

20.130104.000.00.CM

Issue Date: 02/10/21

LAMPONE INVESTMENT INC

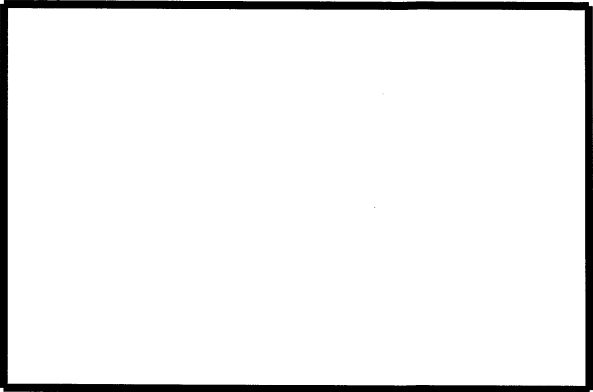
ALL CONSTRUCTION SHALL COMPLY WITH THE
ONTARIO BUILDING CODE.

CITY OF MARKHAM

ROOF TRUSS SHOP DRAWINGS

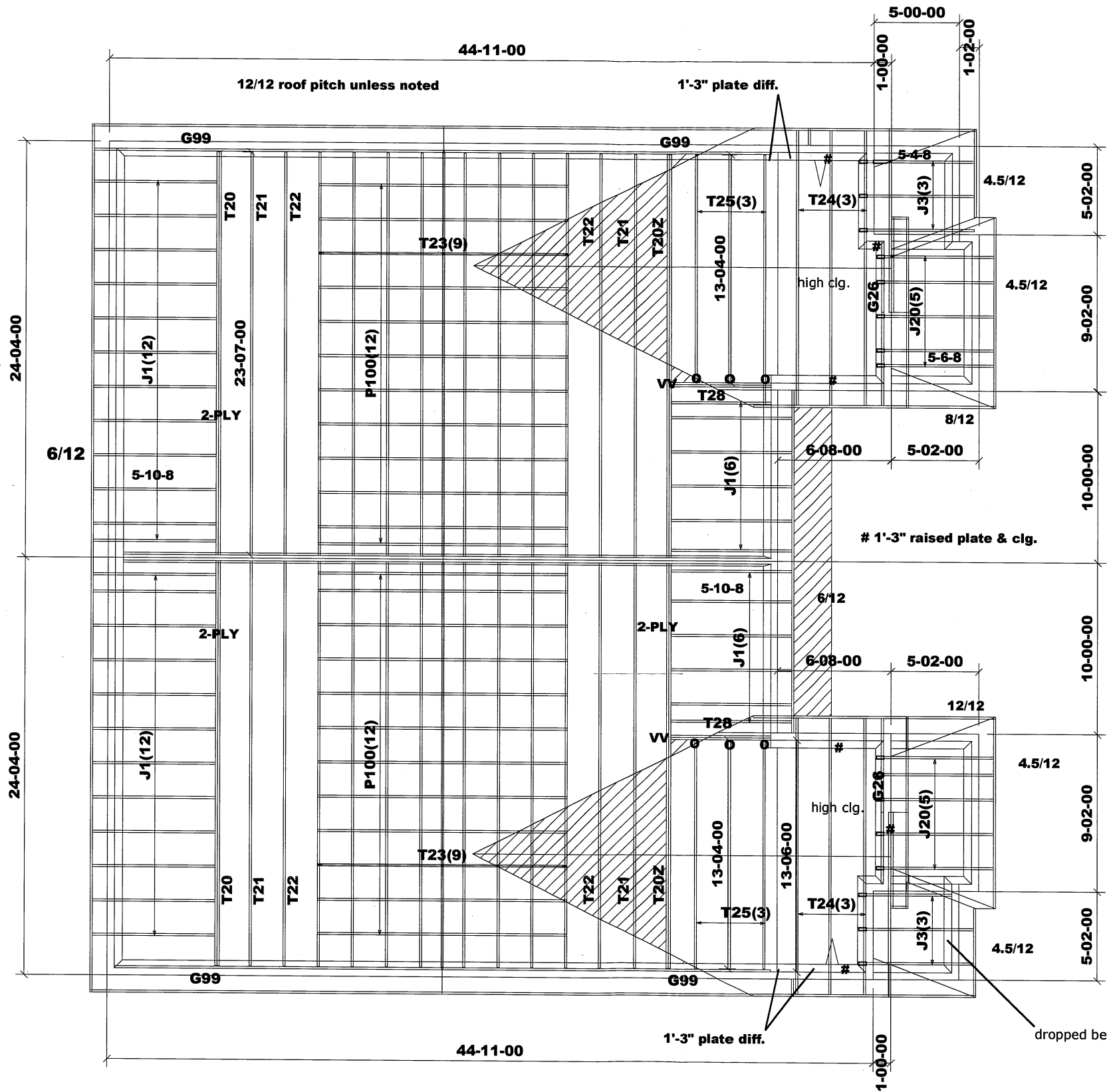
MODEL NAME : PEYTON 2S

ELEV 1, 2A, 2B & 3



EL1-RIGHT : 411574

EL1-LEFT: 411574



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

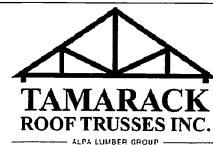
All conventional framing to conform with Part 9
of O.B.C. 2012 (2019 amendment). Roof
rafters that cross over or meet trusses to be
min. 2x4 SPF #2 @ 24" o/c with a vertical
post to the truss at each cross point. Vertical
posts longer than 6' to have lateral bracing so
that the distance between the post end points
and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
LUS26-2 - (VV)

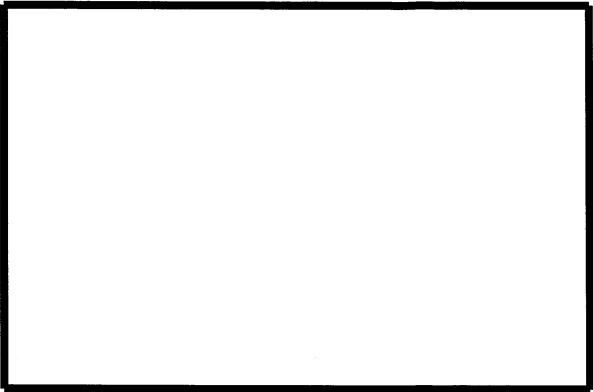
 DENOTES:
CONVENTIONAL
FRAMING



Job Track: **51453**
Layout ID: **411573**
Plan Log: **203184**

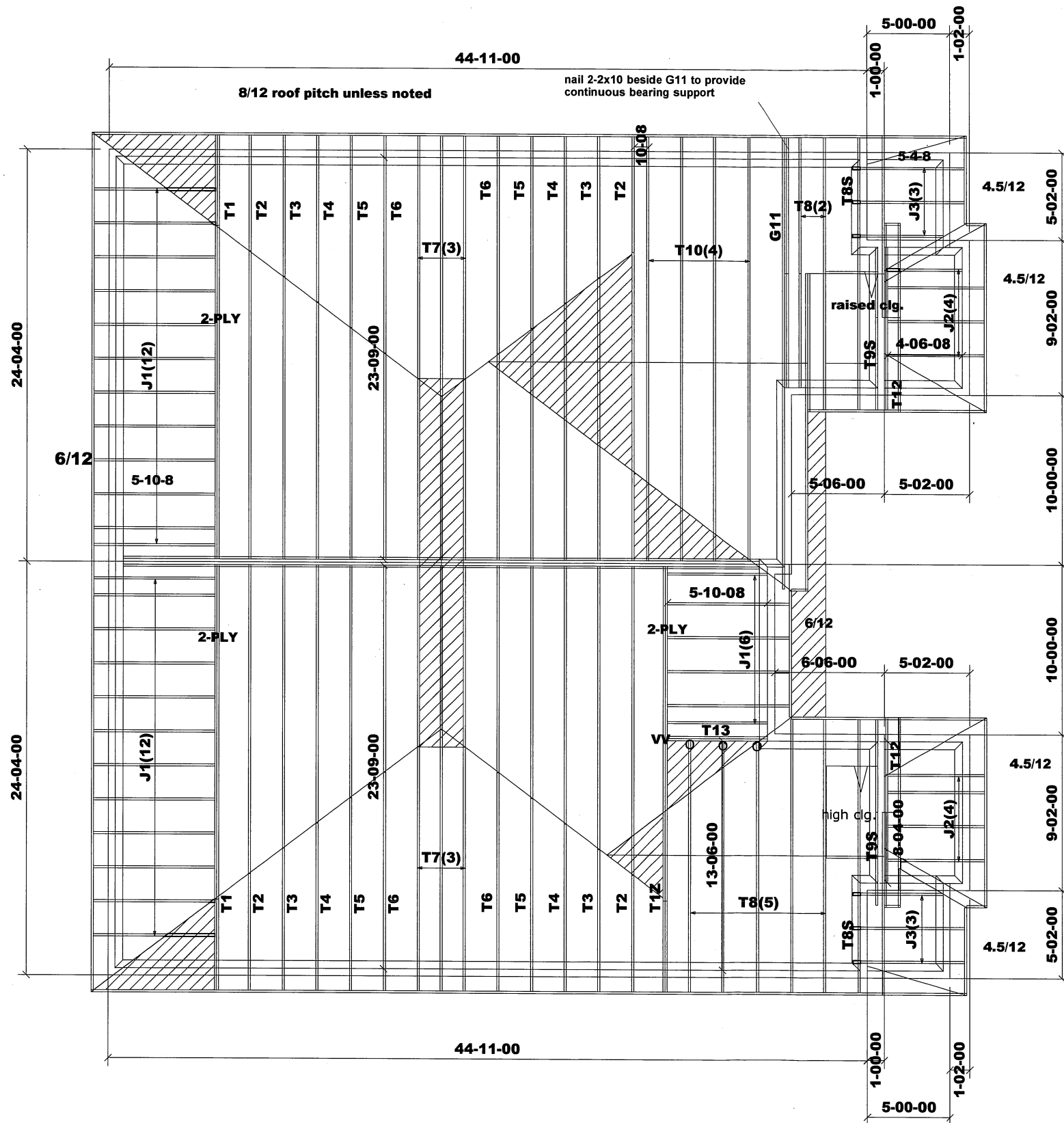
Builder / Location: **GREEN PARK HOMES / MARKHAM**
Project: **LAMPONE INVESTMENTS INC**
Date: 2020-08-19 Designer: JG

Model / Elevation: **PEYTON 2S / 1 - LEFT PAIR WITH RIGHT**
Mitek ver 8.3.3.247
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE
REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER
THAN THE MANUFACTURE OF TRUSSES BY
TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED
FOR ANY OTHER PURPOSE.



2B: 411577

2A : 411576



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

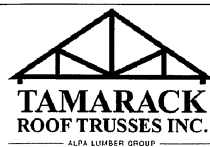
All conventional framing to conform with Part 9
of O.B.C. 2012 (2019 amendment). Roof
rafters that cross over or meet trusses to be
min. 2x4 SPF #2 @ 24" o/c with a vertical
post to the truss at each cross point. Vertical
posts longer than 6' to have lateral bracing so
that the distance between the post end points
and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
LUS26-2 - (VV)

 DENOTES:
CONVENTIONAL
FRAMING



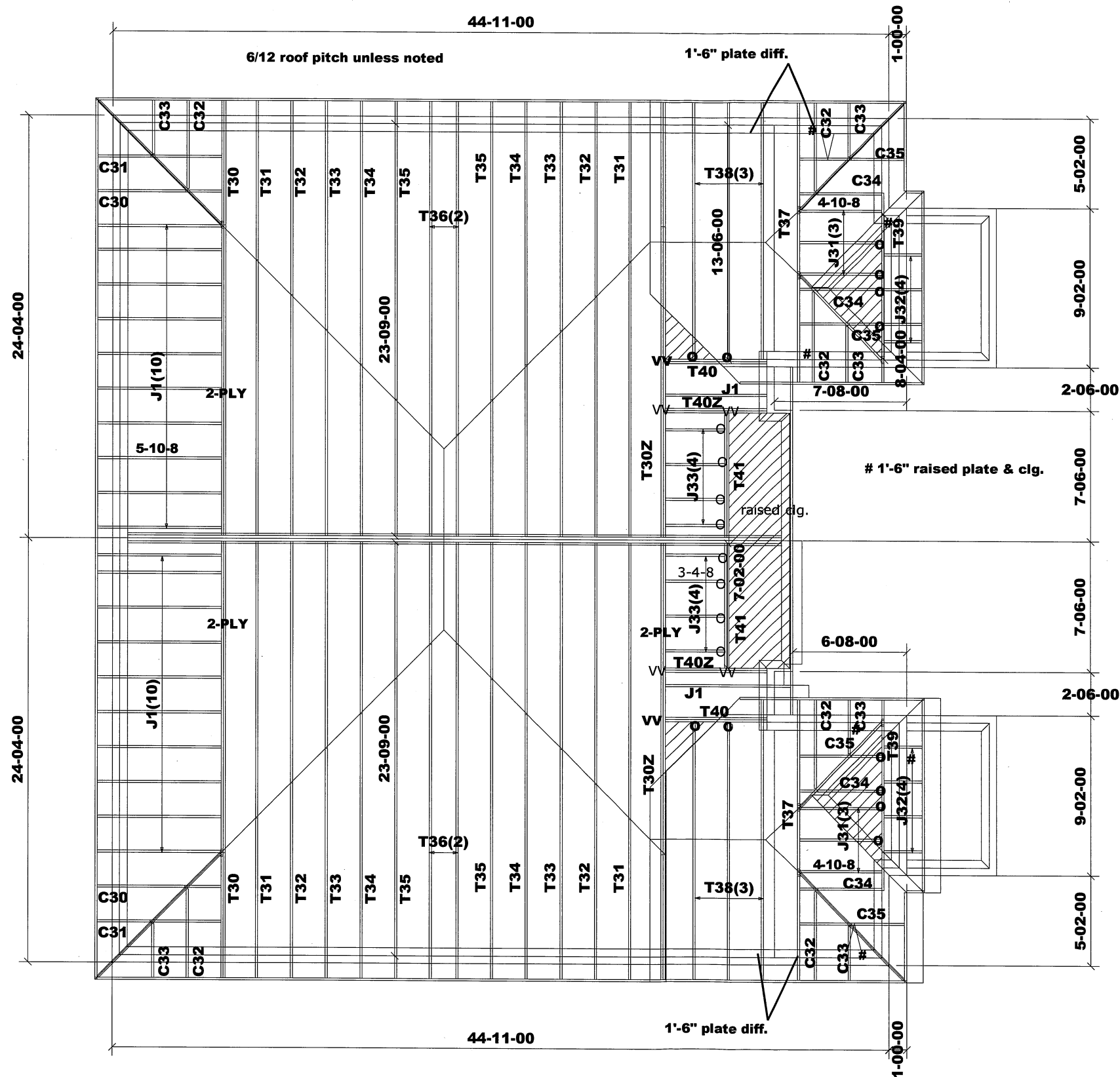
Job Track: **51453**
Layout ID: **411575**
Plan Log: **203184**

Builder / Location: **GREEN PARK HOMES / MARKHAM**
Project: **LAMPONE INVESTMENTS INC**
Date: 2020-08-19 Designer: **JG**

Model / Elevation: **PEYTON 2S / 2A AND 2B** Mitek ver 8.3.3.247
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE
REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER
THAN THE MANUFACTURE OF TRUSSES BY
TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED
FOR ANY OTHER PURPOSE.

EL.3-RIGHT : 411579

EL3-LEFT : 411579



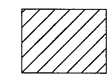
ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

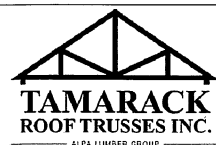
All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
LUS26-2 - (VV)

 DENOTES:
CONVENTIONAL
FRAMING

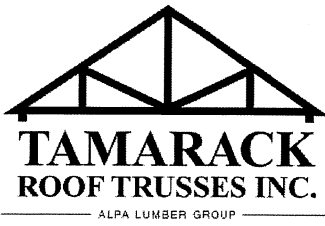


Job Track: **51453**
Layout ID: **411578**
Plan Log: **203184**

Builder / Location: **GREEN PARK HOMES / MARKHAM**
Project: **LAMPONE INVESTMENTS INC**
Date: 2020-08-19 Designer: **JG**

Model / Elevation: **PEYTON 2S / 3 - LEFT PAIR WITH RIGHT**
Mitek ver 8.3.3.247
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

DELIVERY SHIPLIST



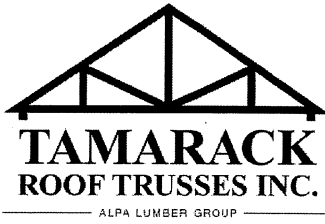
Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMPONE INVESTMENTS INC
 Location: MARKHAM
 Model: PEYTON 2S
 Lot #:
 Elevation: 1- LEFT OR RIGHT

Job Track: 51453
 PlanLog: 203184
 Layout ID: 411574
 Ref #
 Page: 1 of 2
 Date: 08-05-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T20 Flat Girder	0 /12	23-07-00	4-01-04	2 x 6		4-01-04 4-01-04	248.94 152.00		
	1 2-ply	T20Z Flat Girder	0 /12	23-07-00	4-01-04	2 x 6		4-01-04 4-01-04	248.94 152.00		
	2	T21 Flat	0 /12	23-07-00	5-01-04	2 x 4		5-01-04 5-01-04	197.15 124.33		
	2	T22 Flat	0 /12	23-07-00	6-01-04	2 x 4		6-01-04 6-01-04	210.94 130.00		
	9	T23 Flat	0 /12	23-07-00	7-01-04	2 x 4		7-01-04 7-01-04	1013.22 628.50		
	3	T24 Common	12 /12	13-06-00	8-07-08	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	209.98 132.00		
	3	T25 Common	12 /12	13-04-00	9-10-08	2 x 4		3-03-08 3-01-08	218.6 137.00		
	1 2-ply	T28 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1	G26 GABLE	12 /12	8-04-00	6-00-08	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	44.8 30.67		
	2	G99 GABLE	6 /12	19-02-00	10-09-00	2 x 4	1-03-08	1-02-00 10-09-00	196.91 128.67		
	12	P100 Common	6 /12	14-07-12	3-08-00	2 x 4		0-01 0-01	503.04 320.00		
	18	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	302.3 192.00		
	3	J3 Jack-Open	4.5 /12	5-04-08	2-09-12	2 x 4	1-03-08	4-00 2-04-03	43.12 28.00		
	5	J20 Jack-Open	4.5 /12	5-06-08	2-10-08	2 x 4	1-03-08	4-00 2-04-15	73.83 46.67		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMPONE INVESTMENTS INC
 Location: MARKHAM
 Model: PEYTON 2S
 Lot #:
 Elevation: 1- LEFT OR RIGHT

Job Track: 51453
 PlanLog: 203184
 Layout ID: 411574
 Ref #
 Page: 2 of 2
 Date: 08-05-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
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TOTAL # TRUSS= 66

TOTAL BFT OF ALL TRUSSES= 2239.51

BFT.

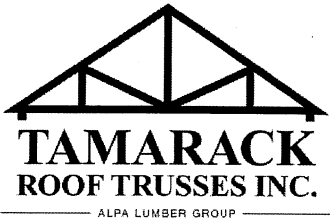
TOTAL WEIGHT OF ALL TRSSES 3570.16 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 4

DELIVERY SHIPLIST



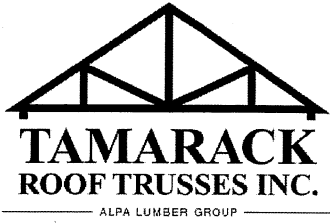
Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMPONE INVESTMENTS INC
 Location: MARKHAM
 Model: PEYTON 2S
 Lot #:
 Elevation: 2A

Job Track: 51453
 PlanLog: 203184
 Layout ID: 411576
 Ref #
 Page: 1 of 2
 Date: 08-05-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Half Hip Girder	8 /12	23-09-00	4-01-04	2 x 4 2 x 6	1-03-08	1-04-13 4-01-04	221.21 135.33		
	1 2-ply	T1Z Half Hip Girder	8 /12	23-09-00	4-01-04	2 x 4 2 x 6	1-03-08	1-04-13 4-01-04	224.79 136.33		
	2	T2 Half Hip	8 /12	23-09-00	5-01-04	2 x 4	1-03-08	1-04-13 5-01-04	193.94 122.33		
	2	T3 Half Hip	8 /12	23-09-00	6-01-04	2 x 4	1-03-08	1-04-13 6-01-04	209.59 130.67		
	2	T4 Half Hip	8 /12	23-09-00	7-01-04	2 x 4	1-03-08	1-04-13 7-01-04	226.41 144.33		
	2	T5 Half Hip	8 /12	23-09-00	8-01-04	2 x 4	1-03-08	1-04-13 8-01-04	218.4 134.33		
	2	T6 Half Hip	8 /12	23-09-00	9-01-04	2 x 4	1-03-08	1-04-13 9-01-04	229.21 143.00		
	3	T7 Half Hip	8 /12	23-09-00	10-01-04	2 x 4	1-03-08	1-04-13 10-01-04	360.69 224.00		
	5	T8 Common	8 /12	13-06-00	5-10-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	293.15 185.00		
	1	T8S Roof Special	8 /12	13-06-00	5-10-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	64.79 42.00		
	1	T9S Roof Special	8 /12	8-04-00	4-02-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	42.27 28.17		
	1	T12 Hip Girder	8 /12	8-04-00	2-06-00	2 x 4 2 x 6	1-03-08 1-03-08	8-15 8-15	35.86 25.83		
	1 2-ply	T13 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	18	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	302.3 192.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMPONE INVESTMENTS INC
 Location: MARKHAM
 Model: PEYTON 2S
 Lot #:
 Elevation: 2A

Job Track: 51453
 PlanLog: 203184
 Layout ID: 411576
 Ref #
 Page: 2 of 2
 Date: 08-05-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	4	J2 Jack-Open	4.5 /12	4-06-08	2-06-00	2 x 4	1-03-08	4-00 2-00-07	49.63 32.00		
	3	J3 Jack-Open	4.5 /12	5-04-08	2-09-12	2 x 4	1-03-08	4-00 2-04-03	43.12 28.00		

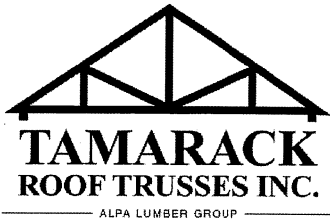
TOTAL # TRUSS= 52 TOTAL BFT OF ALL TRUSSES= 1740.99 BFT. TOTAL WEIGHT OF ALL TRSSES 2773.74 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 4

DELIVERY SHIPLIST



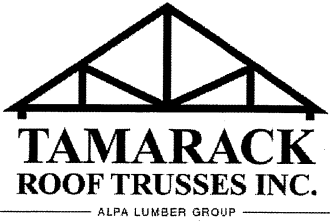
Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMPONE INVESTMENTS INC
 Location: MARKHAM
 Model: PEYTON 2S
 Lot #:
 Elevation: 2B

Job Track: 51453
 PlanLog: 203184
 Layout ID: 411577
 Ref #
 Page: 1 of 2
 Date: 08-05-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Half Hip Girder	8 /12	23-09-00	4-01-04	2 x 4 2 x 6	1-03-08	1-04-13 4-01-04	221.21 135.33		
	2	T2 Half Hip	8 /12	23-09-00	5-01-04	2 x 4	1-03-08	1-04-13 5-01-04	193.94 122.33		
	2	T3 Half Hip	8 /12	23-09-00	6-01-04	2 x 4	1-03-08	1-04-13 6-01-04	209.59 130.67		
	2	T4 Half Hip	8 /12	23-09-00	7-01-04	2 x 4	1-03-08	1-04-13 7-01-04	226.41 144.33		
	2	T5 Half Hip	8 /12	23-09-00	8-01-04	2 x 4	1-03-08	1-04-13 8-01-04	218.4 134.33		
	2	T6 Half Hip	8 /12	23-09-00	9-01-04	2 x 4	1-03-08	1-04-13 9-01-04	229.21 143.00		
	3	T7 Half Hip	8 /12	23-09-00	10-01-04	2 x 4	1-03-08	1-04-13 10-01-04	360.69 224.00		
	2	T8 Common	8 /12	13-06-00	5-10-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	117.26 74.00		
	1	T8S Roof Special	8 /12	13-06-00	5-10-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	64.79 42.00		
	1	T9S Roof Special	8 /12	8-04-00	4-02-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	42.27 28.17		
	4	T10 Common	8 /12	23-09-00	9-05-02	2 x 4	1-03-08	1-04-13 1-07-08	403.69 251.33		
	1	G11 GABLE	8 /12	24-01-00	9-05-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	116.43 74.17		
	1	T12 Hip Girder	8 /12	8-04-00	2-06-00	2 x 4 2 x 6	1-03-08 1-03-08	8-15 8-15	35.86 25.83		
	12	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	201.53 128.00		

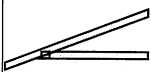
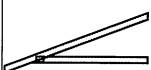
DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMPONE INVESTMENTS INC
 Location: MARKHAM
 Model: PEYTON 2S
 Lot #:
 Elevation: 2B

Job Track: 51453
 PlanLog: 203184
 Layout ID: 411577
 Ref #
 Page: 2 of 2
 Date: 08-05-2020
 Designer:
 Sales Rep: Mario DiCano

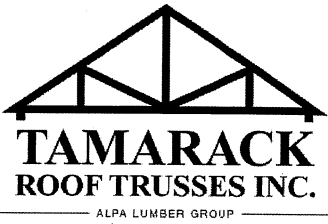
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	4	J2 Jack-Open	4.5 /12	4-06-08	2-06-00	2 x 4	1-03-08	4-00 2-00-07	49.63 32.00		
	3	J3 Jack-Open	4.5 /12	5-04-08	2-09-12	2 x 4	1-03-08	4-00 2-04-03	43.12 28.00		

TOTAL # TRUSS= 44

TOTAL BFT OF ALL TRUSSES= 1717.49 BFT. TOTAL WEIGHT OF ALL TRSSES 2734.03 LBS

DELIVERY SHIPLIST



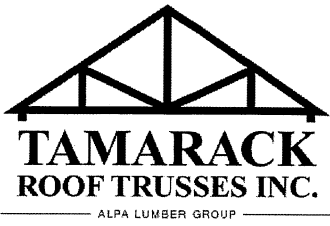
Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMPONE INVESTMENTS INC
 Location: MARKHAM
 Model: PAYTON 2S
 Lot #:
 Elevation: 3- LEFT OR RIGHT

Job Track: 51453
 PlanLog: 203184
 Layout ID: 411579
 Ref #
 Page: 1 of 2
 Date: 08-05-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T30 Half Hip Girder	6 /12	23-09-00	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 4-01-04	223.61 138.33		
	1 2-ply	T30Z Half Hip Girder	6 /12	23-09-00	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 4-01-04	223.61 138.33		
	2	T31 Half Hip	6 /12	23-09-00	5-01-04	2 x 4	1-03-08	1-02-00 5-01-04	195.92 122.00		
	2	T32 Half Hip	6 /12	23-09-00	6-01-04	2 x 4	1-03-08	1-02-00 6-01-04	196.73 122.33		
	2	T33 Half Hip	6 /12	23-09-00	7-01-04	2 x 4	1-03-08	1-02-00 7-01-04	206.87 127.67		
	2	T34 Half Hip	6 /12	23-09-00	8-01-04	2 x 4	1-03-08	1-02-00 8-01-04	218.08 135.33		
	2	T35 Half Hip	6 /12	23-09-00	9-01-04	2 x 4	1-03-08	1-02-00 9-01-04	216.08 134.67		
	2	T36 Half Hip	6 /12	23-09-00	10-01-04	2 x 4	1-03-08	1-02-00 10-01-04	231.84 141.67		
	1	T37 Hip Girder	6 /12	13-06-00	3-07-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	53.45 35.50		
	3	T38 Common	6 /12	13-06-00	6-00-08	2 x 4	1-03-08 1-03-08	2-08-00 2-08-00	184.93 119.00		
	1	T39 Hip Girder	6 /12	8-04-00	1-07-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	33.55 24.00		
	1 2-ply	T40 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T40Z Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T41 Flat Girder	0 /12	7-02-00	2-03-08	2 x 4		2-03-08 2-03-08	55.05 36.67		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMPONE INVESTMENTS INC
 Location: MARKHAM
 Model: PAYTON 2S
 Lot #:
 Elevation: 3- LEFT OR RIGHT

Job Track: 51453
 PlanLog: 203184
 Layout ID: 411579
 Ref #
 Page: 2 of 2
 Date: 08-05-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	11	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	184.74 117.33		
	3	J31 Jack-Open	6 /12	4-10-08	3-07-04	2 x 4		1-02-00 3-07-04	38.2 24.00		
	4	J32 Jack-Open	6 /12	10-08	1-07-04	2 x 4	1-03-08	1-02-00 1-07-04	18.87 13.33		
	4	J33 Jack-Open	6 /12	3-04-08	4-01-04	2 x 4		2-05-00 4-01-04	50.57 32.67		
	1	C30 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	14.13 8.67		
	1	C31 Jack-Open Girder	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	11.58 7.33		
	3	C32 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	28.71 18.00		
	3	C33 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	21.06 14.00		
	2	C34 Jack-Open	6 /12	4-10-08	3-00-12	2 x 4		1-02-00 3-07-04	22.69 14.67		
	2	C35 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 3-01-01	1-02-00 2-00-12	20.88 13.33		

TOTAL # TRUSS= 61

TOTAL BFT OF ALL TRUSSES= 1614.17

BFT.

TOTAL WEIGHT OF ALL TRSSES 2567.94 LBS

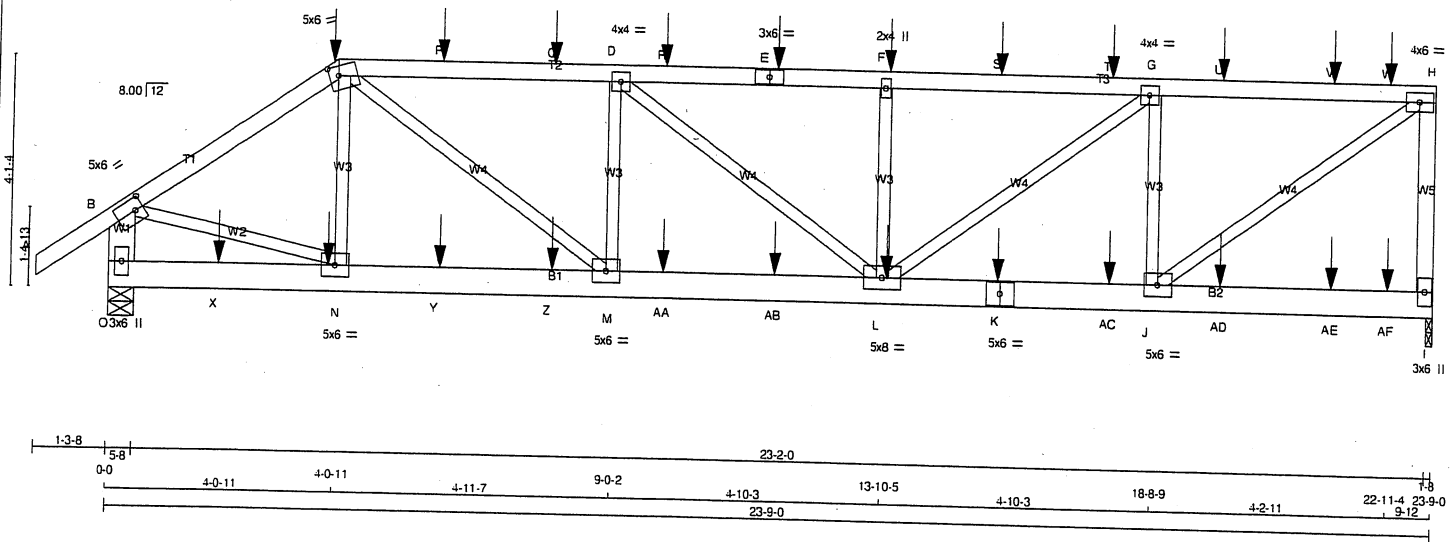
HARDWARE

QTY	TYPE	MODEL	LENGTH
10	Hardware	LUS24	
3	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 13

JOB NAME 410146	TRUSS NAME T1	QUANTITY 2	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.330 S May 6 2020 MITek Industries, Inc. Tue Aug 4 13:53:00 2020 Page 1
ID:W63xGm6okc00SiyZ4EwB6zA2m0-UCaDYfEBFqOtiuv45P0Ch_ZlcyssfPx3I85Q1MyrDe1
13-10-5 13-10-5 18-8-9 4-2-11 22-11-4 23-9-0 9-12-1
Scale = 1:39.0



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - H	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
O - B	2x6	DRY No.2	SPF
O - K	2x6	DRY No.2	SPF
K - I	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 EXCEPT SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE(61.0)
C - E	12	SIDE(61.0)
E - H	12	SIDE(61.0)
H - I	12	TOP
O - B	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
O - K	12	SIDE(183.1)
K - I	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
I	2253	0	2253	0	0
O	2351	0	2351	0	0

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
I	1595	1036.0	0.0
O	1660	1104.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0.35	-91.8	-91.8	N-C	-364.0
B-C	-2640.0	-91.8	-91.8	B-N	0.2255
C-P	-3469.0	-91.8	-91.8	J-H	0.3052
P-Q	-3469.0	-91.8	-91.8	C-M	0.1608
Q-D	-3469.0	-91.8	-91.8	J-G	-1636.0
D-R	-3464.0	-91.8	-91.8	M-D	-812.0
R-E	-3464.0	-91.8	-91.8	L-G	0.1276
E-F	-3464.0	-91.8	-91.8	D-L	-6.16
F-S	-3464.0	-91.8	-91.8	L-F	-641.0
S-T	-3464.0	-91.8	-91.8		
T-G	-3464.0	-91.8	-91.8		
G-U	-2453.0	-91.8	-91.8		
U-V	-2453.0	-91.8	-91.8		
V-W	-2453.0	-91.8	-91.8		
W-H	-2453.0	-91.8	-91.8		
I-H	-2165.0	0.0	0.0		
O-B	-2307.0	0.0	0.0		

CHORDS		MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0.0	-18.5	-18.5	N-C	-364.0
X-N	0.0	-18.5	-18.5	B-N	0.2255
N-Y	0.2183	-18.5	-18.5	J-H	0.3052
Y-Z	0.2183	-18.5	-18.5	C-M	0.1608
Z-M	0.2183	-18.5	-18.5	J-G	-1636.0
M-AA	0.3469	-18.5	-18.5	M-D	-812.0
AA-AB	0.3469	-18.5	-18.5	L-G	0.1276
AB-L	0.3469	-18.5	-18.5	D-L	-6.16
L-K	0.2453	-18.5	-18.5	L-F	-641.0
K-AC	0.2453	-18.5	-18.5		
AC-J	0.2453	-18.5	-18.5		
J-AD	0.0	-18.5	-18.5		
AD-AE	0.0	-18.5	-18.5		
AE-AF	0.0	-18.5	-18.5		
AF-I	0.0	-18.5	-18.5		

FACTORED CONCENTRATED LOADS (LBS)		LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	4-0-11	-51	-57	---	---	FRONT	VERT	DEAD	---	C1
C	4-0-11	-117	-117	---	---	FRONT	VERT	TOTAL	---	C1
C	4-0-11	-260	-260	---	---	FRONT	VERT	SNOW	---	C1
E	11-11-4	-110	-110	---	---	FRONT	VERT	TOTAL	---	C1
F	13-11-4	-110	-110	---	---	FRONT	VERT	TOTAL	---	C1
K	15-11-4	-26	-26	---	---	FRONT	VERT	TOTAL	---	C1
L	13-11-4	-26	-26	---	---	FRONT	VERT	TOTAL	---	C1

TOTAL WEIGHT = 4 X 111 = 442 lb

DESIGN CRITERIA
*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.38/1.00 (F-G:1), BC=0.27/1.00 (L-M:1), WB=0.38/1.00 (H-J:1), SSI=0.19/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.34 (H) (INPUT = 1.00)



Structural component only
DWG# T-2017154 1/2

JOB NAME 410146	TRUSS NAME T1	QUANTITY 2	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.330 S May 6 2020 MiTek Industries, Inc. Tue Aug 4 13:53:00 2020 Page 2 ID:W63xGm6okc00SivZ4EwBx6zA2m0-UCeDYfEBFqOtiVu45P0Ch ZlcyCsFPx3I85Q1MyrDe1	

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	5.0	6.0	2.50 1.75
C	TTWW-m	MT20	5.0	6.0	2.00 2.00
D	TMVW-t	MT20	4.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TMVW-w	MT20	2.0	4.0	
G	TMVW-t	MT20	4.0	4.0	
H	TMVW-t	MT20	4.0	6.0	
I	BMV1+p	MT20	3.0	6.0	
J, M, N					
J	BMWW-t	MT20	5.0	6.0	
K	BS-t	MT20	5.0	6.0	
L	BMWW-t	MT20	5.0	8.0	
O	BMV1+p	MT20	3.0	6.0	

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	5-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Q	7-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
R	9-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
S	15-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
T	17-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
U	19-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	21-11-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	22-11-4	-119	-119	---	FRONT	VERT	TOTAL	---	C1
X	1-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Y	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Z	7-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AA	9-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AB	11-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AC	17-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AD	19-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AE	21-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AF	22-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1

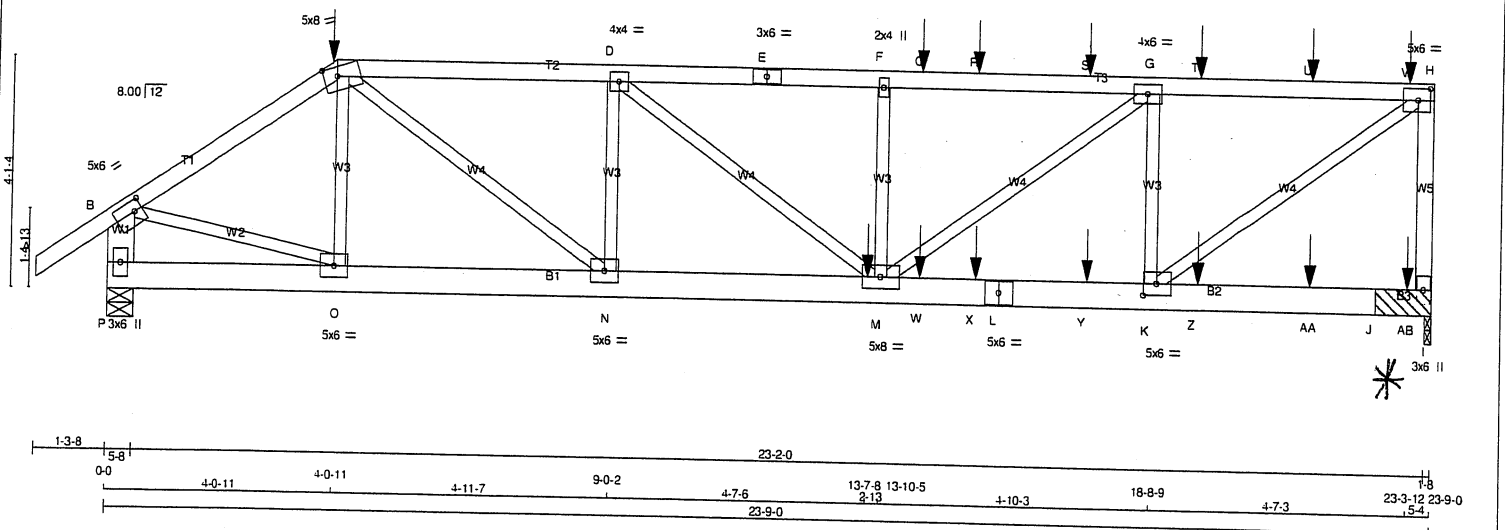
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



JOB NAME 410146	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.330 S May 6 2020 MiTek Industries, Inc. Tue Aug 4 13:53:01 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-yOCbm?Fp08WkK3THf6XRDC6R9LxGOqCDzorzZoyrDe0
23-3-12 23-9-0
Scale = 1:39.0



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT	REQD	*** SPECIAL LOADS ANALYSIS ***	TOTAL WEIGHT = 2 X 112 = 225 lb		
CHORDS				GROSS REACTION	GROSS REACTION	BRG	BRG	GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.			
A - C	2x4	DRY	No.2	VERT	DOWN	IN-SX	IN-SX	LOADS WERE DERIVED FROM USER INPUT			
C - E	2x4	DRY	No.2	HORZ	UP	IN-SX	IN-SX	NO FURTHER MODIFICATIONS WERE MADE			
E - H	2x4	DRY	No.2								
I - H	2x4	DRY	No.2								
P - B	2x6	DRY	No.2								
P - L	2x6	DRY	No.2								
L - I	2x6	DRY	No.2								
ALL WEBS	2x3	DRY	No.2								
EXCEPT											

UNFACTORED REACTIONS				DESIGN CRITERIA			
1ST LCASE	MAX./MIN.	COMPONENT REACTIONS		TOP CH. LL = 25.6 PSF			
JT COMBINED	SNOW	LIVE	PERM. LIVE	DL = 6.0 PSF			
I 2025	1347.0	0.0	0.0	BOT CH. LL = 0.0 PSF			
P 1730	1189.0	0.0	0.0	DL = 7.4 PSF			
				TOTAL LOAD = 39.0 PSF			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P
2x6 DRY SPF NO.2 BEARING BLOCK 12" LONG AT JT. I ATTACHED TO FRONT SIDE WITH 3 ROWS OF (0.122"x3") SPIRAL NAILS SPACED 3" C.C. 12 NAILS TOTAL.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING				WEBS			
TOTAL LOAD CASES: (4)				MEMB.	MAX. FORCE	MAX. CSI (LC)	
CHORDS	MAX. FORCE	FACTORED		FR-TO			
MEMB.	(LBS)	(PLF)		FROM TO			

FR-TO	MAX. FORCE	FACTORED	CS (LC)	UNBRACED LENGTH	FR-TO	MAX. FORCE	MAX. CS (LC)
A-B	0.35	-91.8	-91.8 0.07 (1)	10.00	O-C	-463.0	0.06 (1)
B-C	-2798.0	-91.8	-91.8 0.18 (1)	5.29	B-O	0.2390	0.30 (1)
C-D	-4079.0	-91.8	-91.8 0.24 (1)	4.50	K-H	0.4077	0.50 (1)
D-E	-5023.0	-91.8	-91.8 0.28 (1)	4.09	C-N	0.2210	0.27 (1)
E-F	-5023.0	-91.8	-91.8 0.28 (1)	4.09	K-G	-2217.0	0.28 (1)
F-Q	-5023.0	-91.8	-91.8 0.45 (1)	3.87	N-D	-1211.0	0.15 (1)
Q-R	-5023.0	-91.8	-91.8 0.45 (1)	3.87	M-G	0.2203	0.27 (1)
R-S	-5023.0	-91.8	-91.8 0.45 (1)	3.87	D-M	0.1191	0.15 (1)
S-G	-5023.0	-91.8	-91.8 0.45 (1)	3.87	M-F	-582.0	0.07 (1)
G-T	-3277.0	-91.8	-91.8 0.38 (1)	4.72			
T-U	-3277.0	-91.8	-91.8 0.38 (1)	4.72			
U-V	-3277.0	-91.8	-91.8 0.38 (1)	4.72			
V-H	-3277.0	-91.8	-91.8 0.38 (1)	4.72			
H-I	-2781.0	0.0	0.0 0.35 (1)	6.87			
P-B	-2427.0	0.0	0.0 0.09 (1)	7.81			
P-O	0.0	-18.5	-18.5 0.02 (4)	10.00			
O-N	0.2311	-18.5	-18.5 0.17 (1)	10.00			
N-M	0.4079	-18.5	-18.5 0.32 (1)	10.00			
M-W	0.3277	-18.5	-18.5 0.28 (1)	10.00			
W-X	0.3277	-18.5	-18.5 0.28 (1)	10.00			
X-L	0.3277	-18.5	-18.5 0.28 (1)	10.00			
L-Y	0.3277	-18.5	-18.5 0.28 (1)	10.00			
Y-K	0.3277	-18.5	-18.5 0.28 (1)	10.00			
K-Z	0.0	-18.5	-18.5 0.05 (4)	10.00			
Z-AA	0.0	-18.5	-18.5 0.05 (4)	10.00			
AA-J	0.0	-18.5	-18.5 0.05 (4)	10.00			
J-AB	0.0	-18.5	-18.5 0.05 (4)	10.00			
AB-I	0.0	-18.5	-18.5 0.05 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)				DESIGN CRITERIA			
JT LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL
C 4-0-11	-51	-57	---	FRONT	VERT	DEAD	---
C 4-0-11	-260	-260	---	FRONT	VERT	SNOW	---
M 13-7-8	-1420	-1420	---	FRONT	VERT	TOTAL	---
Q 14-6-12	-110	-110	---	FRONT	VERT	TOTAL	---
R 15-6-12	-110	-110	---	FRONT	VERT	TOTAL	---
S 17-6-12	-110	-110	---	FRONT	VERT	TOTAL	---
T 19-6-12	-110	-110	---	FRONT	VERT	TOTAL	---
U 21-6-12	-110	-110	---	FRONT	VERT	TOTAL	---



Structural component only
DWG# T-2017155 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410146	T1Z	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.25	1.75
C	TTWW-m	MT20	5.0	8.0	2.00	3.00
D	TMVW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMVW-w	MT20	2.0	4.0		
G	TMVW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0	2.50	2.75
I	BMV1+p	MT20	3.0	6.0		
K	BMVW-t	MT20	5.0	6.0	2.50	2.75
L	BS-t	MT20	5.0	6.0		
M	BMVWW-t	MT20	5.0	8.0		
N	BMVW-t	MT20	5.0	6.0		
O	BMVW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	6.0		

FACTORED CONCENTRATED LOADS (LBS)

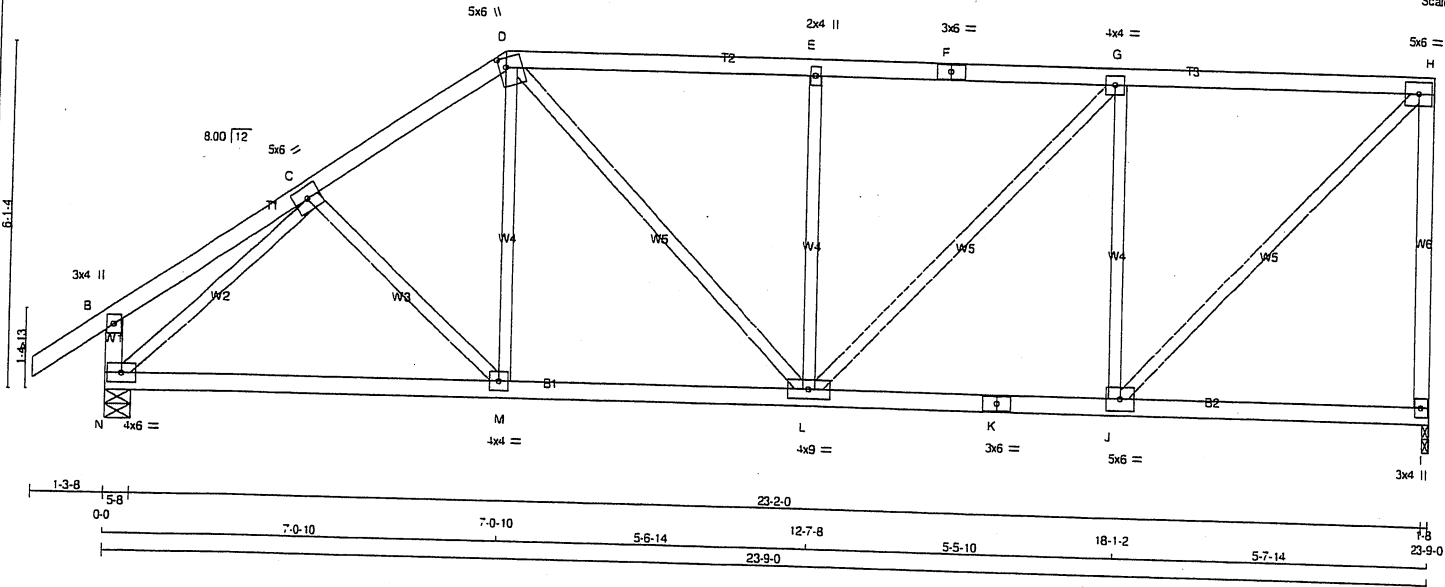
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
V	23-3-12	-136	-136	---	FRONT	VERT	TOTAL	---	C1
W	14-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
X	15-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Y	17-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Z	19-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AA	21-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AB	23-3-12	-32	-32	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2017155 2/2



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
I - H	2x4	DRY	No.2
N - B	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	5.0	6.0	
D	TTWW+m	MT20	5.0	6.0	2.00 1.50
E	TMW-w	MT20	2.0	4.0	
F	TS-t	MT20	3.0	6.0	
G	TMWW-t	MT20	4.0	4.0	
H	TMWW-t	MT20	5.0	6.0	
I	BMV1+p	MT20	3.0	4.0	
J	BMWW-t	MT20	5.0	6.0	
K	BS-t	MT20	3.0	6.0	
L	BMWW-t	MT20	4.0	9.0	
M	BMWW-t	MT20	4.0	4.0	
N	BMV1-t	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
I	1309 0	1309 0	1-8	1-8
N	1435 0	1435 0	5-8	5-8

UNFACTORED REACTIONS							
	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	926	608 0	0 0	0 0	0 0	318 0	0
N	1013	678 0	0 0	0 0	0 0	335 0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	MAX. UNBRACED LENGTH (LC)
FR-TO				FR-TO			
A-B	0 35	-91.8	-91.8 0.12 (1)	10.00	C-M	-36 36	0.01 (1)
B-C	0 19	-91.8	-91.8 0.17 (1)	10.00	M-D	0 164	0.05 (4)
C-D	-1383 0	-91.8	-91.8 0.20 (1)	5.32	N-C	-1617 0	0.62 (1)
D-E	-1382 0	-91.8	-91.8 0.43 (1)	5.00	J-H	0 1475	0.33 (1)
E-F	-1382 0	-91.8	-91.8 0.44 (1)	4.97	D-L	0 355	0.08 (1)
F-G	-1382 0	-91.8	-91.8 0.44 (1)	4.97	J-G	-937 0	0.55 (1)
G-H	-1028 0	-91.8	-91.8 0.42 (1)	5.59	L-E	-549 0	0.32 (1)
I-H	-1267 0	0.0	0.0 0.91 (1)	7.15	L-G	0 516	0.12 (1)
N-B	-253 0	0.0	0.0 0.03 (1)	7.81			
N-M	0 1159	-18.5	-18.5 0.30 (1)	10.00			
M-L	0 1135	-18.5	-18.5 0.31 (4)	10.00			
L-K	0 1028	-18.5	-18.5 0.24 (1)	10.00			
K-J	0 1028	-18.5	-18.5 0.24 (1)	10.00			
J-I	0 0	-18.5	-18.5 0.14 (4)	10.00			

TOTAL WEIGHT = 4 X 105 = 419 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.79")
CALCULATED VERT. DEFL.(LL)= L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.79")
CALCULATED VERT. DEFL.(TL)= L/999 (0.11")

CSI: TC=0.91/1.00 (H-I:1) , BC=0.31/1.00 (L-M:4) , WB=0.62/1.00 (C-N:1) , SSI=0.24/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1 10
COMP=1.10 SHEAR=1.10 TENS=1 10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

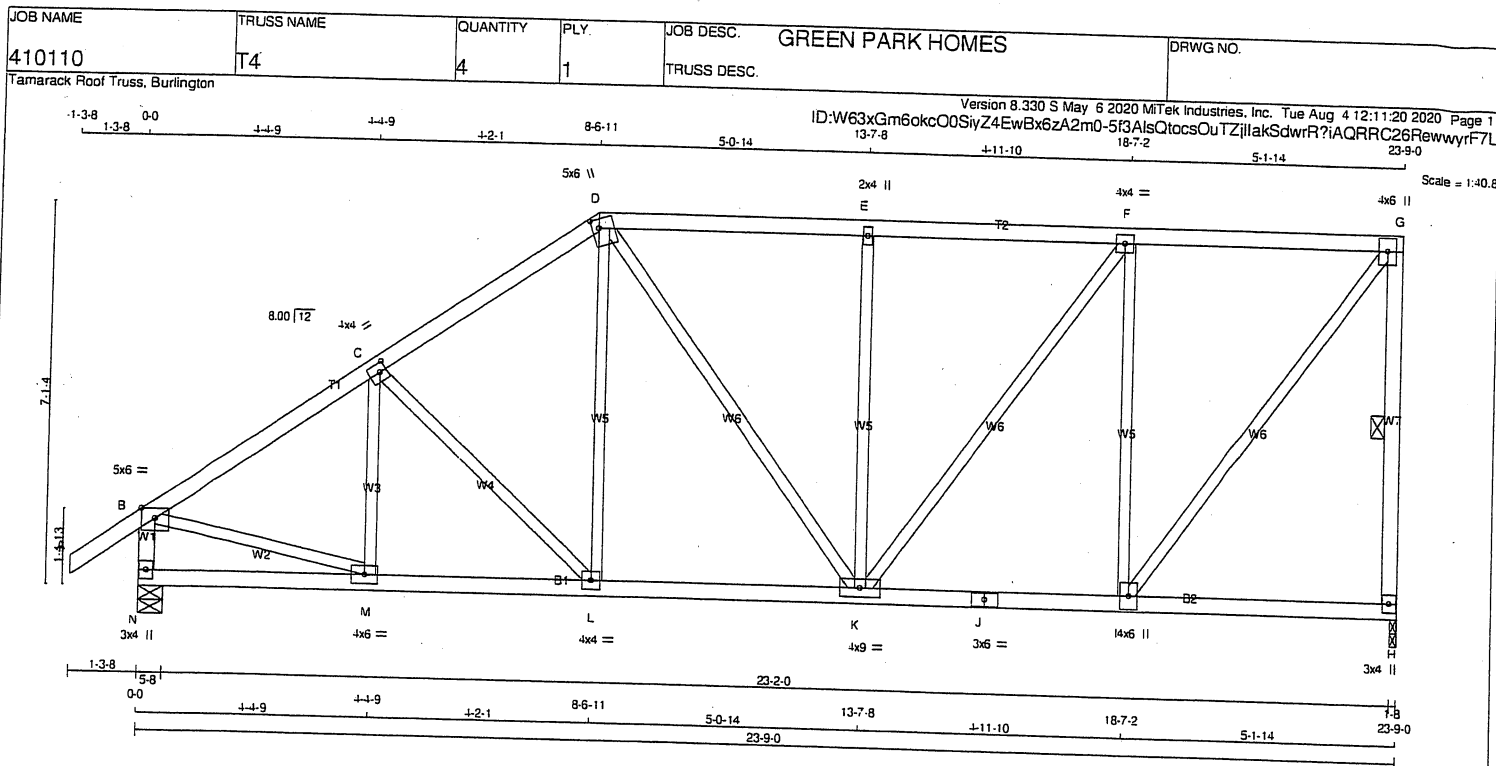
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (N) (INPUT = 0.90)
JSI METAL = 0.36 (C) (INPUT = 1.00)





LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
N - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	6.0	Edge	
C TMWW-t	MT20	4.0	4.0	2.00	1.50
D TTWW-m	MT20	5.0	6.0	2.00	1.50
E TMVW-w	MT20	2.0	4.0		
F TMWW-t	MT20	4.0	4.0		
G TMVW-p	MT20	4.0	6.0		
H BMV1+p	MT20	3.0	4.0		
I BMWW-t	MT20	4.0	6.0		
J BS-t	MT20	3.0	6.0		
K BMWW-w	MT20	4.0	9.0		
L BMWW-t	MT20	4.0	4.0		
M BMWW-w	MT20	4.0	6.0		
N BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
H	1309	0	1309	0	1-8	1-8
N	1435	0	1435	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
H	926	608	0	0	0	318	0
N	1013	678	0	0	0	335	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0.35	-91.8	-91.8 0.12 (1)	10.00	M-C	-220	0.06 (1)
B-C	-1445	-91.8	-91.8 0.32 (1)	5.07	C-L	-229	0.13 (1)
C-D	-1298	-91.8	-91.8 0.31 (1)	5.30	L-D	0	0.06 (1)
D-E	-1152	-91.8	-91.8 0.34 (1)	5.49	B-M	0	0.28 (1)
E-F	-1152	-91.8	-91.8 0.35 (1)	5.47	I-G	0	0.30 (1)
F-G	-816	-91.8	-91.8 0.34 (1)	6.24	D-K	0	0.03 (1)
H-G	-1271	0.0	0.0 0.29 (1)	5.71	I-F	-972	0.85 (1)
N-B	-1399	0.0	0.0 0.14 (1)	6.88	K-E	-498	0.44 (1)
					K-F	0	0.13 (1)
N-M	0	-18.5	-18.5 0.07 (4)	10.00			
M-L	0	-18.5	-18.5 0.24 (1)	10.00			
L-K	0	-18.5	-18.5 0.22 (1)	10.00			
K-J	0	-18.5	-18.5 0.19 (1)	10.00			
J-I	0	-18.5	-18.5 0.19 (1)	10.00			
I-H	0	-18.5	-18.5 0.11 (4)	10.00			

TOTAL WEIGHT = 4 X 113 = 453 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.35/1.00 (E-F:1), BC=0.24/1.00 (L-M:1), WB=0.85/1.00 (F-I:1), SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

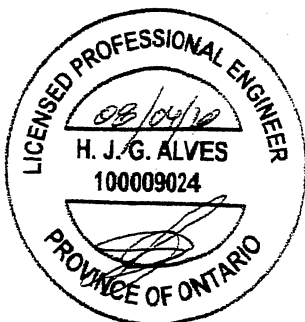
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) (PSI) SHEAR SECTION (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

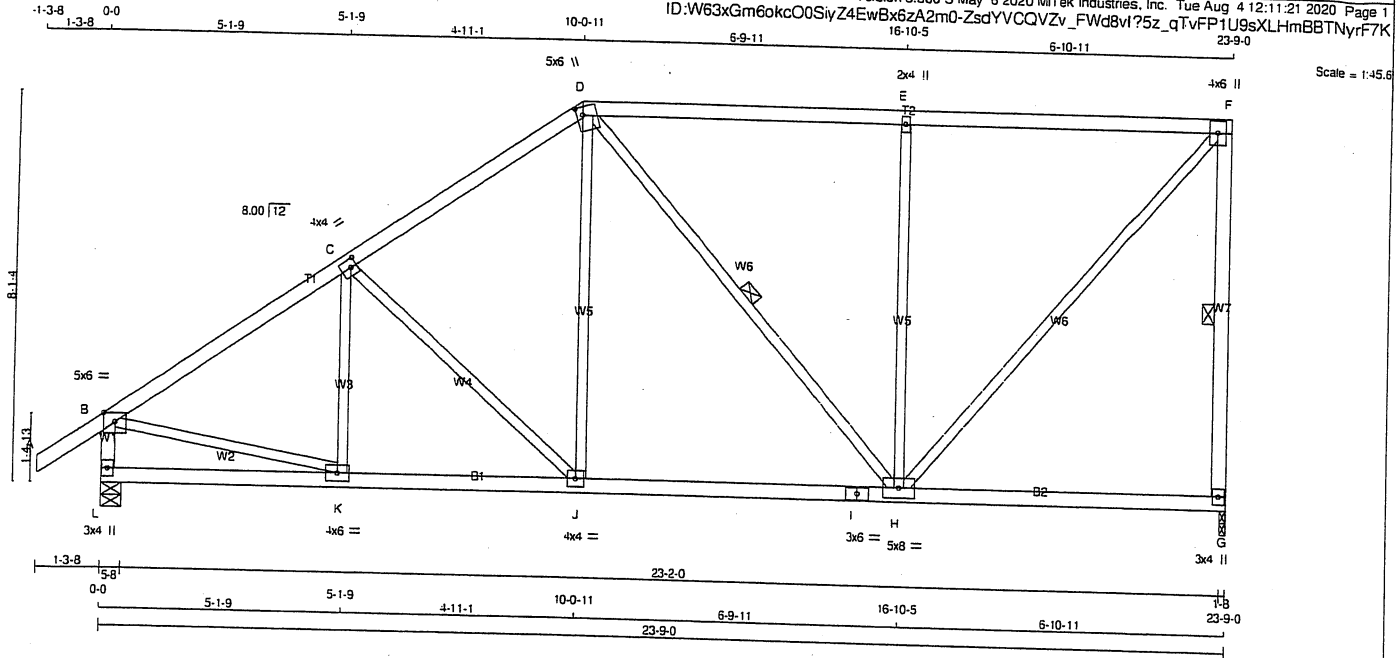
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.85 (M) (INPUT = 0.90)
JSI METAL = 0.37 (G) (INPUT = 1.00)



Structural component only
DWG# T-2017082



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMVW-i	MT20	4.0	4.0	2.00	1.50
D	TTVW+m	MT20	5.0	6.0	2.00	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-i	MT20	5.0	8.0		
I	BS-i	MT20	3.0	6.0		
J	BMVW-i	MT20	4.0	4.0		
K	BMVW-i	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT REQRD BRG
G	1309 0	1309 0	1-8
L	1435 0	1435 0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. SNOW	MAX. MIN. LIVE	MAX. MIN. PERM. LIVE	MAX. MIN. WIND	MAX. MIN. DEAD	MAX. MIN. SOIL
G	926	608 0	0 0	0 0	0 0	318 0	0 0
L	1013	678 0	0 0	0 0	0 0	335 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)
FR-TO			
A-B	0 35	-91.8 -91.8	0.12 (1)
B-C	-1459 0	-91.8 -91.8	0.46 (1)
C-D	-1206 0	-91.8 -91.8	0.43 (1)
D-E	-898 0	-91.8 -91.8	0.79 (1)
E-F	-898 0	-91.8 -91.8	0.79 (1)
G-F	-1258 0	0.0 0.0	0.37 (1)
L-B	-1394 0	0.0 0.0	0.14 (1)
L-K	0 0	-18.5 -18.5	0.10 (4)
K-J	0 1239	-18.5 -18.5	0.26 (1)
J-I	0 980	-18.5 -18.5	0.30 (4)
I-H	0 980	-18.5 -18.5	0.30 (4)
H-G	0 0	-18.5 -18.5	0.22 (4)

WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	MAX. FACTORED FORCE (LBS)
FR-TO			
K-C	-163 26	0.06 (1)	
C-J	-354 0	0.30 (1)	
J-D	0 341	0.08 (1)	
B-K	0 1266	0.28 (1)	
H-F	0 1358	0.31 (1)	
D-H	-125 0	0.10 (1)	
H-E	-780 0	0.99 (1)	

TOTAL WEIGHT = 4 X 109 = 437 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.79/1.00 (E-F:1), BC=0.30/1.00 (H-J:4), WB=0.99/1.00 (E-H:1), SSI=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (FSI)	SECTION (PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

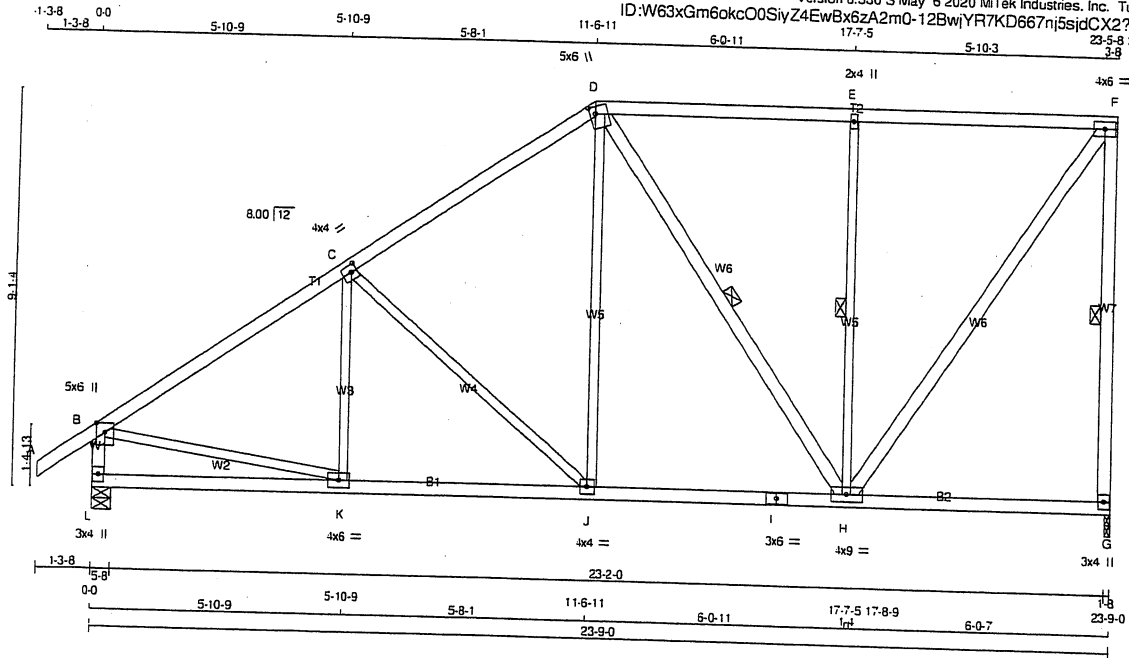
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.83 (F) (INPUT = 0.90)
JSI METAL = 0.40 (F) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410110	T6	4	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

Version 8.330 S May 6 2020 Mitek Industries, Inc. Tue Aug 4 12:11:22 2020 Page 1
ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-12BwYR7KD667nJ5sJdCX277WpN9uQSVWQwI?pyrF7J
23-5-8 23-9-0 3-8
Scale = 1:50.4



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2		
D - F	2x4	DRY	No.2		
G - F	2x4	DRY	No.2		
L - B	2x4	DRY	No.2		
L - I	2x4	DRY	No.2		
I - G	2x4	DRY	No.2		

ALL WEBS EXCEPT	2x3	DRY	No.2
H - F	2x4	DRY	No.2
D - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
G	1309	0	1309	0
L	1435	0	1435	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED	926	608	0	0	0	0
G	926	608	0	0	0	318	0
L	1013	678	0	0	0	335	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MEMB.	FORCE	VERT. LOAD	LC1	MAX	UNBRAC	MEMB.	MAX. FACTORED	FORCE	MAX
FR-TO	(LBS)	(PLF)	CS	(LC)	LENGTH	FR-TO	(LBS)	CS	(LC)	(LBS)	CS	(LC)
A-B	0	35	-91.8	-91.8	0.12	(1)	10.00	K-C	-110	59	0.05	(1)
B-C	-1459	0	-91.8	-91.8	0.63	(1)	4.61	C-J	-476	0	0.56	(1)
C-D	-1100	0	-91.8	-91.8	0.58	(1)	5.18	J-D	0	414	0.09	(1)
D-E	-731	0	-91.8	-91.8	0.61	(1)	5.91	B-K	0	1264	0.28	(1)
E-F	-731	0	-91.8	-91.8	0.61	(1)	5.91	H-F	0	1279	0.21	(1)
G-F	-1263	0	0.0	0.0	0.49	(1)	5.73	D-H	-278	0	0.17	(1)
L-B	-1390	0	0.0	0.0	0.14	(1)	6.90	H-E	-694	0	0.37	(1)
L-K	0	0	-18.5	-18.5	0.15	(4)	10.00					
K-J	0	1243	-18.5	-18.5	0.27	(1)	10.00					
J-I	0	889	-18.5	-18.5	0.24	(4)	10.00					
I-H	0	889	-18.5	-18.5	0.24	(4)	10.00					
H-G	0	0	-18.5	-18.5	0.17	(4)	10.00					

TOTAL WEIGHT = 4 X 121 = 485 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.63/1.00 (B-C:1), BC=0.27/1.00 (J-K:1), WB=0.56/1.00 (C-J:1), SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

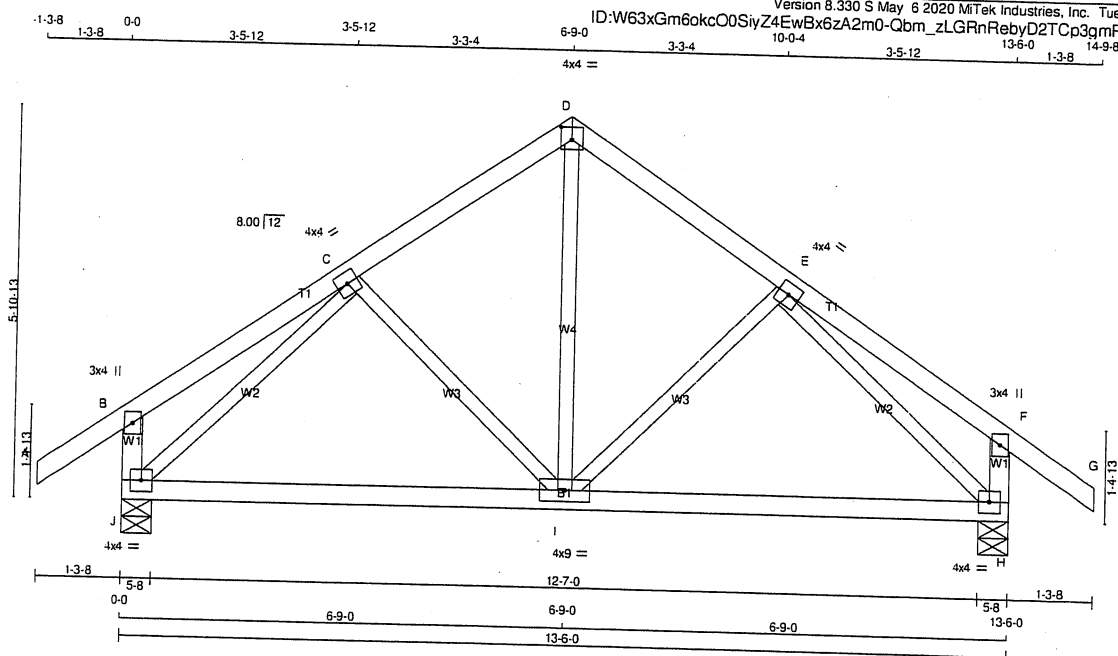
JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.62 (B) (INPUT = 1.00)



Structural component only
DWG# T-2017084

JOB NAME 410146	TRUSS NAME T8	QUANTITY 7	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:33.1

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVW1-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
	VERT	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
J	870	0	870	0	0	5-8	5-8	5-8
H	870	0	870	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS							
JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	613	416 / 0	0 / 0	0 / 0	0 / 0	197 / 0	0 / 0
H	613	416 / 0	0 / 0	0 / 0	0 / 0	197 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0.35	-91.8	-91.8 0.12 (1)	I-D	0.382	0.09 (1)	
B-C	0.19	-91.8	-91.8 0.16 (1)	I-E	-163 / 0	0.06 (1)	
C-D	-581 / 0	-91.8	-91.8 0.13 (1)	C-I	-163 / 0	0.06 (1)	
D-E	-581 / 0	-91.8	-91.8 0.13 (1)	J-C	-821 / 0	0.29 (1)	
E-F	0.19	-91.8	-91.8 0.16 (1)	E-H	-821 / 0	0.29 (1)	
F-G	0.35	-91.8	-91.8 0.12 (1)				
J-B	-245 / 0	0.0	0.0 0.03 (1)				
H-F	-245 / 0	0.0	0.0 0.03 (1)				
J-I	0.584	-18.5	-18.5 0.29 (4)				
I-H	0.584	-18.5	-18.5 0.29 (4)				

TOTAL WEIGHT = 7 X 59 = 410 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.16/1.00 (E-F:1), BC=0.29/1.00 (H-I:4), WB=0.29/1.00 (E-H:1), SS=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.68 (E) (INPUT = 0.90)
JSI METAL = 0.26 (C) (INPUT = 1.00)

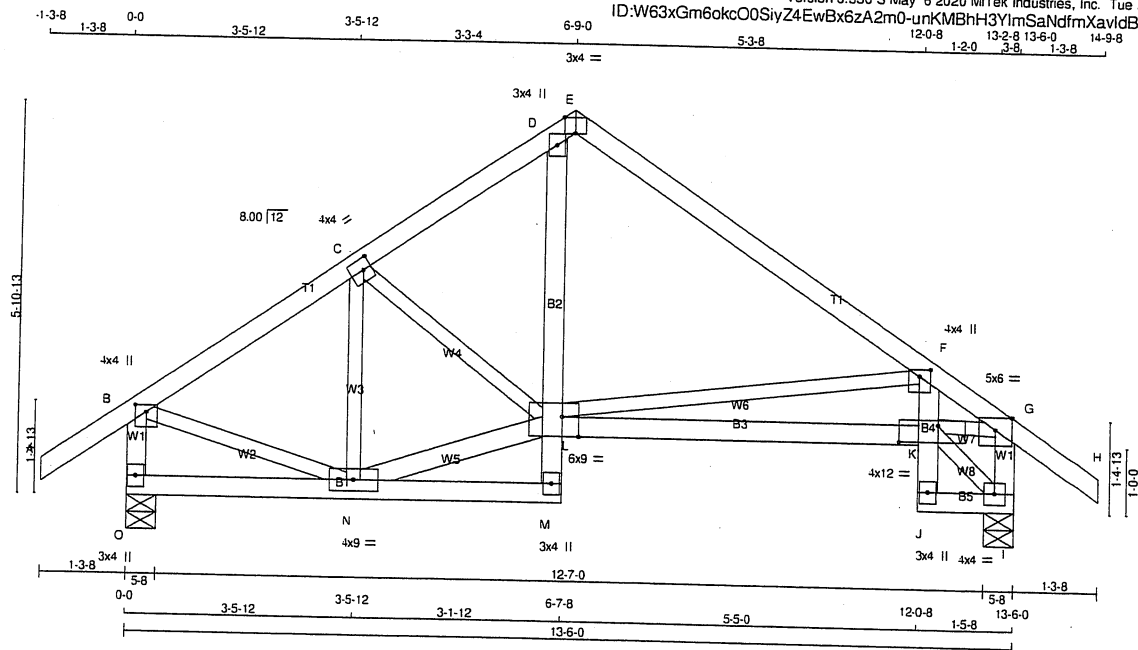


Structural component only
DWG# T-2017156

JOB NAME 410146	TRUSS NAME T8S	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - D	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TMV+p	MT20	3.0	4.0		
E	TT-p	MT20	3.0	4.0	Edge	2.00
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	TMVW-p	MT20	5.0	6.0	Edge	
I	BMVW1-t	MT20	4.0	4.0		
J	BMV+p	MT20	3.0	4.0		
K	BVMWV-t	MT20	4.0	12.0	3.00	7.00
L	BVMWVW-t	MT20	6.0	9.0	Edge	3.00
M	BMV+p	MT20	3.0	4.0		
N	BMVWVW-t	MT20	4.0	9.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
O	864	864	5-8	5-8
I	877	877	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
O	609	412 / 0	0 / 0	0 / 0	197 / 0	0 / 0
I	617	419 / 0	0 / 0	0 / 0	198 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)
FR-TO			FROM TO		FR-TO			
A-B	0:35	-91.8	-91.8	0.12 (1)	10.00	N-C	-316.0	0.07 (1)
B-C	-706.0	-91.8	-91.8	0.21 (1)	6.25	N-L	0.626	0.10 (1)
C-D	-685.0	-91.8	-91.8	0.24 (1)	6.25	C-L	-112.0	0.03 (1)
D-E	-372.0	-91.8	-91.8	0.23 (1)	6.25	L-F	-722.0	0.41 (1)
E-F	-659.0	-91.8	-91.8	0.36 (1)	6.25	B-N	0.636	0.14 (1)
F-G	-1343.0	-91.8	-91.8	0.30 (1)	5.20	K-I	-78.0	0.01 (1)
G-H	0:35	-91.8	-91.8	0.12 (1)	10.00	K-G	0.1161	0.26 (1)
O-B	-836.0	0.0	0.0	0.09 (1)	7.81			
I-G	-813.0	0.0	0.0	0.08 (1)	7.81			
O-N	0.0	-18.5	-18.5	0.05 (4)	10.00			
N-M	0.13	-18.5	-18.5	0.05 (4)	10.00			
M-L	0.23	0.0	0.0	0.02 (1)	10.00			
L-D	0.431	0.0	0.0	0.09 (1)	10.00			
L-K	0.1243	-18.5	-18.5	0.30 (1)	10.00			
J-K	0.14	0.0	0.0	0.09 (1)	10.00			
K-F	0.100	0.0	0.0	0.11 (1)	10.00			
J-I	0.59	-18.5	-18.5	0.01 (1)	10.00			

TOTAL WEIGHT = 2 X 66 = 131 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.36/1.00 (E-F:1), BC=0.30/1.00 (K-L:1),
WB=0.41/1.00 (F-L:1), SS=0.32/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

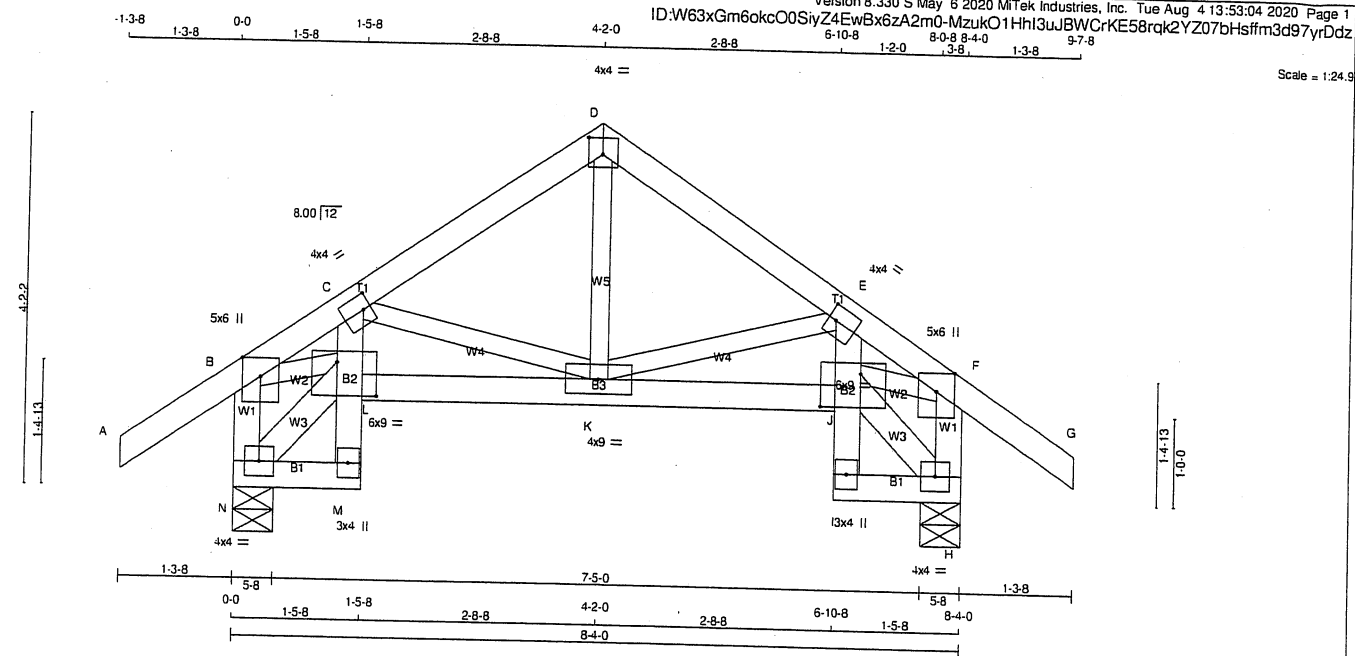
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (B) (INPUT = 0.90)
JSI METAL= 0.26 (K) (INPUT = 1.00)



Structural component only
DWG# T-2017157



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
N - B	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
N - M	2x4	DRY No.2	SPF
M - C	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
I - E	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
N - L	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-t	MT20	4.0	4.0	2.00	1.00
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMVW-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BYMVW-t	MT20	6.0	9.0	4.50	5.50
K	BMVW-t	MT20	4.0	9.0		
L	BYMVW-t	MT20	6.0	9.0	4.50	5.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
N	585	0	585	0	5-8	5-8
H	585	0	585	0	5-8	5-8

UNFACTORED REACTIONS

JT	MAX./MIN. COMPONENT REACTIONS						
	1ST LOASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	412	283 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0
H	412	283 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-91.8 -91.8	0.12 (1)	10.00	K-D	0 / 170	0.04 (1)
B-C	-691 / 0	-91.8 -91.8	0.10 (1)	6.25	C-K	-312 / 0	0.06 (1)
C-D	-378 / 0	-91.8 -91.8	0.10 (1)	6.25	N-L	-44 / 0	0.00 (1)
D-E	-378 / 0	-91.8 -91.8	0.10 (1)	6.25	B-L	0 / 557	0.13 (1)
E-F	-691 / 0	-91.8 -91.8	0.10 (1)	6.25	J-H	-44 / 0	0.00 (1)
F-G	0 / 35	-91.8 -91.8	0.12 (1)	10.00	J-F	0 / 557	0.13 (1)
N-B	-543 / 0	0.0 0.0	0.06 (1)	7.81			
H-F	-543 / 0	0.0 0.0	0.06 (1)	7.81			
N-M	0 / 32	-18.5 -18.5	0.01 (4)	10.00			
M-L	0 / 14	0.0 0.0	0.06 (1)	10.00			
L-C	0 / 108	0.0 0.0	0.07 (1)	10.00			
L-K	0 / 608	-18.5 -18.5	0.12 (1)	10.00			
K-J	0 / 608	-18.5 -18.5	0.12 (1)	10.00			
I-J	0 / 14	0.0 0.0	0.06 (1)	10.00			
J-E	0 / 108	0.0 0.0	0.07 (1)	10.00			
I-H	0 / 32	-18.5 -18.5	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.
 (55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
 ALLOWABLE DEFL.(TL) = L/360 (0.28")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.12/1.00 (A-B:1), BC=0.12/1.00 (J-K:1), WB=0.13/1.00 (F-J:1), SSI=0.09/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

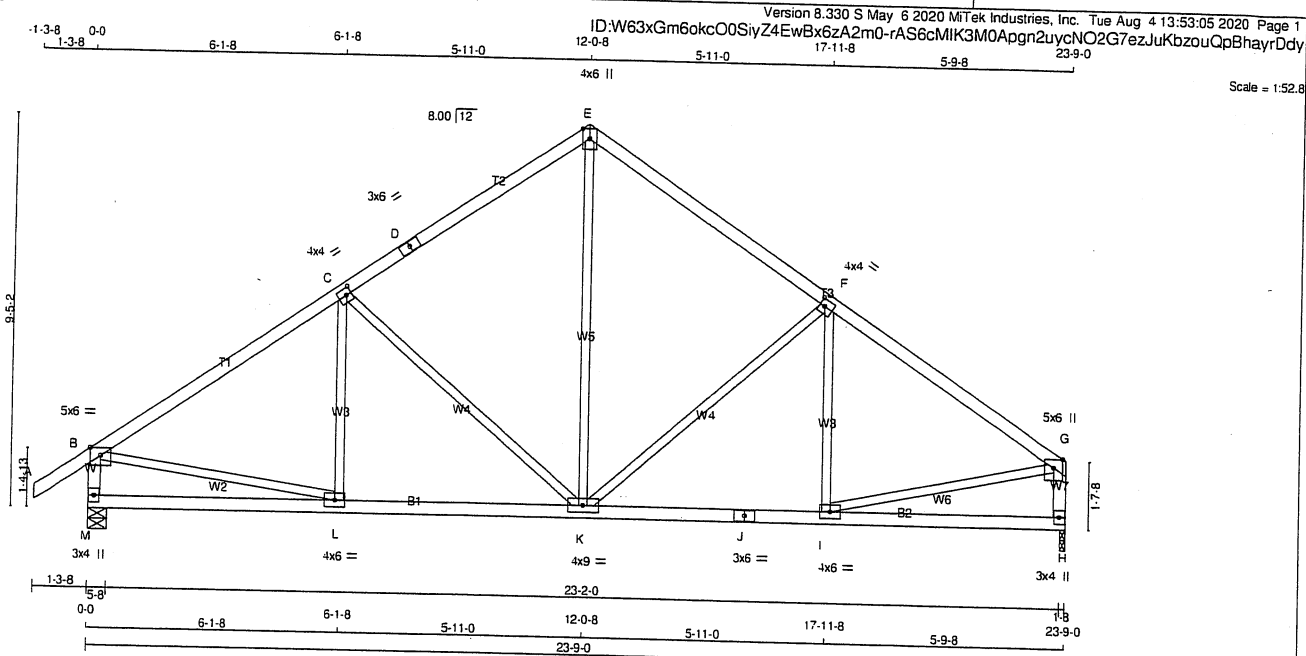
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.48 (F) (INPUT = 0.90)
 JSI METAL = 0.29 (F) (INPUT = 1.00)



JOB NAME 410146	TRUSS NAME T10	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)			
JT	TYPE	PLATES	W LEN Y X
B	TMVW-p	MT20	5.0 6.0 Edge
C	TMVW-t	MT20	4.0 4.0 2.00 1.50
D	TS-t	MT20	3.0 6.0
E	TTW-p	MT20	4.0 6.0 Edge
F	TMVW-t	MT20	4.0 4.0 2.00 1.50
G	TMVW-p	MT20	5.0 6.0 Edge
H	BMV1-p	MT20	3.0 4.0
I	BMVW-t	MT20	4.0 6.0
J	BS-t	MT20	3.0 6.0
K	BMVW-t	MT20	4.0 9.0
L	BMVW-t	MT20	4.0 6.0
M	BMV1-p	MT20	3.0 4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT REQD
	VERT	DOWN	BRG
M	1435	0	5-8
H	1309	0	1-8

UNFACTORED REACTIONS			
JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	
	COMBINED	SNOW	LIVE
M	1013	678 / 0	0 / 0
H	926	608 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO		FROM TO	
A-B	0 / 35	-91.8 -91.8	0.12 (1)
B-C	-1456 / 0	-91.8 -91.8	0.49 (1)
C-D	-1070 / 0	-91.8 -91.8	0.45 (1)
D-E	-1070 / 0	-91.8 -91.8	0.45 (1)
E-F	-1068 / 0	-91.8 -91.8	0.43 (1)
F-G	-1399 / 0	-91.8 -91.8	0.44 (1)
M-B	-1388 / 0	0.0 0.0	0.14 (1)
H-G	-1265 / 0	0.0 0.0	0.13 (1)
M-L	0 / 0	-18.5 -18.5	0.16 (4)
L-K	0 / 1242	-18.5 -18.5	0.28 (1)
K-J	0 / 1195	-18.5 -18.5	0.27 (1)
J-I	0 / 1195	-18.5 -18.5	0.27 (1)
I-H	0 / 0	-18.5 -18.5	0.15 (4)

WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)
FR-TO		FROM TO	
L-C	-96 67	0.04 (1)	
C-K	-509 0	0.65 (1)	
K-E	0 735	0.17 (1)	
K-F	-446 0	0.57 (1)	
I-F	-150 46	0.07 (1)	
B-L	0 1261	0.28 (1)	
I-G	0 1225	0.28 (1)	

TOTAL WEIGHT = 4 X 101 = 404 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.49/1.00 (B-C:1), BC=0.28/1.00 (K-L:1), WB=0.65/1.00 (C-K:1), SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES			
PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

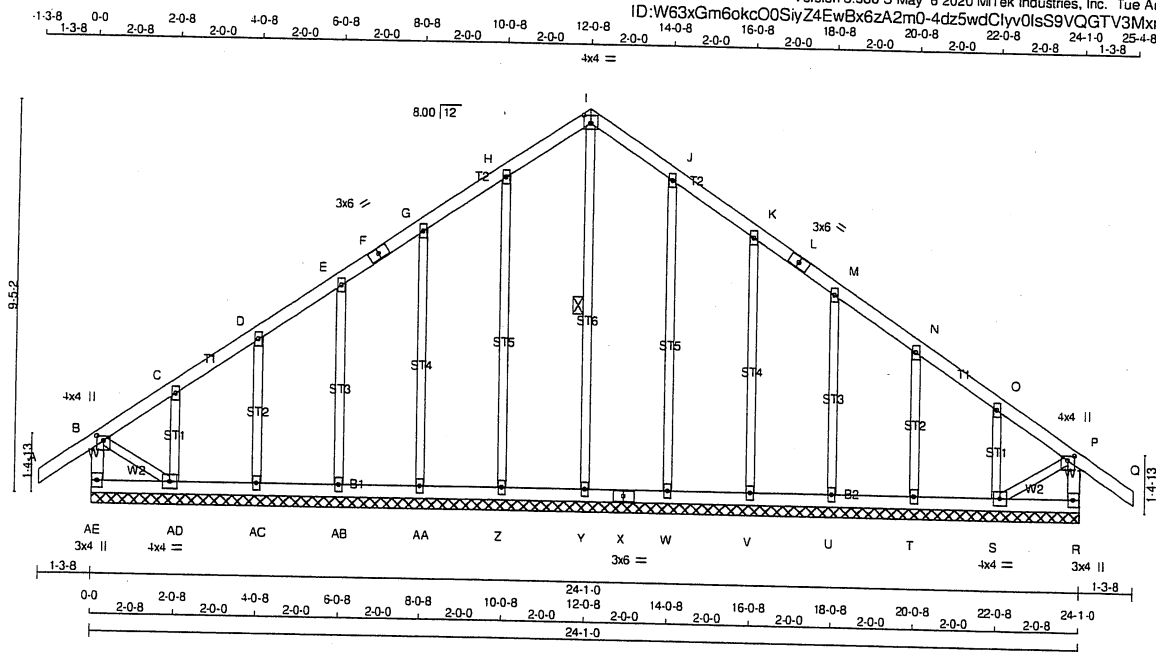
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (G) (INPUT = 0.90)
JSI METAL = 0.57 (G) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410146	G11	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.330 S May 6 2020 Mitek Industries, Inc. Tue Aug 4 13:52:57 2020 Page 1					
ID: W63xGm6okc00SiyZ4EwBx6zA2m0-4dz5wdChyw0lsS9VQGTv3MxrJkr3S3_d2AtmQ1yrDe4					
Scale = 1:52.9					



LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER	BEARINGS	DESIGN CRITERIA
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N. L. G. A. RULES						THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.		SPECIFIED LOADS:
A - F	2x4	DRY	No.2	SPF		THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.		TOP CH. LL = 25.6 PSF
F - I	2x4	DRY	No.2	SPF		BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)		DL = 6.0 PSF
I - L	2x4	DRY	No.2	SPF				BOT CH. LL = 0.0 PSF
L - Q	2x4	DRY	No.2	SPF				DL = 7.4 PSF
AE - B	2x4	DRY	No.2	SPF				TOTAL LOAD = 39.0 PSF
R - P	2x4	DRY	No.2	SPF				
AE - X	2x4	DRY	No.2	SPF				
X - R	2x4	DRY	No.2	SPF				

ALL WEBS	2x3	DRY	No.2	SPF				SPACING = 24.0 IN. C/C
ALL GABLE WEBS	2x3	DRY	No.2	SPF				
DRY: SEASONED LUMBER.								

GABLE STUDS SPACED AT 2'-0" OC.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF I-Y.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

PLATES (table is in inches)	W	LEN	Y	X	CHORDS	FACTORED	MAX.	FACTORED	WEBS	FACTORED	MAX.	FACTORED
JT TYPE	PLATES				MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MEMB.	FORCE (LBS)	CS1 (LC)	CS1 (LC)
B TMVW+p	MT20	4.0	4.0	1.25	2.00	FR-TO	FROM TO	UNBRAC	FR-TO	FROM TO	UNBRAC	FR-TO
C, D, E, G, H, J, K, M, N, O						A-B	0 35	-91.8	-91.8	0.12 (1)	10.00	Y-I
F	TS-I	2.0	4.0			B-C	-30 0	-91.8	-91.8	0.05 (1)	6.25	Z-H
I	TTW-p	2.0	4.0	2.25	2.00	C-D	-35 0	-91.8	-91.8	0.05 (1)	6.25	AA-G
L	TS-I	3.0	6.0			D-E	-27 0	-91.8	-91.8	0.04 (1)	6.25	AB-E
P	TMVW+p	2.0	4.0	1.25	2.00	E-F	-25 0	-91.8	-91.8	0.04 (1)	6.25	AC-D
R	BMV1+p	2.0	4.0			F-G	-25 0	-91.8	-91.8	0.04 (1)	6.25	AD-C
S	BMVW1-t	2.0	4.0			G-H	-19 0	-91.8	-91.8	0.05 (1)	6.25	AD-C
T, U, V, W, Y, Z, AA, AB, AC						H-I	-29 0	-91.8	-91.8	0.05 (1)	6.25	V-K
X	BMV1+w	2.0	4.0			I-J	-29 0	-91.8	-91.8	0.05 (1)	6.25	U-M
AD	BS-t	3.0	6.0			J-K	-19 0	-91.8	-91.8	0.05 (1)	6.25	T-N
AE	BMV1+p	2.0	4.0			K-L	-25 0	-91.8	-91.8	0.04 (1)	6.25	S-O
						L-M	-25 0	-91.8	-91.8	0.04 (1)	6.25	B-AD
						M-N	-27 0	-91.8	-91.8	0.04 (1)	6.25	S-P
						N-O	-35 0	-91.8	-91.8	0.05 (1)	6.25	
						O-P	-30 0	-91.8	-91.8	0.05 (1)	6.25	
						P-Q	0 35	-91.8	-91.8	0.12 (1)	10.00	
						AE-B	-237 0	0.0	0.0	0.02 (1)	7.81	
						R-P	-237 0	0.0	0.0	0.02 (1)	7.81	

AE-AD	0 0	-18.5	-18.5	0.02 (4)	10.00
AD-AC	0 27	-18.5	-18.5	0.02 (4)	10.00
AC-AB	0 23	-18.5	-18.5	0.02 (4)	10.00
AB-AA	0 20	-18.5	-18.5	0.02 (4)	10.00
AA-Z	0 18	-18.5	-18.5	0.02 (4)	10.00
Z-Y	0 16	-18.5	-18.5	0.02 (4)	10.00
Y-X	0 16	-18.5	-18.5	0.02 (4)	10.00
X-W	0 16	-18.5	-18.5	0.02 (4)	10.00
W-V	0 18	-18.5	-18.5	0.02 (4)	10.00
V-U	0 20	-18.5	-18.5	0.02 (4)	10.00
U-T	0 23	-18.5	-18.5	0.02 (4)	10.00
T-S	0 27	-18.5	-18.5	0.02 (4)	10.00
S-R	0 0	-18.5	-18.5	0.02 (4)	10.00

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.54 (S) (INPUT = 0.90)

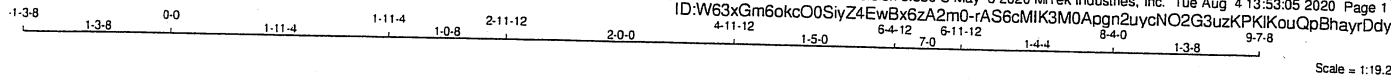
JSI METAL= 0.11 (J) (INPUT = 1.00)



Structural component only
DWG# T-2017151

JOB NAME 410146	TRUSS NAME T12	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-rAS6cMIK3M0Apn2uycNO2G3uzKPKJKouQpBhayrDdy



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - D	2x4 DRY	No.2	SPF
D - F	2x4 DRY	No.2	SPF
B - E	2x6 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	5.0	6.0	2.50	0.50
C	TTWW-m	MT20	5.0	6.0	1.75	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMBH1-m	MT20	5.0	6.0	2.50	0.50
G	BMWW-t	MT20	5.0	6.0		
H	BMW-t	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	INPUT BRG	RECORD BRG	HEEL WEDGE
B	1107	0	1107	0	5-8	5-8	2x4 L
E	1124	0	1124	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	781	523 / 0	0 / 0	0 / 0	0 / 0	258 / 0	0 / 0
E	793	534 / 0	0 / 0	0 / 0	0 / 0	259 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 24	-91.8 -91.8	0.13 (1)	H-C	0 / 120	0.05 (4)	
B-J	-1684 / 0	-91.8 -91.8	0.14 (1)	C-G	-33 / 0	0.01 (1)	
J-C	-1245 / 0	-91.8 -91.8	0.13 (1)	G-D	0 / 124	0.05 (4)	
C-M	-983 / 0	-139.5 -139.5	0.73 (1)	I-J	0 / 533	0.00 (1)	
M-N	-983 / 0	-139.5 -139.5	0.73 (1)	K-L	0 / 438	0.00 (1)	
N-D	-983 / 0	-139.5 -139.5	0.73 (1)				
D-O	-1201 / 0	-91.8 -91.8	0.14 (1)				
O-L	-1201 / 0	-91.8 -91.8	0.15 (1)				
L-E	-1608 / 0	-91.8 -91.8	0.15 (1)				
E-F	0 / 24	-91.8 -91.8	0.13 (1)				
B-I	0 / 1009	-28.1 -28.1	0.25 (1)				
I-H	0 / 1009	-28.1 -28.1	0.25 (1)				
H-P	0 / 1014	-28.1 -28.1	0.23 (1)				
P-Q	0 / 1014	-28.1 -28.1	0.23 (1)				
Q-G	0 / 1014	-28.1 -28.1	0.23 (1)				
G-R	0 / 977	-28.1 -28.1	0.22 (1)				
R-K	0 / 977	-28.1 -28.1	0.22 (1)				
K-E	0 / 977	-28.1 -28.1	0.22 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-11-4	-90	-90		FRONT	VERT	TOTAL		C1
C	1-11-4	-96	-96		BACK	VERT	TOTAL		C1
D	6-4-12	-90	-90		FRONT	VERT	TOTAL		C1
H	1-11-12	-51	-51		BACK	VERT	TOTAL		C1
M	2-11-12	-89	-89		BACK	VERT	TOTAL		C1
N	4-11-12	-89	-89		BACK	VERT	TOTAL		C1
O	6-11-12	-109	-109		BACK	VERT	TOTAL		C1
P	2-11-12	-51	-51		BACK	VERT	TOTAL		C1
Q	4-11-12	-51	-51		BACK	VERT	TOTAL		C1
R	6-11-12	-51	-51		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 36 = 72 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CPrimeHip
 SIDE SETBACK = 1-11-4
 END SETBACK = 4-6-8
 END WALL WIDTH = 5-8
 CORNER FRAMING TYPE: CONVENTIONAL
 END JACK TYPE: CONVENTIONAL
 APPLIED TO FRONT SIDE
 -ADDT'L LOADS BASED ON 55 % OF GSL

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
 ALLOWABLE DEFL.(TL) = L/360 (0.28")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.73/1.00 (C-D:1), BC=0.25/1.00 (B-I:1)
 WB=0.05/1.00 (D-G:4), SSI=0.35/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

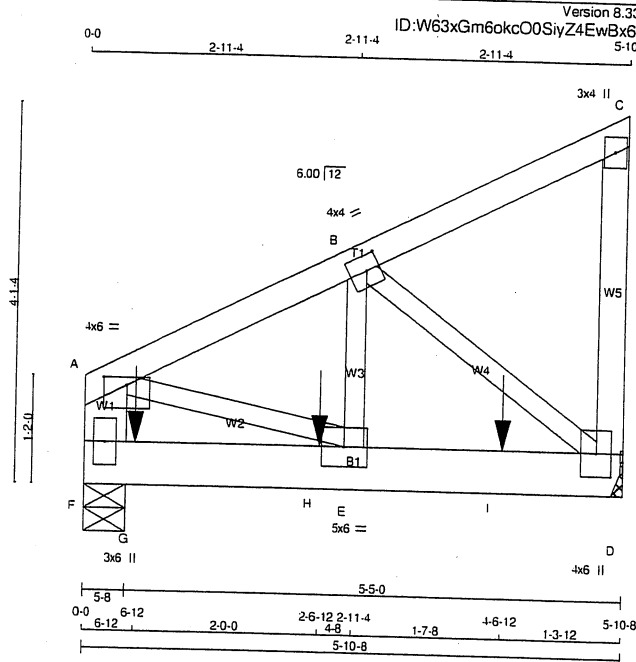
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.86 (D) (INPUT = 0.90)
 JSI METAL = 0.28 (B) (INPUT = 1.00)



Structural component only
 DWG# T-2017160

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410146	T13	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A 2	12	TOP
A - C 1	12	TOP
C - D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TMV-p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	UPLIFT	IN-SX
F 1768	0	0	5-8
D 1439	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
F 1244	853 / 0 0 / 0 0 / 0 390 0 0 0
D 1012	694 / 0 0 / 0 0 / 0 318 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS				
MAX. FACTORED		FACTORED	MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. FACTORED	
FR-TO		FROM TO	LENGTH	FR-TO		CS1 (LC)	
F-A	-1047 / 0	0.0	0.0 0.04 (1)	7.81	A-E	0 1268	0.16 (1)
A-B	-1360 / 0	-91.8	-91.8 0.06 (1)	6.25	E-B	0 1232	0.15 (1)
B-C	-11 / 0	-91.8	-91.8 0.05 (1)	6.25	B-D	-1542 0	0.18 (1)
D-C	-111 / 0	0.0	0.0 0.01 (1)	7.81			
F-G	0 / 0	-18.5	-18.5 0.15 (1)	10.00			
G-H	0 / 0	-18.5	-18.5 0.15 (1)	10.00			
H-E	0 / 0	-18.5	-18.5 0.15 (1)	10.00			
E-I	0 1226	-18.5	-18.5 0.26 (1)	10.00			
I-D	0 1226	-18.5	-18.5 0.26 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	6-12	-856	-856	---	TOP	VERT	TOTAL	---	C1
H	2-6-12	-852	-852	---	TOP	VERT	TOTAL	---	C1
I	4-6-12	-852	-852	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.06/1.00 (A-B:1), BC=0.26/1.00 (D-E:1), WB=0.18/1.00 (B-D:1), SSI=0.35/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

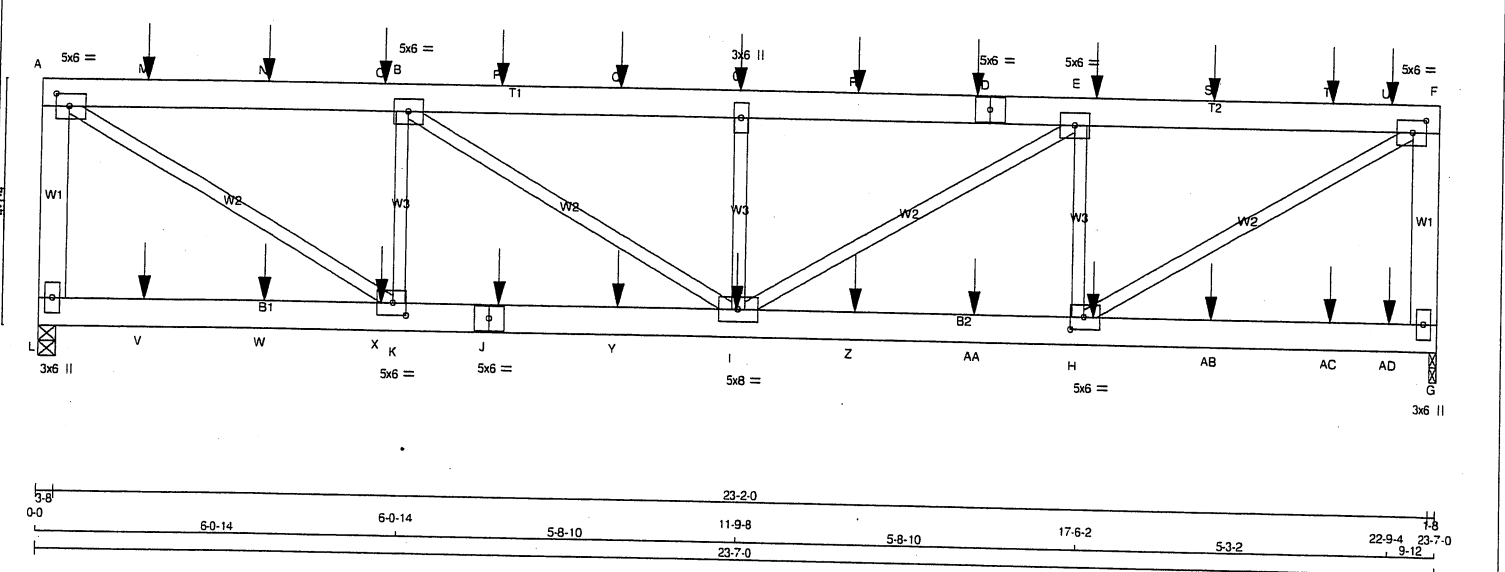
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.80 (B) (INPUT = 0.90)
JSI METAL= 0.28 (D) (INPUT = 1.00)



Structural component only
DWG# T-2017161

JOB NAME 410143	TRUSS NAME T20	QUANTITY 2	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.330 S May 6 2020 MiTek Industries, Inc. Tue Aug 4 13:08:55 2020 Page 1 ID:W63xGm6okc00SiyZ4EwBx6zA2m0-QLKIQEE3899AuQBkP3x_DWJqkuQ8miX8zhAyrEHM 22-9-4 23-7-0 22-9-4 23-7-0 9-12 Scale = 1:36.7	



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
L - A	2x6	DRY	No.2
A - D	2x6	DRY	No.2
D - F	2x6	DRY	No.2
G - F	2x6	DRY	No.2
L - J	2x6	DRY	No.2
J - G	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
L - A	2	12
A - D	2	12
D - F	2	12
F - G	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L - J	2	12
J - G	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMWV-t	MT20	5.0	6.0	2.50 2.75
B	TMWV-t	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		RECORD BRG	
JT	VERT	JT	HORZ	JT	IN-SX	JT	IN-SX
L	2060	0	2060	0	3-8	3-8	
G	2193	0	2193	0	1-8	1-8	

UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS	
JT	COMBINED	JT	COMBINED
L	1459	L	945.0
G	1553	G	1006.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
L-A	-1981.0	H-F	0.3246
A-M	-2758.0	A-K	0.3216
M-N	-2758.0	H-E	-1462.0
N-O	-2758.0	K-B	-1451.0
O-B	-2758.0	I-E	0.840
B-P	-3491.0	B-I	0.870
P-Q	-3491.0	I-C	-758.0
Q-C	-3491.0		
C-R	-3491.0		
R-D	-3491.0		
D-E	-3491.0		
E-S	-2783.0		
S-T	-2783.0		
T-U	-2783.0		
U-F	-2783.0		
F-G	-2092.0		

L-V		V-W	
L-V	0.0	V-W	0.0
V-W	0.0	W-X	0.0
W-X	0.0	X-K	0.0
X-K	0.0	K-J	0.2758
K-J	0.2758	J-Y	0.2758
J-Y	0.2758	Y-I	0.2758
Y-I	0.2758	I-Z	0.2783
I-Z	0.2783	Z-AA	0.2783
Z-AA	0.2783	AA-H	0.2783
AA-H	0.2783	H-AB	0.0
H-AB	0.0	AB-AC	0.0
AB-AC	0.0	AC-AD	0.0
AC-AD	0.0	AD-G	0.0

FACTORED CONCENTRATED LOADS (LBS)		FACE		DIR.		TYPE		HEEL		CONN.	
JT	LOC.	JT	LOC.	JT	LOC.	JT	LOC.	JT	LOC.	JT	LOC.
C	11-9-4	C	11-9-4	C	11-9-4	C	11-9-4	C	11-9-4	C	11-9-4
D	15-9-4	D	15-9-4	D	15-9-4	D	15-9-4	D	15-9-4	D	15-9-4
E	17-9-4	E	17-9-4	E	17-9-4	E	17-9-4	E	17-9-4	E	17-9-4
H	17-9-4	H	17-9-4	H	17-9-4	H	17-9-4	H	17-9-4	H	17-9-4
I	11-9-4	I	11-9-4	I	11-9-4	I	11-9-4	I	11-9-4	I	11-9-4
J	7-9-4	J	7-9-4	J	7-9-4	J	7-9-4	J	7-9-4	J	7-9-4
M	1-9-4	M	1-9-4	M	1-9-4	M	1-9-4	M	1-9-4	M	1-9-4
N	3-9-4	N	3-9-4	N	3-9-4	N	3-9-4	N	3-9-4	N	3-9-4
O	5-9-4	O	5-9-4	O	5-9-4	O	5-9-4	O	5-9-4	O	5-9-4

TOTAL WEIGHT = 4 X 124 = 498 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.20/1.00 (C-E:1), BC=0.22/1.00 (H-I:1), WB=0.40/1.00 (F-H:1), SS=0.19/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (K) (INPUT = 0.90)
JSI METAL= 0.32 (F) (INPUT = 1.00)



Structural component only
DWG# T-2017143 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410143	T20	2	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.330 S May 6 2020 MiTek Industries, Inc. Tue Aug 4 13:08:55 2020 Page 2
ID:W63xGm6okcO0SivZ4EwBx6zA2m0-QLKIQEE3899AuQBkeP3x DWJgkuQ8miX8tzhAvrEHM

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMW+w	MT20	3.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TMW-t	MT20	5.0	6.0		
F	TMW-t	MT20	5.0	6.0	2.50	2.75
G	BMV1+p	MT20	3.0	6.0		
H	BMW-t	MT20	5.0	6.0	2.50	2.75
I	BMW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMW-t	MT20	5.0	6.0	2.50	2.75
L	BMV1+p	MT20	3.0	6.0		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	7-9-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
O	9-9-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
R	13-9-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
S	19-9-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
T	21-9-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
U	22-9-4	-119	-119	---	FRONT	VERT	TOTAL	---	C1
V	1-9-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
W	3-9-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
X	5-9-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Y	9-9-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Z	13-9-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AA	15-9-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AB	19-9-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AC	21-9-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AD	22-9-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

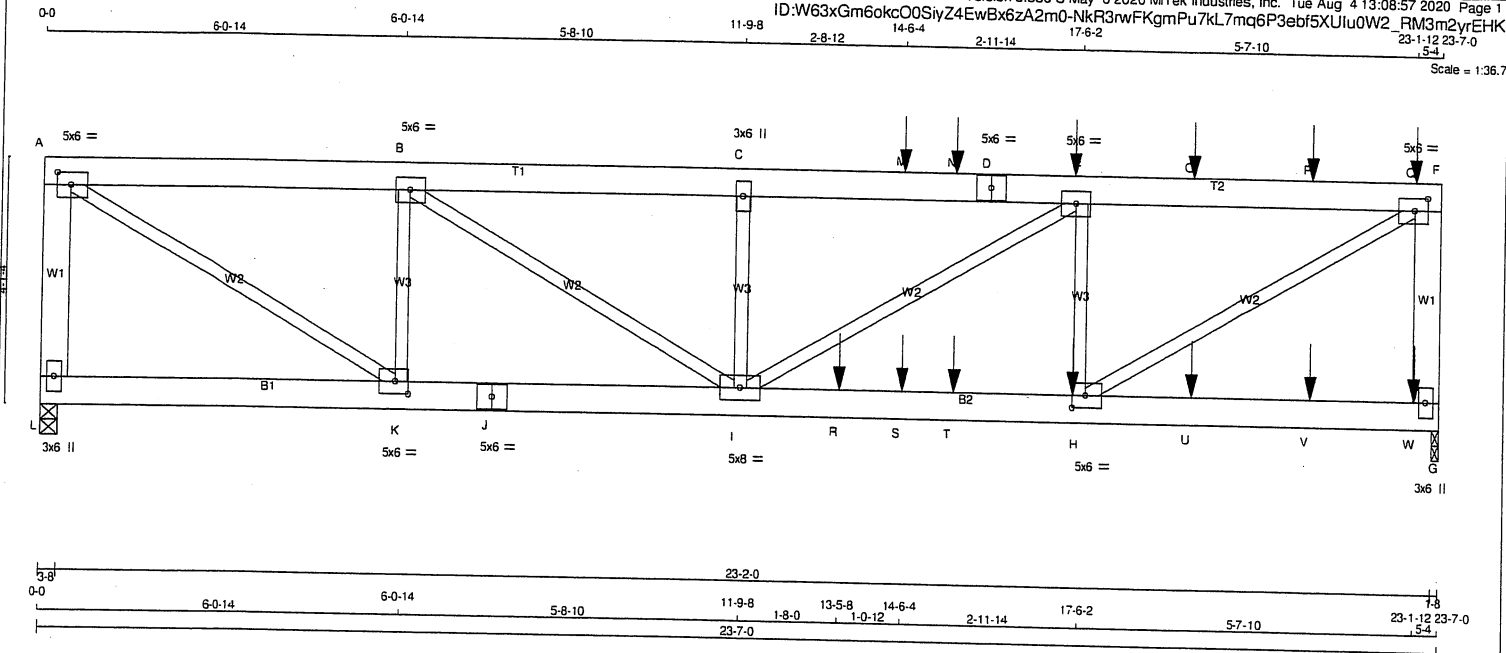


Structural component only
DWG# T-2017143 32

JOB NAME 410143	TRUSS NAME T20Z	QUANTITY 2	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.330 S May 6 2020 MTek Industries, Inc. Tue Aug 4 13:08:57 2020 Page 1
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23-1-12 23-7-0
Scale = 1:38.7



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.
L - A 2x6 DRY No.2 SPF
A - D 2x6 DRY No.2 SPF
D - F 2x6 DRY No.2 SPF
G - F 2x6 DRY No.2 SPF
L - J 2x6 DRY No.2 SPF
J - G 2x6 DRY No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:
CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS
L-A 2 12 TOP
A-D 2 12 SIDE(0.0)
D-F 2 12 SIDE(183.1)
F-G 2 12 TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS
L-J 2 12 TOP
J-G 2 12 SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS
2x3 1 6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.
SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
A TMWW-1 MT20 5.0 6.0 2.50 2.75
B TMWW-1 MT20 5.0 6.0

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF
SPACING = 24.0 IN. C/C
LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015
THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014
(55 % OF 31.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")
CSI: TC=0.22/1.00 (C-E:1), BC=0.60/1.00 (H-I:1), WB=0.51/1.00 (F-H:1), SSI=0.30/1.00 (H-I:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.42 (J) (INPUT = 1.00)

FACTORED CONCENTRATED LOADS (LBS)
JT LOC. LC1 MAX. MAX+ FACE DIR. TYPE HEEL CONN.
E 17-4-12 -110 -110 --- BACK VERT TOTAL --- C1
H 17-4-12 -26 -26 --- BACK VERT TOTAL --- C1
M 14-6-4 -110 -110 --- BACK VERT TOTAL --- C1
N 15-4-12 -110 -110 --- BACK VERT TOTAL --- C1
O 19-4-12 -110 -110 --- BACK VERT TOTAL --- C1
P 21-4-12 -110 -110 --- BACK VERT TOTAL --- C1
Q 23-1-12 -136 -136 --- BACK VERT TOTAL --- C1
R 13-5-8 -1134 -1134 --- BACK VERT TOTAL --- C1
S 14-6-4 -26 -26 --- BACK VERT TOTAL --- C1
T 15-4-12 -26 -26 --- BACK VERT TOTAL --- C1
U 19-4-12 -26 -26 --- BACK VERT TOTAL --- C1
V 21-4-12 -26 -26 --- BACK VERT TOTAL --- C1
W 23-1-12 -32 -32 --- BACK VERT TOTAL --- C1

FACTORED REACTIONS
1ST LOASE MAX./MIN. COMPONENT REACTIONS
JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL
L 1387 916 0 0 0 0 0 0
G 1855 1219 0 0 0 0 636 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

CHORDS
MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. MAX+ FACE DIR. TYPE HEEL CONN.
L-A -1890 0 0.0 0.0 0.14 (1) 7.81 H-F 0 4150 0.51 (1)
A-B -2763 0 -91.8 -91.8 0.12 (1) 6.25 A-K 0 3223 0.40 (1)
B-C -4415 0 -91.8 -91.8 0.14 (1) 5.35 H-E -1581 0 0.19 (1)
C-M -4415 0 -91.8 -91.8 0.22 (1) 5.26 K-B -1664 0 0.20 (1)
M-N -4415 0 -91.8 -91.8 0.22 (1) 5.26 I-E 0 1016 0.13 (1)
N-D -4415 0 -91.8 -91.8 0.22 (1) 5.26 B-I 0 1959 0.24 (1)
D-E -4415 0 -91.8 -91.8 0.22 (1) 5.26 I-C -544 0 0.07 (1)
E-O -3558 0 -91.8 -91.8 0.21 (1) 5.72
O-P -3558 0 -91.8 -91.8 0.21 (1) 5.72
P-Q -3558 0 -91.8 -91.8 0.21 (1) 5.72
Q-F -3558 0 -91.8 -91.8 0.21 (1) 5.72
G-F -2567 0 0.0 0.0 0.19 (1) 7.81

Continued on page 2

JOB NAME 410143	TRUSS NAME T20Z	QUANTITY 2	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.330 S May 6 2020 MiTek Industries, Inc. Tue Aug 4 13:08:57 2020 Page 2 ID:W63xGm6okcQ0SiyZ4EwBx6zA2m0-NkR3nwFKgmPu7kL7mq6P3ebf5XUlu0V2 RM3m2yrEHK	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMW+w	MT20	3.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TMVW-t	MT20	5.0	6.0	2.50	2.75
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	6.0	2.50	2.75
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMV1+p	MT20	3.0	6.0		

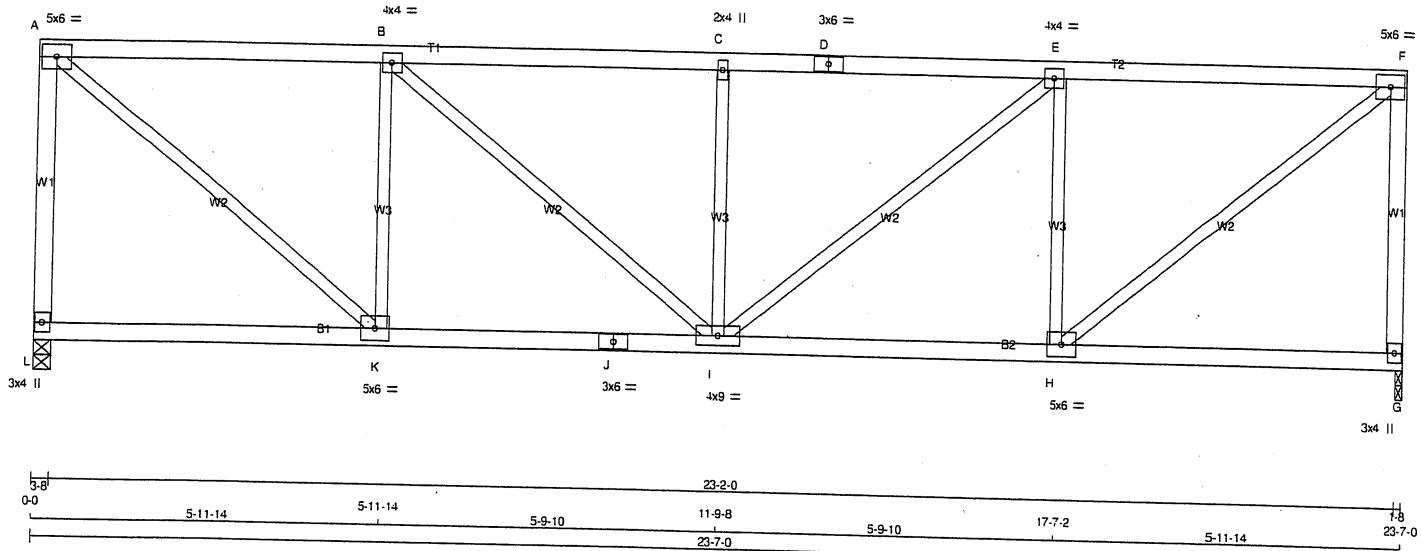
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410143	T21	4	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

Version 8.330 S May 6 2020 Mitek Industries, Inc. Tue Aug 4 13:08:58 2020 Page 1
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23-7-0
Scale = 1:37.5



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
L - A	2x4 DRY	No.2	SPF
A - D	2x4 DRY	No.2	SPF
D - F	2x4 DRY	No.2	SPF
G - J	2x4 DRY	No.2	SPF
J - G	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW-t	MT20	5.0	6.0
B	TMVW-t	MT20	4.0	4.0
C	TMVW-t	MT20	2.0	4.0
D	TS-t	MT20	3.0	6.0
E	TMVW-t	MT20	4.0	4.0
F	TMVW-t	MT20	5.0	6.0
G	BMV1+p	MT20	3.0	4.0
H	BMVW-t	MT20	5.0	6.0
I	BMVW-t	MT20	4.0	9.0
J	SS-t	MT20	3.0	6.0
K	BMVW-t	MT20	5.0	6.0
L	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
L	1300 0	1300 0	0 0	3-8	3-8
G	1300 0	1300 0	0 0	1-8	1-8

UNFACTORED REACTIONS		1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE PERM.LIVE WIND DEAD SOIL
L	919	603 / 0	0 / 0 0 / 0 0 / 0 316 0 0 0
G	919	603 / 0	0 / 0 0 / 0 0 / 0 316 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. FACTORED (LBS)	MEMB.	FORCE (LBS)	MAX. FACTORED (LBS)	MAX. FACTORED (LBS)
L-A	-1255 / 0	0.0	0.0 0.55 (1)	H-F	0 / 1654	0.37 (1)	0.37 (1)
A-B	-1290 / 0	-91.8	-91.8 0.53 (1)	A-K	0 / 1654	0.37 (1)	0.37 (1)
B-C	-1642 / 0	-91.8	-91.8 0.56 (1)	H-E	-912 / 0	0.35 (1)	0.35 (1)
C-D	-1642 / 0	-91.8	-91.8 0.56 (1)	K-B	-912 / 0	0.35 (1)	0.35 (1)
D-E	-1642 / 0	-91.8	-91.8 0.56 (1)	I-E	0 / 457	0.10 (1)	0.10 (1)
E-F	-1290 / 0	-91.8	-91.8 0.53 (1)	B-I	0 / 457	0.10 (1)	0.10 (1)
G-F	-1255 / 0	0.0	0.0 0.55 (1)	I-C	-487 / 0	0.19 (1)	0.19 (1)
L-K	0 / 0	-18.5	-18.5 0.16 (4)				
K-J	0 / 1290	-18.5	-18.5 0.29 (1)				
J-I	0 / 1290	-18.5	-18.5 0.29 (1)				
I-H	0 / 1290	-18.5	-18.5 0.29 (1)				
H-G	0 / 0	-18.5	-18.5 0.16 (4)				

TOTAL WEIGHT = 4 X 99 = 394 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.56/1.00 (B-C:1), BC=0.29/1.00 (I-K:1), WB=0.37/1.00 (F-H:1), SSI=0.26/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

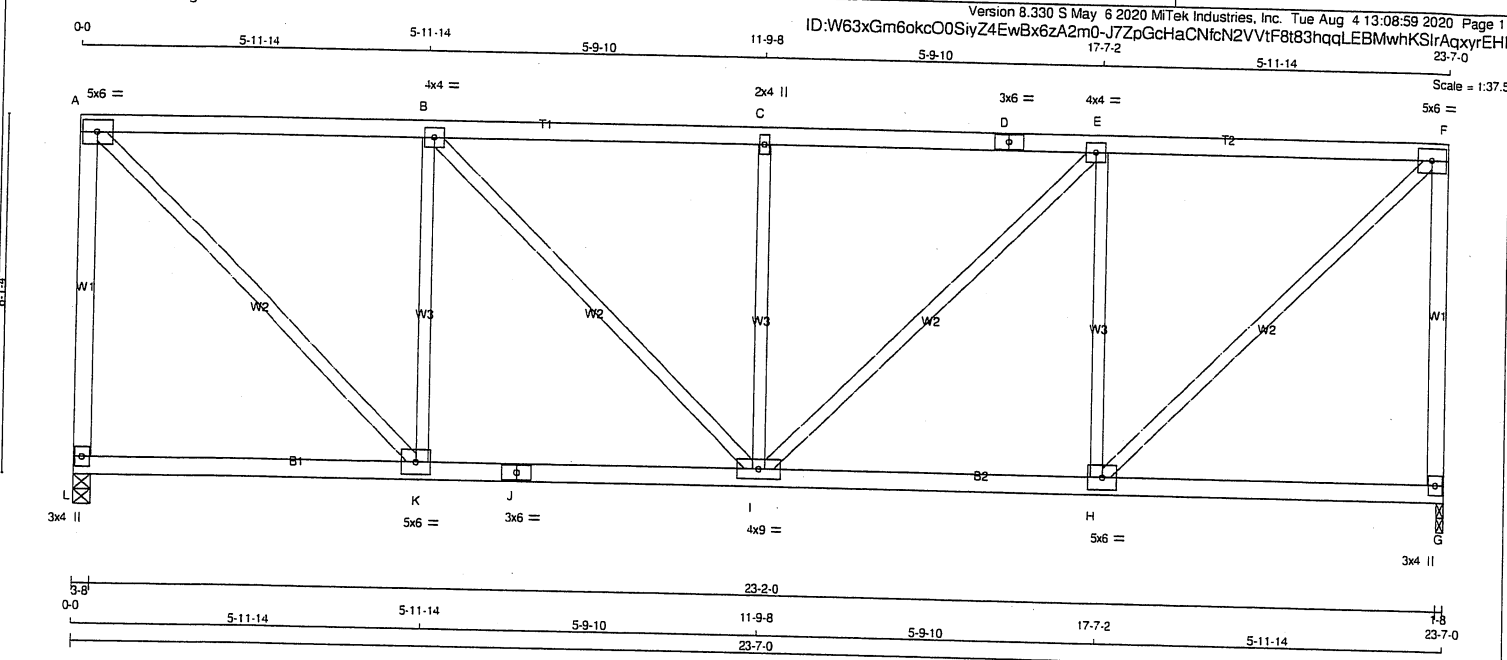
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (F) (INPUT = 0.90)
JSI METAL= 0.43 (J) (INPUT = 1.00)



Structural component only
DWG# T-2017145

JOB NAME 410143	TRUSS NAME T22	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
L - A	2x4	DRY	No.2
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW-t	MT20	5.0	6.0
B	TMVW-t	MT20	4.0	4.0
C	TMVW-w	MT20	2.0	4.0
D	TS-t	MT20	3.0	6.0
E	TMVW-t	MT20	4.0	4.0
F	TMVW-t	MT20	5.0	6.0
G	BMV1+p	MT20	3.0	4.0
H	BMVW-t	MT20	5.0	6.0
I	BMVW-t	MT20	4.0	9.0
J	BS-t	MT20	3.0	6.0
K	BMVW-t	MT20	5.0	6.0
L	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
L	1300	0	1300	0	3-8	IN-SX
G	1300	0	1300	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. LIVE		PERM. LIVE	WIND	DEAD	SOIL
	SNOW	SNOW	SNOW	SNOW				
L	919	603 / 0	0 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
G	919	603 / 0	0 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	WEBS		MAX. UNBRACED LENGTH
	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)	MAX. (LC)		FORCE (LBS)	MAX. (LC)	
FR-TO		FROM	TO		FR-TO			
L-A	-1255 / 0	0.0	0.0	0.91 (1)	7.17	H-F	0 / 1489	0.34 (1)
A-B	-1068 / 0	-91.8	-91.8	0.52 (1)	5.35	A-K	0 / 1489	0.34 (1)
B-C	-1360 / 0	-91.8	-91.8	0.54 (1)	4.85	H-E	-912 / 0	0.53 (1)
C-D	-1360 / 0	-91.8	-91.8	0.54 (1)	4.85	K-B	-912 / 0	0.53 (1)
D-E	-1360 / 0	-91.8	-91.8	0.54 (1)	4.85	I-E	0 / 412	0.09 (1)
E-F	-1068 / 0	-91.8	-91.8	0.52 (1)	5.35	B-I	0 / 412	0.09 (1)
G-F	-1255 / 0	0.0	0.0	0.91 (1)	7.17	I-C	-487 / 0	0.28 (1)
L-K	0 / 0	-18.5	-18.5	0.16 (4)	10.00			
K-J	0 / 1068	-18.5	-18.5	0.26 (1)	10.00			
J-I	0 / 1068	-18.5	-18.5	0.26 (1)	10.00			
I-H	0 / 1068	-18.5	-18.5	0.26 (1)	10.00			
H-G	0 / 0	-18.5	-18.5	0.16 (4)	10.00			

TOTAL WEIGHT = 4 X 105 = 422 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.91/1.00 (A-L:1), BC=0.26/1.00 (H-I:1),
WB=0.53/1.00 (E-H:1), SSI=0.26/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

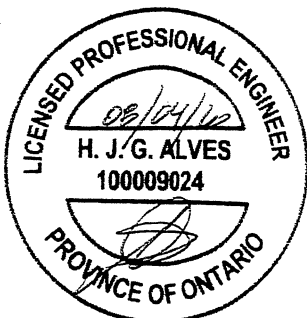
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

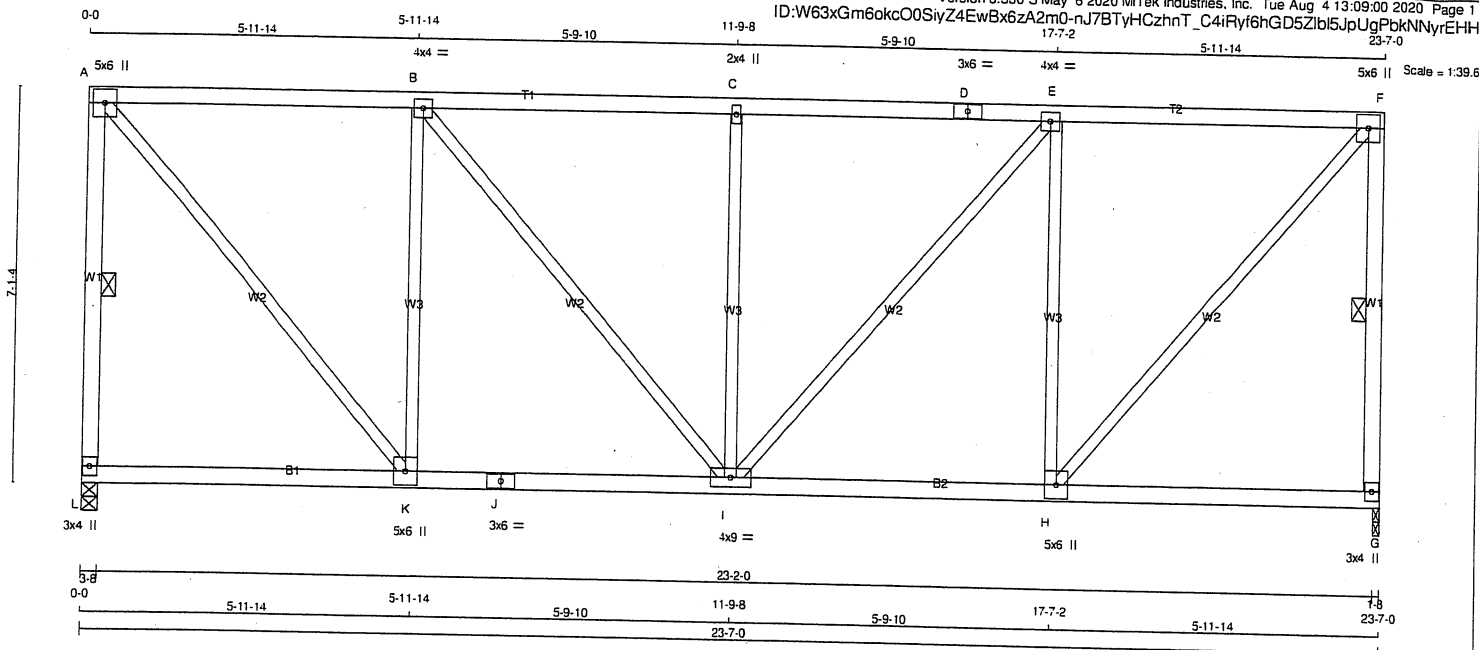
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (H) (INPUT = 0.90)
JSI METAL= 0.32 (J) (INPUT = 1.00)



Structural component only
DWG# T-2017146

JOB NAME 410143	TRUSS NAME T23	QUANTITY 18	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.330 S May 6 2020 MiTek Industries, Inc. Tue Aug 4 13:09:00 2020 Page 1 ID:W63xGm6okcO0SiyZ4EwBx6zA2m0-nJ7BTyHCzhnT_C4iRyf6hGD5Zib5JpUgPbkNNyrEHH	



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
L - A	2x4	DRY	No.2
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - J	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	5.0	6.0	
B	TMVW-t	MT20	4.0	4.0	
C	TMVW-w	MT20	2.0	4.0	
D	TS-t	MT20	3.0	6.0	
E	TMVW-t	MT20	4.0	4.0	
F	TMVW+p	MT20	5.0	6.0	
G	BMV1+p	MT20	3.0	4.0	
H	BMVW+t	MT20	5.0	6.0	
I	BMVW-t	MT20	4.0	9.0	
J	BS-t	MT20	3.0	6.0	
K	BMVW+t	MT20	5.0	6.0	
L	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
L	1300	0	0	1300	0	0	0	3-8	3-8
G	1300	0	0	1300	0	0	0	1-8	1-8

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
L	919	603 / 0	0 / 0	0 / 0	0 / 0
G	919	603 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-L, F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING		TOTAL LOAD CASES: (4)	
CHORDS		MEMB.	
FR-TO	MEMB.	FR-TO	MEMB.
L-A	-1255 / 0	L-A	-1255 / 0
A-B	-912 / 0	A-B	-912 / 0
B-C	-1180 / 0	B-C	-1180 / 0
C-D	-1180 / 0	C-D	-1180 / 0
D-E	-1180 / 0	D-E	-1180 / 0
E-F	-912 / 0	E-F	-912 / 0
F-G	-1255 / 0	F-G	-1255 / 0
L-K	0 / 0	L-K	0 / 0
K-J	0 / 912	K-J	0 / 912
J-I	0 / 912	J-I	0 / 912
I-H	0 / 912	I-H	0 / 912
H-G	0 / 0	H-G	0 / 0

TOTAL WEIGHT = 18 X 113 = 2034 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.52/1.00 (B-C:1), BC=0.24/1.00 (H-I:4), WB=0.80/1.00 (E-H:1), SS=0.26/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES		PLATE GRIP(DRY) SHEAR		SECTION	
(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MT20	650	371	1747	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

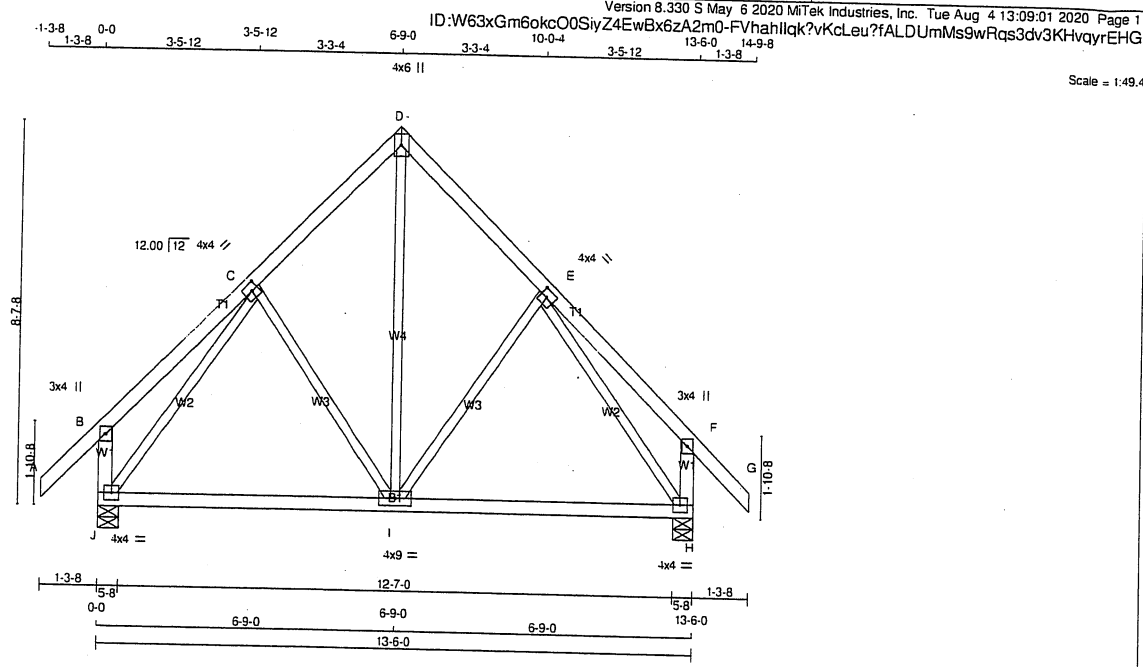
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (H) (INPUT = 0.90)
JSI METAL= 0.33 (A) (INPUT = 1.00)



Structural component only
DWG# T-2017147

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410143	T24	6	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
J	872	0	872	0	5-8	5-8
H	872	0	872	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX / MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
J	614	417.0	0/0	0/0	0/0	198.0	0/0
H	614	417.0	0/0	0/0	0/0	198.0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (CSI) (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (LC)	MAX. FACTORED UNBRACED LENGTH (LC)
FR-TO				FR-TO			
A-B	0.45	-91.8	-91.8 0.13 (1)	I-D	0.387	10.00	0.09 (1)
B-C	0.25	-91.8	-91.8 0.17 (1)	I-E	-143.0	10.00	0.09 (1)
C-D	-471.0	-91.8	-91.8 0.13 (1)	C-I	-143.0	6.25	0.09 (1)
D-E	-471.0	-91.8	-91.8 0.13 (1)	J-C	-703.0	6.25	0.41 (1)
E-F	0.25	-91.8	-91.8 0.17 (1)	E-H	-703.0	10.00	0.41 (1)
F-G	0.45	-91.8	-91.8 0.13 (1)				
J-B	-245.0	0.0	0.0 0.03 (1)				
H-F	-245.0	0.0	0.0 0.03 (1)				
J-I	0.398	-18.5	-18.5 0.27 (4)				
I-H	0.398	-18.5	-18.5 0.27 (4)				

TOTAL WEIGHT = 6 X 70 = 420 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.17/1.00 (E-F:1), BC=0.27/1.00 (H-I:4),
WB=0.41/1.00 (C-J:1), SSI=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
			1987

PLATE PLACEMENT TOL. = 0.250 inches

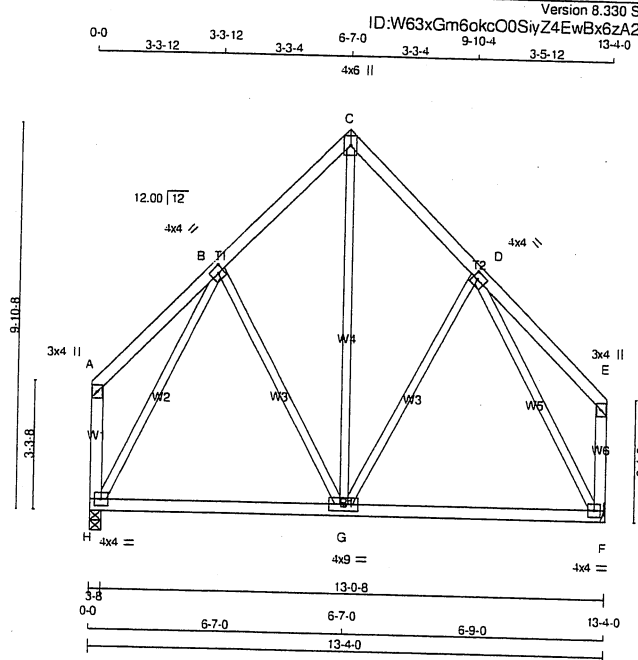
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (E) (INPUT = 0.90)
JSI METAL= 0.24 (C) (INPUT = 1.00)



Structural component only
DWG# T-2017148

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410143	T25	6	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - A	2x4	DRY	No.2
F - E	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TTW+p	MT20	4.0	6.0		
D	TMVW-t	MT20	4.0	4.0	2.00	1.50
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMVW1-t	MT20	4.0	9.0		
H	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
H	735	0	3-8	3-8
F	735	0	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	520	341 / 0	0 / 0	0 / 0	0 / 0	179 / 0	0 / 0
F	520	341 / 0	0 / 0	0 / 0	0 / 0	179 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO			
A-B	0 / 25	-91.8	-91.8	0.16 (1)	B-G	-72 / 13	0.07 (1)	10.00
B-C	-402 / 0	-91.8	-91.8	0.12 (1)	G-C	0 / 299	0.07 (1)	6.25
C-D	-404 / 0	-91.8	-91.8	0.13 (1)	G-D	-94 / 7	0.09 (1)	6.25
D-E	0 / 25	-91.8	-91.8	0.17 (1)	H-B	-650 / 0	0.57 (1)	10.00
H-A	-113 / 0	0.0	0.0	0.02 (1)	D-F	-646 / 0	0.58 (1)	7.81
F-E	-121 / 0	0.0	0.0	0.02 (1)				7.81
H-G	0 / 302	-18.5	-18.5	0.26 (4)				10.00
G-F	0 / 312	-18.5	-18.5	0.26 (4)				10.00

TOTAL WEIGHT = 6 X 73 = 437 b

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.17/1.00 (D-E:1), BC=0.26/1.00 (F-G:4), WB=0.58/1.00 (D-F:1), SSI=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

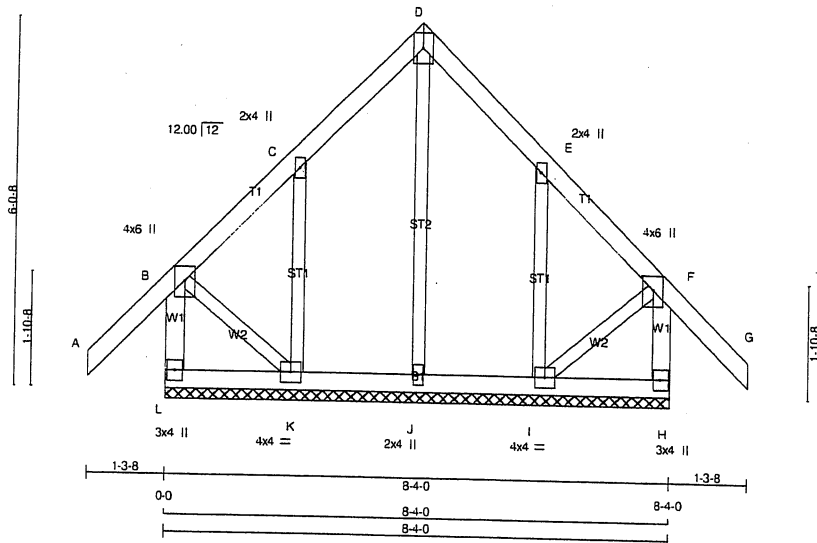
JSI GRIP= 0.67 (B) (INPUT = 0.90)
JSI METAL= 0.22 (D) (INPUT = 1.00)



Structural component only
DWG# T-2017149

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410143	G26	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.330 S May 6 2020 Mittek Industries, Inc. Tue Aug 4 13:08:49 2020 Page 1
ID:W63xGm60kcC0SiyZ4EwBx6zA2m0-cBz1AB9JZJO1AVkaI8yXkyGJVJuX0VYs8CQeUWyrEHS
1-3-8 0-0 4-2-0 4-2-0 3-10-8 8-0-8 8-4-0 9-7-8
4x6 II
Scale = 1:36.0



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.
L - B 2x4 DRY No.2 SPF
A - D 2x4 DRY No.2 SPF
D - G 2x4 DRY No.2 SPF
H - F 2x4 DRY No.2 SPF
L - H 2x4 DRY No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF
ALL GABLE WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.
GABLE STUDS SPACED AT 2-0-0 OC.
TOTAL WEIGHT = 2 X 45 = 90 lb

CHORDS	SIZE	LUMBER	DESCR.
L - B	2x4	DRY No.2	SPF
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
L - H	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
ALL GABLE WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT	2-0-0 OC.		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FACTORED MAX. CSI (LC)
FR-TO		FROM TO		FR-TO			
L-B	-234.0	0.0	0.0 0.03 (1)	J-D	-120.0	7.81	0.06 (1)
A-B	0.45	-91.8	-91.8 0.13 (1)	K-C	-231.0	10.00	0.06 (1)
B-C	-13.0	-91.8	-91.8 0.07 (1)	I-E	-231.0	6.25	0.06 (1)
C-D	-36.0	-91.8	-91.8 0.07 (1)	B-K	0.25	6.25	0.01 (1)
D-E	-36.0	-91.8	-91.8 0.07 (1)	I-F	0.25	6.25	0.01 (1)
E-F	-13.0	-91.8	-91.8 0.07 (1)				
F-G	0.45	-91.8	-91.8 0.13 (1)				
H-F	-234.0	0.0	0.0 0.03 (1)				
L-K	0.0	-18.5	-18.5 0.02 (4)				
K-J	0.14	-18.5	-18.5 0.02 (4)				
J-I	0.14	-18.5	-18.5 0.02 (4)				
I-H	0.0	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF
SPACING = 24.0 IN. C/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015
THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014
(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD
CSI: TC=0.13/1.00 (F-G:1), BC=0.02/1.00 (J-K:4), WB=0.06/1.00 (D-J:1), SSI=0.07/1.00 (F-G:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

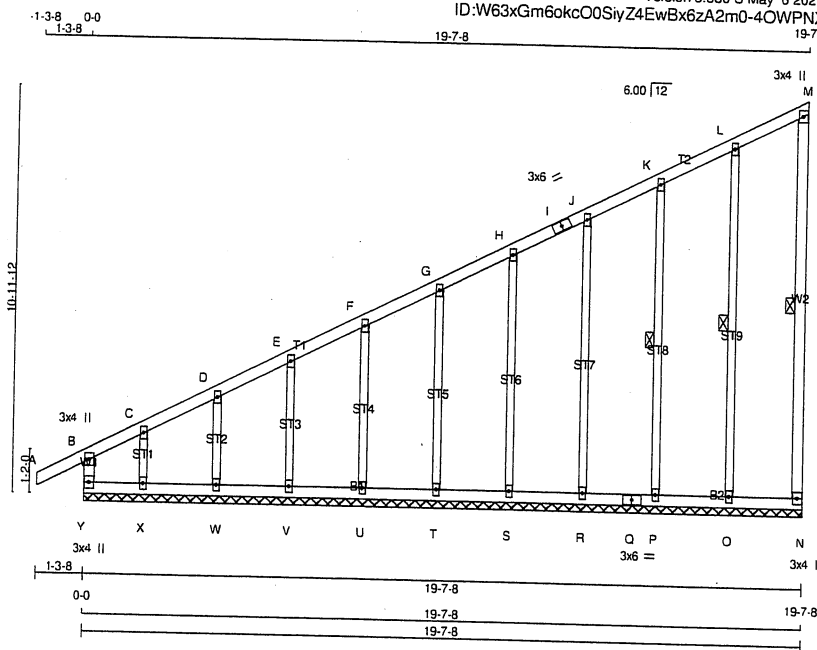
PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.18 (C) (INPUT = 0.90)
JSI METAL= 0.12 (E) (INPUT = 1.00)



Structural component only
DWG# T-2017137

JOB NAME 410143	TRUSS NAME G27	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
Y - B	2x4	DRY	No.2
A - I	2x4	DRY	No.2
I - M	2x4	DRY	No.2
N - M	2x4	DRY	No.2
Y - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2'-0-0 OC.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C, D, E, F, G, H, J, K, L				
C	TMW+w	MT20	2.0	4.0
I	TS-I	MT20	3.0	6.0
M	TMV+p	MT20	3.0	4.0
N	BMV1+p	MT20	3.0	4.0
O, P, R, S, T, U, V, W, X				
O	BMW1+w	MT20	2.0	4.0
Q	BS-I	MT20	3.0	6.0
Y	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF M-N, L-O, K-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
Y-B	-248 / 0	0.0	0.0 0.04 (1)	O-L	-198 / 0	0.13 (1)	
A-B	0 / 28	-91.8	-91.8 0.12 (1)	P-K	-182 / 0	0.10 (1)	
B-C	-45 / 0	-91.8	-91.8 0.10 (1)	R-J	-183 / 0	0.22 (1)	
C-D	-17 / 0	-91.8	-91.8 0.05 (1)	S-H	-183 / 0	0.15 (1)	
D-E	-18 / 0	-91.8	-91.8 0.05 (1)	T-G	-182 / 0	0.10 (1)	
E-F	-13 / 0	-91.8	-91.8 0.04 (1)	U-F	-183 / 0	0.07 (1)	
F-G	-10 / 0	-91.8	-91.8 0.04 (1)	V-E	-179 / 0	0.04 (1)	
G-H	-8 / 0	-91.8	-91.8 0.04 (1)	W-D	-195 / 0	0.03 (1)	
H-I	-6 / 0	-91.8	-91.8 0.04 (1)	X-C	-110 / 0	0.02 (1)	
I-J	-6 / 0	-91.8	-91.8 0.04 (1)				
J-K	-4 / 0	-91.8	-91.8 0.04 (1)				
K-L	-2 / 0	-91.8	-91.8 0.04 (1)				
L-M	-7 / 0	-91.8	-91.8 0.04 (1)				
N-M	-83 / 0	0.0	0.0 0.01 (1)				
Y-X	0 / 24	-18.5	-18.5 0.05 (1)				
X-W	0 / 20	-18.5	-18.5 0.02 (4)				
W-V	0 / 15	-18.5	-18.5 0.02 (4)				
V-U	0 / 12	-18.5	-18.5 0.02 (4)				
U-T	0 / 9	-18.5	-18.5 0.01 (4)				
T-S	0 / 7	-18.5	-18.5 0.01 (4)				
S-R	0 / 5	-18.5	-18.5 0.01 (4)				
R-Q	0 / 4	-18.5	-18.5 0.01 (4)				
Q-P	0 / 4	-18.5	-18.5 0.01 (4)				
P-O	0 / 3	-18.5	-18.5 0.02 (4)				
O-N	0 / 1	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (A-B:1), BC=0.05/1.00 (X-Y:1), WB=0.22/1.00 (J-R:1), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

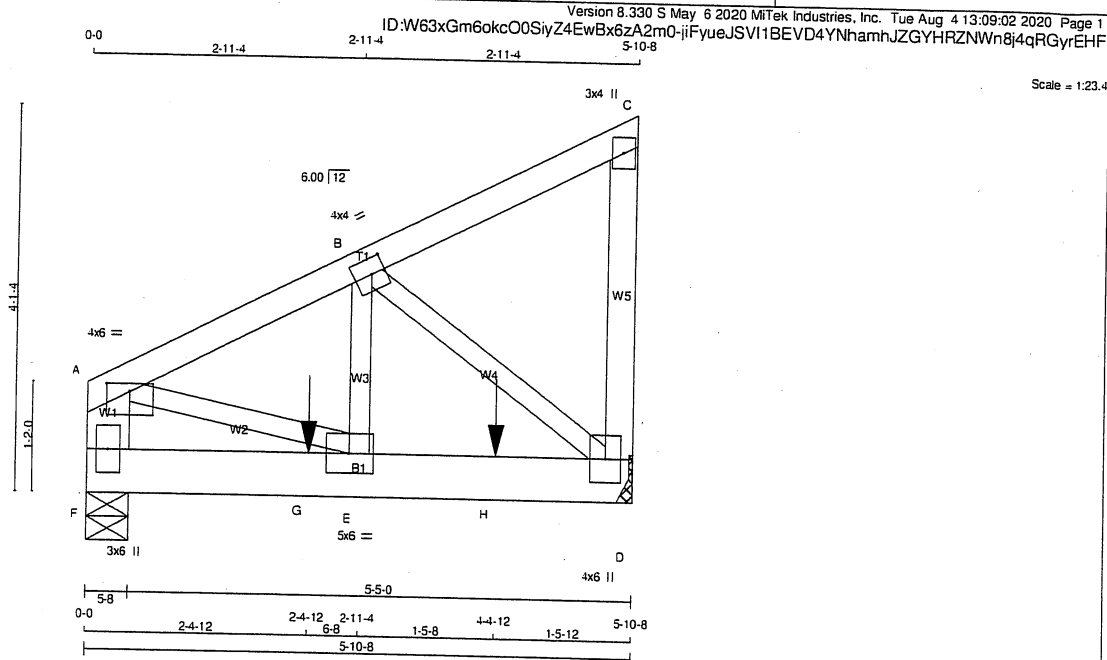
JSI GRIP= 0.38 (C) (INPUT = 0.90)
JSI METAL= 0.08 (L) (INPUT = 1.00)



Structural component only
DWG# T-2017138

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
410143	T28	2	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 2	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00 3.00
B	TMWW-I	MT20	4.0	4.0	2.00 1.75
C	TMV-p	MT20	3.0	4.0	
D	BMVW1+p	MT20	4.0	6.0	
E	BMWW-I	MT20	5.0	6.0	
F	BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	DOWN	UPLIFT	IN-SX
F 929	0	0	5-8
D 1152	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2-0.

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
F 655	438 / 0 0 / 0 0 / 0 0 217 / 0 0 / 0
D 813	545 / 0 0 / 0 0 / 0 0 268 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO		
F-A	-846 / 0	0.0 0.0 0.03 (1)	7.81	A-E	0 998	0.12 (1)	
A-B	-1067 / 0	-91.8 -91.8 0.06 (1)	6.25	E-B	0 898	0.11 (1)	
B-C	-12 / 0	-91.8 -91.8 0.06 (1)	6.25	B-D	-1214 / 0	0.14 (1)	
D-C	-109 / 0	0.0 0.0 0.01 (1)	7.81				
F-G	0 / 0	-18.5 -18.5 0.08 (1)	10.00				
G-E	0 / 0	-18.5 -18.5 0.08 (1)	10.00				
E-H	0 / 965	-18.5 -18.5 0.22 (1)	10.00				
H-D	0 / 965	-18.5 -18.5 0.22 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-4-12	-717	-717	---	FRONT	VERT	TOTAL	---	C1
H	4-4-12	-717	-717	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 29 = 117 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.22/1.00 (D-E:1) , WB=0.14/1.00 (B-D:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

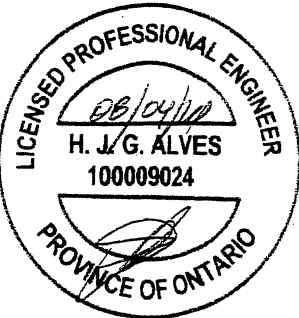
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

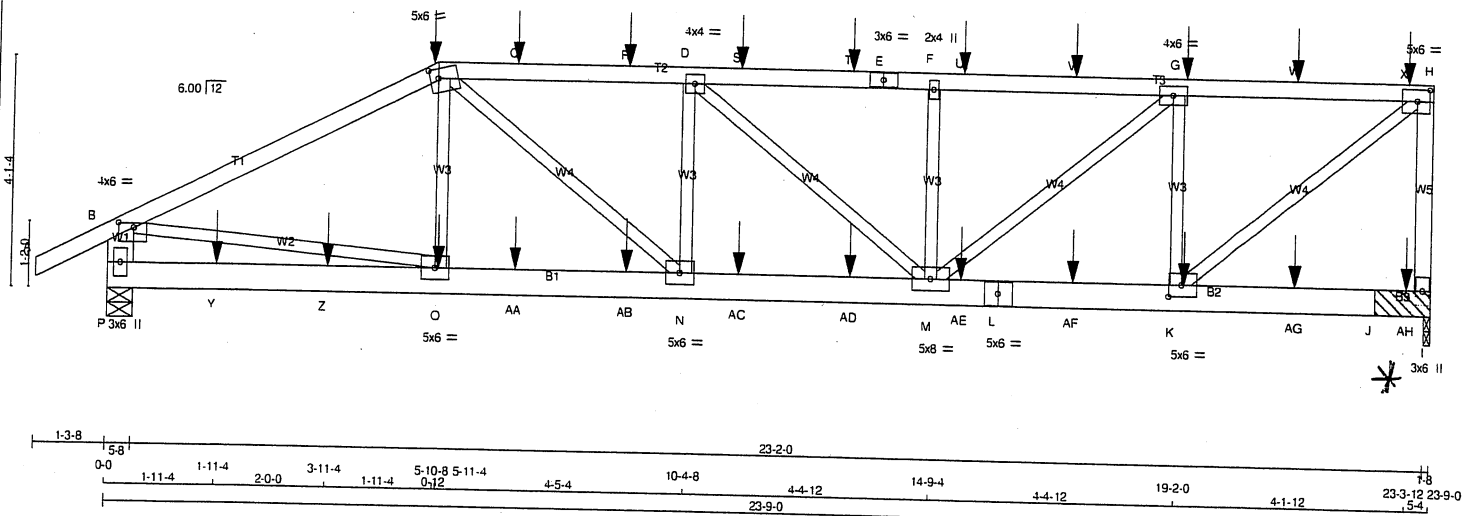
JSI GRIP= 0.58 (B) (INPUT = 0.90)
JSI METAL= 0.22 (D) (INPUT = 1.00)



Structural component only
DWG# T-2017150

JOB NAME 410149	TRUSS NAME T30	QUANTITY 2	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.330 S May 6 2020 Mitek Industries, Inc. Tue Aug 4 13:58:22 2020 Page 1
ID:pqvtrdsyU73tskbsq1clrfEz7lfa-296y8m89QmzBLOWzy1oikuvJ1IW651vTnE5SKiyrDZ?
23-3-12 23-9-0
Scale = 1:39.0



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - H	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
P - B	2x6	DRY No.2	SPF
P - L	2x6	DRY No.2	SPF
L - I	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(61.0)
C-E	12	SIDE(61.0)
E-H	12	SIDE(61.0)
H-I	12	TOP
P-B	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
P-L	12	SIDE(183.1)
L-I	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	GROSS REACTION	DOWN	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
P	2248	0	0	2248	0	0	1-8	1-8	& BLOCK
P	2247	0	0	2247	0	0	5-8	5-8	

UNFACTORED REACTIONS

1ST CASE	MAX./MIN.	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	1591	1034	0	0	0	558	0
P	1589	1041	0	0	0	549	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P

2x6 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. I ATTACHED TO FRONT SIDE WITH 3 ROWS OF (0.122"x3") SPIRAL NAILS SPACED 3" C.C. 12 NAILS TOTAL.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (CSI (LC))	MEMB.	MAX. FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. CSI (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0	-91.8	-91.8 0.07 (1)	O-C	-180	95	0.02 (1)
B-C	-3147	0	-91.8 -91.8 0.40 (1)	B-O	0	2837	0.35 (1)
C-Q	-3610	0	-91.8 -91.8 0.30 (1)	K-H	0	2922	0.36 (1)
Q-R	-3610	0	-91.8 -91.8 0.30 (1)	C-N	0	1041	0.13 (1)
R-D	-3610	0	-91.8 -91.8 0.29 (1)	K-G	-1683	0	0.21 (1)
D-S	-3356	0	-91.8 -91.8 0.29 (1)	N-D	-524	0	0.07 (1)
S-T	-3356	0	-91.8 -91.8 0.29 (1)	M-G	0	1428	0.18 (1)
T-E	-3356	0	-91.8 -91.8 0.29 (1)	D-M	-334	0	0.09 (1)
E-F	-3356	0	-91.8 -91.8 0.29 (1)	M-F	-591	0	0.07 (1)
F-U	-3356	0	-91.8 -91.8 0.29 (1)				
U-V	-3356	0	-91.8 -91.8 0.27 (1)				
V-G	-3356	0	-91.8 -91.8 0.27 (1)				
G-W	-2266	0	-91.8 -91.8 0.25 (1)				
W-X	-2266	0	-91.8 -91.8 0.25 (1)				
X-H	-2266	0	-91.8 -91.8 0.25 (1)				
I-H	-2166	0	0.0 0.0 0.27 (1)				
P-B	-2169	0	0.0 0.0 0.08 (1)				

P-Y	0	0	-18.5	-18.5	0.07 (4)	10.00
Y-Z	0	0	-18.5	-18.5	0.07 (4)	10.00
Z-O	0	0	-18.5	-18.5	0.07 (4)	10.00
O-AA	0	2808	-18.5	-18.5	0.21 (1)	10.00
AA-AB	0	2808	-18.5	-18.5	0.21 (1)	10.00
AB-N	0	2808	-18.5	-18.5	0.21 (1)	10.00
N-AC	0	3611	-18.5	-18.5	0.28 (1)	10.00
AC-AD	0	3611	-18.5	-18.5	0.28 (1)	10.00
AD-M	0	3611	-18.5	-18.5	0.28 (1)	10.00
M-AE	0	2266	-18.5	-18.5	0.18 (1)	10.00
AE-L	0	2266	-18.5	-18.5	0.18 (1)	10.00
L-AF	0	2266	-18.5	-18.5	0.18 (1)	10.00
AF-K	0	2266	-18.5	-18.5	0.18 (1)	10.00
K-AG	0	0	-18.5	-18.5	0.04 (4)	10.00
AG-J	0	0	-18.5	-18.5	0.04 (4)	10.00
J-AH	0	0	-18.5	-18.5	0.04 (4)	10.00
AH-I	0	0	-18.5	-18.5	0.04 (4)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-411	-411	---	FRONT	VERT	TOTAL	---	C1
G	19-3-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1

TOTAL WEIGHT = 4 X 112 = 447 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.40/1.00 (B-C:1), BC=0.28/1.00 (M-N:1), WB=0.39/1.00 (H-K:1), SS=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.34 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
410149	T30	2	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:pqvtrdsyU73tsksq1clrEz7lfa-296v8m89QmzBLOWzy1oikuvJ1IW651vTnE5SKiyRZ?

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TTWW-m	MT20	5.0	6.0	2.00	1.75
D	TMVW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMVW-w	MT20	2.0	4.0		
G	TMVW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0	2.50	2.75
I	BMV1+p	MT20	3.0	6.0		
K	BMVW-t	MT20	5.0	6.0	2.50	2.75
L	BS-t	MT20	5.0	6.0		
M	BMVW-t	MT20	5.0	8.0		
N	BMVW-t	MT20	5.0	6.0		
O	BMVW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	6.0		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
K	19-3-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	5-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Q	7-3-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
R	9-3-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
S	11-3-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
T	13-3-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
U	15-3-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	17-3-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	21-3-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
X	23-3-12	-136	-136	---	FRONT	VERT	TOTAL	---	C1
Y	1-11-4	-25	-26	---	FRONT	VERT	TOTAL	---	C1
Z	3-11-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AA	7-3-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AB	9-3-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AC	11-3-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AD	13-3-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AE	15-3-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AF	17-3-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AG	21-3-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
AH	23-3-12	-32	-32	---	FRONT	VERT	TOTAL	---	C1

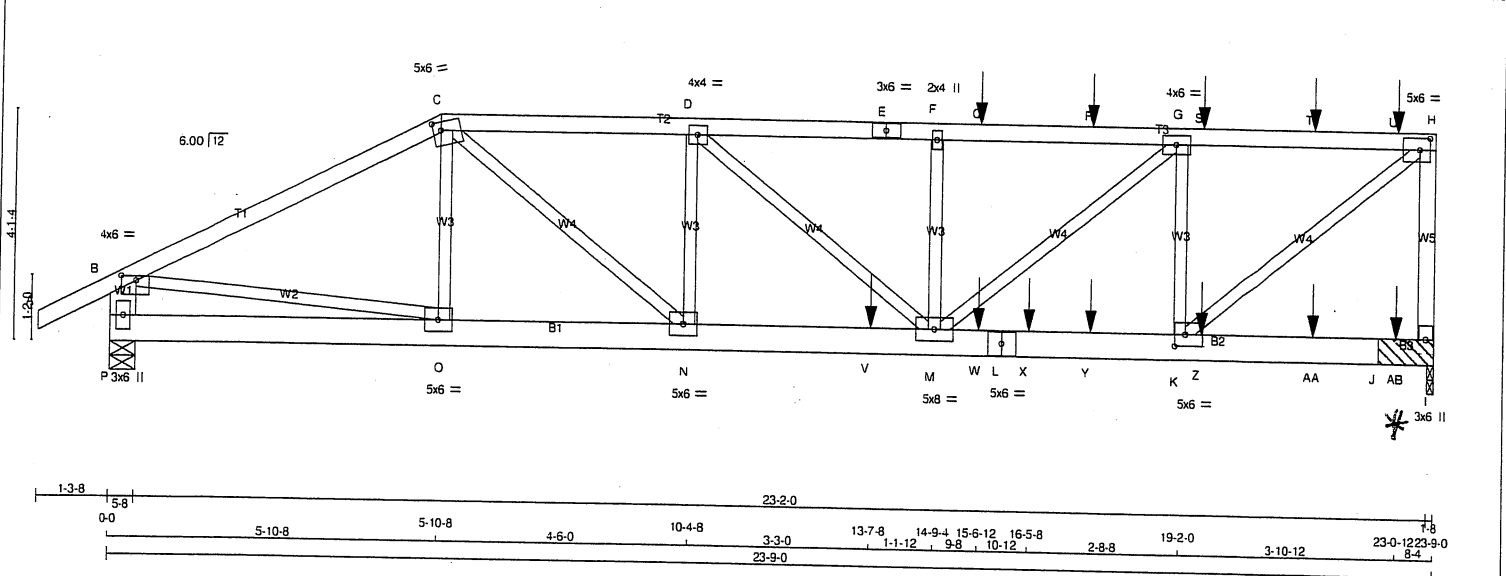
CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2017167 3/2

JOB NAME 410149	TRUSS NAME T30Z	QUANTITY 2	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.330 S May 6 2020 Mitk Industries, Inc. Tue Aug 4 13:58:23 2020 Page 1 ID:pqvtrdsyU73tskbsq1clrEz7lfa-XLlKL68oA352zY49WkXG6RUZ9nAqR3c0ur0t8yrDZ	



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - E	2x4	DRY	No.2 SPF
E - H	2x4	DRY	No.2 SPF
I - B	2x4	DRY	No.2 SPF
P - B	2x6	DRY	No.2 SPF
P - L	2x6	DRY	No.2 SPF
L - I	2x6	DRY	No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1	12	TOP
C-E 1	12	TOP
E-H 1	12	TOP
H-I 1	12	TOP
P-B 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
P-L 2	12	SIDE(0.0)
L-I 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT	IN-SX
I	3134	0	3134	0	1-8
P	2385	0	2385	0	5-8

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	MAX./MIN. COMPONENT REACTIONS	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	2210	1484	0	0	0	0	727	0
P	1681	1137	0	0	0	0	544	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P

2x6 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. I ATTACHED TO FRONT SIDE WITH 3 ROWS OF (0.122"x3") SPIRAL NAILS SPACED 3" C.C. 12 NAILS TOTAL.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	MAX.	WEBS	MAX. FACTORED	FACTORED	MAX.
MEMB.	FORCE (LBS)	VERT.	LC1	MAX	MEMB.	FORCE (LBS)	MAX	LC1
FR-TO		FROM	TO	UNBRAC	FR-TO			
A-B	0.28	-91.8	-91.8	0.07 (1)	10.00	C-C	-338	0.04 (1)
B-C	-3423	0	-91.8	-91.8	0.41 (1)	4.64	B-O	0.3085
C-D	-4773	0	-91.8	-91.8	0.23 (1)	4.23	K-H	0.4521
D-E	-5542	0	-91.8	-91.8	0.26 (1)	3.94	C-N	0.2236
E-F	-5542	0	-91.8	-91.8	0.26 (1)	3.94	K-G	-2393
F-Q	-5542	0	-91.8	-91.8	0.35 (1)	3.84	N-D	-1102
Q-R	-5542	0	-91.8	-91.8	0.35 (1)	3.84	M-G	0.2669
R-G	-5542	0	-91.8	-91.8	0.35 (1)	3.84	D-M	0.1008
G-S	-3507	0	-91.8	-91.8	0.27 (1)	4.74	M-F	-466
S-T	-3507	0	-91.8	-91.8	0.27 (1)	4.74		
T-U	-3507	0	-91.8	-91.8	0.27 (1)	4.74		
U-H	-3507	0	-91.8	-91.8	0.27 (1)	4.74		
I-H	-3109	0	0.0	0.0	0.39 (1)	6.56		
P-B	-2325	0	0.0	0.0	0.08 (1)	7.81		

P-O	0	0	-18.5	-18.5	0.04 (4)	10.00
O-N	0	3050	-18.5	-18.5	0.24 (1)	10.00
N-V	0	4773	-18.5	-18.5	0.54 (1)	10.00
V-M	0	4773	-18.5	-18.5	0.54 (1)	10.00
M-W	0	3507	-18.5	-18.5	0.49 (1)	10.00
W-L	0	3507	-18.5	-18.5	0.49 (1)	10.00
L-X	0	3507	-18.5	-18.5	0.49 (1)	10.00
X-Y	0	3507	-18.5	-18.5	0.49 (1)	10.00
Y-K	0	3507	-18.5	-18.5	0.49 (1)	10.00
K-Z	0	0	-18.5	-18.5	0.07 (1)	10.00
Z-AA	0	0	-18.5	-18.5	0.07 (1)	10.00
AA-J	0	0	-18.5	-18.5	0.07 (1)	10.00
J-AB	0	0	-18.5	-18.5	0.07 (1)	10.00
AB-I	0	0	-18.5	-18.5	0.07 (1)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	15-6-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
R	17-6-12	-63	-63	---	FRONT	VERT	TOTAL	---	C1
S	19-6-12	-63	-63	---	FRONT	VERT	TOTAL	---	C1
T	21-6-12	-63	-63	---	FRONT	VERT	TOTAL	---	C1
U	23-0-12	-77	-77	---	FRONT	VERT	TOTAL	---	C1
V	13-7-8	-1252	-1252	---	FRONT	VERT	TOTAL	---	C1
W	15-6-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
X	16-5-8	-1066	-1066	---	FRONT	VERT	TOTAL	---	C1

TOTAL WEIGHT = 4 X 112 = 447 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.79")
CALCULATED VERT. DEFL.(LL)= L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.79")
CALCULATED VERT. DEFL.(TL)= L/999 (0.20")

CSI: TC=0.41/1.00 (B-C:1), BC=0.54/1.00 (M-N:1), WB=0.56/1.00 (H-K:1), SS=0.36/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.46 (L) (INPUT = 1.00)



Structural component only
DWG# T-2017168 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410149	T30Z	2	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:pqvtrdsyU73tskbsq1clrEz7lfa-XLfKL68oA352zY49WkXjG6RUZ9nAqR3c0ur0t8yrDZ

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TTWW-m	MT20	5.0	6.0	1.75	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW-w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0	2.50	2.25
I	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.25
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	6.0		

FACTORED CONCENTRATED LOADS (LBS)

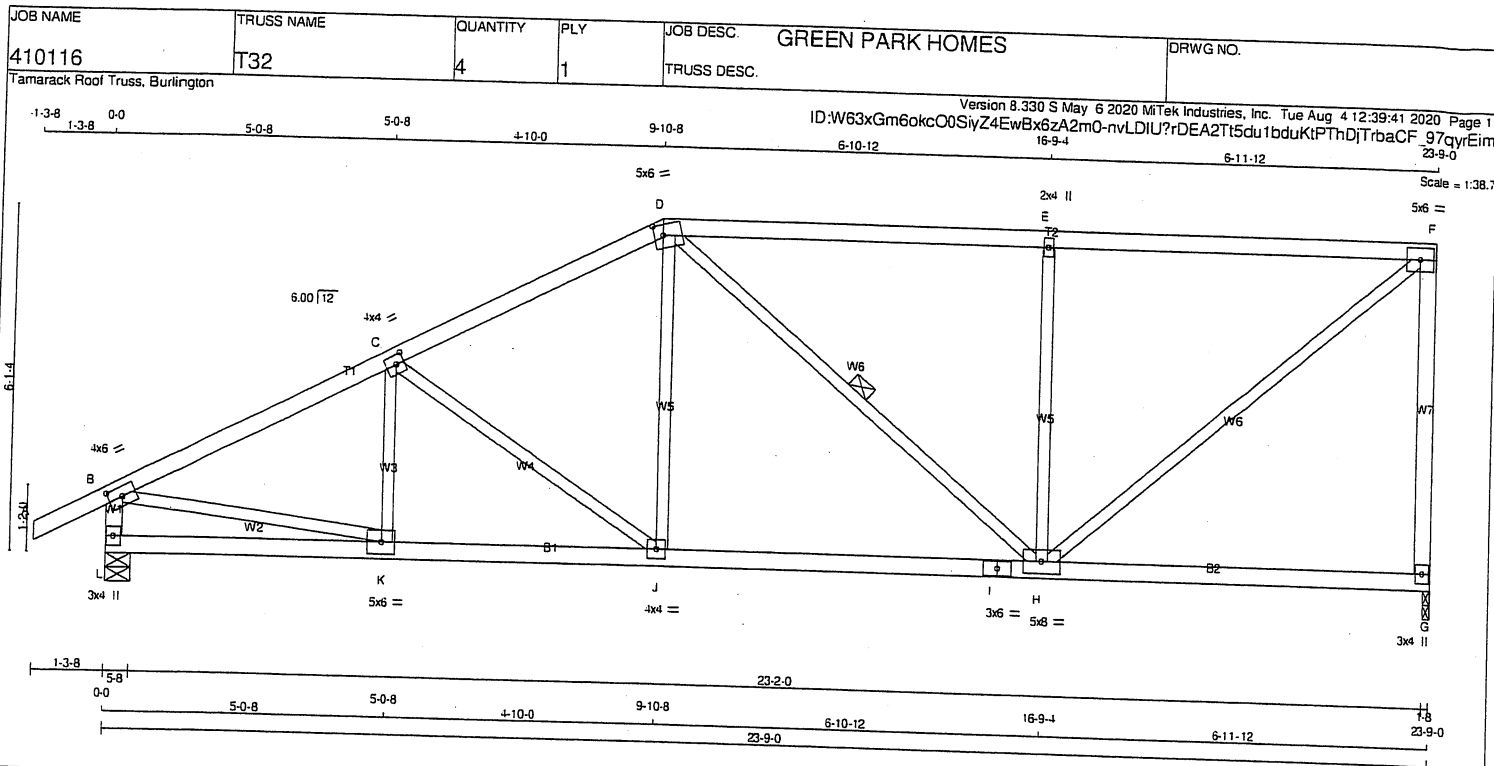
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Y	17-6-12	-13	-13	---	FRONT	VERT	TOTAL	---	C1
Z	19-6-12	-13	-13	---	FRONT	VERT	TOTAL	---	C1
AA	21-6-12	-13	-13	---	FRONT	VERT	TOTAL	---	C1
AB	23-0-12	-16	-16	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2017168 *2/2*



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTVW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1-p	MT20	3.0	4.0		
H	BMVWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMVWW-t	MT20	4.0	4.0		
K	BMVWW-t	MT20	5.0	6.0		
L	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
G	1309	0	1309	0	1-8	1-8
L	1434	0	1434	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	926	608 0	0 0	0 0	0 0	318 0	0 0
L	1012	677 0	0 0	0 0	0 0	335 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

1 LATERAL BRACE(S) AT 1:2 LENGTH OF D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT LOAD (PLF)	MAX LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 MAX CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 28	-91.8 -91.8 0.12 (1)	10.00	K-C	-184 17	0.04 (1)	
B-C	-1791 0	-91.8 -91.8 0.45 (1)	4.50	C-J	-385 0	0.24 (1)	
C-D	-1484 0	-91.8 -91.8 0.42 (1)	4.88	J-D	0 326	0.07 (1)	
D-E	-1217 0	-91.8 -91.8 0.85 (1)	4.27	B-K	0 1646	0.37 (1)	
E-F	-1217 0	-91.8 -91.8 0.86 (1)	4.27	H-F	0 1584	0.36 (1)	
G-F	-1257 0	0.0 0.0 0.91 (1)	7.17	D-H	-122 0	0.07 (1)	
L-B	-1392 0	0.0 0.0 0.14 (1)	6.90	H-E	-790 0	0.46 (1)	
L-K	0 0	-18.5 -18.5 0.10 (4)	10.00				
K-J	0 1623	-18.5 -18.5 0.33 (1)	10.00				
J-I	0 1311	-18.5 -18.5 0.33 (4)	10.00				
I-H	0 1311	-18.5 -18.5 0.33 (4)	10.00				
H-G	0 0	-18.5 -18.5 0.22 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.79")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.79")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.12")

CSI: TC=0.91/1.00 (F-G:1), BC=0.33/1.00 (H-J:4), WB=0.46/1.00 (E-H:1), SSI=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

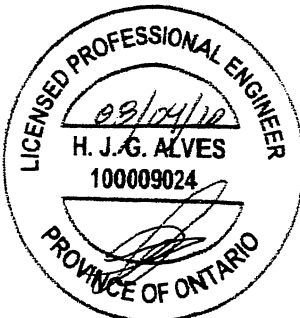
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

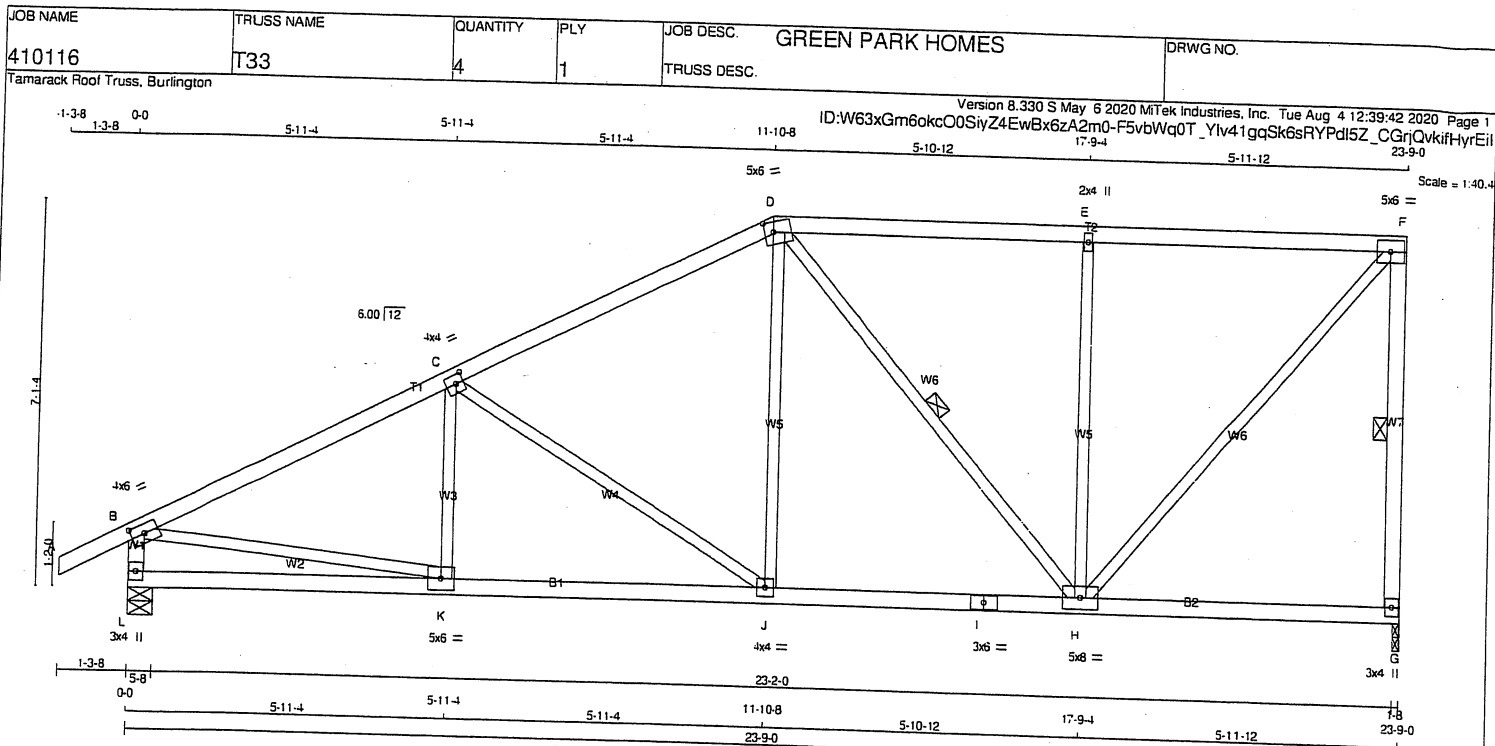
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.83 (B) (INPUT = 0.90)
JSI METAL = 0.47 (B) (INPUT = 1.00)





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	6.0	2.00	3.00
C	TMVW-1	MT20	4.0	4.0	2.00	1.75
D	TTVW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-1	MT20	5.0	6.0		
G	BMV1-p	MT20	3.0	4.0		
H	BMVWV-1	MT20	5.0	8.0		
I	BS-1	MT20	3.0	6.0		
J	BMVWV-1	MT20	4.0	4.0		
K	BMVWV-1	MT20	5.0	6.0		
L	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
G	1309	0	1309	0	1-8	1-8
L	1434	0	1434	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX. MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	SNOW	LIVE	SNOW	LIVE				
G	926	608	0	0	0	0	318	0
L	1012	677	0	0	0	0	335	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (LC)
FR-TO					FR-TO		
A-B	0 28	-91.8	-91.8 0.12 (1)	10.00	K-C	-117 60	0.03 (1)
B-C	-1802 0	-91.8	-91.8 0.68 (1)	4.15	C-J	-584 0	0.58 (1)
C-D	-1309 0	-91.8	-91.8 0.62 (1)	4.78	J-D	0 420	0.09 (1)
D-E	-926 0	-91.8	-91.8 0.58 (1)	5.47	B-K	0 1654	0.37 (1)
E-F	-926 0	-91.8	-91.8 0.58 (1)	5.47	H-F	0 1404	0.32 (1)
G-F	-1264 0	0.0	0.0 0.29 (1)	5.73	D-H	-343 0	0.19 (1)
L-B	-1387 0	0.0	0.0 0.14 (1)	6.90	H-E	-676 0	0.59 (1)
L-K	0 0	-18.5	-18.5 0.15 (4)	10.00			
K-J	0 1637	-18.5	-18.5 0.33 (1)	10.00			
J-I	0 1150	-18.5	-18.5 0.26 (1)	10.00			
I-H	0 1150	-18.5	-18.5 0.26 (1)	10.00			
H-G	0 0	-18.5	-18.5 0.16 (4)	10.00			

TOTAL WEIGHT = 4 X 103 = 414 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 39.0 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.68/1.00 (B-C:1), BC=0.33/1.00 (J-K:1),
WB=0.59/1.00 (E-H:1), SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1 10
COMP=1.10 SHEAR=1.10 TENS=1 10

COMPANION LIVE LOAD FACTOR = 1.00

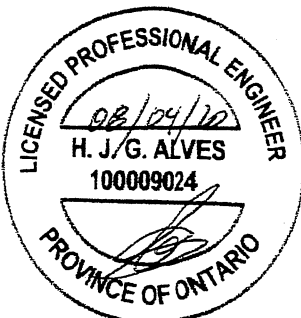
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

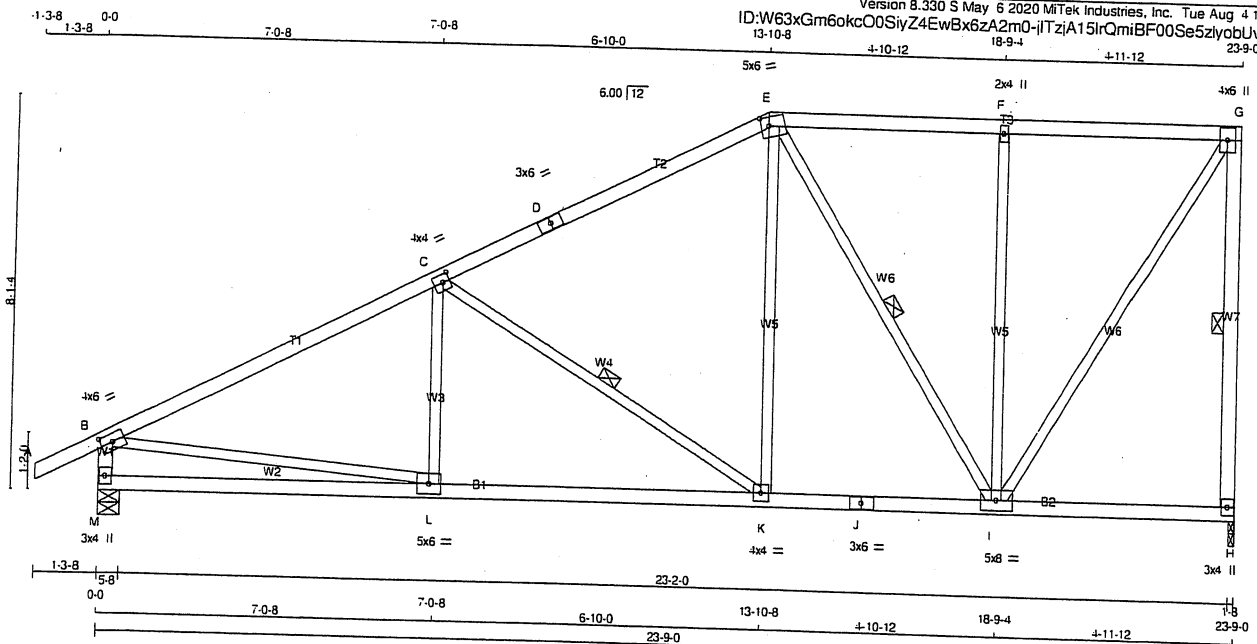
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.83 (B) (INPUT = 0.90)
JSI METAL = 0.45 (B) (INPUT = 1.00)



JOB NAME 410116	TRUSS NAME T34	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



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13-10-8 18-9-4 4-11-12 23-9-0
Scale = 1:45.2

LUMBER

N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	4.0	6.0	2.00	3.00
C TMVW-t	MT20	4.0	4.0	2.00	1.75
D TS-t	MT20	3.0	6.0		
E TTWW-m	MT20	5.0	6.0	2.25	2.00
F TMW-w	MT20	2.0	4.0		
G TMVW+p	MT20	4.0	6.0		
H BMV1+p	MT20	3.0	4.0		
I BMVWW-t	MT20	5.0	8.0		
J BS-t	MT20	3.0	6.0		
K BMVWW-t	MT20	4.0	4.0		
L BMVWW-t	MT20	5.0	6.0		
M BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	VERT	DOWN	UP	IN-SX
H	1309	0	0	1-8
M	1434	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT COMBINED							
H	926	608.0	0.0	0.0	0.0	318.0	0.0
M	1012	677.0	0.0	0.0	0.0	335.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-K, E-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	LC2 MAX	UNBRACED LENGTH	FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	LC1 MAX	LC2 MAX
A-B	0	28	-91.8	-91.8	0.12 (1)	10.00	L-C	-52	100	0.03 (4)	0.35 (1)
B-C	-1780	0	-91.8	-91.8	0.67 (1)	4.17	C-K	-774	0	0.35 (1)	0.12 (1)
C-D	-1118	0	-91.8	-91.8	0.60 (1)	5.11	K-E	0	520	0.12 (1)	0.37 (1)
D-E	-1118	0	-91.8	-91.8	0.60 (1)	5.11	B-L	0	1634	0.37 (1)	0.29 (1)
E-F	-698	0	-91.8	-91.8	0.29 (1)	6.25	I-G	0	1299	0.29 (1)	0.30 (1)
F-G	-698	0	-91.8	-91.8	0.29 (1)	6.25	E-I	-521	0	0.30 (1)	0.71 (1)
G-H	-1270	0	0.0	0.0	0.38 (1)	5.71	I-F	-560	0	0.71 (1)	
M-B	-1380	0	0.0	0.0	0.14 (1)	6.92					
M-L	0	0	-18.5	-18.5	0.22 (4)	10.00					
L-K	0	1622	-18.5	-18.5	0.37 (1)	10.00					
K-J	0	975	-18.5	-18.5	0.22 (1)	10.00					
J-I	0	975	-18.5	-18.5	0.22 (1)	10.00					
I-H	0	0	-18.5	-18.5	0.10 (4)	10.00					

TOTAL WEIGHT = 4 X 109 = 436 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.67/1.00 (B-C:1), BC=0.37/1.00 (K-L:1), WB=0.71/1.00 (F-I:1), SSI=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1 10
COMP=1 10 SHEAR=1.10 TENS=1 10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

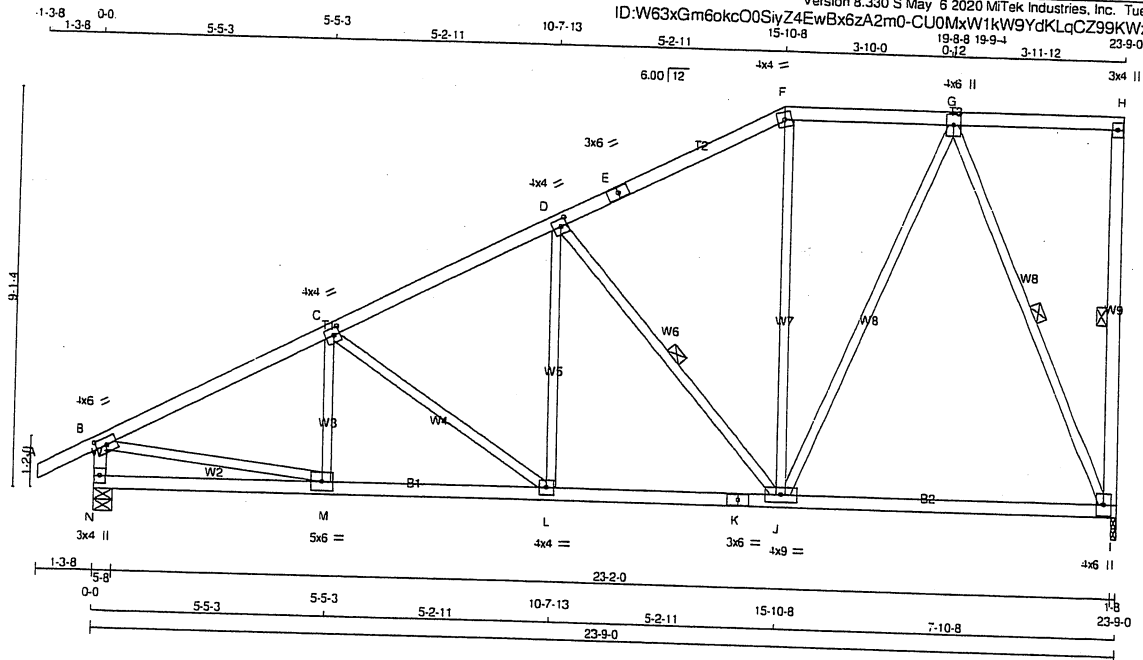
JSI GRIP= 0.81 (B) (INPUT= 0.90)
JSI METAL= 0.45 (B) (INPUT= 1.00)



JOB NAME 410116	TRUSS NAME T35	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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19-8-8 19-9-4 0-12 3-11-12 23-9-0



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
I - H	2x4	DRY	No.2
N - B	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	4.0	6.0	2.00	3.00
C TMVW-t	MT20	4.0	4.0	2.00	1.75
D TMVW-t	MT20	4.0	4.0	2.00	1.75
E TS-t	MT20	3.0	6.0		
F TTW-m	MT20	4.0	4.0		
G TMVW-t	MT20	4.0	6.0		
H TMV-p	MT20	3.0	4.0		
I BMVW1+p	MT20	4.0	6.0		
J BMVW-t	MT20	4.0	9.0		
K BS-t	MT20	3.0	6.0		
L BMVW-t	MT20	4.0	4.0		
M BMVW-t	MT20	5.0	6.0		
N BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
I	1309	0	1309	0	1-8	1-8
N	1434	0	1434	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX. MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	SNOW	LIVE	SNOW	LIVE				
I	926	608.0	0.0	0.0	0.0	0.0	318.0	0.0
N	1012	677.0	0.0	0.0	0.0	0.0	335.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, D-J, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	0 28	-91.8 -91.8	0.12 (1)	10.00	M-C	-145 41	0.03 (1)
B-C	-1785 0	-91.8 -91.8	0.36 (1)	4.64	C-L	-398 0	0.28 (1)
C-D	-1438 0	-91.8 -91.8	0.29 (1)	5.14	L-D	0 303	0.07 (1)
D-E	-901 0	-91.8 -91.8	0.29 (1)	6.12	D-J	-766 0	0.34 (1)
E-F	-901 0	-91.8 -91.8	0.29 (1)	6.12	J-F	0 78	0.03 (4)
F-G	-787 0	-91.8 -91.8	0.19 (1)	6.25	J-G	0 723	0.16 (1)
G-H	0 0	-91.8 -91.8	0.23 (1)	10.00	G-I	-1219 0	0.80 (1)
H-I	-139 0	0.0 0.0	0.05 (1)	6.25	B-M	0 1635	0.37 (1)
I-H	-1391 0	0.0 0.0	0.14 (1)	6.90			
N-M	0 0	-18.5 -18.5	0.13 (4)	10.00			
M-L	0 1615	-18.5 -18.5	0.31 (1)	10.00			
L-K	0 1286	-18.5 -18.5	0.35 (4)	10.00			
K-J	0 1286	-18.5 -18.5	0.35 (4)	10.00			
J-I	0 502	-18.5 -18.5	0.28 (4)	10.00			

TOTAL WEIGHT = 4 X 114 = 455 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 7.4	PSF
DL = 0.4	PSF	
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.79")
CALCULATED VERT. DEFL.(LL)= L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.79")
CALCULATED VERT. DEFL.(TL)= L/999 (0.13")

CSI: TC=0.36/1.00 (B-C:1), BC=0.35/1.00 (J-L:4), WB=0.80/1.00 (G-I:1), SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1 10
COMP=1.10 SHEAR=1 10 TENS=1 10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

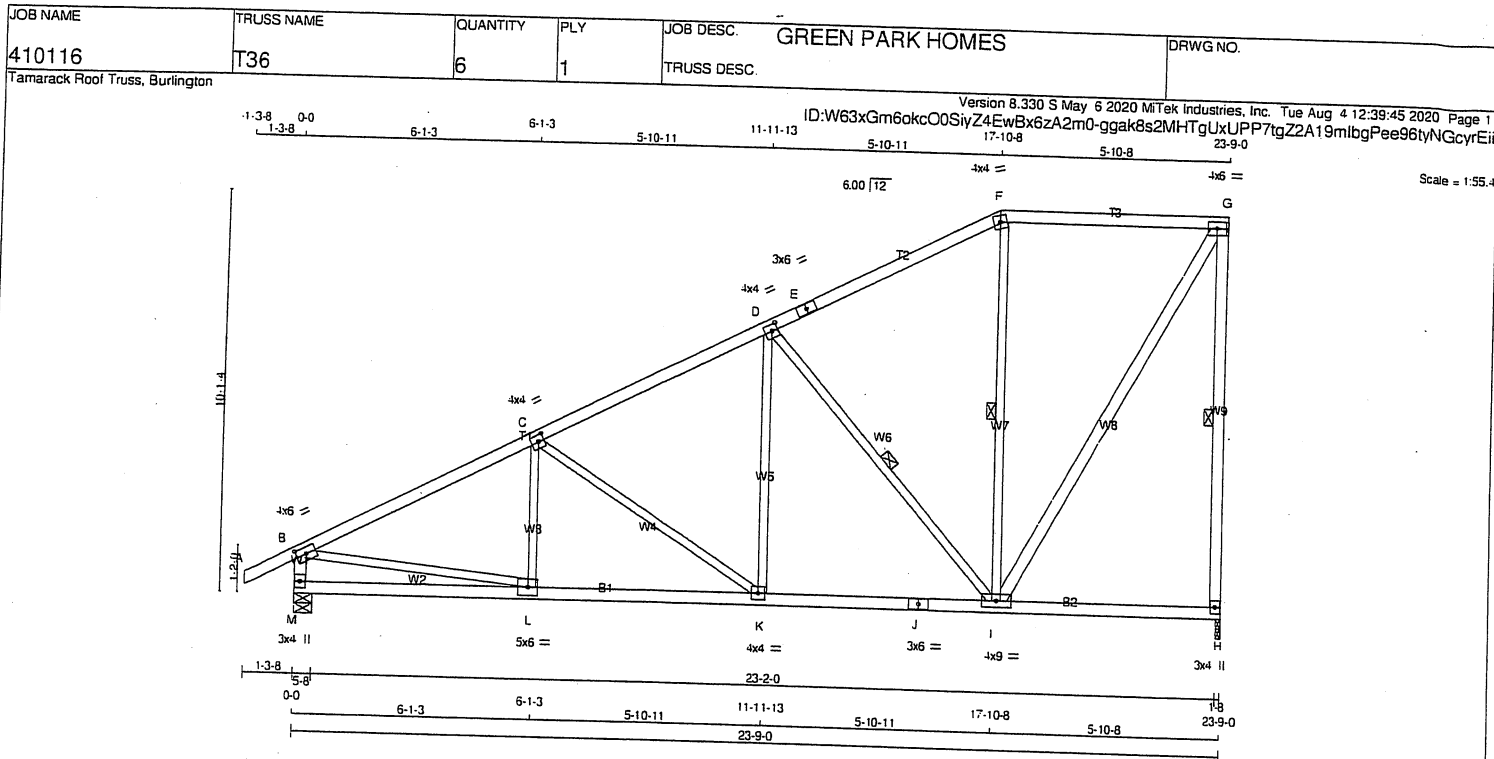
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.48 (K) (INPUT = 1.00)





LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - F	2x4	DRY	No.2
B - G	2x4	DRY	No.2
C - H	2x4	DRY	No.2
D - I	2x4	DRY	No.2
E - J	2x4	DRY	No.2
F - K	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT	1 - G	2x4	DRY
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TMVW-t	MT20	4.0	4.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
F	TTW-m	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMVWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
H	1309	0	1309	0	1-8	1-8
M	1434	0	1434	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
H	926	608	0	0	0	318	0
M	1012	677	0	0	0	335	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, D-I, F-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO			
A-B	0	28	10.00	L-C	-106	63	0.03 (1)
B-C	-1781	0	4.52	C-K	-512	0	0.49 (1)
C-D	-1327	0	5.20	K-D	0	388	0.09 (1)
D-E	-683	0	6.25	D-I	-908	0	0.52 (1)
E-F	-683	0	6.25	I-F	-192	14	0.13 (1)
F-G	-587	0	6.25	I-G	0	1158	0.19 (1)
G-H	-1267	0	5.73	B-L	0	1629	0.37 (1)
H-I	-1386	0	6.91				
M-L	0	0	10.00				
L-K	0	1614	10.00				
K-J	0	1187	10.00				
J-I	0	1187	10.00				
I-H	0	0	10.00				

TOTAL WEIGHT = 6 X 116 = 696 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
BOT CH. LL = 6.0 PSF
DL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.63/1.00 (G-H:1), BC=0.33/1.00 (K-L:1), WB=0.52/1.00 (D-I:1), SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

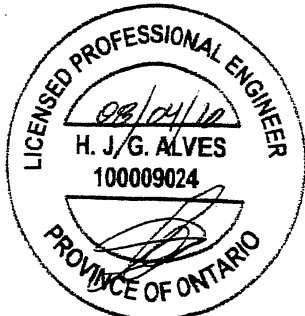
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

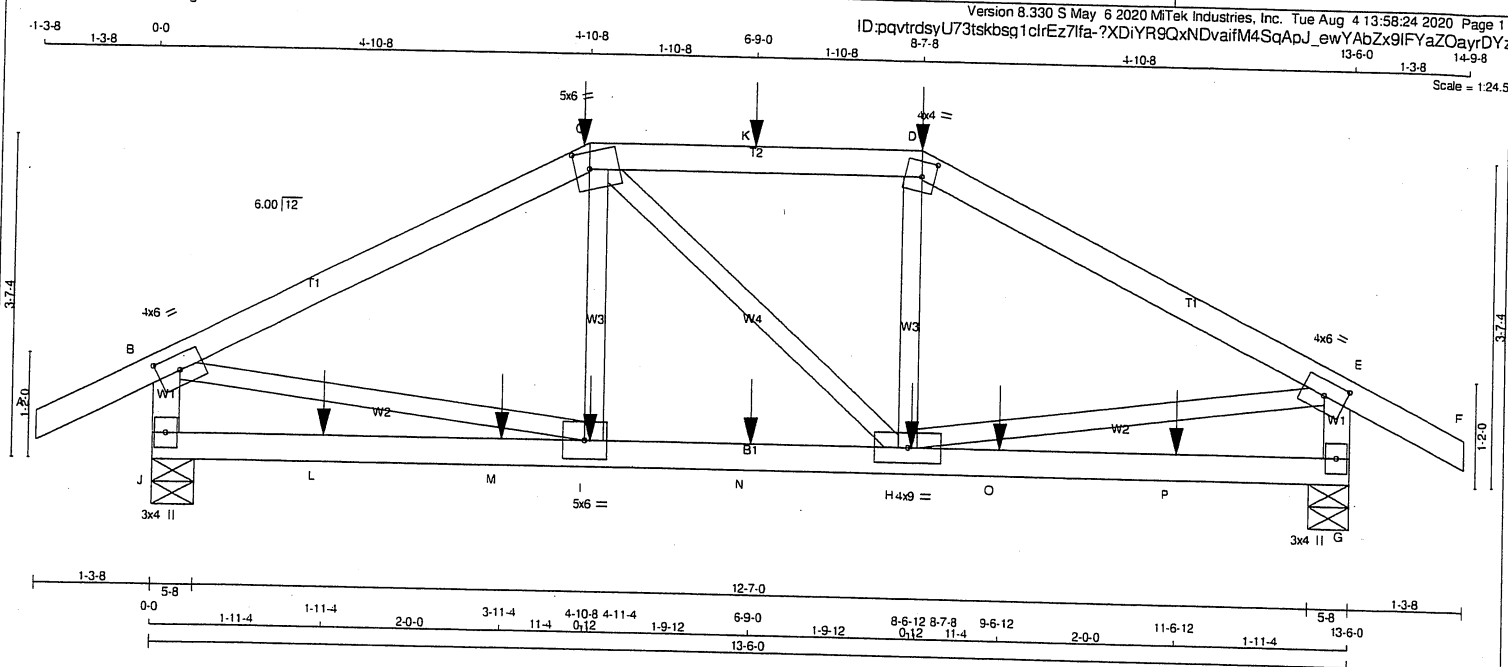
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (I) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410149	T37	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



LUMBER	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES				
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF

ALL WEBS EXCEPT 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMW-t MT20	4.0	6.0	2.00	3.00
C TTWW-m MT20	5.0	6.0	2.25	2.00
D TTW-m MT20	4.0	4.0	2.00	1.75
E TMW-t MT20	4.0	6.0	2.00	3.00
G BMV1-p MT20	3.0	4.0		
H BMWW-t MT20	4.0	9.0		
I BMW-t MT20	5.0	6.0		
J BMV1-p MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REORD BRG		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	1400	0	1400	0	0	5-8	5-8
J G	1409	0	1409	0	0	5-8	5-8

1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
J 989	658 / 0 0 / 0 0 / 0 331 0 0 / 0
G 995	663 / 0 0 / 0 0 / 0 332 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (UNBRACED LENGTH)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
FR-TO							FR-TO			
A-B	0 : 28	-91.8	-91.8	0.13 (1)	10.00	10.00	I-C	-66 : 85	0.03 (4)	0.03 (4)
B-C	-1658 / 0	-91.8	-91.8	0.50 (1)	4.51	4.51	C-H	0 : 22	0.01 (1)	0.01 (1)
C-K	-1495 / 0	-91.8	-91.8	0.45 (1)	4.74	4.74	H-D	-77 : 85	0.03 (4)	0.03 (4)
K-D	-1495 / 0	-91.8	-91.8	0.45 (1)	4.74	4.74	B-I	0 : 1503	0.37 (1)	0.37 (1)
D-E	-1678 / 0	-91.8	-91.8	0.50 (1)	4.49	4.49	H-E	0 : 1521	0.38 (1)	0.38 (1)
E-F	0 : 28	-91.8	-91.8	0.13 (1)	10.00	10.00				
J-B	-1347 / 0	0.0	0.0	0.15 (1)	6.95	6.95				
G-E	-1355 / 0	0.0	0.0	0.15 (1)	6.94	6.94				
J-L	0 : 0	-18.5	-18.5	0.17 (4)	10.00	10.00				
L-M	0 : 0	-18.5	-18.5	0.17 (4)	10.00	10.00				
M-I	0 : 0	-18.5	-18.5	0.17 (4)	10.00	10.00				
I-N	0 : 1479	-18.5	-18.5	0.34 (1)	10.00	10.00				
N-H	0 : 1479	-18.5	-18.5	0.34 (1)	10.00	10.00				
H-O	0 : 0	-18.5	-18.5	0.18 (4)	10.00	10.00				
O-P	0 : 0	-18.5	-18.5	0.18 (4)	10.00	10.00				
P-G	0 : 0	-18.5	-18.5	0.18 (4)	10.00	10.00				

FACTORED CONCENTRATED LOADS (LBS)	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT									
C	4-10-8	-343	-343	---	BACK	VERT	TOTAL	---	C1
D	8-7-8	-378	-378	---	BACK	VERT	TOTAL	---	C1
H	8-6-12	-46	-46	---	BACK	VERT	TOTAL	---	C1
I	4-11-4	-46	-46	---	BACK	VERT	TOTAL	---	C1
K	6-9-0	-113	-113	---	BACK	VERT	TOTAL	---	C1
L	1-11-4	-17	-17	---	BACK	VERT	TOTAL	---	C1
M	3-11-4	-34	-34	---	BACK	VERT	TOTAL	---	C1
N	6-9-0	-46	-46	---	BACK	VERT	TOTAL	---	C1
O	9-6-12	-34	-34	---	BACK	VERT	TOTAL	---	C1
P	11-6-12	-17	-17	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 53 = 107 b

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL)= L/999 (0.07")

CSI: TC=0.50/1.00 (D-E:1), BC=0.34/1.00 (H-I:1), WB=0.38/1.00 (E-H:1), SS=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

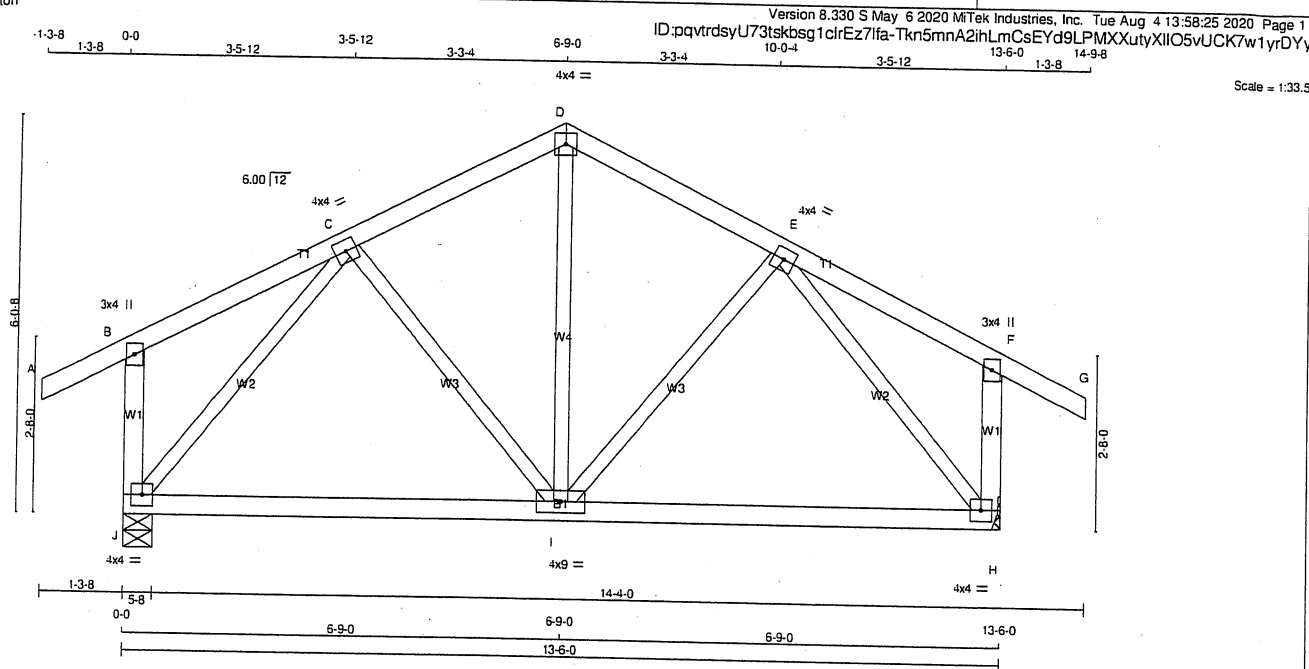
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (H) (INPUT = 0.90)
JSI METAL= 0.42 (E) (INPUT = 1.00)



Structural component only
DWG# T-2017169

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410149	T38	6	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVW1-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
J	869	0	869	0
H	869	0	869	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	612	415 / 0	0 / 0	0 / 0	0 / 0	197	0	0
H	612	415 / 0	0 / 0	0 / 0	0 / 0	197	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	UNBRACED LENGTH	MEMB.	WEBS	MAX. FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO						FR-TO			
A-B	0.28	-91.8	-91.8	0.12 (1)	10.00	I-D	0	218	0.05 (1)
B-C	0.16	-91.8	-91.8	0.17 (1)	10.00	I-E	-41	32	0.02 (1)
C-D	-525.0	-91.8	-91.8	0.13 (1)	6.25	C-I	-41	32	0.02 (1)
D-E	-525.0	-91.8	-91.8	0.13 (1)	6.25	J-C	-752	0	0.33 (1)
E-F	0.16	-91.8	-91.8	0.17 (1)	10.00	E-H	-752	0	0.33 (1)
F-G	0.28	-91.8	-91.8	0.12 (1)	10.00				
J-B	-244.0	0.0	0.0	0.03 (1)	7.81				
H-F	-244.0	0.0	0.0	0.03 (1)	7.81				
J-I	0.482	-18.5	-18.5	0.28 (4)	10.00				
I-H	0.482	-18.5	-18.5	0.28 (4)	10.00				

TOTAL WEIGHT = 6 X 62 = 370 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.17/1.00 (E-F:1), BC=0.28/1.00 (I-J:4), WB=0.33/1.00 (E-H:1), SSI=0.13/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

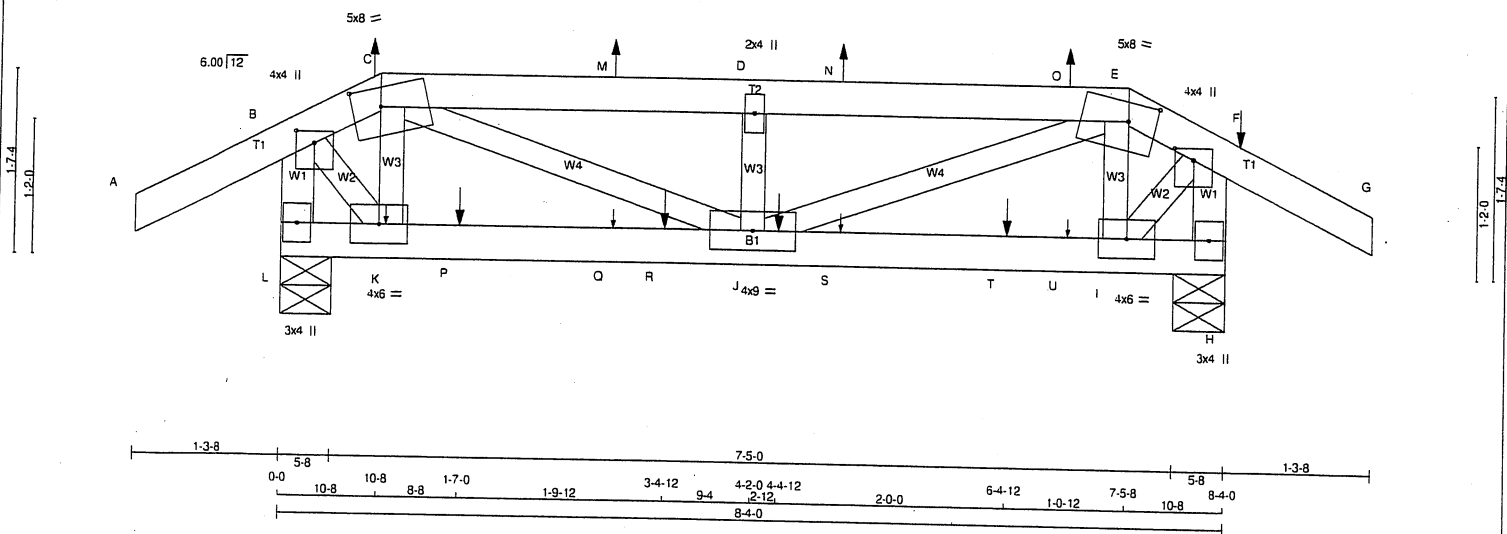
JSI GRIP= 0.62 (E) (INPUT = 0.90)
JSI METAL= 0.23 (E) (INPUT = 1.00)



Structural component only
DWG# T-2017170

JOB NAME 410149	TRUSS NAME T39	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.330 S May 6 2020 Mitek Industries, Inc. Tue Aug 4 13:58:26 2020 Page 1 ID:pqvrdsyU73tskbsq1clrEz7fa-xwLTz7BqT_Tdq?pkBsteuk33VMs?1s22is3qSTyrDYx	

Scale = 1:19.2



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.
A - C 2x4 DRY No.2 SPF
C - E 2x4 DRY No.2 SPF
E - G 2x4 DRY No.2 SPF
L - B 2x4 DRY No.2 SPF
H - F 2x4 DRY No.2 SPF
L - H 2x4 DRY No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMWV+p MT20 4.0 4.0 1.25 2.00
C TTWW-m MT20 5.0 8.0 2.00 3.00
D TMW+w MT20 2.0 4.0
E TTWW-m MT20 5.0 8.0 2.00 3.00
F TMWV+p MT20 4.0 4.0 1.25 2.00
H BMV1+p MT20 3.0 4.0
I BMWV-t MT20 4.0 6.0
J BMWVW-t MT20 4.0 9.0
K BMWV-t MT20 4.0 6.0
L BMV1+p MT20 3.0 4.0

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF
SPACING = 24.0 IN. CC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF
(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.17/1.00 (C-D:1), BC=0.34/1.00 (J-K:1), WB=0.29/1.00 (E-J:1), SS=0.28/1.00 (J-K:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00
AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (C) (INPUT = 0.90)
JSI METAL= 0.24 (F) (INPUT = 1.00)

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

FACTORED CONCENTRATED LOADS (LBS)
JT LOC. LC1 MAX- FACE DIR. TYPE HEEL CONN.
C 10-8 13 1 150 FRONT VERT TOTAL --- C1
F 8-4-12 -280 -280 --- TOP VERT TOTAL --- C1
J 4-4-12 -202 -202 --- TOP VERT TOTAL --- C1
K 11-4 1 1 19 FRONT VERT TOTAL --- C1
M 2-11-4 20 1 159 FRONT VERT TOTAL --- C1
N 4-11-4 20 1 159 FRONT VERT TOTAL --- C1
O 6-11-4 17 1 137 FRONT VERT TOTAL --- C1
P 1-7-0 -250 -250 --- BACK VERT TOTAL --- C1
Q 2-11-4 1 1 19 FRONT VERT TOTAL --- C1
R 3-4-12 -250 -250 --- BACK VERT TOTAL --- C1
S 4-11-4 1 1 19 FRONT VERT TOTAL --- C1
T 6-4-12 -264 -264 --- TOP VERT TOTAL --- C1
U 6-11-4 1 1 19 FRONT VERT TOTAL --- C1

CHORDS
MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD (LBS) MAX. UNBRAC LENGTH (FT) FR-TO FROM TO FR-TO FROM TO
A-B 0.28 -91.8 -91.8 0.13 (1) 10.00 J-D -294 0 0.05 (1)
B-C -808.0 -91.8 -91.8 0.13 (1) 6.25 K-C -113.55 0.02 (1)
C-M -1676.0 -91.8 -91.8 0.17 (1) 4.93 I-E -180 4 0.03 (1)
M-D -1676.0 -91.8 -91.8 0.17 (1) 4.93 J-E 0.1160 0.29 (1)
D-N -1676.0 -91.8 -91.8 0.17 (1) 4.93 I-F 0.813 0.20 (1)
N-O -1676.0 -91.8 -91.8 0.17 (1) 4.93 C-J 0.1080 0.27 (1)
O-E -1676.0 -91.8 -91.8 0.17 (1) 4.93 B-K 0.905 0.22 (1)
E-F -731.0 -91.8 -91.8 0.13 (1) 6.25
F-G 0.28 -91.8 -91.8 0.13 (1) 10.00
L-B -1205.0 0.0 0.0 0.13 (1) 7.26
H-F -1389.0 0.0 0.0 0.15 (1) 6.87

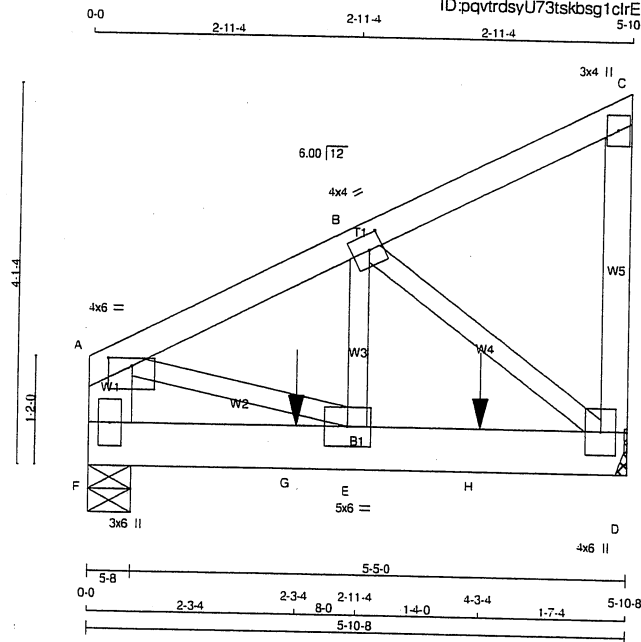
WEBS
MEMB. MAX. FACTORED FORCE (LBS) MAX. UNBRAC LENGTH (FT) FR-TO FROM TO FR-TO FROM TO
L-K 0.0 -18.5 -18.5 0.22 (1) 10.00
K-P 0.672 -18.5 -18.5 0.34 (1) 10.00
P-Q 0.672 -18.5 -18.5 0.34 (1) 10.00
Q-R 0.672 -18.5 -18.5 0.34 (1) 10.00
R-J 0.672 -18.5 -18.5 0.34 (1) 10.00
J-S 0.598 -18.5 -18.5 0.28 (1) 10.00
S-T 0.598 -18.5 -18.5 0.28 (1) 10.00
T-U 0.598 -18.5 -18.5 0.28 (1) 10.00
U-I 0.598 -18.5 -18.5 0.28 (1) 10.00
I-H 0.0 -18.5 -18.5 0.17 (1) 10.00

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2017171

JOB NAME 410149	TRUSS NAME T40	QUANTITY 2	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



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ID:pqvtrdsyU73tskbsg1clrEz7ifa-P6vrBTCIEIbUR9OwlaOtrYcG0mEjmlBCxWpD?vyrDYw
5-10-8

Scale = 1:23.7

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
F - A	2x6 DRY	No.2	SPF	
A - C	2x4 DRY	No.2	SPF	
D - C	2x4 DRY	No.2	SPF	
F - D	2x6 DRY	No.2	SPF	
ALL WEBS	2x3 DRY	No.2	SPF	
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 2	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TMV-p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
F	1078	0	1078	0	5-8	5-8
D	1271	0	1271	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE				
F	758	518 / 0	0 / 0	0 / 0	0 / 0	240 / 0	0 / 0
D	894	612 / 0	0 / 0	0 / 0	0 / 0	282 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)
FR-TO		FROM TO	FR-TO	FROM TO
F-A	-958 / 0	0.0	0.0	0.03 (1)
A-B	-1230 / 0	-91.8	-91.8	0.06 (1)
B-C	-11 / 0	-91.8	-91.8	0.05 (1)
D-C	-110 / 0	0.0	0.0	0.01 (1)
F-G	0 / 0	-18.5	-18.5	0.11 (1)
G-E	0 / 0	-18.5	-18.5	0.11 (1)
E-H	0 / 1110	-18.5	-18.5	0.25 (1)
H-D	0 / 1110	-18.5	-18.5	0.25 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-3-4	-850	-850	---	BACK	VERT	TOTAL	---	C1
H	4-3-4	-850	-850	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 29 = 117 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.25/1.00 (D-E:1) , WB=0.17/1.00 (B-D:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
		1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

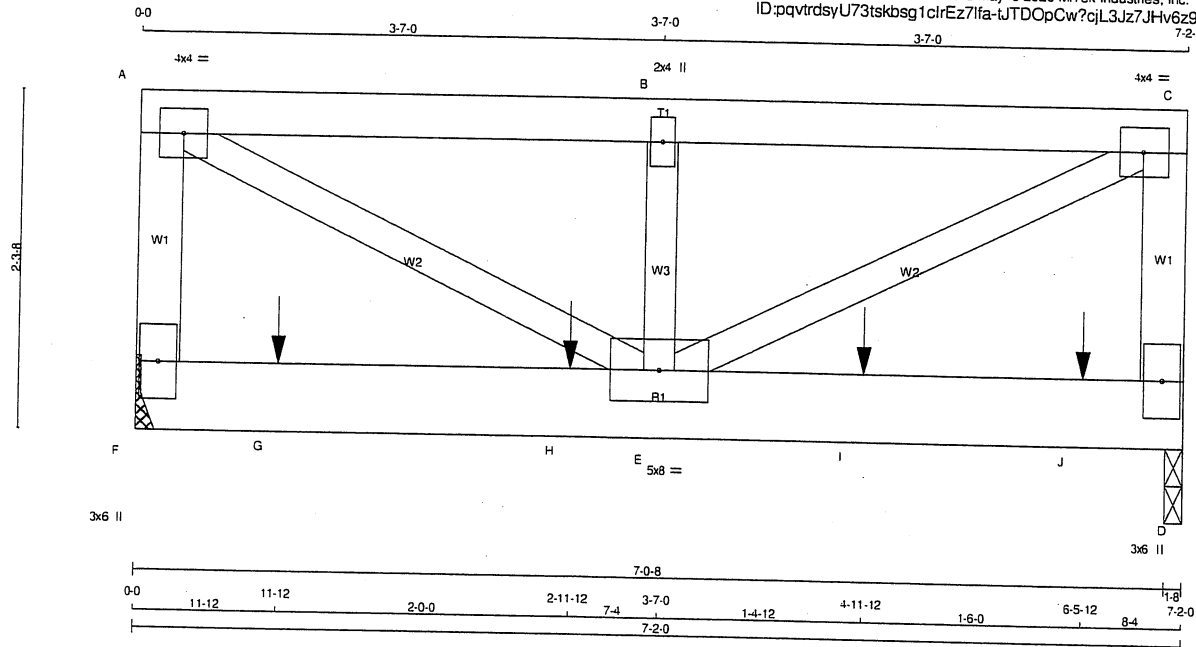
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (B) (INPUT = 0.90)
JSI METAL = 0.25 (D) (INPUT = 1.00)



Structural component only
DWG# T-2017172

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410149	T41	2	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.330 S May 6 2020 Mitek Industries, Inc. Tue Aug 4 13:58:28 2020 Page 1 ID:pqvtrdsyU73tskbsg1clEz7lfa-tJDOpCw?cjL3Jz7JHv6z99OMAcMvovLAAyNXLyrDYv					
7-2-0					



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A 1	12	TOP
A - C 1	12	SIDE(25.8)
C - D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0		
B	TMVW-w	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
D	BMV1-p	MT20	3.0	6.0		
E	BMVWW-t	MT20	5.0	8.0		
F	BMV1-p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	890	0	890	0
D	943	0	943	0
			1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	627	429.0	0.0	0.0	0.0	198.0	0.0
D	664	455.0	0.0	0.0	0.0	209.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. VERT. LOAD (LC2)	MAX. VERT. LOAD (LC3)	MAX. VERT. LOAD (LC4)	MAX. VERT. LOAD (LC5)	MAX. VERT. LOAD (LC6)	MAX. VERT. LOAD (LC7)	MAX. VERT. LOAD (LC8)	MAX. VERT. LOAD (LC9)	MAX. VERT. LOAD (LC10)
FR-TO												
F-A	-723.0	0.0	0.0	0.04 (1)	7.81	A-E	0.0	1103	0.14 (1)			
A-B	-972.0	-143.4	-143.4	0.15 (1)	6.25	E-B	-623.0	0.0	0.05 (1)			
B-C	-972.0	-143.4	-143.4	0.15 (1)	6.25	E-C	0	1103	0.14 (1)			
D-C	-723.0	0.0	0.0	0.04 (1)	7.81							
F-G	0.0	-18.5	-18.5	0.06 (1)	10.00							
G-H	0.0	-18.5	-18.5	0.06 (1)	10.00							
H-E	0.0	-18.5	-18.5	0.06 (1)	10.00							
E-I	0.0	-18.5	-18.5	0.07 (1)	10.00							
I-J	0.0	-18.5	-18.5	0.07 (1)	10.00							
J-D	0.0	-18.5	-18.5	0.07 (1)	10.00							

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	11-12	-168	-168	---	BACK	VERT	TOTAL	---	C1
H	2-11-12	-168	-168	---	BACK	VERT	TOTAL	---	C1
I	4-11-12	-168	-168	---	BACK	VERT	TOTAL	---	C1
J	6-5-12	-170	-170	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 32 = 128 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CSld Girder

START DISTANCE = 0-0
START SPAN CARRIED = 4-0-0
END DISTANCE = 7-2-0
END SPAN CARRIED = 4-0-0
END WALL WIDTH = 1-8
APPLIED TO FRONT SIDE OF TOP CHORD.
- ADD'L LOADS BASED ON 55% OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL)= L/999 (0.02")

CSI: TC=0.15/1.00 (B-C:1), BC=0.07/1.00 (D-E:1).
WB=0.14/1.00 (A-E:1), SSI=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL. = 0.250 inches

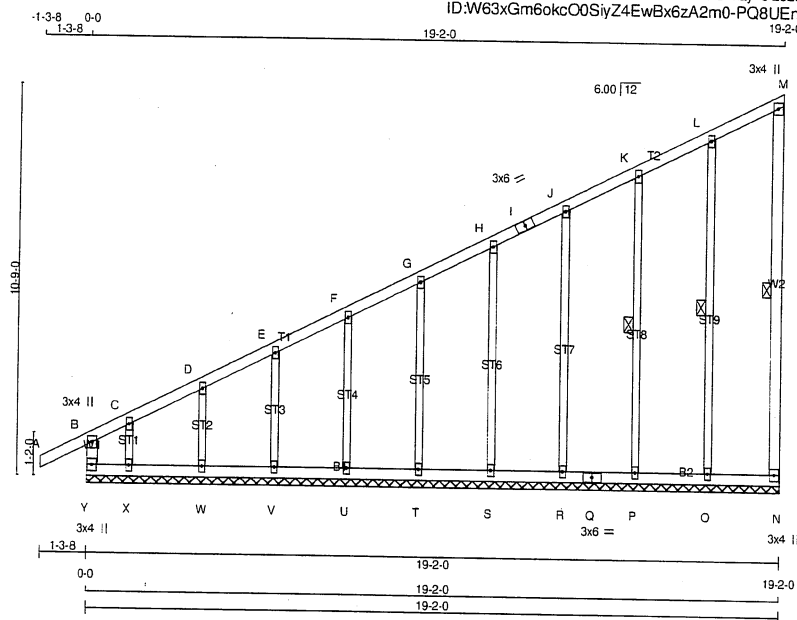
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.57 (C) (INPUT = 0.90)
JSI METAL= 0.17 (C) (INPUT = 1.00)



Structural component only
DWG# T-2017173

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
411573	G99	4	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
Y - B	2x4	DRY	No.2	SPF
A - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
Y - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C, D, E, F, G, H, J, K, L					
C TMW+w	MT20	2.0	4.0		
I TS+I	MT20	3.0	6.0		
M TMV+p	MT20	3.0	4.0		
N BMV1+p	MT20	3.0	4.0		
O, P, R, S, T, U, V, W, X					
O BMW1+w	MT20	2.0	4.0		
Q BS-I	MT20	3.0	6.0		
Y BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF M-N, L-O, K-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	UNBRACED LENGTH
FR-TO				FR-TO			
Y-B	-245 / 0	0.0	0.0 (1)	O-L	-198 / 0	0.13 (1)	7.81
A-B	0 / 28	-91.8	-91.8 (1)	P-K	-182 / 0	0.09 (1)	10.00
B-C	-53 / 0	-91.8	-91.8 (1)	R-J	-183 / 0	0.20 (1)	6.25
C-D	-18 / 0	-91.8	-91.8 (1)	S-H	-183 / 0	0.14 (1)	6.25
D-E	-19 / 0	-91.8	-91.8 (1)	T-G	-182 / 0	0.09 (1)	6.25
E-F	-14 / 0	-91.8	-91.8 (1)	U-F	-183 / 0	0.06 (1)	6.25
F-G	-11 / 0	-91.8	-91.8 (1)	V-E	-179 / 0	0.04 (1)	6.25
G-H	-8 / 0	-91.8	-91.8 (1)	W-D	-195 / 0	0.03 (1)	10.00
H-I	-6 / 0	-91.8	-91.8 (1)	X-C	-71 / 0	0.01 (1)	10.00
I-J	-6 / 0	-91.8	-91.8 (1)				
J-K	-4 / 0	-91.8	-91.8 (1)				
K-L	-2 / 0	-91.8	-91.8 (1)				
L-M	-7 / 0	-91.8	-91.8 (1)				
M-N	-83 / 0	0.0	0.0 (1)				6.25
Y-X	0 / 24	-18.5	-18.5 (1)				10.00
X-W	0 / 21	-18.5	-18.5 (1)				10.00
W-V	0 / 16	-18.5	-18.5 (1)				10.00
V-U	0 / 12	-18.5	-18.5 (1)				10.00
U-T	0 / 10	-18.5	-18.5 (1)				10.00
T-S	0 / 7	-18.5	-18.5 (1)				10.00
S-R	0 / 6	-18.5	-18.5 (1)				10.00
R-Q	0 / 4	-18.5	-18.5 (1)				10.00
Q-P	0 / 4	-18.5	-18.5 (1)				10.00
P-O	0 / 3	-18.5	-18.5 (1)				10.00
O-N	0 / 1	-18.5	-18.5 (1)				10.00

TOTAL WEIGHT = 4 X 98 = 394 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (A-B:1), BC=0.05/1.00 (X-Y:1), WB=0.20/1.00 (J-R:1), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.38 (L) (INPUT = 0.90)
JSI METAL = 0.08 (L) (INPUT = 1.00)



Structural component only
DWG# T-2016964

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
411573	P100	24	1	TRUSS DESC.		

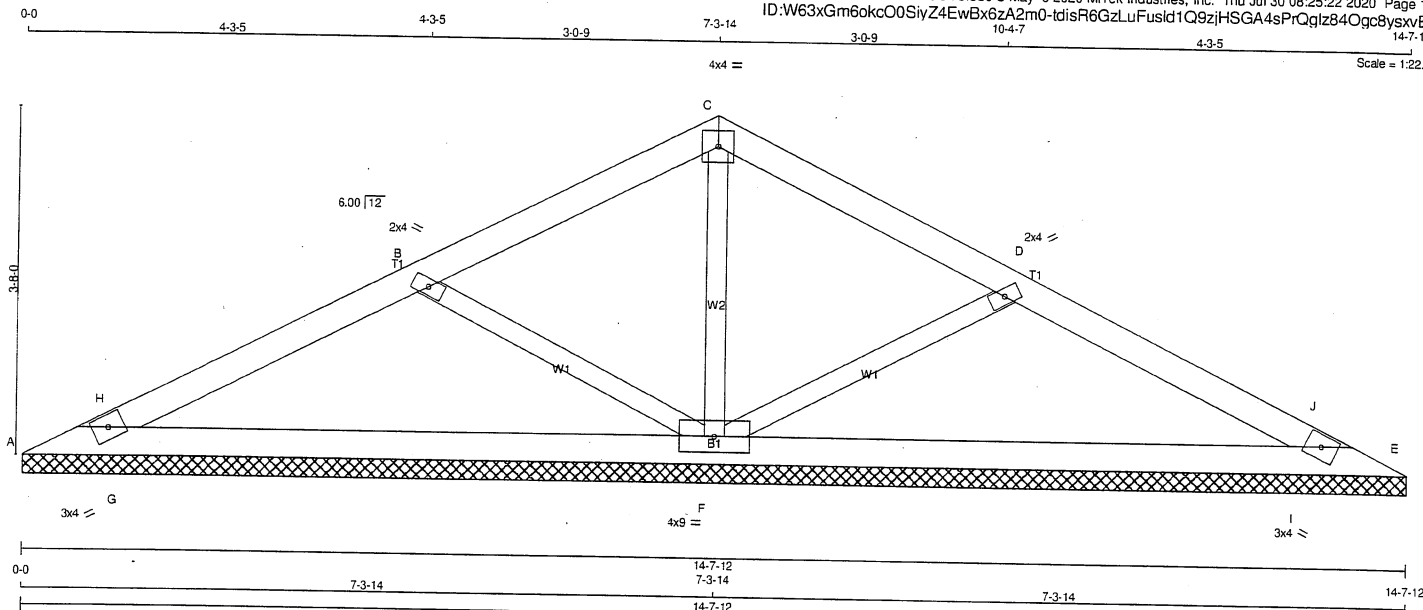
Tamarack Roof Truss, Burlington

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14-7-12

Scale = 1:22.3



LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY

C - E 2x4 DRY

A - E 2x4 DRY

ALL WEBS 2x3 DRY

DRY: SEASONED LUMBER.

LUMBER

No.2

No.2

No.2

No.2

No.2

DESCR.

SPF

SPF

SPF

SPF

SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	UP/LIFT	IN-SX
A	141	0	0	14-7-12
E	141	0	0	14-7-12
F	1332	0	0	14-7-12

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE			
A	98	74 / 0	0 / 0	0 / 0	24 / 0	0 / 0
E	98	74 / 0	0 / 0	0 / 0	24 / 0	0 / 0
F	945	601 / 0	0 / 0	0 / 0	344 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-H	0 / 143	-91.8 -91.8	0.13 (4)	F-C	-727 / 0	0.16 (1)	
H-B	0 / 210	-91.8 -91.8	0.23 (1)	F-D	-435 / 0	0.11 (1)	
B-C	0 / 577	-91.8 -91.8	0.29 (1)	B-F	-435 / 0	0.11 (1)	
C-D	0 / 577	-91.8 -91.8	0.29 (1)	G-H	-22 / 60	0.00 (1)	
D-J	0 / 210	-91.8 -91.8	0.23 (1)	I-J	-22 / 60	0.00 (1)	
J-E	0 / 143	-91.8 -91.8	0.13 (4)				
A-G	-166 / 0	-18.5 -18.5	0.06 (1)				
G-F	-159 / 0	-18.5 -18.5	0.21 (4)				
F-I	-159 / 0	-18.5 -18.5	0.21 (4)				
I-E	-166 / 0	-18.5 -18.5	0.06 (1)				

TOTAL WEIGHT = 24 X 42 = 1006 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.29/1.00 (C-D:1), BC=0.21/1.00 (F-I:4), WB=0.16/1.00 (C-F:1), SSI=0.15/1.00 (D-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.45 (C) (INPUT = 0.90)

JSI METAL = 0.21 (C) (INPUT = 1.00)



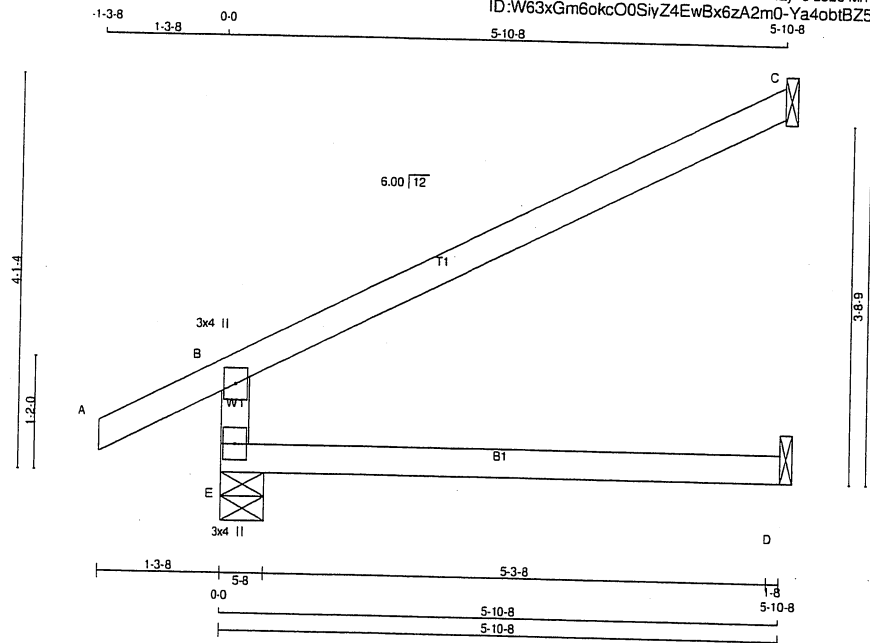
Structural component only
DWG# T-2016965

JOB NAME 410143	TRUSS NAME J1	QUANTITY 36	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:22.9



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	525	0	525	0	5-8	5-8
C	202	0	202	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	369	257.0	0.0	0.0	0.0	111.0	0.0
C	139	113.0	0.0	0.0	0.0	26.0	0.0
D	36	0.0	0.0	0.0	0.0	36.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MAX. UNBRACED LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)
E-B	-461.0	0.0	0.0	0.13 (4)	7.81	
A-B	0.28	-91.8	-91.8	0.12 (1)	10.00	
B-C	-30.0	-91.8	-91.8	0.54 (1)	6.25	
E-D	0.0	-18.5	-18.5	0.13 (4)	10.00	

TOTAL WEIGHT = 36 X 17 = 605 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")

CALCULATED VERT. DEFL.(LL) = L/999 (0.00")

ALLOWABLE DEFL.(TL) = L/360 (0.20")

CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.18 (E) (INPUT = 0.90)
JSI METAL = 0.13 (B) (INPUT = 1.00)

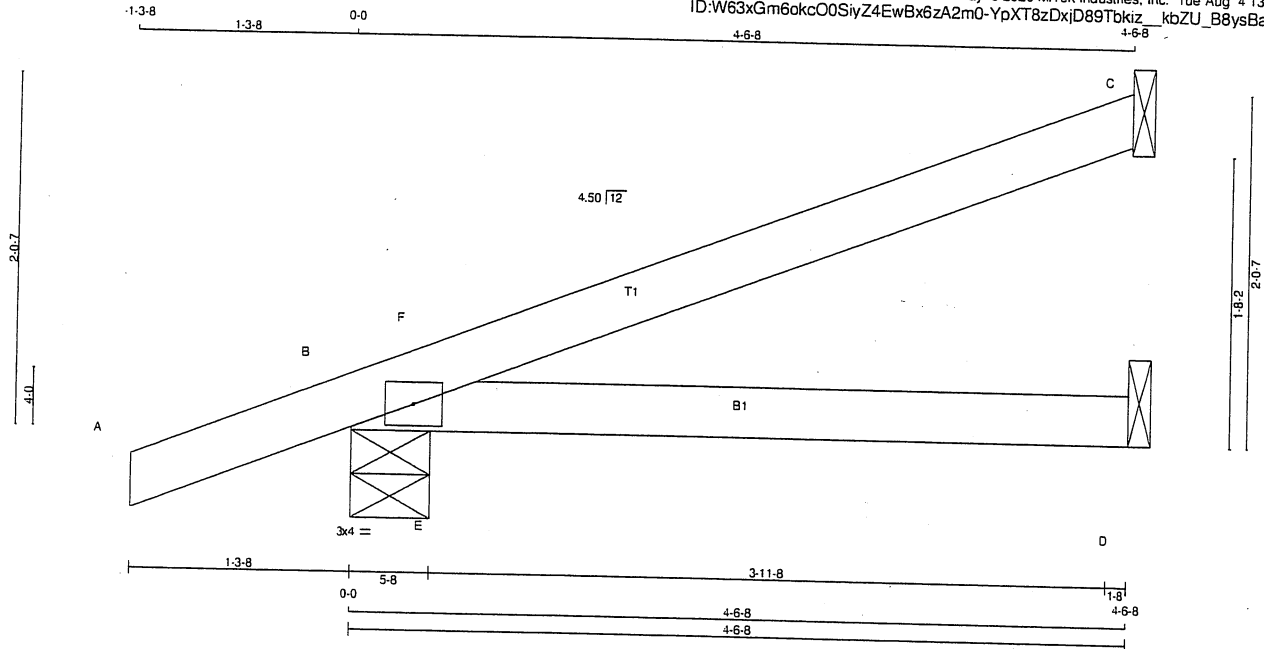


Structural component only
DWG# T-2017139

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410146	J2	8	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:12.7



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
C	181	0	181	0	1-8	1-8
B	374	0	374	0	5-8	5-8
D	69	0	69	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
C	125	97.0	0.0	0.0	28.0	0.0	0.0
B	262	185.0	0.0	0.0	77.0	0.0	0.0
D	52	19.0	0.0	0.0	33.0	0.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)		MAX. CS1 (LC)
	FR-TO	TO-TO	FROM	TO				FR-TO	TO-TO	
A-B	0.20		-91.8	-91.8	0.12 (1)	10.00	E-F	-229.8		0.00 (1)
B-F	-16.7		-91.8	-91.8	0.05 (4)	6.25				
F-C	0.2		-91.8	-91.8	0.24 (1)	10.00				
B-E	0.0		-18.5	-18.5	0.18 (1)	10.00				
E-D	0.0		-18.5	-18.5	0.18 (1)	10.00				

TOTAL WEIGHT = 8 X 12 = 99 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.24/1.00 (C-F:1), BC=0.18/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SSI=0.19/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

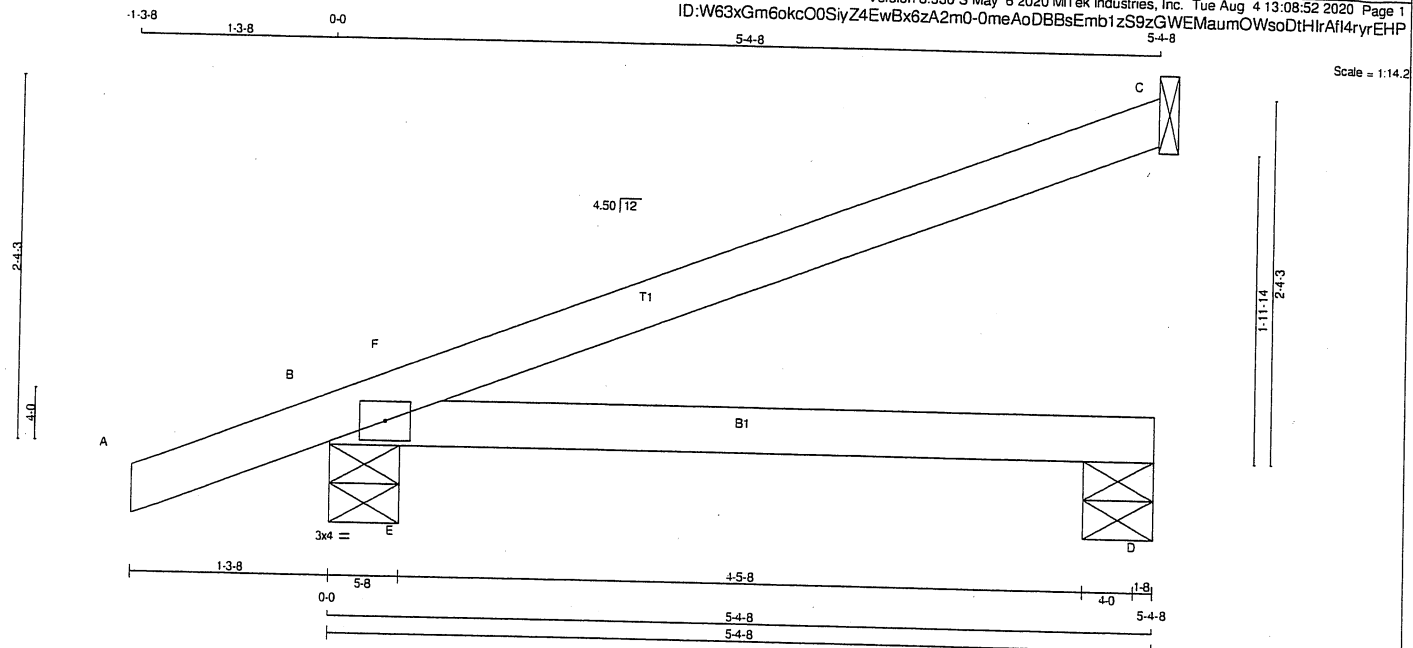


Structural component only
DWG# T-2017152

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
410143	J3	6	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY

B - D 2x4 DRY

LUMBER

No.2

No.2

DESCR.

SPF

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
C	215	0	215	0	1-8	1-8
B	420	0	420	0	5-8	5-8
D	81	0	81	0	5-8	5-8

SEE MITEK STANDARD DETAIL B97911H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	149	116.0	0.0	0.0	0.0	33.0	0.0
B	294	206.0	0.0	0.0	0.0	88.0	0.0
D	60	21.0	0.0	0.0	0.0	39.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0.20	-91.8	-91.8	0.12 (1)	10.00	E-F	-310.7
B-F	-20.23	-91.8	-91.8	0.06 (4)	6.25		0.00 (1)
F-C	-2.2	-91.8	-91.8	0.34 (1)	10.00		
B-E	0.0	-18.5	-18.5	0.24 (1)	10.00		
E-D	0.0	-18.5	-18.5	0.24 (1)	10.00		

TOTAL WEIGHT = 6 X 14 = 86 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6 PSF
	DL	= 6.0 PSF
BOT CH.	LL	= 0.0 PSF
	DL	= 7.4 PSF
TOTAL LOAD		= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/639$ (0.10")

CSI: TC=0.34/1.00 (C-F:1), BC=0.24/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SSI=0.26/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

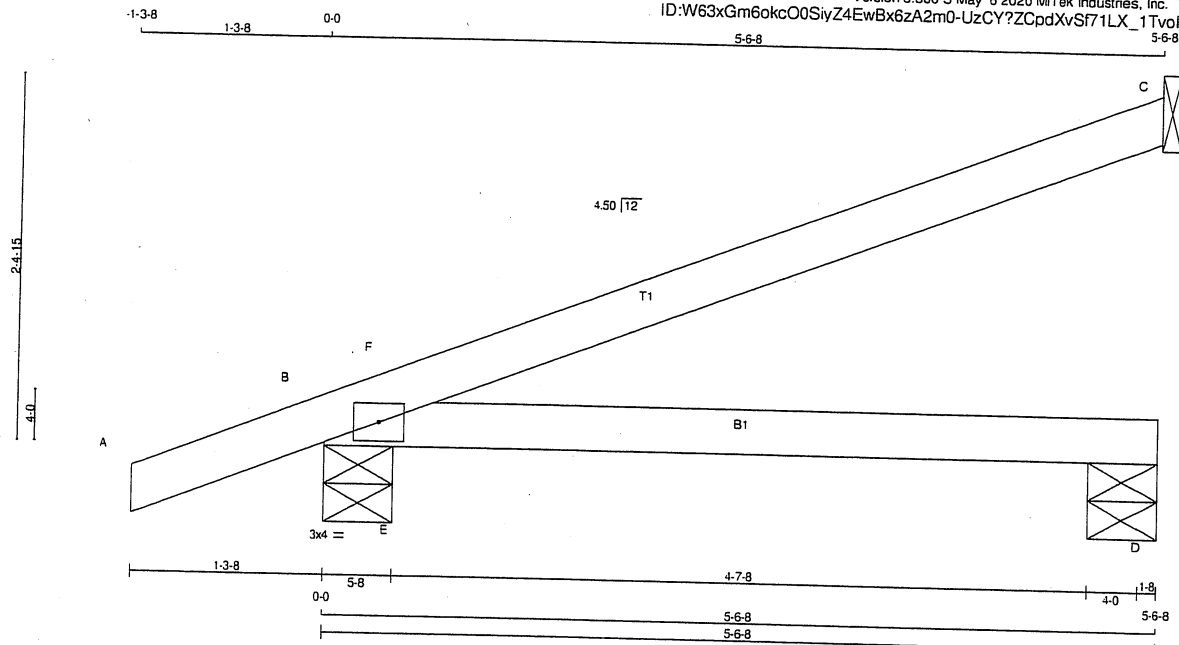


Structural component only
DWG# T-2017140

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410143	J20	10	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:14.5



LUMBER

N.L.G.A. RULES
CHORDS SIZE
A - C 2x4
B - D 2x4

LUMBER
No.2
No.2

DESCR.
SPF
SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-1	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	IN-SX	IN-SX
C	222	0	1-8	1-8
B	429	0	5-8	5-8
D	83	0	5-8	5-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED							
C	154	120.0	0.0	0.0	0.0	0.0	34.0	0.0
B	301	210.0	0.0	0.0	0.0	0.0	90.0	0.0
D	62	22.0	0.0	0.0	0.0	0.0	40.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0.20	-91.8 -91.8	0.12 (1)	10.00	E-F	-328.11	0.00 (1)
B-F	-21.26	-91.8 -91.8	0.06 (4)	6.25			
F-C	-3.2	-91.8 -91.8	0.37 (1)	10.00			
B-E	0.0	-18.5 -18.5	0.25 (1)	10.00			
E-D	0.0	-18.5 -18.5	0.26 (1)	10.00			

TOTAL WEIGHT = 10 X 15 = 148 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L 585 (0.11")

CSI: TC=0.37/1.00 (C-F:1), BC=0.26/1.00 (D-E:1),
WB=0.00/1.00 (E-F:1), SSI=0.27/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



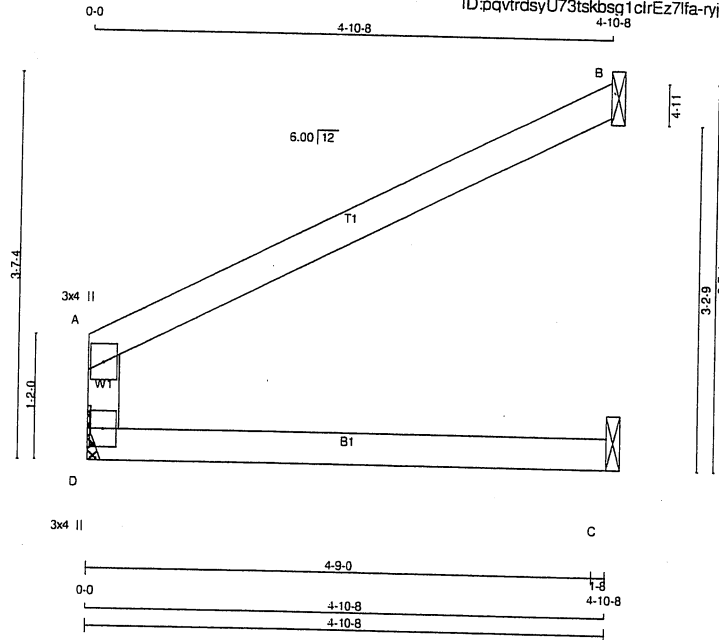
Structural component only
DWG# T-2017141

JOB NAME 410149	TRUSS NAME J31	QUANTITY 6	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:20.5



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
D - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
D - C	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
D	269	269	0	0
B	204	204	0	0
C	64	64	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	190	125 / 0	0 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0
B	141	112 / 0	0 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
C	49	13 / 0	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	FACTORED VERT. LOAD LC1 (PLF)	MAX. MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO										
D-A	-243 / 0	0.0	0.0	0.11 (1)	7.81					
A-B	-9 / 0	-91.8	-91.8	0.30 (1)	10.00					
D-C	0 / 0	-18.5	-18.5	0.15 (1)	10.00					

TOTAL WEIGHT = 6 X 13 = 76 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
DL	= 6.0	PSF	
BOT CH.	LL	= 0.0	PSF
DL	= 7.4	PSF	
TOTAL LOAD	= 39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.06")

CSI: TC=0.30/1.00 (A-B:1), BC=0.15/1.00 (C-D:1), WB=0.00/1.00 (n/a:0), SSI=0.17/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

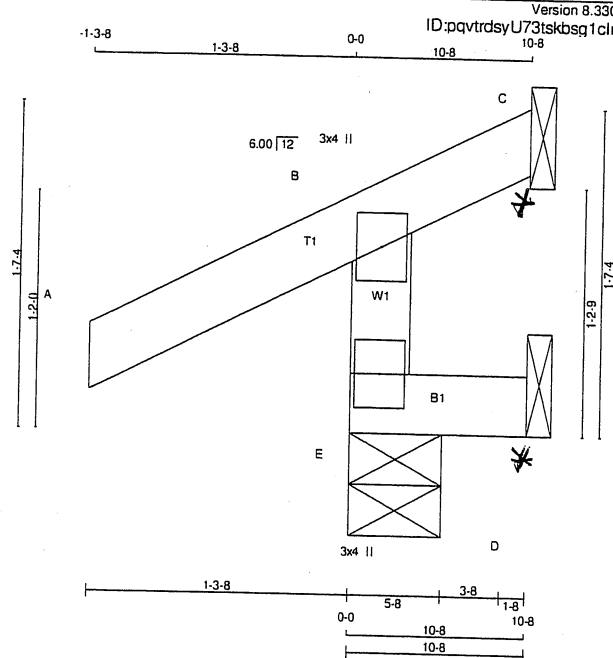
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.10 (D) (INPUT = 0.90)
JSI METAL = 0.07 (A) (INPUT = 1.00)



Structural component only
DWG# T-2017164

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410149	J32	8	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
E - B	2x4	DRY No.2
A - C	2x4	DRY No.2
E - D	2x4	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	269	0	269	0	5-8	5-8
C	-35	0	0	0	1-8	1-8
D	-13	0	5	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	186	146 / 0	0 / 0	0 / 0	0 / 0	41 / 0	0 / 0
C	-24	0 / -41	0 / 0	0 / 0	0 / 0	0 / -4	0 / 0
D	-8	0 / -13	0 / 0	0 / 0	0 / 0	3 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS			WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
E-B	-240 / 0	0.0	0.0	0.03 (5)	7.81			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-34 / 0	-91.8	-91.8	0.09 (1)	6.25			
E-D	0 / 0	-18.5	-18.5	0.03 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 8 X 5 = 38 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.03/1.00 (D-E:1)
WB=0.00/1.00 (n/a:0), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

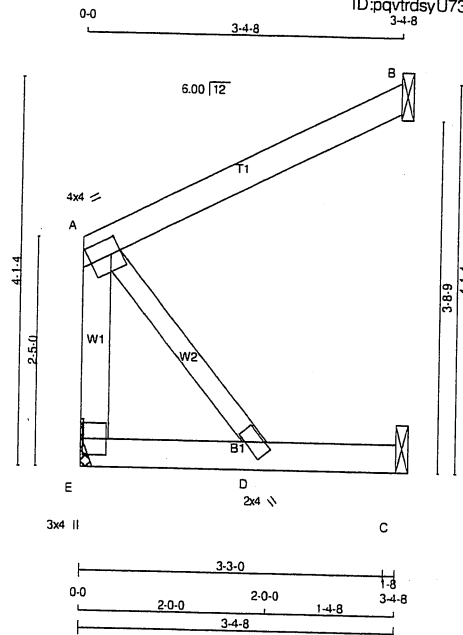


Structural component only
DWG# T-2017165

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410149	J33	8	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:23.3



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	BMW-w	MT20	2.0	4.0		
E	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
E	186	0	186	0	IN-SX	IN-SX
B	155	0	155	0	MECHANICAL	MECHANICAL
C	31	0	35	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B, C.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	132	86 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0
B	107	86 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0
C	25	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. MEMB. FORCE (LBS)	WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (LBS)	LC1	MAX. (PLF)		MAX. (LC)	UNBRACED LENGTH	FR-TO	MAX. FORCE (LBS)
FR-TO									
E-A	-155 / 0	0.0	0.0	0.02 (1)	7.81	A-D	0 / 0	0.00 (1)	
A-B	0 / 0	-91.8	-91.8	0.18 (1)	10.00				
E-D	0 / 0	-18.5	-18.5	0.06 (4)	10.00				
D-C	0 / 0	-18.5	-18.5	0.06 (4)	10.00				

TOTAL WEIGHT = 8 X 13 = 101 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.18/1.00 (A-B:1) , BC=0.06/1.00 (D-E:4) , WB=0.00/1.00 (A-D:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (A) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 1.00)

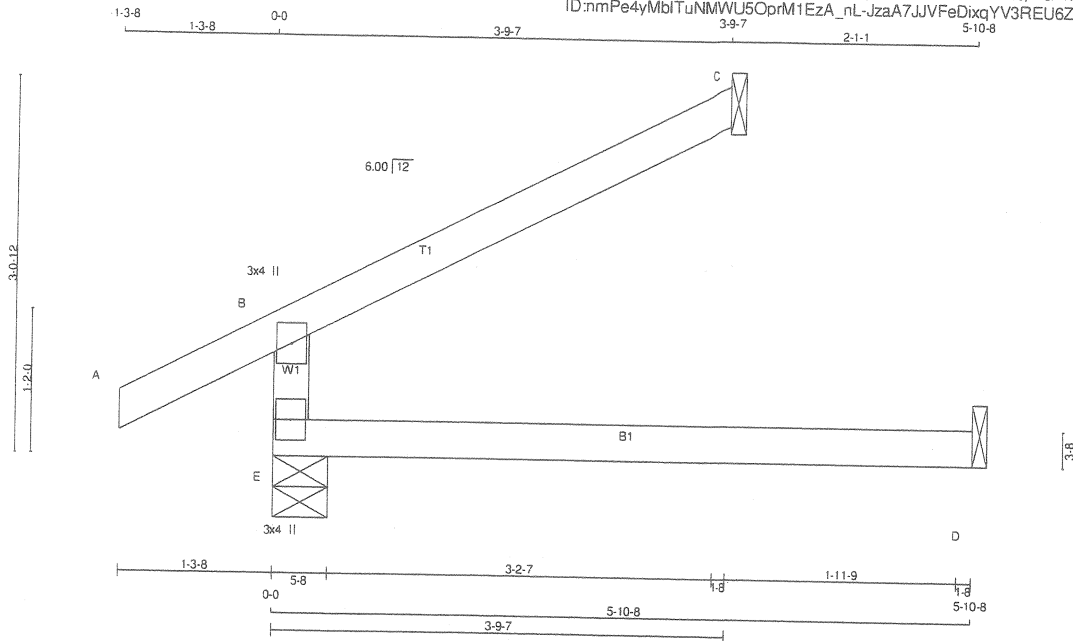


Structural component only
DWG# T-2017166

JOB NAME 409989	TRUSS NAME C30	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:nmPe4yMb1TuNMWU5OprM1EzA_nL-JzaA7JJVFeDixqYV3REU6ZS4WVg8qrgH4RSDmyqxMB

Scale = 1/17.9



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DESCR. SPF SPF SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	405	0	405	0	5-8	5-8
C	130	0	130	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	286	190 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		VERT LOAD	FACTORED		MAX	MAX	MEMB.	WEBS		MAX	MAX
	MAX.	FACTORED		FORCE	PLF	CSI (LC)	UNBRAC		MAX.	FACTORED		
FR-TO	(LBS)						LENGTH	FR-TO	(LBS)			CSI (LC)
E-B	-342	0	0.0	0.0	0.13 (4)	7.81						
A-B	0	28	-91.8	-91.8	0.12 (1)	10.00						
B-C	-19	0	-91.8	-91.8	0.22 (1)	6.25						
E-D	0	0	-18.5	-18.5	0.13 (4)	10.00						

TOTAL WEIGHT = 4 X 14 = 57 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	5.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")

CALCULATED VERT. DEFL.(LL) = L/999 (0.00")

ALLOWABLE DEFL.(TL)= L/360 (0.20")

CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.22/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1 10
COMP=1.10 SHEAR=1 10 TENS=1 10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

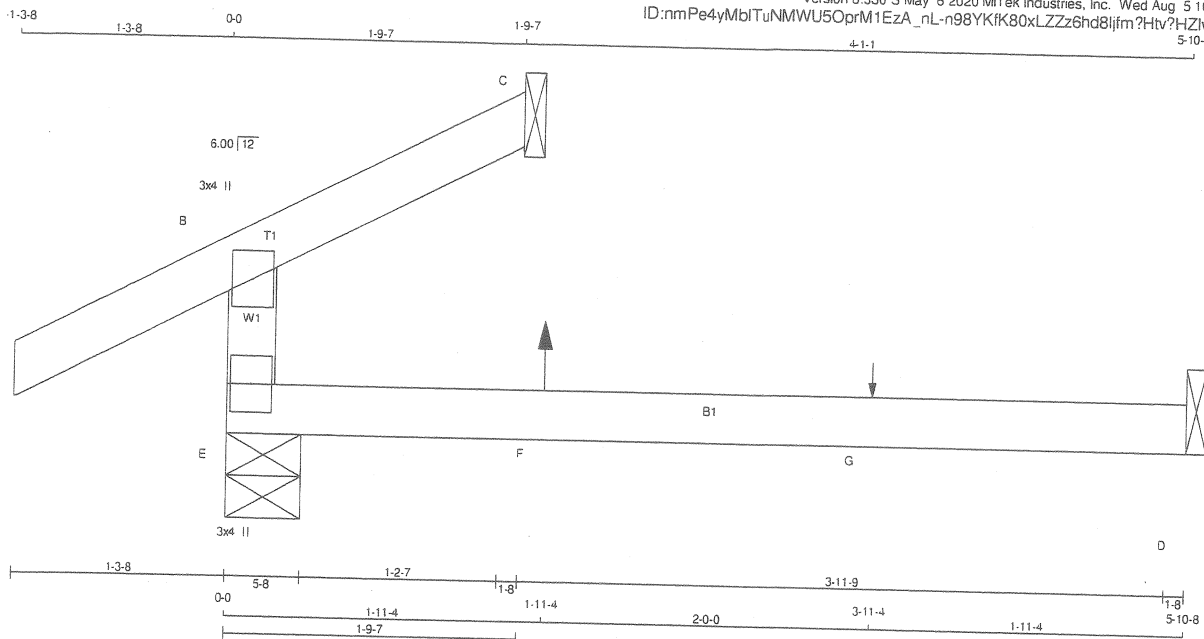
JSI GRIP= 0.14 (E) (INPUT = 0.90)

JSI METAL= 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2017359

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
409989	C31	4	1	TRUSS DESC.		
Tamarack Roof Truss, Burlington						



Scale = 1:13.1

LUMBER		
N. L. G. A. RULES		
CHORDS	SIZE	LUMBER
E - B	2x4	DRY
A - C	2x4	DRY
E - D	2x4	DRY

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	284	0	284	0	5-8	5-8
C	63	0	63	0	1-8	1-8
D	44	0	52	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
E	200	137.0	0.0	0.0	0.0	0.0	0.0
C	46	21.0	0.0	0.0	0.0	25.0	0.0
D	35	0.3	0.0	0.0	0.0	37.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. LC1	MAX. UNBRACED LENGTH FR-TO	MEMB. MAX. FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO	MAX. FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO
FR-TO								
E-B	-227	0	0.0	0.0 0.11 (4)	7.81			
A-B	0	28	-91.8	-91.8 0.12 (1)	10.00			
B-C	-9	9	-91.8	-91.8 0.08 (4)	10.00			
E-F	0	0	-18.5	-18.5 0.14 (4)	10.00			
F-G	0	0	-18.5	-18.5 0.14 (4)	10.00			
G-D	0	0	-18.5	-18.5 0.14 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	FRONT	VERT	---	C1
G	3-11-4	1	1	---	FRONT	VERT	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 12 = 46 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.14/1.00 (D-E:4) ,
WB=0.00/1.00 (n/a:0) , SS=0.09/1.00 (A-B:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

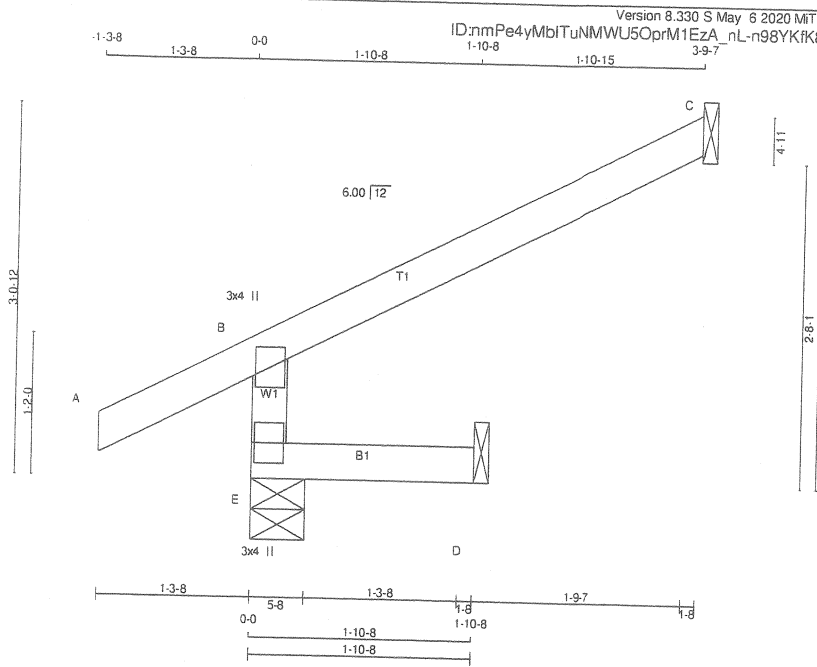
JSI GRIP = 0.09 (E) (INPUT = 0.90)

JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2017360

JOB NAME 409989	TRUSS NAME C32	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	361	0	361	0	5-8	5-8
C	130	0	130	0	1-8	1-8
D	16	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	250	190 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB. MAX. FORCE (LBS)	FACTORED MAX. CSI (LC)	FR-TO
FR-TO		FROM	TO		FR-TO		
E-B	-342 0	0.0	0.0	0.01 (4)	7.81		
A-B	0 / 28	-91.8	-91.8	0.13 (5)	10.00		
B-C	-19 0	-91.8	-91.8	0.22 (1)	6.25		
E-D	0 0	-18.5	-18.5	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 10 = 38 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.22/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

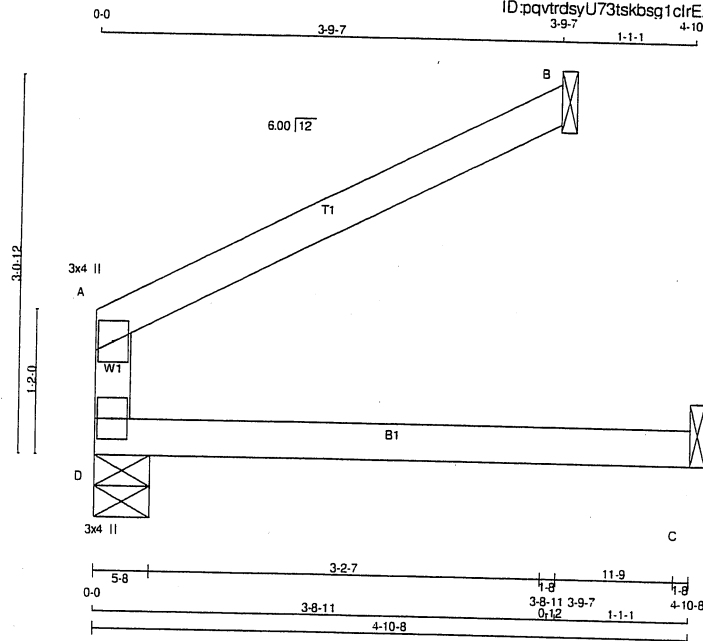


Structural component only
DWG# T-2017361

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410149	C34	4	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:17.8



TOTAL WEIGHT = 4 X 11 = 45 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
D - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
D - C	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
D	221	0	221	0	5-8	5-8
B	164	0	164	0	1-8	1-8
C	53	0	53	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B, C.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	157	99/0	0/0	0/0	0/0	58/0	0/0
B	114	88/0	0/0	0/0	0/0	26/0	0/0
C	41	7/0	0/0	0/0	0/0	34/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	MEMB. MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO		LENGTH	FR-TO	
D-A	-183/0	0.0	0.0	0.04 (1)	7.81	
A-B	-4/2	-91.8	-91.8	0.19 (1)	10.00	
D-C	0/0	-18.5	-18.5	0.11 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CS1: TC=0.19/1.00 (A-B:1), BC=0.11/1.00 (C-D:4),
WB=0.00/1.00 (n/a:0), SS=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.07 (D) (INPUT = 0.90)
JSI METAL= 0.05 (A) (INPUT = 1.00)

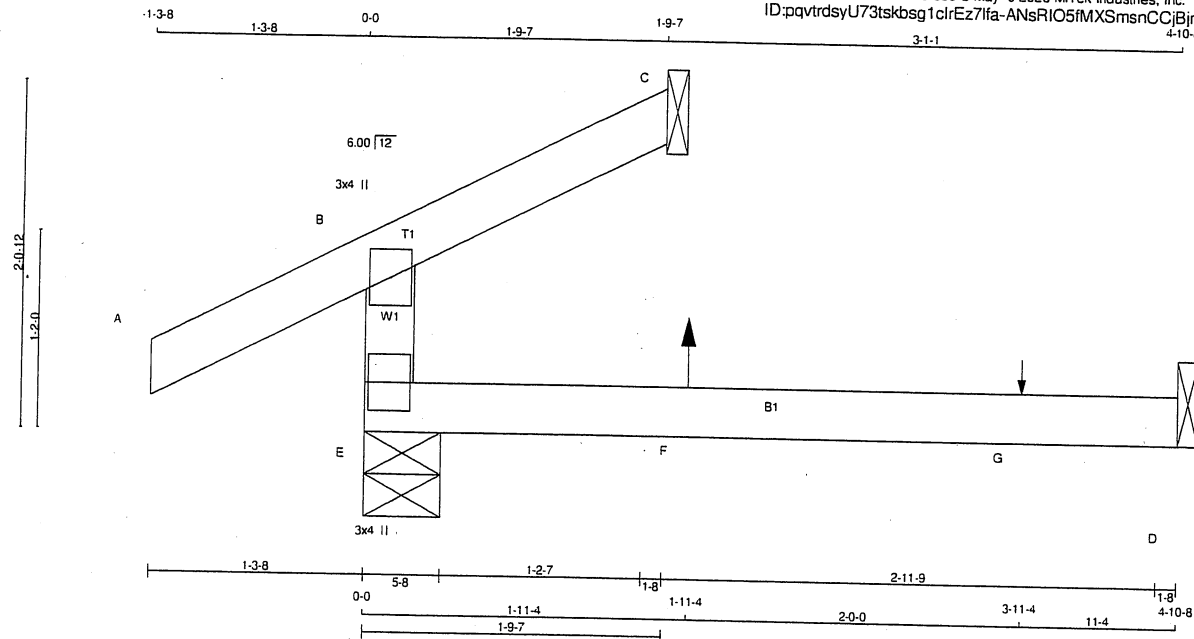


Structural component only
DWG# T-2017162

JOB NAME 410149	TRUSS NAME C35	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:pqvtrdsyU73tskbsg1clrEz7lfa-ANsRIO5fMXSmsnCCjBjmZ2kiN8Bz9JYtd7FBwyrDZ3
4-10-8

Scale = 1:13.1



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	282	0	282	0	5-8	5-8
C	55	0	55	0	1-8	1-8
D	35	0	44	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	198	137.0	0.0	0.0	0.0	61.0	0.0
C	39	21.0	0.0	0.0	0.0	18.0	0.0
D	29	0.4	0.0	0.0	0.0	31.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MAX	MEMB.	MAX. FORCE (LBS)	MAX
FR-TO						FR-TO		
E-B	-235.0	0.0	0.0	0.07 (4)	7.81			
A-B	0.28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-13.5	-91.8	-91.8	0.07 (5)	6.25			
E-F	0.0	-18.5	-18.5	0.10 (4)	10.00			
F-G	0.0	-18.5	-18.5	0.10 (4)	10.00			
G-D	0.0	-18.5	-18.5	0.10 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	BACK	VERT	TOTAL	--	C1
G	3-11-4	1	1	--	BACK	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

- C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 10 = 42 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.02")

CSI: TC=0.12/1.00 (A-B:1), BC=0.10/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)

JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2017163

LUL/LUS/LJS/HUS/HHUS/HGUS



Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

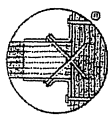
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

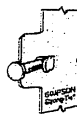
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



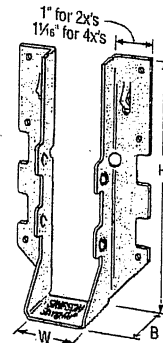
Double-Shear Nailing Top View



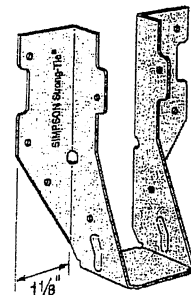
Double-Shear Nailing Side View; Do not bend tab



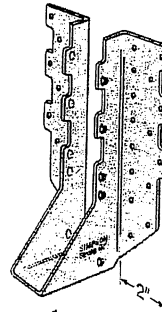
Dome Double-Shear Nailing Side View (available on some models)
U.S. Patent 5,603,580



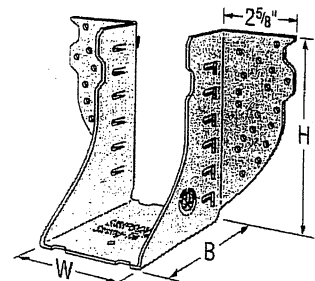
LUS28



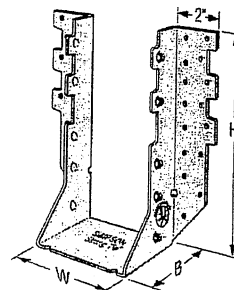
LU26L



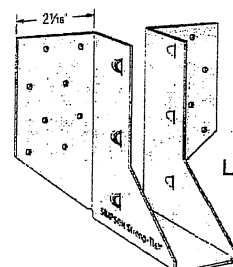
HUS210
(HUS26, HUS28, and HHUS similar)



HGUS28-2

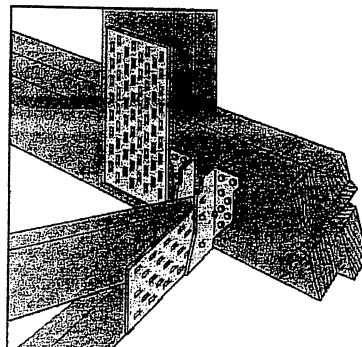


HHUS210-2



LJS26DS

Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)



LUL/LUS/LJS/HUS/HHUS/HGUS

HHUS/HGUS

See Hanger Options information on pp. 125–127.

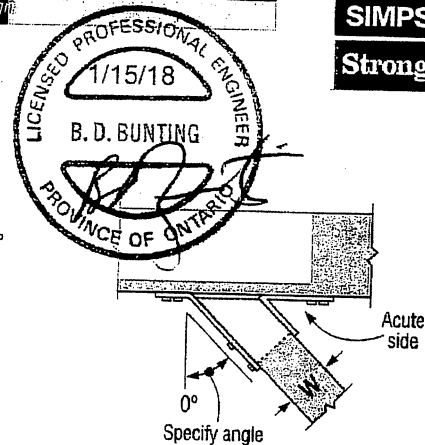
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
$W < 2"$	Bevel or square cut	0.62 of table value	0.46 of table value
$2" < W < 6"$	Bevel cut	0.67 of table value	0.41 of table value
$2" < W < 6"$	Square cut	0.46 of table value	0.41 of table value
$W > 6"$	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d_e^3	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								($K_D = 1.15$)	($K_D = 1.00$)	($K_D = 1.15$)	($K_D = 1.00$)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Single 2x Sizes											
■ LUS24	18	1 1/8	3 1/4	1 3/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155
								3.16	7.23	2.87	5.14
■ LU24L	22	1 1/8	3	1 1/4	2 1/8	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725
								1.60	4.54	1.42	3.22
■ LU26L	22	1 1/8	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"	720	1605	645	1140
								3.20	7.14	2.87	5.07
SS LUS26	18	1 1/8	4 1/4	1 3/4	3 3/4	(4) 10d	(4) 10d	1420	2170	1290	1630
								6.32	9.65	5.74	7.25
■ HUS26	16	1 1/8	5 3/4	3	3 1/8	(14) 16d	(6) 16d	2705	4940	2065	3875
								11.30	21.97	9.20	17.24
■ LJS26DS	18	1 1/8	5	3 1/2	4 1/4	(16) 16d	(6) 16d	2055	4265	1460	4115
								9.14	18.97	6.49	18.31
■ HGUS26	12	1 1/8	5 3/4	5	4 1/4	(20) 16d	(8) 16d	2685	6625	2685	5700
								11.96	29.51	11.96	25.35
■ LU28L	20	1 1/8	6 3/4	1 1/4	5 7/8	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1550
								5.07	9.72	4.54	6.89
SS LUS28	18	1 1/8	6 3/4	1 3/4	3 3/4	(6) 10d	(4) 10d	1420	2520	1290	1790
								6.32	11.21	5.74	7.96
■ HUS28	16	1 1/8	7 1/4	3	6 1/8	(22) 16d	(8) 16d	3605	5365	2675	4345
								16.04	23.86	11.90	19.33
■ HGUS28	12	1 1/8	7 1/4	5	6 1/4	(36) 16d	(12) 16d	3310	7675	3310	6900
								14.74	34.19	14.74	30.73
■ LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(6) 10d x 1 1/2"	1140	2495	1020	1770
								5.07	11.10	4.54	7.87
SS LUS210	18	1 1/8	7 1/4	1 3/4	3 3/4	(8) 10d	(4) 10d	1420	2785	1290	2210
								6.32	12.39	5.74	9.83

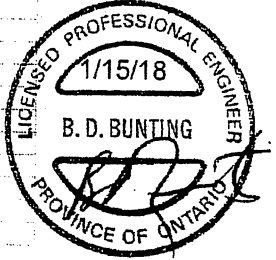
1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _e ³	Header	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
LUS24-2	18	3 1/8	3 1/8	2	1 1/2	(4) 16d	(2) 16d	835	2020	590	1435
SS LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	3.71	8.99	2.62	6.38
HHUS26-2	14	3 1/8	5 3/8	3	3 15/16	(14) 16d	(6) 16d	1720	2595	1545	1920
HGUS26-2	12	3 1/8	5 7/8	4	4 1/8	(20) 16d	(8) 16d	7.65	11.54	6.87	8.54
SS LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	2850	7335	2065	5205
HHUS28-2	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	12.68	32.63	9.20	23.15
HGUS28-2	12	3 1/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d	4385	8950	3110	6355
SS LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	19.51	39.81	13.83	28.27
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	1720	3325	1545	2575
HGUS210-2	12	3 1/8	9 3/8	4	8 1/8	(46) 16d	(16) 16d	7.65	14.79	6.87	11.45
								3765	8940	2675	6345
								16.75	39.77	11.90	28.22
								6070	12980	4310	9215
								27.00	57.74	19.17	40.99
SS LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	11.48	20.02	10.32	14.21
HGUS210-2	12	3 1/8	9 3/8	4	8 1/8	(46) 16d	(16) 16d	4670	9660	4235	7000
								20.77	42.97	18.84	31.14
								6840	14015	4855	10270
								30.43	62.34	21.60	45.69
Triple 2x Sizes											
HGUS26-3	12	4 15/16	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
HGUS28-3	12	4 15/16	7 1/8	4	6 3/8	(36) 16d	(12) 16d	19.51	39.81	13.83	28.27
HHUS210-3	14	4 11/16	9	3	7 15/16	(30) 16d	(10) 16d	6070	12980	4310	9215
HGUS210-3	12	4 15/16	9 1/8	4	8 3/8	(46) 16d	(16) 16d	27.00	57.74	19.17	40.99
								4670	9670	4235	6865
								20.77	43.02	19.84	30.54
								6840	14645	4855	10400
								30.43	65.14	21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	6 3/8	5 7/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
HGUS28-4	12	6 3/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d	19.51	39.81	13.83	28.27
HHUS210-4	14	6 1/8	8 3/8	3	7 15/16	(30) 16d	(10) 16d	6070	12980	4310	9215
HGUS210-4	12	6 3/8	9 3/8	4	8 1/8	(46) 16d	(16) 16d	27.00	57.74	19.17	40.99
								4670	10155	4235	7210
								20.77	45.17	18.84	32.07
								6840	14645	4855	10400
								30.43	65.14	21.60	46.26
HGUS212-4	12	6 3/8	10 3/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 3/8	12 3/8	4	11 1/8	(66) 16d	(22) 16d	33.98	66.70	24.13	47.35
								10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
LUS46	18	3 1/8	4 3/4	2	3 3/8	(4) 16d	(4) 16d	1720	2595	1545	1920
HHUS46	14	3 1/8	5 1/4	3	3 15/16	(14) 16d	(6) 16d	7.65	11.54	6.87	8.54
HGUS46	12	3 1/8	5 1/4	4	4 1/8	(20) 16d	(8) 16d	2540	7335	2065	5205
LUS48	18	3 1/8	6 3/4	2	3 3/8	(6) 16d	(4) 16d	11.30	32.63	9.20	23.15
HHUS48	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	4385	8950	3110	6355
HGUS48	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	19.51	39.81	13.83	28.27
LUS410	18	3 1/8	8 3/4	2	5 3/8	(8) 16d	(6) 16d	1720	3325	1545	2575
HHUS410	14	3 1/8	9	3	8 1/8	(30) 16d	(10) 16d	7.65	14.79	6.87	11.45
HGUS410	12	3 1/8	9	4	8 1/8	(46) 16d	(16) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
								6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS412	18	3 1/8	10 3/4	2	6 3/8	(12) 16d	(8) 16d	2580	4500	2320	3195
HGUS412	12	3 1/8	10 3/8	4	10 1/8	(56) 16d	(20) 16d	11.48	20.02	10.32	14.21
HGUS414	12	3 1/8	12 3/8	4	11 1/8	(66) 16d	(22) 16d	4670	9660	4235	7000
								20.77	42.97	18.84	31.14
								6840	14015	4855	10270
								30.43	62.34	21.60	45.69
								7640	14995	5425	10645
								33.98	66.70	24.13	47.35
								10130	16400	7195	11645
								45.06	72.95	32.00	51.80



TC – Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1/4". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

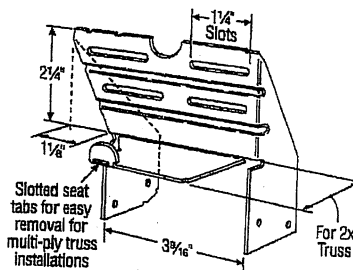
Design: Factored resistances are in accordance with CSA 086-14

Installation:

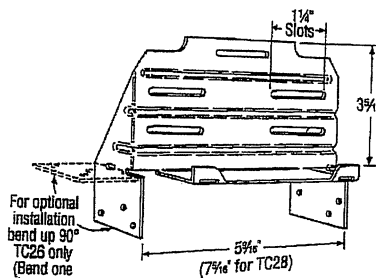
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

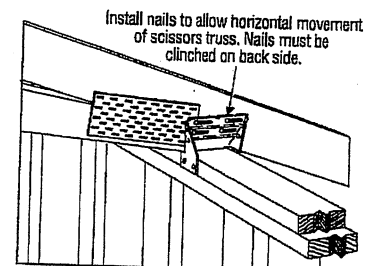
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



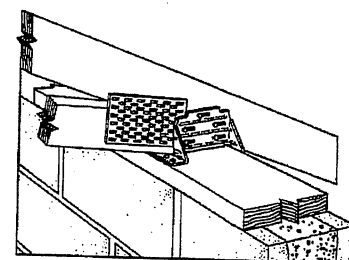
TC24
U.S. Patent 4,932,173



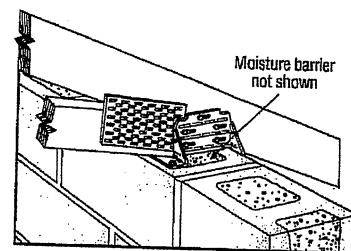
TC26
(TC28 Similar)



Typical TC24 Installation



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer (8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K ₀ =1.15)	Uplift (K ₀ =1.15)
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K ₀ =1.15)	Uplift (K ₀ =1.15)
TC26	(5) 10d	(6) 10d x 1 1/2"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) - 3/8" x 2 1/4" Titen screws has a factored uplift resistance of 275 lb.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2019, and the information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

H/TSP

SIMPSON
Strong-Tie

Seismic and Hurricane Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Fasteners			Factored Resistance ($K_D = 1.15$)					
					D.Fir-L			S-P-F		
		To Rafters/ Truss	To Plates	To Studs	Uplift	Lateral		Uplift	Lateral	
						F ₁	F ₂		F ₁	F ₂
					lb.	lb.	lb.	lb.	lb.	lb.
					kN	kN	kN	kN	kN	kN
H1	18	(6) 8d x 1 1/2"	(4) 8d	—	740	685	300	680	485	215
					3.29	3.05	1.33	3.02	2.16	0.96
SS H2A	18	(5) 8d x 1 1/2"	(2) 8d x 1 1/2"	(5) 8d x 1 1/2"	830	220	75	590	155	55
					3.69	0.98	0.33	2.62	0.69	0.24
SS H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
					3.58	0.71	0.71	3.36	0.71	0.71
H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
					3.71	0.78	0.93	3.29	0.71	0.93
SS H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
					3.29	0.80	1.18	2.74	0.56	0.85
H6	16	—	(8) 8d	(8) 8d	1585	1085	—	1125	770	—
					7.05	4.83	—	5.00	3.43	—
H7Z	16	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
					6.18	2.98	—	4.40	2.11	—
SS H8 ³	18	(5) 10d x 1 1/2"	(5) 10d x 1 1/2"	—	1120	—	—	1025	—	—
					4.98	—	—	4.56	—	—
SS H10A ⁹	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1735	795	410	1505	565	290
					7.72	3.54	1.82	6.69	2.51	1.29
H10AR	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1485	690	430	1220	570	305
					6.61	3.07	1.91	5.43	2.54	1.36
H10A-2	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1835	1275	430	1645	880	305
					8.16	5.67	1.91	7.32	3.91	1.36
H10S ^{7,8}	18	(8) 8d x 1 1/2"	(8) 8d x 1 1/2"	(8) 8d	1465	795	315	1040	565	225
					6.52	3.54	1.40	4.63	2.51	1.00
H11Z	18	(6) 16d x 2 1/2"	(6) 16d x 2 1/2"	—	1095	920	545	780	655	390
					4.87	4.09	2.42	3.47	2.91	1.73
H14	18	[1] (12) 8d x 1 1/2"	(13) 8d	—	2390	855	320	1805	610	230
		[2] (12) 8d x 1 1/2"	(15) 8d	—	10.63	3.80	1.42	8.03	2.71	1.02
					2390	855	320	1805	610	230
TSP	16	(9) 10d x 1 1/2"	(6) 10d x 1 1/2"	—	10.63	3.80	1.42	8.03	2.71	1.02
					1295	440	—	920	310	—
		(9) 10d x 1 1/2"	(6) 10d	—	5.76	1.96	—	4.09	1.38	—
					1560	440	—	1105	310	—
					6.94	1.96	—	4.92	1.38	—

- Factored resistances have been increased 15% for short term loading; no further increase is allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2 1/2" must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
- H8 factored uplift resistances for stud-to-bottom plate installations are 595 lb. (2.65 kN) for D.Fir-L and 390 lb. (1.74 kN) for S-P-F.
- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-HTIECONPATH).
- Factored resistances in the F₁ direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.

- H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.38 kN) D.Fir-L and 1015 lb. (4.51 kN) S-P-F.
- H10S nails to plates are optional for uplift but required for lateral loads.
- H10A may be field-bent up to a slope of 6/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
- The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SSNAILS at strongtie.com.
- D.Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1 1/2" nails into the top plates and (3) 8d x 1 1/2" nails into the lowest three flange holes into the truss bottom chord is 495 lb. (2.20 kN).
- Nails: 16d x 2 1/2" = 0.162" dia. x 2 1/2" long, 10d = 0.148" dia. x 3" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 8d = 0.131" dia. x 2 1/2" long, 8d x 1 1/2" = 0.131" dia. x 1 1/2" long. See pp. 27–28 for other nail sizes and information.

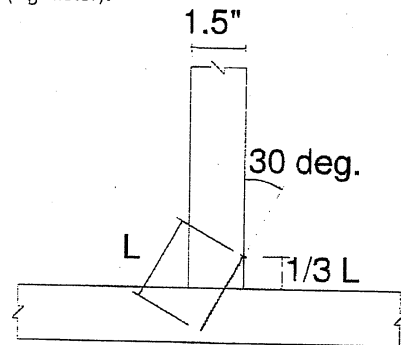
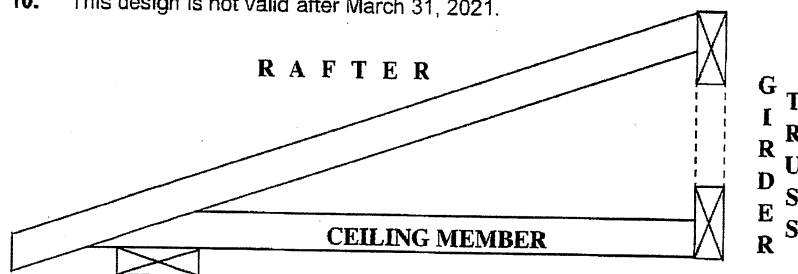
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.

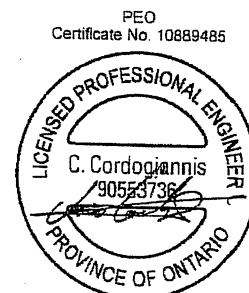


TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	{ 3.5" nail }		{ 3" and 3.25" nail }	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

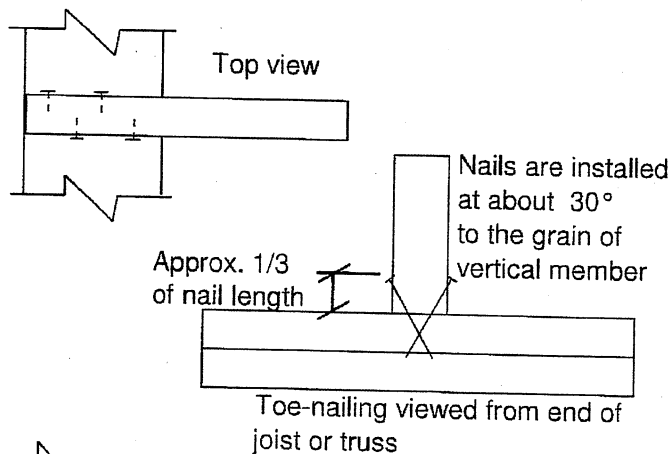
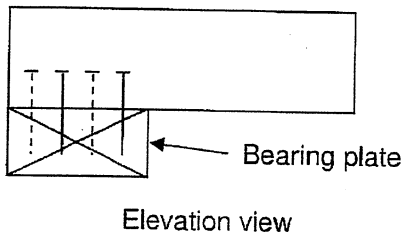
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON	3.00	0.144	30	42
	3.25	0.144	32	45
WIRE	3.50	0.160	38	52
	3.00	0.122	26	36
COMMON	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

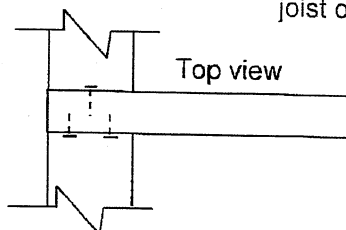
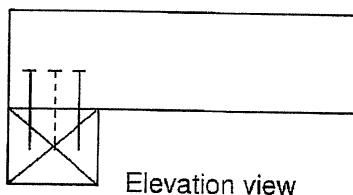
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



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100 Industrial Rd.
Bradford, Ontario L3Z 3G7

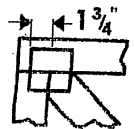
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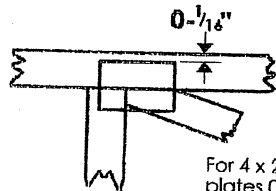
December 2, 2019

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

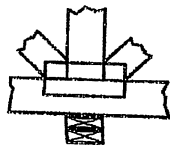
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

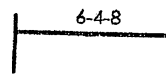


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

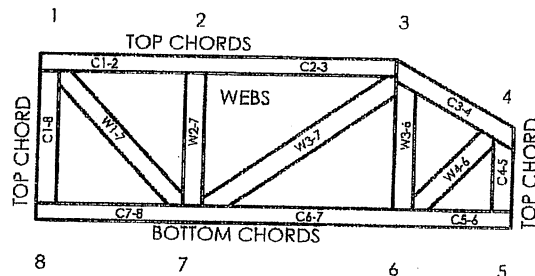
Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm
(Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

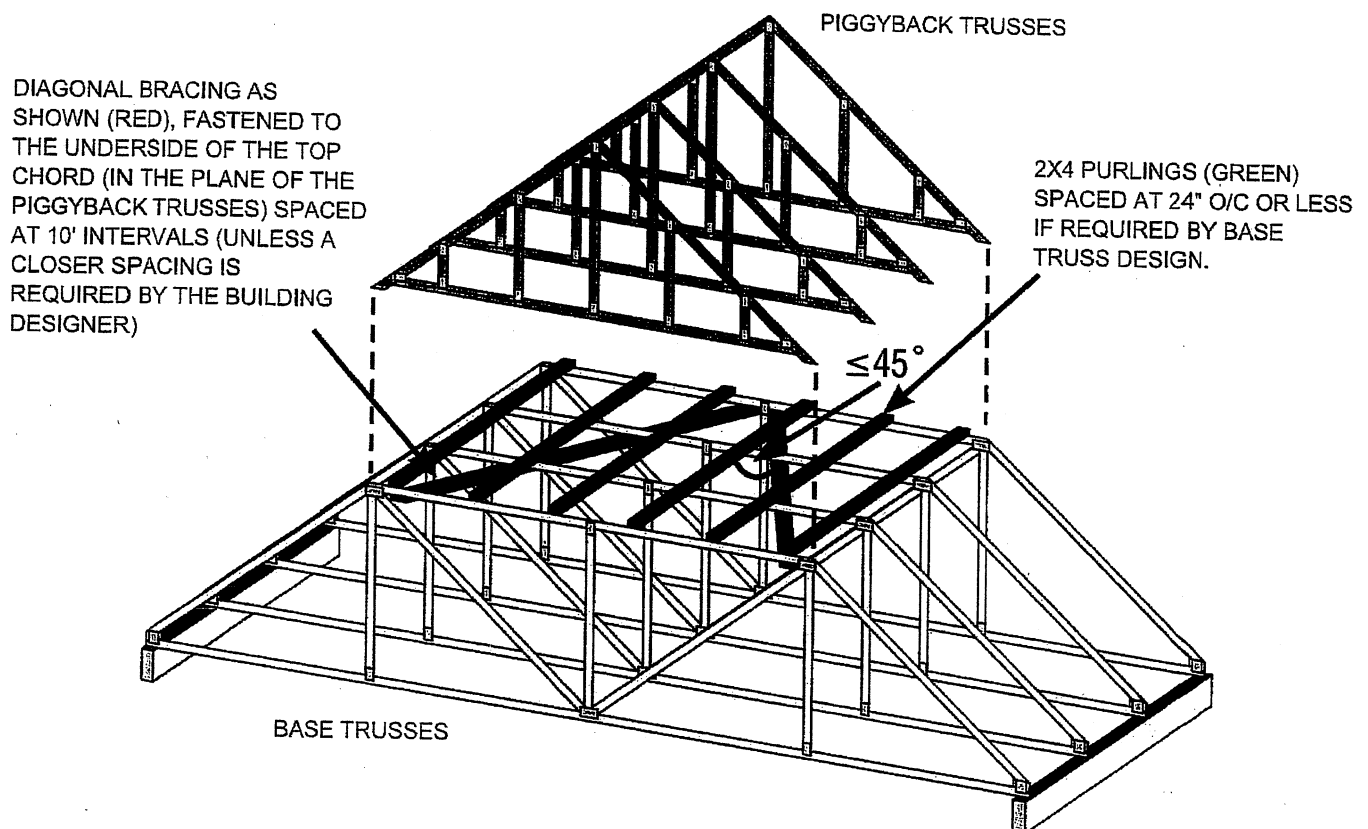
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

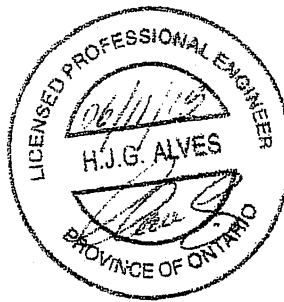


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900219

Feb 09, 2018