71 0	-91.8	-91.8 0.00 (1)				
F-G	0 14	-91.8	-91.8 0.02 (1)				
B-K	0 44	-18.5	-18.5 0.01 (1)				
K-J	0 44	-18.5	-18.5 0.01 (4)				
J-I	0 27	-18.5	-18.5 0.02 (4)				
I-H	0 27	-18.5	-18.5 0.02 (4)				
H-M	0 44	-18.5	-18.5 0.01 (4)				
M-F	0 44	-18.5	-18.5 0.01 (1)				

TOTAL WEIGHT = 2 X 22 = 45 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 080-09, CSA 080-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (C-D:1), BC=0.02/1.00 (H-K:1), WB=0.03/1.00 (C-J:1), SS=0.07/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1867 789 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

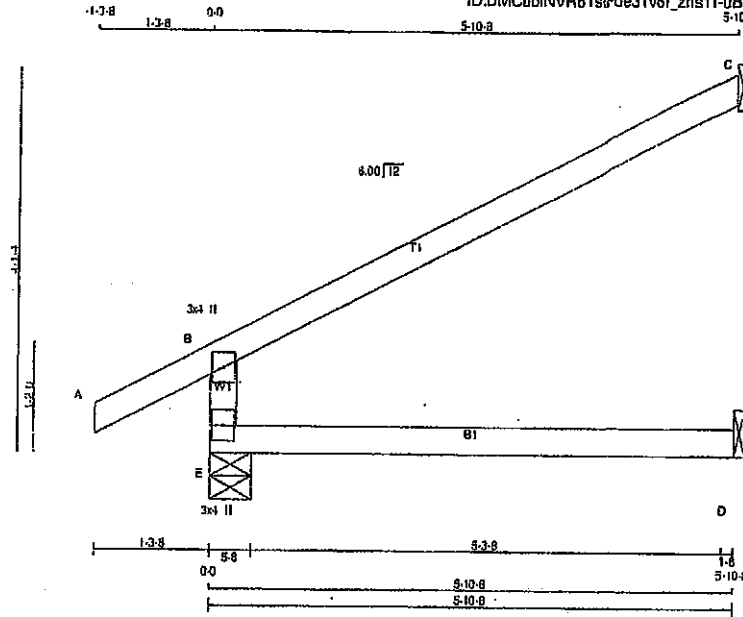
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.14 (E) INPUT = 0.90
JSI METAL = 0.10 (C) INPUT = 1.00



Structural component only
DWG# T-2007630

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	J1	21	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



Scale = 1:22.5

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	No.2	SPF		
A - C	2x4 DRY	No.2	SPF		
E - D	2x4 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (tablets in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0			
E BMV+p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	DOWN	HORIZ	UPLIFT	BRG
JT	VERT	HORIZ	UPLIFT	IN-SX
E	525	0	525	5-8
C	202	0	202	1-8
D	45	0	50	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS	IST CASE	MAX. MIN. COMPONENT REACTIONS	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE
E	369	257	0	0
C	139	113	0	0
D	35	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
FR-TO		FROM	TO		FR-TO		
E-B	-461 0	0.0	0.0	0.13 (4)			7.81
A-B	0 28	-91.8	-91.8	0.12 (1)		10.00	
B-C	-30 0	-91.8	-91.8	0.54 (1)		6.25	
E-D	0 0	-18.5	-18.5	0.13 (4)		10.00	

TOTAL WEIGHT = 21 X 17 = 353 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 6.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/D

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012, NBC 2015
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF.

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CSK TC=0.54 (1.00) (B-C:1), BC=0.13 (1.00) (D-E:4), WB=0.00 (1.00) (r/a:0), SSI=0.24 (1.00) (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

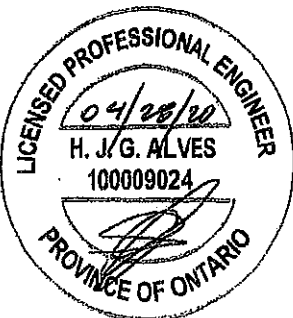
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.19 (E) (INPUT = 0.90)
JSI METAL = 0.13 (B) (INPUT = 1.00)

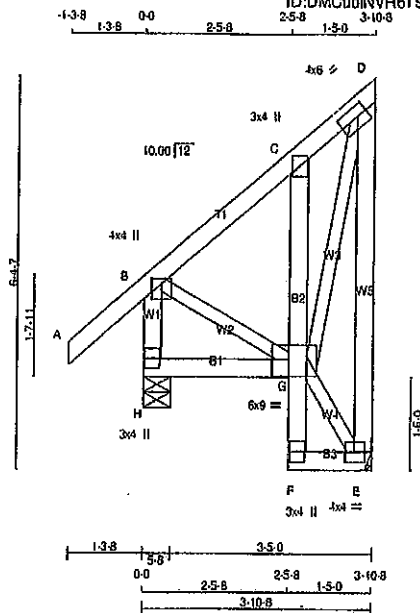


Structural component only
DWG# T-2007598

JOB NAME 408223	TRUSS NAME J20S	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
--------------------	--------------------	---------------	----------	-------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:03:51 2020 Page 1
ID:DMCubINVR6TstFoe31v6I_zns1HJv4McUDJ8C2IAIIT_X?SGzA59ZPAq_xD41ujHzMEMS



Scale = 1/32"

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
H - B		2x4	DRY	No. 2	SPF
A - D		2x4	DRY	No. 2	SPF
H - G		2x4	DRY	No. 2	SPF
F - C		2x4	DRY	No. 2	SPF
F - E		2x4	DRY	No. 2	SPF
ALL WEBS		2x3	DRY	No. 2	SPF
EXCEPT					
E - D		2x4	DRY	No. 2	SPF
DRY; SEASONED LUMBER.					

DRY: SEASONED LUMBER.

PLATES (tablets in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMV+p	MT20	3.0	4.0		
D	TMVW-i	MT20	4.0	6.0	2.00	2.00
E	BMVW1-i	MT20	4.0	4.0		
F	BMV+p	MT20	3.0	4.0		
G	BMVW1-i	MT20	6.0	9.0	3.25	3.50
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	UPLIFT
H	333	0	333	0
E	206	0	206	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM. LIVE
H	233	186	0	0
E	145	95	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED MAX. CS1 (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO						FR-TO			
H-B	-309	0	0.0	0.0	0.03 (1)	7.81	8-G	0	70
A-B	0	41	-91.8	-91.8	0.14 (5)	10.00	E-D	-185	0
B-C	-70	0	-91.8	-91.8	0.07 (1)	6.25	G-E	-13	0
C-D	-92	0	-91.8	-91.8	0.06 (1)	6.25	G-D	0	215
H-G	0	0	-18.5	-18.5	0.04 (4)	10.00			
F-G	0	12	0.0	0.0	0.02 (1)	10.00			
G-G	-215	0	0.0	0.0	0.02 (1)	7.81			
F-E	0	8	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 33 = 66 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19')
CALCULATED VERT. DEFL. (LL) = L/999 (0.00')
ALLOWABLE DEFL. (TL) = L/360 (0.19')
CALCULATED VERT. DEFL. (TL) = L/999 (0.00')

CSI: TC=0.14/1.00 (A-B-5), BC=0.04/1.00 (G-H-4), WB=0.08/1.00 (D-E-1), SS=0.09/1.00 (A-B-5)

DDL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	788	1667	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.24 (B) (INPUT = 0.90)
JSI METAL = 0.08 (C) (INPUT = 1.00)

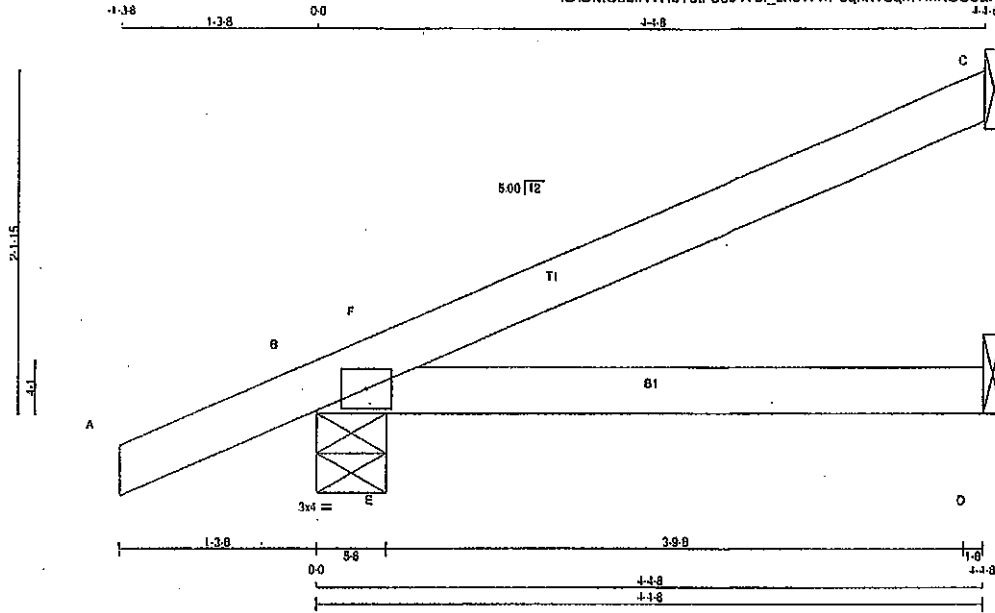


Structural component only
DWG# T-2007623

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DWG NO.
408223	J21	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:03:53 2020 Page 1
ID:DMCubINVR6TslFoe31v6I_zns11-hP0qnIWUqmTmXUS8aPZUYh3VGyCuebnDgOW?oAzMEMQ



Scale = 1:12.4

LUMBER					
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER	DESCR.	SPF	
A - C	2x4	DRY	No.2		
B - D	2x4	DRY	No.2		
DRY: SEASONED LUMBER.					

PLATES (tablets in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	1.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
C	175	0	175	0	0	1-8	1-8	
B	385	0	385	0	0	5-8	5-8	
D	67	0	67	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
	SNOW													
C	121	94	0	0/0	0/0	0/0	0/0	0/0	0/0	0/0	27	0	0	0
B	256	181	0	0/0	0/0	0/0	0/0	0/0	0/0	0/0	75	0	0	0
D	50	18	0	0/0	0/0	0/0	0/0	0/0	0/0	0/0	32	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		W E B S		MAX. FACTORED	
	MAX. FACTORED	FORCE	VERT. LOAD	LC1	MAX. FACTORED	FORCE	MAX. FACTORED	FORCE
FR-TO		(LBS)	(PLF)	CS1 (LC)	UNBRAC	FR-TO		(LBS)
A-B	0	23	-91.8	-91.8	0.12 (1)	10.00	E-F	-235
B-F	-17	13	-91.8	-91.8	0.04 (4)	6.25		
F-C	-1	2	-91.8	-91.8	0.23 (1)	10.00		
G-E	0	0	-18.5	-18.5	0.16 (1)	10.00		
E-D	0	0	-18.5	-18.5	0.16 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.18")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.18")
CALCULATED VERT. DEFL. (TL) = L/999 (0.05")

DSI: TO=0.23/1.00 (C-F:1), BC=0.16/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.19/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.26 (B) (INPUT = 0.90)
JSI METAL = 0.67 (B) (INPUT = 1.00)

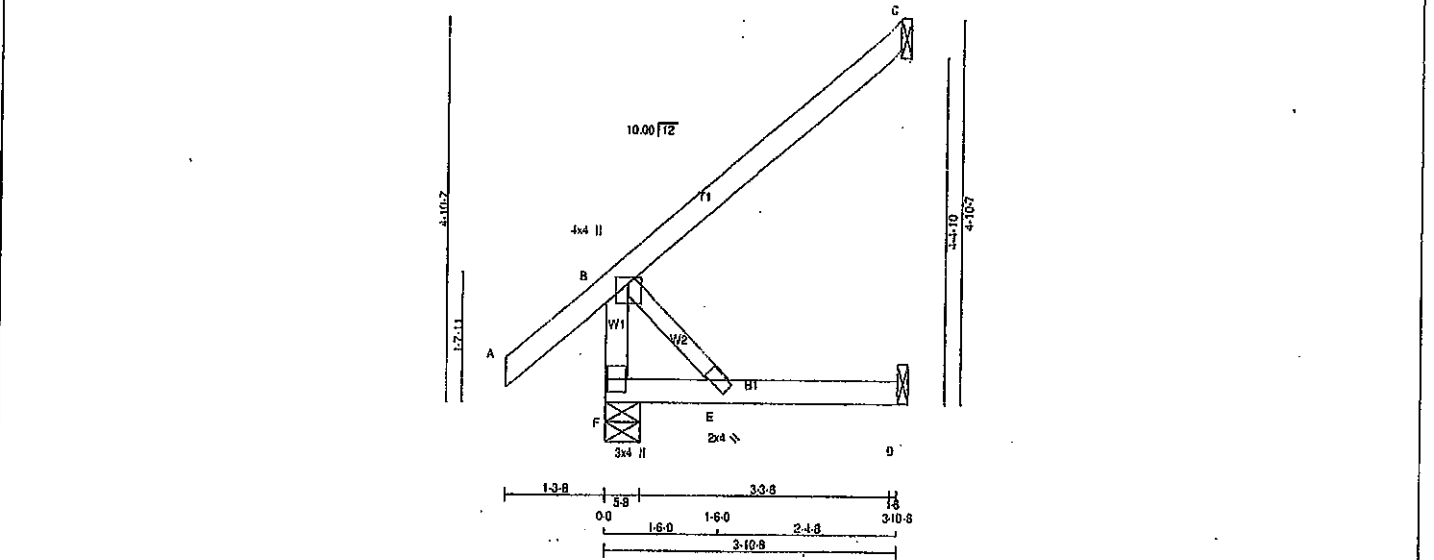


Structural component only
DWG# T-2007624

JOB NAME 408223	TRUSS NAME J22	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:03:54 2020 Page 1
ID:DMCubINVR6TstFoe31v6l zns11-AbaD eX6b3bd8e1K874j4ubgIMZPN21Nv2FYKczMEMp

Scale = 1:27.0



LUMBER				DESIGN CRITERIA			
N. L. G. A. RULES				SPECIFIED LOADS:			
CHORDS	SIZE	DRY	LUMBER	TOP CH.	LL	25.6	PSF
F - B	2x4	DRY	No.2		DL	6.0	PSF
A - C	2x4	DRY	No.2	BOT CH.	LL	0.0	PSF
F - D	2x4	DRY	No.2		DL	7.4	PSF
ALL WEBS	2x3	DRY	No.2	TOTAL LOAD = 39.0 PSF			
DRY: SEASONED LUMBER.				SPACING = 24.0 IN. O.C.			

PLATES (table in inches)				UNFACTORED REACTIONS			
JT	TYPE	PLATES	W. LEN Y X	1ST LOASE	MAX. MIN. COMPONENT REACTIONS		
B	TMVW+p	MT20	4.0 4.0 1.00 2.00	SNOW	LIVE	PERM.LIVE	WIND
E	BMV+w	MT20	2.0 4.0				DEAD
F	BMV1+p	MT20	3.0 4.0				SOIL

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RISID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING				CHORDS			
TOTAL LOAD CASES: (5)				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
				FR-TO		FROM TO	
				F-B	-305 0	0.0 0.0 0.03 (1)	7.81 B-E
				A-B	0 41	-91.8 -91.8 0.14 (5)	10.00
				B-C	0 0	-91.8 -91.8 0.23 (1)	10.00
				F-E	0 0	-18.5 -18.5 0.08 (4)	10.00
				E-D	0 0	-18.5 -18.5 0.08 (4)	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015
THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014
55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD
ALLOWABLE DEFL.(LL) = L/360 (0.19')
CALCULATED VERT. DEFL.(LL) = L/999 (0.00')
ALLOWABLE DEFL.(TL) = L/360 (0.19')
CALCULATED VERT. DEFL.(TL) = L/999 (0.01')
CSI: TC=0.23/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SS=0.11/1.00 (B-C:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00
AUTOSOLVE RIGHT HEEL ONLY
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP = 0.24 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007625

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

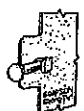
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



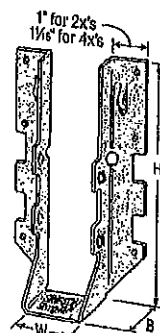
Double-Shear
Nailing
Top View



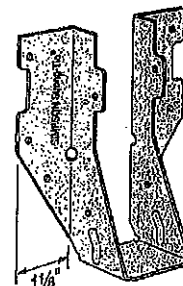
Double-Shear
Nailing
Side View;
Do not
bend tab



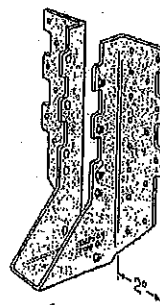
Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580



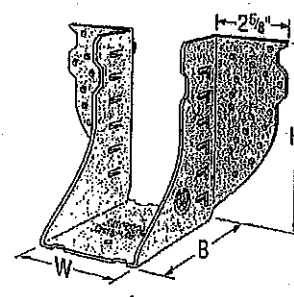
LUS28



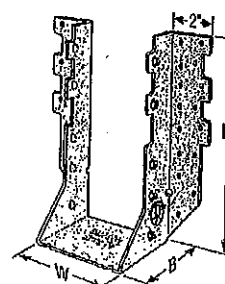
LU26L



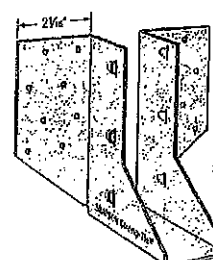
HUS210
(HUS26, HUS28,
and HHUS similar)



HGUS28-2

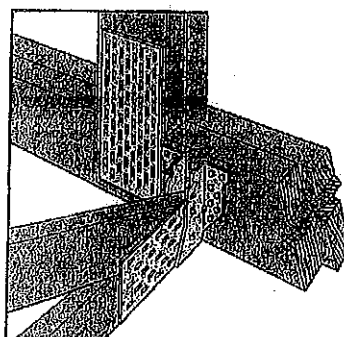


HHUS210-2



LJS26DS

Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS**HHUS/HGUS**

See Hanger Options Information on pp. 125–127.

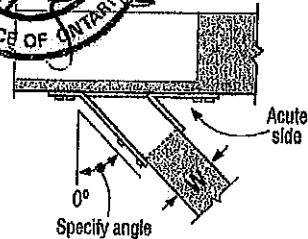
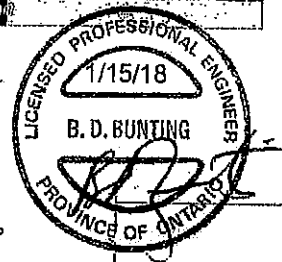
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(Joist must be bevel cut)
All joist nails installed on the outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _g ³	Header	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15) lb.	(K _D = 1.00) lb.	(K _D = 1.15) lb.	(K _D = 1.00) lb.
								kN	kN	kN	kN
Single 2x Sizes											
LUS24	18	1 1/8	3 1/4	1 3/4	2 1/4	(4) 10d	(2) 10d	710	1626	845	1155
LU24L	22	1 1/8	3	1 1/4	2 1/8	(4) 10d	(2) 10d x 1 1/2"	360	723	287	514
LU26L	22	1 1/8	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"	160	454	320	726
								720	1605	645	1140
LUS26	18	1 1/8	4 1/4	1 3/4	3 1/4	(4) 10d	(4) 10d	320	714	287	507
HUS26	16	1 1/8	5 1/4	3	3 1/8	(14) 16d	(6) 16d	1420	2170	1290	1630
								832	965	574	725
LJS26DS	18	1 1/8	5	3 1/2	4 1/4	(16) 16d	(6) 16d	2705	4940	2065	3875
								1130	2197	920	1724
HGUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 16d	(8) 16d	2055	4265	1480	4115
								814	1897	648	1831
LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"	2685	6625	2685	5700
								1196	2851	1196	2535
LUS28	18	1 1/8	6 1/4	1 3/4	3 1/4	(6) 10d	(4) 10d	1140	2185	1020	1550
								507	972	454	689
HUS28	16	1 1/8	7 1/4	3	6 1/8	(22) 16d	(8) 16d	1420	2520	1290	1790
								632	1121	574	796
HGUS28	12	1 1/8	7 1/4	5	6 1/4	(36) 16d	(12) 16d	3605	5365	2675	4345
								1634	2386	1190	1933
LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(8) 10d x 1 1/2"	3310	7675	3310	6900
								1474	3419	1474	3073
LUS210	18	1 1/8	7 1/4	1 3/4	3 1/4	(8) 10d	(4) 10d	1140	2495	1020	1770
								507	1110	454	787
								1420	2785	1290	2210
								632	1239	574	983

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_g is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d ₀ ³	Header	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _p = 1.15) lb.	(K _p = 1.00) lb.	(K _p = 1.15) lb.	(K _p = 1.00) lb.
								kn	kn	kn	kn
Double 2x Sizes											
LUS24-2	18	3 3/4	3 3/4	2	1 1/2	(4) 16d	(2) 16d	835	2020	590	1435
LUS26-2	18	3 3/4	4 3/4	2	4	(4) 16d	(4) 16d	3.71	8.89	2.62	6.38
HHUS26-2	14	3 3/4	5 3/4	3	3 3/4	(14) 16d	(6) 16d	1720	2595	1545	1920
HGUS26-2	12	3 3/4	5 3/4	4	4 1/4	(20) 16d	(8) 16d	7.85	11.54	6.87	8.64
LUS28-2	18	3 3/4	7	2	4	(6) 16d	(4) 16d	2850	7335	2065	5205
HHUS28-2	14	3 3/4	7 1/4	3	6 1/4	(22) 16d	(8) 16d	12.68	32.63	9.20	23.15
HGUS28-2	12	3 3/4	7 1/4	4	6 1/4	(36) 16d	(12) 16d	4385	8950	3110	6355
LUS210-2	18	3 3/4	9	2	6	(8) 16d	(6) 16d	19.51	39.81	13.83	28.27
HHUS210-2	14	3 3/4	9 1/4	3	8	(30) 16d	(10) 16d	1720	3325	1545	2575
HGUS210-2	12	3 3/4	9 1/4	4	8 1/4	(46) 16d	(16) 16d	7.85	14.79	6.87	11.45
								16.75	39.77	11.90	28.22
								6070	12980	4310	9215
								27.00	57.74	19.17	40.99
								2580	4500	2320	3195
								11.48	20.02	10.32	14.21
								6840	14015	4855	10270
								30.43	62.34	21.60	45.69
Triple 2x Sizes											
HGUS26-3	12	4 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3110	6355
HGUS28-3	12	4 1/4	7 1/4	4	6 1/4	(36) 16d	(12) 16d	19.51	39.81	13.83	28.27
HHUS210-3	14	4 1/4	9	3	7 1/4	(30) 16d	(10) 16d	6070	12980	4310	9215
HGUS210-3	12	4 1/4	9 1/4	4	8 1/4	(46) 16d	(16) 16d	27.00	57.74	19.17	40.99
								4670	9670	4235	6865
								20.77	49.02	18.84	30.64
								6840	14645	4855	10400
								30.43	65.14	21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	6 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3110	6355
HGUS28-4	12	6 1/4	7 1/4	4	6 1/4	(36) 16d	(12) 16d	19.51	39.81	13.83	28.27
HHUS210-4	14	6 1/4	8 1/4	3	7 1/4	(30) 16d	(10) 16d	6070	12980	4310	9215
HGUS210-4	12	6 1/4	9 1/4	4	8 1/4	(46) 16d	(16) 16d	27.00	57.74	19.17	40.99
HGUS212-4	12	6 1/4	10 1/4	4	10 1/4	(56) 16d	(20) 16d	4670	10155	4235	7210
HGUS214-4	12	6 1/4	12 1/4	4	11 1/4	(66) 16d	(22) 16d	20.77	45.17	18.84	32.07
								6840	14645	4855	10400
								30.43	65.14	21.60	46.26
								7640	14995	5425	10645
								33.98	66.70	24.13	47.35
								10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
LUS46	18	3 3/4	4 3/4	2	3 3/4	(4) 16d	(4) 16d	1720	2595	1545	1920
HHUS46	14	3 3/4	5 3/4	3	3 3/4	(14) 16d	(6) 16d	7.85	11.54	6.87	8.64
HGUS46	12	3 3/4	5 3/4	4	4 1/4	(20) 16d	(8) 16d	2540	7335	2065	5205
LUS48	18	3 3/4	6 3/4	2	3 3/4	(6) 16d	(4) 16d	11.30	32.63	9.20	23.15
HHUS48	14	3 3/4	7 1/4	3	6 1/4	(22) 16d	(8) 16d	4385	8950	3110	6355
HGUS48	12	3 3/4	7 1/4	4	6 1/4	(36) 16d	(12) 16d	19.51	39.81	13.83	28.27
LUS410	18	3 3/4	8 3/4	2	5 3/4	(8) 16d	(6) 16d	1720	3325	1545	2575
HGUS410	12	3 3/4	9	4	8 1/4	(46) 16d	(16) 16d	7.85	14.79	6.87	11.45
HGUS412	12	3 3/4	10 3/4	4	10 1/4	(56) 16d	(20) 16d	3765	8940	2675	6345
HGUS414	12	3 3/4	12 3/4	4	11 1/4	(66) 16d	(22) 16d	16.75	39.77	11.90	28.22
								6070	12980	4310	9215
								27.00	57.74	19.17	40.99
								2580	4500	2320	3195
								11.48	20.02	10.32	14.21
								6840	14015	4855	10270
								30.43	62.34	21.60	45.60
								7640	14995	5425	10645
								33.98	66.70	24.13	47.35
								10130	16400	7195	11645
								45.06	72.95	32.00	51.80



Plated Truss Connectors

See footnotes
on p. 258.

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1/4". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

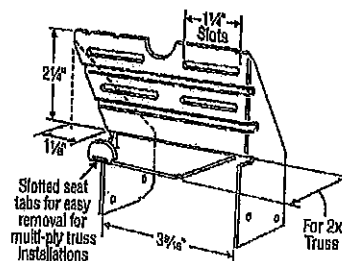
Design: Factored resistances are in accordance with CSA 086-14

Installation:

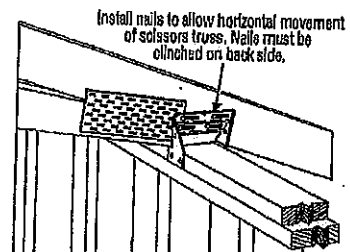
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

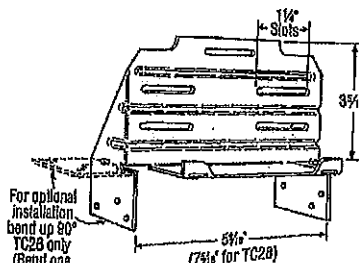
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



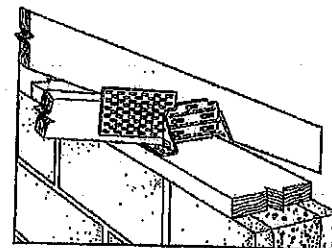
TC24
U.S. Patent 4,932,173



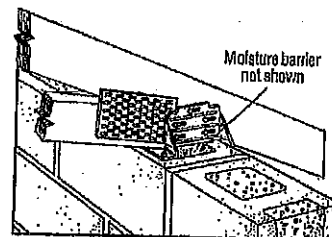
Typical TC24 Installation



TC26
(TC28 Similar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift ($K_c=1.15$) lb.	Uplift ($K_c=1.15$) lb.
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(8) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift ($K_c=1.15$) lb.	Uplift ($K_c=1.15$) lb.
TC26	(5) 10d	(8) 10d x 1 1/4"	810	680
	(5) 10d	(6) 10d	930	680

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.

2. Grout strength is 15 MPa minimum.

3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.

4. TC26 fastened to grouted concrete block with (6) - 9/16" x 2 1/4" Titen screws has a factored uplift resistance of 276 lb.



INTERNATIONAL
STATES
DESIGN

© 2017 Simpson Strong-Tie Company
All rights reserved. Simpson Strong-Tie Company is a registered trademark of Simpson Strong-Tie Company.

SECTION 13.00 EXPANSION

(800) 999-5099
strongtie.com

H/TSP

Seismic and Hurricane Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Fasteners			Factored Resistance (Kp = 1.16)					
					D.Fir-L			S-P-F		
		To Rafters/Truss	To Plates	To Studs	Uplift	Lateral		Uplift	Lateral	
						F1	F2		F1	F2
					lb.	lb.	lb.	lb.	lb.	lb.
					kN	kN	kN	kN	kN	kN
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
					3.29	3.05	1.33	3.02	2.16	0.96
SS H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
					3.69	0.98	0.33	2.62	0.69	0.24
SS H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
					3.58	0.71	0.71	3.36	0.71	0.71
H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
					3.71	0.78	0.93	3.29	0.71	0.93
SS H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
					3.29	0.80	1.18	2.74	0.56	0.85
H6	16	—	(0) 8d	(8) 8d	1585	1085	—	1125	770	—
					7.05	4.83	—	5.00	3.43	—
H7Z	16	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
					6.18	2.98	—	4.40	2.11	—
SS H8 ¹	18	(5) 10d x 1½"	(5) 10d x 1½"	—	1120	—	—	1025	—	—
					4.98	—	—	4.56	—	—
SS H10A ²	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	585	290
					7.72	3.54	1.82	6.69	2.51	1.29
H10AR	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1485	690	430	1220	570	305
					6.61	3.07	1.91	5.43	2.54	1.36
H10A-2	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1835	1275	430	1645	880	305
					8.16	5.67	1.91	7.32	3.91	1.36
H10S ^{3,4}	18	(8) 8d x 1½"	(8) 8d x 1½"	(8) 8d	1465	795	315	1040	585	225
					6.52	3.54	1.40	4.63	2.51	1.00
H11Z	18	(6) 16d x 2½"	(6) 16d x 2½"	—	1095	920	545	780	655	390
					4.87	4.09	2.42	3.47	2.91	1.73
H14	18	[1] (12) 8d x 1½"	(13) 8d	—	2390	855	320	1805	610	230
		[2] (12) 8d x 1½"	(15) 8d	—	10.63	3.80	1.42	8.03	2.71	1.02
					2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
TSP	16	(9) 10d x 1½"	(6) 10d x 1½"	—	1285	440	—	920	310	—
					5.76	1.98	—	4.09	1.38	—
		(9) 10d x 1½"	(6) 10d	—	1560	440	—	1105	310	—
					6.94	1.98	—	4.92	1.38	—

- Factored resistances have been increased 16% for short term loading; no further increase is allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
- H8 factored uplift resistances for stud-to-bottom plate installations are 595 lb. (2.65 kN) for D.Fir-L and 390 lb. (1.74 kN) for S-P-F.
- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-HTIECONPATH).
- Factored resistances in the F1 direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.

- H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.38 kN) D.Fir-L and 1015 lb. (4.51 kN) S-P-F.
- H10S nails to plates are optional for uplift but required for lateral loads.
- H10A may be field-bent up to a slope of 6/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
- The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SSNAILS at strongtie.com.
- D.Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1½" nails into the top plates and (3) 8d x 1½" nails into the lowest three flange holes into the truss bottom chord is 495 lb. (2.20 kN).
- Nails: 16d x 2½" = 0.162" dia. x 2½" long, 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long, 8d x 1½" = 0.131" dia. x 1½" long. See pp. 27-28 for other nail sizes and information.

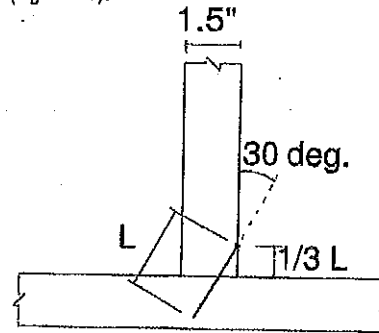
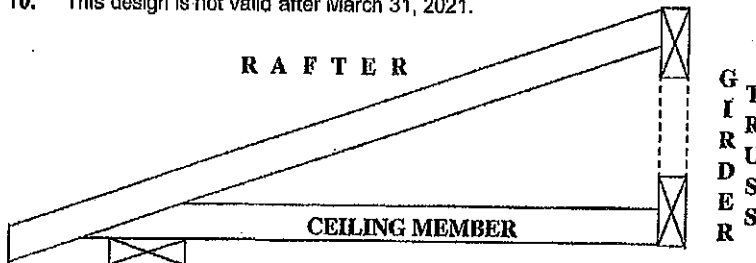
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.



TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (In)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

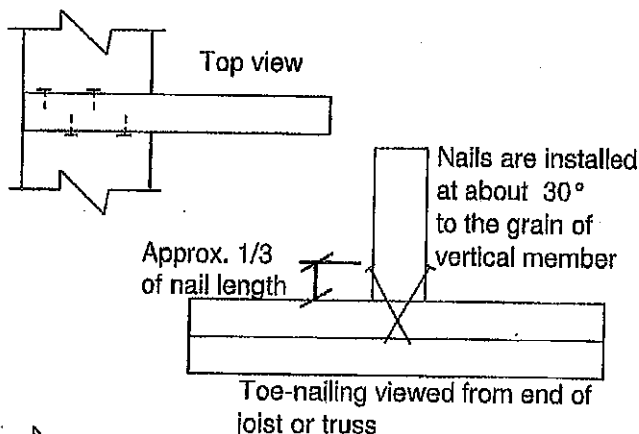
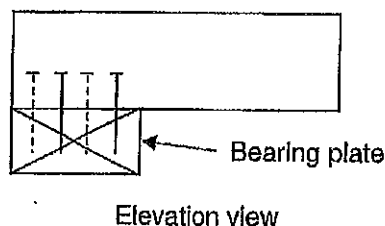
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

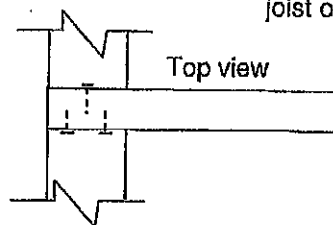
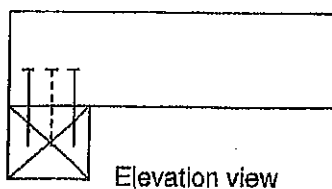
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



MiTek®

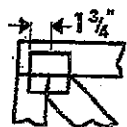
MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7



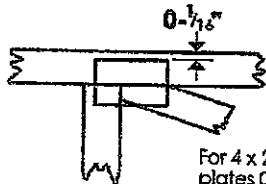
December 2, 2019

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint, unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



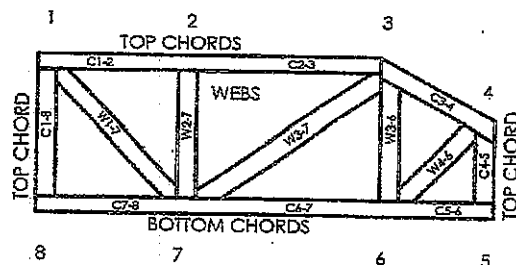
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

© 2007 MiTek® All Rights Reserved

MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MIT-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

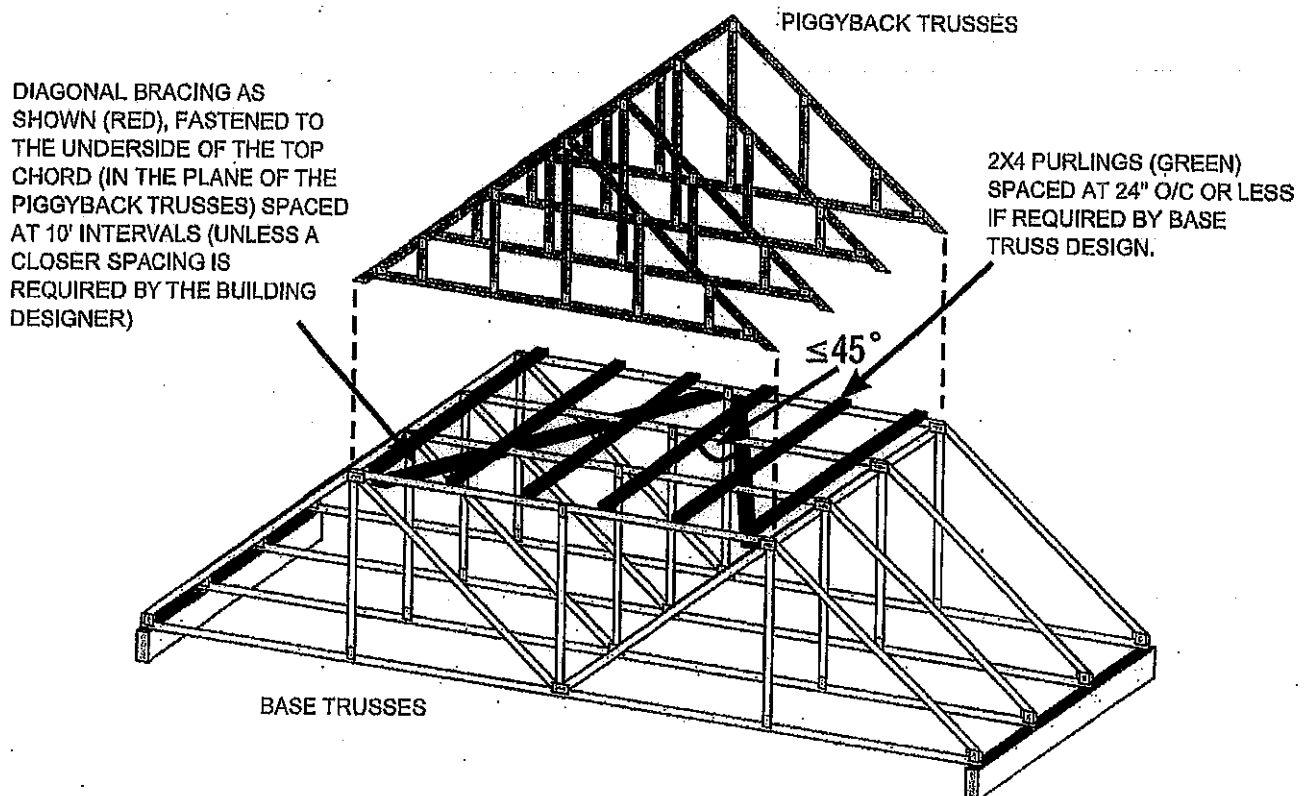
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSL.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane of joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800213

Feb 09, 2018

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 6
 Lot #: 154
 Elevation: 2-STD OR OPT.5 BR

Job Track: 51225
 PlanLog: 202419
 Layout ID: 408223
 Ref #
 Page: 1 of 3
 Date: 04-28-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T11B Common	8 /12	16-01-00	6-09-13	2 x 4		1-06-02 1-04-13	137.47 88.33		
	3	T11S Roof Special	8 /12 8 /12	16-03-00	6-09-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	201.4 133.00		
	1 2-ply	T20 Hip Girder	7 /12	35-02-00	4-01-04	2 x 6	1-03-08 1-03-08	1-03-07 1-03-07	359.46 218.00		
	1	T21 Hip	7 /12	35-02-00	5-01-04	2 x 4	1-03-08 1-03-08	1-03-07 1-03-07	142.49 89.50		
	1	T22 Hip	7 /12	35-02-00	6-01-04	2 x 4	1-03-08 1-03-08	1-03-07 1-03-07	146.47 93.50		
	1	T23 Hip	7 /12	35-02-00	7-01-04	2 x 4	1-03-08 1-03-08	1-03-07 1-03-07	144.95 91.17		
	1	T24 Hip	7 /12	35-02-00	8-01-04	2 x 4	1-03-08 1-03-08	1-03-07 1-03-07	151.12 95.67		
	4	T25 Hip	7 /12	35-02-00	9-01-04	2 x 4	1-03-08 1-03-08	1-03-07 1-03-07	631.9 399.33		
	3	T25A Piggyback Base	7 /12	34-10-00	9-01-04	2 x 4		1-04-09 1-04-09	461.61 286.50		
	1 2-ply	T28 Flat Girder	0 /12	34-10-00	4-10-07	2 x 6		4-10-07 4-10-07	366.38 214.67		
	2	T29 Flat	0 /12	34-10-00	6-06-07	2 x 4		6-06-07 6-06-07	313.97 193.33		
	2	T30 Flat	0 /12	34-10-00	8-02-07	2 x 4		8-02-07 8-02-07	358.85 220.00		
	5	T31 Flat	0 /12	34-10-00	9-10-07	2 x 4		9-10-07 9-10-07	980.54 600.00		
	1	T32 Hip Girder	8 /12	12-02-00	6-04-07	2 x 4 2 x 6		3-00-02 2-10-13	66.45 43.67		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 6
 Lot #: 154
 Elevation: 2-STD OR OPT.5 BR

Job Track: 51225
 PlanLog: 202419
 Layout ID: 408223
 Ref #
 Page: 2 of 3
 Date: 04-28-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T34S Half Hip	10/12	3-10-08	5-05-13	2 x 4	1-03-08	1-07-11 5-05-13	63.75 44.33		
	2	T35S Half Hip	10/12	3-10-08	4-01-13	2 x 4	1-03-08	1-07-11 4-01-13	57.2 41.00		
	3	T36 Common	6/12	8-03-08	3-00-09	2 x 4	1-03-08 1-03-08	5-07 5-07	79.61 53.00		
	1	G36 GABLE	6/12	8-03-08	3-01-05	2 x 4	1-03-08 1-05-00	5-07 5-07	26.93 17.33		
	1	T37 Half Hip Girder	8/12	8-05-00	2-08-03	2 x 4 2 x 6	1-03-08	8-05 2-01-15	36.62 26.00		
	2	G39 GABLE	10/12	27-06-00	9-10-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	303.3 193.33		
	17	P20 Common	10/12	7-08-14	3-02-12	2 x 4		0-01 0-01	363.26 229.50		
	4	PB20 Piggyback	7/12	8-04-05	2-05-04	2 x 4			80.17 49.33		
	1	PB22 Piggyback	7/12	8-04-05	1-00-00	2 x 4			18.26 11.67		
	1	PB21 Piggyback	7/12	8-04-05	2-00-00	2 x 4			21.88 14.33		
	2	PG20 GABLE	10/12	7-09-00	3-02-12	2 x 4			44.53 30.33		
	17	J1 Jack-Open	6/12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	285.51 181.33		
	2	J20S Jack-Open	10/12	3-10-08	6-04-07	2 x 4	1-03-08	1-07-11 6-04-07	65.13 43.33		
	4	J21 Jack-Open	5/12	4-04-08	2-08-02	2 x 4	1-03-08	4-01 2-01-15	48.51 32.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 6
 Lot #: 154
 Elevation: 2-STD OR OPT.5 BR

Job Track: 51225
 PlanLog: 202419
 Layout ID: 408223
 Ref #
 Page: 3 of 3
 Date: 04-28-2020
 Designer:
 Sales Rep: Mario DiCanó

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	3	J22 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	45.68 29.00		

TOTAL # TRUSS= 92

TOTAL BFT OF ALL TRUSSES= 3762.48 BFT.

TOTAL WEIGHT OF ALL TRSSES 6003.41 LBS

HARDWARE

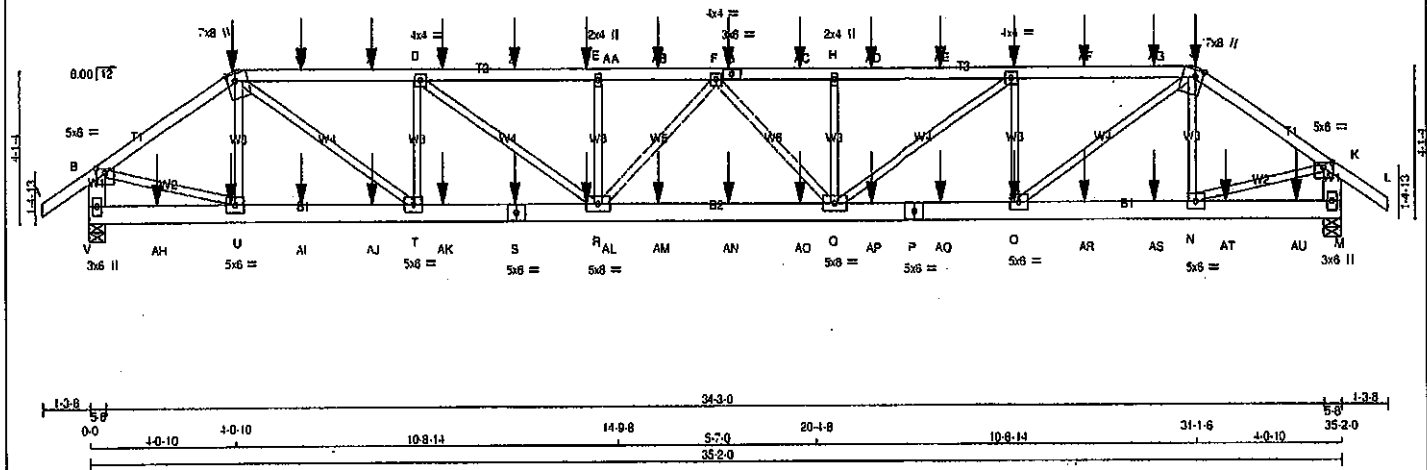
QTY	TYPE	MODEL	LENGTH
3	Hardware	LJS26DS	
4	Hardware	LUS24	
2	Hardware	LUS26-2	

TOTAL NUMBER OF
ITEMS=

9

JOB NAME 408222	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Tue Apr 28 09:00:04 2020 Page 1
 ID:DMCubINVR6TsFoe31v6l zns11-Jm9J9tAhZ122g2EEemNZsEz3HTH?mEdCAUuvczMFif
 1-3-8 0-0 4-0-10 4-0-10 5-3-2 9-3-12 5-5-12 14-9-8 2-9-8 17-7-0 2-9-8 20-4-8 25-10-4 31-1-6 4-0-10 35-2-0 35-5-8
 Scale = 1:57.6



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
V - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
V - S	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE (61.0)
C - G	12	SIDE (61.0)
G - J	12	SIDE (61.0)
J - L	12	SIDE (61.0)
V - B	2	TOP
M - K	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
V - S	2	SIDE (183.1)
S - P	2	SIDE (183.1)
P - M	2	SIDE (183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
		GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT
V	3393	0	3393	0	5-8
M	3342	0	3342	0	5-8

UNFACTORED REACTIONS		1ST LOOSE	MAX. MIN.	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM. LIVE
V	2397	1585	0	0
M	2363	1555	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRAC CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 35	-91.8	-91.8	0.07 (1)	10.00	U-C	-820 0
B-C	-4024 0	-91.8	-91.8	0.22 (1)	4.55	C-T	0 3389
C-W	-8074 0	-91.8	-91.8	0.53 (1)	3.51	T-D	-1848 0
W-X	-8074 0	-91.8	-91.8	0.53 (1)	3.51	O-I	-1889 0
X-D	-8074 0	-91.8	-91.8	0.53 (1)	3.51	O-J	0 3425
D-Y	-7423 0	-91.8	-91.8	0.83 (1)	3.11	N-J	-601 0
Y-Z	-7423 0	-91.8	-91.8	0.83 (1)	3.11	B-U	0 3437
Z-AA	-7423 0	-91.8	-91.8	0.83 (1)	3.11	N-K	0 3371
AA-E	-7423 0	-91.8	-91.8	0.83 (1)	3.11	Q-I	0 1704
E-B	-7423 0	-91.8	-91.8	0.35 (1)	3.38	D-R	0 1677
AB-F	-7423 0	-91.8	-91.8	0.35 (1)	3.38	Q-H	-585 0
F-G	-7410 0	-91.8	-91.8	0.36 (1)	3.38	R-E	-594 0
G-AC	-7410 0	-91.8	-91.8	0.36 (1)	3.38	R-F	-285 0
AC-H	-7410 0	-91.8	-91.8	0.36 (1)	3.38	F-Q	-305 0
H-AD	-7410 0	-91.8	-91.8	0.81 (1)	3.14		
AD-AE	-7410 0	-91.8	-91.8	0.81 (1)	3.14		
AE-I	-7410 0	-91.8	-91.8	0.81 (1)	3.14		
I-AF	-6040 0	-91.8	-91.8	0.51 (1)	3.55		
AF-AG	-6040 0	-91.8	-91.8	0.51 (1)	3.55		
AG-J	-6040 0	-91.8	-91.8	0.51 (1)	3.55		
J-K	-3948 0	-91.8	-91.8	0.22 (1)	4.58		
K-L	0 35	-91.8	-91.8	0.07 (1)	10.00		
L-B	-3355 0	0.0	0.0	0.12 (1)	7.60		
M-K	-3298 0	0.0	0.0	0.12 (1)	7.64		

V-AH	0 0	-18.5	-18.5	0.04 (4)	10.00
AH-U	0 0	-18.5	-18.5	0.04 (4)	10.00
U-AI	0 3325	-18.5	-18.5	0.25 (1)	10.00
AI-AJ	0 3325	-18.5	-18.5	0.25 (1)	10.00
AJ-T	0 3325	-18.5	-18.5	0.25 (1)	10.00
T-AK	0 8074	-18.5	-18.5	0.45 (1)	10.00
AK-S	0 8074	-18.5	-18.5	0.45 (1)	10.00
S-AL	0 8074	-18.5	-18.5	0.45 (1)	10.00
AL-R	0 8074	-18.5	-18.5	0.45 (1)	10.00
R-AM	0 7613	-18.5	-18.5	0.57 (1)	10.00
AM-AN	0 7613	-18.5	-18.5	0.57 (1)	10.00
AN-AO	0 7613	-18.5	-18.5	0.57 (1)	10.00
AO-Q	0 7613	-18.5	-18.5	0.57 (1)	10.00
Q-AP	0 6040	-18.5	-18.5	0.45 (1)	10.00
AP-P	0 6040	-18.5	-18.5	0.45 (1)	10.00
P-AQ	0 6040	-18.5	-18.5	0.45 (1)	10.00
AQ-O	0 6040	-18.5	-18.5	0.45 (1)	10.00
O-AR	0 3281	-18.5	-18.5	0.25 (1)	10.00

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (11.7")
CALCULATED VERT. DEFL. (LL) = L/999 (0.23")
ALLOWABLE DEFL. (TL) = L/360 (11.7")
CALCULATED VERT. DEFL. (TL) = L/970 (0.44")

CS1: TG=0.63/1.00 (D-E-1), BC=0.57/1.00 (Q-R-1), WB=0.43/1.00 (B-U-1), SS1=0.19/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.86 (I) (INPUT = 0.80)
JSI METAL = 0.58 (I) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2007602 1/2

JOB NAME

408222

TRUSS NAME

T1

QUANTITY

1

PLY

2

JOB DESC.

GREEN PARK HOMES

TRUSS DESC.

DRWG NO.

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mittek Industries, Inc. Tue Apr 28 09:00:04 2020 Page 2

ID:DMCubINVR6TsFoe31v6I zns11-Jm9J91Ah2f22o2EEmlNZsEz3HTH7mEdCAUJvczMFH

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWV-p	MT20	5.0	6.0	1.50	3.00
C TTWV+m	MT20	7.0	8.0	Edge	2.50
D, F, I					
D TMWV-l	MT20	4.0	4.0		
E TMWV-w	MT20	2.0	4.0		
G TS-l	MT20	3.0	6.0		
H TMWV-w	MT20	2.0	4.0		
J TTWV+m	MT20	7.0	8.0	Edge	2.50
K TMWV-p	MT20	5.0	6.0	1.50	3.00
M BMVl+p	MT20	3.0	6.0		
N, O, T, U					
N BMWV-l	MT20	5.0	6.0		
P BS-l	MT20	5.0	6.0		
Q BMWVW-l	MT20	5.0	6.0		
R BMWVW-l	MT20	5.0	6.0		
S BS-l	MT20	5.0	6.0		
V BMVl+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLP)	LC1	MAX	CS1 (LC)	UNBRAC	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM	TO			LENGTH FR-TO		
AR-AS	0 3261	-18.5	-18.5	0.25 (1)		10.00		
AS-IN	0 3261	-18.5	-18.5	0.25 (1)		10.00		
N-AT	0 0	-18.5	-18.5	0.04 (4)		10.00		
AT-AU	0 0	-18.5	-18.5	0.04 (4)		10.00		
AU-M	0 0	-18.5	-18.5	0.04 (4)		10.00		

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		
AR-AS	0 3261	-18.5
AS-IN	0 3261	-18.5
N-AT	0 0	-18.5
AT-AU	0 0	-18.5
AU-M	0 0	-18.5

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-10	-49	-55	---	FRONT	VERT	DEAD	---	C1
C	4-0-10	-117	-117	---	FRONT	VERT	TOTAL	---	C1
C	4-0-10	-254	-254	---	FRONT	VERT	SNOW	---	C1
G	17-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
I	25-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
J	31-1-6	-43	-55	---	FRONT	VERT	DEAD	---	C1
J	31-1-6	-254	-254	---	FRONT	VERT	SNOW	---	C1
O	25-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
S	11-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
U	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
W	5-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	11-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	15-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	19-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	21-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AE	23-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AF	27-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AG	29-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AH	1-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AI	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AJ	7-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AK	9-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AL	13-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AM	15-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AN	17-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AO	19-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AP	21-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AQ	23-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AR	27-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AS	29-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AT	31-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AU	33-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER

04/28/20

H. J. G. ALVES

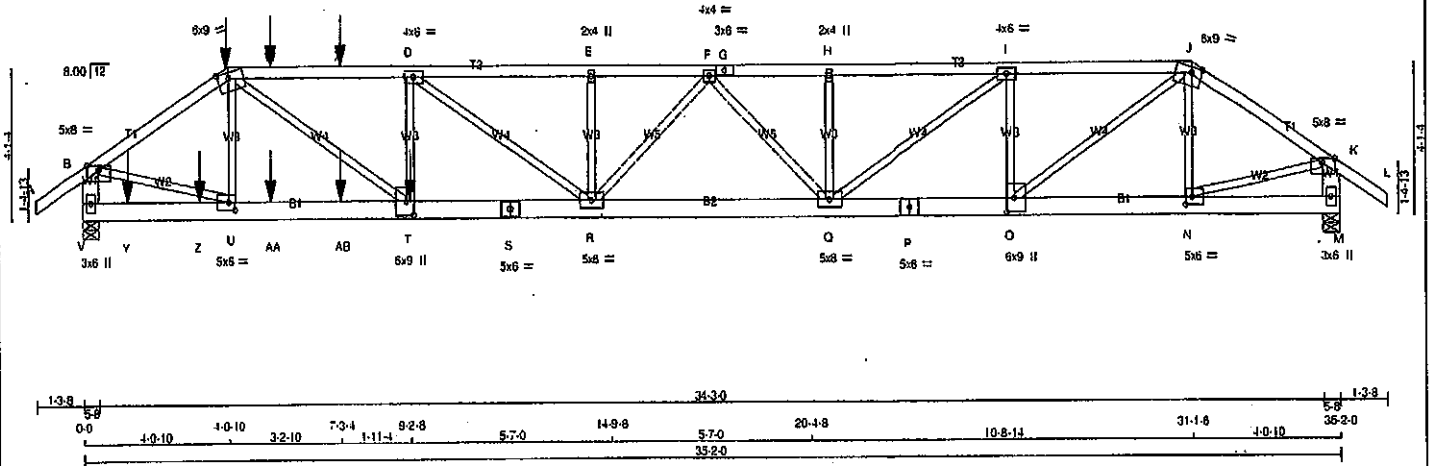
100009024

PROVINCE OF ONTARIO

Structural component only

DWG# T-2007602 2/2

JOB NAME 408222	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitok Industries, Inc. Tue Apr 28 09:00:05 2020 Page 1		
ID:DMCubINVR6TstFoe31v6l zns11-nyihMNBKcAvHCpQKTuoORWDoscNkApnRqDRR2zMFle				1:16 35-20 38-8		
1-3-8 0-0 4-0-10 4-0-10 5-3-4 7-3-4 9-3-12 5-5-12 14-9-8 28-8 17-7-0 20-4-8 25-10-4 5-3-2 4-0-10 1-3-8				Scale = 1:57.6		



TOTAL WEIGHT = 2 X 161 = 323 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
V - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
V - S	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(81.0)
C-G	12	TOP
G-J	12	TOP
J-L	12	TOP
V-B	12	TOP
M-K	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
V-S	12	SIDE(183.1)
S-P	12	TOP
P-M	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	8	SIDE(385.4)
D-T	4	
F-O	4	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	HORIZ	GROSS REACTION	HORIZ	BRG	IN-SX	BRG	IN-SX
V	4215	0	4215	0	0	5-8	5-8	5-8	5-8
M	2722	0	2722	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
V	2974	1987	0	0	0	0
M	1921	1283	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 35	U-C	-783 0
B-C	-5115 0	C-T	0 4817
C-W	-8135 0	T-D	-622 0
W-X	-8135 0	O-I	-1797 0
X-D	-8135 0	O-J	0 3211
D-E	-8106 0	N-J	-512 0
E-F	-8106 0	B-U	0 4370
F-G	-6926 0	N-K	0 2693
G-H	-6926 0	O-I	0 2133
H-I	-6926 0	D-R	-37 24
I-J	-5210 0	Q-H	-365 0
J-K	-3153 0	R-E	-343 0
K-L	0 35	R-F	0 692
V-B	-4180 0	F-O	-1081 0
M-K	-2686 0		

JT	LOC.	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-10	-49	-55	FRONT	VERT	DEAD	---	C1
C	4-0-10	-254	-254	FRONT	VERT	SNOW	---	C1
T	9-2-8	-2179	-2179	BACK	VERT	TOTAL	---	C1
W	5-3-4	-110	-110	BACK	VERT	TOTAL	---	C1
X	7-3-4	-110	-110	BACK	VERT	TOTAL	---	C1
Y	1-3-4	-26	-26	BACK	VERT	TOTAL	---	C1
Z	3-3-4	-26	-26	BACK	VERT	TOTAL	---	C1
AA	5-3-4	-26	-26	BACK	VERT	TOTAL	---	C1
AB	7-3-4	-26	-26	BACK	VERT	TOTAL	---	C1

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-10	-49	-55	FRONT	VERT	DEAD	---	C1
C	4-0-10	-254	-254	FRONT	VERT	SNOW	---	C1
T	9-2-8	-2179	-2179	BACK	VERT	TOTAL	---	C1
W	5-3-4	-110	-110	BACK	VERT	TOTAL	---	C1
X	7-3-4	-110	-110	BACK	VERT	TOTAL	---	C1
Y	1-3-4	-26	-26	BACK	VERT	TOTAL	---	C1
Z	3-3-4	-26	-26	BACK	VERT	TOTAL	---	C1
AA	5-3-4	-26	-26	BACK	VERT	TOTAL	---	C1
AB	7-3-4	-26	-26	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 8.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17)
CALCULATED VERT. DEFL.(LL) = L/999 (0.24)
ALLOWABLE DEFL.(TL) = L/360 (1.17)
CALCULATED VERT. DEFL.(TL) = L/954 (0.44)

CSL: TC=0.84/1.00 (C-D:1), BC=0.61/1.00 (R-T:1), WB=0.60/1.00 (C-T:1), SS=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	518	354
	1867	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (J) (INPUT = 0.90)
JSI METAL = 0.77 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2007603

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408222	T1Z	1	2	TRUSS DESC.		
Tamarack Roof Truss, Burlington				Version 8.310 5 Oct 29 2019 Mitel Industries, Inc. Tue Apr 28 09:00:05 2020 Page 2		
ID:DMCubINVR6TstFoe31v8l zns1l-nvnhMNB,KcAvHCpQKTuoORWDoscNkAonRqDRR2zMFle						

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	6.0	9.0	1.75	3.50
D	TMWW-i	MT20	4.0	6.0		
E	TMW-w	MT20	2.0	4.0		
F	TMWW-i	MT20	4.0	4.0		
G	TS-i	MT20	3.0	6.0		
H	TMW-w	MT20	2.0	4.0		
I	TMWW-i	MT20	4.0	8.0		
J	TTWW-m	MT20	6.0	9.0	1.75	3.50
K	TMVW-p	MT20	5.0	8.0	Edge	
M	BMV1-p	MT20	3.0	6.0		
N	BMWW-i	MT20	5.0	6.0	2.50	2.25
O	BMWW-i	MT20	8.0	9.0	4.50	2.50
P	BS-i	MT20	5.0	6.0		
Q	BMWW-i	MT20	5.0	8.0		
R	BMWW-i	MT20	5.0	8.0		
S	BS-i	MT20	5.0	6.0		
T	BMWW-i	MT20	8.0	9.0	4.50	2.50
U	BMWW-i	MT20	5.0	6.0	2.50	2.25
V	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

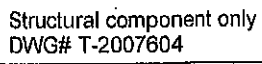
11 C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



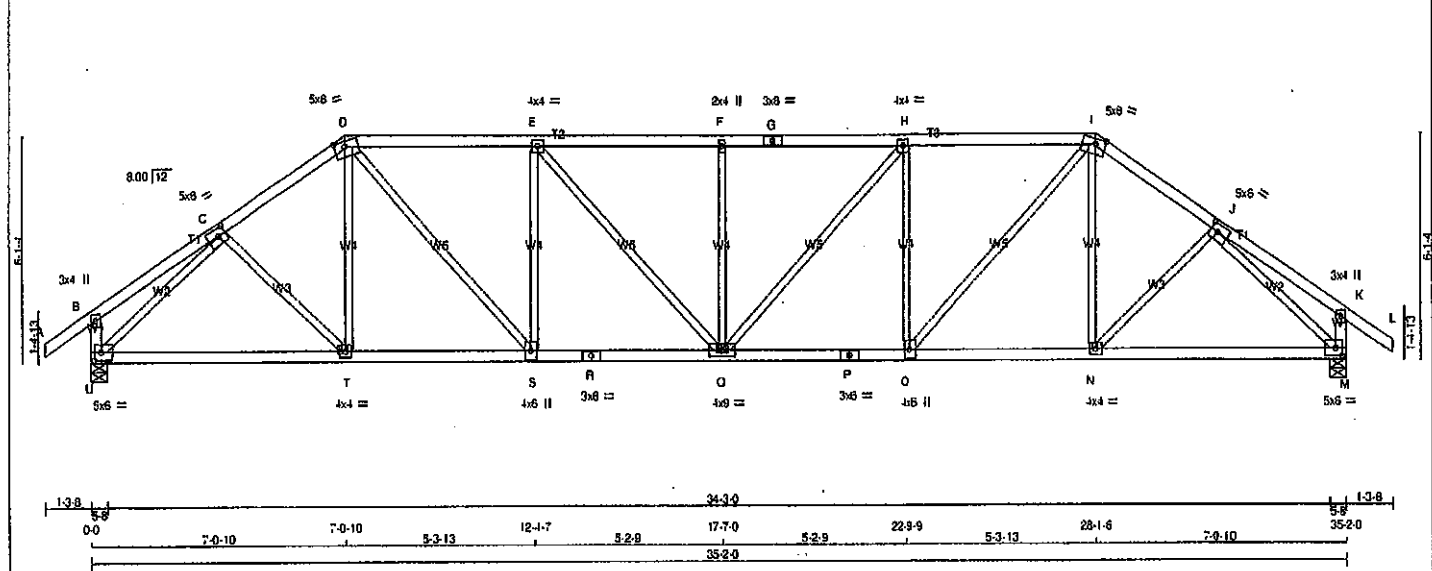
Structural component only
DWG# T-2007603 7/2



JSI GRP = 0.87 (I) (INPUT = 0.90)
JSI METAL = 0.74 (I) (INPUT = 1.00)



JOB NAME 408222	TRUSS NAME T3	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 09:00:08 2020 Page 1	
ID:DMCubINVR6TstFos31v6I_zns11-F8H4ajCt5w/mvMOcuAP1xf2QrG_VTXHw/Uz? UzMFid				28-1-6 31-6-7 35-2-0 36-5-8 1-3-8	
Scale = 1:57.6					



TOTAL WEIGHT = 2 X 151 = 303 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT	RECORD	SPECIFIED LOADS:	
A - D	2x4	DRY	No.2	SPF	JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT	IN-SX
D - G	2x4	DRY	No.2	SPF	U	2065	0	2065	0	5-8	5-8
G - I	2x4	DRY	No.2	SPF	M	2065	0	2065	0	5-8	5-8
I - L	2x4	DRY	No.2	SPF							
L - O	2x4	DRY	No.2	SPF							
O - P	2x4	DRY	No.2	SPF							
P - R	2x4	DRY	No.2	SPF							
R - S	2x4	DRY	No.2	SPF							
S - T	2x4	DRY	No.2	SPF							
T - U	2x4	DRY	No.2	SPF							
U - V	2x4	DRY	No.2	SPF							
V - W	2x4	DRY	No.2	SPF							
W - X	2x4	DRY	No.2	SPF							
X - Y	2x4	DRY	No.2	SPF							
Y - Z	2x4	DRY	No.2	SPF							
ALL WEBS	2x3	DRY	No.2	SPF							
EXCEPT											

DRY, SEASONED LUMBER.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 35	-91.8	-91.8 0.12 (1)	10.00	C-T	0 135	0.03 (4)
B-C	0 18	-91.8	-91.8 0.16 (1)	10.00	T-D	0 90	0.03 (4)
C-D	-2305 0	-91.8	-91.8 0.23 (1)	4.31	D-S	0 1218	0.27 (1)
D-E	-2723 0	-91.8	-91.8 0.54 (1)	3.69	S-E	-814 0	0.48 (1)
E-F	-2986 0	-91.8	-91.8 0.58 (1)	3.53	E-Q	0 383	0.08 (1)
F-G	-2986 0	-91.8	-91.8 0.58 (1)	3.53	Q-F	-440 0	0.28 (1)
G-H	-2986 0	-91.8	-91.8 0.58 (1)	3.53	H-Q	0 383	0.08 (1)
H-I	-2723 0	-91.8	-91.8 0.54 (1)	3.69	Q-H	-814 0	0.48 (1)
I-J	-2305 0	-91.8	-91.8 0.23 (1)	4.31	O-I	0 1218	0.27 (1)
J-K	0 18	-91.8	-91.8 0.16 (1)	10.00	N-I	0 90	0.03 (4)
K-L	0 35	-91.8	-91.8 0.12 (1)	10.00	N-J	0 135	0.03 (4)
U-B	-255 0	0.0	0.0 0.03 (1)	7.81	U-C	-2514 0	0.97 (1)
M-K	-255 0	0.0	0.0 0.03 (1)	7.81	J-M	-2514 0	0.97 (1)

U-T	0 1801	-18.5	-18.5 0.40 (1)	10.00
T-S	0 1900	-18.5	-18.5 0.41 (1)	10.00
S-R	0 2723	-18.5	-18.5 0.49 (1)	10.00
R-O	0 2723	-18.5	-18.5 0.49 (1)	10.00
Q-P	0 2723	-18.5	-18.5 0.49 (1)	10.00
P-O	0 2723	-18.5	-18.5 0.49 (1)	10.00
O-N	0 1900	-18.5	-18.5 0.41 (1)	10.00
N-M	0 1801	-18.5	-18.5 0.40 (1)	10.00

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CS: TC=0.58/1.00 (E-F:1), BC=0.49/1.00 (O-Q:1),
WB=0.97/1.00 (J-M:1), SS=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL = 0.84 (PH) (INPUT = 1.00)

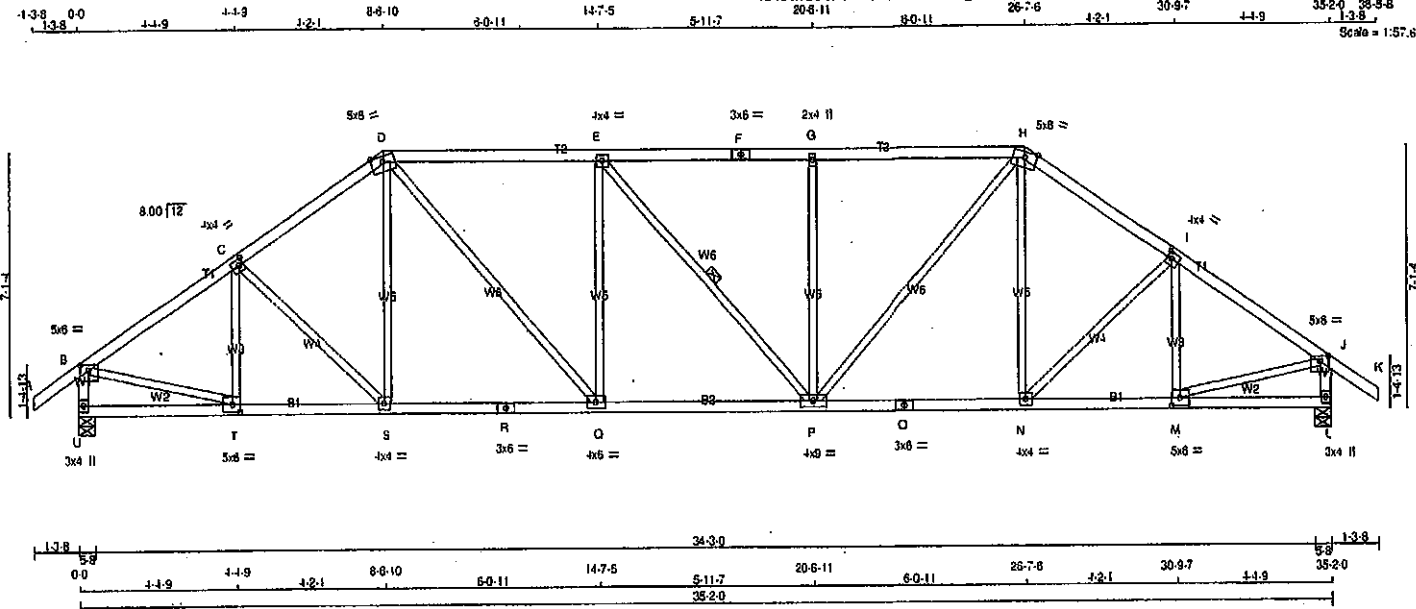


Structural component only
DWG# T-2007605

JOB NAME 408222	TRUSS NAME T4	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Autodesk Industries, Inc. Tue Apr 28 09:00:07 2020 Page 1

ID:DMCubINVR6TslFoe31v8l_zns1l-LtSn2CZsDQdXWzSpSuwGTsbZigK2C5T4u6iVWwzMFIc



TOTAL WEIGHT = 2 X 155 = 309 lb

LUMBER			LUMBER		
N. L. G. A. RULES	CHORDS	SIZE	DRY	No. 2	
A - D	2x4	DRY	No. 2		
D - F	2x4	DRY	No. 2		
F - H	2x4	DRY	No. 2		
H - K	2x4	DRY	No. 2		
U - B	2x4	DRY	No. 2		
L - J	2x4	DRY	No. 2		
U - R	2x4	DRY	No. 2		
R - O	2x4	DRY	No. 2		
O - L	2x4	DRY	No. 2		

ALL WEBS 2x3 DRY No. 2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWW-p	MT20	5.0	6.0	1.75 2.75
C	TMWW-i	MT20	4.0	4.0	2.00 1.50
D	TTWW-m	MT20	5.0	8.0	Edge
E	TMWW-i	MT20	4.0	4.0	
F	TS-1	MT20	3.0	6.0	
G	TMWW-w	MT20	2.0	4.0	
H	TTWW-m	MT20	5.0	8.0	Edge
I	TMWW-i	MT20	4.0	4.0	2.00 1.50
J	TMWW-p	MT20	5.0	8.0	1.75 2.75
L	BMV1-p	MT20	3.0	4.0	
N	BMWW-i	MT20	5.0	8.0	2.50 2.75
O	BS-1	MT20	4.0	4.0	
P	BMWW-i	MT20	4.0	4.0	
Q	BS-1	MT20	4.0	4.0	
R	BS-1	MT20	3.0	6.0	
S	BMWW-i	MT20	4.0	4.0	
T	BMWW-i	MT20	5.0	6.0	2.50 2.75
U	BMV1-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
U	2065	0	2065	0
L	2065	0	2065	0

UNFACTORED REACTIONS

	1ST LOASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERALIVE	WIND	DEAD	SOIL	
U	1458	970	0	0.0	0.0	488	0	0.0
L	1458	970	0	0.0	0.0	488	0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U. L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS						
MEMB.	MAX. FACTORED	FORCE	VERT. LOAD	MEMB.	MAX. FACTORED	FORCE	MAX.			
	(LBS)	(PLF)	LC1		(LBS)	(LBS)	LC1			
FR-TO				FR-TO						
A-B	0	35	-91.8	-91.8	0.12 (1)	10.00	T-C	-397	0	0.11 (1)
B-C	-2269	0	-91.8	-91.8	0.37 (1)	4.19	C-S	-78	0	0.05 (1)
C-D	-2254	0	-91.8	-91.8	0.36 (1)	4.21	S-D	0	155	0.04 (4)
D-E	-2485	0	-91.8	-91.8	0.64 (1)	3.72	D-Q	0	949	0.21 (1)
E-F	-2483	0	-91.8	-91.8	0.84 (1)	3.72	Q-E	-597	0	0.52 (1)
F-G	-2483	0	-91.8	-91.8	0.84 (1)	3.72	E-P	-2	0	0.00 (1)
G-H	-2483	0	-91.8	-91.8	0.83 (1)	3.74	P-G	-597	0	0.52 (1)
H-I	-2254	0	-91.8	-91.8	0.36 (1)	4.21	P-H	0	947	0.21 (1)
I-J	-2269	0	-91.8	-91.8	0.37 (1)	4.19	N-H	0	157	0.04 (4)
J-K	0	35	-91.8	-91.8	0.12 (1)	10.00	N-I	-78	0	0.05 (1)
U-B	-2027	0	0.0	0.0	0.21 (1)	5.94	M-I	-397	0	0.11 (1)
L-J	-2026	0	0.0	0.0	0.21 (1)	5.94	B-T	0	1995	0.44 (1)
							M-J	0	1995	0.44 (1)
U-T	0	0	-18.5	-18.5	0.08 (4)	10.00				
T-S	0	1908	-18.5	-18.5	0.37 (1)	10.00				
S-R	0	1853	-18.5	-18.5	0.36 (1)	10.00				
R-Q	0	1953	-18.5	-18.5	0.35 (1)	10.00				
Q-P	0	2485	-18.5	-18.5	0.47 (1)	10.00				
P-O	0	1854	-18.5	-18.5	0.36 (1)	10.00				
O-N	0	1854	-18.5	-18.5	0.36 (1)	10.00				
N-M	0	1908	-18.5	-18.5	0.37 (1)	10.00				
M-L	0	0	-18.5	-18.5	0.08 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

OL = 8.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 088-09, CSA 088-14

- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")

CALCULATED VERT. DEFL.(LL) = L/999 (0.12")

ALLOWABLE DEFL.(TL) = L/360 (1.17")

CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.84/1.00 (D-E:1), BC=0.47/1.00 (P-Q:1)

WB=0.52/1.00 (E-Q:1), SS=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 616 354 1667 768 1997 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

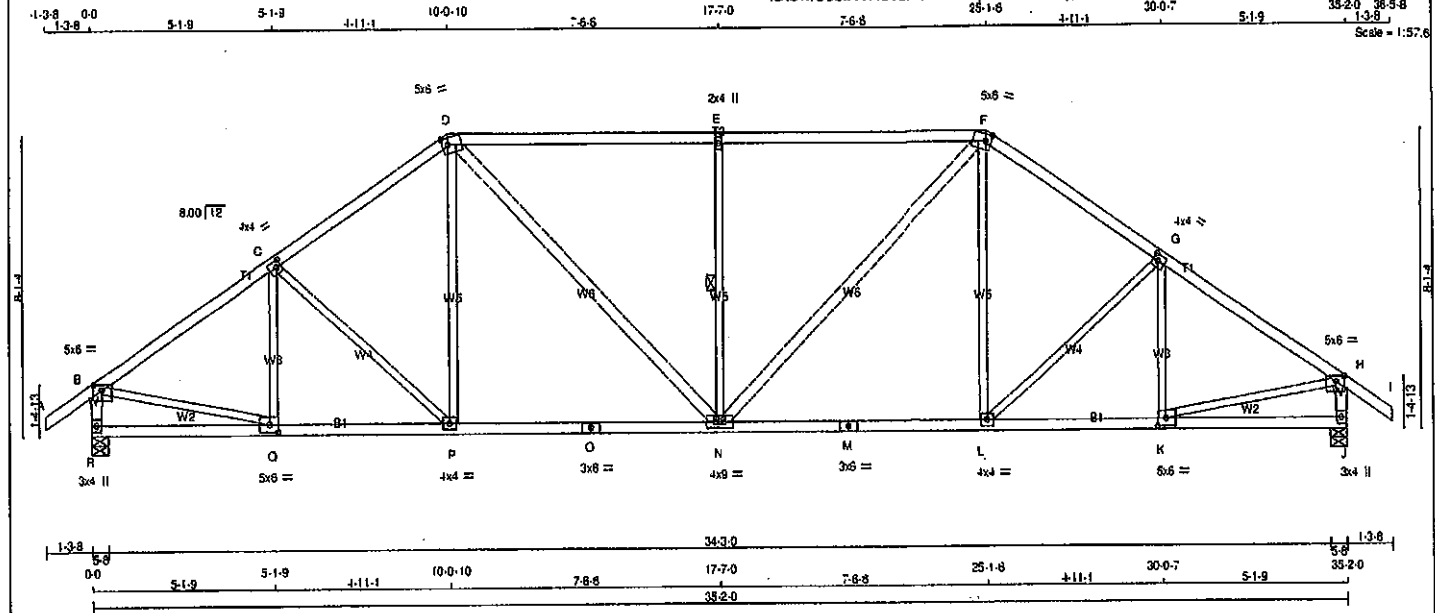
JSI GRIP= 0.90 (0) (INPUT = 0.90)

JSI METAL= 0.62 (0) (INPUT = 1.00)



Structural component only
DWG# T-2007606

JOB NAME 408222	TRUSS NAME T5	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					
Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 09:00:08 2020 Page 1					
ID:DMCubINVR8TsF0e31v6l_zns1l-BXPq_OD8dXYU8Y??bRV048iM4gExZrD7oSS2NzMFk					
Scale = 1:57.6					



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2
DRY; SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
		GROSS REACTION		DOWN		BRG		IN-SX	
		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX
JT	R	2065	0	2065	0	0	5.8	5.8	5.8
J	J	2065	0	2065	0	0	5.8	5.8	5.8

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS		DEAD		SOIL	
		COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
JT	R	1458	970	0	0	0	0
J	J	1458	970	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
		MEMB. FORCE (LBS)				MEMB. FORCE (LBS)	
		VERT. LOAD (LBS)		MAX. MAX. (LBS)		MAX. MAX. (LBS)	
		FROM TO		FROM TO		FROM TO	
FR-TO							
A-B	0 35	-91.8	-91.8 0.12 (1)	10.00	C-C	-321 0	0.11 (1)
B-C	-2317 0	-91.8	-91.8 0.38 (1)	4.18	C-P	-218 0	0.18 (1)
C-D	-2185 0	-91.8	-91.8 0.37 (1)	4.28	P-D	0 266	0.06 (1)
D-E	-2274 0	-91.8	-91.8 0.81 (1)	3.47	D-N	0 691	0.11 (1)
E-F	-2274 0	-91.8	-91.8 0.81 (1)	3.47	N-E	-651 0	0.35 (1)
F-G	-2185 0	-91.8	-91.8 0.37 (1)	4.28	N-F	0 691	0.11 (1)
G-H	-2317 0	-91.8	-91.8 0.38 (1)	4.18	L-F	0 266	0.06 (1)
H-I	0 35	-91.8	-91.8 0.12 (1)	10.00	L-G	-218 0	0.18 (1)
R-B	-2022 0	0.0	0.0 0.21 (1)	5.94	K-G	-321 0	0.11 (1)
J-H	-2022 0	0.0	0.0 0.21 (1)	5.94	B-Q	0 1955	0.45 (1)
					K-H	0 1955	0.45 (1)
R-Q	0 0	-18.5	-18.5 0.10 (4)	10.00			
O-P	0 1953	-18.5	-18.5 0.40 (1)	10.00			
P-O	0 1794	-18.5	-18.5 0.41 (1)	10.00			
O-N	0 1794	-18.5	-18.5 0.41 (1)	10.00			
N-M	0 1794	-18.5	-18.5 0.41 (1)	10.00			
M-L	0 1794	-18.5	-18.5 0.41 (1)	10.00			
L-K	0 1953	-18.5	-18.5 0.40 (1)	10.00			
K-J	0 0	-18.5	-18.5 0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBCG 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G/S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17)
CALCULATED VERT. DEFL.(LL) = L/989 (0.11)
ALLOWABLE DEFL.(TL) = L/360 (1.17)
CALCULATED VERT. DEFL.(TL) = L/999 (0.22)

CSK: TC=0.81 1.00 (E-F); BC=0.41 1.00 (L-N); WB=0.45 1.00 (H-K); SS=0.34 1.00 (D-E)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1556

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

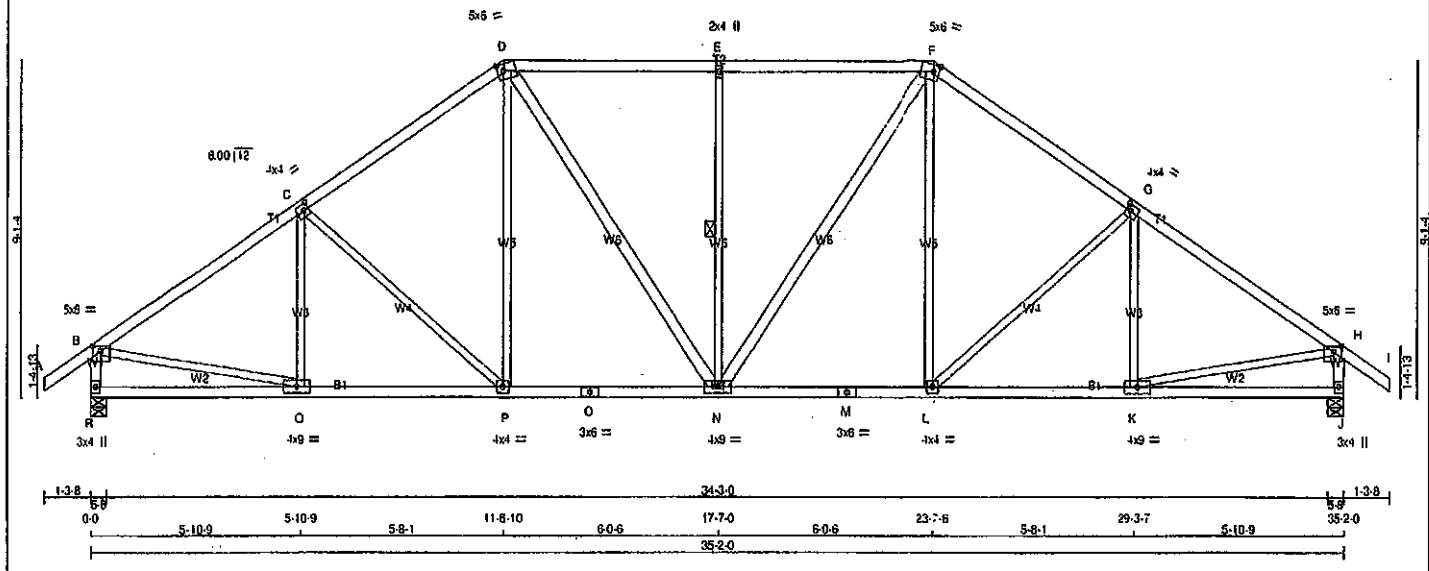
JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.64 (M) (INPUT = 1.00)



Structural component only
DWG# T-2007607

JOB NAME 408222	TRUSS NAME T6	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 5 Oct 2019 M&T Tek Industries, Inc. Tue Apr 28 09:00:09 2020 Page 1
 ID:DMCubINVR6TstFos31v6l_zns11-fjyCCKEpOrgLmp6BZlykZHqkU0ng01MLSBfapzMF1a
 1-3.8 0-0 5-10.2 5-10.9 5-8.1 11-8.10 6-0.6 17-7.0 6-0.6 23-7.6 5-8.1 29-3.7 5-10.9 35-2.0 1-3.8
 Scale = 1:37.6



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
R - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
D - N	2x4	DRY	No.2	SPF	
N - F	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	6.0	1.75	2.75	
C TMVW-w	MT20	4.0	4.0	2.00	1.50	
D TTWW-m	MT20	5.0	6.0	2.25	2.00	
E TMVW-w	MT20	2.0	4.0			
F TTWW-m	MT20	5.0	6.0	2.25	2.00	
G TMVW-w	MT20	4.0	4.0	2.00	1.50	
H TMVW-p	MT20	5.0	6.0	1.75	2.75	
J BMV1-p	MT20	3.0	4.0			
K BMVW-1	MT20	4.0	4.0			
L BMVW-1	MT20	4.0	4.0			
M BS-1	MT20	3.0	6.0			
N BMVW-1	MT20	4.0	4.0			
O BS-1	MT20	3.0	6.0			
P BMVW-1	MT20	4.0	4.0			
Q BMVW-1	MT20	4.0	4.0			
R BMV1-p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT VERT	2065	0	0	5-8
R	2065	0	0	5-8
J	2065	0	0	5-8

UNFACTORED REACTIONS	IST LCASE	MAX. MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT COMBINED	1458	970	0	0	0
R	1458	970	0	0	0
J	1458	970	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
 1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: 14)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLP)	MAX. FACTORED CSI (LC)	UNBRAC LENGTH	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO					FR-TO		
A-B	0 35	-91.8	-91.8	0.12 (1)	10.00	Q-G	-249 11
B-C	-2346 0	-91.8	-91.8	0.52 (1)	3.99	C-P	-356 0
C-D	-2095 0	-91.8	-91.8	0.48 (1)	4.22	P-D	0 341
D-E	-1985 0	-91.8	-91.8	0.40 (1)	4.26	D-N	0 477
E-F	-1985 0	-91.8	-91.8	0.40 (1)	4.26	N-E	-678 0
F-G	-2095 0	-91.8	-91.8	0.48 (1)	4.22	N-F	0 477
G-H	-2346 0	-91.8	-91.8	0.52 (1)	3.99	L-F	0 341
H-I	0 35	-91.8	-91.8	0.12 (1)	10.00	L-G	-356 0
R-B	-2018 0	0.0	0.0	0.21 (1)	5.94	K-G	-249 11
J-H	-2018 0	0.0	0.0	0.21 (1)	5.94	B-Q	0 2014
R-Q	0 0	-18.5	-18.5	0.14 (4)	10.00	K-H	0 2014
Q-P	0 1980	-18.5	-18.5	0.39 (1)	10.00		
P-O	0 1716	-18.5	-18.5	0.34 (1)	10.00		
O-N	0 1716	-18.5	-18.5	0.34 (1)	10.00		
N-M	0 1716	-18.5	-18.5	0.34 (1)	10.00		
M-L	0 1716	-18.5	-18.5	0.34 (1)	10.00		
L-K	0 1981	-18.5	-18.5	0.39 (1)	10.00		
K-J	0 0	-18.5	-18.5	0.14 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 8.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, CBC 2012, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17')
 CALCULATED VERT. DEFL.(LL) = L/999 (0.10')
 ALLOWABLE DEFL.(TL) = L/360 (1.17')
 CALCULATED VERT. DEFL.(TL) = L/999 (0.19')

CS: TC=0.52/1.00 (G-H:1), BC=0.39/1.00 (K-L:1), WB=0.45/1.00 (H-K:1), SS=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 -MAX MIN MAX MIN MAX MIN
 MT20 616 354 1667 788 1997 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) INPUT = 0.90
 JSI METAL= 0.56 (M) INPUT = 1.00



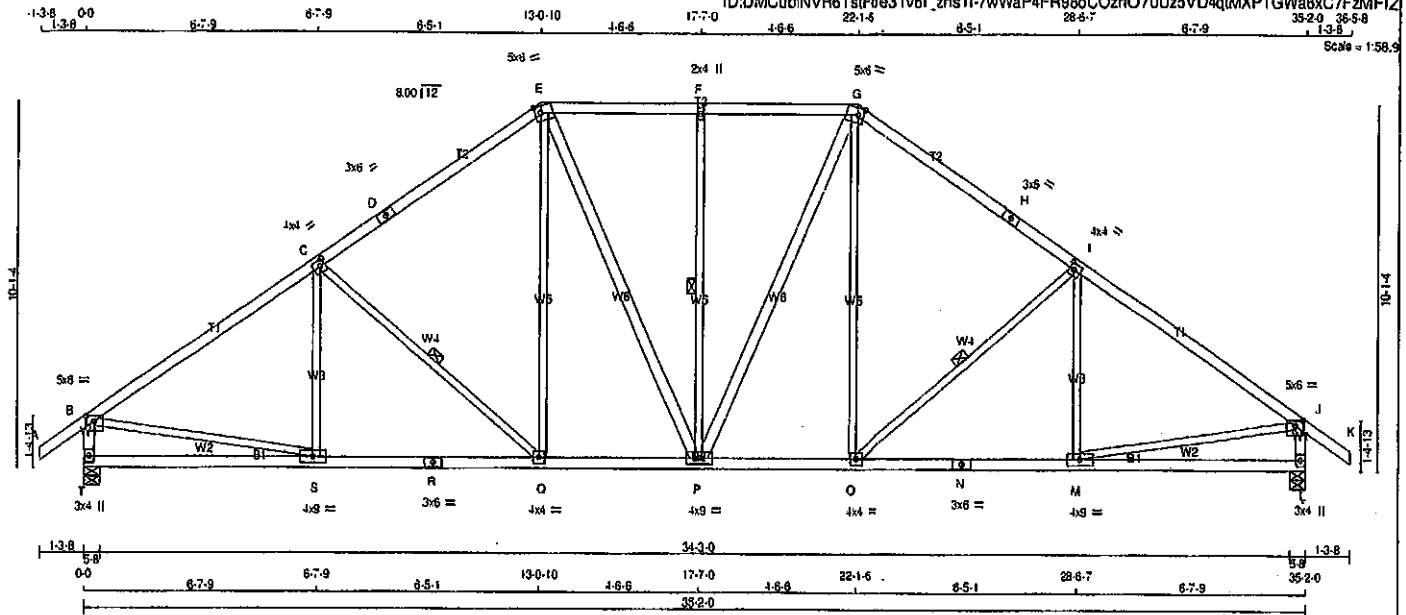
Structural component only
 DWG# T-2007608

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	T7	6	1	GREEN PARK HOMES	
TRUSS DESC.					

Tamrack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 09:00:10 2020 Page 1

ID:DMCubINVR6Tsf0e31v6I_zns1i7wWap4FR98oCOzhO70Uz5VD4qIMXPTGwa8xC7FzMFIZ



TOTAL WEIGHT = 6 X 172 = 1030 lb [M/F]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - K	2x4	DRY	No.2
L - J	2x4	DRY	No.2
T - B	2x4	DRY	No.2
T - R	2x4	DRY	No.2
T - N	2x4	DRY	No.2
T - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
E - P	2x4	DRY	No.2
P - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)			
JT	TYPE	PLATES	W LEN Y X
B	TMWV-p	MT20	5.0 6.0 1.75 2.75
C	TMWV-w	MT20	4.0 4.0 2.00 1.50
D	TS-1	MT20	3.0 6.0
E	TTWV-m	MT20	5.0 6.0 2.25 2.00
F	TMWV-w	MT20	2.0 4.0
G	TTWV-m	MT20	5.0 6.0 2.25 2.00
H	TS-1	MT20	3.0 6.0
I	TMWV-w	MT20	4.0 4.0 2.00 1.50
J	TMWV-p	MT20	5.0 6.0 1.75 2.75
L	BMV1-p	MT20	3.0 4.0
M	BMWV-w	MT20	4.0 9.0
N	BS-1	MT20	3.0 6.0
O	BMWV-w	MT20	4.0 4.0
P	BMWV-w	MT20	4.0 9.0
Q	BMWV-w	MT20	4.0 4.0
R	BS-1	MT20	3.0 6.0
S	BMWV-w	MT20	4.0 9.0
T	BMV1-p	MT20	3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT REGR
T	2065 0	2065 0	5-8
L	2065 0	2065 0	5-8

UNFACTORED REACTIONS			
JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	
T	1458 970 0	0 0	0 0
L	1458 970 0	0 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1:2 LENGTH OF C-Q, F-P, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING			
TOTAL LOAD CASES: (4)			
CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO			
A-B	0 35	-91.8 -91.8 0.12 (1)	10.00
B-C	-2358 0	-91.8 -91.8 0.69 (1)	3.77
C-D	-1996 0	-91.8 -91.8 0.62 (1)	4.11
D-E	-1996 0	-91.8 -91.8 0.62 (1)	4.11
E-F	-1761 0	-91.8 -91.8 0.27 (1)	4.77
F-G	-1761 0	-91.8 -91.8 0.27 (1)	4.77
G-H	-1996 0	-91.8 -91.8 0.62 (1)	4.11
H-I	-1996 0	-91.8 -91.8 0.62 (1)	4.11
I-J	-2358 0	-91.8 -91.8 0.69 (1)	3.77
J-K	0 35	-91.8 -91.8 0.12 (1)	10.00
T-B	-2014 0	0.0 0.0 0.21 (1)	5.85
L-J	-2014 0	0.0 0.0 0.21 (1)	5.85
WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO			
S-C	-189 46	0.10 (1)	0.10 (1)
C-Q	-484 0	0.24 (1)	0.24 (1)
Q-E	0 415	0.08 (1)	0.08 (1)
E-P	0 315	0.05 (1)	0.05 (1)
P-F	-502 0	0.35 (1)	0.35 (1)
F-P	0 315	0.05 (1)	0.05 (1)
O-G	0 415	0.08 (1)	0.08 (1)
O-I	-484 0	0.24 (1)	0.24 (1)
M-I	-189 46	0.10 (1)	0.10 (1)
B-S	0 2021	0.45 (1)	0.45 (1)
M-J	0 2021	0.45 (1)	0.45 (1)

T-S	0 0	-18.5 -18.5 0.19 (4)	10.00
S-R	0 1995	-18.5 -18.5 0.42 (1)	10.00
R-Q	0 1995	-18.5 -18.5 0.42 (1)	10.00
Q-P	0 1829	-18.5 -18.5 0.32 (1)	10.00
P-O	0 1829	-18.5 -18.5 0.32 (1)	10.00
O-N	0 1995	-18.5 -18.5 0.42 (1)	10.00
N-M	0 1995	-18.5 -18.5 0.42 (1)	10.00
M-L	0 0	-18.5 -18.5 0.19 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.69/1.00 (I-J:1), BC=0.42/1.00 (M-O:1), WB=0.45/1.00 (J-M:1), SS=0.24/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (I-J) (INPUT = 0.90)
JSI METAL = 0.87 (N) (INPUT = 1.00)



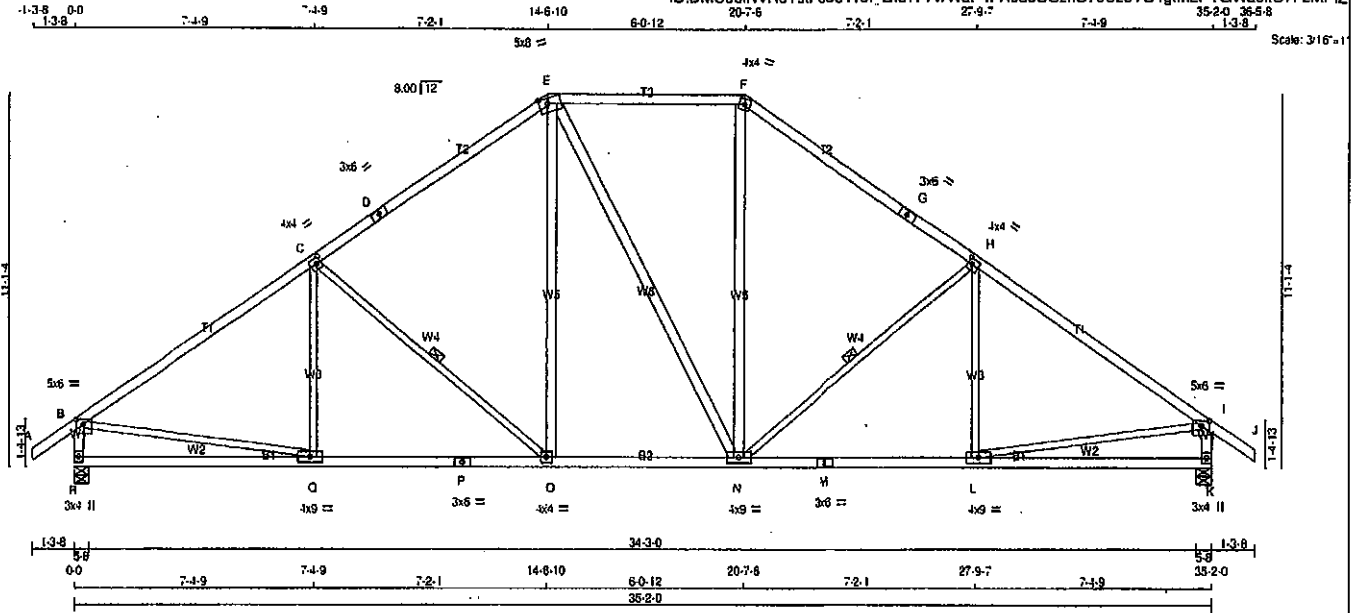
Structural component only
DWG# T-2007609

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408222	T8	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 09:00:10 2020 Page 1

ID:DMCubINVR6TstFoe31v6I, zns11-7wWp4FR98oCOz070Uz5VD1gIM2PTGWa8xC7FzMFIZ



TOTAL WEIGHT = 2 X 166 = 333 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
O - E	2x4	DRY	No.2	SPF
E - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)							
JT	TYPE	PLATES	W	LEN	Y	X	
B	TMVW-p	MT20	5.0	6.0	1.75	2.75	
C	TMVW-w	MT20	4.0	4.0	2.00	1.50	
D	TS-1	MT20	3.0	6.0			
E	TTWW-m	MT20	5.0	6.0	2.00	3.00	
F	TTW-m	MT20	4.0	4.0			
G	TS-1	MT20	3.0	6.0			
H	TMVW-w	MT20	4.0	4.0	2.00	1.50	
I	TMVW-p	MT20	5.0	6.0	1.75	2.75	
K	BMV1+p	MT20	3.0	4.0			
L	BMVW-w	MT20	4.0	9.0			
M	BS-1	MT20	3.0	6.0			
N	BMVW-w	MT20	4.0	9.0			
O	BMVW-w	MT20	4.0	4.0			
P	BS-1	MT20	3.0	6.0			
Q	BMVW-w	MT20	4.0	9.0			
R	BMV1+p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	GROSS REACTION	MAXIMUM FACTORED	GROSS REACTION	INPUT	RECORD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
R	2065	0	2065	0	5-8	5-8
K	2065	0	2065	0	5-8	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
R	1458	970.0	0.0	0.0	0.0	488.0	0.0	
K	1458	970.0	0.0	0.0	0.0	488.0	0.0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

	CHORDS	FACTORED	VERT. LOAD	LC1	MAX	UNBRAC	MEMB.	MAX. FACTORED	WEBS	FACTORED	VERT. LOAD	LC1	MAX	UNBRAC	MEMB.	MAX. FACTORED
FR-TO																
A-B	0	35	-91.8	-91.8	0.12 (1)	10.00	Q-C	-140	76	0.09 (1)						
B-C	-2357	0	-91.8	-91.8	0.89 (1)	3.45	Q-D	-604	0	0.38 (1)						
C-D	-1888	0	-91.8	-91.8	0.78 (1)	3.91	Q-E	0	499	0.08 (1)						
D-E	-1888	0	-91.8	-91.8	0.78 (1)	3.91	E-N	0	0	0.00 (1)						
E-F	-1538	0	-91.8	-91.8	0.47 (1)	4.74	N-F	0	500	0.08 (1)						
F-G	-1888	0	-91.8	-91.8	0.78 (1)	3.91	N-H	-604	0	0.38 (1)						
G-H	-1888	0	-91.8	-91.8	0.78 (1)	3.91	L-H	-140	75	0.09 (1)						
H-I	-2357	0	-91.8	-91.8	0.89 (1)	3.45	B-Q	0	2020	0.45 (1)						
I-J	0	35	-91.8	-91.8	0.12 (1)	10.00	L-I	0	2019	0.45 (1)						
R-B	-2009	0	0.0	0.0	0.21 (1)	5.95										
K-I	-2009	0	0.0	0.0	0.21 (1)	5.95										
R-Q	0	0	-18.5	-18.5	0.25 (4)	10.00										
Q-P	0	1998	-18.5	-18.5	0.45 (1)	10.00										
P-O	0	1998	-18.5	-18.5	0.45 (1)	10.00										
O-N	0	1538	-18.5	-18.5	0.33 (1)	10.00										
N-M	0	1998	-18.5	-18.5	0.45 (1)	10.00										
M-L	0	1998	-18.5	-18.5	0.45 (1)	10.00										
L-K	0	0	-18.5	-18.5	0.24 (4)	10.00										

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.8 PSF
DL	= 8.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.89/1.00 (B-C:1), BC=0.45/1.00 (O-Q:1), WB=0.45/1.00 (B-Q:1), SSI=0.27/1.00 (H-I:1)

DOE LUMBER=1.00 NAIL=1.00 L.S. BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX	MIN	MAX
MIN	MAX	MIN
MT20	618	354
	1887	788
	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

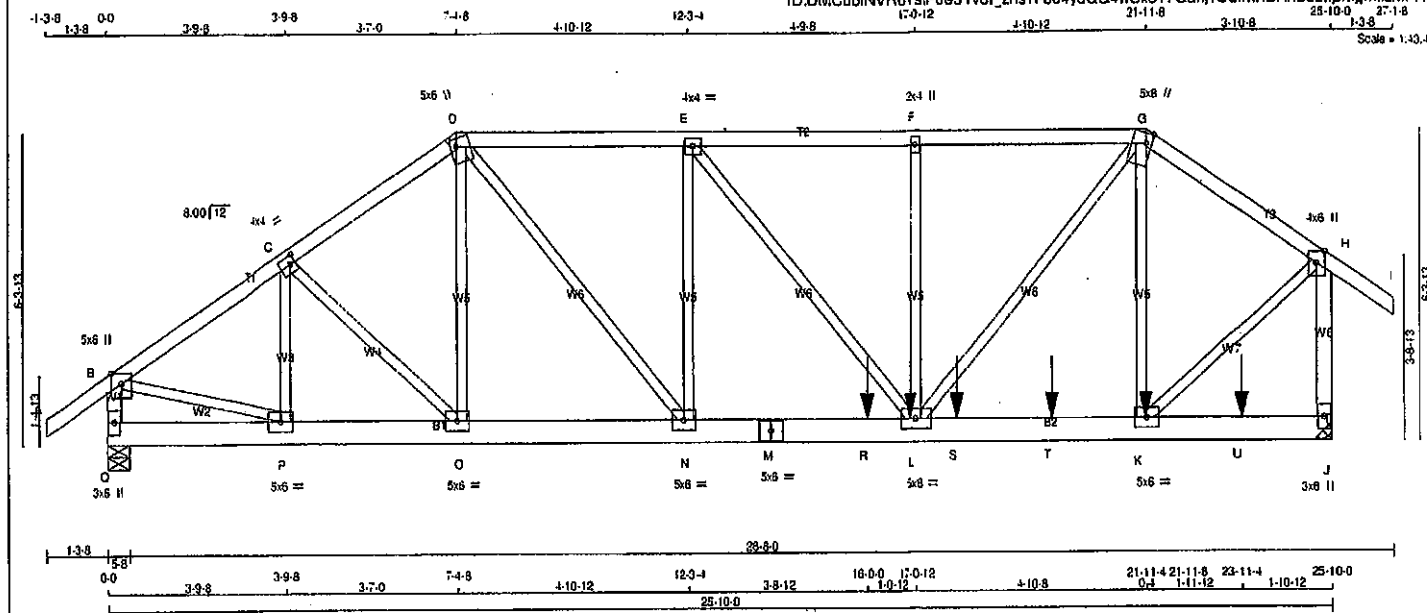
JSI GRIP= 0.90 (F) INPUT = 0.90 (I)

JSI METAL= 0.88 (P) INPUT = 1.00 (I)



Structural component only
DWG# T-2007610

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	T9	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
I - B	2x4	DRY No.2	SPF
B - H	2x4	DRY No.2	SPF
H - M	2x4	DRY No.2	SPF
M - J	2x6	DRY No.2	SPF
J - K	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'X3') SPIRAL NAILS		
A - D 1 12		TOP
D - G 1 12		TOP
G - I 1 12		TOP
I - B 1 12		TOP
B - H 1 12		TOP
H - M 1 12		TOP
M - J 2 12		TOP
J - K 2 12		TOP
WEBS : (0.122'X3') SPIRAL NAILS		
2x3 1 6		TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
Q	2236	0	2236	0
J	3067	0	3067	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 4'-0".

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	1978	1085	0	0	0	510	0
J	2157	1479	0	0	0	678	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	WEBS MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	
FR-TO							
A-B	0 35	-91.8	-91.8	0.07 (1)	10.00	P-C	-503 0
B-C	-2464 0	-91.8	-91.8	0.13 (1)	5.62	C-D	0 70
C-D	-2571 0	-91.8	-91.8	0.13 (1)	5.53	D-E	-2 60
D-E	-3030 0	-91.8	-91.8	0.20 (1)	5.11	E-F	0 1433
E-F	-3235 0	-91.8	-91.8	0.21 (1)	4.97	F-G	-736 0
F-G	-3235 0	-91.8	-91.8	0.20 (1)	4.99	G-H	0 327
G-H	-2123 0	-91.8	-91.8	0.15 (1)	5.91	H-I	-169 0
H-I	0 35	-91.8	-91.8	0.07 (1)	10.00	I-J	0 2339
I-J	-2188 0	0.0	0.0	0.12 (1)	7.53	J-K	-995 0
J-K	-2862 0	0.0	0.0	0.33 (1)	6.70	K-L	0 2137
K-L						L-M	0 2299
L-M						M-N	
M-N	0 0	-18.5	-18.5	0.02 (1)	10.00	N-O	
N-O	0 2087	-18.5	-18.5	0.18 (1)	10.00	O-P	
O-P	0 2118	-18.5	-18.5	0.22 (1)	10.00	P-Q	
P-Q	0 3030	-18.5	-18.5	0.45 (1)	10.00	Q-R	
Q-R	0 3030	-18.5	-18.5	0.45 (1)	10.00	R-S	
R-S	0 3030	-18.5	-18.5	0.45 (1)	10.00	S-T	
S-T	0 1748	-18.5	-18.5	0.27 (1)	10.00	T-U	
T-U	0 1748	-18.5	-18.5	0.27 (1)	10.00	U-V	
U-V	0 1748	-18.5	-18.5	0.27 (1)	10.00	V-W	
V-W	0 1748	-18.5	-18.5	0.27 (1)	10.00	W-X	
W-X	0 1748	-18.5	-18.5	0.27 (1)	10.00	X-Y	
X-Y	0 1748	-18.5	-18.5	0.27 (1)	10.00	Y-Z	
Y-Z	0 1748	-18.5	-18.5	0.27 (1)	10.00	Z-A	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
K	21-11-4	-187	-187	---	FRONT	VERT	TOTAL	---	C1
L	16-11-4	-187	-187	---	FRONT	VERT	TOTAL	---	C1
R	16-0-0	-1259	-1259	---	FRONT	VERT	TOTAL	---	C1
S	17-11-4	-187	-187	---	FRONT	VERT	TOTAL	---	C1
T	19-11-4	-187	-187	---	FRONT	VERT	TOTAL	---	C1
U	23-11-4	-195	-195	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, NBCC 2015
- PART 9 OF NBCC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

GSI: TC=0.33/1.00 (H-J:1), BC=0.45/1.00 (L-N:1), WB=0.30/1.00 (G-K:1), SB=0.35/1.00 (L-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1667 788 1987 1656	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.81 (H) (INPUT = 0.90)
JSI METAL = 0.53 (H) (INPUT = 1.00)



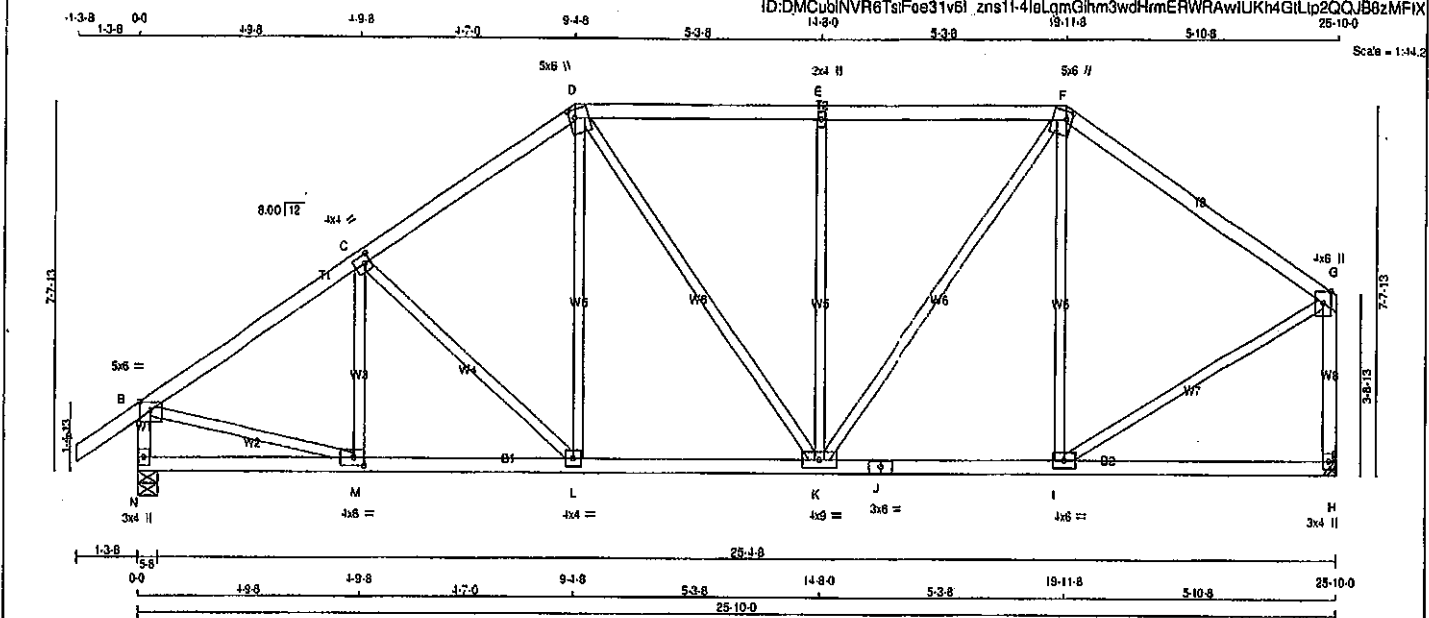
Structural component only
DWG# T-2007611 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.																																																																																				
408222	T9	1	2	GREEN PARK HOMES																																																																																					
Tamarack Roof Truss, Burlington																																																																																									
Version 8.310 \$ Oct 29 2019 MTek Industries, Inc. Tue Apr 28 09:00:11 2020 Page 2																																																																																									
ID:DMCubINVR6TslFog31v6l zns1l-b64ydQG4wSx377Gah?CelmKBHhB8ztfpmgmfizMFIY																																																																																									
PLATES (table is in inches) <table border="1"><thead><tr><th>JT TYPE</th><th>PLATES</th><th>W</th><th>LEN</th><th>Y</th><th>X</th></tr></thead><tbody><tr><td>B TMVW+p</td><td>MT20</td><td>5.0</td><td>6.0</td><td>Edge</td><td></td></tr><tr><td>C TMWW-l</td><td>MT20</td><td>4.0</td><td>4.0</td><td>2.00</td><td>1.50</td></tr><tr><td>D TTWW+m</td><td>MT20</td><td>5.0</td><td>6.0</td><td>2.00</td><td>1.50</td></tr><tr><td>E TMWW-l</td><td>MT20</td><td>4.0</td><td>4.0</td><td></td><td></td></tr><tr><td>F TMV+w</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr><tr><td>G TTWW+m</td><td>MT20</td><td>5.0</td><td>8.0</td><td>2.50</td><td>1.50</td></tr><tr><td>H TMVW+p</td><td>MT20</td><td>4.0</td><td>6.0</td><td>Edge</td><td></td></tr><tr><td>J BMVl+p</td><td>MT20</td><td>3.0</td><td>6.0</td><td></td><td></td></tr><tr><td colspan="6">K, N, O, P</td></tr><tr><td>K BMWW-l</td><td>MT20</td><td>5.0</td><td>6.0</td><td></td><td></td></tr><tr><td>L BMWWW-l</td><td>MT20</td><td>5.0</td><td>8.0</td><td></td><td></td></tr><tr><td>M BS-l</td><td>MT20</td><td>5.0</td><td>6.0</td><td></td><td></td></tr><tr><td>Q BMVl+p</td><td>MT20</td><td>3.0</td><td>6.0</td><td></td><td></td></tr></tbody></table> Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.						JT TYPE	PLATES	W	LEN	Y	X	B TMVW+p	MT20	5.0	6.0	Edge		C TMWW-l	MT20	4.0	4.0	2.00	1.50	D TTWW+m	MT20	5.0	6.0	2.00	1.50	E TMWW-l	MT20	4.0	4.0			F TMV+w	MT20	2.0	4.0			G TTWW+m	MT20	5.0	8.0	2.50	1.50	H TMVW+p	MT20	4.0	6.0	Edge		J BMVl+p	MT20	3.0	6.0			K, N, O, P						K BMWW-l	MT20	5.0	6.0			L BMWWW-l	MT20	5.0	8.0			M BS-l	MT20	5.0	6.0			Q BMVl+p	MT20	3.0	6.0		
JT TYPE	PLATES	W	LEN	Y	X																																																																																				
B TMVW+p	MT20	5.0	6.0	Edge																																																																																					
C TMWW-l	MT20	4.0	4.0	2.00	1.50																																																																																				
D TTWW+m	MT20	5.0	6.0	2.00	1.50																																																																																				
E TMWW-l	MT20	4.0	4.0																																																																																						
F TMV+w	MT20	2.0	4.0																																																																																						
G TTWW+m	MT20	5.0	8.0	2.50	1.50																																																																																				
H TMVW+p	MT20	4.0	6.0	Edge																																																																																					
J BMVl+p	MT20	3.0	6.0																																																																																						
K, N, O, P																																																																																									
K BMWW-l	MT20	5.0	6.0																																																																																						
L BMWWW-l	MT20	5.0	8.0																																																																																						
M BS-l	MT20	5.0	6.0																																																																																						
Q BMVl+p	MT20	3.0	6.0																																																																																						
 Structural component only DWG# T-2007611																																																																																									

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	T10	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Tue Apr 28 09:00:12 2020 Page 1
ID:DMCubINVR6TsiFoe31v6I ,zns11-4laLqmGihm3wdHrmERWRawUKh4GILip2QQJB8zMFIX



TOTAL WEIGHT = 118 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
N - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	6.0	Edge	
C TMVW-i	MT20	4.0	4.0	2.00	1.50
D TTWW+m	MT20	5.0	6.0	2.00	1.50
E TMVW-w	MT20	2.0	4.0		
F TTWW+m	MT20	5.0	6.0	2.00	1.50
G TMVW+p	MT20	4.0	6.0	Edge	
H BMV1+p	MT20	3.0	4.0		
I BMVW-i	MT20	4.0	6.0		
J BS-i	MT20	3.0	6.0		
K BMVWW-i	MT20	4.0	9.0		
L BMVW-i	MT20	4.0	4.0		
M BMVW-i	MT20	4.0	6.0	2.00	2.50
N BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
		GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
N	1550	0	1550	0	5-8
H	1424	0	1424	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1084	731	0	0	0	383	0
H	1007	661	0	0	0	346	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 35	M-C	-217 3
B-C	-1609 0	C-L	-273 0
C-D	-1425 0	L-O	0 280
D-E	-1272 0	D-K	0 185
E-F	-1272 0	K-E	-595 0
F-G	-1077 0	K-F	0 655
N-B	-1511 0	I-F	-390 0
H-G	-1380 0	B-M	0 1396
		I-G	0 1031
N-M	0 0		
M-L	0 1362		
L-K	0 1164		
K-J	0 890		
J-I	0 890		
I-H	0 0		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.43/1.00 (F-G:1), BC=0.27/1.00 (L-M:1).

WB=0.64/1.00 (E-K:1), SS=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1 10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

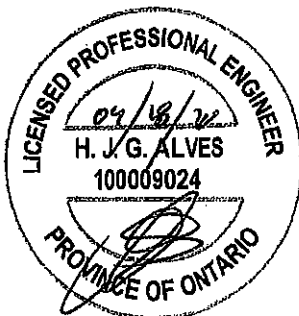
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.56 (G) (INPUT = 1.00)

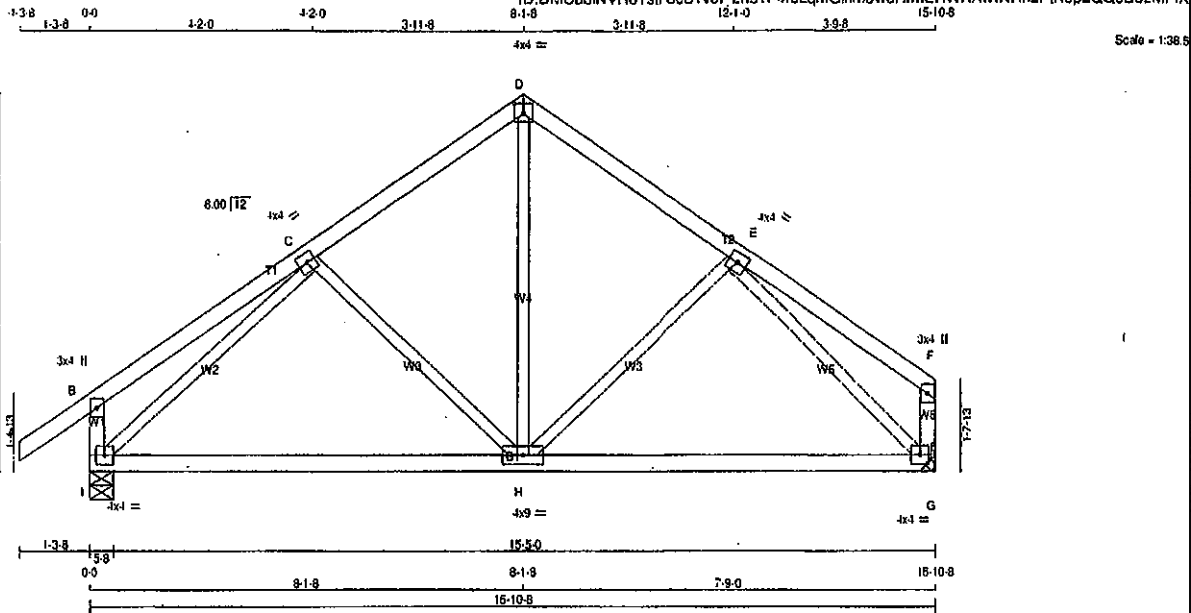


Structural component only
DWG# T-2007612.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408222	T11A	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Tue Apr 28 09:00:12 2020 Page 1



Scale = 1/32

TOTAL WEIGHT = 2 X 85 = 133 lb
(MIF)

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
C	TMVW-1	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-1	MT20	4.0	4.0		
F	TMV-p	MT20	3.0	4.0		
G	BMVW-1	MT20	4.0	4.0		
H	BMVW-1	MT20	4.0	4.0		
I	BMVW-1	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		RECORD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG	IN-SX	IN-SX
I	1001	0	1001	0	0	5-8			
G	875	0	875	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	706	478	0	0	0	0	229	0
G	619	406	0	0	0	0	213	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	MAX. CSI (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 35	-91.8 -91.8 0.12 (1)	10.00	C-H	-228 0	0.12 (1)	
B-C	0 24	-91.8 -91.8 0.24 (1)	10.00	H-D	0 449	0.10 (1)	
C-D	-691 0	-91.8 -91.8 0.19 (1)	6.25	H-E	-168 0	0.09 (1)	
D-E	-689 0	-91.8 -91.8 0.17 (1)	6.25	I-C	-988 0	0.60 (1)	
E-F	0 24	-91.8 -91.8 0.22 (1)	10.00	E-G	-971 0	0.44 (1)	
I-B	-289 0	0.0 0.0 0.03 (1)	7.81				
G-F	-125 0	0.0 0.0 0.01 (1)	7.81				
I-H	0 722	-18.5 -18.5 0.39 (4)	10.00				
H-G	0 677	-18.5 -18.5 0.39 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPC 2011, TPC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.24(1.00) (B-C:1), BC=0.39(1.00) (H-I:4), WB=0.50(1.00) (C-I:1), SS=0.15(1.00) (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (E) (INPUT = 0.90)
JSI METAL = 0.35 (C) (INPUT = 1.00)



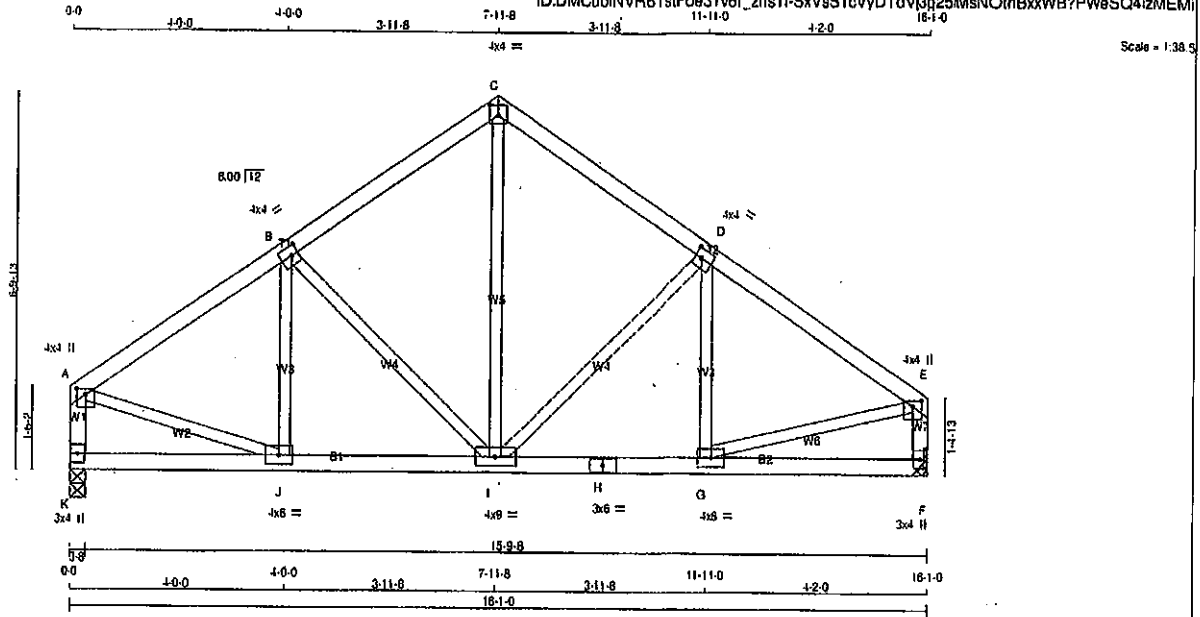
Structural component only
DWG# T-2007613

JOB NAME 408223	TRUSS NAME T11B	QUANTITY 2	PLV 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
--------------------	--------------------	---------------	----------	-------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:04:01 2020 Page 1

ID:DMCubINVR8TstFoe31v6l_zns1l-SxVsS1cVvDTdV3q25MsNoihBxxWB?PWsSQ4izMEMI



TOTAL WEIGHT = 2 X 69 = 137 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
K - A	2x4	DRY	No.2
F - E	2x4	DRY	No.2
K - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMVW-i	MT20	4.0	4.0	2.00	1.50
C	TTW+p	MT20	4.0	4.0	2.25	2.00
D	TMVW-i	MT20	4.0	4.0	2.00	1.50
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVW-i	MT20	4.0	6.0		
H	BS-i	MT20	3.0	6.0		
I	BMVW-i	MT20	4.0	9.0		
J	BMVW-i	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED		MAXIMUM FACTORED		INPUT	REQ'D	
	GROSS	REACTION	GROSS	REACTION	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	887	0	887	0	0	3-8	3-8
F	887	0	887	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	COMBINED	411	0	0	0	216	0
F	COMBINED	411	0	0	0	216	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH FR-TO				
A-B	-888 0	-91.8 -91.8	0.19 (1)	8.25 J-B	-142 18	0.04 (1)	
B-C	-687 0	-91.8 -91.8	0.18 (1)	8.25 B-I	-283 0	0.13 (1)	
C-D	-688 0	-91.8 -91.8	0.19 (1)	8.25 I-C	0 461	0.10 (1)	
D-E	-891 0	-91.8 -91.8	0.20 (1)	8.25 I-D	-292 0	0.15 (1)	
K-A	-855 0	0.0 0.0	0.09 (1)	7.81 G-D	-117 28	0.03 (1)	
F-E	-853 0	0.0 0.0	0.09 (1)	7.81 A-J	0 772	0.17 (1)	
				G-E	0 787	0.18 (1)	
K-J	0 0	-18.5 -18.5	0.07 (4)	10.00			
J-I	0 740	-18.5 -18.5	0.15 (1)	10.00			
I-H	0 761	-18.5 -18.5	0.15 (1)	10.00			
H-G	0 761	-18.5 -18.5	0.15 (1)	10.00			
G-F	0 0	-18.5 -18.5	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
OL = 6.0 PSF
BOT CH. LL = 0.0 PSF
OL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.20/1.00 (D-E:1), BC=0.15/1.00 (G-I:1),
WB=0.18/1.00 (E-G:1), SS=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

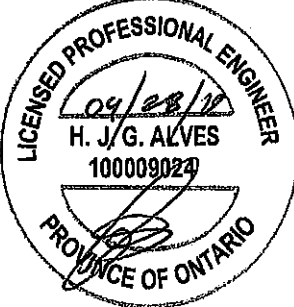
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

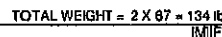
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.25 (H) (INPUT = 1.00)



Structural component only
DWG# T-2007631



ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

BEARINGS

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L. H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

LOADING
TOTAL LOAD CASES: 14}

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	25.6	PSF
	DL	6.0	PSF
BOT CH.	LL	0.0	PSF
	DL	7.4	PSF
TOTAL LOAD		39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9.
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN
LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF
LIVE LOAD

ALLOWABLE DEFL.(LL)= L 360 (0.54")
CALCULATED VERT. DEFL.(LL) = L 999 (0.03")
ALLOWABLE DEFL.(TL)= L 360 (0.54")
CALCULATED VERT. DEFL.(TL) = L 999 (0.08")

CSI: TC=0.42/1.00 (D-E:1) , BC=0.33/1.00 (I-J:1)
WB=0.47/1.00 (E-J:1) , SS=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

1 NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	789	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

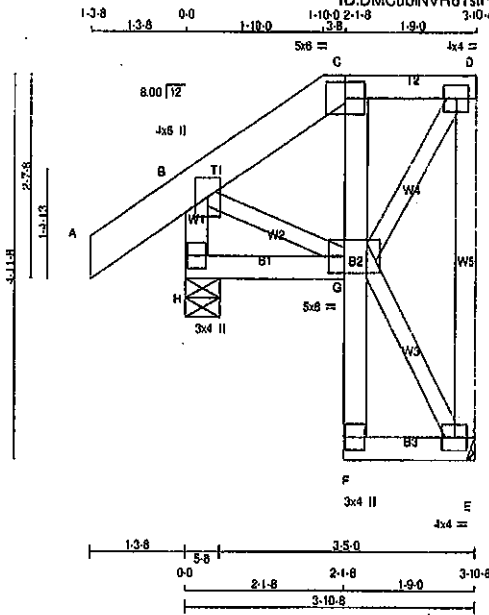
PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.47 (K) (INPUT = 1.00)



Structural component only
DWG# T-2007614

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	T12S	1	1	GREEN PARK HOMES	
Tamegack Roof Truss, Burlington		TRUSS DESC.		Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 23 09:00:14 2020 Page 1	



Scale = 1/2" = 1'-0"

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x6	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
F - C	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY, SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	6.0	
C	TTV-m	MT20	5.0	6.0	
D	TMVW-i	MT20	4.0	4.0	
E	BMVW1-t	MT20	4.0	4.0	
F	BMV+p	MT20	3.0	4.0	
G	BVMWW-i	MT20	5.0	8.0	2.50 2.50
H	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	JT	GROSS REACTION	BRG	BRG	BRG	BRG
	VERT		DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	214	0	214	0	0	MECHANICAL	
H	344	0	344	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1'-8".

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	151	99	0	0	0	0	0
H	241	172	0	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: 15)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	FORCE (LBS)	LC1 MAX CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 36	-91.8	-91.8 0.07 (5)	10.00	G-D	0 138 0.03 (1)	
B-C	-105 0	-91.8	-91.8 0.03 (1)	6.25	B-G	0 96 0.02 (1)	
C-D	-79 0	-91.8	-91.8 0.05 (1)	6.25	G-E	-3 0 0.00 (1)	
E-D	-198 0	0.0	0.0 0.08 (1)	7.81			
H-B	-323 0	0.0	0.0 0.03 (1)	7.81			
H-G	0 0	-18.5	-18.5 0.03 (4)	10.00			
F-G	0 17	0.0	0.0 0.01 (1)	10.00			
G-C	-119 0	0.0	0.0 0.01 (1)	7.81			
F-E	0 2	-18.5	-18.5 0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.08, 1.00 (D-E:1), BC=0.03, 1.00 (G-H:4), WB=0.03, 1.00 (D-G:1), SSI=0.07, 1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

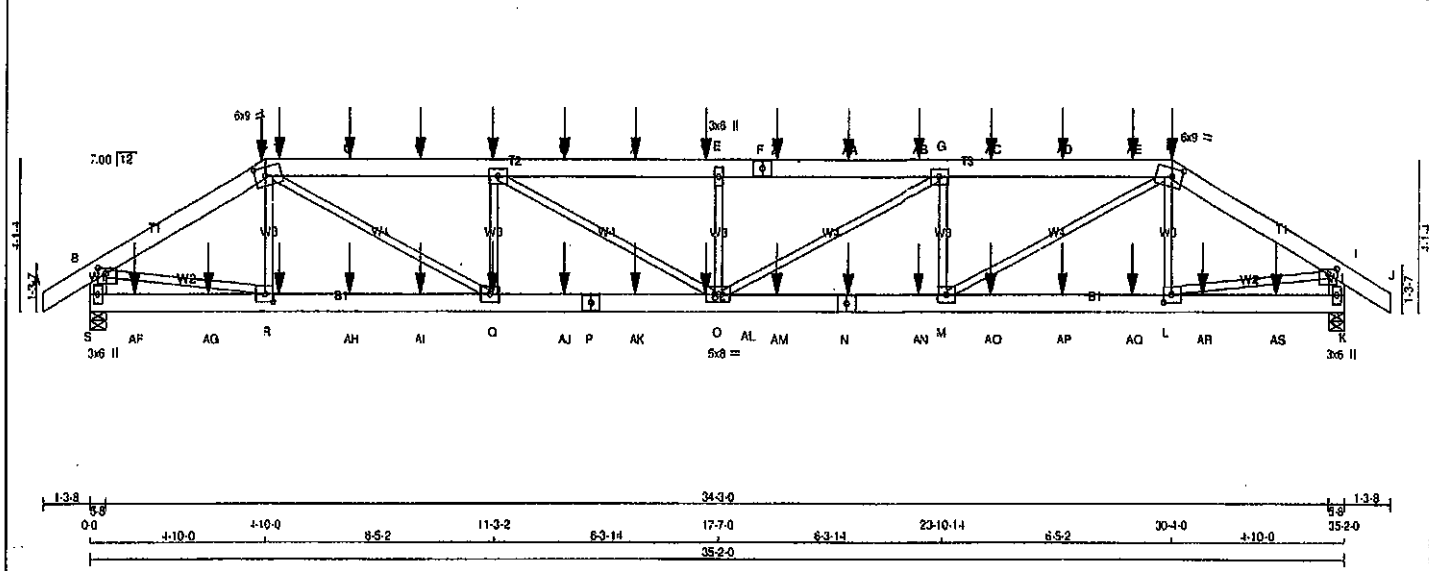
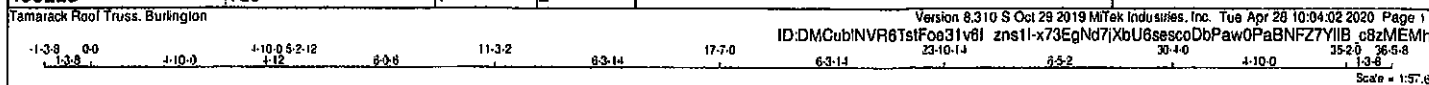
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.19 (B) INPUT = 0.90
JSI METAL = 0.11 (B) INPUT = 1.00



Structural component only
DWG# T-2007615

JOB NAME 408223	TRUSS NAME T20	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:04:02 2020 Page 1	



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2
C - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - J	2x6	DRY	No.2
S - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 2	12	SIDE (122.0)
C - F 2	12	SIDE (183.1)
F - H 2	12	SIDE (61.0)
H - J 2	12	SIDE (122.0)
S - B 2	12	TOP
K - I 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S - P 2	12	SIDE (183.1)
P - N 2	12	SIDE (0.0)
N - K 2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	8	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
S	3423	0	3423	0	5-8	5-8
K	3365	0	3365	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	2418	1600	0	0	0	818	0
K	2378	1573	0	0	0	804	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO	LENGTH	FR-TO		
A-B	0 33	-91.8 -91.8	0.04 (1)	10.00 R-C	-431 9	0.05 (1)
B-C	-4681 0	-91.8 -91.8	0.13 (1)	5.23 C-Q	0 3534	0.44 (1)
C-T	-7102 0	-91.8 -91.8	0.31 (1)	4.26 Q-D	-1555 0	0.19 (1)
T-U	-7102 0	-91.8 -91.8	0.31 (1)	4.26 D-O	0 1048	0.13 (1)
U-V	-7102 0	-91.8 -91.8	0.31 (1)	4.26 O-E	-864 0	0.10 (1)
V-D	-7102 0	-91.8 -91.8	0.31 (1)	4.26 O-G	0 1065	0.13 (1)
D-W	-8010 0	-91.8 -91.8	0.32 (1)	4.05 M-G	-1573 0	0.19 (1)
W-X	-8010 0	-91.8 -91.8	0.32 (1)	4.05 M-H	0 3572	0.44 (1)
X-Y	-8010 0	-91.8 -91.8	0.32 (1)	4.05 L-H	-421 10	0.05 (1)
Y-E	-8010 0	-91.8 -91.8	0.32 (1)	4.05 B-R	0 4088	0.51 (1)
E-F	-8010 0	-91.8 -91.8	0.33 (1)	4.03 L-I	0 -4022	0.50 (1)
F-Z	-8010 0	-91.8 -91.8	0.33 (1)			
Z-AA	-8010 0	-91.8 -91.8	0.33 (1)			
AA-AB	-8010 0	-91.8 -91.8	0.33 (1)			
AB-G	-8010 0	-91.8 -91.8	0.33 (1)			
G-AC	-7070 0	-91.8 -91.8	0.30 (1)			
AC-AD	-7070 0	-91.8 -91.8	0.30 (1)			
AD-AE	-7070 0	-91.8 -91.8	0.30 (1)			
AE-H	-7070 0	-91.8 -91.8	0.30 (1)			
H-I	-4608 0	-91.8 -91.8	0.13 (1)			
I-J	0 33	-91.8 -91.8	0.04 (1)	10.00		
S-B	-3367 0	0.0 0.0	0.12 (1)	7.68		
K-I	-3318 0	0.0 0.0	0.12 (1)	7.63		

S-AF	0	0	-18.5	-18.5	0.09 (4)	10.00
AF-AG	0	0	-18.5	-18.5	0.09 (4)	10.00
AG-R	0	0	-18.5	-18.5	0.09 (4)	10.00
R-AH	0	4024	-18.5	-18.5	0.31 (1)	10.00
AH-AI	0	4024	-18.5	-18.5	0.31 (1)	10.00
AI-O	0	4024	-18.5	-18.5	0.31 (1)	10.00
O-AJ	0	7102	-18.5	-18.5	0.52 (1)	10.00
AJ-P	0	7102	-18.5	-18.5	0.52 (1)	10.00
P-AK	0	7102	-18.5	-18.5	0.52 (1)	10.00
AK-AL	0	7102	-18.5	-18.5	0.52 (1)	10.00
AL-O	0	7102	-18.5	-18.5	0.52 (1)	10.00
O-AM	0	7070	-18.5	-18.5	0.52 (1)	10.00
AM-N	0	7070	-18.5	-18.5	0.52 (1)	10.00
N-AN	0	7070	-18.5	-18.5	0.52 (1)	10.00
AN-M	0	7070	-18.5	-18.5	0.52 (1)	10.00
M-AO	0	3960	-18.5	-18.5	0.30 (1)	10.00
AO-AP	0	3960	-18.5	-18.5	0.30 (1)	10.00
AP-AQ	0	3960	-18.5	-18.5	0.30 (1)	10.00
AQ-L	0	3960	-18.5	-18.5	0.30 (1)	10.00

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	PSF
LL	25.6	PSF
DL	6.0	PSF
BOT CH.	LL	PSF
LL	0.0	PSF
DL	7.4	PSF
TOTAL LOAD	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2018 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17')
CALCULATED VERT. DEFL.(LL) = L/999 (0.20')
ALLOWABLE DEFL.(TL) = L/360 (1.17')
CALCULATED VERT. DEFL.(TL) = L/999 (0.37')

CSI: TC=0.33/1.00 (E-G:1), BC=0.52/1.00 (O-Q:1), WB=0.51/1.00 (R-F:1), SSI=0.18/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HELLS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

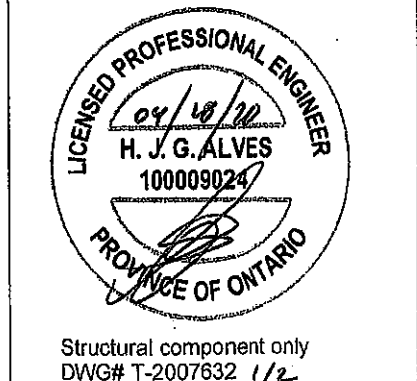
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	IPLI
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	616 354 1867 788	1987 1656	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (Q) INPUT = 0.90
JSI METAL = 0.88 (P) INPUT = 1.00



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T20	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mtek Industries, Inc. Tue Apr 28 10:04:02 2020 Page 2	
				ID:DMCubINVR6TsfFqg31v6I zns1l-x73EqNd7IXbU6sccDbPaw0PaBNFZ7YIIB c8zMEMh	

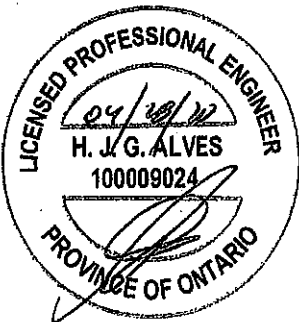
PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	8.0	1.75 2.75
C	TTWW-m	MT20	6.0	9.0	2.75 3.50
D	TMWW-l	MT20	5.0	8.0	
E	TMW-w	MT20	3.0	8.0	
F	TS-l	MT20	5.0	8.0	
G	TMWW-t	MT20	5.0	8.0	
H	TTWW-m	MT20	8.0	9.0	2.75 3.50
I	TMVW-p	MT20	5.0	8.0	1.75 2.75
K	BMV1+p	MT20	3.0	8.0	
L	BMWW-t	MT20	5.0	8.0	2.50 2.75
M	BMWW-t	MT20	5.0	8.0	
N	BS-l	MT20	5.0	8.0	
O	BMWWW-l	MT20	5.0	8.0	
P	BS-l	MT20	5.0	8.0	
Q	BMWW-t	MT20	5.0	8.0	
R	BMWW-t	MT20	5.0	8.0	2.50 2.75
S	BMV1+p	MT20	3.0	8.0	

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MEMB.	MAX. FACTORED FORCE (LBS)	LC1	MAX	UNBRAG
FR-TO					FR-TO				
L-AR	0 0			-18.5	-18.5	0.08 (4)		10.00	
AR-AS	0 0			-18.5	-18.5	0.08 (4)		10.00	
AS-K	0 0			-18.5	-18.5	0.08 (4)		10.00	

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-10-0	-60	-67	---	FRONT	VERT	DEAD	---	C1
C	4-10-0	-311	-311	---	FRONT	VERT	SNOW	---	C1
D	11-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
H	30-4-0	-60	-67	---	FRONT	VERT	DEAD	---	C1
H	30-4-0	-311	-311	---	FRONT	VERT	SNOW	---	C1
N	21-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
Q	11-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
R	5-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
T	5-2-12	-138	-138	---	FRONT	VERT	TOTAL	---	C1
U	7-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	9-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	13-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	15-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	17-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	19-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	21-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	23-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	25-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	27-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AE	29-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AF	1-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AG	3-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AH	7-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AI	9-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AJ	13-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AK	15-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AL	17-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AM	19-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AN	23-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AO	25-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AP	27-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AO	29-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AR	31-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AS	33-2-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

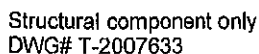


Structural component only
DWG# T-2007632

DRWG NO.

TRUSS DESC.

Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Tue Apr 28 10:04:03 2020 Page 1
ID:DMCubINVR6TstFoe31v6I znsII-PKdctleUqILk0D3AWlqx0T6J Wk xSi vxX9bzMEMr



Tamarack Roof Truss, Burlington

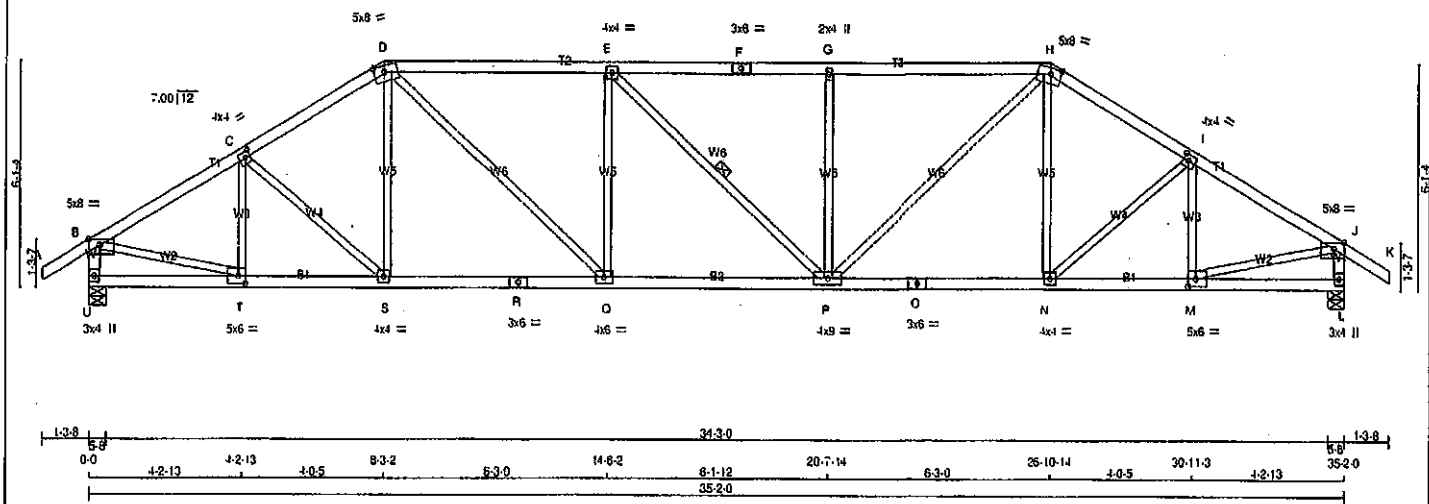
Version 8.310 S Oct 29 2019 MITek Industries, Inc. Tue Apr 28 10:04:04 2020 Page 1

ID:DMCubiNVR6tstFoe31v8i zns11-IWB743eNF8rCMAoFjDG3U?70GFQhTjrCcg4h1zMEM

1-3.8 0-0 4-2.13 4-2.13 3-3.2 14-6.2 20-7.14 26-10.14 30-11.3 4-2.13

1-3.8 4-2.13 4-0.5 6-3.0 6-1.12 4-3.0 4-0.5 1-3.8

Scale = 1:57.6



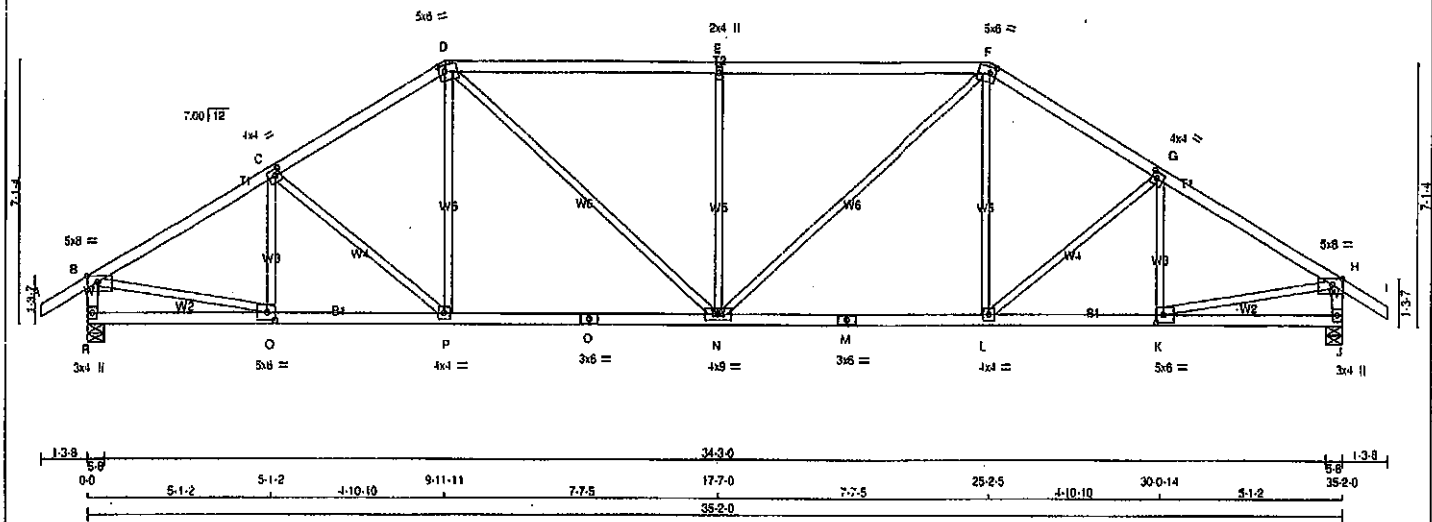
UT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	8.0	Edge	3.50
C	TMWV-t	MT20	4.0	4.0	2.00	1.75
D	TTWV-m	MT20	5.0	8.0	2.00	3.50
E	TMWV-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWV-w	MT20	2.0	4.0		
H	TTWV-m	MT20	5.0	8.0	2.00	3.50
I	TMWV-t	MT20	4.0	4.0	Edge	1.75
J	TMWV-p	MT20	5.0	8.0	Edge	3.50
L	BMV-t-p	MT20	3.0	4.0		
M	BMWV-t	MT20	5.0	6.0	2.50	2.50
N	BMWV-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWVW-t	MT20	4.0	9.0		
Q	BMWV-t	MT20	4.0	6.0		
R	BS-t	MT20	3.0	6.0		
S	BMWV-t	MT20	4.0	4.0		
T	BMWV-t	MT20	5.0	6.0	2.50	2.50
U	BMV-t-p	MT20	3.0	4.0		

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 (PLF)	MAX CSH (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSH (LC)
FR-TO		FROM	TO		FR-TO			
A-B	0 32	-91.8	-91.8	0.12 (1)	10.00	T-C	-428 0	0.10 (11)
S-C	-2468 0	-91.8	-91.8	0.35 (1)	4.07	C-S	-39 0	0.02 (11)
C-D	-2478 0	-91.8	-91.8	0.34 (1)	4.08	S-D	0 131	0.04 (4)
D-E	-2910 0	-91.8	-91.8	0.75 (1)	3.37	D-Q	0 1076	0.24 (11)
E-F	-2809 0	-91.8	-91.8	0.74 (1)	3.38	Q-E	-617 0	0.36 (11)
F-G	-2909 0	-91.8	-91.8	0.74 (1)	3.38	E-P	-2 0	0.00 (11)
G-H	-2909 0	-91.8	-91.8	0.75 (1)	3.38	P-G	-617 0	0.36 (11)
H-I	-2478 0	-91.8	-91.8	0.34 (1)	4.08	P-H	0 1073	0.24 (11)
I-J	-2478 0	-91.8	-91.8	0.35 (1)	4.07	N-H	0 132	0.04 (4)
J-K	0 32	-91.8	-91.8	0.12 (1)	10.00	N-I	-39 0	0.02 (11)
U-B	-2025 0	0.0	0.0	0.21 (1)	5.94	M-I	-428 0	0.10 (11)
L-J	-2025 0	0.0	0.0	0.21 (1)	5.94	B-T	0 2205	0.50 (11)
						M-J	0 2205	0.50 (11)
U-T	0 0	-18.5	-18.5	0.07 (4)	10.00			
T-S	0 2149	-18.5	-18.5	0.41 (1)	10.00			
S-R	0 2122	-18.5	-18.5	0.41 (1)	10.00			
R-Q	0 2122	-18.5	-18.5	0.41 (1)	10.00			
Q-P	0 2910	-18.5	-18.5	0.54 (1)	10.00			
P-O	0 2122	-18.5	-18.5	0.41 (1)	10.00			
O-N	0 2122	-18.5	-18.5	0.41 (1)	10.00			
N-M	0 2149	-18.5	-18.5	0.41 (1)	10.00			
M-L	0 0	-18.5	-18.5	0.07 (4)	10.00			

JSI GRIP= 0.89 (T) ; INPUT = 0.90 ;
JSI METAL= 0.70 (O) ; INPUT = 1.00 ;

Structural component only
DWG# T-2007634

JOB NAME 408223	TRUSS NAME T23	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Tue Apr 28 10:04:06 2020 Page 1			
		ID:DMCubINVR6TstFoe31v6l zns1l-pvJIVkgem/SwbUyarelXZQ5aqCaVBJGBgw9BvzMEMd			
		Scale = 1:57.6			



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
R - B	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
R - O	2x4	DRY No.2	SPF
O - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	8.0	Edge 3.50
D	TMVW-t	MT20	4.0	4.0	2.00 1.75
C	TTVW-m	MT20	5.0	8.0	2.00 2.00
E	TMVW-w	MT20	2.0	4.0	
F	TTVW-m	MT20	5.0	8.0	2.00 2.00
G	TMVW-t	MT20	4.0	4.0	2.00 1.75
H	TMVW-p	MT20	5.0	8.0	Edge 3.50
J	BMV1-p	MT20	3.0	4.0	
K	BMVW-t	MT20	5.0	8.0	2.50 2.50
L	BMVW-t	MT20	4.0	4.0	
M	BS-t	MT20	3.0	8.0	
N	BMVW-t	MT20	4.0	4.0	
O	BS-t	MT20	3.0	8.0	
P	BMVW-t	MT20	4.0	4.0	
Q	BMVW-t	MT20	5.0	8.0	2.50 2.50
R	BMV1-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
<u>BEARINGS</u>									
	FACTORED		MAXIMUM FACTORED		INPUT	REQRD			
	GROSS REACTION		GROSS REACTION		BRG	BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
R	2064	0	2064	0	0	5-8	5-8		
J	2064	0	2064	0	0	5-8	5-8		
<u>UNFACTORED REACTIONS</u>									
	1ST CASE	MAX. MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
R	1457	970.0	0.0	0.0	0.0	488.0	0.0		
J	1457	970.0	0.0	0.0	0.0	488.0	0.0		
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J									
<u>BRACING</u>									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.16 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 32	-91.8 -91.8 0.12 (1)	10.00	Q-C	-335 0	0.09 (1)	
B-C	-2537 0	-91.8 -91.8 0.39 (1)	4.00	C-P	-218 0	0.16 (1)	
C-D	-2358 0	-91.8 -91.8 0.37 (1)	4.11	P-D	0 258	0.08 (4)	
D-E	-2609 0	-91.8 -91.8 0.88 (1)	3.18	D-W	0 752	0.17 (1)	
E-F	-2609 0	-91.8 -91.8 0.88 (1)	3.16	N-E	-960 0	0.75 (1)	
F-G	-2358 0	-91.8 -91.8 0.37 (1)	4.11	N-F	0 752	0.17 (1)	
G-H	-2537 0	-91.8 -91.8 0.39 (1)	4.00	L-F	0 258	0.08 (4)	
H-I	0 32	-91.8 -91.8 0.12 (1)	10.00	L-G	-218 0	0.16 (1)	
R-B	-2020 0	0.0 0.0 0.21 (1)	5.94	K-G	-335 0	0.09 (1)	
J-H	-2020 0	0.0 0.0 0.21 (1)	5.94	B-Q	0 2253	0.51 (1)	
				K-H	0 2253	0.51 (1)	
R-Q	0 0	-18.5 -18.5 0.10 (4)	10.00				
Q-P	0 2213	-18.5 -18.5 0.45 (1)	10.00				
P-O	0 2048	-18.5 -18.5 0.44 (1)	10.00				
O-N	0 2048	-18.5 -18.5 0.44 (1)	10.00				
N-M	0 2048	-18.5 -18.5 0.44 (1)	10.00				
M-L	0 2048	-18.5 -18.5 0.44 (1)	10.00				
L-K	0 2213	-18.5 -18.5 0.45 (1)	10.00				
K-J	0 0	-18.5 -18.5 0.10 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
OL = 6.0 PSF
BOT CH. LL = 0.0 PSF
OL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./G

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.27")

CSI: TC=0.88/1.00 (E-F:1), BC=0.45/1.00 (K-L:1), WB=0.75/1.00 (E-N:1), SS=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 L/S BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 364 1887 768 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

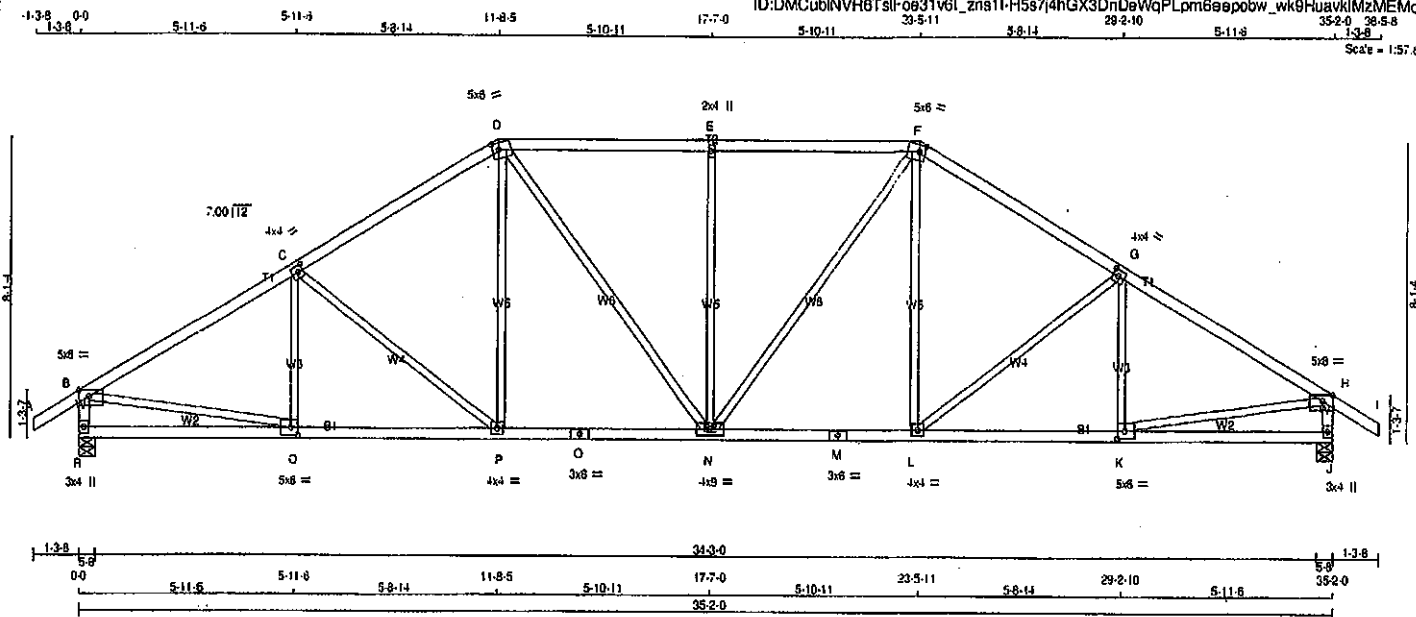
JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.72 (M) (INPUT = 1.00)



Structural component only
DWG# T-2007635

JOB NAME 408223	TRUSS NAME T24	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Tue Apr 28 10:04:07 2020 Page 1
ID:DMCubINVR8TslF0e31v6L_zns11-H5s74hGX3DnDeWqPLpm6eapow_wk9HuaavkMzMEMC
352-0 35-5-8
Scale = 1:57.6



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
H - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN V X
B	TMVW-p	MT20	5.0	8.0 Edge 3.50
C	TMVW-l	MT20	4.0	4.0 2.00 1.75
D	TTWW-m	MT20	5.0	6.0 2.25 2.00
E	TMVW-w	MT20	2.0	4.0
F	TTWW-m	MT20	5.0	6.0 2.25 2.00
G	TMVW-l	MT20	4.0	4.0 2.00 1.75
H	TMVW-p	MT20	5.0	8.0 Edge 3.50
J	BMV1-p	MT20	3.0	4.0
K	BMVW-l	MT20	5.0	6.0 2.50 2.50
L	BMVW-l	MT20	4.0	4.0
M	BS-l	MT20	3.0	6.0
N	BMVW-l	MT20	4.0	9.0
O	BS-l	MT20	3.0	6.0
P	BMVW-l	MT20	4.0	4.0
Q	BMVW-l	MT20	5.0	6.0 2.50 2.50
R	BMV1-p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
	FACTORED	MAXIMUM FACTORED	INPUT
	GROSS REACTION	GROSS REACTION	BRG
JT	VERT	HORZ	DOWN
R	2084	0	2084
J	2084	0	2084

UNFACTORED REACTIONS			
1ST LCASE	MAX. MIN. COMPONENT REACTIONS		
JT	COMBINED	SNOW	LIVE
R	1457	970	0
J	1457	970	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)
FR-TO			
A-B	0	32	-91.8 -91.8 0.12 (1)
B-C	-2575	0	-91.8 -91.8 0.54 (1)
C-D	-2283	0	-91.8 -91.8 0.50 (1)
D-E	-2236	0	-91.8 -91.8 0.49 (1)
E-F	-2236	0	-91.8 -91.8 0.49 (1)
F-G	-2283	0	-91.8 -91.8 0.50 (1)
G-H	-2575	0	-91.8 -91.8 0.54 (1)
H-I	0	32	-91.8 -91.8 0.12 (1)
I-J	-2016	0	0.0 0.0 0.21 (1)
J-H	-2016	0	0.0 0.0 0.21 (1)
R-Q	0	0	-18.5 -18.5 0.14 (4)
Q-P	0	2251	-18.5 -18.5 0.43 (1)
P-O	0	1947	-18.5 -18.5 0.38 (1)
O-N	0	1947	-18.5 -18.5 0.38 (1)
N-M	0	1947	-18.5 -18.5 0.38 (1)
M-L	0	1947	-18.5 -18.5 0.38 (1)
L-K	0	2251	-18.5 -18.5 0.43 (1)
K-J	0	0	-18.5 -18.5 0.14 (4)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2018 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.54/1.00 (B-C); SC=0.43/1.00 (P-Q);
WB=0.84/1.00 (E-N); SS=0.26/1.00 (D-E)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 610 354 1067 768 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

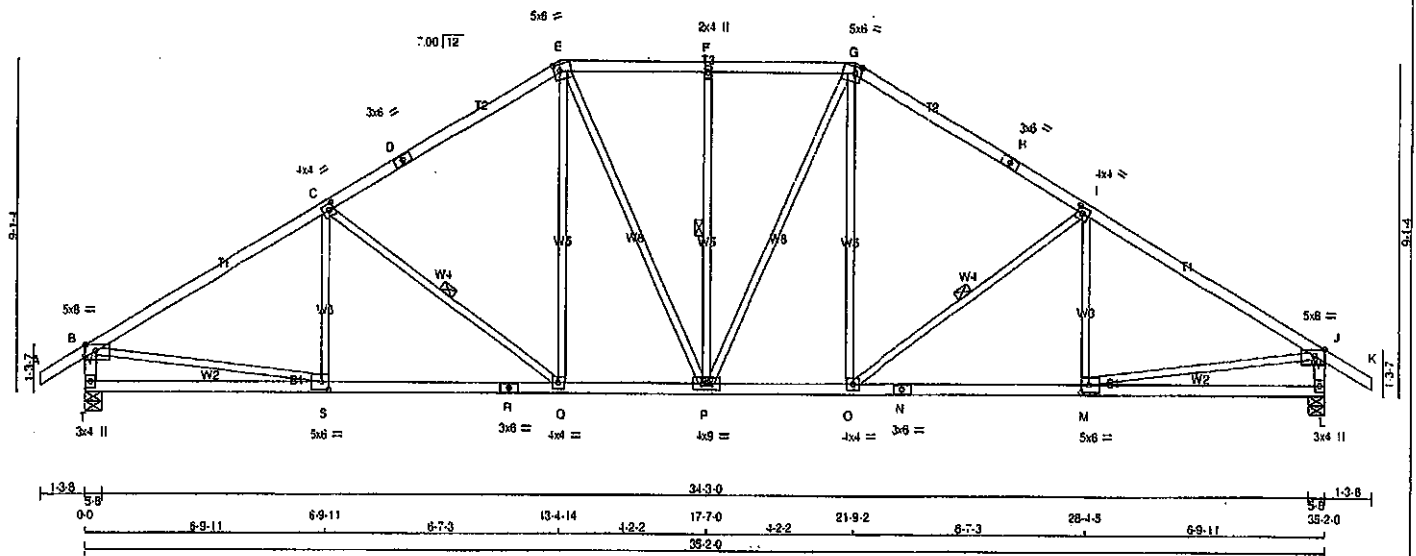
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (Q) (INPUT = 0.90)
JSI METAL= 0.61 (M) (INPUT = 1.00)



Structural component only
DWG# T-2007636

JOB NAME 408223	TRUSS NAME T25	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Tue Apr 28 10:04:08 2020 Page 1	
ID:DMCubINVR6Ts1Foe31v6I zns11HQVwQhulNMdm50y3K7erAxP?FIGSR7DeIqozMEMb				35-2-0 36-5-8	
1-3-8 0-0 6-9-11 6-9-11 6-7-3 13-4-14 1-2-2 17-7-0 1-2-2 21-9-2 6-7-3 28-4-5 6-9-11 1-3-8				Scale = 1:50.3	



LUMBER			
N. L. Q. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
T - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
T - R	2x4	DRY	No.2
R - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMWW-p	MT20	5.0	8.0 Edge 3.50
C	TMWW-I	MT20	4.0	4.0 2.00 1.75
D	TS-I	MT20	3.0	6.0
E	TTWW-m	MT20	5.0	6.0 2.25 2.00
F	TMWW-w	MT20	2.0	4.0
G	TTWW-m	MT20	5.0	6.0 2.25 2.00
H	TS-I	MT20	3.0	6.0
I	TMWW-I	MT20	4.0	4.0 2.00 1.75
J	TMWW-p	MT20	5.0	8.0 Edge 3.50
L	BMV1-p	MT20	3.0	4.0
M	BMWW-I	MT20	5.0	6.0 2.50 2.50
N	BS-I	MT20	3.0	6.0
O	BMWW-I	MT20	4.0	4.0
P	BMWW-I	MT20	4.0	4.0
Q	BMWW-I	MT20	4.0	4.0
R	BS-I	MT20	3.0	6.0
S	BMWW-I	MT20	5.0	6.0 2.50 2.50
T	BMV1-p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER								
<u>BEARINGS</u>								
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
T	2084	0	2084	0	0	5-8	5-8	
L	2084	0	2084	0	0	5-8	5-8	
<u>UNFACTORED REACTIONS</u>								
	1ST LCASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
T	1457	970	0	0	0	0	488	0
L	1457	970	0	0	0	0	488	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L.
BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.54 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-Q, F-P, I-O.
 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS									
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORZ. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)
FR-TO						FR-TO			
A-B	0 32	-91.8	-91.8	0.12 (1)	10.00	S-C	-185 51	0.08 (1)	10.00
B-C	-2590 0	-91.8	-91.8	0.74 (1)	3.54	C-Q	-548 0	0.26 (1)	10.00
C-D	-2158 0	-91.8	-91.8	0.68 (1)	3.91	Q-E	0 431	0.10 (1)	10.00
D-E	-2158 0	-91.8	-91.8	0.68 (1)	3.91	E-P	0 288	0.08 (1)	10.00
E-F	-1958 0	-91.8	-91.8	0.24 (1)	4.61	P-F	-460 0	0.25 (1)	10.00
F-G	-1958 0	-91.8	-91.8	0.24 (1)	4.61	G-G	0 288	0.08 (1)	10.00
G-H	-2158 0	-91.8	-91.8	0.68 (1)	3.91	O-G	0 431	0.10 (1)	10.00
H-I	-2158 0	-91.8	-91.8	0.68 (1)	3.91	O-I	-548 0	0.26 (1)	10.00
I-J	-2590 0	-91.8	-91.8	0.74 (1)	3.54	M-I	-185 51	0.08 (1)	10.00
J-K	0 32	-91.8	-91.8	0.12 (1)	10.00	B-S	0 2292	0.52 (1)	10.00
T-B	-2011 0	0.0	0.0	0.20 (1)	5.95	M-J	0 2292	0.52 (1)	10.00
L-J	-2011 0	0.0	0.0	0.20 (1)	5.95				
T-S	0 0	-18.5	-18.5	0.20 (4)	10.00				
S-R	0 2269	-18.5	-18.5	0.48 (1)	10.00				
R-Q	0 2269	-18.5	-18.5	0.48 (1)	10.00				
Q-P	0 1833	-18.5	-18.5	0.36 (1)	10.00				
P-O	0 1833	-18.5	-18.5	0.36 (1)	10.00				
O-N	0 2269	-18.5	-18.5	0.48 (1)	10.00				
N-M	0 2269	-18.5	-18.5	0.48 (1)	10.00				
M-L	0 0	-18.5	-18.5	0.20 (4)	10.00				

TOTAL WEIGHT = 4 X 158 = 632 lb

[M/F]

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, OBC 2012, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

55 % OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 28.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
 ALLOWABLE DEFL.(TL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.74 (1.00 (I-J:1)) . BC=0.46 (1.00 (M-O:1))
 WB=0.52 (1.00 (J-M:1)) . SSI=0.26 (1.00 (I-J:1))

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1657 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

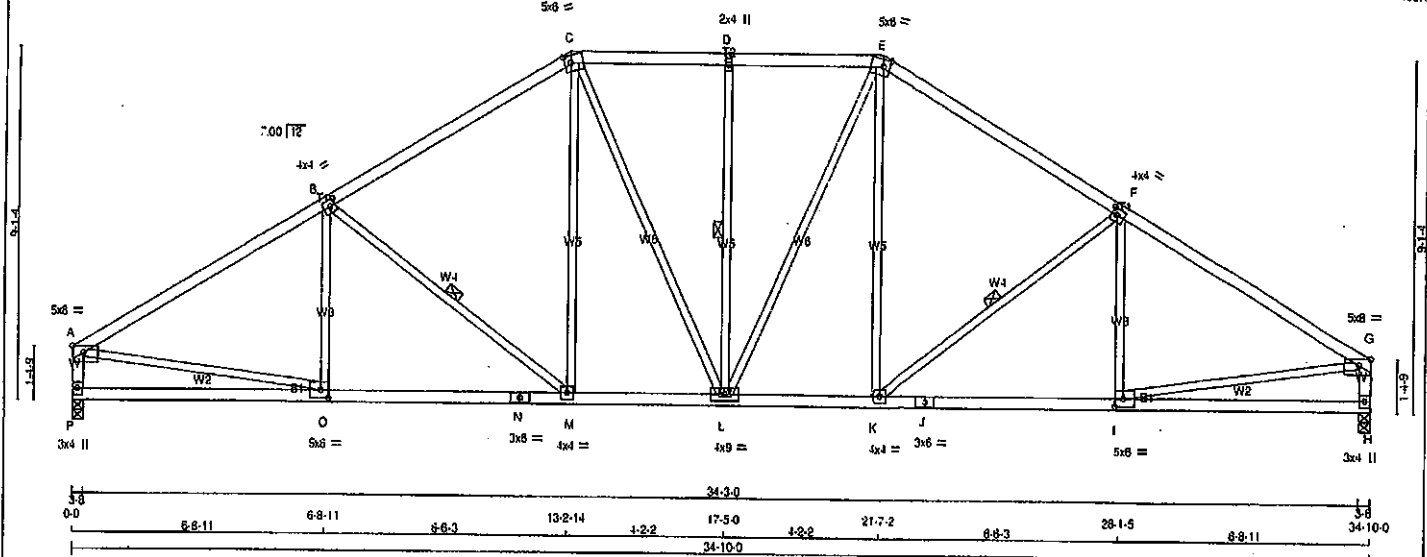
JSI GRIP= 0.85 (S) INPUT = 0.90)
 JSI METAL= 0.69 (IN) INPUT = 1.00)



Structural component only
 DWG# T-2007637

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T25A	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Tue Apr 28 10:04:09 2020 Page 1
 ID:DMCubINVR6Ts[Foe31v6I zns1I-DU_u8miW3gUUSXgCWmsEB3i6iPbBojwaM(OrMeZMEMa
 0.0 6-8.11 6-8.11 6-8.3 13-2.14 4-2.2 17-5.0 21-7.2 8-8.3 28-1.5 8-8.11 34-10.0
 Scale = 1:55.1



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
P - A	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
P - N	2x4	DRY	No.2	SPF	
N - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2
 EXCEPT
 DRY: SEASONED LUMBER.

PLATES (tables in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	2.00	Edge
B	TMVW-i	MT20	4.0	4.0	2.00	1.75
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMVW-w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	2.25	2.00
F	TMVW-i	MT20	4.0	4.0	2.00	1.75
G	TMVW-p	MT20	5.0	8.0	Edge	3.50
H	BMV1-p	MT20	3.0	4.0		
I	BMVW-i	MT20	5.0	6.0	2.50	2.50
J	BS-i	MT20	3.0	6.0		
K	BMVW-i	MT20	4.0	4.0		
L	BMVW-w	MT20	4.0	4.0	9.0	
M	BMVW-i	MT20	4.0	4.0		
N	BS-i	MT20	3.0	6.0		
O	BMVW-i	MT20	5.0	6.0	2.50	2.50
P	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT	IN-SX
P	1920	0	1920	0
H	1920	0	1920	0

UNFACTORED REACTIONS							
	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT							
P	1358	891 0	0 0	0 0	0 0	467 0	0 0
H	1358	891 0	0 0	0 0	0 0	467 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, H

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-M, D-L, F-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. GSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. GSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	-2510 0	-91.8 -91.8 0.71 (1)	3.84	O-B	-212 40	0.09 (1)			
B-C	-2113 0	-91.8 -91.8 0.63 (1)	3.89	B-M	-510 0	0.24 (1)			
C-D	-1918 0	-91.8 -91.8 0.24 (1)	4.64	M-C	0 412	0.09 (1)			
D-E	-1919 0	-91.8 -91.8 0.24 (1)	4.64	C-L	0 287	0.06 (1)			
E-F	-2113 0	-91.8 -91.8 0.63 (1)	3.99	L-D	-481 0	0.25 (1)			
F-G	-2510 0	-91.8 -91.8 0.71 (1)	3.84	L-E	0 287	0.06 (1)			
P-A	-1888 0	0.0 0.0 0.19 (1)	6.13	K-E	0 412	0.09 (1)			
H-G	-1888 0	0.0 0.0 0.19 (1)	6.13	K-F	-510 0	0.24 (1)			
				I-F	-212 40	0.09 (1)			
P-O	0 0	-18.5 -18.5 0.20 (4)	10.00	A-O	0 2227	0.50 (1)			
O-N	0 2199	-18.5 -18.5 0.45 (1)	10.00	I-G	0 2227	0.50 (1)			
N-M	0 2199	-18.5 -18.5 0.45 (1)	10.00						
M-L	0 1797	-18.5 -18.5 0.35 (1)	10.00						
L-K	0 1797	-18.5 -18.5 0.35 (1)	10.00						
K-J	0 2199	-18.5 -18.5 0.45 (1)	10.00						
J-I	0 2199	-18.5 -18.5 0.45 (1)	10.00						
I-H	0 0	-18.5 -18.5 0.20 (4)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, CBC 2012, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.16")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
 ALLOWABLE DEFL.(TL) = L/360 (1.16")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.71 1.00 (A-B:1), BC=0.45 1.00 (M-O:1), WB=0.50 1.00 (A-O:1), SSI=0.28 1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1867 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (I) INPUT = 0.90
 JSI METAL = 0.66 (N) INPUT = 1.00



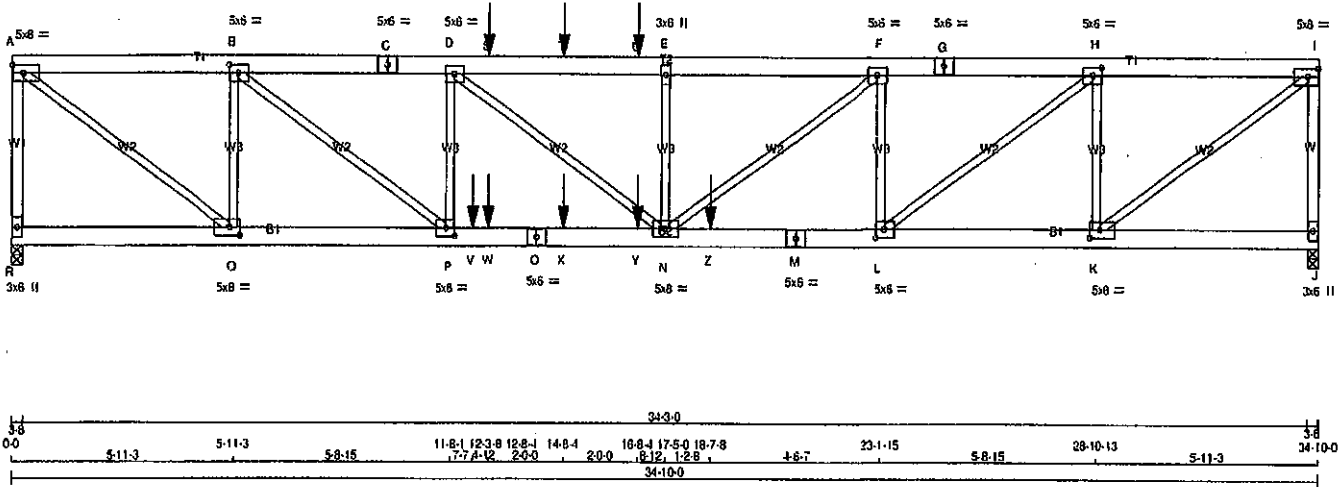
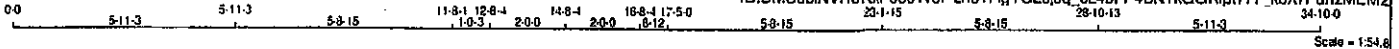
Structural component only
 DWG# T-2007638

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408223	T28	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 9 Oct 2019 MTEK Industries, Inc. Tue Apr 28 10:04:10 2020 Page 1

ID:DMCubINVR6TsFoe31v6l zns1l-igYGL68q_cL45FP4UNTKGGKpt777_kbX7PuhzMEMZ



TOTAL WEIGHT = 2 X 183 = 366 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
R - A	2x4	DRY	No.2
A - C	2x6	DRY	No.2
C - G	2x6	DRY	No.2
G - I	2x6	DRY	No.2
I - J	2x4	DRY	No.2
R - O	2x6	DRY	No.2
O - M	2x6	DRY	No.2
M - J	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY; SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122'X3') SPIRAL NAILS		
R - A	12	TOP
I - J	12	TOP
A - C	12	TOP
C - G	12	TOP
G - I	12	TOP
BOTTOM CHORDS: (0.122'X3') SPIRAL NAILS		
R - O	12	SIDE(0.0)
O - M	12	SIDE(0.0)
M - J	12	TOP
WEBS: (0.122'X3') SPIRAL NAILS		
2x3	8	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED	MAXIMUM FACTORED	INPUT	REQD	
	GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
R	3373	0	3373	0	0
J	3103	0	3103	0	0

UNFACTORED REACTIONS					
1ST CASE	MAX. MIN.	COMPONENT	REACTIONS		
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
R	2380	1591	0	0	0
J	2190	1459	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS						
MEMB.	MAX. FACTORED	FACTORED	MAX.	MEMB.	MAX. FACTORED	FACTORED	MAX.			
	FORCE	VERT. LOAD	CSH (LC)		FORCE	CSH (LC)				
	(LBS)	(LBS)	(LBS)		(LBS)	(LBS)				
FR-TO		FROM TO	LENGTH	FR-TO						
R-A	-3316	0	0.0	0.58 (1)	8.39	0	4719			
A-B	-4149	0	-91.8	-91.8	0.14 (1)	5.47	A-Q	0	5171	0.84 (1)
B-C	-7288	0	-91.8	-91.8	0.20 (1)	4.33	K-H	-2757	0	0.44 (1)
C-D	-7288	0	-91.8	-91.8	0.20 (1)	4.33	Q-B	-2997	0	0.48 (1)
D-S	-8278	0	-91.8	-91.8	0.28 (1)	4.03	L-H	0	3530	0.44 (1)
S-T	-8278	0	-91.8	-91.8	0.28 (1)	4.03	B-P	0	3958	0.49 (1)
T-U	-8278	0	-91.8	-91.8	0.28 (1)	4.03	L-P	-1791	0	0.28 (1)
U-E	-8278	0	-91.8	-91.8	0.28 (1)	4.03	P-D	-1403	0	0.22 (1)
E-F	-8278	0	-91.8	-91.8	0.21 (1)	4.11	N-F	0	2134	0.26 (1)
F-G	-8588	0	-91.8	-91.8	0.19 (1)	4.52	D-N	0	1249	0.15 (1)
G-H	-8588	0	-91.8	-91.8	0.19 (1)	4.52	N-E	-673	0	0.11 (1)
H-I	-3787	0	-91.8	-91.8	0.13 (1)	5.88				
J-I	-3042	0	0.0	0.0	0.51 (1)	6.82				

R-Q	0	0	-18.5	-18.5	0.04 (4)	10.00
Q-P	0	-4149	-18.5	-18.5	0.30 (1)	10.00
P-V	0	7288	-18.5	-18.5	0.67 (1)	10.00
V-W	0	7288	-18.5	-18.5	0.67 (1)	10.00
W-O	0	7288	-18.5	-18.5	0.67 (1)	10.00
O-X	0	7288	-18.5	-18.5	0.67 (1)	10.00
X-Y	0	7288	-18.5	-18.5	0.67 (1)	10.00
Y-N	0	7288	-18.5	-18.5	0.67 (1)	10.00
N-Z	0	6588	-18.5	-18.5	0.72 (1)	10.00
Z-M	0	6588	-18.5	-18.5	0.72 (1)	10.00
M-L	0	6588	-18.5	-18.5	0.72 (1)	10.00
L-K	0	3787	-18.5	-18.5	0.32 (1)	10.00
K-J	0	0	-18.5	-18.5	0.04 (4)	10.00

FACTORED CONCENTRATED LOADS (LBS)					
JT	LOC.	LC1	MAX.	FACE	DIR.
S	12-8-4	-86	-86	---	BACK
T	14-8-4	-86	-86	---	BACK
U	16-8-4	-86	-86	---	BACK
V	12-3-8	-1052	-1052	---	BACK
W	12-8-4	-17	-17	---	BACK
X	14-8-4	-17	-17	---	BACK
Y	16-8-4	-17	-17	---	BACK
Z	18-7-8	-1273	-1273	---	BACK

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(56 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.18")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.18")
CALCULATED VERT. DEFL.(TL) = L/999 (0.39")

CSI: TC=0.58/1.00 (A-R:1), BC=0.72/1.00 (L-N:1), WB=0.64/1.00 (A-Q:1), SS=0.37/1.00 (L-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1967 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) INPUT = 0.99 I
JSI METAL= 0.73 (M) INPUT = 1.00 I



Structural component only
DWG# T-2007639 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T28	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.310 S Oct 20 2019 MTek Industries, Inc. Tue Apr 28 10:04:10 2020 Page 2

ID:DMCubINV6TsIF0e31v6I zns11-igYGL68g cL45FP4UNTkGGK1pt777 kbX7PuhzMEMZ

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-I	MT20	5.0	8.0	2.50	3.25
B	TMWW-I	MT20	5.0	8.0	2.50	2.75
C	TS-I	MT20	5.0	8.0		
D	TMWW-I	MT20	5.0	8.0		
E	TMW+W	MT20	3.0	6.0		
F	TMWW-I	MT20	5.0	6.0		
G	TS-I	MT20	5.0	8.0		
H	TMWW-I	MT20	5.0	6.0	2.50	2.75
I	TMVW-I	MT20	5.0	8.0	2.50	3.25
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-I	MT20	5.0	8.0	2.50	3.25
L	BMWW-I	MT20	5.0	6.0	2.50	2.75
M	BS-I	MT20	5.0	8.0		
N	BMWWW-I	MT20	5.0	8.0		
O	BS-I	MT20	5.0	8.0		
P	BMWW-I	MT20	5.0	6.0	2.50	2.75
Q	BMWW-I	MT20	5.0	8.0	2.50	3.25
R	BMV1+p	MT20	3.0	6.0		

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

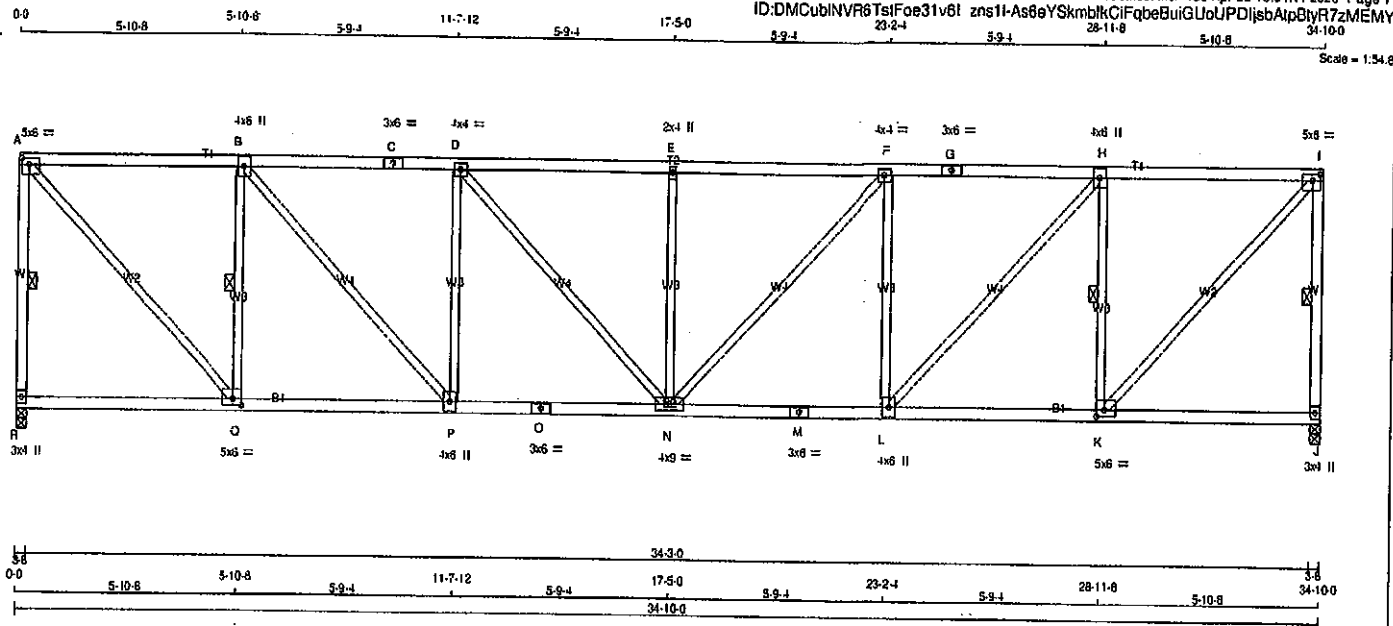


Structural component only
DWG# T-2007639

2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T29	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

Version 8.310 S Oct 29 2019 Mittek Industries, Inc. Tue Apr 28 10:04:11 2020 Page 1
ID:DMCubINVR6TsF0e31v6I zns11-As8eYSkmbkCfFqbeBuiGUoUPDljsbAlpBtyR7zMEMY



LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR
N. L. G. A. RULES					
R - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
C - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
J - I	2x4	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMWV-I	MT20	5.0	8.0	2.00	2.50
B	TMWV-I	MT20	4.0	8.0		
C	TS-I	MT20	3.0	6.0		
D	TMWV-I	MT20	4.0	4.0		
E	TMWV-I	MT20	2.0	4.0		
F	TMWV-I	MT20	4.0	4.0		
G	TS-I	MT20	3.0	6.0		
H	TMWV-I	MT20	4.0	8.0		
I	TMWV-I	MT20	5.0	8.0	2.00	2.50
J	BMV-I-p	MT20	3.0	4.0		
K	BMWV-I	MT20	5.0	8.0	2.00	2.50
L	BMWV-I	MT20	4.0	6.0		
M	BS-I	MT20	3.0	6.0		
N	BMWV-I	MT20	4.0	9.0		
O	BS-I	MT20	3.0	6.0		
P	BMWV-I	MT20	4.0	6.0		
Q	BMWV-I	MT20	5.0	6.0	2.00	2.50
R	BMV-I-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UPLIFT	IN-SX
R	1920	0	0	3-8
J	1920	0	0	3-8

UNFACTORED REACTIONS

1ST CASE	MAX-MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
R	1358	891	0	0-0	0-0	0-0	467	0
J	1358	891	0	0-0	0-0	0-0	467	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-R, I-J, B-Q, H-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LBS)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LBS)
	FR-TO		FROM TO				FR-TO		
A-B	-1878	0	0.0	0.0	0.37 (1)	4.90	A-Q	0	2281
B-C	-1583	0	-91.8	-91.8	0.53 (1)	4.61	Q-B	-1540	0
C-D	-2428	0	-91.8	-91.8	0.53 (1)	3.77	B-P	0	1272
D-E	-2725	0	-91.8	-91.8	0.53 (1)	3.77	P-D	-831	0
E-F	-2725	0	-91.8	-91.8	0.53 (1)	3.72	O-N	0	440
F-G	-2428	0	-91.8	-91.8	0.53 (1)	3.72	N-E	-539	0
G-H	-2428	0	-91.8	-91.8	0.53 (1)	3.77	E-F	0	440
H-I	-1583	0	-91.8	-91.8	0.53 (1)	4.61	F-G	-831	0
I-J	-1878	0	0.0	0.0	0.37 (1)	4.90	L-H	0	1272
							H-K	-1540	0
							K-I	0	2281
R-O	0	0	-18.5	-18.5	0.15 (4)	10.00			
Q-P	0	1583	-18.5	-18.5	0.33 (1)	10.00			
P-O	0	2428	-18.5	-18.5	0.44 (1)	10.00			
O-N	0	2428	-18.5	-18.5	0.44 (1)	10.00			
N-M	0	2428	-18.5	-18.5	0.44 (1)	10.00			
M-L	0	2428	-18.5	-18.5	0.44 (1)	10.00			
L-K	0	1583	-18.5	-18.5	0.33 (1)	10.00			
K-J	0	0	-18.5	-18.5	0.15 (4)	10.00			

TOTAL WEIGHT = 2 X 157 = 314 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPC 2011, TRC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/999 (0.31")

CSF TO=0.63/1.00 (F-H:1), BC=0.44, 1.00 (N-P:1), WB=0.58, 1.00 (F-L:1), SS=0.25/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

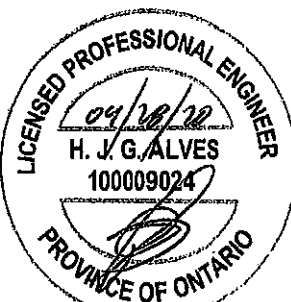
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (A) (INPUT = 0.90)
JSI METAL= 0.78 (A) (INPUT = 1.00)



Structural component only
DWG# T-2007640

JOB NAME

408223

TRUSS NAME

T30

QUANTITY

2

PLY

1

JOB DESC.

GREEN PARK HOMES

TRUSS DESC.

DRWG NO.

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:04:12 2020 Page 1

ID:DMCubINVR6TstFoe31v8l_zns1f-a3q0mokOMbs3JPPnBvPxpHb5cdJb7s02rcVzZMEMIX

00

7-0-6

7-0-6

8-11-2

13-11-7

8-11-2

20-10-9

8-11-2

27-9-10

7-0-6

34-10-0

Scale = 1:55.0

00

7-0-6

7-0-6

8-11-2

13-11-7

8-11-2

20-10-9

8-11-2

27-9-10

7-0-6

34-10-0

34-10-0

TOTAL WEIGHT = 2 X 179 = 358 LB

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
P - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
I - H	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2

ALL WEBS

EXCEPT	2x4	DRY	No.2
O - B	2x3	DRY	No.2
M - D	2x3	DRY	No.2
L - E	2x3	DRY	No.2
J - G	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV-1	MT20	5.0	6.0	2.50	2.50
B	TMWV-1	MT20	4.0	6.0		
C	TS-1	MT20	3.0	6.0		
D	TMWV-1	MT20	4.0	4.0		
E	TMWV-1	MT20	2.0	4.0		
F	TS-1	MT20	3.0	8.0		
G	TMWV-1	MT20	4.0	8.0		
H	TMWV-1	MT20	5.0	8.0	2.50	2.50
I	BMV1+p	MT20	3.0	4.0		
J	BMWV-1	MT20	5.0	6.0	2.50	2.50
K	BS-1	MT20	3.0	8.0		
L	BMWVW-1	MT20	4.0	9.0		
M	BMWV-1	MT20	4.0	8.0		
N	BS-1	MT20	3.0	8.0		
O	BMWV-1	MT20	5.0	8.0	2.50	2.50
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	REGRD
P	1920	0	1920	0	0	3-8	3-8
I	1920	0	1920	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1358	891	0	0	0	0	0
I	1358	891	0	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-P, H-I, B-O, D-L, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (PLF)	MEMB.	FORCE (LBS)
P-A	-1868	0	0.0	0.0	0.57 (1)
A-B	-1432	0	-91.8	-91.8	0.78 (1)
B-C	-2081	0	-91.8	-91.8	0.89 (1)
C-D	-2081	0	-91.8	-91.8	0.89 (1)
D-E	-2080	0	-91.8	-91.8	0.88 (1)
E-F	-2080	0	-91.8	-91.8	0.89 (1)
F-G	-2080	0	-91.8	-91.8	0.89 (1)
G-H	-1432	0	-91.8	-91.8	0.78 (1)
H-I	-1868	0	0.0	0.0	0.57 (1)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (PLF)	MEMB.	FORCE (LBS)
P-A	-1868	0	0.0	0.0	0.57 (1)
A-B	-1432	0	-91.8	-91.8	0.78 (1)
B-C	-2081	0	-91.8	-91.8	0.89 (1)
C-D	-2081	0	-91.8	-91.8	0.89 (1)
D-E	-2080	0	-91.8	-91.8	0.88 (1)
E-F	-2080	0	-91.8	-91.8	0.89 (1)
F-G	-2080	0	-91.8	-91.8	0.89 (1)
G-H	-1432	0	-91.8	-91.8	0.78 (1)
H-I	-1868	0	0.0	0.0	0.57 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

PART 9 OF CBC 2018, CBC 2012, ABC 2018

PART 9 OF CBC 2012 (2019 AMENDMENT)

CSA 086-09, CSA 086-14

TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1/18")

CALCULATED VERT. DEFL.(LL) = L/999 (0.12")

ALLOWABLE DEFL.(TL) = L/360 (1/18")

CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.89/1.00 (E-G:1), BC=0.42/1.00 (L-M:1), WB=0.81/1.00 (D-H:1), SS=0.30/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)

JSI METAL= 0.46 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER

04/28/20

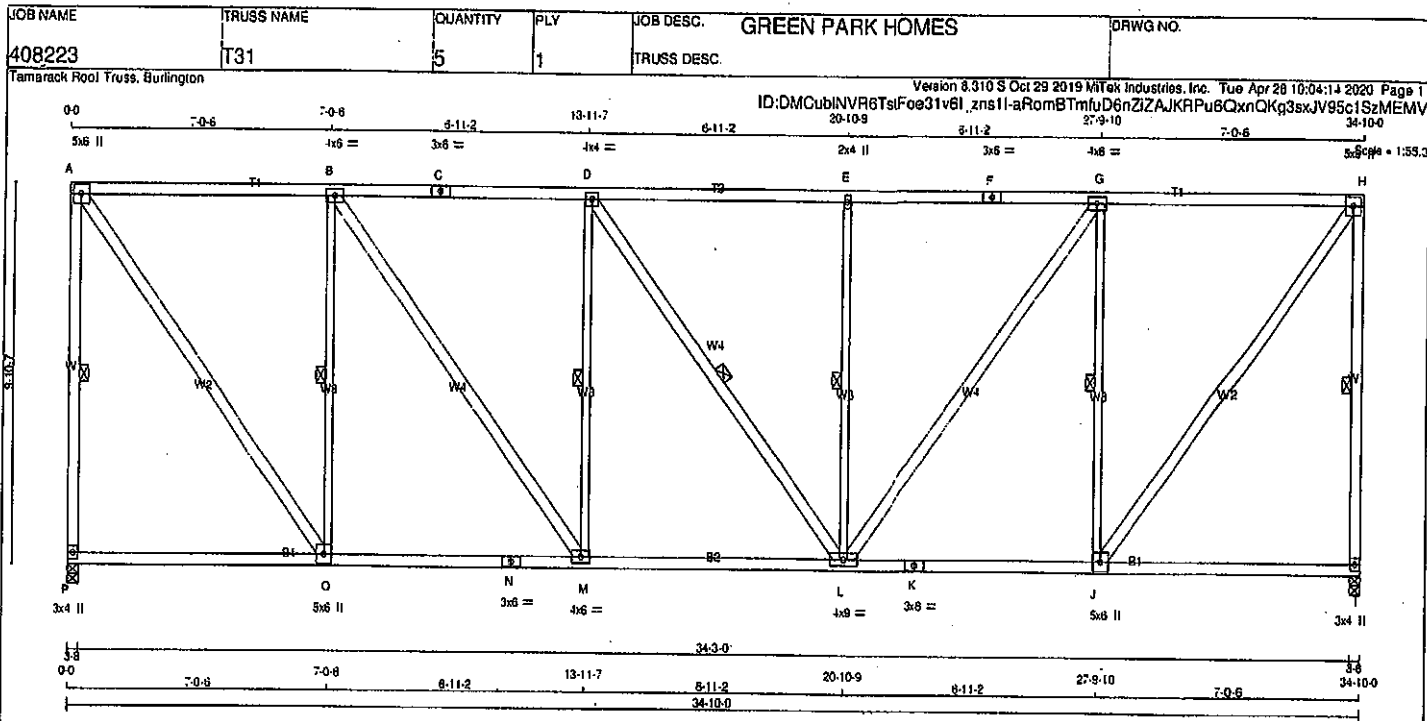
H. J. G. ALVES

100009024

PROVINCE OF ONTARIO

Structural component only

DWG# T-2007641



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
P - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
I - H	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
O - B	2x3	DRY	No.2
M - D	2x3	DRY	No.2
L - E	2x3	DRY	No.2
J - G	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0		
B	TMVW-i	MT20	4.0	6.0		
C	TS-i	MT20	3.0	6.0		
D	TMVW-i	MT20	4.0	6.0		
E	TMVW+i	MT20	2.0	4.0		
F	TS-i	MT20	3.0	6.0		
G	TMVW-i	MT20	4.0	6.0		
H	TMVW+p	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMVW-i	MT20	5.0	6.0		
K	BS-i	MT20	3.0	6.0		
L	BMVW-i	MT20	4.0	6.0		
M	BMVW-i	MT20	4.0	6.0		
N	BS-i	MT20	3.0	6.0		
O	BMVW-i	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECRD BRG
	VERT	HORZ	DOWN	HORZ		
P	1920	0	1920	0	0	3-8
I	1920	0	1920	0	0	3-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1358	891	0	0.0	0.0	487	0
I	1358	891	0	0.0	0.0	487	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-P, H-I, B-O, D-M, D-L, E-L, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING										
TOTAL LOAD CASES: (4)										
CHORDS						WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSH (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSH (LC)
FR-TO		FROM	TO			FR-TO				
P-A	-1868 0	0.0	0.0	0.88 (1)	4.91	A-O	0	1939	0.32 (1)	
A-B	-1183 0	-91.8	-91.8	0.75 (1)	4.69	O-B	-1466 0	0.96 (1)		
B-C	-1719 0	-91.8	-91.8	0.82 (1)	3.96	B-M	0	916	0.15 (1)	
C-D	-1719 0	-91.8	-91.8	0.82 (1)	3.96	M-D	-817 0	0.43 (1)		
D-E	-1718 0	-91.8	-91.8	0.83 (1)	4.37	D-L	-2 0	0.00 (1)		
E-F	-1718 0	-91.8	-91.8	0.83 (1)	3.96	L-E	-818 0	0.41 (1)		
F-G	-1718 0	-91.8	-91.8	0.83 (1)	3.96	L-G	0	913	0.15 (1)	
G-H	-1183 0	-91.8	-91.8	0.75 (1)	4.69	J-G	-1465 0	0.96 (1)		
H-I	-1868 0	0.0	0.0	0.88 (1)	4.91	J-H	0	2000	0.32 (1)	
P-O	0 0	-18.5	-18.5	0.22 (4)	10.00					
O-N	0 1183	-18.5	-18.5	0.32 (4)	10.00					
N-M	0 1183	-18.5	-18.5	0.32 (4)	10.00					
M-L	0 1719	-18.5	-18.5	0.36 (1)	10.00					
L-K	0 1183	-18.5	-18.5	0.32 (4)	10.00					
K-J	0 1183	-18.5	-18.5	0.32 (4)	10.00					
J-I	0 0	-18.5	-18.5	0.22 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.18")
CALCULATED VERT. DEFL.(LL) = L/990 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.18")
CALCULATED VERT. DEFL.(TL) = L/990 (0.22")

CSI: TC=0.88/1.00 (H4:1), BC=0.38/1.00 (L-M:1),
WB=0.88/1.00 (B-O:1), SSI=0.30/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

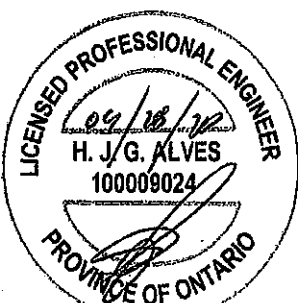
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

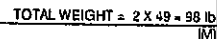
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (H) (INPUT = 0.90)
JSI METAL = 0.43 (H) (INPUT = 1.00)



Structural component only
DWG# T-2007642

Version 8.310 S Oct 29 2019 Mittek Industries, Inc. Tue Apr 28 10:20:14 2020 Page 1
ID:DMCubINVR6TstFoe31v6f| zns11-T7ohgkQqpA3ZkEygYxgXoZB2uPsf_43dGWaQkpzME7V



Structural component only
DWG# T-2007675 1/2

JOB NAME 408224	TRUSS NAME T54S	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DWG NO.
--------------------	--------------------	---------------	----------	-------------------------------	-------------	---------

Tanlarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 10:20:14 2020 Page 2
ID:DMCubINVR6TsIFoe3iv6I zng1I-T7ohgkQopA3ZkEyaYqXoZB2uPsf 43dGWeCkozME7V

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-I	MT20	5.0	8.0	2.50	3.00
B	TMWW-I	MT20	5.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	TMWW-I	MT20	5.0	6.0	2.25	2.00
E	TMWW-I	MT20	4.0	6.0	2.00	2.00
F	BMV+p	MT20	3.0	6.0		
G	BMWWW-I	MT20	6.0	9.0	2.75	4.50
H	BMV+p	MT20	3.0	10.0		
I	BMWWWW-I	MT20	7.0	12.0	4.50	3.75
J	BMWW-I	MT20	5.0	8.0	2.50	3.00
K	BMV+p	MT20	3.0	6.0	3.50	1.50



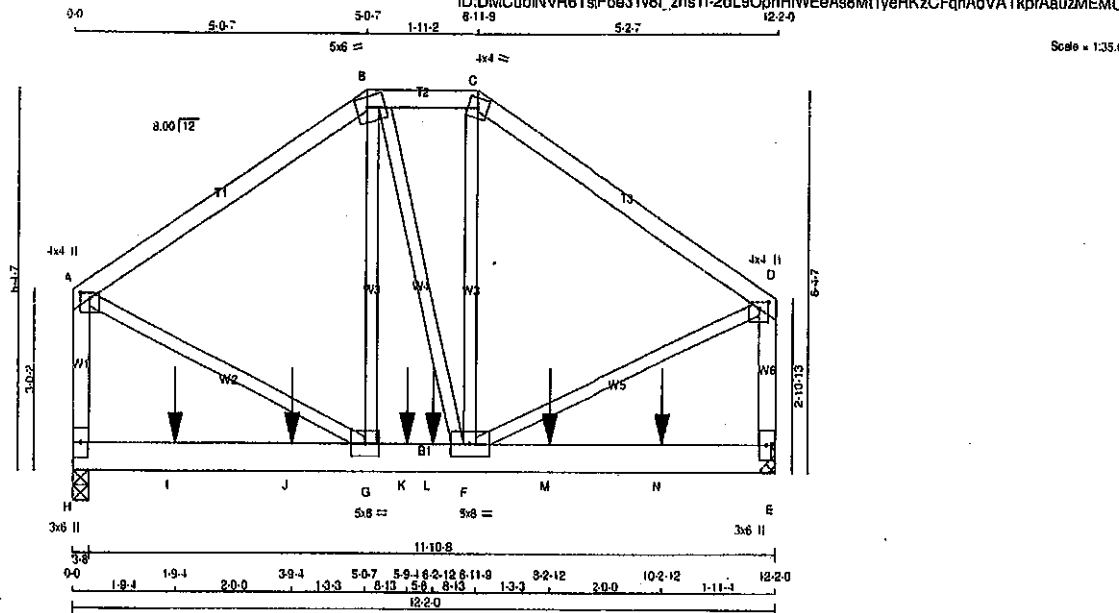
Structural component only
DWG# T-2007675 *HL*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408223	T32	1	1	TRAUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Tue Apr 28 10:04:15 2020 Page 1

ID:DMCubINVR6Tsf0e31v6L_zns11-2dL9OpnHWEeAs8M1yeRKzCFqhAcVATkprAauzMEMU



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2	
B - C	2x4	DRY	No.2	
C - D	2x4	DRY	No.2	
H - A	2x4	DRY	No.2	
E - D	2x4	DRY	No.2	
H - E	2x6	DRY	No.2	

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTWW-m	MT20	5.0	6.0	2.00	2.25
C	TTW-m	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMV1+p	MT20	3.0	6.0		
F	BMWWW-1	MT20	5.0	8.0		
G	BMWW-1	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

DESCR.	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQD
		GROSS REACTION	GROSS REACTION	BRG	BRG
JT		VERT	HORIZ	UPLIFT	IN-SX
H	1233	0	1233	0	0
E	1218	0	1218	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	867	596	0	0	0	271	0
E	856	588	0	0	0	268	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	-853	0	-91.8 -91.8 0.48 (1)	G-B	0	122	0.03 (1)
B-C	-721	0	-91.8 -91.8 0.07 (1)	B-F	0	34	0.01 (1)
C-D	-864	0	-91.8 -91.8 0.51 (1)	F-C	0	154	0.04 (1)
H-A	-1065	0	0.0 0.0 0.17 (1)	A-G	0	795	0.20 (1)
E-D	-1057	0	0.0 0.0 0.18 (1)	F-D	0	794	0.20 (1)
H-I	0	0	-18.5 -18.5 0.20 (1)				
I-J	0	0	-18.5 -18.5 0.20 (1)				
J-G	0	0	-18.5 -18.5 0.20 (1)				
G-K	0	711	-18.5 -18.5 0.28 (1)				
K-L	0	711	-18.5 -18.5 0.28 (1)				
L-F	0	711	-18.5 -18.5 0.28 (1)				
F-M	0	0	-18.5 -18.5 0.20 (1)				
M-N	0	0	-18.5 -18.5 0.20 (1)				
N-E	0	0	-18.5 -18.5 0.20 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	D/R	TYPE	HEEL	CONN.
I	1-9-4	-172	-172	---	BACK	VERT	TOTAL	---	C1
J	3-9-4	-195	-195	---	BACK	VERT	TOTAL	---	C1
K	5-9-4	-187	-187	---	BACK	VERT	TOTAL	---	C1
L	8-2-12	-187	-187	---	BACK	VERT	TOTAL	---	C1
M	8-2-12	-195	-195	---	BACK	VERT	TOTAL	---	C1
N	10-2-12	-172	-172	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.51 1.00 (C-D:1), BC=0.28 1.00 (F-G:1),
WB=0.20 1.00 (A-C:1), SS=0.20 1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	818	354	1887

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.78 (D) (INPUT = 0.90)
JSI METAL = 0.23 (D) (INPUT = 1.00)



Structural component only
DWG# T-2007843

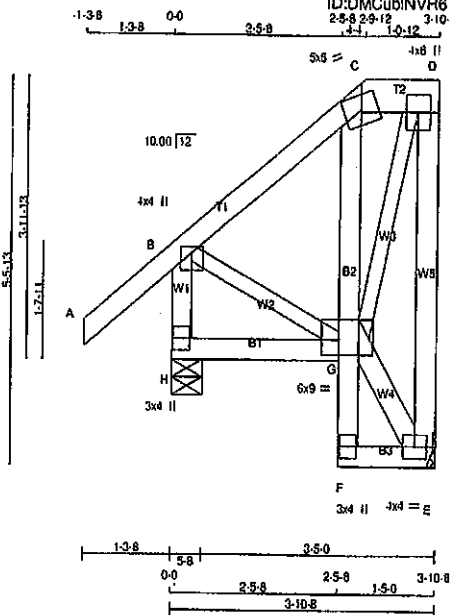
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	T34S	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Tamarack Roof Truss, Burlington

ID:DMCubINVR6TslF0e31v6l_zns1l-WqvXc9ovQqMV0oIZQkUtzXVSIE5FX7ycZTaj6KzMEMT

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 10:04:16 2020 Page 1

Scale = 1:30.1



LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2	SPF	
C - D	2x6	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
F - C	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF	
B - G	2x3	DRY	No.2	SPF	
G - D	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C	TTV-m	MT20	5.0	8.0	
D	TMVW+p	MT20	4.0	8.0	
E	BMVW1-t	MT20	4.0	4.0	
F	BMV+p	MT20	3.0	4.0	
G	BMVWW-t	MT20	6.0	9.0	2.50 3.00
H	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	214	0	214	0	0	MECHANICAL	
H	341	0	341	0	0	5-8	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	151	99.0	0.0	0.0	0.0	52.0	0.0
H	239	170.0	0.0	0.0	0.0	69.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (PLF)	MAX. (LBS)	MEMB.	FORCE (LBS)	MAX. (LBS)
FR-TO		FROM	TO	CS1 (LBS)	FR-TO		CS1 (LBS)
A-B	0 41	-91.8	-91.8 0.14 (5)	10.00	G-E	0 0	0.00 (11)
B-C	-62 0	-91.8	-91.8 0.12 (1)	6.25	B-G	0 53	0.01 (11)
C-D	-51 0	-91.8	-91.8 0.01 (1)	6.25	G-D	0 154	0.03 (11)
E-D	-197 0	0.0	0.0 0.10 (1)	7.81			
H-B	-317 0	0.0	0.0 0.03 (1)	7.81			
H-G	0 0	-18.5	-18.5 0.04 (4)	10.00			
F-G	0 13	0.0	0.0 0.01 (1)	10.00			
G-C	-138 0	0.0	0.0 0.01 (1)	7.81			
F-E	0 4	-18.5	-18.5 0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 32 = 64 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CS1: TC=0.14/1.00 (A-B:5), BC=0.04/1.00 (G-H:4), WB=0.03/1.00 (D-G:1), SS=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX
MT20	618	354
	1667	788
	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.24 (B) (INPUT = 0.60)
JSI METAL = 0.08 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007644

DRWG NO.

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 10:04:17 2020 Page 1
ID:DMCubINVR6TstFoe31v6l zns1l- 0TvpVoXB8UOAH S26W2dVdRQGSEmC7KGenzMEMS



TOTAL WEIGHT = $2 \times 29 = 57 \text{ lb}$

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	191	0	191	0	0	MECHANICAL	
I	393	0	393	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1.8.

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

1ST LCASE MAX., MIN. COMPONENT REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

DATA: UNSTABLE CO1 ON ONCHS CENR11 = 0.25 1 1 ONTR01 BELONG DIRECTET AFFL1

TOTAL LOAD CASES: (5)

CHORDS					WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM			FR-TO			
A-B	0 41	-91.8	-91.8	0.14 (5)	10.00	H-F	-8 0 0.00 (1)	
B-C	-49 0	-91.8	-91.8	0.14 (5)	8.25	H-E	0 141 0.03 (1)	
C-D	-65 0	-91.8	-91.8	0.02 (1)	8.25	C-H	0 48 0.01 (1)	
D-E	-87 0	-91.8	-91.8	0.02 (1)	6.25	I-C	-96 28 0.02 (1)	
F-E	-175 0	0.0	0.0	0.05 (1)	7.81			
I-B	-255 0	0.0	0.0	0.03 (1)	7.81			
I-H	-13 45	-18.5	-18.5	0.04 (4)	6.25			
G-H	0 13	0.0	0.0	0.01 (1)	10.00			
H-D	-135 0	0.0	0.0	0.01 (1)	7.81			
G-F	0 4	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF

DL	n	6.0	PSF
----	---	-----	-----

BOT	CH.	LL	=	0.0	PSF
-----	-----	----	---	-----	-----

OL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

PLATE 13	247	11.99
----------	-----	-------

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9.
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN
LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF
LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (A-B:5) , BC=0.04/1.00 (H-I:4) ,
WB=0.03/1.00 (E-H:1) , SS=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

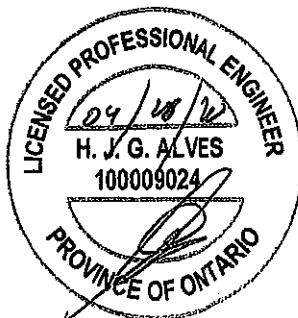
RAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
1	100	100	100
2	100	100	100
3	100	100	100
4	100	100	100
5	100	100	100
6	100	100	100
7	100	100	100
8	100	100	100
9	100	100	100
10	100	100	100
11	100	100	100
12	100	100	100
13	100	100	100
14	100	100	100
15	100	100	100
16	100	100	100
17	100	100	100
18	100	100	100
19	100	100	100
20	100	100	100
21	100	100	100
22	100	100	100
23	100	100	100
24	100	100	100
25	100	100	100
26	100	100	100
27	100	100	100
28	100	100	100
29	100	100	100
30	100	100	100
31	100	100	100
32	100	100	100
33	100	100	100
34	100	100	100
35	100	100	100
36	100	100	100
37	100	100	100
38	100	100	100
39	100	100	100
40	100	100	100
41	100	100	100
42	100	100	100
43	100	100	100
44	100	100	100
45	100	100	100
46	100	100	100
47	100	100	100
48	100	100	100
49	100	100	100
50	100	100	100
51	100	100	100
52	100	100	100
53	100	100	100
54	100	100	100
55	100	100	100
56	100	100	100
57	100	100	100
58	100	100	100
59	100	100	100
60	100	100	100
61	100	100	100
62	100	100	100
63	100	100	100
64	100	100	100
65	100	100	100
66	100	100	100
67	100	100	100
68	100	100	100
69	100	100	100
70	100	100	100
71	100	100	100
72	100	100	100
73	100	100	100
74	100	100	100
75	100	100	100
76	100	100	100
77	100	100	100
78	100	100	100
79	100	100	100
80	100	100	100
81	100	100	100
82	100	100	100
83	100	100	100
84	100	100	100
85	100	100	100
86	100	100	100
87	100	100	100
88	100	100	100
89	100	100	100
90	100	100	100
91	100	100	100
92	100	100	100
93	100	100	100
94	100	100	100
95	100	100	100
96	100	100	100

	TENSILE STRENGTH		TENSILE ELONGATION		TENSILE MODULUS	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



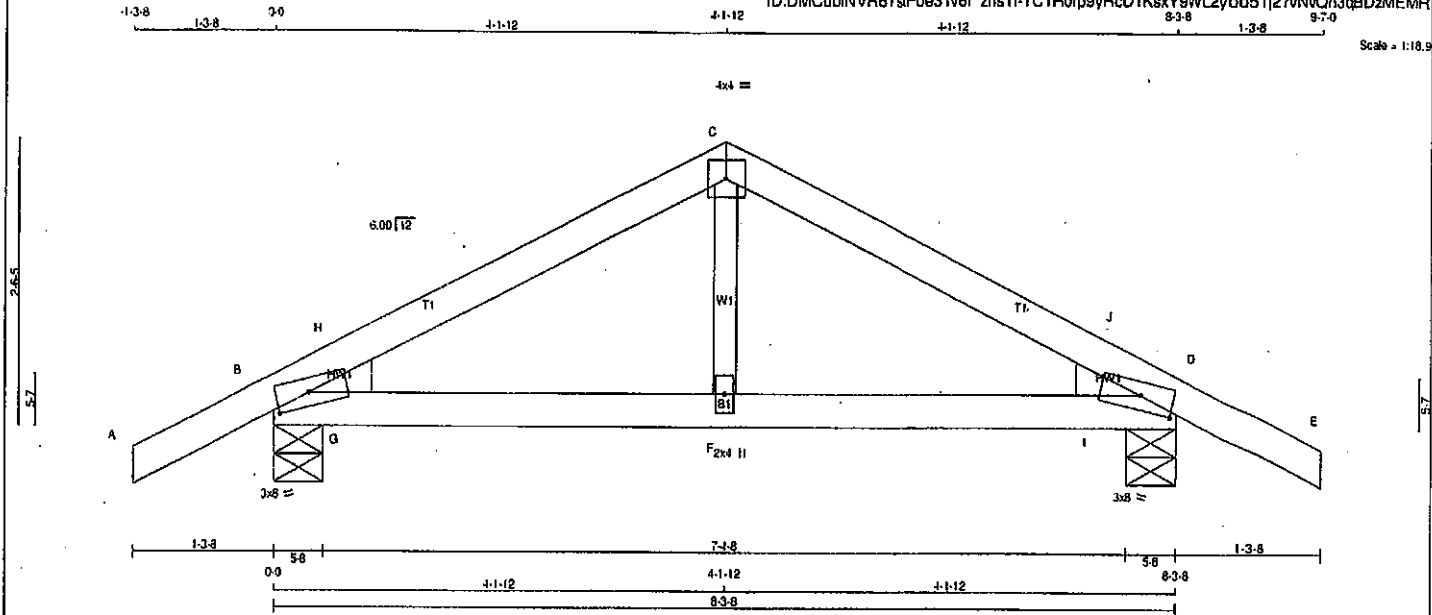
Structural component only
DWG# T-2007645

JOB NAME 408223	TRUSS NAME T36	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
--------------------	-------------------	---------------	----------	-------------------------------	-------------	----------

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Tue Apr 28 10:04:18 2020 Page 1

ID:DMCubINVR6TsFoe31v6l zns11-TC1H0rp9yRcD1KsxY9WL2ybo51j27vNvQn3qBDzMEMR



LUMBER	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES				
CHORDS	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMBH1-m	MT20	3.0	8.0	1.50 3.75
C JTW-p	MT20	4.0	4.0	
D TMBH1-m	MT20	3.0	8.0	1.50 3.75
F 8MW-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
B	582	0	582	0	5-8	5-8	2x4 L
D	582	0	582	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST CASE		MAX-MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
B	409	282	0	0	0	127	0
D	409	282	0	0	0	127	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LOCI (MAX)	CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0 23	-91.8	-91.8	0.12 (1)	10.00	F-C	0 170	0.04 (1)	
B-H	-539 0	-91.8	-91.8	0.04 (1)	6.25	G-H	-158 3	0.00 (1)	
H-C	-525 0	-91.8	-91.8	0.15 (1)	6.25	I-J	-158 3	0.00 (1)	
C-J	-525 0	-91.8	-91.8	0.15 (1)	6.25				
J-D	-539 0	-91.8	-91.8	0.04 (1)	6.25				
D-E	0 23	-91.8	-91.8	0.12 (1)	10.00				
B-G	0 465	-18.5	-18.5	0.21 (1)	10.00				
G-F	0 465	-18.5	-18.5	0.21 (1)	10.00				
F-I	0 465	-18.5	-18.5	0.21 (1)	10.00				
I-O	0 465	-18.5	-18.5	0.21 (1)	10.00				

TOTAL WEIGHT = 3 X 27 = 80 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/360 (0.28")
CALCULATED VERT. DEF. (LL) = L/999 (0.01")
ALLOWABLE DEF. (TL) = L/360 (0.28")
CALCULATED VERT. DEF. (TL) = L/999 (0.02")

CSI: TC=0.15 (1.00 (C-J)), BC=0.21 (1.00 (F-I)), WB=0.04 (1.00 (C-F)), SS=0.12 (1.00 (C-J))

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	818	354	1887 788 1887 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.25 (B) (INPUT = 0.90)
JSI METAL=0.11 (B) (INPUT = 1.00)

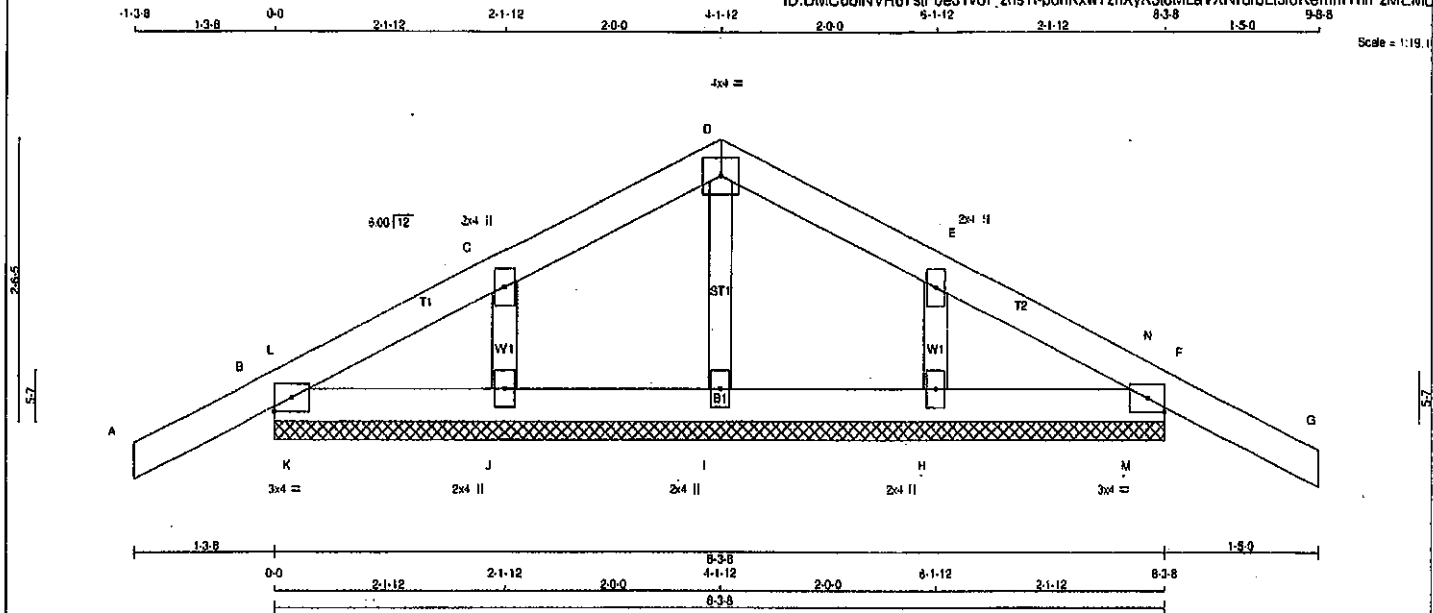


Structural component only
DWG# T-2007646

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408223	G36	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 10:03:49 2020 Page 1
ID:DMCubINVR6TstFoe31v6l, zns11-pdnKxwTznXyK3i8MLaVXNrubL15ioKemmYnfpZMEMu



TOTAL WEIGHT = 27 lb

LUMBER					
N. L. G. A. RULES					
CHORDS,		SIZE	LUMBER		DESCR.
A - D	2x4	DRY	No.2		SPF
D - G	2x4	DRY	No.2		SPF
B - F	2x4	DRY	No.2		SPF
ALL WEBS		2x3	DRY	No.2	SPF
ALL GABLE WEBS		2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.					
GABLE STUDS SPACED AT 2'-0" OC.					

PLATES (table is in inches)	TYPE	PLATES	W	LEN	Y	X
JT	TMB1-I	MT20	3.0	4.0		Edge
B	TMB1-I	MT20	3.0	4.0		Edge
C	TMB1-I	MT20	3.0	4.0		Edge
D	TMB1-I	MT20	3.0	4.0		Edge
E	TMB1-I	MT20	3.0	4.0		Edge
F	TMB1-I	MT20	3.0	4.0		Edge
G	TMB1-I	MT20	3.0	4.0		Edge
H	TMB1-I	MT20	3.0	4.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	MAX. UNBRACED LENGTH (FT)
FR-TO		FROM TO		FR-TO			
A-B	0 23	-91.8 -91.8 0.12 (1)	10.00	I-D	-115 0	0.02 (1)	
B-L	-61 0	-91.8 -91.8 0.01 (4)	6.25	J-C	-218 0	0.03 (1)	
L-C	-39 0	-91.8 -91.8 0.06 (1)	6.25	H-E	-218 0	0.03 (1)	
C-D	-45 0	-91.8 -91.8 0.06 (1)	6.25	K-L	-52 7	0.00 (1)	
D-E	-45 0	-91.8 -91.8 0.06 (1)	6.25	M-N	-52 7	0.00 (1)	
E-N	-39 0	-91.8 -91.8 0.06 (1)	6.25				
N-F	-61 0	-91.8 -91.8 0.01 (4)	6.25				
F-G	0 25	-91.8 -91.8 0.14 (1)	10.00				
S-K	0 44	-18.5 -18.5 0.03 (1)	10.00				
K-J	0 44	-18.5 -18.5 0.03 (1)	10.00				
J-I	0 32	-18.5 -18.5 0.02 (1)	10.00				
I-H	0 32	-18.5 -18.5 0.02 (1)	10.00				
H-M	0 44	-18.5 -18.5 0.03 (1)	10.00				
M-F	0 44	-18.5 -18.5 0.03 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
-PART 9 OF CBC 2010, CBC 2012, ABC 2019
-PART 9 OF CBC 2012 (2019 AMENDMENT)
-CSA 088-09, CSA 088-14
-TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (F-G:1), SC=0.03/1.00 (H-M:1), WB=0.03/1.00 (E-H:1), SS=0.10/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)



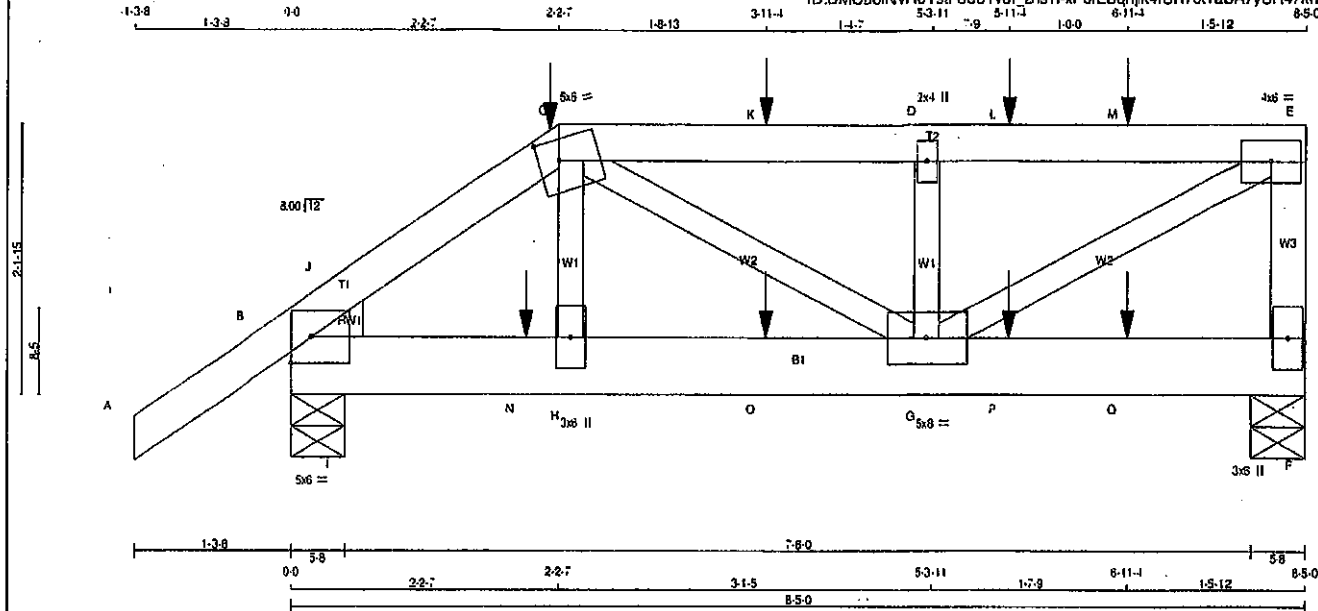
Structural component only
DWG# T-2007621

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DWG NO.
408223	T37	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 26 10:04:19 2020 Page 1
ID:DMCubINVR6TstFos31v6l_zns11-xPbfEBqjK4UR781abA7yUR47k172RPNjzMEMQ

Scale = 1/17.0



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
F - F	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (tablets in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0		Edge
C	TTWW-m	MT20	5.0	6.0	2.00	2.00
D	TMW-w	MT20	2.0	4.0		
E	TMVW-I	MT20	4.0	6.0		
F	BMV1-p	MT20	3.0	6.0		
G	BMVWW-I	MT20	5.0	6.0		
H	BMV-w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	HEEL
F	760	0	760	0	0	5.8	5.8	5.8	WEDGE
B	823	0	823	0	0	5.8	5.8	5.8	2x4 L

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	537	352.0	0.0	0.0	0.0	185.0	0.0
B	580	392.0	0.0	0.0	0.0	188.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX.		WEBS		MAX.	
MEMB.	FORCE (LBS)	VERT.	LOAD (LBS)	MAX.	CS1 (LBS)	MEMB.	FORCE (LBS)	MAX.	CS1 (LBS)
FR-TO		FROM	TO			FR-TO			
A-B	-0.28	-91.8	-91.8	0.13 (1)	10.00	H-C	0.73	0.02 (4)	
B-J	-1068.0	-91.8	-91.8	0.08 (1)	5.99	C-G	0.252	0.06 (1)	
J-C	-880.0	-91.8	-91.8	0.08 (1)	8.25	G-D	-528.0	0.09 (1)	
C-K	-923.0	-91.8	-91.8	0.24 (1)	8.09	G-E	0.1062	0.28 (1)	
K-D	-923.0	-91.8	-91.8	0.24 (1)	8.09	I-J	0.207	0.00 (1)	
D-L	-924.0	-91.8	-91.8	0.24 (1)	8.09				
L-M	-924.0	-91.8	-91.8	0.24 (1)	8.09				
M-E	-924.0	-91.8	-91.8	0.24 (1)	8.09				
F-E	-692.0	0.0	0.0	0.08 (1)	7.81				
P-I	0.701	-18.5	-18.5	0.14 (1)	10.00				
I-N	0.701	-18.5	-18.5	0.14 (1)	10.00				
N-H	0.701	-18.5	-18.5	0.14 (1)	10.00				
H-O	0.705	-18.5	-18.5	0.15 (1)	10.00				
O-G	0.705	-18.5	-18.5	0.15 (1)	10.00				
G-P	0.0	-18.5	-18.5	0.07 (1)	10.00				
P-O	0.0	-18.5	-18.5	0.07 (1)	10.00				
O-F	0.0	-18.5	-18.5	0.07 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	2-2.7	-88	-88	---	FRONT	VERT	TOTAL	---	C1
X	3-11.4	-83	-83	---	FRONT	VERT	TOTAL	---	C1
L	5-11.4	-83	-83	---	FRONT	VERT	TOTAL	---	C1
M	8-11.4	-83	-83	---	FRONT	VERT	TOTAL	---	C1
N	1-11.4	-48	-48	---	FRONT	VERT	TOTAL	---	C1
O	3-11.4	-48	-48	---	FRONT	VERT	TOTAL	---	C1
P	5-11.4	-48	-48	---	FRONT	VERT	TOTAL	---	C1
Q	8-11.4	-48	-48	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CS1: TC=0.24/1.00 (D-E:1), BC=0.15/1.00 (G-H:1),
WB=0.26/1.00 (E-G:1), SS=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

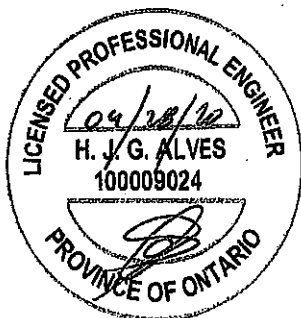
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 789 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (E) (INPUT = 0.90)
JSI METAL= 0.24 (E) (INPUT = 1.00)



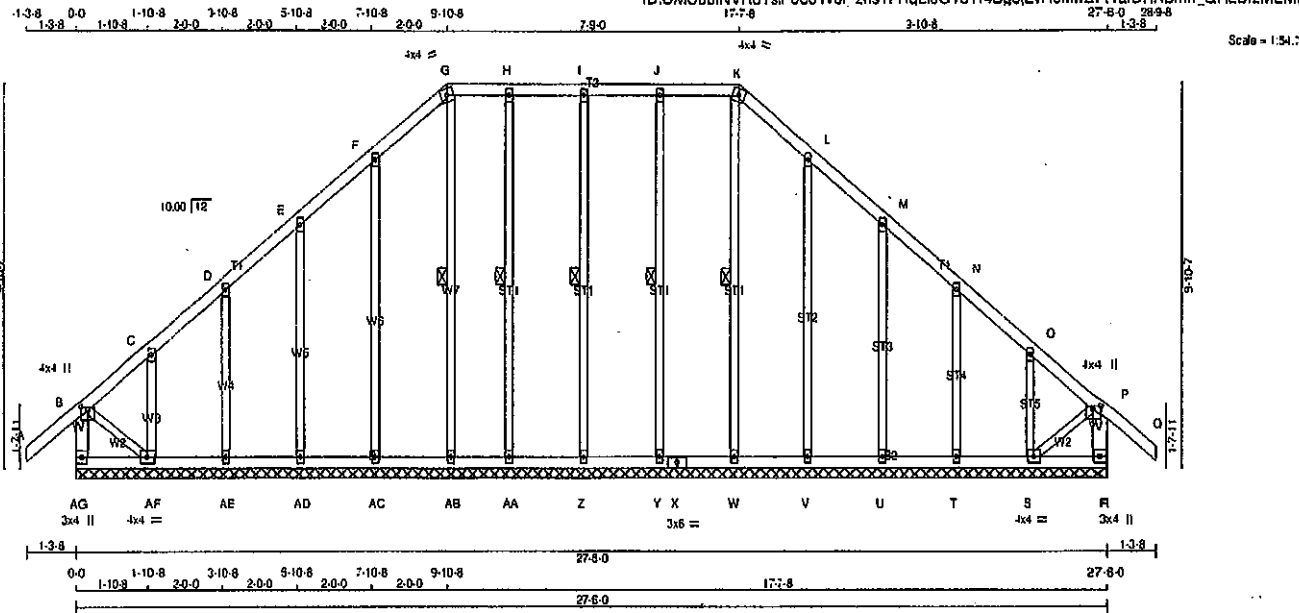
Structural component only
DWG# T-2007647

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408223	G39	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Tue Apr 28 10:03:50 2020 Page 1

ID:DMCubINVR6TsfFoe31v6l zns11-HqL8G7bYr4Bq0ZvH0mw2R1alDRABmn_QHLBrzMEM



TOTAL WEIGHT = 2 X 152 = 303 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
AG - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
K - Q	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
AG - X	2x4	DRY	No.2	SPF
X - R	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2-0-0 OC.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF K-W, J-Y, I-Z, H-AA, G-AB.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
AG-B	-292 0	0.0 0.0 0.03 (1)	7.81	W-K	-140 0	0.09 (1)	
A-B	0 41	-91.8 -91.8 0.13 (1)	10.00	Y-J	-210 0	0.14 (1)	
B-C	-80 0	-91.8 -91.8 0.12 (1)	8.25	Z-I	-181 0	0.12 (1)	
C-D	-23 0	-91.8 -91.8 0.05 (1)	8.25	AA-H	-182 0	0.12 (1)	
D-E	-26 0	-91.8 -91.8 0.05 (1)	8.25	V-L	-203 0	0.27 (1)	
E-F	-19 0	-91.8 -91.8 0.05 (1)	8.25	U-M	-175 0	0.13 (1)	
F-G	-29 0	-91.8 -91.8 0.05 (1)	8.25	T-N	-192 0	0.07 (1)	
G-H	-12 0	-91.8 -91.8 0.04 (1)	8.25	S-O	-126 0	0.02 (1)	
H-I	-12 0	-91.8 -91.8 0.04 (1)	8.25	AB-G	-121 0	0.08 (1)	
I-J	-12 0	-91.8 -91.8 0.05 (1)	8.25	AC-F	-202 0	0.27 (1)	
J-K	-12 0	-91.8 -91.8 0.05 (1)	8.25	AD-E	-176 0	0.13 (1)	
K-L	-30 0	-91.8 -91.8 0.05 (1)	8.25	AE-D	-192 0	0.07 (1)	
L-M	-19 0	-91.8 -91.8 0.05 (1)	8.25	AF-C	-126 0	0.02 (1)	
M-N	-26 0	-91.8 -91.8 0.05 (1)	8.25	B-AF	0 32	0.01 (1)	
N-O	-24 0	-91.8 -91.8 0.05 (1)	8.25	S-P	0 32	0.01 (1)	
O-P	-80 0	-91.8 -91.8 0.12 (1)	8.25				
P-Q	0 41	-91.8 -91.8 0.13 (1)	10.00				
R-P	-293 0	0.0 0.0 0.03 (1)	7.81				
AG-AF	0 0	-18.5 -18.5 0.02 (4)	10.00				
AF-AE	0 23	-18.5 -18.5 0.02 (4)	10.00				
AE-AD	0 19	-18.5 -18.5 0.02 (4)	10.00				
AD-AC	0 16	-18.5 -18.5 0.02 (4)	10.00				
AC-AB	0 14	-18.5 -18.5 0.02 (4)	10.00				
AB-AA	0 12	-18.5 -18.5 0.01 (4)	10.00				
AA-Z	0 12	-18.5 -18.5 0.02 (4)	10.00				
Z-Y	0 12	-18.5 -18.5 0.02 (4)	10.00				
Y-X	0 12	-18.5 -18.5 0.02 (4)	10.00				
X-W	0 12	-18.5 -18.5 0.02 (4)	10.00				
W-V	0 14	-18.5 -18.5 0.02 (4)	10.00				
V-U	0 16	-18.5 -18.5 0.02 (4)	10.00				
U-T	0 19	-18.5 -18.5 0.02 (4)	10.00				
T-S	0 23	-18.5 -18.5 0.02 (4)	10.00				
S-R	0 0	-18.5 -18.5 0.02 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CID

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.0:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSL TO=0.13-1.00 (P-O:1) .8C=0.02-1.00 (S-T:4) .WB=0.27 1.00 (L-V:1) .SSI=0.09-1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1887 788 1887 1556

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.62 (K) (INPUT = 0.80)
JSI METAL = 0.11 (L) (INPUT = 1.00)



Structural component only
DWG# T-2007622

Tamarack Pool Truss, Burlington

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 10:19:57 2020 Page 1
ID=DMCubiNVR6TsfFoE31v6f_zns1l-TsGG5wA9pyw_Bd8P2lsKcz_APYAIVtA7Ln10dlzME7m

-1-8 0-0 5-10-8 11-9-6 17-7-0 23-4-10 29-3-6 35-2-0 38-5-8
1-3-8 5-10-8 11-9-6 17-7-0 23-4-10 29-3-6 35-2-0 38-5-8

Scale: 1"=5'



ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12
					S	2350	1539	0	0 0	0 0	811 0	0 0	
					K	2350	1539	0	0 0	0 0	811 0	0 0	

DRY, SEASONED LUMBER. BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S. K. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:	BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.	SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015
		THIS DESIGN COMPLIES WITH:

RAILS TO BE DRIVEN FROM ONE SIDE ONLY.	D-V	-0004	0	-01.8	-01.8	0.27 (1)	4.08	O-G	0	0.946	0.12 (1)
	V-W	-0004	0	-01.8	-01.8	0.27 (1)	4.08	M-G	-1500	0	0.18 (1)
	W-E	-0004	0	-01.8	-01.8	0.27 (1)	4.08	M-H	0	0.193	0.40 (1)
	E-X	-0004	0	-01.8	-01.8	0.27 (1)	4.08	L-H	-413	20	0.05 (1)
	X-F	-0004	0	-01.8	-01.8	0.27 (1)	4.08	R-R	0	0.4582	0.57 (1)
	F-V	-0004	0	-01.8	-01.8	0.27 (1)	4.08	J-L	0	0.4582	0.57 (1)
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NALS.											
	DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00 COMPANION LIVE LOAD FACTOR = 1.00										

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.	Y -	-8064 0	-91.8	-91.8	0.27 (1)	4.08	0.57 (1)	AUTOSOLVE HELLS OFF
	Y - G	-8064 0	-91.8	-91.8	0.27 (1)	4.08		
	G - Z	-7284 0	-91.8	-91.8	0.27 (1)	4.26		
	Z - AA	-7284 0	-91.8	-91.8	0.27 (1)	4.28		TRUSS PLATE MANUFACTURER IS NOT

AA-H	-7284	0	-91.8	-91.8	0.27 (1)	-4.26	RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
H-I	-5085	0	-91.8	-91.8	0.52 (1)	3.67	
I-J	0	28	-91.8	-91.8	0.07 (1)	10.00	

		NAIL VALUES			
		PLATE	GRIP(DRY)	SHEAR	SECTION
		(PSI)	(PLI)	(PLI)	(PLI)
S-8	-3248 0	0.0	0.0	0.11 (1)	7.69
K-1	-3248 0	0.0	0.0	0.11 (1)	7.69

[illegible]

R-AD	0	4553	-18.5	-18.5	0.22 (1)	10.00
AD-AE	0	4553	-18.5	-18.5	0.22 (1)	10.00
AE-CE	0	4553	-18.5	-18.5	0.22 (1)	10.00

PLATE PLACEMENT TOL. = 0.250 inches

RE-Q	0 4953	-16.3	-16.3	0.22 (1)	10.00
Q-P	0 7263	-18.5	-18.5	0.34 (1)	10.00
P-AF	0 7263	-18.5	-18.5	0.34 (1)	10.00

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP = 0.58 (R) (INPUT = 0.90)

AF-O	0	7263	-18.5	-18.5	0.34 (1)	10.00
O-AG	0	7263	-18.5	-18.5	0.34 (1)	10.00
AG-N	0	7263	-18.5	-18.5	0.34 (1)	10.00

JSI METAL = 0.56 (P) / INPUT = 1.00 ;

N-M	0 7283	-18.5	-18.5	0.34 (1)	10.00
M-AH	0 4553	-18.5	-18.5	0.22 (1)	10.00
ALL AL	0 1550	19.5	19.5	0.20 (1)	10.00

APAI	0	4553	-18.5	-18.5	0.22 (1)	10.00
AI-L	0	4553	-18.5	-18.5	0.22 (1)	10.00
LAI	0	0	-18.5	-18.5	0.04 (4)	10.00

AJ-AK	0 0	-18.5	-18.5	0.04 (4)	10.00
AK-K	0 0	-18.5	-18.5	0.04 (4)	10.00

FACTORED CONCENTRATED LOADS (LBS)											
JT	LOC.	LC1	MAX-	MAX+	FACE FRONT	DIR. VERT.	TYPE TOTAL	HEEL	CONN.		
C	5-10.9	.120	.120								

DWG# T-2007660 1/2

U	5-10-0	-425	-425	---	FRONT	VERT	TOTAL	---	C1
D	11-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ORWG NO.
408224	T40	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 10:19:57 2020 Page 2

ID:DMCubINVR6TslF0e31v6l zns1I-TsGG5wA9pwy Bd8P2sXcz APYAVTa7LN10dlzME7m

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	4.0	12.0	1.00	5.50
C	TTWW+m	MT20	7.0	8.0	Edge	
D	TMWW-l	MT20	5.0	6.0		
E	TMWW-w	MT20	3.0	8.0		
F	TS-l	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	7.0	8.0	Edge	
I	TMWW-p	MT20	4.0	12.0	1.00	5.50
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-l	MT20	5.0	8.0	2.50	3.00
M	BMWW-l	MT20	5.0	8.0	2.50	3.25
N	BS-l	MT20	6.0	9.0		
O	BMWW-l	MT20	5.0	8.0		
P	BS-l	MT20	6.0	9.0		
Q	BMWW-l	MT20	5.0	8.0	2.50	3.25
R	BMWW-l	MT20	5.0	8.0	2.50	3.00
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	17-7-0	-110	-110	---	FRONT	VERT	TOTAL	---	C1
G	23-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
H	29-9-8	-429	-429	---	FRONT	VERT	TOTAL	---	C1
L	29-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
M	23-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
N	21-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	17-7-0	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	13-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Q	11-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
R	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
T	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
U	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	15-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	19-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	21-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	25-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	27-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	1-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AC	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AD	7-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AE	9-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AF	15-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AG	19-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AH	25-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AI	27-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AJ	31-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AK	33-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



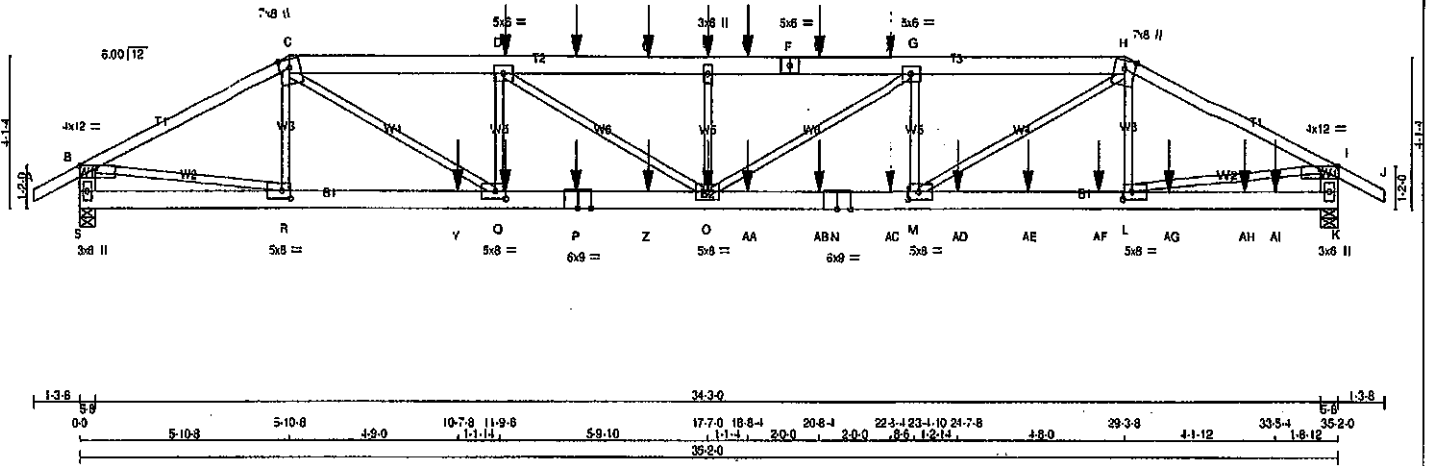
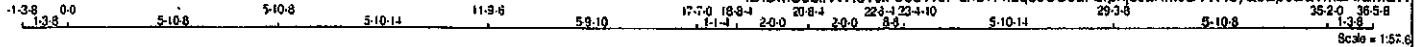
Structural component only
DWG# T-2007660

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	T40Z	1	2	GREEN PARK HOMES	
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 9 Oct 29 2019 MTEK Industries, Inc. Tue Apr 28 10:19:58 2020 Page 1

ID:DMCubINVR6TslFoe31v6l zns1l-x2qeJGBoaF2pnjbcam9BWHeyQ8EpcGa1mZakzME7l



TOTAL WEIGHT = 2 X 169 = 338 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x6	DRY	No.2	SPF	
F - H	2x6	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
S - B	2x6	DRY	No.2	SPF	
K - I	2x6	DRY	No.2	SPF	
S - P	2x6	DRY	1650 F 1.5E	SPF	
P - N	2x8	DRY	1650 F 1.5E	SPF	
N - K	2x8	DRY	1650 F 1.5E	SPF	

ALL WEBS EXCEPT 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
H - J	12	TOP
C - F	12	SIDE(183.1)
F - H	12	SIDE(0.0)
S - B	12	TOP
K - I	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S - P	12	SIDE(183.1)
P - N	12	SIDE(183.1)
N - K	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	UPLIFT	BRG	IN-SX
S	4161	0	4161	0	0	5-8	5-8	5-8	
K	5042	0	5042	0	0	5-8	5-8	5-8	

UNFACTORED REACTIONS		1ST LCASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
S	2938	1985	0	0	0
K	3552	2409	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)	
CHORDS	FACTORED	WEBS	FACTORED
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 28	R-C	-470 0
B-C	-6650 0	C-Q	0 5608
C-D	-10735 0	D-O	-1878 0
D-T	-12119 0	O-E	0 -1638
T-U	-12119 0	E-F	-1004 0
U-E	-12119 0	F-G	-46 33
E-V	-12119 0	G-H	-826 0
V-F	-12119 0	H-I	0 5832
F-W	-12119 0	I-J	0 57
W-X	-12119 0	J-K	0 6014
X-G	-12119 0	K-L	0 7274
G-H	-12119 0		
H-I	-8042 0		
I-J	0 28		
J-K	-4141 0		
K-L	-4926 0		

LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	11-11-4	-110	-110	---	BACK	VERT	---	C1
D	17-7-0	-110	-110	---	BACK	VERT	---	C1
E	17-7-0	-28	-28	---	BACK	VERT	---	C1
P	13-11-4	-28	-28	---	BACK	VERT	---	C1

FACTORED CONCENTRATED LOADS (LBS)	
JT	LOC.
D	11-11-4
E	17-7-0
P	13-11-4

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.32")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/718 (0.59")
CSI: TC=0.81, I=1.00 (H=1.1), BC=0.71, I=1.00 (M=0.1),
WB=0.90, I=1.00 (L=1.1), SSI=0.64, I=1.00 (L=1.1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (R) (INPUT = 0.90)
JSI METAL = 0.93 (P) (INPUT = 1.00)



Structural component only
DWG# T-2007661

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	T40Z	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 10:19:58 2020 Page 2
ID:DMCubINVR6TsIFoe31v6L_zns1l-x2qeJGBoaF2mnibcaNm9BWHeyQ8EpcGa1mZAkzME7I

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	12.0	1.00	5.50
C	TTWW+m	MT20	7.0	8.0	Edge	
D	TMVW-t	MT20	5.0	8.0		
E	TMVW-w	MT20	3.0	8.0		
F	TS-l	MT20	5.0	8.0		
G	TMVW-l	MT20	5.0	8.0		
H	TTWW+m	MT20	7.0	8.0	Edge	
I	TMVW-p	MT20	4.0	12.0	1.00	5.50
K	BMV1+p	MT20	3.0	8.0		
L	BMVW-l	MT20	5.0	8.0	2.50	3.00
M	BMVW-l	MT20	5.0	8.0	2.50	3.25
N	BS-l	MT20	6.0	9.0		
O	BMVWV-l	MT20	5.0	8.0		
P	BS-l	MT20	6.0	9.0		
Q	BMVW-l	MT20	5.0	8.0	2.50	3.25
R	BMVW-l	MT20	5.0	8.0	2.50	3.00
S	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	11-11-4	-28	-28	---	BACK	VERT	TOTAL	---	C1
T	13-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
U	15-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
V	18-8-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
W	20-8-4	-159	-159	---	BACK	VERT	TOTAL	---	C1
X	22-8-4	-159	-159	---	BACK	VERT	TOTAL	---	C1
Y	10-7-8	-1171	-1171	---	BACK	VERT	TOTAL	---	C1
Z	15-11-4	-28	-28	---	BACK	VERT	TOTAL	---	C1
AA	18-8-4	-28	-28	---	BACK	VERT	TOTAL	---	C1
AB	20-8-4	-32	-32	---	BACK	VERT	TOTAL	---	C1
AC	22-8-4	-32	-32	---	BACK	VERT	TOTAL	---	C1
AD	24-7-8	-2237	-2237	---	BACK	VERT	TOTAL	---	C1
AE	26-8-12	-121	-121	---	BACK	VERT	TOTAL	---	C1
AF	28-6-12	-121	-121	---	BACK	VERT	TOTAL	---	C1
AG	30-6-12	-121	-121	---	BACK	VERT	TOTAL	---	C1
AH	32-6-12	-121	-121	---	BACK	VERT	TOTAL	---	C1
AI	33-5-4	-121	-121	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

11 C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

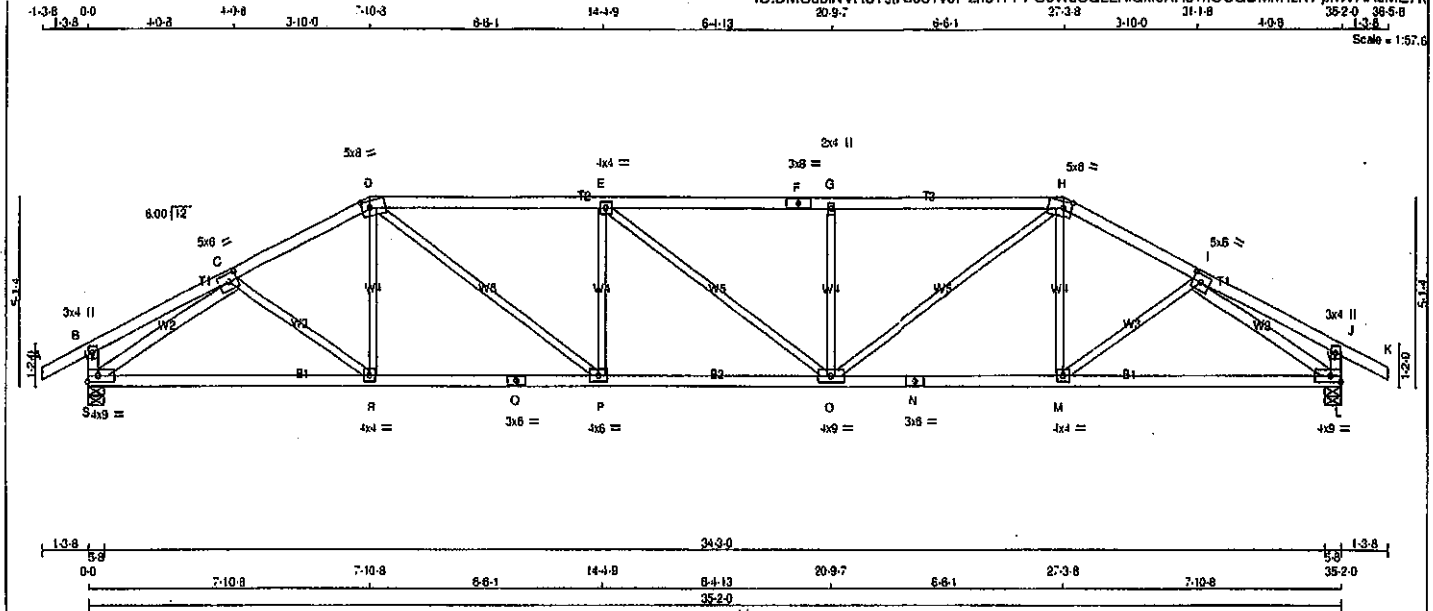


Structural component only
DWG# T-2007661 7c

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408224	T41	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 28 2019 Mitek Industries, Inc. Tue Apr 28 10:19:59 2020 Page 1
ID:DMCubINVR6TslFos31v6l zns11-PFOOWCQLZAIQXloAHu?hO3QDMnRz?PphW7IAzME7k



TOTAL WEIGHT = 2 X 139 = 278 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - O	2x4	DRY	No.2
O - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
EXCEPT			
S - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2
DRY, SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
S	2063	0	2063	0	0
L	2063	0	2063	0	0

UNFACTORED REACTIONS

1ST CASE MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
S	1457	969	0	0	0	0
L	1457	969	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (L)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (L)	
FR-TO				FR-TO			
A-B	0 28	-91.8 -91.8 0.12 (1)	10.00	C-R	0 93	0.03 (4)	
B-C	0 16	-91.8 -91.8 0.20 (1)	10.00	R-D	0 121	0.04 (4)	
C-D	-2799 0	-91.8 -91.8 0.32 (1)	3.88	D-P	0 1267	0.29 (1)	
D-E	-3508 0	-91.8 -91.8 0.95 (1)	2.94	P-E	-643 0	0.25 (1)	
E-F	-3507 0	-91.8 -91.8 0.93 (1)	2.94	E-O	-2 0	0.00 (1)	
F-G	-3507 0	-91.8 -91.8 0.93 (1)	2.94	O-G	-643 0	0.25 (1)	
G-H	-3507 0	-91.8 -91.8 0.94 (1)	2.95	O-H	0 1265	0.28 (1)	
H-I	-2799 0	-91.8 -91.8 0.32 (1)	3.88	M-H	0 121	0.04 (4)	
I-J	0 16	-91.8 -91.8 0.20 (1)	10.00	M-I	0 93	0.03 (4)	
J-K	0 28	-91.8 -91.8 0.12 (1)	10.00	S-C	-2974 0	0.83 (1)	
S-B	-270 0	0.0 0.0 0.03 (1)	7.81	I-L	-2974 0	0.83 (1)	
L-J	-270 0	0.0 0.0 0.03 (1)	7.81				
S-R	0 2417	-18.5 -18.5 0.53 (1)	10.00				
R-O	0 2489	-18.5 -18.5 0.54 (1)	10.00				
O-P	0 2489	-18.5 -18.5 0.54 (1)	10.00				
P-O	0 3509	-18.5 -18.5 0.64 (1)	10.00				
O-N	0 2489	-18.5 -18.5 0.54 (1)	10.00				
N-M	0 2489	-18.5 -18.5 0.54 (1)	10.00				
M-L	0 2417	-18.5 -18.5 0.53 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17)
CALCULATED VERT. DEFL.(LL) = L/999 (0.21)
ALLOWABLE DEFL.(TL) = L/360 (1.17)
CALCULATED VERT. DEFL.(TL) = L/999 (0.41)

CSI: TC=0.95:1.00 (D-E:1), BC=0.64:1.00 (O-P:1), WB=0.83:1.00 (I-L:1), SS=0.20:1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (I) (INPUT = 0.90)
JSI METAL = 0.77 (N) (INPUT = 1.00)

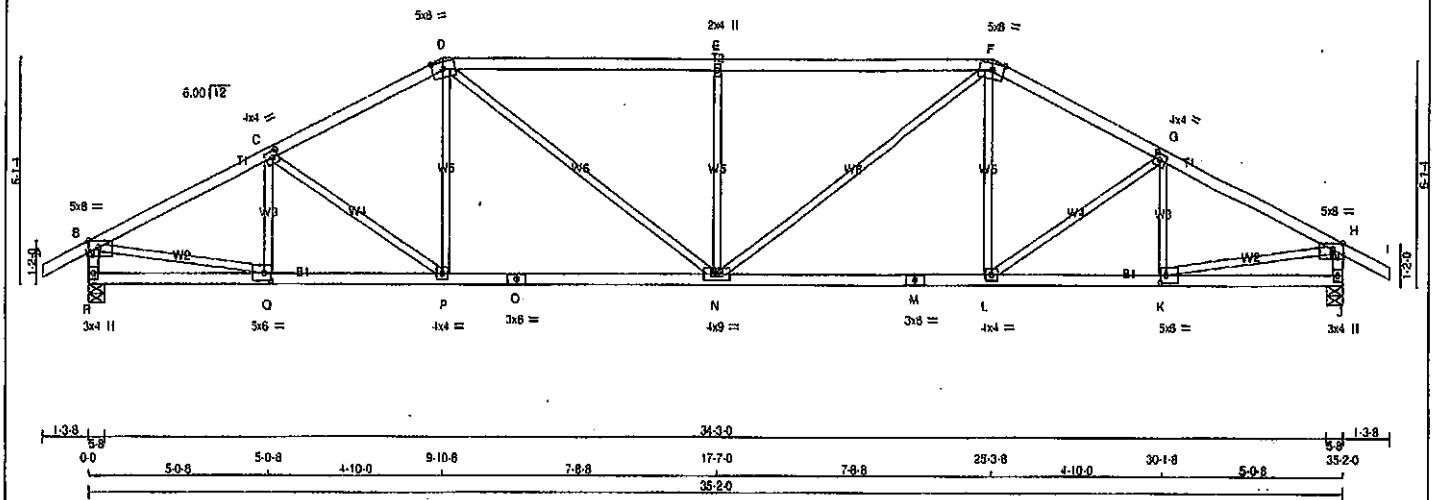
PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0	
C	TMVW-1	MT20	5.0	6.0	2.50 2.75
D	TTWW-m	MT20	5.0	8.0	2.25 2.75
E	TMVW-1	MT20	4.0	4.0	
F	YS-1	MT20	3.0	8.0	
G	TMVW-w	MT20	2.0	4.0	
H	TTWW-m	MT20	5.0	8.0	2.25 2.75
I	TMVW-1	MT20	5.0	6.0	2.50 2.75
J	TMV-p	MT20	3.0	4.0	
L	BMVW-1	MT20	4.0	9.0	Edge
M	BMVW-1	MT20	4.0	4.0	
N	SS-1	MT20	3.0	6.0	
O	BMVW-1	MT20	4.0	9.0	
P	BMVW-1	MT20	4.0	6.0	
Q	SS-1	MT20	3.0	6.0	
R	BMVW-1	MT20	4.0	4.0	
S	BMVW-1	MT20	4.0	9.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



Structural component only
DWG# T-2007662

JOB NAME 408224	TRUSS NAME T42	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 10:20:01 2020 Page 1	
ID:DMCubINVR6TsiFoe31v6I_zns1I-LdWnxHEglAOPgFSAHixTmPBjU9V?RFJIG??En3zME7i				2533 30-18 35-20 38-6 1-3-8	
Scale = 1:57.6					



LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT
SPF

DRY: SEASONED LUMBER.

PLATES (tablets in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	Edge	3.50
C TMWW-I	MT20	4.0	4.0	2.00	1.75
D TTWW-m	MT20	5.0	8.0	2.25	3.75
E TMVW-w	MT20	2.0	4.0		
F TTWW-m	MT20	5.0	8.0	2.25	3.75
G TMWW-I	MT20	4.0	4.0	2.00	1.75
H TMVW-p	MT20	5.0	8.0	Edge	3.50
J BMV1+p	MT20	3.0	4.0		
K BMVW-I	MT20	5.0	8.0	2.50	2.00
L BMVW-I	MT20	4.0	4.0		
M BS-I	MT20	3.0	6.0		
N BMVW-I	MT20	4.0	8.0		
O BS-I	MT20	3.0	6.0		
P BMVW-I	MT20	4.0	4.0		
Q BMVW-I	MT20	5.0	8.0	2.50	2.00
R BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
FACTORED	MAXIMUM FACTORED	INPUT	REQD				
GROSS REACTION	GROSS REACTION	BRG	BRG				
JT VERT HORZ	JT VERT HORZ	UPLIFT IN-SX	IN-SX				
R 2063 0	2063 0	0 5-8	5-8				
J 2063 0	2063 0	0 5-8	5-8				

UNFACTORED REACTIONS							
1ST LCASE	MAX. MIN. COMPONENT REACTIONS						
JT COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
R 1457 969 0	0 0	0 0	0 0	488 0	0 0	0 0	
J 1457 969 0	0 0	0 0	0 0	488 0	0 0	0 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FORCE	VERT. LOAD LC1	MEMB.	MAX. FACTORED	FORCE	MAX
FR-TO	(LBS)	(PLF)	CS1 (LC)	FR-TO	(LBS)	CS1 (LC)	
A-B	0 28	-91.8	-91.8 0.12 (1)	O-C	-353 0	0.08 (1)	
B-C	-2834 0	-91.8	-91.8 0.39 (1)	C-P	-210 0	0.13 (1)	
C-D	-2887 0	-91.8	-91.8 0.37 (1)	P-D	0 248	0.08 (4)	
D-E	-3080 0	-91.8	-91.8 0.67 (1)	D-N	0 844	0.19 (1)	
E-F	-3080 0	-91.8	-91.8 0.97 (1)	N-E	-872 0	0.51 (1)	
F-G	-2887 0	-91.8	-91.8 0.37 (1)	N-F	0 844	0.19 (1)	
G-H	-2834 0	-91.8	-91.8 0.39 (1)	L-F	0 248	0.06 (4)	
H-I	0 28	-91.8	-91.8 0.12 (1)	L-O	-210 0	0.13 (1)	
R-B	-2018 0	0.0	0.0 0.20 (1)	K-G	-353 0	0.08 (1)	
J-H	-2018 0	0.0	0.0 0.20 (1)	B-Q	0 2591	0.58 (1)	
R-Q	0 0	-18.5	-18.5 0.10 (4)	K-H	0 2591	0.58 (1)	
O-P	0 2555	-18.5	-18.5 0.51 (1)				
P-O	0 2388	-18.5	-18.5 0.49 (1)				
O-N	0 2388	-18.5	-18.5 0.49 (1)				
N-M	0 2388	-18.5	-18.5 0.49 (1)				
M-L	0 2388	-18.5	-18.5 0.49 (1)				
L-K	0 2555	-18.5	-18.5 0.51 (1)				
K-J	0 0	-18.5	-18.5 0.10 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 - PSF
OL = 6.0 PSF
BOT CH. LL = 0.0 PSF
OL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/999 (0.17")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = L/999 (0.34")

CS1: TC=0.97 1.00 (D-E:1), BC=0.51 1.00 (P-Q:1),
WB=0.59 1.00 (B-Q:1), SS1=0.34 1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (D) RY1 SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 364 1887 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (F) (INPUT = 0.90)

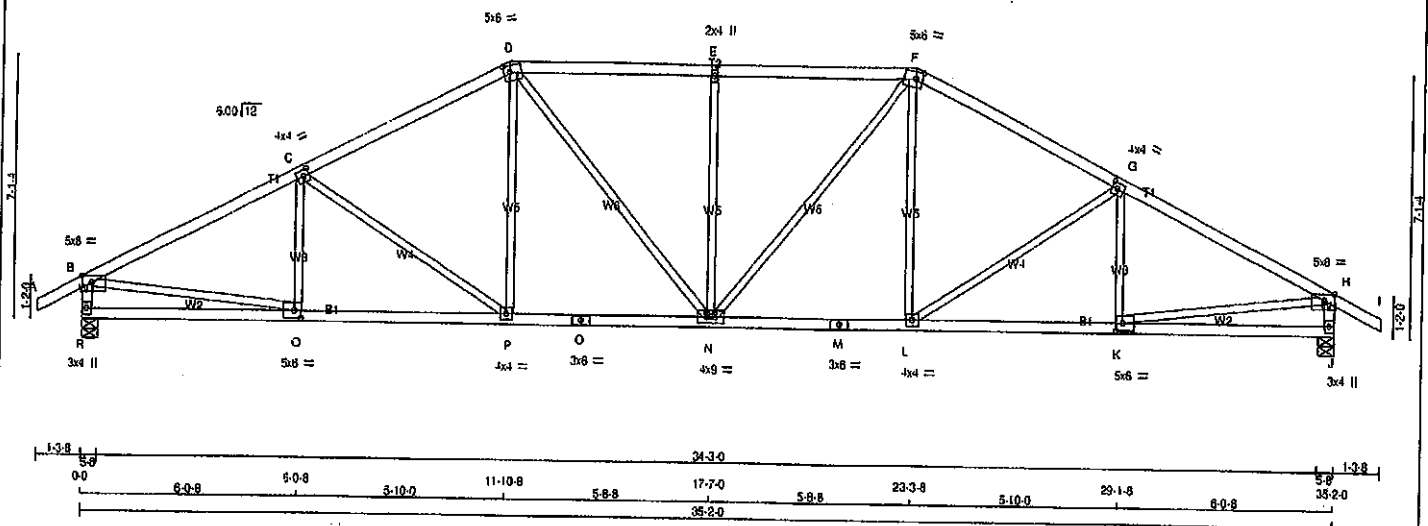
JSI METAL = 0.75 (M) (INPUT = 1.00)



Structural component only
DWG# T-2007663

JOB NAME 408224	TRUSS NAME T43	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:20:02 2020 Page 1
 ID:DMCubINVR6TstFoe31v6l_zns1f-qd498dElsUYQHO1NtQSIJh7BZVAKQsViknJVzME7h
 35-2-0 30-5-8 1-3-8
 Scale = 1:57.6



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-p	MT20	5.0	8.0 Edge 3.50
C	TMVW-l	MT20	4.0	4.0 2.00 1.75
D	TTWW-m	MT20	5.0	6.0 2.25 2.00
E	TMVW-w	MT20	2.0	4.0
F	TTWW-m	MT20	5.0	6.0 2.25 2.00
G	TMVW-l	MT20	4.0	4.0 2.00 1.75
H	TMVW-p	MT20	5.0	8.0 Edge 3.50
J	BMV1-p	MT20	3.0	4.0
K	BMVW-l	MT20	5.0	6.0 2.50 2.25
L	BMVW-l	MT20	4.0	4.0
M	BS-l	MT20	3.0	6.0
N	BMVW-w	MT20	4.0	9.0
O	BS-l	MT20	3.0	6.0
P	BMVW-l	MT20	4.0	4.0
Q	BMVW-l	MT20	5.0	6.0 2.50 2.25
R	BMV1-p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
R	2063 0	2063 0	0 0	5-8	5-8
J	2063 0	2063 0	0 0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	COMBINED	989 0	0 0	0 0	0 0	488 0	0 0	0 0
J	COMBINED	989 0	0 0	0 0	0 0	488 0	0 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (PLF)	MEMB.	FORCE (LBS)
FR-TO				FR-TO	
A-B	0 28	-91.8	-91.8 0.12 (1)	Q-C	-255 12
B-C	-2889 0	-91.8	-91.8 0.58 (1)	C-P	-435 0
C-D	-2539 0	-91.8	-91.8 0.52 (1)	P-D	0 350
D-E	-2559 0	-91.8	-91.8 0.48 (1)	D-N	0 482
E-F	-2559 0	-91.8	-91.8 0.48 (1)	N-E	-841 0
F-G	-2539 0	-91.8	-91.8 0.52 (1)	N-F	0 482
G-H	-2889 0	-91.8	-91.8 0.58 (1)	L-F	0 350
H-I	0 28	-91.8	-91.8 0.12 (1)	L-G	-435 0
R-B	-2014 0	0.0	0.0 0.20 (1)	K-G	-265 12
J-H	-2014 0	0.0	0.0 0.20 (1)	B-Q	0 2635
R-O	0 0	-18.5	-18.5 0.15 (4)	K-H	0 2635
Q-P	0 2809	-18.5	-18.5 0.49 (1)		
P-O	0 2249	-18.5	-18.5 0.43 (1)		
O-N	0 2249	-18.5	-18.5 0.43 (1)		
N-M	0 2249	-18.5	-18.5 0.43 (1)		
M-L	0 2249	-18.5	-18.5 0.43 (1)		
L-K	0 2609	-18.5	-18.5 0.49 (1)		
K-J	0 0	-18.5	-18.5 0.15 (4)		

TOTAL WEIGHT = 2 X 145 = 289 lb
 (M)(F)

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 OL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 OL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(LL) = L/899 (0.15")
 ALLOWABLE DEFL.(TL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(TL) = L/899 (0.28")

CSI: TC=0.58/1.00 (G-H:1), SC=0.49/1.00 (K-L:1),
 WB=0.59/1.00 (H-K:1), SS=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

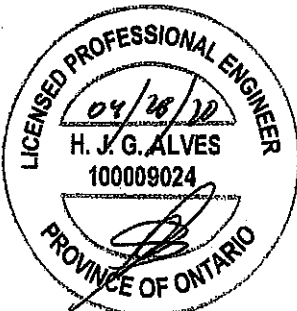
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	616 354	1667 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (Q) (INPUT = 0.90)
 JSI METAL= 0.69 (M) (INPUT = 1.00)



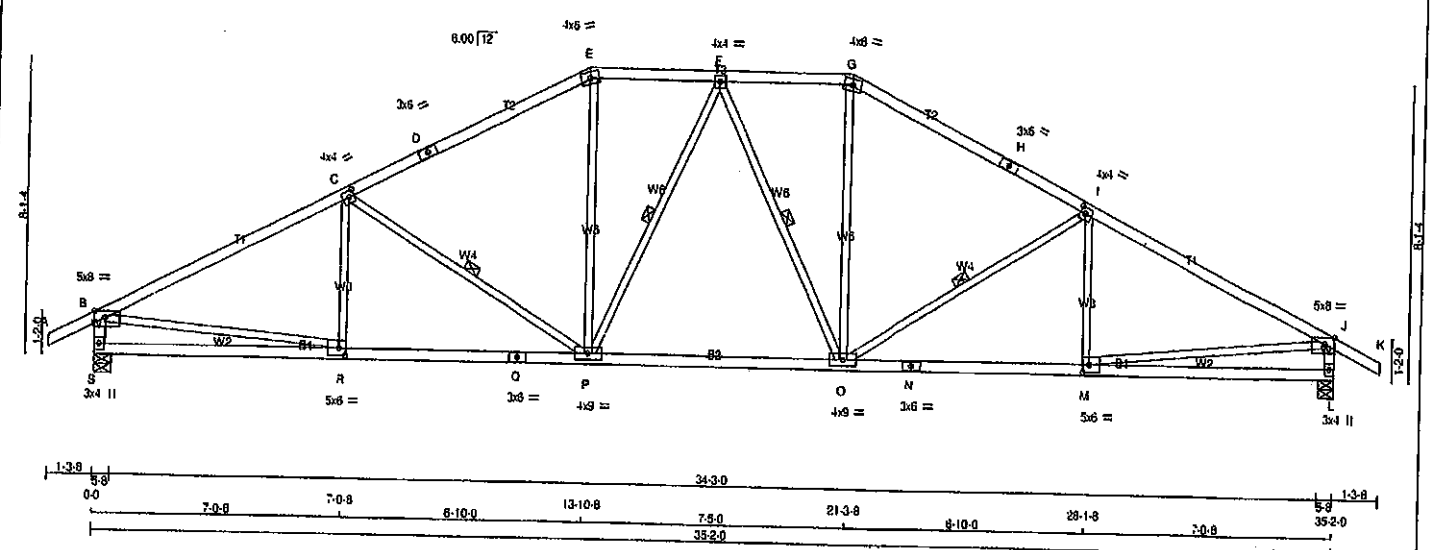
Structural component only
 DWG# T-2007664

JOB NAME: 408224 TRUSS NAME: T44 QUANTITY: 2 PLV: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC: T44 DRWG NO. 35-20 36-5-8

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:20:03 2020 Page 1

ID:DMCubINVR6TstF0e31v6l zns11-10dXMzFwOog7vYcZP7zxsEE72zACv9c7HdUKxzME7G

Scale = 1:58.0



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - E	2x4	DRY No.2
E - G	2x4	DRY No.2
G - H	2x4	DRY No.2
H - K	2x4	DRY No.2
S - B	2x4	DRY No.2
L - J	2x4	DRY No.2
S - Q	2x4	DRY No.2
Q - N	2x4	DRY No.2
N - L	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMVW-I	MT20	4.0	4.0	2.00	1.75
D	TS-I	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	8.0		
F	TMVW-I	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	8.0		
H	TS-I	MT20	3.0	6.0		
I	TMVW-I	MT20	4.0	4.0	2.00	1.75
J	TMVW-p	MT20	5.0	8.0	Edge	3.50
L	BMV1-p	MT20	3.0	4.0		
M	BMVW-I	MT20	5.0	6.0	2.50	2.25
N	BS-I	MT20	3.0	8.0		
O	BMVW-I	MT20	4.0	9.0		
P	BMVW-I	MT20	4.0	9.0		
Q	BS-I	MT20	3.0	8.0		
R	BMVW-I	MT20	5.0	8.0	2.50	2.25
S	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
S	2083	0	2083	0	5-8	5-8
L	2083	0	2083	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND DEAD SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE	
S	1457	969.0	0.0	0.0	488 0 0 0
L	1457	969.0	0.0	0.0	488 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-P, F-P, F-D, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD LC1 MAX (PLF)
FR-TO				
A-B	0 20	-91.8 -91.8 0.12 (1)	10.00	R-C -182 52 0.08 (1)
B-C	-2908 0	-91.8 -91.8 0.83 (1)	3.25	C-P -837 0 0.29 (1)
C-D	-2373 0	-91.8 -91.8 0.72 (1)	3.68	P-E 0 874 0.16 (1)
D-E	-2373 0	-91.8 -91.8 0.72 (1)	3.68	F-F -222 0 0.11 (1)
E-F	-2100 0	-91.8 -91.8 0.19 (1)	4.53	F-O -222 0 0.11 (1)
F-G	-2100 0	-91.8 -91.8 0.19 (1)	4.53	O-G 0 674 0.15 (1)
G-H	-2373 0	-91.8 -91.8 0.72 (1)	3.68	O-I -637 0 0.29 (1)
H-I	-2373 0	-91.8 -91.8 0.72 (1)	3.68	M-I -182 52 0.08 (1)
I-J	-2908 0	-91.8 -91.8 0.83 (1)	3.25	B-R 0 2649 0.80 (1)
J-K	0 20	-91.8 -91.8 0.12 (1)	10.00	M-J 0 2649 0.80 (1)
S-B	-2008 0	0.0 0.0 0.20 (1)	5.96	
L-J	-2008 0	0.0 0.0 0.20 (1)	5.96	
S-R	0 0	-18.5 -18.5 0.20 (4)	10.00	
R-O	0 2830	-18.5 -18.5 0.52 (1)	10.00	
O-P	0 2830	-18.5 -18.5 0.52 (1)	10.00	
P-Q	0 2183	-18.5 -18.5 0.45 (1)	10.00	
Q-N	0 2830	-18.5 -18.5 0.52 (1)	10.00	
N-M	0 2830	-18.5 -18.5 0.52 (1)	10.00	
M-L	0 0	-18.5 -18.5 0.20 (4)	10.00	

TOTAL WEIGHT = 2 X 145 = 291 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.0/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")

CALCULATED VERT. DEFL.(LL) = L/999 (0.14")

ALLOWABLE DEFL.(TL) = L/360 (1.17")

CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.83/1.00 (H-I), BC=0.52/1.00 (M-O-1), WB=0.60/1.00 (J-M-1), SS=0.28/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

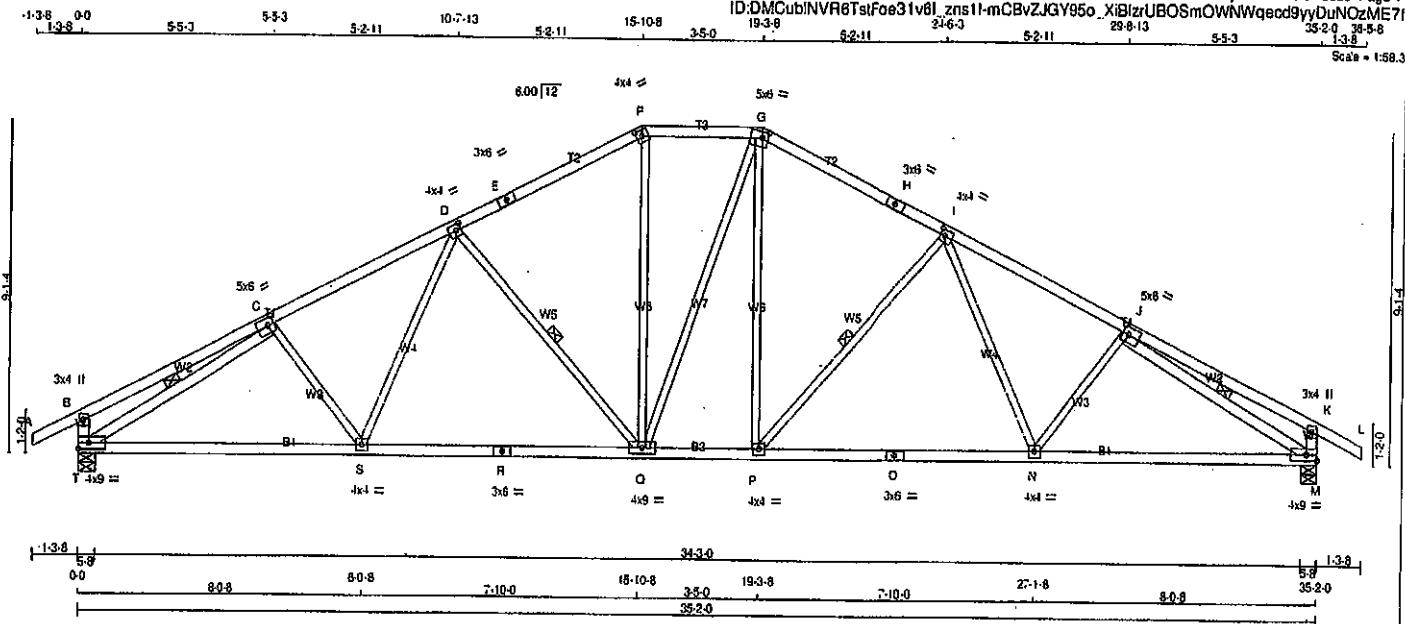
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (M) (INPUT = 0.90)

JSI METAL = 0.79 (Q) (INPUT = 1.00)



Structural component only
DWG# T-2007665



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2
F - G	2x4	DRY	No.2
H - I	2x4	DRY	No.2
J - L	2x4	DRY	No.2
M - K	2x4	DRY	No.2
N - R	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
T - M	2x4	DRY	No.2
O - M	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
T - G	2x4	DRY	No.2
J - M	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tab/size in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - E	MT20	3.0	4.0		
B - C	MT20	5.0	6.0	2.50	2.25
D - E	MT20	4.0	4.0	2.00	1.50
F - G	MT20	3.0	6.0		
H - I	MT20	4.0	4.0	2.00	1.75
J - K	MT20	5.0	6.0	2.00	2.00
L - M	MT20	3.0	6.0		
N - P	MT20	4.0	4.0	2.00	1.50
Q - R	MT20	5.0	6.0	2.50	2.25
S - T	MT20	3.0	4.0		
U - V	MT20	4.0	9.0		Edge
W - X	MT20	4.0	4.0		
Y - Z	MT20	3.0	6.0		
AA - BB	MT20	4.0	9.0		Edge
CC - DD	MT20	3.0	6.0		
EE - FF	MT20	4.0	9.0		Edge
GG - HH	MT20	3.0	6.0		
II - JJ	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	DOWN	HRZ	UPLIFT
T	2083	0	0	5-8
M	2083	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
T	1457	988 0
M	1457	988 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1:2 LENGTH OF D-Q, P-P, C-T, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	MAX. FACTORED UNBRACED LENGTH (FT)
FR-TO				FR-TO			
A-B	0 28	-91.8	-91.8 0.12 (1)	10.00	C-S	-110 37	0.04 (1)
B-C	0 20	-91.8	-91.8 0.32 (1)	10.00	S-D	0 276	0.08 (1)
C-D	-2760 0	-91.8	-91.8 0.40 (1)	3.88	D-Q	-892 0	0.31 (1)
D-E	-2167 0	-91.8	-91.8 0.36 (1)	4.30	Q-F	0 614	0.14 (1)
E-F	-2167 0	-91.8	-91.8 0.36 (1)	4.30	F-G	0 4	0.00 (1)
F-G	-1925 0	-91.8	-91.8 0.18 (1)	4.71	G-H	0 609	0.14 (1)
G-H	-2165 0	-91.8	-91.8 0.36 (1)	4.30	H-I	-883 0	0.31 (1)
H-I	-2165 0	-91.8	-91.8 0.36 (1)	4.30	I-N	0 279	0.08 (1)
I-J	-2761 0	-91.8	-91.8 0.40 (1)	3.88	N-J	-110 37	0.04 (1)
J-K	0 20	-91.8	-91.8 0.32 (1)	10.00	T-C	-3042 0	0.61 (1)
K-L	0 28	-91.8	-91.8 0.12 (1)	10.00	J-M	-3043 0	0.61 (1)
T-B	-325 0	0.0	0.0 0.03 (1)	7.81			
M-K	-325 0	0.0	0.0 0.03 (1)	7.81			
T-S	0 2537	-18.5	-18.5 0.58 (1)	10.00			
S-R	0 2363	-18.5	-18.5 0.53 (1)	10.00			
R-Q	0 2363	-18.5	-18.5 0.53 (1)	10.00			
Q-P	0 1924	-18.5	-18.5 0.40 (1)	10.00			
P-O	0 2362	-18.5	-18.5 0.53 (1)	10.00			
O-N	0 2362	-18.5	-18.5 0.53 (1)	10.00			
N-M	0 2538	-18.5	-18.5 0.58 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 8.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CSI: TC=0.40/1.00 (J-J), BC=0.58/1.00 (M-N), WB=0.61/1.00 (J-M), SS=0.20/1.00 (G-I)

00L LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
MT20	1657	788
MT20	1987	1856

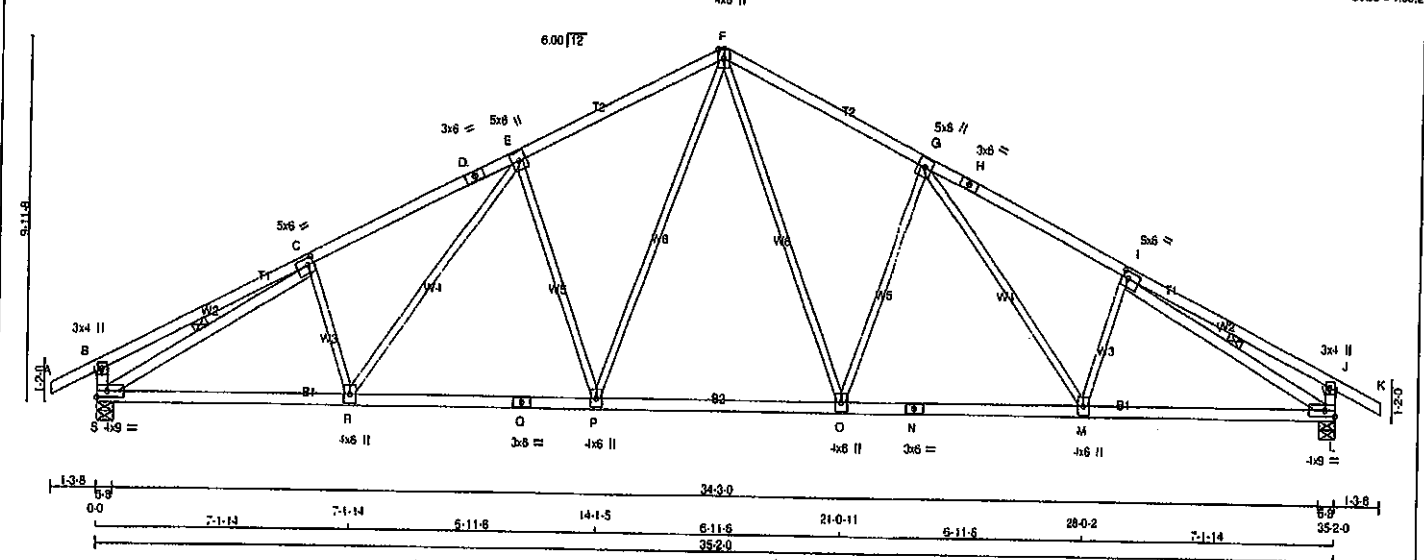
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (F) (INPUT = 0.90)

JSI METAL = 0.83 (O) (INPUT = 1.00)





LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESGR.

A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT S - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-1	MT20	5.0	8.0	2.25	2.00
D	TS-1	MT20	3.0	6.0		
E	TMWW-1	MT20	5.0	8.0		
F	TTWW+p	MT20	4.0	8.0	Edge	
G	TMWW-1	MT20	5.0	8.0		
H	TS-1	MT20	3.0	6.0		
I	TMWW-1	MT20	5.0	8.0	2.25	2.00
J	TMV+p	MT20	3.0	4.0		
L	BMVW-1	MT20	4.0	9.0	Edge	
M, O, P, R						
M	BMWW-1	MT20	4.0	8.0		
N	BS-1	MT20	3.0	6.0		
Q	BS-1	MT20	3.0	6.0		
S	BMVW-1	MT20	4.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
S	2063	0	2063	0
L	2063	0	2063	0

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS
	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
JT	1457	989 0 0 0 0 0 0
S	1457	989 0 0 0 0 0 0
L	1457	989 0 0 0 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.
 1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-S, I-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 28	-91.8 -91.8 0.12 (1)	10.00	F-O	0 868	0.19 (1)	
B-C	0 22	-91.8 -91.8 0.40 (1)	10.00	O-G	-728 0	0.72 (1)	
C-D	-2808 0	-91.8 -91.8 0.49 (1)	3.75	G-M	0 391	0.09 (1)	
D-E	-2808 0	-91.8 -91.8 0.49 (1)	3.75	M-I	-192 18	0.05 (1)	
E-F	-2322 0	-91.8 -91.8 0.46 (1)	4.07	P-F	0 868	0.19 (1)	
F-G	-2322 0	-91.8 -91.8 0.46 (1)	4.07	E-P	-728 0	0.72 (1)	
G-H	-2808 0	-91.8 -91.8 0.49 (1)	3.75	R-E	0 391	0.09 (1)	
H-I	-2808 0	-91.8 -91.8 0.49 (1)	3.75	C-R	-192 18	0.05 (1)	
I-J	0 22	-91.8 -91.8 0.40 (1)	10.00	S-C	-3084 0	0.72 (1)	
J-K	0 28	-91.8 -91.8 0.12 (1)	10.00	I-L	-3084 0	0.72 (1)	
S-B	-344 0	0.0 0.0 0.03 (1)	7.81				
L-J	-344 0	0.0 0.0 0.03 (1)	7.81				
S-R	0 2574	-18.5 -18.5 0.53 (1)	10.00				
R-O	0 2291	-18.5 -18.5 0.47 (1)	10.00				
Q-P	0 2291	-18.5 -18.5 0.47 (1)	10.00				
P-O	0 1782	-18.5 -18.5 0.38 (1)	10.00				
O-N	0 2291	-18.5 -18.5 0.47 (1)	10.00				
N-M	0 2291	-18.5 -18.5 0.47 (1)	10.00				
M-L	0 2574	-18.5 -18.5 0.53 (1)	10.00				

TOTAL WEIGHT = 8 X 152 = 1213 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 PART 9 OF CBC 2018, CBC 2012, ABC 2019
 PART 9 OF CBC 2012 (2019 AMENDMENT)
 CSA 886-09, CSA 886-14
 TPIC 2011, TPIC 2014

155' OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17')
 CALCULATED VERT. DEFL. (LL) = L/999 (0.14')
 ALLOWABLE DEFL. (TL) = L/360 (1.17')
 CALCULATED VERT. DEFL. (TL) = L/999 (0.28')

CSI: TC=0.49/1.00 (C-E-1), BC=0.53/1.00 (L-M-1), WB=0.72/1.00 (E-P-1), SS=0.22/1.00 (I-J-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1687
	618	354	1687

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (C) (INPUT = 0.90)
 JSI METAL = 0.77 (I) (INPUT = 1.00)

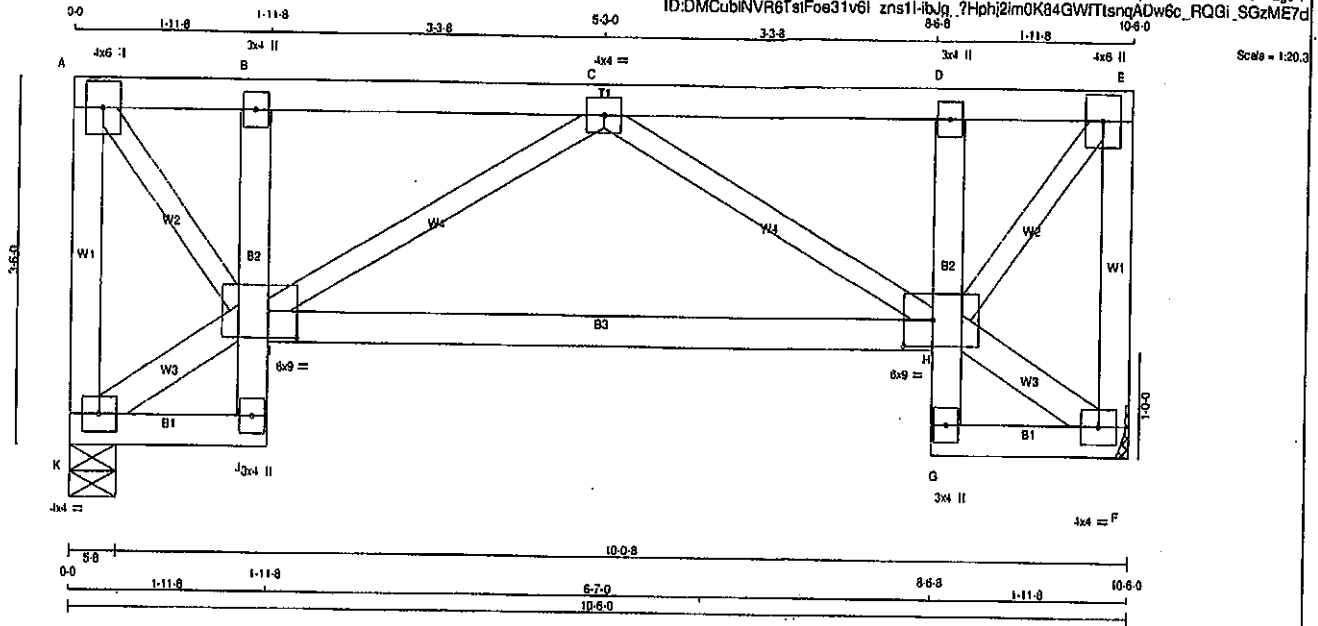


Structural component only
 DWG# T-2007667

JOB NAME 408224	TRUSS NAME T47S	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
--------------------	--------------------	---------------	----------	-------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.310 5 Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:20:06 2020 Page 1
ID:DMCubINVR6TstFoe31v6I zns11-bjg_?Hphj2im0K84GWttsngADw6c_RQGI_SGzME7d



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
K - A	2x4 DRY	No.2	SPF		
A - E	2x4 DRY	No.2	SPF		
F - E	2x4 DRY	No.2	SPF		
K - J	2x4 DRY	No.2	SPF		
J - B	2x4 DRY	No.2	SPF		
I - H	2x4 DRY	No.2	SPF		
G - D	2x4 DRY	No.2	SPF		
G - F	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					
K - I	2x4 DRY	No.2	SPF		
H - F	2x4 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0		
B	TMV+p	MT20	3.0	4.0		
C	TMVW-1	MT20	4.0	4.0		
D	TMV+p	MT20	3.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMVW-1	MT20	4.0	4.0		
G	BMV+p	MT20	3.0	4.0		
H	BMVW-1	MT20	6.0	9.0	3.00	3.50
I	BMVW-1	MT20	6.0	9.0	3.00	3.50
J	BMV+p	MT20	3.0	4.0		
K	BMVW-1	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORZ	DOWN	HORZ
K 697	0	697	0
F 697	0	697	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	504	289.0	0.0	0.0	0.0	235.0	0.0
F	504	289.0	0.0	0.0	0.0	235.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 0.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
K-A	-667	0	0.0	0.0	0.17 (1)	7.81	K-I	-28	0
A-B	-497	0	-114.3	-114.3	0.14 (1)	6.25	A-I	0	783
B-C	-508	0	-114.3	-114.3	0.22 (1)	6.25	H-F	-28	0
C-D	-508	0	-114.3	-114.3	0.22 (1)	6.25	H-E	0	783
D-E	-497	0	-114.3	-114.3	0.14 (1)	6.25	I-C	-384	0
E-F	-667	0	0.0	0.0	0.17 (1)	7.81	C-H	-384	0
K-J	0	25	-18.5	-18.5	0.03 (4)	10.00			
J-I	0	19	0.0	0.0	0.05 (1)	10.00			
I-B	-331	0	0.0	0.0	0.05 (1)	7.81			
I-H	0	832	-18.5	-18.5	0.48 (4)	10.00			
G-H	0	19	0.0	0.0	0.05 (1)	10.00			
H-D	-331	0	0.0	0.0	0.05 (1)	7.81			
G-F	0	25	-18.5	-18.5	0.03 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 15.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 48.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.22/1.00 (B-C:1), BC=0.46/1.00 (H-I:4), WB=0.24/1.00 (E-H:1), SS=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.77 (H) INPUT = 0.90
JSI METAL = 0.30 (E) INPUT = 1.00

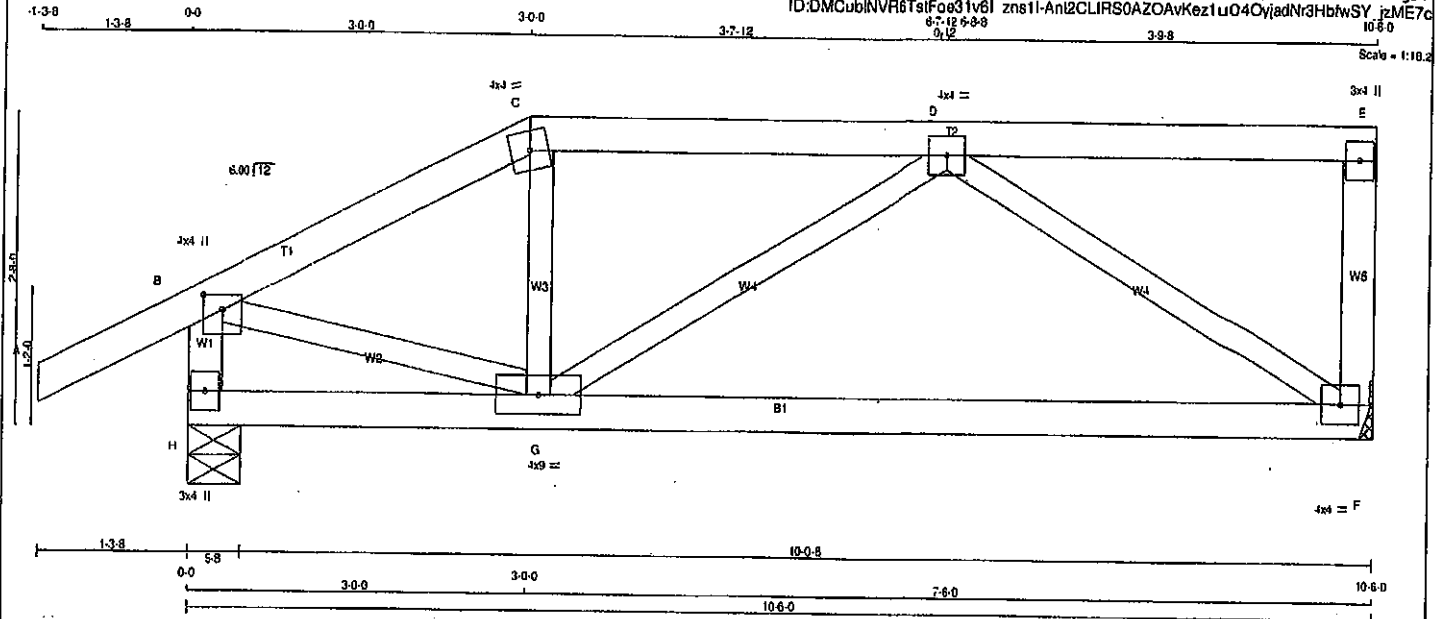


Structural component only
DWG# T-2007668

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408224	T48	1	1	TRUSS DESC.		

Tamack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mtek Industries, Inc. Tue Apr 28 10:20:07 2020 Page 1
ID:DMCubINVR6Tsf0e31v6I zns11-An12CLIRS0AZOAvKez1u04OvjadNr3HbWsy izME7c
6-7-12 5-9-8 0-12 10-5-0
Scale = 1:10.2



LUMBER						
N. L. G. A. RULES						
CHORDS		SIZE		LUMBER		DESCR
A - C	2x4	DRY	No.2			SPF
C - E	2x4	DRY	No.2			SPF
F - H	2x4	DRY	No.2			SPF
H - B	2x4	DRY	No.2			SPF
H - F	2x4	DRY	No.2			SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	PLATES	W	LEN	Y	X
JT TYPE	MT20	4.0	4.0	1.50	2.00
B	MT20	4.0	4.0		
C	MT20	4.0	4.0		
D	MT20	4.0	4.0		
E	MT20	4.0	4.0		
F	MT20	4.0	4.0		
G	MT20	4.0	4.0		
H	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
F	579	0	0	0
H	703	0	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
F	409	289	0	0	0
H	495	338	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	W E B S	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO								
A-B	0	28	-91.8	0.12 (1)	10.00	G-C	0	61
B-C	-835	0	-91.8	0.15 (1)	6.25	G-D	-61	60
C-D	-569	0	-91.8	0.21 (1)	8.25	D-F	-731	0
D-E	0	0	-91.8	0.20 (1)	10.00	B-G	0	587
E-F	-135	0	0.0	0.02 (1)	7.81			
H-B	-899	0	0.0	0.07 (1)	7.81			
H-G	0	0	-18.5	-18.5	0.21 (4)	10.00		
G-F	0	619	-18.5	-18.5	0.26 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.21-1.00 (C-D:1), BC=0.25-1.00 (F-G:4), WB=0.23-1.00 (D-F:1), SSI=0.17-1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

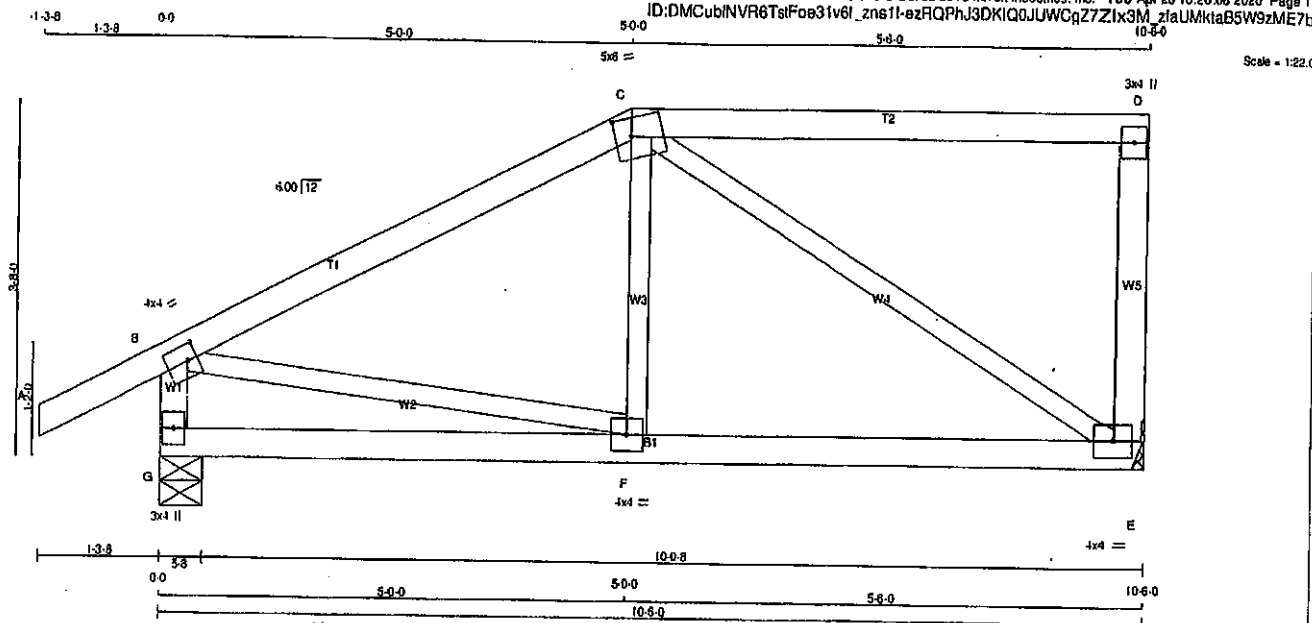
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (B) (INPUT = 0.50)
JSI METAL = 0.23 (F) (INPUT = 1.00)



Structural component only
DWG# T-2007669

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	T49	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:20:08 2020 Page 1	



LUMBER	SIZE	DRY	LUMBER
N. L. G. A. RULES			
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (Tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
E	579	0	579	0
G	703	0	703	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	409	289 0	0 0	0 0	0 0	141 0	0 0
G	495	338 0	0 0	0 0	0 0	157 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
FR-TO													
A-B	0	28	-91.8	-91.8	0.12 (1)	10.00	F-C	0	108	0.04 (4)			
B-C	-523	0	-91.8	-91.8	0.41 (1)	8.25	C-E	-549	0	0.37 (1)			
C-D	0	0	-91.8	-91.8	0.47 (1)	10.00	B-F	0	474	0.11 (1)			
E-D	-252	0	0.0	0.0	0.05 (1)	7.81							
G-B	-667	0	0.0	0.0	0.07 (1)	7.81							
G-F	0	0	-18.5	-18.5	0.14 (4)	10.00							
F-E	0	486	-18.5	-18.5	0.19 (4)	10.00							

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.35")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.35")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CSI: TC=0.47 (1.00 (C-D)), BC=0.19 (1.00 (E-F)), WB=0.37 (1.00 (C-E)), SS=0.20 (1.00 (C-D))

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1687 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.85 (B) (INPUT = 0.90)
JSI METAL=0.28 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007670

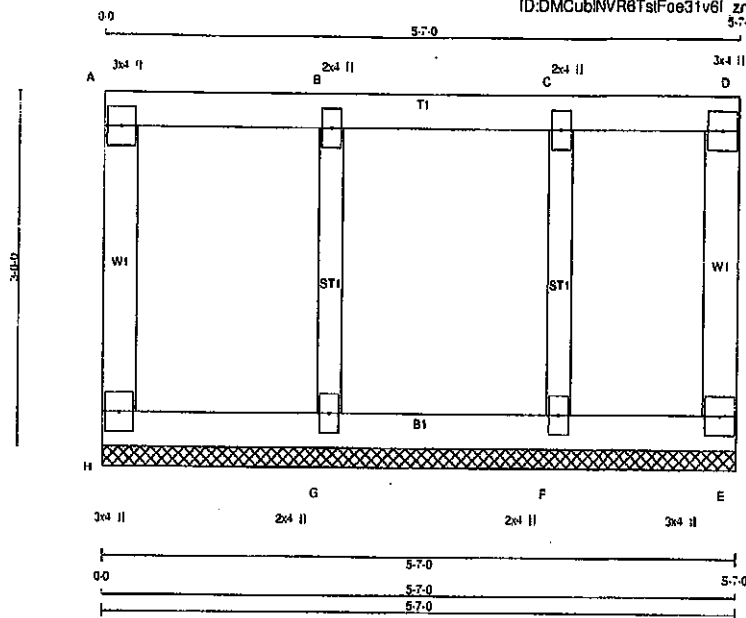
Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 10:20:10 2020 Page 1
ID:DMCub1NVR6Tsf0e31v6l zns11-bMYBqMKJlxY8FdevJ5bbeJOKcoeV2T1LugCb2zME7Z



Structural component only
DWG# T-2007671

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	G50	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mtek Industries, Inc. Tue Apr 28 10:19:51 2020 Page 1
ID:DMCubINVR8TslFoe31v6I zns1t-eiv?r6OD69qTthFid7MikFo7CC5i2EzRaiQezME7s



Scale = 1/16" = 1'

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
H - A	2x4 DRY	No.2	SPF
A - D	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF
H - E	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
ALL GABLE WEBS	2x3 DRY	No.2	SPF
DRY-SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 O.C.

PLATES (tablets in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMV+p	MT20	3.0	4.0
B	TMW+w	MT20	2.0	4.0
C	TMV+p	MT20	2.0	4.0
D	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0
F	BMV1+w	MT20	2.0	4.0
G	BMV1+w	MT20	2.0	4.0
H	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S).

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LBS)	MAX. FACTORED (LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
H-A	-101 0	0.0	0.0 0.03 (1)	7.81	G-B	-248 0	0.05 (1)
A-B	-8 0	-114.3	-114.3 0.08 (1)	10.00	F-C	-209 0	0.05 (1)
B-C	-8 0	-114.3	-114.3 0.08 (1)	10.00			
C-D	-8 0	-114.3	-114.3 0.08 (1)	10.00			
E-D	-80 0	0.0	0.0 0.03 (1)	7.81			
H-G	0 8	-18.5	-18.5 0.02 (4)	10.00			
G-F	0 8	-18.5	-18.5 0.02 (4)	10.00			
F-E	0 8	-18.5	-18.5 0.02 (4)	10.00			

TOTAL WEIGHT = 22 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 15.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 48.0 PSF

SPACING = 24.0 IN. G.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (A-B:1), BC=0.02/1.00 (G-H:1), WB=0.08/1.00 (B-G:1), SS=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	518 354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (G) INPUT = 0.90
JSI METAL= 0.07 (G) INPUT = 1.00



Structural component only
DWG# T-2007656

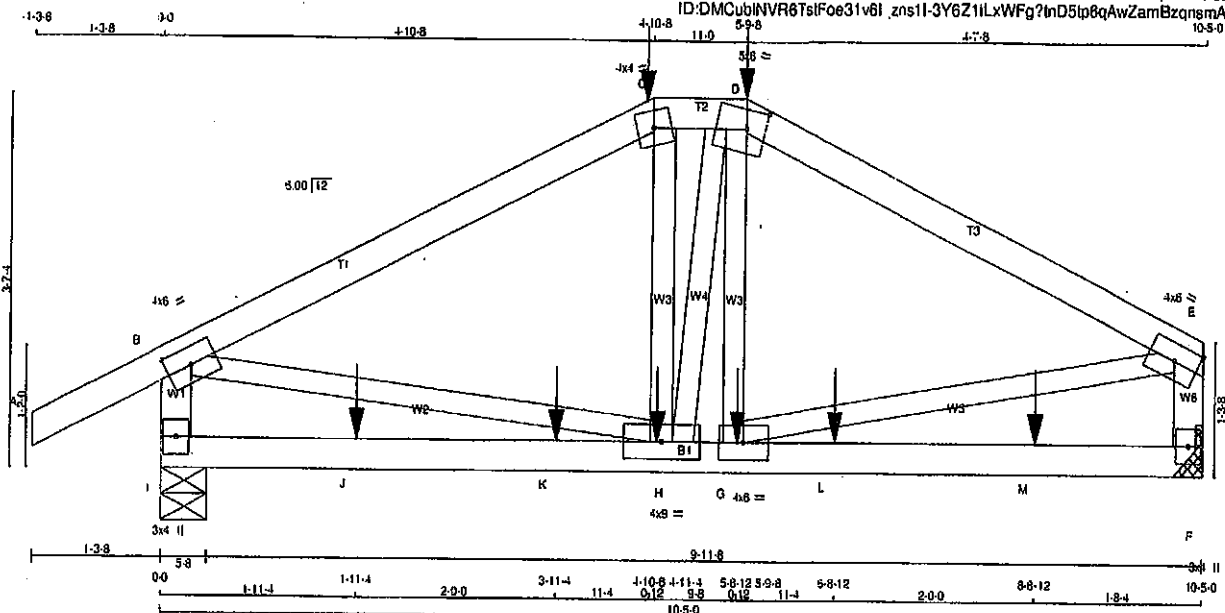
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408224	T51	1	1	TRUSS DESC.		

Tamrack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:20:11 2020 Page 1

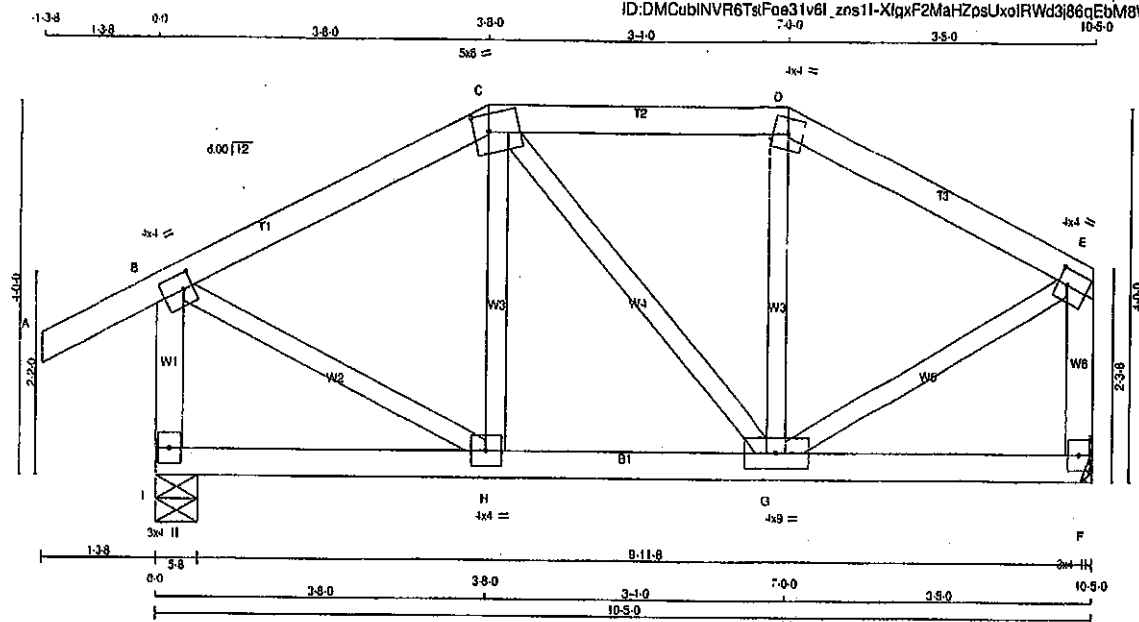
ID:DMCubINVR6TslFoe31v6I_zns11-3Y6Z1ILxWfg?InD5ip8qAwZamBzqnsnMAZYQIUzME7Y

Scale = 1:20.5



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
K - L	2x4	DRY	No.2	SPF
L - M	2x4	DRY	No.2	SPF
M - N	2x4	DRY	No.2	SPF
N - O	2x4	DRY	No.2	SPF
O - P	2x4	DRY	No.2	SPF
P - Q	2x4	DRY	No.2	SPF
Q - R	2x4	DRY	No.2	SPF
R - S	2x4	DRY	No.2	SPF
S - T	2x4	DRY	No.2	SPF
T - U	2x4	DRY	No.2	SPF
U - V	2x4	DRY	No.2	SPF
V - W	2x4	DRY	No.2	SPF
W - X	2x4	DRY	No.2	SPF
X - Y	2x4	DRY	No.2	SPF
Y - Z	2x4	DRY	No.2	SPF
Z - AA	2x4	DRY	No.2	SPF
AA - AB	2x4	DRY	No.2	SPF
AB - AC	2x4	DRY	No.2	SPF
AC - AD	2x4	DRY	No.2	SPF
AD - AE	2x4	DRY	No.2	SPF
AE - AF	2x4	DRY	No.2	SPF
AF - AG	2x4	DRY	No.2	SPF
AG - AH	2x4	DRY	No.2	SPF
AH - AI	2x4	DRY	No.2	SPF
AI - AJ	2x4	DRY	No.2	SPF
AJ - AK	2x4	DRY	No.2	SPF
AK - AL	2x4	DRY	No.2	SPF
AL - AM	2x4	DRY	No.2	SPF
AM - AN	2x4	DRY	No.2	SPF
AN - AO	2x4	DRY	No.2	SPF
AO - AP	2x4	DRY	No.2	SPF
AP - AQ	2x4	DRY	No.2	SPF
AQ - AR	2x4	DRY	No.2	SPF
AR - AS	2x4	DRY	No.2	SPF
AS - AT	2x4	DRY	No.2	SPF
AT - AU	2x4	DRY	No.2	SPF
AU - AV	2x4	DRY	No.2	SPF
AV - AW	2x4	DRY	No.2	SPF
AW - AX	2x4	DRY	No.2	SPF
AX - AY	2x4	DRY	No.2	SPF
AY - AZ	2x4	DRY	No.2	SPF
AZ - BA	2x4	DRY	No.2	SPF
BA - BB	2x4	DRY	No.2	SPF
BB - BC	2x4	DRY	No.2	SPF
BC - BD	2x4	DRY	No.2	SPF
BD - BE	2x4	DRY	No.2	SPF
BE - BF	2x4	DRY	No.2	SPF
BF - BG	2x4	DRY	No.2	SPF
BG - BH	2x4	DRY	No.2	SPF
BH - BI	2x4	DRY	No.2	SPF
BI - BJ	2x4	DRY	No.2	SPF
BJ - BK	2x4	DRY	No.2	SPF
BK - BL	2x4	DRY	No.2	SPF
BL - BM	2x4	DRY	No.2	SPF
BM - BN	2x4	DRY	No.2	SPF
BN - BO	2x4	DRY	No.2	SPF
BO - BP	2x4	DRY	No.2	SPF
BP - BQ	2x4	DRY	No.2	SPF
BQ - BR	2x4	DRY	No.2	SPF
BR - BS	2x4	DRY	No.2	SPF
BS - BT	2x4	DRY	No.2	SPF
BT - BU	2x4	DRY	No.2	SPF
BU - BV	2x4	DRY	No.2	SPF
BV - BW	2x4	DRY	No.2	SPF
BW - BX	2x4	DRY	No.2	SPF
BX - BY	2x4	DRY	No.2	SPF
BY - BZ	2x4	DRY	No.2	SPF
BZ - CA	2x4	DRY	No.2	SPF
CA - CB	2x4	DRY	No.2	SPF
CB - CC	2x4	DRY	No.2	SPF
CC - CD	2x4	DRY	No.2	SPF
CD - CE	2x4	DRY	No.2	SPF
CE - CF	2x4	DRY	No.2	SPF
CF - CG	2x4	DRY	No.2	SPF
CG - CH	2x4	DRY	No.2	SPF
CH - CI	2x4	DRY	No.2	SPF
CI - CJ	2x4	DRY	No.2	SPF
CJ - CK	2x4	DRY	No.2	SPF
CK - CL	2x4	DRY	No.2	SPF
CL - CM	2x4	DRY	No.2	SPF
CM - CN	2x4	DRY	No.2	SPF
CN - CO	2x4	DRY	No.2	SPF
CO - CP	2x4	DRY	No.2	SPF
CP - CQ	2x4	DRY	No.2	SPF
CQ - CR	2x4	DRY	No.2	SPF
CR - CS	2x4	DRY	No.2	SPF
CS - CT	2x4	DRY	No.2	SPF
CT - CU	2x4	DRY	No.2	SPF
CU - CV	2x4	DRY	No.2	SPF
CV - CW	2x4	DRY	No.2	SPF
CW - CX	2x4	DRY	No.2	SPF
CX - CY	2x4	DRY	No.2	SPF
CY - CZ	2x4	DRY	No.2	SPF
CZ - DA	2x4	DRY	No.2	SPF
DA - DB	2x4	DRY	No.2	SPF
DB - DC	2x4	DRY	No.2	SPF
DC - DD	2x4	DRY	No.2	SPF
DD - DE	2x4	DRY	No.2	SPF
DE - DF	2x4	DRY	No.2	SPF
DF - DG	2x4	DRY	No.2	SPF
DG - DH	2x4	DRY	No.2	SPF
DH - DI	2x4	DRY	No.2	SPF
DI - DJ	2x4	DRY	No.2	SPF
DJ - DK	2x4	DRY	No.2	SPF
DK - DL	2x4	DRY	No.2	SPF
DL - DM	2x4	DRY	No.2	SPF
DM - DN	2x4	DRY	No.2	SPF
DN - DO	2x4	DRY	No.2	SPF
DO - DP	2x4	DRY	No.2	SPF
DP - DQ	2x4	DRY	No.2	SPF
DQ - DR	2x4	DRY	No.2	SPF
DR - DS	2x4	DRY	No.2	SPF
DS - DT	2x4	DRY	No.2	SPF
DT - DU	2x4	DRY	No.2	SPF
DU - DV	2x4	DRY	No.2	SPF
DV - DW	2x4	DRY	No.2	SPF
DW - DX	2x4	DRY	No.2	SPF
DX - DY	2x4	DRY	No.2	SPF
DY - DZ	2x4	DRY	No.2	SPF
DZ - EA	2x4	DRY	No.2	SPF
EA - EB	2x4	DRY	No.2	SPF
EB - EC	2x4	DRY	No.2	SPF
EC - ED	2x4	DRY	No.2	SPF
ED - EE	2x4	DRY	No.2	SPF
EE - EF	2x4	DRY	No.2	SPF
EF - EG	2x4	DRY	No.2	SPF
EG - EH	2x4	DRY	No.2	SPF
EH - EI	2x4	DRY	No.2	SPF
EI - EJ	2x4	DRY	No.2	SPF
EJ - EK	2x4	DRY	No.2	SPF
EK - EL	2x4	DRY	No.2	SPF
EL - EM	2x4	DRY	No.2	SPF
EM - EN	2x4	DRY	No.2	SPF
EN - EO	2x4	DRY	No.2	SPF
EO - EP	2x4	DRY	No.2	SPF
EP - EQ	2x4	DRY	No.2	SPF
EQ - ER	2x4	DRY	No.2	SPF
ER - ES	2x4	DRY	No.2	SPF
ES - ET	2x4	DRY	No.2	SPF
ET - EU	2x4	DRY	No.2	SPF
EU - EV	2x4	DRY	No.2	SPF
EV - EW	2x4	DRY	No.2	SPF
EW - EX	2x4	DRY	No.2	SPF
EX - EY	2x4	DRY	No.2	SPF
EY - EZ	2x4	DRY	No.2	SPF
EZ - FA	2x4	DRY	No.2	SPF
FA - FB	2x4	DRY	No.2	SPF
FB - FC	2x4	DRY	No.2	SPF
FC - FD	2x4	DRY	No.2	SPF
FD - FE	2x4	DRY	No.2	SPF
FE - FF	2x4	DRY	No.2	SPF
FF - FG	2x4	DRY	No.2	SPF
FG - FH	2x4	DRY	No.2	SPF
FH - FI	2x4	DRY	No.2	SPF
FI - FJ	2x4	DRY	No.2	SPF
FJ - FK	2x4	DRY	No.2	SPF
FK - FL	2x4	DRY	No.2	SPF
FL - FM	2x4	DRY	No.2	SPF
FM - FN	2x4	DRY	No.2	SPF
FN - FO	2x4	DRY	No.2	SPF
FO - FP	2x4	DRY	No.2	SPF
FP - FQ	2x4	DRY	No.2	SPF
FQ - FR	2x4	DRY	No.2	SPF
FR - FS	2x4	DRY	No.2	SPF
FS - FT	2x4	DRY	No.2	SPF
FT - FU	2x4	DRY	No.2	SPF
FU - FV	2x4	DRY	No.2	SPF
FV - FW	2x4	DRY	No.2	SPF
FW - FX	2x4	DRY	No.2	SPF
FX - FY	2x4	DRY	No.2	SPF
FY - FZ	2x4	DRY	No.2	SPF
FZ - GA	2x4	DRY	No.2	SPF
GA - GB	2x4	DRY	No.2	SPF
GB - GC	2x4	DRY	No.2	SPF
GC - GD	2x4	DRY	No.2	SPF
GD - GE	2x4	DRY	No.2	SPF
GE - GF	2x4	DRY	No.2	SPF
GF - GG	2x4	DRY	No.2	SPF
GG - GH	2x4	DRY	No.2	SPF
GH - GI	2x4	DRY	No.2	SPF
GI - GJ	2x4	DRY	No.2	SPF
GJ - GK	2x4	DRY	No.2	SPF
GK - GL	2x4	DRY	No.2	SPF
GL - GM	2x4	DRY	No.2	SPF
GM - GN	2x4	DRY	No.2	SPF
GN - GO	2x4	DRY	No.2	SPF
GO - GP	2x4	DRY	No.2	SPF
GP - GQ	2x4	DRY	No.2	SPF
GQ - GR	2x4	DRY	No.2	SPF
GR - GS	2x4	DRY	No.2	SPF
GS - GT	2x4	DRY	No.2	SPF
GT - GU	2x4	DRY	No.2	SPF
GU - GV	2x4	DRY	No.2	SPF
GV - GW	2x4	DRY	No.2	SPF
GW - GX	2x4	DRY	No.2	SPF
GX - GY	2x4	DRY	No.2	SPF
GY - GZ	2x4	DRY	No.2	SPF
GZ - HA	2x4	DRY	No.2	SPF
HA - HB	2x4	DRY	No.2	SPF
HB - HC	2x4	DRY	No.2	SPF
HC - HD	2x4	DRY	No.2	SPF
HD - HE	2x4	DRY	No.2	SPF
HE - HF	2x4	DRY	No.2	SPF
HF - HG	2x4	DRY	No.2	SPF
HG - HH	2x4	DRY	No.2	SPF
HH - HI	2x4	DRY	No.2	SPF
HI - HJ	2x4	DRY	No.2	SPF
HJ - HK	2x4	DRY	No.2	SPF
HK - HL	2x4	DRY	No.2	SPF
HL - HM	2x4	DRY	No.2	SPF
HM - HN	2x4	DRY	No.2	SPF
HN - HO	2x4	DRY	No.2	SPF
HO - HP	2x4	DRY	No.2	SPF
HP - HQ	2x4	DRY	No.2	SPF
HQ - HR	2x4	DRY	No.2	SPF
HR - HS	2x4	DRY	No.2	SPF
HS - HT	2x4	DRY	No.2	SPF
HT - HU	2x4	DRY	No.2	SPF
HU - HV	2x4	DRY	No.2	SPF
HV - HW	2x4	DRY	No.2	SPF
HW - HX	2x4	DRY	No.2	SPF
HX - HY	2x4	DRY	No.2	SPF
HY - HZ	2x4	DRY	No.2	SPF
HZ - IA	2x4	DRY	No.2	SPF
IA - IB	2x4	DRY	No.2	SPF
IB - IC	2x4	DRY	No.2	SPF
IC - ID	2x4	DRY	No.2	SPF
ID - IE	2x4	DRY	No.2	SPF
IE - IF	2x4	DRY	No.2	SPF
IF - IG	2x4	DRY	No.2	SPF
IG - IH	2x4	DRY	No.2	SPF
IH - II	2x4	DRY	No.2	SPF
II - IJ	2x4	DRY	No.2	SPF
IJ - IK	2x4	DRY	No.2	SPF
IK - IL	2x4	DRY	No.2	SPF
IL - IM	2x4	DRY	No.2	SPF
IM - IN	2x4	DRY	No.2	SPF
IN - IO	2x4	DRY	No.2	SPF
IO - IP	2x4	DRY	No.2	SPF
IP - IQ	2x4	DRY	No.2	SPF
IQ - IR	2x4	DRY	No.2	SPF
IR - IS	2x4	DRY	No.2	SPF
IS - IT	2x4	DRY	No.2	SPF
IT - IU	2x4	DRY	No.2	SPF
IU - IV	2x4	DRY	No.2	SPF
IV - IW	2x4	DRY	No.2	SPF
IW - IX	2x4	DRY	No.2	SPF
IX - IY	2x4	DRY	No.2	SPF
IY - IZ	2x4	DRY	No.2	SPF
IZ - JA	2x4	DRY	No.2	SPF
JA - JB	2x4	DRY	No.2	SPF
JB - JC	2x4	DRY	No.2	SPF
JC - JD	2x4	DRY	No.2	SPF
JD - JE	2x4	DRY	No.2	SPF
JE - JF	2x4	DRY	No.2	SPF
JF - JG	2x4	DRY	No.2	SPF
JG - JH	2x4	DRY	No.2	SPF
JH - JI	2x4	DRY	No.2	SPF
JI - JJ	2x4	DRY	No.2	SPF
JJ - JK	2x4	DRY	No.2	SPF
JK - JL	2x4	DRY	No.2	SPF
JL - JM	2x4	DRY	No.2	SPF
JM - JN	2x4	DRY	No.2	SPF
JN - JO	2x4	DRY	No.2	SPF
JO - JP	2x4	DRY	No.2	SPF
JP - JQ	2x4	DRY	No.2	SPF
JQ - JR	2x4	DRY	No.2	SPF
JR - JS	2x4	DRY	No.2	SPF
JS - JT	2x4	DRY	No.2	SPF
JT - JU	2x4	DRY	No.2	SPF
JU - JV	2x4	DRY	No.2	SPF
JV - JW	2x4	DRY	No.2	SPF
JW - JX	2x4	DRY	No.2	SPF
JX - JY	2x4	DRY	No.2	SPF
JY - JZ	2x4	DRY	No.2	SPF
JZ - KA	2x4	DRY	No.2	SPF
KA - KB	2x4	DRY	No.2	SPF
KB - KC	2x4	DRY	No.2	SPF
KC - KD	2x4	DRY	No.2	SPF
KD - KE	2x4	DRY	No.2	SPF
KE - KF	2x4	DRY	No.2	SPF
KF - KG	2x4	DRY	No.2	SPF
KG - KH	2x4	DRY	No.2	SPF
KH - KI	2x4	DRY	No.2	SPF
KI - KJ	2x4	DRY	No.2	SPF
KJ - KK	2x4	DRY	No.2	SPF
KK - KL	2x4	DRY	No.2	SPF
KL - KM	2x4	DRY	No.2	SPF
KM - KN	2x4	DRY	No.2	SPF
KN - KO	2x4	DRY	No.2	SPF
KO - KP	2x4	DRY	No.2</	

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	T52	1	1	GREEN PARK HOMES	
Tambark Roof Truss, Burlington		Version 8.310 S Oct 29 2019 Mtek Industries, Inc. Tue Apr 28 10:20:12 2020 Page 1			
		ID:DMCubINVR6TstFoe31v6l_zns11-XlpxF2MaHZpsUxolRWd3j86qEbM8WMqKoc9JwzME7X			



Scale = 1:22

LUMBER			
N. L. O. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
I - B	2x4	DRY	No.2
F - E	2x4	DRY	No.2
I - F	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	9.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
I	699	0	699	0	5-8
F	574	0	574	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
I	492	338 0	0 0	0 0	156 0	0 0
F	406	287 0	0 0	0 0	140 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 28	H-C	-116 14
B-C	-423 0	C-G	-20 0
C-D	-382 0	G-D	-126 8
D-E	-409 0	B-H	0 422
I-B	-669 0	G-E	0 419
F-E	-548 0		
I-H	0 0		
H-G	0 375		
G-F	0 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.0/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.16 (1.00 (B-C:1), BC=0.09 (1.00 (G-H:1), WB=0.09 (1.00 (B-H:1), SI=0.12 (1.00 (G-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

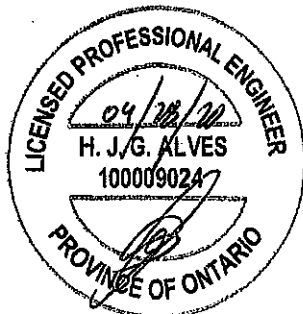
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1867 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.80)
JSI METAL= 0.21 (B) (INPUT = 1.00)

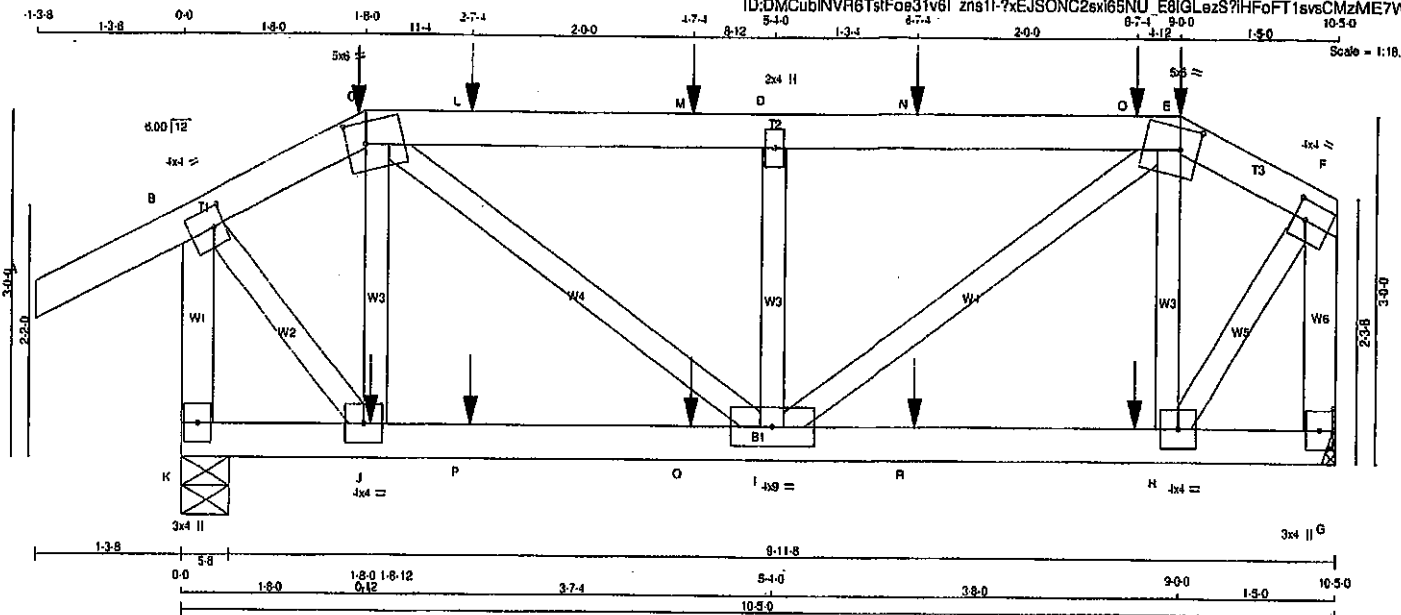


Structural component only
DWG# T-2007673

JOB NAME 408224	TRUSS NAME T53	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
--------------------	-------------------	---------------	----------	-------------------------------	----------

Tamrack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:20:13 2020 Page 1
ID:DMCubINVR6TstFos31v6I zns11-7xEJSONC2sxi65NU E8IGLazS7HFOFT1svsCMzME7W



TOTAL WEIGHT = 46 lb (M)

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-1	MT20	4.0	4.0	2.00	1.25
C TTWW-m	MT20	5.0	6.0	2.25	2.00
D TMVW-w	MT20	2.0	4.0		
E TTWW-m	MT20	5.0	6.0	2.25	2.00
F TMVW-1	MT20	4.0	4.0	2.00	1.25
G BMV1+p	MT20	3.0	4.0		
H BMVW-1	MT20	4.0	4.0		
I BMVW-1	MT20	4.0	9.0		
J BMVW-1	MT20	4.0	4.0		
K BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
K	818	0	818	0
G	679	0	679	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	578	393	0	0	0	183	0
G	480	317	0	0	0	162	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
A-B	0 28	-91.8 -91.8 0.13 (1)	10.00	J-C	-307 0	0.08 (1)	
B-C	-427 0	-91.8 -91.8 0.13 (1)	8.25	C-I	0 438	0.11 (1)	
C-L	-702 0	-91.8 -91.8 0.28 (1)	8.25	I-D	-472 0	0.09 (1)	
L-M	-702 0	-91.8 -91.8 0.28 (1)	8.25	E-E	0 469	0.12 (1)	
M-D	-702 0	-91.8 -91.8 0.28 (1)	8.25	H-E	-373 0	0.07 (1)	
D-N	-702 0	-91.8 -91.8 0.28 (1)	8.25	B-J	0 523	0.13 (1)	
N-O	-702 0	-91.8 -91.8 0.28 (1)	8.25	H-F	0 556	0.14 (1)	
O-E	-702 0	-91.8 -91.8 0.28 (1)	8.25				
E-F	-379 0	-91.8 -91.8 0.04 (1)	8.25				
K-B	-812 0	0.0 0.0 0.10 (1)	7.81				
G-F	-677 0	0.0 0.0 0.09 (1)	7.81				

JT	LOC.	LOC.	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	1-8-0	-53	-53	---	BACK	VERT	TOTAL	---	C1
E	9-0-0	-21	-21	---	FRONT	VERT	TOTAL	---	C1
H	6-7-4	-4	-4	---	BACK	VERT	TOTAL	---	C1
J	1-8-12	-4	-4	---	BACK	VERT	TOTAL	---	C1
L	2-7-4	-28	-28	---	BACK	VERT	TOTAL	---	C1
M	4-7-4	-25	-25	---	BACK	VERT	TOTAL	---	C1
N	6-7-4	-25	-25	---	BACK	VERT	TOTAL	---	C1
O	8-7-4	-53	-53	---	BACK	VERT	TOTAL	---	C1
P	2-7-4	-4	-4	---	BACK	VERT	TOTAL	---	C1
Q	4-7-4	-4	-4	---	BACK	VERT	TOTAL	---	C1
R	6-7-4	-4	-4	---	BACK	VERT	TOTAL	---	C1

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC.	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	1-8-0	-53	-53	---	BACK	VERT	TOTAL	---	C1
E	9-0-0	-21	-21	---	FRONT	VERT	TOTAL	---	C1
H	6-7-4	-4	-4	---	BACK	VERT	TOTAL	---	C1
J	1-8-12	-4	-4	---	BACK	VERT	TOTAL	---	C1
L	2-7-4	-28	-28	---	BACK	VERT	TOTAL	---	C1
M	4-7-4	-25	-25	---	BACK	VERT	TOTAL	---	C1
N	6-7-4	-25	-25	---	BACK	VERT	TOTAL	---	C1
O	8-7-4	-53	-53	---	BACK	VERT	TOTAL	---	C1
P	2-7-4	-4	-4	---	BACK	VERT	TOTAL	---	C1
Q	4-7-4	-4	-4	---	BACK	VERT	TOTAL	---	C1
R	6-7-4	-4	-4	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD			=	39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.25(1.00 (D-E:1), BC=0.09(1.00 (I-K:1), WB=0.14(1.00 (F-H:1), SS=0.21(1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1687	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.77 (F) (INPUT = 0.90)
JSI METAL = 0.22 (F) (INPUT = 1.00)



Structural component only
DWG# T-2007674

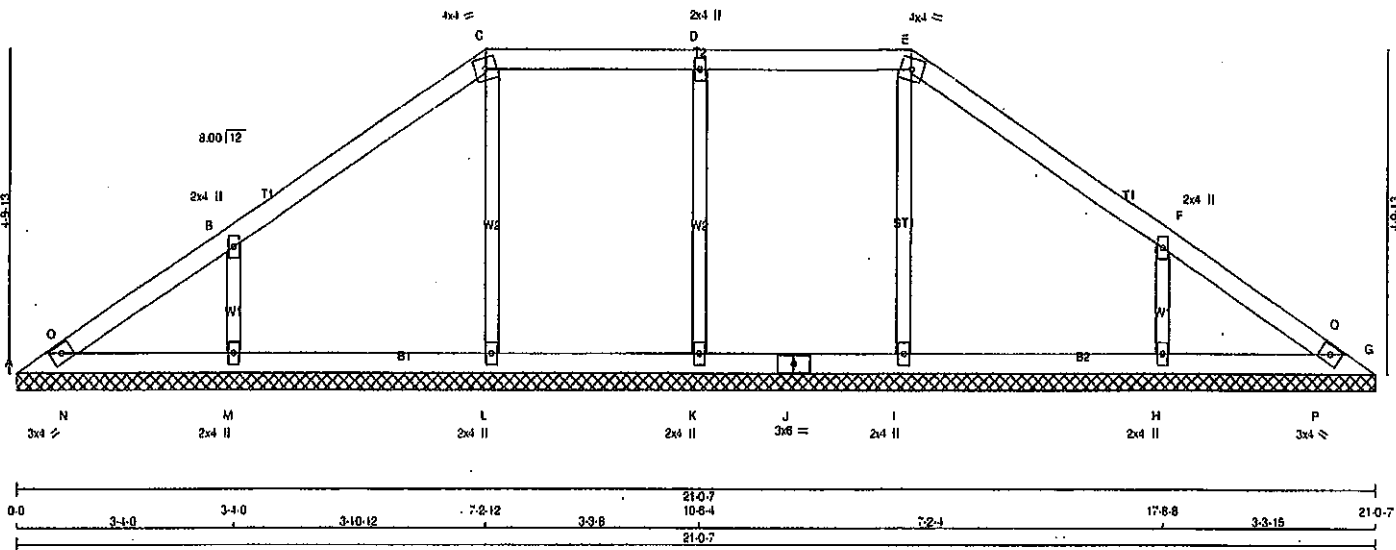
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRAWG NO.
408222	V1	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Tue Apr 28 09:00:14 2020 Page 1

ID:DMCubINVR6TsF0e31v6I zns11-0hm5FSlyDNJda79MsYvGL0sMVq3LML6VkvQG0zMFIV

0-0 3-1-0 3-4-0 3-10-12 7-2-12 3-3-8 10-6-4 3-3-8 13-9-12 3-10-12 17-8-8 3-3-15 21-0-7
Scale: 3/8"=1'



TOTAL WEIGHT = 63 lb (M)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
A - J	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (tablets in inches)	JT TYPE	PLATES	W	LEN	Y	X
A, B, D, F	TBM1-h	MT20	3.0	4.0		
B	TMW-w	MT20	2.0	4.0		
C	TTW-m	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
G	TBM1-h	MT20	3.0	4.0		
H, I, K, L, M						
H	BMW1-w	MT20	2.0	4.0		
J	BS-1	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
A	128	0	128	0	21-0-7 (11-112)0-7	29 0
G	128	0	128	0	21-0-7 (11-112)0-7	29 0
I	330	0	330	0	21-0-7 (11-112)0-7	93 0
L	330	0	330	0	21-0-7 (11-112)0-7	93 0
K	433	0	433	0	21-0-7 (11-112)0-7	92 0
H	485	0	485	0	21-0-7 (11-112)0-7	114 0
M	485	0	485	0	21-0-7 (11-112)0-7	114 0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX. MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	90	61 0	0 0	0 0	0 0	29 0	0 0
G	90	61 0	0 0	0 0	0 0	29 0	0 0
I	235	143 0	0 0	0 0	0 0	93 0	0 0
L	235	143 0	0 0	0 0	0 0	93 0	0 0
K	304	212 0	0 0	0 0	0 0	92 0	0 0
H	342	228 0	0 0	0 0	0 0	114 0	0 0
M	342	228 0	0 0	0 0	0 0	114 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G, I, L, K, H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: 14)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LOCS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO		FROM	TO	FR-TO				
A-O	-23 2	-91.8	-91.8	0.04 (1)	6.25	1-E	266 0	0.09 (1)
O-B	0 33	-91.8	-91.8	0.21 (1)	10.00	L-C	-266 0	0.09 (1)
B-C	-8 8	-91.8	-91.8	0.20 (1)	10.00	K-D	-377 0	0.13 (1)
C-D	0 17	-91.8	-91.8	0.17 (1)	10.00	H-F	-393 0	0.06 (1)
D-E	0 17	-91.8	-91.8	0.17 (1)	10.00	M-B	-393 0	0.06 (1)
E-F	-8 8	-91.8	-91.8	0.20 (1)	10.00	N-O	-39 5	0.00 (1)
F-G	0 33	-91.8	-91.8	0.21 (1)	10.00	P-O	-39 5	0.00 (1)
G-G	-23 2	-91.8	-91.8	0.04 (1)	6.25			
A-N	-9 0	-18.5	-18.5	0.05 (1)	10.00			
N-M	-7 7	-18.5	-18.5	0.06 (1)	10.00			
M-L	-11 0	-18.5	-18.5	0.06 (1)	6.25			
L-K	-17 0	-18.5	-18.5	0.05 (1)	6.25			
K-J	-17 0	-18.5	-18.5	0.05 (1)	6.25			
J-I	-17 0	-18.5	-18.5	0.05 (1)	6.25			
I-H	-11 0	-18.5	-18.5	0.06 (1)	6.25			
H-P	-7 7	-18.5	-18.5	0.06 (1)	10.00			
P-G	-9 0	-18.5	-18.5	0.05 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

ISS 1/4 OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.21 (B-O:1), BC=0.06 (L-M:4), WB=0.13 (D-K:1), SS=0.15 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1997 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.61 (C) INPUT = 0.90
JSI METAL = 0.20 (F) INPUT = 1.00



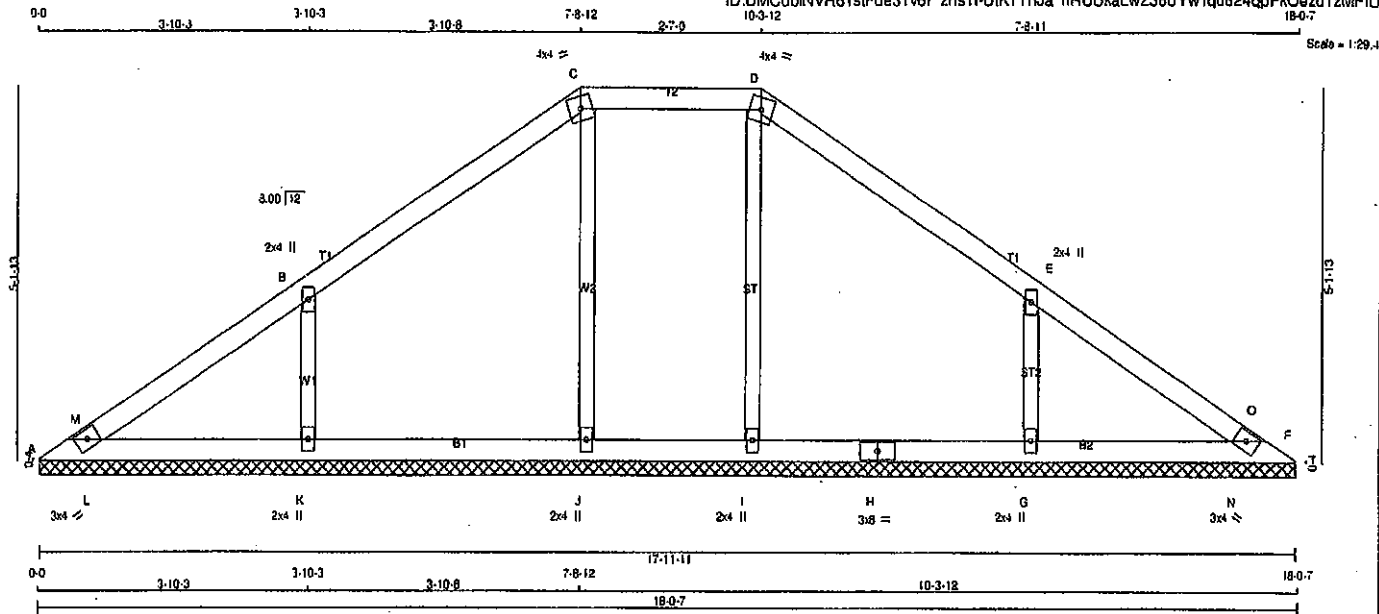
Structural component only
DWG# T-2007616

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	V2	1	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 09:00:15 2020 Page 1

ID:DMCubINVR6TstFoe31v6f zns11-UlKTTnJa hRUkaLwZ38oYw1qu824qpFkOeZdTzMFIL



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
A - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TBM1-h	MT20	3.0	4.0
B	TBM1-h	MT20	2.0	4.0
C	TTW-m	MT20	4.0	4.0
D	TTW-m	MT20	4.0	4.0
E	TBM1-h	MT20	2.0	4.0
F	TBM1-h	MT20	3.0	4.0
G, L, J, K				
G	BMM1-h	MT20	2.0	4.0
H	BS-1	MT20	3.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
A	138	0	138	0	0	17-11-11	(11-17-12-11)		
F	135	0	135	0	0	17-11-11	(11-17-12-11)		
I	344	0	344	0	0	17-11-11	(11-17-12-11)		
G	513	0	513	0	0	17-11-11	(11-17-12-11)		
J	342	0	342	0	0	17-11-11	(11-17-12-11)		
K	513	0	513	0	0	17-11-11	(11-17-12-11)		

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	98	65	0	0	0	30	0
F	95	65	0	0	0	30	0
I	244	155	0	0	0	69	0
G	362	240	0	0	0	122	0
J	242	154	0	0	0	68	0
K	362	240	0	0	0	122	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F, I, G, J, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CS (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	LENGTH FR-TO	FR-TO	
A-M	0 20	-91.8 -91.8 0.04 (1)	10.00	I-D	-292 0
M-B	0 71	-91.8 -91.8 0.22 (1)	10.00	G-E	-408 0
B-C	0 33	-91.8 -91.8 0.22 (1)	10.00	J-C	-291 0
C-D	0 52	-91.8 -91.8 0.11 (1)	10.00	K-B	-408 0
D-E	0 33	-91.8 -91.8 0.22 (1)	10.00	L-M	-71 4
E-O	0 71	-91.8 -91.8 0.22 (1)	10.00	N-O	-69 5
O-F	0 19	-91.8 -91.8 0.04 (1)	10.00		
A-L	-43 0	-18.5 -18.5 0.07 (1)	6.25		
L-K	-30 0	-18.5 -18.5 0.07 (1)	6.25		
K-J	-45 0	-18.5 -18.5 0.07 (4)	6.25		
J-I	-52 0	-18.5 -18.5 0.04 (4)	6.25		
I-H	-45 0	-18.5 -18.5 0.07 (4)	6.25		
H-G	-45 0	-18.5 -18.5 0.07 (4)	6.25		
G-N	-30 0	-18.5 -18.5 0.07 (1)	6.25		
N-F	-43 0	-18.5 -18.5 0.07 (1)	6.25		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.22/1.00 (E-O:1), BC=0.07/1.00 (K-L:1), WB=0.11/1.00 (D:1), SS=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.61 (C) (INPUT = 0.90)
JSI METAL=0.21 (E) (INPUT = 1.00)

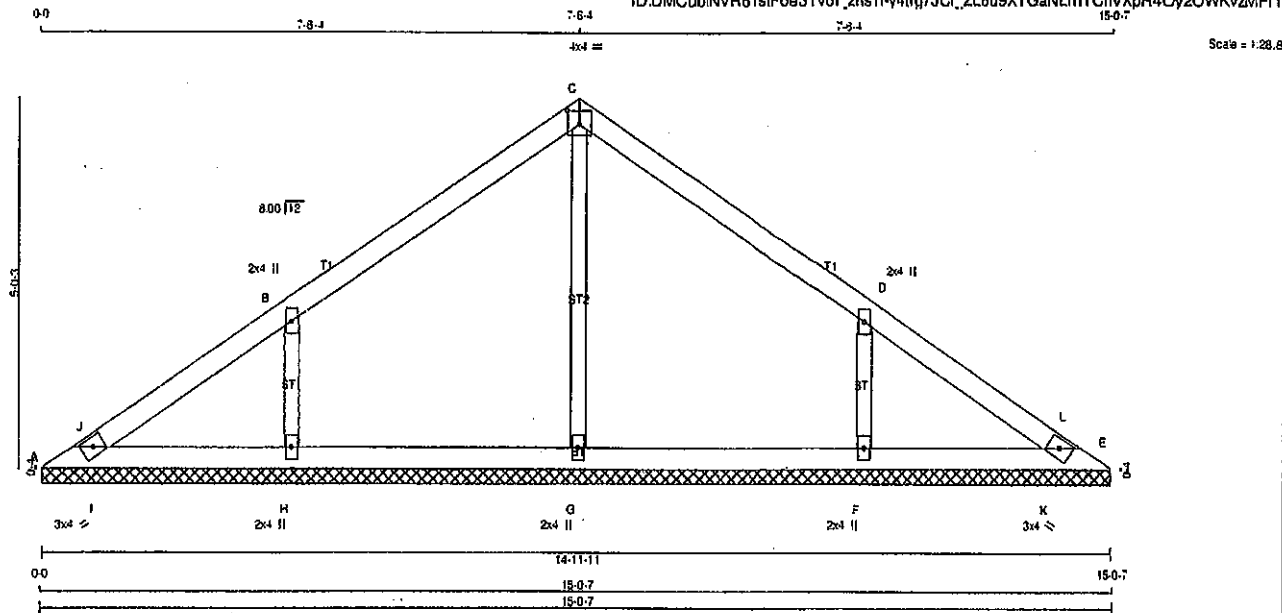


Structural component only
DWG# T-2007617

JOB NAME 408222	TRUSS NAME V3	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 09:00:16 2020 Page 1

ID:DMCubINVR6TsIFoe31v8l_zns1l-y4trg7JCL_ZL8u9XTGaNLmTCIVXpH4Cy2OWKvzMFIIT



TOTAL WEIGHT = 43 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
A - E	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
A TBM1-h	MT20	3.0	4.0	
B TMW+w	MT20	2.0	4.0	
C TTW-p	MT20	4.0	4.0	2.25 2.00
D TMW+w	MT20	2.0	4.0	
E TBM1-h	MT20	3.0	4.0	
F, G, H				
F BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
A	134	0	134	0	0	14-11-11	14-11-11	14-11-11	14-11-11
E	134	0	134	0	0	14-11-11	14-11-11	14-11-11	14-11-11
G	381	0	381	0	0	14-11-11	14-11-11	14-11-11	14-11-11
H	501	0	501	0	0	14-11-11	14-11-11	14-11-11	14-11-11
F	501	0	501	0	0	14-11-11	14-11-11	14-11-11	14-11-11

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
A	95	65	0	0	0	0	0	0
E	95	65	0	0	0	0	0	0
G	272	162	0	0	0	0	0	0
H	353	237	0	0	0	0	0	0
F	353	237	0	0	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS) A, E, G, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-J	-20 4	-91.8 -91.8 0.04 (1)	6.25	G-C	-316 0	0.11 (1)	
J-B	0 38	-91.8 -91.8 0.22 (1)	10.00	H-B	-406 0	0.08 (1)	
B-C	-5 12	-91.8 -91.8 0.21 (1)	10.00	F-D	-406 0	0.08 (1)	
C-D	-5 12	-91.8 -91.8 0.21 (1)	10.00	I-J	-46 8	0.00 (1)	
D-L	0 38	-91.8 -91.8 0.22 (1)	10.00	K-L	-46 6	0.00 (1)	
L-E	-20 4	-91.8 -91.8 0.04 (1)	6.25				
A-I	-13 0	-18.5 -18.5 0.06 (1)	6.25				
I-H	-10 3	-18.5 -18.5 0.06 (4)	6.25				
H-G	-13 0	-18.5 -18.5 0.06 (4)	6.25				
G-F	-13 0	-18.5 -18.5 0.06 (4)	6.25				
F-K	-10 3	-18.5 -18.5 0.06 (4)	6.25				
K-E	-13 0	-18.5 -18.5 0.06 (1)	6.25				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 098-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22/1.00 (B-J:1), BC=0.08/1.00 (G-H:4), WB=0.11/1.00 (C-G:1), SS=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

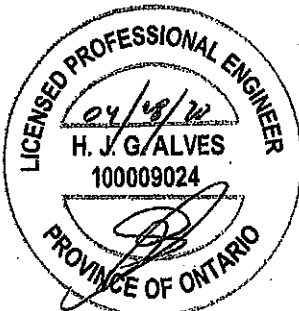
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1867 788 1867 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.28 (D) (INPUT = 0.90)
JSI METAL = 0.21 (D) (INPUT = 1.00)



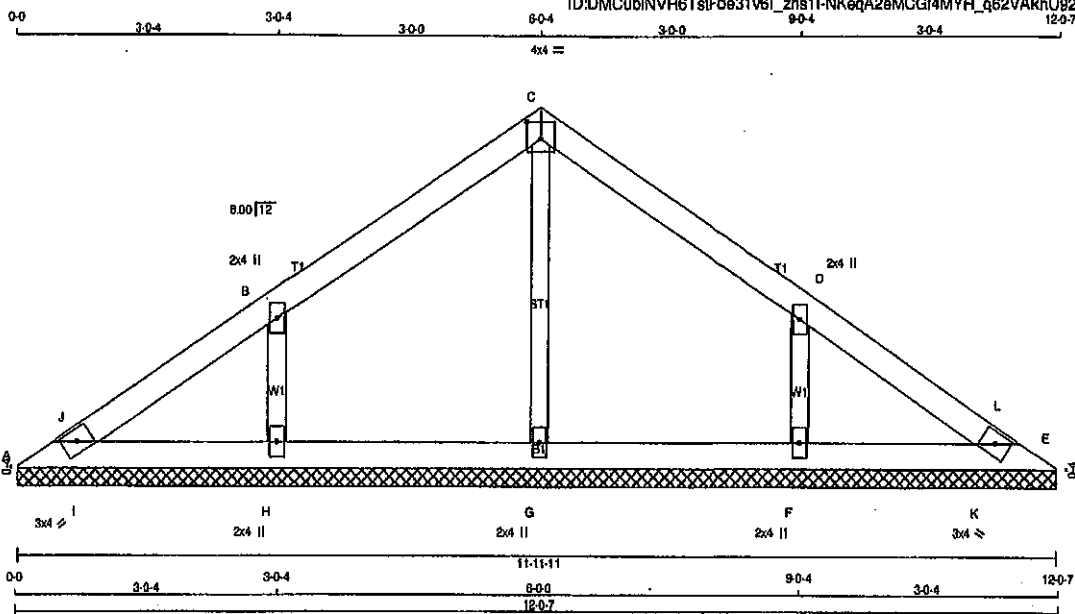
Structural component only
DWG# T-2007618

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408222	V4	1	1	TRUSS DESC.		

Tamirack Roof Truss, Burlington

Version 8.310 5 Oct 2019 Mitek Industries, Inc. Tue Apr 28 11:37:42 2020 Page 1

ID:DMCubINVR6TsIFoe31v6I_zns1I-NKegA2eMCGI4MYH_q62VAkhU82Pb0QNm3IZNsPzMD_6



Scale = 1:23.7

LUMBER				DESIGN CRITERIA			
N. L. G. A. RULES				SPECIFIED LOADS:			
CHORDS	SIZE	LUMBER	DESCR.	TOP CH.	LL	25.8	PSF
A - G	2x4	DRY	No.2	DL		6.0	PSF
C - E	2x4	DRY	No.2	BOT CH.	LL	0.0	PSF
A - E	2x4	DRY	No.2	DL		7.4	PSF
ALL WEBS	2x3	DRY	No.2	TOTAL LOAD		39.0	PSF
DRY: SEASONED LUMBER.				SPACING = 24.0 IN. C/C			

PLATES (table is in inches)			
JT TYPE	PLATES	W	LEN Y X
A TBM1-h	MT20	3.0	4.0
B TMW-w	MT20	2.0	4.0
C TTW-p	MT20	4.0	4.0 2.25 2.00
D TMW-w	MT20	2.0	4.0
E TBM1-h	MT20	3.0	4.0
F, G, H			
F BMW1-w	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
A	117	0	117	0	0	11-11-11	11-11-11	11-11-11	11-11-11
E	117	0	117	0	0	11-11-11	11-11-11	11-11-11	11-11-11
G	290	0	290	0	0	11-11-11	11-11-11	11-11-11	11-11-11
F	398	0	398	0	0	11-11-11	11-11-11	11-11-11	11-11-11
H	398	0	398	0	0	11-11-11	11-11-11	11-11-11	11-11-11

UNFACTORED REACTIONS									
JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS							
		COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
A	83	58/0	0/0	0/0	0/0	28/0	0/0	0/0	
E	83	58/0	0/0	0/0	0/0	28/0	0/0	0/0	
G	207	125/0	0/0	0/0	0/0	82/0	0/0	0/0	
F	281	188/0	0/0	0/0	0/0	93/0	0/0	0/0	
H	281	188/0	0/0	0/0	0/0	93/0	0/0	0/0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-J	-8/5	-31.8 -31.8	0.03 (1)	G-C	-248/0	0.08 (1)	
J-B	0/36	-31.8 -31.8	0.18 (1)	F-D	-314/0	0.05 (1)	
B-C	0/12	-31.8 -31.8	0.18 (1)	H-B	-314/0	0.05 (1)	
C-D	0/12	-31.8 -31.8	0.18 (1)	I-J	-47/4	0.00 (1)	
D-L	0/36	-31.8 -31.8	0.18 (1)	K-L	-47/4	0.00 (1)	
L-E	-8/5	-31.8 -31.8	0.03 (1)				
A-I	-13/0	-18.5 -18.5	0.05 (1)				
I-H	-10/0	-18.5 -18.5	0.05 (1)				
H-G	-20/0	-18.5 -18.5	0.04 (4)				
G-F	-20/0	-18.5 -18.5	0.04 (4)				
F-K	-10/0	-18.5 -18.5	0.05 (1)				
K-E	-13/0	-18.5 -18.5	0.05 (1)				

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TO=0.13/1.00 (B-J:1), BC=0.05/1.00 (A-K:1), WB=0.08/1.00 (C-G:1), SB=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

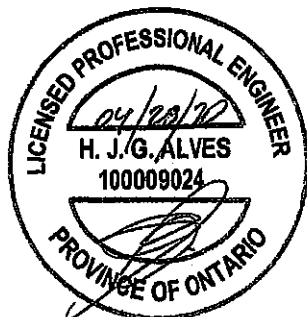
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLJ)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1887 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

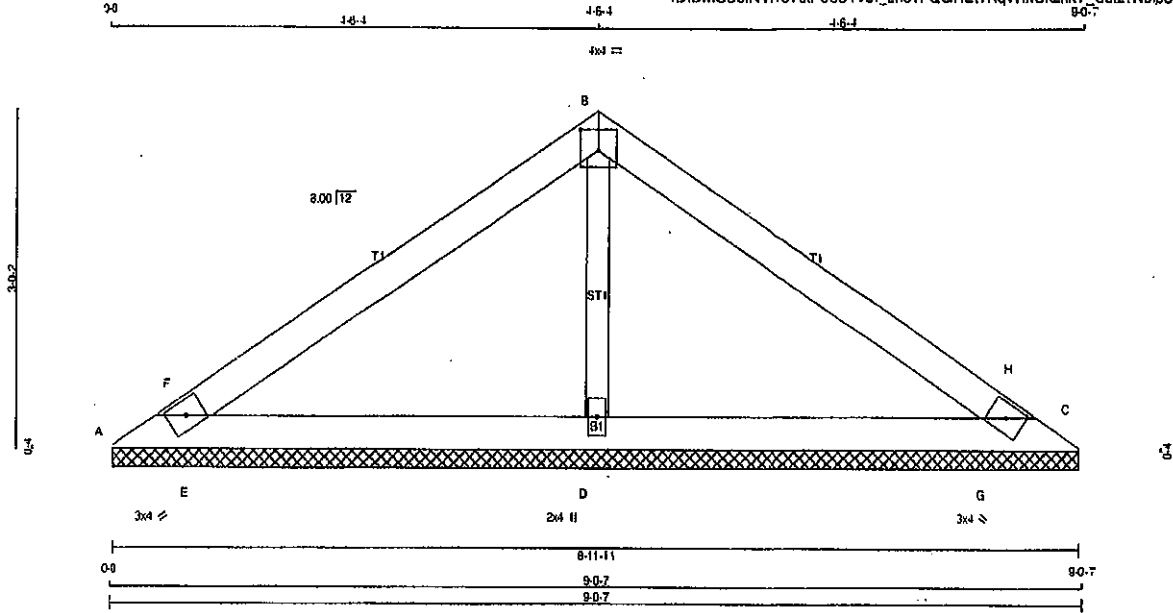
JSI GRIP= 0.22 (D) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007619

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	V5	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 © Oct 29 2019 Mitak Industries, Inc. Tue Apr 28 09:00:17 2020 Page 1
ID:DMCubINVR6TslFoe31v6l_zns11-QGREITKqWlnCk2kk1_6ctz?N9ip5YjBYBt74(LzMfIS



Scale = 1/16.0

TOTAL WEIGHT = 23 b (M)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A - B	TBM1-h	MT20	3.0	4.0			
B - C	TTW-p	MT20	4.0	4.0	2.25	2.00	
C - B	TBM1-h	MT20	3.0	4.0			
D - B	BMW1-w	MT20	2.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	HORZ	DOWN	UPLIFT
A	49	0	49	0
C	49	0	49	0
D	893	0	893	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	34	22.0	0.0	0.0	0.0	0.0	0.0	12.0	0.0
C	34	22.0	0.0	0.0	0.0	0.0	0.0	12.0	0.0
D	631	415.0	0.0	0.0	0.0	0.0	0.0	216.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM	TO			FR-TO		
A-F	0 347	-91.8	-91.8	0.12 (1)	10.00	D-B	-699 0	0.12 (1)
F-B	0 341	-91.8	-91.8	0.23 (1)	10.00	E-F	-225 0	0.00 (1)
B-H	0 341	-91.8	-91.8	0.23 (1)	10.00	G-H	-225 0	0.00 (1)
H-C	0 347	-91.8	-91.8	0.12 (1)	10.00			
A-E	-321 0	-18.5	-18.5	0.16 (1)	6.25			
E-D	-290 0	-18.5	-18.5	0.17 (1)	6.25			
D-G	-290 0	-18.5	-18.5	0.17 (1)	6.25			
G-C	-321 0	-18.5	-18.5	0.16 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. GC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (B-F:1), BC=0.17/1.00 (D-E:1), WB=0.12/1.00 (B-D:1), SSI=0.12/1.00 (A-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1967 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.59 (B) (INPUT = 0.90)
JSI METAL = 0.20 (B) (INPUT = 1.00)

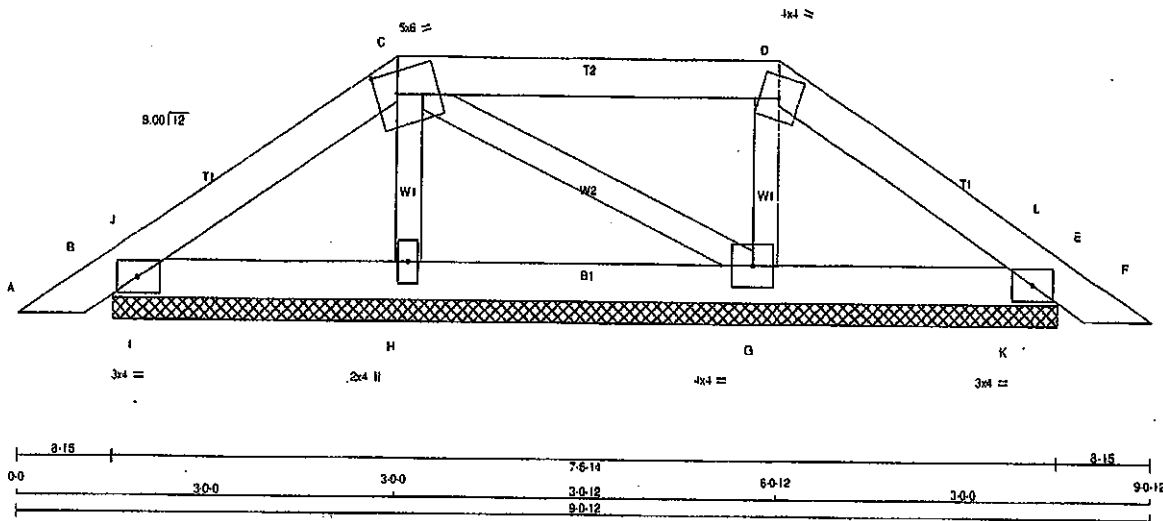


Structural component only
DWG# T-2007620

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	PB1	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Tue Apr 28 09:00:02 2020 Page 1
ID:DMCubINVR6TstFoe3 1v8l zns1l-MN1ZKL9Q2hoKQk4rIKK5mpuqJlFXyLks?nqzMPkh
8-0-12 9-0-12

Scale = 1:16.4



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	QESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TM81-I	MT20	3.0	4.0	
C	TTWW-m	MT20	5.0	6.0	2.00 2.00
D	TTW-m	MT20	4.0	4.0	
E	TM81-I	MT20	3.0	4.0	
G	BMWW1-I	MT20	4.0	4.0	
H	BMWW1-W	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG
B	221	0	221	0	0	0	7-6-14	7-6-14	
E	209	0	209	0	0	0	7-6-14	7-6-14	
H	248	0	248	0	0	0	7-6-14	7-6-14	
G	280	0	280	0	0	0	7-6-14	7-6-14	

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	COMBINED	SNOW	LIVE	PERM. LIVE
B	154	113	0	0	0	0	0	41	0	0	0
E	146	106	0	0	0	0	0	39	0	0	0
H	176	109	0	0	0	0	0	67	0	0	0
G	198	127	0	0	0	0	0	71	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FACTORED	VERT. LOAD	LC1 MAX	MEMB.	MAX. FACTORED	VERT. LOAD	LC1 MAX
	(LBS)	(PLF)	CSI (LC)		(LBS)	(PLF)	CSI (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 15	-91.8	-91.8 0.03 (1)	H-C	-176 0	0.03 (1)	
B-J	-63 0	-91.8	-91.8 0.01 (1)	C-G	-21 0	0.00 (1)	
J-C	-73 0	-91.8	-91.8 0.05 (1)	G-D	-198 0	0.03 (1)	
C-D	-30 0	-91.8	-91.8 0.15 (1)	I-J	-121 0	0.00 (1)	
D-L	-54 0	-91.8	-91.8 0.05 (1)	K-L	-123 0	0.00 (1)	
L-E	-41 0	-91.8	-91.8 0.01 (1)				
E-F	0 15	-91.8	-91.8 0.03 (1)				
B-I	0 60	-18.5	-18.5 0.06 (1)				
I-H	0 60	-18.5	-18.5 0.06 (1)				
H-G	0 48	-18.5	-18.5 0.03 (1)				
G-K	0 43	-18.5	-18.5 0.06 (1)				
K-E	0 43	-18.5	-18.5 0.06 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (C-D:1), BC=0.06/1.00 (B-I:1), WB=0.03/1.00 (D-G:1), SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX	MIN	MAX
MT20	616	354
	1667	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.18 (B) INPUT = 0.90
JSI METAL=0.04 (B) INPUT = 1.00

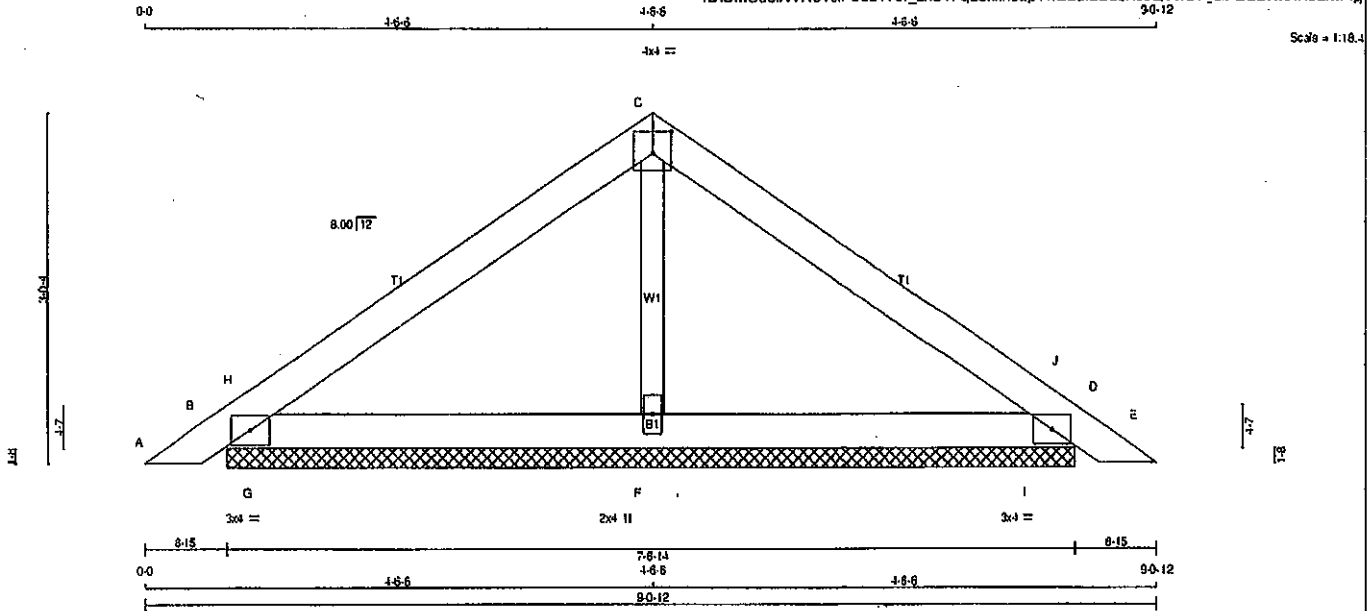


Structural component only
DWG# T-2007600

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	PB2	2	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 26 09:00:03 2020 Page 1
ID:DMCubINVR8TstFoe31v6l_zns11-qabxoh92p7wB2uf2D2sKJQ7w31_GPEUzWkKN9zMF1q



TOTAL WEIGHT = 2 X 23 = 46 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY, SEASONED LUMBER.			

PLATES (tablets in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-p	MT20	4.0	4.0	2.25 2.00
D	TMB1-I	MT20	3.0	4.0	
F	BMW1-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
B	318	0	0	318	0	0	0	7-6-14	7-6-14
D	318	0	0	318	0	0	0	7-6-14	7-6-14
F	320	0	0	320	0	0	0	7-6-14	7-6-14

UNFACTORED REACTIONS

JT	1ST LCASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	223	158	0	0	0	0	63	0
D	223	158	0	0	0	0	63	0
F	229	137	0	0	0	0	92	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. CSF (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSF (LC)	UNBRACED LENGTH (FT)
FR-TO		FROM	TO	FR-TO			
A-B	0 15	-91.8	-91.8 0.03 (1)	10.00	F-C	-148 0	0.03 (1)
B-H	-31 0	-91.8	-91.8 0.07 (1)	6.25	G-H	-349 0	0.00 (1)
H-C	-135 0	-91.8	-91.8 0.16 (1)	6.25	I-J	-349 0	0.00 (1)
C-J	-135 0	-91.8	-91.8 0.16 (1)	6.25			
J-D	-31 0	-91.8	-91.8 0.07 (1)	6.25			
D-E	0 15	-91.8	-91.8 0.03 (1)	10.00			
B-G	0 105	-18.5	-18.5 0.15 (1)	10.00			
G-F	0 105	-18.5	-18.5 0.15 (1)	10.00			
F-I	0 105	-18.5	-18.5 0.15 (1)	10.00			
I-D	0 105	-18.5	-18.5 0.15 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. G.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18, 1.00 (C-J); BC=0.15, 1.00 (F-I); WB=0.03, 1.00 (C-F); SS=0.26, 1.00 (D-E)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	618	354
	1867	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.28 (D) INPUT = 0.90
JSI METAL = 0.07 (D) INPUT = 1.00



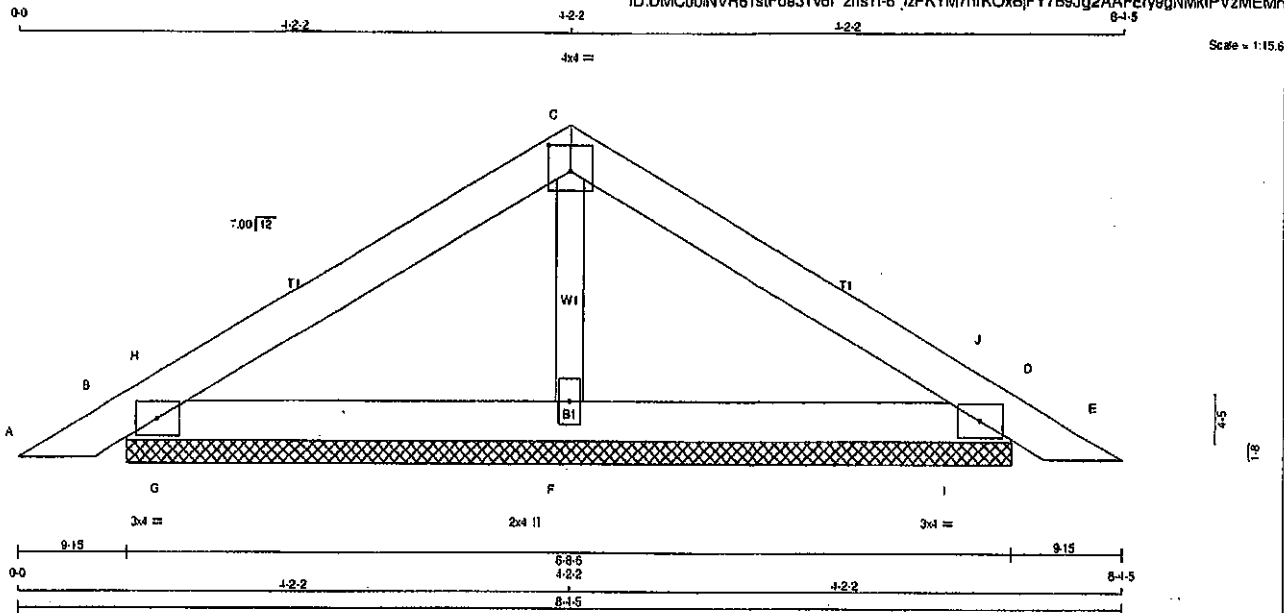
Structural component only
DWG# T-2007601

JOB NAME 408223	TRUSS NAME PB20	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:03:58 2020 Page 1			
		ID:DMCubINVR6TstFos31v6I zns1t-6 izPKYM7hrKOx8IFy7B9Jg2AAFEry9gNMkIPVzMEMr			

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:03:58 2020 Page 1

ID:DMCubINVR6TstFos31v6I zns1t-6 izPKYM7hrKOx8IFy7B9Jg2AAFEry9gNMkIPVzMEMr



Scale = 1:15.6

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
B - D	2x4 DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF			
DRY: SEASONED LUMBER.			

PLATES (tablets in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMB1-I	MT20	3.0	4.0		
C TTW-p	MT20	4.0	3.0	2.25	2.00
D TMB1-I	MT20	3.0	4.0		
F BMW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	281	0	281	0	0	6-8-6	6-8-6
D	281	0	281	0	0	6-8-6	6-8-6
F	314	0	314	0	0	6-8-6	6-8-6

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	197	141	0	0	0	55	0
D	197	141	0	0	0	55	0
F	224	137	0	0	0	87	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 16	-91.8	-91.8 0.04 (1)	10.00	F-C	-159 0	0.02 (1)
B-H	-44 0	-91.8	-91.8 0.05 (1)	6.25	G-H	-250 0	0.00 (1)
H-C	-99 0	-91.8	-91.8 0.12 (1)	6.25	I-J	-250 0	0.00 (1)
C-J	-99 0	-91.8	-91.8 0.12 (1)	6.25			
J-D	-44 0	-91.8	-91.8 0.05 (1)	6.25			
D-E	0 16	-91.8	-91.8 0.04 (1)	10.00			
B-G	0 81	-18.5	-18.5 0.12 (1)	10.00			
G-F	0 81	-18.5	-18.5 0.12 (1)	10.00			
F-I	0 81	-18.5	-18.5 0.12 (1)	10.00			
I-D	0 81	-18.5	-18.5 0.12 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 28.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (C-H:1), BC=0.12/1.00 (B-G:1), WB=0.02/1.00 (C-F:1), SS=0.18/1.00 (D-I:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

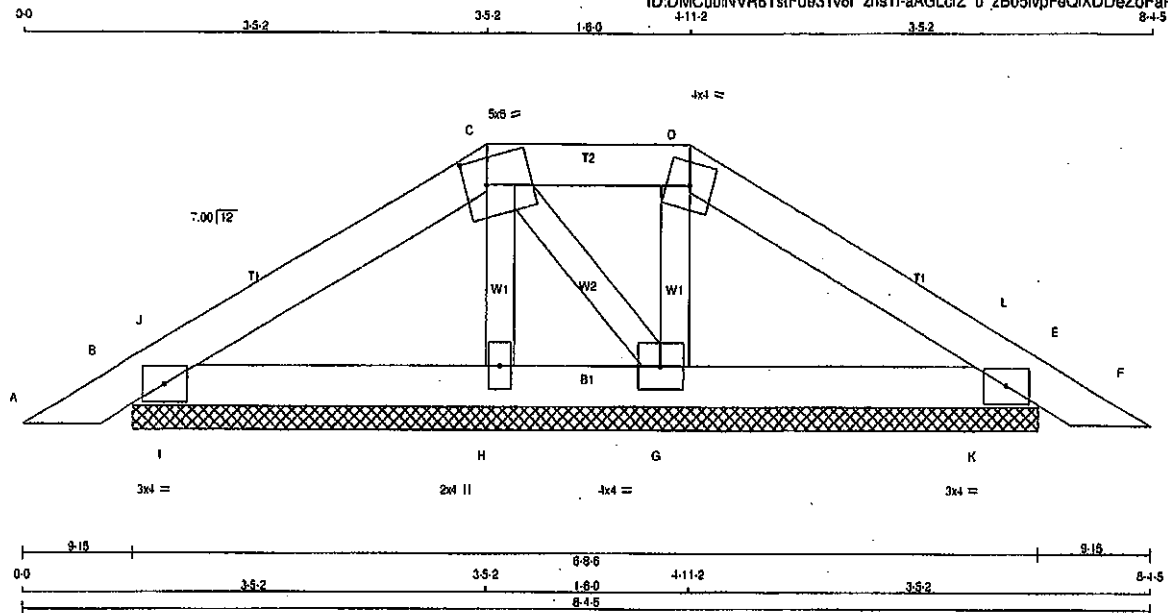
JSI GRIP= 0.24 (D) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007627

JOB NAME 408223	TRUSS NAME PB21	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:03:57 2020 Page 1
 ID:DMCubINVR6TstF0a3iv6I zns1I-aAGLdZ u zB05IvpFeQIXDDeZbFaPRpb0UDxxzMEMm



Scale = 1:15.2

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMB1-J	MT20	3.0	4.0		
C TTW-m	MT20	5.0	6.0	2.25	2.00
D TTW-m	MT20	4.0	4.0		
E TMB1-J	MT20	3.0	4.0		
G BMW1-i	MT20	4.0	4.0		
H BMW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	241	0	241	0	8-8-8	8-8-8
E	228	0	228	0	8-8-8	8-8-8
H	170	0	170	0	8-8-8	8-8-8
G	238	0	238	0	8-8-8	8-8-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX. MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
B	169	121	0	0	0	0	0
E	160	114	0	0	0	0	0
H	122	74	0	0	0	0	0
G	168	111	0	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX. (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX. (PLF)	UNBRACED LENGTH (FT)
FR-TO				FR-TO			
A-B	0 16	-91.8	-91.8 0.04 (1)	10.00	H-C	-107 0	0.02 (1)
B-J	-59 0	-91.8	-91.8 0.01 (1)	6.25	C-G	-36 0	0.01 (1)
J-C	-71 0	-91.8	-91.8 0.07 (1)	6.25	G-D	-147 0	0.02 (1)
C-D	-28 0	-91.8	-91.8 0.04 (1)	6.25	I-J	-139 0	0.00 (1)
D-L	-46 0	-91.8	-91.8 0.07 (1)	6.25	K-L	-141 0	0.00 (1)
L-E	-33 0	-91.8	-91.8 0.01 (1)	6.25			
E-F	0 16	-91.8	-91.8 0.04 (1)	10.00			
B-I	0 59	-18.5	-18.5 0.07 (1)	10.00			
I-H	0 59	-18.5	-18.5 0.07 (1)	10.00			
H-G	0 52	-18.5	-18.5 0.04 (1)	10.00			
G-K	0 38	-18.5	-18.5 0.07 (1)	10.00			
K-E	0 38	-18.5	-18.5 0.07 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, OBC 2012: ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07 1.00 (D-L:1), BC=0.07 1.00 (B-I:1), WB=0.02 1.00 (D-G:1), SS=0.11 1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

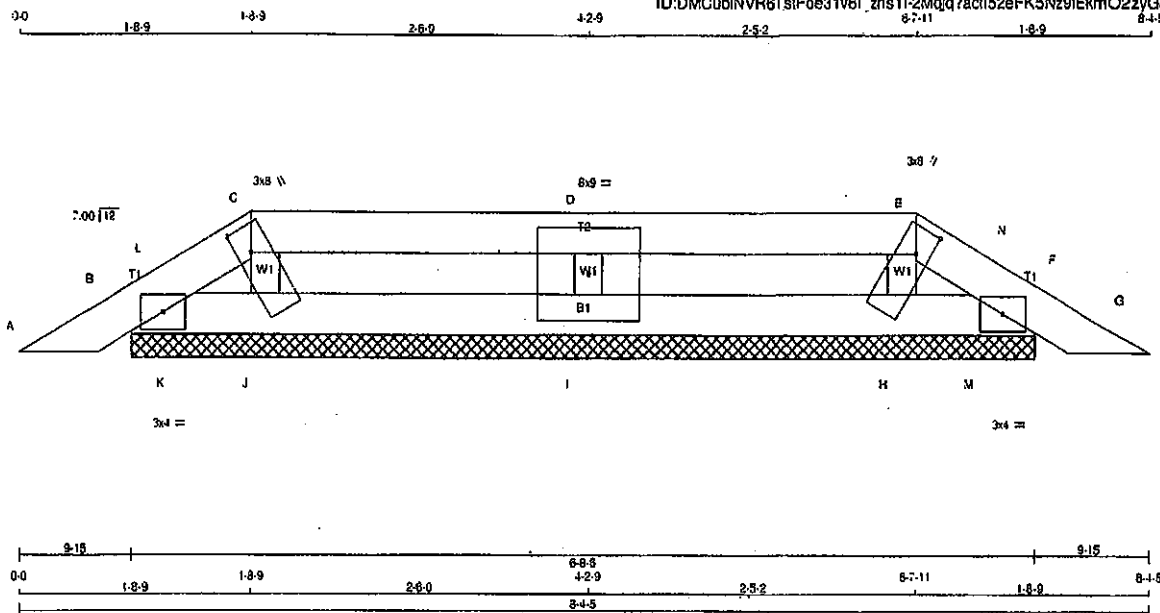
JSI GRIP= 0.19 (B) (INPUT = 0.90)
 JSI METAL= 0.05 (D) (INPUT = 1.00)



Structural component only
 DWG# T-2007628

JOB NAME 408223	TRUSS NAME PB22	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 9 Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:03:58 2020 Page 1
ID:DMCubiNVR6TstFoe31v6l_zns11-2Mojq7acil52eFK5Nz9lEkmoQ2zyGJaPzqgDmTNzMEMi



TOTAL WEIGHT = 18 lb

LUMBER
N. L. G. A. RULES
CHORDS SIZE
A - C 2x4 DRY No.2
C - E 2x4 DRY No.2
E - G 2x4 DRY No.2
B - F 2x4 DRY No.2
ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DESCR.
SPF
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
B	151	0	151	0	6-6-6	6-6-6
F	152	0	152	0	6-6-6	6-6-6
J	125	0	125	0	6-6-6	6-6-6
H	120	0	120	0	6-6-6	6-6-6
I	330	0	330	0	6-6-6	6-6-6

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF
SPACING = 24.0 IN. C/C

PLATES (Tables in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	C. D. E, H, I, J					
C	TTBW1+h	MT20	3.0	8.0	2.25	1.25
D	TMBW1-I	MT20	8.0	9.0		
E	TTBW1+h	MT20	3.0	8.0	2.25	1.25
F	TMB1-I	MT20	3.0	4.0		

UNFACTORED REACTIONS

JT	1ST LOOSE COMBINED		MAX. MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	SNOW	LIVE	SNOW	LIVE				
B	105	83	0	0	0	0	21	0
F	105	83	0	0	0	0	22	0
J	90	49	0	0	0	0	41	0
H	85	47	0	0	0	0	39	0
I	232	158	0	0	0	0	74	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, H, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: 14)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)
FR-TO		FROM TO	FR-TO	
A-B	0 16	-91.8 -91.8 0.04 (1)	J-C	-95 0 0.01 (1)
B-L	-53 0	-91.8 -91.8 0.04 (1)	H-E	-92 0 0.01 (1)
L-C	-22 0	-91.8 -91.8 0.01 (1)	I-D	-282 0 0.04 (1)
C-D	0 3	-91.8 -91.8 0.09 (1)	K-L	-1 12 0.00 (1)
D-E	0 3	-91.8 -91.8 0.09 (1)	M-N	0 13 0.00 (1)
E-N	-21 0	-91.8 -91.8 0.01 (1)		
N-F	-32 0	-91.8 -91.8 0.04 (1)		
F-G	0 16	-91.8 -91.8 0.04 (1)		
B-K	0 14	-18.5 -18.5 0.01 (1)		
K-J	0 14	-18.5 -18.5 0.02 (4)		
J-I	-3 0	-18.5 -18.5 0.02 (4)		
I-H	-3 0	-18.5 -18.5 0.02 (4)		
H-M	0 14	-18.5 -18.5 0.01 (4)		
M-F	0 14	-18.5 -18.5 0.01 (1)		

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (D-E:1), BC=0.02/1.00 (H-J:4), WB=0.04/1.00 (I-D:1), SS=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

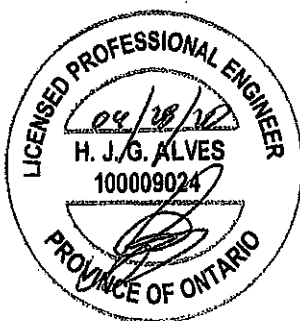
NAIL VALUES

PLATE GRIP (PS) SHEAR (PL) SECTION (PL)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 788 1967 1658

PLATE PLACEMENT TOL. = 0.250 inches

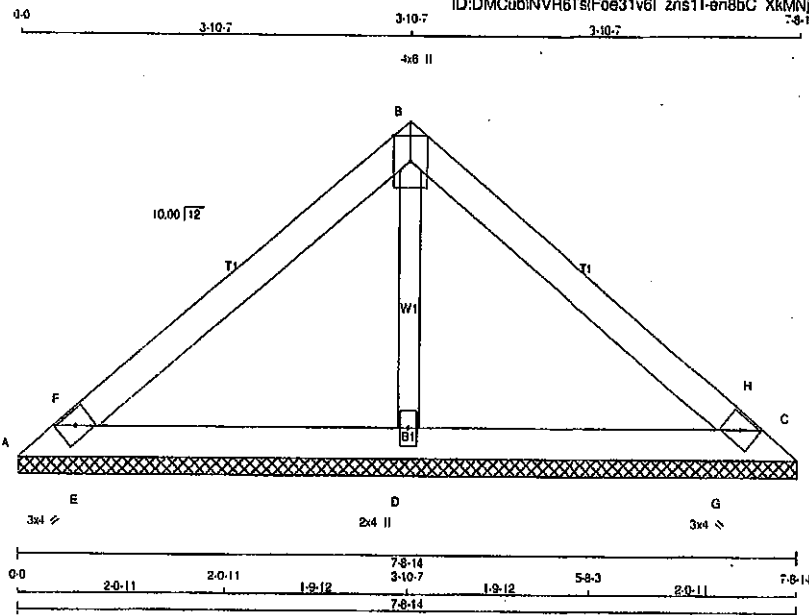
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.13 (C) INPUT = 0.90
JSI METAL = 0.04 (C) INPUT = 1.00



Structural component only
DWG# T-2007629

JOB NAME 408223	TRUSS NAME P20	QUANTITY 17	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



TOTAL WEIGHT = 17 X 21 = 363 lb (M)

LUMBER	N. L. G. A. RULES
CHORDS - SIZE	LUMBER
A - B 2x4 DRY	No 2
B - C 2x4 DRY	No 2
A - C 2x4 DRY	No 2
ALL WEBS 2x3 DRY	No 2
DRY: SEASONED LUMBER.	

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
A	80	0	60	0
C	60	0	60	0
D	733	0	733	0

UNFACTORED REACTIONS

JT	1ST LOASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED							
A	43	28	0	0	0	0	14	0
C	42	28	0	0	0	0	14	0
D	518	339	0	0	0	0	179	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-F	0 227	-91.8	-91.8 0.09 (1)	10.00	D-B	-558 0	0.10 (1)
F-B	0 219	-91.8	-91.8 0.17 (1)	10.00	E-F	-189 0	0.00 (1)
B-H	0 219	-91.8	-91.8 0.17 (1)	10.00	G-H	-189 0	0.00 (1)
H-C	0 228	-91.8	-91.8 0.09 (1)	10.00			
A-E	-207 0	-18.5	-18.5 0.13 (1)	6.25			
E-D	-174 0	-18.5	-18.5 0.13 (1)	6.25			
D-G	-174 0	-18.5	-18.5 0.13 (1)	6.25			
G-C	-207 0	-18.5	-18.5 0.13 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL =	25.6 PSF
DL =	8.0 PSF
BOT CH. LL =	0.0 PSF
DL =	7.4 PSF
TOTAL LOAD =	39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NSCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

ISS 5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 1.00 (B-F:1), 8C=0.13/1.00 (D-E:1), WB=0.10/1.00 (B-D:1), SSI=0.11/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX
MT20	618	354
	1667	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

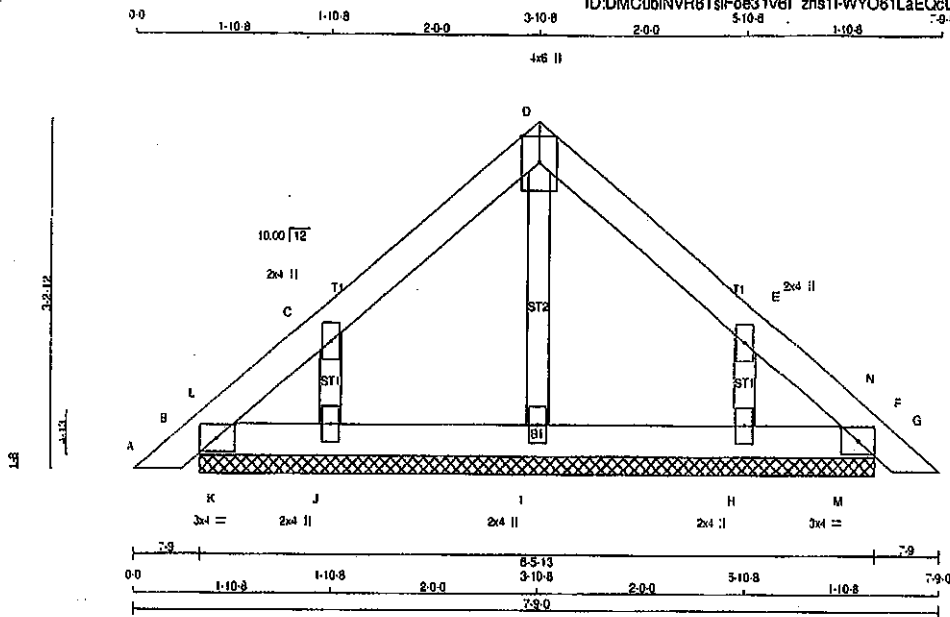
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (D) INPUT = 0.90
JSI METAL= 0.12 (D) INPUT = 1.00



Structural component only
DWG# T-2007626

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	PG20	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.310 S Oct 29 2019 MITek Industries, Inc. Tue Apr 28 10:03:59 2020 Page 1					
ID:DMCubINVR6TsFoe31v6l zns1HWYO61LaEQcDvFPvHxggunylaUNHa2Js63KzJ0pzMEMk					
Scale = 1:19.7					



TOTAL WEIGHT = 2 X 22 = 45 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
B - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0 1.50 2.00
C	TMB1-w	MT20	2.0	4.0
D	TTW1-p	MT20	4.0	6.0 Edge
E	TMB1-w	MT20	2.0	4.0
F	TMB1-I	MT20	3.0	4.0 1.50 2.00
H, I, J	BMW1-w	MT20	2.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS (ILC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (ILC)	UNBRACED LENGTH (FT)
FR-TO		FROM TO		FR-TO			
A-B	0 14	-91.8 -91.8 0.02 (1)	10.00	I-D	-111 0	0.02 (1)	
B-L	-71 0	-91.8 -91.8 0.00 (1)	6.25	J-C	-185 0	0.03 (1)	
L-C	-30 0	-91.8 -91.8 0.05 (1)	6.25	H-E	-185 0	0.03 (1)	
C-D	-47 0	-91.8 -91.8 0.05 (1)	6.25	K-L	0 9	0.00 (1)	
D-E	-47 0	-91.8 -91.8 0.05 (1)	6.25	M-N	0 9	0.00 (1)	
E-N	-30 0	-91.8 -91.8 0.05 (1)	6.25				
N-F	-71 0	-91.8 -91.8 0.00 (1)	6.25				
F-G	0 14	-91.8 -91.8 0.02 (1)	10.00				
B-K	0 44	-18.5 -18.5 0.01 (1)	10.00				
K-J	0 44	-18.5 -18.5 0.01 (4)	10.00				
J-I	0 27	-18.5 -18.5 0.02 (4)	10.00				
I-H	0 27	-18.5 -18.5 0.02 (4)	10.00				
H-M	0 44	-18.5 -18.5 0.01 (4)	10.00				
M-F	0 44	-18.5 -18.5 0.01 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (C-D:1), BC=0.02/1.00 (H-I:1), WB=0.03/1.00 (G-J:1), SS=0.07/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP (DRY)	SHEAR SECTION (PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

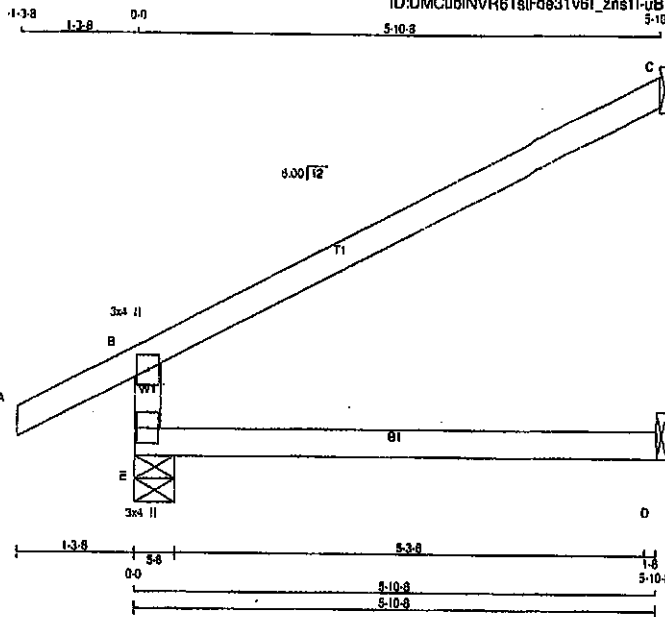
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.14 (E) (INPUT = 0.00)
JSI METAL = 0.10 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007630

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	J1	21	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2		
A - C	2x4	DRY	No.2		
E - D	2x4	DRY	No.2		

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	525	0	525	0	5-8	5-8
C	202	0	202	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	MAX. MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	389	257	0	0	0	111	0
C	139	113	0	0	0	26	0
D	36	0	0	0	0	36	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. VERT. LOAD (LBS)	MAX. HORZ. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)
	FR-TO	TO-TO	FROM	TO					
E-B	-161	0	0.0	0.0	0.13 (4)	7.81			
A-B	0	28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-30	0	-91.8	-91.8	0.54 (1)	6.25			
E-D	0	0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CS: TC=0.54, 1.00 (B-C:1), BC=0.13, 1.00 (D-E:1), WB=0.00, 1.00 (max), SS=0.24, 1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

MAX. VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MIN	MAX	MIN
MT20	618	354
	1687	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.19 (E), INPUT = 0.90
JSI METAL = 0.13 (B), INPUT = 1.00

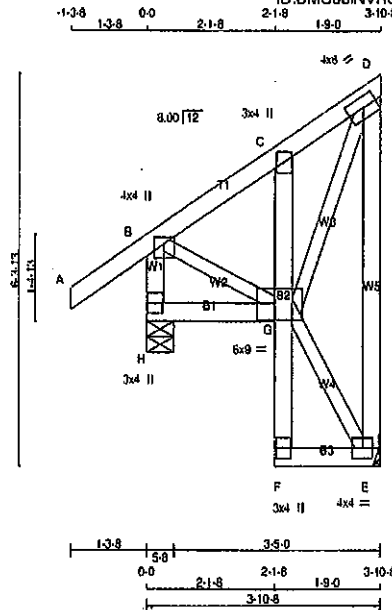


Structural component only
DWG# T-2007598

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408222	J2S	4	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 6.310 S Oct 28 2019 MTek Industries, Inc. Tue Apr 28 09:00:01 2020 Page 1

ID:DMCubINVR6TstFoe31v6I zns11-uBTBW78oHNgUpbVf5dpsEbLhFOVoUrBWCfEHzMFI



Scale = 1/32"

TOTAL WEIGHT = 4 X 31 = 125 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
H - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
H - G	2x4	DRY	No.2
F - C	2x4	DRY	No.2
F - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25 2.00
C	TMV+p	MT20	3.0	4.0	
D	TMVW-1	MT20	4.0	8.0	2.00 2.75
E	BMVW-1	MT20	4.0	4.0	
F	BMV+p	MT20	3.0	4.0	
G	BVMWVW-1	MT20	6.0	9.0	3.25 3.50
H	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
H	332	0	332	0	0
E	208	0	208	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

1ST LOASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	232	188 0	0 0	0 0	0 0	68 0	0 0
E	145	95 0	0 0	0 0	0 0	50 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: 151

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
H-B	-311	0	0
A-B	0	35	0
B-C	-96	0	0
C-D	-105	0	0
H-G	0	0	0
F-G	0	15	0
G-C	-202	0	0
F-E	0	4	0

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

PART 9 OF CBC 2018, OBC 2012, ABC 2019
PART 9 OF OBC 2012 (2019 AMENDMENT)
CSA 086-09, CSA 086-14
TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.14 (A-B:5), SC=0.03 (G-H:4), WB=0.08 (D-E:1), SS=0.09 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PS)	(PLI)	(PLI)	(PLI)
MT20	618	354	1867 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

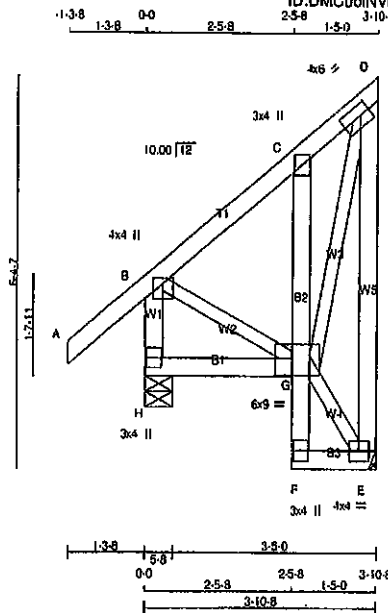
JSI GRIP = 0.22 (B) INPUT = 0.90 ;
JSI METAL = 0.07 (C) INPUT = 1.00 ;



Structural component only
DWG# T-2007599

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408223	J20S	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 10:03:51 2020 Page 1



Scale = 1/4" = 1'-0"

TOTAL WEIGHT = 2 X 33 = 66 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF		
A - D	2x4	DRY	No.2	SPF		
H - G	2x4	DRY	No.2	SPF		
F - C	2x4	DRY	No.2	SPF		
F - E	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						
E - D	2x4	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (tablets in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMV+p	MT20	3.0	4.0		
D	TMWW-I	MT20	4.0	8.0	2.00	2.00
E	BMWW-I	MT20	4.0	4.0		
F	BMV+p	MT20	3.0	4.0		
G	BMWWW-I	MT20	8.0	9.0	3.25	3.50
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
H	333	0	333	0	5-8
E	208	0	208	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	233	166	0	0	0	0	67	0
E	145	95	0	0	0	0	50	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED	FACTORED	MEMB.	FORCE	MAX.	WEBS	MAX. FACTORED	MEMB.	FORCE	MAX.
FR-TO	FORCE	VERT. LOAD	LC1	MAX	CS1(LC)	UNBRAC	LENGTH	FR-TO	FORCE	MAX
H-B	-309	0	0.0	0.0	0.03 (1)	7.81	B-G	0	70	0.02 (1)
A-B	0	41	-91.8	-91.8	0.14 (5)	10.00	E-D	-185	0	0.08 (1)
B-C	-70	0	-91.8	-91.8	0.07 (1)	8.25	G-E	-13	0	0.00 (1)
C-D	-92	0	-91.8	-91.8	0.06 (1)	6.25	G-D	0	215	0.05 (1)
H-G	0	0	-18.5	-18.5	0.04 (4)	10.00				
F-G	0	12	0.0	0.0	0.02 (1)	10.00				
G-C	-216	0	0.0	0.0	0.02 (1)	7.81				
F-E	0	8	-18.5	-18.5	0.01 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (A-B-5), BC=0.04/1.00 (G-H-4),
WB=0.08/1.00 (D-E-1), SSI=0.09/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	618	354
	1887	788
	1887	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.08 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007623

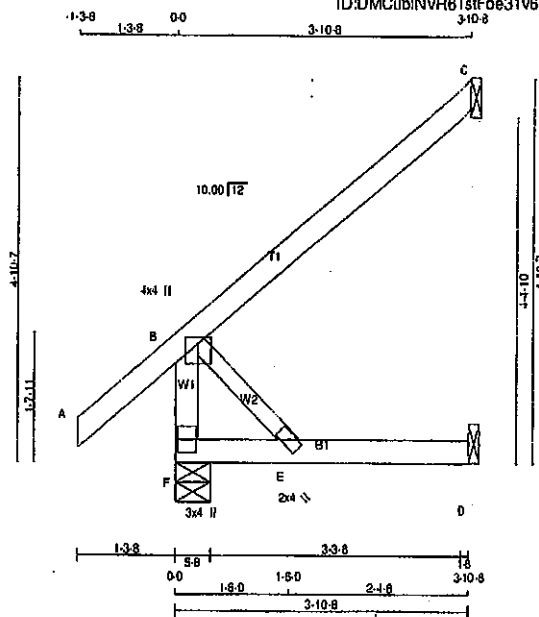
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408223	J22	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 10:03:54 2020 Page 1

ID:DMCubINVR6TstFoe31v6I zns11-AbaD eX8b3bd9e1K874j4ubg(MZPN21Nv2FYKozMEMp

Scale = 1:27.0



LUMBER				DESCR.	
N. L. G. A. RULES				SPF	
CHORDS	SIZE	LUMBER		SPF	
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ORY: SEASONED LUMBER.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
F		341	0	341	0	0	5-8	5-8	
C		178	0	178	0	0	1-8	1-8	
D		36	0	40	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
F	239	170	0	0	0	0	0	
C	122	99	0	0	0	0	0	
D	29	0	0	0	0	0	0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MEMB.	FORCE	MAX	CS1 (LC)
	(LBS)	(PLF)	CS1 (LC)		(LBS)		
F-B	-305	0	0.0	0.0	0.03 (1)	7.61	8-E
A-B	0	41	-91.8	-91.8	0.14 (5)	10.00	
B-C	0	0	-91.8	-91.8	0.23 (1)	10.00	
F-E	0	0	-18.5	-18.5	0.08 (4)	10.00	
E-D	0	0	-18.5	-18.5	0.08 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CS1: TC=0.23/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.09/1.00 (B-E:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT KEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	354	1687
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

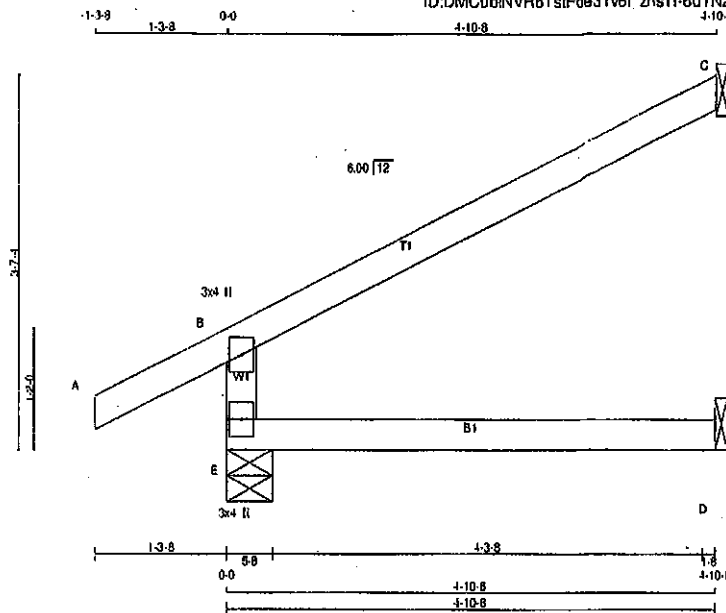


Structural component only
DWG# T-2007625

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	J40	2	1	GREEN PARK HOMES	
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Tue Apr 28 10:19:52 2020 Page 1
ID:DMCubINVR6TstF0e31v6I zns11-6uTN2C71 PHh5sGSQJGMvwHM_XXOqKBNC5JFy4zME7r



Scale = 1:20.0

TOTAL WEIGHT = 2 X 14 = 29 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
JT	VERT	HORZ	UPLIFT	IN-SX
E	458	0	458	0
C	168	0	168	0
D	38	0	42	0

SEE MTEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
E	320	225	0	0	0	95
C	118	94	0	0	0	22
D	30	0	0	0	0	30

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	W E B S	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	MEMB.	FORCE
FR-TO	(LBS)	(PLF)	FR-TO	(LBS)
E-B	-404	0	0.0	0.0
A-B	0	28	-91.8	-91.8
B-C	-25	0	-91.8	-91.8
E-D	0	0	-18.5	-18.5

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF.

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CS: TC=0.37 1.00 (B-C:1), BC=0.09/1.00 (D-E:4), WB=0.00/1.00 (rva:0), SSI=0.20/1.00 (B-C:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1855

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.17 (E) (INPUT = 0.90)
JSI METAL = 0.11 (B) (INPUT = 1.00)



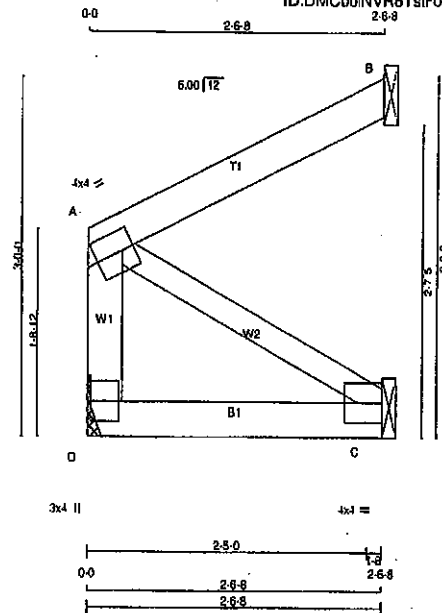
Structural component only
DWG# T-2007657

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408224	J41	5	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:18:54 2020 Page 1
ID:DMCubINVR6TslF0e31v6I zns1I-3Hb7Tu8HW1XPKAQqNkQq.LMmiLEmIEngIPaM1zzME7p

Scale = 1:17.8



TOTAL WEIGHT = 5 X 10 = 49 lb (M/P)

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR.
D - A	2x4	DRY	No.2		SPF
A - B	2x4	DRY	No.2		SPF
D - C	2x4	DRY	No.2		SPF
ALL WEBS		2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	4.0	4.0	2.00	1.25
C	BMV1-1	MT20	4.0	4.0	2.00	Edge
D	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
D	139	0	139	0
B	117	0	117	0
C	23	0	25	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B - C

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	98	65	0	0	0	33	0
B	80	65	0	0	0	15	0
C	18	0	0	0	0	18	0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)
FR-TO					FR-TO		
D-A	-117	0	0.0	0.0 (1)	7.81	A-C	0
A-B	0	0	-91.8	-91.8 (1)	10.00		
D-C	0	0	-18.5	-18.5 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.19")

CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CS: TC=0.10/1.00 (A-B:1) . BC=0.03/1.00 (C-D:4) .

WB=0.00/1.00 (A-C:1) . SS=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618	354
	1687	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.07 (A) INPUT = 0.90)
JSI METAL= 0.02 (A) INPUT = 1.00)



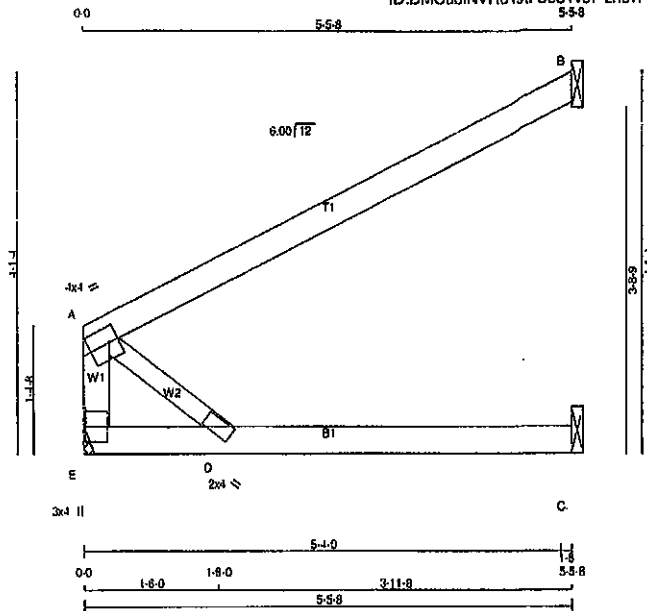
Structural component only
DWG# T-2007658

JOB NAME	TRUSS NAME	QUANTITY	FLY	JOB DESC.	DRWG NO.
408224	J42	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2018 Mitek Industries, Inc. Tue Apr 28 10:19:53 2020 Page 1

ID:DMCubINVR6TstFoe31v6I zns1I-Xt8WgE9vHKIGyk71xSq3XYumIX 1hxqu3YvZPzME7c

Scale = 1:22.5



TOTAL WEIGHT = 2 X 16 = 31 lb (M/F)

LUMBER			
N.L.G.A. RULES	SIZE	DRY	LUMBER
CHORDS			
E - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
E - C	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)			
JT	TYPE	PLATES	W LEN Y X
A	TMVW-I	MT20	4.0 3.0 2.00 1.25
D	BMVW-w	MT20	2.0 4.0
E	BMV1+p	MT20	3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
E	301	0	301	0	0
B	250	0	250	0	1-8
C	50	0	57	0	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	213	140	0	0	0	78	0
B	172	140	0	0	0	33	0
C	40	0	0	0	0	40	0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FACTORED	MEMB.	FACTORED
FR-TO	FORCE (LBS)	FR-TO	FORCE (LBS)
E-A	-250	E-D	0
A-B	0	D-C	0

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.48/1.00 (A-B:1), BC=0.18/1.00 (C-D:4), WB=0.00/1.00 (A-D:1), SS=0.17/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

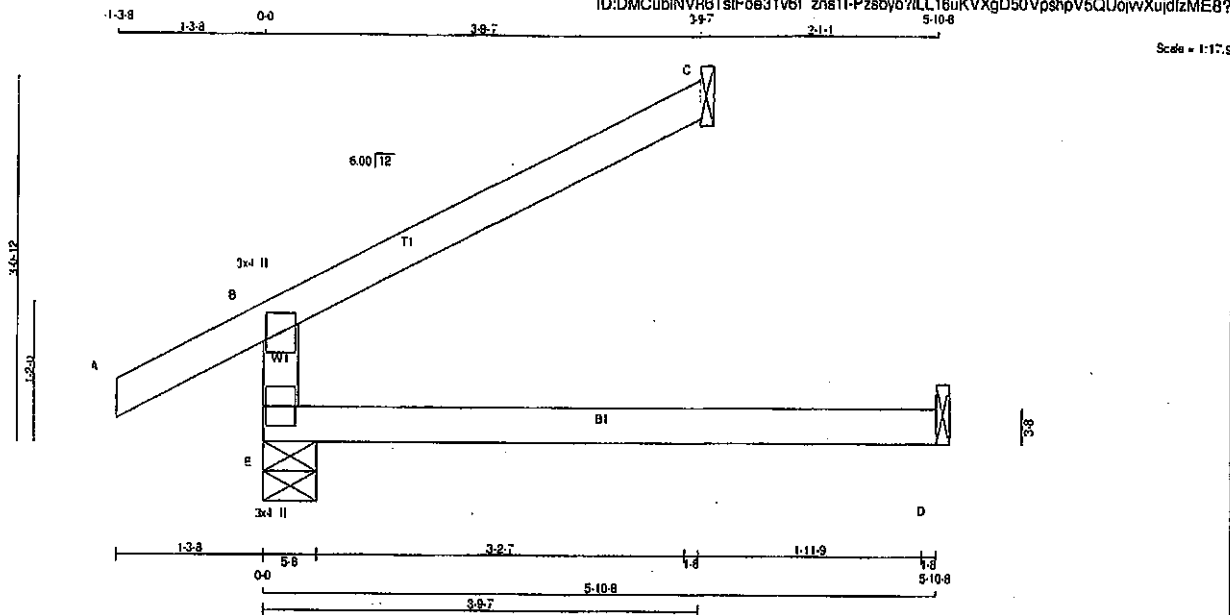
JSI GRIP = 0.15 (A) INPUT = 0.90
JSI METAL = 0.04 (A) INPUT = 1.09



Structural component only
DWG# T-2007659

JOB NAME 408224	TRUSS NAME C40	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	ORWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 MTak Industries, Inc. Tue Apr 28 10:19:42 2020 Page 1
ID:DMCubINVR6TstFoe31v6I zns1I-PzsbYo7LL16uKVXgD50VpshpV5QUojwXujdzME8?



TOTAL WEIGHT = 2 X 14 = 28 lb (M)

LUMBER

N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD
	VERT	HORZ	DOWN	HORZ
E	405	0	405	0
C	130	0	130	0
D	45	0	50	0

SEE MITEK STANDARD DETAIL B87791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	288	190	0	0	0	0	0
C	90	73	0	0	0	0	0
D	38	0	0	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
E-B	-342	0	0.0	0.13 (4)	7.81		
A-B	0	28	-91.8	-91.8	0.12 (1)	10.00	
B-C	-19	0	-91.8	-91.8	0.22 (1)	6.25	
E-D	0	0	-18.5	-18.5	0.13 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.22/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (m/a:0), SS=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1687
	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.14 (E); INPUT = 0.50;
JSI METAL = 0.09 (B); INPUT = 1.00



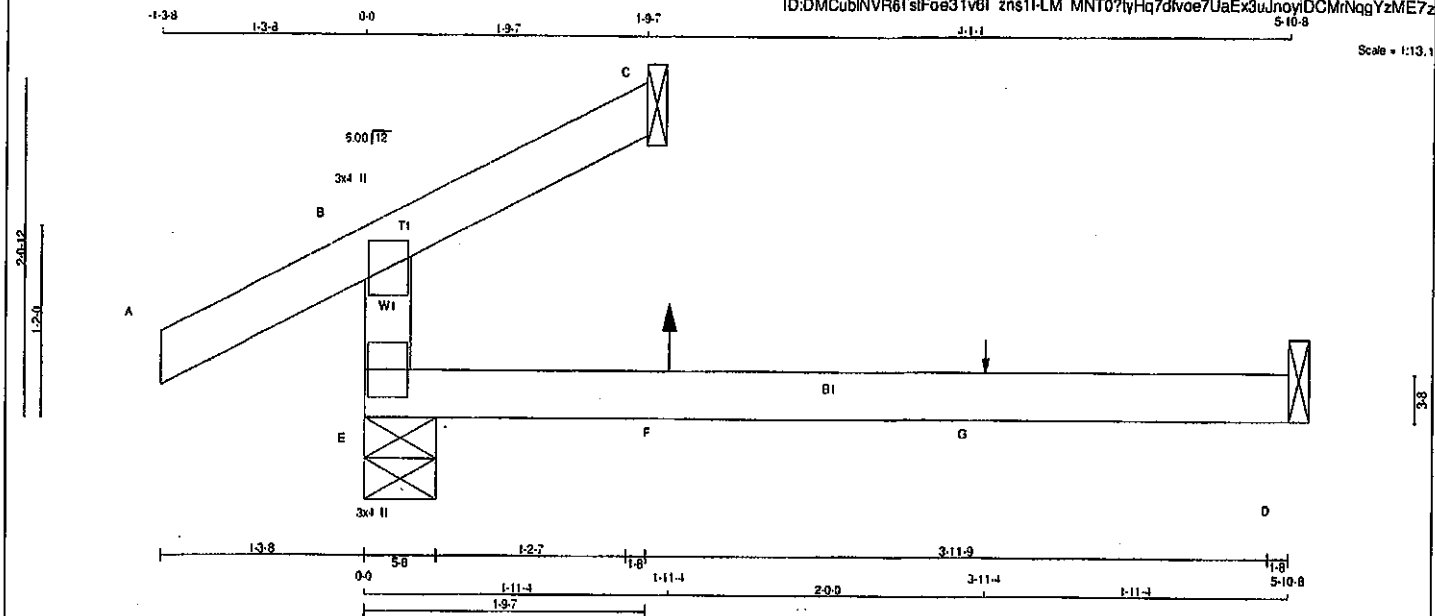
Structural component only
DWG# T-2007648

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	C41	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 5 Oct 2019 Mitek Industries, Inc. Tue Apr 28 10:19:44 2020 Page 1

ID:DMCubINVR6TsFoe31v6I zns1HLM MNT0?yHq7dfoe7UaEx3uJnoyDCMrNqgYzME7z 5-10-8

Scale = 1:13.1



TOTAL WEIGHT = 2 X 12 = 23 lb (M)

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
E	BMV+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	BRG	UPLIFT	BRG	IN-SX
E	284	0	284	0	0	5-8	5-8		
C	63	0	63	0	0	1-8	1-8		
D	44	0	52	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS		1ST LCASE		MAX. MIN. COMPONENT REACTIONS		SNOW		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED																
E	200	137	0	0	0	0	0	0	0	0	0	0	0	62	0	0	0
C	46	21	0	0	0	0	0	0	0	0	0	0	0	25	0	0	0
D	35	0	-3	0	0	0	0	0	0	0	0	0	0	37	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS		MAX. FACTORED		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	CS1 (LC)	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO		FROM	TO			FR-TO			
E-B	-227	0	0.0	0.0	0.11 (4)	7.81			
A-B	0	28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-9	9	-91.8	-91.8	0.08 (4)	10.00			
E-F	0	0	-18.5	-18.5	0.14 (4)	10.00			
F-G	0	0	-18.5	-18.5	0.14 (4)	10.00			
G-D	0	0	-18.5	-18.5	0.14 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)		LOC.		MAX.		FACE		DIR.		TYPE		HEEL		CONN.	
JT															
F	1-11.4	7	1	12	BACK	VERT	TOTAL							C1	
G	3-11.4	1	1		BACK	VERT	TOTAL							C1	

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.04")

CS1: TC=0.12/1.00 (A-B:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (WB:0), SS=0.09/1.00 (A-B:1)
DOL LUMBER=0.98 NAIL=0.98 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) INPUT = 0.90
JSI METAL= 0.06 (B) INPUT = 1.00

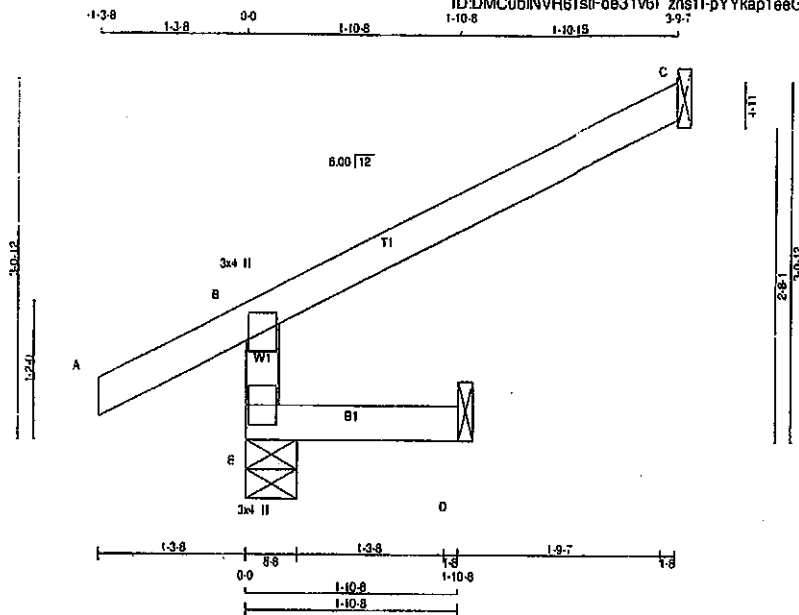


Structural component only
DWG# T-2007649

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DWG NO.
408224	C42	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Tue Apr 28 10:19:45 2020 Page 1
ID:DMCubINVR6TslF0e31v6I zns11-pYYkap1eeGPhInE6Llq7RUC2j9wh9SMbV7NC zME7Y



TOTAL WEIGHT = 3 X 10 = 29 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2	
A - C	2x4	DRY	No.2	
E - D	2x4	DRY	No.2	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
E	361	0	361	0	5-8	5-8
C	130	0	130	0	1-8	1-8
D	18	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
E	250	190	0	0	0	0	0
C	90	73	0	0	0	0	0
D	12	0	0	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (6)

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FORCE (LBS)	FACTORED MAX. CSI (LC)
FR-TO		FROM TO		FR-TO			
E-B	-342 0	0.0 0.0	0.01 (4)	7.81			
A-B	0 28	-91.8 -91.8	0.13 (6)	10.00			
B-C	-19 0	-91.8 -91.8	0.22 (1)	6.25			
E-D	0 0	-18.5 -18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.22(1.00 (B-C:1), BC=0.02(1.00 (D-E:4), WB=0.00(1.00 (m:a:0), SSI=0.15(1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1887
	788	1987	1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.14 (E) (INPUT = 0.90)
JSI METAL=0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007650



TOTAL WEIGHT = 3 X 7 = 21 lb

DRY: SEASONED LUMBER.

B	TMV+p	MT20
---	-------	------

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
----	------	------	------	------	--------	-------	-------

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	188	141 0	0 0	0 0	0 0	47 0	0 0
C	31	24 -18	0 0	0 0	0 0	7 0	0 0
D	7	0 -8	0 0	0 0	0 0	12 0	0 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

TOTAL LOAD CASES: 15)

MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX
-------	-------	------------	-----	-----	------	-------	-------	-----

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	OL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	OL	=	7.4	PSF
TOTAL LOAD				= 39.0 PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L 360 (0.19")
CALCULATED VERT. DEFL.(LL) = L 999 (0.00")
ALLOWABLE DEFL.(TL) = L 360 (0.19")
CALCULATED VERT. DEFL.(TL) = L 999 (0.00")

CSI: TC=0.12/1.00 (A:B:1) , BC=0.04/1.00 (D:E:5) ,
WB=0.00/1.00 (n/a:0) , SS=0.09/1.00 (A:B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1687	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (8) (INPUT = 1.00)



Structural component only
DWG# T-2007651

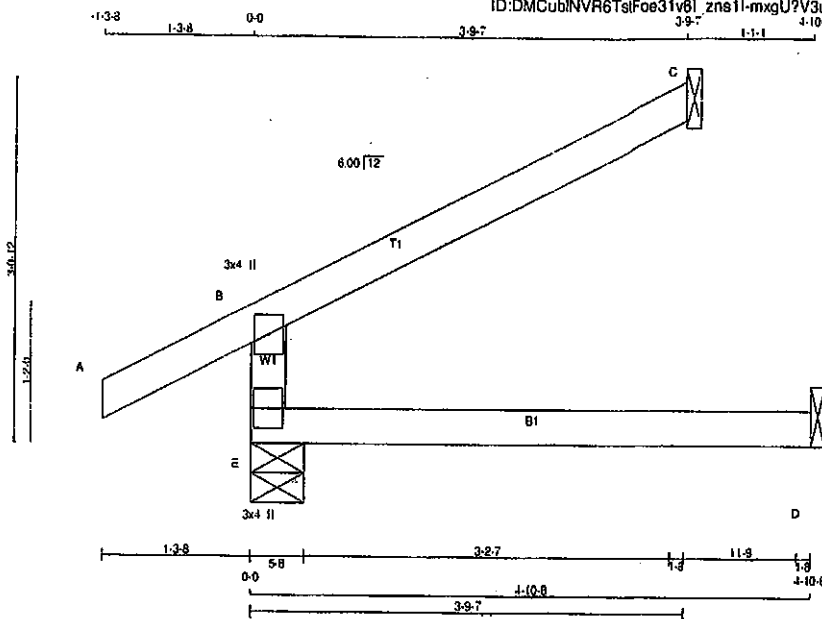
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	C44	2	1	GREEN PARK HOMES	
TAMERACK ROOF TRUSS, BURLINGTON		TRUSS DESC.			

TAMERACK ROOF TRUSS, BURLINGTON

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:19:47 2020 Page 1

ID:DMCubINVR6TslFoe31v8I zns1l-mxgU?V3uAtIP 5OUTmhBCsZXYWQD93ye2pcUHzME7w

Scale = 1:17.5



LUMBER

N. L. G. A. RULES	
CHORDS	SIZE
E - B	2x4 DRY No.2
A - C	2x4 DRY No.2
E - D	2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
E	394	0	394	0
C	130	0	130	0
D	38	0	42	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	277	180	0	0	0	87	0
C	90	73	0	0	0	17	0
D	30	0	0	0	0	30	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO							
E-B	-342	0	0.0	0.08 (4)	7.81		
A-B	0	28	-91.8	-91.8	0.12 (1)	10.00	
B-C	-19	0	-91.8	-91.8	0.22 (1)	6.25	
E-D	0	0	-18.5	-18.5	0.09 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.22:1.00 (B-C:1), BC=0.09:1.00 (D-E:4), WB=0.00:1.00 (max), SSI=0.15:1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

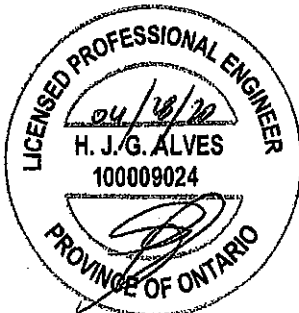
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.14 (E) INPUT = 0.90
JSI METAL = 0.09 (B) INPUT = 1.00



Structural component only
DWG# T-2007652

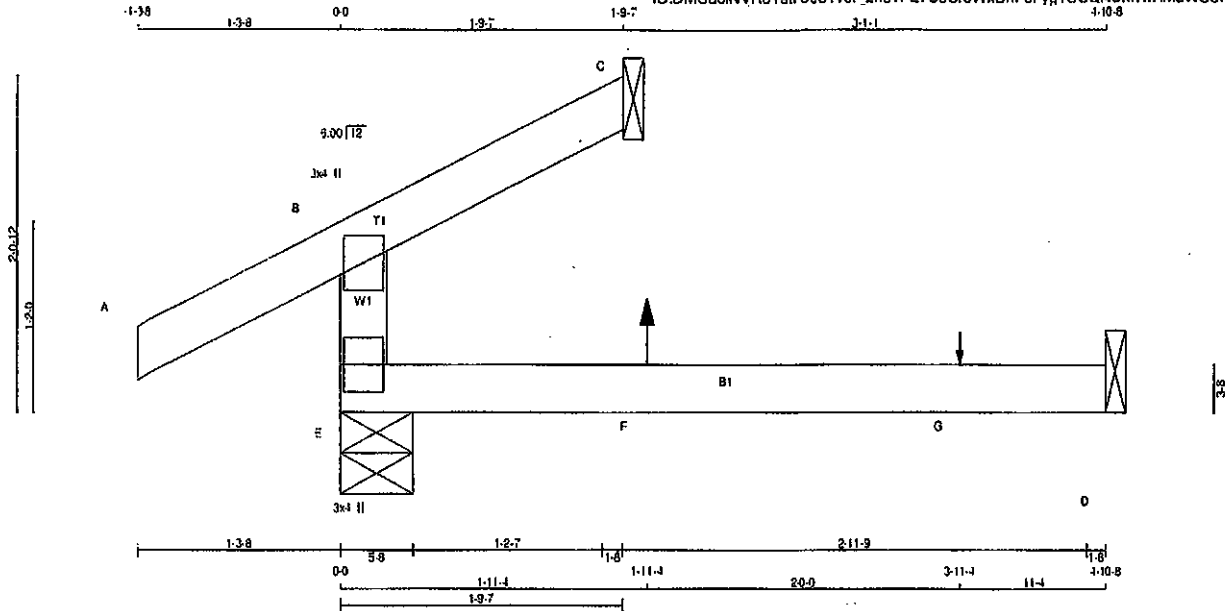
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	C45	2	1	GREEN PARK HOMES	
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:19:48 2020 Page 1

ID:DMCubINVR6TslFoe31vbl_zns11-E7DsCr3WxBnFcFyg1UCQ46kwwAMuWC0HTL1pJzME7v

Scale = 1/12



TOTAL WEIGHT = 2 X 10 = 21 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG			
E	282	0	282	0	5-8	5-8	
C	55	0	55	0	1-8	1-8	
D	35	0	44	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	198	137	0	0	0	81	0
C	39	21	0	0	0	18	0
D	29	0	4	0	0	31	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
E-B	-235	0	0.0	0.0	0.07 (4)	7.81	
A-B	0	26	-91.8	-91.8	0.12 (1)	10.00	
B-C	-13	5	-91.8	-91.8	0.07 (5)	6.25	
E-F	0	0	-18.5	-18.5	0.10 (4)	10.00	
F-G	0	0	-18.5	-18.5	0.10 (4)	10.00	
G-D	0	0	-18.5	-18.5	0.10 (4)	10.00	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	BACK	VERT	TOTAL	---	C1
G	3-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.12/1.00 (A-B:1), BC=0.10/1.00 (D-E:4),
WB=0.00/1.00 (A-B:3), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

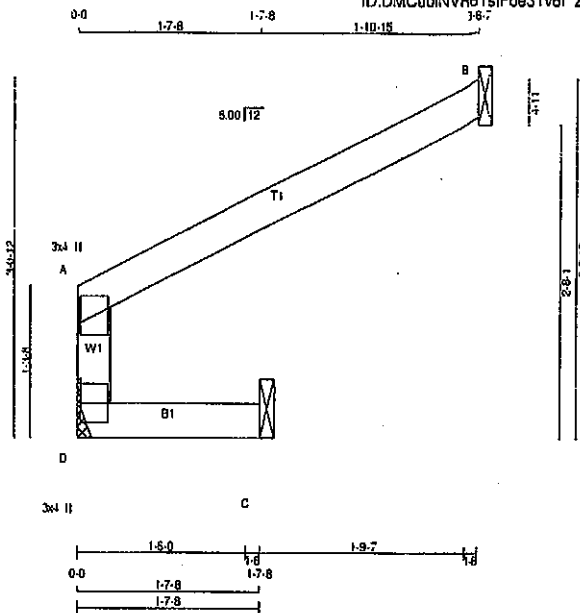
JSI GRIP= 0.10 (E) INPUT = 0.80
JSI METAL= 0.07 (B) INPUT = 1.00



Structural component only
DWG# T-2007653

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
48224	C46	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Tue Apr 26 10:19:49 2020 Page 1
ID:DMCubINVR6TslFoe31v6I zns11-iJnEQB48hUv6EPXtaBjIHfukKVjdSxW75bLtzME7U



Scale = 1:10.2

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
D - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
D - C	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (Table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	BMV+p	MT20	3.0	4.0
D	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
D	156	0	156	0	0	MECHANICAL			
B	144	0	144	0	0	1-8	1-8		
C	55	0	55	0	0	1-8	1-8		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS		1ST CASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
D	103	78	0	0	0
B	99	80	0	0	0
C	40	23	0	0	0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (LBS)	MAX. CS (LBS)	MEMB.	MAX. FORCE (LBS)
FR-TO				FR-TO	
D-A	-181	0	0.0 0.0 0.09 (1)	7.81	
A-B	-8	0	-91.8 -91.8 0.14 (1)	10.00	
D-C	0	0	-18.5 -18.5 0.09 (1)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (A-B:1), BC=0.09/1.00 (C-D:1), WB=0.00/1.00 (A-B:1), SS=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MT20	618	354
	1667	789
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.08 (D) INPUT = 0.90
JSI METAL = 0.05 (A) INPUT = 1.00



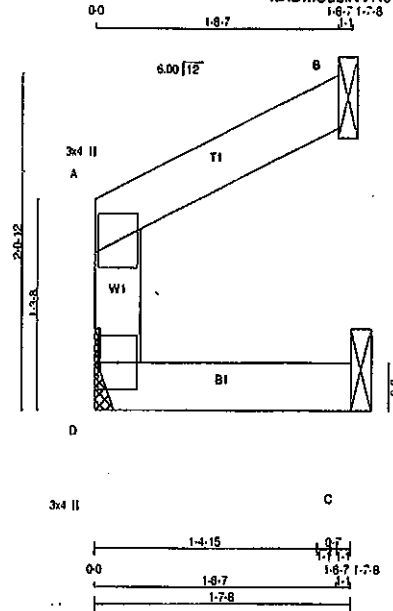
Structural component only
DWG# T-2007654

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408224	C47	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 10:19:50 2020 Page 1

ID:DMCubINVR6TsIFos31v8f zns1I-AWLddX5mSo1zrY638uEuqVB5ake7MQj5knq8uCzME7



Scale = 1:13.1

LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	SPF
CHORDS				
O - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN
A	TMV+p	MT20	3.0	4.0
O	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
D	86	0	86	0	0
B	87	0	67	0	1-8
C	19	0	19	0	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL B87791H FOR CONNECTION TO JOINTS B, C

UNFACTORED REACTIONS		1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
D	61	39 0	0 0	0 0	0 0	21 0	0 0	
B	46	37 0	0 0	0 0	0 0	9 0	0 0	
C	14	2 0	0 0	0 0	0 0	12 0	0 0	

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (PLF) CSI (LC)	MEMB.	MAX. FORCE (LBS) CSI (LC)
FR-TO				FR-TO	
D-A	-74 0	0.0	0.0 0.01 (1)	7.81	
A-B	-2 0	-91.8	-91.8 0.03 (1)	10.00	
D-C	0 0	-18.5	-18.5 0.01 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19')
CALCULATED VERT. DEFL.(LL) = L/999 (0.00')
ALLOWABLE DEFL.(TL) = L/360 (0.19')
CALCULATED VERT. DEFL.(TL) = L/999 (0.00')

CSI: TC=0.03/1.00 (A-B:1), BC=0.01/1.00 (C-D:4), WB=0.00/1.00 (n/a:0), SS=0.05/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.03 (D) INPUT = 0.90
JSI METAL= 0.02 (A) INPUT = 1.00



Structural component only
DWG# T-2007655

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing -- an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258-259.

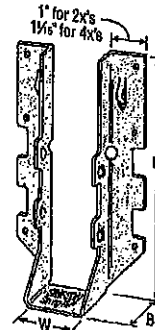
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20-24.

Installation:

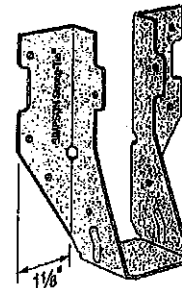
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

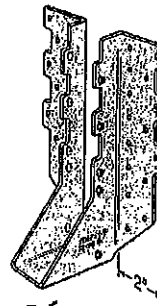
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options Information on p. 126.



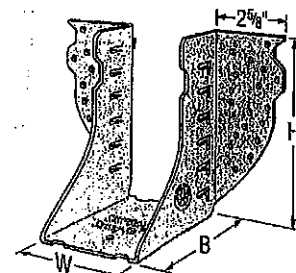
LUS28



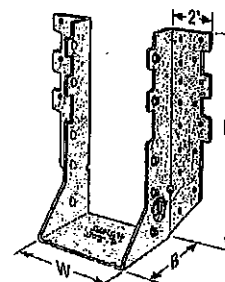
LU26L



HUS210
(HUS26, HUS28,
and HHUS similar)



HGUS28-2



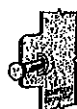
HHUS210-2



Double-Shear
Nailing
Top View

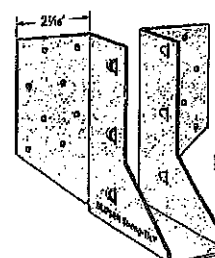
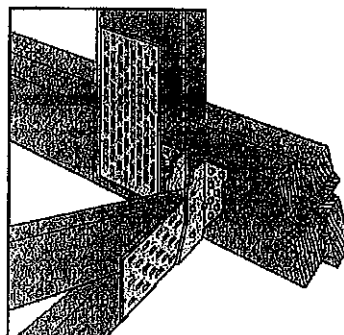


Double-Shear
Nailing
Side View;
Do not
bend tab



Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580

Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LJS26DS

Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS**HHUS/HGUS**

See Hanger Options Information on pp. 125–127.

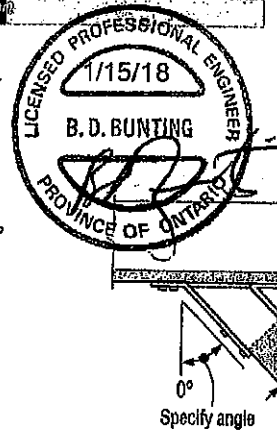
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _g ³	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15) lb.	(K _D = 1.00) lb.	(K _D = 1.15) lb.	(K _D = 1.00) lb.
Single 2x Sizes											
LUS24	18	1 1/8	3 1/2	1 3/4	2 1/4	(4) 10d	(2) 10d	710 3.18	1625 7.23	645 2.87	1155 5.14
LU24L	22	1 1/8	3	1 1/4	2 1/8	(4) 10d	(2) 10d x 1 1/2"	360 1.60	1020 4.54	320 1.42	725 3.22
LU26L	22	1 1/8	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"	720 3.20	1605 7.14	645 2.87	1140 5.07
LUS26	18	1 1/8	4 1/4	1 3/4	3 3/4	(4) 10d	(4) 10d	1420 6.32	2170 9.85	1290 5.74	1830 7.25
HUS26	18	1 1/8	5 1/4	3	3 1/8	(14) 16d	(8) 16d	2705 11.80	4940 21.97	2065 9.20	3875 17.24
LJS26DS	18	1 1/8	5	3 1/2	4 1/4	(16) 16d	(6) 16d	2055 9.14	4265 18.97	1460 6.48	4115 18.31
HGUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 16d	(8) 16d	2885 11.96	6625 28.51	2685 11.96	5700 25.35
LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"	1140 5.07	2185 9.72	1020 4.54	1550 8.89
LUS28	18	1 1/8	6 1/4	1 3/4	3 3/4	(6) 10d	(4) 10d	1420 6.32	2520 11.21	1290 5.74	1790 7.96
HUS28	16	1 1/8	7 1/4	3	6 1/8	(22) 16d	(8) 16d	3605 16.04	5385 23.88	2675 11.90	4345 19.33
HGUS28	12	1 1/8	7 1/4	5	6 1/4	(36) 16d	(12) 16d	3310 14.74	7675 34.19	3310 14.74	6900 30.73
LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(8) 10d x 1 1/2"	1140 5.07	2495 11.10	1020 4.54	1770 7.87
LUS210	18	1 1/8	7 1/4	1 3/4	3 3/4	(8) 10d	(4) 10d	1420 6.32	2785 12.39	1290 5.74	2210 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_g is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/4" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

SIMPSON

Strong-Tie®

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _g ¹	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15) lb.	(K _D = 1.00) lb.	(K _D = 1.15) lb.	(K _D = 1.00) lb.
								kN	kN	kN	kN
Double 2x Sizes											
LUS24-2	18	3 1/8	3 1/8	2	1 1/2	(4) 16d	(2) 16d	835	2020	590	1435
								3.71	8.89	2.62	6.38
LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.64
HHUS20-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2850	7335	2085	5205
								12.68	32.83	9.20	23.15
HGUS20-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8980	3110	6355
								19.51	39.81	13.83	28.27
LUS28-2	18	3 1/8	7	2	4	(8) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS28-2	14	3 1/8	7 1/8	3	8 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS28-2	12	3 1/8	7 1/8	4	8 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
								20.77	42.97	18.84	31.14
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(48) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
Triple 2x Sizes											
HGUS26-3	12	4 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-3	12	4 1/8	7 1/8	4	8 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d	4670	9670	4235	6865
								20.77	43.02	18.84	30.54
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(48) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-4	12	6 1/8	7 1/8	4	8 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-4	14	6 1/8	8 1/8	3	7 1/8	(30) 16d	(10) 16d	4670	10155	4235	7210
								20.77	45.17	18.84	32.07
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(48) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
LUS46	18	3 1/8	4 1/8	2	3 1/8	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS46	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2540	7335	2085	5205
								11.30	32.83	9.20	23.15
HGUS46	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS48	18	3 1/8	6 1/8	2	3 1/8	(8) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS48	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS48	12	3 1/8	7 1/8	4	8 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS410	18	3 1/8	8 1/8	2	5 1/8	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HGUS410	12	3 1/8	9	4	8 1/8	(48) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
HGUS412	12	3 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS414	12	3 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	61.80



Plated Truss Connectors

See footnotes
on p. 258.

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

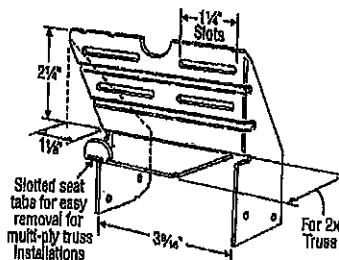
Design: Factored resistances are in accordance with CSA 086-14

Installation:

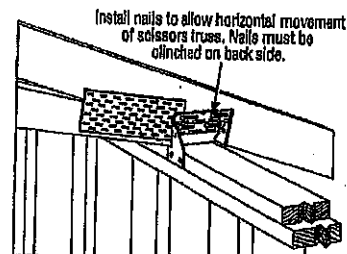
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

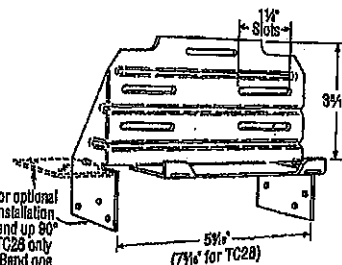
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



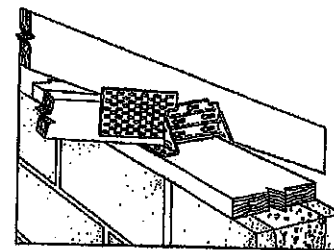
TC24
U.S. Patent 4,932,173



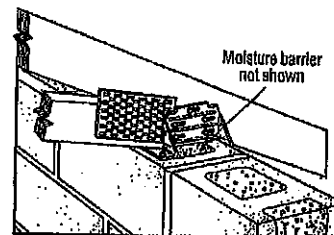
Typical TC24 Installation



TC26
(TC28 Similar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift ($K_F=1.15$)	Uplift ($K_F=1.15$)
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(5) 10d	1015	720
TC28	(5) 10d	(5) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift ($K_F=1.15$)	Uplift ($K_F=1.15$)
TC26	(5) 10d	(8) 10d x 1½"	810	660
	(5) 10d	(5) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.

2. Grout strength is 15 MPa minimum.

3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.

4. TC26 fastened to grouted concrete block with (5) - ¾" x 2¼" Titen screws has a factored uplift resistance of 275 lb.



UNITED STATES DESIGN

2017 Simpson Strong-Tie Company Inc.
All rights reserved. Simpson Strong-Tie Company Inc.
Simpson Strong-Tie Company Inc. is a registered trademark of Simpson Strong-Tie Company Inc.

SP-2017-12-28-01

(800) 999-5099
strongtie.com

Seismic and Hurricane Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Fasteners			Factored Resistance ($K_D = 1.15$)					
					D.Fir-L			S-P-F		
		To Rafter/Truss	To Plates	To Studs	Uplift	Lateral		Uplift	Lateral	
						F ₁	F ₂		F ₁	F ₂
					lb.	lb.	lb.	lb.	lb.	lb.
					kN	kN	kN	kN	kN	kN
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
					3.29	3.05	1.33	3.02	2.16	0.98
SS H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
					3.69	0.98	0.33	2.62	0.69	0.24
SS H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
					3.58	0.71	0.71	3.36	0.71	0.71
H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	180	210
					3.71	0.78	0.93	3.29	0.71	0.93
SS H3	18	(4) 8d	(4) 8d	—	740	180	285	615	125	180
					3.29	0.80	1.18	2.74	0.66	0.85
H6	16	—	(8) 8d	(8) 8d	1585	1085	—	1125	770	—
					7.05	4.83	—	5.00	3.43	—
H7Z	16	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
					6.18	2.99	—	4.40	2.11	—
SS H8 ³	18	(5) 10d x 1½"	(5) 10d x 1½"	—	1120	—	—	1025	—	—
					4.98	—	—	4.56	—	—
SS H10A ³	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290
					7.72	3.54	1.82	6.69	2.51	1.29
H10AR	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1485	690	430	1220	570	305
					6.61	3.07	1.91	5.43	2.54	1.36
H10A-2	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1835	1275	430	1845	880	305
					8.16	5.67	1.91	7.32	3.91	1.36
H10S ^{3/2}	18	(8) 8d x 1½"	(8) 8d x 1½"	(8) 8d	1465	795	315	1040	565	225
					6.52	3.54	1.40	4.63	2.51	1.00
H11Z	18	(6) 16d x 2½"	(6) 16d x 2½"	—	1095	920	545	780	655	390
					4.87	4.09	2.42	3.47	2.91	1.73
H14	18	[1] (12) 8d x 1½"	(13) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
		[2] (12) 8d x 1½"	(15) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
TSP	18	(9) 10d x 1½"	(6) 10d x 1½"	—	1295	440	—	920	310	—
					5.76	1.96	—	4.09	1.38	—
		(9) 10d x 1½"	(6) 10d	—	1560	440	—	1105	310	—
					6.94	1.96	—	4.92	1.38	—

- Factored resistances have been increased 15% for short term loading; no further increase is allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
- H8 factored uplift resistances for stud-to-bottom plate installations are 595 lb. (2.65 kN) for D.Fir-L and 390 lb. (1.74 kN) for S-P-F.
- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-H/TIECONPATH).
- Factored resistances in the F₁ direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.

- H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.38 kN) D.Fir-L and 1015 lb. (4.51 kN) S-P-F.
- H10S nails to plates are optional for uplift but required for lateral loads.
- H10A may be field-bent up to a slope of 6/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
- The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SSNAILS at strongtie.com.
- D.Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1½" nails into the top plates and (3) 8d x 1½" nails into the lowest three flange holes into the truss bottom chord is 495 lb. (2.20 kN).
- Nails: 16d x 2½" = 0.162" dia. x 2½" long, 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long, 8d x 1½" = 0.131" dia. x 1½" long. See pp. 27-28 for other nail sizes and information.

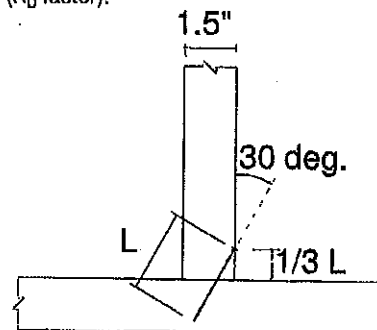
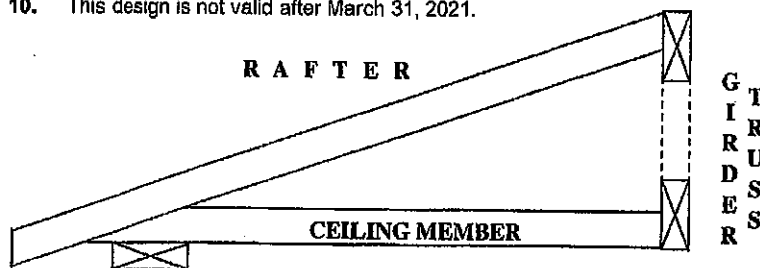
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON	3.00	0.144	132	147
	3.25	0.144	132	147
WIRE	3.50	0.160	159	177
	3.00	0.122	97	108
COMMON	3.25	0.122	97	108
	3.50	0.152	145	162
SPIRAL				

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.



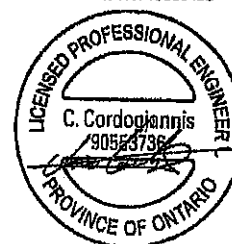
TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (In)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019

PEO
Certificate No. 10889485



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

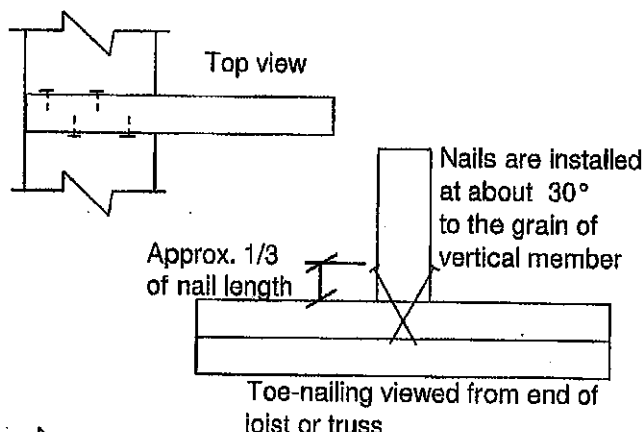
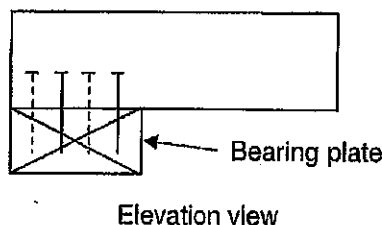
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	28	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

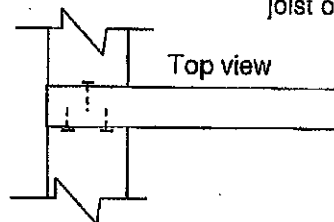
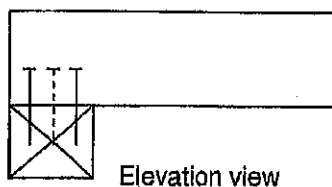
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



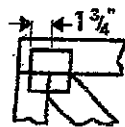
MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7



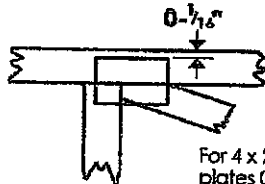
December 2, 2019

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint, unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/4" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

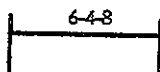


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

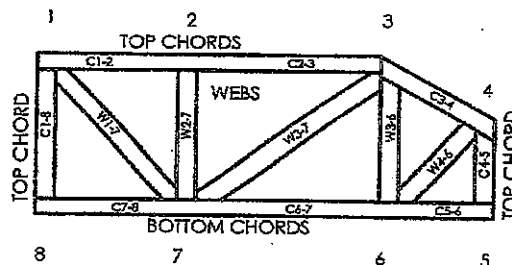
Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm.
(Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

© 2007 MiTek® All Rights Reserved

MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MIT-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane of joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

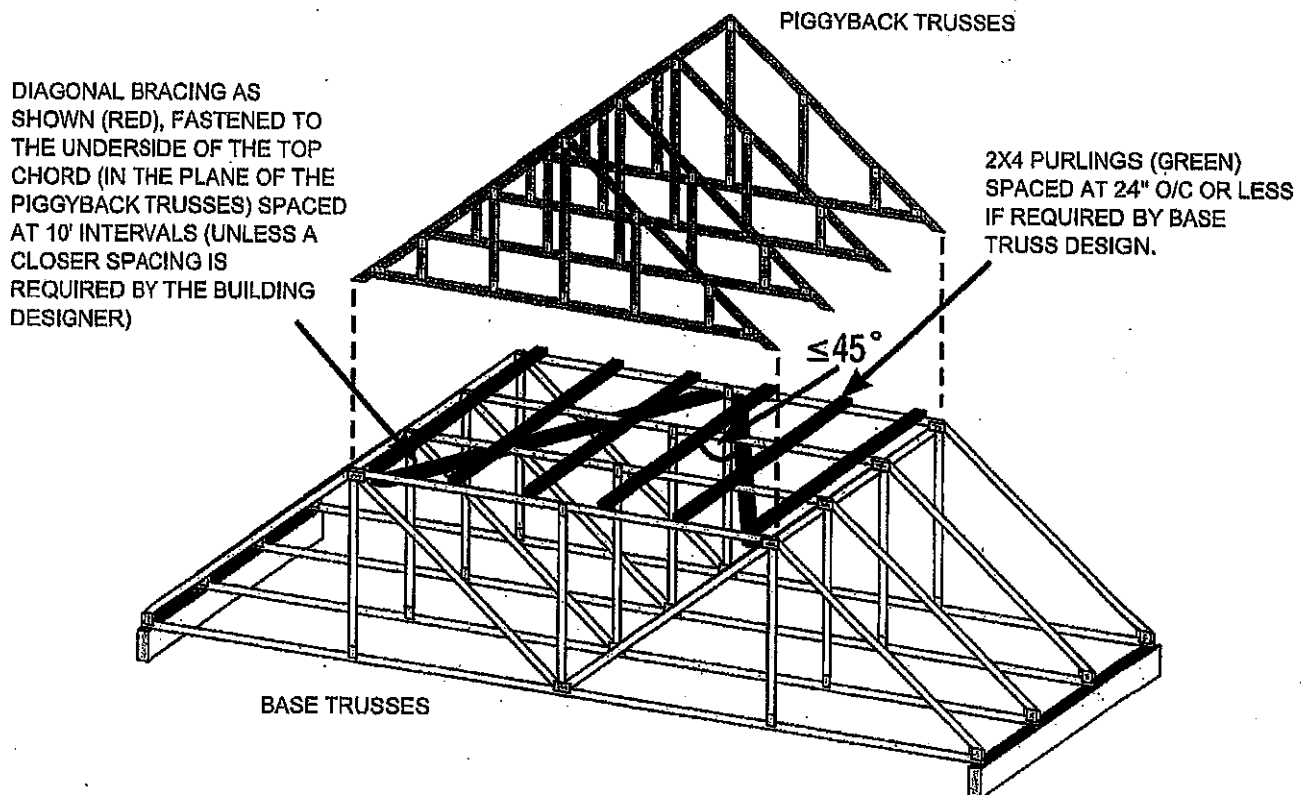
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

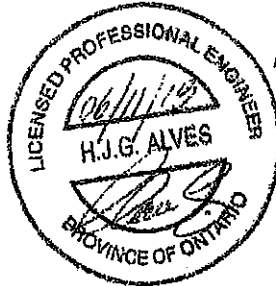


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218

Feb 09, 2018