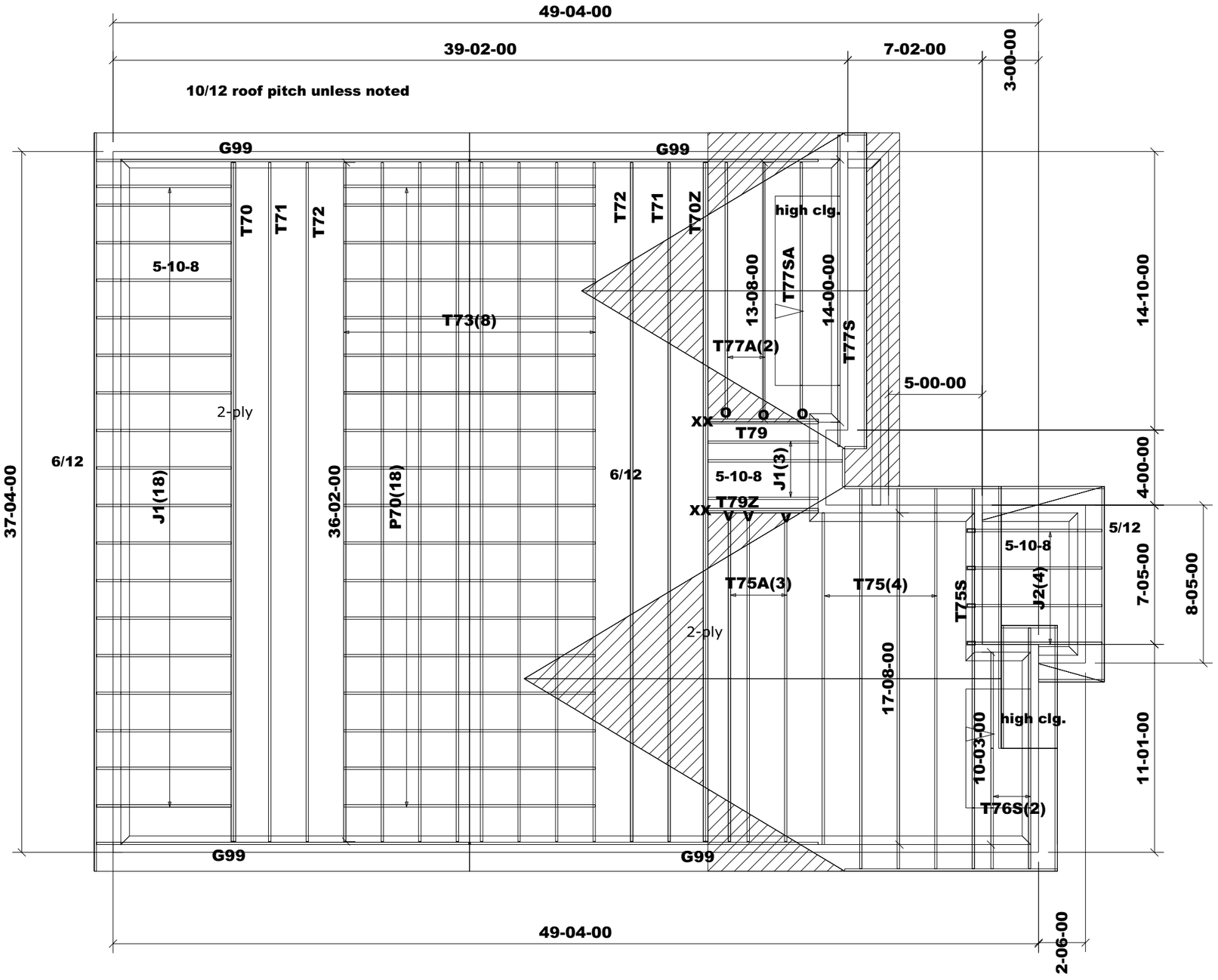
LAMPONE INVESTMENT INC

CITY OF MARKHAM

ROOF TRUSS SHOP DRAWINGS

MODEL NAME : KIMBERLY 4

ELEV 1, 2 & 3



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

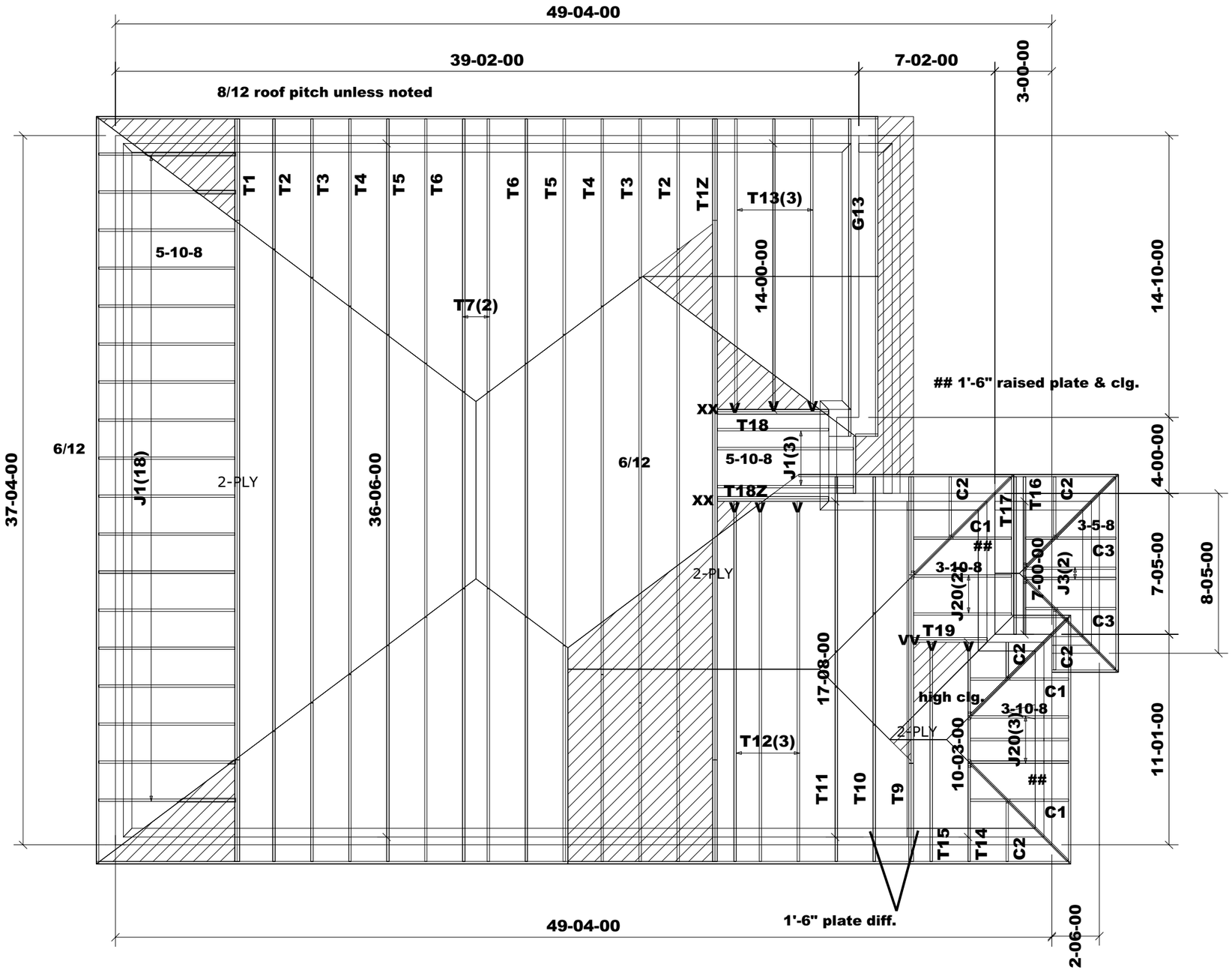
DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26-2 - (**XX**)

DENOTES:
CONVENTIONAL FRAMING

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Job Track: 51453	Builder / Location: GREEN PARK HOMES / MARKHAM		Mitek ver 8.3.3.247
Layout ID: 410045	Project: LAMPONE INVESTMENTS INC	Model / Elevation: KIMBERLY 4 / 1	
Plan Log: 202866	Date: 2020-10-15	Designer: JG	



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
- TCSL = 25.6 psf
- TCDL = 6.0 psf
- BCLL = 0.0 psf
- BCDL = 7.4 psf

HARDWARE:
LJS26DS - (**V**)
HGUS26-2 - (**XX**)
LUS26-2 - (VV)

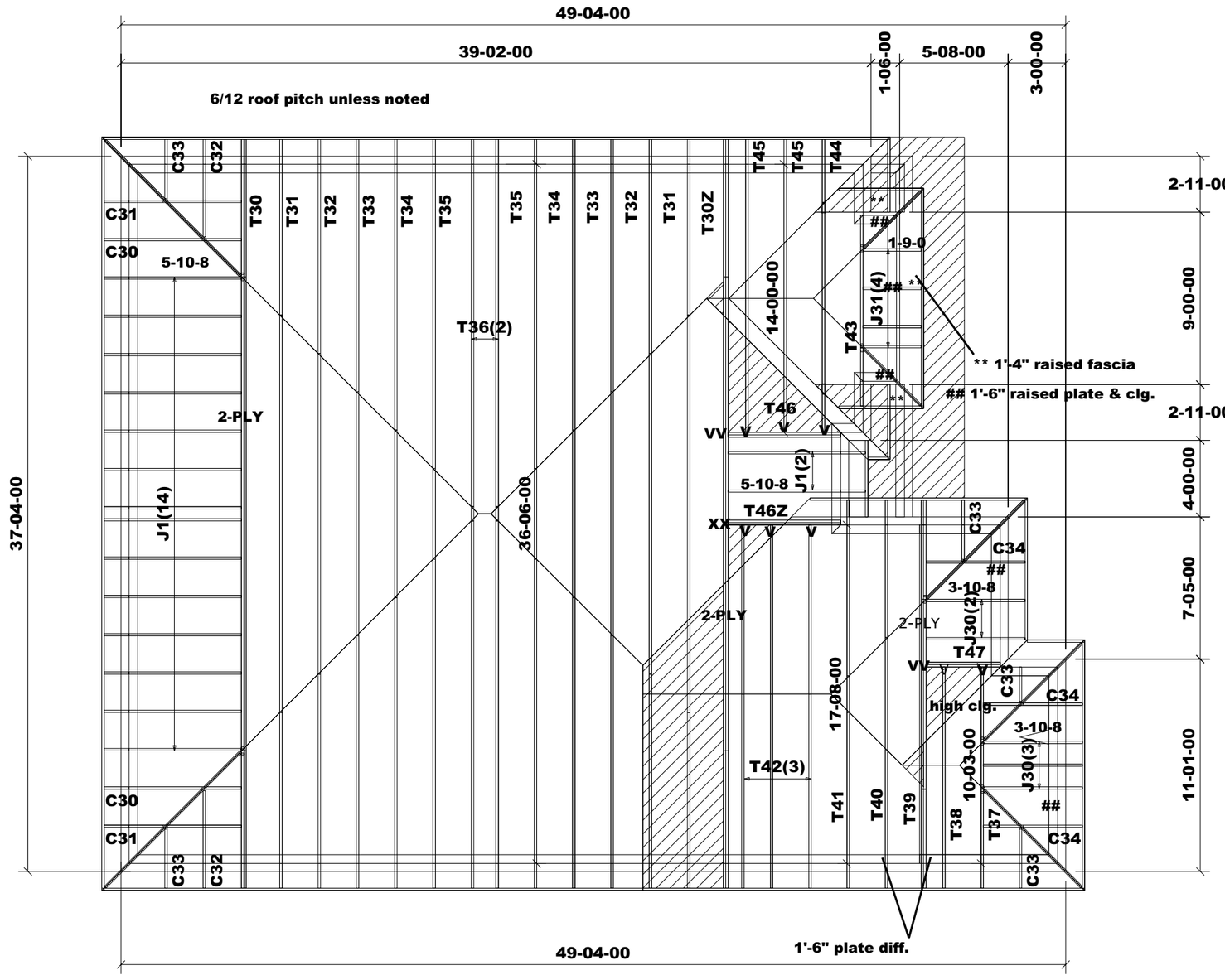
DENOTES:
CONVENTIONAL FRAMING

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Job Track:	51453
Layout ID:	410046
Plan Log:	202866

Builder / Location: **GREEN PARK HOMES / MARKHAM**
Project: **LAMPONE INVESTMENTS INC**
Date: 2020-10-15 | Designer: **JG**

Model / Elevation: **KIMBERLY 4 / 2**
Mitek ver 8.3.3.247



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LJS26DS - (**V**)
HGUS26-2 - (**XX**)
LUS26-2- (VV)

DENOTES:
CONVENTIONAL FRAMING

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

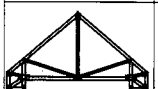
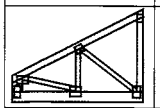
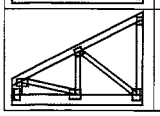
Job Track: 51453	Builder / Location: GREEN PARK HOMES / MARKHAM	Model / Elevation: KIMBERLY 4 / 3	Mitek ver 8.3.3.247
Layout ID: 410047	Project: LAMPONE INVESTMENTS INC		
Plan Log: 202866	Date: 2020-10-15	Designer: JG	

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREEN PARK HOMES	PlanLog:	202866
Project:	LAMPONE INVESTMENTS INC	Layout ID:	410045
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 4	Page:	1 of 2
Lot #:		Date:	08-06-2020
Elevation:	1	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

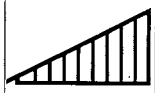
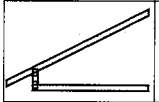
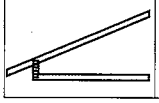
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T70 Flat Girder	0 /12	36-02-00	4-01-04	2 x 6		4-01-04 4-01-04	371.88 226.00		
	1 2-ply	T70Z Flat Girder	0 /12	36-02-00	4-01-04	2 x 6		4-01-04 4-01-04	371.88 226.00		
	2	T71 Flat	0 /12	36-02-00	5-01-04	2 x 6		5-01-04 5-01-04	382.15 237.00		
	2	T72 Flat	0 /12	36-02-00	6-01-04	2 x 6		6-01-04 6-01-04	400.42 245.00		
	8	T73 Flat	0 /12	36-02-00	7-01-04	2 x 6		7-01-04 7-01-04	1677.13 1032.00		
	4	T75 Common	10 /12	17-08-00	9-00-00	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	344.65 217.33		
	3	T75A Common	10 /12	17-08-00	9-00-00	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	251.63 159.00		
	1	T75S Roof Special	10 /12	17-08-00	9-00-00	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	91.89 59.17		
	2	T76S Roof Special	10 /12	10-03-00	5-10-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	110.51 74.33		
	1	T77S Roof Special	10 /12	14-00-00	7-05-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	69.2 46.33		
	1	T77SA Roof Special	10 /12	13-08-00	7-05-11	2 x 4		1-09-05 1-09-05	64.3 43.67		
	2	T77A Common	10 /12	13-08-00	7-05-11	2 x 4		1-09-05 1-09-05	123.42 81.67		
	1 2-ply	T79 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T79Z Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREEN PARK HOMES	PlanLog:	202866
Project:	LAMPONE INVESTMENTS INC	Layout ID:	410045
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 4	Page:	2 of 2
Lot #:		Date:	08-06-2020
Elevation:	1	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	4	G99 GABLE	6 /12	**18-06-08**	10-05-04	2 x 4	1-03-08	1-02-00 10-05-04	374.35 229.33		
	18	P70 Common	6 /12	**13-04-12**	3-04-04	2 x 4		0-01 0-01	684.81 435.00		
	21	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	352.68 224.00		
	4	J2 Jack-Open	5 /12	**5-10-08**	3-05-15	2 x 4	1-03-08	1-00-09 3-05-15	65.32 40.00		

TOTAL # TRUSS= **81** TOTAL BFT OF ALL TRUSSES= **3651.17** BFT. TOTAL WEIGHT OF ALL TRSSES **5853** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
3	Hardware	LJS26DS	
3	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= **8**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREEN PARK HOMES	PlanLog:	202866
Project:	LAMPONE INVESTMENTS INC	Layout ID:	410046
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 4	Page:	1 of 3
Lot #:		Date:	08-06-2020
Elevation:	2	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

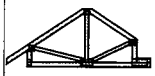
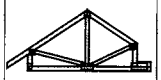
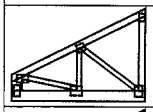
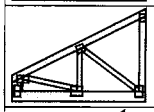
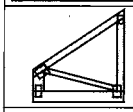
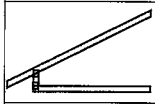
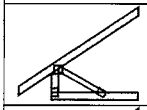
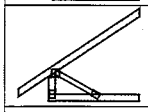
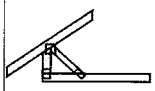
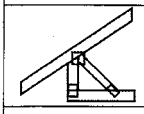
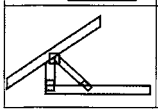
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	8 /12	36-06-00	4-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	387.89 238.00		
	1 2-ply	T1Z Hip Girder	8 /12	36-06-00	4-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	387.89 238.00		
	2	T2 Hip	8 /12	36-06-00	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	345.94 215.67		
	2	T3 Hip	8 /12	36-06-00	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	356.75 222.00		
	2	T4 Hip	8 /12	36-06-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	363.18 224.33		
	2	T5 Hip	8 /12	36-06-00	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	378.65 233.00		
	2	T6 Hip	8 /12	36-06-00	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	383.87 235.67		
	2	T7 Hip	8 /12	36-06-00	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	397.28 244.00		
	1 2-ply	T9 Hip Girder	8 /12	17-08-00	3-11-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	167.33 104.00		
	1	T10 Hip	8 /12	17-08-00	6-09-13	2 x 4	1-03-08 1-03-08	2-10-13 2-10-13	79.44 50.50		
	1	T11 Hip	8 /12	17-08-00	8-01-13	2 x 4	1-03-08 1-03-08	2-10-13 2-10-13	92.28 59.33		
	3	T12 Common	8 /12	17-08-00	8-09-08	2 x 4	1-03-08 1-03-08	2-10-13 2-10-13	263.4 166.00		
	3	T13 Common	8 /12	14-00-00	6-00-13	2 x 4	1-03-08	1-04-13 1-04-13	176.22 111.00		
	1	G13 GABLE	8 /12	14-00-00	6-00-13	2 x 4	1-03-08 1-05-00	1-04-13 1-04-13	61.57 40.33		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREEN PARK HOMES	PlanLog:	202866
Project:	LAMPONE INVESTMENTS INC	Layout ID:	410046
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 4	Page:	2 of 3
Lot #:		Date:	08-06-2020
Elevation:	2	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T14 Hip Girder	8 /12	10-03-00	3-11-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	46.14 30.00		
	1	T15 Common	8 /12	10-03-00	4-09-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	43.15 26.83		
	1	T16 Hip Girder	8 /12	7-00-00	3-08-08	2 x 4	1-03-08	1-04-13 1-09-08	32.21 21.33		
	1	T17 Common	8 /12	7-00-00	3-11-02	2 x 4	1-03-08	1-04-13 1-09-08	32.49 22.00		
	1 2-ply	T18 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T18Z Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T19 Jack-Closed Girder	8 /12	3-10-08	3-11-13	2 x 4 2 x 6		1-04-13 3-11-13	40.7 26.00		
	21	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	352.68 224.00		
	2	J3 Jack-Open	8 /12	3-05-08	3-08-08	2 x 4	1-03-08	1-04-13 3-08-08	27.01 17.67		
	5	J20 Jack-Open	8 /12	3-10-08	3-11-13	2 x 4	1-03-08	1-04-13 3-11-13	72.77 47.50		
	3	C1 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 2-01-01	1-04-13 2-07-02	33.56 21.00		
	5	C2 Jack-Open	8 /12	1-10-08	2-07-02	2 x 4	1-03-08	1-04-13 2-07-13	44.32 28.33		
	2	C3 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 1-08-01	1-04-13 2-07-02	21.42 14.00		

TOTAL # TRUSS= **75** TOTAL BFT OF ALL TRUSSES= **2935.83** BFT. TOTAL WEIGHT OF ALL TRSSES **4704.92** LBS

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREEN PARK HOMES	PlanLog:	202866
Project:	LAMPONE INVESTMENTS INC	Layout ID:	410046
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 4	Page:	3 of 3
Lot #:		Date:	08-06-2020
Elevation:	2	Designer:	
		Sales Rep:	Mario DiCano

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
8	Hardware	LJS26DS	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= **11**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREEN PARK HOMES	PlanLog:	202866
Project:	LAMPONE INVESTMENTS INC	Layout ID:	410047
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 4	Page:	1 of 3
Lot #:		Date:	08-06-2020
Elevation:	3	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

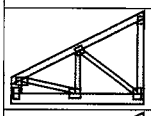
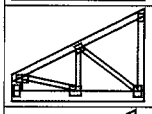
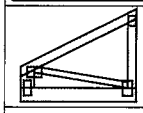
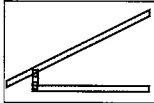
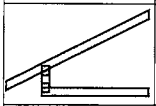
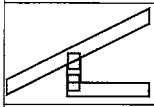
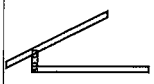
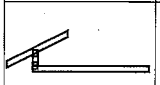
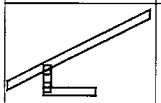
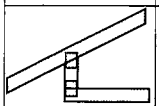
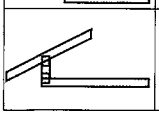
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T30 Hip Girder	6 /12	**36-06-00**	4-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	396.1 242.00		
	1 2-ply	T30Z Hip Girder	6 /12	**36-06-00**	4-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	396.1 242.00		
	2	T31 Hip	6 /12	**36-06-00**	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	337.16 210.33		
	2	T32 Hip	6 /12	**36-06-00**	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	345.25 213.33		
	2	T33 Hip	6 /12	**36-06-00**	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	342.76 208.33		
	2	T34 Hip	6 /12	**36-06-00**	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	355.98 218.00		
	2	T35 Hip	6 /12	**36-06-00**	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	354.56 220.00		
	2	T36 Hip	6 /12	**36-06-00**	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	351.52 212.67		
	1	T37 Hip Girder	6 /12	**10-03-00**	3-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	42.34 27.67		
	1	T38 Common	6 /12	**10-03-00**	3-08-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	40.29 26.33		
	1 2-ply	T39 Hip Girder	6 /12	**17-08-00**	3-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	157.1 98.33		
	1	T40 Hip	6 /12	**17-08-00**	5-07-04	2 x 4	1-03-08 1-03-08	2-08-00 2-08-00	74.52 48.00		
	1	T41 Hip	6 /12	**17-08-00**	6-07-04	2 x 4	1-03-08 1-03-08	2-08-00 2-08-00	83.91 54.50		
	3	T42 Common	6 /12	**17-08-00**	7-01-00	2 x 4	1-03-08 1-03-08	2-08-00 2-08-00	239.77 153.00		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREEN PARK HOMES	PlanLog:	202866
Project:	LAMPONE INVESTMENTS INC	Layout ID:	410047
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 4	Page:	2 of 3
Lot #:		Date:	08-06-2020
Elevation:	3	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T43 Hip Girder	6 /12	**8-08-00**	1-10-08	2 x 4	1-03-08 1-03-08	1-00-00 1-00-00	33.7 23.17		
	1	T44 Hip	6 /12	**14-00-00**	4-04-08	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	60.44 38.00		
	2	T45 Common	6 /12	**14-00-00**	4-08-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	111.1 73.00		
	1 2-ply	T46 Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T46Z Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T47 Jack-Closed Girder	6 /12	**3-10-08**	3-01-04	2 x 4 2 x 6		1-02-00 3-01-04	36.77 24.67		
	16	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	268.71 170.67		
	5	J30 Jack-Open	6 /12	**3-10-08**	3-01-04	2 x 4	1-03-08	1-02-00 3-01-04	59.82 36.67		
	4	J31 Jack-Open	6 /12	**1-09-00**	1-10-08	2 x 4	1-03-08	1-00-00 1-10-08	26.57 18.67		
	2	C30 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	C31 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	C32 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	5	C33 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	35.1 23.33		
	3	C34 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 2-01-01	1-02-00 2-00-12	27.9 18.00		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREEN PARK HOMES	PlanLog:	202866
Project:	LAMPONE INVESTMENTS INC	Layout ID:	410047
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 4	Page:	3 of 3
Lot #:		Date:	08-06-2020
Elevation:	3	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS

TOTAL # TRUSS= **74** TOTAL BFT OF ALL TRUSSES= **2720.01** BFT. TOTAL WEIGHT OF ALL TRSSES **4364.85** LBS

HARDWARE

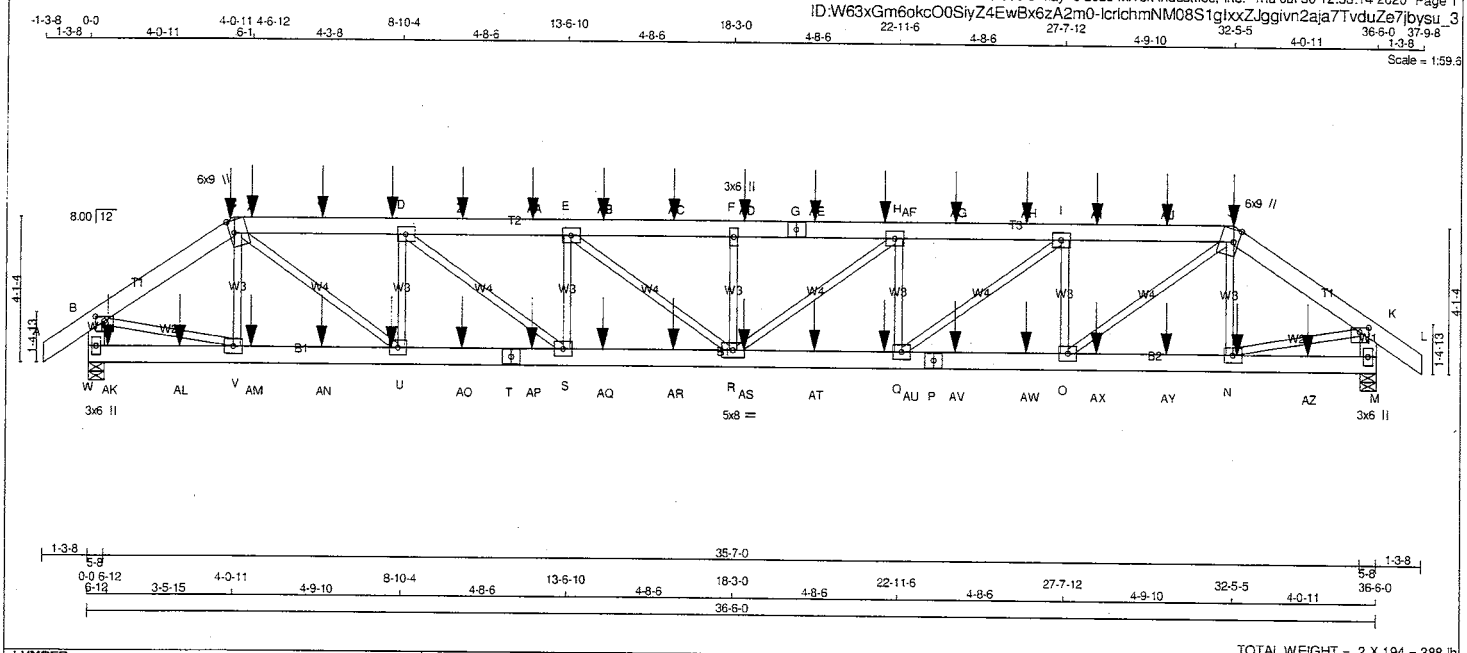
QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
8	Hardware	LJS26DS	
2	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= **11**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410046	T1	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:53:14 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-lcrlchmNM08S1gIxxZJggivn2aja7TvduZe7jbysu_3



TOTAL WEIGHT = 2 X 194 = 388 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x6	DRY	1650F 1.5E	SPF
T - P	2x6	DRY	1650F 1.5E	SPF
P - M	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- G	2	12	SIDE(183.1)
G- J	2	12	SIDE(61.0)
J- L	2	12	SIDE(122.0)
W- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- T	2	12	SIDE(183.1)
T- P	2	12	SIDE(183.1)
P- M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2017045/12

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	3538	0	3538	0	0	5-8	5-8
M	3533	0	3533	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2501	1644 / 0	0 / 0	0 / 0	0 / 0	857 / 0	0 / 0
M	2497	1651 / 0	0 / 0	0 / 0	0 / 0	845 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B		0 / 36	-91.8	-91.8	0.04 (1)		10.00
B-C		-4293 / 0	-91.8	-91.8	0.10 (1)		5.45
C-X		-6342 / 0	-91.8	-91.8	0.19 (1)		4.58
X-Y		-6342 / 0	-91.8	-91.8	0.19 (1)		4.58
Y-D		-6342 / 0	-91.8	-91.8	0.19 (1)		4.58
D-Z		-7945 / 0	-91.8	-91.8	0.20 (1)		4.17
Z-AA		-7945 / 0	-91.8	-91.8	0.20 (1)		4.17
AA-E		-7945 / 0	-91.8	-91.8	0.20 (1)		4.17
E-AB		-8475 / 0	-91.8	-91.8	0.22 (1)		4.05
AB-AC		-8475 / 0	-91.8	-91.8	0.22 (1)		4.05
AC-F		-8475 / 0	-91.8	-91.8	0.22 (1)		4.05
F-AD		-8475 / 0	-91.8	-91.8	0.23 (1)		4.03
AD-G		-8475 / 0	-91.8	-91.8	0.23 (1)		4.03
G-AE		-8475 / 0	-91.8	-91.8	0.23 (1)		4.03
AE-AF		-8475 / 0	-91.8	-91.8	0.23 (1)		4.03
AF-H		-8475 / 0	-91.8	-91.8	0.23 (1)		4.03
H-AG		-7944 / 0	-91.8	-91.8	0.21 (1)		4.17
AG-AH		-7944 / 0	-91.8	-91.8	0.21 (1)		4.17
AH-I		-7944 / 0	-91.8	-91.8	0.21 (1)		4.17
I-AI		-6347 / 0	-91.8	-91.8	0.19 (1)		4.58
AI-AJ		-6347 / 0	-91.8	-91.8	0.19 (1)		4.58
AJ-J		-6347 / 0	-91.8	-91.8	0.19 (1)		4.58
J-K		-4315 / 0	-91.8	-91.8	0.10 (1)		5.44
K-L		0 / 36	-91.8	-91.8	0.04 (1)		10.00
W-B		-3479 / 0	0.0	0.0	0.12 (1)		7.49
M-K		-3496 / 0	0.0	0.0	0.12 (1)		7.48
W-AK		0 / 0	-18.5	-18.5	0.03 (4)		10.00
AK-AL		0 / 0	-18.5	-18.5	0.03 (4)		10.00
AL-V		0 / 0	-18.5	-18.5	0.03 (4)		10.00
V-AM		0 / 3545	-18.5	-18.5	0.16 (1)		10.00
AM-AN		0 / 3545	-18.5	-18.5	0.16 (1)		10.00
AN-U		0 / 3545	-18.5	-18.5	0.16 (1)		10.00
U-AO		0 / 6342	-18.5	-18.5	0.29 (1)		10.00
AO-T		0 / 6342	-18.5	-18.5	0.29 (1)		10.00
T-AP		0 / 6342	-18.5	-18.5	0.29 (1)		10.00
AP-S		0 / 6342	-18.5	-18.5	0.29 (1)		10.00
S-AQ		0 / 7945	-18.5	-18.5	0.36 (1)		10.00
AQ-AR		0 / 7945	-18.5	-18.5	0.36 (1)		10.00
AR-R		0 / 7945	-18.5	-18.5	0.36 (1)		10.00
R-AS		0 / 7944	-18.5	-18.5	0.36 (1)		10.00
AS-AT		0 / 7944	-18.5	-18.5	0.36 (1)		10.00
AT-AU		0 / 7944	-18.5	-18.5	0.36 (1)		10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V-C		-588 / 0	0.07 (1)
C-U		0 / 3504	0.43 (1)
U-D		-1984 / 0	0.24 (1)
D-S		0 / 2028	0.25 (1)
S-E		-1088 / 0	0.13 (1)
E-R		0 / 672	0.08 (1)
R-F		-689 / 0	0.08 (1)
R-H		0 / 672	0.08 (1)
Q-H		-1085 / 0	0.13 (1)
Q-I		0 / 2022	0.25 (1)
O-I		-1972 / 0	0.24 (1)
O-J		0 / 3486	0.43 (1)
N-J		-593 / 0	0.07 (1)
B-V		0 / 3644	0.45 (1)
N-K		0 / 3663	0.45 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CSI: TC=0.23/1.00 (F-H:1) , BC=0.36/1.00 (Q-R:1) , WB=0.45/1.00 (K-N:1) , SSI=0.13/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (C) (INPUT = 0.90)
JSI METAL= 0.59 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410046	T1	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	6.0	9.0	4.25	1.50
D, E, H, I						
D	TMWW-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
J	TTWW+m	MT20	6.0	9.0	4.25	1.50
K	TMVW-p	MT20	5.0	6.0	1.50	3.00
M	BMV1+p	MT20	3.0	6.0		
N, O, Q, S, U, V						
N	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
T	BS-t	MT20	5.0	6.0		
W	BMV1+p	MT20	3.0	6.0		

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
AU- Q	0 / 7944	-18.5	-18.5	0.36 (1)	10.00
Q- P	0 / 6347	-18.5	-18.5	0.29 (1)	10.00
P-AV	0 / 6347	-18.5	-18.5	0.29 (1)	10.00
AV-AW	0 / 6347	-18.5	-18.5	0.29 (1)	10.00
AW- O	0 / 6347	-18.5	-18.5	0.29 (1)	10.00
O-AX	0 / 3564	-18.5	-18.5	0.17 (1)	10.00
AX-AY	0 / 3564	-18.5	-18.5	0.17 (1)	10.00
AY- N	0 / 3564	-18.5	-18.5	0.17 (1)	10.00
N-AZ	0 / 0	-18.5	-18.5	0.03 (4)	10.00
AZ- M	0 / 0	-18.5	-18.5	0.03 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-51	-57	---	FRONT	VERT	DEAD	---	C1
C	4-0-11	-260	-260	---	FRONT	VERT	SNOW	---	C1
D	8-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
J	32-5-5	-51	-57	---	FRONT	VERT	DEAD	---	C1
J	32-5-5	-117	-117	---	FRONT	VERT	TOTAL	---	C1
J	32-5-5	-260	-260	---	FRONT	VERT	SNOW	---	C1
N	32-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
U	8-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
X	4-6-12	-133	-133	---	FRONT	VERT	TOTAL	---	C1
Y	6-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	10-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	12-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	14-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	16-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	18-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AE	20-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AF	22-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AG	24-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AH	26-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AI	28-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AJ	30-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AK	6-12	-30	-30	---	FRONT	VERT	TOTAL	---	C1
AL	2-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AM	4-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AN	6-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AO	10-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AP	12-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AQ	14-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AR	16-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AS	18-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AT	20-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AU	22-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AV	24-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AW	26-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AX	28-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AY	30-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AZ	34-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

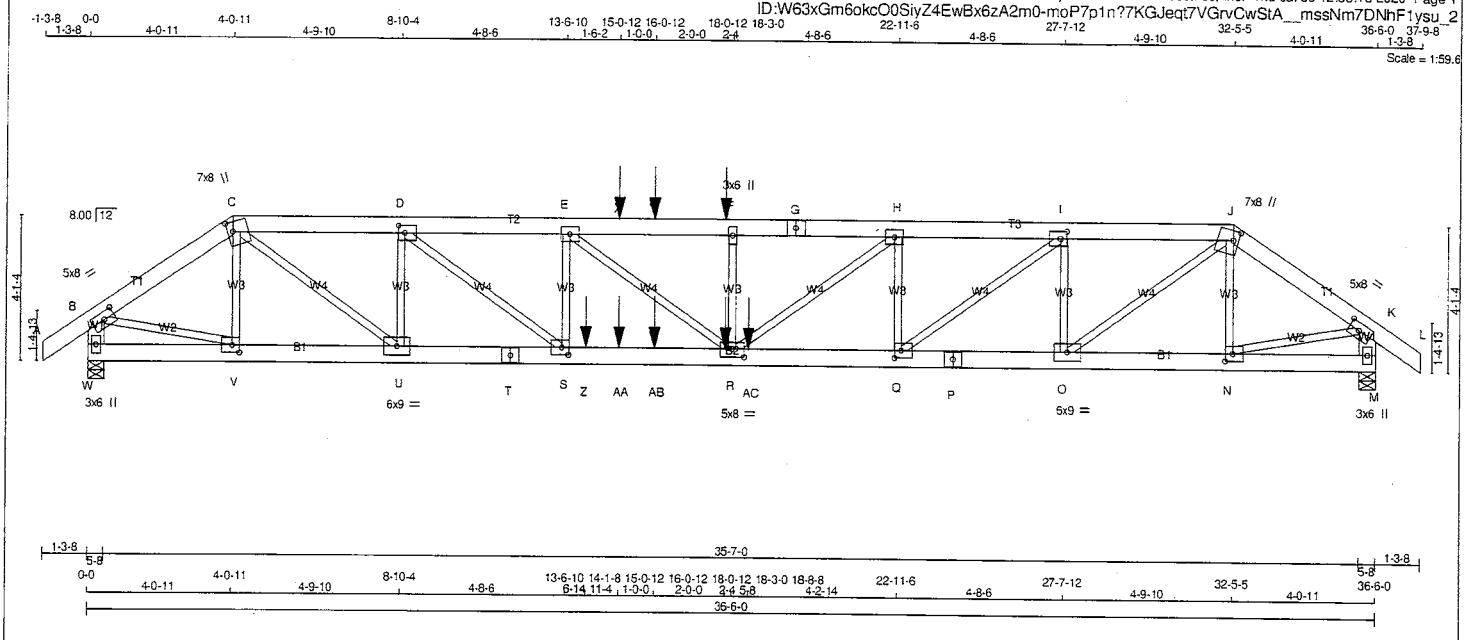
Structural component only
DWG# T-2017045 7/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410046	T1Z	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:53:15 2020 Page 1

ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-moP7p1n??KGJeqt7VGrvCwStA__mssNm7DNhF1ysu_2


-1-3-8 0-0 4-0-11 8-10-4 13-6-10 15-0-12 16-0-12 18-0-12 18-3-0 22-11-6 27-7-12 32-5-5 36-6-0 37-9-8
1-3-8 4-0-11 4-9-10 4-8-6 1-6-2 1-0-0 2-0-0 2-4 4-8-6 4-8-6 4-9-10 4-0-11 1-3-8
Scale = 1:59.6
7x8 \\
3x6 II
7x8 //
8.00 12
5x8 =
5x8 =
4-1-4
1-4-13
3x6 II
6x9 =
5x8 =
5x9 =
3x6 II

TOTAL WEIGHT = 2 X 194 = 388 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x6	DRY	1650F 1.5E	SPF
T - P	2x6	DRY	1650F 1.5E	SPF
P - M	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- G	2	12	SIDE(183.1)
G- J	2	12	TOP
J- L	2	12	TOP
W- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- T	2	12	TOP
T- P	2	12	SIDE(183.1)
P- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only

DWG# T-2017046 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	4301	0	4301	0	0	5-8	5-8
M	3993	0	3993	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	3033	2039 / 0	0 / 0	0 / 0	0 / 0	993 / 0	0 / 0
M	2816	1893 / 0	0 / 0	0 / 0	0 / 0	922 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 36	-91.8	-91.8	0.04 (1)	10.00	V- C	-814 / 0	0.10 (1)
B- C	-5385 / 0	-91.8	-91.8	0.11 (1)	4.97	C- U	0 / 5622	0.70 (1)
C- D	-8933 / 0	-91.8	-91.8	0.23 (1)	3.95	U- D	-3387 / 0	0.41 (1)
D- E	-12764 / 0	-91.8	-91.8	0.36 (1)	3.27	D- S	0 / 4849	0.60 (1)
E- X	-14096 / 0	-91.8	-91.8	0.52 (1)	2.94	S- E	-1606 / 0	0.19 (1)
X- Y	-14096 / 0	-91.8	-91.8	0.52 (1)	2.94	E- R	0 / 1686	0.21 (1)
Y- F	-14096 / 0	-91.8	-91.8	0.52 (1)	2.94	R- F	-608 / 0	0.07 (1)
F- G	-14096 / 0	-91.8	-91.8	0.45 (1)	3.03	R- H	0 / 3161	0.39 (1)
G- H	-14096 / 0	-91.8	-91.8	0.45 (1)	3.03	Q- H	-2359 / 0	0.28 (1)
H- I	-11599 / 0	-91.8	-91.8	0.31 (1)	3.45	Q- I	0 / 4305	0.53 (1)
I- J	-8198 / 0	-91.8	-91.8	0.20 (1)	4.12	O- I	-3079 / 0	0.37 (1)
J- K	-4965 / 0	-91.8	-91.8	0.10 (1)	5.14	O- J	0 / 5136	0.64 (1)
K- L	0 / 36	-91.8	-91.8	0.04 (1)	10.00	N- J	-746 / 0	0.09 (1)
W- B	-4284 / 0	0.0	0.0	0.15 (1)	6.91	B- V	0 / 4572	0.57 (1)
M- K	-3975 / 0	0.0	0.0	0.14 (1)	7.13	N- K	0 / 4215	0.52 (1)
W- V	0 / 0	-18.5	-18.5	0.02 (1)	10.00			
V- U	0 / 4445	-18.5	-18.5	0.21 (1)	10.00			
U- T	0 / 8933	-18.5	-18.5	0.40 (1)	10.00			
T- S	0 / 8933	-18.5	-18.5	0.40 (1)	10.00			
S- Z	0 / 12764	-18.5	-18.5	0.69 (1)	10.00			
Z- AA	0 / 12764	-18.5	-18.5	0.69 (1)	10.00			
AA- AB	0 / 12764	-18.5	-18.5	0.69 (1)	10.00			
AB- R	0 / 12764	-18.5	-18.5	0.69 (1)	10.00			
R- AC	0 / 11599	-18.5	-18.5	0.64 (1)	10.00			
AC- Q	0 / 11599	-18.5	-18.5	0.64 (1)	10.00			
Q- P	0 / 8198	-18.5	-18.5	0.36 (1)	10.00			
P- O	0 / 8198	-18.5	-18.5	0.36 (1)	10.00			
O- N	0 / 4099	-18.5	-18.5	0.19 (1)	10.00			
N- M	0 / 0	-18.5	-18.5	0.02 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	18-0-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
R	18-0-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
X	15-0-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
Y	16-0-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
Z	14-1-8	-1425	-1425	---	BACK	VERT	TOTAL	---	C1
AA	15-0-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
AB	16-0-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
AC	18-8-8	-2173	-2173	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.34")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 711 (0.62")

CSI: TC=0.52/1.00 (E-F:1) , BC=0.69/1.00 (R-S:1) , WB=0.70/1.00 (C-U:1) , SSI=0.68/1.00 (Q-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.77 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410046	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.75
C	TTWW+m	MT20	7.0	8.0	3.25	1.75
D	TMWW-t	MT20	5.0	6.0	2.50	2.25
E	TMWW-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.25
J	TTWW+m	MT20	7.0	8.0	3.25	1.75
K	TMVW-t	MT20	5.0	8.0	2.50	3.75
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMWW-t	MT20	6.0	9.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.25
R	BMWWW-t	MT20	5.0	8.0	2.75	4.00
S	BMWW-t	MT20	5.0	6.0	2.50	2.25
T	BS-t	MT20	5.0	6.0		
U	BMWW-t	MT20	6.0	9.0		
V	BMWW-t	MT20	5.0	6.0	2.50	2.50
W	BMV1+p	MT20	3.0	6.0		

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

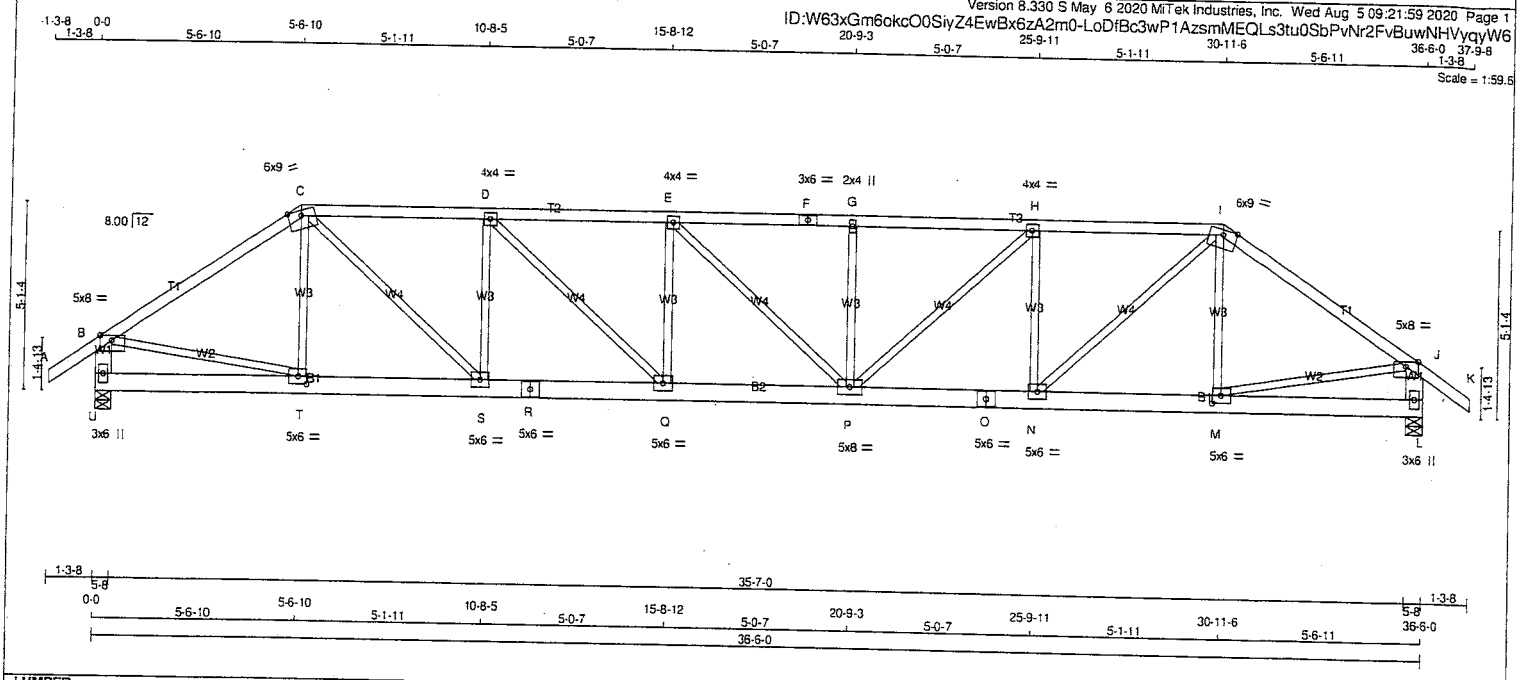
LICENSED PROFESSIONAL ENGINEER
07/31/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017046

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T2	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:21:59 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-LoDfBc3wP1AzsmMEQLs3tu0SbPvNr2FvBuwNHVyqyW6



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	6.0	9.0	Edge	
D, E, H						
D	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
I	TTWW-m	MT20	6.0	9.0	Edge	
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N, Q, S						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.75
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2138	0	2138	0	0	5-8	5-8
L	2138	0	2138	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 0
L	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)
A-B	0 / 35
B-C	-2437 / 0
C-D	-3265 / 0
D-E	-3843 / 0
E-F	-3835 / 0
F-G	-3835 / 0
G-H	-3835 / 0
H-I	-3267 / 0
I-J	-2436 / 0
J-K	0 / 35
U-B	-2093 / 0
L-J	-2092 / 0
U-T	0 / 0
T-S	0 / 2019
S-R	0 / 3265
R-Q	0 / 3265
Q-P	0 / 3843
P-O	0 / 3267
O-N	0 / 3267
N-M	0 / 2019
M-L	0 / 0

FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
-91.8	-91.8	0.12 (1)		10.00
-91.8	-91.8	0.73 (1)		3.67
-91.8	-91.8	0.56 (1)		3.41
-91.8	-91.8	0.63 (1)		3.09
-91.8	-91.8	0.52 (1)		3.23
-91.8	-91.8	0.52 (1)		3.23
-91.8	-91.8	0.63 (1)		3.09
-91.8	-91.8	0.56 (1)		3.41
-91.8	-91.8	0.73 (1)		3.67
-91.8	-91.8	0.12 (1)		10.00
0.0	0.0	0.14 (1)		7.02
0.0	0.0	0.14 (1)		7.02
-18.5	-18.5	0.06 (4)		10.00
-18.5	-18.5	0.27 (1)		10.00
-18.5	-18.5	0.43 (1)		10.00
-18.5	-18.5	0.43 (1)		10.00
-18.5	-18.5	0.51 (1)		10.00
-18.5	-18.5	0.44 (1)		10.00
-18.5	-18.5	0.44 (1)		10.00
-18.5	-18.5	0.27 (1)		10.00
-18.5	-18.5	0.06 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-C	-241 / 4	0.09 (1)
C-S	0 / 1690	0.38 (1)
S-D	-1058 / 0	0.38 (1)
D-Q	0 / 792	0.18 (1)
Q-E	-444 / 0	0.16 (1)
E-P	-11 / 0	0.01 (1)
P-G	-446 / 0	0.16 (1)
P-H	0 / 779	0.18 (1)
N-H	-1051 / 0	0.38 (1)
N-I	0 / 1693	0.38 (1)
M-I	-243 / 4	0.09 (1)
B-T	0 2056	0.46 (1)
M-J	0 2056	0.46 (1)

TOTAL WEIGHT = 2 X 173 = 346 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.40")

CSI: TC=0.73/1.00 (B-C:1) , BC=0.51/1.00 (P-Q:1) , WB=0.46/1.00 (B-T:1) , SSI=0.22/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

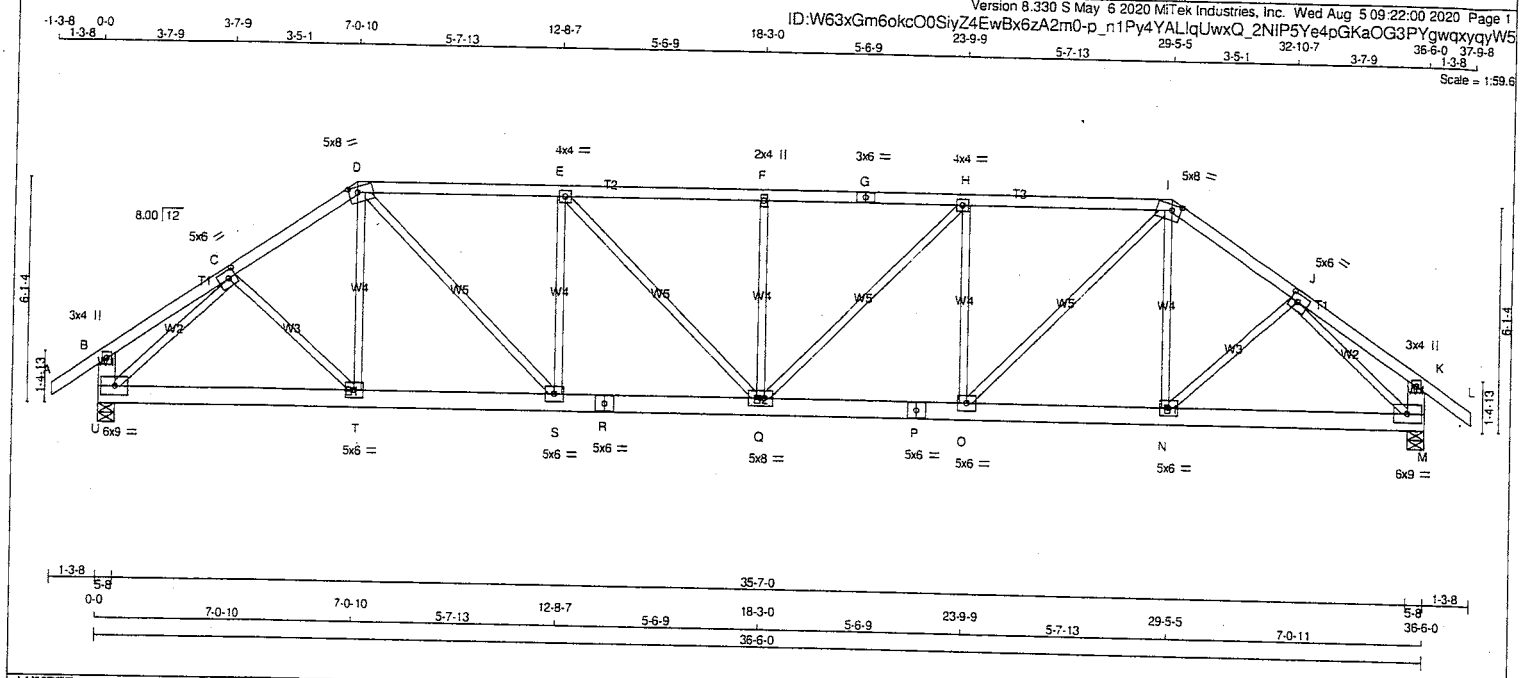
JSI GRIP= 0.83 (T) (INPUT = 0.90)
JSI METAL= 0.60 (O) (INPUT = 1.00)

Structural component only
DWG# T-2017331

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T3	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:22:00 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-p_n1Py4YALlqUwxQ_2NiP5Ye4pGKaOG3PYgwqxyqyW5



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW-m	MT20	5.0	8.0	1.75	3.00
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	8.0	1.75	3.00
J	TMWW-t	MT20	5.0	6.0	2.50	2.50
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	6.0	9.0		
N, O, S, T						
N	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
U	BMVW1-t	MT20	6.0	9.0		

Structural component only
DWG# T-2017332

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 2 X 178 = 357 lb [M]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2138	0	2138	0	0	5-8	5-8
M	2138	0	2138	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0
M	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 18	-91.8	-91.8	0.16 (1)	10.00
C- D	-2445 / 0	-91.8	-91.8	0.24 (1)	4.19
D- E	-2963 / 0	-91.8	-91.8	0.65 (1)	3.45
E- F	-3234 / 0	-91.8	-91.8	0.68 (1)	3.28
F- G	-3234 / 0	-91.8	-91.8	0.68 (1)	3.28
G- H	-3234 / 0	-91.8	-91.8	0.68 (1)	3.28
H- I	-2963 / 0	-91.8	-91.8	0.65 (1)	3.45
I- J	-2445 / 0	-91.8	-91.8	0.24 (1)	4.19
J- K	0 / 18	-91.8	-91.8	0.16 (1)	10.00
K- L	0 / 35	-91.8	-91.8	0.12 (1)	10.00
U- B	-255 / 0	0.0	0.0	0.02 (1)	7.81
M- K	-255 / 0	0.0	0.0	0.02 (1)	7.81
U- T	0 / 1922	-18.5	-18.5	0.29 (1)	10.00
T- S	0 / 2016	-18.5	-18.5	0.28 (1)	10.00
S- R	0 / 2963	-18.5	-18.5	0.40 (1)	10.00
R- Q	0 / 2963	-18.5	-18.5	0.40 (1)	10.00
Q- P	0 / 2963	-18.5	-18.5	0.40 (1)	10.00
P- O	0 / 2963	-18.5	-18.5	0.40 (1)	10.00
O- N	0 / 2016	-18.5	-18.5	0.28 (1)	10.00
N- M	0 / 1922	-18.5	-18.5	0.29 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	0 / 126	0.03 (4)
T- D	0 / 93	0.03 (4)
D- S	0 / 1347	0.30 (1)
S- E	-859 / 0	0.47 (1)
E- Q	0 / 389	0.09 (1)
Q- F	-468 / 0	0.26 (1)
Q- H	0 / 389	0.09 (1)
O- H	-859 / 0	0.47 (1)
O- I	0 / 1347	0.30 (1)
N- I	0 / 93	0.03 (4)
N- J	0 / 126	0.03 (4)
U- C	-2651 / 0	0.93 (1)
J- M	-2651 / 0	0.93 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.68/1.00 (F-H:1) , BC=0.40/1.00 (O-Q:1) , WB=0.93/1.00 (J-M:1) , SSI=0.24/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

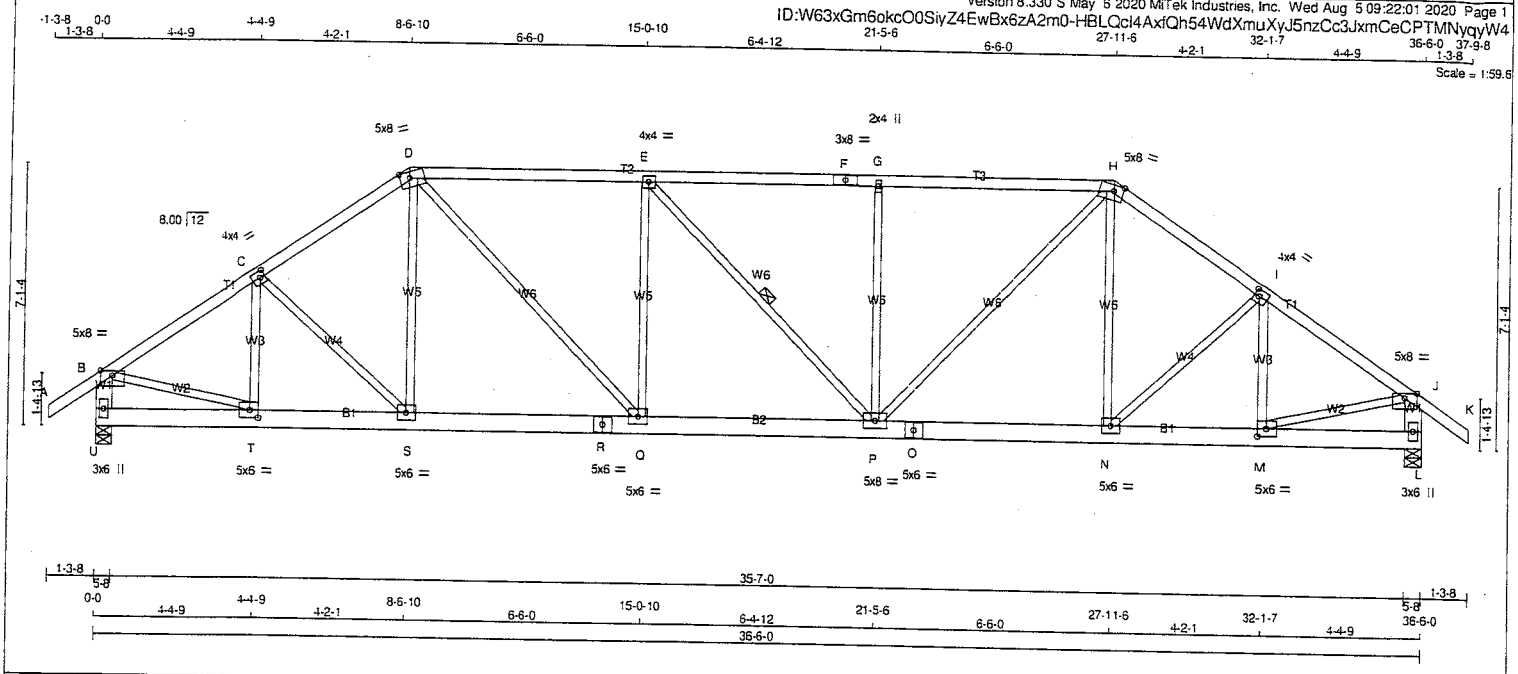
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (D) (INPUT = 0.90)
JSI METAL= 0.60 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T4	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:22:01 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-HBLQcl4AxiQh54WdXmuXyJ5nzCc3JxmCeCPTMNyqyW4



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	8.0	2.00	3.25
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.00	3.25
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N, Q, S						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.50
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2017333

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2138	0	2138	0	0	5-8	5-8
L	2138	0	2138	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0
L	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)		10.00
B- C	-2399 / 0	-91.8	-91.8	0.38 (1)		4.09
C- D	-2398 / 0	-91.8	-91.8	0.37 (1)		4.09
D- E	-2706 / 0	-91.8	-91.8	0.80 (1)		3.42
E- F	-2701 / 0	-91.8	-91.8	0.79 (1)		3.42
F- G	-2701 / 0	-91.8	-91.8	0.79 (1)		3.42
G- H	-2701 / 0	-91.8	-91.8	0.79 (1)		3.44
H- I	-2399 / 0	-91.8	-91.8	0.37 (1)		4.09
I- J	-2398 / 0	-91.8	-91.8	0.38 (1)		4.09
J- K	0 / 35	-91.8	-91.8	0.12 (1)		10.00
U- B	-2086 / 0	0.0	0.0	0.14 (1)		7.03
L- J	-2086 / 0	0.0	0.0	0.14 (1)		7.03
U- T	0 / 0	-18.5	-18.5	0.05 (1)		10.00
T- S	0 / 2016	-18.5	-18.5	0.28 (1)		10.00
S- R	0 / 1973	-18.5	-18.5	0.28 (1)		10.00
R- Q	0 / 1973	-18.5	-18.5	0.28 (1)		10.00
Q- P	0 / 2706	-18.5	-18.5	0.37 (1)		10.00
P- O	0 / 1974	-18.5	-18.5	0.28 (1)		10.00
O- N	0 / 1974	-18.5	-18.5	0.28 (1)		10.00
N- M	0 / 2016	-18.5	-18.5	0.29 (1)		10.00
M- L	0 / 0	-18.5	-18.5	0.05 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-417 / 0	0.11 (1)
C- S	-62 / 0	0.03 (1)
S- D	0 / 171	0.04 (4)
D- Q	0 / 1053	0.24 (1)
Q- E	-639 / 0	0.52 (1)
E- P	-7 / 0	0.00 (1)
P- G	-641 / 0	0.53 (1)
P- H	0 / 1045	0.24 (1)
N- H	0 / 177	0.05 (4)
N- I	-59 / 0	0.03 (1)
M- I	-419 / 0	0.11 (1)
B- T	0 / 2068	0.47 (1)
M- J	0 / 2067	0.47 (1)

TOTAL WEIGHT = 2 X 182 = 363 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.80/1.00 (D-E:1) , BC=0.37/1.00 (P-Q:1) , WB=0.53/1.00 (G-P:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

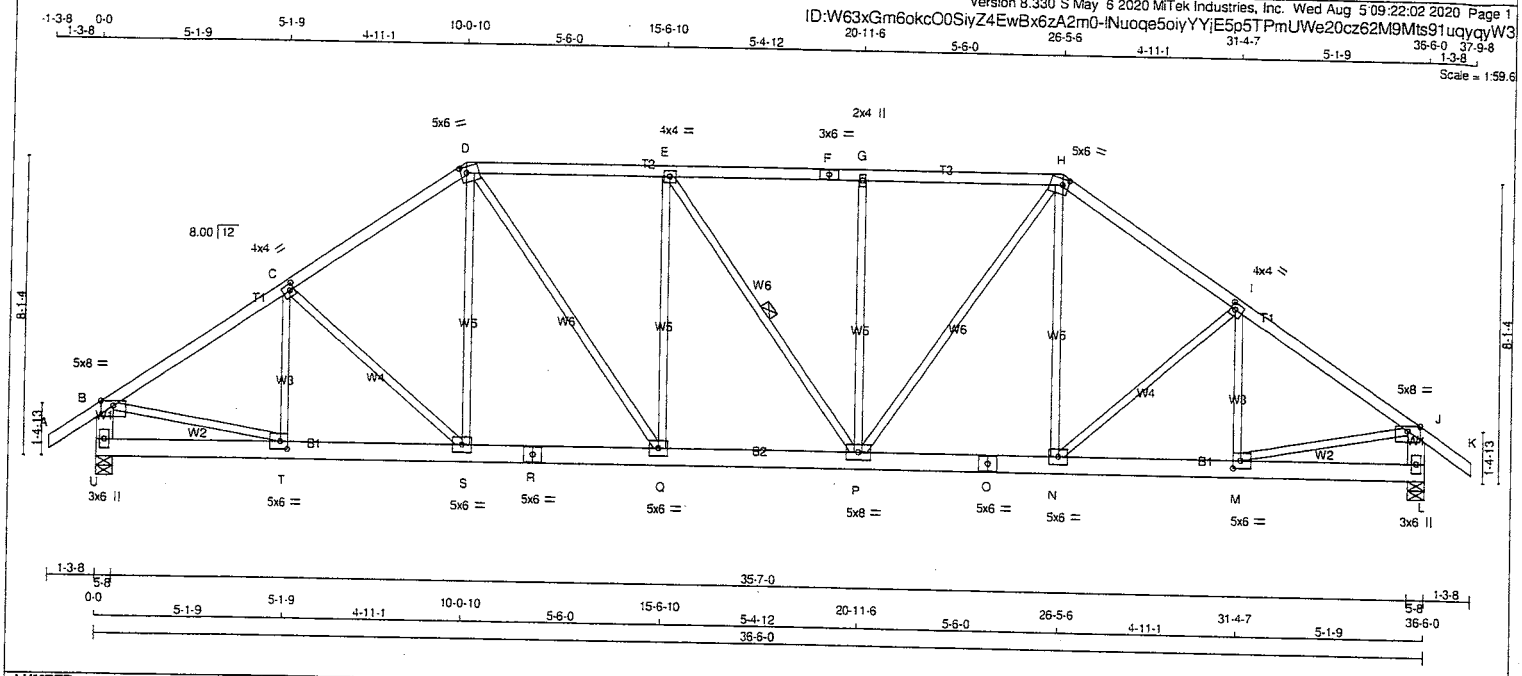
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (T) (INPUT = 0.90)
JSI METAL= 0.46 (T) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T5	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:22:02 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-lNuoqe5oiyYYjE5p5TPmUWe20cz62M9Mts91uqyqyW3



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.00	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.00	2.00
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N, Q, S						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.50
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2017334

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 2 X 189 = 379 lb [M][F]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
U	2138	0	2138	0	0	IN-SX	IN-SX
L	2138	0	2138	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 0	0 / 0
L	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-2451 / 0	-91.8	-91.8	0.40 (1)	4.05
C- D	-2322 / 0	-91.8	-91.8	0.38 (1)	4.16
D- E	-2362 / 0	-91.8	-91.8	0.39 (1)	4.10
E- F	-2357 / 0	-91.8	-91.8	0.37 (1)	4.14
F- G	-2357 / 0	-91.8	-91.8	0.37 (1)	4.14
G- H	-2357 / 0	-91.8	-91.8	0.39 (1)	4.11
H- I	-2323 / 0	-91.8	-91.8	0.38 (1)	4.16
I- J	-2451 / 0	-91.8	-91.8	0.40 (1)	4.05
J- K	0 / 35	-91.8	-91.8	0.12 (1)	10.00
U- B	-2085 / 0	0.0	0.0	0.14 (1)	7.03
L- J	-2085 / 0	0.0	0.0	0.14 (1)	7.03
U- T	0 / 0	-18.5	-18.5	0.06 (4)	10.00
T- S	0 / 2064	-18.5	-18.5	0.28 (1)	10.00
S- R	0 / 1907	-18.5	-18.5	0.26 (1)	10.00
R- Q	0 / 1907	-18.5	-18.5	0.26 (1)	10.00
Q- P	0 2362	-18.5	-18.5	0.32 (1)	10.00
P- O	0 / 1909	-18.5	-18.5	0.26 (1)	10.00
O- N	0 / 1909	-18.5	-18.5	0.26 (1)	10.00
N- M	0 2063	-18.5	-18.5	0.28 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.06 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-328 / 0	0.11 (1)
C- S	-213 / 0	0.17 (1)
S- D	0 / 257	0.06 (1)
D- Q	0 / 783	0.18 (1)
Q- E	-536 / 0	0.64 (1)
E- P	-8 / 0	0.01 (1)
P- G	-539 0	0.64 (1)
P- H	0 / 773	0.17 (1)
N- H	0 / 265	0.06 (1)
N- I	-211 / 0	0.17 (1)
M- I	-330 / 0	0.11 (1)
B- T	0 2102	0.47 (1)
M- J	0 / 2102	0.47 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.40/1.00 (B-C:1) , BC=0.32/1.00 (P-Q:1) , WB=0.64/1.00 (G-P:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

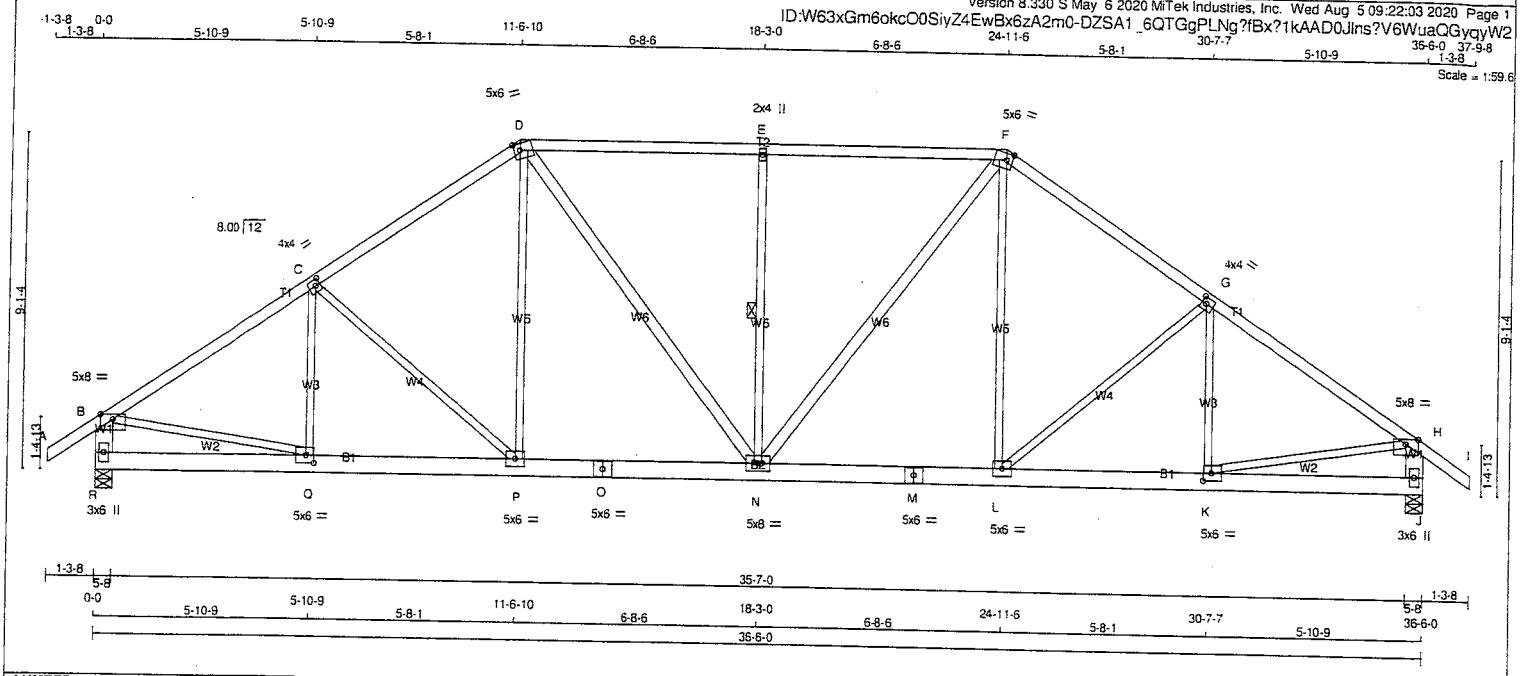
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (T) (INPUT = 0.90)
JSI METAL= 0.47 (T) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T6	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:22:03 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-DZSA1_6QTGgPLNg?fBx?1kAAD0Jlns?V6WuaQGyqyW2



TOTAL WEIGHT = 2 X 192 = 384 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BS-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.75
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2138	0	2138	0	0	5-8	5-8
J	2138	0	2138	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 0
J	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)		10.00
B- C	-2479 / 0	-91.8	-91.8	0.53 (1)		3.89
C- D	-2234 / 0	-91.8	-91.8	0.49 (1)		4.10
D- E	-2165 / 0	-91.8	-91.8	0.62 (1)		3.91
E- F	-2165 / 0	-91.8	-91.8	0.62 (1)		3.91
F- G	-2234 / 0	-91.8	-91.8	0.49 (1)		4.10
G- H	-2479 / 0	-91.8	-91.8	0.53 (1)		3.89
H- I	0 / 35	-91.8	-91.8	0.12 (1)		10.00
R- B	-2082 / 0	0.0	0.0	0.14 (1)		7.03
J- H	-2082 / 0	0.0	0.0	0.14 (1)		7.03
R- Q	0 / 0	-18.5	-18.5	0.07 (4)		10.00
Q- P	0 / 2091	-18.5	-18.5	0.29 (1)		10.00
P- O	0 / 1831	-18.5	-18.5	0.26 (1)		10.00
O- N	0 / 1831	-18.5	-18.5	0.26 (1)		10.00
N- M	0 / 1831	-18.5	-18.5	0.26 (1)		10.00
M- L	0 / 1831	-18.5	-18.5	0.26 (1)		10.00
L- K	0 / 2091	-18.5	-18.5	0.29 (1)		10.00
K- J	0 / 0	-18.5	-18.5	0.07 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-261 6	0.10 (1)
C- P	-347 0	0.39 (1)
P- D	0 / 355	0.08 (1)
D- N	0 / 549	0.09 (1)
N- E	-754 0	0.39 (1)
N- F	0 / 549	0.09 (1)
L- F	0 / 355	0.08 (1)
L- G	-347 / 0	0.39 (1)
K- G	-261 / 6	0.10 (1)
B- Q	0 / 2121	0.48 (1)
K- H	0 2121	0.48 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.62/1.00 (D-E:1) , BC=0.29/1.00 (K-L:1) , WB=0.48/1.00 (H-K:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (Q) (INPUT = 0.90)
JSI METAL= 0.48 (Q) (INPUT = 1.00)

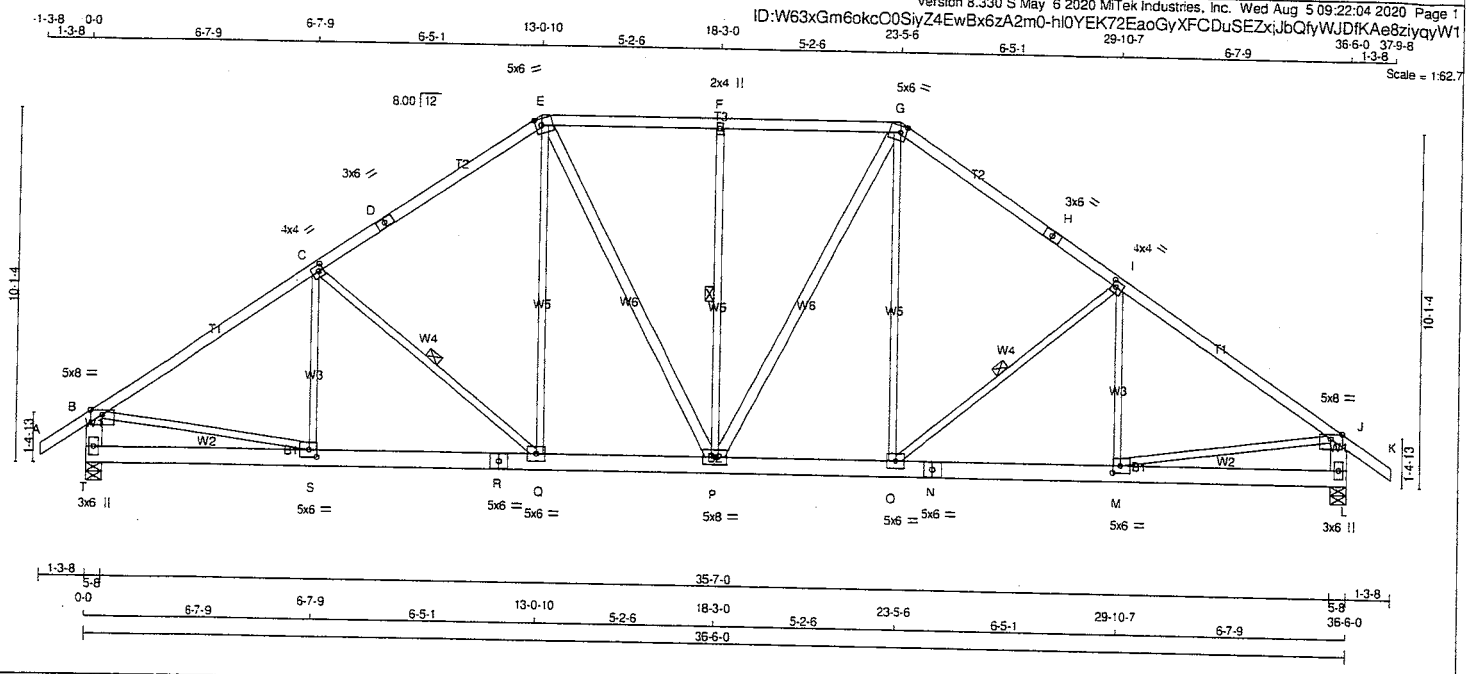
LICENSED PROFESSIONAL ENGINEER
08/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017335

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T7	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 09:22:04 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-hl0YEK72EaoGyXFCDuSEZxjJbQfyWJDfKAe8ziyqyW1



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BS-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	5.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.75
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	2138	0	2138	0	0	IN-SX	IN-SX
L	2138	0	2138	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 0	0 / 0
L	1510	1004 / 0	0 / 0	0 / 0	0 0	506 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-Q, F-P, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)		10.00
B- C	-2492 / 0	-91.8	-91.8	0.71 (1)		3.66
C- D	-2132 / 0	-91.8	-91.8	0.64 (1)		3.98
D- E	-2132 / 0	-91.8	-91.8	0.64 (1)		3.98
E- F	-1920 / 0	-91.8	-91.8	0.36 (1)		4.49
F- G	-1920 / 0	-91.8	-91.8	0.36 (1)		4.49
G- H	-2132 / 0	-91.8	-91.8	0.64 (1)		3.98
H- I	-2132 / 0	-91.8	-91.8	0.64 (1)		3.98
I- J	-2492 / 0	-91.8	-91.8	0.71 (1)		3.66
J- K	0 35	-91.8	-91.8	0.12 (1)		10.00
T- B	-2080 / 0	0.0	0.0	0.14 (1)		7.03
L- J	-2080 / 0	0.0	0.0	0.14 (1)		7.03
T- S	0 0	-18.5	-18.5	0.08 (4)		10.00
S- R	0 2107	-18.5	-18.5	0.29 (1)		10.00
R- Q	0 / 2107	-18.5	-18.5	0.29 (1)		10.00
Q- P	0 / 1743	-18.5	-18.5	0.24 (1)		10.00
P- O	0 / 1743	-18.5	-18.5	0.24 (1)		10.00
O- N	0 / 2107	-18.5	-18.5	0.29 (1)		10.00
N- M	0 / 2107	-18.5	-18.5	0.29 (1)		10.00
M- L	0 0	-18.5	-18.5	0.08 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-197 43	0.10 (1)
C- Q	-479 / 0	0.23 (1)
Q- E	0 / 425	0.10 (1)
E- P	0 375	0.06 (1)
P- F	-580 / 0	0.39 (1)
P- G	0 / 375	0.06 (1)
O- G	0 / 425	0.10 (1)
O- I	-479 / 0	0.23 (1)
M- I	-197 43	0.10 (1)
B- S	0 2131	0.48 (1)
M- J	0 / 2131	0.48 (1)

TOTAL WEIGHT = 3 X 199 = 596 lb
[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.71/1.00 (I-J:1) , BC=0.29/1.00 (Q-S:1) , WB=0.48/1.00 (B-S:1) , SSI=0.24/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

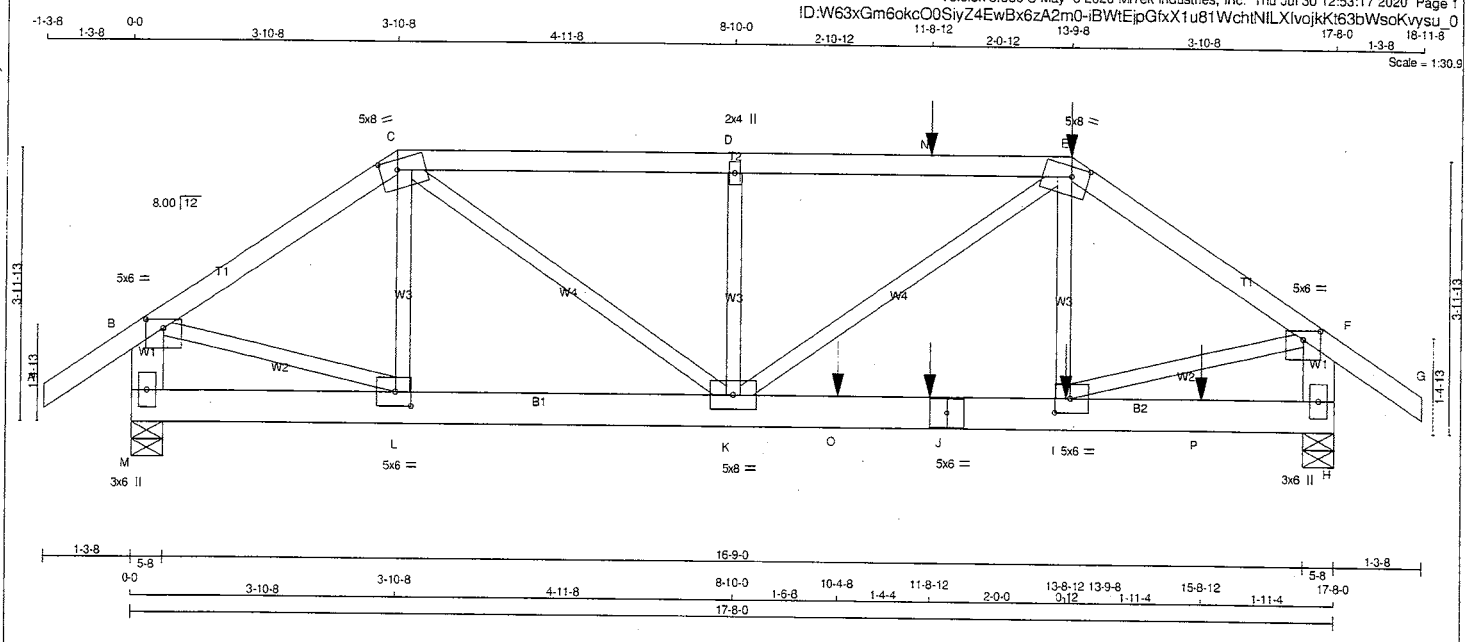
JSI GRIP= 0.84 (M) (INPUT = 0.90)
JSI METAL= 0.48 (S) (INPUT = 1.00)

Structural component only
DWG# T-2017336

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410046	T9	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:53:17 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-iBWtEjpGfxX1u81WchtNiLXlvojkKt63bWsoKvysu_0



TOTAL WEIGHT = 2 X 84 = 167 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	SIDE(61.0)
E- G	1	12	SIDE(61.0)
M- B	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	SIDE(0.0)
J- H	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1588	0	1588	0	0	5-8	5-8
H	1953	0	1953	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1118	763 / 0	0 / 0	0 / 0	0 / 0	355 / 0	0 / 0
H	1375	937 / 0	0 / 0	0 / 0	0 / 0	437 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	L- C	-336 / 0		0.04 (1)
B- C	-1616 / 0	-91.8	-91.8	0.14 (1)	6.25	C- K	0 / 1501		0.19 (1)
C- D	-2547 / 0	-91.8	-91.8	0.25 (1)	5.39	K- D	-591 / 0		0.07 (1)
D- N	-2548 / 0	-91.8	-91.8	0.25 (1)	5.39	K- E	0 / 898		0.11 (1)
N- E	-2548 / 0	-91.8	-91.8	0.25 (1)	5.39	I- E	0 / 105		0.02 (4)
E- F	-2191 / 0	-91.8	-91.8	0.15 (1)	5.85	B- L	0 / 1384		0.17 (1)
F- G	0 / 35	-91.8	-91.8	0.07 (1)	10.00	I- F	0 / 1876		0.23 (1)
M- B	-1540 / 0	0.0	0.0	0.06 (1)	7.81				
H- F	-1980 / 0	0.0	0.0	0.07 (1)	7.81				
M- L	0 / 0	-18.5	-18.5	0.02 (1)	10.00				
L- K	0 / 1333	-18.5	-18.5	0.20 (1)	10.00				
K- O	0 / 1821	-18.5	-18.5	0.40 (1)	10.00				
O- J	0 / 1821	-18.5	-18.5	0.40 (1)	10.00				
J- I	0 / 1821	-18.5	-18.5	0.40 (1)	10.00				
I- P	0 / 0	-18.5	-18.5	0.11 (1)	10.00				
P- H	0 / 0	-18.5	-18.5	0.11 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	13-9-8	-253	-253	---	FRONT	VERT	TOTAL	--	C1
I	13-8-12	-17	-17	---	FRONT	VERT	TOTAL	--	C1
J	11-8-12	-17	-17	---	FRONT	VERT	TOTAL	--	C1
N	11-8-12	-86	-86	---	FRONT	VERT	TOTAL	--	C1
O	10-4-8	-950	-950	---	FRONT	VERT	TOTAL	--	C1
P	15-8-12	-17	-17	---	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.25/1.00 (D-E:1) , BC=0.40/1.00 (I-K:1) , WB=0.23/1.00 (F-I:1) , SSI=0.23/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (E) (INPUT = 0.90)
JSI METAL= 0.29 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07/31/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017047 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410046	T9	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-iBWtEjpGfxX1u81WchtNtLXivojkKt63bWsoKvysu_0

PLATES (table is in inches)

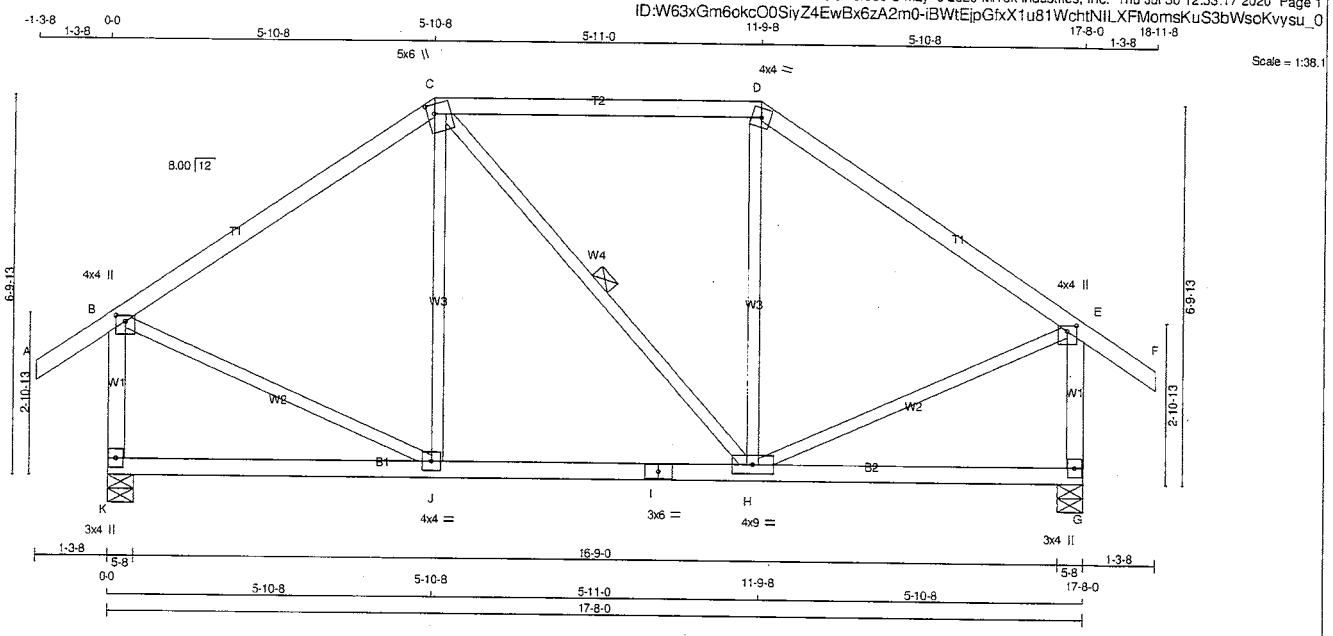
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-m	MT20	5.0	8.0	1.75	3.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	1.75	3.00
F	TMVW-p	MT20	5.0	6.0	1.50	3.00
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0	2.50	2.75
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2017047

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410046	T10	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:53:17 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-iBWtEjpGfxX1u81WchtNILXFMomsKuS3bWscKvysu_0



TOTAL WEIGHT = 79 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1100	0	1100	0	0	5-8	5-8
G	1100	0	1100	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	775	522 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
G	775	522 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH FR-TO	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	J- C	-140 / 42	0.11 (1)
B- C	-721 / 0	-91.8	-91.8	0.42 (1)	6.25	C- H	0 / 0	0.00 (1)
C- D	-596 / 0	-91.8	-91.8	0.42 (1)	6.25	H- D	-140 / 42	0.11 (1)
D- E	-720 / 0	-91.8	-91.8	0.42 (1)	6.25	B- J	0 / 652	0.15 (1)
E- F	0 / 35	-91.8	-91.8	0.12 (1)	10.00	H- E	0 / 652	0.15 (1)
K- B	-1055 / 0	0.0	0.0	0.16 (1)	7.66			
G- E	-1054 / 0	0.0	0.0	0.16 (1)	7.66			
K- J	0 / 0	-18.5	-18.5	0.15 (4)	10.00			
J- I	0 / 597	-18.5	-18.5	0.20 (4)	10.00			
I- H	0 / 597	-18.5	-18.5	0.20 (4)	10.00			
H- G	0 / 0	-18.5	-18.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.42/1.00 (B-C:1) , BC=0.20/1.00 (H-J:4) , WB=0.15/1.00 (B-J:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (B) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

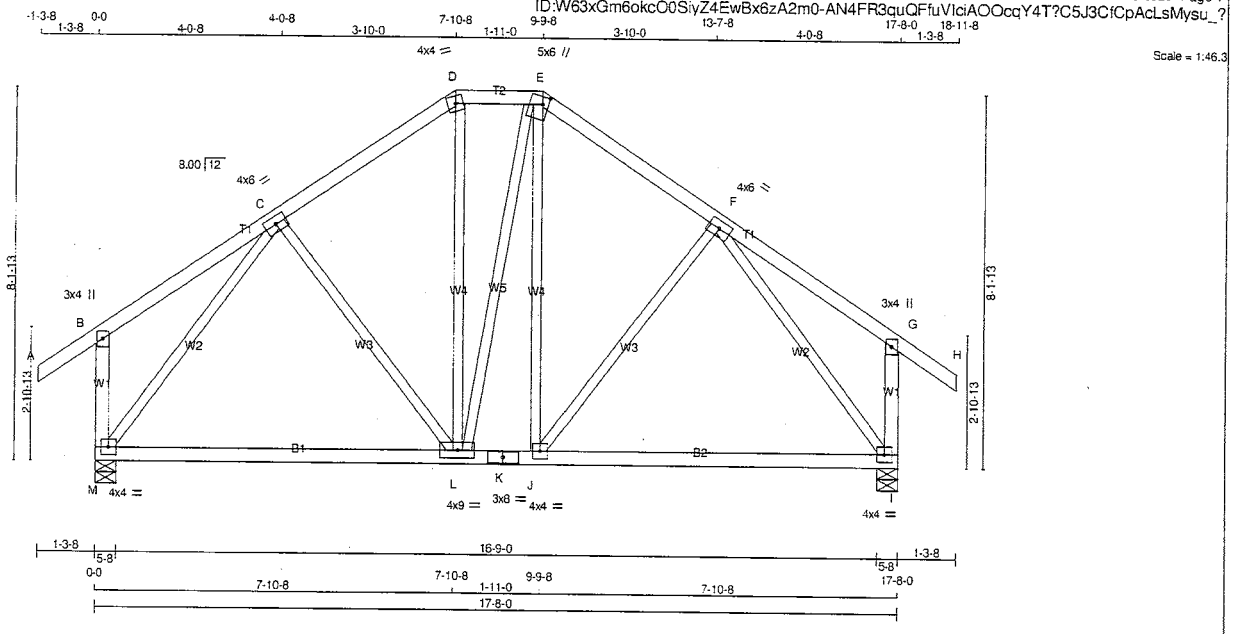
Structural component only
DWG# T-2017048

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410046	T11	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:53:18 2020 Page 1

ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-AN4FR3quQFfuVIciAOOcqY4T?C5J3CfCpAcLsMysu_?



TOTAL WEIGHT = 92 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	8.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1100	0	1100	0	0	5-8	5-8
I	1100	0	1100	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	775	522 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
I	775	522 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 24	-91.8	-91.8	0.23 (1)	10.00
C- D	-691 / 0	-91.8	-91.8	0.18 (1)	6.25
D- E	-556 / 0	-91.8	-91.8	0.05 (1)	6.25
E- F	-690 / 0	-91.8	-91.8	0.18 (1)	6.25
F- G	0 / 24	-91.8	-91.8	0.23 (1)	10.00
G- H	0 / 35	-91.8	-91.8	0.12 (1)	10.00
M- B	-265 / 0	0.0	0.0	0.04 (1)	7.81
I- G	-265 / 0	0.0	0.0	0.04 (1)	7.81
M- L	0 / 587	-18.5	-18.5	0.31 (4)	10.00
L- K	0 / 555	-18.5	-18.5	0.31 (4)	10.00
K- J	0 / 555	-18.5	-18.5	0.31 (4)	10.00
J- I	0 / 587	-18.5	-18.5	0.30 (4)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-54 / 30	0.04 (1)
L- D	0 / 146	0.03 (1)
L- E	0 / 3	0.00 (1)
J- E	0 / 143	0.03 (1)
J- F	-55 / 30	0.04 (1)
M- C	-972 / 0	0.72 (1)
F- I	-972 / 0	0.72 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.23/1.00 (F-G:1) , BC=0.31/1.00 (J-L:4) , WB=0.72/1.00 (C-M:1) , SSI=0.15/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

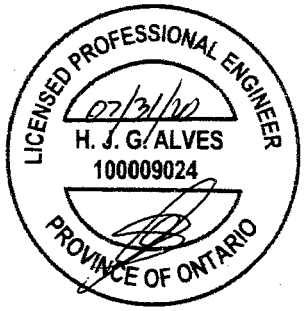
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (M) (INPUT = 0.90)
JSI METAL= 0.56 (K) (INPUT = 1.00)

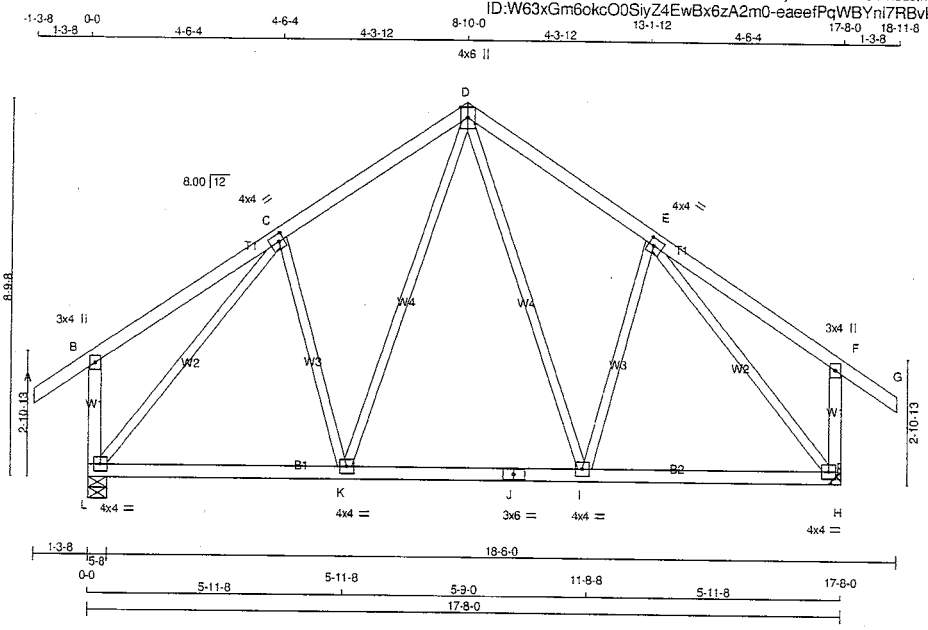


Structural component only
DWG# T-2017049

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410046	T12	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:53:19 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-eaeefPqWBYni7RBvk6vrNmddmbSOoc0M2qLuOoysu



TOTAL WEIGHT = 3 X 88 = 263 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1100	0	1100	0	0	5-8	5-8
H	1100	0	1100	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	775	522 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
H	775	522 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 26	-91.8	-91.8	0.29 (1)	10.00
C- D	-732 / 0	-91.8	-91.8	0.23 (1)	6.25
D- E	-732 / 0	-91.8	-91.8	0.23 (1)	6.25
E- F	0 / 26	-91.8	-91.8	0.29 (1)	10.00
F- G	0 / 35	-91.8	-91.8	0.12 (1)	10.00
L- B	-284 / 0	0.0	0.0	0.04 (1)	7.81
H- F	-284 / 0	0.0	0.0	0.04 (1)	7.81
L- K	0 / 620	-18.5	-18.5	0.19 (4)	10.00
K- J	0 / 506	-18.5	-18.5	0.19 (4)	10.00
J- I	0 / 506	-18.5	-18.5	0.19 (4)	10.00
I- H	0 / 620	-18.5	-18.5	0.19 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 253	0.06 (1)
I- E	-127 / 22	0.07 (1)
K- D	0 / 253	0.06 (1)
C- K	-127 / 22	0.07 (1)
L- C	-988 / 0	0.91 (1)
E- H	-988 / 0	0.91 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.19/1.00 (H-I:4) , WB=0.91/1.00 (C-L:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

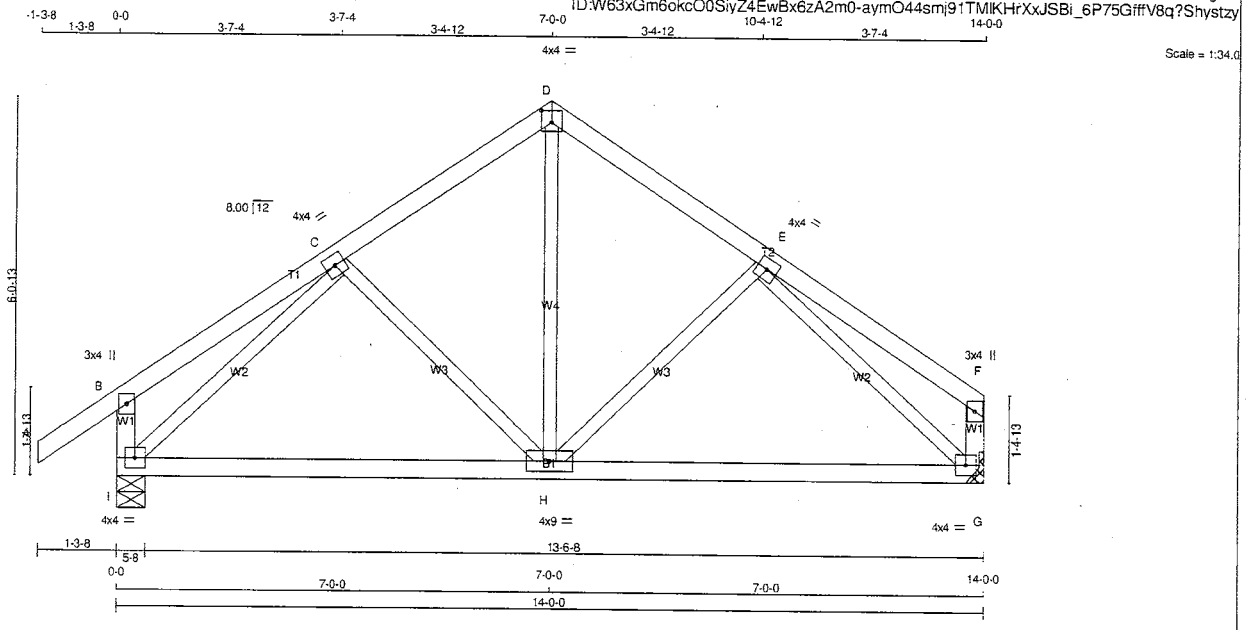
JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.34 (E) (INPUT = 1.00)

Structural component only
DWG# T-2017050

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410046	T13	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:53:21 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-aymO44smj91TMlKHrXxJSBi_6P75GiffV8q?Shystzy



TOTAL WEIGHT = 3 X 59 = 176 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	898	0	898	0	0	5-8	5-8
G	772	0	772	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	632	428 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0
G	546	358 / 0	0 / 0	0 / 0	0 / 0	188 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 20	-91.8	-91.8	0.18 (1)	10.00
C- D	-607 / 0	-91.8	-91.8	0.14 (1)	6.25
D- E	-607 / 0	-91.8	-91.8	0.14 (1)	6.25
E- F	0 / 20	-91.8	-91.8	0.18 (1)	10.00
I- B	-250 / 0	0.0	0.0	0.03 (1)	7.81
G- F	-124 / 0	0.0	0.0	0.01 (1)	7.81
I- H	0 / 613	-18.5	-18.5	0.31 (4)	10.00
H- G	0 / 613	-18.5	-18.5	0.31 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- H	-174 / 0	0.07 (1)
H- D	0 / 401	0.09 (1)
H- E	-173 / 0	0.07 (1)
I- C	-856 / 0	0.32 (1)
E- G	-856 / 0	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.31/1.00 (H-I:4) , WB=0.32/1.00 (C-I:1) , SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

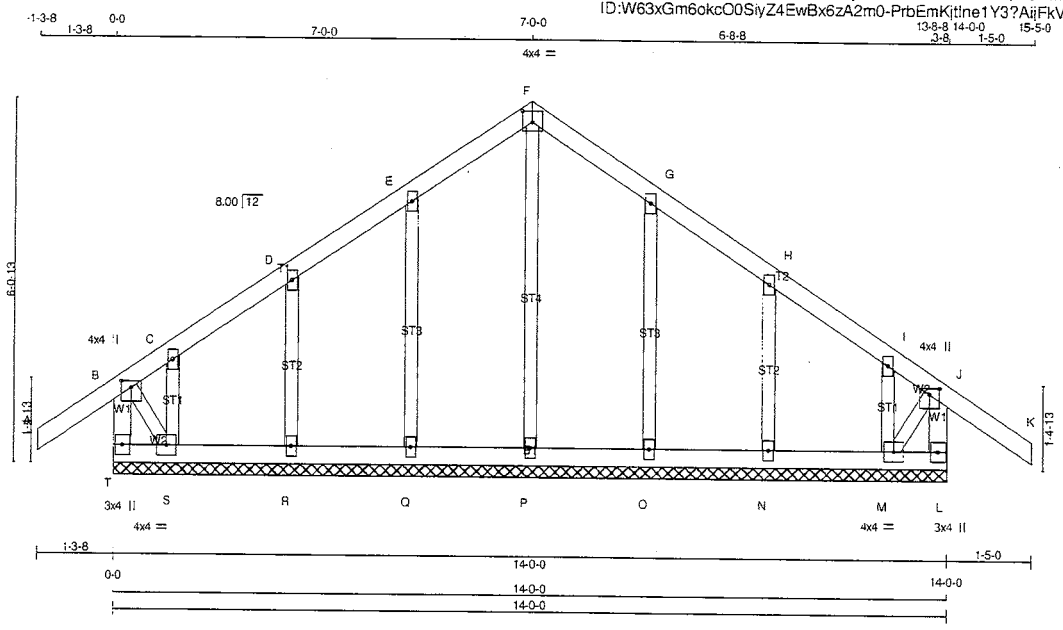
JSI GRIP= 0.70 (E) (INPUT = 0.90)
JSI METAL= 0.27 (E) (INPUT = 1.00)

Structural component only
DWG# T-2017051

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410046	G13	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:53:10 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-PrbEmKjtlne1Y3?AiiFkVsl6LzR4BIh1_xgwapysu_7



Scale = 1:35.3

TOTAL WEIGHT = 62 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	2.25	2.00
J	TMVW+p	MT20	4.0	4.0	1.25	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
T- B	-271 / 0	0.0	0.0	0.03 (1)		7.81
A- B	0 / 35	-91.8	-91.8	0.12 (1)		10.00
B- C	-60 / 0	-91.8	-91.8	0.12 (1)		6.25
C- D	-8 / 0	-91.8	-91.8	0.04 (1)		10.00
D- E	-6 / 0	-91.8	-91.8	0.05 (1)		10.00
E- F	-13 / 0	-91.8	-91.8	0.05 (1)		6.25
F- G	-13 / 0	-91.8	-91.8	0.05 (1)		6.25
G- H	-6 / 0	-91.8	-91.8	0.05 (1)		10.00
H- I	-8 / 0	-91.8	-91.8	0.04 (1)		10.00
I- J	-70 / 0	-91.8	-91.8	0.14 (1)		6.25
J- K	0 / 38	-91.8	-91.8	0.15 (1)		10.00
L- J	-301 / 0	0.0	0.0	0.03 (1)		7.81
T- S	0 / 0	-18.5	-18.5	0.01 (4)		10.00
S- R	0 / 11	-18.5	-18.5	0.02 (4)		10.00
R- Q	0 / 6	-18.5	-18.5	0.02 (4)		10.00
Q- P	0 / 3	-18.5	-18.5	0.01 (4)		10.00
P- O	0 / 3	-18.5	-18.5	0.01 (4)		10.00
O- N	0 / 6	-18.5	-18.5	0.02 (4)		10.00
N- M	0 / 11	-18.5	-18.5	0.02 (4)		10.00
M- L	0 / 0	-18.5	-18.5	0.01 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- F	-150 / 0	0.08 (1)
Q- E	-206 / 0	0.07 (1)
R- D	-187 / 0	0.04 (1)
S- C	-43 / 0	0.01 (1)
O- G	-206 / 0	0.07 (1)
N- H	-188 / 0	0.04 (1)
M- I	-22 / 0	0.00 (1)
B- S	0 / 20	0.00 (1)
M- J	0 / 18	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (J-K:1) , BC=0.02/1.00 (M-N:4) , WB=0.08/1.00 (F-P:1) , SSI=0.09/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

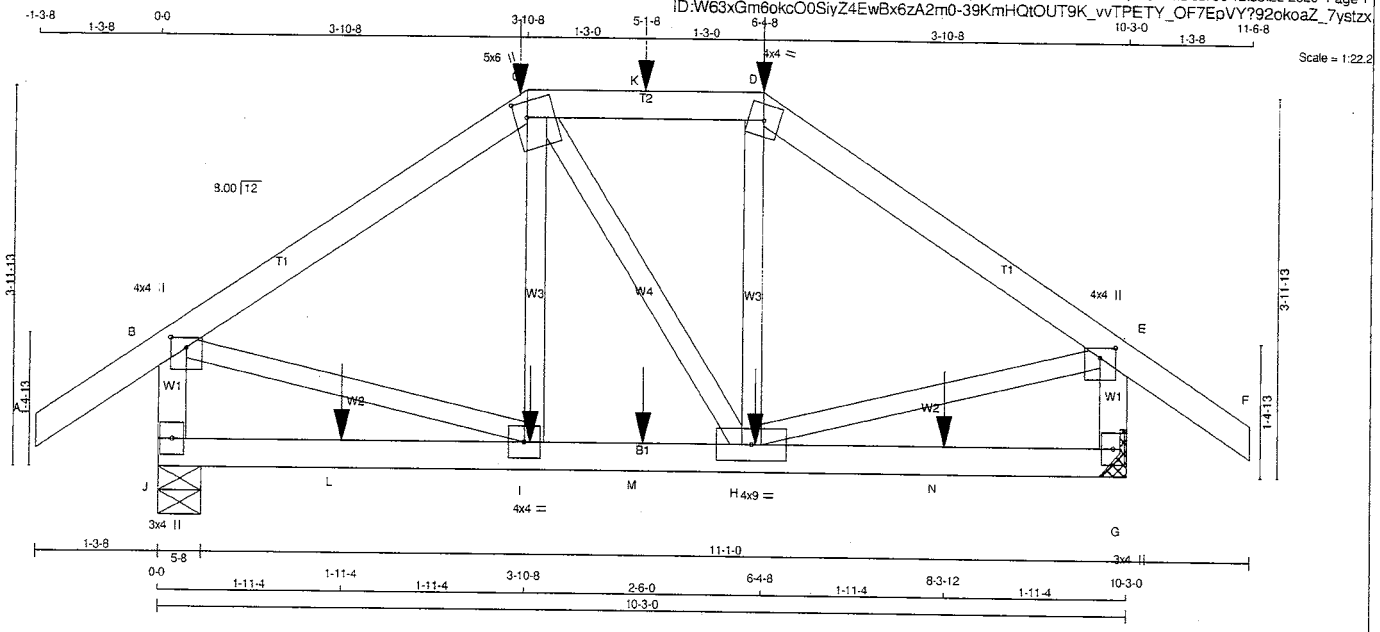
JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.11 (E) (INPUT = 1.00)

Structural component only
DWG# T-2017042

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410046	T14	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:53:22 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-39KmHQtOUT9K_vvTPETY_OF7EpVY?92okoaZ_7ystzx



TOTAL WEIGHT = 46 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1030	0	1030	0	0	IN-SX	IN-SX
G	1030	0	1030	0	0	5-8	5-8
						MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	726	494 / 0	0 / 0	0 / 0	0 / 0	232 / 0	0 / 0
G	725	494 / 0	0 / 0	0 / 0	0 / 0	232 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 35	-91.8	-91.8	0.14 (1)	10.00	I- C	-100 / 45	0.03 (1)
B-C	-878 / 0	-91.8	-91.8	0.28 (1)	6.15	C- H	0 / 2	0.00 (4)
C-K	-727 / 0	-91.8	-91.8	0.20 (1)	6.25	H- D	-98 / 47	0.03 (1)
K-D	-727 / 0	-91.8	-91.8	0.20 (1)	6.25	B- I	0 / 756	0.19 (1)
D-E	-880 / 0	-91.8	-91.8	0.28 (1)	6.14	H- E	0 / 757	0.19 (1)
E-F	0 / 35	-91.8	-91.8	0.14 (1)	10.00			
J-B	-991 / 0	0.0	0.0	0.11 (1)	7.81			
G-E	-990 / 0	0.0	0.0	0.11 (1)	7.81			
J-L	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
L-I	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
I-M	0 / 727	-18.5	-18.5	0.16 (1)	10.00			
M-H	0 / 727	-18.5	-18.5	0.16 (1)	10.00			
H-N	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
N-G	0 / 0	-18.5	-18.5	0.10 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-253	-253	---	FRONT	VERT	TOTAL	---	C1
D	6-4-8	-253	-253	---	FRONT	VERT	TOTAL	---	C1
H	6-3-12	-17	-17	---	FRONT	VERT	TOTAL	---	C1
I	3-11-4	-17	-17	---	FRONT	VERT	TOTAL	---	C1
K	5-1-8	-86	-86	---	FRONT	VERT	TOTAL	---	C1
L	1-11-4	-17	-17	---	FRONT	VERT	TOTAL	---	C1
M	5-1-8	-17	-17	---	FRONT	VERT	TOTAL	---	C1
N	8-3-12	-17	-17	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.28/1.00 (D-E:1) , BC=0.16/1.00 (H-I:1) , WB=0.19/1.00 (E-H:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

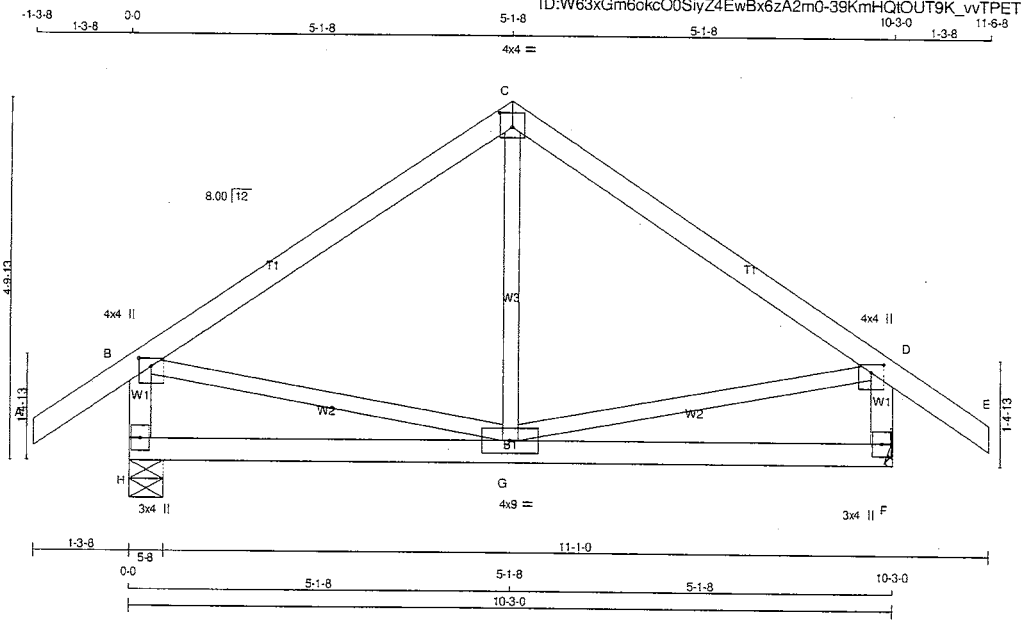
JSI GRIP= 0.86 (I) (INPUT = 0.90)
JSI METAL= 0.25 (I) (INPUT = 1.00)

Structural component only
DWG# T-2017052

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410046	T15	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:53:22 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-39KmHQtOUT9K_vvTPETY_OF7lpVz?AnokoaZ_7ystzx



TOTAL WEIGHT = 43 lb
(M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	691	0	691	0	0	5-8	5-8
F	691	0	691	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	486	332 / 0	0 / 0	0 / 0	0 / 0	154 / 0	0 / 0
F	486	332 / 0	0 / 0	0 / 0	0 / 0	154 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	G- C	-25 / 79	0.03 (4)
B- C	-402 / 0	-91.8	-91.8	0.31 (1)	6.25	B- G	0 / 342	0.08 (1)
C- D	-402 / 0	-91.8	-91.8	0.31 (1)	6.25	G- D	0 / 342	0.08 (1)
D- E	0 / 35	-91.8	-91.8	0.12 (1)	10.00			
H- B	-654 / 0	0.0	0.0	0.07 (1)	7.81			
F- D	-654 / 0	0.0	0.0	0.07 (1)	7.81			
H- G	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.14/1.00 (G-H:4) , WB=0.08/1.00 (D-G:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

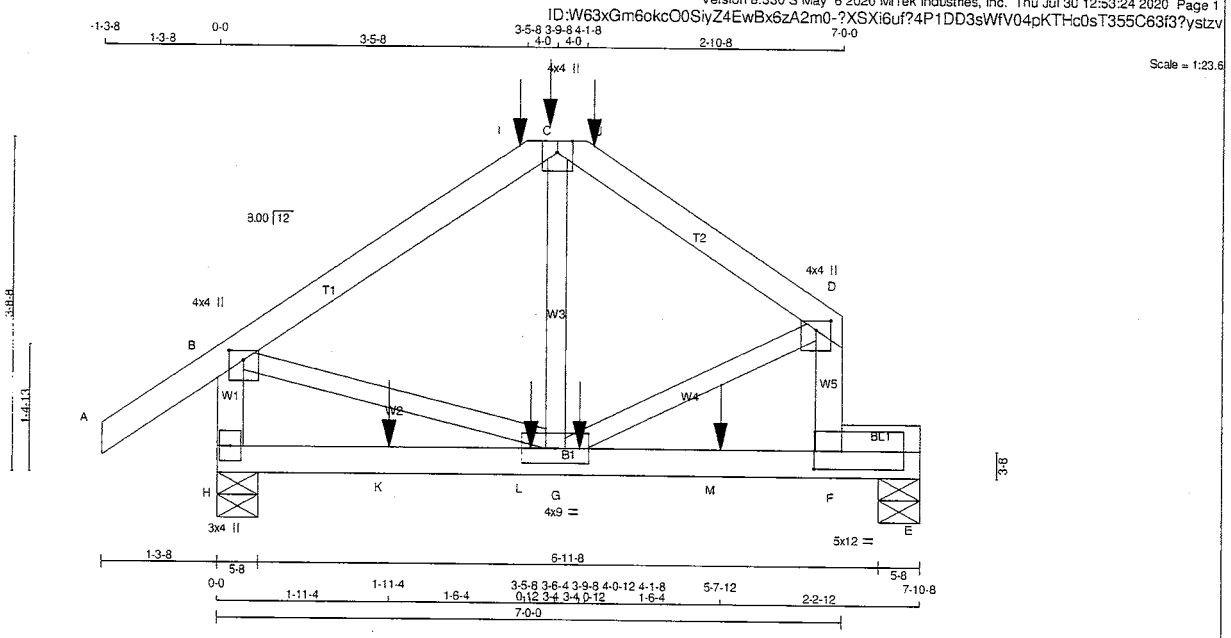
JSI GRIP= 0.46 (D) (INPUT = 0.90)
JSI METAL= 0.13 (D) (INPUT = 1.00)

Structural component only
DWG# T-2017053

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410046	T16	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:53:24 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-?XSXi6uf?4P1DD3sWfV04pKTHc0sT355C63f3?ystzv



TOTAL WEIGHT = 32 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMTMW+p	MT20	4.0	4.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMVKm	MT20	5.0	12.0	2.25	3.75
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2017054

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	849	0	849	0	0	5-8	5-8
E	630	0	630	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	596	414 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0
E	445	294 / 0	0 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI
FR-TO					(LC)	FR-TO			(LC)
A- B	0 / 35	-91.8	-91.8	0.14 (1)	10.00	B- G	0 / 579		0.14 (1)
B- I	-629 / 0	-91.8	-91.8	0.31 (1)	6.25	G- D	0 / 616		0.15 (1)
I- C	-629 / 0	-91.8	-91.8	0.31 (1)	6.25	G- C	-54 / 84		0.03 (4)
C- J	-629 / 0	-91.8	-91.8	0.23 (1)	6.25				
J- D	-629 / 0	-91.8	-91.8	0.23 (1)	6.25				
H- B	-842 / 0	0.0	0.0	0.10 (1)	7.81				
F- D	-802 / 0	0.0	0.0	0.10 (1)	7.81				
H- K	0 / 0	-18.5	-18.5	0.22 (1)	10.00				
K- L	0 / 0	-18.5	-18.5	0.22 (1)	10.00				
L- G	0 / 0	-18.5	-18.5	0.22 (1)	10.00				
G- M	0 / 0	-18.5	-18.5	0.81 (1)	10.00				
M- F	0 / 0	-18.5	-18.5	0.81 (1)	10.00				
F- E	0 / 0	-18.5	-18.5	0.81 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-9-8	-145	-145	---	BACK	VERT	TOTAL	---	C1
G	4-0-12	-13	-13	---	BACK	VERT	TOTAL	---	C1
I	3-5-8	-183	-183	---	BACK	VERT	TOTAL	---	C1
J	4-1-8	-183	-183	---	BACK	VERT	TOTAL	---	C1
K	1-11-4	-13	-13	---	BACK	VERT	TOTAL	---	C1
L	3-6-4	-13	-13	---	BACK	VERT	TOTAL	---	C1
M	5-7-12	-13	-13	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 623 (0.15")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.81/1.00 (F-G:1) , WB=0.15/1.00 (D-G:1) , SSI=0.54/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

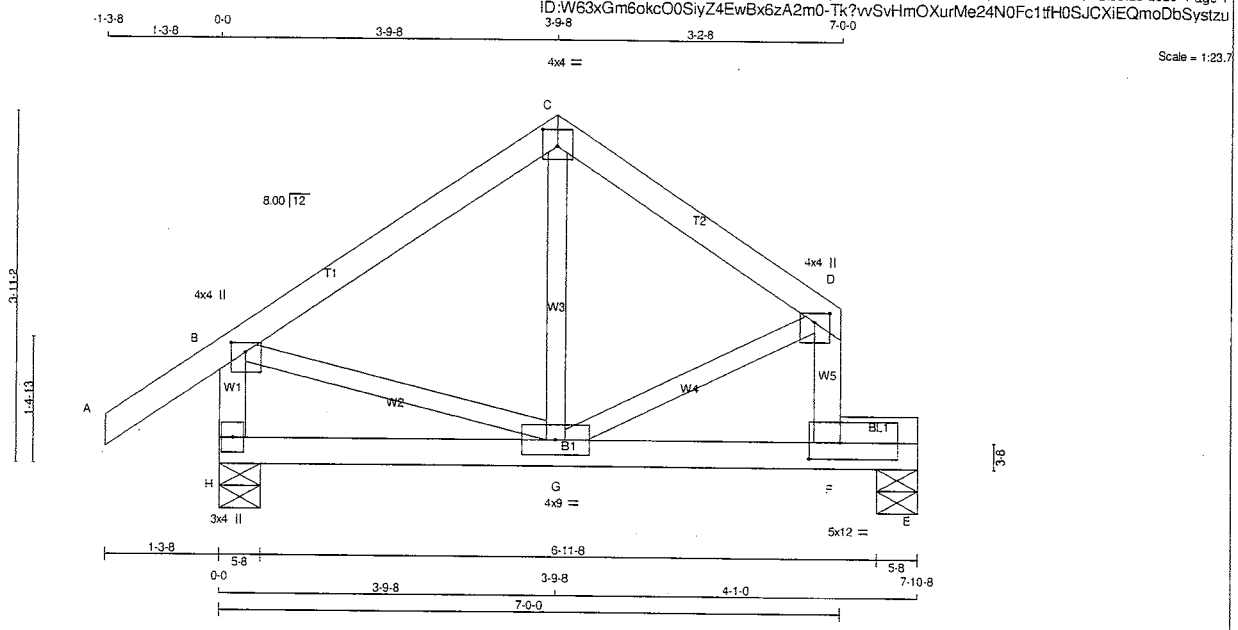
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (B) (INPUT = 0.90)
JSI METAL= 0.19 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410046	T17	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:53:25 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-Tk?vvSvHmOXurMe24N0Fc1tfH0SJCXiEQmoDbSystzu



TOTAL WEIGHT = 32 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMVKm	MT20	5.0	12.0	2.25	4.25
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	556	0	556	0	0	5-8	5-8
E	358	0	358	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	391	269 / 0	0 / 0	0 / 0	0 / 0	121 / 0	0 / 0
E	255	159 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO					(LC)		FR-TO		
A- B	0 / 35	-91.8	-91.8	0.12 (1)		10.00	G- C	0 / 68	0.02 (4)
B- C	-309 / 0	-91.8	-91.8	0.23 (1)		6.25	B- G	0 / 267	0.06 (1)
C- D	-310 / 0	-91.8	-91.8	0.16 (1)		6.25	G- D	0 / 286	0.06 (1)
H- B	-543 / 0	0.0	0.0	0.06 (1)		7.81			
F- D	-442 / 0	0.0	0.0	0.05 (1)		7.81			
H- G	0 / 0	-18.5	-18.5	0.12 (1)		10.00			
G- F	0 / 0	-18.5	-18.5	0.42 (1)		10.00			
F- E	0 / 0	-18.5	-18.5	0.42 (1)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.42/1.00 (F-G:1) , WB=0.06/1.00 (D-G:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

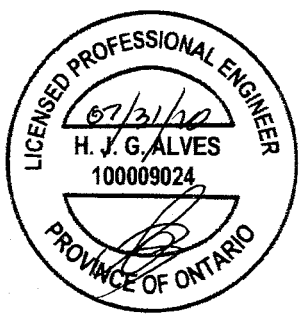
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.37 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)



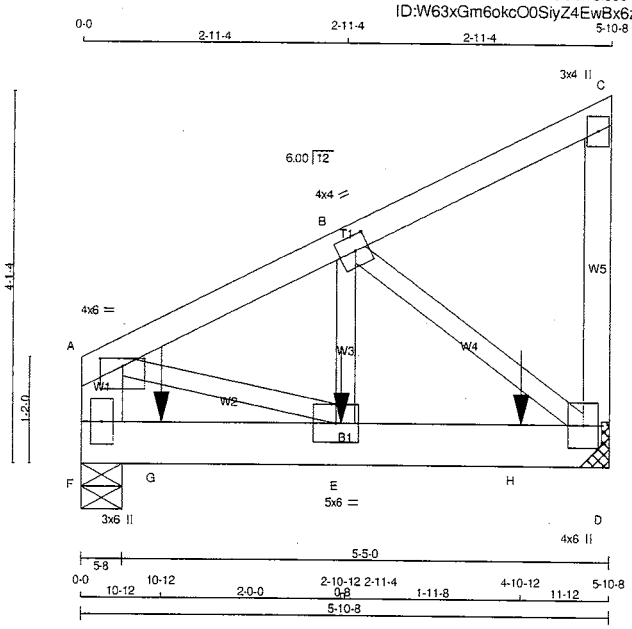
Structural component only
DWG# T-2017055

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410046	T18	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:53:26 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-xwZH7owvXiflTWDFe4XU9EPtfQrJxzJOfQYm7uystzt

Scale = 1:23.4



TOTAL WEIGHT = 2 X 29 = 58 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	6	SIDE(13.4)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		

LICENSED PROFESSIONAL ENGINEER
07/31/20
H.J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017056 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1465	0	1465	0	0	5-8	5-8
D	1443	0	1443	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1034	693 / 0	0 / 0	0 / 0	0 / 0	341 / 0	0 / 0
D	1018	683 / 0	0 / 0	0 / 0	0 / 0	336 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
F- A	-973 / 0	0.0	0.0	0.03 (1)	7.81		A- E	0 / 1169	0.14 (1)
A- B	-1253 / 0	-91.8	-91.8	0.06 (1)	6.25		E- B	0 / 1110	0.14 (1)
B- C	-11 / 0	-91.8	-91.8	0.05 (1)	6.25		B- D	-1422 / 0	0.17 (1)
D- C	-110 / 0	0.0	0.0	0.01 (1)	7.81				
F- G	0 / 0	-18.5	-18.5	0.16 (1)	10.00				
G- E	0 / 0	-18.5	-18.5	0.16 (1)	10.00				
E- H	0 / 1131	-18.5	-18.5	0.24 (1)	10.00				
H- D	0 / 1131	-18.5	-18.5	0.24 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-10-12	-753	-753	---	FRONT	VERT	TOTAL	---	C1
G	10-12	-754	-754	---	FRONT	VERT	TOTAL	---	C1
H	4-10-12	-753	-753	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.24/1.00 (D-E:1) , WB=0.17/1.00 (B-D:1) , SSI=0.16/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (B) (INPUT = 0.90)
JSI METAL= 0.25 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410046	T18	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-xwZH7owvXifiTWDFe4XU9EPtfQrJxzJOfQYm7uystzt

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

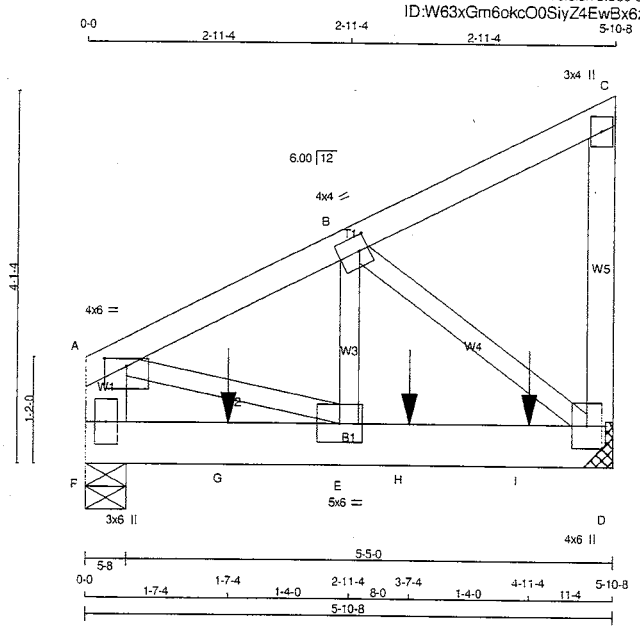
LICENSED PROFESSIONAL ENGINEER
07/31/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017056

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410046	T18Z	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:53:27 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-P67fK8xXI?nc4goRCn2jhSy1Jq8FgPLXu4HJfKystzs



TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2017057

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1701	0	1701	0	0	5-8	5-8
D	2192	0	2192	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1197	815 / 0	0 / 0	0 / 0	0 / 0	382 / 0	0 / 0
D	1543	1052 / 0	0 / 0	0 / 0	0 / 0	491 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1373 / 0	0.0	0.0	0.05 (1)	7.81	A- E	0 / 1704	0.21 (1)
A- B	-1833 / 0	-91.8	-91.8	0.07 (1)	6.25	E- B	0 / 1772	0.22 (1)
B- C	-9 / 0	-91.8	-91.8	0.05 (1)	10.00	B- D	-2073 / 0	0.25 (1)
D- C	-114 / 0	0.0	0.0	0.01 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.21 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.21 (1)	10.00			
E- H	0 / 1648	-18.5	-18.5	0.39 (1)	10.00			
H- I	0 / 1648	-18.5	-18.5	0.39 (1)	10.00			
I- D	0 / 1648	-18.5	-18.5	0.39 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-7-4	-1081	-1081	---	BACK	VERT	TOTAL	---	C1
H	3-7-4	-1081	-1081	---	BACK	VERT	TOTAL	---	C1
I	4-11-4	-1082	-1082	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.07/1.00 (A-B:1) , BC=0.39/1.00 (D-E:1) , WB=0.25/1.00 (B-D:1) , SSI=0.46/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

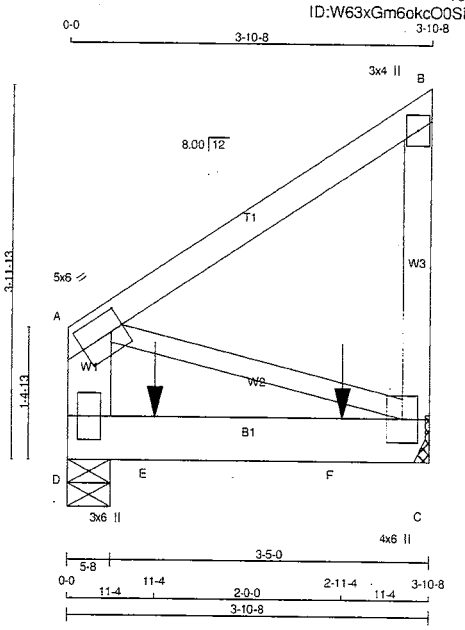
JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.37 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410046	T19	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:53:28 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-tlh1YUx93JvTiqNdlVZyEfUB4EVMPvSg6k1tCnystzr

Scale = 1:22.6



TOTAL WEIGHT = 2 X 20 = 41 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x6	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	2	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	1.75
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2017058

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	1144	0	1144	0	0	5-8	5-8
C	969	0	969	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	804	554 / 0	0 / 0	0 / 0	0 / 0	250 / 0	0 / 0
C	681	471 / 0	0 / 0	0 / 0	0 / 0	210 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
D- A	-178 / 0	0.0	0.0	0.01 (1)	7.81	A- C	0 / 0	0.00 (1)
A- B	0 / 0	-91.8	-91.8	0.13 (1)	10.00			
C- B	-178 / 0	0.0	0.0	0.02 (1)	7.81			
D- E	0 / 0	-18.5	-18.5	0.33 (1)	10.00			
E- F	0 / 0	-18.5	-18.5	0.33 (1)	10.00			
F- C	0 / 0	-18.5	-18.5	0.33 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	11-4	-1012	-1012	---	BACK	VERT	TOTAL	---	C1
F	2-11-4	-673	-673	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.33/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.32/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

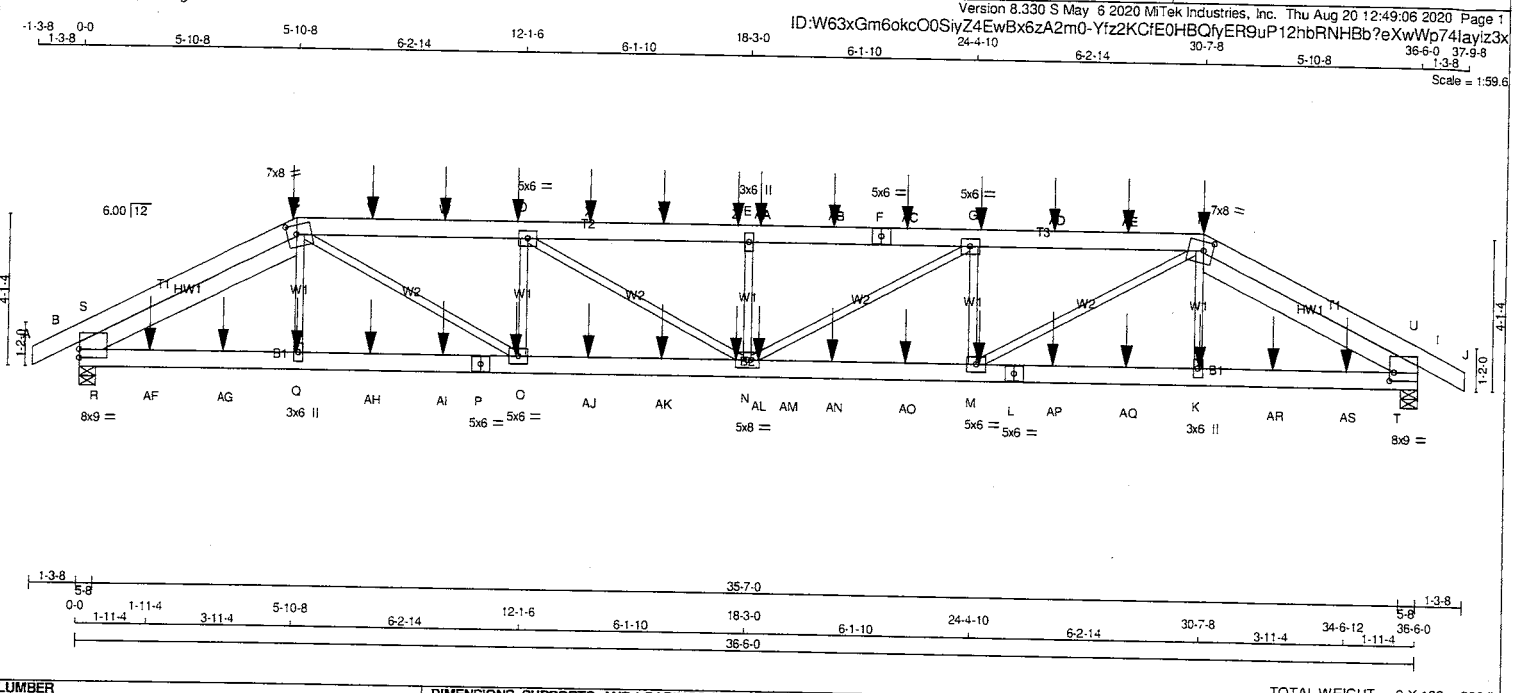
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.04 (B) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410047	T30	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Aug 20 12:49:06 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-Yfz2KCfE0HBQfyER9uP12hbRNHBb?eXwWp74layIz3x



TOTAL WEIGHT = 2 X 198 = 396 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
B - P	2x6	DRY	1650F 1.5E	SPF
P - L	2x6	DRY	1650F 1.5E	SPF
L - I	2x6	DRY	1650F 1.5E	SPF
REINFORCING MEMBERS				
HW2	2x6	DRY	No.2	SPF
HW1	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- F	2	12	SIDE(183.1)
F- H	2	12	SIDE(183.1)
H- J	2	12	SIDE(122.0)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- P	2	12	SIDE(183.1)
P- L	2	12	SIDE(183.1)
L- I	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

Structural component only
DWG# T-2018775 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	3448	0	3448	0	0	5-8	5-8
I	3448	0	3448	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	2439	1595 / 0	0 / 0	0 / 0	0 / 0	845 0	0 / 0
I	2439	1595 / 0	0 / 0	0 / 0	0 / 0	845 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 0	-91.8	-91.8	0.03 (1)	10.00
B- S	-3838 / 0	-91.8	-91.8	0.15 (1)	5.61
S- C	-2355 / 0	-91.8	-91.8	0.15 (1)	6.25
C- V	-7812 / 0	-91.8	-91.8	0.30 (1)	4.10
V- W	-7812 / 0	-91.8	-91.8	0.30 (1)	4.10
W- D	-7812 / 0	-91.8	-91.8	0.30 (1)	4.10
D- X	-8765 / 0	-91.8	-91.8	0.32 (1)	3.89
X- Y	-8765 / 0	-91.8	-91.8	0.32 (1)	3.89
Y- Z	-8765 / 0	-91.8	-91.8	0.32 (1)	3.89
Z- E	-8765 / 0	-91.8	-91.8	0.32 (1)	3.89
E-AA	-8765 / 0	-91.8	-91.8	0.32 (1)	3.89
AA-AB	-8765 / 0	-91.8	-91.8	0.32 (1)	3.89
AB- F	-8765 / 0	-91.8	-91.8	0.32 (1)	3.89
F-AC	-8765 / 0	-91.8	-91.8	0.32 (1)	3.89
AC- G	-8765 / 0	-91.8	-91.8	0.32 (1)	3.89
G-AD	-7812 / 0	-91.8	-91.8	0.30 (1)	4.10
AD-AE	-7812 / 0	-91.8	-91.8	0.30 (1)	4.10
AE- H	-7812 / 0	-91.8	-91.8	0.30 (1)	4.10
H- U	-2355 / 0	-91.8	-91.8	0.15 (1)	6.25
U- I	-3838 / 0	-91.8	-91.8	0.15 (1)	5.61
I- J	0 / 0	-91.8	-91.8	0.03 (1)	10.00
B- R	0 / 2077	-18.5	-18.5	0.09 (1)	10.00
R-AF	0 / 4836	-18.5	-18.5	0.23 (1)	10.00
AF-AG	0 / 4836	-18.5	-18.5	0.23 (1)	10.00
AG- Q	0 / 4836	-18.5	-18.5	0.23 (1)	10.00
Q-AH	0 / 4843	-18.5	-18.5	0.23 (1)	10.00
AH-AI	0 / 4843	-18.5	-18.5	0.23 (1)	10.00
AI- P	0 / 4843	-18.5	-18.5	0.23 (1)	10.00
P- O	0 / 4843	-18.5	-18.5	0.23 (1)	10.00
O-AJ	0 / 7812	-18.5	-18.5	0.37 (1)	10.00
AJ-AK	0 / 7812	-18.5	-18.5	0.37 (1)	10.00
AK-AL	0 / 7812	-18.5	-18.5	0.37 (1)	10.00
AL- N	0 / 7812	-18.5	-18.5	0.37 (1)	10.00
N-AM	0 / 7812	-18.5	-18.5	0.37 (1)	10.00
AM-AN	0 / 7812	-18.5	-18.5	0.37 (1)	10.00
AN-AO	0 / 7812	-18.5	-18.5	0.37 (1)	10.00
AO- M	0 / 7812	-18.5	-18.5	0.37 (1)	10.00
M- L	0 / 4843	-18.5	-18.5	0.23 (1)	10.00
L-AP	0 / 4843	-18.5	-18.5	0.23 (1)	10.00
AP-AQ	0 / 4843	-18.5	-18.5	0.23 (1)	10.00
AQ- K	0 / 4843	-18.5	-18.5	0.23 (1)	10.00
K-AR	0 / 4836	-18.5	-18.5	0.23 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	0 / 238	0.04 (4)
C- O	0 / 3436	0.43 (1)
O- D	-1560 / 0	0.19 (1)
D- N	0 / 1108	0.14 (1)
N- E	-920 / 0	0.11 (1)
N- G	0 / 1108	0.14 (1)
M- G	-1560 / 0	0.19 (1)
M- H	0 / 3436	0.43 (1)
K- H	0 / 238	0.04 (4)
C- R	-3325 / 0	0.67 (1)
H- T	-3325 / 0	0.67 (1)
R- S	0 / 1866	0.00 (1)
T- U	0 / 1866	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CSI: TC=0.32/1.00 (D-E:1) , BC=0.37/1.00 (N-O:1) , WB=0.67/1.00 (C-R:1) , SSI=0.15/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (C) (INPUT = 0.90)
JSI METAL= 0.52 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410047	T30	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	8.0	9.0	2.75	
C	TTWWW-m	MT20	7.0	8.0	3.00	3.00
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWWW-m	MT20	7.0	8.0	3.00	3.00
I	TMBMW1-I	MT20	8.0	9.0	2.75	1.50
K	BMW+w	MT20	3.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMW+w	MT20	3.0	6.0		

LOADING

TOTAL LOAD CASES: (4)

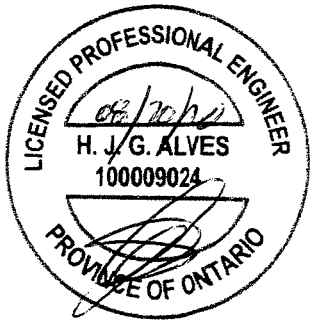
CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. MEMB. UNBRAC LENGTH	MAX. FACTORED FORCE (LBS) FR-TO	MAX CSI (LC)
AR-AS	0 / 4836	-18.5	-18.5	0.23 (1)	10.00		
AS- T	0 / 4836	-18.5	-18.5	0.23 (1)	10.00		
T- I	0 / 2077	-18.5	-18.5	0.09 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-409	-409	---	FRONT	VERT	TOTAL	---	C1
D	11-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
G	24-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
H	30-7-8	-409	-409	---	FRONT	VERT	TOTAL	---	C1
K	30-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
M	24-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	11-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Q	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
V	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	15-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	17-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	18-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	20-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	22-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	26-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AE	28-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AF	1-11-4	-25	-26	---	FRONT	VERT	TOTAL	---	C1
AG	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AH	7-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AI	9-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AJ	13-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AK	15-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AL	17-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AM	18-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AN	20-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AO	22-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AP	26-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AQ	28-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AR	32-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AS	34-6-12	-25	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

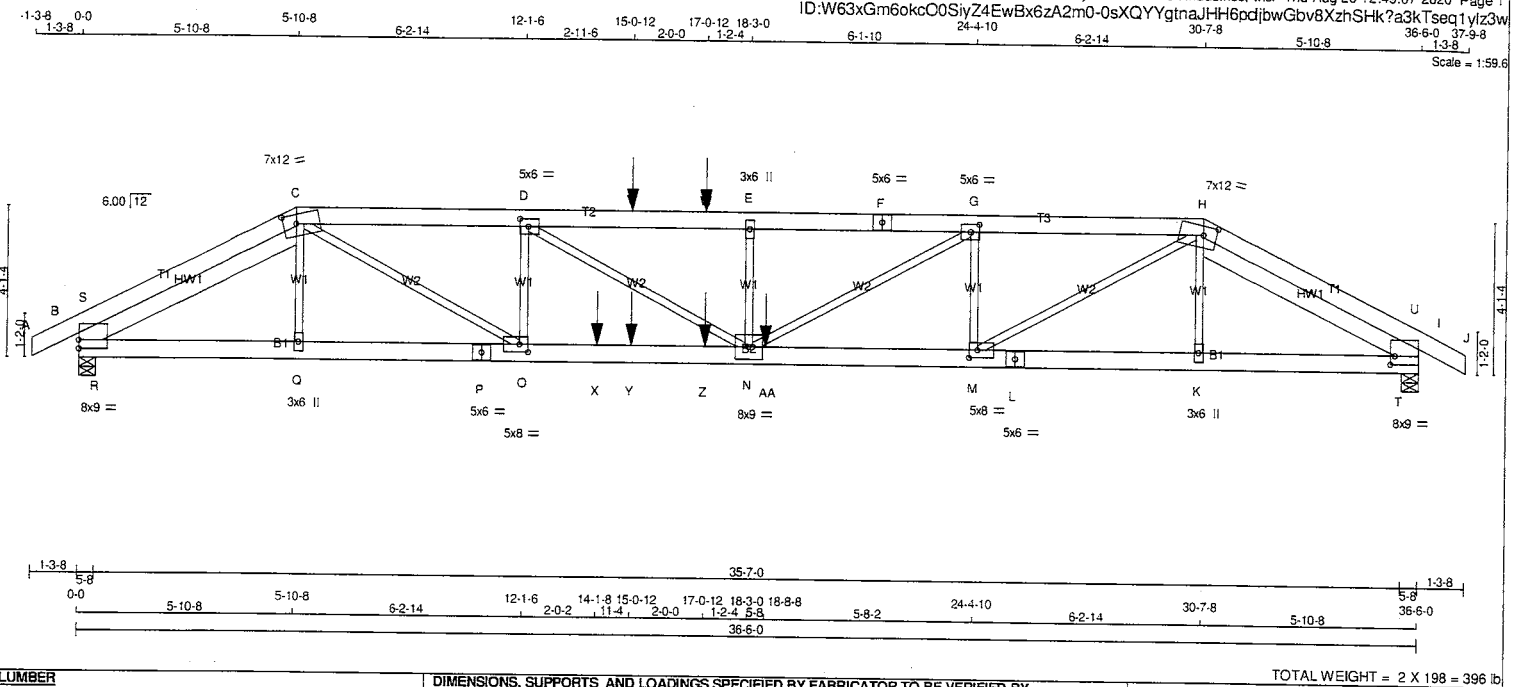


Structural component only
DWG# T-2018775 3/12

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410047	T30Z	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Aug 20 12:49:07 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-0sXQYYgtnaJHH6pdjbwGbv8XzhSHk?a3kTseq1ylz3w



TOTAL WEIGHT = 2 X 198 = 396 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
B - P	2x6	DRY	2100F 1.8E	SPF
P - L	2x6	DRY	2100F 1.8E	SPF
L - I	2x6	DRY	2100F 1.8E	SPF
REINFORCING MEMBERS				
HW2	2x6	DRY	No.2	SPF
HW1	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- F	2	12	SIDE(0.0)
F- H	2	12	TOP
H- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- P	2	12	TOP
P- L	2	12	SIDE(0.0)
L- I	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2018776 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	4354	0	4354	0	0	5-8	5-8
I	4012	0	4012	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3068	2075 / 0	0 / 0	0 / 0	0 / 0	993 0	0 / 0
I	2828	1910 / 0	0 / 0	0 / 0	0 / 0	919 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 0	-91.8	-91.8	0.03	(1)	10.00	Q- C	0 93	0.02 (4)
B- S	-4878 / 0	-91.8	-91.8	0.18	(1)	5.08	C- O	0 6644	0.82 (1)
S- C	-2936 / 0	-91.8	-91.8	0.17	(1)	6.19	O- D	-2055 / 0	0.25 (1)
C- D	-12050 / 0	-91.8	-91.8	0.40	(1)	3.32	D- N	0 / 2652	0.33 (1)
D- V	-14330 / 0	-91.8	-91.8	0.59	(1)	2.88	N- E	-637 / 0	0.08 (1)
V- W	-14330 / 0	-91.8	-91.8	0.59	(1)	2.88	N- G	0 / 4180	0.52 (1)
W- E	-14330 / 0	-91.8	-91.8	0.59	(1)	2.88	M- G	-2751 / 0	0.33 (1)
E- F	-14330 / 0	-91.8	-91.8	0.50	(1)	2.98	M- H	0 / 5709	0.71 (1)
F- G	-14330 / 0	-91.8	-91.8	0.50	(1)	2.98	K- H	0 / 183	0.03 (4)
G- H	-10736 / 0	-91.8	-91.8	0.35	(1)	3.55	C- R	-4477 / 0	0.90 (1)
H- U	-2671 / 0	-91.8	-91.8	0.16	(1)	6.25	H- T	-4147 0	0.83 (1)
U- I	-4379 / 0	-91.8	-91.8	0.17	(1)	5.31	R- S	0 2503	0.00 (1)
I- J	0 / 0	-91.8	-91.8	0.03	(1)	10.00	T- U	0 2182	0.00 (1)
B- R	0 / 2593	-18.5	-18.5	0.07	(1)	10.00			
R- Q	0 / 6308	-18.5	-18.5	0.18	(1)	10.00			
Q- P	0 / 6308	-18.5	-18.5	0.28	(1)	10.00			
P- O	0 / 6308	-18.5	-18.5	0.28	(1)	10.00			
O- X	0 / 12050	-18.5	-18.5	0.66	(1)	10.00			
X- Y	0 / 12050	-18.5	-18.5	0.66	(1)	10.00			
Y- Z	0 / 12050	-18.5	-18.5	0.66	(1)	10.00			
Z- N	0 / 12050	-18.5	-18.5	0.66	(1)	10.00			
N-AA	0 / 10736	-18.5	-18.5	0.42	(1)	10.00			
AA- M	0 / 10736	-18.5	-18.5	0.42	(1)	10.00			
M- L	0 / 5803	-18.5	-18.5	0.18	(1)	10.00			
L- K	0 / 5803	-18.5	-18.5	0.18	(1)	10.00			
K- T	0 5798	-18.5	-18.5	0.18	(1)	10.00			
T- I	0 2357	-18.5	-18.5	0.08	(1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
V	15-0-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
W	17-0-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
X	14-1-8	-1610	-1610	---	BACK	VERT	TOTAL	---	C1
Y	15-0-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
Z	17-0-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
AA	18-8-8	-2203	-2203	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.35")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 695 (0.63")

CSI: TC=0.59/1.00 (D-E:1) , BC=0.66/1.00 (N-O:1) , WB=0.90/1.00 (C-R:1) , SSI=0.73/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.73 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410047	T30Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-l	MT20	8.0	9.0	2.75	
C	TTWWW-m	MT20	7.0	12.0	3.00	4.25
D	TMWW-t	MT20	5.0	6.0	2.50	2.75
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.75
H	TTWWW-m	MT20	7.0	12.0	3.00	4.25
I	TMBMW1-l	MT20	8.0	9.0	2.75	1.50
K	BMW+w	MT20	3.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	8.0	2.50	2.75
N	BMWWW-t	MT20	8.0	9.0		
O	BMWW-t	MT20	5.0	8.0	2.50	2.75
P	BS-t	MT20	5.0	6.0		
Q	BMW+w	MT20	3.0	6.0		

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

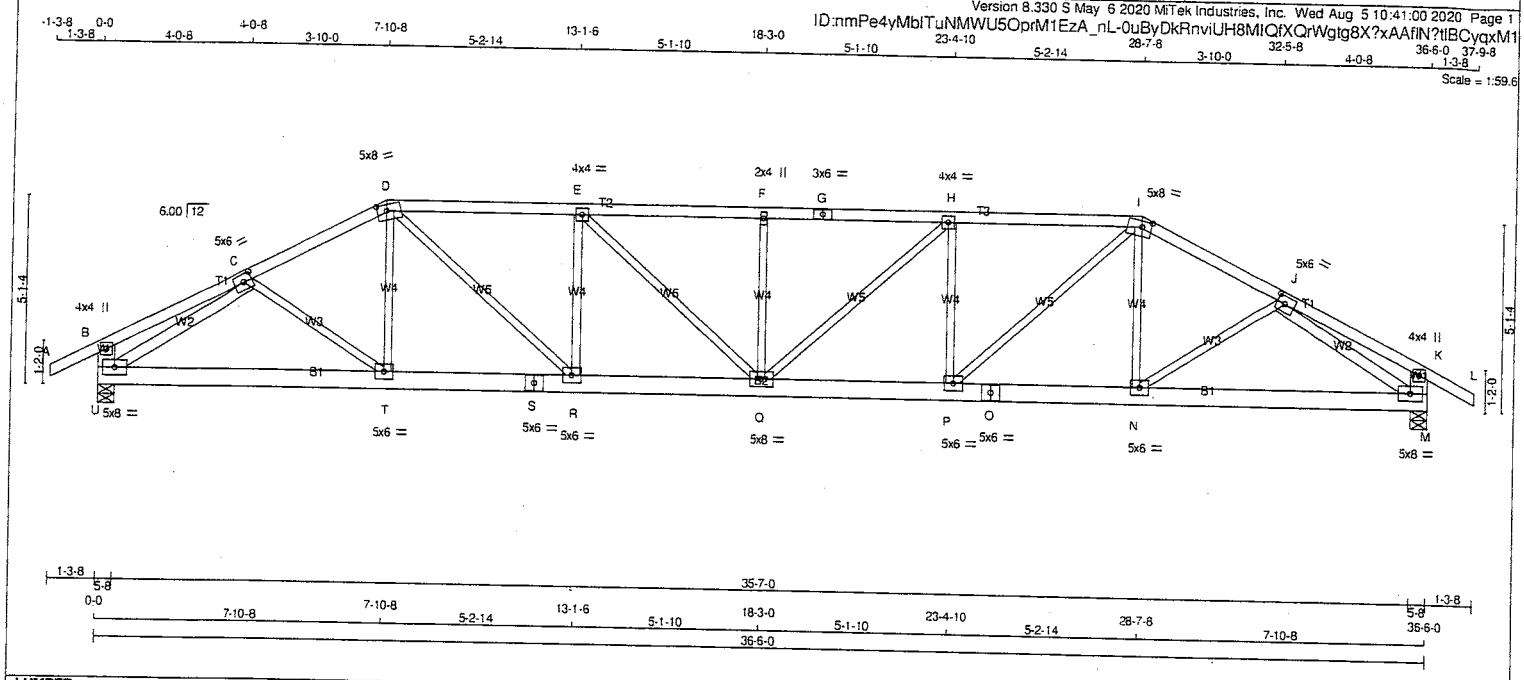
Structural component only
DWG# T-2018776

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T31	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:41:00 2020 Page 1

ID:nmPe4yMblTuNMWU5OprM1EzA_nL-0uByDkRnviUH8MIQfXQrWgtg8X?xAAfIN?tlBCyqxM1



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	8.0	2.00	3.25
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	8.0	2.00	3.25
J	TMWW-t	MT20	5.0	6.0	2.50	2.75
K	TMV+p	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	8.0		
N, P, R, T						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
U	BMVW1-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2137	0	2137	0	0	5-8	5-8
M	2137	0	2137	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 0
M	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 17	-91.8	-91.8	0.20 (1)	10.00
C- D	-2978 / 0	-91.8	-91.8	0.34 (1)	3.76
D- E	-3625 / 0	-91.8	-91.8	0.63 (1)	3.17
E- F	-3907 / 0	-91.8	-91.8	0.66 (1)	3.03
F- G	-3907 / 0	-91.8	-91.8	0.66 (1)	3.03
G- H	-3907 / 0	-91.8	-91.8	0.66 (1)	3.03
H- I	-3625 / 0	-91.8	-91.8	0.63 (1)	3.17
I- J	-2978 / 0	-91.8	-91.8	0.34 (1)	3.76
J- K	0 / 17	-91.8	-91.8	0.20 (1)	10.00
K- L	0 / 28	-91.8	-91.8	0.12 (1)	10.00
U- B	-269 / 0	0.0	0.0	0.02 (1)	7.81
M- K	-269 / 0	0.0	0.0	0.02 (1)	7.81
U- T	0 2592	-18.5	-18.5	0.38 (1)	10.00
T- S	0 / 2649	-18.5	-18.5	0.36 (1)	10.00
S- R	0 / 2649	-18.5	-18.5	0.36 (1)	10.00
R- Q	0 / 3626	-18.5	-18.5	0.48 (1)	10.00
Q- P	0 3626	-18.5	-18.5	0.48 (1)	10.00
P- O	0 2649	-18.5	-18.5	0.36 (1)	10.00
O- N	0 / 2649	-18.5	-18.5	0.36 (1)	10.00
N- M	0 / 2592	-18.5	-18.5	0.38 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	0 / 87	0.03 (4)
T- D	0 / 118	0.04 (4)
D- R	0 / 1314	0.30 (1)
R- E	-797 / 0	0.29 (1)
E- Q	0 / 383	0.09 (1)
Q- F	-433 / 0	0.16 (1)
Q- H	0 / 383	0.09 (1)
P- H	-797 0	0.29 (1)
P- I	0 1314	0.30 (1)
N- I	0 118	0.04 (4)
N- J	0 87	0.03 (4)
U- C	-3157 0	0.80 (1)
J- M	-3157 0	0.80 (1)

TOTAL WEIGHT = 2 X 171 = 343 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.40")

CSI: TC=0.66/1.00 (E-F:1) , BC=0.48/1.00 (Q-R:1) , WB=0.80/1.00 (J-M:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

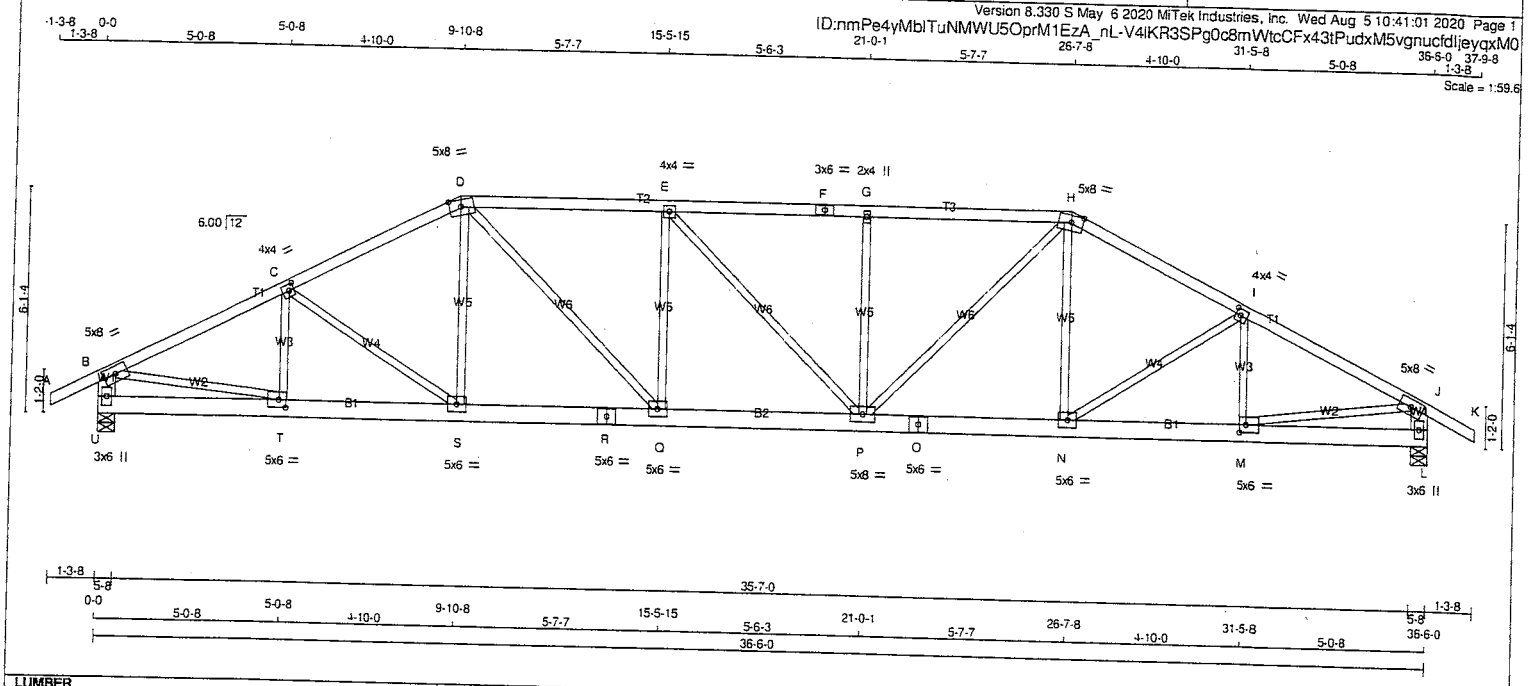
JSI GRIP= 0.86 (D) (INPUT = 0.90)
JSI METAL= 0.72 (C) (INPUT = 1.00)

Structural component only
DWG# T-2017369

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T32	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:41:01 2020 Page 1
ID:nmPe4yMblTuNMWU5OprM1EzA_nL-V4lKR3SPg0c8mWtcCFx43tPudxM5vgnucfdIjeyqxM0



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	2.25	3.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.25	3.75
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-t	MT20	5.0	8.0		
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.25
N, Q, S						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.25
U	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 2 X 173 = 345 lb [M][F]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
U	2137	0	2137	0	0	5-8	5-8
L	2137	0	2137	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0
L	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)		10.00
B- C	-3003 / 0	-91.8	-91.8	0.41 (1)		3.70
C- D	-2866 / 0	-91.8	-91.8	0.39 (1)		3.79
D- E	-3181 / 0	-91.8	-91.8	0.49 (1)		3.52
E- F	-3175 / 0	-91.8	-91.8	0.45 (1)		3.56
F- G	-3175 / 0	-91.8	-91.8	0.45 (1)		3.56
G- H	-3175 / 0	-91.8	-91.8	0.49 (1)		3.52
H- I	-2867 / 0	-91.8	-91.8	0.39 (1)		3.79
I- J	-3002 / 0	-91.8	-91.8	0.41 (1)		3.70
J- K	0 / 28	-91.8	-91.8	0.12 (1)		10.00
U- B	-2075 / 0	0.0	0.0	0.13 (1)		7.03
L- J	-2074 / 0	0.0	0.0	0.13 (1)		7.04
U- T	0 / 0	-18.5	-18.5	0.07 (1)		10.00
T- S	0 / 2706	-18.5	-18.5	0.38 (1)		10.00
S- R	0 / 2545	-18.5	-18.5	0.34 (1)		10.00
R- Q	0 / 2545	-18.5	-18.5	0.34 (1)		10.00
Q- P	0 / 3182	-18.5	-18.5	0.43 (1)		10.00
P- O	0 / 2547	-18.5	-18.5	0.35 (1)		10.00
O- N	0 / 2547	-18.5	-18.5	0.35 (1)		10.00
N- M	0 / 2705	-18.5	-18.5	0.38 (1)		10.00
M- L	0 / 0	-18.5	-18.5	0.07 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-366 / 0	0.08 (1)
C- S	-199 / 0	0.12 (1)
S- D	0 / 243	0.05 (1)
D- Q	0 / 908	0.20 (1)
Q- E	-550 / 0	0.30 (1)
E- P	-9 / 0	0.01 (1)
P- G	-552 / 0	0.30 (1)
P- H	0 / 897	0.20 (1)
N- H	0 / 250	0.06 (1)
N- I	-196 / 0	0.12 (1)
M- I	-368 / 0	0.08 (1)
B- T	0 / 2738	0.62 (1)
M- J	0 / 2737	0.62 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.49/1.00 (D-E:1) , BC=0.43/1.00 (P-Q:1) , WB=0.62/1.00 (B-T:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

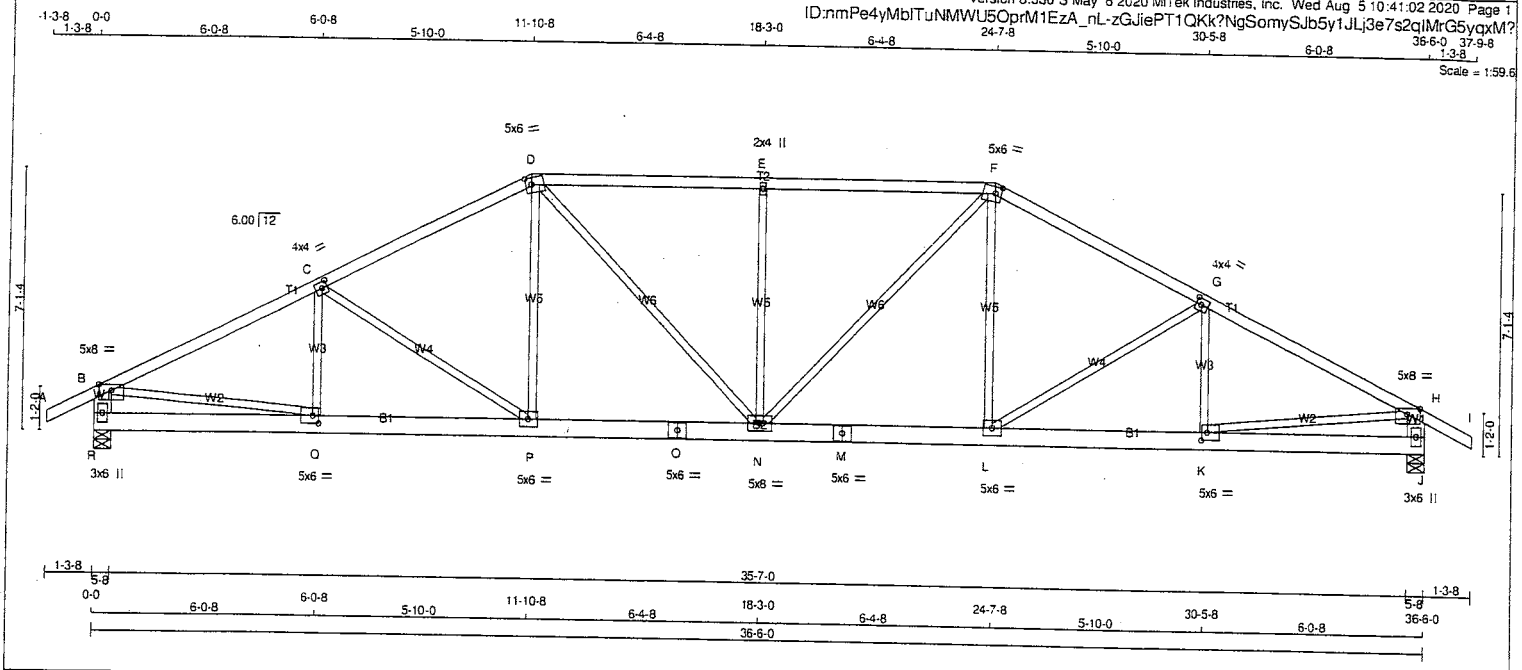
JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.62 (T) (INPUT = 1.00)

Structural component only
DWG# T-2017370

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T33	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:41:02 2020 Page 1
ID:nmPe4yMblTuNMWU5OprM1EzA_nL-zGJiePT1QKk?NgSomySJb5y1JLj3e7s2qIMrG5yqxM?



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.00
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BS-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.00
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2017371

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 2 X 171 = 343 lb

[M][F]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2137	0	2137	0	0	5-8	5-8
J	2137	0	2137	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 0	0 0
J	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 28	-91.8	-91.8	0.12 (1)	10.00
B-C	-3058 0	-91.8	-91.8	0.60 (1)	3.47
C-D	-2720 0	-91.8	-91.8	0.54 (1)	3.70
D-E	-2796 0	-91.8	-91.8	0.62 (1)	3.53
E-F	-2796 0	-91.8	-91.8	0.62 (1)	3.53
F-G	-2720 0	-91.8	-91.8	0.54 (1)	3.70
G-H	-3058 0	-91.8	-91.8	0.60 (1)	3.47
H-I	0 28	-91.8	-91.8	0.12 (1)	10.00
R-B	-2073 0	0.0	0.0	0.13 (1)	7.04
J-H	-2073 0	0.0	0.0	0.13 (1)	7.04
R-Q	0 0	-18.5	-18.5	0.08 (4)	10.00
Q-P	0 2760	-18.5	-18.5	0.38 (1)	10.00
P-O	0 2411	-18.5	-18.5	0.34 (1)	10.00
O-N	0 2411	-18.5	-18.5	0.34 (1)	10.00
N-M	0 2411	-18.5	-18.5	0.34 (1)	10.00
M-L	0 2411	-18.5	-18.5	0.34 (1)	10.00
L-K	0 2760	-18.5	-18.5	0.38 (1)	10.00
K-J	0 0	-18.5	-18.5	0.08 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-C	-271 6	0.07 (1)
C-P	-421 0	0.39 (1)
P-D	0 372	0.08 (1)
D-N	0 559	0.13 (1)
N-E	-718 0	0.59 (1)
N-F	0 559	0.13 (1)
L-F	0 372	0.08 (1)
L-G	-421 0	0.39 (1)
K-G	-271 6	0.07 (1)
B-Q	0 2783	0.63 (1)
K-H	0 2783	0.63 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.62/1.00 (D-E:1) , BC=0.38/1.00 (P-Q:1) , WB=0.63/1.00 (B-Q:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

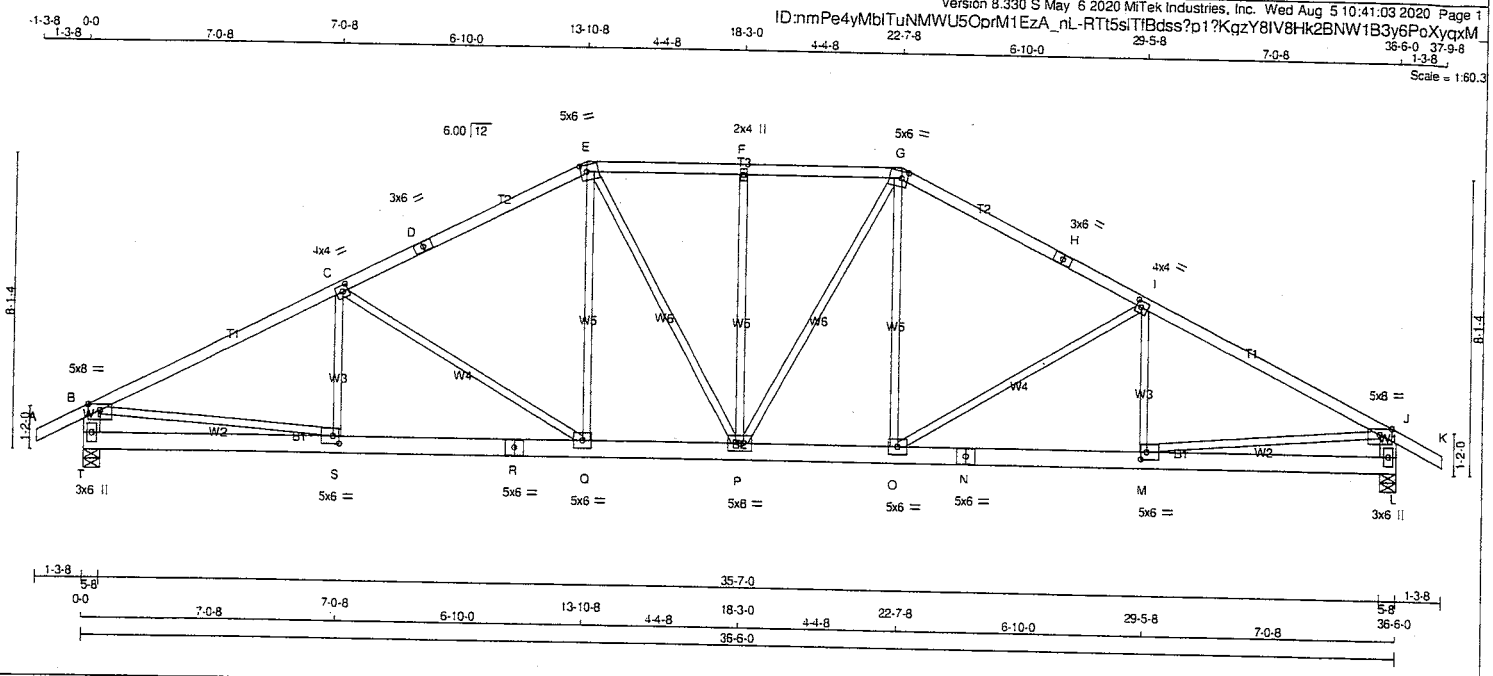
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.63 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T34	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:41:03 2020 Page 1
ID:nmPe4yMblTuNMWU5OprM1EzA_nL-RTt5slTfBdss?p1?KgzY8IV8Hk2BNW1B3y6PoXyqxM



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.00
N	BS-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.00
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2017372

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2137	0	2137	0	0	5-8	5-8
L	2137	0	2137	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0
L	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)		10.00
B- C	-3079 / 0	-91.8	-91.8	0.86 (1)		3.14
C- D	-2545 / 0	-91.8	-91.8	0.74 (1)		3.53
D- E	-2545 / 0	-91.8	-91.8	0.74 (1)		3.53
E- F	-2400 / 0	-91.8	-91.8	0.28 (1)		4.19
F- G	-2400 / 0	-91.8	-91.8	0.28 (1)		4.19
G- H	-2545 / 0	-91.8	-91.8	0.74 (1)		3.53
H- I	-2545 / 0	-91.8	-91.8	0.74 (1)		3.53
I- J	-3079 / 0	-91.8	-91.8	0.86 (1)		3.14
J- K	0 / 28	-91.8	-91.8	0.12 (1)		10.00
T- B	-2071 / 0	0.0	0.0	0.13 (1)		7.05
L- J	-2071 / 0	0.0	0.0	0.13 (1)		7.05
T- S	0 / 0	-18.5	-18.5	0.10 (4)		10.00
S- R	0 / 2784	-18.5	-18.5	0.39 (1)		10.00
R- Q	0 / 2784	-18.5	-18.5	0.39 (1)		10.00
Q- P	0 / 2251	-18.5	-18.5	0.31 (1)		10.00
P- O	0 / 2251	-18.5	-18.5	0.31 (1)		10.00
O- N	0 / 2784	-18.5	-18.5	0.39 (1)		10.00
N- M	0 / 2784	-18.5	-18.5	0.39 (1)		10.00
M- L	0 / 0	-18.5	-18.5	0.10 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-188 / 54	0.06 (1)
C- Q	-635 / 0	0.89 (1)
Q- E	0 / 467	0.11 (1)
E- P	0 / 302	0.07 (1)
P- F	-484 / 0	0.58 (1)
P- G	0 / 302	0.07 (1)
O- G	0 / 467	0.11 (1)
O- I	-635 / 0	0.89 (1)
M- I	-188 / 54	0.06 (1)
B- S	0 / 2801	0.63 (1)
M- J	0 / 2801	0.63 (1)

TOTAL WEIGHT = 2 X 178 = 356 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.86/1.00 (I-J:1) , BC=0.39/1.00 (M-O:1) , WB=0.89/1.00 (C-Q:1) , SSI=0.28/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

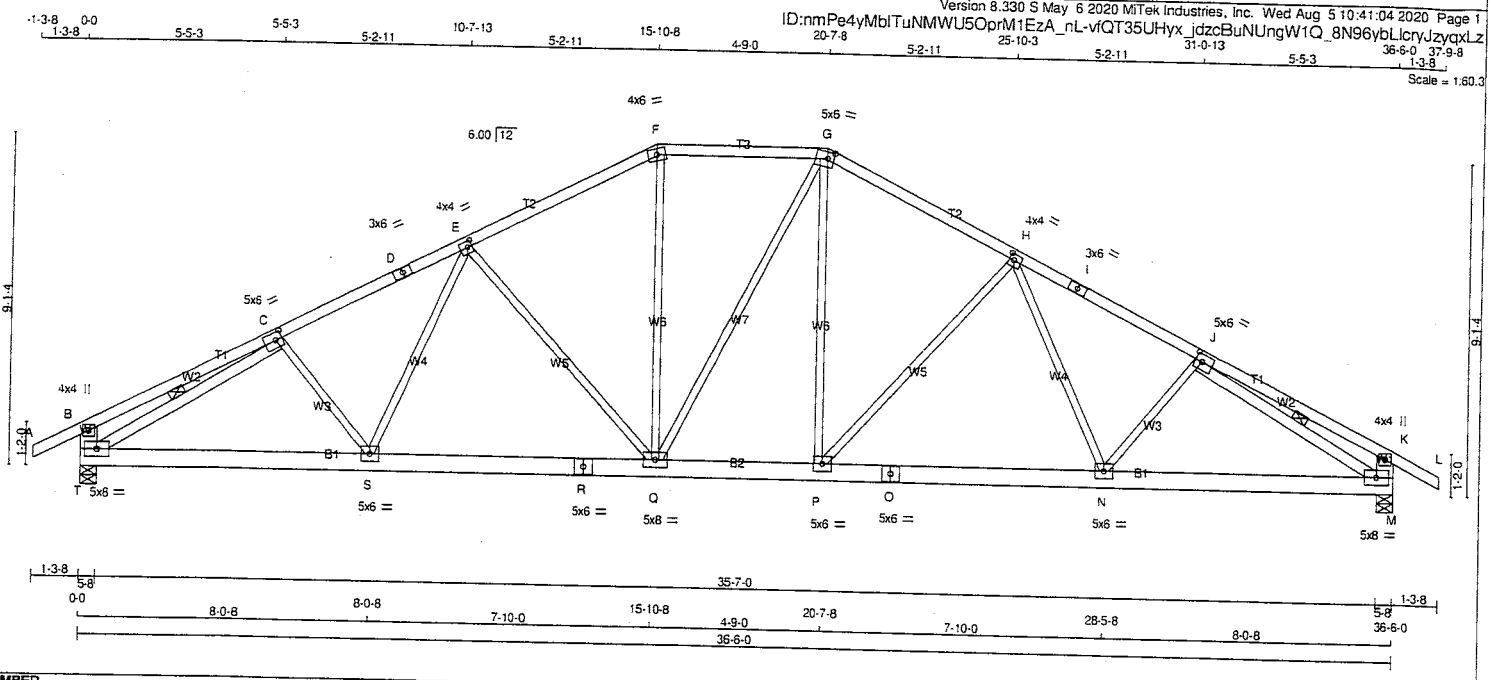
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.63 (S) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T35	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:41:04 2020 Page 1
ID:nmPe4yMbITuNMWU5OprM1EzA_nL-vfQT35UHyx_jdzcBuNUngW1Q_8N96ybLIcryJzyqxLz



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TTW-m	MT20	4.0	6.0		
G	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TS-t	MT20	3.0	6.0		
J	TMWW-t	MT20	5.0	6.0	2.50	2.25
K	TMV+p	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	8.0		
N, P, S						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
T	BMVW1-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 2 X 181 = 362 lb [M][F]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	2137	0	2137	0	0	IN-SX	IN-SX
M	2137	0	2137	0	0	5-8	5-8
						5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 0	0 0
M	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-T, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 20	-91.8	-91.8	0.32 (1)	10.00
C- D	-2933 / 0	-91.8	-91.8	0.42 (1)	3.75
D- E	-2933 / 0	-91.8	-91.8	0.42 (1)	3.75
E- F	-2339 / 0	-91.8	-91.8	0.38 (1)	4.16
F- G	-2079 / 0	-91.8	-91.8	0.32 (1)	4.40
G- H	-2335 / 0	-91.8	-91.8	0.38 (1)	4.16
H- I	-2935 0	-91.8	-91.8	0.42 (1)	3.75
I- J	-2935 / 0	-91.8	-91.8	0.42 (1)	3.75
J- K	0 / 20	-91.8	-91.8	0.32 (1)	10.00
K- L	0 / 28	-91.8	-91.8	0.12 (1)	10.00
T- B	-324 0	0.0	0.0	0.02 (1)	7.81
M- K	-324 / 0	0.0	0.0	0.02 (1)	7.81
T- S	0 / 2702	-18.5	-18.5	0.40 (1)	10.00
S- R	0 2517	-18.5	-18.5	0.35 (1)	10.00
R- Q	0 2517	-18.5	-18.5	0.35 (1)	10.00
Q- P	0 / 2076	-18.5	-18.5	0.30 (1)	10.00
P- O	0 2517	-18.5	-18.5	0.36 (1)	10.00
O- N	0 / 2517	-18.5	-18.5	0.36 (1)	10.00
N- M	0 2704	-18.5	-18.5	0.40 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- S	-126 32	0.04 (1)
S- E	0 272	0.06 (1)
E- Q	-678 / 0	0.93 (1)
Q- F	0 629	0.14 (1)
Q- G	0 / 6	0.00 (1)
P- G	0 622	0.14 (1)
P- H	-681 0	0.93 (1)
H- N	0 / 277	0.06 (1)
N- J	-125 / 32	0.04 (1)
T- C	-3217 0	0.61 (1)
J- M	-3218 0	0.61 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.42/1.00 (H-J:1) , BC=0.40/1.00 (S-T:1) , WB=0.93/1.00 (H-P:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.72 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

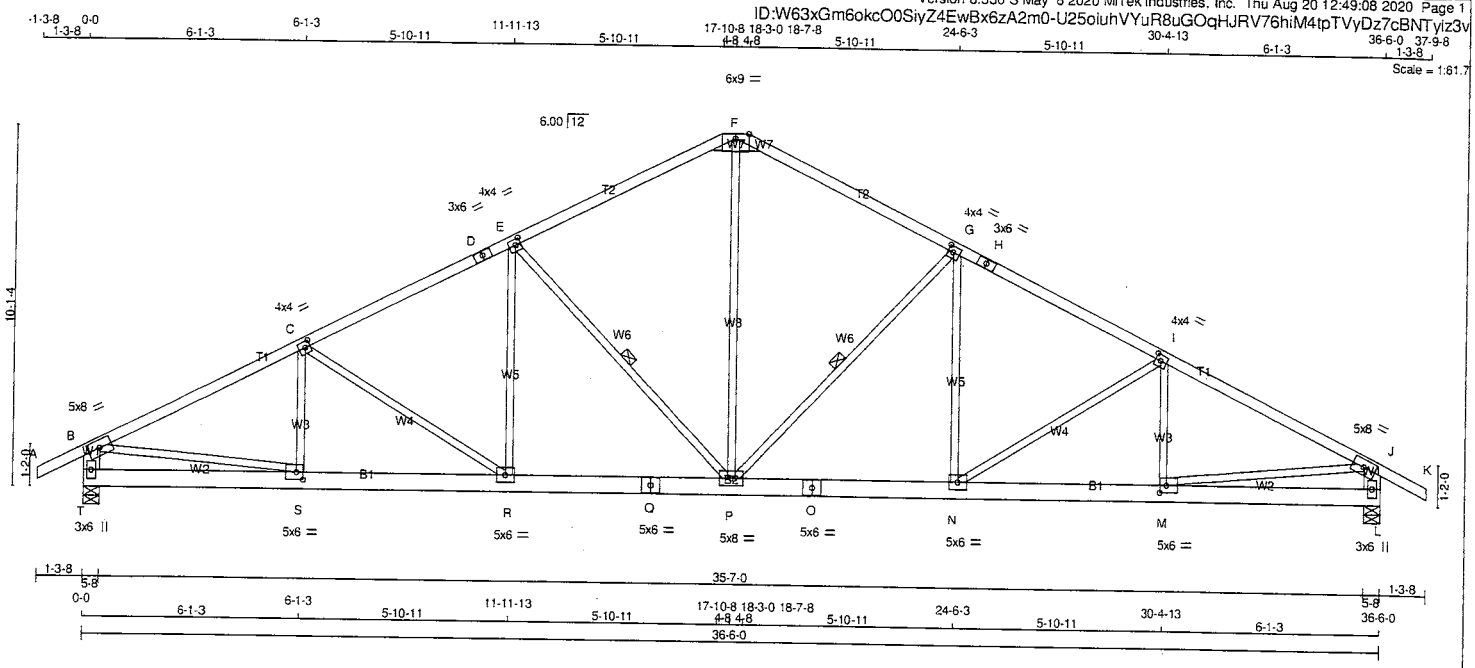
Structural component only
DWG# T-2017373

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410047	T36	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Aug 20 12:49:08 2020 Page 1

ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-U25oiuhVYuR8uGOqHJRV76hiM4tpTVyDz7cBNTyiz3v



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C, E, G, I						
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
F	TMTMWWW-t	MT20	6.0	9.0	1.50	4.50
H	TS-t	MT20	3.0	6.0		
J	TMVW-t	MT20	5.0	8.0		
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.25
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.25
T	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2137	0	2137	0	0	5-8	5-8
L	2137	0	2137	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0
L	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)		10.00
B- C	-3045 / 0	-91.8	-91.8	0.61 (1)		3.49
C- D	-2734 / 0	-91.8	-91.8	0.52 (1)		3.76
D- E	-2734 / 0	-91.8	-91.8	0.52 (1)		3.76
E- F	-2106 / 0	-91.8	-91.8	0.52 (1)		4.16
F- G	-2106 / 0	-91.8	-91.8	0.52 (1)		4.16
G- H	-2734 / 0	-91.8	-91.8	0.52 (1)		3.76
H- I	-2734 / 0	-91.8	-91.8	0.52 (1)		3.76
I- J	-3045 / 0	-91.8	-91.8	0.61 (1)		3.49
J- K	0 / 28	-91.8	-91.8	0.12 (1)		10.00
T- B	-2075 / 0	0.0	0.0	0.13 (1)		7.03
L- J	-2075 / 0	0.0	0.0	0.13 (1)		7.03
T- S	0 / 0	-18.5	-18.5	0.08 (4)		10.00
S- R	0 / 2744	-18.5	-18.5	0.38 (1)		10.00
R- Q	0 / 2447	-18.5	-18.5	0.34 (1)		10.00
Q- P	0 / 2447	-18.5	-18.5	0.34 (1)		10.00
P- O	0 / 2447	-18.5	-18.5	0.34 (1)		10.00
O- N	0 / 2447	-18.5	-18.5	0.34 (1)		10.00
N- M	0 / 2744	-18.5	-18.5	0.38 (1)		10.00
M- L	0 / 0	-18.5	-18.5	0.08 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-253 / 11	0.06 (1)
C- R	-353 / 0	0.33 (1)
R- E	0 / 308	0.07 (1)
E- P	-862 / 0	0.50 (1)
P- G	-862 / 0	0.50 (1)
N- G	0 / 308	0.07 (1)
N- I	-353 / 0	0.33 (1)
M- I	-253 / 11	0.06 (1)
B- S	0 / 2766	0.62 (1)
M- J	0 / 2766	0.62 (1)
P- F	0 / 1395	0.31 (1)

TOTAL WEIGHT = 2 X 177 = 354 lb

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.61/1.00 (I-J:1) , BC=0.38/1.00 (R-S:1) , WB=0.62/1.00 (B-S:1) , SSI=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

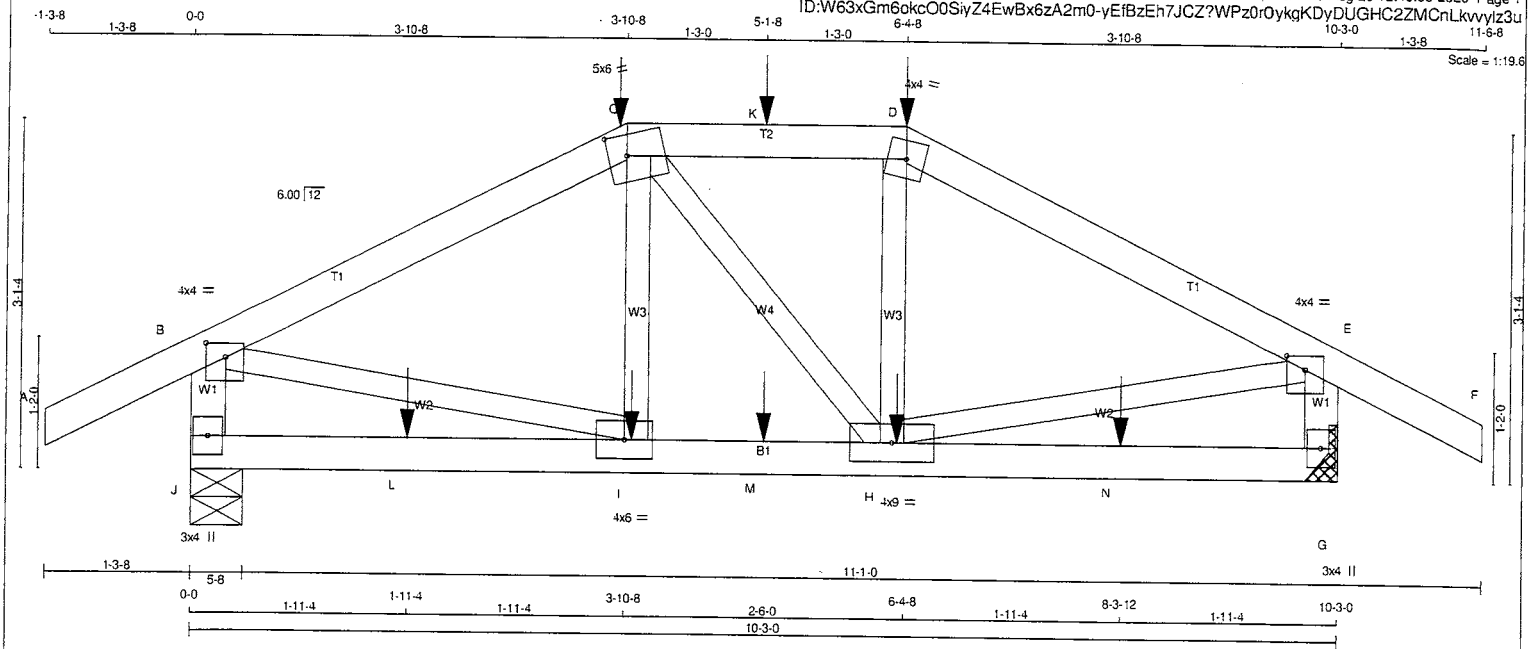
JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.62 (S) (INPUT = 1.00)

Structural component only
DWG# T-2018777

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410047	T37	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Aug 20 12:49:09 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-yEfBzEh7JCZ?WPz0rOykqKDyDUGHC2ZMCnLkvvylz3u



TOTAL WEIGHT = 42 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.50	2.00
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMVW-p	MT20	4.0	4.0	1.50	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	949	0	949	0	0	5-8	5-8
G	948	0	948	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	668	455 / 0	0 / 0	0 / 0	0 / 0	213 / 0	0 / 0
G	668	455 / 0	0 / 0	0 / 0	0 / 0	213 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.13 (1)	10.00
B- C	-950 / 0	-91.8	-91.8	0.28 (1)	5.96
C- K	-846 / 0	-91.8	-91.8	0.16 (1)	6.25
K- D	-846 / 0	-91.8	-91.8	0.16 (1)	6.25
D- E	-953 / 0	-91.8	-91.8	0.28 (1)	5.96
E- F	0 / 28	-91.8	-91.8	0.13 (1)	10.00
J- B	-912 / 0	0.0	0.0	0.10 (1)	7.81
G- E	-911 / 0	0.0	0.0	0.10 (1)	7.81
J- L	0 / 0	-18.5	-18.5	0.09 (4)	10.00
L- I	0 / 0	-18.5	-18.5	0.09 (4)	10.00
I- M	0 / 845	-18.5	-18.5	0.17 (1)	10.00
M- H	0 / 845	-18.5	-18.5	0.17 (1)	10.00
H- N	0 / 0	-18.5	-18.5	0.10 (4)	10.00
N- G	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-100 / 37	0.02 (1)
C- H	0 / 2	0.00 (4)
H- D	-97 / 39	0.02 (1)
B- I	0 / 868	0.21 (1)
H- E	0 / 871	0.22 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-212	-212	---	FRONT	VERT	TOTAL	---	C1
D	6-4-8	-212	-212	---	FRONT	VERT	TOTAL	---	C1
H	6-3-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1
I	3-11-4	-12	-12	---	FRONT	VERT	TOTAL	---	C1
K	5-1-8	-42	-42	---	FRONT	VERT	TOTAL	---	C1
L	1-11-4	-8	-10	---	FRONT	VERT	TOTAL	---	C1
M	5-1-8	-12	-12	---	FRONT	VERT	TOTAL	---	C1
N	8-3-12	-8	-10	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.28/1.00 (D-E:1) , BC=0.17/1.00 (H-I:1) , WB=0.22/1.00 (E-H:1) , SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.30 (E) (INPUT = 1.00)

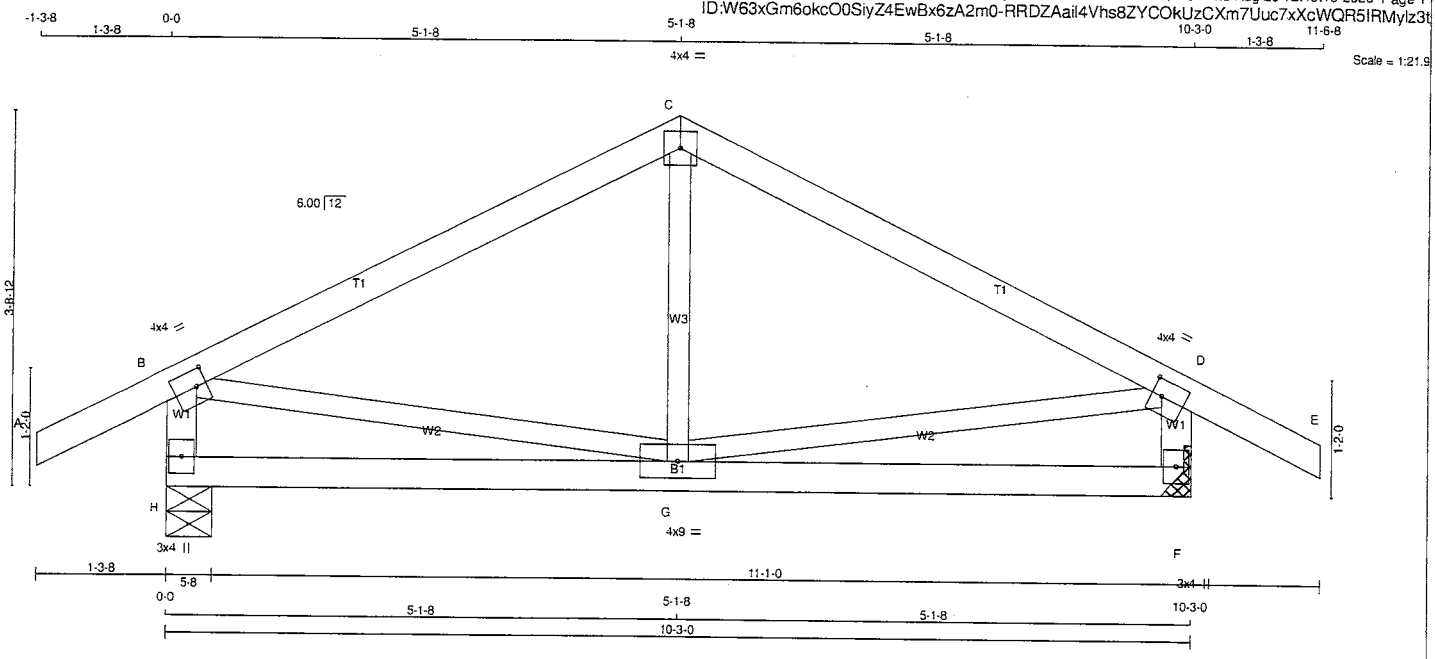
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018778

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410047	T38	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Aug 20 12:49:10 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-RRDZAail4Vhs8ZYCOkUzCXm7Uuc7xXcWQR5IRMylz3t



TOTAL WEIGHT = 40 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-p	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	690	0	690	0	0	5-8	5-8
F	690	0	690	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	485	332 / 0	0 / 0	0 / 0	0 / 0	154 / 0	0 / 0
F	485	332 / 0	0 / 0	0 / 0	0 / 0	154 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B-C	-490 / 0	-91.8	-91.8	0.31 (1)	6.25
C-D	-490 / 0	-91.8	-91.8	0.31 (1)	6.25
D-E	0 / 28	-91.8	-91.8	0.12 (1)	10.00
H-B	-652 / 0	0.0	0.0	0.07 (1)	7.81
F-D	-652 / 0	0.0	0.0	0.07 (1)	7.81
H-G	0 / 0	-18.5	-18.5	0.14 (4)	10.00
G-F	0 / 0	-18.5	-18.5	0.14 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-C	-32 / 77	0.03 (4)
B-G	0 / 445	0.10 (1)
G-D	0 / 445	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.31/1.00 (C-D:1) , BC=0.14/1.00 (G-H:4) , WB=0.10/1.00 (B-G:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (D) (INPUT = 0.90)
JSI METAL= 0.22 (D) (INPUT = 1.00)

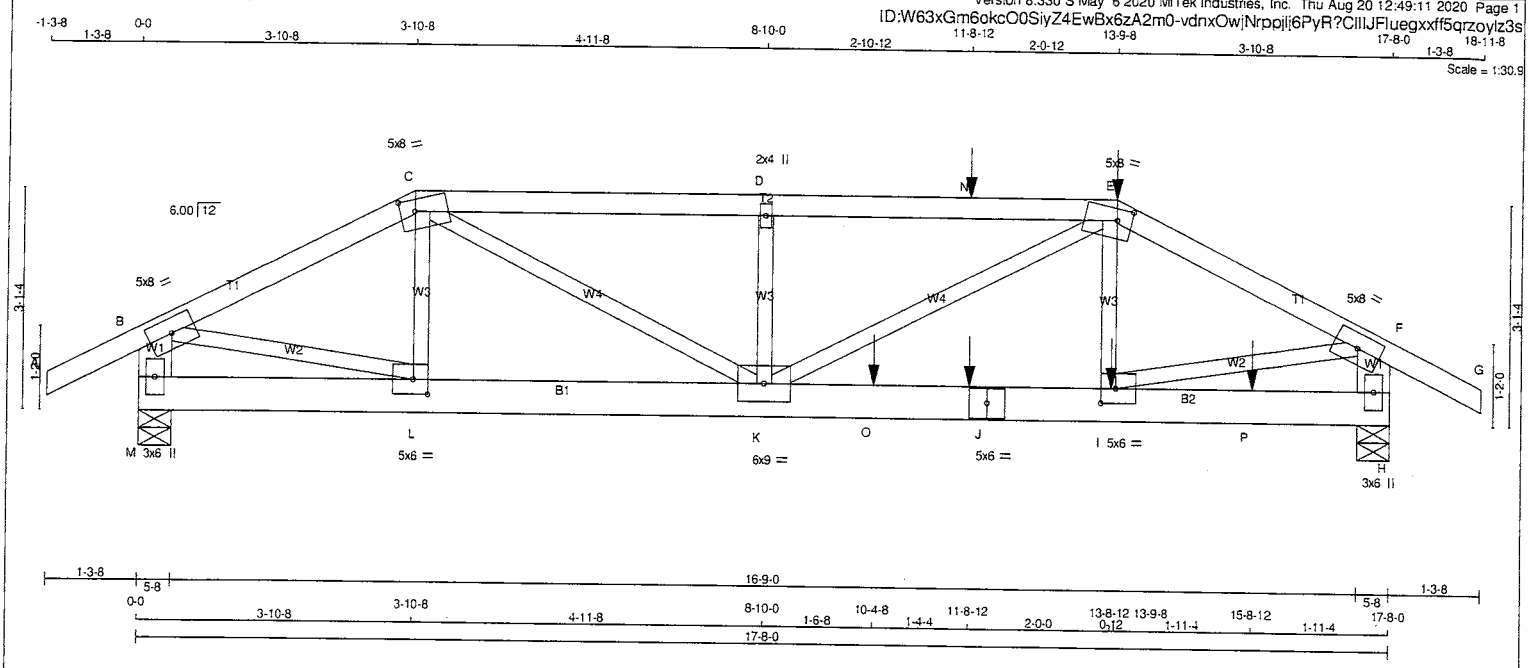
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018779

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410047	T39	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Aug 20 12:49:11 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-vdnxOwjNrppjIj6PyR?CIIIJFluegxxff5qrzoylz3s



TOTAL WEIGHT = 2 X 79 = 157 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	SIDE(61.0)
E- G	1	12	SIDE(61.0)
M- B	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	SIDE(0.0)
J- H	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1550	0	1550	0	0	5-8	5-8
H	1862	0	1862	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1091	744 / 0	0 / 0	0 / 0	0 / 0	347 / 0	0 / 0
H	1311	894 / 0	0 / 0	0 / 0	0 / 0	417 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI	(LC)	MAX. UNBRAC LENGTH
A- B	0 / 28		-91.8	-91.8	0.07	(1)	10.00
B- C	-1926 / 0		-91.8	-91.8	0.15	(1)	6.14
C- D	-3216 / 0		-91.8	-91.8	0.25	(1)	4.94
D- N	-3216 / 0		-91.8	-91.8	0.25	(1)	4.94
N- E	-3216 / 0		-91.8	-91.8	0.25	(1)	4.94
E- F	-2545 / 0		-91.8	-91.8	0.16	(1)	5.51
F- G	0 / 28		-91.8	-91.8	0.07	(1)	10.00
M- B	-1502 / 0		0.0	0.0	0.05	(1)	7.81
H- F	-1887 / 0		0.0	0.0	0.07	(1)	7.81
M- L	0 / 0		-18.5	-18.5	0.02	(1)	10.00
L- K	0 / 1709		-18.5	-18.5	0.20	(1)	10.00
K- O	0 / 2276		-18.5	-18.5	0.44	(1)	10.00
O- J	0 / 2276		-18.5	-18.5	0.44	(1)	10.00
J- I	0 / 2276		-18.5	-18.5	0.44	(1)	10.00
I- P	0 / 0		-18.5	-18.5	0.10	(1)	10.00
P- H	0 / 0		-18.5	-18.5	0.10	(1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI	(LC)
L- C	-322 / 0		0.03	(1)
C- K	0 / 1719		0.21	(1)
K- D	-563 / 0		0.05	(1)
K- E	0 / 1072		0.13	(1)
I- E	0 / 94		0.02	(4)
B- L	0 / 1754		0.22	(1)
I- F	0 / 2317		0.29	(1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	13-9-8	-212	-212	---	FRONT	VERT	TOTAL	---	C1
I	13-8-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1
J	11-8-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1
N	11-8-12	-42	-42	---	FRONT	VERT	TOTAL	---	C1
O	10-4-8	-929	-929	---	FRONT	VERT	TOTAL	---	C1
P	15-8-12	-8	-10	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.25/1.00 (D-E:1) , BC=0.44/1.00 (I-K:1) , WB=0.29/1.00 (F-I:1) , SSI=0.22/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (C) (INPUT = 0.90)
JSI METAL= 0.34 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018780 H

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410047	T39	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TTWW-m	MT20	5.0	8.0	2.00	2.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	2.00	2.50
F	TMVW-t	MT20	5.0	8.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0	2.50	2.50
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	6.0	9.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M	BMV1+p	MT20	3.0	6.0		

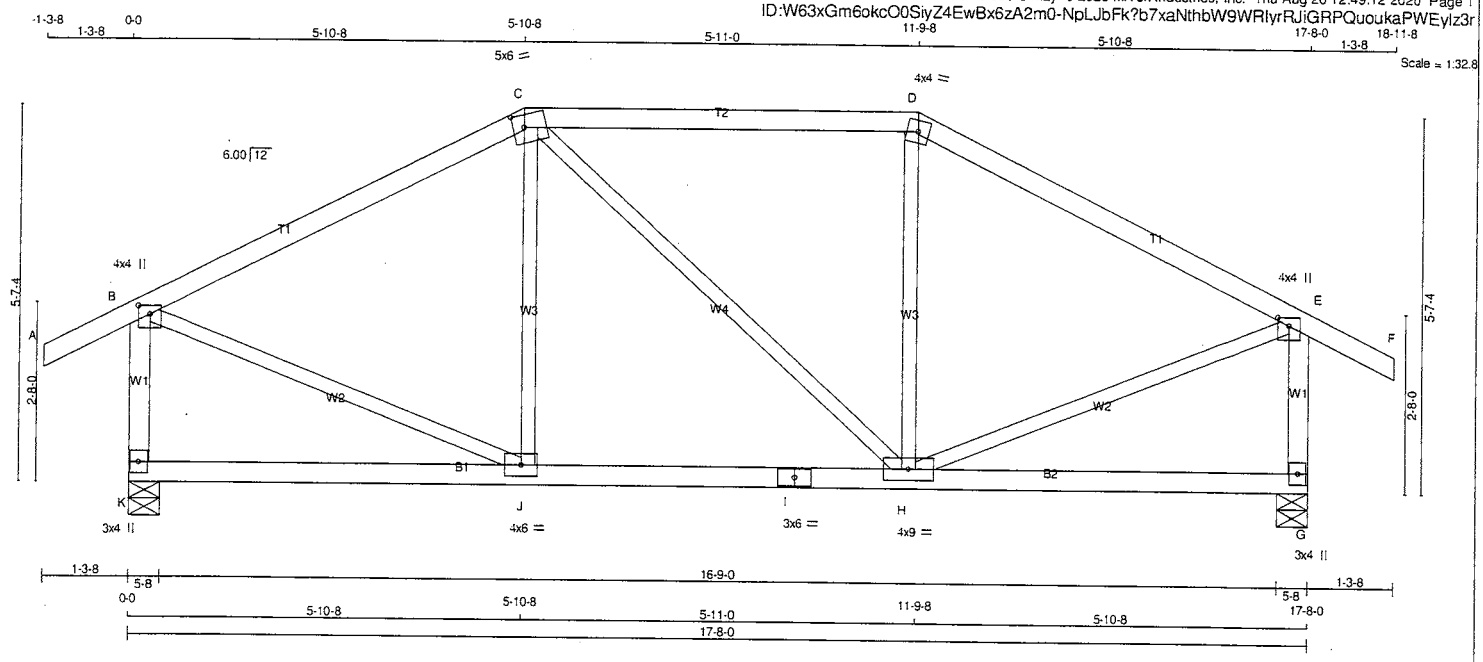
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018780

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410047	T40	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Aug 20 12:49:12 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-NpLJbFk?b7xaNthbW9WRIyrRJiGRPQuoukaPWEyIz3r



TOTAL WEIGHT = 75 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.50	2.00
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.50	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1098	0	1098	0	0	5-8	5-8
G	1098	0	1098	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	774	521 / 0	0 / 0	0 / 0	0 0	253 0	0 0
G	774	521 / 0	0 / 0	0 / 0	0 0	253 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	-823 / 0	-91.8	-91.8	0.42 (1)	6.12
C- D	-732 / 0	-91.8	-91.8	0.42 (1)	6.25
D- E	-823 / 0	-91.8	-91.8	0.42 (1)	6.12
E- F	0 / 28	-91.8	-91.8	0.12 (1)	10.00
K- B	-1053 / 0	0.0	0.0	0.15 (1)	7.67
G- E	-1053 / 0	0.0	0.0	0.15 (1)	7.67
K- J	0 / 0	-18.5	-18.5	0.14 (4)	10.00
J- I	0 / 732	-18.5	-18.5	0.21 (4)	10.00
I- H	0 / 732	-18.5	-18.5	0.21 (4)	10.00
H- G	0 / 0	-18.5	-18.5	0.15 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-172 / 31	0.08 (1)
C- H	0 / 0	0.00 (1)
H- D	-172 / 31	0.08 (1)
B- J	0 / 791	0.18 (1)
H- E	0 / 790	0.18 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.42/1.00 (C-D:1) , BC=0.21/1.00 (H-J:4) , WB=0.18/1.00 (B-J:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (B) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

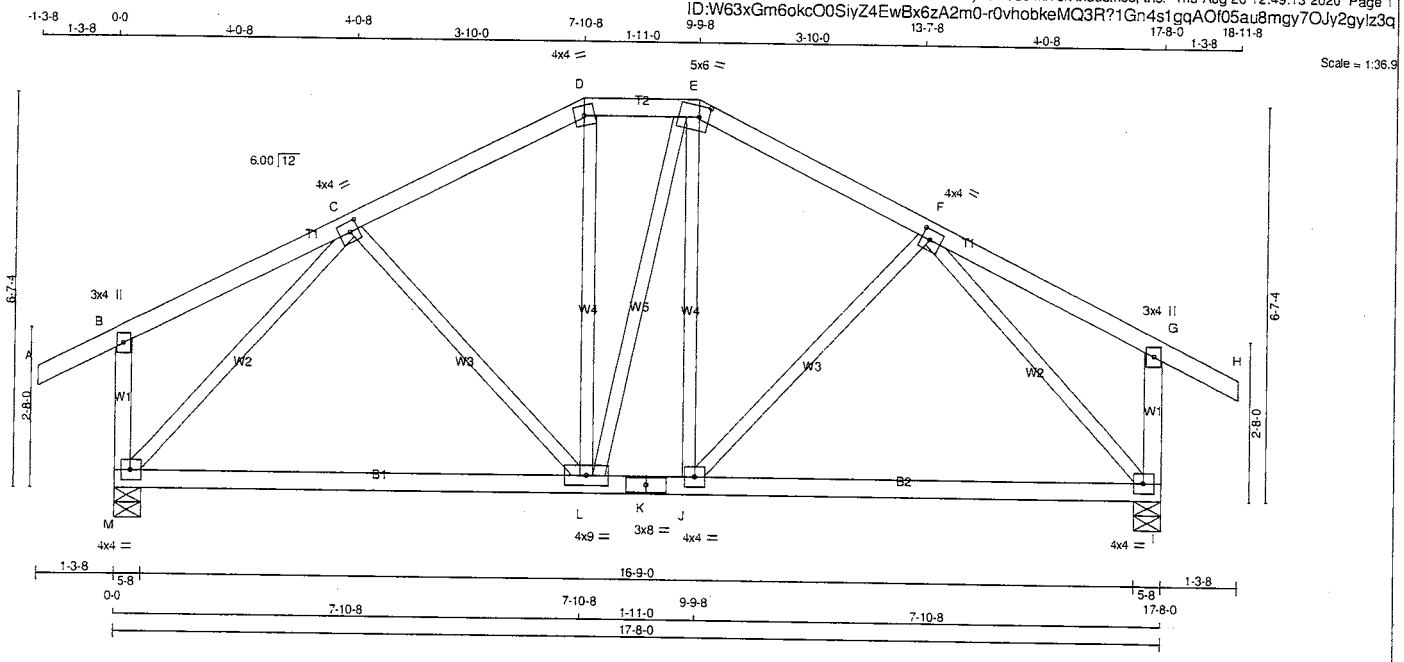
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018781

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410047	T41	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Aug 20 12:49:13 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-r0vhobkeMQ3R?1Gn4s1gqAOf05au8mgy7OJy2gylz3q



TOTAL WEIGHT = 84 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	8.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1098	0	1098	0	0	5-8	5-8
I	1098	0	1098	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	774	521 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
I	774	521 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)		10.00
B- C	0 / 19	-91.8	-91.8	0.23 (1)		10.00
C- D	-791 / 0	-91.8	-91.8	0.18 (1)		6.25
D- E	-692 / 0	-91.8	-91.8	0.05 (1)		6.25
E- F	-790 / 0	-91.8	-91.8	0.18 (1)		6.25
F- G	0 / 19	-91.8	-91.8	0.23 (1)		10.00
G- H	0 / 28	-91.8	-91.8	0.12 (1)		10.00
M- B	-263 / 0	0.0	0.0	0.04 (1)		7.81
I- G	-263 / 0	0.0	0.0	0.04 (1)		7.81
M- L	0 / 705	-18.5	-18.5	0.32 (4)		10.00
L- K	0 / 691	-18.5	-18.5	0.32 (4)		10.00
K- J	0 / 691	-18.5	-18.5	0.32 (4)		10.00
J- I	0 / 705	-18.5	-18.5	0.31 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-23 / 45	0.02 (4)
L- D	0 / 120	0.03 (4)
L- E	0 / 2	0.00 (4)
J- E	0 / 118	0.03 (4)
J- F	-23 / 45	0.02 (4)
M- C	-1048 / 0	0.59 (1)
F- I	-1048 / 0	0.59 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.23/1.00 (F-G:1) , BC=0.32/1.00 (J-L:4) , WB=0.59/1.00 (C-M:1) , SSI=0.16/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (M) (INPUT = 0.90)
JSI METAL= 0.58 (K) (INPUT = 1.00)

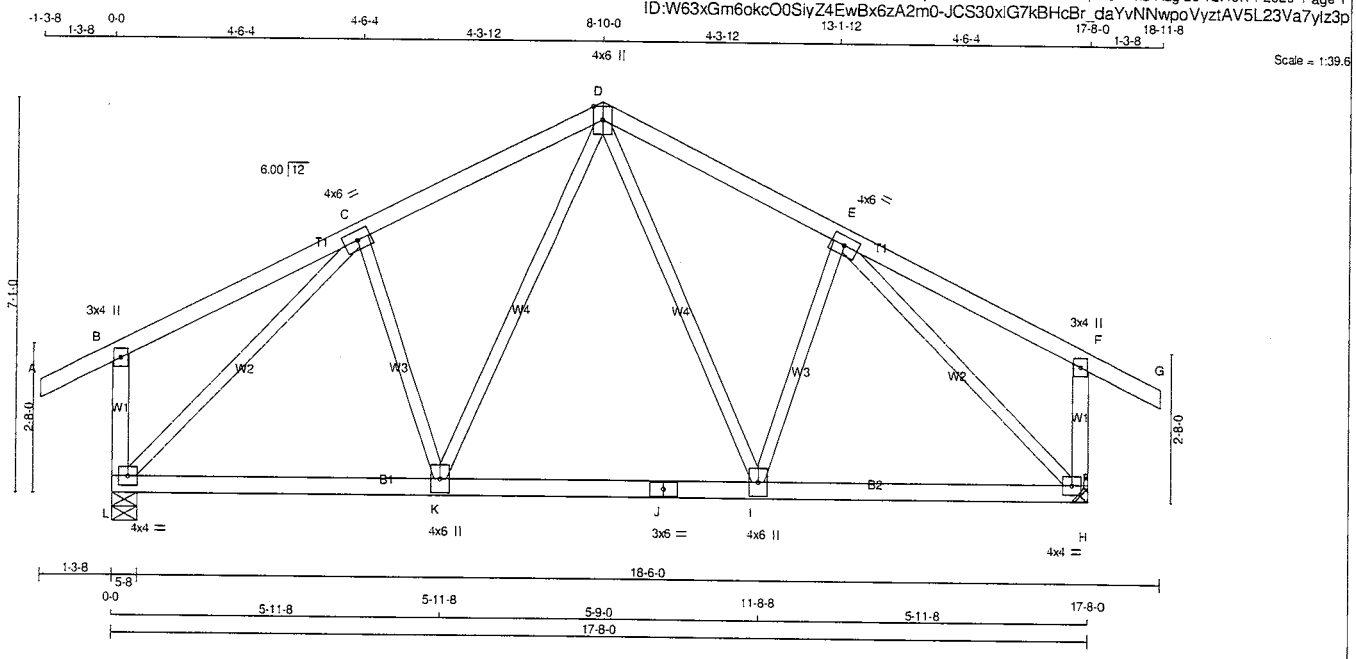
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018782

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410047	T42	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Aug 20 12:49:14 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-JCS30xlG7kBHcBr_daYvNNwpoVyztAV5L23Va7ylz3p



TOTAL WEIGHT = 3 X 80 = 240 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW+t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1098	0	1098	0	0	5-8	5-8
H	1098	0	1098	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	774	521 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
H	774	521 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 21	-91.8	-91.8	0.29 (1)	10.00
C- D	-828 / 0	-91.8	-91.8	0.23 (1)	6.25
D- E	-828 / 0	-91.8	-91.8	0.23 (1)	6.25
E- F	0 / 21	-91.8	-91.8	0.29 (1)	10.00
F- G	0 / 28	-91.8	-91.8	0.12 (1)	10.00
L- B	-283 / 0	0.0	0.0	0.04 (1)	7.81
H- F	-283 / 0	0.0	0.0	0.04 (1)	7.81
L- K	0 / 750	-18.5	-18.5	0.21 (4)	10.00
K- J	0 / 632	-18.5	-18.5	0.20 (4)	10.00
J- I	0 / 632	-18.5	-18.5	0.20 (4)	10.00
I- H	0 / 750	-18.5	-18.5	0.21 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 227	0.05 (1)
I- E	-96 / 35	0.04 (1)
K- D	0 / 227	0.05 (1)
C- K	-96 / 35	0.04 (1)
L- C	-1075 / 0	0.75 (1)
E- H	-1075 / 0	0.75 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.21/1.00 (H-I:4) , WB=0.75/1.00 (C-L:1) , SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

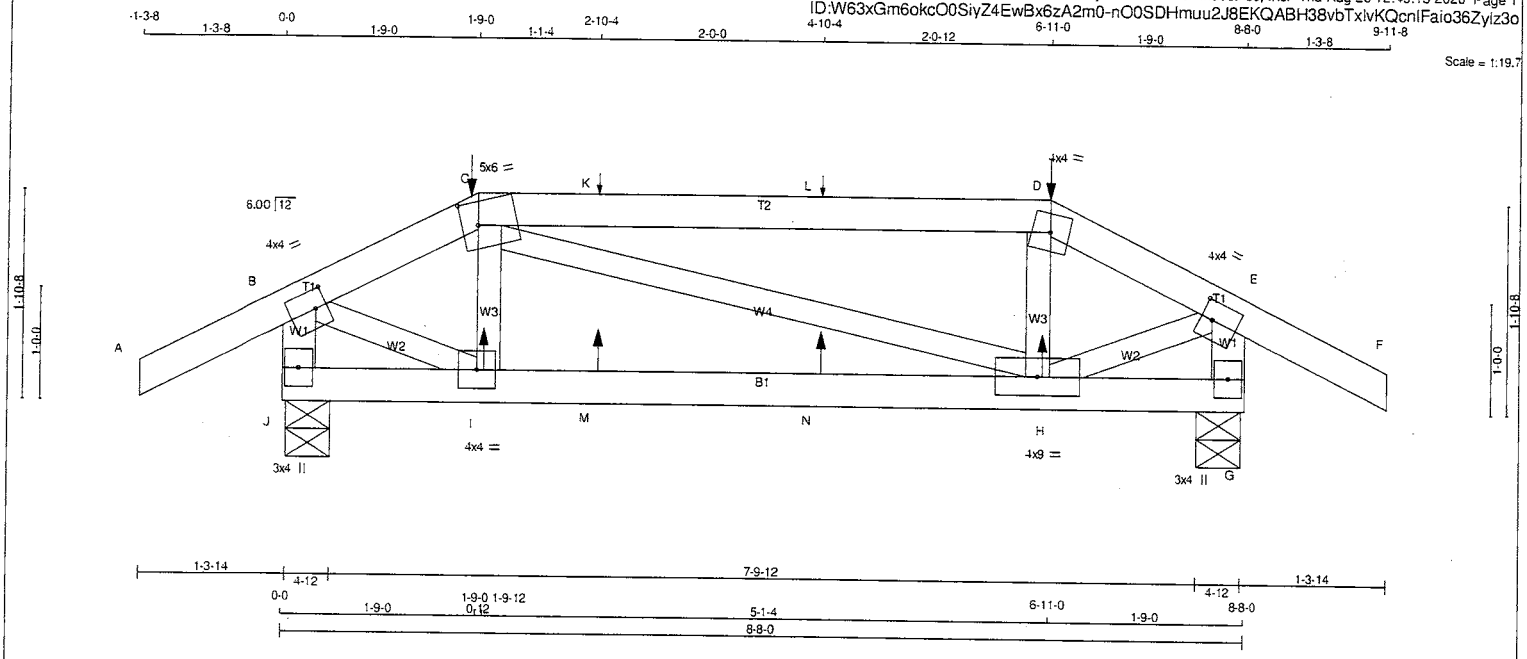
JSI GRIP= 0.84 (L) (INPUT = 0.90)
JSI METAL= 0.26 (H) (INPUT = 1.00)

Structural component only
DWG# T-2018783

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410047	T43	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Aug 20 12:49:15 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-nO0SDHmuu2J8EKQABH38vbTxlvKQcnIFaio36Zyiz3o



TOTAL WEIGHT = 34 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	6.0	2.50	1.75
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	613	0	613	0	0	4-12	4-12
G	615	0	615	0	0	4-12	4-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	431	295 / 0	0 / 0	0 / 0	0 / 0	136 / 0	0 / 0
G	432	296 / 0	0 / 0	0 / 0	0 / 0	136 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.13 (1)		10.00
B- C	-473 / 0	-91.8	-91.8	0.13 (1)		6.25
C- K	-396 / 0	-91.8	-91.8	0.47 (1)		6.25
K- L	-396 / 0	-91.8	-91.8	0.47 (1)		6.25
L- D	-396 / 0	-91.8	-91.8	0.47 (1)		6.25
D- E	-462 / 0	-91.8	-91.8	0.13 (1)		6.25
E- F	0 / 28	-91.8	-91.8	0.13 (1)		10.00
J- B	-607 / 0	0.0	0.0	0.07 (1)		7.81
G- E	-611 / 0	0.0	0.0	0.07 (1)		7.81
J- I	0 / 0	-18.5	-18.5	0.08 (4)		10.00
I- M	0 / 397	-18.5	-18.5	0.13 (4)		10.00
M- N	0 / 397	-18.5	-18.5	0.13 (4)		10.00
N- H	0 / 397	-18.5	-18.5	0.13 (4)		10.00
H- G	0 / 0	-18.5	-18.5	0.08 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-93 / 30	0.01 (1)
C- H	-5 / 28	0.01 (5)
H- D	-98 / 27	0.02 (1)
B- I	0 / 430	0.11 (1)
H- E	0 / 425	0.11 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-9-0	-28	-28	103	FRONT	VERT	TOTAL	---	C1
D	6-11-0	-28	-28	103	FRONT	VERT	TOTAL	---	C1
H	6-10-4	8	1	14	FRONT	VERT	TOTAL	---	C1
I	1-9-12	8	1	14	FRONT	VERT	TOTAL	---	C1
K	2-10-4	1	1	103	FRONT	VERT	TOTAL	---	C1
L	4-10-4	1	1	103	FRONT	VERT	TOTAL	---	C1
M	2-10-4	8	1	14	FRONT	VERT	TOTAL	---	C1
N	4-10-4	8	1	14	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.47/1.00 (C-D:1) , BC=0.13/1.00 (H-I:4) , WB=0.11/1.00 (B-I:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

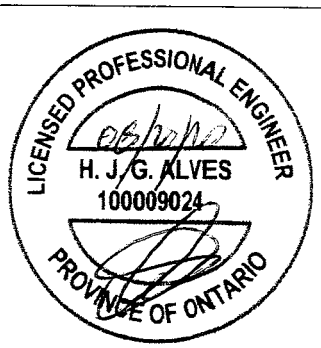
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (B) (INPUT = 0.90)
JSI METAL= 0.19 (B) (INPUT = 1.00)



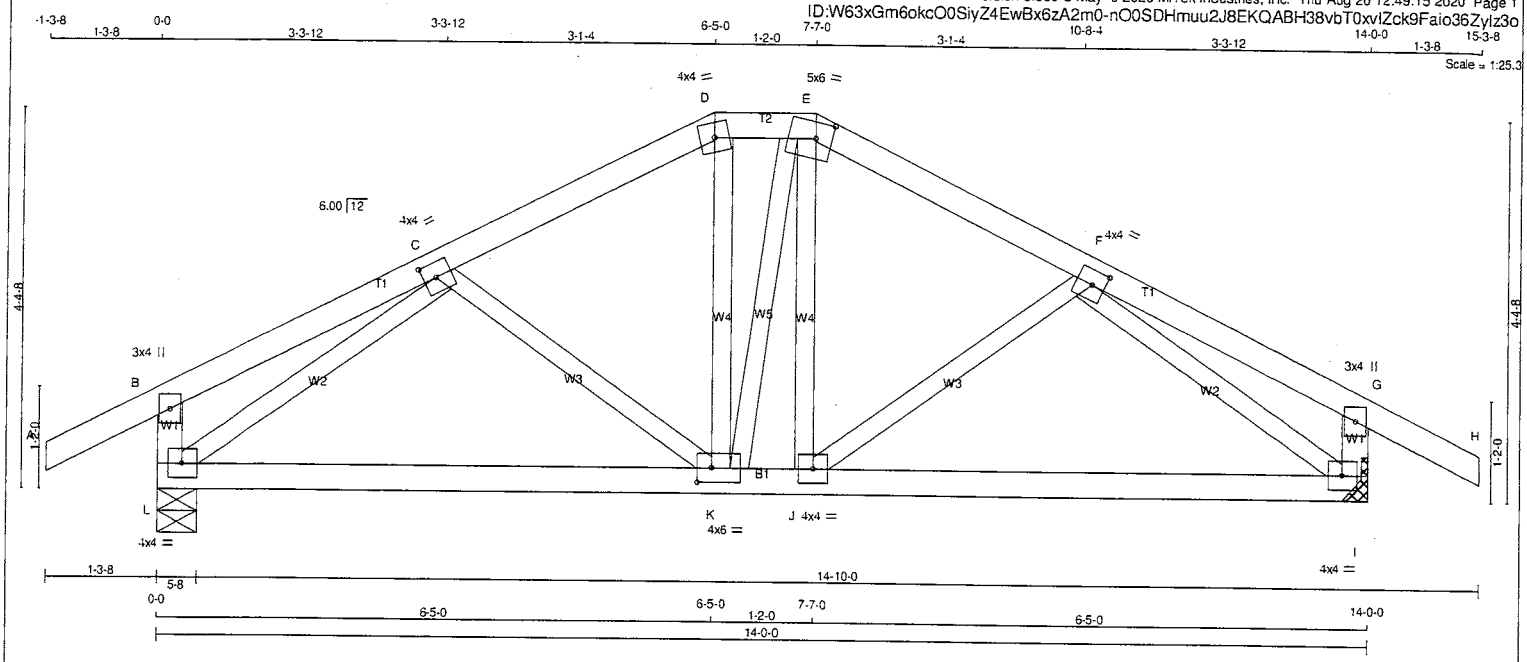
Structural component only
DWG# T-2018784

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410047	T44	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Aug 20 12:49:15 2020 Page 1

ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-nO0SDHmuu2J8EKQABH38vbT0xvIZck9Faio36ZyIz3o



TOTAL WEIGHT = 61 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.25
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWWW-t	MT20	4.0	6.0	2.00	2.00
L	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	896	0	896	0	0	5-8	5-8
I	896	0	896	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	631	428 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0
I	631	428 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 14	-91.8	-91.8	0.14 (1)	10.00
C- D	-767 / 0	-91.8	-91.8	0.11 (1)	6.25
D- E	-677 / 0	-91.8	-91.8	0.02 (1)	6.25
E- F	-765 / 0	-91.8	-91.8	0.11 (1)	6.25
F- G	0 / 14	-91.8	-91.8	0.14 (1)	10.00
G- H	0 / 28	-91.8	-91.8	0.12 (1)	10.00
L- B	-241 / 0	0.0	0.0	0.02 (1)	7.81
I- G	-241 / 0	0.0	0.0	0.02 (1)	7.81
L- K	0 / 791	-18.5	-18.5	0.25 (4)	10.00
K- J	0 / 675	-18.5	-18.5	0.25 (4)	10.00
J- I	0 / 790	-18.5	-18.5	0.24 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-148 / 0	0.04 (1)
K- D	0 / 169	0.04 (1)
K- E	0 / 6	0.00 (1)
J- E	0 / 163	0.04 (1)
J- F	-148 / 0	0.04 (1)
L- C	-997 / 0	0.27 (1)
F- I	-995 / 0	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.14/1.00 (F-G:1) , BC=0.25/1.00 (J-K:4) , WB=0.27/1.00 (C-L:1) , SSI=0.12/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

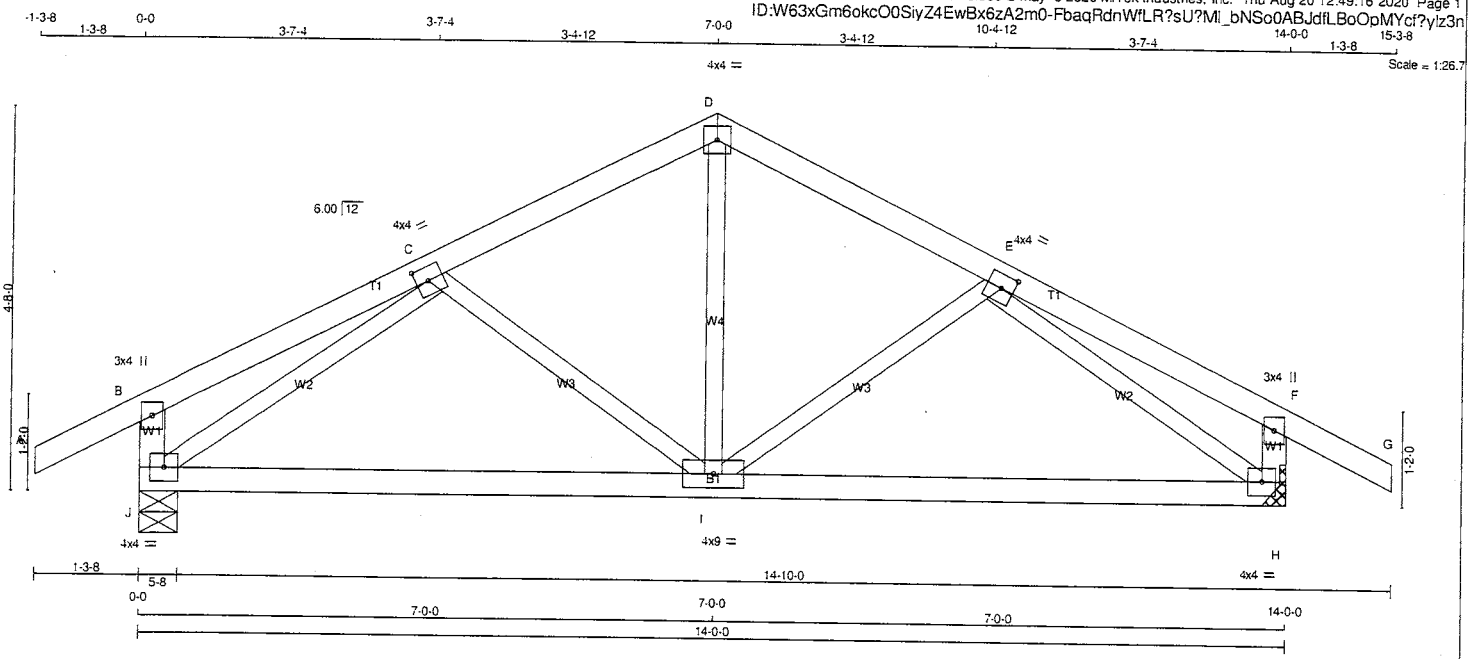
JSI GRIP= 0.85 (L) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)

Structural component only
DWG# T-2018785

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410047	T45	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Aug 20 12:49:16 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-FbaqRdnWfLR?sU?Ml_bNSo0ABJdfLBoOpMYcf?ylz3n



TOTAL WEIGHT = 2 X 56 = 111 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-p	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	896	0	896	0	0	5-8	5-8
H	896	0	896	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	631	428 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0
H	631	428 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	I-D	0 / 394	0.09 (1)
B-C	0 / 16	-91.8	-91.8	0.17 (1)	10.00	I-E	-196 / 0	0.06 (1)
C-D	-735 / 0	-91.8	-91.8	0.13 (1)	6.25	C-I	-196 / 0	0.06 (1)
D-E	-735 / 0	-91.8	-91.8	0.13 (1)	6.25	J-C	-997 0	0.31 (1)
E-F	0 / 16	-91.8	-91.8	0.17 (1)	10.00	E-H	-997 0	0.31 (1)
F-G	0 28	-91.8	-91.8	0.12 (1)	10.00			
J-B	-251 0	0.0	0.0	0.03 (1)	7.81			
H-F	-251 0	0.0	0.0	0.03 (1)	7.81			
J-I	0 / 800	-18.5	-18.5	0.32 (4)	10.00			
I-H	0 800	-18.5	-18.5	0.32 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.17/1.00 (B-C:1) , BC=0.32/1.00 (I-J:4) , WB=0.31/1.00 (E-H:1) , SSI=0.14/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.30 (E) (INPUT = 1.00)

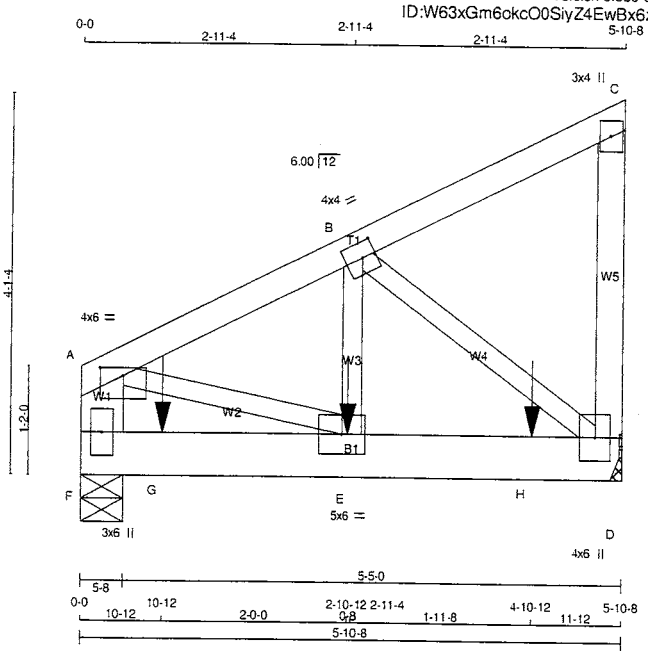
Structural component only
DWG# T-2018786

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410047	T46	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Aug 20 12:49:17 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-jn8Cezo8QfZsTeaYJi6c?0YNgjza4gwY10HABSylz3m

Scale = 1:23.7



TOTAL WEIGHT = 2 X 29 = 58 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	6	SIDE(45.9)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING.. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018787

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1654	0	1654	0	0	5-8	5-8
D	1628	0	1628	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1163	798 / 0	0 / 0	0 / 0	0 / 0	366 / 0	0 / 0
D	1146	786 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1086 / 0	0.0	0.0	0.04 (1)	7.81	A- E	0 / 1320	0.16 (1)
A- B	-1416 / 0	-91.8	-91.8	0.06 (1)	6.25	E- B	0 / 1296	0.16 (1)
B- C	-10 / 0	-91.8	-91.8	0.05 (1)	6.25	B- D	-1606 / 0	0.19 (1)
D- C	-111 / 0	0.0	0.0	0.01 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.18 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.18 (1)	10.00			
E- H	0 / 1276	-18.5	-18.5	0.28 (1)	10.00			
H- D	0 / 1276	-18.5	-18.5	0.28 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-10-12	-878	-878	---	FRONT	VERT	TOTAL	---	C1
G	10-12	-879	-879	---	FRONT	VERT	TOTAL	---	C1
H	4-10-12	-878	-878	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.28/1.00 (D-E:1) , WB=0.19/1.00 (B-D:1) , SSI=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.29 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410047	T46	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-jn8Cezo8QfZsTeaYJi6c?0YNqjza4gwY10HABSylz3m

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

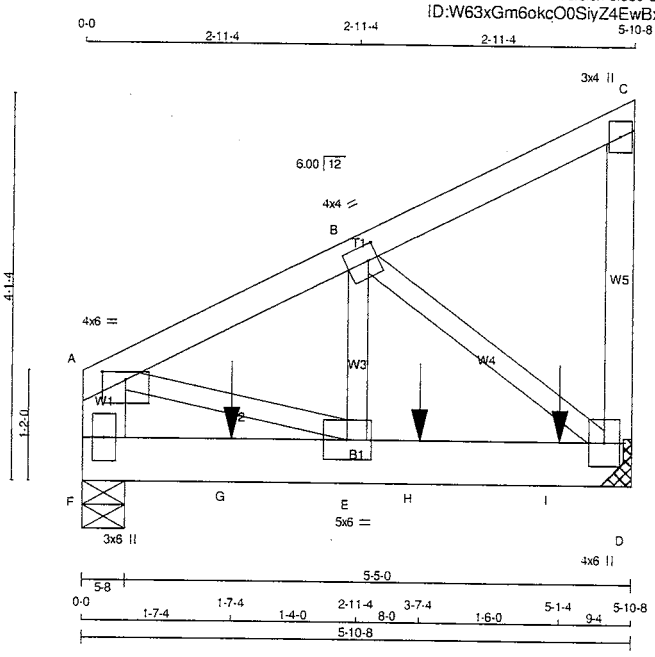
Structural component only
DWG# T-2018787

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410047	T46Z	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Aug 20 12:49:18 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-CziasJomBzhj5o9isPdrXD5XK6Hbp6PhGg1jjuylz3I

Scale = 1:23.4



TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2018788

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1668	0	1668	0	0	5-8	5-8
D	2221	0	2221	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1175	799 / 0	0 / 0	0 / 0	0 / 0	375 / 0	0 / 0
D	1564	1066 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1332 / 0	0.0	0.0	0.05 (1)	7.81	A- E	0 / 1650	0.20 (1)
A- B	-1774 / 0	-91.8	-91.8	0.06 (1)	6.25	E- B	0 / 1705	0.21 (1)
B- C	-9 / 0	-91.8	-91.8	0.05 (1)	10.00	B- D	-2007 / 0	0.24 (1)
D- C	-114 / 0	0.0	0.0	0.01 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.20 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.20 (1)	10.00			
E- H	0 / 1595	-18.5	-18.5	0.35 (1)	10.00			
H- I	0 / 1595	-18.5	-18.5	0.35 (1)	10.00			
I- D	0 / 1595	-18.5	-18.5	0.35 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-7-4	-1080	-1080	---	BACK	VERT	TOTAL	---	C1
H	3-7-4	-1080	-1080	---	BACK	VERT	TOTAL	---	C1
I	5-1-4	-1082	-1082	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.35/1.00 (D-E:1) , WB=0.24/1.00 (B-D:1) , SSI=0.44/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

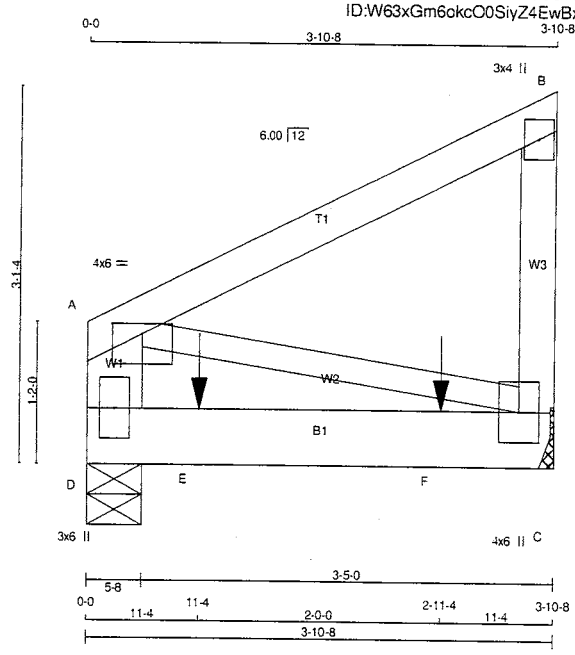
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.36 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410047	T47	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Aug 20 12:49:19 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-g9Gy3fpOyGpaiykxQ7844Reh4WeXYcOqVKmGFKylz3k



TOTAL WEIGHT = 2 X 18 = 37 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x6	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	2	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1082	0	1082	0	0	5-8	5-8
C	948	0	948	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	761	524 / 0	0 / 0	0 / 0	0 / 0	236 / 0	0 / 0
C	666	460 / 0	0 / 0	0 / 0	0 / 0	206 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- A	-178 / 0	0.0	0.0	0.01 (1)	7.81	A- C	0 0	0.00 (1)
A- B	0 / 0	-91.8	-91.8	0.13 (1)	10.00			
C- B	-178 / 0	0.0	0.0	0.01 (1)	7.81			
D- E	0 0	-18.5	-18.5	0.31 (1)	10.00			
E- F	0 / 0	-18.5	-18.5	0.31 (1)	10.00			
F- C	0 / 0	-18.5	-18.5	0.31 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	11-4	-930	-930	---	BACK	VERT	TOTAL	---	C1
F	2-11-4	-672	-672	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.31/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.04 (A) (INPUT = 0.90)
JSI METAL= 0.02 (B) (INPUT = 1.00)

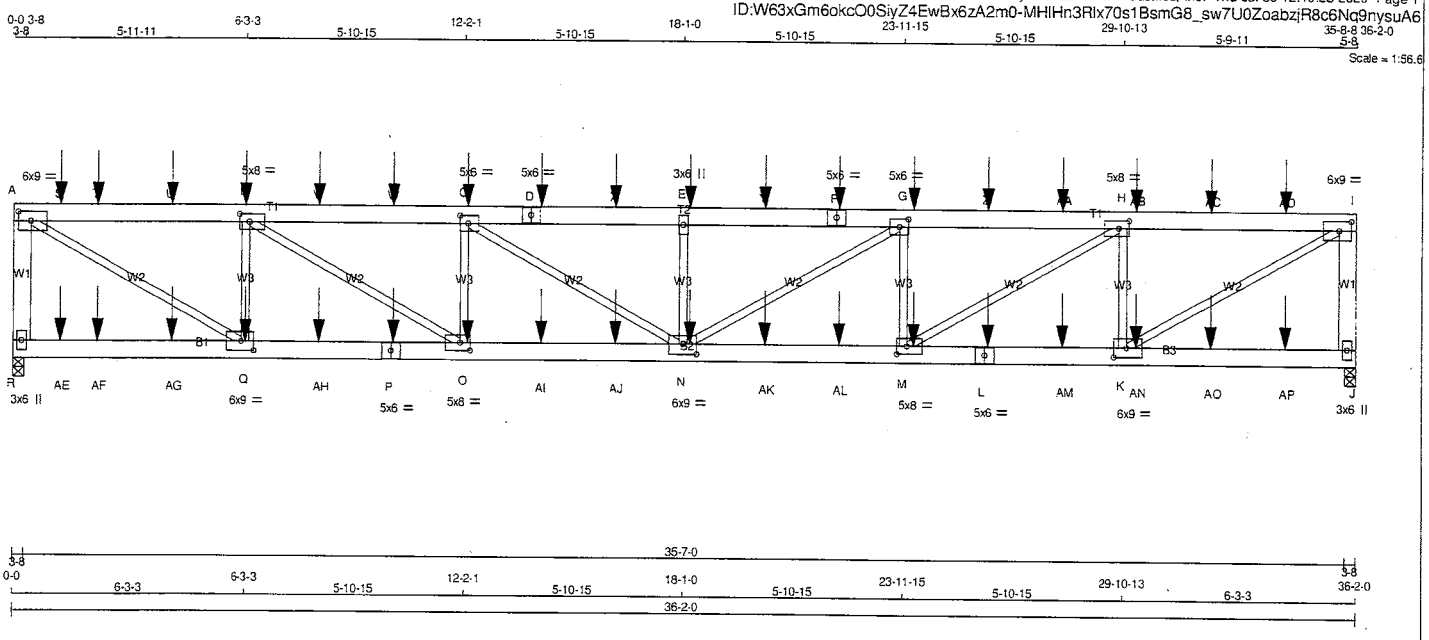
LICENSED PROFESSIONAL ENGINEER
08/20/20
H. J.G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018789

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410045	T70	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:40:23 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-MHlHn3RIx70s1BsmG8_sw7U0ZoabzjR8c6Nq9nysuA6



TOTAL WEIGHT = 2 X 186 = 372 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
R - A	2x6	DRY	No.2	SPF
A - D	2x6	DRY	1650F 1.5E	SPF
D - F	2x6	DRY	1650F 1.5E	SPF
F - I	2x6	DRY	1650F 1.5E	SPF
J - I	2x6	DRY	No.2	SPF
R - P	2x6	DRY	1650F 1.5E	SPF
P - L	2x6	DRY	1650F 1.5E	SPF
L - J	2x6	DRY	1650F 1.5E	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS				
R- A	2	12		TOP
A- D	2	12		SIDE(183.1)
D- F	2	12		SIDE(183.1)
F- I	2	12		SIDE(183.1)
I- J	2	12		TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS				
R- P	2	12		SIDE(183.1)
P- L	2	12		SIDE(183.1)
L- J	2	12		SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS				
2x3	1	6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

Structural component only
DWG# T-2017025 //L

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	3280	0	3280	0	0	3-8	3-8
J	3170	0	3170	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2323	1504 / 0	0 / 0	0 / 0	0 / 0	819 / 0	0 / 0
J	2245	1455 / 0	0 / 0	0 / 0	0 / 0	790 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
R- A	-3181 / 0	0.0	0.0	0.23 (1)	7.75
A- S	-4771 / 0	-91.8	-91.8	0.17 (1)	5.74
S- T	-4771 / 0	-91.8	-91.8	0.17 (1)	5.74
T- U	-4771 / 0	-91.8	-91.8	0.17 (1)	5.74
U- B	-4771 / 0	-91.8	-91.8	0.17 (1)	5.74
B- V	-7260 / 0	-91.8	-91.8	0.17 (1)	4.91
V- W	-7260 / 0	-91.8	-91.8	0.17 (1)	4.91
W- C	-7260 / 0	-91.8	-91.8	0.17 (1)	4.91
C- D	-8107 / 0	-91.8	-91.8	0.14 (1)	4.74
D- X	-8107 / 0	-91.8	-91.8	0.14 (1)	4.74
X- E	-8107 / 0	-91.8	-91.8	0.14 (1)	4.74
E- Y	-8107 / 0	-91.8	-91.8	0.14 (1)	4.73
Y- F	-8107 / 0	-91.8	-91.8	0.14 (1)	4.73
F- G	-8107 / 0	-91.8	-91.8	0.14 (1)	4.73
G- Z	-7235 / 0	-91.8	-91.8	0.17 (1)	4.92
Z-AA	-7235 / 0	-91.8	-91.8	0.17 (1)	4.92
AA- H	-7235 / 0	-91.8	-91.8	0.17 (1)	4.92
H-AB	-4723 / 0	-91.8	-91.8	0.15 (1)	5.79
AB-AC	-4723 / 0	-91.8	-91.8	0.15 (1)	5.79
AC-AD	-4723 / 0	-91.8	-91.8	0.15 (1)	5.79
AD- I	-4723 / 0	-91.8	-91.8	0.15 (1)	5.79
J- I	-3087 / 0	0.0	0.0	0.23 (1)	7.81
R-AE	0 / 0	-18.5	-18.5	0.06 (4)	10.00
AE-AF	0 / 0	-18.5	-18.5	0.06 (4)	10.00
AF-AG	0 / 0	-18.5	-18.5	0.06 (4)	10.00
AG- Q	0 / 0	-18.5	-18.5	0.06 (4)	10.00
Q-AH	0 / 4771	-18.5	-18.5	0.23 (1)	10.00
AH- P	0 / 4771	-18.5	-18.5	0.23 (1)	10.00
P- O	0 / 4771	-18.5	-18.5	0.23 (1)	10.00
O-AI	0 / 7260	-18.5	-18.5	0.34 (1)	10.00
AI-AJ	0 / 7260	-18.5	-18.5	0.34 (1)	10.00
AJ- N	0 / 7260	-18.5	-18.5	0.34 (1)	10.00
N-AK	0 / 7235	-18.5	-18.5	0.34 (1)	10.00
AK-AL	0 / 7235	-18.5	-18.5	0.34 (1)	10.00
AL- M	0 / 7235	-18.5	-18.5	0.34 (1)	10.00
M- L	0 / 4723	-18.5	-18.5	0.23 (1)	10.00
L-AM	0 / 4723	-18.5	-18.5	0.23 (1)	10.00
AM- K	0 / 4723	-18.5	-18.5	0.23 (1)	10.00
K-AN	0 / 0	-18.5	-18.5	0.05 (4)	10.00
AN-AO	0 / 0	-18.5	-18.5	0.05 (4)	10.00
AO-AP	0 / 0	-18.5	-18.5	0.05 (4)	10.00
AP- J	0 / 0	-18.5	-18.5	0.05 (4)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
K- I	0 / 5464	0.68 (1)
A- Q	0 / 5520	0.68 (1)
K- H	-2545 / 0	0.30 (1)
Q- B	-2565 / 0	0.31 (1)
M- H	0 / 2952	0.37 (1)
B- O	0 / 2925	0.36 (1)
M- G	-1368 / 0	0.16 (1)
O- C	-1355 / 0	0.16 (1)
N- G	0 / 1025	0.13 (1)
C- N	0 / 996	0.12 (1)
N- E	-878 / 0	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.41")

CSI: TC=0.23/1.00 (A-R:1) , BC=0.34/1.00 (N-O:1) , WB=0.68/1.00 (A-Q:1) , SSI=0.19/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (I) (INPUT = 0.90)
JSI METAL= 0.46 (L) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410045	T70	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-MHlHn3Rlx70s1BsmG8_sw7U0Zoabzj R8c6Nq9nysuA6

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	9.0	3.00	4.00
B	TMWW-t	MT20	5.0	8.0	2.50	3.25
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TS-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.75
H	TMWW-t	MT20	5.0	8.0	2.50	3.25
I	TMVW-t	MT20	6.0	9.0	3.00	4.00
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	6.0	9.0	3.00	4.00
L	BS-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	8.0	2.50	3.25
N	BMWWW-t	MT20	6.0	9.0	3.25	4.50
O	BMWW-t	MT20	5.0	8.0	2.50	3.25
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	6.0	9.0	3.00	4.00
R	BMV1+p	MT20	3.0	6.0		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	6-3-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
C	12-3-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
D	14-3-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
E	18-3-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
F	22-3-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
G	24-3-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
L	26-3-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
M	24-3-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
N	18-3-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	12-3-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	10-3-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Q	6-3-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
S	1-3-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
T	2-3-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
U	4-3-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	8-3-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	10-3-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	16-3-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	20-3-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	26-3-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	28-3-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	30-3-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	32-3-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	34-3-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AE	1-3-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AF	2-3-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AG	4-3-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AH	8-3-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AI	14-3-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AJ	16-3-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AK	20-3-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AL	22-3-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AM	28-3-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AN	30-3-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AO	32-3-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AP	34-3-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

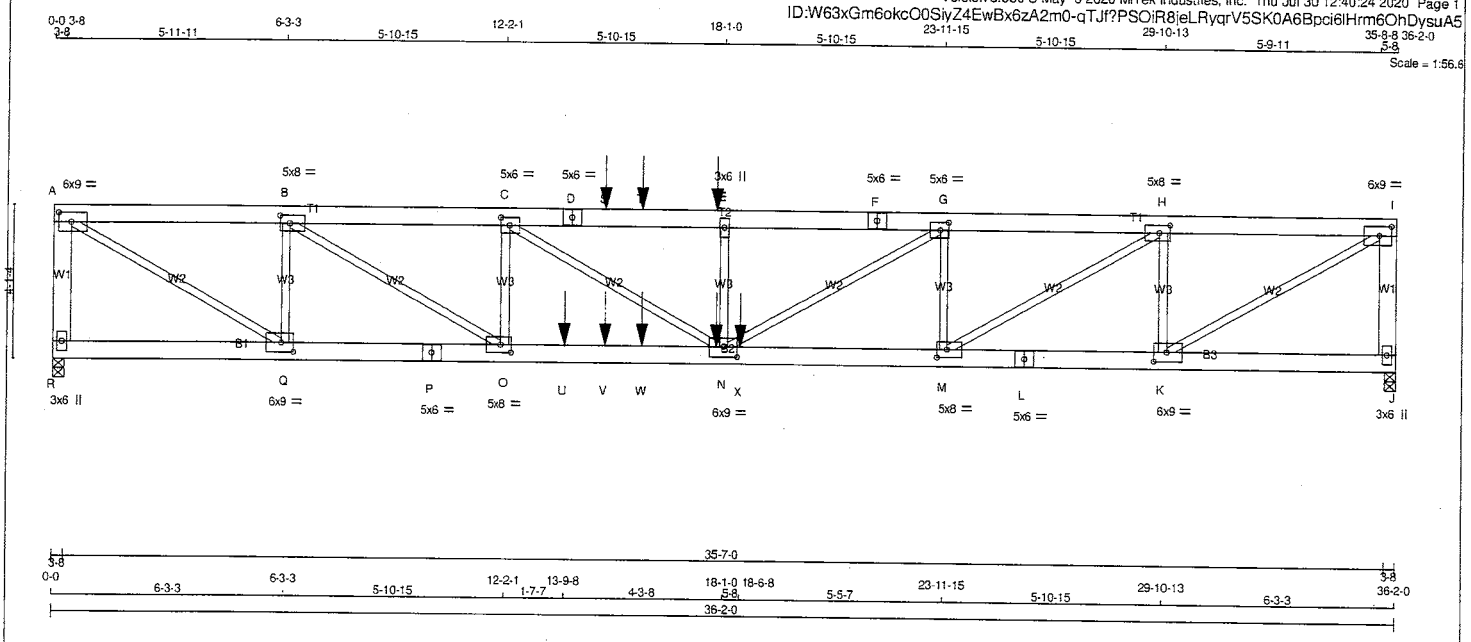
1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2017025 Y2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410045	T70Z	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:40:24 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-qTJf?PSOiR8jeLRyqrV5SK0A6Bpci6lHrm6OhDysuA5



TOTAL WEIGHT = 2 X 186 = 372 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
R - A	2x6	DRY	No.2	SPF
A - D	2x6	DRY	1650F 1.5E	SPF
D - F	2x6	DRY	1650F 1.5E	SPF
F - I	2x6	DRY	1650F 1.5E	SPF
J - I	2x6	DRY	No.2	SPF
R - P	2x6	DRY	1650F 1.5E	SPF
P - L	2x6	DRY	1650F 1.5E	SPF
L - J	2x6	DRY	1650F 1.5E	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS				
R- A	2	12		TOP
A- D	2	12		TOP
D- F	2	12		SIDE(183.1)
F- I	2	12		TOP
I- J	2	12		TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS				
R- P	2	12		TOP
P- L	2	12		SIDE(183.1)
L- J	2	12		TOP
WEBS : (0.122"X3") SPIRAL NAILS				
2x3	1	6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

Structural component only
DWG# T-2017026 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	4133	0	4133	0	0	3-8	3-8
J	3815	0	3815	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2916	1950 / 0	0 / 0	0 / 0	0 / 0	967 / 0	0 / 0
J	2692	1799 / 0	0 / 0	0 / 0	0 / 0	894 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
FR-TO							
R- A	-4052 / 0	0.0	0.0	0.30	(1)		7.06
A- B	-6535 / 0	-91.8	-91.8	0.13	(1)		5.16
B- C	-11728 / 0	-91.8	-91.8	0.18	(1)		4.06
C- D	-13889 / 0	-91.8	-91.8	0.31	(1)		3.67
D- S	-13889 / 0	-91.8	-91.8	0.31	(1)		3.67
S- T	-13889 / 0	-91.8	-91.8	0.31	(1)		3.67
T- E	-13889 / 0	-91.8	-91.8	0.31	(1)		3.67
E- F	-13889 / 0	-91.8	-91.8	0.24	(1)		3.74
F- G	-13889 / 0	-91.8	-91.8	0.24	(1)		3.74
G- H	-10531 / 0	-91.8	-91.8	0.16	(1)		4.25
H- I	-6025 / 0	-91.8	-91.8	0.12	(1)		5.33
J- I	-3752 / 0	0.0	0.0	0.27	(1)		7.28
R- Q	0 / 0	-18.5	-18.5	0.04	(1)		10.00
Q- P	0 / 6535	-18.5	-18.5	0.38	(1)		10.00
P- O	0 / 6535	-18.5	-18.5	0.38	(1)		10.00
O- U	0 / 11728	-18.5	-18.5	0.80	(1)		10.00
U- V	0 / 11728	-18.5	-18.5	0.80	(1)		10.00
V- W	0 / 11728	-18.5	-18.5	0.80	(1)		10.00
W- N	0 / 11728	-18.5	-18.5	0.80	(1)		10.00
N- X	0 / 10531	-18.5	-18.5	0.58	(1)		10.00
X- M	0 / 10531	-18.5	-18.5	0.58	(1)		10.00
M- L	0 / 6025	-18.5	-18.5	0.27	(1)		10.00
L- K	0 / 6025	-18.5	-18.5	0.27	(1)		10.00
K- J	0 / 0	-18.5	-18.5	0.03	(4)		10.00

WEBS MEMB.	FORCE (LBS)	MAX CSI	(LC)
K- I	0 / 8971	0.86	(1)
A- Q	0 / 7561	0.94	(1)
K- H	-3405 / 0	0.41	(1)
Q- B	-3812 / 0	0.46	(1)
M- H	0 / 5294	0.66	(1)
B- O	0 / 6101	0.76	(1)
M- G	-2595 / 0	0.31	(1)
O- C	-1979 / 0	0.24	(1)
N- G	0 / 3946	0.49	(1)
C- N	0 / 2540	0.31	(1)
N- E	-753 / 0	0.09	(1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	17-10-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
N	17-10-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
S	14-10-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
T	15-10-12	-110	-110	---	BACK	VERT	TOTAL	---	C1
U	13-9-8	-1398	-1398	---	BACK	VERT	TOTAL	---	C1
V	14-10-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
W	15-10-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
X	18-6-8	-2152	-2152	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.37")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 636 (0.68")

CSI: TC=0.31/1.00 (C-E:1) , BC=0.80/1.00 (N-O:1) , WB=0.94/1.00 (A-Q:1) , SSI=0.71/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.63 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410045	T70Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

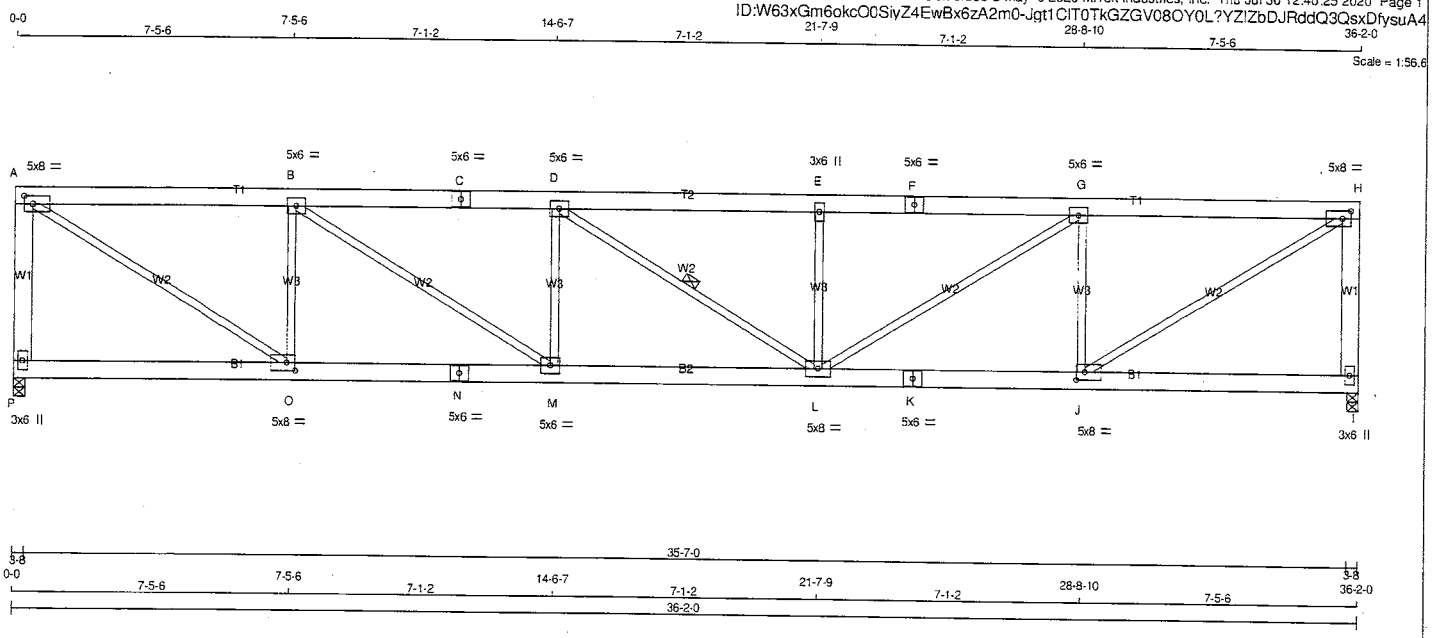
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	9.0	3.00	4.00
B	TMWW-t	MT20	5.0	8.0	2.50	3.25
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TS-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.75
H	TMWW-t	MT20	5.0	8.0	2.50	3.25
I	TMVW-t	MT20	6.0	9.0	3.00	4.00
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	6.0	9.0	3.00	4.00
L	BS-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	8.0	2.50	3.25
N	BMWWW-t	MT20	6.0	9.0	3.25	4.50
O	BMWW-t	MT20	5.0	8.0	2.50	3.25
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	6.0	9.0	3.00	4.00
R	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2017026

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410045	T71	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:40:25 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-Jgt1CIT0TkGZGV08OY0L?YZIZbDJRddQ3QsxDfysuA4



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - A	2x6	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	5.0	8.0	2.50	2.75
B, D, G						
B	TMWW-t	MT20	5.0	6.0		
C	TS-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	8.0	2.50	2.75
I	BMV1+p	MT20	3.0	6.0		
J	BMWW-t	MT20	5.0	8.0	2.50	2.75
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	8.0	2.50	2.75
P	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2017027

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1994	0	1994	0	0	3-8	3-8
I	1994	0	1994	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1410	925 / 0	0 / 0	0 / 0	0 / 0	485 / 0	0 / 0
I	1410	925 / 0	0 / 0	0 / 0	0 / 0	485 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH FR-TO	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
P- A	-1933 / 0	0.0	0.0	0.46 (1)	7.23	J- H	0 / 3125	0.70 (1)
A- B	-2650 / 0	-91.8	-91.8	0.39 (1)	4.72	A- O	0 / 3124	0.70 (1)
B- C	-3805 / 0	-91.8	-91.8	0.45 (1)	4.03	J- G	-1505 / 0	0.50 (1)
C- D	-3805 / 0	-91.8	-91.8	0.45 (1)	4.03	O- B	-1507 / 0	0.50 (1)
D- E	-3801 / 0	-91.8	-91.8	0.34 (1)	4.16	L- G	0 / 1374	0.31 (1)
E- F	-3801 / 0	-91.8	-91.8	0.45 (1)	4.03	B- M	0 / 1380	0.31 (1)
F- G	-3801 / 0	-91.8	-91.8	0.45 (1)	4.03	L- E	-627 / 0	0.21 (1)
G- H	-2651 / 0	-91.8	-91.8	0.39 (1)	4.72	M- D	-628 / 0	0.21 (1)
I- H	-1933 / 0	0.0	0.0	0.46 (1)	7.23	D- L	-5 / 0	0.00 (1)
P- O	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
O- N	0 / 2650	-18.5	-18.5	0.36 (1)	10.00			
N- M	0 / 2650	-18.5	-18.5	0.36 (1)	10.00			
M- L	0 / 3805	-18.5	-18.5	0.51 (1)	10.00			
L- K	0 / 2651	-18.5	-18.5	0.37 (1)	10.00			
K- J	0 / 2651	-18.5	-18.5	0.37 (1)	10.00			
J- I	0 / 0	-18.5	-18.5	0.10 (4)	10.00			

TOTAL WEIGHT = 2 X 191 = 382 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.40")

CSI: TC=0.46/1.00 (H-I:1) , BC=0.51/1.00 (L-M:1) , WB=0.70/1.00 (H-J:1) , SSI=0.24/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

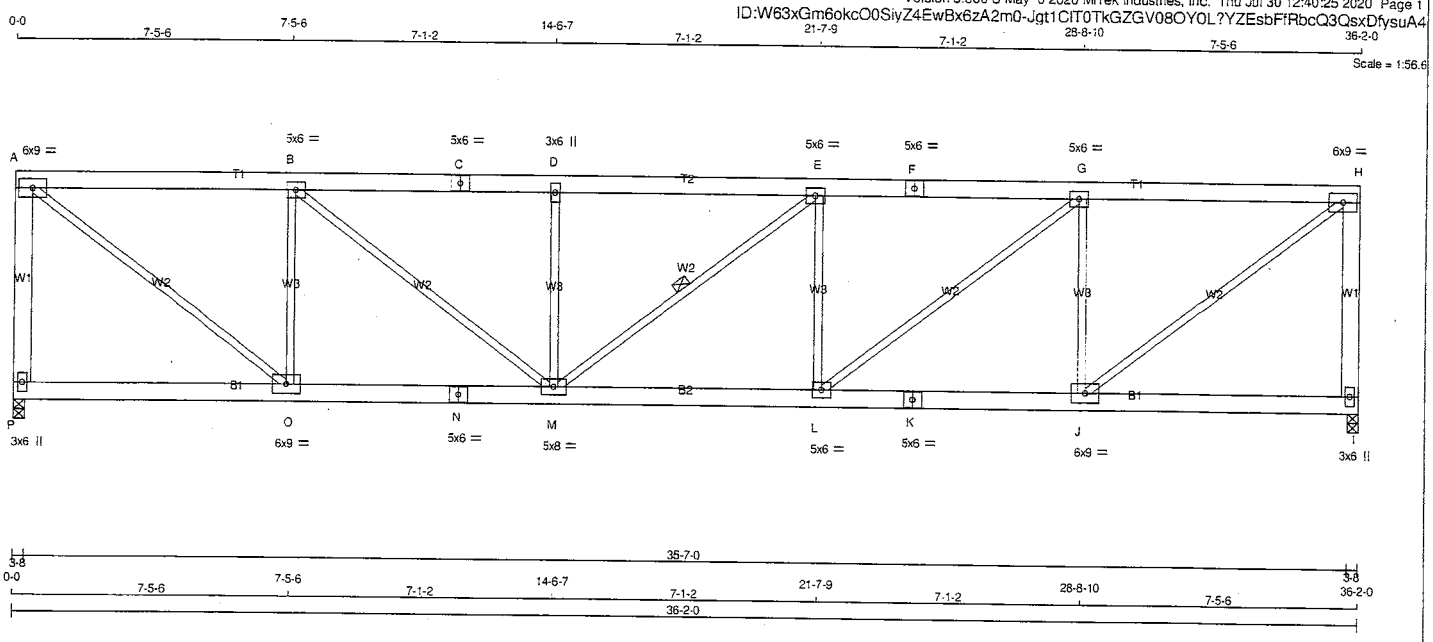
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.52 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410045	T72	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:40:25 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-Jgt1CIT0TkGZGV08OY0L?YZEsbFfRbcQ3QsxDfysuA4



TOTAL WEIGHT = 2 X 200 = 400 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - A	2x6	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	9.0		
B, E, G						
B	TMWW-t	MT20	5.0	6.0		
C	TS-t	MT20	5.0	6.0		
D	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
H	TMVW-t	MT20	6.0	9.0		
I	BMV1+p	MT20	3.0	6.0		
J	BMWW-t	MT20	6.0	9.0		
K	BS-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BS-t	MT20	5.0	6.0		
O	BMWW-t	MT20	6.0	9.0		
P	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2017028

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1994	0	1994	0	0	3-8	3-8
I	1994	0	1994	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1410	925 / 0	0 / 0	0 / 0	0 / 0	485 / 0	0 / 0
I	1410	925 / 0	0 / 0	0 / 0	0 / 0	485 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
P- A	-1935 / 0	0.0	0.0	0.76 (1)	7.23
A- B	-2186 / 0	-91.8	-91.8	0.38 (1)	5.10
B- C	-3131 / 0	-91.8	-91.8	0.42 (1)	4.38
C- D	-3131 / 0	-91.8	-91.8	0.42 (1)	4.38
D- E	-3131 / 0	-91.8	-91.8	0.31 (1)	4.52
E- F	-3135 / 0	-91.8	-91.8	0.42 (1)	4.38
F- G	-3135 / 0	-91.8	-91.8	0.42 (1)	4.38
G- H	-2185 / 0	-91.8	-91.8	0.37 (1)	5.10
I- H	-1935 / 0	0.0	0.0	0.76 (1)	7.23
P- O	0 / 0	-18.5	-18.5	0.11 (4)	10.00
O- N	0 / 2186	-18.5	-18.5	0.31 (1)	10.00
N- M	0 / 2186	-18.5	-18.5	0.31 (1)	10.00
M- L	0 / 3135	-18.5	-18.5	0.43 (1)	10.00
L- K	0 / 2185	-18.5	-18.5	0.31 (1)	10.00
K- J	0 / 2185	-18.5	-18.5	0.31 (1)	10.00
J- I	0 / 0	-18.5	-18.5	0.11 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- H	0 / 2742	0.62 (1)
A- O	0 / 2743	0.62 (1)
J- G	-1508 / 0	0.77 (1)
O- B	-1506 / 0	0.77 (1)
L- G	0 / 1214	0.27 (1)
B- M	0 / 1208	0.27 (1)
L- E	-628 / 0	0.32 (1)
M- D	-626 / 0	0.32 (1)
M- E	-5 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.76/1.00 (A-P:1) , BC=0.43/1.00 (L-M:1) , WB=0.77/1.00 (G-J:1) , SSI=0.24/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

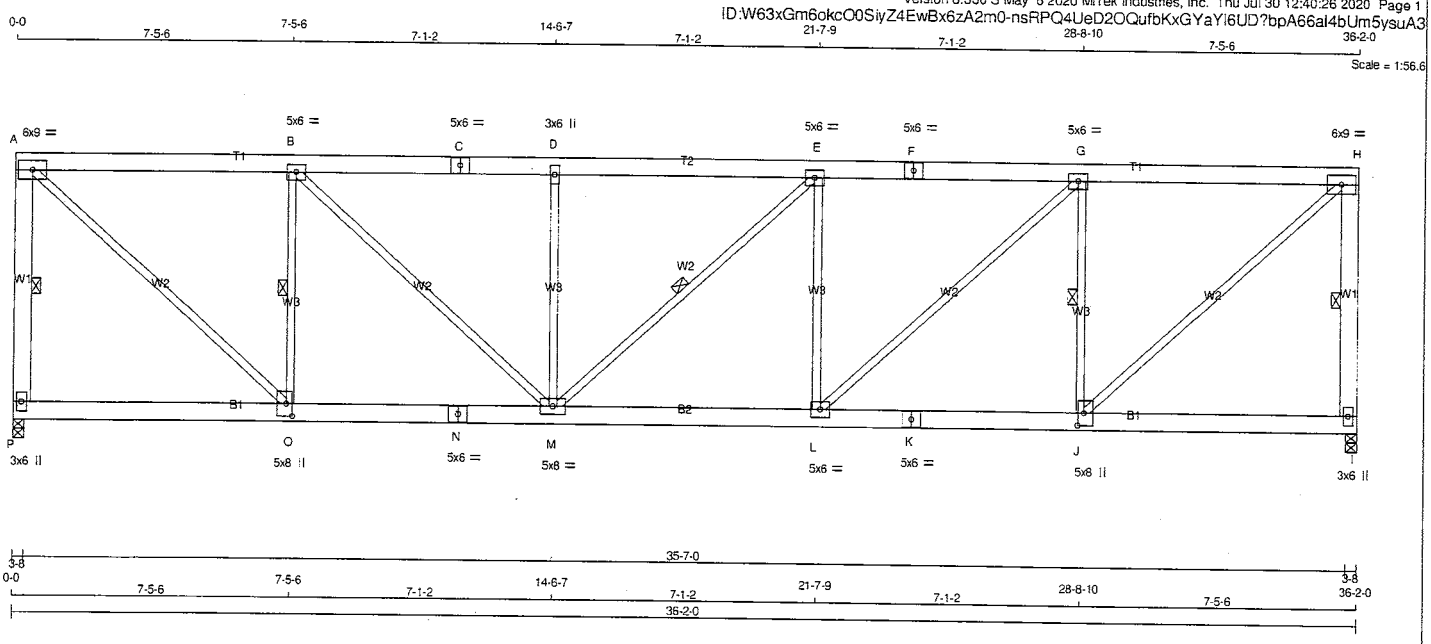
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (A) (INPUT = 0.90)
JSI METAL= 0.43 (N) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410045	T73	8	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:40:26 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-nsRPQ4UeD2OQufbKxGYaYl6UD?bpA66al4bUm5ysuA3



TOTAL WEIGHT = 8 X 210 = 1677 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - A	2x6	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	9.0		
B, E, G						
B	TMWW-t	MT20	5.0	6.0		
C	TS-t	MT20	5.0	6.0		
D	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
H	TMVW-t	MT20	6.0	9.0		
I	BMV1+p	MT20	3.0	6.0		
J	BMWW+t	MT20	5.0	8.0	4.00	2.00
K	BS-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BS-t	MT20	5.0	6.0		
O	BMWW+t	MT20	5.0	8.0	4.00	2.00
P	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2017029

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1994	0	1994	0	0	3-8	3-8
I	1994	0	1994	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1410	925 / 0	0 / 0	0 / 0	0 / 0	485 / 0	0 / 0
I	1410	925 / 0	0 / 0	0 / 0	0 / 0	485 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF A-P, H-I, G-J, B-O, E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- A	-1936 / 0	0.0	0.0	0.26 (1)	5.79	J- H	0 / 2491	0.56 (1)	
A- B	-1859 / 0	-91.8	-91.8	0.37 (1)	5.43	A- O	0 / 2492	0.56 (1)	
B- C	-2661 / 0	-91.8	-91.8	0.40 (1)	4.69	J- G	-1508 / 0	0.44 (1)	
C- D	-2661 / 0	-91.8	-91.8	0.40 (1)	4.69	O- B	-1506 / 0	0.44 (1)	
D- E	-2661 / 0	-91.8	-91.8	0.29 (1)	4.84	L- G	0 / 1106	0.25 (1)	
E- F	-2665 / 0	-91.8	-91.8	0.40 (1)	4.69	B- M	0 / 1099	0.25 (1)	
F- G	-2665 / 0	-91.8	-91.8	0.40 (1)	4.69	L- E	-628 / 0	0.48 (1)	
G- H	-1858 / 0	-91.8	-91.8	0.36 (1)	5.43	M- D	-626 / 0	0.48 (1)	
I- H	-1936 / 0	0.0	0.0	0.26 (1)	5.79	M- E	-6 / 0	0.00 (1)	
P- O	0 / 0	-18.5	-18.5	0.11 (4)	10.00				
O- N	0 / 1859	-18.5	-18.5	0.28 (1)	10.00				
N- M	0 / 1859	-18.5	-18.5	0.28 (1)	10.00				
M- L	0 / 2665	-18.5	-18.5	0.37 (1)	10.00				
L- K	0 / 1858	-18.5	-18.5	0.27 (1)	10.00				
K- J	0 / 1858	-18.5	-18.5	0.27 (1)	10.00				
J- I	0 / 0	-18.5	-18.5	0.11 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.40/1.00 (B-D:1) , BC=0.37/1.00 (L-M:1) , WB=0.56/1.00 (A-O:1) , SSI=0.25/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

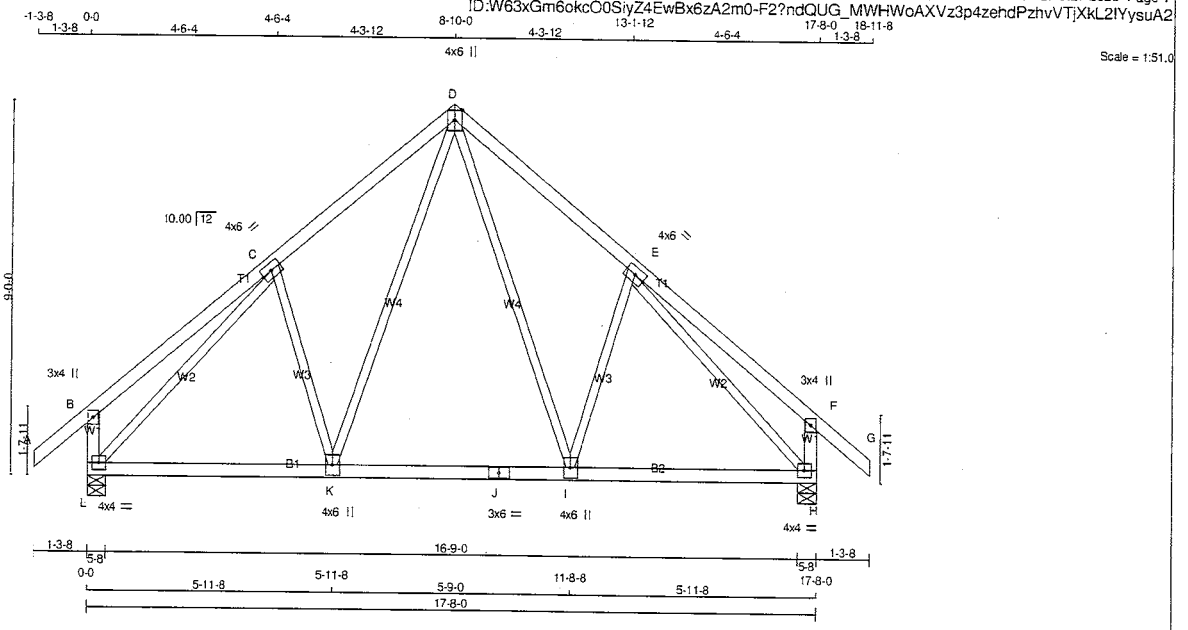
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (O) (INPUT = 0.90)
JSI METAL= 0.67 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410045	T75	4	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:40:27 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-F2?ndQUG_MWHWoAXVz3p4zehdPzhvVTjXkL2IYysuA2



TOTAL WEIGHT = 4 X 86 = 345 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW+t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1101	0	1101	0	0	5-8	5-8
H	1101	0	1101	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	776	523 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
H	776	523 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	D- I	0 / 370	0.08 (1)
B- C	0 / 31	-91.8	-91.8	0.29 (1)	10.00	I- E	-245 / 0	0.12 (1)
C- D	-825 / 0	-91.8	-91.8	0.23 (1)	6.25	K- D	0 / 370	0.08 (1)
D- E	-825 / 0	-91.8	-91.8	0.23 (1)	6.25	C- K	-245 / 0	0.12 (1)
E- F	0 / 31	-91.8	-91.8	0.29 (1)	10.00	L- C	-1036 / 0	0.81 (1)
F- G	0 / 41	-91.8	-91.8	0.13 (1)	10.00	E- H	-1036 / 0	0.81 (1)
L- B	-279 / 0	0.0	0.0	0.03 (1)	7.81			
H- F	-279 / 0	0.0	0.0	0.03 (1)	7.81			
L- K	0 / 686	-18.5	-18.5	0.20 (4)	10.00			
K- J	0 / 494	-18.5	-18.5	0.19 (4)	10.00			
J- I	0 / 494	-18.5	-18.5	0.19 (4)	10.00			
I- H	0 / 686	-18.5	-18.5	0.20 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.29/1.00 (E-F:1) , BC=0.20/1.00 (K-L:4) , WB=0.81/1.00 (C-L:1) , SSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR (PSI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (H) (INPUT = 0.90)
JSI METAL= 0.25 (E) (INPUT = 1.00)

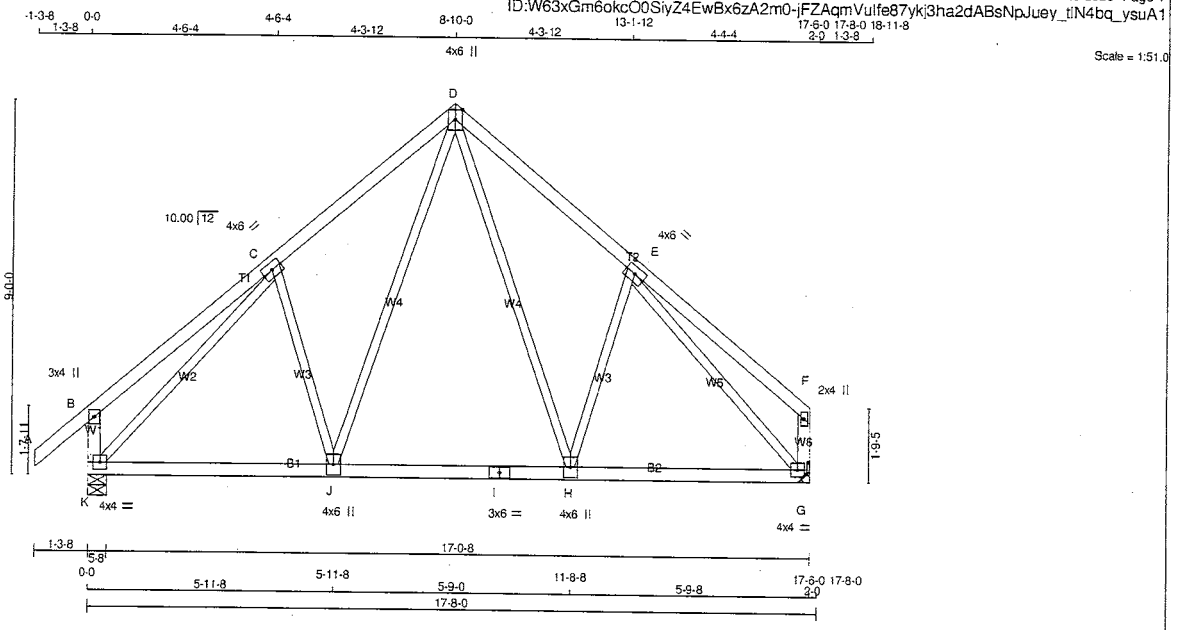
LICENSED PROFESSIONAL ENGINEER
07/31/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017030

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410045	T75A	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:40:28 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-jFZAqmVulfe87ykj3ha2dABsNpJuey_tN4bq_ysuA1



TOTAL WEIGHT = 3 X 84 = 252 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW+t	MT20	4.0	6.0		
K	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1084	0	1084	0	0	5-8	5-8
G	965	0	965	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	764	515 / 0	0 / 0	0 / 0	0 / 0	249 / 0	0 / 0
G	682	449 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00
B-C	0 / 31	-91.8	-91.8	0.29 (1)	10.00
C-D	-804 / 0	-91.8	-91.8	0.23 (1)	6.25
D-E	-780 / 0	-91.8	-91.8	0.22 (1)	6.25
E-F	0 / 42	-91.8	-91.8	0.28 (1)	10.00
K-B	-279 / 0	0.0	0.0	0.03 (1)	7.81
K-J	0 / 671	-18.5	-18.5	0.20 (4)	10.00
J-I	0 / 476	-18.5	-18.5	0.19 (4)	10.00
I-H	0 / 476	-18.5	-18.5	0.19 (4)	10.00
H-G	0 / 638	-18.5	-18.5	0.18 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D-H	0 / 319	0.07 (1)
H-E	-199 / 0	0.09 (1)
J-D	0 / 375	0.08 (1)
C-J	-249 / 0	0.12 (1)
K-C	-1013 / 0	0.79 (1)
G-F	-135 / 0	0.01 (1)
E-G	-1019 / 0	0.76 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	5.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.20/1.00 (J-K:4) , WB=0.79/1.00 (C-K:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

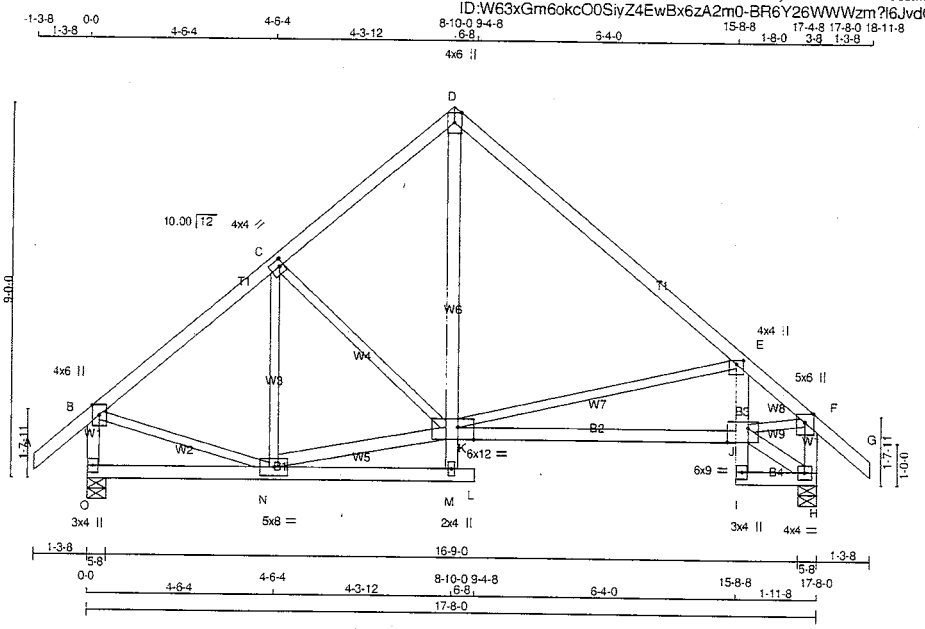
JSI GRIP= 0.79 (E) (INPUT = 0.90)
JSI METAL= 0.25 (C) (INPUT = 1.00)

Structural component only
DWG# T-2017031

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410045	T75S	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:40:29 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-BR6Y26WWWzm?l6JvdO5H9OkznCddNR10_1q9NQysuA0



TOTAL WEIGHT = 92 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
M - D	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-I	MT20	6.0	9.0	4.00	6.00
K	BWMWWW-I	MT20	6.0	12.0	Edge	4.50
M	BMW+w	MT20	2.0	4.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1102	0	1102	0	0	5-8	5-8
H	1112	0	1112	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	777	520 / 0	0 / 0	0 / 0	0 / 0	257 / 0	0 / 0
H	784	526 / 0	0 / 0	0 / 0	0 / 0	258 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-91.8	-91.8	0.13 (1)		10.00
B- C	-869 / 0	-91.8	-91.8	0.24 (1)		6.25
C- D	-765 / 0	-91.8	-91.8	0.24 (1)		6.25
D- E	-768 / 0	-91.8	-91.8	0.57 (1)		5.98
E- F	-1345 / 0	-91.8	-91.8	0.47 (1)		4.97
F- G	0 / 41	-91.8	-91.8	0.13 (1)		10.00
O- B	-1070 / 0	0.0	0.0	0.11 (1)		7.62
H- F	-1075 / 0	0.0	0.0	0.11 (1)		7.61
O- N	0 / 0	-18.5	-18.5	0.10 (4)		10.00
N- M	0 / 9	-18.5	-18.5	0.10 (4)		10.00
M- L	0 / 0	-18.5	-18.5	0.01 (4)		10.00
K- J	0 / 1146	-18.5	-18.5	0.36 (4)		10.00
I- J	0 / 19	0.0	0.0	0.05 (1)		10.00
J- E	-31 / 56	0.0	0.0	0.04 (4)		7.81
I- H	0 / 32	-18.5	-18.5	0.02 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-265 / 0	0.12 (1)
K- E	-604 / 0	0.61 (1)
B- N	0 / 722	0.16 (1)
J- H	-38 / 0	0.00 (1)
J- F	0 / 1130	0.25 (1)
M- K	0 / 47	0.02 (1)
K- D	0 / 527	0.10 (1)
N- K	0 / 709	0.11 (1)
C- K	-190 / 0	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
·OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.57/1.00 (D-E:1) , BC=0.36/1.00 (J-K:4) , WB=0.61/1.00 (E-K:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (F) (INPUT = 0.90)
JSI METAL= 0.49 (F) (INPUT = 1.00)

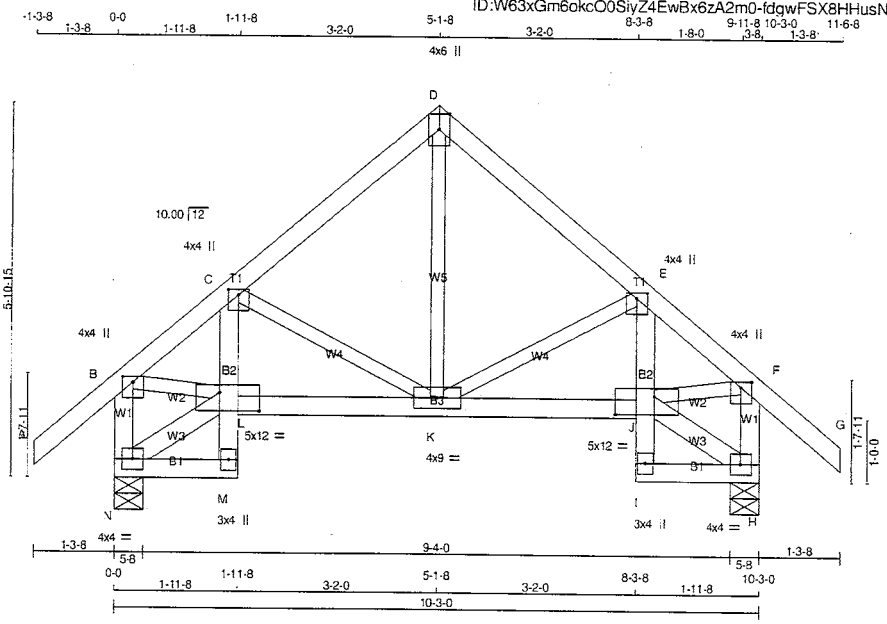
LICENSED PROFESSIONAL ENGINEER
07/31/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017032

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410045	T76S	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:40:30 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-fdqwFSX8HHusNGu6A6cWibGFSc1p60AADhZivtysuA?



TOTAL WEIGHT = 2 X 55 = 111 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, C, E, F						
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTW+p	MT20	4.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-I	MT20	5.0	12.0	3.50	7.50
K	BMWWW-t	MT20	4.0	9.0		
L	BVMWW-I	MT20	5.0	12.0	3.50	7.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	692	0	692	0	0	5-8	5-8
H	692	0	692	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	487	333 / 0	0 / 0	0 / 0	0 / 0	154 / 0	0 / 0
H	487	333 / 0	0 / 0	0 / 0	0 / 0	154 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	K- D	0 / 240	0.05 (1)
B- C	-626 / 0	-91.8	-91.8	0.10 (1)	6.25	K- E	-209 / 0	0.05 (1)
C- D	-406 / 0	-91.8	-91.8	0.12 (1)	6.25	C- K	-209 / 0	0.05 (1)
D- E	-406 / 0	-91.8	-91.8	0.12 (1)	6.25	N- L	-18 / 0	0.00 (1)
E- F	-626 / 0	-91.8	-91.8	0.10 (1)	6.25	B- L	0 / 478	0.11 (1)
F- G	0 / 41	-91.8	-91.8	0.13 (1)	10.00	J- H	-18 / 0	0.00 (1)
N- B	-666 / 0	0.0	0.0	0.07 (1)	7.81	J- F	0 / 478	0.11 (1)
H- F	-666 / 0	0.0	0.0	0.07 (1)	7.81			
N- M	0 / 15	-18.5	-18.5	0.02 (4)	10.00			
M- L	0 / 19	0.0	0.0	0.03 (1)	10.00			
L- C	0 / 31	0.0	0.0	0.02 (1)	10.00			
L- K	0 / 486	-18.5	-18.5	0.11 (1)	10.00			
K- J	0 / 486	-18.5	-18.5	0.11 (1)	10.00			
I- J	0 / 19	0.0	0.0	0.03 (1)	10.00			
J- E	0 / 31	0.0	0.0	0.02 (1)	10.00			
I- H	0 / 15	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.11/1.00 (J-K:1) , WB=0.11/1.00 (F-J:1) , SSI=0.10/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (B) (INPUT = 0.90)
JSI METAL= 0.19 (B) (INPUT = 1.00)

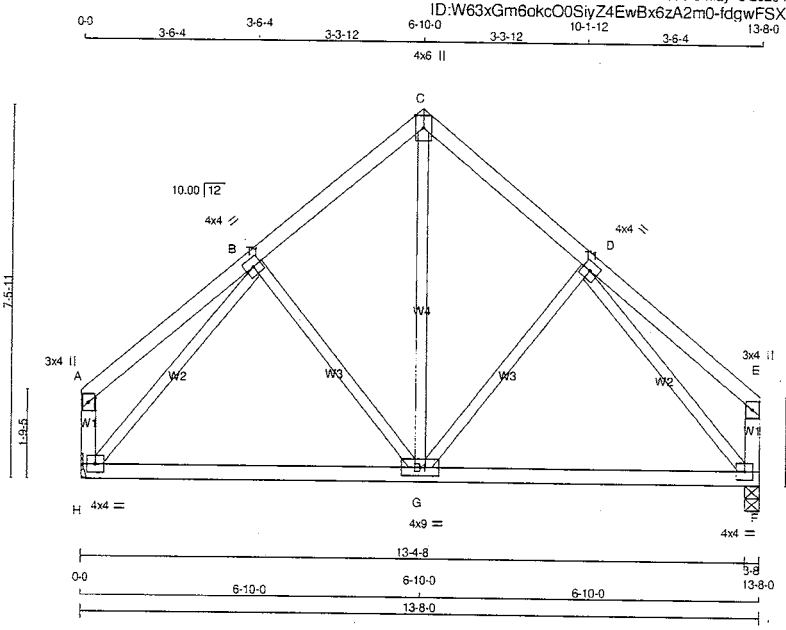
LICENSED PROFESSIONAL ENGINEER
07/31/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017033

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410045	T77A	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:40:30 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-fdgwFSX8HHusNGu6A6cWibGEmc_46yBADhZivtysuA?



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0		
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	753	0	753	0	0	MECHANICAL	
F	753	0	753	0	0	3-8	3-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	533	350 / 0	0 / 0	0 / 0	0 / 0	183 / 0	0 / 0
F	533	350 / 0	0 / 0	0 / 0	0 / 0	183 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 23	-91.8	-91.8	0.17 (1)	10.00	G- C	0 / 380	0.09 (1)
B-C	-511 / 0	-91.8	-91.8	0.13 (1)	6.25	G- D	-144 / 0	0.07 (1)
C-D	-511 / 0	-91.8	-91.8	0.13 (1)	6.25	B- G	-144 / 0	0.07 (1)
D-E	0 / 23	-91.8	-91.8	0.17 (1)	10.00	H- B	-748 / 0	0.36 (1)
H-A	-119 / 0	0.0	0.0	0.01 (1)	7.81	D- F	748 / 0	0.36 (1)
F-E	-119 / 0	0.0	0.0	0.01 (1)	7.81			
H-G	0 / 465	-18.5	-18.5	0.28 (4)	10.00			
G-F	0 / 465	-18.5	-18.5	0.28 (4)	10.00			

TOTAL WEIGHT = 2 X 62 = 123 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	5.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.17/1.00 (D-E:1) , BC=0.28/1.00 (G-H:4) , WB=0.36/1.00 (D-F:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

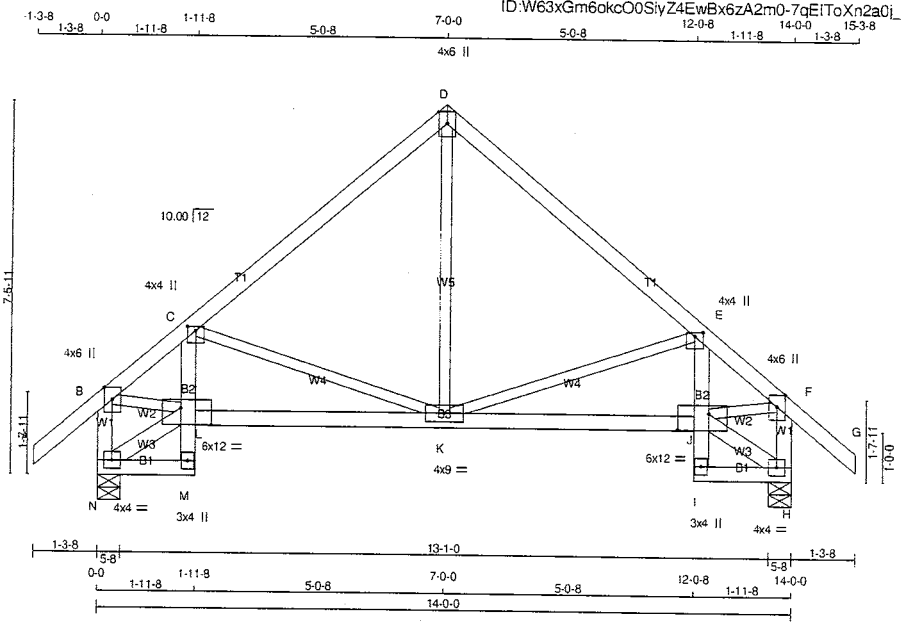
JSI GRIP= 0.83 (B) (INPUT = 0.90)
JSI METAL= 0.25 (D) (INPUT = 1.00)

Structural component only
DWG# T-2017034

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410045	T77S	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:40:31 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-7qEIToXn2a0i_QTIkp7IFppNV0LVrR5JSLJFRJysuA_



TOTAL WEIGHT = 69 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-l	MT20	6.0	12.0	4.00	7.50
K	BMWWW-t	MT20	4.0	9.0		
L	BVMWW-l	MT20	6.0	12.0	4.00	7.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2017035

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	899	0	899	0	0	5-8	5-8
H	899	0	899	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	633	429 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0
H	633	429 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B		0 / 41	-91.8	-91.8	0.13 (1)	10.00
B-C		-967 / 0	-91.8	-91.8	0.23 (1)	6.05
C-D		-593 / 0	-91.8	-91.8	0.30 (1)	6.25
D-E		-593 / 0	-91.8	-91.8	0.30 (1)	6.25
E-F		-967 / 0	-91.8	-91.8	0.23 (1)	6.05
F-G		0 / 41	-91.8	-91.8	0.13 (1)	10.00
N-B		-868 / 0	0.0	0.0	0.09 (1)	7.81
H-F		-868 / 0	0.0	0.0	0.09 (1)	7.81
N-M		0 / 23	-18.5	-18.5	0.02 (4)	10.00
M-L		0 / 19	0.0	0.0	0.04 (1)	10.00
L-C		-21 / 35	0.0	0.0	0.03 (1)	7.81
L-K		0 / 791	-18.5	-18.5	0.21 (1)	10.00
K-J		0 / 791	-18.5	-18.5	0.21 (1)	10.00
I-J		0 / 19	0.0	0.0	0.04 (1)	10.00
J-E		-21 / 35	0.0	0.0	0.03 (1)	7.81
I-H		0 / 23	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-D		0 / 331	0.07 (1)
K-E		-377 / 0	0.19 (1)
C-K		-377 / 0	0.19 (1)
N-L		-27 / 0	0.00 (1)
B-L		0 / 780	0.18 (1)
J-H		-27 / 0	0.00 (1)
J-F		0 / 780	0.18 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.30/1.00 (D-E:1) , BC=0.21/1.00 (J-K:1) , WB=0.19/1.00 (E-K:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

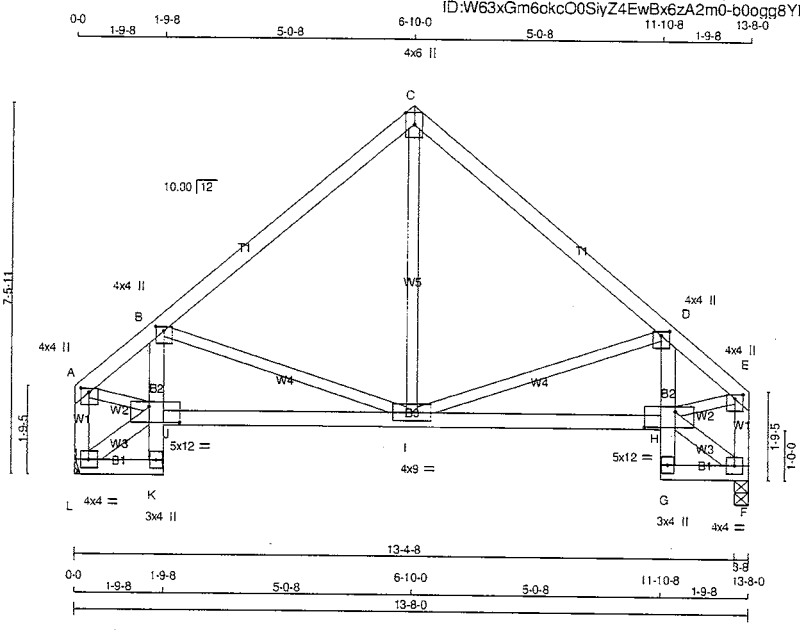
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (F) (INPUT = 0.90)
JSI METAL= 0.46 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410045	T77SA	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:40:32 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-b0ogg8YPpu8aca2UIXe_n0MYPQqkauKSq?2pyiysu9z



TOTAL WEIGHT = 64 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A, B, D, E						
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
F	BMVW1-t	MT20	4.0	4.0		
G	BMV+p	MT20	3.0	4.0		
H	BVMWW-I	MT20	5.0	12.0	3.75	7.50
I	BMWWW-t	MT20	4.0	9.0		
J	BVMWW-I	MT20	5.0	12.0	3.75	7.50
K	BMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	753	0	753	0	0	MECHANICAL	
F	753	0	753	0	0	3-8	3-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	533	350 / 0	0 / 0	0 / 0	0 / 0	183 / 0	0 / 0
F	533	350 / 0	0 / 0	0 / 0	0 / 0	183 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-919 / 0	-91.8	-91.8	0.24 (1)	6.14
B- C	-586 / 0	-91.8	-91.8	0.29 (1)	6.25
C- D	-586 / 0	-91.8	-91.8	0.29 (1)	6.25
D- E	-919 / 0	-91.8	-91.8	0.24 (1)	6.14
L- A	-724 / 0	0.0	0.0	0.08 (1)	7.81
F- E	-724 / 0	0.0	0.0	0.08 (1)	7.81
L- K	0 / 21	-18.5	-18.5	0.02 (4)	10.00
K- J	0 / 17	0.0	0.0	0.03 (1)	10.00
J- B	-82 / 16	0.0	0.0	0.02 (1)	7.81
J- I	0 / 787	-18.5	-18.5	0.21 (1)	10.00
I- H	0 / 787	-18.5	-18.5	0.21 (1)	10.00
G- H	0 / 17	0.0	0.0	0.03 (1)	10.00
H- D	-82 / 16	0.0	0.0	0.02 (1)	7.81
G- F	0 / 21	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	0 / 326	0.07 (1)
I- D	-379 / 0	0.19 (1)
B- I	-379 / 0	0.19 (1)
L- J	-25 / 0	0.00 (1)
A- J	0 / 786	0.18 (1)
H- F	-25 / 0	0.00 (1)
H- E	0 / 786	0.18 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.29/1.00 (C-D:1) , BC=0.21/1.00 (H-I:1) , WB=0.19/1.00 (D-I:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

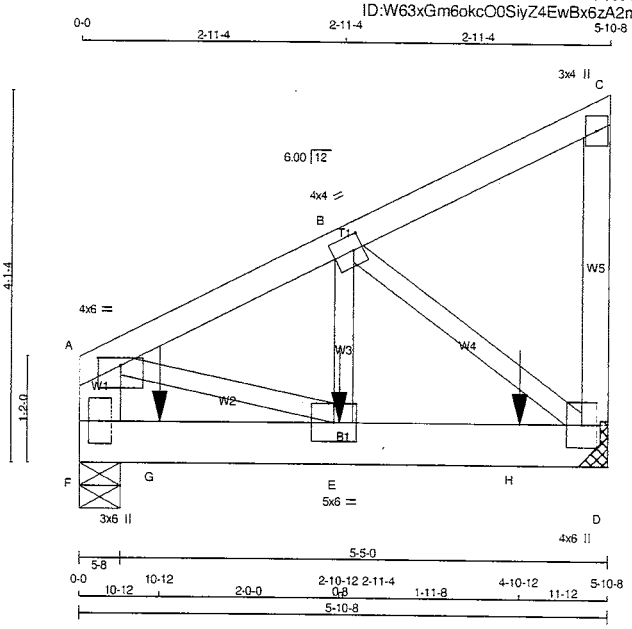
JSI GRIP= 0.88 (A) (INPUT = 0.90)
JSI METAL= 0.24 (A) (INPUT = 1.00)

Structural component only
DWG# T-2017036

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410045	T79	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:40:33 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-4CM3uUZ1aCGREjdhsEADKEunjq0YJM?cvfoMVCysu9y



TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	6	SIDE(8.6)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		

LICENSED PROFESSIONAL ENGINEER
07/31/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017037 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1437	0	1437	0	0	5-8	5-8
D	1416	0	1416	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1014	680 / 0	0 / 0	0 / 0	0 / 0	334 / 0	0 / 0
D	999	670 / 0	0 / 0	0 / 0	0 / 0	329 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-957 / 0	0.0	0.0	0.03 (1)	7.81	A- E	0 / 1147	0.14 (1)
A- B	-1229 / 0	-91.8	-91.8	0.06 (1)	6.25	E- B	0 / 1082	0.13 (1)
B- C	-11 / 0	-91.8	-91.8	0.05 (1)	6.25	B- D	-1395 / 0	0.17 (1)
D- C	-110 / 0	0.0	0.0	0.01 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.16 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.16 (1)	10.00			
E- H	0 / 1109	-18.5	-18.5	0.23 (1)	10.00			
H- D	0 / 1109	-18.5	-18.5	0.23 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-10-12	-735	-735	---	FRONT	VERT	TOTAL	---	C1
G	10-12	-736	-736	---	FRONT	VERT	TOTAL	---	C1
H	4-10-12	-735	-735	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.23/1.00 (D-E:1) , WB=0.17/1.00 (B-D:1) , SSI=0.16/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (B) (INPUT = 0.90)
JSI METAL= 0.25 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410045	T79	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

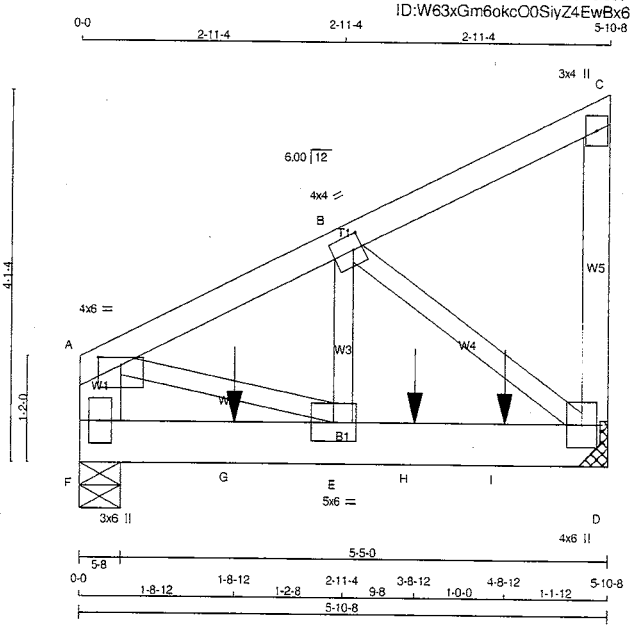
LICENSED PROFESSIONAL ENGINEER
01/31/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017037

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410045	T79Z	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:40:34 2020 Page 1
ID:W63xGm6okcC0SiyZ4EwBx6zA2m0-YPwR5pafLVOIrtCtPxhSsRRxNDJn2otl8JXv1eysu9x



TOTAL WEIGHT = 2 X 29 = 58 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2017038

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1672	0	1672	0	0	5-8	5-8
D	2171	0	2171	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1178	802 / 0	0 / 0	0 / 0	0 / 0	375 / 0	0 / 0
D	1528	1043 / 0	0 / 0	0 / 0	0 / 0	485 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1411 / 0	0.0	0.0	0.05 (1)	7.81	A- E	0 / 1755	0.22 (1)
A- B	-1889 / 0	-91.8	-91.8	0.07 (1)	6.25	E- B	0 / 1835	0.23 (1)
B- C	-9 / 0	-91.8	-91.8	0.05 (1)	10.00	B- D	-2136 / 0	0.25 (1)
D- C	-115 / 0	0.0	0.0	0.01 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.22 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.22 (1)	10.00			
E- H	0 / 1697	-18.5	-18.5	0.43 (1)	10.00			
H- I	0 / 1697	-18.5	-18.5	0.43 (1)	10.00			
I- D	0 / 1697	-18.5	-18.5	0.43 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-8-12	-1065	-1065	---	TOP	VERT	TOTAL	---	C1
H	3-8-12	-1065	-1065	---	TOP	VERT	TOTAL	---	C1
I	4-8-12	-1065	-1065	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.07/1.00 (A-B:1) , BC=0.43/1.00 (D-E:1) , WB=0.25/1.00 (B-D:1) , SSI=0.47/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

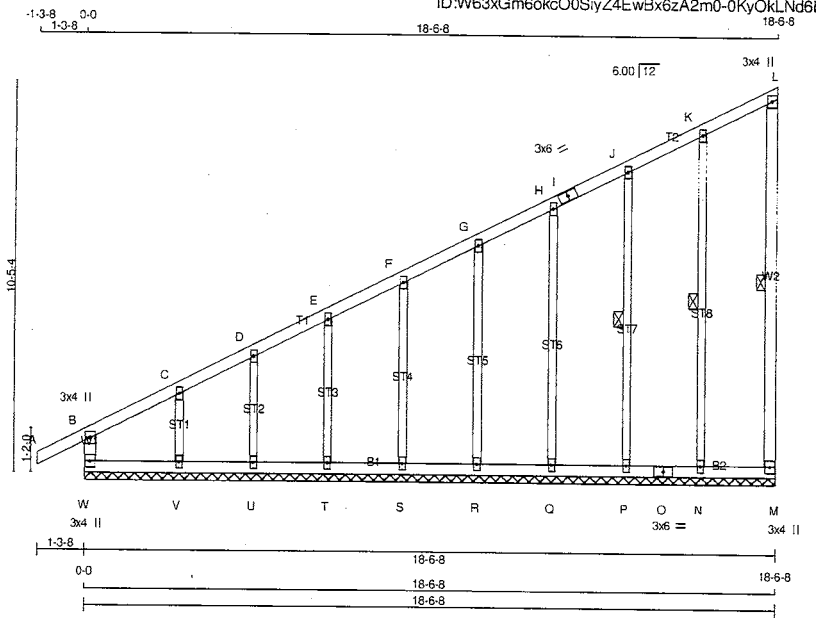
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.38 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410045	G99	4	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:40:18 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-0KyOkLNd6bNZwQ_oTaOhD3rnCbnyLla1PSqf3UZysuAB



TOTAL WEIGHT = 4 X 94 = 374 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
W - B	2x4	DRY	No.2	SPF
A - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
W - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C, D, E, F, G, H, J, K					
C	TMW+w	MT20	2.0	4.0	
I	TS-t	MT20	3.0	6.0	
L	TMV+p	MT20	3.0	4.0	
M	BMV1+p	MT20	3.0	4.0	
N, P, Q, R, S, T, U, V					
N	BMW1+w	MT20	2.0	4.0	
O	BS-t	MT20	3.0	6.0	
W	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF L-M, K-N, J-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- B	-255 / 0	0.0	0.0	0.01 (1)	7.81	N- K	-198 / 0	0.12 (1)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	P- J	-182 / 0	0.08 (1)
B- C	-28 / 0	-91.8	-91.8	0.07 (1)	6.25	Q- H	-183 / 0	0.18 (1)
C- D	-21 / 0	-91.8	-91.8	0.05 (1)	6.25	R- G	-183 / 0	0.12 (1)
D- E	-14 / 0	-91.8	-91.8	0.04 (1)	6.25	S- F	-182 / 0	0.08 (1)
E- F	-12 / 0	-91.8	-91.8	0.04 (1)	6.25	T- E	-183 / 0	0.05 (1)
F- G	-9 / 0	-91.8	-91.8	0.04 (1)	10.00	U- D	-176 / 0	0.04 (1)
G- H	-7 / 0	-91.8	-91.8	0.04 (1)	10.00	V- C	-201 / 0	0.03 (1)
H- I	-5 / 0	-91.8	-91.8	0.04 (1)	10.00			
I- J	-5 / 0	-91.8	-91.8	0.04 (1)	10.00			
J- K	-3 / 0	-91.8	-91.8	0.04 (1)	10.00			
K- L	-7 / 0	-91.8	-91.8	0.04 (1)	10.00			
M- L	-83 / 0	0.0	0.0	0.01 (1)	6.25			
W- V	0 / 23	-18.5	-18.5	0.02 (4)	10.00			
V- U	0 / 17	-18.5	-18.5	0.02 (4)	10.00			
U- T	0 / 13	-18.5	-18.5	0.02 (4)	10.00			
T- S	0 / 10	-18.5	-18.5	0.02 (4)	10.00			
S- R	0 / 8	-18.5	-18.5	0.01 (4)	10.00			
R- Q	0 / 6	-18.5	-18.5	0.01 (4)	10.00			
Q- P	0 / 4	-18.5	-18.5	0.01 (4)	10.00			
P- O	0 / 3	-18.5	-18.5	0.02 (4)	10.00			
O- N	0 / 3	-18.5	-18.5	0.02 (4)	10.00			
N- M	0 / 1	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (V-W:4) , WB=0.18/1.00 (H-Q:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

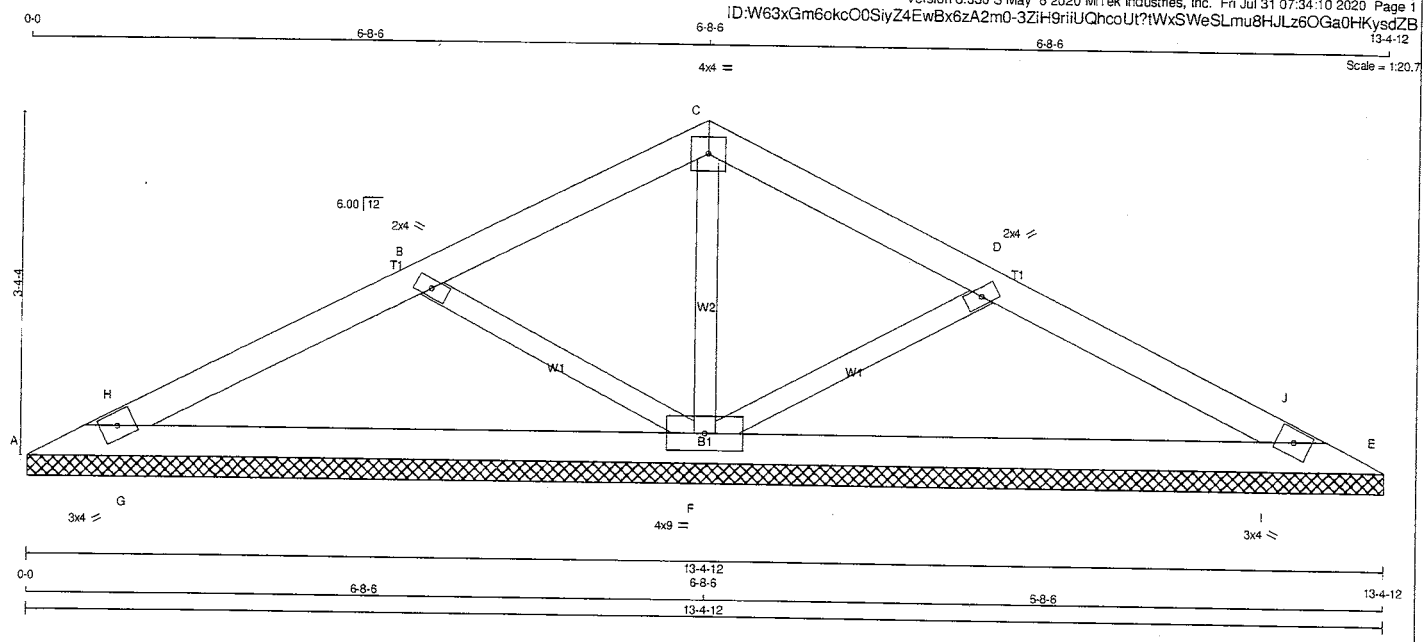
JSI GRIP= 0.38 (F) (INPUT = 0.90)
JSI METAL= 0.08 (C) (INPUT = 1.00)

Structural component only
DWG# T-2017021

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410045	P70	18	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Fri Jul 31 07:34:10 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-3ZiH9riiUQhcoUt?tWxSWeSLmu8HJLz6OGa0HKysdZB



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW+w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F	BMWWW1-t	MT20	4.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	137	0	137	0	0	13-4-12	13-4-12
E	137	0	137	0	0	13-4-12	13-4-12
F	1202	0	1202	0	0	13-4-12	13-4-12

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	96	70 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
E	96	70 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
F	853	545 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- H	0 / 123	-91.8	-91.8	0.12 (1)	10.00
H- B	0 / 184	-91.8	-91.8	0.20 (1)	10.00
B- C	0 / 511	-91.8	-91.8	0.25 (1)	10.00
C- D	0 / 511	-91.8	-91.8	0.25 (1)	10.00
D- J	0 / 184	-91.8	-91.8	0.20 (1)	10.00
J- E	0 / 123	-91.8	-91.8	0.12 (1)	10.00
A- G	-144 / 0	-18.5	-18.5	0.06 (1)	6.25
G- F	-137 / 0	-18.5	-18.5	0.18 (4)	6.25
F- I	-137 / 0	-18.5	-18.5	0.18 (4)	6.25
I- E	-144 / 0	-18.5	-18.5	0.06 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-641 / 0	0.12 (1)
B- F	-393 / 0	0.08 (1)
F- D	-393 / 0	0.08 (1)
G- H	-24 / 51	0.00 (1)
I- J	-24 / 51	0.00 (1)

TOTAL WEIGHT = 18 X 38 = 685 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.25/1.00 (B-C:1) , BC=0.18/1.00 (F-G:4) , WB=0.12/1.00 (C-F:1) , SSI=0.14/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (C) (INPUT = 0.90)
JSI METAL= 0.19 (C) (INPUT = 1.00)

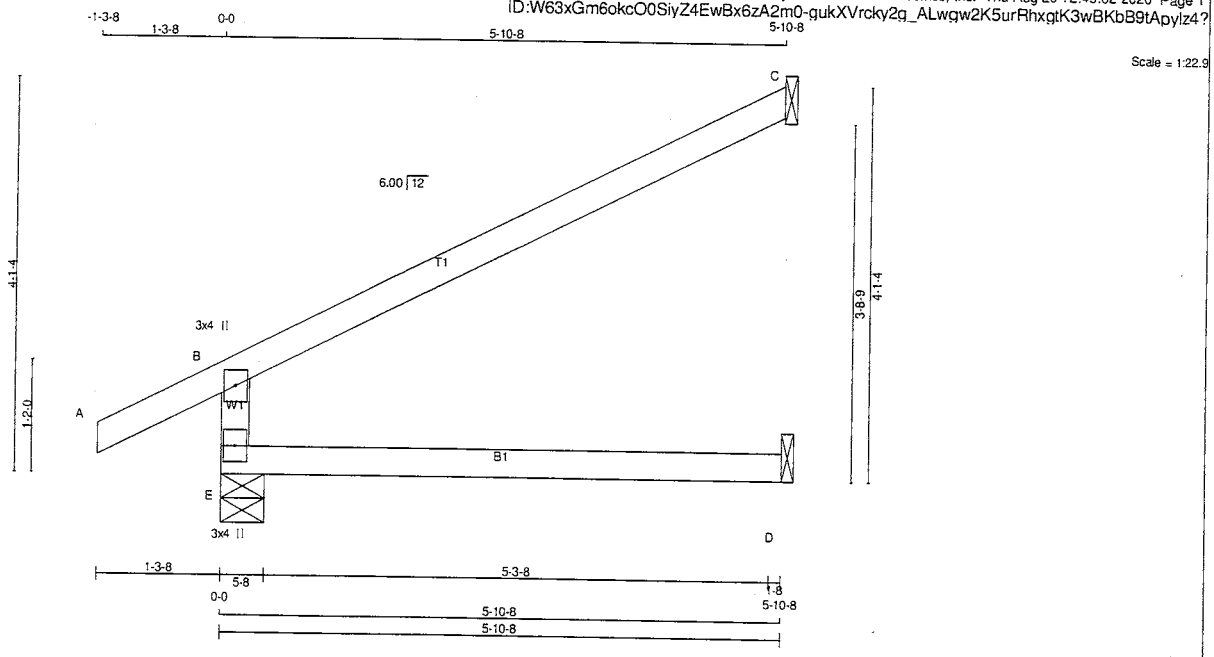
LICENSED PROFESSIONAL ENGINEER
07/31/20
H.J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017024

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410047	J1	16	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Aug 20 12:49:02 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-gukXVrcky2g_ALwgw2K5urRhxgtK3wBKbB9tApylz4?



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	525	0	525	0	0	5-8	5-8
C	202	0	202	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	369	257 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-461 / 0	0.0	0.0	0.13 (4)	7.81			
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B- C	-30 / 0	-91.8	-91.8	0.54 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

TOTAL WEIGHT = 16 X 17 = 269 lb

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

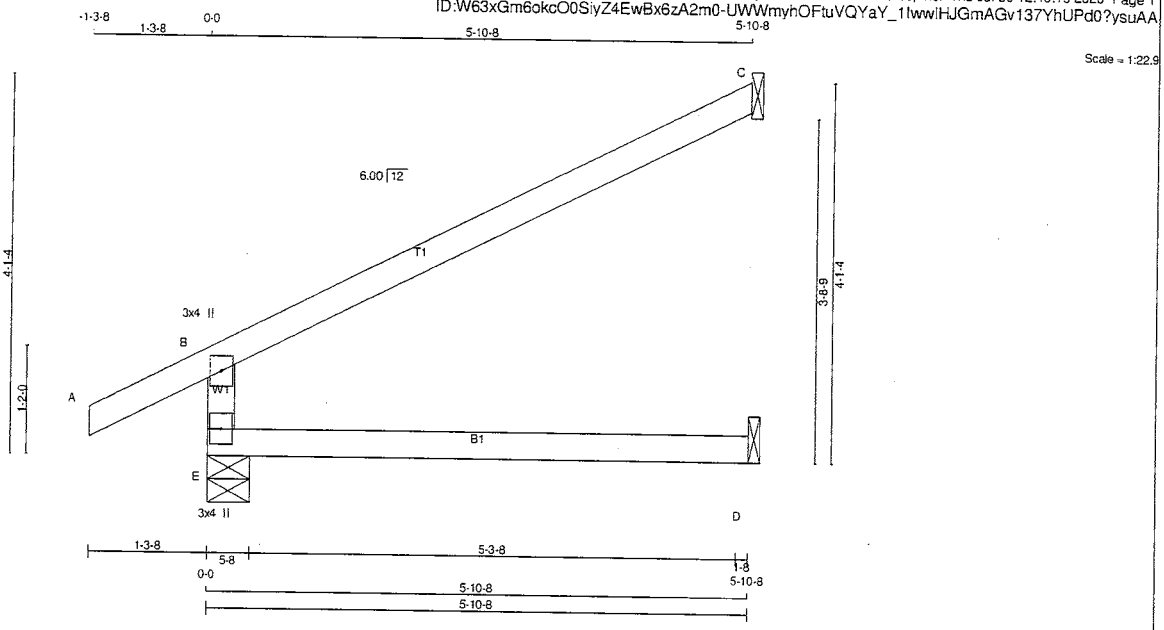
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018772

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410045	J1	21	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:40:19 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-UWWmyhOFtuVQYaY_1IwwIHJGmAGv137YhUPd0?ysuAA



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	525	0	525	0	0	5-8	5-8
C	202	0	202	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	369	257 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-461 / 0	0.0	0.0	0.13 (4)		7.81			
A-B	0 / 28	-91.8	-91.8	0.12 (1)		10.00			
B-C	-30 / 0	-91.8	-91.8	0.54 (1)		6.25			
E-D	0 / 0	-18.5	-18.5	0.13 (4)		10.00			

TOTAL WEIGHT = 21 X 17 = 353 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

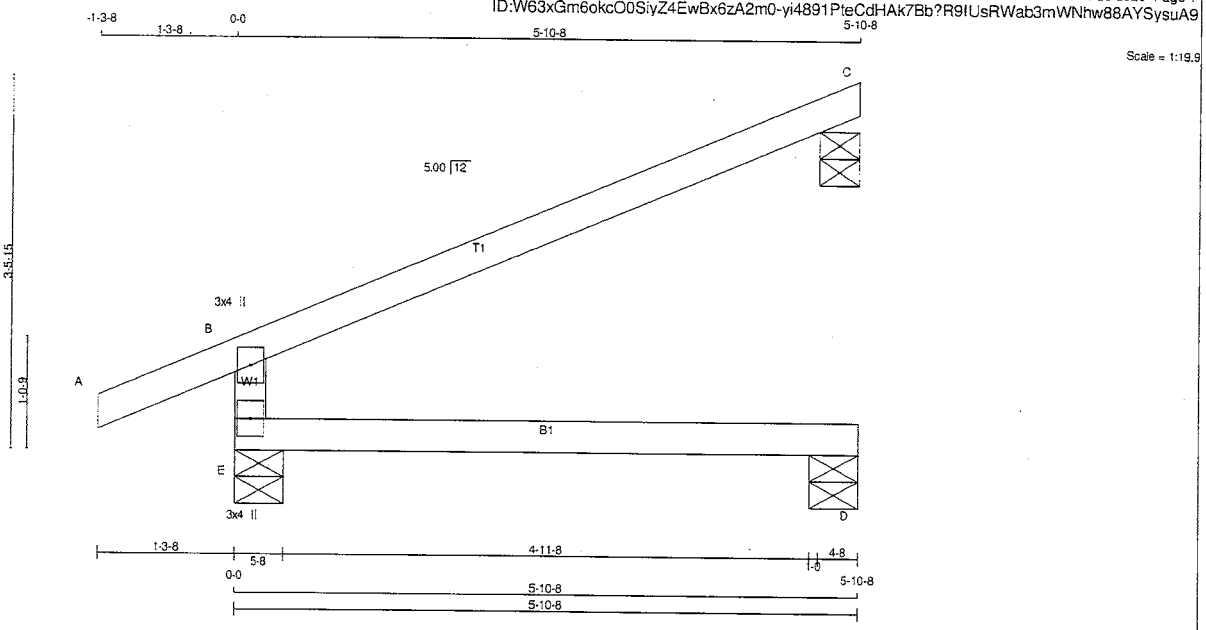
LICENSED PROFESSIONAL ENGINEER
07/30/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017022

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410045	J2	4	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:40:20 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-yi4891PteCdHAk7Bb?R9fUsRWab3mWNhw88AYSysuA9



TOTAL WEIGHT = 4 X 16 = 65 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	525	0	525	0	0	5-8	5-8
C	202	0	202	0	0	4-8	4-8
D	44	0	50	0	0	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	368	257 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-461 / 0	0.0	0.0	0.14 (4)	7.81			
A-B	0 / 24	-91.8	-91.8	0.12 (1)	10.00			
B-C	-26 / 0	-91.8	-91.8	0.54 (1)	6.25			
E-D	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.14/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

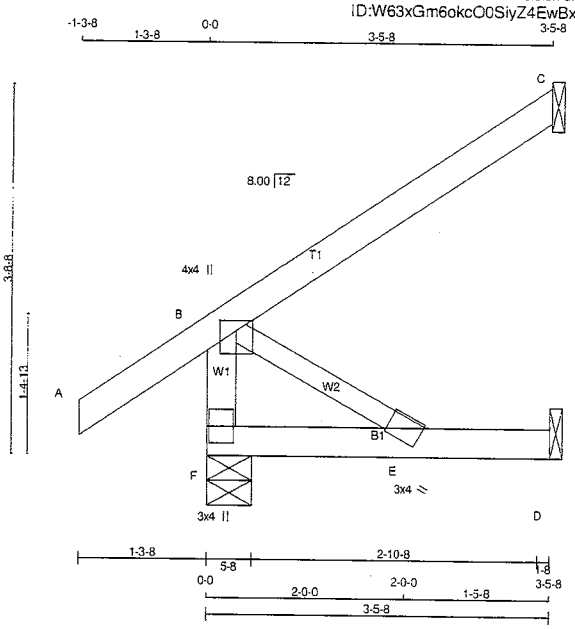
JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

Structural component only
DWG# T-2017023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410046	J3	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:53:12 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-LDj_B?i7qPuknN8Yp8HCbHqSDn5nfgUKRF91eiysu_5



TOTAL WEIGHT = 2 X 14 = 27 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	317	0	317	0	0	5-8	5-8
C	159	0	159	0	0	1-8	1-8
D	32	0	36	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	222	159 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0
C	109	88 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0
D	26	0 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-285 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.14 (5)	10.00			
B- C	0 / 0	-91.8	-91.8	0.19 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.06 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.06 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.19/1.00 (B-C:1) , BC=0.06/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

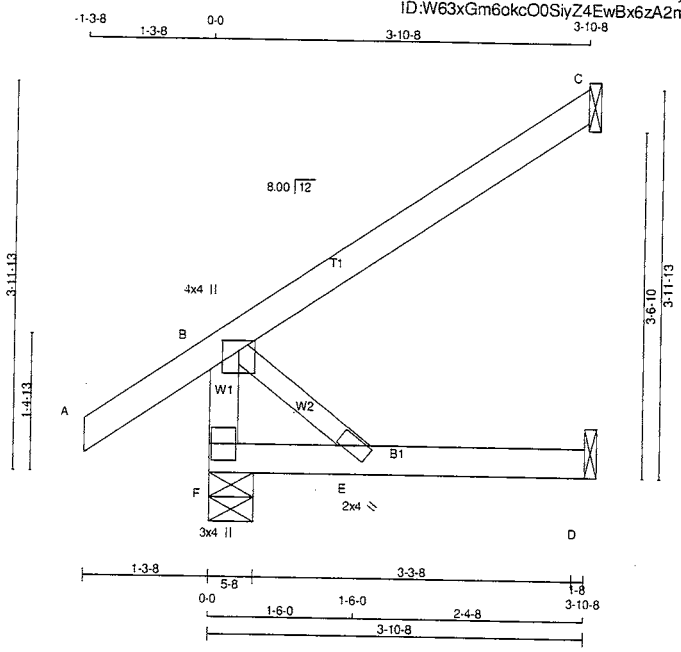
LICENSED PROFESSIONAL ENGINEER
07/31/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017044

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410046	J20	5	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Aug 20 12:32:13 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-inG5kqOqa1Z9dHlIJekUU38LLxW18aOpZiEzFoyIzJm



TOTAL WEIGHT = 5 X 14 = 71 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	340	0	340	0	0	5-8	5-8
C	178	0	178	0	0	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	238	169 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0
C	122	99 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-304 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.14 (5)	10.00			
B- C	0 / 0	-91.8	-91.8	0.23 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.08/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

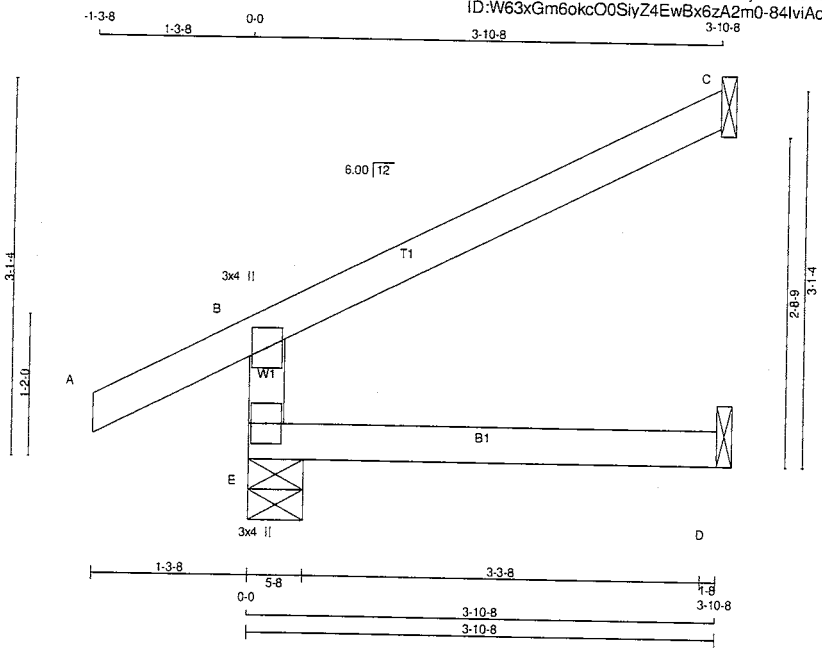
LICENSED PROFESSIONAL ENGINEER
06/20/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018770

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410047	J30	5	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Aug 20 12:49:03 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-84IviAdMjMoroUVsUmsKQ3zxT3FhoNRUpruQiGyIz4_



TOTAL WEIGHT = 5 X 12 = 60 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	388	0	388	0	0	5-8	5-8
C	133	0	133	0	0	1-8	1-8
D	30	0	34	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	272	193 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0
C	92	74 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	24	0 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
E-B	-347 / 0	0.0	0.0		0.05	(4)	7.81				
A-B	0 / 28	-91.8	-91.8		0.14	(5)	10.00				
B-C	-20 / 0	-91.8	-91.8		0.23	(1)	6.25				
E-D	0 / 0	-18.5	-18.5		0.06	(4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.06/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

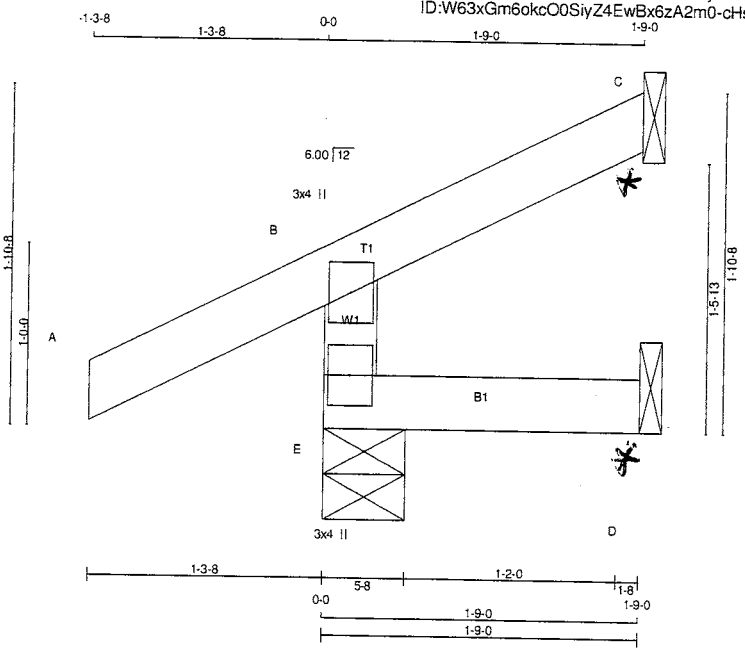
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018773

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410047	J31	4	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Aug 20 12:49:04 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-cHslwWd_UfxiQe432TNZzGW8?TbDXqhd2Ve_Diylz3z



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	269	0	269	0	0	5-8	5-8
C	43	0	43	0	-22	1-8	1-8
D	5	0	15	0	-5	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	187	141 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0
C	30	23 / -18	0 / 0	0 / 0	0 / 0	6 / 0	0 / 0
D	5	0 / -9	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-242	0	0.0	0.0	0.04 (5)	7.81			
A- B	0	28	-91.8	-91.8	0.12 (1)	10.00			
B- C	-17	0	-91.8	-91.8	0.09 (1)	6.25			
E- D	0	0	-18.5	-18.5	0.04 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 7 = 27 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.04/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

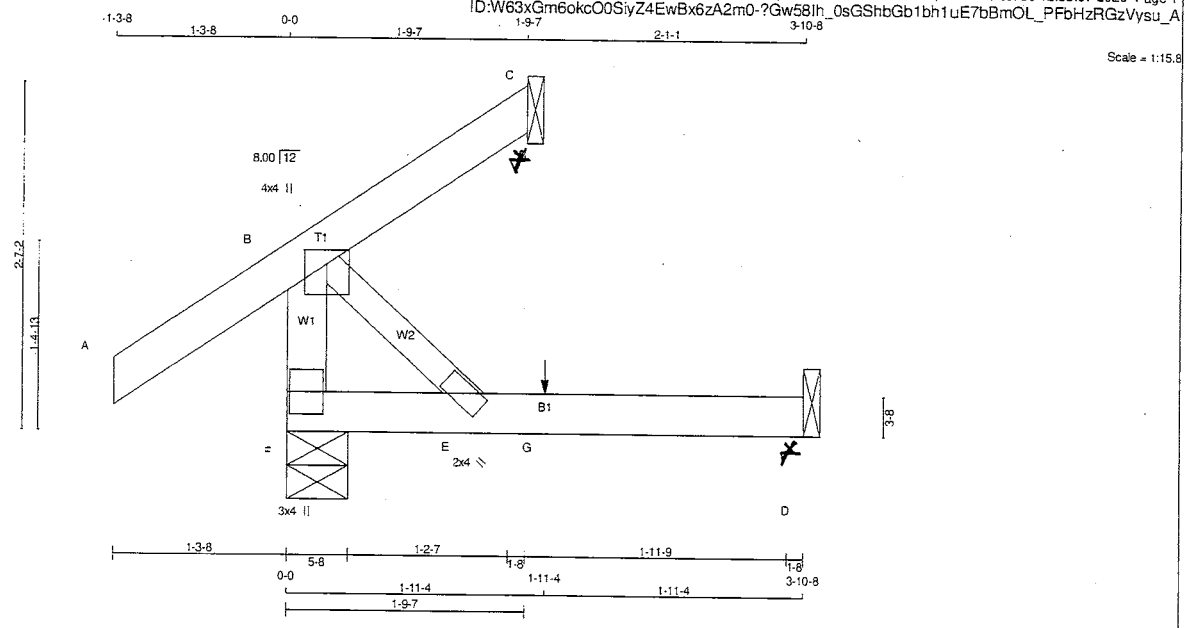
JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

Structural component only
DWG# T-2018774

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410046	C1	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:53:07 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-?Gw58Ih_0sGShbGb1bh1uE7bBmOL_PFbHzRGzVysu_A



TOTAL WEIGHT = 3 X 11 = 34 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	292	0	292	0	0	5-8	5-8
C	34	0	34	0	-39	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	205	143 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	23	19 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-256 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.14 (5)	10.00			
B- C	-27 / 0	-91.8	-91.8	0.13 (5)	6.25			
F- E	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
E- G	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
G- D	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14/1.00 (A-B:5) , BC=0.08/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

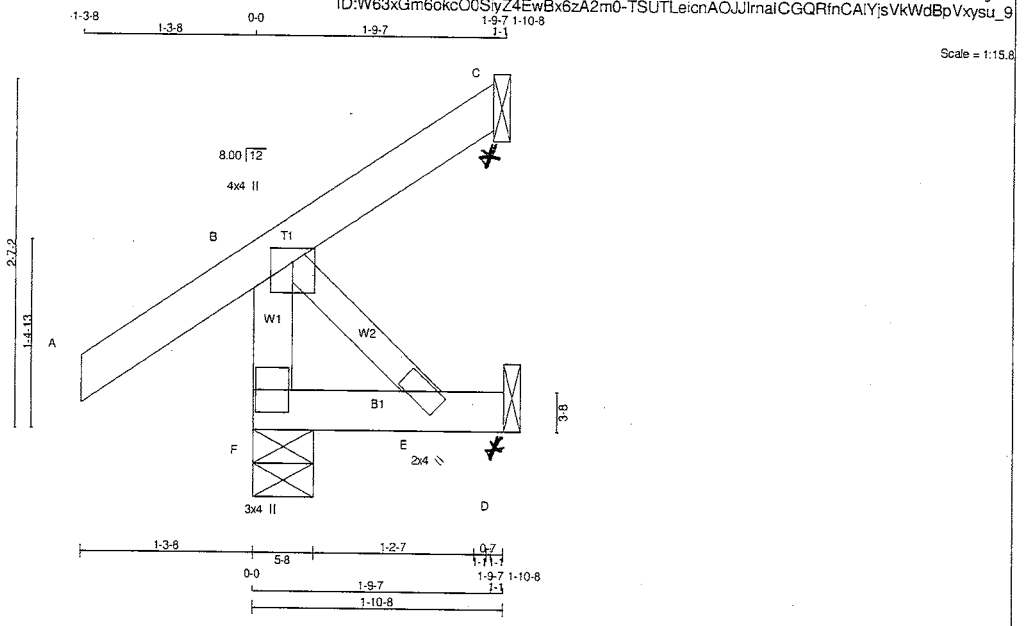
LICENSED PROFESSIONAL ENGINEER
07/31/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017039

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410046	C2	5	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:53:08 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-TSUTLeicnAOJJlrnaiCGQRfnCAtYjsVkWdBpVxysu_9



TOTAL WEIGHT = 5 X 9 = 44 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	274	0	274	0	0	5-8	5-8
C	34	0	34	0	-39	1-8	1-8
D	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	190	143 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	23	19 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-256 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00			
B- C	-27 / 0	-91.8	-91.8	0.12 (1)	6.25			
F- E	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

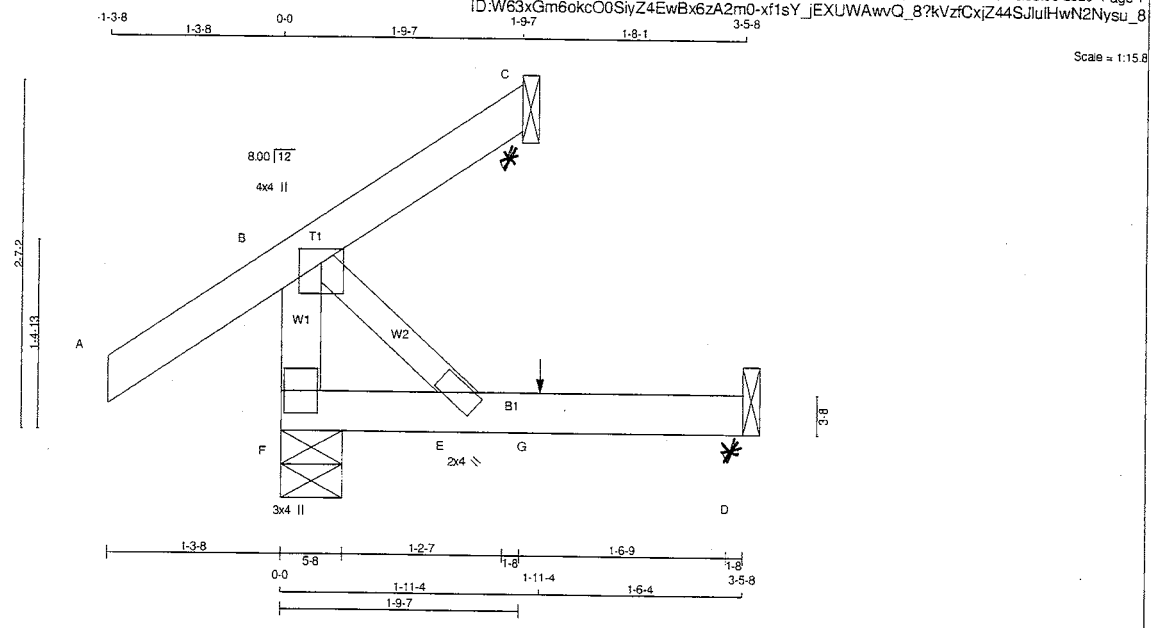
JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

Structural component only
DWG# T-2017040

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410046	C3	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Jul 30 12:53:09 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-xf1sY_jEXUWAwvQ_8?kVzfCxjZ44SJluIHwN2Nysu_8



TOTAL WEIGHT = 2 X 11 = 21 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	288	0	288	0	0	5-8	5-8
C	34	0	34	0	-39	1-8	1-8
D	32	0	36	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	202	143 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
C	23	19 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	26	0 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-256 / 0	0.0	0.0	0.03	(1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.14	(5)	10.00			
B- C	-27 / 0	-91.8	-91.8	0.13	(5)	6.25			
F- E	0 / 0	-18.5	-18.5	0.06	(4)	10.00			
E- G	0 / 0	-18.5	-18.5	0.06	(4)	10.00			
G- D	0 / 0	-18.5	-18.5	0.06	(4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14/1.00 (A-B:5) , BC=0.06/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

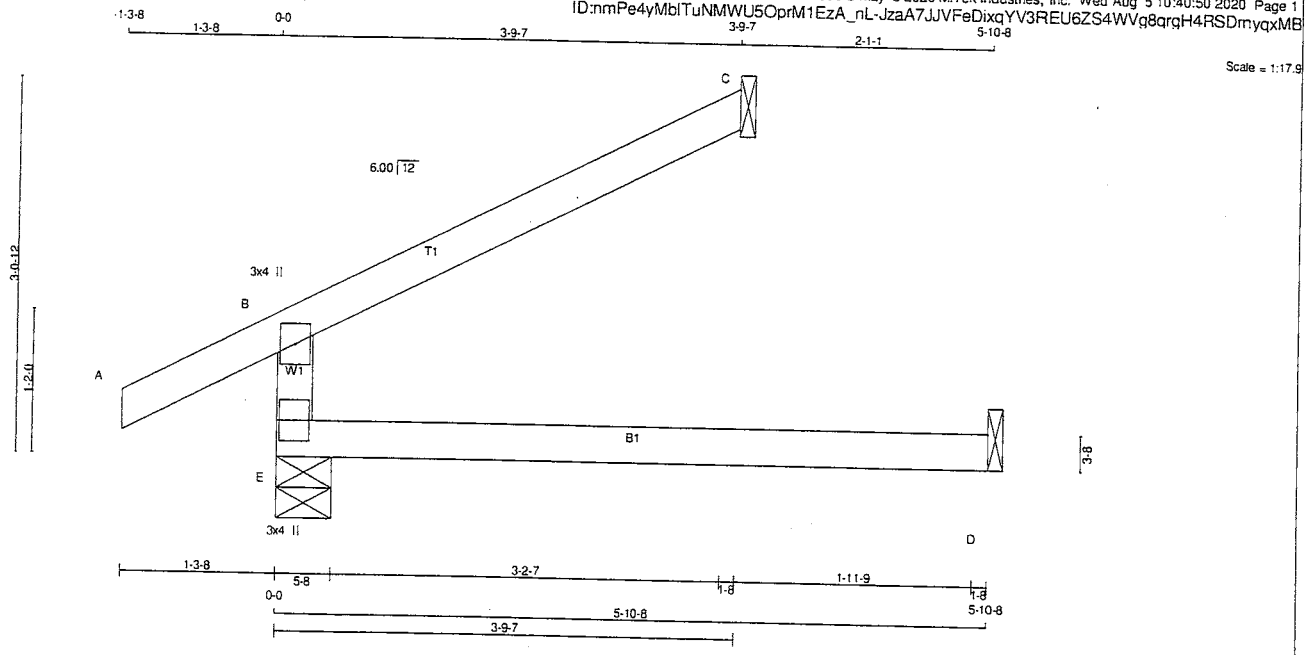
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017041

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	C30	4	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:40:50 2020 Page 1
ID:nmPe4yMblTuNMWU5OprM1EzA_nL-JzaA7JJVFeDixqYV3REU6ZS4WVg8qrgH4RSDmyqxMB



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	405	0	405	0	0	5-8	5-8
C	130	0	130	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	286	190 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-342 0	0.0	0.0	0.13 (4)	7.81			
A-B	0 28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-19 0	-91.8	-91.8	0.22 (1)	6.25			
E-D	0 0	-18.5	-18.5	0.13 (4)	10.00			

TOTAL WEIGHT = 4 X 14 = 57 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

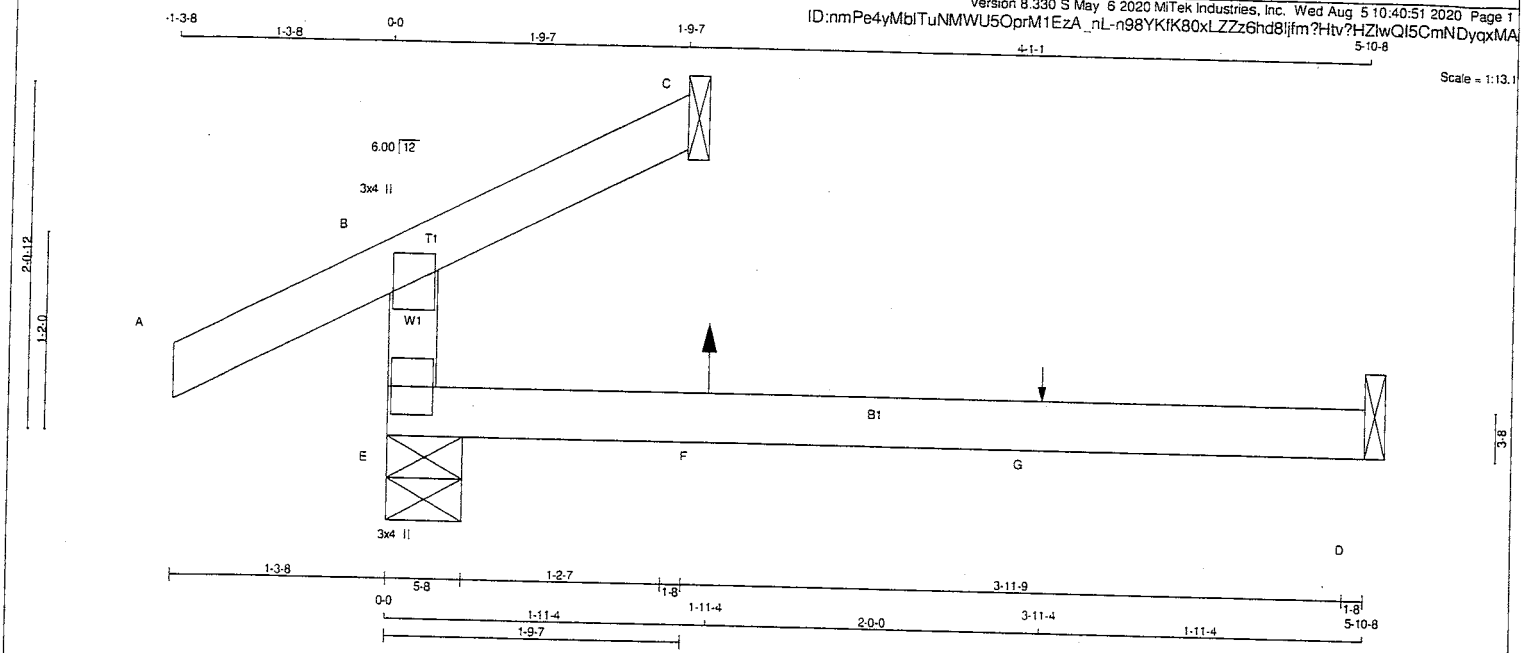
LICENSED PROFESSIONAL ENGINEER
08/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017359

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	C31	4	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:40:51 2020 Page 1
ID:nmPe4yMbITuNMWU5OprM1EzA_nL-n98YKfK80xLZZz6hd8ljfm?Htv?HZlwQI5CmNDyqxMA



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	284	0	284	0	0	5-8	5-8
C	63	0	63	0	0	1-8	1-8
D	44	0	52	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	200	137 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	46	21 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
D	35	0 / -3	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-227 0	0.0	0.0	0.11 (4)	7.81			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-9 / 9	-91.8	-91.8	0.08 (4)	10.00			
E-F	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
F-G	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
G-D	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	FRONT	VERT	TOTAL	--	C1
G	3-11-4	1	1	---	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 12 = 46 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.14/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

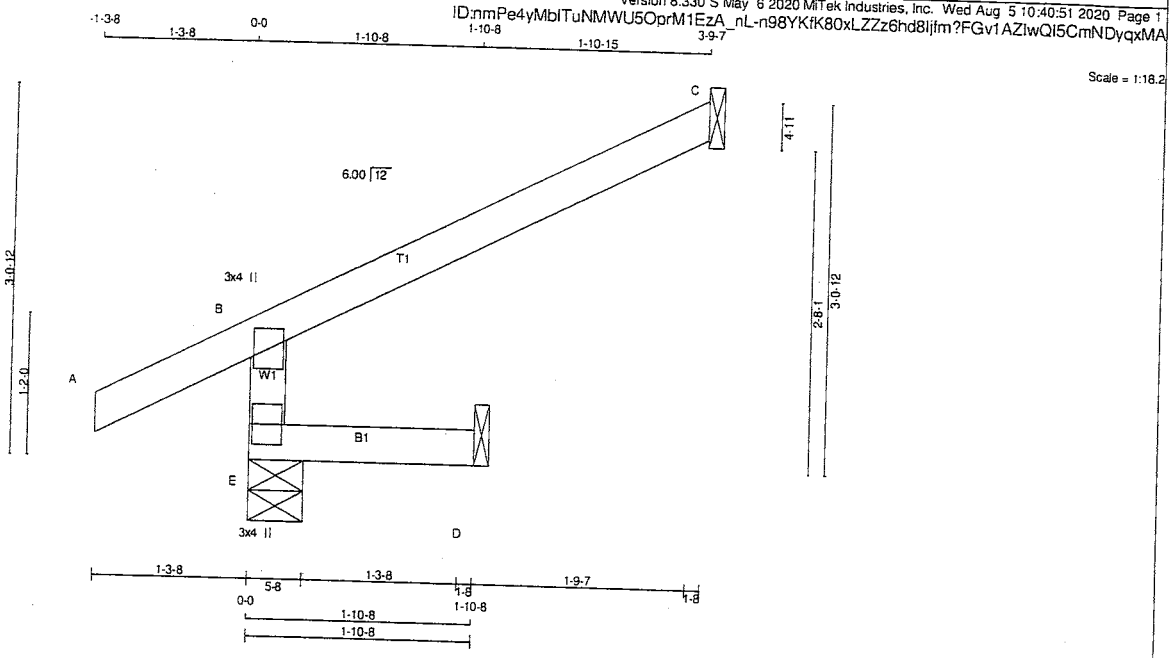
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017360

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	C32	4	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:40:51 2020 Page 1
ID:nmPe4yMblTuNMWU5OprM1EzA_nL-n98YKfK80xLZZz6hd8ljfm?FGv1AZIwQI5CmNDyqxMA



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 4 X 10 = 38 lb [M]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	361	0	361	0	0	5-8	5-8
C	130	0	130	0	0	1-8	1-8
D	16	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	250	190 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)
E- B	-342 0	0.0	0.0	0.01		(4)
A- B	0 / 28	-91.8	-91.8	0.13		(5)
B- C	-19 0	-91.8	-91.8	0.22		(1)
E- D	0 0	-18.5	-18.5	0.02		(4)

WEBS

MEMB. FR-TO	MAX. UNBRAC LENGTH	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
E- B	7.81			
A- B	10.00			
B- C	6.25			
E- D	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

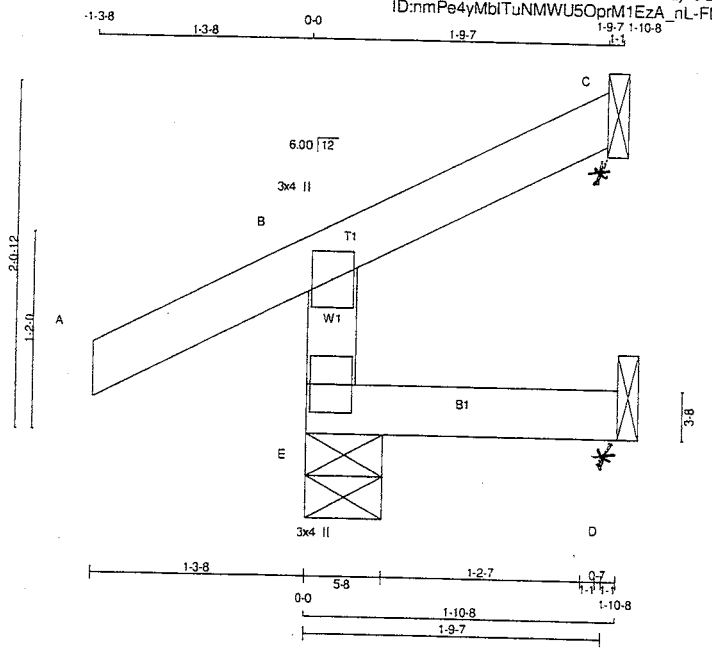
LICENSED PROFESSIONAL ENGINEER
08/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017361

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	C33	6	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Wed Aug 5 10:40:52 2020 Page 1
ID:nmPe4yMbITuNMWU5OprM1EzA_nL-FLiwY_LmnFTQB7htBsGyB_XSeJN4IIAZXlxKvgyqxM9



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	271	0	271	0	0	5-8	5-8
C	45	0	45	0	-23	1-8	1-8
D	8	0	17	0	-2	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	188	141 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	31	24 / -18	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	7	0 / -8	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH FR-TO	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-244 / 0	0.0	0.0	0.04 (5)	7.81			
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B- C	-17 / 0	-91.8	-91.8	0.09 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.04 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 6 X 7 = 42 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.04/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

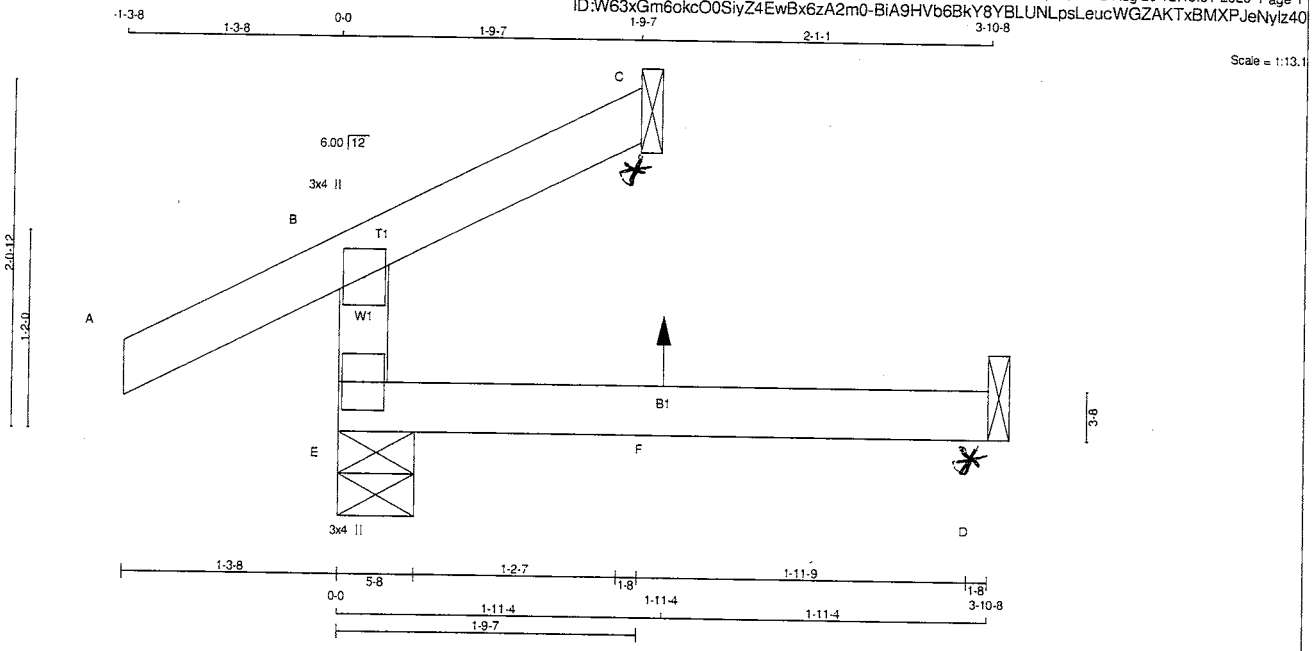
LICENSED PROFESSIONAL ENGINEER
08/05/10
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017362

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410047	C34	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Aug 20 12:49:01 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-BiA9HVb6BkY8YBLUNLpsLeucWGZAKTxBMXPJeNylz40



TOTAL WEIGHT = 3 X 9 = 28 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	279	0	279	0	0	5-8	5-8
C	49	0	49	0	-20	1-8	1-8
D	26	0	35	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	195	138 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
C	35	22 / -20	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
D	21	0 / -5	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (9)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
E- B	-241 / 0	0.0	0.0	0.05 (5)	7.81			
A- B	0 / 28	-91.8	-91.8	0.14 (5)	10.00			
B- C	-15 / 1	-91.8	-91.8	0.08 (5)	6.25			
E- F	0 / 0	-18.5	-18.5	0.06 (4)	10.00			
F- D	0 / 0	-18.5	-18.5	0.06 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14/1.00 (A-B:5) , BC=0.06/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.10/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018771

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

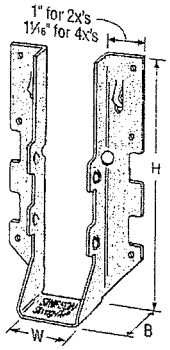
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

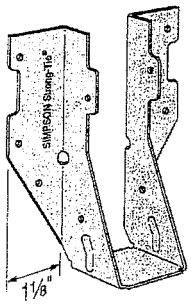
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

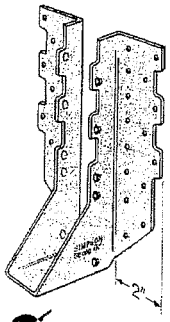
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



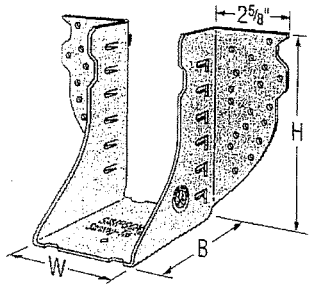
LUS28



LU26L

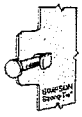


HUS210
(HUS26, HUS28, and HHUS similar)

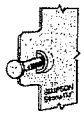


HGUS28-2

Double-Shear Nailing Top View

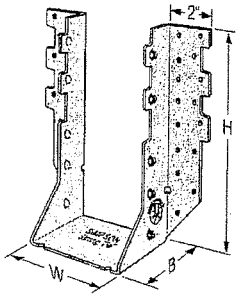


Double-Shear Nailing Side View; Do not bend tab



Dome Double-Shear Nailing Side View *(available on some models)*

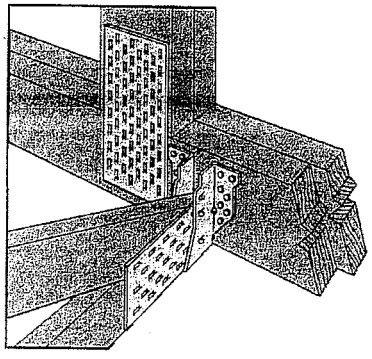
U.S. Patent 5,603,580

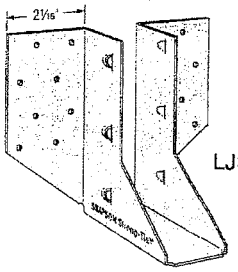


HHUS210-2

Typical HUS26 Installation with Reduced Heel Height

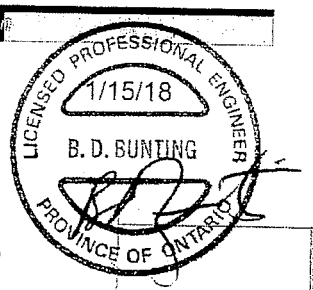
(Truss Designer to provide fastener quantity for connecting multiple members together)





LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS



HHUS/HGUS

See Hanger Options information on pp. 125–127.

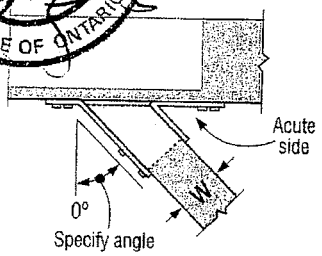
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Single 2x Sizes											
LUS24	18	1 9/16	3 1/8	1 3/4	2 1/4	(4) 10d	(2) 10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
LU24L	22	1 9/16	3	1 5/8	2 11/16	(4) 10d	(2) 10d x 1 1/2"	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
LU26L	22	1 9/16	5	1 5/8	4 5/8	(6) 10d	(4) 10d x 1 1/2"	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
LUS26	18	1 9/16	4 3/4	1 3/4	3 3/4	(4) 10d	(4) 10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
HUS26	16	1 5/8	5 3/8	3	3 15/16	(14) 16d	(6) 16d	2705 / 11.30	4940 / 21.97	2065 / 9.20	3875 / 17.24
LJS26DS	18	1 9/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055 / 9.14	4265 / 18.97	1460 / 6.49	4115 / 18.31
HGUS26	12	1 5/8	5 3/8	5	4 1/8	(20) 16d	(8) 16d	2685 / 11.96	6625 / 29.51	2685 / 11.96	5700 / 25.35
LU28L	20	1 9/16	6 3/4	1 5/8	5 7/8	(8) 10d	(6) 10d x 1 1/2"	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(4) 10d	1420 / 6.32	2520 / 11.21	1290 / 5.74	1790 / 7.96
HUS28	16	1 5/8	7 1/16	3	6 1/16	(22) 16d	(8) 16d	3605 / 16.04	5365 / 23.86	2675 / 11.90	4345 / 19.33
HGUS28	12	1 5/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310 / 14.74	7675 / 34.19	3310 / 14.74	6900 / 30.73
LU210L	20	1 9/16	8	1 5/8	7 3/8	(10) 10d	(6) 10d x 1 1/2"	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420 / 6.32	2785 / 12.39	1290 / 5.74	2210 / 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. **Nails:** 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

SIMPSON Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Double 2x Sizes											
LUS24-2	18	3⅛	3⅛	2	1½	(4) 16d	(2) 16d	835 3.71	2020 8.99	590 2.62	1435 6.38
SS LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720 7.65	2595 11.54	1545 6.87	1920 8.54
HHUS26-2	14	3⁵⁄₁₆	5⅜	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2850 12.68	7335 32.63	2065 9.20	5205 23.15
HGUS26-2	12	3⁵⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
SS LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720 7.65	3325 14.79	1545 6.87	2575 11.45
HHUS28-2	14	3⁵⁄₁₆	7⁵⁄₁₆	3	6⅛	(22) 16d	(8) 16d	3765 16.75	8940 39.77	2675 11.90	6345 28.22
HGUS28-2	12	3⁵⁄₁₆	7⅜	4	6⅛	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
SS LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580 11.48	4500 20.02	2320 10.32	3195 14.21
HHUS210-2	14	3⁵⁄₁₆	9⅜	3	8	(30) 16d	(10) 16d	4670 20.77	9660 42.97	4235 18.84	7000 31.14
HGUS210-2	12	3⁵⁄₁₆	9⅜	4	8⅛	(46) 16d	(16) 16d	6840 30.43	14015 62.34	4855 21.60	10270 45.69
Triple 2x Sizes											
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅛	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
HHUS210-3	14	4¹³⁄₁₆	9	3	7¹⁵⁄₁₆	(30) 16d	(10) 16d	4670 20.77	9670 43.02	4235 18.84	6865 30.54
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	(46) 16d	(16) 16d	6840 30.43	14645 65.14	4855 21.60	10400 46.26
Quadruple 2x Sizes											
HGUS26-4	12	6⁹⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
HGUS28-4	12	6⁹⁄₁₆	7⅜	4	6⅛	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
HHUS210-4	14	6⅛	8⅞	3	7¹³⁄₁₆	(30) 16d	(10) 16d	4670 20.77	10155 45.17	4235 18.84	7210 32.07
HGUS210-4	12	6⁹⁄₁₆	9⅜	4	8⅛	(46) 16d	(16) 16d	6840 30.43	14645 65.14	4855 21.60	10400 46.26
HGUS212-4	12	6⁹⁄₁₆	10⅝	4	10⅛	(56) 16d	(20) 16d	7640 33.98	14995 66.70	5425 24.13	10645 47.35
HGUS214-4	12	6⁹⁄₁₆	12⅝	4	11⅛	(66) 16d	(22) 16d	10130 45.06	16400 72.95	7195 32.00	11645 51.80
4x Sizes											
LUS46	18	3⁹⁄₁₆	4¾	2	3⁷⁄₁₆	(4) 16d	(4) 16d	1720 7.65	2595 11.54	1545 6.87	1920 8.54
HHUS46	14	3⅝	5¼	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2540 11.30	7335 32.63	2065 9.20	5205 23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
LUS48	18	3⁹⁄₁₆	6¾	2	3⁷⁄₁₆	(6) 16d	(4) 16d	1720 7.65	3325 14.79	1545 6.87	2575 11.45
HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 16.75	8940 39.77	2675 11.90	6345 28.22
HGUS48	12	3⅝	7¼	4	6⅛	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
LUS410	18	3⁹⁄₁₆	8¾	2	5⁹⁄₁₆	(8) 16d	(6) 16d	2580 11.48	4500 20.02	2320 10.32	3195 14.21
HGUS410	12	3⅝	9	4	8⅛	(46) 16d	(16) 16d	6840 30.43	14015 62.34	4855 21.60	10270 45.69
HGUS412	12	3⅝	10⅞	4	10⅛	(56) 16d	(20) 16d	7640 33.98	14995 66.70	5425 24.13	10645 47.35
HGUS414	12	3⅝	12⅞	4	11⅛	(66) 16d	(22) 16d	10130 45.06	16400 72.95	7195 32.00	11645 51.80

See footnotes on p. 258.

TECHNICAL BULLETIN

TC – Truss Connectors

SIMPSON Strong-Tie®

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

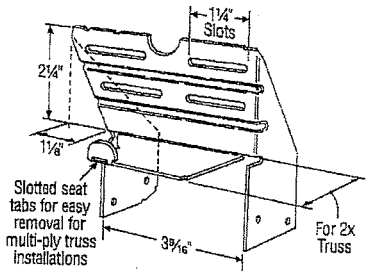
Design: Factored resistances are in accordance with CSA 086-14

Installation:

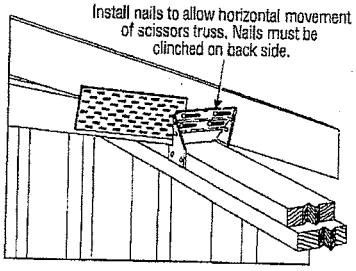
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½ = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss–allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

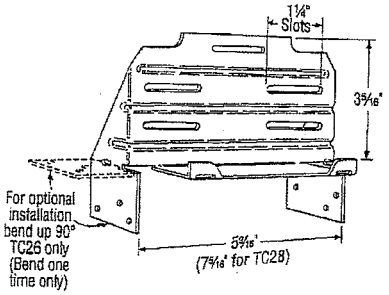
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



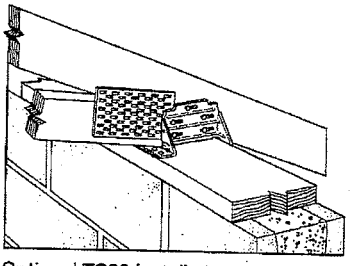
TC24
U.S. Patent 4,932,173



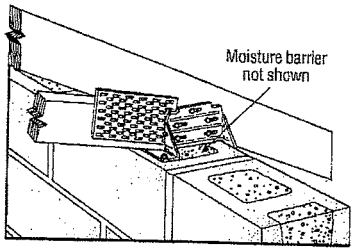
Typical TC24 Installation



TC26
(TC28 Similiar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC26	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) – ⅜" x 2¼" Titen screws has a factored uplift resistance of 275 lb.

LIMIT STATES DESIGN

(800) 999-5099
strongtie.com

H/TSP

SIMPSON Strong-Tie®

Seismic and Hurricane Ties (cont.)

■ These products are available with additional corrosion protection. For more information, see p. 24.

◤ These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

	Model No.	Ga.	Fasteners			Factored Resistance (K_D = 1.15)					
						D.Fir-L			S-P-F		
						Uplift	Lateral		Uplift	Lateral	
			To Rafters/ Truss	To Plates	To Studs		F_1	F_2		F_1	F_2
						lb.	lb.	lb.	lb.	lb.	lb.
						kN	kN	kN	kN	kN	kN
■	H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
						3.29	3.05	1.33	3.02	2.16	0.96
SS ■	H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
						3.69	0.98	0.33	2.62	0.69	0.24
SS ■	H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
						3.58	0.71	0.71	3.36	0.71	0.71
	H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
						3.71	0.78	0.93	3.29	0.71	0.93
SS ◤	H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
						3.29	0.80	1.18	2.74	0.56	0.85
■	H6	16	—	(8) 8d	(8) 8d	1585	1085	—	1125	770	—
						7.05	4.83	—	5.00	3.43	—
■	H7Z	16	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
						6.18	2.98	—	4.40	2.11	—
SS ◤	H8[3]	18	(5) 10d x 1½"	(5) 10d x 1½"	—	1120	—	—	1025	—	—
						4.98	—	—	4.56	—	—
SS ◤	H10A[9]	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290
						7.72	3.54	1.82	6.69	2.51	1.29
	H10AR	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1485	690	430	1220	570	305
						6.61	3.07	1.91	5.43	2.54	1.36
■	H10A-2	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1835	1275	430	1645	880	305
						8.16	5.67	1.91	7.32	3.91	1.36
	H10S[2,8]	18	(8) 8d x 1½"	(8) 8d x 1½"	(8) 8d	1465	795	315	1040	565	225
						6.52	3.54	1.40	4.63	2.51	1.00
■	H11Z	18	(6) 16d x 2½"	(6) 16d x 2½"	—	1095	920	545	780	655	390
						4.87	4.09	2.42	3.47	2.91	1.73
	H14	18	1 (12) 8d x 1½"	(13) 8d	—	2390	855	320	1805	610	230
						10.63	3.80	1.42	8.03	2.71	1.02
			2 (12) 8d x 1½"	(15) 8d	—	2390	855	320	1805	610	230
						10.63	3.80	1.42	8.03	2.71	1.02
	TSP	16	(9) 10d x 1½"	(6) 10d x 1½"	—	1295	440	—	920	310	—
						5.76	1.96	—	4.09	1.38	—
			(9) 10d x 1½"	(6) 10d	—	1560	440	—	1105	310	—
						6.94	1.96	—	4.92	1.38	—

1. Factored resistances have been increased 15% for short term loading; no further increase is allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
3. H8 factored uplift resistances for stud-to-bottom plate installations are 595 lb. (2.65 kN) for D.Fir-L and 390 lb. (1.74 kN) for S-P-F.
4. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
5. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-HTIECONPATH).
6. Factored resistances in the F_1 direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.
7. H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.38 kN) D.Fir-L and 1015 lb. (4.51 kN) S-P-F.
8. H10S nails to plates are optional for uplift but required for lateral loads.
9. H10A may be field-bent up to a slope of 6/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
10. The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SSNAILS at strongtie.com.
11. D.Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1½" nails into the top plates and (3) 8d x 1½" nails into the lowest three flange holes into the truss bottom chord is 485 lb. (2.20 kN).
12. **Nails:** 16d x 2½" = 0.162" dia. x 2½" long, 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long, 8d x 1½" = 0.131" dia. x 1½" long. See pp. 27–28 for other nail sizes and information.

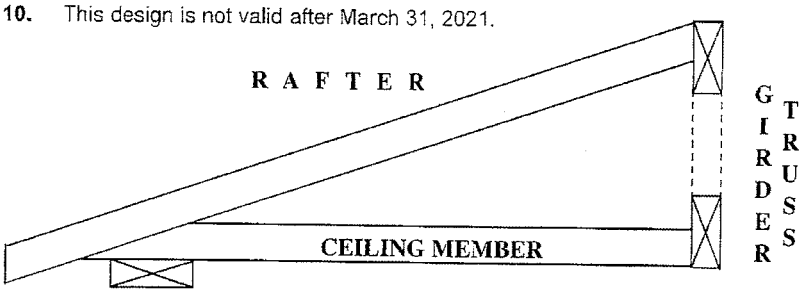
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

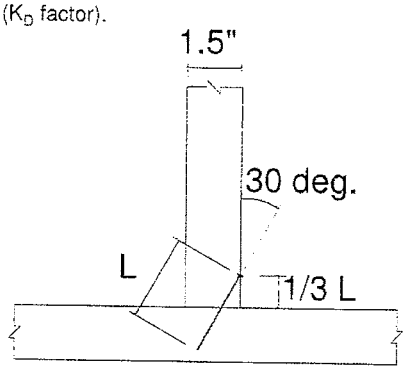
B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.





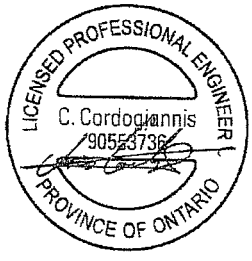
TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		**(3" and 3.25" nail)**	
LUMBER SIZE	**MAXIMUM NUMBER OF TOE-NAILS**			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek®
MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019

PEO
Certificate No. 10889485



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

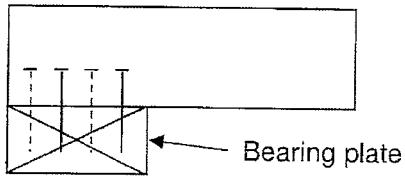
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

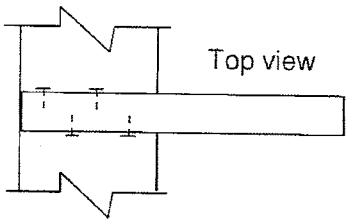
NOTES:

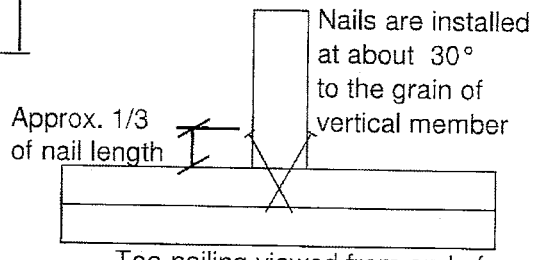
1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



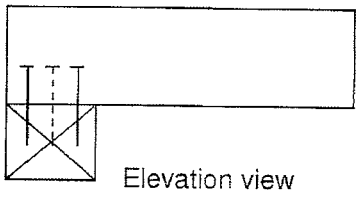
Elevation view



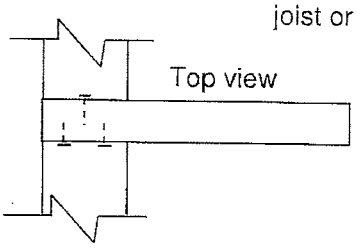


Toe-nailing viewed from end of joist or truss

Toe-nailing on 2x4 Bearing Plate



Elevation view



MiTek®

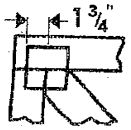
MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485

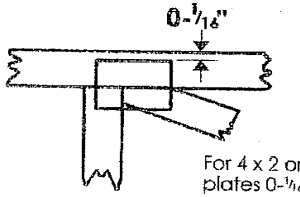
December 2, 2019

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

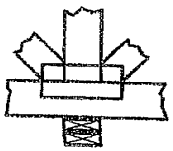
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

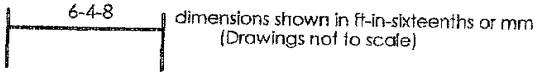


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

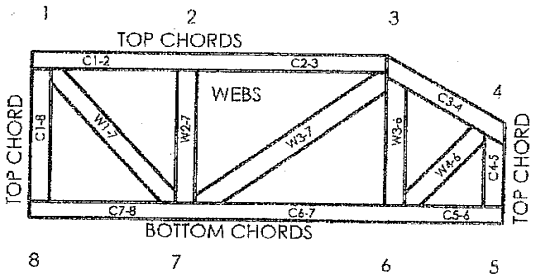
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

ONTARIO WOOD TRUSS
FABRICATORS ASSOCIATION

TECH-NOTES

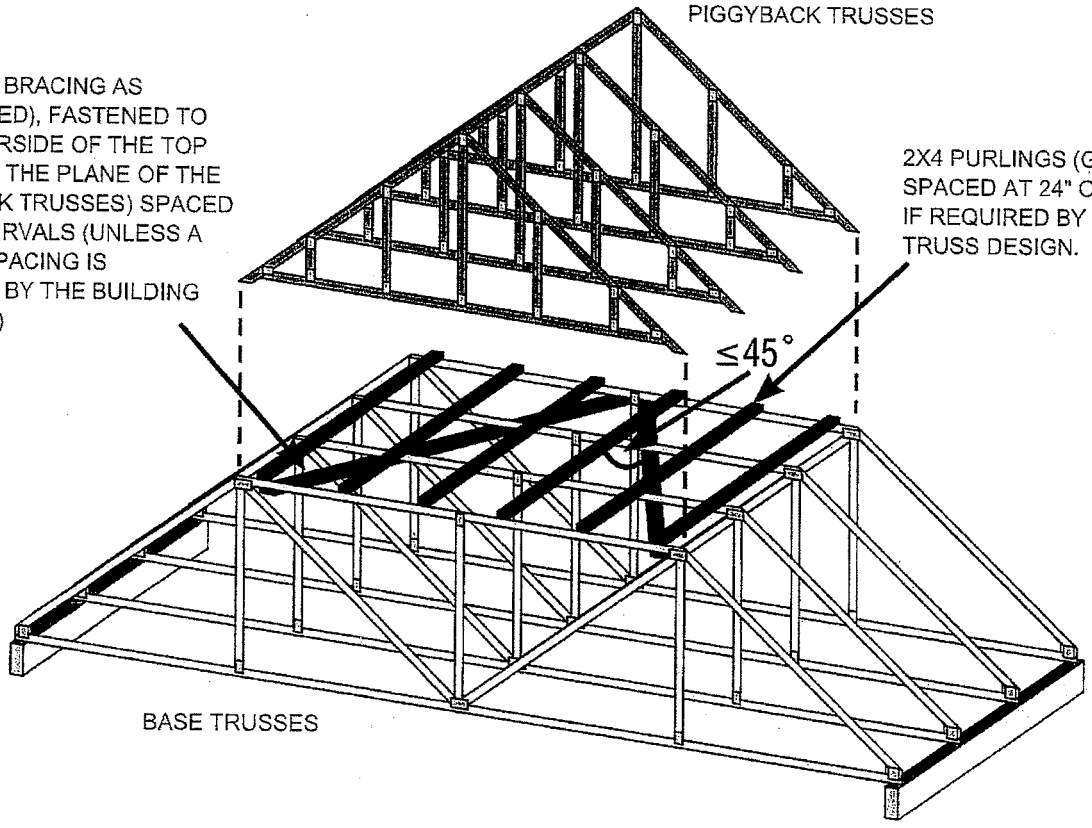
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

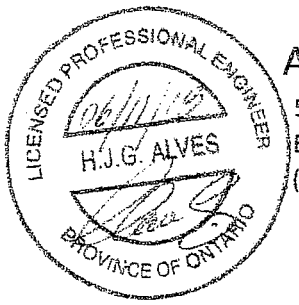
Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900215 Feb 09, 2018