

52-06-00
49-06-00
3-00-00
5-00-00

ALL ROOF PITCHES ARE 10/12
UNLESS OTHERWISE NOTED

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

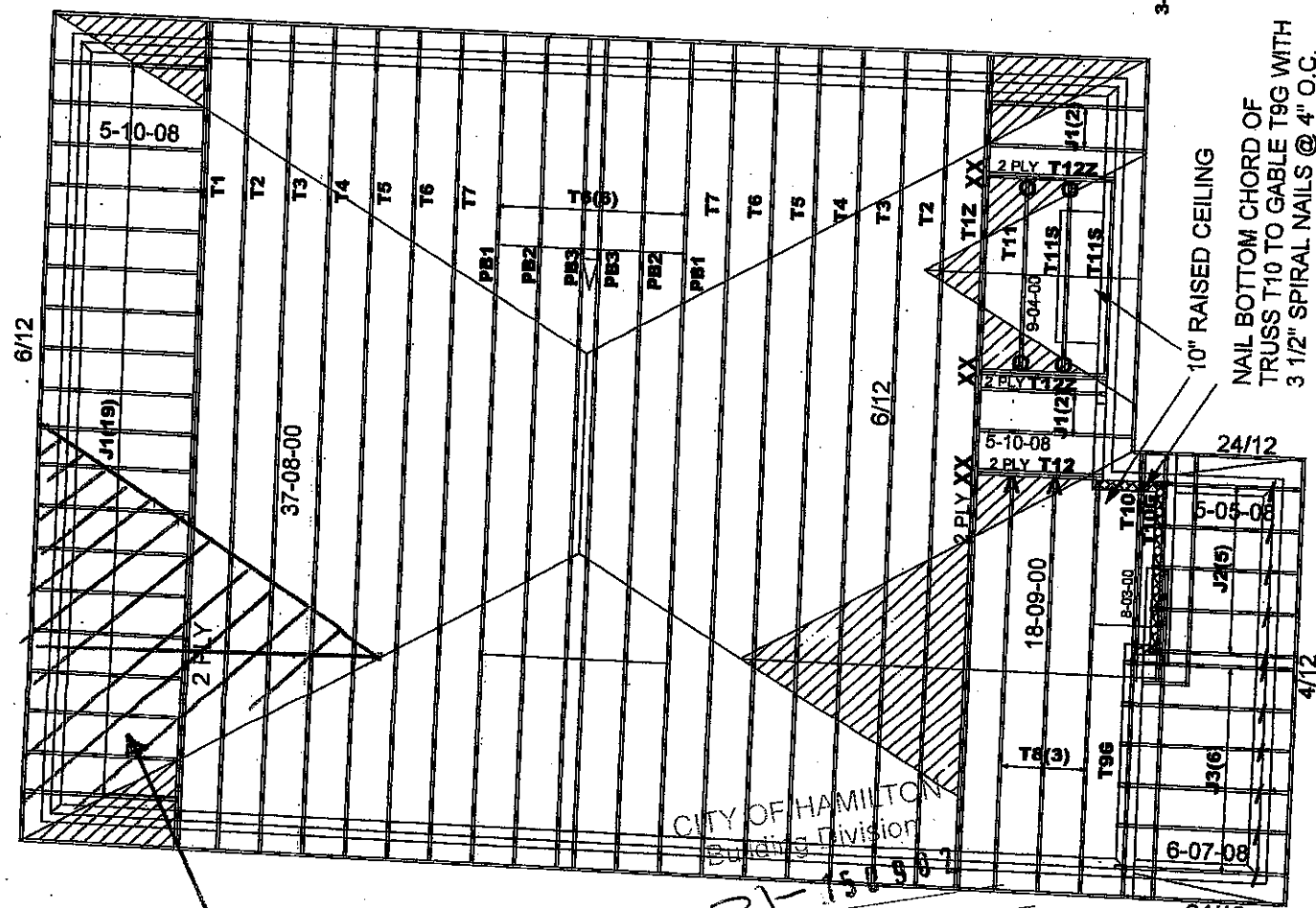
HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
H2.5T - (I)

DENOTES
CONV.
FRAMING 

DESIGN CONFORMS WITH
OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL
PART: 9
Ss = 31.3 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012
(2019 AMENDMENT) ROOF RAFTERS THAT CROSS MEET OVER
TRUSSES TO BE 2X4 SPF @ 24" O.C. WITH A 2X4 VERT. POST
TO THE TRUSS UNDER NEATH AT EACH CROSS PT. VERT. POST
LONGER THAN 8" TO HAVE LATERAL BRACING SO THAT THE
DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING
DOES NOT EXCEED 8"



CITY OF HAMILTON
Building Division

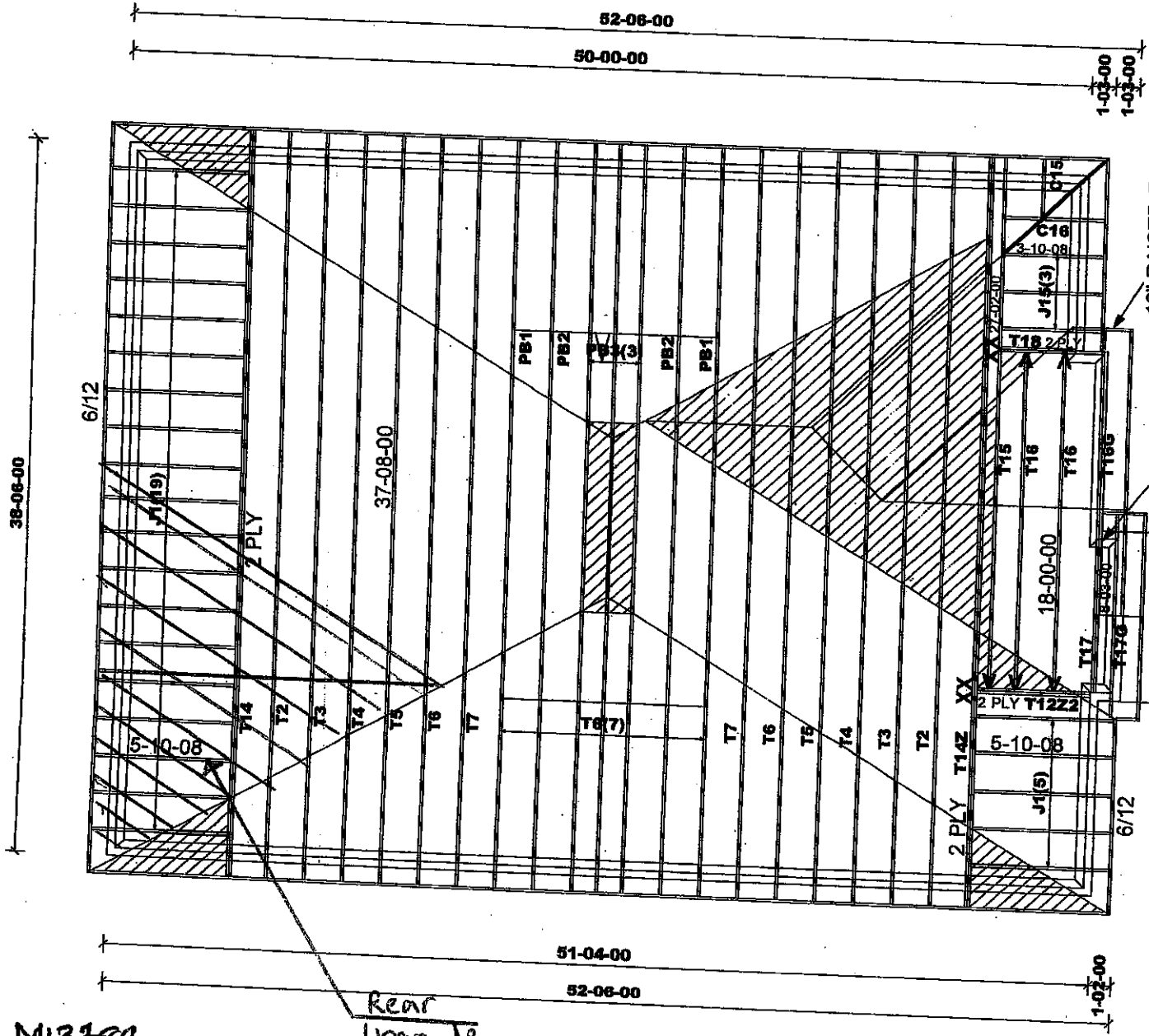
51-04-00 Permit No. 21-150907
52-06-00 THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE
THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH
THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW
These drawings and/or specifications have been reviewed by
DATE OCT 04 2021

Rear
Upgrade

M13789

 TAMARACK ROOF TRUSSES INC.	Job Track: 52166	Builder / Location:	Model / Elevation:
	Plan Log: 204472	GREENPARK HOMES / HAMILTON	SPRINGFIELD 1 / EL.1 (WALKOUT / DECK)
	Layout ID: 417375	Project: RUSSELL GARDENS PH.4	(CONDITION + REAR UPGR)
	Date: 2021-03-22 Sales: Rick DiCiano Designer: LC	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE	

Milek ver 8.4.2.286



ALL ROOF PITCHES ARE 10/12
UNLESS OTHERWISE NOTED

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES
CONV.
FRAMING 

DESIGN CONFORMS WITH
OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL
PART: 9
Ss = 31.3 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

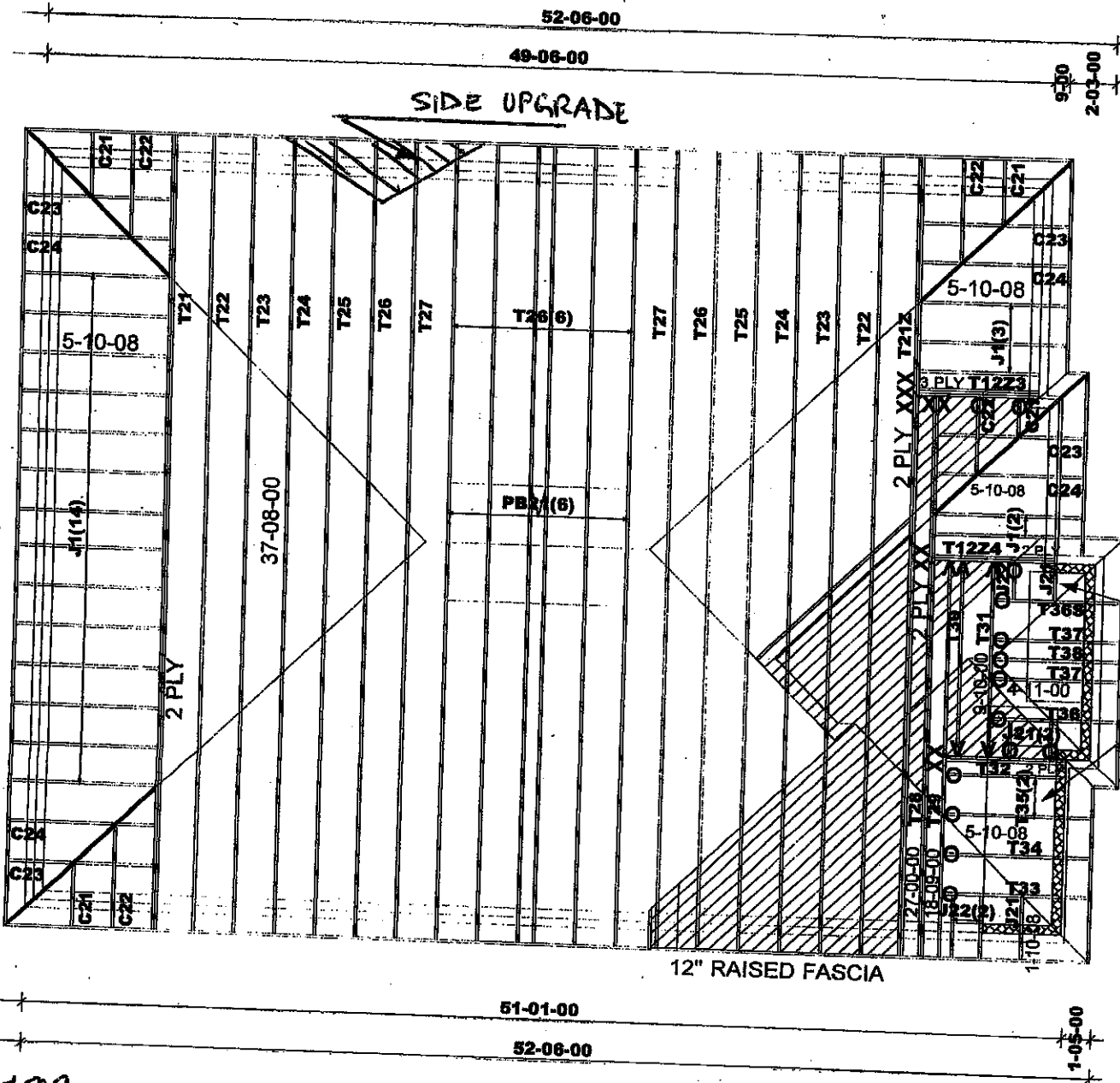
ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C.2012
(2019 AMENDMENT) ROOF RAFTERS THAT CROSS MEET OVER
TRUSSES TO BE 2X4 SPF @ 24" O.C. WITH A 2X4 VERT. POST
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LONGER THAN 6" TO HAVE LATERAL BRACING SO THAT THE
DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING
DOES NOT EXCEED 6"

Rear
Upgrade

M13789



Job Trac: 52166	Builder / Location: GREENPARK HOMES / HAMILTON	Model / Elevation: SPRINGFIELD 1 / EL.2
Plan Log: 204472	Project: RUSSELL GARDENS PH.4	WALKOUT / DECK CONDITION + REAR UPGR
Layout ID: 417406	Date: 2021-03-22 Sales: Rick DiCiano Designer: LC	
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE. Mitek ver 8.4.2.286		



ALL ROOF PITCHES ARE 6/12
UNLESS OTHERWISE NOTED

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
HGUS26-3 - (XXX)

DENOTES
CONV.
FRAMING



DESIGN CONFORMS WITH
OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL
PART: 9
Ss = 31.3 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C.2012
(2019 AMENDMENT) ROOF RAFTERS THAT CROSS MEET OVER
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DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING
DOES NOT EXCEED 6"

M13489



Job Track: 52166

Plan Log: 204472

Layout ID: 417413

Builder / Location:

GREENPARK HOMES / HAMILTON

Project: RUSSELL GARDENS PH.4

Date: 2021-03-22

Sales: Rick DiClano

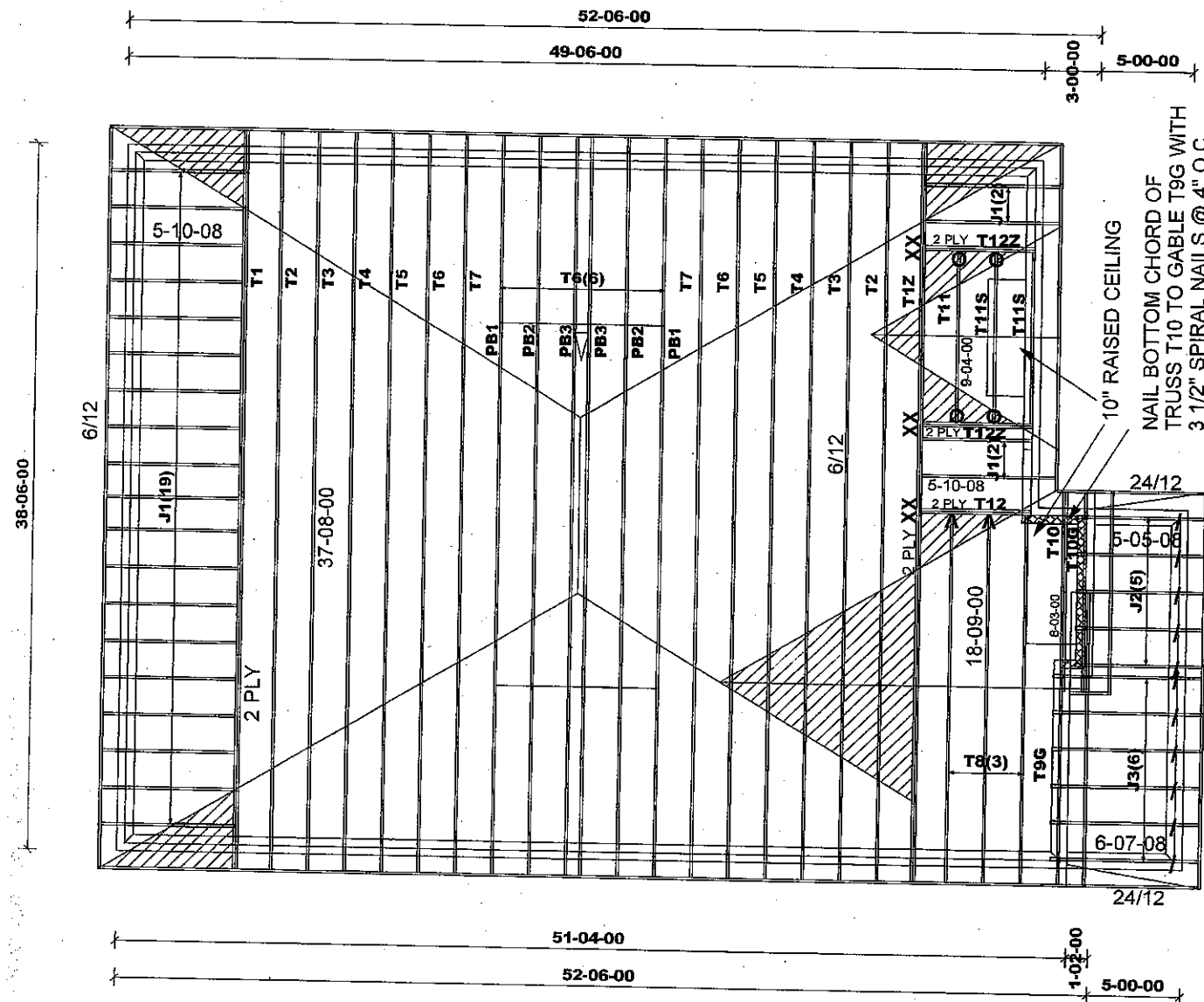
Designer: LC

Model / Elevation:

SPRINGFIELD 1 / EL.3 (LOT 582 SIDE UPGRADE)

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TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER
PURPOSE

Mitek ver 8.4.2.286



ALL ROOF PITCHES ARE 10/12
UNLESS OTHERWISE NOTED

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
H2.5T - (I)

DENOTES
CONV.
FRAMING

DESIGN CONFORMS WITH
OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL
PART: 9
Ss = 31.3 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
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BCDL = 7.4 psf

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C.2012
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DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING
DOES NOT EXCEED 6"

M13789



Job Track: **52166**
Plan Log: **204472**
Layout ID: **417375**

Builder / Location:

GREENPARK HOMES / HAMILTON

Project: **RUSSELL GARDENS PH.4**

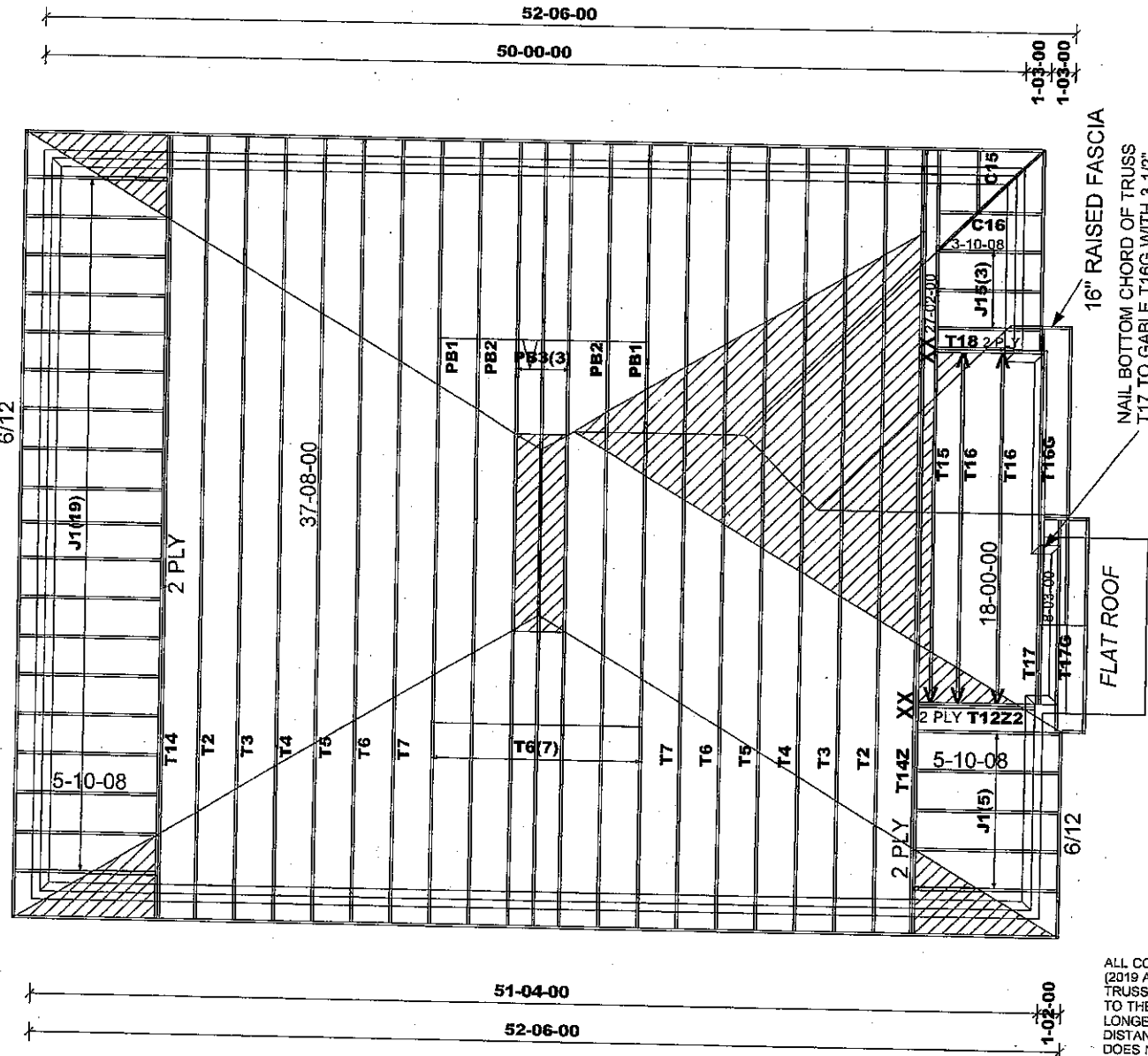
Date: 2021-03-22 Sales: Rick DiCiano Designer: LC

Model / Elevation:

SPRINGFIELD 1 / EL.1

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Mitek ver 8.4.2.286



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12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES
CONV.
FRAMING



DESIGN CONFORMS WITH
OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL
PART: 9
Ss = 31.3 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C.2012
(2019 AMENDMENT) ROOF RAFTERS THAT CROSS MEET OVER
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M13789



Job Track: **52166**
Plan Log: **204472**
Layout ID: **417406**

Builder / Location:

GREENPARK HOMES / HAMILTON

Project: **RUSSELL GARDENS PH.4**

Date: 2021-03-22 Sales: Rick DiCiano Designer: LC

Model / Elevation:

SPRINGFIELD 1 / EL.2

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PURPOSE

Mitek ver 8.4.2.286



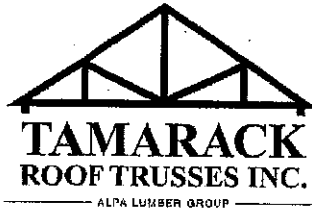
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: SPRINGFIELD 1
 Lot #:
 Elevation: EL.1

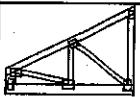
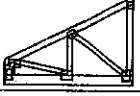
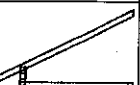
Job Track: 52166
 PlanLog: 204472
 Layout ID: 417375
 Ref #
 Page: 1 of 2
 Date: 03-22-2021
 Designer: Leo Chen
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	10 / 12	37-08-00	4-01-10	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	401.53 238.00		
	1 2-ply	T12 Hip Girder	10 / 12	37-08-00	4-01-10	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	401.53 238.00		
	2	T2 Hip	10 / 12	37-08-00	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	357 223.33		
	2	T3 Hip	10 / 12	37-08-00	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	374.73 232.00		
	2	T4 Hip	10 / 12	37-08-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	391.79 241.00		
	2	T5 Hip	10 / 12	37-08-00	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	407.62 254.67		
	8	T6 Hip	10 / 12	37-08-00	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	1700.25 1042.67		
	2	T7 Hip	10 / 12	37-08-00	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	443.09 272.67		
	3	T8 Common	10 / 12	18-09-00	9-05-07	2 x 4	1-03-08	1-07-11 1-07-11	279.43 178.00		
	1	T9G GABLE	10 / 12	18-09-00	9-05-07	2 x 4	1-03-08 1-03-08	1-07-11 9-11	100.98 66.33		
	1	T10 Common	10 / 12	8-03-00	4-10-12	2 x 4	1-03-08 1-03-08	9-11 9-11	35.92 24.00		
	1	T10G GABLE	10 / 12	8-03-00	4-10-12	2 x 4	1-03-08 1-03-08	9-11 9-11	33.06 22.00		
	1	T11 Common	10 / 12	9-04-00	5-06-05	2 x 4		1-07-11 1-07-11	39 26.17		
	2	T11S Roof Special	10 / 12	9-04-00	5-06-05	2 x 4		1-07-11 1-07-11	91.09 65.00		

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREENPARK HOMES Project: RUSSELL GARDENS PH.4 Location: HAMILTON Model: SPRINGFIELD 1 Lot #: Elevation: EL.1	Job Track: 52166 PlanLog: 204472 Layout ID: 417375 Ref # Page: 2 of 2 Date: 03-22-2021 Designer: Leo Chen Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T12 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	2 2-ply	T12Z Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	116.79 75.33		
	2	PB1 Piggyback	10 /12	19-09-01	2-00-00	2 x 4			122.69 79.00		
	2	PB2 Piggyback	10 /12	19-09-01	3-00-00	2 x 4			124.11 79.00		
	2	PB3 Piggyback	10 /12	19-09-01	4-00-00	2 x 4			128.7 82.67		
	23	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	386.27 245.33		
	5	J2 Jack-Open	4 /12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	72.24 46.67		
	6	J3 Jack-Partial	4 /12	6-07-08	2-11-06	2 x 4	1-03-08	3-15 2-06-07	115.95 73.00		

TOTAL # TRUSS= 77

TOTAL BFT OF ALL TRUSSES= 3842.51

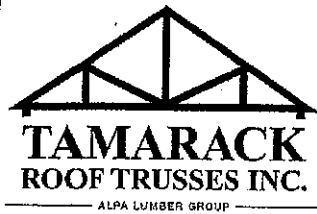
BFT.

TOTAL WEIGHT OF ALL TRSSES 6183.18 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
11	Hardware	H2.5T	
3	Hardware	HCUS26-2	
2	Hardware	LJS26DS	
4	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 20



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: SPRINGFIELD 1
 Lot #:
 Elevation: EL.2

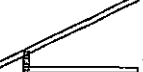
Job Track: 52166
 PlanLog: 204472
 Layout ID: 417406
 Ref #
 Page: 1 of 2
 Date: 03-22-2021
 Designer: Leo Chen
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T2 Hip	10/12	37-08-00	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	357 223.33		
	2	T3 Hip	10/12	37-08-00	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	374.73 232.00		
	2	T4 Hip	10/12	37-08-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	391.79 241.00		
	2	T5 Hip	10/12	37-08-00	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	407.62 254.67		
	9	T6 Hip	10/12	37-08-00	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	1912.78 1173.00		
	2	T7 Hip	10/12	37-08-00	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	443.09 272.67		
	1 2-ply	T12Z2 Monopitch Girder	6/12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T14 Hip Girder	10/12	37-08-00	4-01-10	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	451.99 269.33		
	1 2-ply	T14Z Hip Girder	10/12	37-08-00	4-01-10	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	451.99 269.33		
	1	T15 Hip Girder	10/12	27-02-00	4-10-07	2 x 4 2 x 6	1-03-08	1-07-11 1-07-11	131.81 84.50		
	2	T16 Common	10/12	18-00-00	9-09-11	2 x 4		2-11-11 1-07-11	178.77 111.33		
	1	T16G GABLE	10/12	18-00-00	9-09-11	2 x 4	1-03-08 1-03-08	2-11-11 1-07-11	98.28 63.17		
	1	T17 Common	10/12	8-03-00	5-00-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	38.78 25.83		
	1	T17G GABLE	10/12	8-03-00	5-00-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	40.44 27.83		

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREENPARK HOMES Project: RUSSELL GARDENS PH.4 Location: HAMILTON Model: SPRINGFIELD 1 Lot #: Elevation: EL.2	Job Track: 52166 PlanLog: 204472 Layout ID: 417406 Ref # Page: 2 of 2 Date: 03-22-2021 Designer: Leo Chen Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T18 Monopitch Girder	10/12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	44.83 30.67		
	2	PB1 Piggyback	10/12	19-09-01	2-00-00	2 x 4			122.69 79.00		
	2	PB2 Piggyback	10/12	19-09-01	3-00-00	2 x 4			124.11 79.00		
	3	PB3 Piggyback	10/12	19-09-01	4-00-00	2 x 4			193.05 124.00		
	24	J1 Jack-Open	6/12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	403.07 256.00		
	3	J15 Jack-Open	10/12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	45.68 29.00		
	1	C15 Jack-Open	10/12	1-09-07	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	9.85 7.00		
	1	C16 Jack-Open	10/12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.13 8.33		

TOTAL # TRUSS= 69

TOTAL BFT OF ALL TRUSSES= 3898.66

BFT.

TOTAL WEIGHT OF ALL TRSSES 6292.88 LBS

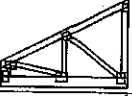
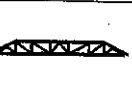
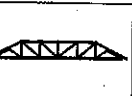
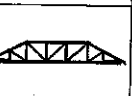
HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
5	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 7

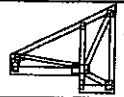
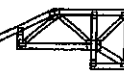
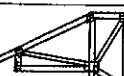
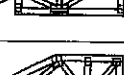
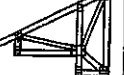
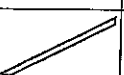
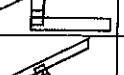
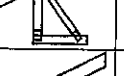
 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREENPARK HOMES Project: RUSSELL GARDENS PH.4 Location: HAMILTON Model: SPRINGFIELD 1 Lot #: Elevation: EL.3	Job Track: 52166 PlanLog: 204472 Layout ID: 417413 Ref # Page: 1 of 3 Date: 03-22-2021 Designer: Leo Chen Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 3-ply	T12Z3 Monopitch Girder	6/12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	87.59 56.50		
	1 2-ply	T12Z4 Monopitch Girder	6/12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T21 Hip Girder	6/12	37-08-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	414.88 267.33		
	1 2-ply	T21Z Hip Girder	6/12	37-08-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	414.88 267.33		
	2	T22 Hip	6/12	37-08-00	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	351.23 215.00		
	2	T23 Hip	6/12	37-08-00	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	353.45 214.67		
	2	T24 Hip	6/12	37-08-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	350.94 213.67		
	2	T26 Hip	6/12	37-08-00	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	363.79 221.33		
	8	T26 Hip	6/12	37-08-00	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1520.23 922.67		
	2	T27 Hip	6/12	37-08-00	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	388.36 237.33		
	1 2-ply	T28 Hip Girder	6/12	27-00-00	4-01-04	2 x 4 2 x 6	1-03-08	2-02-00 1-02-00	251.85 157.33		
	1	T29 Hip Girder	6/12	18-09-00	5-01-04	2 x 4 2 x 6	1-03-08	2-02-00 2-02-00	102.1 63.67		
	1	T30 Common	6/12	9-10-00	4-07-08	2 x 4		2-02-00 2-02-00	39.08 24.83		
	1	T31 Roof Special Girder	6/12	9-10-00	4-07-08	2 x 4		2-02-00 1-02-00	46.36 30.83		

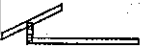
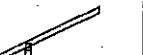
 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREENPARK HOMES Project: RUSSELL GARDENS PH.4 Location: HAMILTON Model: SPRINGFIELD 1 Lot #: Elevation: EL.3	Job Track: 52166 PlanLog: 204472 Layout ID: 417413 Ref # Page: 2 of 3 Date: 03-22-2021 Designer: Leo Chen Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T32 Monopitch Girder	6 /12	5-10-08	5-01-04	2 x 4 2 x 6		1-02-00 5-01-04	73.96 49.33		
	1	T33 Half Hip Girder	6 /12	5-10-08	3-01-04	2 x 4	1-03-08	1-02-00 3-01-04	29.56 20.50		
	1	T34 Half Hip	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	31.87 21.50		
	2	T35 Monopitch	6 /12	5-10-08	5-01-04	2 x 4	1-03-08	1-02-00 5-01-04	67.9 45.33		
	1	T36 Half Hip Girder	6 /12	4-11-00	2-01-04	2 x 4	1-03-08	1-02-00 2-01-04	20.86 14.17		
	1	T36S Half Hip Girder	6 /12	4-11-00	3-01-04	2 x 4	1-03-08	1-02-00 3-01-04	26.55 18.83		
	2	T37 Half Hip	6 /12	4-11-00	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	62.39 43.33		
	1	T38 Monopitch	6 /12	4-11-00	4-07-08	2 x 4	1-03-08	1-02-00 4-07-08	29.48 20.83		
	6	PB21 Piggyback	6 /12	5-11-00	1-05-12	2 x 4			72.31 52.00		
	19	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	319.1 202.67		
	4	J21 Jack-Open	6 /12	1-10-08	2-01-04	2 x 4	1-03-08	1-02-00 2-01-04	28.53 18.67		
	3	J22 Jack-Open	6 /12	1-10-08	3-01-04	2 x 4	1-03-08	2-02-00 3-01-04	30.09 20.50		
	4	C21 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	28.08 18.67		
	4	C22 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	38.28 24.00		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREENPARK HOMES Project: RUSSELL GARDENS PH.4 Location: HAMILTON Model: SPRINGFIELD 1 Lot #: Elevation: EL.3	Job Track: 52166 PlanLog: 204472 Layout ID: 417413 Ref # Page: 3 of 3 Date: 03-22-2021 Designer: Leo Chen Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	4	C23 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	48.33 29.33		
	4	C24 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	56.53 34.67		

TOTAL # TRUSS= 91

TOTAL BFT OF ALL TRUSSES= 3544.49

BFT.

TOTAL WEIGHT OF ALL TRSSES 5704.97 LBS

HARDWARE

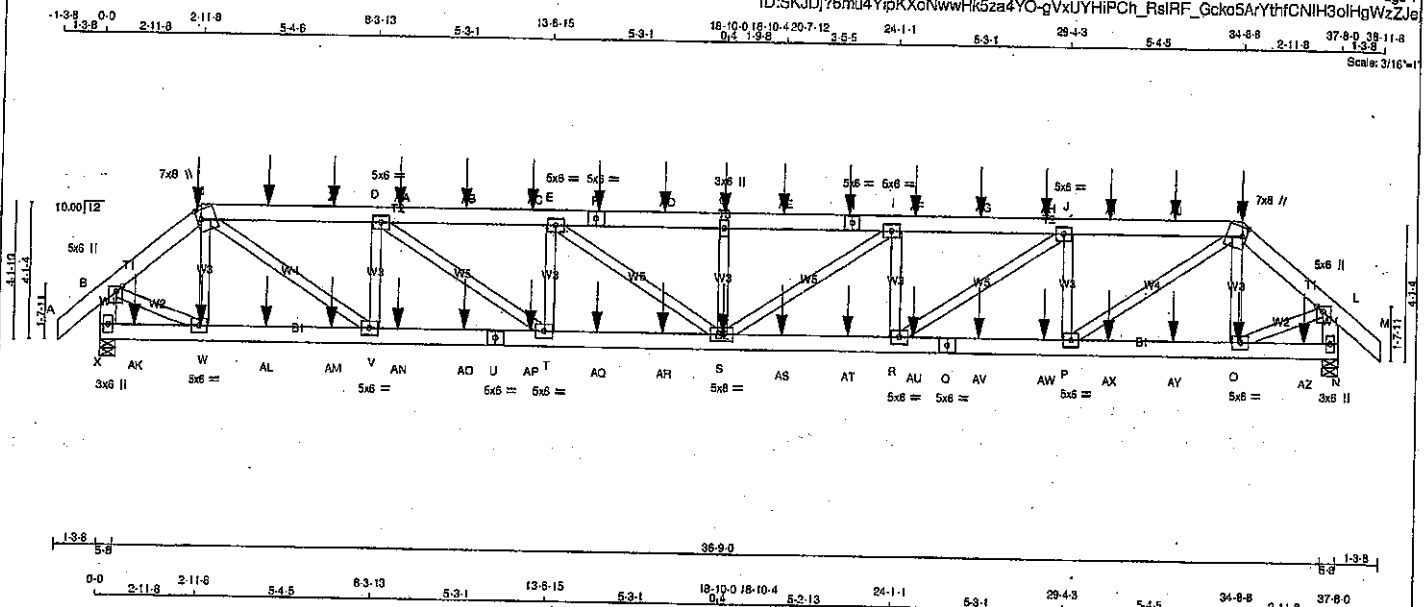
QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
1	Hardware	HGUS26-3	
5	Hardware	LJS26DS	
14	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 23

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417375	T1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR. BEARINGS

CHORDS	SIZE	LUMBER	DESCR.	BEARINGS
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - M	2x6	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
X - U	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE (122.0)
C - F	12	SIDE (61.0)
F - H	12	SIDE (183.1)
H - K	12	SIDE (61.0)
K - M	12	SIDE (122.0)
M - L	12	TOP
N - L	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
X - U	12	SIDE (183.1)
U - Q	12	SIDE (183.1)
Q - N	12	SIDE (183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT VERT	DOWN	0	0
X	3663	0	0
N	3633	0	0

UNFACTORED REACTIONS	1ST LOOSE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
X	2589	1708 / 0
N	2589	1688 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

CHORDS	FACTORED	FACTORED	WEBS	FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MEMB.	FORCE (LBS)
FR-TO			FR-TO	
A-B	0 / 42	-91.8	W-C	-818 / 0
B-C	-3676 / 0	-91.8	C-V	0 / 4222
C-Y	-6277 / 0	-91.8	V-D	-2218 / 0
Y-Z	-6277 / 0	-91.8	D-T	0 / 2452
Z-D	-6277 / 0	-91.8	T-E	-1225 / 0
D-AA	-8292 / 0	-91.8	E-S	0 / 834
AA-AB	-8292 / 0	-91.8	S-G	-794 / 0
AB-AC	-8292 / 0	-91.8	G-I	0 / 843
AC-E	-8292 / 0	-91.8	I-J	-1231 / 0
E-F	-8977 / 0	-91.8	J-K	0 / 2452
F-AD	-8977 / 0	-91.8	K-P	-2223 / 0
AD-G	-8977 / 0	-91.8	P-K	0 / 4232
G-AE	-8977 / 0	-91.8	D-K	-809 / 0
AE-H	-8977 / 0	-91.8	K-W	0 / 2983
H-I	-8977 / 0	-91.8	W-L	0 / 2957
I-AF	-8284 / 0	-91.8	L-C	
AF-AG	-8284 / 0	-91.8		
AG-AH	-8284 / 0	-91.8		
AH-J	-8284 / 0	-91.8		
J-AI	-8281 / 0	-91.8		
AI-AJ	-8281 / 0	-91.8		
AJ-K	-8281 / 0	-91.8		
K-L	-3644 / 0	-91.8		
L-M	0 / 42	-91.8		
M-B	-3648 / 0	0.0		
B-L	-3618 / 0	0.0		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2015 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = L/960 (1.25")
CALCULATED VERT. DEFL (LL) = L/999 (0.22")
ALLOWABLE DEFL (TL) = L/960 (1.25")
CALCULATED VERT. DEFL (TL) = L/999 (0.42")

CSI: TC=0.26/1.00 (D-E-1), BC=0.59/1.00 (S-T-1),
WB=0.37/1.00 (K-P-1), SS=0.14/1.00 (J-K-1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

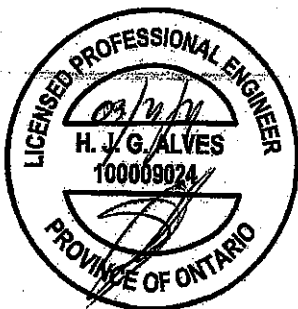
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1907 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.85 (P) (INPUT = 0.80)
JSI METAL = 0.58 (U) (INPUT = 1.00)



Structural component only
DWG# T-2108011 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417375	T1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	2.00	2.25
C	TTWW+m	MT20	7.0	8.0	3.50	1.50
D, E, I, J						
D	TMVW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMVW-w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
K	TTWW+m	MT20	7.0	8.0	3.50	1.50
L	TMVW+p	MT20	5.0	6.0	2.00	2.25
N	BMV1+p	MT20	3.0	6.0		
O, P, R, T, V, W						
O	BMVW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
S	BMVWVW-t	MT20	5.0	6.0		
U	BS-t	MT20	5.0	6.0		
X	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO					
AT-R	0/8284	-18.5	-18.5 0.59 (1)	10.00			
R-AU	0/6261	-18.5	-18.5 0.46 (1)	10.00			
AU-Q	0/6261	-18.5	-18.5 0.46 (1)	10.00			
Q-AV	0/6261	-18.5	-18.5 0.46 (1)	10.00			
AV-AW	0/6261	-18.5	-18.5 0.46 (1)	10.00			
AW-P	0/6261	-18.5	-18.5 0.46 (1)	10.00			
P-AX	0/2751	-18.5	-18.5 0.21 (1)	10.00			
AX-AY	0/2751	-18.5	-18.5 0.21 (1)	10.00			
AY-O	0/2751	-18.5	-18.5 0.21 (1)	10.00			
O-AZ	0/0	-18.5	-18.5 0.04 (4)	10.00			
AZ-N	0/0	-18.5	-18.5 0.04 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-106	-106	---	FRONT	VERT	TOTAL	---	C1
C	2-11-8	-125	-125	---	FRONT	VERT	SNOW	---	C1
F	15-0-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
G	18-10-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
H	22-7-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
K	34-8-8	-50	-50	---	FRONT	VERT	DEAD	---	C1
K	34-8-8	-82	-82	---	FRONT	VERT	TOTAL	---	C1
K	34-8-8	-125	-125	---	FRONT	VERT	SNOW	---	C1
O	34-7-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
S	18-10-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
W	9-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Y	9-0-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
Z	7-0-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AA	9-9-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AB	11-0-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AC	13-0-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AD	17-0-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AE	20-7-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AF	24-7-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AG	26-7-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AH	28-7-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AI	30-7-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AJ	32-7-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AK	1-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AL	5-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AM	7-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AN	9-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AO	11-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AP	13-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AQ	15-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AR	17-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AS	20-7-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AT	22-7-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AU	24-7-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AV	26-7-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AW	28-7-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AX	30-7-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AY	32-7-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AZ	35-7-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

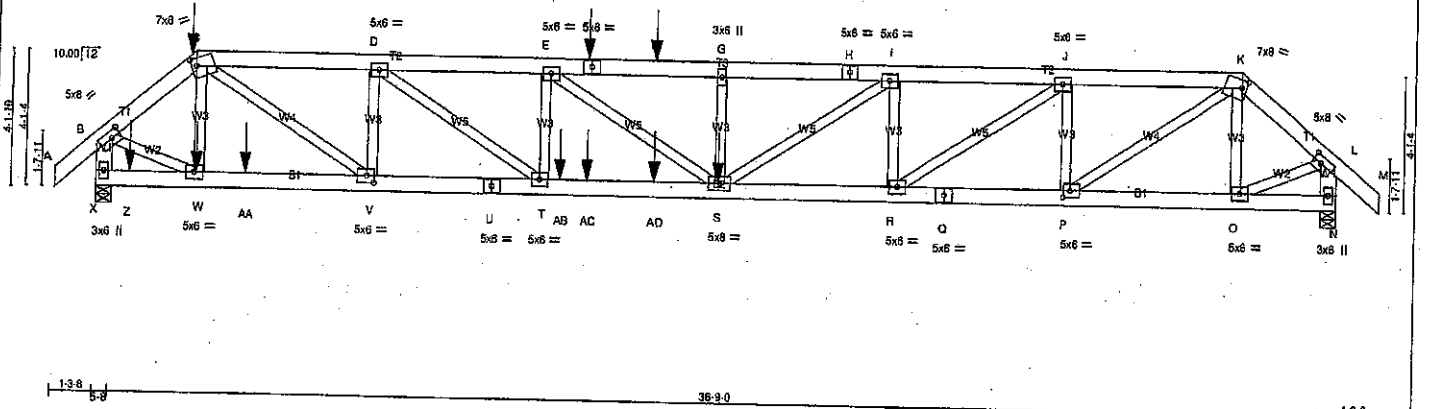
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2108011 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
417375	T1Z	1	2	GREENPARK HOMES		
Tamarack Roof Truss, Burlington						

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ID:SKJDj78mu4YipKXoNwwHk5za4YO-8hVsdj1z76HTS0RXz8zKjL_wHzxpQQHsvrCyzZJe
Scale: 3/16"=1'



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2
C - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - K	2x6	DRY	No.2
K - M	2x6	DRY	No.2
X - B	2x6	DRY	No.2
N - L	2x6	DRY	No.2
X - U	2x6	DRY	No.2
U - Q	2x6	DRY	No.2
Q - N	2x6	DRY	No.2

ALL WEBS 2x4 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF)

TOP CHORDS: (0.122"x3") SPIRAL NAILS

A-C	2	12	SIDE(122.0)
C-F	2	12	SIDE(61.0)
F-H	2	12	SIDE(0.0)
H-K	2	12	TOP
K-M	2	12	TOP
X-B	2	12	TOP
N-L	2	12	TOP

BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS

X-U	2	12	SIDE(183.1)
U-Q	2	12	SIDE(183.1)
Q-N	2	12	TOP

WEBS: (0.122"x3") SPIRAL NAILS

2x4	6	SIDE(187.7)
G-S	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	DOWN	GROSS REACTION	UP		
JT	VERT	HORZ	VERT	HORZ	BRG	BRG
X	4717	0	4717	0	5-8	5-8
N	3471	0	3471	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE		MAX/MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	3325	2244.0	0.0	0.0	0.0	1081.0	0.0
N	2449	1841.0	0.0	0.0	0.0	808.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED		MAX. UNBRAC LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
			VERT. LOAD (PLF)	CSF (LC)				
A-B	0.142							
B-C	-4954.0	-91.8	-91.8	0.04 (1)	10.00	W-C	-407.0	0.03 (1)
C-D	-7954.0	-91.8	-91.8	0.07 (1)	5.18	C-V	0.5048	0.45 (1)
D-E	-11011.0	-91.8	-91.8	0.22 (1)	4.16	V-D	-2583.0	0.22 (1)
E-F	-11897.0	-91.8	-91.8	0.29 (1)	3.56	D-T	0.3720	0.33 (1)
F-Y	-11897.0	-91.8	-91.8	0.41 (1)	3.32	T-E	-1228.0	0.10 (1)
Y-G	-11897.0	-91.8	-91.8	0.41 (1)	3.32	E-S	0.1078	0.10 (1)
G-H	-11897.0	-91.8	-91.8	0.35 (1)	3.32	S-G	-580.0	0.05 (1)
H-I	-11897.0	-91.8	-91.8	0.35 (1)	3.39	I-J	0.2734	0.24 (1)
I-J	-9651.0	-91.8	-91.8	0.25 (1)	3.39	J-R	-2014.0	0.17 (1)
J-K	-6610.0	-91.8	-91.8	0.17 (1)	4.53	R-J	0.3701	0.33 (1)
K-L	-3485.0	-91.8	-91.8	0.05 (1)	5.94	P-J	-2809.0	0.22 (1)
L-M	0.142	-91.8	-91.8	0.04 (1)	10.00	O-K	0.4800	0.42 (1)
X-B	-4823.0	0.0	0.0	0.17 (1)	6.80	K-O	-822.0	0.07 (1)
N-L	-3472.0	0.0	0.0	0.13 (1)	7.50	B-W	0.4020	0.36 (1)
						O-L	0.2828	0.25 (1)
X-Z	0.0	-18.5	-18.5	0.17 (1)	10.00			
Z-W	0.0	-18.5	-18.5	0.17 (1)	10.00			
W-AA	0.3768	-18.5	-18.5	0.43 (1)	10.00			
AA-V	0.3768	-18.5	-18.5	0.43 (1)	10.00			
V-U	0.7954	-18.5	-18.5	0.54 (1)	10.00			
U-T	0.7954	-18.5	-18.5	0.54 (1)	10.00			
T-AB	0.11011	-18.5	-18.5	0.85 (1)	10.00			
AB-AC	0.11011	-18.5	-18.5	0.85 (1)	10.00			
AC-AD	0.11011	-18.5	-18.5	0.85 (1)	10.00			
AD-S	0.11011	-18.5	-18.5	0.85 (1)	10.00			
S-R	0.9651	-18.5	-18.5	0.87 (1)	10.00			
R-Q	0.6610	-18.5	-18.5	0.47 (1)	10.00			
Q-P	0.6610	-18.5	-18.5	0.47 (1)	10.00			
P-O	0.2629	-18.5	-18.5	0.20 (1)	10.00			
O-N	0.0	-18.5	-18.5	0.03 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-30	-108		FRONT	VERT	DEAD		C1
C	2-11-8	-106	-108		BACK	VERT	TOTAL		C1
C	2-11-8	-125	-125		FRONT	VERT	SNOW		C1
C	14-10-4	-76	-76		BACK	VERT	TOTAL		C1
S	18-9-8	-1001	-1001		BACK	VERT	TOTAL		C1
S	3-0-4	-21	-21		BACK	VERT	TOTAL		C1
Y	16-10-4	-76	-76		BACK	VERT	TOTAL		C1
Z	1-0-4	-21	-21		BACK	VERT	TOTAL		C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=		39.0	PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
-PART 9 OF CBC 2018, ABC 2019
-PART 9 OF OBC 2012 (2019 AMENDMENT)
-CSA 086-14
-TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.29")
ALLOWABLE DEFL.(TL) = L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/840 (0.54")

CSF: TC=0.41/1.00 (E-G:1), BC=0.85/1.00 (S-T:1), WB=0.45/1.00 (C-V:1), SS=0.28/1.00 (S-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00, COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1887 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.87 (U) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2108012 1/L

JOB NAME 417375	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	8.0	2.50	3.25
C	TTWW-m	MT20	7.0	8.0	2.75	1.75
D, E, I, J						
D	TMWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
K	TTWW-m	MT20	7.0	8.0	2.75	1.75
L	TMWW-t	MT20	5.0	8.0	2.50	3.25
N	BMV-t+p	MT20	3.0	6.0		
O, R, T, W						
O	BMWW-t	MT20	5.0	8.0		
P	BMWW-t	MT20	5.0	8.0	2.50	2.50
Q	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	8.0		
U	BS-t	MT20	5.0	6.0		
V	BMWW-t	MT20	5.0	8.0	2.50	2.50
X	BMV-t+p	MT20	3.0	6.0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AA	4-5-8	-578	-578	---	BACK	VERT	TOTAL	---	C1
AB	14-0-8	-578	-578	---	BACK	VERT	TOTAL	---	C1
AC	14-10-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AD	16-10-4	-21	-21	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

NOTES- (1)

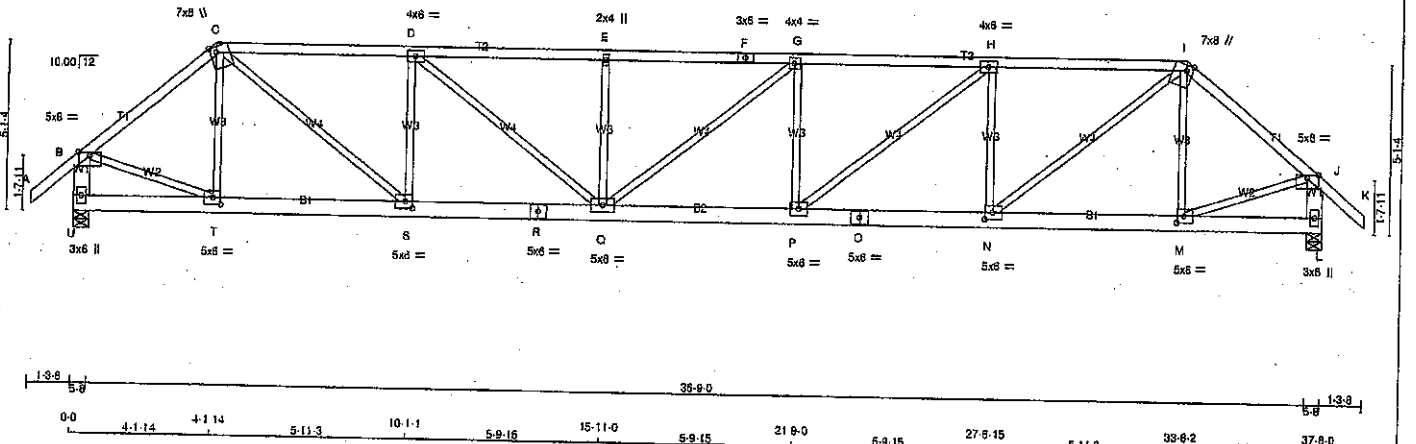
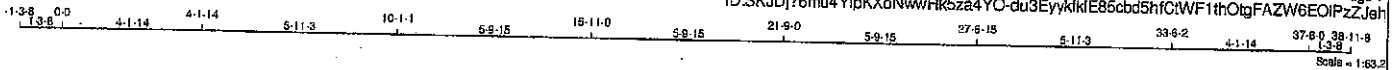
- 1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108012 *9M*

JOB NAME 417375	TRUSS NAME T2	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
I - K	2x4	DRY	No.2	SPF	
U - B	2x6	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	
U - R	2x6	DRY	No.2	SPF	
R - O	2x6	DRY	No.2	SPF	
O - L	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge 2.25	
D	TMVW-l	MT20	4.0	6.0		
E	TMVW-w	MT20	2.0	4.0		
F	TS-l	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	4.0		
H	TMVW-l	MT20	4.0	6.0		
I	TTWW+m	MT20	7.0	8.0	Edge 2.25	
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMVW-l	MT20	5.0	6.0	2.50	2.75
O	BS-l	MT20	5.0	6.0		
P	BMVW-l	MT20	5.0	6.0		
Q	BMVW-w	MT20	5.0	6.0		
R	BS-l	MT20	5.0	6.0		
S	BMVW-t	MT20	5.0	6.0	2.50	2.50
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQ'D
JT	VERT	DOWN	BRG	BRG
U	2204	0	5-8	5-8
L	2204	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
U	1556	1035	0	0	0	521
L	1556	1035	0	0	0	521

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO						FR-TO			
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	T-C	-357 / 0	0.13 (1)	
B-C	-2158 / 0	-91.8	-91.8	0.38 (1)	4.27	M-I	-355 / 0	0.13 (1)	
C-D	-3312 / 0	-91.8	-91.8	0.78 (1)	3.17	B-T	0 / 1720	0.39 (1)	
D-E	-4077 / 0	-91.8	-91.8	0.91 (1)	2.78	M-J	0 / 1720	0.39 (1)	
E-F	-4077 / 0	-91.8	-91.8	0.75 (1)	2.98	N-I	0 / 2125	0.48 (1)	
F-G	-4077 / 0	-91.8	-91.8	0.75 (1)	2.98	C-S	0 / 2125	0.48 (1)	
G-H	-4085 / 0	-91.8	-91.8	0.91 (1)	2.78	N-H	-1224 / 0	0.44 (1)	
H-I	-3310 / 0	-91.8	-91.8	0.77 (1)	3.19	S-D	-1218 / 0	0.44 (1)	
I-J	-2158 / 0	-91.8	-91.8	0.38 (1)	4.27	P-H	0 / 987	0.22 (1)	
J-K	0 / 41	-91.8	-91.8	0.13 (1)	10.00	D-Q	0 / 985	0.22 (1)	
U-B	-2178 / 0	0.0	0.0	0.15 (1)	6.91	P-G	-515 / 0	0.18 (1)	
L-J	-2178 / 0	0.0	0.0	0.15 (1)	6.91	Q-E	-517 / 0	0.19 (1)	
						Q-G	-10 / 0	0.01 (1)	
U-T	0 / 0	-18.5	-18.5	0.06 (4)	10.00				
T-S	0 / 1645	-18.5	-18.5	0.23 (1)	10.00				
S-R	0 / 3313	-18.5	-18.5	0.45 (1)	10.00				
R-Q	0 / 3313	-18.5	-18.5	0.45 (1)	10.00				
Q-P	0 / 4085	-18.5	-18.5	0.54 (1)	10.00				
P-O	0 / 3311	-18.5	-18.5	0.44 (1)	10.00				
O-N	0 / 3311	-18.5	-18.5	0.44 (1)	10.00				
N-M	0 / 1946	-18.5	-18.5	0.24 (1)	10.00				
M-L	0 / 0	-18.5	-18.5	0.06 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/899 (0.24")
ALLOWABLE DEFL.(TL) = L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/985 (0.45")

CSF: TC=0.91/1.00 (D-E:1), BC=0.54/1.00 (P-Q:1), WB=0.48/1.00 (C-S:1), SS=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
		1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

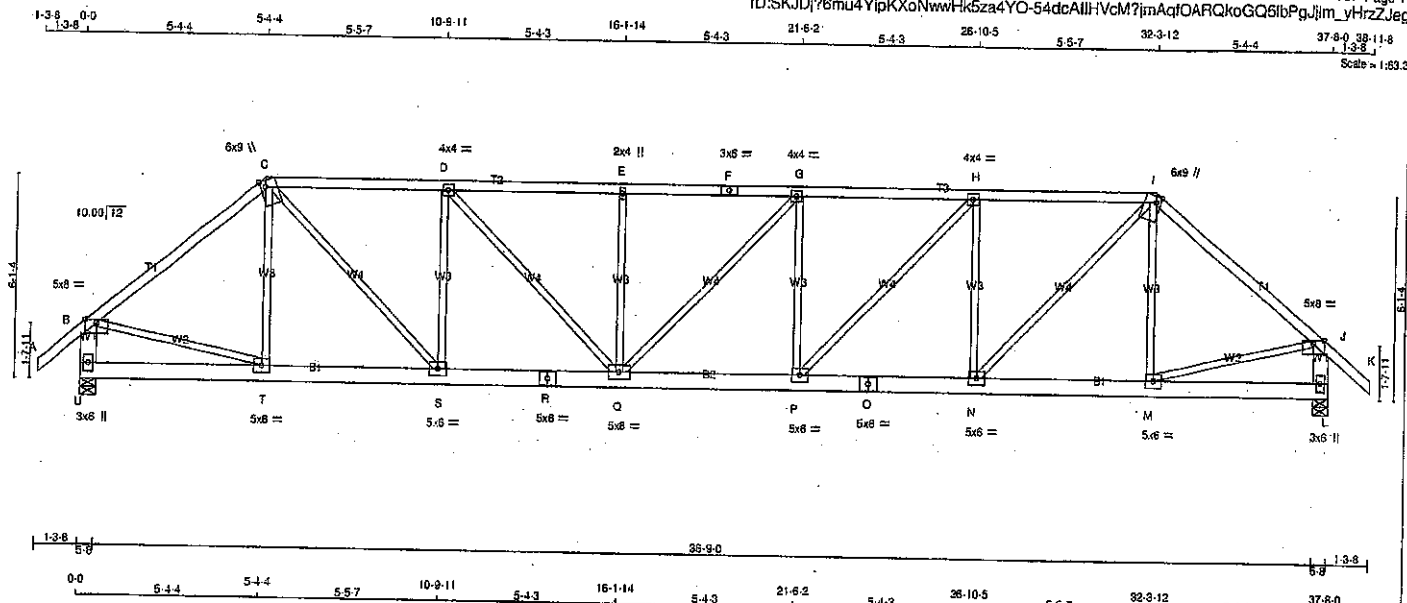
JSI GRIP = 0.90 (N) (INPUT = 0.90)
JSI METAL = 0.64 (R) (INPUT = 1.00)



Structural component only
DWG# T-2108013

JOB NAME 417375	TRUSS NAME T3	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
U - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - O	2x6	DRY	No.2
O - L	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	Edge	
C TTWW-m	MT20	6.0	9.0	Edge	1.75
D, G, H					
D TMVW-l	MT20	4.0	4.0		
E TMW-w	MT20	2.0	4.0		
F TS-t	MT20	3.0	6.0		
I TTWW-m	MT20	6.0	9.0	Edge	1.75
J TMVW-p	MT20	5.0	8.0	Edge	
L BMV1+p	MT20	3.0	6.0		
M, N, P, S, T					
M BMVW-l	MT20	5.0	8.0		
O BS-t	MT20	5.0	8.0		
Q BMVW-w	MT20	5.0	8.0		
R BS-t	MT20	5.0	8.0		
U BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG
U	VERT	DOWN	HORZ	UPLIFT	IN-SX
U	2204	0	2204	0	5-8
L	2204	0	2204	0	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
U	1556	1035.0	0.0	0.0	0.0	521.0	0.0
L	1556	1035.0	0.0	0.0	0.0	521.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FACTORED	MEMB.	FACTORED
FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
PR-TO	FROM	PR-TO	FROM
A-B	0.41	T-C	-281.0
B-C	-2200.0	M-I	-258.0
C-D	-2843.0	B-T	0.1728
D-E	-3373.0	M-J	0.1728
E-F	-3373.0	N-I	0.1679
F-G	-3373.0	C-S	0.1682
G-H	-3380.0	N-H	-1123.0
H-I	-2642.0	S-D	-1117.0
I-J	-2200.0	P-H	0.1788
J-K	0.41	D-Q	0.1777
U-S	-2182.0	P-G	-472.0
L-J	-2169.0	Q-E	-474.0
		Q-G	-9.0
U-T	0.0		
T-S	0.1682		
S-R	0.2844		
R-Q	0.2844		
Q-P	0.3380		
P-O	0.2842		
O-N	0.2842		
N-M	0.1682		
M-L	0.0		

TOTAL WEIGHT = 2 X 187 = 375 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIQ 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 0.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/989 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.33")

CSI: TC=0.67/1.00 (I-J-I), BC=0.45/1.00 (P-Q-I), WB=0.61/1.00 (H-N-I), SS=0.23/1.00 (C-D-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

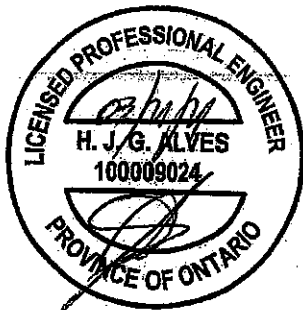
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 766 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.82 (F) (INPUT = 0.90)
JSI METAL= 0.55 (R) (INPUT = 1.00)



Structural component only
DWG# T-2108014

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ID:SKJDj?6mu4YipKXoNwwHk5za4YO-ZGB_NelwGwUsKwID6hgyxKPFV7B844szQ/pHzJel



DRY; SEASONED LUMBER.

PLATES (table is in inches)						
J	T	TYPE	PLATES	W	LEN	Y X
B	T	TMW+P	MT20	3.0	4.0	
C	T	TMWW-I	MT20	5.0	6.0	2.50 2.25
D	T	TMWW+M	MT20	6.0	9.0	Edge 1.75
E	T	TMWW-I	MT20	4.0	4.0	
F	T	TMW+P	MT20	2.0	4.0	
G	T	TS-I	MT20	3.0	6.0	
H	T	TMWW-I	MT20	4.0	4.0	
I	T	TMWW+M	MT20	6.0	9.0	Edge 1.75
J	T	TMWW-I	MT20	5.0	6.0	2.50 2.25
K	T	TMW+P	MT20	3.0	4.0	
L	M	BMWW-I	MT20	5.0	6.0	
N, C, S, T						
N	P	BMWW-I	MT20	5.0	6.0	
P	Q	BS-I	MT20	5.0	6.0	
R	U	BMWW-I	MT20	5.0	6.0	
R	U	BS-I	MT20	5.0	6.0	
U		BMWW-I	MT20	5.0	6.0	

Circumstance	Percentage (%)
If someone is attacking you	85
If someone is threatening you	75
If someone is harassing you	65
If someone is insulting you	55
If someone is annoying you	45

BREAKINGS		FACTORED		MAXIMUM FACTORED		INPUT	RECORD
GROSS REACTION		GROSS REACTION				BRG	BRG
VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX		IN-SX
JT	2204	0	2204	0	0	5-8	5-8
U	2204	0	2204	0	0	5-8	5-8
M	2204	0	2204	0	0	5-8	5-8

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 11 M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.08 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				FACTORED				WEBB			
MEMB.	FORCE	VERT. LOAD	MAX. CSI	MEMB.	FORCE	VERT. LOAD	MAX. CSI	MEMB.	FORCE	VERT. LOAD	MAX. CSI
	(LBS)	(PLF)	(LC)		(LBS)	(PLF)	(LC)		(LBS)	(PLF)	(LC)
FR-TO		FROM	TO	FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	C-T	0 / 168	0.04 (1)			
B-C	0 / 20	-91.8	-91.8	0.14 (1)	10.00	D-D	0 / 78	0.03 (4)			
C-D	-2225 / 0	-91.8	-91.8	0.21 (1)	4.39	D-S	0 / 1434	0.32 (1)			
D-E	-2861 / 0	-91.8	-91.8	0.75 (1)	3.48	S-E	-943 / 0	0.77 (1)			
E-F	-2939 / 0	-91.8	-91.8	0.79 (1)	3.29	E-Q	0 / 415	0.09 (1)			
F-G	-2939 / 0	-91.8	-91.8	0.79 (1)	3.29	Q-H	-514 / 0	0.42 (1)			
G-H	-2939 / 0	-91.8	-91.8	0.79 (1)	3.29	H-H	0 / 415	0.09 (1)			
H-I	-2661 / 0	-91.8	-91.8	0.75 (1)	3.48	O-H	-943 / 0	0.77 (1)			
I-J	-2225 / 0	-91.8	-91.8	0.21 (1)	4.39	O-I	0 / 1434	0.32 (1)			
J-K	0 / 20	-91.8	-91.8	0.14 (1)	10.00	N-I	0 / 78	0.03 (4)			
K-L	0 / 41	-91.8	-91.8	0.13 (1)	10.00	N-J	0 / 168	0.04 (1)			
U-B	-244 / 0	0.0	0.0	0.02 (1)	7.81	U-C	-2476 / 0	0.89 (1)			
M-K	-244 / 0	0.0	0.0	0.02 (1)	7.81	J-M	-2476 / 0	0.89 (1)			
U-T	0 / 1578	-18.5	-18.5	0.24 (1)	10.00						
T-S	0 / 1688	-18.5	-18.5	0.25 (1)	10.00						
S-R	0 / 2661	-18.5	-18.5	0.36 (1)	10.00						
R-Q	0 / 2861	-18.5	-18.5	0.35 (1)	10.00						
Q-P	0 / 2661	-18.5	-18.5	0.36 (1)	10.00						
P-O	0 / 2661	-18.5	-18.5	0.36 (1)	10.00						
C-N	0 / 1688	-18.5	-18.5	0.25 (1)	10.00						
N-M	0 / 1578	-18.5	-18.5	0.24 (1)	10.00						

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00:1.00

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.28")

CSI: TC=0.79/1.00 (F-H:1), BC=0.36/1.00 (Q-S:1),
WB=0.77/1.00 (H-O:1), SSI=0.27/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

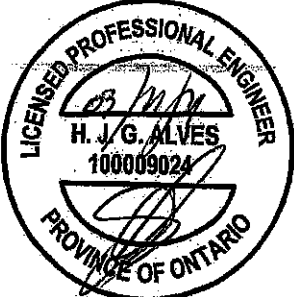
NAIL VALUES	GRIP (DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP= 0.86 (C) (INPUT = 0.90)

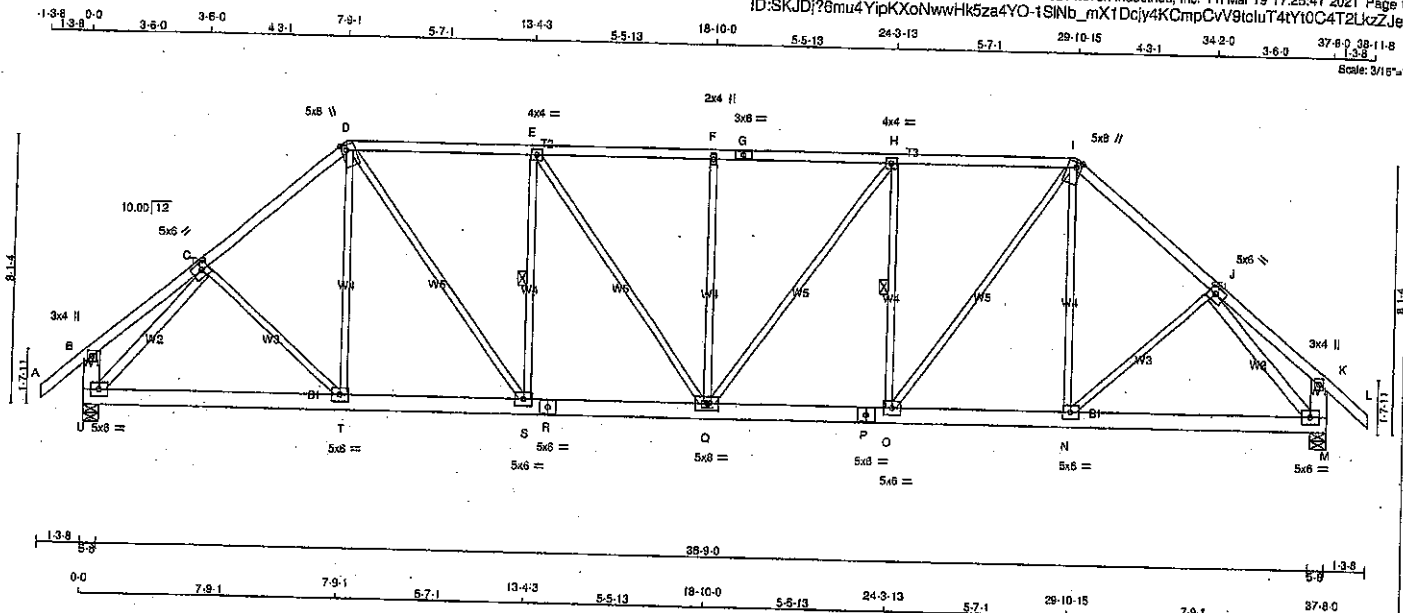
VS1 METAL= 0.55 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108015

JOB NAME 417375	TRUSS NAME T5	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						

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 ID:SKJDJ?6mu4YipKXoNwwHk5za4YO-1SINb_mX1DcJy4KCmpCvV8tclut4tY10C4T2UozZJee
 Scale: 3/16"=1'



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
U - C	2x4	DRY	No.2
J - M	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMVW+1	MT20	5.0	6.0	2.50	2.50
D TMVW+m	MT20	5.0	8.0	2.00	1.75
E TMVW+1	MT20	4.0	4.0		
F TMVW+1	MT20	2.0	4.0		
G TS+1	MT20	3.0	6.0		
H TMVW+1	MT20	4.0	4.0		
J TMVW+m	MT20	5.0	8.0	2.00	1.75
K TMVW+1	MT20	5.0	6.0	2.50	2.50
L TMV+p	MT20	3.0	4.0		
M BMVW+1	MT20	5.0	6.0		
N, O, S, T					
N BMVW+1	MT20	5.0	6.0		
P BS+1	MT20	5.0	6.0		
Q BMVW+1	MT20	5.0	8.0		
R BS+1	MT20	5.0	6.0		
U BMVW+1	MT20	5.0	6.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQ'D BRG
JT	VERT	DOWN	UP	IN-SX	IN-SX
U	2204	0	2204	0	5-8
M	2204	0	2204	0	5-8

UNFACTORED REACTIONS

JT	1ST LOSE	MAX/MIN, COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1556	1035	0	0	0	0	521	0
M	1556	1035	0	0	0	0	521	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-O, E-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 41	C-T	0 / 99
B-C	0 / 31	T-D	0 / 113
C-D	-2204 / 0	N-I	0 / 113
D-E	-2356 / 0	N-J	0 / 99
E-F	-2552 / 0	U-C	-2489 / 0
F-G	-2552 / 0	J-M	-2489 / 0
G-H	-2552 / 0	O-I	0 / 1165
H-I	-2356 / 0	D-S	0 / 1165
I-J	-2204 / 0	C-H	-848 / 0
J-K	0 / 31	S-E	-848 / 0
K-L	0 / 41	Q-H	0 / 338
U-B	-234 / 0	E-Q	0 / 338
M-K	-234 / 0	Q-F	-463 / 0
U-T	0 / 1597		
T-S	0 / 1672		
S-R	0 / 2356		
R-Q	0 / 2356		
Q-P	0 / 2356		
P-O	0 / 2356		
O-N	0 / 1672		
N-M	0 / 1597		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
 BOT CH. DL = 6.0 PSF
 LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA C88-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
 CALCULATED VERT. DEFL.(LL) = L/989 (0.12")
 ALLOWABLE DEFL.(TL) = L/360 (1.26")
 CALCULATED VERT. DEFL.(TL) = L/989 (0.22")

CSI: TC=0.58/1.00 (F-H:1), BC=0.32/1.00 (Q-S:1),
 WB=0.74/1.00 (J-M:1), SS=0.24/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

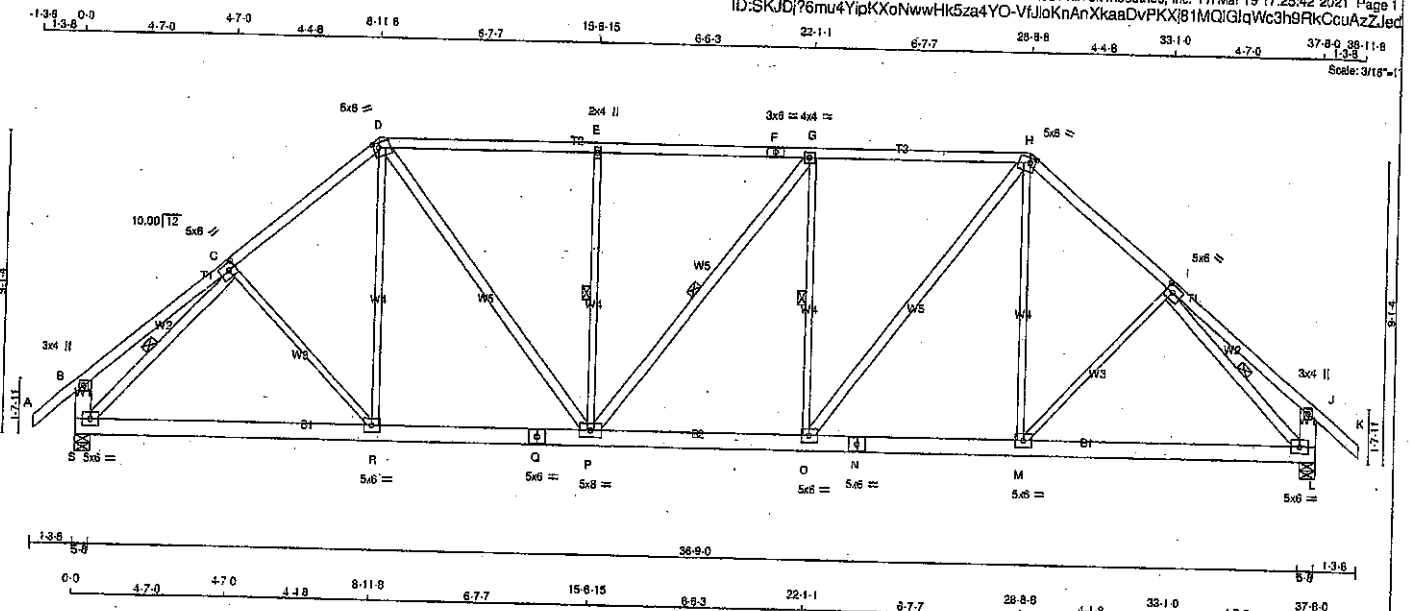
JSI GRIP = 0.89 (D) (INPUT = 0.90)
 JSI METAL = 0.56 (C) (INPUT = 1.00)



Structural component only
 DWG# T-2108016

JOB NAME 417375	TRUSS NAME T6	QUANTITY 8	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:SKJD\76mu4YipKXoNwwHk5za4YO-VfJloKnAnXkaaDvPKX\81MQ\GlqWc3h9RkCcuAzZjed



CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - F	2x4	DRY No.2
F - H	2x4	DRY No.2
H - K	2x4	DRY No.2
S - B	2x6	DRY No.2
L - J	2x6	DRY No.2
S - Q	2x6	DRY No.2
Q - N	2x6	DRY No.2
N - L	2x6	DRY No.2
ALL WEBS	2x3	DRY No.2
EXCEPT		
D - P	2x4	DRY No.2
P - G	2x4	DRY No.2
O - H	2x4	DRY No.2
S - C	2x4	DRY No.2
I - L	2x4	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMVW-1	MT20	5.0	6.0	2.50	2.50
D TTWW-m	MT20	5.0	6.0	1.75	2.00
E TMV-w	MT20	2.0	4.0		
F TS-t	MT20	3.0	6.0		
G TMVW-1	MT20	4.0	4.0		
H TTWW-m	MT20	5.0	6.0	1.75	2.00
I TMVW-1	MT20	5.0	6.0	2.50	2.50
J TMV+p	MT20	3.0	4.0		
L BMVW-1	MT20	5.0	6.0		
M O, R					
N BMVW-1	MT20	5.0	6.0		
M BS-1	MT20	5.0	6.0		
P BMVW-1	MT20	5.0	6.0		
Q BS-1	MT20	5.0	6.0		
S BMVW-1	MT20	5.0	6.0		

NOTES:

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	2204	2204	0	5-8
L VERT	2204	2204	0	5-8
S VERT	2204	2204	0	5-8

UNFACTORED REACTIONS

1ST LOOSE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
L	1556 1035 / 0 0 0 0 521 0 0 0
S	1556 1035 / 0 0 0 0 521 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P, G-P, G-O, O-S, H-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)		(LBS)	CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00	R-D	0 / 205	0.06 (4)
B-C	0 / 31	-91.8	-91.8 0.29 (1)	10.00	D-P	0 / 952	0.15 (1)
C-D	-2171 / 0	-91.8	-91.8 0.42 (1)	4.23	P-E	-652 / 0	0.34 (1)
D-E	-2219 / 0	-91.8	-91.8 0.75 (1)	3.76	P-G	-6 / 0	0.00 (1)
E-F	-2219 / 0	-91.8	-91.8 0.75 (1)	3.74	C-G	-650 / 0	0.34 (1)
F-G	-2219 / 0	-91.8	-91.8 0.75 (1)	3.74	O-H	0 / 959	0.15 (1)
G-H	-2223 / 0	-91.8	-91.8 0.75 (1)	3.74	M-H	0 / 200	0.06 (4)
H-I	-2170 / 0	-91.8	-91.8 0.42 (1)	4.24	C-R	-43 / 42	0.04 (1)
I-J	0 / 31	-91.8	-91.8 0.29 (1)	10.00	S-C	-2492 / 0	0.51 (1)
J-K	0 / 41	-91.8	-91.8 0.13 (1)	10.00	M-I	-43 / 42	0.04 (1)
S-B	-283 / 0	0.0	0.0 0.02 (1)	7.81	I-L	-2491 / 0	0.51 (1)
L-J	-283 / 0	0.0	0.0 0.02 (1)	7.81			

TOTAL WEIGHT = 8 X 213 = 1706 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/960 (1.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/960 (1.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.78/1.00 (G+H:1), BC=0.31/1.00 (O-P:1), WB=0.51/1.00 (C-S:1), SS=0.28/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

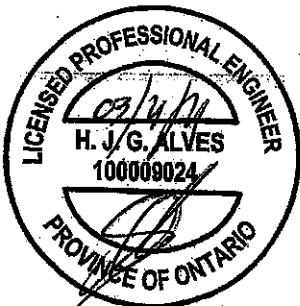
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 Inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.86 (C) (INPUT = 0.90)

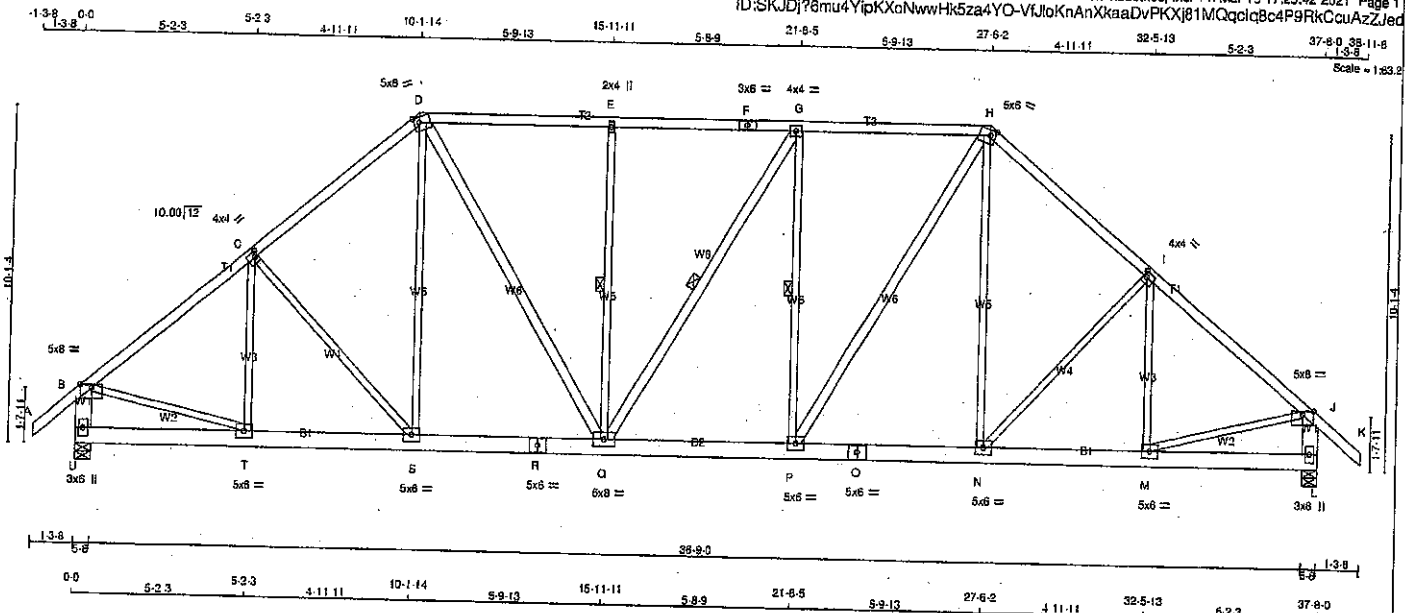
JSI METAL = 0.58 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108017

JOB NAME 417375	TRUSS NAME T7	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 ID:SKJDJ76mu4YipKXoNwwHk5za4YO-VfJtoKnAnXkaaDvPKXJ81MQqlcBc4P9RkCcuAzJed



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - Q	2x4	DRY	No.2	SPF
Q - G	2x4	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	6.0	2.00	2.00
E	TMW-w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.00	2.00
I	TMWW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M, N, P, S, T						
M	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	8.0		
Q	BMWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	8.0		
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)
 1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
U	2204 0	2204 0	5-8	5-8
L	2204 0	2204 0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
U	1556	1035 / 0	0 / 0	0 / 0	521 / 0	0 / 0
L	1556	1035 / 0	0 / 0	0 / 0	521 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, G-Q, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO				FR-TO			
A-B	0 / 41	-91.8	-91.8 0.13 (1)	T-C	-325 / 0	0.17 (1)	
B-C	-2210 / 0	-91.8	-91.8 0.40 (1)	C-S	-199 / 0	0.22 (1)	
C-D	-2111 / 0	-91.8	-91.8 0.38 (1)	S-D	0 / 265	0.06 (1)	
D-E	-1995 / 0	-91.8	-91.8 0.41 (1)	D-Q	0 / 783	0.13 (1)	
E-F	-1995 / 0	-91.8	-91.8 0.38 (1)	Q-E	-571 / 0	0.38 (1)	
F-G	-1995 / 0	-91.8	-91.8 0.38 (1)	Q-G	-8 / 0	0.01 (1)	
G-H	-2110 / 0	-91.8	-91.8 0.42 (1)	P-G	-568 / 0	0.38 (1)	
H-I	-2110 / 0	-91.8	-91.8 0.38 (1)	P-H	0 / 793	0.13 (1)	
I-J	-2211 / 0	-91.8	-91.8 0.40 (1)	N-H	0 / 257	0.06 (1)	
J-K	0 / 41	-91.8	-91.8 0.13 (1)	N-I	-201 / 0	0.22 (1)	
U-B	-2154 / 0	0.0	0.0 0.15 (1)	M-I	-323 / 0	0.17 (1)	
L-J	-2154 / 0	0.0	0.0 0.15 (1)	B-T	0 / 1772	0.40 (1)	
				M-J	0 / 1772	0.40 (1)	
U-T	0 / 0	-18.5	-18.5 0.05 (4)				
T-S	0 / 1725	-18.5	-18.5 0.24 (1)				
S-R	0 / 1593	-18.5	-18.5 0.22 (1)				
R-Q	0 / 1593	-18.5	-18.5 0.22 (1)				
Q-P	0 / 1999	-18.5	-18.5 0.22 (1)				
P-O	0 / 1592	-18.5	-18.5 0.22 (1)				
O-N	0 / 1592	-18.5	-18.5 0.22 (1)				
N-M	0 / 1725	-18.5	-18.5 0.23 (1)				
M-L	0 / 0	-18.5	-18.5 0.05 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 8.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
 ALLOWABLE DEFL.(TL) = L/360 (1.26")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.42/1.00 (G-H:1), BC=0.27/1.00 (P-Q:1), WB=0.40/1.00 (J-M:1), SSI=0.25/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTO SOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

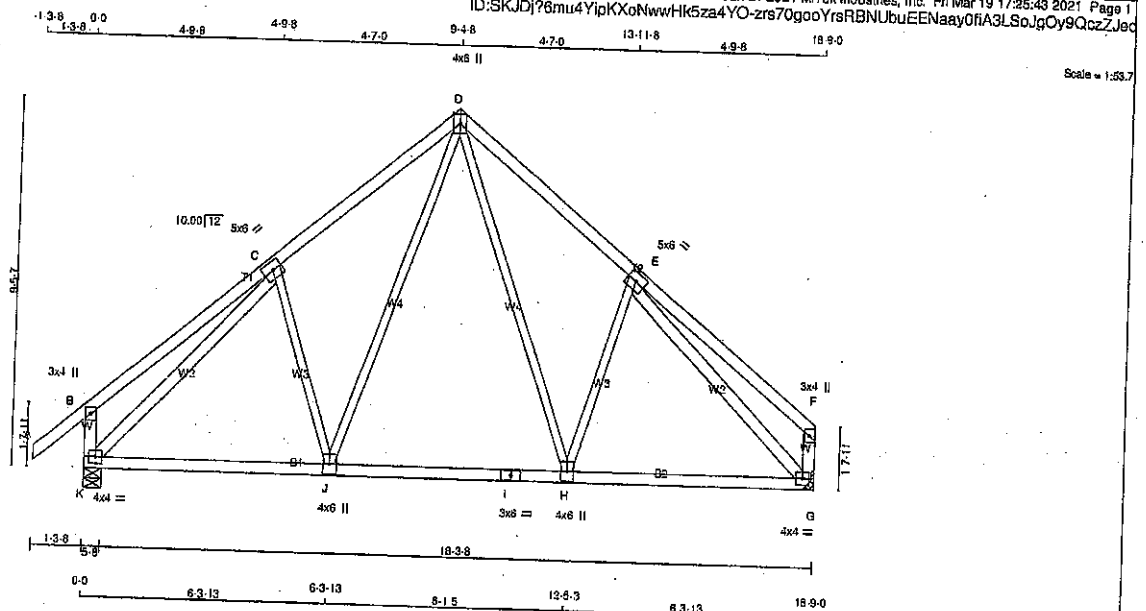
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
 JSI METAL= 0.40 (D) (INPUT = 1.00)



Structural component only
 DWG# T-2108018

JOB NAME 417375 Tamarack Roof Truss, Burlington	TRUSS NAME T8	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4 DRY	No.2		
D - F	2x4 DRY	No.2		
K - B	2x4 DRY	No.2		
G - I	2x4 DRY	No.2		
I - G	2x4 DRY	No.2		
ALL WEBS	2x3 DRY	No.2		
EXCEPT				
K - C	2x4 DRY	No.2		
E - G	2x4 DRY	No.2		

DRY-SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A - D	TMV+p	MT20	3.0	4.0			
D - F	TMVW+I	MT20	5.0	6.0			
K - B	TTVW+p	MT20	4.0	6.0			Edge
G - I	TMVW+I	MT20	5.0	6.0			
I - G	TMV+p	MT20	3.0	4.0			
K - C	BMVW+I	MT20	4.0	4.0			
E - G	BMVW+I	MT20	4.0	6.0			
	BS+I	MT20	3.0	6.0			
	BMVW+I	MT20	4.0	6.0			
	BMVW+I	MT20	4.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
K	1181	0	0	5-8
G	1034	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

1ST CASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT							
K	818	551	0	0	0	268	0
G	731	480	0	0	0	251	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	FORCE	MAX
			(LBS)	(PLF)							(LBS)	(LC)
FR-TO									FR-TO			
A-B	0:41	-91.8	-91.8	0.13 (1)	10.00	D-H	0:400	0.09 (1)				
B-C	0:33	-91.8	-91.8	0.38 (1)	10.00	H-E	-267:0	0.14 (1)				
C-D	-887:0	-91.8	-91.8	0.27 (1)	6.20	J-D	0:400	0.09 (1)				
D-E	-887:0	-91.8	-91.8	0.27 (1)	6.20	C-J	-267:0	0.14 (1)				
E-F	0:33	-91.8	-91.8	0.38 (1)	10.00	K-C	-1108:0	0.71 (1)				
K-B	-287:0	0.0	0.0	0.03 (1)	7.81	E-G	-1109:0	0.71 (1)				
G-F	-180:0	0.0	0.0	0.02 (1)	7.81							
K-J	0:740	-18.5	-18.5	0.22 (4)	10.00							
J-I	0:529	-18.5	-18.5	0.21 (4)	10.00							
I-H	0:529	-18.5	-18.5	0.21 (4)	10.00							
H-G	0:740	-18.5	-18.5	0.22 (4)	10.00							

TOTAL WEIGHT = 3 X 93 = 279 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TO=0.33/1.00 (B-C:1), BC=0.22/1.00 (J-K:4), WB=0.71/1.00 (C-K:1), SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.86 (K) (INPUT = 0.90)
JSI METAL = 0.27 (C) (INPUT = 1.00)

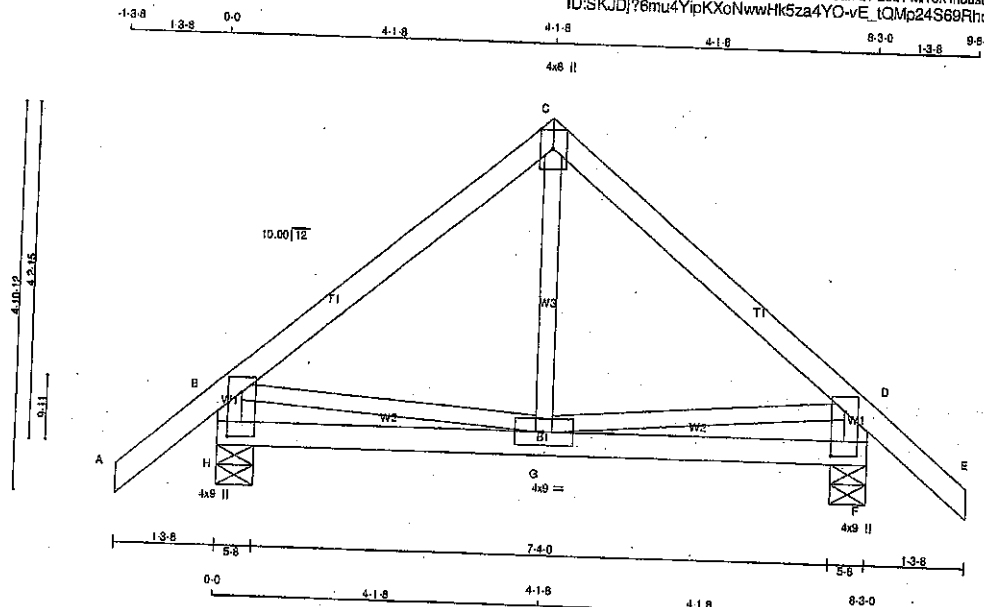


Structural component only
DWG# T-2108019

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417375	T10	1	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington					

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Scale = 1/2" = 1'-0"



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2	
C - E	2x4	DRY	No.2	
H - B	2x4	DRY	No.2	
F - D	2x4	DRY	No.2	
H - F	2x4	DRY	No.2	

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B, D, F, H					
C	TTW+P MT20	4.0	6.0	Edge	
F	TMBMWV1+PMT20	4.0	9.0	6.75	2.00
G	BMVWV1 MT20	4.0	9.0		
H	TMBMWV1+PMT20	4.0	9.0	6.75	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ
H	717	0	717	0
F	915	0	915	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS	DEAD	SOIL
H	511	315	0	0	0
F	656	381	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLINE SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO						FR-TO			
A-B	0.41	-91.8	-91.8	0.14 (1)	10.00	G-C	0.325	0.11 (4)	
B-C	-549.70	-91.8	-91.8	0.31 (1)	6.25	B-G	0.425	0.11 (1)	
C-D	-549.70	-91.8	-91.8	0.31 (1)	6.25	G-D	0.425	0.11 (1)	
D-E	0.41	-91.8	-91.8	0.14 (1)	10.00				
H-B	-718.70	0.0	0.0	0.08 (1)	7.81				
F-D	-718.70	0.0	0.0	0.08 (1)	7.81				
H-G	0.0	-18.5	-18.5	0.28 (4)	10.00				
G-F	0.0	-185.2	-79.0	0.32 (4)	10.00				

TOTAL WEIGHT = 36 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL	8.0	PSF
BOT CH.	LL = 0.0	PSF
DL	7.4	PSF
TOTAL LOAD	= 38.0	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.28")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.28")
CALCULATED VERT. DEFL. (TL) = L/999 (0.07")

CSI: TC=0.31/1.00 (B-C:1), BC=0.32/1.00 (F-G:4), WB=0.11/1.00 (C-G:4), SS=0.35/1.00 (F-G:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

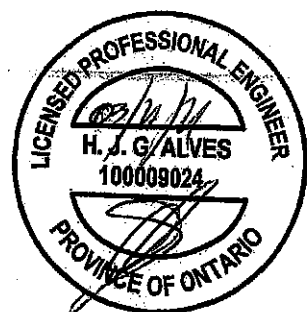
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)	(PL)
MT20	850	371	1747	788
				1987

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.48 (H) (INPUT = 0.90)
JSI METAL = 0.31 (F) (INPUT = 1.00)

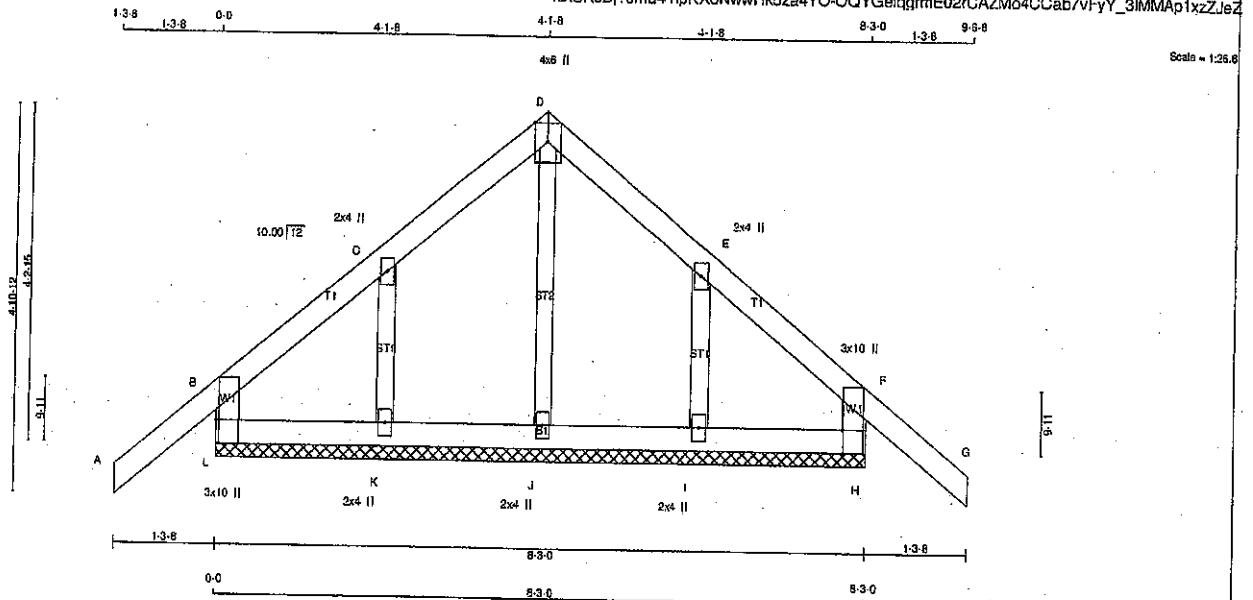


Structural component only
DWG# T-2108021

JOB NAME 417375	TRUSS NAME T10G	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MTek Industries, Inc. Fri Mar 19 17:25:46 2021 Page 1
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TOTAL WEIGHT = 33 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B, F, H, L					
C	TMW+w	MT20	2.0	4.0	
D	TTW+p	MT20	4.0	6.0	Edge
E	TMW+w	MT20	2.0	4.0	
F	TMBMV1+p	MT20	3.0	10.0	Edge
I, J, K					
L	BMW1+w	MT20	2.0	4.0	
L	TMBMV1+p	MT20	3.0	10.0	Edge 0.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	UNBRACED LENGTH
FR-TO		FROM TO		FR-TO			
L-B	-254 / 0	0.0 0.0	0.02 (1)	J-D	-141 / 0	0.04 (1)	
A-B	0 / 41	-91.8 -91.8	0.13 (1)	K-C	-181 / 0	0.03 (1)	
B-C	-52 / 0	-91.8 -91.8	0.04 (1)	I-E	-181 / 0	0.03 (1)	
C-D	-41 / 0	-91.8 -91.8	0.05 (1)				
D-E	-41 / 0	-91.8 -91.8	0.05 (1)				
E-F	-52 / 0	-91.8 -91.8	0.04 (1)				
F-G	0 / 41	-91.8 -91.8	0.13 (1)				
H-F	-254 / 0	0.0 0.0	0.02 (1)				
L-K	0 / 40	-18.5 -18.5	0.02 (4)				
K-J	0 / 32	-18.5 -18.5	0.02 (4)				
J-I	0 / 32	-18.5 -18.5	0.02 (4)				
I-H	0 / 40	-18.5 -18.5	0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TO=0.13/1.00 (A-B:1), BC=0.02/1.00 (K-L:4), WB=0.04/1.00 (D-J:1), SS=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

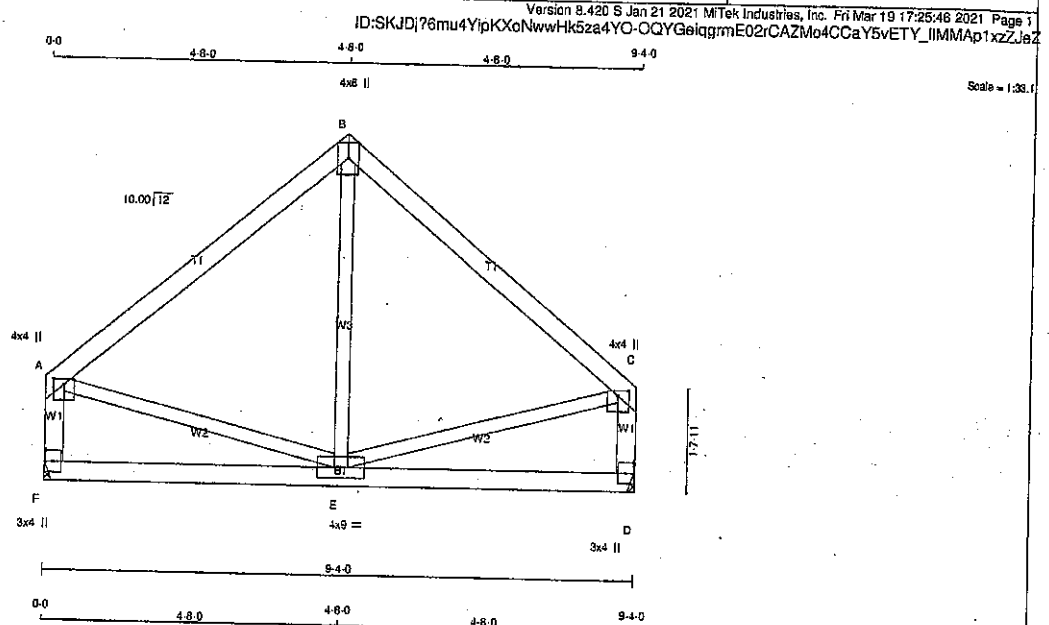
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.13 (C) (INPUT = 0.90)
JSI METAL = 0.10 (L) (INPUT = 1.00)



Structural component only
DWG# T-2108022

JOB NAME 417375	TRUSS NAME T11	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - B 2x4 DRY No.2

B - C 2x4 DRY No.2

F - A 2x4 DRY No.2

D - C 2x4 DRY No.2

F - D 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

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DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REQD

JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX

F 515 0 515 0 0 MECHANICAL

D 515 0 515 0 0 MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

1ST CASE MAX. MIN. COMPONENT REACTIONS

JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL

F 384 239 0 0 0 0 125 0 0 0

D 384 239 0 0 0 0 125 0 0 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MAX. FACTORED FORCE VERT. LOAD LC1 MAX

MEMB. (LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO

FR-TO A-B -313 0 -91.8 -91.8 0.26 (1) 6.25 E-B -28 70 0.02 (4)

B-C -313 0 -91.8 -91.8 0.26 (1) 6.25 A-E 0 249 0.05 (1)

F-A -481 0 0.0 0.0 0.05 (1) 7.81 E-C 0 249 0.08 (1)

D-C -481 0 0.0 0.0 0.05 (1) 7.81

P-E 0 0 -18.5 -18.5 0.11 (4) 10.00

E-D 0 0 -18.5 -18.5 0.11 (4) 10.00

WEBS

MAX. FACTORED FORCE MAX

MEMB. (LBS) CSI (LC)

FR-TO A-B -313 0 -91.8 -91.8 0.26 (1) 6.25 E-B -28 70 0.02 (4)

B-C -313 0 -91.8 -91.8 0.26 (1) 6.25 A-E 0 249 0.05 (1)

F-A -481 0 0.0 0.0 0.05 (1) 7.81 E-C 0 249 0.08 (1)

D-C -481 0 0.0 0.0 0.05 (1) 7.81

P-E 0 0 -18.5 -18.5 0.11 (4) 10.00

E-D 0 0 -18.5 -18.5 0.11 (4) 10.00

TOTAL WEIGHT = 39 LB

[M/F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019

- PART 9 OF CBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.31")

CALCULATED VERT. DEFL. (LL) = L/999 (0.00")

ALLOWABLE DEFL. (TL) = L/360 (0.31")

CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.28/1.00 (A-B:1), BC=0.11/1.00 (E-F:4),

WB=0.08/1.00 (C-E:1), SS=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (C) (INPUT = 0.90)

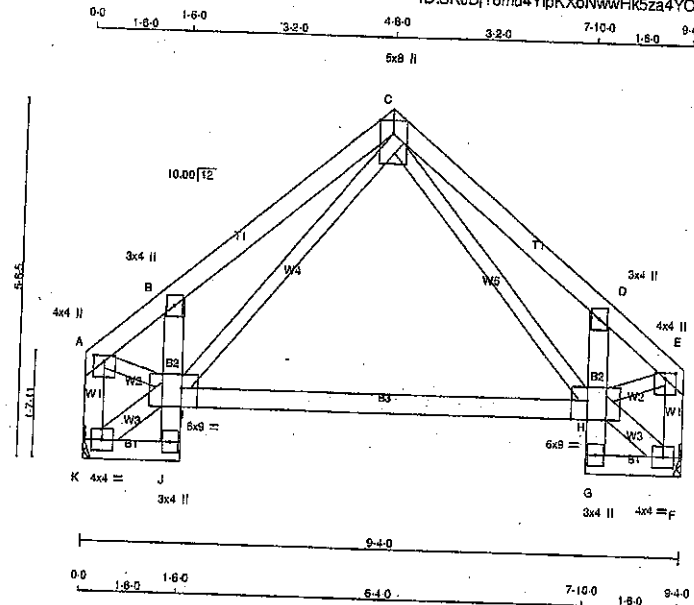
JSI METAL= 0.11 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108023

JOB NAME 417375 Tamarack Roof Truss, Burlington	TRUSS NAME T11S	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Mar 19 17:25:47 2021 Page 1
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Scale = 1/32"

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4 DRY	No.2
C - E	2x4 DRY	No.2
K - A	2x4 DRY	No.2
F - E	2x4 DRY	No.2
K - J	2x4 DRY	No.2
J - B	2x4 DRY	No.2
I - H	2x4 DRY	No.2
G - D	2x4 DRY	No.2
G - F	2x4 DRY	No.2

ALL WEBS	2x3 DRY	No.2
EXCEPT		
K - I	2x4 DRY	No.2
H - F	2x4 DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p MT20	4.0	4.0	1.00	2.00
B	TMV+p MT20	3.0	4.0		
C	TTWV+p MT20	3.0	8.0	Edge	
D	TMV+p MT20	3.0	4.0		
E	TMVW+p MT20	4.0	4.0	1.00	2.00
F	BMVW+1 MT20	4.0	4.0		
G	BMV+p MT20	3.0	4.0		
H	BMVWVW-1 MT20	5.0	9.0	Edge	3.00
I	BMVWVW-1 MT20	5.0	9.0	Edge	3.00
J	BMV+p MT20	3.0	4.0		
K	BMVW+1 MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES:

- (1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT REQRD BRG	REQRD BRG
K	515 0	515 0	0 0	0 0
F	515 0	515 0	0 0	0 0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K, F. MINIMUM BEARING LENGTH AT JOINT K = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	364	239 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0
F	364	239 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (PLF)
A-B		-509 / 0	-91.8	-91.8 0.09 (1)	8.25	0 / 266	0.06 (1)
B-C		-553 / 0	-91.8	-91.8 0.13 (1)	8.25	0 / 266	0.06 (1)
C-D		-553 / 0	-91.8	-91.8 0.13 (1)	8.25	0 / 266	0.06 (1)
D-E		-509 / 0	-91.8	-91.8 0.09 (1)	8.25	0 / 266	0.06 (1)
K-A		-492 / 0	0.0	0.0 0.05 (1)	7.81	0 / 448	0.10 (1)
F-E		-492 / 0	0.0	0.0 0.05 (1)	7.81	0 / 448	0.10 (1)
K-J		0 / 16	-18.5	-18.5 0.01 (4)	10.00		
J-I		0 / 14	0.0	0.0 0.02 (1)	10.00		
I-B		-295 / 0	0.0	0.0 0.01 (1)	7.81		
I-H		0 / 252	-18.5	-18.5 0.25 (4)	10.00		
G-H		0 / 14	0.0	0.0 0.02 (1)	10.00		
H-D		-295 / 0	0.0	0.0 0.01 (1)	7.81		
G-F		0 / 16	-18.5	-18.5 0.01 (4)	10.00		

TOTAL WEIGHT = 2 X 46 = 91 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

155 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.13/1.00 (B-C-1), BC=0.25/1.00 (H-I-1), WB=0.10/1.00 (A-I-1), SS=0.11/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1887 1873

PLATE PLACEMENT TOL. = 0.250 inches

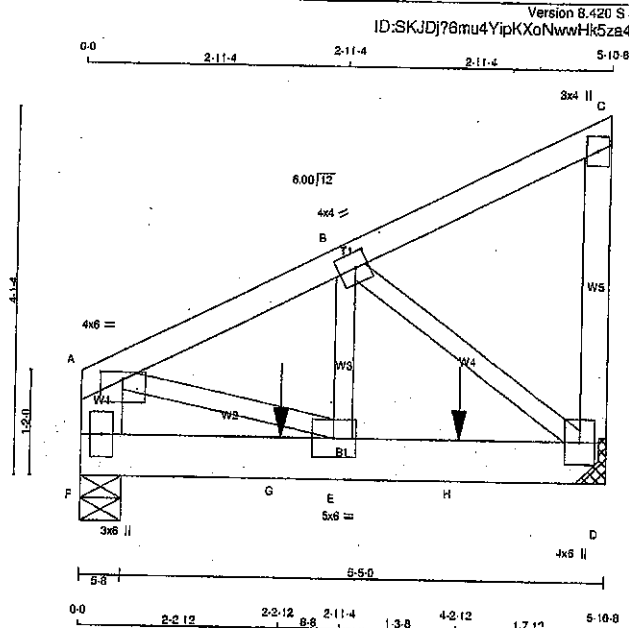
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (E) (INPUT = 0.99)
JSI METAL= 0.14 (A) (INPUT = 1.00)



Structural component only
DWG# T-2108024

JOB NAME 417375	TRUSS NAME T12	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - A	2x6	DRY	No.2
F - D	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1	12	TOP
C-D 1	12	TOP
F-A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMVW-l	MT20	4.0	4.0	2.00	1.75
C	TMV-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQ'D
JT	VERT	HORZ	DOWN	UP/LIFT
D	1440	0	1440	0
F	1238	0	1238	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	1016	678	0	0	0	339	0
F	874	582	0	0	0	292	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSF (L/C)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSF (L/C)	
FR-TO		FROM TO		FR-TO			
A-B	-1421	-91.8	-91.8 0.06 (1)	E-B	0	0.1302	0.16 (1)
B-C	-10	-91.8	-91.8 0.05 (1)	B-D	-1611	0.19 (1)	
D-C	-111	0	0.0 0.01 (1)	A-E	0	0.1324	0.16 (1)
F-A	-1089	0	0.0 0.04 (1)				
F-G	0	-18.5	-18.5 0.13 (1)				
G-E	0	-18.5	-18.5 0.13 (1)				
E-H	0/1281	-18.5	-18.5 0.29 (1)				
H-D	0/1281	-18.5	-18.5 0.29 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-2-12	-716	-716		BACK	VERT	TOTAL		C1
H	4-2-12	-716	-716		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.02")

CSF: TO=0.06/1.00 (A-B:1), BC=0.29/1.00 (D-E:1), WB=0.19/1.00 (B-D:1), SS=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

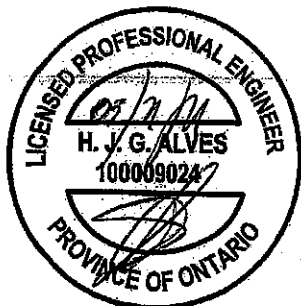
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1887	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (B) (INPUT = 0.90)
JSI METAL = 0.29 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108025

CONTINUED ON PAGE 2

JOB NAME 417375	TRUSS NAME T12	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
D BMWt+p	MT20	4.0	6.0		
E BMW-t	MT20	5.0	6.0		
F BMW+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

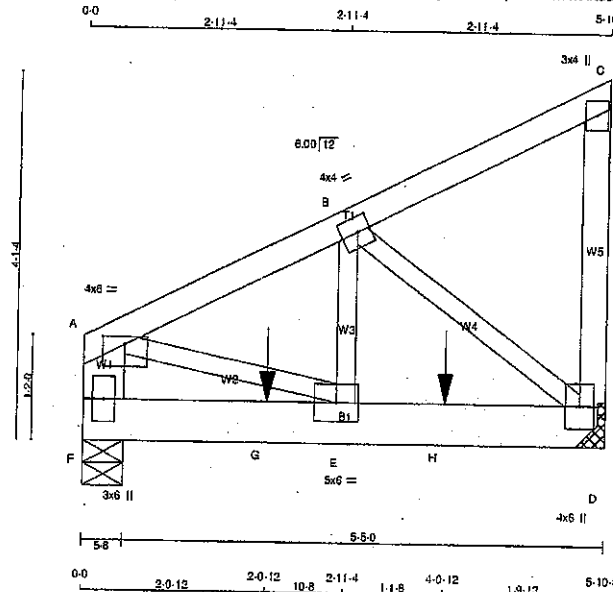


Structural component only
DWG# T-2108025 712

JOB NAME 417375	TRUSS NAME T12Z	QUANTITY 2	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/23.5

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
D - C	2x4	DRY	No.2	SPF		
F - A	2x6	DRY	No.2	SPF		
F - D	2x6	DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD/PLF
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - D	12	TOP
F - A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE (L.O.)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	8.0	1.00	3.00
B	TMVW-l	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
D	841	0	841	0	0	0
F	799	0	799	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LOASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	593	399	0	0	0	0	0
F	563	379	0	0	0	194	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8-25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10-00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)
FR-TO				
A-B	-835	-91.8	825	82.5
B-C	-12	-91.8	825	82.5
D-C	-107	0.0	7.81	7.81
F-A	-686	0.0	7.81	7.81
F-G	0	-18.5	10.00	10.00
G-E	0	-18.5	10.00	10.00
E-H	0	-18.5	10.00	10.00
H-D	0	-18.5	10.00	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-349	-349	---	FRONT	VERT	TOTAL	---	C1
H	4-0-12	-349	-349	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 29 = 117 lb

DESIGN CRITERIA

SPECIFIED CRITERIA:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.08/1.00 (A-B:1), BC=0.14/1.00 (D-E:1),
WB=0.11/1.00 (B-D:1), SS=0.15/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PS)	SECTION (PLI)
MT20	850	371
	1747	789
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.41 (B) (INPUT = 0.90)
JSI METAL = 0.17 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108026 1/2

CONTINUED ON PAGE 2

JOB NAME 417375	TRUSS NAME T12Z	QUANTITY 2	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri Mar 19 17:25:49 2021 Page 2 ID:SKJDI76mu4YipKXoNwwHk5za4YO-o7EOGjsZ8hcbvixiEVLnqrC7L7EIKbB2KPTeGzZJeW	

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
D BM/W1+p	MT20	4.0	6.0		
E BM/W1-t	MT20	5.0	6.0		
F BM/W1+p	MT20	3.0	6.0		

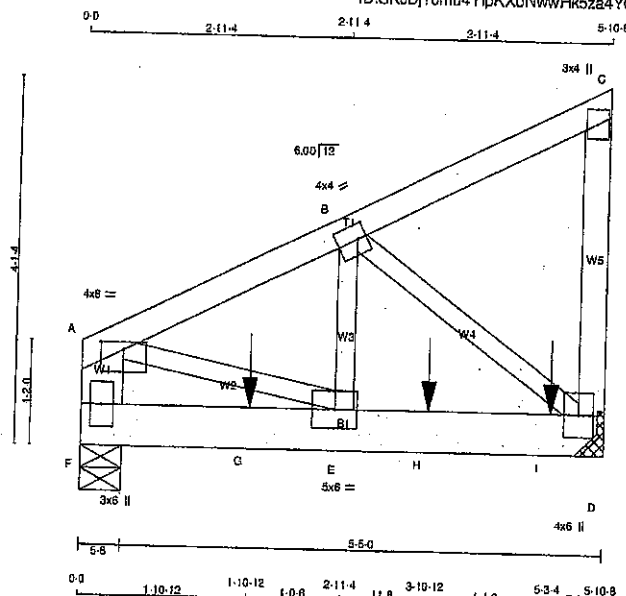
NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108026 *ml*

JOB NAME 417406	TRUSS NAME T12Z2	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri Mar 19 17:34:13 2021 Page 1
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Scale = 1/23.5

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4 DRY	No.2	SPF
D - C	2x4 DRY	No.2	SPF
F - A	2x6 DRY	No.2	SPF
F - D	2x6 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD/PLF
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C 1	12	TOP
C-D 1	12	TOP
F-A 2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE(0.0)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00 3.00
B	TMVW-t	MT20	4.0	4.0	2.00 1.25
C	TMV-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG	IN-SX	IN-SX
D	2979	0	2979	0	0	5-8	5-8		
F	1506	0	1506	0	0	5-8	5-8		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	2102	1402	0	0	0	700	0
F	1063	709	0	0	0	351	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED MAX. FORCE (LBS)
FR-TO			
A-B	-1778	-91.8	-91.8
B-C	-9	-91.8	-91.8
D-C	-114	0.0	0.0
F-A	-1335	0.0	0.0
F-G	0	-18.5	-18.5
G-E	0	-18.5	-18.5
E-H	0/1599	-18.5	-18.5
H-I	0/1599	-18.5	-18.5
I-D	0/1599	-18.5	-18.5

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	FACE	DIR.	TYPE	HEEL	CONN.
G	1-10-12	-687	-687	---	FRONT	VERT	TOTAL	C1
H	3-10-12	-687	-687	---	FRONT	VERT	TOTAL	C1
I	5-3-4	-1333	-1333	---	FRONT	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.06/1.00 (A-B:1), BC=0.47/1.00 (D-E:1), WB=0.24/1.00 (B-D:1), SSI=0.55/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PL)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	650	371	1747
			1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.82 (B) (INPUT = 0.90)
JSI METAL = 0.38 (D) (INPUT = 1.00)



Structural component only.
DWG# T-2108030

JOB NAME 417406	TRUSS NAME T12Z2	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

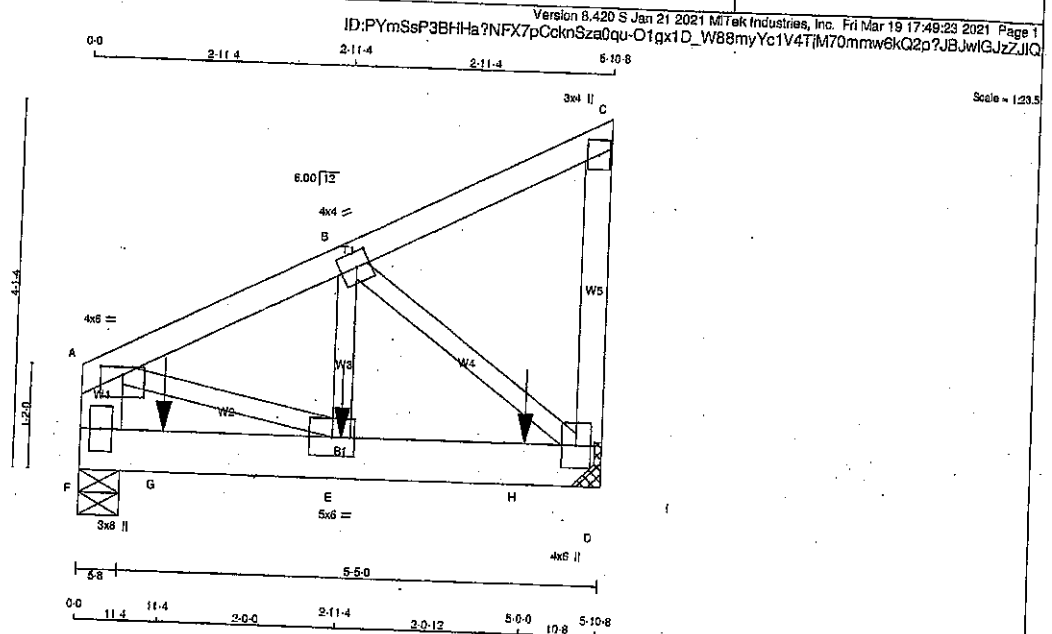
JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW1-l	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108030 772

JOB NAME 417413 Tamarack Roof Truss, Burlington	TRUSS NAME T12Z3	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - A	2x6	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1	12	TOP
C-D 1	12	TOP
F-A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 3	5	SIDE (1094.2)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW-p	MT20	4.0	6.0	1.00	3.00
B TMVW-l	MT20	4.0	4.0	2.00	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
D	4108	0	4108	0	5-8	5-8
F	1337	0	1337	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
D	2894	1960	0	0	0	935	0
F	938	660	0	0	0	277	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO		
A-B	-1821	-91.8	-91.8 0.04 (1)	6.25	E-B	0 / 1757
B-C	-97	-91.8	-91.8 0.03 (1)	10.00	B-D	-2059
D-C	-114	0.0	0.0 0.01 (1)	7.81	A-E	0 / 1893
F-A	-1364	0.0	0.0 0.03 (1)	7.81		
F-G	0 / 0	-18.5	-18.5 0.15 (1)	10.00		
Q-E	0 / 0	-18.5	-18.5 0.15 (1)	10.00		
E-H	0 / 1637	-18.5	-18.5 0.80 (1)	10.00		
H-D	0 / 1637	-18.5	-18.5 0.60 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-11.4	-239	-239	---	TOP	VERT	TOTAL	---	C1
G	11-4	-177	-177	---	TOP	VERT	TOTAL	---	C1
H	5-0-0	-2958	-2958	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 3 X 29 = 88 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CS1: TO=0.04/1.00 (A-B:1), BC=0.60/1.00 (D-E:1), WB=0.15/1.00 (B-D:1), SS=0.55/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

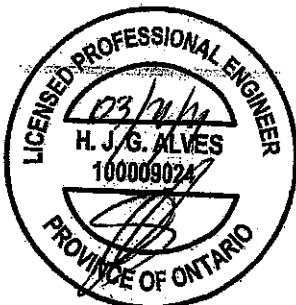
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	850	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.76 (B) (INPUT = 0.90)
JSI METAL = 0.25 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108047

JOB NAME 417413	TRUSS NAME T12Z3	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	TRUSS DESG.	DRWG NO.
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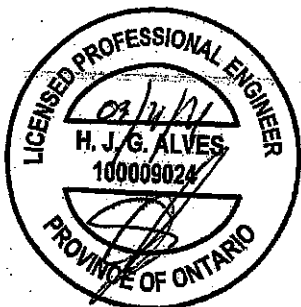
Tamarack Roof Truss, Burlington

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ID:PYmSaP3BHha?NFX7pCcknSza0qu-O1gx1D W88myYc1V4TjM70mmw6kQ2p?JBjWIGJzZJIQ

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
C TMV+p	MT20	3.0	4.0		
D BMVW1+p	MT20	4.0	6.0		
E BMVW1	MT20	5.0	6.0		
F BMV1+p	MT20	3.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108047 *in*

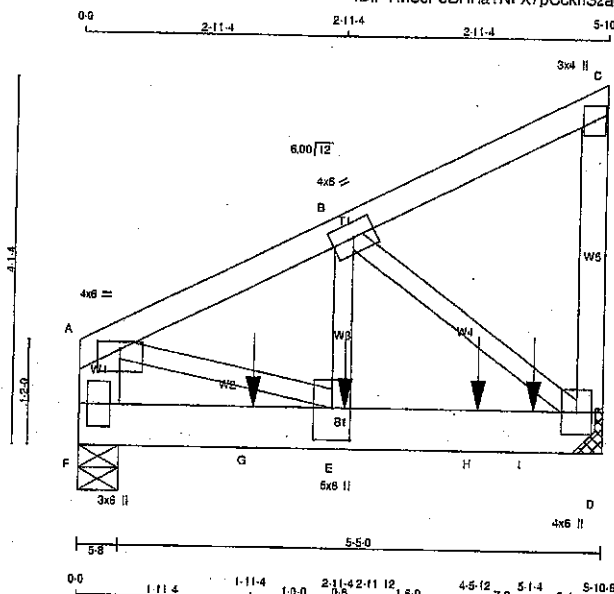
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417413	T12Z4	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:PYmSsP3BHf-a?NFX7pCoknSza0qu-sDEKFZ_8vRupAlciaAEbgEJwAW3PnESTQzgsolzZJIP

Scale = 1:23.5



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD/PLF
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12"	TOP
C - D	12"	TOP
F - A	12"	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12"	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
B - E	6"	SIDE(119.7)
2x3	6"	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW-p	MT20	4.0	6.0	1.00	3.00
B TMVW-w	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
D	3250	0	3250	0
F	1483	0	1483	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	2267	1569	0	0	0	718	0
F	1041	725	0	0	0	317	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO				FR-TO		
A-B	-2001	0	-91.8	-91.8	0.07 (1)	E-B	0	0.24 (1)
B-C	-9	0	-91.8	-91.8	0.05 (1)	B-D	-2262	0.27 (1)
D-C	-116	0	0	0	0.01 (1)	A-E	0	0.23 (1)
F-A	-1488	0	0	0	0.05 (1)			
F-G	0	0	-18.5	-18.5	0.13 (1)			
G-E	0	0	-18.5	-18.5	0.13 (1)			
E-H	0	1798	-18.5	-18.5	0.81 (1)			
H-I	0	1798	-18.5	-18.5	0.81 (1)			
I-D	0	1798	-18.5	-18.5	0.81 (1)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIRL	TYPE	HEEL	CONN.
E	2-11-12	-815	-815		BACK	VERT	TOTAL		C1
G	1-11-4	-178	-178		TOP	VERT	TOTAL		C1
H	4-5-12	-369	-369		BACK	VERT	TOTAL		C1
I	5-1-4	-1508	-1508		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TO=0.07/1.00 (A-B:1), BC=0.61/1.00 (D-E:1), WB=0.27/1.00 (B-D:1), SSI=0.58/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	850	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (D) (INPUT = 0.90)
JSI METAL = 0.40 (D) (INPUT = 1.00)



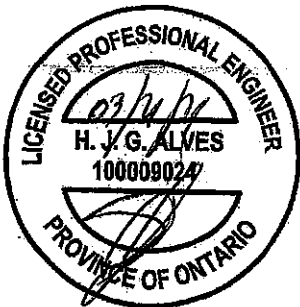
Structural component only
DWG# T-2108048

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRAWG NO.
417413	T12Z4	1	2	TRUSS DESC.		
Tamarack Roof Truss, Burlington						
Version: 8.420 S Jan 21 2021 MITek Industries, Inc. Fri Mar 19 17:49:24 2021 Page 2 ID:PYmSsP3BHla7NFX7pCcknSza0gu-sDEKFZ 8vRUpAlcieAEbgEjwAW3PnESTOzqsolzZJP						

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW1+	MT20	5.0	8.0	4.25	2.50
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



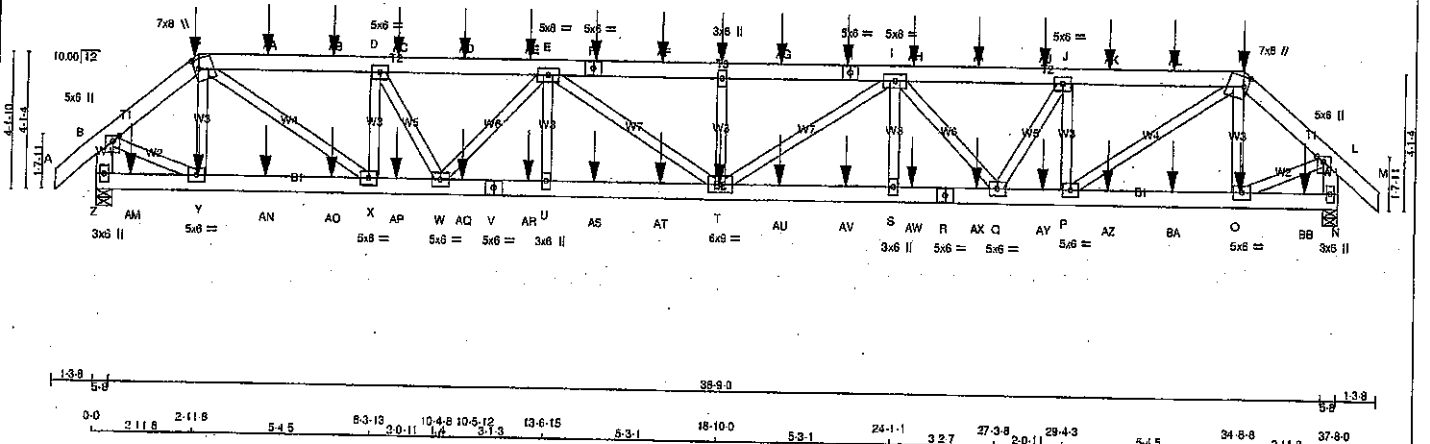
Structural component only
DWG# T-2108048 *7/2*

JOB NAME 417406	TRUSS NAME T14	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mittek Industries, Inc. Fri Mar 19 17:34:16 2021 Page 1
ID:SKJDJ?6mu4YpKXoNwwHk5za4YO_LohTk_y29Nod3d7umQRKY9Yh5q7J8pgP2zF0xzZJwC

Scale: 3/16"=1'



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2
C - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - K	2x6	DRY	No.2
K - M	2x6	DRY	No.2
Z - B	2x6	DRY	No.2
N - L	2x6	DRY	No.2
V - V	2x6	DRY	No.2
V - R	2x6	DRY	No.2
R - N	2x6	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 2	12	SIDE(122.0)
C-F 2	12	SIDE(81.0)
F-H 2	12	SIDE(183.1)
H-K 2	12	SIDE(81.0)
K-M 2	12	SIDE(122.0)
Z-B 2	12	TOP
N-L 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
Z-V 2	12	SIDE(183.1)
V-R 2	12	SIDE(183.1)
R-N 2	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2108031

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS									
		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
Z	3663	0	3663	0	0	5-8	5-8		
N	3633	0	3633	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS									
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
Z	2589	1708 / 0	0 / 0	0 / 0	0 / 0	881 / 0	0 / 0		
N	2589	1688 / 0	0 / 0	0 / 0	0 / 0	881 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS									
MEMB.		MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 (PLF)		MAX. UNBRAC LENGTH FR-TO		WEBS	
								MEMB.	
								MAX. FACTORED FORCE (LBS)	
								MAX. UNBRAC LENGTH FR-TO	
FR-TO		FROM TO		CSI (LC)					
A-B	0 / 42	-91.8	-91.8	0.04 (1)	10.00	Y-C	-809 / 0	0.07 (1)	
B-C	-3679 / 0	-91.8	-91.8	0.05 (1)	5.82	O-K	-799 / 0	0.07 (1)	
C-AA	-8261 / 0	-91.8	-91.8	0.22 (1)	4.58	B-Y	0 / 2986	0.26 (1)	
AA-AB	-8261 / 0	-91.8	-91.8	0.22 (1)	4.58	O-L	0 / 2959	0.26 (1)	
AB-D	-8261 / 0	-91.8	-91.8	0.22 (1)	4.58	X-D	-2235 / 0	0.19 (1)	
D-AC	-7069 / 0	-91.8	-91.8	0.21 (1)	4.37	P-J	-2241 / 0	0.19 (1)	
AC-AD	-7069 / 0	-91.8	-91.8	0.21 (1)	4.37	P-K	0 / 4210	0.37 (1)	
AD-AE	-7069 / 0	-91.8	-91.8	0.21 (1)	4.37	C-X	0 / 4200	0.37 (1)	
AE-E	-7069 / 0	-91.8	-91.8	0.21 (1)	4.37	U-E	0 / 166	0.02 (4)	
E-F	-8977 / 0	-91.8	-91.8	0.25 (1)	3.92	S-I	0 / 166	0.02 (4)	
F-AF	-8977 / 0	-91.8	-91.8	0.25 (1)	3.92	T-I	0 / 834	0.07 (1)	
AF-G	-8977 / 0	-91.8	-91.8	0.25 (1)	3.92	E-T	0 / 824	0.07 (1)	
G-AG	-8977 / 0	-91.8	-91.8	0.25 (1)	3.92	T-G	-775 / 0	0.07 (1)	
AG-H	-8977 / 0	-91.8	-91.8	0.25 (1)	3.92	Q-J	0 / 1650	0.15 (1)	
H-I	-8977 / 0	-91.8	-91.8	0.25 (1)	3.92	I-Q	-1874 / 0	0.26 (1)	
I-AH	-7058 / 0	-91.8	-91.8	0.21 (1)	4.38	D-W	0 / 1643	0.15 (1)	
AH-AI	-7058 / 0	-91.8	-91.8	0.21 (1)	4.38	W-E	-1866 0	0.25 (1)	
AI-AJ	-7058 / 0	-91.8	-91.8	0.21 (1)	4.38				
AJ-J	-7058 / 0	-91.8	-91.8	0.21 (1)	4.38				
J-AK	-6245 / 0	-91.8	-91.8	0.22 (1)	4.58				
AK-AL	-6245 / 0	-91.8	-91.8	0.22 (1)	4.58				
AL-K	-6245 / 0	-91.8	-91.8	0.22 (1)	4.58				
K-L	-3847 / 0	-91.8	-91.8	0.05 (1)	5.95				
L-M	0 / 42	-91.8	-91.8	0.04 (1)	10.00				
Z-B	-3651 / 0	0.0	0.0	0.13 (1)	7.35				
N-L	-3821 / 0	0.0	0.0	0.13 (1)	7.39				

Z-AM	0 / 0	-18.5	-18.5	0.05 (4)	10.00
AM-Y	0 / 0	-18.5	-18.5	0.05 (4)	10.00
Y-AN	0 / 2778	-18.5	-18.5	0.22 (1)	10.00
AN-AO	0 / 2778	-18.5	-18.5	0.22 (1)	10.00
AO-X	0 / 2778	-18.5	-18.5	0.22 (1)	10.00
X-AP	0 / 6281	-18.5	-18.5	0.45 (1)	10.00
AP-W	0 / 8251	-18.5	-18.5	0.45 (1)	10.00
W-AQ	0 / 8300	-18.5	-18.5	0.59 (1)	10.00
AQ-V	0 / 8300	-18.5	-18.5	0.59 (1)	10.00
V-AR	0 / 8300	-18.5	-18.5	0.59 (1)	10.00
AR-U	0 / 8300	-18.5	-18.5	0.59 (1)	10.00
U-AS	0 / 8300	-18.5	-18.5	0.59 (1)	10.00
AS-AT	0 / 8300	-18.5	-18.5	0.59 (1)	10.00
AT-T	0 / 8300	-18.5	-18.5	0.59 (1)	10.00
T-AU	0 / 8292	-18.5	-18.5	0.59 (1)	10.00

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.22")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.41")

CSI: TC=0.25/1.00 (E-G-T), BC=0.59/1.00 (T-U-1), WS=0.37/1.00 (K-P-1), SS=0.14/1.00 (J-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (X) (INPUT = 0.90)
JSI METAL= 0.75 (V) (INPUT = 1.00)



Structural component only
DWG# T-2108031

CONTINUED ON PAGE 2

JOB NAME 417406 Tamarack Roof Truss, Burlington	TRUSS NAME T14	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
TRUSS DESC.				Version 8.420 S Jan 21 2021 MTEK Industries, Inc. Fri Mar 19 17:34:15 2021 Page 2 ID:SKJDI76mu4YipKXoNwwrk5za4YO- LohTK yz9Nad3d?umQRKY8Yh5q7J8pgP2zFOxzZJWc	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	7.0	8.0	3.50	1.50
D	TMWW-t	MT20	5.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0		
J	TMWW-t	MT20	5.0	6.0		
K	TTWW+m	MT20	7.0	8.0	3.50	1.50
L	TMW+p	MT20	5.0	6.0	2.00	2.25
N	BMV1+p	MT20	3.0	6.0		
O, P, Q, W, X, Y						
C	BMW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMW+w	MT20	3.0	6.0		
T	BMWW-t	MT20	5.0	9.0		
U	BMW+w	MT20	3.0	6.0		
V	BS-t	MT20	5.0	6.0		
Z	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO		UNBRAC			
AL-AV	0/8292	-18.5	-18.5 0.59 (1)	10.00			
AV-S	0/8292	-18.5	-18.5 0.59 (1)	10.00			
S-AW	0/8292	-18.5	-18.5 0.58 (1)	10.00			
AW-R	0/8292	-18.5	-18.5 0.58 (1)	10.00			
R-AX	0/8292	-18.5	-18.5 0.58 (1)	10.00			
AX-Q	0/8292	-18.5	-18.5 0.58 (1)	10.00			
Q-AY	0/8245	-18.5	-18.5 0.58 (1)	10.00			
AY-P	0/8245	-18.5	-18.5 0.45 (1)	10.00			
P-AZ	0/2753	-18.5	-18.5 0.45 (1)	10.00			
AZ-BA	0/2753	-18.5	-18.5 0.22 (1)	10.00			
BA-O	0/2753	-18.5	-18.5 0.22 (1)	10.00			
O-BB	0/0	-18.5	-18.5 0.05 (4)	10.00			
BB-N	0/0	-18.5	-18.5 0.05 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-105	-105	---	FRONT	VERT	TOTAL	---	C1
C	2-11-8	-125	-125	---	FRONT	VERT	SNOW	---	C1
F	15-0-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
G	18-9-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
H	22-7-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
K	34-8-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
K	34-8-8	-82	-82	---	FRONT	VERT	TOTAL	---	C1
K	34-8-8	-125	-125	---	FRONT	VERT	SNOW	---	C1
O	34-7-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	18-9-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Y	3-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AA	5-0-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AB	7-0-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AC	9-0-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AD	11-0-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AE	13-0-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AF	17-0-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AG	20-7-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AH	24-7-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AI	28-7-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AJ	28-7-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AK	30-7-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AL	32-7-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AM	1-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AN	5-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AO	7-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AP	9-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AQ	11-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AR	13-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AS	15-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AT	17-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AU	20-7-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AV	22-7-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AW	24-7-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AX	26-7-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AY	28-7-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AZ	30-7-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
BA	32-7-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
BB	36-7-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

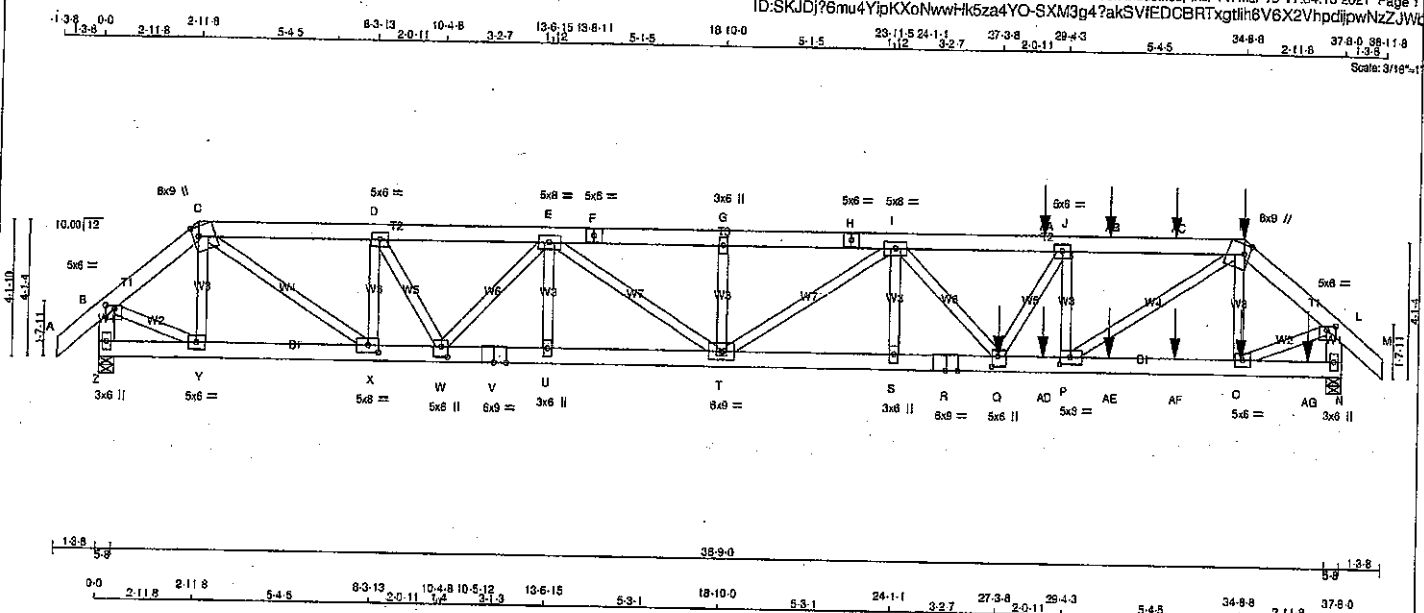
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2108031 *ML*

JOB NAME 417406	TRUSS NAME T14Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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 ID:SKJDJ?6mu4YpKXoNwwHk6za4YO-SXM3g4?akSVIEDCBRTxgtlin6V6X2VhpdipwNzZJWd



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - M	2x6	DRY	No.2	SPF
M - P	2x6	DRY	No.2	SPF
P - R	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2
 EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 2 12		TOP
C-F 2 12		TOP
F-H 2 12		TOP
H-K 2 12		TOP
K-M 2 12		TOP
M-P 2 12		TOP
P-R 2 12		TOP
R-N 2 12		TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
Z-V 2 12		TOP
V-R 2 12		TOP
R-N 2 12		TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1 6		TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
Z	3130	0	0	5-8
N	5053	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL
Z	2209 1473.0 0.0 0.0 0.0 737.0 0.0
N	3587 2375.0 0.0 0.0 0.0 1192.0 0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED TORSION (LBS)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED TORSION (LBS)
FR-TO						FR-TO				
A-B	0.42	-91.8	-91.8	0.04 (1)	10.00	Y-C	-718.0	0.06 (1)		
B-C	-3114.0	-91.8	-91.8	0.05 (1)	6.21	O-K	-1207.0	0.10 (1)		
C-D	-5920.0	-91.8	-91.8	0.16 (1)	4.79	B-Y	0.2527	0.22 (1)		
D-E	-6929.0	-91.8	-91.8	0.15 (1)	4.50	O-L	0.4220	0.37 (1)		
E-F	-10130.0	-91.8	-91.8	0.27 (1)	3.71	X-D	-2272.0	0.19 (1)		
F-G	-10130.0	-91.8	-91.8	0.27 (1)	3.71	P-J	-3526.0	0.30 (1)		
G-H	-10130.0	-91.8	-91.8	0.28 (1)	3.70	P-K	0.6855	0.59 (1)		
H-I	-10130.0	-91.8	-91.8	0.28 (1)	3.70	C-X	0.4185	0.37 (1)		
I-AA	-11035.0	-91.8	-91.8	0.32 (1)	3.53	U-E	0.7114	0.01 (4)		
AA-J	-11035.0	-91.8	-91.8	0.32 (1)	3.53	S-I	0.7166	0.02 (4)		
J-AB	-9444.0	-91.8	-91.8	0.34 (1)	3.75	T-I	-1105.0	0.28 (1)		
AB-AC	-9444.0	-91.8	-91.8	0.34 (1)	3.75	E-T	0.2097	0.19 (1)		
AC-K	-9444.0	-91.8	-91.8	0.34 (1)	3.75	T-G	-481.0	0.04 (1)		
K-L	-5201.0	-91.8	-91.8	0.07 (1)	5.08	Q-J	0.3236	0.29 (1)		
L-M	0.42	-91.8	-91.8	0.04 (1)	10.00	I-Q	-12.0	0.00 (4)		
Z-B	-3131.0	0.0	0.0	0.11 (1)	7.80	D-W	0.2054	0.18 (1)		
N-L	-6050.0	0.0	0.0	0.18 (1)	6.48	W-E	-2390.0	0.33 (1)		

LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
JT	34-8-8	-30	-30	---	FRONT	VERT	DEAD
K	34-8-8	-82	-82	---	BACK	VERT	TOTAL
K	34-8-8	-125	-125	---	FRONT	VERT	SNOW
O	34-7-12	-21	-21	---	BACK	VERT	TOTAL
Q	27-3-8	-2088	-2088	---	BACK	VERT	TOTAL

SPECIFIED CONCENTRATED LOADS (LBS)

LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
JT	34-8-8	-30	-30	---	FRONT	VERT	DEAD
K	34-8-8	-82	-82	---	BACK	VERT	TOTAL
K	34-8-8	-125	-125	---	FRONT	VERT	SNOW
O	34-7-12	-21	-21	---	BACK	VERT	TOTAL
Q	27-3-8	-2088	-2088	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2015, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.26")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.26")
 ALLOWABLE DEFL. (TL) = L/360 (1.26")
 CALCULATED VERT. DEFL. (TL) = L/934 (0.48")

CSI: TC=0.34/1.00 (J-K:1), BC=0.83/1.00 (Q-S:1),
 WB=0.59/1.00 (K-P:1), SSI=0.13/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.80 (X) (INPUT = 0.80)
 JSI METAL = 0.85 (V) (INPUT = 1.00)



Structural component only
 DWG# T-2108032 1/2

JOB NAME 417406	TRUSS NAME T14Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 8 Jan 21 2021 MTEK Industries, Inc. Fri Mar 19 17:34:16 2021 Page 2 ID:SKJDI?6mu4YipKXoNwwHk5za4YO-SXM3p4?akSVIEDCBRTxqtihsV6X2VhpdipwNzZJw	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.25	3.25
C	TTWW+m	MT20	8.0	9.0	3.50	2.00
D	TMVW-t	MT20	5.0	6.0		
E	TMVWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMVW-w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMVWW-t	MT20	5.0	6.0		
J	TMVW-t	MT20	5.0	6.0		
K	TTWW+m	MT20	8.0	9.0	3.50	2.00
L	TMVW-p	MT20	5.0	6.0	1.25	3.25
N	BMV1-p	MT20	3.0	6.0		
O	BMVW-t	MT20	5.0	6.0		
P	BMVW-t	MT20	5.0	6.0	2.50	3.75
Q	BMVW-t	MT20	5.0	6.0	3.75	2.25
R	BS-t	MT20	6.0	9.0		
S	BMVW-w	MT20	3.0	6.0		
T	BMVWW-t	MT20	6.0	9.0		
U	BMVW-w	MT20	3.0	6.0		
V	BS-t	MT20	6.0	9.0		
W	BMVWW-t	MT20	5.0	6.0	3.75	2.25
X	BMVW-t	MT20	5.0	6.0	2.50	3.75
Y	BMVW-t	MT20	5.0	6.0		
Z	BMV1-p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AA	28-7-12	-78	-78	---	BACK	VERT	TOTAL	---	C1
AB	30-7-12	-78	-78	---	BACK	VERT	TOTAL	---	C1
AC	32-7-12	-78	-78	---	BACK	VERT	TOTAL	---	C1
AD	28-7-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AE	30-7-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AF	32-7-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AG	36-7-12	-21	-21	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

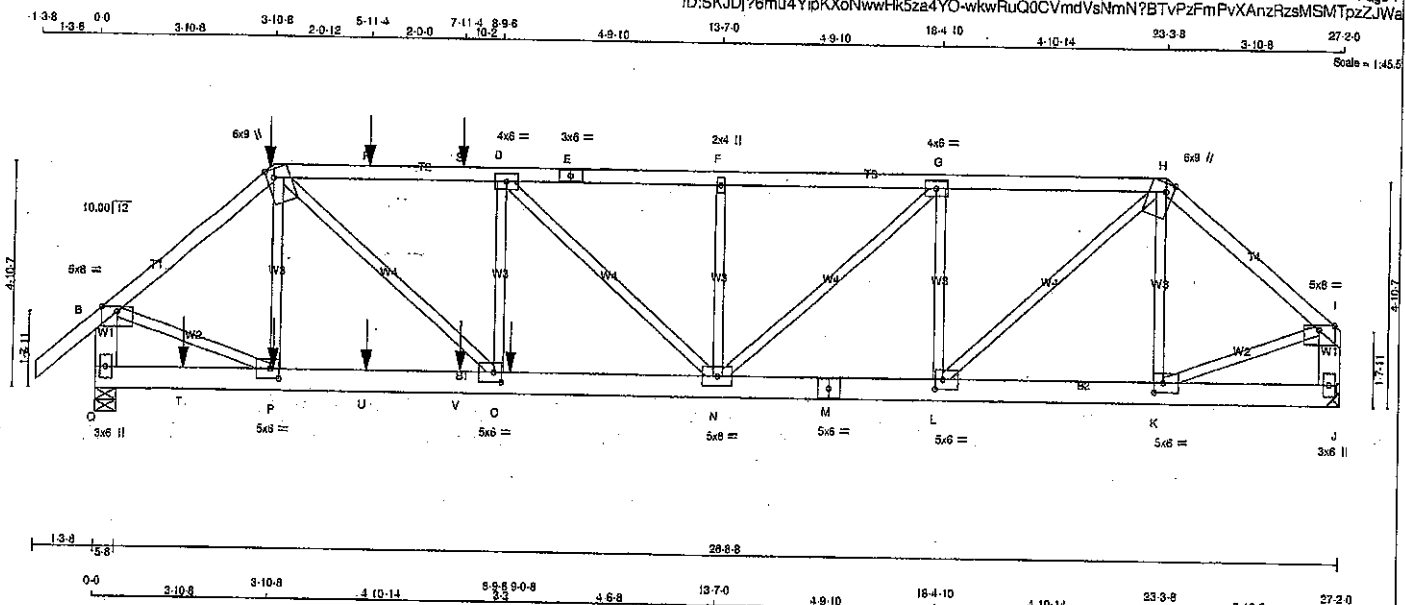


Structural component only
DWG# T-2108032 *2/2*

JOB NAME 417406	TRUSS NAME T15	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4	DRY No.2
C - E	2x4	DRY No.2
E - H	2x4	DRY No.2
H - I	2x4	DRY No.2
Q - B	2x6	DRY No.2
J - I	2x6	DRY No.2
Q - M	2x6	DRY No.2
M - J	2x6	DRY No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge 1.75	
D	TMVW-i	MT20	4.0	6.0		
E	TS-i	MT20	3.0	3.0		
F	TMVW-w	MT20	2.0	4.0		
G	TMVW-i	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge 1.75	
I	TMVW-p	MT20	5.0	8.0	Edge	
J	BMVW-i	MT20	3.0	6.0		
K	BMVW-i	MT20	5.0	6.0	2.50	2.25
L	BMVW-i	MT20	5.0	6.0	2.50	2.00
M	BS-i	MT20	5.0	6.0		
N	BMVW-i	MT20	5.0	6.0		
O	BMVW-i	MT20	5.0	6.0	2.50	2.00
P	BMVW-i	MT20	5.0	6.0	2.50	2.25
Q	BMVW-i	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES-

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
Q	2669	0	2669	0
J	1904	0	1904	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3'-8".

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM LIVE WIND DEAD SOIL
Q	1883	1261.0 0.0 0.0 0.0 621.0 0.0
J	1345	889.0 0.0 0.0 0.0 456.0 0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO						FR-TO			
A-B	0.41	-91.8	-91.8	0.14 (1)	10.00	P-C	-460.0	0.16 (1)	
B-C	-2661.0	-91.8	-91.8	0.41 (1)	3.83	C-O	0.2249	0.56 (1)	
C-R	-3689.0	-91.8	-91.8	0.75 (1)	2.89	O-D	-458.0	0.16 (1)	
R-S	-3689.0	-91.8	-91.8	0.75 (1)	2.89	D-N	-245.0	0.19 (1)	
S-D	-3689.0	-91.8	-91.8	0.75 (1)	2.89	N-F	-385.0	0.13 (1)	
D-E	-3510.0	-91.8	-91.8	0.70 (1)	3.02	N-G	0.7943	0.23 (1)	
E-F	-3510.0	-91.8	-91.8	0.70 (1)	3.02	L-G	-1145.0	0.39 (1)	
F-G	-3510.0	-91.8	-91.8	0.60 (1)	3.17	L-H	0.1821	0.45 (1)	
G-H	-2622.0	-91.8	-91.8	0.52 (1)	3.60	K-H	-352.0	0.12 (1)	
H-I	-1937.0	-91.8	-91.8	0.35 (1)	4.43	B-P	0.2132	0.53 (1)	
Q-B	-2641.0	0.0	0.0	0.19 (1)	6.36	K-I	0.1552	0.36 (1)	
J-I	-1879.0	0.0	0.0	0.14 (1)	7.28				
Q-T	0.0	-18.5	-18.5	0.07 (4)	10.00				
T-P	0.0	-18.5	-18.5	0.07 (4)	10.00				
P-U	0.2028	-18.5	-18.5	0.33 (1)	10.00				
U-V	0.2028	-18.5	-18.5	0.33 (1)	10.00				
V-O	0.2028	-18.5	-18.5	0.33 (1)	10.00				
O-N	0.3690	-18.5	-18.5	0.55 (1)	10.00				
N-M	0.2822	-18.5	-18.5	0.42 (1)	10.00				
M-L	0.2822	-18.5	-18.5	0.42 (1)	10.00				
L-K	0.1476	-18.5	-18.5	0.23 (1)	10.00				
K-J	0.0	-18.5	-18.5	0.05 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIRE.	TYPE	HEEL	CONN.
C	3-10-8	-157	-157	---	BACK	VERT	---	C1
O	9-0-8	-691	-691	---	BACK	VERT	---	C1
P	3-11-4	-14	-14	---	BACK	VERT	---	C1
R	5-11-4	-59	-59	---	BACK	VERT	---	C1
S	7-11-4	-59	-59	---	BACK	VERT	---	C1
T	1-11-4	-14	-14	---	BACK	VERT	---	C1
U	5-11-4	-14	-14	---	BACK	VERT	---	C1
V	7-11-4	-14	-14	---	BACK	VERT	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 132 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00-12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.91")
CALCULATED VERT. DEFL.(LL) = L/599 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (0.91")
CALCULATED VERT. DEFL.(TL) = L/599 (0.25")

CSI: TC=0.75/1.00 (C-D:1), BC=0.55/1.00 (N-O:1), WB=0.59/1.00 (C-O:1), SI=0.39/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

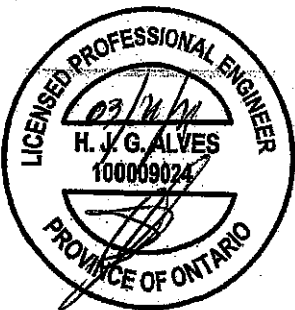
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 550 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.90 (C) (INPUT = 0.90)
JSI METAL=0.71 (M) (INPUT = 1.00)

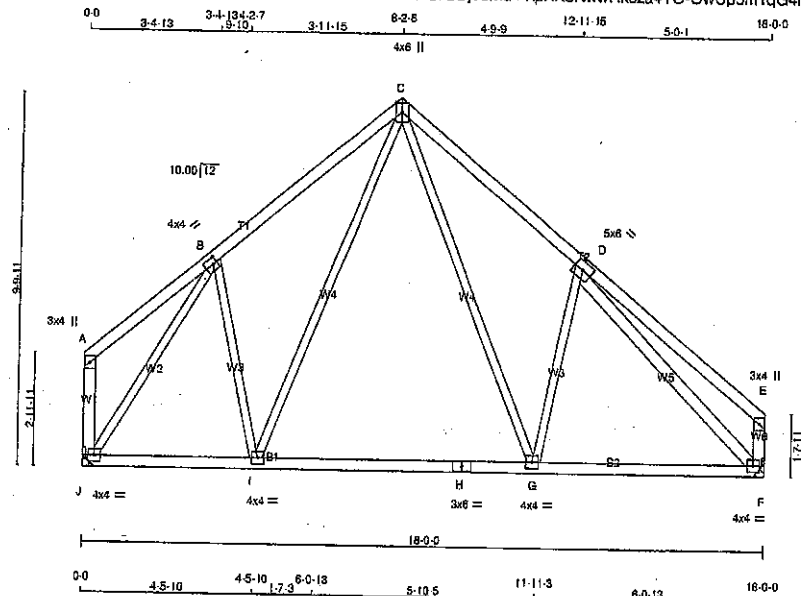


Structural component only
DWG# T-2108033

JOB NAME 417406	TRUSS NAME T16	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:SKJD76mu4YipKXoNwwHk5za4YO-OwUp5mtqG4IMJXLazU_ByAn1CkxWNh850Cv?GzZJWZ



Scale = 1:55.4

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
J - A	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
D - F	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMVW-i	MT20	4.0	4.0	2.00	1.25
C	TTVW+p	MT20	4.0	6.0	Edge	
D	TMVW-i	MT20	5.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW-i	MT20	4.0	4.0		
G	BMVW-i	MT20	4.0	4.0		
H	BS-i	MT20	3.0	6.0		
I	BMVW-i	MT20	4.0	4.0		
J	BMVW-i	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	992	0	992	0
J	992	0	992	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, J. MINIMUM BEARING LENGTH AT JOINT F = 3-8, JOINT J = 3-8.

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	702	461 '0	0 '0	0 '0	0 '0	241 '0	0 '0
J	702	461 '0	0 '0	0 '0	0 '0	241 '0	0 '0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	FORCE	VERT. LOAD	LC1	MAX	UNBRAC	MEMB.	FORCE	MAX
FR-TO		(LBS)				LENGTH	FR-TO	(LBS)	
A-B	0	738	-91.8	-91.8	0.28 (1)	10.00	C-G	0	478
B-C	-714	0	-91.8	-91.8	0.25 (1)	6.25	G-D	-913	0
C-D	-862	0	-91.8	-91.8	0.29 (1)	6.23	D-F	-1056	0
D-E	0	35	-91.8	-91.8	0.36 (1)	10.00	I-C	0	182
J-A	-95	0	0	0	0.02 (1)	7.81	B-I	-48	47
F-E	-167	0	0	0	0.02 (1)	7.81	J-B	-1023	0
J-I	0	538	-18.5	-18.5	0.20 (4)	10.00			
I-H	0	482	-18.5	-18.5	0.24 (4)	10.00			
H-G	0	482	-18.5	-18.5	0.24 (4)	10.00			
G-F	0	708	-18.5	-18.5	0.28 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019.
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.80")
CALCULATED VERT. DEFL. (LL) = $L/990$ (0.02")
ALLOWABLE DEFL. (TL) = $L/360$ (0.80")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.07")

CSI: TC=0.36/1.00 (D-E:1), BC=0.28/1.00 (F-G:4),
WB=0.75/1.00 (D-F:1), SS=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

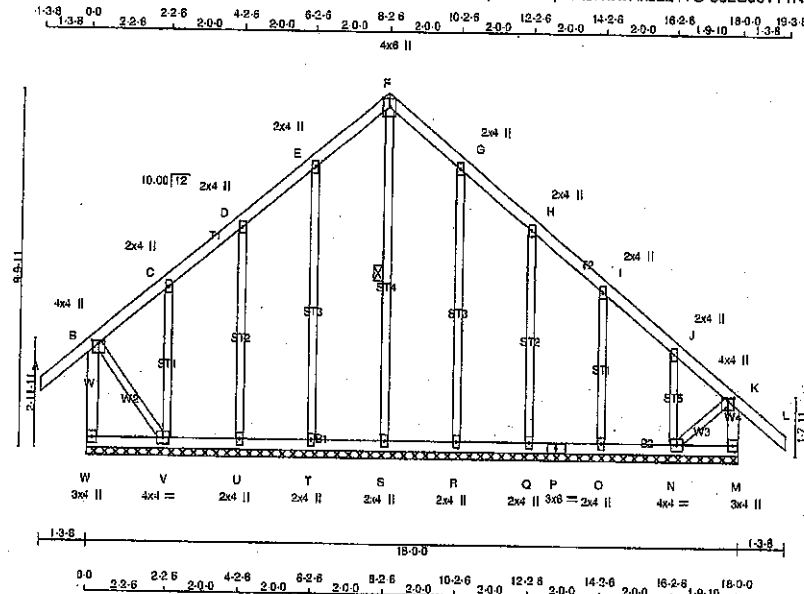
JSI GRIP = 0.82 (F) (INPUT = 0.80)
JSI METAL = 0.34 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108034

JOB NAME 417406	TRUSS NAME T16G	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MTek Industries, Inc. Fri Mar 19 17:34:19 2021 Page 1
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Scale = 1:57.7

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - F	2x4	DRY	No.2	SPF
F - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER				
GABLE STUDS SPACED AT 2'-0" OC.				

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I, J						
C	TMVW+w	MT20	2.0	4.0		
F	TTW+p	MT20	4.0	6.0	Edge	
K	TMVW+p	MT20	4.0	4.0	1.00	2.00
M	BMV1+p	MT20	3.0	4.0		
N	BMVW1-i	MT20	4.0	4.0		
O, Q, R, S, T, U						
O	BMV1+w	MT20	2.0	4.0		
P	BS-i	MT20	3.0	6.0		
V	BMVW1-i	MT20	4.0	4.0		
W	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	CSF (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSF (LC)	UNBRACED LENGTH (FT)
FR-TO				FR-TO			
A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00	S-F	-139 / 0	0.09 (1)
B-C	-13 / 0	-91.8	-91.8 0.06 (1)	6.25	T-E	-210 / 0	0.27 (1)
C-D	-23 / 0	-91.8	-91.8 0.06 (1)	6.25	U-D	-169 / 0	0.12 (1)
D-E	-12 / 0	-91.8	-91.8 0.05 (1)	6.25	V-C	-211 / 0	0.07 (1)
E-F	-24 / 0	-91.8	-91.8 0.05 (1)	6.25	R-G	-209 / 0	0.27 (1)
F-G	-24 / 0	-91.8	-91.8 0.05 (1)	6.25	Q-H	-173 / 0	0.12 (1)
G-H	-12 / 0	-91.8	-91.8 0.05 (1)	6.25	O-I	-194 / 0	0.07 (1)
H-I	-21 / 0	-91.8	-91.8 0.05 (1)	6.25	N-J	-111 / 0	0.02 (1)
I-J	-17 / 0	-91.8	-91.8 0.05 (1)	6.25	B-V	0 / 30	0.01 (1)
J-K	-58 / 0	-91.8	-91.8 0.12 (1)	6.25	N-K	0 / 28	0.01 (1)
K-L	0 / 41	-91.8	-91.8 0.13 (1)	10.00			
W-B	-243 / 0	0.0	0.0 0.04 (1)	7.81			
M-K	-285 / 0	0.0	0.0 0.03 (1)	7.81			
W-V	0 / 0	-18.5	-18.5 0.02 (4)	10.00			
V-U	0 / 15	-18.5	-18.5 0.02 (4)	10.00			
U-T	0 / 12	-18.5	-18.5 0.02 (4)	10.00			
T-S	0 / 10	-18.5	-18.5 0.02 (4)	10.00			
S-R	0 / 10	-18.5	-18.5 0.01 (4)	10.00			
R-Q	0 / 12	-18.5	-18.5 0.02 (4)	10.00			
Q-P	0 / 15	-18.5	-18.5 0.02 (4)	10.00			
P-O	0 / 15	-18.5	-18.5 0.02 (4)	10.00			
O-N	0 / 19	-18.5	-18.5 0.02 (4)	10.00			
N-M	0 / 0	-18.5	-18.5 0.02 (4)	10.00			

TOTAL WEIGHT = 98 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (K-L:1), BC=0.02/1.00 (U-V:4), WB=0.27/1.00 (E-T:1), SSI=0.08/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1997 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.49 (N) (INPUT = 0.50)
JSI METAL = 0.11 (C) (INPUT = 1.00)

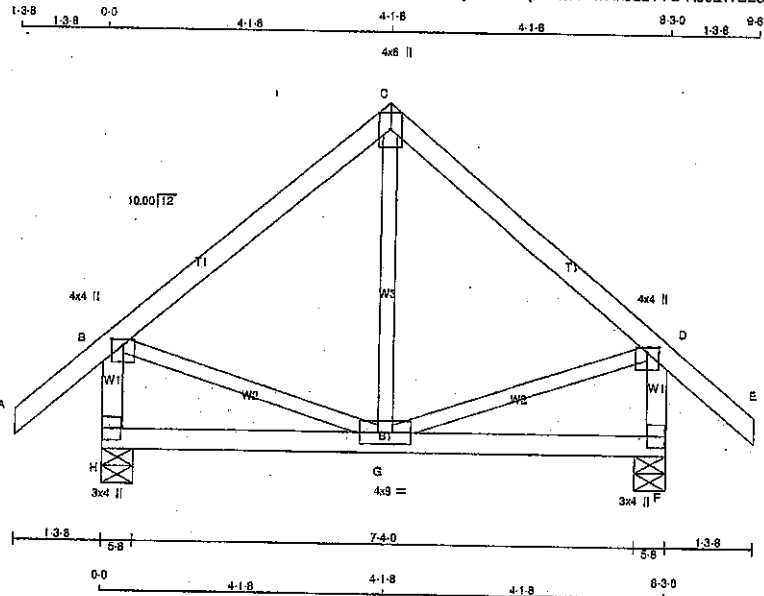


Structural component only
DWG# T-2108035

JOB NAME 417406	TRUSS NAME T17	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/20.6

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4 DRY	No.2
C - E	2x4 DRY	No.2
H - B	2x4 DRY	No.2
F - D	2x4 DRY	No.2
H - F	2x4 DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - TMVW+p	MT20	4.0	4.0	1.00	2.00
C - TTW+p	MT20	4.0	6.0	Edge	
D - TMVW+p	MT20	4.0	4.0	1.00	2.00
F - BMV1+p	MT20	3.0	4.0		
G - BMVWW+H	MT20	4.0	9.0		
H - BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES-

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
JT	952	0	952	0
H	952	0	952	0
F	952	0	952	0

UNFACTORED REACTIONS

	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED						
H	685	381.0	0.0	0.0	0.0	304.0	0.0
F	482	315.0	0.0	0.0	0.0	167.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0.41	-91.8 -91.8 0.14 (1)	10.00	G-C	0.163	0.06 (4)	
B-C	-410.0	-91.8 -91.8 0.30 (1)	6.25	B-G	0.331	0.08 (1)	
C-D	-410.0	-91.8 -91.8 0.30 (1)	6.25	G-D	0.331	0.08 (1)	
D-E	0.41	-91.8 -91.8 0.14 (1)	10.00				
H-B	-679.0	0.0 0.0 0.08 (1)	7.81				
F-D	-679.0	0.0 0.0 0.08 (1)	7.81				
H-G	0.0	-185.3 -79.0 0.38 (4)	10.00				
G-F	0.0	-18.5 -18.5 0.26 (4)	10.00				

TOTAL WEIGHT = 39 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL	= 25.8 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.28")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.28")
CALCULATED VERT. DEFL. (TL) = L/999 (0.07")

CSI: TG=0.30/1.00 (C-D:1), BC=0.38/1.00 (G-H:4), WB=0.08/1.00 (B-G:1), SS=0.27/1.00 (G-H:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	850 371	1747 768 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

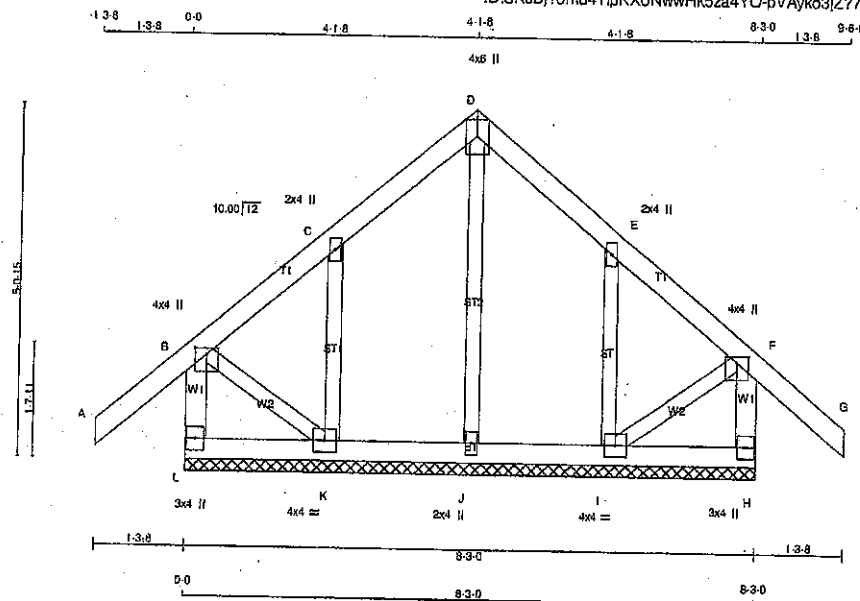
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.56 (B) (INPUT = 0.90)
JSI METAL = 0.15 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108036

JOB NAME 417406	TRUSS NAME T17G	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Mar 19 17:34:21 2021 Page 1 ID:SKJDj76mu4YipKXoNwwHk5za4YO-pVayko3jZ77xL_48E1XrapPc9W0QjvOYn_QZcbzZJWW			



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
L - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT	2'-0" OC.		

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C TMVW+w	MT20	2.0	4.0		
D TTVW+p	MT20	4.0	6.0	Edge	
E TMVW+w	MT20	2.0	4.0		
F TMVW+p	MT20	4.0	4.0	1.00	2.00
H BMV1+p	MT20	3.0	4.0		
I BMVW1+1	MT20	4.0	4.0		
J BMV1+w	MT20	2.0	4.0		
K BMVW1+1	MT20	4.0	4.0		
L BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES:

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
L-B	-226 / 0	0.0	0.0 0.02 (1)	J-D	-126 / 0	0.05 (1)	
A-B	0 / 41	-91.8	-91.8 0.13 (1)	K-C	-229 / 0	0.05 (1)	
B-C	-11 / 0	-91.8	-91.8 0.08 (1)	I-E	-229 / 0	0.05 (1)	
C-D	-31 / 0	-91.8	-91.8 0.08 (1)	B-K	0 / 23	0.01 (1)	
D-E	-31 / 0	-91.8	-91.8 0.08 (1)	I-F	0 / 23	0.01 (1)	
E-F	-11 / 0	-91.8	-91.8 0.08 (1)				
F-G	0 / 41	-91.8	-91.8 0.13 (1)				
H-F	-226 / 0	0.0	0.0 0.02 (1)				
L-K	0 / 0	-18.5	-18.5 0.02 (4)				
K-J	0 / 13	-18.5	-18.5 0.02 (4)				
J-I	0 / 13	-18.5	-18.5 0.02 (4)				
I-H	0 / 0	-18.5	-18.5 0.02 (4)				

TOTAL WEIGHT = 40 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (F-G:1), BC=0.02/1.00 (J-K:4), WB=0.05/1.00 (D-J:1), SS=0.08/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

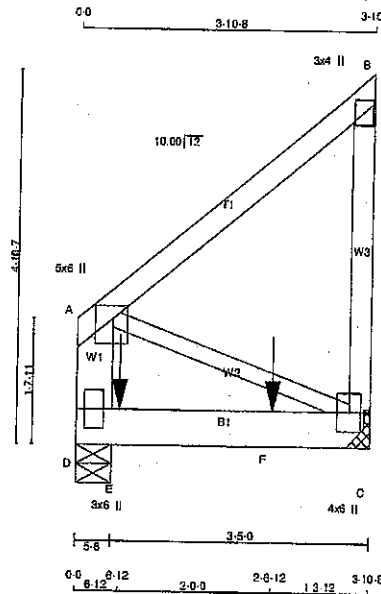
JSI GRIP= 0.16 (C) (INPUT = 0.90)
JSI METAL= 0.12 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108037

JOB NAME 417406	TRUSS NAME T18	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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ID:SKJDj76mu4YipKXoNwwHk5za4YO-pVayko3JZ77xL_48E1XrapPb7Wweiv6Yn_QZcbzZJWW



Scale = 1/2" = 1'-0"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
C - B	2x4	DRY	No.2	SPF	
D - A	2x6	DRY	No.2	SPF	
D - C	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-B 1	12	TOP
B-C 1	12	TOP
D-A 2	3	SIDE(72.0)
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D-C 2	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	UPLIFT
C	1000	0	1000	0
D	1379	0	1379	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4-0.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED
C	706
D	974

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	WEBS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)
FR-TO				FR-TO			
A-B	0.0	-18.5	-18.5 0.39 (1)	A-C	0.0	0.00 (1)	
C-B	-178.0	0.0	0.0 0.03 (1)				
D-A	-178.0	0.0	0.0 0.01 (1)				
D-E	0.0	-18.5	-18.5 0.39 (1)				
E-F	0.0	-18.5	-18.5 0.39 (1)				
F-C	0.0	-18.5	-18.5 0.39 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	6-12	-690	-690	--	BACK	VERT	TOTAL	--	C1
F	2-6-12	-687	-687	--	BACK	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 22 = 45 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.13/1.00 (A-B:1), BC=0.39/1.00 (C-D:1), WB=0.00/1.00 (A-C:1), SSI=0.40/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (D) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108038

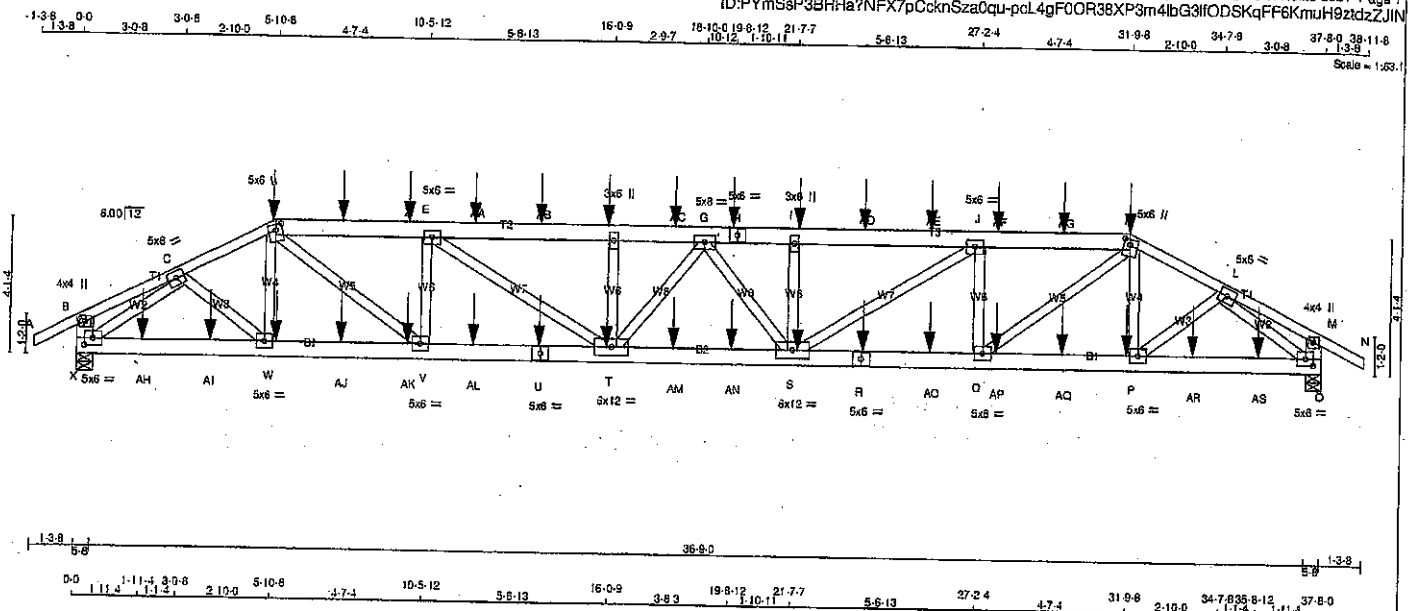
JOB NAME 417413	TRUSS NAME T21	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:50



CHORDS	SIZE	DRY	SPF	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - N	2x6	DRY	No.2	SPF
N - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
M - P	2x6	DRY	No.2	SPF
P - Q	2x6	DRY	No.2	SPF
Q - R	2x6	DRY	No.2	SPF
R - S	2x6	DRY	No.2	SPF
S - T	2x6	DRY	No.2	SPF
T - U	2x6	DRY	No.2	SPF
U - V	2x6	DRY	No.2	SPF
V - W	2x6	DRY	No.2	SPF
W - X	2x6	DRY	No.2	SPF
X - Y	2x6	DRY	No.2	SPF
Y - Z	2x6	DRY	No.2	SPF
Z - A	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D	12	SIDE (61.0)
D - H	12	SIDE (61.0)
H - K	12	SIDE (183.1)
K - N	12	SIDE (183.1)
N - O	12	SIDE (183.1)
O - M	12	SIDE (183.1)
M - P	12	SIDE (183.1)
P - Q	12	SIDE (183.1)
Q - R	12	SIDE (183.1)
R - S	12	SIDE (183.1)
S - T	12	SIDE (183.1)
T - U	12	SIDE (183.1)
U - V	12	SIDE (183.1)
V - W	12	SIDE (183.1)
W - X	12	SIDE (183.1)
X - Y	12	SIDE (183.1)
Y - Z	12	SIDE (183.1)
Z - A	12	SIDE (183.1)

WEBS : (0.122"x3") SPIRAL NAILS

2x4 1 6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD. BRG
JT	VERT	DOWN	UPLIFT	IN-SX
O	3488	0	0	5-8
X	3488	0	0	5-8

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
O	2455	1601.0 0 0 0 0 854.0 0 0
X	2454	1600.0 0 0 0 0 854.0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, X

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.93 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED UPLIFT (LBS)	MAX. FACTORED IN-SX (LBS)	MAX. FACTORED OUT-SX (LBS)
FR-TO						
A-B	0.28	-18.5	-18.5	0.07 (1)	10.00	0.00
B-C	0.7	-18.5	-18.5	0.08 (1)	10.00	0.00
C-D	-5312.0	-18.5	-18.5	0.15 (1)	10.00	0.00
D-E	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
E-F	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
F-G	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
G-H	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
H-I	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
I-J	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
J-K	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
K-L	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
L-M	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
M-N	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
N-O	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
O-P	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
P-Q	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
Q-R	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
R-S	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
S-T	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
T-U	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
U-V	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
V-W	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
W-X	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
X-Y	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
Y-Z	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00
Z-A	-7313.0	-18.5	-18.5	0.22 (1)	10.00	0.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 5.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019

- PART 9 OF CBC 2012 (2019 AMENDMENT)

- CSA 688-14

- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.26")

CALCULATED VERT. DEFL. (LL) = U/999 (0.21")

ALLOWABLE DEFL. (TL) = L/360 (1.26")

CALCULATED VERT. DEFL. (TL) = U/999 (0.35")

CSI: TC=0.27/1.00 (E-F:1), BC=0.27/1.00 (S-T:1), WB=0.44/1.00 (L-O:1), SS=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

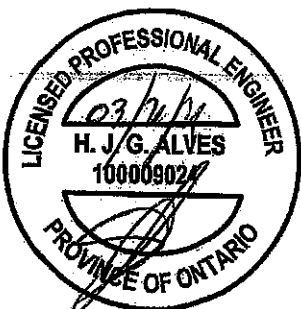
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.85 (C) (INPUT = 0.90)

JSI METAL = 0.69 (U) (INPUT = 1.00)



Structural component only

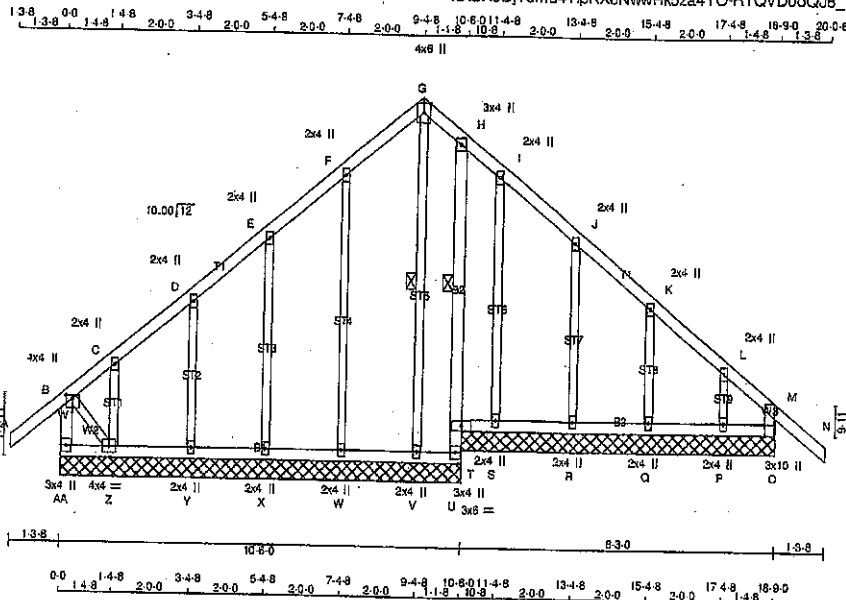
DWG# T-2108049 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
417375	T9G	1	1	GREENPARK HOMES		

Tamarack Roof Truss, Burlington

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Scale = 1:54.6

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - G	2x4	DRY	No.2
G - N	2x4	DRY	No.2
AA - B	2x4	DRY	No.2
O - M	2x4	DRY	No.2
AA - U	2x4	DRY	No.2
U - H	2x4	DRY	No.2
T - O	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT. TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	4.0	4.0	1.00	2.00
C, D, E, F, I, J, K, L					
C TMW-w	MT20	2.0	4.0		
G TTW-p	MT20	4.0	8.0	Edge	
H TMV-p	MT20	3.0	4.0		
M					
O TBMV1+p	MT20	3.0	10.0	Edge	0.25
P, Q, R, S, V, W, X, Y					
P BMV1-w	MT20	2.0	4.0		
T BMV1-i	MT20	3.0	6.0		
U BMV1-p	MT20	3.0	4.0		
Z BMVW1-i	MT20	4.0	4.0		
AA BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-T, G-V.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/41	-91.8 -91.8	0.13 (1)	10.00	V-G	-136/0	0.08 (1)
B-C	-36/0	-91.8 -91.8	0.12 (1)	6.25	W-F	-209/0	0.24 (1)
C-D	0/15	-91.8 -91.8	0.05 (1)	10.00	X-E	-173/0	0.10 (1)
D-E	0/10	-91.8 -91.8	0.05 (1)	10.00	Y-D	-196/0	0.06 (1)
E-F	0/19	-91.8 -91.8	0.06 (1)	10.00	Z-C	-75/0	0.01 (1)
F-G	0/8	-91.8 -91.8	0.05 (1)	10.00	S-I	-133/0	0.11 (1)
G-H	0/9	-91.8 -91.8	0.02 (1)	10.00	R-J	-184/0	0.08 (1)
H-I	0/19	-91.8 -91.8	0.03 (1)	10.00	Q-K	-195/0	0.04 (1)
I-J	0/14	-91.8 -91.8	0.04 (1)	10.00	P-L	-70/0	0.01 (1)
J-K	0/11	-91.8 -91.8	0.05 (1)	10.00	B-Z	-3/0	0.00 (1)
K-L	0/15	-91.8 -91.8	0.05 (1)	10.00			
L-M	-40/0	-91.8 -91.8	0.12 (1)	8.25			
M-N	0/41	-91.8 -91.8	0.13 (1)	10.00			
AA-B	-243/0	0.0 0.0	0.03 (1)	7.81			
O-M	-249/0	0.0 0.0	0.02 (1)	7.81			
AA-Z	0/0	-18.5 -18.5	0.01 (4)	10.00			
Z-Y	-5/0	-18.5 -18.5	0.02 (4)	10.00			
Y-X	-10/0	-18.5 -18.5	0.02 (4)	10.00			
X-W	-12/0	-18.5 -18.5	0.02 (4)	6.25			
W-V	-15/0	-18.5 -18.5	0.02 (4)	6.25			
V-U	-15/0	-18.5 -18.5	0.01 (4)	10.00			
U-T	0/0	0.0 0.0	0.02 (1)	10.00			
T-H	-110/0	0.0 0.0	0.02 (1)	6.25			
T-S	-15/0	-18.5 -18.5	0.01 (4)	6.25			
S-R	-13/0	-18.5 -18.5	0.02 (4)	6.25			
R-Q	-10/0	-18.5 -18.5	0.02 (4)	10.00			
Q-P	-4/0	-18.5 -18.5	0.01 (4)	10.00			
P-O	0/0	-18.5 -18.5	0.01 (4)	10.00			

TOTAL WEIGHT = 101 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/899$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CS: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (H-T:1),
WB=0.24/1.00 (F-W:1), SS=0.08/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT.

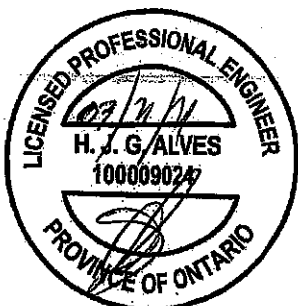
NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (Z) (INPUT = 0.90)
JSI METAL= 0.11 (F) (INPUT = 1.00)



Structural component only
DWG# T-2108020

JOB NAME 417413	TRUSS NAME T21	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Mar 19 17:49:26 2021 Page 2
ID:PYmSsP3BHha?NFX7pCcknSze0qu-pcl.4qF0QR38XP3m4bG3fODSKoFF6KmuH9zdzZJIN

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C, E, J, L						
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	2.25
F	TMW+w	MT20	3.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMW+w	MT20	3.0	6.0		
K	TTWW+m	MT20	5.0	6.0	2.00	2.25
M	TMV+p	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0	2.50	2.75
P, Q, V, W						
F	BMWW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMWWW-t	MT20	6.0	12.0		
T	BMWWW-t	MT20	6.0	12.0		
U	BS-t	MT20	5.0	6.0		
X	BMVW1-t	MT20	5.0	6.0	2.50	2.75

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	CS1 (LC)	MEMB. MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO			LENGTH FR-TO		
AQ-P	0/4781	-18.5	-18.5	0.14 (1)	10.00		
P-AR	0/4123	-18.5	-18.5	0.13 (1)	10.00		
AR-AS	0/4123	-18.5	-18.5	0.13 (1)	10.00		
AS-O	0/4123	-18.5	-18.5	0.13 (1)	10.00		

WEBS

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-255	-255	---	FRONT	VERT	TOTAL	---	C1
F	15-11-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
H	18-8-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
I	21-8-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
K	31-9-8	-255	-255	---	FRONT	VERT	TOTAL	---	C1
P	31-8-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
R	23-8-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
S	21-8-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	15-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
U	13-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
W	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Y	7-11-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
Z	9-11-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AA	11-11-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AB	13-11-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AC	17-11-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AD	23-8-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AE	25-8-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AF	27-8-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AG	29-8-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AH	1-11-4	-20	-20	---	FRONT	VERT	TOTAL	---	C1
AI	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AJ	7-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AK	9-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AL	11-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AM	17-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AN	19-8-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AO	25-8-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AP	27-8-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AQ	29-8-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AR	33-8-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AS	35-8-12	-20	-20	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

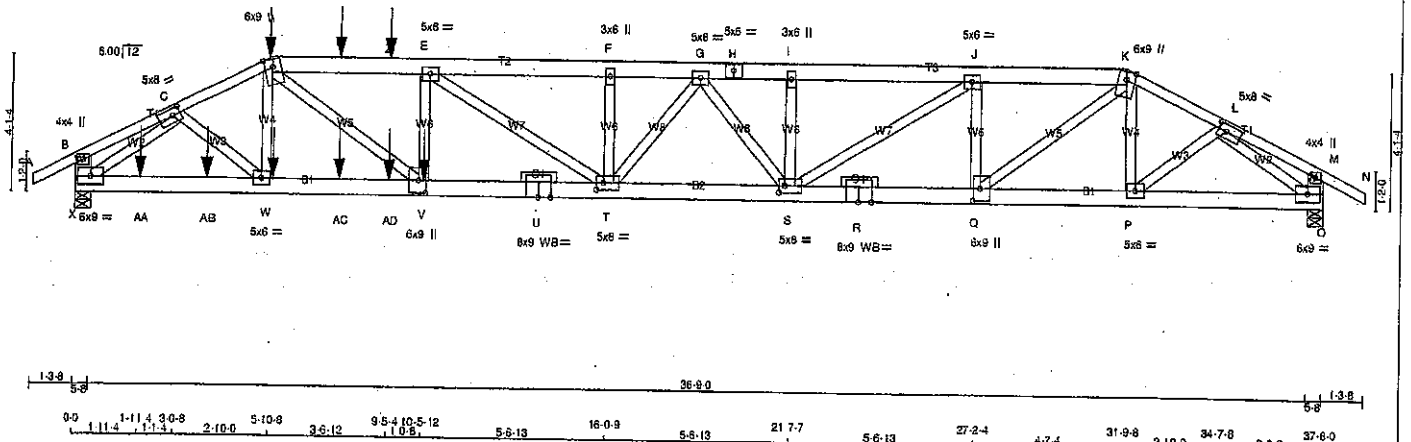
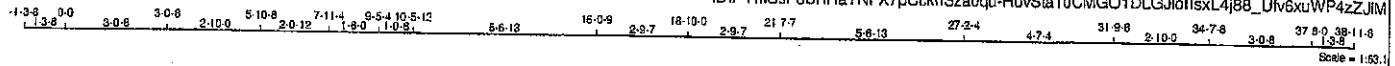


Structural component only
DWG# T-2108049 *m*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417413	T21Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Fri Mar 19 17:49:27 2021 Page 1
ID:PyMSp3BHha7NFX7pCcknSza0qu-HovSta10CMGO1DLGJlloisxL4j88_Ulv6xuWP4zZJIM



CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - H	2x6	DRY	No.2
H - K	2x6	DRY	No.2
K - N	2x4	DRY	No.2
N - R	2x6	DRY	No.2
R - O	2x6	DRY	No.2
O - M	2x6	DRY	2100F 1.8E
M - U	2x6	DRY	2100F 1.8E
U - R	2x6	DRY	2100F 1.8E
R - O	2x6	DRY	2100F 1.8E

ALL WEBS 2x4 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-D 1	12	SIDE(61.0)
K-N 1	12	TOP
D-H 2	12	SIDE(61.0)
H-K 2	12	TOP
X-B 2	12	TOP
O-M 2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
X-U 2	12	SIDE(197.3)
U-R 2	12	TOP
R-O 2	12	TOP
WEBS: (0.122"x3") SPIRAL NAILS		
2x4 1	6	SIDE(868.9)
E-V 1	2	
J-O 1	2	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	IN-SX	IN-SX
O 3487	0	0	5-8
X 5743	0	0	5-8

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW
O 2446	1638	0
X 4053	2709	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, X

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (CSI (LC))	MEMB.	FORCE (LBS)	MAX (LC)
FR-TO				FR-TO		
A-B 0/28	-91.8	-91.8	0.07 (1)	W-D -798/0	0.07 (1)	
B-C -2/1	-91.8	-91.8	0.07 (1)	P-K -428/0	0.04 (1)	
C-D -9334/0	-91.8	-91.8	0.37 (1)	V-E -108/43	0.01 (1)	
D-Y -13885/0	-91.8	-91.8	0.47 (1)	Q-J -2417/0	0.21 (1)	
Y-Z -13885/0	-91.8	-91.8	0.47 (1)	D-V 0/6742	0.80 (1)	
Z-E -13885/0	-91.8	-91.8	0.47 (1)	Q-K 0/4087	0.36 (1)	
E-F -12747/0	-91.8	-91.8	0.41 (1)	S-J 0/3346	0.30 (1)	
F-G -12747/0	-91.8	-91.8	0.30 (1)	C-W 0/1881	0.15 (1)	
G-H -10798/0	-91.8	-91.8	0.23 (1)	X-C -8922/0	0.75 (1)	
H-I -10798/0	-91.8	-91.8	0.23 (1)	P-L 0/833	0.07 (1)	
I-J -10798/0	-91.8	-91.8	0.32 (1)	L-O -5220/0	0.44 (1)	
J-K -7993/0	-91.8	-91.8	0.18 (1)	E-T -1117/0	0.31 (1)	
K-L -5347/0	-91.8	-91.8	0.16 (1)	S-I -382/0	0.03 (1)	
L-M 0/7	-91.8	-91.8	0.06 (1)	T-F -424/0	0.04 (1)	
M-N 0/28	-91.8	-91.8	0.07 (1)	T-G 0/1507	0.13 (1)	
X-B -257/0	0.0	0.0	0.01 (1)	G-S -1735/0	0.21 (1)	
O-M -244/0	0.0	0.0	0.01 (1)			

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (CSI (LC))
X-AA 0/7091	-18.5	-18.5	0.21 (1)
AA-AB 0/7091	-18.5	-18.5	0.21 (1)
AB-W 0/7091	-18.5	-18.5	0.21 (1)
W-AC 0/8417	-18.5	-18.5	0.28 (1)
AC-AD 0/8417	-18.5	-18.5	0.28 (1)
AD-V 0/8417	-18.5	-18.5	0.28 (1)
V-U 0/13884	-18.5	-18.5	0.42 (1)
U-T 0/13884	-18.5	-18.5	0.42 (1)
T-S 0/11842	-18.5	-18.5	0.35 (1)
S-R 0/7992	-18.5	-18.5	0.25 (1)
R-Q 0/7992	-18.5	-18.5	0.25 (1)
Q-P 0/4815	-18.5	-18.5	0.14 (1)
P-O 0/4149	-18.5	-18.5	0.13 (1)

LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
JT							
D 5-10-8	-255	-255	---	BACK	VERT	---	C1
V 10-5-12	-2879	-2879	---	BACK	VERT	---	C1
W 5-11-4	-21	-21	---	BACK	VERT	---	C1
Y 7-11-4	-76	-76	---	BACK	VERT	---	C1
Z 9-5-4	-76	-76	---	BACK	VERT	---	C1
AA 1-11-4	-20	-20	---	BACK	VERT	---	C1
AB 3-11-4	-21	-21	---	BACK	VERT	---	C1
AC 7-11-4	-21	-21	---	BACK	VERT	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL =	25.6 PSF
DL =	6.0 PSF
BOT CH. LL =	0.0 PSF
DL =	7.4 PSF
TOTAL LOAD =	39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/899 (0.29")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/845 (0.54")

CSI: TC=0.47/1.00 (D-E:1), BC=0.42/1.00 (T-V:1), WB=0.75/1.00 (C-X:1), SS=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (C) (INPUT = 0.90)

JSI METAL = 0.94 (U) (INPUT = 1.00)



Structural component only
DWG# T-2108050

JOB NAME 417413	TRUSS NAME T21Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Mar 19 17:49:27 2021 Page 2 ID:PYmSsP3BHh?NFX7pCdknSza0qu-HovSta10CMGO1DLGJlplIsxl4l88 Ufv6xuWP4zZJRM	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVap	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	8.0	2.50	2.75
D	TTWW+m	MT20	6.0	9.0	Edge	
E, G, J						
E	TMWW-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMW+w	MT20	3.0	6.0		
K	TTWW+m	MT20	6.0	9.0	Edge	
L	TMWW-t	MT20	5.0	8.0	2.50	2.75
M	TMVap	MT20	4.0	4.0		
O	BMVW-t	MT20	6.0	9.0		
P	BMWW-t	MT20	6.0	6.0		
Q	BMWW-t	MT20	6.0	8.0	4.50	2.50
R	BS-t	MT20	8.0	9.0		
S	BMWWW-t	MT20	5.0	8.0	2.50	2.00
T	BMWWW-t	MT20	5.0	8.0	2.50	2.50
U	BS-t	MT20	8.0	9.0		
V	BMWW-t	MT20	6.0	6.0	4.50	2.50
W	BMWW-t	MT20	5.0	6.0		
X	BMVW-t	MT20	6.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AD	9-5-4	-21	-21	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

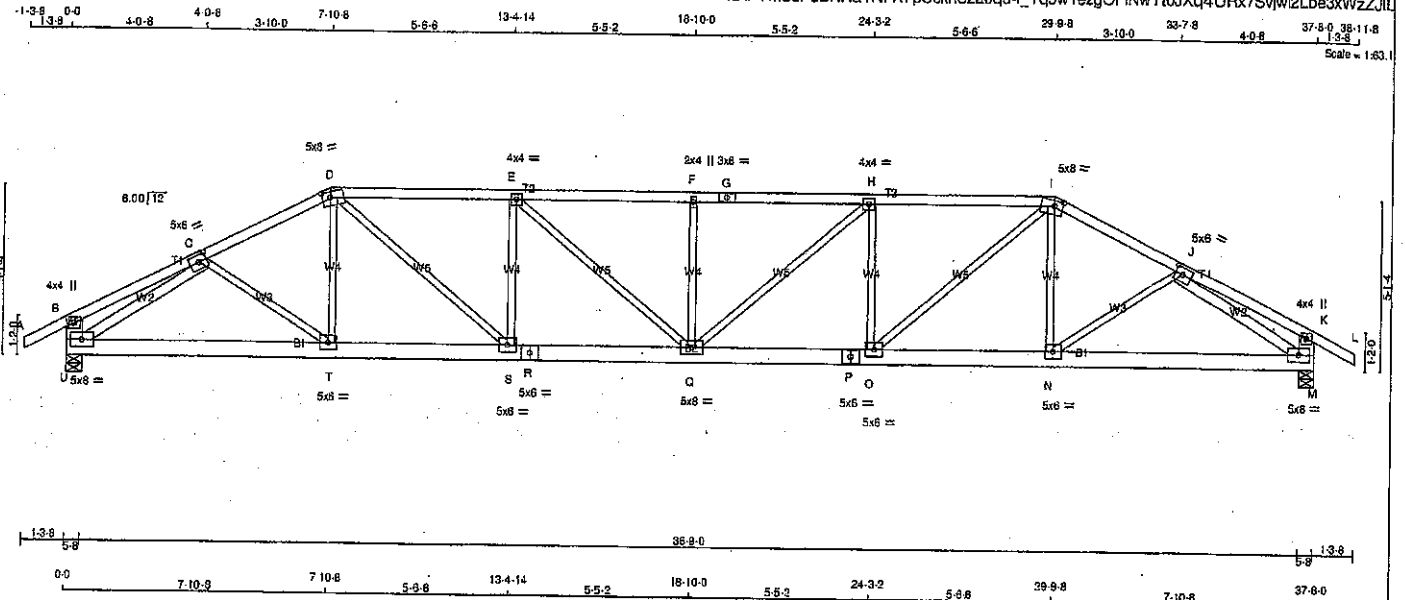


Structural component only
DWG# T-2108050

JOB NAME 417413	TRUSS NAME T22	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri Mar 19 17:49:28 2021 Page 1
ID:PYmSsP3BHha7NFX7pCcknSza0qu-l_Tq5w1ezgOFInwT10JXq4URx7SvWz2Lbe3xWzZJL



LUMBER

N.L.G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4 DRY	No.2
D - G	2x4 DRY	No.2
G - I	2x4 DRY	No.2
I - L	2x4 DRY	No.2
U - B	2x6 DRY	No.2
M - K	2x6 DRY	No.2
U - R	2x6 DRY	No.2
R - P	2x6 DRY	No.2
P - M	2x6 DRY	No.2

ALL WEBS	2x3 DRY	No.2
EXCEPT		
U - C	2x4 DRY	No.2
J - M	2x4 DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+P	MT20	4.0	4.0		
C TMWW-1	MT20	5.0	6.0	2.50	2.75
D TMWW-m	MT20	6.0	8.0	2.00	3.00
E TMWW-1	MT20	4.0	4.0		
F TMW+H	MT20	2.0	4.0		
G TS-1	MT20	3.0	6.0		
H TMWW-1	MT20	4.0	4.0		
I TMWW-m	MT20	5.0	8.0	2.00	3.00
J TMWW-1	MT20	5.0	6.0	2.50	2.75
K TMV+P	MT20	4.0	4.0		
M BMVW-1	MT20	5.0	8.0		
N O.S.T					
N BMVW-1	MT20	5.0	8.0		
P BS-1	MT20	5.0	8.0		
Q BMVW-1	MT20	5.0	8.0		
R BS-1	MT20	5.0	8.0		
U BMVW-1	MT20	5.0	8.0		

NOTES:

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD IN-SX
	VERT	HORZ	DOWN	HORZ		
U	2201	0	2201	0	5-8	5-8
M	2201	0	2201	0	5-8	5-8

UNFACTORED REACTIONS

1ST LOOSE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1554	1033 0	0 0	0 0	0 0	521 0	0 0
M	1554	1033 0	0 0	0 0	0 0	521 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (L/C)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (L/C)	UNBRACED LENGTH
FR-TO				FR-TO			
A-B	0/28	-91.8	-91.8 0.12 (1)	C-T	0/94	0.03 (4)	10.00
B-C	0/16	-91.8	-91.8 0.20 (1)	T-D	0/118	0.04 (4)	10.00
C-D	-3099/0	-91.8	-91.8 0.34 (1)	N-I	0/115	0.04 (4)	3.89
D-E	-3848/0	-91.8	-91.8 0.74 (1)	N-J	0/94	0.03 (4)	2.98
E-F	-4164/0	-91.8	-91.8 0.79 (1)	U-C	-3270/0	0.83 (1)	2.83
F-G	-4164/0	-91.8	-91.8 0.79 (1)	J-M	-3270/0	0.83 (1)	2.83
G-H	-4164/0	-91.8	-91.8 0.79 (1)	O-I	0/1435	0.32 (1)	2.83
H-I	-3848/0	-91.8	-91.8 0.74 (1)	D-S	0/1435	0.32 (1)	2.98
I-J	-3099/0	-91.8	-91.8 0.34 (1)	O-H	-843/0	0.30 (1)	3.89
J-K	0/16	-91.8	-91.8 0.20 (1)	S-E	-843/0	0.30 (1)	10.00
K-L	0/28	-91.8	-91.8 0.12 (1)	Q-H	0/419	0.09 (1)	10.00
U-B	-270/0	0.0	0.0 0.02 (1)	E-Q	0/419	0.09 (1)	7.81
M-K	-270/0	0.0	0.0 0.02 (1)	Q-F	-458/0	0.16 (1)	7.81
U-T	0/2885	-18.5	-18.5 0.39 (1)				10.00
T-S	0/2757	-18.5	-18.5 0.37 (1)				10.00
S-R	0/3848	-18.5	-18.5 0.51 (1)				10.00
R-Q	0/3848	-18.5	-18.5 0.51 (1)				10.00
Q-P	0/3848	-18.5	-18.5 0.51 (1)				10.00
P-O	0/3848	-18.5	-18.5 0.51 (1)				10.00
O-N	0/2757	-18.5	-18.5 0.37 (1)				10.00
N-M	0/2885	-18.5	-18.5 0.39 (1)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.24")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.44")

CSI: TO=0.79/1.00 (E-F:1), BC=0.51/1.00 (O-S:1), WB=0.83/1.00 (J-M:1), SS=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION	(PSI)	(PLI)	(PLI)
MAX MIN MAX MIN MAX MIN			
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (C) (INPUT = 0.90)
JSI METAL = 0.74 (C) (INPUT = 1.00)

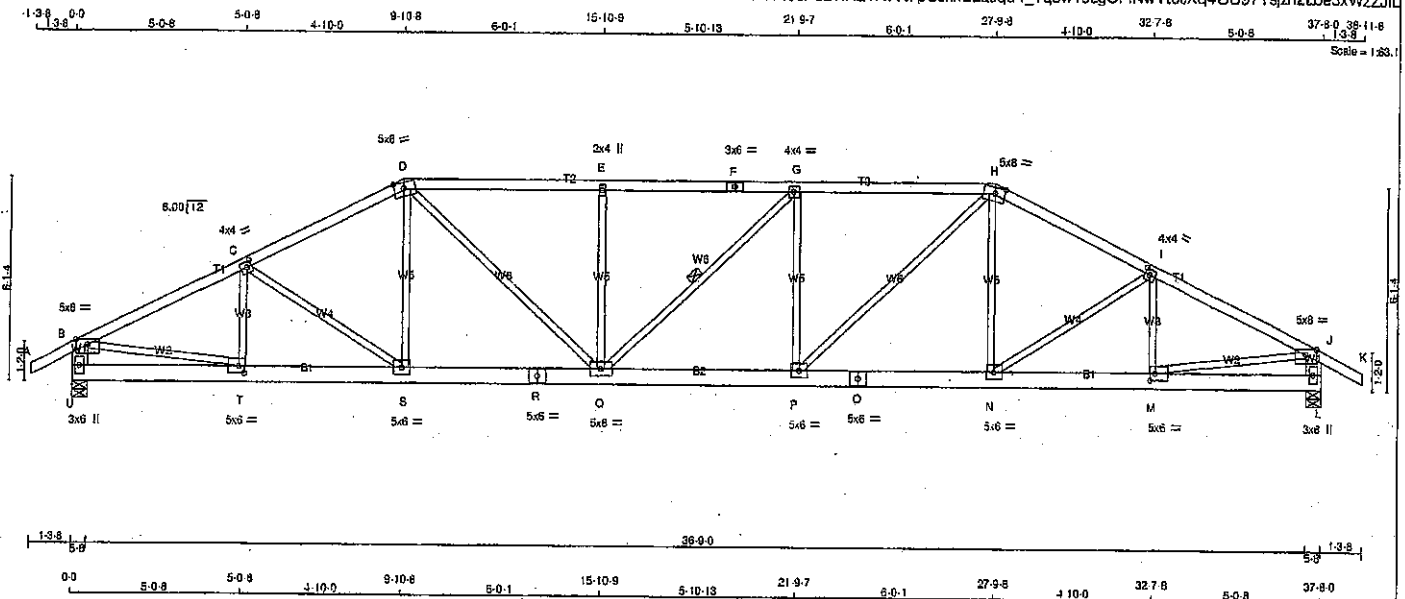


Structural component only
DWG# T-2108051

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417413	T23	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:PyMsSp3BHia?NFX7pCckmSza0qu-I_Tq5w1ezgOFInWtTt0JXq4UU97Tsjzh2Lbe3xWzZJIL



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
U - B	2x6	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	
U - R	2x6	DRY	No.2	SPF	
R - O	2x6	DRY	No.2	SPF	
O - L	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	Edge	
C TMVW-i	MT20	4.0	4.0	2.00	1.75
D TTWW-m	MT20	5.0	8.0	2.25	3.50
E TMVW-w	MT20	2.0	4.0		
F TS-i	MT20	3.0	6.0		
G TMVW-i	MT20	4.0	4.0		
H TTWW-m	MT20	5.0	8.0	2.25	3.50
I TMVW-i	MT20	4.0	4.0	2.00	1.75
J TMVW-p	MT20	5.0	8.0	Edge	
L BMV1+p	MT20	3.0	6.0		
M BMVW-i	MT20	5.0	6.0	2.50	2.00
N P S					
N BMVW-i	MT20	5.0	6.0		
O BS-i	MT20	5.0	6.0		
Q BMVW-w	MT20	5.0	6.0		
R BS-i	MT20	5.0	6.0		
T BMVW-i	MT20	5.0	6.0	2.50	2.00
U BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
U	2201	0	2201	0
L	2201	0	2201	0

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
U	1554	1033	0	0	0	0
L	1554	1033	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB. FORCE (LBS)
FR-TO		FROM TO		FR-TO
A-B	0.28	-91.8 -91.8	0.12 (1)	10.00
B-C	-3111 / 0	-91.8 -91.8	0.42 (1)	3.83
C-D	-2993 / 0	-91.8 -91.8	0.40 (1)	3.71
D-E	-3281 / 0	-91.8 -91.8	0.58 (1)	3.33
E-F	-3381 / 0	-91.8 -91.8	0.53 (1)	3.39
F-G	-3381 / 0	-91.8 -91.8	0.53 (1)	3.39
G-H	-3387 / 0	-91.8 -91.8	0.58 (1)	3.33
H-I	-2991 / 0	-91.8 -91.8	0.40 (1)	3.71
I-J	-3111 / 0	-91.8 -91.8	0.42 (1)	3.83
J-K	0.28	-91.8 -91.8	0.12 (1)	10.00
U-B	-2138 / 0	0.0 0.0	0.14 (1)	6.95
L-J	-2138 / 0	0.0 0.0	0.14 (1)	6.95
U-T	0 / 0	-18.5 -18.5	0.07 (1)	10.00
T-S	0.2802	-18.5 -18.5	0.39 (1)	10.00
S-R	0.2859	-18.5 -18.5	0.36 (1)	10.00
R-Q	0.2859	-18.5 -18.5	0.36 (1)	10.00
Q-P	0.3387	-18.5 -18.5	0.45 (1)	10.00
P-O	0.2857	-18.5 -18.5	0.36 (1)	10.00
O-N	0.2857	-18.5 -18.5	0.36 (1)	10.00
N-M	0.2802	-18.5 -18.5	0.39 (1)	10.00
M-L	0 / 0	-18.5 -18.5	0.07 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF CBC 2012 (2015 AMENDMENT)
- CSA 086-14
- TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.36")

CSI: TC=0.58/1.00 (G-H:1), BC=0.45/1.00 (P-Q:1),
WB=0.64/1.00 (J-M:1), SSI=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

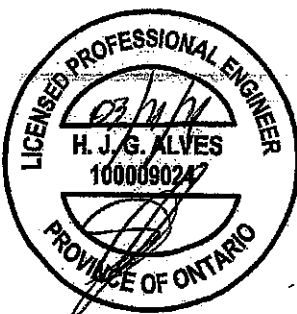
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.86 (M) (INPUT = 0.90)
JSI METAL= 0.64 (M) (INPUT = 1.00)



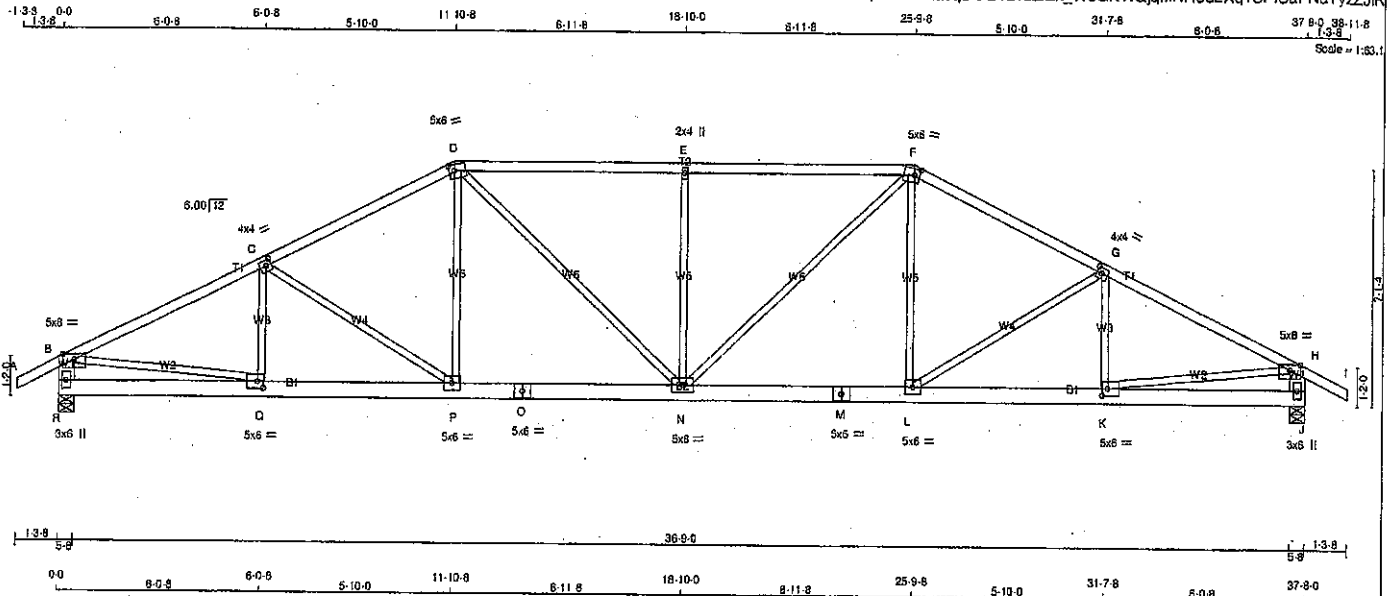
Structural component only
DWG# T-2108052

JOB NAME 417413	TRUSS NAME T24	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:PYmSsP3BHh2NFX7pCcknSza0qu-DB1DIG2Gk_W6GXVfQjnmH0d2Xq1SPICaFNdTyzZJK



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY
D - F	2x4	DRY
F - I	2x4	DRY
I - B	2x6	DRY
B - H	2x6	DRY
H - O	2x6	DRY
O - M	2x6	DRY
M - J	2x6	DRY

ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	1.75
C	TMVW-l	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-l	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1-p	MT20	3.0	6.0		
K	BMVW-l	MT20	5.0	6.0	2.50	2.00
L	BMVW-l	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMVW-w	MT20	5.0	8.0		
O	BS-t	MT20	5.0	6.0		
P	BMVW-l	MT20	5.0	6.0		
Q	BMVW-l	MT20	5.0	6.0	2.50	2.00
R	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES:

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	DOWN	HORZ	UPLIFT
R	2201	0	0	5-8
J	2201	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SO
R	1554	1033 '0	0 '0	0 '0	0 '0	521 0	0
J	1554	1033 '0	0 0	0 0	0 0	521 0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO				FR-TO			
A-B	0/28	-91.8	-91.8 0.12 (1)	Q-C	-288/0	0.07 (1)	0.07 (1)
B-C	-3171/0	-91.8	-91.8 0.62 (1)	C-P	-404/0	0.38 (1)	0.38 (1)
C-D	-2848/0	-91.8	-91.8 0.56 (1)	P-D	0/369	0.06 (1)	0.06 (1)
D-E	-2988/0	-91.8	-91.8 0.76 (1)	D-N	0/642	0.14 (1)	0.14 (1)
E-F	-2988/0	-91.8	-91.8 0.76 (1)	N-E	-785/0	0.84 (1)	0.84 (1)
F-G	-2848/0	-91.8	-91.8 0.56 (1)	N-F	0/642	0.14 (1)	0.14 (1)
G-H	-3171/0	-91.8	-91.8 0.62 (1)	L-F	0/369	0.06 (1)	0.06 (1)
H-I	0/28	-91.8	-91.8 0.12 (1)	L-G	-404/0	0.38 (1)	0.38 (1)
R-B	-2136/0	0.0	0.0 0.14 (1)	K-G	-287/0	0.07 (1)	0.07 (1)
J-H	-2136/0	0.0	0.0 0.14 (1)	B-Q	0/2884	0.85 (1)	0.85 (1)
R-Q	0/0	-18.5	-18.5 0.08 (4)	K-H	0/2884	0.85 (1)	0.85 (1)
Q-P	0/2881	-18.5	-18.5 0.39 (1)				
P-O	0/2526	-18.5	-18.5 0.35 (1)				
O-N	0/2526	-18.5	-18.5 0.35 (1)				
N-M	0/2526	-18.5	-18.5 0.35 (1)				
M-L	0/2526	-18.5	-18.5 0.35 (1)				
L-K	0/2881	-18.5	-18.5 0.39 (1)				
K-J	0/0	-18.5	-18.5 0.08 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.32")

CSI: TO=0.78/1.00 (D-E:1), BC=0.39/1.00 (P-Q:1), WB=0.65/1.00 (B-Q:1), SS=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

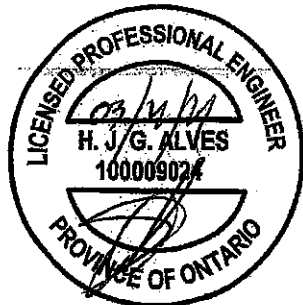
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	850	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)

JSI METAL= 0.55 (C) (INPUT = 1.00)



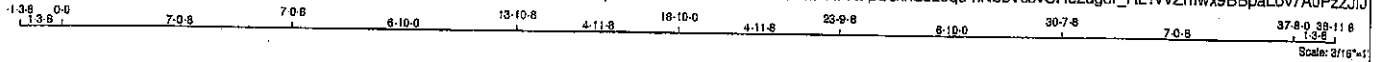
Structural component only
DWG# T-2108053

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417413	T25	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:PYmSsP3BHhA?NFX7pCcknSza0qu-hNbbVc3vUHzug3r_RL7vVZmw9BBpaLov7A0PzZJJU



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	
T - R	2x8	DRY	No.2	SPF	
N - L	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE	MT20	5.0	8.0	Edge
B TMVW-p	MT20	4.0	4.0	2.00 1.75
C TMVW-t	MT20	3.0	6.0	
D TS-t	MT20	5.0	6.0	2.25 2.00
E TTWW-m	MT20	2.0	4.0	
F TMW-w	MT20	5.0	6.0	2.25 2.00
G TTWW-m	MT20	3.0	6.0	
H TS-t	MT20	4.0	4.0	2.00 1.75
I TMVW-t	MT20	5.0	8.0	Edge
L BMV1-p	MT20	3.0	6.0	
M BMVW-t	MT20	5.0	6.0	2.50 2.00
N BS-t	MT20	5.0	6.0	
O BMVW-t	MT20	5.0	6.0	
P BMVW-t	MT20	5.0	6.0	
Q BMVW-t	MT20	5.0	6.0	
R BS-t	MT20	5.0	6.0	
S BMVW-t	MT20	5.0	6.0	2.50 2.00
T BMV1-p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	IN-SX	IN-SX
T 2201 0	2201 0	5.8	5.8
L 2201 0	2201 0	5.8	5.8

UNFACTORED REACTIONS

1ST LOASE	MAX. MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT COMBINED	SNOW	LIVE	PERM. LIVE	
T 1554 1033 0	0 0	0 0	521 0	0 0
L 1554 1033 0	0 0	0 0	521 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO	FR-TO		
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	S-C	-202 / 49 0.06 (1)
B-C	-3196 / 0	-91.8	-91.8 0.89 (1)	3.06	C-Q	-621 / 0 0.87 (1)
C-D	-2675 / 0	-91.8	-91.8 0.76 (1)	3.43	Q-E	0 / 464 0.10 (1)
D-E	-2675 / 0	-91.8	-91.8 0.76 (1)	3.43	E-P	0 / 382 0.08 (1)
E-F	-2563 / 0	-91.8	-91.8 0.37 (1)	3.99	P-F	-563 / 0 0.88 (1)
F-G	-2563 / 0	-91.8	-91.8 0.37 (1)	3.99	G-O	0 / 382 0.08 (1)
G-H	-2675 / 0	-91.8	-91.8 0.76 (1)	3.43	O-G	0 / 464 0.10 (1)
H-I	-2675 / 0	-91.8	-91.8 0.76 (1)	3.43	O-I	-621 / 0 0.87 (1)
I-J	-3196 / 0	-91.8	-91.8 0.88 (1)	3.06	M-I	-202 / 49 0.06 (1)
J-K	0 / 28	-91.8	-91.8 0.12 (1)	10.00	B-S	0 / 2906 0.65 (1)
T-B	-2134 / 0	0.0	0.0 0.14 (1)	6.96	M-J	0 / 2906 0.65 (1)
L-J	-2134 / 0	0.0	0.0 0.14 (1)	6.96		
T-S	0 / 0	-18.5	-18.5 0.10 (4)	10.00		
S-R	0 / 2888	-18.5	-18.5 0.40 (1)	10.00		
R-Q	0 / 2888	-18.5	-18.5 0.40 (1)	10.00		
Q-P	0 / 2367	-18.5	-18.5 0.33 (1)	10.00		
P-O	0 / 2367	-18.5	-18.5 0.33 (1)	10.00		
O-N	0 / 2888	-18.5	-18.5 0.40 (1)	10.00		
N-M	0 / 2888	-18.5	-18.5 0.40 (1)	10.00		
M-L	0 / 0	-18.5	-18.5 0.10 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/898 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CSI: TC=0.88/1.00 (I-J:1), BC=0.40/1.00 (M-O:1), WB=0.87/1.00 (I-O:1), SS=0.28/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

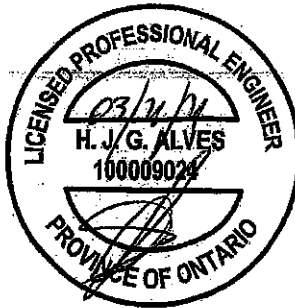
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PU) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (J) (INPUT = 0.90)

JSI METAL = 0.86 (S) (INPUT = 1.00)

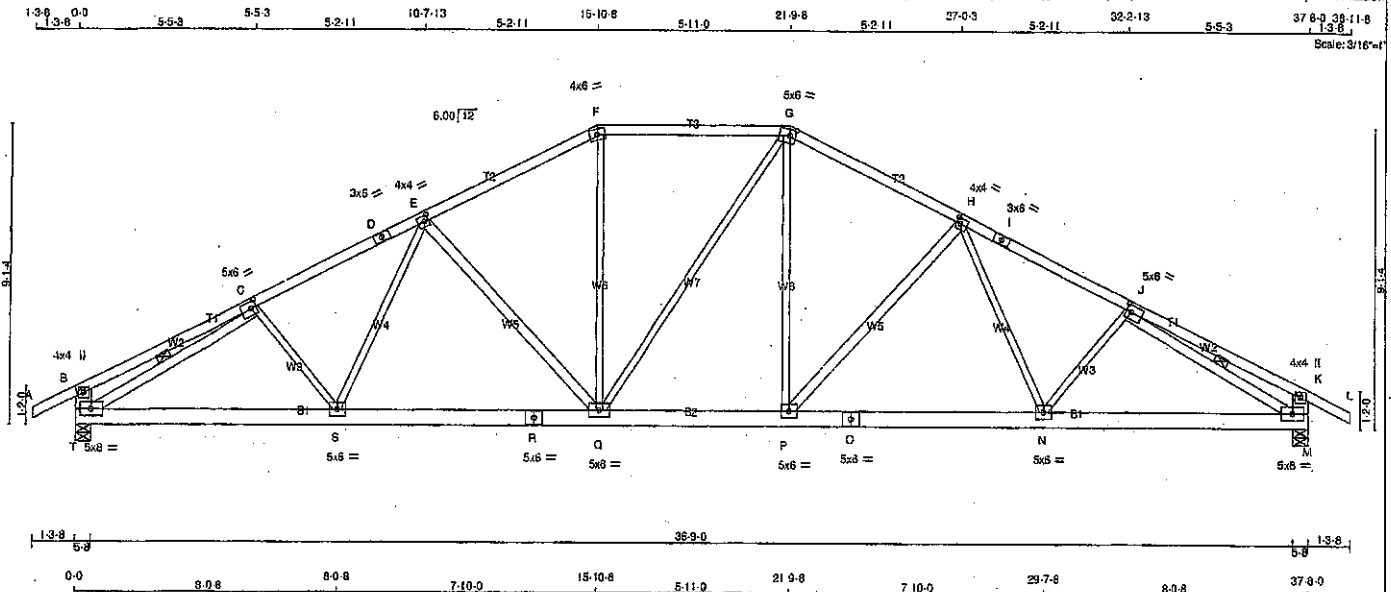


Structural component only
DWG# T-2108054

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417413	T26	8	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Mar 19 17:49:31 2021 Page 1
ID:PYmSsP3BH1a?NFX7pCcknSza0qu-9Z9zy4XFbnqWqe2Y8sESi51nLVawJ7V1ZsKyRZZJl



TOTAL WEIGHT = 8 X 190 = 1520 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - D 2x4 DRY No.2

D - F 2x4 DRY No.2

F - G 2x4 DRY No.2

G - I 2x4 DRY No.2

I - L 2x4 DRY No.2

T - B 2x6 DRY No.2

M - K 2x6 DRY No.2

T - R 2x6 DRY No.2

R - O 2x6 DRY No.2

O - M 2x6 DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

S - Q 2x4 DRY No.2

P - H 2x4 DRY No.2

T - C 2x4 DRY No.2

J - M 2x4 DRY No.2

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

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SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION MAXIMUM FACTORED INPUT REQRD

JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX

T 2201 0 0 0 5-8 5-8

M 2201 0 0 0 5-8 5-8

UNFACTORED REACTIONS

1ST LOOSE MAX-MIN COMPONENT REACTIONS

JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL

T 1554 1033 0 0 0 0 0 0 0 0

M 1554 1033 0 0 0 0 0 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-T, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. MAX. FACTORED FORCE (LBS) VERT. LOAD LC1 MAX. MAX. UNBRACED LENGTH FR-TO

A-B 0/28 -91.8 -91.8 0.12 (1) 10.00 C-S -112/38 0.04 (1)

B-C 0/20 -91.8 -91.8 0.32 (1) 10.00 S-E 0/260 0.08 (1)

C-D -3053/0 -91.8 -91.8 0.43 (1) 3.67 E-Q -671/0 0.65 (1)

D-E -3053/0 -91.8 -91.8 0.43 (1) 3.67 Q-F 0/635 0.14 (1)

E-F -2470/0 -91.8 -91.8 0.39 (1) 4.05 Q-G 0/4 0.00 (1)

F-G -2196/0 -91.8 -91.8 0.49 (1) 4.09 P-G 0/630 0.14 (1)

G-H -2468/0 -91.8 -91.8 0.39 (1) 4.05 P-H -673/0 0.65 (1)

H-I -3054/0 -91.8 -91.8 0.43 (1) 3.67 H-N 0/264 0.08 (1)

I-J -3054/0 -91.8 -91.8 0.43 (1) 3.67 N-J -112/38 0.04 (1)

J-K 0/20 -91.8 -91.8 0.32 (1) 10.00 T-C -3335/0 0.83 (1)

K-L 0/28 -91.8 -91.8 0.12 (1) 10.00 J-M -3336/0 0.63 (1)

T-B -324/0 0.0 0.0 0.02 (1) 7.81

M-K -324/0 0.0 0.0 0.02 (1) 7.81

T-S 0/2802 -18.5 -18.5 0.41 (1) 10.00

S-R 0/2830 -18.5 -18.5 0.37 (1) 10.00

R-Q 0/2830 -18.5 -18.5 0.37 (1) 10.00

Q-P 0/2194 -18.5 -18.5 0.31 (1) 10.00

P-O 0/2629 -18.5 -18.5 0.37 (1) 10.00

O-N 0/2629 -18.5 -18.5 0.37 (1) 10.00

N-M 0/2802 -18.5 -18.5 0.41 (1) 10.00

PLATES (table is in inches)

JT TYPE PLATES

B TMV+P MT20 4.0 4.0

C TMWW+ MT20 5.0 6.0 2.50 2.00

D TS+ MT20 3.0 6.0

E TMWW+ MT20 4.0 4.0 2.00 1.50

F TTW+ MT20 4.0 6.0

G TTWW+ MT20 5.0 6.0 2.25 2.00

H TTWW+ MT20 4.0 4.0 2.00 1.50

I TS+ MT20 3.0 6.0

J TMWW+ MT20 5.0 6.0 2.50 2.00

K TMV+P MT20 4.0 4.0

M BMVW+ MT20 5.0 8.0

N, P, S

O BMWW+ MT20 5.0 8.0

Q BS+ MT20 5.0 8.0

R BMWW+ MT20 5.0 8.0

Q BS+ MT20 5.0 8.0

T BMVW+ MT20 5.0 8.0

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019

- PART 9 OF CBC 2012 (2019 AMENDMENT)

- CSA 188-14

- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")

CALCULATED VERT. DEFL.(LL) = L/999 (0.13")

ALLOWABLE DEFL.(TL) = L/360 (1.26")

CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.49/1.00 (F-G:1), BC=0.41/1.00 (S-T:1),

WB=0.66/1.00 (H-P:1), SS=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PS) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

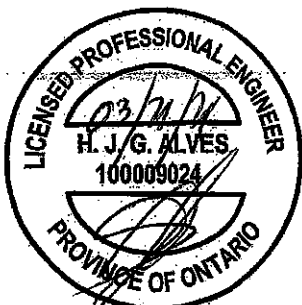
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.85 (M) (INPUT = 0.90)

JSI METAL = 0.75 (J) (INPUT = 1.00)

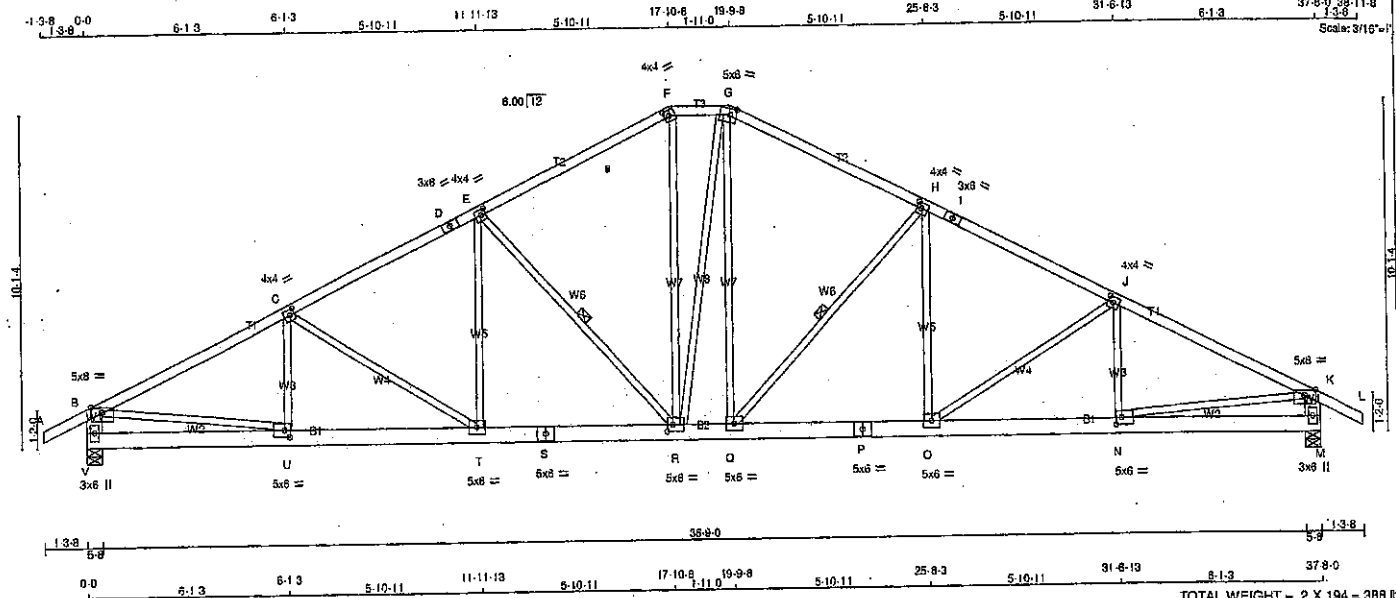


Structural component only
DWG# T-2108055

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417413	T27	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitel Industries, Inc. Fri Mar 19 17:49:32 2021 Page 1
ID:PYmSsP3BHha?NFX7pCcknSza0qu-dmLwI580vvg7_DE6rNT?weAWkgfmZeGDch4HzZJH



TOTAL WEIGHT = 2 X 194 = 388 lb (M/F)

LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	8.0	Edge	
C, E, H, J						
C	TMWV-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	5.0		
F	TTW-h	MT20	4.0	4.0	2.00	1.75
G	TTWV-m	MT20	5.0	8.0	2.25	2.25
I	TS-t	MT20	3.0	5.0		
K	TMWV-p	MT20	5.0	8.0	Edge	
M	BMV1-p	MT20	3.0	8.0		
N	BMWV-t	MT20	5.0	6.0	2.50	2.00
O, Q, T						
O	BMWV-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	8.0		
R	BMWV-t	MT20	5.0	8.0	2.50	2.00
S	BS-t	MT20	5.0	8.0		
U	BMWV-t	MT20	5.0	6.0	2.50	2.00
V	BMV1-p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REORD BRG IN-SX
JT	VERT	HORZ	DOWN	UPLIFT
V	2201	0	0	5-8
M	2201	0	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW					
V	1554	1033	0	0	0	521	0
M	1554	1033	0	0	0	521	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-R, H-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0/28	-91.8	-91.8 0.12 (1)	10.00	U-C	-264/8	0.07 (1)
B-C	-3181/0	-91.8	-91.8 0.82 (1)	3.42	C-T	-349/0	0.32 (1)
C-D	-2857/0	-91.8	-91.8 0.50 (1)	3.72	T-E	0/293	0.07 (1)
D-E	-2857/0	-91.8	-91.8 0.50 (1)	3.72	E-R	-823/0	0.48 (1)
E-F	-2272/0	-91.8	-91.8 0.47 (1)	4.10	R-F	0/702	0.16 (1)
F-G	-2017/0	-91.8	-91.8 0.08 (1)	4.71	R-G	0/69	0.02 (1)
G-H	-2258/0	-91.8	-91.8 0.47 (1)	4.11	C-G	0/628	0.14 (1)
H-I	-2862/0	-91.8	-91.8 0.50 (1)	3.71	Q-H	-847/0	0.47 (1)
I-J	-2862/0	-91.8	-91.8 0.50 (1)	3.71	O-H	0/320	0.07 (1)
J-K	-3159/0	-91.8	-91.8 0.82 (1)	3.42	O-J	-342/0	0.32 (1)
K-L	0/28	-91.8	-91.8 0.12 (1)	10.00	N-J	-272/5	0.07 (1)
V-B	-2139/0	0.0	0.0 0.14 (1)	6.95	B-U	0/2871	0.65 (1)
M-K	-2138/0	0.0	0.0 0.14 (1)	6.95	N-K	0/2869	0.65 (1)
V-U	0/0	-18.5	-18.5 0.08 (4)	10.00			
U-T	0/2848	-18.5	-18.5 0.40 (1)	10.00			
T-S	0/2555	-18.5	-18.5 0.37 (1)	10.00			
S-R	0/2555	-18.5	-18.5 0.37 (1)	10.00			
R-Q	0/2004	-18.5	-18.5 0.31 (1)	10.00			
Q-P	0/2559	-18.5	-18.5 0.35 (1)	10.00			
P-O	0/2559	-18.5	-18.5 0.35 (1)	10.00			
O-N	0/2845	-18.5	-18.5 0.39 (1)	10.00			
N-M	0/0	-18.5	-18.5 0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.0	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BOBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.28")
CALCULATED VERT. DEFL. (LL) = U/999 (0.15")
ALLOWABLE DEFL. (TL) = L/360 (1.28")
CALCULATED VERT. DEFL. (TL) = U/999 (0.29")

CSK: TC=0.62/1.00 (B-C:1), BC=0.40/1.00 (T-U:1), WB=0.65/1.00 (B-U:1), SS=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (F) (INPUT = 0.90)
JSI METAL = 0.65 (U) (INPUT = 1.00)

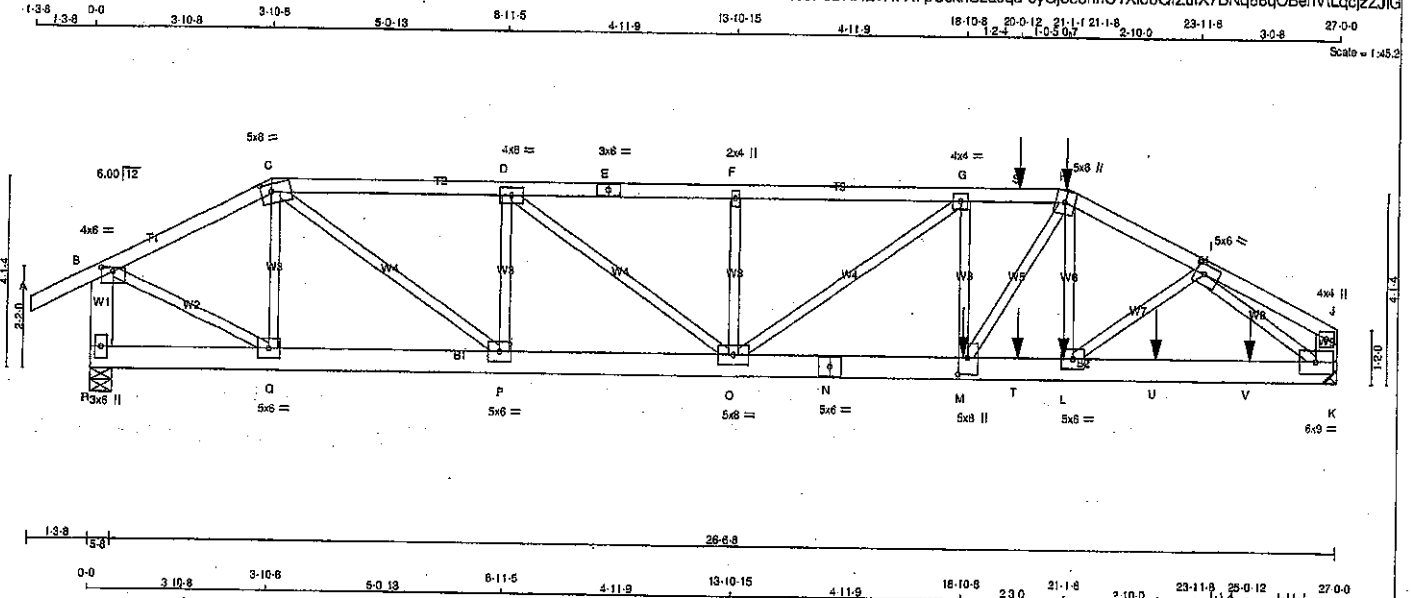


Structural component only
DWG# T-2108056

JOB NAME 417413	TRUSS NAME T28	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 M/Tek Industries, Inc. Fri Mar 19 17:49:39 2021 Page 1
ID:PYmSeP3BHha?NFX7pCcknSza0qu-6yGj8e5nnC1X18oQZuIX7BNq88qOBenVtLqczZJIG



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
R - B	2x6	DRY	No.2	SPF	
K - J	2x6	DRY	No.2	SPF	
R - N	2x6	DRY	No.2	SPF	
N - K	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - E	12	TOP
E - H	12	SIDE(61.0)
H - J	12	SIDE(61.0)
R - B	12	TOP
K - J	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
R - N	12	TOP
N - K	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	
G - M	3	SIDE(660.0)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INLET BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
R	2715	0	2715	0	5-8	5-8
K	4212	0	4212	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 4-0.

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
R	1914	1232.0	0.0	0.0	0.0	522.0	0.0
K	2972	1987.0	0.0	0.0	0.0	985.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)
FR-TO				
A-B	0/28	-91.8	-91.8	0.07 (1)
B-C	-2823/0	-91.8	-91.8	0.17 (1)
C-D	-5074/0	-91.8	-91.8	0.30 (1)
D-E	-6828/0	-91.8	-91.8	0.39 (1)
E-F	-6828/0	-91.8	-91.8	0.39 (1)
F-G	-6828/0	-91.8	-91.8	0.34 (1)
G-H	-7808/0	-91.8	-91.8	0.28 (1)
H-I	-7808/0	-91.8	-91.8	0.28 (1)
I-J	-6832/0	-91.8	-91.8	0.23 (1)
J-K	-2/2	-91.8	-91.8	0.07 (1)
R-B	-2694/0	0.0	0.0	0.11 (1)
K-J	-132/0	0.0	0.0	0.00 (1)
R-Q	0/0	-18.5	-18.5	0.03 (4)
Q-P	0/2494	-18.5	-18.5	0.19 (1)
P-O	0/5074	-18.5	-18.5	0.37 (1)
O-N	0/7808	-18.5	-18.5	0.59 (1)
N-M	0/7808	-18.5	-18.5	0.59 (1)
M-T	0/6117	-18.5	-18.5	0.48 (1)
T-L	0/6117	-18.5	-18.5	0.48 (1)
L-U	0/5250	-18.5	-18.5	0.39 (1)
U-V	0/5250	-18.5	-18.5	0.39 (1)
V-K	0/5250	-18.5	-18.5	0.39 (1)

JT	SPECIFIED CONCENTRATED LOADS (LBS)		FACE	DIR.	TYPE	HEEL	CONN.
	LOC.	MAX.					
H	21-11	-263	---	FRONT	VERT	TOTAL	C1
L	21-0-12	-21	---	FRONT	VERT	TOTAL	C1
M	18-10-8	-2272	---	FRONT	VERT	TOTAL	C1
S	20-0-12	-76	---	FRONT	VERT	TOTAL	C1
T	20-0-12	-21	---	FRONT	VERT	TOTAL	C1
U	23-0-12	-21	---	FRONT	VERT	TOTAL	C1
V	25-0-12	-20	---	FRONT	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.90")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/380 (0.90")
CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.39/1.00 (D-F:1), BC=0.59/1.00 (M-D:1), WB=0.78/1.00 (H-K:1), SS=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

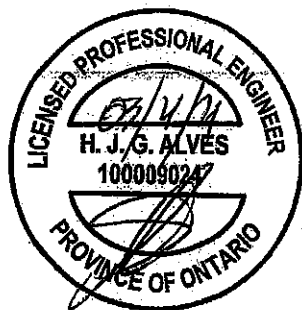
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (1) (INPUT = 0.90)
JSI METAL = 0.79 (1) (INPUT = 1.00)



Structural component only
DWG# T-2108057/12

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417413	T28	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Fri Mar 19 17:49:33 2021 Page 2
ID:PYmSsP3BHha7NFX7pCcknSza0qu-6yG8e5nnC1Xl8qQZulX7BNq88gOBenVtLqclzZJIG

PLATES (table is in inches)

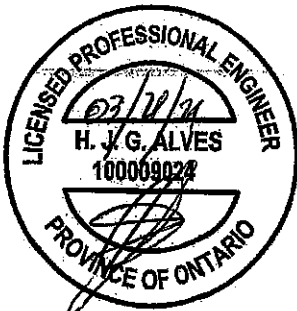
JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	4.0	6.0	1.00	3.00
C	TTWW-m	MT20	5.0	8.0	2.00	2.75
D	TMWW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	6.0		
F	TMW-w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.25
J	TMV-p	MT20	4.0	4.0		
K	BMVW-t	MT20	6.0	9.0		
L, P, Q						
L	BMW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	8.0	4.25	2.50
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
R	BMV-t	MT20	3.0	6.0		

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

NOTES- (1)

- 1) Lateral braces to be a minimum of 2X4 SPF #2.



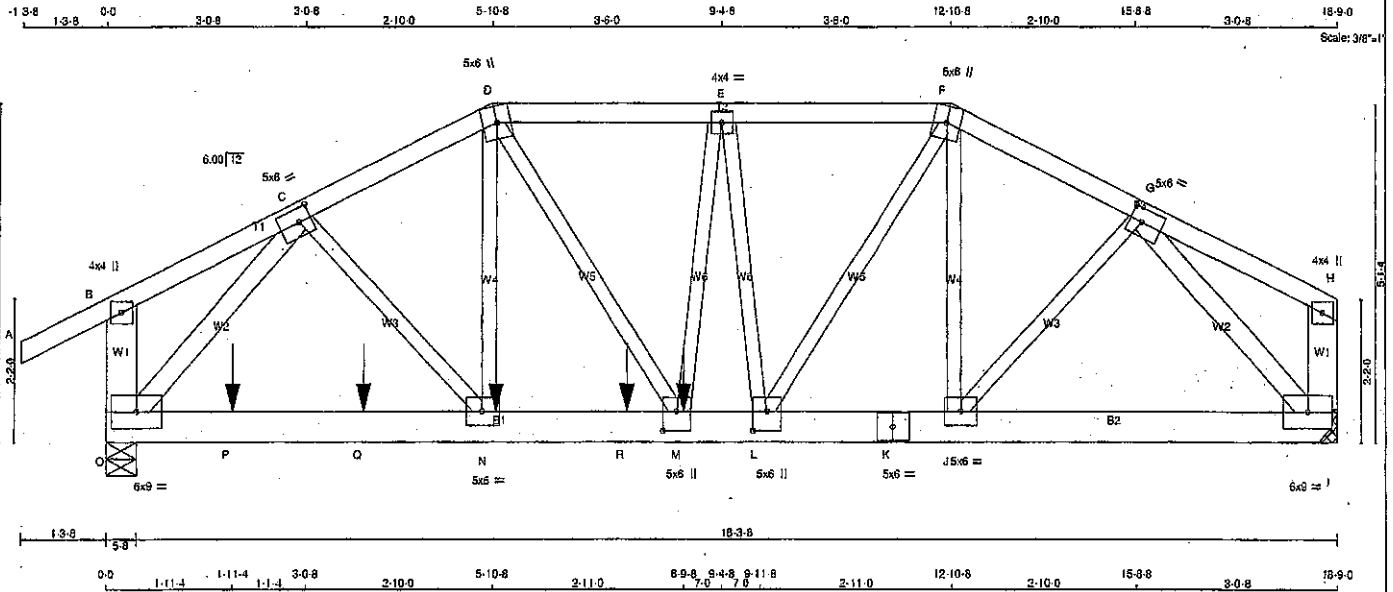
Structural component only
DWG# T-2108057 *ML*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417413	T29	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MTEK Industries, Inc. Fri Mar 19 17:49:38 2021 Page 1

ID:PYmSsP3BHha?NFX7pCcknSza0qu-6yGj9e5nnC1Xl8oQZuiX7BP286jOCmVLqcZJIG



LUMBER

N. L. G. A. RULES

CHORDS SIZE LUMBER

A - D 2x4 DRY No.2

D - F 2x4 DRY No.2

F - H 2x4 DRY No.2

O - B 2x6 DRY No.2

I - H 2x6 DRY No.2

O - K 2x6 DRY No.2

K - I 2x6 DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

O - C 2x4 DRY No.2

G - I 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE PLATES W LEN Y X

B TMV+p MT20 4.0 4.0

C TMVW+ MT20 5.0 6.0 2.50 2.25

D TTWW+m MT20 5.0 6.0

E TMVW+ MT20 4.0 4.0

F TTWW+m MT20 5.0 6.0

G TMVW+ MT20 5.0 6.0 2.50 2.25

H TMV+p MT20 4.0 4.0

I BMVW+ MT20 6.0 9.0

J BMVW+ MT20 5.0 6.0

K BS+ MT20 5.0 6.0

L BMVW+ MT20 5.0 6.0 3.50 2.50

M BMVW+ MT20 5.0 6.0 3.50 2.50

N BMVW+ MT20 5.0 6.0

O BMVW+ MT20 6.0 9.0

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED MAXIMUM FACTORED INPUT REQD

GROSS REACTION GROSS REACTION BRG BRG

JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX

I 2180 0 2180 0 0 5-8 5-8

O 2999 0 2999 0 0 5-8 5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

1ST CASE MAX MIN COMPONENT REACTIONS

JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL

I 1521 1031 0 0 0 0 0 0 0 0

O 2102 1444 0 0 0 0 0 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.55 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. MAX. FACTORED FORCE VERT. LOAD LC1 MAX MAX. FACTORED

(LBS) (PLF) CSI (LC) UNBRAC MEMB. MAX. FACTORED

FR-TO FROM TO LENGTH FR-TO

A-B 0 / 28 -91.8 -91.8 0.13 (1) 10.00 N-D 0 / 432 0.11 (1)

B-C 0 / 11 -91.8 -91.8 0.11 (1) 10.00 J-F -451 / 0 0.17 (1)

C-D -3225 / 0 -91.8 -91.8 0.25 (1) 3.67 D-M 0 / 888 0.22 (1)

D-E -3328 / 0 -91.8 -91.8 0.32 (1) 3.55 M-E 0 / 639 0.16 (1)

E-F -3124 / 0 -91.8 -91.8 0.29 (1) 3.67 E-L -1009 / 0 0.38 (1)

F-G -2576 / 0 -91.8 -91.8 0.22 (1) 4.08 L-F 0 / 1593 0.39 (1)

G-H 0 / 12 -91.8 -91.8 0.12 (1) 10.00 C-N 0 / 957 0.24 (1)

O-B -235 / 0 0.0 0.0 0.02 (1) 7.81 O-C -3532 / 0 0.71 (1)

I-H -107 / 0 0.0 0.0 0.01 (1) 7.81 J-G 0 / 715 0.18 (1)

O-P 0 / 2237 -18.5 -18.5 0.87 (1) 10.00 G-I -2706 / 0 0.57 (1)

P-O 0 / 2237 -18.5 -18.5 0.87 (1) 10.00

Q-N 0 / 2237 -18.5 -18.5 0.87 (1) 10.00

N-R 0 / 2889 -18.5 -18.5 0.73 (1) 10.00

R-M 0 / 2889 -18.5 -18.5 0.73 (1) 10.00

M-L 0 / 3234 -18.5 -18.5 0.65 (1) 10.00

L-K 0 / 2300 -18.5 -18.5 0.34 (1) 10.00

K-J 0 / 2300 -18.5 -18.5 0.34 (1) 10.00

J-I 0 / 1817 -18.5 -18.5 0.28 (1) 10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT LOC. LC1 MAX. MAX+ FACE DIR. TYPE HEEL CONN.

M 8-9-8 -1208 -1208 --- FRONT VERT TOTAL --- C1

N 5-11-4 -214 -214 --- FRONT VERT TOTAL --- C1

P 1-11-4 -225 -225 --- FRONT VERT TOTAL --- C1

Q 3-11-4 -214 -214 --- FRONT VERT TOTAL --- C1

R 7-11-4 -214 -214 --- FRONT VERT TOTAL --- C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.63")

CALCULATED VERT. DEFL. (LL) = L/999 (0.07")

ALLOWABLE DEFL. (TL) = L/380 (0.63")

CALCULATED VERT. DEFL. (TL) = L/899 (0.13")

CSI: TC=0.32/1.00 (D-E:1), BC=0.73/1.00 (M-N:1),

WB=0.71/1.00 (C-O:1), SS=0.30/1.00 (N-O:1)

DCL LUMBER=1.00 NAIL=1.00 LB BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PS) (PL)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (L) (INPUT = 0.90)

JSI METAL = 0.78 (C) (INPUT = 1.00)



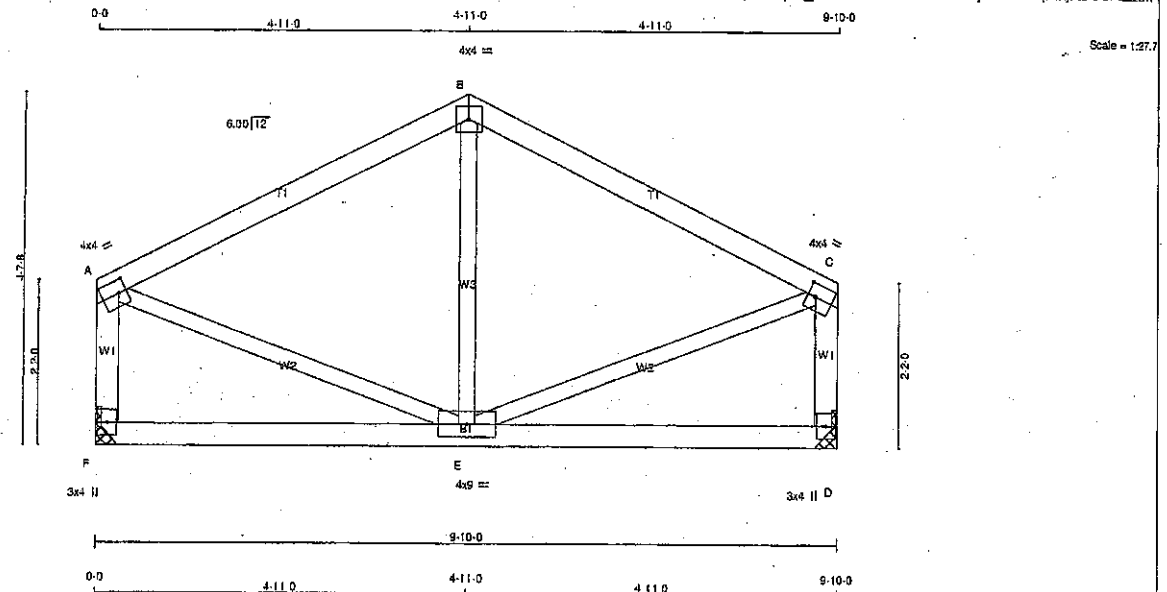
Structural component only

DWG# T-2108058

JOB NAME 417413	TRUSS NAME T30	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 39 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4 DRY	No.2		
B - C	2x4 DRY	No.2		
F - A	2x4 DRY	No.2		
D - C	2x4 DRY	No.2		
F - D	2x4 DRY	No.2		

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A - B	TMVW-1	MT20	4.0	4.0	2.00	1.25
B - C	TMVW-1	MT20	4.0	4.0		
C - D	TMVW-1	MT20	4.0	4.0	2.00	1.25
D - E	BMV1-p	MT20	3.0	4.0		
E - F	BMVWW-1	MT20	4.0	9.0		
F - G	BMV1-p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT HORZ	DOWN HORZ UPLIFT	IN-SX	IN-SX
F 542 0	542 0 0	MECHANICAL	
D 542 0	542 0 0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 3-8, JOINT D = 3-8.

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT COMBINED						
F 383	252'0	0'0	0'0	0'0	132'0	0'0
D 383	252'0	0'0	0'0	0'0	132'0	0'0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE	FORCE	(PLF)	CSI (LC)	UNBRAC	LENGTH	FR-TO	FORCE
FR-TO	(LBS)	(LBS)						(LBS)
A-B	-358 / 0	-91.8	-91.8	0.28 (1)	6.25	E-B	-131 / 36	0.04 (1)
B-C	-358 / 0	-91.8	-91.8	0.28 (1)	6.25	A-E	0'343	0.08 (1)
F-A	-507 / 0	0.0	0.0	0.08 (1)	7.81	E-C	0'343	0.08 (1)
D-C	-507 / 0	0.0	0.0	0.08 (1)	7.81			
F-E	0 / 0	-18.5	-18.5	0.13 (4)	10.00			
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 6.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.33")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.33")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.28/1.00 (A-B:1), BC=0.13/1.00 (E-F:4), WB=0.09/1.00 (D-E:1), SS=0.18/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL = 0.250 inches

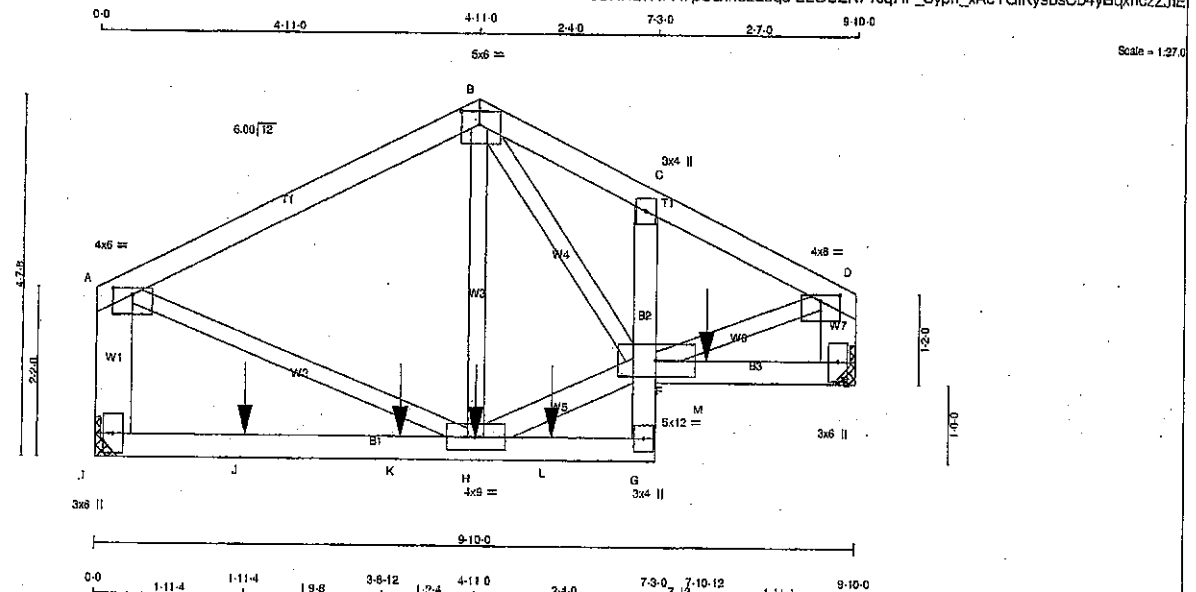
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.52 (C) (INPUT = 0.90)
JSI METAL = 0.18 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108059

JOB NAME 417413	TRUSS NAME T31	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



TOTAL WEIGHT = 46 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
I - A	2x8	DRY	No.2
E - D	2x8	DRY	No.2
I - G	2x4	DRY	No.2
G - C	2x4	DRY	No.2
F - E	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
H - F	2x4	DRY	No.2

PLATES (table is in inches)			
JT TYPE	PLATES	W	LEN Y X
A	TMVW-p	MT20	4.0 5.0 1.00 3.00
B	TTWW-p	MT20	5.0 5.0 2.00 2.75
C	TMV-p	MT20	3.0 4.0
D	TMVW-p	MT20	4.0 5.0 1.00 3.00
E	BMV1-p	MT20	3.0 5.0
F	BMVWW-W	MT20	5.0 12.0
G	BMV-p	MT20	3.0 4.0
H	BMVWW-W	MT20	4.0 9.0
I	BMV1-p	MT20	3.0 5.0

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG	
I	1179 0	1179 0	0	MECHANICAL	
E	1165 0	1165 0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I, E. MINIMUM BEARING LENGTH AT JOINT I = 3-8, JOINT E = 3-8.

UNFACTORED REACTIONS		1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE PERM. LIVE WIND DEAD SOIL
I	830	567.0	0.0 0.0 0.0 283.0 0.0
E	820	559.0	0.0 0.0 0.0 281.0 0.0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF) CSI (LC)	MEMB.	FORCE (LBS) CSI (LC)
FR-TO				FR-TO	
A-B	-977.0	-91.8	-91.8 0.48 (1) 5.80	F-D	0.1238 0.31 (1)
B-C	-1301.0	-91.8	-91.8 0.08 (1) 5.58	H-B	0.274 0.07 (1)
C-D	-1315.0	-91.8	-91.8 0.12 (1) 5.49	A-H	0.934 0.23 (1)
I-A	-993.0	0.0	0.0 0.08 (1) 7.81	H-F	0.919 0.16 (1)
E-D	-1070.0	0.0	0.0 0.08 (1) 7.81	B-F	0.514 0.13 (1)
I-J	0.0	-18.5	-18.5 0.50 (1) 10.00		
J-K	0.0	-18.5	-18.5 0.50 (1) 10.00		
K-H	0.0	-18.5	-18.5 0.50 (1) 10.00		
H-L	0.38	-18.5	-18.5 0.51 (1) 10.00		
L-G	0.38	-18.5	-18.5 0.51 (1) 10.00		
G-F	-21.0	0.0	0.0 0.06 (1) 7.81		
F-C	-244.0	0.0	0.0 0.06 (1) 7.81		
F-M	0.0	-18.5	-18.5 0.22 (1) 10.00		
M-E	0.0	-18.5	-18.5 0.22 (1) 10.00		

SPECIFIED CONCENTRATED LOADS (LBS)		JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	4-11-0	-177	-177				BACK	VERT	TOTAL		C1
J	1-11-4	-184	-184				BACK	VERT	TOTAL		C1
K	3-11-4	-177	-177				BACK	VERT	TOTAL		C1
L	5-10-12	-177	-177				BACK	VERT	TOTAL		C1
M	7-10-12	-189	-189				BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/862 (0.13")

CSI: TC=0.46/1.00 (A-B:1), SC=0.51/1.00 (B-H:1), WB=0.31/1.00 (D-F:1), SS=0.36/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(FBI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 768 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

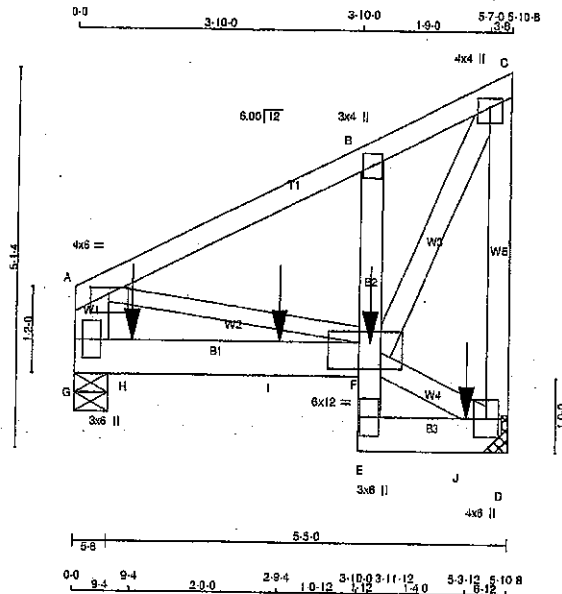
JSI GRIP = 0.73 (D) (INPUT = 0.90)
JSI METAL = 0.29 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108060

JOB NAME 417413	TRUSS NAME T32	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

ID:PYmSsP3BHHa7NFX7pCcknSza0qu-WXysmf847P6cbX7LhSP9mpw0MI1bhzEBraUD2zJID
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Scale = 1/8" = 1'-0"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
D - C	2x4	DRY	No.2	SPF		
G - A	2x6	DRY	No.2	SPF		
G - F	2x6	DRY	No.2	SPF		
E - B	2x4	DRY	No.2	SPF		
E - D	2x6	DRY	No.2	SPF		
ALL WEBS	2x4	DRY	No.2	SPF		
EXCEPT						
A - F	2x3	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - D	12	TOP
G - A	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
G - F	12	SIDE(61.0)
E - D	12	SIDE(0.0)
B - E	8	SIDE(240.2)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	6	
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	GROSS REACTION	DOWN	HORIZ		
JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT	BRG
D	1739	0	1739	0	0	BRG
G	1109	0	1109	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST LOASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	1223	842 : 0	0 : 0	0 : 0	0 : 0	380 : 0	0 : 0
G	777	554 : 0	0 : 0	0 : 0	0 : 0	223 : 0	0 : 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	-1002 / 0	-91.8	-91.8 0.10 (1)	6.25	A-F	0 : 922	0.11 (1)
B-C	-1001 / 0	-91.8	-91.8 0.07 (1)	6.25	F-D	-67 : 0	0.00 (1)
D-C	-1319 / 0	0.0	0.0 0.27 (1)	7.81	F-C	0 : 1926	0.17 (1)
G-A	-784 / 0	0.0	0.0 0.03 (1)	7.81			
G-H	0 / 0	-18.5	-18.5 0.11 (1)	10.00			
H-I	0 / 0	-18.5	-18.5 0.11 (1)	10.00			
I-F	0 / 0	-18.5	-18.5 0.11 (1)	10.00			
E-F	0 / 174	0.0	0.0 0.06 (1)	10.00			
F-B	-328 / 0	0.0	0.0 0.05 (1)	7.81			
E-J	0 / 60	-18.5	-18.5 0.08 (1)	10.00			
J-D	0 / 60	-18.5	-18.5 0.08 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	3-11-12	-812	-812	---	FRONT	VERT	TOTAL	---	C1
H	8-4	-179	-179	---	TOP	VERT	TOTAL	---	C1
I	2-9-4	-178	-178	---	TOP	VERT	TOTAL	---	C1
J	5-3-12	-372	-372	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
BOT CH.	LL	=	6.0	PSF
	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.27/1.00 (C-D:1), BC=0.11/1.00 (F-G:1), WB=0.17/1.00 (C-F:1), SSI=0.13/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

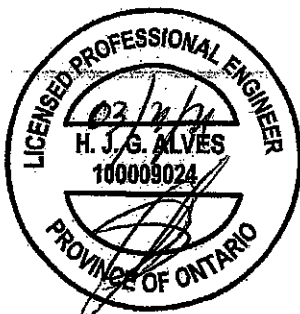
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	550 371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.22 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108061 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417413	T32	1	2	GREENPARK HOMES	
Famarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Mar 19 17:49:36 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMV+p	MT20	3.0	4.0		
C	TMVW+p	MT20	4.0	4.0	1.50	2.00
D	BMVW1+p	MT20	4.0	8.0		
E	BMV+p	MT20	3.0	8.0		
F	BVMWW-1	MT20	6.0	12.0	4.25	5.00
G	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

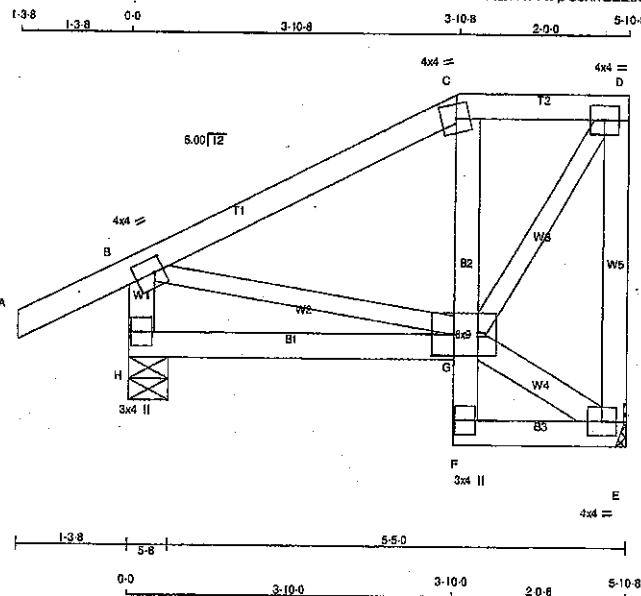


Structural component only
DWG# T-2108061 *ML*

JOB NAME 417413	TRUSS NAME T34	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/4" = 1'-0"

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4	DRY No.2
C - D	2x4	DRY No.2
E - D	2x4	DRY No.2
H - B	2x4	DRY No.2
H - C	2x4	DRY No.2
F - E	2x4	DRY No.2

ALL WEBS	2x3	DRY No.2
EXCEPT		
G - E	2x4	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-I	MT20	4.0	4.0	2.00	1.25
C TTV-m	MT20	4.0	4.0		
D TMVW-I	MT20	4.0	4.0		
E BMVW-I	MT20	4.0	4.0		
F BMV+P	MT20	3.0	4.0		
G BVMWW-I	MT20	6.0	9.0	3.00	3.00
H BMV+P	MT20	3.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
E	324	0	324	0
H	448	0	448	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1'-8".

UNFACTORED REACTIONS

1ST LOASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	229	150 0	0 0	0 0	0 0	79 0	0 0
H	315	220 0	0 0	0 0	0 0	95 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.61 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LBS)	LC2 MAX (LBS)	UNBRACED LENGTH	FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LBS)	LC2 MAX (LBS)
A-B		0/28	-91.8	-91.8	0.12 (1)	10.00	B-G		0/158	0.04 (1)	10.00	
B-C		-173/0	-91.8	-91.8	0.23 (1)	6.25	G-E		-9/0	0.00 (1)	10.00	
C-D		-141/0	-91.8	-91.8	0.07 (1)	6.25	G-D		0/253	0.06 (1)	10.00	
E-D		-302/0	0.0	0.0	0.08 (1)	7.61						
H-B		-412/0	0.0	0.0	0.04 (1)	7.61						
H-G		0/0	-18.5	-18.5	0.09 (4)	10.00						
F-G		0/20	0.0	0.0	0.01 (1)	10.00						
G-C		-193/0	0.0	0.0	0.01 (1)	7.61						
F-E		0/8	-18.5	-18.5	0.02 (4)	10.00						

TOTAL WEIGHT = 32 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
DL	= 6.0	PSF	
BOT CH.	LL	= 0.0	PSF
DL	= 7.4	PSF	
TOTAL LOAD	= 39.0	PSF	

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSK TC=0.23/1.00 (B-C:1), BC=0.09/1.00 (G-H:4), WB=0.06/1.00 (D-G:1), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 783

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.26 (D) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

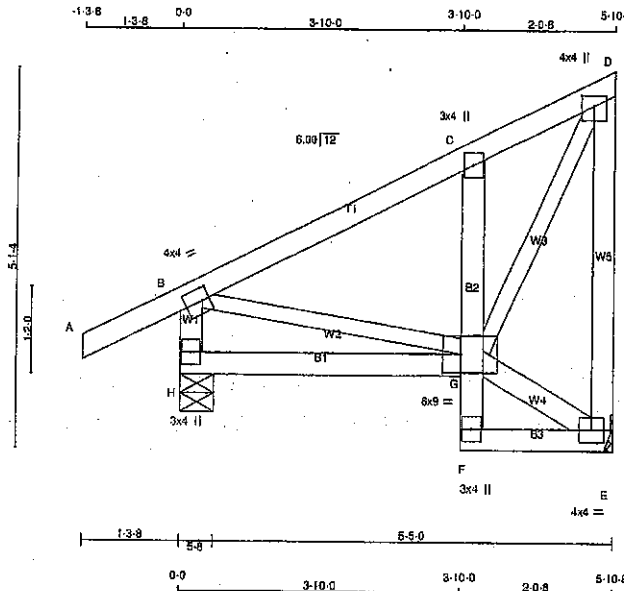


Structural component only
DWG# T-2108063

JOB NAME 417413	TRUSS NAME T35	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/8" = 1'-0"

LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT		2x3	DRY	No.2
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-p	MT20	3.0	4.0		
D	TMVW-p	MT20	4.0	4.0	1.75	2.00
E	BMVW1-t	MT20	4.0	4.0		
F	BMVW-p	MT20	3.0	4.0		
G	BMVWW-t	MT20	6.0	9.0	3.00	3.00
H	BMV1-p	MT20	3.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	HORZ	DOWN	UP/LIFT
E	324	0	324	0
H	448	0	448	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1'-8".

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	229	150 0	0 0	0 0	0 0	79 0	0 0
H	315	220 0	0 0	0 0	0 0	95 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	W E B S	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	CS1 (LC)	MEMB. FORCE (LBS)
FR-TO		FROM TO	UNBRAC LENGTH	FR-TO
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00
B-C	-202 / 0	-91.8 -91.8	0.17 (1)	6.25
C-D	-226 / 0	-91.8 -91.8	0.16 (1)	6.25
E-D	-298 / 0	0.0 0.0	0.13 (1)	7.81
H-B	-412 / 0	0.0 0.0	0.04 (1)	7.81
H-G	0 / 0	-18.5 -18.5	0.09 (4)	10.00
F-G	0 / 20	0.0 0.0	0.02 (1)	10.00
G-C	-355 / 0	0.0 0.0	0.03 (1)	7.81
F-E	0 / 16	-18.5 -18.5	0.02 (4)	10.00

TOTAL WEIGHT = 2 X 34 = 68 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 6.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.17/1.00 (B-C:1), BC=0.09/1.00 (G-H:4),
WB=0.09/1.00 (D-G:1), SS=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

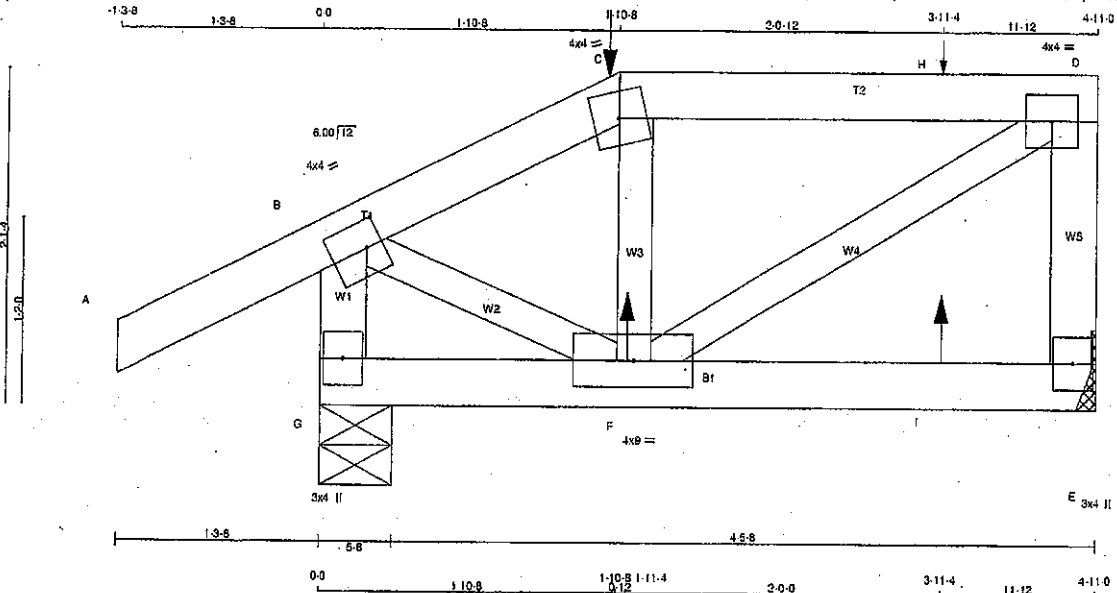
JSI GRIP = 0.43 (D) (INPUT = 0.90)
JSI METAL = 0.12 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108064

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417413	T36	1	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington					

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ID:PymSaP3BHha?NFX7pCcknSza0qu-w6e?OhAYN2nhT3Fa0q?6nORTzZKxo4ggtpo8qNzZJIA



Scale = 1:12

TOTAL WEIGHT = 21 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
G - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
ALL WEBS, 2x3 DRY No.2 SPF					
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE	PLATES			
B TMVW-t	MT20	4.0	4.0	2.00 1.25
C TTW-m	MT20	4.0	4.0	
D TMVW-t	MT20	4.0	4.0	
E BMV1+p	MT20	3.0	4.0	
F BMVWW-t	MT20	4.0	9.0	
G BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT REQD BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
E	256	0	256	0
G	424	0	424	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	182	116	0	0	0	0	0
G	298	210	0	0	0	87	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO						FR-TO			
A-B	0.28	-91.8	-91.8	0.13 (1)	10.00	F-C	-126.0	0.02 (1)	
B-C	-204.0	-91.8	-91.8	0.13 (1)	6.25	F-D	0.184	0.05 (1)	
C-H	-157.0	-91.8	-91.8	0.16 (1)	6.25	B-F	0.179	0.04 (1)	
H-D	-157.0	-91.8	-91.8	0.16 (1)	6.25				
E-D	-236.0	0.0	0.0	0.03 (1)	7.81				
G-B	-409.0	0.0	0.0	0.05 (1)	7.81				
G-F	0.0	-18.5	-18.5	0.03 (4)	10.00				
F-I	0.0	-18.5	-18.5	0.04 (4)	10.00				
I-E	0.0	-18.5	-18.5	0.04 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX	FACE	DIR.	TYPE	HEEL	CONN.
C	1-10-8	-4	-4	FRONT	VERT	DEAD	---	C1
B-C	1-10-8	-1	-1	BACK	VERT	TOTAL	---	C1
C	1-10-8	-14	-14	FRONT	VERT	SNOW	---	C1
C	1-11-4	4	4	BACK	VERT	TOTAL	---	C1
H	3-11-4	1	1	BACK	VERT	TOTAL	---	C1
I	3-11-4	4	4	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.16/1.00 (C-D:1), BC=0.04/1.00 (E-F:4), WB=0.05/1.00 (D-F:1), SS=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

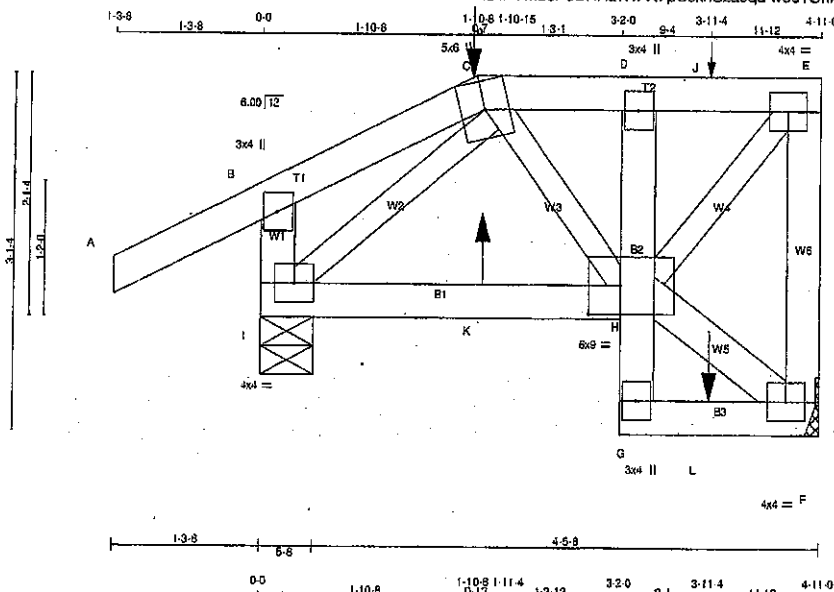


Structural component only
DWG# T-2108065

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417413	T36S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:18.3

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
I - J	2x4	DRY	No.2	SPF	
K - L	2x4	DRY	No.2	SPF	
M - N	2x4	DRY	No.2	SPF	
O - P	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
H - F	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TWV+p	MT20	3.0	4.0		
C	TWV+m	MT20	5.0	6.0		
D	TWV+p	MT20	3.0	4.0		
E	TWVW-1	MT20	4.0	4.0		
F	BMVW-1	MT20	4.0	4.0		
G	BMV+p	MT20	3.0	4.0		
H	BMVWVW-1	MT20	6.0	9.0	3.00	3.50
I	BMVW-1	MT20	4.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT HORZ	DOWN HORZ UPLIFT	IN-SX	IN-SX	
F 281 0	281 0 0	MECHANICAL	5-8	5-8
I 409 0	409 0 0			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS						
JT COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
F 199 130 0 0 0 0 0 0							
I 287 201 0 0 0 0 0 0							

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF) CSI (LC)	MEMB.	FORCE (LBS) MAX (LC)
FR-TO	FROM TO	LENGTH	FR-TO	FROM TO
A-B 0 28	-91.8 -91.8 0.13 (1)	10.00	H-F -12 0	0.00 (1)
B-C 0 0	-91.8 -91.8 0.08 (1)	10.00	H-E 0 281	0.08 (1)
C-D -179 0	-91.8 -91.8 0.03 (5)	6.25	O-H 0 18	0.01 (4)
D-J -173 0	-91.8 -91.8 0.03 (1)	6.25	I-C -245 0	0.05 (1)
J-E -173 0	-91.8 -91.8 0.03 (1)	6.25		
F-E -259 0	0.0 0.0 0.04 (1)	7.81		
I-B -212 0	0.0 0.0 0.02 (1)	7.81		
I-K 0 178	-18.5 -18.5 0.08 (4)	10.00		
K-H 0 178	-18.5 -18.5 0.08 (4)	10.00		
G-H 0 18	0.0 0.0 0.02 (1)	10.00		
H-D -180 0	0.0 0.0 0.02 (1)	7.81		
G-L 0 10	-18.5 -18.5 0.02 (4)	10.00		
L-F 0 10	-18.5 -18.5 0.02 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	1-10-15	-4	-4	FRONT	VERT	DEAD	---	C1
C	1-10-15	-1	-1	FRONT	VERT	TOTAL	---	C1
C	1-10-15	-14	-14	FRONT	VERT	SNOW	---	C1
J	3-11-4	1	1	FRONT	VERT	TOTAL	---	C1
K	1-11-4	4	1	FRONT	VERT	TOTAL	---	C1
L	3-11-4	-2	-2	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.19/1.00 (A-B:1), BC=0.08/1.00 (H-I:4), WB=0.08/1.00 (E-H:1), SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN
MT20	650	371	1747	788 1987 1873

PLATE PLACEMENT TOL = 0.250 Inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.26 (E) (INPUT = 0.90)
JSI METAL= 0.07 (C) (INPUT = 1.00)

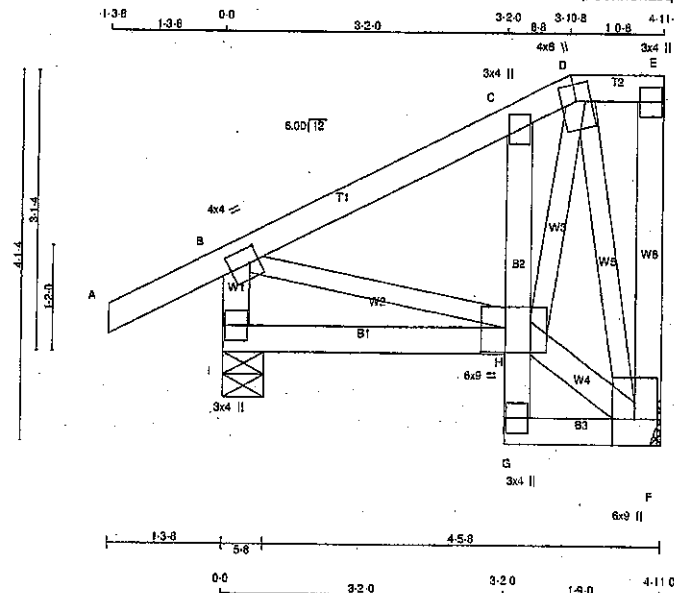


Structural component only
DWG# T-2108066

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417413	T37	2	1	GREENPARK HOMES	

Tamarack Roof Truss, Burlington

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Scale = 1/2\"/>

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
G - C	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT H - F	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-i	MT20	4.0	4.0	2.00	1.25
C TMV-i+p	MT20	3.0	4.0		
D TTWW-i+m	MT20	4.0	6.0	2.25	2.00
E TMV-i+p	MT20	3.0	4.0		
F BMVWW-i+p	MT20	6.0	9.0	Edge	
G BMV-i+p	MT20	3.0	4.0		
H BMVWW-i	MT20	6.0	9.0	3.25	3.25
I BMV-i+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES-

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
VERT	HORZ	DOWN	HORZ	UPLIFT
F	271	0	271	0
I	398	0	398	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	192	126 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0
I	277	195 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)
FR-TO						FR-TO			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	B-H	0 / 158	0.04 (1)	
B-C	-160 / 0	-91.8	-91.8	0.12 (1)	6.25	H-F	0 / 54	0.01 (1)	
C-D	-207 / 0	-91.8	-91.8	0.11 (1)	6.25	H-D	0 / 358	0.08 (1)	
D-E	0 / 0	-91.8	-91.8	0.02 (1)	10.00	D-F	-247 / 0	0.08 (1)	
E-F	-46 / 0	0.0	0.0	0.01 (1)	7.81				
F-G	-365 / 0	0.0	0.0	0.04 (1)	7.81				
I-H	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
G-H	0 / 17	0.0	0.0	0.03 (1)	10.00				
H-C	-316 / 0	0.0	0.0	0.03 (1)	7.81				
G-F	0 / 17	-18.5	-18.5	0.02 (4)	10.00				

TOTAL WEIGHT = 2 X 31 = 62 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.12/1.00 (A-B:1), BC=0.08/1.00 (H-I:4), WB=0.08/1.00 (D-H:1), SS=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

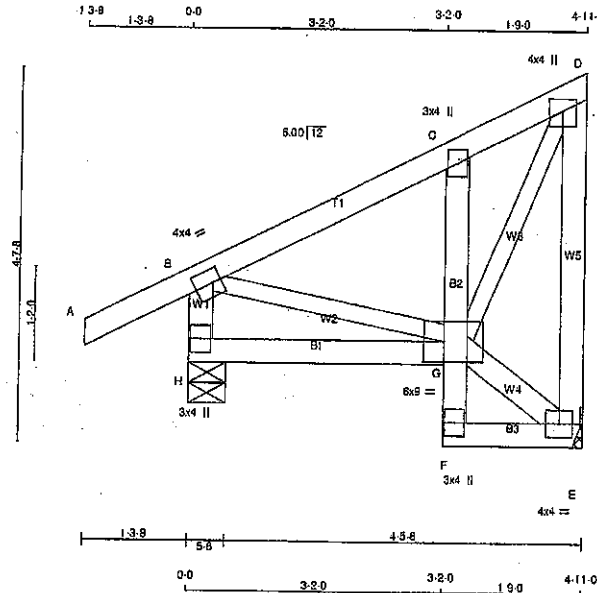
JSI GRIP = 0.26 (B) (INPUT = 0.90)
JSI METAL = 0.10 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108067

JOB NAME 417413	TRUSS NAME T38	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:PYmSsP3BHha?NFX7pCcknSza0qu-tVlpNBovg1PjNPz7E2bspWq4N02GzmzK6HFvGzZJl8



Scale = 1/28.0

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
H - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
F - C	2x4	DRY	No.2
F - E	2x4	DRY	No.2
ALL WEBS EXCEPT G - E	2x3	DRY	No.2
	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.75	2.00
E	BMVW1-t	MT20	4.0	4.0		
F	BMV+p	MT20	3.0	4.0		
G	BMVWW1-t	MT20	6.0	9.0	3.00	3.00
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	DOWN	GROSS REACTION	BRG	BRG
E	VERT	271	0	0	0
H	HCRZ	396	0	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
E	COMBINED	126.0	0.0	0.0	0.0	68.0	0.0
H		195.0	0.0	0.0	0.0	82.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	UNBRACED LENGTH
FR-TO		FROM TO		FR-TO			
A-B	0.28	-91.8	-91.8 0.12 (1)	10.00	B-G	0.158	0.04 (1)
B-C	-160.0	-91.8	-91.8 0.12 (1)	8.25	G-E	-17.0	0.00 (1)
C-D	-177.0	-91.8	-91.8 0.11 (1)	-6.25	G-D	0.320	0.07 (1)
E-D	-247.0	0.0	0.0 0.08 (1)	7.81			
H-B	-365.0	0.0	0.0 0.04 (1)	7.81			
H-G	0.0	-18.5	-18.5 0.06 (4)	10.00			
F-G	0.17	0.0	0.0 0.02 (1)	10.00			
G-C	-291.0	0.0	0.0 0.02 (1)	7.81			
F-E	0.15	-18.5	-18.5 0.02 (4)	10.00			

TOTAL WEIGHT = 29 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.12/1.00 (A-B:1), BC=0.06/1.00 (G-H:4), WB=0.07/1.00 (D-G:1), SSI=0.12/1.00 (B-C:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

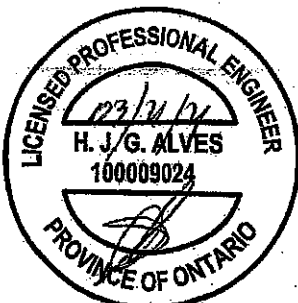
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 789 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

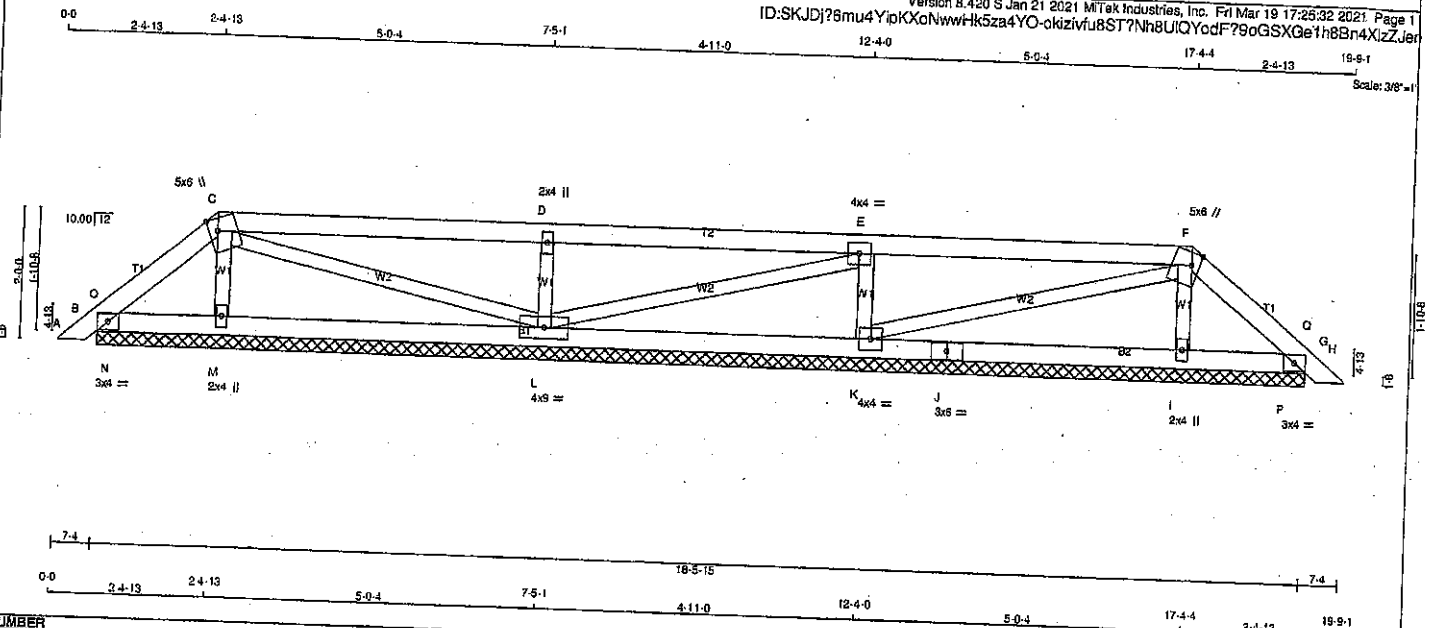
JSI GRIP = 0.34 (D) (INPUT = 0.90)
JSI METAL = 0.10 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108068

JOB NAME 417375	TRUSS NAME PB1	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:SKJDJ76mu4YipKXoNwwHk5za4YO-ckizvfu8ST7Nh8UIQYodF79oGSXGe1h8Bn4XizZJer



LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY No.2

C - F 2x4 DRY No.2

F - H 2x4 DRY No.2

B - J 2x4 DRY No.2

J - G 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

DRY: SEASONED LUMBER.

DESCR.

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	INPUT BRG IN-SX	RECORD BRG IN-SX
B	164	0	164	0	18-5-15 (12-118-8-15)	
M	301	0	301	0	18-5-15 (12-118-8-15)	
I	298	0	298	0	18-5-15 (12-118-8-15)	
G	172	0	172	0	18-5-15 (12-118-8-15)	
K	591	0	591	0	18-5-15 (12-118-8-15)	
L	610	0	610	0	18-5-15 (12-118-8-15)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	113	91/0	0/0	0/0	0/0	22/0	0/0
M	216	125/0	0/0	0/0	0/0	91/0	0/0
I	213	123/0	0/0	0/0	0/0	91/0	0/0
G	118	95/0	0/0	0/0	0/0	23/0	0/0
K	417	278/0	0/0	0/0	0/0	139/0	0/0
L	430	268/0	0/0	0/0	0/0	142/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, M, I, G, K, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORZ. LOAD (LBS)	WEBS MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LBS)
FR-TO					
A-B	0/14	-91.8	-91.8 0.02 (1)	10.00	M-C -207/0
B-O	-25/6	-91.8	-91.8 0.01 (1)	6.25	I-F -203/0
O-C	-55/0	-91.8	-91.8 0.03 (1)	6.25	K-F -18/0
C-D	0/6	-91.8	-91.8 0.31 (1)	10.00	C-L -32/0
D-E	0/6	-91.8	-91.8 0.31 (1)	10.00	K-E -493/0
E-F	-17/0	-91.8	-91.8 0.31 (1)	6.25	L-D -501/0
F-Q	-37/0	-91.8	-91.8 0.03 (1)	6.25	L-E -24/0
Q-G	-38/4	-91.8	-91.8 0.01 (1)	6.25	N-O -122/0
G-H	0/14	-91.8	-91.8 0.02 (1)	10.00	P-Q -121/0
B-N	0/38	-18.5	-18.5 0.04 (1)	10.00	
N-M	0/38	-18.5	-18.5 0.07 (4)	10.00	
M-L	0/25	-18.5	-18.5 0.09 (4)	10.00	
L-K	0/17	-18.5	-18.5 0.09 (4)	10.00	
K-J	0/34	-18.5	-18.5 0.09 (4)	10.00	
J-I	0/34	-18.5	-18.5 0.09 (4)	10.00	
I-P	0/48	-18.5	-18.5 0.07 (4)	10.00	
P-G	0/48	-18.5	-18.5 0.04 (1)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2015, ABC 2019

- PART 9 OF NBC 2012 (2015 AMENDMENT)

- CSA 096-14

- TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSL: TC=0.31/1.00 (D-E:1), BC=0.09/1.00 (H-K:4), WB=0.07/1.00 (D-L:1), SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	550	971	1747
		788	1887
		1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (E) (INPUT = 0.80)

JSI METAL = 0.11 (D) (INPUT = 1.00)

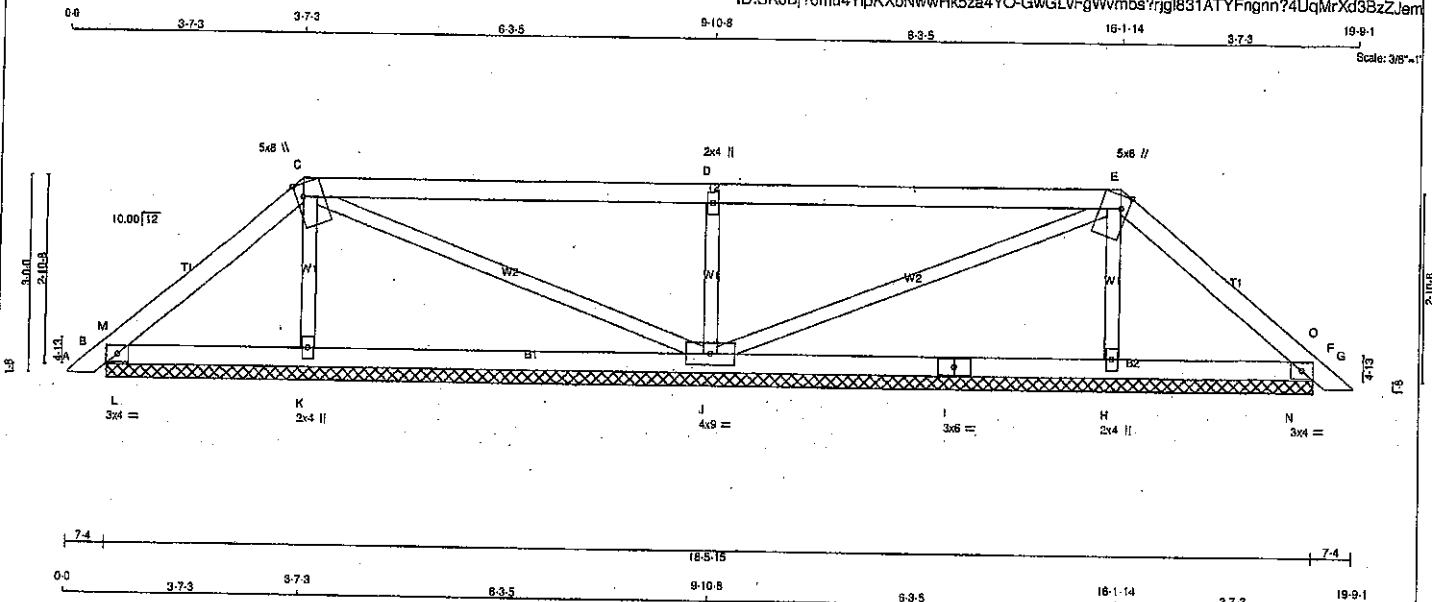


Structural component only
DWG# T-2108008

JOB NAME 417375	TRUSS NAME PB2	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:SKJDj76mu4YpKXoNwwHk5za4YO-GwGLvFgWvmba?rjgl831ATYFngnn?4UqMrXd3BzZJem



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
E - G	2x4 DRY	No.2	SPF
G - I	2x4 DRY	No.2	SPF
I - F	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMB1-I	MT20	3.0	4.0		
C TTWW+m	MT20	5.0	8.0	Edge	1.25
D TWW+m	MT20	2.0	4.0		
E TTWW+m	MT20	5.0	8.0	Edge	1.25
F TMB1-I	MT20	3.0	4.0		
H BMW1+w	MT20	2.0	4.0		
I BS-I	MT20	3.0	6.0		
J BMW/WW1-I	MT20	4.0	8.0		
K BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
B	257	0	257	0	0	18-5-15 (5-6-38-5-15)			
K	375	0	375	0	0	18-5-15 (5-6-38-5-15)			
J	869	0	869	0	0	18-5-15 (5-6-38-5-15)			
H	375	0	375	0	0	18-5-15 (5-6-38-5-15)			
F	257	0	257	0	0	18-5-15 (5-6-38-5-15)			

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	179	136 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0
K	269	158 / 0	0 / 0	0 / 0	0 / 0	114 / 0	0 / 0
J	612	417 / 0	0 / 0	0 / 0	0 / 0	195 / 0	0 / 0
H	269	158 / 0	0 / 0	0 / 0	0 / 0	114 / 0	0 / 0
F	179	136 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)
FR-TO					FR-TO			FR-TO	
A-B	0 / 14	-91.8	-91.8 0.02 (1)	10.00	K-C	-250 / 0	0.04 (1)		
B-M	-3 / 0	-91.8	-91.8 0.05 (1)	10.00	C-J	-46 / 0	0.04 (1)		
M-C	-103 / 0	-91.8	-91.8 0.09 (1)	6.25	J-D	-719 / 0	0.12 (1)		
C-D	-19 / 0	-91.8	-91.8 0.61 (1)	6.25	J-E	-46 / 0	0.04 (1)		
D-E	-19 / 0	-91.8	-91.8 0.61 (1)	6.25	H-E	-250 / 0	0.04 (1)		
E-O	-103 / 0	-91.8	-91.8 0.09 (1)	6.25	L-M	-273 / 0	0.00 (1)		
O-F	-3 / 0	-91.8	-91.8 0.05 (1)	10.00	N-O	-273 / 0	0.00 (1)		
F-G	0 / 14	-91.8	-91.8 0.02 (1)	10.00					
B-L	0 / 73	-18.5	-18.5 0.10 (1)	10.00					
L-K	0 / 73	-18.5	-18.5 0.11 (4)	10.00					
K-J	0 / 63	-18.5	-18.5 0.16 (4)	10.00					
J-I	0 / 63	-18.5	-18.5 0.16 (4)	10.00					
I-H	0 / 63	-18.5	-18.5 0.16 (4)	10.00					
H-N	0 / 73	-18.5	-18.5 0.11 (4)	10.00					
N-F	0 / 73	-18.5	-18.5 0.10 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55% OF 91.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.61/1.00 (C-D:1), BC=0.16/1.00 (J-K:4), WB=0.12/1.00 (D-J:1), SS=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

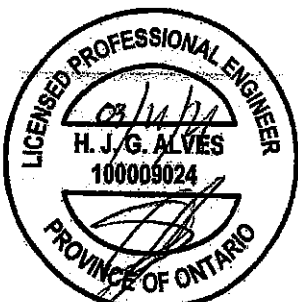
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

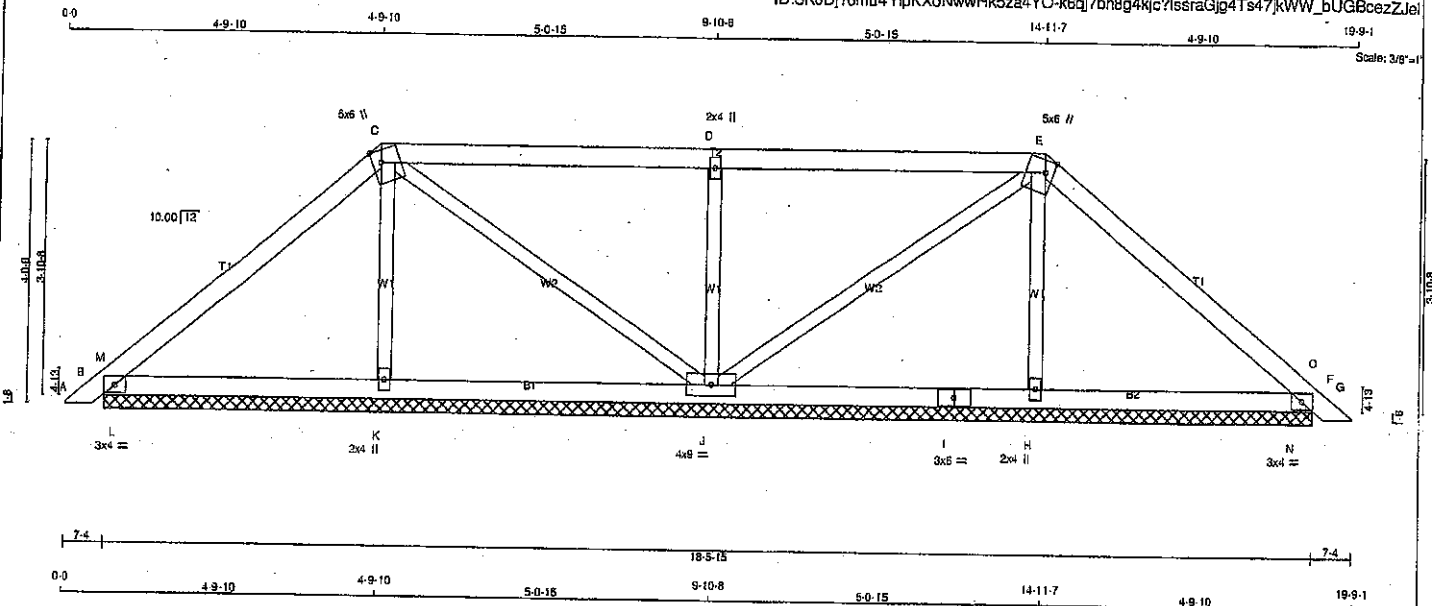
JSI GRIP = 0.84 (C) (INPUT = 0.80)
JSI METAL = 0.15 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108009

JOB NAME 417375	TRUSS NAME PB3	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:SKJDJ76mu4YipKXoNwwHk5za4YO-k6qj7bh8g4Kjc?IssraGjg4Ts47jKWW_bUGBceZJel



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TWW+m	MT20	2.0	4.0	
E	TTWW+m	MT20	5.0	6.0	2.25 1.50
F	TMB1-I	MT20	3.0	4.0	
H	BMV1-w	MT20	2.0	4.0	
I	BS-I	MT20	3.0	6.0	
J	BMVWW1-I	MT20	4.0	9.0	
K	BMV1-w	MT20	2.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
B	359	0	359	0	0	18-5-15 (5-6-38-5-15)			
K	335	0	335	0	0	18-5-15 (5-6-38-5-15)			
J	747	0	747	0	0	18-5-15 (5-6-38-5-15)			
H	335	0	335	0	0	18-5-15 (5-6-38-5-15)			
F	359	0	359	0	0	18-5-15 (5-6-38-5-15)			

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS							
JT	1ST LCASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
B	251	180 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0
K	241	138 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
J	525	364 / 0	0 / 0	0 / 0	0 / 0	161 / 0	0 / 0
H	241	138 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
F	251	180 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 14	-91.8 -91.8	0.02 (1)	10.00	K-C	-198 / 0	0.05 (1)
B-M	-29 / 52	-91.8 -91.8	0.11 (1)	6.25	C-J	-81 / 0	0.05 (1)
M-C	-159 / 0	-91.8 -91.8	0.20 (1)	6.25	J-D	-581 / 0	0.14 (1)
C-D	-40 / 0	-91.8 -91.8	0.40 (1)	6.25	J-E	-81 / 0	0.05 (1)
D-E	-40 / 0	-91.8 -91.8	0.40 (1)	6.25	H-E	-198 / 0	0.05 (1)
E-O	-159 / 0	-91.8 -91.8	0.20 (1)	6.25	L-M	-477 / 0	0.00 (1)
O-F	-29 / 52	-91.8 -91.8	0.11 (1)	6.25	N-O	-477 / 0	0.00 (1)
F-G	0 / 14	-91.8 -91.8	0.02 (1)	10.00			
B-L	0 / 112	-18.5 -18.5	0.17 (1)	10.00			
L-K	0 / 112	-18.5 -18.5	0.17 (1)	10.00			
K-J	0 / 107	-18.5 -18.5	0.11 (1)	10.00			
J-I	0 / 107	-18.5 -18.5	0.11 (1)	10.00			
I-H	0 / 107	-18.5 -18.5	0.11 (1)	10.00			
H-N	0 / 112	-18.5 -18.5	0.17 (1)	10.00			
N-F	0 / 112	-18.5 -18.5	0.17 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TFC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.40/1.00 (C-D:1), BC=0.17/1.00 (B-L:1), WB=0.14/1.00 (D-J:1), SS=0.37/1.00 (F-N:1)

DOI: LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

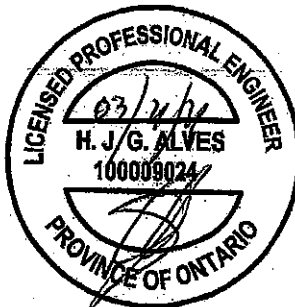
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.62 (C) (INPUT = 0.90)
JSI METAL = 0.12 (D) (INPUT = 1.00)

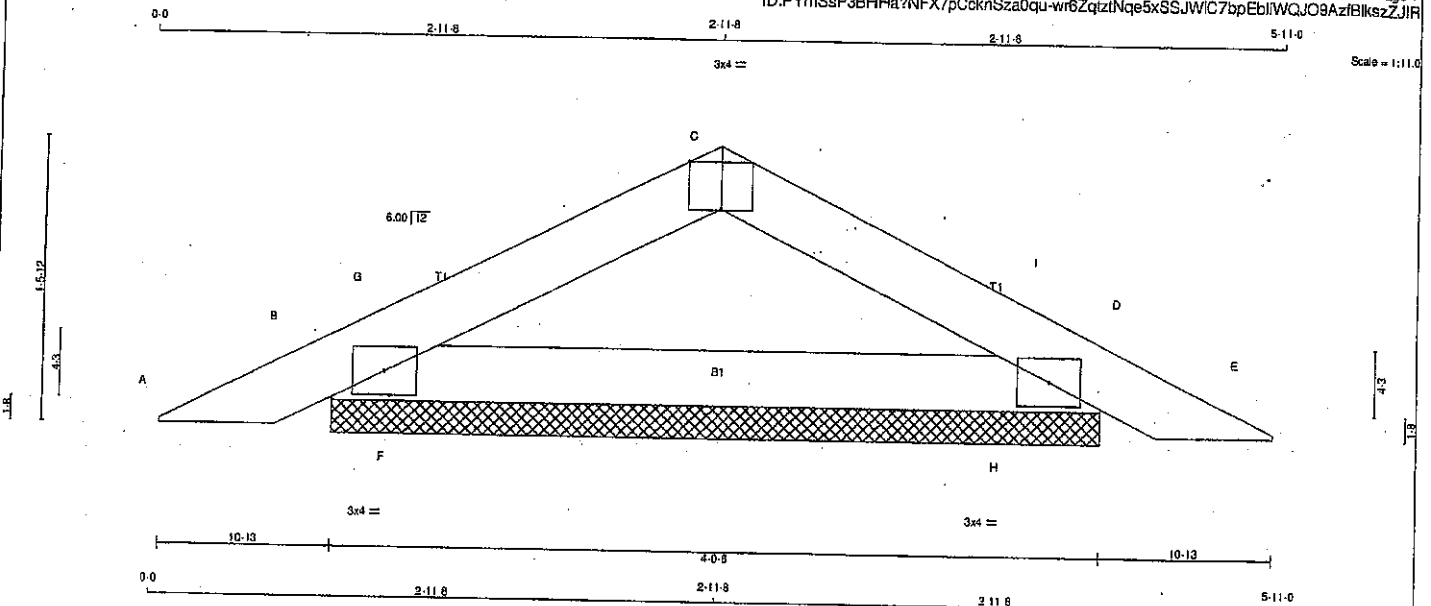


Structural component only
DWG# T-2108010

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417413	PB21	6	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR. SPF
A - C 2x4 DRY No.2
C - E 2x4 DRY No.2
B - D 2x4 DRY No.2
DRY: SEASONED LUMBER.

CHORDS	SIZE	LUMBER	DESCR.	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES	3.0	4.0		
B TMB1-1 MT20	3.0	4.0	Edge	2.00
C TT-p MT20	3.0	4.0		
D TMB1-1 MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
B	300	0	300	0
D	300	0	300	0

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
B	210	146.0 0.0 0.0 0.0 64.0 0.0
D	210	146.0 0.0 0.0 0.0 64.0 0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)
FR-TO	A-B	0 / 16	-91.8	-91.8	0.05 (1)	10.00	F-G	0 / 54	0.00 (1)
	B-G	-252 / 0	-91.8	-91.8	0.04 (4)	8.25	H-I	0 / 54	0.00 (1)
	G-C	-198 / 0	-91.8	-91.8	0.06 (1)	6.25			
	C-I	-198 / 0	-91.8	-91.8	0.06 (1)	6.25			
	I-D	-252 / 0	-91.8	-91.8	0.04 (4)	8.25			
	D-E	0 / 16	-91.8	-91.8	0.05 (1)	10.00			
	B-F	0 / 185	-18.5	-18.5	0.04 (1)	10.00			
	F-H	0 / 185	-18.5	-18.5	0.07 (1)	10.00			
	H-D	0 / 185	-18.5	-18.5	0.04 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (C-I), BC=0.07/1.00 (F-H), WB=0.00/1.00 (F-G), SSI=0.06/1.00 (C-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

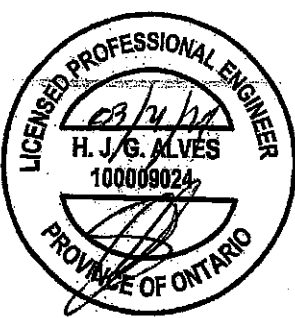
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

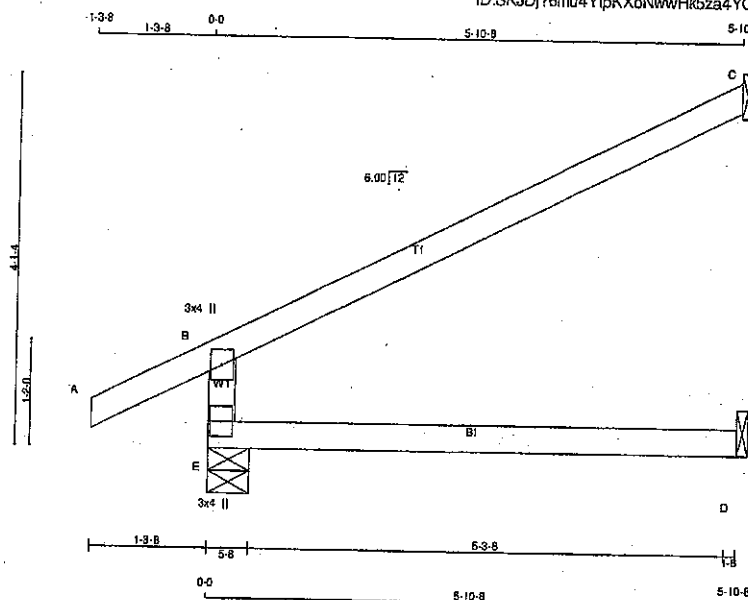
JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108046

JOB NAME 417375	TRUSS NAME J1	QUANTITY 23	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/2" = 1'-0"

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES-

- (1) Lateral bracing to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
E	525	0	0	5-8
C	202	0	0	1-8
D	45	0	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	369	257.0	0.0	0.0	0.0	111.0	0.0
C	139	113.0	0.0	0.0	0.0	26.0	0.0
D	38	0.0	0.0	0.0	0.0	36.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD	LC1	MAX CS1 (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM	TO					
E-B	-461 / 0	0.0	0.0	0.13 (4)	7.81			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-30 / 0	-91.8	-91.8	0.54 (1)	6.25			
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

TOTAL WEIGHT = 23 X 17 = 391 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (0.20")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.00")
ALLOWABLE DEFL. (TL) = 1/360 (0.20")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.00")

CSI: TC=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

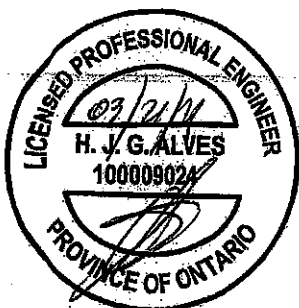
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 785

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

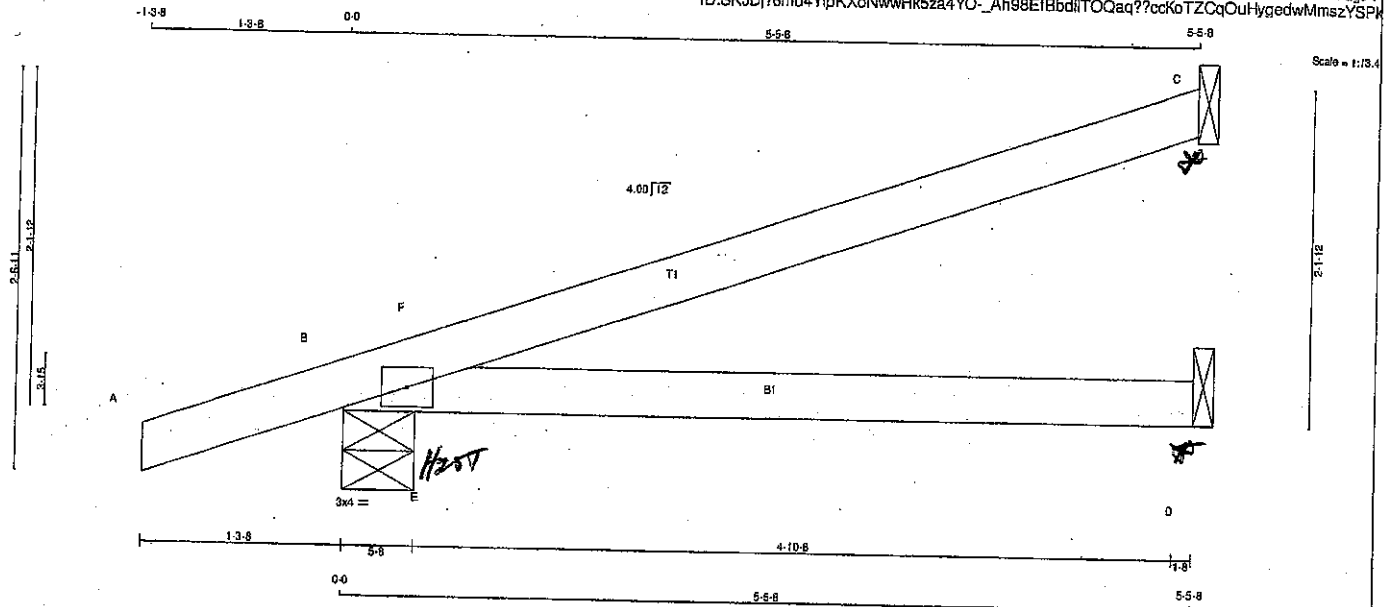
JSI GRIP = 0.18 (E) (INPUT = 0.90)
JSI METAL = 0.13 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108005

JOB NAME 417375	TRUSS NAME J2	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 5 X 14 = 72 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMB14	MT20	3.0	4.0	

NOTES - (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	DOWN	IN-SX	IN-SX
C	218	0	-152	1-8
B	424	0	-271	5-8
D	83	0	-108	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 152 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 271 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 108 LBS. FACTORED UPLIFT

PROVIDE FOR 137 LBS. FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED						
C	151	118 / 0	0 / 0	0 / 0	0 / -130	34 / 0	0 / 0
B	297	208 / 0	0 / 0	0 / 0	0 / -251	69 / 0	0 / 0
D	62	22 / 0	0 / 0	0 / 0	0 / -103	40 / 0	0 / 0

HORIZONTAL REACTIONS

B	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 18	-91.8	-91.8 0.11 (1)	10.00	E-F	-305 / 116	0.00 (1)
B-F	-97 / 14	-91.8	-91.8 0.08 (12)	6.25			
F-C	-56 / 2	-91.8	-91.8 0.35 (1)	6.25			
B-E	0 / 0	-18.5	-18.5 0.25 (1)	10.00			
E-D	0 / 0	-18.5	-18.5 0.25 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (8.6) PSF AT (20-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM) INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BOBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")

CALCULATED VERT. DEFL.(LL) = $L/778$ (0.08")

ALLOWABLE DEFL.(TL) = $L/360$ (0.19")

CALCULATED VERT. DEFL.(TL) = $L/807$ (0.11")

CSI: $TC=0.35/1.00$ (C-F:1), $BC=0.25/1.00$ (D-E:1), $WB=0.00/1.00$ (E-F:1), $SSI=0.24/1.00$ (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

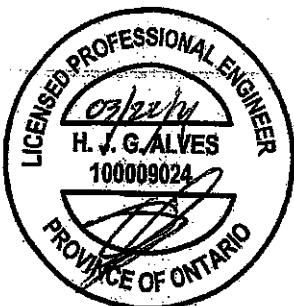
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PS)	(PL)	(PL)
	MAX	MIN	MAX MIN
MT20	650	371	1747 786 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.28 (B) (INPUT = 0.50)
JSI METAL = 0.07 (B) (INPUT = 1.00)

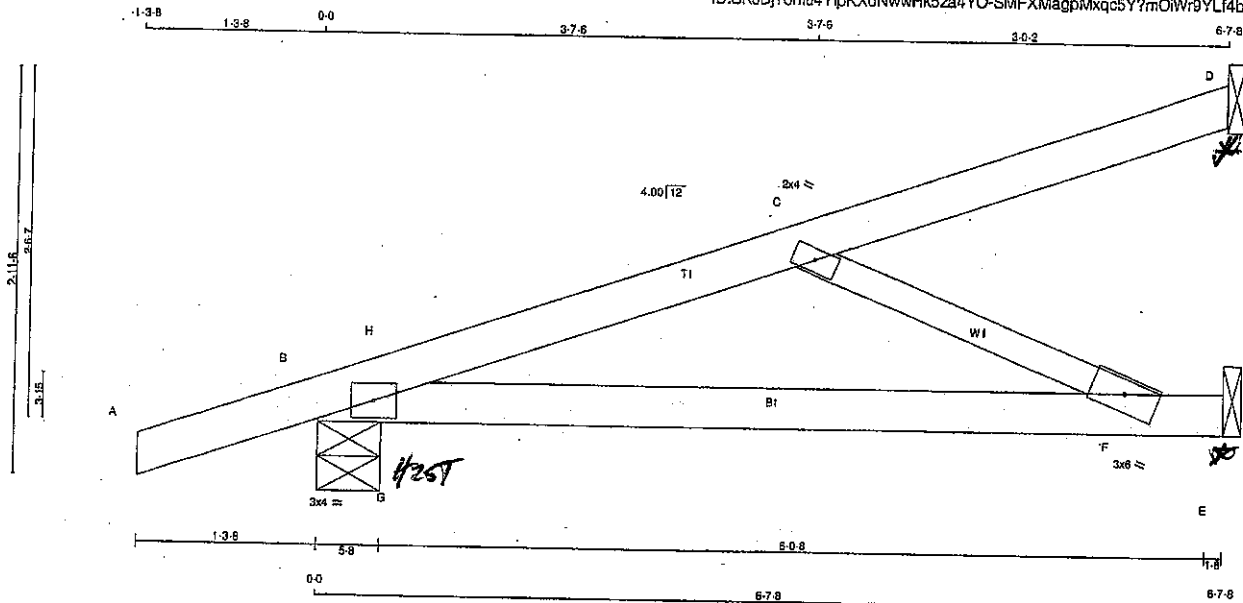


Structural component only
DWG# T-2108006

JOB NAME 417375	TRUSS NAME J3	QUANTITY 6	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 6 X 19 = 117 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
B - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMB1-i	MT20	3.0	4.0		
C TMW-w	MT20	2.0	4.0		
F BMW-w	MT20	3.0	6.0		

NOTES-

- 1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
D	112	0	112	0
B	488	0	488	162
E	253	0	253	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D, E

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 318 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT E FOR 244 LBS. FACTORED UPLIFT

PROVIDE FOR 162 LBS. FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	77	63 / 0	0 / 0	0 / 0	0 / -60	14 / 0	0 / 0
B	343	238 / 0	0 / 0	0 / 0	0 / -295	105 / 0	0 / 0
E	181	106 / 0	0 / 0	0 / 0	0 / -223	75 / 0	0 / 0

HORIZONTAL REACTIONS

B	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)
FR-TO				FR-TO			
A-B	0 / 18	-91.8	-91.8 0.11 (1)	10.00	C-F	-498 / 384	0.10 (1)
B-H	-544 / 300	-91.8	-91.8 0.24 (5)	8.25	G-H	-256 / 148	0.00 (1)
H-C	-440 / 215	-91.8	-91.8 0.23 (5)	8.25			
C-D	-26 / 0	-91.8	-91.8 0.09 (1)	5.25			
B-G	-323 / 439	-18.5	-18.5 0.11 (12)	6.25			
G-F	-323 / 439	-18.5	-18.5 0.32 (1)	8.25			
F-E	0 / 0	-18.5	-18.5 0.25 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (20-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpG, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	25.6	PSF
	DL	6.0	PSF
BOT CH.	LL	0.0	PSF
	DL	7.4	PSF
TOTAL LOAD		39.0	PSF

SPACING = 24.0 IN. G.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/594 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/570 (0.14")

CSI: TC=0.24/1.00 (B-H:5), BC=0.32/1.00 (F-G:1), WB=0.10/1.00 (C-F:1), SS=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

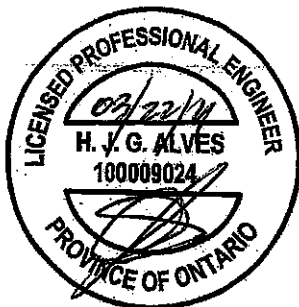
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 798	1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.40 (C) (INPUT = 0.90)
JSI METAL= 0.25 (C) (INPUT = 1.00)

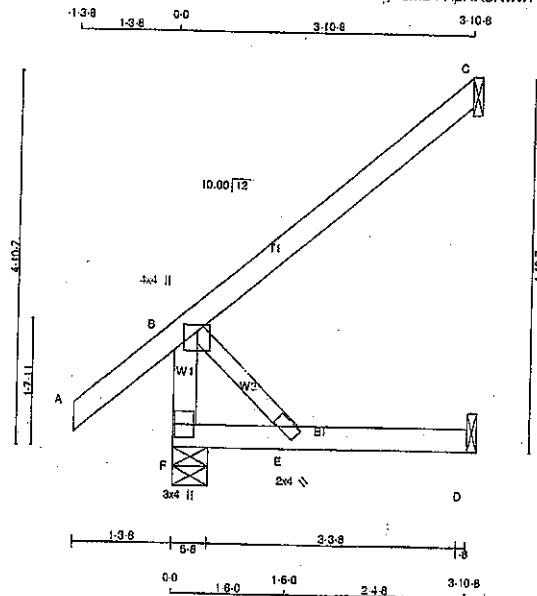


Structural component only
DWG# T-2108007

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417406	J15	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:SKJIDj76mu4YipKXoNwwHk5za4YO-Zm7Yrjy4hE_DmCuQCetkivY1kuxM6quDi4lbnCzZJW



Scale = 1:27.5

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	No.2
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

DESCR.
SPF
SPF
SPF

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
E BMV+w	MT20	2.0	4.0		
F BMV1+p	MT20	3.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
F	341	341	5-8	5-8
C	178	178	1-8	1-8
D	38	40	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	239	170	0	0	0	69	0
C	122	99	0	0	0	23	0
D	29	0	0	0	0	29	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (1)	MAX. LC2 (1)	MAX. UNBRACED LENGTH (1)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CS1 (1)
F-B	-305	0	0	0	0.03	7.81	0	0
A-B	0	41	-91.8	-91.8	0.14	10.00	0	0
B-C	0	0	-91.8	-91.8	0.23	10.00	0	0
F-E	0	0	-18.5	-18.5	0.08	10.00	0	0
E-D	0	0	-18.5	-18.5	0.08	10.00	0	0

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 15 = 45 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	38.0	PSF	

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 089-14
- TFC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.01")

CS1: TC=0.23/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SS1=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

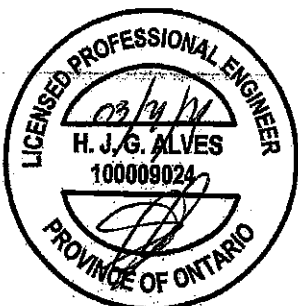
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.22 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)

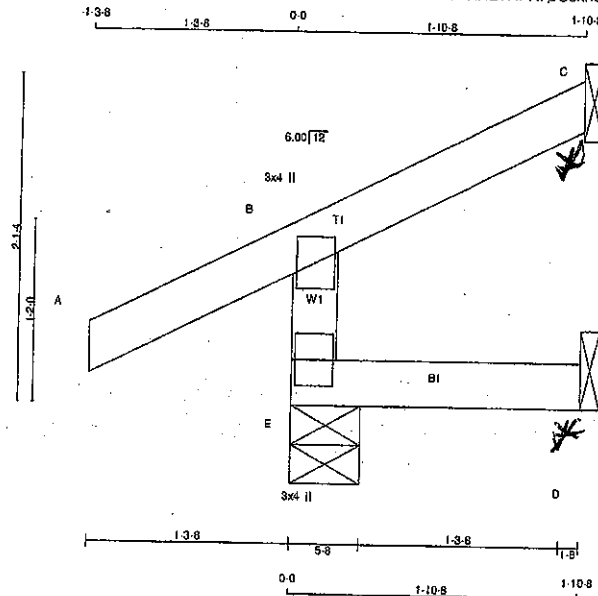


Structural component only
DWG# T-2108029

JOB NAME 417413	TRUSS NAME J21	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 4 X 7 = 28 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES:

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	273	0	273	0	5-8	5-8
C	50	0	50	0	1-8	1-8
D	8	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	190	143.0	0.0	0.0	0.0	47.0	0.0
C	35	27.17	0.0	0.0	0.0	7.0	0.0
D	8	0.8	0.0	0.0	0.0	12.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LOAD LC1 (LC)	MAX. CSI (LC)	MAX. UNBRACED LENGTH FR-TO	WEBS	
						MEMB. FORCE (LBS)	MAX. CSI (LC)
E-B	-247.0	0.0	0.0	0.04 (5)	7.81		
A-B	0.28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-16.0	-91.8	-91.8	0.09 (1)	6.25		
E-D	0.0	-18.5	-18.5	0.04 (5)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. GIG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF CBC 2012 (2015 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5), WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	850 371	1747 788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

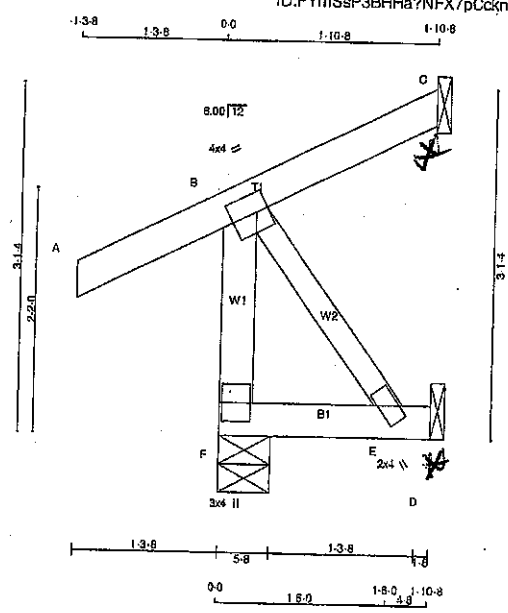
JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108044

JOB NAME 417413	TRUSS NAME J22	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

ID:PYmSsP3BHha?NFX7pCcknSza0qu-SeYBcXfCvWWEJl7y2hu2bhP8JB?axw0k?RCBQZZJIS



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
F - B	2x4 DRY	No.2
A - C	2x4 DRY	No.2
F - D	2x4 DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	4.0	4.0	2.00	1.25
E BMVW+w	MT20	2.0	4.0		
F BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	273	0	273	0	5-8	5-8
C	41	0	41	0	1-8	1-8
D	17	0	19	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D
PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	COMBINED	MAX. MIN. COMPONENT REACTIONS				WIND	DEAD	SOIL
		1ST CASE	SNOW	LIVE	PERM. LIVE			
F	190	143.0	0.0	0.0	0.0	0.0	47.0	0.0
C	28	23.0	-25.0	0.0	0.0	0.0	5.0	0.0
D	14	0.0	0.0	0.0	0.0	0.0	14.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F.

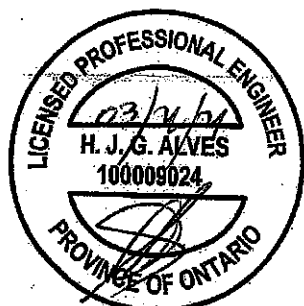
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS					WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO				FR-TO		
F-B	-256.0	0.0	0.0	0.03 (1)	7.81	B-E	0.0	0.00 (1)
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-20 / 0	-91.8	-91.8	0.11 (1)	6.25			
F-E	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
E-D	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



Structural component only
DWG# T-2108045

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (E-F:4),
WB=0.00/1.00 (B-E:1), SS1=0.09/1.00 (B-C:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1073

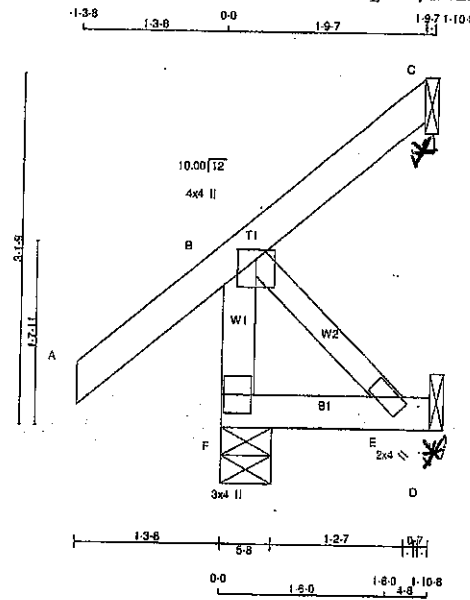
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.15 (B) (INPUT = 0.90)
JSI METAL = 0.04 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417406	C15	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:18.8

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMVW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	276	0	276	0	5-8	5-8
C	33	0	33	0	1-8	1-8
D	17	0	19	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
F	192	144	0	0	0	48	0
C	23	18	27	0	0	4	0
D	14	0	0	0	0	14	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	VERT. LOAD (PLF)	CS1 (LC)			MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO								
F-B	-258	0	0	0	0.03 (1)	7.81	0	0.00 (1)
A-B	0	41	-91.8	-91.8	0.13 (1)	10.00		
B-C	-32	0	-91.8	-91.8	0.12 (1)	6.25		
F-E	0	0	-18.5	-18.5	0.02 (4)	10.00		
E-D	0	0	-18.5	-18.5	0.01 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 10 LB

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/380 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CS1: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (E-F:4), WB=0.09/1.00 (B-E:1), SS1=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

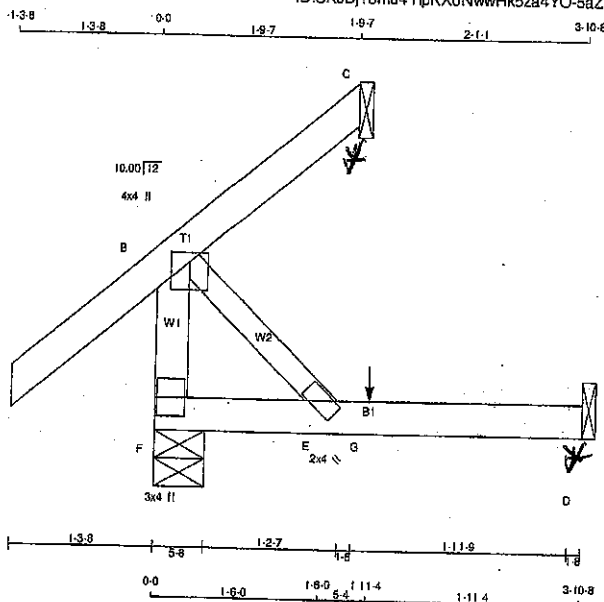


Structural component only
DWG# T-2108027

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417406	C16	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:16.8

LUMBER	N.L.C.A. RULES	CHORDS	SIZE	LUMBER
F - B	2x4	DRY	No.2	
A - C	2x4	DRY	No.2	
F - D	2x4	DRY	No.2	
ALL WEBS	2x3	DRY	No.2	
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00	
E BMVW+p	MT20	2.0	4.0			
F BMV1+p	MT20	3.0	4.0			

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	JT VERT	IN-SX	IN-SX
F 294	294	0	5-8
C 33	33	-40	1-8
D 36	40	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS	DEAD	SOIL
JT	SNOW	LIVE	PERM. LIVE
F 207	144	0	0
C 23	18	27	0
D 29	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1	MAX. CS1 (LC)	MEMB. FORCE (LBS)
FR-TO				
F-B	-258 / 0	0.0	0.0	0.03 (1)
A-B	0 / 41	-91.8	-91.8	0.14 (5)
B-C	-32 / 0	-91.8	-91.8	0.14 (5)
F-E	0 / 0	-18.5	-18.5	0.08 (4)
E-G	0 / 0	-18.5	-18.5	0.08 (4)
G-D	0 / 0	-18.5	-18.5	0.08 (4)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11.4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.
CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 12 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 6.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.14/1.00 (A-B-S), BC=0.08/1.00 (D-E-F), WB=0.00/1.00 (B-E-T), SSI=0.09/1.00 (A-B-S)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

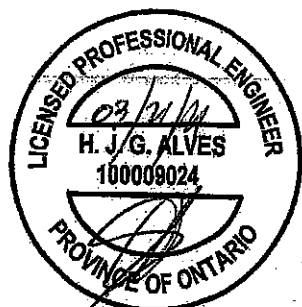
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.19 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108028

Tamarack Roof Truss, Burlington



DRY: SEASONED LUMBER.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

TOTAL WEIGHT = 4 X 7 = 28 lb

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E. C

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

TOTAL LOAD CASES: (5)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

•OVERHANG NOT TO BE ALTERED OR CUT OFF

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5),
WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

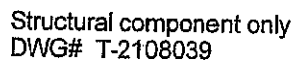
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(FSL)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL = 0.250 inches

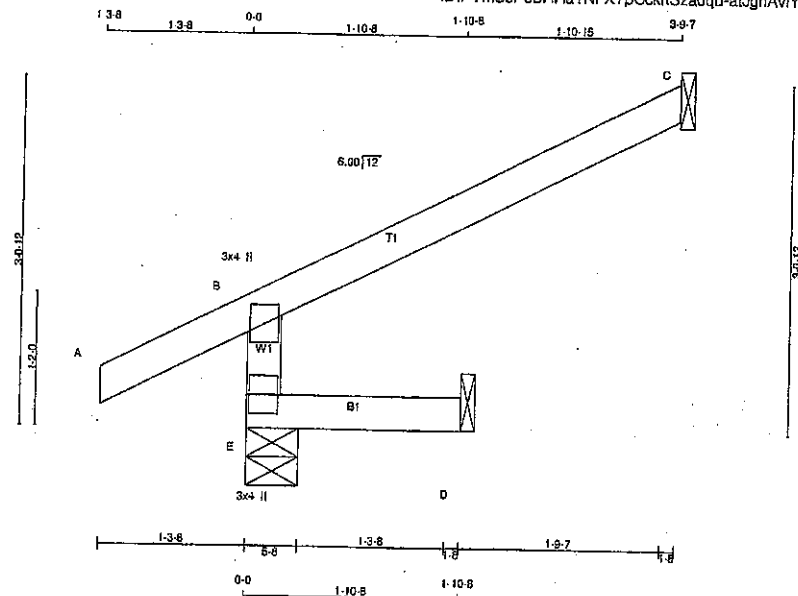
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



JOB NAME 417413	TRUSS NAME C22	QUANTITY 4	PLY 1	JOB DESC GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						

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Scale = 1:18.4

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
E	361	361	0	0
C	130	130	0	0
D	16	17	0	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	250	190	0	0	0	60	0
C	90	73	0	0	0	17	0
D	12	0	0	0	0	12	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	FACTORED (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO										
E-B		-342	0	0.0	0.0	0.01 (4)				7.81
A-B		0	28	-91.8	-91.8	0.13 (5)				10.00
B-C		-19	0	-91.8	-91.8	0.22 (1)				6.25
E-D		0	0	-18.5	-18.5	0.02 (4)				10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 10 = 38 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.22/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (via:0), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00, NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

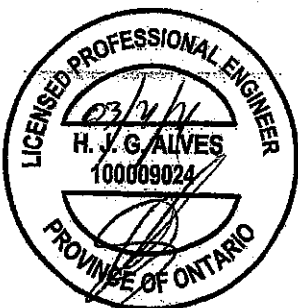
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	660 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

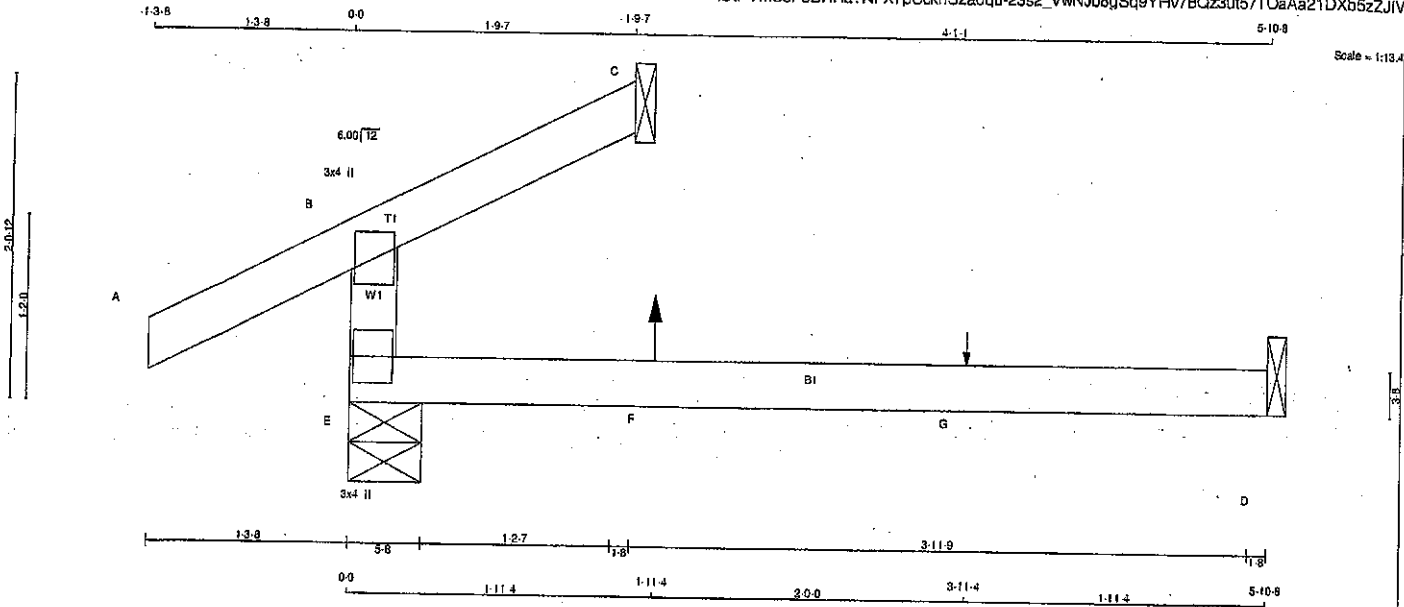
JSI GRIP=0.14 (E) (INPUT = 0.90)
JSI METAL=0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108040

JOB NAME 417413	TRUSS NAME C23	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER

N.L.G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
E BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
E	284	0	284	0	5-8	5-8
C	63	0	63	0	1-8	1-8
D	44	0	52	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX. MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	200	137.0	0.0	0.0	0.0	82.0	0.0
C	48	21.0	0.0	0.0	0.0	25.0	0.0
D	35	0.3	0.0	0.0	0.0	37.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FORCE (LBS)	FACTORED MAX CS (LC)
FR-TO		FROM TO			FR-TO		
E-B	-227.0	0.0	0.0	0.11 (4)	7.81		
A-B	0.28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-9.9	-91.8	-91.8	0.08 (4)	10.00		
E-F	0.0	-18.5	-18.5	0.14 (4)	10.00		
F-G	0.0	-18.5	-18.5	0.14 (4)	10.00		
G-D	0.0	-18.5	-18.5	0.14 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11.4	5	1	8	FRONT	VERT	TOTAL	---	C1
G	3-11.4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CS1=0.12/1.00 (A-B:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS1=0.09/1.00 (A-B:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

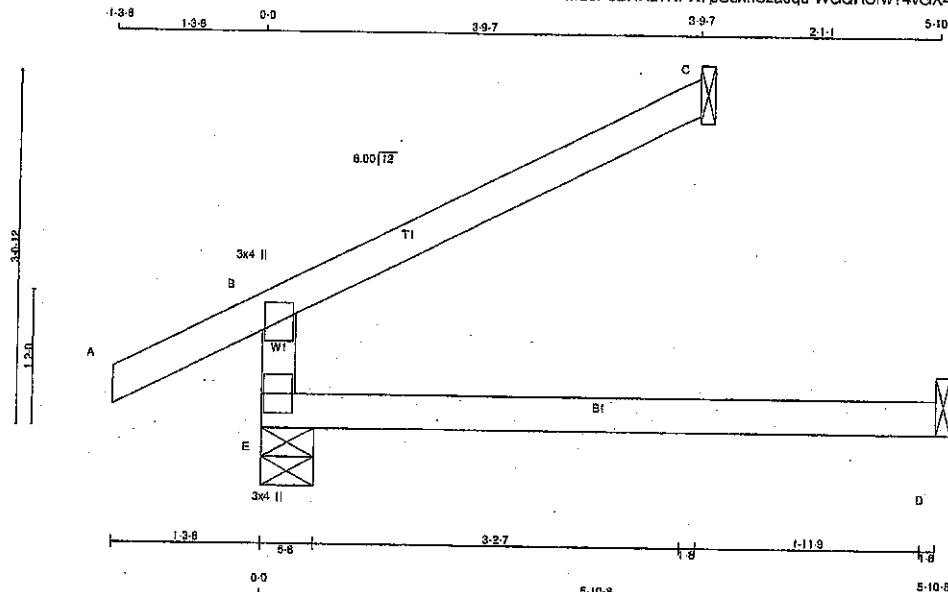


Structural component only
DWG# T-2108041

JOB NAME 417413	TRUSS NAME C24	QUANTITY 4	PLY 1	JOB DESC GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:18.2

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
E	405	405	5-8	5-8	
C	130	130	1-8	1-8	
D	45	50	1-8	1-8	

SEE MITEK STANDARD DETAIL B87731H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS						
JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD
E	288	190	0	0	0	95
C	90	73	0	0	0	17
D	36	0	0	0	0	36

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING						
TOTAL LOAD CASES: (4)						
CHORDS			WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MEMB. LENGTH	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO			FR-TO	
E-B	-342	0	0	0	7.81	
A-B	0	-28	-91.8	-91.8	10.00	
B-C	-19	0	-91.8	-91.8	0.22 (1)	8.25
E-D	0	0	-18.5	-18.5	0.13 (4)	10.00

TOTAL WEIGHT = 4 X 14 = 57 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.03")

CSI: TO=0.22/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	PLI	PLI	PLI
MT20	850	371	1747	788	1987

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108042



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1300218

Feb 09, 2018

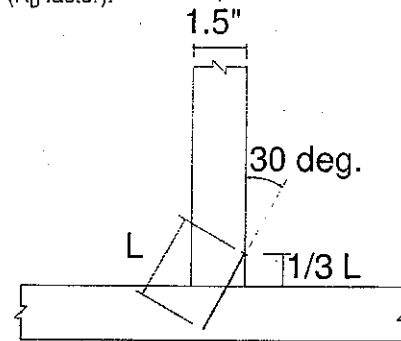
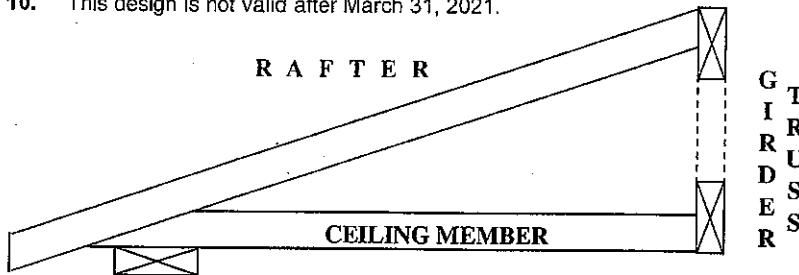
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.

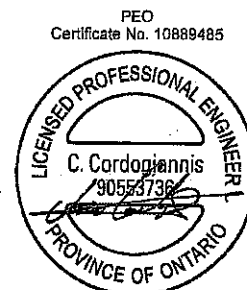


TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
(3.5" nail)		(3" and 3.25" nail)		
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

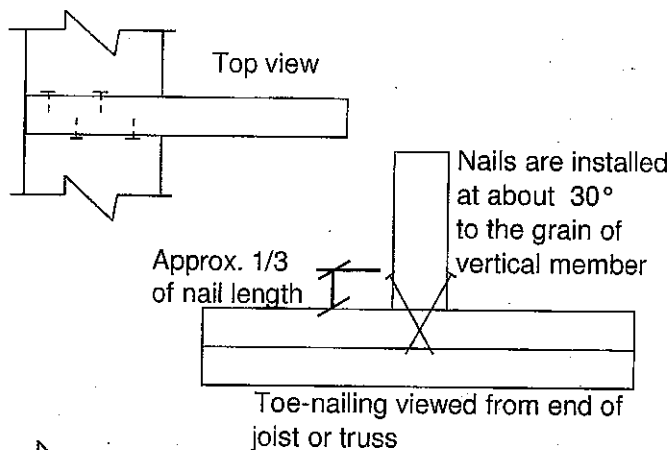
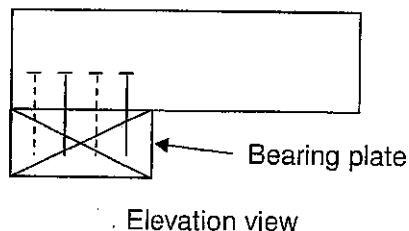
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

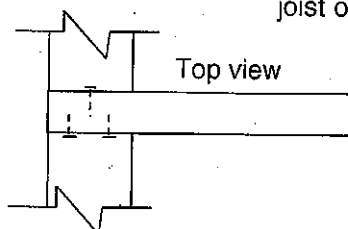
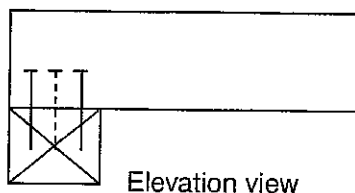
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate

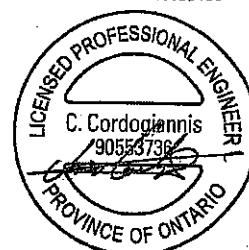


Toe-nailing on 2x4 Bearing Plate



MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



December 2, 2019

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

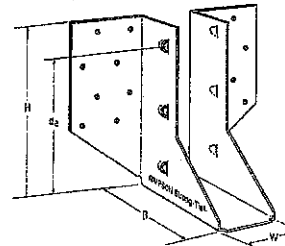
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

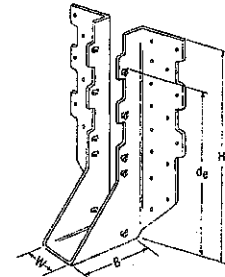
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

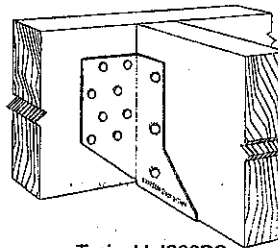
- See current catalogue for options



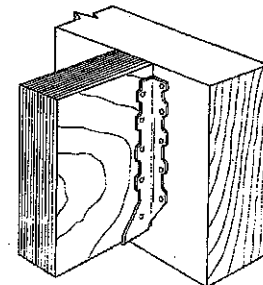
LJS26DS



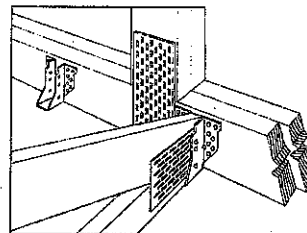
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



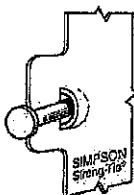
Typical HUS
Installation



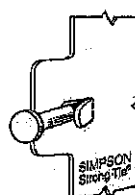
Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _a ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LJS26DS	18	1½	5	3½	4¾	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1½	5¾	3	3½	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1½	7½	3	6¾	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1½	9¾	3	7¾	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1½	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

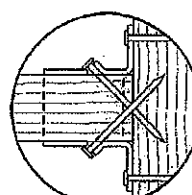
1. d_a is the distance from the seat of the hanger to the highest joist nail.



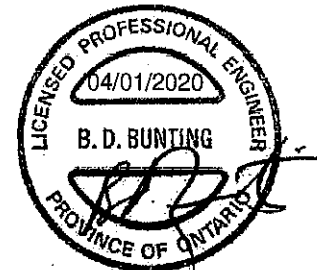
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,803,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPEC-HUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

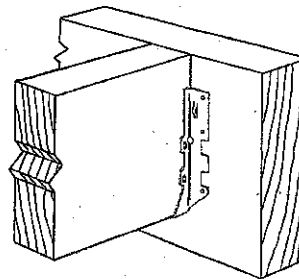
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

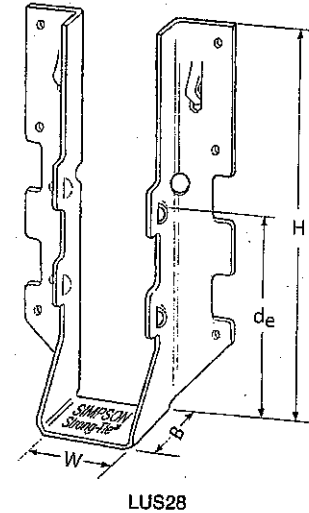
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



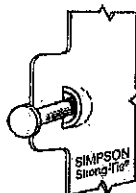
Typical LUS
Installation



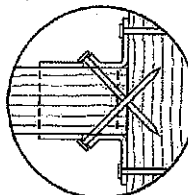
LUS28

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)	Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)
LUS24	18	1 9/16	3 3/8	1 3/4	1 9/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 3/8	2	1 9/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 3/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 3/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 3/8	4 3/8	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 3/8	1 3/4	3 3/8	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 3/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 3/8	8 3/8	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

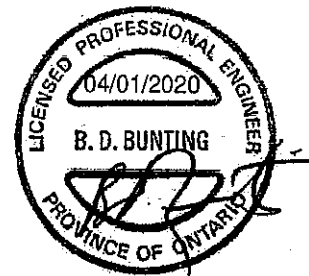
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing
Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPECLUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

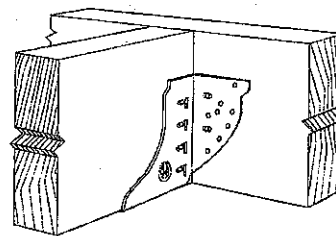
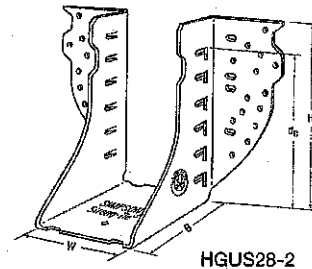
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

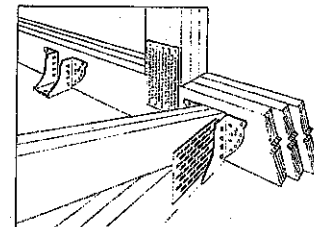
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



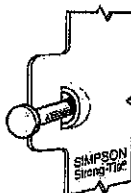
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

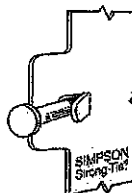
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₀ ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)	Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)
HGUS26	12	1 1/8	5 3/8	5	4 3/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 1/8	5 7/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 7/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1 1/8	9 1/8	5	7 7/8	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3 1/8	9 3/8	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 15/16	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 3/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 3/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 3/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

¹ d₀ is the distance from the seat of the hanger to the highest joist nail.

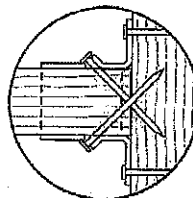


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHGUS20 3/20 exp. 6/22

(800) 999-5099
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TECHNICAL BULLETIN

H – Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

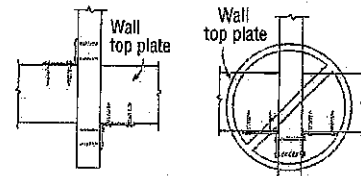
Design: • Factored resistances are in accordance with CSA O86-14
• Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

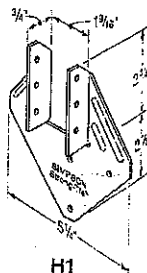
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

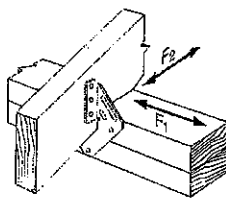


Install diagonally across from each other for minimum 2x truss.

Nailing into both sides of a single ply 2x truss may cause the wood to split.



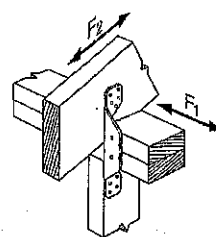
H1



H1 Installation



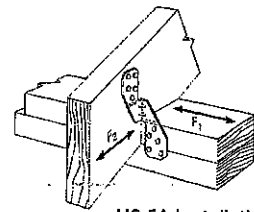
H2A



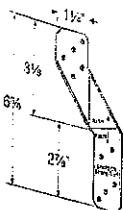
H2A Installation



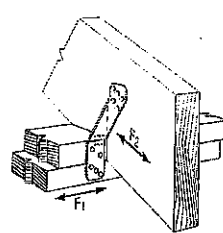
H2.5A



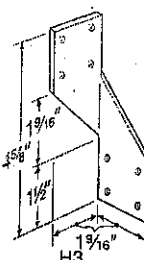
H2.5A Installation



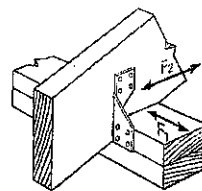
H2.5T



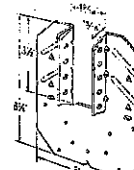
H2.5T Installation
(Nails into both top plates)



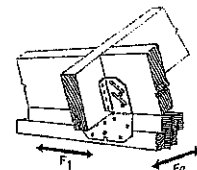
H3



H3 Installation



H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
		To Rafter	To Plates	To Studs	D.Fir-L			S-P-F		
					Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
						(K _p =1.15)			(K _p =1.15)	
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



**LIMIT
STATES
DESIGN**

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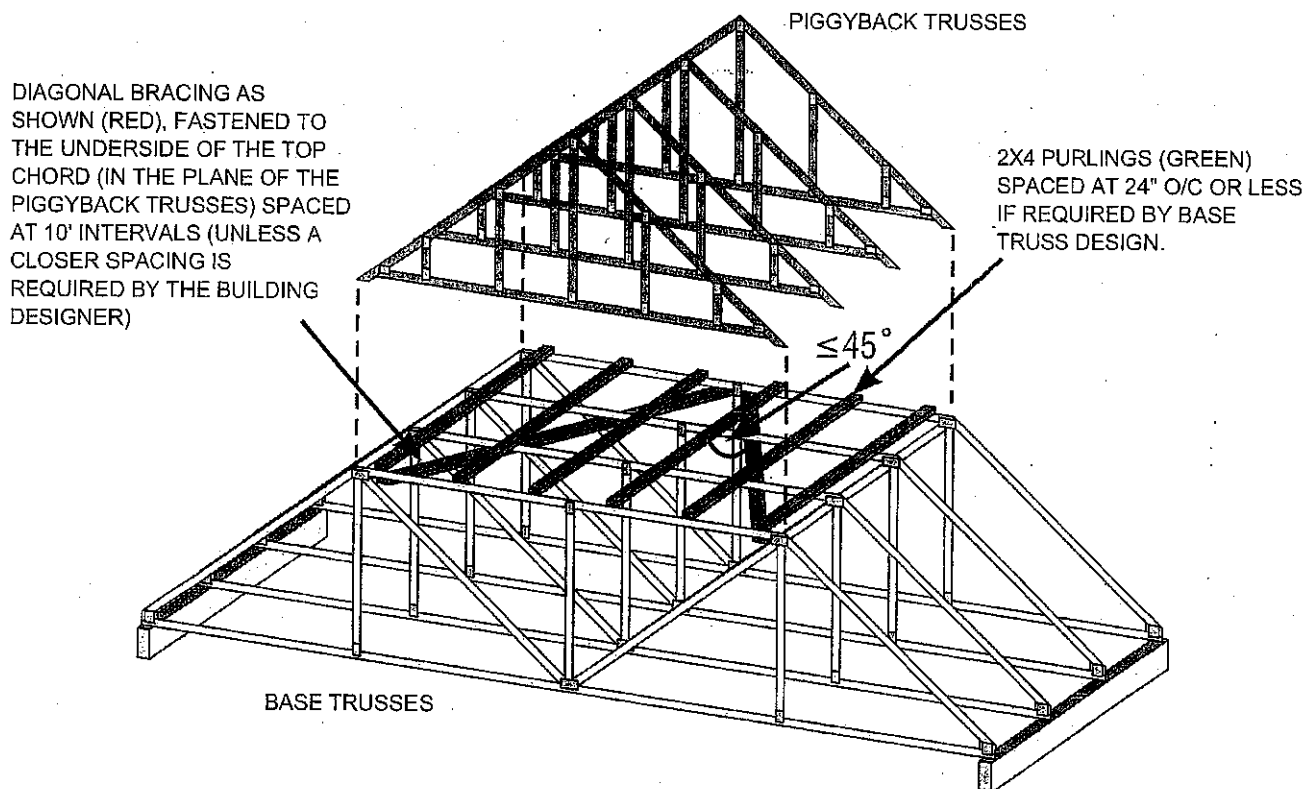
(800) 999-5099
strongtie.com

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

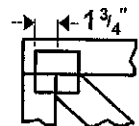
SKETCH FROM BCSI-CANADA 2013

Disclaimer:

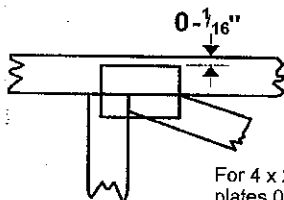
OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- $\frac{1}{16}$ \"/>



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4

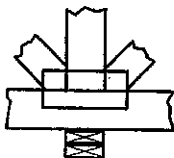
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

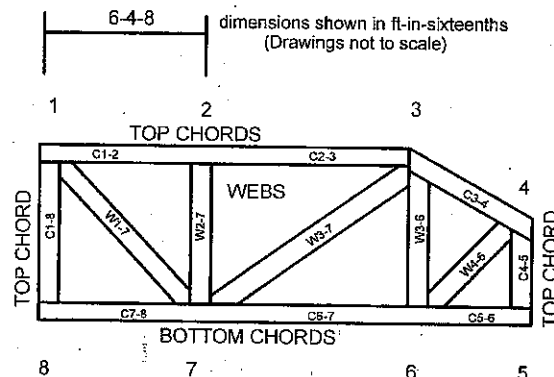


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek

MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.