

CITY OF HAMILTON
BUILDING DIVISION
Planning & Development Department

APR 09 2021

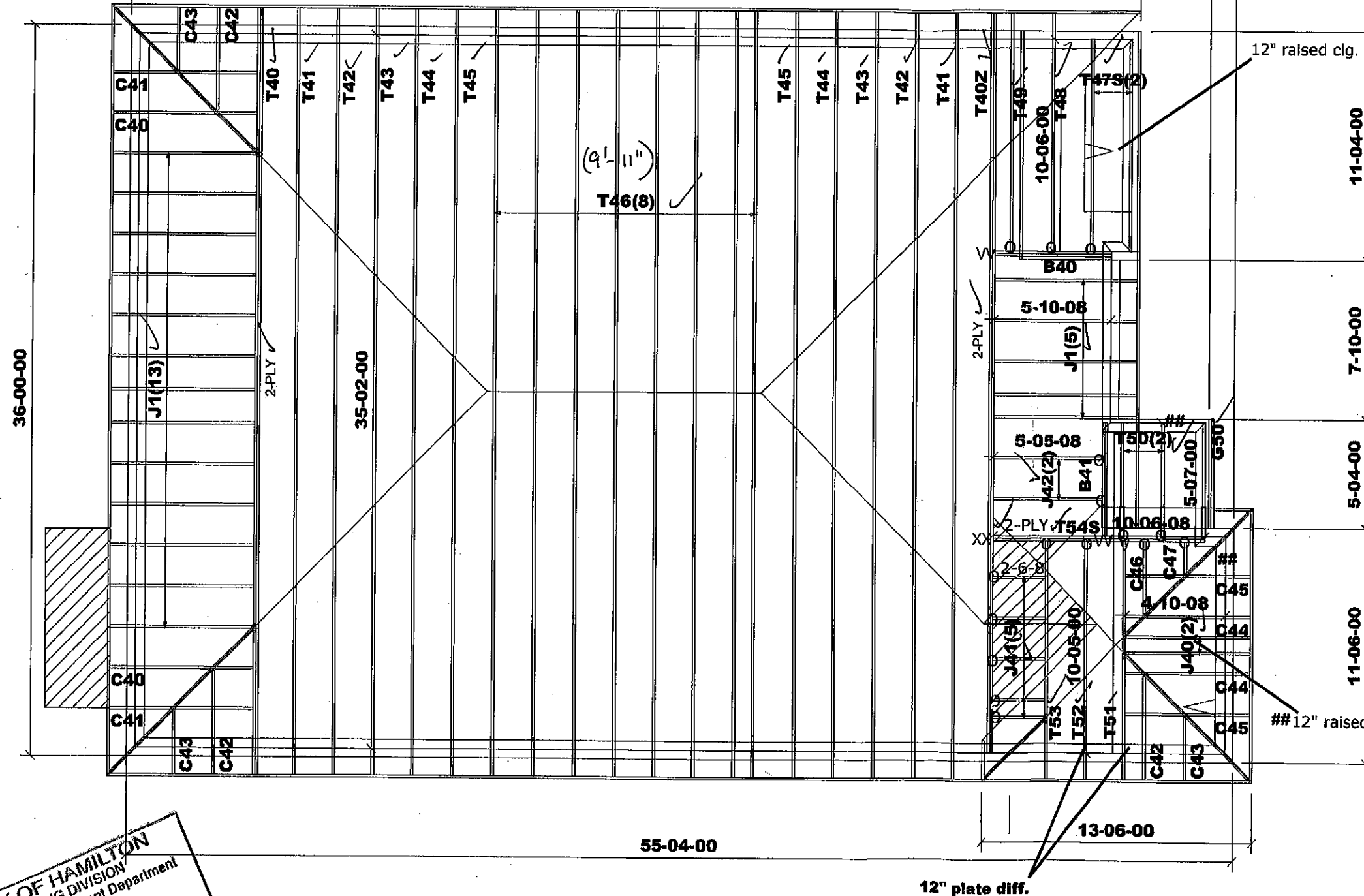
REC BY _____ DATE _____
REF'D TO _____ DATE _____

SINGLE FAMILY DWELLING ONLY

6/12 roof pitch unless noted

55-04-00
50-08-00

3-05-00
1-03-00



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
LUS26-2-(VV)

BEAMS:
B40= B41 =
2 - 2x10 SPF #2

DENOTES: CONVENTIONAL FRAMING

CITY OF HAMILTON
Building Division

Permit No. 21-105903

THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE
THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW

These drawings and/or specifications have been reviewed by

FOR CHIEF BUILDING OFFICIAL DATE

CITY OF HAMILTON
BUILDING DIVISION
Planning & Development Department

APR 09 2021

REC BY _____ DATE _____
REF'D TO _____ DATE _____

M13138



Job Track: 51225
Plan Log: 202419
Layout ID: 408224

Builder / Location: GREEN PARK HOMES / WATERDOWN
Project: RUSSELL GARDENS PH.3
Date: 2020-04-27 Sales: Mario DiCano Designer: JG

Model / Elevation: MOUNTAINASH 6/3- STD OR OPT.5 BR

Lot 216

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 6
 Lot #: 216
 Elevation: 3- STD OR OPT.5 BR

Job Track: 51225
 Plan Log: 202419
 Layout ID: 408224
 Ref #: 1 of 3
 Page: 1 of 3
 Date: 04-27-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T40 Hip Girder	6 /12	35-02-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	337.56 208.00		
	1 2-ply	T40Z Hip Girder	6 /12	35-02-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	337.56 208.00		
	2	T41 Hip	6 /12	35-02-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	277.99 174.00		
	2	T42 Hip	6 /12	35-02-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	277.12 174.67		
	2	T43 Hip	6 /12	35-02-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	289.07 179.67		
	2	T44 Hip	6 /12	35-02-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	290.73 183.33		
	2	T45 Hip	6 /12	35-02-00	9-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	301.67 191.33		
	8	T46 Common	6 /12	35-02-00	9-11-08	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1178.98 725.33		
	2	T47S Roof Special	0 /12	10-06-00	3-06-00	2 x 4		3-06-00 3-06-00	101.09 83.67		
	1	T48 Half Hip	6 /12	10-06-00	2-08-00	2 x 4	1-03-08	1-02-00 2-08-00	39.94 28.67		
	1	T49 Half Hip	6 /12	10-06-00	3-08-00	2 x 4	1-03-08	1-02-00 3-08-00	41.91 27.50		
	2	T50 Flat	0 /12	5-07-00	3-00-00	2 x 4		3-00-00 3-00-00	45.54 30.00		
	1	G50 GABLE	0 /12	5-07-00	3-00-00	2 x 4		3-00-00 3-00-00	22.18 15.00		
	1	T51 Hip Girder	6 /12	10-05-00	3-07-04	2 x 4	1-03-08	1-02-00 1-03-08	43.52 29.67		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 6
 Lot #: 216
 Elevation: 3- STD OR OPT.5 BR

Job Track: 51225
 PlanLog: 202419
 Layout ID: 408224
 Ref #
 Page: 2 of 3
 Date: 04-27-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T52 Hip	6 /12	10-05-00	4-00-00	2 x 4	1-03-08	2-02-00 2-03-08	45.94 29.83		
	1	T53 Hip Girder	6 /12	10-05-00	3-00-00	2 x 4	1-03-08	2-02-00 2-03-08	46.32 31.83		
	1 2-ply	T54S Roof Special Girder	0 /12	10-06-08	2-02-00	2 x 4 2 x 6		1-02-00 2-02-00	97.7 64.33		
	18	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	302.3 192.00		
	2	J40 Jack-Open	6 /12	4-10-08	3-07-04	2 x 4	1-03-08	1-02-00 3-07-04	28.76 18.67		
	5	J41 Jack-Open	6 /12	2-06-08	3-00-00	2 x 4		1-08-12 3-00-00	49.19 34.17		
	2	J42 Jack-Open	6 /12	5-05-08	4-01-04	2 x 4		1-04-08 4-01-04	31.44 20.67		
	2	C40 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	C41 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	3	C42 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	28.71 18.00		
	3	C43 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	21.08 14.00		
	2	C44 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 1-01-01	1-02-00 3-00-12	25.98 16.00		
	2	C45 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 3-01-01	1-02-00 2-00-12	20.88 13.33		
	1	C46 Jack-Open	6 /12	1-07-08	3-00-12	2 x 4	1-10-15	1-03-08 2-01-04	7.48 5.33		

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: RUSSELL GARDENS PH.3 Location: WATERDOWN Model: MOUNTAINASH 6 Lot #: 216 Elevation: 3- STD OR OPT.5 BR	Job Track: 51225 PlanLog: 202419 Layout ID: 408224 Ref # Page: 3 of 3 Date: 04-27-2020 Designer: Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	C47 Jack-Open	6 /12	1-07-08	2-00-12	2 x 4		1-03-08 2-01-04	4.91 3.33		

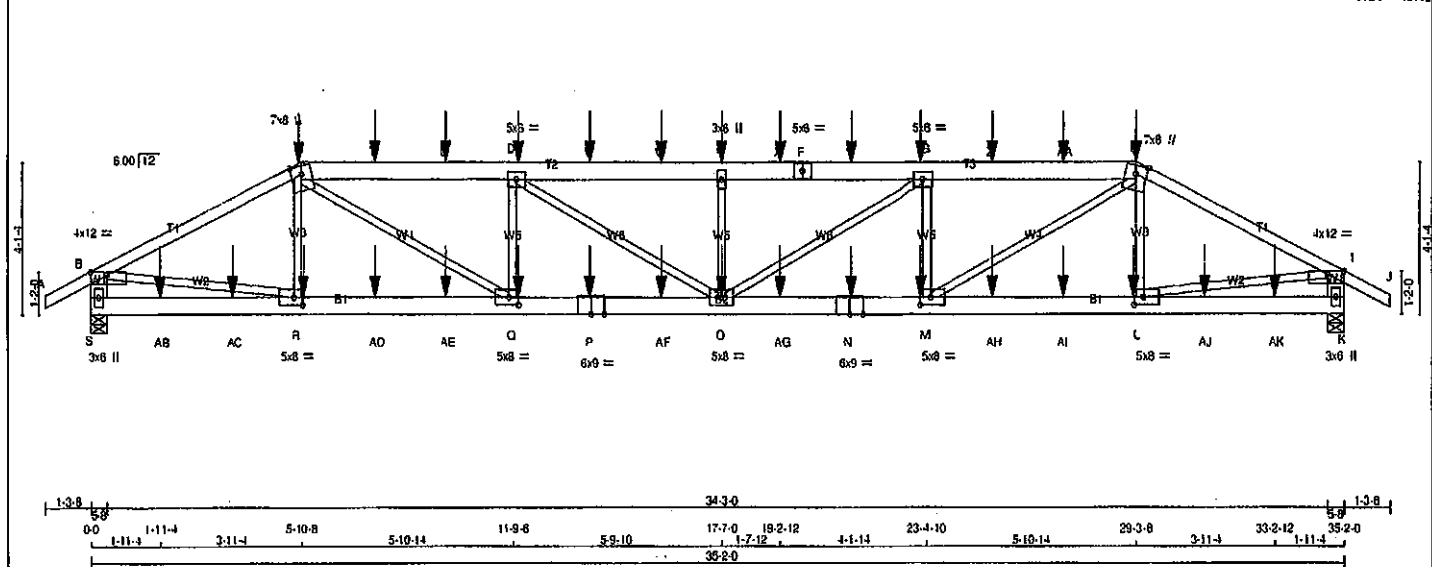
TOTAL # TRUSS= 77 TOTAL BFT OF ALL TRUSSES= 2730.33 BFT. TOTAL WEIGHT OF ALL TRSSES 4346.93 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
1	Hardware	LJS26DS	
14	Hardware	LUS24	
2	Hardware	LUS26-2	

TOTAL NUMBER OF
ITEMS= 18

JOB NAME 408224	TRUSS NAME T40	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 10:19:57 2020 Page 1	
ID:DMCubINVR81stFoe31v6l_zns11-TsGG5wA9pyw_Bd8P2IsXGz_APYAVTa7LN10dzME7m				Scale = 1:57.6	



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x4 DRY No.2 SPF C - F 2x6 DRY No.2 SPF F - H 2x6 DRY No.2 SPF H - J 2x4 DRY No.2 SPF S - B 2x6 DRY No.2 SPF K - I 2x6 DRY No.2 SPF S - P 2x6 DRY 1650F 1.5E SPF P - N 2x6 DRY 1650F 1.5E SPF N - K 2x6 DRY 1650F 1.5E SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF) TOP CHORDS : 10.122"x3" SPIRAL NAILS A-G 1 12 SIDE(61.0) H-J 1 12 SIDE(61.0) C-F 2 12 SIDE(183.1) F-H 2 12 SIDE(183.1) S-B 2 12 TOP K-I 2 12 TOP BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS S-P 2 12 SIDE(183.1) P-N 2 12 SIDE(183.1) N-K 2 12 SIDE(183.1) WEBS : 10.122"x3" SPIRAL NAILS 2x3 1 6 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.		DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION DOWN BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX S 3323 0 3323 0 0 5-8 5-8 K 3323 0 3323 0 0 5-8 5-8 UNFACTORED REACTIONS 1ST LCASE MAX MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL S 2350 1539 0 0 0 0 0 811 0 0 0 K 2350 1539 0 0 0 0 0 811 0 0 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) <table border="1"> <thead> <tr> <th colspan="2">CHORDS</th> <th colspan="2">WEBS</th> </tr> <tr> <th>MEMB.</th> <th>FORCE (LBS)</th> <th>MEMB.</th> <th>FORCE (LBS)</th> </tr> </thead> <tbody> <tr> <td>FR-TO</td> <td>0 28</td> <td>FR-TO</td> <td>0 28</td> </tr> <tr> <td>A-B</td> <td>0 28</td> <td>A-B</td> <td>0 28</td> </tr> <tr> <td>B-C</td> <td>-5065 0</td> <td>B-C</td> <td>-5065 0</td> </tr> <tr> <td>C-T</td> <td>-7284 0</td> <td>C-T</td> <td>-7284 0</td> </tr> <tr> <td>T-U</td> <td>-7284 0</td> <td>T-U</td> <td>-7284 0</td> </tr> <tr> <td>U-D</td> <td>-7284 0</td> <td>U-D</td> <td>-7284 0</td> </tr> <tr> <td>D-V</td> <td>-8084 0</td> <td>D-V</td> <td>-8084 0</td> </tr> <tr> <td>V-W</td> <td>-8084 0</td> <td>V-W</td> <td>-8084 0</td> </tr> <tr> <td>W-E</td> <td>-8084 0</td> <td>W-E</td> <td>-8084 0</td> </tr> <tr> <td>E-X</td> <td>-8084 0</td> <td>E-X</td> <td>-8084 0</td> </tr> <tr> <td>X-F</td> <td>-8084 0</td> <td>X-F</td> <td>-8084 0</td> </tr> <tr> <td>F-Y</td> <td>-8084 0</td> <td>F-Y</td> <td>-8084 0</td> </tr> <tr> <td>Y-G</td> <td>-8084 0</td> <td>Y-G</td> <td>-8084 0</td> </tr> <tr> <td>G-Z</td> <td>-7284 0</td> <td>G-Z</td> <td>-7284 0</td> </tr> <tr> <td>Z-AA</td> <td>-7284 0</td> <td>Z-AA</td> <td>-7284 0</td> </tr> <tr> <td>AA-H</td> <td>-7284 0</td> <td>AA-H</td> <td>-7284 0</td> </tr> <tr> <td>H-I</td> <td>-5065 0</td> <td>H-I</td> <td>-5065 0</td> </tr> <tr> <td>I-J</td> <td>0 28</td> <td>I-J</td> <td>0 28</td> </tr> <tr> <td>S-B</td> <td>-3248 0</td> <td>S-B</td> <td>-3248 0</td> </tr> <tr> <td>K-I</td> <td>-3248 0</td> <td>K-I</td> <td>-3248 0</td> </tr> </tbody> </table>		CHORDS		WEBS		MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)	FR-TO	0 28	FR-TO	0 28	A-B	0 28	A-B	0 28	B-C	-5065 0	B-C	-5065 0	C-T	-7284 0	C-T	-7284 0	T-U	-7284 0	T-U	-7284 0	U-D	-7284 0	U-D	-7284 0	D-V	-8084 0	D-V	-8084 0	V-W	-8084 0	V-W	-8084 0	W-E	-8084 0	W-E	-8084 0	E-X	-8084 0	E-X	-8084 0	X-F	-8084 0	X-F	-8084 0	F-Y	-8084 0	F-Y	-8084 0	Y-G	-8084 0	Y-G	-8084 0	G-Z	-7284 0	G-Z	-7284 0	Z-AA	-7284 0	Z-AA	-7284 0	AA-H	-7284 0	AA-H	-7284 0	H-I	-5065 0	H-I	-5065 0	I-J	0 28	I-J	0 28	S-B	-3248 0	S-B	-3248 0	K-I	-3248 0	K-I	-3248 0
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Structural component only
DWG# T-2007660 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	T40	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	
version 8.310 S Oct 29 2019 MTek Industries, Inc. Tue Apr 28 10:19:57 2020 Page 2					
ID:DMCubINVR6TsIFoe31v6I zns1I-TsGG5wA9pyw Bd8P2IsXcz APYAIVTa7LN10dlzME7m					

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	12.0	1.00	5.50
C	TTWW-m	MT20	7.0	8.0	Edge	
D	TMVW-t	MT20	5.0	6.0		
E	TMVW-w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMVW-t	MT20	5.0	6.0		
H	TTWW-m	MT20	7.0	8.0	Edge	
I	TMVW-p	MT20	4.0	12.0	1.00	5.50
K	BMV1-p	MT20	3.0	8.0		
L	BMVW-t	MT20	5.0	8.0	2.50	3.00
M	BMVW-t	MT20	5.0	8.0	2.50	3.25
N	BS-t	MT20	6.0	9.0		
O	BMVWV-t	MT20	5.0	8.0		
P	BS-t	MT20	6.0	9.0		
Q	BMVW-t	MT20	5.0	8.0	2.50	3.25
R	BMVW-t	MT20	5.0	8.0	2.50	3.00
S	BMV1-p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	17-7-0	-110	-110	---	FRONT	VERT	TOTAL	---	C1
G	23-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
H	29-3-8	-429	-429	---	FRONT	VERT	TOTAL	---	C1
L	29-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
M	23-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
N	21-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	17-7-0	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	13-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Q	11-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
R	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
T	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
U	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	15-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	19-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	21-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	25-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	27-2-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	1-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
AC	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AD	7-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AE	9-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AF	15-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AG	19-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AH	25-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AI	27-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AJ	31-2-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AK	33-2-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1

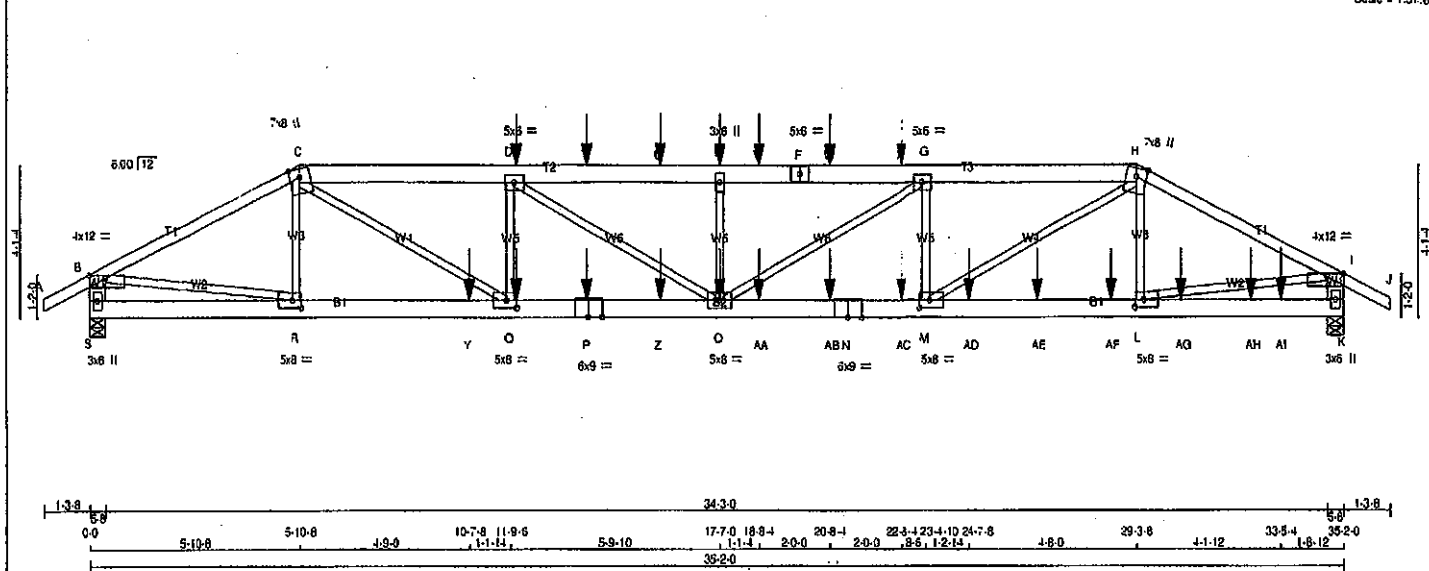
CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007660

Tamarack Roof Truss, Burlington Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:19:58 2020 Page 1
 ID:DMCubINVR6TslFos31v6I zns1l-x2qsJGBoaF2rpnjbcaNm9BWHayQ8EpcGa1mZakzME7I
 1-3-8 0-0 5-10-8 5-10-8 5-10-14 11-9-8 5-9-10 17-7-0 18-8-4 20-8-4 22-4-23-4-10 5-10-14 29-3-8 35-2-0 35-5-8 1-3-8
 1-3-8 0-0 5-10-8 5-10-8 5-10-14 11-9-8 5-9-10 17-7-0 18-8-4 20-8-4 22-4-23-4-10 5-10-14 29-3-8 35-2-0 35-5-8 1-3-8
 Scale = 1:57.6



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - F	2x6	DRY No.2	SPF
F - H	2x6	DRY No.2	SPF
H - J	2x4	DRY No.2	SPF
S - B	2x6	DRY No.2	SPF
K - I	2x6	DRY No.2	SPF
S - P	2x8	DRY 1650F 1.5E	SPF
P - N	2x8	DRY 1650F 1.5E	SPF
N - K	2x8	DRY 1650F 1.5E	SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF)

TOP CHORDS : (0.122"x3") SPIRAL NAILS

CHORD	SIZE	SPACING (IN)	LOAD (PLF)
A-C	12	12	TOP
H-J	12	12	TOP
C-F	12	12	SIDE(183.1)
F-H	12	12	SIDE(0.0)
S-B	12	12	TOP
K-I	12	12	TOP

BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS

CHORD	SIZE	SPACING (IN)	LOAD (PLF)
S-P	12	12	SIDE(183.1)
P-N	12	12	SIDE(183.1)
N-K	12	12	SIDE(0.0)

WEBS : (0.122"x3") SPIRAL NAILS

WEB	SIZE	SPACING (IN)	LOAD (PLF)
2x3	6	6	TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	4161	0	5-8
S	5042	0	5-8

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	2936 1965 0 0 0 0 0 0 970 0 0 0
S	3552 2409 0 0 0 0 0 0 1143 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.95 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MEMB.	FORCE
FR-TO	0	0	FR-TO	0
A-B	-8650	-91.8	R-C	-470
B-C	-10735	-91.8	C-Q	0
C-D	-12119	-91.8	D-O	0
D-E	-12119	-91.8	E-F	-1004
E-F	-12119	-91.8	F-G	-46
F-G	-12119	-91.8	G-H	-826
G-H	-12119	-91.8	H-I	0
H-I	-12119	-91.8	I-J	0
I-J	-12119	-91.8	J-K	0
J-K	-12119	-91.8	K-L	0

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
D	11-11-4	-110	-110	BACK	VERT	TOTAL	---	C1
E	17-7-0	-110	-110	BACK	VERT	TOTAL	---	C1
O	17-7-0	-26	-26	BACK	VERT	TOTAL	---	C1
P	13-11-4	-26	-26	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	DL	PSF
25.6	6.0	PSF
0.0	0.0	PSF
7.4	7.4	PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")

CALCULATED VERT. DEFL.(LL) = L/999 (0.32")

ALLOWABLE DEFL.(TL) = L/360 (1.17")

CALCULATED VERT. DEFL.(TL) = L/716 (0.59")

CSI: TC=0.81 (1.00 (H-I)), BC=0.71 (1.00 (M-O)), WB=0.90 (1.00 (L-I)), SS=0.64 (1.00 (L-M))

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(OR)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (R) (INPUT = 0.90)

JSI METAL = 0.93 (P) (INPUT = 1.00)



Structural component only
 DWG# T-2007661

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	T40Z	1	2	GREEN PARK HOMES	
T40Z		1		TRUSS DESC.	

T40Z Roof Truss, Burlington

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ID:DMCubjNVR6TsIFoe31v6l zns1l-x2qeJGBaF2pnibcaNm9BWHeyQ8EpcGatmZAKzME7l

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	12.0	1.00	5.50
C	TTWW+m	MT20	7.0	8.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	7.0	8.0	Edge	
I	TMVW-p	MT20	4.0	12.0	1.00	5.50
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	8.0	2.50	3.00
M	BMWW-t	MT20	5.0	8.0	2.50	3.25
N	BS-t	MT20	6.0	9.0		
O	BMWW-t	MT20	5.0	8.0		
P	BS-t	MT20	6.0	9.0		
Q	BMWW-t	MT20	5.0	8.0	2.50	3.25
R	BMWW-t	MT20	5.0	8.0	2.50	3.00
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

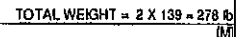
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	11-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
T	13-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
U	15-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
V	18-8-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
W	20-8-4	-159	-159	---	BACK	VERT	TOTAL	---	C1
X	22-8-4	-159	-159	---	BACK	VERT	TOTAL	---	C1
Y	10-7-8	-1171	-1171	---	BACK	VERT	TOTAL	---	C1
Z	15-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AA	18-8-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AB	20-8-4	-32	-32	---	BACK	VERT	TOTAL	---	C1
AC	22-8-4	-32	-32	---	BACK	VERT	TOTAL	---	C1
AD	24-7-8	-2237	-2237	---	BACK	VERT	TOTAL	---	C1
AE	26-8-12	-121	-121	---	BACK	VERT	TOTAL	---	C1
AF	28-8-12	-121	-121	---	BACK	VERT	TOTAL	---	C1
AG	30-8-12	-121	-121	---	BACK	VERT	TOTAL	---	C1
AH	32-8-12	-121	-121	---	BACK	VERT	TOTAL	---	C1
AI	33-5-4	-121	-121	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

11 C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007661 7/2



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00-12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

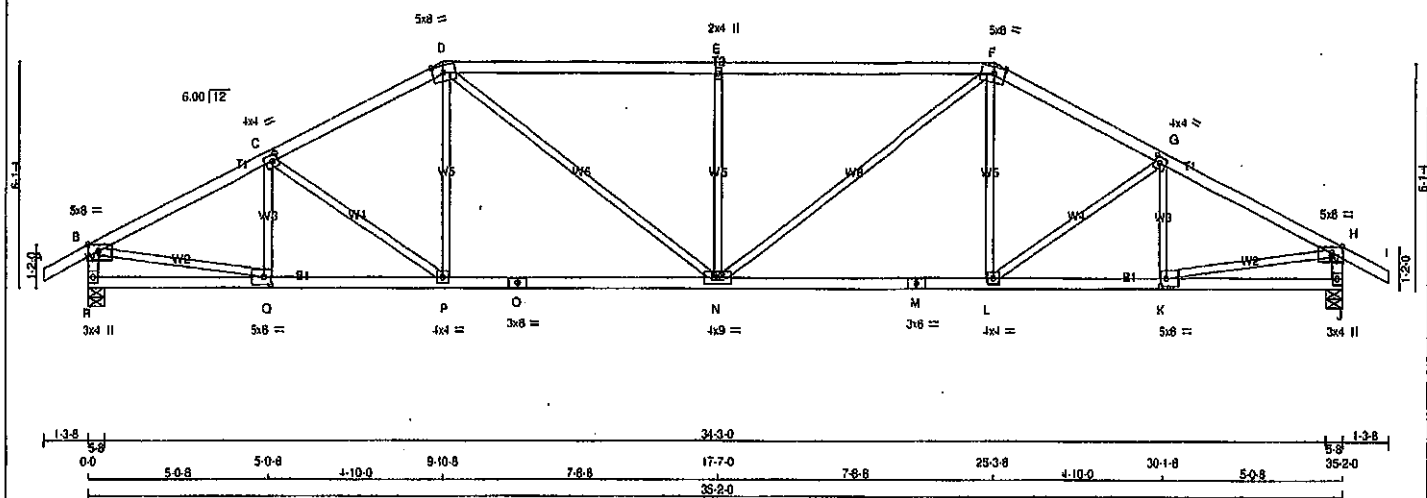
BRACING											
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.94 FT.											
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED											
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.											
LOADING											
TOTAL LOAD CASES: (4)											
CHORDS						WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)			
FR-TO		FROM TO				FR-TO					
A-B	0 28	-91.8	-91.8	0.12 (1)	10.00	C-R	0 93	0.03 (4)			
B-C	0 16	-91.8	-91.8	0.20 (1)	10.00	R-D	0 121	0.04 (4)			
C-D	-2799 0	-91.8	-91.8	0.32 (1)	3.88	D-P	0 1267	0.29 (1)			
D-E	-3508 0	-91.8	-91.8	0.95 (1)	2.94	P-E	-943 0	0.25 (1)			
E-F	-3507 0	-91.8	-91.8	0.93 (1)	2.94	E-O	-2 0	0.00 (1)			
F-G	-3507 0	-91.8	-91.8	0.93 (1)	2.94	O-G	-943 0	0.25 (1)			
G-H	-3507 0	-91.8	-91.8	0.94 (1)	2.95	O-H	0 1266	0.28 (1)			
H-I	-2799 0	-91.8	-91.8	0.32 (1)	3.88	M-H	0 121	0.04 (4)			
I-J	0 16	-91.8	-91.8	0.20 (1)	10.00	M-I	0 93	0.03 (4)			
J-K	0 28	-91.8	-91.8	0.12 (1)	10.00	S-C	-2974 0	0.83 (1)			
S-B	-270 0	0.0	0.0	0.03 (1)	7.81	I-L	-2974 0	0.83 (1)			
L-J	-270 0	0.0	0.0	0.03 (1)	7.81						
S-R	0 2417	-18.5	-18.5	0.53 (1)	10.00						
R-O	0 2469	-18.5	-18.5	0.54 (1)	10.00						
Q-P	0 2469	-18.5	-18.5	0.54 (1)	10.00						
P-Q	0 3509	-18.5	-18.5	0.64 (1)	10.00						
Q-N	0 2469	-18.5	-18.5	0.54 (1)	10.00						
N-M	0 2469	-18.5	-18.5	0.54 (1)	10.00						
M-L	0 2417	-18.5	-18.5	0.53 (1)	10.00						

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408224	T42 ✓	2 ✓	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TsiFos31v6l_zns11-LdWnxHEqIAQpFSAHixTmPBU9V7RFJIG??En3zME7i

1-3.8 0.0 5-0.8 5-0.8 4-10-0 9-10-8 7-6.8 17-7-0 7-6.8 25-3-8 4-10-0 30-1-8 5-0.8 1-3.8
Scale = 1:57.6



TOTAL WEIGHT = 2 X 139 = 277 lb (MKT)

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
E - F	2x4	DRY	No.2
G - I	2x4	DRY	No.2
J - K	2x4	DRY	No.2
L - M	2x4	DRY	No.2
N - O	2x4	DRY	No.2
P - Q	2x4	DRY	No.2
R - S	2x4	DRY	No.2
T - U	2x4	DRY	No.2
V - W	2x4	DRY	No.2
X - Y	2x4	DRY	No.2
Z	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	Edge	3.50
C	TMWW-i	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	2.25	3.75
E	TMWW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	2.25	3.75
G	TMWW-i	MT20	4.0	4.0	2.00	1.75
H	TMWW-p	MT20	5.0	8.0	Edge	3.50
J	BMVt-p	MT20	3.0	4.0		
K	BMWW-i	MT20	5.0	6.0	2.50	2.00
L	BMWW-i	MT20	4.0	4.0		
M	BS-i	MT20	3.0	6.0		
N	BMWW-w	MT20	4.0	4.0	2.00	1.75
O	BS-i	MT20	3.0	6.0		
P	BMWW-i	MT20	4.0	4.0		
Q	BMWW-i	MT20	5.0	6.0	2.50	2.00
R	BMVt-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
R	2063	0	2063	0	5-8	5-8
J	2063	0	2063	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
R	1457	969	0	0	0	488	0
J	1457	969	0	0	0	488	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM TO	LENGTH	FR-TO		FROM TO	LENGTH
A-B	0 28	-91.8 -91.8	0.12 (1)	10.00	Q-C	-353 0	0.08 (1)
B-C	-2834 0	-91.8 -91.8	0.39 (1)	3.81	C-P	-210 0	0.13 (1)
C-D	-2867 0	-91.8 -91.8	0.37 (1)	3.91	P-D	0 246	0.06 (4)
D-E	-3060 0	-91.8 -91.8	0.97 (1)	2.78	D-N	0 844	0.19 (1)
E-F	-3060 0	-91.8 -91.8	0.97 (1)	2.78	N-E	-872 0	0.51 (1)
F-G	-2867 0	-91.8 -91.8	0.37 (1)	3.91	N-F	0 844	0.19 (1)
G-H	-2834 0	-91.8 -91.8	0.39 (1)	3.81	L-F	0 246	0.06 (4)
H-I	0 28	-91.8 -91.8	0.12 (1)	10.00	L-G	-210 0	0.13 (1)
I-J	-2018 0	0.0 0.0	0.20 (1)	5.94	K-G	-353 0	0.08 (1)
J-H	-2018 0	0.0 0.0	0.20 (1)	5.94	B-Q	0 2591	0.58 (1)
					K-H	0 2591	0.58 (1)
R-Q	0 0	-18.5 -18.5	0.10 (4)	10.00			
Q-P	0 2555	-18.5 -18.5	0.51 (1)	10.00			
P-O	0 2386	-18.5 -18.5	0.49 (1)	10.00			
O-N	0 2386	-18.5 -18.5	0.49 (1)	10.00			
N-M	0 2386	-18.5 -18.5	0.49 (1)	10.00			
M-L	0 2386	-18.5 -18.5	0.49 (1)	10.00			
L-K	0 2555	-18.5 -18.5	0.51 (1)	10.00			
K-J	0 0	-18.5 -18.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 6.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/899 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/939 (0.34")
CSI: TC=0.97 (1.00) (D-E:1), BC=0.51 (1.00) (P-Q:1),
WB=0.58 (1.00) (B-Q:1), SS=0.34 (1.00) (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1687	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

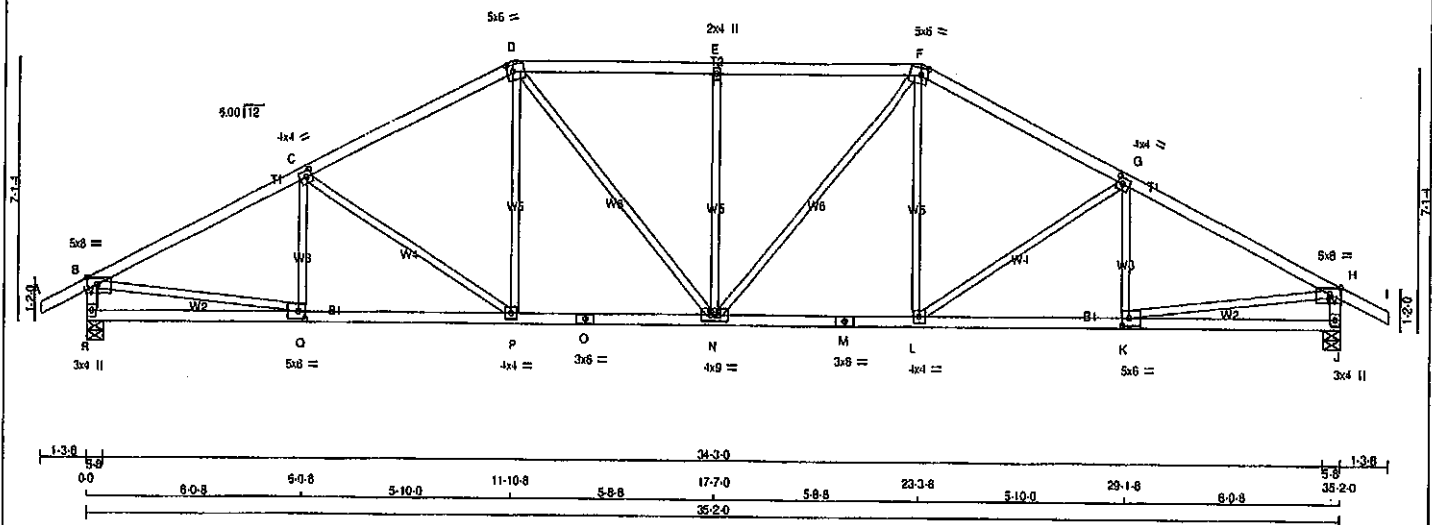
JSI GRIP = 0.87 (F) INPUT = 0.90

JSI METAL = 0.75 (M) INPUT = 1.00



Structural component only
DWG# T-2007663

JOB NAME 408224	TRUSS NAME T43	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Pamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 AM Tek Industries, Inc. Tue Apr 28 10:20:02 2020 Page 1	
ID:DMCubINVR6TstFoe31v6l_zns1l-qp488dEteUYGH01NqQSIU1h?BZrVAQsViknJvzME7h				35-2-0 35-5-8 1-3-8	
Scale = 1:57.6					



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-p	MT20	5.0	8.0 Edge 3.50
C	TMVW-I	MT20	4.0	4.0 2.00 1.75
D	TTVW-m	MT20	5.0	8.0 2.25 2.00
E	TMVW-w	MT20	2.0	4.0
F	TTVW-m	MT20	5.0	6.0 2.25 2.00
G	TMVW-I	MT20	4.0	4.0 2.00 1.75
H	TMVW-p	MT20	5.0	8.0 Edge 3.50
J	BMV1-p	MT20	3.0	4.0
K	BMVW-I	MT20	5.0	8.0 2.50 2.25
L	BS-I	MT20	4.0	4.0
M	BS-I	MT20	3.0	8.0
N	BMVW-I	MT20	4.0	8.0
O	BS-I	MT20	3.0	8.0
P	BMVW-I	MT20	4.0	4.0
Q	BMVW-I	MT20	5.0	6.0 2.50 2.25
R	BMV1-p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
R	2063	0	0	2063	0	0	5-8	5-8	5-8
J	2063	0	0	2063	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	COMBINED	SNOW	LIVE	PERM. LIVE
R	1457	869	0	0	0	0	0	488	0	0	0
J	1457	869	0	0	0	0	0	488	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		WEBS		MAX. FACTORED		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MAX. UNBRACED LENGTH	FR-TO	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MAX. UNBRACED LENGTH	FR-TO
A-B	0	28	-91.8	-91.8 0.12 (1)	10.00	O-C	-255	12	0.07 (1)		
B-C	-2869	0	-91.8	-91.8 0.58 (1)	3.58	C-P	-435	0	0.42 (1)		
C-D	-2539	0	-91.8	-91.8 0.52 (1)	3.83	P-D	0	360	0.08 (1)		
D-E	-2559	0	-91.8	-91.8 0.48 (1)	3.85	D-N	0	462	0.11 (1)		
E-F	-2539	0	-91.8	-91.8 0.48 (1)	3.85	N-E	-541	0	0.86 (1)		
F-G	-2539	0	-91.8	-91.8 0.52 (1)	3.83	N-F	0	482	0.11 (1)		
G-H	-2869	0	-91.8	-91.8 0.58 (1)	3.58	L-F	0	360	0.08 (1)		
H-I	0	28	-91.8	-91.8 0.12 (1)	10.00	L-G	-435	0	0.42 (1)		
R-B	-2014	0	0.0	0.0 0.20 (1)	5.95	K-G	-255	12	0.07 (1)		
J-H	-2014	0	0.0	0.0 0.20 (1)	5.95	B-Q	0	2635	0.59 (1)		
R-Q	0	0	-18.5	-18.5 0.15 (4)	10.00	K-H	0	2635	0.59 (1)		
Q-P	0	2809	-18.5	-18.5 0.49 (1)	10.00						
P-O	0	2249	-18.5	-18.5 0.43 (1)	10.00						
O-N	0	2249	-18.5	-18.5 0.43 (1)	10.00						
N-M	0	2249	-18.5	-18.5 0.43 (1)	10.00						
M-L	0	2249	-18.5	-18.5 0.43 (1)	10.00						
L-K	0	2609	-18.5	-18.5 0.49 (1)	10.00						
K-J	0	0	-18.5	-18.5 0.15 (4)	10.00						

TOTAL WEIGHT = 2 X 145 = 289 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (11.7")
CALCULATED VERT. DEFL. (LL) = L/999 (0.15")
ALLOWABLE DEFL. (TL) = L/360 (11.7")
CALCULATED VERT. DEFL. (TL) = L/999 (0.28")
CS1: TC=0.58/1.00 (G-H:1), BC=0.49/1.00 (K-L:1), WB=0.59/1.00 (H-K:1), SS=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

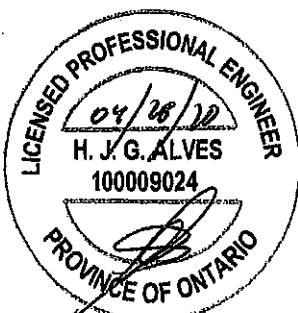
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1997 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

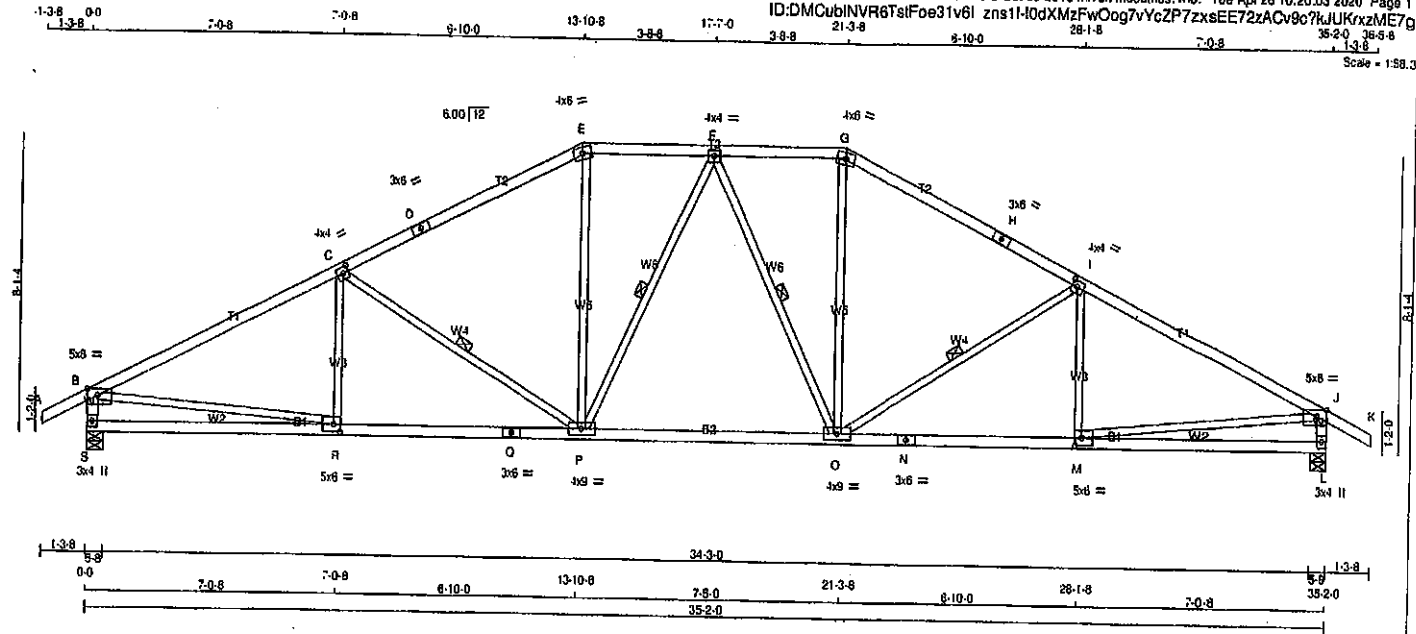
JSI GRIP = 0.89 (Q) (INPUT = 0.90)
JSI METAL = 0.89 (M) (INPUT = 1.00)



Structural component only
DWG# T-2007664

JOB NAME 408224	TRUSS NAME T44 ✓	QUANTITY 2 ✓	PLY 1 ✓	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mtek Industries, Inc. Tue Apr 28 10:20:03 2020 Page 1
 ID:DMCubINVR6TstF0e31v6l zns1f-10dXMzFwOog7vYcZP7zxsEE72zACv9c7%JUKrxzME7Q
 35-2-0 35-8-8 13-8-8
 Scale = 1:50.0



LUMBER

N. L. Q. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TS-1	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	6.0		
F	TMVW-i	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	6.0		
H	TS-1	MT20	3.0	6.0		
I	TMVW-i	MT20	4.0	4.0	2.00	1.75
J	TMVW-p	MT20	5.0	8.0	Edge	3.50
L	BMV1-p	MT20	3.0	4.0		
M	BMVW-i	MT20	5.0	6.0	2.50	2.25
N	BS-1	MT20	3.0	6.0		
O	BMVW-w	MT20	4.0	9.0		
P	BMVW-w	MT20	4.0	9.0		
Q	BS-1	MT20	3.0	6.0		
R	BMVW-i	MT20	5.0	6.0	2.50	2.25
S	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
	VERT	HORZ	DOWN	UP
S	2083	0	2083	0
L	2083	0	2083	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1457	989	0	0	0	488	0
L	1457	989	0	0	0	488	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-P, F-D, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		MAX. UNBRACED LENGTH (FT)	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	
		FROM	TO			FROM	TO
A-B	0	-91.8	-91.8	0.12 (1)	10.00	R-C	-182
B-C	-2906	0	-91.8	-91.8	0.83 (1)	3.25	C-P
C-D	-2373	0	-91.8	-91.8	0.72 (1)	3.68	P-E
D-E	-2373	0	-91.8	-91.8	0.72 (1)	3.68	P-F
E-F	-2100	0	-91.8	-91.8	0.19 (1)	4.53	F-O
F-G	-2100	0	-91.8	-91.8	0.19 (1)	4.53	O-G
G-H	-2373	0	-91.8	-91.8	0.72 (1)	3.68	O-I
H-I	-2373	0	-91.8	-91.8	0.72 (1)	3.68	M-I
I-J	-2906	0	-91.8	-91.8	0.83 (1)	3.25	B-R
J-K	0	29	-91.8	-91.8	0.12 (1)	10.00	M-J
S-B	-2008	0	0.0	0.0	0.20 (1)	5.96	
L-J	-2008	0	0.0	0.0	0.20 (1)	5.96	
S-R	0	0	-18.5	-18.5	0.20 (4)	10.00	
R-Q	0	2630	-18.5	-18.5	0.52 (1)	10.00	
Q-P	0	2630	-18.5	-18.5	0.52 (1)	10.00	
P-O	0	2193	-18.5	-18.5	0.45 (1)	10.00	
O-N	0	2630	-18.5	-18.5	0.52 (1)	10.00	
N-M	0	2630	-18.5	-18.5	0.52 (1)	10.00	
M-L	0	0	-18.5	-18.5	0.20 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.0012

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. O.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(LL) = L/989 (0.14")
 ALLOWABLE DEFL.(TL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.82/1.00 (I-J:1), BC=0.52/1.00 (M-C:1), WS=0.60/1.00 (J-M:1), SS=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

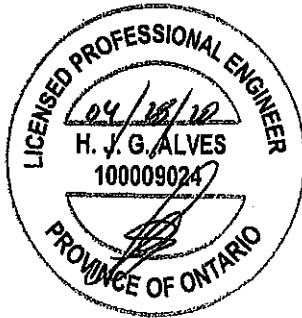
MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 788 1987 1858

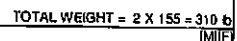
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (M) (INPUT = 0.90)
 JSI METAL = 0.79 (Q) (INPUT = 1.00)



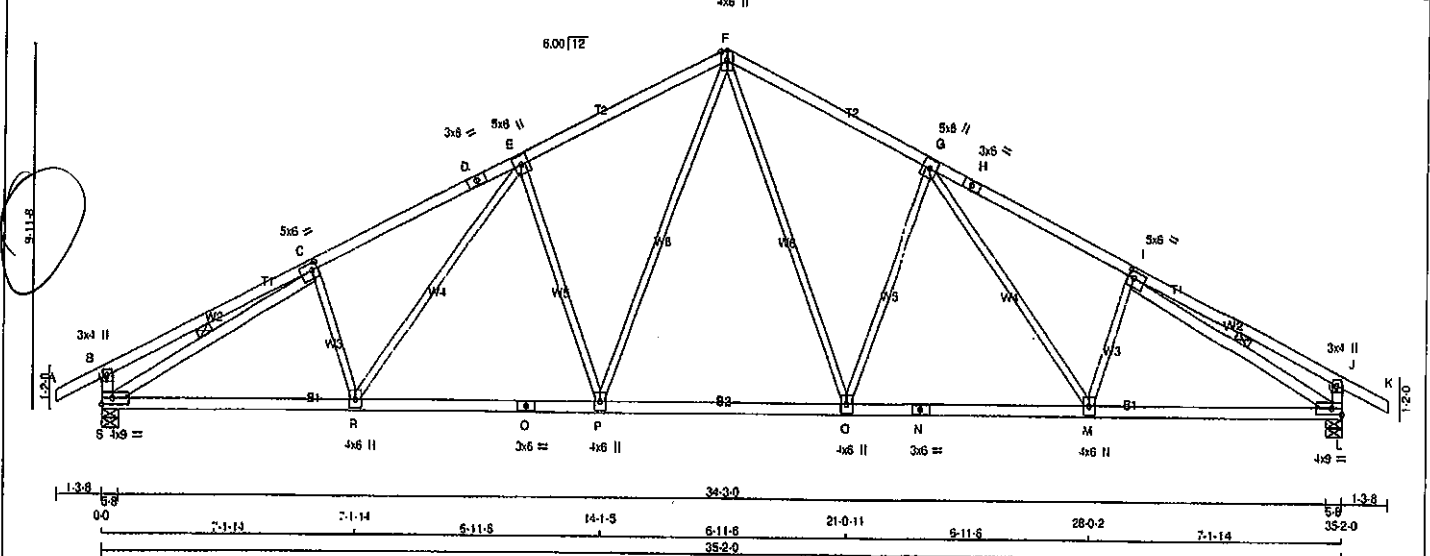
Structural component only
 DWG# T-2007665



JSI GRIP = 0.89 (F) (INPUT = 0.90)
JSI METAL = 0.83 (O) (INPUT = 1.00)



JOB NAME 408224	TRUSS NAME T46	QUANTITY 8	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 MTak Industries, Inc. Tue Apr 28 10:20:05 2020 Page 1	ID:DMCubINVR6TslF0e31v8I_zns11-EOlnH-BW9smY7QxJJYonsaN17IBczRwqzME7e



LUMBER			
N. L. Q. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
S - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	5.0	6.0	2.25 2.00
D	TS-t	MT20	3.0	6.0	
E	TMWW-t	MT20	5.0	6.0	
F	TTWW+p	MT20	4.0	6.0	Edge
G	TMWW-t	MT20	5.0	6.0	
H	TS-t	MT20	3.0	6.0	
I	TMWW-t	MT20	5.0	6.0	2.25 2.00
J	TMV+p	MT20	3.0	4.0	
L	BMVW-t	MT20	4.0	9.0	Edge
M, O, P, R					
M	BMWW-t	MT20	4.0	6.0	
N	BS-t	MT20	3.0	6.0	
Q	BS-t	MT20	3.0	6.0	
S	BMVW-t	MT20	4.0	9.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
S	2063	0	0	2063	0	0	5-8	5-8	5-8
L	2063	0	0	2063	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST CASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
S	1457	999	0	0	0
L	1457	999	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-S, I-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 28	-91.8 -91.8	0.12 (1)	10.00	F-O	0 866	0.19 (1)
B-C	0 22	-91.8 -91.8	0.40 (1)	10.00	O-G	-726 0	0.72 (1)
C-D	-2808 0	-91.8 -91.8	0.49 (1)	3.75	G-M	0 391	0.09 (1)
D-E	-2808 0	-91.8 -91.8	0.49 (1)	3.75	M-I	-192 18	0.05 (1)
E-F	-2322 0	-91.8 -91.8	0.46 (1)	4.07	P-F	0 888	0.19 (1)
F-G	-2322 0	-91.8 -91.8	0.46 (1)	4.07	E-P	-726 0	0.72 (1)
G-H	-2808 0	-91.8 -91.8	0.49 (1)	3.75	R-E	0 391	0.09 (1)
H-I	-2808 0	-91.8 -91.8	0.49 (1)	3.75	C-R	-192 18	0.05 (1)
I-J	0 22	-91.8 -91.8	0.40 (1)	10.00	S-C	-3064 0	0.72 (1)
J-K	0 28	-91.8 -91.8	0.12 (1)	10.00	I-L	-3064 0	0.72 (1)
S-B	-344 0	0.0 0.0	0.03 (1)	7.81			
L-J	-344 0	0.0 0.0	0.03 (1)	7.81			
S-R	0 2574	-18.5 -18.5	0.53 (1)	10.00			
R-Q	0 2291	-18.5 -18.5	0.47 (1)	10.00			
Q-P	0 2291	-18.5 -18.5	0.47 (1)	10.00			
P-O	0 1762	-18.5 -18.5	0.38 (1)	10.00			
O-N	0 2291	-18.5 -18.5	0.47 (1)	10.00			
N-M	0 2291	-18.5 -18.5	0.47 (1)	10.00			
M-L	0 2574	-18.5 -18.5	0.53 (1)	10.00			

TOTAL WEIGHT = 8 X 152 = 1213 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/999 (0.14")
ALLOWABLE DEFL. (TL) = L/380 (1.17")
CALCULATED VERT. DEFL. (TL) = L/999 (0.28")

CSI: TC=0.49/1.00 (C-E:1), BC=0.53/1.00 (L-M:1), WB=0.72/1.00 (E-P:1), SS=0.22/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

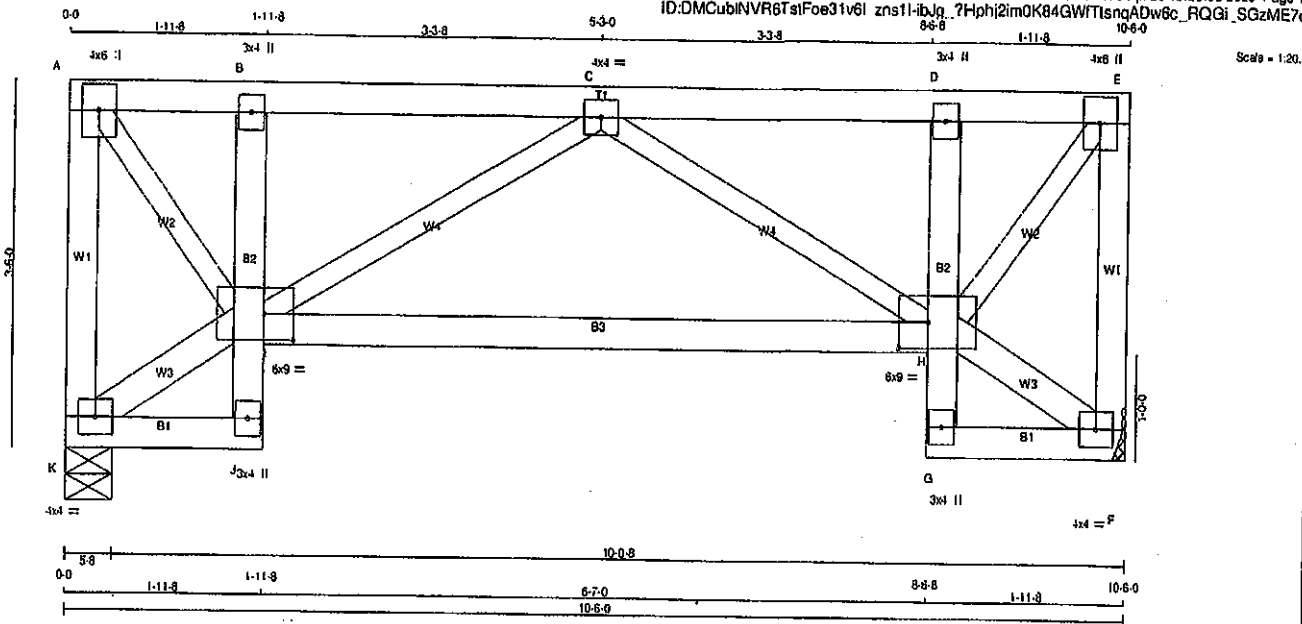
JSI GRIP = 0.87 (C) INPUT = 0.90
JSI METAL = 0.77 (I) INPUT = 1.00



Structural component only
DWG# T-2007667

JOB NAME 408224	TRUSS NAME T47S	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:DMCubINVR6TstFoe31v6l zns11-ibJg_7Hph2im0K84GWTTsqADw6c_RQGj_SGzME7d



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
K - A	2x4	DRY	No.2
A - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
K - J	2x4	DRY	No.2
J - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
G - D	2x4	DRY	No.2
G - F	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
K - I	2x4	DRY	No.2
H - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0		
B	TMV+p	MT20	3.0	4.0		
C	TMVW-I	MT20	4.0	4.0		
D	TMV+p	MT20	3.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMVW-I	MT20	4.0	4.0		
G	BMV+p	MT20	3.0	4.0		
H	BMVW-W-I	MT20	6.0	9.0	3.00	3.50
I	BMVW-W-I	MT20	6.0	9.0	3.00	3.50
J	BMV+p	MT20	3.0	4.0		
K	BMVW-I	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
K	697	0	697	0	5-8	5-8
F	697	0	697	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1.8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	504	269	0	0	0	235	0
F	504	269	0	0	0	235	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH FR-TO (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO (LBS)	MEMB.
K-A	-667	0	0.0	0.0	0.17	(1)	7.81
A-B	-497	0	-114.3	-114.3	0.14	(1)	6.25
B-C	-508	0	-114.3	-114.3	0.22	(1)	6.25
C-D	-508	0	-114.3	-114.3	0.22	(1)	6.25
D-E	-497	0	-114.3	-114.3	0.14	(1)	6.25
E-F	-667	0	0.0	0.0	0.17	(1)	7.81
K-J	0	25	-18.5	-18.5	0.03	(4)	10.00
J-I	0	19	0.0	0.0	0.05	(1)	10.00
I-B	-331	0	0.0	0.0	0.05	(1)	7.81
I-H	0	832	-18.5	-18.5	0.46	(4)	10.00
G-H	0	19	0.0	0.0	0.05	(1)	10.00
H-D	-331	0	0.0	0.0	0.05	(1)	7.81
G-F	0	25	-18.5	-18.5	0.03	(4)	10.00

TOTAL WEIGHT = 2 X 51 = 102 lb (M/R)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 15.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 48.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.22/1.00 (B-C:1), BC=0.46/1.00 (H-I:4), WB=0.24/1.00 (E-H:1), SS=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MIN	MAX	MIN
MT20	618	354
	1687	788
	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

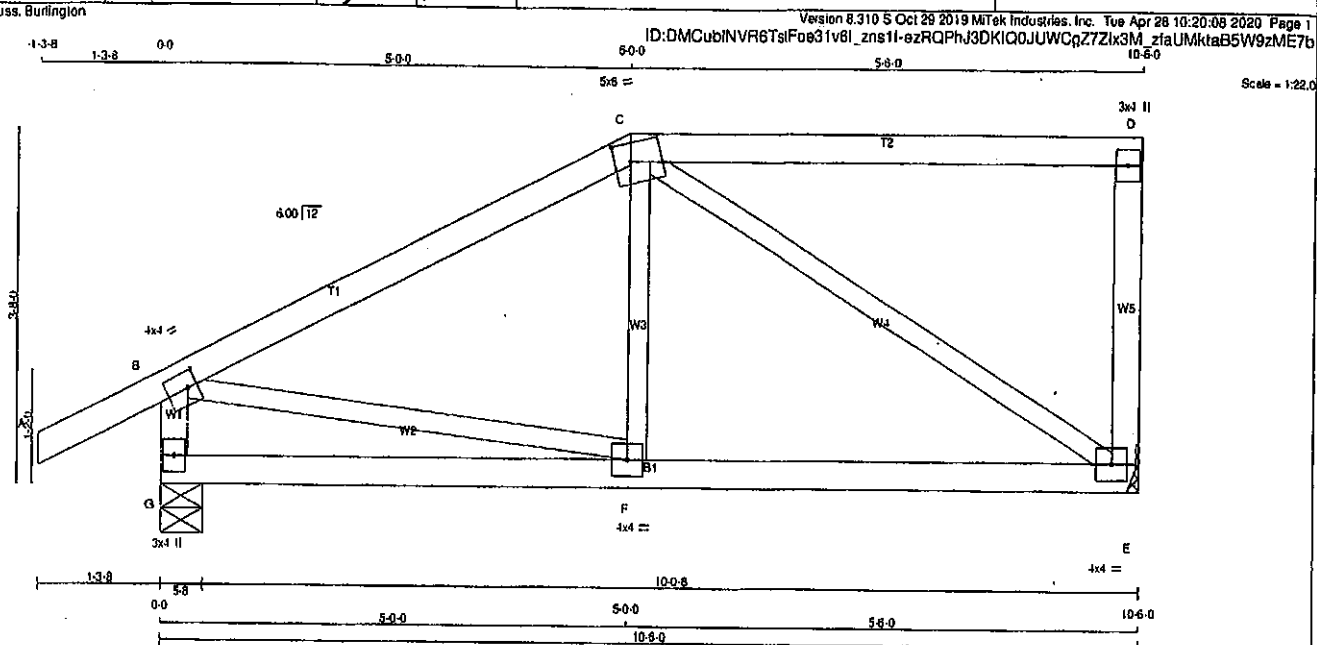
JSI GRIP = 0.77 (H) INPUT = 0.90
JSI METAL = 0.30 (E) INPUT = 1.00



Structural component only
DWG# T-2007688

Structural component only
DWG# T-2007669

JOB NAME 408224	TRUSS NAME T49	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Trussack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY, SEASONED LUMBER.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	4.0	2.00 1.25
C	TTWW-m	MT20	5.0	6.0	2.25 2.00
D	TMV-p	MT20	3.0	4.0	
E	BMVW-1	MT20	4.0	4.0	
F	BMVW-1	MT20	4.0	4.0	
G	BMV-1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
E	579	0	0	579	0	0	0	MECHANICAL	IN-SX
G	703	0	0	703	0	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	COMBINED	SNOW	LIVE	PERM. LIVE
E	409	269	0	0	0	0	0	141	0	0	0
G	495	338	0	0	0	0	0	157	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	LC2 MAX	UNBRAC	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	LC2 MAX	UNBRAC
FR-TO					LENGTH	FR-TO					LENGTH
A-B	0 28	-91.8	-91.8	0.12 (1)	10.00	F-C	0 106	0.04 (4)			
B-C	-523 0	-91.8	-91.8	0.41 (1)	6.25	C-E	-549 0	0.37 (1)			
C-D	0 0	-91.8	-91.8	0.47 (1)	10.00	B-F	0 474	0.11 (1)			
E-D	-252 0	0.0	0.0	0.05 (1)	7.81						
G-B	-667 0	0.0	0.0	0.07 (1)	7.81						
G-F	0 0	-18.5	-18.5	0.14 (4)	10.00						
F-E	0 488	-18.5	-18.5	0.19 (4)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, OBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.47 1.00 (C-D:1), BC=0.19/1.00 (E-F:4), WB=0.37 1.00 (C-E:1), SS=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.28 (B) (INPUT = 1.00)

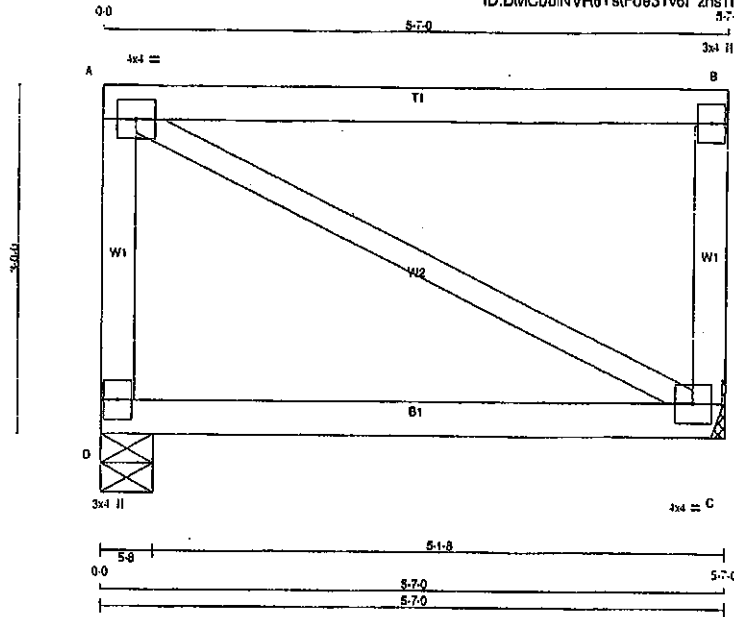


Structural component only
DWG# T-2007670

JOB NAME 408224	TRUSS NAME T50	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:18.4

LUMBER		
N. L. G. A. RULES		
CHORDS	SIZE	LUMBER
D - A	2x4	DRY No.2
A - B	2x4	DRY No.2
C - B	2x4	DRY No.2
D - C	2x4	DRY No.2
ALL WEBS 2x3 DRY No.2		
DRY: SEASONED LUMBER.		

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
D	371	0	371	0	5-8	5-8
C	371	0	371	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-1	MT20	4.0	4.0	
B	TMV-p	MT20	3.0	4.0	
C	BMVW1-1	MT20	4.0	4.0	
D	BMV1-p	MT20	3.0	4.0	

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
D	268	143.0	0.0	0.0	0.0	125.0	0.0
C	268	143.0	0.0	0.0	0.0	125.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	UNBRAC LENGTH	FR-TO			
D-A	-319.0	0.0 0.0 0.06 (1)	7.81	A-C	0 0	0.00 (1)	
A-B	0 0	-114.3 -114.3 0.81 (1)	10.00				
C-B	-319.0	0.0 0.0 0.06 (1)	7.81				
D-C	0 0	-18.5 -18.5 0.23 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 15.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 48.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.81 1.00 (A-B:1), BC=0.23/1.00 (C-D:4),
WB=0.00/1.00 (A-C:1), SSI=0.33/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1887
		788	1987
			1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.25 (A) (INPUT = 0.80)

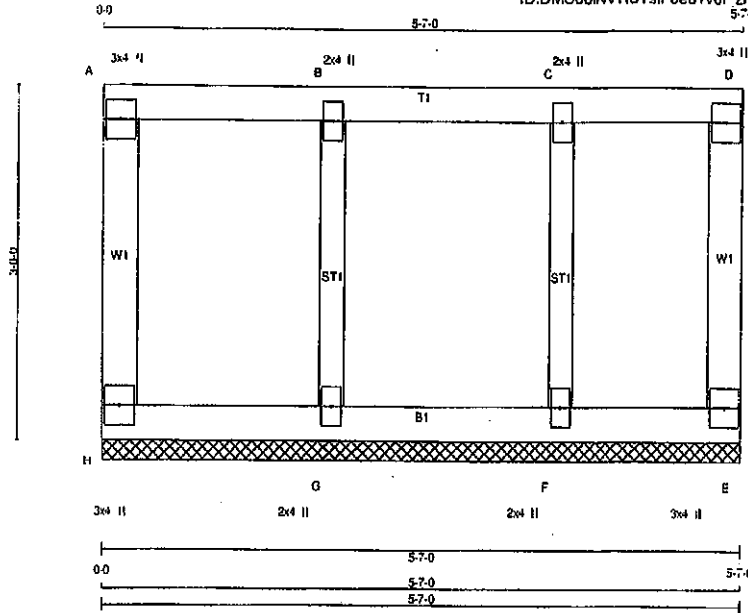
JSI METAL = 0.07 (A) (INPUT = 1.00)



Structural component only
DWG# T-2007671

JOB NAME 408224	TRUSS NAME G50	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	PRWG NO.
Tamarack Roof Truss, Building				TRUSS DESC.	

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Scale = 1/16"

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
H - A	2x4	DRY	No.2
A - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
H - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" O.C.

PLATES (inches in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMV+p	MT20	3.0	4.0
B	TMW+w	MT20	2.0	4.0
C	TMW+w	MT20	2.0	4.0
D	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0
F	BMW1+w	MT20	2.0	4.0
G	BMW1+w	MT20	2.0	4.0
H	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	UNBRACED LENGTH
FR-TO		FROM TO		FR-TO			
H-A	-101 0	0.0 0.0 0.03 (1)	7.81	G-B	-248 0	0.08 (1)	
A-B	-8 0	-114.3 -114.3 0.08 (1)	10.00	F-C	-209 0	0.05 (1)	
B-C	-8 0	-114.3 -114.3 0.08 (1)	10.00				
C-D	-8 0	-114.3 -114.3 0.08 (1)	10.00				
E-D	-80 0	0.0 0.0 0.03 (1)	7.81				
H-G	0 8	-18.5 -18.5 0.02 (4)	10.00				
G-F	0 8	-18.5 -18.5 0.02 (4)	10.00				
F-E	0 8	-18.5 -18.5 0.02 (4)	10.00				

TOTAL WEIGHT = 22 lb (M/R)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 15.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 48.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019

- PART 9 OF CBC 2012 (2019 AMENDMENT)

- CSA 086-09, CSA 086-14

- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (A-B:1), BC=0.02/1.00 (G-H:4), WB=0.08/1.00 (B-G:1), SS=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.17 (G) (INPUT = 0.90)

JSI METAL = 0.07 (G) (INPUT = 1.00)

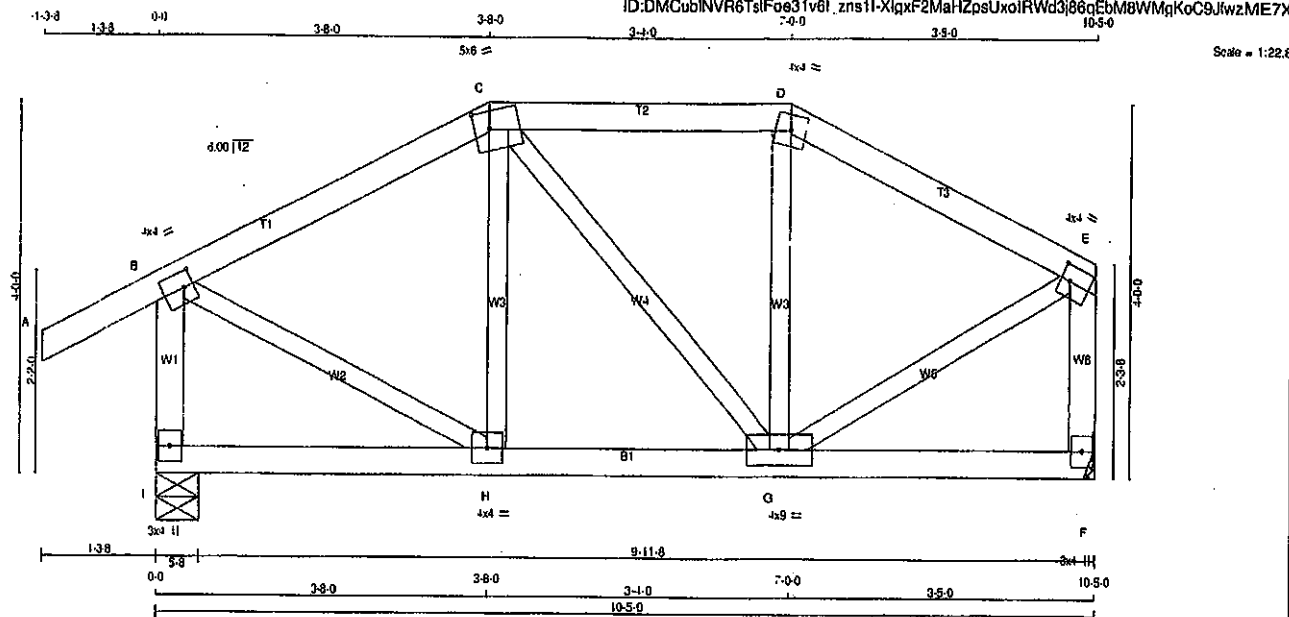


Structural component only
DWG# T-2007656

JOB NAME 408224	TRUSS NAME T52	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
TRUSS DESC.					

Lamarack Roof Truss, Burlington

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Scale = 1:22.8

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
I - J	2x4	DRY	No.2
J - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - N	2x4	DRY	No.2
N - O	2x4	DRY	No.2
O - P	2x4	DRY	No.2
P - Q	2x4	DRY	No.2
Q - R	2x4	DRY	No.2
R - S	2x4	DRY	No.2
S - T	2x4	DRY	No.2
T - U	2x4	DRY	No.2
U - V	2x4	DRY	No.2
V - W	2x4	DRY	No.2
W - X	2x4	DRY	No.2
X - Y	2x4	DRY	No.2
Y - Z	2x4	DRY	No.2
Z - AA	2x4	DRY	No.2
AA - AB	2x4	DRY	No.2
AB - AC	2x4	DRY	No.2
AC - AD	2x4	DRY	No.2
AD - AE	2x4	DRY	No.2
AE - AF	2x4	DRY	No.2
AF - AG	2x4	DRY	No.2
AG - AH	2x4	DRY	No.2
AH - AI	2x4	DRY	No.2
AI - AJ	2x4	DRY	No.2
AJ - AK	2x4	DRY	No.2
AK - AL	2x4	DRY	No.2
AL - AM	2x4	DRY	No.2
AM - AN	2x4	DRY	No.2
AN - AO	2x4	DRY	No.2
AO - AP	2x4	DRY	No.2
AP - AQ	2x4	DRY	No.2
AQ - AR	2x4	DRY	No.2
AR - AS	2x4	DRY	No.2
AS - AT	2x4	DRY	No.2
AT - AU	2x4	DRY	No.2
AU - AV	2x4	DRY	No.2
AV - AW	2x4	DRY	No.2
AW - AX	2x4	DRY	No.2
AX - AY	2x4	DRY	No.2
AY - AZ	2x4	DRY	No.2
AZ - BA	2x4	DRY	No.2
BA - BB	2x4	DRY	No.2
BB - BC	2x4	DRY	No.2
BC - BD	2x4	DRY	No.2
BD - BE	2x4	DRY	No.2
BE - BF	2x4	DRY	No.2
BF - BG	2x4	DRY	No.2
BG - BH	2x4	DRY	No.2
BH - BI	2x4	DRY	No.2
BI - BJ	2x4	DRY	No.2
BJ - BK	2x4	DRY	No.2
BK - BL	2x4	DRY	No.2
BL - BM	2x4	DRY	No.2
BM - BN	2x4	DRY	No.2
BN - BO	2x4	DRY	No.2
BO - BP	2x4	DRY	No.2
BP - BQ	2x4	DRY	No.2
BQ - BR	2x4	DRY	No.2
BR - BS	2x4	DRY	No.2
BS - BT	2x4	DRY	No.2
BT - BU	2x4	DRY	No.2
BU - BV	2x4	DRY	No.2
BV - BW	2x4	DRY	No.2
BW - BX	2x4	DRY	No.2
BX - BY	2x4	DRY	No.2
BY - BZ	2x4	DRY	No.2
BZ - CA	2x4	DRY	No.2
CA - CB	2x4	DRY	No.2
CB - CC	2x4	DRY	No.2
CC - CD	2x4	DRY	No.2
CD - CE	2x4	DRY	No.2
CE - CF	2x4	DRY	No.2
CF - CG	2x4	DRY	No.2
CG - CH	2x4	DRY	No.2
CH - CI	2x4	DRY	No.2
CI - CJ	2x4	DRY	No.2
CJ - CK	2x4	DRY	No.2
CK - CL	2x4	DRY	No.2
CL - CM	2x4	DRY	No.2
CM - CN	2x4	DRY	No.2
CN - CO	2x4	DRY	No.2
CO - CP	2x4	DRY	No.2
CP - CQ	2x4	DRY	No.2
CQ - CR	2x4	DRY	No.2
CR - CS	2x4	DRY	No.2
CS - CT	2x4	DRY	No.2
CT - CU	2x4	DRY	No.2
CU - CV	2x4	DRY	No.2
CV - CW	2x4	DRY	No.2
CW - CX	2x4	DRY	No.2
CX - CY	2x4	DRY	No.2
CY - CZ	2x4	DRY	No.2
CZ - DA	2x4	DRY	No.2
DA - DB	2x4	DRY	No.2
DB - DC	2x4	DRY	No.2
DC - DD	2x4	DRY	No.2
DD - DE	2x4	DRY	No.2
DE - DF	2x4	DRY	No.2
DF - DG	2x4	DRY	No.2
DG - DH	2x4	DRY	No.2
DH - DI	2x4	DRY	No.2
DI - DJ	2x4	DRY	No.2
DJ - DK	2x4	DRY	No.2
DK - DL	2x4	DRY	No.2
DL - DM	2x4	DRY	No.2
DM - DN	2x4	DRY	No.2
DN - DO	2x4	DRY	No.2
DO - DP	2x4	DRY	No.2
DP - DQ	2x4	DRY	No.2
DQ - DR	2x4	DRY	No.2
DR - DS	2x4	DRY	No.2
DS - DT	2x4	DRY	No.2
DT - DU	2x4	DRY	No.2
DU - DV	2x4	DRY	No.2
DV - DW	2x4	DRY	No.2
DW - DX	2x4	DRY	No.2
DX - DY	2x4	DRY	No.2
DY - DZ	2x4	DRY	No.2
DZ - EA	2x4	DRY	No.2
EA - EB	2x4	DRY	No.2
EB - EC	2x4	DRY	No.2
EC - ED	2x4	DRY	No.2
ED - EE	2x4	DRY	No.2
EE - EF	2x4	DRY	No.2
EF - EG	2x4	DRY	No.2
EG - EH	2x4	DRY	No.2
EH - EI	2x4	DRY	No.2
EI - EJ	2x4	DRY	No.2
EJ - EK	2x4	DRY	No.2
EK - EL	2x4	DRY	No.2
EL - EM	2x4	DRY	No.2
EM - EN	2x4	DRY	No.2
EN - EO	2x4	DRY	No.2
EO - EP	2x4	DRY	No.2
EP - EQ	2x4	DRY	No.2
EQ - ER	2x4	DRY	No.2
ER - ES	2x4	DRY	No.2
ES - ET	2x4	DRY	No.2
ET - EU	2x4	DRY	No.2
EU - EV	2x4	DRY	No.2
EV - EW	2x4	DRY	No.2
EW - EX	2x4	DRY	No.2
EX - EY	2x4	DRY	No.2
EY - EZ	2x4	DRY	No.2
EZ - FA	2x4	DRY	No.2
FA - FB	2x4	DRY	No.2
FB - FC	2x4	DRY	No.2
FC - FD	2x4	DRY	No.2
FD - FE	2x4	DRY	No.2
FE - FF	2x4	DRY	No.2
FF - FG	2x4	DRY	No.2
FG - FH	2x4	DRY	No.2
FH - FI	2x4	DRY	No.2
FI - FJ	2x4	DRY	No.2
FJ - FK	2x4	DRY	No.2
FK - FL	2x4	DRY	No.2
FL - FM	2x4	DRY	No.2
FM - FN	2x4	DRY	No.2
FN - FO	2x4	DRY	No.2
FO - FP	2x4	DRY	No.2
FP - FQ	2x4	DRY	No.2
FQ - FR	2x4	DRY	No.2
FR - FS	2x4	DRY	No.2
FS - FT	2x4	DRY	No.2
FT - FU	2x4	DRY	No.2
FU - FV	2x4	DRY	No.2
FV - FW	2x4	DRY	No.2
FW - FX	2x4	DRY	No.2
FX - FY	2x4	DRY	No.2
FY - FZ	2x4	DRY	No.2
FZ - GA	2x4	DRY	No.2
GA - GB	2x4	DRY	No.2
GB - GC	2x4	DRY	No.2
GC - GD	2x4	DRY	No.2
GD - GE	2x4	DRY	No.2
GE - GF	2x4	DRY	No.2
GF - GG	2x4	DRY	No.2
GG - GH	2x4	DRY	No.2
GH - GI	2x4	DRY	No.2
GI - GJ	2x4	DRY	No.2
GJ - GK	2x4	DRY	No.2
GK - GL	2x4	DRY	No.2
GL - GM	2x4	DRY	No.2
GM - GN	2x4	DRY	No.2
GN - GO	2x4	DRY	No.2
GO - GP	2x4	DRY	No.2
GP - GQ	2x4	DRY	No.2
GQ - GR	2x4	DRY	No.2
GR - GS	2x4	DRY	No.2
GS - GT	2x4	DRY	No.2
GT - GU	2x4	DRY	No.2
GU - GV	2x4	DRY	No.2
GV - GW	2x4	DRY	No.2
GW - GX	2x4	DRY	No.2
GX - GY	2x4	DRY	No.2
GY - GZ	2x4	DRY	No.2
GZ - HA	2x4	DRY	No.2
HA - HB	2x4	DRY	No.2
HB - HC	2x4	DRY	No.2
HC - HD	2x4	DRY	No.2
HD - HE	2x4	DRY	No.2
HE - HF	2x4	DRY	No.2
HF - HG	2x4	DRY	No.2
HG - HH	2x4	DRY	No.2
HH - HI	2x4	DRY	No.2
HI - HJ	2x4	DRY	No.2
HJ - HK	2x4	DRY	No.2
HK - HL	2x4	DRY	No.2
HL - HM	2x4	DRY	No.2
HM - HN	2x4	DRY	No.2
HN - HO	2x4	DRY	No.2
HO - HP	2x4	DRY	No.2
HP - HQ	2x4	DRY	No.2
HQ - HR	2x4	DRY	No.2
HR - HS	2x4	DRY	No.2
HS - HT	2x4	DRY	No.2
HT - HU	2x4	DRY	No.2
HU - HV	2x4	DRY	No.2
HV - HW	2x4	DRY	No.2
HW - HX	2x4	DRY	No.2
HX - HY	2x4	DRY	No.2
HY - HZ	2x4	DRY	No.2
HZ - IA	2x4	DRY	No.2
IA - IB	2x4	DRY	No.2
IB - IC	2x4	DRY	No.2
IC - ID	2x4	DRY	No.2
ID - IE	2x4	DRY	No.2
IE - IF	2x4	DRY	No.2
IF - IG	2x4	DRY	No.2
IG - IH	2x4	DRY	No.2
IH - II	2x4	DRY	No.2
II - IJ	2x4	DRY	No.2
IJ - IK	2x4	DRY	No.2
IK - IL	2x4	DRY	No.2
IL - IM	2x4	DRY	No.2
IM - IN	2x4	DRY	No.2
IN - IO	2x4	DRY	No.2
IO - IP	2x4	DRY	No.2
IP - IQ	2x4	DRY	No.2
IQ - IR	2x4	DRY	No.2
IR - IS	2x4	DRY	No.2
IS - IT	2x4	DRY	No.2
IT - IU	2x4	DRY	No.2
IU - IV	2x4	DRY	No.2
IV - IV	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-1	MT20	4.0	4.0	2.00	1.25
C	TTW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMW-1	MT20	4.0	4.0	2.00	1.25
F	BMV1-p	MT20	3.0	4.0		
G	BMWW-1	MT20	4.0	9.0		
H	BMWW-1	MT20	4.0	4.0		
I	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
I	699 0	699 0	5-8	5-8
F	574 0	574 0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	DEAD	SOIL
I	492	338 0	0 0	158 0
F	406	267 0	0 0	140 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

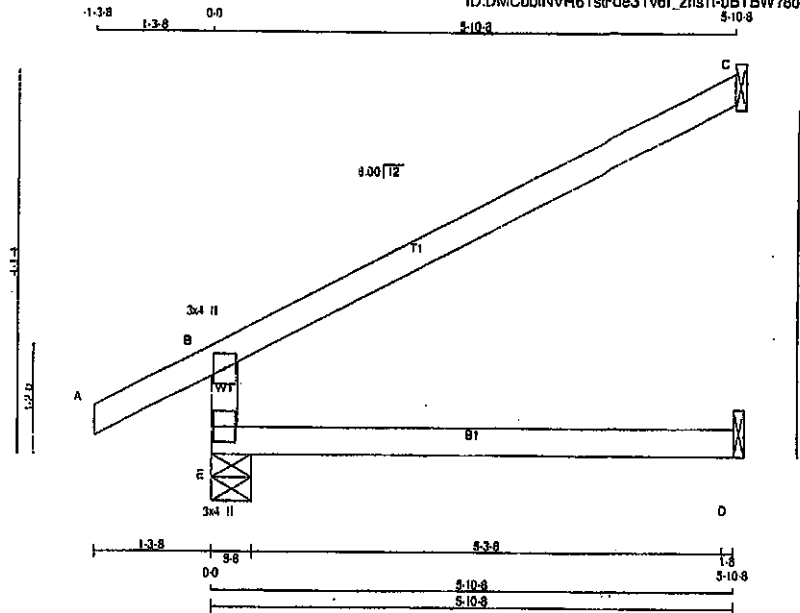
CHORDS				WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	0 28	-91.8	-91.8 0.12 (1)	10.00	H-C	-118 14	0.03 (1)		
B-C	-423 0	-91.8	-91.8 0.16 (1)	6.25	C-G	-20 0	0.01 (1)		
C-D	-362 0	-91.8	-91.8 0.13 (1)	6.25	G-D	-126 8	0.03 (1)		
D-E	-409 0	-91.8	-91.8 0.14 (1)	6.25	B-H	0 422	0.09 (1)		
I-B	-669 0	0.0	0.0 0.08 (1)	7.81	G-E	0 -119	0.09 (1)		
F-E	-848 0	0.0	0.0 0.07 (1)	7.81					
I-H	0 0	-16.5	-16.5 0.05 (4)	10.00					
H-G	0 375	-16.5	-16.5 0.09 (1)	10.00					
G-F	0 0	-16.5	-16.5 0.05 (4)	10.00					

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408222	U1	21	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

ID:DMCubiNVR6TsFoe31v6I_zns1f-uBTBW78oHNgUpbV5dpsEbLZQFMuoV7BWCFEHzMFii

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Scale = 1/2" = 1'-0"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2		
A - C	2x4	DRY	No.2		
E - D	2x4	DRY	No.2		

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
JT						
E	525	0	525	0	5-8	5-8
C	202	0	202	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS				DEAD	SOIL
		COMBINED	SNOW	LIVE	PERM. LIVE		
E	389	257	0	0	0	111	0
C	139	113	0	0	0	28	0
D	36	0	0	0	0	36	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO						
E-B	-461	0	0.0	0.0	0.13 (4)	7.81
A-B	0	28	-91.8	-91.8	0.12 (1)	10.00
B-C	-30	0	-91.8	-91.8	0.54 (1)	6.25
E-D	0	0	-18.5	-18.5	0.13 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CS: TC=0.54 (1.00 (B-C:1), BC=0.13 (1.00 (D-E:4), WB=0.00 (1.00 (rva:0), SS=0.24 (1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION	(PS)	(PL)	(PL)
MAX MIN MAX MIN MAX MIN			
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.19 (E), INPUT = 0.90
JSI METAL = 0.13 (B), INPUT = 1.00



Structural component only
DWG# T-2007598

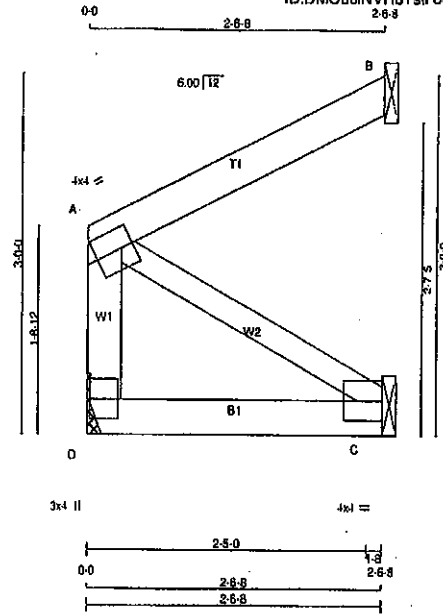
Structural component only
DWG# T-2007657

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	J41	5	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Tamarack Roof Truss, Burlington

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ID:DMCubINV6TstFoe31v6l zns11-3Hb7Tu8HW1XPKAQqNkQ.LMmiEmlEhgtPoM1zzME7p

Scale = 1:17.8



LUMBER				DESCR.
N. L. G. A. RULES	SIZE	LUMBER		
CHORDS				
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (Table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMWV-1	MT20	4.0	4.0	2.00 1.25
C	BMV1-1	MT20	4.0	4.0	2.00 Edge
D	BMV1-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	COMBINED	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
D	139	0	139	0	0	MECHANICAL			
B	117	0	117	0	0	1-8	1-8		
C	23	0	25	0	0	1-8	1-8		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	COMBINED	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	98	65	0	0.0	0.0	0.0	33	0.0
B	80	65	0	0.0	0.0	0.0	15	0.0
C	18	0	0	0.0	0.0	0.0	18	0.0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
D-A	-117 0	0.0 0.0	0.01 (1)	A-C	0 0	0.00 (1)	
A-B	0 0	-91.8 -91.8	0.10 (1)				
D-C	0 0	-18.5 -18.5	0.03 (4)				

TOTAL WEIGHT = 5 X 10 = 49 lb (MIF)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CS1: TC=0.10 (1.00 (A-B:1), BC=0.03 (1.00 (C-D:4), WB=0.00 (1.00 (A-C:1), SS1=0.08 (1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1867 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.07 (A) (INPUT = 0.90)
JSI METAL = 0.02 (A) (INPUT = 1.00)

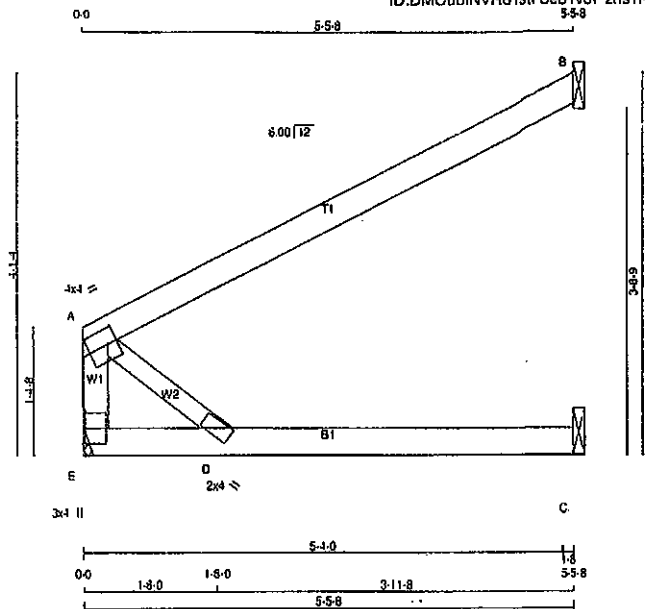


Structural component only
DWG# T-2007658

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	J42 ✓	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 10:19:35 2020 Page 1
ID:DMCubINVR6TstFoe31v61 zns1I-XT8WgE9vHKIGyK71xSq3XYumIX 1hxqu3YvZPzME7c



Scale = 1:22.9

TOTAL WEIGHT = 2 X 16 = 31 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
E - C	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	BRG	UPLIFT	BRG	IN-SX
E	301	0	0	301	0	0	0	0	1-8
B	250	0	0	250	0	0	0	0	1-8
C	50	0	0	57	0	0	0	0	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B - C

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	213	140.0	0.0	0.0	0.0	0.0	73.0	0.0
B	172	140.0	0.0	0.0	0.0	0.0	33.0	0.0
C	40	0.0	0.0	0.0	0.0	0.0	40.0	0.0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (LC)	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
E-A	-250.0	0.0	0.0 0.03 (1)	A-D	0.0	0.0 0.00 (1)	
A-B	0.0	-91.8	-91.8 0.46 (1)				
E-D	0.0	-18.5	-18.5 0.13 (4)				
D-C	0.0	-18.5	-18.5 0.16 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.46/1.00 (A-B:1), BC=0.16/1.00 (C-D:4), WB=0.00/1.00 (A-D:1), SSI=0.17/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1650

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.15 (A) INPUT = 0.90
JSI METAL = 0.04 (A) INPUT = 1.00

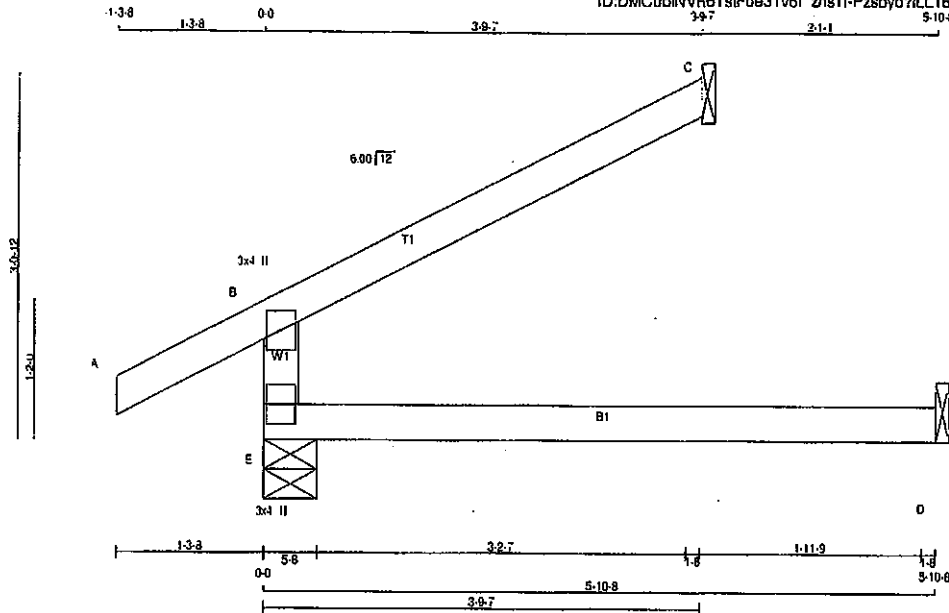


Structural component only
DWG# T-2007659

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408224	C40	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TslFoe31v6l zns11-Pzsbvo7LL16uKVXgD50VpshpV5QUojvXujdfzME8?



Scale = 1:12.5

TOTAL WEIGHT = 2 X 14 = 28 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	COMBINED	GROSS REACTION	GROSS REACTION	DOWN	HORIZ	UP/LIFT	IN-SX	IN-SX	BRG
E	405	0	405	0	0	0	5-8	5-8	
C	130	0	130	0	0	0	1-8	1-8	
D	45	0	50	0	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	288	180	0	0	0	98	0
C	90	73	0	0	0	17	0
D	36	0	0	0	0	36	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	MAX. MEMB. FORCE (LBS)	UNBRAC LENGTH	FR-TO	MAX. MEMB. FORCE (LBS)
FR-TO		FROM	TO				
E-B	-342	0	0.0	0.0	0.13 (4)	7.81	
A-B	0	28	-91.8	-91.8	0.12 (1)	10.00	
B-C	-19	0	-91.8	-91.8	0.22 (1)	6.25	
E-D	0	0	-18.5	-18.5	0.13 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH. LL	= 25.8 PSF
DL	= 8.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/380 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.22/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (m/a:0), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	818 354	1667 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E1) INPUT = 0.90
JSI METAL= 0.09 (E1) INPUT = 1.00



Structural component only
DWG# T-2007648



TOTAL WEIGHT = $2 \times 12 = 23$ lb

DRY: SEASONED LUMBER.

6	14V4p
5	84V1+p

BUILDING DESIGNERS BEARINGS

C	63
D	44

UNFACTORED REACTIONS

C	48
D	35

BRACING

BRACING

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING

TOTAL LOAD CASES: (7)

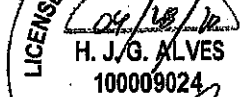
MAX. F.

FACTORED CONCENTRATED LOADS (LBS)

F 1-11

CONNECTION REQUIREMENTS

11



Structural component only
DWG# T-2007649

DESIGN CRITERIA

SPECIFIED LOADS-

TOTAL 1

SPACING = 24.0 IN. C/C

THIS DESIGN COMPLIES WITH:

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN
LOAD] EQUALS 25.6 P.S.F. SPECIFIED ROOF
LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L.999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L.999 (0.04")

CSI: TC=0.12/1.00 (A-B:1), BC=0.14/1.00 (D-E:4),
WB=0.00/1.00 (v8:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=0.98 NAIL=0.98 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

TABLE 1

PLATE PLACEMENT TOL. = 0.250 inches

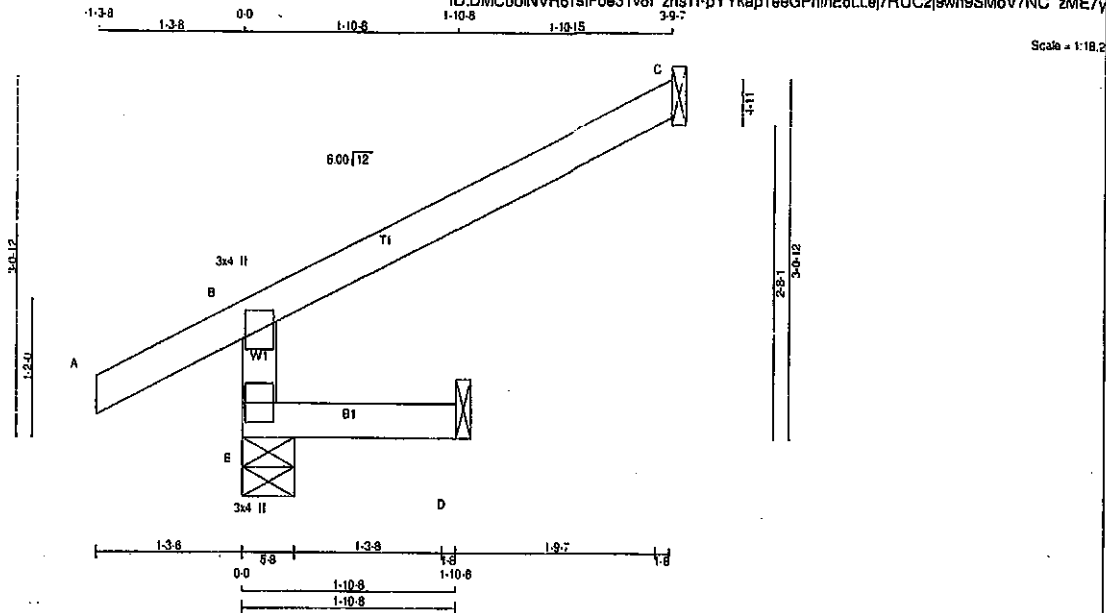
PLATE ROTATION TOL. = 5.0 Deg.

```
JSI GRIP= 0.10 ;EJ INPUT = 0.90 ;
JSI METAL= 0.06 ;B INPUT = 1.00 ;
```

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	C42	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:DMCubINVR6TsIFoe31v6I zns11-pYYkap1eeGPhnE6LL6j7RUC2j9wh9SMoV7NC zME7y



LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DESCR. SPF
SPF
SPF

DRY; SEASONED LUMBER.

PLATES (table in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
E	381	0	361	0	0	5-8	5-8		
C	130	0	130	0	0	1-8	1-8		
D	18	0	17	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) E

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	250	190 0	0 0	0 0	0 0	0 0	60 0	0 0
C	90	73 0	0 0	0 0	0 0	0 0	17 0	0 0
D	12	0 0	0 0	0 0	0 0	0 0	12 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (8)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (CST)(LC)	MEMB.	FORCE (LBS)	LC1 MAX (CST)(LC)	UNBRAC LENGTH FR-TO
FR-TO		FROM TO		FR-TO			
E-B	-342 0	0.0	0.0 0.01 (4)	7.81			
A-B	0 28	-81.8	-81.8 0.13 (5)	10.00			
B-C	-19 0	-91.8	-91.8 0.22 (1)	6.25			
E-D	0 0	-18.5	-18.5 0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/989 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/989 (0.00")

CSI: TC=0.22(1.00 (B-C:1)), BC=0.02(1.00 (D-E:4)), WB=0.00(1.00 (rva:0)), SSI=0.15(1.00 (B-C:1))

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	518 354	1887 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.14 (E) (INPUT = 0.90)
JSI METAL = 0.09 (B) (INPUT = 1.00)

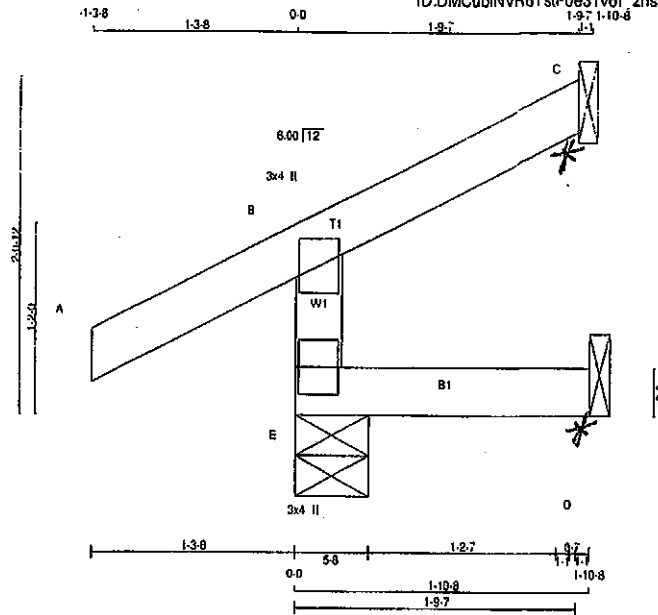


Structural component only
DWG# T-2007650

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408224	C43	3	1	TRUSS DESC.		

Vamarack Roof Truss, Burlington

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ID:DMCubINVR6TstFoe31v6I zns11-k66o92GPaxYNxplv39yif1OQ6VqCqV9sxiRzME7x



Scale = 1"=13.1'

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
E	271	0	271	0	0	5-8	5-8		
C	45	0	45	0	23	1-8	1-8		
D	8	0	17	0	2	1-8	1-8		

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	188	141	0	0.0	0.0	0.0	47	0
C	31	24	-18	0.0	0.0	0.0	7	0
D	7	0	-8	0.0	0.0	0.0	12	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1	MAX. CS (LC)	UNBRAC	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	LENGTH	FR-TO		
E-B	-244	0	0.0	0.04 (5)	7.81		
A-B	0	28	-91.8	-91.8	0.12 (1)	10.00	
B-C	-17	0	-91.8	-91.8	0.09 (1)	6.25	
E-D	0	0	-18.5	-18.5	0.04 (5)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 7 = 21 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
PART 9 OF BCBC 2018, CBC 2012, ABC 2019
PART 9 OF CBC 2012 (2019 AMENDMENT)
CSA 088-09, CSA 088-14
TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) . BC=0.04/1.00 (D-E:5) .
WB=0.09/1.00 (max:0) . SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.10 (E) INPUT = 0.90
JSI METAL = 0.07 (B) INPUT = 1.00



Structural component only
DWG# T-2007651

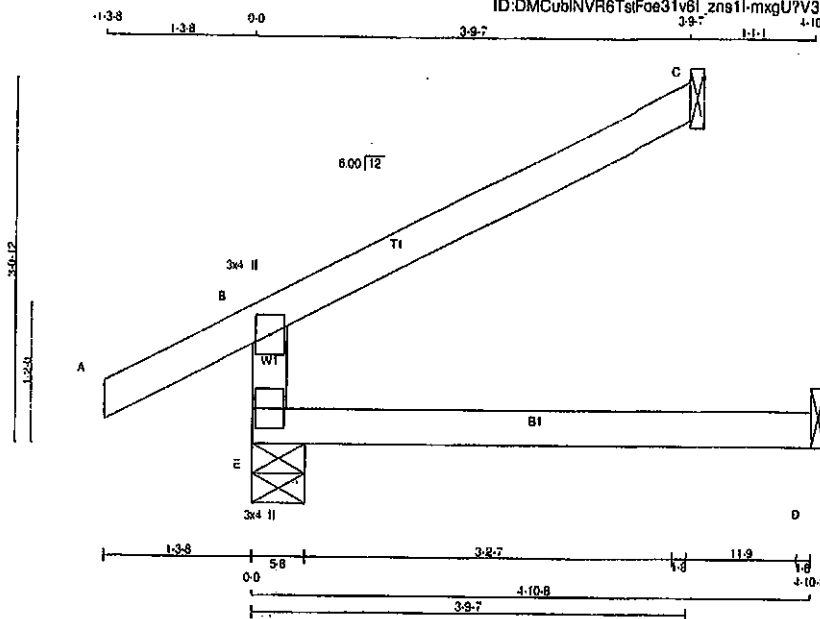
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	C44	2	1	GREEN PARK HOMES	
amarack Roof Truss, Burlington		TRUSS DESC.			

amarack Roof Truss, Burlington

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ID:DMCubINVR6TstFos31v6I_zns11-mxgU7V3uAtIP 5OUTmhBCsZXVWqD93ye2pcUHzME7w

Scale = 1:17.5



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2	
A - C	2x4	DRY	No.2	
E - D	2x4	DRY	No.2	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
8	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX
E	394	0	394	0	0	5-8	5-8	
C	130	0	130	0	0	1-8	1-8	
D	38	0	42	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL 697791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	MAX. MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	277	190	0	0	0	87	0
C	90	73	0	0	0	17	0
D	30	0	0	0	0	30	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. CS (LBS)	MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. CS (LBS)
FR-TO		FROM TO	MAX. UNBRAC LENGTH	FR-TO			
E-B	-342	0.0	0.0	0.08 (4)	7.81		
A-B	0	-91.8	-91.8	0.12 (1)	10.00		
B-C	-19	-91.8	-91.8	0.22 (1)	8.25		
E-D	0	-18.5	-18.5	0.09 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.22;1.00 (B-C:1) , BC=0.09;1.00 (D-E:4) ,
WB=0.09;1.00 (max) , SSI=0.15;1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSB)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) INPUT = 0.90)
JSI METAL= 0.09 (B) INPUT = 1.00)



Structural component only
DWG# T-2007652

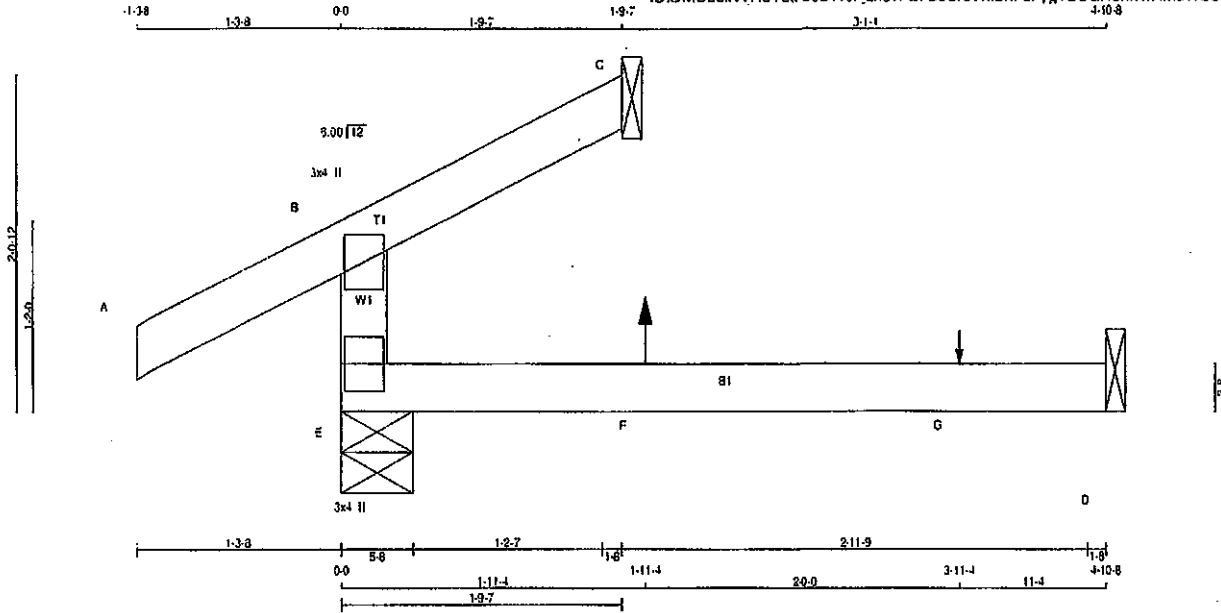
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408224	C45	2	1	TRUSS DESC.		

Yamarrack Roof Truss, Burlington

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Scale = 1/13.1



TOTAL WEIGHT = 2 X 10 = 21 lb

<u>LUMBER</u>					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR.
E - B	2x4	DRY	No.2		SPF
A - C	2x4	DRY	No.2		SPF
E - D	2x4	DRY	No.2		SPF
DRY: SEASONED LUMBER.					

PLATES (tablets in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
E	282	0	282	0	0	5-8
C	55	0	55	0	0	1-8
D	35	0	44	0	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
		SNOW	LIVE	PERM. LIVE	WIND			
E	198	137	0	0	0	0	61	0
C	39	21	0	0	0	0	18	0
D	29	0	4	0	0	0	31	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO			FR-TO		
E-B	-235 0	0.0 0.0	0.07 (4)	7.81			
A-B	0 28	-91.8 -91.8	0.12 (1)	10.00			
B-C	-13 5	-91.8 -91.8	0.07 (5)	6.25			
E-F	0 0	-18.5 -18.5	0.10 (4)	10.00			
F-G	0 0	-18.5 -18.5	0.10 (4)	10.00			
G-D	0 0	-18.5 -18.5	0.10 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	BACK	VERT	TOTAL	---	C1
G	3-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD			=	39.0 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CS1: TC=0.12/1.00 (A-B:1), BC=0.10/1.00 (D-E:4), WB=0.00/1.00 (m:a:0), SS1=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	618	354
	1687	788
	1687	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

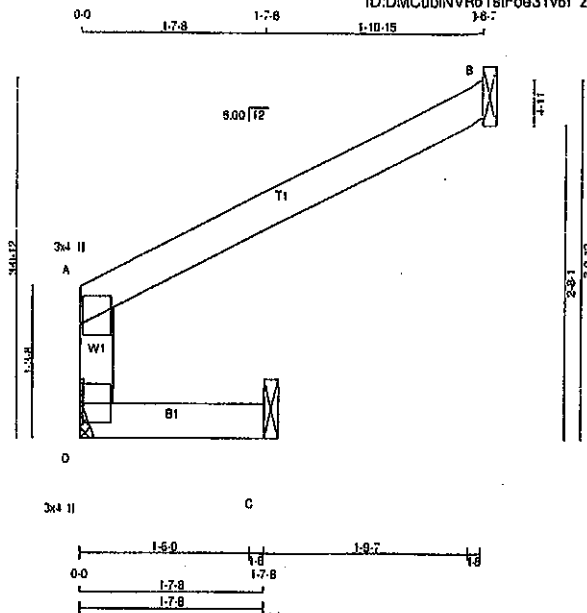
JSI GRIP= 0.10 (E) INPUT = 0.90
JSI METAL= 0.07 (B) INPUT = 1.00



Structural component only
DWG# T-2007653

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408224	C46	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.310 S Oct 29 2019 MITek Industries, Inc. Tue Apr 28 10:19:49 2020 Page 1	

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Scale = 1:18.2

LUMBER	SIZE	LUMBER
N. L. G. A. RULES		
CHORDS	SIZE	No.2
D - A	2x4 DRY	No.2
A - B	2x4 DRY	No.2
A - C	2x4 DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE	PLATES			
A	TMV+p	MT20	3.0	4.0
D	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
D	158	0	158	0	MECHANICAL	
B	144	0	144	0	1-8	1-8
C	55	0	55	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	COMBINED	MAX. MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	109	78 0	0 0	0 0	0 0	31 0	0 0
B	99	80 0	0 0	0 0	0 0	19 0	0 0
C	40	23 0	0 0	0 0	0 0	17 0	0 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED (PLF)	MAX. FACTORED (LBS)	MAX. FACTORED (LBS)	MAX. FACTORED (LBS)
FR-TO						
D-A	-181 0	0.0	0.0	0.08 (1)	7.81	
A-B	-8 0	-91.8	-91.8	0.14 (1)	10.00	
D-C	0 0	-18.5	-18.5	0.09 (1)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, NBCC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.18")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.18")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (A-B:1), BC=0.09/1.00 (C-D:1), WB=0.00/1.00 (A-B:1), SSI=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.08 (D) INPUT = 0.90
JSI METAL = 0.05 (A) INPUT = 1.00

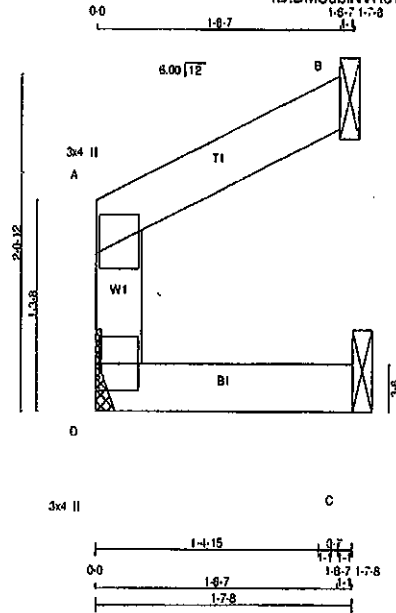


Structural component only
DWG# T-2007654

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408224	C47	1	1	GREEN PARK HOMES		

Hamrack Roof Truss, Burlington

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Scale = 1/16"

TOTAL WEIGHT = 5 lb (M/F)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	

DRY, SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMV+P	MT20	3.0	4.0		
D	BMV+P	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
D	86	0	86	0
B	87	0	87	0
C	19	0	19	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B, C.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	61	39 0	0 0	0 0	0 0	21 0	0 0	0 0
B	48	37 0	0 0	0 0	0 0	9 0	0 0	0 0
C	14	2 0	0 0	0 0	0 0	12 0	0 0	0 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. MEMB. FORCE (LBS)	MAX. MEMB. FORCE (LBS)
FR-TO				
D-A	-74 0	0.0 0.0 0.01 (1)	7.81	
A-B	-2 0	-91.8 -91.8 0.03 (1)	10.00	
D-C	0 0	-18.5 -18.5 0.01 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:	TOP CH.	LL	PSF
	DL	6.0	PSF
SPECIFIED LOADS:	BOT CH.	LL	PSF
	DL	0.0	PSF
	DL	7.4	PSF
	TOTAL LOAD	39.0	PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.03 (1.00 (A-B:1), BC=0.01 (1.00 (C-D:4), WB=0.00 (1.00 (m/a:0), SSI=0.05 (1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.03 (D) INPUT = 0.90
JSI METAL = 0.02 (A) INPUT = 1.00



Structural component only
DWG# T-2007655

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers



*This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.*

Most hangers in this series have double-shear nailing -- an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258-259.

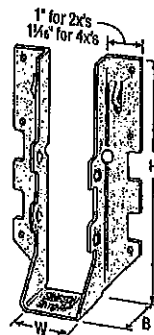
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20-24.

Installation:

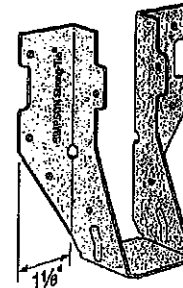
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/4" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

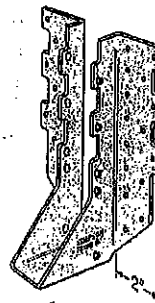
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options Information on p. 126.



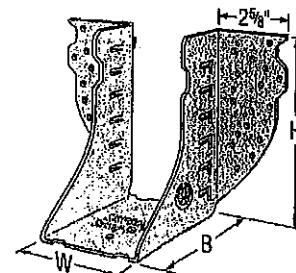
LUS28



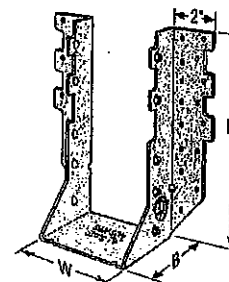
LU26L



HUS210
(HUS26, HUS28,
and HHUS similar)



HGUS28-2



HHUS210-2



Double-Shear
Nailing
Top View

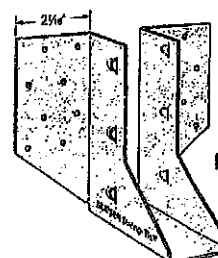
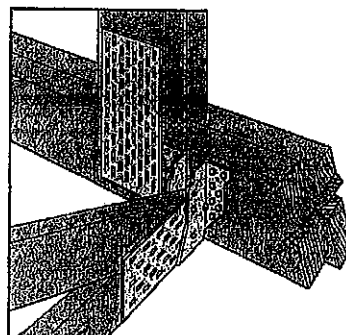


Double-Shear
Nailing
Side View;
Do not
bend tab



Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580

**Typical HUS26
Installation
with Reduced
Heel Height**
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS**HHUS/HGUS**

See Hanger Options Information on pp. 125-127.

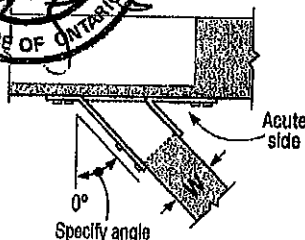
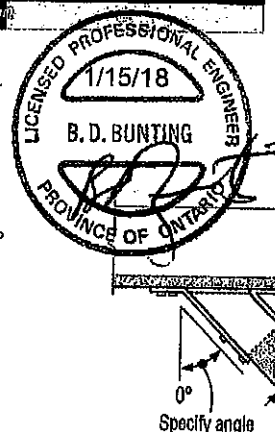
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.87 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _g ³	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Single 2x Sizes											
LUS24	18	1 1/8	3 1/4	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1625	845	1155
								3.18	7.23	2.87	5.14
LU24L	22	1 1/8	3	1 1/4	2 1/4	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725
								1.60	4.54	1.42	3.22
LU26L	22	1 1/8	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"	720	1605	645	1140
								3.20	7.14	2.87	5.07
LUS26	18	1 1/8	4 1/4	1 1/4	3 3/4	(4) 10d	(4) 10d	1420	2170	1290	1830
								6.32	9.65	5.74	7.25
HUS26	18	1 1/8	5 1/4	3	3 3/4	(14) 10d	(8) 16d	2705	4940	2065	3875
								11.80	21.97	9.20	17.24
LJS26DS	18	1 1/8	5	3 1/2	4 1/4	(16) 16d	(8) 16d	2055	4265	1480	4115
								9.14	18.97	6.49	18.31
HGUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 16d	(8) 16d	2885	6625	2685	5700
								11.98	29.51	11.96	25.35
LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1550
								5.07	9.72	4.54	6.89
LUS28	18	1 1/8	6 1/4	1 1/4	3 3/4	(6) 10d	(4) 10d	1420	2520	1290	1790
								6.32	11.21	5.74	7.96
HUS28	16	1 1/8	7 1/4	3	6 1/4	(22) 18d	(8) 16d	3805	5385	2675	4345
								16.04	23.88	11.90	19.33
HGUS28	12	1 1/8	7 1/4	5	6 1/4	(35) 16d	(12) 16d	3310	7675	3310	6900
								14.74	34.19	14.74	30.73
LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(8) 10d x 1 1/2"	1140	2495	1020	1770
								5.07	11.10	4.54	7.87
LUS210	18	1 1/8	7 1/4	1 1/4	3 3/4	(8) 10d	(4) 10d	1420	2785	1290	2210
								6.32	12.39	5.74	9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.

2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.

3. d_g is the distance from the bearing seat to the top joist nail.

4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.

5. Nails: 16d = 0.182" dia. x 3 1/2" long. See pp. 27-28 for other nail sizes and information.

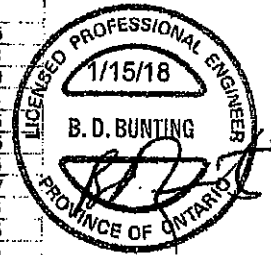
Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _o ^a	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _d = 1.15)	(K _d = 1.00)	(K _d = 1.15)	(K _d = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
LUS24-2	18	3 3/8	3 3/8	2	1 1/2	(4) 16d	(2) 16d	835	2020	590	1435
SS LUS26-2	18	3 3/8	4 1/8	2	4	(4) 16d	(4) 16d	3.71	8.89	2.62	6.38
								1720	2595	1545	1920
HHUS26-2	14	3 3/8	5 3/8	3	3 1/8	(14) 16d	(6) 16d	7.85	11.54	6.87	8.64
								2850	7335	2085	5205
HGUS26-2	12	3 3/8	5 3/8	4	4 1/8	(20) 16d	(8) 16d	12.68	32.83	9.20	23.15
								4385	8950	3110	6355
SS LUS28-2	18	3 3/8	7	2	4	(6) 16d	(4) 16d	19.51	39.81	13.83	28.27
								1720	3325	1545	2675
HHUS28-2	14	3 3/8	7 3/8	3	6 3/8	(22) 16d	(8) 16d	7.85	14.79	6.87	11.45
								3765	8940	2675	6345
HGUS28-2	12	3 3/8	7 3/8	4	8 3/8	(36) 16d	(12) 16d	16.75	39.77	11.90	28.22
								6070	12980	4310	9215
SS LUS210-2	18	3 3/8	9	2	6	(8) 16d	(6) 16d	27.00	57.74	19.17	40.99
								2580	4500	2320	3195
HHUS210-2	14	3 3/8	9 3/8	3	8	(30) 16d	(10) 16d	11.48	20.02	10.32	14.21
								4670	9660	4235	7000
HGUS210-2	12	3 3/8	9 3/8	4	8 3/8	(46) 16d	(16) 16d	20.77	42.97	18.84	31.14
								6840	14015	4855	10270
								30.43	62.34	21.60	45.69
Triple 2x Sizes											
HGUS26-3	12	4 1/8	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-3	12	4 1/8	7 1/4	4	6 3/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-3	14	4 1/8	9	3	7 3/8	(30) 16d	(10) 16d	4670	9670	4235	6865
								20.77	43.02	18.84	30.54
HGUS210-3	12	4 1/8	9 1/4	4	8 3/8	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	6 3/8	5 3/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-4	12	6 3/8	7 3/8	4	6 3/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-4	14	6 3/8	8 3/8	3	7 3/8	(30) 16d	(10) 16d	4670	10155	4235	7210
								20.77	45.17	18.84	32.07
HGUS210-4	12	6 3/8	9 3/8	4	8 3/8	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
HGUS212-4	12	6 3/8	10 3/8	4	10 3/8	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS214-4	12	6 3/8	12 3/8	4	11 3/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
LUS46	18	3 3/8	4 3/8	2	3 3/8	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.64
HHUS46	14	3 3/8	5 1/4	3	3 3/8	(14) 16d	(6) 16d	2540	7335	2085	5205
								11.30	32.83	9.20	23.15
HGUS46	12	3 3/8	5 1/4	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS48	18	3 3/8	6 3/8	2	3 3/8	(6) 16d	(4) 16d	1720	3325	1545	2675
								7.65	14.79	6.87	11.45
HHUS48	14	3 3/8	7 1/8	3	6 3/8	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS48	12	3 3/8	7 1/8	4	6 3/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS410	18	3 3/8	8 3/8	2	5 3/8	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HGUS410	12	3 3/8	9	4	8 3/8	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
HGUS412	12	3 3/8	10 3/8	4	10 3/8	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS414	12	3 3/8	12 3/8	4	11 3/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80



Plated Truss Connectors

See footnotes
on p. 258.

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

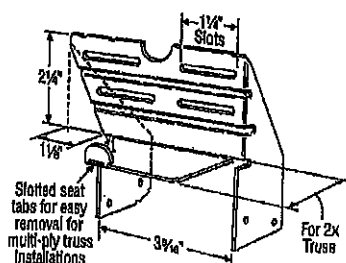
Design: Factored resistances are in accordance with CSA 086-14

Installation:

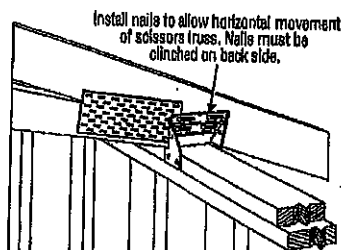
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

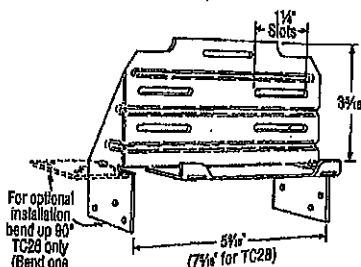
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



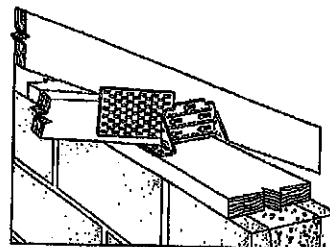
TC24
U.S. Patent 4,932,173



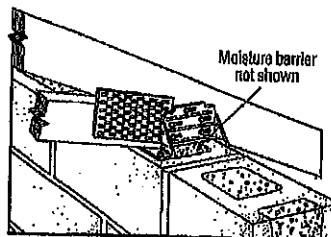
Typical TC24 Installation



TC26
(TC28 Similar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer (8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift ($K_F=1.15$)	Uplift ($K_F=1.15$)
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift ($K_F=1.15$)	Uplift ($K_F=1.15$)
TC26	(5) 10d	(8) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) — 9/16" x 2¼" Titen screws has a factored uplift resistance of 276 lb.



DESIGN

DESIGN

DESIGN

DESIGN

(800) 999-5099
strongtie.com

H/TSP

Seismic and Hurricane Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Fasteners			Factored Resistance ($K_D = 1.15$)					
					D.Fir-L			S-P-F		
		To Rafter/Truss	To Plates	To Studs	Uplift	Lateral		Uplift	Lateral	
						F ₁	F ₂		F ₁	F ₂
					lb.	lb.	lb.	lb.	lb.	lb.
					kN	kN	kN	kN	kN	kN
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
					3.29	3.05	1.33	3.02	2.16	0.98
SS H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
					3.69	0.98	0.33	2.62	0.69	0.24
SS H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
					3.58	0.71	0.71	3.36	0.71	0.71
H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
					3.71	0.78	0.93	3.29	0.71	0.93
SS H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
					3.29	0.80	1.18	2.74	0.56	0.85
H6	16	—	(8) 8d	(8) 8d	1585	1085	—	1125	770	—
					7.05	4.83	—	5.00	3.43	—
H7Z	16	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
					6.18	2.98	—	4.40	2.11	—
SS H8*	18	(5) 10d x 1½"	(5) 10d x 1½"	—	1120	—	—	1025	—	—
					4.98	—	—	4.56	—	—
SS H10A*	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290
					7.72	3.54	1.82	6.69	2.51	1.29
H10AR	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1485	690	430	1220	570	305
					6.61	3.07	1.91	5.43	2.54	1.36
H10A-2	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1835	1275	430	1645	880	305
					8.16	5.67	1.91	7.32	3.91	1.36
H10S ^{2a}	18	(8) 8d x 1½"	(8) 8d x 1½"	(8) 8d	1465	795	315	1040	565	225
					6.52	3.54	1.40	4.63	2.51	1.00
H11Z	18	(6) 16d x 2½"	(6) 16d x 2½"	—	1095	920	545	780	655	390
					4.87	4.09	2.42	3.47	2.91	1.73
H14	18	[1] (12) 8d x 1½"	(13) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
		[2] (12) 8d x 1½"	(15) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
TSP	16	(9) 10d x 1½"	(6) 10d x 1½"	—	1295	440	—	920	310	—
					5.76	1.98	—	4.09	1.38	—
		(9) 10d x 1½"	(6) 10d	—	1560	440	—	1105	310	—
					6.94	1.98	—	4.92	1.38	—

- Factored resistances have been increased 16% for short term loading; no further increase is allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
- H8 factored uplift resistances for stud-to-bottom plate installations are 595 lb. (2.65 kN) for D.Fir-L and 390 lb. (1.74 kN) for S-P-F.
- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-H/TIECONPATH).
- Factored resistances in the F₁ direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.

- H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.38 kN) D.Fir-L and 1015 lb. (4.51 kN) S-P-F.
- H10S nails to plates are optional for uplift but required for lateral loads.
- H10A may be field-bent up to a slope of 6/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
- The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SSNAILS at strongtie.com.
- D.Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1½" nails into the top plates and (3) 8d x 1½" nails into the lowest three flange holes into the truss bottom chord is 485 lb. (2.20 kN).
- Nails: 16d x 2½" = 0.162" dia. x 2½" long, 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long, 8d x 1½" = 0.131" dia. x 1½" long. See pp. 27–28 for other nail sizes and information.

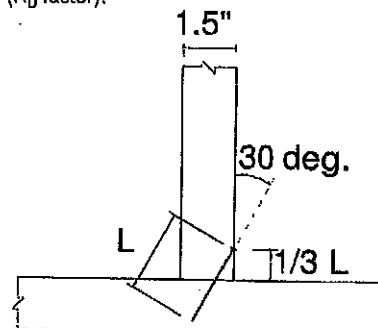
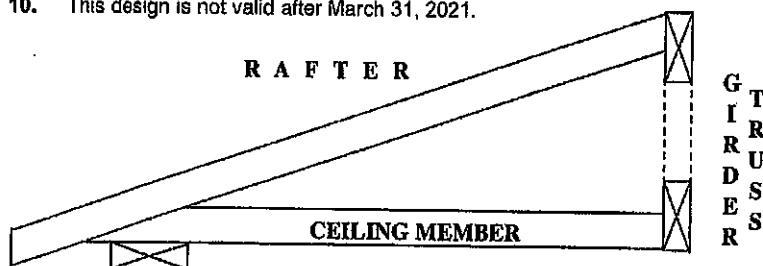
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON	3.00	0.144	132	147
	3.25	0.144	132	147
WIRE	3.50	0.160	159	177
	3.00	0.122	97	108
COMMON	3.25	0.122	97	108
	3.50	0.152	145	162
SPIRAL				

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4.
10. This design is not valid after March 31, 2021.



TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (In)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019

PEO
Certificate No. 10889485



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

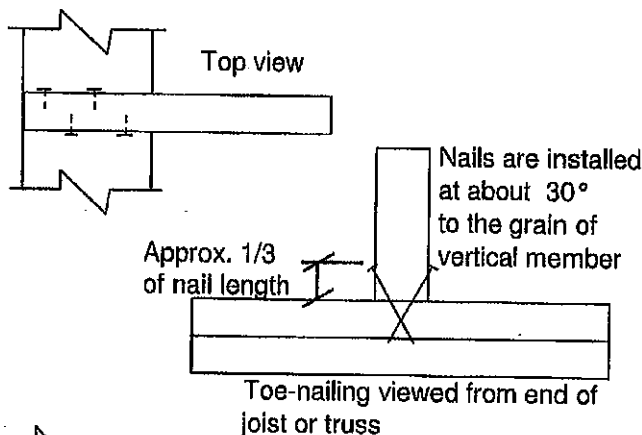
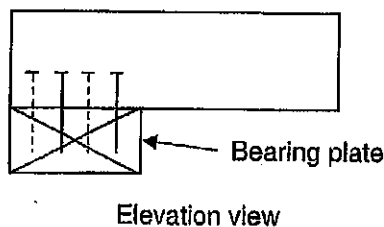
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

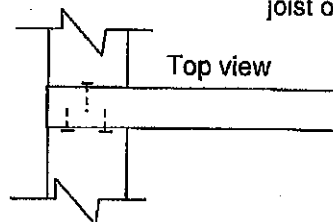
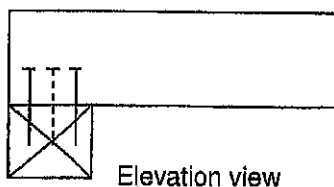
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to wind or earthquake load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37679H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



MiTek® MiTek Canada Inc
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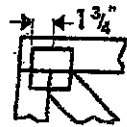
PEO
Certificate No. 10889485



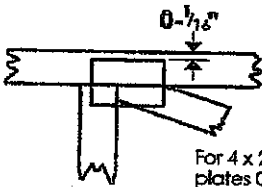
December 2, 2019

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint, unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0 1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

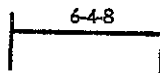


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

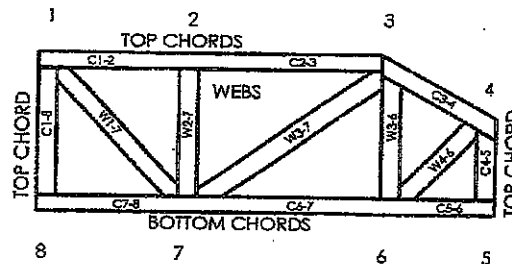
Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MIT-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane of joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

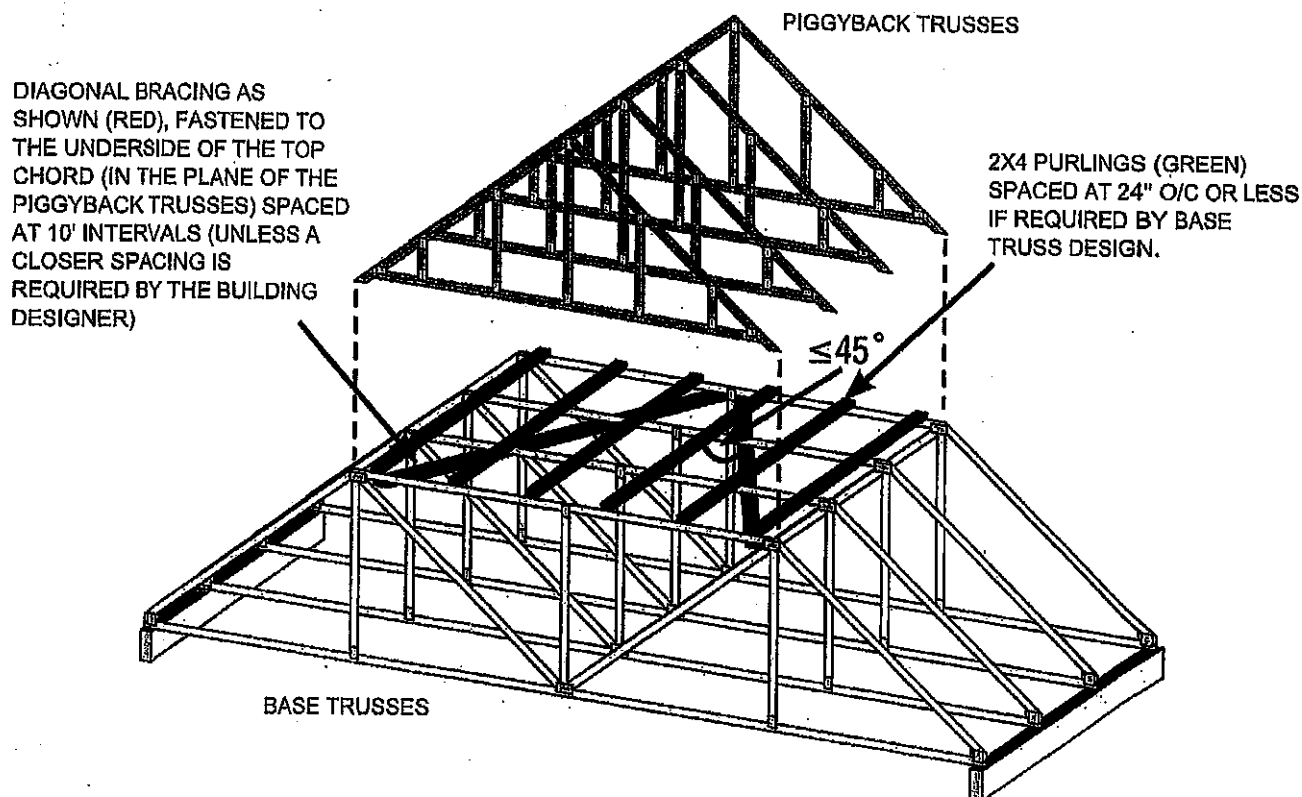
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

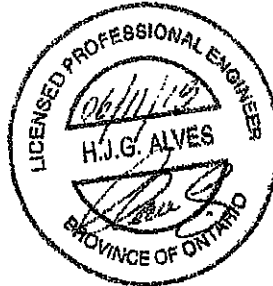
Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.



Alves Engineering Services Inc.

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Burlington, Ontario L7L 6N6

(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for Information on symbols, numbering system and General Safety notes.

F-1900219

Feb 09, 2018