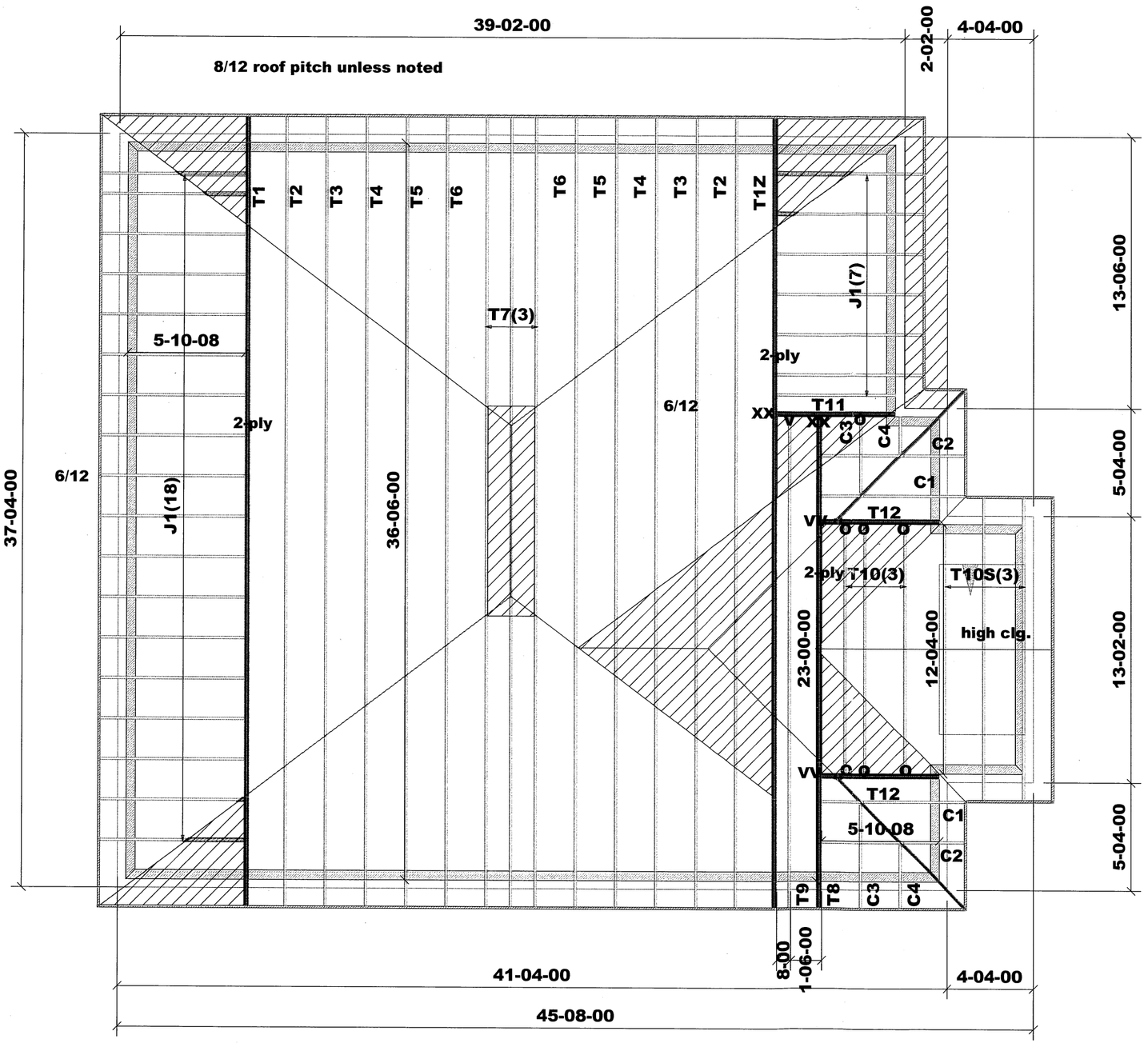
LAMPONE INVESTMENT INC

CITY OF MARKHAM

ROOF TRUSS SHOP DRAWINGS

MODEL NAME : KIMBERLY 1

ELEV 1, 2 & 3



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
- TCSL = 25.6 psf
- TCDL = 6.0 psf
- BCLL = 0.0 psf
- BCDL = 7.4 psf

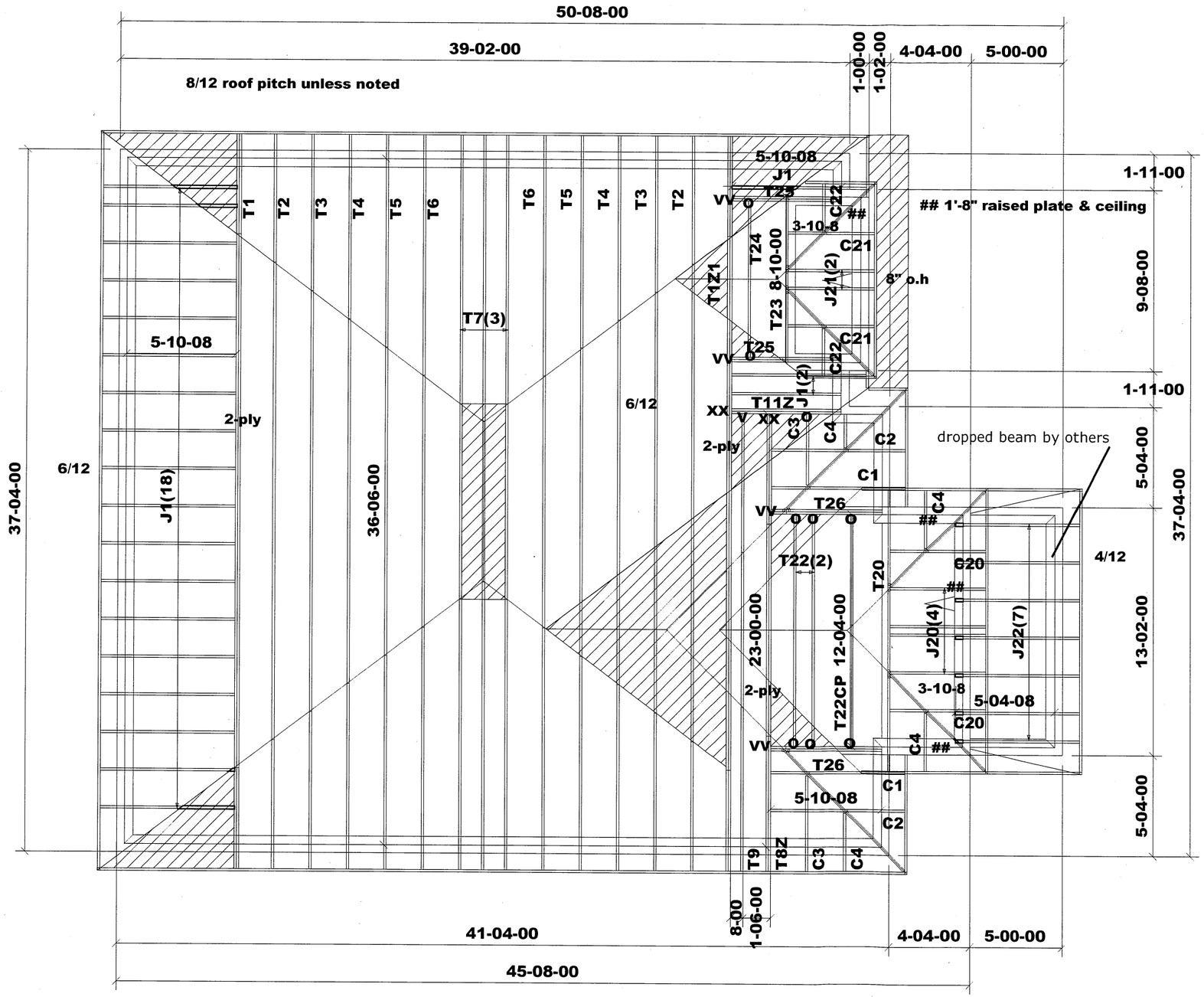
HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26-2 - (**XX**)
LUS26-2 - (VV)

DENOTES:
CONVENTIONAL FRAMING

M-13333

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Job Track: 51453	Builder / Location: GREENPARK HOMES / MARKHAM	Model / Elevation: KIMBERLY 1 / 1	Mitek ver 8.3.3.247
Layout ID: 409707	Project: LAMPONE INVESTMENTS INC		
Plan Log: 202819	Date: 2020-08-06	Designer: JG	



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

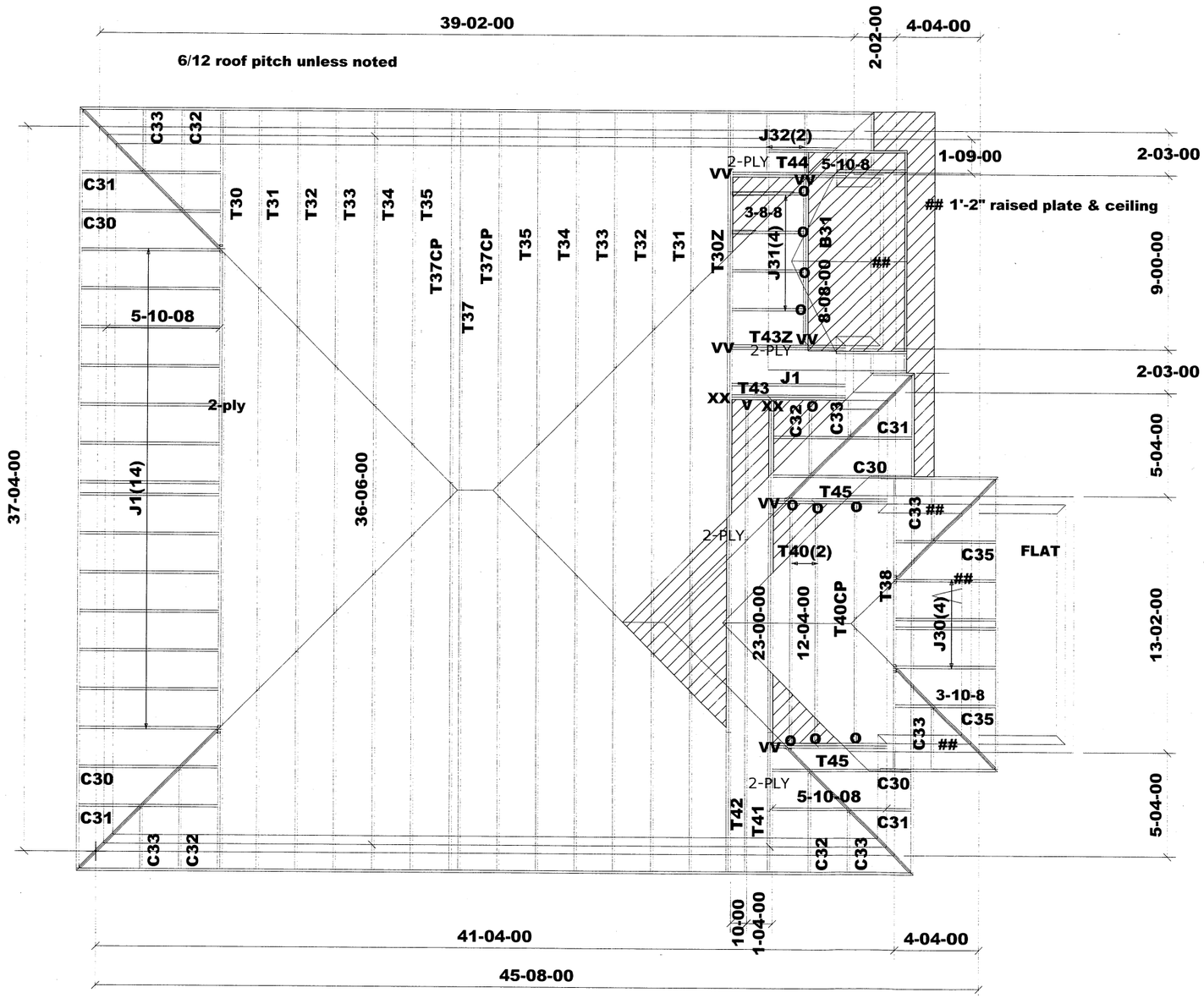
DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26-2 - (**XX**)
LUS26- (VV)

DENOTES:
CONVENTIONAL FRAMING

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Job Track: 51453	Builder / Location: GREENPARK HOMES / MARKHAM	Model / Elevation: KIMBERLY 1 / 2	Mitek ver 8.3.3.247
Layout ID: 409988	Project: LAMPONE INVESTMENTS INC		
Plan Log: 202819	Date: 2020-08-06	Designer: JG	



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26-2 - (**XX**
LUS26-2 - (VV))

DENOTES:
CONVENTIONAL FRAMING

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Job Track:	51453
Layout ID:	409989
Plan Log:	202819

Builder / Location: **GREENPARK HOMES / MARKHAM**
Project: **LAMPONE INVESTMENTS INC**
Date: 2020-08-06 | Designer: JG

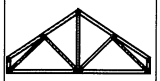
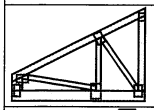
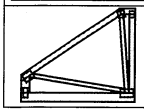
Model / Elevation: **KIMBERLY 1 / 3**
Mitek ver 8.3.3.247

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREENPARK HOMES	PlanLog:	202819
Project:	LAMPONE INVESTMENTS INC	Layout ID:	409707
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 1	Page:	1 of 2
Lot #:		Date:	08-06-2020
Elevation:	1	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

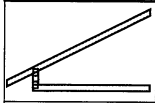
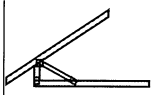
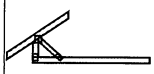
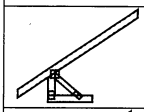
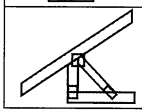
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1 Hip Girder	8 /12	**36-06-00**	4-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	387.89 238.00		
	1 2-ply	T1Z Hip Girder	8 /12	**36-06-00**	4-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	387.89 238.00		
	2	T2 Hip	8 /12	**36-06-00**	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	345.94 215.67		
	2	T3 Hip	8 /12	**36-06-00**	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	356.75 222.00		
	2	T4 Hip	8 /12	**36-06-00**	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	363.18 224.33		
	2	T5 Hip	8 /12	**36-06-00**	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	378.65 233.00		
	2	T6 Hip	8 /12	**36-06-00**	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	383.87 235.67		
	3	T7 Hip	8 /12	**36-06-00**	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	595.92 369.00		
	1 2-ply	T8 Hip Girder	8 /12	**23-00-00**	5-03-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	217.73 135.00		
	1	T9 Hip	8 /12	**23-00-00**	6-03-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	102.88 64.00		
	3	T10 Common	8 /12	**12-04-00**	5-06-02	2 x 4		1-04-13 1-04-13	152.13 96.50		
	3	T10S Roof Special	8 /12	**12-04-00**	5-06-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	174.29 117.00		
	1 2-ply	T11 Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	60.05 38.67		
	2 2-ply	T12 Half Hip Girder	8 /12	**5-10-08**	4-09-08	2 x 4 2 x 6		1-04-13 4-09-08	128.84 82.67		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREENPARK HOMES	PlanLog:	202819
Project:	LAMPONE INVESTMENTS INC	Layout ID:	409707
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 1	Page:	2 of 2
Lot #:		Date:	08-06-2020
Elevation:	1	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	25	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	419.86 266.67		
	2	C1 Jack-Open	8 /12	**3-09-07**	3-11-02	2 x 4	1-05-00 2-01-01	1-04-13 3-11-02	33.77 21.67		
	2	C2 Jack-Open	8 /12	**1-09-07**	2-07-02	2 x 4	1-03-08 4-01-01	1-04-13 2-07-02	26.93 16.67		
	2	C3 Jack-Open	8 /12	**1-10-08**	3-11-02	2 x 4	1-03-08 1-10-15	1-04-13 2-07-13	23.21 15.33		
	2	C4 Jack-Open	8 /12	**1-09-07**	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	17.62 11.33		

TOTAL # TRUSS= **65** TOTAL BFT OF ALL TRUSSES= **2841.18** BFT. TOTAL WEIGHT OF ALL TRSSES **4557.39** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
1	Hardware	LJS26DS	
7	Hardware	LUS24	
2	Hardware	LUS26-2	

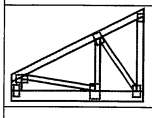
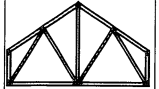
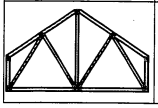
TOTAL NUMBER OF ITEMS= **12**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREENPARK HOMES	PlanLog:	202819
Project:	LAMPONE INVESTMENTS INC	Layout ID:	409988
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 1	Page:	1 of 3
Lot #:		Date:	08-06-2020
Elevation:	2	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

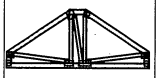
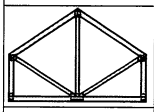
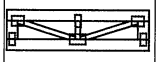
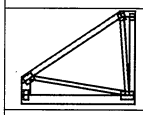
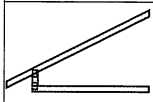
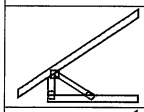
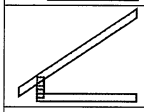
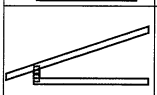
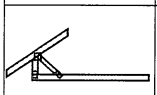
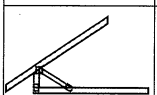
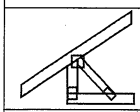
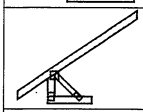
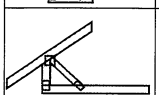
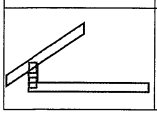
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	8 /12	**36-06-00**	4-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	392.65 240.67		
	1 2-ply	T1Z1 Hip Girder	8 /12	**36-06-00**	4-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	392.65 240.67		
	2	T2 Hip	8 /12	**36-06-00**	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	345.94 215.67		
	2	T3 Hip	8 /12	**36-06-00**	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	356.75 222.00		
	2	T4 Hip	8 /12	**36-06-00**	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	363.18 224.33		
	2	T5 Hip	8 /12	**36-06-00**	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	378.65 233.00		
	2	T6 Hip	8 /12	**36-06-00**	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	383.87 235.67		
	3	T7 Hip	8 /12	**36-06-00**	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	595.92 369.00		
	1 2-ply	T8Z Hip Girder	8 /12	**23-00-00**	5-03-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	217.73 135.00		
	1	T9 Hip	8 /12	**23-00-00**	6-03-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	102.88 64.00		
	1 2-ply	T11Z Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	60.05 38.67		
	1	T20 Hip Girder	8 /12	**12-04-00**	3-11-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	54.14 34.67		
	2	T22 Common	8 /12	**12-04-00**	7-02-02	2 x 4		3-00-13 3-00-13	120.23 77.67		
	1	T22CP Hip	8 /12	**12-04-00**	6-11-13	2 x 4		3-00-13 3-00-13	59.87 38.83		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREENPARK HOMES	PlanLog:	202819
Project:	LAMPONE INVESTMENTS INC	Layout ID:	409988
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 1	Page:	2 of 3
Lot #:		Date:	08-06-2020
Elevation:	2	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

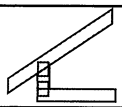
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T23 Hip Girder	8 /12	**8-10-00**	3-07-03	2 x 4		1-00-02 1-00-03	37 25.33		
	1	T24 Common	8 /12	**8-04-00**	5-07-08	2 x 4		2-10-03 2-10-03	38.18 25.50		
	2 2-ply	T25 Flat Girder	0 /12	**5-10-08**	1-08-00	2 x 6		1-08-00 1-08-00	113.64 70.00		
	2 2-ply	T26 Half Hip Girder	8 /12	**5-10-08**	4-09-08	2 x 4 2 x 6		1-04-13 4-09-08	128.84 82.67		
	21	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	352.68 224.00		
	4	J20 Jack-Open	8 /12	**3-10-08**	3-11-13	2 x 4	1-03-08	1-04-13 3-11-13	57.6 36.00		
	2	J21 Jack-Open	8 /12	**3-10-08**	3-07-03	2 x 4	8-08	1-00-03 3-07-03	23.14 14.67		
	7	J22 Jack-Open	4 /12	**5-04-08**	2-08-11	2 x 4	1-03-08	11-03 2-08-11	103.21 70.00		
	2	C2 Jack-Open	8 /12	**1-09-07**	2-07-02	2 x 4	1-03-08 4-01-01	1-04-13 2-07-02	26.93 16.67		
	2	C1 Jack-Open	8 /12	**3-09-07**	3-11-02	2 x 4	1-05-00 2-01-01	1-04-13 3-11-02	33.46 20.67		
	4	C4 Jack-Open	8 /12	**1-09-07**	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	35.24 22.67		
	2	C3 Jack-Open	8 /12	**1-10-08**	3-11-02	2 x 4	1-03-08 1-10-15	1-04-13 2-07-13	23.21 15.33		
	2	C20 Jack-Open	8 /12	**1-09-07**	2-07-02	2 x 4	1-03-08 2-01-01	1-04-13 2-07-02	22.37 14.00		
	2	C21 Jack-Open	8 /12	**1-09-07**	2-02-08	2 x 4	8-08 2-01-01	1-00-03 2-02-08	17.41 12.00		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREENPARK HOMES	PlanLog:	202819
Project:	LAMPONE INVESTMENTS INC	Layout ID:	409988
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 1	Page:	3 of 3
Lot #:		Date:	08-06-2020
Elevation:	2	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	C22 Jack-Open	8 /12	1-09-07	2-02-08	2 x 4	8-08 1-01	1-00-03 2-02-08	12.85 9.33		

TOTAL # TRUSS= **86** TOTAL BFT OF ALL TRUSSES= **3028.69** BFT. TOTAL WEIGHT OF ALL TRSSES **4850.27** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
1	Hardware	LJS26DS	
9	Hardware	LUS24	
4	Hardware	LUS26-2	

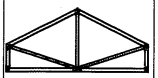
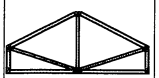
TOTAL NUMBER OF ITEMS= **16**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREENPARK HOMES	PlanLog:	202819
Project:	LAMPONE INVESTMENTS INC	Layout ID:	409989
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 1	Page:	1 of 3
Lot #:		Date:	08-06-2020
Elevation:	3	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

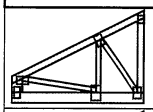
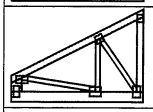
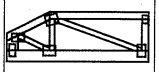
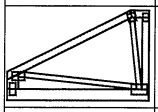
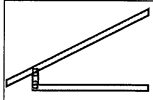
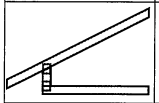
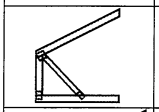
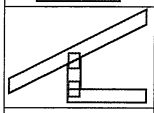
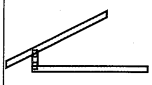
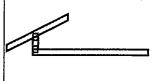
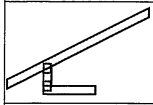
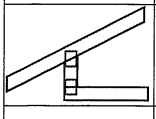
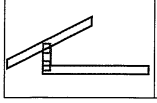
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T30 Hip Girder	6 /12	**36-06-00**	4-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	399.35 247.33		
	1 2-ply	T30Z Hip Girder	6 /12	**36-06-00**	4-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	399.35 247.33		
	2	T31 Hip	6 /12	**36-06-00**	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	342.94 213.67		
	2	T32 Hip	6 /12	**36-06-00**	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	345.25 213.33		
	2	T33 Hip	6 /12	**36-06-00**	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	342.76 208.33		
	2	T34 Hip	6 /12	**36-06-00**	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	355.98 220.00		
	2	T35 Hip	6 /12	**36-06-00**	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	362.17 224.00		
	1	T37 Common	6 /12	**36-06-00**	10-03-08	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	186.6 114.67		
	2	T37CP Hip	6 /12	**36-06-00**	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	375.48 232.00		
	1	T38 Hip Girder	6 /12	**12-04-00**	3-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	49.76 32.33		
	2	T40 Common	6 /12	**12-04-00**	5-05-00	2 x 4		2-04-00 2-04-00	96.3 63.00		
	1	T40CP Hip	6 /12	**12-04-00**	5-03-04	2 x 4		2-04-00 2-04-00	48 30.17		
	1 2-ply	T41 Hip Girder	6 /12	**23-00-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	204.3 130.00		
	1	T42 Hip	6 /12	**23-00-00**	4-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	93 60.17		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREENPARK HOMES	PlanLog:	202819
Project:	LAMPONE INVESTMENTS INC	Layout ID:	409989
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 1	Page:	2 of 3
Lot #:		Date:	08-06-2020
Elevation:	3	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T43 Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	60.05 38.67		
	1 2-ply	T43Z Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	60.05 38.67		
	1 2-ply	T44 Half Hip Girder	6 /12	**5-10-08**	2-00-08	2 x 4 2 x 6		1-02-00 2-00-08	51.09 35.33		
	2 2-ply	T45 Half Hip Girder	6 /12	**5-10-08**	3-08-08	2 x 4 2 x 6		1-02-00 3-08-08	116.05 70.67		
	15	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	251.92 160.00		
	4	J30 Jack-Open	6 /12	**3-10-08**	3-01-04	2 x 4	1-03-08	1-02-00 3-01-04	47.86 29.33		
	4	J31 Jack-Open	6 /12	**3-08-08**	4-01-04	2 x 4		2-03-00 4-01-04	52.96 35.33		
	2	J32 Jack-Open	6 /12	**1-09-00**	2-00-08	2 x 4	1-03-08	1-02-00 2-00-08	13.66 9.33		
	4	C30 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	56.53 34.67		
	4	C31 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	46.33 29.33		
	4	C32 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	38.28 24.00		
	6	C33 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	42.13 28.00		
	2	C35 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 2-01-01	1-02-00 2-00-12	18.6 12.00		

TOTAL # TRUSS= **79** TOTAL BFT OF ALL TRUSSES= **2781.66** BFT. TOTAL WEIGHT OF ALL TRSSES **4456.75** LBS

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREENPARK HOMES	PlanLog:	202819
Project:	LAMPONE INVESTMENTS INC	Layout ID:	409989
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 1	Page:	3 of 3
Lot #:		Date:	08-06-2020
Elevation:	3	Designer:	
		Sales Rep:	Mario DiCano

HARDWARE

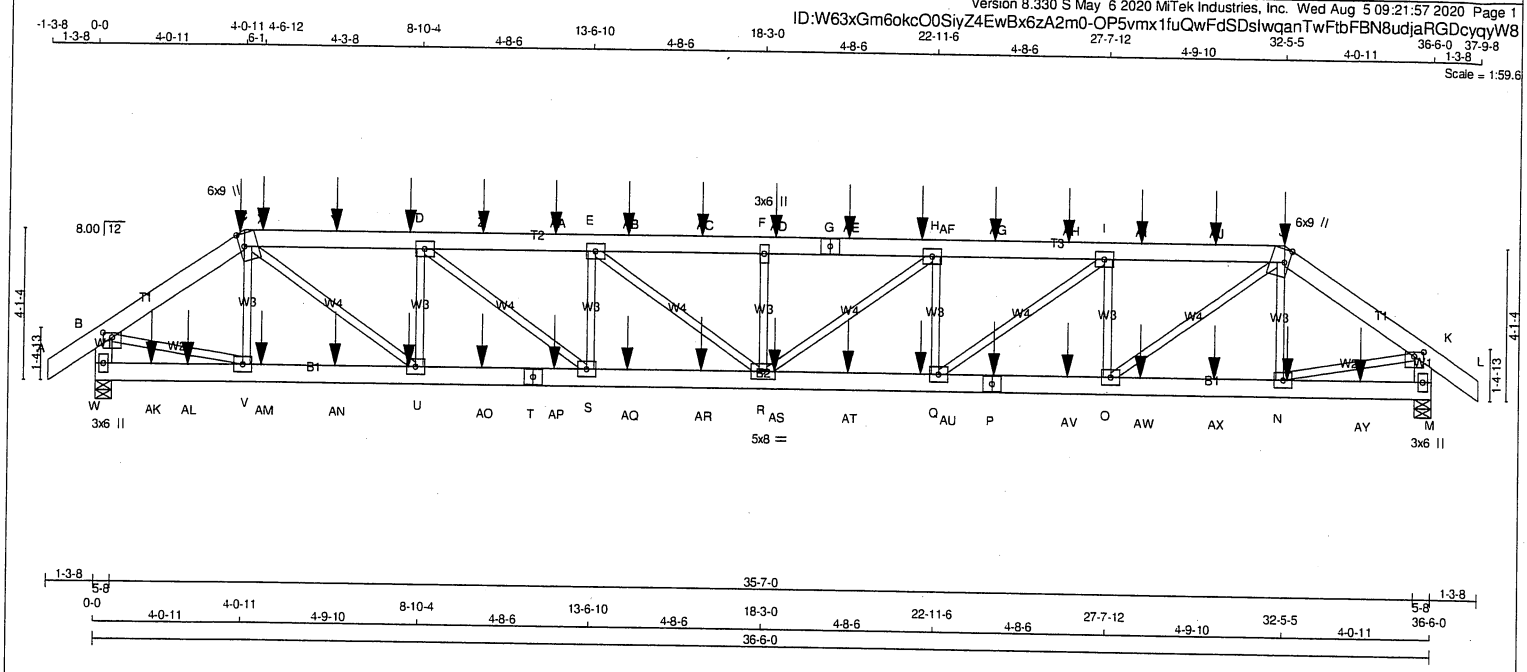
QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
1	Hardware	LJS26DS	
11	Hardware	LUS24	
6	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= **20**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T1	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:21:57 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-OP5vmx1fuQwFdSDsIwqanTwFtbFBN8udjaRGDcyqyW8



TOTAL WEIGHT = 2 X 194 = 388 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x6	DRY	1650F 1.5E	SPF
T - P	2x6	DRY	1650F 1.5E	SPF
P - M	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- G	2	12	SIDE(183.1)
G- J	2	12	SIDE(61.0)
J- L	2	12	SIDE(122.0)
W- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- T	2	12	SIDE(183.1)
T- P	2	12	SIDE(183.1)
P- M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2017329

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	3533	0	3533	0	0	5-8	5-8
M	3534	0	3534	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2497	1644 / 0	0 / 0	0 / 0	0 / 0	853 / 0	0 / 0
M	2497	1651 / 0	0 / 0	0 / 0	0 / 0	846 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 36	-91.8	-91.8	0.04 (1)	10.00
B- C	-4301 / 0	-91.8	-91.8	0.10 (1)	5.44
C- X	-6347 / 0	-91.8	-91.8	0.19 (1)	4.58
X- Y	-6347 / 0	-91.8	-91.8	0.19 (1)	4.58
Y- D	-6347 / 0	-91.8	-91.8	0.19 (1)	4.58
D- Z	-7949 / 0	-91.8	-91.8	0.20 (1)	4.17
Z-AA	-7949 / 0	-91.8	-91.8	0.20 (1)	4.17
AA- E	-7949 / 0	-91.8	-91.8	0.20 (1)	4.17
E-AB	-8479 / 0	-91.8	-91.8	0.22 (1)	4.04
AB-AC	-8479 / 0	-91.8	-91.8	0.22 (1)	4.04
AC- F	-8479 / 0	-91.8	-91.8	0.22 (1)	4.04
F-AD	-8479 / 0	-91.8	-91.8	0.23 (1)	4.03
AD- G	-8479 / 0	-91.8	-91.8	0.23 (1)	4.03
G-AE	-8479 / 0	-91.8	-91.8	0.23 (1)	4.03
AE-AF	-8479 / 0	-91.8	-91.8	0.23 (1)	4.03
AF- H	-8479 / 0	-91.8	-91.8	0.23 (1)	4.03
H-AG	-7947 / 0	-91.8	-91.8	0.21 (1)	4.17
AG-AH	-7947 / 0	-91.8	-91.8	0.21 (1)	4.17
AH- I	-7947 / 0	-91.8	-91.8	0.21 (1)	4.17
I-AI	-6348 / 0	-91.8	-91.8	0.19 (1)	4.58
AI-AJ	-6348 / 0	-91.8	-91.8	0.19 (1)	4.58
AJ- J	-6348 / 0	-91.8	-91.8	0.19 (1)	4.58
J- K	-4316 / 0	-91.8	-91.8	0.10 (1)	5.44
K- L	0 / 36	-91.8	-91.8	0.04 (1)	10.00
W- B	-3485 / 0	0.0	0.0	0.12 (1)	7.49
M- K	-3497 / 0	0.0	0.0	0.12 (1)	7.48
W-AK	0 / 0	-18.5	-18.5	0.03 (4)	10.00
AK-AL	0 / 0	-18.5	-18.5	0.03 (4)	10.00
AL- V	0 / 0	-18.5	-18.5	0.03 (4)	10.00
V-AM	0 / 3553	-18.5	-18.5	0.16 (1)	10.00
AM-AN	0 / 3553	-18.5	-18.5	0.16 (1)	10.00
AN- U	0 / 3553	-18.5	-18.5	0.16 (1)	10.00
U-AO	0 / 6347	-18.5	-18.5	0.29 (1)	10.00
AO- T	0 / 6347	-18.5	-18.5	0.29 (1)	10.00
T-AP	0 / 6347	-18.5	-18.5	0.29 (1)	10.00
AP- S	0 / 6347	-18.5	-18.5	0.29 (1)	10.00
S-AQ	0 / 7949	-18.5	-18.5	0.36 (1)	10.00
AQ-AR	0 / 7949	-18.5	-18.5	0.36 (1)	10.00
AR- R	0 / 7949	-18.5	-18.5	0.36 (1)	10.00
R-AS	0 / 7947	-18.5	-18.5	0.36 (1)	10.00
AS-AT	0 / 7947	-18.5	-18.5	0.36 (1)	10.00
AT-AU	0 / 7947	-18.5	-18.5	0.36 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-581 / 0	0.07 (1)
C- U	0 / 3501	0.43 (1)
U- D	-1984 / 0	0.24 (1)
D- S	0 / 2028	0.25 (1)
S- E	-1087 / 0	0.13 (1)
E- R	0 / 671	0.08 (1)
R- F	-689 / 0	0.08 (1)
R- H	0 / 673	0.08 (1)
Q- H	-1086 / 0	0.13 (1)
Q- I	0 / 2023	0.25 (1)
O- I	-1973 / 0	0.24 (1)
O- J	0 / 3487	0.43 (1)
N- J	-593 / 0	0.07 (1)
B- V	0 / 3651	0.45 (1)
N- K	0 / 3664	0.45 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CSI: TC=0.23/1.00 (F-H:1) , BC=0.36/1.00 (Q-R:1) , WB=0.45/1.00 (K-N:1) , SSI=0.13/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (N) (INPUT = 0.90)
JSI METAL= 0.59 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T1	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	6.0	9.0	4.25	1.50
D, E, H, I						
D	TMWW-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
J	TTWW+m	MT20	6.0	9.0	4.25	1.50
K	TMVW-p	MT20	5.0	6.0	1.50	3.00
M	BMV1+p	MT20	3.0	6.0		
N, O, Q, S, U, V						
N	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
T	BS-t	MT20	5.0	6.0		
W	BMV1+p	MT20	3.0	6.0		

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)
AU- Q	0 / 7947	-18.5	-18.5	0.36 (1)
Q- P	0 / 6349	-18.5	-18.5	0.29 (1)
P-AV	0 / 6349	-18.5	-18.5	0.29 (1)
AV- O	0 / 6349	-18.5	-18.5	0.29 (1)
O-AW	0 / 3565	-18.5	-18.5	0.17 (1)
AW-AX	0 / 3565	-18.5	-18.5	0.17 (1)
AX- N	0 / 3565	-18.5	-18.5	0.17 (1)
N-AY	0 / 0	-18.5	-18.5	0.03 (4)
AY- M	0 / 0	-18.5	-18.5	0.03 (4)

WEBS

MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
10.00			
10.00			
10.00			
10.00			
10.00			
10.00			
10.00			
10.00			
10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-51	-57	---	FRONT	VERT	DEAD	---	C1
C	4-0-11	-260	-260	---	FRONT	VERT	SNOW	---	C1
D	8-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
J	32-5-5	-51	-57	---	FRONT	VERT	DEAD	---	C1
J	32-5-5	-117	-117	---	FRONT	VERT	TOTAL	---	C1
J	32-5-5	-260	-260	---	FRONT	VERT	SNOW	---	C1
N	32-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	24-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
U	8-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
X	4-6-12	-133	-133	---	FRONT	VERT	TOTAL	---	C1
Y	6-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	10-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	12-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	14-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	16-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	18-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AE	20-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AF	22-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AG	24-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AH	26-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AI	28-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AJ	30-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AK	1-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AL	2-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AM	4-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AN	6-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AO	10-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AP	12-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AQ	14-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AR	16-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AS	18-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AT	20-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AU	22-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AV	26-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AW	28-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AX	30-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AY	34-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

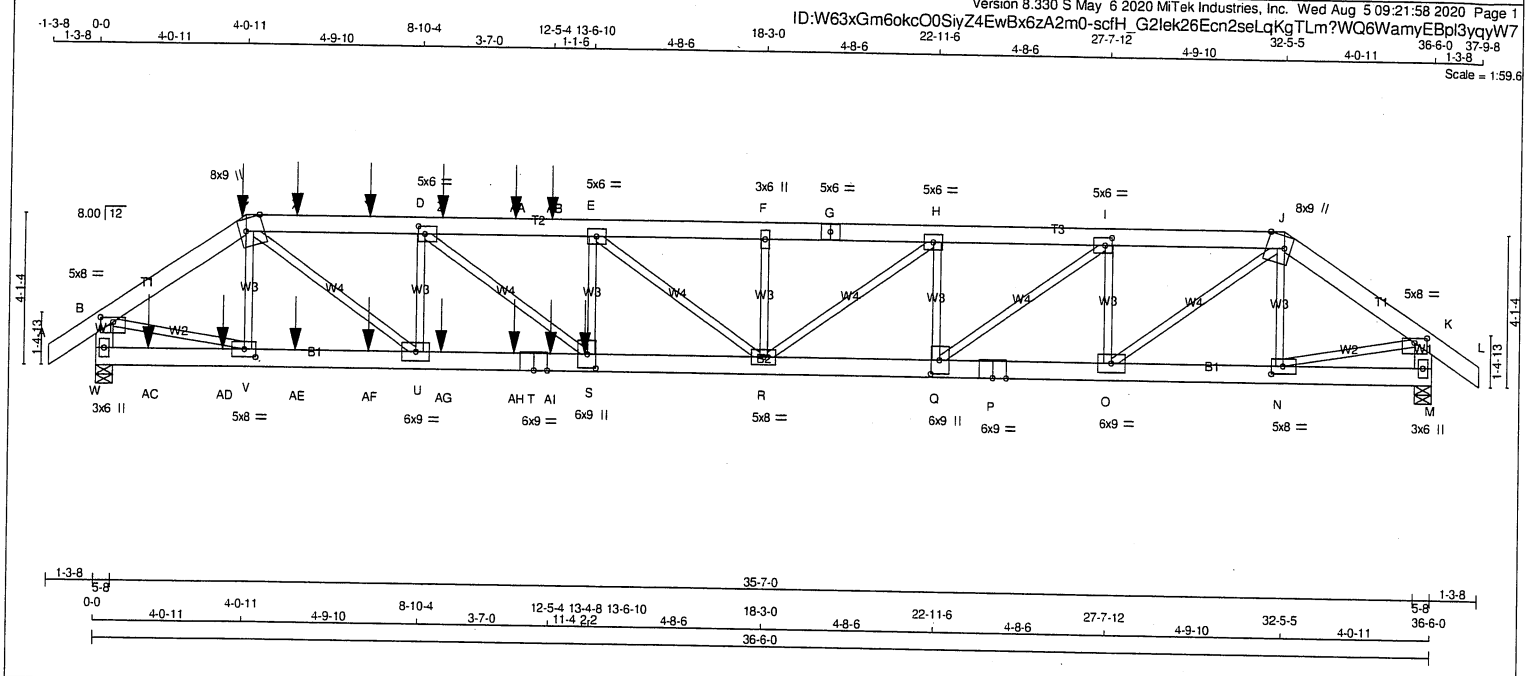
1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2017329

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T1Z	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:21:58 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-scfH_G2Iek26Ecn2seLqKgTLm?WQ6WamyEBpl3yqyW7



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x6	DRY	1650F 1.5E	SPF
T - P	2x6	DRY	1650F 1.5E	SPF
P - M	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- G	2	12	SIDE(61.0)
G- J	2	12	TOP
J- L	2	12	TOP
W- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- T	2	12	SIDE(0.0)
T- P	2	12	SIDE(183.1)
P- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
E- S	1	2	SIDE(785.4)
H- Q	1	2	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2017330 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	5313	0	5313	0	0	5-8	5-8
M	3732	0	3732	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	3749	2509 / 0	0 / 0	0 / 0	0 / 0	1240 / 0	0 / 0
M	2633	1764 / 0	0 / 0	0 / 0	0 / 0	869 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 36	-91.8	-91.8	0.04 (1)	10.00
B- C	-6729 / 0	-91.8	-91.8	0.13 (1)	4.52
C- X	-10775 / 0	-91.8	-91.8	0.33 (1)	3.55
X- Y	-10775 / 0	-91.8	-91.8	0.33 (1)	3.55
Y- D	-10775 / 0	-91.8	-91.8	0.33 (1)	3.55
D- Z	-14592 / 0	-91.8	-91.8	0.54 (1)	2.88
Z-AA	-14592 / 0	-91.8	-91.8	0.54 (1)	2.88
AA-AB	-14592 / 0	-91.8	-91.8	0.54 (1)	2.88
AB- E	-14592 / 0	-91.8	-91.8	0.54 (1)	2.88
E- F	-12983 / 0	-91.8	-91.8	0.40 (1)	3.19
F- G	-12983 / 0	-91.8	-91.8	0.38 (1)	3.22
G- H	-12983 / 0	-91.8	-91.8	0.38 (1)	3.22
H- I	-10593 / 0	-91.8	-91.8	0.27 (1)	3.63
I- J	-7579 / 0	-91.8	-91.8	0.18 (1)	4.28
J- K	-4607 / 0	-91.8	-91.8	0.10 (1)	5.30
K- L	0 / 36	-91.8	-91.8	0.04 (1)	10.00
W- B	-5274 / 0	0.0	0.0	0.19 (1)	6.36
M- K	-3711 / 0	0.0	0.0	0.13 (1)	7.31
W-AC	0 / 0	-18.5	-18.5	0.03 (4)	10.00
AC-AD	0 / 0	-18.5	-18.5	0.03 (4)	10.00
AD- V	0 / 0	-18.5	-18.5	0.03 (4)	10.00
V-AE	0 / 5555	-18.5	-18.5	0.25 (1)	10.00
AE-AF	0 / 5555	-18.5	-18.5	0.25 (1)	10.00
AF- U	0 / 5555	-18.5	-18.5	0.25 (1)	10.00
U-AG	0 / 10775	-18.5	-18.5	0.53 (1)	10.00
AG-AH	0 / 10775	-18.5	-18.5	0.53 (1)	10.00
AH- T	0 / 10775	-18.5	-18.5	0.53 (1)	10.00
T-AI	0 / 10775	-18.5	-18.5	0.53 (1)	10.00
AI- S	0 / 10775	-18.5	-18.5	0.53 (1)	10.00
S- R	0 / 14592	-18.5	-18.5	0.68 (1)	10.00
R- Q	0 / 10593	-18.5	-18.5	0.47 (1)	10.00
Q- P	0 / 7579	-18.5	-18.5	0.34 (1)	10.00
P- O	0 / 7579	-18.5	-18.5	0.34 (1)	10.00
O- N	0 / 3803	-18.5	-18.5	0.17 (1)	10.00
N- M	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-1018 / 0	0.12 (1)
C- U	0 / 6540	0.81 (1)
U- D	-3740 / 0	0.45 (1)
D- S	0 / 4833	0.60 (1)
S- E	0 / 748	0.09 (1)
E- R	-2037 / 0	0.61 (1)
R- F	-459 / 0	0.05 (1)
R- H	0 / 3026	0.37 (1)
Q- H	-2264 / 0	0.27 (1)
Q- I	0 / 3815	0.47 (1)
O- I	-2783 / 0	0.33 (1)
O- J	0 / 4730	0.59 (1)
N- J	-697 / 0	0.08 (1)
B- V	0 / 5712	0.71 (1)
N- K	0 / 3911	0.48 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-51	-57	---	FRONT	VERT	DEAD	---	C1
C	4-0-11	-260	-260	---	FRONT	VERT	SNOW	---	C1
S	13-4-8	-3713	-3713	---	BACK	VERT	TOTAL	---	C1
X	5-5-4	-110	-110	---	BACK	VERT	TOTAL	---	C1

TOTAL WEIGHT = 2 X 194 = 388 lb [M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.33")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 717 (0.61")

CSI: TC=0.54/1.00 (D-E:1) , BC=0.68/1.00 (R-S:1) , WB=0.81/1.00 (C-U:1) , SSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.87 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T1Z	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	4.00
C	TTWW+m	MT20	8.0	9.0	Edge	5.75
D	TMWW-t	MT20	5.0	6.0	2.50	2.25
E	TMWW-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.25
J	TTWW+m	MT20	8.0	9.0	Edge	5.75
K	TMVW-p	MT20	5.0	8.0	1.50	4.00
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	8.0	2.50	3.75
O	BMWW-t	MT20	6.0	9.0		
P	BS-t	MT20	6.0	9.0		
Q	BMWW+t	MT20	6.0	9.0	4.50	2.75
R	BMWWW-t	MT20	5.0	8.0		
S	BMWW+t	MT20	6.0	9.0	4.50	2.75
T	BS-t	MT20	6.0	9.0		
U	BMWW-t	MT20	6.0	9.0		
V	BMWW-t	MT20	5.0	8.0	2.50	3.75
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Y	7-5-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
Z	9-5-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
AA	11-5-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
AB	12-5-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
AC	1-5-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AD	3-5-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AE	5-5-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AF	7-5-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AG	9-5-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AH	11-5-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AI	12-5-4	-26	-26	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

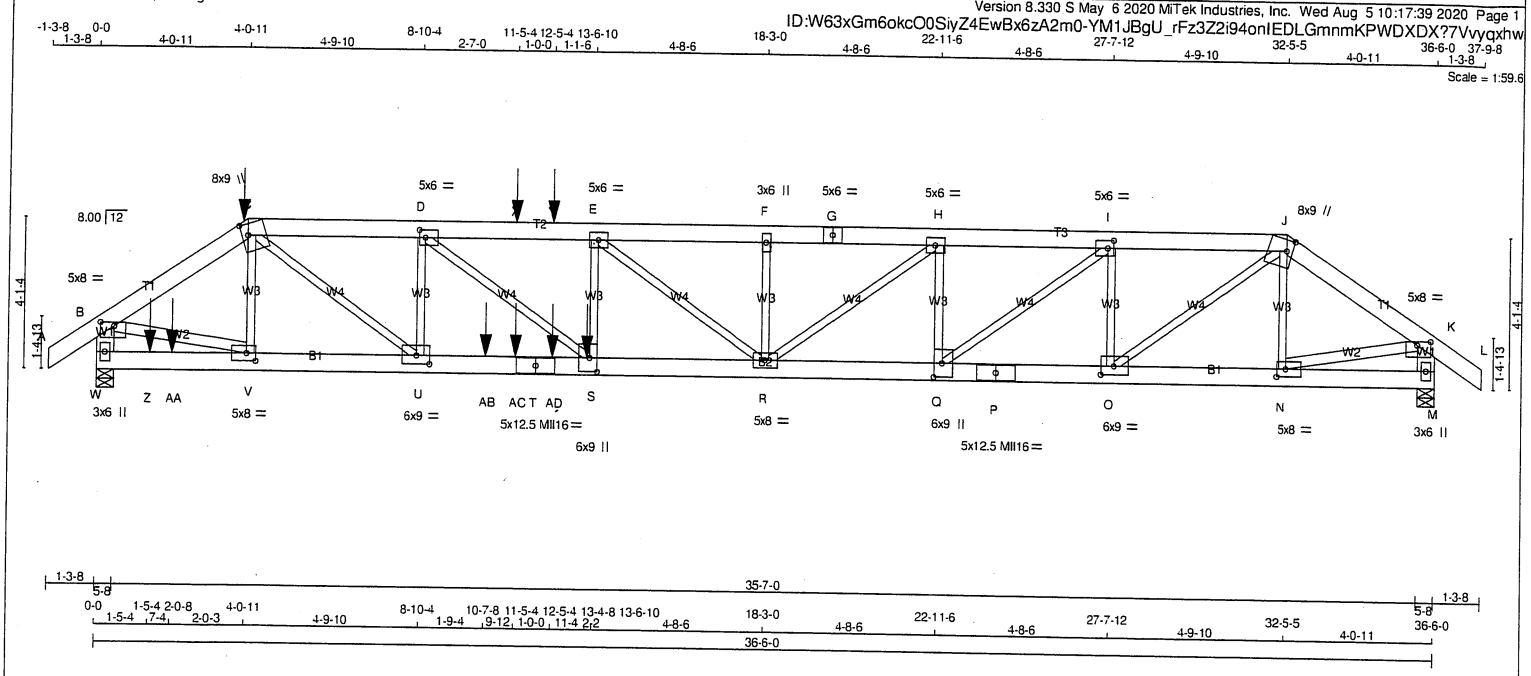
1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2017330 3/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409988	T1Z1	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:17:39 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-YM1JBgU_rFz3Z2i94onIEDLGmnmKPWDXDX?7VvyqxhW



TOTAL WEIGHT = 2 X 196 = 393 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x6	DRY	1650F 1.5E	SPF
T - P	2x6	DRY	1650F 1.5E	SPF
P - M	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - V	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- G	2	12	SIDE(61.0)
G- J	2	12	TOP
J- L	2	12	TOP
W- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- T	2	12	SIDE(0.0)
T- P	2	12	SIDE(183.1)
P- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
E- S	1	2	SIDE(777.6)
H- Q	1	2	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017349 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	6720	0	6720	0	0	5-8	5-8
M	4008	0	4008	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	4748	3140 / 0	0 / 0	0 / 0	0 / 0	1608 / 0	0 / 0
M	2829	1887 / 0	0 / 0	0 / 0	0 / 0	942 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 36	-91.8	-91.8	0.04 (1)	10.00
B- C	-8000 / 0	-91.8	-91.8	0.16 (1)	4.18
C- D	-12438 / 0	-91.8	-91.8	0.36 (1)	3.30
D- X	-16408 / 0	-91.8	-91.8	0.66 (1)	2.60
X- Y	-16408 / 0	-91.8	-91.8	0.66 (1)	2.60
Y- E	-16408 / 0	-91.8	-91.8	0.66 (1)	2.60
E- F	-14335 / 0	-91.8	-91.8	0.48 (1)	2.98
F- G	-14335 / 0	-91.8	-91.8	0.45 (1)	3.00
G- H	-14335 / 0	-91.8	-91.8	0.45 (1)	3.00
H- I	-11617 / 0	-91.8	-91.8	0.31 (1)	3.45
I- J	-8243 / 0	-91.8	-91.8	0.20 (1)	4.11
J- K	-4990 / 0	-91.8	-91.8	0.11 (1)	5.13
K- L	0 / 36	-91.8	-91.8	0.04 (1)	10.00
W- B	-6211 / 0	0.0	0.0	0.22 (1)	5.95
M- K	-3993 / 0	0.0	0.0	0.14 (1)	7.11
W- Z	0 / 0	-18.5	-18.5	0.25 (1)	10.00
Z-AA	0 / 0	-18.5	-18.5	0.25 (1)	10.00
AA- V	0 / 0	-18.5	-18.5	0.25 (1)	10.00
V- U	0 / 6619	-18.5	-18.5	0.37 (1)	10.00
U-AB	0 / 12438	-18.5	-18.5	0.79 (1)	10.00
AB-AC	0 / 12438	-18.5	-18.5	0.79 (1)	10.00
AC- T	0 12438	-18.5	-18.5	0.79 (1)	10.00
T-AD	0 / 12438	-18.5	-18.5	0.79 (1)	10.00
AD- S	0 / 12438	-18.5	-18.5	0.79 (1)	10.00
S- R	0 16408	-18.5	-18.5	0.72 (1)	10.00
R- Q	0 / 11617	-18.5	-18.5	0.52 (1)	10.00
Q- P	0 / 8243	-18.5	-18.5	0.37 (1)	10.00
P- O	0 8243	-18.5	-18.5	0.37 (1)	10.00
O- N	0 / 4119	-18.5	-18.5	0.19 (1)	10.00
N- M	0 / 0	-18.5	-18.5	0.02 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-686 / 0	0.08 (1)
C- U	0 / 7289	0.90 (1)
U- D	-3589 / 0	0.43 (1)
D- S	0 / 5026	0.62 (1)
S- E	0 / 1098	0.14 (1)
E- R	-2624 / 0	0.78 (1)
R- F	-463 / 0	0.06 (1)
R- H	0 / 3441	0.43 (1)
Q- H	-2515 / 0	0.30 (1)
Q- I	0 / 4272	0.53 (1)
O- I	-3061 / 0	0.37 (1)
O- J	0 / 5165	0.64 (1)
N- J	-749 / 0	0.09 (1)
B- V	0 / 6792	0.60 (1)
N- K	0 / 4236	0.37 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-51	-57	---	FRONT	VERT	DEAD	---	C1
C	4-0-11	-260	-260	---	FRONT	VERT	SNOW	---	C1
S	13-4-8	-3683	-3683	---	BACK	VERT	TOTAL	---	C1
X	11-5-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
Y	12-5-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
Z	1-5-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AA	2-0-8	-1075	-1075	---	BACK	VERT	TOTAL	---	C1
AB	10-7-8	-1075	-1075	---	BACK	VERT	TOTAL	---	C1
AC	11-5-4	-26	-26	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.36")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 662 (0.66")

CSI: TC=0.66/1.00 (D-E:1) , BC=0.79/1.00 (S-U:1) , WB=0.90/1.00 (C-U:1) , SSI=0.26/1.00 (S-U:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873
MII16	438	302	2547	1256	4283	1816

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.64 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409988	T1Z1	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	4.25
C	TTWW+m	MT20	8.0	9.0	3.75	2.00
D	TMWW-t	MT20	5.0	6.0	2.50	2.00
E	TMWW-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.00
J	TTWW+m	MT20	8.0	9.0	3.75	2.00
K	TMVW-p	MT20	5.0	8.0	1.25	4.25
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	8.0	2.50	3.00
O	BMWW-t	MT20	6.0	9.0	2.75	4.25
P	BS-t	MII16	5.0	12.5		
Q	BMWW+t	MT20	6.0	9.0	4.50	2.50
R	BMWWW-t	MT20	5.0	8.0		
S	BMWW+t	MT20	6.0	9.0	4.50	2.50
T	BS-t	MII16	5.0	12.5		
U	BMWW-t	MT20	6.0	9.0	2.75	4.25
V	BMWW-t	MT20	5.0	8.0	2.50	3.00
W	BMV1+p	MT20	3.0	6.0		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AD	12-5-4	-26	-26	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

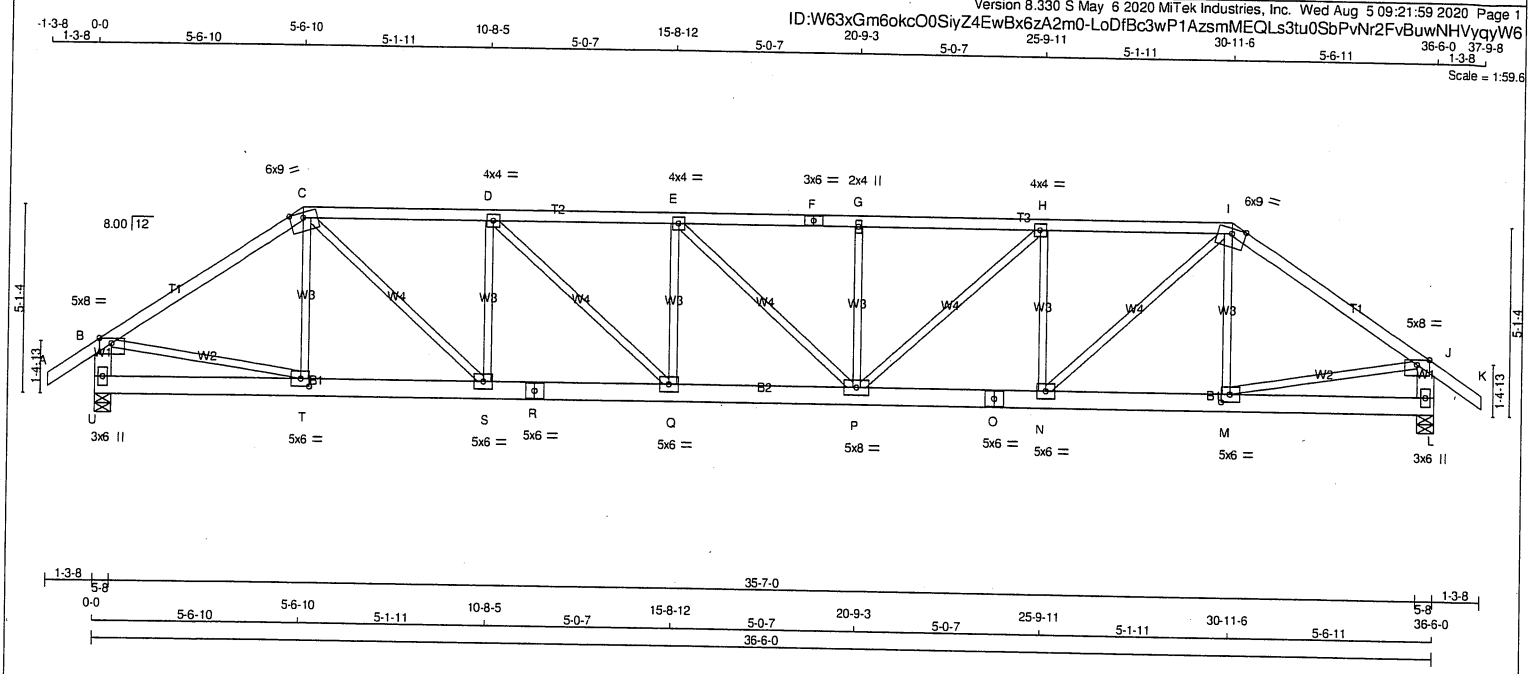
LICENSED PROFESSIONAL ENGINEER
08/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017349 R2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T2	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:21:59 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-LoDfBc3wP1AzsmMEQLs3tu0SbPvNr2FvBuwNHVyqyW6



TOTAL WEIGHT = 2 X 173 = 346 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	6.0	9.0	Edge	
D, E, H						
D	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
I	TTWW-m	MT20	6.0	9.0	Edge	
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N, Q, S						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.75
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2017331

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2138	0	2138	0	0	5-8	5-8
L	2138	0	2138	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0
L	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-2437 / 0	-91.8	-91.8	0.73 (1)	3.67
C- D	-3265 / 0	-91.8	-91.8	0.56 (1)	3.41
D- E	-3843 / 0	-91.8	-91.8	0.63 (1)	3.09
E- F	-3835 / 0	-91.8	-91.8	0.52 (1)	3.23
F- G	-3835 / 0	-91.8	-91.8	0.52 (1)	3.23
G- H	-3835 / 0	-91.8	-91.8	0.63 (1)	3.09
H- I	-3267 / 0	-91.8	-91.8	0.56 (1)	3.41
I- J	-2436 / 0	-91.8	-91.8	0.73 (1)	3.67
J- K	0 / 35	-91.8	-91.8	0.12 (1)	10.00
U- B	-2093 / 0	0.0	0.0	0.14 (1)	7.02
L- J	-2092 / 0	0.0	0.0	0.14 (1)	7.02
U- T	0 / 0	-18.5	-18.5	0.06 (4)	10.00
T- S	0 / 2019	-18.5	-18.5	0.27 (1)	10.00
S- R	0 / 3265	-18.5	-18.5	0.43 (1)	10.00
R- Q	0 / 3265	-18.5	-18.5	0.43 (1)	10.00
Q- P	0 / 3843	-18.5	-18.5	0.51 (1)	10.00
P- O	0 / 3267	-18.5	-18.5	0.44 (1)	10.00
O- N	0 / 3267	-18.5	-18.5	0.44 (1)	10.00
N- M	0 / 2019	-18.5	-18.5	0.27 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.06 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-241 / 4	0.09 (1)
C- S	0 / 1690	0.38 (1)
S- D	-1058 / 0	0.38 (1)
D- Q	0 / 792	0.18 (1)
Q- E	-444 / 0	0.16 (1)
E- P	-11 / 0	0.01 (1)
P- G	-446 / 0	0.16 (1)
P- H	0 / 779	0.18 (1)
N- H	-1051 / 0	0.38 (1)
N- I	0 / 1693	0.38 (1)
M- I	-243 / 4	0.09 (1)
B- T	0 / 2056	0.46 (1)
M- J	0 / 2056	0.46 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.40")

CSI: TC=0.73/1.00 (B-C:1) , BC=0.51/1.00 (P-Q:1) , WB=0.46/1.00 (B-T:1) , SSI=0.22/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

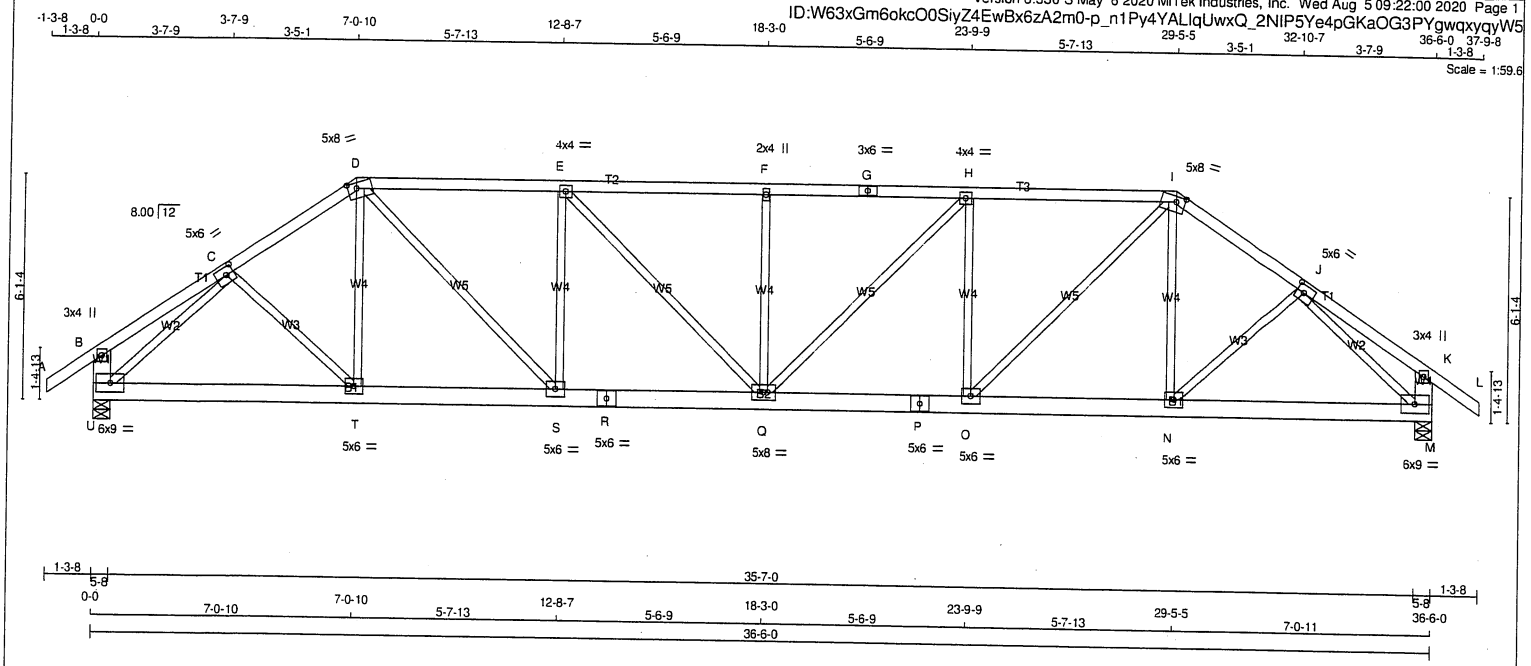
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (T) (INPUT = 0.90)
JSI METAL= 0.60 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T3	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:22:00 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-p_n1Py4YALlqUwxQ_2NIP5Ye4pGKaOG3PYgwqxyqyW5



TOTAL WEIGHT = 2 X 178 = 357 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW-m	MT20	5.0	8.0	1.75	3.00
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	8.0	1.75	3.00
J	TMWW-t	MT20	5.0	6.0	2.50	2.50
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	6.0	9.0		
N, O, S, T						
N	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
U	BMVW1-t	MT20	6.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2138	0	2138	0	0	5-8	5-8
M	2138	0	2138	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0
M	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 18	-91.8	-91.8	0.16 (1)	10.00
C- D	-2445 / 0	-91.8	-91.8	0.24 (1)	4.19
D- E	-2963 / 0	-91.8	-91.8	0.65 (1)	3.45
E- F	-3234 / 0	-91.8	-91.8	0.68 (1)	3.28
F- G	-3234 / 0	-91.8	-91.8	0.68 (1)	3.28
G- H	-3234 / 0	-91.8	-91.8	0.68 (1)	3.28
H- I	-2963 / 0	-91.8	-91.8	0.65 (1)	3.45
I- J	-2445 / 0	-91.8	-91.8	0.24 (1)	4.19
J- K	0 / 18	-91.8	-91.8	0.16 (1)	10.00
K- L	0 / 35	-91.8	-91.8	0.12 (1)	10.00
U- B	-255 / 0	0.0	0.0	0.02 (1)	7.81
M- K	-255 / 0	0.0	0.0	0.02 (1)	7.81
U- T	0 / 1922	-18.5	-18.5	0.29 (1)	10.00
T- S	0 / 2016	-18.5	-18.5	0.28 (1)	10.00
S- R	0 / 2963	-18.5	-18.5	0.40 (1)	10.00
R- Q	0 / 2963	-18.5	-18.5	0.40 (1)	10.00
Q- P	0 / 2963	-18.5	-18.5	0.40 (1)	10.00
P- O	0 / 2963	-18.5	-18.5	0.40 (1)	10.00
O- N	0 / 2016	-18.5	-18.5	0.28 (1)	10.00
N- M	0 / 1922	-18.5	-18.5	0.29 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	0 / 126	0.03 (4)
T- D	0 / 93	0.03 (4)
D- S	0 / 1347	0.30 (1)
S- E	-859 / 0	0.47 (1)
E- Q	0 / 389	0.09 (1)
Q- F	-468 / 0	0.26 (1)
Q- H	0 / 389	0.09 (1)
O- H	-859 / 0	0.47 (1)
O- I	0 / 1347	0.30 (1)
N- I	0 / 93	0.03 (4)
N- J	0 / 126	0.03 (4)
U- C	-2651 / 0	0.93 (1)
J- M	-2651 / 0	0.93 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.68/1.00 (F-H:1) , BC=0.40/1.00 (O-Q:1) , WB=0.93/1.00 (J-M:1) , SSI=0.24/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (D) (INPUT = 0.90)
JSI METAL= 0.60 (C) (INPUT = 1.00)

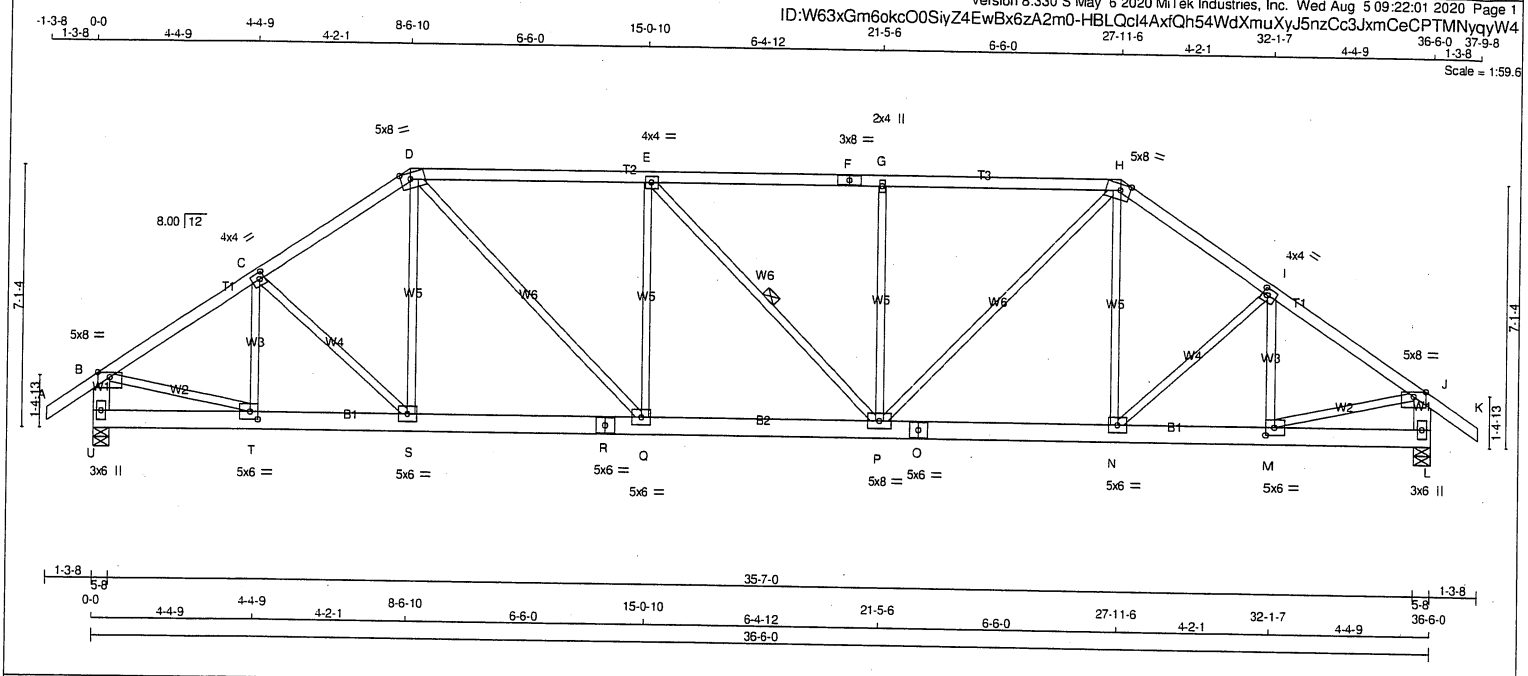
LICENSED PROFESSIONAL ENGINEER
09/05/10
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017332

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T4	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:22:01 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-HBLQcl4AxfQh54WdXmuXyJ5nzCc3JxmCeCPTMNyqyW4



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	8.0	2.00	3.25
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.00	3.25
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N, Q, S						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.50
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2017333

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2138	0	2138	0	0	5-8	5-8
L	2138	0	2138	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0
L	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-2399 / 0	-91.8	-91.8	0.38 (1)	4.09
C- D	-2398 / 0	-91.8	-91.8	0.37 (1)	4.09
D- E	-2706 / 0	-91.8	-91.8	0.80 (1)	3.42
E- F	-2701 / 0	-91.8	-91.8	0.79 (1)	3.42
F- G	-2701 / 0	-91.8	-91.8	0.79 (1)	3.42
G- H	-2701 / 0	-91.8	-91.8	0.79 (1)	3.44
H- I	-2399 / 0	-91.8	-91.8	0.37 (1)	4.09
I- J	-2398 / 0	-91.8	-91.8	0.38 (1)	4.09
J- K	0 / 35	-91.8	-91.8	0.12 (1)	10.00
U- B	-2086 / 0	0.0	0.0	0.14 (1)	7.03
L- J	-2086 / 0	0.0	0.0	0.14 (1)	7.03
U- T	0 / 0	-18.5	-18.5	0.05 (1)	10.00
T- S	0 / 2016	-18.5	-18.5	0.28 (1)	10.00
S- R	0 / 1973	-18.5	-18.5	0.28 (1)	10.00
R- Q	0 / 1973	-18.5	-18.5	0.28 (1)	10.00
Q- P	0 / 2706	-18.5	-18.5	0.37 (1)	10.00
P- O	0 / 1974	-18.5	-18.5	0.28 (1)	10.00
O- N	0 / 1974	-18.5	-18.5	0.28 (1)	10.00
N- M	0 / 2016	-18.5	-18.5	0.29 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.05 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-417 / 0	0.11 (1)
C- S	-62 / 0	0.03 (1)
S- D	0 / 171	0.04 (4)
D- Q	0 / 1053	0.24 (1)
Q- E	-639 / 0	0.52 (1)
E- P	-7 / 0	0.00 (1)
P- G	-641 / 0	0.53 (1)
P- H	0 / 1045	0.24 (1)
N- H	0 / 177	0.05 (4)
N- I	-59 / 0	0.03 (1)
M- I	-419 / 0	0.11 (1)
B- T	0 / 2068	0.47 (1)
M- J	0 / 2067	0.47 (1)

TOTAL WEIGHT = 2 X 182 = 363 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.80/1.00 (D-E:1) , BC=0.37/1.00 (P-Q:1) , WB=0.53/1.00 (G-P:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

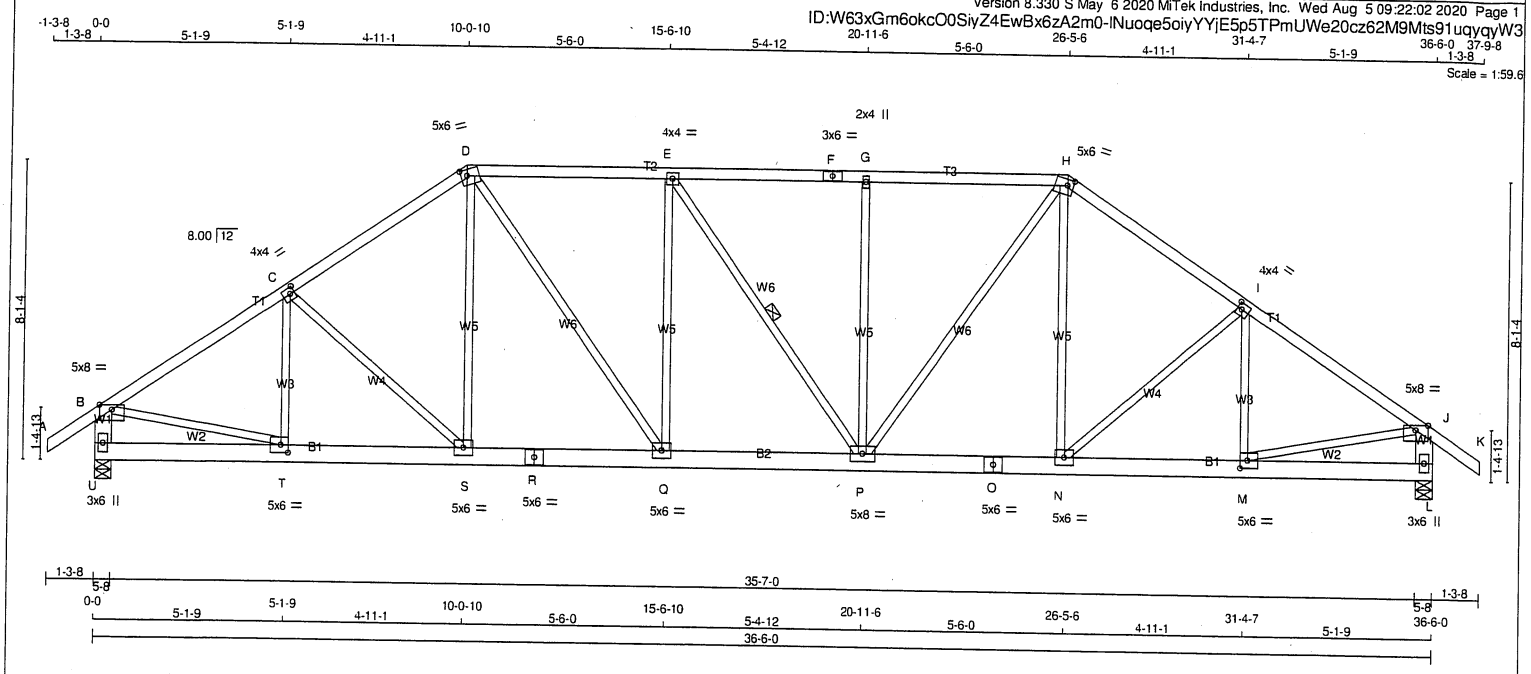
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (T) (INPUT = 0.90)
JSI METAL= 0.46 (T) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T5	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:22:02 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-INuoqe5oiyYYjE5p5TPmUWe20cz62M9Mts91uqyqyW3



TOTAL WEIGHT = 2 X 189 = 379 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.00	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.00	2.00
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N, Q, S						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.50
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2017334

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2138	0	2138	0	0	5-8	5-8
L	2138	0	2138	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0
L	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-2451 / 0	-91.8	-91.8	0.40 (1)	4.05
C- D	-2322 / 0	-91.8	-91.8	0.38 (1)	4.16
D- E	-2362 / 0	-91.8	-91.8	0.39 (1)	4.10
E- F	-2357 / 0	-91.8	-91.8	0.37 (1)	4.14
F- G	-2357 / 0	-91.8	-91.8	0.37 (1)	4.14
G- H	-2357 / 0	-91.8	-91.8	0.39 (1)	4.11
H- I	-2323 / 0	-91.8	-91.8	0.38 (1)	4.16
I- J	-2451 / 0	-91.8	-91.8	0.40 (1)	4.05
J- K	0 / 35	-91.8	-91.8	0.12 (1)	10.00
U- B	-2085 / 0	0.0	0.0	0.14 (1)	7.03
L- J	-2085 / 0	0.0	0.0	0.14 (1)	7.03
U- T	0 / 0	-18.5	-18.5	0.06 (4)	10.00
T- S	0 / 2064	-18.5	-18.5	0.28 (1)	10.00
S- R	0 / 1907	-18.5	-18.5	0.26 (1)	10.00
R- Q	0 / 1907	-18.5	-18.5	0.26 (1)	10.00
Q- P	0 / 2362	-18.5	-18.5	0.32 (1)	10.00
P- O	0 / 1909	-18.5	-18.5	0.26 (1)	10.00
O- N	0 / 1909	-18.5	-18.5	0.26 (1)	10.00
N- M	0 / 2063	-18.5	-18.5	0.28 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.06 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-328 / 0	0.11 (1)
C- S	-213 / 0	0.17 (1)
S- D	0 / 257	0.06 (1)
D- Q	0 / 783	0.18 (1)
Q- E	-536 / 0	0.64 (1)
E- P	-8 / 0	0.01 (1)
P- G	-539 / 0	0.64 (1)
P- H	0 / 773	0.17 (1)
N- H	0 / 265	0.06 (1)
N- I	-211 / 0	0.17 (1)
M- I	-330 / 0	0.11 (1)
B- T	0 / 2102	0.47 (1)
M- J	0 / 2102	0.47 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.40/1.00 (B-C:1) , BC=0.32/1.00 (P-Q:1) , WB=0.64/1.00 (G-P:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

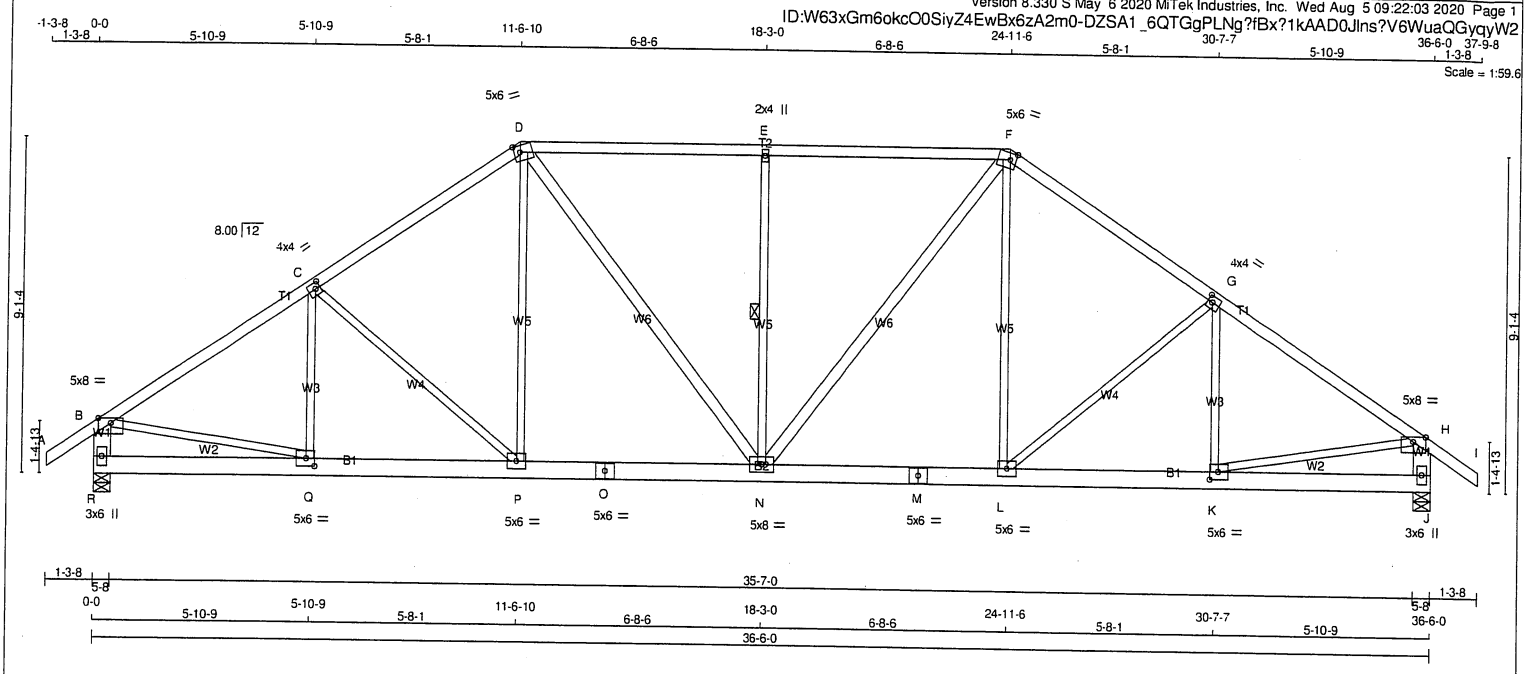
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (T) (INPUT = 0.90)
JSI METAL= 0.47 (T) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T6	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:22:03 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-DZSA1_6QTGgPLNg?fBx?1kAAD0JIns?V6WuaQGyqyW2



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BS-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.75
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2017335

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2138	0	2138	0	0	5-8	5-8
J	2138	0	2138	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0
J	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-2479 / 0	-91.8	-91.8	0.53 (1)	3.89
C- D	-2234 / 0	-91.8	-91.8	0.49 (1)	4.10
D- E	-2165 / 0	-91.8	-91.8	0.62 (1)	3.91
E- F	-2165 / 0	-91.8	-91.8	0.62 (1)	3.91
F- G	-2234 / 0	-91.8	-91.8	0.49 (1)	4.10
G- H	-2479 / 0	-91.8	-91.8	0.53 (1)	3.89
H- I	0 / 35	-91.8	-91.8	0.12 (1)	10.00
R- B	-2082 / 0	0.0	0.0	0.14 (1)	7.03
J- H	-2082 / 0	0.0	0.0	0.14 (1)	7.03
R- Q	0 / 0	-18.5	-18.5	0.07 (4)	10.00
Q- P	0 / 2091	-18.5	-18.5	0.29 (1)	10.00
P- O	0 / 1831	-18.5	-18.5	0.26 (1)	10.00
O- N	0 / 1831	-18.5	-18.5	0.26 (1)	10.00
N- M	0 / 1831	-18.5	-18.5	0.26 (1)	10.00
M- L	0 / 1831	-18.5	-18.5	0.26 (1)	10.00
L- K	0 / 2091	-18.5	-18.5	0.29 (1)	10.00
K- J	0 / 0	-18.5	-18.5	0.07 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-261 / 6	0.10 (1)
C- P	-347 / 0	0.39 (1)
P- D	0 / 355	0.08 (1)
D- N	0 / 549	0.09 (1)
N- E	-754 / 0	0.39 (1)
N- F	0 / 549	0.09 (1)
L- F	0 / 355	0.08 (1)
L- G	-347 / 0	0.39 (1)
K- G	-261 / 6	0.10 (1)
B- Q	0 / 2121	0.48 (1)
K- H	0 / 2121	0.48 (1)

TOTAL WEIGHT = 2 X 192 = 384 lb
[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.62/1.00 (D-E:1) , BC=0.29/1.00 (K-L:1) , WB=0.48/1.00 (H-K:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

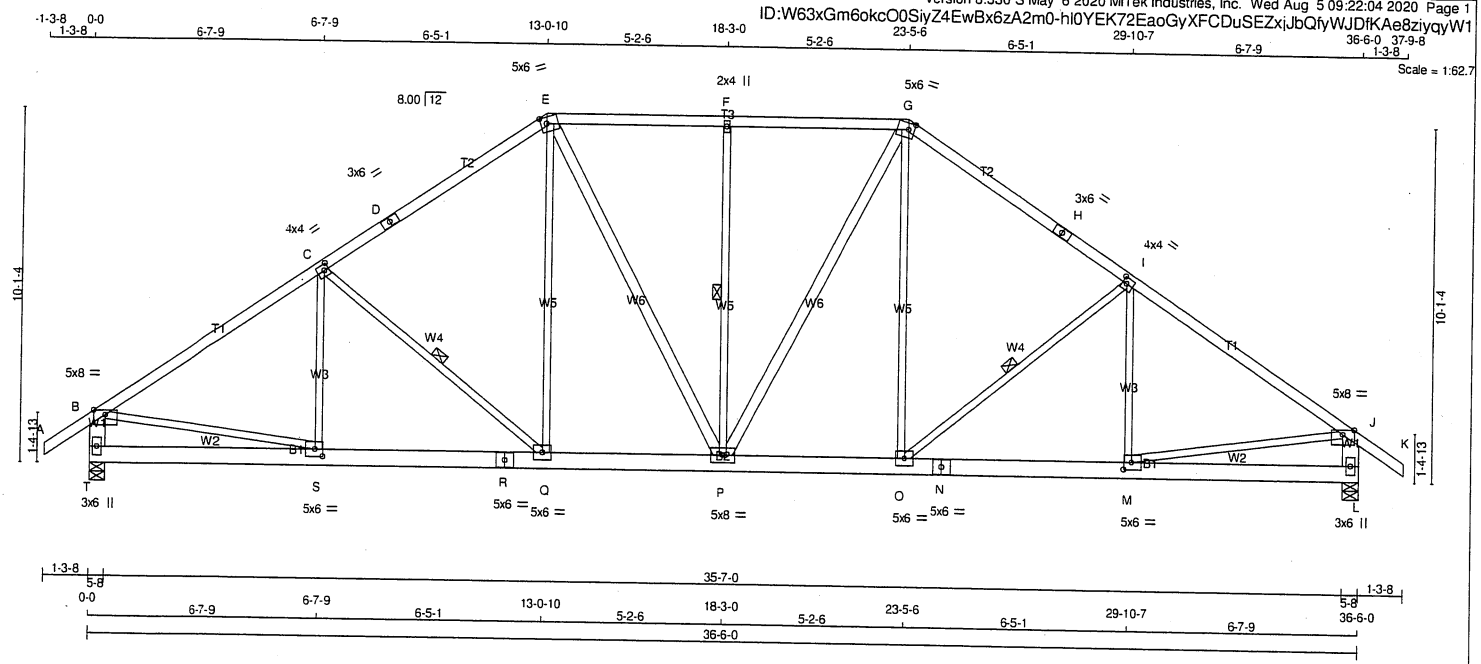
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (Q) (INPUT = 0.90)
JSI METAL= 0.48 (Q) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T7	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:22:04 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-hl0YEK72EaoGyXFCDuSEZxjJbQfyWJDfKAe8ziyqyW1



LUMBER

N. L. G. A. RULES

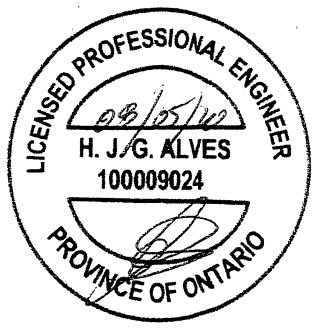
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BS-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.75
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



Structural component only
DWG# T-2017336

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 3 X 199 = 596 lb

[M][F]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	2138	0	2138	0	0	5-8	5-8
L	2138	0	2138	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0
L	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-Q, F-P, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-2492 / 0	-91.8	-91.8	0.71 (1)	3.66
C- D	-2132 / 0	-91.8	-91.8	0.64 (1)	3.98
D- E	-2132 / 0	-91.8	-91.8	0.64 (1)	3.98
E- F	-1920 / 0	-91.8	-91.8	0.36 (1)	4.49
F- G	-1920 / 0	-91.8	-91.8	0.36 (1)	4.49
G- H	-2132 / 0	-91.8	-91.8	0.64 (1)	3.98
H- I	-2132 / 0	-91.8	-91.8	0.64 (1)	3.98
I- J	-2492 / 0	-91.8	-91.8	0.71 (1)	3.66
J- K	0 / 35	-91.8	-91.8	0.12 (1)	10.00
T- B	-2080 / 0	0.0	0.0	0.14 (1)	7.03
L- J	-2080 / 0	0.0	0.0	0.14 (1)	7.03
T- S	0 / 0	-18.5	-18.5	0.08 (4)	10.00
S- R	0 / 2107	-18.5	-18.5	0.29 (1)	10.00
R- Q	0 / 2107	-18.5	-18.5	0.29 (1)	10.00
Q- P	0 / 1743	-18.5	-18.5	0.24 (1)	10.00
P- O	0 / 1743	-18.5	-18.5	0.24 (1)	10.00
O- N	0 / 2107	-18.5	-18.5	0.29 (1)	10.00
N- M	0 / 2107	-18.5	-18.5	0.29 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.08 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-197 / 43	0.10 (1)
C- Q	-479 / 0	0.23 (1)
Q- E	0 / 425	0.10 (1)
E- P	0 / 375	0.06 (1)
P- F	-580 / 0	0.39 (1)
P- G	0 / 375	0.06 (1)
O- G	0 / 425	0.10 (1)
O- I	-479 / 0	0.23 (1)
M- I	-197 / 43	0.10 (1)
B- S	0 / 2131	0.48 (1)
M- J	0 / 2131	0.48 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.71/1.00 (I-J:1) , BC=0.29/1.00 (Q-S:1) , WB=0.48/1.00 (B-S:1) , SSI=0.24/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

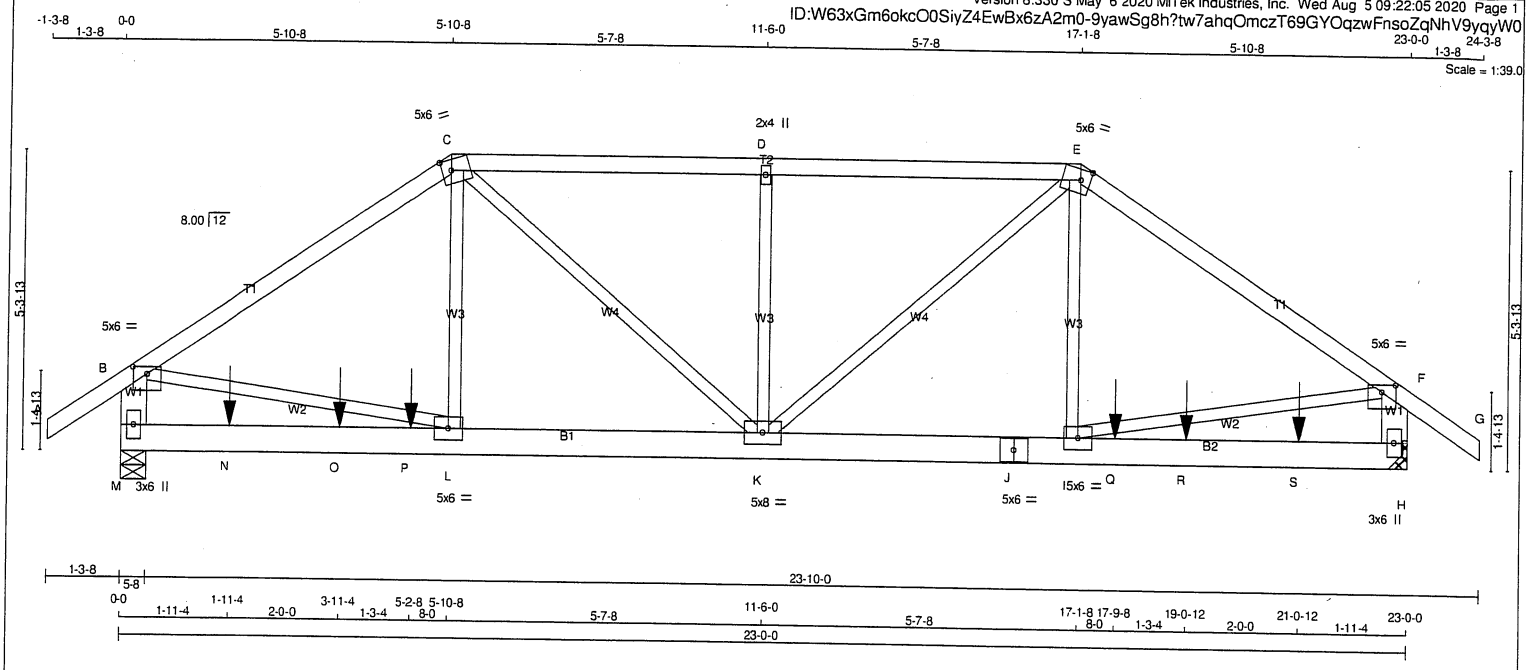
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (M) (INPUT = 0.90)
JSI METAL= 0.48 (S) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T8	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:22:05 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-9yawSg8h?tw7ahqOmczT69GYOqzwFnsoZqNhV9yqyW0



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- G	1	12	TOP
M- B	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	SIDE(0.0)
J- H	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

Structural component only
DWG# T-2017337 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	3205	0	3205	0	0	5-8	5-8
H	3205	0	3205	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	2261	1513 / 0	0 / 0	0 / 0	0 / 0	748 / 0	0 / 0
H	2261	1513 / 0	0 / 0	0 / 0	0 / 0	748 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35		-91.8	-91.8	0.07 (1)	10.00
B- C	-3741 / 0		-91.8	-91.8	0.45 (1)	4.47
C- D	-3361 / 0		-91.8	-91.8	0.34 (1)	4.74
D- E	-3361 / 0		-91.8	-91.8	0.34 (1)	4.74
E- F	-3741 / 0		-91.8	-91.8	0.45 (1)	4.47
F- G	0 / 35		-91.8	-91.8	0.07 (1)	10.00
M- B	-2994 / 0		0.0	0.0	0.11 (1)	7.81
H- F	-2994 / 0		0.0	0.0	0.11 (1)	7.81
M- N	0 / 0		-18.5	-18.5	0.25 (1)	10.00
N- O	0 / 0		-18.5	-18.5	0.25 (1)	10.00
O- P	0 / 0		-18.5	-18.5	0.25 (1)	10.00
P- L	0 / 0		-18.5	-18.5	0.25 (1)	10.00
L- K	0 / 3139		-18.5	-18.5	0.44 (1)	10.00
K- J	0 / 3139		-18.5	-18.5	0.44 (1)	10.00
J- I	0 / 3139		-18.5	-18.5	0.44 (1)	10.00
I- Q	0 / 0		-18.5	-18.5	0.25 (1)	10.00
Q- R	0 / 0		-18.5	-18.5	0.25 (1)	10.00
R- S	0 / 0		-18.5	-18.5	0.25 (1)	10.00
S- H	0 / 0		-18.5	-18.5	0.25 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	0 / 1411	0.17 (1)
C- K	0 / 295	0.04 (1)
K- D	-620 / 0	0.13 (1)
K- E	0 / 295	0.04 (1)
I- E	0 / 1411	0.17 (1)
B- L	0 / 3152	0.39 (1)
I- F	0 / 3152	0.39 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	1-11-4	-36	-36	---	FRONT	VERT	TOTAL	---	C1
O	3-11-4	-36	-36	---	FRONT	VERT	TOTAL	---	C1
P	5-2-8	-1739	-1739	---	FRONT	VERT	TOTAL	---	C1
Q	17-9-8	-1739	-1739	---	FRONT	VERT	TOTAL	---	C1
R	19-0-12	-36	-36	---	FRONT	VERT	TOTAL	---	C1
S	21-0-12	-36	-36	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 109 = 218 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.45/1.00 (B-C:1) , BC=0.44/1.00 (K-L:1) , WB=0.39/1.00 (F-I:1) , SSI=0.57/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (B) (INPUT = 0.90)
JSI METAL= 0.46 (J) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
409707	T8	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMVW-p	MT20	5.0	6.0	1.50	3.00
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

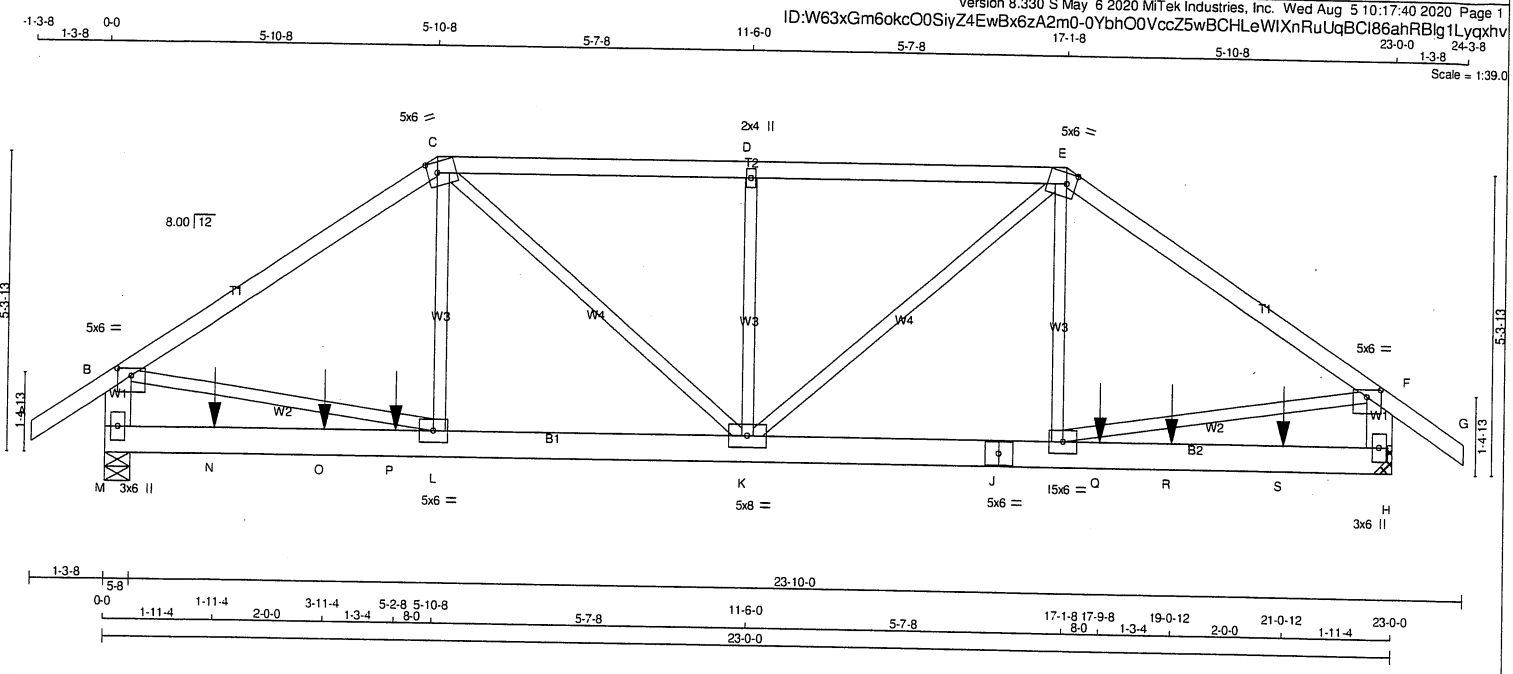
LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017337

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409988	T8Z	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:17:40 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-0YbhO0VccZ5wBCHLeWIXnRuUqBCI86ahRBlg1Lyqxhv



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- G	1	12	TOP
M- B	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	SIDE(0.0)
J- H	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

LICENSED PROFESSIONAL ENGINEER
08/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017350 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	3159	0	3159	0	0	5-8	5-8
H	3159	0	3159	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	2229	1491 / 0	0 / 0	0 / 0	0 / 0	738 / 0	0 / 0
H	2229	1491 / 0	0 / 0	0 / 0	0 / 0	738 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	L- C	0 / 1373	0.17 (1)
B- C	-3680 / 0	-91.8	-91.8	0.45 (1)	4.50	C- K	0 / 301	0.04 (1)
C- D	-3314 / 0	-91.8	-91.8	0.33 (1)	4.77	K- D	-621 / 0	0.13 (1)
D- E	-3314 / 0	-91.8	-91.8	0.33 (1)	4.77	K- E	0 / 301	0.04 (1)
E- F	-3680 / 0	-91.8	-91.8	0.45 (1)	4.50	I- E	0 / 1373	0.17 (1)
F- G	0 / 35	-91.8	-91.8	0.07 (1)	10.00	B- L	0 / 3100	0.38 (1)
M- B	-2952 / 0	0.0	0.0	0.11 (1)	7.81	I- F	0 / 3100	0.38 (1)
H- F	-2952 / 0	0.0	0.0	0.11 (1)	7.81			
M- N	0 / 0	-18.5	-18.5	0.24 (1)	10.00			
N- O	0 / 0	-18.5	-18.5	0.24 (1)	10.00			
O- P	0 / 0	-18.5	-18.5	0.24 (1)	10.00			
P- L	0 / 0	-18.5	-18.5	0.24 (1)	10.00			
L- K	0 / 3087	-18.5	-18.5	0.43 (1)	10.00			
K- J	0 / 3087	-18.5	-18.5	0.43 (1)	10.00			
J- I	0 / 3087	-18.5	-18.5	0.43 (1)	10.00			
I- Q	0 / 0	-18.5	-18.5	0.24 (1)	10.00			
Q- R	0 / 0	-18.5	-18.5	0.24 (1)	10.00			
R- S	0 / 0	-18.5	-18.5	0.24 (1)	10.00			
S- H	0 / 0	-18.5	-18.5	0.24 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	1-11-4	-36	-36	---	FRONT	VERT	TOTAL	---	C1
O	3-11-4	-36	-36	---	FRONT	VERT	TOTAL	---	C1
P	5-2-8	-1693	-1693	---	FRONT	VERT	TOTAL	---	C1
Q	17-9-8	-1693	-1693	---	FRONT	VERT	TOTAL	---	C1
R	19-0-12	-36	-36	---	FRONT	VERT	TOTAL	---	C1
S	21-0-12	-36	-36	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 109 = 218 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.45/1.00 (B-C:1) , BC=0.43/1.00 (K-L:1) , WB=0.38/1.00 (F-I:1) , SSI=0.55/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (B) (INPUT = 0.90)
JSI METAL= 0.45 (J) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
409988	T8Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMVW-p	MT20	5.0	6.0	1.50	3.00
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

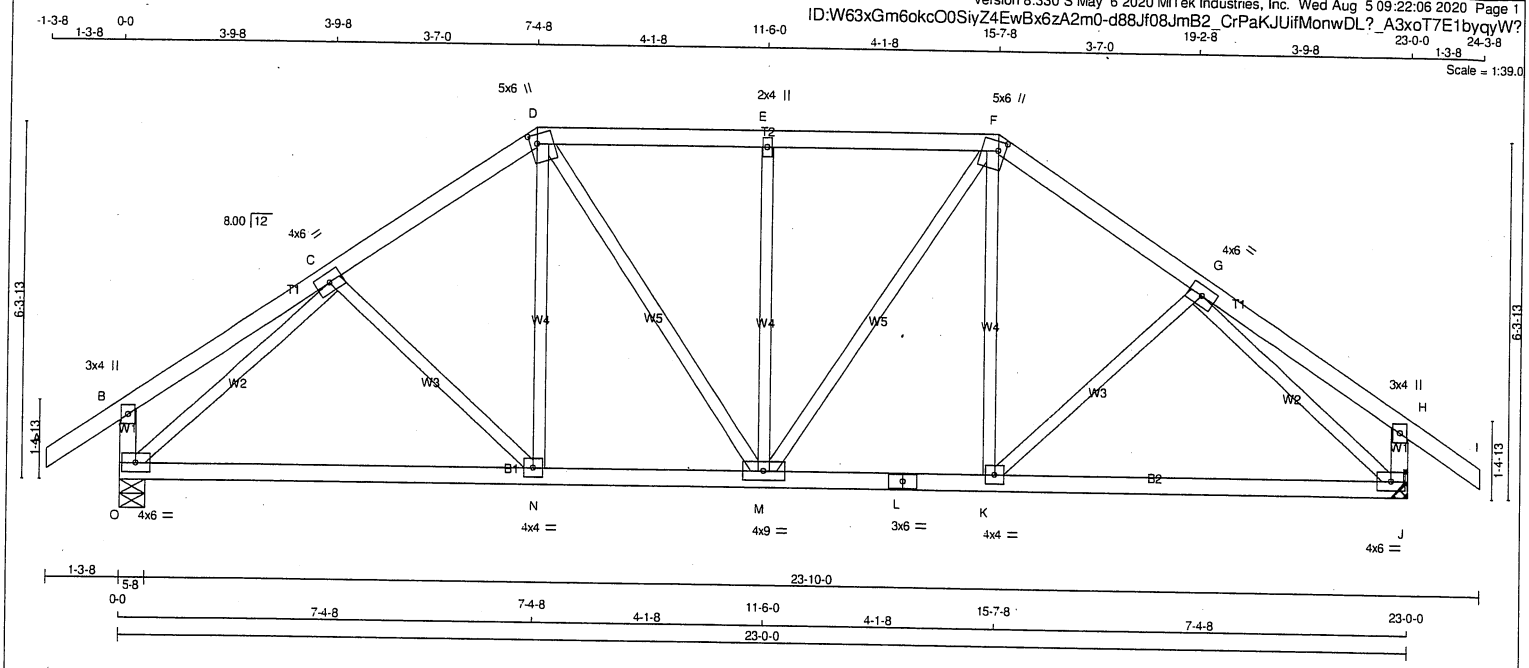
LICENSED PROFESSIONAL ENGINEER
08/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017350

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T9	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:22:06 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-d88Jf08JmB2_CrPaKJUifMonwDL?_A3xoT7E1byqyW?



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	1.50
G	TMWW-t	MT20	4.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	4.0	6.0		

Structural component only
DWG# T-2017338

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1394	0	1394	0	0	5-8	5-8
J	1394	0	1394	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	983	659 / 0	0 / 0	0 / 0	0 / 0	325 / 0	0 / 0
J	983	659 / 0	0 / 0	0 / 0	0 / 0	325 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 21	-91.8	-91.8	0.19 (1)	10.00
C- D	-1308 / 0	-91.8	-91.8	0.16 (1)	5.49
D- E	-1237 / 0	-91.8	-91.8	0.21 (1)	5.55
E- F	-1237 / 0	-91.8	-91.8	0.21 (1)	5.55
F- G	-1308 / 0	-91.8	-91.8	0.16 (1)	5.49
G- H	0 / 21	-91.8	-91.8	0.19 (1)	10.00
H- I	0 / 35	-91.8	-91.8	0.12 (1)	10.00
O- B	-258 / 0	0.0	0.0	0.03 (1)	7.81
J- H	-258 / 0	0.0	0.0	0.03 (1)	7.81
O- N	0 / 1124	-18.5	-18.5	0.31 (4)	10.00
N- M	0 / 1073	-18.5	-18.5	0.32 (4)	10.00
M- L	0 / 1073	-18.5	-18.5	0.32 (4)	10.00
L- K	0 / 1073	-18.5	-18.5	0.32 (4)	10.00
K- J	0 / 1124	-18.5	-18.5	0.31 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	-74 / 25	0.03 (1)
N- D	0 / 195	0.05 (4)
D- M	0 / 291	0.07 (1)
M- E	-458 / 0	0.29 (1)
M- F	0 / 291	0.07 (1)
K- F	0 / 195	0.05 (4)
K- G	-74 / 25	0.03 (1)
O- C	-1558 / 0	0.65 (1)
G- J	-1558 / 0	0.65 (1)

TOTAL WEIGHT = 103 lb
[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.21/1.00 (D-E:1) , BC=0.32/1.00 (M-N:4) , WB=0.65/1.00 (C-O:1) , SSI=0.18/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

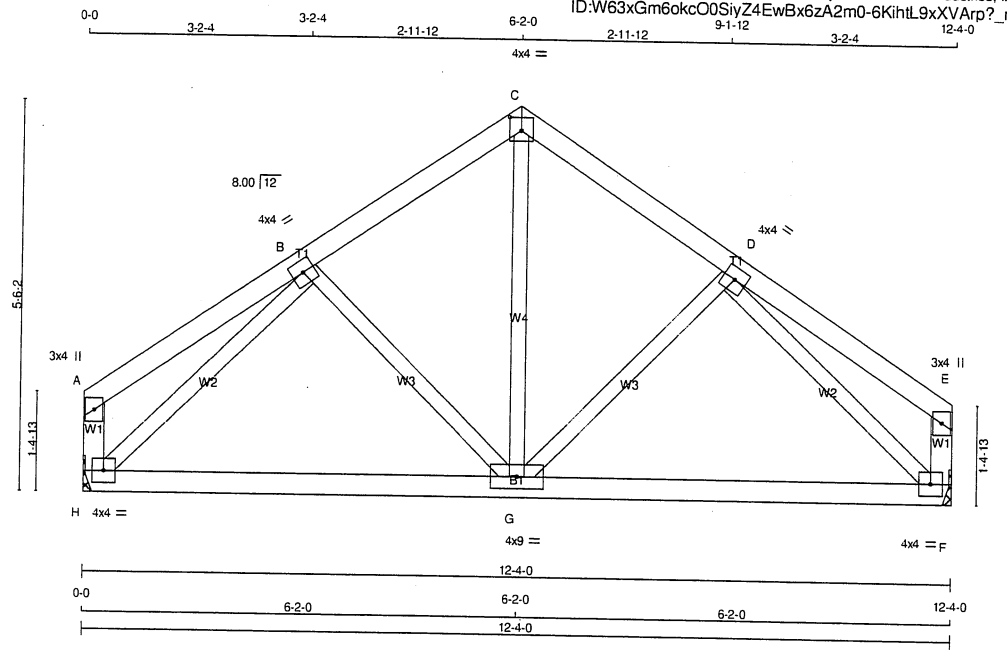
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (G) (INPUT = 0.90)
JSI METAL= 0.34 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T10	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:22:07 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-6KihtL9xXVArp?_nu0?xBaLzrdiTjkw507soa1yqyW_



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	680	0	680	0	0	MECHANICAL	
F	680	0	680	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, F. MINIMUM BEARING LENGTH AT JOINT H = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	481	316 / 0	0 / 0	0 / 0	0 / 0	165 / 0	0 / 0
F	481	316 / 0	0 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-91.8	-91.8	0.13 (1)	10.00
B- C	-521 / 0	-91.8	-91.8	0.10 (1)	6.25
C- D	-521 / 0	-91.8	-91.8	0.10 (1)	6.25
D- E	0 / 17	-91.8	-91.8	0.13 (1)	10.00
H- A	-110 / 0	0.0	0.0	0.01 (1)	7.81
F- E	-110 / 0	0.0	0.0	0.01 (1)	7.81
H- G	0 / 517	-18.5	-18.5	0.24 (4)	10.00
G- F	0 / 517	-18.5	-18.5	0.24 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 337	0.08 (1)
G- D	-139 / 0	0.04 (1)
B- G	-139 / 0	0.04 (1)
H- B	-737 / 0	0.23 (1)
D- F	-737 / 0	0.23 (1)

TOTAL WEIGHT = 3 X 51 = 152 lb
[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.24/1.00 (F-G:4) , WB=0.23/1.00 (D-F:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (D) (INPUT = 0.90)
JSI METAL= 0.24 (D) (INPUT = 1.00)

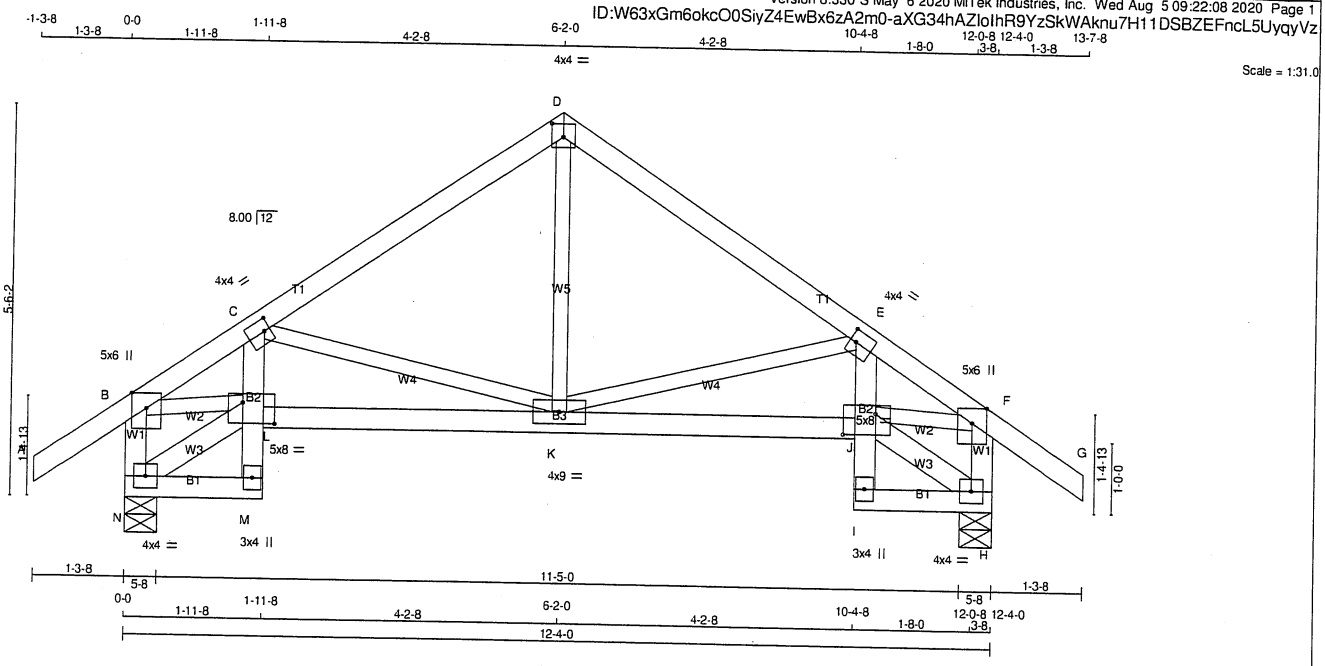
LICENSED PROFESSIONAL ENGINEER
08/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017339

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T10S	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:22:08 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-aXG34hAZloIhR9YzSkWAknu7H11DSBZEFncL5UyqyVz



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-t	MT20	4.0	4.0	2.00	1.00
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-l	MT20	5.0	8.0	3.50	5.50
K	BMWWW-t	MT20	4.0	9.0		
L	BVMWW-l	MT20	5.0	8.0	3.50	5.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 3 X 58 = 174 lb [M][F]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	806	0	806	0	0	5-8	5-8
H	806	0	806	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	568	386 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0
H	568	386 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-1063 / 0	-91.8	-91.8	0.14 (1)	5.96
C- D	-604 / 0	-91.8	-91.8	0.22 (1)	6.25
D- E	-604 / 0	-91.8	-91.8	0.22 (1)	6.25
E- F	-1063 / 0	-91.8	-91.8	0.14 (1)	5.96
F- G	0 / 35	-91.8	-91.8	0.12 (1)	10.00
N- B	-765 / 0	0.0	0.0	0.08 (1)	7.81
H- F	-765 / 0	0.0	0.0	0.08 (1)	7.81

WEBS

MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
K- D	0 / 309	0.07 (1)
K- E	-460 / 0	0.16 (1)
C- K	-460 / 0	0.16 (1)
N- L	-45 / 0	0.00 (1)
B- L	0 / 894	0.20 (1)
J- H	-45 / 0	0.00 (1)
J- F	0 / 894	0.20 (1)

N- M	0 / 39	-18.5	-18.5	0.02 (4)	10.00
M- L	0 / 19	0.0	0.0	0.06 (1)	10.00
L- C	0 / 70	0.0	0.0	0.07 (1)	10.00
L- K	0 / 931	-18.5	-18.5	0.21 (1)	10.00
K- J	0 / 931	-18.5	-18.5	0.21 (1)	10.00
I- J	0 / 19	0.0	0.0	0.06 (1)	10.00
J- E	0 / 70	0.0	0.0	0.07 (1)	10.00
I- H	0 / 39	-18.5	-18.5	0.02 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.22/1.00 (C-D:1) , BC=0.21/1.00 (J-K:1) , WB=0.20/1.00 (F-J:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (F) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)

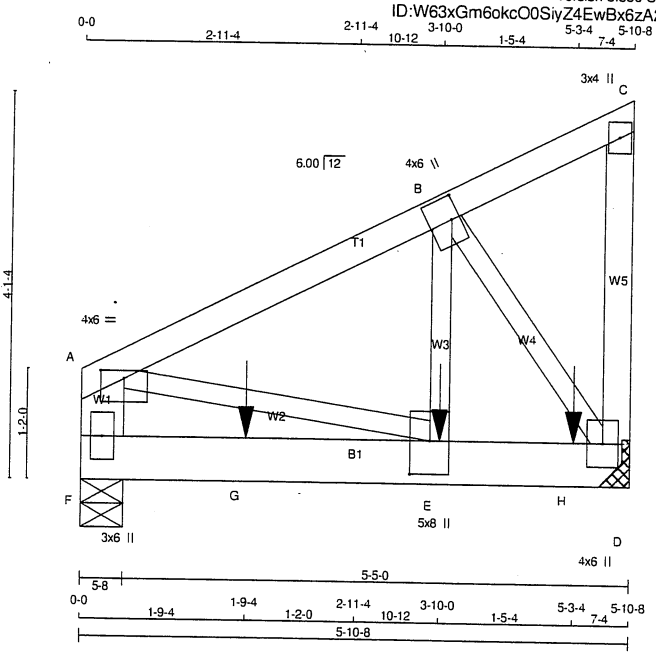
Structural component only
DWG# T-2017340

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T11	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:22:08 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-aXG34hAZIoIhR9YzSkWAknu9m1?rS8IEFncL5UyqyVz

Scale = 1:23.4



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	11	SIDE(244.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	2	SIDE(587.0)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMVW+t	MT20	4.0	6.0	3.00	1.00
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW+t	MT20	5.0	8.0	4.25	2.50

LICENSED PROFESSIONAL ENGINEER
08/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017341 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1784	0	1784	0	0	5-8	5-8
D	3731	0	3731	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1255	861 / 0	0 / 0	0 / 0	0 / 0	394 / 0	0 / 0
D	2629	1779 / 0	0 / 0	0 / 0	0 / 0	850 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
F- A	-1562 / 0	0.0	0.0	0.06 (1)	7.81	A- E	0 / 2042	0.25 (1)
A- B	-2230 / 0	-91.8	-91.8	0.12 (1)	5.85	E- B	0 / 3316	0.41 (1)
B- C	-16 / 0	-91.8	-91.8	0.05 (1)	6.25	B- D	-3312 / 0	0.36 (1)
D- C	-58 / 0	0.0	0.0	0.01 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.13 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.13 (1)	10.00			
E- H	0 / 2002	-18.5	-18.5	0.36 (1)	10.00			
H- D	0 / 2002	-18.5	-18.5	0.36 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-10-0	-3186	-3186	---	BACK	VERT	TOTAL	---	C1
G	1-9-4	-302	-302	---	TOP	VERT	TOTAL	---	C1
H	5-3-4	-1379	-1379	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 30 = 60 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.36/1.00 (D-E:1) , WB=0.41/1.00 (B-E:1) , SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

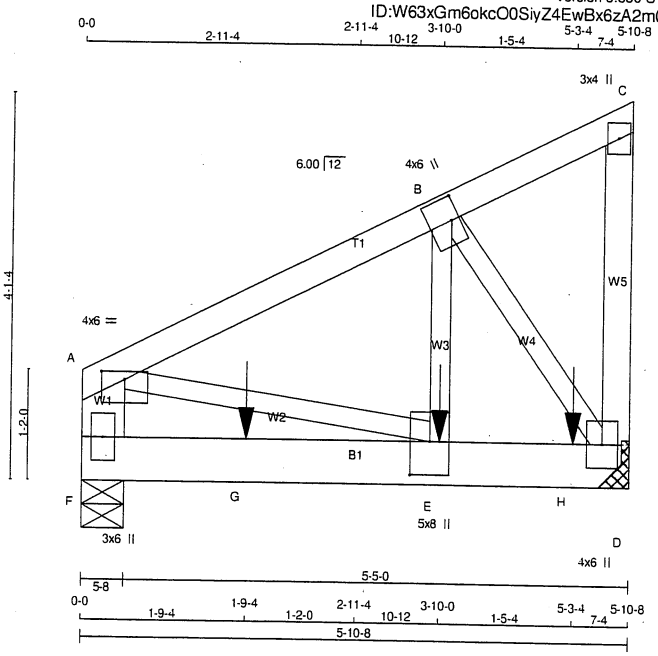
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (E) (INPUT = 0.90)
JSI METAL= 0.52 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409988	T11Z	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:17:41 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-Uk94cMVEMsDnoMsYCDpmJeQkeaYetYUqgrUDaoyqxhu



TOTAL WEIGHT = 2 X 30 = 60 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	11	SIDE(244.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	2	SIDE(575.0)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW+t	MT20	4.0	6.0	3.00	1.00
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	5.0	8.0	4.25	2.50

LICENSED PROFESSIONAL ENGINEER
08/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017351

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1768	0	1768	0	0	5-8	5-8
D	3701	0	3701	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1244	853 / 0	0 / 0	0 / 0	0 / 0	391 / 0	0 / 0
D	2608	1765 / 0	0 / 0	0 / 0	0 / 0	843 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1547 / 0	0.0	0.0	0.05 (1)	7.81	A- E	0 / 2021	0.25 (1)
A- B	-2206 / 0	-91.8	-91.8	0.12 (1)	5.87	E- B	0 / 3277	0.41 (1)
B- C	-16 / 0	-91.8	-91.8	0.05 (1)	6.25	B- D	-3277 / 0	0.35 (1)
D- C	-58 / 0	0.0	0.0	0.01 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.13 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.13 (1)	10.00			
E- H	0 / 1981	-18.5	-18.5	0.36 (1)	10.00			
H- D	0 / 1981	-18.5	-18.5	0.36 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-10-0	-3140	-3140	---	BACK	VERT	TOTAL	---	C1
G	1-9-4	-302	-302	---	TOP	VERT	TOTAL	---	C1
H	5-3-4	-1379	-1379	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.36/1.00 (D-E:1) , WB=0.41/1.00 (B-E:1) , SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

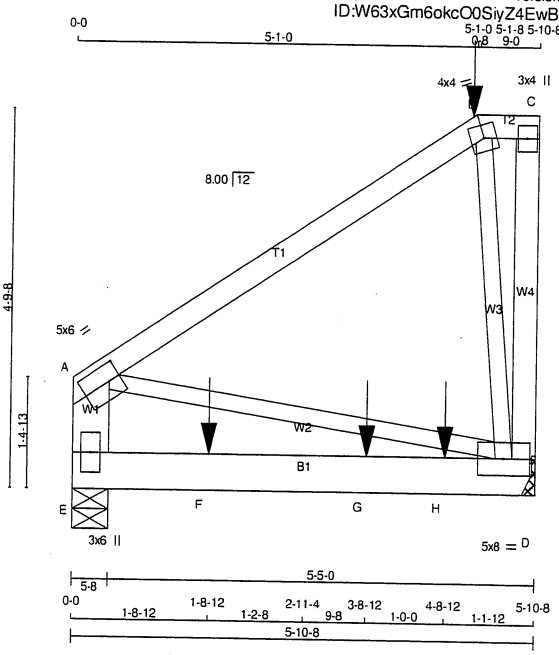
JSI GRIP= 0.81 (E) (INPUT = 0.90)
JSI METAL= 0.51 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T12	2	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:22:09 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-2jqRI1BB36QY3J79?R2PG?QluRGvBgXOURLudwyqyVy

Scale = 1:27.8



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - A	2x6	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	SIDE(61.0)
B- C	1	12	SIDE(61.0)
C- D	1	12	TOP
E- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
E- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	1.75
B	TTW-m	MT20	4.0	4.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVWW1-t	MT20	5.0	8.0		

Structural component only
DWG# T-2017342

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1758	0	1758	0	0	MECHANICAL	
E	1203	0	1203	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1235	855 / 0	0 / 0	0 / 0	0 / 0	380 / 0	0 / 0
E	848	573 / 0	0 / 0	0 / 0	0 / 0	275 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B		-109 / 0	-91.8	-91.8	0.23 (1)	6.25	A- D	0 / 92	0.01 (1)
B- C		0 / 0	-91.8	-91.8	0.00 (1)	10.00	B- D	-545 / 0	0.09 (1)
D- C		-34 / 0	0.0	0.0	0.01 (1)	7.81			
E- A		-311 / 0	0.0	0.0	0.01 (1)	7.81			
E- F		0 / 0	-18.5	-18.5	0.69 (1)	10.00			
F- G		0 / 0	-18.5	-18.5	0.69 (1)	10.00			
G- H		0 / 0	-18.5	-18.5	0.69 (1)	10.00			
H- D		0 / 0	-18.5	-18.5	0.69 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	5-1-8	-329	-329	---	BACK	VERT	TOTAL	---	C1
F	1-8-12	-661	-661	---	FRONT	VERT	TOTAL	---	C1
G	3-8-12	-661	-661	---	FRONT	VERT	TOTAL	---	C1
H	4-8-12	-661	-661	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 32 = 129 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 783 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 415 (0.17")

CSI: TC=0.23/1.00 (A-B:1) , BC=0.69/1.00 (D-E:1) , WB=0.09/1.00 (B-D:1) , SSI=0.40/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

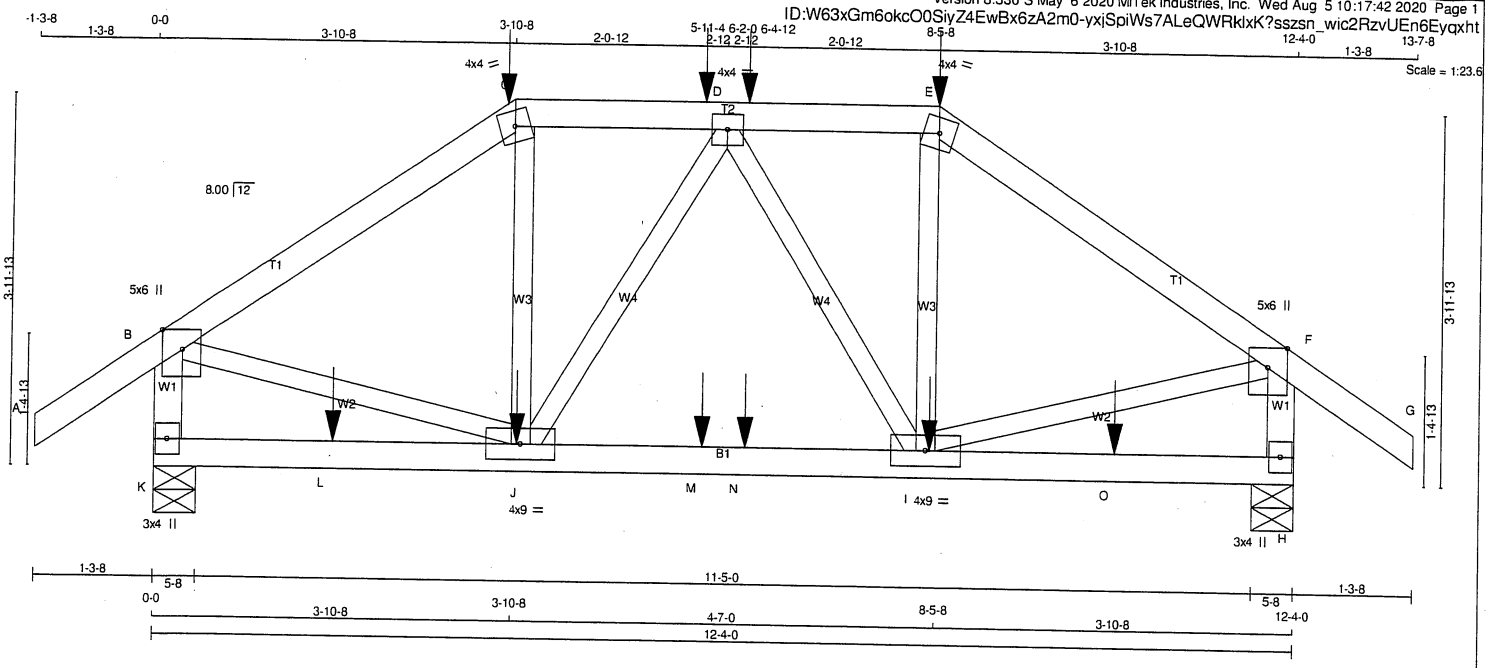
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409988	T20	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:17:42 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-yxjSpiWs7ALeQWRklxK?sszsn_wic2RzvUEn6Eyqxht



TOTAL WEIGHT = 54 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1191	0	1191	0	0	5-8	5-8
H	1191	0	1191	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	839	568 / 0	0 / 0	0 / 0	0 / 0	272 / 0	0 / 0
H	839	568 / 0	0 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.14 (1)	10.00
B- C	-1092 / 0	-91.8	-91.8	0.29 (1)	5.65
C- D	-911 / 0	-91.8	-91.8	0.08 (1)	6.25
D- E	-911 / 0	-91.8	-91.8	0.08 (1)	6.25
E- F	-1092 / 0	-91.8	-91.8	0.29 (1)	5.65
F- G	0 / 35	-91.8	-91.8	0.14 (1)	10.00
K- B	-1154 / 0	0.0	0.0	0.13 (1)	7.38
H- F	-1154 / 0	0.0	0.0	0.13 (1)	7.38
K- L	0 / 0	-18.5	-18.5	0.12 (4)	10.00
L- J	0 / 0	-18.5	-18.5	0.12 (4)	10.00
J- M	0 / 1029	-18.5	-18.5	0.24 (1)	10.00
M- N	0 / 1029	-18.5	-18.5	0.24 (1)	10.00
N- I	0 / 1029	-18.5	-18.5	0.24 (1)	10.00
I- O	0 / 0	-18.5	-18.5	0.12 (4)	10.00
O- H	0 / 0	-18.5	-18.5	0.12 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	0 / 101	0.04 (4)
I- E	0 / 101	0.04 (4)
B- J	0 / 939	0.23 (1)
I- F	0 / 939	0.23 (1)
J- D	-237 / 0	0.08 (1)
D- I	-237 / 0	0.08 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-247	-247	---	BACK	VERT	TOTAL	---	C1
D	5-11-4	-86	-86	---	BACK	VERT	TOTAL	---	C1
D	6-4-12	-86	-86	---	BACK	VERT	TOTAL	---	C1
E	8-5-8	-247	-247	---	BACK	VERT	TOTAL	---	C1
I	8-4-12	-17	-17	---	BACK	VERT	TOTAL	---	C1
J	3-11-4	-17	-17	---	BACK	VERT	TOTAL	---	C1
L	1-11-4	-17	-17	---	BACK	VERT	TOTAL	---	C1
M	5-11-4	-17	-17	---	BACK	VERT	TOTAL	---	C1
N	6-4-12	-17	-17	---	BACK	VERT	TOTAL	---	C1
O	10-4-12	-17	-17	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.29/1.00 (E-F:1) , BC=0.24/1.00 (I-J:1) , WB=0.23/1.00 (B-J:1) , SSI=0.13/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (F) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

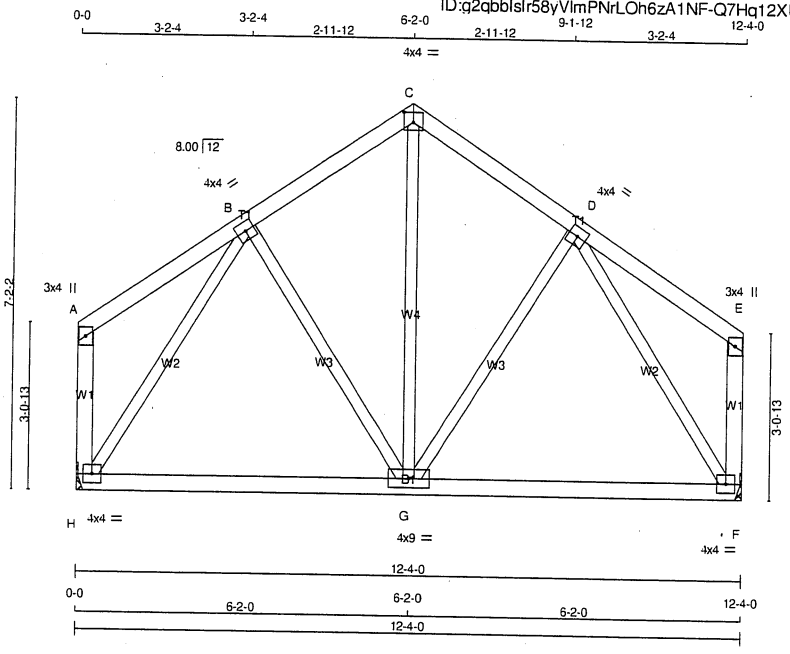
LICENSED PROFESSIONAL ENGINEER
09/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017352

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409988	T22	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:17:43 2020 Page 1
ID:g2qbbIsIr58yVlmPNrLOh6zA1NF-Q7Hq12XUuUUV2g?wJesEO3W3tOG7LT6788zKegyqxhs



TOTAL WEIGHT = 2 X 60 = 120 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	680	0	680	0	0	IN-SX	IN-SX
F	680	0	680	0	0	MECHANICAL	
						MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, F. MINIMUM BEARING LENGTH AT JOINT H = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	481	316 / 0	0 / 0	0 / 0	0 / 0	165 / 0	0 / 0
F	481	316 / 0	0 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 18	-91.8	-91.8	0.14 (1)	10.00
B- C	-400 / 0	-91.8	-91.8	0.11 (1)	6.25
C- D	-400 / 0	-91.8	-91.8	0.11 (1)	6.25
D- E	0 / 18	-91.8	-91.8	0.14 (1)	10.00
H- A	-111 / 0	0.0	0.0	0.02 (1)	7.81
F- E	-111 0	0.0	0.0	0.02 (1)	7.81
H- G	0 / 339	-18.5	-18.5	0.23 (4)	10.00
G- F	0 / 339	-18.5	-18.5	0.23 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 208	0.05 (1)
G- D	-40 / 25	0.02 (1)
B- G	-40 / 25	0.02 (1)
H- B	-625 / 0	0.33 (1)
D- F	-625 / 0	0.33 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.14/1.00 (D-E:1) , BC=0.23/1.00 (G-H:4) , WB=0.33/1.00 (B-H:1) , SSI=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

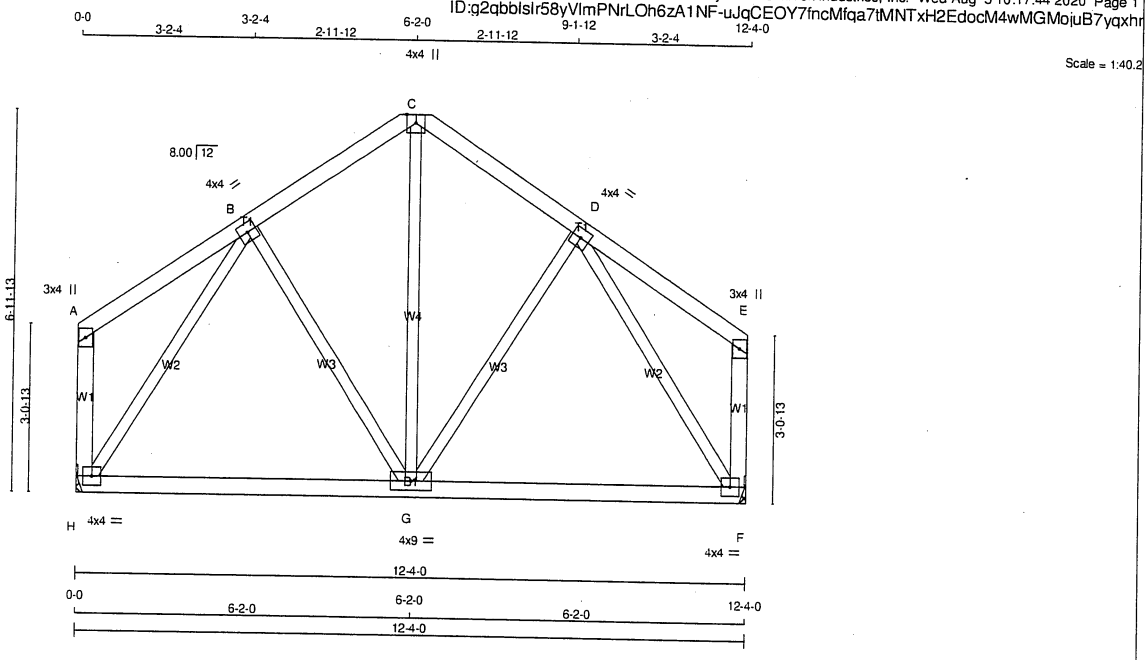
JSI GRIP= 0.62 (D) (INPUT = 0.90)
JSI METAL= 0.20 (D) (INPUT = 1.00)

Structural component only
DWG# T-2017353

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409988	T22CP	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:17:44 2020 Page 1
ID:g2qbblsIr58yVlmPNrLOh6zA1NF-uJqCEOY7fncMfqa7tMNTxH2EdocM4wMGMojuB7yqxhr



TOTAL WEIGHT = 60 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0		
C	TMTMW+p	MT20	4.0	4.0	Edge	
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	680	0	680	0	0	MECHANICAL	
F	680	0	680	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, F. MINIMUM BEARING LENGTH AT JOINT H = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	481	316 / 0	0 / 0	0 / 0	0 / 0	165 / 0	0 / 0
F	481	316 / 0	0 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 18	-91.8	-91.8	0.14 (1)	10.00
B- C	-400 / 0	-91.8	-91.8	0.11 (1)	6.25
C- D	-400 / 0	-91.8	-91.8	0.11 (1)	6.25
D- E	0 / 18	-91.8	-91.8	0.14 (1)	10.00
H- A	-111 / 0	0.0	0.0	0.02 (1)	7.81
F- E	-111 / 0	0.0	0.0	0.02 (1)	7.81
H- G	0 / 339	-18.5	-18.5	0.23 (4)	10.00
G- F	0 / 339	-18.5	-18.5	0.23 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 208	0.05 (1)
G- D	-40 / 25	0.02 (1)
B- G	-40 / 25	0.02 (1)
H- B	-625 / 0	0.33 (1)
D- F	-625 / 0	0.33 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.14/1.00 (D-E:1) , BC=0.23/1.00 (G-H:4) , WB=0.33/1.00 (B-H:1) , SSI=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

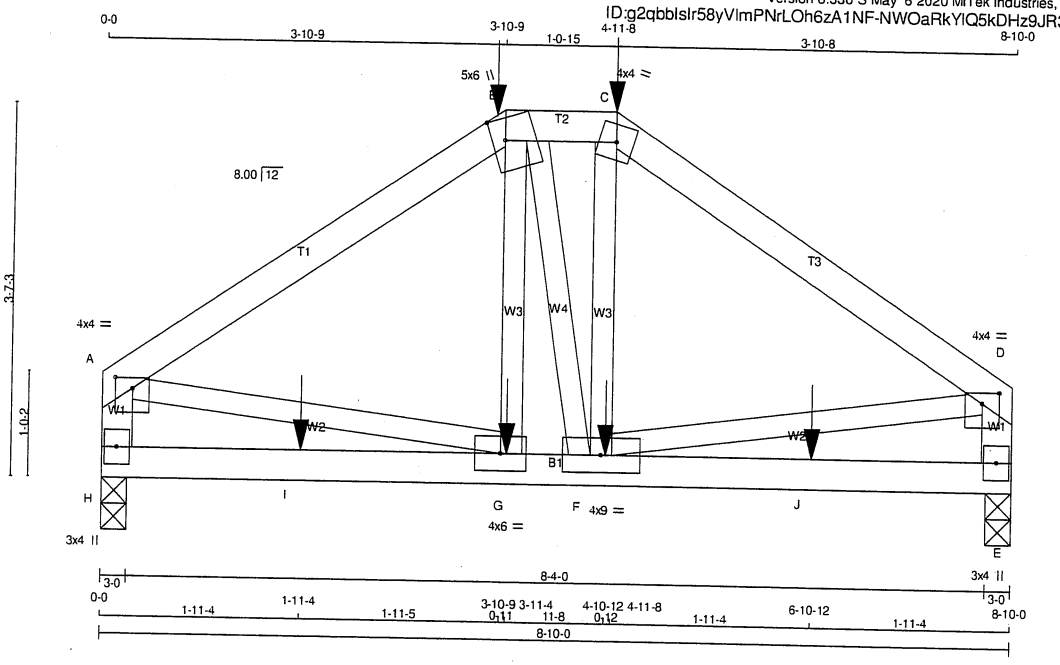
JSI GRIP= 0.62 (D) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

Structural component only
DWG# T-2017354

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409988	T23	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:17:45 2020 Page 1
ID:g2qbblslr58yVlmPNrLOh6zA1NF-NWOaRkYlQ5kDHz9JR3uiUUbNGCzupQXQbSSRjZyqxhq



TOTAL WEIGHT = 37 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	4.0	1.25	2.00
B	TTWW+m	MT20	5.0	6.0	2.50	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW-p	MT20	4.0	4.0	1.25	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	704	0	704	0	0	3-0	3-0
E	703	0	703	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	498	328 / 0	0 / 0	0 / 0	0 / 0	170 / 0	0 / 0
E	497	328 / 0	0 / 0	0 / 0	0 / 0	169 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-699 / 0	-91.8	-91.8	0.27 (1)	6.25	G- B	-34 / 37	0.01 (4)
B- C	-579 / 0	-91.8	-91.8	0.02 (1)	6.25	B- F	-2 / 3	0.00 (4)
C- D	-699 / 0	-91.8	-91.8	0.27 (1)	6.25	F- C	-28 / 49	0.02 (4)
H- A	-668 / 0	0.0	0.0	0.07 (1)	7.81	A- G	0 / 589	0.15 (1)
E- D	-666 / 0	0.0	0.0	0.07 (1)	7.81	F- D	0 / 589	0.15 (1)
H- I	0 / 0	-18.5	-18.5	0.09 (4)	10.00			
I- G	0 / 0	-18.5	-18.5	0.09 (4)	10.00			
G- F	0 / 579	-18.5	-18.5	0.14 (4)	10.00			
F- J	0 / 0	-18.5	-18.5	0.09 (4)	10.00			
J- E	0 / 0	-18.5	-18.5	0.09 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	3-10-9	-197	-197	---	BACK	VERT	TOTAL	---	C1
C	4-11-8	-190	-190	---	BACK	VERT	TOTAL	---	C1
F	4-10-12	-12	-12	---	BACK	VERT	TOTAL	---	C1
G	3-11-4	-12	-12	---	BACK	VERT	TOTAL	---	C1
I	1-11-4	-12	-12	---	BACK	VERT	TOTAL	---	C1
J	6-10-12	-12	-12	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.27/1.00 (A-B:1) , BC=0.14/1.00 (F-G:4) , WB=0.15/1.00 (A-G:1) , SSI=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (A) (INPUT = 0.90)
JSI METAL= 0.20 (D) (INPUT = 1.00)

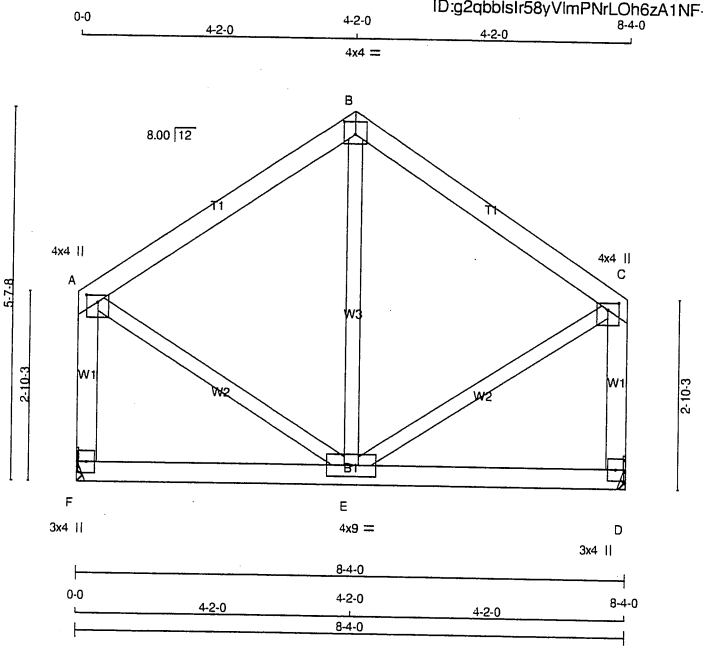
LICENSED PROFESSIONAL ENGINEER
08/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017355

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
409988	T24	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:17:46 2020 Page 1
ID:g2qbblsIr58yVlmPNrLOh6zA1NF-riyzf4ZNBPs4v7kV_mPx0i8Z6bKzYv4Zq6C_F?yqxhp



TOTAL WEIGHT = 38 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTW-p	MT20	4.0	4.0	2.25	2.00
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	459	0	459	0	0	MECHANICAL	
D	459	0	459	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	325	213 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
D	325	213 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-225 / 0	-91.8	-91.8	0.20 (1)	6.25	E- B	-133 / 23	0.06 (1)
B- C	-225 / 0	-91.8	-91.8	0.20 (1)	6.25	A- E	0 / 219	0.05 (1)
F- A	-430 / 0	0.0	0.0	0.06 (1)	7.81	E- C	0 / 219	0.05 (1)
D- C	-430 / 0	0.0	0.0	0.06 (1)	7.81			
F- E	0 / 0	-18.5	-18.5	0.09 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.20/1.00 (A-B:1) , BC=0.09/1.00 (E-F:4) , WB=0.06/1.00 (B-E:1) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

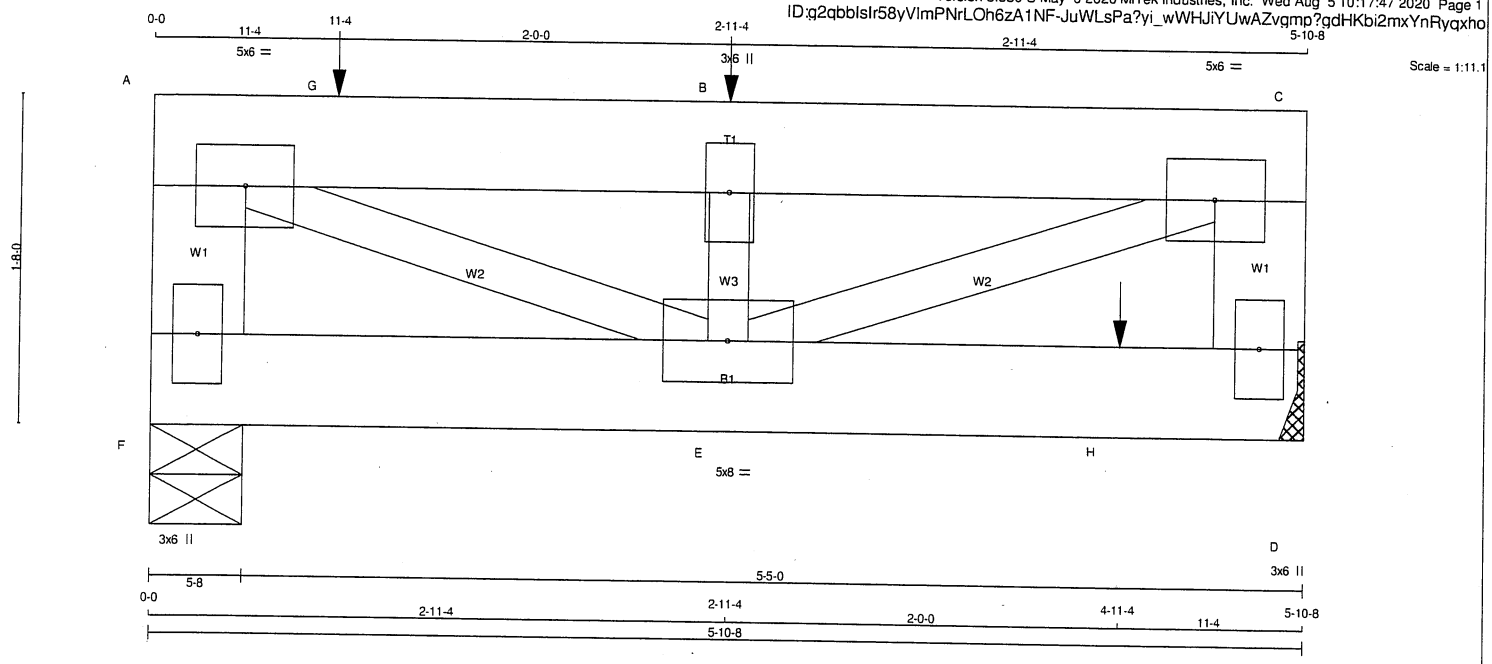
JSI GRIP= 0.29 (A) (INPUT = 0.90)
JSI METAL= 0.09 (A) (INPUT = 1.00)

Structural component only
DWG# T-2017356

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409988	T25	2	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:17:47 2020 Page 1
ID:g2qbbIsIr58yVlmPNrLOh6zA1NF-JuWLsPa?yi_wWHJiYUwAZvgmp?gdHKbi2mxYnRyqxho



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	2	12	TOP
C- D	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(14.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

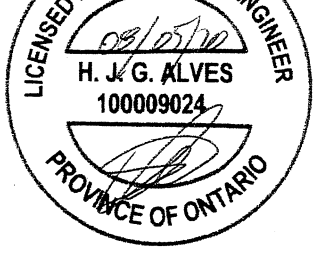
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		
B	TMW+w	MT20	3.0	6.0		
C	TMVW-t	MT20	5.0	6.0		
D	BMV1+p	MT20	3.0	6.0		
E	BMWWW-t	MT20	5.0	8.0		
F	BMV1+p	MT20	3.0	6.0		



Structural component only
DWG# T-2017357

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	874	0	874	0	0	5-8	5-8
D	1093	0	1093	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	625	369 / 0	0 / 0	0 / 0	0 / 0	256 / 0	0 / 0
D	779	477 / 0	0 / 0	0 / 0	0 / 0	303 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-783 / 0	0.0	0.0	0.03 (1)	7.81	A- E	0 / 1400	0.17 (1)
A- G	-1295 / 0	-91.8	-91.8	0.08 (1)	6.25	E- B	-850 / 0	0.07 (1)
G- B	-1295 / 0	-91.8	-91.8	0.08 (1)	6.25	E- C	0 / 1400	0.17 (1)
B- C	-1295 / 0	-91.8	-91.8	0.06 (1)	6.25			
D- C	-702 / 0	0.0	0.0	0.02 (1)	7.81			
F- E	0 / 0	-43.5	-43.5	0.03 (1)	10.00			
E- H	0 / 0	-43.5	-43.5	0.13 (1)	10.00			
H- D	0 / 0	-43.5	-43.5	0.13 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	2-11-4	-612	-612	---	TOP	VERT	TOTAL	---	C1
G	11-4	-119	-119	---	TOP	VERT	TOTAL	---	C1
H	4-11-4	-441	-441	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 28 = 114 lb [M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.08/1.00 (A-B:1) , BC=0.13/1.00 (D-E:1) , WB=0.17/1.00 (A-E:1) , SSI=0.13/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

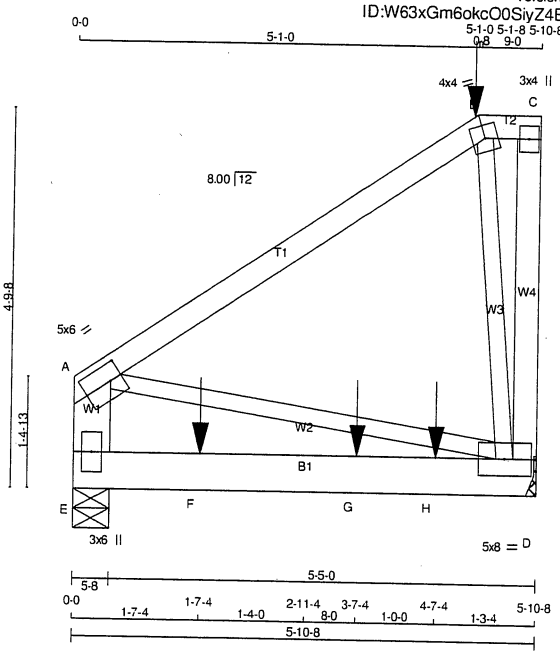
JSI GRIP= 0.48 (E) (INPUT = 0.90)
JSI METAL= 0.15 (A) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409988	T26	2	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:17:48 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-n54j4lbdj06n8Ruu6BRP57DvFPro0o9sHQh5Kuyqxhn

Scale = 1:27.8



TOTAL WEIGHT = 4 X 32 = 129 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - A	2x6	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	SIDE(61.0)
B- C	1	12	SIDE(61.0)
C- D	1	12	TOP
E- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
E- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	1.75
B	TTW-m	MT20	4.0	4.0		

Structural component only
DWG# T-2017358 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	1712	0	1712	0	0	MECHANICAL	
E	1245	0	1245	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1203	832 / 0	0 / 0	0 / 0	0 / 0	370 / 0	0 / 0
E	877	593 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-109 / 0	-91.8	-91.8	0.23 (1)	6.25		B- D	-541 / 0	0.09 (1)
B- C	0 / 0	-91.8	-91.8	0.00 (1)	10.00		A- D	0 / 92	0.01 (1)
D- C	-34 / 0	0.0	0.0	0.01 (1)	7.81				
E- A	-311 / 0	0.0	0.0	0.01 (1)	7.81				
E- F	0 / 0	-18.5	-18.5	0.71 (1)	10.00				
F- G	0 / 0	-18.5	-18.5	0.71 (1)	10.00				
G- H	0 / 0	-18.5	-18.5	0.71 (1)	10.00				
H- D	0 / 0	-18.5	-18.5	0.71 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	5-1-8	-324	-324	---	FRONT	VERT	TOTAL	---	C1
F	1-7-4	-661	-661	---	BACK	VERT	TOTAL	---	C1
G	3-7-4	-661	-661	---	BACK	VERT	TOTAL	---	C1
H	4-7-4	-661	-661	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 770 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 409 (0.17")

CSI: TC=0.23/1.00 (A-B:1) , BC=0.71/1.00 (D-E:1) , WB=0.09/1.00 (B-D:1) , SSI=0.38/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
409988	T26	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMV+p	MT20	3.0	4.0		
D	BMVWW1-t	MT20	5.0	8.0		
E	BMV1+p	MT20	3.0	6.0		

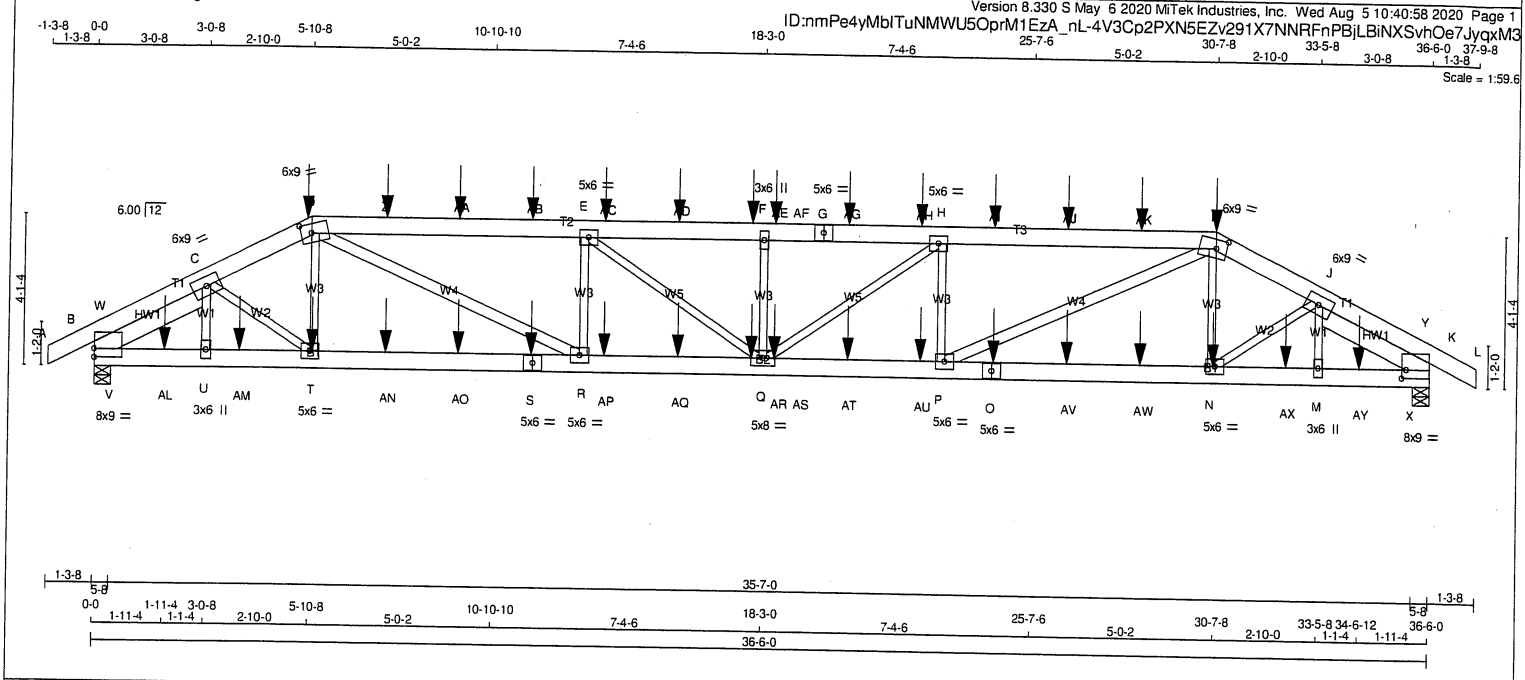
LICENSED PROFESSIONAL ENGINEER
08/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017358 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T30	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:40:58 2020 Page 1
ID:nmPe4yMbITuNMWU5OprM1EzA_nL-4V3Cp2PXN5EZv291X7NNRFnPBjLBiNXSvhOe7JyqxM3



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - L	2x6	DRY	No.2	SPF
B - S	2x6	DRY	1650F 1.5E	SPF
S - O	2x6	DRY	1650F 1.5E	SPF
O - K	2x6	DRY	1650F 1.5E	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - R	2x4	DRY	No.2	SPF
P - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	SIDE(122.0)
D- G	2	12	SIDE(183.1)
G- I	2	12	SIDE(61.0)
I- L	2	12	SIDE(122.0)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- S	2	12	SIDE(183.1)
S- O	2	12	SIDE(183.1)
O- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

Structural component only
DWG# T-2017367 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	3450	0	3450	0	0	5-8	5-8
K	3450	0	3450	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	2441	1596 / 0	0 / 0	0 / 0	0 / 0	845 / 0	0 / 0
K	2441	1596 / 0	0 / 0	0 / 0	0 / 0	845 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 0	-91.8	-91.8	0.03 (1)	10.00
B- W	-3568 / 0	-91.8	-91.8	0.10 (1)	5.84
W- C	-2103 / 0	-91.8	-91.8	0.09 (1)	6.25
C- D	-5438 / 0	-91.8	-91.8	0.09 (1)	4.97
D- Z	-8210 / 0	-91.8	-91.8	0.44 (1)	3.89
Z-AA	-8210 / 0	-91.8	-91.8	0.44 (1)	3.89
AA-AB	-8210 / 0	-91.8	-91.8	0.44 (1)	3.89
AB- E	-8210 / 0	-91.8	-91.8	0.44 (1)	3.89
E-AC	-8665 / 0	-91.8	-91.8	0.37 (1)	3.83
AC-AD	-8665 / 0	-91.8	-91.8	0.37 (1)	3.83
AD-AE	-8665 / 0	-91.8	-91.8	0.37 (1)	3.83
AE- F	-8665 / 0	-91.8	-91.8	0.37 (1)	3.83
F-AF	-8665 / 0	-91.8	-91.8	0.37 (1)	3.83
AF- G	-8665 / 0	-91.8	-91.8	0.37 (1)	3.83
G-AG	-8665 / 0	-91.8	-91.8	0.37 (1)	3.83
AG-AH	-8665 / 0	-91.8	-91.8	0.37 (1)	3.83
AH- H	-8665 / 0	-91.8	-91.8	0.37 (1)	3.83
H-AI	-8210 / 0	-91.8	-91.8	0.44 (1)	3.89
AI-AJ	-8210 / 0	-91.8	-91.8	0.44 (1)	3.89
AJ-AK	-8210 / 0	-91.8	-91.8	0.44 (1)	3.89
AK- I	-8210 / 0	-91.8	-91.8	0.44 (1)	3.89
I- J	-5438 / 0	-91.8	-91.8	0.09 (1)	4.97
J- Y	-2103 / 0	-91.8	-91.8	0.09 (1)	6.25
Y- K	-3568 / 0	-91.8	-91.8	0.10 (1)	5.84
K- L	0 / 0	-91.8	-91.8	0.03 (1)	10.00
B- V	0 / 1830	-18.5	-18.5	0.10 (1)	10.00
V-AL	0 / 4259	-18.5	-18.5	0.21 (1)	10.00
AL- U	0 / 4259	-18.5	-18.5	0.21 (1)	10.00
U-AM	0 / 4260	-18.5	-18.5	0.22 (1)	10.00
AM- T	0 / 4260	-18.5	-18.5	0.22 (1)	10.00
T-AN	0 / 4865	-18.5	-18.5	0.25 (1)	10.00
AN-AO	0 / 4865	-18.5	-18.5	0.25 (1)	10.00
AO- S	0 / 4865	-18.5	-18.5	0.25 (1)	10.00
S- R	0 / 4865	-18.5	-18.5	0.25 (1)	10.00
R-AP	0 / 8210	-18.5	-18.5	0.38 (1)	10.00
AP-AQ	0 / 8210	-18.5	-18.5	0.38 (1)	10.00
AQ-AR	0 / 8210	-18.5	-18.5	0.38 (1)	10.00
AR- Q	0 / 8210	-18.5	-18.5	0.38 (1)	10.00
Q-AS	0 / 8210	-18.5	-18.5	0.38 (1)	10.00
AS-AT	0 / 8210	-18.5	-18.5	0.38 (1)	10.00
AT-AU	0 / 8210	-18.5	-18.5	0.38 (1)	10.00
AU- P	0 / 8210	-18.5	-18.5	0.38 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-32 / 20	0.00 (4)
C- T	0 / 770	0.10 (1)
T- D	-165 / 92	0.02 (1)
D- R	0 / 3717	0.33 (1)
R- E	-1453 / 0	0.17 (1)
P- H	-1453 / 0	0.17 (1)
P- I	0 / 3717	0.33 (1)
N- I	-165 / 92	0.02 (1)
N- J	0 / 770	0.10 (1)
M- J	-32 / 20	0.00 (4)
Q- H	0 / 568	0.07 (1)
E- Q	0 / 568	0.07 (1)
Q- F	-518 / 0	0.06 (1)
V- W	0 / 1887	0.00 (1)
V- C	-3189 / 0	0.19 (1)
J- X	-3189 / 0	0.19 (1)
X- Y	0 / 1887	0.00 (1)

TOTAL WEIGHT = 2 X 200 = 399 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CSI: TC=0.44/1.00 (H-I:1) , BC=0.38/1.00 (Q-R:1) , WB=0.33/1.00 (I-P:1) , SSI=0.20/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (D) (INPUT = 0.90)
JSI METAL= 0.47 (S) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T30	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	8.0	9.0	2.75	
C	TMWWW-t	MT20	6.0	9.0		
D	TTWW-m	MT20	6.0	9.0	3.00	3.50
E	TMWW-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0		
I	TTWW-m	MT20	6.0	9.0	3.00	3.50
J	TMWWW-t	MT20	6.0	9.0		
K	TMBMW1-I	MT20	8.0	9.0	2.75	1.50
M	BMW+w	MT20	3.0	6.0		
N, P, R, T						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
U	BMW+w	MT20	3.0	6.0		

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
P- O	0 / 4865	-18.5	-18.5	0.25 (1)	10.00
O-AV	0 / 4865	-18.5	-18.5	0.25 (1)	10.00
AV-AW	0 / 4865	-18.5	-18.5	0.25 (1)	10.00
AW- N	0 / 4865	-18.5	-18.5	0.25 (1)	10.00
N-AX	0 / 4260	-18.5	-18.5	0.22 (1)	10.00
AX- M	0 / 4260	-18.5	-18.5	0.22 (1)	10.00
M-AY	0 / 4259	-18.5	-18.5	0.21 (1)	10.00
AY- X	0 / 4259	-18.5	-18.5	0.21 (1)	10.00
X- K	0 / 1830	-18.5	-18.5	0.10 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-411	-411	---	FRONT	VERT	TOTAL	---	C1
I	30-7-8	-411	-411	---	FRONT	VERT	TOTAL	---	C1
N	30-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	24-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
S	11-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
T	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Z	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	11-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	15-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AE	17-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AF	18-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AG	20-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AH	22-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AI	24-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AJ	26-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AK	28-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AL	1-11-4	-25	-26	---	FRONT	VERT	TOTAL	---	C1
AM	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AN	7-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AO	9-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AP	13-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AQ	15-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AR	17-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AS	18-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AT	20-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AU	22-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AV	26-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AW	28-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AX	32-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AY	34-6-12	-25	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

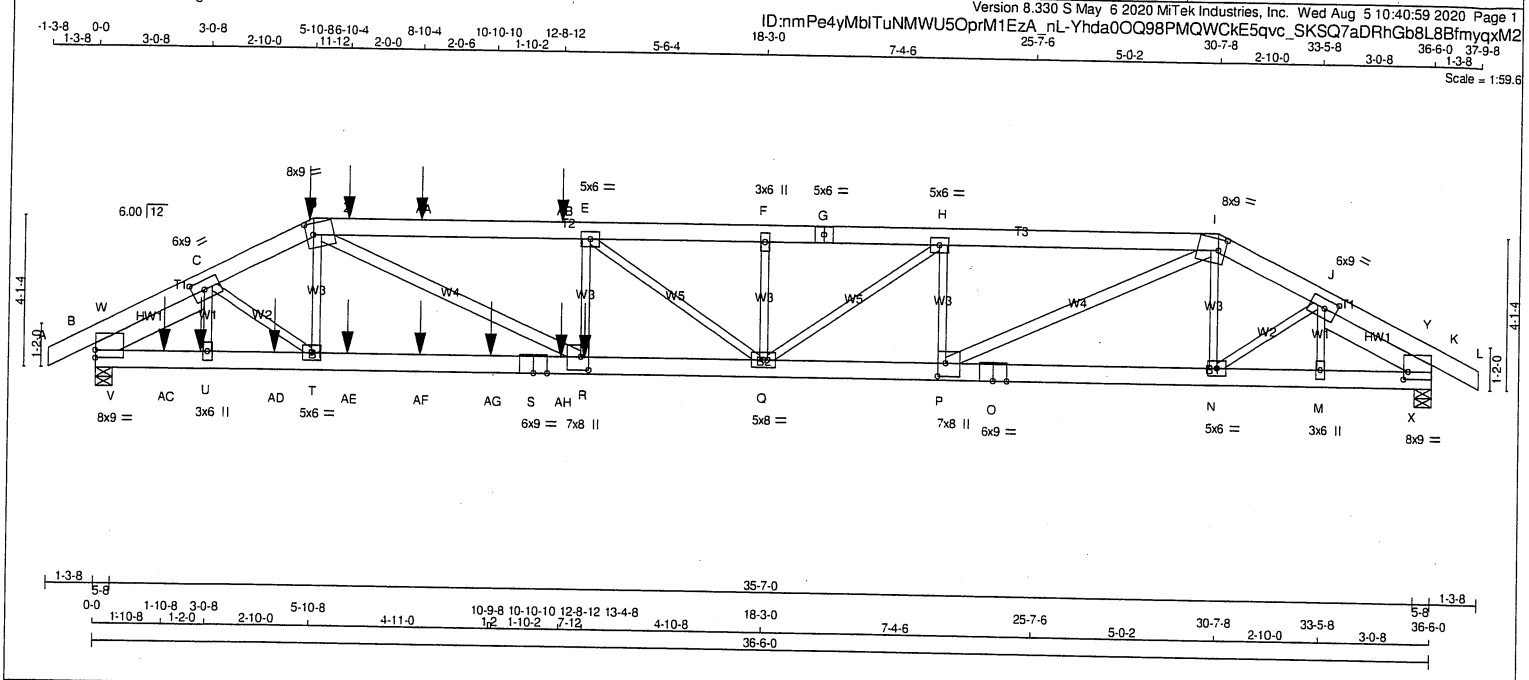
Structural component only
DWG# T-2017367

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T30Z	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:40:59 2020 Page 1

ID:nmPe4yMbITuNMWU5OprM1EzA_nL-Yhda0OQ98PMQWCkE5qvc_SKSQ7aDRhGb8L8BfmyqxM2



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - L	2x6	DRY	No.2	SPF
B - S	2x6	DRY	1650F 1.5E	SPF
S - O	2x6	DRY	1650F 1.5E	SPF
O - K	2x6	DRY	1650F 1.5E	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - R	2x4	DRY	No.2	SPF
P - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	SIDE(122.0)
D- G	2	12	SIDE(61.0)
G- I	2	12	TOP
I- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- S	2	12	SIDE(183.1)
S- O	2	12	SIDE(183.1)
O- K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
E- R	1	2	SIDE(764.9)
H- P	1	2	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

Structural component only

DWG# T-2017368 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	6422	0	6422	0	0	5-8	5-8
K	3996	0	3996	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	4529	3041 / 0	0 / 0	0 / 0	0 / 0	1488 / 0	0 / 0
K	2819	1891 / 0	0 / 0	0 / 0	0 / 0	928 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 0	-91.8	-91.8	0.03 (1)	10.00
B- W	-7123 / 0	-91.8	-91.8	0.20 (1)	4.33
W- C	-4120 / 0	-91.8	-91.8	0.16 (1)	5.45
C- D	-10286 / 0	-91.8	-91.8	0.23 (1)	3.66
D- Z	-16610 / 0	-91.8	-91.8	0.92 (1)	2.40
Z-AA	-16610 / 0	-91.8	-91.8	0.92 (1)	2.40
AA-AB	-16610 / 0	-91.8	-91.8	0.92 (1)	2.40
AB- E	-16610 / 0	-91.8	-91.8	0.92 (1)	2.40
E- F	-14235 / 0	-91.8	-91.8	0.44 (1)	3.02
F- G	-14235 / 0	-91.8	-91.8	0.47 (1)	2.99
G- H	-14235 / 0	-91.8	-91.8	0.47 (1)	2.99
H- I	-11536 / 0	-91.8	-91.8	0.49 (1)	3.33
I- J	-6476 / 0	-91.8	-91.8	0.11 (1)	4.61
J- Y	-2443 / 0	-91.8	-91.8	0.10 (1)	6.25
Y- K	-4145 / 0	-91.8	-91.8	0.12 (1)	5.49
K- L	0 / 0	-91.8	-91.8	0.03 (1)	10.00
B- V	0 / 3589	-18.5	-18.5	0.16 (1)	10.00
V-AC	0 / 7902	-18.5	-18.5	0.48 (1)	10.00
AC- U	0 / 7902	-18.5	-18.5	0.48 (1)	10.00
U-AD	0 / 7905	-18.5	-18.5	0.58 (1)	10.00
AD- T	0 / 7905	-18.5	-18.5	0.58 (1)	10.00
T-AE	0 / 9221	-18.5	-18.5	0.76 (1)	10.00
AE-AF	0 / 9221	-18.5	-18.5	0.76 (1)	10.00
AF-AG	0 / 9221	-18.5	-18.5	0.76 (1)	10.00
AG- S	0 / 9221	-18.5	-18.5	0.76 (1)	10.00
S-AH	0 / 9221	-18.5	-18.5	0.76 (1)	10.00
AH- R	0 / 9221	-18.5	-18.5	0.76 (1)	10.00
R- Q	0 / 16610	-18.5	-18.5	0.77 (1)	10.00
Q- P	0 / 11537	-18.5	-18.5	0.54 (1)	10.00
P- O	0 / 5790	-18.5	-18.5	0.29 (1)	10.00
O- N	0 / 5790	-18.5	-18.5	0.29 (1)	10.00
N- M	0 / 5009	-18.5	-18.5	0.25 (1)	10.00
M- X	0 / 5007	-18.5	-18.5	0.24 (1)	10.00
X- K	0 / 2126	-18.5	-18.5	0.12 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-122 / 0	0.01 (1)
C- T	0 / 1661	0.21 (1)
T- D	0 / 126	0.02 (4)
D- R	0 / 8211	0.73 (1)
R- E	0 / 1009	0.12 (1)
P- H	-2682 / 0	0.32 (1)
P- I	0 / 6385	0.56 (1)
N- I	-355 / 0	0.04 (1)
N- J	0 / 1000	0.12 (1)
M- J	-77 / 0	0.01 (1)
Q- H	0 / 3369	0.42 (1)
E- Q	-2967 / 0	0.94 (1)
Q- F	-311 / 0	0.04 (1)
V- W	0 / 3986	0.00 (1)
V- C	-5663 / 0	0.34 (1)
J- X	-3782 / 0	0.22 (1)
X- Y	0 / 2213	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-331	-331	---	FRONT	VERT	TOTAL	---	C1
R	13-4-8	-3634	-3634	---	BACK	VERT	TOTAL	---	C1
U	2-10-4	-16	-16	---	BACK	VERT	TOTAL	---	C1
Z	6-10-4	-79	-79	---	BACK	VERT	TOTAL	---	C1
AA	8-10-4	-78	-78	---	BACK	VERT	TOTAL	---	C1

TOTAL WEIGHT = 2 X 200 = 399 lb [M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.36")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 664 (0.66")

CSI: TC=0.92/1.00 (D-E:1) , BC=0.77/1.00 (Q-R:1) , WB=0.94/1.00 (E-Q:1) , SSI=0.26/1.00 (R-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (R) (INPUT = 0.90)
JSI METAL= 0.95 (R) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
409989	T30Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-l	MT20	8.0	9.0	2.50	
C	TMWWW-t	MT20	6.0	9.0	3.00	3.75
D	TTWW-m	MT20	8.0	9.0	3.75	2.25
E	TMWW-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0		
I	TTWW-m	MT20	8.0	9.0	3.75	2.25
J	TMWWW-t	MT20	6.0	9.0	3.00	3.75
K	TMBMW1-l	MT20	8.0	9.0	2.50	1.50
M	BMW+w	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	6.0	9.0		
P	BMWW+t	MT20	7.0	8.0	4.25	2.50
Q	BMWWW-t	MT20	5.0	8.0		
R	BMWW+t	MT20	7.0	8.0	4.25	2.50
S	BS-t	MT20	6.0	9.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMW+w	MT20	3.0	6.0		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AB	12-8-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
AC	1-10-8	-644	-644	---	BACK	VERT	TOTAL	---	C1
AD	4-10-4	-16	-16	---	BACK	VERT	TOTAL	---	C1
AE	6-10-4	-16	-16	---	BACK	VERT	TOTAL	---	C1
AF	8-10-4	-16	-16	---	BACK	VERT	TOTAL	---	C1
AG	10-9-8	-1171	-1171	---	BACK	VERT	TOTAL	---	C1
AH	12-8-12	-26	-26	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

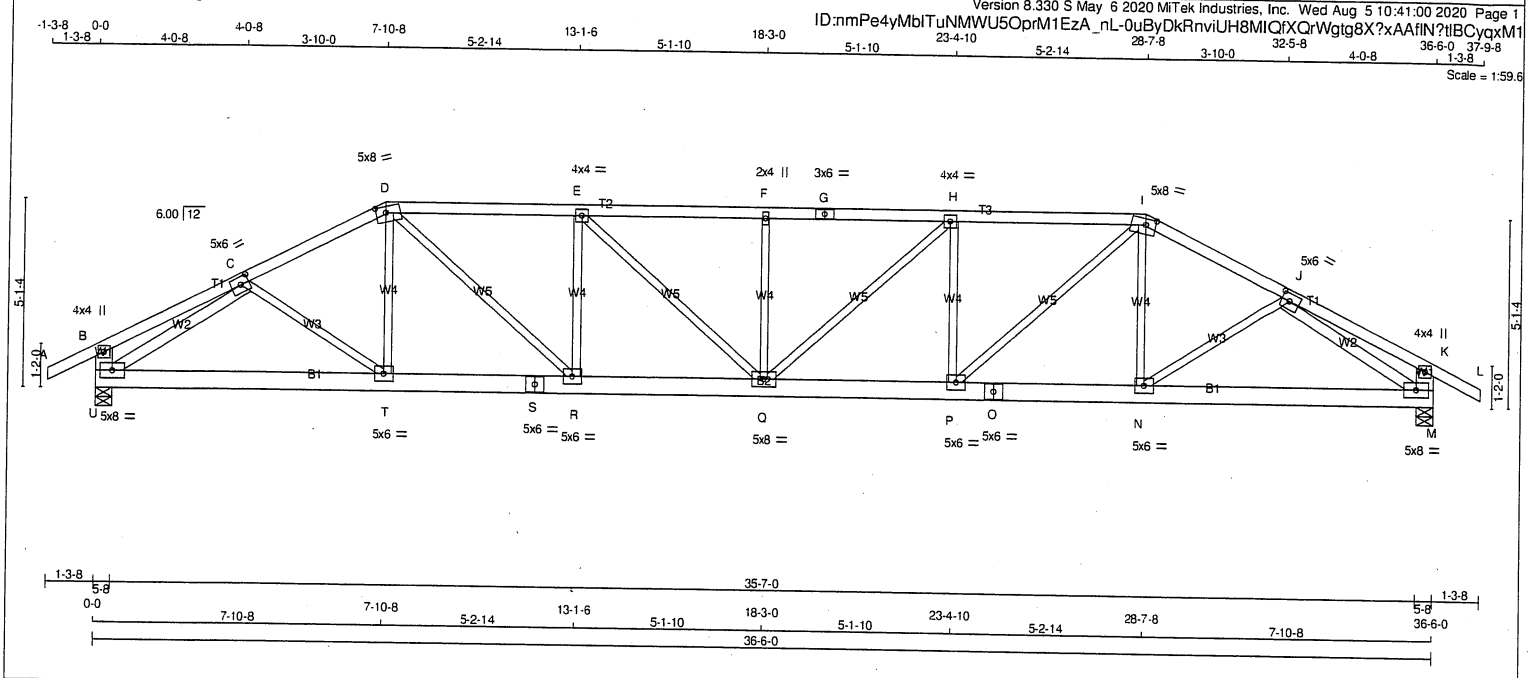
1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2017368 Y2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T31	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:41:00 2020 Page 1
ID:nmPe4yMbITuNMWU5OprM1EzA_nL-0uByDkRnviUH8MIQfXQrWgtg8X?xAAfIN?tlBCyqxM1



TOTAL WEIGHT = 2 X 171 = 343 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	8.0	2.00	3.25
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	8.0	2.00	3.25
J	TMWW-t	MT20	5.0	6.0	2.50	2.75
K	TMV+p	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	8.0		
N, P, R, T						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
U	BMVW1-t	MT20	5.0	8.0		

Structural component only
DWG# T-2017369

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM GROSS REACTION DOWN	MAXIMUM GROSS REACTION HORZ	FACTORED UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2137	0	2137	0	0	5-8	5-8
M	2137	0	2137	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0
M	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 17	-91.8	-91.8	0.20 (1)	10.00
C- D	-2978 / 0	-91.8	-91.8	0.34 (1)	3.76
D- E	-3625 / 0	-91.8	-91.8	0.63 (1)	3.17
E- F	-3907 / 0	-91.8	-91.8	0.66 (1)	3.03
F- G	-3907 / 0	-91.8	-91.8	0.66 (1)	3.03
G- H	-3907 / 0	-91.8	-91.8	0.66 (1)	3.03
H- I	-3625 / 0	-91.8	-91.8	0.63 (1)	3.17
I- J	-2978 / 0	-91.8	-91.8	0.34 (1)	3.76
J- K	0 / 17	-91.8	-91.8	0.20 (1)	10.00
K- L	0 / 28	-91.8	-91.8	0.12 (1)	10.00
U- B	-269 / 0	0.0	0.0	0.02 (1)	7.81
M- K	-269 / 0	0.0	0.0	0.02 (1)	7.81
U- T	0 / 2592	-18.5	-18.5	0.38 (1)	10.00
T- S	0 / 2649	-18.5	-18.5	0.36 (1)	10.00
S- R	0 / 2649	-18.5	-18.5	0.36 (1)	10.00
R- Q	0 / 3626	-18.5	-18.5	0.48 (1)	10.00
Q- P	0 / 3626	-18.5	-18.5	0.48 (1)	10.00
P- O	0 / 2649	-18.5	-18.5	0.36 (1)	10.00
O- N	0 / 2649	-18.5	-18.5	0.36 (1)	10.00
N- M	0 / 2592	-18.5	-18.5	0.38 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	0 / 87	0.03 (4)
T- D	0 / 118	0.04 (4)
D- R	0 / 1314	0.30 (1)
R- E	-797 / 0	0.29 (1)
E- Q	0 / 383	0.09 (1)
Q- F	-433 / 0	0.16 (1)
Q- H	0 / 383	0.09 (1)
P- H	-797 / 0	0.29 (1)
P- I	0 / 1314	0.30 (1)
N- I	0 / 118	0.04 (4)
N- J	0 / 87	0.03 (4)
U- C	-3157 / 0	0.80 (1)
J- M	-3157 / 0	0.80 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.40")

CSI: TC=0.66/1.00 (E-F:1) , BC=0.48/1.00 (Q-R:1) , WB=0.80/1.00 (J-M:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

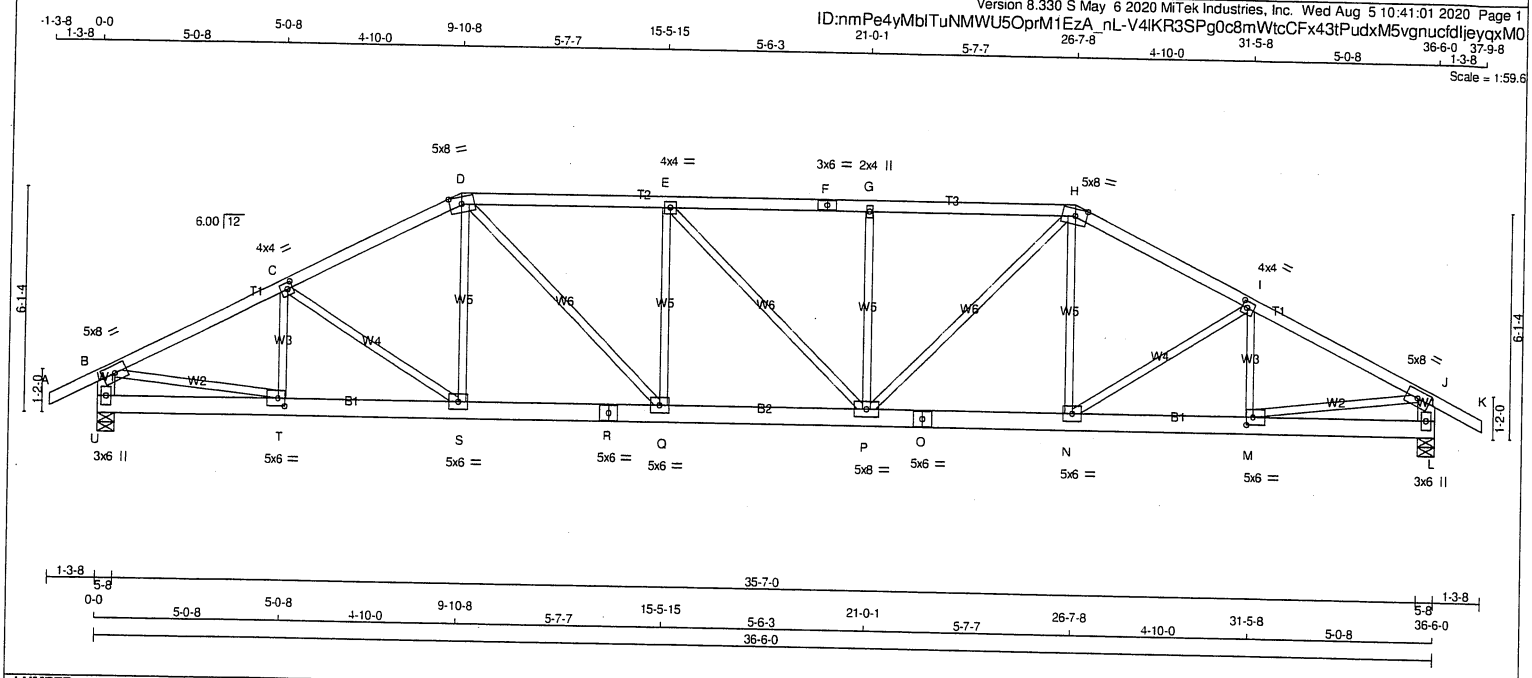
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (D) (INPUT = 0.90)
JSI METAL= 0.72 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T32	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:41:01 2020 Page 1
ID:nmPe4yMblTuNMWU5OprM1EzA_nL-V4lKR3SPg0c8mWtcCFx43tPudxM5vgnucfdljeyqxM0



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	2.25	3.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.25	3.75
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-t	MT20	5.0	8.0		
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.25
N, Q, S						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.25
U	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	2137	0	2137	0	0	5-8	5-8
L	2137	0	2137	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0
L	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	-3003 / 0	-91.8	-91.8	0.41 (1)	3.70
C- D	-2866 / 0	-91.8	-91.8	0.39 (1)	3.79
D- E	-3181 / 0	-91.8	-91.8	0.49 (1)	3.52
E- F	-3175 / 0	-91.8	-91.8	0.45 (1)	3.56
F- G	-3175 / 0	-91.8	-91.8	0.45 (1)	3.56
G- H	-3175 / 0	-91.8	-91.8	0.49 (1)	3.52
H- I	-2867 / 0	-91.8	-91.8	0.39 (1)	3.79
I- J	-3002 / 0	-91.8	-91.8	0.41 (1)	3.70
J- K	0 / 28	-91.8	-91.8	0.12 (1)	10.00
U- B	-2075 / 0	0.0	0.0	0.13 (1)	7.03
L- J	-2074 / 0	0.0	0.0	0.13 (1)	7.04
U- T	0 / 0	-18.5	-18.5	0.07 (1)	10.00
T- S	0 / 2706	-18.5	-18.5	0.38 (1)	10.00
S- R	0 / 2545	-18.5	-18.5	0.34 (1)	10.00
R- Q	0 / 2545	-18.5	-18.5	0.34 (1)	10.00
Q- P	0 / 3182	-18.5	-18.5	0.43 (1)	10.00
P- O	0 / 2547	-18.5	-18.5	0.35 (1)	10.00
O- N	0 / 2547	-18.5	-18.5	0.35 (1)	10.00
N- M	0 / 2705	-18.5	-18.5	0.38 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.07 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-366 / 0	0.08 (1)
C- S	-199 / 0	0.12 (1)
S- D	0 / 243	0.05 (1)
D- Q	0 / 908	0.20 (1)
Q- E	-550 / 0	0.30 (1)
E- P	-9 / 0	0.01 (1)
P- G	-552 / 0	0.30 (1)
P- H	0 / 897	0.20 (1)
N- H	0 / 250	0.06 (1)
N- I	-196 / 0	0.12 (1)
M- I	-368 / 0	0.08 (1)
B- T	0 / 2738	0.62 (1)
M- J	0 / 2737	0.62 (1)

TOTAL WEIGHT = 2 X 173 = 345 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.49/1.00 (D-E:1) , BC=0.43/1.00 (P-Q:1) , WB=0.62/1.00 (B-T:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

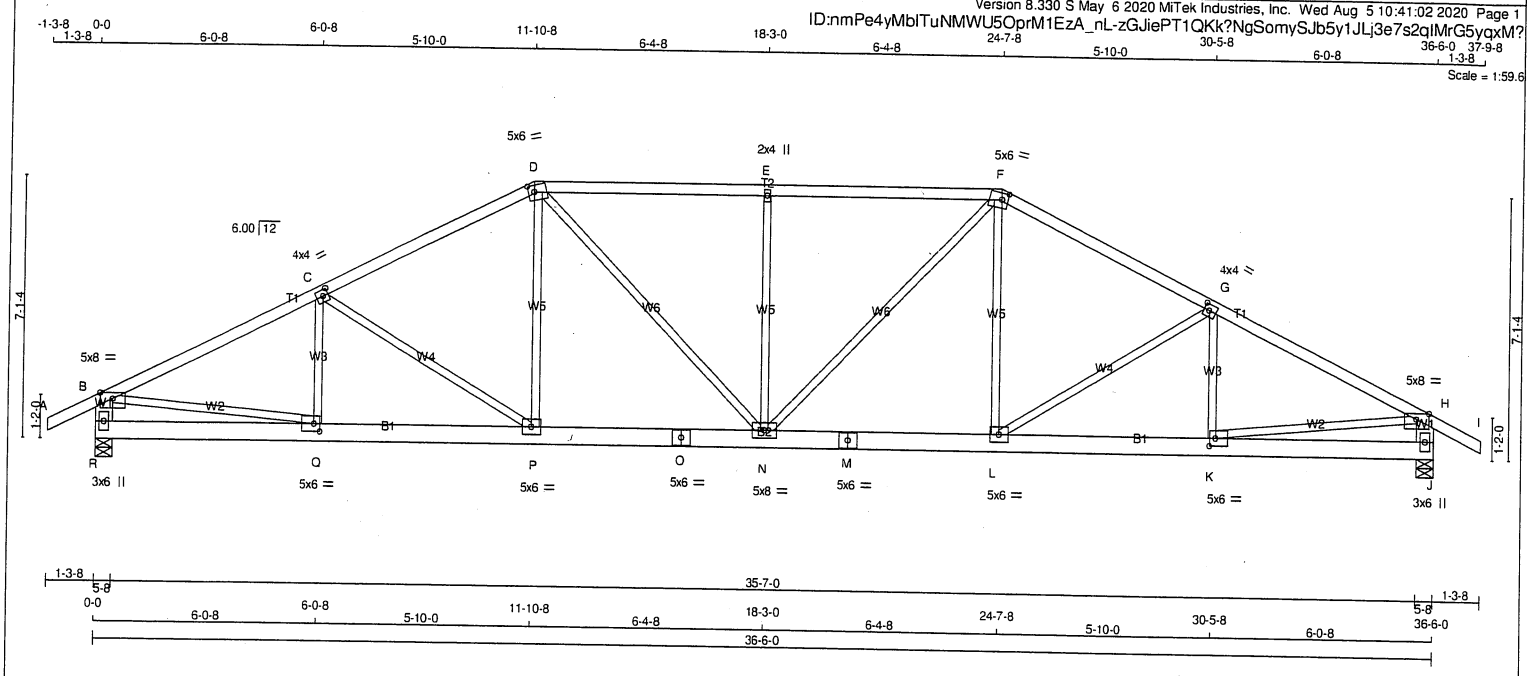
JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.62 (T) (INPUT = 1.00)

Structural component only
DWG# T-2017370

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T33	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:41:02 2020 Page 1
ID:nmPe4yMblTuNMWU5OprM1EzA_nL-zGJiePT1QKk?NgSomySJb5y1JLj3e7s2qIMrG5yqxM?



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.00
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BS-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.00
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2017371

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	2137	0	2137	0	0	5-8	5-8
J	2137	0	2137	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0
J	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
B- C	-3058 / 0	-91.8	-91.8	0.60 (1)	3.47	
C- D	-2720 / 0	-91.8	-91.8	0.54 (1)	3.70	
D- E	-2796 / 0	-91.8	-91.8	0.62 (1)	3.53	
E- F	-2796 / 0	-91.8	-91.8	0.62 (1)	3.53	
F- G	-2720 / 0	-91.8	-91.8	0.54 (1)	3.70	
G- H	-3058 / 0	-91.8	-91.8	0.60 (1)	3.47	
H- I	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
R- B	-2073 / 0	0.0	0.0	0.13 (1)	7.04	
J- H	-2073 / 0	0.0	0.0	0.13 (1)	7.04	
R- Q	0 / 0	-18.5	-18.5	0.08 (4)	10.00	
Q- P	0 / 2760	-18.5	-18.5	0.38 (1)	10.00	
P- O	0 / 2411	-18.5	-18.5	0.34 (1)	10.00	
O- N	0 / 2411	-18.5	-18.5	0.34 (1)	10.00	
N- M	0 / 2411	-18.5	-18.5	0.34 (1)	10.00	
M- L	0 / 2411	-18.5	-18.5	0.34 (1)	10.00	
L- K	0 / 2760	-18.5	-18.5	0.38 (1)	10.00	
K- J	0 / 0	-18.5	-18.5	0.08 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-271 / 6	0.07 (1)
C- P	-421 / 0	0.39 (1)
P- D	0 / 372	0.08 (1)
D- N	0 / 559	0.13 (1)
N- E	-718 / 0	0.59 (1)
N- F	0 / 559	0.13 (1)
L- F	0 / 372	0.08 (1)
L- G	-421 / 0	0.39 (1)
K- G	-271 / 6	0.07 (1)
B- Q	0 / 2783	0.63 (1)
K- H	0 / 2783	0.63 (1)

TOTAL WEIGHT = 2 X 171 = 343 lb
[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.62/1.00 (D-E:1) , BC=0.38/1.00 (P-Q:1) , WB=0.63/1.00 (B-Q:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

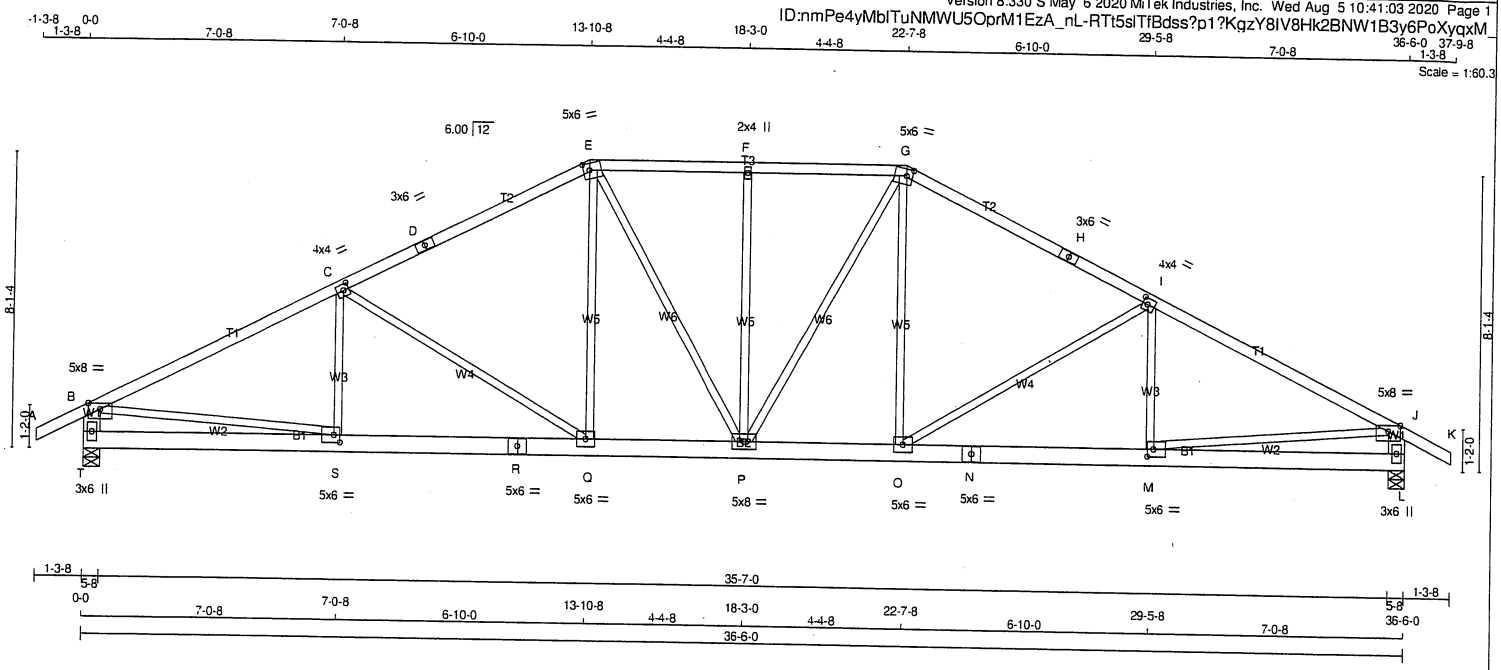
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.63 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T34	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:41:03 2020 Page 1
ID:nmPe4yMblTuNMWU5OprM1EzA_nL-RTt5slTfBdss?p1?KqzY8IV8Hk2BNW1B3y6PoXyqxM



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.00
N	BS-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.00
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2017372

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2137	0	2137	0	0	5-8	5-8
L	2137	0	2137	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0
L	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)		10.00
B- C	-3079 / 0	-91.8	-91.8	0.86 (1)		3.14
C- D	-2545 / 0	-91.8	-91.8	0.74 (1)		3.53
D- E	-2545 / 0	-91.8	-91.8	0.74 (1)		3.53
E- F	-2400 / 0	-91.8	-91.8	0.28 (1)		4.19
F- G	-2400 / 0	-91.8	-91.8	0.28 (1)		4.19
G- H	-2545 / 0	-91.8	-91.8	0.74 (1)		3.53
H- I	-2545 / 0	-91.8	-91.8	0.74 (1)		3.53
I- J	-3079 / 0	-91.8	-91.8	0.86 (1)		3.14
J- K	0 / 28	-91.8	-91.8	0.12 (1)		10.00
T- B	-2071 / 0	0.0	0.0	0.13 (1)		7.05
L- J	-2071 / 0	0.0	0.0	0.13 (1)		7.05
T- S	0 / 0	-18.5	-18.5	0.10 (4)		10.00
S- R	0 / 2784	-18.5	-18.5	0.39 (1)		10.00
R- Q	0 / 2784	-18.5	-18.5	0.39 (1)		10.00
Q- P	0 / 2251	-18.5	-18.5	0.31 (1)		10.00
P- O	0 / 2251	-18.5	-18.5	0.31 (1)		10.00
O- N	0 / 2784	-18.5	-18.5	0.39 (1)		10.00
N- M	0 / 2784	-18.5	-18.5	0.39 (1)		10.00
M- L	0 / 0	-18.5	-18.5	0.10 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-188 / 54	0.06 (1)
C- Q	-635 / 0	0.89 (1)
Q- E	0 / 467	0.11 (1)
E- P	0 / 302	0.07 (1)
P- F	-484 / 0	0.58 (1)
P- G	0 / 302	0.07 (1)
O- G	0 / 467	0.11 (1)
O- I	-635 / 0	0.89 (1)
M- I	-188 / 54	0.06 (1)
B- S	0 / 2801	0.63 (1)
M- J	0 / 2801	0.63 (1)

TOTAL WEIGHT = 2 X 178 = 356 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.86/1.00 (I-J:1) , BC=0.39/1.00 (M-O:1) , WB=0.89/1.00 (C-Q:1) , SSI=0.28/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

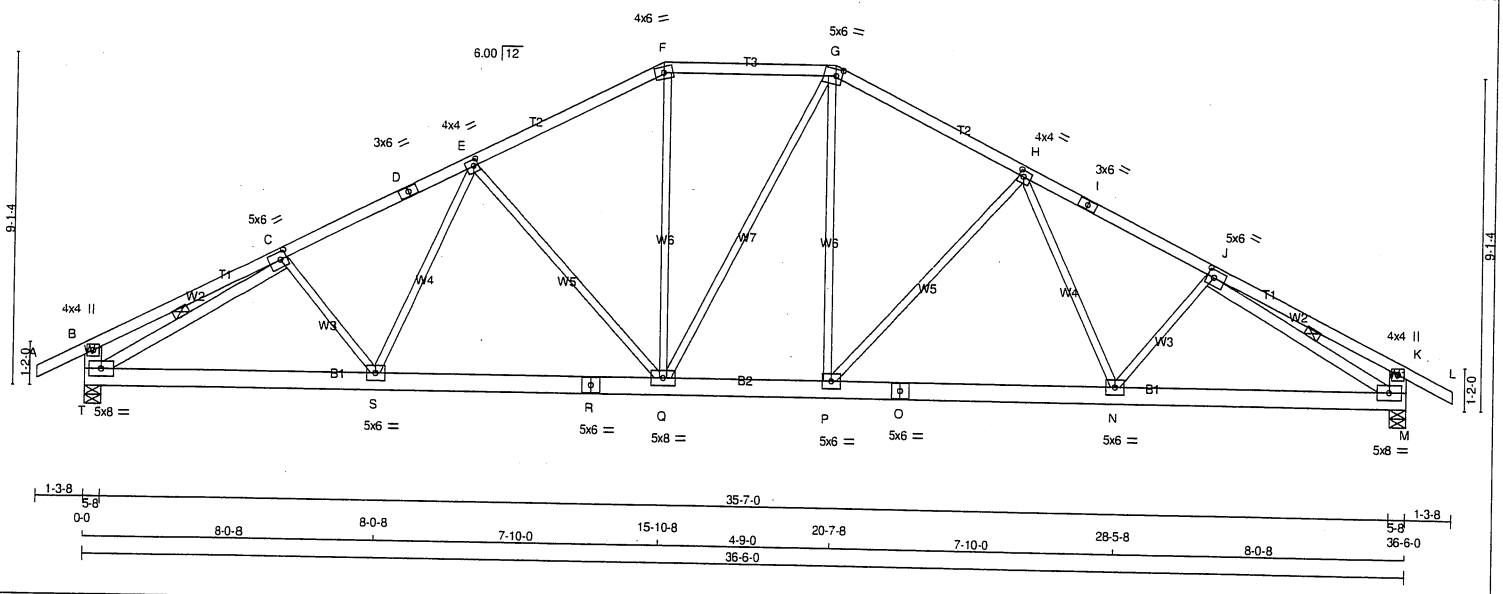
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.63 (S) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T35	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:41:04 2020 Page 1
ID:nmPe4yMblTuNMWU5OprM1EzA_nL-vfQT35UHyx_jdzcBuNUngW1Q_8N96ybLlcryJzyqxLz



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TTW-m	MT20	4.0	6.0		
G	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TS-t	MT20	3.0	6.0		
J	TMWW-t	MT20	5.0	6.0	2.50	2.25
K	TMV+p	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	8.0		
N, P, S						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
T	BMVW1-t	MT20	5.0	8.0		

Structural component only
DWG# T-2017373

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 2 X 181 = 362 lb [M][F]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2137	0	2137	0	0	5-8	5-8
M	2137	0	2137	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0
M	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-T, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 20	-91.8	-91.8	0.32 (1)	10.00
C- D	-2933 / 0	-91.8	-91.8	0.42 (1)	3.75
D- E	-2933 / 0	-91.8	-91.8	0.42 (1)	3.75
E- F	-2339 / 0	-91.8	-91.8	0.38 (1)	4.16
F- G	-2079 / 0	-91.8	-91.8	0.32 (1)	4.40
G- H	-2335 / 0	-91.8	-91.8	0.38 (1)	4.16
H- I	-2935 / 0	-91.8	-91.8	0.42 (1)	3.75
I- J	-2935 / 0	-91.8	-91.8	0.42 (1)	3.75
J- K	0 / 20	-91.8	-91.8	0.32 (1)	10.00
K- L	0 / 28	-91.8	-91.8	0.12 (1)	10.00
T- B	-324 / 0	0.0	0.0	0.02 (1)	7.81
M- K	-324 / 0	0.0	0.0	0.02 (1)	7.81
T- S	0 / 2702	-18.5	-18.5	0.40 (1)	10.00
S- R	0 / 2517	-18.5	-18.5	0.35 (1)	10.00
R- Q	0 / 2517	-18.5	-18.5	0.35 (1)	10.00
Q- P	0 / 2076	-18.5	-18.5	0.30 (1)	10.00
P- O	0 / 2517	-18.5	-18.5	0.36 (1)	10.00
O- N	0 / 2517	-18.5	-18.5	0.36 (1)	10.00
N- M	0 / 2704	-18.5	-18.5	0.40 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- S	-126 / 32	0.04 (1)
S- E	0 / 272	0.06 (1)
E- Q	-678 / 0	0.93 (1)
Q- F	0 / 629	0.14 (1)
Q- G	0 / 6	0.00 (1)
P- G	0 / 622	0.14 (1)
P- H	-681 / 0	0.93 (1)
H- N	0 / 277	0.06 (1)
N- J	-125 / 32	0.04 (1)
T- C	-3217 / 0	0.61 (1)
J- M	-3218 / 0	0.61 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.42/1.00 (H-J:1) , BC=0.40/1.00 (S-T:1) , WB=0.93/1.00 (H-P:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

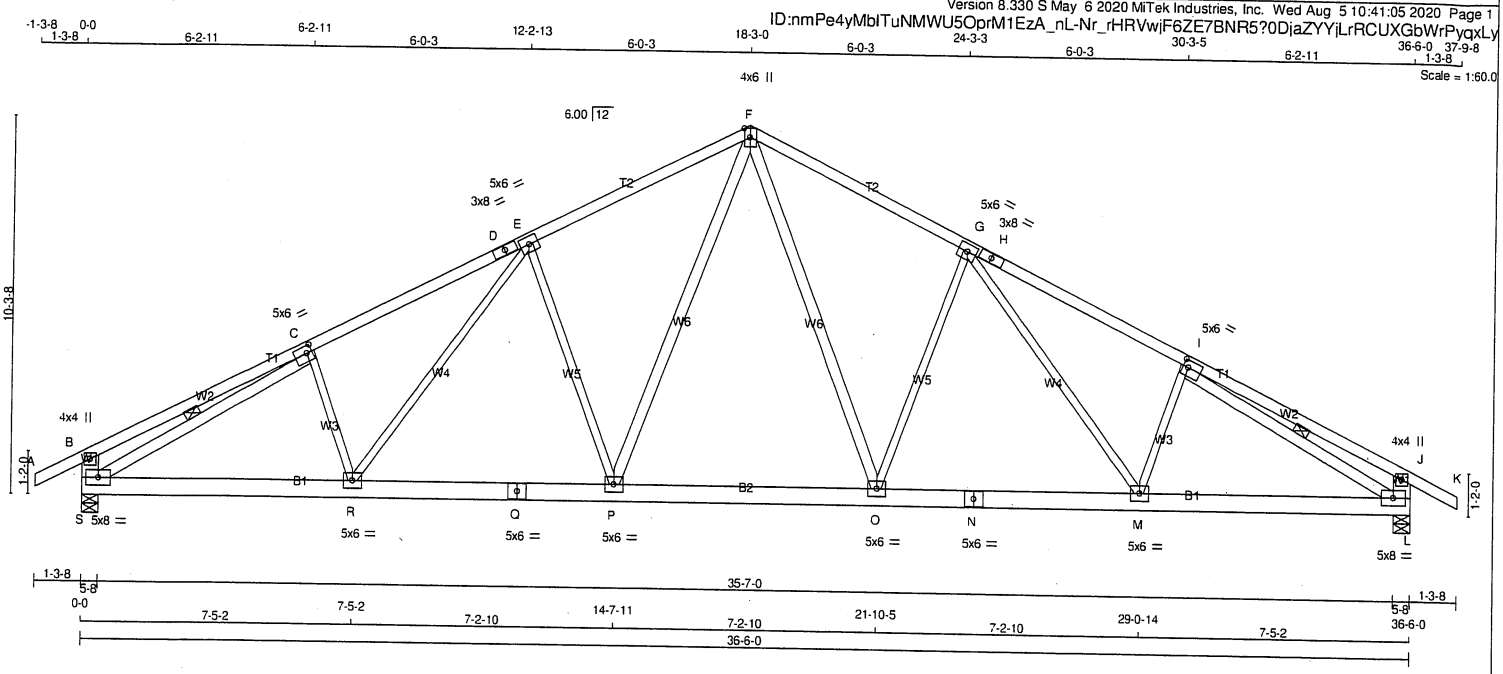
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.72 (J) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T37	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:41:05 2020 Page 1
ID:nmPe4yMbITuNMWU5OprM1EzA_nL-Nr_rHRVwjF6ZE7BNR5?0DjaZYYjLrRCUXGbWrPyqxLy



TOTAL WEIGHT = 187 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - O	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	1.75
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	5.0	6.0		
F	TTWW+p	MT20	4.0	6.0	Edge	
G	TMWW-t	MT20	5.0	6.0		
H	TS-t	MT20	3.0	8.0		
I	TMWW-t	MT20	5.0	6.0	2.25	1.75
J	TMV+p	MT20	4.0	4.0		
L	BMVW1-t	MT20	5.0	8.0		
M, O, P, R						
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
S	BMVW1-t	MT20	5.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2137	0	2137	0	0	5-8	5-8
L	2137	0	2137	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0
L	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-S, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 23	-91.8	-91.8	0.44 (1)	10.00
C- D	-2974 / 0	-91.8	-91.8	0.56 (1)	3.61
D- E	-2974 / 0	-91.8	-91.8	0.56 (1)	3.61
E- F	-2445 / 0	-91.8	-91.8	0.52 (1)	3.93
F- G	-2445 / 0	-91.8	-91.8	0.52 (1)	3.93
G- H	-2974 / 0	-91.8	-91.8	0.56 (1)	3.61
H- I	-2974 / 0	-91.8	-91.8	0.56 (1)	3.61
I- J	0 / 23	-91.8	-91.8	0.44 (1)	10.00
J- K	0 / 28	-91.8	-91.8	0.12 (1)	10.00
S- B	-352 / 0	0.0	0.0	0.02 (1)	7.81
L- J	-352 / 0	0.0	0.0	0.02 (1)	7.81
S- R	0 / 2738	-18.5	-18.5	0.41 (1)	10.00
R- Q	0 / 2416	-18.5	-18.5	0.34 (1)	10.00
Q- P	0 / 2416	-18.5	-18.5	0.34 (1)	10.00
P- O	0 / 1851	-18.5	-18.5	0.27 (1)	10.00
O- N	0 / 2416	-18.5	-18.5	0.34 (1)	10.00
N- M	0 / 2416	-18.5	-18.5	0.34 (1)	10.00
M- L	0 / 2738	-18.5	-18.5	0.41 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- O	0 / 917	0.15 (1)
O- G	-761 / 0	0.78 (1)
G- M	0 / 428	0.10 (1)
M- I	-238 / 6	0.07 (1)
P- F	0 / 917	0.15 (1)
E- P	-761 / 0	0.78 (1)
R- E	0 / 428	0.10 (1)
C- R	-238 / 6	0.07 (1)
S- C	-3232 / 0	0.76 (1)
I- L	-3232 / 0	0.76 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.56/1.00 (G-I:1) , BC=0.41/1.00 (R-S:1) , WB=0.78/1.00 (E-P:1) , SSI=0.23/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (S) (INPUT = 0.90)
JSI METAL= 0.74 (C) (INPUT = 1.00)

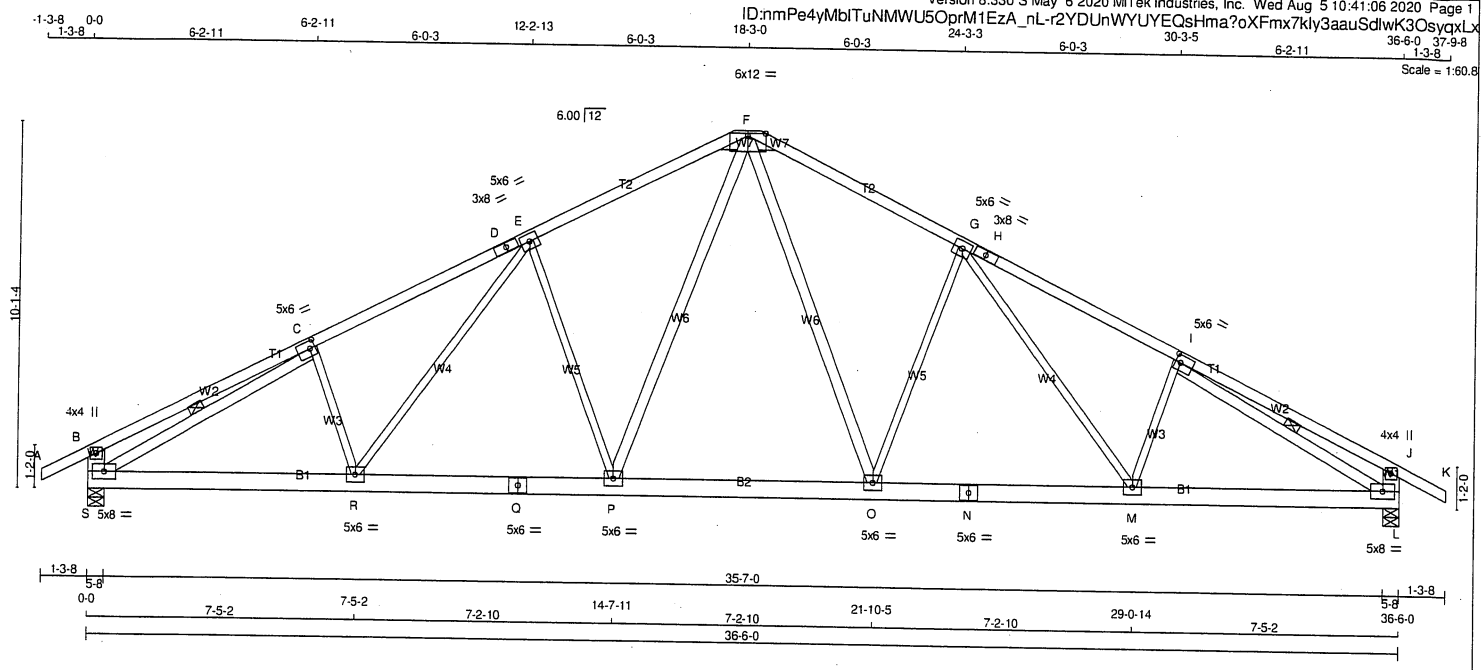
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017374

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T37CP	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:41:06 2020 Page 1
ID:nmPe4yMblTuNMWU5OprM1EzA_nL-r2YDUnWYUYEQsHma?oXFmx7kly3aauSdlwK3OsyqxLx



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - O	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	5.0	6.0		
F	TMTMWWWWMT20		6.0	12.0	0.75	6.00
G	TMWW-t	MT20	5.0	6.0		
H	TS-t	MT20	3.0	8.0		
I	TMWW-t	MT20	5.0	6.0	2.50	1.75
J	TMV+p	MT20	4.0	4.0		
L	BMVW1-t	MT20	5.0	8.0		
M, O, P, R						
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
S	BMVW1-t	MT20	5.0	8.0		

Structural component only
DWG# T-2017375

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2137	0	2137	0	0	5-8	5-8
L	2137	0	2137	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
	MAX./MIN. COMPONENT REACTIONS						
S	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0
L	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-S, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 23	-91.8	-91.8	0.44 (1)	10.00
C- D	-2974 / 0	-91.8	-91.8	0.56 (1)	3.61
D- E	-2974 / 0	-91.8	-91.8	0.56 (1)	3.61
E- F	-2445 / 0	-91.8	-91.8	0.52 (1)	3.93
F- G	-2445 / 0	-91.8	-91.8	0.52 (1)	3.93
G- H	-2974 / 0	-91.8	-91.8	0.56 (1)	3.61
H- I	-2974 / 0	-91.8	-91.8	0.56 (1)	3.61
I- J	0 / 23	-91.8	-91.8	0.44 (1)	10.00
J- K	0 / 28	-91.8	-91.8	0.12 (1)	10.00
S- B	-352 / 0	0.0	0.0	0.02 (1)	7.81
L- J	-352 / 0	0.0	0.0	0.02 (1)	7.81
S- R	0 / 2738	-18.5	-18.5	0.41 (1)	10.00
R- Q	0 / 2416	-18.5	-18.5	0.34 (1)	10.00
Q- P	0 / 2416	-18.5	-18.5	0.34 (1)	10.00
P- O	0 / 1851	-18.5	-18.5	0.27 (1)	10.00
O- N	0 / 2416	-18.5	-18.5	0.34 (1)	10.00
N- M	0 / 2416	-18.5	-18.5	0.34 (1)	10.00
M- L	0 / 2738	-18.5	-18.5	0.41 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- O	0 / 917	0.15 (1)
O- G	-761 / 0	0.78 (1)
G- M	0 / 428	0.10 (1)
M- I	-238 / 6	0.07 (1)
P- F	0 / 917	0.15 (1)
E- P	-761 / 0	0.78 (1)
R- E	0 / 428	0.10 (1)
C- R	-238 / 6	0.07 (1)
S- C	-3232 / 0	0.76 (1)
I- L	-3232 / 0	0.76 (1)

TOTAL WEIGHT = 2 X 188 = 375 lb
[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.56/1.00 (C-E:1) , BC=0.41/1.00 (R-S:1) , WB=0.78/1.00 (E-P:1) , SSI=0.23/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

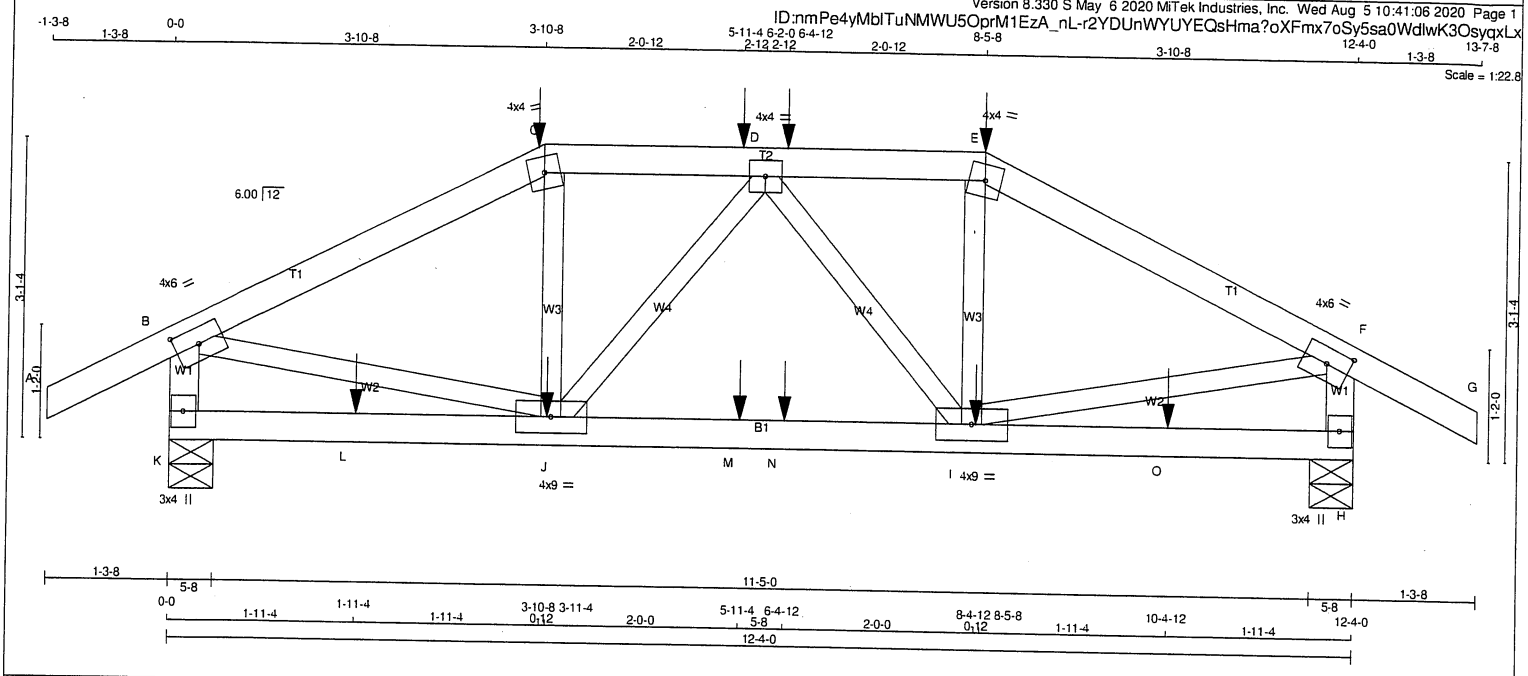
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.74 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T38	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:41:06 2020 Page 1
ID:nmPe4yMbITuNMWU5OprM1EzA_nL-r2YDUnWYUYEQsHma?oXFmx7oSy5sa0WdlwK3OsyqxLx



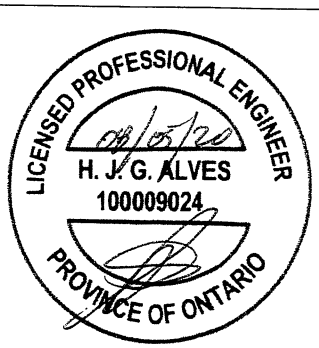
LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0	2.00	3.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMV1+p	MT20	3.0	4.0		



Structural component only
DWG# T-2017376

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1084	0	1084	0	0	5-8	5-8
H	1084	0	1084	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	764	516 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0
H	764	516 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.13 (1)	10.00
B- C	-1170 / 0	-91.8	-91.8	0.29 (1)	5.50
C- D	-1049 / 0	-91.8	-91.8	0.08 (1)	6.03
D- E	-1049 / 0	-91.8	-91.8	0.08 (1)	6.03
E- F	-1170 / 0	-91.8	-91.8	0.29 (1)	5.50
F- G	0 / 28	-91.8	-91.8	0.13 (1)	10.00
K- B	-1049 / 0	0.0	0.0	0.12 (1)	7.64
H- F	-1049 / 0	0.0	0.0	0.12 (1)	7.64
K- L	0 / 0	-18.5	-18.5	0.10 (4)	10.00
L- J	0 / 0	-18.5	-18.5	0.10 (4)	10.00
J- M	0 / 1172	-18.5	-18.5	0.26 (1)	10.00
M- N	0 / 1172	-18.5	-18.5	0.26 (1)	10.00
N- I	0 / 1172	-18.5	-18.5	0.26 (1)	10.00
I- O	0 / 0	-18.5	-18.5	0.10 (4)	10.00
O- H	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	0 / 83	0.03 (4)
J- D	-204 / 0	0.05 (1)
D- I	-204 / 0	0.05 (1)
I- E	0 / 83	0.03 (4)
B- J	0 / 1069	0.26 (1)
I- F	0 / 1069	0.26 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-205	-205	---	BACK	VERT	TOTAL	---	C1
D	5-11-4	-42	-42	---	BACK	VERT	TOTAL	---	C1
D	6-4-12	-42	-42	---	BACK	VERT	TOTAL	---	C1
E	8-5-8	-205	-205	---	BACK	VERT	TOTAL	---	C1
I	8-4-12	-12	-12	---	BACK	VERT	TOTAL	---	C1
J	3-11-4	-12	-12	---	BACK	VERT	TOTAL	---	C1
L	1-11-4	-8	-10	---	BACK	VERT	TOTAL	---	C1
M	5-11-4	-12	-12	---	BACK	VERT	TOTAL	---	C1
N	6-4-12	-12	-12	---	BACK	VERT	TOTAL	---	C1
O	10-4-12	-8	-10	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 50 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.26/1.00 (I-J:1) , WB=0.26/1.00 (F-I:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

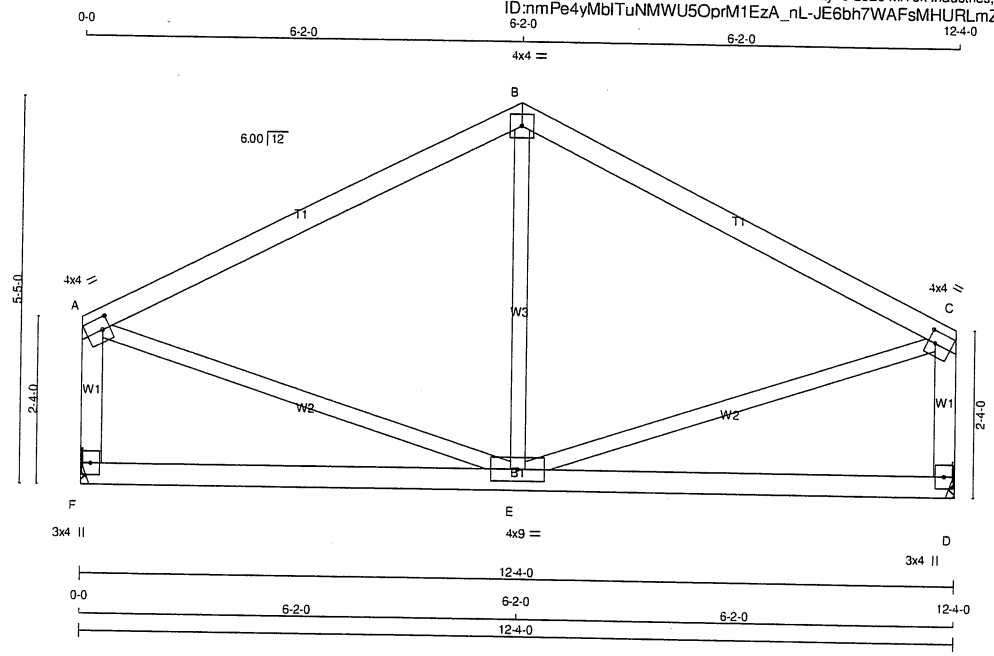
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (I) (INPUT = 0.90)
JSI METAL= 0.30 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T40	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:41:07 2020 Page 1
ID:nmPe4yMbITuNMWU5OprM1EzA_nL-JE6bh7WAFsMHURLmZW2UI8fwhMS?JWJn_a4cwlyqxLw



TOTAL WEIGHT = 2 X 48 = 96 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TTW-p	MT20	4.0	4.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	680	0	680	0	0	MECHANICAL	
D	680	0	680	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	481	316 / 0	0 / 0	0 / 0	0 / 0	165 / 0	0 / 0
D	481	316 / 0	0 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-477 / 0	-91.8	-91.8	0.45 (1)	6.25	E- B	-139 / 55	0.06 (1)
B- C	-477 / 0	-91.8	-91.8	0.45 (1)	6.25	A- E	0 / 449	0.10 (1)
F- A	-636 / 0	0.0	0.0	0.08 (1)	7.81	E- C	0 / 449	0.10 (1)
D- C	-636 / 0	0.0	0.0	0.08 (1)	7.81			
F- E	0 / 0	-18.5	-18.5	0.20 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.20 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.45/1.00 (A-B:1) , BC=0.20/1.00 (E-F:4) , WB=0.10/1.00 (C-E:1) , SSI=0.20/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

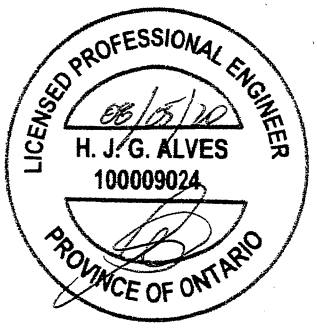
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (A) (INPUT = 0.90)
JSI METAL= 0.21 (A) (INPUT = 1.00)

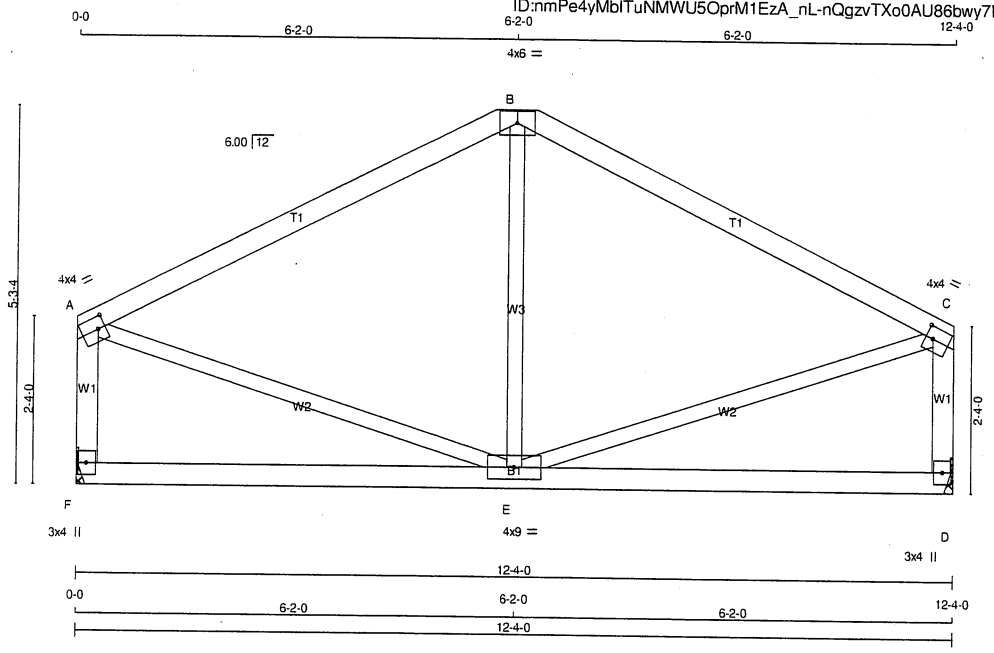


Structural component only
DWG# T-2017377

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T40CP	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:41:08 2020 Page 1
ID:nmPe4yMbITuNMWU5OprM1EzA_nL-nQgzvTXo0AU86bwy7DZjrMC5RIoE2zYwDEpASkyqxLv



TOTAL WEIGHT = 48 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TTW-p	MT20	4.0	6.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	680	0	680	0	0	MECHANICAL	
D	680	0	680	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	481	316 / 0	0 / 0	0 / 0	0 / 0	165 / 0	0 / 0
D	481	316 / 0	0 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-477 / 0	-91.8	-91.8	0.45 (1)	6.25	E- B	-139 / 55	0.06 (1)
B- C	-477 / 0	-91.8	-91.8	0.45 (1)	6.25	A- E	0 / 449	0.10 (1)
F- A	-636 / 0	0.0	0.0	0.08 (1)	7.81	E- C	0 / 449	0.10 (1)
D- C	-636 . 0	0.0	0.0	0.08 (1)	7.81			
F- E	0 / 0	-18.5	-18.5	0.20 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.20 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.45/1.00 (A-B:1) , BC=0.20/1.00 (E-F:4) , WB=0.10/1.00 (C-E:1) , SSI=0.20/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (A) (INPUT = 0.90)
JSI METAL= 0.21 (A) (INPUT = 1.00)

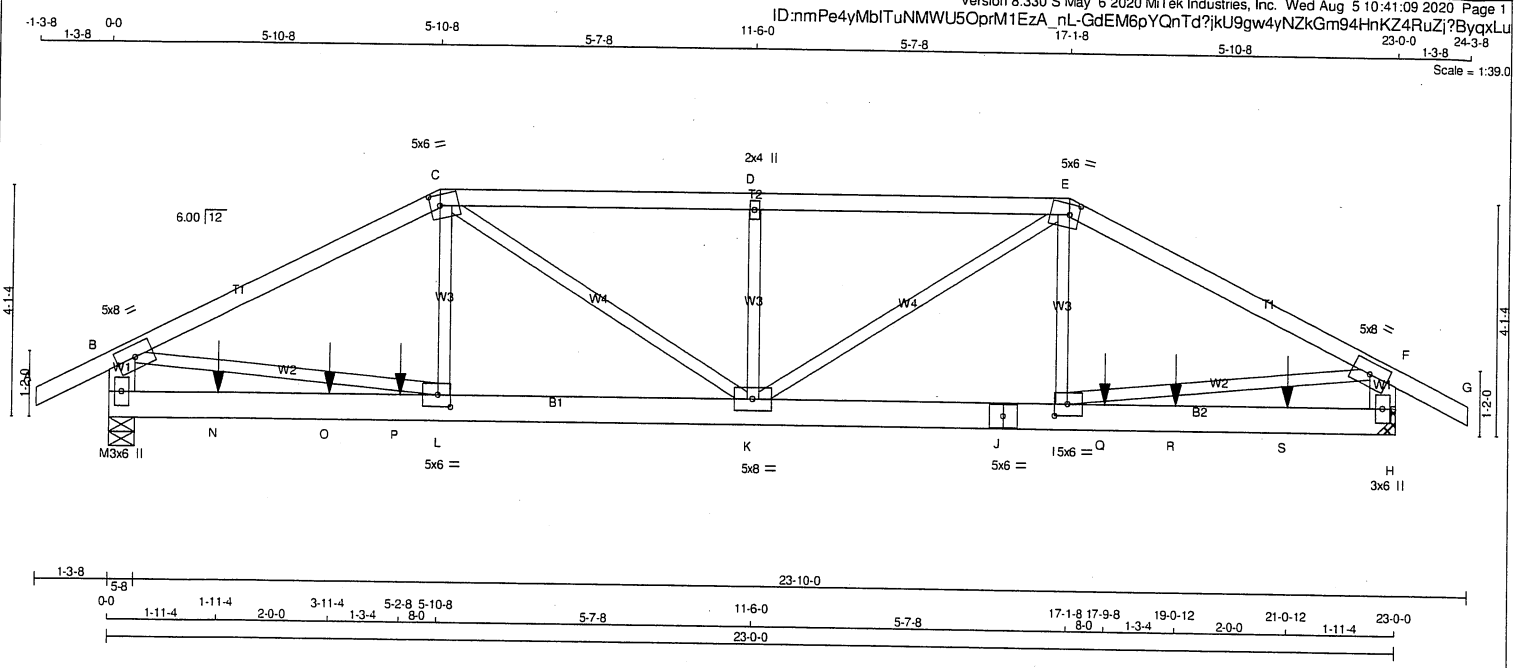
LICENSED PROFESSIONAL ENGINEER
08/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017378

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T41	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:41:09 2020 Page 1
ID:nmPe4yMbITuNMWU5OprM1EzA_nL-GdEM6pYQnTd?jkU9gw4yNZkGm94HnKZ4RuZj?ByqxLu



TOTAL WEIGHT = 2 X 102 = 204 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- G	1	12	TOP
M- B	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	SIDE(0.0)
J- H	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

Structural component only
DWG# T-2017379 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	3128	0	3128	0	0	5-8	5-8
H	3128	0	3128	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	2207	1478 / 0	0 / 0	0 / 0	0 / 0	729 / 0	0 / 0
H	2207	1478 / 0	0 / 0	0 / 0	0 / 0	729 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B		0 / 28	-91.8	-91.8	0.07 (1)	10.00	L- C	0 / 1331	0.16 (1)
B- C		-4484 / 0	-91.8	-91.8	0.48 (1)	4.11	C- K	0 / 370	0.05 (1)
C- D		-4355 / 0	-91.8	-91.8	0.37 (1)	4.24	K- D	-621 / 0	0.08 (1)
D- E		-4355 / 0	-91.8	-91.8	0.37 (1)	4.24	K- E	0 / 370	0.05 (1)
E- F		-4484 / 0	-91.8	-91.8	0.48 (1)	4.11	I- E	0 / 1331	0.16 (1)
F- G		0 / 28	-91.8	-91.8	0.07 (1)	10.00	B- L	0 / 4042	0.50 (1)
M- B		-2923 / 0	0.0	0.0	0.10 (1)	7.81	I- F	0 / 4042	0.50 (1)
H- F		-2923 / 0	0.0	0.0	0.10 (1)	7.81			
M- N		0 / 0	-18.5	-18.5	0.26 (1)	10.00			
N- O		0 / 0	-18.5	-18.5	0.26 (1)	10.00			
O- P		0 / 0	-18.5	-18.5	0.26 (1)	10.00			
P- L		0 / 0	-18.5	-18.5	0.26 (1)	10.00			
L- K		0 / 4046	-18.5	-18.5	0.47 (1)	10.00			
K- J		0 / 4046	-18.5	-18.5	0.47 (1)	10.00			
J- I		0 / 4046	-18.5	-18.5	0.47 (1)	10.00			
I- Q		0 / 0	-18.5	-18.5	0.26 (1)	10.00			
Q- R		0 / 0	-18.5	-18.5	0.26 (1)	10.00			
R- S		0 / 0	-18.5	-18.5	0.26 (1)	10.00			
S- H		0 / 0	-18.5	-18.5	0.26 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	1-11-4	-25	-26	---	FRONT	VERT	TOTAL	---	C1
O	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	5-2-8	-1684	-1684	---	FRONT	VERT	TOTAL	---	C1
Q	17-9-8	-1684	-1684	---	FRONT	VERT	TOTAL	---	C1
R	19-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
S	21-0-12	-25	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.48/1.00 (B-C:1) , BC=0.47/1.00 (K-L:1) , WB=0.50/1.00 (B-L:1) , SSI=0.54/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (I) (INPUT = 0.90)
JSI METAL= 0.51 (J) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
409989	T41	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:nmPe4yMblTuNMWU5OprM1EzA_nL-GdEM6pYQnTd?jkU9gw4yNZkGm94HnKZ4RuZj?BygxLu

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMVW-t	MT20	5.0	8.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0	2.50	2.75
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMV1+p	MT20	3.0	6.0		

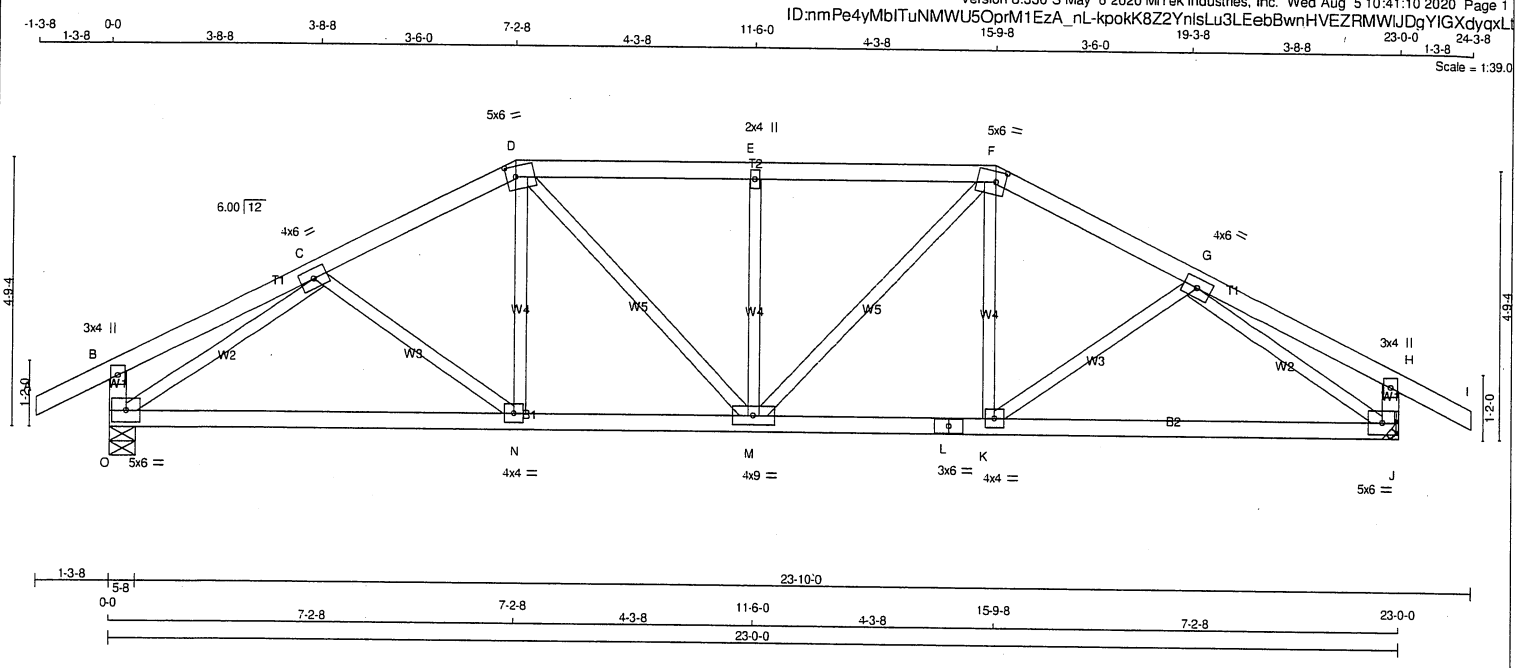
Structural component only
DWG# T-2017379 rev

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T42	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:41:10 2020 Page 1

ID:nmPe4yMbITuNMWU5OprM1EzA_nL-kpokK8Z2YnlsLu3LEebBwnHVEZRMWlJDgYIGXdyqxLt



TOTAL WEIGHT = 93 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1392	0	1392	0	0	5-8	5-8
J	1392	0	1392	0	0	MECHANICAL	IN-SX

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	982	658 / 0	0 / 0	0 / 0	0 / 0	324 / 0	0 / 0
J	982	658 / 0	0 / 0	0 / 0	0 / 0	324 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 15	-91.8	-91.8	0.17 (1)	10.00
C- D	-1604 / 0	-91.8	-91.8	0.16 (1)	5.08
D- E	-1668 / 0	-91.8	-91.8	0.24 (1)	4.91
E- F	-1668 / 0	-91.8	-91.8	0.24 (1)	4.91
F- G	-1604 / 0	-91.8	-91.8	0.16 (1)	5.08
G- H	0 / 15	-91.8	-91.8	0.17 (1)	10.00
H- I	0 / 28	-91.8	-91.8	0.12 (1)	10.00
O- B	-257 / 0	0.0	0.0	0.03 (1)	7.81
J- H	-257 / 0	0.0	0.0	0.03 (1)	7.81
O- N	0 / 1464	-18.5	-18.5	0.35 (1)	10.00
N- M	0 / 1424	-18.5	-18.5	0.34 (1)	10.00
M- L	0 / 1424	-18.5	-18.5	0.34 (1)	10.00
L- K	0 / 1424	-18.5	-18.5	0.34 (1)	10.00
K- J	0 / 1464	-18.5	-18.5	0.35 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	-55 / 39	0.02 (1)
N- D	0 / 173	0.05 (4)
D- M	0 / 353	0.08 (1)
M- E	-478 / 0	0.16 (1)
M- F	0 / 353	0.08 (1)
K- F	0 / 173	0.05 (4)
K- G	-55 / 39	0.02 (1)
O- C	-1819 / 0	0.60 (1)
G- J	-1819 / 0	0.60 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.24/1.00 (D-E:1) , BC=0.35/1.00 (N-O:1) , WB=0.60/1.00 (C-O:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

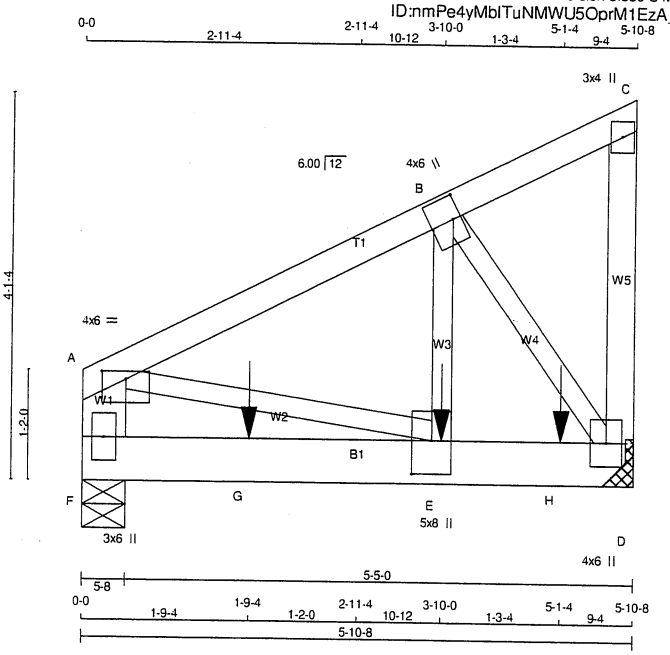
JSI GRIP= 0.78 (G) (INPUT = 0.90)
JSI METAL= 0.52 (L) (INPUT = 1.00)

Structural component only
DWG# T-2017380

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
409989	T43	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:41:11 2020 Page 1
ID:nmPe4yMblTuNMWU5OprM1EzA_nL-C?L6XUZhJ5tjz2eXoL6QT_qinzm2FFKMvC2q33yqxLs



TOTAL WEIGHT = 2 X 30 = 60 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	11	SIDE(244.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	2	SIDE(567.0)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW+t	MT20	4.0	6.0	3.00	1.00
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	5.0	8.0	4.25	2.50

Structural component only
DWG# T-2017381

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1827	0	1827	0	0	5-8	5-8
D	3652	0	3652	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1285	883 / 0	0 / 0	0 / 0	0 / 0	401 / 0	0 / 0
D	2573	1744 / 0	0 / 0	0 / 0	0 / 0	830 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1591 / 0	0.0	0.0	0.06 (1)	7.81		A- E	0 / 2084	0.26 (1)
A- B	-2276 / 0	-91.8	-91.8	0.12 (1)	5.80		E- B	0 / 3391	0.42 (1)
B- C	-16 / 0	-91.8	-91.8	0.05 (1)	6.25		B- D	-3380 / 0	0.37 (1)
D- C	-59 / 0	0.0	0.0	0.01 (1)	7.81				
F- G	0 / 0	-18.5	-18.5	0.14 (1)	10.00				
G- E	0 / 0	-18.5	-18.5	0.14 (1)	10.00				
E- H	0 / 2043	-18.5	-18.5	0.39 (1)	10.00				
H- D	0 / 2043	-18.5	-18.5	0.39 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-10-0	-3109	-3109	---	BACK	VERT	TOTAL	---	C1
G	1-9-4	-346	-346	---	TOP	VERT	TOTAL	---	C1
H	5-1-4	-1376	-1376	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.39/1.00 (D-E:1) , WB=0.42/1.00 (B-E:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

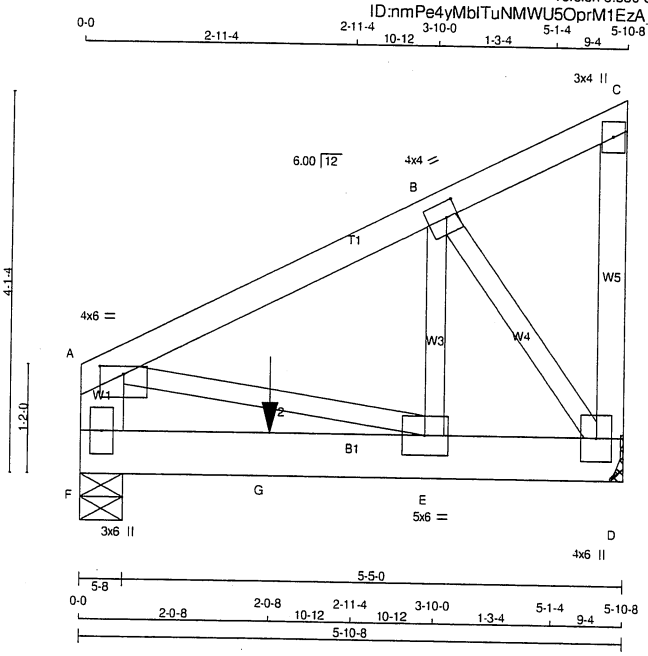
JSI GRIP= 0.83 (E) (INPUT = 0.90)
JSI METAL= 0.52 (E) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
409989	T43Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:41:12 2020 Page 1
ID:nmPe4yMbITuNMWU5OprM1EzA_nL-gBvUlqaJ4O?aaCDjM3df?CMtyN7V_mWW8snNbVyqxLr



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(87.3)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2017382

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1498	0	1498	0	0	5-8	5-8
D	1189	0	1189	0	0	IN-SX MECHANICAL	IN-SX

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2.00

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1056	711 / 0	0 / 0	0 / 0	0 / 0	346 / 0	0 / 0
D	839	560 / 0	0 / 0	0 / 0	0 / 0	279 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- A	-805 / 0	0.0	0.0	0.03 (1)		7.81
A- B	-1032 / 0	-91.8	-91.8	0.10 (1)		6.25
B- C	-22 / 0	-91.8	-91.8	0.08 (1)		6.25
D- C	-44 / 0	0.0	0.0	0.01 (1)		7.81
F- G	0 / 0	-193.1	-193.1	0.37 (1)		10.00
G- E	0 / 0	-193.1	-193.1	0.37 (1)		10.00
E- D	0 / 934	-193.1	-193.1	0.27 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- E	0 953	0.12 (1)
E- B	0 / 1352	0.17 (1)
B- D	-1545 / 0	0.17 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-8	-1014	-1014	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 30 = 60 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 8-8-0
END DISTANCE = 5-10-8
END SPAN CARRIED = 8-8-0
END WALL WIDTH = 4-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.10/1.00 (A-B:1) , BC=0.37/1.00 (E-F:1) , WB=0.17/1.00 (B-E:1) , SSI=0.35/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

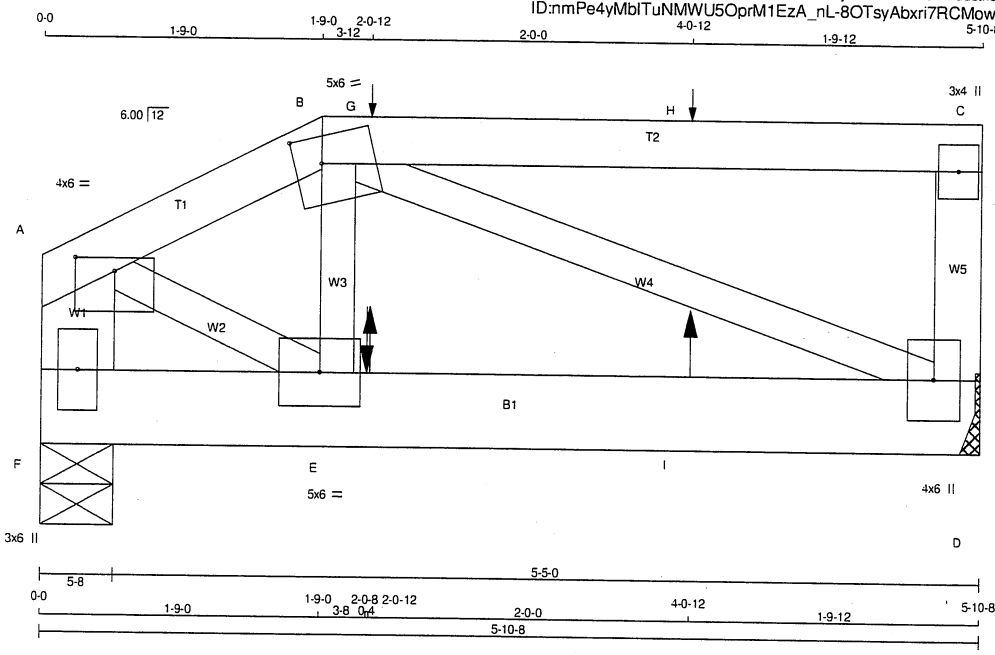
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (B) (INPUT = 0.90)
JSI METAL= 0.21 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T44	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:41:13 2020 Page 1
ID:nmPe4yMblTuNMWU5OprM1EzA_nL-8OTsyAbxri7RCMowvm9uYPv1wmXpjDmfMWXx8yyqxLq



TOTAL WEIGHT = 2 X 26 = 51 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	TOP
B- C	1	12	SIDE(61.0)
C- D	1	12	TOP
F- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	10	SIDE(244.1)
WEBS : (0.122"X3") SPIRAL NAILS			
E- B	1	2	SIDE(76.3)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00

LICENSED PROFESSIONAL ENGINEER
08/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017383

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	663	0	663	0	0	MECHANICAL	
F	1067	0	1067	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = ~~2-0-0~~

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	467	315 / 0	0 / 0	0 / 0	0 / 0	153 / 0	0 / 0
F	751	512 / 0	0 / 0	0 / 0	0 / 0	239 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	-1097 / 0	-91.8	-91.8	0.03 (1)		6.25	E- B	0 / 636	0.08 (1)
B- G	0 / 0	-91.8	-91.8	0.15 (1)		10.00	B- D	-1098 / 0	0.17 (1)
G- H	0 / 0	-91.8	-91.8	0.15 (1)		10.00	A- E	0 / 1061	0.13 (1)
H- C	0 / 0	-91.8	-91.8	0.15 (1)		10.00			
D- C	-189 / 0	0.0	0.0	0.01 (1)		7.81			
F- A	-984 / 0	0.0	0.0	0.03 (1)		7.81			
F- E	0 / 0	-18.5	-18.5	0.05 (1)		10.00			
E- I	0 / 1019	-18.5	-18.5	0.11 (1)		10.00			
I- D	0 / 1019	-18.5	-18.5	0.11 (1)		10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-0-8	-1097	-1097	---	BACK	VERT	TOTAL	---	C1
E	2-0-12	8	1	13	FRONT	VERT	TOTAL	---	C1
G	2-0-12	1	1	78	FRONT	VERT	TOTAL	---	C1
H	4-0-12	1	1	104	FRONT	VERT	TOTAL	---	C1
I	4-0-12	8	1	13	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.11/1.00 (D-E:1) , WB=0.17/1.00 (B-D:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (D) (INPUT = 0.90)
JSI METAL= 0.26 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
409989	T44	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:nmPe4yMbITuNMWU5OprM1EzA_nL-8OTsyAbxri7RCMowvm9uYPv1wmXpjDmfMWXx8yyqxLg

PLATES (table is in inches)

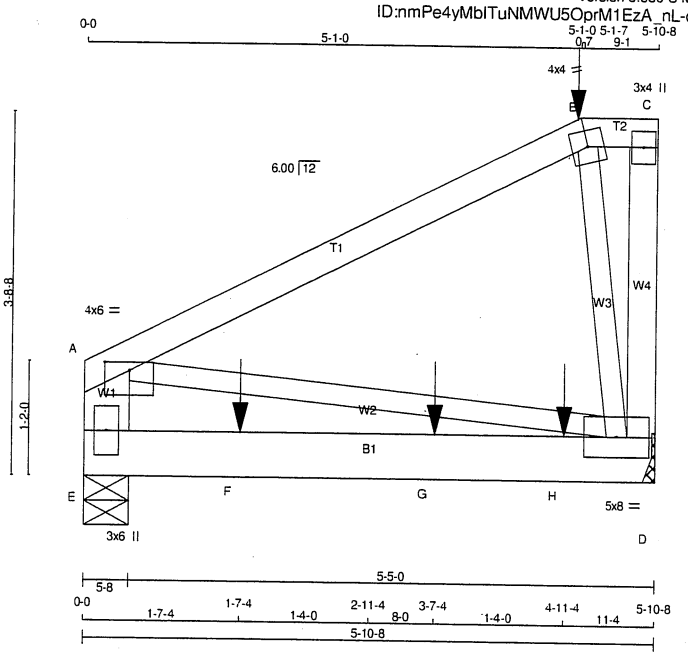
JT	TYPE	PLATES	W	LEN	Y	X
B	TTWW-m	MT20	5.0	6.0	2.00	2.00
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2017383

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
409989	T45	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:41:14 2020 Page 1
ID:nmPe4yMbITuNMWU5OprM1EzA_nL-ca1F9WcZc0FlqWN6TUg74dSBRAkWSimpbAGUgOyqxLp



TOTAL WEIGHT = 4 X 29 = 116 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - A	2x6	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	SIDE(61.0)
B- C	1	12	SIDE(61.0)
C- D	1	12	TOP
E- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
E- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TTW-m	MT20	4.0	4.0		

LICENSED PROFESSIONAL ENGINEER
09/5/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017384 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1703	0	1703	0	0	MECHANICAL	
E	1201	0	1201	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2.00

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1198	822 / 0	0 / 0	0 / 0	0 / 0	376 / 0	0 / 0
E	847	571 / 0	0 / 0	0 / 0	0 / 0	276 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-123 / 0	-91.8	-91.8	0.23 (1)		6.25	B- D	-498 / 0	0.06 (1)
B- C	0 / 0	-91.8	-91.8	0.00 (1)		10.00	A- D	0 / 111	0.01 (1)
D- C	-35 / 0	0.0	0.0	0.00 (1)		7.81			
E- A	-304 / 0	0.0	0.0	0.01 (1)		7.81			
E- F	0 / 0	-18.5	-18.5	0.66 (1)		10.00			
F- G	0 / 0	-18.5	-18.5	0.66 (1)		10.00			
G- H	0 / 0	-18.5	-18.5	0.66 (1)		10.00			
H- D	0 / 0	-18.5	-18.5	0.66 (1)		10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	5-1-7	-271	-271	---	FRONT	VERT	TOTAL	---	C1
F	1-7-4	-661	-661	---	BACK	VERT	TOTAL	---	C1
G	3-7-4	-661	-661	---	BACK	VERT	TOTAL	---	C1
H	4-11-4	-662	-662	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 824 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 436 (0.16")

CSI: TC=0.23/1.00 (A-B:1) , BC=0.66/1.00 (D-E:1) , WB=0.06/1.00 (B-D:1) , SSI=0.40/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
409989	T45	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:nmPe4yMbITuNMWU5OprM1EzA_nL-ca1F9WcZc0FlqWN6TUg74dSBRAkWSimpbAGUgOyqxLp

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMV+p	MT20	3.0	4.0		
D	BMVWW1-t	MT20	5.0	8.0		
E	BMV1+p	MT20	3.0	6.0		

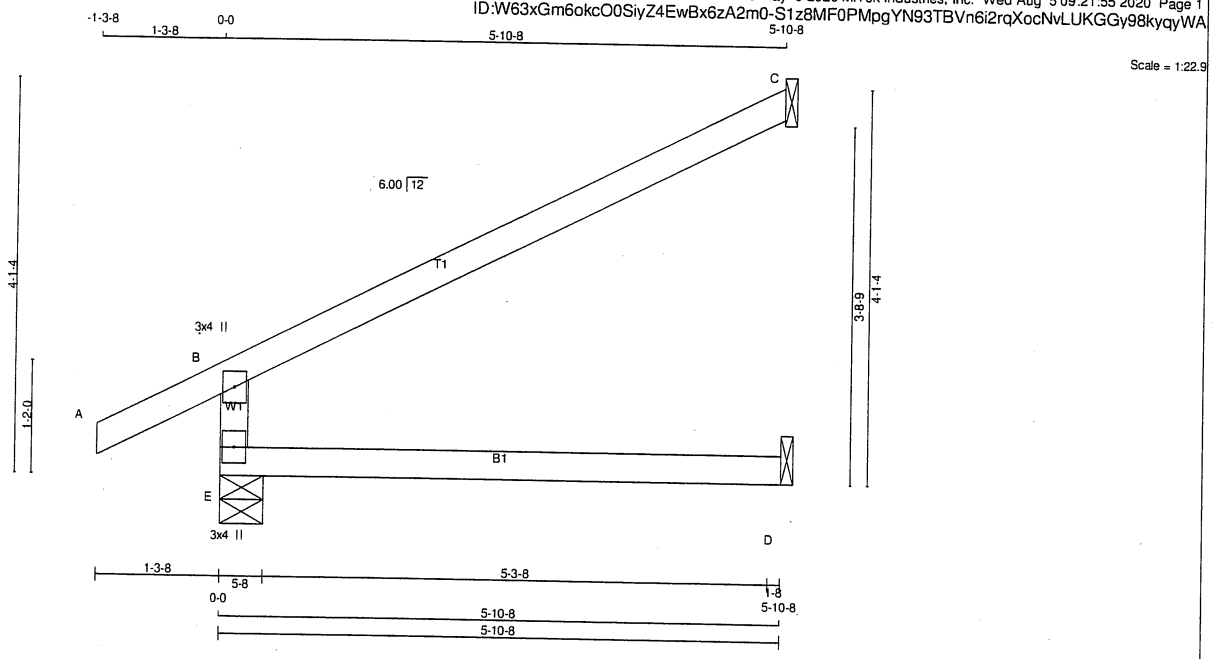
LICENSED PROFESSIONAL ENGINEER
08/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017384 YL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	J1	25	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:21:55 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-S1z8MF0PMpgYN93TBVn6i2rqXocNvLUKGGy98kyqyWA



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	525	0	525	0	0	5-8	5-8
C	202	0	202	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	369	257 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
E- B	-461 / 0	0.0	0.0	0.13 (4)	7.81			
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B- C	-30 / 0	-91.8	-91.8	0.54 (1)	6.25			
E- D	0 0	-18.5	-18.5	0.13 (4)	10.00			

TOTAL WEIGHT = 25 X 17 = 420 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

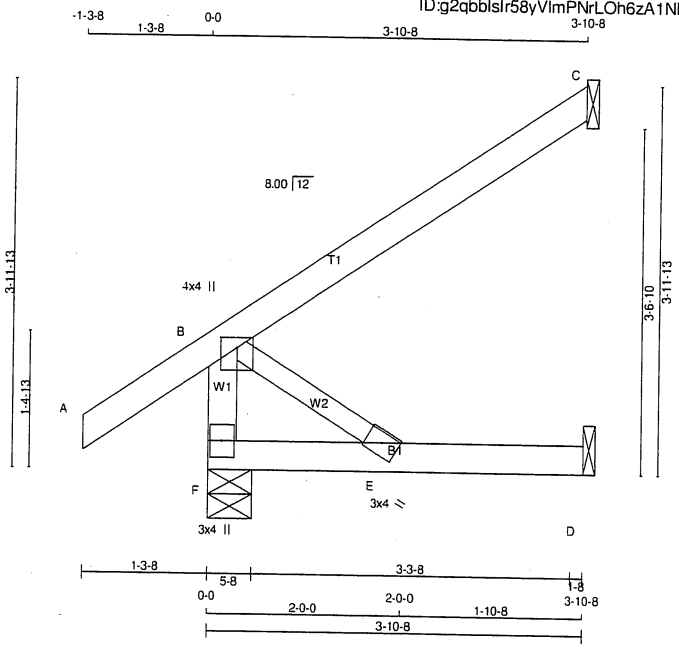
Structural component only
DWG# T-2017328

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409988	J20	4	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:17:36 2020 Page 1
ID:g2qbblslr58yVlmPNrLOh6zA1NF-7nLBZfS5YKbUibzaPgEbcbjs8ZvmCNa5XZnSuayqxhz

Scale = 1:22.6



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	340	0	340	0	0	5-8	5-8
C	178	0	178	0	0	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	238	169 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0
C	122	99 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-304 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.14 (5)	10.00			
B- C	0 / 0	-91.8	-91.8	0.23 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 14 = 58 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.08/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

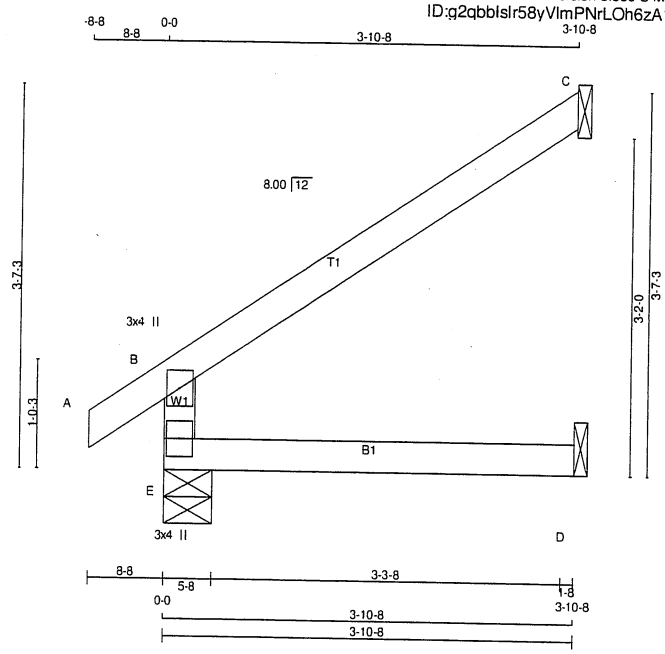
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017346

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409988	J21	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:17:37 2020 Page 1
ID:g2qbbIslr58yVlmPNrLOh6zA1NF-czvZm?SkJejLKlYnzNlq9oG2lzGNxqqElDW0R0yqxhy



TOTAL WEIGHT = 2 X 12 = 23 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	336	0	336	0	0	5-8	5-8
C	133	0	133	0	0	1-8	1-8
D	30	0	34	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	236	164 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0
C	92	74 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	24	0 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-295 / 0	0.0	0.0	0.04 (4)	7.81			
A- B	0 / 20	-91.8	-91.8	0.04 (1)	10.00			
B- C	-25 / 0	-91.8	-91.8	0.18 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.06 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.06/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

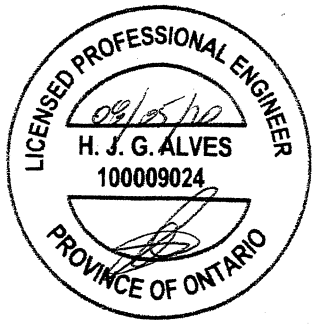
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

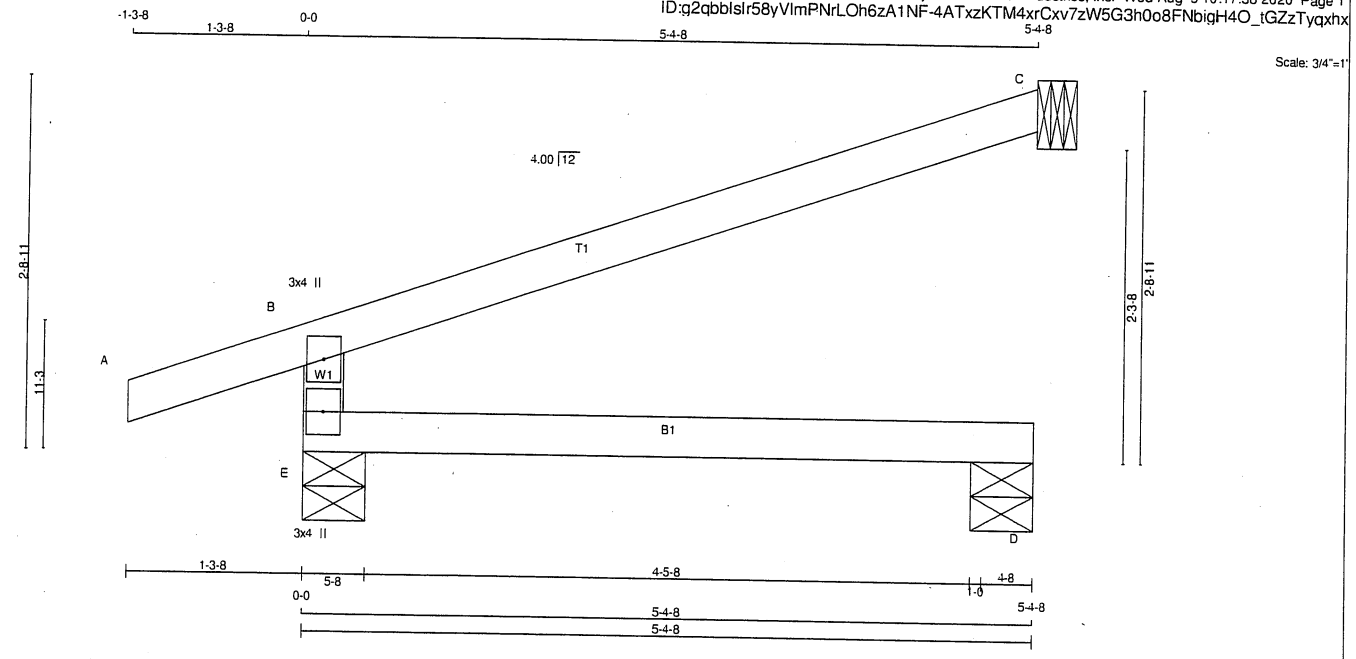


Structural component only
DWG# T-2017347

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409988	J22	7	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:17:38 2020 Page 1
ID:g2qbbIsIr58yVImPNrLOh6zA1NF-4ATxzKTM4xrCxv7zW5G3h0o8FNbigH4O_tGZzTyqxhx



TOTAL WEIGHT = 7 X 15 = 103 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	490	0	490	0	0	5-8	5-8
C	185	0	185	0	0	4-8	4-8
D	41	0	45	0	0	5-8	5-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	344	240 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
C	127	103 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0
D	32	0 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-431 / 0	0.0	0.0	0.12 (4)	7.81			
A-B	0 / 19	-91.8	-91.8	0.11 (1)	10.00			
B-C	-19 / 0	-91.8	-91.8	0.45 (1)	6.25			
E-D	0 0	-18.5	-18.5	0.12 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.45/1.00 (B-C:1) , BC=0.12/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

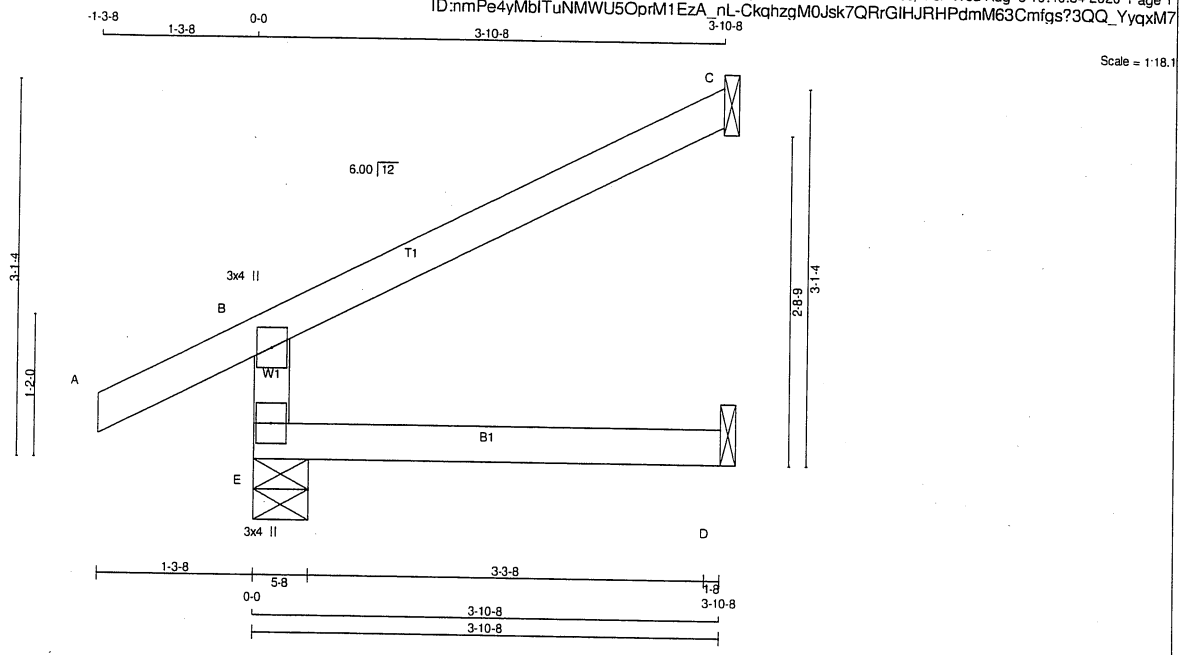
JSI GRIP= 0.17 (E) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

Structural component only
DWG# T-2017348

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	J30	4	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:40:54 2020 Page 1
ID:nmPe4yMbITuNMWU5OprM1EzA_nL-CkqhzgM0Jsk7QRrGIHJRHPdmM63Cmfgs?3QQ_YyqxM7



TOTAL WEIGHT = 4 X 12 = 48 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	388	0	388	0	0	5-8	5-8
C	133	0	133	0	0	1-8	1-8
D	30	0	34	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	272	193 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0
C	92	74 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	24	0 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO					
E-B	-347 / 0	0.0 0.0	0.05 (4)	7.81			
A-B	0 / 28	-91.8 -91.8	0.14 (5)	10.00			
B-C	-20 / 0	-91.8 -91.8	0.23 (1)	6.25			
E-D	0 / 0	-18.5 -18.5	0.06 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.06/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

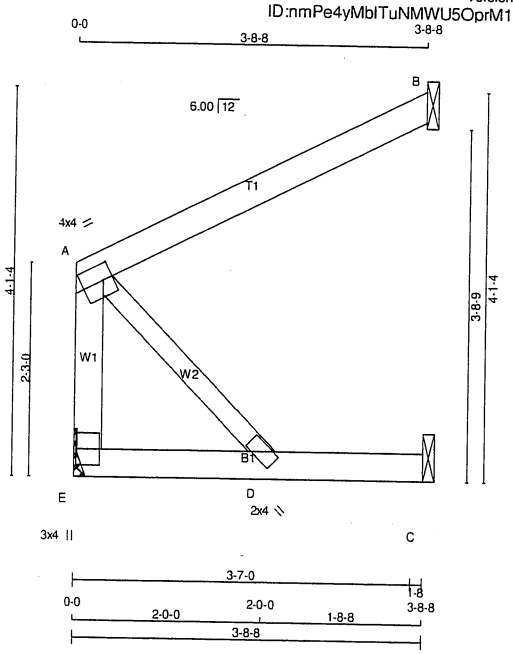
JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

Structural component only
DWG# T-2017364

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	J31	4	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:40:55 2020 Page 1
ID:nmPe4yMbITuNMWU5OprM1EzA_nL-gwO3A0Ne4As_2bQSs_qgpc9xPWOBV6v0DjA_W?yqxM6



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	BMW+w	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	204	0	204	0	0	MECHANICAL	
B	170	0	170	0	0	1-8	1-8
C	34	0	38	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	145	95 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0
B	117	95 / 0	0 / 0	0 / 0	0 / 0	22 / 0	0 / 0
C	27	0 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- A	-170 / 0	0.0	0.0	0.02 (1)	7.81	A- D	0 / 0	0.00 (1)
A- B	0 / 0	-91.8	-91.8	0.21 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
D- C	0 / 0	-18.5	-18.5	0.07 (4)	10.00			

TOTAL WEIGHT = 4 X 13 = 53 lb
[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.21/1.00 (A-B:1) , BC=0.07/1.00 (D-E:4) , WB=0.00/1.00 (A-D:1) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (A) (INPUT = 0.90)
JSI METAL= 0.03 (A) (INPUT = 1.00)

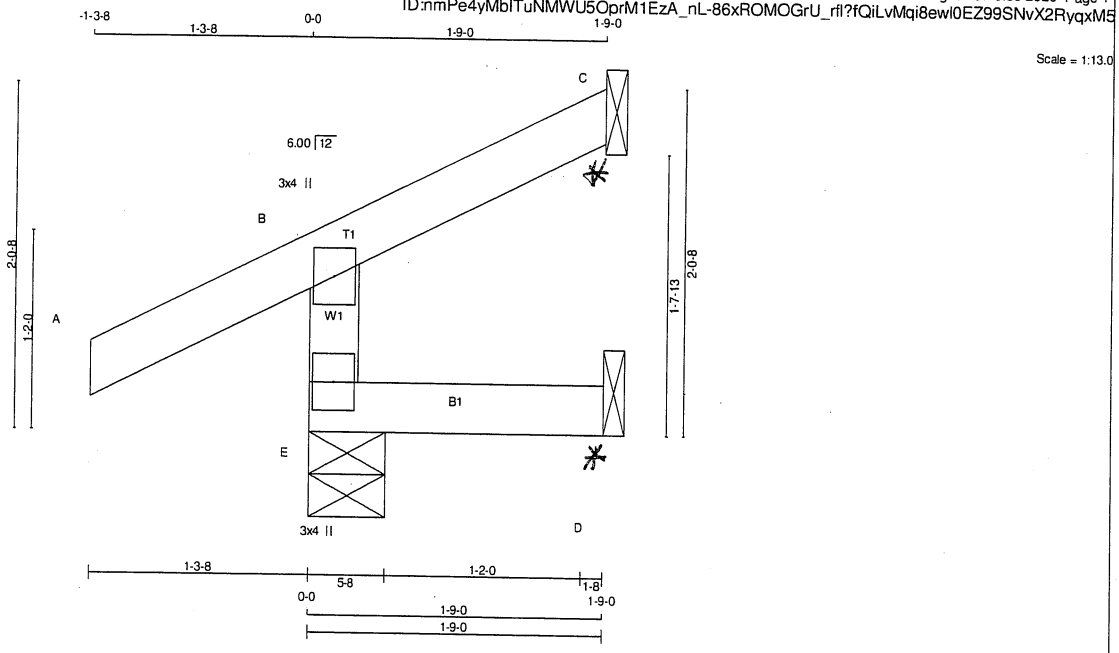
LICENSED PROFESSIONAL ENGINEER
09/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017365

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	J32	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:40:56 2020 Page 1
ID:nmPe4yMbITuNMWU5OprM1EzA_nL-86xROMOGrU_rfl?fQiLvMqi8ewl0EZ99SNvX2RyqxM5



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	269	0	269	0	0	5-8	5-8
C	42	0	42	0	-24	1-8	1-8
D	6	0	15	0	-4	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	187	141 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0
C	29	23 / -18	0 / 0	0 / 0	0 / 0	6 / 0	0 / 0
D	6	0 / -8	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MEMB.	MAX. UNBRAC LENGTH	MAX. FACTORED FORCE (LBS)	MAX CSI (LC) FR-TO
E- B	-243 / 0	0.0	0.0	0.04 (5)		7.81		
A- B	0 / 28	-91.8	-91.8	0.12 (1)		10.00		
B- C	-17 / 0	-91.8	-91.8	0.09 (1)		6.25		
E- D	0 / 0	-18.5	-18.5	0.04 (5)		10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 7 = 14 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
·OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.04/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

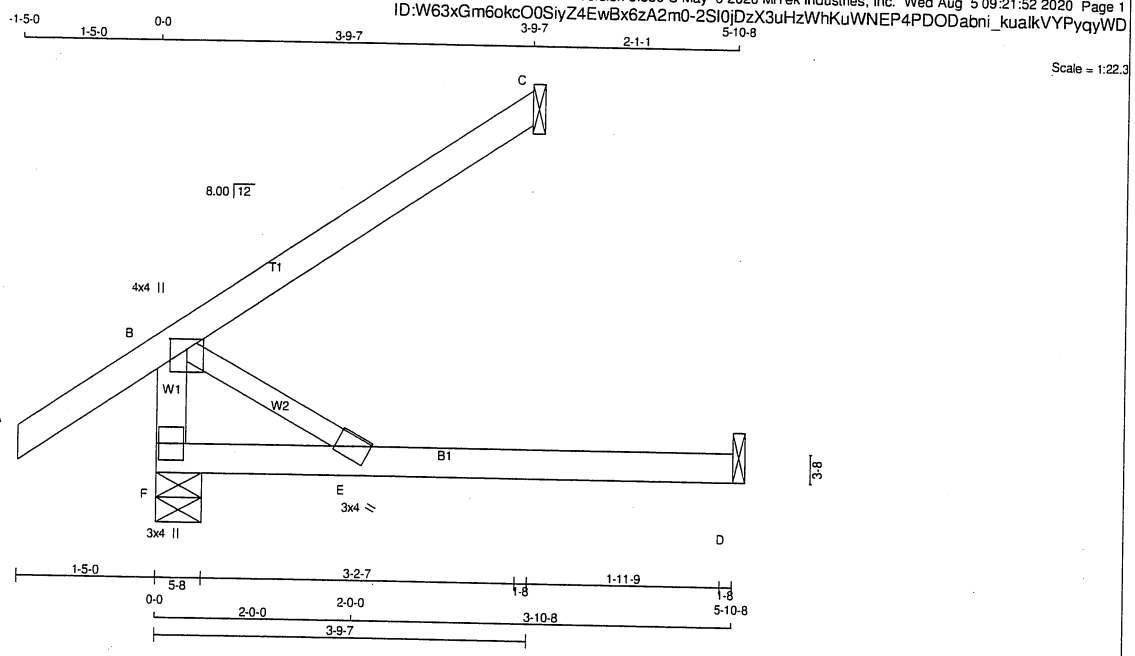
JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

Structural component only
DWG# T-2017366

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	C1	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:21:52 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-2Sl0jDzX3uHzWhKuWNEP4PDODabni_kualkVYPyqyWD



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	365	0	365	0	0	5-8	5-8
C	174	0	174	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	258	174 / 0	0 / 0	0 / 0	0 / 0	84 / 0	0 / 0
C	120	97 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-311 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 38	-91.8	-91.8	0.15 (1)	10.00			
B- C	0 / 0	-91.8	-91.8	0.22 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.17 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.19 (4)	10.00			

TOTAL WEIGHT = 2 X 17 = 34 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

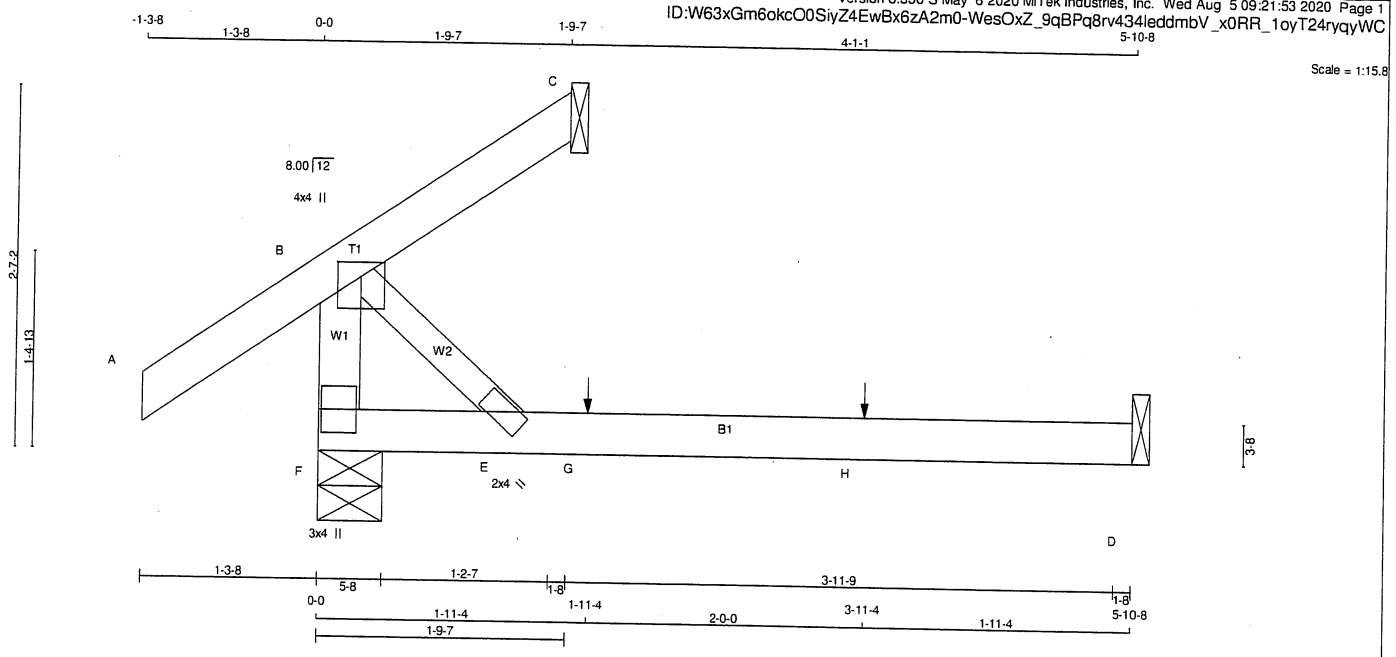
JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

Structural component only
DWG# T-2017324

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	C2	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:21:53 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-WesOxZ_9qBPq8rv434leddmbV_x0RR_1oyT24ryqyWC



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

TOTAL WEIGHT = 2 X 13 = 27 lb [M]

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	311	0	311	0	0	5-8	5-8
C	34	0	34	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	220	143 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
C	23	19 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-256 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.13 (1)	10.00			
B- C	-27 / 0	-91.8	-91.8	0.12 (1)	6.25			
F- E	0 / 0	-18.5	-18.5	0.13 (4)	10.00			
E- G	0 / 0	-18.5	-18.5	0.19 (4)	10.00			
G- H	0 / 0	-18.5	-18.5	0.19 (4)	10.00			
H- D	0 / 0	-18.5	-18.5	0.19 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	FRONT	VERT	TOTAL	--	C1
H	3-11-4	1	1	---	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=0.98 NAIL=0.98 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

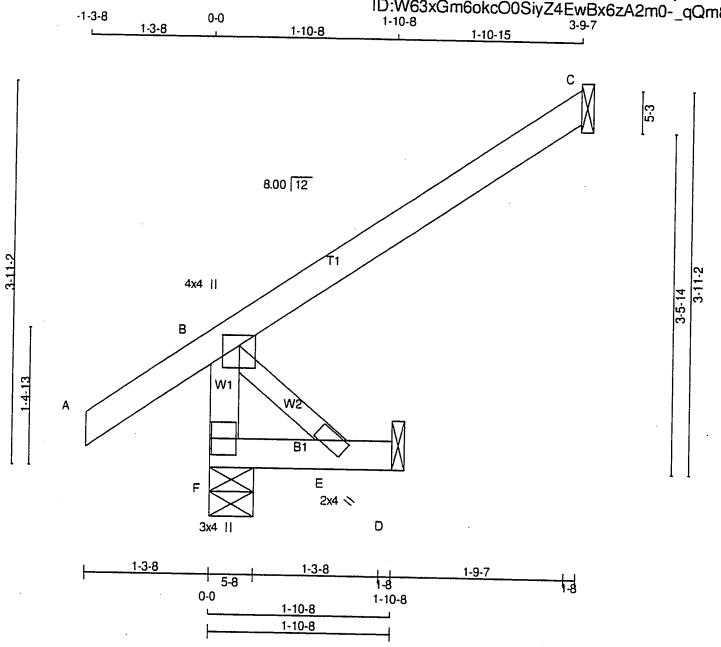
JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

Structural component only
DWG# T-2017325

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	C3	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:21:54 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-_qQm8v?nbVXhm?UHdoGtAqIkjOJtAuEA1cDccHyqyWB



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	317	0	317	0	0	5-8	5-8
C	174	0	174	0	0	1-8	1-8
D	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	220	167 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
C	120	97 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-300 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 35	-91.8	-91.8	0.13 (5)	10.00
B- C	0 / 0	-91.8	-91.8	0.22 (1)	10.00
F- E	0 / 0	-18.5	-18.5	0.02 (4)	10.00
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 12 = 23 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

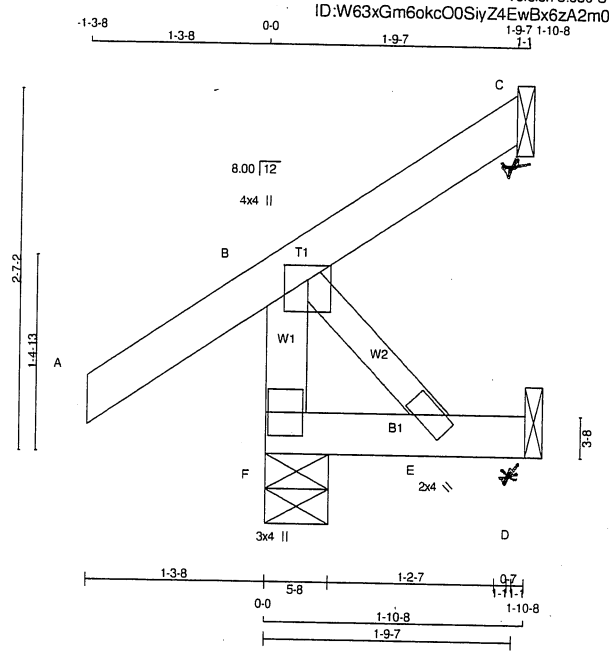
LICENSED PROFESSIONAL ENGINEER
08/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017326

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	C4	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:21:54 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-_qQm8v?nbVXhm?UHdoGtAqImHOJtAuEA1cDccHyqyWB



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	274	0	274	0	0	5-8	5-8
C	34	0	34	0	-39	1-8	1-8
D	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	190	143 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	23	19 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-256 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-27 / 0	-91.8	-91.8	0.12 (1)	6.25
F- E	0 / 0	-18.5	-18.5	0.02 (4)	10.00
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 9 = 18 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

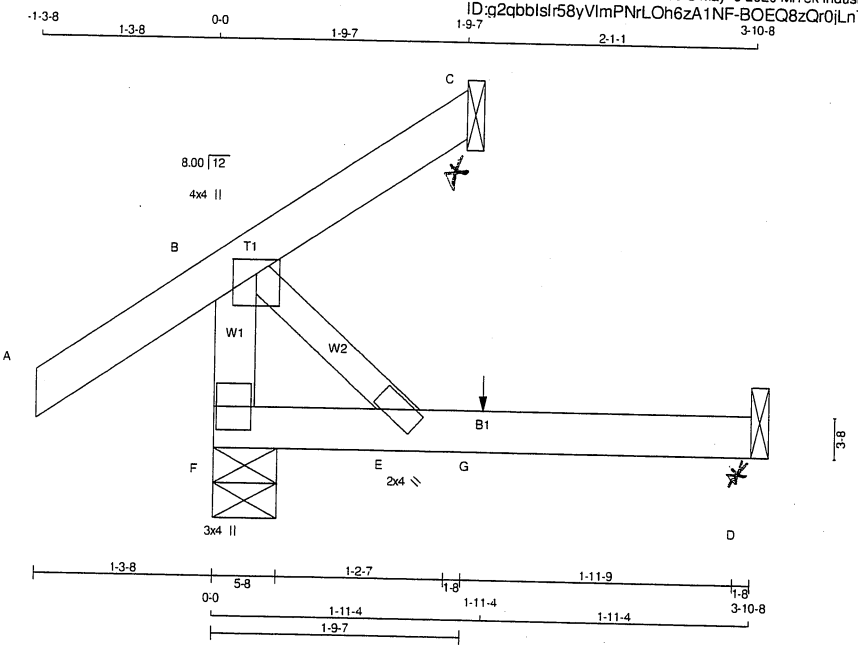
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO
08/05/20

Structural component only
DWG# T-2017327

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409988	C20	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:17:34 2020 Page 1
ID:g2qbbIsIr58yVImPNrLOh6zA1NF-BOEQ8zQr0iLnTHpCHFB7XAeX7mEIkT4o3FIMriyqxi?



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	292	0	292	0	0	5-8	5-8
C	34	0	34	0	-39	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	205	143 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	23	19 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-256 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 35	-91.8	-91.8	0.14 (5)	10.00
B- C	-27 / 0	-91.8	-91.8	0.13 (5)	6.25
F- E	0 / 0	-18.5	-18.5	0.07 (4)	10.00
E- G	0 / 0	-18.5	-18.5	0.08 (4)	10.00
G- D	0 / 0	-18.5	-18.5	0.08 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 11 = 22 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14/1.00 (A-B:5) , BC=0.08/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

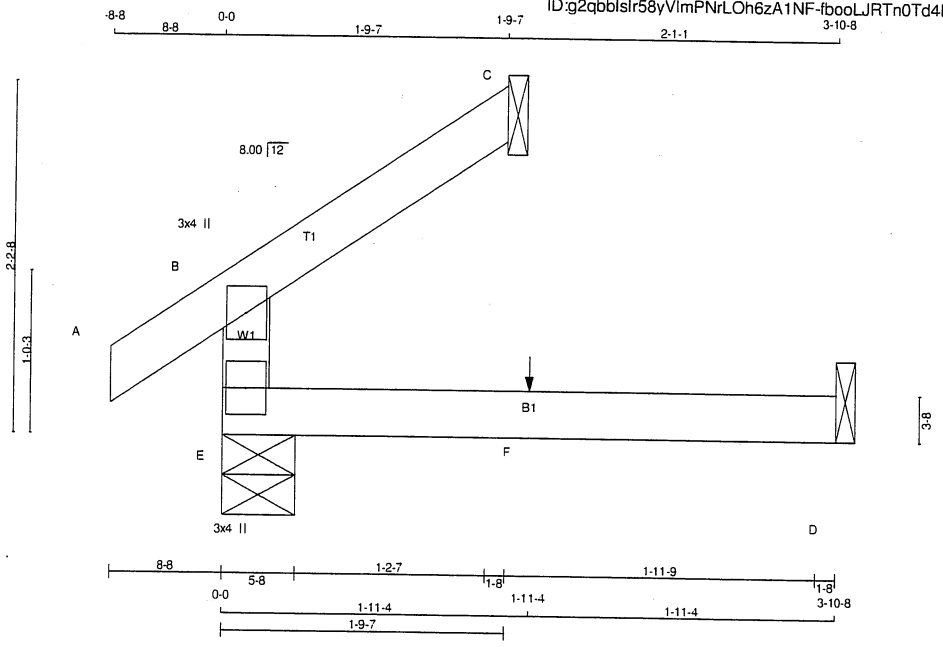
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017343

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409988	C21	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:17:35 2020 Page 1
ID:g2qbbIsIr58yVlmPNrLOh6zA1NF-fbooLJRTn0Td4ROOrziM4NAkPAavTwKxlv1vN8yqxi_



TOTAL WEIGHT = 2 X 9 = 17 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	216	0	216	0	0	5-8	5-8
C	62	0	62	0	0	1-8	1-8
D	30	0	34	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	153	97 / 0	0 / 0	0 / 0	0 / 0	56 / 0	0 / 0
C	43	35 / 0	0 / 0	0 0	0 0	8 0	0 / 0
D	24	0 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-174 / 0	0.0	0.0	0.04 (4)	7.81			
A-B	0 / 20	-91.8	-91.8	0.04 (1)	10.00			
B-C	-11 / 0	-91.8	-91.8	0.04 (1)	6.25			
E-F	0 / 0	-18.5	-18.5	0.06 (4)	10.00			
F-D	0 / 0	-18.5	-18.5	0.06 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.04/1.00 (B-E:4) , BC=0.06/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

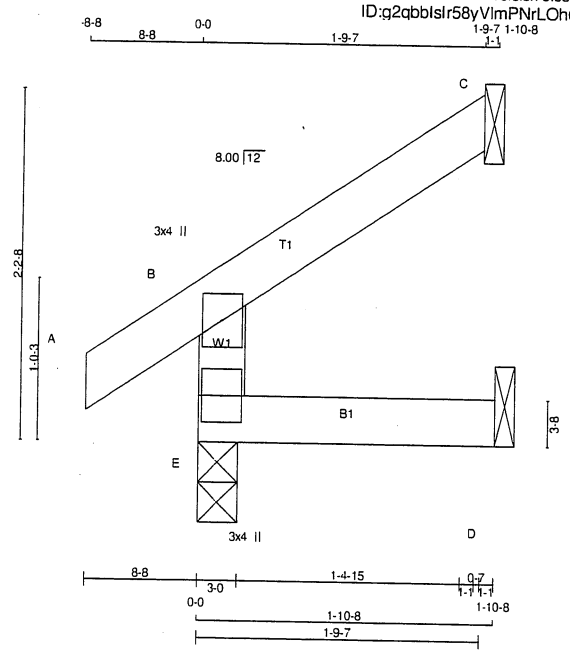
JSI GRIP= 0.07 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

Structural component only
DWG# T-2017344

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409988	C22	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:17:36 2020 Page 1
ID:g2qbbIsIr58yVlmPNrLOh6zA1NF-7nLBZfS5YKbUibzaPgEbcbjv2ZwpCNa5XZnSuayqxhz



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	194	0	194	0	0	3-0	3-0
C	62	0	62	0	0	1-8	1-8
D	15	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	136	97 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0
C	43	35 / 0	0 / 0	0 / 0	0 / 0	8 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B		-174 / 0	0.0	0.0	0.01 (4)	7.81			
A-B		0 / 20	-91.8	-91.8	0.05 (5)	10.00			
B-C		-11 / 0	-91.8	-91.8	0.05 (1)	6.25			
E-D		0 / 0	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 6 = 13 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.05/1.00 (B-C:1) , BC=0.01/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

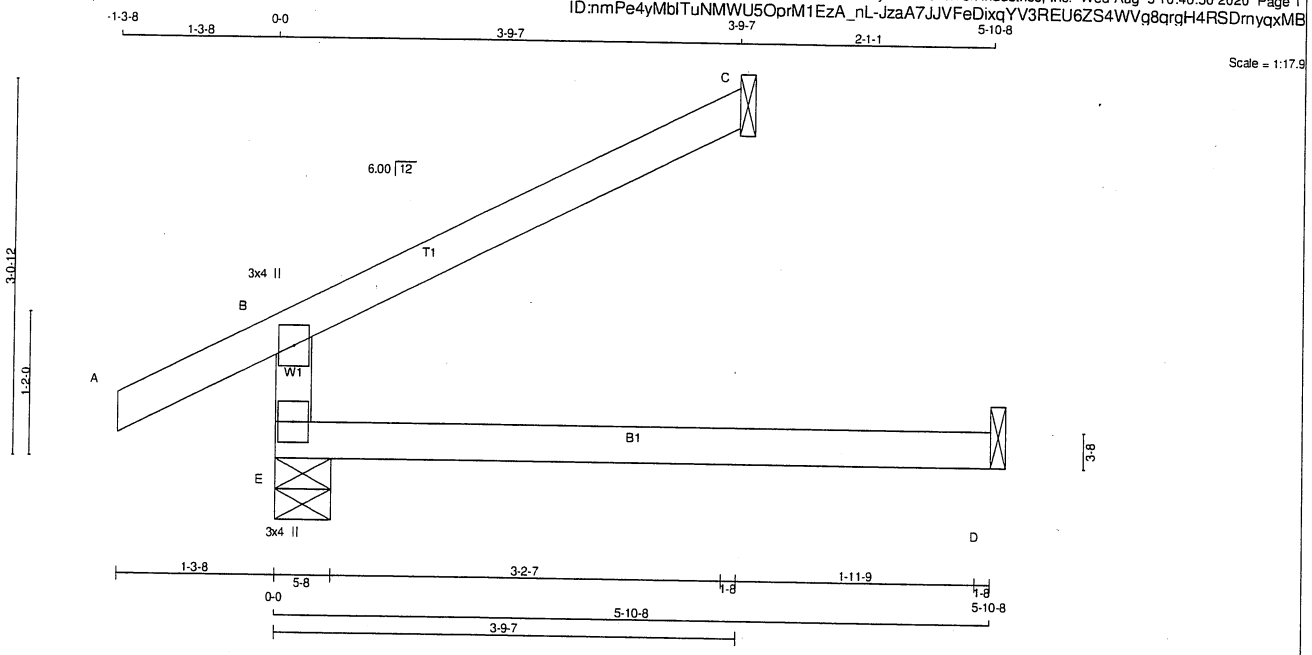
JSI GRIP= 0.07 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

Structural component only
DWG# T-2017345

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	C30	4	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:40:50 2020 Page 1
ID:nmPe4yMbITuNMWU5OprM1EzA_nL-JzaA7JJVFeDixqYV3REU6ZS4WVq8qrgH4RSDrnyqxMB



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	405	0	405	0	0	5-8	5-8
C	130	0	130	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	286	190 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-342 / 0	0.0	0.0	0.13 (4)	7.81			
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B- C	-19 / 0	-91.8	-91.8	0.22 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

TOTAL WEIGHT = 4 X 14 = 57 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

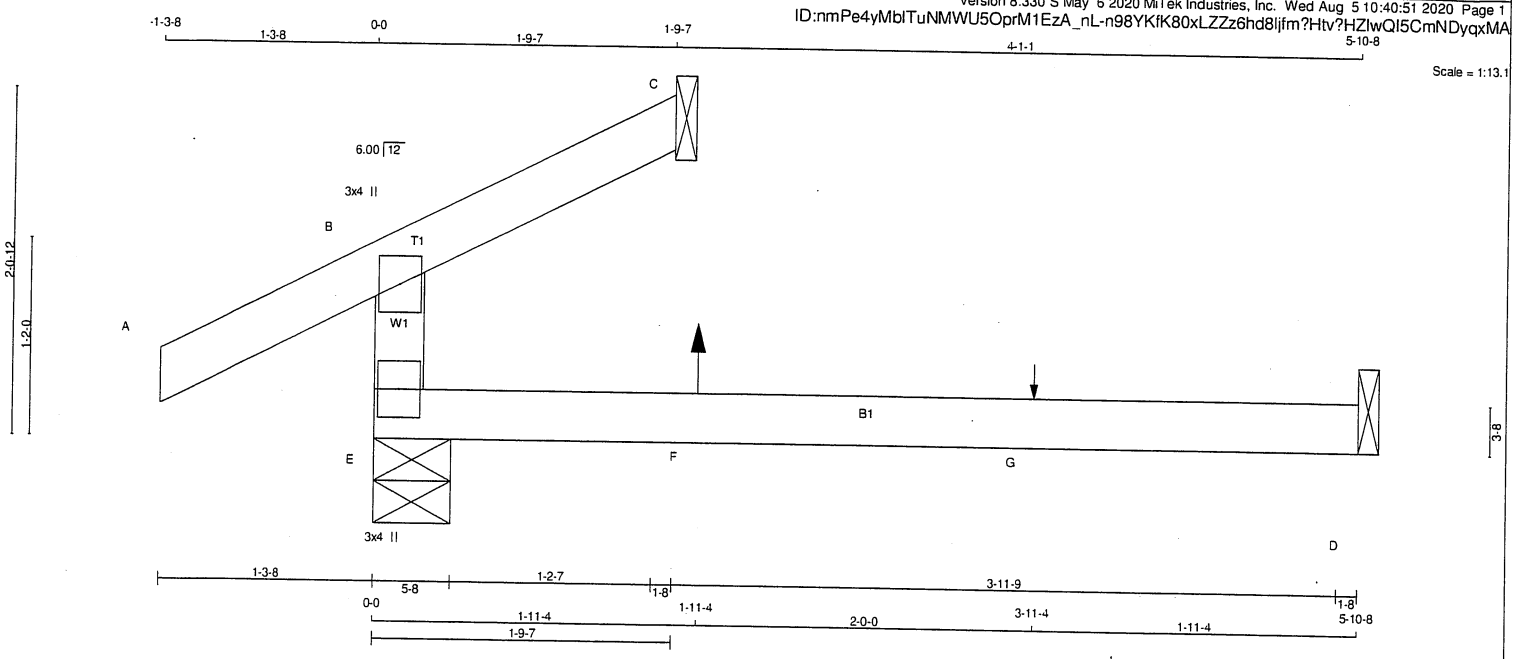
LICENSED PROFESSIONAL ENGINEER
08/15/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017359

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	C31	4	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:40:51 2020 Page 1
ID:nmPe4yMbITuNMWU5OprM1EzA_nL-n98YKfK80xLZZz6hd8ljfm?Htv?HZlwQl5CmNDyqxMA



TOTAL WEIGHT = 4 X 12 = 46 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	284	0	284	0	0	5-8	5-8
C	63	0	63	0	0	1-8	1-8
D	44	0	52	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	200	137 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	46	21 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
D	35	0 / -3	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-227 / 0	0.0	0.0	0.11 (4)	7.81			
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B- C	-9 / 9	-91.8	-91.8	0.08 (4)	10.00			
E- F	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
F- G	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
G- D	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	FRONT	VERT	TOTAL	--	C1
G	3-11-4	1	1	---	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.14/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

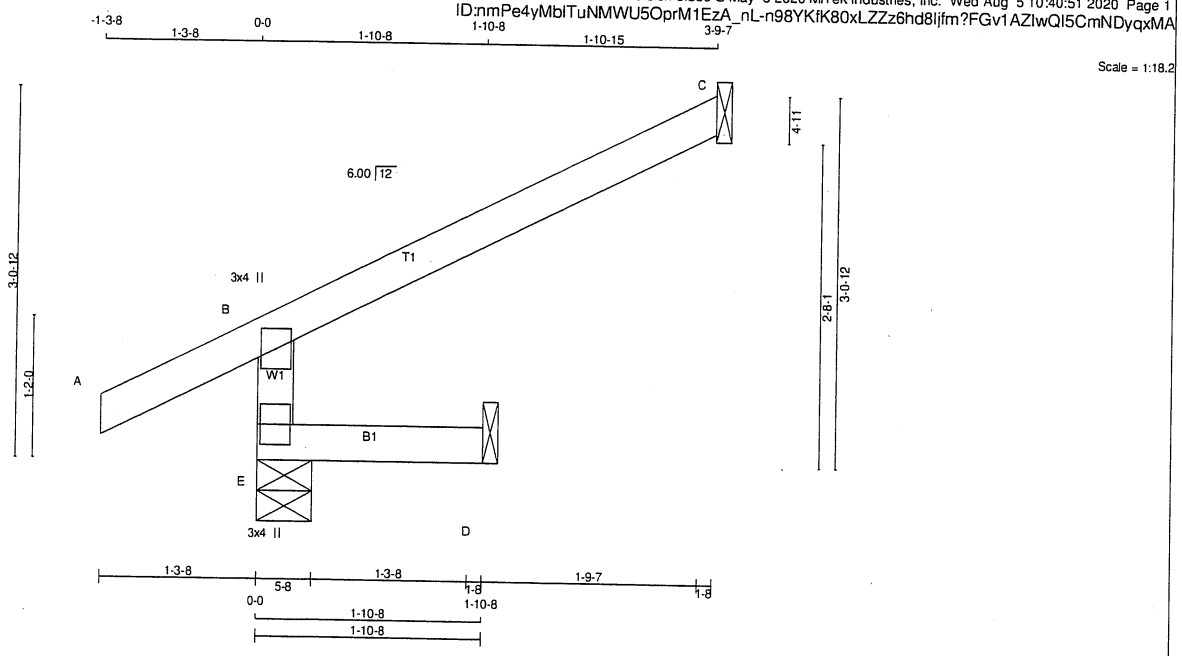
JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

Structural component only
DWG# T-2017360

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	C32	4	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:40:51 2020 Page 1
ID:nmPe4yMblTuNMWU5OprM1EzA_nL-n98YKfK80xLZZz6hd8ljfm?FGv1AZIwQI5CmNDyqxMA



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	361	0	361	0	0	5-8	5-8
C	130	0	130	0	0	1-8	1-8
D	16	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	250	190 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
E- B	-342 / 0	0.0	0.0	0.01 (4)	7.81			
A- B	0 / 28	-91.8	-91.8	0.13 (5)	10.00			
B- C	-19 / 0	-91.8	-91.8	0.22 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 10 = 38 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

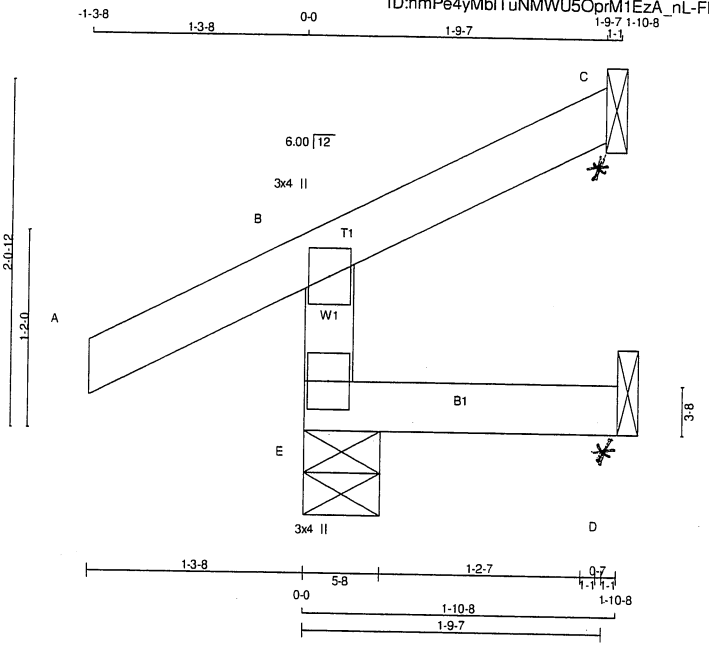
LICENSED PROFESSIONAL ENGINEER
08/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017361

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	C33	6	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:40:52 2020 Page 1
ID:nmPe4yMblTuNMWU5OprM1EzA_nL-FLiwY_LmnFTQB7htBsGyB_XSeJN4IlAZXlxKvgyqxM9



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	271	0	271	0	0	5-8	5-8
C	45	0	45	0	-23	1-8	1-8
D	8	0	17	0	-2	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	188	141 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	31	24 / -18	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	7	0 / -8	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-244 / 0	0.0	0.0	0.04 (5)	7.81			
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B- C	-17 / 0	-91.8	-91.8	0.09 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.04 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 6 X 7 = 42 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.04/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

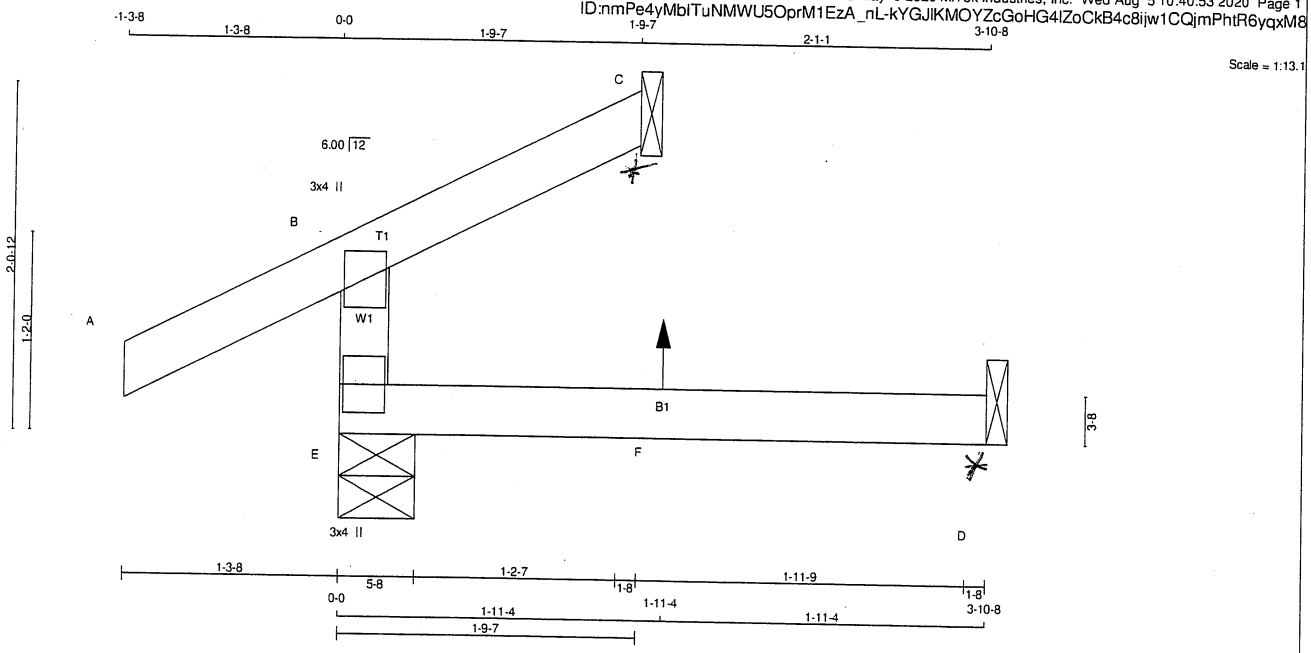
LICENSED PROFESSIONAL ENGINEER
08/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017362

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	C35	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:40:53 2020 Page 1
ID:nmPe4yMbITuNMWU5OprM1EzA_nL-kYGJlKMOYZcGoHG4IZoCkB4c8ijw1CQjmPhtR6yqxM8



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	279	0	279	0	0	5-8	5-8
C	49	0	49	0	-20	1-8	1-8
D	26	0	35	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	195	138 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
C	35	22 / -20	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
D	21	0 / -5	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (9)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-241 / 0	0.0	0.0	0.05 (5)	7.81			
A-B	0 / 28	-91.8	-91.8	0.14 (5)	10.00			
B-C	-15 / 1	-91.8	-91.8	0.08 (5)	6.25			
E-F	0 / 0	-18.5	-18.5	0.06 (4)	10.00			
F-D	0 / 0	-18.5	-18.5	0.06 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 9 = 19 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14/1.00 (A-B:5) , BC=0.06/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.10/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

Structural component only
DWG# T-2017363

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

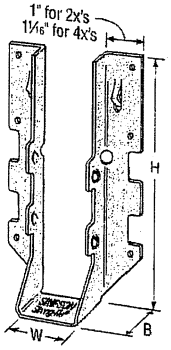
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

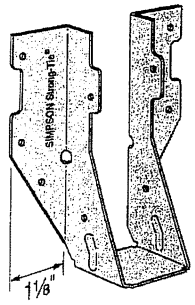
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

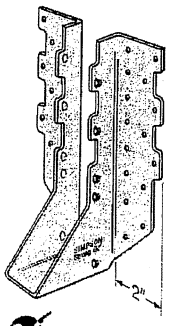
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



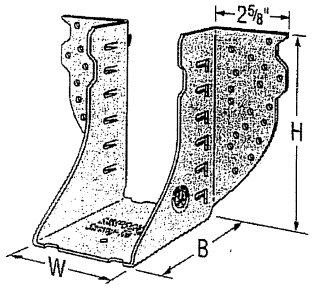
LUS28



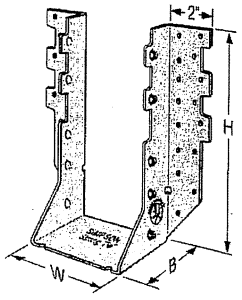
LU26L



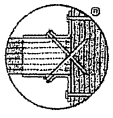
HUS210
(HUS26, HUS28, and HHUS similar)



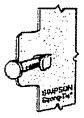
HGUS28-2



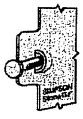
HHUS210-2



Double-Shear Nailing Top View



Double-Shear Nailing Side View; Do not bend tab

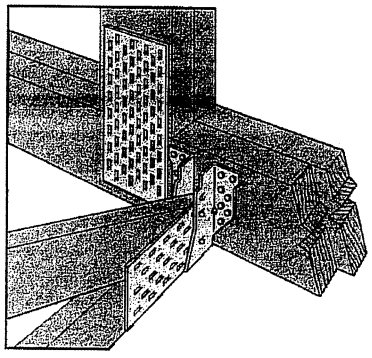


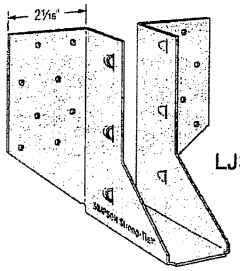
Dome Double-Shear Nailing Side View *(available on some models)*

U.S. Patent 5,603,580

Typical HUS26 Installation with Reduced Heel Height

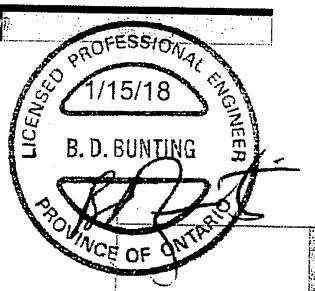
(Truss Designer to provide fastener quantity for connecting multiple members together)





LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS



SIMPSON Strong-Tie

HHUS/HGUS

See Hanger Options information on pp. 125–127.

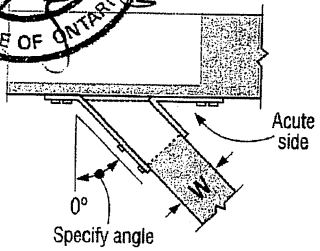
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Single 2x Sizes											
LUS24	18	1 9/16	3 1/8	1 3/4	2 1/4	(4) 10d	(2) 10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
LU24L	22	1 9/16	3	1 5/8	2 11/16	(4) 10d	(2) 10d x 1 1/2"	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
LU26L	22	1 9/16	5	1 5/8	4 5/8	(6) 10d	(4) 10d x 1 1/2"	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
SS LUS26	18	1 9/16	4 3/4	1 3/4	3 3/4	(4) 10d	(4) 10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
HUS26	16	1 5/8	5 3/8	3	3 15/16	(14) 16d	(6) 16d	2705 / 11.30	4940 / 21.97	2065 / 9.20	3875 / 17.24
LJS26DS	18	1 9/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055 / 9.14	4265 / 18.97	1460 / 6.49	4115 / 18.31
HGUS26	12	1 5/8	5 3/8	5	4 1/8	(20) 16d	(8) 16d	2685 / 11.96	6625 / 29.51	2685 / 11.96	5700 / 25.35
LU28L	20	1 9/16	6 3/4	1 5/8	5 7/8	(8) 10d	(6) 10d x 1 1/2"	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
SS LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(4) 10d	1420 / 6.32	2520 / 11.21	1290 / 5.74	1790 / 7.96
HUS28	16	1 5/8	7 1/16	3	6 1/16	(22) 16d	(8) 16d	3605 / 16.04	5365 / 23.86	2675 / 11.90	4345 / 19.33
HGUS28	12	1 5/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310 / 14.74	7675 / 34.19	3310 / 14.74	6900 / 30.73
LU210L	20	1 9/16	8	1 5/8	7 5/8	(10) 10d	(6) 10d x 1 1/2"	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
SS LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420 / 6.32	2785 / 12.39	1290 / 5.74	2210 / 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. **Nails:** 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

■ These products are available with additional corrosion protection. For more information, see p. 24.

◤ These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Double 2x Sizes											
■ LUS24-2	18	3⅛	3⅛	2	1½	(4) 16d	(2) 16d	835 / 3.71	2020 / 8.99	590 / 2.62	1435 / 6.38
SS ■ LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
■ HHUS26-2	14	3⁹⁄₁₆	5⅜	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2850 / 12.68	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS26-2	12	3⁹⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
SS LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
■ HHUS28-2	14	3⁹⁄₁₆	7⁹⁄₁₆	3	6⅛	(22) 16d	(8) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS28-2	12	3⁹⁄₁₆	7⅜	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
SS LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
■ HHUS210-2	14	3⁹⁄₁₆	9⅜	3	8	(30) 16d	(10) 16d	4670 / 20.77	9660 / 42.97	4235 / 18.84	7000 / 31.14
HGUS210-2	12	3⁹⁄₁₆	9⅜	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
Triple 2x Sizes											
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
■ HHUS210-3	14	4¹¹⁄₁₆	9	3	7¹⁵⁄₁₆	(30) 16d	(10) 16d	4670 / 20.77	9670 / 43.02	4235 / 18.84	6865 / 30.54
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
Quadruple 2x Sizes											
HGUS26-4	12	6⁹⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-4	12	6⁹⁄₁₆	7⁹⁄₁₆	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
■ HHUS210-4	14	6⅛	8⅞	3	7¹³⁄₁₆	(30) 16d	(10) 16d	4670 / 20.77	10155 / 45.17	4235 / 18.84	7210 / 32.07
HGUS210-4	12	6⁹⁄₁₆	9⅜	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
HGUS212-4	12	6⁹⁄₁₆	10⅝	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS214-4	12	6⁹⁄₁₆	12⅝	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80
4x Sizes											
■ LUS46	18	3⁹⁄₁₆	4¾	2	3⁷⁄₁₆	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
■ HHUS46	14	3⅝	5¼	3	3¹³⁄₁₆	(14) 16d	(6) 16d	2540 / 11.30	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
■ LUS48	18	3⁹⁄₁₆	6¾	2	3⁷⁄₁₆	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
■ HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS48	12	3⅝	7¼	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
■ LUS410	18	3⁹⁄₁₆	8¾	2	5⁹⁄₁₆	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
HGUS410	12	3⅝	9	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
HGUS412	12	3⅝	10⅞	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS414	12	3⅝	12⅞	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80

See footnotes on p. 258.

TECHNICAL BULLETIN

TC – Truss Connectors

SIMPSON Strong-Tie®

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

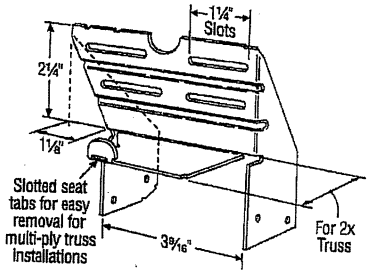
Design: Factored resistances are in accordance with CSA 086-14

Installation:

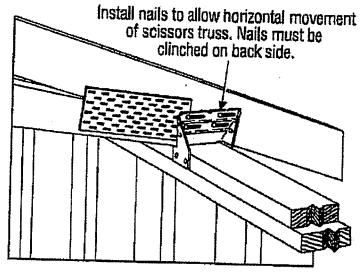
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½ = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss–allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

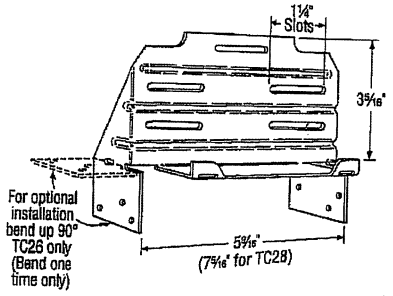
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



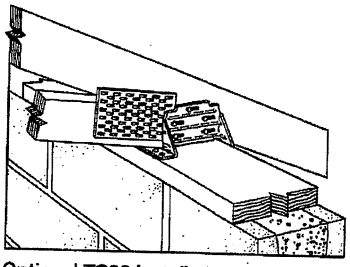
TC24
U.S. Patent 4,932,173



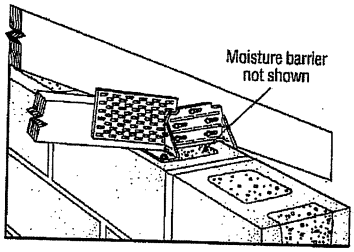
Typical TC24 Installation



TC26
(TC28 Similiar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC26	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) – ⅜" x 2¼" Titen screws has a factored uplift resistance of 275 lb.

LIMIT STATES DESIGN

(800) 999-5099
strongtie.com

H/TSP

SIMPSON Strong-Tie®

Seismic and Hurricane Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Fasteners: To Rafters/ Truss	Fasteners: To Plates	Fasteners: To Studs	Factored Resistance (K_D = 1.15): D.Fir-L Uplift	D.Fir-L Lateral F_1	D.Fir-L Lateral F_2	S-P-F Uplift	S-P-F Lateral F_1	S-P-F Lateral F_2
					lb.	lb.	lb.	lb.	lb.	lb.
					kN	kN	kN	kN	kN	kN
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
					3.29	3.05	1.33	3.02	2.16	0.96
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
					3.69	0.98	0.33	2.62	0.69	0.24
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
					3.58	0.71	0.71	3.36	0.71	0.71
H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
					3.71	0.78	0.93	3.29	0.71	0.93
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
					3.29	0.80	1.18	2.74	0.56	0.85
H6	16	—	(8) 8d	(8) 8d	1585	1085	—	1125	770	—
					7.05	4.83	—	5.00	3.43	—
H7Z	16	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
					6.18	2.98	—	4.40	2.11	—
H8[3]	18	(5) 10d x 1½"	(5) 10d x 1½"	—	1120	—	—	1025	—	—
					4.98	—	—	4.56	—	—
H10A[9]	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290
					7.72	3.54	1.82	6.69	2.51	1.29
H10AR	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1485	690	430	1220	570	305
					6.61	3.07	1.91	5.43	2.54	1.36
H10A-2	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1835	1275	430	1645	880	305
					8.16	5.67	1.91	7.32	3.91	1.36
H10S[7,8]	18	(8) 8d x 1½"	(8) 8d x 1½"	(8) 8d	1465	795	315	1040	565	225
					6.52	3.54	1.40	4.63	2.51	1.00
H11Z	18	(6) 16d x 2½"	(6) 16d x 2½"	—	1095	920	545	780	655	390
					4.87	4.09	2.42	3.47	2.91	1.73
H14	18	[1] (12) 8d x 1½"	(13) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
		[2] (12) 8d x 1½"	(15) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
TSP	16	(9) 10d x 1½"	(6) 10d x 1½"	—	1295	440	—	920	310	—
					5.76	1.96	—	4.09	1.38	—
		(9) 10d x 1½"	(6) 10d	—	1560	440	—	1105	310	—
					6.94	1.96	—	4.92	1.38	—

1. Factored resistances have been increased 15% for short term loading; no further increase is allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
3. H8 factored uplift resistances for stud-to-bottom plate installations are 595 lb. (2.65 kN) for D.Fir-L and 390 lb. (1.74 kN) for S-P-F.
4. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
5. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-HTIECONPATH).
6. Factored resistances in the F_1 direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.
7. H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.38 kN) D.Fir-L and 1015 lb. (4.51 kN) S-P-F.
8. H10S nails to plates are optional for uplift but required for lateral loads.
9. H10A may be field-bent up to a slope of 6/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
10. The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SSNAILS at **strongtie.com**.
11. D.Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1½" nails into the top plates and (3) 8d x 1½" nails into the lowest three flange holes into the truss bottom chord is 495 lb. (2.20 kN).
12. **Nails:** 16d x 2½" = 0.162" dia. x 2½" long, 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long, 8d x 1½" = 0.131" dia. x 1½" long. See pp. 27–28 for other nail sizes and information.

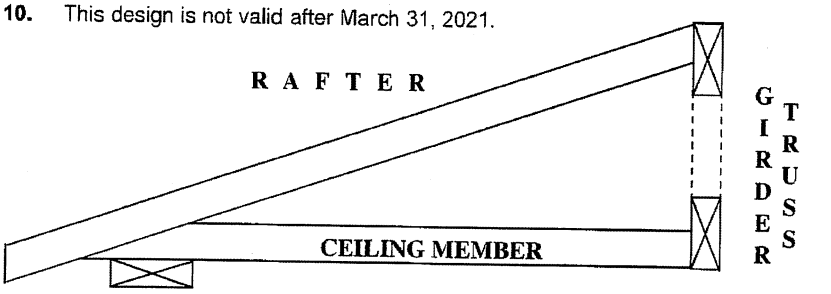
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

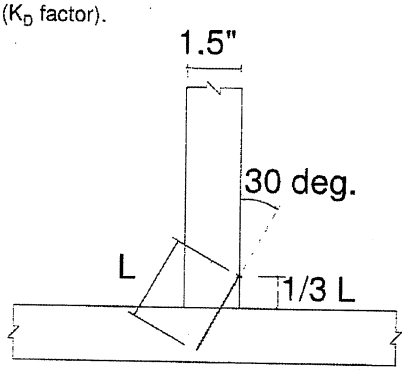
B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.





TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		**(3" and 3.25" nail)**	
LUMBER SIZE	**MAXIMUM NUMBER OF TOE-NAILS**			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek®
MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019

PEO
Certificate No. 10889485

LICENSED PROFESSIONAL ENGINEER
C. Cordogiannis
90553736
PROVINCE OF ONTARIO

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

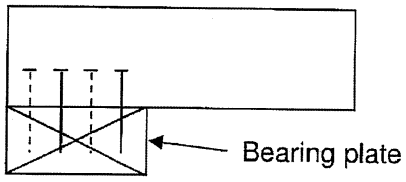
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

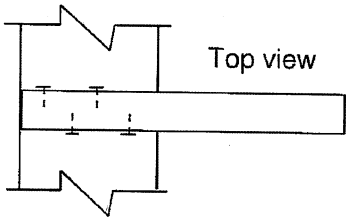
NOTES:

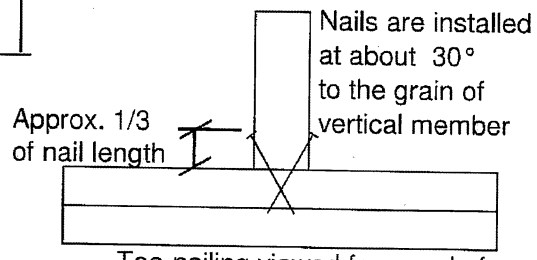
1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



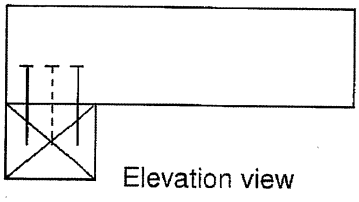
Elevation view



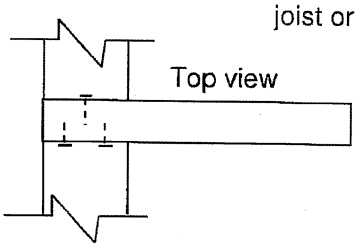


Toe-nailing viewed from end of joist or truss

Toe-nailing on 2x4 Bearing Plate



Elevation view



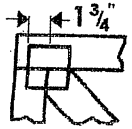
PEO
Certificate No. 10889485

MiTek®
MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

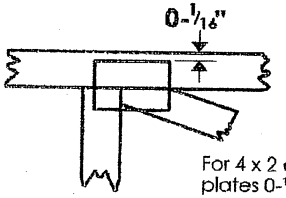
December 2, 2019

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

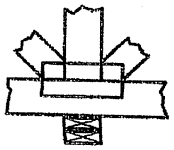
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

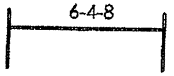


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

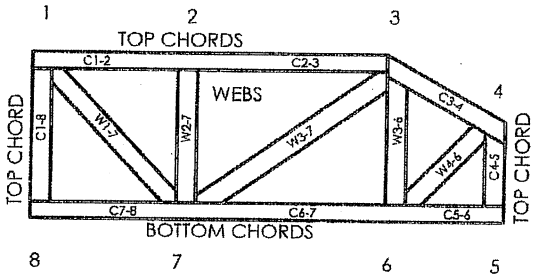
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

ONTARIO WOOD TRUSS
FABRICATORS ASSOCIATION

TECH-NOTES

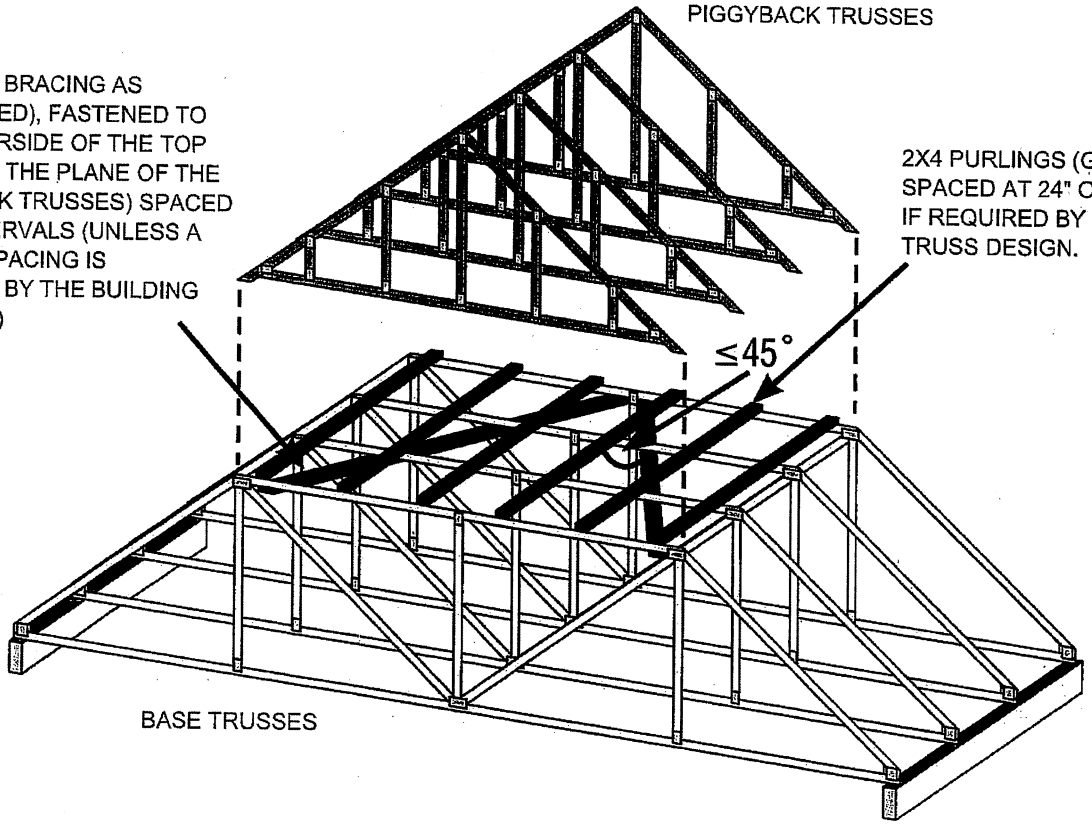
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

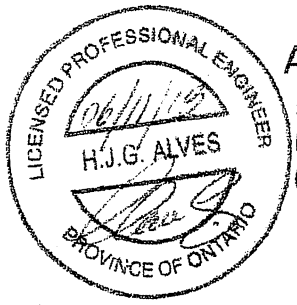
Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800213 Feb 09, 2018