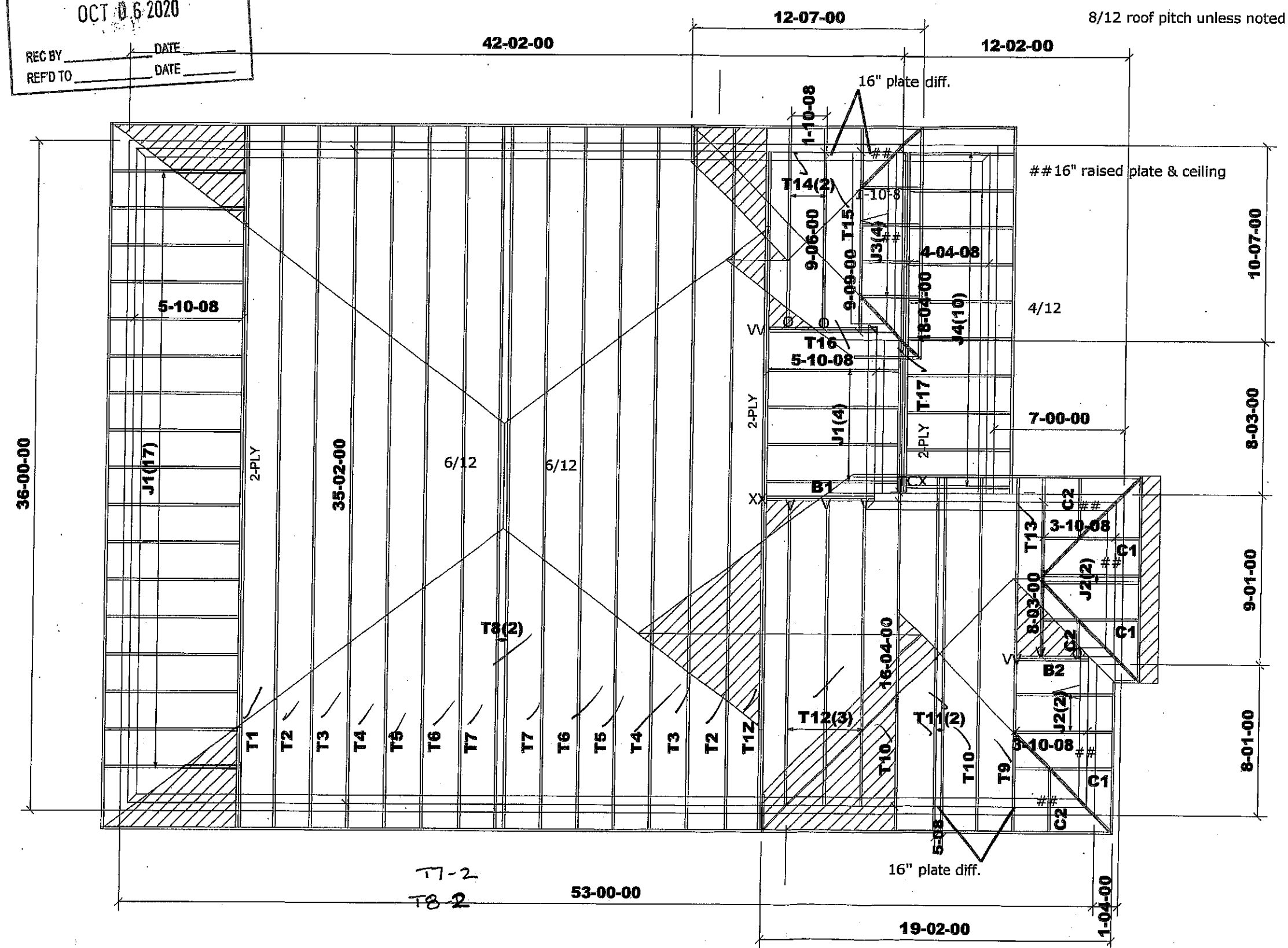


CITY OF HAMILTON
BUILDING DIVISION
Planning & Development Department

OCT 06 2020

REC BY _____ DATE _____
REF'D TO _____ DATE _____



ASPHALT SHINGLES
 FINISHED OVERHANG: 12"
 2x6 EXTERIOR WALLS
 2x6 FASCIA BOARD
 HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment).
 Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY:
 RESIDENTIAL | PART: 9
 Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
 TCSL = 25.6 psf
 TCDL = 6.0 psf
 BCLL = 0.0 psf
 BCDL = 7.4 psf

CITY OF HAMILTON
 Building Division
 Permit No. 196110
 THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE
 THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW
 These drawings and/or specifications have been reviewed by
 JDL DEC 15 2020
 FOR CHIEF BUILDING OFFICIAL DATE

HARDWARE:
 LUS24 - (O)
 LJS26DS - (V)
 HGUS26-2 - (XX)
 LUS26-2-(VV)

BEAMS:
 B2 = 2 - 2x10 SPF #2

DENOTES:
 CONVENTIONAL
 FRAMING

TAMARACK
ROOF TRUSSES INC.

Job Track: 51225
Plan Log: 202395
Layout ID: 408131

Builder / Location:
GREEN PARK HOMES / WATERDOWN
Project: RUSSELL GARDENS PH.3
Date: 2020-04-27 Sales: Mario DiCano Designer: JG

Model / Elevation:
MOUNTAINASH 3/1 -STD OR OPT.5 BR
Milek ver 8.3.1.215
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

AUG 25 2021

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:

TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

CITY OF HAMILTON
Building Division

Permit No. 20-186110-01

HARDWARE:

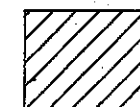
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
LUS26-2-(VV)

THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE
THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW

These drawings and/or specifications have been reviewed by
SEP 28 2021

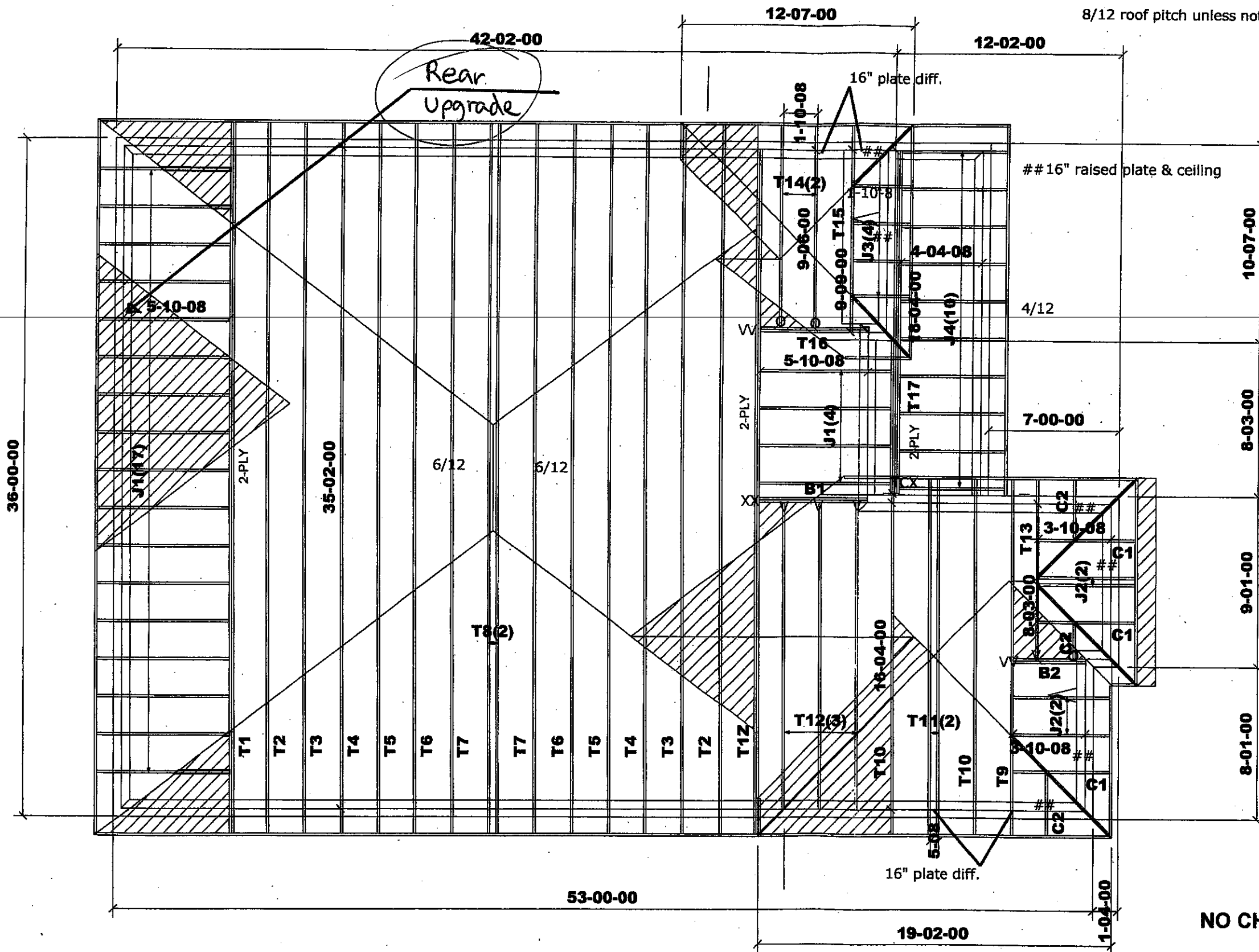
BEAMS:

B2 = 2 - 2x10 SPF #2



DENOTES:
CONVENTIONAL
FRAMING

NO CHANGES TO TRUSSES



Job Track: 51225
Plan Log: 202395
Layout ID: 408131

Builder / Location: GREEN PARK HOMES / WATERDOWN
Project: RUSSELL GARDENS PH. 4
Date: 2021-04-01 Sales: Mario DiCano Designer: JG

Model / Elevation:

MOUNTAINASH 3/1 -STD OR OPT.5 BR (+DECK CONDITION WITH

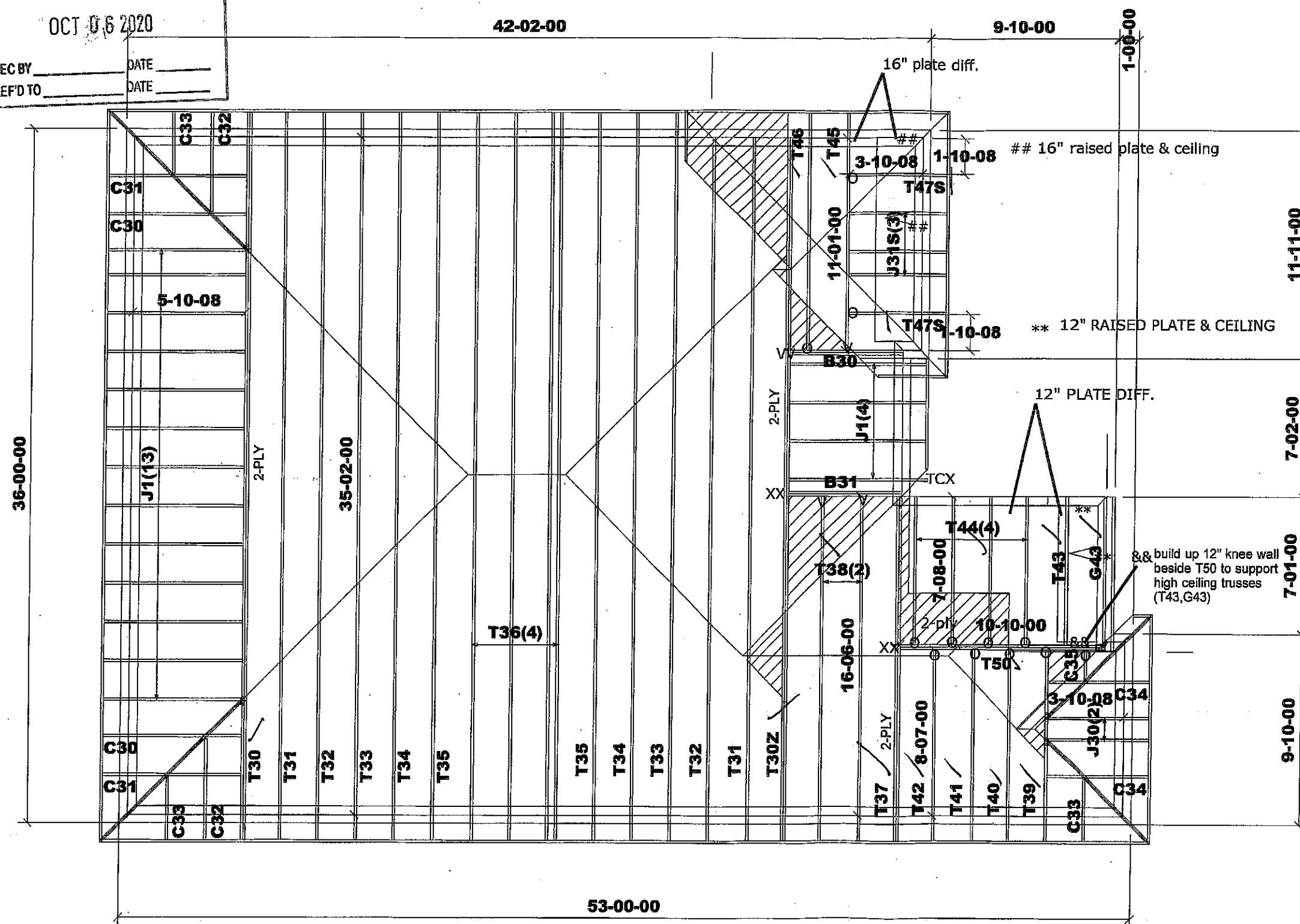
Model ver 8.4.2.286

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

REAR UPGRADE

OCT 06 2020

REC BY _____ DATE _____
REF'D TO _____ DATE _____



All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:

TCSL = 25.6 psf

TCDL = 6.0 psf

BCLL = 0.0 psf

BCDL = 7.4 psf

CITY OF HAMILTON
Building Division

HARDWARE:

LUS24 - (O)

Permit No. 196111

LJS26DS - (V)

THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE

HGUS26-2 - (XX)

THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW

These drawings and/or specifications have been reviewed by

BEAMS:

B30 = B31 =

IDE

DEC 15 2020

FOR CHIEF BUILDING OFFICIAL

DATE _____

2 - 2x10 SPF #2

 **DENOTES:
CONVENTIONAL
FRAMING**

M13124

Job Track: **51225**Plan Log: **202395**

Layout ID: 408133

Builder / Location:

GREEN PARK HOMES / WATERDOWN

Project: **RUSSELL GARDENS PH.3**

Date: 2020-04-27

Sales: **Mario DiCaro**

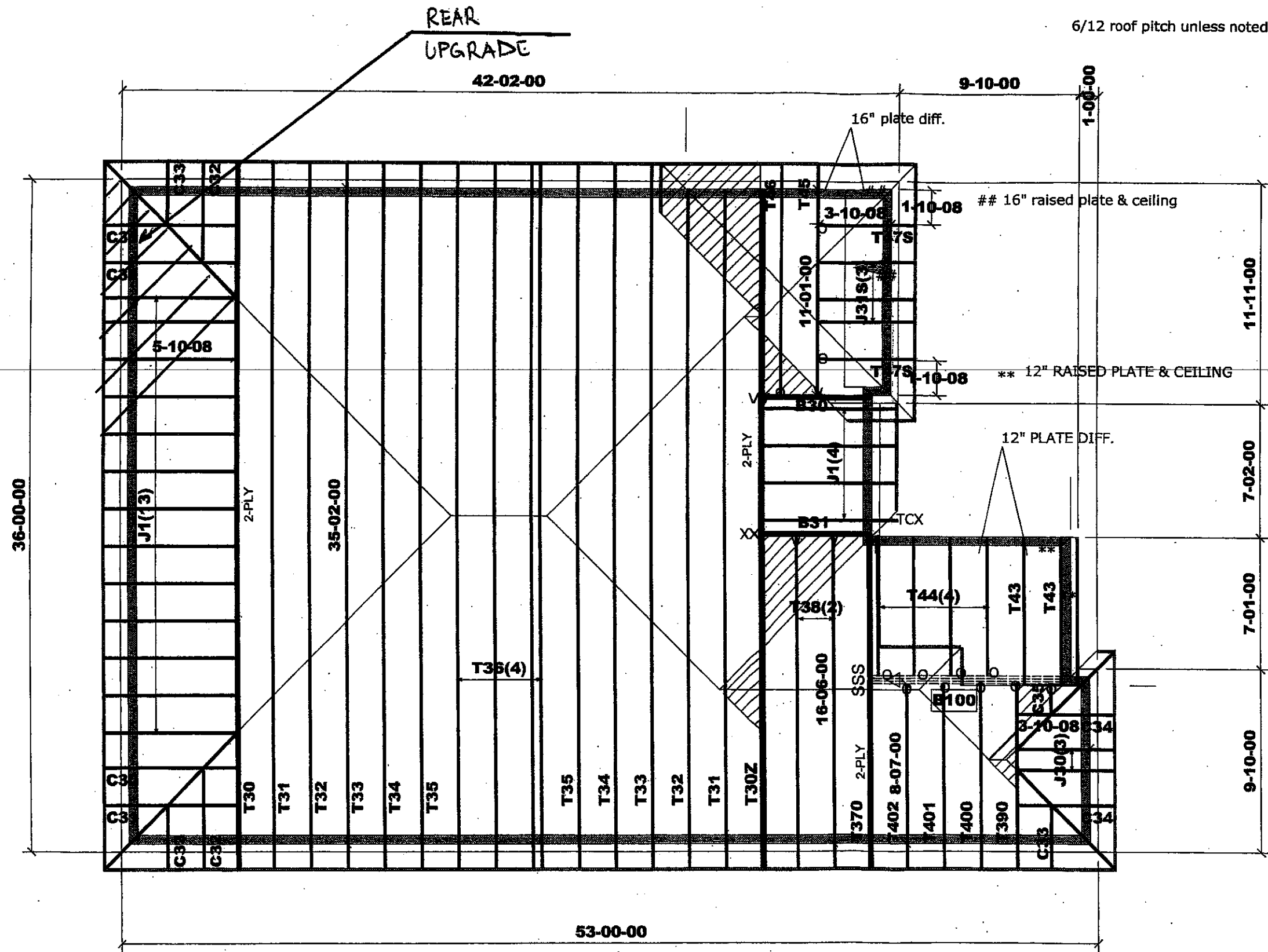
Designer: JG

Model / Elevation:

MOUNTAINASH 3/3- STD OR OPT.5 BR

nrdek ver 8 3 1 215

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

CITY OF HAMILTON
BUILDING DIVISION
Planning & Development Department

AUG 25 2021

REC BY _____ DATE _____
REF'D TO _____ DATE _____

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS5.50/8 (SSS)
LUS26-2-(VV)
HGUS26-2 - (XX)

BEAMS:
B30 = B31 =
2 - 2x10 SPF #2
B100 = 3 - 1 3/4" X 11 7/8" LVL

 DENOTES:
CONVENTIONAL
FRAMING

CITY OF HAMILTON
Building Division

Permit No. 20-196110-02
THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE
THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH
THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW
These drawings and/or specifications have been reviewed by

 **SEP 28 2021**
FOR CHIEF BUILDING OFFICIAL DATE



Job Track: **51225**
Plan Log: **202395**
Layout ID: **417866**

Builder / Location: **GREEN PARK HOMES / WATERDOWN**
Project: **RUSSELL GARDENS PH.3**
Date: 2021-08-11 Sales: **Mario DiCamo** Designer: **xx**

Model / Elevation: **MOUNTAINASH 3 / 3 (STD)**

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

(+DECK CONDITION WITH
REAR UPGRADE)

CITY OF HAMILTON
BUILDING DIVISION
Planning & Development Department

AUG 25 2021

REC BY _____ DATE _____
REF'D TO _____ DATE _____

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS5.50/8 (SSS)
LUS26-2-(VV)
HGUS26-2 - (XX)

BEAMS:
B30 = B31 =
2 - 2x10 SPF #2
B100 = 3 - 1 3/4" X 11 7/8" LVL

CITY OF HAMILTON
Building Division
20-196110-02

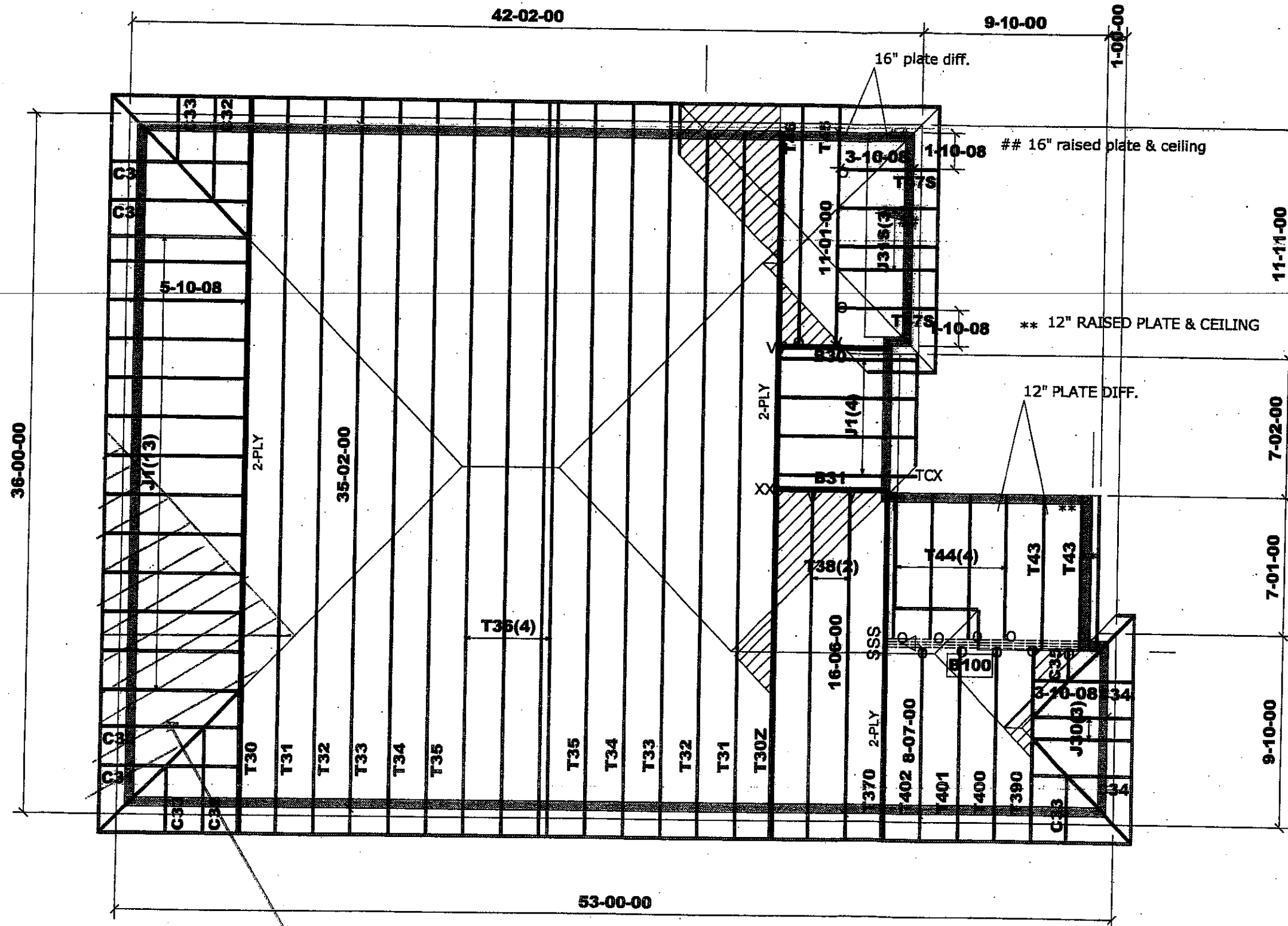
Permit No. 28 150 178

THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE

THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH
THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW

These drawings and/or specifications have been reviewed by _____

SD SEP 28 2021
FOR CHIEF BUILDING OFFICIAL DATE

 **DENOTES:
CONVENTIONAL
FRAMING**

Rear
Upgrade



Job Track:	51225
Plan Log:	202395
Layout ID:	417866

Builder / Location:		
GREEN PARK HOMES / WATERDOWN		
Project: RUSSELL GARDENS PH.3		
Date: 2021-08-11	Sales: Mario DiCano	Designer: xx

Model / Elevation: **MOUNTAINASH 3 / 3 (OPT. 582D) / DECK CONDITION** Mitek ver 8.4.2.288

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF IT IS DETERMINED THAT SUCH



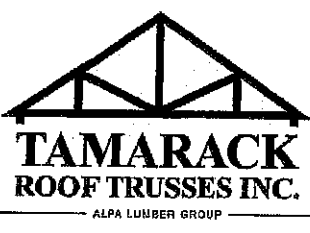
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 3
 Lot #:
 Elevation: 1-STD OR OPT.5 BR

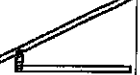
Job Track: 51225
 PlanLog: 202395
 Layout ID: 408131
 Ref #
 Page: 1 of 2
 Date: 04-27-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	8/12	35-02-00	4-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	359.46 218.67		
	1 2-ply	T1Z Hip Girder	8/12	35-02-00	4-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	359.48 218.67		
	2	T2 Hip	8/12	35-02-00	5-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	279.73 177.00		
	2	T3 Hip	8/12	35-02-00	6-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	302.74 191.33		
	2	T4 Hip	8/12	35-02-00	7-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	309.1 195.67		
	2	T5 Hip	8/12	35-02-00	8-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	316.76 196.67		
	2	T6 Hip	8/12	35-02-00	9-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	329.25 205.00		
	2	T7 Hip	8/12	35-02-00	10-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	343.15 215.33		
	2	T8 Hip	8/12	35-02-00	11-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	332.52 206.67		
	1	T9 Hip Girder	8/12	16-04-00	3-11-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	77.68 49.67		
	2	T10 Hip	8/12	16-04-00	6-07-13	2 x 4	1-03-08 1-03-08	2-08-13 2-08-13	149.72 97.33		
	2	T11 Hip	8/12	16-04-00	7-11-13	2 x 4	1-03-08 1-03-08	2-08-13 2-08-13	161.86 103.67		
	3	T12 Common	8/12	16-04-00	7-06-02	2 x 4		1-04-13 2-08-13	214.34 135.00		
	1	T13 Hip Girder	8/12	8-03-00	3-11-13	2 x 4	1-03-08	1-04-13 1-04-13	34.02 22.00		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST			
	Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: RUSSELL GARDENS PH.3 Location: WATERDOWN Model: MOUNTAINASH 3 Lot #: Elevation: 1-STD OR OPT.5 BR	Job Track: 51225 Plan Log: 202395 Layout ID: 408131 Ref #: Page: 2 of 2 Date: 04-27-2020 Designer: Sales Rep: Mario DiCano		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T14 Hip	8/12	9-06-00	5-03-13	2 x 4	1-03-08	2-08-13 2-10-13	99.24 66.00		
	1	T15 Hip Girder	8/12	9-09-00	2-07-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	42.84 27.83		
	1 2-ply	T16 Flat Girder	0/12	5-10-08	1-04-00	2 x 4		1-04-00 1-04-00	40.07 26.67		
	1 2-ply	T17 Flat Girder	0/12	18-04-00	1-09-07	2 x 6 2 x 4		1-09-07 1-09-07	149.42 94.00		
	21	J1 Jack-Open	6/12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	352.64 224.00		
	4	J2 Jack-Open	8/12	3-10-08	3-11-13	2 x 4	1-03-08	1-04-13 3-11-13	56.68 36.00		
	4	J3 Jack-Open	8/12	1-10-08	2-07-13	2 x 4	1-03-08	1-04-13 2-07-13	37.17 22.67		
	10	J4 Jack-Open	4/12	4-04-08	2-02-06	2 x 4	1-03-08	3-15 1-09-07	119.11 80.00		
	3	C1 Jack-Open	8/12	1-10-15	2-08-02	2 x 4	1-03-08 1-11-09	1-04-13 2-08-02	34.33 23.00		
	3	C2 Jack-Open	8/12	1-10-15	2-08-02	2 x 4	1-03-08 1-01	1-04-13 2-08-02	28.72 19.00		

TOTAL # TRUSS= 79

TOTAL BFT OF ALL TRUSSES= 2849.85

BFT.

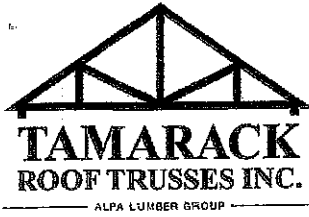
TOTAL WEIGHT OF ALL TRUSSES 4530.03 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
2	Hardware	LJS26DS	
3	Hardware	LUS24	
3	Hardware	LUS26-2	

TOTAL NUMBER OF
ITEMS=

9



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 3
 Lot #:
 Elevation: 2-STD OR OPT.5 BR

Job Track: 51225
 PlanLog: 202395
 Layout ID: 408132
 Ref #
 Page: 1 of 2
 Date: 04-28-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	8/12	35-02-00	4-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	359.46 218.67		
	1 2-ply	T122 Hip Girder	8/12	35-02-00	4-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	359.48 218.67		
	2	T2 Hip	8/12	35-02-00	5-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	279.73 177.00		
	2	T3 Hip	8/12	35-02-00	6-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	302.74 191.33		
	2	T4 Hip	8/12	35-02-00	7-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	309.1 195.67		
	2	T5 Hip	8/12	35-02-00	8-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	316.76 196.67		
	2	T6 Hip	8/12	35-02-00	9-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	329.25 205.00		
	3	T7 Hip	8/12	35-02-00	10-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	514.72 323.00		
	1 2-ply	T17 Flat Girder	0/12	18-04-00	1-09-07	2 x 6 2 x 4		1-09-07 1-09-07	149.42 94.00		
	4	T20 Common	8/12	25-09-00	9-11-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	485.71 296.00		
	4	T21 Common	8/12	16-04-00	6-10-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	291.14 182.67		
	2	T21S Roof Spécial	8/12	16-04-00	6-10-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	144.83 94.67		
	1	V20 Valley	8/12	17-05-07	5-09-13	2 x 4			50.89 31.33		
	1	V21 Valley	8/12	14-05-07	4-09-13	2 x 4			40.92 26.50		

DELIVERY SHIPLIST		
 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	Lumber Yard: TAMARACK LUMBER	Job Track: 51225
	Builder: GREEN PARK HOMES	PlanLog: 202395
	Project: RUSSELL GARDENS PH.3	Layout ID: 408132
	Location: WATERDOWN	Ref #
	Model: MOUNTAINASH 3	Page: 2 of 2
	Lot #:	Date: 04-28-2020
Elevation: 2-STD OR OPT.5 BR	Designer:	Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	V22 Valley	8 /12	11-05-07	3-09-13	2 x 4			30 19.33		
	1	V23 Valley	8 /12	8-05-07	2-09-13	2 x 4			21.65 14.17		
	21	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	352.64 224.00		
	10	J4 Jack-Open	4 /12	4-04-08	2-02-06	2 x 4	1-03-08	3-15 1-09-07	119.11 80.00		

TOTAL # TRUSS= 64 TOTAL BFT OF ALL TRUSSES= 2788.68 BFT. TOTAL WEIGHT OF ALL TRSSES 4457.56 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LJS26DS	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF
ITEMS= 3



DELIVERY SHIPLIST

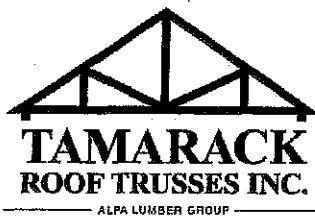
Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 3
 Lot #:
 Elevation: 3- STD OR OPT.5 BR

Job Track: 51225
 PlanLog: 202395
 Layout ID: 408133
 Ref #
 Page: 1 of 3
 Date: 04-27-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T30 Hip Girder	6 / 12	35-02-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	337.56 208.00		
	1 2-ply	T30Z Hip Girder	6 / 12	35-02-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	337.56 208.00		
	2	T31 Hip	6 / 12	35-02-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	277.96 174.00		
	2	T32 Hip	6 / 12	35-02-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	277.1 174.67		
	2	T33 Hip	6 / 12	35-02-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	289.06 179.67		
	2	T34 Hip	6 / 12	35-02-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	290.71 183.33		
	2	T35 Hip	6 / 12	35-02-00	9-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	301.64 191.33		
	4	T36 Common	6 / 12	35-02-00	9-11-08	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	606.72 372.00		
	1 2-ply	T37 Common Girder	6 / 12	16-06-00	5-03-08	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	158.36 98.00		
	2	T38 Common	6 / 12	16-06-00	5-03-08	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	132.54 82.67		
	1	T39 Hip Girder	6 / 12	8-07-00	3-01-04	2 x 4	1-03-08	1-02-00 1-04-08	36.28 24.50		
	1	T40 Half Hip	6 / 12	8-07-00	3-07-04	2 x 4	1-03-08	1-02-00 3-07-04	36.11 24.33		
	1	T41 Half Hip	6 / 12	8-07-00	4-07-04	2 x 4	1-03-08	1-02-00 4-07-04	40.96 27.50		
	1	T42 Common	6 / 12	8-07-00	5-03-08	2 x 4	1-03-08	1-02-00 5-01-08	37.97 24.00		

DELIVERY SHIPLIST



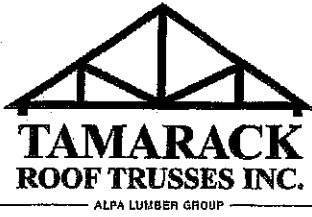
Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 3
 Lot #:
 Elevation: 3- STD OR OPT.5 BR

Job Track: 51225
 PlanLog: 202395
 Layout ID: 408133
 Ref #
 Page: 2 of 3
 Date: 04-27-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T43 Flat	0 / 12	7-08-00	2-08-00	2 x 4		2-08-00 2-08-00	30.48 21.17		
	1	G43 GABLE	0 / 12	7-08-00	2-08-00	2 x 4		2-08-00 2-08-00	27.33 19.17		
	4	T44 Flat	0 / 12	7-08-00	3-08-00	2 x 4		3-08-00 3-08-00	138.2 92.00		
	1	T45 Hip Girder	6 / 12	11-01-00	4-05-04	2 x 4	1-03-08 1-03-08	2-08-00 2-08-00	51.49 34.17		
	1	T46 Hip	6 / 12	11-01-00	5-00-08	2 x 4	1-03-08 1-03-08	2-06-00 2-06-00	51.04 34.33		
	2	T47S Half Hip	6 / 12	3-10-08	3-05-04	2 x 4	1-03-08	1-02-00 3-05-04	48.24 33.00		
	1 2-ply	T50 Half Hip Girder	6 / 12	10-10-00	5-00-00	2 x 4 2 x 6		1-02-00 5-00-00	109.49 73.00		
	17	J1 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	285.47 181.33		
	2	J30 Jack-Open	6 / 12	3-10-08	3-01-04	2 x 4	1-03-08	1-02-00 3-01-04	23.93 14.67		
	3	J31S Jack-Open	6 / 12	3-10-08	4-05-04	2 x 4	1-03-08	1-02-00 4-05-04	75.74 52.00		
	2	C30 Jack-Open	6 / 12	3-11-00	3-01-08	2 x 4	1-03-08 1-11-08	1-02-00 3-01-08	28.59 17.33		
	2	C31 Jack-Open	6 / 12	1-11-00	2-01-08	2 x 4	1-03-08 3-11-08	1-02-00 2-01-08	23.49 14.67		
	2	C32 Jack-Open	6 / 12	2-00-00	3-01-07	2 x 4	1-03-08 1-10-14	1-02-00 2-02-00	19.74 12.00		
	3	C33 Jack-Open	6 / 12	1-10-14	2-01-07	2 x 4	1-03-08 1-02	1-02-00 2-01-07	21.96 14.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: RUSSELL GARDENS PH.3
 Location: WATERDOWN
 Model: MOUNTAINASH 3
 Lot #:
 Elevation: 3- STD OR OPT.5 BR

Job Track: 51225
 PlanLog: 202395
 Layout ID: 408133
 Ref #
 Page: 3 of 3
 Date: 04-27-2020
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	C34 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 1-11-09	1-02-00 2-01-08	18.92 12.00		
	1	C35 Jack-Open	6 /12	1-07-00	2-01-08	2 x 4		1-04-08 2-02-00	4.91 3.33		

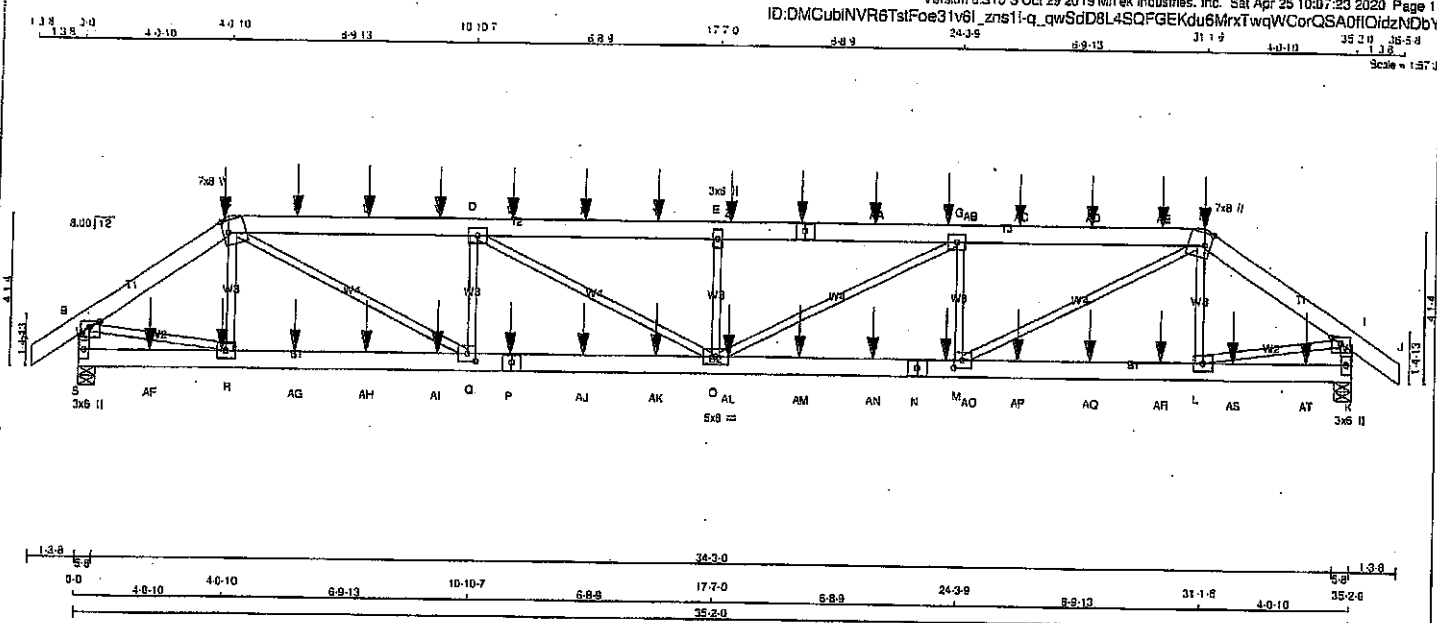
TOTAL # TRUSS= 72 TOTAL BFT OF ALL TRUSSES= 2600.17 BFT. TOTAL WEIGHT OF ALL TRSSES 4119.53 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
3	Hardware	LJS26DS	
12	Hardware	LUS24	
1	Hardware	LUS26-2	

TOTAL NUMBER OF
ITEMS= 18

JOB NAME 408131	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2
C - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - J	2x6	DRY	No.2
S - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
S - P	2x6	DRY	No.2
P - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

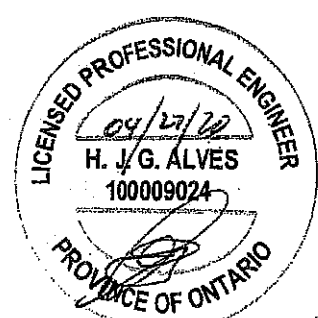
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 2	12	SIDE (122.0)
C-F 2	12	SIDE (61.0)
F-H 2	12	SIDE (61.0)
H-J 2	12	SIDE (122.0)
S-B 1	12	TOP
K-I 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S-P 2	12	SIDE (183.1)
P-N 2	12	SIDE (4.0)
N-K 2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.



Structural component only
DWG# T-2007017 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT 3397	DOWN 3397	0	5-8
S	3397	0	0	5-8
K	3346	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
S	2400	1588 / 0 0 / 0 0 / 0 0 / 0 812 / 0 0 / 0
K	2366	1557 / 0 0 / 0 0 / 0 0 / 0 809 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.92 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO	LENGTH FR-TO		
A-B	0 / 36	-91.8	-91.8 0.04 (1)	10.00	R-C	-518 / 0 0.06 (1)
B-C	-4152 / 0	-91.8	-91.8 0.09 (1)	5.52	C-Q	0 / 3920 0.49 (1)
C-T	-6692 / 0	-91.8	-91.8 0.33 (1)	4.29	Q-D	-1649 / 0 0.20 (1)
T-U	-6692 / 0	-91.8	-91.8 0.33 (1)	4.29	D-O	0 / 1172 0.15 (1)
U-V	-6692 / 0	-91.8	-91.8 0.33 (1)	4.29	O-E	-916 / 0 0.11 (1)
V-D	-6692 / 0	-91.8	-91.8 0.33 (1)	4.29	E-G	0 / 1203 0.15 (1)
D-W	-7922 / 0	-91.8	-91.8 0.36 (1)	4.02	M-H	-1670 / 0 0.20 (1)
W-X	-7922 / 0	-91.8	-91.8 0.36 (1)	4.02	M-H	0 / 3563 0.49 (1)
X-Y	-7922 / 0	-91.8	-91.8 0.36 (1)	4.02	L-H	-501 / 0 0.06 (1)
Y-E	-7922 / 0	-91.8	-91.8 0.36 (1)	4.02	R-L	0 / 3525 0.44 (1)
E-Z	-7922 / 0	-91.8	-91.8 0.36 (1)	4.02	L-I	0 / 3457 0.43 (1)
Z-F	-7922 / 0	-91.8	-91.8 0.36 (1)	4.02		
F-AA	-7922 / 0	-91.8	-91.8 0.36 (1)	4.02		
AA-AB	-7922 / 0	-91.8	-91.8 0.36 (1)	4.02		
AB-G	-7922 / 0	-91.8	-91.8 0.36 (1)	4.02		
G-AC	-6692 / 0	-91.8	-91.8 0.33 (1)	4.30		
AC-AD	-6692 / 0	-91.8	-91.8 0.33 (1)	4.30		
AD-AE	-6692 / 0	-91.8	-91.8 0.33 (1)	4.30		
AE-H	-6692 / 0	-91.8	-91.8 0.33 (1)	4.30		
H-I	-4072 / 0	-91.8	-91.8 0.09 (1)	5.56		
I-J	0 / 36	-91.8	-91.8 0.04 (1)	10.00		
S-B	-3376 / 0	0.0	0.0 0.19 (1)	6.35		
K-I	-3317 / 0	0.0	0.0 0.18 (1)	6.39		
S-AF	0 / 0	-18.5	-18.5 0.06 (4)	10.00		
AF-R	0 / 0	-18.5	-18.5 0.06 (4)	10.00		
R-AG	0 / 3431	-18.5	-18.5 0.27 (1)	10.00		
AG-AH	0 / 3431	-18.5	-18.5 0.27 (1)	10.00		
AH-AI	0 / 3431	-18.5	-18.5 0.27 (1)	10.00		
AI-Q	0 / 3431	-18.5	-18.5 0.27 (1)	10.00		
Q-P	0 / 6693	-18.5	-18.5 0.52 (1)	10.00		
P-AJ	0 / 6693	-18.5	-18.5 0.52 (1)	10.00		
AJ-AK	0 / 6693	-18.5	-18.5 0.52 (1)	10.00		
AK-O	0 / 6693	-18.5	-18.5 0.52 (1)	10.00		
O-AL	0 / 6665	-18.5	-18.5 0.51 (1)	10.00		
AL-AM	0 / 6665	-18.5	-18.5 0.51 (1)	10.00		
AM-AN	0 / 6665	-18.5	-18.5 0.51 (1)	10.00		
AN-N	0 / 6665	-18.5	-18.5 0.51 (1)	10.00		
N-AQ	0 / 6665	-18.5	-18.5 0.51 (1)	10.00		
AQ-M	0 / 6665	-18.5	-18.5 0.51 (1)	10.00		
M-AP	0 / 3365	-18.5	-18.5 0.27 (1)	10.00		
AP-AQ	0 / 3365	-18.5	-18.5 0.27 (1)	10.00		
AQ-AR	0 / 3365	-18.5	-18.5 0.27 (1)	10.00		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER

LOADS WERE DERIVED FROM USER INPUT

NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 8.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ORC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")

CALCULATED VERT. DEFL. (LL) = L/999 (0.20")

ALLOWABLE DEFL. (TL) = L/360 (1.17")

CALCULATED VERT. DEFL. (TL) = L/999 (0.38")

CSI: TC=0.36/1.00 (D-E:1), SC=0.52/1.00 (C-Q:1), WB=0.49/1.00 (H-M:1), SS=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	918	354	1587
	788	1587	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) INPUT = 0.90

JSI METAL= 0.63 (N) INPUT = 1.00

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	7.0	8.0	4.00	1.75
D	TMVW-l	MT20	5.0	6.0		
E	TMVW-w	MT20	3.0	6.0		
F	TS-l	MT20	5.0	6.0		
G	TMVW-l	MT20	5.0	6.0		
H	TTWW+m	MT20	7.0	8.0	4.00	1.75
I	TMVW-p	MT20	5.0	6.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMVW-l	MT20	5.0	6.0		
M	BMVW-l	MT20	5.0	6.0	2.50	2.75
N	BS-l	MT20	5.0	6.0		
O	BMVW-w-l	MT20	5.0	6.0		
P	BS-l	MT20	5.0	6.0		
Q	BMVW-l	MT20	5.0	6.0	2.50	2.75
R	BMVW-l	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. MEMB. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO					
AR-L	0 - 3365	-18.5	-18.5	0.27 (1)	10.00		
L-AS	0 0	-18.5	-18.5	0.07 (4)	10.00		
AS-AT	0 0	-18.5	-18.5	0.07 (4)	10.00		
AT-K	0 0	-18.5	-18.5	0.07 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-10	-49	-55		FRONT	VERT	DEAD		C1
C	4-0-10	-117	-117		BACK	VERT	TOTAL		C1
C	4-0-10	-254	-254		FRONT	VERT	SNOW		C1
F	19-11-4	-110	-110		BACK	VERT	TOTAL		C1
H	31-1-6	-49	-55		FRONT	VERT	DEAD		C1
H	31-1-6	-254	-254		FRONT	VERT	SNOW		C1
P	11-11-4	-26	-26		BACK	VERT	TOTAL		C1
R	3-11-4	-26	-26		BACK	VERT	TOTAL		C1
T	5-11-4	-110	-110		BACK	VERT	TOTAL		C1
U	7-11-4	-110	-110		BACK	VERT	TOTAL		C1
V	9-11-4	-110	-110		BACK	VERT	TOTAL		C1
W	11-11-4	-110	-110		BACK	VERT	TOTAL		C1
X	13-11-4	-110	-110		BACK	VERT	TOTAL		C1
Y	15-11-4	-110	-110		BACK	VERT	TOTAL		C1
Z	17-11-4	-110	-110		BACK	VERT	TOTAL		C1
AA	21-11-4	-110	-110		BACK	VERT	TOTAL		C1
AB	23-11-4	-110	-110		BACK	VERT	TOTAL		C1
AC	25-11-4	-110	-110		BACK	VERT	TOTAL		C1
AD	27-11-4	-110	-110		BACK	VERT	TOTAL		C1
AE	29-11-4	-110	-110		BACK	VERT	TOTAL		C1
AF	1-11-4	-26	-26		BACK	VERT	TOTAL		C1
AG	5-11-4	-26	-26		BACK	VERT	TOTAL		C1
AH	7-11-4	-26	-26		BACK	VERT	TOTAL		C1
AI	9-11-4	-26	-26		BACK	VERT	TOTAL		C1
AJ	13-11-4	-26	-26		BACK	VERT	TOTAL		C1
AK	15-11-4	-26	-26		BACK	VERT	TOTAL		C1
AL	17-11-4	-26	-26		BACK	VERT	TOTAL		C1
AM	19-11-4	-26	-26		BACK	VERT	TOTAL		C1
AN	21-11-4	-26	-26		BACK	VERT	TOTAL		C1
AO	23-11-4	-26	-26		BACK	VERT	TOTAL		C1
AP	25-11-4	-26	-26		BACK	VERT	TOTAL		C1
AQ	27-11-4	-26	-26		BACK	VERT	TOTAL		C1
AR	29-11-4	-26	-26		BACK	VERT	TOTAL		C1
AS	31-11-4	-26	-26		BACK	VERT	TOTAL		C1
AT	33-11-4	-26	-26		BACK	VERT	TOTAL		C1

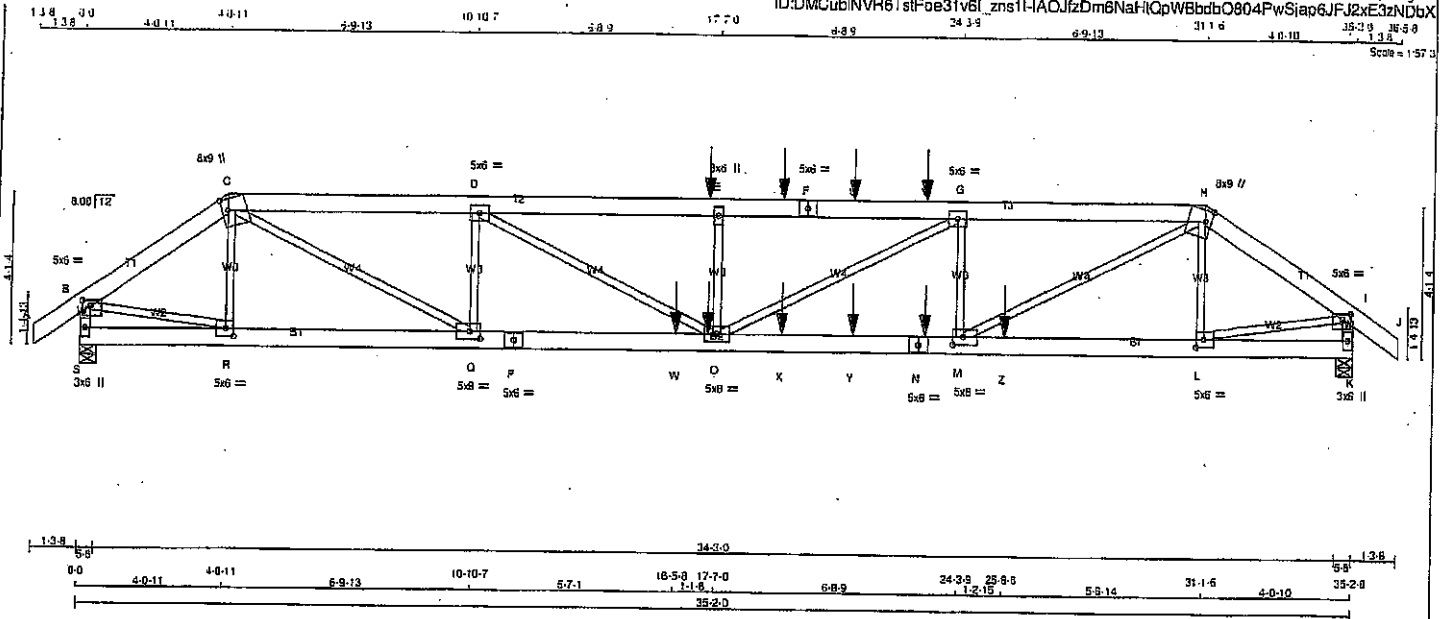
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



JOB NAME 408131	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 8 Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:07:24 2020 Page 1
ID:DMCubINVR6TstFoe31v6I_zns11-IAOJfzDm6NafIQpWBbdbO804PwSjap6JFJ2xE3zND6X



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY No.2	SPF
C - F	2x6	DRY No.2	SPF
F - H	2x6	DRY No.2	SPF
H - J	2x6	DRY No.2	SPF
S - B	2x4	DRY No.2	SPF
K - I	2x4	DRY No.2	SPF
S - P	2x6	DRY No.2	SPF
P - N	2x6	DRY No.2	SPF
N - K	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C 2 12	TOP	
C-F 2 12	SIDE(183.1)	
F-H 2 12	SIDE(0.0)	
H-J 2 12	TOP	
S-B 1 12	TOP	
K-I 1 12	TOP	
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
S-P 2 12	TOP	
P-N 2 12	SIDE(183.1)	
N-K 2 12	SIDE(0.0)	
WEBS: (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3/4 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
S	3388	0	3388	0	0	5-8	5-8	5-8	5-8
K	3900	0	3900	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	COMBINED	SNOW	LIVE	PERM. LIVE
S	2393	1588/0	0/0	0/0	0/0	805/0	0/0	2393	1588/0	0/0	0/0
K	2757	1818/0	0/0	0/0	0/0	839/0	0/0	2757	1818/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.08 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD LC1	MAX. (PLF)	MAX. (PLF)	CSI (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED	MAX. (PLF)
FR-TO						FR-TO			
A-B	0/36	-91.8	-91.8	0.04 (1)	10.00	R-C	-659/0	0.08 (1)	
B-C	-4130/0	-91.8	-91.8	0.09 (1)	5.53	C-Q	0/5226	0.85 (1)	
C-D	-8076/0	-91.8	-91.8	0.29 (1)	4.07	Q-D	-2102/0	0.25 (1)	
D-E	-10755/0	-91.8	-91.8	0.35 (1)	3.55	D-O	0/3048	0.38 (1)	
E-T	-10755/0	-91.8	-91.8	0.44 (1)	3.46	O-E	-837/0	0.10 (1)	
T-F	-10755/0	-91.8	-91.8	0.44 (1)	3.46	O-G	0/1721	0.21 (1)	
F-U	-10755/0	-91.8	-91.8	0.44 (1)	3.46	M-G	-1681/0	0.20 (1)	
U-V	-10755/0	-91.8	-91.8	0.44 (1)	3.46	M-H	0/5821	0.72 (1)	
V-G	-10755/0	-91.8	-91.8	0.44 (1)	3.46	L-H	-438/0	0.05 (1)	
G-H	-9242/0	-91.8	-91.8	0.33 (1)	3.81	B-R	0/3505	0.43 (1)	
H-I	-4959/0	-91.8	-91.8	0.10 (1)	5.15	L-I	0/4210	0.52 (1)	
I-J	0/36	-91.8	-91.8	0.04 (1)	10.00				
S-B	-3359/0	0.0	0.0	0.19 (1)	6.36				
K-I	-3970/0	0.0	0.0	0.22 (1)	5.94				
S-R	0/0	-18.5	-18.5	0.03 (4)	10.00				
R-Q	0/3408	-18.5	-18.5	0.31 (1)	10.00				
Q-P	0/8076	-18.5	-18.5	0.92 (1)	10.00				
P-W	0/8076	-18.5	-18.5	0.92 (1)	10.00				
W-O	0/8076	-18.5	-18.5	0.92 (1)	10.00				
O-X	0/9242	-18.5	-18.5	0.74 (1)	10.00				
X-Y	0/9242	-18.5	-18.5	0.74 (1)	10.00				
Y-N	0/9242	-18.5	-18.5	0.74 (1)	10.00				
N-M	0/9242	-18.5	-18.5	0.74 (1)	10.00				
M-Z	0/4102	-18.5	-18.5	0.61 (1)	10.00				
Z-L	0/4102	-18.5	-18.5	0.61 (1)	10.00				
L-K	0/0	-18.5	-18.5	0.17 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
E	17-4-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
N	23-4-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	17-4-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
T	19-4-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
U	21-4-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	23-4-4	-110	-110	---	FRONT	VERT	TOTAL	---	C1
W	16-5-2	-1460	-1460	---	FRONT	VERT	TOTAL	---	C1
X	19-4-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Y	21-4-4	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Z	25-6-8	-1144	-1144	---	FRONT	VERT	TOTAL	---	C1

TOTAL WEIGHT = 2 X 180 = 359 LB

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = U/999 (0.30")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = U/775 (0.54")

CSI: TC=0.44/1.00 (E-G:1), BC=0.92/1.00 (Q-Q:1), WB=0.72/1.00 (H-M:1), SS=0.43/1.00 (O-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1857 788 1987 1556

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.89 (C) (INPUT = 0.90)
JSI METAL=0.86 (N) (INPUT = 1.00)



Structural component only
DWG# T-2007018 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408131	T1Z	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:07:34 2020 Page 2
ID:DMCubINVR6TsFce31v6l zns1H-AOJtzDm8NaHQPWBdbO804PwSjap6JFJ2xE3zNDbX

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TTWW+m	MT20	8.0	9.0	3.75	2.00
D	TMWW-l	MT20	5.0	6.0		
E	TMW-w	MT20	3.0	6.0		
F	TS-l	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	8.0	9.0	3.75	2.00
I	TMVW-p	MT20	5.0	6.0	1.75	2.75
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-l	MT20	5.0	6.0	2.50	2.75
M	BMWW-l	MT20	5.0	6.0	2.50	3.50
N	BS-l	MT20	5.0	6.0		
O	BMWW-l	MT20	5.0	6.0		
P	BS-l	MT20	5.0	6.0		
Q	BMWW-l	MT20	5.0	6.0	2.50	3.50
R	BMWW-l	MT20	5.0	6.0	2.50	2.75
S	BMV1+p	MT20	3.0	6.0		

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007018 3/2

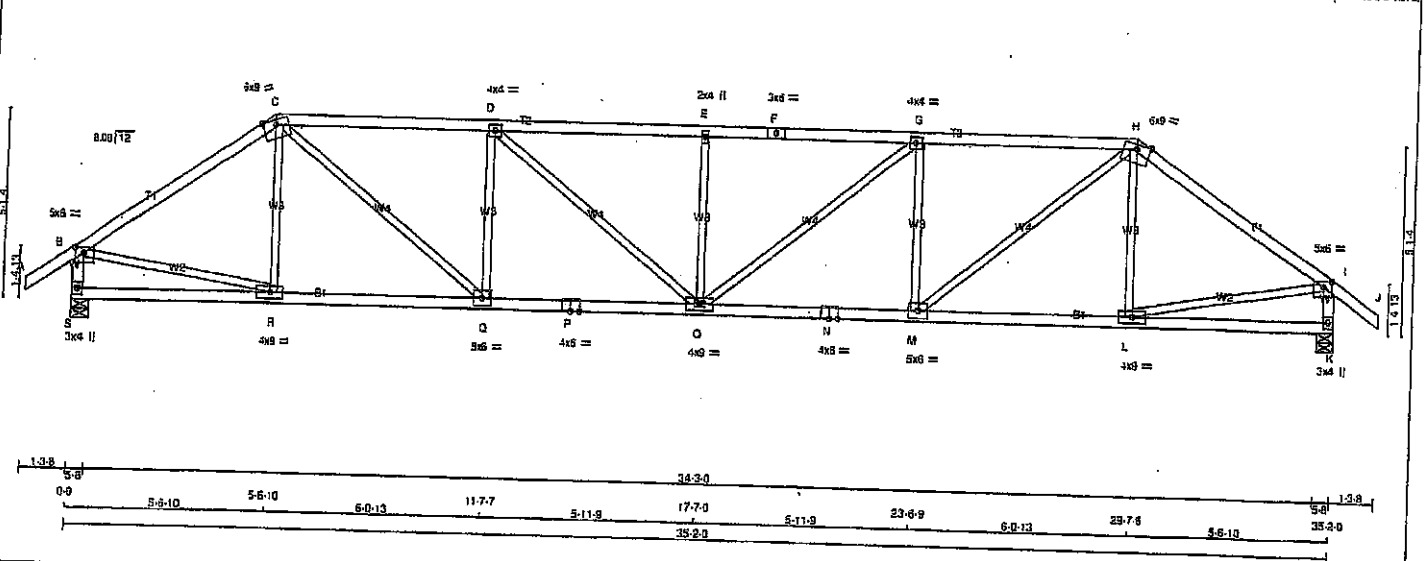
JOB NAME 408087	TRUSS NAME T2	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
---------------------------	-------------------------	----------------------	-----------------	--------------------------------------	-------------	----------

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mittek Industries, Inc. Sat Apr 25 08:53:16 2020 Page 1

ID:DMCubINVR6TstFoe31v6I_zns11-XSdn720dkAPEsmtdRqX69orNvQSPCCoRIZZ7zNEH1

Scale = 1:37.5



LUMBER			
N.L.G.A. RULES			
CHORDS	SIZE	LUMBER	
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	5.0	1.75	2.75
C	TTWW-m	MT20	6.0	5.0	Edge	
D	TMVW-l	MT20	4.0	4.0		
E	TMVW-w	MT20	2.0	4.0		
F	TS-l	MT20	3.0	6.0		
G	TMVW-l	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	9.0	Edge	
I	TMVW-p	MT20	5.0	6.0	1.75	2.75
K	BMVW-l	MT20	3.0	4.0		
L	BMVW-l	MT20	4.0	9.0		
M	BMVW-l	MT20	5.0	6.0		
N	BS-l	MT20	4.0	5.0		
O	BMVWVW-l	MT20	4.0	9.0		
P	BS-l	MT20	4.0	6.0		
Q	BMVW-l	MT20	5.0	6.0		
R	BMVW-l	MT20	4.0	9.0		
S	BMVW-l	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
S	2065	0	2065	0
K	2065	0	2065	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
S	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
K	1458	970 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO				FR-TO			
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	R-C	-243 / 10	0.09 (1)
B-C	-2297 / 0	-91.8	-91.8 0.70 (1)	3.78	C-D	0 / 1670	0.38 (1)
C-D	-3214 / 0	-91.8	-91.8 0.80 (1)	3.18	D-E	-929 / 0	0.35 (1)
D-E	-3595 / 0	-91.8	-91.8 0.85 (1)	2.97	E-F	0 / 490	0.11 (1)
E-F	-3595 / 0	-91.8	-91.8 0.85 (1)	2.97	F-G	-505 / 0	0.19 (1)
F-G	-3214 / 0	-91.8	-91.8 0.80 (1)	3.18	G-H	-929 / 0	0.35 (1)
G-H	-2297 / 0	-91.8	-91.8 0.70 (1)	3.78	H-I	0 / 1670	0.38 (1)
H-I	0 / 35	-91.8	-91.8 0.12 (1)	10.00	I-J	-243 / 10	0.09 (1)
I-J	-2023 / 0	0.0	0.0 0.21 (1)	5.94	J-K	0 / 1943	0.44 (1)
J-K	-2023 / 0	0.0	0.0 0.21 (1)	5.94	K-L	0 / 1943	0.44 (1)
S-R	0 / 0	-18.5	-18.5 0.15 (4)	10.00			
R-Q	0 / 1904	-18.5	-18.5 0.39 (1)	10.00			
Q-P	0 / 3214	-18.5	-18.5 0.58 (1)	10.00			
P-O	0 / 3214	-18.5	-18.5 0.58 (1)	10.00			
O-N	0 / 3214	-18.5	-18.5 0.58 (1)	10.00			
N-M	0 / 3214	-18.5	-18.5 0.58 (1)	10.00			
M-L	0 / 1904	-18.5	-18.5 0.39 (1)	10.00			
L-K	0 / 0	-18.5	-18.5 0.15 (4)	10.00			

TOTAL WEIGHT = 2 X 140 = 280 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = 1/360 (1.17")
CALCULATED VERT. DEF. (LL) = 1/959 (0.21")
ALLOWABLE DEF. (TL) = 1/360 (1.17")
CALCULATED VERT. DEF. (TL) = 1/959 (0.39")

CSI: TC=0.86/1.00 (E-G:1), BC=0.58/1.00 (C-O:1),
WB=0.44/1.00 (B-R:1), SS=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

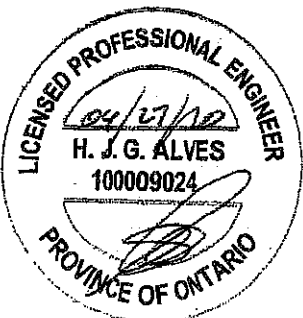
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

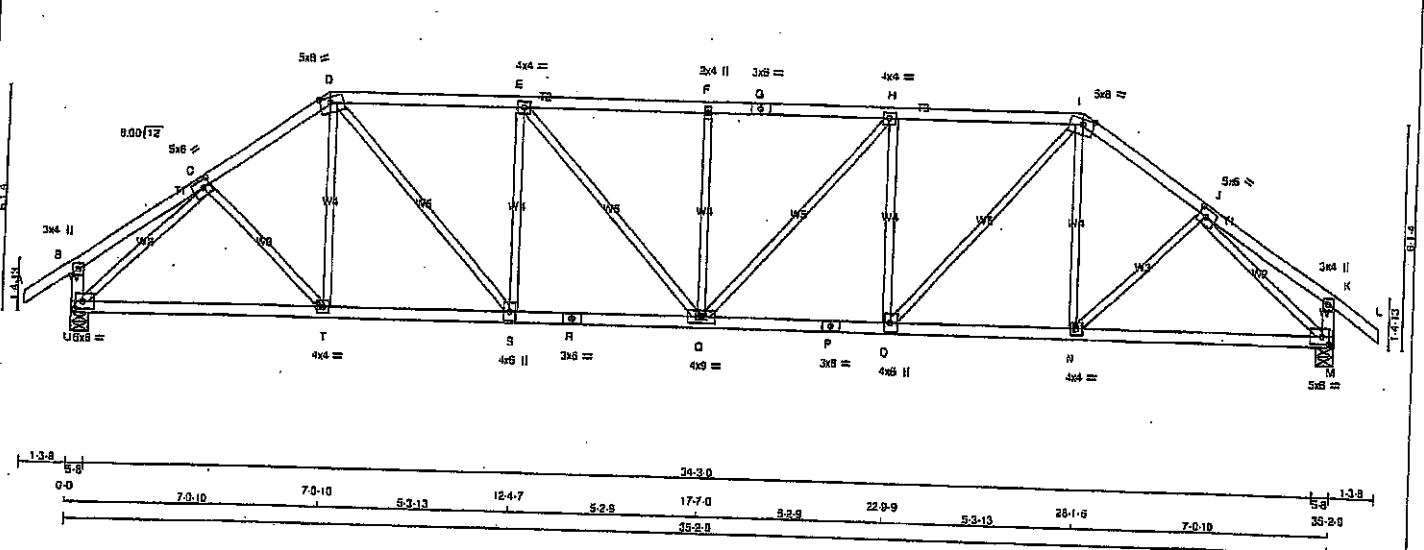
JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.74 (P) (INPUT = 1.00)



Structural component only
DWG# T-2006954

JOB NAME 408087	TRUSS NAME T3	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Sat Apr 25 08:53:16 2020 Page 1
 ID:DMCubINVR6TstFoe31v6I_zns1I-XSdn72OldkAPEmfdRqX69owDvSsP37cRIZZ7zNEH1
 1.38 0.0 3.79 3.79 7.010 5.313 12.47 5.29 17.70 5.29 22.99 5.313 26.10 7.010 33.20
 Scale = 1:57.3



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
U - R	2x4	DRY	No.2
R - P	2x4	DRY	No.2
P - M	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMVW-1	MT20	5.0	6.0 2.50 2.50
D	TTVW-m	MT20	5.0	8.0 1.75 3.50
E	TMVW-1	MT20	4.0	4.0
F	TMVW-w	MT20	2.0	4.0
G	TS-1	MT20	3.0	6.0
H	TMVW-1	MT20	4.0	4.0
I	TTVW-m	MT20	5.0	8.0 1.75 3.50
J	TMVW-1	MT20	5.0	6.0 2.50 2.50
K	BMV+p	MT20	3.0	4.0
M	BMVW-1	MT20	5.0	6.0 2.50 2.25
N	BMVW-1	MT20	4.0	4.0
O	BMVW-1	MT20	4.0	6.0
P	BS-1	MT20	3.0	6.0
Q	BMVW-1	MT20	4.0	9.0
R	BS-1	MT20	3.0	6.0
S	BMVW-1	MT20	4.0	6.0
T	BMVW-1	MT20	4.0	4.0
U	BMVW-1	MT20	5.0	6.0 2.50 2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	JT	GROSS REACTION	BRG	BRG	BRG	BRG
U	2065 0	U	2065 0	0 0	0 0	488 0	0 0
M	2065 0	M	2065 0	0 0	0 0	488 0	0 0

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1458	970 0	0 0	0 0	0 0	488 0	0 0
M	1458	970 0	0 0	0 0	0 0	488 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 3.53 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 135	-91.8	-91.8 0.12 (1)	G-T	0 / 135	0.03 (4)	
B-C	0 / 118	-91.8	-91.8 0.16 (1)	T-D	0 / 90	0.03 (4)	
C-D	-2306 / 0	-91.8	-91.8 0.23 (1)	D-S	0 / 1218	0.27 (1)	
D-E	-2724 / 0	-91.8	-91.8 0.54 (1)	S-E	-814 / 0	0.48 (1)	
E-F	-2967 / 0	-91.8	-91.8 0.58 (1)	E-Q	0 / 363	0.08 (1)	
F-G	-2967 / 0	-91.8	-91.8 0.58 (1)	Q-F	-440 / 0	0.26 (1)	
G-H	-2967 / 0	-91.8	-91.8 0.58 (1)	Q-H	0 / 363	0.08 (1)	
H-I	-2724 / 0	-91.8	-91.8 0.54 (1)	O-H	-814 / 0	0.48 (1)	
I-J	-2306 / 0	-91.8	-91.8 0.23 (1)	O-I	0 / 1218	0.27 (1)	
J-K	0 / 118	-91.8	-91.8 0.16 (1)	N-I	0 / 90	0.03 (4)	
K-L	0 / 135	-91.8	-91.8 0.12 (1)	N-J	0 / 135	0.03 (4)	
U-B	-255 / 0	0.0	0.0 0.03 (1)	U-C	-2515 / 0	0.97 (1)	
M-K	-255 / 0	0.0	0.0 0.03 (1)	J-M	-2515 / 0	0.97 (1)	
T-S	0 / 1802	-18.5	-18.5 0.40 (1)				
S-R	0 / 1901	-18.5	-18.5 0.41 (1)				
R-Q	0 / 2724	-18.5	-18.5 0.49 (1)				
Q-P	0 / 2724	-18.5	-18.5 0.49 (1)				
P-O	0 / 2724	-18.5	-18.5 0.49 (1)				
O-N	0 / 1901	-18.5	-18.5 0.41 (1)				
N-M	0 / 1802	-18.5	-18.5 0.40 (1)				

TOTAL WEIGHT = 2 X 151 = 303 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 8.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 39.0 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012, ABC 2010
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIG 2011, TPIG 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
 ALLOWABLE DEFL.(TL) = L/360 (1.17")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CSI: TC=0.56/1.00 (E-F:1), BC=0.49/1.00 (Q-S:1), WB=0.97/1.00 (C-U:1), SS=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

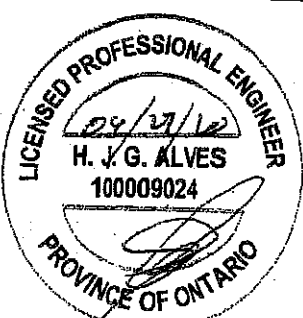
PLATE	GRIP(DRY)	SHEAR (PS)	SECTION (PL)
MT20	618	354	1667 796 1937 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

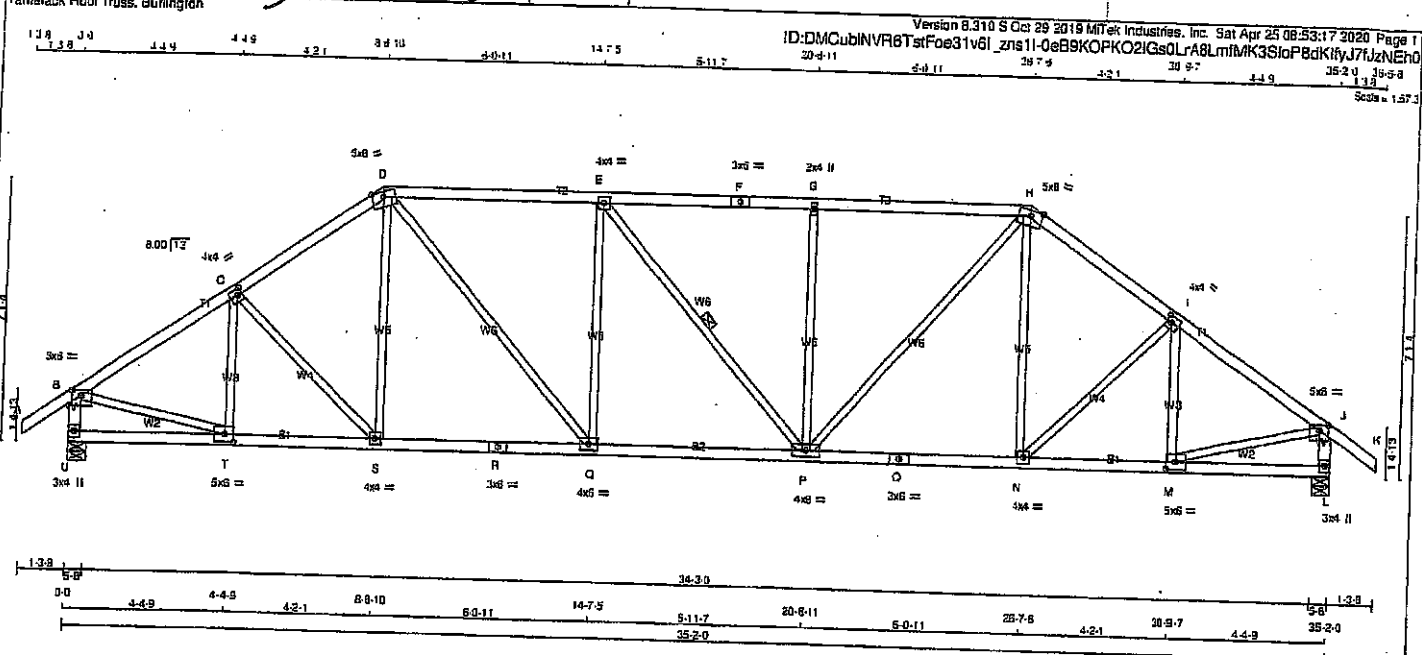
JSI GRIP = 0.90 (U) (INPUT = 0.90)

JSI METAL = 0.84 (P) (INPUT = 1.00)



Structural component only
 DWG# T-2006955

JOB NAME 408087 Tamarack Roof Truss, Burlington	TRUSS NAME T4	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
---	------------------	---------------	----------	-------------------------------	-------------	----------



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
U - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
U - R	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	6.0	1.75	2.75
C TMWW-I	MT20	4.0	4.0	2.00	1.50
D TTWW-m	MT20	5.0	6.0	Edge	
E TMWW-I	MT20	4.0	4.0		
F TS-I	MT20	3.0	6.0		
G TMVW-w	MT20	2.0	4.0		
H TTWW-m	MT20	5.0	6.0	Edge	
I TMWW-I	MT20	4.0	4.0	2.00	1.50
J TMVW-p	MT20	5.0	6.0	1.75	2.75
L BMV1-p	MT20	3.0	4.0		
M BMWW-I	MT20	5.0	6.0	2.50	2.75
N BMWW-I	MT20	4.0	4.0		
P BS-I	MT20	3.0	6.0		
Q BMVWW-I	MT20	4.0	9.0		
R BMWW-I	MT20	4.0	6.0		
S BS-I	MT20	3.0	6.0		
T BMVW-I	MT20	4.0	4.0		
U BMVW-I	MT20	5.0	6.0	2.50	2.75
V BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
U	2065	0	2065	0	0	5-8	5-8	
L	2065	0	2065	0	0	5-8	5-8	

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
U	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
L	1458	970 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	FR-TO	CHORDS		MAX. UNBRACED LENGTH	FR-TO	WEBS	
		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)			MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
A-B	0 / 35		-91.8	-91.8	10.00	T-C	-396 / 0
B-C	-2270 / 0		-91.8	-91.8	0.37 (1)	C-S	-79 / 0
C-D	-2254 / 0		-91.8	-91.8	0.36 (1)	S-D	0 / 156
D-E	-2485 / 0		-91.8	-91.8	0.84 (1)	D-Q	0 / 949
E-F	-2484 / 0		-91.8	-91.8	0.84 (1)	Q-E	-587 / 0
F-G	-2484 / 0		-91.8	-91.8	0.84 (1)	E-P	-2 / 0
G-H	-2484 / 0		-91.8	-91.8	0.84 (1)	P-G	-587 / 0
H-I	-2254 / 0		-91.8	-91.8	0.36 (1)	I-H	0 / 947
I-J	-2270 / 0		-91.8	-91.8	0.37 (1)	J-I	0 / 157
J-K	0 / 35		-91.8	-91.8	0.12 (1)	K-J	-78 / 0
U-B	-2027 / 0		0.0	0.0	0.21 (1)	B-U	-397 / 0
L-J	-2026 / 0		0.0	0.0	0.21 (1)	J-L	0 / 1956
U-T	0 / 0		-18.5	-18.5	0.08 (4)	T-U	0 / 1956
T-S	0 / 1909		-18.5	-18.5	0.37 (1)	S-T	0 / 1956
S-R	0 / 1854		-18.5	-18.5	0.36 (1)	R-S	0 / 1956
R-Q	0 / 1854		-18.5	-18.5	0.36 (1)	Q-R	0 / 1956
Q-P	0 / 2485		-18.5	-18.5	0.47 (1)	P-Q	0 / 1956
P-O	0 / 1854		-18.5	-18.5	0.36 (1)	O-P	0 / 1956
O-N	0 / 1854		-18.5	-18.5	0.36 (1)	N-O	0 / 1956
N-M	0 / 1909		-18.5	-18.5	0.37 (1)	M-N	0 / 1956
M-L	0 / 0		-18.5	-18.5	0.08 (4)	L-M	0 / 1956

TOTAL WEIGHT = 2 X 155 = 309 lb

SPECIFIED CRITERIA

DESIGNED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 188-09, CSA 086-14
- TFC 2011, TFC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.64/1.00 (D-E:1), BC=0.47/1.00 (P-Q:1), WB=0.52/1.00 (E-Q:1), SS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

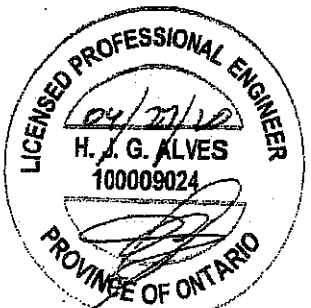
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1907 1666

PLATE PLACEMENT TOL. = 0.250 inches

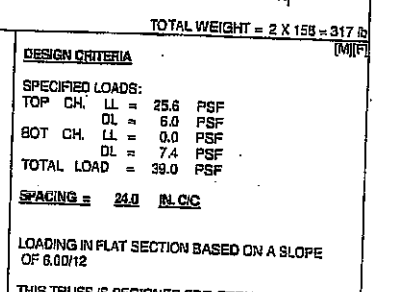
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (D) (INPUT = 0.90)
JSI METAL = 0.92 (O) (INPUT = 1.00)



Structural component only
DWG# T-2006956

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Sat Apr 25 08:53:18 2020 Page 1
ID:DMCubiNVR6TstF0631v6l_zns1l-UdXyKQy8LO7TAw1kss?BZlCWj9a4hvuuc2gCmznENh?
1.38 0.0 5.19 10.0 10 17.0 25.1 31.1 36.9 38.9
1.38 5.19 10.0 10 17.0 25.1 31.1 36.9 38.9
Scale = 1:57.1



BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				W E B S				
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (L)	MAX. (PLF)	MAX. UNBRACED LENGTH (L)	MEMB.	FORCE (LBS)	MAX. FACTORED CSI (L)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	C-C	-321 / 0	0.11 (1)
B-C	-2318 / 0	-91.8	-91.8	0.35 (1)	4.16	C-P	-219 / 0	0.18 (1)
C-O	-2186 / 0	-91.8						

THIS DESIGN IS DESIGNED FOR RESIDENTIAL OR
SMALL BUSINESS REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA R86-09, CSA R86-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN
LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF
LIVE LOAD

ALLOWABLE DEF. (L/L) = L/360 (1.17)
CALCULATED VERT. DEF. (L/L) = L/909 (0.11)
ALLOWABLE DEF. (L/L) = L/360 (1.17)
CALCULATED VERT. DEF. (L/L) = L/999 (0.22)

CSI: $TG = 0.81/1.00$ (D-E-I), $EC = 0.41/1.00$ (N-P-I),
 $WB = 0.45/1.00$ (B-Q-T), $SS = 0.34/1.00$ (D-E-I)

DOL NUMBER-1 ON MAIL ROOM ROOF

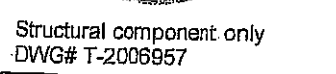
COMP=1.10 SHEAR=1.10 TENS= 1.10
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

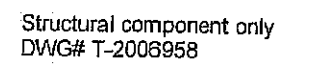
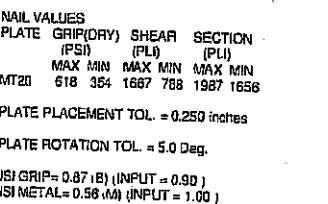
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1657 788	1987 1655

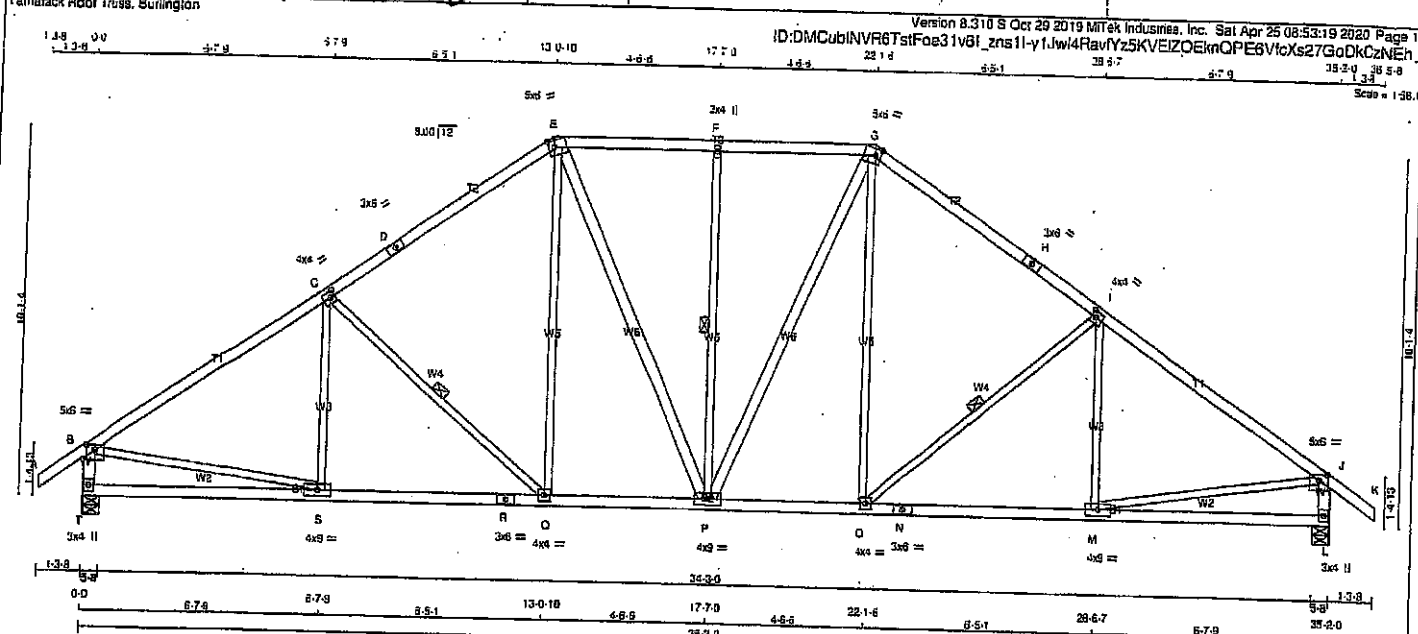
PLATE PLACEMENT TOL. = 0.250 inches



Version 8.310 S Oct 29 2019 MITek Industries, Inc. Sat Apr 25 08:53:19 2020 Page 1
ID:DMCubINVR61stFoe31v6l_zns1l-y1Jw4RavYz5KVEIZOEImQR16V7cK127GoDKCznNeh
Scale = 1:57.7



JOB NAME: 408087 TRUSS NAME: T7 QUANTITY: 3 PLY: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC: TRUSS NO. 1



LUMBER

N.L.G. A RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
T - B	2x4	DRY	No.2
T - J	2x4	DRY	No.2
T - R	2x4	DRY	No.2
R - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
E - P	2x4	DRY	No.2
P - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

TYPES (table is in inches)						
TYPE	PLATES	W	LEN	Y	X	
TMW-w	MT20	5.0	8.0	1.75	2.75	
TMW-w	MT20	4.0	4.0	2.00	1.50	
TS4	MT20	3.0	8.0			
TTW-w	MT20	5.0	6.0	2.25	2.00	
TMW-w	MT20	2.0	4.0			
TTW-w	MT20	5.0	6.0	2.25	2.00	
TS4	MT20	3.0	8.0			
TMW-w	MT20	4.0	4.0	2.00	1.50	
TMW-p	MT20	5.0	8.0	1.75	2.75	
BMV1-p	MT20	3.0	4.0			
BMW-w	MT20	4.0	9.0			
GS4	MT20	3.0	6.0			
BMW-w	MT20	4.0	4.0			
BMW-w	MT20	4.0	9.0			
BMW-w	MT20	4.0	4.0			
BS4	MT20	3.0	6.0			
BMW-w	MT20	4.0	9.0			
BMV1-p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	DOWN	BRG	BRG
T	2065	0	0	5-8
L	2065	0	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
T	1458	970/0	0/0	0/0	0/0	488/0
L	1458	970/0	0/0	0/0	0/0	488/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-Q, F-P, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WESS				
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO			FR-TO			
A-B	0/35	-91.8	-91.8	0.12 (1)	10.00	S-C	-189/46	0.10 (1)	
B-C	-2359/0	-91.8	-91.8	0.69 (1)	3.77	C-Q	-485/0	0.24 (1)	
C-D	-1998/0	-91.8	-91.8	0.62 (1)	4.11	Q-E	0/416	0.09 (1)	
D-E	-1998/0	-91.8	-91.8	0.62 (1)	4.11	E-P	0/315	0.05 (1)	
E-F	-1782/0	-91.8	-91.8	0.27 (1)	4.77	P-F	-502/0	0.35 (1)	
F-G	-1782/0	-91.8	-91.8	0.27 (1)	4.77	P-G	0/315	0.05 (1)	
G-H	-1998/0	-91.8	-91.8	0.62 (1)	4.11	O-G	0/416	0.09 (1)	
H-I	-1998/0	-91.8	-91.8	0.62 (1)	4.11	O-I	-485/0	0.24 (1)	
I-J	-2359/0	-91.8	-91.8	0.69 (1)	3.77	M-I	-189/46	0.10 (1)	
J-K	0/35	-91.8	-91.8	0.12 (1)	10.00	B-S	0/2022	0.45 (1)	
K-L	-2014/0	0.0	0.0	0.21 (1)	5.95	M-J	0/2022	0.45 (1)	
L-J	-2014/0	0.0	0.0	0.21 (1)	5.95				
T-S	0/0	-18.5	-18.5	0.19 (4)	10.00				
S-R	0/1998	-18.5	-18.5	0.42 (1)	10.00				
R-O	0/1998	-18.5	-18.5	0.42 (1)	10.00				
O-P	0/1630	-18.5	-18.5	0.32 (1)	10.00				
P-O	0/1630	-18.5	-18.5	0.32 (1)	10.00				
O-N	0/1998	-18.5	-18.5	0.42 (1)	10.00				
N-M	0/1998	-18.5	-18.5	0.42 (1)	10.00				
M-L	0/0	-18.5	-18.5	0.19 (4)	10.00				

TOTAL WEIGHT = 3 X 172 = 515 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")
CALCULATED VERT. DEFL. (LL) = L/999 (0.03")
ALLOWABLE DEFL. (TL) = L/360 (1.17")
CALCULATED VERT. DEFL. (TL) = L/999 (0.18")

CS1: TC=0.89/1.00 (I-J:1), BC=0.42/1.00 (M-O:1), WB=0.45/1.00 (J-M:1), SS1=0.24/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

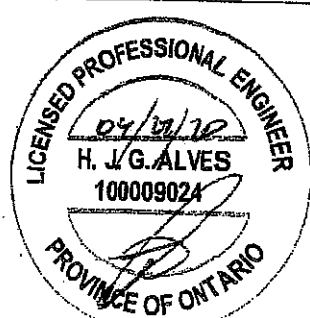
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1587 789 1987 1655

PLATE PLACEMENT: TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

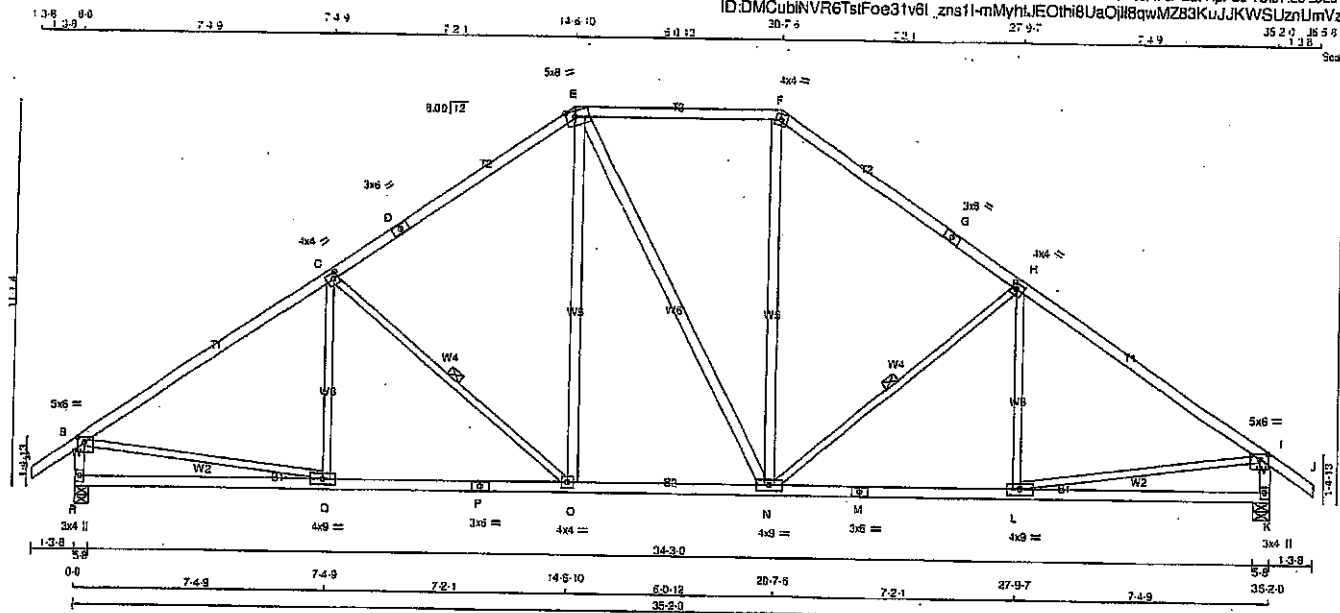
JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.58 (N) (INPUT = 1.00)



Structural component only
DWG# T-2006959

JOB NAME 408131	TRUSS NAME T8	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Sat Apr 25 10:07:25 2020 Page 1
ID:DMCubINVR6TstFoe3tv6l_znsf1-mMyhlJE0th8UaQjIRqwmZ83KuJJKWSUznUmVzNDbW



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - E	2x4	DRY	No.2	SPF
E - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	2.75
C	TMVW-I	MT20	4.0	4.0	2.00	1.50
D	TS-I	MT20	3.0	6.0		
E	TTWV-m	MT20	5.0	8.0	2.00	3.00
F	TTW-m	MT20	4.0	4.0		
G	TS-I	MT20	3.0	6.0		
H	TMVW-I	MT20	4.0	4.0	2.00	1.50
I	TMVW-p	MT20	5.0	8.0	1.75	2.75
K	BMV1+p	MT20	3.0	4.0		
L	BMVW-I	MT20	4.0	9.0		
M	BS-I	MT20	3.0	6.0		
N	BMVWW-I	MT20	4.0	9.0		
O	BMVW-I	MT20	4.0	4.0		
P	BS-I	MT20	3.0	6.0		
Q	BMVW-I	MT20	4.0	9.0		
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	0	0
R	HORZ	HORZ	0	0
K	UPLIFT	UPLIFT	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
R	1458	970 / 0
K	1458	970 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
FR-TO					FR-TO		
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	C-C	-139.76
B-C	-2358 / 0	-91.8	-91.8	0.89 (1)	3.45	C-O	-805.0
C-D	-1888 / 0	-91.8	-91.8	0.78 (1)	3.91	O-E	0 / 499
D-E	-1888 / 0	-91.8	-91.8	0.78 (1)	3.91	E-N	0 / 0
E-F	-1538 / 0	-91.8	-91.8	0.47 (1)	4.74	N-F	0 / 500
F-G	-1888 / 0	-91.8	-91.8	0.78 (1)	3.91	N-H	-604.0
G-H	-1888 / 0	-91.8	-91.8	0.78 (1)	3.91	H-H	-140.75
H-I	-2357 / 0	-91.8	-91.8	0.89 (1)	3.45	B-Q	0 / 2020
I-J	0 / 35	-91.8	-91.8	0.12 (1)	10.00	L-I	0 / 2020
R-B	-2009 / 0	0.0	0.0	0.21 (1)	5.95		
K-I	-2009 / 0	0.0	0.0	0.21 (1)	5.95		
R-Q	0 / 0	-18.5	-18.5	0.25 (4)	10.00		
Q-P	0 / 1999	-18.5	-18.5	0.45 (1)	10.00		
P-O	0 / 1999	-18.5	-18.5	0.45 (1)	10.00		
O-N	0 / 1538	-18.5	-18.5	0.33 (1)	10.00		
N-M	0 / 1999	-18.5	-18.5	0.45 (1)	10.00		
M-L	0 / 1999	-18.5	-18.5	0.45 (1)	10.00		
L-K	0 / 0	-18.5	-18.5	0.24 (4)	10.00		

TOTAL WEIGHT = 2 X 166 = 333 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 8.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CS: TC=0.89/1.00 (B-C:1), BC=0.45/1.00 (O-Q:1), WB=0.45/1.00 (B-C:1), SS=0.27/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

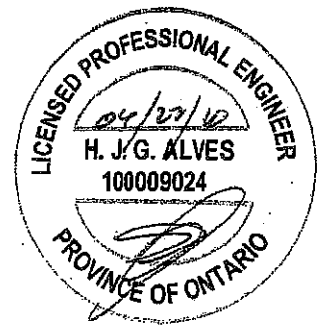
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PS)	SECTION (PL)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1567	789	1987

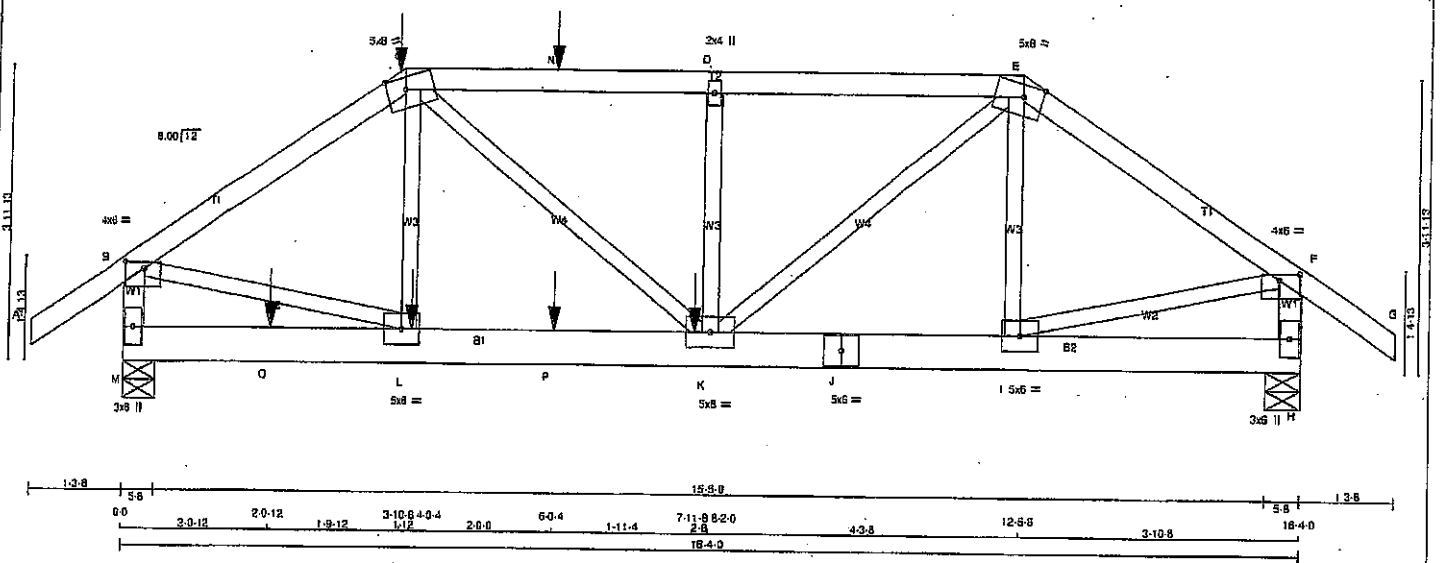
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (F) (INPUT = 0.90)
JSI METAL = 0.88 (P) (INPUT = 1.00)



Structural component only
DWG# T-2007019



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - J	2x6	DRY	No.2
J - H	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	4.0	6.0	1.00 3.25
C	TTWW-m	MT20	5.0	8.0	2.00 3.25
D	TMVW-w	MT20	2.0	4.0	
E	TTWW-m	MT20	5.0	8.0	2.00 3.25
F	TMVW-p	MT20	4.0	6.0	1.00 3.25
H	BMV1-p	MT20	3.0	6.0	
I	BMVWV-1	MT20	5.0	6.0	
J	BS-t	MT20	5.0	6.0	
K	BMVWV-1	MT20	5.0	6.0	
L	BMVWV-1	MT20	5.0	6.0	
M	BMV1-p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		GROSS REACTION		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG	IN-SX	IN-SX	IN-SX	IN-SX
M	1679	0	1679	0	0	5-8	5-8	5-8	5-8	5-8	5-8
H	1488	0	1488	0	0	5-8	5-8	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1183	803 / 0	0 / 0	0 / 0	0 / 0	380 / 0	0 / 0
H	1053	717 / 0	0 / 0	0 / 0	0 / 0	336 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	MAX. CSI (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-91.8	-91.8 0.14 (1)	10.00	L-C	-215 / 26	0.05 (1)
B-C	-1751 / 0	-91.8	-91.8 0.33 (1)	4.65	C-K	0 / 874	3.22 (1)
C-N	-2118 / 0	-91.8	-91.8 0.42 (1)	4.17	K-D	-523 / 0	0.13 (1)
D-E	-2118 / 0	-91.8	-91.8 0.42 (1)	4.17	E-F	0 / 1120	0.28 (1)
E-F	-1523 / 0	-91.8	-91.8 0.31 (1)	4.94	F-G	-203 / 0	0.05 (1)
F-G	0 / 35	-91.8	-91.8 0.14 (1)	10.00	G-H	0 / 1499	0.37 (1)
G-H	-1643 / 0	0.0	0.0 0.18 (1)	8.42	H-I	0 / 1304	0.32 (1)
H-I	-1489 / 0	0.0	0.0 0.18 (1)	6.71			
M-O	0 / 0	-18.5	-18.5 0.05 (4)	10.00			
O-L	0 / 0	-18.5	-18.5 0.05 (4)	10.00			
L-P	0 / 1448	-18.5	-18.5 0.26 (1)	10.00			
P-K	0 / 1448	-18.5	-18.5 0.26 (1)	10.00			
K-J	0 / 1259	-18.5	-18.5 0.23 (1)	10.00			
J-I	0 / 1259	-18.5	-18.5 0.23 (1)	10.00			
I-H	0 / 0	-18.5	-18.5 0.04 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)		FACE		DIR.		TYPE		HEEL		CONN.	
C	LOC.	LC1	MAX.	MAX+	MAX-	FACE	DIR.	TYPE	HEEL	CONN.	CONN.
C	3-10-8	-255	-255	---	---	FRONT	VERT	TOTAL	---	C1	C1
K	7-11-8	-729	-729	---	---	FRONT	VERT	TOTAL	---	C1	C1
L	4-0-4	-17	-17	---	---	FRONT	VERT	TOTAL	---	C1	C1
N	6-0-4	-86	-86	---	---	FRONT	VERT	TOTAL	---	C1	C1
O	2-0-12	-17	-17	---	---	FRONT	VERT	TOTAL	---	C1	C1
P	6-0-4	-17	-17	---	---	FRONT	VERT	TOTAL	---	C1	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.42/1.00 (C-D:1), BC=0.26/1.00 (K-L:1),
WB=0.37/1.00 (B-L:1), SSI=0.25/1.00 (C-D:1).

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 518 354 1657 788 1987 1656

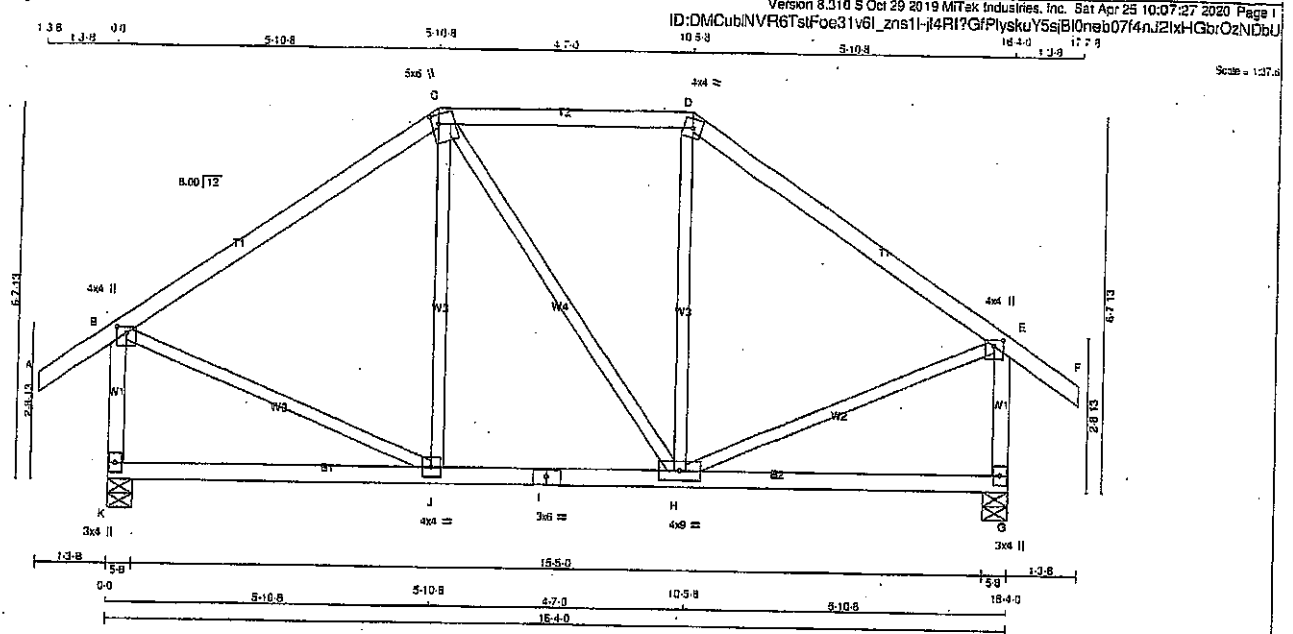
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.40 (F) (INPUT = 1.00)



JOB NAME 408131	TRUSS NAME T10	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 5 Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:07:27 2020 Page 1	



LUMBER				
N. L. & A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	4.0	4.0 1.25 2.00
C	TTWW+m	MT20	5.0	6.0 2.00 1.50
D	TTW-m	MT20	4.0	4.0
E	TMVW+p	MT20	4.0	4.0 1.25 2.00
G	BMV1+p	MT20	3.0	4.0
H	BMVWW-t	MT20	4.0	9.0
I	BS-t	MT20	3.0	6.0
J	BMVWW-t	MT20	4.0	4.0
K	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	1026	0	1026	0	0	5-8	5-8
G	1026	0	1026	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN.	COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
K	723	488 / 0	0 / 0	0 / 0	0 / 0	235 / 0	0 / 0	
G	723	488 / 0	0 / 0	0 / 0	0 / 0	235 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
A-B	0 / 35	J-C	-115 / 40
B-C	-655 / 0	C-H	0 / 0
C-D	-542 / 0	H-D	-115 / 40
D-E	-655 / 0	B-J	0 / 587
E-F	0 / 35	H-E	0 / 587
K-B	-980 / 0		
G-E	-979 / 0		
K-J	0 / 0		
J-I	0 / 543		
I-H	0 / 543		
H-G	0 / 0		

TOTAL WEIGHT = 2 X 75 = 150 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.41/1.00 (B-C:1), BC=0.17/1.00 (H-J:4), WB=0.13/1.00 (B-J:1), SS=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

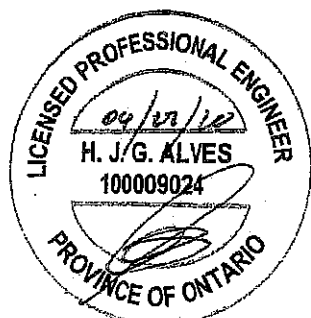
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES		PLATE GRIP (DRY)		SHEAR SECTION	
(PSI)	(PL)	(PSI)	(PL)	(PSI)	(PL)
MT20	818	354	1657	785	1987

PLATE PLACEMENT TOL = 0.250 inches

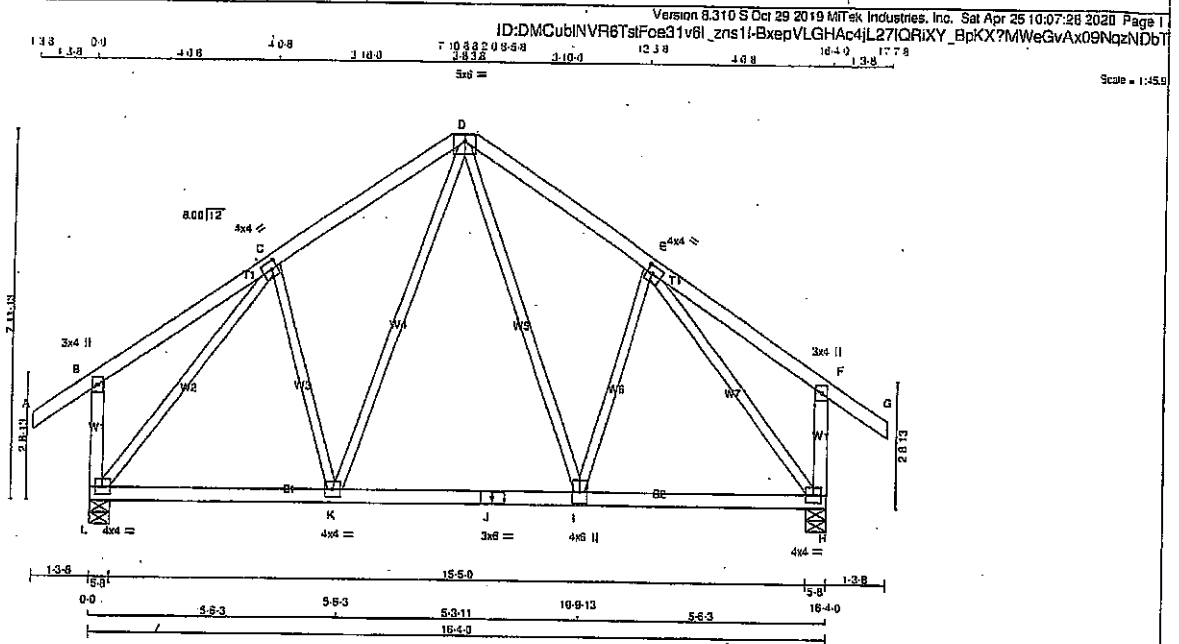
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (B) (INPUT = 0.90)
JSI METAL = 0.21 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007021

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408131	T11	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A-B	TMV+p	MT20	3.0	4.0		
B-C	TMVW-H	MT20	4.0	4.0	2.00	1.50
C-D	TMVW-W	MT20	5.0	6.0	1.50	3.00
D-E	TMVW-H	MT20	4.0	4.0	2.00	1.50
E-F	TMV+p	MT20	3.0	4.0		
F-G	BMVW-H	MT20	4.0	4.0		
G-H	BMVW-H	MT20	4.0	4.0		
H-I	BS-1	MT20	3.0	6.0		
I-J	BMVW-H	MT20	4.0	4.0		
J-K	BMVW-H	MT20	4.0	4.0		
K-L	BMVW-H	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
L	1026	0	1026	0
H	1026	0	1026	0

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED						
L	723	488 / 0	0 / 0	0 / 0	0 / 0	235 / 0	0 / 0
H	723	488 / 0	0 / 0	0 / 0	0 / 0	235 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	C-K	-105 / 24	0.05 (1)
B-C	0 / 25	-91.8	-91.8 0.25 (1)	10.00	K-D	0 / 222	0.05 (1)
C-D	-671 / 0	-91.8	-91.8 0.20 (1)	6.25	D-I	0 / 222	0.05 (1)
D-E	-869 / 0	-91.8	-91.8 0.20 (1)	6.25	I-E	-105 / 24	0.05 (1)
E-F	0 / 25	-91.8	-91.8 0.25 (1)	10.00	L-C	-917 / 0	0.65 (1)
F-G	0 / 35	-91.8	-91.8 0.12 (1)	10.00	E-H	-917 / 0	0.65 (1)
G-H	-264 / 0	0.0	0.0 0.04 (1)	7.81			
H-I	-264 / 0	0.0	0.0 0.04 (1)	7.81			
L-K	0 / 597	-18.5	-18.5 0.17 (4)	10.00			
K-J	0 / 468	-18.5	-18.5 0.16 (4)	10.00			
J-I	0 / 468	-18.5	-18.5 0.16 (4)	10.00			
I-H	0 / 597	-18.5	-18.5 0.17 (4)	10.00			

TOTAL WEIGHT = 2 X 81 = 162 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.25/1.00 (E-F-1), BC=0.17/1.00 (H-I-4),
WB=0.63/1.00 (C-L-1), SI=0.15/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1687
		788	1987
		1656	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.81 (C) (INPUT = 0.90)
JSI METAL = 0.34 (C) (INPUT = 1.00)

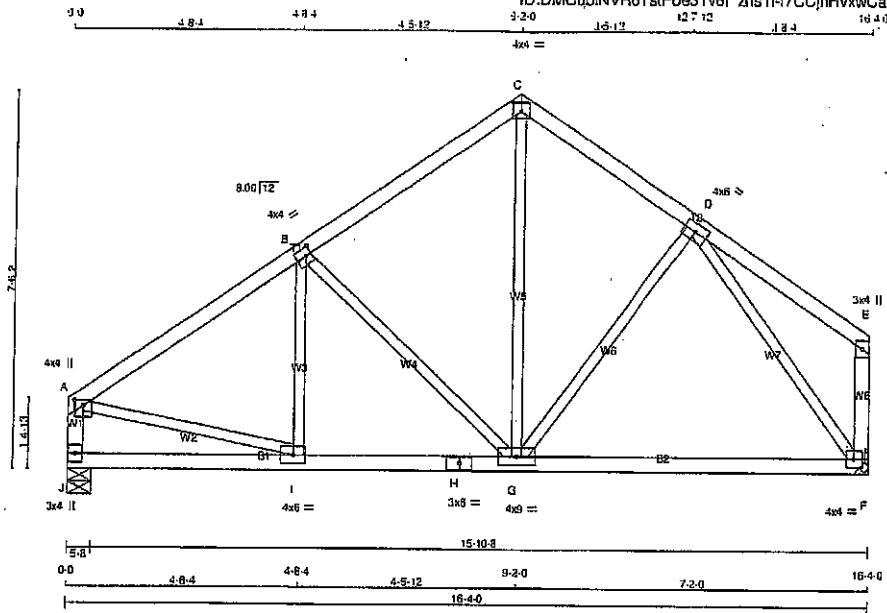


Structural component only
DWG# T-2007022

JOB NAME 408131	TRUSS NAME T12	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 5 Oct 29 2019 MITex Industries, Inc. Sat Apr 25 10:07:29 2020 Page 1
ID:DMCubINVR6TstFoe31v6I zns11-f7CCjhHvxxCazBhU 8Dm5C_?xJ9F7C2PblivGzNDbs

Scale = 1:42.3



TOTAL WEIGHT = 3 X 71 = 214 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
J - A	2x4	DRY	No.2
F - E	2x4	DRY	No.2
J - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMVW1-t	MT20	4.0	3.0		
H	BS-t	MT20	3.0	3.0		
I	BMVW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
J	900	0	900	0	5-8
F	900	0	900	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	637	418/0	0/0	0/0	0/0	219/0	0/0
F	637	418/0	0/0	0/0	0/0	219/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. LCI (L)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LCI (L)	MAX. LCI (L)
FR-TO		FROM TO		FR-TO			
A-B	-504/0	-91.8	-91.8 0.25 (1)	1-B	-104/26	0.03 (1)	
B-C	-648/0	-91.8	-91.8 0.25 (1)	5-25	B-G	-358/0	0.24 (1)
C-D	-540/0	-91.8	-91.8 0.15 (1)	8-25	G-C	0/407	0.09 (1)
D-E	0/21	-91.8	-91.8 0.19 (1)	10-00	G-D	-37/32	0.02 (1)
J-A	-561/0	0.0	0.0 0.09 (1)	7-81	A-I	0/795	0.18 (1)
F-E	-128/0	0.0	0.0 0.02 (1)	7-81	D-F	-895/0	0.54 (1)
J-I	0/0	-18.5	-18.5 0.03 (4)	10-00			
I-H	0/775	-18.5	-18.5 0.26 (4)	10-00			
H-G	0/775	-18.5	-18.5 0.26 (4)	10-00			
G-F	0/537	-18.5	-18.5 0.24 (4)	10-00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.25/1.00 (A-B:1), BC=0.28/1.00 (G-H:4),
WB=0.54/1.00 (D-F:1), SS=0.17/1.00 (A-B:1)
COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

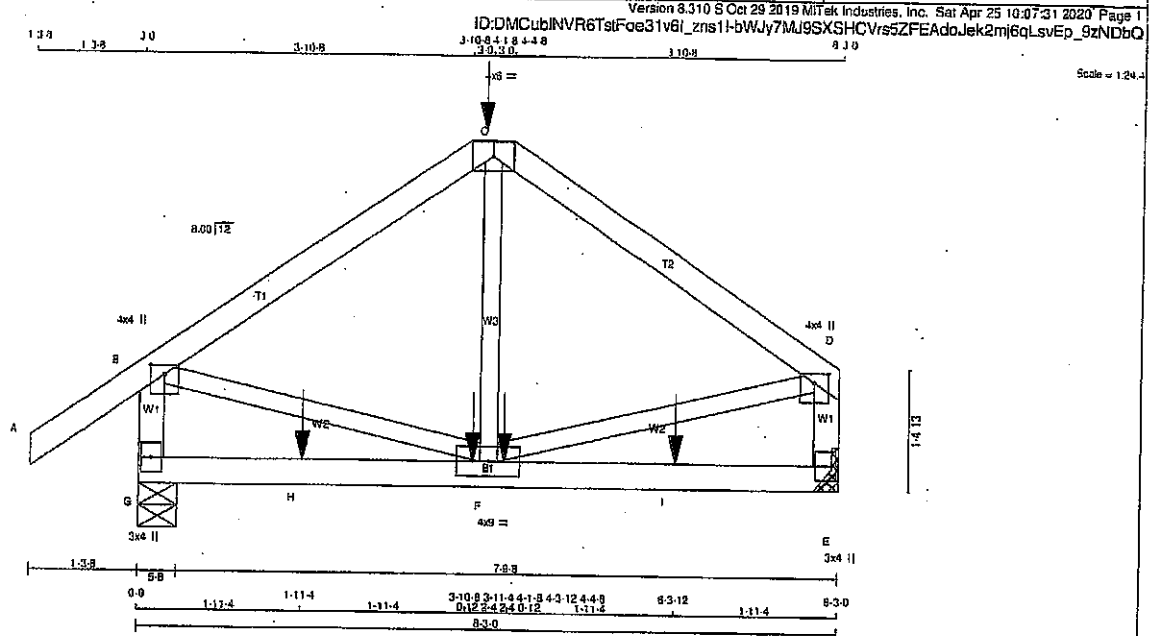
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (A) (INPUT = 0.90)
JSI METAL = 0.29 (H) (INPUT = 1.00)



Structural component only
DWG# T-2007023

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408131	T13	1	1	GREEN PARK HOMES		
Tamarack Roof Truss, Burlington						



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW+p	MT20	4.0	6.0		
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMV+p	MT20	3.0	4.0		
F	BMVW+p	MT20	4.0	9.0		
G	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

DESCR.	SPF	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
		GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
G	879	0	879	0	5-8
E	753	0	753	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 3-8.

UNFACTORED REACTIONS

1ST LOASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	819	420 / 0	0 / 0	0 / 0	0 / 0	199 / 0	0 / 0
E	533	350 / 0	0 / 0	0 / 0	0 / 0	163 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 35	-91.8	-91.8 0.14 (1)	10.00	B-F	0 / 586	0.15 (1)
B-C	-682 / 0	-91.8	-91.8 0.31 (1)	6.25	F-D	0 / 586	0.15 (1)
C-D	-682 / 0	-91.8	-91.8 0.31 (1)	6.25	F-C	-149 / 67	0.04 (1)
G-B	-942 / 0	0.0	0.0 0.10 (1)	7.61			
E-D	-716 / 0	0.0	0.0 0.08 (1)	7.61			
G-H	0 / 0	-18.5	-18.5 0.13 (4)	10.00			
H-F	0 / 0	-18.5	-18.5 0.13 (4)	10.00			
F-I	0 / 0	-18.5	-18.5 0.13 (4)	10.00			
I-E	0 / 0	-18.5	-18.5 0.13 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC1	MAX.	MAX+	FACE	DIRL	TYPE	HEEL	CONN.
C	4-1-8	-527	-527	---	BACK	VERT	TOTAL	---	C1
F	3-11-4	-17	-17	---	BACK	VERT	TOTAL	---	C1
F	4-3-12	-17	-17	---	BACK	VERT	TOTAL	---	C1
H	1-11-4	-17	-17	---	BACK	VERT	TOTAL	---	C1
I	5-3-12	-17	-17	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 34 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	8.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA D86-09, CSA D86-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.31/1.00 (C-D:1), BC=0.13/1.00 (E-F:4), WB=0.15/1.00 (B-F:1), SS=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1867
	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (B) (INPUT = 0.90)
JSI METAL = 0.20 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007024

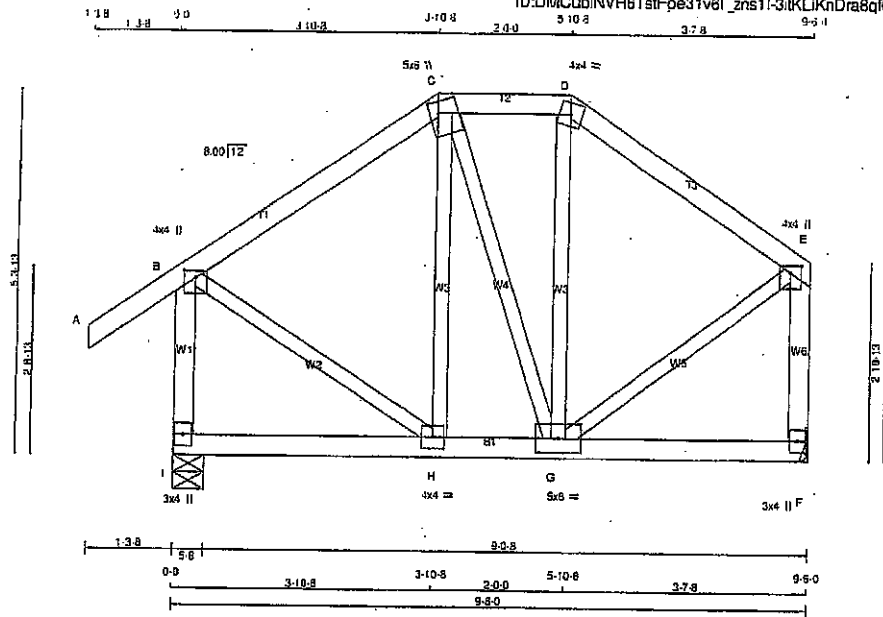
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408131	T14	2	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Sat Apr 25 10:07:32 2020 Page 1

ID:DMCubINVR6TatFpe31v6l_zns11-3IKLKndra8qIC3fHnTqLWS8NnSaKU5Z_MWbzNDbP

Scale = 1:31.1



TOTAL WEIGHT = 2 X 50 = 99 lb

LUMBER	N. L. G. A. RULES	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
I - J	2x4	DRY	No.2
J - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - N	2x4	DRY	No.2
N - O	2x4	DRY	No.2
O - P	2x4	DRY	No.2
P - Q	2x4	DRY	No.2
Q - R	2x4	DRY	No.2
R - S	2x4	DRY	No.2
S - T	2x4	DRY	No.2
T - U	2x4	DRY	No.2
U - V	2x4	DRY	No.2
V - W	2x4	DRY	No.2
W - X	2x4	DRY	No.2
X - Y	2x4	DRY	No.2
Y - Z	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	5.0	6.0		
H	BMVWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

DESCR.	SPF	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	SPF	VERT	HORZ	DOWN	HORZ
I	SPF	650	0	650	0
F	SPF	524	0	524	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	COMBINED	313 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0
F	370	243 / 0	0 / 0	0 / 0	0 / 0	127 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO				
A-B	0 / 35	-91.8	-91.8	0.12 (1)	H-C	-87 / 16	0.04 (1)		
B-C	-297 / 0	-91.8	-91.8	0.18 (1)	C-G	-19 / 0	0.01 (1)		
C-D	-238 / 0	-91.8	-91.8	0.05 (1)	G-D	-97 / 9	0.04 (1)		
D-E	-289 / 0	-91.8	-91.8	0.15 (1)	B-H	0 / 288	0.06 (1)		
E-F	-618 / 0	0.0	0.0	0.69 (1)	G-E	0 / 292	0.07 (1)		
F-G	-493 / 0	0.0	0.0	0.08 (1)					
I-H	0 / 0	-18.5	-18.5	0.05 (4)					
H-G	0 / 245	-18.5	-18.5	0.05 (4)					
G-F	0 / 0	-18.5	-18.5	0.05 (4)					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	5.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	38.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2018, ORC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.18/1.00 (B-C:1), SC=0.08/1.00 (G-H:4),
WB=0.07/1.00 (E-G:1), SS=0.12/1.00 (B-C:1)

DCL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

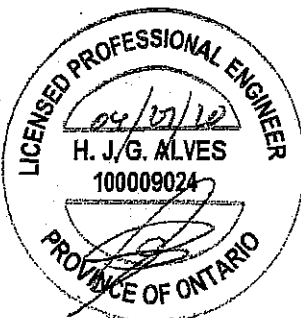
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1687 788 1967 1656	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

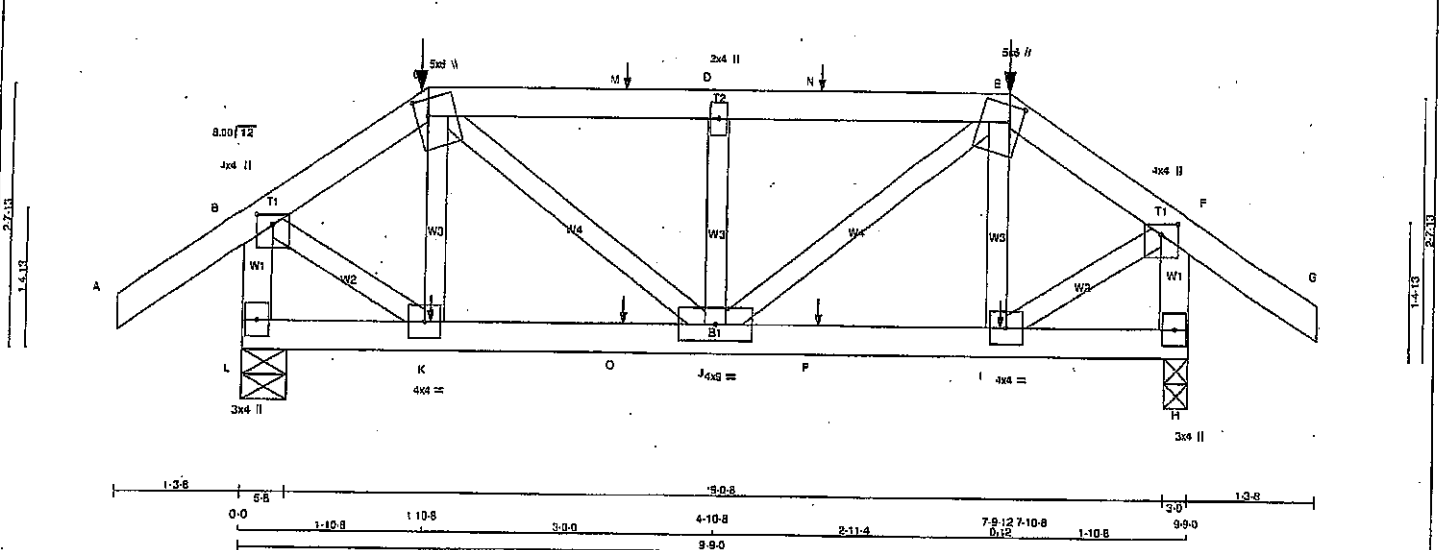
JSI GRIP= 0.43 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007025

JOB NAME: 408131 TRUSS NAME: T15 QUANTITY: 1 PLY: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC: DRWG NO. ID:DMCubiNVR6TstF0e31v6l zns1l-XvRIY2KQ_8l?Sp?FD IIF2uhnYjvB11eJDjv22zNDbo

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:07:33 2020 Page 1
Scale = 1/2" = 1'-0"



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	8.0	2.00	1.50
D	TMVW+m	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	5.0	2.00	1.50
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMVW+p	MT20	3.0	4.0		
I	BMVW+p	MT20	4.0	4.0		
J	BMVW+p	MT20	4.0	9.0		
K	BMVW+p	MT20	4.0	4.0		
L	BMVW+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	706	0	0
L VERT	706	0	0
H VERT	706	0	0

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	496	343 / 0	0 / 0	0 / 0	0 / 0	153 / 0	0 / 0
H	496	343 / 0	0 / 0	0 / 0	0 / 0	153 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD LC1	MAX. FACTORED	MEMB.	FORCE (LBS)	MAX. FACTORED	
FR-TO				FR-TO			
A-B	0 / 35	-91.8	-91.8 0.14 (1)	10.00	K-C	-140 / 0	0.02 (1)
B-C	-456 / 0	-91.8	-91.8 0.13 (1)	6.25	C-J	0 / 304	0.08 (1)
C-M	-590 / 0	-91.8	-91.8 0.14 (1)	6.25	J-D	-329 / 0	0.06 (1)
M-D	-590 / 0	-91.8	-91.8 0.14 (1)	6.25	J-E	0 / 304	0.08 (1)
D-N	-590 / 0	-91.8	-91.8 0.14 (1)	6.25	I-E	-140 / 0	0.02 (1)
N-E	-590 / 0	-91.8	-91.8 0.14 (1)	6.25	B-K	0 / 407	0.10 (1)
E-F	-456 / 0	-91.8	-91.8 0.13 (1)	6.25	I-F	0 / 407	0.10 (1)
F-G	0 / 35	-91.8	-91.8 0.14 (1)	10.00			
L-B	-693 / 0	0.0	0.0 0.03 (1)	7.81			
H-F	-693 / 0	0.0	0.0 0.03 (1)	7.81			
L-K	0 / 0	-18.5	-18.5 0.03 (4)	10.00			
K-O	0 / 351	-18.5	-18.5 0.03 (1)	10.00			
O-J	0 / 351	-18.5	-18.5 0.03 (1)	10.00			
J-P	0 / 351	-18.5	-18.5 0.03 (1)	10.00			
P-I	0 / 351	-18.5	-18.5 0.03 (1)	10.00			
I-H	0 / 0	-18.5	-18.5 0.03 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	1-10-8	-42	-42	115	BACK	VERT	TOTAL	C1
E	7-10-8	-42	-42	115	BACK	VERT	TOTAL	C1
I	7-9-12	1	1	---	BACK	VERT	TOTAL	C1
K	1-11-4	1	1	---	BACK	VERT	TOTAL	C1
M	3-11-4	1	1	115	BACK	VERT	TOTAL	C1
N	5-11-4	1	1	115	BACK	VERT	TOTAL	C1
O	3-11-4	1	1	---	BACK	VERT	TOTAL	C1
P	5-11-4	1	1	---	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.14/1.00 (D-E:1), 8C=0.08/1.00 (I-J:1), WS=0.10/1.00 (F-I:1), SS=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

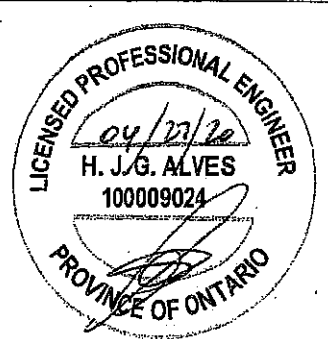
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1657 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

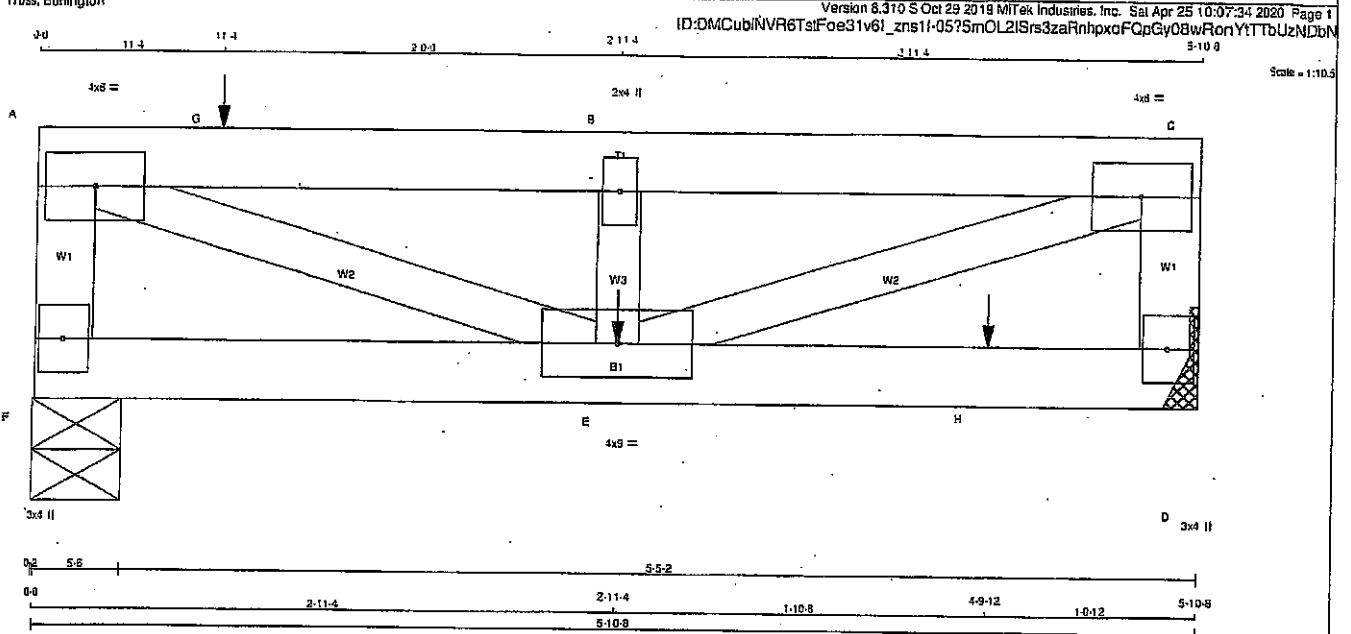
JSI GRIP = 0.48 (F) (INPUT = 0.90)

JSI METAL = 0.14 (F) (INPUT = 1.00)



Structural component only
DWG# T-2007026

JOB NAME 408131	TRUSS NAME T16	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:07:34 2020 Page 1	



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS	(0.122'x3') SPIRAL NAILS		
F-A	1	12	TOP
A-C	1	12	TOP
C-D	1	12	TOP
BOTTOM CHORDS	(0.122'x3') SPIRAL NAILS		
F-D	1	11	SIDE(97.1)
WEBS	(0.122'x3') SPIRAL NAILS		
B-E	1	6	SIDE(34.7)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	4.0	6.0		
B	TMVW-1	MT20	2.0	4.0		
C	TMVW-1	MT20	4.0	6.0		
D	BMV1+P	MT20	3.0	4.0		
E	BMVWW-1	MT20	4.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECRD BRG
	VERT	HORZ	DOWN	HORZ		
F	1250	0	1250	0	5-8 (5-6)	5-8
D	1162	0	1162	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2-0.

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
F	895	563/0	0/0	0/0	0/0	333/0	0/0
D	825	518/0	0/0	0/0	0/0	308/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS				WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1	MAX. CSI (LC)	MEMB. MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO			
F-A	-1214/0	0.0	0.0	0.07 (1)	A-E	0/2094	0.26 (1)	
A-G	-1973/0	-91.8	-91.8	0.35 (1)	E-B	-548/0	0.04 (1)	
G-B	-1973/0	-91.8	-91.8	0.35 (1)	E-C	0/2094	0.26 (1)	
B-C	-1973/0	-91.8	-91.8	0.10 (1)				
D-C	-794/0	0.0	0.0	0.04 (1)				
F-E	0/0	-43.5	-43.5	0.08 (4)				
E-H	0/0	-43.5	-43.5	0.27 (1)				
H-D	0/0	-43.5	-43.5	0.27 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-11-4	-505	-505		FRONT	VERT	TOTAL		C1
G	11-4	-617	-617		TOP	VERT	TOTAL		C1
H	4-8-12	-505	-505		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	PSF
LL	25.6	PSF
BOT CH.	LL	PSF
LL	0.0	PSF
LL	7.4	PSF
TOTAL LOAD	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIG 2011, TPIG 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.35/1.00 (A-B:1), BC=0.27/1.00 (D-E:1), WB=0.26/1.00 (A-E:1), SI=0.22/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

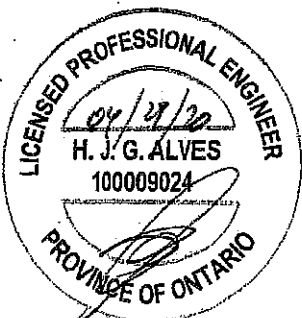
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1657	789	1997

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) INPUT = 0.90
JSI METAL= 0.25 (C) INPUT = 1.00



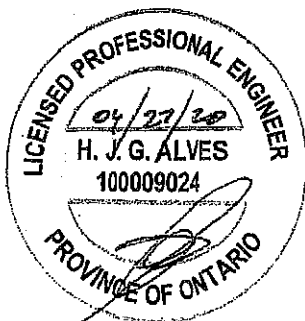
Structural component only
DWG# T-2007027 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408131	T16	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:07:34 2020 Page 2
ID:DMCubINVR6TslFoe31v6l_zns11-05?5mOL2ISrs3zaRnhpxoFQpGy08wRonYtTTqUzNObn

PLATES (table is in inches)

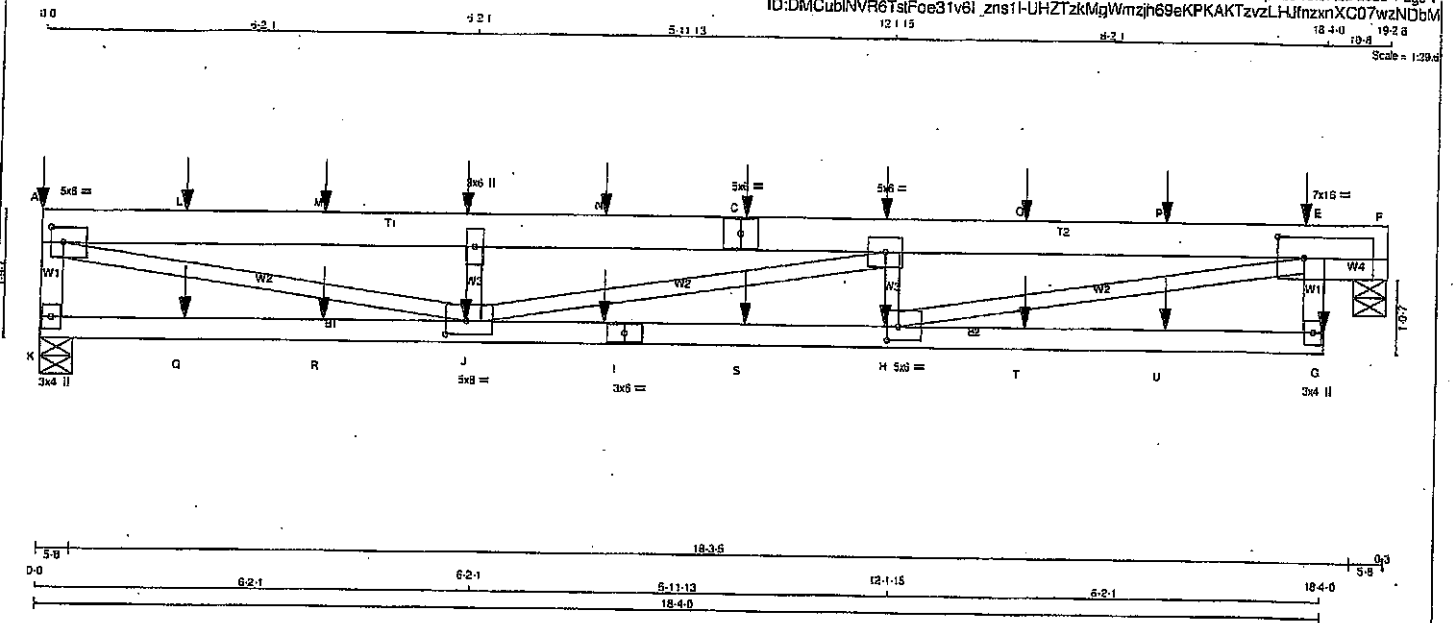
JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+0	MT20	3.0	4.0		



Structural component only
DWG# T-2007027 3/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408131	T17	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:07:35 2020 Page 1
ID:DMCubINVR6TsFoe31v6I_zns1fUHZTzKMGWmzjh69eKPAKTzvlHJfnznXC07wzNDbm



LUMBER	N. L. & A. RULES	CHORDS	SIZE	LUMBER	DESCR.
K - A	2x4	DRY	No.2	SPF	
A - C	2x6	DRY	No.2	SPF	
C - F	2x6	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
K - A	12	TOP
E - G	6	SIDE(35.8)
A - C	12	SIDE(183.1)
C - F	12	SIDE(183.1)
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
K - I	12	SIDE(61.0)
I - G	12	SIDE(61.0)
WEBS : (0.122"x3") SPIRAL NAILS		
H - D	6	SIDE(34.0)
J - B	6	SIDE(34.0)
2x3	1	
2x4	1	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
JT VERT	1740	0	5-8	5-8
K	1740	0	5-8	5-8
F	1670	0	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): F

UNFACTORED REACTIONS

1ST LCASE	MAX /MIN COMPONENT REACTIONS						
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
K	1229	816 / 0	0 / 0	0 / 0	413 / 0	0 / 0	
F	1180	779 / 0	0 / 0	0 / 0	401 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LBS)	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO			FR-TO		
K-A	-1645 / 0	0.0	0.0	0.09 (1)	H-E	0 / 5759	0.71 (1)
A-L	-5118 / 0	-91.8	-91.8	0.28 (1)	A-J	0 / 5250	0.85 (1)
L-M	-5118 / 0	-91.8	-91.8	0.28 (1)	H-D	-1023 / 0	0.08 (1)
M-B	-5118 / 0	-91.8	-91.8	0.26 (1)	J-B	-775 / 0	0.06 (1)
B-N	-5118 / 0	-91.8	-91.8	0.19 (1)	J-D	-510 / 0	0.18 (1)
N-C	-5118 / 0	-91.8	-91.8	0.19 (1)			
C-D	-5118 / 0	-91.8	-91.8	0.19 (1)			
D-O	-5615 / 0	-91.8	-91.8	0.81 (1)			
O-P	-5615 / 0	-91.8	-91.8	0.81 (1)			
P-E	-5615 / 0	-91.8	-91.8	0.81 (1)			
E-F	-6693 / 0	-91.8	-91.8	0.32 (1)			
G-E	0 / 154	0.0	0.0	0.01 (4)			
K-Q	0 / 0	-18.5	-18.5	0.12 (4)			
Q-R	0 / 0	-18.5	-18.5	0.12 (4)			
R-J	0 / 0	-18.5	-18.5	0.12 (4)			
J-I	0 / 5615	-18.5	-18.5	0.80 (1)			
I-S	0 / 5615	-18.5	-18.5	0.80 (1)			
S-H	0 / 5615	-18.5	-18.5	0.80 (1)			
H-T	0 / 0	-18.5	-18.5	0.13 (1)			
T-U	0 / 0	-18.5	-18.5	0.13 (1)			
U-G	0 / 0	-18.5	-18.5	0.13 (1)			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
A	0-0	-125	-125	---	BACK	VERT	---	C1
B	4-0-12	-82	-82	---	BACK	VERT	---	C1
C	10-0-12	-82	-82	---	BACK	VERT	---	C1
D	12-0-12	-82	-82	---	BACK	VERT	---	C1
E	18-0-12	-82	-82	---	BACK	VERT	---	C1
G	18-4-0	-56	-56	---	BACK	VERT	---	C1
H	12-0-12	-49	-49	---	BACK	VERT	---	C1
I	8-0-12	-49	-49	---	BACK	VERT	---	C1
J	5-0-12	-49	-49	---	BACK	VERT	---	C1
L	2-0-12	-82	-82	---	BACK	VERT	---	C1
M	4-0-12	-82	-82	---	BACK	VERT	---	C1
N	3-0-12	-82	-82	---	BACK	VERT	---	C1
O	14-0-12	-82	-82	---	BACK	VERT	---	C1
P	16-0-12	-82	-82	---	BACK	VERT	---	C1
Q	2-0-12	-49	-49	---	BACK	VERT	---	C1

TOTAL WEIGHT = 2 X 75 = 149 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/984 (0.23")
ALLOWABLE DEFL.(TL) = L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/521 (0.42")

CSI: TC=0.61/1.00 (D-E-T), SC=0.80/1.00 (H-J-I), WB=0.71/1.00 (E-H-I), SI=0.31/1.00 (E-F-T)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.95 (I) (INPUT = 1.00)



Structural component only
DWG# T-2007028

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408131	T17	1	2	TRUSS DESC.		

Tamerack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mtek Industries, Inc. Sat Apr 25 10:07:35 2020 Page 2
ID:DMCubINVR6TsiFoe31v6I_zns1I-UHZTzkMqWmzih69eKPKAKTzyzLHJInzxnXC07wzNDpM

PLATES (table is in inches)

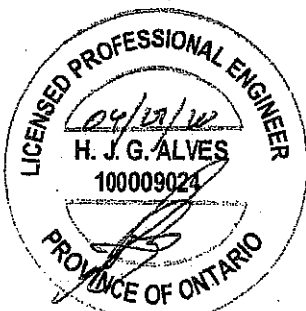
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	5.0	5.0	2.50	2.00
B	TMW+w	MT20	3.0	6.0		
C	TS-1	MT20	5.0	6.0		
D	TMVWW-1	MT20	5.0	6.0		
E	TMVWW-1	MT20	7.0	16.0	3.50	4.50
G	BMV+P	MT20	3.0	4.0		
H	BMVW-1	MT20	5.0	6.0	2.25	2.00
I	BS-1	MT20	3.0	5.0		
J	BMVWW-1	MT20	5.0	6.0	2.25	3.50
K	BMV1+P	MT20	3.0	4.0		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	4-0-12	-49	-49	---	BACK	VERT	TOTAL	---	C1
S	10-0-12	-49	-49	---	BACK	VERT	TOTAL	---	C1
T	14-0-12	-49	-49	---	BACK	VERT	TOTAL	---	C1
U	16-0-12	-49	-49	---	BACK	VERT	TOTAL	---	C1

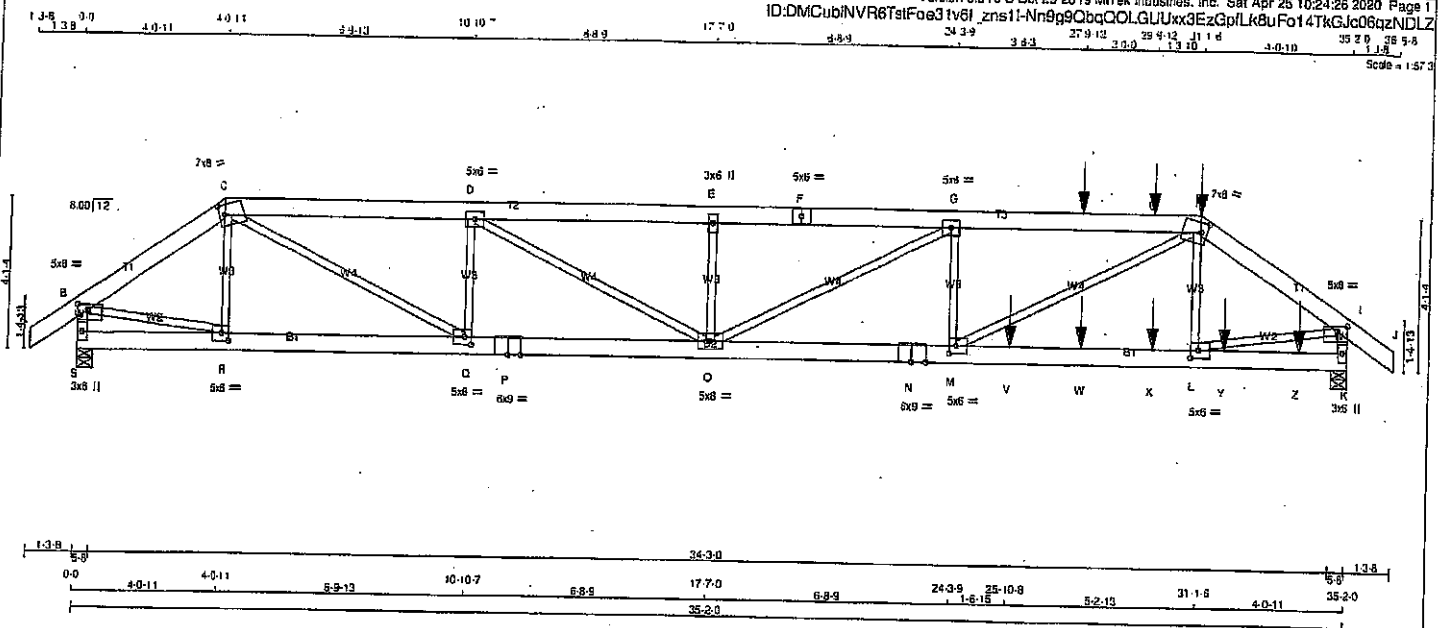
CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007028

JOB NAME 408132	TRUSS NAME T1Z2	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 26 10:24:26 2020 Page 1		



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x6	DRY	No.2
C - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - J	2x6	DRY	No.2
J - L	2x6	DRY	No.2
L - M	2x6	DRY	No.2
M - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
C-F	12	TOP
F-H	12	TOP
H-J	12	TOP
J-L	12	TOP
L-M	12	TOP
M-N	12	TOP
N-K	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S-P	12	TOP
P-N	12	TOP
N-K	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	UP	BRG	IN-SX	BRG	IN-SX
S	2692	0	2692	0	0	5-8	5-8	5-8	5-8
K	4101	0	4101	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1900	1271 / 0	0 / 0	0 / 0	0 / 0	629 / 0	0 / 0
K	2693	1938 / 0	0 / 0	0 / 0	0 / 0	955 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.03 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	CS1 (LC)	MEMB.	FORCE (LBS)	VERT. LOAD	CS1 (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 36	-91.8	-91.8 0.04 (1)	R-C	-442 / 0	0.05 (1)	
B-C	-3201 / 0	-91.8	-91.8 0.03 (1)	C-Q	0 / 3750	0.46 (1)	
C-D	-5955 / 0	-91.8	-91.8 0.22 (1)	Q-D	-1603 / 0	0.19 (1)	
D-E	-7669 / 0	-91.8	-91.8 0.25 (1)	D-O	0 / 1951	0.24 (1)	
E-F	-7669 / 0	-91.8	-91.8 0.25 (1)	O-E	-575 / 0	0.07 (1)	
F-G	-7669 / 0	-91.8	-91.8 0.25 (1)	G-H	-915 / 0	0.53 (1)	
G-T	-8473 / 0	-91.8	-91.8 0.40 (1)	H-M	0 / 4812	0.04 (1)	
T-U	-8473 / 0	-91.8	-91.8 0.40 (1)	M-H	0 / 4812	0.57 (1)	
U-H	-8473 / 0	-91.8	-91.8 0.40 (1)	H-L	-128.99	0.02 (4)	
H-I	-5308 / 0	-91.8	-91.8 0.11 (1)	B-R	0 / 2717	0.34 (1)	
I-J	0 / 36	-91.8	-91.8 0.04 (1)	L-I	0 / 4506	0.56 (1)	
S-P	-2675 / 0	0.0	0.0 0.15 (1)				
K-I	-4227 / 0	0.0	0.0 0.23 (1)				
S-R	0 / 0	-18.5	-18.5 0.04 (4)				
R-Q	0 / 2644	-18.5	-18.5 0.20 (1)				
Q-P	0 / 5955	-18.5	-18.5 0.48 (1)				
P-O	0 / 5955	-18.5	-18.5 0.48 (1)				
O-N	0 / 8473	-18.5	-18.5 0.88 (1)				
N-M	0 / 8473	-18.5	-18.5 0.88 (1)				
M-V	0 / 4401	-18.5	-18.5 0.82 (1)				
V-W	0 / 4401	-18.5	-18.5 0.82 (1)				
W-X	0 / 4401	-18.5	-18.5 0.82 (1)				
X-L	0 / 4401	-18.5	-18.5 0.82 (1)				
L-Y	0 / 0	-18.5	-18.5 0.29 (1)				
Y-Z	0 / 0	-18.5	-18.5 0.29 (1)				
Z-K	0 / 0	-18.5	-18.5 0.29 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
H	31-1-6	-49	-55	---	FRONT	VERT	DEAD	---	C1
H	31-1-6	-254	-254	---	FRONT	VERT	SNOW	---	C1
T	27-9-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
U	29-9-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
V	25-10-8	-2027	-2027	---	FRONT	VERT	TOTAL	---	C1
W	27-9-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
X	29-9-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Y	31-9-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Z	33-9-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

TOTAL WEIGHT = 2 X 180 = 359 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
DL	=	8.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	33.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17")

CALCULATED VERT. DEFL. (LL) = L/999 (0.22")

ALLOWABLE DEFL. (TL) = L/360 (1.17")

CALCULATED VERT. DEFL. (TL) = L/999 (0.40")

CS1: TC=0.40/1.00 (G-H:1), BC=0.92/1.00 (L-M:1), WB=0.57/1.00 (H-M:1), SS=0.54/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

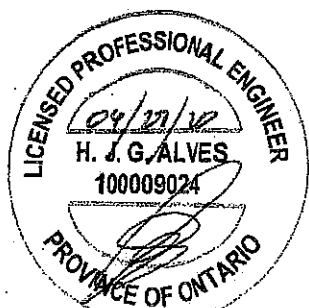
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1657
	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (C) (INPUT = 0.90)
JSI METAL = 0.81 (P) (INPUT = 1.00)



Structural component only
DWG# T-2007029 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408132	T122	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

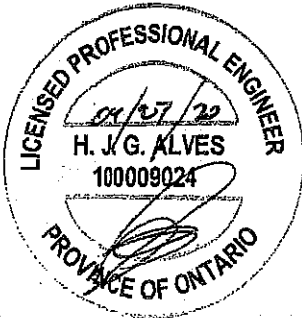
Version 8.310 5 Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:24:26 2020 Page 2
ID:DMCubINVR6T8tFoe31v6I zns1I-Nn9n9QbgQOLGUUxx3EzGp/Lk8uFo14TkGJc06qzNDLZ

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	3.50
C	TTWW-m	MT20	7.0	8.0	3.25	2.00
D	TMWW-t	MT20	5.0	6.0		
E	TMW-tw	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW-m	MT20	7.0	8.0	3.25	2.00
I	TMVW-p	MT20	5.0	8.0	1.75	3.50
K	BMV1-p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M	BMWW-t	MT20	5.0	6.0	2.50	2.25
N	BS-t	MT20	6.0	9.0		
O	BMWW-t	MT20	5.0	8.0		
P	BS-t	MT20	6.0	9.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.25
R	BMWW-t	MT20	5.0	6.0	2.50	2.50
S	BMV1-p	MT20	3.0	6.0		

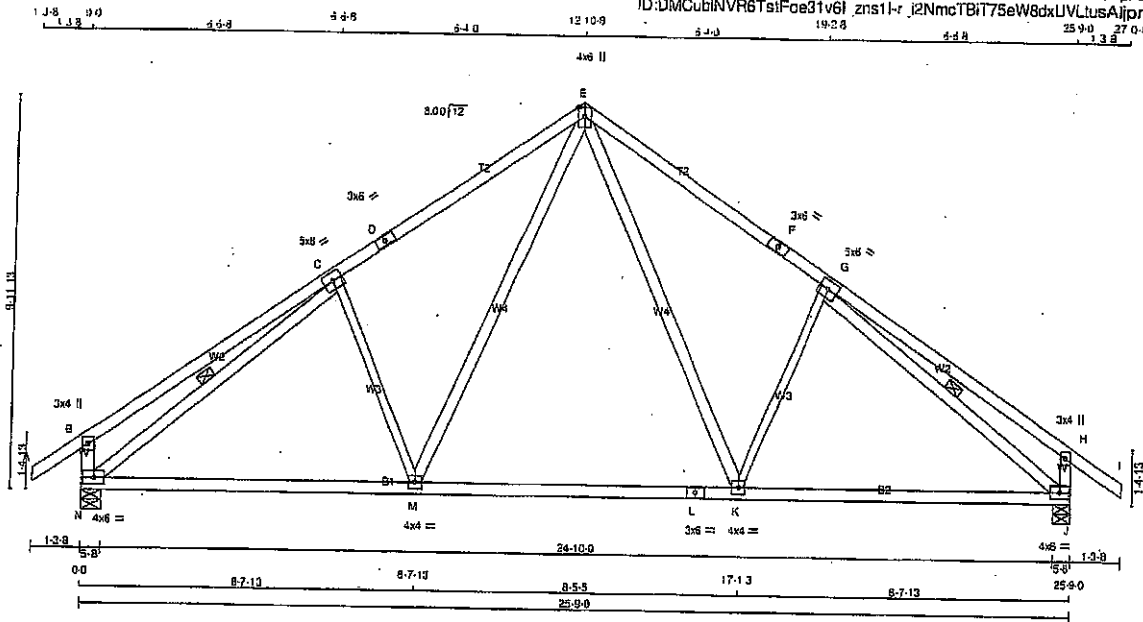
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



JOB NAME 408132	TRUSS NAME T20	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Famarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mittek Industries, Inc. Sat Apr 25 10:24:27 2020 Page 1
ID:DMCubINVR6TstFoe31v6l zns1-r j2NmctBt75eW8dxUvLusAljpmWQuUzLaeGzNDLY



Scale = 1/32"

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
N - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
K - G	2x3	DRY	No.2
C - M	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0		
D	TS-t	MT20	3.0	6.0		
E	TTWW+p	MT20	4.0	6.0		Edge
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	EMVW-t	MT20	4.0	6.0		
K	EMVW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	EMVW-t	MT20	4.0	4.0		
N	EMVW-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
N	1545	0	1545	0	5-8	5-8
J	1545	0	1545	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX/MIN COMPONENT REACTIONS		PERM. LIVE WIND	DEAD	SOIL
	SNOW	LIVE	MAX	MIN			
N	1091	729 / 0	0 / 0	0 / 0	0 / 0	352 / 0	0 / 0
J	1091	729 / 0	0 / 0	0 / 0	0 / 0	352 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-N, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	E-K	0 / 619	0.10 (1)
B-C	0 / 40	-91.8	-91.8	0.63 (1)	10.00	K-G	-428 / 0	0.25 (1)
C-D	-1469 / 0	-91.8	-91.8	0.56 (1)	4.72	M-E	0 / 619	0.10 (1)
D-E	-1469 / 0	-91.8	-91.8	0.56 (1)	4.72	C-M	-428 / 0	0.25 (1)
E-F	-1469 / 0	-91.8	-91.8	0.56 (1)	4.72	N-C	-1767 / 0	0.59 (1)
F-G	-1469 / 0	-91.8	-91.8	0.56 (1)	4.72	G-J	-1767 / 0	0.59 (1)
G-H	0 / 40	-91.8	-91.8	0.63 (1)	10.00			
H-I	0 / 35	-91.8	-91.8	0.12 (1)	10.00			
N-B	-347 / 0	0.0	0.0	0.04 (1)	7.81			
J-H	-347 / 0	0.0	0.0	0.04 (1)	7.81			
N-M	0 / 1356	-18.5	-18.5	0.42 (4)	10.00			
M-L	0 / 942	-18.5	-18.5	0.39 (4)	10.00			
L-K	0 / 942	-18.5	-18.5	0.39 (4)	10.00			
K-J	0 / 1356	-18.5	-18.5	0.42 (4)	10.00			

TOTAL WEIGHT = 4 X 121 = 485 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.86")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.06")
ALLOWABLE DEFL. (TL) = $L/360$ (0.86")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.19")

CSI: TC=0.63/1.00 (B-C:1), BC=0.42/1.00 (J-K:4), WB=0.59/1.00 (C-N:1), SS=0.24/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

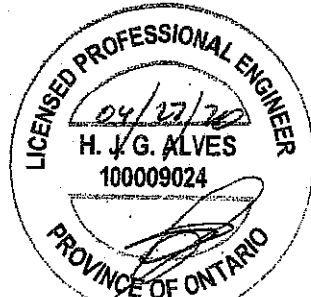
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1997 1665

PLATE PLACEMENT TOL. = 0.250 inches

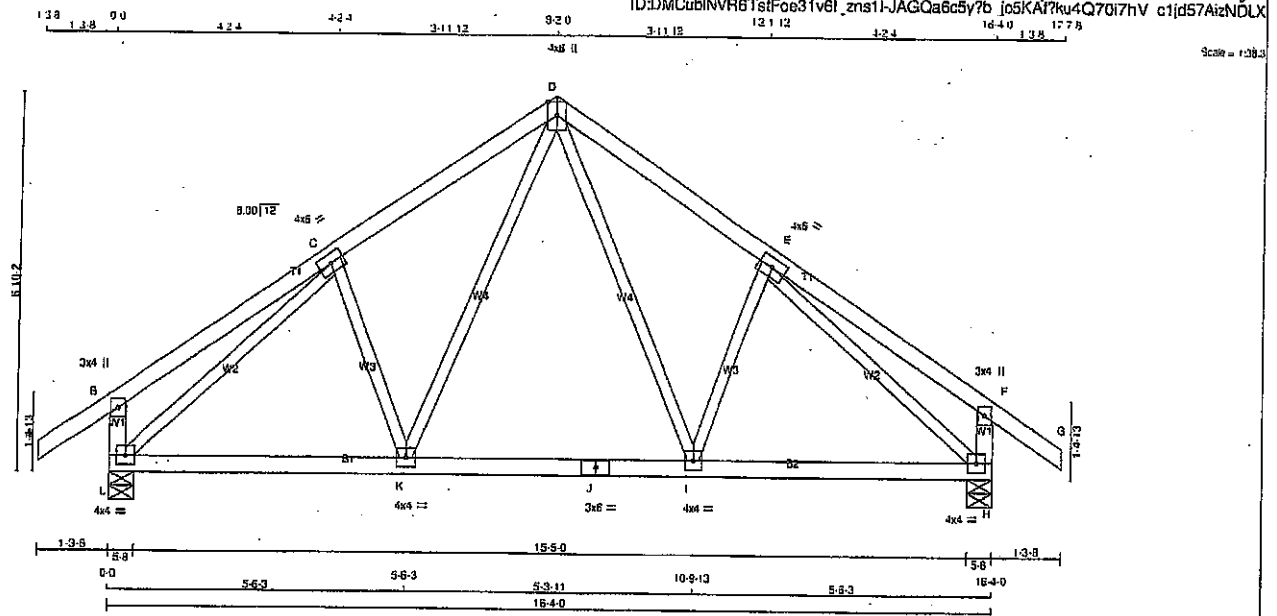
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (J) (INPUT = 0.90)
JSI METAL = 0.46 (G) (INPUT = 1.00)



Structural component only
DWG# T-2007030

JOB NAME 408132	TRUSS NAME T21	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 MITek Industries, Inc. Sat Apr 25 10:24:28 2020 Page 1 ID:DMCubINVR6TstFoe31vbl_zns11-JAGQa6c5y7b_jo5KA7ku4Q70i7hV_c1jd57AizNDLX		



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (tablets in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMVW-t	MT20	4.0	6.0
D	TTWW+p	MT20	4.0	6.0
E	TMVW-t	MT20	4.0	6.0
F	TMV+p	MT20	3.0	4.0
H	BMVW1-t	MT20	4.0	4.0
I	BMVW1-t	MT20	4.0	4.0
J	BS-t	MT20	3.0	6.0
K	BMVW1-t	MT20	4.0	4.0
L	BMVW1-t	MT20	4.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	VERT	GROSS REACTION	BRG	BRG
L	1025	0	1025	0	5-8
H	1025	0	1025	0	5-8

UNFACTORED REACTIONS		1ST LOSE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
L	723	488 / 0	0 / 0
H	723	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)		CHORDS	WEBS
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF) CSI (LC)	FACTORED MAX. FORCE (LBS) MAX CSI (LC)
FR-TO			
A-B	0 / 35	-91.8 -91.8 0.12 (1)	10.00
B-C	0 / 24	-91.8 -91.8 0.24 (1)	10.00
C-D	-851 / 0	-91.8 -91.8 0.19 (1)	6.25
D-E	-851 / 0	-91.8 -91.8 0.19 (1)	6.25
E-F	0 / 24	-91.8 -91.8 0.24 (1)	10.00
F-G	0 / 35	-91.8 -91.8 0.12 (1)	10.00
L-B	-289 / 0	0.0 0.0 0.03 (1)	7.81
H-F	-289 / 0	0.0 0.0 0.03 (1)	7.81
L-K	0 / 787	-18.5 -18.5 0.19 (1)	10.00
K-J	0 / 581	-18.5 -18.5 0.17 (4)	10.00
J-I	0 / 581	-18.5 -18.5 0.17 (4)	10.00
I-H	0 / 787	-18.5 -18.5 0.19 (1)	10.00

TOTAL WEIGHT = 4 X 73 = 291 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.24/1.00 (B-C:1), BC=0.19/1.00 (H-I:1), WB=0.53/1.00 (C-L:1), SS=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

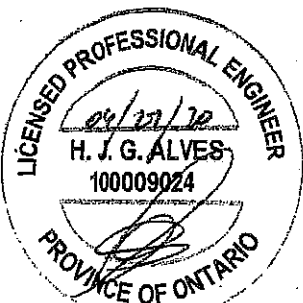
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (H) (INPUT = 0.90)
JSI METAL = 0.29 (L) (INPUT = 1.00)

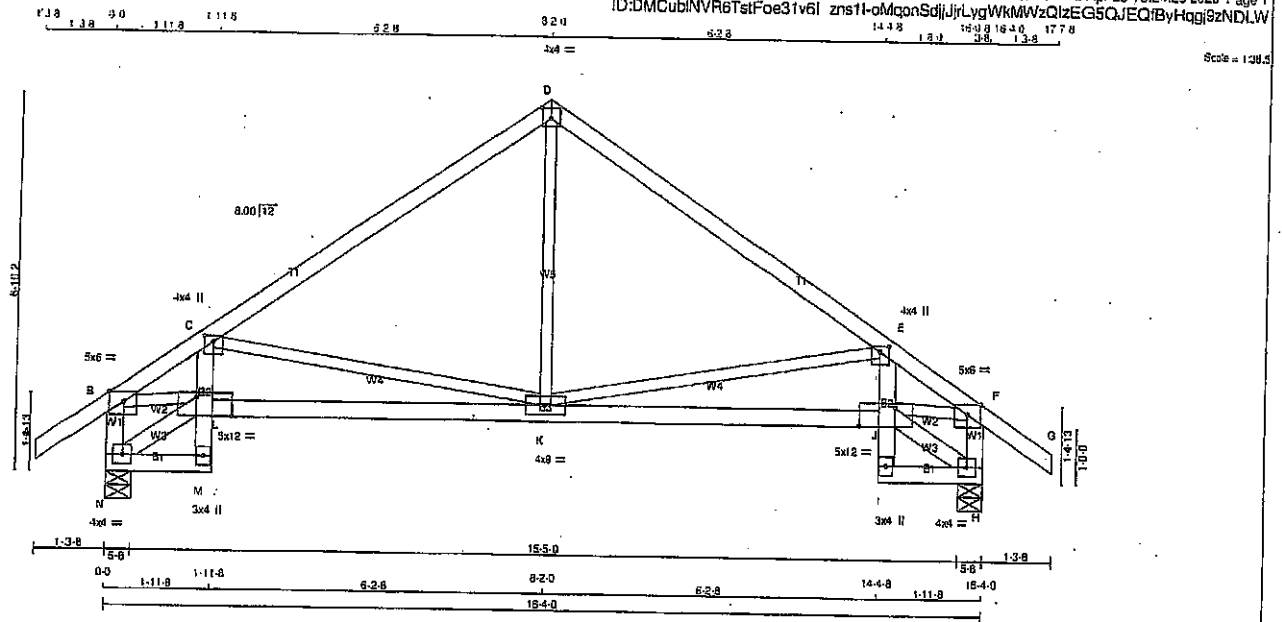


Structural component only
DWG# T-2007031

JOB NAME 408132	TRUSS NAME T21S	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
--------------------	--------------------	---------------	----------	-------------------------------	-------------	----------

Tamarack Roof Truss, Burlington

Version 8.310 5 Oct 25 2019 Mitek Industries, Inc. Sat Apr 25 10:24:29 2020 Page 1
ID:DMCubINVR6TstFoe31v6I zns11-oMgonSdijLrLgWkMWzQlZEG5QJEqByHggI9zNDLW



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
N - M	2x4	DRY	No.2
M - C	2x4	DRY	No.2
L - J	2x4	DRY	No.2
I - E	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
N - L	2x4	DRY	No.2
J - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-p	MT20	4.0	4.0	1.25	2.00
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-p	MT20	4.0	4.0	1.25	2.00
F	TMVW-p	MT20	5.0	6.0	Edge	
H	BMVW-1	MT20	4.0	4.0		
I	BMVW-p	MT20	3.0	4.0		
J	BVMWV-1	MT20	5.0	12.0	4.00	8.00
K	BVMWV-1	MT20	4.0	9.0		
L	BVMWV-1	MT20	5.0	12.0	4.00	8.00
M	BMVW-p	MT20	3.0	4.0		
N	BMVW-1	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
N	1025	0	1025	0	0	5-8	5-8	5-8	5-8
H	1025	0	1025	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	723	488/0	0/0	0/0	0/0	235/0	0/0
H	723	488/0	0/0	0/0	0/0	235/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX.		WEBS		FACTORED		MAX.	
MEMB.	FORCE (LBS)	VERT.	LOAD (PL)	LC1	MAX	MEMB.	FORCE (LBS)	VERT.	LOAD (PL)	LC1	MAX
FR-TO						FR-TO					
A-B	0/35	-91.8	-91.8	0.12 (1)	10.00	K-D	0/405	0.09 (1)			
B-C	-1592/0	-91.8	-91.8	0.36 (1)	4.83	K-E	-787/0	0.61 (1)			
C-D	-829/0	-91.8	-91.8	0.47 (1)	6.01	C-K	-787/0	0.61 (1)			
D-E	-829/0	-91.8	-91.8	0.47 (1)	6.01	N-L	-68/0	0.01 (1)			
E-F	-1592/0	-91.8	-91.8	0.36 (1)	4.83	B-L	0/1383	0.31 (1)			
F-G	0/35	-91.8	-91.8	0.12 (1)	10.00	J-H	-68/0	0.01 (1)			
N-B	-974/0	0.0	0.0	0.10 (1)	7.81	J-F	0/1383	0.31 (1)			
H-F	-974/0	0.0	0.0	0.10 (1)	7.81						
N-M	0/58	-18.5	-18.5	0.02 (4)	10.00						
M-L	0/19	0.0	0.0	0.09 (1)	10.00						
L-C	0/84	0.0	0.0	0.10 (1)	10.00						
L-K	0/1438	-18.5	-18.5	0.36 (1)	10.00						
K-J	0/1438	-18.5	-18.5	0.36 (1)	10.00						
I-J	0/19	0.0	0.0	0.09 (1)	10.00						
J-E	0/84	0.0	0.0	0.10 (1)	10.00						
I-H	0/58	-18.5	-18.5	0.02 (4)	10.00						

TOTAL WEIGHT = 2 X 72 = 145 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OSC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.54")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.03")
ALLOWABLE DEFL.(TL) = 1/360 (0.54")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.03")

CSI: TC=0.47/1.00 (C-D:1), BC=0.35/1.00 (K-L:1),
WB=0.61/1.00 (C-K:1), SS=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

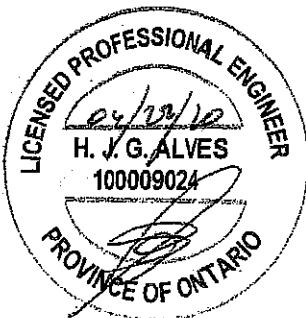
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 788 1997 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.69 (F) (INPUT = 0.90)
JSI METAL = 0.33 (L) (INPUT = 1.00)

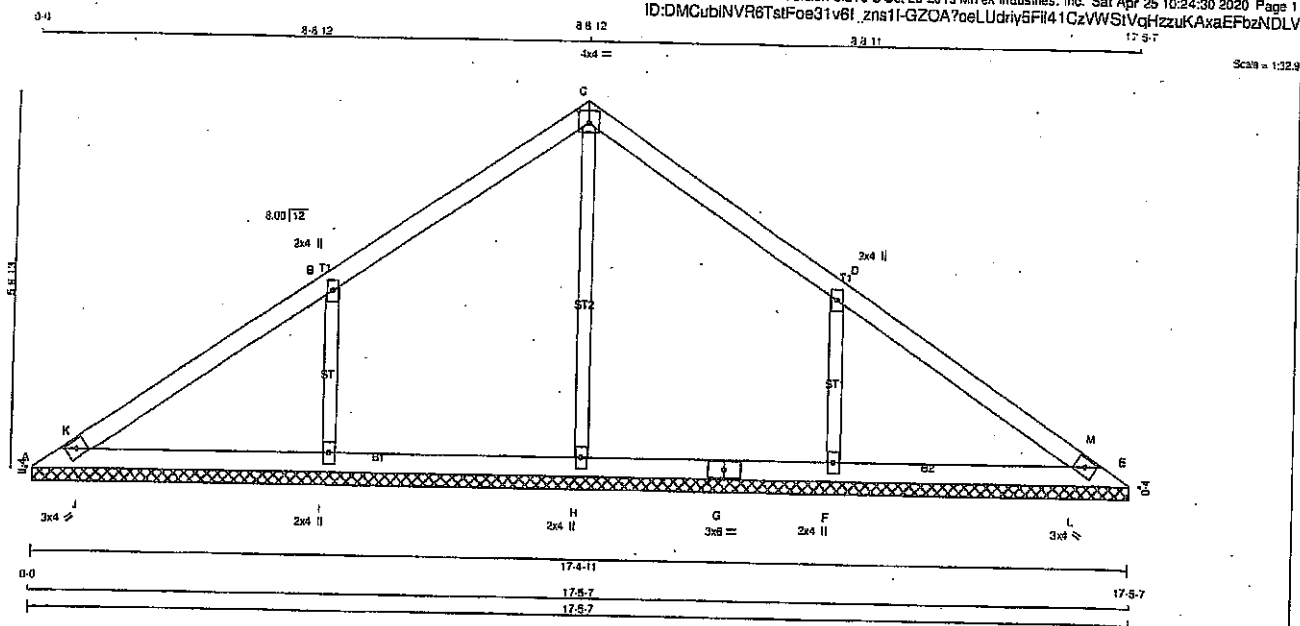


Structural component only
DWG# T-2007032

JOB NAME 408132	TRUSS NAME V20 ✓	QUANTITY 1 ✓	PLY 1 ✓	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
--------------------	---------------------	-----------------	------------	-------------------------------	-------------	----------

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Sat Apr 26 10:24:30 2020 Page 1
ID:DMCubINVR6TstFoe31v6f_zns1f-GZOA7oeLUdriy5Flt41CzVWSIVqHzzuKXaEFbzNDLV



TOTAL WEIGHT = 51 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
A - G	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	HORIZ	GROSS REACTION	HORIZ	BRG	IN-SK	BRG	IN-SK
A	137	0	137	0	0	17'-4-11 (6'-4-15/4-11			
E	137	0	137	0	0	17'-4-11 (6'-4-15/4-11			
H	482	0	482	0	0	17'-4-11 (6'-4-15/4-11			
I	580	0	580	0	0	17'-4-11 (6'-4-15/4-11			
F	580	0	580	0	0	17'-4-11 (6'-4-15/4-11			

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
A	97	66 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
E	97	66 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
H	344	211 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0
I	410	273 / 0	0 / 0	0 / 0	0 / 0	137 / 0	0 / 0
F	410	273 / 0	0 / 0	0 / 0	0 / 0	137 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, H, I, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	MAX	LC1 (LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-K	0 / 109	-91.8	-91.8 0.05 (4)	H-C	-439 / 0	0.22	(1)
K-B	0 / 158	-91.8	-91.8 0.28 (1)	I-B	-453 / 0	0.08	(1)
B-C	0 / 116	-91.8	-91.8 0.28 (1)	F-D	-453 / 0	0.08	(1)
C-D	0 / 116	-91.8	-91.8 0.28 (1)	J-K	-122 / 6	0.00	(1)
D-M	0 / 158	-91.8	-91.8 0.28 (1)	L-M	-122 / 6	0.00	(1)
M-E	0 / 109	-91.8	-91.8 0.05 (4)				
A-J	-126 / 0	-18.5	-18.5 0.12 (1)				
J-I	-105 / 0	-18.5	-18.5 0.12 (1)				
I-H	-118 / 0	-18.5	-18.5 0.09 (1)				
H-G	-118 / 0	-18.5	-18.5 0.09 (1)				
G-F	-118 / 0	-18.5	-18.5 0.09 (1)				
F-L	-105 / 0	-18.5	-18.5 0.12 (1)				
L-E	-126 / 0	-18.5	-18.5 0.12 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.28/1.00 (B-K:1), BC=0.12/1.00 (I-J:1),
WB=0.22/1.00 (C-H:1), SS=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1987 1558

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

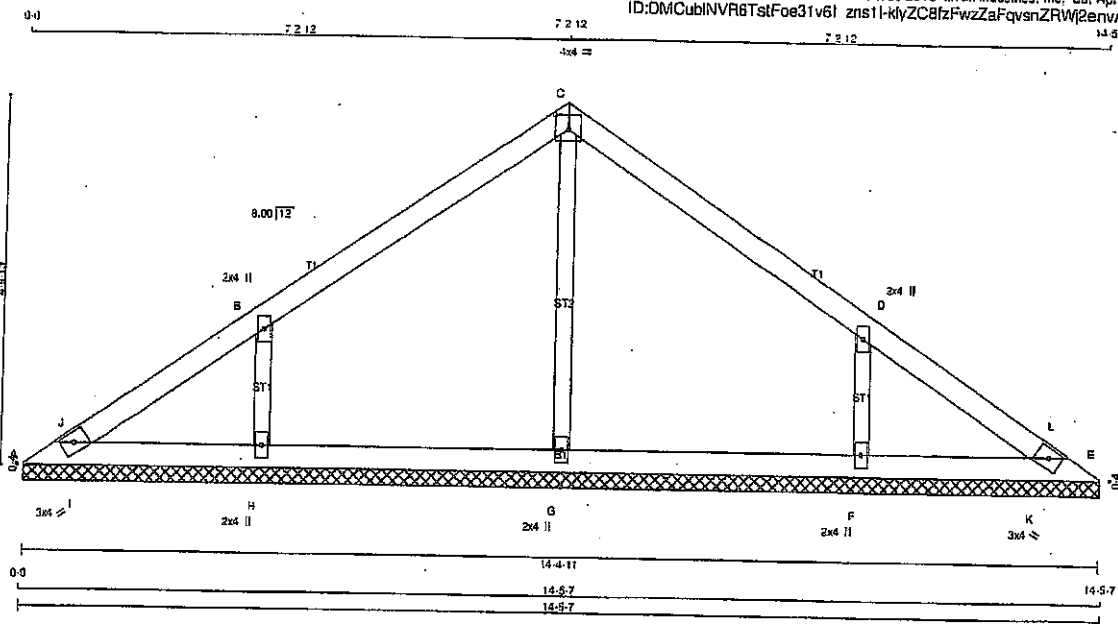
JSI GRIP= 0.40 (D) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007033

JOB NAME 408132	TRUSS NAME V21	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mittek Industries, Inc. Sat Apr 25 10:24:31 2020 Page 1
ID:DMCubINVR6TstFoe31v6I zns1l-klyZC8fzFwzZaFqsnZRWj2envAQiS5UPbJnn1zNDLU



TOTAL WEIGHT = 41 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
A - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
A	128	0	128	0	14-4-11	14-4-11
E	128	0	128	0	14-4-11	14-4-11
G	361	0	361	0	14-4-11	14-4-11
H	485	0	485	0	14-4-11	14-4-11
F	485	0	485	0	14-4-11	14-4-11

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW-w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMW-w	MT20	2.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F, G, H						
F	BMW1-w	MT20	2.0	4.0		

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	90	62/0	0/0	0/0	0/0	28/0	0/0
E	90	62/0	0/0	0/0	0/0	28/0	0/0
G	259	153/0	0/0	0/0	0/0	105/0	0/0
H	342	230/0	0/0	0/0	0/0	112/0	0/0
F	342	230/0	0/0	0/0	0/0	112/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

FR-TO	CHORDS			WEBS		
	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF) CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
A-J	-44/0	-91.8	-91.8 0.04 (1)	G-C	-292/0	0.10 (1)
J-B	0/15	-91.8	-91.8 0.21 (1)	H-B	-399/0	0.06 (1)
B-C	-28/4	-91.8	-91.8 0.21 (1)	F-D	-399/0	0.06 (1)
C-D	-28/4	-91.8	-91.8 0.21 (1)	I-J	-28/7	0.00 (1)
D-L	0/15	-91.8	-91.8 0.21 (1)	K-L	-28/7	0.00 (1)
L-E	-44/0	-91.8	-91.8 0.04 (1)			
A-I	-5/18	-18.5	-18.5 0.04 (1)			
I-H	-3/24	-18.5	-18.5 0.05 (4)			
H-G	-7/6	-18.5	-18.5 0.06 (4)			
G-F	-7/6	-18.5	-18.5 0.06 (4)			
F-K	-3/24	-18.5	-18.5 0.05 (4)			
K-E	-5/18	-18.5	-18.5 0.04 (1)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
OL = 6.0 PSF
BOT CH. LL = 0.0 PSF
OL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (B-J:1), BC=0.06/1.00 (G-H:4), WB=0.10/1.00 (C-G:1), SS=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

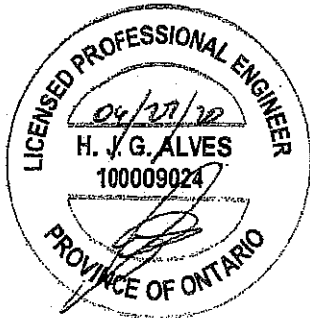
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1567 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.27 (D) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

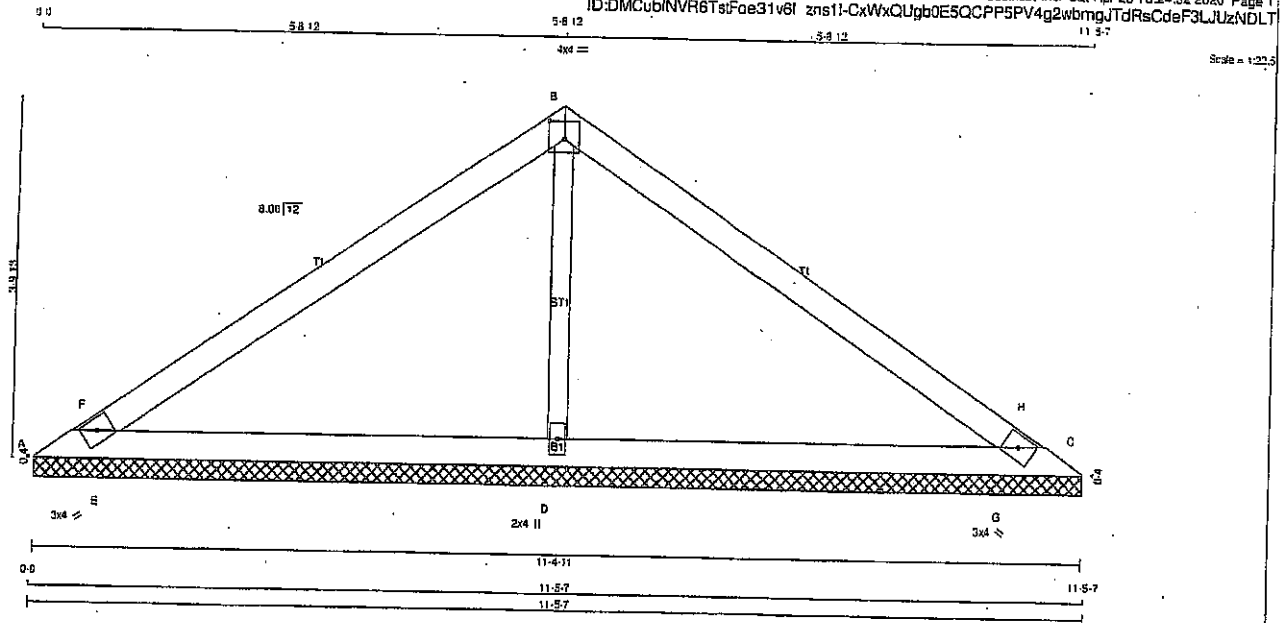


Structural component only
DWG# T-2007034

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408132	V22	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 5 Oct 29 2019 MITek Industries, Inc. Sat Apr 25 10:24:32 2020 Page 1
ID:DMCub\NVR6Ts\Fae31v6f zns11-CxWxQUgb0ESQCPP5PV4g2wbmgJTdRsCdeF3LJuzNDLT



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF		
B - C	2x4	DRY	No.2	SPF		
A - C	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

PLATES (tablets in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A - B	TBM1-h	MT20	3.0	4.0			
B - C	TTW-p	MT20	4.0	4.0	2.25	2.00	
C - B	TBM1-h	MT20	3.0	4.0			
D - B	BMW1-w	MT20	2.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
A	-11	0	0	0	-11	11-4-11	11-4-11	
C	-11	0	0	0	-11	11-4-11	11-4-11	
D	1277	0	1277	0	0	11-4-11	11-4-11	

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

7012

UNFACTORED REACTIONS							
1ST CASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	-7	0/-7	0/0	0/0	0/0	0/0	0/0
C	-7	0/-7	0/0	0/0	0/0	0/0	0/0
D	902	597/0	0/0	0/0	0/0	305/0	0/0

BEARING MATERIAL TO BE SPF NO 2 OR BETTER AT JOINT(S) A, C, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	FACTORED LCH1 MAX	MAX. FACTORED FORCE (LBS)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LCH1 MAX
FR-TO						FR-TO			
A-F	0/591	-91.8	-91.8	0.25 (1)	10.00	D-B	-1043/0	0.23 (1)	
F-B	0/564	-91.8	-91.8	0.39 (1)	10.00	E-F	-332/0	0.00 (1)	
B-H	0/564	-91.8	-91.8	0.39 (1)	10.00	G-H	-332/0	0.00 (1)	
H-C	0/591	-91.8	-91.8	0.25 (1)	10.00				
A-E	-528/0	-18.5	-18.5	0.25 (1)	6.25				
E-D	-480/0	-18.5	-18.5	0.25 (1)	6.25				
D-G	-480/0	-18.5	-18.5	0.25 (1)	6.25				
G-C	-528/0	-18.5	-18.5	0.25 (1)	6.25				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 085-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.39/1.00 (B-F), BC=0.25/1.00 (D-E), WB=0.23/1.00 (B-D), SSI=0.18/1.00 (A-E)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

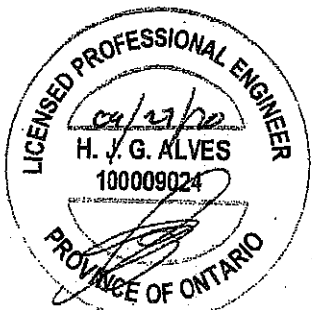
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

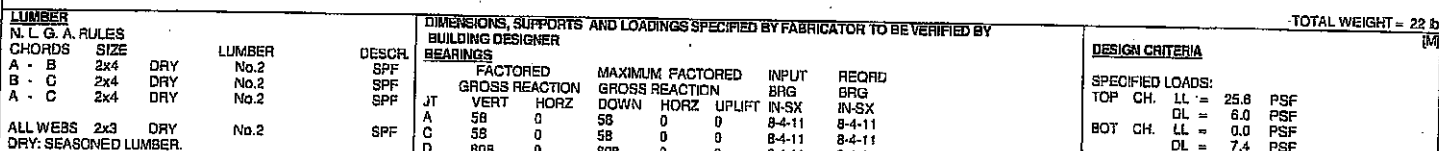
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (B) (INPUT = 0.90)
JSI METAL = 0.30 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007035

Version 8.3.10 S Oct 29 2019 MiTek Industries, Inc. Sat Apr 25 10:24:33 2020 Page 1
ID:DMCubINVR8TstF0e31v6l zns1l-g74JddgDnYDH0z, HzCbvb87zCjQWALUmtvouswzNDLS
3-0 1-2 12 1-2 12 85-7 1-2 12 4x4 = Scale = 1:12.5



UNFACTORED REACTIONS							
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	41	27.0 / 0	0 / 0	0 / 0	0 / 0	14.0 / 0	0 / 0
C	41	27.0 / 0	0 / 0	0 / 0	0 / 0	14.0 / 0	0 / 0
D	572	375 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX FLG	MAX. CS1 (L)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CS1 (L)
FR-TO		FROM	TO	LENGTH	FR-TO			
A-F	0/297	-91.8	-91.8	0.10 (1)	10.00	D-B	-623 / 0	0.10 (1)
F-B	0/234	-91.8	-91.8	0.20 (1)	10.00	E-F	-202 / 0	0.00 (1)
B-H	0/234	-91.8	-91.8	0.20 (1)	10.00	G-H	-202 / 0	0.00 (1)
H-C	0/297	-91.8	-91.8	0.10 (1)	10.00			
A-E	-278 / 0	-18.5	-18.5	0.15 (1)	5.25			
E-D	-250 / 0	-18.5	-18.5	0.15 (1)	6.25			
D-G	-250 / 0	-18.5	-18.5	0.15 (1)	5.25			
G-C	-278 / 0	-18.5	-18.5	0.15 (1)	5.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.8	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCG 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.20/1.00 (B-F:1), BC=0.15/1.00 (D-E:1),
WB=0.10/1.00 (B-D:1), SS=0.11/1.00 (A-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

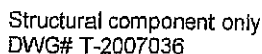
NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1657	786	1957	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

```
SI GRIP= 0.52 (B) (INPUT = 0.90 )
SI METAL= 0.18 (B) (INPUT = 1.00 )
```



Item CS

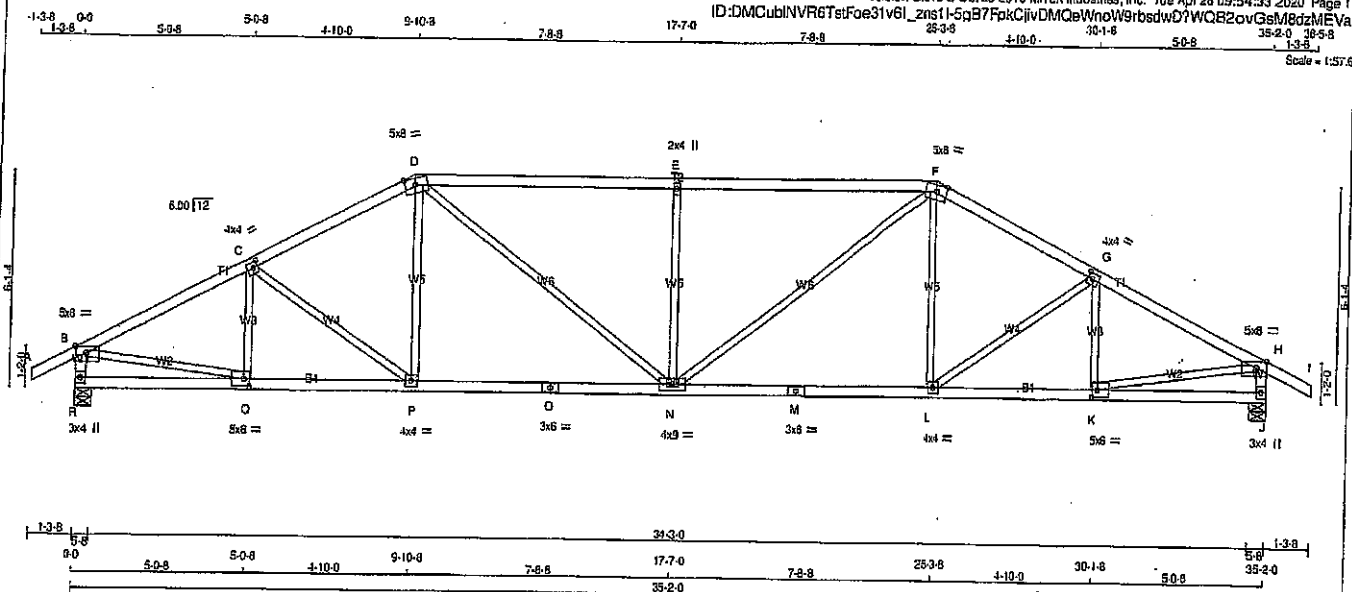
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408268	T32	2	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

Version 8.310 9 Oct 2019 MTek Industries, Inc. Tue Apr 28 09:54:33 2020 Page 1

ID:DMCubINVR6TstFoe31v6I_2nst11-5q97FpkCjvDMCdeWnoW9rbsdwd7WQ82ovGsm8dzMEVa

Scale = 1:57.6



N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
----------	-----	-----	------

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWW-p	MT20	5.0	8.0	Edge	3.50
C TMWW-l	MT20	4.0	4.0	2.00	1.75
D TTWW-m	MT20	5.0	8.0	2.25	3.75
E TMWW-w	MT20	2.0	4.0		
F TTWW-m	MT20	5.0	8.0	2.25	3.75
G TMWW-l	MT20	4.0	4.0	2.00	1.75
H TMWW-p	MT20	5.0	8.0	Edge	3.50
J BMV1+p	MT20	3.0	4.0		
K BMWW-l	MT20	5.0	6.0	2.50	2.00
L BMWW-l	MT20	4.0	4.0		
M BS-l	MT20	3.0	6.0		
N BMWW-l	MT20	4.0	4.0		
O BS-l	MT20	3.0	6.0		
P BMWW-l	MT20	4.0	4.0		
Q BMWW-l	MT20	5.0	6.0	2.50	2.00
R BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
R	2063	0	0	5-8
J	2063	0	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1457	969 / 0	0 / 0	0 / 0	0 / 0	498	0
J	1457	969 / 0	0 / 0	0 / 0	0 / 0	498	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	MAX
MEMB.	FORCE	(PLF)	CSH (LC)	UNBRAC	LENGTH	PR-TO	MEMB.	FORCE	MAX
FR-TO	(LBS)	FROM TO						(LBS)	CSH (LC)
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	Q-C	-353	0	0.08 (1)
B-C	-2834 / 0	-91.8	-91.8	0.39 (1)	3.81	C-P	-210	0	0.13 (1)
C-D	-2557 / 0	-91.8	-91.8	0.37 (1)	3.31	P-D	0 / 246	0.08 (1)	
D-E	-3060 / 0	-91.8	-91.8	0.57 (1)	2.78	D-N	0 / 844	0.19 (1)	
E-F	-3060 / 0	-91.8	-91.8	0.57 (1)	2.78	N-E	-572	0	0.51 (1)
F-G	-2857 / 0	-91.8	-91.8	0.37 (1)	3.31	N-F	0 / 844	0.19 (1)	
G-H	-2834 / 0	-91.8	-91.8	0.39 (1)	3.81	L-F	0 / 246	0.08 (1)	
H-I	0 / 28	-91.8	-91.8	0.12 (1)	10.00	L-G	-210	0	0.13 (1)
I-B	-2018 / 0	0.0	0.0	0.20 (1)	5.94	K-G	-353	0	0.08 (1)
J-H	-2018 / 0	0.0	0.0	0.20 (1)	5.94	B-Q	0 / 2591	0.58 (1)	
R-Q	0 / 0	-18.5	-18.5	0.10 (4)	10.00	K-I	0 / 2591	0.58 (1)	
Q-P	0 / 2555	-18.5	-18.5	0.51 (1)	10.00				
P-O	0 / 2385	-18.5	-18.5	0.49 (1)	10.00				
O-N	0 / 2385	-18.5	-18.5	0.49 (1)	10.00				
N-M	0 / 2385	-18.5	-18.5	0.49 (1)	10.00				
M-L	0 / 2385	-18.5	-18.5	0.49 (1)	10.00				
L-K	0 / 2555	-18.5	-18.5	0.51 (1)	10.00				
K-J	0 / 0	-18.5	-18.5	0.10 (4)	10.00				

TOTAL WEIGHT = 2 X 139 = 277 lb

(MRF)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 8.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012, ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA C88-09, CSA C88-14

- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.17')

CALCULATED VERT. DEFL. (LL) = L/999 (0.17')

ALLOWABLE DEFL. (TL) = L/360 (1.17')

CALCULATED VERT. DEFL. (TL) = L/999 (0.34')

CSI: TC=0.97/1.00 (D-E:1), BC=0.51/1.00 (K-L:1),

WB=0.58/1.00 (H-K:1), SB=0.34/1.00 (D-E:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION

(FSI) (PL) (PL)

MAX MIN MAX MIN MAX MIN

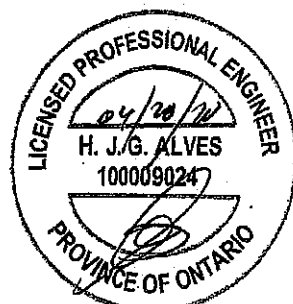
MT20 618 354 1687 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

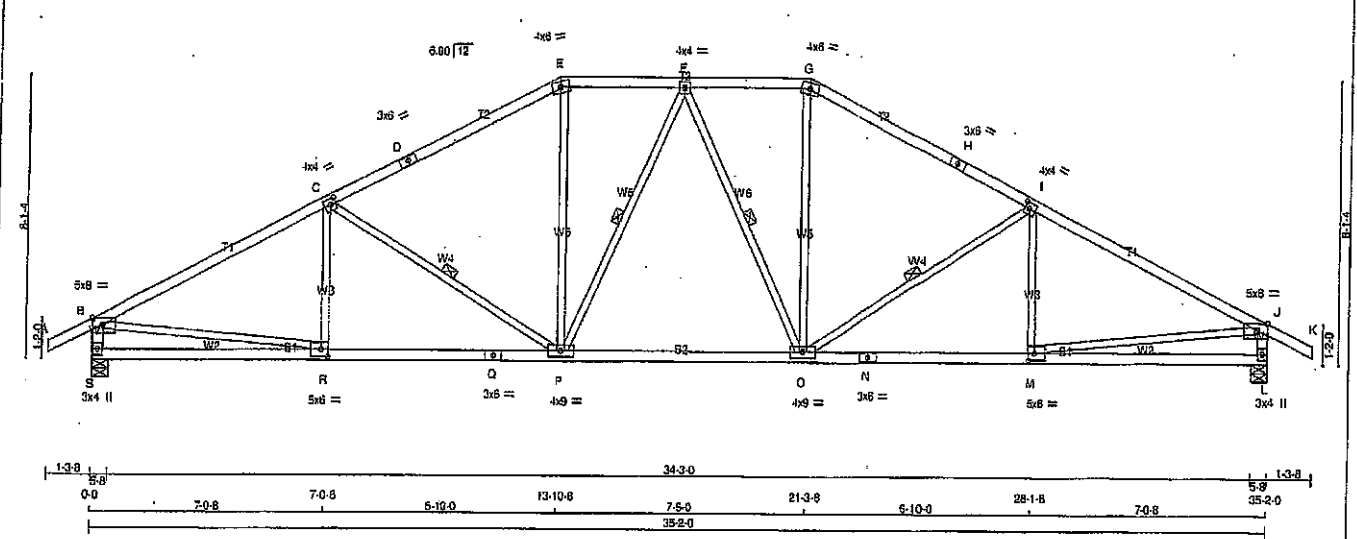
JSI GRIP=0.87 (D) (INPUT = 0.80)

JSI METAL=0.82 (M) (INPUT = 1.00)

Structural component only
DWG# T-2007716

JOB NAME 408268	TRUSS NAME T34	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 23 09:54:35 2020 Page 1
 ID:DMCubINVR6TsFoe31v6I_zns11-12luVmtFJ9xbkouuDYdw0x711gu5K6MaLTOWzMEVY
 33-2-0 30-8
 1-3-8 1-3-8
 Scale = 1:58.3



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - K	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
O - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge 3.50
C	TMVW-i	MT20	4.0	4.0	2.00 1.75
D	TS-i	MT20	3.0	6.0	
E	TTW-m	MT20	4.0	6.0	
F	TMVW-i	MT20	4.0	4.0	
G	TTW-m	MT20	4.0	6.0	
H	TS-i	MT20	3.0	6.0	
I	TMVW-i	MT20	4.0	4.0	2.00 1.75
J	TMVW-p	MT20	5.0	8.0	Edge 3.50
L	BMV-i	MT20	3.0	4.0	
M	BMVW-i	MT20	5.0	8.0	2.50 2.25
N	BS-i	MT20	3.0	6.0	
O	BMVWV-i	MT20	4.0	9.0	
P	BMVWVW-i	MT20	4.0	9.0	
Q	BS-i	MT20	3.0	6.0	
R	BMVW-i	MT20	5.0	6.0	2.50 2.25
S	BMV-i	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
S	2063	0	0	5-8
L	2063	0	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
S	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 - 0	0 / 0
L	1457	969 / 0	0 / 0	0 / 0	0 / 0	488 - 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-P, F-O, I-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO					FR-TO		
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	R-C	-182.52	0.08 (1)
B-C	-2908 / 0	-91.8	-91.8 0.83 (1)	3.25	C-P	-637 / 0	0.29 (1)
C-D	-2373 / 0	-91.8	-91.8 0.72 (1)	3.68	P-E	0 / 574	0.15 (1)
D-E	-2373 / 0	-91.8	-91.8 0.72 (1)	3.68	F-O	-222.0	0.11 (1)
E-F	-2100 / 0	-91.8	-91.8 0.19 (1)	4.53	O-G	0 / 574	0.15 (1)
F-G	-2100 / 0	-91.8	-91.8 0.72 (1)	4.53	G-H	-182.52	0.28 (1)
G-H	-2373 / 0	-91.8	-91.8 0.72 (1)	3.68	H-I	-637.0	0.08 (1)
H-I	-2373 / 0	-91.8	-91.8 0.72 (1)	3.68	I-J	0 / 2649	0.69 (1)
I-J	-2908 / 0	-91.8	-91.8 0.83 (1)	3.25	J-K	0 / 2649	0.69 (1)
J-K	0 / 28	-91.8	-91.8 0.12 (1)	10.00	K-L	0 / 2649	0.69 (1)
K-L	-2908 / 0	0.0	0.0 0.20 (1)	5.95			
L-M	-2908 / 0	0.0	0.0 0.20 (1)	5.95			
M-N	0 / 0	-18.5	-18.5 0.20 (4)	10.00			
N-O	0 / 2630	-18.5	-18.5 0.52 (1)	10.00			
O-P	0 / 2630	-18.5	-18.5 0.52 (1)	10.00			
P-Q	0 / 2193	-18.5	-18.5 0.45 (1)	10.00			
Q-R	0 / 2630	-18.5	-18.5 0.52 (1)	10.00			
R-S	0 / 2630	-18.5	-18.5 0.52 (1)	10.00			
S-T	0 / 0	-18.5	-18.5 0.20 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TFC 2011, TFC 2014

85% OF 91.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/599 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/599 (0.30")

CS1: TC=0.83/1.00 (J-K:1), BC=0.52/1.00 (M-O:1), WB=0.60/1.00 (J-M:1), SS=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

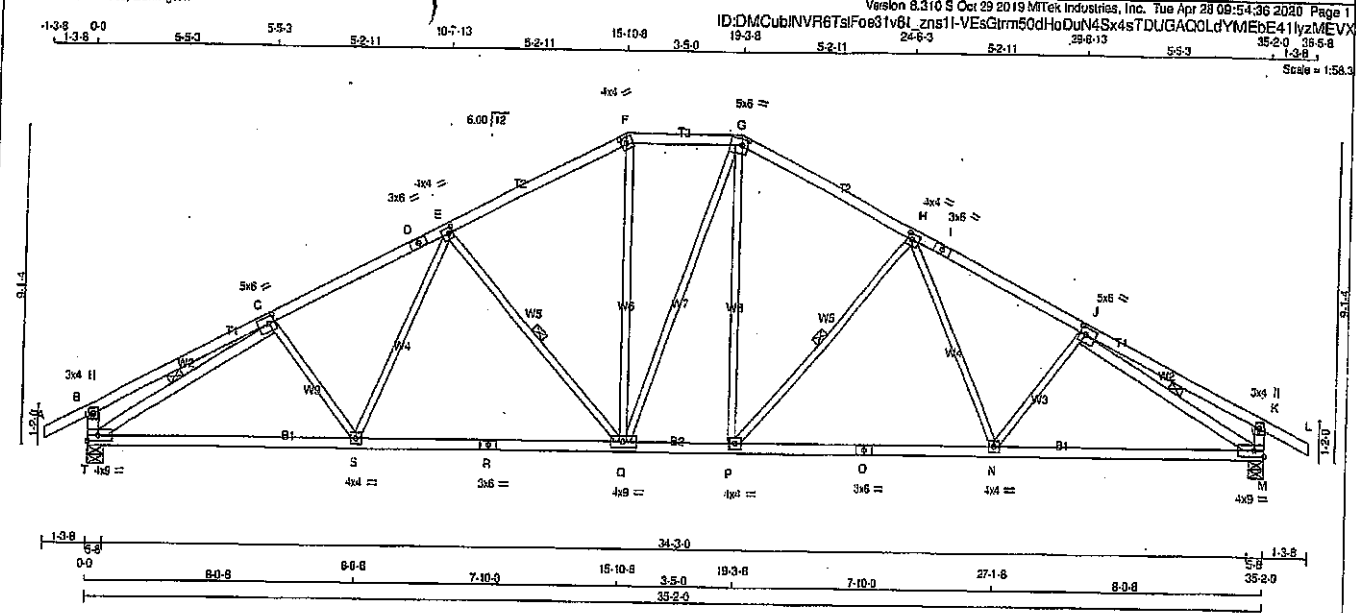
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (M) (INPUT = 0.90)
JSI METAL = 0.79 (C) (INPUT = 1.00)



Structural component only
DWG# T-2007718

JOB NAME 408268	TRUSS NAME T35	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
L - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
K - R	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
EXCEPT			
T - C	2x4	DRY	No.2
J - M	2x4	DRY	No.2

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV+p	MT20	3.0	4.0	
C TMWW-1	MT20	5.0	6.0	2.50 2.25
D TS-1	MT20	3.0	6.0	
E TMWW-1	MT20	4.0	4.0	2.00 1.50
F TTW-a	MT20	4.0	4.0	2.00 1.75
G TTWW-m	MT20	5.0	6.0	2.00 2.00
H TMWW-1	MT20	4.0	4.0	2.00 1.50
I TS-1	MT20	3.0	6.0	
J TMWW-1	MT20	5.0	6.0	2.50 2.25
K TMV+p	MT20	3.0	4.0	
M BMVW-1	MT20	4.0	9.0	Edge
N P-S				
N BMWW-1	MT20	4.0	4.0	
O BS-1	MT20	3.0	6.0	
Q BMWW-1	MT20	4.0	9.0	
R BS-1	MT20	3.0	6.0	
T BMVW-1	MT20	4.0	9.0	Edge

Edges - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
	FACTORED	MAXIMUM FACTORED	INPUT	REORD					
	GROSS REACTION	GROSS REACTION	BRG	BRG					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
T	2063	0	2063	0	0	5-8	5-8		
M	2083	0	2083	0	0	5-8	5-8		

UNFACTORED REACTIONS									
JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS						
T	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
M	1457	989 / 0	0 / 0	0 / 0	0 / 0	488 / 3	0 / 0		
M	1457	989 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.86 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, H-P, G-T, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. UNBRACED LENGTH (FT)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO				
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	C-S	-110 / 37	0.04 (1)	0.04 (1)
B-C	0 / 20	-91.8	-91.8	0.32 (1)	10.00	S-E	0 / 277	0.08 (1)	0.08 (1)
C-D	-2760 / 0	-91.8	-91.8	0.40 (1)	3.86	E-Q	-881 / 0	0.31 (1)	0.31 (1)
D-E	-2760 / 0	-91.8	-91.8	0.40 (1)	3.86	Q-F	0 / 613	0.14 (1)	0.14 (1)
E-F	-2157 / 0	-91.8	-91.8	0.38 (1)	4.30	F-G	0 / 4	0.00 (1)	0.00 (1)
F-G	-1925 / 0	-91.8	-91.8	0.18 (1)	4.71	P-G	0 / 509	0.14 (1)	0.14 (1)
G-H	-2165 / 0	-91.8	-91.8	0.38 (1)	4.30	H-N	-683 / 0	0.31 (1)	0.31 (1)
H-I	-2761 / 0	-91.8	-91.8	0.40 (1)	3.86	I-N	0 / 279	0.05 (1)	0.05 (1)
I-J	-2761 / 0	-91.8	-91.8	0.40 (1)	3.86	N-J	-110 / 37	0.04 (1)	0.04 (1)
J-K	0 / 20	-91.8	-91.8	0.32 (1)	10.00	T-C	-3042 / 0	0.81 (1)	0.81 (1)
K-L	0 / 28	-91.8	-91.8	0.12 (1)	10.00	J-M	-3043 / 0	0.81 (1)	0.81 (1)
T-B	-325 / 0	0.0	0.0	0.03 (1)	7.81				
M-K	-325 / 0	0.0	0.0	0.03 (1)	7.81				
T-S	0 / 2538	-18.5	-18.5	0.58 (1)	10.00				
S-R	0 / 2382	-18.5	-18.5	0.53 (1)	10.00				
R-Q	0 / 2382	-18.5	-18.5	0.53 (1)	10.00				
Q-P	0 / 1924	-18.5	-18.5	0.40 (1)	10.00				
P-O	0 / 2382	-18.5	-18.5	0.53 (1)	10.00				
O-N	0 / 2382	-18.5	-18.5	0.53 (1)	10.00				
N-M	0 / 2538	-18.5	-18.5	0.58 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NSCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17')

CALCULATED VERT. DEFL.(LL) = L/999 (0.14')

ALLOWABLE DEFL.(TL) = L/360 (1.17')

CALCULATED VERT. DEFL.(TL) = L/999 (0.29')

CSI: TC=0.40/1.00 (H-J:1), BC=0.56/1.00 (M-N:1), WB=0.61/1.00 (J-M:1), SS=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(FSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	818 354 1697 785 1987 1656	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)

JSI METAL= 0.83 (C) (INPUT = 1.00)



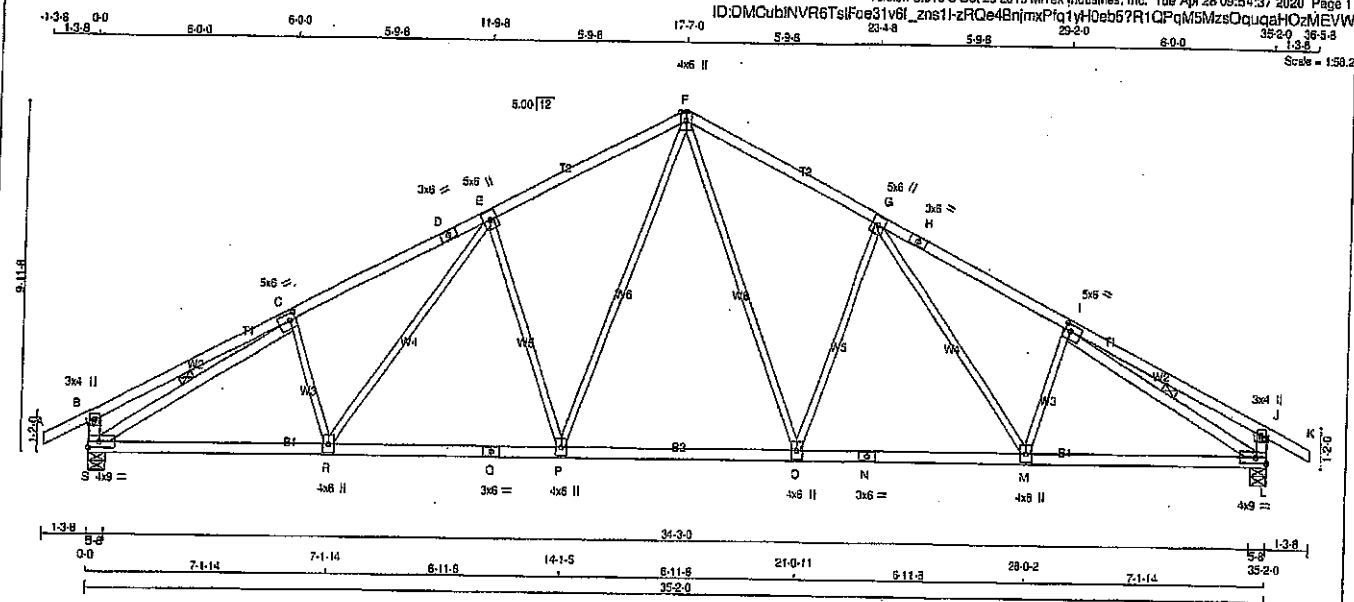
Structural component only
DWG# T-2007719

JOB NAME 408268	TRUSS NAME T36	QUANTITY 5	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRAWG NO.
--------------------	-------------------	---------------	----------	-------------------------------	-------------	-----------

Tamarrack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Tue Apr 28 09:54:37 2020 Page 1

ID:DMCubINVR6TsIFoe31v6I_zns1I-zRQe4BnImxPq1yH0eb57R1QPqM5MzsOquqahOzMEVW



TOTAL WEIGHT = 5 X 162 = 759 lb

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
S - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
S - Q	2x4	DRY	No.2	SPF	
Q - N	2x4	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
S - C	2x4	DRY	No.2	SPF	
I - L	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+1	MT20	5.0	6.0	2.25	2.00
D	TS-1	MT20	3.0	6.0		
E	TMVW+1	MT20	5.0	6.0		
F	TTVW+p	MT20	4.0	6.0	Edge	
G	TMVW+1	MT20	5.0	6.0		
H	TS-1	MT20	3.0	6.0		
I	TMVW+1	MT20	5.0	6.0	2.25	2.00
J	TMV+p	MT20	3.0	4.0		
L	BMVW+1	MT20	4.0	9.0	Edge	
M	BMVW+1	MT20	4.0	9.0		
N	BS-1	MT20	3.0	6.0		
Q	BS-1	MT20	3.0	6.0		
S	BMVW+1	MT20	4.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	DOWN	DOWN	BRG	BRG
S	2063	0	2063	0	5-8
L	2063	0	2063	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1457	989 / 0	0 / 0	0 / 0	0 / 0	0 / 0	483 / 0	0 / 0
L	1457	989 / 0	0 / 0	0 / 0	0 / 0	0 / 0	483 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-S, H, L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0:28	-91.8	-91.8 0.12 (1)	10.00	F-O	0:886	0.19 (1)
B-C	0:22	-91.8	-91.8 0.40 (1)	10.00	O-G	-726:0	0.72 (1)
C-D	-2808:0	-91.8	-91.8 0.49 (1)	3.75	G-M	0:391	0.09 (1)
D-E	-2808:0	-91.8	-91.8 0.49 (1)	3.75	M-I	-192:18	0.05 (1)
E-F	-2322:0	-91.8	-91.8 0.46 (1)	4.07	P-F	0:886	0.19 (1)
F-G	-2322:0	-91.8	-91.8 0.49 (1)	3.75	P-E	-726:0	0.72 (1)
G-H	-2808:0	-91.8	-91.8 0.49 (1)	3.75	R-E	0:391	0.09 (1)
H-I	-2808:0	-91.8	-91.8 0.49 (1)	3.75	C-F	-192:18	0.05 (1)
I-J	0:22	-91.8	-91.8 0.40 (1)	10.00	S-C	-3064:0	0.72 (1)
J-K	0:28	-91.8	-91.8 0.12 (1)	10.00	I-L	-3064:0	0.72 (1)
K-S	-344:0	0.0	0.0 0.03 (1)	7.81			
L-J	-344:0	0.0	0.0 0.03 (1)	7.81			
S-R	0:2574	-18.5	-18.5 0.53 (1)	10.00			
R-Q	0:2291	-18.5	-18.5 0.47 (1)	10.00			
Q-P	0:2291	-18.5	-18.5 0.47 (1)	10.00			
P-O	0:1782	-18.5	-18.5 0.38 (1)	10.00			
O-N	0:2291	-18.5	-18.5 0.47 (1)	10.00			
N-M	0:2291	-18.5	-18.5 0.47 (1)	10.00			
M-L	0:2574	-18.5	-18.5 0.53 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2018, CBC 2012, ABC 2015
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.17)
CALCULATED VERT. DEFL.(LL) = L/999 (0.14)
ALLOWABLE DEFL.(TL) = L/360 (1.17)
CALCULATED VERT. DEFL.(TL) = L/999 (0.28)

CS: TC=0.48/1.00 (C-E:1), BC=0.53/1.00 (L-M:1), WB=0.72/1.00 (E-F:1), SS=0.22/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (C) (INPUT = 0.80)

JSI METAL = 0.77 (I) (INPUT = 1.00)



Structural component only
DWG# T-2007720

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Sat Apr 25 10:35:49 2020 Page 1
ID:DMCubINVR6TsIFoe31v6l zns1-lHwrlVz3FaFOMwS(CaA)Q_h7QdSjZzWGGJ4ZgskCNDAu
1.3.8 0.0 5.10.8 5.10.8 5.2.10 11.3.2 5.10.14 17.7.0 5.10.14 5.2.14 39.3.8 5.2.10 5.5.0 5.8.1.3.8
Scale = 1:57.3



BODY: SEASONED LUMBER. READING MATERIAL TO BE SPREADSHEET AT MINIMUM	11/10/12	11/10/12	11/10/12	11/10/12	11/10/12
---	----------	----------	----------	----------	----------

DESIGN CONSISTS OF 2 TRUSSES BUILT
SEPARATELY WITH JOINT BRACING

THIS DESIGN COMPLIES WITH:

TOP CHORDS : (0.122"x3") SPIRAL NAIL S

	SIDE(01.0)	(56% OF 31.3 PSE Q-S) PLUS B-1.0 P-E PAW
--	------------	--

-B		2	12	TOP	MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE	MAX	LIVE LOAD
----	--	---	----	-----	-------	-------	------------	-----	-----	-----	-------	-------	-----	-----------

	SIDE	FR-TO	FROM	TO	LENGTH	FR-TO
-P	2	A-B	31.8	31.9	0.07 (2)	0.000 (0.00")
ADJUSTED VERT. DEFL. (LL) =						1/999 (0.21")

K	2	12		SIDE(183.1)	C-T	-7207 / 0	-91.8	-91.8	0.26 (1)	4.28	B-R	0 / 4581	0.57 (1)	CALCULATED VERT. DEF.(TL) = L/999 (0.39%)
---	---	----	--	-------------	-----	-----------	-------	-------	----------	------	-----	----------	----------	---

D	-0.20	-0.10	0.25 (1)	4.28	M-H	0 / 3123	0.39 (1)
D-V	-8028 / 0	-91.8	-91.8	0.26 (1)	C-D	0 / 3143	0.39 (1)

WB=0.57/1.00 (H-L:1), SSI=0.18/1.00 (G-H:1)

ORDER NAILING ASSUMES NAILED HANGERS ARE	W-FL-8028/0	-91.8	-91.8	0.28 (1)	4.09	Q-D -1496/0	0.18 (1)	COMP=1.00 SHEAR=1.00 TENS=1.00
	W-FL-8028/0	-91.8	-91.8	0.28 (1)	4.09			

DE - COMPONENTS ARE LOADED FROM THE TOP AND	DE - Y	-8028 / 0	-91.8	-91.8	0.27 (1)	4.08	O - E	-821 / 0	0.12 (1)	COMPARISON LIVE LOAD FACTOR = 1.00
	DE - X	0.00 / 0							0.10 (1)	

LOAD TO BE TRANSFERRED TO EACH PLY.	Z-AA	-7234 / 0	-91.8	-91.8	0.26 (1)	4.27

AD-H	-723470	-91.8	-91.8	0.26 (1)	4.27
H-I	-511470	-91.8	-91.8	0.53 (4)	3.65

TRUSS MANUFACTURING PLANT.

S-B	-3248 / 0	0.0	0.0	0.11 (1)	7.69
V-L	9279 / 0				

[illegible]

AD-M	0.00	-18.5	-18.5	0.06 (4)	10.00
RAE	0.14552	-18.5	-18.5	0.34 (1)	10.00

PLATE PLACEMENT TOL. = 0.250 inches

AF-PQ	0 4552	-18.5	-18.5	0.34 (1)	10.00	
0 7000						

AG-O	0.7208	-18.5	-18.5	0.53 (1)	10.00
					10.00

JSI METAL = 0.68 (P) (INPUT = 1.00)

AP-AI	0.7234	-18.5	-18.5	0.53 (1)	10.00
AI-N	0.7234	-18.5	-18.5	0.53 (1)	10.00

AJ-M	0.7234	-18.5	-18.5	0.53 (1)	10.00
M-100	0.7234	-18.5	-18.5	0.53 (1)	10.00

AL-AM	0	4596	-18.5	-18.5	0.35 (1)	10.00
			-18.5	-18.5	0.35 (1)	10.00

Structural component only	AN-AN	0 0	-18.5	-18.5	0.06 (4)	10.00
	AN-AC	0 0	-18.5	-18.5	0.06 (4)	10.00

DWG# -2007041

JOB NAME 408133	TRUSS NAME T30	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 MITek Industries, Inc. Sat Apr 25 10:35:49 2020 Page 2		
ID:DMCubiNVR6TstFoe31v6f zns1t-BWtVzsFaFOMwSfCaAiQ h7QoDSizZWGJ4ZgakzNDAu						

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	9.0	1.00	4.75
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMWW-w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	
I	TMWW-p	MT20	4.0	9.0	1.00	4.75
K	BMV1-p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.00
M	BMWW-t	MT20	5.0	6.0	2.50	3.75
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	3.75
R	BMWW-t	MT20	5.0	6.0	2.50	2.00
S	BMV1-p	MT20	3.0	6.0		

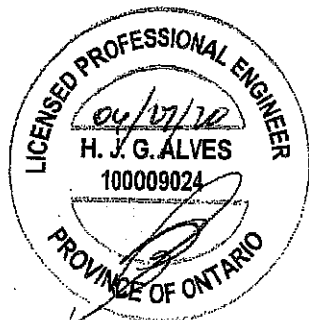
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-441	-441	---	BACK	VERT	TOTAL	---	C1
D	11-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
F	19-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
H	29-3-8	-441	-441	---	BACK	VERT	TOTAL	---	C1
L	29-2-13	-26	-26	---	BACK	VERT	TOTAL	---	C1
P	13-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
Q	11-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
R	5-11-3	-26	-26	---	BACK	VERT	TOTAL	---	C1
T	7-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
U	9-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
V	13-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
W	15-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
X	17-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
Y	21-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
Z	23-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
AA	25-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
AB	27-11-4	-110	-110	---	BACK	VERT	TOTAL	---	C1
AC	2-0-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
AD	4-0-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
AE	7-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AF	9-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AG	15-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AH	17-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AI	19-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AJ	21-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AK	23-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AL	25-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AM	27-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AN	31-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AO	33-11-4	-26	-26	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2007041

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408133	T30Z	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 MiTek Industries, Inc. Sat Apr 25 10:35:50 2020 Page 2
ID:DMCubINVR6TstFoe31v6I zns1H-LNUFJtuLYWDXbEP7uEPXvgatcGINbQYkiD6BzNDAI

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	9.0	1.00	4.75
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMWW-l	MT20	5.0	6.0		
E	TMW-w	MT20	3.0	6.0		
F	TS-l	MT20	5.0	6.0		
G	TMWW-l	MT20	5.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	
I	TMVW-p	MT20	4.0	9.0	1.00	4.75
K	BMV1-p	MT20	3.0	6.0		
L	BMWW-l	MT20	5.0	6.0	2.50	2.00
M	BMWW-l	MT20	5.0	6.0	2.50	3.75
N	BS-l	MT20	5.0	6.0		
O	BMWW-l	MT20	5.0	6.0		
P	BS-l	MT20	5.0	6.0		
Q	BMWW-l	MT20	5.0	6.0	2.50	3.75
R	BMWW-l	MT20	5.0	6.0	2.50	2.00
S	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

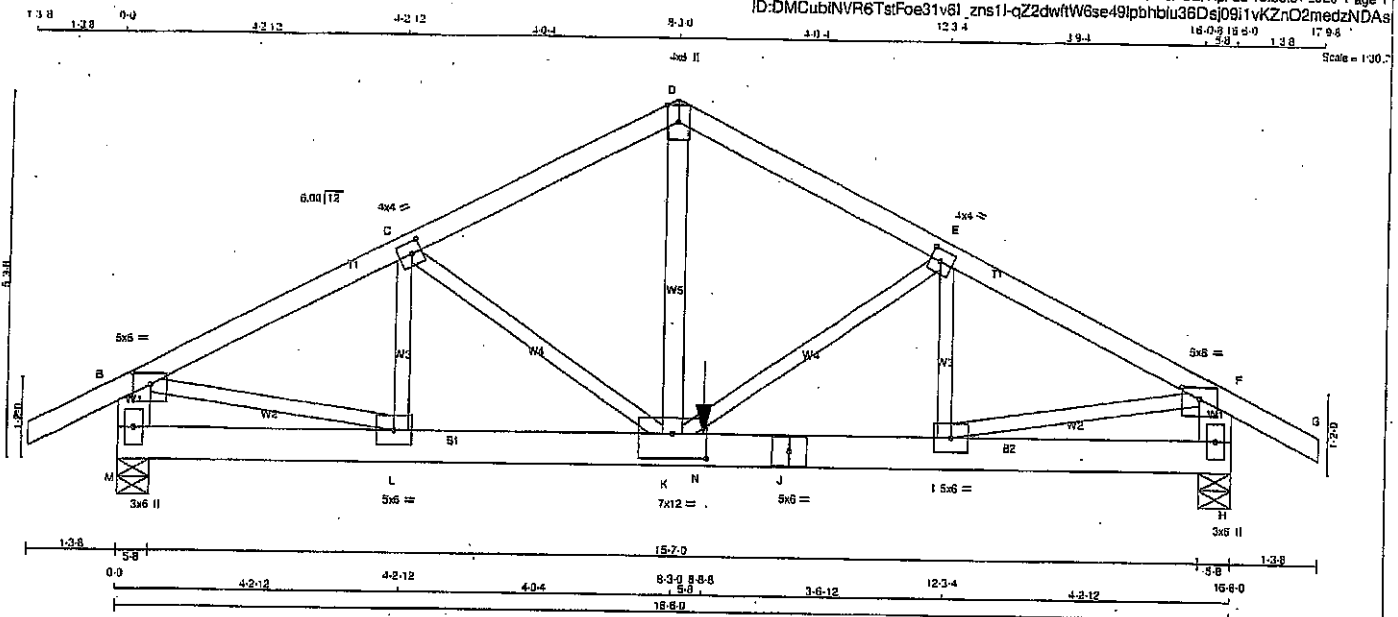


Structural component only
DWG# T-2007042 32

JOB NAME 408133	TRUSS NAME T37	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:35:57 2020 Page 1	

ID:DMCubINVR6TstFoe31v8l_zns1l-qZ2dwfW8se49lphbhu36Dsj09l1vKZnO2medzNDAs

Scale = 1/30



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
M - B	2x6	DRY	No.2
H - F	2x6	DRY	No.2
M - J	2x6	DRY	No.2
J - H	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
K - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1	12	TOP
D-G 1	12	TOP
M-B 2	12	TOP
H-F 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
M-J 2	12	SIDE (0.0)
J-H 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT
M	2447	0	2447	0	5-8
H	2613	0	2613	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	1733	1123/0	0/0	0/0	0/0	611/0	0/0
H	1851	1197/0	0/0	0/0	0/0	654/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0/28	K-D	0/2959
B-C	-3375/0	K-E	-150/0
C-D	-3654/0	L-E	-301/0
D-E	-3653/0	C-K	0/271
E-F	-3759/0	L-C	-721/0
F-G	0/28	B-L	0/3084
M-B	-2354/0	L-F	0/3431
H-F	-2590/0		
M-L	0/0		
L-K	0/3033		
K-N	0/3375		
N-J	0/3375		
J-I	0/3375		
I-H	0/0		

FACTORED CONCENTRATED LOADS (LBS)		LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	8-8-8	-2992	-2992	---	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 79 = 158 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL	LD = 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.55")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.05")
ALLOWABLE DEFL. (TL) = $L/360$ (0.55")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.10")

CSI: $TC=0.17/1.00$ (E-F:1), $BC=0.50/1.00$ (H-K:1), $WB=0.42/1.00$ (F-L:1), $SSI=0.93/1.00$ (H-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MAX	MIN	MAX
MT20	618	354
	1667	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (B) (INPUT = 0.90)
JSI METAL = 0.52 (J) (INPUT = 1.00)



Structural component only
DWG# T-2007043 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408133	T37	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 5 Oct 29 2019 Mittek Industries, Inc. Sat Apr 25 10:35:51 2020 Page 2
ID:DMCubINVR6TstFog31v6l zns11-qZ2dwtW6se49lphbhtu36Dsi0911vKZnO2medzNDAs

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMVW-p	MT20	5.0	6.0	2.00	3.00
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW-t	MT20	7.0	12.0	4.25	6.00
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

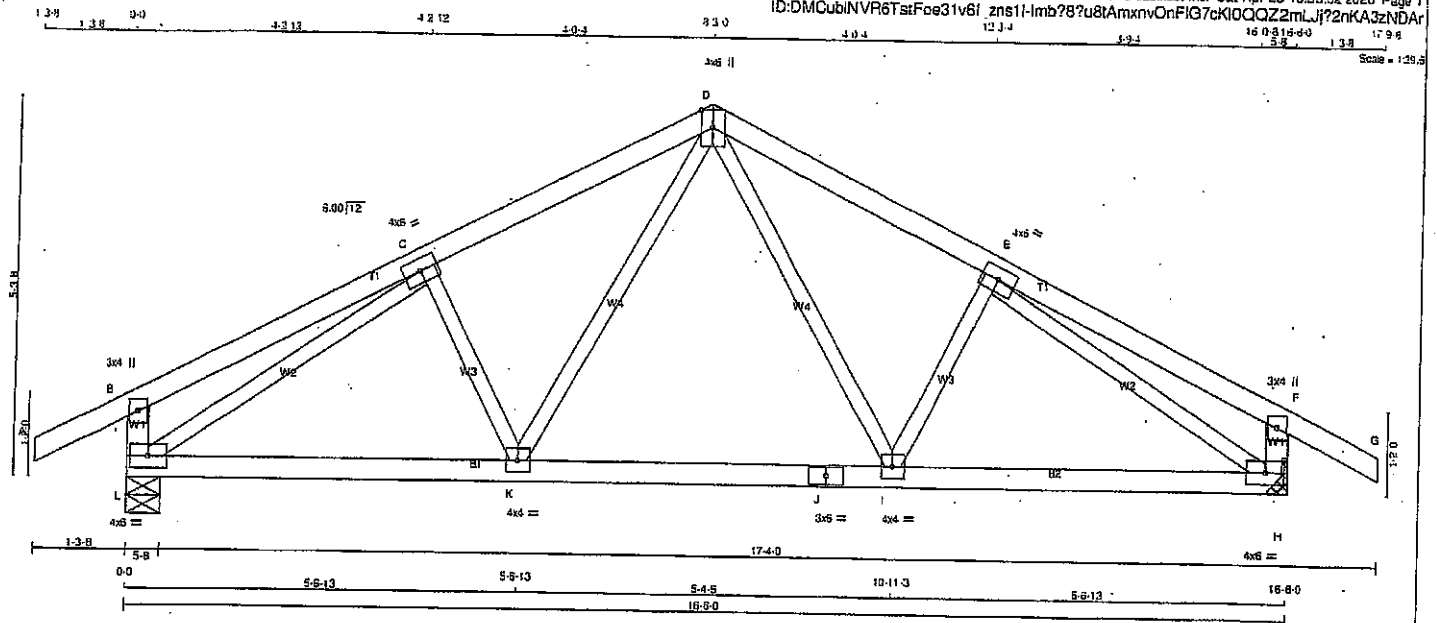
Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



Structural component only
DWG# T-2007043 2/2

JOB NAME 408133	TRUSS NAME T38	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:35:52 2020 Page 1
ID:DMCubINVR6TstFoe31v6f_zns1l-lmb78u8tAmxrvOnF1G7cK10CQZ2mLj?2nKA3zNDAr



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
L - B	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+P	MT20	3.0	4.0		
C	TMVW+1	MT20	4.0	6.0		
D	TMVW+P	MT20	4.0	6.0		Edge
E	TMV+P	MT20	4.0	6.0		
F	TMV+P	MT20	3.0	4.0		
H	BMVW+1	MT20	4.0	6.0		
I	BMVW+1	MT20	4.0	4.0		
J	BS+1	MT20	3.0	5.0		
K	BMVW+1	MT20	4.0	4.0		
L	BMVW+1	MT20	4.0	5.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
L	1034	0	1034	0	5-8
H	1034	0	1034	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
L	729	492 / 0	0 / 0	0 / 0	237 / 0
H	729	492 / 0	0 / 0	0 / 0	237 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (PLF)	MEMB.
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 29	-91.8	-91.8 0.12 (1)	10.00	D-I	0 / 354	0.08 (1)
B-C	0 / 19	-91.8	-91.8 0.24 (1)	10.00	I-E	-226 / 0	0.05 (1)
C-D	-1044 / 0	-91.8	-91.8 0.20 (1)	5.93	K-D	0 / 354	0.08 (1)
D-E	-1044 / 0	-91.8	-91.8 0.20 (1)	5.93	C-K	-226 / 0	0.05 (1)
E-F	0 / 19	-91.8	-91.8 0.24 (1)	10.00	L-C	-1245 / 0	0.51 (1)
F-G	0 / 29	-91.8	-91.8 0.12 (1)	10.00	E-H	-1245 / 0	0.51 (1)
L-B	-271 / 0	0.0	0.0 0.03 (1)	7.81			
H-F	-271 / 0	0.0	0.0 0.03 (1)	7.81			
L-K	0 / 1015	-18.5	-18.5 0.23 (1)	10.00			
K-J	0 / 749	-18.5	-18.5 0.19 (4)	10.00			
J-I	0 / 749	-18.5	-18.5 0.19 (4)	10.00			
I-H	0 / 1015	-18.5	-18.5 0.23 (1)	10.00			

TOTAL WEIGHT = 2 X 66 = 133 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ORC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09; CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) =	L/360 (0.55")
CALCULATED VERT. DEFL. (LL) =	L/999 (0.03")
ALLOWABLE DEFL. (TL) =	L/360 (0.55")
CALCULATED VERT. DEFL. (TL) =	L/999 (0.06")

CSI: TC=0.24/1.00 (B-C:1), BC=0.23/1.00 (H:1), WB=0.51/1.00 (E-H:1), SS=0.16/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1867 788	1987 1555

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.78 (E) (INPUT = 0.90)
JSI METAL = 0.31 (E) (INPUT = 1.00)



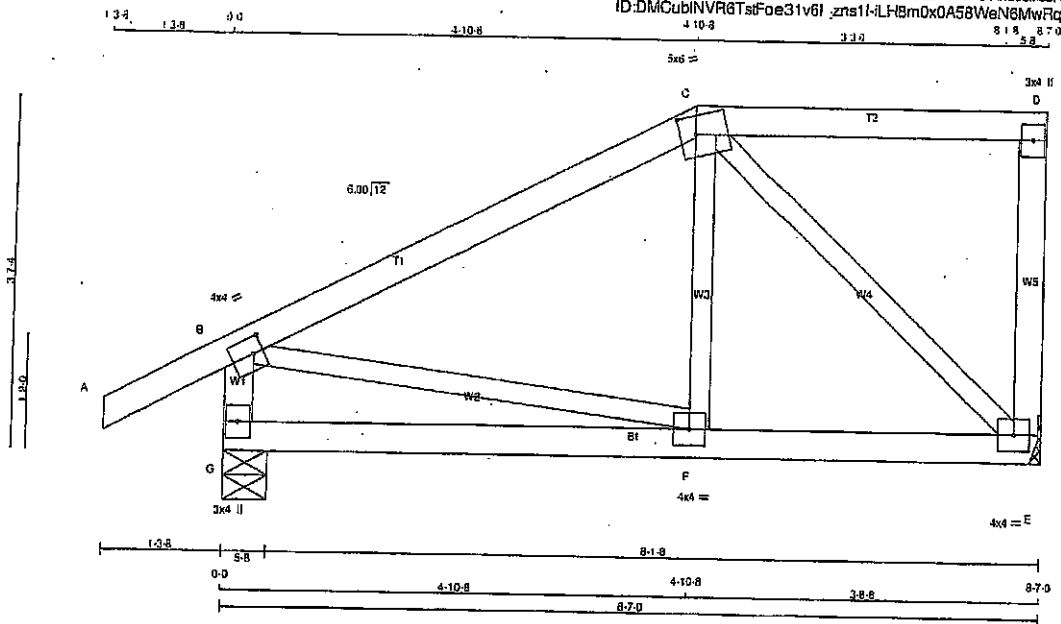
Structural component only
DWG# T-2007044

Structural component only
DWG# T-2007045

SI GRIP= 0.84 (B) (INPUT = 0.90)
SI METAL= 0.25 (H) (INPUT = 1.00)

JOB NAME 408133	TRUSS NAME T40	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:35:55 2020 Page 1
ID:DMCubINVR8TstFoe31v6f_zns1i-lLH8m0x0A58W6N6MwRqQEyNW3dbLznX9h00_nOzNDAd



Scale = 1/2\"/>

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-4	MT20	4.0	4.0	2.00	1.25
C	TTWV-m	MT20	5.0	5.0	2.25	2.00
D	TMVW-p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	BMVW-4	MT20	4.0	4.0		
G	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
E	473 0	473 0	5-8	5-8
G	588 0	588 0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	335	220 / 0	0 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
G	420	289 / 0	0 / 0	0 / 0	0 / 0	0 / 0	131 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)	MAX. FACTORED VERT. LOAD LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD LC1 (LC)
FR-TO		FROM			FR-TO		
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	F-C	0 / 92
B-C	-350 / 0	-91.8	-91.8	0.28 (1)	6.25	C-E	-421 / 0
C-D	0 / 0	-91.8	-91.8	0.21 (1)	10.00	B-F	0 / 318
E-D	-170 / 0	0.0	0.0	0.03 (1)	7.81		
G-B	-559 / 0	0.0	0.0	0.08 (1)	7.81		
G-F	0 / 0	-18.5	-18.5	0.10 (4)	10.00		
F-E	0 / 315	-18.5	-18.5	0.13 (4)	10.00		

TOTAL WEIGHT = 35 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2911, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSK TC=0.28/1.00 (B-C:1), BC=0.13/1.00 (E-F:4), WB=0.15/1.00 (C-E:1), SS=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

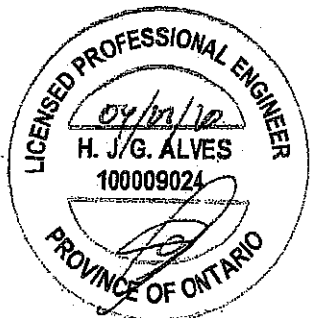
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.57 (B) (INPUT = 0.90)
JSI METAL = 0.19 (B) (INPUT = 1.00)

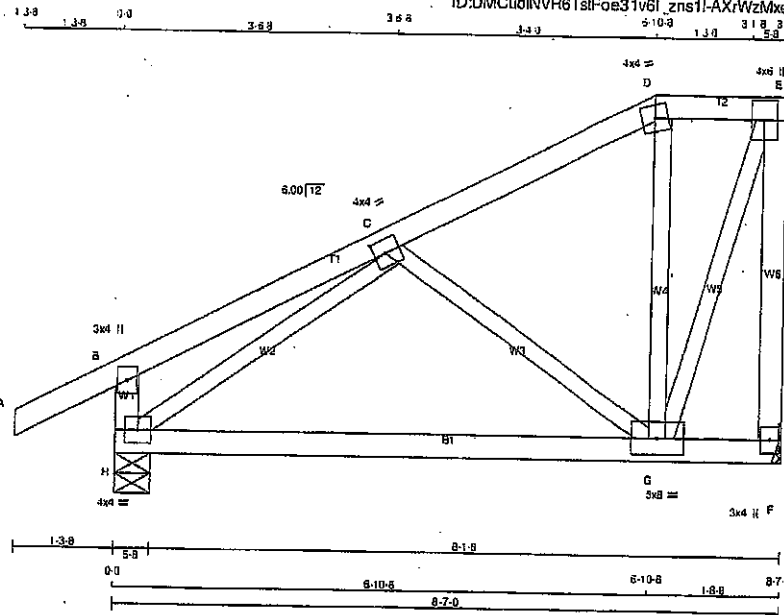


Structural component only
DWG# T-2007046

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
408133	T41	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:35:56 2020 Page 1
ID:DMCubINVR6TstFoe31v6l_zns11-AXtWzMxexOGNGWWhYU8L3mAwjT1wylEqIwgIXJzNDAn



Scale = 1/28.7

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - E	2x4	DRY	No.2	SPF		
F - E	2x4	DRY	No.2	SPF		
H - B	2x4	DRY	No.2	SPF		
H - F	2x4	DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	5.0	8.0		
H	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
F	473	473	0	0
H	598	598	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	335	220 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
H	420	289 / 0	0 / 0	0 / 0	0 / 0	131 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	WESS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO	A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	C-G	-292 / 0	0.09 (1)		
	B-C	0 / 16	-91.8	-91.8	0.17 (1)	10.00	G-D	-117 / 0	0.04 (1)		
	C-D	-199 / 0	-91.8	-91.8	0.13 (1)	6.25	G-E	0 / 460	0.10 (1)		
	D-E	-160 / 0	-91.8	-91.8	0.03 (1)	6.25	H-C	-498 / 0	0.15 (1)		
	F-E	-509 / 0	0.0	0.0	0.17 (1)	7.81					
	H-B	-246 / 0	0.0	0.0	0.02 (1)	7.81					
	H-G	0 / 399	-18.5	-18.5	0.23 (4)	10.00					
	G-F	0 / 0	-18.5	-18.5	0.21 (4)	10.00					

TOTAL WEIGHT = 41 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.29")

CALCULATED VERT. DEFL.(LL) = L/999 (0.01")

ALLOWABLE DEFL.(TL) = L/360 (0.29")

CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.17/1.00 (B-C:1), BC=0.23/1.00 (G-H:4), WB=0.15/1.00 (C-H:1), SB=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

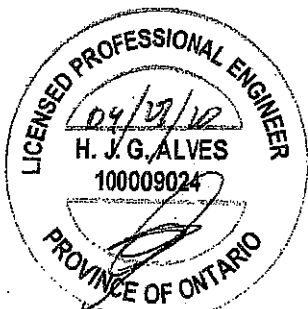
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667	788	1987

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.46 (G) (INPUT = 0.90)
JSI METAL = 0.15 (H) (INPUT = 1.00)



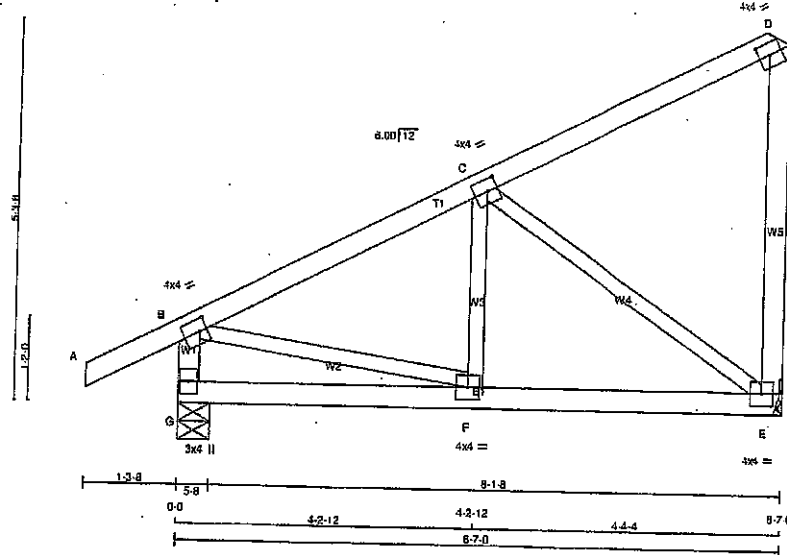
Structural component only
DWG# T-2007047

JOB NAME 408133	TRUSS NAME T42	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
---------------------------	--------------------------	----------------------	-----------------	--------------------------------------	-------------	----------

Tamarack Roof Truss, Burlington

Version 8.310 5 Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:35:57 2020 Page 1
ID:DMCubINVR6TstFoe31v6I_zns11-ejPuBijyhhiOEIqGi2ssiJNTskRiwRg6S9KV5sHzNDAm

Scale = 1/20



N. L. G. A. RULES			
CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS	SIZE	DRY	LUMBER
EXCEPT	2x3	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-1	MT20	4.0	4.0	2.00 1.25
C	TMVW-1	MT20	4.0	4.0	2.00 1.75
D	TMVW-1	MT20	4.0	4.0	2.00 1.75
E	BMVW-1	MT20	4.0	4.0	
F	BMVW-1	MT20	4.0	4.0	
G	BMVW-1	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG	IN-SX	IN-SX
G	590	0	590	0	0	5-8	5-8
E	459	0	459	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

1ST CASE		MAX/MIN COMPONENT REACTIONS		PERM LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL	COMBINED	SNOW	LIVE	PERM LIVE
G	415	285/0	0/0	0/0	0/0	129/0	0/0	415	285/0	0/0	0/0
E	325	212/0	0/0	0/0	0/0	112/0	0/0	325	212/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT.	LOAD LC1 MAX	MEMB.	FORCE (LBS)	VERT.	LOAD LC1 MAX
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/28	-91.8	-91.8 0.12 (1)	F-C	0/80	0.03 (4)	
B-C	-433/0	-91.8	-91.8 0.20 (1)	B-F	0/414	0.09 (1)	
C-D	-23/0	-91.8	-91.8 0.20 (1)	E-D	-142/0	0.04 (1)	
G-B	-558/0	0.0	0.0 0.06 (1)	C-E	-494/0	0.21 (1)	
G-F	0/0	-18.5	-18.5 0.09 (4)				
F-E	0/406	-18.5	-18.5 0.12 (4)				

TOTAL WEIGHT = 39 lb (MIF)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.28")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.28")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.01")

CSI: TC=0.20/1.00 (B-C:1), BC=0.12/1.00 (E-F:4), WB=0.21/1.00 (C-E:1), SS=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

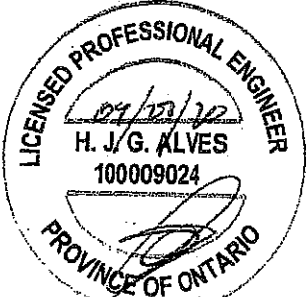
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

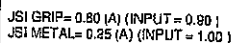
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.73 (B) (INPUT = 0.80)
JSI METAL = 0.22 (B) (INPUT = 1.00)



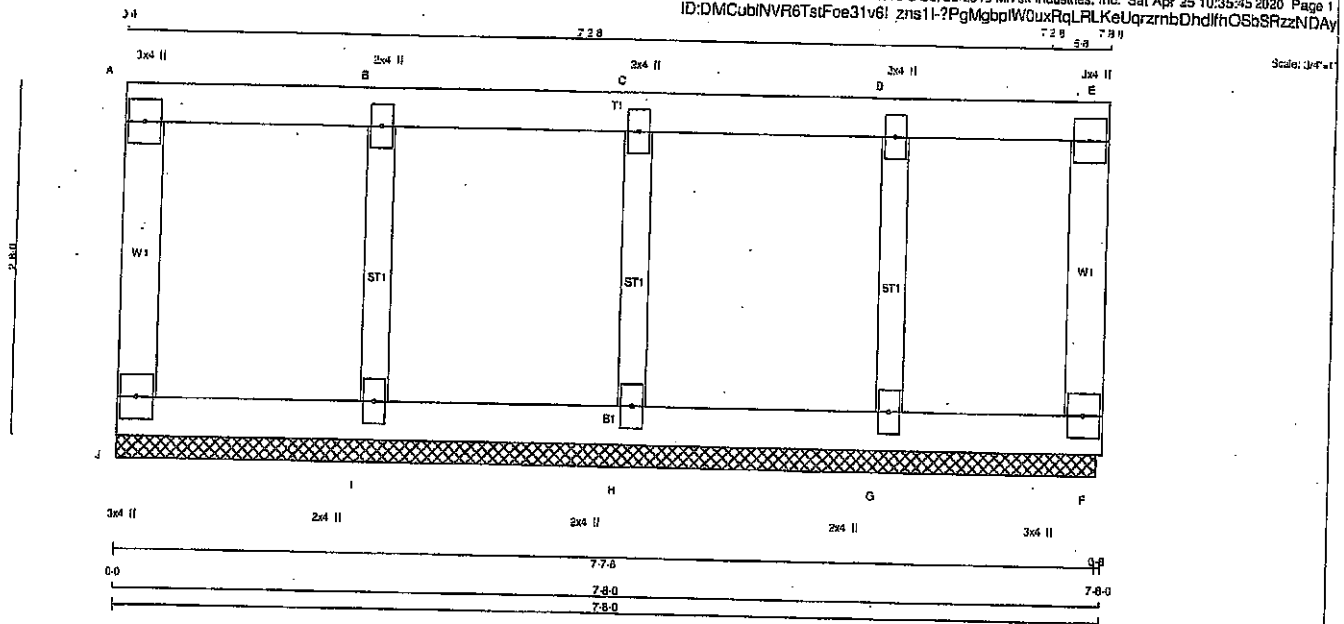
Structural component only
DWG# T-2007048

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:35:59 2020 Page 1
ID:DMCubINVR6tSiFoe31v6l_zns1l-b6WecO_XDJk7_Q79HmOoYApF_?vbAlce_Bw9zNDak



JOB NAME 408133	TRUSS NAME G43	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:35:45 2020 Page 1
ID:DMCubINVR6TsFoe31v6I zns1I-PgMgbpIW0uxRqLLKKeUgrzmbDhdIfhOSbSRzzNDAY



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
J - A	2x4 DRY	No.2	SPF
A - E	2x4 DRY	No.2	SPF
F - E	2x4 DRY	No.2	SPF
J - F	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
ALL GABLE WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
A TMV+p	MT20	3.0	4.0	
B, C, D				
B TMV+w	MT20	2.0	4.0	
E TMV+p	MT20	3.0	4.0	
F BMV1+p	MT20	3.0	4.0	
G, H, I				
G BMV1+w	MT20	2.0	4.0	
J BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING
BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNSPACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSF (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSF (LC)
J-A	-102 / 0	0.0	0.0 0.03 (1)	I-B	-242 / 0	0.05 (1)	0.05 (1)
A-B	-10 / 0	-114.3	-114.3 0.08 (1)	H-C	-229 / 0	0.05 (1)	0.05 (1)
B-C	-10 / 0	-114.3	-114.3 0.08 (1)	G-D	-218 / 0	0.05 (1)	0.05 (1)
C-D	-10 / 0	-114.3	-114.3 0.07 (1)				
D-E	-10 / 0	-114.3	-114.3 0.08 (1)				
E-F	-84 / 0	0.0	0.0 0.03 (1)				
J-I	0 / 10	-18.5	-18.5 0.02 (4)				
I-H	0 / 10	-18.5	-18.5 0.02 (4)				
H-G	0 / 10	-18.5	-18.5 0.02 (4)				
G-F	0 / 10	-18.5	-18.5 0.02 (4)				

TOTAL WEIGHT = 27 lb
(MIR)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 15.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 48.0 PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSF: TC=0.08/1.00 (B-C:1), BC=0.02/1.00 (I-J:4),
WB=0.05/1.00 (B-I:1), SS=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1697 788 1987 1655

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.17 (B) (INPUT = 0.30)
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007039

DRWG NO.

Version 8.310 S Oct 29 2019 MTEK Industries, Inc. Sat Apr 25 10:36:00 2020 Page 1
ID:DMCubINVR6TslFoe31v6l_zns1l-3l4lpk_9_dnok8?Kj_Q?x04LWeJ8e2kurjIITbzND

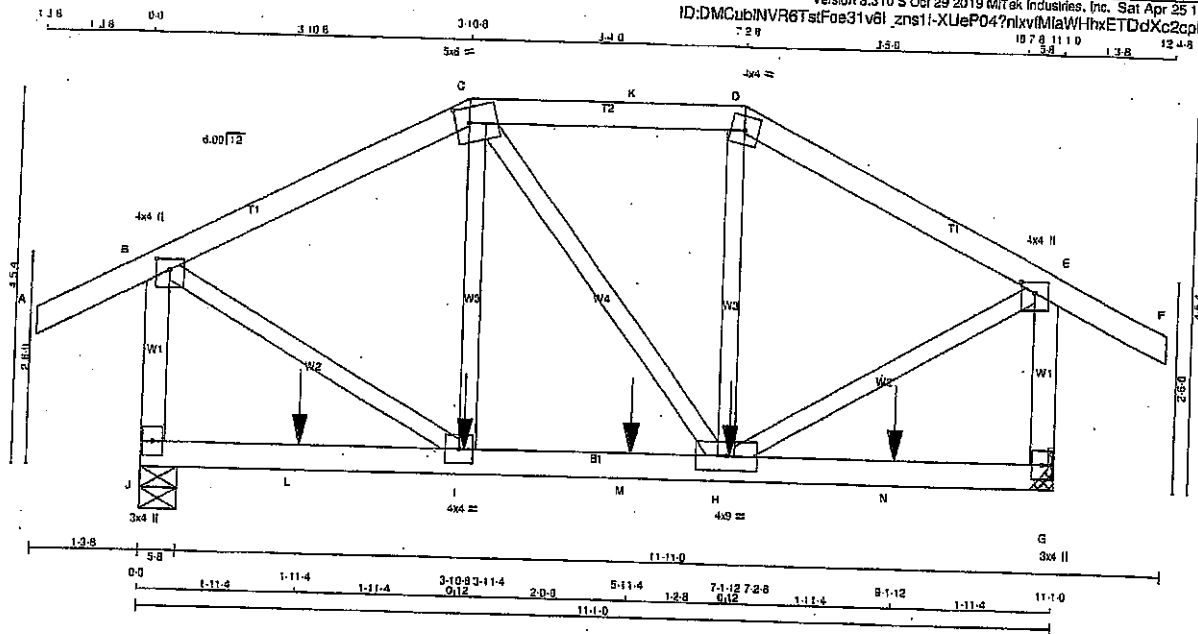


JSI GRIP= 0.64 (C) (INPUT = 0.90)
JSI METAL= 0.18 (A) (INPUT = 1.00)

Structural component only
DWG# T-2007050

JOB NAME 408133	TRUSS NAME T45	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 MITek Industries, Inc. Sat Apr 25 10:36:01 2020 Page 1
 ID:DMCubINVR6TstFos31v6l_zns1l-XUeP04?nkvMlaWlthxETDdXc2cpNU114yTl?2zNDA



Scale = 1/25.1

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4 DRY	No.2	SPF
C - D	2x4 DRY	No.2	SPF
D - F	2x4 DRY	No.2	SPF
J - B	2x4 DRY	No.2	SPF
G - E	2x4 DRY	No.2	SPF
J - G	2x4 DRY	No.2	SPF
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.50 2.00
C	TTWV-m	MT20	5.0	6.0	2.25 2.00
D	TTW-m	MT20	4.8	4.0	
E	TMVW+p	MT20	4.0	4.0	1.50 2.00
G	BMV1+p	MT20	3.0	4.0	
H	BMVWW-t	MT20	4.0	3.0	
I	BMVWW-t	MT20	4.0	4.0	
J	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	JT VERT	UPLIFT	IN-SX
J 1205	0	5-8	5-8
G 1218	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	845	592 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
G	855	599 / 0	0 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.20 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO		FROM TO	MAX. FACTORED VERT. LOAD (LBS)
A-B	0 / 28	-91.8 -91.8 0.13 (1)	10.00
B-C	-944 / 0	-91.8 -91.8 0.28 (1)	8.24
C-K	-771 / 0	-91.8 -91.8 0.20 (1)	6.25
K-D	-771 / 0	-91.8 -91.8 0.20 (1)	6.25
D-E	-861 / 0	-91.8 -91.8 0.28 (1)	8.20
E-F	0 / 28	-91.8 -91.8 0.13 (1)	10.00
J-B	-1098 / 0	0.0 0.0 0.15 (1)	7.52
G-E	-1112 / 0	0.0 0.0 0.15 (1)	7.48
J-L	0 / 0	-18.5 -18.5 0.25 (1)	10.00
L-I	0 / 0	-18.5 -18.5 0.25 (1)	10.00
I-M	0 / 755	-18.5 -18.5 0.32 (1)	10.00
M-H	0 / 755	-18.5 -18.5 0.32 (1)	10.00
H-N	0 / 0	-18.5 -18.5 0.25 (1)	10.00
N-G	0 / 0	-18.5 -18.5 0.25 (1)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
H	7-1-12	-187	-187	---	BACK	VERT	TOTAL	---	C1
I	3-11-4	-187	-187	---	BACK	VERT	TOTAL	---	C1
L	1-11-4	-195	-195	---	BACK	VERT	TOTAL	---	C1
M	5-11-4	-187	-187	---	BACK	VERT	TOTAL	---	C1
N	9-1-12	-195	-195	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 51 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 BOT CH. LL = 8.0 PSF
 BOT CH. LL = 0.0 PSF
 BOT CH. LL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBCC 2010, ORC 2012, ABC 2019
 - PART 9 OF ORC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55% OF 31.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.37")
 CALCULATED VERT. DEFL.(LL) = 1/999 (0.03")
 ALLOWABLE DEFL.(TL) = 1/360 (0.37")
 CALCULATED VERT. DEFL.(TL) = 1/999 (0.05")

CSI: TC=0.28/1.00 (D-E:1), BC=0.32/1.00 (H-I:1), WB=0.22/1.00 (E-H:1), SSI=0.14/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

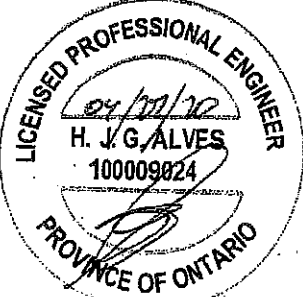
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 518 354 1987 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

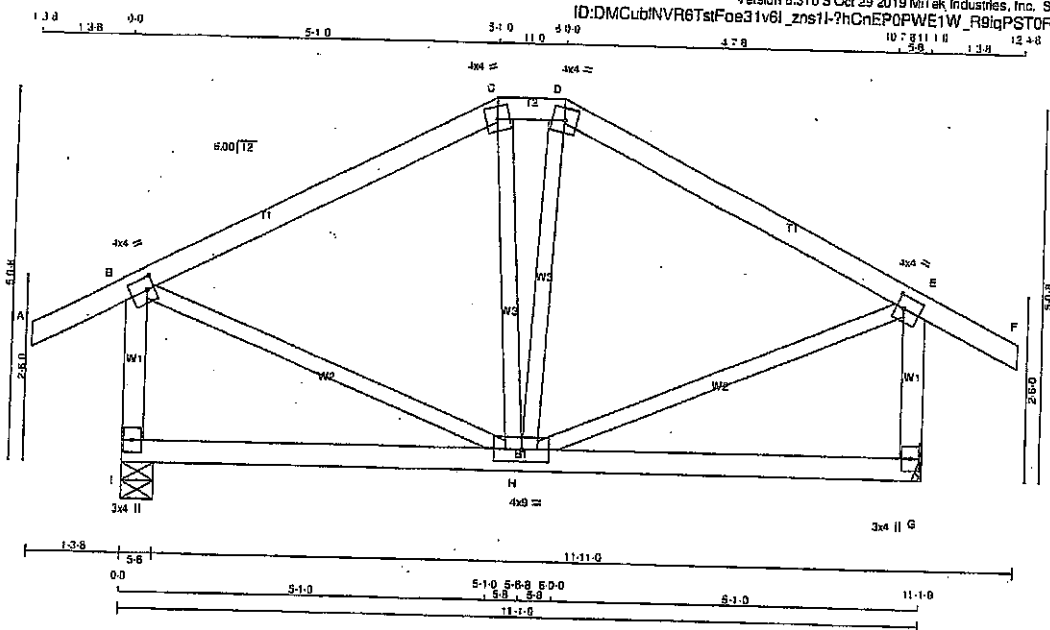
JSI GRIP= 0.89 (I) (INPUT = 0.90)
 JSI METAL= 0.28 (I) (INPUT = 1.00)



Structural component only
 DWG# T-2007051

JOB NAME: 408133 TRUSS NAME: T46 QUANTITY: 1 PLY: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC: Tamarack Roof Truss, Burlington DRWG NO.

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:36:02 2020 Page 1
ID:DMCubINVR6TstFoe31v8l_zns1l-7hCnEP0PWE1W_R9iqPST0RAivS_U6zFBibCsXUzNDah



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
I - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TM/W-1	MT20	4.0	4.0	2.00	1.25
C TTW-m	MT20	4.0	4.0		
D TTW-m	MT20	4.0	4.0		
E TM/W-1	MT20	4.0	4.0	2.00	1.25
G BMV1-p	MT20	3.0	4.0		
H BMWVW-1	MT20	4.0	4.0		
I BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	JT	GROSS REACTION	BRG	BRG	IN-SX	IN-SX
I	735 0	I	735 0	5-8	5-8		
G	735 0	G	735 0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	518	353 / 0	0 / 0	0 / 0	0 / 0	165 / 0	0 / 0
G	518	353 / 0	0 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (LCI)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (LCI)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 28	-91.8	-91.8 0.12 (1)	B-H	0 / 405	0.09 (1)	0.09 (1)
B-C	-422 / 0	-91.8	-91.8 0.31 (1)	H-E	0 / 405	0.09 (1)	0.09 (1)
C-D	-399 / 0	-91.8	-91.8 0.01 (1)	D-E	-88 / 18	0.03 (1)	0.03 (1)
D-E	-422 / 0	-91.8	-91.8 0.31 (1)	E-F	0 / 28	-91.8	-91.8 0.12 (1)
E-F	0 / 28	-91.8	-91.8 0.12 (1)	F-G	-686 / 0	0.0	0.0 0.09 (1)
F-G	-686 / 0	0.0	0.0 0.09 (1)	G-H	-686 / 0	0.0	0.0 0.09 (1)
G-H	0 / 0	-18.5	-18.5 0.16 (4)	H-I	0 / 0	-18.5	-18.5 0.16 (4)
H-I	0 / 0	-18.5	-18.5 0.16 (4)	I-G	0 / 0	-18.5	-18.5 0.16 (4)

TOTAL WEIGHT = 51 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2018 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(95% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.37")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.37")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CSI: TC=0.31/1.00 (D-E:1), BC=0.16/1.00 (G-H:4), WB=0.09/1.00 (B-H:1), SS=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

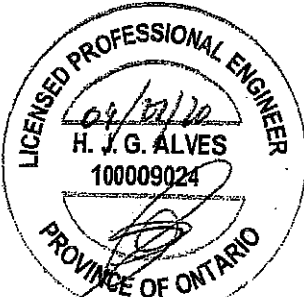
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 798 1987 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

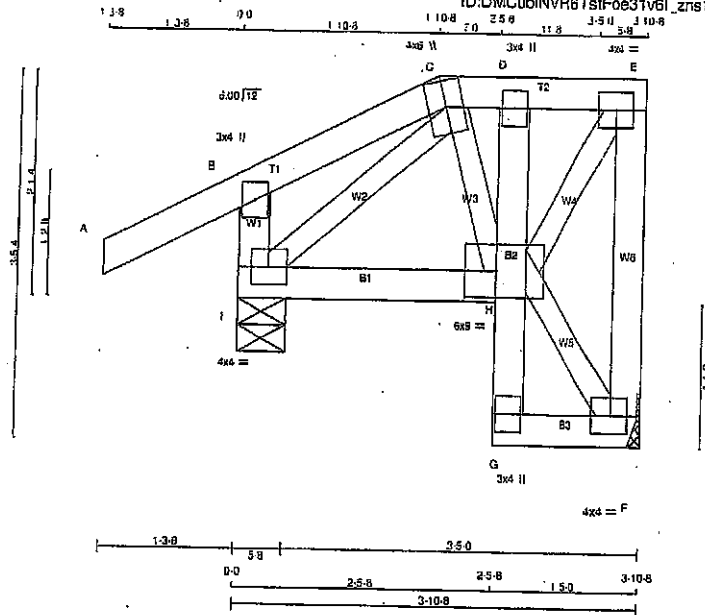
JSI GRIP=0.64 (E) (INPUT = 0.50)
JSI METAL=0.22 (E) (INPUT = 1.00)



Structural component only
DWG# T-2007052

JOB NAME 408133	TRUSS NAME T47S ✓	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 5 Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:36:04 2020 Page 1
ID:DMCubINVR8TstFoe31v6l_zns11-x3KK52g2sHEDI5yqUx5eF42GinauaUmvhycNzNDAl



Scale = 1:19.8

LUMBER			
N. L. & A. RULES	SIZE	LUMBER	
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
I - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
G - D	2x4	DRY	No.2
G - F	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW+m	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
E	TMVW-i	MT20	4.0	4.0		
F	BMVW-i	MT20	4.0	4.0		
G	BMV+p	MT20	3.0	4.0		
H	BMVWW-i	MT20	6.0	9.0	3.00	3.50
I	BMVW-i	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG			
F	214	214	0	0	MECHANICAL	IN-SX	IN-SX
I	338	338	0	0	5-8	5-8	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS				
F	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
I	151	99/0	0/0	0/0	0/0	52/0
I	237	169/0	0/0	0/0	0/0	68/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. (CSI) (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (CSI) (LC)	
FR-TO		FROM TO	UNBRACED LENGTH	FR-TO			
A-B	0/26	-91.8 -91.8	0.14 (5)	10.00	C-H	-13/7	0.00 (4)
B-C	0/0	-91.8 -91.8	0.08 (1)	10.00	H-F	-8/0	0.00 (1)
C-D	-103/0	-91.8 -91.8	0.01 (1)	6.25	I-C	-148/0	0.03 (1)
D-E	-99/0	-91.8 -91.8	0.02 (1)	6.25	H-E	0/172	0.04 (1)
F-E	-198/0	0.0 0.0	0.04 (1)	7.81			
I-B	-212/0	0.0 0.0	0.02 (1)	7.81			
I-H	0/107	-18.5 -18.5	0.04 (4)	10.00			
G-H	0/13	0.0 0.0	0.01 (1)	10.00			
H-D	-98/0	0.0 0.0	0.01 (1)	7.81			
G-F	0/5	-18.5 -18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 24 = 48 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.19")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL) = 1/360 (0.19")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.00")

CSI: TC=0.14/1.00 (A-B-S), BC=0.04/1.00 (H-I-K), WB=0.04/1.00 (E-H-I), SS=0.10/1.00 (A-B-S)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

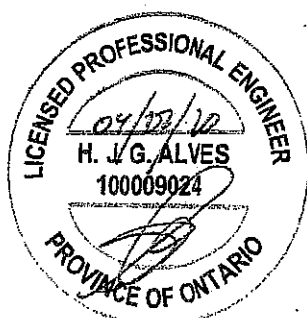
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1587 1656

PLATE PLACEMENT TOL. = 0.250 inches

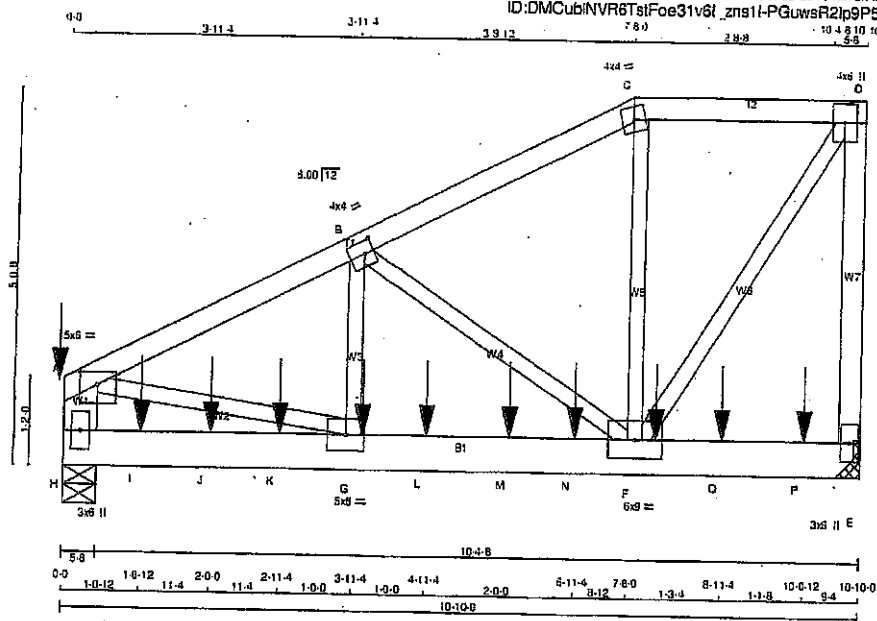
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.21 (H) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007053

JOB NAME 408133	TRUSS NAME T50	QUANTITY 1	PLY. 2	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
H - A	2x6	DRY	No.2
H - E	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
C-D	12	TOP
D-E	12	TOP
H-A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
H-E	12	SIDE (183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (Tables in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV-p	MT20	5.0	6.0	2.00	3.00
B	TMWV-t	MT20	4.0	4.0	2.00	1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
E	3011	0	3011	0	5-8
H	2825	0	2825	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 4-0.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	2142	1396/0	0/0	0/0	0/0	805/0	0/0
H	2004	1276/0	0/0	0/0	0/0	726/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS		MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (PLF)	MAX. (LBS)	MEMB.	FORCE (LBS)	MAX. (LBS)
FR-TO					FR-TO		
A-B	-3575/0	-91.8	-91.8 0.15 (1)	4.85	G-B	0/1536	0.19 (1)
B-C	-1609/0	-91.8	-91.8 0.10 (1)	6.25	B-F	-1974/0	0.37 (1)
C-D	-1516/0	-91.8	-91.8 0.10 (1)	6.25	F-C	0/516	0.08 (1)
E-D	-2581/0	0.0	0.0 0.51 (1)	7.05	F-D	0/2927	0.38 (1)
H-A	-2441/0	0.0	0.0 0.09 (1)	7.81	A-G	0/3271	0.40 (1)
H-I	0/0	-18.5	-18.5 0.24 (1)	10.00			
I-J	0/0	-18.5	-18.5 0.24 (1)	10.00			
J-K	0/0	-18.5	-18.5 0.24 (1)	10.00			
K-G	0/0	-18.5	-18.5 0.24 (1)	10.00			
G-L	0/3210	-18.5	-18.5 0.39 (1)	10.00			
L-M	0/3210	-18.5	-18.5 0.39 (1)	10.00			
M-N	0/3210	-18.5	-18.5 0.39 (1)	10.00			
N-F	0/3210	-18.5	-18.5 0.39 (1)	10.00			
F-O	0/0	-18.5	-18.5 0.15 (1)	10.00			
O-P	0/0	-18.5	-18.5 0.15 (1)	10.00			
P-E	0/0	-18.5	-18.5 0.15 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC1	MAX	MAX	FACE	DIR.	TYPE	HEEL	CONN.
A	0-0	-53	-53	---	TOP	VERT	TOTAL	---	C1
F	8-0-12	-490	-490	---	FRONT	VERT	TOTAL	---	C1
G	4-0-12	-490	-490	---	FRONT	VERT	TOTAL	---	C1
I	1-0-12	-70	-70	---	BACK	VERT	TOTAL	---	C1
J	2-0-0	-499	-499	---	FRONT	VERT	TOTAL	---	C1
K	2-11-4	-705	-705	---	BACK	VERT	TOTAL	---	C1
L	4-11-4	-455	-455	---	BACK	VERT	TOTAL	---	C1
M	6-0-12	-490	-490	---	FRONT	VERT	TOTAL	---	C1
N	6-11-4	-455	-455	---	BACK	VERT	TOTAL	---	C1
O	8-11-4	-440	-440	---	BACK	VERT	TOTAL	---	C1
P	10-0-12	-493	-493	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 55 = 109 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.09/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012, ABC 2019
- PART 8 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

155 LBS OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.36")

CALCULATED VERT. DEFL.(LL) = L/999 (0.03")

ALLOWABLE DEFL.(TL) = L/600 (0.36")

CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TO=0.51/1.00 (D-E:1), BC=0.39/1.00 (F-G:1),

WB=0.40/1.00 (A-G:1), SI=0.32/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

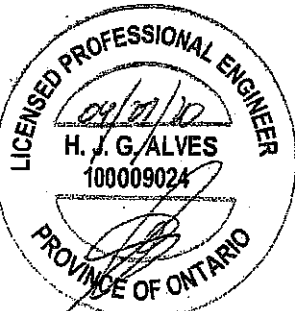
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1657 788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.87 (B) (INPUT = 0.90)
JSI METAL = 0.40 (G) (INPUT = 1.00)



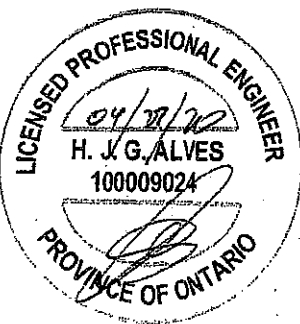
Structural component only
DWG# T-2007054 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408133	T50	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:36:05 2020 Page 2
ID:DMCubINVR6TstF0e31v6I zns11-PGuwsR2lp9P5rvH-WX?Ae3oA IVYJF6d?ZRW8pzNDAe

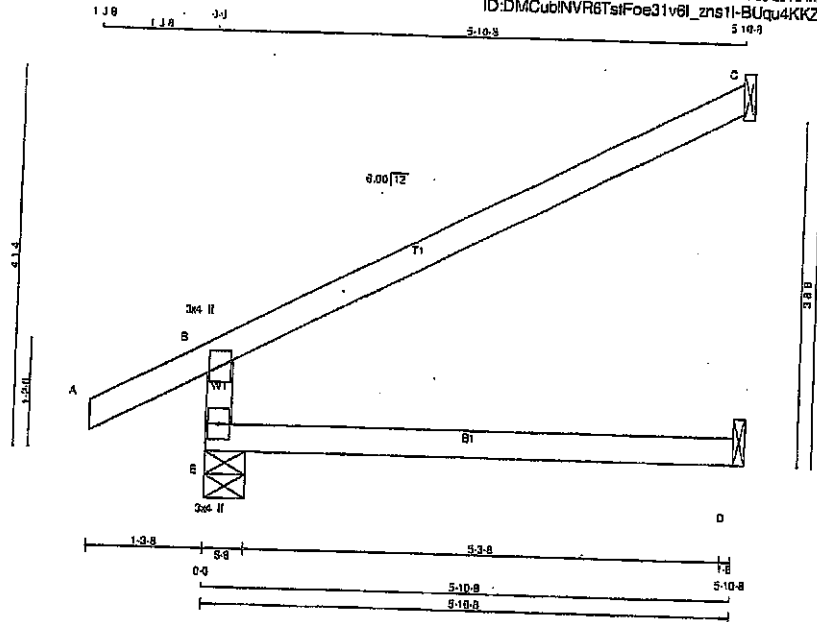
PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
C	TTW-m	MT20	4.0	4.0	
D	TMVW+p	MT20	4.0	8.0	
E	BMV1+p	MT20	3.0	6.0	
F	BMVWW-t	MT20	6.0	9.0	
G	BMVW-t	MT20	5.0	6.0	
H	BMV1+p	MT20	3.0	6.0	



Structural component only
DWG# T-2007054 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408087	J1	21	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 9.310 5 Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 08:33:11 2020 Page 1
ID:DMCubINVR6TstFoe31v6l_zns1t-BUqu4KKZaCY688uhquEMP543auqFk1_H0soSgzNEh6



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT30	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ UPLIFT		
E	525	0	525	0	5-8	5-8
C	202	0	202	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	369	257 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LOAD (LBS)	MAX. UNBRAC LENGTH (IN)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LOAD (LBS)
FR-TO		FROM TO	CSI (LC)		FR-TO		CSI (LC)
E-B	-461 / 0	0.0	0.0	0.13 (4)	7.81		
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-30 / 0	-91.8	-91.8	0.54 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00		

TOTAL WEIGHT = 21 X 17 = 353 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCG 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CSI: TC=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.03/1.00 (n/a:0), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HERE ONLY

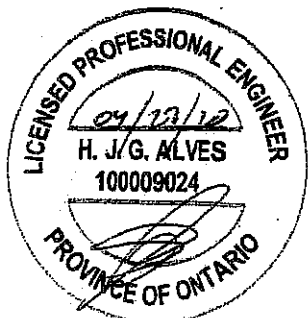
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE GRIP(DRY)	SHEAR	SECTION	(PS)	(PL)	(PL)
MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1997

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.19 (B) (INPUT = 0.90)
JSI METAL = 0.13 (B) (INPUT = 1.00)



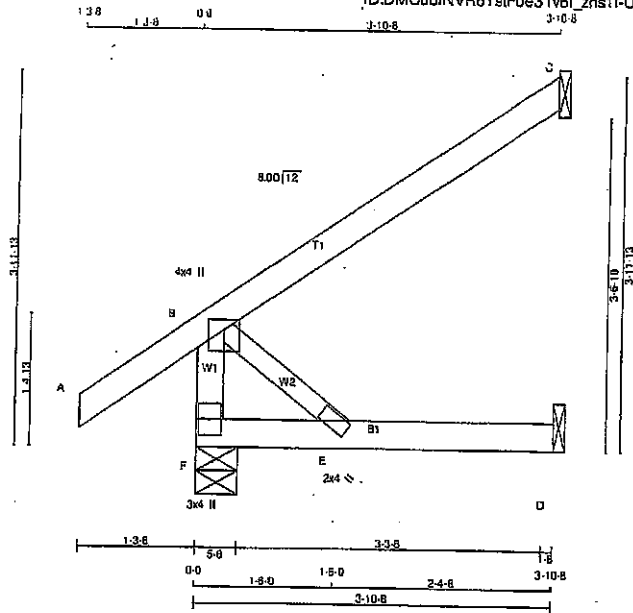
Structural component only
DWG# T-2006949

JOB NAME 408131	TRUSS NAME J2	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

ID:DMCubINVR6TstFoe31v6l_2ns1l-U012Pw97WXq88VMNRKWB8tm65VcPAGtQINbd1PzNDdbd

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:07:18 2020 Page 1

Scale = 1/22.6



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY No.2	SPF
A - C	2x4	DRY No.2	SPF
F - D	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMVW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	REORD
F	340	0	340	0	0	5-8	5-8	
C	178	0	178	0	0	1-8	1-8	
D	36	0	40	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	238	169/0	0/0	0/0	0/0	68/0	0/0
C	122	99/0	0/0	0/0	0/0	23/0	0/0
D	29	0/0	0/0	0/0	0/0	29/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
F-B	-304/0	0.0	0.0 0.03 (1)	7.81	0/0	0.00 (1)	
A-B	0/35	-91.8	-91.8 0.14 (5)	10.00			
B-C	0/0	-91.8	-91.8 0.23 (1)	10.00			
F-E	0/0	-18.5	-18.5 0.08 (4)	10.00			
E-D	0/0	-18.5	-18.5 0.08 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 14 = 57 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NSCC 2010, NSCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.19")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL) = 1/380 (0.18")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.01")

CSI: TC=0.23/1.00 (B-C:1), BC=0.09/1.00 (D-E:1), WB=0.00/1.00 (B-E:1), SS=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

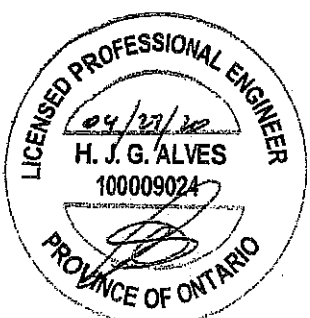
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1857 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.21 (B) (INPUT = 0.90)
JSI METAL = 0.08 (B) (INPUT = 1.00)



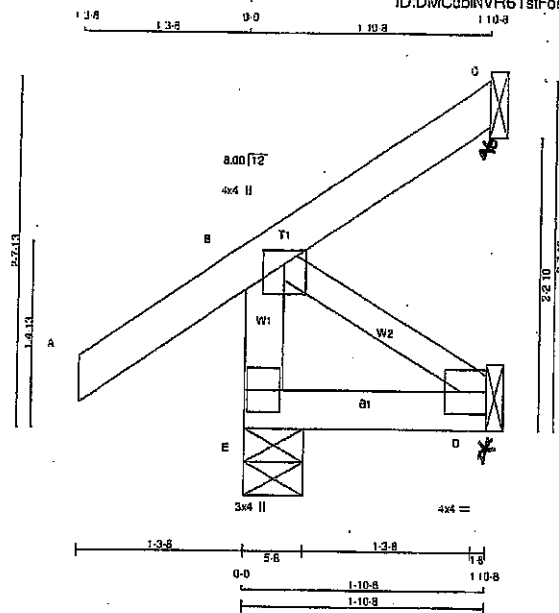
Structural component only
DWG# T-2007014

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408131	J3	4	1	GREEN PARK HOMES	
TRUSS DESC.					

Tarnareck Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:07:20 2020 Page 1
ID:DMCubINVR6TstFos31v6l_zns1l-QP9oqcAG294rOpVlyZfDlrTJlIteANjKh4j5IzNDbb

Scale: 3/4"=1'



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
E - B	2x4 DRY	No.2
A - C	2x4 DRY	No.2
E - D	2x4 DRY	No.2

ALL WEBS 2x3 DRY No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	274	0	274	0	0	5-8	5-8
C	40	0	40	0	-36	1-8	1-8
D	16	0	16	0	0	1-8	1-8

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

PLATES (table is in inches)

JT. TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.25	2.00
D BMV1-t	MT20	4.0	4.0	2.00	Edge
E BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

SEE MITEK STANDARD DETAIL B87791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	191	144 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	27	22 / -25	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 085-08, CSA 085-14
- TPIC 2011, TPIC 2014

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	FR-TO	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	FR-TO
E-B		-258 / 0	0.0	0.0	0.03 (1)	7.81	B-D	0 / 0	0.00 (1)			
A-B		0 / 35	-91.8	-91.8	0.12 (1)	10.00						
B-C		-26 / 0	-91.8	-91.8	0.12 (1)	6.25						
E-D		0 / 0	-18.5	-18.5	0.02 (4)	10.00						

ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CS1: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (B-D:1), SS1=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

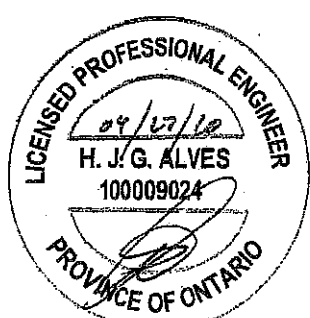
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 789
		1987 1655

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.18 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)

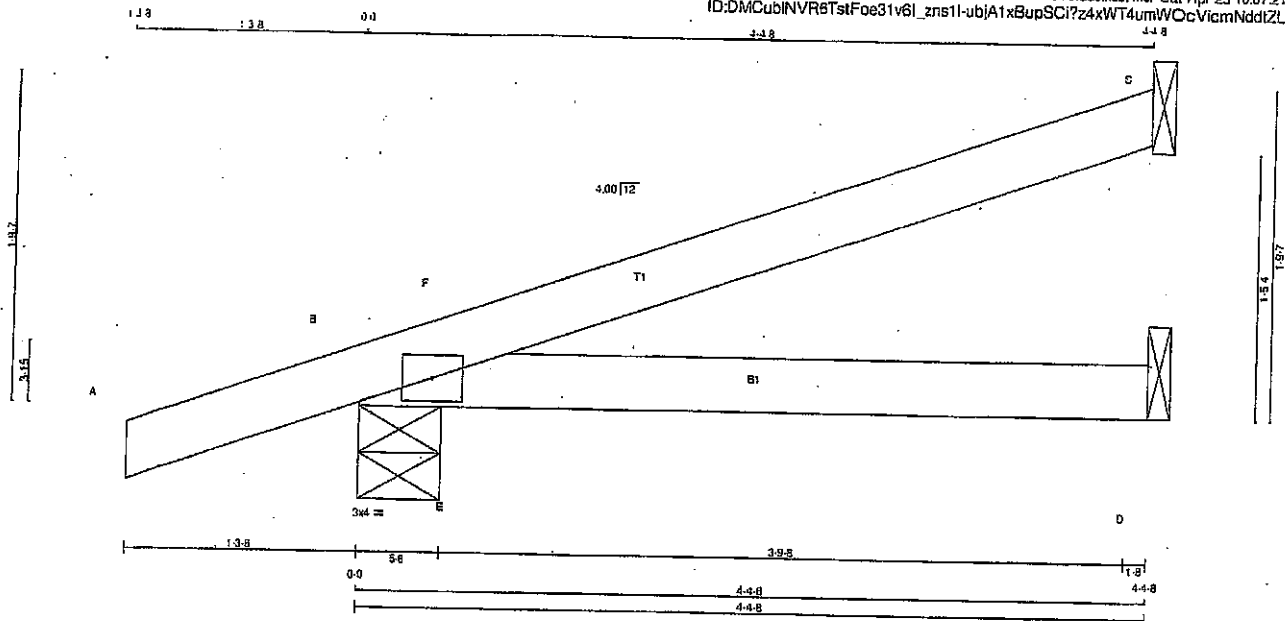


Structural component only.
DWG# T-2007015

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408131	U4	10	1	GREEN PARK HOMES		
Tamarack Roof Truss, Burlington						

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:07:21 2020 Page 1
ID:DMCubINVRtstFoe31v6l_zns11-ubjA1x8upSCI?z4xWT4umWOCvcmNddIZLpHdkzNDba

Scale = 1/16"



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS		No.2	SPF
A - C	2x4	DRY	
B - D	2x4	DRY	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
C	174	0	174	0	1-8	1-8	
B	364	0	364	0	5-8	5-8	
D	68	0	68	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
C	120	93/0	0/0
B	255	180/0	0/0
D	50	19/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S	
MEMB.		FORCE		MAX. FACTORED	
FR-TO		VERT. LOAD		MEMB.	
		FROM TO		LENGTH FR-TO	
		CSF (LC)		E-F	
A-B	0/18	-91.8	-91.8 0.11 (1)	10.00	
B-F	-14/0	-91.8	-91.8 0.05 (4)	8.25	
F-C	0/2	-91.8	-91.8 0.22 (1)	10.00	
B-E	0/0	-18.5	-18.5 0.17 (1)	10.00	
E-D	0/0	-18.5	-18.5 0.17 (1)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2010, CBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 085-14
- TPC 2011, TPC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.02")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.05")

CSF: TC=0.22/1.00 (C-F:1), BC=0.17/1.00 (D-E:1),
WB=0.00/1.00 (E-F:1), SS=0.16/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	618	354
	1667	788
	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.30)

JSI METAL= 0.07 (B) (INPUT = 1.00)



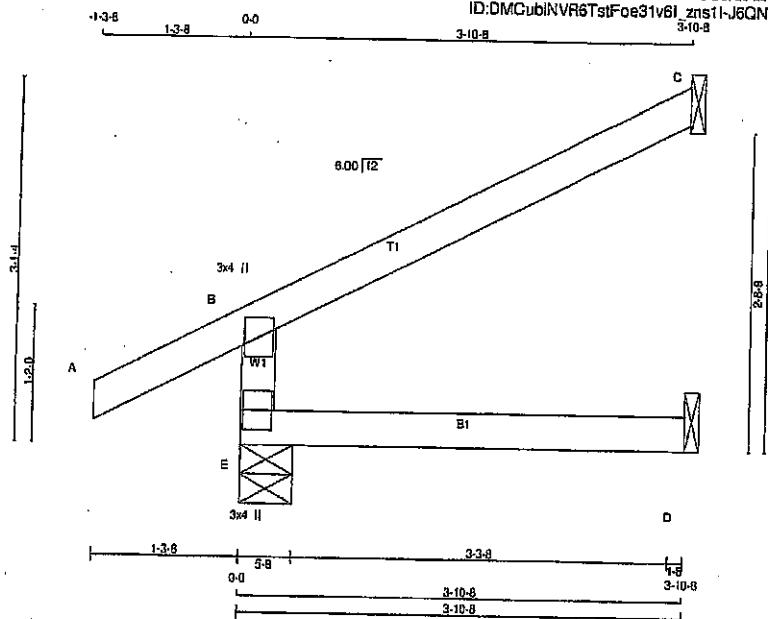
Structural component only
DWG# T-2007016

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408133	J30	2	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Fri May 6 12:53:48 2020 Page 1
ID:DMCubINVR6tstFoe31v6l_zns1l-J6QNC3e?McJWALKpv1nZJVMmex0A13P9Wblc7zIZX

Scale = 1:18.1



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
E	388	0	388	0	0	5-8	5-8	5-8	5-8
C	133	0	133	0	0	1-8	1-8	1-8	1-8
D	30	0	34	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	272	193 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0
C	92	74 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	24	0 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	UNBRACED LENGTH FR-TO
FR-TO				FR-TO			
E-B	-347 / 0	0.0	0.05 (4)	E-B	0.28	-91.8	0.14 (5)
A-B	0.28	-91.8	0.14 (5)	A-B	-20 / 0	-91.8	0.23 (1)
B-C	-20 / 0	-91.8	0.23 (1)	B-C	0.0	-18.5	0.05 (4)
E-D	0.0	-18.5	0.05 (4)	E-D			10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 12 = 24 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCG 2018, NBC 2012, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.01")

CSK TC=0.23/1.00 (B-C:1), BC=0.06/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

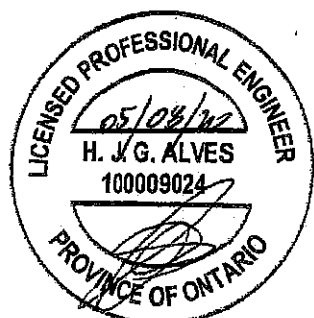
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES			
PLATE GRIP (DRY)	SHEAR	SECTION	
(FS)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

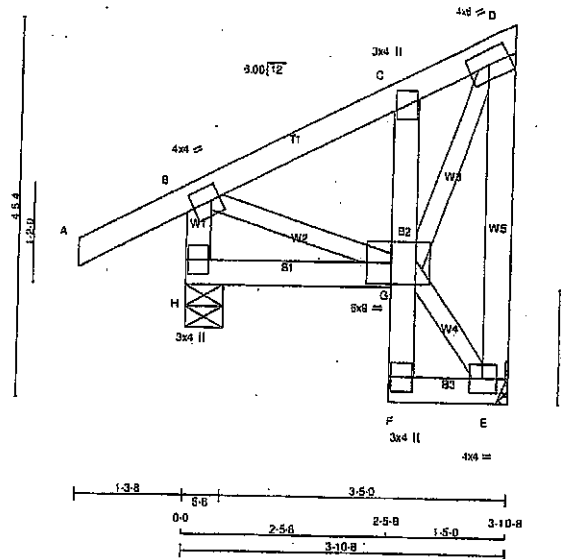
JSI GRIP= 0.15 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



Structural component only
DWG# T-2008522

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
408133	J31S	3	1	GREEN PARK HOMES		
Tamarack Roof Truss, Burlington						

Version 8.310 S Cst 29 2019 MITek Industries, Inc. Sat Apr 25 10:35:47 2020 Page 1
ID:DMCubINVR6TstFoe31v6I_zns11-xoc64Hq72d8eg8VqSihyvG2AMPux5B9_sm4ZVszNDaw



Scale = 1/24.0

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
H - B	2x4 DRY	No.2	SPF
A - D	2x4 DRY	No.2	SPF
H - G	2x4 DRY	No.2	SPF
F - C	2x4 DRY	No.2	SPF
F - E	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
EXCEPT			
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	4.0	4.0	2.00 1.25
C	TMV+p	MT20	3.0	4.0	
D	TMVW-I	MT20	4.0	6.0	2.00 3.00
E	BMVW-I	MT20	4.0	4.0	
F	BMV+p	MT20	3.0	4.0	
G	BVMWVW-I	MT20	6.0	9.0	3.00 3.50
H	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
H	330	0	330	0	0	5-8	5-8		
E	206	0	206	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	231	165 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0
E	145	95 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT.	LOAD LC1 MAX	MEMB.	FORCE (LBS)	MAX	FORCE MAX
FR-TO		FROM	TO	LENGTH	FR-TO		
H-B	-306 / 0	0.0	0.0 0.02 (1)	7.81	B-G	0 / 105	0.02 (1)
A-B	0 / 28	-91.8	-91.8 0.13 (5)	10.00	E-D	-185 / 0	0.04 (1)
B-C	-104 / 0	-91.8	-91.8 0.07 (1)	6.25	G-E	-14 / 0	0.00 (1)
C-D	-114 / 0	-91.8	-91.8 0.06 (1)	5.25	G-D	0 / 229	0.05 (1)
H-G	0 / 0	-18.5	-18.5 0.04 (4)	10.00			
F-G	0 / 12	0.0	0.0 0.02 (1)	10.00			
G-C	-221 / 0	0.0	0.0 0.02 (1)	7.81			
F-E	0 / 9	-18.5	-18.5 0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 25 = 75 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.13/1.00 (A-B-S), BC=0.04/1.00 (G-H-M),
WB=0.05/1.00 (D-G-I), SS=0.10/1.00 (A-B-S)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

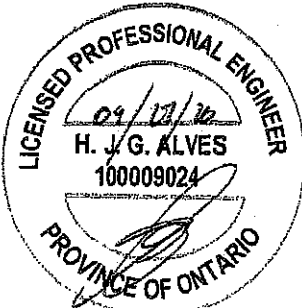
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1987 788 1987 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (D) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

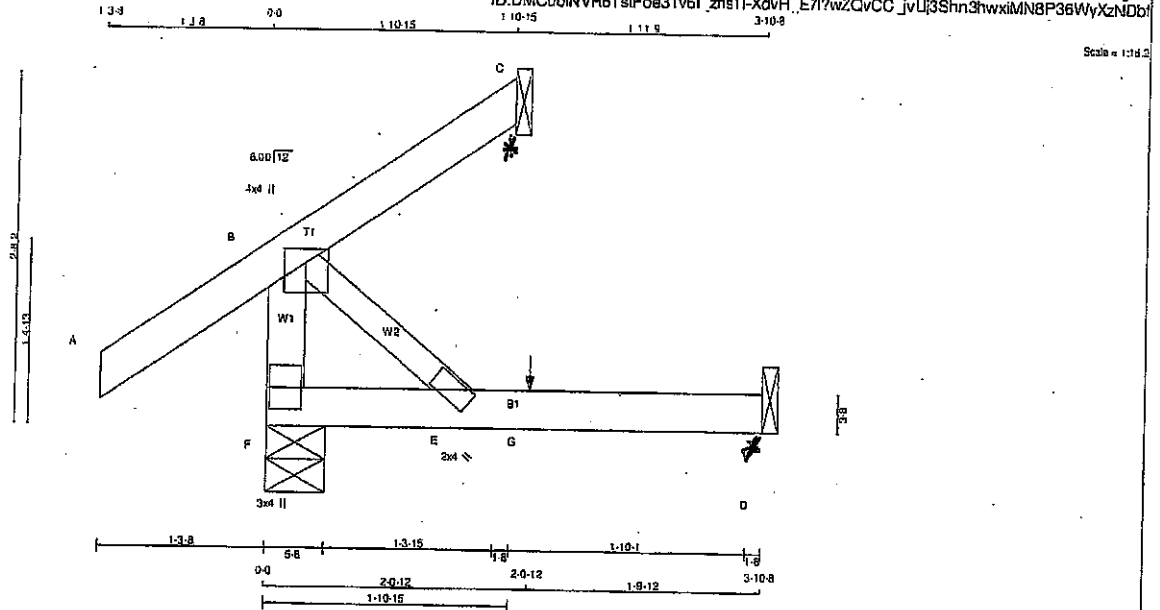


Structural component only
DWG# T-2007040

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408131	C1	3	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:07:16 2020 Page 1
ID:DMCubINVR6TsFoe31v6I_zns1I-XdvH_ET1?wZQvCC_jvUj3Shn3hwxiMN8P36WvXzNDbf



TOTAL WEIGHT = 3 X 11 = 34 lb

LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER.			

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
JT	FACTORED		MAXIMUM FACTORED		INPUT	REQRD	
	GROSS REACTION		GROSS REACTION		BRG	BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	
	IN-SX						
F	295	0	295	0	0	5-8	5-8
C	42	0	42	0	35	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	207	144 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0
C	29	24 / 25	0 / 0	0 / 0	0 / 0	6 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS					WEBS				
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LCH (LC)	MAX. UNBRAC CSI (LC)	MEMB.	MAX. FORCE (LBS)	MAX. FACTORED CSI (LC)		
FR-TO		FROM	TO	LENGTH	FR-TO				
F-B	-259 / 0	0.0	0.0	0.03 (1)	7.81	0 / 0	0.00 (1)		
A-B	0 / 35	-91.8	-91.8	0.14 (5)	10.00				
B-C	-25 / 0	-91.8	-91.8	0.13 (5)	6.25				
F-E	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
E-G	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
G-D	0 / 0	-18.5	-18.5	0.08 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/990$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/990$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.14/1.00 (A-B-S), BC=0.08/1.00 (D-E-F),
WB=0.00/1.00 (B-E-F), SS=0.09/1.00 (A-B-S)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

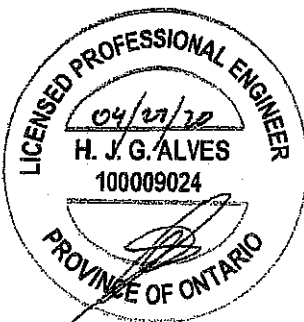
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667
		753	1987
			1655

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.18 (B) (INPUT = 0.90)

JSI METAL = 0.05 (B) (INPUT = 1.00)

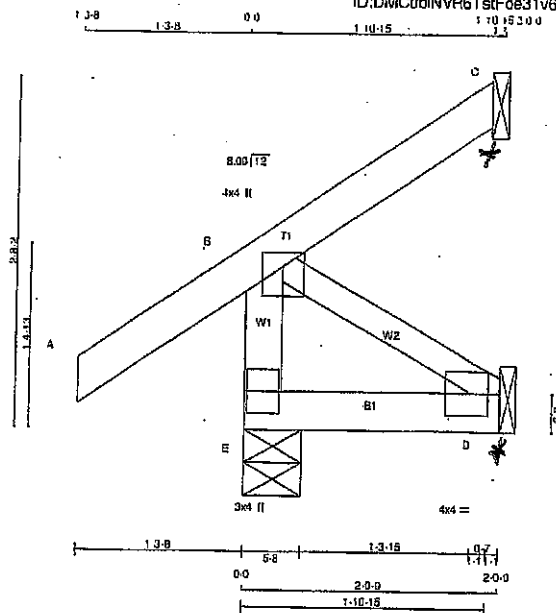


Structural component only
DWG# T-2007012

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408131	C2	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:07:17 2020 Page 1
ID:DMCubNVR6TstFos31v6l_zns11-?qTIBa8NIEhXLnAHd?yqDz45H8RpdHejr3UzzNDbe

Scale = 1:16.2



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMW+p	MT20	4.0	4.0	1.25 2.00
D	BMW1-t	MT20	4.0	4.0	2.00 1.50
E	BMW1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
E	276	0	276	0	0
C	42	0	42	0	0
O	18	0	20	0	0

SEE MITEK STANDARD DETAIL B97791 H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS		1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
E	192	144/0	0/0	0/0	0/0	48/0	0/0	
C	29	24/-25	0/0	0/0	0/0	6/0	0/0	
D	14	0/0	0/0	0/0	0/0	14/0	0/0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO	
E-B	-259/0	0.0	0.0 0.03 (1)	B-D	0/0
A-B	0/35	-91.8	-91.8 0.12 (1)		
B-C	-25/0	-91.8	-91.8 0.12 (1)		
E-D	0/0	-18.5	-18.5 0.02 (4)		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 10 = 29 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPC 2011, TPC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (B-D:1), SS=0.09/1.00 (B-C:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 819 354 1657 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

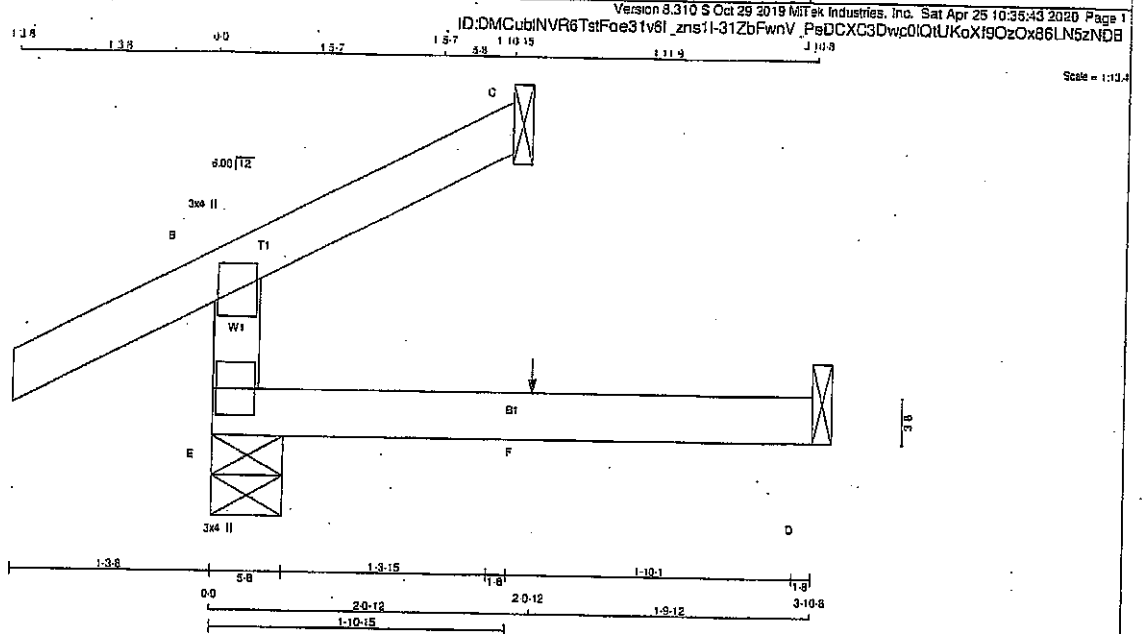
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007013

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408133	C34	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.310 S Oct 29 2019 MITek Industries, Inc. Sat Apr 25 10:35:43 2020 Page 1	



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS	REACTION	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
E	275	0	275	0	0	5-8	5-8	5-8	5-8
C	66	0	66	0	0	1-8	1-8	1-8	1-8
D	30	0	34	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS		1ST CASE		MAX/MIN COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
E	194	130/0	0/0	0/0	0/0
C	46	37/0	0/0	0/0	0/0
D	24	0/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. FACTORED (LBS)	MAX. FACTORED (LBS)	MAX. FACTORED (LBS)
FR-TO		FROM	TO	LENGTH	FR-TO
E-B	-234/0	0.0	0.0	0.05 (4)	7.81
A-B	0/28	-91.8	-91.8	0.14 (5)	10.00
B-C	-10/0	-91.8	-91.8	0.08 (1)	10.00
E-F	0/0	-18.5	-18.5	0.05 (4)	10.00
F-D	0/0	-18.5	-18.5	0.08 (4)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	1	1	1	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 9 = 19 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-03, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.18")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.18")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSF: TC=0.14/1.00 (A-B-5), EC=0.06/1.00 (D-E-4),
WB=0.00/1.00 (A-B-5), SS=0.10/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 L.S.BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

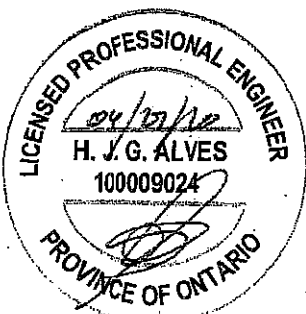
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

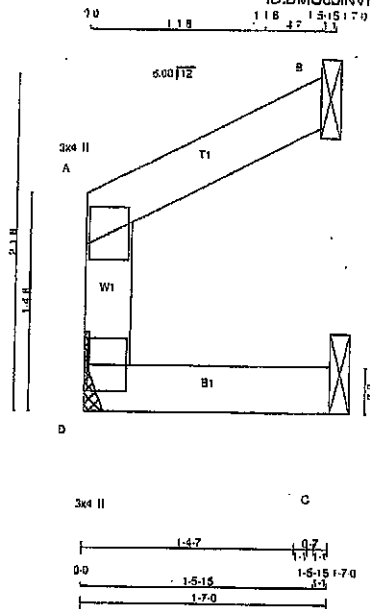
JSI GRIP = 0.10 (E) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2007037

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
408133	C35	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

Version 8.310 S Oct 29 2019 Mitek Industries, Inc. Sat Apr 25 10:35:44 2020 Page 1
ID:DMCubINVR6TstFoe31v6I_zns11-XD6_SGo7lm4ggmFnd7FHeQhpCiburDX9orvuXzNDAz



Scale = 1/13.4

LUMBER	DESCR.
N. L. G. A. RULES	
CHORDS SIZE	
D - A 2x4 DRY No.2	SPF
A - B 2x4 DRY No.2	SPF
D - C 2x4 DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
A TMV-p MT20 3.0 4.0				
D BMV1-p MT20 3.0 4.0				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
D	83	0	83	0
B	55	0	55	0
C	18	0	18	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B, C.

UNFACTORED REACTIONS	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
D	59	38/0 0/0 0/0 0/0 21/0 0/0
B	45	38/0 0/0 0/0 0/0 9/0 0/0
C	14	2/0 0/0 0/0 0/0 12/0 0/0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MEMB.	FORCE	FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)	UNBRAC	MEMB.	FORCE	FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)
		(LBS)						LENGTH							
								FR-TO							
D-A		-72/0		0.0	0.0	0.01 (1)	7.81								
A-B		-2/0		-91.5	-91.5	0.03 (1)	10.00								
D-C		0/0		-18.5	-18.5	0.01 (4)	10.00								

TOTAL WEIGHT = 5 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2010, CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD.

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.00")

CS1: TC=0.03/1.00 (A-B:1), BC=0.01/1.00 (C-D:4),
WB=0.00/1.00 (n/a:0), SS1=0.05/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.03 (D) (INPUT = 0.90)
JSI METAL=0.02 (A) (INPUT = 1.00)



Structural component only
DWG# T-2007038

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

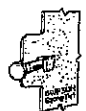
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

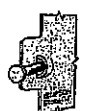
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



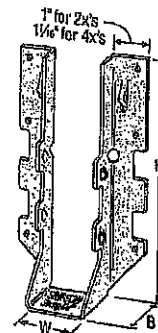
Double-Shear Nailing Top View



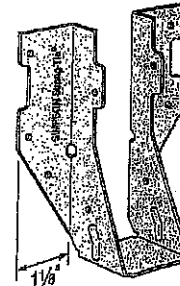
Double-Shear Nailing Side View; Do not bend tab



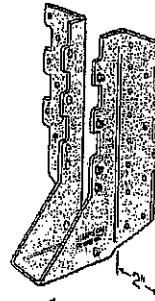
Dome Double-Shear Nailing Side View (available on some models) U.S. Patent 5,603,580



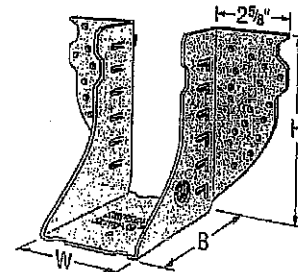
LUS28



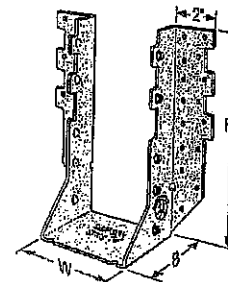
LU26L



HUS210
(HUS26, HUS28, and HHUS similar)

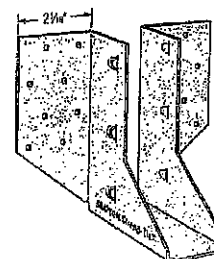
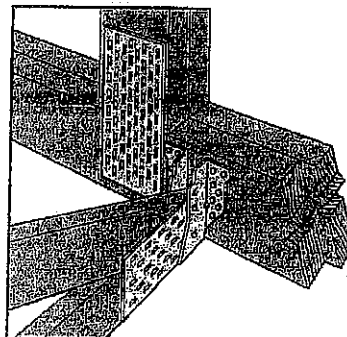


HGUS28-2



HHUS210-2

Typical HUS26 Installation with Reduced Heel Height (Truss Designer to provide fastener quantity for connecting multiple members together)



LJS26DS

Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS**SIMPSON**
Strong-Tie**HHUS/HGUS**

See Hanger Options Information on pp. 125–127.

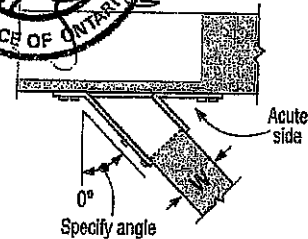
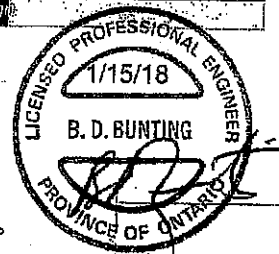
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
$W < 2"$	Bevel or square cut	0.62 of table value	0.46 of table value
$2" < W < 6"$	Bevel cut	0.67 of table value	0.41 of table value
$2" < W < 6"$	Square cut	0.48 of table value	0.41 of table value
$W > 6"$	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(Joist must be bevel cut)
All joist nails installed on the outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _g ³	Header	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15) lb. kN	(K _D = 1.00) lb. kN	(K _D = 1.15) lb. kN	(K _D = 1.00) lb. kN
Single 2x Sizes											
LUS24	18	1 1/8	3 1/2	1 1/2	2 1/4	(4) 10d	(2) 10d	710	1625	845	1155
LU24L	22	1 1/8	3	1 1/2	2 1/8	(4) 10d	(2) 10d x 1 1/2"	316	723	287	514
LU26L	22	1 1/8	5	1 1/2	4 5/8	(6) 10d	(4) 10d x 1 1/2"	360	1020	320	725
LUS26	18	1 1/8	4 1/4	1 1/2	3 3/4	(4) 10d	(4) 10d	1.60	4.54	1.42	3.22
HUS26	16	1 1/8	5 1/2	3	3 1/8	(14) 16d	(8) 16d	720	1605	845	1140
LJS26DS	18	1 1/8	5	3 1/2	4 5/8	(16) 16d	(6) 16d	3.20	714	2.87	5.07
HGUS26	12	1 1/8	5 1/2	5	4 1/4	(20) 16d	(8) 16d	1420	2170	1290	1630
LU28L	20	1 1/8	6 1/2	1 1/2	5 1/8	(8) 10d	(6) 10d x 1 1/2"	6.32	9.85	5.74	7.25
LUS28	18	1 1/8	6 1/2	1 1/2	3 3/4	(6) 10d	(4) 10d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 1/8	(22) 16d	(8) 16d	11.30	21.97	9.20	17.24
HGUS28	12	1 1/8	7 1/2	5	6 1/4	(35) 16d	(12) 16d	2055	4265	1460	4115
LU210L	20	1 1/8	8	1 1/2	7 1/8	(10) 10d	(6) 10d x 1 1/2"	9.14	18.97	6.49	18.31
LUS210	18	1 1/8	7 1/4	1 1/2	3 3/4	(8) 10d	(4) 10d	2685	6625	2685	5700
								11.96	29.51	11.96	25.35
								1140	2185	1020	1550
								5.07	9.72	4.54	6.89
								1420	2520	1290	1790
								6.32	11.21	5.74	7.96
								3605	5385	2675	4345
								16.04	23.88	11.90	19.33
								3310	7875	3310	6900
								14.74	34.19	14.74	30.73
								1140	2495	1020	1770
								5.07	11.10	4.54	7.87
								1420	2785	1290	2210
								6.32	12.39	5.74	9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_g is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

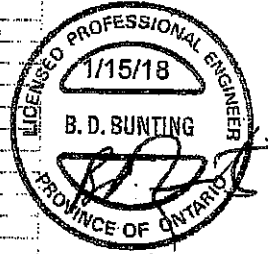
Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance			
		W	H	B	d _a ^a	Header	Joist		D.Fir-L		S-P-F	
									Uplift (K _g = 1.15)	Normal (K _g = 1.00)	Uplift (K _g = 1.15)	Normal (K _g = 1.00)
									lb.	lb.	lb.	lb.
									kN	kN	kN	kN
Double 2x Sizes												
LUS24-2	18	3 1/4	3 1/4	2	1 1/4	(4) 16d	(2) 16d		835	2020	590	1435
SS LUS26-2	18	3 1/4	4 1/4	2	4	(4) 16d	(4) 16d		3.71	8.99	2.62	6.38
									1720	2595	1545	1920
									7.65	11.54	6.87	8.54
HHUS26-2	14	3 1/4	5 1/4	3	3 1/4	(14) 16d	(6) 16d		2850	7335	2065	5205
HGUS26-2	12	3 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d		12.68	32.63	9.20	23.15
									4385	8950	3110	6355
SS LUS28-2	18	3 1/4	7	2	4	(6) 16d	(4) 16d		19.51	39.81	13.83	28.27
									1720	3325	1545	2575
									7.65	14.79	6.87	11.45
HHUS28-2	14	3 1/4	7 1/4	3	6 1/4	(22) 16d	(8) 16d		3765	8940	2675	6345
HGUS28-2	12	3 1/4	7 1/4	4	6 1/4	(36) 16d	(12) 16d		16.75	39.77	11.90	28.22
									6070	12980	4310	9215
SS LUS210-2	18	3 1/4	9	2	6	(8) 16d	(6) 16d		27.00	57.74	19.17	40.99
									2580	4500	2320	3195
									11.48	20.02	10.32	14.21
HHUS210-2	14	3 1/4	9 1/4	3	8	(30) 16d	(10) 16d		4670	9660	4235	7000
HGUS210-2	12	3 1/4	9 1/4	4	8 1/4	(46) 16d	(16) 16d		20.77	42.97	18.84	31.14
									6840	14015	4855	10270
									30.43	62.34	21.60	45.69
Triple 2x Sizes												
HGUS26-3	12	4 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d		4385	8950	3110	6355
									19.51	39.81	13.83	28.27
HGUS28-3	12	4 1/4	7 1/4	4	6 1/4	(36) 16d	(12) 16d		6070	12980	4310	9215
									27.00	57.74	19.17	40.99
HHUS210-3	14	4 1/4	9	3	7 1/4	(30) 16d	(10) 16d		4670	9660	4235	6865
									20.77	42.97	18.84	30.54
HGUS210-3	12	4 1/4	9 1/4	4	8 1/4	(46) 16d	(16) 16d		6840	14645	4855	10400
									30.43	65.14	21.60	46.26
Quadruple 2x Sizes												
HGUS26-4	12	6 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d		4385	8950	3110	6355
									19.51	39.81	13.83	28.27
HGUS28-4	12	6 1/4	7 1/4	4	6 1/4	(36) 16d	(12) 16d		6070	12980	4310	9215
									27.00	57.74	19.17	40.99
HHUS210-4	14	6 1/4	8 1/4	3	7 1/4	(30) 16d	(10) 16d		4670	10155	4235	7210
									20.77	45.17	18.84	32.07
HGUS210-4	12	6 1/4	8 1/4	4	8 1/4	(46) 16d	(16) 16d		6840	14645	4855	10400
									30.43	65.14	21.60	46.26
HGUS212-4	12	6 1/4	10 1/4	4	10 1/4	(56) 16d	(20) 16d		7640	14995	5425	10645
									33.98	66.70	24.13	47.35
HGUS214-4	12	6 1/4	12 1/4	4	11 1/4	(66) 16d	(22) 16d		10130	16400	7195	11645
									45.06	72.95	32.00	61.80
4x Sizes												
LUS46	18	3 3/4	4 3/4	2	3 3/4	(4) 16d	(4) 16d		1720	2595	1545	1920
									7.65	11.54	6.87	8.54
HHUS46	14	3 3/4	5 3/4	3	3 3/4	(14) 16d	(6) 16d		2540	7335	2065	5205
									11.30	32.63	9.20	23.15
HGUS46	12	3 3/4	5 3/4	4	4 3/4	(20) 16d	(8) 16d		4385	8950	3110	6355
									19.51	39.81	13.83	28.27
LUS48	18	3 3/4	6 3/4	2	3 3/4	(6) 16d	(4) 16d		1720	3325	1545	2575
									7.65	14.79	6.87	11.45
HHUS48	14	3 3/4	7 3/4	3	6 3/4	(22) 16d	(8) 16d		3765	8940	2675	6345
									16.75	39.77	11.90	28.22
HGUS48	12	3 3/4	7 3/4	4	6 3/4	(36) 16d	(12) 16d		6070	12980	4310	9215
									27.00	57.74	19.17	40.99
LUS410	18	3 3/4	8 3/4	2	5 3/4	(8) 16d	(6) 16d		2580	4500	2320	3195
									11.48	20.02	10.32	14.21
HGUS410	12	3 3/4	9	4	8 1/4	(46) 16d	(16) 16d		6840	14015	4855	10270
									30.43	62.34	21.60	45.69
HGUS412	12	3 3/4	10 3/4	4	10 3/4	(56) 16d	(20) 16d		7640	14995	5425	10645
									33.98	66.70	24.13	47.35
HGUS414	12	3 3/4	12 3/4	4	11 3/4	(66) 16d	(22) 16d		10130	16400	7195	11645
									45.06	72.95	32.00	61.80



Plated Truss Connectors

See footnotes
on p. 258.

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1/4". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

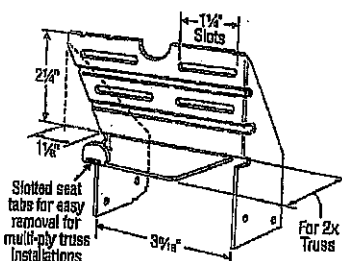
Design: Factored resistances are in accordance with CSA 086-14

Installation:

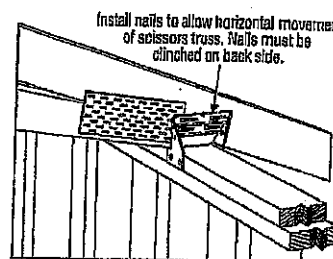
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

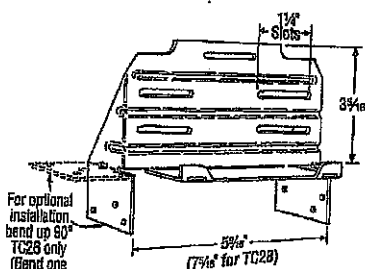
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



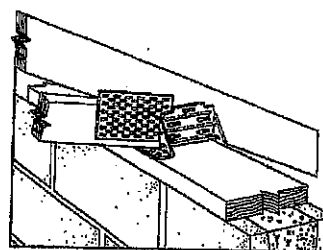
TC24
U.S. Patent 4,932,173



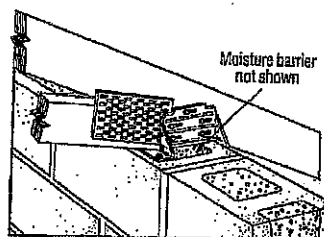
Typical TC24 Installation



TC25
(TC28 Similar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



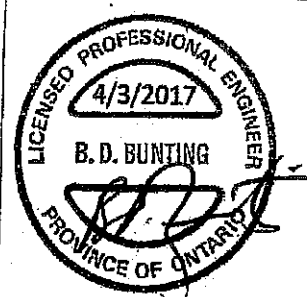
Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _u =1.15)	Uplift (K _u =1.15)
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _u =1.15)	Uplift (K _u =1.15)
TC26	(5) 10d	(6) 10d x 1 1/4"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC28 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) - 3/16" x 2 1/4" Titen screws has a factored uplift resistance of 276 lb.



LIMITED WARRANTY
DESIGN

This technical bulletin is intended to provide information for the design of TC truss connectors. It is not intended to be a substitute for the design professional's responsibility. The information is provided for informational purposes only. It is not intended to be a substitute for the design professional's responsibility. The information is provided for informational purposes only. It is not intended to be a substitute for the design professional's responsibility.

© 2017 Simpson Strong-Tie Company

INSPECTOR: [Signature] exp. 1/1/18

(800) 999-5099
strongtie.com

H/TSP

SIMPSON
Strong-Tie®

Seismic and Hurricane Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Fasteners			Factored Resistance ($K_0 = 1.15$)					
		To Rafters/ Truss	To Plates	To Studs	D.Fir-L			S-P-F		
					Uplift	Lateral		Uplift	Lateral	
						F ₁	F ₂		F ₁	F ₂
					lb.	lb.	lb.	lb.	lb.	lb.
					kN	kN	kN	kN	kN	kN
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
					3.29	3.05	1.33	3.02	2.16	0.96
SS H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
					3.69	0.98	0.33	2.62	0.69	0.24
SS H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
					3.58	0.71	0.71	3.36	0.71	0.71
H2.5T	18	(5) 8d	(5) 8d	—	835	175	210	740	160	210
					3.71	0.78	0.93	3.29	0.71	0.93
SS H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
					3.29	0.80	1.18	2.74	0.56	0.85
H6	16	—	(8) 8d	(8) 8d	1585	1085	—	1125	770	—
					7.05	4.83	—	5.00	3.43	—
H7Z	16	(4) 8d	(2) 8d	(8) 8d	1390	670	—	990	475	—
					6.18	2.98	—	4.40	2.11	—
SS H8 ^a	18	(5) 10d x 1½"	(5) 10d x 1½"	—	1120	—	—	1025	—	—
					4.98	—	—	4.56	—	—
SS H10A ^a	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290
					7.72	3.54	1.82	6.69	2.51	1.29
H10AR	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1485	690	430	1220	570	305
					6.61	3.07	1.91	5.43	2.54	1.36
H10A-2	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1835	1275	430	1645	880	305
					8.16	5.67	1.91	7.32	3.91	1.36
H10S ^{a,b}	18	(8) 8d x 1½"	(8) 8d x 1½"	(8) 8d	1485	795	315	1040	565	225
					6.52	3.54	1.40	4.63	2.51	1.00
H11Z	18	(6) 16d x 2½"	(6) 16d x 2½"	—	1095	920	545	780	655	390
					4.87	4.09	2.42	3.47	2.91	1.73
H14	18	[1] (12) 8d x 1½"	(13) 8d	—	2390	855	320	1805	610	230
		[2] (12) 8d x 1½"	(15) 8d	—	10.63	3.80	1.42	8.03	2.71	1.02
					2390	855	320	1805	610	230
TSP	16	(9) 10d x 1½"	(6) 10d x 1½"	—	10.63	3.80	1.42	8.03	2.71	1.02
					1295	440	—	920	310	—
					5.76	1.96	—	4.09	1.38	—
		(9) 10d x 1½"	(6) 10d	—	1560	440	—	1105	310	—
					6.94	1.96	—	4.92	1.38	—

- Factored resistances have been increased 15% for short term loading; no further increase is allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
- H8 factored uplift resistances for stud-to-bottom plate installations are 595 lb. (2.65 kN) for D.Fir-L and 390 lb. (1.74 kN) for S-P-F.
- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-HTIECONPATH).
- Factored resistances in the F₁ direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.

- H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lb. (6.38 kN) D.Fir-L and 1015 lb. (4.61 kN) S-P-F.
- H10S nails to plates are optional for uplift but required for lateral loads.
- H10A may be field-bent up to a slope of 6/12. Multiply the tabulated uplift value x 0.75. Full tabulated lateral resistances apply.
- The factored resistances of stainless-steel connectors match carbon-steel connectors when installed with Simpson Strong-Tie® stainless-steel, SCNR ring-shank nails. For more information, refer to engineering letter L-F-SSNAIL.S at strongtie.com.
- D.Fir-L/S-P-F factored uplift resistances for the H2.5A fastened to a 2x4 truss bottom chord and double top plates using (5) 8d x 1½" nails into the top plates and (3) 8d x 1½" nails into the lowest three flange holes into the truss bottom chord is 485 lb. (2.20 kN).
- Nails: 16d x 2½" = 0.162" dia. x 2½" long, 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long, 8d x 1½" = 0.131" dia. x 1½" long. See pp. 27–28 for other nail sizes and information.

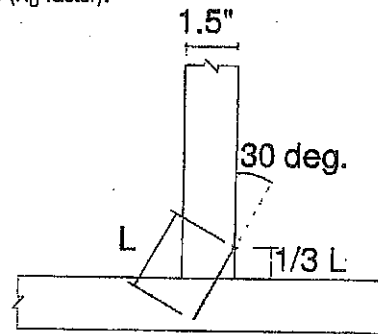
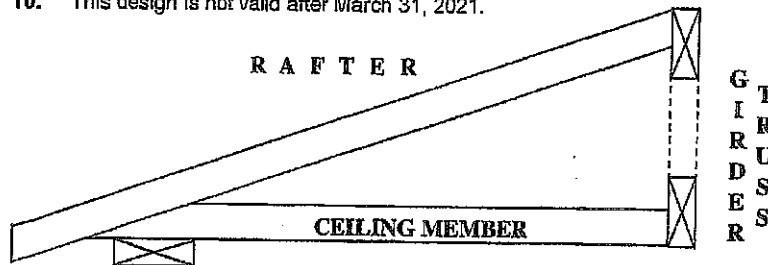
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4.
10. This design is not valid after March 31, 2021.



TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019

PEO
Certificate No. 10880485



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

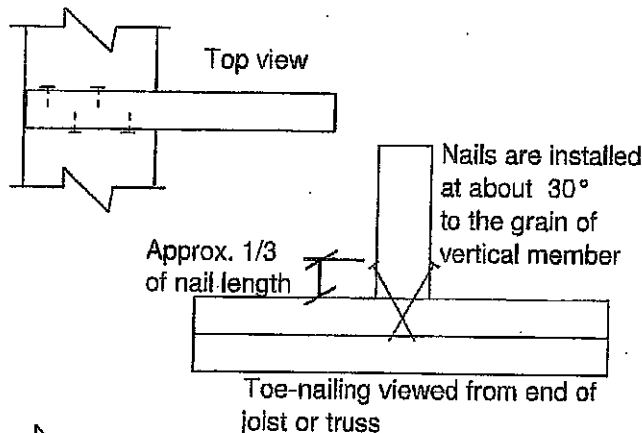
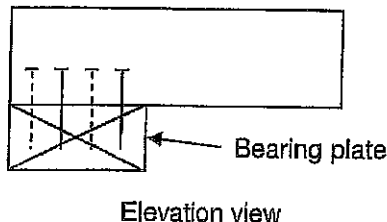
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

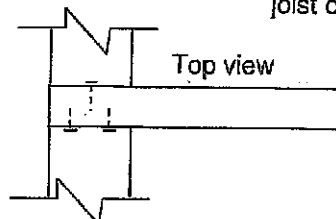
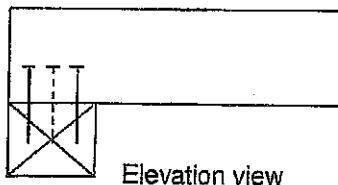
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to wind or earthquake load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

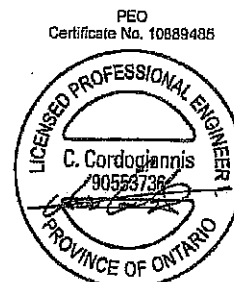
Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



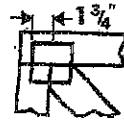
MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7



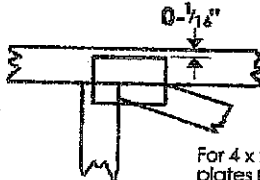
December 2, 2019

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint, unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

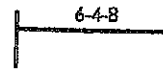


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

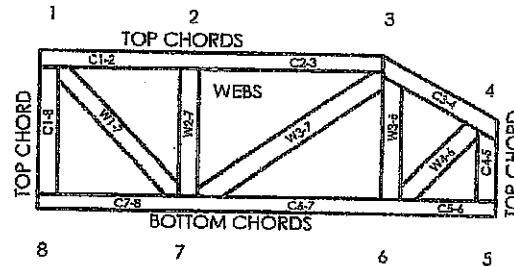
Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

© 2007 MiTek® All Rights Reserved

MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MiT-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

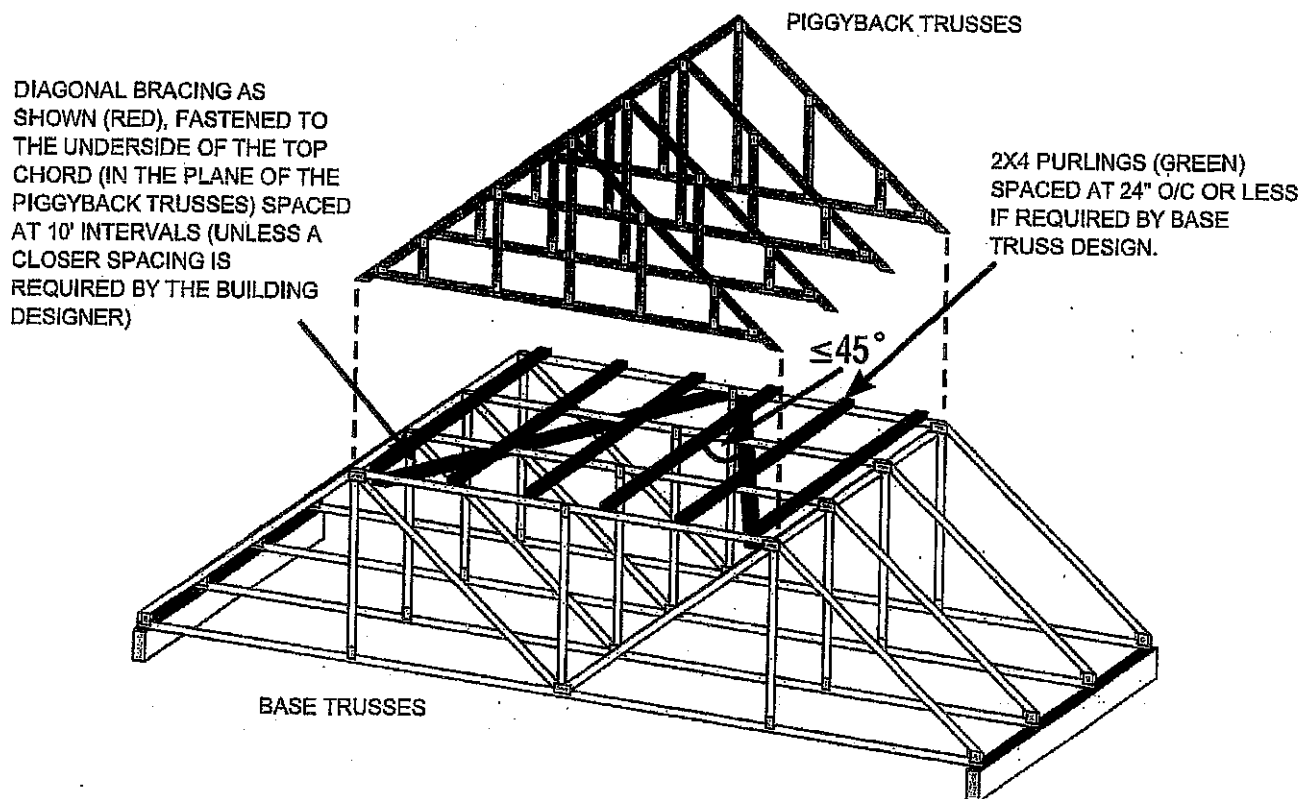
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900219

Feb 09, 2018