

ALL ROOF PITCHES ARE 10/12
UNLESS OTHERWISE NOTED

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LUS26DS - (V)
LUS26-2 - (VV)
HGUS26-2 - (XX)

DENOTES
CONV.
FRAMING

DESIGN CONFORMS WITH
OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL
PART: 9
Ss = 31.3 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C.2012
(2018 AMENDMENT) ROOF RAFTERS THAT CROSS MEET OVER
TRUSSES TO BE 2X4 SPF @ 24" O.C. WITH A 2X4 VERT. POST
TO THE TRUSS UNDER NEATH AT EACH CROSS PT. VERT. POST
LONGER THAN 8" TO HAVE LATERAL BRACING SO THAT THE
DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING
DOES NOT EXCEED 8'

CITY OF HAMILTON
Building Division

Permit No. 22-1588

THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE

THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH
THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAWS

These drawings and/or specifications have been reviewed by

[Signature] DATE

FOR CHIEF BUILDING OFFICIAL

OCT 04 2002

OCT 04 2021
DATE

Rear
Upgrade



Job Track: 52166

Plan Log: **204480**

Layout ID: 417451

Builder / Location:

GREENPARK HOMES / HAMILTON

Project: **RUSSELL GARDENS PH.4**

Date: 2021-03-22

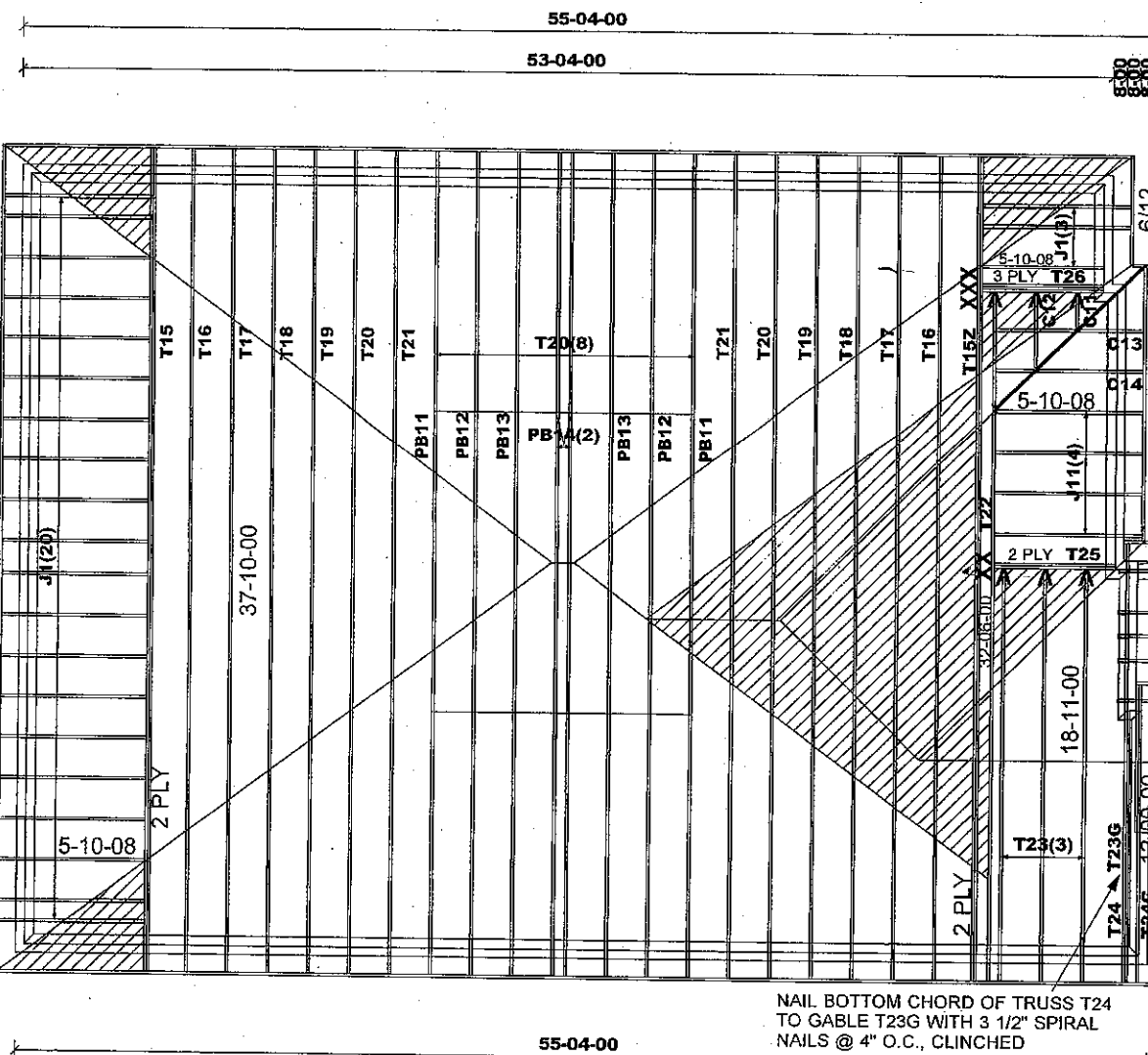
Sales: Rick DiCenzo

Designer: LC

Model / Elevation:

SPRINGFIELD 3 / EL.2 (WOOD/WALKOUT
CONDITION WITH REAR UP)

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



ALL ROOF PITCHES ARE 8/12
UNLESS OTHERWISE NOTED

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)
HGUS26-2 - (XX)
HGUS26-3 - (XXX)
H2.5T - (I)

DENOTES
CONV.
FRAMING



DESIGN CONFORMS WITH
OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL
PART: 9
Ss = 31.3 psf Sr = 8.4 psf

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ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C.2012
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LONGER THAN 6" TO HAVE LATERAL BRACING SO THAT THE
DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING
DOES NOT EXCEED 6"

NAIL BOTTOM CHORD OF TRUSS T24
TO GABLE T23G WITH 3 1/2" SPIRAL
NAILS @ 4" O.C., CLINCHED

M13788



Job Track: 52166

Plan Log: 204480

Layout ID: 417449

Builder / Location:

GREENPARK HOMES / HAMILTON

Project: RUSSELL GARDENS PH.4

Date: 2021-03-22

Sales:

Rick DiCiano

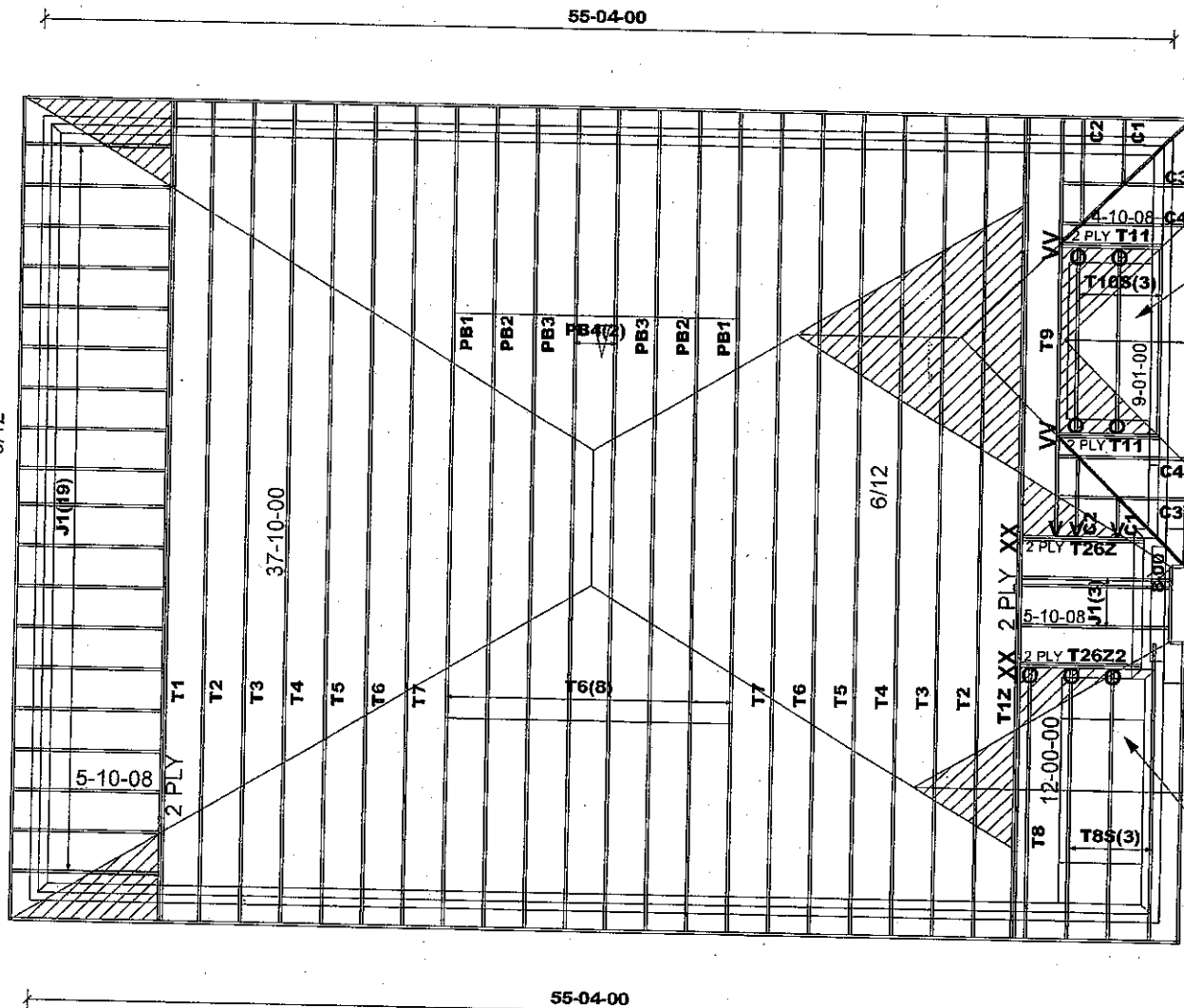
Designer: LC

Model / Elevation:

SPRINGFIELD 3 / EL.1

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TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER
PURPOSE.

Milek ver 8.4.2.286



ALL ROOF PITCHES ARE 10/12
UNLESS OTHERWISE NOTED

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
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HARDWARE:
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LJS26DS - (V)
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DENOTES
CONV.
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DOES NOT EXCEED 6"

M13788



Job Track: 52166

Plan Log: 204480

Layout ID: 417451

Builder / Location:

GREENPARK HOMES / HAMILTON

Project: RUSSELL GARDENS PH.4

Date: 2021-03-22

Sales:

Rick DiCiano

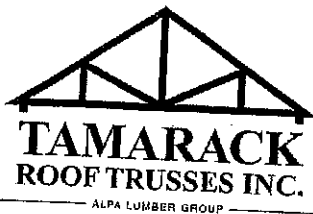
Designer: LC

Model / Elevation:

SPRINGFIELD 3 / EL.2

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PURPOSE

Mitek ver 8.4.2.286



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: SPRINGFIELD 3
 Lot #:
 Elevation: EL.1

Job Track: 52166
 PlanLog: 204480
 Layout ID: 417449
 Ref #
 Page: 1 of 3
 Date: 03-22-2021
 Designer: Leo Chen
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T15 Hip Girder	8/12	37-10-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	366.39 226.67		
	1 2-ply	T15Z Hip Girder	8/12	37-10-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	366.39 226.67		
	2	T16 Hip	8/12	37-10-00	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	365.39 217.00		
	2	T17 Hip	8/12	37-10-00	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	371.77 226.67		
	2	T18 Hip	8/12	37-10-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	388.4 239.00		
	2	T19 Hip	8/12	37-10-00	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	387.76 237.33		
	10	T20 Hip	8/12	37-10-00	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	1967.97 1185.00		
	2	T21 Hip	8/12	37-10-00	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	406.58 247.33		
	1	T22 Hip Girder	8/12	32-06-00	5-03-13	2 x 4 2 x 6	1-03-08	1-04-13 1-04-13	178.83 110.33		
	3	T23 Common	8/12	18-11-00	7-08-08	2 x 4	1-03-08	1-04-13 1-04-13	254.88 157.00		
	1	T23G GABLE	8/12	18-11-00	7-08-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	84.17 53.67		
	1	T24 Common	8/12	12-00-00	5-04-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	55.89 35.33		
	1	T24G GABLE	8/12	12-00-00	5-04-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	49.38 30.83		
	1 2-ply	T25 Monopitch Girder	8/12	5-10-08	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	65.45 41.33		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: SPRINGFIELD 3
 Lot #:
 Elevation: EL.1

Job Track: 52166
 PlanLog: 204480
 Layout ID: 417449
 Ref #
 Page: 2 of 3
 Date: 03-22-2021
 Designer: Leo Chen
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 3-ply	T26 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	87.59 56.50		
	2	PB11 Piggyback	8 /12	14-08-11	2-00-00	2 x 4			86.57 57.33		
	2	PB12 Piggyback	8 /12	14-08-11	3-00-00	2 x 4			84.67 54.67		
	2	PB13 Piggyback	8 /12	14-08-11	4-00-00	2 x 4			86.44 57.00		
	2	PB14 Piggyback	8 /12	14-08-11	4-10-14	2 x 4			89.33 55.67		
	23	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	386.27 245.33		
	4	J11 Jack-Open	8 /12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	78.76 46.67		
	5	J12 Jack-Partial	4 /12	6-05-08	2-10-11	2 x 4	1-03-08	3-15 2-05-12	95.15 60.83		
	1	C11 Jack-Open	8 /12	1-10-08	2-07-02	2 x 4	1-03-08	1-04-13 2-07-13	9.03 5.67		
	1	C12 Jack-Open	8 /12	1-10-08	3-11-02	2 x 4	1-03-08 1-10-15	1-04-13 2-07-13	11.77 7.67		
	1	C13 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 4-01-01	1-04-13 2-07-02	13.59 8.33		
	1	C14 Jack-Open	8 /12	3-09-07	3-11-02	2 x 4	1-03-08 2-01-01	1-04-13 3-11-02	16.33 10.33		

TOTAL # TRUSS= 80

TOTAL BFT OF ALL TRUSSES= 3900.16 BFT.

TOTAL WEIGHT OF ALL TRSSES 6342.72 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
1	Hardware	HGUS26-3	



DELIVERY SHIPLIST

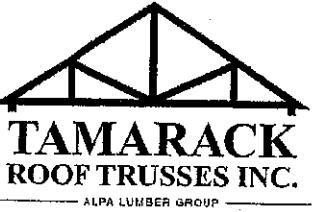
Lumber Yard: TAMARACK LUMBER
Builder: GREENPARK HOMES
Project: RUSSELL GARDENS PH.4
Location: HAMILTON
Model: SPRINGFIELD 3
Lot #:
Elevation: EL.1

Job Track: 52166
PlanLog: 204480
Layout ID: 417449
Ref #
Page: 3 of 3
Date: 03-22-2021
Designer: Leo Chen
Sales Rep: Rick DiCiano

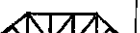
HARDWARE

QTY	TYPE	MODEL	LENGTH
6	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 8

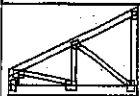
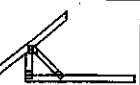
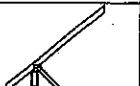
 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREENPARK HOMES Project: RUSSELL GARDENS PH.4 Location: HAMILTON Model: SPRINGFIELD 3 Lot #: Elevation: EL.2	Job Track: 52166 PlanLog: 204480 Layout ID: 417451 Ref # Page: 1 of 2 Date: 03-22-2021 Designer: Leo Chen Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	10/12	37-10-00	4-01-04	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	392.89 242.00		
	1 2-ply	T1Z Hip Girder	10/12	37-10-00	4-01-04	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	392.89 242.00		
	2	T2 Hip	10/12	37-10-00	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	369.62 230.33		
	2	T3 Hip	10/12	37-10-00	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	376.9 233.33		
	2	T4 Hip	10/12	37-10-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	392.95 241.00		
	2	T5 Hip	10/12	37-10-00	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	408.76 254.67		
	10	T6 Hip	10/12	37-10-00	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	2131.35 1303.33		
	2	T7 Hip	10/12	37-10-00	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	444.26 272.67		
	1	T8 Common	10/12	12-00-00	6-07-11	2 x 4	1-03-08	1-07-11 1-07-11	56.18 36.33		
	3	T8S Roof Special	10/12 6/12	12-00-00	6-07-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	176.41 116.00		
	1	T9 Hip Girder	10/12	19-04-00	5-08-07	2 x 4 2 x 6	1-03-08	1-07-11 1-07-11	97.19 60.83		
	3	T10S Scissor	10/12 6/12	9-01-00	5-06-05	2 x 4 2 x 6		1-08-15 1-08-15	143.6 97.00		
	2 2-ply	T11 Monopitch Girder	10/12	4-10-08	5-08-07	2 x 4 2 x 6		1-07-11 5-08-07	109.66 70.00		
	1 2-ply	T26Z Monopitch Girder	6/12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		

DELIVERY SHIPLIST		
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	Lumber Yard: TAMARACK LUMBER Builder: GREENPARK HOMES Project: RUSSELL GARDENS PH.4 Location: HAMILTON Model: SPRINGFIELD 3 Lot #: Elevation: EL.2	Job Track: 52166 PlanLog: 204480 Layout ID: 417451 Ref # Page: 2 of 2 Date: 03-22-2021 Designer: Leo Chen Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T26Z2 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	2	PB1 Piggyback	10 /12	19-11-01	2-00-00	2 x 4			123.71 80.33		
	2	PB2 Piggyback	10 /12	19-11-01	3-00-00	2 x 4			125.12 79.00		
	2	PB3 Piggyback	10 /12	19-11-01	4-00-00	2 x 4			129.68 82.67		
	2	PB4 Piggyback	10 /12	19-11-01	5-00-00	2 x 4			128.71 79.33		
	22	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	369.48 234.67		
	2	C1 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	19.69 14.00		
	2	C2 Jack-Open	10 /12	1-10-08	4-09-09	2 x 4	1-03-08 1-10-15	1-07-11 3-02-07	25.63 16.67		
	2	C3 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 3-01-01	1-07-11 3-01-09	26.53 18.00		
	2	C4 Jack-Open	10 /12	3-09-07	4-09-09	2 x 4	1-03-08 1-01-01	1-07-11 4-09-09	32.47 20.67		

TOTAL # TRUSS= 78

TOTAL BFT OF ALL TRUSSES= 4100.17

BFT.

TOTAL WEIGHT OF ALL TRSSES 6589.45 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
3	Hardware	LJS26DS	
7	Hardware	LUS24	
2	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 14



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: SPRINGFIELD 3
 Lot #:
 Elevation: EL.3

Job Track: 52166
 PlanLog: 204480
 Layout ID: 417455
 Ref #
 Page: 1 of 3
 Date: 03-22-2021
 Designer: Leo Chen
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T26Z3 Monopitch Girder	6/12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T26Z4 Monopitch Girder	6/12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T31 Hip Girder	6/12	37-10-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	389.15 242.67		
	1 2-ply	T31Z Hip Girder	6/12	37-10-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	389.15 242.67		
	2	T32 Hip	6/12	37-10-00	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	352.41 216.33		
	2	T33 Hip	6/12	37-10-00	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	354.63 216.00		
	2	T34 Hip	6/12	37-10-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	352.12 215.00		
	2	T35 Hip	6/12	37-10-00	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	364.92 222.67		
	10	T36 Hip	6/12	37-10-00	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1938.81 1183.33		
	2	T37 Hip	6/12	37-10-00	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	389.39 238.67		
	1	T38 Hip Girder	6/12	19-04-00	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 1-02-00	91.25 60.00		
	1	T39 Hip Girder	6/12	14-00-00	3-10-12	2 x 4		1-02-00 1-02-00	53.62 35.83		
	1	T40 Hip Girder	6/12	12-00-00	4-10-08	2 x 4 2 x 6	1-03-08	2-02-00 2-02-00	63.02 40.83		
	1 2-ply	T41 Half Hip Girder	6/12	5-10-08	3-08-08	2 x 4 2 x 6		1-02-00 3-08-08	61.95 39.33		



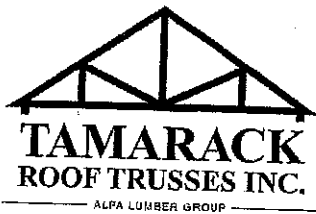
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: SPRINGFIELD 3
 Lot #:
 Elevation: EL.3

Job Track: 52166
 PlanLog: 204480
 Layout ID: 417455
 Ref #
 Page: 2 of 3
 Date: 03-22-2021
 Designer: Leo Chen
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T42 Half Hip Girder	6 /12	5-05-00	3-01-04	2 x 4	1-03-08	1-02-00 3-01-04	57.62 40.00		
	2	T43 Half Hip	6 /12	5-05-00	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	65.59 44.67		
	2	T44 Monopitch	6 /12	5-05-00	4-10-08	2 x 4	1-03-08	1-02-00 4-10-08	63.57 43.00		
	8	PB31 Piggyback	6 /12	6-01-00	1-06-04	2 x 4			99.64 69.33		
	17	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	285.51 181.33		
	3	J31 Jack-Open	6 /12	5-05-08	3-10-12	2 x 4	1-03-08	1-02-00 3-10-12	47.36 30.00		
	2	J32 Jack-Open	6 /12	1-10-08	2-01-04	2 x 4	1-03-08	1-02-00 2-01-04	14.27 9.33		
	2	J33 Jack-Open	6 /12	1-10-08	3-01-04	2 x 4	1-03-08	2-02-00 3-01-04	20.06 13.67		
	5	C31 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	35.1 23.33		
	5	C32 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	47.85 30.00		
	3	C33 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	34.75 22.00		
	3	C34 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	42.4 26.00		
	2	C35 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 3-08-01	1-02-00 2-00-12	22.21 14.67		
	2	C36 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 1-08-01	1-02-00 3-00-12	27.31 17.33		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: SPRINGFIELD 3
 Lot #:
 Elevation: EL.3

Job Track: 52166
 PlanLog: 204480
 Layout ID: 417455
 Ref #
 Page: 3 of 3
 Date: 03-22-2021
 Designer: Leo Chen
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
---------	------------	--------------	-------	------	--------	--------	---------------------------	------------------------------	--------------	---------------------	--------------------

TOTAL # TRUSS= 91

TOTAL BFT OF ALL TRUSSES= 3593.33

BFT.

TOTAL WEIGHT OF ALL TRSSES 5780.45 LBS

HARDWARE

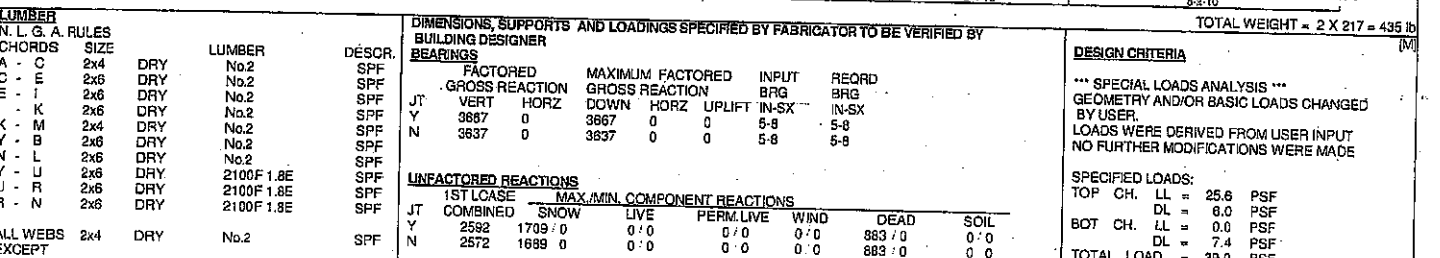
QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
10	Hardware	LJS26DS	
6	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 19

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ID:VlgrMMUy6rtfMpK04hHlxTzZhOf-hJEzKCUQtoNdHwUnkBmilH?wydE0NmOPWLMfZ5GS

The image shows a horizontal scale bar with various numerical markings. Above the bar, there is a title block containing version information, a date, and a page number. Below the bar, there is a scale indicator. The markings on the bar include: -13.8, 0.0, 2.11.8, 2.11.8, 5.3.3, 8.2.10, 5.4.15, 13.7.9, 5.2.3, 18.5.12, 10.11.0, 5.3.7, 24.2.7, 4.4.15, 29.7.5, 5.3.3, 34.10.8, 2.11.8, 37.10.0, 39.1.8, 1.3.8. The scale indicator at the bottom right reads 'Scale: 3/16"=1'.

Scale: 3/16"=1'



HORDS #ROWS SURFACE SPACING (IN) OP CHORDS : (0.122"x3") SPIRAL NAILS C 1 12 SIPS (61.0)	LOAD (PLF) ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. <u>LOADING</u> TOTAL LOAD CASES: (4)	*** NON STANDARD GIRDER *** ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.
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Structural component only
DWG# T-2108181 *W*

AH-AI	3234 / 0	-91.8	-91.8	0.27 (1)	4.03
AI-J	3234 / 0	-91.8	-91.8	0.27 (1)	4.03
J-AJ	3234 / 0	-91.8	-91.8	0.27 (1)	4.03
AJ-AK	3234 / 0	-91.8	-91.8	0.21 (1)	4.80
AK-K	3234 / 0	-91.8	-91.8	0.21 (1)	4.80
K-L	3460 / 0	-91.8	-91.8	0.12 (1)	4.94
L-M	0 / 41	-91.8	-91.8	0.07 (1)	10.00
Y-B	3655 / 0	0.0	0.0	0.13 (1)	7.35
N-L	3526 / 0	0.0	0.0	0.13 (1)	7.36

Y-AL	0 / 0	-18.5	-18.5	0.02 (4)	10.00
AL-X	0 / 0	-18.5	-18.5	0.02 (4)	10.00
X-AM	0 / 2729	-18.5	-18.5	0.09 (1)	10.00
AM-AN	0 / 2729	-18.5	-18.5	0.09 (1)	10.00
AN-W	0 / 2729	-18.5	-18.5	0.09 (1)	10.00
W-AO	0 / 6248	-18.5	-18.5	0.20 (1)	10.00
AO-AP	0 / 6248	-18.5	-18.5	0.20 (1)	10.00
AP-AQ	0 / 6248	-18.5	-18.5	0.20 (1)	10.00
AQ-V	0 / 6248	-18.5	-18.5	0.20 (1)	10.00
V-AR	0 / 8336	-18.5	-18.5	0.25 (1)	10.00
AR-U	0 / 8336	-18.5	-18.5	0.25 (1)	10.00
U-AS	0 / 8336	-18.5	-18.5	0.25 (1)	10.00
AS-T	0 / 8336	-18.5	-18.5	0.25 (1)	10.00
T-AT	0 / 9008	-18.5	-18.5	0.26 (1)	10.00
AT-S	0 / 9008	-18.5	-18.5	0.26 (1)	10.00

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION	
(PSI) (PLI)	
MAX MIN MAX MIN MAX MIN	
MT20 650 371 1747 798 1987 1873	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.86 (P) (INPUT = 0.50)
JSI METAL = 0.78 (U) (INPUT = 1.00)



Structural component only
DWG# T-2108181 *WR*

JOB NAME 417451	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:VlarMMUy6rt?MpkQ4hHxTzZhoI-hJezkCUQtoNdHWUnkBmlLH?wydE0NfmOPWLMFzZ5GS

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	5.0	8.0	2.50	2.25
D, F, G, H, J						
D	TMWW-t	MT20	5.0	6.0		
E	TS-t	MT20	5.0	6.0		
I	TS-t	MT20	5.0	6.0		
K	TTWW+m	MT20	5.0	8.0	2.50	2.25
L	TMVW+p	MT20	5.0	8.0	2.00	2.25
N	BMV1+p	MT20	3.0	6.0		
O, P, Q, S, T, V, W, X						
O	BMWW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
U	BS-t	MT20	5.0	6.0		
Y	BMV1+p	MT20	3.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO	0 / 8324						
S-AU	0 / 8324	-18.5	-18.5 0.25 (1)	10.00			
AU-R	0 / 8324	-18.5	-18.5 0.25 (1)	10.00			
R-AV	0 / 8324	-18.5	-18.5 0.25 (1)	10.00			
AV-Q	0 / 8324	-18.5	-18.5 0.25 (1)	10.00			
Q-AW	0 / 8324	-18.5	-18.5 0.25 (1)	10.00			
AW-AX	0 / 8324	-18.5	-18.5 0.20 (1)	10.00			
AX-AY	0 / 8324	-18.5	-18.5 0.20 (1)	10.00			
AY-P	0 / 8324	-18.5	-18.5 0.20 (1)	10.00			
P-AZ	0 / 2705	-18.5	-18.5 0.09 (1)	10.00			
AZ-BA	0 / 2705	-18.5	-18.5 0.09 (1)	10.00			
BA-O	0 / 2705	-18.5	-18.5 0.09 (1)	10.00			
O-BB	0 / 0	-18.5	-18.5 0.02 (4)	10.00			
BB-N	0 / 0	-18.5	-18.5 0.02 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-108	-108	---	FRONT	VERT	TOTAL	---	C1
C	2-11-8	-125	-125	---	FRONT	VERT	SNOW	---	C1
E	13-0-4	-78	-78	---	FRONT	VERT	TOTAL	---	C1
G	18-9-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
I	24-9-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
K	34-10-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
K	34-10-8	-82	-82	---	FRONT	VERT	TOTAL	---	C1
O	34-10-8	-125	-125	---	FRONT	VERT	SNOW	---	C1
O	34-9-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
X	3-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Z	5-0-4	-78	-78	---	FRONT	VERT	TOTAL	---	C1
AA	7-0-4	-78	-78	---	FRONT	VERT	TOTAL	---	C1
AB	9-0-4	-78	-78	---	FRONT	VERT	TOTAL	---	C1
AC	11-0-4	-78	-78	---	FRONT	VERT	TOTAL	---	C1
AD	15-0-4	-78	-78	---	FRONT	VERT	TOTAL	---	C1
AE	17-0-4	-78	-78	---	FRONT	VERT	TOTAL	---	C1
AF	20-9-12	-78	-78	---	FRONT	VERT	TOTAL	---	C1
AG	22-9-12	-78	-78	---	FRONT	VERT	TOTAL	---	C1
AH	26-9-12	-78	-78	---	FRONT	VERT	TOTAL	---	C1
AI	28-9-12	-78	-78	---	FRONT	VERT	TOTAL	---	C1
AJ	30-9-12	-78	-78	---	FRONT	VERT	TOTAL	---	C1
AK	32-9-12	-78	-78	---	FRONT	VERT	TOTAL	---	C1
AL	1-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AM	5-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AN	7-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AO	9-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AP	11-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AQ	13-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AR	15-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AS	17-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AT	18-9-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AU	20-9-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AV	22-9-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AW	24-9-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AX	26-9-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AY	28-9-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AZ	30-9-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
BA	32-9-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
BB	36-9-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

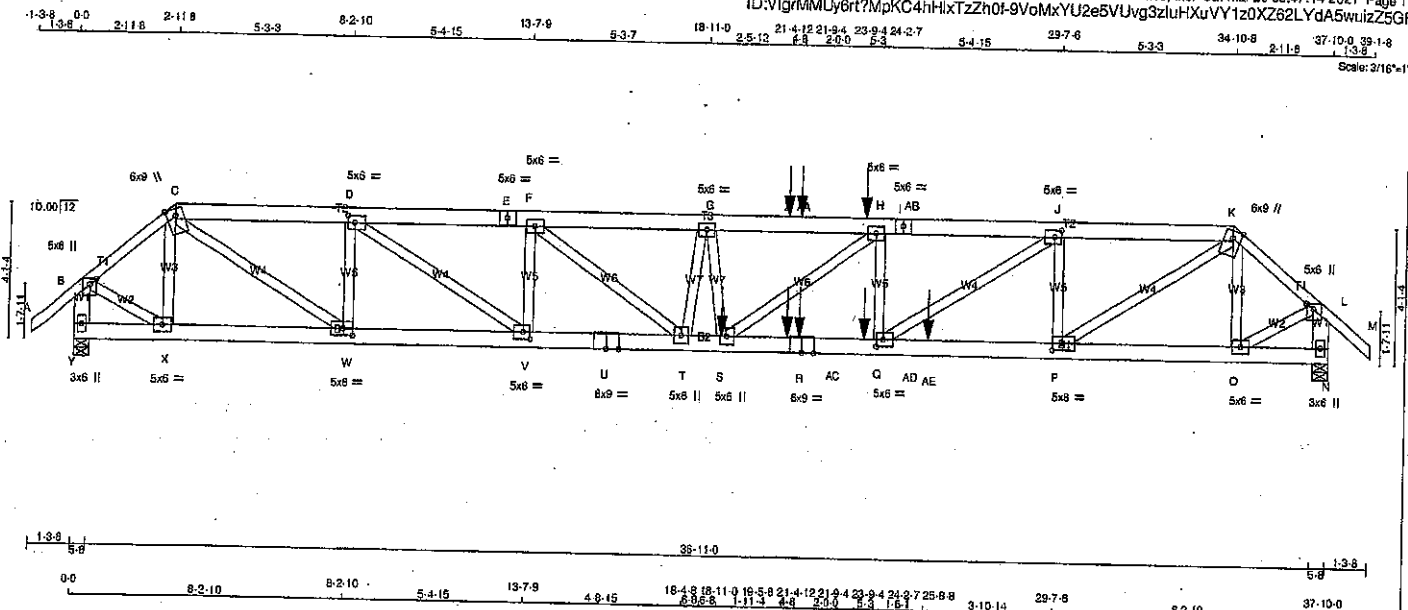


Structural component only
DWG# T-2108181 *m*

JOB NAME 417451	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x6	DRY No.2	SPF
E - I	2x6	DRY No.2	SPF
I - K	2x6	DRY No.2	SPF
K - M	2x4	DRY No.2	SPF
M - O	2x6	DRY No.2	SPF
O - R	2x6	DRY No.2	SPF
R - N	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122'X3') SPIRAL NAILS		
A-C	12	TOP
K-M	12	TOP
C-E	12	TOP
E-I	12	TOP
I-K	12	TOP
Y-B	12	TOP
N-L	12	TOP
BOTTOM CHORDS: (0.122'X3') SPIRAL NAILS		
Y-U	12	TOP
U-R	12	TOP
R-N	12	TOP
WEBS (0.122'X3') SPIRAL NAILS		
G-S	5	SIDE(0.0)
2x4	5	SIDE(0.0)
T-G	5	SIDE(365.4)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT 3918	DOWN 3918	0	5-8
N	HORZ 4605	0	0	5-8

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
Y	2752	1882.0 0.0 0.0 0.0 900.0 0.0
N	3245	2193.0 0.0 0.0 0.0 1053.0 0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO									
A-B	0.41	-91.8	-91.8	0.07 (1)	10.00	X-C	-1125.0	0.10 (1)	0.10 (1)
B-C	-3768.0	-91.8	-91.8	0.13 (1)	4.27	V-F	-2471.0	0.21 (1)	0.21 (1)
C-D	-7583.0	-91.8	-91.8	0.20 (1)	4.27	Q-H	-1348.0	0.12 (1)	0.12 (1)
D-E	-11350.0	-91.8	-91.8	0.32 (1)	3.49	O-K	-1465.0	0.13 (1)	0.13 (1)
E-F	-11350.0	-91.8	-91.8	0.32 (1)	3.49	B-X	0.3166	0.28 (1)	0.28 (1)
F-G	-13948.0	-91.8	-91.8	0.46 (1)	3.03	O-L	0.3732	0.33 (1)	0.33 (1)
G-Z	-14345.0	-91.8	-91.8	0.57 (1)	2.89	F-T	0.3281	0.29 (1)	0.29 (1)
Z-AA	-14345.0	-91.8	-91.8	0.57 (1)	2.89	T-G	-1549.0	0.14 (1)	0.14 (1)
AA-AB	-14345.0	-91.8	-91.8	0.57 (1)	2.89	G-S	0.912	0.08 (1)	0.08 (1)
AB-H	-14345.0	-91.8	-91.8	0.57 (1)	2.89	S-H	0.1074	0.09 (1)	0.09 (1)
H-I	-13485.0	-91.8	-91.8	0.42 (1)	3.14	D-V	0.4586	0.40 (1)	0.40 (1)
I-J	-13485.0	-91.8	-91.8	0.42 (1)	3.14	C-W	0.5623	0.50 (1)	0.50 (1)
J-K	-9192.0	-91.8	-91.8	0.26 (1)	3.88	W-D	-3128.0	0.27 (1)	0.27 (1)
K-L	-4450.0	-91.8	-91.8	0.15 (1)	4.44	P-K	0.6959	0.62 (1)	0.62 (1)
L-M	0.41	-91.8	-91.8	0.07 (1)	10.00	Q-J	0.5189	0.46 (1)	0.46 (1)
Y-B	-3928.0	0.0	0.0	0.14 (1)	7.16	P-J	-3454.0	0.30 (1)	0.30 (1)
N-L	-4580.0	0.0	0.0	0.17 (1)	6.74				

Y-X	0.0	-18.5	-18.5	0.02 (1)	10.00
X-W	0.2949	-18.5	-18.5	0.10 (1)	10.00
W-V	0.7583	-18.5	-18.5	0.23 (1)	10.00
V-U	0.11350	-18.5	-18.5	0.23 (1)	10.00
U-T	0.11350	-18.5	-18.5	0.33 (1)	10.00
T-S	0.74233	-18.5	-18.5	0.50 (1)	10.00
S-AC	0.13495	-18.5	-18.5	0.48 (1)	10.00
AC-R	0.13495	-18.5	-18.5	0.48 (1)	10.00
R-AD	0.13495	-18.5	-18.5	0.48 (1)	10.00
AD-Q	0.13495	-18.5	-18.5	0.48 (1)	10.00
Q-AE	0.9192	-18.5	-18.5	0.48 (1)	10.00
AE-P	0.9192	-18.5	-18.5	0.48 (1)	10.00
P-O	0.3481	-18.5	-18.5	0.15 (1)	10.00
O-N	0.0	-18.5	-18.5	0.01 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)	JT	LOC	LC1	MAX	MAX+	FACE	DIR	TYPE	HEEL	CONN.
R	21-9-4	-21	-21	---	---	BACK	VERT	TOTAL	---	C1
S	19-5-8	-1517	-1517	---	---	BACK	VERT	TOTAL	---	C1
Z	21-4-12	-76	-76	---	---	BACK	VERT	TOTAL	---	C1
AA	21-9-4	-76	-76	---	---	BACK	VERT	TOTAL	---	C1
AB	23-9-4	-76	-76	---	---	BACK	VERT	TOTAL	---	C1
AC	21-4-12	-21	-21	---	---	BACK	VERT	TOTAL	---	C1
AD	23-9-4	-21	-21	---	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/898 (0.34")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/748 (0.61")

CSI: TC=0.571/0.0 (G-H:1), BC=0.50/1.00 (S-T:1), WB=0.62/1.00 (K-P:1), SS=0.40/1.00 (P-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (W) (INPUT = 0.80)

JSI METAL= 0.99 (U) (INPUT = 1.00)



Structural component only
DWG# T-2108182

CONTINUED ON PAGE 2

JOE NAME: 417451	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 09:47:14 2021 Page 2
ID:VlqrMMLv6r1?MpKC4hHxTzZh0f-9VoMxYU2e5VUva3zluHXuVY1z0XZ82LYdA5wulz25GR

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMVW-t	MT20	5.0	6.0	2.50	2.50
E	TS-t	MT20	5.0	6.0		
F, G, H						
F	TMVW-t	MT20	5.0	6.0		
I	TS-t	MT20	5.0	6.0		
J	TMVW-t	MT20	5.0	6.0	2.50	2.50
K	TTWW+m	MT20	6.0	9.0	Edge	
L	TMVW+p	MT20	5.0	6.0	2.00	2.25
N	BMV1+p	MT20	3.0	6.0		
O	BMVW-t	MT20	5.0	6.0		
P	BMVW-t	MT20	5.0	6.0	2.50	3.50
Q	BMVW-t	MT20	5.0	6.0	2.50	2.60
R	BS-t	MT20	6.0	9.0		
S	BMVW-t	MT20	5.0	6.0		
T	BMVW-t	MT20	5.0	6.0		
U	BS-t	MT20	6.0	9.0		
V	BMVW-t	MT20	5.0	6.0	2.50	2.50
W	BMVW-t	MT20	5.0	6.0	2.50	3.50
X	BMVW-t	MT20	5.0	6.0		
Y	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOLCHES EDGE OF CHORD.

NOTES: (1)

- 1) Lateral bracing to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AE	25-8-8	-1075	-1075	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2108182 *ML*

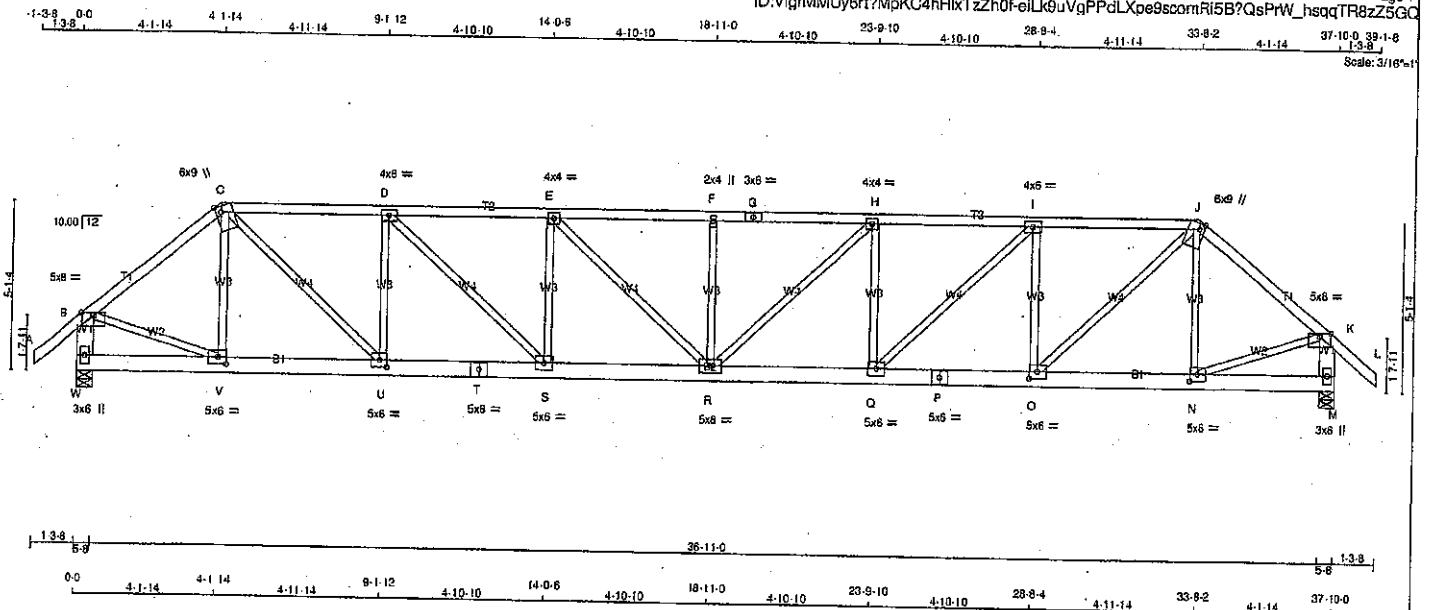
JOB NAME 417451	TRUSS NAME T2	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitak Industries, Inc. Sat Mar 20 09:47:15 2021 Page 1

ID:VlgrMMUy8r1?MpKC4hHtZzh0f-elLk9uVgPPdLXpe9scomRi5B?QsPrW_hsqQTR8zZ5GQ

Scale: 3/16"=1'



TOTAL WEIGHT = 2 X 186 = 370 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	Edge	
C TTWW-m	MT20	5.0	9.0	Edge 1.75	
D TMVW-l	MT20	4.0	8.0		
E TMVW-t	MT20	4.0	4.0		
F TMVW-w	MT20	2.0	4.0		
G TS-l	MT20	3.0	6.0		
H TMVW-t	MT20	4.0	4.0		
I TMVW-l	MT20	4.0	6.0		
J TTWW-m	MT20	6.0	9.0	Edge 1.75	
K TMVW-p	MT20	5.0	8.0	Edge	
M BMV1-p	MT20	3.0	6.0		
N, O, U, V					
N BMVW-t	MT20	5.0	6.0	2.50	2.75
P BS-l	MT20	5.0	6.0		
Q BMVW-t	MT20	5.0	6.0		
R BMVW-t	MT20	5.0	6.0		
S BMVW-t	MT20	5.0	6.0		
T BS-l	MT20	5.0	6.0		
W BMV1-p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	DOWN	BRG	BRG
W	2213	0	0	5-8
M	2213	0	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX MIN	COMPONENT REACTIONS	PERM LIVE	WIND	DEAD	SOIL
W	1562	1039	0	0	0	524	0
M	1562	1039	0	0	0	524	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00	V-C	-368 / 0	0.13 (1)
B-C	-2165 / 0	-91.8	-91.8 0.38 (1)	4.27	N-J	-368 / 0	0.13 (1)
C-D	-3103 / 0	-91.8	-91.8 0.51 (1)	3.53	B-V	0 / 1728	0.39 (1)
D-E	-3923 / 0	-91.8	-91.8 0.62 (1)	3.08	N-K	0 / 1728	0.39 (1)
E-F	-4197 / 0	-91.8	-91.8 0.57 (1)	3.06	O-J	0 / 1997	0.45 (1)
F-G	-4197 / 0	-91.8	-91.8 0.57 (1)	3.06	C-U	0 / 1997	0.45 (1)
G-H	-4197 / 0	-91.8	-91.8 0.57 (1)	3.06	O-I	-1291 / 0	0.48 (1)
H-I	-3923 / 0	-91.8	-91.8 0.82 (1)	3.06	U-D	-1291 / 0	0.48 (1)
I-J	-3103 / 0	-91.8	-91.8 0.51 (1)	3.53	O-I	0 / 1141	0.28 (1)
J-K	-2165 / 0	-91.8	-91.8 0.38 (1)	4.27	D-S	0 / 1141	0.28 (1)
K-L	0 / 41	-91.8	-91.8 0.13 (1)	10.00	Q-H	-897 / 0	0.25 (1)
W-B	-2165 / 0	0.0	0.0 0.15 (1)	6.90	S-E	-897 / 0	0.25 (1)
M-K	-2165 / 0	0.0	0.0 0.15 (1)	6.90	R-H	0 / 381	0.09 (1)
					E-R	0 / 381	0.09 (1)
					R-F	-454 / 0	0.16 (1)
W-V	0 / 0	-18.5	-18.5 0.05 (4)	10.00			
V-U	0 / 1651	-18.5	-18.5 0.23 (1)	10.00			
U-T	0 / 3104	-18.5	-18.5 0.41 (1)	10.00			
T-S	0 / 3104	-18.5	-18.5 0.41 (1)	10.00			
S-R	0 / 3923	-18.5	-18.5 0.52 (1)	10.00			
R-Q	0 / 3923	-18.5	-18.5 0.52 (1)	10.00			
Q-P	0 / 3104	-18.5	-18.5 0.41 (1)	10.00			
P-O	0 / 3104	-18.5	-18.5 0.41 (1)	10.00			
O-N	0 / 1651	-18.5	-18.5 0.23 (1)	10.00			
N-M	0 / 0	-18.5	-18.5 0.05 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/989 (0.24")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.45")

CSI: TC=0.62/1.00 (D-E:1), BC=0.52/1.00 (R-S:1), WB=0.46/1.00 (D-U:1), SS=0.21/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

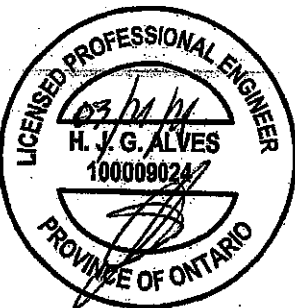
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

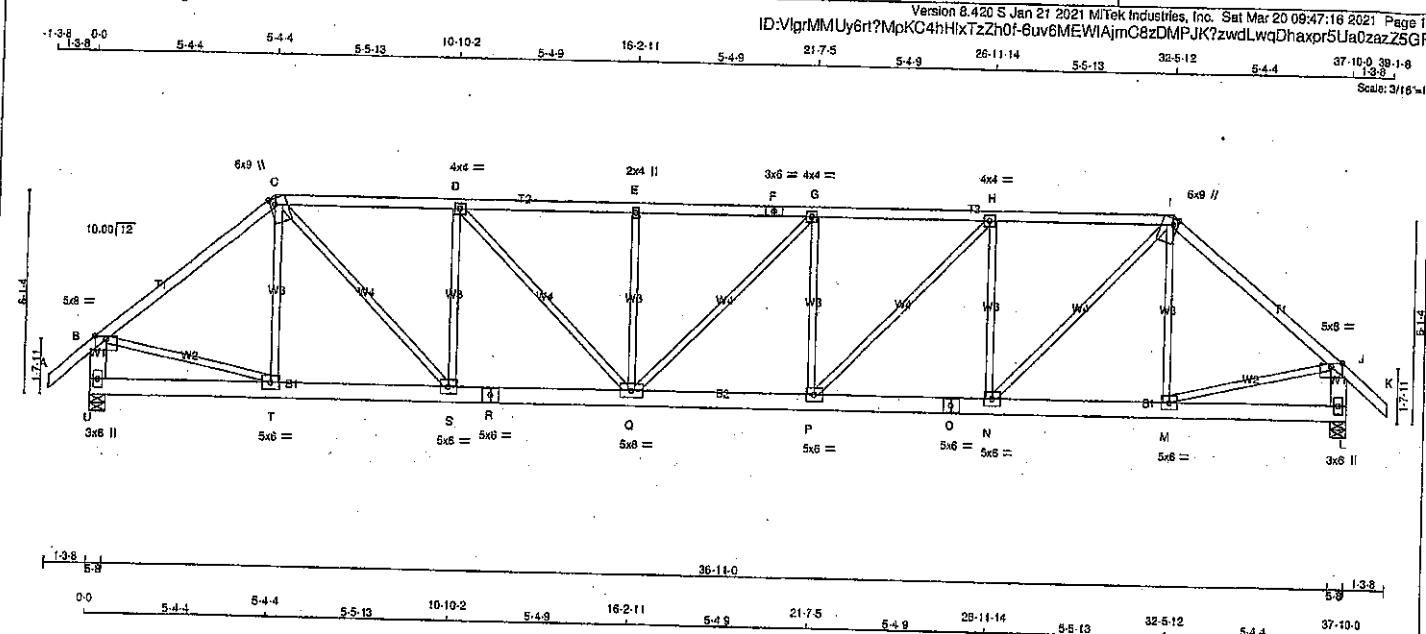
JSI GRIP= 0.90 (V) (INPUT = 0.90)

JSI METAL= 0.58 (T) (INPUT = 1.00)



Structural component only
DWG# T-2108183

JOB NAME 417451	TRUSS NAME T3	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 09:47:16 2021 Page 1	



LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
U - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - O	2x6	DRY	No.2
O - L	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	8.0 Edge
C	TTWW+m	MT20	6.0	9.0 Edge 1.75
D, G, H				
D	TMVW-l	MT20	4.0	4.0
E	TMVW-w	MT20	2.0	4.0
F	TS-l	MT20	3.0	6.0
I	TTWW+m	MT20	6.0	9.0 Edge 1.75
J	TMVW-p	MT20	5.0	8.0 Edge
L	BMV1-p	MT20	3.0	6.0
M, N, P, S, T				
M	BMVW-l	MT20	5.0	6.0
O	BS-l	MT20	5.0	6.0
Q	BMVW-w	MT20	5.0	8.0
R	BS-l	MT20	5.0	6.0
U	BMV1-p	MT20	3.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
U	2213	0	0	2213	0	0	0	5-8	5-8
L	2213	0	0	2213	0	0	0	5-8	5-8

UNFACTORED REACTIONS		1ST LOASE		MAX/MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
U	1562	1039	0	0	0
L	1562	1039	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MEMB.		WEBS	
MAX. FACTORED	FORCE (LBS)	MAX. FACTORED	FORCE (LBS)	MAX. FACTORED	FORCE (LBS)
FR-TO		FR-TO		FR-TO	
A-B	0/41	A-B	0/41	T-C	-263/0
B-C	-2211/0	B-C	-2211/0	M-I	-261/0
C-D	-2867/0	C-D	-2867/0	B-T	0/1737
D-E	-3403/0	D-E	-3403/0	M-J	0/1737
E-F	-3403/0	E-F	-3403/0	N-I	0/1695
F-G	-3403/0	F-G	-3403/0	C-S	0/1697
G-H	-3409/0	G-H	-3409/0	N-H	-1192/0
H-I	-2865/0	H-I	-2865/0	S-D	-1124/0
I-J	-2211/0	I-J	-2211/0	P-H	0/785
J-K	0/41	J-K	0/41	D-Q	0/784
U-B	-2172/0	U-B	-2172/0	P-G	-475/0
L-J	-2172/0	L-J	-2172/0	Q-E	-477/0
U-T	0/0	U-T	0/0	Q-G	-9/0
T-S	0/1690	T-S	0/1690		
S-R	0/2867	S-R	0/2867		
R-Q	0/2867	R-Q	0/2867		
Q-P	0/3409	Q-P	0/3409		
P-O	0/2865	P-O	0/2865		
O-N	0/2865	O-N	0/2865		
N-M	0/1691	N-M	0/1691		
M-L	0/0	M-L	0/0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (1.26")
CALCULATED VERT. DEFL.(LL) = L/599 (0.18")
ALLOWABLE DEFL.(TL) = L/380 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.34")

CSI: TC=0.67/1.00 (I-J:1); BC=0.45/1.00 (P-Q:1); WB=0.62/1.00 (H-N:1); SS=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.82 (S) (INPUT = 0.90)
JSI METAL=0.52 (R) (INPUT = 1.00)

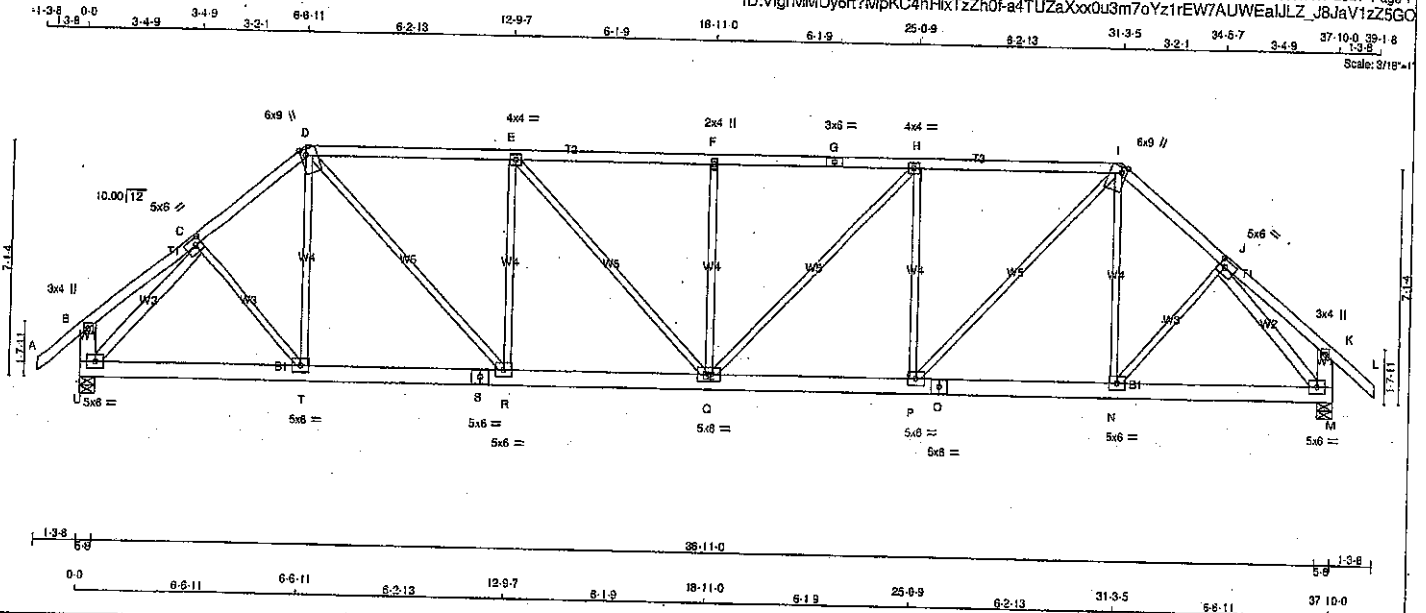


Structural component only
DWG# T-2108184

JOB NAME 417451	TRUSS NAME T4	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:VlgrMMUy6rt?MpKC4hHtZzh0F-a4TUZaXxx0u3m7oYz1rEW7AUWEaJLZ_J8JaV1zZ5GO



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - G	2x4 DRY	No.2	SPF		
G - I	2x4 DRY	No.2	SPF		
I - L	2x4 DRY	No.2	SPF		
U - B	2x6 DRY	No.2	SPF		
M - K	2x6 DRY	No.2	SPF		
U - S	2x6 DRY	No.2	SPF		
S - O	2x6 DRY	No.2	SPF		
O - M	2x6 DRY	No.2	SPF		

ALL WEBS EXCEPT	2x3 DRY	No.2	SPF
U - C	2x4 DRY	No.2	SPF
J - M	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMWV+1	MT20	5.0	6.0	2.50	2.25
D TMWV+m	MT20	6.0	9.0	Edge	1.75
E TMWV+1	MT20	4.0	4.0		
F TMW+w	MT20	2.0	4.0		
G TS+1	MT20	3.0	6.0		
H TMWV+1	MT20	4.0	4.0		
I TMWV+m	MT20	6.0	9.0	Edge	1.75
J TMWV+1	MT20	5.0	6.0	2.50	2.25
K TMV+p	MT20	3.0	4.0		
M BMWV+1	MT20	5.0	6.0		
N, P, R, T					
N BMWV+1	MT20	5.0	6.0		
O BS+1	MT20	5.0	6.0		
Q BMWV+1	MT20	5.0	6.0		
S BS+1	MT20	5.0	6.0		
U BMWV+1	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
U	2213	0	0	2213	0	0	0	5-8	5-8
M	2213	0	0	2213	0	0	0	5-8	5-8

UNFACTORED REACTIONS		1ST CASE		MAX/MIN COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
U	1562	1039 / 0	0 / 0	0 / 0	524 / 0
M	1562	1039 / 0	0 / 0	0 / 0	524 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED			WEBS		
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 (PLF)	MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00	C-T	0 / 171	0.04 (1)
B-C	0 / 20	-91.8	-91.8 0.14 (1)	10.00	T-D	0 / 79	0.03 (4)
C-D	-2237 / 0	-91.8	-91.8 0.21 (1)	4.38	D-R	0 / 1448	0.33 (1)
D-E	-2683 / 0	-91.8	-91.8 0.77 (1)	3.45	R-E	-949 / 0	0.78 (1)
E-F	-2968 / 0	-91.8	-91.8 0.81 (1)	3.25	E-Q	0 / 419	0.09 (1)
F-G	-2686 / 0	-91.8	-91.8 0.81 (1)	3.25	Q-F	-517 / 0	0.42 (1)
G-H	-2968 / 0	-91.8	-91.8 0.81 (1)	3.25	H-Q	0 / 419	0.09 (1)
H-I	-2683 / 0	-91.8	-91.8 0.77 (1)	3.45	P-H	-949 / 0	0.78 (1)
I-J	-2237 / 0	-91.8	-91.8 0.21 (1)	4.38	P-I	0 / 1448	0.33 (1)
J-K	0 / 20	-91.8	-91.8 0.14 (1)	10.00	N-I	0 / 79	0.03 (4)
K-L	0 / 41	-91.8	-91.8 0.13 (1)	10.00	N-J	0 / 171	0.04 (1)
L-B	-244 / 0	0.0	0.0 0.02 (1)	7.81	U-C	-2488 / 0	0.59 (1)
M-K	-244 / 0	0.0	0.0 0.02 (1)	7.81	J-M	-2488 / 0	0.59 (1)
U-T	0 / 1585	-18.5	-18.5 0.24 (1)	10.00			
T-S	0 / 1697	-18.5	-18.5 0.25 (1)	10.00			
S-R	0 / 1697	-18.5	-18.5 0.25 (1)	10.00			
R-Q	0 / 2683	-18.5	-18.5 0.37 (1)	10.00			
Q-P	0 / 2683	-18.5	-18.5 0.37 (1)	10.00			
P-O	0 / 1697	-18.5	-18.5 0.25 (1)	10.00			
O-N	0 / 1697	-18.5	-18.5 0.25 (1)	10.00			
N-M	0 / 1585	-18.5	-18.5 0.24 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.81/1.00 (E-F:1), BC=0.37/1.00 (Q-R:1), WB=0.78/1.00 (E-R:1), SS=0.27/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

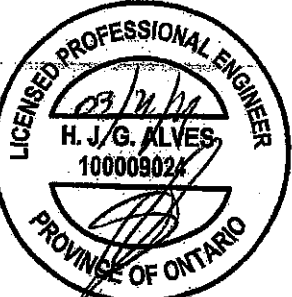
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(F3) (F4) (F5)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)

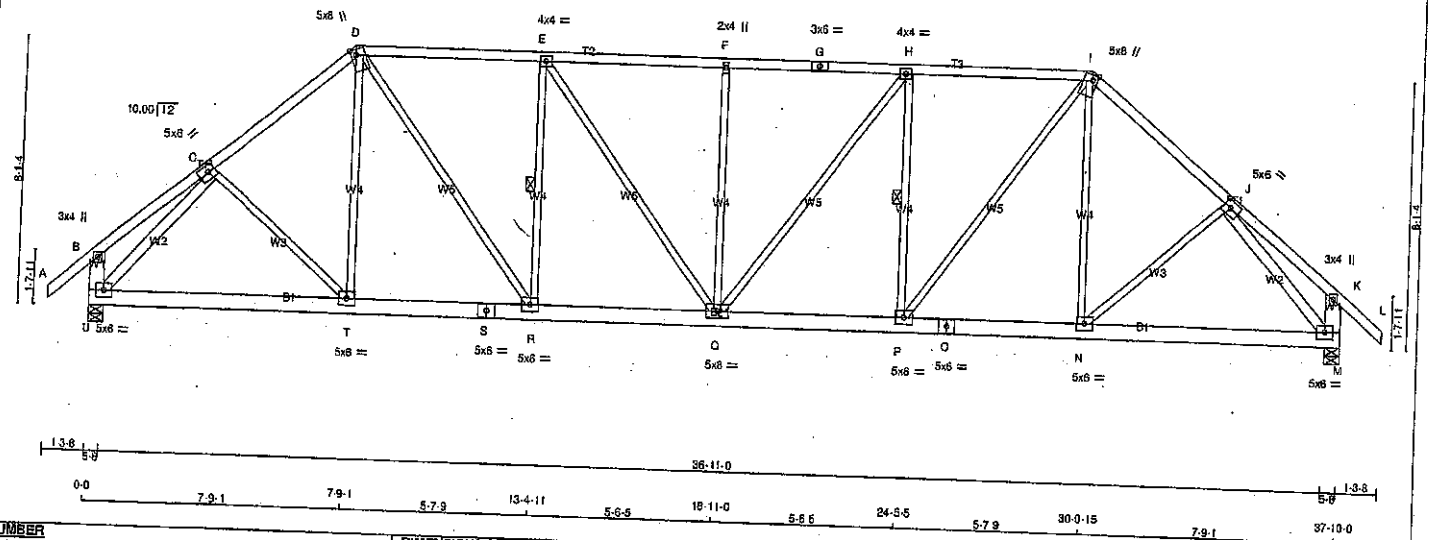
JSI METAL= 0.58 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108185

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
417451	T5	2	1	GREENPARK HOMES		
Tamarack Roof Truss, Burlington						

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 09:47:18 2021 Page 1
 ID: VgrMMUy6rt?MpKC4hHxTzZh0f-2H1snvYZIK0wOHXKMT2LteewB2pN8Y0371TzZ5GN
 Scale: 3/16"=1'



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DRY
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
U - S	2x6	DRY	No.2
S - O	2x6	DRY	No.2
O - M	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
U - C	2x4	DRY	No.2
J - M	2x4	DRY	No.2

PLATES (table is in inches)			
JT TYPE	PLATES	W	LEN Y X
B TMV+p	MT20	3.0	4.0
C TMVW-i	MT20	5.0	6.0 2.50 2.50
D TTWW+m	MT20	5.0	8.0 2.00 1.75
E TMVW-i	MT20	4.0	4.0
F TMVW-i	MT20	2.0	4.0
G TS-i	MT20	3.0	6.0
H TMVW-i	MT20	4.0	4.0
I TTWW+m	MT20	5.0	8.0 2.00 1.75
J TMVW-i	MT20	5.0	6.0 2.50 2.50
K TMV+p	MT20	3.0	4.0
M BMVW-i	MT20	5.0	6.0
N, P, R, T			
N BMVW-i	MT20	5.0	6.0
O BS-i	MT20	5.0	6.0
Q BMVW-i	MT20	5.0	8.0
S BS-i	MT20	5.0	8.0
U BMVW-i	MT20	5.0	8.0

NOTES: (1)
 1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT	REQD
JT	VERT	DOWN	HORIZ	BRG	IN-SX
U	2213	0	0	5-8	5-8
M	2213	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOOSE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
U	1562	1039 / 0	0 / 0	0 / 0	524 / 0	0 / 0
M	1562	1039 / 0	0 / 0	0 / 0	524 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.71 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-P, E-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD	MAX. FACTORED (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO	
A-B	0 / 41	-91.8	-91.8 0.13 (1)	C-T	0 / 102
B-C	0 / 31	-91.8	-91.8 0.23 (1)	T-D	0 / 112
C-D	-2217 / 0	-91.8	-91.8 0.34 (1)	N-I	0 / 112
D-E	-2376 / 0	-91.8	-91.8 0.57 (1)	N-J	0 / 102
E-F	-2575 / 0	-91.8	-91.8 0.59 (1)	U-C	-2501 / 0
F-G	-2575 / 0	-91.8	-91.8 0.59 (1)	J-M	-2501 / 0
G-H	-2575 / 0	-91.8	-91.8 0.59 (1)	P-I	0 / 1178
H-I	-2376 / 0	-91.8	-91.8 0.34 (1)	D-R	0 / 1178
I-J	-2217 / 0	-91.8	-91.8 0.23 (1)	R-E	-855 / 0
J-K	0 / 31	-91.8	-91.8 0.13 (1)	Q-H	0 / 342
K-L	0 / 41	0.0	0.0 0.02 (1)	E-Q	0 / 342
U-B	-234 / 0	0.0	0.0 0.02 (1)	Q-F	-468 / 0
M-K	-234 / 0	0.0	0.0 0.02 (1)		
U-T	0 / 1805	-18.5	-18.5 0.28 (1)		
T-S	0 / 1681	-18.5	-18.5 0.28 (1)		
S-R	0 / 1681	-18.5	-18.5 0.28 (1)		
R-Q	0 / 2376	-18.5	-18.5 0.32 (1)		
Q-P	0 / 2376	-18.5	-18.5 0.32 (1)		
P-O	0 / 1681	-18.5	-18.5 0.28 (1)		
O-N	0 / 1681	-18.5	-18.5 0.28 (1)		
N-M	0 / 1805	-18.5	-18.5 0.28 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.8 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2015, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA S086-14
 - TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.28")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.12")
 ALLOWABLE DEFL. (TL) = L/360 (1.28")
 CALCULATED VERT. DEFL. (TL) = L/999 (0.22")

CSI: TC=0.59/1.00 (F-H:1), BC=0.32/1.00 (Q-R:1), WB=0.74/1.00 (J-M:1), SSI=0.24/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 768 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

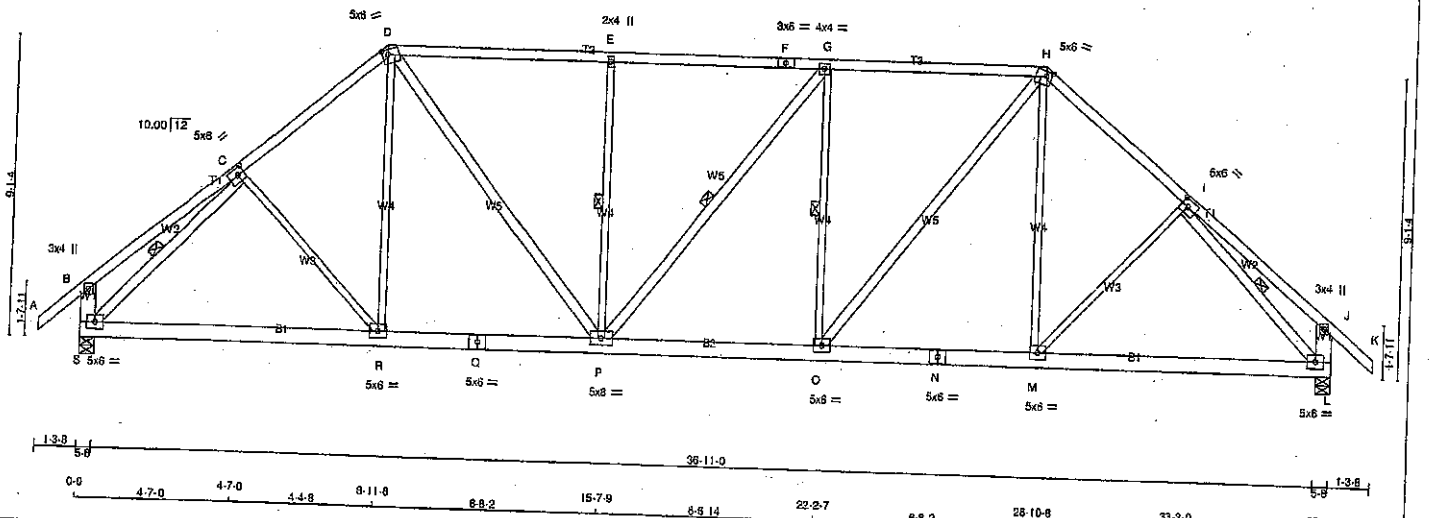
JSI GRIP = 0.89 (D) (INPUT = 0.90)
 JSI METAL = 0.56 (C) (INPUT = 1.00)



Structural component only
 DWG# T-2108186

JOB NAME 417451	TRUSS NAME T6	QUANTITY 10	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Sat Mar 20 09:47:19 2021 Page 1
 ID:VlgrMMUy6rt?MpKC4hHxTzZh0f-WTbF_FYBTe8n?Ryx5StibYFpV1HdnKCHnSohavzZ5GM
 Scale: 2/16"=1'



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
S - Q	2x6	DRY	No.2
Q - N	2x6	DRY	No.2
N - L	2x6	DRY	No.2

ALL WEBS

EXCEPT	2x3	DRY	No.2
D - P	2x4	DRY	No.2
P - G	2x4	DRY	No.2
O - H	2x4	DRY	No.2
S - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+P	MT20	3.0	4.0		
C TMVW+L	MT20	5.0	6.0	2.50	2.50
D TTWW-m	MT20	5.0	6.0	1.75	2.00
E TMVW-w	MT20	2.0	4.0		
F TS-L	MT20	3.0	6.0		
G TMVW-L	MT20	4.0	4.0		
H TTWW-m	MT20	5.0	6.0	1.75	2.00
I TMVW-L	MT20	5.0	6.0	2.50	2.50
J TMV+P	MT20	3.0	4.0		
L BMVW-L	MT20	5.0	6.0		
M, O, R					
N BMVW-L	MT20	5.0	6.0		
P BS-L	MT20	5.0	6.0		
Q BMVW-L	MT20	5.0	6.0		
R BS-L	MT20	5.0	6.0		
S BMVW-L	MT20	5.0	6.0		

NOTES

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
L	2213	0	2213	0	5-8	5-8
S	2213	0	2213	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
L	1562	1039.0	0.0	0.0	0.0	524.0	0.0
S	1562	1039.0	0.0	0.0	0.0	524.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P, G-P, G-O, C-S, I-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED		
EMB.	FORCE (LBS)	VERT. LOAD	LC1 (PLF)	MAX CSI (LC)			FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	R-D	0 / 204	0.08 (4)	
B-C	0 / 31	-91.8	-91.8	0.29 (1)	10.00	D-P	0 / 963	0.15 (1)	
C-D	-2183 / 0	-91.8	-91.8	0.42 (1)	4.22	P-E	-658 / 0	0.34 (1)	
D-E	-2239 / 0	-91.8	-91.8	0.78 (1)	3.72	E-F	-6 / 0	0.00 (1)	
E-F	-2239 / 0	-91.8	-91.8	0.77 (1)	3.70	F-G	-658 / 0	0.34 (1)	
F-G	-2239 / 0	-91.8	-91.8	0.77 (1)	3.79	G-H	0 / 971	0.16 (1)	
G-H	-2242 / 0	-91.8	-91.8	0.78 (1)	4.22	H-I	-41 / 199	0.08 (4)	
H-I	-2183 / 0	-91.8	-91.8	0.42 (1)	10.00	I-J	-41 / 43	0.03 (1)	
I-J	0 / 31	-91.8	-91.8	0.29 (1)	10.00	J-K	-2504 / 0	0.51 (1)	
J-K	0 / 41	-91.8	-91.8	0.13 (1)	7.81	K-L	-41 / 42	0.03 (1)	
K-L	-283 / 0	0.0	0.0	0.02 (1)	7.81	L-L	-2504 / 0	0.51 (1)	
L-L	-283 / 0	0.0	0.0	0.02 (1)	7.81				
S-R	0 / 1878	-18.5	-18.5	0.29 (1)	10.00				
R-Q	0 / 1853	-18.5	-18.5	0.28 (1)	10.00				
Q-P	0 / 1853	-18.5	-18.5	0.28 (1)	10.00				
P-O	0 / 2243	-18.5	-18.5	0.31 (1)	10.00				
O-N	0 / 1852	-18.5	-18.5	0.27 (1)	10.00				
N-M	0 / 1852	-18.5	-18.5	0.27 (1)	10.00				
M-L	0 / 1878	-18.5	-18.5	0.29 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2015, ABC 2019
 - PART 9 OF OSC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.28")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
 ALLOWABLE DEFL.(TL) = L/360 (1.28")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.78/1.00 (G-H-I), BC=0.31/1.00 (O-P-I), WB=0.51/1.00 (C-S-I), SS=0.29/1.00 (G-H-I)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	850 371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.88 (C) (INPUT = 0.90)
 JSI METAL=0.56 (C) (INPUT = 1.00)

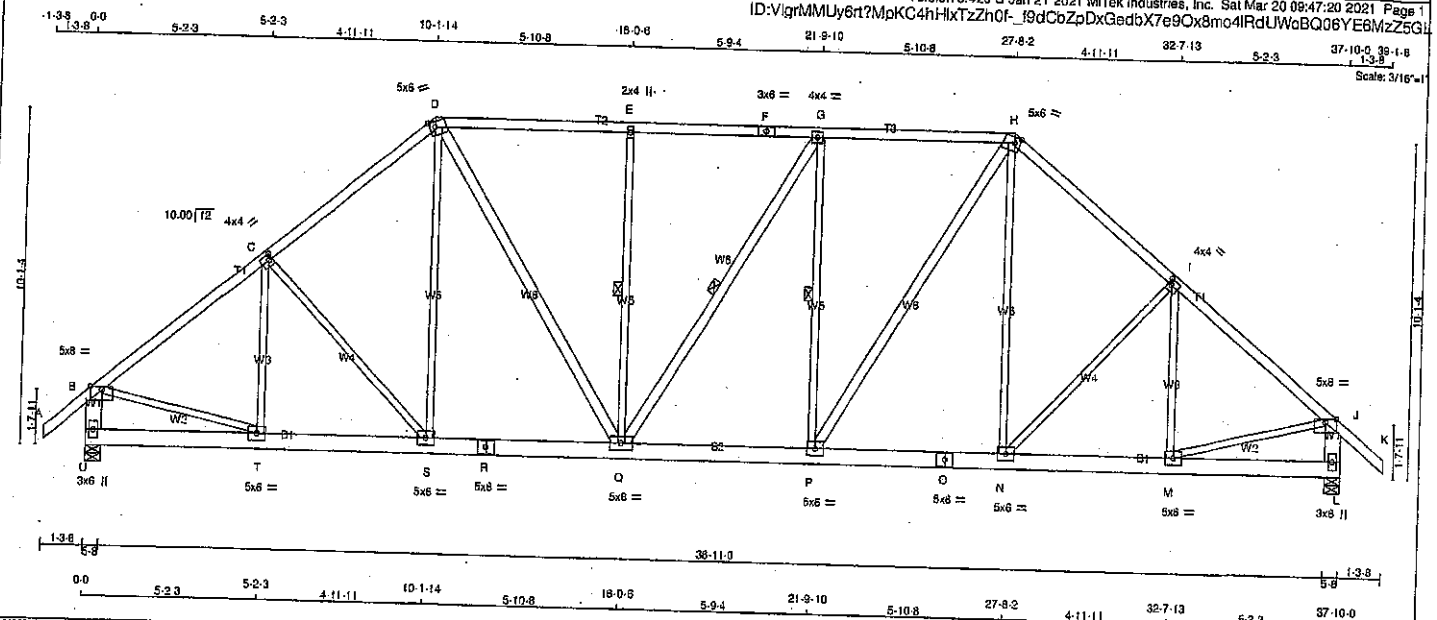


Structural component only
 DWG# T-2108187

JOB NAME 417451	TRUSS NAME T7	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x8	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2

EXCEPT	CHORDS	SIZE	LUMBER	DESCR.
D - Q	2x4	DRY	No.2	SPF
Q - G	2x4	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	Edge	
C TMVW-t	MT20	4.0	4.0	2.00	1.25
D TTWW-m	MT20	5.0	8.0	2.00	2.00
E TMVW-w	MT20	2.0	4.0		
F TS-t	MT20	3.0	8.0		
G TMVW-t	MT20	4.0	4.0		
H TTWW-m	MT20	5.0	8.0	2.00	2.00
I TMVW-t	MT20	4.0	4.0	2.00	1.25
J TMVW-p	MT20	5.0	8.0	Edge	
L BMV1+p	MT20	3.0	6.0		
M, N, P, S, T					
M BMVW-t	MT20	5.0	6.0		
Q BS-t	MT20	5.0	6.0		
Q BMVW-t	MT20	5.0	6.0		
R BS-t	MT20	5.0	6.0		
U BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	2213	0	0	0
U VERT	2213	0	0	0
L VERT	2213	0	0	0
JT HORZ	0	2213	0	0
U HORZ	0	2213	0	0
L HORZ	0	2213	0	0

UNFACTORED REACTIONS

1ST LOASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT COMBINED	1562	1039	0	0	0
U COMBINED	1562	1039	0	0	0
L COMBINED	1562	1039	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, G-Q, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORZ. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LBS)
FR-TO						FR-TO			
A-B	0.41	-91.8	-91.8	0.13 (1)	10.00	T-C	-327.0	0.17 (1)	
B-C	-2221.0	-91.8	-91.8	0.40 (1)	4.22	C-S	-189.0	0.22 (1)	
C-D	-2123.0	-91.8	-91.8	0.39 (1)	4.32	D-Q	0.285	0.08 (1)	
D-E	-2013.0	-91.8	-91.8	0.42 (1)	4.34	Q-D	0.793	0.13 (1)	
E-F	-2013.0	-91.8	-91.8	0.39 (1)	4.38	Q-E	-577.0	0.39 (1)	
F-G	-2013.0	-91.8	-91.8	0.39 (1)	4.38	Q-G	-8.0	0.01 (1)	
G-H	-2017.0	-91.8	-91.8	0.42 (1)	4.33	P-G	574.0	0.38 (1)	
H-I	-2122.0	-91.8	-91.8	0.38 (1)	4.32	P-H	0.802	0.13 (1)	
I-J	-2222.0	-91.8	-91.8	0.40 (1)	4.22	N-H	0.257	0.05 (1)	
J-K	0.41	-91.8	-91.8	0.13 (1)	10.00	N-I	-199.0	0.22 (1)	
U-B	-2163.0	0.0	0.0	0.15 (1)	6.93	M-I	-325.0	0.17 (1)	
L-J	-2164.0	0.0	0.0	0.15 (1)	6.93	B-T	0.1781	0.40 (1)	
						M-J	0.1781	0.40 (1)	
U-T	0.0	-18.5	-18.5	0.05 (4)	10.00				
T-S	0.1733	-18.5	-18.5	0.24 (1)	10.00				
S-R	0.1802	-18.5	-18.5	0.22 (1)	10.00				
R-Q	0.1802	-18.5	-18.5	0.22 (1)	10.00				
Q-P	0.2017	-18.5	-18.5	0.27 (1)	10.00				
P-O	0.1802	-18.5	-18.5	0.22 (1)	10.00				
O-N	0.1802	-18.5	-18.5	0.22 (1)	10.00				
N-M	0.1734	-18.5	-18.5	0.23 (1)	10.00				
M-L	0.0	-18.5	-18.5	0.05 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CS: TC=0.42/1.00 (G-H-I), BC=0.27/1.00 (P-Q-I), WB=0.40/1.00 (J-M-I), SS=0.25/1.00 (G-H-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

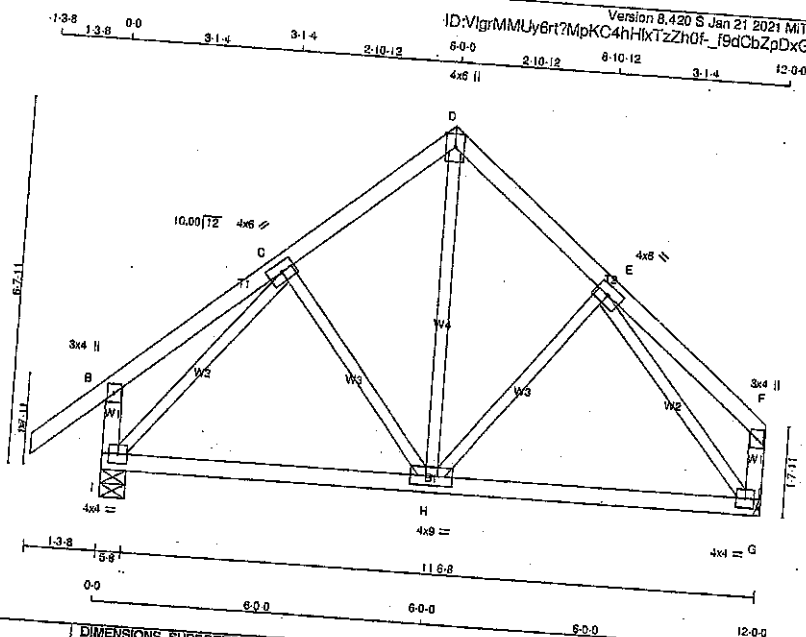
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.88 (H) (INPUT = 0.90)
JSI METAL=0.40 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108188

JOB NAME 417451 Tamarack Roof Truss, Burlington	TRUSS NAME T8	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2	
D - F	2x4	DRY	No.2	
I - B	2x4	DRY	No.2	
G - F	2x4	DRY	No.2	
I - G	2x4	DRY	No.2	
ALL WEBS	2x3	DRY	No.2	
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-1	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0		
E	TMVW-1	MT20	4.0	6.0		Edge
F	TMV+p	MT20	4.0	4.0		
G	BMVW-1	MT20	3.0	4.0		
H	BMVW-1	MT20	4.0	4.0		
I	BMVW-1	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
I	VERT	DOWN	IN-SX	IN-SX
I	789	789	0	5-8
G	662	662	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	565	378 '0	0 '0				
G	468	307 '0	0 '0	0 '0	0 '0	177 '0	0 '0
				0 '0	0 '0	161 0	0 '0

BEARING MATERIAL TO BE SPECIFIED

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	W E B S	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MAX. MEMB.	FORCE
FR-TO	(LBS)	FROM TO	LENGTH	(LBS)
A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00
B-C	0 / 20	-91.8	-91.8 0.13 (1)	10.00
C-D	-445 / 0	-91.8	-91.8 0.10 (1)	6.25
D-E	-445 / 0	-91.8	-91.8 0.10 (1)	6.25
E-F	0 / 20	-91.8	-91.8 0.13 (1)	10.00
F-B	-232 / 0	0.0	0.0 0.02 (1)	7.81
G-F	-105 / 0	0.0	0.0 0.01 (1)	7.81
I-H	0 / 403	-18.5	-18.5 0.22 (4)	10.00
H-G	0 / 403	-18.5	-18.5 0.22 (4)	10.00

TOTAL WEIGHT = 56 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.40")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.40")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CSI: TC=0.13/1.00 (B-C:1), BC=0.22/1.00 (H-I:4), WB=0.24/1.00 (C-I:1), SS=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

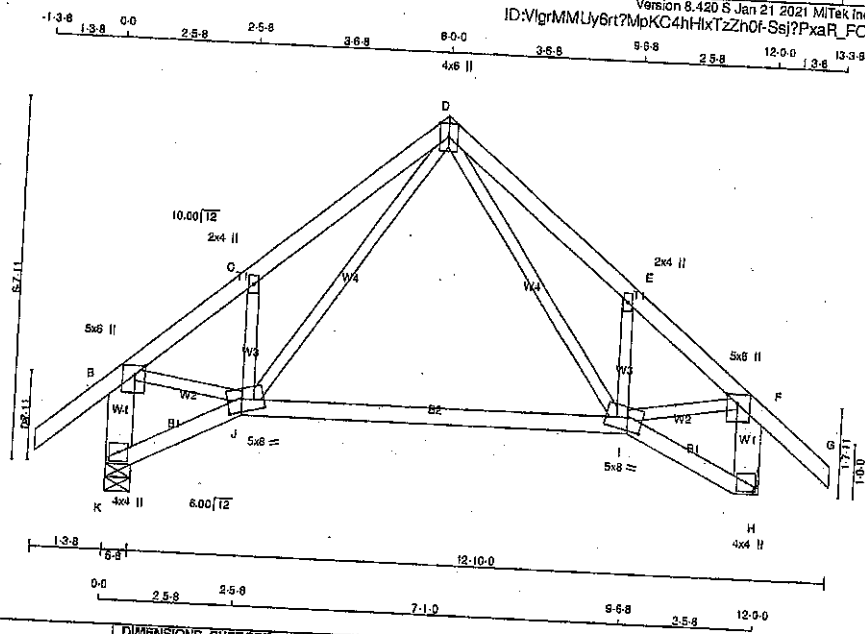
JSI GRIP = 0.49 (I) (INPUT = 0.80)
JSI METAL = 0.15 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108189

JOB NAME 417451 Tamarack Roof Truss, Burlington	TRUSS NAME T8S	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Scale = 1/32"

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	NO.2
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
K - B	2x6	DRY	No.2
H - F	2x6	DRY	No.2
K - J	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMW+p	MT20	5.0	6.0	2.00	2.25
C TMW+w	MT20	2.0	4.0		
D TTW+p	MT20	4.0	6.0	Edge	
E TMW+w	MT20	2.0	4.0		
F TMW+p	MT20	5.0	6.0	2.00	2.25
H BMV1+p	MT20	4.0	4.0		
I BBWW+m	MT20	5.0	8.0	2.75	3.25
J BBWW+m	MT20	5.0	8.0	2.75	3.25
K BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
VERT	DOWN	HORIZ	UPLIFT
JT	789	0	0
K	789	0	0
H	789	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS							
	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT							
K	555	378 '0	0 '0	0 '0	0 '0	177 '0	0 '0
H	555	378 '0	0 '0	0 '0	0 '0	177 '0	0 '0

BEARING MATERIAL TO BE SPREAD

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.
FR-TO				FR-TO			
A-B	0.41	-91.8	0.13 (1)	10.00	D-I	0.429	0.10 (1)
B-C	-744.0	-91.8	0.10 (1)	6.25	I-E	-339.0	0.06 (1)
C-D	-789.0	-91.8	0.14 (1)	6.25	J-D	0.429	0.10 (1)
D-E	-789.0	-91.8	0.14 (1)	6.25	J-C	-339.0	0.06 (1)
E-F	-744.0	-91.8	0.10 (1)	10.00	B-J	0.594	0.13 (1)
F-G	0.41	-91.8	0.13 (1)	10.00	I-F	0.594	0.13 (1)
K-B	-766.0	0.0	0.06 (1)	7.81			
H-F	-766.0	0.0	0.06 (1)	7.81			
K-J	0.0	-18.5	-18.5	0.03 (4)	10.00		
J-I	0.353	-18.5	-18.5	0.30 (4)	10.00		
I-H	0.0	-18.5	-18.5	0.03 (4)	10.00		

TOTAL WEIGHT = 3 X 59 = 176 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 91.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/960 (0.40")
CALCULATED VERT. DEFL.(LL) = L/989 (0.01")
ALLOWABLE DEFL.(TL) = L/960 (0.40")
CALCULATED VERT. DEFL.(TL) = L/959 (0.14")

CSI: TC=0.14/1.00 (C-D:1), BC=0.30/1.00 (I-J:4), WB=0.13/1.00 (B-J:1), SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 765 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.49 (B) (INPUT = 0.90)
JSI METAL = 0.18 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108190

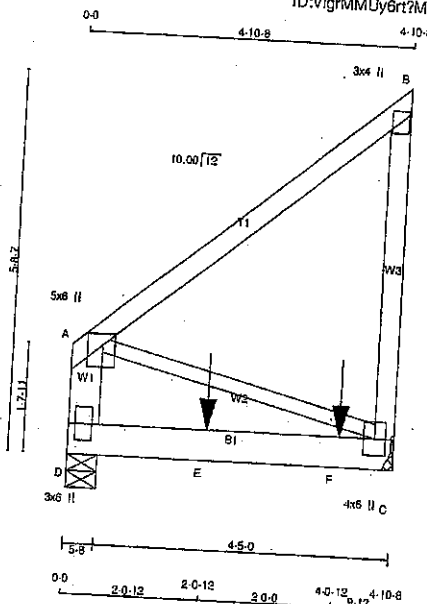
Structural component only
DWG# T-2108191

Structural component only
DWG# T-2108192

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
417451	T11	2	2	GREENPARK HOMES		
Tamarack Roof Truss, Burlington						

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Scale = 1:31.5



LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
C - B	2x4	DRY	No.2
D - A	2x6	DRY	No.2
D - C	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - B 1	12	TOP
B - C 1	12	TOP
D - A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D - C 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW+p	MT20	5.0	6.0	2.00	2.25
B TMV+p	MT20	3.0	4.0		
C BMVW1+p	MT20	4.0	6.0		
D BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
C	876	0	0	5-8
D	628	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 2-0.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED				
C	618	417.0	0.0	0.0	201.0
D	443	297.0	0.0	0.0	145.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX. (LC)	MAX. FACTORED UNBRACED LENGTH FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX. (LC)
FR-TO									
A-B	0.0					A-C	0.0		
C-B	-224.0	-91.8	0.0	0.06 (1)	7.81				
D-A	-224.0	0.0	0.0	0.01 (1)	7.81				
D-E	0.0	-18.5	-18.5	0.29 (1)	10.00				
E-F	0.0	-18.5	-18.5	0.29 (1)	10.00				
F-C	0.0	-18.5	-18.5	0.29 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-0-12	-339	-339		BACK	VERT	TOTAL		C1
F	4-0-12	-341	-341		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

- C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 27 = 110 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.20/1.00 (A-B:1), BC=0.29/1.00 (C-D:1), WB=0.00/1.00 (A-C:1), SS=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	789	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

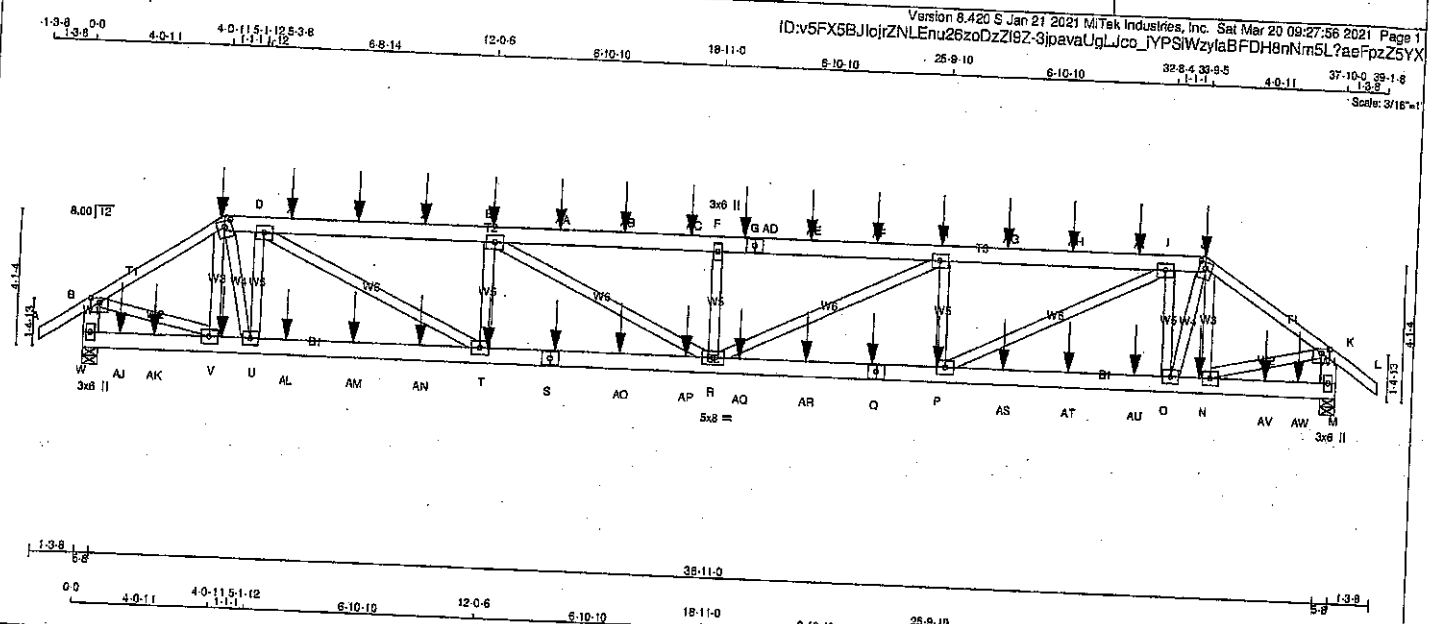
JSI GRIP= 0.05 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108193

JOB NAME 417449	TRUSS NAME T15	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 09:27:56 2021 Page 1
 ID:v5FX5Bj0rZNLenu26zoDz19Z-3jpavaUgJco_YPSIWzylaBFDH8nNmSL?aeFpzZ5YX



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - G	2x6	DRY	No.2	SPF	
G - J	2x6	DRY	No.2	SPF	
J - L	2x4	DRY	No.2	SPF	
W - B	2x6	DRY	No.2	SPF	
W - K	2x6	DRY	No.2	SPF	
W - S	2x6	DRY	No.2	SPF	
S - Q	2x6	DRY	No.2	SPF	
Q - M	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.
 DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE(61.0)
J - L	12	SIDE(61.0)
C - G	12	SIDE(183.1)
G - J	12	SIDE(183.1)
W - B	12	TOP
M - K	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
W - S	12	SIDE(183.1)
S - Q	12	SIDE(0.0)
Q - M	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.
 GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.
 TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
JT	VERT	JT	DOWN	IN-SX	IN-SX
W	3680	W	3680	5-8	5-8
M	3680	M	3680	5-8	5-8

UNFACTORED REACTIONS		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
JT	1ST LOASE	JT	COMBINED	WIND	DEAD	SOIL
W	2603	W	2603	0/0	897/0	0/0
M	2603	M	2603	0/0	897/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M
 BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
 UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0/35	A-B	0/35
B-C	-4340/0	B-C	-4340/0
C-D	-4556/0	C-D	-4556/0
D-X	-8084/0	D-X	-8084/0
X-Y	-8084/0	X-Y	-8084/0
Y-Z	-8084/0	Y-Z	-8084/0
Z-E	-8084/0	Z-E	-8084/0
E-AA	-9243/0	E-AA	-9243/0
AA-AB	-9243/0	AA-AB	-9243/0
AB-AC	-9243/0	AB-AC	-9243/0
AC-F	-9243/0	AC-F	-9243/0
F-AD	-9243/0	F-AD	-9243/0
AD-G	-9243/0	AD-G	-9243/0
G-AE	-9243/0	G-AE	-9243/0
AE-AF	-9243/0	AE-AF	-9243/0
AF-H	-9243/0	AF-H	-9243/0
H-AG	-8084/0	H-AG	-8084/0
AG-AH	-8084/0	AG-AH	-8084/0
AH-AI	-8084/0	AH-AI	-8084/0
AI-I	-8084/0	AI-I	-8084/0
I-J	-4556/0	I-J	-4556/0
J-K	-4340/0	J-K	-4340/0
K-L	0/35	K-L	0/35
W-B	-3626/0	W-B	-3626/0
M-K	-3626/0	M-K	-3626/0

DESIGN CRITERIA
 *** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
 ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC0 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBC0 2015, ABC 2019
 - PART 9 OF NBC0 2012 (2019 AMENDMENT)
 - CSA 088-14
 - TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
 CALCULATED VERT. DEFL.(LL) = L/889 (0.25")
 ALLOWABLE DEFL.(TL) = L/360 (1.26")
 CALCULATED VERT. DEFL.(TL) = L/984 (0.48")

CSI: TC=0.36/1.00 (E-F:1), BC=0.59/1.00 (R-T:1), WB=0.35/1.00 (P-T:1), SS=0.17/1.00 (F-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 786 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (N) (INPUT = 0.90)
 JSI METAL = 0.75 (S) (INPUT = 1.00)



Structural component only
 DWG# T-2108163

JOB NAME 417449	TRUSS NAME T15	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 09:27:57 2021 Page 2 ID:v5FX5Bj0irZNLEnu26zoDzZ19Z-YwNv6wUI8ckdbs6c0P1CVV7L?dcNWq0FaUBnGzZ5YW		

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	5.0	6.0	2.25	2.50
D, E, H, I						
D	TMWW-t	MT20	5.0	6.0		
F	TMW-w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
J	TTWW+m	MT20	5.0	6.0	2.25	2.50
K	TMVW-p	MT20	5.0	6.0	1.50	3.00
M	BMV1+p	MT20	3.0	6.0		
N, O, P, T, U, V						
N	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	6.0		
S	BS-t	MT20	5.0	6.0		
W	BMV1+p	MT20	3.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LOCI	MAX. CSI (LC)	UNBRAC LENGTH	MEMB. MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO					
P-AS	0 / 4548	-18.5	-18.5	0.35 (1)	10.00		
AS-AT	0 / 4548	-18.5	-18.5	0.35 (1)	10.00		
AT-AU	0 / 4548	-18.5	-18.5	0.35 (1)	10.00		
AU-O	0 / 4548	-18.5	-18.5	0.35 (1)	10.00		
O-N	0 / 3855	-18.5	-18.5	0.28 (1)	10.00		
N-AV	0 / 0	-18.5	-18.5	0.03 (4)	10.00		
AV-AW	0 / 0	-18.5	-18.5	0.03 (4)	10.00		
AW-M	0 / 0	-18.5	-18.5	0.03 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LOCI	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-41	-41	---	FRONT	VERT	DEAD	---	C1
C	4-0-11	-82	-82	---	FRONT	VERT	TOTAL	---	C1
E	12-1-7	-175	-175	---	FRONT	VERT	SNOW	---	C1
H	25-8-9	-76	-76	---	FRONT	VERT	TOTAL	---	C1
J	33-9-6	-41	-41	---	FRONT	VERT	TOTAL	---	C1
J	33-9-6	-82	-82	---	FRONT	VERT	DEAD	---	C1
J	33-9-6	-175	-175	---	FRONT	VERT	TOTAL	---	C1
N	33-9-6	-21	-21	---	FRONT	VERT	SNOW	---	C1
P	25-8-9	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Q	23-8-9	-21	-21	---	FRONT	VERT	TOTAL	---	C1
S	14-1-7	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	12-1-7	-21	-21	---	FRONT	VERT	TOTAL	---	C1
V	4-1-6	-21	-21	---	FRONT	VERT	TOTAL	---	C1
X	6-1-7	-76	-76	---	FRONT	VERT	TOTAL	---	C1
Y	8-1-7	-76	-76	---	FRONT	VERT	TOTAL	---	C1
Z	10-1-7	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AA	14-1-7	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AB	16-1-7	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AC	18-1-7	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AD	19-8-9	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AE	21-8-9	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AF	23-8-9	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AG	27-8-9	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AH	29-8-9	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AI	31-8-9	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AJ	1-1-7	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AK	2-1-7	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AL	6-1-7	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AM	8-1-7	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AN	10-1-7	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AO	16-1-7	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AP	18-1-7	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AQ	19-8-9	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AR	21-8-9	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AS	27-8-9	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AT	29-8-9	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AU	31-8-9	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AV	35-8-9	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AW	38-8-9	-21	-21	---	FRONT	VERT	TOTAL	---	C1

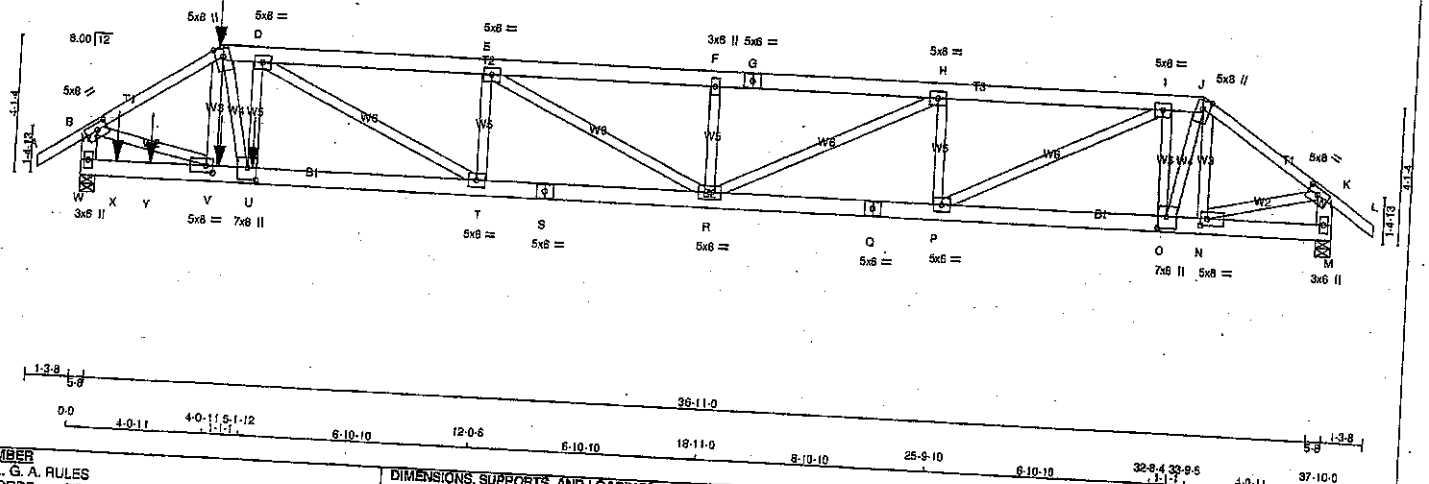
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2108163 2/11

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ID: v5FX5BjqrZNLenu26zoDzZi9Z-06xLKGVxtwsWD0hca7YR2lgVK1ywFDjOpJ3kkizZ5YV
Scale: 3/16"=1"



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
A - C	2x4	DRY	No.2
C - G	2x6	DRY	-No.2
G - J	2x6	DRY	No.2
J - L	2x4	DRY	No.2
W - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
W - S	2x6	DRY	No.2
S - Q	2x6	DRY	No.2
Q - M	2x6	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			

DRY; SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	1	12
J-L	1	12
C-G	2	12
G-J	2	12
N-B	2	12
M-K	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
V-S	2	12
C-Q	2	12
N-M	2	12
WEBBS : (0.122"x3") SPIRAL NAILS		
W	1	6
U	1	2
O	1	2

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
DESCR.		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
SFF	JT	5185	0	5185	0	0	5-8	5-8	
SFF	M	2816	0	2816	0	0	5-8	5-8	

UNFACTORED REACTIONS

1ST LOASE		MAX/MIN COMPONENT REACTIONS					
COMBINED		SNOW	LIVE	PERM/LIVE	WIND	DEAD	SOIL
JT	4365	2913 0	0 0	0 0	0 0	1452 0	0 0
W	1988	1325 0	0 0	0 0	0 0	462 0	0 0
M							

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W. M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.29 FT
MAX. UNBRACED BOTTOM CHORD LENGTH

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS			FACTORED			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	V-C	-1290 / 0	
B-C	-7628 / 0	-91.8	-91.8	0.40 (1)	3.29	N-J	-636 / 0	
C-D	-7912 / 0	-91.8	-91.8	0.11 (1)	4.27	B-V	0 / 6568	
D-E	-9064 / 0	-91.8	-91.8	0.32 (1)	3.85	N-K	0 / 2795	
E-F	-8600 / 0	-91.8	-91.8	0.26 (1)	3.98	U-D	-1137 / 0	
F-G	-8800 / 0	-91.8	-91.8	0.27 (1)	3.99	C-U	0 / 5244	
G-H	-8600 / 0	-91.8	-91.8	0.22 (1)	3.99	O-I	-2377 / 0	
H-I	-8769 / 0	-91.8	-91.8	0.27 (1)	4.46	C-J	0 / 2573	
I-J	-3472 / 0	-91.8	-91.8	0.10 (1)	5.91	P-I	0 / 3736	
J-K	-3245 / 0	-91.8	-91.8	0.19 (1)	4.09	D-T	0 / 1307	
K-L	0 / 35	-91.8	-91.8	0.07 (1)	10.00	P-H	-1628 / 0	
L-M	-6135 / 0	0.0	0.0	0.22 (1)	5.98	T-E	-445 / 0	
M-A	-2790 / 0	0.0	0.0	0.10 (1)	7.81	R-F	0 / 2072	
						E-R	-525 / 0	
						R-F	-508 / 0	
J-X	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
Y-Y	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
Z-Z	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
U-U	0 / 6416	-18.5	-18.5	0.49 (1)	10.00			
T-T	0 / 7809	-18.5	-18.5	0.60 (1)	10.00			
S-S	0 / 9064	-18.5	-18.5	0.64 (1)	10.00			
R-R	0 / 9064	-18.5	-18.5	0.64 (1)	10.00			
Q-Q	0 / 6789	-18.5	-18.5	0.49 (1)	10.00			
P-P	0 / 6739	-18.5	-18.5	0.49 (1)	10.00			
O-O	0 / 3467	-18.5	-18.5	0.26 (1)	10.00			
N-N	0 / 2734	-18.5	-18.5	0.19 (1)	10.00			
M-M	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

C	LOC.	LOC.	MAX.	MAX.	FACE	DIRL	TYPE	HEEL	CONN.
0	4-0-11	-41	-41	---	FRONT	VERT	DEAD	---	C1
0	4-0-11	-82	-82	---	FRONT	VERT	TOTAL	---	C1
U	4-0-11	-175	-175	---	BACK	VERT	TOTAL	---	C1
C	5-1-12	-2889	-2889	---	BACK	VERT	SNOW	---	C1
V	4-1-7	-21	-21	---	BACK	VERT	TOTAL	---	C1
X	1-1-7	-21	-21	---	BACK	VERT	TOTAL	---	C1
Y	2-1-7	-21	-21	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL
LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
PART 9 OF BCBC 2018, ABC 2019
PART 9 OF OBC 2012 (2019 AMENDMENT)
CSA 086-14
TPIC 2014

5 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F.
(RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED
FOR LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.24")
 ALLOWABLE DEFL.(TL) = L/360 (1.26")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.44")

$$I: TC=0.40/1.00 (B-C:1), BC=0.64/1.00 (R-T:1), B=0.58/1.00 (B-V:1), SS=0.12/1.00 (D-E:1)$$

L LUMBER=1.00 NAIL=1.00 LS BEND=1.00
IMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

RESOLVE HEELS OFF

SS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE SS MANUFACTURING PLANT.

VALUES

GRIP(DRY)		SHEAR		SECTION	
(PSI)		(PLI)		(PLI)	
MAX	MIN	MAX	MIN	MAX	MIN
650	371	1747	788	1987	1873

E PLACEMENT TOL. = 0.250 inches

ROTATION TOL. = 5.0 Deg.

RIP = 0.89 (B) (INPUT = 0.90)
ETAL = 0.80 (S) (INPUT = 1.00)



Structural component only
DWG# T-2108164

JOB NAME 417449 Tamarack Roof Truss, Burlington	TRUSS NAME T15Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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ID:v5FX5BJ0itZNLEnu26zoDzZ19Z-06xL KGVxIwsWD0haZYR21qVK1vwFDiOpJ3kKizZ5YV

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	3.75
C	TTWW+m	MT20	5.0	8.0	Edge	
D, E, H, I						
D	TMWW-t	MT20	5.0	6.0		
F	TMWW-w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
J	TTWW+m	MT20	5.0	8.0	Edge	
K	TMVW-t	MT20	5.0	8.0	2.25	3.75
M	BMV1+p	MT20	3.0	6.0		
N	BMVW-t	MT20	5.0	8.0	2.50	2.25
O	BMVW-t	MT20	7.0	8.0	4.25	3.25
P	BMVW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
R	BMVW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
T	BMVW-t	MT20	5.0	6.0		
U	BMVW-t	MT20	7.0	8.0	4.25	3.25
V	BMVW-t	MT20	5.0	8.0	2.50	2.25
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

- 1) Lateral braces to be a minimum of 2X4 SPF #2.

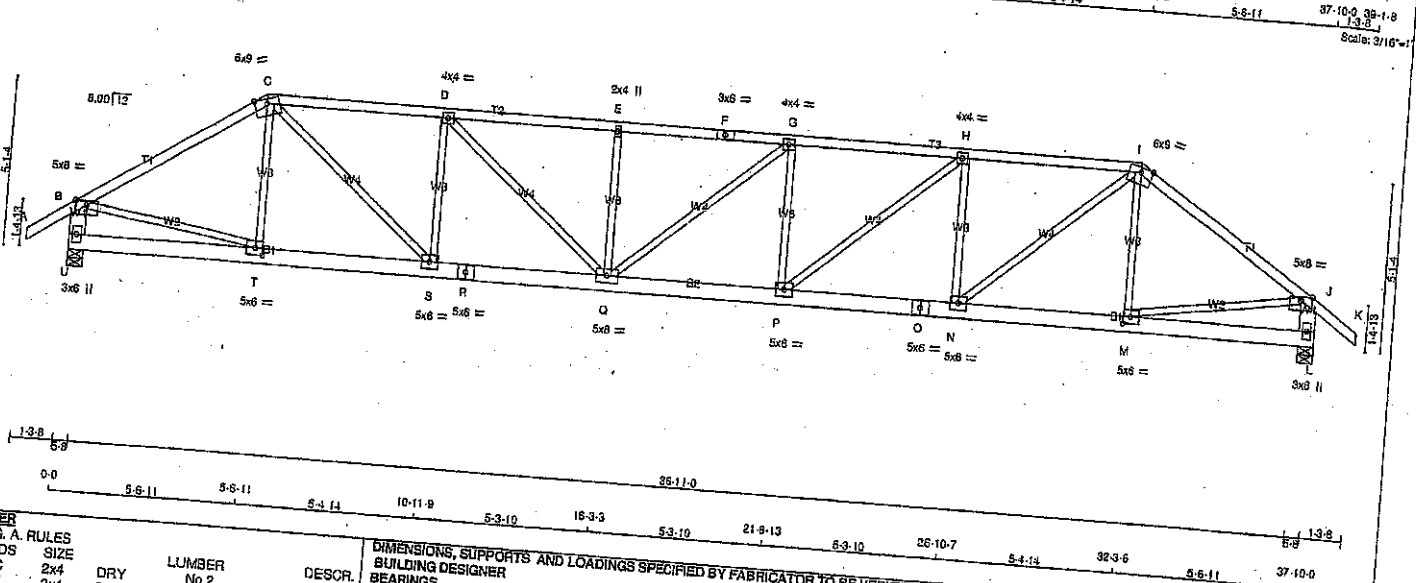
CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2108164 *ml*

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ID:v5FX5BUjqrZNLenu26zoDzZi9Z-k?R95eSJRdYCKCSFZbsoVnLkAFNmxmVnLxcGtzZ6H2



LUMBER			N. L. G. A. RULES	
CHORDS	SIZE			LUMBER
A - C	2x4	DRY	No. 2	
C - F	2x4	DRY	No. 2	
F - I	2x4	DRY	No. 2	
I - K	2x4	DRY	No. 2	
U - B	2x6	DRY	No. 2	
L - J	2x6	DRY	No. 2	
U - R	2x6	DRY	No. 2	
R - O	2x6	DRY	No. 2	
O - L	2x6	DRY	No. 2	
ALL WEBS EXCEPT	2x3	DRY	No. 2	

DRY; SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	Edge	
C	TTWV-m	MT20	5.0	9.0	Edge	
D, G, H			6.0	9.0	Edge	
D	TMWV-w	MT20	4.0	4.0		
E	TMW-w	MT20	2.9	4.0		
F	TS-t	MT20	3.0	6.0		
J	TTWV-m	MT20	6.0	9.0	Edge	
J	TMWV-p	MT20	5.0	8.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
M	BMWV-t	MT20	5.0	6.0	2.50	2.75
F, S						
S	BMWV-t	MT20	5.0	6.0		
BS-t		MT20	5.0	6.0		
S	BMWVW-t	MT20	5.0	8.0		
BS-t		MT20	5.0	6.0		
S	BMWV-w	MT20	5.0	8.0	2.50	2.75
S	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

6-3-10

26-10

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESIGNER

BEARINGS

FACTORED		MAXIMUM FACTORED		INPUT	REQHD		
GROSS REACTION		GROSS REACTION					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	2212	0	2212	0	0	5-8	5-8
L	2212	0	2212	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE

MAX. MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1562	1038	0	0	0	523	0
L	1562	1038	0	0	0	523	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

CENTER JOINTS MUST BE LATERALLY RESTRAINED

CHORDS				FACTORED				WEBS							
MEMB.		MAX. FACTORED FORCE (LBS)		VERT. LOAD (PLF)		MAX. OSI (LC)		MAX. UNBRAC. LENGTH		MEMB.		MAX. FACTORED FORCE (LBS)		MAX. OSI (LC)	
FR-TO				FROM	TO					FR-TO					
A-B	0 / 35			-91.8	-91.8	0.12 (1)	10.00	T-C	-256 / 0						
B-C	-2541 / 0			-91.8	-91.8	0.74 (1)	3.59	M-1	-254 / 2					0.09 (1)	
C-D	-3489 / 0			-91.8	-91.8	0.56 (1)	3.22	B-T			0 / 2145			0.48 (1)	
D-E	-4120 / 0			-91.8	-91.8	0.75 (1)	2.89	M-2			0 / 2145			0.48 (1)	
E-F	-4120 / 0			-91.8	-91.8	0.62 (1)	3.05	N-S			0 / 1832			0.41 (1)	
F-G	-4120 / 0			-91.8	-91.8	0.82 (1)	3.05	C-1			0 / 1835			0.41 (1)	
G-H	-4128 / 0			-91.8	-91.8	0.75 (1)	2.89	N-H	-1115 / 0					0.41 (1)	
H-I	-3487 / 0			-91.8	-91.8	0.85 (1)	3.22	S-D	-1108 / 0					0.40 (1)	
I-J	-2542 / 0			-91.8	-91.8	0.74 (1)	3.59	P-H			0 / 858			0.40 (1)	
J-K	0 / 35			0.0	0.0	0.14 (1)	10.00	D-Q			0 / 845			0.19 (1)	
U-B	-2166 / 0			0.0	0.0	0.14 (1)	8.93	P-G			-467 / 0			0.19 (1)	
L-J	-2167 / 0			0.0	0.0	0.14 (1)	6.93	Q-E			-470 / 0			0.17 (1)	
U-T	0 / 0			-18.5	-18.5	0.08 (4)	10.00	Q-G			-11 / 0			0.01 (1)	
T-S	0 / 2106			-18.5	-18.5	0.28 (1)	10.00								
S-R	0 / 3489			-18.5	-18.5	0.47 (1)	10.00								
R-Q	0 / 3489			-18.5	-18.5	0.47 (1)	10.00								
Q-P	0 / 4128			-18.5	-18.5	0.55 (1)	10.00								
P-O	0 / 3437			-18.5	-18.5	0.48 (1)	10.00								
O-N	0 / 3437			-18.5	-18.5	0.46 (1)	10.00								
N-M	0 / 2107			-18.5	-18.5	0.28 (1)	10.00								
M-L	0 / 0			-18.5	-18.5	0.08 (4)	10.00								

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF ECBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.26")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.25")
ALLOWABLE DEFL.(TL) = $L/360$ (1.26")
CALCULATED VERT. DEFL.(TL) = $L/1020$ (0.25")

CSI: TC=0.75/1.00 (D-E:1), BC=0.55/1.00 (P-Q:1),
WB=0.48/1.00 (J-M:1), SS=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

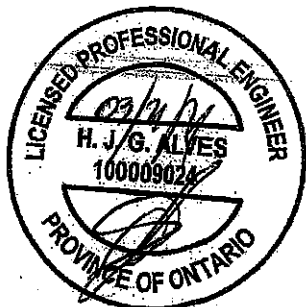
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	GRIP (DRY)		SHEAR		SECTION
	(FSI)	(PLI)	(PLI)	(PLI)	(PLI)
MT20	MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN
	650 371	1747 788	1987	1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (F) (INPUT = 0.90)
JSI METAL = 0.63 (R) (INPUT = 1.00)

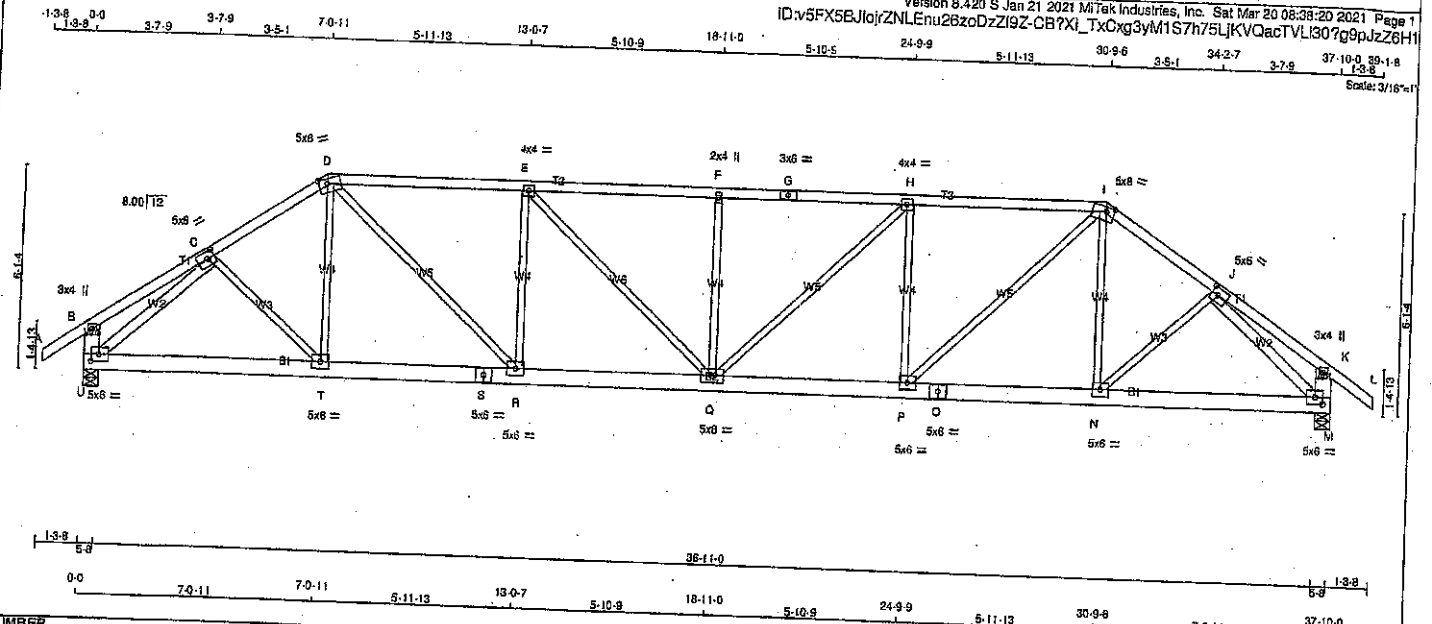


Structural component only
DWG# T-2108112

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417441	T17	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:V5FX5EJ0jrZNLenu2BzoDzZi9Z-CB?Xl_TxCg3yM1S7h75LJKVQacTVL130?g9pJzZ6H1



CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	PLATES	W	LEN	Y	X
JT	TMV+p	MT20	3.0	4.0	
B	TMVW-t	MT20	6.0	6.0	2.50
C	TTWW-m	MT20	5.0	8.0	1.75
D	TTWW-t	MT20	4.0	4.0	
E	TMVW-t	MT20	2.0	4.0	
F	TMVW-t	MT20	2.0	4.0	
G	TS-t	MT20	3.0	6.0	
H	TMVW-t	MT20	4.0	4.0	
I	TTWW-m	MT20	5.0	8.0	1.75
J	TMVW-t	MT20	5.0	6.0	2.50
K	TMV+p	MT20	3.0	4.0	
M	BMVW-t	MT20	5.0	6.0	2.50
N, P, R, T					
N	BMVW-t	MT20	5.0	6.0	
O	BS-t	MT20	5.0	6.0	
Q	BMVW-t	MT20	5.0	8.0	
S	BS-t	MT20	5.0	6.0	
U	BMVW-t	MT20	5.0	6.0	2.75

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
U	VERT	DOWN	0	5-8
M	HORZ	HORZ	0	5-8

UNFACTORED REACTIONS

1ST LOOSE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1562	1038 / 0	0 / 0	0 / 0	0 / 0	523 / 0	0 / 0
M	1562	1038 / 0	0 / 0	0 / 0	0 / 0	523 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX	MAX	WEBS	MAX. FACTORED	MAX
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CSF (LC)	UNBRAC	MEMB.	FORCE (LBS)	CSF (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 35	-91.8 -91.8	0.12 (1)	10.00	C-T	0 / 143	0.03 (4)
B-C	0 / 19	-91.8 -91.8	0.16 (1)	10.00	T-D	0 / 92	0.03 (4)
C-D	-2557 / 0	-91.8 -91.8	0.26 (1)	4.10	N-I	0 / 92	0.03 (4)
D-E	-3172 / 0	-91.8 -91.8	0.77 (1)	3.22	N-J	0 / 143	0.03 (4)
E-F	-3478 / 0	-91.8 -91.8	0.82 (1)	3.04	U-C	-2762 / 0	0.69 (1)
F-G	-3478 / 0	-91.8 -91.8	0.82 (1)	3.04	J-M	-2762 / 0	0.69 (1)
G-H	-3478 / 0	-91.8 -91.8	0.82 (1)	3.04	P-I	0 / 1470	0.33 (1)
H-I	-3172 / 0	-91.8 -91.8	0.77 (1)	3.22	D-R	0 / 1470	0.33 (1)
I-J	-2557 / 0	-91.8 -91.8	0.26 (1)	4.10	P-H	-912 / 0	0.50 (1)
J-K	0 / 19	-91.8 -91.8	0.16 (1)	10.00	R-E	-912 / 0	0.50 (1)
K-L	0 / 35	-91.8 -91.8	0.12 (1)	10.00	Q-H	0 / 427	0.10 (1)
U-B	-254 / 0	0.0 0.0	0.02 (1)	7.81	E-Q	0 / 327	0.10 (1)
M-K	-254 / 0	0.0 0.0	0.02 (1)	7.81	Q-F	-486 / 0	0.27 (1)
U-T	0 / 2003	-18.5 -18.5	0.29 (1)	10.00			
T-S	0 / 2109	-18.5 -18.5	0.30 (1)	10.00			
S-R	0 / 2109	-18.5 -18.5	0.30 (1)	10.00			
R-Q	0 / 3172	-18.5 -18.5	0.43 (1)	10.00			
Q-P	0 / 3172	-18.5 -18.5	0.43 (1)	10.00			
P-O	0 / 2109	-18.5 -18.5	0.30 (1)	10.00			
O-N	0 / 2109	-18.5 -18.5	0.30 (1)	10.00			
N-M	0 / 2003	-18.5 -18.5	0.29 (1)	10.00			

TOTAL WEIGHT = 2 X 186 = 372 LB

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.33")

CSI: TC=0.82/1.00 (F-H:1), BC=0.43/1.00 (Q-R:1), WB=0.69/1.00 (C-L:1), SS=0.26/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

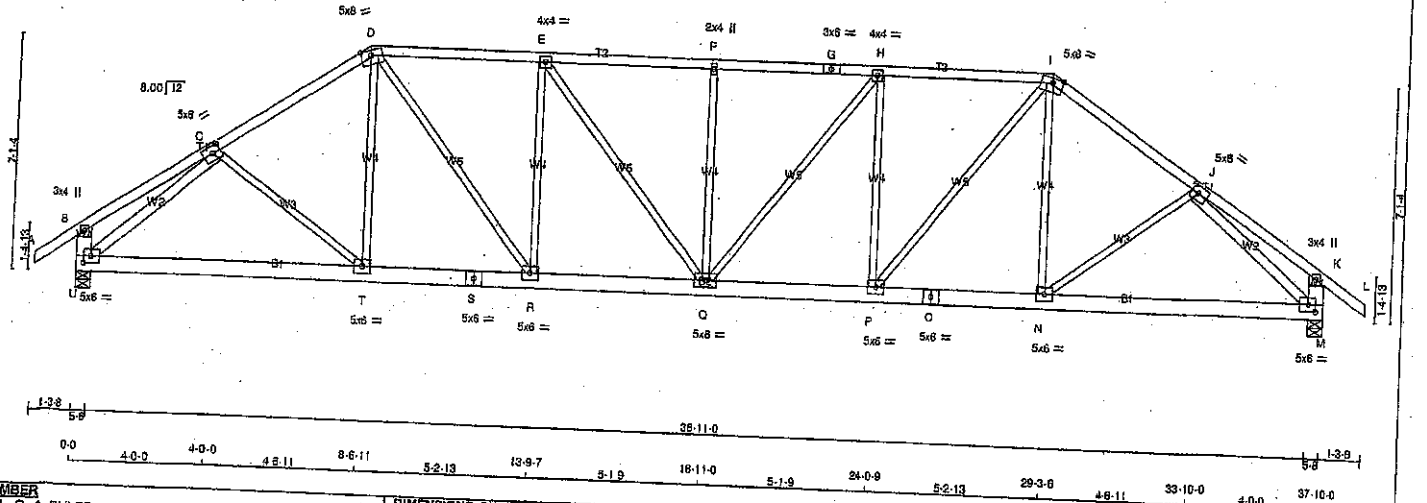
JSI GRIP=0.86 (I) (INPUT = 0.80)
JSI METAL=0.83 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108113

JOB NAME 417441	TRUSS NAME T18	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						

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 ID: v5FX5Bj0jrZNLEnu26zoDzZl9Z-gOZVWK7ZzEowZWcehOeKuwtKQ_zcEiWCFtQjLzZ6H0
 Scale: 3/16" = 1'



LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
U - S	2x6	DRY	No.2
S - O	2x6	DRY	No.2
O - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
U - C	2x4	DRY	No.2
J - M	2x4	DRY	No.2

DRY, SEASONED LUMBER.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	5.0	6.0	2.50 2.75
D	TTWW-m	MT20	5.0	8.0	1.75 3.75
E	TMWW-t	MT20	4.0	4.0	
F	TMWW-w	MT20	2.0	4.0	
G	TS-t	MT20	3.0	6.0	
H	TMWW-t	MT20	4.0	4.0	
I	TTWW-m	MT20	5.0	8.0	1.75 3.75
J	TMWW-t	MT20	5.0	6.0	2.50 2.75
K	TMV+p	MT20	3.0	4.0	
M	BMVW1-t	MT20	5.0	6.0	2.50 2.75
N, P, R, T					
N	BMWW-t	MY20	5.0	6.0	
O	BS-t	MT20	5.0	6.0	
Q	BMWWW-t	MT20	5.0	8.0	
S	BS-t	MT20	5.0	6.0	
U	BMVW1-t	MT20	5.0	6.0	2.50 2.75

NOTES- (1)
 1) Lateral bracing to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	VERT	HORIZ	DOWN	HORIZ	BRG	IN-SX	BRG	IN-SX
M	2212	0		2212	0	0	5-8	5-8	
U	2212	0		2212	0	0	5-8	5-8	

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
M	1582	1038 / 0	0 / 0
U	1582	1038 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.55 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0 / 95	-91.8	-91.8 0.12 (1)	T-D	0 / 140	0.05 (4)	
B-C	0 / 27	-91.8	-91.8 0.27 (1)	N-I	0 / 140	0.05 (4)	
C-D	-2515 / 0	-91.8	-91.8 0.41 (1)	P-I	0 / 1110	0.25 (1)	
D-E	-2755 / 0	-91.8	-91.8 0.53 (1)	D-R	0 / 1110	0.25 (1)	
E-F	-2952 / 0	-91.8	-91.8 0.54 (1)	P-H	-795 / 0	0.85 (1)	
F-G	-2952 / 0	-91.8	-91.8 0.54 (1)	R-E	-785 / 0	0.65 (1)	
G-H	-2952 / 0	-91.8	-91.8 0.54 (1)	Q-H	0 / 325	0.07 (1)	
H-I	-2755 / 0	-91.8	-91.8 0.53 (1)	E-Q	0 / 325	0.07 (1)	
I-J	-2515 / 0	-91.8	-91.8 0.41 (1)	Q-F	-432 / 0	0.35 (1)	
J-K	0 / 27	-91.8	-91.8 0.27 (1)	C-T	0 / 71	0.02 (4)	
K-L	0 / 95	-91.8	-91.8 0.12 (1)	U-C	-2785 / 0	0.85 (1)	
L-M	-255 / 0	0.0	0.0 0.02 (1)	N-J	0 / 71	0.02 (4)	
M-K	-255 / 0	0.0	0.0 0.02 (1)	J-M	-2785 / 0	0.85 (1)	
U-T	0 / 2047	-18.5	-18.5 0.33 (1)				
T-S	0 / 2073	-18.5	-18.5 0.31 (1)				
S-R	0 / 2073	-18.5	-18.5 0.31 (1)				
R-Q	0 / 2755	-18.5	-18.5 0.37 (1)				
Q-P	0 / 2755	-18.5	-18.5 0.37 (1)				
P-O	0 / 2073	-18.5	-18.5 0.31 (1)				
O-N	0 / 2073	-18.5	-18.5 0.31 (1)				
N-M	0 / 2047	-18.5	-18.5 0.33 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPC 2014

(55% OF 81.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.26")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.14")
 ALLOWABLE DEFL. (TL) = L/360 (1.26")
 CALCULATED VERT. DEFL. (TL) = L/999 (0.26")

CSI: TC=0.54/1.00 (F-H:1), BC=0.37/1.00 (D-R:1), WB=0.85/1.00 (J-M:1), SI=0.23/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HELLS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

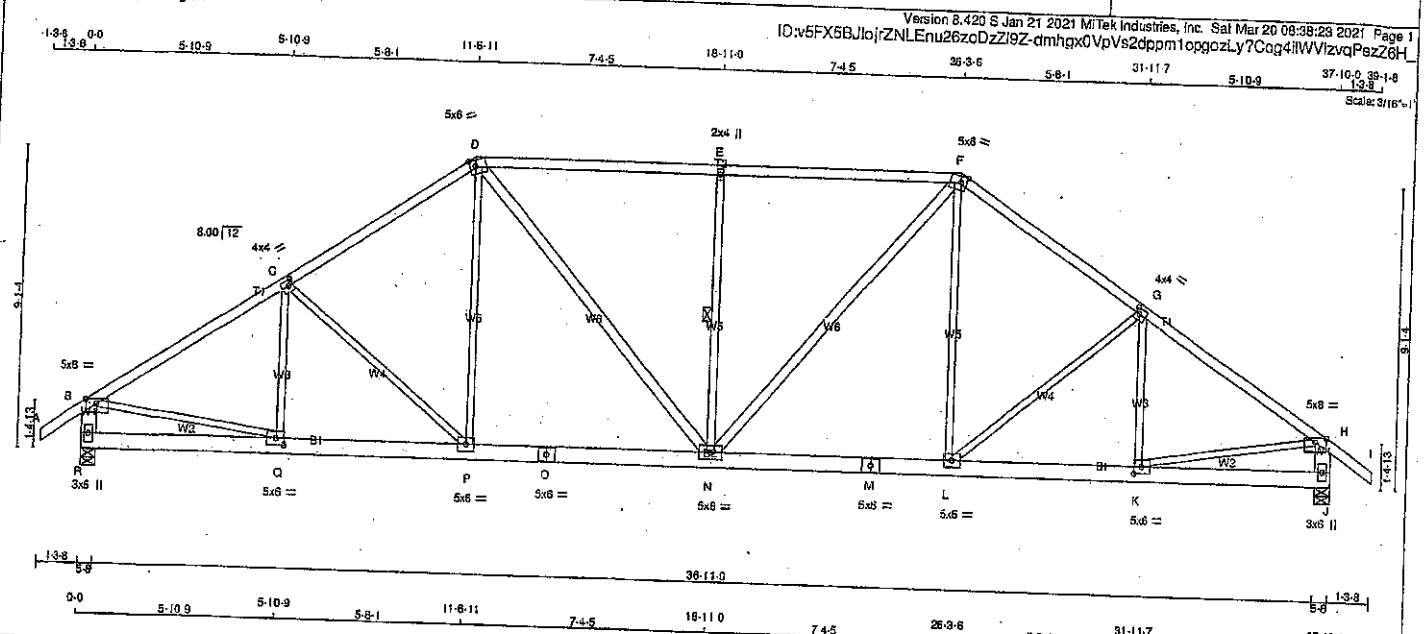
JSI GRIP = 0.88 (U) (INPUT = 0.90.)

JSI METAL = 0.83 (U) (INPUT = 1.00.)



Structural component only
 DWG# T-2108114

JOB NAME 417441	TRUSS NAME T20	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2	
D - F	2x4	DRY	No.2	
F - I	2x4	DRY	No.2	
R - B	2x6	DRY	No.2	
J - H	2x6	DRY	No.2	
R - O	2x6	DRY	No.2	
O - M	2x6	DRY	No.2	
M - J	2x6	DRY	No.2	
ALL WEBS	2x3	DRY	No.2	
EXCEPT				
D - N	2x4	DRY	No.2	
N - F	2x4	DRY	No.2	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMVW-p	MT20	5.0	8.0	Edge
C TMVW-i	MT20	4.0	4.0	2.00 1.50
D TTWW-m	MT20	5.0	6.0	2.25 2.00
E TMVW-w	MT20	2.0	4.0	
F TTWW-m	MT20	5.0	6.0	2.25 2.00
G TMVW-i	MT20	4.0	4.0	2.00 1.50
H TMVW-p	MT20	5.0	8.0	Edge
J BMV1-p	MT20	3.0	6.0	
K BMVW-i	MT20	5.0	6.0	2.50 2.75
L BMVW-i	MT20	5.0	6.0	
M BS-i	MT20	5.0	6.0	
N BMVW-w	MT20	5.0	8.0	
O BS-i	MT20	5.0	6.0	
P BMVW-i	MT20	5.0	6.0	
Q BMVW-i	MT20	5.0	6.0	2.50 2.75
R BMV1-p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
R	2212	0	0	5-8
J	2212	0	0	5-8

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
R	1562	1038 / 0
J	1562	1038 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	MAX	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. (PLF)	LC1	CS1 (LC)	UNBRAC	FR-TO	FORCE (LBS)
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	Q-C	-279 / 0
B-C	-2583 / 0	-91.8	-91.8	0.55 (1)	3.81	C-P	-332 / 0
C-D	-2336 / 0	-91.8	-91.8	0.50 (1)	4.00	P-D	0 / 353
D-E	-2336 / 0	-91.8	-91.8	0.78 (1)	3.50	D-N	0 / 629
E-F	-2336 / 0	-91.8	-91.8	0.78 (1)	3.50	N-E	-831 / 0
F-G	-2336 / 0	-91.8	-91.8	0.55 (1)	3.81	E-F	0 / 629
G-H	-2583 / 0	-91.8	-91.8	0.55 (1)	3.81	L-F	0 / 353
H-I	0 / 35	-91.8	-91.8	0.12 (1)	10.00	L-G	-332 / 0
I-J	-2155 / 0	0.0	0.0	0.14 (1)	6.94	K-G	-279 / 0
J-H	-2155 / 0	0.0	0.0	0.14 (1)	6.94	B-Q	0 / 2209
R-Q	0 / 0	-18.5	-18.5	0.07 (4)	10.00	K-H	0 / 2209
Q-P	0 / 2178	-18.5	-18.5	0.31 (1)	10.00		
P-O	0 / 1930	-18.5	-18.5	0.28 (1)	10.00		
O-N	0 / 1930	-18.5	-18.5	0.28 (1)	10.00		
N-M	0 / 1930	-18.5	-18.5	0.28 (1)	10.00		
M-L	0 / 1930	-18.5	-18.5	0.28 (1)	10.00		
L-K	0 / 2178	-18.5	-18.5	0.31 (1)	10.00		
K-J	0 / 0	-18.5	-18.5	0.07 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 089-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.78/1.00 (D-E:1), SC=0.31/1.00 (P-Q:1), WB=0.50/1.00 (B-Q:1), SS=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

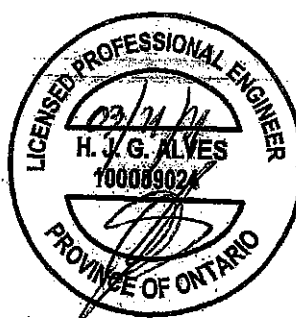
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

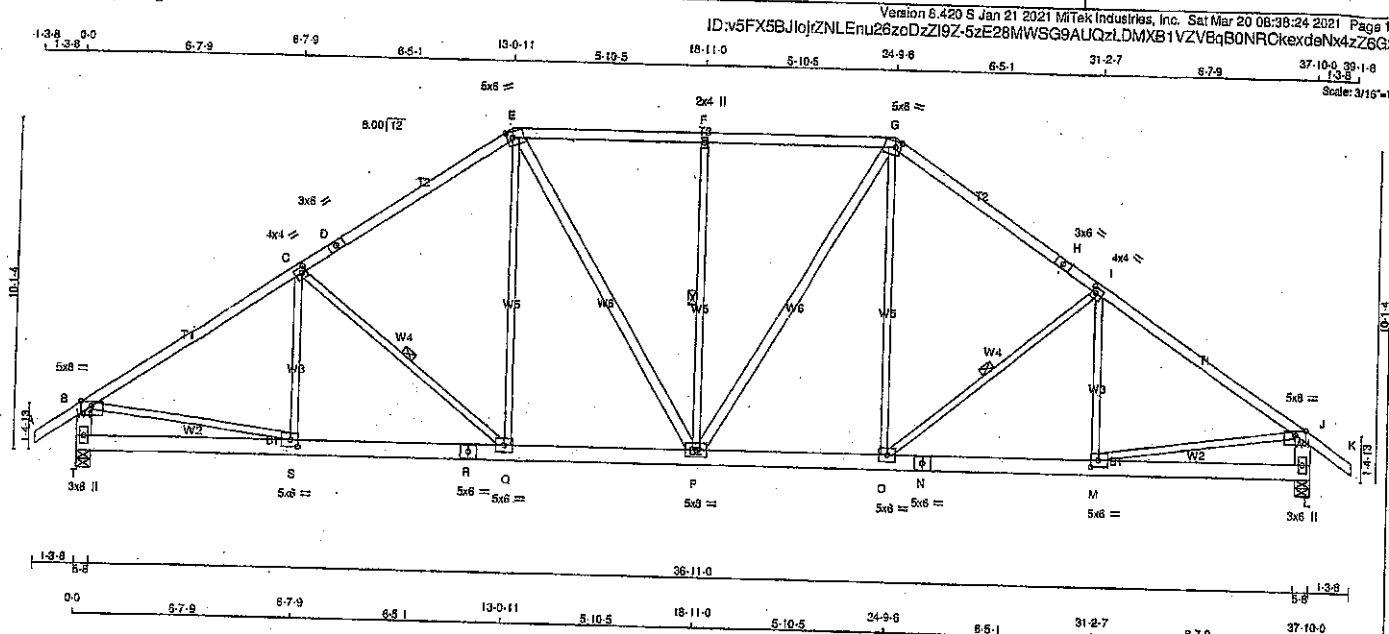
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.88 (C) (INPUT = 0.90)
JSI METAL=0.50 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108116

JOB NAME 417441	TRUSS NAME T21	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DWG NO.
Tamarack Roof Truss, Burlington						



LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF
A - D	2x4	DRY	No.2	SPF	SPF
D - E	2x4	DRY	No.2	SPF	SPF
E - G	2x4	DRY	No.2	SPF	SPF
G - H	2x4	DRY	No.2	SPF	SPF
H - K	2x4	DRY	No.2	SPF	SPF
T - B	2x6	DRY	No.2	SPF	SPF
L - J	2x6	DRY	No.2	SPF	SPF
T - R	2x6	DRY	No.2	SPF	SPF
R - N	2x6	DRY	No.2	SPF	SPF
N - L	2x6	DRY	No.2	SPF	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	SPF
E - P	2x4	DRY	No.2	SPF	SPF
P - G	2x4	DRY	No.2	SPF	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	X
B	TMVW-p	MT20	5.0	8.0	Edge
C	TMVW-t	MT20	4.0	4.0	2.00 1.50
D	TS-t	MT20	3.0	6.0	
E	TTWV-m	MT20	5.0	6.0	2.25 2.00
F	TMVW-m	MT20	2.0	4.0	
G	TTWV-m	MT20	5.0	6.0	2.25 2.00
H	TS-t	MT20	3.0	6.0	
I	TMVW-t	MT20	4.0	4.0	2.00 1.50
J	TMVW-p	MT20	5.0	8.0	Edge
L	BMV1-p	MT20	3.0	6.0	
M	BMVW-t	MT20	5.0	6.0	2.50 2.75
N	BS-t	MT20	5.0	6.0	
O	BMVW-t	MT20	5.0	6.0	
P	BMVW-t	MT20	5.0	6.0	
Q	BMVW-t	MT20	5.0	6.0	
R	BS-t	MT20	5.0	6.0	
S	BMVW-t	MT20	5.0	6.0	2.50 2.75
T	BMV1-p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	JT	DOWN	JT	IN-SX	JT	IN-SX
T	2212	0	2212	0	5-8	5-8	5-8
L	2212	0	2212	0	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE	MAX. MIN. COMPONENT REACTIONS	JT	1ST LOASE	MAX. MIN. COMPONENT REACTIONS
T	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
L	1592	1038	0	0	0
L	1592	1038	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.59 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

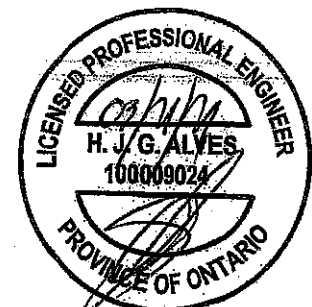
1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-Q, F-P, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	S-C	-213 / 37	0.10 (1)
B-C	-2599 / 0	-91.8	-91.8 0.73 (1)	3.59	C-Q	-465 / 0	0.23 (1)
C-D	-2252 / 0	-91.8	-91.8 0.85 (1)	3.88	Q-E	0 / 423	0.10 (1)
D-E	-2252 / 0	-91.8	-91.8 0.85 (1)	3.88	E-P	0 / 440	0.07 (1)
E-F	-2070 / 0	-91.8	-91.8 0.47 (1)	4.22	P-F	-657 / 0	0.44 (1)
F-G	-2070 / 0	-91.8	-91.8 0.47 (1)	4.22	P-G	0 / 440	0.07 (1)
G-H	-2252 / 0	-91.8	-91.8 0.85 (1)	3.88	O-G	0 / 423	0.10 (1)
H-I	-2252 / 0	-91.8	-91.8 0.85 (1)	3.88	O-I	-465 / 0	0.23 (1)
I-J	-2599 / 0	-91.8	-91.8 0.73 (1)	3.59	M-I	-213 / 37	0.10 (1)
J-K	0 / 35	-91.8	-91.8 0.12 (1)	10.00	B-S	0 / 2220	0.50 (1)
T-B	-2152 / 0	0.0	0.0 0.14 (1)	6.94	M-J	0 / 2220	0.50 (1)
L-J	-2152 / 0	0.0	0.0 0.14 (1)	6.94			
T-S	0 / 0	-18.5	-18.5 0.08 (4)	10.00			
S-R	0 / 2196	-18.5	-18.5 0.30 (1)	10.00			
R-Q	0 / 2196	-18.5	-18.5 0.30 (1)	10.00			
Q-P	0 / 1842	-18.5	-18.5 0.26 (1)	10.00			
P-O	0 / 1842	-18.5	-18.5 0.26 (1)	10.00			
O-N	0 / 2196	-18.5	-18.5 0.30 (1)	10.00			
N-M	0 / 2196	-18.5	-18.5 0.30 (1)	10.00			
M-L	0 / 0	-18.5	-18.5 0.08 (4)	10.00			



Structural component only
DWG# T-2108117

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2018 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.28")
CALCULATED VERT. DEFL. (LL) = L/999 (0.10")
ALLOWABLE DEFL. (TL) = L/360 (1.28")
CALCULATED VERT. DEFL. (TL) = L/999 (0.19")

CS: TC=0.73/1.00 (B-C:1), BC=0.30/1.00 (Q-S:1), WS=0.50/1.00 (B-S:1), SS=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 789 1987 1873

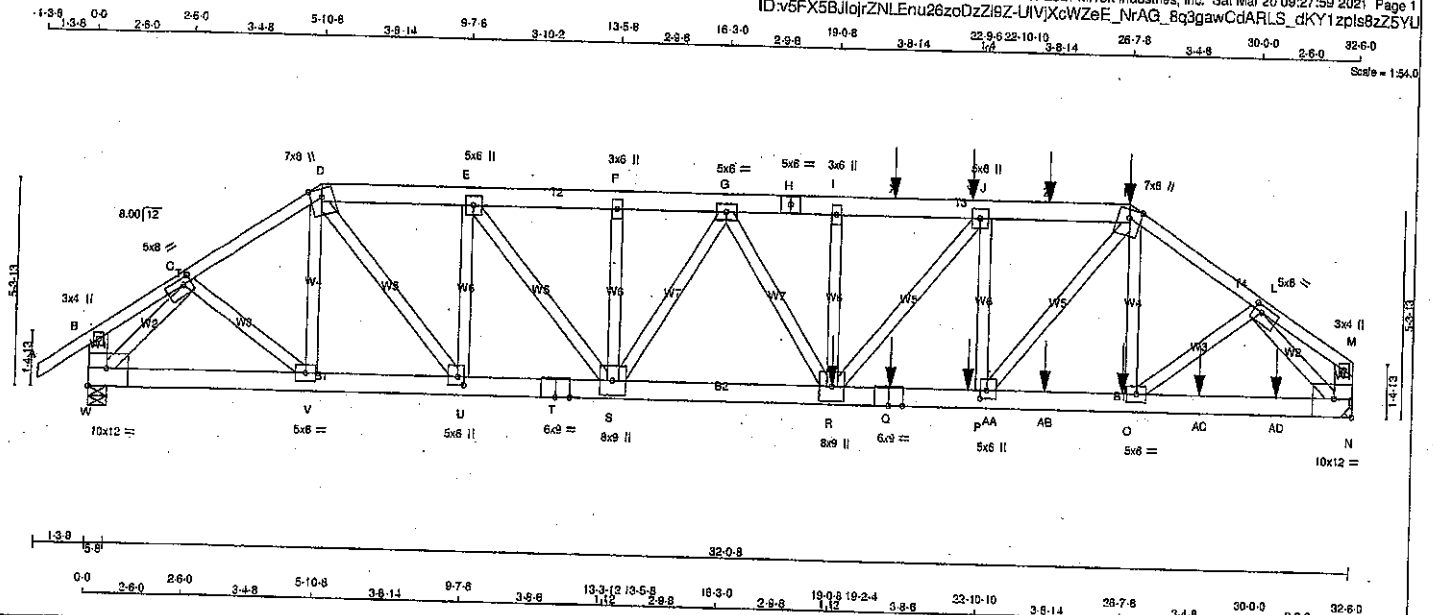
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.87 (M) (INPUT = 0.90)
JSI METAL = 0.50 (S) (INPUT = 1.00)

JOB NAME 417449	TRUSS NAME T22	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Sat Mar 20 09:27:59 2021 Page 1
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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
E - H	2x6	DRY No.2	SPF
H - K	2x6	DRY No.2	SPF
K - M	2x4	DRY No.2	SPF
N - B	2x6	DRY No.2	SPF
N - M	2x6	DRY No.2	SPF
W - T	2x6	DRY No.2	SPF
T - Q	2x6	DRY 2100F 1.8E	SPF
Q - N	2x6	DRY 2100F 1.8E	SPF

ALL WEBS 2x4 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	8.0	2.00	2.50
D	TTWV+m	MT20	7.0	8.0	Edge	
E	TMVW-t	MT20	5.0	6.0		
F	TMVW-w	MT20	3.0	6.0		
G	TMVW-t	MT20	5.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMVW-w	MT20	3.0	6.0		
J	TMVW-t	MT20	5.0	6.0		
K	TTWV+m	MT20	7.0	8.0	Edge	
L	TMVW-t	MT20	5.0	8.0	2.00	2.50
M	TMV+p	MT20	3.0	4.0		
N	BMVW-t	MT20	10.0	12.0	5.50	Edge
O	BMVW-t	MT20	5.0	6.0		
P	BMVW-t	MT20	5.0	6.0	2.50	2.00
Q	BS-t	MT20	6.0	9.0		
R	BMVWW-t	MT20	8.0	9.0		
S	BMVWW-t	MT20	8.0	9.0		
T	BS-t	MT20	6.0	9.0		
U	BMVW-t	MT20	5.0	6.0	2.50	2.00
V	BMVW-t	MT20	5.0	6.0		
W	BMVW-t	MT20	10.0	12.0	5.50	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
N	3959 0	3959 0	5-8	5-8
W	3080 0	3080 0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT N. MINIMUM BEARING LENGTH AT JOINT N = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	2799	1841 / 0	0 / 0	0 / 0	0 / 0	0 / 0	958 / 0	0 / 0
W	2173	1452 / 0	0 / 0	0 / 0	0 / 0	0 / 0	722 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)
A-B	0 / 35	-91.8	-91.8 0.14 (1)	10.00	V-D	-326 / 0
B-C	0 / 18	-91.8	-91.8 0.12 (1)	10.00	O-K	-443 / 0
C-D	-3790 / 0	-91.8	-91.8 0.39 (1)	3.28	R-I	-359 / 0
D-E	-4802 / 0	-91.8	-91.8 0.24 (1)	3.80	S-F	-285 / 0
E-F	-6123 / 0	-91.8	-91.8 0.33 (1)	3.31	C-V	0 / 696
F-G	-6123 / 0	-91.8	-91.8 0.28 (1)	3.37	W-C	-3943 / 0
G-H	-7432 / 0	-91.8	-91.8 0.40 (1)	2.96	L-N	-5161 / 0
H-I	-7432 / 0	-91.8	-91.8 0.40 (1)	2.96	O-L	0 / 1039
I-J	-7432 / 0	-91.8	-91.8 0.57 (1)	2.77	E-S	0 / 2124
J-K	-6237 / 0	-91.8	-91.8 0.57 (1)	2.77	D-U	0 / 2702
K-L	-6237 / 0	-91.8	-91.8 0.41 (1)	3.20	U-E	-2079 / 0
L-M	-5163 / 0	-91.8	-91.8 0.41 (1)	3.20	P-K	0 / 3144
M-N	0 / 15	-91.8	-91.8 0.59 (1)	2.81	R-J	0 / 1920
W-B	-203 / 0	-91.8	-91.8 0.10 (1)	10.00	P-J	-2334 / 0
N-M	-92 / 0	0.0	0.0 0.01 (1)	7.81	S-G	-1474 / 0
					G-R	0 / 1153
W-V	0 / 2604	-18.5	-18.5 0.16 (1)	10.00		
V-U	0 / 3165	-18.5	-18.5 0.18 (1)	10.00		
U-T	0 / 4801	-18.5	-18.5 0.27 (1)	10.00		
T-S	0 / 4801	-18.5	-18.5 0.27 (1)	10.00		
S-R	0 / 6858	-18.5	-18.5 0.42 (1)	10.00		
R-Q	0 / 6236	-18.5	-18.5 0.39 (1)	10.00		
Q-AA	0 / 6236	-18.5	-18.5 0.39 (1)	10.00		
AA-P	0 / 6236	-18.5	-18.5 0.39 (1)	10.00		
P-AB	0 / 4333	-18.5	-18.5 0.25 (1)	10.00		
AB-O	0 / 4333	-18.5	-18.5 0.25 (1)	10.00		
O-AC	0 / 3496	-18.5	-18.5 0.23 (1)	10.00		
AC-AD	0 / 3496	-18.5	-18.5 0.23 (1)	10.00		
AD-N	0 / 3496	-18.5	-18.5 0.23 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
K	26-7-8	-341	-341	---	FRONT	VERT	TOTAL	C1
O	28-8-12	-29	-29	---	FRONT	VERT	TOTAL	C1
Q	20-8-12	-29	-29	---	FRONT	VERT	TOTAL	C1
R	19-0-8	-1471	-1471	---	FRONT	VERT	TOTAL	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 8.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 33.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(65% OF 91.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.08")
CALCULATED VERT. DEFL. (LL) = L/999 (0.22")
ALLOWABLE DEFL. (TL) = L/360 (1.08")
CALCULATED VERT. DEFL. (TL) = L/954 (0.41")

CSI: TC=0.59/1.00 (K-L-1), BC=0.42/1.00 (R-S-1),
WB=0.81/1.00 (L-N-1), SS=0.31/1.00 (I-J-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 789 1987 1879

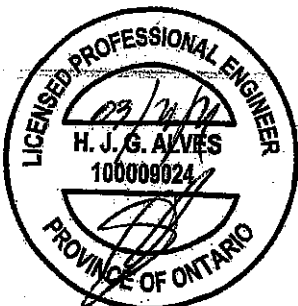
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.87 (P) (INPUT = 0.80)

JSI METAL = 0.98 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2108165

JOB NAME 417449	TRUSS NAME T22	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Sat Mar 20 09:27:59 2021 Page 2
ID:v5FX5BjloirZNLEnu26zoDzZl9Z-UlVIXoWZeE NrAG 8a3gawCdARLS dKY1zpl68zZ5YU

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
X	20-6-12	-122	-122	---	FRONT	VERT	TOTAL	---	C1
Y	22-6-12	-122	-122	---	FRONT	VERT	TOTAL	---	C1
Z	24-6-12	-122	-122	---	FRONT	VERT	TOTAL	---	C1
AA	22-6-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AB	24-6-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AC	28-6-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AD	30-6-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

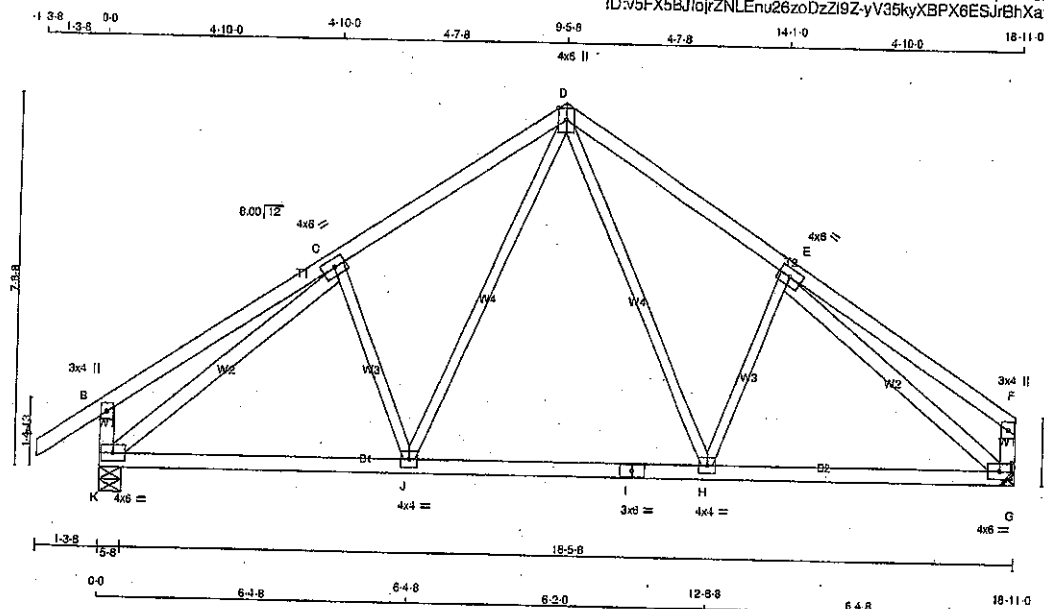


Structural component only
DWG# T-2108165 *M*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417449	T23	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 09:28:00 2021 Page 1
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Scale = 1/4" = 1'-0"

LUMBER

N.L.G.A. RULES

CHORDS SIZE

A - D 2x4 DRY No.2

D - F 2x4 DRY No.2

K - B 2x4 DRY No.2

G - F 2x4 DRY No.2

K - I 2x4 DRY No.2

I - G 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

K - C 2x4 DRY No.2

E - G 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW+1	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW+1	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1+1	MT20	4.0	6.0		
H	BMVW+1	MT20	4.0	4.0		
I	BS+1	MT20	3.0	6.0		
J	BMVW+1	MT20	4.0	4.0		
K	BMVW1+1	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
K	1169	0	1169	0
G	1043	0	1043	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3'-8".

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	824	554 / 0	0 / 0	0 / 0	0 / 0	0 / 0	270 / 0	0 / 0
G	737	484 / 0	0 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO				FR-TO			
A-B	0 / 35	-91.8	-91.8 0.12 (1)	D-H	0 / 413	0.09 (1)	0.09 (1)
B-C	0 / 29	-91.8	-91.8 0.33 (1)	H-E	-275 / 0	0.10 (1)	0.10 (1)
C-D	-1018 / 0	-91.8	-91.8 0.27 (1)	J-D	0 / 413	0.09 (1)	0.09 (1)
D-E	-1018 / 0	-91.8	-91.8 0.27 (1)	C-J	-275 / 0	0.10 (1)	0.10 (1)
E-F	0 / 29	-91.8	-91.8 0.33 (1)	K-C	-1244 / 0	0.62 (1)	0.62 (1)
K-B	-289 / 0	0.0	0.0 0.03 (1)	E-G	-1244 / 0	0.62 (1)	0.62 (1)
G-F	-183 / 0	0.0	0.0 0.02 (1)				
K-J	0 / 926	-18.5	-18.5 0.24 (4)				
J-I	0 / 665	-18.5	-18.5 0.22 (4)				
I-H	0 / 665	-18.5	-18.5 0.22 (4)				
H-G	0 / 926	-18.5	-18.5 0.24 (4)				

TOTAL WEIGHT = 3 X 85 = 255 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019

- PART 9 OF CBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.63")
CALCULATED VERT. DEFL. (LL) = L/999 (0.03")
ALLOWABLE DEFL. (TL) = L/360 (0.63")
CALCULATED VERT. DEFL. (TL) = L/999 (0.07")

CSI: TC=0.33/1.00 (B-C:1), BC=0.24/1.00 (J-K:4),
WS=0.82/1.00 (C-K:1), SS=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

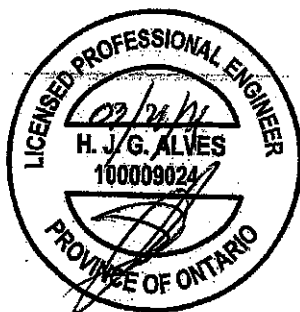
PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (E) (INPUT = 0.90)

JSI METAL= 0.29 (C) (INPUT = 1.00)

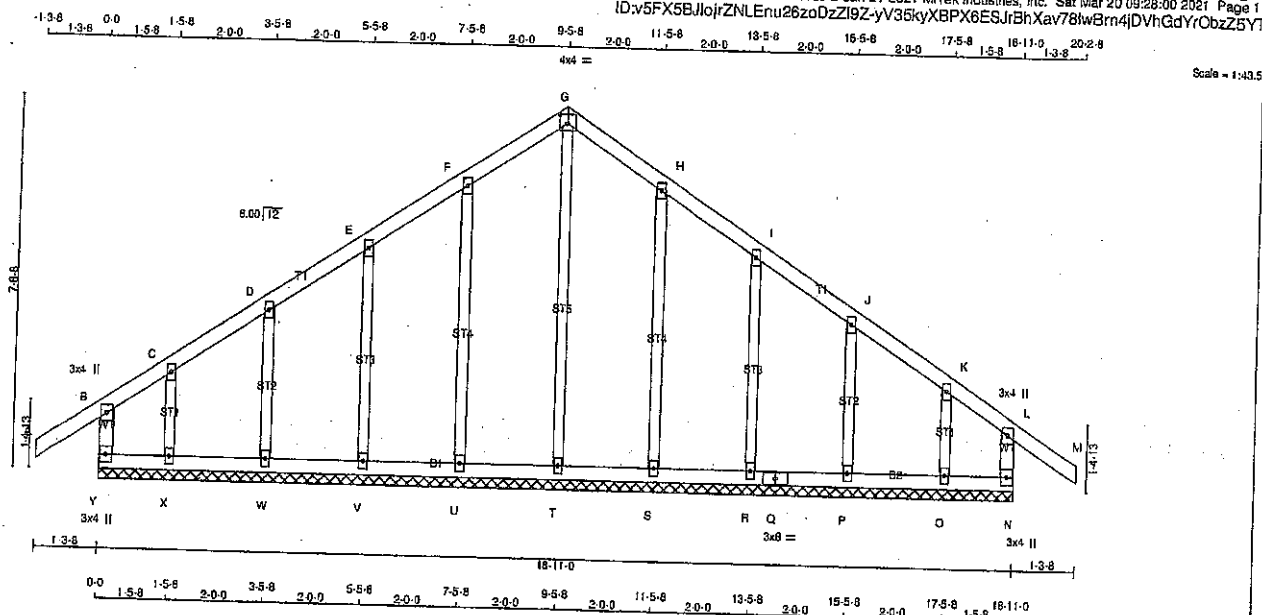


Structural component only
DWG# T-2108166

JOB NAME 417449	TRUSS NAME T23G	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - G	2x4 DRY	No.2	SPF
G - M	2x4 DRY	No.2	SPF
Y - B	2x4 DRY	No.2	SPF
N - L	2x4 DRY	No.2	SPF
Y - Q	2x4 DRY	No.2	SPF
Q - N	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
ALL GABLE WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0.0" OC.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C, D, E, F, H, I, J, K					
C TMW+w	MT20	2.0	4.0		
G TTW+p	MT20	4.0	4.0	2.25	2.00
L TMV+p	MT20	3.0	4.0		
N BMV1+p	MT20	3.0	4.0		
O, P, R, S, T, U, V, W, X					
O BMW1+w	MT20	2.0	4.0		
Q BS-I	MT20	3.0	6.0		
Y BMV1+p	MT20	3.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S).

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0/35	T-G	-225/0
B-C	-2/0	U-F	-190/0
C-D	0/27	V-E	-177/0
D-E	0/28	W-D	-193/0
E-F	0/32	X-C	-109/0
F-G	0/35	S-H	-190/0
G-H	0/33	R-I	-177/0
H-I	0/32	P-J	-193/0
I-J	0/28	O-K	-109/0
J-K	0/27		
K-L	-2/0		
L-M	0/35		
Y-B	-213/0		
N-L	-213/0		
Y-X	-14/0		
X-W	-18/0		
W-V	-23/0		
V-U	-28/0		
U-T	-28/0		
T-S	-28/0		
S-R	-25/0		
R-Q	-23/0		
Q-P	-23/0		
P-O	-18/0		
O-N	-14/0		

TOTAL WEIGHT = 64 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL	= 6.0	PSF
BOT CH.	LL = 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSF: TC=0.12/1.00 (L-M:1), BC=0.02/1.00 (N-O:1), WB=0.24/1.00 (G-T:1), SS=0.08/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

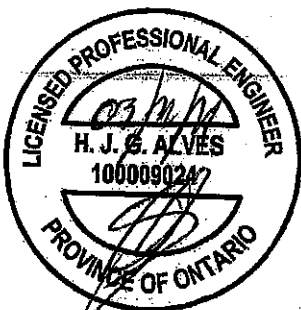
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1997 1873

PLATE PLACEMENT TOL = 0.250 inches

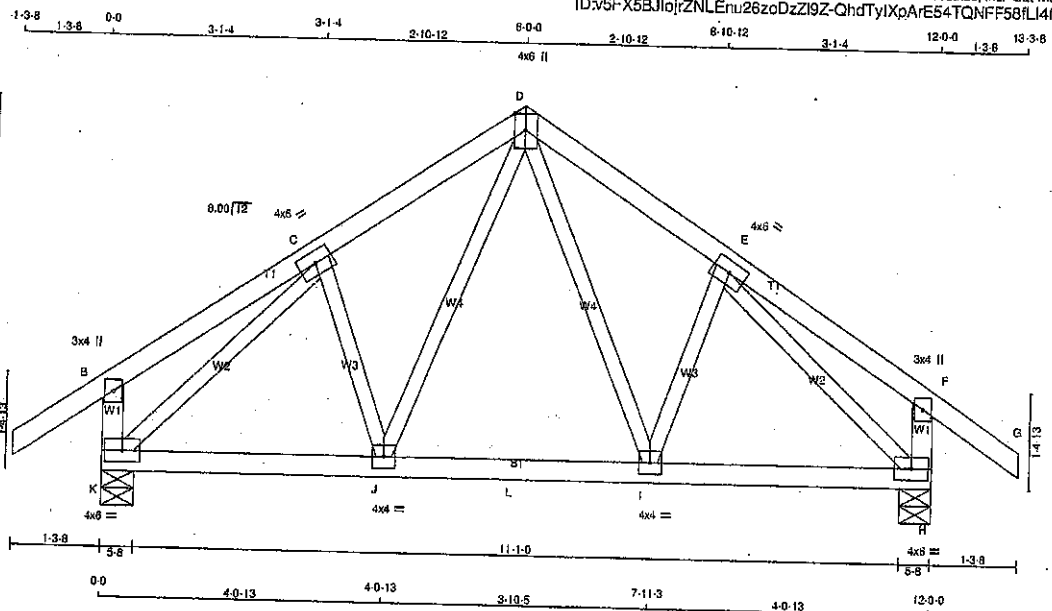
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.38 (I) (INPUT = 0.90)
JSI METAL=0.10 (J) (INPUT = 1.00)



Structural component only
DWG# T-2108167

JOB NAME 417449	TRUSS NAME T24	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
O - G	2x4	DRY	No.2
K - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
K - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-l	MT20	4.0	6.0		
D	TTVW+p	MT20	4.0	6.0	Edge	
E	TMVW-l	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW-l	MT20	4.0	6.0		
I	BMVW-l	MT20	4.0	4.0		
J	BMVW-l	MT20	4.0	4.0		
K	BMVW-l	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES:

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
K	963	0	863	0
H	1313	0	1313	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	685	425 / 0	0 / 0	0 / 0	0 / 0	260 / 0	0 / 0
H	946	521 / 0	0 / 0	0 / 0	0 / 0	425 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
A-B	0 / 35	-91.8	-91.8 0.14 (1)	10.00	D-I	0 / 636	0.16 (1)
B-C	0 / 16	-91.8	-91.8 0.13 (1)	10.00	I-E	-24 / 69	0.03 (4)
C-D	-801 / 0	-91.8	-91.8 0.14 (1)	6.25	J-D	0 / 174	0.04 (1)
D-E	-997 / 0	-91.8	-91.8 0.14 (1)	6.07	C-J	-75 / 36	0.02 (1)
E-F	0 / 15	-91.8	-91.8 0.12 (1)	10.00	K-C	-974 / 0	0.30 (1)
F-G	0 / 35	-91.8	-91.8 0.14 (1)	10.00	E-H	-1185 / 0	0.37 (1)
K-B	-238 / 0	0.0	0.0 0.03 (1)	7.81			
H-F	-236 / 0	0.0	0.0 0.03 (1)	7.81			
K-J	0 / 678	-18.5	-18.5 0.16 (1)	10.00			
J-L	0 / 591	-18.5	-18.5 0.41 (4)	10.00			
L-I	0 / 591	-135.2	-135.2 0.41 (4)	10.00			
I-H	0 / 826	-135.2	-135.2 0.44 (1)	10.00			

TOTAL WEIGHT = 59 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT), CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.40")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.40")
CALCULATED VERT. DEFL. (TL) = L/999 (0.07")

CSI: TC=0.14/1.00 (C-D-I), BC=0.44/1.00 (H-I-I), WB=0.37/1.00 (E-H-I), SH=0.31/1.00 (H-I-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES.

PLATE	GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (C) (INPUT = 0.90)
JSI METAL = 0.26 (C) (INPUT = 1.00)

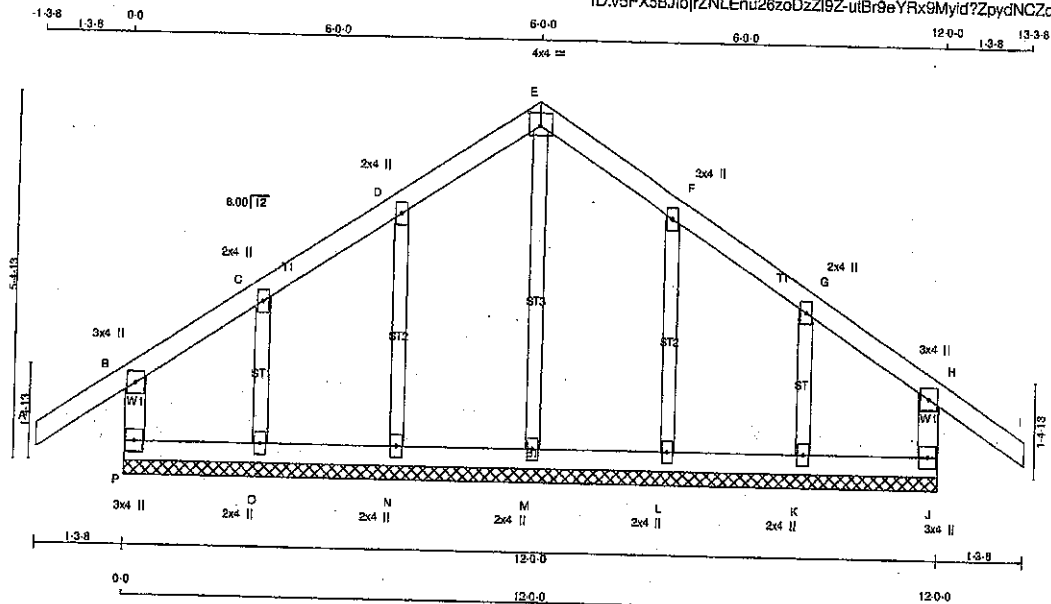


Structural component only
DWG# T-2108168

JOB NAME 417449	TRUSS NAME T24G	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 09:28:02 2021 Page 1
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Scale = 1/32"

TOTAL WEIGHT = 49 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER				

GABLE STUDS SPACED AT 2-0-0 O.C.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C, D, F, G					
C TMW+ww	MT20	2.0	4.0		
E TTW+p	MT20	4.0	4.0	2.25	2.00
H TMV+p	MT20	3.0	4.0		
J BMV1+p	MT20	3.0	4.0		
K, L, M, N, O					
K BMV1+ww	MT20	2.0	4.0		
P BMV1+p	MT20	3.0	4.0		

NOTES:

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S).

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. UNBRACED LENGTH (FT)
P-B	-221 / 0	0.0	0.0 0.04 (1)	M-E	-223 / 0	7.81	
A-B	0 / 35	-91.8	-91.8 0.12 (1)	N-D	-194 / 0	0.05 (1)	
B-C	-1 / 12	-91.8	-91.8 0.08 (1)	O-C	-150 / 0	0.02 (1)	
C-D	0 / 33	-91.8	-91.8 0.06 (1)	L-F	-194 / 0	0.05 (1)	
D-E	0 / 33	-91.8	-91.8 0.05 (1)	K-G	-150 / 0	0.02 (1)	
E-F	0 / 33	-91.8	-91.8 0.05 (1)				
F-G	0 / 33	-91.8	-91.8 0.05 (1)				
G-H	-1 / 12	-91.8	-91.8 0.08 (1)				
H-I	0 / 35	-91.8	-91.8 0.12 (1)				
J-H	-221 / 0	0.0	0.0 0.04 (1)				
P-O	-19 / 0	-18.5	-18.5 0.02 (4)				
O-N	-24 / 0	-18.5	-18.5 0.02 (4)				
N-M	-28 / 0	-18.5	-18.5 0.01 (4)				
M-L	-28 / 0	-18.5	-18.5 0.01 (4)				
L-K	-24 / 0	-18.5	-18.5 0.02 (4)				
K-J	-19 / 0	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6 PSF
DL	= 6.0 PSF	
BOY CH.	LL	= 0.0 PSF
DL	= 7.4 PSF	
TOTAL LOAD	= 39.0 PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 ; ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (N-O:4), WB=0.09/1.00 (E-M:1), SS=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

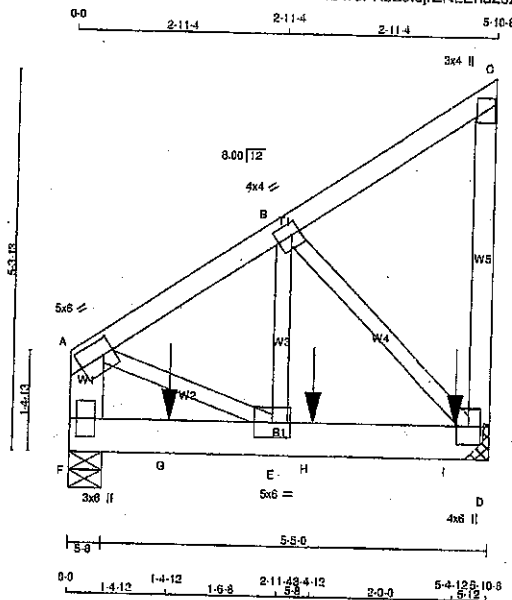
JSI GRIP = 0.18 (E) (INPUT = 0.90)

JSI METAL = 0.10 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108169

JOB NAME 417449	TRUSS NAME T25	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



Scale = 1/2" = 1'-0"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - A	2x6	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	

EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD/PLF
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1 12	TOP	
C-D 1 12	SIDE(146.4)	
F-A 2 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2 12	SIDE(122.0)	
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	5.0	6.0	2.50 1.75
B	TMVW-1	MT20	4.0	4.0	2.00 1.50
C	TMV+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORIZ	DOWN	HORIZ
D	2105	0	2105	0
F	1621	0	1621	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	1485	990	0	0	0	0	496	0
F	1144	763	0	0	0	0	381	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB. FORCE (LBS)
FR-TO		FROM TO		
A-B	-1291	0	-81.8	-81.8
B-C	-15	0	-81.8	-81.8
D-C	-108	0	0.0	0.0
F-A	-1201	0	0.0	0.0
F-G	0	0	-18.5	-18.5
G-E	0	0	-18.5	-18.5
E-H	0	1086	-18.5	-18.5
H-I	0	1086	-18.5	-18.5
I-D	0	1086	-18.5	-18.5

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-4-12	-723	-723	---	BACK	VERT	TOTAL	---	C1
H	3-4-12	-723	-723	---	BACK	VERT	TOTAL	---	C1
I	5-4-12	-727	-727	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 33 = 65 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCS 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.07/1.00 (A-3:1), BC=0.23/1.00 (D-E:1), WB=0.22/1.00 (B-D:1), SS=0.40/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

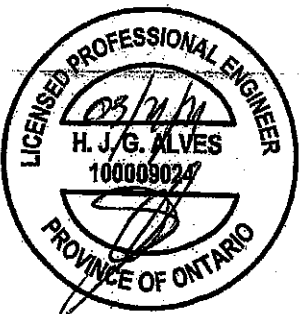
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PSI)	(PSI)	(PSI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (B) (INPUT = 0.80)
JSI METAL = 0.24 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108170

CONTINUED ON PAGE 2

JOB NAME 417449	TRUSS NAME T25	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMW1+p	MT20	4.0	6.0		
E	BMW1-l	MT20	5.0	6.0		
F	BMW1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

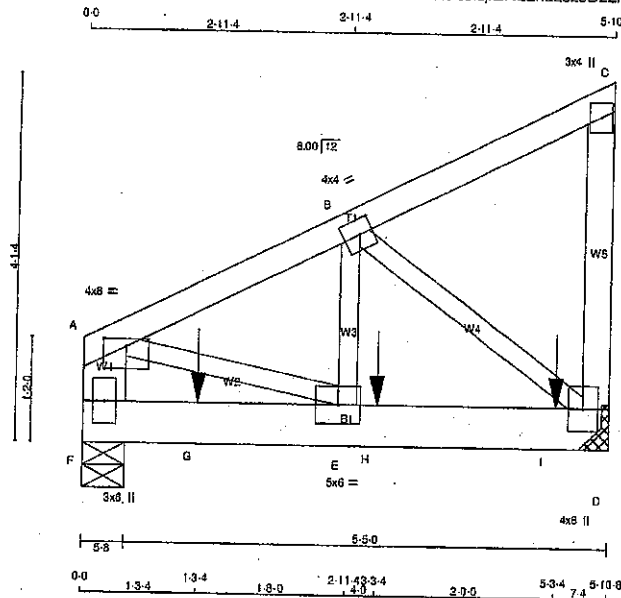


Structural component only
DWG# T-2108170 *ML*

JOB NAME 417449	TRUSS NAME T26	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/23.5

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - A	2x6	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
A - C	1	12	TOP
C - D	1	12	TOP
F - A	2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
F - D	3	4	SIDE(1556.5)
WEBS : (0.122"x3") SPIRAL NAILS			
2x3	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	8.0	1.00	3.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
D	4086	0	4086	0
F	1065	0	1065	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	2884	1928.0	0.0	0.0	0.0	0.0	958.0	0.0
F	747	526.0	0.0	0.0	0.0	0.0	222.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	W E B S	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1	MAX. CS1 (LC)	MAX. MEMB. FORCE (LBS)
FR-TO				
A-B	-1391.0	-91.8	-91.8 0.03 (1)	6.25
B-C	-11.0	-91.8	-91.8 0.03 (1)	5.25
D-C	-111.0	0.0	0.0 0.01 (1)	7.81
F-A	-1068.0	0.0	0.0 0.02 (1)	7.81
F-G	0.0	-18.5	-18.5 0.12 (1)	10.00
G-E	0.0	-18.5	-18.5 0.12 (1)	10.00
E-H	0.0/1253	-18.5	-18.5 0.48 (1)	10.00
H-I	0.0/1253	-18.5	-18.5 0.48 (1)	10.00
I-D	0.0/1253	-18.5	-18.5 0.48 (1)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
G	1-3-4	-178	-178	---	BACK	VERT	TOTAL	---	C1
H	3-3-4	-208	-208	---	BACK	VERT	TOTAL	---	C1
I	5-3-4	-2787	-2787	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 3 X 29 = 88 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	9.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/959 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/959 (0.02")

CS1: TC=0.03/1.00 (A-B:1), BC=0.46/1.00 (D-E:1), WB=0.12/1.00 (B-D:1), SS1=0.61/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.55 (B) (INPUT = 0.80)
JSI METAL = 0.19 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108171

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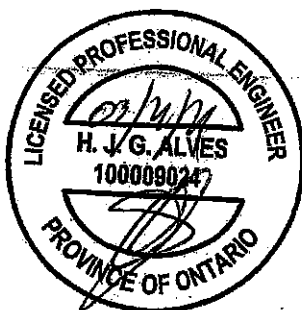
JOB NAME 417449 Tamarack Roof Truss, Burlington	TRUSS NAME T26	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-1	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	5.0		

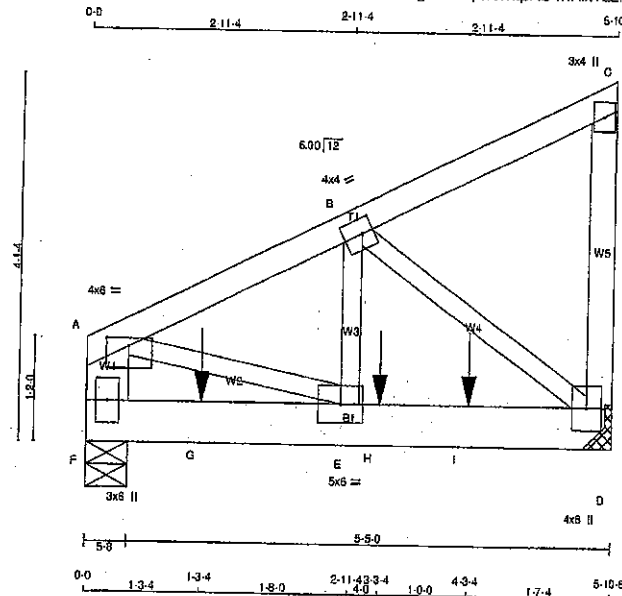
NOTES- (1)
 1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
 DWG# T-2108171 *M*

JOB NAME 417451	TRUSS NAME T26Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1/23.5

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - A	2x6	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - D	12	TOP
F - A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TMV-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
D	2177	0	2177	0
F	1274	0	1274	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4'-0."

UNFACTORED REACTIONS

JT	1ST LOASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	1532	1052 / 0	0 / 0	0 / 0	0 / 0	0 / 0	480 / 0	0 / 0
F	894	629 / 0	0 / 0	0 / 0	0 / 0	0 / 0	264 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1	MAX	CS1 (LC)	MAX	WEBS	MAX. FACTORED FORCE (LBS)	MAX	CS1 (LC)
FR-TO								FR-TO			
A-B	-1702 / 0	-91.8	-91.8	0.08 (1)	8.25	E-B	0 / 1822	0.20 (1)			
B-C	-10 / 0	-91.8	-91.8	0.05 (1)	10.00	B-D	-1926 / 0	0.23 (1)			
D-C	-113 / 0	0.0	0.0	0.01 (1)	7.81	A-E	0 / 1583	0.20 (1)			
F-A	-1282 / 0	0.0	0.0	0.05 (1)	7.81						
F-G	0 / 0	-18.5	-18.5	0.20 (1)	10.00						
G-E	0 / 0	-18.5	-18.5	0.20 (1)	10.00						
E-H	0 / 1530	-18.5	-18.5	0.82 (1)	10.00						
H-I	0 / 1530	-18.5	-18.5	0.82 (1)	10.00						
I-D	0 / 1530	-18.5	-18.5	0.82 (1)	10.00						

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-3-4	-180	-180	---	TOP	VERT	TOTAL	---	C1
H	3-3-4	-209	-209	---	TOP	VERT	TOTAL	---	C1
I	4-3-4	-1578	-1578	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/989 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CS1: TC=0.05/1.00 (A-B:1), BC=0.62/1.00 (D-E:1), WB=0.23/1.00 (B-D:1), SS=0.56/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	850 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.34 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108194 1/2

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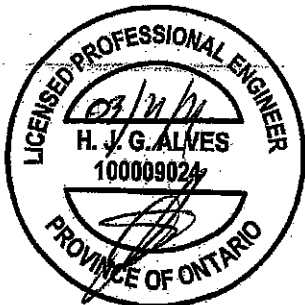
JOB NAME 417451	TRUSS NAME T26Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMW1+p	MT20	4.0	6.0		
E	BMW1-i	MT20	5.0	6.0		
F	BMW1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

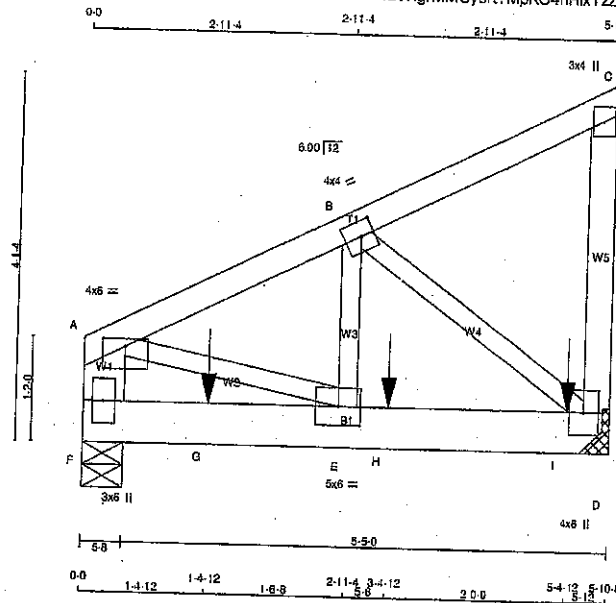


Structural component only
DWG# T-2108194 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417451	T26Z2	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/23.5

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - A	2x6	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1	12	TOP
C-D 1	12	SIDE(47.0)
F-A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE(122.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	8	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00 3.00
B	TMVW-l	MT20	4.0	4.0	2.00 1.75
C	TMV+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT		REQD	
	GROSS REACTION	GROSS REACTION	DOWN	HORIZ	BRG	BRG	IN-SX	IN-SX
JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT	IN-SX	IN-SX	
D	1547	0	1547	0	0	MECHANICAL	5-8	
F	1289	0	1289	0	0	MECHANICAL	5-8	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST LOASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	1090	741 : 0	0 : 0	0 : 0	0 : 0	349 : 0	0 : 0
F	908	623 : 0	0 : 0	0 : 0	0 : 0	284 : 0	0 : 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRACED LENGTH	MEMB. FORCE (LBS)	MAX	CS1 (LC)
FR-TO								
A-B	-1224 / 0	-91.8	-91.8	0.08 (1)	6.25	E-B	0 / 1077	0.13 (1)
B-C	-11 / 0	-91.8	-91.8	0.05 (1)	6.25	B-D	-1390 / 0	0.17 (1)
D-C	-110 / 0	0.0	0.0	0.01 (1)	7.81	A-E	0 / 1143	0.14 (1)
F-A	-954 / 0	0.0	0.0	0.03 (1)	7.81			
F-G	0 / 0	-18.5	-18.5	0.17 (1)	10.00			
G-E	0 / 0	-18.5	-18.5	0.17 (1)	10.00			
E-H	0 / 1105	-18.5	-18.5	0.18 (1)	10.00			
H-I	0 / 1105	-18.5	-18.5	0.18 (1)	10.00			
I-D	0 / 1105	-18.5	-18.5	0.18 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-4-12	-541	-541	---	BACK	VERT	TOTAL	---	C1
H	3-4-12	-541	-541	---	BACK	VERT	TOTAL	---	C1
I	5-4-12	-457	-457	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 39 = 78 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIG 2014

(55 % OF 31.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = 1/899 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = 1/899 (0.01")

CSI: TC=0.05/1.00 (A-B:1), BC=0.18/1.00 (D-E:1),
WB=0.17/1.00 (B-D:1), SS=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	850 371	1747 788
	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.70 (B) (INPUT = 0.90)
JSI METAL= 0.25 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108195 1/12

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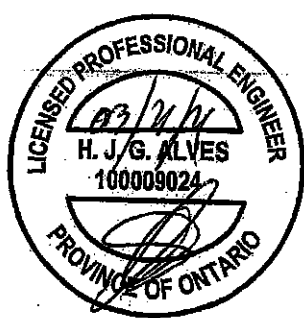
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Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

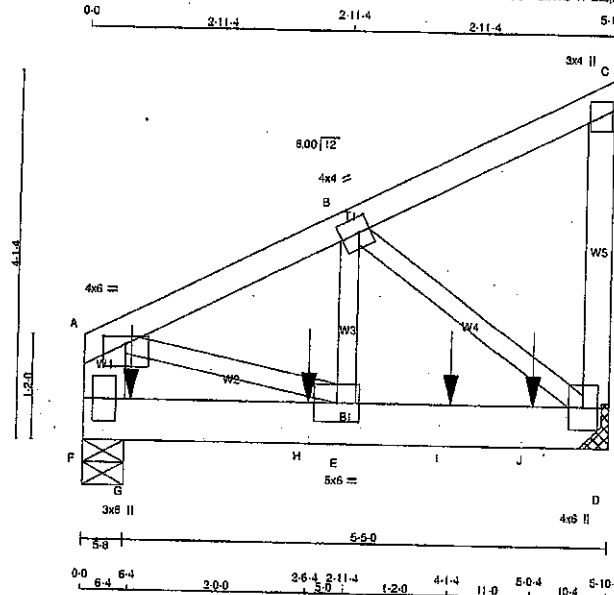
JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW1-i	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES: (1)
 1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
 DWG# T-2108195 *ML*

JOB NAME 417455	TRUSS NAME T26Z3	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 09:58:11 2021 Page 1 ID:wa216usWauA15BCJTGM04FzixO-Qv_kqCR9wzhGEdoEHNab02FQ7slaYHAFukBS/zZ56A			
		Scale = 1:23.5			



LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - A	2x6	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - D	12	TOP
F - A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT - TYPE	PLATES	W	LEN	Y	X
A - TMWW-p	MT20	4.0	6.0	1.00	3.00
B - TMWW-t	MT20	4.0	4.0	2.00	1.50
C - TMV-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
D	2657	0	2657	0
F	1341	0	1341	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
D	1873	1280 : 0	0 : 0	0 : 0	613 : 0	0 : 0
F	941	650 : 0	0 : 0	0 : 0	281 : 0	0 : 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MAX. UNBRAC LENGTH	MEMB. FORCE (LBS)
FR-TO				
A-B	-1632 / 0	-91.8	-91.8 0.08 (1)	6.25
B-C	-10 / 0	-91.8	-91.8 0.05 (1)	10.00
D-C	-113 / 0	0.0	0.0 0.01 (1)	7.81
F-A	-1234 / 0	0.0	0.0 0.04 (1)	7.81
F-G	0 / 0	-18.5	-18.5 0.20 (1)	10.00
G-H	0 / 0	-18.5	-18.5 0.20 (1)	10.00
H-E	0 / 0	-18.5	-18.5 0.20 (1)	10.00
E-I	0 / 1468	-18.5	-18.5 0.54 (1)	10.00
I-J	0 / 1468	-18.5	-18.5 0.54 (1)	10.00
J-D	0 / 1468	-18.5	-18.5 0.54 (1)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	6-4	-179	-179	---	TOP	VERT	TOTAL	---	C1
H	2-6-4	-239	-239	---	TOP	VERT	TOTAL	---	C1
I	4-1-4	-837	-837	---	FRONT	VERT	TOTAL	---	C1
J	5-0-4	-1102	-1102	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TFC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/998 (0.03")

CSI: TC=0.06/1.00 (A-B:1), BC=0.54/1.00 (D-E:1), WB=0.22/1.00 (B-D:1), SS=0.47/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

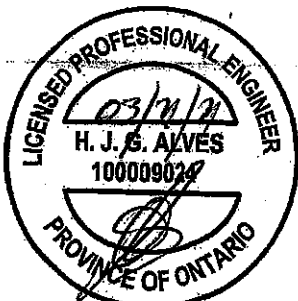
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.83 (B) (INPUT = 0.90)
JSI METAL = 0.33 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108201

CONTINUED ON PAGE 2

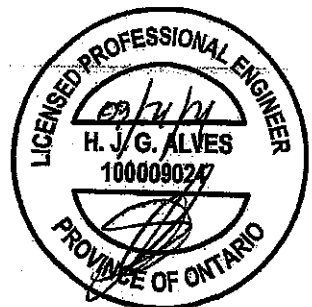
JOB NAME 417455	TRUSS NAME T26Z3	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 09:56:11 2021 Page 2
ID:wa2t6usWauA5BCJTGMC4FzZixO-Qv kqCR9wzHGEdoEHNa02FQ7slaYHAFJuk8S7zZ58A

PLATES (table is in inches)

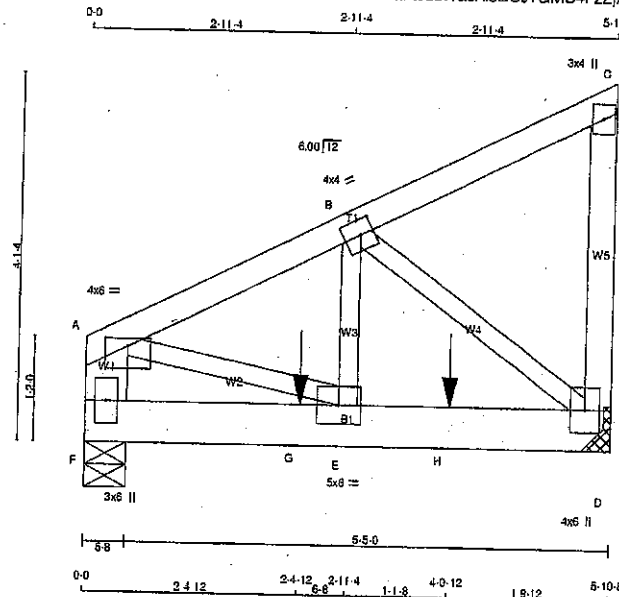
JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108201 *mn*

JOB NAME 417455	TRUSS NAME T26Z4	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 09:58:12 2021 Page 1			
		ID:wa2i6usWauAi5BCJTGMc4FzZjxO-u5Y62YSogGP7mNRf55qZFobvG7ChkCOxYUk7ZzZ569			
		Scale = 1:23.5			



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - A	2x6	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	TOP
C - D 1	12	TOP
F - A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D 2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMVW-l	MT20	4.0	4.0	2.00	1.75
C	TMV-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
D	1480	0	1480	0
F	937	0	937	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

2ND FLOOR STEEL JOISTS							
1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	1026	711 °	0 °	0 °	0 °	315 °	0 °
F	658	459 °	0 °	0 °	0 °	199 °	0 °

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MEMB.	FORCE (LBS)
FR-TO			FR-TO	
A-B	-1253	0	A-B	0
B-C	-11	0	B-D	-1423
D-C	-110	0	A-E	0
F-A	-974	0		
F-G	0	0		
G-E	0	0		
E-H	0	1131		
H-D	0	1131		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
G	2-4-12	-178	-178	---	TOP	VERT	TOTAL	---	C1
H	4-0-12	-1047	-1047	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPO 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.08/1.00 (A-B:1), BC=0.39/1.00 (D-E:1), WB=0.17/1.00 (B-D:1), SSI=0.35/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

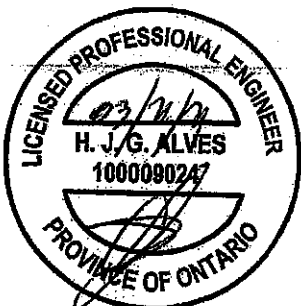
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.72 (B) (INPUT = 0.90)
JSI METAL = 0.25 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108202

CONTINUED ON PAGE 2

JOB NAME 417455	TRUSS NAME T26Z4	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 09:58:12 2021 Page 2
ID:wa2i6usWauA5BCJTGMC4FzixQ-u5Y62YSngGP7mNRi55qZFobvG7ChkCOxYUk7ZzZ569

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
D BMW1+p	MT20	4.0	6.0		
E BMWW-t	MT20	5.0	6.0		
F BMWi+p	MT20	3.0	6.0		

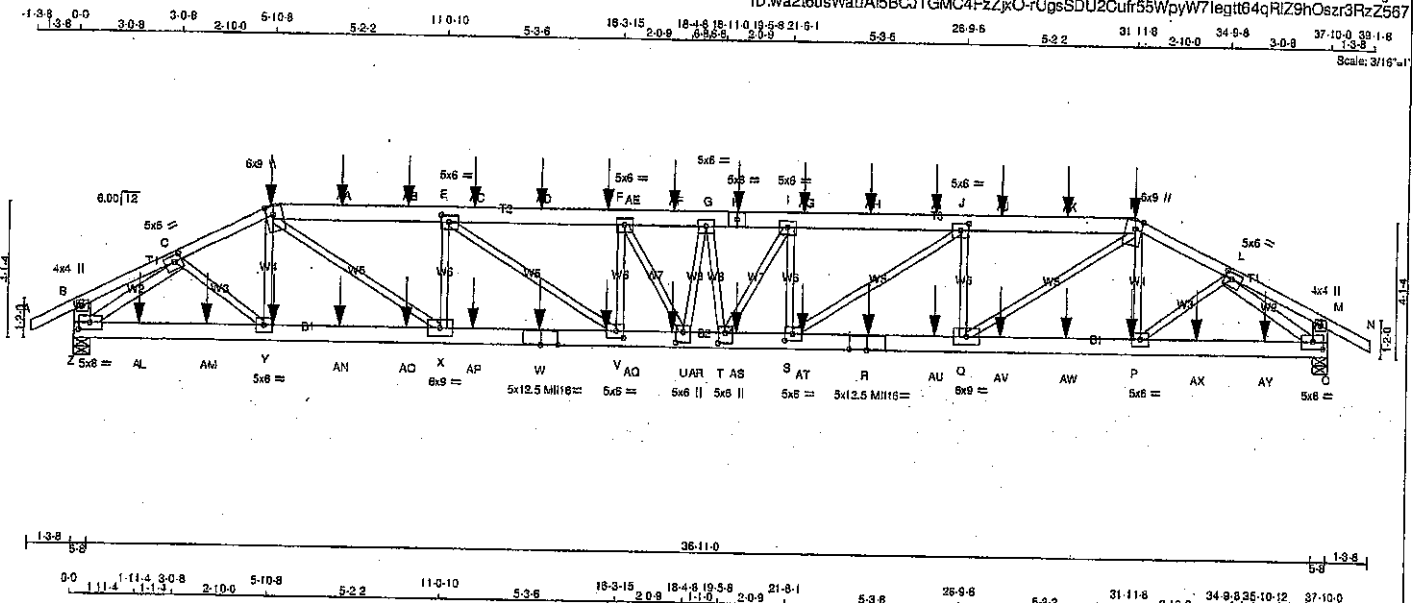
NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108202 212

JOB NAME 417455	TRUSS NAME T31	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:wa2i6usWauA15BCJTGMC4FzZjK0-rUgsSDU2Cuf955WpyW7legtt64qRIZ9hOszr3RzZ567



CHORDS	SIZE	DRY	NO.2	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - N	2x4	DRY	No.2	SPF
Z - B	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
Z - W	2x6	DRY	2100F 1.8E	SPF
W - R	2x6	DRY	2100F 1.8E	SPF
R - O	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
Z - C	2x4	DRY	No.2	SPF
L - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D	12	SIDE (61.0)
K-N	12	SIDE (61.0)
D-H	12	SIDE (61.0)
H-K	12	SIDE (61.0)
Z-B	12	TOP
O-M	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
Z-W	12	SIDE (197.8)
W-R	12	SIDE (0.0)
R-O	12	SIDE (197.8)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP/LIFT	IN-SX
O	3568	0	0	5-8
Z	3568	0	0	5-8

UNFACTORED REACTIONS

1ST LOOSE COMBINED	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	2524	1655	0	0	0	869	0
O	2524	1655	0	0	0	869	0
Z	2524	1655	0	0	0	869	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, Z

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED LENGTH (FT)	MAX. FACTORED CS (LBS)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LBS)
FR-TO						FR-TO			
A-B	0/28	-91.8	-91.8	0.07 (1)	10.00	Y-D	-333/12	0.04 (1)	
B-C	0/7	-91.8	-91.8	0.06 (1)	10.00	P-K	-333/12	0.04 (1)	
C-D	-5511/0	-91.8	-91.8	0.18 (1)	4.03	V-F	-743/0	0.09 (1)	
D-AA	-7706/0	-91.8	-91.8	0.24 (1)	4.19	S-I	-743/0	0.09 (1)	
AA-AB	-7706/0	-91.8	-91.8	0.24 (1)	4.19	F-U	0/139	0.02 (1)	
AB-E	-7706/0	-91.8	-91.8	0.24 (1)	4.19	U-G	-114/0	0.01 (1)	
E-AC	-8995/0	-91.8	-91.8	0.27 (1)	3.89	G-T	-118/0	0.01 (1)	
AC-AD	-8995/0	-91.8	-91.8	0.27 (1)	3.89	T-I	0/151	0.02 (1)	
AD-AE	-8995/0	-91.8	-91.8	0.27 (1)	3.89	Z-C	-5371/0	0.46 (1)	
AE-F	-8995/0	-91.8	-91.8	0.27 (1)	3.89	C-Y	0/661	0.11 (1)	
F-AF	-9062/0	-91.8	-91.8	0.19 (1)	3.96	P-L	0/661	0.11 (1)	
AF-G	-9062/0	-91.8	-91.8	0.19 (1)	3.96	E-V	-5371/0	0.46 (1)	
G-H	-9066/0	-91.8	-91.8	0.18 (1)	3.96	D-X	0/1566	0.19 (1)	
H-I	-9066/0	-91.8	-91.8	0.18 (1)	3.96	X-E	-1802/0	0.42 (1)	
I-AG	-8983/0	-91.8	-91.8	0.27 (1)	3.89	Q-K	0/3376	0.22 (1)	
AG-AH	-8983/0	-91.8	-91.8	0.27 (1)	3.89	S-J	0/1565	0.19 (1)	
AH-AI	-8983/0	-91.8	-91.8	0.27 (1)	3.89	Q-J	-1802/0	0.22 (1)	
AI-J	-8983/0	-91.8	-91.8	0.27 (1)	3.89				
J-AJ	-7706/0	-91.8	-91.8	0.24 (1)	4.19				
AJ-AK	-7706/0	-91.8	-91.8	0.24 (1)	4.19				
AK-K	-7706/0	-91.8	-91.8	0.24 (1)	4.19				
K-L	-5510/0	-91.8	-91.8	0.16 (1)	4.03				
L-M	0/7	-91.8	-91.8	0.08 (1)	10.00				
M-N	0/28	-91.8	-91.8	0.07 (1)	10.00				
Z-B	-245/0	0.0	0.0	0.01 (1)	7.81				
O-M	-245/0	0.0	0.0	0.01 (1)	7.81				
Z-AL	0/4272	-18.5	-18.5	0.13 (1)	10.00				
AL-AM	0/4272	-18.5	-18.5	0.13 (1)	10.00				
AM-Y	0/4272	-18.5	-18.5	0.13 (1)	10.00				
Y-AN	0/4954	-18.5	-18.5	0.15 (1)	10.00				
AN-AO	0/4954	-18.5	-18.5	0.15 (1)	10.00				
AO-X	0/4954	-18.5	-18.5	0.15 (1)	10.00				
X-AP	0/7706	-18.5	-18.5	0.15 (1)	10.00				
AP-W	0/7706	-18.5	-18.5	0.24 (1)	10.00				
W-AQ	0/7706	-18.5	-18.5	0.24 (1)	10.00				
AQ-V	0/7706	-18.5	-18.5	0.24 (1)	10.00				
V-AR	0/8995	-18.5	-18.5	0.27 (1)	10.00				
AR-U	0/8995	-18.5	-18.5	0.27 (1)	10.00				
U-T	0/9062	-18.5	-18.5	0.27 (1)	10.00				
T-S	0/8993	-18.5	-18.5	0.27 (1)	10.00				
AS-S	0/8993	-18.5	-18.5	0.27 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
ALLOWABLE DEFL.(TL) = L360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.43")

CSI: TC=0.27/1.00 (I-J:1), BC=0.27/1.00 (T-U:1),

WB=0.46/1.00 (C-Z:1), SS=0.14/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(ORY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1673
ML16 438 302 2547 1256 4283 1516

PLATE PLACEMENT TOL.= 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (C) (INPUT = 0.90)

JSI METAL= 0.64 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108203 /K

CONTINUED ON PAGE 2

JOB NAME 417455	TRUSS NAME T31	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 09:58:14 2021 Page 2
ID:wa216usWauA15BCJTGMC4FzZkQ-UqsSDU2CufR55WpyW71ectt64cRIZ9hQsr3RzZ567

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	2.50
D	TTWW+m	MT20	8.0	9.0	Edge	2.50
E	TMWW-t	MT20	5.0	6.0	2.50	2.75
F, G, I						
F	TMWW-t	MT20	5.0	6.0		
H	TS-t	MT20	5.0	6.0		
J	TMWW-t	MT20	5.0	6.0	2.50	2.75
K	TTWW+m	MT20	8.0	9.0	Edge	2.50
L	TMWW-t	MT20	5.0	6.0	2.25	2.50
M	TMV+p	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	8.0	2.50	3.75
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	8.0	9.0		
R	BS-t	MI16	5.0	12.5	0.25	6.25
S	BMWW-t	MT20	5.0	6.0	2.50	2.75
T	BMWW-t	MT20	5.0	6.0	3.75	2.50
U	BMWW-t	MT20	5.0	6.0	3.75	2.50
V	BMWW-t	MT20	5.0	8.0	2.50	2.75
W	BS-t	MI16	5.0	12.5	0.25	6.25
X	BMWW-t	MT20	6.0	9.0		
Y	BMWW-t	MT20	5.0	6.0		
Z	BMVW1-t	MT20	5.0	8.0	2.50	3.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)
FR-TO					FR-TO		
S-AT	0/7705	-18.5	-18.5	0.24 (1)	10.00		
AT-R	0/7705	-18.5	-18.5	0.24 (1)	10.00		
R-AU	0/7705	-18.5	-18.5	0.24 (1)	10.00		
AU-Q	0/7705	-18.5	-18.5	0.24 (1)	10.00		
Q-AV	0/4954	-18.5	-18.5	0.16 (1)	10.00		
AV-AW	0/4954	-18.5	-18.5	0.16 (1)	10.00		
AW-P	0/4954	-18.5	-18.5	0.16 (1)	10.00		
P-AX	0/4272	-18.5	-18.5	0.13 (1)	10.00		
AX-AY	0/4272	-18.5	-18.5	0.13 (1)	10.00		
AY-O	0/4272	-18.5	-18.5	0.13 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-318	-318		FRONT	VERT	TOTAL		C1
H	19-10-12	-76	-76		FRONT	VERT	TOTAL		C1
K	31-11-8	-318	-318		FRONT	VERT	TOTAL		C1
P	31-10-12	-21	-21		FRONT	VERT	TOTAL		C1
R	23-10-12	-21	-21		FRONT	VERT	TOTAL		C1
W	13-11-4	-21	-21		FRONT	VERT	TOTAL		C1
Y	5-11-4	-21	-21		FRONT	VERT	TOTAL		C1
AA	7-11-4	-76	-76		FRONT	VERT	TOTAL		C1
AB	9-11-4	-76	-76		FRONT	VERT	TOTAL		C1
AC	11-11-4	-76	-76		FRONT	VERT	TOTAL		C1
AD	13-11-4	-76	-76		FRONT	VERT	TOTAL		C1
AE	15-11-4	-76	-76		FRONT	VERT	TOTAL		C1
AF	17-11-4	-76	-76		FRONT	VERT	TOTAL		C1
AG	21-10-12	-76	-76		FRONT	VERT	TOTAL		C1
AH	23-10-12	-76	-76		FRONT	VERT	TOTAL		C1
AI	25-10-12	-76	-76		FRONT	VERT	TOTAL		C1
AJ	27-10-12	-76	-76		FRONT	VERT	TOTAL		C1
AK	29-10-12	-76	-76		FRONT	VERT	TOTAL		C1
AL	1-11-4	-20	-20		FRONT	VERT	TOTAL		C1
AM	3-11-4	-21	-21		FRONT	VERT	TOTAL		C1
AN	7-11-4	-21	-21		FRONT	VERT	TOTAL		C1
AO	9-11-4	-21	-21		FRONT	VERT	TOTAL		C1
AP	11-11-4	-21	-21		FRONT	VERT	TOTAL		C1
AQ	15-11-4	-21	-21		FRONT	VERT	TOTAL		C1
AR	17-11-4	-21	-21		FRONT	VERT	TOTAL		C1
AS	19-10-12	-21	-21		FRONT	VERT	TOTAL		C1
AT	21-10-12	-21	-21		FRONT	VERT	TOTAL		C1
AU	25-10-12	-21	-21		FRONT	VERT	TOTAL		C1
AV	27-10-12	-21	-21		FRONT	VERT	TOTAL		C1
AW	29-10-12	-21	-21		FRONT	VERT	TOTAL		C1
AX	33-10-12	-21	-21		FRONT	VERT	TOTAL		C1
AY	35-10-12	-20	-20		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

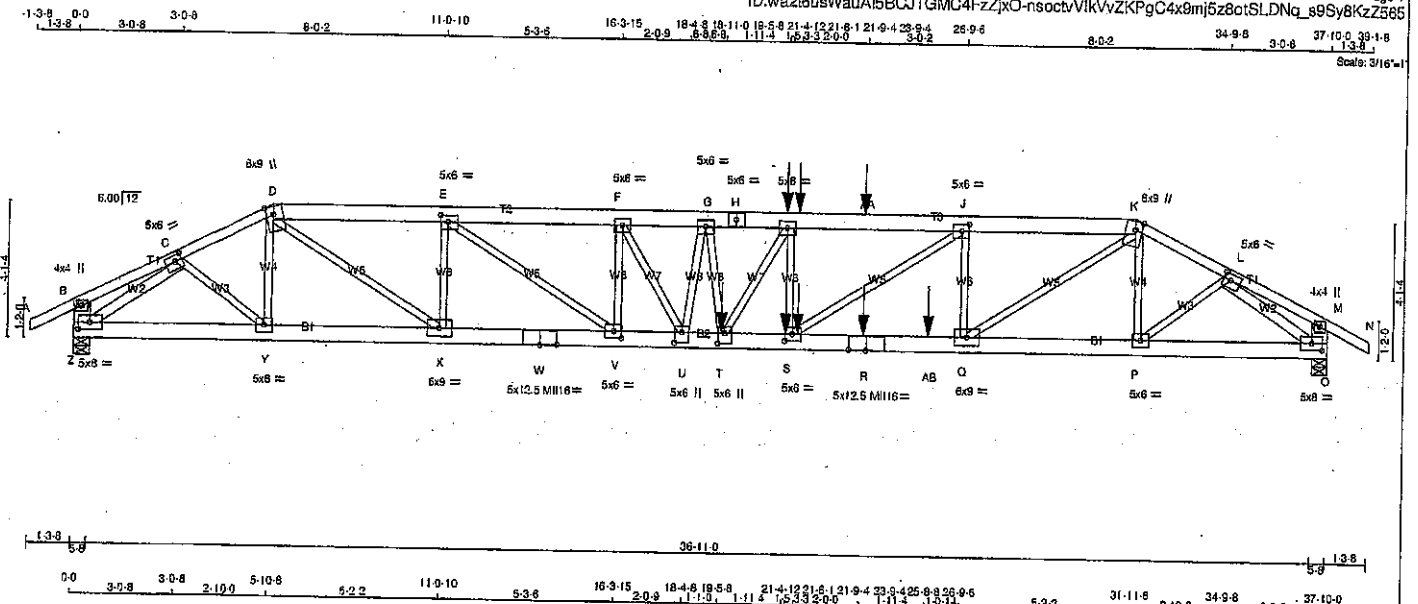


Structural component only
DWG# T-2108203 *ML*

JOB NAME 417455	TRUSS NAME T31Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mittek Industries, Inc. Sat Mar 20 09:58:16 2021 Page 1
ID:wa2t6usWauA15BCJTGM04FzZjXo-nsoctvVlkVzKpGc4x9mj5z8otSLDNq_s9Sy8KzZ585



LUMBER	N.L. & A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - H	2x6	DRY	No.2	SPF	
H - K	2x6	DRY	No.2	SPF	
K - N	2x6	DRY	No.2	SPF	
Z - B	2x6	DRY	No.2	SPF	
O - M	2x6	DRY	No.2	SPF	
Z - W	2x6	DRY	2100F 1.8E	SPF	
W - R	2x6	DRY	2100F 1.8E	SPF	
R - O	2x6	DRY	2100F 1.8E	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
L - C	2x4	DRY	No.2	SPF	
L - O	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1 12	TOP	
K-N 1 12	TOP	
D-H 2 12	TOP	
H-K 2 12	TOP	
Z-B 2 12	TOP	
O-M 2 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
Z-W 2 12	TOP	
W-R 2 12	TOP	
R-O 2 12	TOP	
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6	TOP	
G-T 1 3	TOP	
U-G 1 3	TOP	
2x4 1 6	TOP	

NAILED TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
O	4790 0	4790 0	0 0	5-8 5-8
Z	4120 0	4120 0	0 0	5-8 5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
O	3377	2278 / 0	0 / 0	0 / 0	1099 / 0	0 / 0
Z	2906	1952 / 0	0 / 0	0 / 0	954 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, Z

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MEMB.	FORCE (LBS)
FR-TO			FR-TO	
A-B 0 / 28	-91.8	-91.8 0.07 (1)	Y-D -534 / 0	0.07 (1)
B-C 0 / 6	-91.8	-91.8 0.05 (1)	P-K -788 / 0	0.10 (1)
C-D -6528 / 0	-91.8	-91.8 0.20 (1)	V-F -2412 / 0	0.29 (1)
D-E -10286 / 0	-91.8	-91.8 0.28 (1)	S-I -970 / 0	0.12 (1)
E-F -13805 / 0	-91.8	-91.8 0.44 (1)	F-U 0 / 2301	0.28 (1)
F-G -14919 / 0	-91.8	-91.8 0.45 (1)	U-G -1349 / 0	0.18 (1)
G-H -15324 / 0	-91.8	-91.8 0.47 (1)	G-T 0 / 1248	0.15 (1)
H-I -15324 / 0	-91.8	-91.8 0.47 (1)	T-I 0 / 322	0.04 (1)
I-AA -15169 / 0	-91.8	-91.8 0.58 (1)	Z-C -6316 / 0	0.54 (1)
AA-J -15169 / 0	-91.8	-91.8 0.58 (1)	C-Y 0 / 1071	0.43 (1)
J-K -12412 / 0	-91.8	-91.8 0.37 (1)	P-L 0 / 1309	0.16 (1)
K-L -7687 / 0	-91.8	-91.8 0.28 (1)	L-O -7393 / 0	0.53 (1)
L-M 0 / 4	-91.8	-91.8 0.08 (1)	E-V 0 / 4278	0.53 (1)
M-N 0 / 28	-91.8	-91.8 0.07 (1)	D-X 0 / 5412	0.36 (1)
Z-B -247 / 0	0.0	0.0 0.01 (1)	X-E -3044 / 0	0.36 (1)
O-M -250 / 0	0.0	0.0 0.01 (1)	Q-K 0 / 6731	0.39 (1)
			S-J 0 / 3951	0.41 (1)
			Q-J -2563 / 0	0.31 (1)
Z-Y 0 / 5024	-18.5	-18.5 0.14 (1)		
Y-X 0 / 5875	-18.5	-18.5 0.17 (1)		
X-W 0 / 10286	-18.5	-18.5 0.31 (1)		
W-V 0 / 10286	-18.5	-18.5 0.31 (1)		
V-U 0 / 13806	-18.5	-18.5 0.40 (1)		
U-T 0 / 15155	-18.5	-18.5 0.57 (1)		
T-S 0 / 15169	-18.5	-18.5 0.57 (1)		
S-R 0 / 12411	-18.5	-18.5 0.53 (1)		
R-AB 0 / 12411	-18.5	-18.5 0.53 (1)		
AB-Q 0 / 12411	-18.5	-18.5 0.53 (1)		
Q-P 0 / 6925	-18.5	-18.5 0.24 (1)		
P-O 0 / 5980	-18.5	-18.5 0.17 (1)		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	21-4-12	-76	-76		BACK	VERT	TOTAL		C1
I	21-9-4	-76	-76		BACK	VERT	TOTAL		C1
I	23-9-4	-21	-21		BACK	VERT	TOTAL		C1
I	21-4-12	-21	-21		BACK	VERT	TOTAL		C1
I	21-9-4	-21	-21		BACK	VERT	TOTAL		C1
I	19-5-8	-1858	-1858		BACK	VERT	TOTAL		C1
AA	23-9-4	-76	-76		BACK	VERT	TOTAL		C1
AB	25-8-8	-1011	-1011		BACK	VERT	TOTAL		C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/959 (0.38")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/682 (0.69")

CSI: TC=0.59/1.00 (J-I), BC=0.57/1.00 (S-T), WB=0.83/1.00 (K-C), SS=0.41/1.00 (G-S)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE GRIP (DRY) SHEAR SECTION

PLATE GRIP (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873
MI16 438 302 2547 1256 4283 1816

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (L) (INPUT = 0.90)

JSI METAL = 0.89 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108204

CONTINUED ON PAGE 2

JOB NAME 417455	TRUSS NAME T31Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 09:58:16 2021 Page 2 ID:wa2i6usWauA15BCJTGMC4FzZixO-nsocivVikVvZKPaC4x9mI5z8otSLDNq_s9Sy8KzZ565	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	2.50
D	TTWW+m	MT20	6.0	9.0	Edge	2.50
E	TMWW-t	MT20	5.0	6.0	2.50	2.75
F, G, I						
F	TMWW-t	MT20	5.0	6.0		
H	TS-t	MT20	5.0	6.0		
J	TMWW-t	MT20	5.0	6.0	2.50	2.75
K	TTWW+m	MT20	6.0	9.0	Edge	2.50
L	TMWW-t	MT20	5.0	6.0	2.25	2.50
M	TMV+p	MT20	4.0	4.0		
O	BMVW-t	MT20	5.0	6.0	2.50	3.75
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	6.0	9.0		
R	BS-t	MI16	5.0	12.5	0.25	6.25
S	BMWW-t	MT20	5.0	6.0	2.50	2.75
T	BMWW-t	MT20	5.0	6.0	3.75	2.50
U	BMWW-t	MT20	5.0	6.0	3.75	2.50
V	BMWW-t	MT20	5.0	6.0	2.50	2.75
W	BS-t	MI16	5.0	12.5	0.25	6.25
X	BMWW-t	MT20	6.0	9.0		
Y	BMWW-t	MT20	5.0	6.0		
Z	BMVW-t	MT20	5.0	6.0	2.50	3.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

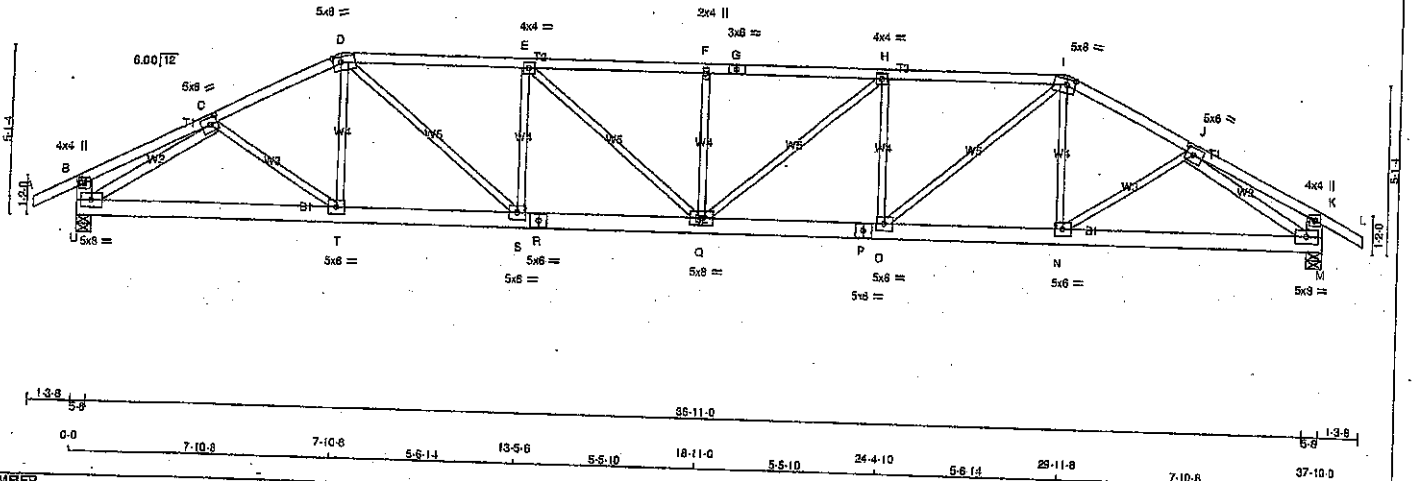
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2108204 *W*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417443	T32	2	1	TRUSS DESC.		
Tamarack Roof Truss, Burlington						

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Sat Mar 20 08:52:43 2021 Page 1
ID:wa216usWauA15BCJTGM4FzZjxO-RDnQ4vGz5Cc_VJAQ5TqZznT7wCcA4TmEJbozZ63Y
Scale: 3/16"=1'



TOTAL WEIGHT = 2 X 176 = 352 lb

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - G	2x4 DRY	No.2	SPF		
G - I	2x4 DRY	No.2	SPF		
I - L	2x4 DRY	No.2	SPF		
L - M	2x6 DRY	No.2	SPF		
M - K	2x6 DRY	No.2	SPF		
K - R	2x6 DRY	No.2	SPF		
R - P	2x6 DRY	No.2	SPF		
P - M	2x6 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					
U - C	2x4 DRY	No.2	SPF		
J - M	2x4 DRY	No.2	SPF		

DRY, SEASONED LUMBER.

PLATES (table is in inches)

PLATE	TYPE	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0	
C	TMVW+1	MT20	5.0	6.0	2.50 2.75
D	TTVW+m	MT20	5.0	8.0	2.00 3.00
E	TMVW+1	MT20	4.0	4.0	
F	TMVW+1	MT20	2.0	4.0	
G	TS-1	MT20	3.0	6.0	
H	TMVW+1	MT20	4.0	4.0	
I	TTVW+m	MT20	5.0	8.0	2.00 3.00
J	TMVW+1	MT20	5.0	6.0	2.50 2.75
K	TMV+p	MT20	4.0	4.0	
M	BMVW+1	MT20	5.0	8.0	
N, O, S, T					
N	BMVW+1	MT20	5.0	6.0	
P	BS-1	MT20	5.0	6.0	
Q	BMVW+1	MT20	5.0	8.0	
R	BS-1	MT20	5.0	6.0	
U	BMVW+1	MT20	5.0	8.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
VERT	HORIZ	UPLIFT	IN-SX
U	2210 0	2210 0	5-8
M	2210 0	2210 0	5-8

UNFACTORED REACTIONS

1ST LOOSE	MAX. MIN. COMPONENT REACTIONS
COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
U	1561 1037.0 0.0 0.0 0.0 523.0 0.0
M	1561 1037.0 0.0 0.0 0.0 523.0 0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF) CSI (LC)	MEMB.	FORCE (LBS) MAX CSI (LC)
FR-TO		FROM TO	FR-TO	
A-B	0.128	-91.8 -91.8 0.12 (1)	C-T	0.195 0.03 (4)
B-C	0.116	-91.8 -91.8 0.20 (1)	T-D	0.116 0.04 (4)
C-D	-3116.0	-91.8 -91.8 0.35 (1)	N-J	0.116 0.04 (4)
D-E	-3881.0	-91.8 -91.8 0.76 (1)	N-J	0.195 0.03 (4)
E-F	-4201.0	-91.8 -91.8 0.81 (1)	U-C	-3286.0 0.83 (1)
F-G	-4201.0	-91.8 -91.8 0.81 (1)	J-M	-3286.0 0.83 (1)
G-H	-3881.0	-91.8 -91.8 0.76 (1)	D-S	0.1453 0.33 (1)
H-I	-3116.0	-91.8 -91.8 0.20 (1)	S-E	-849.0 0.30 (1)
I-J	0.116	-91.8 -91.8 0.35 (1)	C-H	-849.0 0.30 (1)
J-K	0.128	-91.8 -91.8 0.12 (1)	Q-H	0.1424 0.10 (1)
K-L	-270.0	0.0 0.0 0.02 (1)	E-Q	0.1424 0.10 (1)
U-B	-270.0	0.0 0.0 0.02 (1)	Q-F	-481.0 0.17 (1)
M-K	-270.0	0.0 0.0 0.02 (1)		
U-T	0.12898	-18.5 -18.5 0.39 (1)		
T-S	0.12772	-18.5 -18.5 0.38 (1)		
S-R	0.13881	-18.5 -18.5 0.52 (1)		
R-Q	0.13881	-18.5 -18.5 0.52 (1)		
Q-P	0.13881	-18.5 -18.5 0.52 (1)		
P-O	0.13881	-18.5 -18.5 0.52 (1)		
O-N	0.12772	-18.5 -18.5 0.38 (1)		
N-M	0.12898	-18.5 -18.5 0.39 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.25")
CALCULATED VERT. DEFL.(LL) = L/969 (0.24")
ALLOWABLE DEFL.(TL) = L/360 (1.25")
CALCULATED VERT. DEFL.(TL) = L/969 (0.45")

CSH: TC=0.81/1.00 (F-H:1), BC=0.52/1.00 (C-S:1), WB=0.83/1.00 (C-U:1), SSI=0.24/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

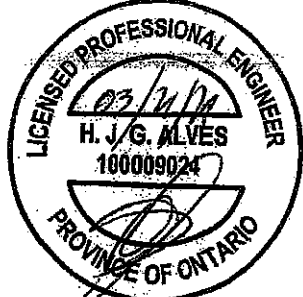
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (J) (INPUT = 0.80)

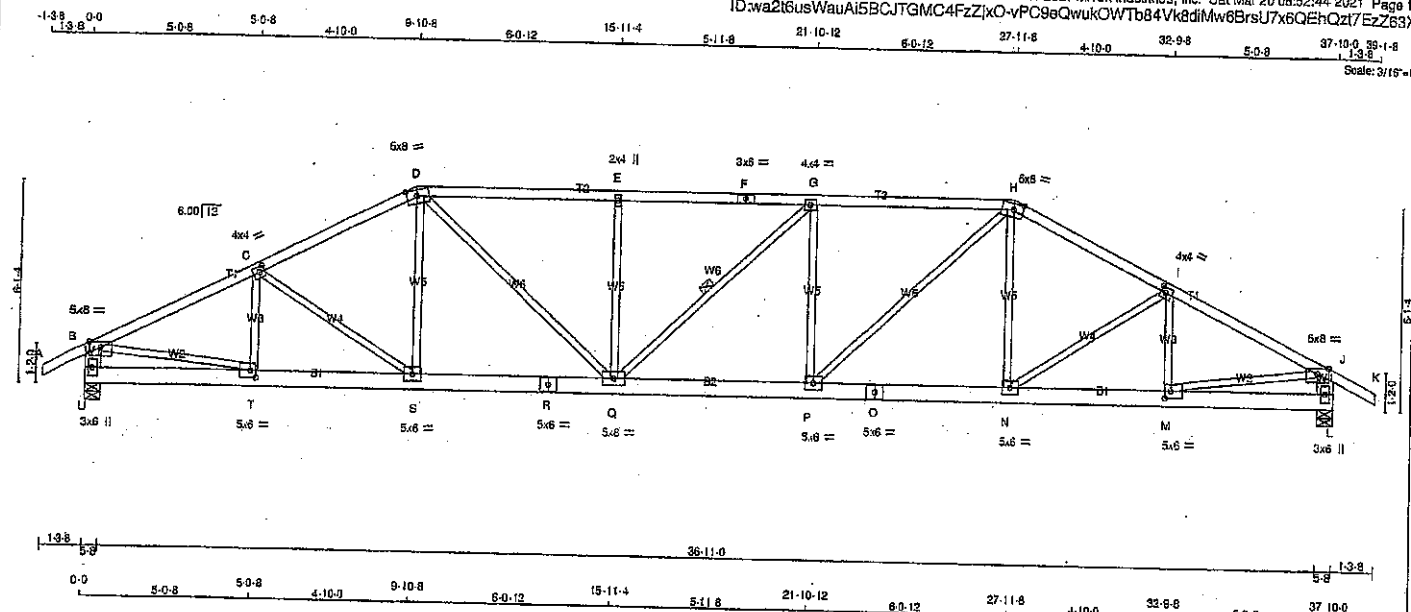
JSI METAL = 0.75 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108140

JOB NAME 417443	TRUSS NAME T33	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 5 Jan 21 2021 MTEK Industries, Inc. Sat Mar 20 08:52:44 2021 Page 1
ID:wa2t6usWauA15BCJTGM4FZZXO-VPC9eQwukOWTb84VkdMw6Br5U7x6QEHQZ17Ez2B3X



LUMBER	N. L. G. A. RULES	SIZE	LUMBER
CHORDS			
A - D	2x4	DRY	No.2
E - H	2x4	DRY	No.2
I - K	2x4	DRY	No.2
L - J	2x8	DRY	No.2
M - R	2x8	DRY	No.2
S - O	2x6	DRY	No.2
T - L	2x8	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT
SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMVW-p	MT20	5.0	8.0	Edge
C TMVW-t	MT20	4.0	4.0	2.00 1.75
D TMVW-m	MT20	5.0	8.0	2.25 3.50
E TMVW-w	MT20	2.0	4.0	
F TS-t	MT20	3.0	6.0	
G TMVW-x	MT20	4.0	4.0	
H TMVW-m	MT20	5.0	8.0	2.25 3.50
I TMVW-t	MT20	4.0	4.0	2.00 1.75
J TMVW-p	MT20	5.0	8.0	Edge
L BMV1-p	MT20	3.0	6.0	
M BMVW-t	MT20	5.0	6.0	2.50 2.00
N, P, S				
O BMVW-t	MT20	5.0	6.0	
Q BS-t	MT20	5.0	6.0	
R BMVW-t	MT20	5.0	6.0	
T BMV1-p	MT20	5.0	6.0	2.50 2.00
U BMV1-p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX		
U	2210	0	2210	0	0	5-8	5-8		
L	2210	0	2210	0	0	5-8	5-8		

UNFACTORED REACTIONS									
1ST LCASE		MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SC		
U	1561	1037 0	0 0	0 0	0 0	523 0	0		
L	1561	1037 0	0 0	0 0	0 0	523 0	0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0' 28	-91.8	-91.8	0.12 (1)	10.00	T-C	-389 / 0	0.08 (1)
B-C	-3125 / 0	-91.8	-91.8	0.42 (1)	3.62	C-S	-175 / 0	0.10 (1)
C-D	-3011 / 0	-91.8	-91.8	0.40 (1)	3.70	S-D	0 / 244	0.05 (1)
D-E	-3410 / 0	-91.8	-91.8	0.59 (1)	3.30	D-Q	0 / 1012	0.23 (1)
E-F	-3410 / 0	-91.8	-91.8	0.55 (1)	3.38	Q-E	-597 / 0	0.33 (1)
F-G	-3410 / 0	-91.8	-91.8	0.59 (1)	3.30	G-Q	-8 / 0	0.00 (1)
G-H	-3418 / 0	-91.8	-91.8	0.40 (1)	3.70	P-G	-595 / 0	0.32 (1)
H-I	-3009 / 0	-91.8	-91.8	0.42 (1)	3.62	P-H	0 / 1022	0.23 (1)
I-J	-3127 / 0	-91.8	-91.8	0.12 (1)	10.00	N-I	-177 / 0	0.11 (1)
J-K	0' 28	-91.8	-91.8	0.12 (1)	10.00	M-I	-387 / 0	0.09 (1)
U-B	-2147 / 0	0.0	0.0	0.14 (1)	6.94	B-T	0 / 2848	0.84 (1)
L-J	-2147 / 0	0.0	0.0	0.14 (1)	6.94	M-J	0 / 2848	0.84 (1)
U-T	0 / 0	-18.5	-18.5	0.07 (1)	10.00			
T-S	0' 2816	-18.5	-18.5	0.39 (1)	10.00			
S-R	0' 2675	-18.5	-18.5	0.38 (1)	10.00			
R-Q	0' 2675	-18.5	-18.5	0.38 (1)	10.00			
Q-P	0' 3417	-18.5	-18.5	0.46 (1)	10.00			
P-O	0' 2673	-18.5	-18.5	0.38 (1)	10.00			
O-N	0' 2573	-18.5	-18.5	0.38 (1)	10.00			
N-M	0' 2816	-18.5	-18.5	0.38 (1)	10.00			
M-L	0 / 0	-18.5	-18.5	0.07 (1)	10.00			



Structural component only
DWG# T-2108141

TOTAL WEIGHT = 2 X 177 = 355 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/960$ (1.28")
CALCULATED VERT. DEFL. (LL) = $1/999$ (0.20")
ALLOWABLE DEFL. (TL) = $L/360$ (1.28")
CALCULATED VERT. DEFL. (TL) = $1/999$ (0.37")

CS1: TC=0.59/1.00 (G-H:1), BC=0.48/1.00 (P-Q:1), WB=0.64/1.00 (J-K:1), SS=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEEPS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

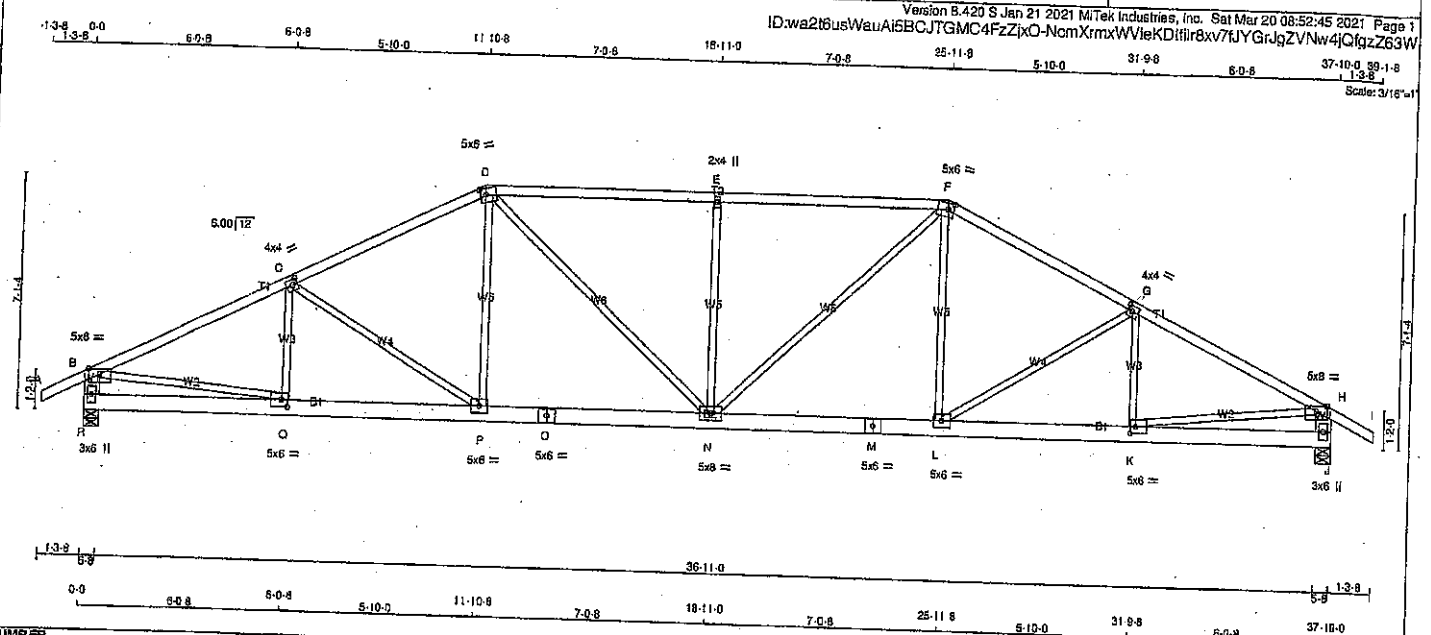
NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.87 (H) (INPUT = 0.90)
JSI METAL=0.64 (M) (INPUT = 1.00)

JOB NAME 417443	TRUSS NAME T34	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - B	2x4	DRY	No.2	SPF	
J - H	2x6	DRY	No.2	SPF	
R - O	2x6	DRY	No.2	SPF	
O - M	2x6	DRY	No.2	SPF	
M - J	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	

EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	Edge		
C TMVW-w	MT20	4.0	4.0	2.00	1.75	
D TTWW-m	MT20	5.0	6.0	2.25	2.00	
E TMVW-w	MT20	2.0	4.0			
F TTWW-m	MT20	5.0	8.0	2.25	2.00	
G TMVW-w	MT20	4.0	4.0	2.00	1.75	
H TMVW-p	MT20	5.0	8.0	Edge		
J BMV1-p	MT20	3.0	6.0			
K BMVW-l	MT20	5.0	6.0	2.50	2.00	
L BMVW-l	MT20	5.0	6.0			
M BS-t	MT20	5.0	6.0			
N BMVW-l	MT20	5.0	8.0			
O BS-t	MT20	5.0	6.0			
P BMVW-l	MT20	5.0	6.0			
Q BMVW-l	MT20	5.0	6.0	2.50	2.00	
R BMV1-p	MT20	3.0	6.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
JT	2210	0	2210	0	5-8	5-8
R	2210	0	2210	0	5-8	5-8

UNFACTORED REACTIONS	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
JT	1581	1037.0	0.0	0.0	0.0	523.0	0.0
R	1581	1037.0	0.0	0.0	0.0	523.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.		FORCE (LBS)		VERT. LOAD		LGT MAX		MAX. UNBRAC		MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
FR-TO		FROM TO		PLF	CSI (LC)	PLF	CSI (LC)	LENGTH	FR-TO			FROM TO			
A-B	0.28	-91.8	-91.8	0.12 (1)	10.00	Q-C	-290.0	0.07 (1)							
B-C	-3187.0	-91.8	-91.8	0.82 (1)	3.39	C-P	-402.0	0.37 (1)							
C-D	-2887.0	-91.8	-91.8	0.56 (1)	3.61	P-D	0.369	0.08 (1)							
D-E	-3016.0	-91.8	-91.8	0.79 (1)	3.61	D-N	0.654	0.15 (1)							
E-F	-3018.0	-91.8	-91.8	0.78 (1)	3.17	N-E	-795.0	0.65 (1)							
F-G	-2887.0	-91.8	-91.8	0.58 (1)	3.61	N-F	0.654	0.15 (1)							
G-H	-3187.0	-91.8	-91.8	0.82 (1)	3.39	L-F	0.369	0.08 (1)							
H-I	0.28	-91.8	-91.8	0.12 (1)	10.00	L-G	-402.0	0.37 (1)							
R-B	-2145.0	0.0	0.0	0.14 (1)	6.95	K-G	-290.0	0.07 (1)							
J-H	-2145.0	0.0	0.0	0.14 (1)	6.95	B-Q	0.2899	0.65 (1)							
						K-H	0.2899	0.65 (1)							
R-Q	0.0	-18.5	-18.5	0.08 (4)	10.00										
Q-P	0.2875	-18.5	-18.5	0.40 (1)	10.00										
P-O	0.2542	-18.5	-18.5	0.36 (1)	10.00										
O-N	0.2542	-18.5	-18.5	0.36 (1)	10.00										
N-M	0.2542	-18.5	-18.5	0.36 (1)	10.00										
M-L	0.2542	-18.5	-18.5	0.36 (1)	10.00										
L-K	0.2875	-18.5	-18.5	0.40 (1)	10.00										
K-J	0.0	-18.5	-18.5	0.08 (4)	10.00										

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.26")
CALCULATED VERT. DEFL. (LL) = L/999 (0.17")
ALLOWABLE DEFL. (TL) = L/360 (1.26")
CALCULATED VERT. DEFL. (TL) = L/999 (0.32")

CSI: TC=0.79/1.00 (E-F:1), BC=0.40/1.00 (P-Q:1), WB=0.65/1.00 (B-Q:1), SSI=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650-371 1747 788 1987 1873

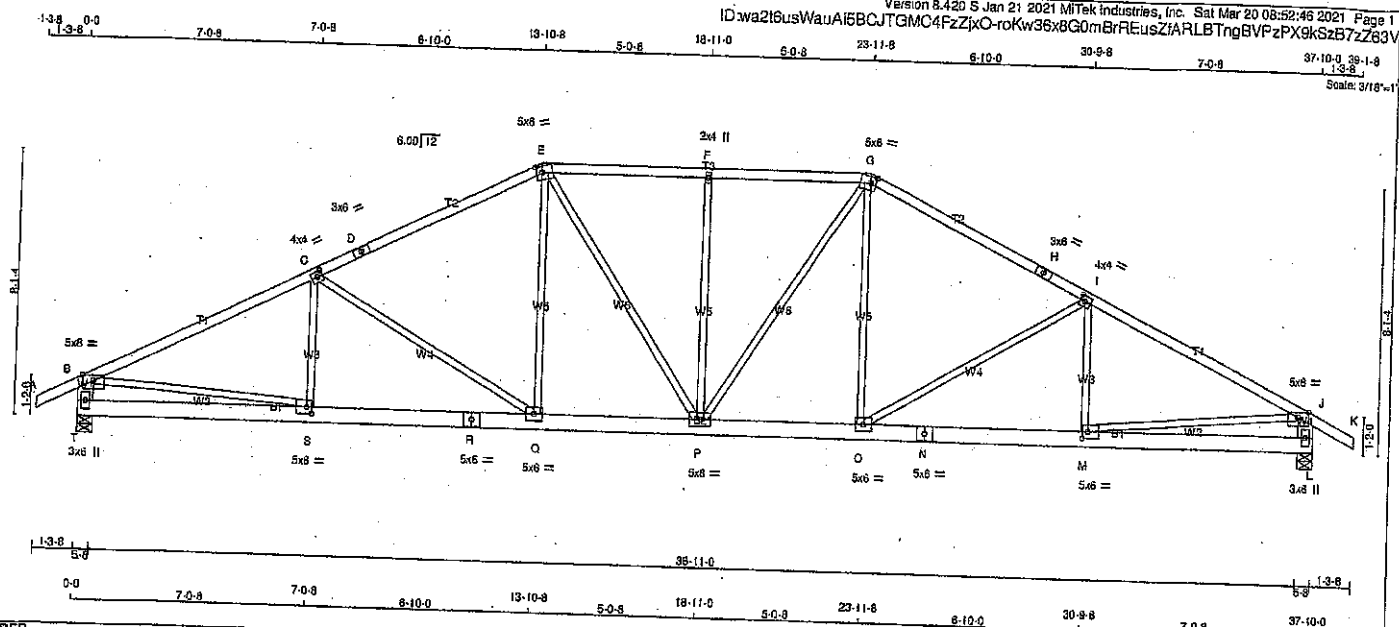
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (B) (INPUT = 0.90)
JSI METAL = 0.65 (Q) (INPUT = 1.00)



JOB NAME 417443 Tamarack Roof Truss, Burlington	TRUSS NAME T35	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
				TRUSS DESC.	



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - R	2x4	DRY	No.2
R - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	8.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMVW-w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TS-t	MT20	3.0	8.0		
I	TMVW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV-t	MT20	3.0	6.0		
M	BMVW-t	MT20	5.0	6.0	2.50	2.00
N	BS-t	MT20	5.0	8.0		
O	BMVW-t	MT20	5.0	6.0		
P	BMVW-t	MT20	5.0	8.0		
Q	BMVW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
S	BMVW-t	MT20	5.0	6.0	2.50	2.00
T	BMV-t	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
JT	2210	0	2210	0	5-8	5-8	
T	2210	0	2210	0	5-8	5-8	
L	2210	0	2210	0	5-8	5-8	

UNFACTORED REACTIONS

1ST LCASE		MAX-MIN. COMPONENT REACTIONS					
COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
JT	1581	1037.0	0.0	0.0	0.0	523.0	0.0
T	1581	1037.0	0.0	0.0	0.0	523.0	0.0
L	1581	1037.0	0.0	0.0	0.0	523.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.05 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0.28	-91.8 -91.8	0.12 (1)	10.00	S-C	-204.48	0.06 (1)
B-C	-3212.0	-91.8 -91.8	0.89 (1)	3.05	C-Q	-618.0	0.87 (1)
C-D	-2694.0	-91.8 -91.8	0.76 (1)	3.42	Q-E	0.464	0.10 (1)
D-E	-2694.0	-91.8 -91.8	0.76 (1)	3.42	E-P	0.371	0.08 (1)
E-F	-2588.0	-91.8 -91.8	0.38 (1)	3.96	P-F	-563.0	0.67 (1)
F-G	-2588.0	-91.8 -91.8	0.38 (1)	3.96	P-G	0.464	0.10 (1)
G-H	-2694.0	-91.8 -91.8	0.76 (1)	3.42	O-G	0.371	0.08 (1)
H-I	-2694.0	-91.8 -91.8	0.76 (1)	3.42	O-I	-618.0	0.87 (1)
I-J	-3212.0	-91.8 -91.8	0.89 (1)	3.05	M-I	-204.48	0.06 (1)
J-K	0.28	-91.8 -91.8	0.12 (1)	10.00	B-S	0.2921	0.68 (1)
T-B	-2143.0	0.0 0.0	0.14 (1)	6.95	M-J	0.2921	0.68 (1)
L-J	-2143.0	0.0 0.0	0.14 (1)	6.95			
T-S	0.0	-18.5 -18.5	0.10 (4)	10.00			
S-R	0.2903	-18.5 -18.5	0.40 (1)	10.00			
R-Q	0.2903	-18.5 -18.5	0.40 (1)	10.00			
Q-P	0.2384	-18.5 -18.5	0.33 (1)	10.00			
P-O	0.2384	-18.5 -18.5	0.33 (1)	10.00			
O-N	0.2903	-18.5 -18.5	0.40 (1)	10.00			
N-M	0.2903	-18.5 -18.5	0.40 (1)	10.00			
M-L	0.0	-18.5 -18.5	0.10 (4)	10.00			

TOTAL WEIGHT = 2 X 182 = 365 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 81.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.89/1.00 (I-I); BC=0.40/1.00 (M-O); WB=0.87/1.00 (O-I); SS=0.28/1.00 (I-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650 671 1747 788 1987 1873	

PLATE PLACEMENT TOL. = 0.250 inches

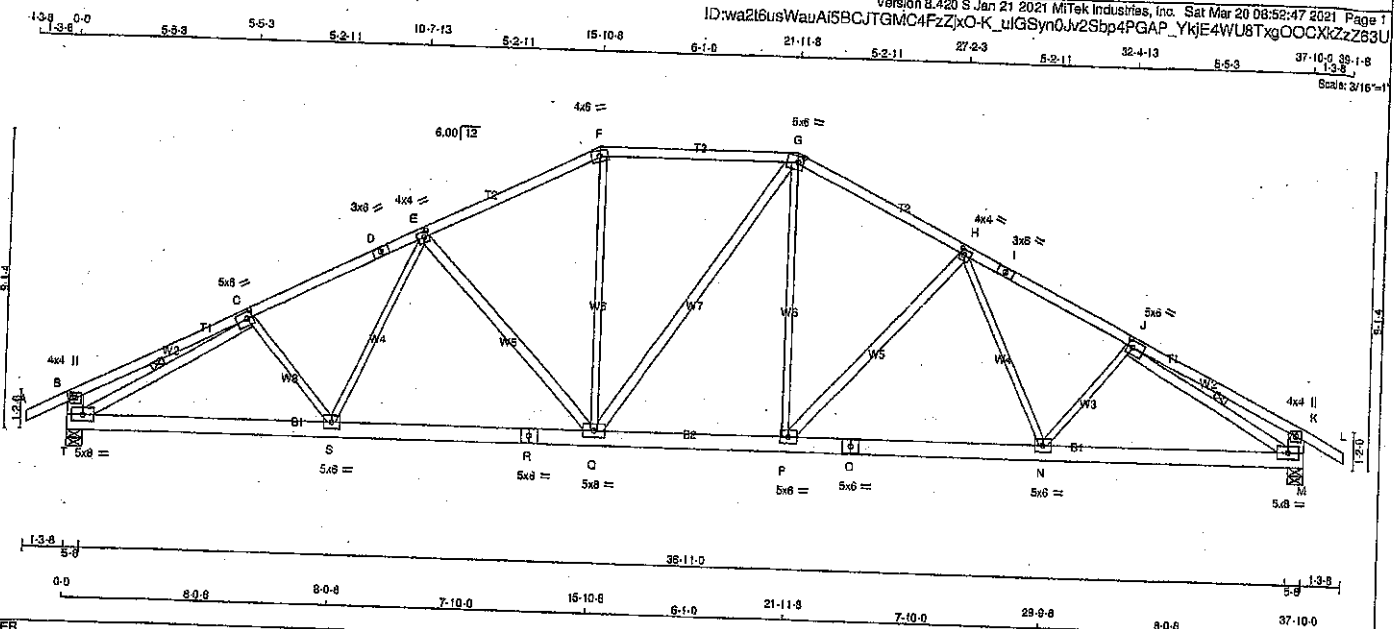
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.88 (8) (INPUT=0.90)
JSI METAL=0.68 (M) (INPUT=1.00)



Structural component only
DWG# T-2108143

JOB NAME 417443	TRUSS NAME T36	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tannerack Roof Truss, Burlington					



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - L	2x4	DRY	No.2	SPF	
L - B	2x6	DRY	No.2	SPF	
M - K	2x6	DRY	No.2	SPF	
T - R	2x6	DRY	No.2	SPF	
R - O	2x6	DRY	No.2	SPF	
O - M	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
E - Q	2x4	DRY	No.2	SPF	
O - G	2x4	DRY	No.2	SPF	
P - H	2x4	DRY	No.2	SPF	
T - C	2x4	DRY	No.2	SPF	
J - M	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+P	MT20	4.0	4.0		
C TMVW+T	MT20	5.0	6.0	2.50	2.00
D TS+T	MT20	3.0	6.0		
E TMVW+T	MT20	4.0	4.0	2.00	1.50
F TTW+M	MT20	4.0	6.0		
G TTW+M	MT20	5.0	6.0	2.25	2.00
H TMVW+T	MT20	4.0	4.0	2.00	1.50
I TS+T	MT20	3.0	6.0		
J TMVW+T	MT20	5.0	6.0	2.50	2.00
K TMV+P	MT20	4.0	4.0		
M BMVW+T	MT20	5.0	6.0		
N P.S.					
N BMVW+T	MT20	5.0	6.0		
O BS+T	MT20	5.0	6.0		
Q BMVW+T	MT20	5.0	6.0		
R BS+T	MT20	5.0	6.0		
T BMVW+T	MT20	5.0	6.0		

NOTES: (1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
T	2210	0	2210	0
M	2210	0	2210	0

UNFACTORED REACTIONS

1ST LOASE	MAX./MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE		
T	1561	1037	0	0	0
M	1561	1037	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-T, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
A-B	0.28								
B-C	0.20								
C-D	-3071.0								
D-E	-3071.0								
E-F	-2489.0								
F-G	-2213.0								
G-H	-2486.0								
H-I	-3072.0								
I-J	-3072.0								
J-K	0.20								
K-L	0.28								
T-B	-324.0								
M-K	-324.0								
T-S	0.2816								
S-R	0.2646								
R-Q	0.2646								
Q-P	0.2211								
P-O	0.2645								
O-N	0.2645								
N-M	0.2816								

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.52/1.00 (F-G:1), BC=0.42/1.00 (S-T:1), WB=0.69/1.00 (H-P:1), SS=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION:
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 550 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

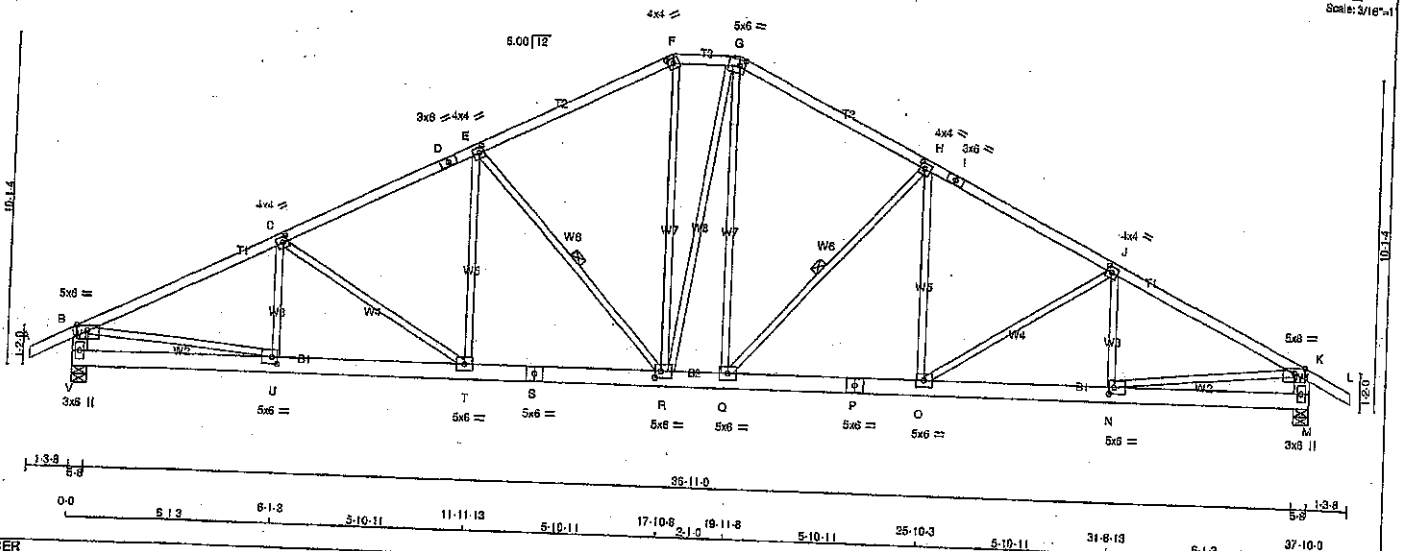
JSI GRIP = 0.85 (M) (INPUT = 0.90)
JSI METAL = 0.75 (J) (INPUT = 1.00)



Structural component only
DWG# T-2108144

JOB NAME 417443	TRUSS NAME T37	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MTEK Industries, Inc. Sat Mar 20 08:52:47 2021 Page 1
ID:wa2t6usWauAI5BCJTGMC4FzZxo-K_uIGSyn0Jv2Sbp4PGAP_Ykid4Wk8T1gOOCXkZz63U
Scale: 3/16"=1'



LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
V - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
V - S	2x6	DRY	No.2
S - P	2x8	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C, E, H, J						
O	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
F	TTW-t	MT20	4.0	4.0	2.00	1.75
G	TTW-m	MT20	5.0	6.0	2.25	2.00
I	TS-t	MT20	3.0	6.0		
K	TMVW-p	MT20	5.0	8.0	Edge	
M	BMV1-p	MT20	3.0	6.0		
N	BMVW-t	MT20	5.0	6.0	2.50	2.00
O, Q, T						
P	BS-t	MT20	5.0	6.0		
R	BMVW-t	MT20	5.0	6.0	2.50	2.00
S	BS-t	MT20	5.0	6.0		
U	BMVW-t	MT20	5.0	6.0	2.50	2.00
V	BMV1-p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ UPLIFT		
V	2210	0	2210	0	5-8	5-8
M	2210	0	2210	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		PERM.LIVE	WIND	DEAD	SOIL
	SNOW	LIVE	SNOW	LIVE				
V	1581	1037.0	0.0	0.0	0.0	0.0	523.0	0.0
M	1581	1037.0	0.0	0.0	0.0	0.0	523.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-R, H-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 MAX (PLF)		MAX. UNBRACED LENGTH FR-TO	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	
	FR-TO	CS1 (LC)	FROM	TO			FR-TO	CS1 (LC)
A-B	0/28		-91.8	-91.8	0.12 (1)	U-C	-267.7	0.07 (1)
B-C	-3177.0		-91.8	-91.8	0.62 (1)	C-T	-347.0	0.32 (1)
C-D	-2875.0		-91.8	-91.8	0.50 (1)	T-E	0/293	0.07 (1)
D-E	-2875.0		-91.8	-91.8	0.50 (1)	E-R	-823.0	0.46 (1)
E-F	-2250.0		-91.8	-91.8	0.47 (1)	R-F	0/703	0.16 (1)
F-G	-2034.0		-91.8	-91.8	0.47 (1)	Q-G	0/639	0.01 (1)
G-H	-2278.0		-91.8	-91.8	0.09 (1)	Q-H	-848.0	0.47 (1)
H-I	-2880.0		-91.8	-91.8	0.50 (1)	O-H	0/318	0.07 (1)
I-J	-2880.0		-91.8	-91.8	0.50 (1)	O-J	-340.0	0.31 (1)
J-K	-3175.0		-91.8	-91.8	0.12 (1)	N-J	-273.4	0.07 (1)
K-L	0/28		-91.8	-91.8	0.62 (1)	B-U	0/2885	0.65 (1)
V-B	-2148.0		0.0	0.0	0.14 (1)	N-K	0/2883	0.65 (1)
M-K	-2147.0		0.0	0.0	0.14 (1)			
V-U	0/0		-18.5	-18.5	0.08 (4)			
U-T	0/2862		-18.5	-18.5	0.40 (1)			
T-S	0/2571		-18.5	-18.5	0.37 (1)			
S-R	0/2571		-18.5	-18.5	0.37 (1)			
R-Q	0/2022		-18.5	-18.5	0.30 (1)			
Q-P	0/2576		-18.5	-18.5	0.35 (1)			
P-O	0/2576		-18.5	-18.5	0.35 (1)			
O-N	0/2860		-18.5	-18.5	0.39 (1)			
N-M	0/0		-18.5	-18.5	0.08 (4)			

TOTAL WEIGHT = 2 X 195 = 389 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = U/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = U/999 (0.23")

CS1: TC=0.62/1.00 (B-C:1), BC=0.40/1.00 (T-U:1), WB=0.65/1.00 (B-U:1), SS1=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

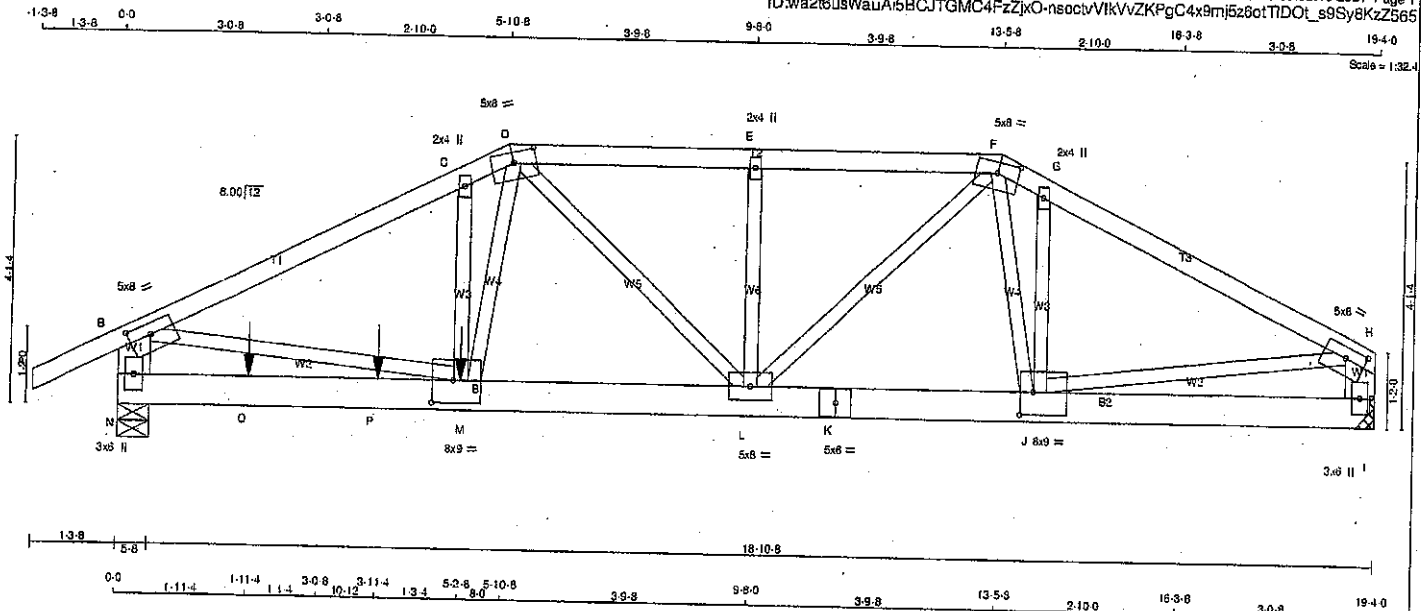
JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.65 (U) (INPUT = 1.00)



Structural component only
DWG# T-2108145

JOB NAME 417455	TRUSS NAME T38	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:wa2i6usWauA15BCJTGM04FzZjX0-nsoctvVikVvZKPgC4x9mj5z6otTIDot_s9Sy8KzZ565



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	No.2	SPF		
F - H	2x4	DRY	No.2	SPF		
N - B	2x8	DRY	No.2	SPF		
I - H	2x8	DRY	No.2	SPF		
N - K	2x8	DRY	No.2	SPF		
K - I	2x8	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY, SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	5.0	8.0	2.00	3.75
C, E, G					
D TMVW-w	MT20	2.0	4.0		
D TTWW-m	MT20	5.0	8.0	2.00	4.00
F TTWW-m	MT20	5.0	8.0	2.00	4.00
H TMVW-t	MT20	5.0	8.0	2.00	3.75
I BMV1+p	MT20	3.0	6.0		
J BMVWW-t	MT20	8.0	9.0	4.25	2.50
K BS-t	MT20	5.0	6.0		
L BMVWW-t	MT20	5.0	8.0		
M BMVWW-t	MT20	8.0	9.0	4.25	4.00
N BMV1+p	MT20	3.0	6.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
N	2589	0	2589	0
I	1580	0	1580	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	1812	1216	0	0	0	596	0
I	1115	742	0	0	0	373	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PL)	MEMB.	FORCE (LBS)
FR-TO			FR-TO	
A-B	0/28	-91.8	M-C	-493/0
B-C	-3700/0	-91.8	L-E	-418/0
C-D	-3776/0	-91.8	J-G	-575/0
D-E	-2758/0	-91.8	B-M	0/3358
E-F	-2758/0	-91.8	J-H	0/2082
F-G	-2395/0	-91.8	M-D	0/1936
G-H	-2285/0	-91.8	F-J	0/349
N-B	-2475/0	0.0	D-L	-327/0
I-H	-1525/0	0.0	L-F	0/1102
N-O	0/0	-18.5		
O-P	0/0	-18.5		
P-M	0/0	-18.5		
M-L	0/2891	-18.5		
L-K	0/1976	-18.5		
K-J	0/1976	-18.5		
J-I	0/0	-18.5		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	5-2-8	-1292	-1292	--	BACK	VERT	TOTAL	--	C1
O	1-11-4	-20	-20	--	BACK	VERT	TOTAL	--	C1
P	3-11-4	-21	-21	--	BACK	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/959 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.71/1.00 (B-C:1), BC=0.45/1.00 (L-M:1), WB=0.83/1.00 (B-M:1), SI=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	(PSI)	(PLI)	SECTION
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (B) (INPUT = 0.90)
JSI METAL = 0.89 (H) (INPUT = 1.00)



Structural component only
DWG# T-2108205

amarack Roof Truss, Burlington



NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER		MAXIMUM FACTORED		INPUT		REQRD	
BEARINGS		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1201	0	1201	0	0	MECHANICAL	
E	1201	0	1201	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, E. MINIMUM BEARING LENGTH AT JOINT H = 3-8, JOINT E = 3-8.

UNFACTORED REACTIONS

1ST CASE		MAX-MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	852	546 '0	0 '0	0 '0	0 '0	306 '0	0 '0
E	852	546 '0	0 '0	0 '0	0 '0	306 '0	0 '0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHOROS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	-1518 / 0	-91.8	-91.8 0.62 (1)	4.47	G-B	-63 / 93	0.04 (4)
B-C	-1355 / 0	-91.8	-91.8 0.30 (1)	5.16	B-F	0 / 3	0.00 (4)
I-C	-1355 / 0	-91.8	-91.8 0.30 (1)	5.16	F-C	-60 / 96	0.04 (4)
C-D	-1519 / 0	-91.8	-91.8 0.62 (1)	4.47	A-G	0 / 1371	0.34 (1)
H-A	-1138 / 0	0.0	0.0 0.08 (1)	7.81	F-D	0 / 1373	0.34 (1)
E-D	-1138 / 0	0.0	0.0 0.08 (1)	7.81			
H-J	0 / 0	-18.5	-18.5 0.22 (4)	10.00			
J-K	0 / 0	-18.5	-18.5 0.22 (4)	10.00			
K-G	0 / 0	-18.5	-18.5 0.22 (4)	10.00			
G-L	0 / 1353	-18.5	-18.5 0.33 (1)	10.00			
L-F	0 / 1353	-18.5	-18.5 0.33 (1)	10.00			
F-M	0 / 0	-18.5	-18.5 0.22 (4)	10.00			
M-N	0 / 0	-18.5	-18.5 0.22 (4)	10.00			
N-E	0 / 0	-18.5	-18.5 0.22 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

	L.O.C.	L.C1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONF.
	5-8-8	-208	-208	---	BACK	VERT	TOTAL	---	C1
	8-8-8	-208	-208	---	BACK	VERT	TOTAL	---	C1
	8-5-12	-19	-19	---	BACK	VERT	TOTAL	---	C1
	5-6-4	-19	-19	---	BACK	VERT	TOTAL	---	C1
	7-0-0	-66	-66	---	BACK	VERT	TOTAL	---	C1
	1-11-4	-18	-18	---	BACK	VERT	TOTAL	---	C1
	3-11-4	-19	-19	---	BACK	VERT	TOTAL	---	C1
	7-0-0	-19	-19	---	BACK	VERT	TOTAL	---	C1
	10-0-12	-19	-19	---	BACK	VERT	TOTAL	---	C1
	12-0-12	-18	-18	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

doi:10.1017/S0022278X12001007

SPECIFIED LOADS:				
TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 [N. C/C]

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.47")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.47")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.08")

CSI: TC=0.62/1.00 (C-D:1), BC=0.33/1.00 (F-G:1),
WB=0.34/1.00 (D-F:1), SSI=0.19/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

THUSS PLATE MANUFACTURE

TRUSS MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION

	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1872

PLATE PLACEMENT TOL. = 0.250 inches

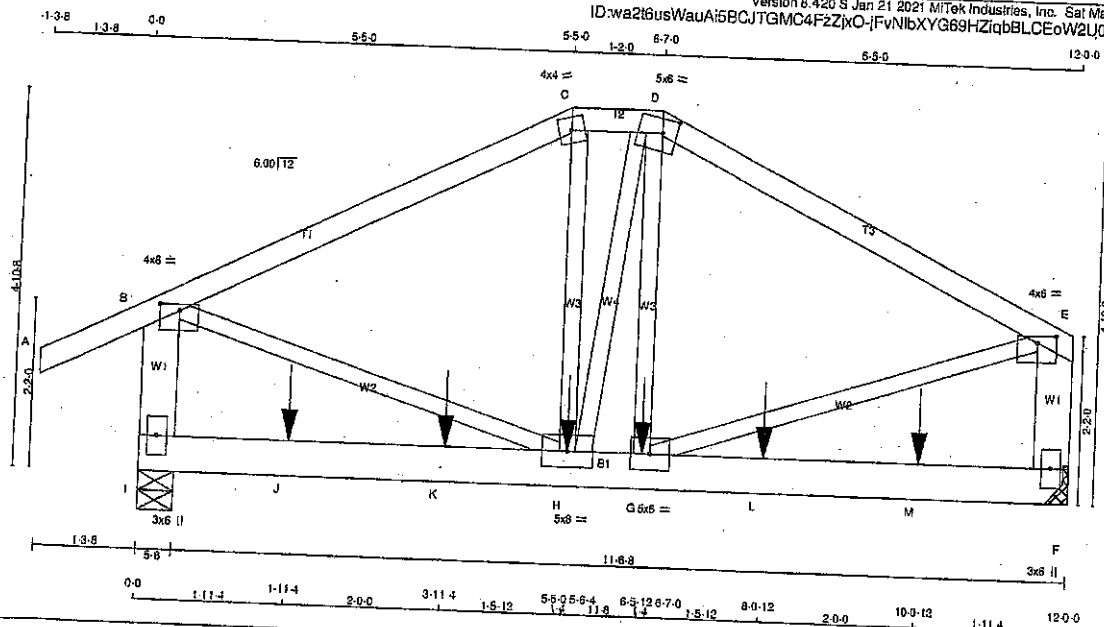
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.35 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108206

JOB NAME 417455	TRUSS NAME T40	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x6	DRY	No.2
F - G	2x6	DRY	No.2
G - H	2x6	DRY	No.2
H - I	2x6	DRY	No.2
I - J	2x6	DRY	No.2
J - K	2x6	DRY	No.2
K - L	2x6	DRY	No.2
L - M	2x6	DRY	No.2
M - N	2x6	DRY	No.2
N - O	2x6	DRY	No.2
O - P	2x6	DRY	No.2
P - Q	2x6	DRY	No.2
Q - R	2x6	DRY	No.2
R - S	2x6	DRY	No.2
S - T	2x6	DRY	No.2
T - U	2x6	DRY	No.2
U - V	2x6	DRY	No.2
V - W	2x6	DRY	No.2
W - X	2x6	DRY	No.2
X - Y	2x6	DRY	No.2
Y - Z	2x6	DRY	No.2
Z - A	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	4.0	6.0	1.00 3.00
C	ITW-m	MT20	4.0	4.0	
D	TTWW-m	MT20	5.0	6.0	2.25 2.25
E	TMVW-p	MT20	4.0	6.0	1.00 3.00
F	BMV1-p	MT20	3.0	6.0	
G	BMVW-1	MT20	5.0	6.0	
H	BMVWW-1	MT20	5.0	6.0	
I	BMV1-p	MT20	3.0	6.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	1634	0	1634	0	0	5-8	5-8
F	1509	0	1509	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

1ST LOSE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	1148	796	0	0	0	352	0
F	1062	728	0	0	0	336	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. CS (LC)	MEMB.	FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/28	-91.8	-91.8 0.13 (1)	H-C	0/319	0.08 (1)	
B-C	-1386/0	-91.8	-91.8 0.50 (1)	G-D	0/274	0.07 (1)	
C-D	-1246/0	-91.8	-91.8 0.04 (1)	B-H	0/1301	0.32 (1)	
D-E	-1376/0	-91.8	-91.8 0.59 (1)	G-E	0/1292	0.32 (1)	
E-F	-1392/0	0.0	0.0 0.11 (1)	H-D	0/42	0.01 (1)	
F-G	-1251/0	0.0	0.0 0.10 (1)				
I-J	0/0	-18.5	-18.5 0.32 (1)				
J-K	0/0	-18.5	-18.5 0.32 (1)				
K-H	0/0	-18.5	-18.5 0.32 (1)				
H-G	0/1236	-18.5	-18.5 0.47 (1)				
G-L	0/0	-18.5	-18.5 0.33 (1)				
L-M	0/0	-18.5	-18.5 0.33 (1)				
M-F	0/0	-18.5	-18.5 0.33 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	6-5-12	-196	-196	---	FRONT	VERT	TOTAL	---	C1
H	5-6-4	-196	-196	---	FRONT	VERT	TOTAL	---	C1
J	1-11-4	-202	-202	---	FRONT	VERT	TOTAL	---	C1
K	3-11-4	-196	-196	---	FRONT	VERT	TOTAL	---	C1
L	8-0-12	-196	-196	---	FRONT	VERT	TOTAL	---	C1
M	10-0-12	-202	-202	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
BOT CH.	LL	=	6.0	PSF
	DL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.40")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.40")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.09")

CSI: TC=0.80/1.00 (B-C:1), BC=0.47/1.00 (G-H:1), WB=0.32/1.00 (B-H:1), SS=0.28/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
MT20	650	371	1747
	MAX MIN	MAX MIN	MAX MIN
	788	1967	1873

PLATE PLACEMENT TOL. = 0.250 inches

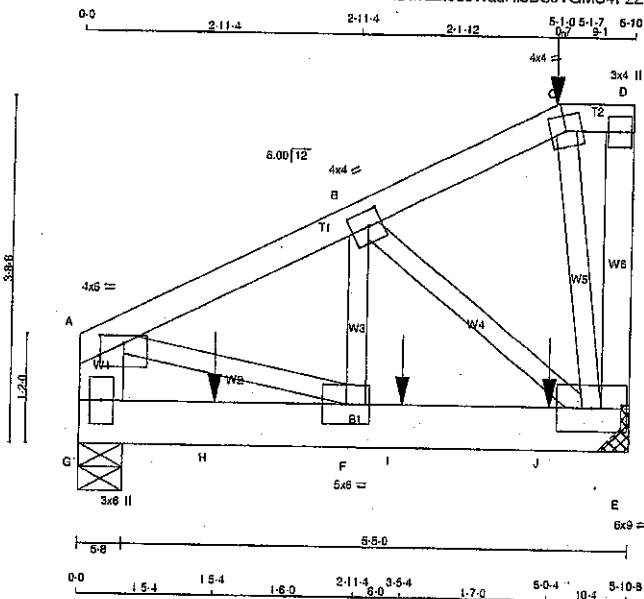
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.33 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108207

JOB NAME 417455	TRUSS NAME T41	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - D	2x4 DRY	No.2	SPF		
E - D	2x4 DRY	No.2	SPF		
G - A	2x6 DRY	No.2	SPF		
G - E	2x6 DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS: (0.122\"X3\") SPIRAL NAILS		
A-C 1 12	SIDE(61.0)	
C-D 1 12	SIDE(61.0)	
D-E 1 12	TOP	
G-A 2 12	TOP	
BOTTOM CHORDS: (0.122\"X3\") SPIRAL NAILS		
G-E 2 12	SIDE(0.0)	
WEBS: (0.122\"X3\") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW-p	MT20	4.0	6.0	1.00	3.00
B TMVW-t	MT20	4.0	4.0	2.00	1.75
C TTW-m	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
E	1859	0	1859	0	MECHANICAL	5-8
G	871	0	871	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 4-0.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	1307	902.0	0.0	0.0	0.0	405.0	0.0
G	610	436.0	0.0	0.0	0.0	174.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. MAX. UNBRACED LENGTH	
FR-TO		FROM TO	CSI (LC)	FR-TO		CSI (LC)	
A-B	-999.0	-91.8	-91.8 0.08 (1)	C-E	-374.0	0.04 (1)	
B-C	-104.0	-91.8	-91.8 0.04 (1)	A-F	0.932	0.12 (1)	
C-D	0.0	-91.8	-91.8 0.00 (1)	F-B	0.754	0.09 (1)	
E-D	-35.0	0.0	0.0 0.00 (1)	B-E	-1029.0	0.12 (1)	
G-A	-804.0	0.0	0.0 0.03 (1)				
G-H	0.0	-18.5	-18.5 0.10 (1)				
H-F	0.0	-18.5	-18.5 0.10 (1)				
F-I	0.901	-18.5	-18.5 0.32 (1)				
I-J	0.901	-18.5	-18.5 0.32 (1)				
J-E	0.901	-18.5	-18.5 0.32 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-1-7	-206	-206		FRONT	VERT	TOTAL		C1
H	1-5-4	-177	-177		TOP	VERT	TOTAL		C1
I	3-5-4	-239	-239		TOP	VERT	TOTAL		C1
J	5-0-4	-838	-838		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 31 = 62 LB

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.08/1.00 (A-B:1), BC=0.32/1.00 (E-F:1), WB=0.12/1.00 (B-E:1), SI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

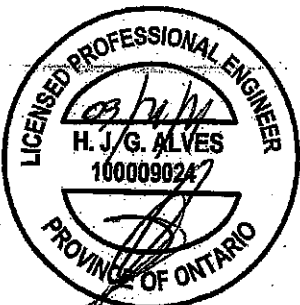
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 768 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.49 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108208 1/12

CONTINUED ON PAGE 2

JOB NAME 417455	TRUSS NAME T41	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	TMV+p	MT20	3.0	4.0		
E	BMVWW1-t	MT20	6.0	9.0		
F	BMVWW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108208 *2/11*

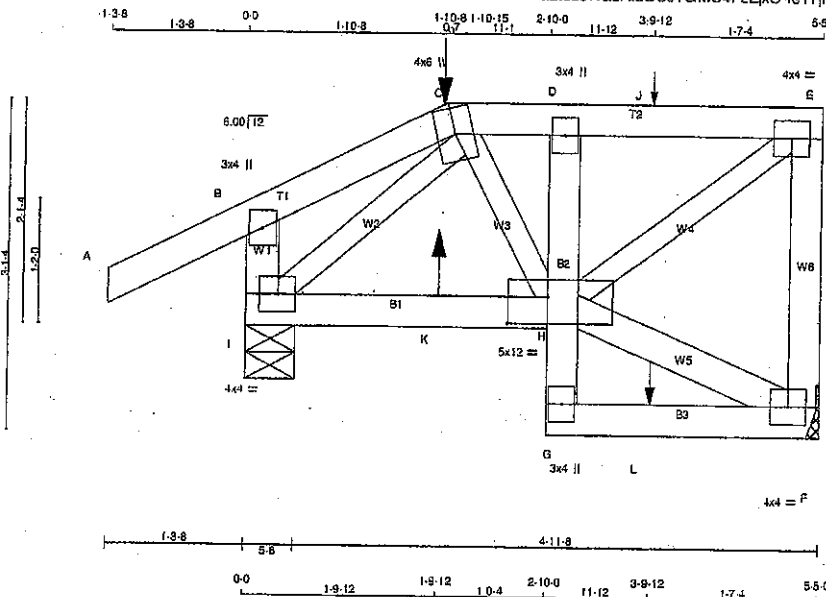
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417455	T42	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER

N.L.G.A. RULES

CHORDS SIZE

A - C 2x4 DRY No.2

C - E 2x4 DRY No.2

F - E 2x4 DRY No.2

I - B 2x4 DRY No.2

J - H 2x4 DRY No.2

G - D 2x4 DRY No.2

G - F 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

H - F 2x4 DRY No.2

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

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SPF

SPF

SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED MAXIMUM FACTORED INPUT REORD

GROSS REACTION GROSS REACTION BRG BRG

JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX

F 306 0 306 0 0 MECHANICAL

I 436 0 436 0 0 5-8 5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST CASE MAX/MIN COMPONENT REACTIONS

JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL

F 217 142 0 0 0 0 0 74 0 0 0

I 306 214 0 0 0 0 0 92 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS

MAX. FACTORED FACTORED WEBS

MEMB. FORCE VERT. LOAD LC1 MAX MAX. MEMB. MAX. FACTORED

FR-TO (LBS) FROM TO CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC)

A-B 0 28 -91.8 -91.8 0.13 (1) 10.00 H-F -12 0 0.00 (1)

B-C 0 0 -91.8 -91.8 0.08 (1) 10.00 H-E 0 306 0.08 (1)

C-D -252 0 -91.8 -91.8 0.08 (1) 6.25 C-H 0 83 0.02 (1)

D-J -246 0 -91.8 -91.8 0.08 (1) 6.25 I-C -289 0 0.05 (1)

E-E -246 0 -91.8 -91.8 0.08 (1) 6.25

F-E -279 0 0.0 0.05 (1) 7.81

I-B -212 0 0.0 0.02 (1) 7.81

I-K 0 210 -18.5 -18.5 0.07 (4) 10.00

K-H 0 210 -18.5 -18.5 0.07 (4) 10.00

G-H 0 26 0.0 0.02 (1) 10.00

H-D -212 0 0.0 0.02 (1) 7.81

G-L 0 11 -18.5 -18.5 0.04 (4) 10.00

L-F 0 11 -18.5 -18.5 0.04 (4) 10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT LOC. LC1 MAX. MAX+ FACE DIR. TYPE HEEL CONN.

C 1-10-15 -4 -4 -- FRONT VERT DEAD -- C1

C 1-10-15 -1 -1 67 FRONT VERT TOTAL -- C1

C 1-10-15 -14 -14 -- FRONT VERT SNOW -- C1

J 3-9-12 1 1 76 FRONT VERT TOTAL -- C1

K 1-9-12 4 1 8 FRONT VERT TOTAL -- C1

L 3-9-12 -0 -0 -- FRONT VERT TOTAL -- C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT

NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF

DL = 8.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

*** NON STANDARD GIRDER ***

ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 8 OF CBC 2018, ABC 2019

- PART 9 OF CBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")

CALCULATED VERT. DEFL. (LL) = L/999 (0.00")

ALLOWABLE DEFL. (TL) = L/800 (0.19")

CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.13/1.00 (A-B:1), BC=0.07/1.00 (H-I:4),

WB=0.08/1.00 (E-H:1), SS=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

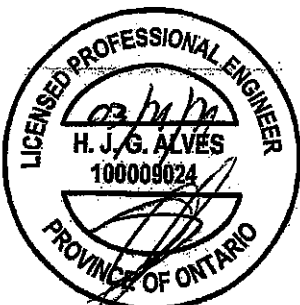
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.30 (E) (INPUT = 0.90)

JSI METAL = 0.12 (C) (INPUT = 1.00)

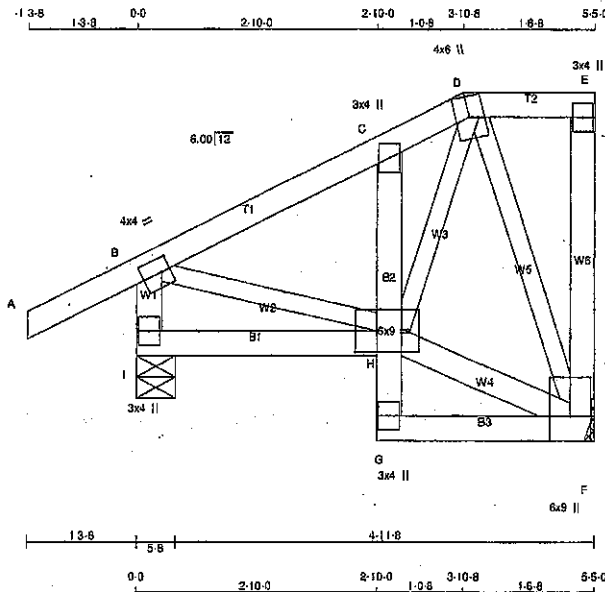


Structural component only
DWG# T-2108209

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
417455	T43	2	1	GREENPARK HOMES		

Tamarack Roof Truss, Burlington

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ID:wa216usWauA15BCJTGMC4FzZpXO-8qbVwcZRZ1YrQAZ9tUlxQ9g7uHbuq2?R9ipYzZ560



Scale = 1/2" = 1'-0"

TOTAL WEIGHT = 2 X 33 = 66 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
I - B	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
G - C	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT	H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	TTWW+m	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVWW1+p	MT20	6.0	9.0	Edge	
G	BMV+p	MT20	3.0	4.0		
H	BMVWW-1	MT20	6.0	9.0	3.00	3.00
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG.	REQD. BRG.
	VERT	HORZ	DOWN	HORZ		
F	299	0	299	0	0	0
I	423	0	423	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	MAX. MIN. COMPONENT REACTIONS						
	1ST LOASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
F	211	139	0	0	0	0	73
I	297	208	0	0	0	0	89

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. FACTORED VERT. LOAD (LBS)	LC1 MAX. FACTORED HORIZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	LC1 MAX. FACTORED VERT. LOAD (LBS)	
FR-TO					FR-TO				
A-B	0/28	-91.8	-91.8	0.12 (1)	10.00	B-H	0/209	0.05 (1)	
B-C	-215/0	-91.8	-91.8	0.10 (1)	6.25	H-F	0/87	0.01 (1)	
C-D	-236/0	-91.8	-91.8	0.08 (1)	6.25	H-D	0/294	0.07 (1)	
D-E	0/0	-91.8	-91.8	0.04 (1)	10.00	D-F	-258/0	0.07 (1)	
F-E	-69/0	0.0	0.0	0.02 (1)	7.81				
I-B	-395/0	0.0	0.0	0.04 (1)	7.81				
I-H	0/0	-18.5	-18.5	0.05 (4)	10.00				
G-H	0/25	0.0	0.0	0.02 (1)	10.00				
H-C	-254/0	0.0	0.0	0.02 (1)	7.81				
G-F	0/14	-18.5	-18.5	0.03 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2013
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 089-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CS: TC=0.12/1.00 (A-B:1), SC=0.05/1.00 (H-H:4), WB=0.07/1.00 (D-H:1), SL=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.33 (B) (INPUT = 0.90)

JSI METAL= 0.11 (B) (INPUT = 1.00)

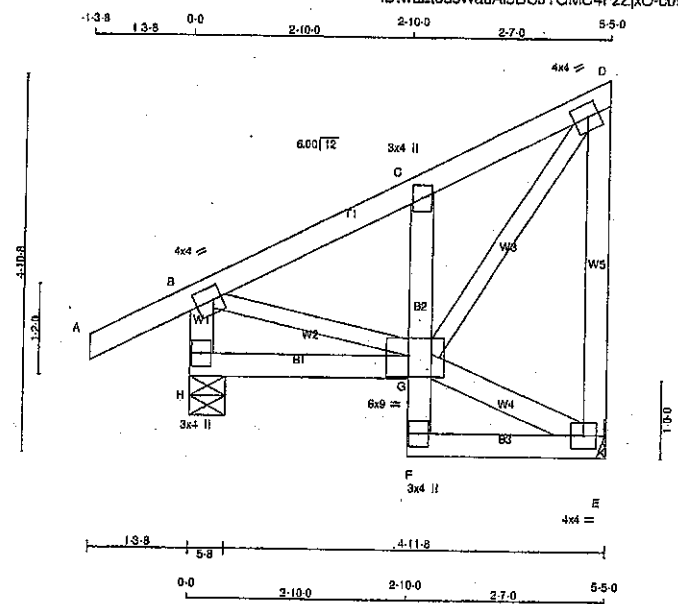


Structural component only
DWG# T-2108210

JOB NAME 417455	TRUSS NAME T44	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/27.2

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (tablets in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-I	MT20	4.0	4.0	2.00 1.25
C	TMVW-p	MT20	3.0	4.0	
D	TMVW-I	MT20	4.0	4.0	2.00 1.75
E	BMVW-I	MT20	4.0	4.0	
F	BMVW-p	MT20	3.0	4.0	
G	BMVW-I	MT20	6.0	9.0	3.25 3.50
H	BMVW-p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
E	296	0	293	0	0
H	423	0	423	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	211	139 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
H	297	208 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	FORCE (LBS)	FACTORED MAX. CS (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	B-G	0 / 215	0.05 (1)
B-C	-220 / 0	-91.8	-91.8 0.10 (1)	6.25	G-E	-17 / 0	0.00 (1)
C-D	-228 / 0	-91.8	-91.8 0.10 (1)	6.25	G-D	0 / 340	0.08 (1)
E-D	-270 / 0	0.0	0.0 0.10 (1)	7.81			
H-B	-396 / 0	0.0	0.0 0.04 (1)	7.81			
H-G	0 / 0	-18.5	-18.5 0.05 (4)	10.00			
F-G	0 / 25	0.0	0.0 0.02 (1)	10.00			
G-C	-297 / 0	0.0	0.0 0.02 (1)	7.81			
F-E	0 / 15	-18.5	-18.5 0.03 (4)	10.00			

TOTAL WEIGHT = 2 X 32 = 64 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.18")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.18")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.12/1.00 (A-B:1), BC=0.06/1.00 (G-H:4), WB=0.03/1.00 (D-G:1), SS=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1967 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

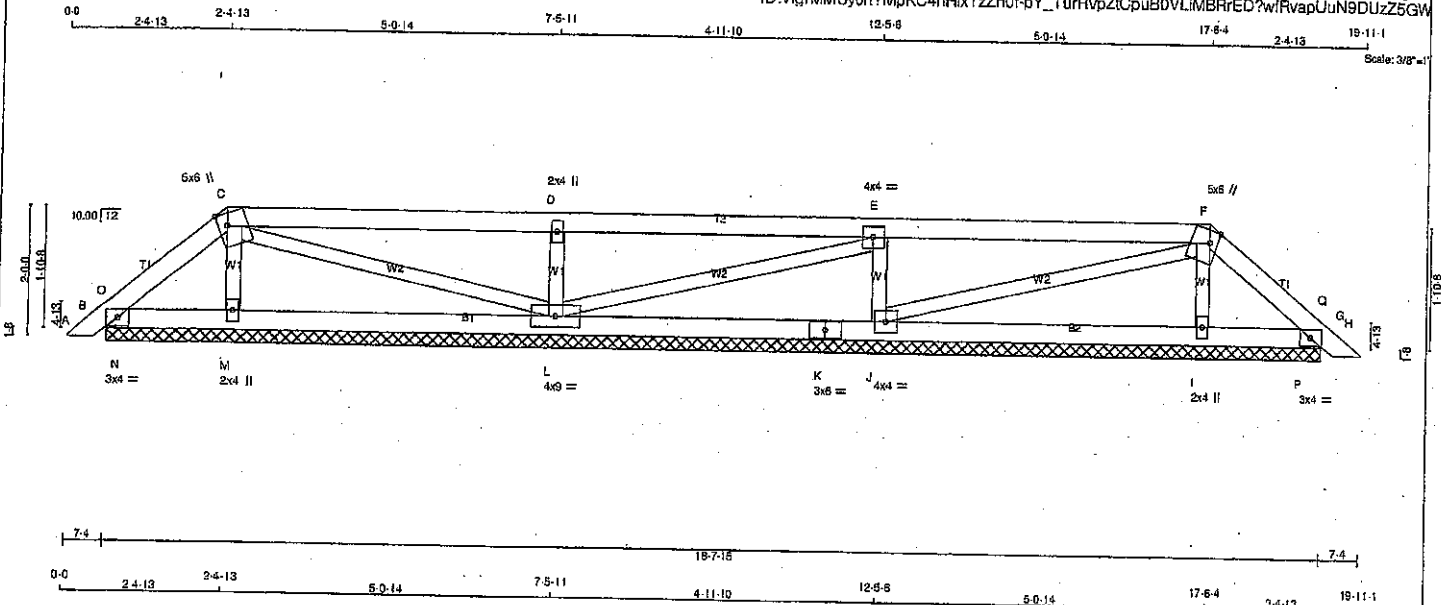
JSI GRIP = 0.35 (D) (INPUT = 0.80)
JSI METAL = 0.11 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108211

JOB NAME 417451	TRUSS NAME PB1	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:VlgrMMUy8t?MpKC4HhXtZzh0f-pY_TurRvpZiCpuB0VLIMBRrED?w/RvapUuN9DUZZ5GW



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
B - K	2x4	DRY	No.2	SPF	
K - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMB1-I	MT20	3.0	4.0		
C TTWW+m	MT20	5.0	8.0	2.25	1.50
D TMW+w	MT20	2.0	4.0		
E TMWW-I	MT20	4.0	4.0		
F TTWW+m	MT20	5.0	8.0	2.25	1.50
G TMB1-I	MT20	3.0	4.0		
J BMW1+w	MT20	2.0	4.0		
K BMWW1-I	MT20	4.0	4.0		
L BS-I	MT20	3.0	8.0		
M BMWW1-I	MT20	4.0	9.0		
N BMW1+w	MT20	2.0	4.0		

NOTES:

- (1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	DOWN	GROSS REACTION	UP		
JT	VERT	HORZ	DOWN	HORZ	IN-SX	BRG
B	163	0	163	0	18-7-15 (10-1182-5)	IN-SX
M	304	0	304	0	18-7-15 (10-1182-5)	IN-SX
I	300	0	300	0	18-7-15 (10-1182-5)	IN-SX
G	171	0	171	0	18-7-15 (10-1182-5)	IN-SX
J	597	0	597	0	18-7-15 (10-1182-5)	IN-SX
L	618	0	618	0	18-7-15 (10-1182-5)	IN-SX

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	112	91.0	0.0	0.0	0.0	22.0	0.0
M	218	126.0	0.0	0.0	0.0	92.0	0.0
I	216	124.0	0.0	0.0	0.0	92.0	0.0
G	118	95.0	0.0	0.0	0.0	23.0	0.0
J	422	281.0	0.0	0.0	0.0	141.0	0.0
L	435	291.0	0.0	0.0	0.0	143.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, M, I, G, J, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0.14	-91.8	-91.8 0.02 (1)	10.00	M-C	-209.0	0.03 (1)
B-O	-24.7	-91.8	-91.8 0.01 (1)	6.25	I-F	-205.0	0.03 (1)
O-C	-55.0	-91.8	-91.8 0.03 (1)	6.25	J-F	-17.0	0.01 (1)
C-D	0.6	-91.8	-91.8 0.32 (1)	10.00	C-L	-31.0	0.01 (1)
D-E	0.6	-91.8	-91.8 0.32 (1)	10.00	J-E	-499.0	0.07 (1)
E-F	-17.0	-91.8	-91.8 0.32 (1)	6.25	L-D	-507.0	0.07 (1)
F-O	-67.0	-91.8	-91.8 0.03 (1)	6.25	L-E	-24.0	0.01 (1)
Q-G	-37.4	-91.8	-91.8 0.02 (1)	6.25	N-O	-123.0	0.00 (1)
G-H	0.14	-91.8	-91.8 0.02 (1)	10.00	P-Q	-122.0	0.00 (1)
B-N	0.38	-18.5	-18.5 0.04 (1)	10.00			
N-M	0.38	-18.5	-18.5 0.07 (4)	10.00			
M-L	0.25	-18.5	-18.5 0.09 (4)	10.00			
L-K	0.17	-18.5	-18.5 0.08 (4)	10.00			
K-J	0.17	-18.5	-18.5 0.09 (4)	10.00			
J-I	0.34	-18.5	-18.5 0.09 (4)	10.00			
I-P	0.48	-18.5	-18.5 0.07 (4)	10.00			
P-Q	0.48	-18.5	-18.5 0.04 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (D-E:1), BC=0.09/1.00 (I-J:4), WB=0.07/1.00 (D-L:1), SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 6.0 Deg.

JSI GRIP = 0.68 (J) (INPUT = 0.90)
JSI METAL = 0.11 (J) (INPUT = 1.00)

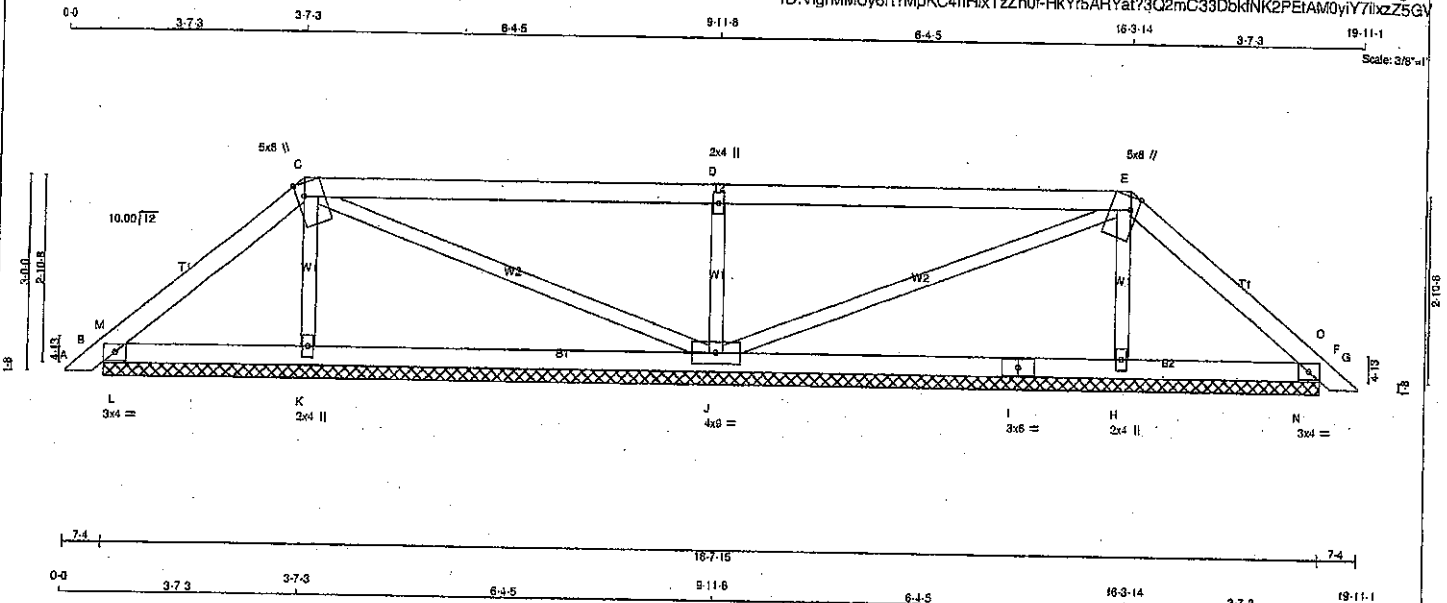


Structural component only
DWG# T-2108177

JOB NAME 417451	TRUSS NAME PB2	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY No.2

E - G 2x4 DRY No.2

B - I 2x4 DRY No.2

I - F 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE PLATES W LEN Y X

B TMB1-I MT20 3.0 4.0

C TTWW+m MT20 5.0 8.0 Edge 1.25

D TMW+w MT20 2.0 4.0

E TTWW+m MT20 5.0 8.0 Edge 1.25

F TMB1-I MT20 3.0 4.0

H BMW1+w MT20 2.0 4.0

I BS-I MT20 3.0 6.0

J BMW1+w MT20 4.0 9.0

K BMW1+w MT20 2.0 4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	IN-SX	IN-SX
B	257	0	257	0
K	379	0	379	0
J	880	0	880	0
H	379	0	379	0
F	257	0	257	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	179	136 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0
K	272	157 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
J	620	422 / 0	0 / 0	0 / 0	0 / 0	197 / 0	0 / 0
H	272	157 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
F	179	136 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED LCI (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS MEMB. MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO						
A-B	0 / 14	-91.8	-91.8	0.02 (1)	10.00	0.04 (1)
B-M	-2 / 1	-91.8	-91.8	0.05 (1)	10.00	0.04 (1)
M-C	-103 / 0	-91.8	-91.8	0.09 (1)	6.25	0.13 (1)
C-D	-20 / 0	-91.8	-91.8	0.63 (1)	6.25	0.04 (1)
D-E	-20 / 0	-91.8	-91.8	0.63 (1)	6.25	0.04 (1)
E-O	-103 / 0	-91.8	-91.8	0.09 (1)	6.25	0.04 (1)
O-F	-2 / 1	-91.8	-91.8	0.05 (1)	10.00	0.00 (1)
F-G	0 / 14	-91.8	-91.8	0.02 (1)	10.00	0.00 (1)
B-L	0 / 73	-18.5	-18.5	0.10 (1)	10.00	
L-K	0 / 73	-18.5	-18.5	0.11 (4)	10.00	
K-J	0 / 62	-18.5	-18.5	0.16 (4)	10.00	
J-I	0 / 62	-18.5	-18.5	0.16 (4)	10.00	
I-H	0 / 62	-18.5	-18.5	0.16 (4)	10.00	
H-N	0 / 73	-18.5	-18.5	0.11 (4)	10.00	
N-F	0 / 73	-18.5	-18.5	0.10 (1)	10.00	

TOTAL WEIGHT = 2 X 63 = 126 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.63/1.00 (D-E:1), BC=0.16/1.00 (H-J:4), WB=0.13/1.00 (D-J:1), SS=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

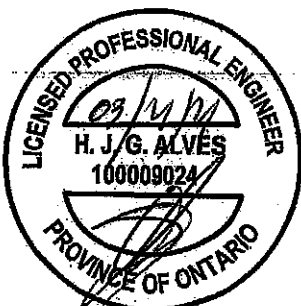
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (C) (INPUT = 0.80)

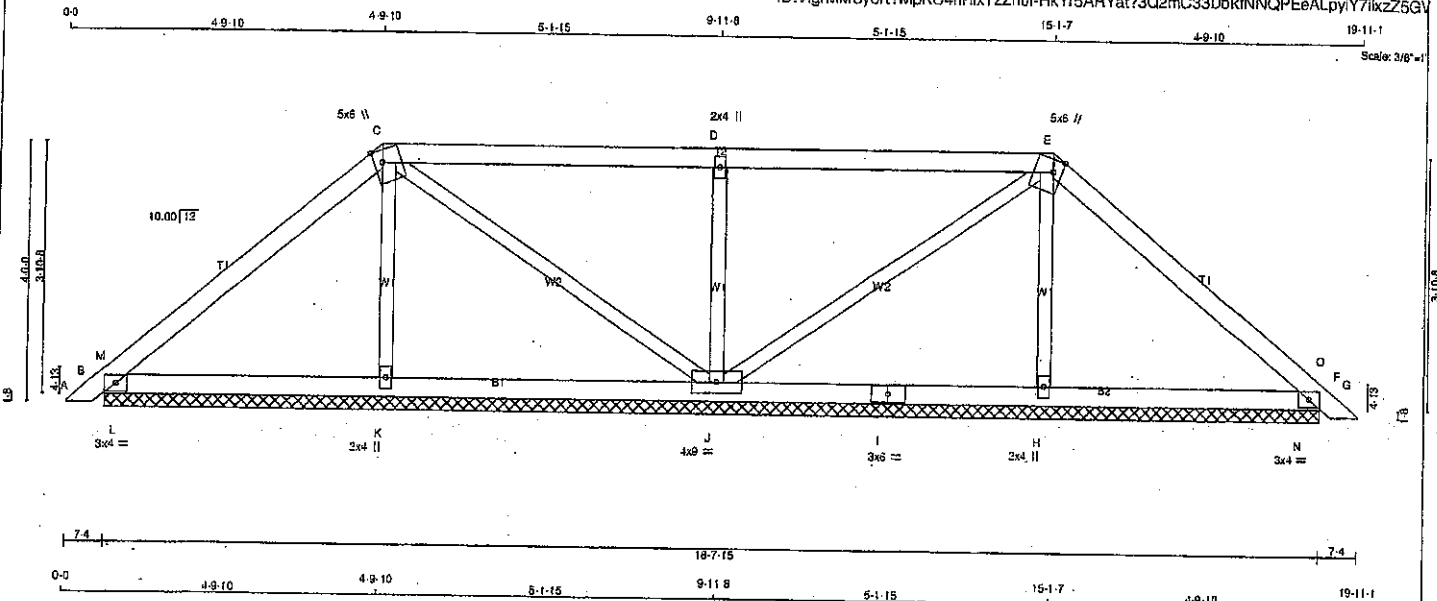
JSI METAL = 0.15 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108178

JOB NAME 417451	TRUSS NAME PB3	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW+M	MT20	5.0	6.0	2.25 1.50
D	TTWW+M	MT20	2.0	4.0	
E	TTWW+M	MT20	5.0	6.0	2.25 1.50
F	TMB1-I	MT20	3.0	4.0	
H	BMW1+M	MT20	2.0	4.0	
I	SS-I	MT20	3.0	8.0	
J	BMW1+M	MT20	4.0	9.0	
K	BMW1+M	MT20	2.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	RECORD
	GROSS REACTION	DOWN	GROSS REACTION	UP		
JT	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
B	359	0	359	0	18-7-15 (6-8-38-7-15)	
K	339	0	339	0	18-7-15 (6-8-38-7-15)	
J	758	0	758	0	18-7-15 (6-8-38-7-15)	
H	339	0	339	0	18-7-15 (6-8-38-7-15)	
F	359	0	359	0	18-7-15 (6-8-38-7-15)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LOOSE	MAX. MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	251	180.0	0.0	0.0	0.0	71.0	0.0
K	249	139.0	0.0	0.0	0.0	104.0	0.0
J	532	369.0	0.0	0.0	0.0	163.0	0.0
H	249	139.0	0.0	0.0	0.0	104.0	0.0
F	251	180.0	0.0	0.0	0.0	71.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE	FACTORED		MAX.	MEMB.	MAX. FACTORED	
	(LBS)	VERT. LOAD	MAX.	UNBRAC		FORCE	MAX.
		(PLF)	CSI (LC)			(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0.14	-91.8	-91.8 0.02 (1)	10.00	K-C	-199.0	0.05 (1)
B-M	-28.53	-91.8	-91.8 0.11 (1)	6.25	C-J	-81.0	0.05 (1)
M-C	-159.0	-91.8	-91.8 0.20 (1)	6.25	J-D	-591.0	0.14 (1)
C-D	-40.0	-91.8	-91.8 0.41 (1)	6.25	J-E	-81.0	0.05 (1)
D-E	-40.0	-91.8	-91.8 0.41 (1)	6.25	H-E	-199.0	0.05 (1)
E-O	-159.0	-91.8	-91.8 0.20 (1)	6.25	L-M	-477.0	0.00 (1)
O-F	-28.53	-91.8	-91.8 0.11 (1)	6.25	N-O	-477.0	0.00 (1)
F-G	0.14	-91.8	-91.8 0.02 (1)	10.00			
B-L	0.112	-18.5	-18.5 0.17 (1)	10.00			
L-K	0.112	-18.5	-18.5 0.17 (1)	10.00			
K-J	0.107	-18.5	-18.5 0.11 (1)	10.00			
J-I	0.107	-18.5	-18.5 0.11 (1)	10.00			
I-H	0.107	-18.5	-18.5 0.11 (1)	10.00			
H-N	0.112	-18.5	-18.5 0.17 (1)	10.00			
N-F	0.112	-18.5	-18.5 0.17 (1)	10.00			

TOTAL WEIGHT = 2 X 65 = 130 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2015
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.41/1.00 (C-D:1), BC=0.17/1.00 (K-L:1), WB=0.14/1.00 (D-J:1), SS=0.37/1.00 (B-L:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

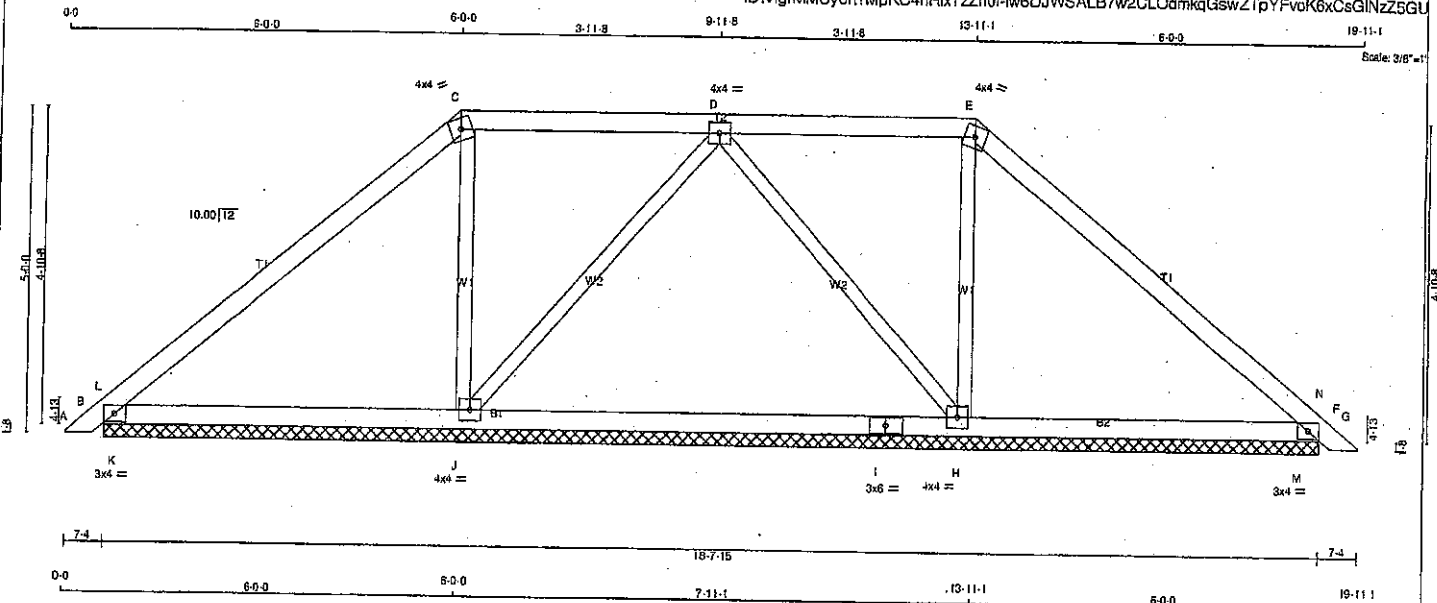
JSI GRIP= 0.63 (C) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108179

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417451	PB4	2	1	TRUSS DESC.		
Tamarack Roof Truss, Burlington						

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ID:VigrMMUy6r7MpKC4hHtZzh0f-lw6DJWSALB7w2CLOdmkqGswZTpYFvK6xCsGInZ5GU



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x8	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0			
C	TTW-m	MT20	4.0	4.0			
D	TMWV-I	MT20	4.0	4.0			
E	TTW-m	MT20	4.0	4.0			
F	TMB1-I	MT20	3.0	4.0			
H	BMWV1-I	MT20	4.0	4.0			
I	BS-I	MT20	3.0	8.0			
J	BMWV1-I	MT20	4.0	4.0			

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
B	447	0	447	0
J	830	0	830	0
H	830	0	830	0
F	447	0	447	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS	JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	312	225	0	0	0	0	0	87	0
J	448	279	0	0	0	0	0	169	0
H	448	279	0	0	0	0	0	169	0
F	312	225	0	0	0	0	0	87	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, J, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO									
A-B	0 / 14	-91.8	-91.8	0.02 (1)	10.00	J-C	-227	0	0.08 (1)
B-L	-13 / 188	-91.8	-91.8	0.21 (1)	6.25	H-E	-227	0	0.08 (1)
L-C	-204 / 0	-91.8	-91.8	0.33 (1)	6.25	J-D	-295	0	0.19 (1)
C-D	-139 / 0	-91.8	-91.8	0.24 (1)	6.25	D-H	-295	0	0.19 (1)
D-E	-139 / 0	-91.8	-91.8	0.24 (1)	6.25	K-L	-778	0	0.00 (1)
E-N	-204 / 0	-91.8	-91.8	0.33 (1)	6.25	M-N	-778	0	0.00 (1)
N-F	-13 / 188	-91.8	-91.8	0.21 (1)	6.25				
F-G	0 / 14	-91.8	-91.8	0.02 (1)	10.00				
B-K	0 / 144	-18.5	-18.5	0.28 (1)	10.00				
K-J	0 / 144	-18.5	-18.5	0.28 (1)	10.00				
J-I	0 / 328	-18.5	-18.5	0.22 (4)	10.00				
I-H	0 / 328	-18.5	-18.5	0.22 (4)	10.00				
H-M	0 / 144	-18.5	-18.5	0.28 (1)	10.00				
M-F	0 / 144	-18.5	-18.5	0.28 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2018, ABC 2018
- PART 9 OF NBC 2012 (2018 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.33/1.00 (C-L:1), BC=0.28/1.00 (B-K:1),
WB=0.19/1.00 (D-J:1), SSI=0.60/1.00 (F-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

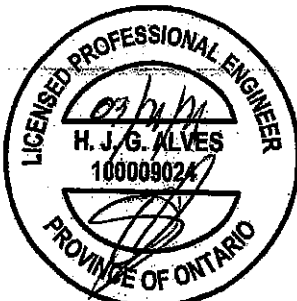
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1887 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.82 (C) (INPUT = 0.90)
JSI METAL= 0.20 (I) (INPUT = 1.00)

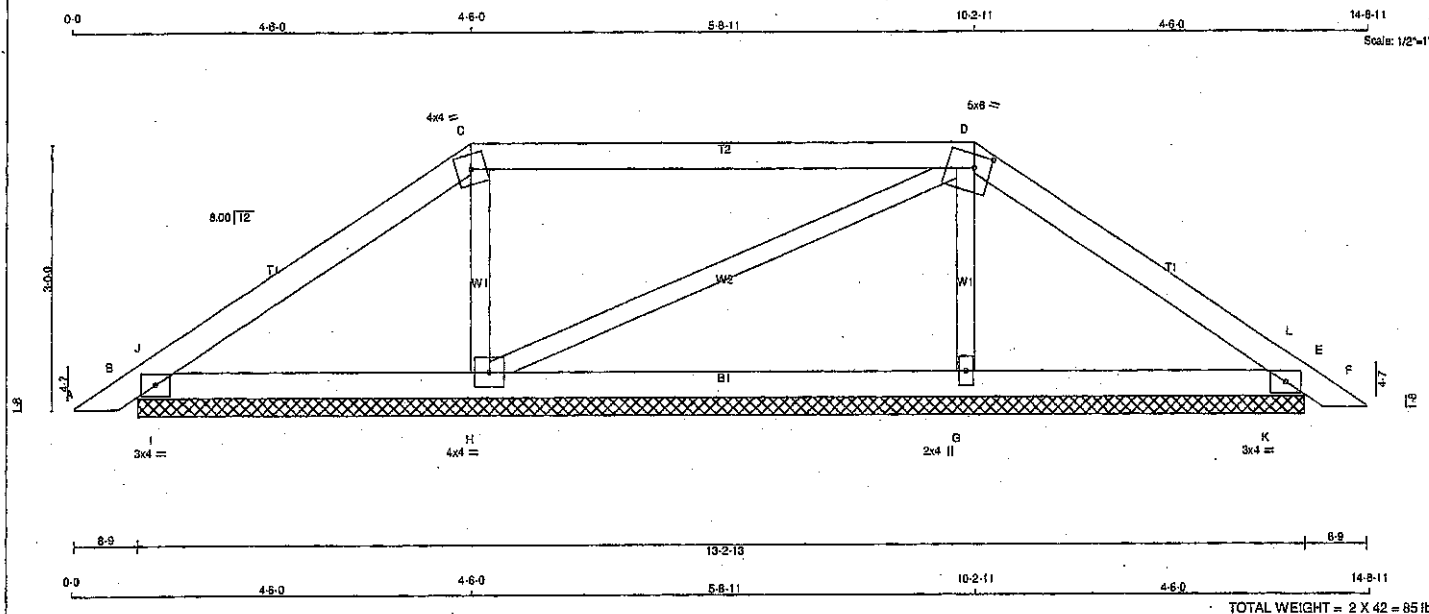


Structural component only
DWG# T-2108180

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417449	PB12	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:v5FX5Bj0jrZNLenu26zoDzZi9Z-198SGZRo3OED7FpmZyGLydt0MOa5cft2L_eVzZ5Ya



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY, SEASONED LUMBER.			

PLATES (table in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B-TM81-I	MT20	3.0	4.0		
C-TTW-m	MT20	4.0	4.0		
D-TTW-m	MT20	5.0	6.0	1.75	2.25
E-TM81-I	MT20	3.0	4.0		
G-BMW1-w	MT20	2.0	4.0		
H-BMW1-I	MT20	4.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
B	314	0	314	0	0	13-2-13	13-2-13		
E	332	0	332	0	0	13-2-13	13-2-13		
H	487	0	487	0	0	13-2-13	13-2-13		
G	444	0	444	0	0	13-2-13	13-2-13		

UNFACTORED REACTIONS

1ST CASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	219	160.0	0.0	0.0	0.0	59.0	0.0
E	231	170.0	0.0	0.0	0.0	62.0	0.0
H	346	218.0	0.0	0.0	0.0	127.0	0.0
G	317	195.0	0.0	0.0	0.0	122.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM	TO	FR-TO				
A-B	0/15	-91.8	-91.8	0.03 (1)	10.00	H-C	-348.0	0.08 (1)
B-J	-24.0	-91.8	-91.8	0.06 (1)	6.25	H-D	-30.0	0.02 (1)
J-C	-116.0	-91.8	-91.8	0.18 (1)	8.25	G-D	-318.0	0.05 (1)
C-D	-78.0	-91.8	-91.8	0.51 (1)	8.25	F-J	-317.0	0.00 (1)
D-L	-147.0	-91.8	-91.8	0.16 (1)	6.25	K-L	-314.0	0.00 (1)
L-E	-59.0	-91.8	-91.8	0.06 (1)	6.25			
E-F	0/15	-91.8	-91.8	0.03 (1)	10.00			
B-I	0/90	-18.5	-18.5	0.14 (1)	10.00			
I-H	0/90	-18.5	-18.5	0.14 (1)	10.00			
H-G	0/109	-18.5	-18.5	0.11 (4)	10.00			
G-K	0/116	-18.5	-18.5	0.15 (1)	10.00			
K-E	0/116	-18.5	-18.5	0.15 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2016
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.51/1.00 (C-D:1), SC=0.15/1.00 (G-K:1), WB=0.09/1.00 (C-H:1), SSI=0.24/1.00 (B-I:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

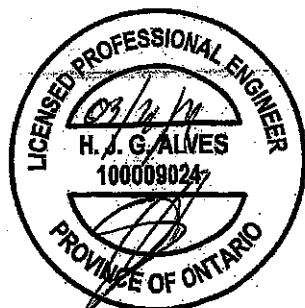
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
		1873	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

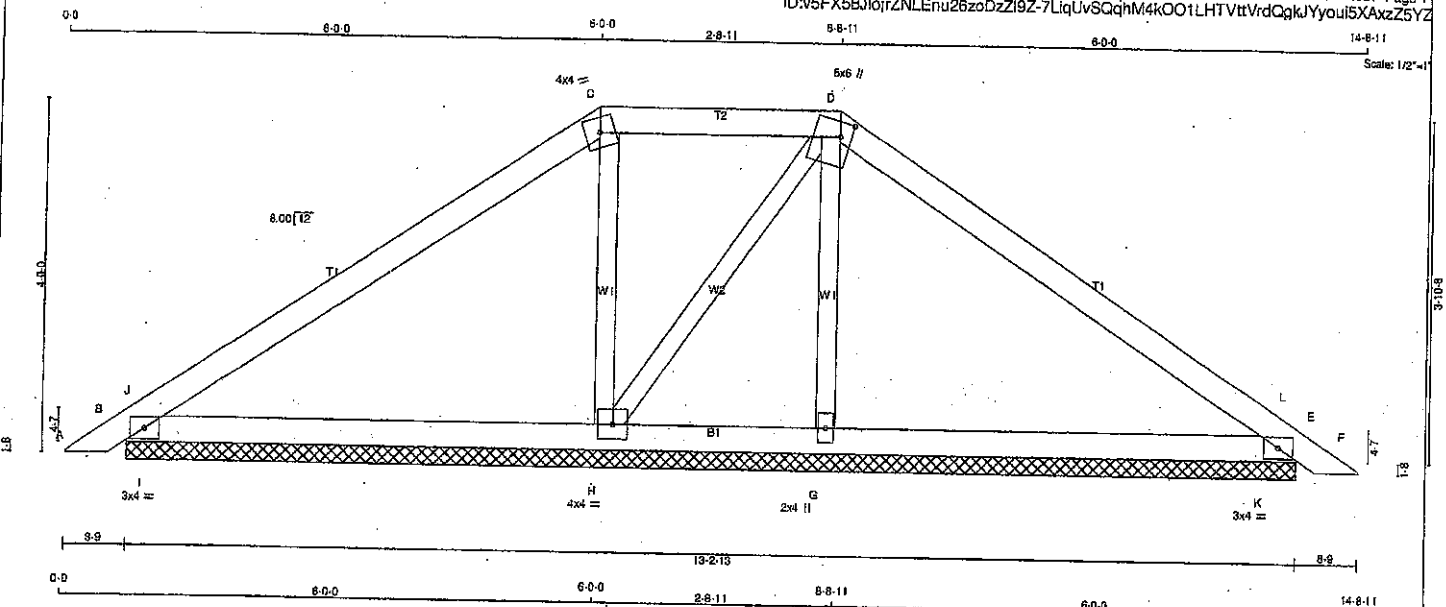
JSI GRIP= 0.27 (E) (INPUT = 0.90)
JSI METAL= 0.07 (E) (INPUT = 1.00)



Structural component only
DWG# T-2108160

JOB NAME 417449	TRUSS NAME PB13	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 09:27:54 2021 Page 1
ID:v5FX5Bj0jrZNLenu26zoDzZi9Z-7LqUvSQqM4k001LHTVttVrdCqkYyoui5XAxzZ5Yz



LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
B - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	W	LEN	Y	X
JT TMB1-I	MT20	3.0	4.0	
B TMB1-I	MT20	3.0	4.0	
C TTW+m	MT20	4.0	4.0	
D TTW+m	MT20	5.0	6.0	2.00 1.50
E TMB1-I	MT20	3.0	4.0	
G BMW1-w	MT20	2.0	4.0	
H BMW1-w	MT20	4.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
B	434	434	13-2-13	13-2-13
E	458	458	13-2-13	13-2-13
H	405	405	13-2-13	13-2-13
G	280	280	13-2-13	13-2-13

UNFACTORED REACTIONS

1ST LOASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT COMBINED	SNOW	LIVE	PERM LIVE	
B	305	212 / 0	0 / 0	93 / 0
E	322	225 / 0	0 / 0	96 / 0
H	286	189 / 0	0 / 0	97 / 0
G	200	117 / 0	0 / 0	83 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
MEMB.									
FR-TO									
A-B	0 / 15	-91.8	-91.8	0.03 (1)	10.00	H-C	-231 / 0	0.05 (1)	
B-J	-62 / 24	-91.8	-91.8	0.15 (1)	6.25	H-D	-63 / 0	0.02 (1)	
J-C	-192 / 0	-91.8	-91.8	0.32 (1)	6.25	G-D	-157 / 0	0.04 (1)	
C-D	-143 / 0	-91.8	-91.8	0.12 (1)	6.25	I-J	-581 / 0	0.00 (1)	
D-L	-234 / 0	-91.8	-91.8	0.32 (1)	6.25	K-L	-577 / 0	0.00 (1)	
L-E	-69 / 0	-91.8	-91.8	0.14 (1)	6.25				
E-F	0 / 15	-91.8	-91.8	0.03 (1)	10.00				
B-I	0 / 150	-18.5	-18.5	0.26 (1)	10.00				
I-H	0 / 150	-18.5	-18.5	0.26 (1)	10.00				
H-G	0 / 180	-18.5	-18.5	0.16 (1)	10.00				
G-K	0 / 185	-18.5	-18.5	0.27 (1)	10.00				
K-E	0 / 185	-18.5	-18.5	0.27 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 089-14
- TPIC 2014

(56% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (D-L:1), BC=0.27/1.00 (G-K:1), WB=0.05/1.00 (C-H:1), SI=0.45/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.37 (E) (INPUT = 0.90)
JSI METAL = 0.10 (E) (INPUT = 1.00)



Structural component only
DWG# T-2108161

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MitTek Industries, Inc. Sat Mar 20 09:27:55 2021 Page 1
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ALL WEBS 2x3 DRY
 DRY: SEASONED LUMBER

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

TOTAL WEIGHT = 2 X 45 = 89 lb

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD =		39.0	PSF

1ST LOASE MAX./MIN. COMPONENT REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS			FACTORED			WEBS		
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 15	-91.8	-91.8	0.03 (1)	10.00	H-D	-332 / 0	0.11 (1)
B-J	-325 / 0	-91.8	-91.8	0.09 (4)	8.25	H-E	-339 / 0	0.09 (1)
J-C	-211 / 0	-91.8	-91.8	0.17 (1)	8.25	C-H	-399 / 0	0.09 (1)
C-D	0 / 74	-91.8	-91.8	0.18 (1)	10.00	I-J	0 / 148	0.00 (1)
D-E	0 / 74	-91.8	-91.8	0.19 (1)	10.00	K-L	0 / 148	0.00 (1)
E-L	-211 / 0	-91.8	-91.8	0.17 (4)	6.25			
L-F	-326 / 0	-91.8	-91.8	0.09 (4)	6.25			
F-G	0 / 15	-91.8	-91.8	0.03 (1)	10.00			
B-I	0 / 199	-18.5	-18.5	0.07 (4)	10.00			
I-H	0 / 199	-18.5	-18.5	0.20 (4)	10.00			
H-K	0 / 199	-18.5	-18.5	0.20 (4)	10.00			
K-F	0 / 199	-18.5	-18.5	0.07 (4)	10.00			

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCSC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (D-E:1) , BC=0.20/1.00 (H-I:4) ,
WB=0.11/1.00 (D-H:1) , SSI=0.13/1.00 (F-L:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

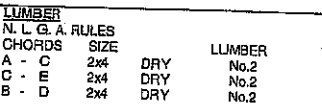
ISI GRIP= 0.26 (D) (INPUT = 0.90)
ISI METAL= 0.11 (C) (INPUT = 1.00)

Structural component only
DWG# T-2108162

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Sat Mar 20 08:52:37 2021 Page 1
ID:wa2i6usWauA15BCJTGM4FzZxO-c3HWA1cVNFeSG329qA?3aRK7p2BS05iC4qn?N8Z63e

Scale = 1:11.3



DRY: SEASONED LUMBER.

PLATES (tablets in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB-1	MT20	3.0	4.0	
C	TT-p	MT20	3.0	4.0	Edge 2.00
D	TMB-1	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INFUT	REQD
JT	GROSS REACTION		GROSS REACTION		BRG	REQD	
	VERT	HORZ	DOWN	HORZ			
B	309	0	309	0	IN-SX	IN-SX	
D	309	0	309	0	4-2-6	4-2-6	
					4-2-6	4-2-6	

UNFACTORED REACTIONS

1ST CASE COMBINED		MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	217	151 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0
B							
D	217	151 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B. D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 16	-91.8	-91.8	0.05 (1)	10.00		
A-G	-264 / 0	-91.8	-91.8	0.05 (4)	6.25	0 / 58	0.00 (1)
G-C	-207 / 0	-91.8	-91.8	0.07 (1)	6.25		
C-D	-207 / 0	-91.8	-91.8	0.07 (1)	6.25	0 / 58	0.00 (1)
I-D	-264 / 0	-91.8	-91.8	0.05 (4)	6.25		
D-E	0 / 16	-91.8	-91.8	0.05 (1)	10.00		
B-F	0 / 193	-18.5	-18.5	0.04 (1)	10.00		
F-H	0 / 193	-18.5	-18.5	0.08 (1)	10.00		
H-D	0 / 193	-18.5	-18.5	0.04 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF 8CBC 2018, ABC 2019
- PART 9 OF QBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07/1.00 (C-I:1), BC=0.08/1.00 (F-H:1),
WB=0.00/1.00 (F-G:1), SS=0.07/1.00 (C-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SI GRIP = 0.22 (D) (INPUT = 0.90)

SI METAL=0.08 (B) (INPUT = 1.00)



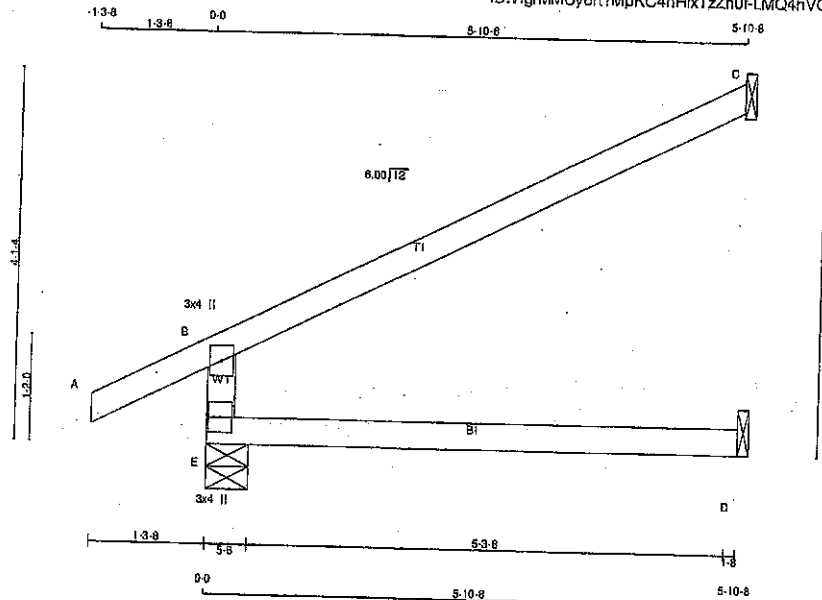
Structural component only
DWG# T-2108134

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417451	J1	22	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 6.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 09:47:08 2021 Page 1
ID:VlgrMMUy6rt?MpkKC4hHixTzZh0f-LMQ4hVQH2GILB/opxeA7IEI?_cZsiTTjFEebh2z25Gx

Scale = 1/20.0



LUMBER

N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
E - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
E BMV1+p	MT20	3.0	4.0		

NOTES:

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	525	0	525	0	5-8	5-8
C	202	0	202	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MITEK STANDARD DETAIL B9779:1H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
E	369	257.0	0.0	0.0	0.0	111.0	0.0
C	139	118.0	0.0	0.0	0.0	28.0	0.0
D	36	0.0	0.0	0.0	0.0	36.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH FR-TO	MAX. FACTORED FORCE (LBS)
FR-TO				
E-B	-481.0	0.0	0.0 0.13 (4)	7.81
A-B	0.28	-91.8	-91.8 0.12 (1)	10.00
B-C	-30.0	-91.8	-91.8 0.54 (1)	6.25
E-D	0.0	-18.5	-18.5 0.13 (4)	10.00

TOTAL WEIGHT = 22 X 17 = 369 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF OSC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.18 (E) (INPUT = 0.90)
JSI METAL = 0.13 (B) (INPUT = 1.00)



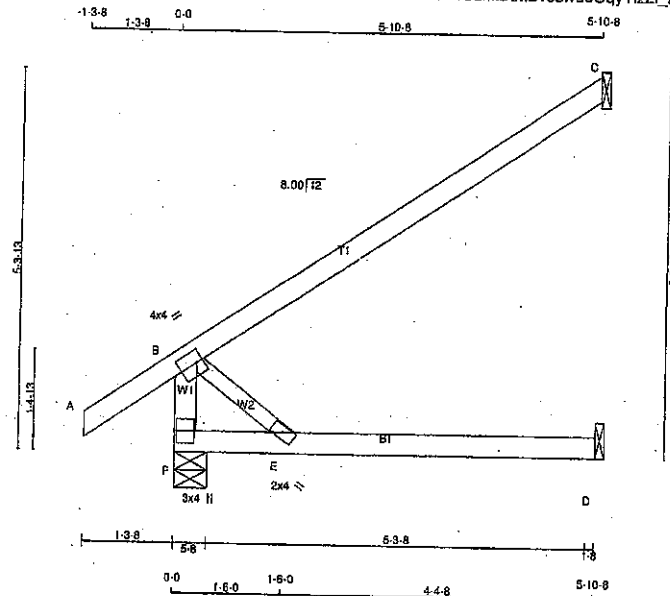
Structural component only
DWG# T-2108176

JOB NAME 417449	TRUSS NAME J11	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 09:27:51 2021 Page 1
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Scale = 1/25.4



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	450	0	450	0	5-8	5-8
C	270	0	270	0	1-8	1-8
D	54	0	61	0	1-8	1-8

SEE MTEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX/MIN COMPONENT REACTIONS				
		SNOW	LIVE	PERM.LIVE	WIND	DEAD
F	318	221.0	0.0	0.0	0.0	95.0
C	186	180.0	0.0	0.0	0.0	35.0
D	43	0.0	0.0	0.0	0.0	43.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PL)	LC1 MAX (PL)	LC2 MAX (PL)			MAX. FACTORED FORCE (LBS)	MAX. (LC)
FR-TO								
F-B	-395.0	0.0	0.0	0.04 (1)	7.81	B-E	0.0	0.00 (1)
A-B	0.35	-91.8	-91.8	0.12 (1)	10.00			
B-C	0.0	-91.8	-91.8	0.54 (1)	10.00			
F-E	0.0	-18.5	-18.5	0.14 (4)	10.00			
E-D	0.0	-18.5	-18.5	0.19 (4)	10.00			

TOTAL WEIGHT = 4 X 19 = 77 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 8.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.54/1.00 (B-C:1), BC=0.19/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

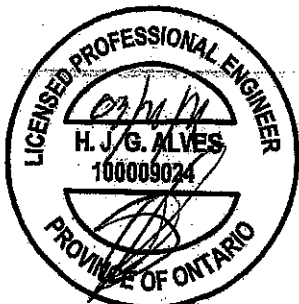
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	850	371	1747

PLATE PLACEMENT TOL = 0.250 inches

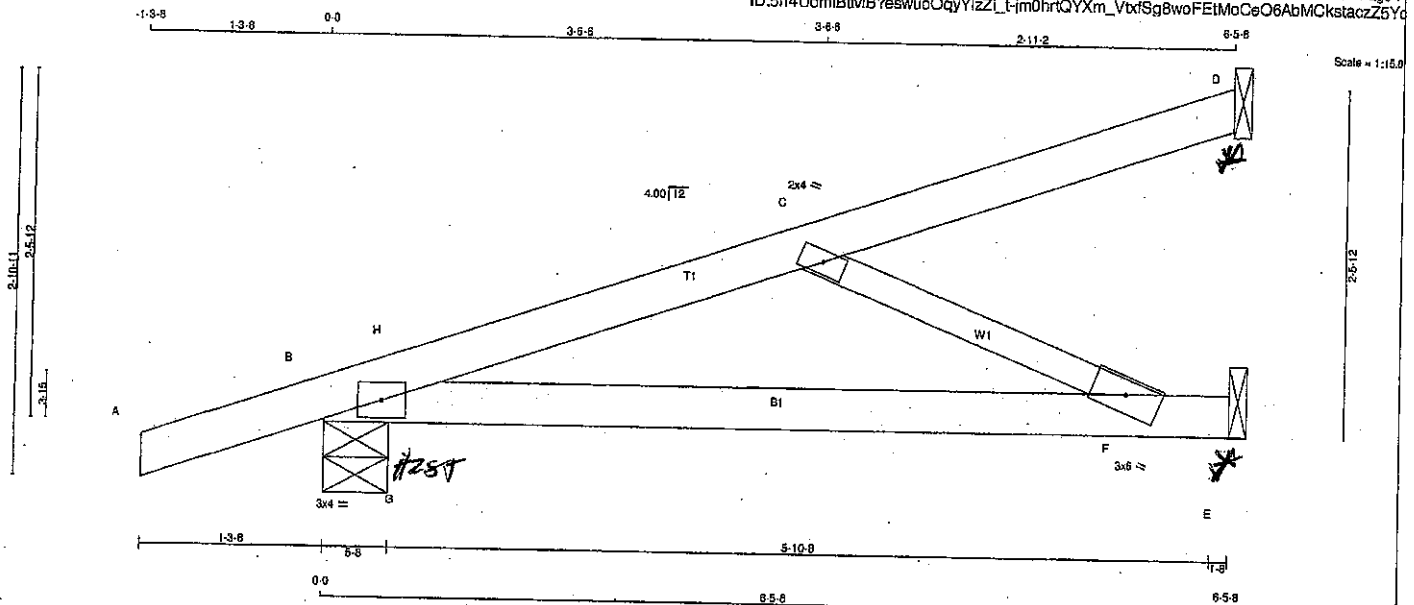
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108157

JOB NAME 417449	TRUSS NAME J12	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 09:27:51 2021 Page 11 ID:5h4UdmlBIMB?eswucQqYizZL_t-m0hrtQYXm_VbtSg8woFEIMoCeO6AbMCKstaczZ5Yc		



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	3.0	4.0
C	TMW-w	MT20	2.0	4.0
F	BMW-w	MT20	3.0	6.0

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
D	110	0	110	0	-70	1-8
B	479	0	479	159	-311	5-8
E	246	0	246	0	-238	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D, E

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 311 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT E FOR 238 LBS FACTORED UPLIFT

PROVIDE FOR 159 LBS FACTORED HORIZONTAL REACTION AT JOINT B

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
D	75	62/0	0/0	0/0	0/-59	14/0	0/0
B	396	234/0	0/0	0/0	0/-288	103/0	0/0
E	176	104/0	0/0	0/0	0/-217	73/0	0/0

HORIZONTAL REACTIONS						
B	0/0	0/0	0/0	113/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID GELING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (12)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
A-B	0/118	-91.8	-91.8	0.11 (1)
B-H	-324/287	-91.8	-91.8	0.23 (5)
H-C	-424/206	-91.8	-91.8	0.22 (5)
C-D	-27/0	-91.8	-91.8	0.08 (1)
B-G	-311/423	-18.5	-18.5	0.10 (12)
G-F	-311/423	-18.5	-18.5	0.31 (1)
F-E	0/0	-18.5	-18.5	0.24 (1)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {9.6} PSF AT (20'-0") FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_p , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0'-0") FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPOC 2014

(66% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.22")
CALCULATED VERT. DEFL. (LL) = L/837 (0.12")
ALLOWABLE DEFL. (TL) = L/360 (0.22")
CALCULATED VERT. DEFL. (TL) = L/606 (0.13")

CSI: TC=0.23/1.00 (B-H:5), BC=0.31/1.00 (F-G:1), WB=0.09/1.00 (C-F:1), SS=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

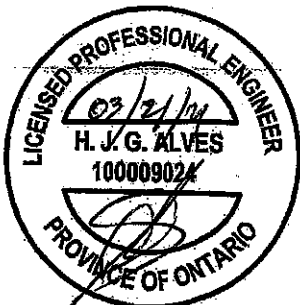
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLJ) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 778 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.39 (C) (INPUT = 0.50)
JSI METAL = 0.24 (C) (INPUT = 1.00)



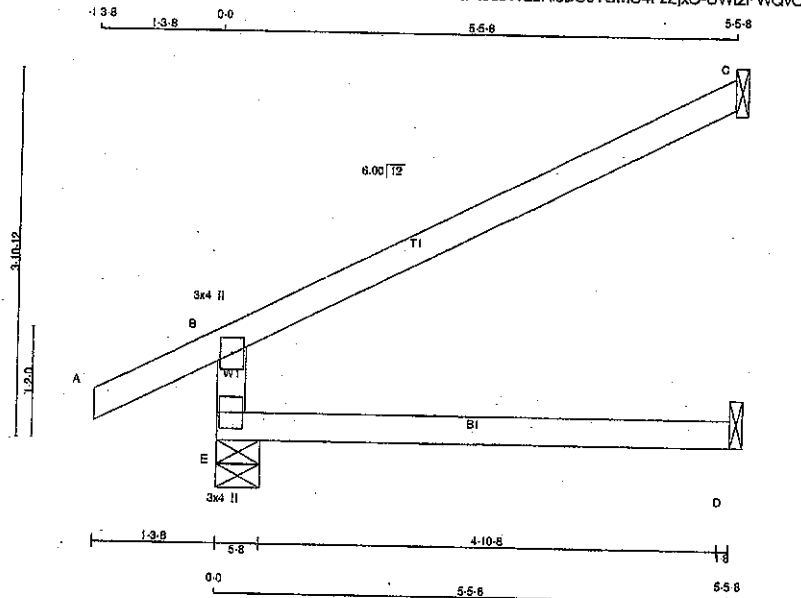
Structural component only
DWG# T-2108158

JOB NAME 417455	TRUSS NAME J31	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 9 Jan 21 2021 M/Tek Industries, Inc. Sat Mar 20 09:58:09 2021 Page 1
ID:wa216usWauAi5BCJTGM4FzJxO-UWltzPWQvOL1Y_KesAyX7xdA_K3Ac4Q6yFaF4OEzZ56C

Scale = 1/22.5



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2	
A - C	2x4	DRY	No.2	
E - D	2x4	DRY	No.2	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	497	0	497	0	5-8	5-8
C	188	0	188	0	1-8	1-8
D	42	0	47	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	348	244.0	0.0	0.0	0.0	104.0	0.0
C	129	105.0	0.0	0.0	0.0	25.0	0.0
D	34	0.0	0.0	0.0	0.0	34.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO
E-B	-437.0	0.0	0.0 0.11 (4)	7.81		
A-B	0.28	-91.8	-91.8 0.12 (1)	10.00		
B-C	-26.0	-91.8	-91.8 0.47 (1)	6.25		
E-D	0.0	-18.5	-18.5 0.11 (4)	10.00		

TOTAL WEIGHT = 3 X 16 = 47 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = .249 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSK: TC=0.47/1.00 (B-C:1), BC=0.11/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

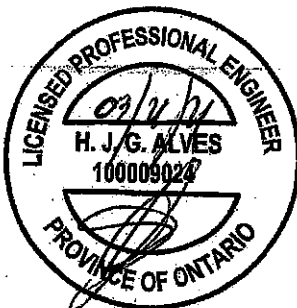
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE, GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 768 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.17 (E) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)



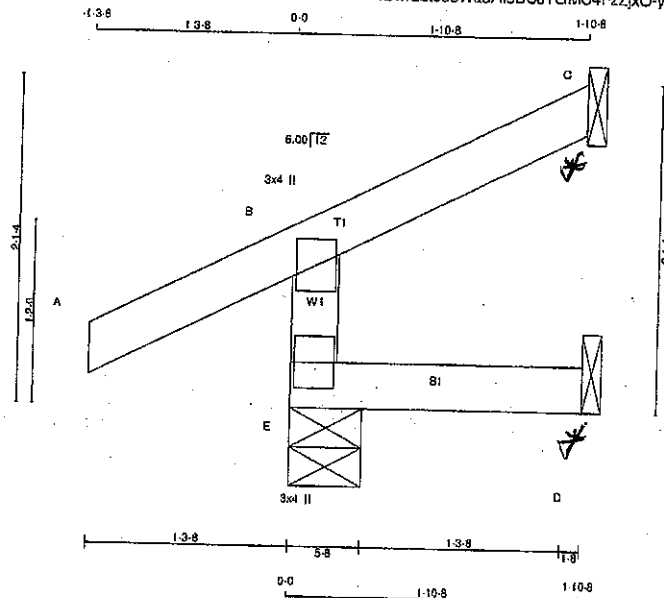
Structural component only
DWG# T-2108198

JOB NAME 417455	TRUSS NAME J32	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 3 Jan 21 2021 MTek Industries, Inc. Sat Mar 20 09:56:10 2021 Page 1
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Scale = 1:12.5



LUMBER

N.L.G.A. RULES	CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2	
A - C	2x4	DRY	No.2	
E - D	2x4	DRY	No.2	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES:

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
E	273	0	273	0	5-8	5-8
C	50	0	50	-20	1-8	1-8
D	8	0	17	-2	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	190	143.0	0.0	0.0	0.0	47.0	0.0
C	35	27.17	0.0	0.0	0.0	7.0	0.0
D	8	0.8	0.0	0.0	0.0	12.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO				
E-B	-247.0	0.0	0.0	0.04 (5)	7.81		
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-16.0	-91.8	-91.8	0.09 (1)	6.25		
E-D	0.0	-18.5	-18.5	0.04 (5)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 7 = 14 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.8 PSF
DL	= 8.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5), WB=0.00/1.00 (n/a:0), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

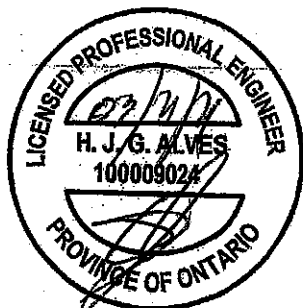
PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MT20	850	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.10 (E) (INPUT = 0.90)

JSI METAL=0.07 (B) (INPUT = 1.00)

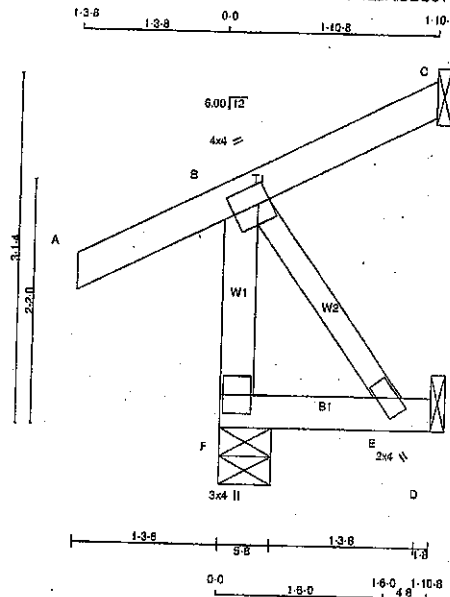


Structural component only
DWG# T-2108199

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417455	J33	2	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Sat Mar 20 09:58:11 2021 Page 1
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Scale = 1:16.7



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
F - B	2x4 DRY	No.2
A - C	2x4 DRY	No.2
F - D	2x4 DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	4.0	2.00 1.25
E	BMVW-w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES:

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT	REORD
	GROSS REACTION	GROSS REACTION	GROSS REACTION	GROSS REACTION		
F	273	0	273	0	5-8	5-8
C	41	0	41	0	1-8	1-8
D	17	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	190	143 '0	0 '0	0 '0	0 '0	47 '0	0 '0
C	28	23 '25	0 '0	0 '0	0 '0	5 '0	0 '0
D	14	0 '0	0 '0	0 '0	0 '0	14 '0	0 '0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED		MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
		VERT. LOAD (PLF)	CSI (LC)				
F-B	-256.0	0.0	0.0	7.81	B-E	0.0	0.00 (1)
A-B	0.28	-91.8	-91.8	10.00			
B-C	-20.0	-91.8	-91.8	6.25			
F-E	0.0	-18.5	-18.5	10.00			
E-D	0.0	-18.5	-18.5	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 10 = 20 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF ORC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SS=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.15 (B) (INPUT = 0.90)
JSI METAL = 0.04 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108200

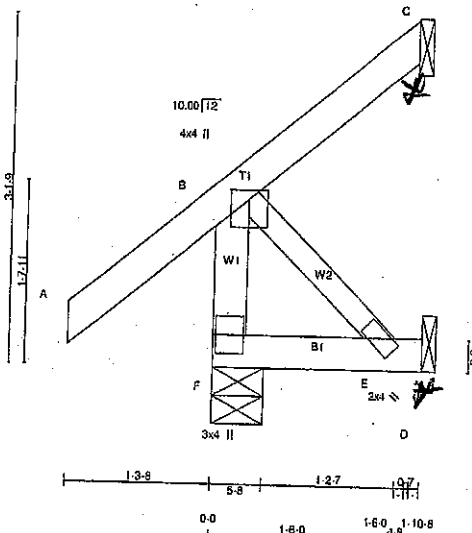
JOB NAME 417451	TRUSS NAME C1	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 09:47:05 2021 Page 1
ID:VlgrMMUy6rt?MpKC4hHlxTzZh0l-wnly2TNPmLnmKHIEGVdQ1bgbBOZuV6kDZGPx4izZ5Ga

1-3.8 1-3.8 0-0 1-2.7 1-9.7 1-10.8

Scale = 1:18.6



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
E BMW+w	MT20	2.0	4.0		
F BMV1+p	MT20	3.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	276	0	276	0
C	33	0	33	0
D	17	0	19	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE			
F	192	144 / 0	0 / 0	0 / 0	48 / 0	0 / 0
C	23	18 / -27	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED REACTIONS	FACTORED VERT. LOAD	MAX. FACTORED VERT. LOAD	MAX. FACTORED VERT. LOAD	MAX. FACTORED VERT. LOAD	MAX. FACTORED VERT. LOAD	MAX. FACTORED VERT. LOAD	MAX. FACTORED VERT. LOAD	MAX. FACTORED VERT. LOAD
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	VERT. LOAD (LBS)	VERT. LOAD (LBS)	VERT. LOAD (LBS)	VERT. LOAD (LBS)	VERT. LOAD (LBS)	VERT. LOAD (LBS)	VERT. LOAD (LBS)
FR-TO									
F-B	-258 / 0	0.0	0.0	0.03 (1)	7.81	0.0	0.0	0.0	0.0
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	0.0	0.0	0.0	0.0
B-C	-32 / 0	-91.8	-91.8	0.12 (1)	6.25	0.0	0.0	0.0	0.0
F-E	0 / 0	-18.5	-18.5	0.02 (4)	10.00	0.0	0.0	0.0	0.0
E-D	0 / 0	-18.5	-18.5	0.01 (4)	10.00	0.0	0.0	0.0	0.0

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 10 = 20 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 8.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (E-F:4),
WB=0.00/1.00 (B-E:1), SS=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

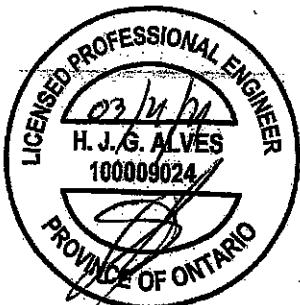
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 768 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.19 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108172

Tamarack Roof Truss, Burlington

Scale = 1:27.4



LUMBER
No.2
No.2
No.2

DESCR.
SPF
SPF
SPF

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMW1+p	MT20	3.0	4.0		

1) Lateral braces to be a minimum of 2X4 SPF #2.

BEARINGS

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

IT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS
	COMBINED	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

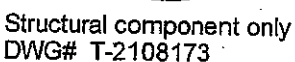
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

TOTAL LOAD CASES: (5)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

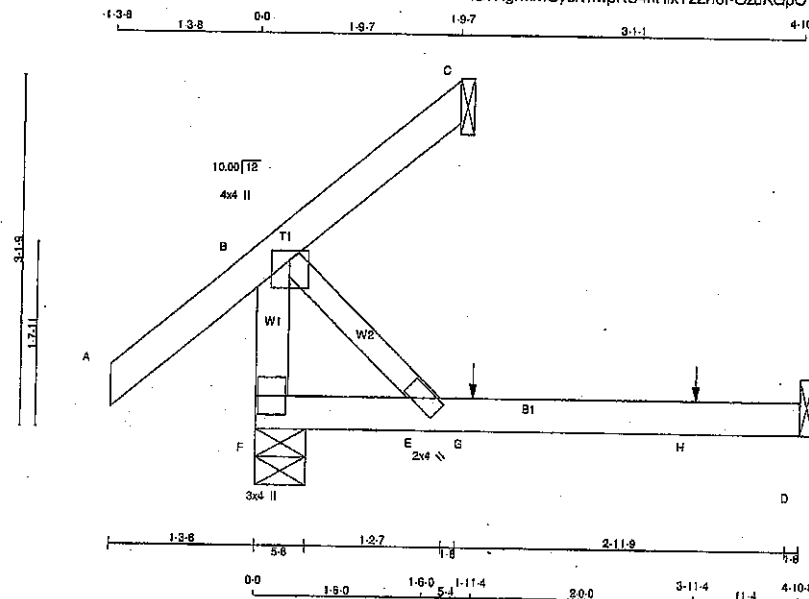


JOB NAME 417451	TRUSS NAME C3	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mittek Industries, Inc. Sat Mar 20 09:47:06 2021 Page 1
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Scale = 1:16.8



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMVW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES-

- (1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
F	303	303	0	0
C	33	33	0	0
D	45	51	0	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOSE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
F	214	144	0	0	70	0
C	23	18	0	0	4	0
D	36	0	0	0	36	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH FR-TO	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO
F-B	-258	0	0.0	0.03 (1)	7.81	0	0.00 (1)
A-B	0/41	-91.8	-91.8	0.13 (1)	10.00		
B-C	-32	0	-91.8	-91.8	0.12 (1)	6.25	
F-E	0/0	-18.5	-18.5	0.11 (4)	10.00		
E-G	0/0	-18.5	-18.5	0.13 (4)	10.00		
G-H	0/0	-18.5	-18.5	0.13 (4)	10.00		
H-D	0/0	-18.5	-18.5	0.13 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1		FRONT	VERT	TOTAL		C1
H	3-11-4	1	1		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN.CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.13/1.00 (A-B:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	850	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)

JSI METAL= 0.05 (B) (INPUT = 1.00)

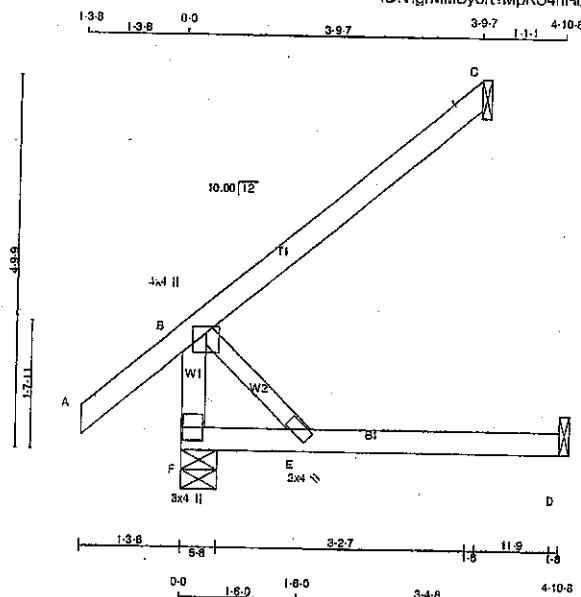


Structural component only
DWG# T-2108174

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417451	C4	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 09:47:07 2021 Page 1
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Scale = 1/2" = 1'-0"

LUMBER

N. L. G. A. RULES

CHORDS SIZE

F - B 2x4 DRY No.2

A - C 2x4 DRY No.2

F - D 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

DRY: SEASONED LUMBER.

LUMBER

DESCR.

SPF

SPF

SPF

SPF

SPF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	MAXIMUM FACTORED GROSS REACTION DOWN	INPUT BRG IN-SX	REQRD BRG IN-SX
F	346	0	0	0
C	174	0	0	0
D	45	0	0	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	243	168.0	0.0	0.0	0.0	75.0	0.0
C	120	97.0	0.0	0.0	0.0	23.0	0.0
D	35	0.0	0.0	0.0	0.0	36.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. HORIZ. LOAD (LC2)	MAX. UNBRACED LENGTH (FR-TO)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. HORIZ. LOAD (LC2)
FR-TO										
F-B		-301.0	0.0	0.0	0.03 (1)	7.81	B-E		0.0	0.00 (1)
A-B		0.0	-91.8	-91.8	0.13 (1)	10.00				
B-C		0.0	-91.8	-91.8	0.22 (1)	10.00				
F-E		0.0	-18.5	-18.5	0.11 (4)	10.00				
E-D		0.0	-18.5	-18.5	0.13 (4)	10.00				

TOTAL WEIGHT = 2 X 16 = 32 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8 PSF
DL = 8.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 39.0 PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.22/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.09/1.00 (B-E:1), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

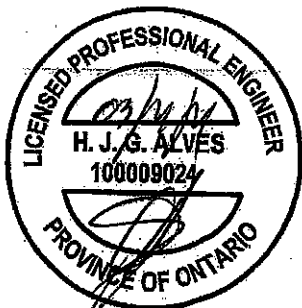
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.22 (B) (INPUT = 0.90)
JSI METAL=0.06 (B) (INPUT = 1.00)

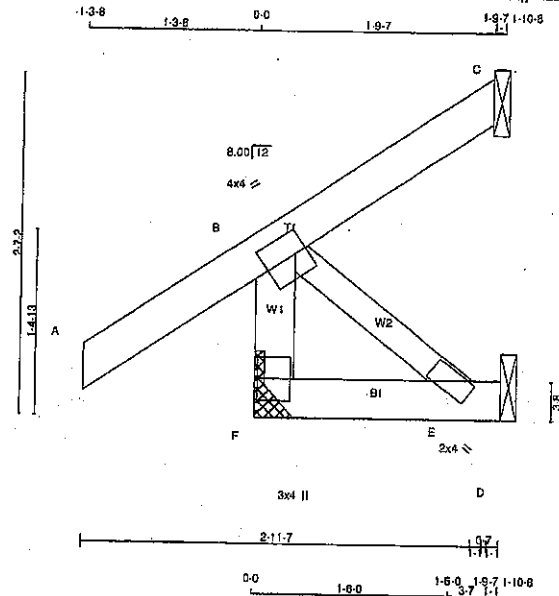


Structural component only
DWG# T-2108175

JOB NAME 417449	TRUSS NAME C11	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale: 3/4"=1'

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-I	MT20	4.0	4.0	2.00	1.00
E BMW-w	MT20	2.0	4.0		
F BMW1-p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
F	274	274	0	0
C	34	34	0	0
D	17	19	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
F	190	143 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	23	19 / 27	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (LC)	MAX. FACTORED UNBRACED LENGTH (FR-TO)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LC)
FR-TO						FR-TO			
F-B		-256 / 0	0.0	0.0	0.03 (1)	B-E		0 / 0	0.00 (1)
A-B		0 / 35	-91.8	-91.8	0.12 (1)				
B-C		-27 / 0	-91.8	-91.8	0.12 (1)				
F-E		0 / 0	-18.5	-18.5	0.02 (4)				
E-D		0 / 0	-18.5	-18.5	0.01 (4)				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 9 lb

(M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF CBC 2012 (2015 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(58 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = 1/959 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = 1/959 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.16 (B) (INPUT = 0.80)

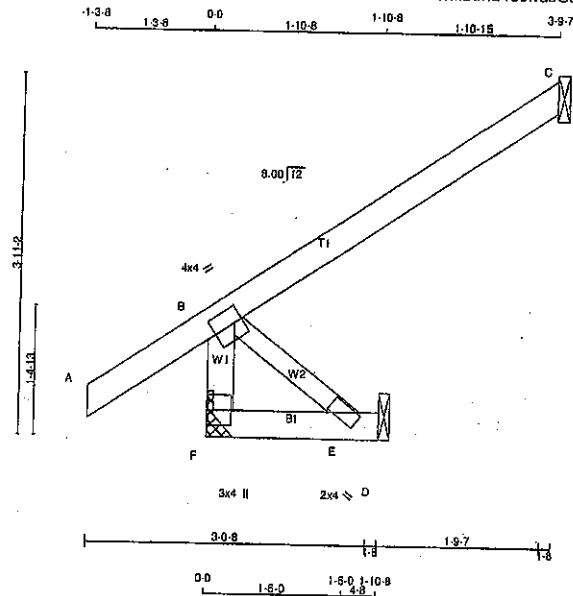
JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108153

JOB NAME 417449	TRUSS NAME C12	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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Scale = 1:22.9

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	4.0	2.00	1.00	
E	BMVW-1w	MT20	2.0	4.0			
F	BMV1+1p	MT20	3.0	4.0			

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	317	0	317	0
C	174	0	174	0
D	17	0	19	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS	FACTORED	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
F	220	167/0	0/0
C	120	97/0	0/0
D	14	0/0	0/0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO					FR-TO		
F-B	-300/0	0.0	0.03 (1)	7.81	B-E	0/0	0.00 (1)
A-B	0/35	-91.8	-91.8 0.13 (5)	10.00			
B-C	0/0	-91.8	-91.8 0.22 (1)	10.00			
F-E	0/0	-18.5	-18.5 0.02 (4)	10.00			
E-D	0/0	-18.5	-18.5 0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 12.5

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(56% OF 21.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.18")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.18")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.00")

CSI: TC=0.22/1.00 (B-C:1), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL-VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.30)
JSI METAL = 0.06 (B) (INPUT = 1.00)



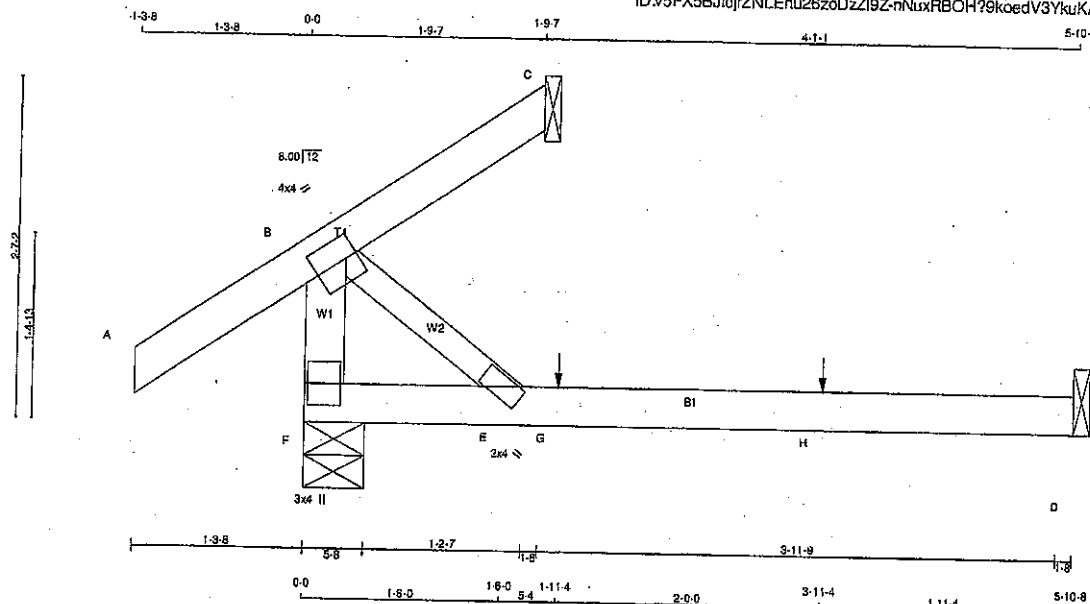
Structural component only
DWG# T-2108154

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417449	C13	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale: 3/4"=1'



LUMBER

N. L. G. A. RULES

CHORDS SIZE

F - B 2x4 DRY LUMBER No.2

A - C 2x4 DRY No.2

F - D 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REQD BRG

JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX

F 311 0 311 0 0 5-8 5-8

C 34 0 34 0 0 1-8 1-8

D 54 0 61 0 0 1-8 1-8

TOTAL WEIGHT = 14 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF

BOT CH. LL = 6.0 PSF

DL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

NOTES

(1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST CASE MAX. MIN. COMPONENT REACTIONS

JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL

F 220 143.0 0.0 0.0 0.0 77.0 0.0

C 23 19.0 0.0 0.0 0.0 4.0 0.0

D 43 0.0 0.0 0.0 0.0 43.0 0.0

SPECIFIED CONCENTRATED LOADS (LBS)

JT LOC. LC1 MAX. MAX. FACE DIR. TYPE HEEL CONN.

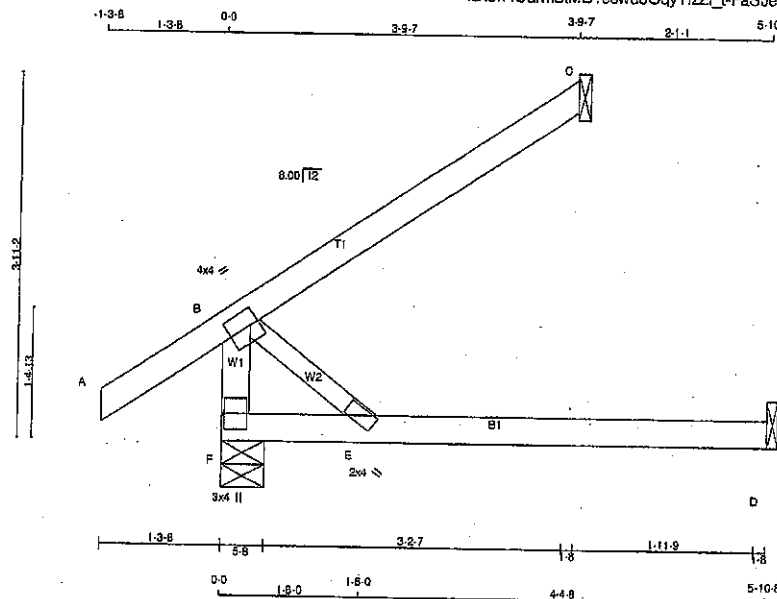
G 1-11.4 1 1 -- FRONT VERT TOTAL -- C1

H 3-11.4 1 1 -- FRONT VERT TOTAL -- C1

JOB NAME 417449	TRUSS NAME C14	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1/22.7



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+1	MT20	4.0	4.0	2.00	1.00
E	BMVW+1	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	354	0	354	0
C	174	0	174	0
D	54	0	54	0

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	250	167.0	0.0	0.0	0.0	83.0	0.0
C	120	97.0	0.0	0.0	0.0	23.0	0.0
D	48	0.0	0.0	0.0	0.0	43.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)	UNBRAC	LENGTH	FR-TO	WEBS	MAX. FACTORED	FORCE	MAX	CS1 (LC)
MEMB.	FORCE (LBS)	VERT. (PLF)	FROM	TO	0.0	0.0	0.03 (1)	7.81	B-E	0.0	0.0	0.0	0.0	0.0 (1)
F-B	-300.0	0.0	0.0	0.0	0.03 (1)	7.81	B-E	0.0	0.0	0.0	0.0	0.0	0.0	0.0 (1)
A-B	0.35	-91.9	-91.8	0.12 (1)	10.00									
B-C	0.0	-91.8	-91.8	0.22 (1)	10.00									
F-E	0.0	-18.5	-18.5	0.14 (4)	10.00									
E-D	0.0	-18.5	-18.5	0.19 (4)	10.00									

TOTAL WEIGHT = 16 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.22/1.00 (B-C:1), BC=0.19/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), CSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.19 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)

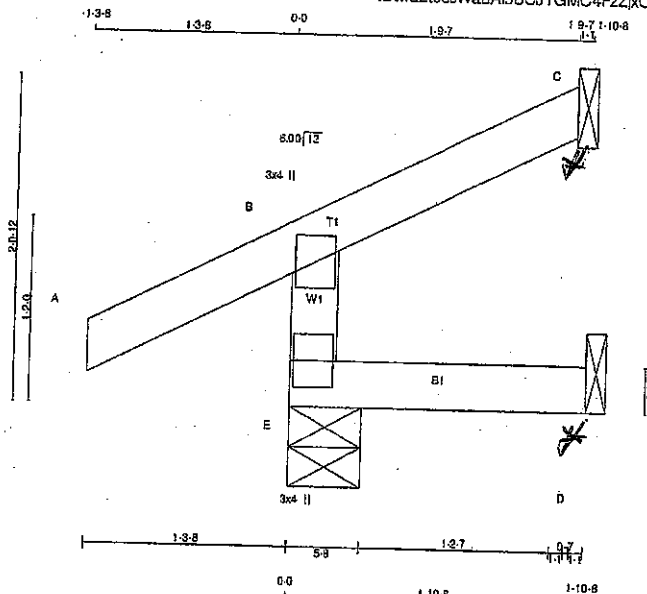


Structural component only
DWG# T-2108156

JOB NAME 417443	TRUSS NAME C31	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:12.5

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4 DRY	No.2		
A - C	2x4 DRY	No.2		
E - D	2x4 DRY	No.2		

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
E	271	0	271	0	0	5-8	5-8	
C	45	0	45	0	-23	1-8	1-8	
D	8	0	17	0	-2	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	188	141 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	31	24 / -18	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	7	0 / -8	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD		MAX. VERT. LOAD (LBS)	MAX. HORZ. LOAD (LBS)	MAX. UNBRACED LENGTH	WEBS MAX. FACTORED FORCE (LBS)
		FROM	TO				
E-B	-244 / 0	0.0	0.0	0.04 (5)	7.81		
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-17 / 0	-91.8	-91.8	0.09 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.04 (5)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 5 X 7 = 35 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2015

- PART 9 OF NBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")

CALCULATED VERT. DEFL.(LL) = L/999 (0.00")

ALLOWABLE DEFL.(TL) = L/360 (0.19")

CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5), WB=0.00/1.00 (n/a:0), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

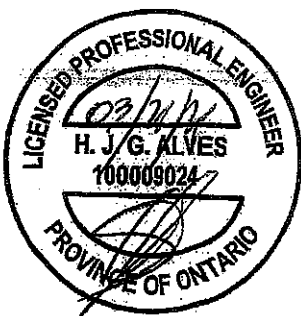
MT20 950 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.10 (E) (INPUT = 0.80)

JSI METAL = 0.07 (B) (INPUT = 1.00)

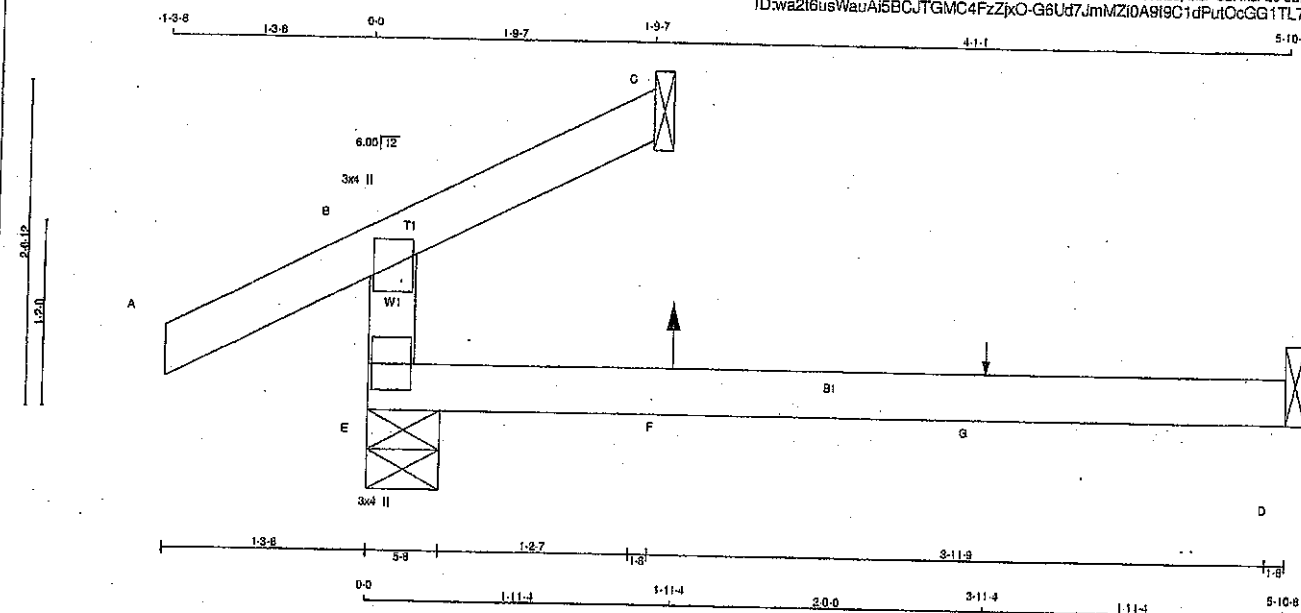


Structural component only
DWG# T-2108126

JOB NAME 417443	TRUSS NAME C33	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 08:52:32 2021 Page 1
ID:wa216usWauA15BCJT GMC4FzZpXO-G8Ud7JmMZIOA91C1dPutCcGG1TL7qTTxZ3EiwzZ63

Scale = 1:13.4



TOTAL WEIGHT = 5 X 12 = 58 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	No.2
E - B	2x4	DRY	No.2			
A - C	2x4	DRY	No.2			
E - D	2x4	DRY	No.2			

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQRD
JT	GROSS REACTION		GROSS REACTION		BRG	BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	284	0	284	0	0	5-8	5-8
C	83	0	83	0	0	1-8	1-8
D	44	0	52	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B87791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	200	137.0	0.0	0.0	0.0	62.0	0.0
C	46	21.0	0.0	0.0	0.0	25.0	0.0
D	35	0.0	0.0	0.0	0.0	37.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO									
E-B	-227.0	0.0	0.0	0.11 (4)	7.81				
A-B	0.28	-91.8	-91.8	0.12 (1)	10.00				
B-C	-9.0	-91.8	-91.8	0.08 (4)	10.00				
E-F	0.0	-18.5	-18.5	0.14 (4)	10.00				
F-G	0.0	-18.5	-18.5	0.14 (4)	10.00				
G-D	0.0	-18.5	-18.5	0.14 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	5	1	8	FRONT	VERT	TOTAL	---	C1
G	3-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) CT: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	8.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.04")

CSI: TC=0.12/1.00 (A-B:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (r/a:0), SS=0.09/1.00 (A-B:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.09 (E) (INPUT = 0.90)

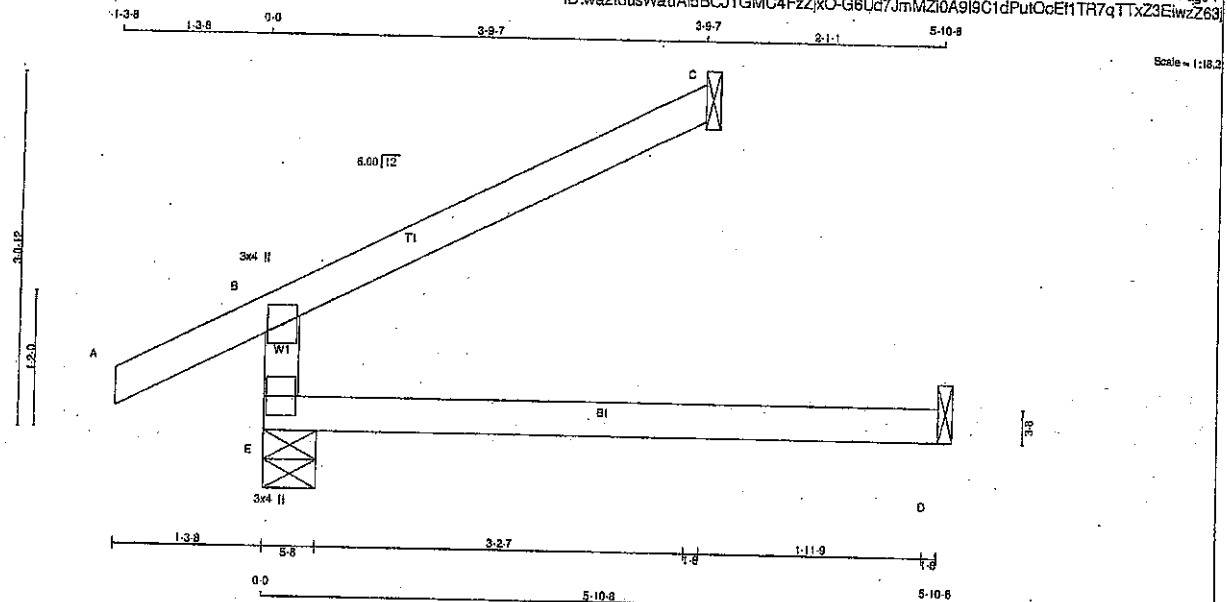
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108128

JOB NAME 417443	TRUSS NAME C34	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 6 Jan 21 2021 MITek Industries, Inc. Sat Mar 20 08:52:32 2021 Page 1
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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4 DRY	No.2		
A - C	2x4 DRY	No.2		
E - D	2x4 DRY	No.2		

DRY: SEASONED LUMBER.

PLATES (tablets in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	405	0	405	0	5-8	5-8
C	130	0	130	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT E C D	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	288	190.0	0.0	0.0	0.0	98.0	0.0
	90	73.0	0.0	0.0	0.0	17.0	0.0
	38	0.0	0.0	0.0	0.0	36.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			FACTORED			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)		VERT. LOAD LC1 (PLF)	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO			FROM. TO	CSI (LC)				CSI (LC)
E-B	-342	0	0.0	0.0	0.13 (4)	7.81		
A-B	0	28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-19	0	-91.8	-91.8	0.22 (1)	6.25		
E-D	0	0	-18.5	-18.5	0.13 (4)	10.00		

TOTAL WEIGHT = 5 X 14 = 71 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.22/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WS=0.00/1.00 (N/A:0), SS=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

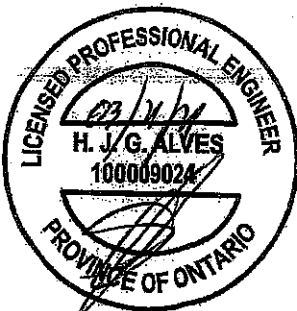
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (FLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 768 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

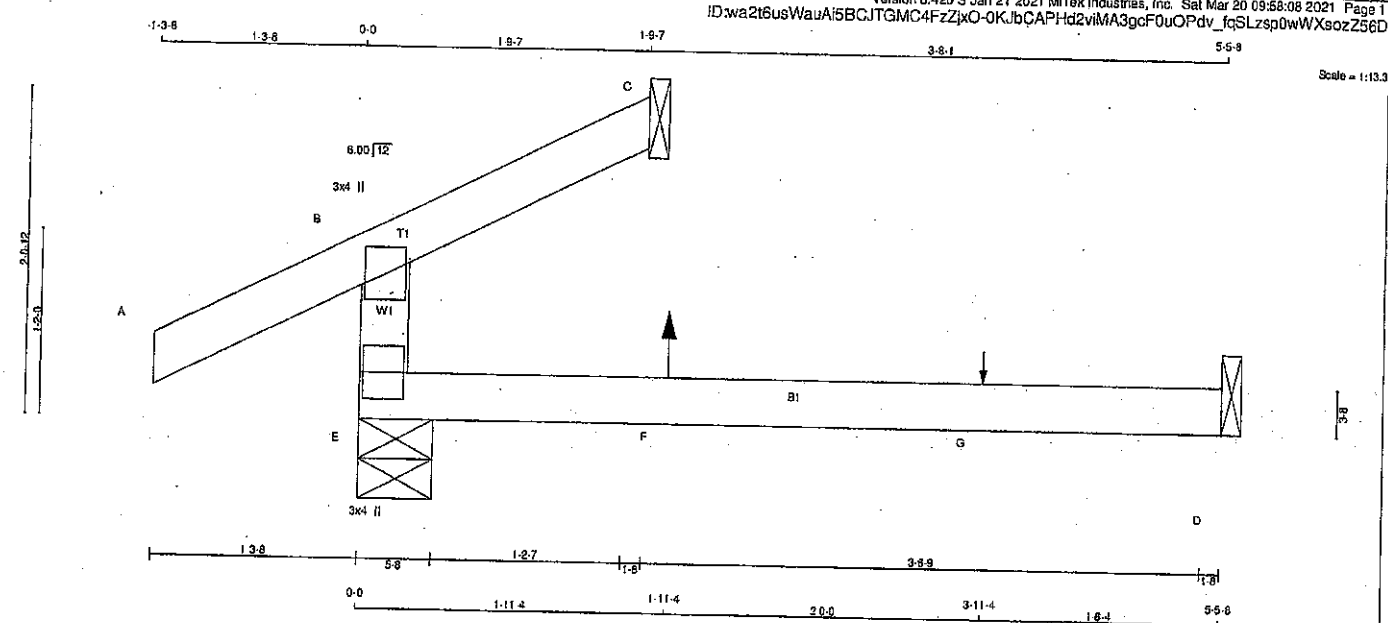
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.14 (E) (INPUT = 0.90)
JSI METAL = 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108129

JOB NAME 417455	TRUSS NAME C35	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 09:58:08 2021 Page 1 ID:wa2t6usWauA/5BCJTGMC4FzZjxO-0KJbCAPHd2viMA3gcF0uOPdv_fcSLzsp0wWXsozZ56D			



LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN. Y	X
B TMV+p	MT20	3.0	4.0	
E BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQ'D BRG
E	283	283	5-8	5-8
C	59	59	1-8	1-8
D	40	49	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	199	137.0	0.0	0.0	0.0	62.0	0.0
C	43	21.0	0.0	0.0	0.0	22.0	0.0
D	33	0.3	0.0	0.0	0.0	35.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	FACTORED (FL)	MAX. CS (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
FR-TO	E-B	-231.0	0.0	0.0	0.09 (4)	7.81	FR-TO	E-B	-231.0	0.09 (4)
	A-B	0.28	-91.8	-91.8	0.12 (1)	10.00		A-B	0.28	0.12 (1)
	B-C	-11.7	-91.8	-91.8	0.06 (4)	6.25		B-C	-11.7	0.06 (4)
	E-F	0.0	-18.5	-18.5	0.12 (4)	10.00		E-F	0.0	0.12 (4)
	F-G	0.0	-18.5	-18.5	0.12 (4)	10.00		F-G	0.0	0.12 (4)
	G-D	0.0	-18.5	-18.5	0.12 (4)	10.00		G-D	0.0	0.12 (4)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11.4	5	1	8	FRONT	VERT	TOTAL	---	C1
G	3-11.4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CS1: TC=0.12/1.00 (A-B:1), BC=0.12/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS1=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

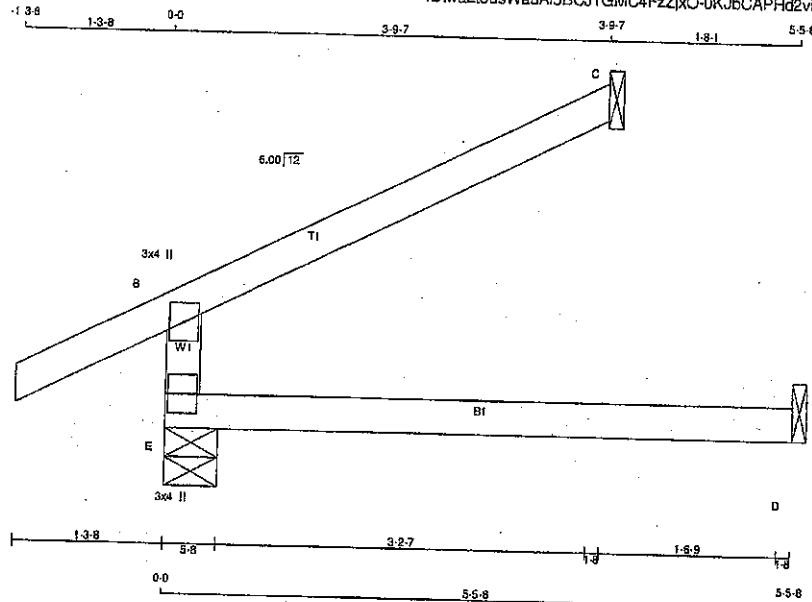
JSI GRIP = 0.09 (E) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108196

JOB NAME 417455	TRUSS NAME C36	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:wa2f6usWauAi5BCJTGMC4FzZjXO-0KJbCAPHd2viMA3goF0uOPdtMfqZLzsp0wWXsozZ56D



TOTAL WEIGHT = 2 X 14 = 27 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (tbl/s is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	401	0	401	0	5-8	5-8
C	130	0	130	0	1-8	1-8
D	42	0	47	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
		SNOW	LIVE				
E	282	190.0	0.0	0.0	0.0	92.0	0.0
C	90	73.0	0.0	0.0	0.0	17.0	0.0
D	34	0.0	0.0	0.0	0.0	34.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED				WEBS			
MEMB.	MAX. FORCE (LBS)	VERT. LOAD	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FORCE (LBS)	FACTORED MAX CSI (LC)	
FR-TO		FROM	TO			FR-TO			
E-B	-342.0	0.0	0.0	0.11 (4)	7.81				
A-B	0.728	-81.8	-91.8	0.12 (1)	10.00				
B-C	-19.0	-91.8	-91.8	0.22 (1)	6.25				
E-D	0.0	-18.5	-18.5	0.11 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 33.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.22/1.00 (B-C:1), BC=0.11/1.00 (D-E:4), WB=0.00/1.00 (na:0), SS=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
	1987	1873

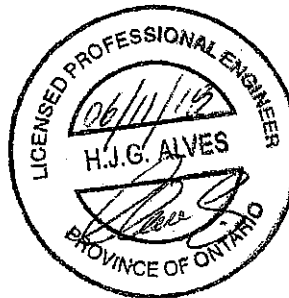
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.14 (E) (INPUT = 0.80)
JSI METAL = 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108197



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-190021B

Feb 09, 2018

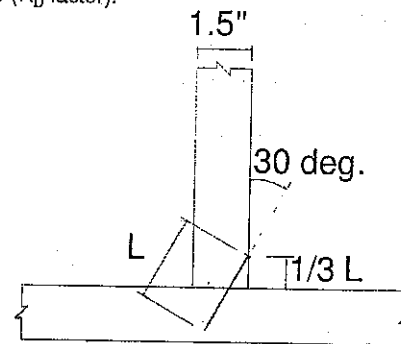
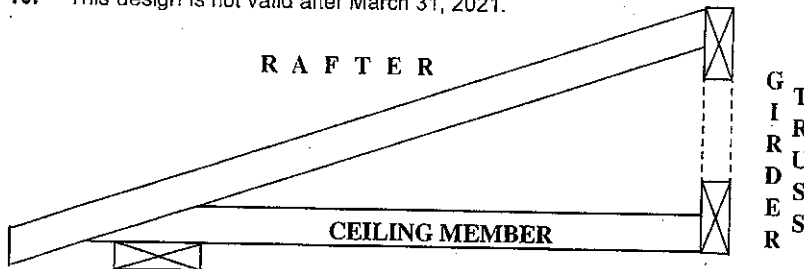
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

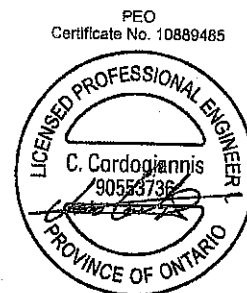
1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.



Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

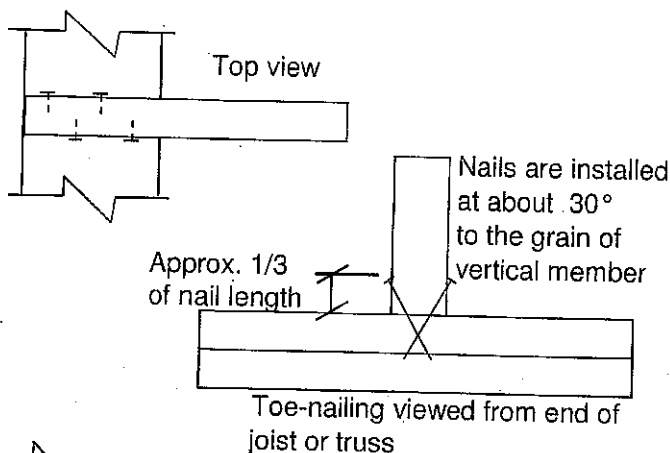
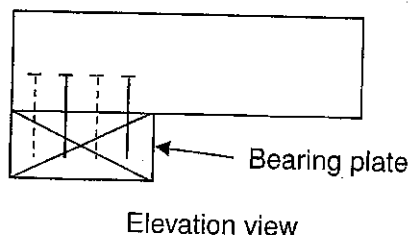
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

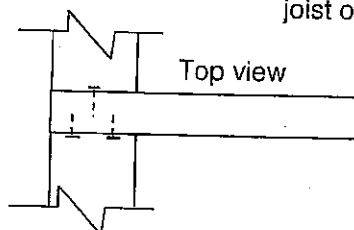
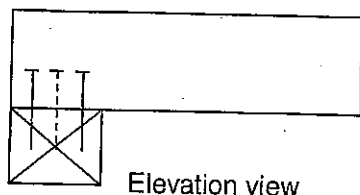
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



December 2, 2019

HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

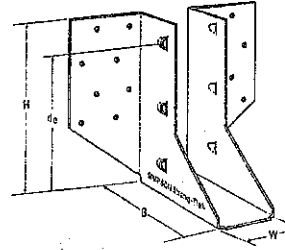
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

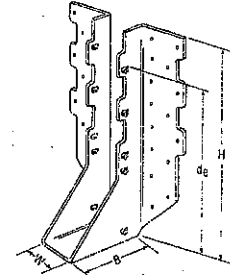
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

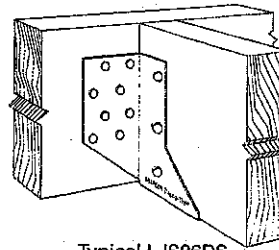
- See current catalogue for options



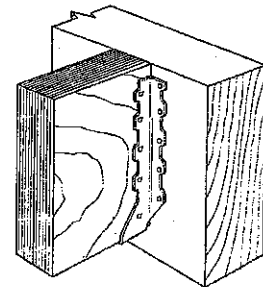
LJS26DS



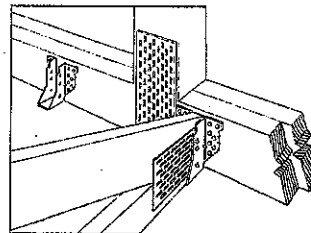
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



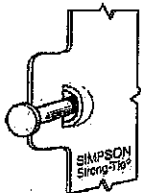
Typical HUS
Installation



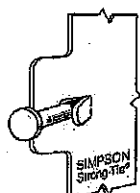
Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _s ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LJS26DS	18	1 1/8	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/2	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

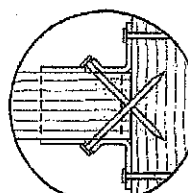
1. d_s is the distance from the seat of the hanger to the highest joist nail.



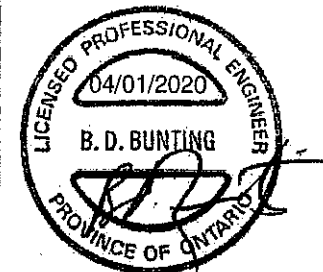
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

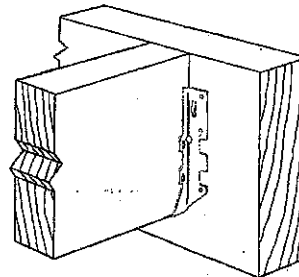
Finish: G90 galvanized

Design:

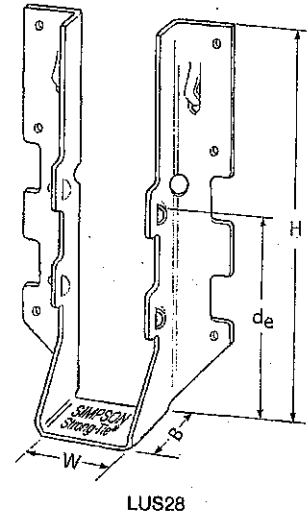
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire,
10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.



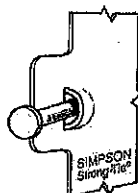
Typical LUS
Installation



LUS28

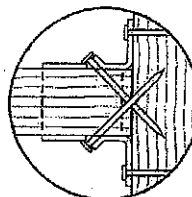
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LUS24	18	1½	3½	1¾	1 11/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1 11/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1½	4¾	1¾	3½	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1½	6¾	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1½	7 11/16	1¾	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4¾	8 11/16	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

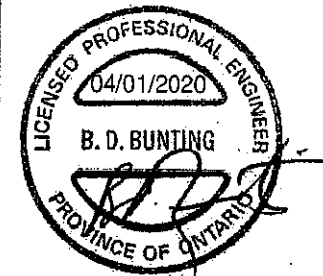


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent
5,603,580



Double Shear Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECLUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

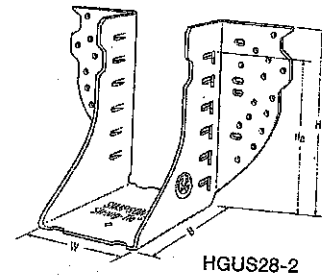
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

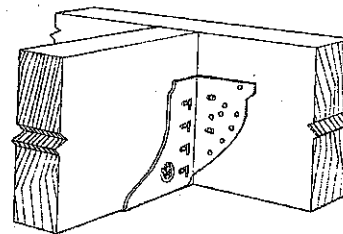
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

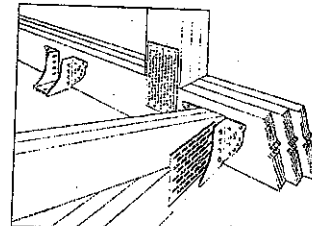
- See current catalogue for options



HGUS28-2



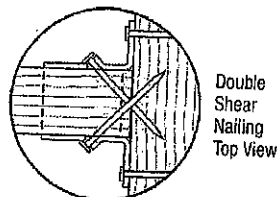
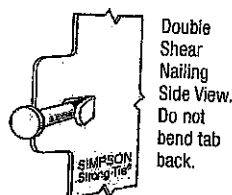
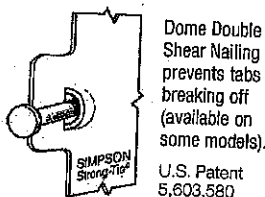
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HGUS26	12	1 1/8	5 1/8	5	4 5/8	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 1/8	5 7/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1 1/8	9 1/8	5	7 1/8	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(58) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHGUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

H – Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.
Material: 18 gauge **Finish:** G90 galvanized

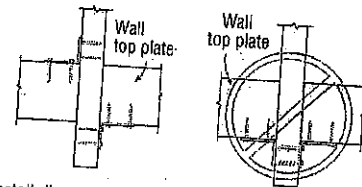
Design: • Factored resistances are in accordance with CSA O86-14
• Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: $8d = 0.131"$ dia. x $2\frac{1}{2}"$ long common wire, $8d \times 1\frac{1}{2}" = 0.131" \times 1\frac{1}{2}"$ long, $10d \times 1\frac{1}{2}" = 0.146" \times 1\frac{1}{2}"$ long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

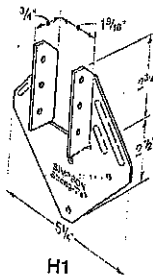
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0 .

Hurricane Tie Installations to Achieve Twice the Load (Top View)

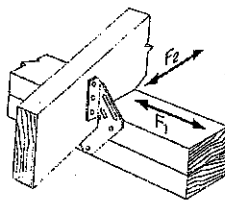


Install diagonally across from each other for minimum 2x truss.

Nailing into both sides of a single ply 2x truss may cause the wood to split.



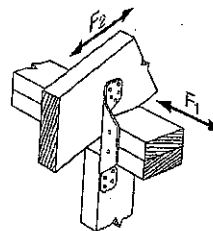
H1



H1 Installation



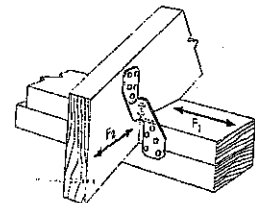
H2A



H2A Installation



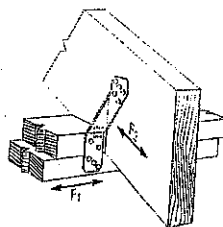
H2.5A



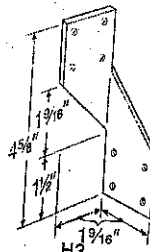
H2.5A Installation



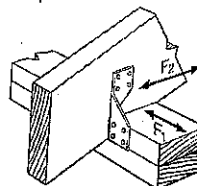
H2.5T



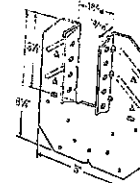
H2.5T Installation
(Nails into both top plates)



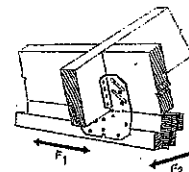
H3



H3 Installation



H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
		To Rafter	To Plates	To Studs	D.Fir-L			S-P-F		
					Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
H1	18	(6) 8d x 1½"	(4) 8d	—	(K ₀ =1.15)			(K ₀ =1.15)		
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	740	685	300	680	485	215
H2.5A	18	(5) 8d	(5) 8d	—	830	220	75	590	155	55
H2.5T	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H3	18	(4) 8d	(4) 8d	—	835	175	240	740	160	210
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	740	180	265	615	125	190
					1735	795	410	1505	565	290

* Factored resistances have been increased 15% for earthquake or wind.

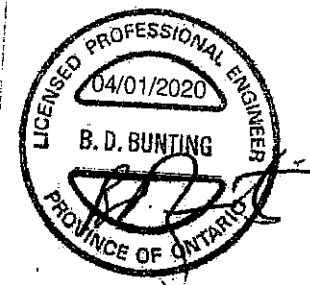
3. When

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.

2. Factored resistances are for one anchor. A minimum rafter thickness of 2 1/2" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.

4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECH20 3/20 exp. 6/22

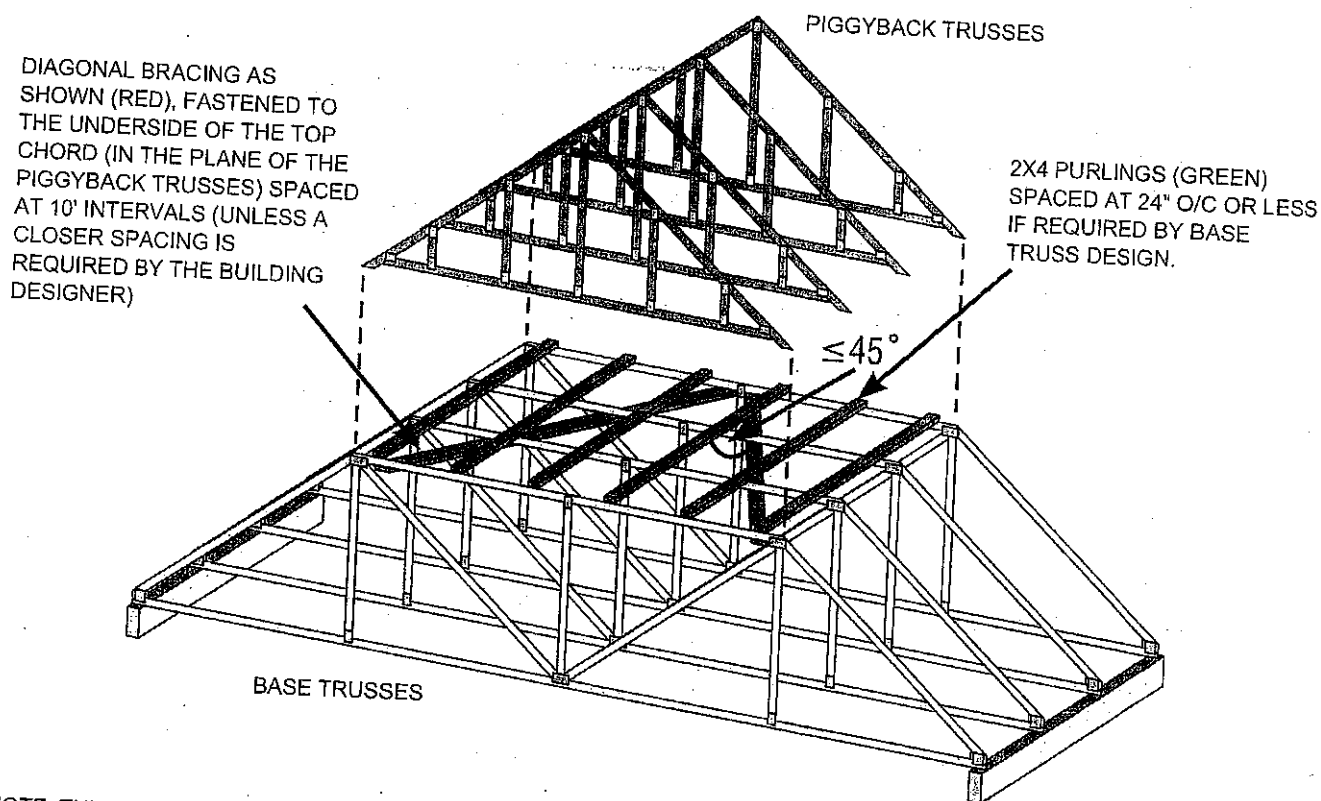
(800) 999-5099
strongtie.com

Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

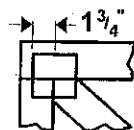
SKETCH FROM BCSI-CANADA 2013

Disclaimer:

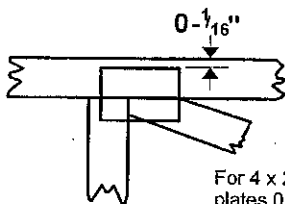
OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- $\frac{1}{16}$ \"/>



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4

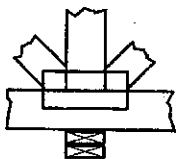
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



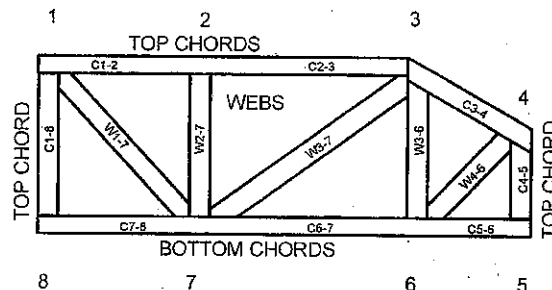
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek

MiTek Engineering Reference Sheet MII-7473 rev. 10/03/2015



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor 1 bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.