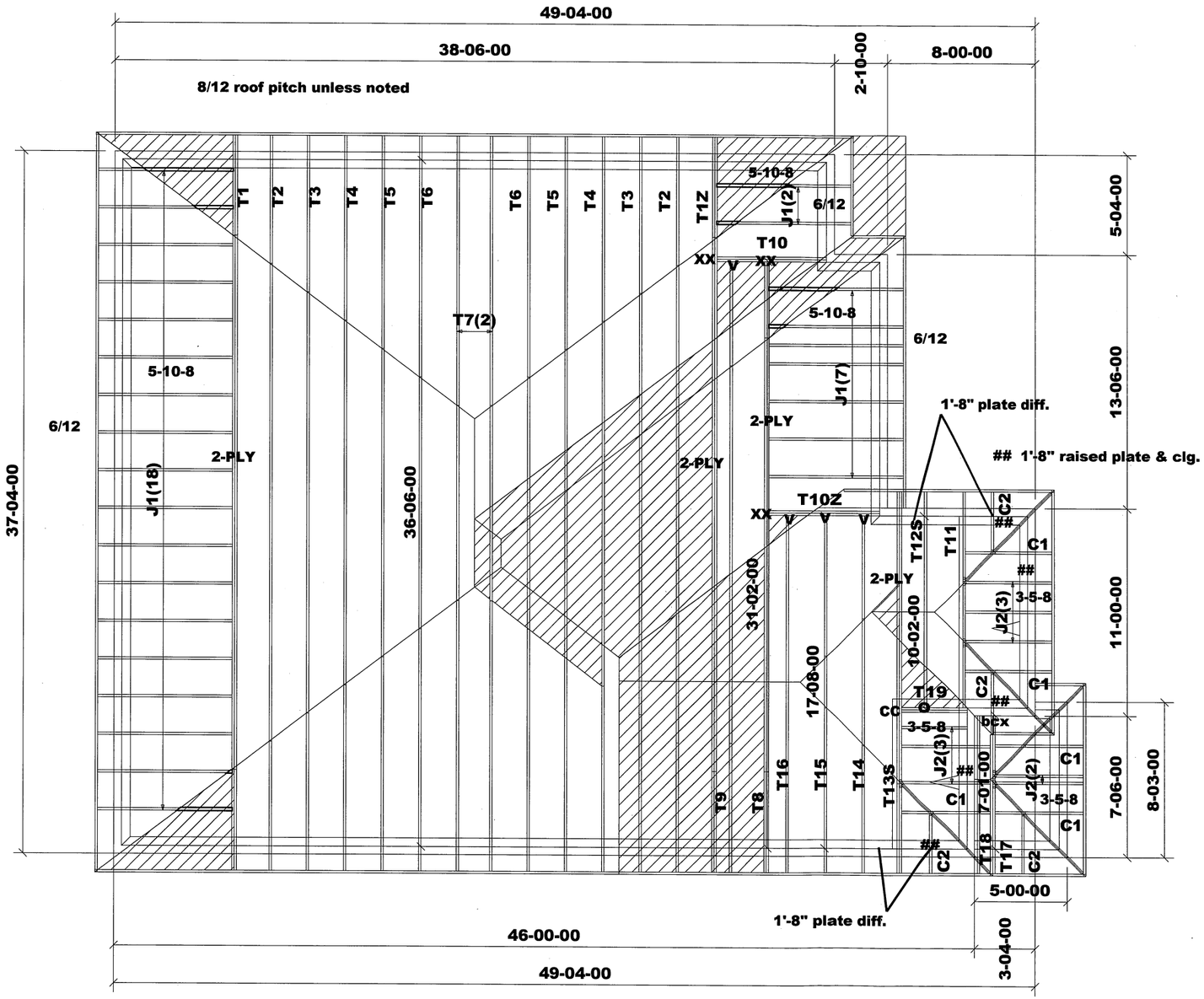
LAMPONE INVESTMENT INC

CITY OF MARKHAM

ROOF TRUSS SHOP DRAWINGS

MODEL NAME : KIMBERLY 5

ELEV 1, 2 & 3



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

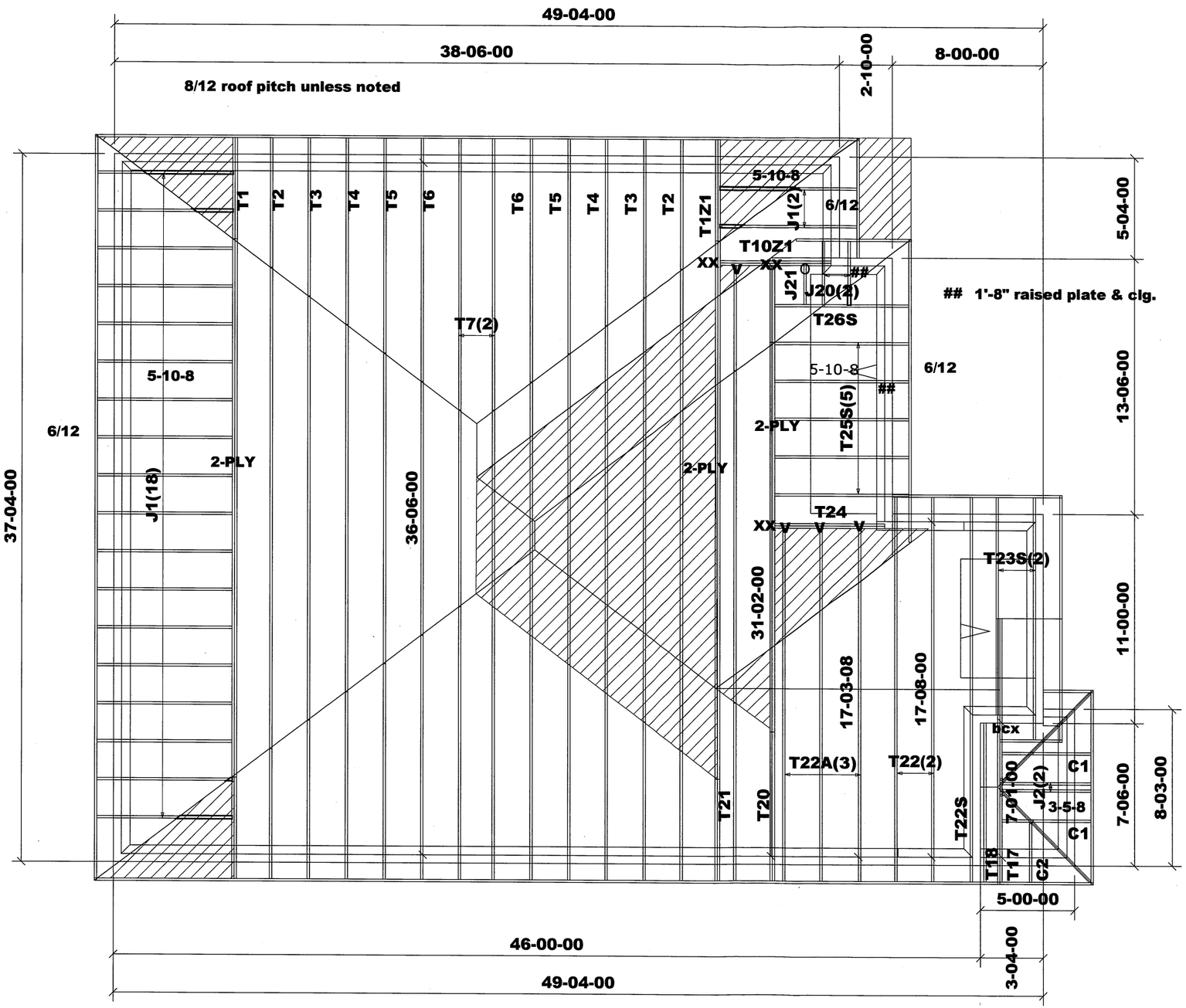
HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26-2 - (**XX**)
HUSC26-2 - (CC)

DENOTES:
CONVENTIONAL FRAMING

M13370

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Job Track: 51453	Builder / Location: GREEN PARK HOMES / MARKHAM	Model / Elevation: KIMBERLY 5 / 1	Mitek ver 8.3.3.247
Layout ID: 410068	Project: LAMPONE INVESTMENTS INC		
Plan Log: 202871	Date: 2020-08-22	Designer: JG	



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

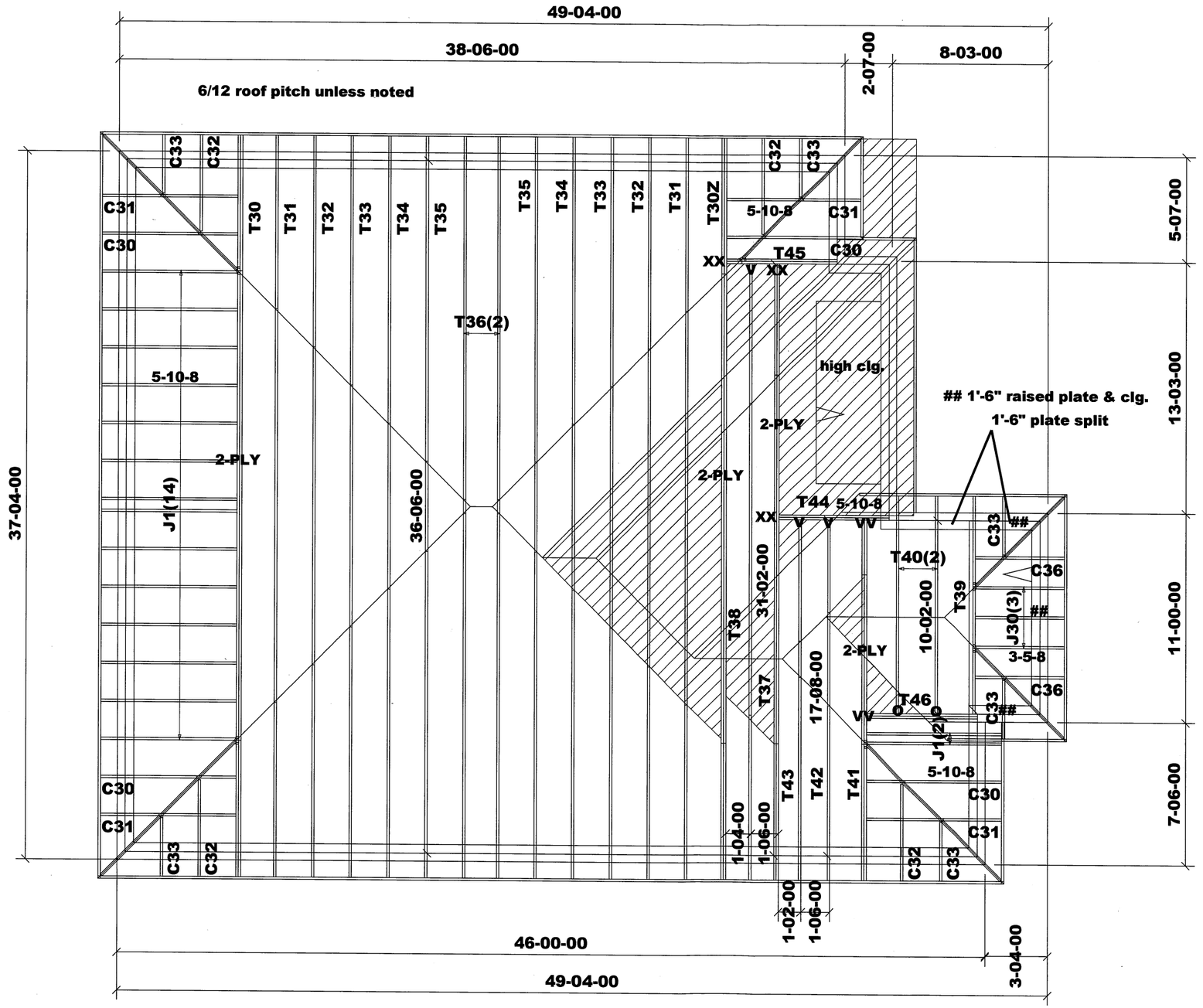
DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26-2 - (**XX**)

DENOTES:
CONVENTIONAL FRAMING

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Job Track: 51453	Builder / Location: GREEN PARK HOMES / MARKHAM	Model / Elevation: KIMBERLY 5 / 2	Mitek ver 8.3.3.247
Layout ID: 410069	Project: LAMPONE INVESTMENTS INC		
Plan Log: 202871	Date: 2020-08-21	Designer: JG	



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
- TCSL = 25.6 psf
- TCDL = 6.0 psf
- BCLL = 0.0 psf
- BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26-2 - (**XX**)
LUS26-2- (VV)

DENOTES:
CONVENTIONAL FRAMING

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

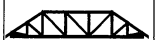
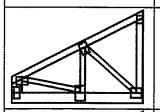
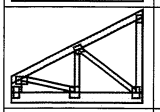
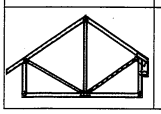
Job Track: 51453	Builder / Location: GREEN PARK HOMES / MARKHAM	Model / Elevation: KIMBERLY 5 / 3	Mitek ver 8.3.3.247
Layout ID: 410070	Project: LAMPONE INVESTMENTS INC		
Plan Log: 202871	Date: 2020-08-21	Designer: JG	

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREEN PARK HOMES	PlanLog:	202871
Project:	LAMPONE INVESTMENTS INC	Layout ID:	410068
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 5	Page:	1 of 2
Lot #:		Date:	08-21-2020
Elevation:	1	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

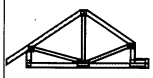
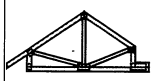
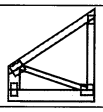
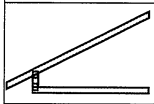
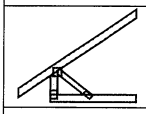
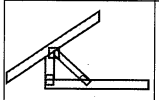
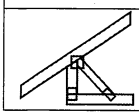
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	8 /12	**36-06-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	374.29 234.67		
	1 2-ply	T1Z Hip Girder	8 /12	**36-06-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	374.29 234.67		
	2	T2 Hip	8 /12	**36-06-00**	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	345.94 215.67		
	2	T3 Hip	8 /12	**36-06-00**	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	362.39 225.33		
	2	T4 Hip	8 /12	**36-06-00**	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	363.18 224.33		
	2	T5 Hip	8 /12	**36-06-00**	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	378.65 233.00		
	2	T6 Hip	8 /12	**36-06-00**	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	383.87 235.67		
	2	T7 Hip	8 /12	**36-06-00**	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	397.28 244.00		
	1 2-ply	T8 Hip Girder	8 /12	**31-02-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	294.65 182.67		
	1	T9 Hip	8 /12	**31-02-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	122.4 76.67		
	1 2-ply	T10 Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.77 38.67		
	1 2-ply	T10Z Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1	T11 Hip Girder	8 /12	**10-02-00**	3-08-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	44.85 29.83		
	1	T12S Roof Special	8 /12	**10-02-00**	6-05-08	2 x 4	1-03-08	3-00-13 1-04-13	49.27 31.67		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREEN PARK HOMES	PlanLog:	202871
Project:	LAMPONE INVESTMENTS INC	Layout ID:	410068
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 5	Page:	2 of 2
Lot #:		Date:	08-21-2020
Elevation:	1	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	**1 2-ply**	**T13S Hip Girder**	8 /12	**17-08-00**	5-04-08	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 3-00-13	194.27 118.33		
	**1**	**T14 Hip**	8 /12	**17-08-00**	6-08-08	2 x 4	1-03-08 1-03-08	3-00-13 3-00-13	83.4 52.67		
	**1**	**T15 Hip**	8 /12	**17-08-00**	8-00-08	2 x 4	1-03-08 1-03-08	3-00-13 3-00-13	92.03 58.67		
	**1**	**T16 Common**	8 /12	**17-08-00**	8-11-08	2 x 4	1-03-08 1-03-08	3-00-13 3-00-13	88.91 55.33		
	**1**	**T17 Hip Girder**	8 /12	**7-01-00**	3-08-08	2 x 4	1-03-08	1-04-13 1-07-08	32.26 22.00		
	**1**	**T18 Common**	8 /12	**7-01-00**	3-10-08	2 x 4	1-03-08	1-04-13 1-07-08	32.47 22.00		
	**1 2-ply**	**T19 Jack-Closed Girder**	8 /12	**3-05-08**	3-08-08	2 x 4 2 x 6		1-04-13 3-08-08	36.79 23.67		
	**27**	**J1 Jack-Open**	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	453.45 288.00		
	**8**	**J2 Jack-Open**	8 /12	**3-05-08**	3-08-08	2 x 4	1-03-08	1-04-13 3-08-08	104.98 66.67		
	**5**	**C1 Jack-Open**	8 /12	**1-09-07**	2-07-02	2 x 4	1-03-08 1-08-01	1-04-13 2-07-02	53.55 35.00		
	**4**	**C2 Jack-Open**	8 /12	**1-09-07**	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	35.24 22.67		

TOTAL # TRUSS= **78** TOTAL BFT OF ALL TRUSSES= **3009.53** BFT. TOTAL WEIGHT OF ALL TRSSES **4815.56** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
4	Hardware	LJS26DS	
1	Hardware	LUS24	
1	Hardware	HUSC26-2	

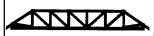
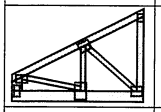
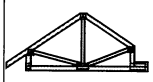
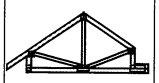
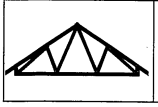
TOTAL NUMBER OF ITEMS= **9**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREEN PARK HOMES	PlanLog:	202871
Project:	LAMPONE INVESTMENTS INC	Layout ID:	410069
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 5	Page:	1 of 3
Lot #:		Date:	08-21-2020
Elevation:	2	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

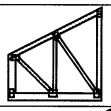
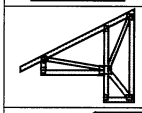
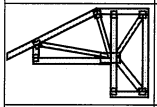
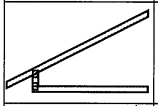
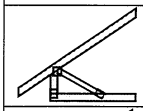
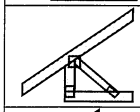
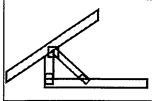
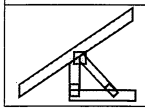
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	8 /12	**36-06-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	374.29 234.67		
	1 2-ply	T1Z1 Hip Girder	8 /12	**36-06-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	374.29 234.67		
	2	T2 Hip	8 /12	**36-06-00**	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	345.94 215.67		
	2	T3 Hip	8 /12	**36-06-00**	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	356.75 222.00		
	2	T4 Hip	8 /12	**36-06-00**	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	363.18 224.33		
	2	T5 Hip	8 /12	**36-06-00**	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	378.65 233.00		
	2	T6 Hip	8 /12	**36-06-00**	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	383.87 235.67		
	2	T7 Hip	8 /12	**36-06-00**	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	397.28 244.00		
	1 2-ply	T10Z1 Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.77 38.67		
	1	T17 Hip Girder	8 /12	**7-01-00**	3-08-08	2 x 4	1-03-08	1-04-13 1-07-08	32.26 22.00		
	1	T18 Common	8 /12	**7-01-00**	3-10-08	2 x 4	1-03-08	1-04-13 1-07-08	32.47 22.00		
	1 2-ply	T20 Hip Girder	8 /12	**31-02-00**	5-09-04	2 x 4 2 x 6	1-03-08	1-04-13 3-00-13	314.67 195.33		
	1	T21 Hip	8 /12	**31-02-00**	6-09-04	2 x 4	1-03-08	1-04-13 3-00-13	136.12 86.17		
	2	T22 Common	8 /12	**17-08-00**	7-03-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	155.97 99.33		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREEN PARK HOMES	PlanLog:	202871
Project:	LAMPONE INVESTMENTS INC	Layout ID:	410069
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 5	Page:	2 of 3
Lot #:		Date:	08-21-2020
Elevation:	2	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	3	T22A Common	8 /12	17-08-00	7-03-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	226.01 147.00		
	1	T22S Roof Special	8 /12	17-08-00	7-03-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	88.39 57.67		
	2	T23S Roof Special	8 /12	10-02-00	4-09-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	99.02 67.67		
	1 2-ply	T24 Jack-Closed Girder	6 /12	5-10-08	5-09-04	2 x 4 2 x 6		2-10-00 5-09-04	74.22 47.00		
	5	T25S Jack-Closed	6 /12	5-10-08	5-09-04	2 x 4	1-03-08	1-02-00 5-09-04	180.43 120.00		
	1	T26S Half Hip Girder	6 /12	5-10-08	4-05-04	2 x 4	1-03-08	1-02-00 4-05-04	34.62 23.00		
	20	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	335.89 213.33		
	2	J2 Jack-Open	8 /12	3-05-08	3-08-08	2 x 4	1-03-08	1-04-13 3-08-08	27.01 17.67		
	2	J20 Jack-Open	8 /12	2-00-10	2-09-04	2 x 4	1-03-08	1-04-13 2-09-04	19.1 14.00		
	1	J21 Jack-Open	8 /12	2-00-10	4-05-04	2 x 4		3-00-13 4-05-04	10.94 8.00		
	2	C1 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 1-08-01	1-04-13 2-07-02	21.42 14.00		
	1	C2 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-05-00 1-01	1-04-13 2-07-02	8.98 6.33		

TOTAL # TRUSS= **67** TOTAL BFT OF ALL TRUSSES= **3043.18** BFT. TOTAL WEIGHT OF ALL TRSSES **4830.54** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
4	Hardware	LJS26DS	

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREEN PARK HOMES	PlanLog:	202871
Project:	LAMPONE INVESTMENTS INC	Layout ID:	410069
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 5	Page:	3 of 3
Lot #:		Date:	08-21-2020
Elevation:	2	Designer:	
		Sales Rep:	Mario DiCano

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	LUS24	

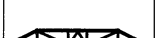
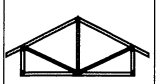
TOTAL NUMBER OF ITEMS= **8**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREEN PARK HOMES	PlanLog:	202871
Project:	LAMPONE INVESTMENTS INC	Layout ID:	410070
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 5	Page:	1 of 2
Lot #:		Date:	08-21-2020
Elevation:	3	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

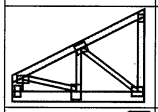
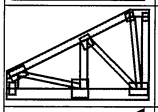
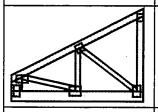
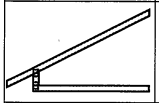
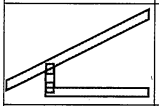
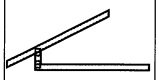
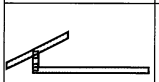
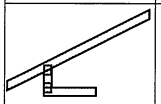
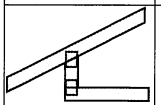
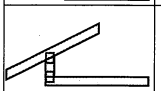
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T30 Hip Girder	6 /12	**36-06-00**	4-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	383.35 241.33		
	1 2-ply	T30Z Hip Girder	6 /12	**36-06-00**	4-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	384.36 242.67		
	2	T31 Hip	6 /12	**36-06-00**	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	337.16 210.33		
	2	T32 Hip	6 /12	**36-06-00**	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	345.25 213.33		
	2	T33 Hip	6 /12	**36-06-00**	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	342.76 210.33		
	2	T34 Hip	6 /12	**36-06-00**	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	355.98 220.00		
	2	T35 Hip	6 /12	**36-06-00**	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	354.56 220.00		
	2	T36 Hip	6 /12	**36-06-00**	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	351.52 214.67		
	1 2-ply	T37 Hip Girder	6 /12	**31-02-00**	4-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	327 204.00		
	1	T38 Hip	6 /12	**31-02-00**	4-10-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	122.68 77.83		
	1	T39 Hip Girder	6 /12	**10-02-00**	2-10-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	41.35 28.33		
	2	T40 Common	6 /12	**10-02-00**	5-02-08	2 x 4	1-03-08 1-03-08	2-08-00 2-08-00	90.71 61.00		
	1 2-ply	T41 Hip Girder	6 /12	**17-08-00**	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 2-08-00	164.73 105.33		
	1	T42 Hip	6 /12	**17-08-00**	5-01-04	2 x 4	1-03-08	1-02-00 2-08-00	71.46 45.83		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51453
Builder:	GREEN PARK HOMES	PlanLog:	202871
Project:	LAMPONE INVESTMENTS INC	Layout ID:	410070
Location:	MARKHAM	Ref #	
Model:	KIMBERLY 5	Page:	2 of 2
Lot #:		Date:	08-21-2020
Elevation:	3	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T43 Hip	6 /12	17-08-00	5-10-04	2 x 4	1-03-08	1-02-00 2-08-00	78.92 51.00		
	1 2-ply	T44 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T45 Half Hip Girder	6 /12	5-10-08	3-08-08	2 x 4 2 x 6		1-02-00 3-08-08	63.63 40.33		
	1 2-ply	T46 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	16	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	268.71 170.67		
	3	J30 Jack-Open	6 /12	3-05-08	2-10-12	2 x 4	1-03-08	1-02-00 2-10-12	32.87 22.00		
	4	C30 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-05-00 2-01-01	1-02-00 3-00-12	57.17 34.67		
	4	C31 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	46.33 29.33		
	4	C32 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	38.28 24.00		
	6	C33 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	42.13 28.00		
	2	C36 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-08-01	1-02-00 2-00-12	17.65 12.00		

TOTAL # TRUSS= **71** TOTAL BFT OF ALL TRUSSES= **2782.32** BFT. TOTAL WEIGHT OF ALL TRSSES **4435.36** LBS

HARDWARE

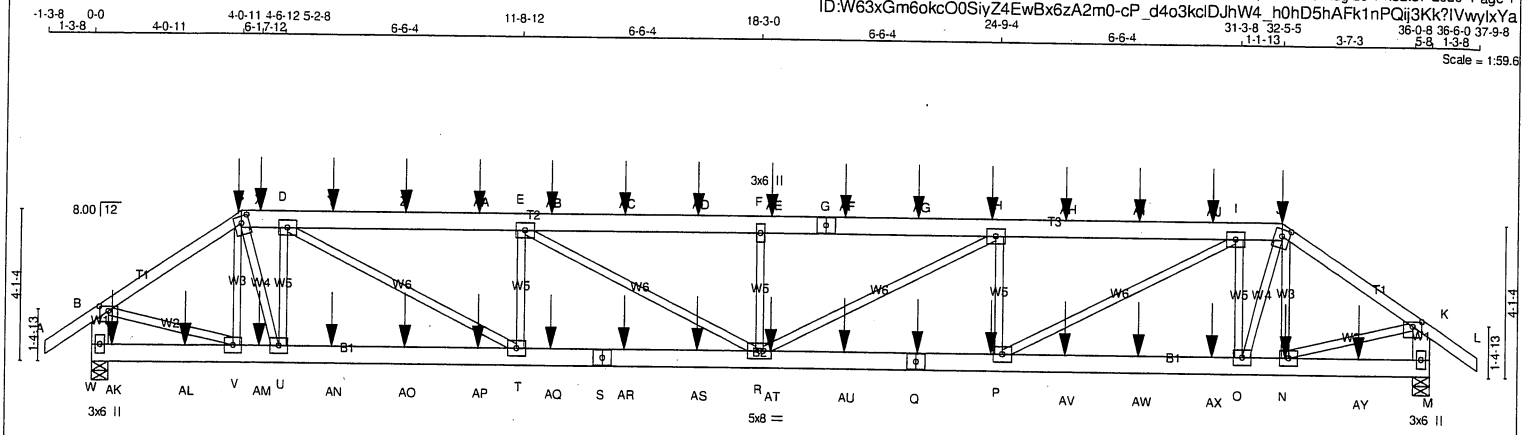
QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
3	Hardware	LJS26DS	
2	Hardware	LUS24	
2	Hardware	LUS26-2	

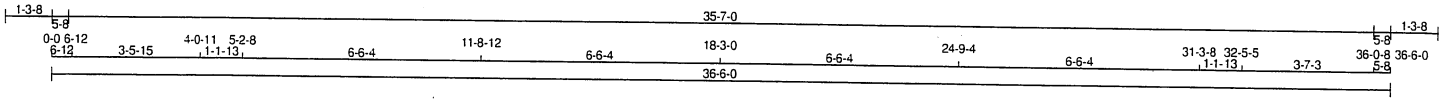
TOTAL NUMBER OF ITEMS= **10**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410068	T1	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MiTek Industries, Inc. Thu Aug 20 14:32:57 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-cP_d4o3kclDJhW4_h0hD5hAFk1nPQij3Kk?IVwylxYa





TOTAL WEIGHT = 2 X 187 = 374 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - S	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
J- L	1	12	SIDE(61.0)
C- G	2	12	SIDE(183.1)
G- J	2	12	SIDE(183.1)
W- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- S	2	12	SIDE(183.1)
S- Q	2	12	SIDE(183.1)
Q- M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2018793 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	3533	0	3533	0	0	5-8	5-8
M	3529	0	3529	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2498	1642 / 0	0 / 0	0 / 0	0 / 0	856 / 0	0 / 0
M	2494	1649 / 0	0 / 0	0 / 0	0 / 0	845 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 35	-91.8	-91.8	0.07 (1)	10.00
B- C	-4167 / 0	-91.8	-91.8	0.23 (1)	4.48
C- X	-4370 / 0	-91.8	-91.8	0.13 (1)	5.37
X- D	-4370 / 0	-91.8	-91.8	0.13 (1)	5.37
D- Y	-7526 / 0	-91.8	-91.8	0.30 (1)	4.18
Y- Z	-7526 / 0	-91.8	-91.8	0.30 (1)	4.18
Z-AA	-7526 / 0	-91.8	-91.8	0.30 (1)	4.18
AA- E	-7526 / 0	-91.8	-91.8	0.30 (1)	4.18
E-AB	-8541 / 0	-91.8	-91.8	0.33 (1)	3.93
AB-AC	-8541 / 0	-91.8	-91.8	0.33 (1)	3.93
AC-AD	-8541 / 0	-91.8	-91.8	0.33 (1)	3.93
AD- F	-8541 / 0	-91.8	-91.8	0.33 (1)	3.93
F-AE	-8541 / 0	-91.8	-91.8	0.32 (1)	3.94
AE- G	-8541 / 0	-91.8	-91.8	0.32 (1)	3.94
G-AF	-8541 / 0	-91.8	-91.8	0.32 (1)	3.94
AF-AG	-8541 / 0	-91.8	-91.8	0.32 (1)	3.94
AG- H	-8541 / 0	-91.8	-91.8	0.32 (1)	3.94
H-AH	-7531 / 0	-91.8	-91.8	0.28 (1)	4.19
AH-AI	-7531 / 0	-91.8	-91.8	0.28 (1)	4.19
AI-AJ	-7531 / 0	-91.8	-91.8	0.28 (1)	4.19
AJ- I	-7531 / 0	-91.8	-91.8	0.28 (1)	4.19
I- J	-4382 / 0	-91.8	-91.8	0.14 (1)	5.36
J- K	-4186 / 0	-91.8	-91.8	0.23 (1)	4.47
K- L	0 / 35	-91.8	-91.8	0.07 (1)	10.00
W- B	-3484 / 0	0.0	0.0	0.12 (1)	7.49
M- K	-3499 / 0	0.0	0.0	0.13 (1)	7.48
W-AK	0 / 0	-18.5	-18.5	0.03 (4)	10.00
AK-AL	0 / 0	-18.5	-18.5	0.03 (4)	10.00
AL- V	0 / 0	-18.5	-18.5	0.03 (4)	10.00
V-AM	0 / 3498	-18.5	-18.5	0.26 (1)	10.00
AM- U	0 / 3498	-18.5	-18.5	0.26 (1)	10.00
U-AN	0 / 4364	-18.5	-18.5	0.34 (1)	10.00
AN-AO	0 / 4364	-18.5	-18.5	0.34 (1)	10.00
AO-AP	0 / 4364	-18.5	-18.5	0.34 (1)	10.00
AP- T	0 / 4364	-18.5	-18.5	0.34 (1)	10.00
T-AQ	0 / 7526	-18.5	-18.5	0.55 (1)	10.00
AQ- S	0 / 7526	-18.5	-18.5	0.55 (1)	10.00
S-AR	0 / 7526	-18.5	-18.5	0.55 (1)	10.00
AR-AS	0 / 7526	-18.5	-18.5	0.55 (1)	10.00
AS- R	0 / 7526	-18.5	-18.5	0.55 (1)	10.00
R-AT	0 / 7531	-18.5	-18.5	0.55 (1)	10.00
AT-AU	0 / 7531	-18.5	-18.5	0.55 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
V- C	-742 / 0	0.09 (1)
N- J	-753 / 0	0.09 (1)
B- V	0 / 3586	0.44 (1)
N- K	0 / 3603	0.45 (1)
U- D	-2652 / 0	0.32 (1)
C- U	0 / 2887	0.36 (1)
O- I	-2632 / 0	0.32 (1)
O- J	0 / 2870	0.36 (1)
P- I	0 / 3615	0.45 (1)
D- T	0 / 3623	0.45 (1)
P- H	-1565 / 0	0.19 (1)
T- E	-1568 / 0	0.19 (1)
R- H	0 / 1157	0.14 (1)
E- R	0 / 1163	0.14 (1)
R- F	-929 / 0	0.11 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.43")

CSI: TC=0.33/1.00 (E-F:1) , BC=0.55/1.00 (R-T:1) , WB=0.45/1.00 (D-T:1) , SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.71 (S) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410068	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	5.0	6.0	2.25	2.50
D, E, H, I						
D	TMWW-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
J	TTWW+m	MT20	5.0	6.0	2.25	2.50
K	TMVW-p	MT20	5.0	6.0	1.50	3.00
M	BMV1+p	MT20	3.0	6.0		
N, O, P, T, U, V						
N	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
W	BMV1+p	MT20	3.0	6.0		

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
AU- Q	0 / 7531	-18.5	-18.5	0.55 (1)	10.00
Q- P	0 / 7531	-18.5	-18.5	0.55 (1)	10.00
P-AV	0 / 4375	-18.5	-18.5	0.34 (1)	10.00
AV-AW	0 / 4375	-18.5	-18.5	0.34 (1)	10.00
AW-AX	0 / 4375	-18.5	-18.5	0.34 (1)	10.00
AX- O	0 / 4375	-18.5	-18.5	0.34 (1)	10.00
O- N	0 / 3515	-18.5	-18.5	0.26 (1)	10.00
N-AY	0 / 0	-18.5	-18.5	0.03 (1)	10.00
AY- M	0 / 0	-18.5	-18.5	0.03 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-51	-57	---	FRONT	VERT	DEAD	---	C1
C	4-0-11	-260	-260	---	FRONT	VERT	SNOW	---	C1
H	24-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
J	32-5-5	-51	-57	---	FRONT	VERT	DEAD	---	C1
J	32-5-5	-117	-117	---	FRONT	VERT	TOTAL	---	C1
J	32-5-5	-260	-260	---	FRONT	VERT	SNOW	---	C1
N	32-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	24-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Q	22-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
X	4-6-12	-133	-133	---	FRONT	VERT	TOTAL	---	C1
Y	6-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	8-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	10-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	12-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	14-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	16-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AE	18-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AF	20-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AG	22-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AH	26-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AI	28-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AJ	30-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AK	6-12	-30	-30	---	FRONT	VERT	TOTAL	---	C1
AL	2-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AM	4-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AN	6-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AO	8-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AP	10-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AQ	12-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AR	14-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AS	16-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AT	18-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AU	20-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AV	26-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AW	28-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AX	30-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AY	34-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
08/20/20
H. J/G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018793

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410068	T1Z	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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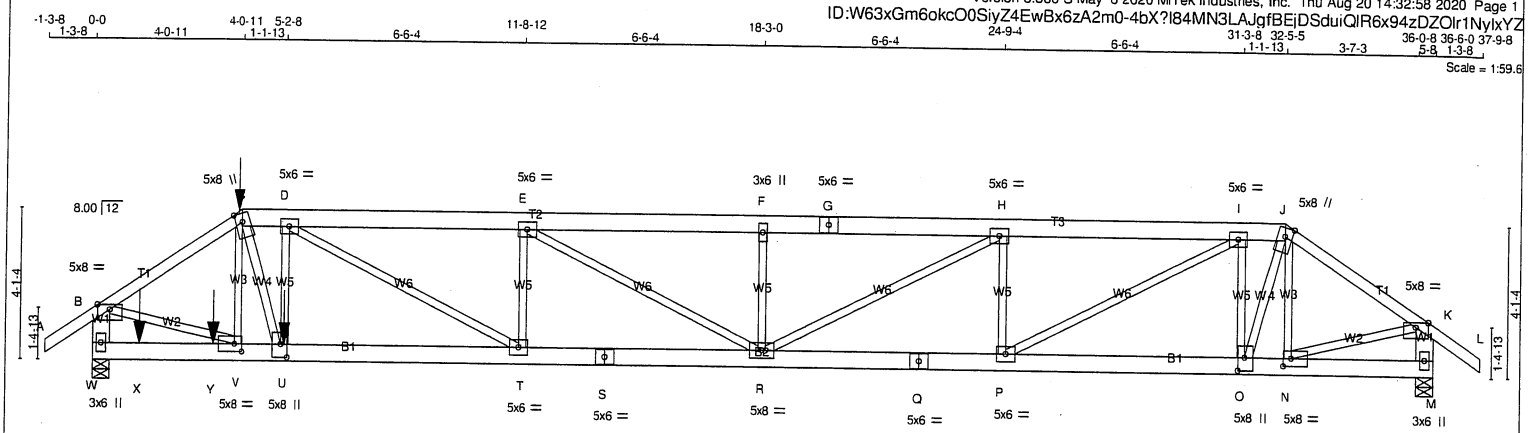

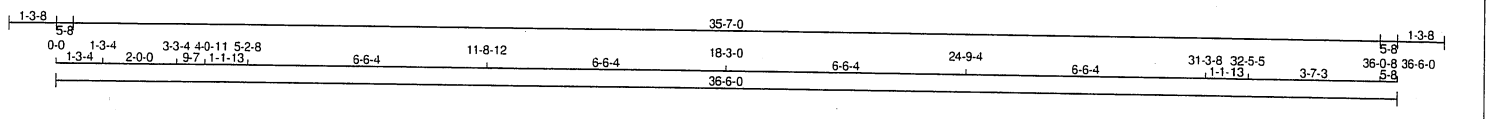

TOTAL WEIGHT = 2 X 187 = 374 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - S	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
J- L	1	12	TOP
C- G	2	12	SIDE(61.0)
G- J	2	12	TOP
W- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- S	2	12	SIDE(183.1)
S- Q	2	12	TOP
Q- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
U- D	1	2	SIDE(841.3)
O- I	1	2	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2018794

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	5830	0	5830	0	0	5-8	5-8
M	2736	0	2736	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	4112	2760 / 0	0 / 0	0 / 0	0 / 0	1352 / 0	0 / 0
M	1931	1290 / 0	0 / 0	0 / 0	0 / 0	641 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.07 (1)	10.00
B- C	-7215 / 0	-91.8	-91.8	0.38 (1)	3.40
C- D	-7502 / 0	-91.8	-91.8	0.12 (1)	4.35
D- E	-8544 / 0	-91.8	-91.8	0.30 (1)	3.96
E- F	-8068 / 0	-91.8	-91.8	0.25 (1)	4.11
F- G	-8068 / 0	-91.8	-91.8	0.24 (1)	4.13
G- H	-8068 / 0	-91.8	-91.8	0.24 (1)	4.13
H- I	-6385 / 0	-91.8	-91.8	0.19 (1)	4.59
I- J	-3375 / 0	-91.8	-91.8	0.08 (1)	5.99
J- K	-3160 / 0	-91.8	-91.8	0.19 (1)	5.04
K- L	0 / 35	-91.8	-91.8	0.07 (1)	10.00
W- B	-5805 / 0	0.0	0.0	0.21 (1)	6.12
M- K	-2718 / 0	0.0	0.0	0.10 (1)	7.81
W- X	0 / 0	-18.5	-18.5	0.05 (1)	10.00
X- Y	0 / 0	-18.5	-18.5	0.05 (1)	10.00
Y- V	0 / 0	-18.5	-18.5	0.05 (1)	10.00
V- U	0 / 6052	-18.5	-18.5	0.49 (1)	10.00
U- T	0 / 7502	-18.5	-18.5	0.59 (1)	10.00
T- S	0 / 8544	-18.5	-18.5	0.60 (1)	10.00
S- R	0 / 8544	-18.5	-18.5	0.60 (1)	10.00
R- Q	0 / 6385	-18.5	-18.5	0.47 (1)	10.00
Q- P	0 / 6385	-18.5	-18.5	0.47 (1)	10.00
P- O	0 / 3371	-18.5	-18.5	0.25 (1)	10.00
O- N	0 / 2654	-18.5	-18.5	0.20 (1)	10.00
N- M	0 / 0	-18.5	-18.5	0.03 (1)	10.00

WEBS FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-1083 / 0	0.13 (1)
N- J	-570 / 0	0.07 (1)
B- V	0 / 6210	0.77 (1)
N- K	0 / 2720	0.34 (1)
U- D	-984 / 0	0.12 (1)
C- U	0 / 4837	0.60 (1)
O- I	-2231 / 0	0.27 (1)
O- J	0 / 2394	0.30 (1)
P- I	0 / 3454	0.43 (1)
D- T	0 / 1195	0.15 (1)
P- H	-1569 / 0	0.19 (1)
T- E	-406 / 0	0.05 (1)
R- H	0 / 1928	0.24 (1)
E- R	-545 / 0	0.30 (1)
R- F	-570 / 0	0.07 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-51	-57	---	FRONT	VERT	DEAD	---	C1
C	4-0-11	-260	-260	---	FRONT	VERT	SNOW	---	C1
U	5-2-8	-3927	-3927	---	BACK	VERT	TOTAL	---	C1
X	1-3-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
Y	3-3-4	-26	-26	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.41")

CSI: TC=0.38/1.00 (B-C:1) , BC=0.60/1.00 (R-T:1) , WB=0.77/1.00 (B-V:1) , SSI=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (V) (INPUT = 0.90)
JSI METAL= 0.76 (S) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410068	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	8.0	Edge	
D, E, H, I						
D	TMWW-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
J	TTWW+m	MT20	5.0	8.0	Edge	
K	TMVW-p	MT20	5.0	8.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	8.0	2.50	2.75
O	BMWW+t	MT20	5.0	8.0	4.25	2.25
P	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMWW+t	MT20	5.0	8.0	4.25	2.25
V	BMWW-t	MT20	5.0	8.0	2.50	2.75
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
08/20/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018794 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410069	T1Z1	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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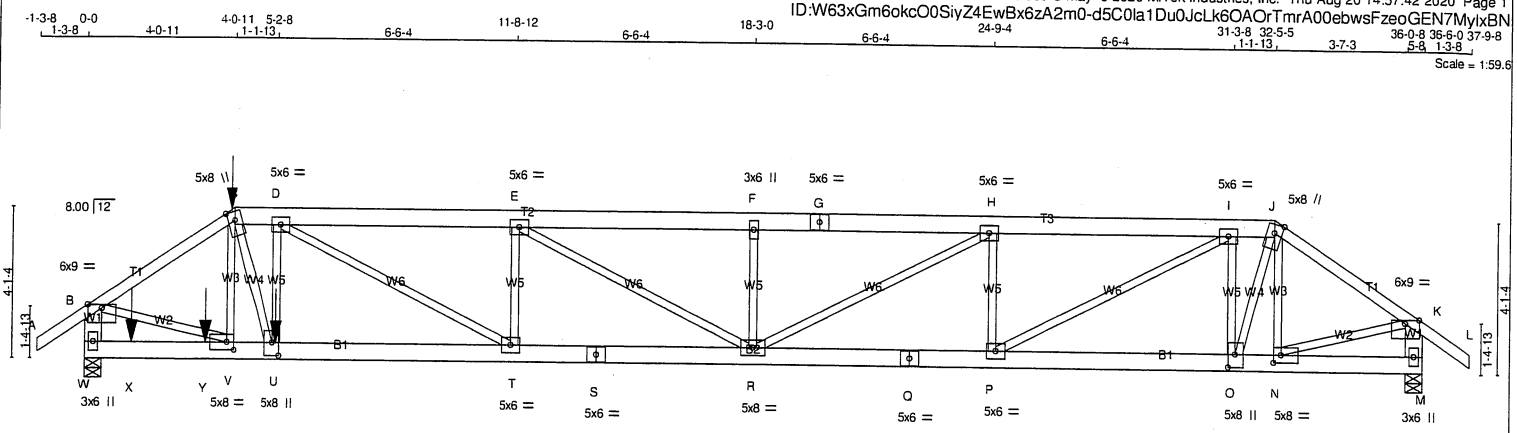

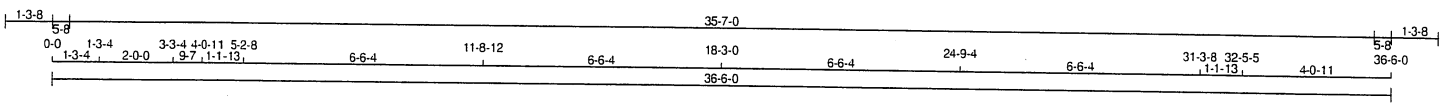

TOTAL WEIGHT = 2 X 187 = 374 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - S	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
J- L	1	12	TOP
C- G	2	12	SIDE(61.0)
G- J	2	12	TOP
W- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- S	2	12	SIDE(183.1)
S- Q	2	12	TOP
Q- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
U- D	1	2	SIDE(894.6)
O- I	1	2	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2018810 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	6005	0	6005	0	0	5-8	5-8
M	2765	0	2765	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	4236	2844 / 0	0 / 0	0 / 0	0 / 0	1392 / 0	0 / 0
M	1951	1304 / 0	0 / 0	0 / 0	0 / 0	648 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.07 (1)	10.00
B- C	-7446 / 0	-91.8	-91.8	0.39 (1)	3.34
C- D	-7744 / 0	-91.8	-91.8	0.13 (1)	4.28
D- E	-8743 / 0	-91.8	-91.8	0.31 (1)	3.91
E- F	-8213 / 0	-91.8	-91.8	0.25 (1)	4.08
F- G	-8213 / 0	-91.8	-91.8	0.24 (1)	4.10
G- H	-8213 / 0	-91.8	-91.8	0.24 (1)	4.10
H- I	-6479 / 0	-91.8	-91.8	0.19 (1)	4.56
I- J	-3416 / 0	-91.8	-91.8	0.08 (1)	5.97
J- K	-3199 / 0	-91.8	-91.8	0.19 (1)	5.01
K- L	0 / 35	-91.8	-91.8	0.07 (1)	10.00
W- B	-5981 / 0	0.0	0.0	0.21 (1)	6.04
M- K	-2747 / 0	0.0	0.0	0.10 (1)	7.81
W- X	0 / 0	-18.5	-18.5	0.05 (1)	10.00
X- Y	0 / 0	-18.5	-18.5	0.05 (1)	10.00
Y- V	0 / 0	-18.5	-18.5	0.05 (1)	10.00
V- U	0 / 6246	-18.5	-18.5	0.51 (1)	10.00
U- T	0 / 7744	-18.5	-18.5	0.61 (1)	10.00
T- S	0 / 8743	-18.5	-18.5	0.61 (1)	10.00
S- R	0 / 8743	-18.5	-18.5	0.61 (1)	10.00
R- Q	0 / 6479	-18.5	-18.5	0.47 (1)	10.00
Q- P	0 / 6479	-18.5	-18.5	0.47 (1)	10.00
P- O	0 / 3412	-18.5	-18.5	0.26 (1)	10.00
O- N	0 / 2686	-18.5	-18.5	0.20 (1)	10.00
N- M	0 / 0	-18.5	-18.5	0.03 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-1115 / 0	0.14 (1)
N- J	-578 / 0	0.07 (1)
B- V	0 / 6409	0.79 (1)
N- K	0 / 2753	0.34 (1)
U- D	-952 / 0	0.11 (1)
C- U	0 / 4998	0.62 (1)
O- I	-2260 / 0	0.27 (1)
O- J	0 / 2424	0.30 (1)
P- I	0 / 3513	0.43 (1)
D- T	0 / 1145	0.14 (1)
P- H	-1598 / 0	0.19 (1)
T- E	-379 / 0	0.05 (1)
R- H	0 / 1987	0.25 (1)
E- R	-607 / 0	0.33 (1)
R- F	-569 / 0	0.07 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-51	-57	---	FRONT	VERT	DEAD	---	C1
C	4-0-11	-260	-260	---	FRONT	VERT	SNOW	---	C1
U	5-2-8	-4131	-4131	---	BACK	VERT	TOTAL	---	C1
X	1-3-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
Y	3-3-4	-26	-26	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.42")

CSI: TC=0.39/1.00 (B-C:1) , BC=0.61/1.00 (R-T:1) , WB=0.79/1.00 (B-V:1) , SSI=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (V) (INPUT = 0.90)
JSI METAL= 0.78 (S) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410069	T1Z1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	5.0	8.0	Edge	
D, E, H, I						
D	TMWW-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
J	TTWW+m	MT20	5.0	8.0	Edge	
K	TMVW-p	MT20	6.0	9.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	8.0	2.50	2.50
O	BMWW+t	MT20	5.0	8.0	4.25	2.25
P	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMWW+t	MT20	5.0	8.0	4.25	2.25
V	BMWW-t	MT20	5.0	8.0	2.50	2.50
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018810

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T2	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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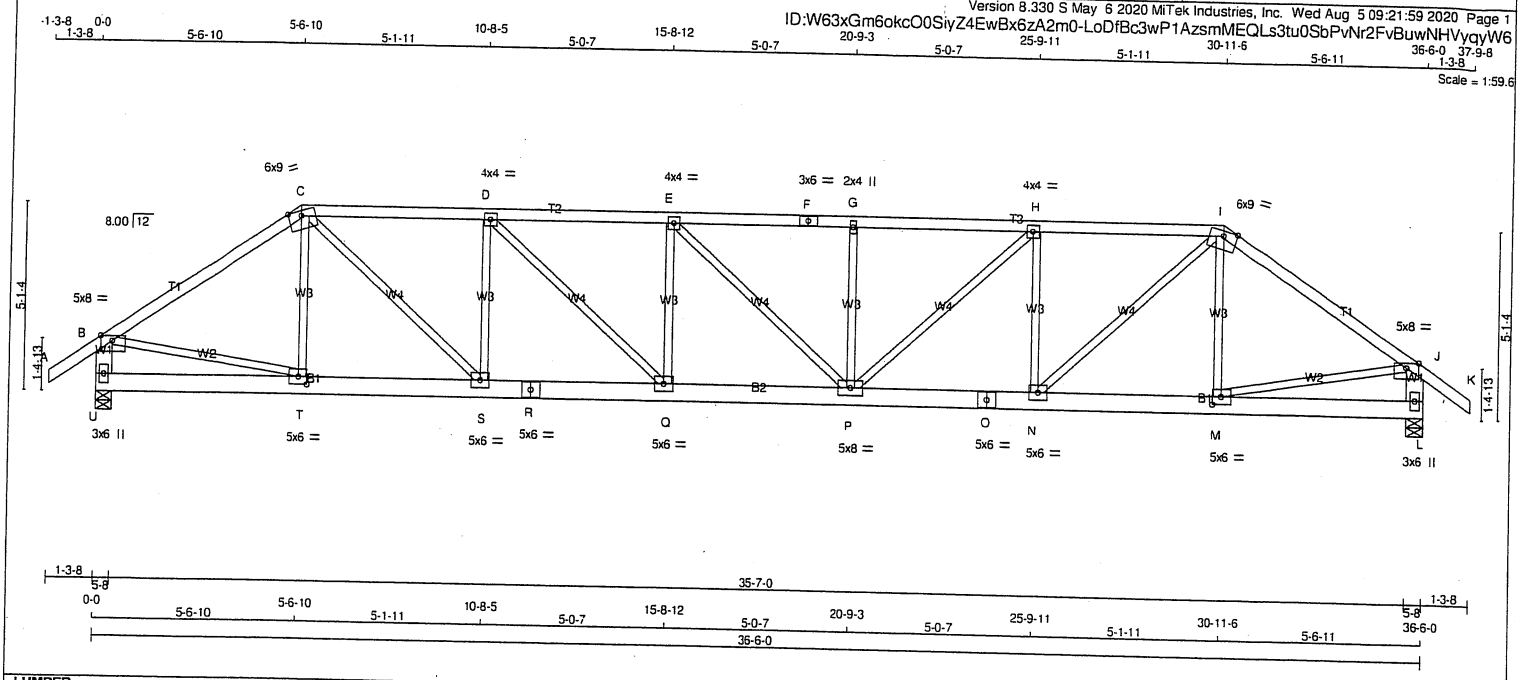

TOTAL WEIGHT = 2 X 173 = 346 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	6.0	9.0	Edge	
D, E, H						
D	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
I	TTWW-m	MT20	6.0	9.0	Edge	
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N, Q, S						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.75
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2017331

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2138	0	2138	0	0	5-8	5-8
L	2138	0	2138	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 0
L	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)
A- B	0 / 35
B- C	-2437 / 0
C- D	-3265 / 0
D- E	-3843 / 0
E- F	-3835 / 0
F- G	-3835 / 0
G- H	-3835 / 0
H- I	-3267 / 0
I- J	-2436 / 0
J- K	0 / 35
U- B	-2093 / 0
L- J	-2092 / 0
U- T	0 / 0
T- S	0 / 2019
S- R	0 / 3265
R- Q	0 / 3265
Q- P	0 / 3843
P- O	0 / 3267
O- N	0 / 3267
N- M	0 / 2019
M- L	0 / 0

FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
-91.8	-91.8	0.12 (1)	10.00
-91.8	-91.8	0.73 (1)	3.67
-91.8	-91.8	0.56 (1)	3.41
-91.8	-91.8	0.63 (1)	3.09
-91.8	-91.8	0.52 (1)	3.23
-91.8	-91.8	0.52 (1)	3.23
-91.8	-91.8	0.63 (1)	3.09
-91.8	-91.8	0.56 (1)	3.41
-91.8	-91.8	0.73 (1)	3.67
-91.8	-91.8	0.12 (1)	10.00
0.0	0.0	0.14 (1)	7.02
0.0	0.0	0.14 (1)	7.02
-18.5	-18.5	0.06 (4)	10.00
-18.5	-18.5	0.27 (1)	10.00
-18.5	-18.5	0.43 (1)	10.00
-18.5	-18.5	0.43 (1)	10.00
-18.5	-18.5	0.51 (1)	10.00
-18.5	-18.5	0.44 (1)	10.00
-18.5	-18.5	0.44 (1)	10.00
-18.5	-18.5	0.27 (1)	10.00
-18.5	-18.5	0.06 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-241 / 4	0.09 (1)
C- S	0 / 1690	0.38 (1)
S- D	-1058 / 0	0.38 (1)
D- Q	0 / 792	0.18 (1)
Q- E	-444 / 0	0.16 (1)
E- P	-11 / 0	0.01 (1)
P- G	-446 / 0	0.16 (1)
P- H	0 / 779	0.18 (1)
N- H	-1051 / 0	0.38 (1)
N- I	0 / 1693	0.38 (1)
M- I	-243 / 4	0.09 (1)
B- T	0 2056	0.46 (1)
M- J	0 2056	0.46 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.40")

CSI: TC=0.73/1.00 (B-C:1) , BC=0.51/1.00 (P-Q:1) , WB=0.46/1.00 (B-T:1) , SSI=0.22/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (T) (INPUT = 0.90)
JSI METAL= 0.60 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T3	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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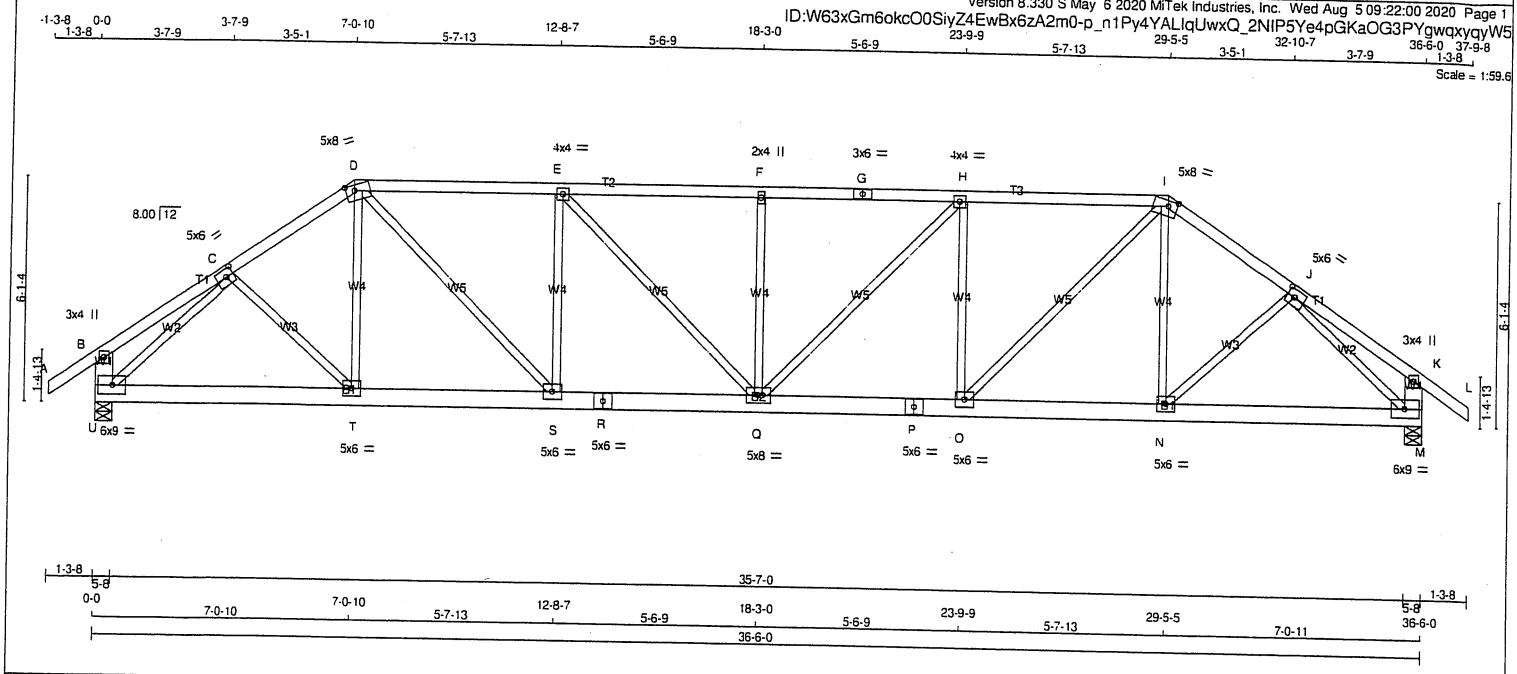

TOTAL WEIGHT = 2 X 178 = 357 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW-m	MT20	5.0	8.0	1.75	3.00
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	8.0	1.75	3.00
J	TMWW-t	MT20	5.0	6.0	2.50	2.50
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	6.0	9.0		
N, O, S, T						
N	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
U	BMVW1-t	MT20	6.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2138	0	2138	0	0	5-8	5-8
M	2138	0	2138	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0
M	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 18	-91.8	-91.8	0.16 (1)	10.00
C- D	-2445 / 0	-91.8	-91.8	0.24 (1)	4.19
D- E	-2963 / 0	-91.8	-91.8	0.65 (1)	3.45
E- F	-3234 / 0	-91.8	-91.8	0.68 (1)	3.28
F- G	-3234 / 0	-91.8	-91.8	0.68 (1)	3.28
G- H	-3234 / 0	-91.8	-91.8	0.68 (1)	3.28
H- I	-2963 / 0	-91.8	-91.8	0.65 (1)	3.45
I- J	-2445 / 0	-91.8	-91.8	0.24 (1)	4.19
J- K	0 / 18	-91.8	-91.8	0.16 (1)	10.00
K- L	0 / 35	-91.8	-91.8	0.12 (1)	10.00
U- B	-255 / 0	0.0	0.0	0.02 (1)	7.81
M- K	-255 / 0	0.0	0.0	0.02 (1)	7.81
U- T	0 / 1922	-18.5	-18.5	0.29 (1)	10.00
T- S	0 / 2016	-18.5	-18.5	0.28 (1)	10.00
S- R	0 / 2963	-18.5	-18.5	0.40 (1)	10.00
R- Q	0 / 2963	-18.5	-18.5	0.40 (1)	10.00
Q- P	0 / 2963	-18.5	-18.5	0.40 (1)	10.00
P- O	0 / 2963	-18.5	-18.5	0.40 (1)	10.00
O- N	0 / 2016	-18.5	-18.5	0.28 (1)	10.00
N- M	0 / 1922	-18.5	-18.5	0.29 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	0 / 126	0.03 (4)
T- D	0 / 93	0.03 (4)
D- S	0 / 1347	0.30 (1)
S- E	-859 / 0	0.47 (1)
E- Q	0 / 389	0.09 (1)
Q- F	-468 / 0	0.26 (1)
Q- H	0 / 389	0.09 (1)
O- H	-859 / 0	0.47 (1)
O- I	0 / 1347	0.30 (1)
N- I	0 / 93	0.03 (4)
N- J	0 / 126	0.03 (4)
U- C	-2651 / 0	0.93 (1)
J- M	-2651 / 0	0.93 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.68/1.00 (F-H:1) , BC=0.40/1.00 (O-Q:1) , WB=0.93/1.00 (J-M:1) , SSI=0.24/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (D) (INPUT = 0.90)
JSI METAL= 0.60 (C) (INPUT = 1.00)

Structural component only
DWG# T-2017332

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T4	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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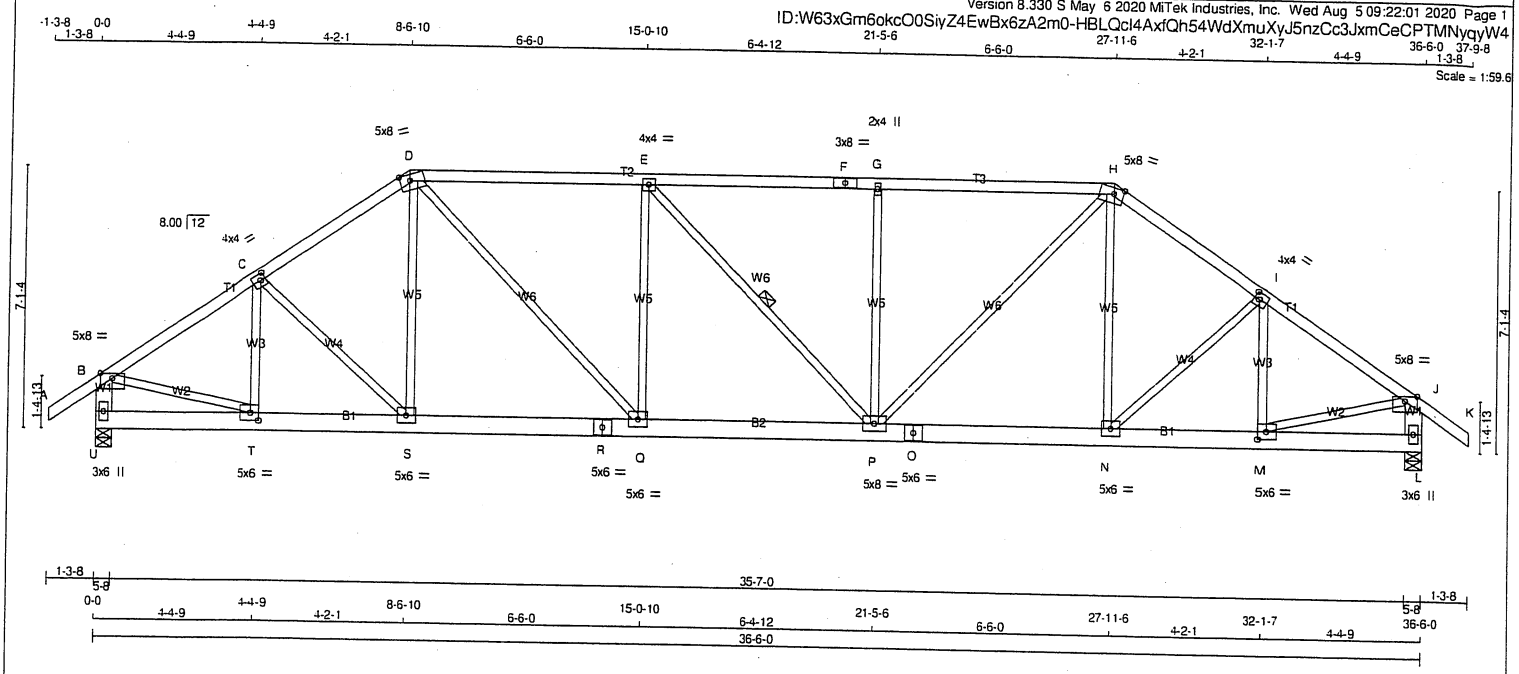


LUMBER
N. L. G. A. RULES

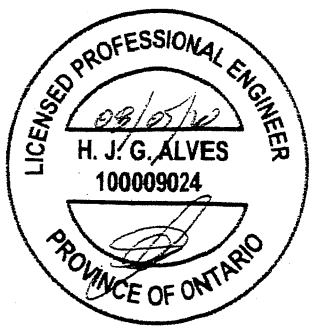
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	8.0	2.00	3.25
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.00	3.25
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N, Q, S						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.50
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



Structural component only
DWG# T-2017333

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2138	0	2138	0	0	5-8	5-8
L	2138	0	2138	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0
L	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-2399 / 0	-91.8	-91.8	0.38 (1)	4.09
C- D	-2398 / 0	-91.8	-91.8	0.37 (1)	4.09
D- E	-2706 / 0	-91.8	-91.8	0.80 (1)	3.42
E- F	-2701 / 0	-91.8	-91.8	0.79 (1)	3.42
F- G	-2701 / 0	-91.8	-91.8	0.79 (1)	3.42
G- H	-2701 / 0	-91.8	-91.8	0.79 (1)	3.44
H- I	-2399 / 0	-91.8	-91.8	0.37 (1)	4.09
I- J	-2398 / 0	-91.8	-91.8	0.38 (1)	4.09
J- K	0 / 35	-91.8	-91.8	0.12 (1)	10.00
U- B	-2086 / 0	0.0	0.0	0.14 (1)	7.03
L- J	-2086 / 0	0.0	0.0	0.14 (1)	7.03
U- T	0 / 0	-18.5	-18.5	0.05 (1)	10.00
T- S	0 / 2016	-18.5	-18.5	0.28 (1)	10.00
S- R	0 / 1973	-18.5	-18.5	0.28 (1)	10.00
R- Q	0 / 1973	-18.5	-18.5	0.28 (1)	10.00
Q- P	0 / 2706	-18.5	-18.5	0.37 (1)	10.00
P- O	0 / 1974	-18.5	-18.5	0.28 (1)	10.00
O- N	0 / 1974	-18.5	-18.5	0.28 (1)	10.00
N- M	0 / 2016	-18.5	-18.5	0.29 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.05 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-417 / 0	0.11 (1)
C- S	-62 / 0	0.03 (1)
S- D	0 / 171	0.04 (4)
D- Q	0 / 1053	0.24 (1)
Q- E	-639 / 0	0.52 (1)
E- P	-7 / 0	0.00 (1)
P- G	-641 / 0	0.53 (1)
P- H	0 / 1045	0.24 (1)
N- H	0 / 177	0.05 (4)
N- I	-59 / 0	0.03 (1)
M- I	-419 / 0	0.11 (1)
B- T	0 / 2068	0.47 (1)
M- J	0 / 2067	0.47 (1)

TOTAL WEIGHT = 2 X 182 = 363 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.80/1.00 (D-E:1) , BC=0.37/1.00 (P-Q:1) , WB=0.53/1.00 (G-P:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (T) (INPUT = 0.90)
JSI METAL= 0.46 (T) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T5	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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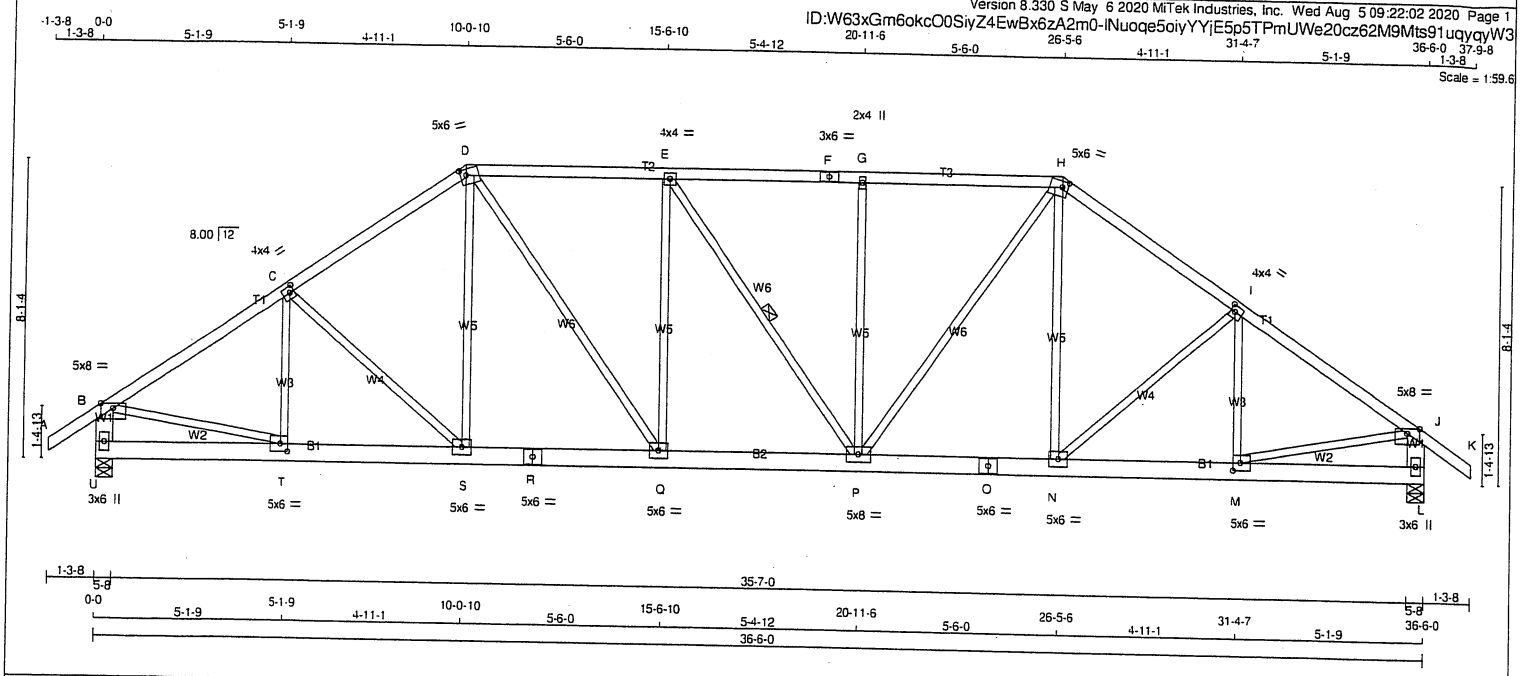

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.00	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.00	2.00
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N, Q, S						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.50
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2017334

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2138	0	2138	0	0	5-8	5-8
L	2138	0	2138	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0
L	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-2451 / 0	-91.8	-91.8	0.40 (1)	4.05
C- D	-2322 / 0	-91.8	-91.8	0.38 (1)	4.16
D- E	-2362 / 0	-91.8	-91.8	0.39 (1)	4.10
E- F	-2357 / 0	-91.8	-91.8	0.37 (1)	4.14
F- G	-2357 / 0	-91.8	-91.8	0.37 (1)	4.14
G- H	-2357 0	-91.8	-91.8	0.39 (1)	4.11
H- I	-2323 / 0	-91.8	-91.8	0.38 (1)	4.16
I- J	-2451 / 0	-91.8	-91.8	0.40 (1)	4.05
J- K	0 / 35	-91.8	-91.8	0.12 (1)	10.00
U- B	-2085 / 0	0.0	0.0	0.14 (1)	7.03
L- J	-2085 / 0	0.0	0.0	0.14 (1)	7.03
U- T	0 0	-18.5	-18.5	0.06 (4)	10.00
T- S	0 / 2064	-18.5	-18.5	0.28 (1)	10.00
S- R	0 / 1907	-18.5	-18.5	0.26 (1)	10.00
R- Q	0 / 1907	-18.5	-18.5	0.26 (1)	10.00
Q- P	0 / 2362	-18.5	-18.5	0.32 (1)	10.00
P- O	0 / 1909	-18.5	-18.5	0.26 (1)	10.00
O- N	0 / 1909	-18.5	-18.5	0.26 (1)	10.00
N- M	0 2063	-18.5	-18.5	0.28 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.06 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-328 / 0	0.11 (1)
C- S	-213 / 0	0.17 (1)
S- D	0 / 257	0.06 (1)
D- Q	0 / 783	0.18 (1)
Q- E	-536 / 0	0.64 (1)
E- P	-8 / 0	0.01 (1)
P- G	-539 0	0.64 (1)
P- H	0 / 773	0.17 (1)
N- H	0 / 265	0.06 (1)
N- I	-211 / 0	0.17 (1)
M- I	-330 / 0	0.11 (1)
B- T	0 / 2102	0.47 (1)
M- J	0 / 2102	0.47 (1)

TOTAL WEIGHT = 2 X 189 = 379 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.40/1.00 (B-C:1) , BC=0.32/1.00 (P-Q:1) , WB=0.64/1.00 (G-P:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (T) (INPUT = 0.90)
JSI METAL= 0.47 (T) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T6	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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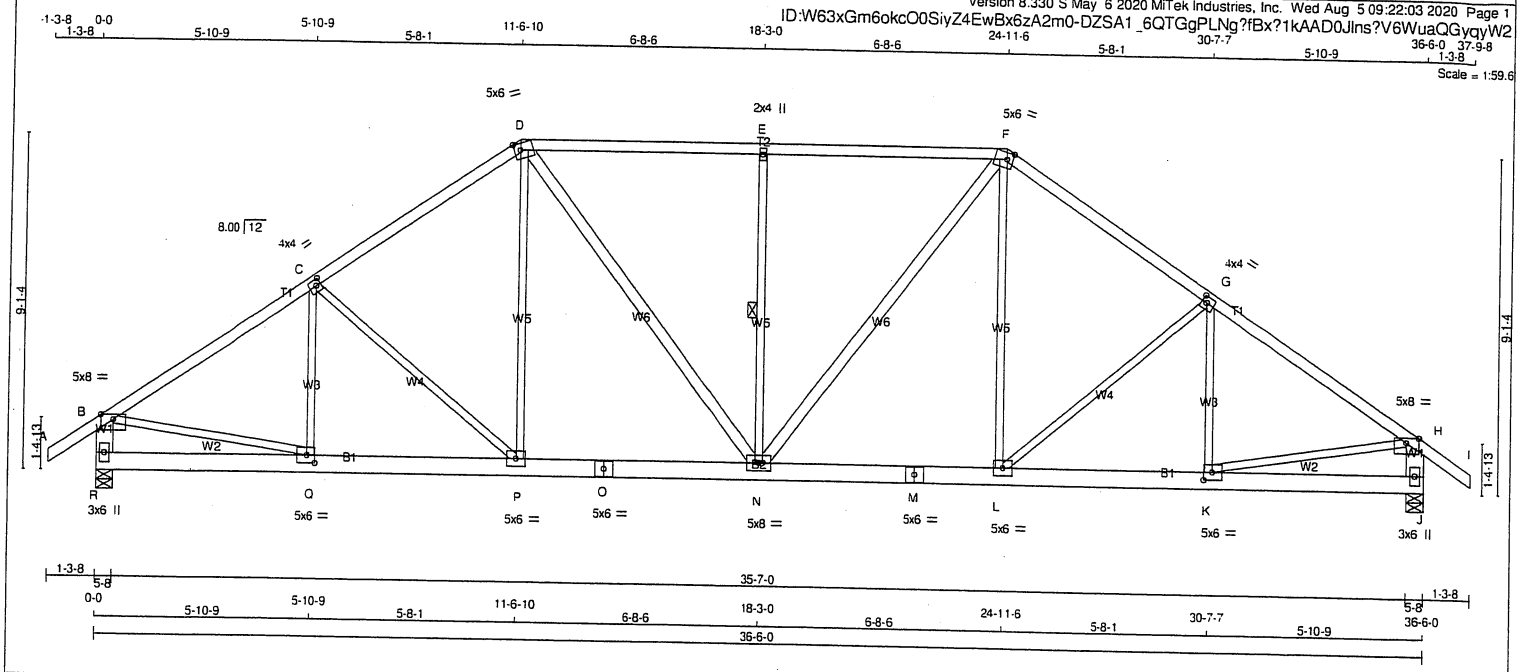

TOTAL WEIGHT = 2 X 192 = 384 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BS-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.75
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2017335

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2138	0	2138	0	0	5-8	5-8
J	2138	0	2138	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 0
J	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-2479 / 0	-91.8	-91.8	0.53 (1)	3.89
C- D	-2234 / 0	-91.8	-91.8	0.49 (1)	4.10
D- E	-2165 / 0	-91.8	-91.8	0.62 (1)	3.91
E- F	-2165 / 0	-91.8	-91.8	0.62 (1)	3.91
F- G	-2234 / 0	-91.8	-91.8	0.49 (1)	4.10
G- H	-2479 / 0	-91.8	-91.8	0.53 (1)	3.89
H- I	0 / 35	-91.8	-91.8	0.12 (1)	10.00
R- B	-2082 / 0	0.0	0.0	0.14 (1)	7.03
J- H	-2082 / 0	0.0	0.0	0.14 (1)	7.03
R- Q	0 / 0	-18.5	-18.5	0.07 (4)	10.00
Q- P	0 / 2091	-18.5	-18.5	0.29 (1)	10.00
P- O	0 / 1831	-18.5	-18.5	0.26 (1)	10.00
O- N	0 / 1831	-18.5	-18.5	0.26 (1)	10.00
N- M	0 / 1831	-18.5	-18.5	0.26 (1)	10.00
M- L	0 / 1831	-18.5	-18.5	0.26 (1)	10.00
L- K	0 / 2091	-18.5	-18.5	0.29 (1)	10.00
K- J	0 / 0	-18.5	-18.5	0.07 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-261 6	0.10 (1)
C- P	-347 0	0.39 (1)
P- D	0 / 355	0.08 (1)
D- N	0 / 549	0.09 (1)
N- E	-754 0	0.39 (1)
N- F	0 / 549	0.09 (1)
L- F	0 / 355	0.08 (1)
L- G	-347 / 0	0.39 (1)
K- G	-261 / 6	0.10 (1)
B- Q	0 / 2121	0.48 (1)
K- H	0 2121	0.48 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.62/1.00 (D-E:1) , BC=0.29/1.00 (K-L:1) , WB=0.48/1.00 (H-K:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (Q) (INPUT = 0.90)
JSI METAL= 0.48 (Q) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	T7	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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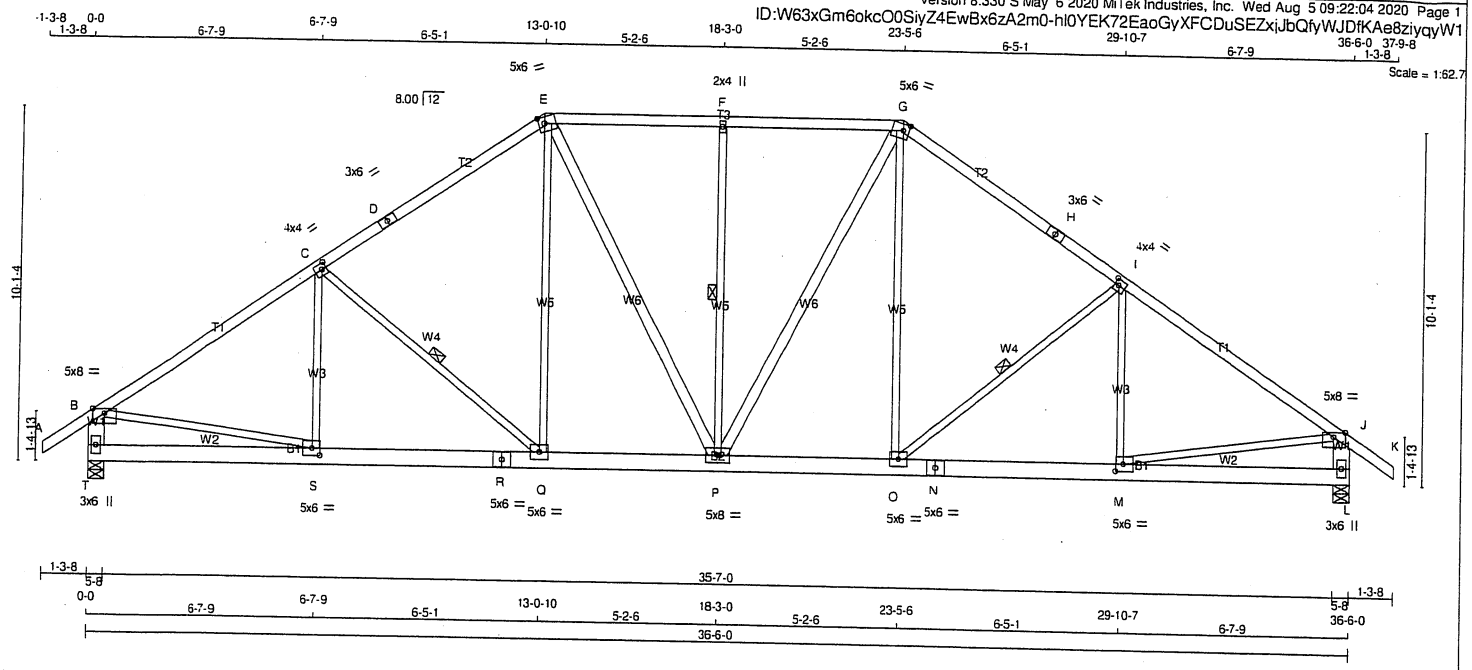

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BS-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.75
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	2138	0	2138	0	0	IN-SX	IN-SX
L	2138	0	2138	0	0	5-8	5-8
						5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1510	1004 / 0	0 / 0	0 / 0	0 / 0	506 0	0 / 0
L	1510	1004 / 0	0 / 0	0 / 0	0 0	506 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-Q, F-P, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-2492 / 0	-91.8	-91.8	0.71 (1)	3.66
C- D	-2132 / 0	-91.8	-91.8	0.64 (1)	3.98
D- E	-2132 . 0	-91.8	-91.8	0.64 (1)	3.98
E- F	-1920 / 0	-91.8	-91.8	0.36 (1)	4.49
F- G	-1920 / 0	-91.8	-91.8	0.36 (1)	4.49
G- H	-2132 / 0	-91.8	-91.8	0.64 (1)	3.98
H- I	-2132 / 0	-91.8	-91.8	0.64 (1)	3.98
I- J	-2492 / 0	-91.8	-91.8	0.71 (1)	3.66
J- K	0 35	-91.8	-91.8	0.12 (1)	10.00
T- B	-2080 : 0	0.0	0.0	0.14 (1)	7.03
L- J	-2080 . 0	0.0	0.0	0.14 (1)	7.03
T- S	0 0	-18.5	-18.5	0.08 (4)	10.00
S- R	0 2107	-18.5	-18.5	0.29 (1)	10.00
R- Q	0 : 2107	-18.5	-18.5	0.29 (1)	10.00
Q- P	0 : 1743	-18.5	-18.5	0.24 (1)	10.00
P- O	0 / 1743	-18.5	-18.5	0.24 (1)	10.00
O- N	0 / 2107	-18.5	-18.5	0.29 (1)	10.00
N- M	0 . 2107	-18.5	-18.5	0.29 (1)	10.00
M- L	0 0	-18.5	-18.5	0.08 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-197 43	0.10 (1)
C- Q	-479 / 0	0.23 (1)
Q- E	0 / 425	0.10 (1)
E- P	0 375	0.06 (1)
P- F	-580 . 0	0.39 (1)
P- G	0 / 375	0.06 (1)
O- G	0 / 425	0.10 (1)
O- I	-479 . 0	0.23 (1)
M- I	-197 . 43	0.10 (1)
B- S	0 / 2131	0.48 (1)
M- J	0 / 2131	0.48 (1)

TOTAL WEIGHT = 3 X 199 = 596 lb

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.71/1.00 (I-J:1) , BC=0.29/1.00 (Q-S:1) , WB=0.48/1.00 (B-S:1) , SSI=0.24/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (M) (INPUT = 0.90)
JSI METAL= 0.48 (S) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2017336

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410068	T8	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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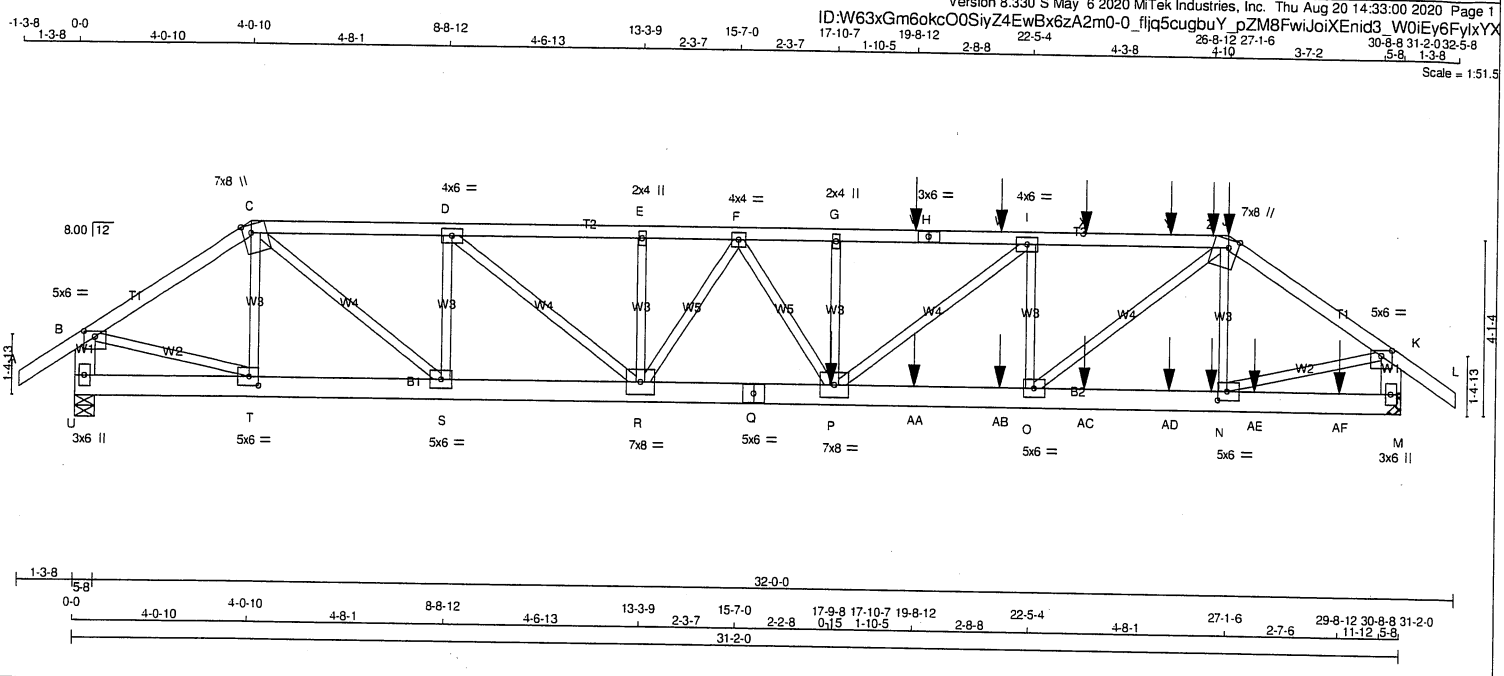

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- H	1	12	SIDE(0.0)
H- J	1	12	SIDE(61.0)
J- L	1	12	SIDE(61.0)
U- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- Q	2	12	TOP
Q- M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
G- P	1	5	SIDE(311.7)
E- R	1	5	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2018795 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2869	0	2869	0	0	5-8	5-8
M	3792	0	3792	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2024	1360 / 0	0 / 0	0 / 0	0 / 0	664 / 0	0 / 0
M	2675	1791 / 0	0 / 0	0 / 0	0 / 0	884 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.07 (1)	10.00
B- C	-3346 / 0	-91.8	-91.8	0.20 (1)	4.92
C- D	-5334 / 0	-91.8	-91.8	0.29 (1)	3.99
D- E	-7162 / 0	-91.8	-91.8	0.39 (1)	3.41
E- F	-7162 / 0	-91.8	-91.8	0.23 (1)	3.53
F- G	-8395 / 0	-91.8	-91.8	0.31 (1)	3.20
G- V	-8395 / 0	-91.8	-91.8	0.61 (1)	2.92
V- H	-8395 / 0	-91.8	-91.8	0.61 (1)	2.92
H- W	-8395 / 0	-91.8	-91.8	0.61 (1)	2.92
W- I	-8395 / 0	-91.8	-91.8	0.61 (1)	2.92
I- X	-6700 / 0	-91.8	-91.8	0.50 (1)	3.38
X- Y	-6700 / 0	-91.8	-91.8	0.50 (1)	3.38
Y- Z	-6700 / 0	-91.8	-91.8	0.50 (1)	3.38
Z- J	-6700 / 0	-91.8	-91.8	0.50 (1)	3.38
J- K	-4538 / 0	-91.8	-91.8	0.24 (1)	4.31
K- L	0 / 35	-91.8	-91.8	0.07 (1)	10.00
U- B	-2841 / 0	0.0	0.0	0.10 (1)	7.81
M- K	-3742 / 0	0.0	0.0	0.13 (1)	7.29
U- T	0 / 0	-18.5	-18.5	0.02 (4)	10.00
T- S	0 / 2764	-18.5	-18.5	0.20 (1)	10.00
S- R	0 / 5334	-18.5	-18.5	0.39 (1)	10.00
R- Q	0 / 7826	-18.5	-18.5	0.58 (1)	10.00
Q- P	0 / 7826	-18.5	-18.5	0.58 (1)	10.00
P-AA	0 / 6700	-18.5	-18.5	0.51 (1)	10.00
AA-AB	0 / 6700	-18.5	-18.5	0.51 (1)	10.00
AB- O	0 / 6700	-18.5	-18.5	0.51 (1)	10.00
O-AC	0 / 3750	-18.5	-18.5	0.27 (1)	10.00
AC-AD	0 / 3750	-18.5	-18.5	0.27 (1)	10.00
AD- N	0 / 3750	-18.5	-18.5	0.27 (1)	10.00
N-AE	0 / 0	-18.5	-18.5	0.03 (4)	10.00
AE-AF	0 / 0	-18.5	-18.5	0.03 (4)	10.00
AF- M	0 / 0	-18.5	-18.5	0.03 (4)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-556 / 0	0.07 (1)
C- S	0 / 3284	0.41 (1)
S- D	-1970 / 0	0.25 (1)
D- R	0 / 2359	0.29 (1)
R- E	-311 / 0	0.04 (1)
P- G	-367 / 0	0.05 (1)
P- I	0 / 2188	0.27 (1)
O- I	-2183 / 0	0.28 (1)
O- J	0 / 3768	0.47 (1)
N- J	-712 / 0	0.09 (1)
B- T	0 / 2858	0.35 (1)
N- K	0 / 3876	0.48 (1)
R- F	-1270 / 0	0.21 (1)
F- P	0 / 1090	0.13 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
J	27-1-6	-51	-57	---	FRONT	VERT	DEAD	---	C1
J	27-1-6	-260	-260	---	FRONT	VERT	SNOW	---	C1
N	26-8-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1

TOTAL WEIGHT = 2 X 147 = 295 lb

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.36")

CSI: TC=0.61/1.00 (G-I:1) , BC=0.58/1.00 (P-R:1) , WB=0.48/1.00 (K-N:1) , SSI=0.18/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (O) (INPUT = 0.90)
JSI METAL= 0.75 (Q) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410068	T8	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	7.0	8.0	Edge	2.50
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.50
K	TMVW-p	MT20	5.0	6.0	1.50	3.00
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.75
O	BMWW-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	7.0	8.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	7.0	8.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.75
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	17-9-8	-1897	-1897	---	FRONT	VERT	TOTAL	---	C1
V	19-8-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	21-8-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	23-8-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	25-8-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	26-8-12	-139	-139	---	FRONT	VERT	TOTAL	---	C1
AA	19-8-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AB	21-8-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AC	23-8-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AD	25-8-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AE	27-8-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AF	29-8-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2018795

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410068	T9	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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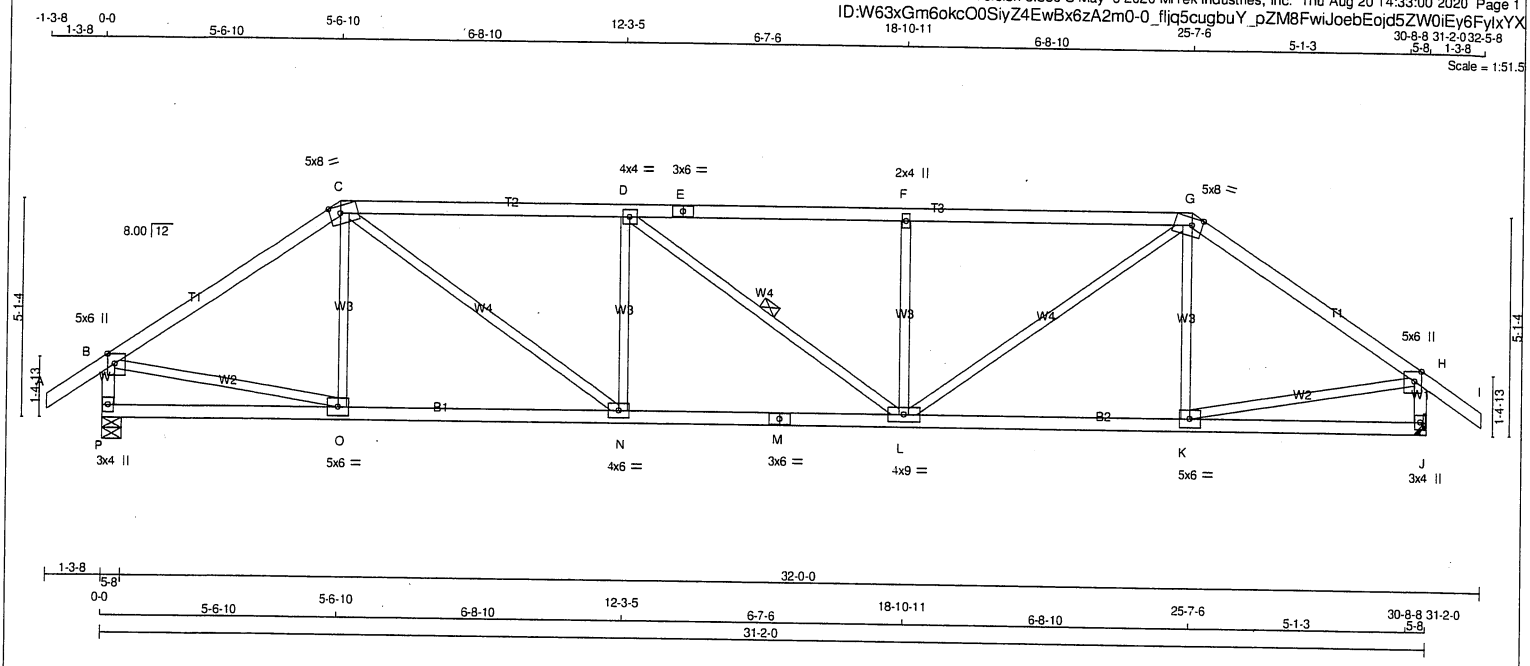

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.75	2.00
C	TTWW-m	MT20	5.0	8.0	2.00	3.00
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	2.00	3.00
H	TMVW+p	MT20	5.0	6.0	2.75	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1844	0	1844	0	0	5-8	5-8
J	1844	0	1844	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1302	868 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0
J	1302	868 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-1990 / 0	-91.8	-91.8	0.66 (1)	4.05
C- D	-2749 / 0	-91.8	-91.8	0.87 (1)	3.32
D- E	-2748 / 0	-91.8	-91.8	0.86 (1)	3.32
E- F	-2748 / 0	-91.8	-91.8	0.86 (1)	3.32
F- G	-2748 / 0	-91.8	-91.8	0.86 (1)	3.33
G- H	-1991 / 0	-91.8	-91.8	0.66 (1)	4.05
H- I	0 / 35	-91.8	-91.8	0.12 (1)	10.00
P- B	-1803 / 0	0.0	0.0	0.19 (1)	6.23
J- H	-1804 / 0	0.0	0.0	0.19 (1)	6.23
P- O	0 / 0	-18.5	-18.5	0.16 (4)	10.00
O- N	0 / 1650	-18.5	-18.5	0.36 (1)	10.00
N- M	0 / 2750	-18.5	-18.5	0.52 (1)	10.00
M- L	0 / 2750	-18.5	-18.5	0.52 (1)	10.00
L- K	0 / 1650	-18.5	-18.5	0.36 (1)	10.00
K- J	0 / 0	-18.5	-18.5	0.16 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-188 / 32	0.07 (1)
C- N	0 / 1351	0.30 (1)
N- D	-664 / 0	0.26 (1)
D- L	-2 / 0	0.00 (1)
L- F	-664 / 0	0.26 (1)
L- G	0 / 1349	0.30 (1)
K- G	-188 / 33	0.07 (1)
B- O	0 / 1684	0.38 (1)
K- H	0 / 1684	0.38 (1)

TOTAL WEIGHT = 122 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.87/1.00 (C-D:1) , BC=0.52/1.00 (L-N:1) , WB=0.38/1.00 (H-K:1) , SSI=0.29/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.90 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018796

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410068	T10	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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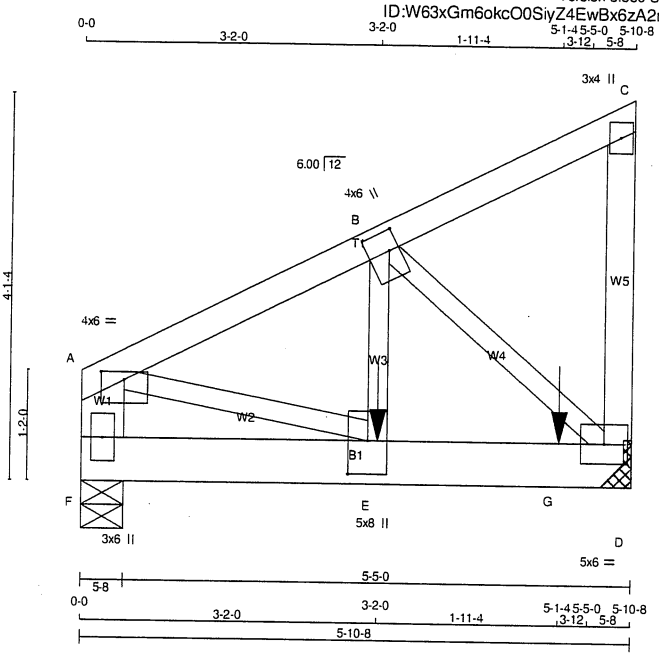

TOTAL WEIGHT = 2 X 29 = 59 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	5	SIDE(549.3)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	2	SIDE(434.9)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW+t	MT20	4.0	6.0	2.50	1.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		
E	BMWW+t	MT20	5.0	8.0	4.25	2.50

LICENSED PROFESSIONAL ENGINEER
08/nov/22
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018797 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	2303	0	2303	0	0	5-8	5-8
D	3945	0	3945	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1625	1090 / 0	0 / 0	0 / 0	0 / 0	535 / 0	0 / 0
D	2782	1869 / 0	0 / 0	0 / 0	0 / 0	913 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-2237 / 0	0.0	0.0	0.08 (1)	7.81	A- E	0 / 2907	0.36 (1)
A- B	-3152 / 0	-91.8	-91.8	0.11 (1)	5.13	E- B	0 / 3578	0.44 (1)
B- C	-6 / 0	-91.8	-91.8	0.05 (1)	10.00	B- D	-3744 / 0	0.43 (1)
D- C	-112 / 0	0.0	0.0	0.01 (1)	7.81			
F- E	0 / 0	-18.5	-18.5	0.04 (1)	10.00			
E- G	0 / 2824	-18.5	-18.5	0.57 (1)	10.00			
G- D	0 / 2824	-18.5	-18.5	0.57 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-2-0	-3773	-3773	---	BACK	VERT	TOTAL	---	C1
G	5-1-4	-1828	-1828	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.57/1.00 (D-E:1) , WB=0.44/1.00 (B-E:1) , SSI=0.46/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.67 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410068	T10	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

LICENSED PROFESSIONAL ENGINEER
08/20/20
H. J/G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018797

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410068	T10Z	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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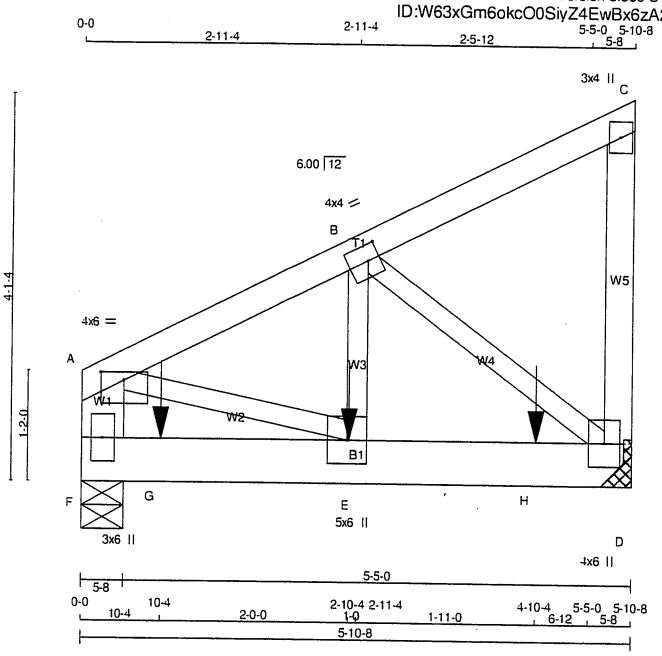

TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	6	SIDE(99.0)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	5.0	6.0		

LICENSED PROFESSIONAL ENGINEER
H. J./G. ALVES
100009024
PROVINCE OF ONTARIO
08/20/20

Structural component only
DWG# T-2018798 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1978	0	1978	0	0	5-8	5-8
D	1915	0	1915	0	0	IN-SX MECHANICAL	IN-SX

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1393	948 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0
D	1349	919 / 0	0 / 0	0 / 0	0 / 0	430 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F- A	-1269 / 0	0.0	0.0	0.04 (1)	7.81
A- B	-1683 / 0	-91.8	-91.8	0.06 (1)	6.25
B- C	-10 / 0	-91.8	-91.8	0.05 (1)	10.00
D- C	-113 / 0	0.0	0.0	0.01 (1)	7.81
F- G	0 / 0	-18.5	-18.5	0.22 (1)	10.00
G- E	0 / 0	-18.5	-18.5	0.22 (1)	10.00
E- H	0 / 1514	-18.5	-18.5	0.34 (1)	10.00
H- D	0 / 1514	-18.5	-18.5	0.34 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- E	0 / 1566	0.19 (1)
E- B	0 / 1600	0.20 (1)
B- D	-1905 / 0	0.23 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-10-4	-1081	-1081	---	BACK	VERT	TOTAL	---	C1
G	10-4	-1083	-1083	---	BACK	VERT	TOTAL	---	C1
H	4-10-4	-1081	-1081	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.34/1.00 (D-E:1) , WB=0.23/1.00 (B-D:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.34 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410068	T10Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

LICENSED PROFESSIONAL ENGINEER
08/20/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018798

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410069	T10Z1	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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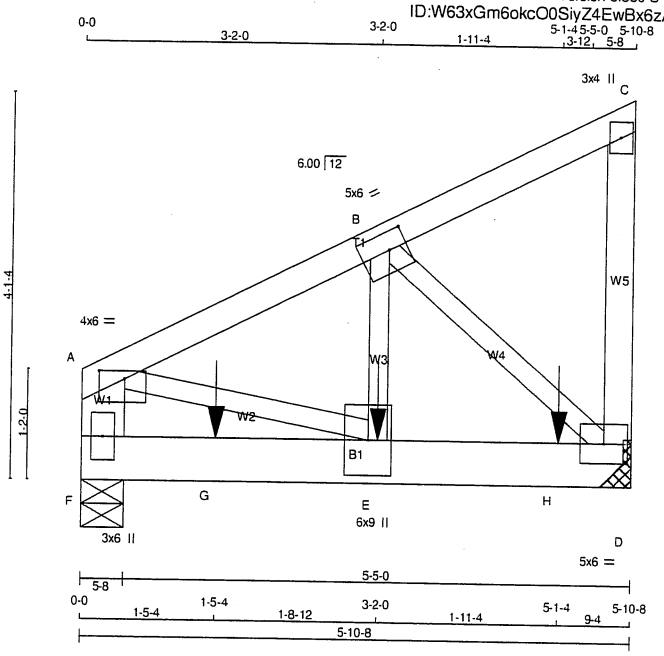

TOTAL WEIGHT = 2 X 29 = 59 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	4	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	2	SIDE(453.4)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.25
B	TMWW-t	MT20	5.0	6.0	2.25	2.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		
E	BMWW+t	MT20	6.0	9.0		

Structural component only
DWG# T-2018811

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	2607	0	2607	0	0	IN-SX	IN-SX
D	4150	0	4150	0	0	5-8	5-8
						MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1836	1244 / 0	0 / 0	0 / 0	0 / 0	592 / 0	0 / 0
D	2926	1966 / 0	0 / 0	0 / 0	0 / 0	960 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F- A	-2476 / 0	0.0	0.0	0.09 (1)	7.81
A- B	-3508 / 0	-91.8	-91.8	0.12 (1)	4.91
B- C	-4 / 0	-91.8	-91.8	0.05 (1)	10.00
D- C	-115 / 0	0.0	0.0	0.01 (1)	7.81
F- G	0 / 0	-18.5	-18.5	0.06 (1)	10.00
G- E	0 / 0	-18.5	-18.5	0.06 (1)	10.00
E- H	0 / 3141	-18.5	-18.5	0.57 (1)	10.00
H- D	0 / 3141	-18.5	-18.5	0.57 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- E	0 / 3234	0.40 (1)
E- B	0 / 4018	0.50 (1)
B- D	-4165 / 0	0.48 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-2-0	-4312	-4312	---	BACK	VERT	TOTAL	---	C1
G	1-5-4	-95	-95	---	BACK	VERT	TOTAL	---	C1
H	5-1-4	-1702	-1702	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.57/1.00 (D-E:1) , WB=0.50/1.00 (B-E:1) , SSI=0.43/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.51 (E) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410069	T10Z1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

LICENSED PROFESSIONAL ENGINEER
H. J. GALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018811 Y2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410068	T11	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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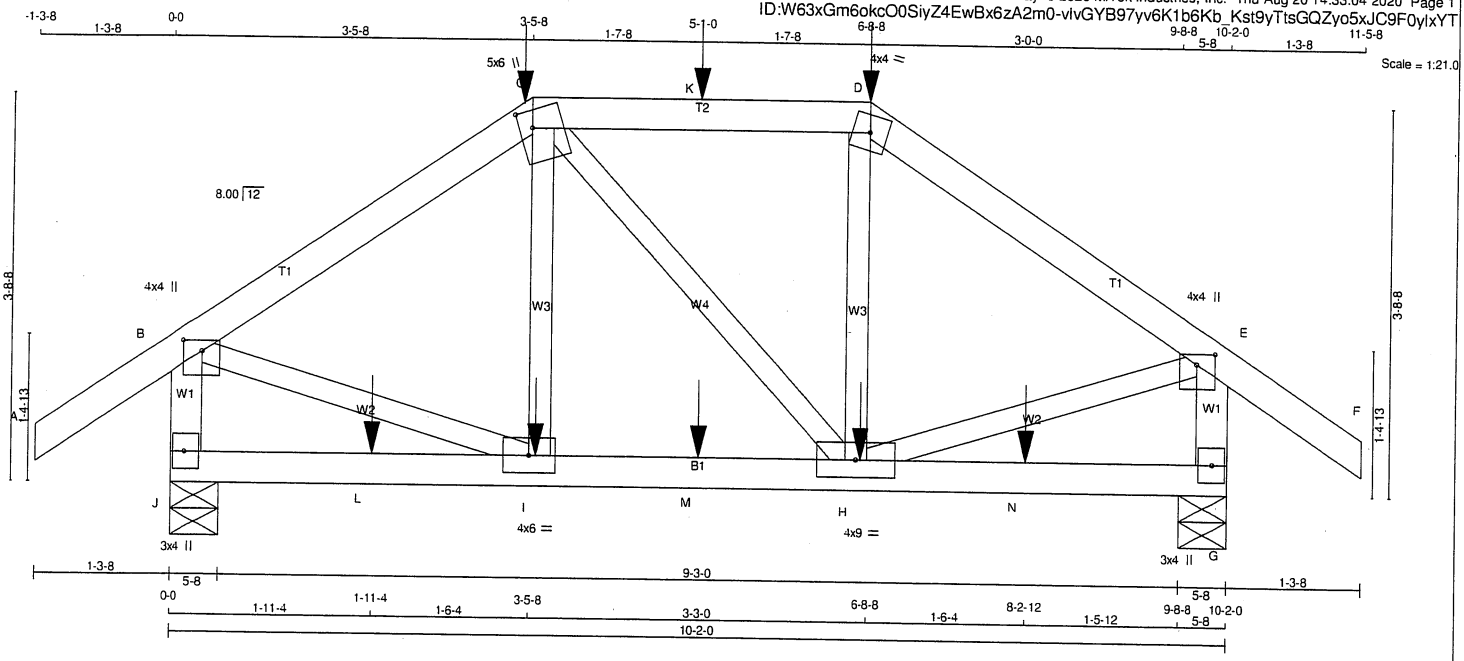

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	960	0	960	0	0	5-8	5-8
G	960	0	960	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	676	460 / 0	0 / 0	0 / 0	0 / 0	216 / 0	0 / 0
G	676	460 / 0	0 / 0	0 / 0	0 / 0	216 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.14 (1)	10.00	I- C	-105 / 40	0.02 (1)
B- C	-794 / 0	-91.8	-91.8	0.22 (1)	6.25	C- H	0 / 0	0.00 (1)
C- K	-656 / 0	-91.8	-91.8	0.27 (1)	6.25	H- D	-106 / 41	0.02 (1)
K- D	-656 / 0	-91.8	-91.8	0.27 (1)	6.25	B- I	0 / 689	0.17 (1)
D- E	-793 / 0	-91.8	-91.8	0.22 (1)	6.25	H- E	0 / 688	0.17 (1)
E- F	0 / 35	-91.8	-91.8	0.14 (1)	10.00			
J- B	-927 / 0	0.0	0.0	0.10 (1)	7.81			
G- E	-926 / 0	0.0	0.0	0.10 (1)	7.81			
J- L	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
L- I	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
I- M	0 / 656	-18.5	-18.5	0.15 (1)	10.00			
M- H	0 / 656	-18.5	-18.5	0.15 (1)	10.00			
H- N	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
N- G	0 / 0	-18.5	-18.5	0.07 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-5-8	-207	-207	---	BACK	VERT	TOTAL	---	C1
D	6-8-8	-207	-207	---	BACK	VERT	TOTAL	---	C1
H	6-7-12	-13	-13	---	BACK	VERT	TOTAL	---	C1
I	3-6-4	-13	-13	---	BACK	VERT	TOTAL	---	C1
K	5-1-0	-67	-67	---	BACK	VERT	TOTAL	---	C1
L	1-11-4	-13	-13	---	BACK	VERT	TOTAL	---	C1
M	5-1-0	-13	-13	---	BACK	VERT	TOTAL	---	C1
N	8-2-12	-13	-13	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 45 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.27/1.00 (C-D:1) , BC=0.15/1.00 (H-I:1) , WB=0.17/1.00 (B-I:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (B) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018799

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410068	T12S	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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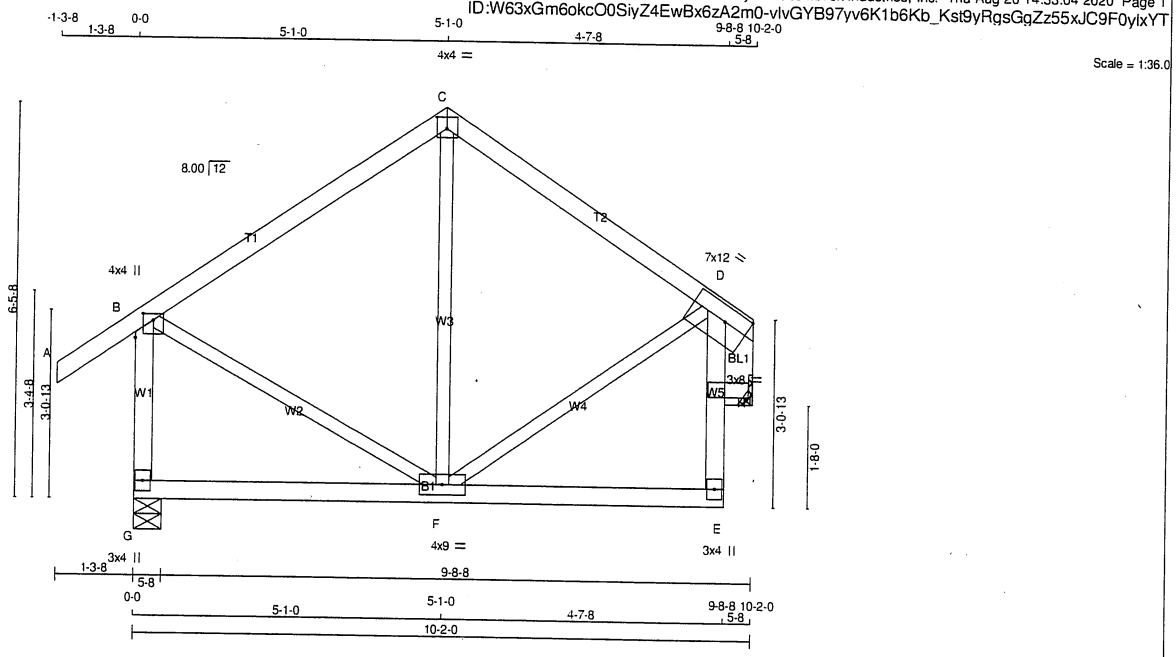

TOTAL WEIGHT = 49 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TP-p	MT20	3.0	8.0	3.50	3.50
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVWK1-t	MT20	7.0	12.0	3.00	Edge
E	BMV+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I(D)	517	0	517	0	0	MECHANICAL	
G	679	0	679	0	0	5-8	5-8

(** SEE "BEARING NOTE" **)

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I(D). MINIMUM BEARING LENGTH AT JOINT I(D) = 3-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I(D)	366	240 / 0	0 / 0	0 / 0	0 / 0	126 / 0	0 / 0
G	478	327 / 0	0 / 0	0 / 0	0 / 0	151 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B-C	-282 / 0	-91.8	-91.8	0.41 (1)	6.25
C-D	-282 / 0	-91.8	-91.8	0.34 (1)	6.25
G-B	-642 / 0	0.0	0.0	0.11 (1)	7.81
E-H	0 / 35	0.0	0.0	0.12 (1)	10.00
H-D	0 35	0.0	0.0	0.12 (1)	10.00
G-F	0 / 0	-18.5	-18.5	0.13 (4)	10.00
F-E	0 57	-18.5	-18.5	0.13 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	-133 / 34	0.09 (1)
B-F	0 / 266	0.06 (1)
F-D	0 / 213	0.05 (1)
D-I	-530 / 0	0.04 (1)
H-I	0 / 113	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.41/1.00 (B-C:1) , BC=0.13/1.00 (E-F:4) , WB=0.09/1.00 (C-F:1) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

Structural component only
DWG# T-2018800

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410068	T13S	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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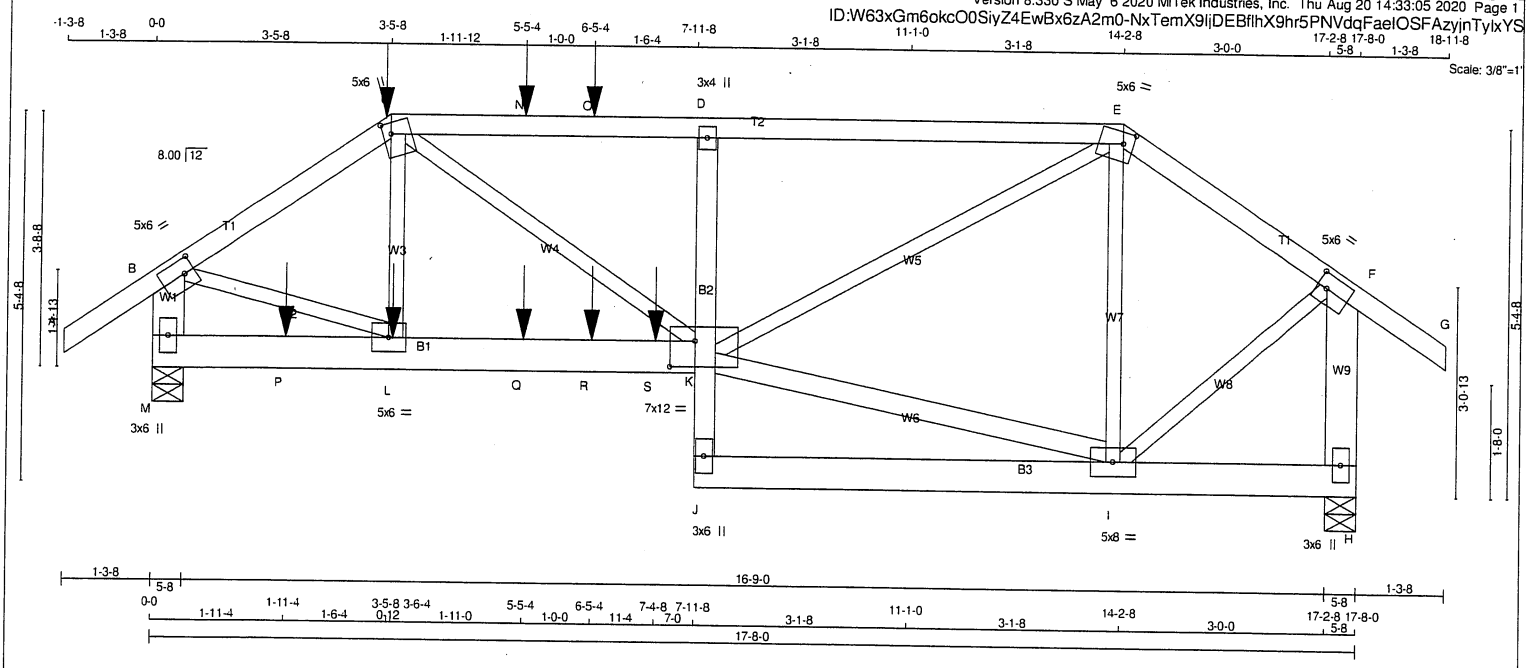

TOTAL WEIGHT = 2 X 97 = 194 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
J - D	2x4	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- E	1	12	SIDE(61.0)
E- G	1	12	TOP
M- B	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- K	2	12	SIDE(183.1)
J- H	2	12	TOP
D- J	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2018801 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1681	0	1681	0	0	5-8	5-8
H	1403	0	1403	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1185	801 / 0	0 / 0	0 / 0	0 / 0	383 / 0	0 / 0
H	989	669 / 0	0 / 0	0 / 0	0 / 0	319 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	L- C	-162 / 47	0.02 (1)
B- C	-1762 / 0	-91.8	-91.8	0.12 (1)	6.25	C- K	0 / 989	0.12 (1)
C- N	-2262 / 0	-91.8	-91.8	0.31 (1)	5.55	K- I	0 / 761	0.07 (1)
N- O	-2262 / 0	-91.8	-91.8	0.31 (1)	5.55	K- E	0 / 1682	0.21 (1)
O- D	-2262 / 0	-91.8	-91.8	0.31 (1)	5.55	I- E	-670 / 0	0.14 (1)
D- E	-2245 / 0	-91.8	-91.8	0.32 (1)	5.57	B- L	0 / 1519	0.19 (1)
E- F	-928 / 0	-91.8	-91.8	0.11 (1)	6.25	I- F	0 / 971	0.12 (1)
F- G	0 / 35	-91.8	-91.8	0.07 (1)	10.00			
M- B	-1676 / 0	0.0	0.0	0.06 (1)	7.81			
H- F	-1391 / 0	0.0	0.0	0.08 (1)	7.81			
M- P	0 / 0	-18.5	-18.5	0.04 (1)	10.00			
P- L	0 / 0	-18.5	-18.5	0.04 (1)	10.00			
L- Q	0 / 1458	-18.5	-18.5	0.21 (1)	10.00			
Q- R	0 / 1458	-18.5	-18.5	0.21 (1)	10.00			
R- S	0 / 1458	-18.5	-18.5	0.21 (1)	10.00			
S- K	0 / 1458	-18.5	-18.5	0.21 (1)	10.00			
J- K	0 / 51	0.0	0.0	0.05 (1)	10.00			
K- D	-691 / 0	0.0	0.0	0.05 (1)	7.81			
J- I	0 / 34	-18.5	-18.5	0.04 (4)	10.00			
I- H	0 / 0	-18.5	-18.5	0.04 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-5-8	-207	-207	---	FRONT	VERT	TOTAL	---	C1
L	3-6-4	-13	-13	---	FRONT	VERT	TOTAL	---	C1
N	5-5-4	-67	-67	---	FRONT	VERT	TOTAL	---	C1
O	6-5-4	-67	-67	---	FRONT	VERT	TOTAL	---	C1
P	1-11-4	-13	-13	---	FRONT	VERT	TOTAL	---	C1
Q	5-5-4	-13	-13	---	FRONT	VERT	TOTAL	---	C1
R	6-5-4	-13	-13	---	FRONT	VERT	TOTAL	---	C1
S	7-4-8	-490	-490	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.32/1.00 (D-E:1) , BC=0.21/1.00 (K-L:1) , WB=0.21/1.00 (E-K:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (E) (INPUT = 0.90)
JSI METAL= 0.22 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410068	T13S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.75
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMV+p	MT20	3.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.00	1.75
F	TMVW-t	MT20	5.0	6.0	2.50	1.75
H	BMV1+p	MT20	3.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMV+p	MT20	3.0	6.0		
K	BVMWWW-l	MT20	7.0	12.0	4.50	4.50
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2018801

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410068	T14	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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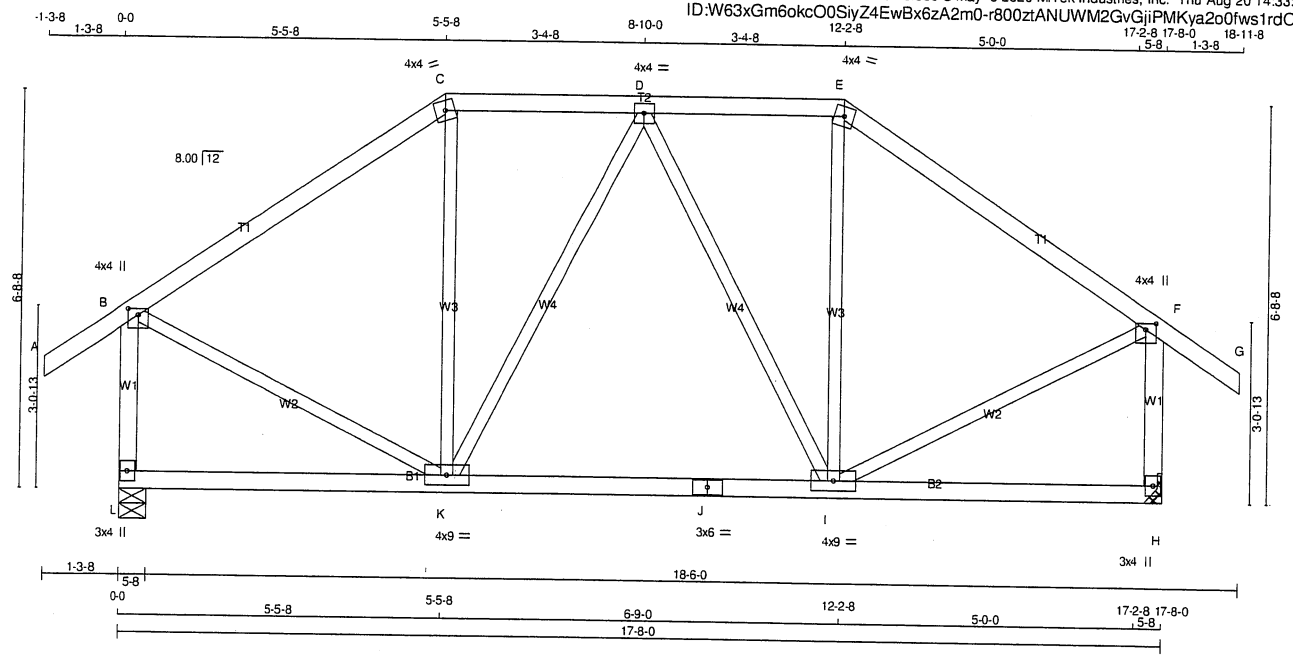

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMV1+p	MT20	3.0	4.0		

Structural component only
DWG# T-2018802

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1100	0	1100	0	0	5-8	5-8
H	1100	0	1100	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	775	522 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
H	775	522 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-707 / 0	-91.8	-91.8	0.36 (1)	6.25
C- D	-588 / 0	-91.8	-91.8	0.13 (1)	6.25
D- E	-588 / 0	-91.8	-91.8	0.13 (1)	6.25
E- F	-707 / 0	-91.8	-91.8	0.36 (1)	6.25
F- G	0 / 35	-91.8	-91.8	0.12 (1)	10.00
L- B	-1060 / 0	0.0	0.0	0.18 (1)	7.64
H- F	-1060 / 0	0.0	0.0	0.18 (1)	7.64
L- K	0 / 0	-18.5	-18.5	0.16 (4)	10.00
K- J	0 / 684	-18.5	-18.5	0.21 (4)	10.00
J- I	0 / 684	-18.5	-18.5	0.21 (4)	10.00
I- H	0 / 0	-18.5	-18.5	0.16 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 70	0.02 (4)
K- D	-212 / 0	0.21 (1)
D- I	-212 / 0	0.21 (1)
I- E	0 / 70	0.02 (4)
B- K	0 / 655	0.15 (1)
I- F	0 / 655	0.15 (1)

TOTAL WEIGHT = 83 lb

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.36/1.00 (B-C:1) , BC=0.21/1.00 (I-K:4) , WB=0.21/1.00 (D-I:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (B) (INPUT = 0.90)
JSI METAL= 0.25 (J) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410068	T15	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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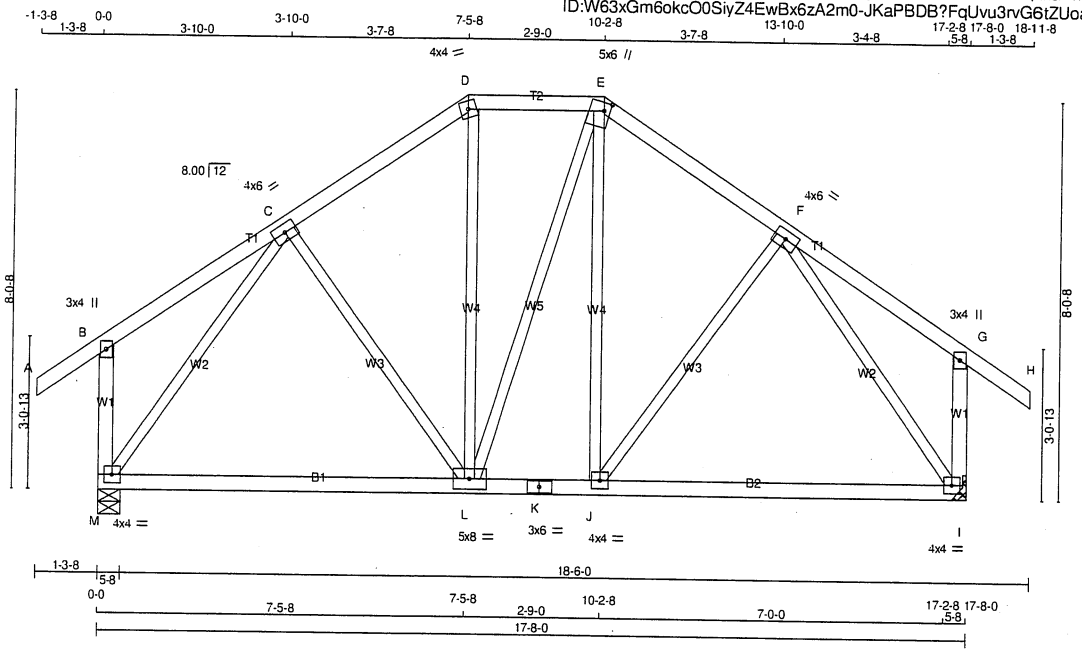

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT		
M	1100	0	1100	0	0	5-8	5-8
I	1100	0	1100	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	775	522 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
I	775	522 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 22	-91.8	-91.8	0.21 (1)	10.00
C- D	-688 / 0	-91.8	-91.8	0.16 (1)	6.25
D- E	-554 / 0	-91.8	-91.8	0.09 (1)	6.25
E- F	-688 / 0	-91.8	-91.8	0.16 (1)	6.25
F- G	0 / 22	-91.8	-91.8	0.21 (1)	10.00
G- H	0 / 35	-91.8	-91.8	0.12 (1)	10.00
M- B	-258 / 0	0.0	0.0	0.04 (1)	7.81
I- G	-258 / 0	0.0	0.0	0.04 (1)	7.81
M- L	0 / 560	-18.5	-18.5	0.26 (4)	10.00
L- K	0 / 554	-18.5	-18.5	0.26 (4)	10.00
K- J	0 / 554	-18.5	-18.5	0.26 (4)	10.00
J- I	0 / 560	-18.5	-18.5	0.26 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-12 / 41	0.01 (4)
L- D	0 / 115	0.03 (4)
L- E	0 / 0	0.00 (1)
J- E	0 / 113	0.03 (4)
J- F	-12 / 41	0.01 (4)
M- C	-963 / 0	0.69 (1)
F- I	-963 / 0	0.69 (1)

TOTAL WEIGHT = 92 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.21/1.00 (F-G:1) , BC=0.26/1.00 (J-L:4) , WB=0.69/1.00 (C-M:1) , SSI=0.14/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (M) (INPUT = 0.90)
JSI METAL= 0.43 (K) (INPUT = 1.00)

Structural component only
DWG# T-2018803

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410068	T16	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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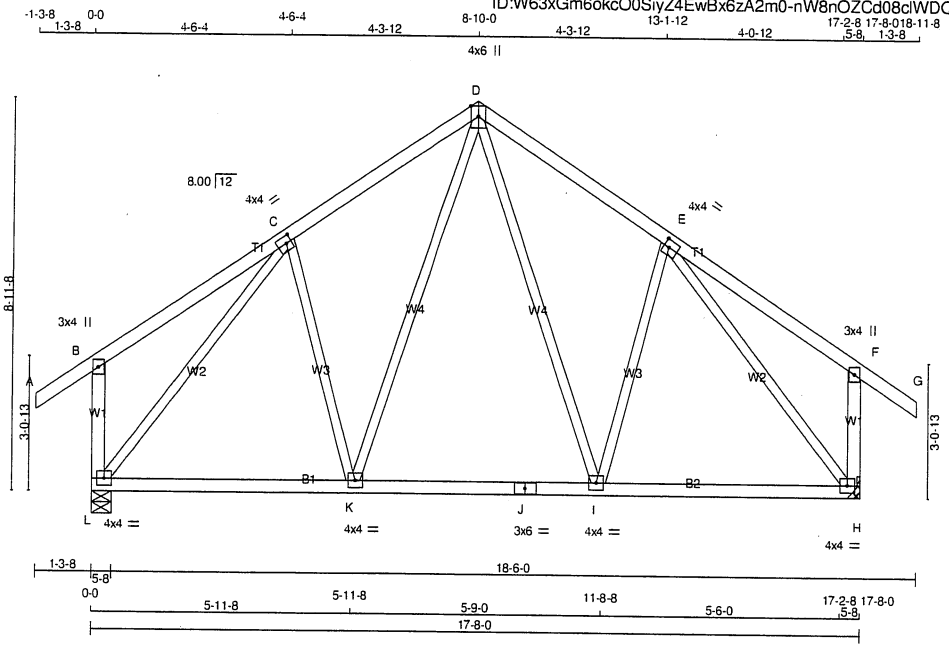

TOTAL WEIGHT = 89 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1100	0	1100	0	0	IN-SX	IN-SX
H	1100	0	1100	0	0	5-8 MECHANICAL	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	775	522 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
H	775	522 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)		10.00
B- C	0 / 26	-91.8	-91.8	0.29 (1)		10.00
C- D	-715 / 0	-91.8	-91.8	0.23 (1)		6.25
D- E	-715 / 0	-91.8	-91.8	0.23 (1)		6.25
E- F	0 / 26	-91.8	-91.8	0.29 (1)		10.00
F- G	0 / 35	-91.8	-91.8	0.12 (1)		10.00
L- B	-284 / 0	0.0	0.0	0.05 (1)		7.81
H- F	-284 / 0	0.0	0.0	0.05 (1)		7.81
L- K	0 / 602	-18.5	-18.5	0.19 (4)		10.00
K- J	0 / 496	-18.5	-18.5	0.19 (4)		10.00
J- I	0 / 496	-18.5	-18.5	0.19 (4)		10.00
I- H	0 / 602	-18.5	-18.5	0.19 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 243	0.05 (1)
I- E	-117 / 26	0.07 (1)
K- D	0 / 243	0.05 (1)
C- K	-117 / 26	0.07 (1)
L- C	-977 / 0	0.95 (1)
E- H	-977 / 0	0.95 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.19/1.00 (H-I:4) , WB=0.95/1.00 (C-L:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (E) (INPUT = 0.90)
JSI METAL= 0.33 (C) (INPUT = 1.00)

Structural component only
DWG# T-2018804

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410068	T17	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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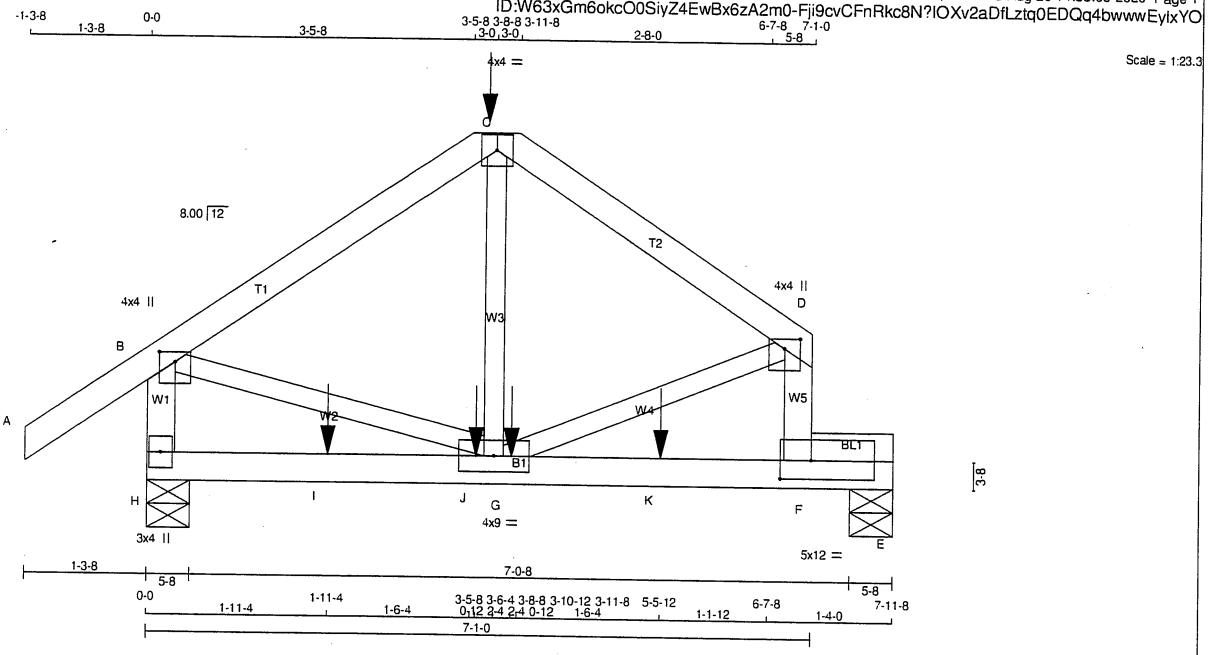

TOTAL WEIGHT = 32 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMVKm	MT20	5.0	12.0	2.25	4.00
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	818	0	818	0	0	5-8	5-8
E	588	0	588	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	576	390 / 0	0 / 0	0 / 0	0 / 0	186 / 0	0 / 0
E	417	265 / 0	0 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.14 (1)	10.00
B- C	-655 / 0	-91.8	-91.8	0.25 (1)	6.25
C- D	-653 / 0	-91.8	-91.8	0.21 (1)	6.25
H- B	-812 / 0	0.0	0.0	0.09 (1)	7.81
F- D	-736 / 0	0.0	0.0	0.09 (1)	7.81
H- I	0 / 0	-18.5	-18.5	0.22 (1)	10.00
I- J	0 / 0	-18.5	-18.5	0.22 (1)	10.00
J- G	0 / 0	-18.5	-18.5	0.22 (1)	10.00
G- K	0 / 0	-18.5	-18.5	0.76 (1)	10.00
K- F	0 / 0	-18.5	-18.5	0.76 (1)	10.00
F- E	0 / 0	-18.5	-18.5	0.76 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- G	0 / 566	0.14 (1)
G- D	0 / 586	0.15 (1)
G- C	-28 / 89	0.03 (4)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-8-8	-429	-429	---	FRONT	VERT	TOTAL	---	C1
G	3-10-12	-13	-13	---	FRONT	VERT	TOTAL	---	C1
I	1-11-4	-13	-13	---	FRONT	VERT	TOTAL	---	C1
J	3-6-4	-13	-13	---	FRONT	VERT	TOTAL	---	C1
K	5-5-12	-13	-13	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.27").
CALCULATED VERT. DEFL.(TL) = L/ 640 (0.15")

CSI: TC=0.25/1.00 (B-C:1) , BC=0.76/1.00 (F-G:1) , WB=0.15/1.00 (D-G:1) , SSI=0.50/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018805

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410068	T18	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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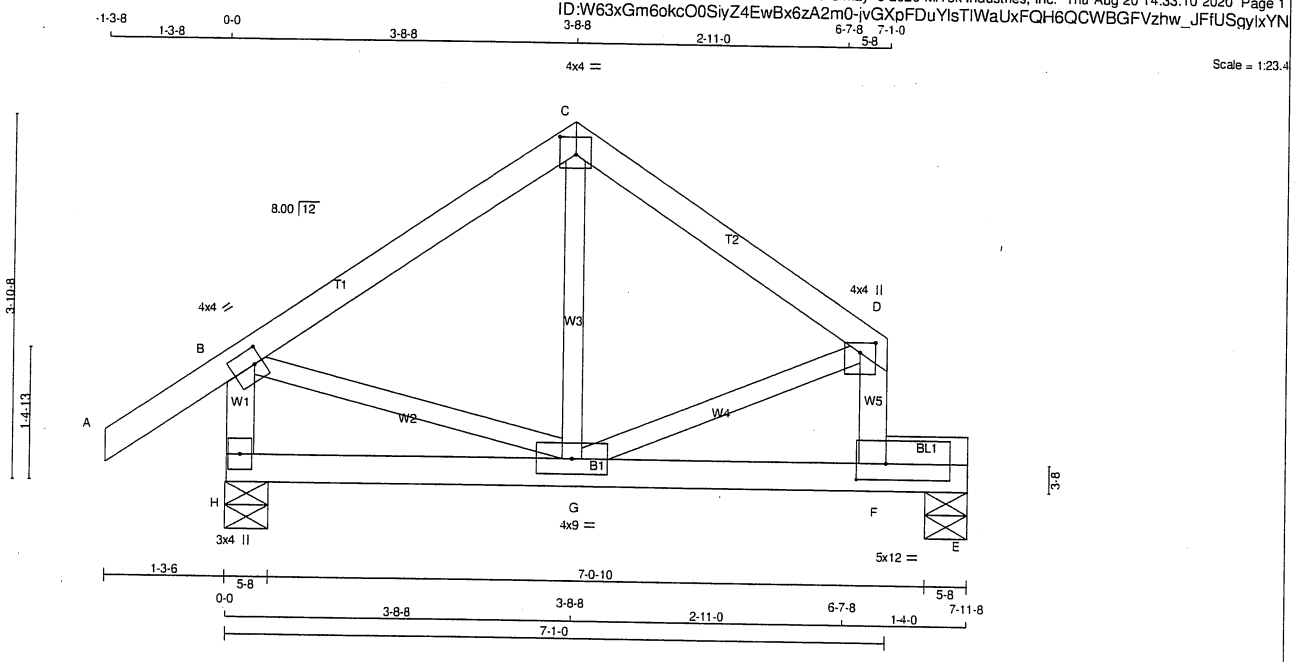

TOTAL WEIGHT = 32 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMVKm	MT20	5.0	12.0	2.00	3.75
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	560	0	560	0	0	5-8 (5-6)	5-8
E	363	0	363	0	0	5-8	5-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	394	271 / 0	0 / 0	0 / 0	0 / 0	123 / 0	0 / 0
E	258	161 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	G- C	0 / 72	0.02 (4)
B- C	-321 / 0	-91.8	-91.8	0.22 (1)	6.25	B- G	0 / 277	0.06 (1)
C- D	-323 / 0	-91.8	-91.8	0.18 (1)	6.25	G- D	0 / 289	0.07 (1)
H- B	-549 / 0	0.0	0.0	0.06 (1)	7.81			
F- D	-442 / 0	0.0	0.0	0.05 (1)	7.81			
H- G	0 / 0	-18.5	-18.5	0.13 (1)	10.00			
G- F	0 / 0	-18.5	-18.5	0.42 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.42 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.42/1.00 (F-G:1) , WB=0.07/1.00 (D-G:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

Structural component only
DWG# T-2018806

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410068	T19	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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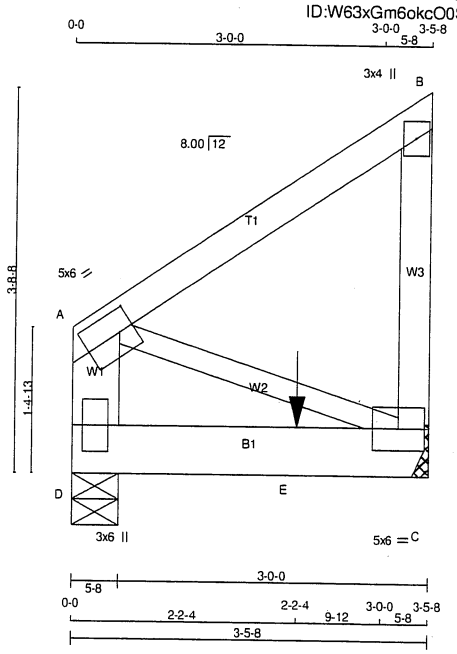


Scale = 1:21.2

TOTAL WEIGHT = 2 X 18 = 37 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x6	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	2	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

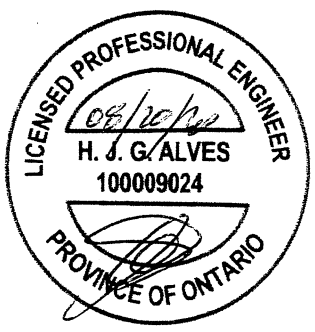
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	1.75
B	TMV+p	MT20	3.0	4.0		
C	BMVW1-t	MT20	5.0	6.0		
D	BMV1+p	MT20	3.0	6.0		



Structural component only
DWG# T-2018807

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	374	0	374	0	0	5-8	5-8
C	506	0	506	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	264	177 / 0	0 / 0	0 / 0	0 / 0	87 / 0	0 / 0
C	357	240 / 0	0 / 0	0 / 0	0 / 0	117 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- A	-159 / 0	0.0	0.0	0.01 (1)	7.81	A- C	0 / 0	0.00 (1)
A- B	0 / 0	-91.8	-91.8	0.10 (1)	10.00			
C- B	-159 / 0	0.0	0.0	0.02 (1)	7.81			
D- E	0 / 0	-18.5	-18.5	0.16 (1)	10.00			
E- C	0 / 0	-18.5	-18.5	0.16 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-2-4	-499	-499	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.10/1.00 (A-B:1) , BC=0.16/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.03 (B) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410069	T20	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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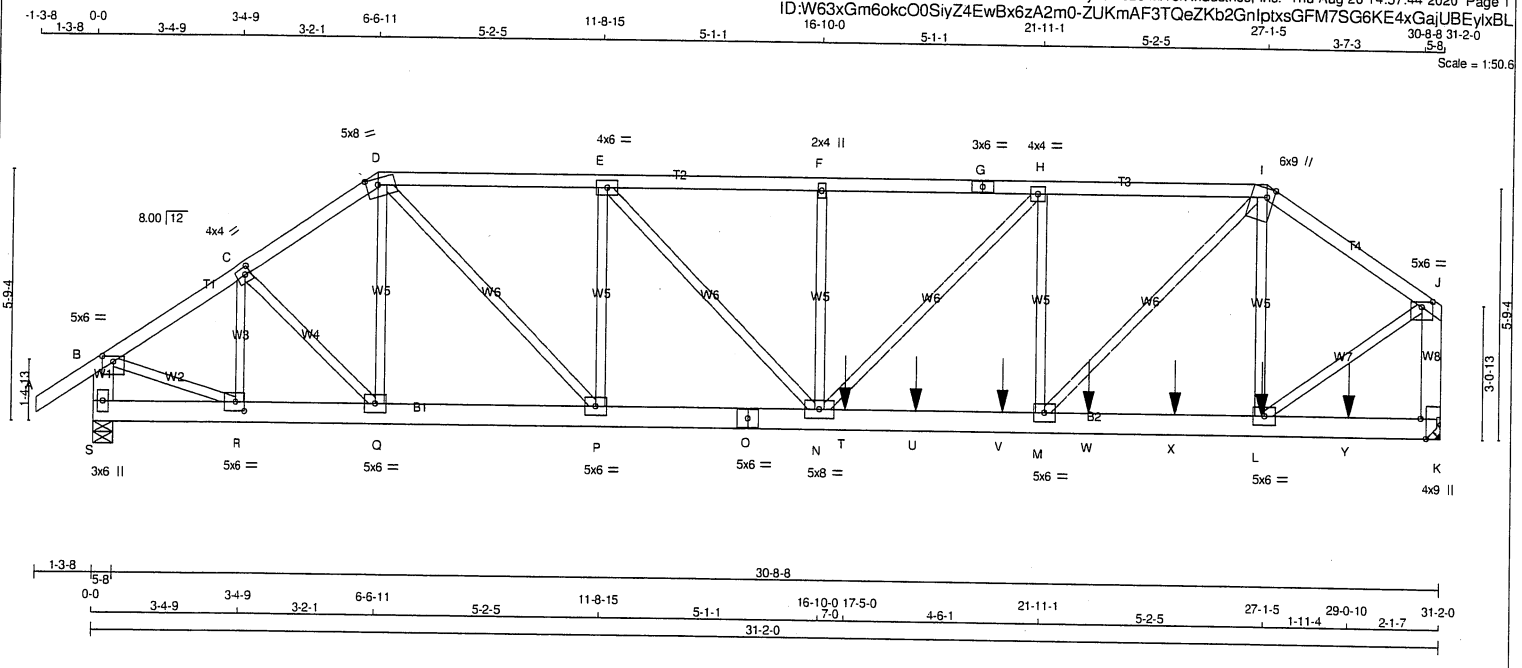

TOTAL WEIGHT = 2 X 157 = 315 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - J	2x6	DRY	No.2	SPF
S - O	2x6	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- G	1	12	TOP
G- I	1	12	TOP
I- J	1	12	TOP
S- B	2	12	TOP
K- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- O	2	12	TOP
O- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

Structural component only

DWG# T-2018812 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
S	3123	0	3123	0	0	5-8	IN-SX
K	4330	0	4330	0	0	MECHANICAL	IN-SX

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2202	1483 / 0	0 / 0	0 / 0	0 / 0	719 / 0	0 / 0
K	3052	2060 / 0	0 / 0	0 / 0	0 / 0	992 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.07 (1)	10.00
B- C	-3490 / 0	-91.8	-91.8	0.12 (1)	4.93
C- D	-3915 / 0	-91.8	-91.8	0.13 (1)	4.69
D- E	-5135 / 0	-91.8	-91.8	0.33 (1)	4.01
E- F	-6557 / 0	-91.8	-91.8	0.42 (1)	3.53
F- G	-6557 / 0	-91.8	-91.8	0.41 (1)	3.54
G- H	-6557 / 0	-91.8	-91.8	0.41 (1)	3.54
H- I	-5584 / 0	-91.8	-91.8	0.35 (1)	3.85
I- J	-3645 / 0	-91.8	-91.8	0.21 (1)	4.75
S- B	-3057 / 0	0.0	0.0	0.11 (1)	7.81
K- J	-4155 / 0	0.0	0.0	0.23 (1)	7.00
S- R	0 / 0	-18.5	-18.5	0.04 (1)	10.00
R- Q	0 / 2917	-18.5	-18.5	0.23 (1)	10.00
Q- P	0 / 3234	-18.5	-18.5	0.26 (1)	10.00
P- O	0 / 5136	-18.5	-18.5	0.49 (1)	10.00
O- N	0 / 5136	-18.5	-18.5	0.49 (1)	10.00
N- T	0 / 5584	-18.5	-18.5	0.63 (1)	10.00
T- U	0 / 5584	-18.5	-18.5	0.63 (1)	10.00
U- V	0 / 5584	-18.5	-18.5	0.63 (1)	10.00
V- M	0 / 5584	-18.5	-18.5	0.63 (1)	10.00
M- W	0 / 3008	-18.5	-18.5	0.39 (1)	10.00
W- X	0 / 3008	-18.5	-18.5	0.39 (1)	10.00
X- L	0 / 3008	-18.5	-18.5	0.39 (1)	10.00
L- Y	0 / 0	-18.5	-18.5	0.12 (1)	10.00
Y- K	0 / 0	-18.5	-18.5	0.12 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-904 / 0	0.10 (1)
C- Q	0 / 451	0.06 (1)
Q- D	-119 / 20	0.03 (1)
D- P	0 / 2738	0.34 (1)
P- E	-2036 / 0	0.50 (1)
E- N	0 / 2072	0.26 (1)
N- F	-422 / 0	0.10 (1)
N- H	0 / 1418	0.18 (1)
M- H	-1555 / 0	0.38 (1)
M- I	0 / 3710	0.46 (1)
L- I	-1035 / 0	0.25 (1)
B- R	0 / 3041	0.38 (1)
L- J	0 / 3596	0.45 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
L	27-0-10	-305	-305	---	FRONT	VERT	TOTAL	---	C1
T	17-5-0	-1930	-1930	---	FRONT	VERT	TOTAL	---	C1
U	19-0-10	-305	-305	---	FRONT	VERT	TOTAL	---	C1
V	21-0-10	-305	-305	---	FRONT	VERT	TOTAL	---	C1
W	23-0-10	-305	-305	---	FRONT	VERT	TOTAL	---	C1
X	25-0-10	-305	-305	---	FRONT	VERT	TOTAL	---	C1
Y	29-0-10	-435	-435	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.42/1.00 (E-F:1) , BC=0.63/1.00 (M-N:1) , WB=0.50/1.00 (E-P:1) . SSI=0.65/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.52 (O) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410069	T20	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	8.0	1.75	3.25
E	TMWW-t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	9.0	Edge	2.00
J	TMVW-p	MT20	5.0	6.0	1.50	3.00
K	BMV1+t	MT20	4.0	9.0	Edge	1.50
L, M, P, Q						
L	BMWW-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BS-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.75
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2018812 JJ2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410069	T21	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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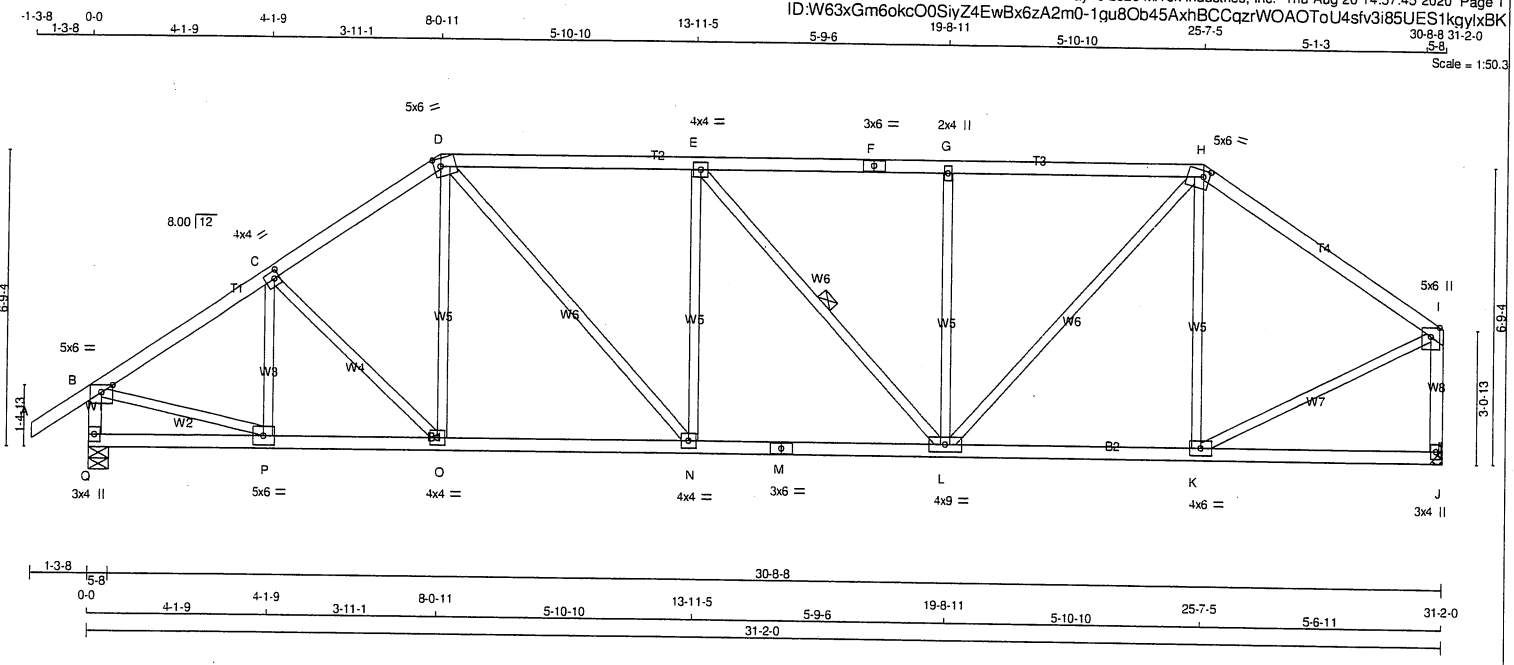

TOTAL WEIGHT = 136 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	1.75	1.75
I	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2018813

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	1844	0	1844	0	0	5-8	5-8
J	1718	0	1718	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1302	868 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0
J	1215	797 / 0	0 / 0	0 / 0	0 / 0	418 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-1963 / 0	-91.8	-91.8	0.31 (1)	4.52
C- D	-1941 / 0	-91.8	-91.8	0.30 (1)	4.54
D- E	-2098 / 0	-91.8	-91.8	0.56 (1)	4.09
E- F	-1972 / 0	-91.8	-91.8	0.54 (1)	4.21
F- G	-1972 / 0	-91.8	-91.8	0.54 (1)	4.21
G- H	-1972 / 0	-91.8	-91.8	0.54 (1)	4.22
H- I	-1482 / 0	-91.8	-91.8	0.59 (1)	4.63
Q- B	-1808 / 0	0.0	0.0	0.19 (1)	6.22
J- I	-1677 / 0	0.0	0.0	0.28 (1)	6.41
Q- P	0 / 0	-18.5	-18.5	0.07 (4)	10.00
P- O	0 / 1652	-18.5	-18.5	0.32 (1)	10.00
O- N	0 / 1595	-18.5	-18.5	0.32 (1)	10.00
N- M	0 / 2098	-18.5	-18.5	0.40 (1)	10.00
M- L	0 / 2098	-18.5	-18.5	0.40 (1)	10.00
L- K	0 / 1224	-18.5	-18.5	0.28 (1)	10.00
K- J	0 / 0	-18.5	-18.5	0.15 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-362 / 0	0.09 (1)
C- O	-83 / 0	0.04 (1)
O- D	0 / 155	0.04 (4)
D- N	0 / 747	0.17 (1)
N- E	-442 / 0	0.34 (1)
E- L	-189 / 0	0.10 (1)
L- G	-581 / 0	0.45 (1)
L- H	0 / 1111	0.25 (1)
K- H	-476 / 0	0.36 (1)
B- P	0 / 1708	0.38 (1)
K- I	0 / 1369	0.31 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.59/1.00 (H-I:1) , BC=0.40/1.00 (L-N:1) , WB=0.45/1.00 (G-L:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.66 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410069	T22	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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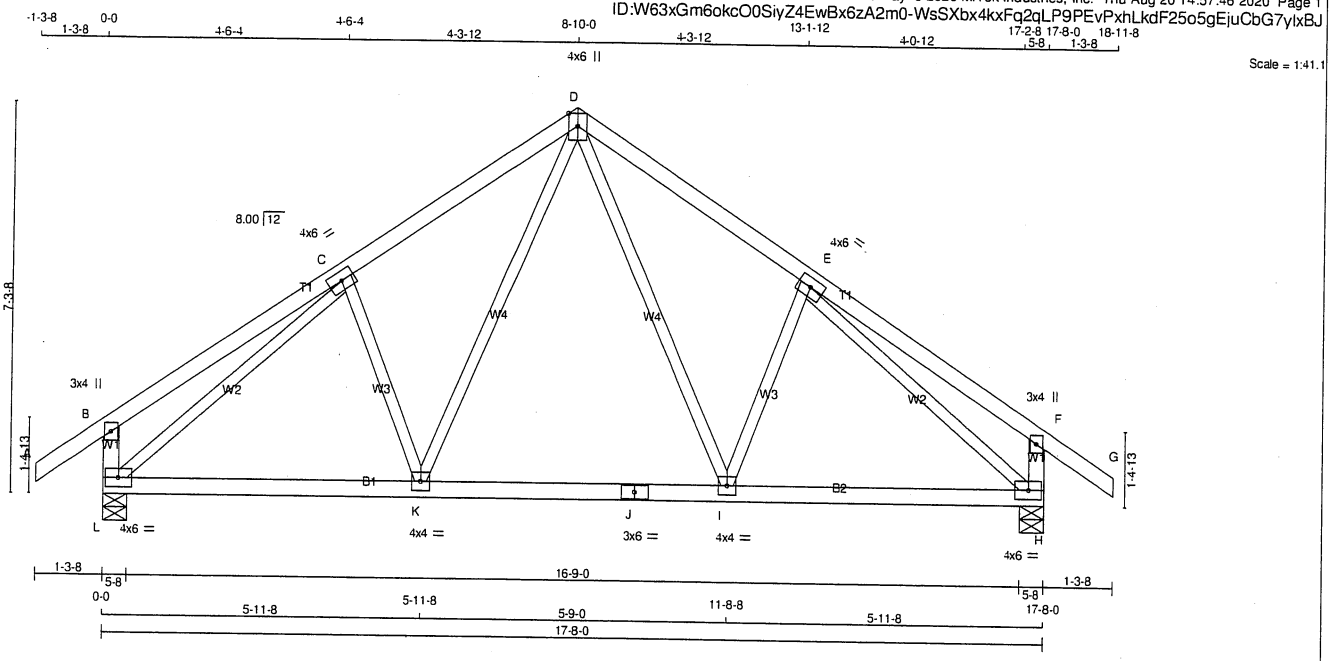

TOTAL WEIGHT = 2 X 78 = 156 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1100	0	1100	0	0	5-8	5-8
H	1100	0	1100	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	775	522 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
H	775	522 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)		10.00
B- C	0 / 26	-91.8	-91.8	0.29 (1)		10.00
C- D	-937 / 0	-91.8	-91.8	0.23 (1)		6.12
D- E	-937 / 0	-91.8	-91.8	0.23 (1)		6.12
E- F	0 / 26	-91.8	-91.8	0.29 (1)		10.00
F- G	0 / 35	-91.8	-91.8	0.12 (1)		10.00
L- B	-280 / 0	0.0	0.0	0.03 (1)		7.81
H- F	-280 / 0	0.0	0.0	0.03 (1)		7.81
L- K	0 / 848	-18.5	-18.5	0.21 (1)		10.00
K- J	0 / 614	-18.5	-18.5	0.20 (4)		10.00
J- I	0 / 614	-18.5	-18.5	0.20 (4)		10.00
I- H	0 / 848	-18.5	-18.5	0.21 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 377	0.08 (1)
I- E	-248 / 0	0.08 (1)
K- D	0 / 377	0.08 (1)
C- K	-248 / 0	0.08 (1)
L- C	-1148 / 0	0.68 (1)
E- H	-1148 / 0	0.68 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.29/1.00 (E-F:1) , BC=0.21/1.00 (H-I:1) , WB=0.68/1.00 (C-L:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (E) (INPUT = 0.90)
JSI METAL= 0.27 (C) (INPUT = 1.00)

Structural component only
DWG# T-2018814

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410069	T22A	3	1	GREEN PARK HOMES TRUSS DESC.	

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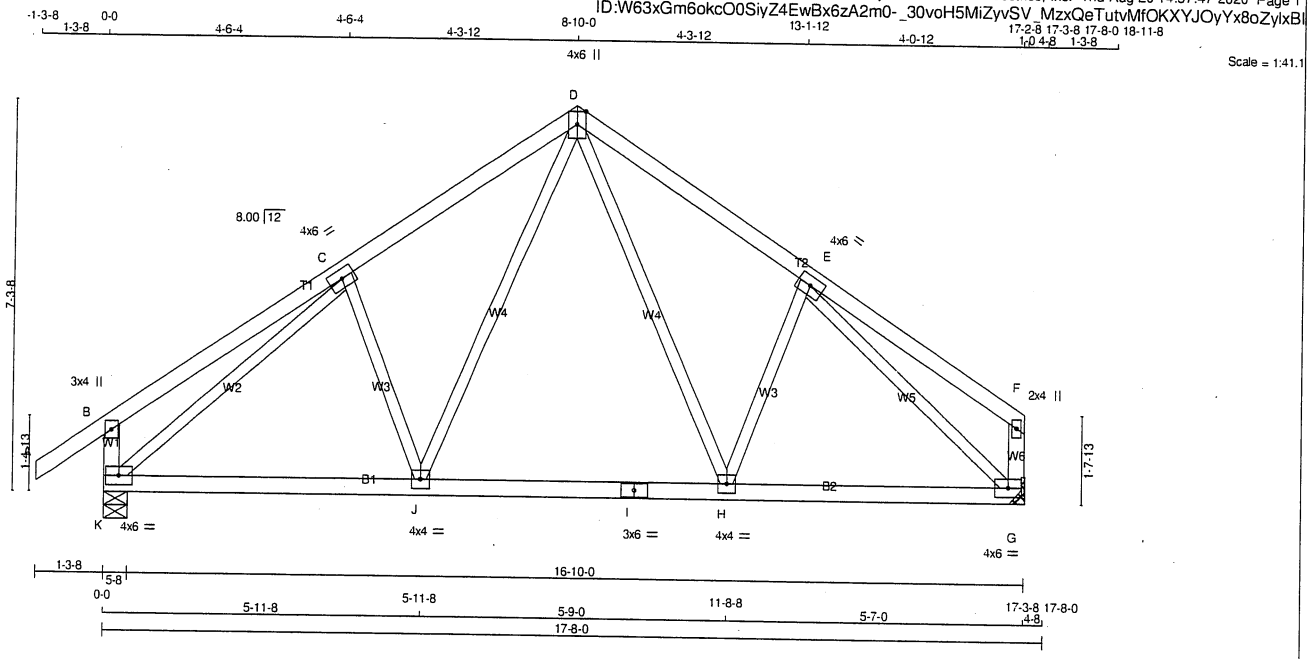

TOTAL WEIGHT = 3 X 75 = 226 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	BMWW1-t	MT20	4.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1071	0	1071	0	0	5-8	5-8
G	953	0	953	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	755	509 / 0	0 / 0	0 / 0	0 / 0	246 / 0	0 / 0
G	674	443 / 0	0 / 0	0 / 0	0 / 0	231 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 26	-91.8	-91.8	0.29 (1)	10.00
C- D	-897 / 0	-91.8	-91.8	0.23 (1)	6.22
D- E	-850 / 0	-91.8	-91.8	0.21 (1)	6.25
E- F	0 / 36	-91.8	-91.8	0.26 (1)	10.00
K- B	-280 / 0	0.0	0.0	0.03 (1)	7.81
K- J	0 / 817	-18.5	-18.5	0.21 (4)	10.00
J- I	0 / 578	-18.5	-18.5	0.20 (4)	10.00
I- H	0 / 578	-18.5	-18.5	0.20 (4)	10.00
H- G	0 / 749	-18.5	-18.5	0.19 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- H	0 / 289	0.07 (1)
H- E	-168 / 4	0.05 (1)
J- D	0 / 386	0.09 (1)
C- J	-255 / 0	0.08 (1)
K- C	-1106 / 0	0.66 (1)
E- G	-1089 / 0	0.58 (1)
G- F	-128 / 0	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.21/1.00 (J-K:4) , WB=0.66/1.00 (C-K:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (E) (INPUT = 0.90)
JSI METAL= 0.26 (C) (INPUT = 1.00)

Structural component only
DWG# T-2018815

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410069	T22S	1	1	TRUSS DESC.	

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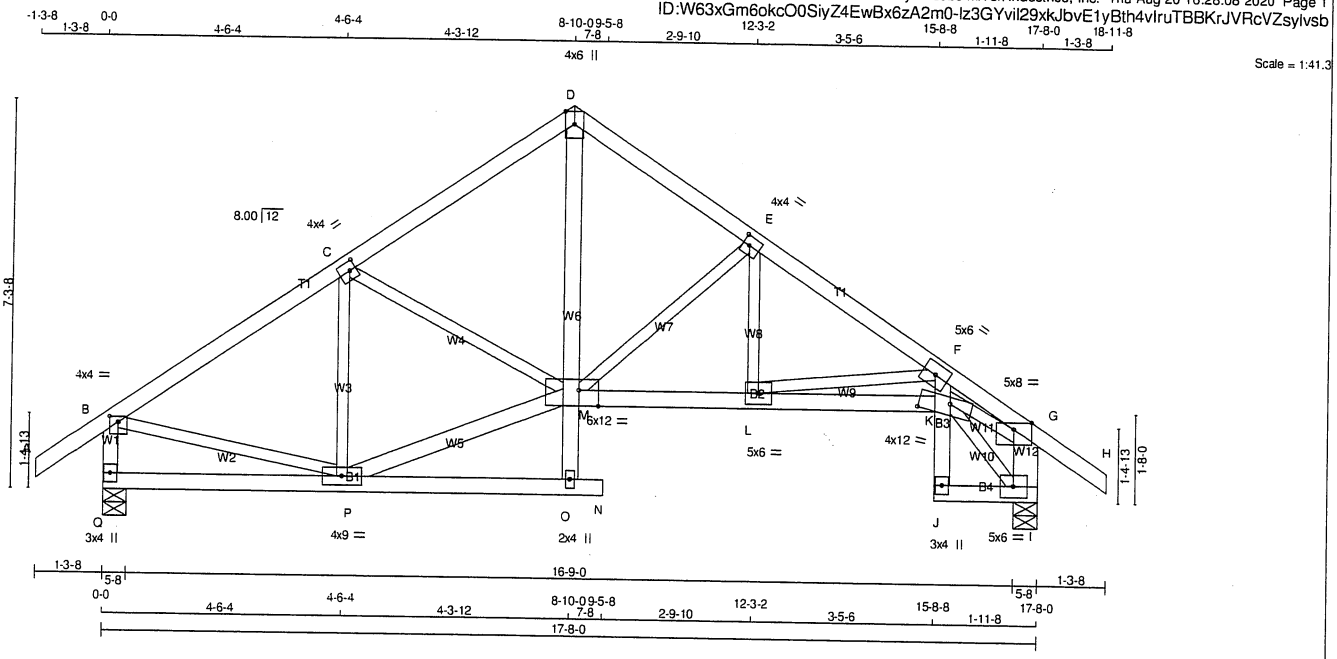

TOTAL WEIGHT = 88 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
M - K	2x4	DRY	2100F 1.8E	SPF
J - F	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - D	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.25	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMVW-t	MT20	5.0	6.0		
G	TMVW-p	MT20	5.0	8.0	Edge	
I	BMVW1-t	MT20	5.0	6.0		
J	BMV+p	MT20	3.0	4.0		
K	BVMWWm	MT20	4.0	12.0	2.50	7.00
L	BMWW-t	MT20	5.0	6.0		
M	BWMWWW*-I	MT20	6.0	12.0	Edge	4.50
O	BMW+w	MT20	2.0	4.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2018816

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	1101	0	1101	0	0	IN-SX 5-8	5-8
I	1110	0	1110	0	0	IN-SX 5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	777	519 / 0	0 / 0	0 / 0	0 / 0	257 / 0	0 / 0
I	783	525 / 0	0 / 0	0 / 0	0 / 0	258 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)
A- B	0 / 35
B- C	-1019 / 0
C- D	-999 / 0
D- E	-1009 / 0
E- F	-1463 / 0
F- G	-2846 / 0
G- H	0 / 35
Q- B	-1065 / 0
I- G	-909 / 0
Q- P	0 / 0
P- O	0 / 9
O- N	0 / 0
M- L	0 / 1271
L- K	0 / 3097
J- K	0 / 19
K- F	0 / 1035
J- I	0 / 179

FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
-91.8	-91.8	0.12 (1)	10.00
-91.8	-91.8	0.24 (1)	5.93
-91.8	-91.8	0.24 (1)	5.97
-91.8	-91.8	0.21 (1)	5.98
-91.8	-91.8	0.38 (1)	4.96
-91.8	-91.8	0.32 (1)	3.83
-91.8	-91.8	0.12 (1)	10.00
0.0	0.0	0.11 (1)	7.64
0.0	0.0	0.06 (1)	7.81
-18.5	-18.5	0.10 (4)	10.00
-18.5	-18.5	0.10 (4)	10.00
-18.5	-18.5	0.01 (4)	10.00
-18.5	-18.5	0.11 (1)	10.00
-18.5	-18.5	0.25 (1)	10.00
0.0	0.0	0.46 (1)	10.00
0.0	0.0	0.62 (1)	10.00
-18.5	-18.5	0.04 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-440 / 0	0.13 (1)
B- P	0 / 892	0.20 (1)
O- M	0 / 44	0.03 (1)
M- D	0 / 832	0.15 (1)
P- M	0 / 904	0.15 (1)
C- M	-44 / 0	0.02 (1)
L- E	0 / 215	0.05 (1)
L- F	-1848 / 0	0.44 (1)
M- E	-611 / 0	0.21 (1)
K- G	0 / 2497	0.56 (1)
K- I	-258 / 0	0.04 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.38/1.00 (E-F:1) , BC=0.62/1.00 (F-K:1) , WB=0.56/1.00 (G-K:1) , SSI=0.63/1.00 (F-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.64 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410069	T23S	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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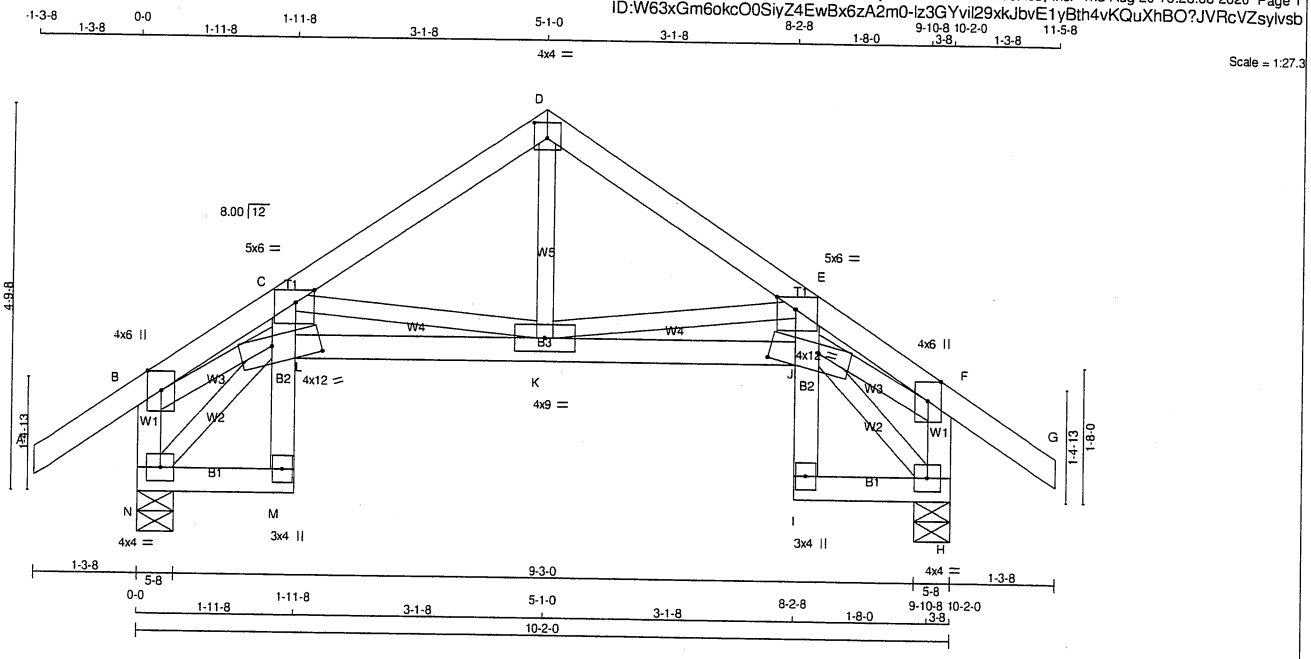

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	2100F 1.8E	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMVW-p	MT20	5.0	6.0	Edge	2.75
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-p	MT20	5.0	6.0	Edge	2.75
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWWm	MT20	4.0	12.0	2.50	7.25
K	BMWWW-t	MT20	4.0	9.0		
L	BVMWWm	MT20	4.0	12.0	2.50	7.25
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2018817

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 2 X 50 = 99 lb [M][F]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	686	0	686	0	0	5-8	5-8
H	686	0	686	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	483	330 / 0	0 / 0	0 / 0	0 / 0	153 / 0	0 / 0
H	483	330 / 0	0 / 0	0 / 0	0 / 0	153 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-1494 / 0	-91.8	-91.8	0.11 (1)	5.27
C- D	-576 / 0	-91.8	-91.8	0.22 (1)	6.25
D- E	-576 / 0	-91.8	-91.8	0.22 (1)	6.25
E- F	-1494 / 0	-91.8	-91.8	0.11 (1)	5.27
F- G	0 / 35	-91.8	-91.8	0.12 (1)	10.00
N- B	-571 / 0	0.0	0.0	0.06 (1)	7.81
H- F	-571 / 0	0.0	0.0	0.06 (1)	7.81
N- M	0 / 95	-18.5	-18.5	0.03 (4)	10.00
M- L	0 / 19	0.0	0.0	0.25 (1)	10.00
L- C	0 / 569	0.0	0.0	0.33 (1)	10.00
L- K	0 / 1637	-18.5	-18.5	0.15 (1)	10.00
K- J	0 / 1637	-18.5	-18.5	0.15 (1)	10.00
I- J	0 / 19	0.0	0.0	0.25 (1)	10.00
J- E	0 / 569	0.0	0.0	0.33 (1)	10.00
I- H	0 / 95	-18.5	-18.5	0.03 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- D	0 / 309	0.07 (1)
K- E	-1166 / 0	0.25 (1)
C- K	-1166 / 0	0.25 (1)
B- L	0 / 1313	0.30 (1)
J- F	0 / 1313	0.30 (1)
N- L	-137 / 0	0.02 (1)
J- H	-137 / 0	0.02 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.22/1.00 (C-D:1) , BC=0.33/1.00 (C-L:1) , WB=0.30/1.00 (B-L:1) , SSI=0.34/1.00 (C-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.90)
JSI METAL= 0.74 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410069	T24	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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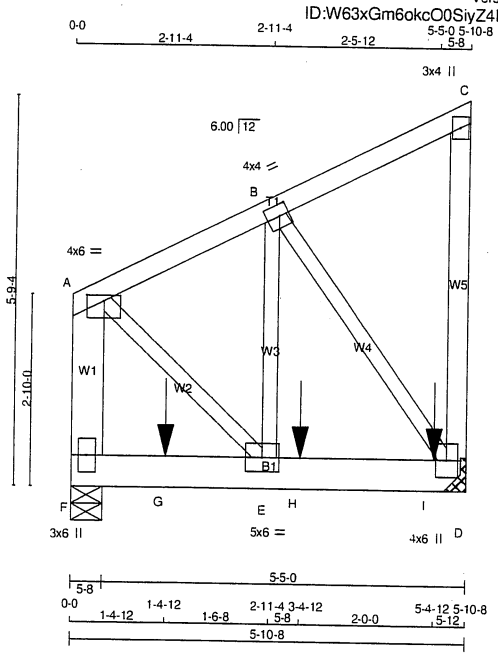

TOTAL WEIGHT = 2 X 37 = 74 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	SIDE(122.9)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(122.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2018818

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1507	0	1507	0	0	5-8	5-8
D	1948	0	1948	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1063	711 / 0	0 / 0	0 / 0	0 / 0	352 / 0	0 / 0
D	1375	918 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1128 / 0	0.0	0.0	0.06 (1)	7.81	A- E	0 / 998	0.12 (1)
A- B	-846 / 0	-91.8	-91.8	0.07 (1)	6.25	E- B	0 / 1079	0.13 (1)
B- C	-13 / 0	-91.8	-91.8	0.06 (1)	6.25	B- D	-1278 / 0	0.24 (1)
D- C	-106 / 0	0.0	0.0	0.03 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.19 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.19 (1)	10.00			
E- H	0 / 768	-18.5	-18.5	0.20 (1)	10.00			
H- I	0 / 768	-18.5	-18.5	0.20 (1)	10.00			
I- D	0 / 768	-18.5	-18.5	0.20 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-4-12	-934	-934	---	BACK	VERT	TOTAL	---	C1
H	3-4-12	-934	-934	---	BACK	VERT	TOTAL	---	C1
I	5-4-12	-939	-939	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.07/1.00 (A-B:1) , BC=0.20/1.00 (D-E:1) , WB=0.24/1.00 (B-D:1) , SSI=0.37/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (B) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410069	T25S	5	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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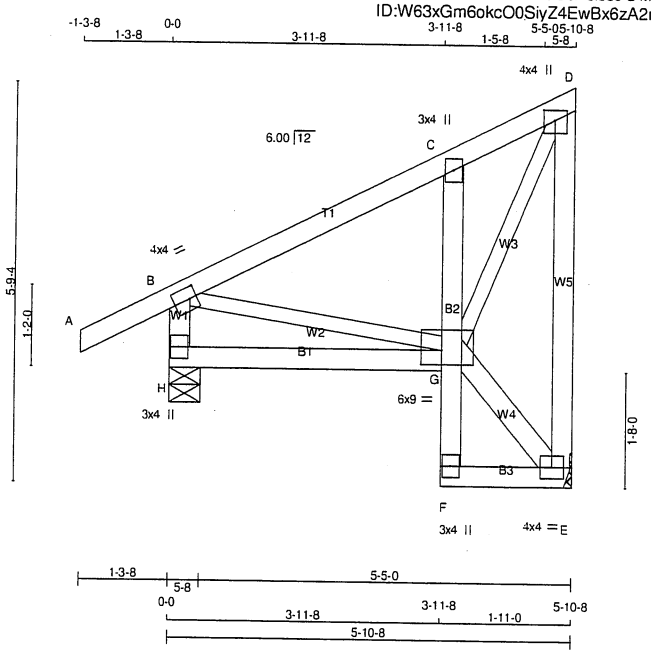

TOTAL WEIGHT = 5 X 36 = 180 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.75	2.00
E	BMVW1-t	MT20	4.0	4.0		
F	BMV+p	MT20	3.0	4.0		
G	BVMWWW-I	MT20	6.0	9.0	2.50	3.50
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	448	0	448	0	0	5-8	5-8
E	324	0	324	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	315	220 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
E	229	150 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
H- B	-410 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	-195 / 0	-91.8	-91.8	0.18 (1)	6.25
C- D	-225 / 0	-91.8	-91.8	0.18 (1)	6.25
E- D	-299 / 0	0.0	0.0	0.18 (1)	7.81
H- G	0 / 0	-18.5	-18.5	0.09 (4)	10.00
F- G	0 / 18	0.0	0.0	0.02 (1)	10.00
G- C	-365 / 0	0.0	0.0	0.03 (1)	7.81
F- E	0 / 9	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- G	0 / 191	0.04 (1)
G- E	-12 / 0	0.00 (1)
G- D	0 / 410	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.18/1.00 (D-E:1) , BC=0.09/1.00 (G-H:4) , WB=0.09/1.00 (D-G:1) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (G) (INPUT = 0.90)
JSI METAL= 0.12 (G) (INPUT = 1.00)

Structural component only
DWG# T-2018819

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410069	T26S	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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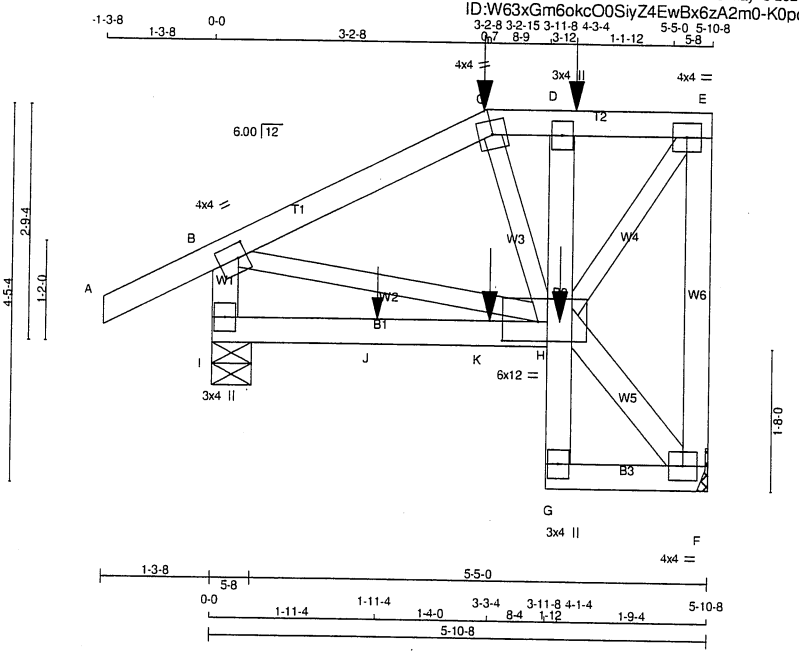

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-m	MT20	4.0	4.0		
D	TMV+p	MT20	3.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMV+p	MT20	3.0	4.0		
H	BVMWWWW*-t	MT20	6.0	12.0	2.75	5.00
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	453	0	453	0	0	MECHANICAL	
I	526	0	526	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	321	208 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0
I	371	251 / 0	0 / 0	0 / 0	0 / 0	120 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.13 (1)	10.00
B- C	-321 / 0	-91.8	-91.8	0.18 (1)	6.25
C- D	-243 / 0	-117.4	-117.4	0.03 (1)	6.25
D- E	-238 / 0	-117.4	-117.4	0.05 (1)	6.25
F- E	-425 / 0	0.0	0.0	0.14 (1)	7.81
I- B	-477 / 0	0.0	0.0	0.05 (1)	7.81
I- J	0 / 0	-23.7	-23.7	0.13 (4)	10.00
J- K	0 / 0	-23.7	-23.7	0.13 (4)	10.00
K- H	0 / 0	-23.7	-23.7	0.13 (4)	10.00
G- H	0 / 23	0.0	0.0	0.02 (1)	10.00
H- D	-193 / 0	0.0	0.0	0.02 (1)	7.81
G- F	0 / 7	-23.7	-23.7	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- H	-135 / 0	0.03 (1)
H- F	-10 / 0	0.00 (1)
H- E	0 / 409	0.10 (1)
B- H	0 / 293	0.07 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-2-15	-96	-96	---	FRONT	VERT	TOTAL	---	C1
D	4-3-4	-3	-3	---	FRONT	VERT	TOTAL	---	C1
H	4-1-4	-8	-8	---	FRONT	VERT	TOTAL	---	C1
J	1-11-4	-1	-1	---	FRONT	VERT	TOTAL	---	C1
K	3-3-4	-2	-2	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 35 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 3-2-15
RIGHT SETBACK = 0-0
END SETBACK = 3-2-15
END WALL WIDTH = 1-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.13/1.00 (H-I:4) , WB=0.10/1.00 (E-H:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (H) (INPUT = 0.90)
JSI METAL= 0.15 (B) (INPUT = 1.00)

Structural component only
DWG# T-2018820

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410070	T30	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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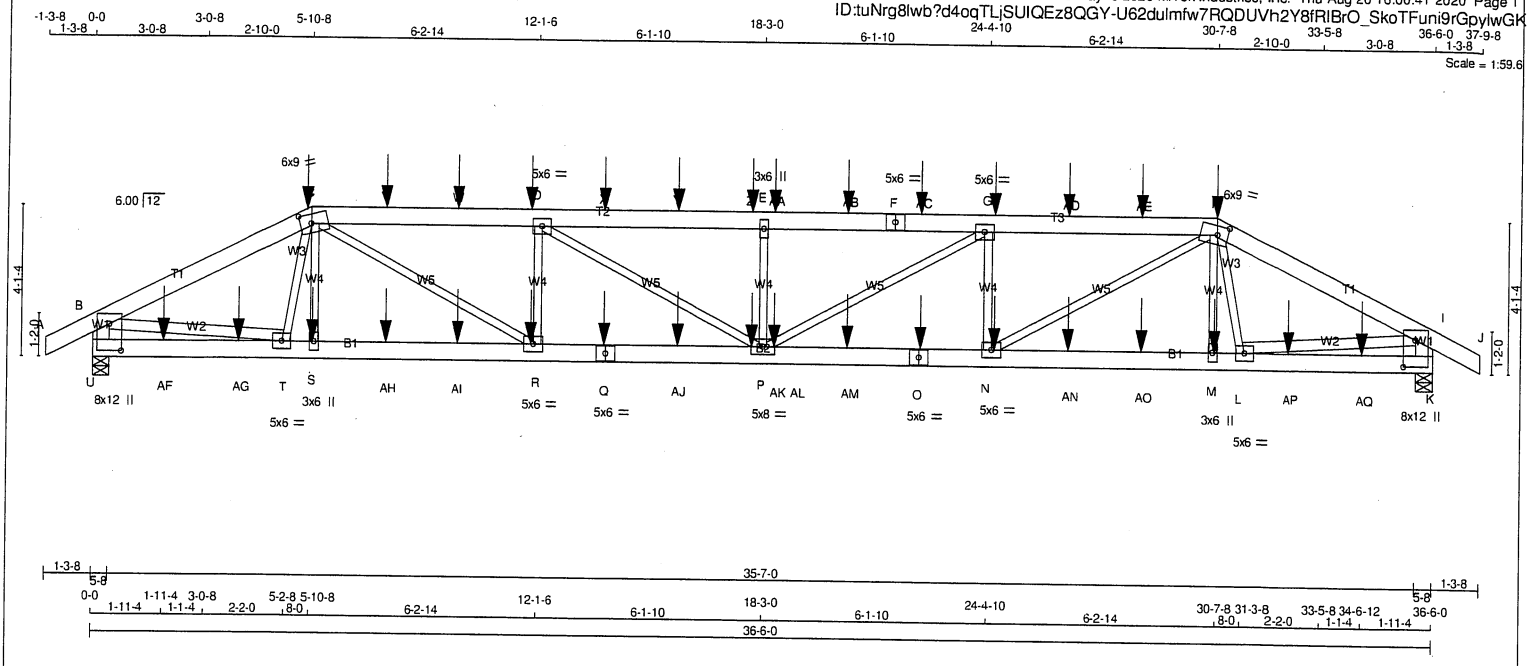

TOTAL WEIGHT = 2 X 192 = 383 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	2100F 1.8E	SPF
Q - O	2x6	DRY	2100F 1.8E	SPF
O - K	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
B - T	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- F	2	12	SIDE(183.1)
F- H	2	12	SIDE(183.1)
H- J	2	12	SIDE(122.0)
U- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- Q	2	12	SIDE(197.8)
Q- O	2	12	SIDE(197.8)
O- K	2	12	SIDE(197.8)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2018823

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	3447	0	3447	0	0	5-8	5-8
K	3447	0	3447	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2439	1594 / 0	0 / 0	0 / 0	0 / 0	844 0	0 / 0
K	2439	1594 / 0	0 / 0	0 / 0	0 / 0	844 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 29	-91.8	-91.8	0.03 (1)	10.00
B- C	-5251 / 0	-91.8	-91.8	0.20 (1)	4.93
C- V	-7809 / 0	-91.8	-91.8	0.30 (1)	4.10
V- W	-7809 / 0	-91.8	-91.8	0.30 (1)	4.10
W- D	-7809 / 0	-91.8	-91.8	0.30 (1)	4.10
D- X	-8753 / 0	-91.8	-91.8	0.32 (1)	3.89
X- Y	-8753 / 0	-91.8	-91.8	0.32 (1)	3.89
Y- Z	-8753 / 0	-91.8	-91.8	0.32 (1)	3.89
Z- E	-8753 / 0	-91.8	-91.8	0.32 (1)	3.89
E-AA	-8753 / 0	-91.8	-91.8	0.32 (1)	3.89
AA-AB	-8753 / 0	-91.8	-91.8	0.32 (1)	3.89
AB- F	-8753 / 0	-91.8	-91.8	0.32 (1)	3.89
F-AC	-8753 / 0	-91.8	-91.8	0.32 (1)	3.89
AC- G	-8753 / 0	-91.8	-91.8	0.32 (1)	3.89
G-AD	-7809 / 0	-91.8	-91.8	0.30 (1)	4.10
AD-AE	-7809 / 0	-91.8	-91.8	0.30 (1)	4.10
AE- H	-7809 / 0	-91.8	-91.8	0.30 (1)	4.10
H- I	-5251 / 0	-91.8	-91.8	0.20 (1)	4.93
I- J	0 / 29	-91.8	-91.8	0.03 (1)	10.00
U- B	-3368 / 0	0.0	0.0	0.12 (1)	7.58
K- I	-3368 / 0	0.0	0.0	0.12 (1)	7.58

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	0 / 375	0.05 (1)
M- H	0 / 375	0.05 (1)
T- C	-777 / 0	0.09 (1)
H- L	-777 / 0	0.09 (1)
N- H	0 / 3432	0.42 (1)
C- R	0 / 3432	0.42 (1)
N- G	-1559 / 0	0.19 (1)
R- D	-1559 / 0	0.19 (1)
P- G	0 / 1098	0.14 (1)
D- P	0 / 1098	0.14 (1)
P- E	-915 / 0	0.11 (1)
L- I	0 / 4732	0.42 (1)
B- T	0 / 4732	0.42 (1)

U-AF	0 / 0	-18.5	-18.5	0.03 (4)	10.00
AF-AG	0 / 0	-18.5	-18.5	0.03 (4)	10.00
AG- T	0 / 0	-18.5	-18.5	0.03 (4)	10.00
T- S	0 / 4833	-18.5	-18.5	0.15 (1)	10.00
S-AH	0 / 4843	-18.5	-18.5	0.15 (1)	10.00
AH-AI	0 / 4843	-18.5	-18.5	0.15 (1)	10.00
AI- R	0 / 4843	-18.5	-18.5	0.15 (1)	10.00
R- Q	0 / 7809	-18.5	-18.5	0.25 (1)	10.00
Q-AJ	0 / 7809	-18.5	-18.5	0.25 (1)	10.00
AJ-AK	0 / 7809	-18.5	-18.5	0.25 (1)	10.00
AK- P	0 / 7809	-18.5	-18.5	0.25 (1)	10.00
P-AL	0 / 7809	-18.5	-18.5	0.25 (1)	10.00
AL-AM	0 / 7809	-18.5	-18.5	0.25 (1)	10.00
AM- O	0 / 7809	-18.5	-18.5	0.25 (1)	10.00
O- N	0 / 7809	-18.5	-18.5	0.25 (1)	10.00
N-AN	0 / 4843	-18.5	-18.5	0.15 (1)	10.00
AN-AO	0 / 4843	-18.5	-18.5	0.15 (1)	10.00
AO- M	0 / 4843	-18.5	-18.5	0.15 (1)	10.00
M- L	0 / 4833	-18.5	-18.5	0.15 (1)	10.00
L-AP	0 / 0	-18.5	-18.5	0.03 (4)	10.00
AP-AQ	0 / 0	-18.5	-18.5	0.03 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.40")

CSI: TC=0.32/1.00 (D-E:1) , BC=0.25/1.00 (P-R:1) , WB=0.42/1.00 (C-R:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (T) (INPUT = 0.90)
JSI METAL= 0.75 (Q) (INPUT = 1.00)

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, I, K, U						
B						
C	TTWWW-m	MT20	6.0	9.0	3.00	3.50
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWWW-m	MT20	6.0	9.0	3.00	3.50
K	TMBMVW1"+p	MT20	8.0	12.0	8.75	4.00
L, N, R, T						
L	BMWW-t	MT20	5.0	6.0		
M	BMW+w	MT20	3.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
S	BMW+w	MT20	3.0	6.0		
U	TMBMVW1"+p	MT20	8.0	12.0	8.75	4.00

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI	(LC)	WEBS MAX. MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
FR-TO		FROM	TO			UNBRAC LENGTH	FR-TO		
AQ- K	0 / 0	-18.5	-18.5	0.03	(4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-408	-408	---	FRONT	VERT	TOTAL	---	C1
D	11-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
G	24-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
H	30-7-8	-408	-408	---	FRONT	VERT	TOTAL	---	C1
M	30-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
N	24-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	22-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Q	13-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
R	11-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
S	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
V	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	13-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Y	15-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	17-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AA	18-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AB	20-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AC	22-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AD	26-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AE	28-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
AF	1-11-4	-25	-26	---	FRONT	VERT	TOTAL	---	C1
AG	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AH	7-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AI	9-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AJ	15-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AK	17-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AL	18-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AM	20-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AN	26-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AO	28-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AP	32-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AQ	34-6-12	-25	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2018823 JJ

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410070	T30Z	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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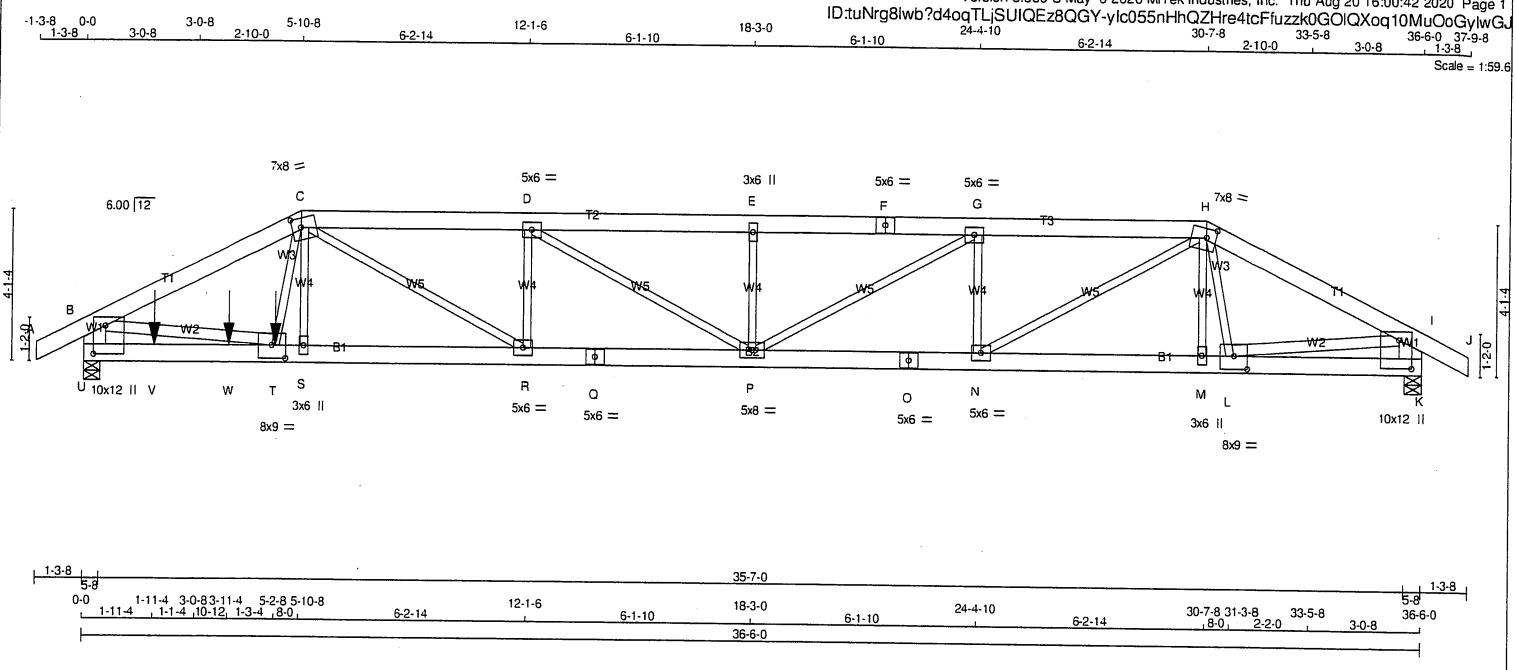

TOTAL WEIGHT = 2 X 192 = 384 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
U - B	2x8	DRY	No.2	SPF
K - I	2x8	DRY	No.2	SPF
U - Q	2x6	DRY	2100F 1.8E	SPF
Q - O	2x6	DRY	2100F 1.8E	SPF
O - K	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
B - T	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- F	2	12	TOP
F- H	2	12	TOP
H- J	2	12	TOP
U- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- Q	2	12	SIDE(0.0)
Q- O	2	12	TOP
O- K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2018824 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
U	7116	0	7116	0	0	5-8	5-8
K	2965	0	2965	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	5048	3223 / 0	0 / 0	0 / 0	0 / 0	1826 / 0	0 / 0
K	2097	1374 / 0	0 / 0	0 / 0	0 / 0	722 / 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, K
BEARING SIZE FACTOR = 1.15 AT JNT(S) U (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 29	-91.8	-91.8	0.03 (1)	10.00
B- C	-11487 / 0	-91.8	-91.8	0.38 (1)	3.38
C- D	-10013 / 0	-91.8	-91.8	0.32 (1)	3.68
D- E	-9178 / 0	-91.8	-91.8	0.26 (1)	3.88
E- F	-9178 / 0	-91.8	-91.8	0.26 (1)	3.88
F- G	-9178 / 0	-91.8	-91.8	0.26 (1)	3.88
G- H	-7263 / 0	-91.8	-91.8	0.22 (1)	4.32
H- I	-4435 / 0	-91.8	-91.8	0.18 (1)	5.29
I- J	0 / 29	-91.8	-91.8	0.03 (1)	10.00
U- B	-6896 / 0	0.0	0.0	0.18 (1)	6.39
K- I	-2907 / 0	0.0	0.0	0.08 (1)	7.81
U- V	0 / 0	-18.5	-18.5	0.16 (1)	10.00
V- W	0 / 0	-18.5	-18.5	0.16 (1)	10.00
W- T	0 / 0	-18.5	-18.5	0.16 (1)	10.00
T- S	0 / 9706	-18.5	-18.5	0.41 (1)	10.00
S- R	0 / 9747	-18.5	-18.5	0.30 (1)	10.00
R- Q	0 / 10013	-18.5	-18.5	0.30 (1)	10.00
Q- P	0 / 10013	-18.5	-18.5	0.30 (1)	10.00
P- O	0 / 7263	-18.5	-18.5	0.22 (1)	10.00
O- N	0 / 7263	-18.5	-18.5	0.22 (1)	10.00
N- M	0 / 4102	-18.5	-18.5	0.13 (1)	10.00
M- L	0 / 4092	-18.5	-18.5	0.13 (1)	10.00
L- K	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
S- C	0 / 1424	0.18 (1)
M- H	0 / 326	0.04 (1)
T- C	0 / 3092	0.38 (1)
H- L	-714 / 0	0.09 (1)
N- H	0 / 3657	0.45 (1)
C- R	0 / 308	0.04 (1)
N- G	-1758 / 0	0.21 (1)
R- D	-110 / 84	0.02 (4)
P- G	0 / 2228	0.28 (1)
D- P	-971 / 0	0.47 (1)
P- E	-520 / 0	0.06 (1)
L- I	0 / 3997	0.35 (1)
B- T	0 / 10352	0.92 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	5-2-8	-5749	-5749	---	BACK	VERT	TOTAL	---	C1
V	1-11-4	-25	-26	---	BACK	VERT	TOTAL	---	C1
W	3-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.43")

CSI: TC=0.38/1.00 (B-C:1) , BC=0.41/1.00 (S-T:1) , WB=0.92/1.00 (B-T:1) , SSI=0.43/1.00 (S-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.97 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410070	T30Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, I, K, U						
B						
C	TTWWW-m	MT20	7.0	8.0	3.00	3.00
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWWW-m	MT20	7.0	8.0	3.00	3.00
K	TMBMVW1"+p	MT20	10.0	12.0	9.25	4.00
L	BMWW-t	MT20	8.0	9.0	4.25	4.50
M	BMW+w	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0		
S	BMW+w	MT20	3.0	6.0		
T	BMWW-t	MT20	8.0	9.0	4.25	4.50
U	TMBMVW1"+p	MT20	10.0	12.0	9.25	4.00

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018824

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T31	2	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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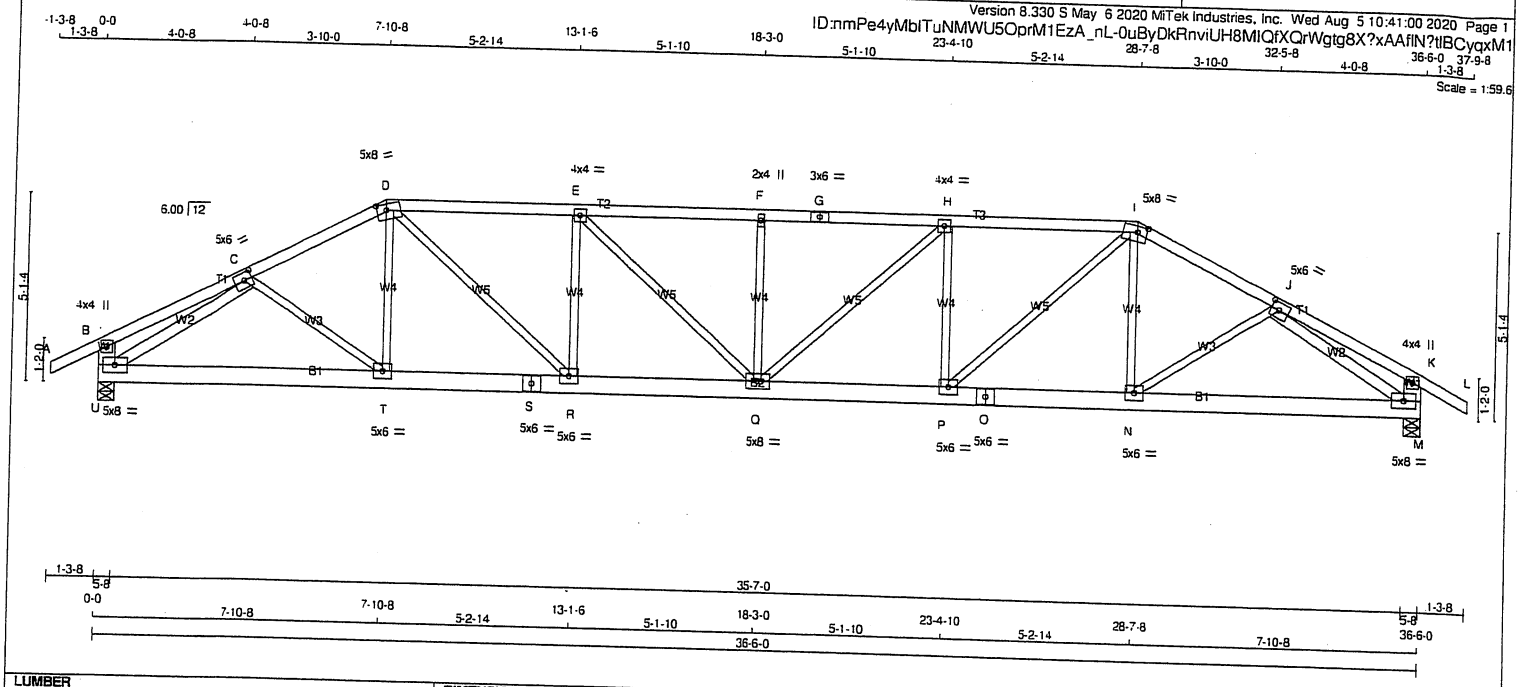

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	8.0	2.00	3.25
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	8.0	2.00	3.25
J	TMWW-t	MT20	5.0	6.0	2.50	2.75
K	TMV+p	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	8.0		
N, P, R, T						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
U	BMVW1-t	MT20	5.0	8.0		

Structural component only
DWG# T-2017369

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 2 X 171 = 343 lb (M)

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
U	2137	0	2137	0	0	IN-SX	IN-SX
M	2137	0	2137	0	0	5-8	5-8
						5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 0
M	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12	(1)		10.00
B- C	0 / 17	-91.8	-91.8	0.20	(1)		10.00
C- D	-2978 / 0	-91.8	-91.8	0.34	(1)		3.76
D- E	-3625 / 0	-91.8	-91.8	0.63	(1)		3.17
E- F	-3907 / 0	-91.8	-91.8	0.66	(1)		3.03
F- G	-3907 0	-91.8	-91.8	0.66	(1)		3.03
G- H	-3907 0	-91.8	-91.8	0.66	(1)		3.03
H- I	-3625 / 0	-91.8	-91.8	0.63	(1)		3.17
I- J	-2978 0	-91.8	-91.8	0.34	(1)		3.76
J- K	0 / 17	-91.8	-91.8	0.20	(1)		10.00
K- L	0 / 28	-91.8	-91.8	0.12	(1)		10.00
U- B	-269 0	0.0	0.0	0.02	(1)		7.81
M- K	-269 0	0.0	0.0	0.02	(1)		7.81
U- T	0 2592	-18.5	-18.5	0.38	(1)		10.00
T- S	0 2649	-18.5	-18.5	0.36	(1)		10.00
S- R	0 / 2649	-18.5	-18.5	0.36	(1)		10.00
R- Q	0 / 3626	-18.5	-18.5	0.48	(1)		10.00
Q- P	0 3626	-18.5	-18.5	0.48	(1)		10.00
P- O	0 2649	-18.5	-18.5	0.36	(1)		10.00
O- N	0 / 2649	-18.5	-18.5	0.36	(1)		10.00
N- M	0 2592	-18.5	-18.5	0.38	(1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
C- T	0 / 87	0.03	(4)
T- D	0 / 118	0.04	(4)
D- R	0 1314	0.30	(1)
R- E	-797 0	0.29	(1)
E- Q	0 / 383	0.09	(1)
Q- F	-433 0	0.16	(1)
Q- H	0 / 383	0.09	(1)
P- H	-797 0	0.29	(1)
P- I	0 1314	0.30	(1)
N- I	0 / 118	0.04	(4)
N- J	0 87	0.03	(4)
U- C	-3157 0	0.80	(1)
J- M	-3157 0	0.80	(1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.40")

CSI: TC=0.66/1.00 (E-F:1) , BC=0.48/1.00 (Q-R:1) , WB=0.80/1.00 (J-M:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (D) (INPUT = 0.90)
JSI METAL= 0.72 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T32	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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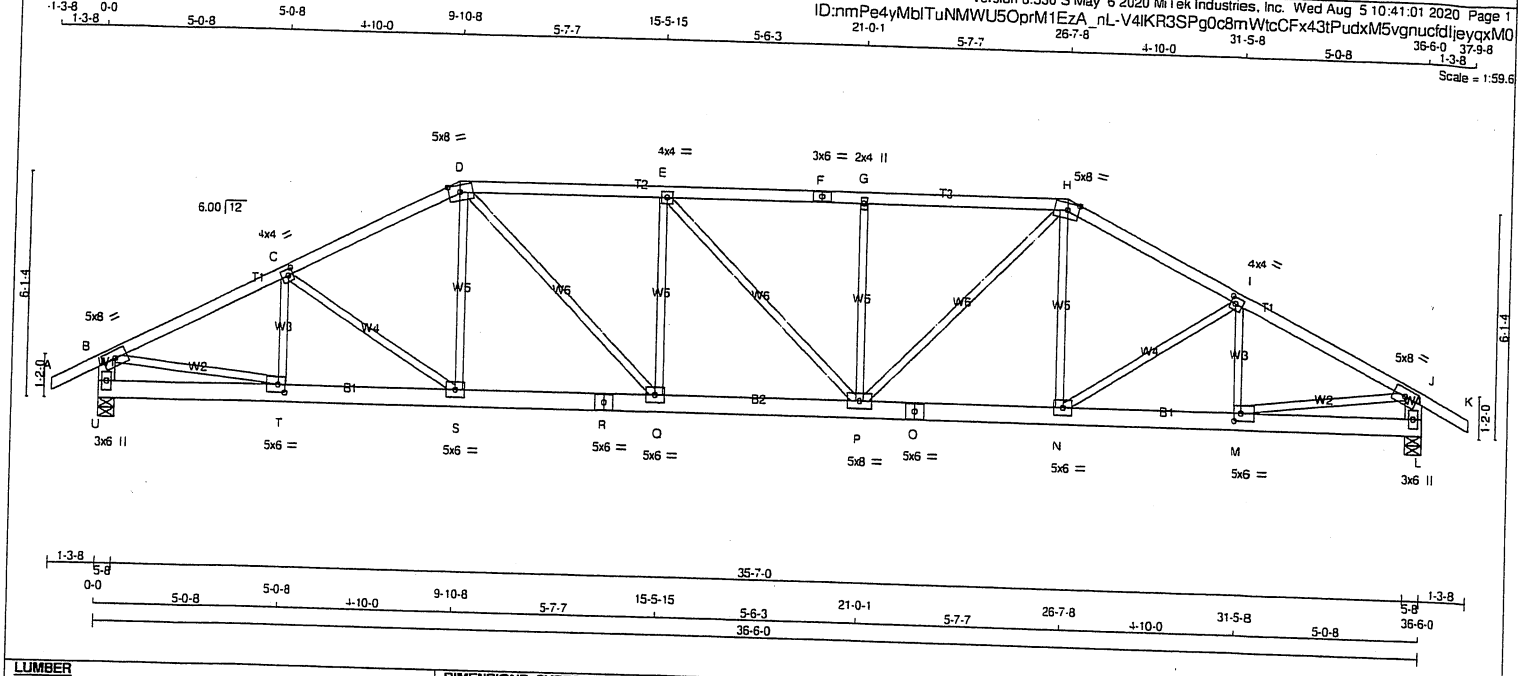

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	2.25	3.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.25	3.75
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-t	MT20	5.0	8.0		
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.25
N, Q, S						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.25
U	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2017370

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 2 X 173 = 345 lb [M][F]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
U	2137	0	2137	0	0	IN-SX	IN-SX
L	2137	0	2137	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 0	0 0
L	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 28	-91.8	-91.8	0.12 (1)	10.00
B- C	-3003 / 0	-91.8	-91.8	0.41 (1)	3.70
C- D	-2866 / 0	-91.8	-91.8	0.39 (1)	3.79
D- E	-3181 / 0	-91.8	-91.8	0.49 (1)	3.52
E- F	-3175 / 0	-91.8	-91.8	0.45 (1)	3.56
F- G	-3175 / 0	-91.8	-91.8	0.45 (1)	3.56
G- H	-3175 / 0	-91.8	-91.8	0.49 (1)	3.52
H- I	-2867 / 0	-91.8	-91.8	0.39 (1)	3.79
I- J	-3002 / 0	-91.8	-91.8	0.41 (1)	3.70
J- K	0 28	-91.8	-91.8	0.12 (1)	10.00
U- B	-2075 / 0	0.0	0.0	0.13 (1)	7.03
L- J	-2074 / 0	0.0	0.0	0.13 (1)	7.04
U- T	0 0	-18.5	-18.5	0.07 (1)	10.00
T- S	0 / 2706	-18.5	-18.5	0.38 (1)	10.00
S- R	0 / 2545	-18.5	-18.5	0.34 (1)	10.00
R- Q	0 / 2545	-18.5	-18.5	0.34 (1)	10.00
Q- P	0 / 3182	-18.5	-18.5	0.43 (1)	10.00
P- O	0 / 2547	-18.5	-18.5	0.35 (1)	10.00
O- N	0 / 2547	-18.5	-18.5	0.35 (1)	10.00
N- M	0 / 2705	-18.5	-18.5	0.38 (1)	10.00
M- L	0 0	-18.5	-18.5	0.07 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-366 0	0.08 (1)
C- S	-199 0	0.12 (1)
S- D	0 243	0.05 (1)
D- Q	0 / 908	0.20 (1)
Q- E	-550 0	0.30 (1)
E- P	-9 / 0	0.01 (1)
P- G	-552 0	0.30 (1)
P- H	0 897	0.20 (1)
N- H	0 250	0.06 (1)
N- I	-196 / 0	0.12 (1)
M- I	-368 0	0.08 (1)
B- T	0 / 2738	0.62 (1)
M- J	0 2737	0.62 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.49/1.00 (D-E:1) , BC=0.43/1.00 (P-Q:1) , WB=0.62/1.00 (B-T:1) . SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.62 (T) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T33	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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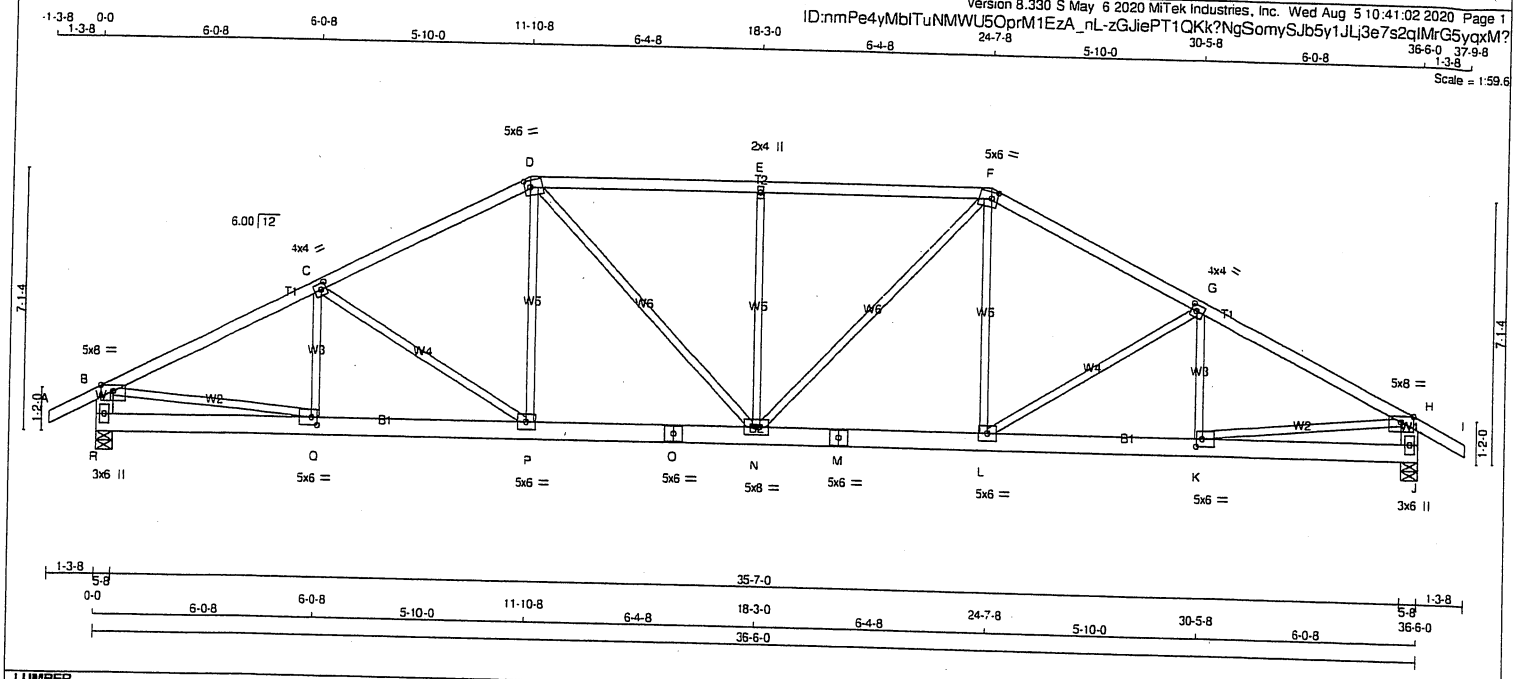

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.00
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BS-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.00
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2017371

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2137	0	2137	0	0	5-8	5-8
J	2137	0	2137	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 0	0 0
J	1509	1003 / 0	0 / 0	0 / 0	0 0	505 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 28	-91.8	-91.8	0.12 (1)	10.00
B- C	-3058 0	-91.8	-91.8	0.60 (1)	3.47
C- D	-2720 0	-91.8	-91.8	0.54 (1)	3.70
D- E	-2796 0	-91.8	-91.8	0.62 (1)	3.53
E- F	-2796 0	-91.8	-91.8	0.62 (1)	3.53
F- G	-2720 0	-91.8	-91.8	0.54 (1)	3.70
G- H	-3058 0	-91.8	-91.8	0.60 (1)	3.47
H- I	0 28	-91.8	-91.8	0.12 (1)	10.00
R- B	-2073 0	0.0	0.0	0.13 (1)	7.04
J- H	-2073 0	0.0	0.0	0.13 (1)	7.04
R- Q	0 0	-18.5	-18.5	0.08 (4)	10.00
Q- P	0 2760	-18.5	-18.5	0.38 (1)	10.00
P- O	0 2411	-18.5	-18.5	0.34 (1)	10.00
O- N	0 2411	-18.5	-18.5	0.34 (1)	10.00
N- M	0 2411	-18.5	-18.5	0.34 (1)	10.00
M- L	0 2411	-18.5	-18.5	0.34 (1)	10.00
L- K	0 2760	-18.5	-18.5	0.38 (1)	10.00
K- J	0 0	-18.5	-18.5	0.08 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-271 6	0.07 (1)
C- P	-421 0	0.39 (1)
P- D	0 372	0.08 (1)
D- N	0 559	0.13 (1)
N- E	-718 0	0.59 (1)
N- F	0 559	0.13 (1)
L- F	0 372	0.08 (1)
L- G	-421 0	0.39 (1)
K- G	-271 6	0.07 (1)
B- Q	0 2783	0.63 (1)
K- H	0 2783	0.63 (1)

TOTAL WEIGHT = 2 X 171 = 343 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.62/1.00 (D-E:1) , BC=0.38/1.00 (P-Q:1) , WB=0.63/1.00 (B-Q:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.63 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T34	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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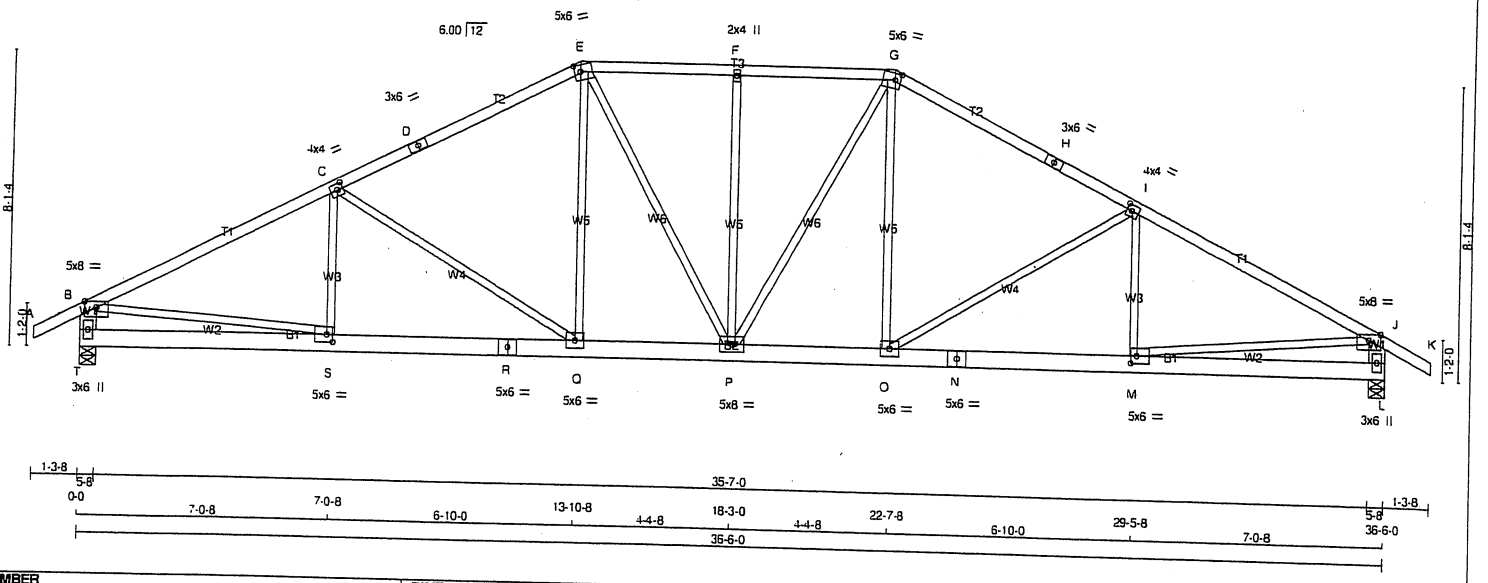

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.00
N	BS-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.00
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2017372

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 2 X 178 = 356 lb [M][F]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	2137	0	2137	0	0	5-8	5-8
L	2137	0	2137	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 0	0 0
L	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	-3079 / 0	-91.8	-91.8	0.86 (1)	3.14
C- D	-2545 / 0	-91.8	-91.8	0.74 (1)	3.53
D- E	-2545 0	-91.8	-91.8	0.74 (1)	3.53
E- F	-2400 / 0	-91.8	-91.8	0.28 (1)	4.19
F- G	-2400 / 0	-91.8	-91.8	0.28 (1)	4.19
G- H	-2545 0	-91.8	-91.8	0.74 (1)	3.53
H- I	-2545 / 0	-91.8	-91.8	0.74 (1)	3.53
I- J	-3079 / 0	-91.8	-91.8	0.86 (1)	3.14
J- K	0 28	-91.8	-91.8	0.12 (1)	10.00
T- B	-2071 0	0.0	0.0	0.13 (1)	7.05
L- J	-2071 0	0.0	0.0	0.13 (1)	7.05
T- S	0 0	-18.5	-18.5	0.10 (4)	10.00
S- R	0 2784	-18.5	-18.5	0.39 (1)	10.00
R- Q	0 2784	-18.5	-18.5	0.39 (1)	10.00
Q- P	0 2251	-18.5	-18.5	0.31 (1)	10.00
P- O	0 2251	-18.5	-18.5	0.31 (1)	10.00
O- N	0 2784	-18.5	-18.5	0.39 (1)	10.00
N- M	0 2784	-18.5	-18.5	0.39 (1)	10.00
M- L	0 0	-18.5	-18.5	0.10 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-188 54	0.06 (1)
C- Q	-635 0	0.89 (1)
Q- E	0 467	0.11 (1)
E- P	0 302	0.07 (1)
P- F	-484 0	0.58 (1)
P- G	0 302	0.07 (1)
O- G	0 467	0.11 (1)
O- I	-635 0	0.89 (1)
M- I	-188 54	0.06 (1)
B- S	0 2801	0.63 (1)
M- J	0 2801	0.63 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.86/1.00 (I-J:1) , BC=0.39/1.00 (M-O:1) , WB=0.89/1.00 (C-Q:1) , SSI=0.28/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.63 (S) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	T35	2	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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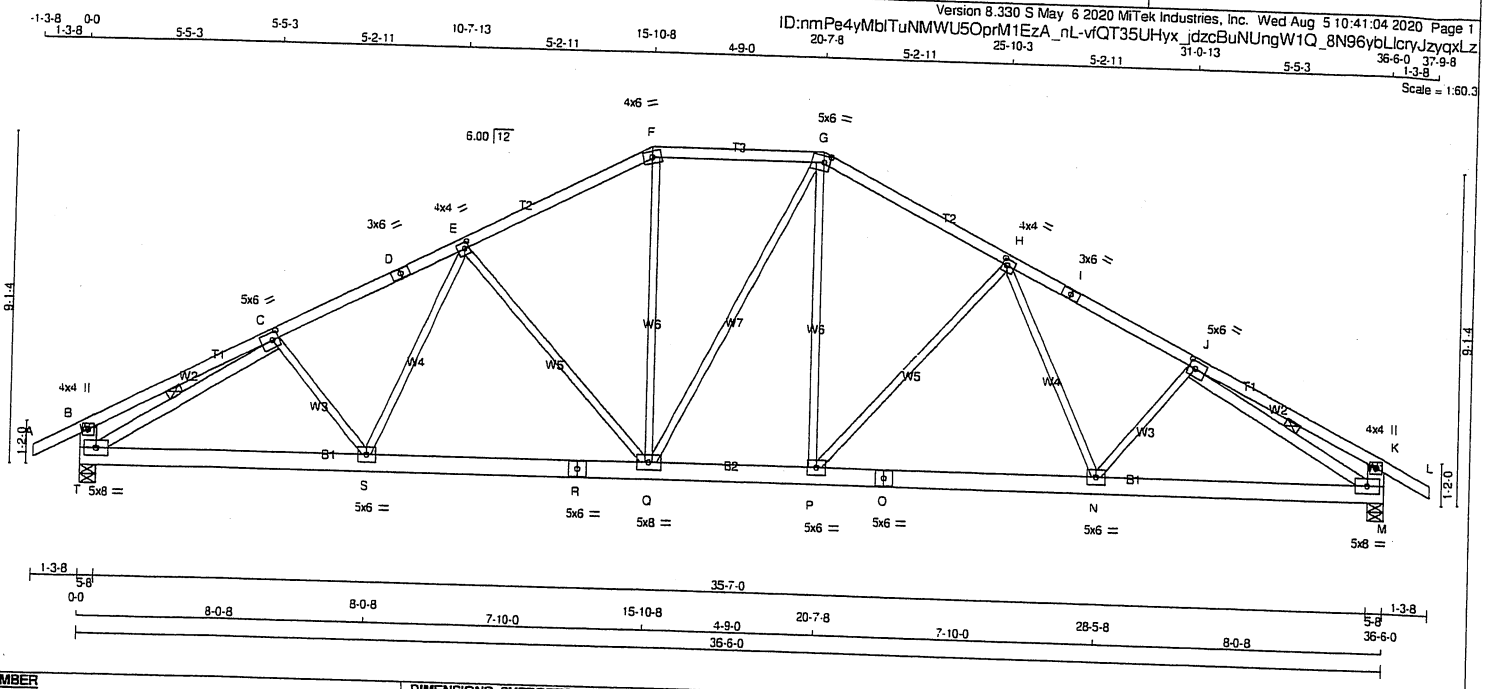


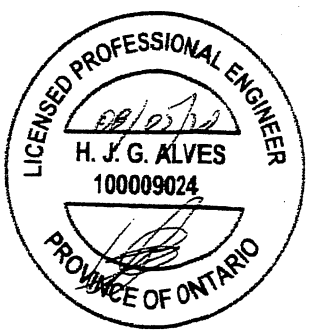
LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TTW-m	MT20	4.0	6.0		
G	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TS-t	MT20	3.0	6.0		
J	TMWW-t	MT20	5.0	6.0	2.50	2.25
K	TMV+p	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	8.0		
N, P, S						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
T	BMVW1-t	MT20	5.0	8.0		



Structural component only
DWG# T-2017373

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	2137	0	2137	0	0	5-8	5-8
M	2137	0	2137	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 0	0 0
M	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-T, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	
A- B	0 28
B- C	0 20
C- D	-2933 0
D- E	-2933 0
E- F	-2339 0
F- G	-2079 0
G- H	-2335 0
H- I	-2935 0
I- J	-2935 0
J- K	0 20
K- L	0 28
T- B	-324 0
M- K	-324 0
T- S	0 2702
S- R	0 2517
R- Q	0 2517
Q- P	0 2076
P- O	0 2517
O- N	0 2517
N- M	0 2704

FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
-91.8	-91.8	0.12 (1)	10.00
-91.8	-91.8	0.32 (1)	10.00
-91.8	-91.8	0.42 (1)	3.75
-91.8	-91.8	0.42 (1)	3.75
-91.8	-91.8	0.38 (1)	4.16
-91.8	-91.8	0.32 (1)	4.40
-91.8	-91.8	0.38 (1)	4.16
-91.8	-91.8	0.42 (1)	3.75
-91.8	-91.8	0.42 (1)	3.75
-91.8	-91.8	0.32 (1)	10.00
-91.8	-91.8	0.12 (1)	10.00
0.0	0.0	0.02 (1)	7.81
0.0	0.0	0.02 (1)	7.81
-18.5	-18.5	0.40 (1)	10.00
-18.5	-18.5	0.35 (1)	10.00
-18.5	-18.5	0.35 (1)	10.00
-18.5	-18.5	0.30 (1)	10.00
-18.5	-18.5	0.36 (1)	10.00
-18.5	-18.5	0.36 (1)	10.00
-18.5	-18.5	0.40 (1)	10.00

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
C- S	-126 32	0.04 (1)
S- E	0 272	0.06 (1)
E- Q	-678 / 0	0.93 (1)
Q- F	0 629	0.14 (1)
Q- G	0 6	0.00 (1)
P- G	0 622	0.14 (1)
P- H	-681 0	0.93 (1)
H- N	0 277	0.06 (1)
N- J	-125 32	0.04 (1)
T- C	-3217 0	0.61 (1)
J- M	-3218 0	0.61 (1)

TOTAL WEIGHT = 2 X 181 = 362 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.42/1.00 (H-J:1) , BC=0.40/1.00 (S-T:1) , WB=0.93/1.00 (H-P:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.72 (J) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410047	T36	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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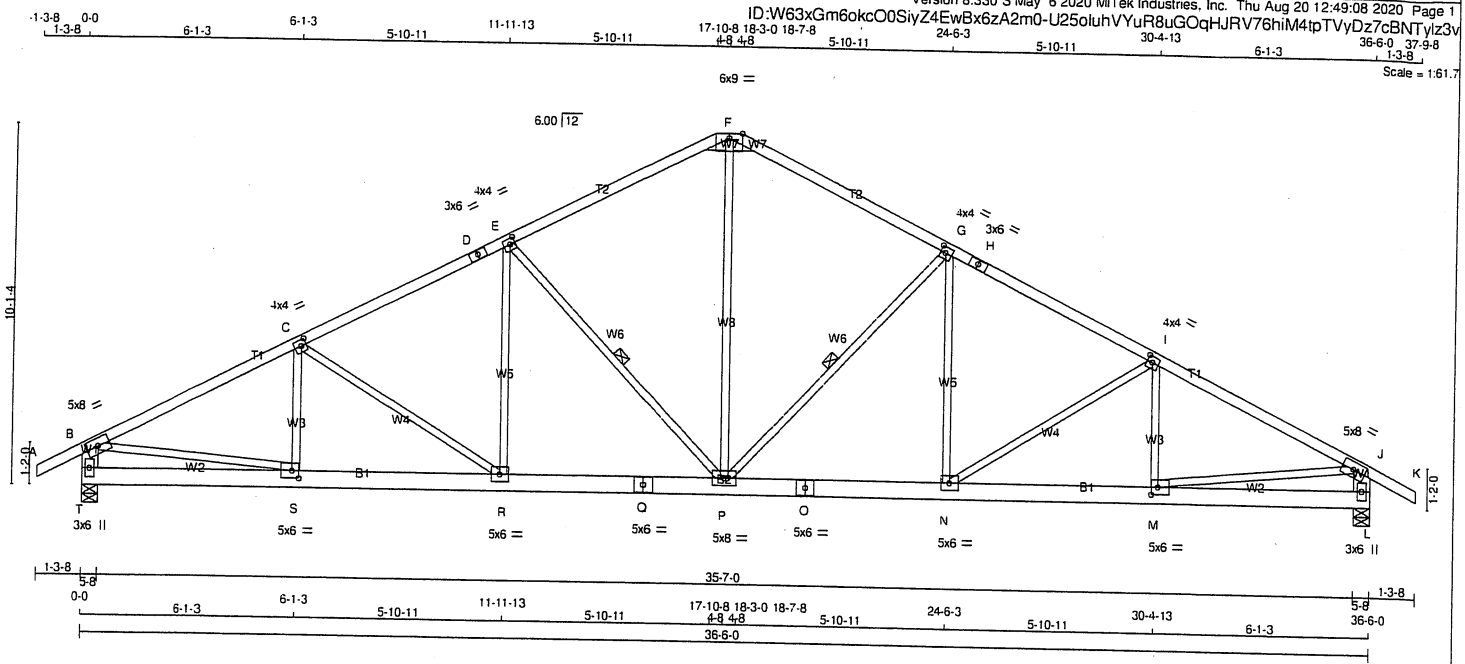

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C, E, G, I						
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
F	TMTMWWW-m	MT20	6.0	9.0	1.50	4.50
H	TS-t	MT20	3.0	6.0		
J	TMVW-t	MT20	5.0	8.0		
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.25
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.25
T	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 2 X 177 = 354 lb [M][F]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	2137	0	2137	0	0	IN-SX	IN-SX
L	2137	0	2137	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0
L	1509	1003 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	-3045 / 0	-91.8	-91.8	0.61 (1)	3.49
C- D	-2734 / 0	-91.8	-91.8	0.52 (1)	3.76
D- E	-2734 / 0	-91.8	-91.8	0.52 (1)	3.76
E- F	-2106 / 0	-91.8	-91.8	0.52 (1)	4.16
F- G	-2106 / 0	-91.8	-91.8	0.52 (1)	4.16
G- H	-2734 / 0	-91.8	-91.8	0.52 (1)	3.76
H- I	-2734 / 0	-91.8	-91.8	0.52 (1)	3.76
I- J	-3045 / 0	-91.8	-91.8	0.61 (1)	3.49
J- K	0 / 28	-91.8	-91.8	0.12 (1)	10.00
T- B	-2075 / 0	0.0	0.0	0.13 (1)	7.03
L- J	-2075 / 0	0.0	0.0	0.13 (1)	7.03
T- S	0 / 0	-18.5	-18.5	0.08 (4)	10.00
S- R	0 / 2744	-18.5	-18.5	0.38 (1)	10.00
R- Q	0 / 2447	-18.5	-18.5	0.34 (1)	10.00
Q- P	0 2447	-18.5	-18.5	0.34 (1)	10.00
P- O	0 2447	-18.5	-18.5	0.34 (1)	10.00
O- N	0 2447	-18.5	-18.5	0.34 (1)	10.00
N- M	0 2744	-18.5	-18.5	0.38 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.08 (4)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
S- C	-253 / 11	0.06 (1)
C- R	-353 / 0	0.33 (1)
R- E	0 / 308	0.07 (1)
E- P	-862 / 0	0.50 (1)
P- G	-862 / 0	0.50 (1)
N- G	0 308	0.07 (1)
N- I	-353 / 0	0.33 (1)
M- I	-253 / 11	0.06 (1)
B- S	0 2766	0.62 (1)
M- J	0 2766	0.62 (1)
P- F	0 1395	0.31 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.61/1.00 (I-J:1) , BC=0.38/1.00 (R-S:1) , WB=0.62/1.00 (B-S:1) , SSI=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.62 (S) (INPUT = 1.00)

Structural component only
DWG# T-2018777

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410070	T37	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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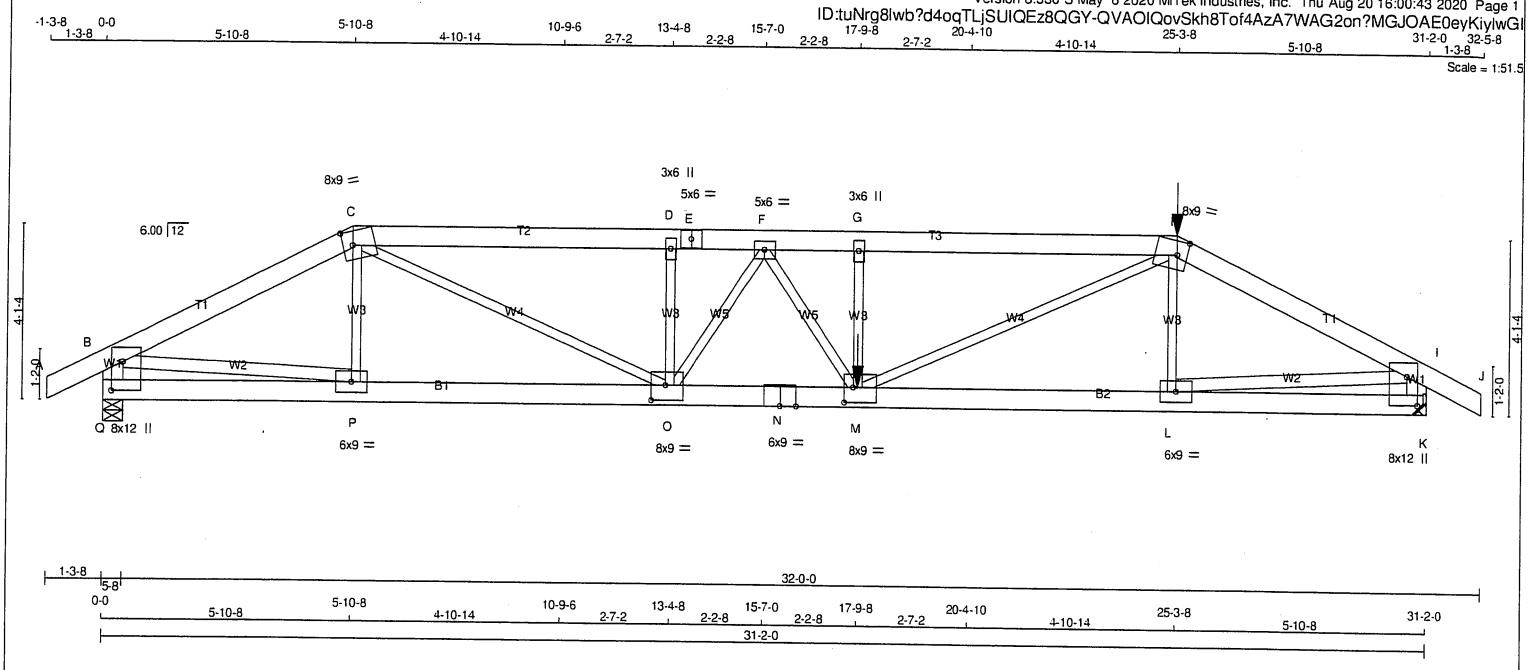

TOTAL WEIGHT = 2 X 164 = 327 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - P	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- E	2	12	TOP
E- H	2	12	SIDE(64.0)
H- J	2	12	SIDE(122.0)
Q- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q- N	2	12	TOP
N- K	2	12	SIDE(145.2)
WEBS : (0.122"X3") SPIRAL NAILS			
L- H	1	6	SIDE(24.4)
M- G	1	3	SIDE(600.6)
2x3	1	6	
O- D	1	3	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

LICENSED PROFESSIONAL ENGINEER
06/08/22
H. J./G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018825 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	3695	0	3695	0	0	5-8	5-8
K	5403	0	5403	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2621	1675 / 0	0 / 0	0 / 0	0 / 0	946 / 0	0 / 0
K	3869	2270 / 0	0 / 0	0 / 0	0 / 0	1598 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 29	-91.8	-91.8	0.03 (1)	10.00	P- C	-412 / 0	0.05 (1)
B- C	-5931 / 0	-91.8	-91.8	0.21 (1)	4.68	C- O	0 / 5681	0.70 (1)
C- D	-10401 / 0	-91.8	-91.8	0.43 (1)	3.52	M- H	0 / 5582	0.69 (1)
D- E	-10401 / 0	-91.8	-91.8	0.26 (1)	3.64	L- H	-104 / 477	0.09 (4)
E- F	-10401 / 0	-91.8	-91.8	0.26 (1)	3.64	B- P	0 / 5335	0.47 (1)
F- G	-12815 / 0	-91.8	-91.8	0.54 (1)	2.97	L- I	0 / 7845	0.69 (1)
G- H	-12815 / 0	-219.8	-219.8	0.84 (1)	2.70	O- D	-685 / 0	0.08 (1)
H- I	-8723 / 0	-91.8	-91.8	0.28 (1)	3.92	M- G	-1465 / 0	0.18 (1)
I- J	0 / 29	-91.8	-91.8	0.03 (1)	10.00	O- F	-2049 / 0	0.31 (1)
Q- B	-3664 / 0	0.0	0.0	0.13 (1)	7.35	F- M	0 / 2611	0.32 (1)
K- I	-5201 / 0	0.0	0.0	0.18 (1)	6.40			
Q- P	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
P- O	0 / 5288	-18.5	-18.5	0.42 (1)	10.00			
O- N	0 / 11463	-18.5	-18.5	0.81 (1)	10.00			
N- M	0 / 11463	-18.5	-18.5	0.81 (1)	10.00			
M- L	0 / 7792	-94.3	-94.3	0.68 (1)	10.00			
L- K	0 / 0	-94.3	-94.3	0.29 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	25-3-8	-573	-573	---	FRONT	VERT	TOTAL	---	C1
M	17-9-8	-2859	-2859	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 0-0
RIGHT SETBACK = 5-10-8
END SETBACK = 7-10-8
END WALL WIDTH = 3-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 13-4-8 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 787 (0.48")

CSI: TC=0.84/1.00 (G-H:1) , BC=0.81/1.00 (M-O:1) , WB=0.70/1.00 (C-O:1) , SSI=0.31/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410070	T37	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:tuNrg8lwb?d4oqTLjSUlQEz8QGY-QVAOlQovSkh8Tof4AzA7WAG2on?MGJOAE0eyKiylwGl

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, I, K, Q						
B						
C	TTWW-m	MT20	8.0	9.0	4.00	2.75
D	TMW+w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWW-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TTWW-m	MT20	8.0	9.0	4.00	2.75
K	TMBMVW1"+p	MT20	8.0	12.0	8.00	3.00
L	BMWW-t	MT20	6.0	9.0		
M	BMWWW-t	MT20	8.0	9.0	4.25	2.50
N	BS-t	MT20	6.0	9.0		
O	BMWWW-t	MT20	8.0	9.0	4.25	4.00
P	BMWW-t	MT20	6.0	9.0		
Q	TMBMVW1"+p	MT20	8.0	12.0	8.00	3.00

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.86 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018825 JL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410070	T38	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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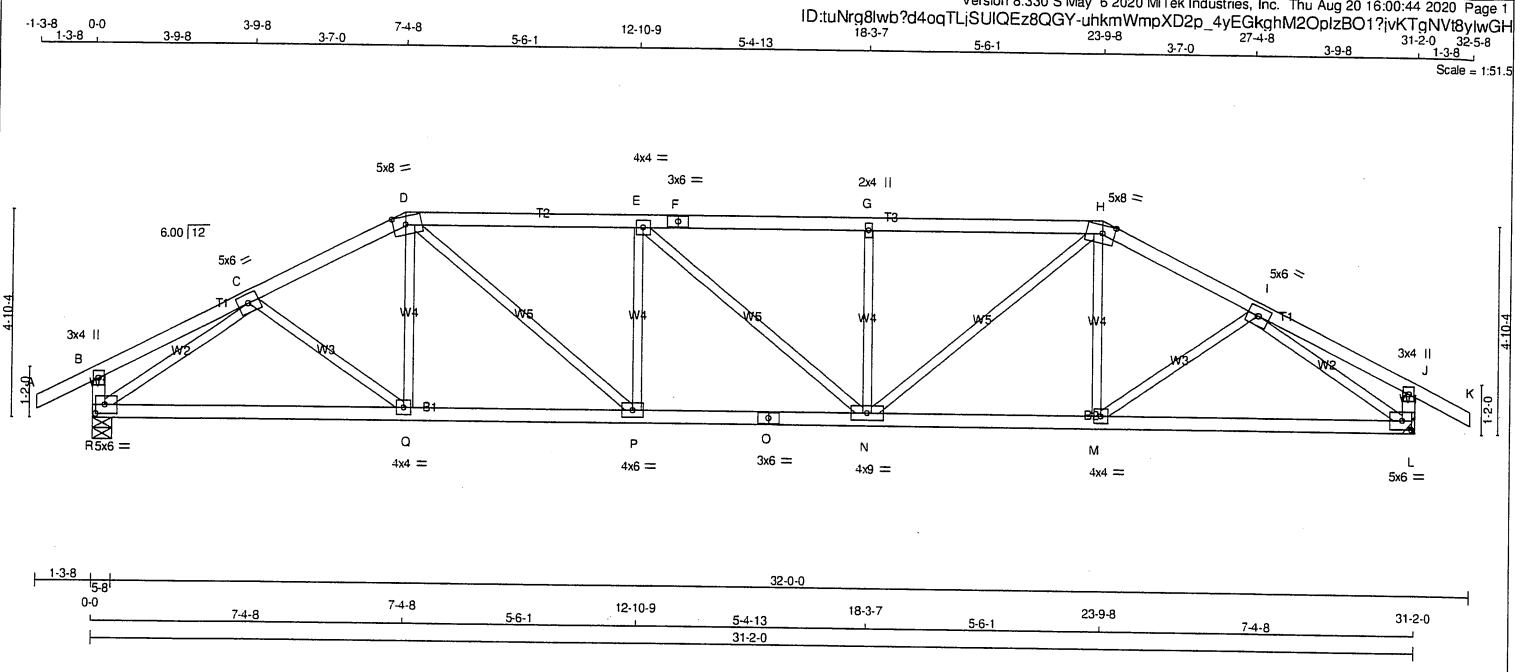

TOTAL WEIGHT = 123 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	8.0	2.25	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.25	3.50
I	TMWW-t	MT20	5.0	6.0		
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.50	2.50
M	BMWW-t	MT20	4.0	4.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMVW1-t	MT20	5.0	6.0	2.50	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	1843	0	1843	0	0	5-8	5-8
L	1843	0	1843	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1301	867 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0
L	1301	867 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	C- Q	0 / 85	0.03 (4)
B- C	0 / 14	-91.8	-91.8	0.16 (1)	10.00	Q- D	0 / 110	0.04 (4)
C- D	-2411 / 0	-91.8	-91.8	0.25 (1)	4.21	D- P	0 / 991	0.22 (1)
D- E	-2908 / 0	-91.8	-91.8	0.56 (1)	3.58	P- E	-542 / 0	0.19 (1)
E- F	-2906 / 0	-91.8	-91.8	0.55 (1)	3.58	E- N	-2 / 0	0.00 (1)
F- G	-2906 / 0	-91.8	-91.8	0.55 (1)	3.58	N- G	-542 / 0	0.19 (1)
G- H	-2906 / 0	-91.8	-91.8	0.55 (1)	3.59	N- H	0 / 989	0.22 (1)
H- I	-2411 / 0	-91.8	-91.8	0.25 (1)	4.21	M- H	0 / 111	0.04 (4)
I- J	0 / 14	-91.8	-91.8	0.16 (1)	10.00	M- I	0 / 85	0.03 (4)
J- K	0 / 28	-91.8	-91.8	0.12 (1)	10.00	R- C	-2578 / 0	0.88 (1)
R- B	-262 / 0	0.0	0.0	0.03 (1)	7.81	I- L	-2578 / 0	0.88 (1)
L- J	-262 / 0	0.0	0.0	0.03 (1)	7.81			
R- Q	0 / 2081	-18.5	-18.5	0.46 (1)	10.00			
Q- P	0 / 2144	-18.5	-18.5	0.46 (1)	10.00			
P- O	0 / 2908	-18.5	-18.5	0.53 (1)	10.00			
O- N	0 / 2908	-18.5	-18.5	0.53 (1)	10.00			
N- M	0 / 2144	-18.5	-18.5	0.46 (1)	10.00			
M- L	0 / 2081	-18.5	-18.5	0.46 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.56/1.00 (D-E:1) , BC=0.53/1.00 (N-P:1) , WB=0.88/1.00 (I-L:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.93 (O) (INPUT = 1.00)

Structural component only
DWG# T-2018826

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410070	T39	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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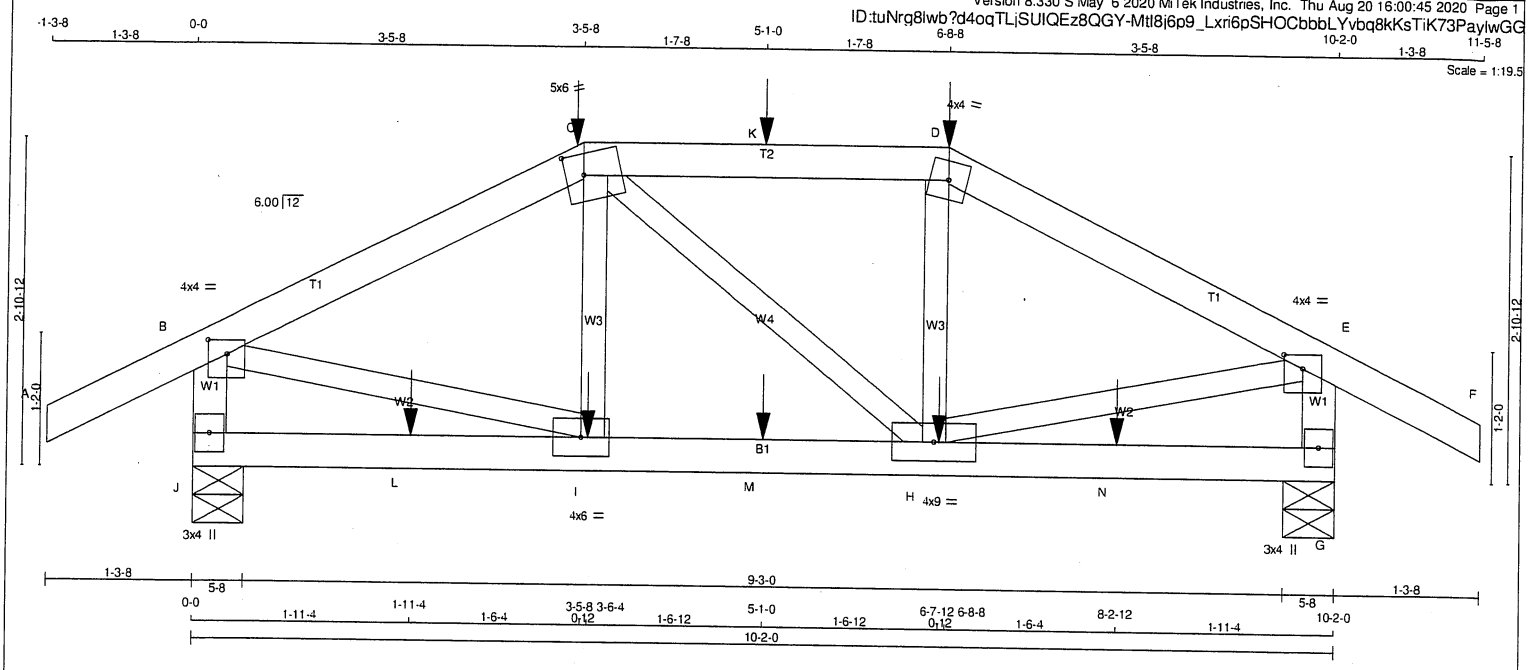

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.50	2.00
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMVW-p	MT20	4.0	4.0	1.50	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	876	0	876	0	0	5-8	5-8
G	876	0	876	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	617	419 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0
G	617	419 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD FROM (PLF)	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.13 (1)	10.00	I- C	-103 / 33	0.02 (1)
B- C	-846 / 0	-91.8	-91.8	0.22 (1)	6.25	C- H	0 / 3	0.00 (4)
C- K	-753 / 0	-91.8	-91.8	0.23 (1)	6.25	H- D	-100 / 35	0.02 (1)
K- D	-753 / 0	-91.8	-91.8	0.23 (1)	6.25	B- I	0 / 778	0.19 (1)
D- E	-851 / 0	-91.8	-91.8	0.22 (1)	6.25	H- E	0 / 781	0.19 (1)
E- F	0 / 28	-91.8	-91.8	0.13 (1)	10.00			
J- B	-845 / 0	0.0	0.0	0.09 (1)	7.81			
G- E	-844 / 0	0.0	0.0	0.09 (1)	7.81			
J- L	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
L- I	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
I- M	0 / 752	-18.5	-18.5	0.15 (1)	10.00			
M- H	0 / 752	-18.5	-18.5	0.15 (1)	10.00			
H- N	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
N- G	0 / 0	-18.5	-18.5	0.07 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-5-8	-160	-160	---	BACK	VERT	TOTAL	---	C1
D	6-8-8	-160	-160	---	BACK	VERT	TOTAL	---	C1
H	6-7-12	-9	-9	---	BACK	VERT	TOTAL	---	C1
I	3-6-4	-9	-9	---	BACK	VERT	TOTAL	---	C1
K	5-1-0	-27	-27	---	BACK	VERT	TOTAL	---	C1
L	1-11-4	-4	-6	---	BACK	VERT	TOTAL	---	C1
M	5-1-0	-9	-9	---	BACK	VERT	TOTAL	---	C1
N	8-2-12	-4	-6	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 41 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.23/1.00 (C-D:1) , BC=0.15/1.00 (H-I:1) , WB=0.19/1.00 (E-H:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (E) (INPUT = 0.90)
JSI METAL= 0.27 (E) (INPUT = 1.00)

Structural component only
DWG# T-2018827

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410070	T40	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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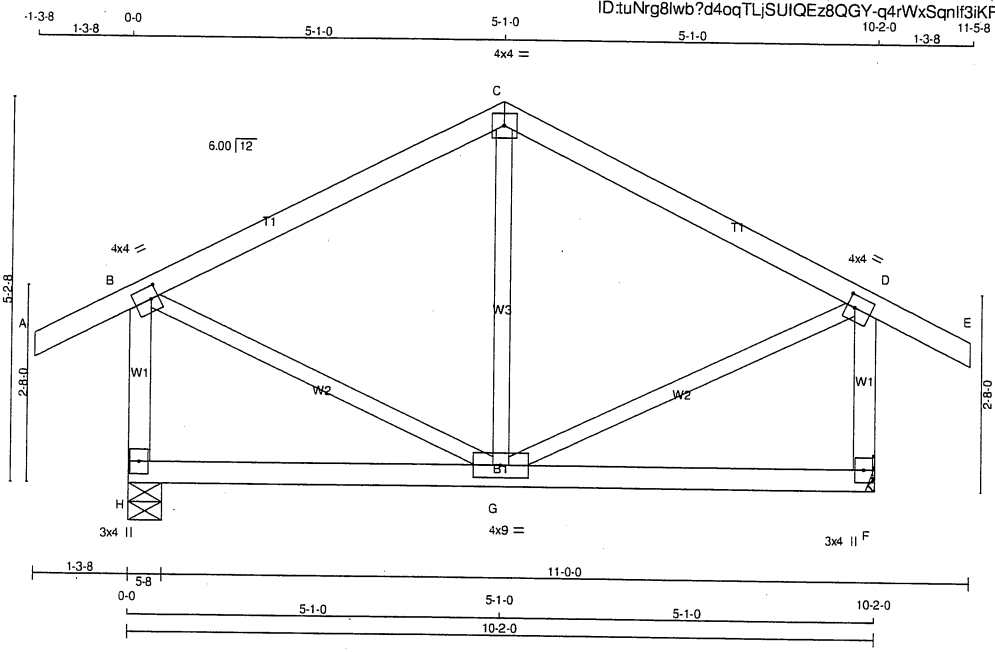

TOTAL WEIGHT = 2 X 45 = 91 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-p	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	685	0	685	0	0	5-8	5-8
F	685	0	685	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	482	330 / 0	0 / 0	0 / 0	0 / 0	153 / 0	0 / 0
F	482	330 / 0	0 / 0	0 / 0	0 / 0	153 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	G- C	-165 / 27	0.07 (1)
B- C	-338 / 0	-91.8	-91.8	0.30 (1)	6.25	B- G	0 / 333	0.07 (1)
C- D	-338 / 0	-91.8	-91.8	0.30 (1)	6.25	G- D	0 / 333	0.07 (1)
D- E	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
H- B	-649 / 0	0.0	0.0	0.09 (1)	7.81			
F- D	-649 / 0	0.0	0.0	0.09 (1)	7.81			
H- G	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.30/1.00 (B-C:1) , BC=0.14/1.00 (F-G:4) , WB=0.07/1.00 (D-G:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (B) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

Structural component only
DWG# T-2018828

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410070	T41	1	2	GREEN PARK HOMES TRUSS DESC.	

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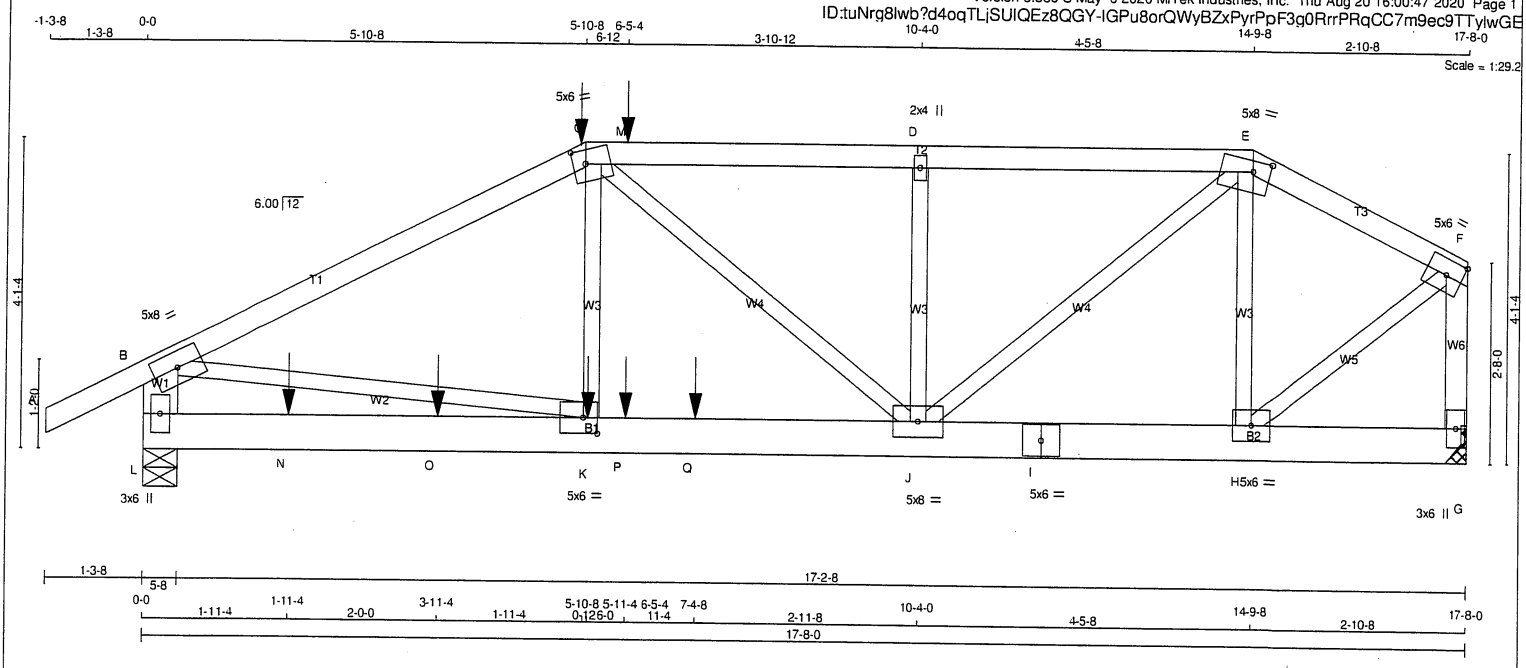

TOTAL WEIGHT = 2 X 82 = 165 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- E	1	12	SIDE(61.0)
E- F	1	12	TOP
G- F	1	12	TOP
L- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- I	2	12	SIDE(183.1)
I- G	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

Structural component only
DWG# T-2018829 R2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	2130	0	2130	0	0	5-8	5-8
G	1614	0	1614	0	0	IN-SX MECHANICAL	IN-SX

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1502	1010 / 0	0 / 0	0 / 0	0 / 0	492 / 0	0 / 0
G	1138	764 / 0	0 / 0	0 / 0	0 / 0	374 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	K- C	0 / 583	0.07 (1)
B- C	-3015 / 0	-91.8	-91.8	0.39 (1)	4.90	C- J	-286 / 0	0.08 (1)
C- M	-2492 / 0	-91.8	-91.8	0.20 (1)	5.51	J- D	-514 / 0	0.07 (1)
M- D	-2492 / 0	-91.8	-91.8	0.20 (1)	5.51	J- E	0 / 1786	0.22 (1)
D- E	-2492 / 0	-91.8	-91.8	0.20 (1)	5.51	H- E	-861 / 0	0.11 (1)
E- F	-1281 / 0	-91.8	-91.8	0.08 (1)	6.25	B- K	0 / 2717	0.34 (1)
L- B	-2094 / 0	0.0	0.0	0.07 (1)	7.81	H- F	0 / 1440	0.18 (1)
G- F	-1581 / 0	0.0	0.0	0.11 (1)	7.81			
L- N	0 / 0	-18.5	-18.5	0.10 (1)	10.00			
N- O	0 / 0	-18.5	-18.5	0.10 (1)	10.00			
O- K	0 / 0	-18.5	-18.5	0.10 (1)	10.00			
K- P	0 / 2711	-18.5	-18.5	0.46 (1)	10.00			
P- Q	0 / 2711	-18.5	-18.5	0.46 (1)	10.00			
Q- J	0 / 2711	-18.5	-18.5	0.46 (1)	10.00			
J- I	0 / 1120	-18.5	-18.5	0.15 (1)	10.00			
I- H	0 / 1120	-18.5	-18.5	0.15 (1)	10.00			
H- G	0 / 0	-18.5	-18.5	0.01 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-408	-408	---	FRONT	VERT	TOTAL	---	C1
K	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
M	6-5-4	-131	-131	---	FRONT	VERT	TOTAL	---	C1
N	1-11-4	-25	-26	---	FRONT	VERT	TOTAL	---	C1
O	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	6-5-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Q	7-4-8	-1029	-1029	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.39/1.00 (B-C:1) , BC=0.46/1.00 (J-K:1) , WB=0.34/1.00 (B-K:1) , SSI=0.26/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (J) (INPUT = 0.90)
JSI METAL= 0.31 (K) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410070	T41	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:tuNrg8lwb?d4oqTLjSUIQEz8QGY-nSzHM8s2HGJQZZX1zWmIDEz0bon3xfMvOILj0vyIwGD

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	1.75	2.75
F	TMVW-t	MT20	5.0	6.0		Edge
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.25
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2018829 9/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410070	T42	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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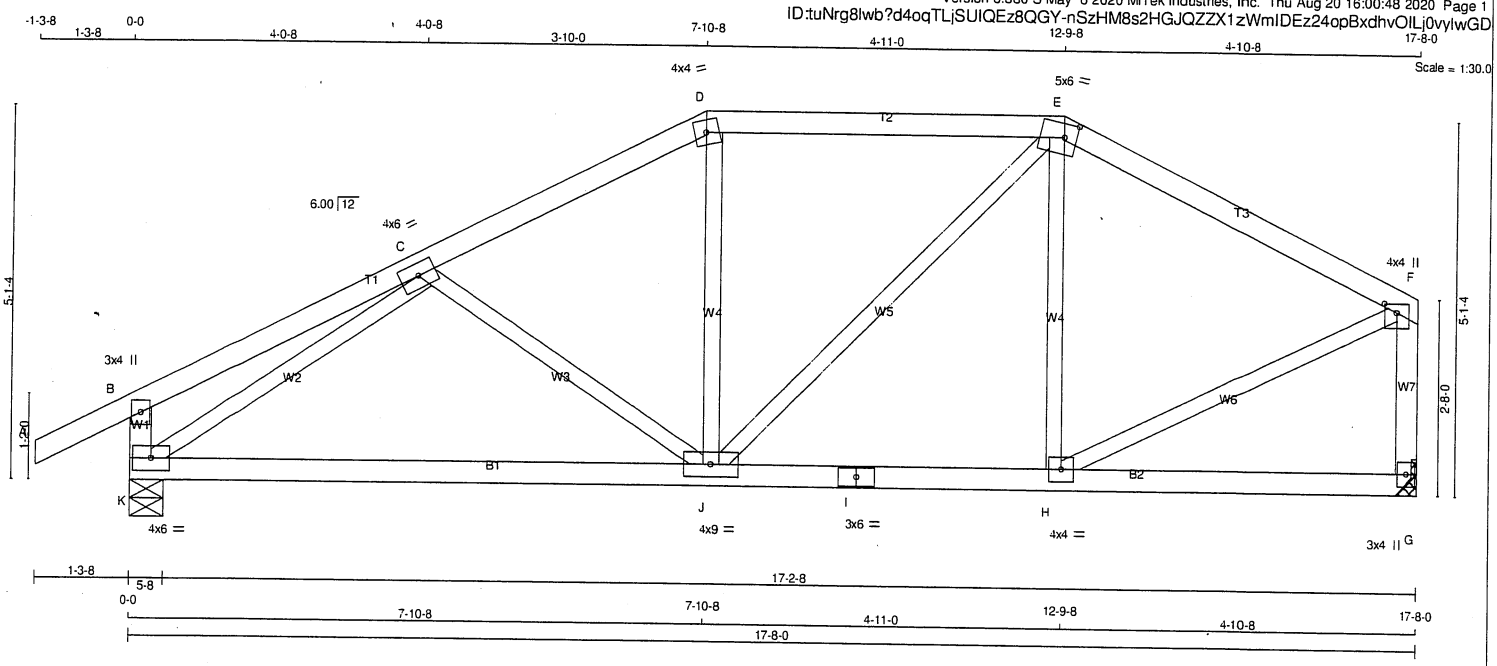

TOTAL WEIGHT = 71 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMVW+p	MT20	4.0	4.0	1.50	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1098	0	1098	0	0	5-8	5-8
G	974	0	974	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	774	521 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
G	689	452 / 0	0 / 0	0 / 0	0 / 0	237 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	C-J	-214 / 0		0.09 (1)
B-C	0 / 18	-91.8	-91.8	0.22 (1)	10.00	J-D	0 / 95		0.03 (4)
C-D	-1026 / 0	-91.8	-91.8	0.18 (1)	5.99	J-E	0 / 255		0.06 (1)
D-E	-902 / 0	-91.8	-91.8	0.29 (1)	6.11	H-E	-265 / 0		0.10 (1)
E-F	-814 / 0	-91.8	-91.8	0.29 (1)	6.25	K-C	-1323 / 0		0.51 (1)
K-B	-266 / 0	0.0	0.0	0.03 (1)	7.81	H-F	0 / 805		0.18 (1)
G-F	-933 / 0	0.0	0.0	0.13 (1)	7.81				
K-J	0 / 1075	-18.5	-18.5	0.32 (4)	10.00				
J-I	0 / 722	-18.5	-18.5	0.30 (4)	10.00				
I-H	0 / 722	-18.5	-18.5	0.30 (4)	10.00				
H-G	0 / 0	-18.5	-18.5	0.10 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.29/1.00 (D-E:1) , BC=0.32/1.00 (J-K:4) , WB=0.51/1.00 (C-K:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (H) (INPUT = 0.90)
JSI METAL= 0.27 (C) (INPUT = 1.00)

Structural component only
DWG# T-2018830

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410070	T43	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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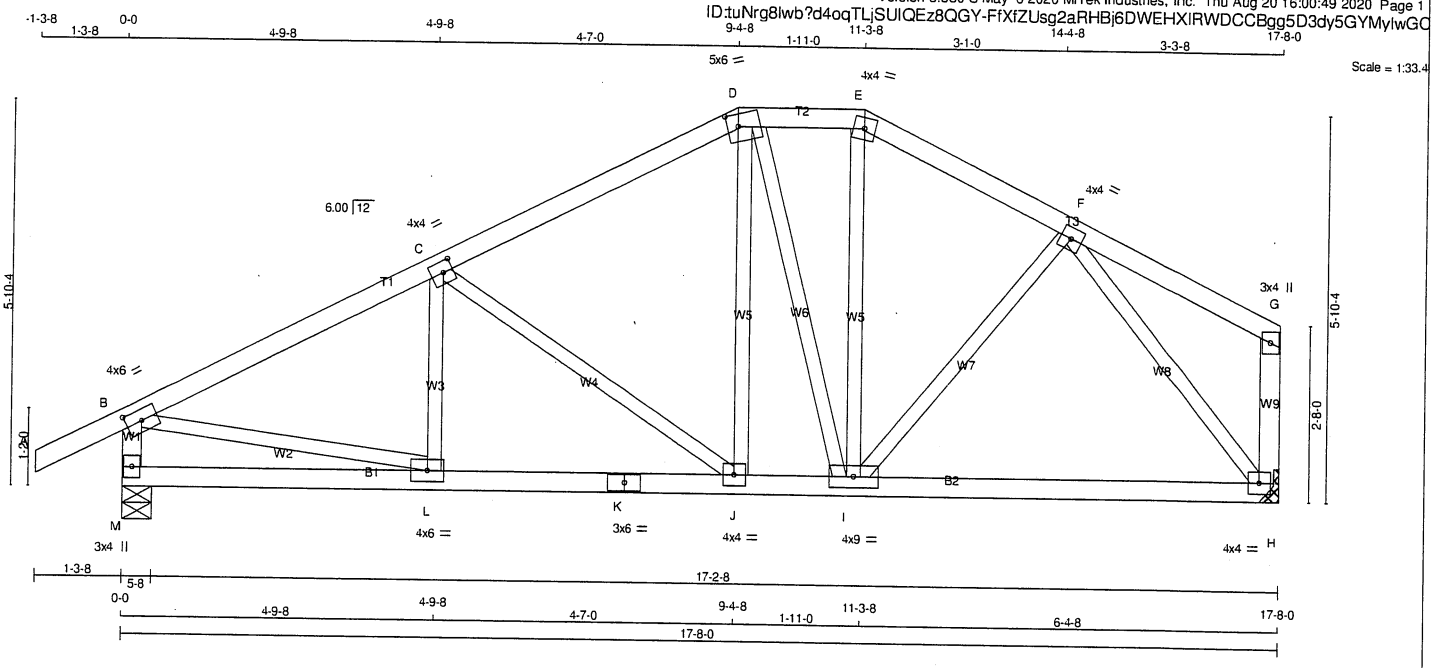

TOTAL WEIGHT = 79 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1098	0	1098	0	0	5-8	5-8
H	974	0	974	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	774	521 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
H	689	452 / 0	0 / 0	0 / 0	0 / 0	237 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	-1239 / 0	-91.8	-91.8	0.27 (1)	5.47
C- D	-882 / 0	-91.8	-91.8	0.26 (1)	6.22
D- E	-731 / 0	-91.8	-91.8	0.05 (1)	6.25
E- F	-830 / 0	-91.8	-91.8	0.12 (1)	6.25
F- G	0 / 14	-91.8	-91.8	0.14 (1)	10.00
M- B	-1061 0	0.0	0.0	0.11 (1)	7.64
H- G	-114 / 0	0.0	0.0	0.02 (1)	7.81
M- L	0 / 0	-18.5	-18.5	0.10 (4)	10.00
L- K	0 / 1128	-18.5	-18.5	0.23 (1)	10.00
K- J	0 / 1128	-18.5	-18.5	0.23 (1)	10.00
J- I	0 / 773	-18.5	-18.5	0.24 (4)	10.00
I- H	0 / 655	-18.5	-18.5	0.23 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-102 / 48	0.02 (1)
C- J	-438 / 0	0.24 (1)
J- D	0 / 266	0.06 (1)
D- I	-133 / 0	0.08 (1)
I- E	0 / 165	0.04 (1)
I- F	0 / 114	0.03 (4)
B- L	0 / 1146	0.26 (1)
F- H	-1043 / 0	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.27/1.00 (B-C:1) , BC=0.24/1.00 (I-J:4) , WB=0.43/1.00 (F-H:1) , SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.40 (K) (INPUT = 1.00)

Structural component only
DWG# T-2018831

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410070	T44	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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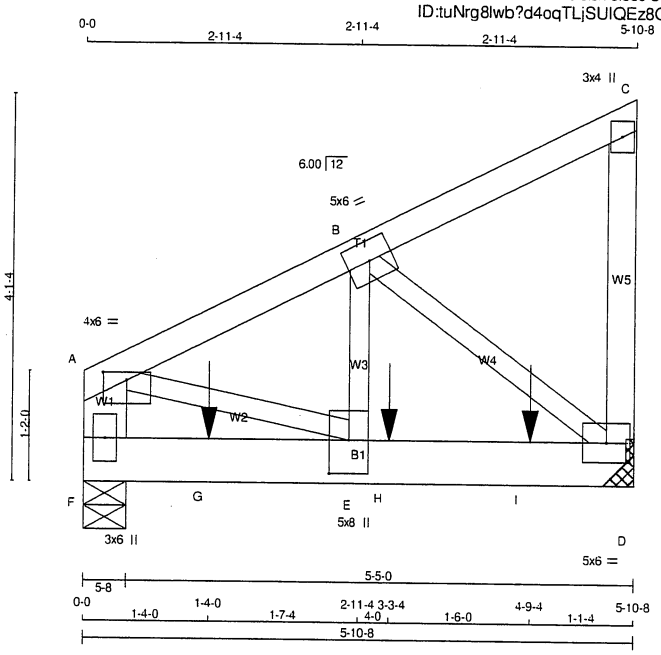

TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	6	SIDE(66.2)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	5.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		
E	BMWW+t	MT20	5.0	8.0	4.25	2.50

LICENSED PROFESSIONAL ENGINEER
05/20/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018832 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	3044	0	3044	0	0	5-8	5-8
D	2878	0	2878	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	2148	1436 / 0	0 / 0	0 / 0	0 / 0	712 / 0	0 / 0
D	2031	1353 / 0	0 / 0	0 / 0	0 / 0	679 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
F- A	-1966 / 0	0.0	0.0	0.07 (1)	7.81	A- E	0 / 2499	0.31 (1)
A- B	-2695 / 0	-91.8	-91.8	0.08 (1)	5.48	E- B	0 / 2755	0.34 (1)
B- C	-6 / 0	-91.8	-91.8	0.06 (1)	10.00	B- D	-3040 / 0	0.36 (1)
D- C	-121 / 0	0.0	0.0	0.01 (1)	7.81			
F- G	0 / 0	-319.4	-319.4	0.42 (1)	10.00			
G- E	0 / 0	-319.4	-319.4	0.42 (1)	10.00			
E- H	0 / 2416	-319.4	-319.4	0.46 (1)	10.00			
H- I	0 / 2416	-319.4	-319.4	0.46 (1)	10.00			
I- D	0 / 2416	-319.4	-319.4	0.46 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-4-0	-1595	-1595	---	BACK	VERT	TOTAL	---	C1
H	3-3-4	-955	-955	---	BACK	VERT	TOTAL	---	C1
I	4-9-4	-955	-955	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 13-3-0
END DISTANCE = 5-10-8
END SPAN CARRIED = 13-3-0
END WALL WIDTH = 4-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.08/1.00 (A-B:1) , BC=0.46/1.00 (D-E:1) , WB=0.36/1.00 (B-D:1) , SSI=0.64/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.44 (E) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410070	T44	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

LICENSED PROFESSIONAL ENGINEER
08/20/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018832

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410070	T45	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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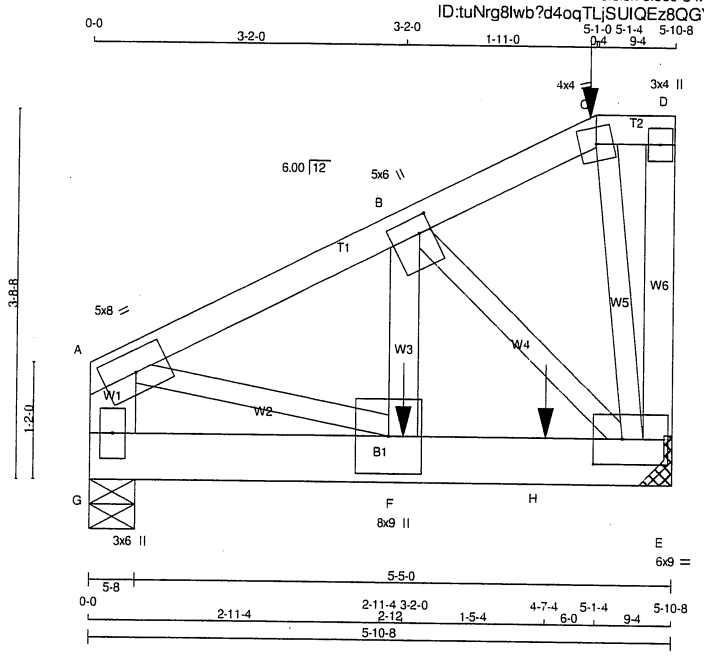

TOTAL WEIGHT = 2 X 32 = 64 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - A	2x6	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - B	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- D	1	12	SIDE(61.0)
D- E	1	12	TOP
G- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
G- E	2	5	SIDE(549.3)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
B- F	2	2	SIDE(855.4)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018833

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	5768	0	5768	0	0	MECHANICAL	
G	4120	0	4120	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	4097	2589 / 0	0 / 0	0 / 0	0 / 0	1508 / 0	0 / 0
G	2933	1814 / 0	0 / 0	0 / 0	0 / 0	1119 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-5235 / 0	-91.8	-91.8	0.21 (1)	4.07	A- F	0 / 4813	0.60 (1)	
B- C	-86 / 0	-91.8	-91.8	0.06 (1)	6.25	F- B	0 / 6100	0.54 (1)	
C- D	0 / 0	-91.8	-91.8	0.01 (1)	10.00	C- E	-383 / 0	0.04 (1)	
E- D	-36 / 0	0.0	0.0	0.00 (1)	7.81	B- E	-6081 / 0	0.66 (1)	
G- A	-3644 / 0	0.0	0.0	0.13 (1)	7.36				
G- F	0 / 0	-319.4	-319.4	0.13 (1)	10.00				
F- H	0 / 4676	-319.4	-319.4	0.85 (1)	10.00				
H- E	0 / 4676	-319.4	-319.4	0.85 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-1-0	-264	-264	---	FRONT	VERT	TOTAL	---	C1
F	3-2-0	-5385	-5385	---	BACK	VERT	TOTAL	---	C1
H	4-7-4	-1824	-1824	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 13-3-0
END DISTANCE = 5-10-8
END SPAN CARRIED = 13-3-0
END WALL WIDTH = 4-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.21/1.00 (A-B:1) , BC=0.85/1.00 (E-F:1) , WB=0.66/1.00 (B-E:1) , SSI=0.45/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.89 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
410070	T45	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0		
B	TMWW+t	MT20	5.0	6.0	2.00	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMV+p	MT20	3.0	4.0		
E	BMVWW1-t	MT20	6.0	9.0	3.00	3.50
F	BMWW+t	MT20	8.0	9.0		
G	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2018833 Y2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410070	T46	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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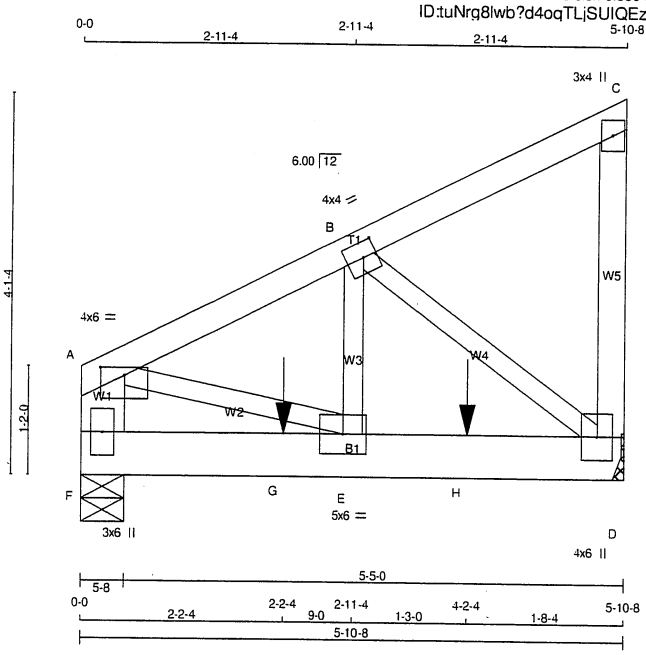

TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

LICENSED PROFESSIONAL ENGINEER
08/20/20
H. J./G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018834

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	933	0	933	0	0	5-8	5-8
D	1047	0	1047	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	656	452 / 0	0 / 0	0 / 0	0 / 0	205 / 0	0 / 0
D	736	508 / 0	0 / 0	0 / 0	0 / 0	228 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
F- A	-818 / 0	0.0	0.0	0.03 (1)	7.81	A- E	0 / 960	0.12 (1)	
A- B	-1026 / 0	-91.8	-91.8	0.06 (1)	6.25	E- B	0 / 851	0.11 (1)	
B- C	-12 / 0	-91.8	-91.8	0.06 (1)	6.25	B- D	-1168 / 0	0.14 (1)	
D- C	-109 / 0	0.0	0.0	0.01 (1)	7.81				
F- G	0 / 0	-18.5	-18.5	0.09 (1)	10.00				
G- E	0 / 0	-18.5	-18.5	0.09 (1)	10.00				
E- H	0 / 929	-18.5	-18.5	0.19 (1)	10.00				
H- D	0 / 929	-18.5	-18.5	0.19 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-2-4	-666	-666	---	FRONT	VERT	TOTAL	---	C1
H	4-2-4	-666	-666	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.19/1.00 (D-E:1) , WB=0.14/1.00 (B-D:1) , SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (B) (INPUT = 0.90)
JSI METAL= 0.21 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409707	J1	25	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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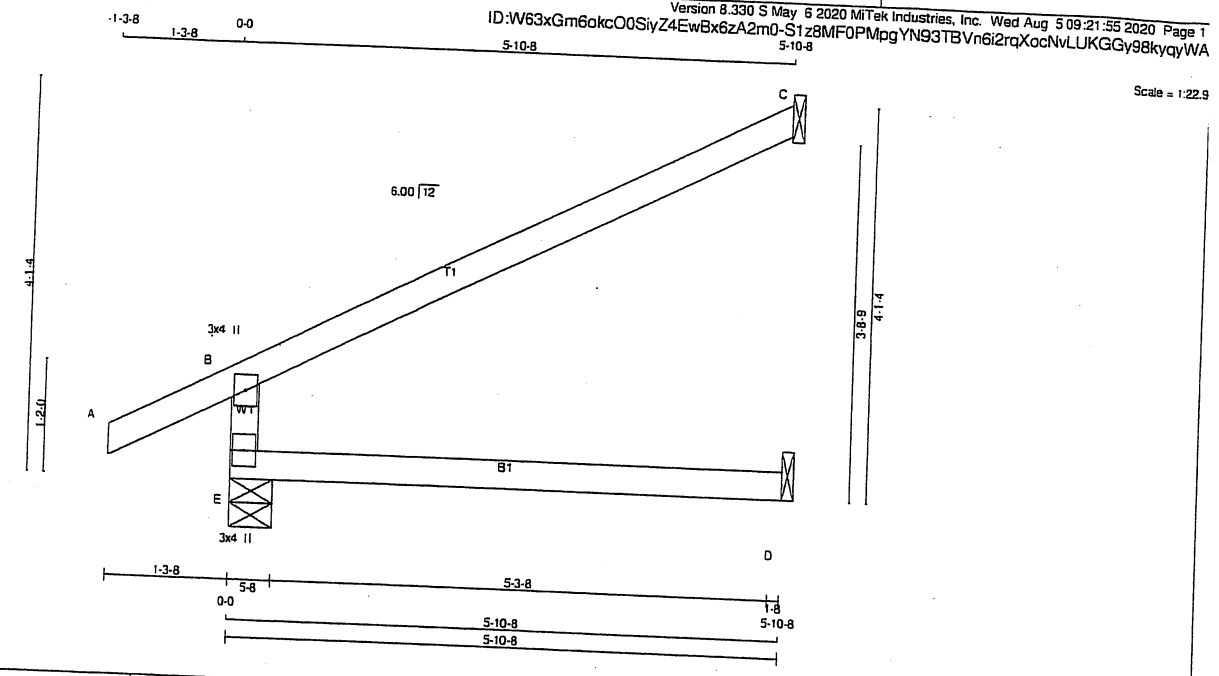

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 25 X 17 = 420 lb [M]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	525	0	525	0	0	5-8	5-8
C	202	0	202	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	369	257 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 0
C	139	113 . 0	0 / 0	0 . 0	0 . 0	26 0	0 0
D	36	0 / 0	0 / 0	0 / 0	0 0	36 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)		FACTORED VERT LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
E- B	-461	0	0.0	0.0	0.13 (4)	7.81
A- B	0	28	-91.8	-91.8	0.12 (1)	10.00
B- C	-30	0	-91.8	-91.8	0.54 (1)	6.25
E- D	0	0	-18.5	-18.5	0.13 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1 10 COMP=1.10 SHEAR=1 10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410068	J2	8	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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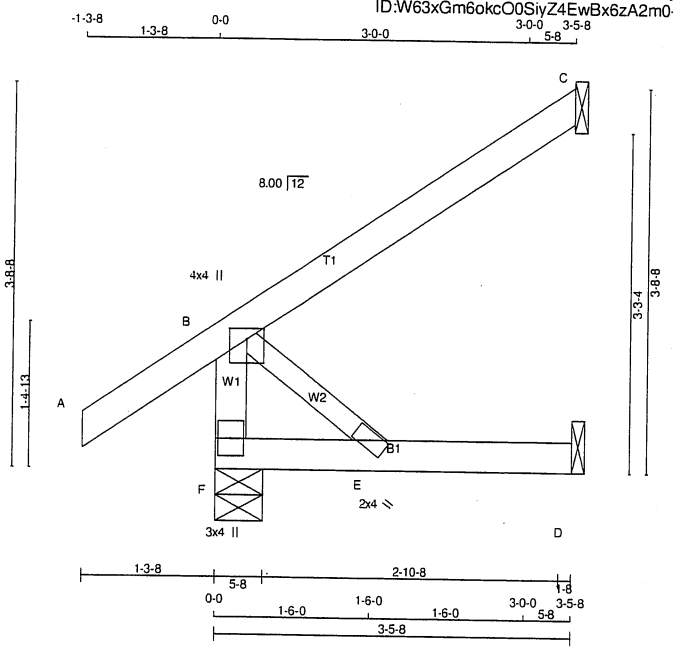

TOTAL WEIGHT = 8 X 13 = 105 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	317	0	317	0	0	5-8	5-8
C	159	0	159	0	0	1-8	1-8
D	32	0	36	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	222	159 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0
C	109	88 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0
D	26	0 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
F- B	-285 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)	
A- B	0 / 35	-91.8	-91.8	0.14 (5)	10.00				
B- C	0 / 0	-91.8	-91.8	0.19 (1)	10.00				
F- E	0 / 0	-18.5	-18.5	0.06 (4)	10.00				
E- D	0 / 0	-18.5	-18.5	0.06 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.19/1.00 (B-C:1) , BC=0.06/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018792

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410069	J20	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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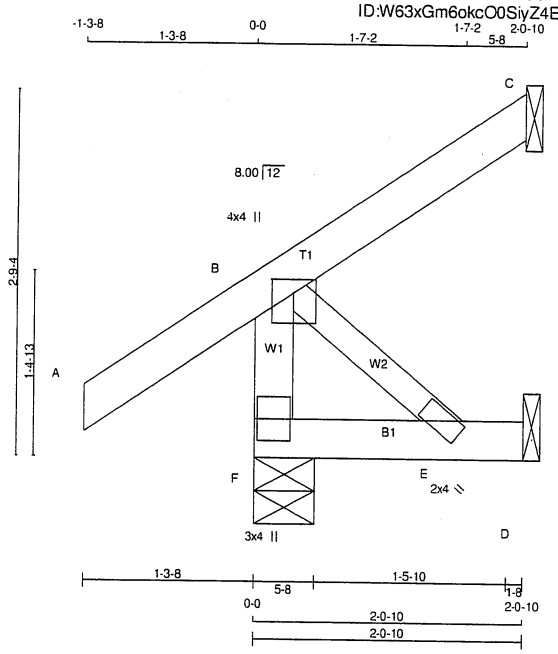

TOTAL WEIGHT = 2 X 10 = 19 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	239	0	239	0	0	5-8	5-8
C	94	0	94	0	0	1-8	1-8
D	19	0	21	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	167	123 / 0	0 / 0	0 / 0	0 / 0	44 / 0	0 / 0
C	65	53 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0
D	15	0 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-220 / 0	0.0	0.0	0.02 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00			
B- C	0 / 0	-91.8	-91.8	0.07 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

Structural component only
DWG# T-2018808

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410069	J21	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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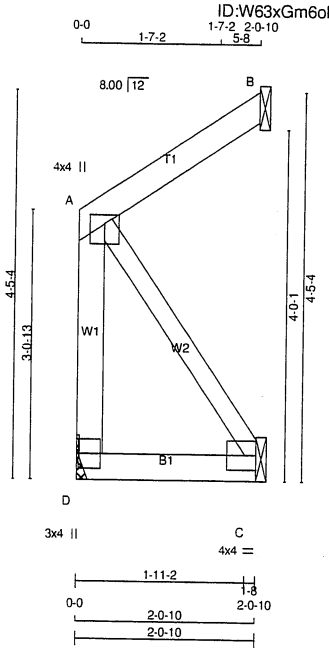

TOTAL WEIGHT = 11 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	BMW1-t	MT20	4.0	4.0		Edge
D	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	113	0	113	0	0	MECHANICAL	
B	94	0	94	0	0	1-8	1-8
C	18	0	21	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	80	53 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
B	65	53 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0
C	15	0 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- A	-94 / 0	0.0	0.0	0.02 (1)	7.81	A- C	0 / 0	0.00 (1)
A- B	0 / 0	-91.8	-91.8	0.07 (1)	10.00			
D- C	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.07/1.00 (A-B:1) , BC=0.02/1.00 (C-D:4) , WB=0.00/1.00 (A-C:1) , SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.06 (A) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018809

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410070	J30	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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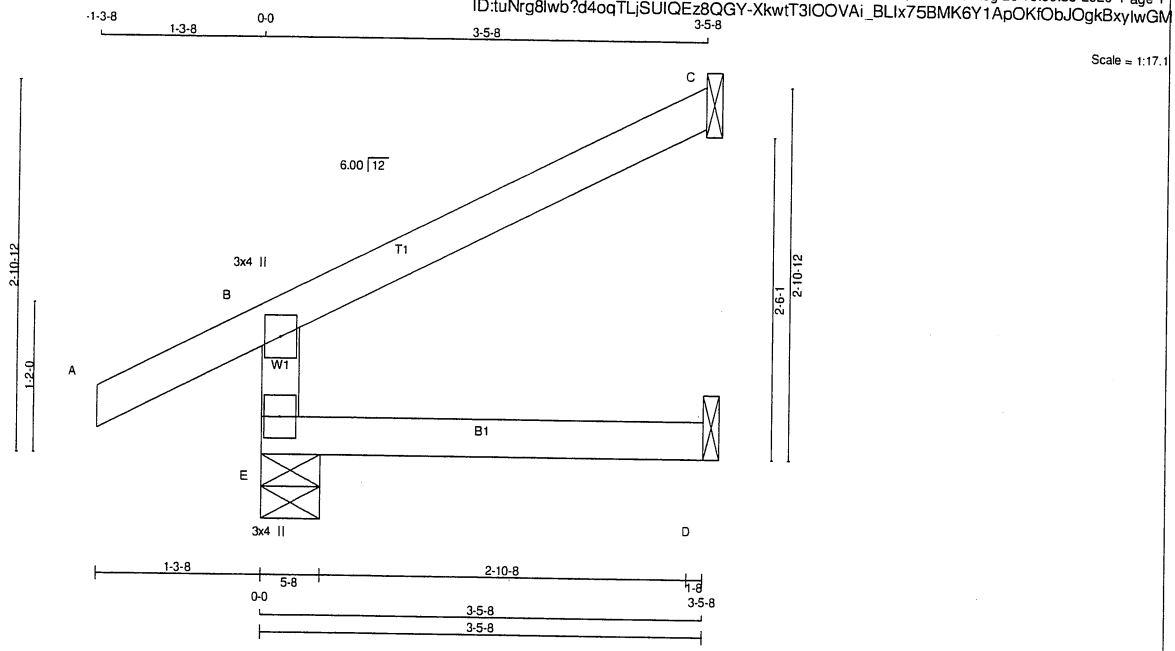

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	359	0	359	0	0	5-8	5-8
C	119	0	119	0	0	1-8	1-8
D	27	0	31	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	251	180 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0
C	82	66 / 0	0 / 0	0 / 0	0 / 0	16 / 0	0 / 0
D	22	0 / 0	0 / 0	0 / 0	0 / 0	22 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO			
E-B	-323 / 0	0.0	0.0	0.04 (4)	7.81			
A-B	0 / 28	-91.8	-91.8	0.13 (5)	10.00			
B-C	-18 / 0	-91.8	-91.8	0.19 (1)	6.25			
E-D	0 / 0	-18.5	-18.5	0.05 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 11 = 33 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 .

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.19/1.00 (B-C:1) , BC=0.05/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

Structural component only
DWG# T-2018822

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410068	C1	5	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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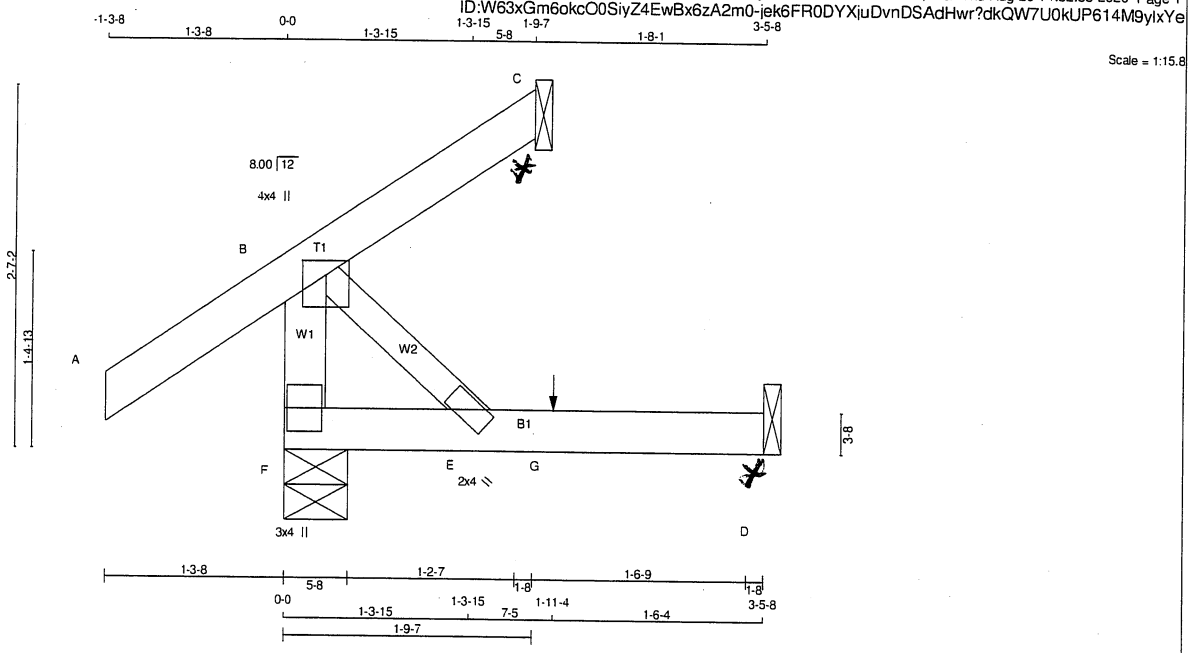


TOTAL WEIGHT = 5 X 11 = 54 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	288	0	288	0	0	5-8	5-8
C	34	0	34	0	-39	1-8	1-8
D	32	0	36	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	202	143 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
C	23	19 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	26	0 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-256 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.14 (5)	10.00			
B- C	-27 / 0	-91.8	-91.8	0.13 (5)	6.25			
F- E	0 / 0	-18.5	-18.5	0.06 (4)	10.00			
E- G	0 / 0	-18.5	-18.5	0.06 (4)	10.00			
G- D	0 / 0	-18.5	-18.5	0.06 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14/1.00 (A-B:5) , BC=0.06/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

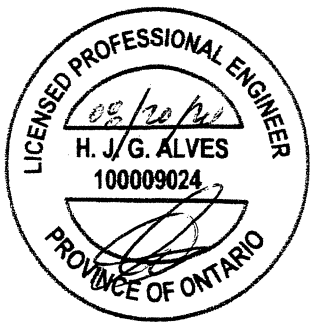
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2018790

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410068	C2	4	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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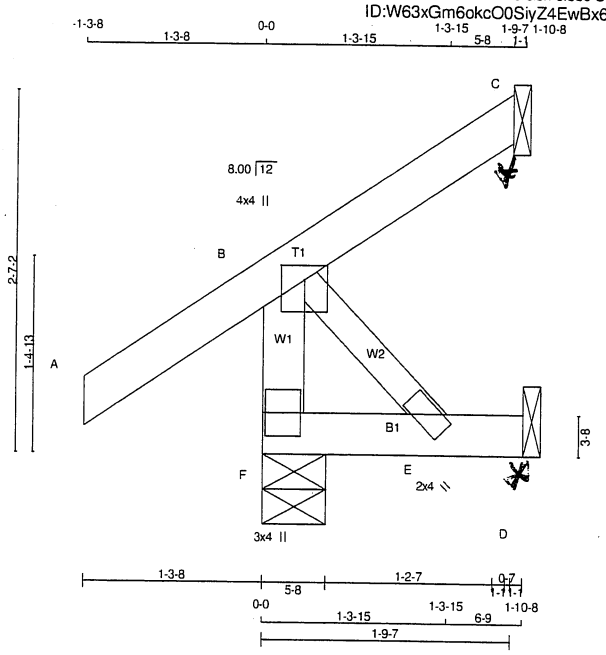

TOTAL WEIGHT = 4 X 9 = 35 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	274	0	274	0	0	5-8	5-8
C	34	0	34	0	-39	1-8	1-8
D	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	190	143 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	23	19 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-256 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00			
B- C	-27 / 0	-91.8	-91.8	0.12 (1)	6.25			
F- E	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018791

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	C30	4	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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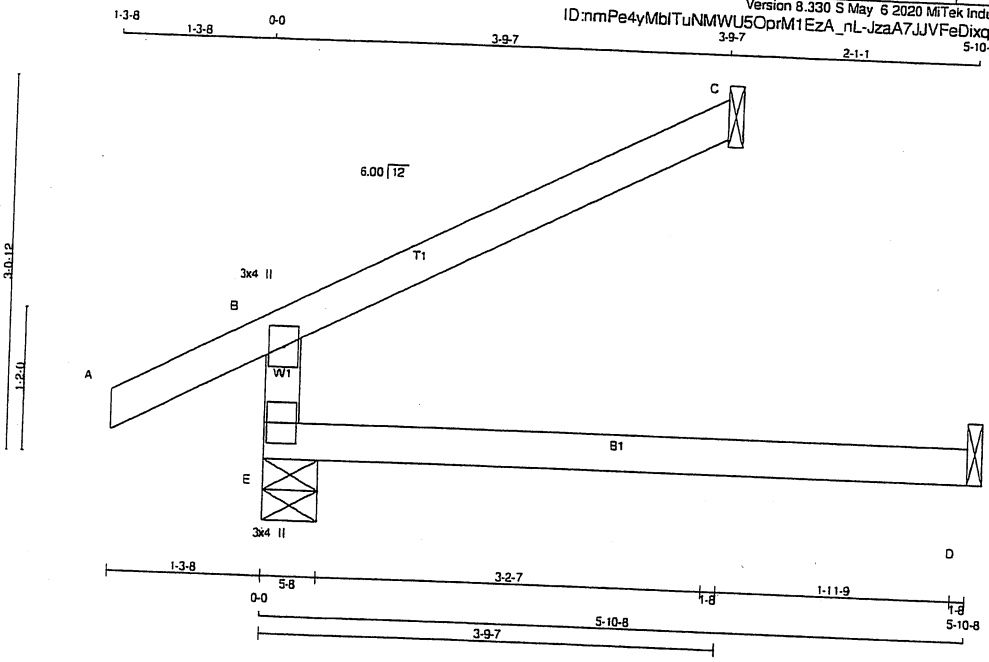

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 4 X 14 = 57 lb [M]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	405	0	405	0	0	5-8	5-8
C	130	0	130	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	286	190 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
E-B	-342 0	0.0	0.0	0.13	(4)	7.81			
A-B	0 28	-91.8	-91.8	0.12	(1)	10.00			
B-C	-19 0	-91.8	-91.8	0.22	(1)	6.25			
E-D	0 0	-18.5	-18.5	0.13	(4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/15/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	C31	4	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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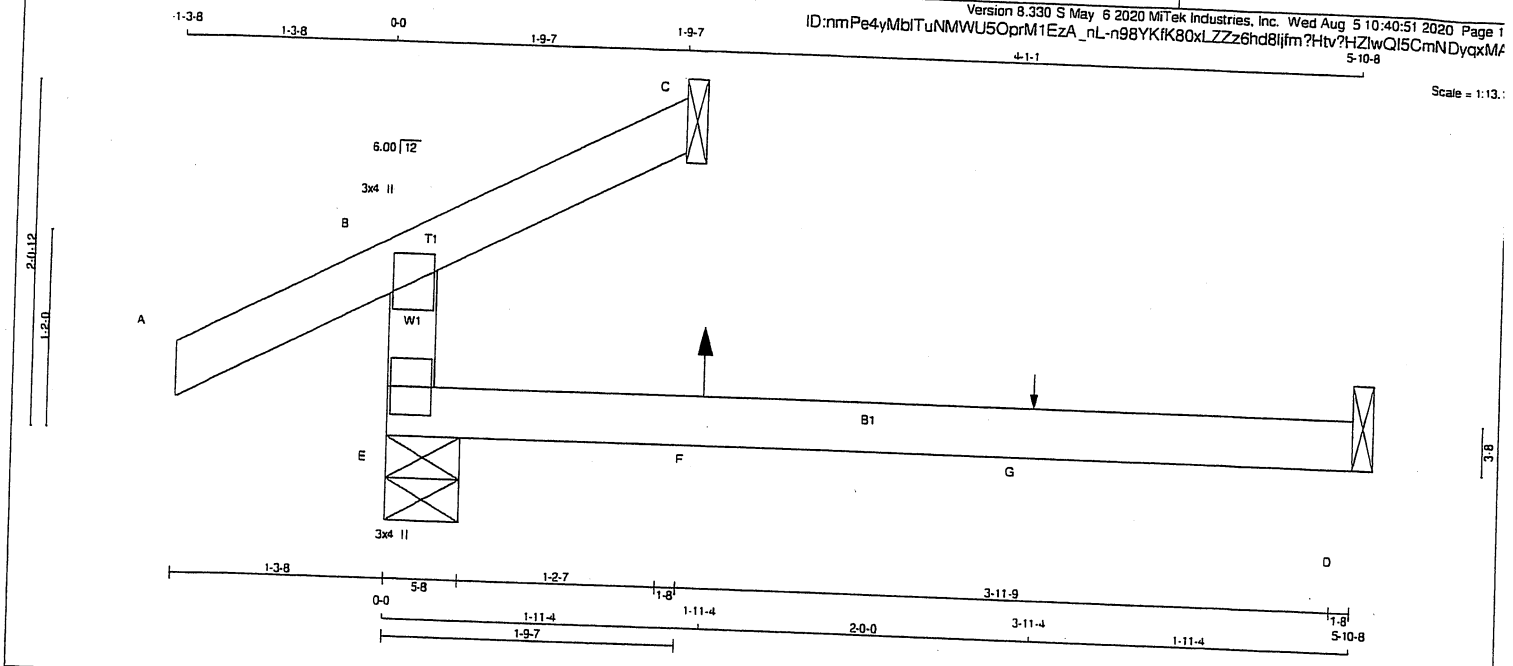

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	284	0	284	0	0	5-8	5-8
C	63	0	63	0	0	1-8	1-8
D	44	0	52	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	200	137 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	46	21 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
D	35	0 / -3	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH FR-TO
E- B	-227	0	0.0	0.0	0.11 (4)	7.81
A- B	0	28	-91.8	-91.8	0.12 (1)	10.00
B- C	-9	9	-91.8	-91.8	0.08 (4)	10.00
E- F	0	0	-18.5	-18.5	0.14 (4)	10.00
F- G	0	0	-18.5	-18.5	0.14 (4)	10.00
G- D	0	0	-18.5	-18.5	0.14 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	FRONT	VERT	TOTAL	--	C1
G	3-11-4	1	1	---	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

TOTAL WEIGHT = 4 X 12 = 46 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.14/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	C32	4	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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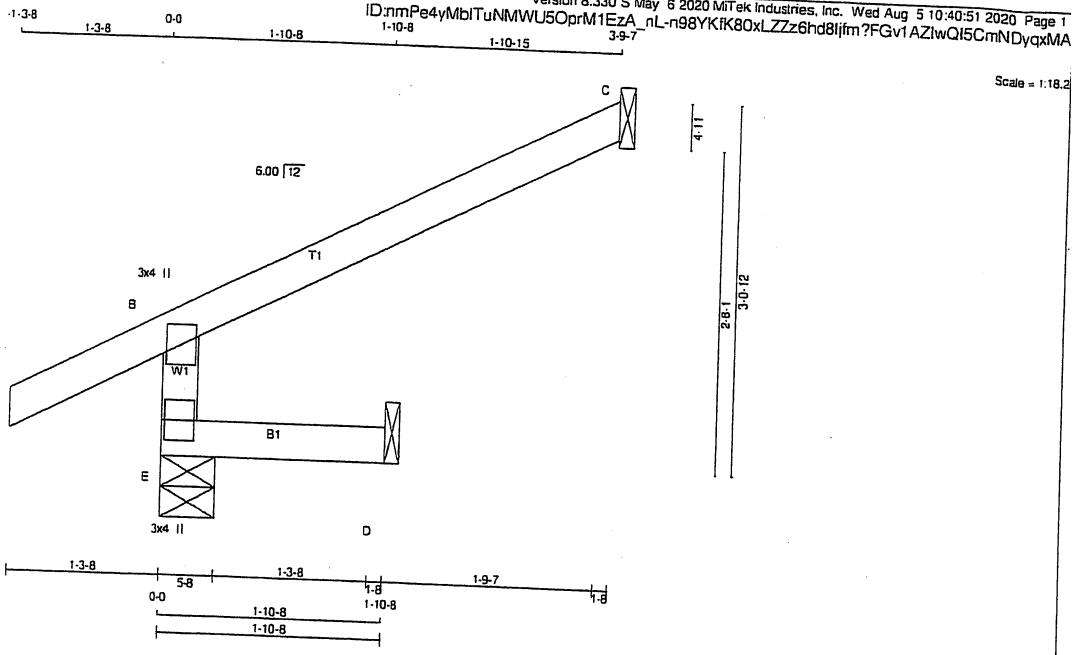

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	361	0	361	0	0	5-8	5-8
C	130	0	130	0	0	1-8	1-8
D	16	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	250	190 / 0	0 / 0	0 / 0	0 0	60 0	0 0
C	90	73 0	0 0	0 / 0	0 0	17 0	0 0
D	12	0 / 0	0 / 0	0 / 0	0 0	12 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
E- B	-342 0	0.0	0.0	0.01 (4)	7.81
A- B	0 28	-91.8	-91.8	0.13 (5)	10.00
B- C	-19 0	-91.8	-91.8	0.22 (1)	6.25
E- D	0 0	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 10 = 38 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS.

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural components

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
409989	C33	6	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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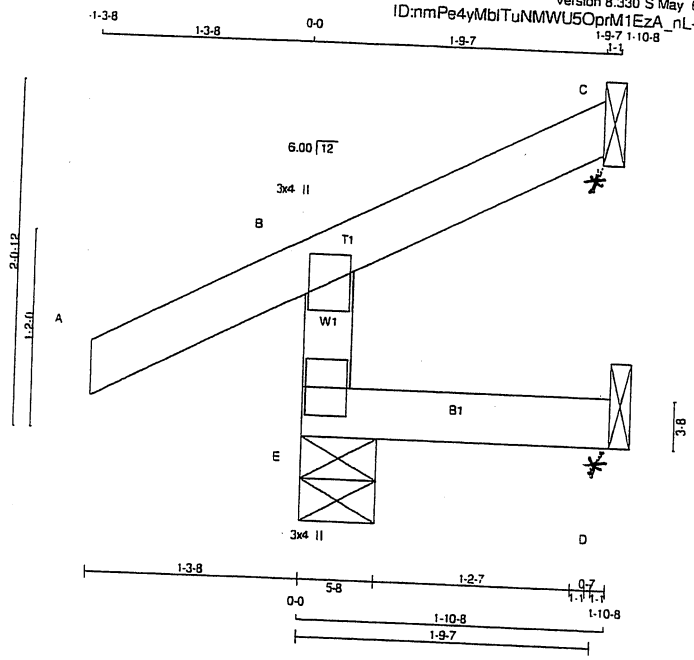

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	271	0	271	0	0	5-8	5-8
C	45	0	45	0	-23	1-8	1-8
D	8	0	17	0	-2	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	188	141 / 0	0 / 0	0 / 0	0 / 0	47 0	0 0
C	31	24 / -18	0 / 0	0 / 0	0 / 0	7 0	0 0
D	7	0 / -8	0 / 0	0 / 0	0 / 0	12 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
E - B	-244 0	0.0	0.0	0.04 (5)	7.81
A - B	0 28	-91.8	-91.8	0.12 (1)	10.00
B - C	-17 0	-91.8	-91.8	0.09 (1)	6.25
E - D	0 0	-18.5	-18.5	0.04 (5)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 6 X 7 = 42 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.04/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/15/20
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410070	C36	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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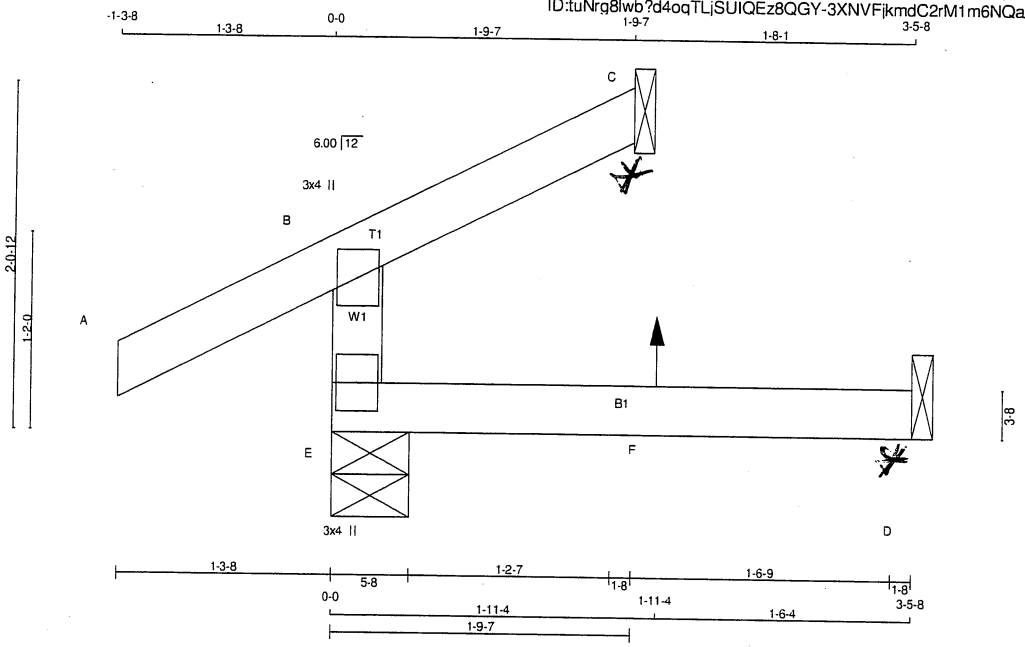


TOTAL WEIGHT = 2 X 9 = 18 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	277	0	277	0	0	5-8	5-8
C	47	0	47	0	-22	1-8	1-8
D	22	0	31	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	194	138 / 0	0 / 0	0 / 0	0 / 0	56 / 0	0 / 0
C	33	22 / -20	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
D	18	0 / -6	0 / 0	0 / 0	0 / 0	22 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (9)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-242 / 0	0.0	0.0	0.04 (5)	7.81			
A-B	0 / 28	-91.8	-91.8	0.13 (5)	10.00			
B-C	-16 / 0	-91.8	-91.8	0.09 (6)	6.25			
E-F	0 / 0	-18.5	-18.5	0.05 (4)	10.00			
F-D	0 / 0	-18.5	-18.5	0.05 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.05/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.10/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2018821

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

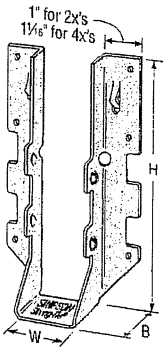
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

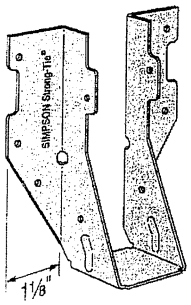
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

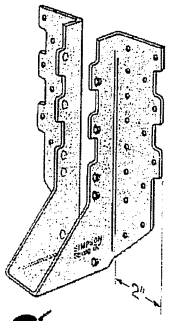
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



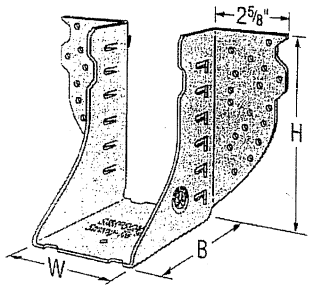
LUS28



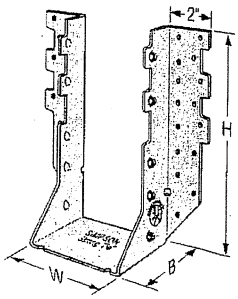
LU26L



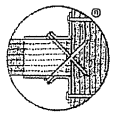
HUS210
(HUS26, HUS28, and HHUS similar)



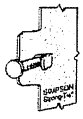
HGUS28-2



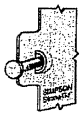
HHUS210-2



Double-Shear Nailing Top View



Double-Shear Nailing Side View; Do not bend tab

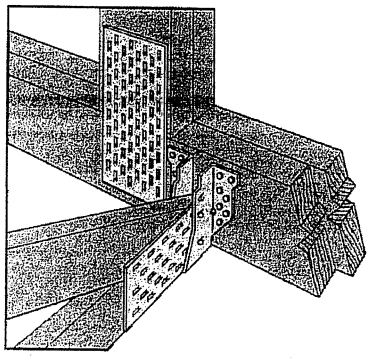


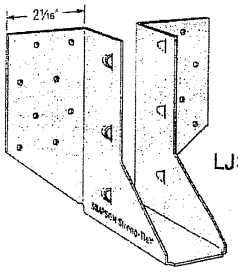
Dome Double-Shear Nailing Side View *(available on some models)*

U.S. Patent 5,603,580

Typical HUS26 Installation with Reduced Heel Height

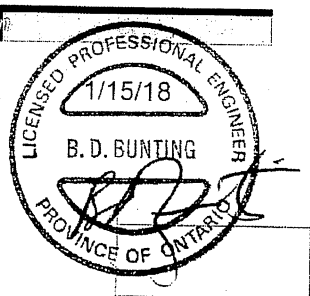
(Truss Designer to provide fastener quantity for connecting multiple members together)





LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS



HHUS/HGUS

See Hanger Options information on pp. 125–127.

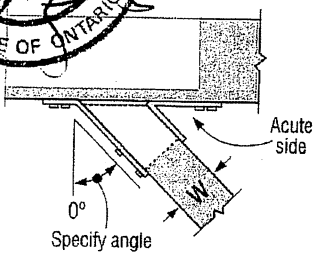
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Single 2x Sizes											
LUS24	18	1 9/16	3 1/8	1 3/4	2 1/4	(4) 10d	(2) 10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
LU24L	22	1 9/16	3	1 5/8	2 11/16	(4) 10d	(2) 10d x 1 1/2"	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
LU26L	22	1 9/16	5	1 5/8	4 5/8	(6) 10d	(4) 10d x 1 1/2"	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
SS LUS26	18	1 9/16	4 3/4	1 3/4	3 3/4	(4) 10d	(4) 10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
HUS26	16	1 5/8	5 3/8	3	3 15/16	(14) 16d	(6) 16d	2705 / 11.30	4940 / 21.97	2065 / 9.20	3875 / 17.24
LJS26DS	18	1 9/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055 / 9.14	4265 / 18.97	1460 / 6.49	4115 / 18.31
HGUS26	12	1 5/8	5 3/8	5	4 1/8	(20) 16d	(8) 16d	2685 / 11.96	6625 / 29.51	2685 / 11.96	5700 / 25.35
LU28L	20	1 9/16	6 3/4	1 5/8	5 7/8	(8) 10d	(6) 10d x 1 1/2"	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
SS LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(4) 10d	1420 / 6.32	2520 / 11.21	1290 / 5.74	1790 / 7.96
HUS28	16	1 5/8	7 1/16	3	6 1/16	(22) 16d	(8) 16d	3605 / 16.04	5365 / 23.86	2675 / 11.90	4345 / 19.33
HGUS28	12	1 5/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310 / 14.74	7675 / 34.19	3310 / 14.74	6900 / 30.73
LU210L	20	1 9/16	8	1 5/8	7 5/8	(10) 10d	(6) 10d x 1 1/2"	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
SS LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420 / 6.32	2785 / 12.39	1290 / 5.74	2210 / 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. **Nails:** 16d = 0.162" dia. x 3½" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

SIMPSON StrongTie®

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Double 2x Sizes											
LUS24-2	18	3⅛	3⅛	2	1½	(4) 16d	(2) 16d	835 / 3.71	2020 / 8.99	590 / 2.62	1435 / 6.38
SS LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
HHUS26-2	14	3⁵⁄₁₆	5⅜	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2850 / 12.68	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS26-2	12	3⁵⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
SS LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
HHUS28-2	14	3⁵⁄₁₆	7⁵⁄₁₆	3	6⅛	(22) 16d	(8) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS28-2	12	3⁵⁄₁₆	7⅜	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
SS LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
HHUS210-2	14	3⁵⁄₁₆	9³⁄₁₆	3	8	(30) 16d	(10) 16d	4670 / 20.77	9660 / 42.97	4235 / 18.84	7000 / 31.14
HGUS210-2	12	3⁵⁄₁₆	9³⁄₁₆	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
Triple 2x Sizes											
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅜	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
HHUS210-3	14	4¹³⁄₁₆	9	3	7⁵⁄₁₆	(30) 16d	(10) 16d	4670 / 20.77	9670 / 43.02	4235 / 18.84	6865 / 30.54
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
Quadruple 2x Sizes											
HGUS26-4	12	6⁹⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-4	12	6⁹⁄₁₆	7⁵⁄₁₆	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
HHUS210-4	14	6⅛	8⅞	3	7¹³⁄₁₆	(30) 16d	(10) 16d	4670 / 20.77	10155 / 45.17	4235 / 18.84	7210 / 32.07
HGUS210-4	12	6⁹⁄₁₆	9³⁄₁₆	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
HGUS212-4	12	6⁹⁄₁₆	10⅝	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS214-4	12	6⁹⁄₁₆	12⅝	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80
4x Sizes											
LUS46	18	3⁹⁄₁₆	4¾	2	3⅞	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
HHUS46	14	3⅝	5¼	3	3¹³⁄₁₆	(14) 16d	(6) 16d	2540 / 11.30	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
LUS48	18	3⁹⁄₁₆	6¾	2	3⅞	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS48	12	3⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
LUS410	18	3⁹⁄₁₆	8¾	2	5⅞	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
HGUS410	12	3⅝	9	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
HGUS412	12	3⅝	10⁷⁄₁₆	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS414	12	3⅝	12⁷⁄₁₆	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80

See footnotes on p. 258.

TECHNICAL BULLETIN

TC – Truss Connectors

SIMPSON Strong-Tie®

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

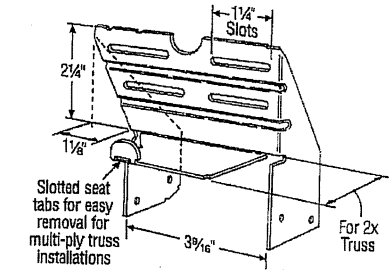
Design: Factored resistances are in accordance with CSA 086-14

Installation:

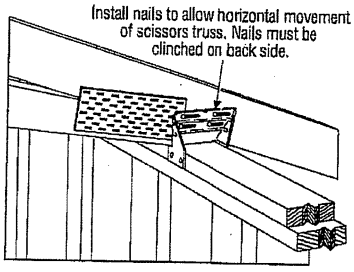
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½ = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss–allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

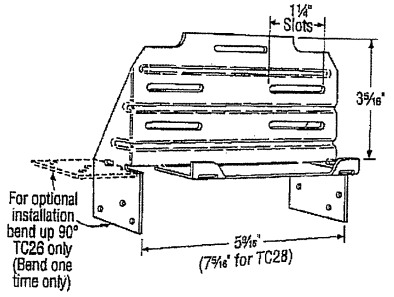
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



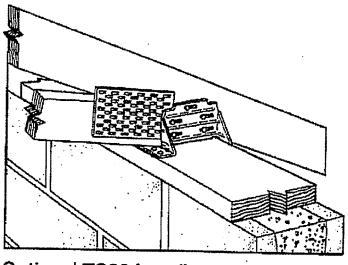
TC24
U.S. Patent 4,932,173



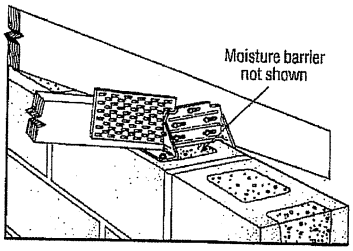
Typical TC24 Installation



TC26
(TC28 Similiar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC26	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) – ⅜" x 2¼" Titen screws has a factored uplift resistance of 275 lb.

LIMIT STATES DESIGN

(800) 999-5099
strongtie.com

H/TSP

SIMPSON Strong-Tie®

Seismic and Hurricane Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Fasteners			Factored Resistance (K_D = 1.15)					
					D.Fir-L			S-P-F		
					Uplift	Lateral		Uplift	Lateral	
		To Rafters/ Truss	To Plates	To Studs		F_1	F_2		F_1	F_2
					lb.	lb.	lb.	lb.	lb.	lb.
					kN	kN	kN	kN	kN	kN
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
					3.29	3.05	1.33	3.02	2.16	0.96
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
					3.69	0.98	0.33	2.62	0.69	0.24
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
					3.58	0.71	0.71	3.36	0.71	0.71
H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
					3.71	0.78	0.93	3.29	0.71	0.93
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
					3.29	0.80	1.18	2.74	0.56	0.85
H6	16	—	(8) 8d	(8) 8d	1585	1085	—	1125	770	—
					7.05	4.83	—	5.00	3.43	—
H7Z	16	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
					6.18	2.98	—	4.40	2.11	—
H8[3]	18	(5) 10d x 1½"	(5) 10d x 1½"	—	1120	—	—	1025	—	—
					4.98	—	—	4.56	—	—
H10A[9]	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290
					7.72	3.54	1.82	6.69	2.51	1.29
H10AR	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1485	690	430	1220	570	305
					6.61	3.07	1.91	5.43	2.54	1.36
H10A-2	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1835	1275	430	1645	880	305
					8.16	5.67	1.91	7.32	3.91	1.36
H10S[7,9]	18	(8) 8d x 1½"	(8) 8d x 1½"	(8) 8d	1465	795	315	1040	565	225
					6.52	3.54	1.40	4.63	2.51	1.00
H11Z	18	(6) 16d x 2½"	(6) 16d x 2½"	—	1095	920	545	780	655	390
					4.87	4.09	2.42	3.47	2.91	1.73
H14	18	[1] (12) 8d x 1½"	(13) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
		[2] (12) 8d x 1½"	(15) 8d	—	2390	855	320	1805	610	230
					10.63	3.80	1.42	8.03	2.71	1.02
TSP	16	(9) 10d x 1½"	(6) 10d x 1½"	—	1295	440	—	920	310	—
					5.76	1.96	—	4.09	1.38	—
		(9) 10d x 1½"	(6) 10d	—	1560	440	—	1105	310	—
					6.94	1.96	—	4.92	1.38	—

1. Factored resistances have been increased 15% for short term loading; no further increase is allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
3. H8 factored uplift resistances for stud-to-bottom plate installations are 595 lb. (2.65 kN) for D.Fir-L and 390 lb. (1.74 kN) for S-P-F.
4. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
5. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-HTIECONPATH).
6. Factored resistances in the F_1 direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.
7. H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.38 kN) D.Fir-L and 1015 lb. (4.51 kN) S-P-F.
8. H10S nails to plates are optional for uplift but required for lateral loads.
9. H10A may be field-bent up to a slope of 6/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
10. The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SSNAILS at **strongtie.com**.
11. D.Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1½" nails into the top plates and (3) 8d x 1½" nails into the lowest three flange holes into the truss bottom chord is 495 lb. (2.20 kN).
12. **Nails:** 16d x 2½" = 0.162" dia. x 2½" long, 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long, 8d x 1½" = 0.131" dia. x 1½" long. See pp. 27–28 for other nail sizes and information.

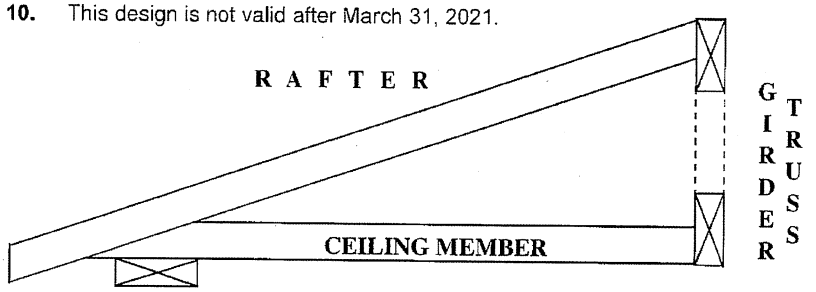
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

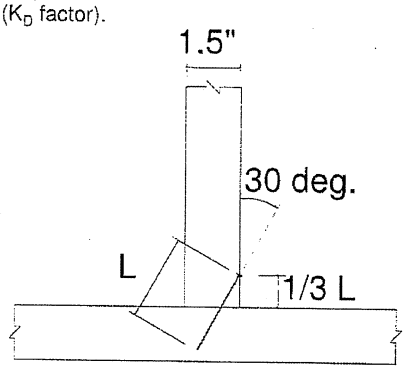
B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.





TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		**(3" and 3.25" nail)**	
LUMBER SIZE	**MAXIMUM NUMBER OF TOE-NAILS**			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek®
MiTek Canada Inc
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Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485

LICENSED PROFESSIONAL ENGINEER
C. Cordogiannis
90553736
PROVINCE OF ONTARIO

December 2, 2019

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

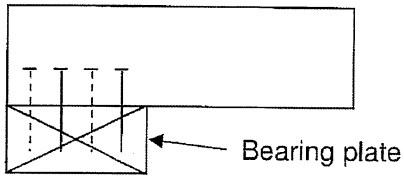
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

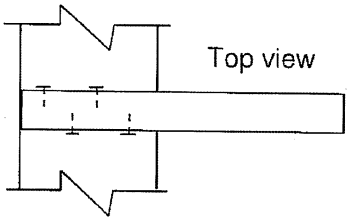
NOTES:

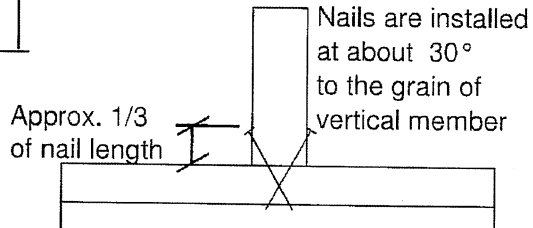
1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



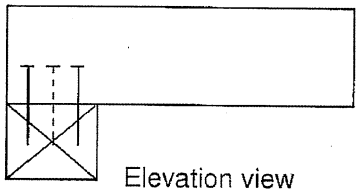
Elevation view



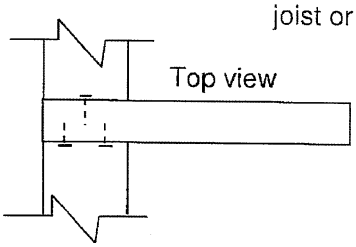


Toe-nailing viewed from end of joist or truss

Toe-nailing on 2x4 Bearing Plate



Elevation view



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MiTek Canada Inc
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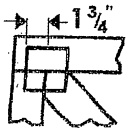
Bradford, Ontario L3Z 3G7

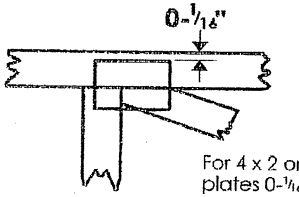
December 2, 2019

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

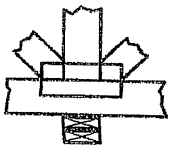
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

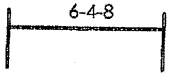


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

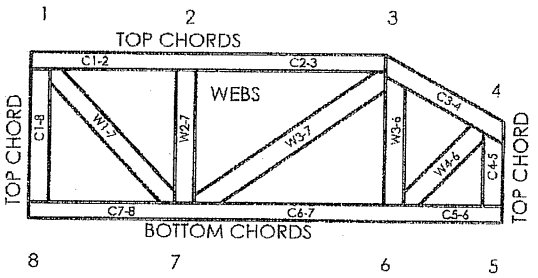
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

ONTARIO WOOD TRUSS
FABRICATORS ASSOCIATION

TECH-NOTES

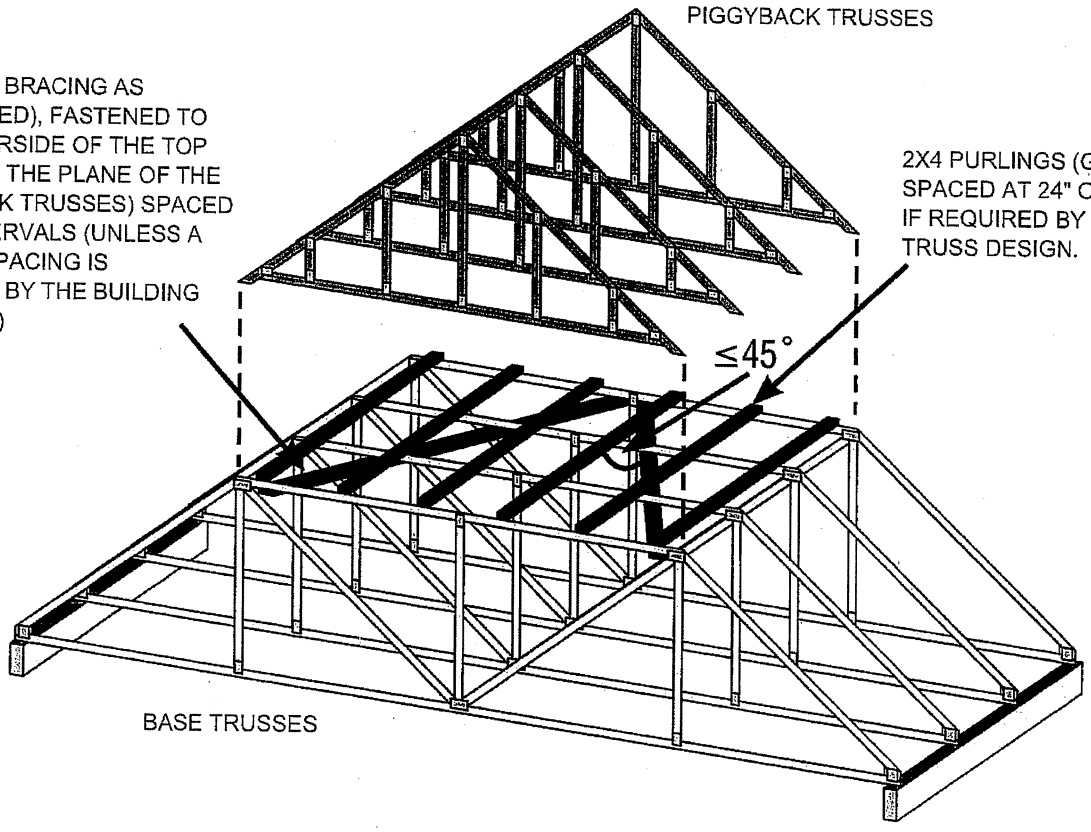
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

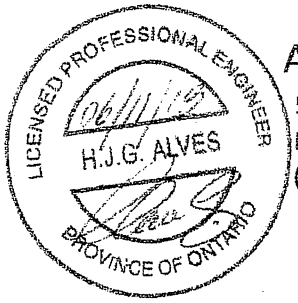
Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800215 Feb 09, 2018