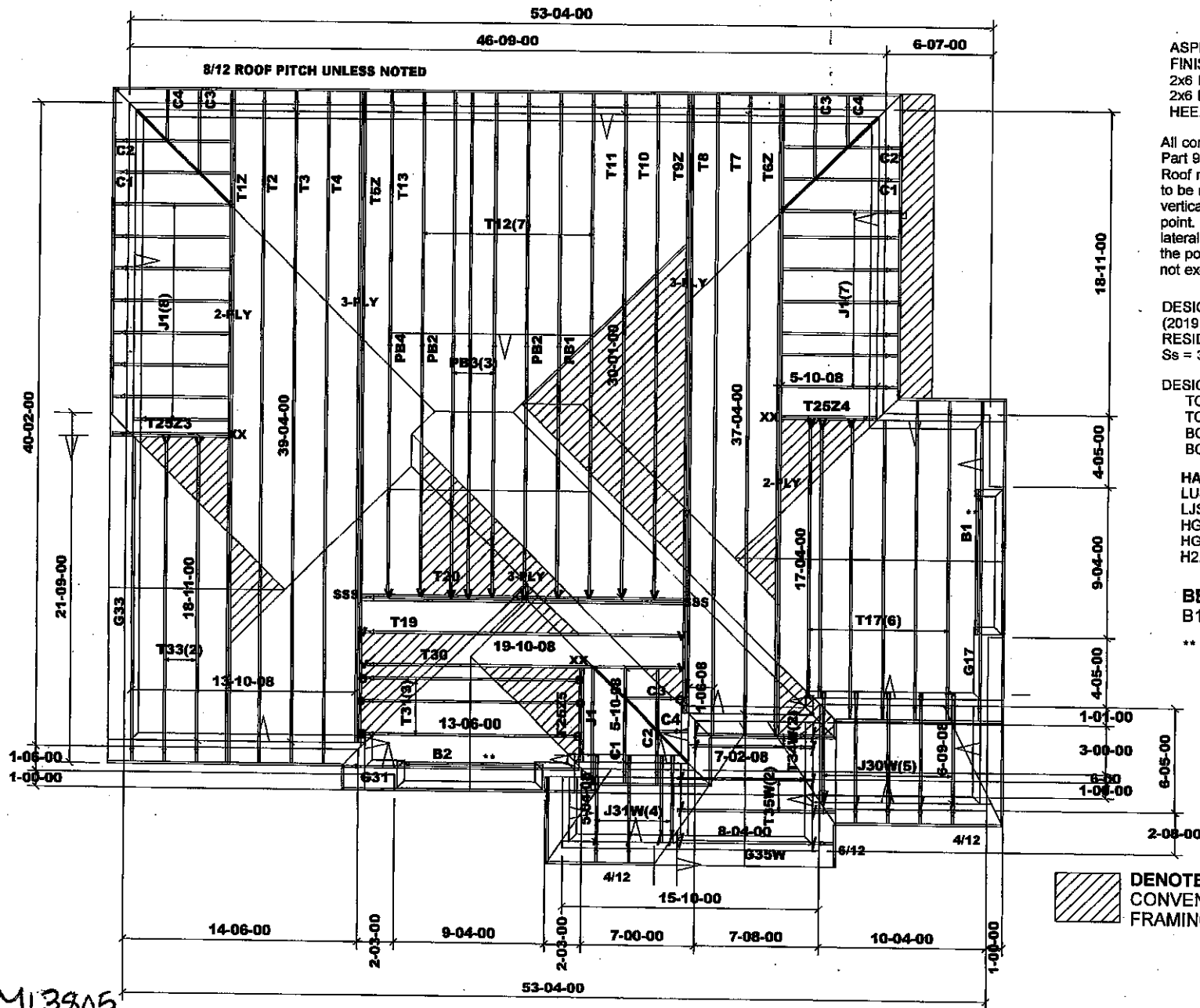




ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
HGUS28-3 (SSS)
H2.5T (I)

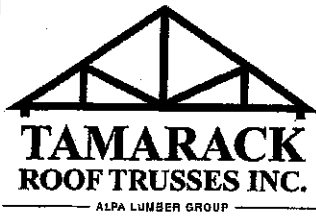
BEAMS:
B1 = B2 = 2 - 2x10 SPF #2

** CONNECT BC OF G17 TO B1 WITH 3 1/2" COMMON SPIRAL NAILS @ 4" O/C IN 2 ROWS

CONNECT BC OF G31 TO B2 WITH 3 1/2" COMMON SPIRAL NAILS @ 4" O/C IN 2 ROWS

 DENOTES: CONVENTIONAL FRAMING

 TAMARACK ROOF TRUSSES INC.	Job Track: 52166	Builder / Location: GREENPARK HOMES / HAMILTON		Model / Elevation: SPRINGFIELD 11 / 2	
	Plan Log: 204505	Project: RUSSELL GARDENS PH.4			
	Layout ID: 417565	Date: 2021-03-25	Sales: William Garcia	Designer: JG	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.
					Milek ver 8.4.2.288



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: SPRINGFIELD 11
 Lot #:
 Elevation: 1

Job Track: 52166
 PlanLog: 204505
 Layout ID: 417564
 Ref #
 Page: 1 of 4
 Date: 03-25-2021
 Designer:
 Sales Rep: William Garcia

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	8 / 12	39-04-00	5-03-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	424.16 254.67		
	1	T2 Hip	8 / 12	39-04-00	6-07-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	195.99 119.67		
	1	T3 Hip	8 / 12	39-04-00	7-11-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	198.12 122.00		
	1	T4 Hip	8 / 12	39-04-00	9-03-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	203.62 123.83		
	1 3-ply	T5 Hip Girder	8 / 12	39-04-00	10-07-15	2 x 6 2 x 8	1-03-08 1-03-08	1-04-13 1-04-13	950.28 562.00		
	1 2-ply	T6 Roof Special Girder	8 / 12	37-04-00	5-03-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	424.02 260.00		
	1	T7 Roof Special	8 / 12	37-04-00	6-07-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	159.97 101.50		
	1	T8 Roof Special	8 / 12	37-04-00	7-11-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	174.36 111.17		
	1 3-ply	T9 Hip Girder	8 / 12	37-04-00	6-06-04	2 x 6 2 x 8	1-03-08 1-03-08	1-04-13 1-04-13	794.51 484.00		
	1	T10 Hip	8 / 12	30-01-00	7-11-02	2 x 4	1-03-08	1-04-13 6-02-13	145.3 92.33		
	1	T11 Hip	8 / 12	30-01-00	9-03-02	2 x 4	1-03-08	1-04-13 6-02-13	148.77 91.00		
	7	T12 Hip	8 / 12	30-01-00	10-07-02	2 x 4	1-03-08	1-04-13 6-02-13	1211.41 751.33		
	1	T13 Piggyback Base	8 / 12	30-01-00	10-07-02	2 x 4	1-03-08	1-04-13 7-06-13	151.08 95.33		
	6	T14S Roof Special	8 / 12 6 / 12	12-06-00	5-06-13	2 x 4		1-04-13 1-04-13	302.6 202.00		



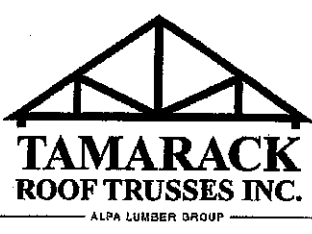
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: SPRINGFIELD 11
 Lot #:
 Elevation: 1

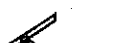
Job Track: 52166
 Plan Log: 204505
 Layout ID: 417564
 Ref #
 Page: 2 of 4
 Date: 03-25-2021
 Designer:
 Sales Rep: William Garcia

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T15 Hip Girder	8 / 12	17-04-00	5-03-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	89.3 55.50		
	1	T16 Hip	8 / 12	17-04-00	6-07-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	80.84 53.17		
	2	T17 Common	8 / 12	17-04-00	7-02-02	2 x 4		1-04-13 1-04-13	146.29 92.00		
	1	T18 Hip Girder	8 / 12 6 / 12	19-10-08	5-03-13	2 x 4 2 x 6		1-02-13 1-07-13	108.09 70.83		
	1	T19 Roof Special	-8 / 12	19-10-08	5-11-13	2 x 4		5-11-13 4-07-13	90.78 58.33		
	1 3-ply	T20 Roof Special Girder	-8 / 12	19-10-08	7-03-13	2 x 6		7-03-13 5-11-13	441.23 289.00		
	3	T21S Roof Special	8 / 12 6 / 12	11-06-00	5-00-13	2 x 4 2 x 6		1-02-13 1-02-13	159.89 114.00		
	1	T22 Common	8 / 12	12-06-00	5-06-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	54.88 35.83		
	2	T23 Common	8 / 12	16-00-00	6-08-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	136.04 87.00		
	2 2-ply	T24 Half Hip Girder	8 / 12	5-10-08	2-10-02	2 x 4 2 x 6		1-04-13 2-10-02	110.87 73.33		
	1 2-ply	T25 Jack-Closed Girder	8 / 12	5-10-08	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	64.23 40.67		
	1 2-ply	T25Z Jack-Closed Girder	8 / 12	5-10-08	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	64.23 40.67		
	1 2-ply	T25Z1 Jack-Closed Girder	8 / 12	5-10-08	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	64.23 40.67		
	1 2-ply	T25Z2 Jack-Closed Girder	8 / 12	5-10-08	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	64.23 40.67		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST			
	Lumber Yard:	TAMARACK LUMBER	Job Track:	52166
Builder:	GREENPARK HOMES	PlanLog:	204505	
Project:	RUSSELL GARDENS PH.4	Layout ID:	417564	
Location:	HAMILTON	Ref #		
Model:	SPRINGFIELD 11	Page:	3 of 4	
Lot #:		Date:	03-25-2021	
Elevation:	1	Designer:		
		Sales Rep:	William Garcia	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	PB1 Piggyback	8 / 12	9-09-01	1-04-00	2 x 4			26.8 18.00		
	2	PB2 Piggyback	8 / 12	9-09-01	2-08-00	2 x 4			54.91 35.00		
	3	PB3 Piggyback	8 / 12	9-09-01	3-03-00	2 x 4			74.28 46.50		
	1	PB4 Piggyback	8 / 12	11-09-01	1-04-00	2 x 4			33.39 22.17		
	14	J1 Jack-Open	8 / 12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	268.67 163.33		
	4	J2 Jack-Open	8 / 12	2-02-00	2-10-02	2 x 4	1-03-08	1-04-13 2-10-02	40.14 28.00		
	2	C1 Jack-Open	8 / 12	3-09-07	3-11-02	2 x 4	1-03-08 2-01-01	1-04-13 3-11-02	32.66 20.67		
	2	C2 Jack-Open	8 / 12	1-09-07	2-07-02	2 x 4	1-03-08 4-01-01	1-04-13 2-07-02	26.93 16.67		
	2	C3 Jack-Open	8 / 12	1-10-08	3-11-02	2 x 4	1-03-08 1-10-15	1-04-13 2-07-13	23.42 15.33		
	4	C4 Jack-Open	8 / 12	1-09-07	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	35.62 22.67		
	2	C5 Jack-Open	8 / 12	1-09-07	2-07-02	2 x 4	1-03-08 4-09	1-04-13 2-07-02	18.47 12.67		
	1	G23 GABLE	8 / 12	16-00-00	6-08-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	71.41 46.17		

TOTAL # TRUSS= 96

TOTAL BFT OF ALL TRUSSES= 4889.68

BFT.

TOTAL WEIGHT OF ALL TRSSES 7865.98 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
8	Hardware	HGUS26-2	
2	Hardware	HGUS28-3	

DELIVERY SHIPLIST		
 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	Lumber Yard: TAMARACK LUMBER	Job Track: 52166
	Builder: GREENPARK HOMES	PlanLog: 204505
	Project: RUSSELL GARDENS PH.4	Layout ID: 417564
	Location: HAMILTON	Ref #
	Model: SPRINGFIELD 11	Page: 4 of 4
	Lot #:	Date: 03-25-2021
	Elevation: 1	Designer:
		Sales Rep: William Garcia

HARDWARE

QTY	TYPE	MODEL	LENGTH
26	Hardware	LJS26DS	
5	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 39



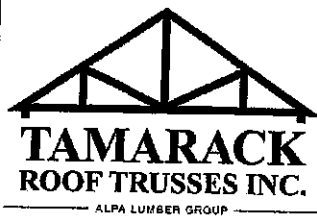
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: SPRINGFIELD 11
 Lot #:
 Elevation: 2

Job Track: 52166
 PlanLog: 204505
 Layout ID: 417565
 Ref #
 Page: 1 of 4
 Date: 03-25-2021
 Designer:
 Sales Rep: William Garcia

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T12 Hip Girder	8 / 12	39-04-00	5-03-15	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	455.67 272.00		
	1	T2 Hip	8 / 12	39-04-00	6-07-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	195.99 119.67		
	1	T3 Hip	8 / 12	39-04-00	7-11-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	198.12 122.00		
	1	T4 Hip	8 / 12	39-04-00	9-03-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	203.62 123.83		
	1 3-ply	T5Z Hip Girder	8 / 12	39-04-00	10-07-15	2 x 6 2 x 8	1-03-08 1-03-08	1-04-13 1-04-13	930.32 554.00		
	1 2-ply	T6Z Roof Special Girder	8 / 12	37-04-00	5-03-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	380.96 236.00		
	1	T7 Roof Special	8 / 12	37-04-00	6-07-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	159.97 101.50		
	1	T8 Roof Special	8 / 12	37-04-00	7-11-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	174.36 111.17		
	1 3-ply	T9Z Hip Girder	8 / 12	37-04-00	6-06-04	2 x 6 2 x 8	1-03-08 1-03-08	1-04-13 1-04-13	794.51 484.00		
	1	T10 Hip	8 / 12	30-01-00	7-11-02	2 x 4	1-03-08	1-04-13 6-02-13	145.3 92.33		
	1	T11 Hip	8 / 12	30-01-00	9-03-02	2 x 4	1-03-08	1-04-13 6-02-13	148.77 91.00		
	7	T12 Hip	8 / 12	30-01-00	10-07-02	2 x 4	1-03-08	1-04-13 6-02-13	1211.41 751.33		
	1	T13 Piggyback Base	8 / 12	30-01-00	10-07-02	2 x 4	1-03-08	1-04-13 7-06-13	151.08 95.33		
	6	T17 Common	8 / 12	17-04-00	7-02-02	2 x 4		1-04-13 1-04-13	438.87 276.00		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: SPRINGFIELD 11
 Lot #:
 Elevation: 2

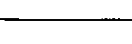
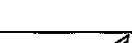
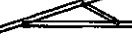
Job Track: 52166
 PlanLog: 204505
 Layout ID: 417565
 Ref #
 Page: 2 of 4
 Date: 03-25-2021
 Designer:
 Sales Rep: William Garcia

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	G17 GABLE	8 / 12	17-04-00	7-02-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	78.27 50.17		
	1	T19 Roof Special	-8 / 12	19-10-08	5-11-13	2 x 4		5-11-13 4-07-13	90.78 58.33		
	1 3-ply	T20 Roof Special Girder	-8 / 12	19-10-08	7-03-13	2 x 6		7-03-13 5-11-13	395.31 243.00		
	1 2-ply	T25Z3 Jack-Closed Girder	8 / 12	5-10-08	5-03-13	2 x 4 2 x 6	1-03-08	1-04-13 5-03-13	67.77 42.00		
	1 2-ply	T25Z4 Jack-Closed Girder	8 / 12	5-10-08	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	64.23 40.67		
	1 2-ply	T25Z5 Jack-Closed Girder	8 / 12	5-10-08	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	64.23 40.67		
	1	T30 Hip Girder	8 / 12	19-10-08	5-03-13	2 x 4 2 x 6		1-02-13 1-07-13	97.3 60.50		
	3	T31 Common	8 / 12	13-06-00	5-08-13	2 x 4		1-02-13 1-02-13	162.67 107.00		
	1	G31 GABLE	8 / 12	13-06-00	5-08-13	2 x 4	1-00-08 1-00-08	1-02-13 1-02-13	55.33 36.00		
	2	T33 Common	8 / 12	18-11-00	7-08-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	165.73 103.33		
	1	G33 GABLE	8 / 12	18-11-00	7-08-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	86.5 55.67		
	2	T34W Common	6 / 12	7-02-08	2-07-12	2 x 4		6-12 1-01-08	50.04 34.00		
	2	T35W Common	6 / 12	8-04-00	3-01-07	2 x 4	1-05-00	6-12 6-12	52.45 34.33		
	1	G35W GABLE	6 / 12	8-04-00	3-00-11	2 x 4	1-03-08 1-03-08	6-12 6-12	28.5 20.17		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST			
	Lumber Yard:	TAMARACK LUMBER	Job Track:	52166
	Builder:	GREENPARK HOMES	PlanLog:	204505
	Project:	RUSSELL GARDENS PH.4	Layout ID:	417565
	Location:	HAMILTON	Ref #	
	Model:	SPRINGFIELD 11	Page:	3 of 4
Lot #:		Date:	03-25-2021	
Elevation:	2	Designer:		
		Sales Rep:	William Garcia	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	PB1 Piggyback	8 / 12	9-09-01	1-04-00	2 x 4			26.8 18.00		
	2	PB2 Piggyback	8 / 12	9-09-01	2-08-00	2 x 4			54.91 35.00		
	3	PB3 Piggyback	8 / 12	9-09-01	3-03-00	2 x 4			74.28 46.50		
	1	PB4 Piggyback	8 / 12	11-09-01	1-04-00	2 x 4			33.39 22.17		
	16	J1 Jack-Open	8 / 12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	307.05 188.67		
	5	J30W Jack-Open	4 / 12	6-09-08	3-00-00	2 x 4	1-03-08	3-15 2-07-02	97.54 60.83		
	4	J31W Jack-Open	4 / 12	5-04-08	2-06-06	2 x 4	1-03-08	3-15 2-01-07	57.01 37.33		
	3	C1 Jack-Open	8 / 12	3-09-07	3-11-02	2 x 4	1-03-08 2-01-01	1-04-13 3-11-02	48.99 31.00		
	3	C2 Jack-Open	8 / 12	1-09-07	2-07-02	2 x 4	1-03-08 4-01-01	1-04-13 2-07-02	40.4 25.00		
	3	C3 Jack-Open	8 / 12	1-10-08	3-11-02	2 x 4	1-03-08 1-10-15	1-04-13 2-07-13	35.12 23.00		
	3	C4 Jack-Open	8 / 12	1-09-07	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	26.71 17.00		

TOTAL # TRUSS= 99

TOTAL BFT OF ALL TRUSSES= 4858.5

BFT.

TOTAL WEIGHT OF ALL TRSSES 7850.26 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
19	Hardware	H2.5T	
3	Hardware	HGUS26-2	
2	Hardware	HGUS28-3	
18	Hardware	LJS26DS	
7	Hardware	LUS24	

DELIVERY SHIPLIST		
	Lumber Yard:	TAMARACK LUMBER
	Builder:	GREENPARK HOMES
	Project:	RUSSELL GARDENS PH.4
	Location:	HAMILTON
	Model:	SPRINGFIELD 11
	Lot #:	
	Elevation:	2
	Job Track:	52166
	PlanLog:	204505
	Layout ID:	417565
	Ref #	
	Page:	4 of 4
	Date:	03-25-2021
	Designer:	
	Sales Rep:	William Garcia

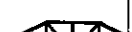
HARDWARE

QTY	TYPE	MODEL	LENGTH
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TOTAL NUMBER OF ITEMS= 50

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST			
	Lumber Yard: TAMARACK LUMBER Builder: GREENPARK HOMES Project: RUSSELL GARDENS PH.4 Location: HAMILTON Model: SPRINGFIELD 11 Lot #: Elevation: 3	Job Track: 52166 PlanLog: 204505 Layout ID: 417566 Ref # Page: 1 of 4 Date: 03-25-2021 Designer: Sales Rep: William Garcia		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T25Z6 Jack-Closed Girder	6 / 12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T40 Hip Girder	6 / 12	39-04-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	409.39 249.33		
	1	T41 Hip	6 / 12	39-04-00	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	191.01 115.83		
	1	T42 Hip	6 / 12	39-04-00	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	194.77 118.83		
	1	T43 Hip	6 / 12	39-04-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	191.13 115.50		
	1 3-ply	T44 Hip Girder	6 / 12	39-04-00	8-01-04	2 x 6 2 x 8	1-03-08 1-03-08	1-02-00 1-02-00	839.25 504.00		
	1 2-ply	T45 Roof Special Girder	6 / 12	37-04-00	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 1-02-00	394.79 245.33		
	1	T46 Roof Special	6 / 12	37-04-00	5-01-04	2 x 4 2 x 6	1-03-08	1-02-00 1-02-00	182.88 113.17		
	1	T47 Roof Special	6 / 12	37-04-00	6-01-04	2 x 4 2 x 6	1-03-08	1-02-00 1-02-00	179.57 112.50		
	1 3-ply	T48 Hip Girder	6 / 12	37-04-00	5-04-04	2 x 6 2 x 8		1-02-00 1-02-00	744.18 453.00		
	1	T49 Hip	6 / 12	30-01-00	6-04-04	2 x 4	1-03-08	1-02-00 4-09-08	131.71 82.50		
	1	T50 Hip	6 / 12	30-01-00	7-04-04	2 x 4	1-03-08	1-02-00 4-09-08	128.83 81.00		
	1	T51 Hip	6 / 12	30-01-00	8-04-04	2 x 4	1-03-08	1-02-00 4-09-08	139.36 87.17		
	1	T52 Hip	6 / 12	30-01-00	9-04-04	2 x 4	1-03-08	1-02-00 4-09-08	142.26 90.17		



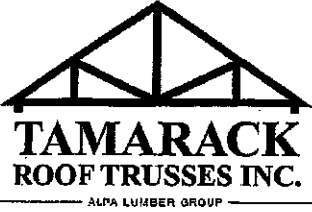
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: SPRINGFIELD 11
 Lot #:
 Elevation: 3

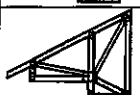
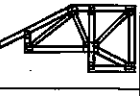
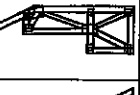
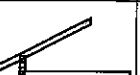
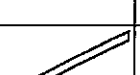
Job Track: 52166
 PlanLog: 204505
 Layout ID: 417566
 Ref #
 Page: 2 of 4
 Date: 03-25-2021
 Designer:
 Sales Rep: William Garcia

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T53 Hip	6 /12	30-01-00	10-01-04	2 x 4	1-03-08	1-02-00 4-09-08	140.03 87.83		
	3	T54 Common	6 /12	30-01-00	10-06-00	2 x 4	1-03-08	1-02-00 4-09-08	400.11 251.00		
	1	T55 Hip	6 /12	30-01-00	9-01-04	2 x 4	1-03-08	1-02-00 5-09-08	141.65 89.83		
	1	T56 Hip	6 /12	30-01-00	10-01-04	2 x 4	1-03-08	1-02-00 5-09-08	150.88 94.17		
	1	T57 Hip Girder	6 /12	17-04-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	79.46 51.00		
	1	T58 Hip	6 /12	17-04-00	5-01-04	2 x 4		1-02-00 1-02-00	69.7 44.17		
	1	T59 Common	6 /12	17-04-00	5-06-00	2 x 4		1-02-00 1-02-00	65.98 41.67		
	1	T60 Common	6 /12	18-05-00	5-06-00	2 x 4		7-08 1-02-00	68.26 43.50		
	1	T61 Hip Girder	6 /12	12-06-00	2-11-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	48.53 32.00		
	1	T62 Hip	6 /12	12-03-00	4-11-12	2 x 4	1-03-08	2-02-00 2-03-08	55.16 36.17		
	1	T63 Hip Girder	6 /12	20-03-00	5-01-04	2 x 4 2 x 6		2-00-08 2-02-00	103.14 65.83		
	1	T64 Hip	6 /12	20-03-00	6-01-04	2 x 4		2-00-08 2-02-00	85.79 55.67		
	1 3-ply	T65 Hip Girder	6 /12	20-03-00	7-01-04	2 x 4 2 x 6		2-00-08 2-02-00	327.29 201.00		
	1	T66 Hip Girder	6 /12	11-04-00	2-11-12	2 x 4		1-02-00 1-02-00	41.75 28.83		

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREENPARK HOMES Project: RUSSELL GARDENS PH.4 Location: HAMILTON Model: SPRINGFIELD 11 Lot #: Elevation: 3	Job Track: 52166 PlanLog: 204505 Layout ID: 417566 Ref # Page: 3 of 4 Date: 03-25-2021 Designer: Sales Rep: William Garcia

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T67 Common	6 / 12	11-04-00	5-00-00	2 x 4		2-02-00 2-02-00	44.16 28.50		
	1 2-ply	T68S Jack-Closed Girder	6 / 12	5-10-08	5-01-04	2 x 4 2 x 6		1-02-00 5-01-04	73.36 50.67		
	1	T69S Jack-Closed	6 / 12	5-10-08	5-01-04	2 x 4	1-03-08	1-02-00 5-01-04	33.93 23.33		
	1	T70S Half Hip	6 / 12	5-10-08	4-05-04	2 x 4	1-03-08	1-02-00 4-05-04	35.48 24.17		
	1	T71S Half Hip	6 / 12	5-10-08	3-05-04	2 x 4	1-03-08	1-02-00 3-05-04	31.35 21.83		
	1	T72S Half Hip	6 / 12	5-10-08	2-05-04	2 x 4	1-03-08	1-02-00 2-05-04	27.73 20.33		
	22	J1 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	369.48 234.67		
	7	J40 Jack-Open	6 / 12	3-07-08	2-11-12	2 x 4	1-03-08	1-02-00 2-11-12	79.52 51.33		
	4	C40 Jack-Open	6 / 12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	56.53 34.67		
	4	C41 Jack-Open	6 / 12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	46.33 29.33		
	4	C42 Jack-Open	6 / 12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	38.28 24.00		
	8	C43 Jack-Open	6 / 12	1-10-08	2-00-12	2 x 4	1-03-08	1-02-00 2-01-04	56.17 37.33		
	4	C44 Jack-Open	6 / 12	1-09-07	2-00-12	2 x 4	1-03-08 1-10-01	1-02-00 2-00-12	36.07 24.00		

TOTAL # TRUSS= 99

TOTAL BFT OF ALL TRUSSES= 4132.16

BFT.

TOTAL WEIGHT OF ALL TRSSES 6632.67 LBS

DELIVERY SHIPLIST		
 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	Lumber Yard: TAMARACK LUMBER	Job Track: 52166
	Builder: GREENPARK HOMES	PlanLog: 204505
	Project: RUSSELL GARDENS PH.4	Layout ID: 417566
	Location: HAMILTON	Ref #
	Model: SPRINGFIELD 11	Page: 4 of 4
	Lot #:	Date: 03-25-2021
	Elevation: 3	Designer:
		Sales Rep: William Garcia

HARDWARE

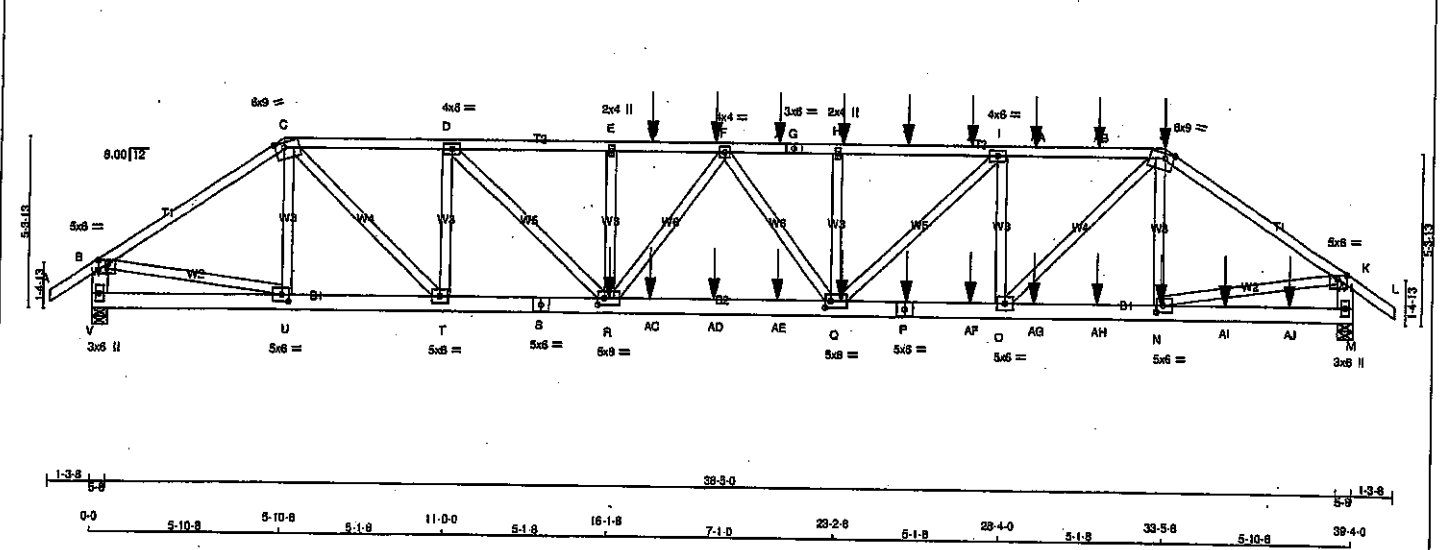
QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
2	Hardware	HGUS28-3	
18	Hardware	LJS26DS	
7	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 30

JOB NAME 417564	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 M/Tek Industries, Inc. Wed Mar 24 16:11:39 2021 Page 1
ID:AYS6Go91LGyvx7zbnOxdIzY1Yd-HbTxev4fWRPDWaBReetq8U1Ac7JvRjDp0gFYvpzXhG2

Scale = 1:25.5



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - G	2x4 DRY	No.2	SPF		
G - J	2x4 DRY	No.2	SPF		
J - L	2x4 DRY	No.2	SPF		
V - B	2x8 DRY	No.2	SPF		
M - K	2x8 DRY	No.2	SPF		
V - S	2x8 DRY	No.2	SPF		
S - P	2x8 DRY	No.2	SPF		
P - M	2x8 DRY	No.2	SPF		
ALL WEBS EXCEPT	2x4 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP SIDE(0.0)
C - G	12	SIDE(81.0)
G - J	12	SIDE(81.0)
J - L	12	TOP
V - B	2	TOP
M - K	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
V - S	2	TOP
S - P	2	SIDE(183.1)
P - M	2	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
N - J	6	SIDE(5.6)
E - R	1	SIDE(165.7)
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REGRD. BRG
VERT	HORZ	DOWN	HORZ
JT 3818	0	3818	0
M 4415	0	4415	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	WIND	DEAD
V	2692	1801.0	0.0	0.0	891.0
M	3119	2064.0	0.0	0.0	1055.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.58 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	W EBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
A-B	0/35	-91.8	-91.8	0.07 (1)	10.00	U-C	-551.0	0.08 (1)
B-C	-4858/0	-91.8	-91.8	0.54 (1)	3.96	C-T	0/4001	0.35 (1)
C-D	-6904/0	-91.8	-91.8	0.45 (1)	3.44	T-D	-2890/0	0.39 (1)
D-E	-9158/0	-91.8	-91.8	0.84 (1)	2.86	D-R	0/3129	0.28 (1)
E-W	-9158/0	-91.8	-91.8	0.46 (1)	2.96	R-E	-487/0	0.07 (1)
W-F	-9158/0	-91.8	-91.8	0.46 (1)	2.96	F-Q	-230/0	0.05 (1)
F-X	-8984/0	-91.8	-91.8	0.50 (1)	2.94	Q-I	-560/0	0.13 (1)
X-G	-8984/0	-91.8	-91.8	0.50 (1)	2.94	I-H	-759/0	0.11 (1)
G-H	-8984/0	-91.8	-91.8	0.87 (1)	2.94	H-J	0/2088	0.19 (1)
H-Y	-8984/0	-91.8	-91.8	0.87 (1)	2.58	O-I	-2492/0	0.38 (1)
Y-Z	-8984/0	-91.8	-91.8	0.87 (1)	2.58	O-J	0/3819	0.34 (1)
Z-I	-8984/0	-91.8	-91.8	0.87 (1)	2.58	N-J	-552/0	0.08 (1)
I-AA	-7454/0	-91.8	-91.8	0.73 (1)	2.97	B-U	0/4091	0.36 (1)
AA-AB	-7454/0	-91.8	-91.8	0.73 (1)	2.97	N-K	0/4782	0.42 (1)
AB-J	-7454/0	-91.8	-91.8	0.73 (1)	2.97			
J-K	-5878/0	-91.8	-91.8	0.81 (1)	3.65			
K-L	0/35	-91.8	-91.8	0.07 (1)	10.00			
V-B	-3784/0	0.0	0.0	0.13 (1)	7.27			
M-K	-4333/0	0.0	0.0	0.18 (1)	6.88			

V-U	0/0	-18.5	-18.5	0.04 (4)	10.00
U-T	0/4020	-18.5	-18.5	0.28 (1)	10.00
T-S	0/8904	-18.5	-18.5	0.48 (1)	10.00
S-R	0/8904	-18.5	-18.5	0.48 (1)	10.00
R-AC	0/8290	-18.5	-18.5	0.89 (1)	10.00
AC-AD	0/8290	-18.5	-18.5	0.89 (1)	10.00
AD-AE	0/8290	-18.5	-18.5	0.89 (1)	10.00
AE-Q	0/8290	-18.5	-18.5	0.89 (1)	10.00
Q-P	0/7454	-18.5	-18.5	0.53 (1)	10.00
P-AF	0/7454	-18.5	-18.5	0.53 (1)	10.00
AF-O	0/7454	-18.5	-18.5	0.53 (1)	10.00
O-AG	0/4701	-18.5	-18.5	0.34 (1)	10.00
AG-AH	0/4701	-18.5	-18.5	0.34 (1)	10.00
AH-N	0/4701	-18.5	-18.5	0.34 (1)	10.00
N-AI	0/0	-18.5	-18.5	0.07 (4)	10.00
AI-AJ	0/0	-18.5	-18.5	0.07 (4)	10.00
AJ-M	0/0	-18.5	-18.5	0.07 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
F	19-4-12	-122	-122	---	FRONT	VERT	TOTAL	C1
H	23-4-12	-122	-122	---	FRONT	VERT	TOTAL	C1

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.31')
CALCULATED VERT. DEFL.(LL) = L/998 (0.28')
ALLOWABLE DEFL.(TL) = L/360 (1.31')
CALCULATED VERT. DEFL.(TL) = L/958 (0.49')

CSI: TO=0.87/1.00 (H-H:1), BC=0.69/1.00 (Q-R:1), WB=0.42/1.00 (K-N:1), SS=0.23/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

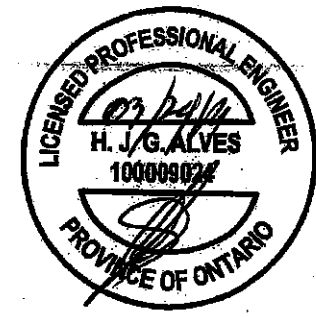
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (K) (INPUT = 0.90)
JSI METAL = 0.88 (S) (INPUT = 1.00)



Structural component only
DWG# T-2108946

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417564	T1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MTek Industries, Inc. Wed Mar 24 16:11:59 2021 Page 2
ID:AYS6Gc91LGYvx7zbnOxdizY1Yd-HbTxsv4fWRPDWaBRetq8U1Ac7JvRiDp0gFYvqzXhG2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.25
C	TTWW-m	MT20	6.0	9.0	1.75	3.50
D	TMWW-t	MT20	4.0	6.0		
E	TMWW-w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-w	MT20	2.0	4.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW-m	MT20	6.0	9.0	1.75	3.50
K	TMVW-p	MT20	5.0	6.0	1.50	3.25
M	BMV1-p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-w	MT20	5.0	8.0	2.50	2.00
R	BMWW-w	MT20	5.0	8.0	2.50	2.50
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMWW-t	MT20	5.0	8.0	2.50	2.50
V	BMV1-p	MT20	3.0	6.0		

SPECIFIED CONCENTRATED LOADS (LBS)

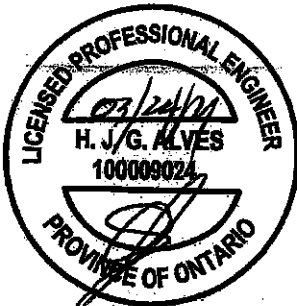
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
J	33-5-6	-337	-337	---	FRONT	VERT	TOTAL	---	C1
N	33-4-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
P	25-4-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
Q	23-4-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
R	18-1-8	-940	-940	---	FRONT	VERT	TOTAL	---	C1
W	17-4-12	-122	-122	---	FRONT	VERT	TOTAL	---	C1
X	21-4-12	-122	-122	---	FRONT	VERT	TOTAL	---	C1
Y	25-4-12	-122	-122	---	FRONT	VERT	TOTAL	---	C1
Z	27-4-12	-122	-122	---	FRONT	VERT	TOTAL	---	C1
AA	29-4-12	-122	-122	---	FRONT	VERT	TOTAL	---	C1
AB	31-4-12	-122	-122	---	FRONT	VERT	TOTAL	---	C1
AC	17-4-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AD	19-4-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AE	21-4-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AF	27-4-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AG	29-4-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AH	31-4-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AI	35-4-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AJ	37-4-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

NOTES: (1)

- 1) Lateral braces to be a minimum of 2X4 SPF #2.

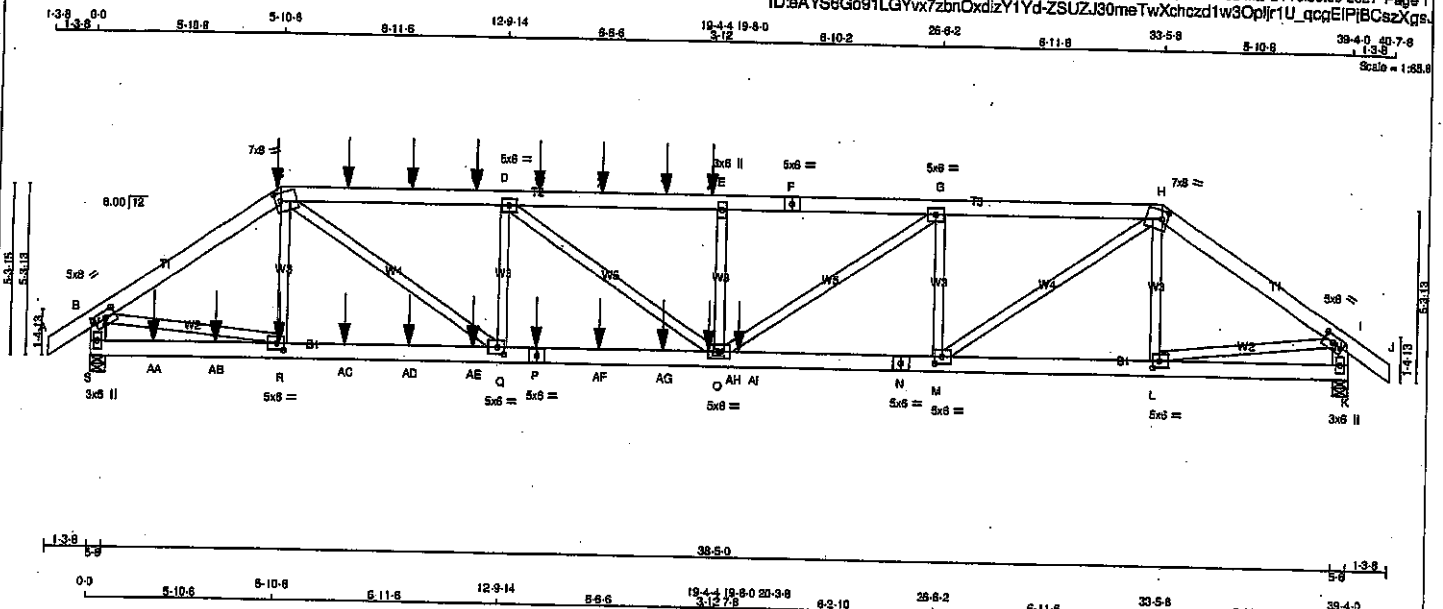


Structural component only
DWG# T-2108946 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417565	T1Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MTEK Industries, Inc. Wed Mar 24 16:39:06 2021 Page 1
ID:AYS6Go91LGvYx7zbnOxdzY1Yd-ZSUJ30meTwXhcZd1w3OpIrlU_qcgEIPiBCszXgs.



CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6 DRY	No.2	SPF
C - F	2x6 DRY	No.2	SPF
F - H	2x6 DRY	No.2	SPF
H - J	2x6 DRY	No.2	SPF
S - B	2x6 DRY	No.2	SPF
K - I	2x6 DRY	No.2	SPF
S - P	2x6 DRY	1650F 1.5E	SPF
P - N	2x6 DRY	1650F 1.5E	SPF
N - K	2x6 DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	2	12
C-F	2	12
F-H	2	12
H-J	2	12
S-B	2	12
K-I	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S-P	2	12
P-N	2	12
N-K	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
S	4508	0	0	5-8
K	3685	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
S	3185	2110	0	0	0	1075
K	2600	1739	0	0	0	861

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.43 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO						FR-TO			
A-B	0/36	-91.8	-91.8	0.04 (1)	10.00	R-C	-448	750	0.06 (1)
B-C	-5936	-91.8	-91.8	0.22 (1)	4.88	C-Q	0/4470		0.40 (1)
C-T	-5584	-91.8	-91.8	0.50 (1)	3.72	Q-D	-2387	70	0.32 (1)
T-U	-5584	-91.8	-91.8	0.50 (1)	3.72	D-O	0/1672		0.15 (1)
U-V	-5584	-91.8	-91.8	0.50 (1)	3.72	O-E	-960	70	0.13 (1)
V-W	-5584	-91.8	-91.8	0.50 (1)	3.72	E-G	0/2856		0.25 (1)
W-X	-5584	-91.8	-91.8	0.50 (1)	3.43	G-H	-2315	70	0.31 (1)
X-Y	-5584	-91.8	-91.8	0.50 (1)	3.43	H-I	0/4490		0.40 (1)
Y-Z	-5584	-91.8	-91.8	0.50 (1)	3.43	I-H	-480	70	0.07 (1)
Z-E	-5584	-91.8	-91.8	0.50 (1)	3.43	B-R	0/4994		0.44 (1)
E-F	-5584	-91.8	-91.8	0.34 (1)	3.68	L-I	0/3992		0.35 (1)
F-G	-5584	-91.8	-91.8	0.34 (1)	3.68				
G-H	-7619	-91.8	-91.8	0.27 (1)	4.19				
H-I	-4754	-91.8	-91.8	0.19 (1)	5.14				
I-J	0/36	-91.8	-91.8	0.04 (1)	10.00				
S-B	-4440	0.0	0.0	0.16 (1)	6.82				
K-I	-3635	0.0	0.0	0.13 (1)	7.37				

S-AA	0/0	-18.5	-18.5	0.06 (4)	10.00
AA-AB	0/0	-18.5	-18.5	0.06 (4)	10.00
AB-R	0/0	-18.5	-18.5	0.06 (4)	10.00
R-AC	0/4916	-18.5	-18.5	0.24 (1)	10.00
AC-AD	0/4916	-18.5	-18.5	0.24 (1)	10.00
AD-AE	0/4916	-18.5	-18.5	0.24 (1)	10.00
AE-AF	0/4916	-18.5	-18.5	0.24 (1)	10.00
AF-AG	0/4916	-18.5	-18.5	0.24 (1)	10.00
AG-AH	0/4916	-18.5	-18.5	0.24 (1)	10.00
AH-O	0/4916	-18.5	-18.5	0.24 (1)	10.00
O-AI	0/7619	-18.5	-18.5	0.45 (1)	10.00
AI-N	0/7619	-18.5	-18.5	0.45 (1)	10.00
N-M	0/7619	-18.5	-18.5	0.45 (1)	10.00
M-L	0/3934	-18.5	-18.5	0.20 (1)	10.00
L-K	0/0	-18.5	-18.5	0.03 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-337	-337	FRONT	VERT	TOTAL	---	C1
P	13-11-4	-29	-29	FRONT	VERT	TOTAL	---	C1
R	5-11-4	-29	-29	FRONT	VERT	TOTAL	---	C1

TOTAL WEIGHT = 2 X 228 = 456 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	8.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BOSC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.31")
CALCULATED VERT. DEFL.(LL) = L/889 (0.23")
ALLOWABLE DEFL.(TL) = L/360 (1.31")
CALCULATED VERT. DEFL.(TL) = L/999 (0.42")

CS1: TC=0.56/1.00 (D-E); BC=0.46/1.00 (M-O); WB=0.44/1.00 (B-R); SS=0.48/1.00 (M-O);

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION	(FS)	(PL)	(PL)
MAX MIN. MAX MIN	650	371	1747
MT20	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.89 (H) (INPUT = 0.90)
JSI METAL=0.77 (P) (INPUT = 1.00)



Structural component only
DWG# T-2108980

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
417565	T1Z	1	2	GREENPARK HOMES		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Mar 24 16:39:06 2021 Page 2
ID:AYS6Gq91LGYv7zbnOxdizY1Yd-ZSUZJ30meTwXhcZd1w3Oqlr1U qcnEIPiBCszXqs.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.75
C	TTWW-m	MT20	7.0	8.0	3.00	2.00
D	TMWW-t	MT20	5.0	6.0		
E	TMWW-w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	8.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW-m	MT20	7.0	8.0	3.00	2.00
I	TMVW-t	MT20	5.0	8.0	2.50	3.75
K	BMV1-p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BS-t	MT20	6.0	6.0		
O	BMVWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.75
R	BMWW-t	MT20	5.0	6.0	2.50	2.50
S	BMV1-p	MT20	3.0	6.0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
T	7-11.4	-122	-122	---	FRONT	VERT	TOTAL	---	C1
U	9-11.4	-122	-122	---	FRONT	VERT	TOTAL	---	C1
V	11-11.4	-122	-122	---	FRONT	VERT	TOTAL	---	C1
W	13-11.4	-122	-122	---	FRONT	VERT	TOTAL	---	C1
X	15-11.4	-122	-122	---	FRONT	VERT	TOTAL	---	C1
Y	17-11.4	-122	-122	---	FRONT	VERT	TOTAL	---	C1
Z	19-4.4	-122	-122	---	FRONT	VERT	TOTAL	---	C1
AA	1-11.4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AB	3-11.4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AC	7-11.4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AD	9-11.4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AE	11-11.4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AF	15-11.4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AG	17-11.4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AH	19-4.4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AI	20-3.8	-1058	-1058	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

NOTES: (1)

- 1) Lateral braces to be a minimum of 2X4 SPF #2.

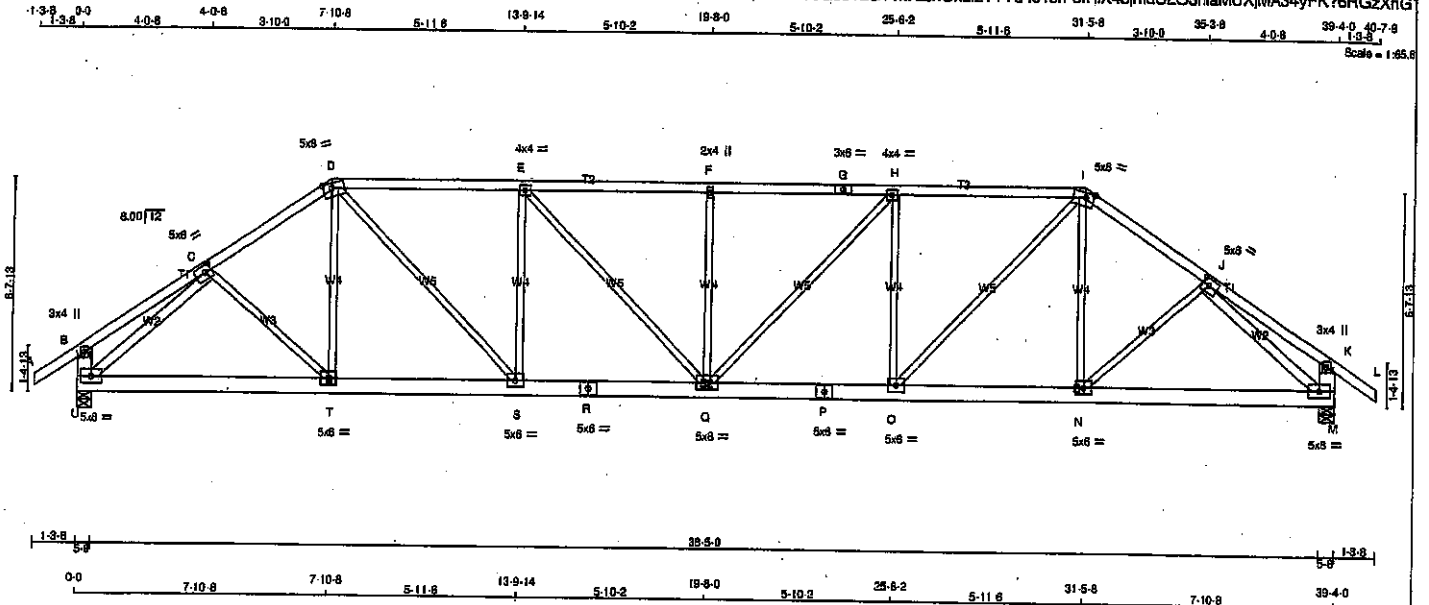


Structural component only
DWG# T-2108980

JOB NAME 417564	TRUSS NAME T2	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 6.420 S Jan 21 2021 Mitek Industries, Inc. Wed Mar 24 16:11:40 2021 Page 1
ID:AYS8G091LGYvx7zbnOxdizY1Yd4otJrF5IHIX48jmdCLO3hiaMUMXJMA34yFK76RQzXhG



CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
I - L	2x4	DRY No.2	SPF
U - B	2x6	DRY No.2	SPF
M - K	2x6	DRY No.2	SPF
U - R	2x6	DRY No.2	SPF
R - P	2x6	DRY No.2	SPF
P - M	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
U - C	2x4	DRY No.2	SPF
J - M	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMWW-t	MT20	5.0	8.0	2.50	2.50
D TTWW-m	MT20	5.0	8.0	1.75	3.25
E TMWW-t	MT20	4.0	4.0		
F TMW-w	MT20	2.0	4.0		
G TS-t	MT20	3.0	6.0		
H TMWW-t	MT20	4.0	4.0		
I TTWW-m	MT20	5.0	8.0	1.75	3.25
J TMWW-t	MT20	5.0	8.0	2.50	2.50
K TMV+p	MT20	3.0	4.0		
M BMVW-t	MT20	5.0	8.0		
N, O, S, T					
N BMWW-t	MT20	5.0	8.0		
P BS-t	MT20	5.0	8.0		
Q BMVW-t	MT20	5.0	8.0		
R BS-t	MT20	5.0	8.0		
U BMVW-t	MT20	5.0	8.0		

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
U	2294	0	5-8	5-8
M	2294	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
U	1620	1077	0	0	0	544
M	1620	1077	0	0	0	544

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO		
A-B	0/35	-91.8	-91.8 0.12 (1)	10.00	C-T	0/82
B-C	0/22	-91.8	-91.8 0.21 (1)	10.00	T-D	0/117
C-D	-2664/0	-91.8	-91.8 0.33 (1)	3.96	D-S	0/1389
D-E	-3153/0	-91.8	-91.8 0.76 (1)	3.24	S-E	-907/0
E-F	-3429/0	-91.8	-91.8 0.80 (1)	3.08	E-Q	0/405
F-G	-3429/0	-91.8	-91.8 0.80 (1)	3.08	Q-F	-493/0
G-H	-3429/0	-91.8	-91.8 0.80 (1)	3.08	H-Q	0/405
H-I	-3153/0	-91.8	-91.8 0.76 (1)	3.24	O-H	-907/0
I-J	-2664/0	-91.8	-91.8 0.33 (1)	3.96	O-I	0/1389
J-K	0/22	-91.8	-91.8 0.21 (1)	10.00	N-I	0/117
K-L	0/35	-91.8	-91.8 0.12 (1)	10.00	N-J	0/82
L-M	-267/0	0.0	0.0 0.02 (1)	7.81	U-C	-2501/0
M-K	-267/0	0.0	0.0 0.02 (1)	7.81	J-M	-2501/0
U-T	0/2196	-18.5	-18.5 0.32 (1)	10.00		
T-S	0/2196	-18.5	-18.5 0.32 (1)	10.00		
S-R	0/3153	-18.5	-18.5 0.42 (1)	10.00		
R-Q	0/3153	-18.5	-18.5 0.42 (1)	10.00		
Q-P	0/3153	-18.5	-18.5 0.42 (1)	10.00		
P-O	0/3153	-18.5	-18.5 0.42 (1)	10.00		
O-N	0/2196	-18.5	-18.5 0.32 (1)	10.00		
N-M	0/2196	-18.5	-18.5 0.32 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.31")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (1.31")
CALCULATED VERT. DEFL.(TL) = L/999 (0.33")

CSI: TC=0.90/1.00 (E-F:1), BC=0.42/1.00 (Q-S:1),
WB=0.90/1.00 (J-M:1), SS=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371
	1747	768
	1967	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.98 (C) (INPUT = 1.00)

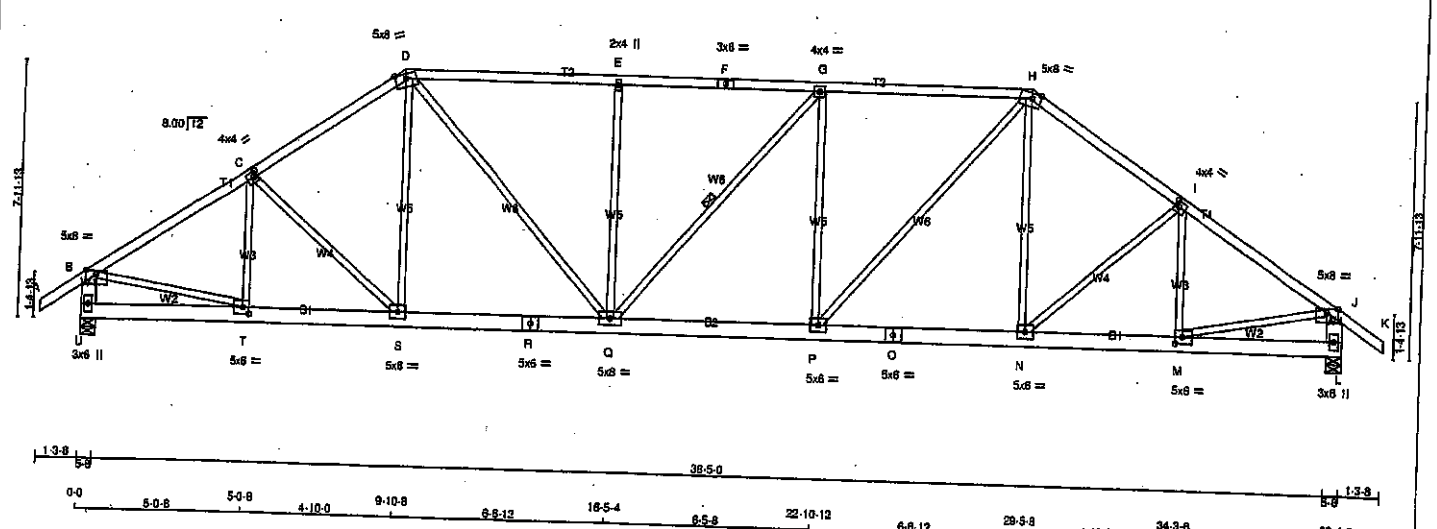


Structural component only
DWG# T-2108947

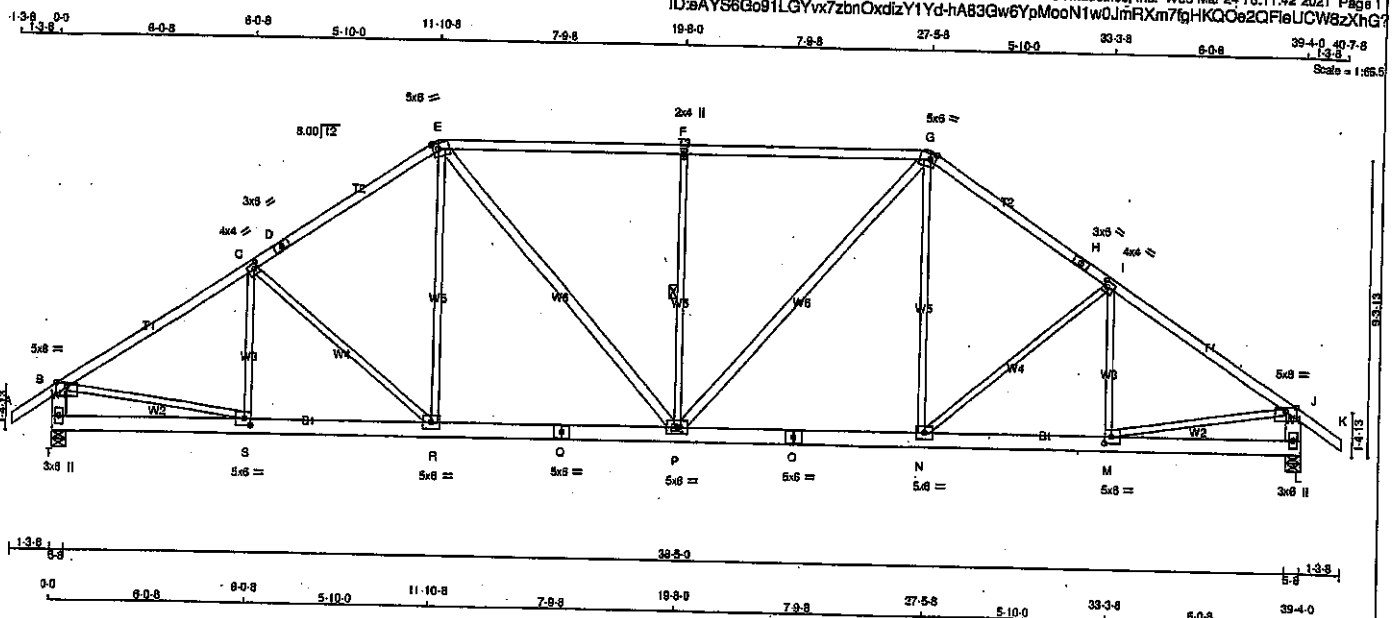
JOB NAME 417564	TRUSS NAME T3	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MTEK Industries, Inc. Wed Mar 24 16:11:41 2021 Page 1
ID:sAYS8G091LGYvx7zbnOxdizY1Yd-D_bh3a6w22fdt1pl3wDv6Wgw4MvY16T_kfzizXhG0

1-3-8 0-0 5-0-8 5-0-8 4-10-0 9-10-8 6-6-12 18-5-4 6-6-8 22-10-12 6-6-12 29-5-8 4-10-0 34-3-8 5-0-8 39-4-0 40-7-8 1-3-8
Scale = 1:85.8



JOB NAME 417564	TRUSS NAME T4	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mtek Industries, Inc. Wed Mar 24 16:11:42 2021 Page 1	



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY No.2	SPF
D - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
G - H	2x4	DRY No.2	SPF
H - K	2x4	DRY No.2	SPF
T - B	2x6	DRY No.2	SPF
L - J	2x6	DRY No.2	SPF
T - Q	2x6	DRY No.2	SPF
Q - O	2x6	DRY No.2	SPF
O - L	2x6	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
E - P	2x4	DRY No.2	SPF
P - G	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMVW-p	MT20	5.0	8.0	Edge
C TMVW-i	MT20	4.0	4.0	2.00 1.50
D TS-i	MT20	3.0	6.0	
E TTMW-m	MT20	5.0	6.0	2.25 2.00
F TMVW-w	MT20	2.0	4.0	
G TTMW-w	MT20	5.0	6.0	2.25 2.00
H TS-i	MT20	3.0	6.0	
I TTMW-i	MT20	4.0	4.0	2.00 1.50
J TMVW-p	MT20	6.0	8.0	Edge
L BMV1-p	MT20	3.0	6.0	
N BMVW-i	MT20	5.0	6.0	2.50 2.50
O BS-i	MT20	5.0	6.0	
P BMVW-w	MT20	5.0	6.0	
Q BS-i	MT20	5.0	6.0	
R BMVW-i	MT20	5.0	6.0	
S BMVW-i	MT20	5.0	6.0	2.50 2.50
T BMV1-p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT	REQD
JT	VERT	JT	DOWN	BRG	BRG
T	2294	0	2294	0	5-8
L	2294	0	2294	0	5-8

UNFACTORED REACTIONS		MAX. MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	JT	SNOW	JT	WIND	JT	SOIL
T	1820	1077	0	0	0	544	0
L	1820	1077	0	0	0	544	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FACTORED FORCE (LBS)	MEMB.	FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 85	S-C	-283 / 0
B-C	-2707 / 0	C-R	-344 / 0
C-D	-2468 / 0	R-E	0 / 368
D-E	-2468 / 0	E-P	0 / 676
E-F	-2468 / 0	P-F	-881 / 0
F-G	-2468 / 0	G-G	0 / 676
G-H	-2468 / 0	H-G	0 / 366
H-I	-2468 / 0	I-I	-344 / 0
I-J	-2707 / 0	J-I	-283 / 0
J-K	0 / 35	K-S	0 / 2313
K-S	-2236 / 0	S-L	0 / 2313
L-J	-2236 / 0		
T-S	0 / 0		
S-R	0 / 2282		
R-Q	0 / 2023		
Q-P	0 / 2023		
P-O	0 / 2023		
O-N	0 / 2023		
N-M	0 / 2282		
M-L	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN CC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2018
- PART 9 OF CBC 2012 (2018 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.31")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (1.31")
CALCULATED VERT. DEFL.(TL) = L/999 (0.23")

CSI: TC=0.91/1.00 (E-F:1), BC=0.32/1.00 (R-S:1), WB=0.52/1.00 (B-S:1), SS=0.35/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.85 (G) (INPUT = 0.80)
JSI METAL = 0.52 (S) (INPUT = 1.00)



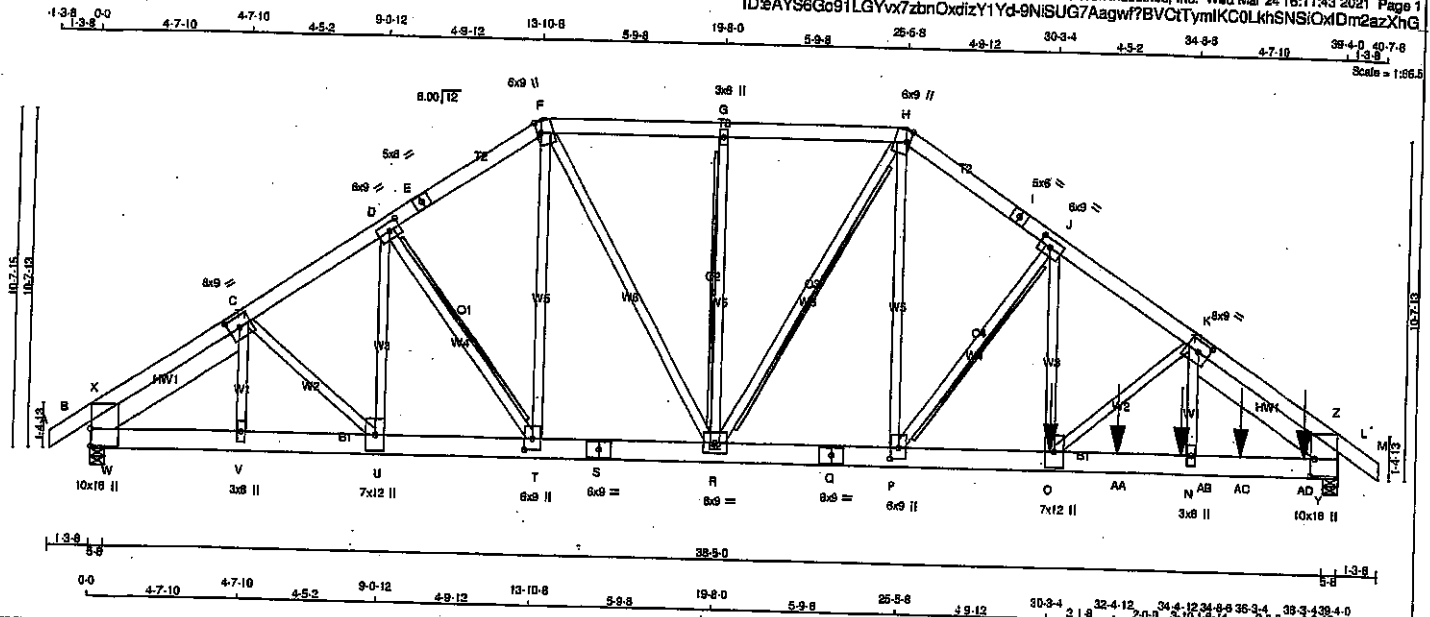
Structural component only
DWG# T-2108949

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
417564	T5	1	3	GREENPARK HOMES		

Tamarack Roof Trusses, Burlington

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ID:AYS8G091LGyVx7zbnOxdizY1Yd-9N1SUG7Aagw7BVCTYmIKC0LkhsNSiOxDM2azXhG



TOTAL WEIGHT = 3 X 317 = 950 LB

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x6 DRY No.2	SPF			
F - H	2x6 DRY No.2	SPF			
I - M	2x6 DRY No.2	SPF			
N - S	2x6 DRY No.2	SPF			
T - V	2x6 DRY No.2	SPF			
W - Z	2x6 DRY No.2	SPF			

REINFORCING MEMBERS	SIZE	LUMBER	DESCR.
HW1	2x6 DRY	No.2	SPF
HW2	2x6 DRY	No.2	SPF
ALL WEBS	2x4 DRY	No.2	SPF
DRY: SEASONED LUMBER			

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122'X3') SPIRAL NAILS		
A-E 2	12	TOP
F-H 2	12	TOP
I-M 2	12	TOP
N-S 2	12	TOP
T-V 2	12	TOP
W-Z 2	12	TOP
BOTTOM CHORDS : (0.122'X3') SPIRAL NAILS		
B-S 2	5	TOP
Q-L 2	5	TOP
WEBS : (0.122'X3') SPIRAL NAILS		
J-O 2	4	SIDE(824.0)
2x4 1	6	SIDE(1907.9)
D-U 2	4	
2x8 2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
B	VERT 4760	DOWN 4760	IN-SX	IN-SX
L	HORIZ 12505	0	5-8	5-8

UNFACTORED REACTIONS	1ST CASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERMILIVE WIND DEAD SOIL
B	3362	2228 / 0 0 / 0 0 / 0 1135 / 0 0 / 0
L	8832	5859 / 0 0 / 0 0 / 0 2973 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT D-T, G-R, H-R, J-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)
FR-TO						FR-TO			
A-B	0 / 8	-91.8	-91.8	0.02 (1)	10.00	V-C	0 / 179	0.01 (4)	
B-X	-4327 / 0	-91.8	-91.8	0.08 (1)	8.25	C-U	0 / 394	0.02 (1)	
X-C	-2445 / 0	-91.8	-91.8	0.07 (1)	8.25	U-D	-226 / 0	0.04 (1)	
C-D	-6545 / 0	-91.8	-91.8	0.07 (1)	5.47	D-T	-280 / 0	0.02 (1)	
D-E	-6427 / 0	-91.8	-91.8	0.07 (1)	5.51	T-F	0 / 339	0.02 (1)	
E-F	-6427 / 0	-91.8	-91.8	0.07 (1)	5.51	F-R	0 / 3244	0.17 (1)	
F-G	-8936 / 0	-91.8	-91.8	0.10 (1)	5.32	R-G	-587 / 0	0.07 (1)	
G-H	-6936 / 0	-91.8	-91.8	0.10 (1)	5.32	H-R	-2427 / 0	0.41 (1)	
H-I	-9697 / 0	-91.8	-91.8	0.11 (1)	4.66	P-H	0 / 7708	0.38 (1)	
I-J	-9697 / 0	-91.8	-91.8	0.11 (1)	4.66	J-K	-8458 / 0	0.71 (1)	
J-K	-15553 / 0	-91.8	-91.8	0.25 (1)	3.71	K-Q	0 / 9564	0.51 (1)	
K-Z	-8605 / 0	-91.8	-91.8	0.18 (1)	5.33	Q-K	-825 / 0	0.11 (1)	
Z-L	-11838 / 0	-91.8	-91.8	0.22 (1)	4.17	W-X	0 / 3330	0.18 (1)	
L-M	0 / 6	-91.8	-91.8	0.02 (1)	10.00	X-W	0 / 2221	0.00 (1)	
B-W	0 / 2001	-18.5	-18.5	0.13 (1)	10.00	W-X	-4987 / 0	0.32 (1)	
W-V	0 / 5180	-18.5	-18.5	0.23 (1)	10.00	Y-Z	-11427 / 0	0.84 (1)	
V-U	0 / 5177	-18.5	-18.5	0.18 (1)	10.00				
U-T	0 / 5469	-18.5	-18.5	0.19 (1)	10.00				
T-S	0 / 5322	-18.5	-18.5	0.19 (1)	10.00				
S-R	0 / 5322	-18.5	-18.5	0.19 (1)	10.00				
R-Q	0 / 8143	-18.5	-18.5	0.28 (1)	10.00				
Q-P	0 / 8143	-18.5	-18.5	0.28 (1)	10.00				
P-O	0 / 12993	-18.5	-18.5	0.50 (1)	10.00				
O-AA	0 / 13604	-18.5	-18.5	0.65 (1)	10.00				
AA-AB	0 / 13604	-18.5	-18.5	0.65 (1)	10.00				
AB-N	0 / 13604	-18.5	-18.5	0.65 (1)	10.00				
N-AC	0 / 13658	-18.5	-18.5	0.82 (1)	10.00				
AC-AD	0 / 13658	-18.5	-18.5	0.82 (1)	10.00				
AD-Y	0 / 13658	-18.5	-18.5	0.82 (1)	10.00				
Y-L	0 / 5378	-18.5	-18.5	0.32 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	30-3-4	-6111	-6111	---	BACK	VERT	TOTAL	---	C1
AA	32-4-12	-760	-760	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIG 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.31")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (1.31")
CALCULATED VERT. DEFL.(TL) = L/999 (0.23")

CSI: TO=0.25/1.00 (J-K:1), BC=0.65/1.00 (N-O:1), WB=0.84/1.00 (K-Y:1), SS=0.43/1.00 (N-O:1)

DDL LUMBER: 1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.81 (L) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2108950

JOB NAME 417564	TRUSS NAME T5	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						

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ID:AYS6G091LGYvx7zbnOxdizY1Yd-9N/SUG7Aagw17BVCITymIKOOLknSNSiOxIDm2azXhG

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1+I	MT20	10.0	16.0	6.50	0.25
C	TMWWW+I	MT20	8.0	9.0	4.00	4.25
D	TMWW+I	MT20	6.0	9.0	3.00	4.25
E	TS-I	MT20	5.0	8.0		
F	TTWW+M	MT20	6.0	9.0	4.25	1.50
G	TMWW+M	MT20	3.0	6.0		
H	TTWW+M	MT20	6.0	9.0	4.25	1.50
I	TS-I	MT20	5.0	8.0		
J	TMWW+I	MT20	6.0	9.0	3.00	4.25
K	TMWWW+I	MT20	8.0	9.0	4.00	4.25
L	TMBMW1+I	MT20	10.0	16.0	6.50	1.50
N	BMW+M	MT20	3.0	6.0		
O	BMWW+I	MT20	7.0	12.0		
P	BMWW+I	MT20	6.0	9.0	4.25	3.00
Q	BS-I	MT20	6.0	9.0		
R	BMWWW+I	MT20	8.0	9.0		
S	BS-I	MT20	6.0	9.0		
T	BMWW+I	MT20	6.0	9.0	4.25	3.00
U	BMWW+I	MT20	7.0	12.0		
V	BMW+M	MT20	3.0	6.0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AB	34-4-12	-1210	-1210		BACK	VERT	TOTAL		C1
AC	36-3-4	-434	-434		BACK	VERT	TOTAL		C1
AD	38-3-4	-434	-434		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

NOTES- (1)

- 1) Lateral braces to be a minimum of 2X4 SPF #2.

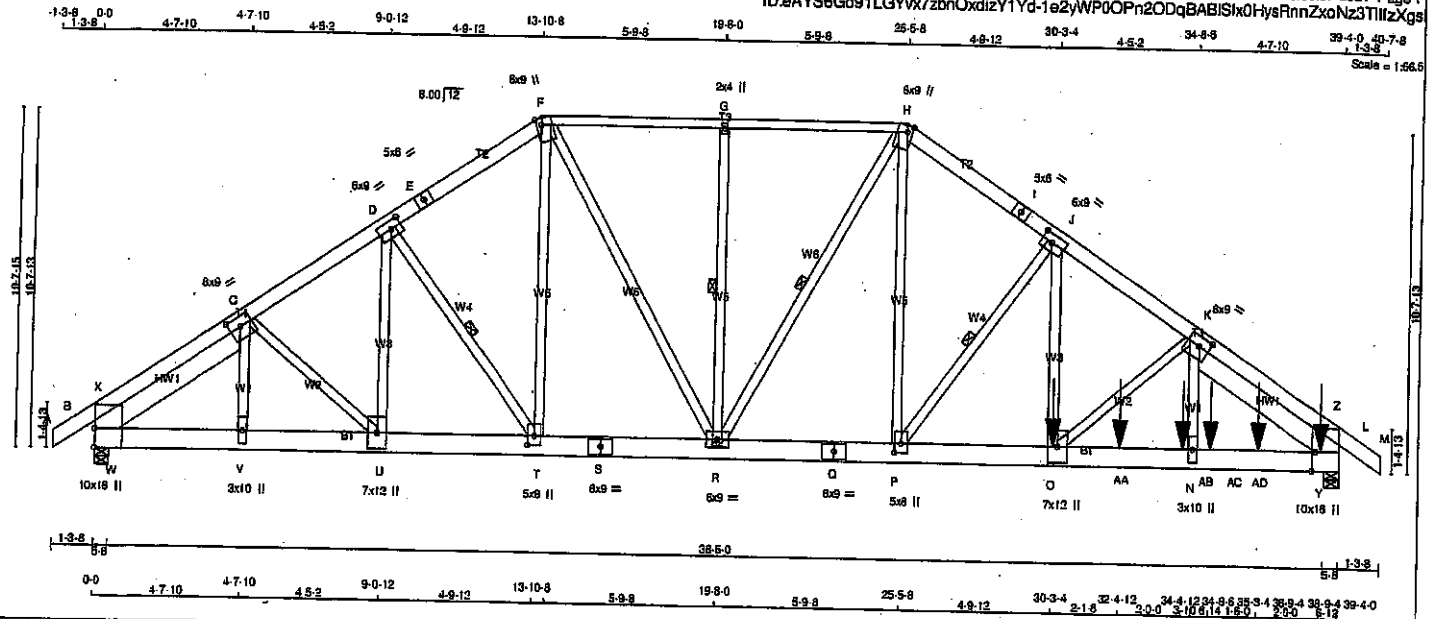


Structural component only
DWG# T-2108950 *210*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417565	T5Z	1	3	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x8 DRY	No.2	SPF		
E - F	2x8 DRY	No.2	SPF		
F - H	2x8 DRY	No.2	SPF		
H - I	2x8 DRY	No.2	SPF		
I - M	2x8 DRY	No.2	SPF		
B - S	2x8 DRY	No.2	SPF		
S - Q	2x8 DRY	No.2	SPF		
Q - L	2x8 DRY	No.2	SPF		

REINFORCING MEMBERS	SIZE	LUMBER	DESCR.
HW1	2x8 DRY	No.2	SPF
HW2	2x8 DRY	No.2	SPF

ALL WEBS 2x4 DRY SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-E	2 12	TOP
E-F	2 12	TOP
F-H	2 12	TOP
H-I	2 12	TOP
I-M	2 12	TOP
F-H	1 12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
B-S	2 5	TOP
S-Q	2 12	TOP
Q-L	2 5	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
J-O	2 4	SIDE(1919.8)
2x4	1 4	SIDE(1919.8)
D-U	2 4	SIDE(1919.8)
2x8	2 6	SIDE(1919.8)

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-4 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	4837	0	5-8
B DOWN	13421	0	5-8
L UPLIFT	0	0	5-8

UNFACTORED REACTIONS

1ST LC CASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT COMBINED	3416	0	1152	0
B SNOW	2284	0	0	0
L LIVE	6293	0	3185	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-T, G-R, H-R, J-P.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO			
A-B	0 / 6	-91.8	-91.8	0.02 (1)	V-C	0 / 182	0.01 (4)	
B-X	-4398 / 0	-91.8	-91.8	0.08 (1)	C-U	0 / 415	0.02 (1)	
X-C	-2488 / 0	-91.8	-91.8	0.07 (1)	U-D	-244 / 0	0.05 (1)	
C-D	-8666 / 0	-91.8	-91.8	0.08 (1)	D-T	-295 / 0	0.03 (1)	
D-E	-8556 / 0	-91.8	-91.8	0.08 (1)	T-F	0 / 379	0.02 (1)	
E-F	-6558 / 0	-91.8	-91.8	0.08 (1)	F-R	0 / 3269	0.18 (1)	
F-G	-7027 / 0	-91.8	-91.8	0.28 (1)	R-G	-500 / 0	0.09 (1)	
G-H	-7027 / 0	-91.8	-91.8	0.28 (1)	H-H	-2572 / 0	0.63 (1)	
H-I	-9829 / 0	-91.8	-91.8	0.13 (1)	P-H	0 / 7337	0.39 (1)	
I-J	-9829 / 0	-91.8	-91.8	0.13 (1)	P-J	-8770 / 0	0.96 (1)	
J-K	-15951 / 0	-91.8	-91.8	0.26 (1)	O-J	0 / 8840	0.53 (1)	
K-Z	-7048 / 0	-91.8	-91.8	0.19 (1)	O-K	-1236 / 0	0.16 (1)	
Z-L	-12668 / 0	-91.8	-91.8	0.24 (1)	N-K	0 / 3932	0.21 (1)	
L-M	0 / 6	-91.8	-91.8	0.02 (1)	W-X	0 / 2257	0.00 (1)	
					W-C	-4464 / 0	0.33 (1)	
					K-Y	-11830 / 0	0.87 (1)	
					Y-Z	0 / 8992	0.00 (1)	
B-W	0 / 2084	-18.5	-18.5	0.13 (1)				
V-U	0 / 5269	-18.5	-18.5	0.23 (1)				
U-T	0 / 5266	-18.5	-18.5	0.19 (1)				
T-S	0 / 5406	-18.5	-18.5	0.20 (1)				
S-R	0 / 5406	-18.5	-18.5	0.19 (1)				
R-Q	0 / 8303	-18.5	-18.5	0.19 (1)				
Q-P	0 / 8303	-18.5	-18.5	0.28 (1)				
P-O	0 / 13332	-18.5	-18.5	0.28 (1)				
O-AA	0 / 14248	-18.5	-18.5	0.52 (1)				
AA-AB	0 / 14248	-18.5	-18.5	0.67 (1)				
AB-N	0 / 14248	-18.5	-18.5	0.67 (1)				
N-AC	0 / 14311	-18.5	-18.5	0.63 (1)				
AC-AD	0 / 14311	-18.5	-18.5	0.63 (1)				
AD-Y	0 / 14311	-18.5	-18.5	0.63 (1)				
Y-L	0 / 5740	-18.5	-18.5	0.34 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
JT	30-34	-6135	-6135	---	BACK	VERT	---	C1
O	38-44	-515	-515	---	BACK	VERT	---	C1
Y	32-4-12	-760	-760	---	BACK	VERT	---	C1
AA	34-4-12	-1216	-1216	---	BACK	VERT	---	C1
AB	35-3-4	-511	-511	---	BACK	VERT	---	C1
AC	36-9-4	-511	-511	---	BACK	VERT	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
BOT CH. LL = 6.0 PSF
DL = 0.0 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIG 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.31")
CALCULATED VERT. DEFL.(LL) = L/899 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.31")
CALCULATED VERT. DEFL.(TL) = L/899 (0.24")

CSI: TC=0.28/1.00 (G-H:1), BC=0.67/1.00 (N-O:1), WB=0.96/1.00 (J-P:1), SS=0.44/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

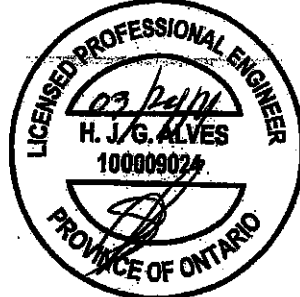
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.84 (C) (INPUT = 0.80)
JSI METAL= 0.84 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108981 / 1/2

JOB NAME 417565	TRUSS NAME T5Z	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Wed Mar 24 18:39:07 2021 Page 2
ID:eAYS6Go81LGYvx7zbnOxdizY1Yd-1e2yWP0OPn2ODgBABISix0HysRnnZxoNz3TllizXqs

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-t	MT20	10.0	18.0	7.25	
C	TMWWW-t	MT20	8.0	9.0	3.50	4.25
D	TMWW-t	MT20	8.0	9.0	3.00	4.00
E	TS-t	MT20	5.0	8.0		
F	TTWW+m	MT20	8.0	9.0	Edge	2.00
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	8.0	9.0	Edge	2.00
I	TS-t	MT20	5.0	8.0		
J	TMWW-t	MT20	8.0	9.0	3.00	4.00
K	TMWWW-t	MT20	8.0	9.0	3.50	4.25
L	TMBMW1-t	MT20	10.0	18.0	Edge	1.25
N	BMW+w	MT20	3.0	10.0		
O	BMWW-t	MT20	7.0	12.0		
P	BMWW-t	MT20	5.0	8.0	3.50	2.50
Q	BS-t	MT20	6.0	9.0		
R	BMWWW-t	MT20	6.0	9.0		
S	BS-t	MT20	6.0	9.0		
T	BMWW-t	MT20	5.0	8.0	3.50	2.50
U	BMWW-t	MT20	7.0	12.0		
V	BMW+w	MT20	3.0	10.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

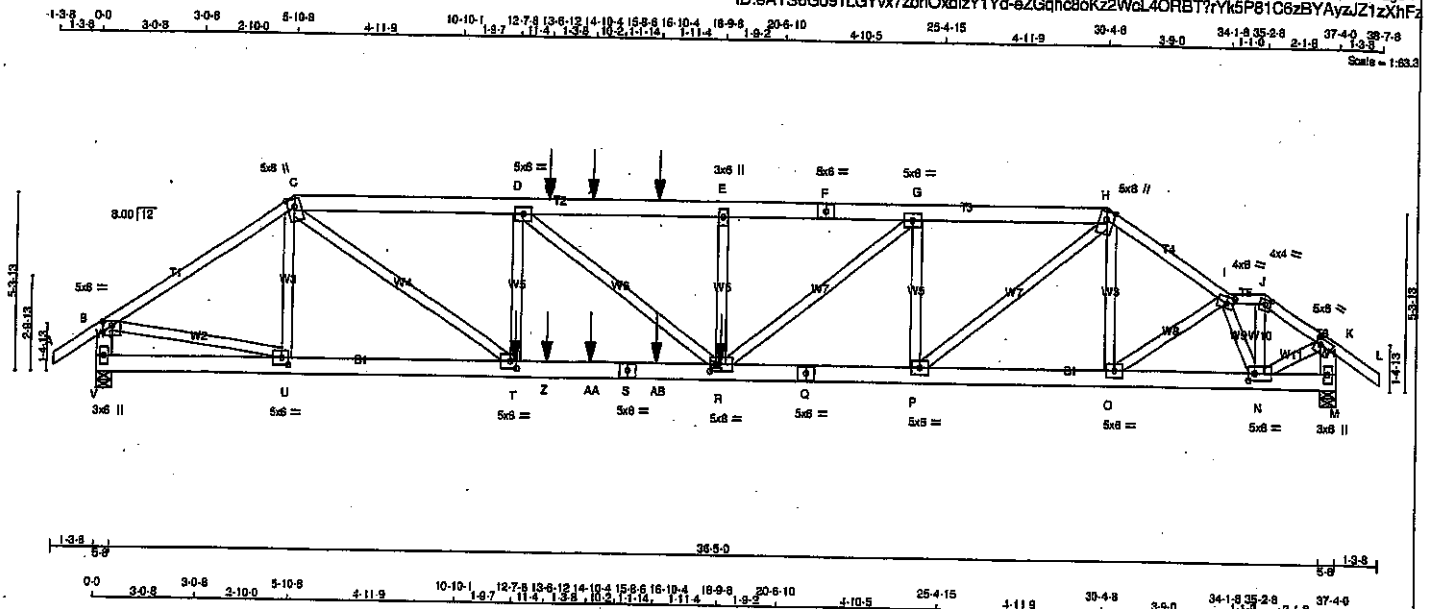
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2108981 *21*

JOB NAME 417564	TRUSS NAME T6	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Mar 24 16:11:44 2021 Page 1			
		ID: eAYS6Go91LGYvx7zbnOxdizY1Yd-eZGqhc8oK2Wol40RBT7Yk5P81C6zBYAyZJ1zXhFz			
		Scale = 1:50.0			



CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
L - M	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
K - N	2x6	DRY	No.2	SPF
N - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
H-I	12	TOP
I-J	12	TOP
J-L	12	TOP
C-F	12	SIDE(0.0)
F-H	12	TOP
V-B	12	TOP
M-K	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
V-S	12	SIDE(183.1)
S-Q	12	SIDE(183.1)
Q-M	12	TOP
WEBS: (0.122"x3") SPIRAL NAILS		
D-T	8	SIDE(185.2)
E-R	8	SIDE(289.1)
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
V	4386	0	0	5-8
M	3806	0	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	3093	2080 / 0	0 / 0	0 / 0	0 / 0	1013 / 0	0 / 0
M	2884	1801 / 0	0 / 0	0 / 0	0 / 0	884 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
A-B	0 / 35								
B-C	-5660 / 0	-91.8	-91.8	0.07 (1)	10.00	U-C	-689 / 0	0.10 (1)	
C-D	-9357 / 0	-91.8	-91.8	0.81 (1)	3.66	O-T	0 / 5899	0.50 (1)	
D-W	-10191 / 0	-91.8	-91.8	0.36 (1)	3.75	P-G	0 / 3082	0.27 (1)	
W-X	-10191 / 0	-91.8	-91.8	0.39 (1)	3.57	P-G	-2552 / 0	0.35 (1)	
X-Y	-10191 / 0	-91.8	-91.8	0.39 (1)	3.57	O-H	0 / 1103	0.01 (1)	
Y-E	-10191 / 0	-91.8	-91.8	0.39 (1)	3.57	N-J	0 / 1928	0.17 (1)	
E-F	-10191 / 0	-91.8	-91.8	0.31 (1)	3.66	B-U	0 / 4780	0.42 (1)	
F-G	-10191 / 0	-91.8	-91.8	0.31 (1)	3.66	N-K	0 / 3431	0.30 (1)	
G-H	-7818 / 0	-91.8	-91.8	0.22 (1)	4.19	T-D	-1705 / 0	0.23 (1)	
H-I	-5637 / 0	-91.8	-91.8	0.25 (1)	3.91	D-R	-754 / 0	0.10 (1)	
I-J	-3257 / 0	-91.8	-91.8	0.04 (1)	5.14	R-E	0 / 1062	0.09 (1)	
J-K	-3788 / 0	-91.8	-91.8	0.08 (1)	4.80	O-I	0 / 4085	0.36 (1)	
K-L	0 / 35	-91.8	-91.8	0.07 (1)	10.00	O-I	0 / 124	0.01 (1)	
V-B	-4341 / 0	0.0	0.0	0.16 (1)	6.88	I-N	-3813 / 0	0.24 (1)	
M-K	-3718 / 0	0.0	0.0	0.13 (1)	7.31				
V-U	0 / 0	-18.5	-18.5	0.04 (4)	10.00				
U-T	0 / 4740	-18.5	-18.5	0.34 (1)	10.00				
T-Z	0 / 9357	-18.5	-18.5	0.69 (1)	10.00				
Z-AA	0 / 9357	-18.5	-18.5	0.69 (1)	10.00				
AA-S	0 / 9357	-18.5	-18.5	0.69 (1)	10.00				
S-AB	0 / 9357	-18.5	-18.5	0.69 (1)	10.00				
AB-R	0 / 9357	-18.5	-18.5	0.69 (1)	10.00				
R-Q	0 / 7817	-18.5	-18.5	0.55 (1)	10.00				
Q-P	0 / 7817	-18.5	-18.5	0.55 (1)	10.00				
P-O	0 / 4704	-18.5	-18.5	0.33 (1)	10.00				
O-N	0 / 4604	-18.5	-18.5	0.35 (1)	10.00				
N-M	0 / 0	-18.5	-18.5	0.08 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	18-9-8	-1221	-1221	---	BACK	VERT	TOTAL	---	C1
T	12-7-9	-1019	-1019	---	BACK	VERT	TOTAL	---	C1
W	13-6-12	-122	-122	---	BACK	VERT	TOTAL	---	C1
X	14-10-4	-122	-122	---	BACK	VERT	TOTAL	---	C1
Y	16-10-4	-122	-122	---	BACK	VERT	TOTAL	---	C1
Z	13-6-12	-29	-29	---	BACK	VERT	TOTAL	---	C1
AA	14-10-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
AB	16-10-4	-29	-29	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.24")
CALCULATED VERT. DEFL.(LL) = L/999 (0.22")
ALLOWABLE DEFL.(TL) = L/360 (1.24")
CALCULATED VERT. DEFL.(TL) = L/999 (0.40")

CSI: TO=0.61/1.00 (B-C:1), BC=0.69/1.00 (R-T:1), WB=0.50/1.00 (C-T:1), SS=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1997 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (K) (INPUT = 0.90)
JSI METAL = 0.89 (S) (INPUT = 1.00)



Structural component only
DWG# T-2108951

CONTINUED ON PAGE 2

JOB NAME 417564	TRUSS NAME T6	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.25
C	TTWW+m	MT20	5.0	6.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMW-w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	5.0	6.0	Edge	
I	TTWW-m	MT20	4.0	6.0	2.50	2.75
J	TTW-m	MT20	4.0	4.0	1.75	2.00
K	TMWW-t	MT20	5.0	6.0	2.25	1.75
M	BMV1+p	MT20	3.0	6.0		
N	BMVWW-t	MT20	5.0	6.0	2.50	2.00
O	BMWW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMVWW-t	MT20	5.0	6.0	2.50	2.00
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.25
U	BMWW-t	MT20	5.0	6.0	2.50	2.50
V	BMV1+p	MT20	3.0	6.0		

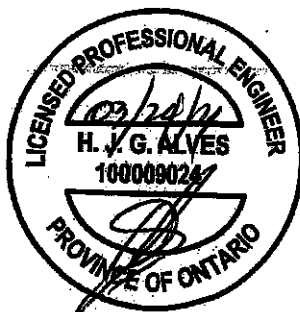
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

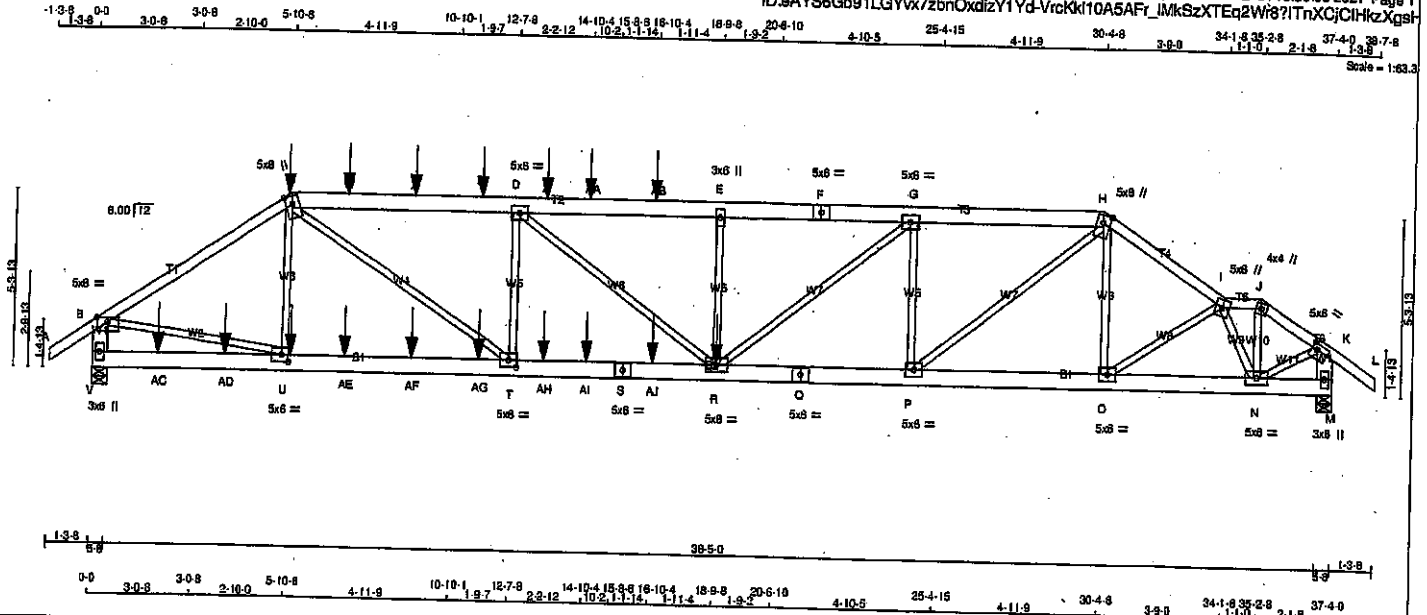


Structural component only
DWG# T-2108951

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
417565	T6Z	1	2	GREENPARK HOMES		

Tamarack Roof Truss, Burlington

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ID: aYS6Go91LGYvXzbnOxdizY1YdVrKk10A5AFr_lmkSzXTEq2Wf8?ITnXCjCihKzXgsh



CHORDS	SIZE	DRY	NO.2
A - C	2x4	DRY	No.2
C - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - I	2x4	DRY	No.2
I - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
V - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
V - S	2x6	DRY	No.2
S - Q	2x6	DRY	No.2
Q - M	2x6	DRY	No.2

ALL WEBS EXCEPT

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE (61.0)
H-I	12	TOP
J-L	12	TOP
C-F	12	TOP
F-H	12	SIDE (61.0)
V-B	12	TOP
M-K	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
V-S	12	SIDE (183.1)
S-Q	12	SIDE (183.1)
Q-M	12	TOP
WEBS: (0.122"x3") SPIRAL NAILS		
U-C	6	SIDE (5.6)
E-R	6	SIDE (266.7)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

MEMB.	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
JT	4407	0	4407	0	5-8	5-8
V	4407	0	4407	0	5-8	5-8
M	3553	0	3553	0	5-8	5-8

UNFACTORED REACTIONS

MEMB.	1ST CASE		MAX./MIN. COMPONENT REACTIONS		DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE		
JT	3114	2058 / 0	0 / 0	0 / 0	1056 / 0	0 / 0
V	3114	2058 / 0	0 / 0	0 / 0	1056 / 0	0 / 0
M	2515	1678 / 0	0 / 0	0 / 0	837 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS		MAX. UNBRACED LENGTH
	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MAX. UNBRACED LENGTH			FORCE (LBS)	CS1 (LC)	
FR-TO						FR-TO			
A-B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	U-C	-555 / 16	0.11 (1)	
B-C	-5662 / 0	-91.8	-91.8	0.61 (1)	3.66	C-T	0 / 4298	0.59 (1)	
C-W	-8214 / 0	-91.8	-91.8	0.44 (1)	3.85	R-G	0 / 2649	0.23 (1)	
W-X	-8214 / 0	-91.8	-91.8	0.44 (1)	3.85	P-G	-2300 / 0	0.44 (1)	
X-Y	-8214 / 0	-91.8	-91.8	0.44 (1)	3.85	O-H	0 / 108	0.02 (4)	
Y-D	-8214 / 0	-91.8	-91.8	0.47 (1)	3.63	N-J	0 / 1787	0.22 (1)	
D-Z	-9236 / 0	-91.8	-91.8	0.47 (1)	3.63	B-U	0 / 4791	0.59 (1)	
Z-AA	-9236 / 0	-91.8	-91.8	0.47 (1)	3.63	K-K	0 / 3177	0.39 (1)	
AA-AB	-9236 / 0	-91.8	-91.8	0.47 (1)	3.63	T-D	-2253 / 0	0.43 (1)	
AB-E	-9236 / 0	-91.8	-91.8	0.47 (1)	3.63	R-E	-888 / 0	0.13 (1)	
E-F	-9236 / 0	-91.8	-91.8	0.24 (1)	3.89	D-R	0 / 1303	0.16 (1)	
F-G	-9236 / 0	-91.8	-91.8	0.24 (1)	3.89	P-H	0 / 3728	0.46 (1)	
G-H	-7200 / 0	-91.8	-91.8	0.20 (1)	4.35	O-I	0 / 100	0.01 (1)	
H-I	-5228 / 0	-91.8	-91.8	0.23 (1)	4.05	I-N	-3379 / 0	0.31 (1)	
I-J	-2976 / 0	-91.8	-91.8	0.04 (1)	5.33				
J-K	-3497 / 0	-91.8	-91.8	0.08 (1)	4.96				
K-L	0 / 35	-91.8	-91.8	0.07 (1)	10.00				
V-B	-4336 / 0	0.0	0.0	0.16 (1)	6.88				
M-K	-3473 / 0	0.0	0.0	0.12 (1)	7.50				

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS		MAX. UNBRACED LENGTH
	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MAX. UNBRACED LENGTH			FORCE (LBS)	CS1 (LC)	
V-AC	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
AC-AD	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
AD-U	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
U-AE	0 / 4733	-18.5	-18.5	0.36 (1)	10.00				
AE-AF	0 / 4733	-18.5	-18.5	0.36 (1)	10.00				
AF-AG	0 / 4733	-18.5	-18.5	0.36 (1)	10.00				
AG-T	0 / 4733	-18.5	-18.5	0.36 (1)	10.00				
T-AH	0 / 8213	-18.5	-18.5	0.61 (1)	10.00				
AH-AI	0 / 8213	-18.5	-18.5	0.61 (1)	10.00				
AI-S	0 / 8213	-18.5	-18.5	0.61 (1)	10.00				
S-AJ	0 / 8213	-18.5	-18.5	0.61 (1)	10.00				
AJ-R	0 / 8213	-18.5	-18.5	0.61 (1)	10.00				
R-Q	0 / 7199	-18.5	-18.5	0.52 (1)	10.00				
Q-P	0 / 7199	-18.5	-18.5	0.52 (1)	10.00				
P-O	0 / 4392	-18.5	-18.5	0.31 (1)	10.00				
O-N	0 / 4282	-18.5	-18.5	0.33 (1)	10.00				
N-M	0 / 0	-18.5	-18.5	0.08 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-337	-337	---	BACK	VERT	TOTAL	---	C1
R	18-9-8	-1215	-1215	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
BOT CH. LL = 8.0 PSF
DL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.24")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.24")
CALCULATED VERT. DEFL.(TL) = L/999 (0.38")

CSI: TC=0.61/1.00 (B-C:1), BC=0.61/1.00 (R-T:1),
WB=0.59/1.00 (B-U:1), SS=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

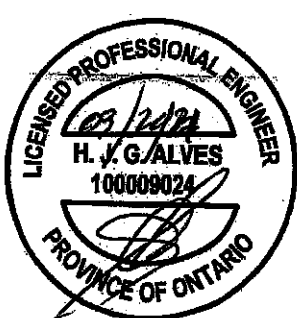
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (T) (INPUT = 0.90)
JSI METAL = 0.79 (S) (INPUT = 1.00)



Structural component only
DWG# T-2108982

CONTINUED ON PAGE 2

JOB NAME 417565	TRUSS NAME T6Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamareck Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Mar 24 16:39:06 2021 Page 2 ID:eAYS6Gp91LGyvx7zbnOxdizY1Yd-VrcKk10A5AFr IMkSzXTEq2Wr8?ITnXCjCIHkzXgsf	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMW-w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	5.0	8.0	Edge	
I	TTWW+m	MT20	5.0	6.0		
J	TTW+m	MT20	4.0	4.0		
K	TMVW-t	MT20	5.0	6.0	2.00	1.75
M	BMV1+P	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	6.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.75
U	BMWW-t	MT20	5.0	6.0	2.50	2.50
V	BMV1+P	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
U	5-11-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
W	7-6-12	-122	-122	---	BACK	VERT	TOTAL	---	C1
X	9-6-12	-122	-122	---	BACK	VERT	TOTAL	---	C1
Y	11-8-12	-122	-122	---	BACK	VERT	TOTAL	---	C1
Z	13-6-12	-122	-122	---	BACK	VERT	TOTAL	---	C1
AA	14-10-4	-122	-122	---	BACK	VERT	TOTAL	---	C1
AB	16-10-4	-122	-122	---	BACK	VERT	TOTAL	---	C1
AC	1-11-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
AD	3-11-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
AE	7-6-12	-29	-29	---	BACK	VERT	TOTAL	---	C1
AF	9-6-12	-29	-29	---	BACK	VERT	TOTAL	---	C1
AG	11-8-12	-29	-29	---	BACK	VERT	TOTAL	---	C1
AH	13-6-12	-29	-29	---	BACK	VERT	TOTAL	---	C1
AI	14-10-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
AJ	16-10-4	-29	-29	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2108982 *em*

Tamarack Roof Truss, Burlington

Version 8.420 © Jan 21 2021 Mitek Industries, Inc. Wed Mar 24 16:11:45 2021 Page 1
ID: AYS6G091LGyvx7zbnOxdizY1Yd-6lqCvy9Q5HANEVfb_u_FOIHASyOMrJwhOcs5TzXhF

1-3-8 0-0 4-0-8 4-0-8 3-10-0 7-10-8 6-10-7 14-5-15 6-9-3 21-8-1 10-10-7 28-4-8 3-9-0 32-1-8 33-2-8 37-4-0 38-7-8 1-3-8

Scale = 1/8" = 1'-0"



Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C/C

LOADING IN ALL PLATE SECTIONS BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF.(LL) = L/360 (1.24")
CALCULATED VERT. DEF.(LL) = L/999 (0.17")
ALLOWABLE DEF.(TL) = L/360 (1.24")
CALCULATED VERT. DEF.(TL) = L/999 (0.33")

**CSI: TC=0.98/1.00 (G-H:1), BC=0.58/1.00 (Q-R:1),
WB=0.92/1.00 (C-U:1), SS=0.29/1.00 (D-E:1)**

**DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10**

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

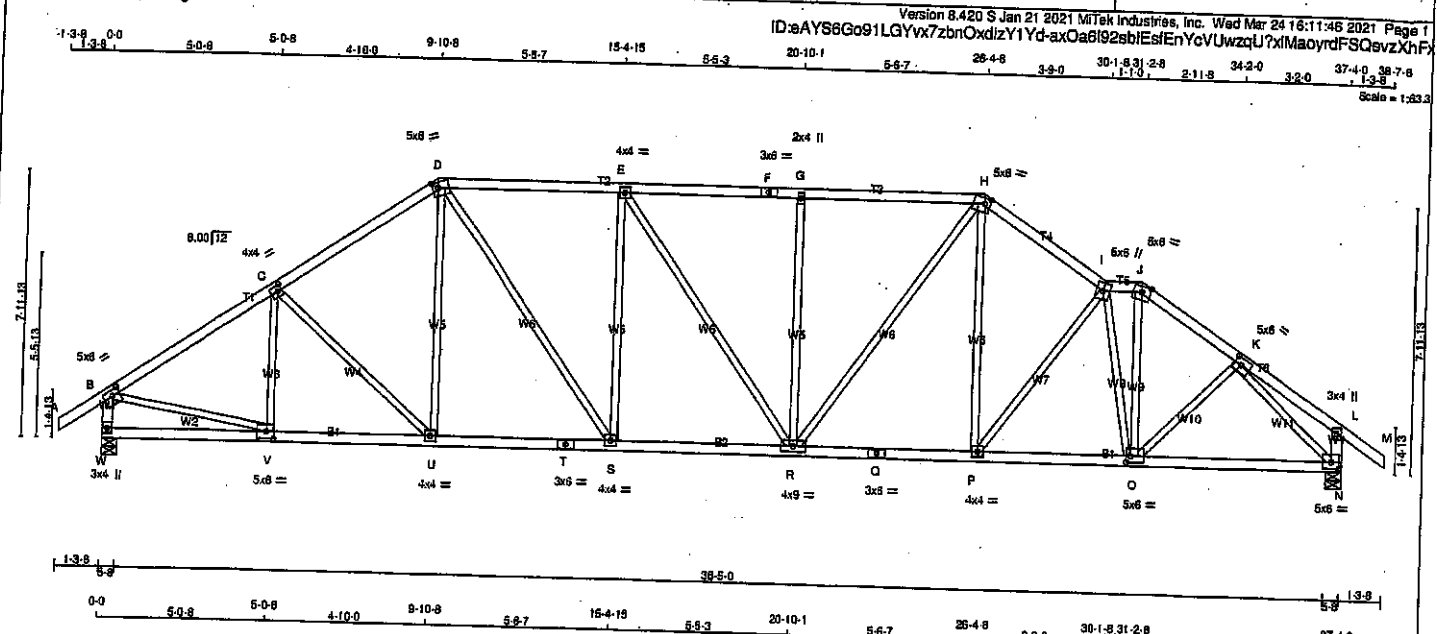
PLATE ROTATION TOL = 5.0 Deg.

**JSI GRIP = 0.89 (N) (INPUT = 0.90)
JSI METAL = 0.74 (P) (INPUT = 1.00)**



Structural component only
DWG# T-2108952

JOB NAME 417564	TRUSS NAME T8	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Mar 24 16:11:46 2021 Page 1	



LUMBER			
N. L. & A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
I - J	2x4	DRY	No.2
J - M	2x4	DRY	No.2
W - B	2x4	DRY	No.2
N - L	2x4	DRY	No.2
W - T	2x4	DRY	No.2
T - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			

PLATES (tablets in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-t	MT20	5.0	6.0 2.00 2.75
C	TMVW-t	MT20	4.0	4.0 2.00 1.50
D	TMVW-m	MT20	5.0	6.0 2.00 2.00
E	TMVW-t	MT20	4.0	4.0
F	TS-t	MT20	3.0	6.0
G	TMVW-w	MT20	2.0	4.0
H	TMVW-m	MT20	5.0	6.0 2.00 2.00
I	TMVW-m	MT20	5.0	6.0
J	TMVW-t	MT20	5.0	6.0 Edge
K	TMVW-t	MT20	5.0	6.0 2.50 2.50
L	TMVW-p	MT20	3.0	4.0
N	BMVW-t	MT20	5.0	6.0 2.25 2.75
O	BMVW-t	MT20	5.0	6.0 2.25 1.50
P, S, U	BMVW-t	MT20	4.0	4.0
Q	BS-t	MT20	3.0	6.0
R	BMVW-t	MT20	4.0	9.0
T	BS-t	MT20	3.0	6.0
V	BMVW-t	MT20	5.0	6.0 2.50 2.75
W	BMV-t	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
W	2184	2184	0	5-8	5-8
N	2184	2184	0	5-8	5-8

UNFACTORED REACTIONS					
JT	1ST LCASE COMBINED	MAX. MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND
W	1542	1025 / 0	0 / 0	0 / 0	517 / 0
N	1542	1025 / 0	0 / 0	0 / 0	517 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRAC LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRAC LENGTH (LC)	
FR-TO				FR-TO			
A-B	0 / 35	-91.8	-91.8 0.12 (1)	V-C	-353 / 0	0.12 (1)	
B-C	-2478 / 0	-91.8	-91.8 0.38 (1)	C-U	-182 / 0	0.15 (1)	
C-D	-2377 / 0	-91.8	-91.8 0.37 (1)	U-D	0 / 221	0.05 (1)	
D-E	-2461 / 0	-91.8	-91.8 0.41 (1)	D-S	0 / 869	0.20 (1)	
E-F	-2502 / 0	-91.8	-91.8 0.38 (1)	S-E	-604 / 0	0.73 (1)	
F-G	-2502 / 0	-91.8	-91.8 0.38 (1)	E-R	0 / 72	0.02 (1)	
G-H	-2502 / 0	-91.8	-91.8 0.41 (1)	R-G	-543 / 0	0.66 (1)	
H-I	-2487 / 0	-91.8	-91.8 0.24 (1)	I-H	0 / 722	0.16 (1)	
I-J	-2071 / 0	-91.8	-91.8 0.06 (1)	P-H	0 / 422	0.09 (1)	
J-K	-2477 / 0	-91.8	-91.8 0.17 (1)	O-K	0 / 1178	0.28 (1)	
K-L	0 / 12	-91.8	-91.8 0.11 (1)	D-K	0 / 273	0.08 (1)	
L-M	0 / 35	-91.8	-91.8 0.12 (1)	B-V	0 / 2133	0.48 (1)	
W-B	-2142 / 0	0.0	0.0 0.22 (1)	K-N	-2648 / 0	0.81 (1)	
N-L	-245 / 0	0.0	0.0 0.03 (1)	I-O	-1278 / 0	0.60 (1)	
				P-I	-412 / 0	0.31 (1)	
W-V	0 / 0	-18.5	-18.5 0.10 (4)				
V-U	0 / 2085	-18.5	-18.5 0.39 (1)				
U-T	0 / 1953	-18.5	-18.5 0.37 (1)				
T-S	0 / 1953	-18.5	-18.5 0.37 (1)				
S-R	0 / 2461	-18.5	-18.5 0.45 (1)				
R-Q	0 / 2080	-18.5	-18.5 0.39 (1)				
Q-P	0 / 2080	-18.5	-18.5 0.39 (1)				
P-O	0 / 2218	-18.5	-18.5 0.44 (1)				
O-N	0 / 1852	-18.5	-18.5 0.39 (1)				

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 25.8 PSF
DL	= 8.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.24")
CALCULATED VERT. DEFL. (LL) = L/999 (0.14")
ALLOWABLE DEFL. (TL) = L/360 (1.24")
CALCULATED VERT. DEFL. (TL) = L/999 (0.26")

CSI: TC=0.41/1.00 (G-H:1), BC=0.45/1.00 (R-S:1), WB=0.81/1.00 (K-N:1), SS=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

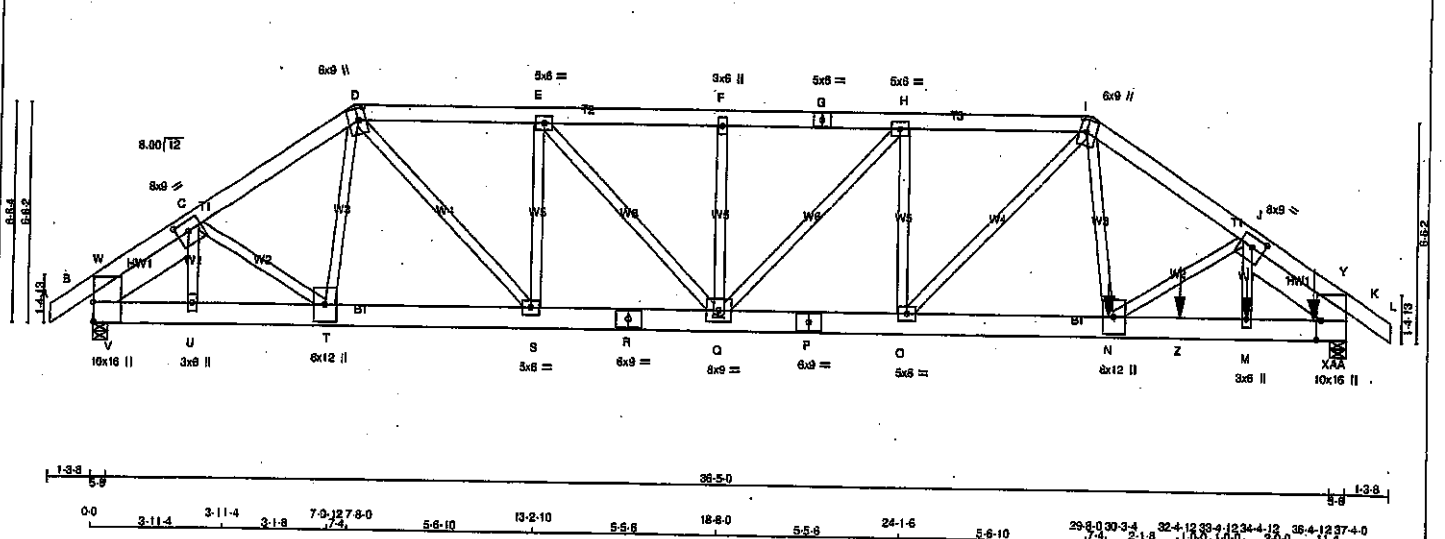
JSI GRIP = 0.89 (K) (INPUT = 0.90)
JSI METAL = 0.67 (Q) (INPUT = 1.00)



Structural component only
DWG# T-2108953

JOB NAME 417564	TRUSS NAME T9	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Mar 24 16:11:47 2021 Page 1
ID:AYS6Go91LGYvx7zbnOxdizY1Yd-28yyJ9AhduQ5Topz6JQITAMIFL3Uhw_svBzAMzXhFw
Scale = 1/32.6



LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6 DRY	No.2	SPF		
D - G	2x6 DRY	No.2	SPF		
G - I	2x6 DRY	No.2	SPF		
I - L	2x6 DRY	No.2	SPF		
B - R	2x6 DRY	No.2	SPF		
R - P	2x6 DRY	No.2	SPF		
P - K	2x6 DRY	No.2	SPF		
REINFORCING MEMBERS					
HW1	2x8 DRY	No.2	SPF		
HW2	2x8 DRY	No.2	SPF		
ALL WEBS 2x4 DRY					
DRY: SEASONED LUMBER					

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 2	12	TOP
D-G 2	12	TOP
G-I 2	12	TOP
I-L 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
B-R 2	4	TOP
R-P 2	12	TOP
P-K 2	4	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 2	5	SIDE(1007.1)
I-N 2	4	SIDE(2495.6)
E-S 1	6	
Q-E 1	6	
F-D 1	6	
Q-H 1	6	
H-O 1	6	
D-T 2	4	
O-T 1	6	
N-J 1	6	
S-D 1	6	
O-I 1	6	
2x8 2	6	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
B	4140	0	0	5-8
K	12472	0	0	5-8

UNFACTORED REACTIONS	1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
B	2924	1937 / 0
K	8809	5845 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO	A-B	0 / 6	-91.8	-91.8	0.02 (1)	10.00	U-C	0 / 170
	B-W	-3419 / 0	-91.8	-91.8	0.05 (1)	6.25	S-E	-2758 / 0
	W-C	-1819 / 0	-91.8	-91.8	0.05 (1)	6.25	E-Q	0 / 2975
	C-D	-5554 / 0	-91.8	-91.8	0.07 (1)	5.82	Q-F	-466 / 0
	D-E	-7289 / 0	-91.8	-91.8	0.09 (1)	5.24	Q-H	-2325 / 0
	E-F	-9292 / 0	-91.8	-91.8	0.11 (1)	4.75	O-H	0 / 1159
	F-G	-9292 / 0	-91.8	-91.8	0.11 (1)	4.75	M-J	0 / 3574
	G-H	-9292 / 0	-91.8	-91.8	0.11 (1)	4.75	T-D	-228 / 0
	H-I	-10858 / 0	-91.8	-91.8	0.13 (1)	4.44	C-T	0 / 677
	I-J	-15319 / 0	-91.8	-91.8	0.25 (1)	3.72	I-N	0 / 3020
	J-Y	-8209 / 0	-91.8	-91.8	0.18 (1)	5.44	N-J	0 / 337
	Y-K	-10701 / 0	-91.8	-91.8	0.22 (1)	4.35	D-S	0 / 3915
	K-L	0 / 6	-91.8	-91.8	0.02 (1)	10.00	O-I	-1288 / 0
	B-V	0 / 1587	-18.5	-18.5	0.13 (1)	10.00	V-W	0 / 1795
	V-U	0 / 4056	-18.5	-18.5	0.21 (1)	10.00	V-C	-3803 / 0
	U-T	0 / 4062	-18.5	-18.5	0.15 (1)	10.00	J-X	-11631 / 0
	T-S	0 / 4643	-18.5	-18.5	0.16 (1)	10.00	X-Y	0 / 5599
	S-R	0 / 7289	-18.5	-18.5	0.27 (1)	10.00		
	R-Q	0 / 7289	-18.5	-18.5	0.27 (1)	10.00		
	Q-P	0 / 10858	-18.5	-18.5	0.38 (1)	10.00		
	P-O	0 / 10858	-18.5	-18.5	0.38 (1)	10.00		
	O-N	0 / 11728	-18.5	-18.5	0.42 (1)	10.00		
	N-Z	0 / 12511	-18.5	-18.5	0.59 (1)	10.00		
	Z-M	0 / 12511	-18.5	-18.5	0.59 (1)	10.00		
	M-AA	0 / 12585	-18.5	-18.5	0.63 (1)	10.00		
	AA-X	0 / 12585	-18.5	-18.5	0.63 (1)	10.00		
	X-K	0 / 4952	-18.5	-18.5	0.39 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	34-4-12	-1563	-1563	---	---	FRONT	VERT	TOTAL	---	C1
N	30-3-4	-6111	-6111	---	---	FRONT	VERT	TOTAL	---	C1
Z	32-4-12	-760	-760	---	---	FRONT	VERT	TOTAL	---	C1
AA	36-4-12	-209	-209	---	---	TOP	VERT.	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.24")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.24")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.25/1.00 (I-J:1), BC=0.63/1.00 (M-X:1), WB=0.70/1.00 (H-Q:1), SS=0.50/1.00 (K-X:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

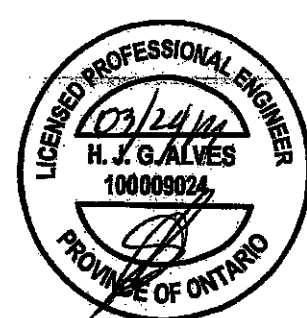
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.80)
JSI METAL= 0.75 (K) (INPUT = 1.00)



Structural component only
DWG# T-2108954

JOB NAME 417564	TRUSS NAME T9	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Wed Mar 24 18:11:47 2021 Page 2
ID: aAYS6G091LGYvx7zbnOxdizY1Yd-2BvyJeAhdUQ5Topz6J0ITAMiFL3iJHw svBzAMzXhFv

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
S	TMBMW1-i	MT20	10.0	16.0	7.00	0.25
C	TMBWW-t	MT20	8.0	9.0	3.50	4.25
D	TTWW-m	MT20	6.0	9.0	4.25	2.50
E	TMBWW-t	MT20	5.0	6.0		
F	TBW-w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TBW-t	MT20	6.0	6.0		
I	TTWW-m	MT20	6.0	9.0	4.25	2.50
J	TMBWW-t	MT20	8.0	9.0	3.50	4.25
K	TMBMW1-i	MT20	10.0	16.0	7.00	1.50
M	BMW-w	MT20	3.0	6.0		
N	BMWW-t	MT20	8.0	12.0		
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	6.0	9.0		
Q	BMWW-t	MT20	8.0	9.0		
R	BS-t	MT20	6.0	9.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMWW-t	MT20	8.0	12.0		
U	BMW-w	MT20	3.0	6.0		

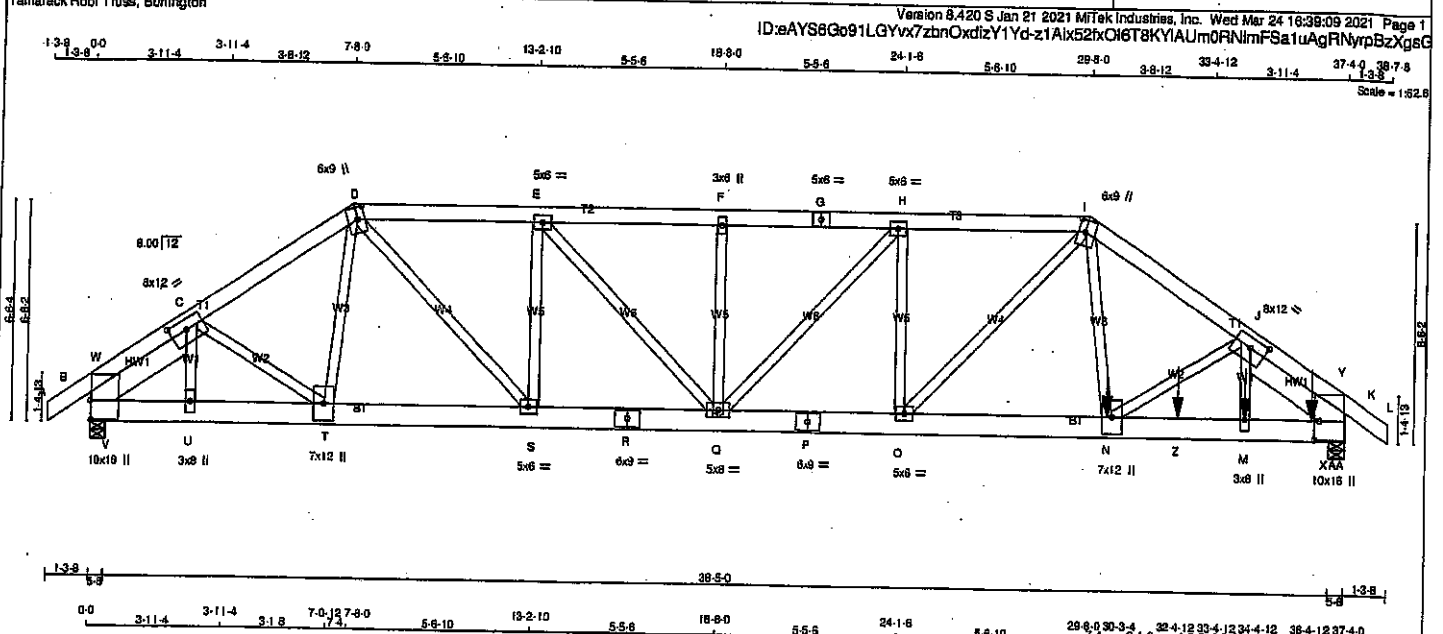
NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

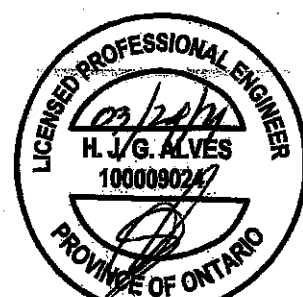


Structural component only
DWG# T-2108954 *2/2*

JOB NAME 417565	TRUSS NAME T9Z	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Mar 24 16:38:09 2021 Page 1 ID:AYS8G091LGYvx7zbnOxdizY1Yd-z1Aix52fxOIB8KYIAUm0RNmFSa1uAgRNYrpBzXgsC			



LUMBER N. L. G. A. RULES CHORDS SIZE DRY LUMBER A - D 2x8 DRY No.2 D - G 2x8 DRY No.2 G - I 2x8 DRY No.2 I - L 2x8 DRY No.2 B - R 2x8 DRY No.2 R - P 2x8 DRY No.2 P - K 2x8 DRY No.2 REINFORCING MEMBERS HW1 2x8 DRY No.2 HW2 2x8 DRY No.2 ALL WEBS 2x4 DRY No.2 DRY: SEASONED LUMBER DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF) TOP CHORDS: (0.122"x3") SPIRAL NAILS A-D 2 12 TOP D-G 2 12 TOP G-I 2 12 TOP I-L 2 12 TOP BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS B-R 2 4 TOP R-P 2 12 TOP P-K 2 4 TOP WEBS: (0.122"x3") SPIRAL NAILS 2x4 2 4 SIDE(2513.4) I-N 2 4 E-S 1 6 Q-E 1 6 F-Q 1 6 Q-H 1 6 H-O 1 6 C-T 1 6 N-J 1 6 S-D 1 6 O-I 1 6 2x8 2 6 STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.		DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT RECORD GROSS REACTION GROSS REACTION BRG BRG VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX JT 4176 0 4176 0 0 5-8 5-8 B 2950 1955 / 0 0 / 0 0 0 995 / 0 0 0 K 9075 6029 / 0 0 / 0 0 0 3046 / 0 0 0 UNFACTORED REACTIONS 1ST LOASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERMLIVE WIND DEAD SOIL B 2950 1955 / 0 0 / 0 0 0 995 / 0 0 0 K 9075 6029 / 0 0 / 0 0 0 3046 / 0 0 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS FACTORED FACTORED WEBS FACTORED MEMB. FORCE VERT. LOAD LG1 MAX MAX. MEMB. MAX. FORCE MAX. (LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC) FR-TO FROM TO A-B 0 / 8 -91.8 -91.8 0.02 (1) 10.00 U-C 0 / 171 0.01 (1) B-W -3450 / 0 -91.8 -91.8 0.05 (1) 6.25 S-E -2794 / 0 0.38 (1) W-C -1937 / 0 -91.8 -91.8 0.05 (1) 6.25 E-Q 0 / 3024 0.16 (1) C-D -5610 / 0 -91.8 -91.8 0.07 (1) 5.80 Q-F -468 / 0 0.06 (1) D-E -7369 / 0 -91.8 -91.8 0.09 (1) 5.21 Q-H 0 / 1198 0.71 (1) E-F -9408 / 0 -91.8 -91.8 0.11 (1) 4.72 C-H 0 / 3931 0.21 (1) F-G -9408 / 0 -91.8 -91.8 0.11 (1) 4.72 T-D -234 / 0 0.03 (1) G-H -9408 / 0 -91.8 -91.8 0.11 (1) 4.41 C-T 0 / 888 0.04 (1) H-I -11004 / 0 -91.8 -91.8 0.13 (1) 3.69 I-N 0 / 9189 0.49 (1) I-J -15598 / 0 -91.8 -91.8 0.19 (1) 5.36 N-J 0 / 132 0.01 (4) J-Y -6419 / 0 -91.8 -91.8 0.22 (1) 4.27 D-S 0 / 5964 0.21 (1) Y-K -11086 / 0 -91.8 -91.8 0.22 (1) 10.00 Q-I -1340 / 0 0.41 (1) K-L 0 / 8 -91.8 -91.8 0.02 (1) 10.00 V-W 0 / 1812 0.00 (1) B-V 0 / 1581 -18.5 -18.5 0.13 (1) 10.00 V-C -3839 / 0 0.12 (1) V-U 0 / 4104 -18.5 -18.5 0.21 (1) 10.00 J-X -11951 / 0 0.38 (1) U-T 0 / 4100 -18.5 -18.5 0.15 (1) 10.00 X-Y 0 / 5795 0.00 (1) T-S 0 / 4680 -18.5 -18.5 0.17 (1) 10.00 S-R 0 / 7369 -18.5 -18.5 0.27 (1) 10.00 R-Q 0 / 7369 -18.5 -18.5 0.27 (1) 10.00 Q-P 0 / 11004 -18.5 -18.5 0.39 (1) 10.00 P-O 0 / 11004 -18.5 -18.5 0.39 (1) 10.00 O-N 0 / 11909 -18.5 -18.5 0.42 (1) 10.00 N-M 0 / 12879 -18.5 -18.5 0.61 (1) 10.00 Z-M 0 / 12879 -18.5 -18.5 0.61 (1) 10.00 M-AA 0 / 12973 -18.5 -18.5 0.65 (1) 10.00 AA-X 0 / 12973 -18.5 -18.5 0.65 (1) 10.00 X-K 0 / 5119 -18.5 -18.5 0.40 (1) 10.00 SPECIFIED CONCENTRATED LOADS (LBS) JT LOC. LG1 MAX+ FACE DIR. TYPE HEEL CONN. M 34-4-12 -1830 -1830 --- FRONT VERT TOTAL --- C1 N 30-3-4 -8135 -8135 --- FRONT VERT TOTAL --- C1 Z 32-4-12 -780 -780 --- FRONT VERT TOTAL --- C1 AA 38-4-12 -209 -209 --- TOP VERT TOTAL --- C1 CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.		DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 25.6 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 39.0 PSF SPACING = 24.0 IN C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BOSC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 089-14 - TPIC 2014 (55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (1.24") CALCULATED VERT. DEFL.(LL) = L/999 (0.11") ALLOWABLE DEFL.(TL) = L/360 (1.24") CALCULATED VERT. DEFL.(TL) = L/999 (0.20") CSI: TC=0.26/1.00 (H-J-1), BC=0.85/1.00 (M-X-1), WB=0.71/1.00 (H-Q-1), SS=0.51/1.00 (K-X-1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.87 (I) (INPUT = 0.90) JSI METAL= 0.77 (B) (INPUT = 1.00)	
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Structural component only
DWG# T-2108983

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417565	T9Z	1	3	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MTek Industries, Inc. Wed Mar 24 16:39:10 2021 Page 2
ID:AYS6G691LGYvx7zbnOxdizY1Yd-RDk49R3H-iQz5lvist77YfyTWoomLQpf1hPLdzXqsf

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-i	MT20	10.0	16.0	6.75	0.25
C	TMBWW-i	MT20	8.0	12.0	3.50	5.75
D	TTWW+m	MT20	6.0	9.0	4.25	2.50
E	TMBWW-i	MT20	5.0	6.0		
F	TMBW+w	MT20	3.0	6.0		
G	TS-i	MT20	5.0	6.0		
H	TMBWW-i	MT20	5.0	6.0		
I	TTWW+m	MT20	6.0	9.0	4.25	2.50
J	TMBWW-i	MT20	8.0	12.0	3.50	5.75
K	TMBMW1-i	MT20	10.0	16.0	6.75	1.50
M	BMW+w	MT20	3.0	8.0		
N	BMWW-i	MT20	7.0	12.0		
O	BMWW-i	MT20	5.0	6.0		
P	BS-i	MT20	6.0	9.0		
Q	BMWW-i	MT20	5.0	6.0		
R	BS-i	MT20	6.0	9.0		
S	BMWW-i	MT20	6.0	6.0		
T	BMWW-i	MT20	7.0	12.0		
U	BMW+w	MT20	3.0	8.0		

NOTES- (1)

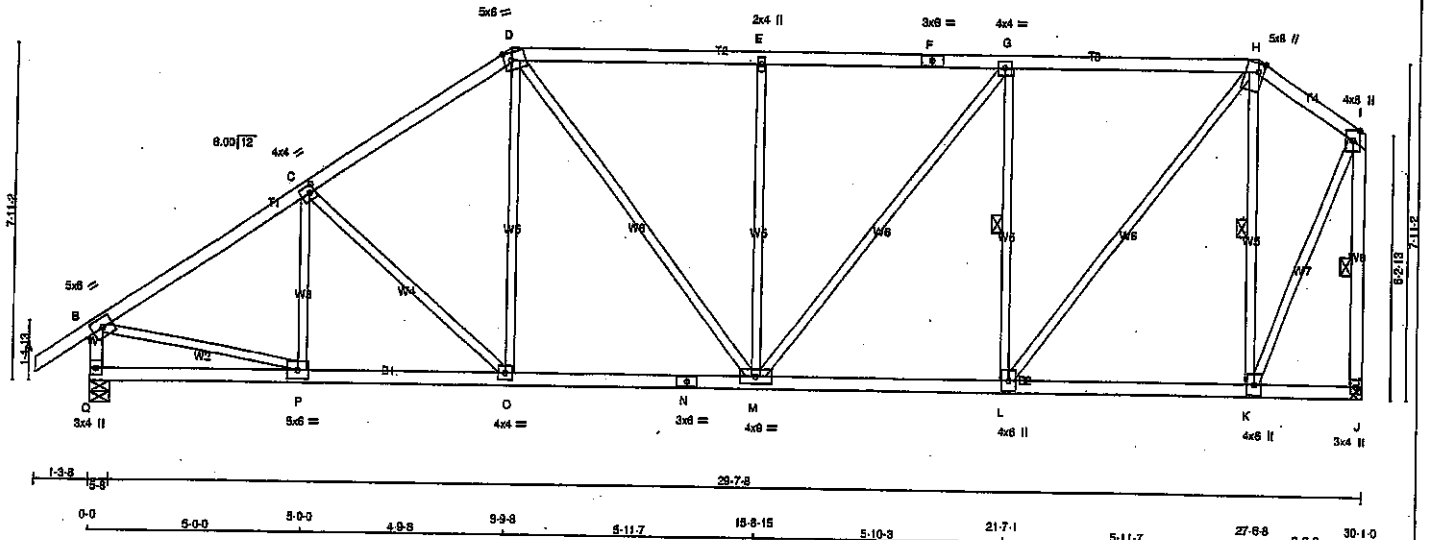
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108983 *W*

JOB NAME 417564	TRUSS NAME T10	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Mar 24 16:11:48 2021 Page 1
ID: eAYS6Go91LGYvx7zbnOxdizY1Yd-WKWKX_BJOCYx5yNag1Yy?OvoilTn2k674ZxXiozXhFv
Scale = 1/48



TOTAL WEIGHT = 145 lb

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - I	2x4	DRY No.2	SPF
O - B	2x4	DRY No.2	SPF
J - I	2x4	DRY No.2	SPF
Q - N	2x4	DRY No.2	SPF
N - J	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	8.0	2.25	2.00
E	TMVW-t	MT20	2.0	4.0		
F	TS-1	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.50	1.50
I	TMVW-t	MT20	4.0	6.0	Edge	
J	BMV1-p	MT20	3.0	4.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMVW-t	MT20	4.0	6.0		
M	BMVW-t	MT20	4.0	6.0		
N	BS-1	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	5.0	6.0		
Q	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
Q	1784	0	0	5-8
J	1658	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	1259	840.0	0.0	0.0	0.0	420.0	0.0
J	1173	770.0	0.0	0.0	0.0	403.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-L, H-K, I-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		FR-TO			
A-B	0/35	-91.8	-91.8 0.12 (1)	10.00	P-C	-259/0	0.09 (1)
B-C	-1932/0	-91.8	-91.8 0.47 (1)	4.36	C-O	-258/0	0.20 (1)
C-D	-1763/0	-91.8	-91.8 0.45 (1)	4.53	O-D	0/276	0.06 (1)
D-E	-1678/0	-91.8	-91.8 0.52 (1)	4.51	D-M	0/377	0.06 (1)
E-F	-1678/0	-91.8	-91.8 0.53 (1)	4.50	M-E	-588/0	0.70 (1)
F-G	-1678/0	-91.8	-91.8 0.53 (1)	4.50	M-G	0/501	0.11 (1)
G-H	-1371/0	-91.8	-91.8 0.50 (1)	4.90	L-G	-989/0	0.39 (1)
H-I	-636/0	-91.8	-91.8 0.11 (1)	6.25	L-H	0/1395	0.51 (1)
Q-B	-1744/0	0.0	0.0 0.18 (1)	6.31	K-H	-1098/0	0.43 (1)
J-I	-1649/0	0.0	0.0 0.31 (1)	5.18	B-P	0/1869	0.38 (1)
					K-I	0/1291	0.29 (1)
Q-P	0/0	-18.5	-18.5 0.09 (4)	10.00			
P-O	0/1832	-18.5	-18.5 0.32 (1)	10.00			
O-N	0/1444	-18.5	-18.5 0.29 (1)	10.00			
N-M	0/1444	-18.5	-18.5 0.29 (1)	10.00			
M-L	0/1371	-18.5	-18.5 0.29 (1)	10.00			
L-K	0/512	-18.5	-18.5 0.17 (4)	10.00			
K-J	0/0	-18.5	-18.5 0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 6.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015.

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.53/1.00 (E-G:1), BC=0.32/1.00 (O-P:1), WB=0.70/1.00 (E-K:1), SSI=0.25/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

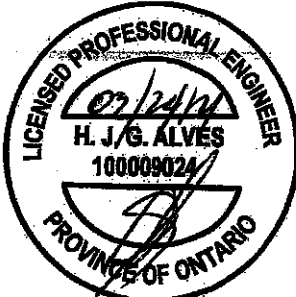
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

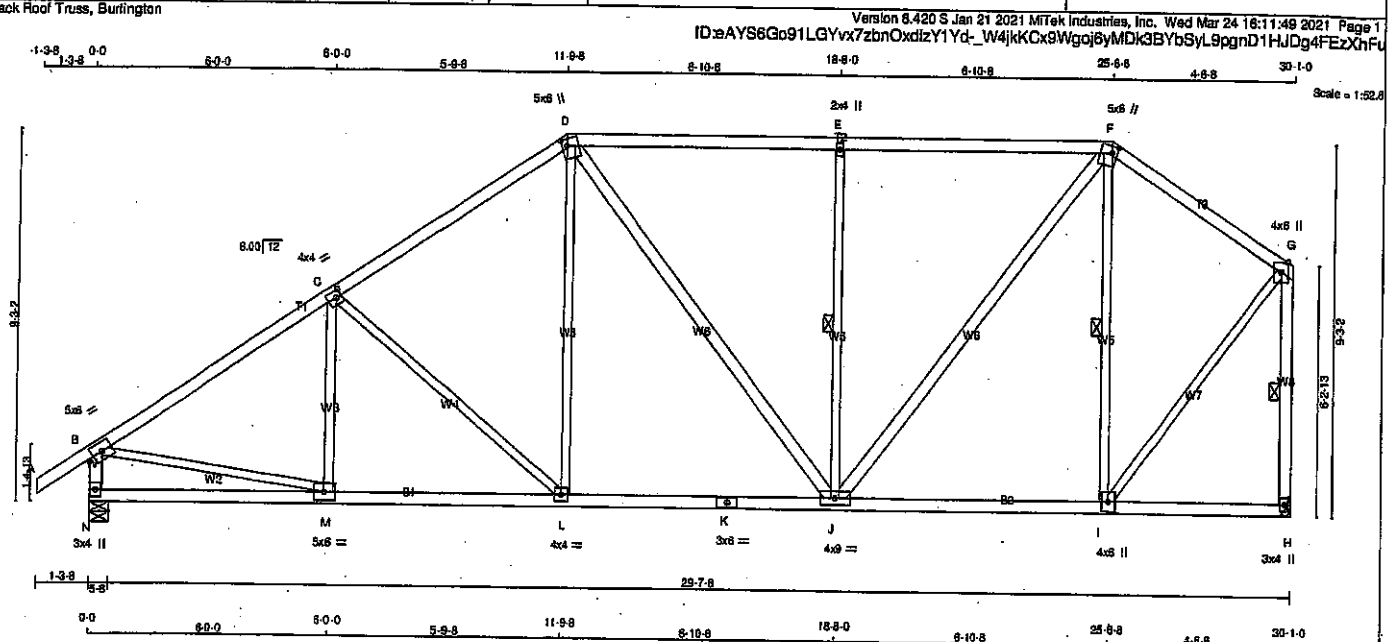
JSI GRIP=0.86 (I) (INPUT = 0.90)

JSI METAL=0.48 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108955

JOB NAME 417564	TRUSS NAME T11	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



TOTAL WEIGHT = 149 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	QESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
D - J	2x4	DRY	No.2
J - F	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT	IN-SX
N	1784	0	0	5-8
H	1658	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	1258	840 / 0	0 / 0	0 / 0	0 / 0	420 / 0	0 / 0
H	1173	770 / 0	0 / 0	0 / 0	0 / 0	403 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-J, F-I, G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX. (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	M-C	-181 / 35	0.08 (1)
B-C	-1952 / 0	-91.8	-91.8 0.50 (1)	4.31	C-L	-427 / 0	0.53 (1)
C-D	-1838 / 0	-91.8	-91.8 0.46 (1)	4.88	L-D	0 / 396	0.09 (1)
D-E	-1378 / 0	-91.8	-91.8 0.60 (1)	4.72	D-J	0 / 68	0.01 (1)
E-F	-1378 / 0	-91.8	-91.8 0.60 (1)	4.72	J-E	-779 / 0	0.44 (1)
F-G	-890 / 0	-91.8	-91.8 0.25 (1)	6.23	F-F	0 / 1075	0.17 (1)
N-B	-1737 / 0	0.0	0.0 0.18 (1)	6.32	I-F	-913 / 0	0.48 (1)
H-G	-1629 / 0	0.0	0.0 0.31 (1)	5.19	B-M	0 / 1880	0.38 (1)
N-M	0 / 0	-18.5	-18.5 0.15 (4)	10.00	I-G	0 / 1183	0.27 (1)
M-L	0 / 1654	-18.5	-18.5 0.35 (1)	10.00			
L-K	0 / 1335	-18.5	-18.5 0.31 (1)	10.00			
K-J	0 / 1335	-18.5	-18.5 0.31 (1)	10.00			
J-I	0 / 722	-18.5	-18.5 0.24 (4)	10.00			
I-H	0 / 0	-18.5	-18.5 0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 8.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015.

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ASC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPO 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.00")
CALCULATED VERT. DEFL. (LL) = L/999 (0.08")
ALLOWABLE DEFL. (TL) = L/360 (1.00")
CALCULATED VERT. DEFL. (TL) = L/999 (0.14")

CSI: TC=0.80/1.00 (D-E:1), BC=0.35/1.00 (L-M:1), WB=0.53/1.00 (C-L:1), SS=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 OOMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

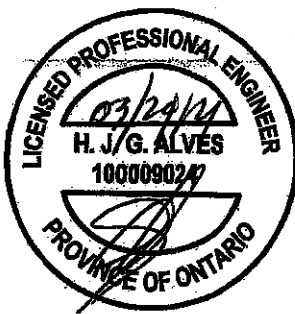
PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	850	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

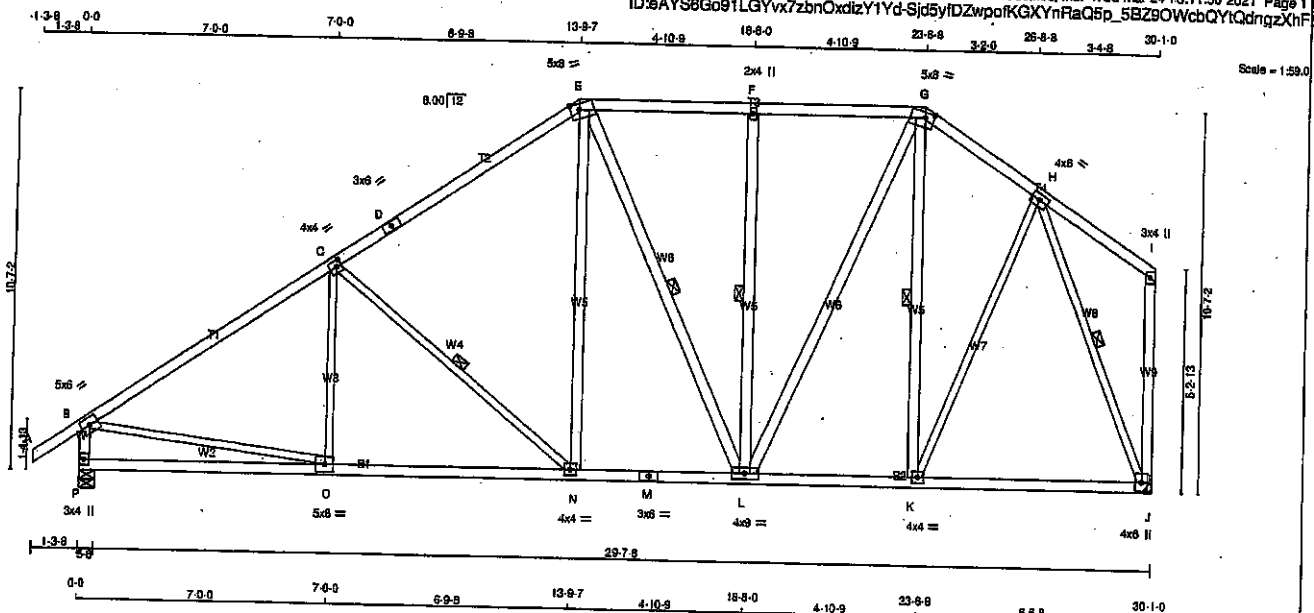
JSI GRIP = 0.84 (3) (INPUT = 0.80)

JSI METAL = 0.49 (5) (INPUT = 1.00)



Structural component only
DWG# T-2108956

JOB NAME 417564	TRUSS NAME T12	QUANTITY 7	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - E	2x4	DRY No.2
E - G	2x4	DRY No.2
G - I	2x4	DRY No.2
I - J	2x4	DRY No.2
J - K	2x4	DRY No.2
K - L	2x4	DRY No.2
L - M	2x4	DRY No.2
M - J	2x4	DRY No.2
ALL WEBS	2x4	DRY No.2
EXCEPT		
O - C	2x3	DRY No.2
C - N	2x3	DRY No.2
K - H	2x3	DRY No.2
B - O	2x3	DRY No.2
H - J	2x3	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-1	MT20	5.0	6.0		
C TMVW-1	MT20	4.0	4.0	2.00	1.50
D TS-1	MT20	3.0	6.0		
E TTWW-m	MT20	5.0	8.0	2.00	3.00
F TMVW-w	MT20	2.0	4.0		
G TTWW-m	MT20	5.0	8.0	1.75	3.00
H TMVW-1	MT20	4.0	6.0		
I TMVW-p	MT20	3.0	4.0		
J BMVW-1+p	MT20	4.0	6.0		
K BMVW-1	MT20	4.0	4.0		
L BMVW-1	MT20	4.0	9.0		
M BS-1	MT20	3.0	6.0		
N BMVW-1	MT20	4.0	4.0		
O BMVW-1	MT20	5.0	6.0		
P BMV-1+p	MT20	3.0	4.0		

NOTES:

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
JT	VERT	DOWN	UP	IN-SX
P	1784	0	1784	0
J	1658	0	1658	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3.8.

UNFACTORED REACTIONS

1ST LC CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
P	1259 840 0 0 0 0 420 0 0 0
J	1173 770 0 0 0 0 403 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-N, E-L, F-L, G-K, H-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO		
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	O-C	-111 / 79
B-C	-1950 / 0	-91.8	-91.8 0.72 (1)	3.99	C-N	-590 / 0
C-D	-1491 / 0	-91.8	-91.8 0.64 (1)	4.53	E-L	-100 / 0
D-E	-1491 / 0	-91.8	-91.8 0.64 (1)	4.53	N-E	0 / 489
E-F	-1167 / 0	-91.8	-91.8 0.29 (1)	5.55	L-F	-548 / 0
F-G	-1167 / 0	-91.8	-91.8 0.29 (1)	5.55	G-K	0 / 779
G-H	-1030 / 0	-91.8	-91.8 0.13 (1)	8.04	H-J	-427 / 0
H-I	0 / 19	-91.8	-91.8 0.16 (1)	10.00	K-H	0 / 603
P-B	-1731 / 0	0.0	0.0 0.18 (1)	6.33	B-O	0 / 1677
J-I	-118 / 0	0.0	0.0 0.10 (1)	7.81	H-J	-1609 / 0
P-O	0 / 0	-18.5	-18.5 0.22 (4)	10.00		
O-N	0 / 1658	-18.5	-18.5 0.38 (1)	10.00		
N-M	0 / 1210	-18.5	-18.5 0.27 (1)	10.00		
M-L	0 / 1210	-18.5	-18.5 0.27 (1)	10.00		
L-K	0 / 833	-18.5	-18.5 0.25 (4)	10.00		
K-J	0 / 808	-18.5	-18.5 0.23 (4)	10.00		

TOTAL WEIGHT = 7 X 173 = 1211 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.00")
CALCULATED VERT. DEFL. (LL) = L/999 (0.06")
ALLOWABLE DEFL. (TL) = L/360 (1.00")
CALCULATED VERT. DEFL. (TL) = L/999 (0.14")

CSI: TC=0.72/1.00 (B-C:1), BC=0.38/1.00 (N-O:1), WB=0.89/1.00 (H-J:1), SS=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN
MT20	850	371	1747	788	1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

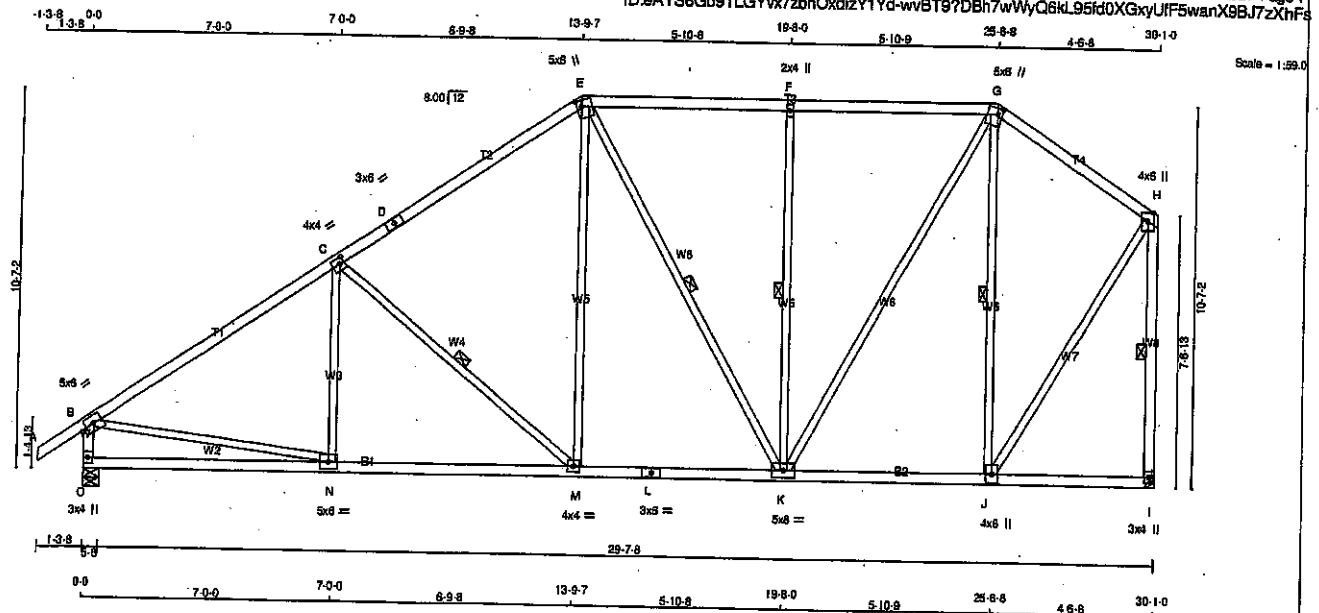
JSI GRIP = 0.85 (H) (INPUT = 0.90)
JSI METAL = 0.50 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108957

JOB NAME 417564	TRUSS NAME T13	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:AYS6G091LGyvx7zbnOxdizY1Yd-wvBT9?DBH7wWYQ6kL95fd0XGxyUfF5wanX9Bj7ZxhFs



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
E - G	2x4	DRY	No.2	SPF		
H - I	2x4	DRY	No.2	SPF		
J - L	2x4	DRY	No.2	SPF		
O - L	2x4	DRY	No.2	SPF		
L - I	2x4	DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	5.0	8.0			
C TMVW-t	MT20	4.0	4.0	2.00	1.50	
D TS-t	MT20	3.0	8.0			
E TTWW-t	MT20	5.0	8.0	2.00	1.50	
F TMVW-t	MT20	2.0	4.0			
G TTWW-t	MT20	5.0	8.0	2.00	1.50	
H TMVW-t	MT20	4.0	8.0	Edge		
I BMV1-p	MT20	3.0	4.0			
J BMVW-t	MT20	4.0	8.0			
K BMVW-t	MT20	5.0	8.0			
L BS-t	MT20	3.0	8.0			
M BMVW-t	MT20	4.0	4.0			
N BMVW-t	MT20	5.0	8.0			
O BMV1-p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	UP
O	1784	0	1784	0
I	1858	0	1858	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS		MAX. MIN. COMPONENT REACTIONS						
JT	1ST LCASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
		1259	840 / 0	0 / 0	0 / 0	0 / 0	420 / 0	0 / 0
I		1173	770 / 0	0 / 0	0 / 0	0 / 0	403 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, E-K, F-K, G-J, H-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO						FR-TO		
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	N-C	-112 / 78	0.08 (1)
B-C	-1949 / 0	-91.8	-91.8	0.72 (1)	3.99	C-M	-588 / 0	0.33 (1)
C-D	-1493 / 0	-91.8	-91.8	0.84 (1)	4.53	M-E	-0 / 494	0.11 (1)
D-E	-1493 / 0	-91.8	-91.8	0.84 (1)	4.53	E-K	-147 / 0	0.16 (1)
E-F	-1137 / 0	-91.8	-91.8	0.42 (1)	5.41	K-F	-684 / 0	0.52 (1)
F-G	-1137 / 0	-91.8	-91.8	0.42 (1)	5.41	G-J	-0 / 1028	0.23 (1)
G-H	-786 / 0	-91.8	-91.8	0.25 (1)	6.25	J-G	-882 / 0	0.70 (1)
H-I	-1731 / 0	0.0	0.0	0.18 (1)	6.33	B-N	0 / 1877	0.38 (1)
I-J	-1827 / 0	0.0	0.0	0.43 (1)	5.19	J-H	0 / 1179	0.27 (1)
O-N	0 / 0	-18.5	-18.5	0.22 (4)	10.00			
N-M	0 / 1657	-18.5	-18.5	0.38 (1)	10.00			
M-L	0 / 1210	-18.5	-18.5	0.27 (1)	10.00			
L-K	0 / 1210	-18.5	-18.5	0.27 (1)	10.00			
K-J	0 / 627	-18.5	-18.5	0.18 (1)	10.00			
J-I	0 / 0	-18.5	-18.5	0.12 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.00")
CALCULATED VERT. DEFL. (LL) = L/999 (0.07")
ALLOWABLE DEFL. (TL) = L/360 (1.00")
CALCULATED VERT. DEFL. (TL) = L/999 (0.14")

CS: TC=0.72/1.00 (B-C:1), BC=0.38/1.00 (M-N:1), WB=0.70/1.00 (G-J:1), SS=0.28/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.84 (H) (INPUT = 0.90)

JSI METAL= 0.50 (B) (INPUT = 1.00)



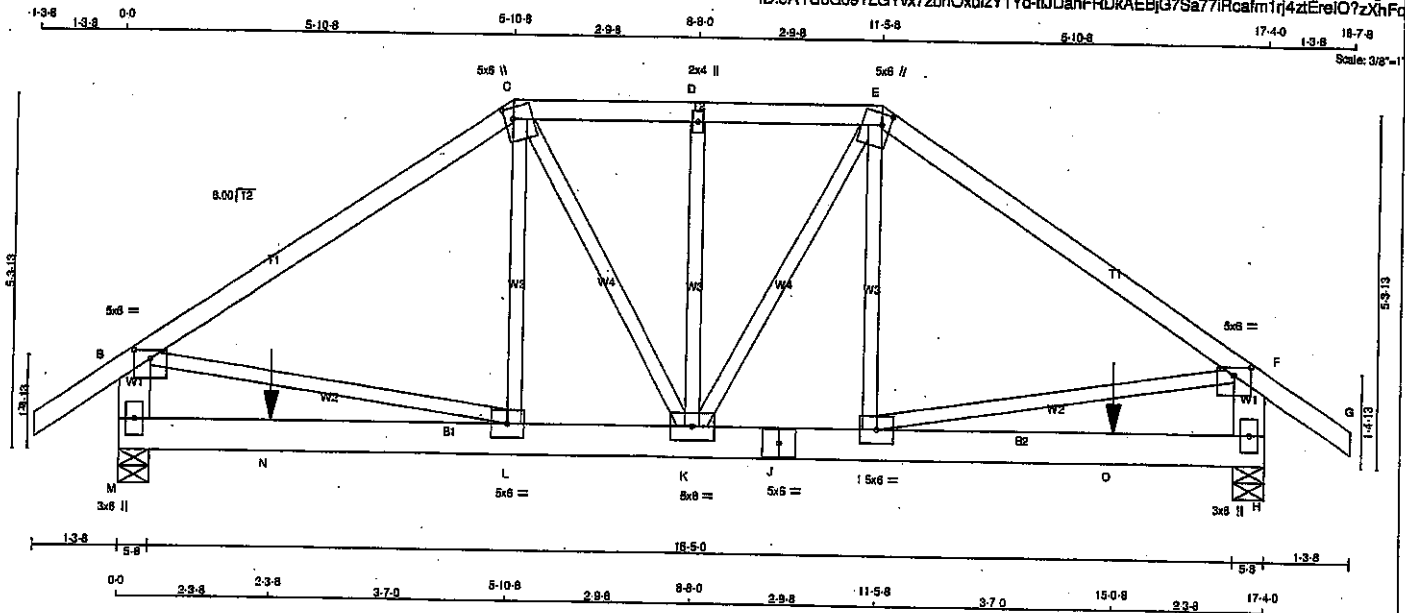
Structural component only
DWG# T-2108958

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JOB NAME 417564	TRUSS NAME T15	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID: eAYS6Gg91LGYvx7zbnOxdlzY1Yd-IJDahFRDkAEBjG7Sa77IRcafmlr4ztEreIQ7zXhFc



TOTAL WEIGHT = 89 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - B	2x6	DRY	No.2
H - F	2x6	DRY	No.2
M - J	2x6	DRY	No.2
J - H	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50 3.00
C	TTWW-m	MT20	5.0	6.0	2.00 1.50
D	TMVW-w	MT20	2.0	4.0	
E	TTWW-m	MT20	6.0	6.0	2.00 1.50
F	TMVW-p	MT20	5.0	6.0	1.50 3.00
H	BMV1-p	MT20	3.0	6.0	
I	BMVW-t	MT20	5.0	6.0	
J	BS-t	MT20	5.0	6.0	
K	BMVW-t	MT20	5.0	6.0	
L	BMVW-t	MT20	5.0	6.0	
M	BMV1-p	MT20	3.0	6.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED..		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
M	2088	0	2088	0	5-8	5-8	
H	2088	0	2088	0	5-8	5-8	

UNFACTORED REACTIONS		MAX/MIN COMPONENT REACTIONS	
1ST CASE			
JT	COMBINED	SNOW	LIVE
M	1471	996 / 0	0 / 0
H	1471	996 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FORCE (LBS)	MAX. (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 35	-91.8	-91.8 0.14 (1)	10.00	L-C	6 / 719	0.18 (1)
B-C	-1832 / 0	-91.8	-91.8 0.77 (1)	4.13	C-K	-213 / 0	0.11 (1)
C-D	-1266 / 0	-91.8	-91.8 0.15 (1)	5.53	K-D	-303 / 0	0.12 (1)
D-E	-1266 / 0	-91.8	-91.8 0.15 (1)	5.53	K-E	-213 / 0	0.11 (1)
E-F	-1831 / 0	-91.8	-91.8 0.77 (1)	4.13	I-E	0 / 719	0.18 (1)
F-G	0 / 35	-91.8	-91.8 0.14 (1)	10.00	B-L	0 / 1375	0.34 (1)
M-B	-1529 / 0	0.0	0.0 0.11 (1)	7.81	I-F	0 / 1375	0.34 (1)
H-F	-1529 / 0	0.0	0.0 0.11 (1)	7.81			
M-N	0 / 0	-18.5	-18.5 0.91 (1)	10.00			
N-L	0 / 0	-18.5	-18.5 0.91 (1)	10.00			
L-K	0 / 1371	-18.5	-18.5 0.89 (1)	10.00			
K-J	0 / 1371	-18.5	-18.5 0.89 (1)	10.00			
J-I	0 / 1371	-18.5	-18.5 0.89 (1)	10.00			
I-O	0 / 0	-18.5	-18.5 0.91 (1)	10.00			
O-H	0 / 0	-18.5	-18.5 0.91 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)			
JT	LOC.	LC1 MAX-	MAX+
N	2-3-8	-709	-709
O	15-0-8	-709	-709

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.58")
CALCULATED VERT. DEFL. (LL) = L/999 (0.10")
ALLOWABLE DEFL. (TL) = L/360 (0.58")
CALCULATED VERT. DEFL. (TL) = L/999 (0.18")

CSI: TC=0.77/1.00 (B-C:1), BC=0.91/1.00 (L-M:1),
WB=0.34/1.00 (B-L:1), SSI=0.37/1.00 (I-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

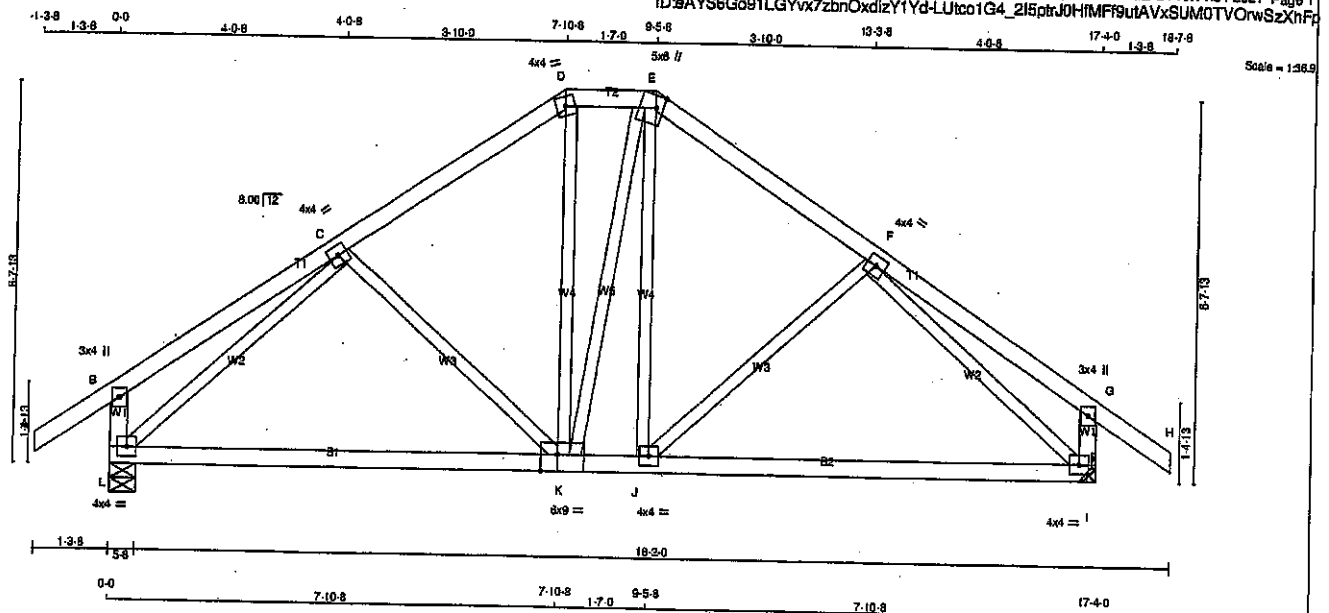
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.80 (C) (INPUT = 0.90)
JSI METAL= 0.41 (J) (INPUT = 1.00)



Structural component only
DWG# T-2108960

JOB NAME 417564	TRUSS NAME T16	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			



LUMBER	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES				
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0	
C	TMVW-t	MT20	4.0	4.0	
D	TTW-m	MT20	4.0	4.0	
E	TTWW-m	MT20	5.0	8.0	2.50 1.50
F	TMVW-t	MT20	4.0	4.0	
G	TMV-p	MT20	3.0	4.0	
I	BMVW-t	MT20	4.0	4.0	
J	BMVW-t	MT20	4.0	4.0	
K	BSWW-t	MT20	6.0	9.0	Edge 3.50
L	BMVW-t	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
L	1081 0	1081 0	5-8	5-8
I	1081 0	1081 0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS
	COMBINED	SNOW LIVE PERM LIVE WIND DEAD SOIL
L	762	514 0 0 0 0 0 0 0
I	762	514 0 0 0 0 0 249 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WELLS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO			
A-B	0 135	-91.8	-91.8 0.12 (1)	10.00	C-K	-197 0	0.10 (1)	
B-C	0 23	-91.8	-91.8 0.23 (1)	10.00	K-D	0 219	0.05 (1)	
C-D	-797 0	-91.8	-91.8 0.18 (1)	6.25	K-E	-102 0	0.09 (4)	
D-E	-847 0	-91.8	-91.8 0.03 (1)	6.25	J-E	-193 0	0.08 (4)	
E-F	-819 0	-91.8	-91.8 0.18 (1)	6.25	J-F	-193 0	0.10 (1)	
F-G	0 23	-91.8	-91.8 0.23 (1)	10.00	L-C	-1084 0	0.51 (1)	
G-H	0 35	-91.8	-91.8 0.12 (1)	10.00	F-I	-1104 0	0.52 (1)	
L-B	-264 0	0.0	0.0 0.03 (1)	7.81				
I-G	-285 0	0.0	0.0 0.03 (1)	7.81				
L-K	0 789	-18.5	-18.5 0.40 (4)	10.00				
K-J	0 666	-18.5	-18.5 0.34 (4)	10.00				
J-I	0 804	-18.5	-18.5 0.34 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL -OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
-PART 9 OF NBC 2018, ABC 2019
-PART 9 OF OBC 2012 (2019 AMENDMENT)
-CSA 086-14
-TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.58")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.58")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CS: TC=0.23/1.00 (B-C:1), BC=0.40/1.00 (K-L:4), WB=0.52/1.00 (F-I:1), SS=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (I) (INPUT = 0.90)
JSI METAL = 0.36 (F) (INPUT = 1.00)



Structural component only
DWG# T-2108961

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13.8 0.0 8.0 2.0 2.0 4.8 8.0 2.0 2.0 10.8 2.0 2.0 14.8 2.0 2.0 16.8 17.4 18.7
1.38 8.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0
444 =



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD = 39.0 PSF			

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED		MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD L1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	Q-F	-133 / 0	0.12 (1)	
B-C	-22 / 0	-91.8	-91.8	0.08 (1)	6.25	R-E	-212 / 0	0.11 (1)	
C-D	-35 / 0	-91.8	-91.8	0.08 (1)	6.25	S-D	-157 / 0	0.05 (1)	
D-E	-17 / 0	-91.8	-91.8	0.05 (1)	6.25	T-C	-245 / 0	0.05 (1)	
E-F	-29 / 0	-91.8	-91.8	0.05 (1)	6.25	P-G	-212 / 0	0.11 (1)	
F-G	-29 / 0	-91.8	-91.8	0.05 (1)	6.25	N-H	-157 / 0	0.05 (1)	
G-H	-17 / 0	-91.8	-91.8	0.05 (1)	6.25	B-T	-245 / 0	0.05 (1)	
H-I	-35 / 0	-91.8	-91.8	0.08 (1)	6.25	B-T	0 / 30	0.01 (1)	
I-J	-22 / 0	-91.8	-91.8	0.08 (1)	6.25	M-J	0 / 30	0.01 (1)	
J-K	0 / 35	-91.8	-91.8	0.12 (1)	10.00				
U-B	-253 / 0	0.0	0.0	0.03 (1)	7.81				
L-J	-253 / 0	0.0	0.0	0.03 (1)	7.81				
U-T	0 / 0	-18.5	-18.5	0.03 (4)	10.00				
T-S	0 / 21	-18.5	-18.5	0.03 (4)	10.00				
S-R	0 / 18	-18.5	-18.5	0.02 (4)	10.00				
R-Q	0 / 15	-18.5	-18.5	0.02 (4)	10.00				
Q-P	0 / 15	-18.5	-18.5	0.02 (4)	10.00				
P-O	0 / 18	-18.5	-18.5	0.02 (4)	10.00				
O-N	0 / 18	-18.5	-18.5	0.02 (4)	10.00				
N-M	0 / 21	-18.5	-18.5	0.03 (4)	10.00				
M-L	0 / 0	-18.5	-18.5	0.03 (4)	10.00				

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA C86-14
- TPC 2018

(55 % OF 31.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (A-B-1), BC=0.03/1.00 (S-T-4),
WB=0.12/1.00 (F-Q-1), SS=0.09/1.00 (J-K-1)

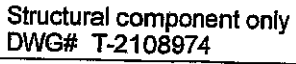
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPARISON LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

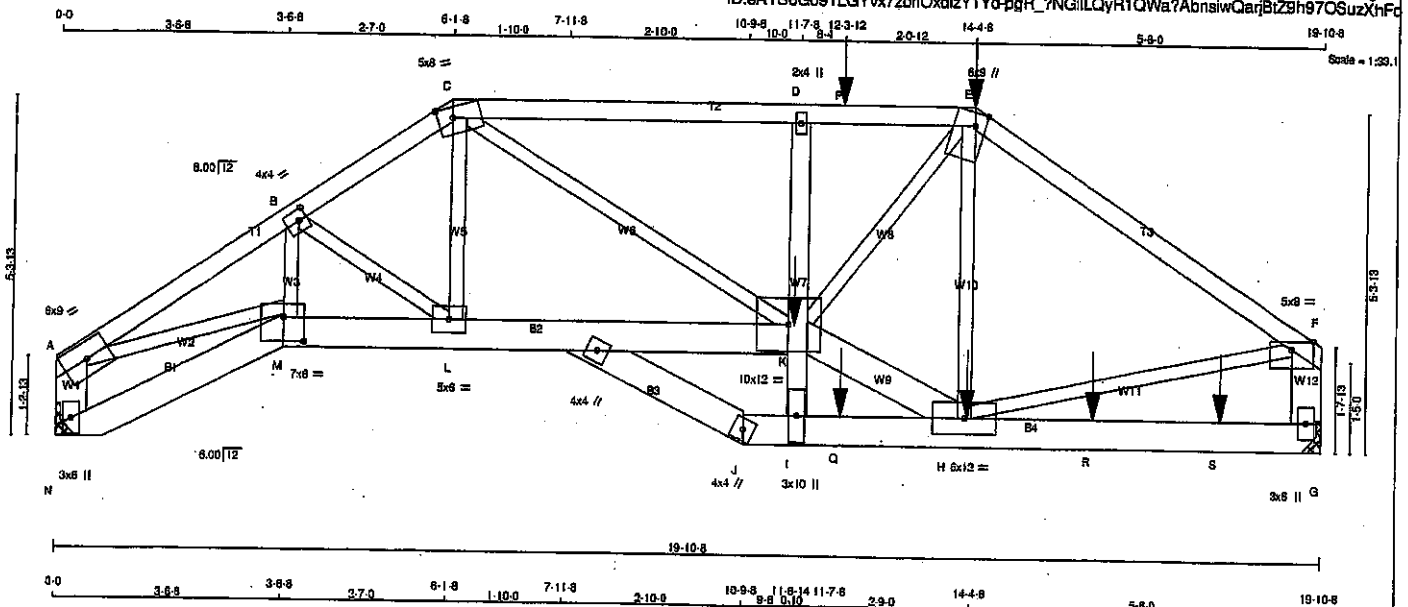
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417564	T18	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 108 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
N - A	2x6	DRY	No.2
G - F	2x6	DRY	No.2
N - M	2x6	DRY	No.2
M - K	2x6	DRY	No.2
J - G	2x6	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			
I - D	2x4	DRY	No.2
K - H	2x6	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-I	MT20	6.0	9.0		
B	TMVW-I	MT20	4.0	4.0	2.00	1.50
C	TTWW-m	MT20	5.0	8.0	2.00	2.75
D	TMVW-w	MT20	2.0	4.0		
E	TTWW-m	MT20	6.0	9.0	Edge	2.00
F	TMVW-p	MT20	5.0	8.0		
G	BMV1-p	MT20	3.0	6.0		
H	BMVWV-I	MT20	6.0	12.0		
I	BMV-w	MT20	3.0	10.0		
J	NP-t	MT20	4.0	4.0		
K	BMVWVW-I	MT20	10.0	12.0		
L	BMVW-I	MT20	5.0	6.0		
M	BBVW-p	MT20	7.0	8.0	4.50	3.75
N	BMV1-p	MT20	3.0	6.0		
O	NP-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
N	1734	0	1734	0	0	MECHANICAL
G	2230	0	2230	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT N, G. MINIMUM BEARING LENGTH AT JOINT N = 3-8, JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1225	812.0	0.0	0.0	0.0	414.0	0.0
G	1578	1030.0	0.0	0.0	0.0	548.0	0.0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LOC 1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	-3732.0	-91.8	-91.8	0.43 (1)	3.26	M-B	0.584	0.14 (1)	
B-C	-2945.0	-91.8	-91.8	0.19 (1)	3.88	B-L	-790.0	0.17 (1)	
C-D	-3660.0	-91.8	-91.8	0.70 (1)	3.03	L-C	0.544	0.13 (1)	
D-E	-3648.0	-91.8	-91.8	0.53 (1)	3.08	C-K	0.1438	0.36 (1)	
E-F	-2586.0	-91.8	-91.8	0.53 (1)	3.05	H-E	-1400.0	0.57 (1)	
F-G	-1702.0	-91.8	-91.8	0.82 (1)	3.40	H-F	0.2206	0.55 (1)	
N-A	-1702.0	0.0	0.0	0.12 (1)	7.55	I-K	0.21	0.12 (1)	
G-F	-2156.0	0.0	0.0	0.16 (1)	6.90	K-D	-609.0	0.13 (1)	
						K-H	0.2313	0.30 (1)	
N-M	0.0	-18.5	-18.5	0.04 (4)	10.00	K-E	0.2550	0.63 (1)	
M-L	0.3078	-18.5	-18.5	0.43 (1)	10.00	A-M	0.3155	0.78 (1)	
L-K	0.2451	-18.5	-18.5	0.35 (1)	10.00				
J-I	0.0	-18.5	-18.5	0.01 (4)	10.00				
I-Q	0.55	-18.5	-18.5	0.14 (4)	10.00				
Q-H	0.55	-18.5	-18.5	0.14 (4)	10.00				
H-R	0.0	-18.5	-18.5	0.14 (4)	10.00				
R-S	0.0	-18.5	-18.5	0.14 (4)	10.00				
S-G	0.0	-18.5	-18.5	0.14 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	14-4-8	-337	-337	--	FRONT	VERT	TOTAL	--	C1
H	14-3-12	-29	-29	--	FRONT	VERT	TOTAL	--	C1
K	11-8-14	-667	-667	--	FRONT	VERT	TOTAL	--	C1
P	12-3-12	-122	-122	--	FRONT	VERT	TOTAL	--	C1
Q	12-3-12	-29	-29	--	FRONT	VERT	TOTAL	--	C1
R	16-3-12	-29	-29	--	FRONT	VERT	TOTAL	--	C1
S	18-3-12	-29	-29	--	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.68")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.12")
ALLOWABLE DEFL.(TL) = $L/360$ (0.68")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.22")

CSI: TC=0.82/1.00 (E-F:1), BC=0.43/1.00 (L-M:1), WB=0.78/1.00 (A-M:1), SS=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
650	371	1747	788
1987	1873		

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

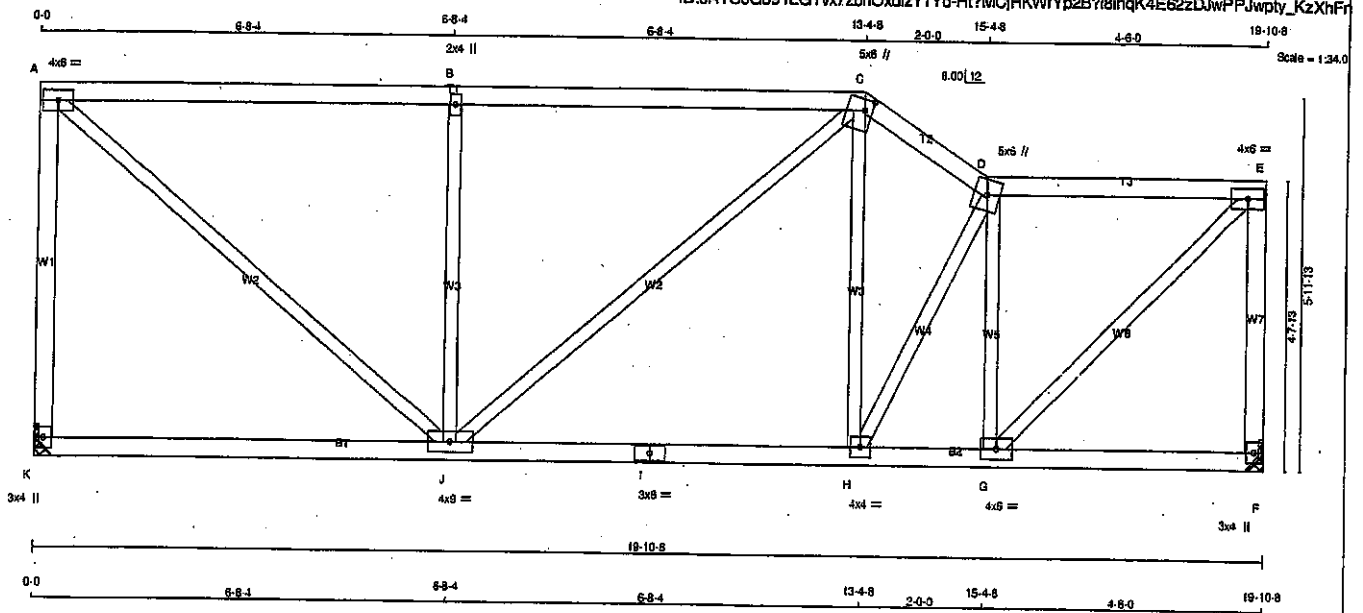
JSI GRIP=0.90 (A) (INPUT = 0.90)
JSI METAL=0.85 (M) (INPUT = 1.00)



Structural component only
DWG# T-2108963

JOB NAME 417564	TRUSS NAME T19	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:AYS6Go9 1LGyvx7zbnOxdizY1Yd-Ht?MCJHKWfYp2B?i8hqK4E62zDJwPPJwpty_KzXhFr



CHORDS	SIZE	LUMBER	DESCR.
K - A	2x4	DRY No.2	SPF
A - C	2x4	DRY No.2	SPF
C - D	2x4	DRY No.2	SPF
D - E	2x4	DRY No.2	SPF
E - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
I - F	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

JT TYPE	PLATES	W	LEN	Y	X
A	MTWV-t	MT20	4.0	6.0	
B	TMW-w	MT20	2.0	4.0	
C	TTWW-m	MT20	5.0	6.0	2.00 1.50
D	TTWW-m	MT20	5.0	6.0	
E	MTWV-t	MT20	4.0	6.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMWV-t	MT20	4.0	6.0	
H	BMWV-t	MT20	4.0	4.0	
I	BS-t	MT20	3.0	6.0	
J	BMWVW-t	MT20	4.0	6.0	
K	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
K	1098	0	1098	0
F	1098	0	1098	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K, F. MINIMUM BEARING LENGTH AT JOINT K = 3-8, JOINT F = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	775	508 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0
F	775	508 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (PLF)	LC2 MAX (PLF)	UNBRAC LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX (LC)
FR-TO							FR-TO			
K-A	-1048 / 0	0.0	0.0	0.71 (1)	7.89	A-J	0 / 1256	0.28 (1)		
A-B	-958 / 0	-91.8	-91.8	0.76 (1)	4.98	J-B	-761 / 0	0.42 (1)		
B-C	-958 / 0	-91.8	-91.8	0.76 (1)	4.98	J-C	0 / 1141	0.03 (1)		
C-D	-1020 / 0	-91.8	-91.8	0.07 (1)	6.12	H-C	0 / 150	0.04 (4)		
D-E	-862 / 0	-91.8	-91.8	0.34 (1)	6.11	H-D	-75 / 0	0.03 (1)		
E-F	-1058 / 0	0.0	0.0	0.37 (1)	7.85	G-D	-800 / 0	0.25 (1)		
						G-E	0 / 1214	0.27 (1)		
K-J	0 / 0	-18.5	-18.5	0.21 (4)	10.00					
J-I	0 / 851	-18.5	-18.5	0.28 (4)	10.00					
I-H	0 / 851	-18.5	-18.5	0.28 (4)	10.00					
H-G	0 / 881	-18.5	-18.5	0.19 (1)	10.00					
G-F	0 / 0	-18.5	-18.5	0.08 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.66")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/380 (0.66")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.78/1.00 (A-B:1), BC=0.28/1.00 (H-J:4), WB=0.42/1.00 (B-J:1), SS=0.30/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

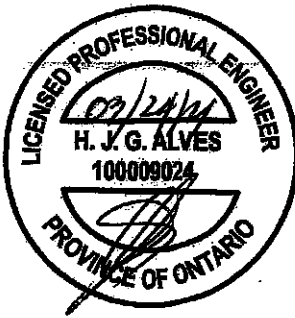
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PLJ) (PLJ)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

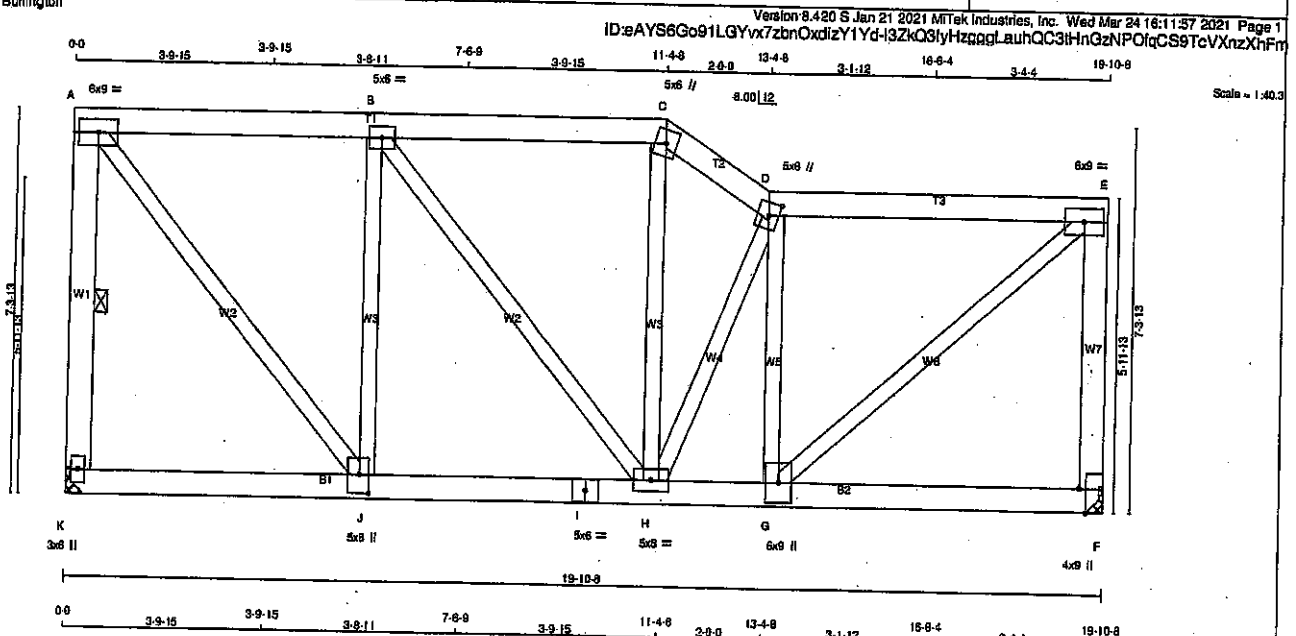
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.81 (E) (INPUT = 0.90)
JSI METAL= 0.34 (I) (INPUT = 1.00)



Structural component only
DWG# T-2108964

JOB NAME 417564	TRUSS NAME T20	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
K - A	2x6 DRY	No.2	SPF
A - C	2x6 DRY	No.2	SPF
C - D	2x6 DRY	No.2	SPF
D - E	2x6 DRY	No.2	SPF
E - F	2x6 DRY	No.2	SPF
K - I	2x6 DRY	No.2	SPF
I - F	2x6 DRY	No.2	SPF
ALL WEBS	2x4 DRY	No.2	SPF
DRY: SEASONED LUMBER			

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
K-A 2 12		TOP
A-C 2 12		TOP
C-D 2 12		TOP
D-E 2 12		TOP
E-F 2 12		TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
K-I 2 5		SIDE (507.8)
I-F 2 5		SIDE (507.8)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1 6		

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
A TMVW-t MT20	6.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
K	8862	0	8862	0	0	MECHANICAL
F	8862	0	8862	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K, F. MINIMUM BEARING LENGTH AT JOINT K = 4-0, JOINT F = 4-0.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
K	8126	4020	0	0	0	2108	0
F	8126	4020	0	0	0	2108	0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
K-A	-6883 / 0	0.0	0.0	0.32 (1)	5.42	A-J	0 / 8687
A-B	-5593 / 0	-91.8	-91.8	0.09 (1)	5.79	J-B	-1438 / 0
B-C	-6295 / 0	-91.8	-91.8	0.09 (1)	5.54	H-C	0 / 3789
C-D	-7468 / 0	-91.8	-91.8	0.04 (1)	5.24	H-D	-3029 / 0
D-E	-7268 / 0	-91.8	-91.8	0.15 (1)	5.18	G-D	-1738 / 0
E-F	-6593 / 0	0.0	0.0	0.81 (1)	6.88	B-H	0 / 1127
						G-E	0 / 9632
K-J	0 / 0	-779.9	-779.9	0.81 (1)	10.00		
J-I	0 / 5593	-779.9	-779.9	0.83 (1)	10.00		
I-H	0 / 5593	-779.9	-779.9	0.83 (1)	10.00		
H-G	0 / 7314	-779.9	-779.9	0.87 (1)	10.00		
G-F	0 / 0	-779.9	-779.9	0.81 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: C510 Girder
START DISTANCE = 0-0
START SPAN CARRIED = 30-1-0
END DISTANCE = 19-10-8
END SPAN CARRIED = 30-1-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/360 (0.66")
CALCULATED VERT. DEF. (LL) = L/999 (0.12")
ALLOWABLE DEF. (TL) = L/360 (0.66")
CALCULATED VERT. DEF. (TL) = L/999 (0.23")

CS1: TC=0.81/1.00 (E-F:1), BC=0.87/1.00 (G-H:1), WB=0.52/1.00 (E-G:1), SS1=0.58/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

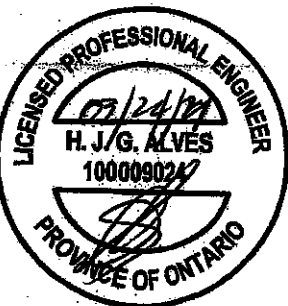
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.85 (G) (INPUT = 0.90)
JSI METAL = 0.73 (G) (INPUT = 1.00)



Structural component only
DWG# T-2108965

JOB NAME 417564	TRUSS NAME T20	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0		
C	TTW+m	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	3.00	2.25
E	TMVW-t	MT20	6.0	9.0		
F	BMV1-t	MT20	4.0	9.0	Edge	1.50
G	BMWW-t	MT20	6.0	9.0		
H	BMWWW-t	MT20	6.0	8.0		
I	BS-t	MT20	5.0	6.0		
J	BMWW-t	MT20	5.0	8.0	4.25	2.25
K	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

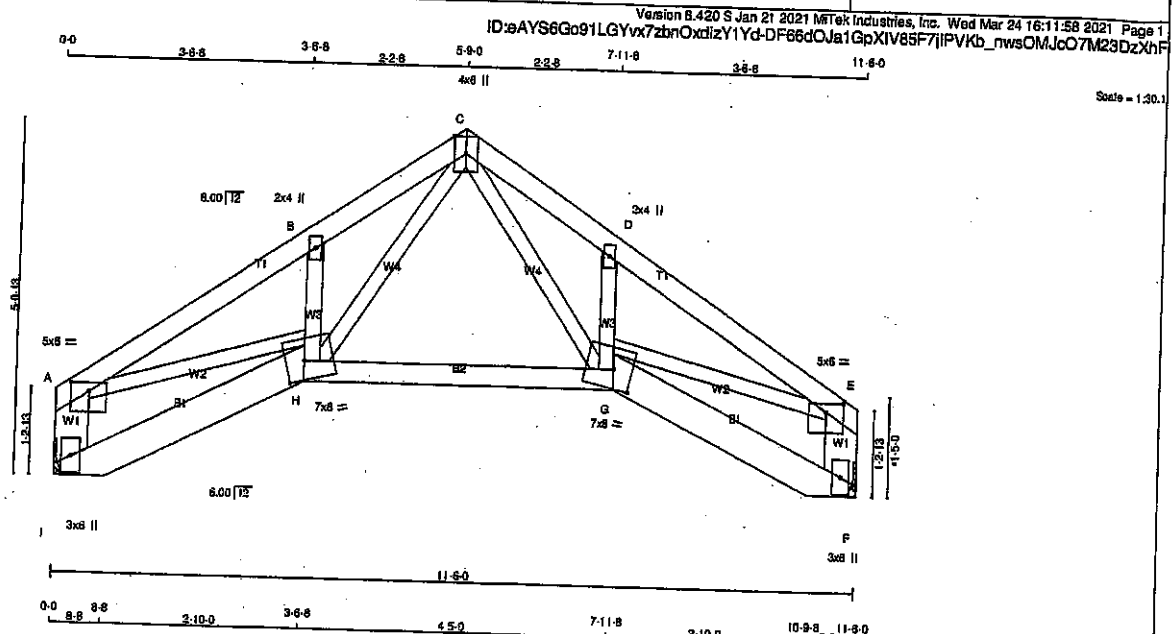
NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108965 *me*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
417564	T21S	3	1	GREENPARK HOMES		
Tamarack Roof Truss, Burlington						



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2	
C - E	2x4	DRY	No.2	
E - G	2x4	DRY	No.2	
G - F	2x4	DRY	No.2	

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (Table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	1.50	3.00	
B	TMVW-w	MT20	2.0	4.0			
C	TMVW-p	MT20	4.0	6.0	Edge		
D	TMVW-w	MT20	2.0	4.0			
E	TMVW-p	MT20	5.0	6.0	1.50	3.00	
F	BMV1-p	MT20	3.0	6.0			
G	BBVWW-m	MT20	7.0	8.0	3.25	3.25	
H	BBVWW-m	MT20	7.0	8.0	3.25	3.25	
I	BMV1-p	MT20	3.0	6.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN
I	834	0	834
F	834	0	834

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I, F. MINIMUM BEARING LENGTH AT JOINT I = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	448	294.0	0.0	0.0	0.0	154.0	0.0
F	448	294.0	0.0	0.0	0.0	154.0	0.0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX	MEMB.	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	FORCE (LBS)	FROM	TO	CSI (LC)	UNBRAC	LENGTH	FR-TO	MEMB.	FORCE (LBS)
A-B	-1071.0	-91.8	-91.8	0.16 (1)	5.93	C-G	0 / 848	0.15 (1)		
B-C	-1107.0	-91.8	-91.8	0.09 (1)	5.93	G-D	-322.0	0.05 (1)		
C-D	-1107.0	-91.8	-91.8	0.09 (1)	5.93	H-C	0 / 848	0.15 (1)		
D-E	-1071.0	-91.8	-91.8	0.16 (1)	5.93	H-B	-322.0	0.05 (1)		
I-A	-801.0	0.0	0.0	0.04 (1)	7.81	A-H	0 / 912	0.21 (1)		
F-E	-801.0	0.0	0.0	0.04 (1)	7.81	G-E	0 / 912	0.21 (1)		
I-H	0 / 0	-18.5	-18.5	0.03 (4)	10.00					
H-G	0 / 543	-18.5	-18.5	0.15 (4)	10.00					
G-F	0 / 0	-18.5	-18.5	0.03 (4)	10.00					

TOTAL WEIGHT = 3 X 53 = 160 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.38")
CALCULATED VERT. DEFL. (LL) = L/999 (0.03")
ALLOWABLE DEFL. (TL) = L/360 (0.38")
CALCULATED VERT. DEFL. (TL) = L/999 (0.07")

CSI: TO=0.16/1.00 (D-E:1), BC=0.15/1.00 (G-H:4), WB=0.21/1.00 (E-G:1), SSI=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

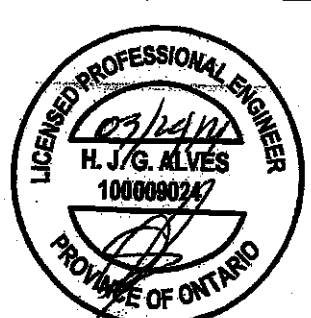
PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	850 371	1747 788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.65 (G) (INPUT = 0.90)

JSI METAL = 0.23 (H) (INPUT = 1.00)

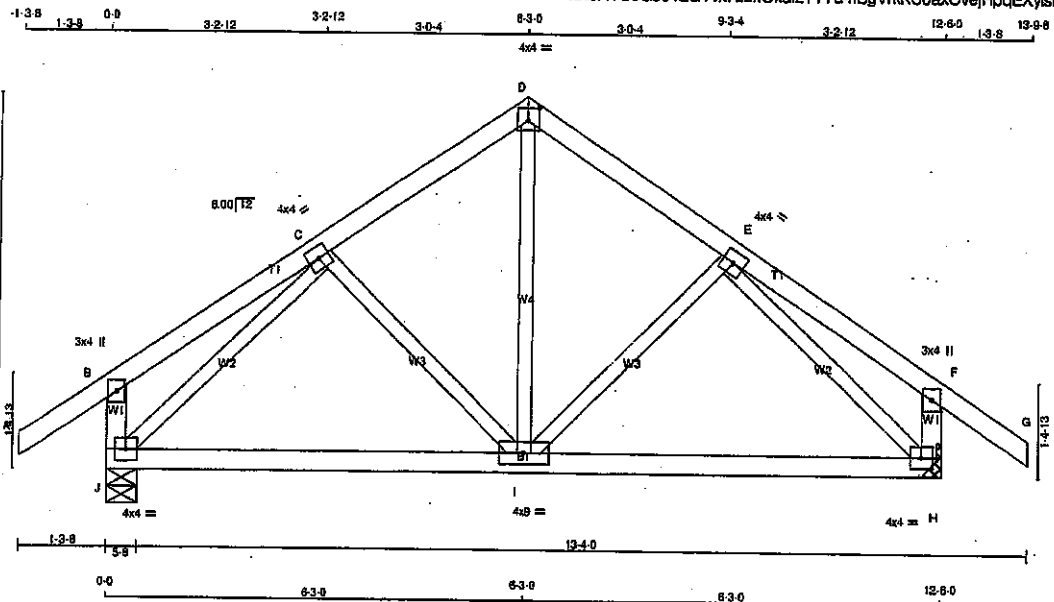


Structural component only
DWG# T-2108966

JOB NAME 417564	TRUSS NAME T22	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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Scale = 1/32.5

TOTAL WEIGHT = 55 lb

LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER			
CHORDS					
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	4.0	
D	TTW-p	MT20	4.0	4.0	2.25 2.00
E	TMWW-t	MT20	4.0	4.0	
F	TMV+p	MT20	3.0	4.0	
H	BMVW1-t	MT20	4.0	4.0	
I	BMVWW-t	MT20	4.0	9.0	
J	BMVW1-t	MT20	4.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		RECRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	815	0	815	0	0	5-8	5-8
H	815	0	815	0	0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	574	390.0	0.0	0.0	0.0	184.0	0.0
H	574	390.0	0.0	0.0	0.0	184.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0.35	-91.8	-91.8 0.12 (1)	10.00	I-D	0.344	0.08 (1)
B-C	0.18	-91.8	-91.8 0.14 (1)	10.00	I-E	-142.0	0.05 (1)
C-D	-530.0	-91.8	-91.8 0.11 (1)	6.25	C-I	-142.0	0.05 (1)
D-E	-530.0	-91.8	-91.8 0.11 (1)	6.25	J-O	-749.0	0.24 (1)
E-F	0.18	-91.8	-91.8 0.14 (1)	10.00	E-H	-749.0	0.24 (1)
F-G	0.35	-91.8	-91.8 0.12 (1)	10.00			
J-B	-237.0	0.0	0.0 0.02 (1)	7.81			
H-F	-237.0	0.0	0.0 0.02 (1)	7.81			
J-I	0.526	-18.5	-18.5 0.25 (4)	10.00			
I-H	0.526	-18.5	-18.5 0.25 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.04")

CSI: TC=0.14/1.00 (E-F:1), BC=0.25/1.00 (I-J:4), WB=0.24/1.00 (E-H:1), SS=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

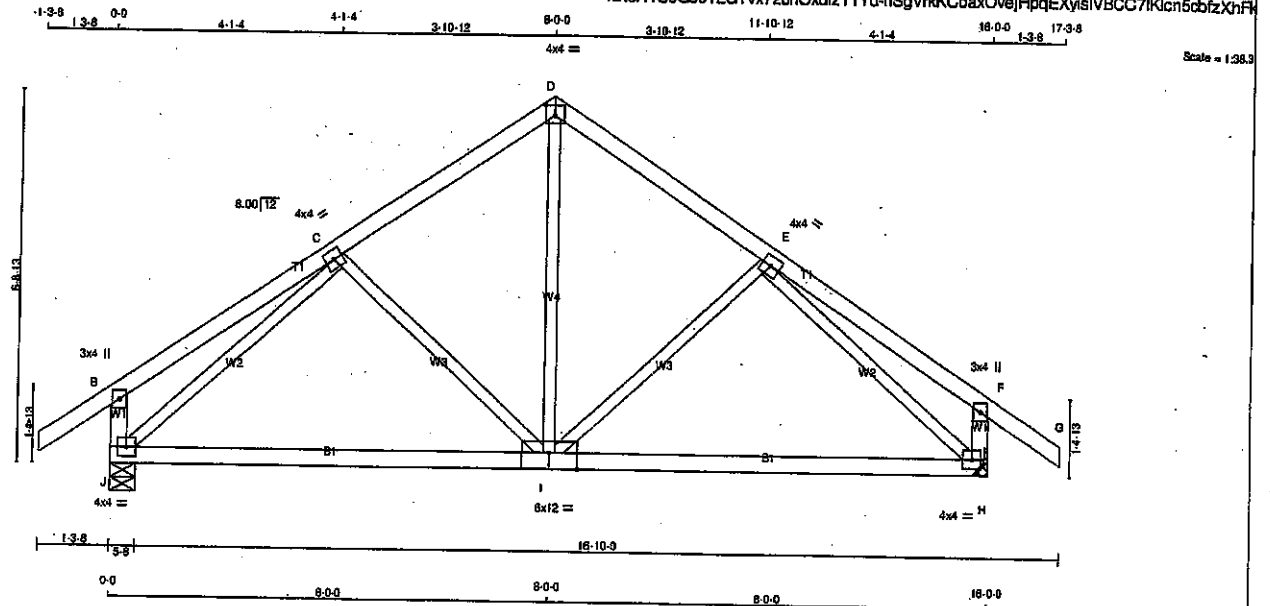
JSI GRIP = 0.63 (C) (INPUT = 0.90)
JSI METAL = 0.24 (E) (INPUT = 1.00)



Structural component only.
DWG# T-2108967

JOB NAME 417564	TRUSS NAME T23	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 2 X 68 = 136 lb
(M/F)

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW+1	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMV+1	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW+1	MT20	4.0	4.0		
H	BSWW+1	MT20	6.0	12.0	Edge	6.00
I	BMVW+1	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
J	1008	0	1008	0	0
H	1008	0	1008	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3'-8".

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	710	480 / 0	0 / 0	0 / 0	0 / 0	231 / 0	0 / 0
H	710	480 / 0	0 / 0	0 / 0	0 / 0	231 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 35	I-D	0 / 450
B-C	0 / 24	I-E	-222 / 0
C-D	-685 / 0	C-I	-222 / 0
D-E	-685 / 0	J-C	-878 / 0
E-F	0 / 24	E-H	-976 / 0
F-G	0 / 35		
J-B	-287 / 0		
H-F	-287 / 0		
J-I	0 / 712		
I-H	0 / 712		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCG 2015, ABC 2019
- PART 9 OF NBCG 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/946 (0.20")

CSI: TC=0.23/1.00 (E-F:1), BC=0.40/1.00 (H-I:4), WB=0.48/1.00 (E-H:1), SS=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	650	371	1747

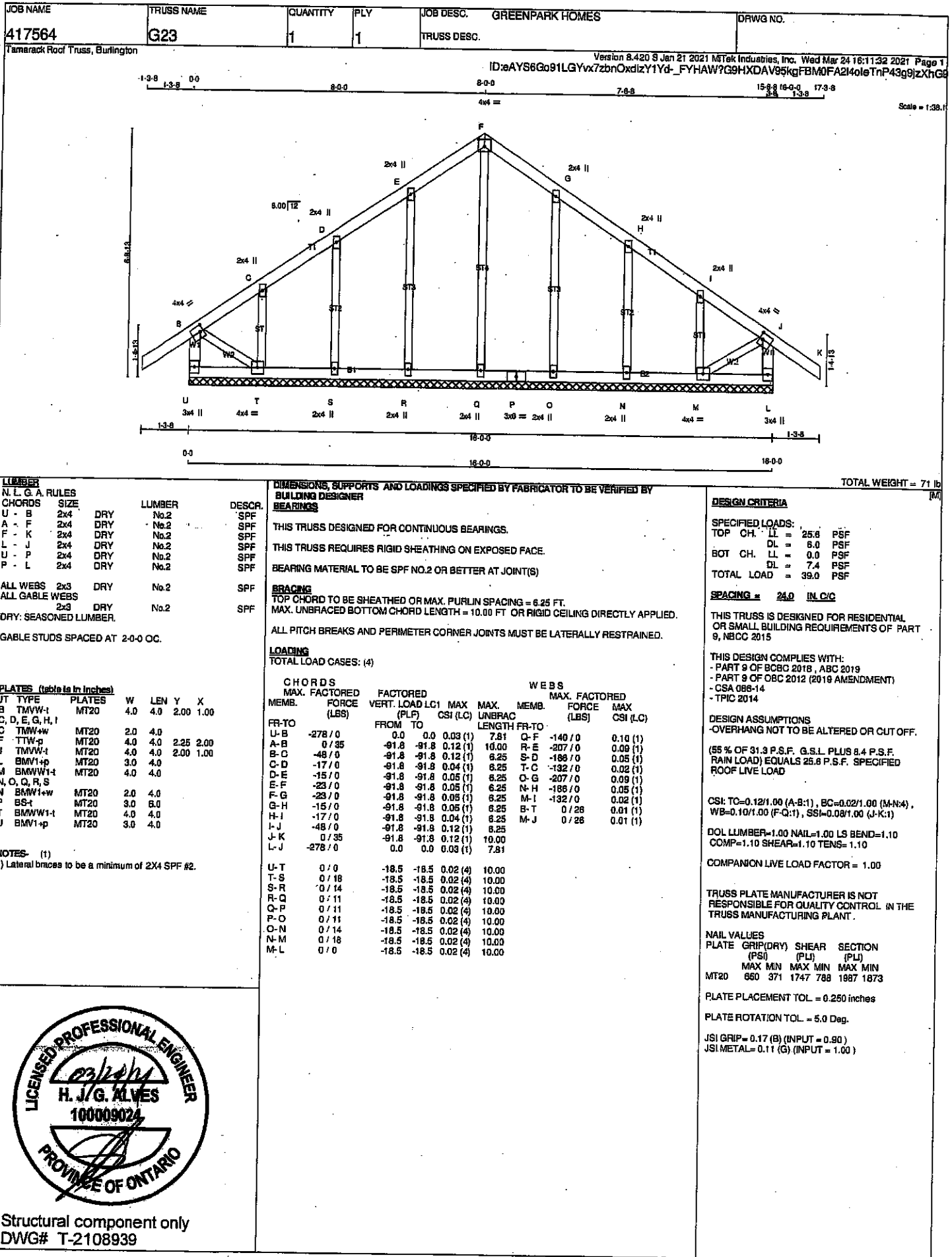
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

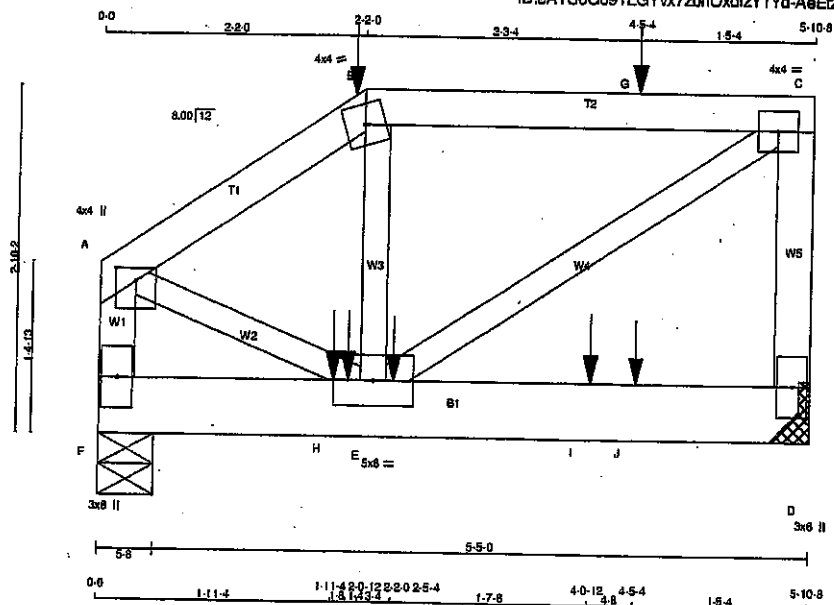
JSI GRIP = 0.78 (J) (INPUT = 0.90)
JSI METAL = 0.31 (E) (INPUT = 1.00)



Structural component only
DWG# T-2108968



JOB NAME 417564	TRUSS NAME T24	QUANTITY 2	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR.
A - B	2x4	DRY	No.2		SPF
B - C	2x4	DRY	No.2		SPF
D - C	2x4	DRY	No.2		SPF
F - A	2x4	DRY	No.2		SPF
F - D	2x6	DRY	No.2		SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-B 1 12	SIDE(81.0)	
B-C 1 12	SIDE(81.0)	
C-D 1 12	TOP	
F-A 1 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2 12	SIDE(183.1)	
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 8		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A T&WV+p	MT20	4.0	4.0	1.25	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
D	1025	0	1025	0
F	1000	0	1000	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4'-0".

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
D	724 483 / 0 0 / 0 0 / 0 0 241 / 0 0 / 0
F	706 469 / 0 0 / 0 0 / 0 0 237 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CS1 (LC)	UNBRACED LENGTH	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO				FR-TO		
A-B	-970 / 0	-91.8	-91.8	0.04 (1)	6.25	E-B	0 / 248	0.03 (1)
B-G	-815 / 0	-91.8	-91.8	0.13 (1)	6.25	E-C	0 / 988	0.12 (1)
G-C	-815 / 0	-91.8	-91.8	0.13 (1)	6.25	A-E	0 / 878	0.11 (1)
D-C	-730 / 0	0.0	0.0	0.05 (1)	7.81			
F-A	-994 / 0	0.0	0.0	0.06 (1)	7.81			
F-H	0 / 0	-18.5	-18.5	0.10 (1)	10.00			
H-E	0 / 0	-18.5	-18.5	0.10 (1)	10.00			
E-I	0 / 0	-18.5	-18.5	0.19 (1)	10.00			
I-J	0 / 0	-18.5	-18.5	0.19 (1)	10.00			
J-D	0 / 0	-18.5	-18.5	0.19 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	2-2-0	-13	-13	124	BACK	VERT	TOTAL		C1
E	2-5-4	-1	-1		BACK	VERT	TOTAL		C1
G	4-5-4	-5	-5		BACK	VERT	TOTAL		C1
H	1-11-4	-6	-6		BACK	VERT	TOTAL		C1
I	2-0-12	-473	-473		FRONT	VERT	TOTAL		C1
J	4-0-12	-473	-473		FRONT	VERT	TOTAL		C1
J	4-5-4	-1	-1		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 28 = 111 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2018
- PART 8 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CS1: TC=0.13/1.00 (B-C:1), BC=0.19/1.00 (D-E:1), WB=0.12/1.00 (C-E:1), SS=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 690 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.48 (C) (INPUT = 0.90)
JSI METAL = 0.14 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108969

CONTINUED ON PAGE 2

JOB NAME 417564	TRUSS NAME T24	QUANTITY 2	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 5 Jan 21 MITek Industries, Inc. Wed Mar 24 18:12:00 2021 Page 2 ID: 8AYS8Go91LGYvx7zbnOxdizY1Yd-AgE24KgZu3FXolITNYImUwPyxhclsl5vRr975zXhF	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TTW-m	MT20	4.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
D	BMV1+p	MT20	3.0	6.0		
E	BMVWW-t	MT20	5.0	8.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

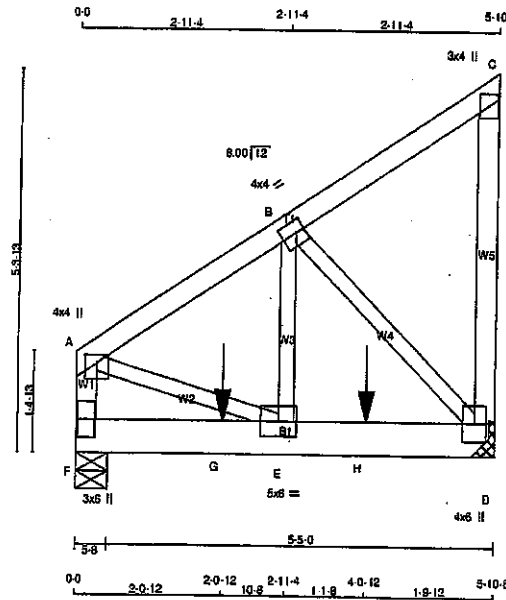


Structural component only
DWG# T-2108969 *MC*

JOB NAME 417564	TRUSS NAME T25	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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Scale = 1/2" = 1'-0"

TOTAL WEIGHT = 2 X 32 = 64 lb

CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
D - C	2x4 DRY	No.2	SPF
F - D	2x6 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 1	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	PLATES	W	LEN	Y	X
A TMVW+p	MT20	4.0	4.0	1.25	2.00
B TMVW-i	MT20	4.0	4.0	2.00	1.50
C TMV+p	MT20	3.0	4.0		
D BMVW1+p	MT20	4.0	6.0		
E BMVW-i	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
F	1271	0	1271	0
D	1356	0	1356	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	895	809'0	0'0	0'0	0'0	286'0	0'0
D	884	850'0	0'0	0'0	0'0	304'0	0'0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO								
F-A	-1085'0	0.0	0.0	0.08 (1)	7.81	A-E	0'1027	0.13 (1)
A-B	-1153'0	-91.8	-91.8	0.07 (1)	6.25	E-B	0'1302	0.16 (1)
B-C	-15'0	-91.8	-91.8	0.08 (1)	6.25	B-D	-1379'0	0.19 (1)
D-C	-107'0	0.0	0.0	0.02 (1)	7.81			
F-G	0'0	-18.5	-18.5	0.15 (1)	10.00			
G-E	0'0	-18.5	-18.5	0.15 (1)	10.00			
E-H	0'972	-18.5	-18.5	0.23 (1)	10.00			
H-D	0'972	-18.5	-18.5	0.23 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-696	-696		BACK	VERT	TOTAL		C1
H	4-0-12	-696	-696		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP GH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.071/0.0 (A-B:1), BC=0.231/0.0 (D-E:1), WB=0.19/1.00 (B-D:1), SS=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1887 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.22 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108970

CONTINUED ON PAGE 2

JOB NAME 417564	TRUSS NAME T25	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MTek Industries, Inc. Wed Mar 24 16:12:01 2021 Page 2 ID: aAYS8Go91LGYvx7ZbnOxdizY1Yd-egoFGQLTKBB59vfwFH717x7d xPbID245ajqYzXhF	

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
F	BMV1+p	MT20	3.0	6.0	

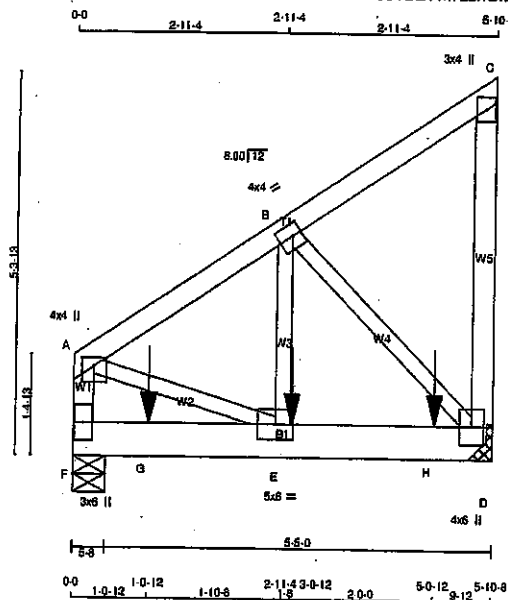
NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108970 *22*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417564	T25Z	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Mar 24 16:12:02 2021 Page 1
ID:AYS8G091LGYvx7zbnOxdizY1Yd-60MdTmM55VJyn6SsUzoEaLUIMOHpKBxBIKGC_zXhRf



Scale = 1/25.4

TOTAL WEIGHT = 2 X 32 = 64 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'X3') SPIRAL NAILS		
F - A	12	TOP
A - C	12	TOP
C - D	12	TOP
BOTTOM CHORDS : (0.122'X3') SPIRAL NAILS		
F - D	12	SIDE (183.1)
WEBS : (0.122'X3') SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25 2.00
B	TMVW-t	MT20	4.0	4.0	2.00 1.50
C	TMVW+p	MT20	3.0	4.0	
D	BMVW1+p	MT20	4.0	6.0	
E	BMVW-t	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT REGRD BRG	REGRD IN-SX
JT	VERT	HORZ	DOWN	HORZ
F	1319	0	1319	0
D	1468	0	1468	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	930	626 / 0	0 / 0	0 / 0	0 / 0	304 / 0	0 / 0
D	1034	704 / 0	0 / 0	0 / 0	0 / 0	330 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. CS1 (LC)	UNBRAC LENGTH	FR-TO	WEBS MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
F-A	-930 / 0	0.0	0.0	0.05 (1)	7.81	A-E	0 / 884	0.11 (1)
A-B	-968 / 0	-91.8	-91.8	0.07 (1)	6.25	E-B	0 / 1043	0.13 (1)
B-C	-18 / 0	-91.8	-91.8	0.08 (1)	6.25	B-D	-1161 / 0	0.16 (1)
D-C	-106 / 0	0.0	0.0	0.02 (1)	7.81			
F-G	0 / 0	-18.5	-18.5	0.15 (1)	10.00			
G-E	0 / 0	-18.5	-18.5	0.16 (1)	10.00			
E-H	0 / 819	-18.5	-18.5	0.21 (1)	10.00			
H-D	0 / 819	-18.5	-18.5	0.21 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-0-12	-473	-473	---	FRONT	VERT	TOTAL	---	C1
G	1-0-12	-473	-473	---	FRONT	VERT	TOTAL	---	C1
H	5-0-12	-561	-561	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.01")

CSI: TO=0.07/1.00 (A-B:1), BC=0.21/1.00 (D-E:1), WB=0.16/1.00 (B-D:1), SS=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	971	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.62 (B) (INPUT = 0.90)
JSI METAL=0.18 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2108971 1/1

JOB NAME 417564	TRUSS NAME T25Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MITak Industries, Inc. Wed Mar 24 16:12:02 2021 Page 2 ID:AYS6Go91LGYvx7zbnOxdizY1Yd-80MdTmM55VJvn6SsUzoEaLUIMOHPKbXBIKGC zXhPF		

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
F	BMV1+p	MT20	3.0	6.0	

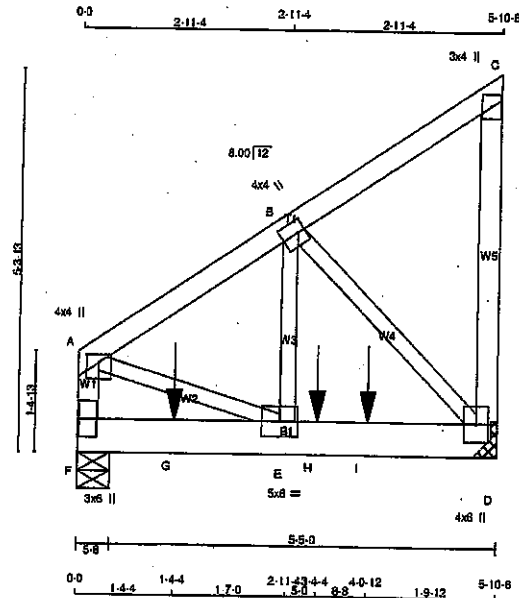
NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108971 *zh*

JOB NAME 417564	TRUSS NAME T25Z1	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Mar 24 16:12:03 2021 Page 1
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Scale = 1/2" = 1'-0"

TOTAL WEIGHT = 2 X 32 = 64 lb

CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY No.2	SPF
A - C	2x4	DRY No.2	SPF
D - C	2x4	DRY No.2	SPF
F - D	2x6	DRY No.2	SPF
ALL WEBS 2x8 DRY No.2 SPF			
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 1	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW+p	MT20	4.0	4.0	1.25	2.00
B TMVW+H	MT20	4.0	4.0	2.00	1.25
C TMV+p	MT20	3.0	4.0		
D BMVW+p	MT20	4.0	6.0		
E BMVW+H	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	DOWN	UP/LIFT	IN-SX
F 1833	0	1833	5-8
D 1752	0	1752	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST CASE	MAX/MIN COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT COMBINED							
F 1282	873 / 0	0 / 0	0 / 0	0 / 0	419 / 0	0 / 0	0 / 0
D 1235	828 / 0	0 / 0	0 / 0	0 / 0	407 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	CS1 (LC)	UNBRACED LENGTH	FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
F-A	-1407 / 0	0.0	0.0	0.08 (1)	7.81	A-E	0 / 1362	0.17 (1)				
A-B	-1536 / 0	-91.8	-91.8	0.08 (1)	6.25	E-B	0 / 1837	0.23 (1)				
B-C	-14 / 0	-91.8	-91.8	0.06 (1)	6.25	B-D	-1829 / 0	0.26 (1)				
D-C	-109 / 0	0.0	0.0	0.02 (1)	7.81							
F-G	0 / 0	-18.5	-18.5	0.21 (1)	10.00							
G-E	0 / 0	-18.5	-18.5	0.21 (1)	10.00							
E-H	0 / 1289	-18.5	-18.5	0.30 (1)	10.00							
H-I	0 / 1289	-18.5	-18.5	0.30 (1)	10.00							
I-D	0 / 1289	-18.5	-18.5	0.30 (1)	10.00							

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-4-4	-748	-748	---	BACK	VERT	TOTAL	---	C1
H	3-4-4	-661	-661	---	BACK	VERT	TOTAL	---	C1
I	4-0-12	-661	-661	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TC=0.08/1.00 (A-F:1), BC=0.30/1.00 (D-E:1), WB=0.26/1.00 (B-D:1), SS=0.53/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	PSI	PLI	SECTION
		MAX	MIN	MAX MIN
MT20	850	371	1747	788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.86 (B) (INPUT = 0.80)

JSI METAL = 0.30 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108972 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417564	T25Z1	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
F BMV1+p MT20 3.0 6.0

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

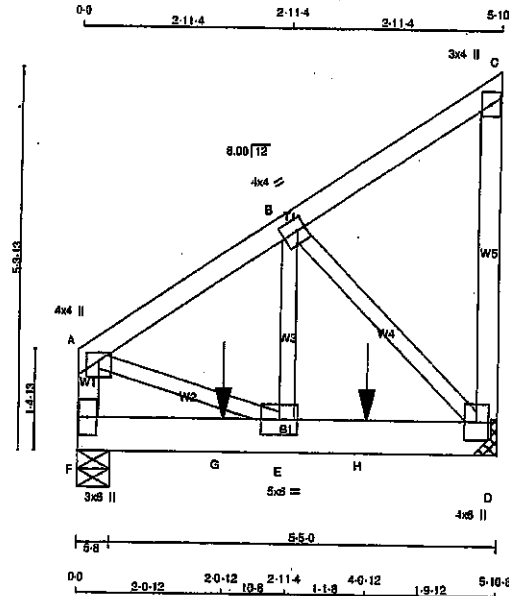


Structural component only
DWG# T-2108972 *ML*

JOB NAME 417564	TRUSS NAME T25Z2	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRAWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/2\"/>

TOTAL WEIGHT = 2 X 32 = 64 lb

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122\"X3\") SPIRAL NAILS		
F - A	12	TOP
A - C	12	TOP
C - D	12	TOP
BOTTOM CHORDS : (0.122\"X3\") SPIRAL NAILS		
F - D	12	SIDE (0.0)
WEBS : (0.122\"X3\") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW+p	MT20	4.0	6.0		
E	BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	913	0	913	0	5-8	5-8
D	966	0	966	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	644	432'0	0'0	0'0	0'0	212'0	0'0
D	681	457'0	0'0	0'0	0'0	224'0	0'0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4).

MEMB.	CHORDS		FACTORED		MAX. UNBRAC LENGTH	MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)			MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO				FR-TO		
F-A	-787'0	0.0	0.0	0.04 (1)	7.81	A-E	0'716	0.09 (1)
A-B	-798'0	-91.8	-91.8	0.07 (1)	6.25	E-B	0'805	0.10 (1)
B-C	-18'0	-91.8	-91.8	0.05 (1)	8.25	B-D	-961'0	0.14 (1)
D-C	-105'0	0.0	0.0	0.02 (1)	7.81			
F-G	0'0	-18.5	-18.5	0.09 (1)	10.00			
G-E	0'0	-18.5	-18.5	0.09 (1)	10.00			
E-H	0'678	-18.5	-18.5	0.15 (1)	10.00			
H-D	0'678	-18.5	-18.5	0.15 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-434	-434	---	BACK	VERT	TOTAL	---	C1
H	4-0-12	-434	-434	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BOSC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.07/1.00 (A-B:1), BC=0.15/1.00 (D-E:1), WB=0.14/1.00 (B-D:1), SS=0.16/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	850	371	1747

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.48 (B) (INPUT = 0.90)

JSI METAL = 0.15 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108973

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417564	T25Z2	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Mar 24 18:12:04 2021 Page 2
ID:eAYS6G091LGYvx7zbnOxdizY1Yd-2PUOuSNld6Zg0QcEcOqfmZerCzD05uUm3pNGtzXhF

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
F BMV1+p MT20 3.0 6.0

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

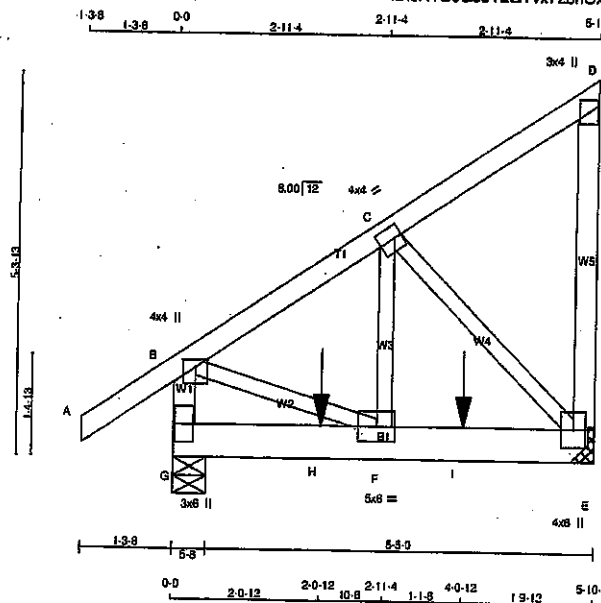


Structural component only
DWG# T-2108973 *mn*

JOB NAME 417565	TRUSS NAME T25Z3	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 6.420 S Jan 21 2021 MTek Industries, Inc. Wed Mar 24 16:39:10 2021 Page 1
ID:AYS6G091LGYvx7zbnOxdizY1Yd-RDK49R3HlQz5ivst??YfWOfuqmT5p1hPLdzXgsF



Scale = 1/2" = 1'-0"

TOTAL WEIGHT = 2 X 34 = 68 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESOR.
G - B	2x4 DRY No.2	SPF			
A - D	2x4 DRY No.2	SPF			
E - D	2x4 DRY No.2	SPF			
G - E	2x6 DRY No.2	SPF			

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		TOP
G-B 1 12		TOP
A-D 1 12		TOP
D-E 1 12		TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		SIDE(0.0)
G-E 2 12		SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.25	2.00	
C TMVW-t	MT20	4.0	4.0	2.00	1.25	
D TMV+p	MT20	3.0	4.0			
E BMVW1+p	MT20	4.0	4.0			
F BMVW-t	MT20	5.0	6.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	UP/LIFT	IN-SX
G	1551	0	1551	0
E	1523	0	1523	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 4-0.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1091	751 / 0	0 / 0	0 / 0	0 / 0	339 / 0	0 / 0
E	1073	728 / 0	0 / 0	0 / 0	0 / 0	345 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CS1 (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB. MAX. FORCE (LBS)	MAX. CS1 (LC)
FR-TO					FR-TO		
G-B	-1339 / 0	0.0	0.0	0.07 (1)	7.81	B-F	0 / 1160
A-B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	F-C	0 / 1515
B-C	-1308 / 0	-91.8	-91.8	0.07 (1)	6.25	C-E	-1558 / 0
C-D	-15 / 0	-91.8	-91.8	0.06 (1)	6.25		
E-D	-108 / 0	0.0	0.0	0.02 (1)	7.81		
G-H	0 / 0	-18.5	-18.5	0.17 (1)	10.00		
H-F	0 / 0	-18.5	-18.5	0.17 (1)	10.00		
F-I	0 / 1099	-18.5	-18.5	0.27 (1)	10.00		
I-E	0 / 1099	-18.5	-18.5	0.27 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	2-0-12	-809	-809	---	FRONT	VERT	TOTAL	---	C1
I	4-0-12	-809	-809	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.20")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.01")
ALLOWABLE DEFL.(TL) = 1/360 (0.20")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.02")

CSI: TC=0.07/1.00 (B-G:1), BC=0.27/1.00 (E-F:1), WB=0.22/1.00 (C-E:1), SI=0.33/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 789 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.75 (C) (INPUT = 0.90)
JSI METAL = 0.25 (E) (INPUT = 1.00)



Structural component only
DWG# T-2108984 1/2

CONTINUED ON PAGE 2

JOB NAME 417565	TRUSS NAME T25Z3	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Wed Mar 24 16:39:10 2021 Page 2
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PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
G	BMV1+p	MT20	3.0	6.0	

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



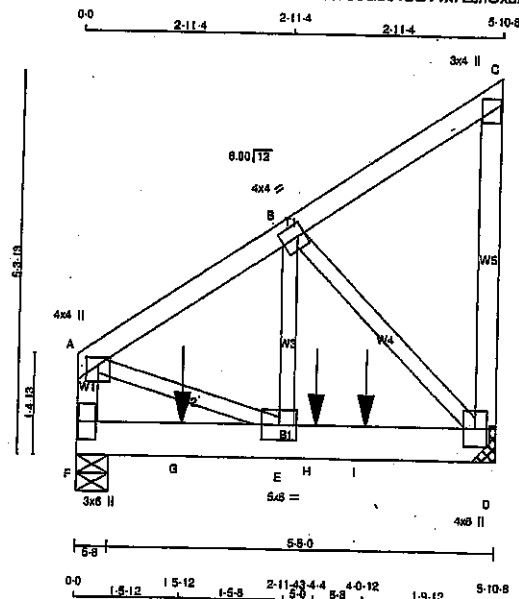
Structural component only
DWG# T-2108984

JOB NAME 417565	TRUSS NAME T25Z4	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:AYS6Go91LGYvx7zbnOxdizY1Yd-vQHSMn4vT0YqISUXQaWE5a5g52ERVwozuhRyu3zXgsE



Scale = 1/2" = 1'-0"

TOTAL WEIGHT = 2 X 32 = 64 lb

LUMBER

N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
F - A	2x4	DRY	No.2		SPF
A - C	2x4	DRY	No.2		SPF
D - C	2x4	DRY	No.2		SPF
F - D	2x6	DRY	No.2		SPF
ALL WEBS	2x3	DRY	No.2		SPF

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A	12	TOP
A - C	12	TOP
C - D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMVW+1	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW+1	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	UP/LIFT	IN-SX
F	1716	0	0	5-8
D	1743	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST LOASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM LIVE WIND DEAD SOIL
F	1211 809 / 0 0 / 0 0 / 0 402 / 0 0 / 0
D	1230 822 / 0 0 / 0 0 / 0 408 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. VERT. LOAD (LC2)	MAX. VERT. LOAD (LC3)	MAX. VERT. LOAD (LC4)	MAX. VERT. LOAD (LC5)	MAX. VERT. LOAD (LC6)	MAX. VERT. LOAD (LC7)	MAX. VERT. LOAD (LC8)	MAX. VERT. LOAD (LC9)	MAX. VERT. LOAD (LC10)
FR-TO												
F-A	-1390 / 0	0.0	0.0	0.08 (1)	7.81	A-E	0 / 1344	0.17 (1)				
A-B	-1518 / 0	-91.8	-91.8	0.06 (1)	6.25	E-B	0 / 1808	0.22 (1)				
B-C	-14 / 0	-91.8	-91.8	0.06 (1)	6.25	B-D	-1808 / 0	0.26 (1)				
D-C	-109 / 0	0.0	0.0	0.02 (1)	7.81							
F-G	0 / 0	-18.5	-18.5	0.18 (1)	10.00							
G-E	0 / 0	-18.5	-18.5	0.18 (1)	10.00							
E-H	0 / 1273	-18.5	-18.5	0.31 (1)	10.00							
H-I	0 / 1273	-18.5	-18.5	0.31 (1)	10.00							
I-D	0 / 1273	-18.5	-18.5	0.31 (1)	10.00							

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
G	1-5-12	-661	-661	---	BACK	VERT	TOTAL	C1
H	3-4-4	-661	-661	---	BACK	VERT	TOTAL	C1
I	4-0-12	-661	-661	---	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, ABC 2015
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TC=0.08/1.00 (A-F:1), BC=0.31/1.00 (D-E:1), WB=0.26/1.00 (B-D:1), SS=0.52/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

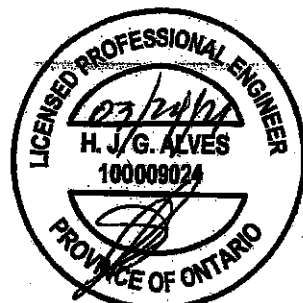
NAIL VALUES

PLATE GRIP (DRY) SHEAR	SECTION
(PSI)	(PLI)
MAX MIN MAX MIN MAX MIN	
MT20 650 371 1747 788 1987 1873	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.29 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108985

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417565	T25Z4	1	2	TRUSS DESC.		

Tamrack Roof Truss, Burlington

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PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
F BMV1+p MT20 3.0 6.0

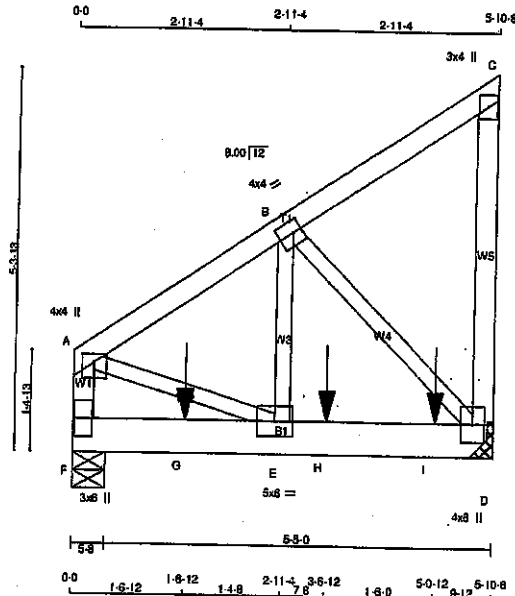
NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108985 *in*

JOB NAME 417565	TRUSS NAME T25Z5	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:AYS6Go91LGYvx7zbnOxdizY1Yd-NcrqZ74XEJghKc37z1Te4?r0SbuEOF67LAWQVzXgsD



CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY No.2	SPF
A - C	2x4	DRY No.2	SPF
D - C	2x4	DRY No.2	SPF
F - D	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 1	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW+p	MT20	4.0	4.0	1.25	2.00
B TMVW-t	MT20	4.0	4.0	2.00	1.50
C TMV+p	MT20	3.0	4.0		
D BMVW1+p	MT20	4.0	6.0		
E BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BEG	REQD SRG
JT VERT	877	0	0
F 1243	0	1243	0
D 1584	0	1584	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	
F 877	588 / 0
D 1117	749 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
F-A	-1004 / 0	0.0	0.0	0.08 (1)	7.81	A-E	0 / 942
A-B	-1057 / 0	-91.8	-91.8	0.07 (1)	6.25	E-B	0 / 1167
B-C	-15 / 0	-91.8	-91.8	0.08 (1)	6.25	B-D	-1265 / 0
D-C	-107 / 0	0.0	0.0	0.02 (1)	7.81		
F-G	0 / 0	-18.5	-18.5	0.14 (1)	10.00		
G-E	0 / 0	-18.5	-18.5	0.14 (1)	10.00		
E-H	0 / 892	-18.5	-18.5	0.23 (1)	10.00		
H-I	0 / 892	-18.5	-18.5	0.23 (1)	10.00		
I-D	0 / 892	-18.5	-18.5	0.23 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
G	1-6-12	-511	-511		BACK	VERT	TOTAL		C1
H	3-6-12	-511	-511		BACK	VERT	TOTAL		C1
I	5-0-12	-513	-513		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 32 = 64 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 8.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.01")

CSI: TC=0.07/1.00 (A-B:1), BC=0.23/1.00 (D-E:1), WB=0.18/1.00 (B-D:1), SS=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	850	371	1747
		788	1987
		1873	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

ISI GRIP = 0.89 (B) (INPUT = 0.90)

ISI METAL = 0.20 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108986

CONTINUED ON PAGE 2

JOB NAME 417565	TRUSS NAME T25Z5	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 8 Jan 21 2021 MITek Industries, Inc. Wed Mar 24 18:39:12 2021 Page 2
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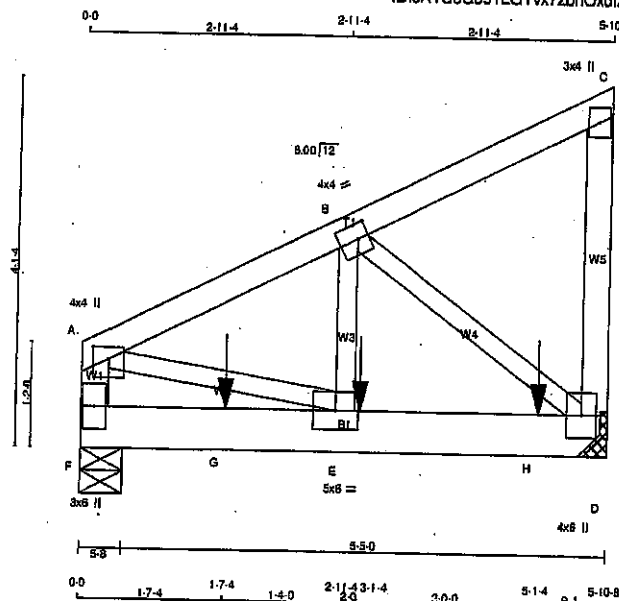
PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
F BMV1+p MT20 3.0 6.0

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108986 *in*

JOB NAME 417566	TRUSS NAME T25Z6	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Mar 24 17:18:19 2021 Page 1	



CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY No.2	SPF
A - C	2x4	DRY No.2	SPF
D - C	2x4	DRY No.2	SPF
F - D	2x6	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 1	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE (183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
B-E 1	6	SIDE (81.4)
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
A TMW+p	MT20	4.0	4.0	1.50 2.00
B TMW-l	MT20	4.0	4.0	2.00 1.50
C TMW+p	MT20	3.0	4.0	
D BMW1+p	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
F	1805	0	0	5-8
D	1916	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
F	1132 756 / 0 0 / 0 0 / 0 0 376 0 0 / 0
D	1352 902 / 0 0 / 0 0 / 0 0 450 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (UNBRACED LENGTH)	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
F-A	-1236 / 0	0.0	0.0	0.07 (1)	7.81	A-E	0 / 1522	0.19 (1)
A-B	-1635 / 0	-91.8	-91.8	0.06 (1)	6.25	E-B	0 / 1546	0.19 (1)
B-C	-10 / 0	-91.8	-91.8	0.05 (1)	10.00	B-D	-1851 / 0	0.22 (1)
D-C	-113 / 0	0.0	0.0	0.01 (1)	7.81			
F-G	0 / 0	-18.5	-18.5	0.21 (1)	10.00			
G-E	0 / 0	-18.5	-18.5	0.21 (1)	10.00			
E-H	0 / 1471	-18.5	-18.5	0.29 (1)	10.00			
H-D	0 / 1471	-18.5	-18.5	0.29 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-1-4	-661	-661	---	BACK	VERT	TOTAL	---	C1
G	1-7-4	-661	-661	---	BACK	VERT	TOTAL	---	C1
H	5-1-4	-705	-705	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:	TOP CH.	LL	PSF
	DL	=	6.0
	BOT CH.	LL	= 0.0
	DL	=	7.4
	TOTAL LOAD	=	39.0

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/880 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TC=0.07/1.00 (A-F:1), BC=0.29/1.00 (D-E:1), WB=0.22/1.00 (B-D:1), SB=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

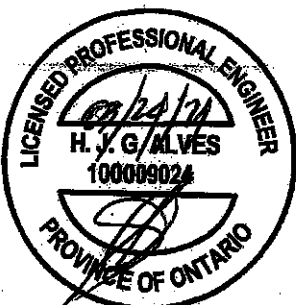
PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MT20	850	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (A) (INPUT = 0.90)

JSI METAL = 0.33 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108999 1/2

JOB NAME 417566	TRUSS NAME T25Z6	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRAWG NO.
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Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	BMWW-t	MT20	5.0	6.0		
F	BMV1-p	MT20	3.0	6.0		

NOTES: (1)
 1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
 DWG# T-2108999 *42*

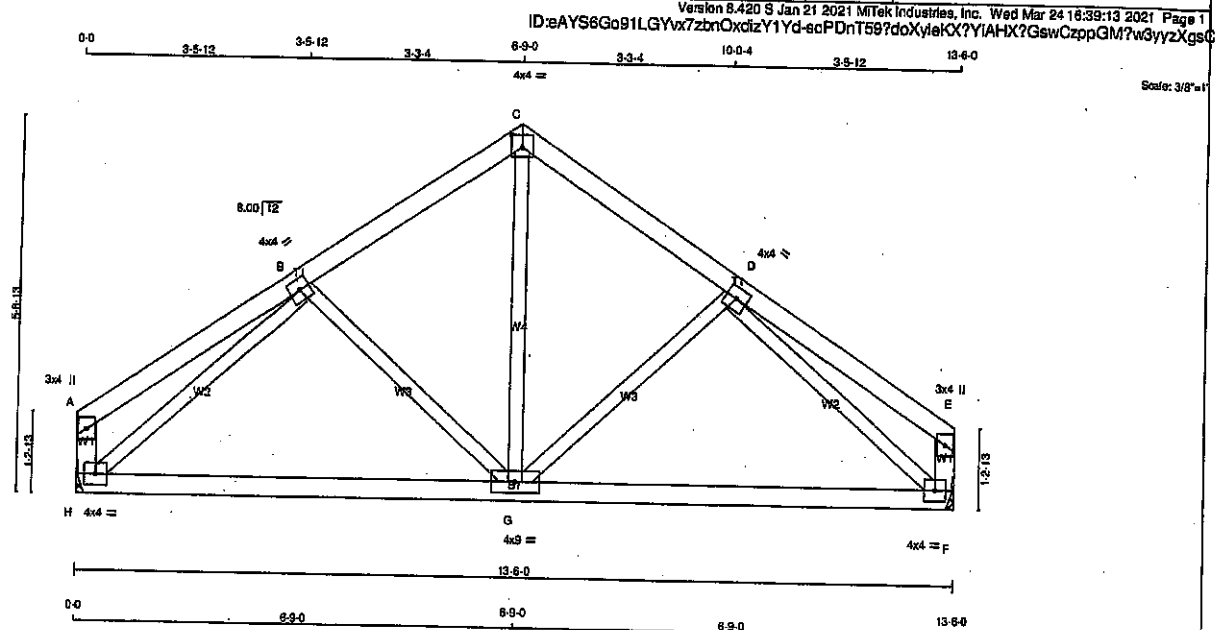
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C7: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



JOB NAME 417565	TRUSS NAME T31	QUANTITY 3	PLY. 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



TOTAL WEIGHT = 3 X 54 = 163 lb
(MIF)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - A	2x4	DRY	No.2
F - E	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMVW-t	MT20	4.0	4.0		
C	TMV+p	MT20	4.0	4.0	2.25	2.00
D	TMVW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW-t	MT20	4.0	4.0		
G	BMVW-t	MT20	4.0	9.0		
H	BMVW-t	MT20	4.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	RECORD
JT	VERT	DOWN	BRG	BRG
H	744	0	0	IN-SX
F	744	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, F. MINIMUM BEARING LENGTH AT JOINT H = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	526	345 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
F	526	345 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO					FR-TO		
A-B	0 / 19	-91.8	-91.8	0.16 (1)	G-C	0 / 401	0.09 (1)
B-C	-598 / 0	-91.8	-91.8	0.13 (1)	G-D	-181 / 0	0.06 (1)
C-D	-598 / 0	-91.8	-91.8	0.13 (1)	G-E	-181 / 0	0.06 (1)
D-E	0 / 19	-91.8	-91.8	0.16 (1)	H-B	-842 / 0	0.29 (1)
H-A	-120 / 0	0.0	0.0	0.01 (1)	D-F	-842 / 0	0.29 (1)
F-E	-120 / 0	0.0	0.0	0.01 (1)			
H-G	0 / 614	-18.5	-18.5	0.29 (4)			
G-F	0 / 614	-18.5	-18.5	0.29 (4)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 098-14
- TPC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.16/1.00 (D-E:1), BC=0.29/1.00 (F-G:4),
WB=0.29/1.00 (D-F:1), SS=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX	MIN	MAX
MIN	MAX	MIN
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

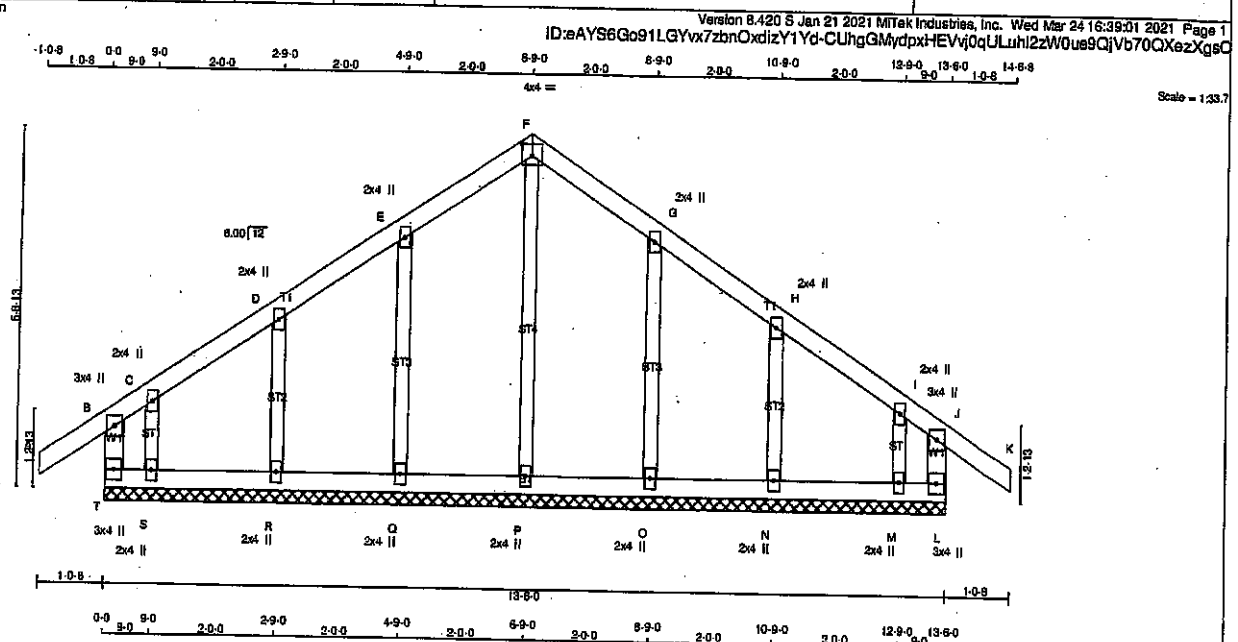
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.27 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108988

JOB NAME 417565	TRUSS NAME G31	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



TOTAL WEIGHT = 55 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - F	2x4	DRY	No.2
F - K	2x4	DRY	No.2
T - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
T - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV-p	MT20	3.0	4.0	
C, D, E, G, H, I					
C	TMW-w	MT20	2.0	4.0	
F	TTW-p	MT20	4.0	4.0	2.25 2.00
J	TMV-p	MT20	3.0	4.0	
L	BMV1-p	MT20	3.0	4.0	
M, N, O, P, Q, R, S					
M	BMV1-w	MT20	2.0	4.0	
T	BMV1-p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1-MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FR-TO
A-B	0/29	-91.8	-91.8 0.08 (1)	P-F	-207/0	10.00	0.10 (1)
B-C	-24/0	-91.8	-91.8 0.08 (1)	Q-E	-185/0	10.00	0.05 (1)
C-D	0/8	-91.8	-91.8 0.05 (1)	R-D	-187/0	10.00	0.03 (1)
D-E	0/11	-91.8	-91.8 0.05 (1)	S-C	-72/0	10.00	0.01 (1)
E-F	0/15	-91.8	-91.8 0.05 (1)	O-G	-185/0	10.00	0.05 (1)
F-G	0/15	-91.8	-91.8 0.05 (1)	N-H	-187/0	10.00	0.03 (1)
G-H	0/11	-91.8	-91.8 0.05 (1)	M-I	-72/0	10.00	0.01 (1)
H-I	0/8	-91.8	-91.8 0.05 (1)				
I-J	-24/0	-91.8	-91.8 0.08 (1)				
J-K	0/29	-91.8	-91.8 0.08 (1)				
T-B	-176/0	0.0	0.0 0.02 (1)				
L-J	-176/0	0.0	0.0 0.02 (1)				
T-S	0/0	-18.5	-18.5 0.02 (1)				
S-R	-3/0	-18.5	-18.5 0.02 (4)				
R-Q	-9/0	-18.5	-18.5 0.02 (4)				
Q-P	-13/0	-18.5	-18.5 0.01 (4)				
P-O	-13/0	-18.5	-18.5 0.01 (4)				
O-N	-9/0	-18.5	-18.5 0.02 (4)				
N-M	-3/0	-18.5	-18.5 0.02 (4)				
M-L	0/0	-18.5	-18.5 0.02 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (A-B:1), BC=0.02/1.00 (S-T:1), WB=0.10/1.00 (F-P:1), SS=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (F) (INPUT = 0.90)
JSI METAL= 0.10 (H) (INPUT = 1.00)



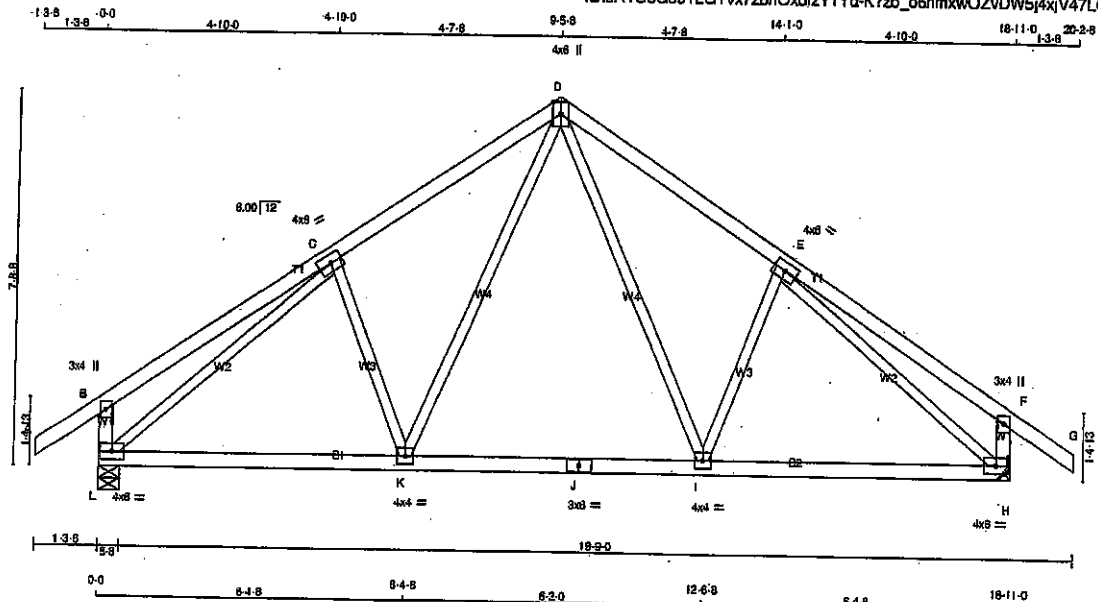
Structural component only
DWG# T-2108975

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417565	T33	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: aAYS6Go91LGyvx7zbnOxdizY1Yd-K7zb_o8nmkwOZvDW5j4xjV47LGHAI74PaficUOzXgsB



Scale = 1:43.5

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - G	2x4	DRY	No.2	SPF		
L - B	2x4	DRY	No.2	SPF		
H - F	2x4	DRY	No.2	SPF		
L - J	2x4	DRY	No.2	SPF		
J - H	2x4	DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-1	MT20	4.0	6.0		
D	TMWW+p	MT20	4.0	6.0	Edge	
E	TMWW-1	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMWW-1	MT20	4.0	6.0		
I	BMWW-1	MT20	4.0	4.0		
J	BS-1	MT20	3.0	6.0		
K	BMWW-1	MT20	4.0	4.0		
L	BMWW-1	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
L	1169	0	1169	0	5-8	5-8
H	1169	0	1169	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3'-8".

UNFACTORED REACTIONS

JT	1ST LOASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	824	554.0	0.0	0.0	0.0	0.0	270.0	0.0
H	824	554.0	0.0	0.0	0.0	0.0	270.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
		FROM	TO				
FR-TO							
A-B	0.735	-91.8	-91.8	0.12 (1)	10.00	D-I	0.7414
B-C	0.728	-91.8	-91.8	0.33 (1)	10.00	I-E	-275.0
C-D	-1019.0	-91.8	-91.8	0.27 (1)	5.88	K-D	0.7414
D-E	-1019.0	-91.8	-91.8	0.27 (1)	5.88	C-K	-275.0
E-F	0.728	-91.8	-91.8	0.33 (1)	10.00	L-C	-1243.0
F-G	0.735	-91.8	-91.8	0.12 (1)	10.00	E-H	-1243.0
L-B	-291.0	0.0	0.0	0.03 (1)	7.81		
H-F	-291.0	0.0	0.0	0.03 (1)	7.81		
L-K	0.7925	-18.5	-18.5	0.24 (4)	10.00		
K-J	0.7885	-18.5	-18.5	0.22 (4)	10.00		
J-I	0.7885	-18.5	-18.5	0.22 (4)	10.00		
I-H	0.7925	-18.5	-18.5	0.24 (4)	10.00		

TOTAL WEIGHT = 2 X 83 = 166 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA C86-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.63")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.03")
ALLOWABLE DEFL. (TL) = $L/360$ (0.63")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.07")

CSI: $TC=0.33/1.00$ (E-F:1), $BC=0.24/1.00$ (K-L:4),
 $WB=0.86/1.00$ (E-H:1), $SSI=0.17/1.00$ (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

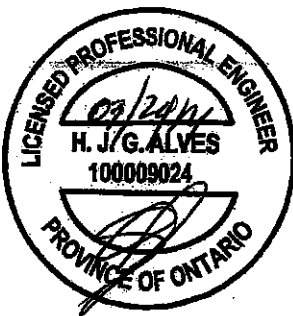
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1887 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

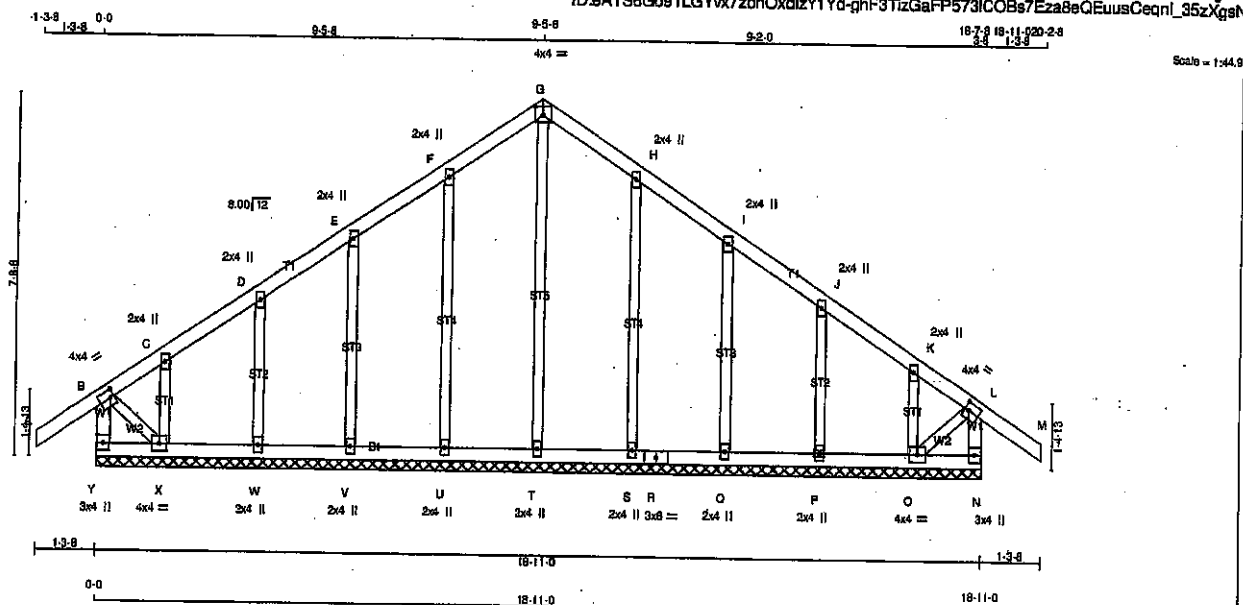
JSI GRIP= 0.82 (C) (INPUT = 0.90)
JSI METAL= 0.29 (E) (INPUT = 1.00)



Structural component only
DWG# T-2108989

JOB NAME 417565	TRUSS NAME G33	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID: sAYS8G091LGyvx7zbnOxdizY1Yd-phF3TizGaFP573ICOBs7Eza8eQEusCaqnl_35zXgsN



TOTAL WEIGHT = 88 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
Y - B	2x4 DRY	No.2	SPF
A - G	2x4 DRY	No.2	SPF
G - M	2x4 DRY	No.2	SPF
N - L	2x4 DRY	No.2	SPF
Y - R	2x4 DRY	No.2	SPF
R - N	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
ALL GABLE WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)				
BT TYPE	PLATES	W	LEN	Y X
B TMVW-t	MT20	4.0	4.0	2.00 1.00
C, D, E, F, H, I, J, K				
C TMW-w	MT20	2.0	4.0	
G TTV-p	MT20	4.0	4.0	2.25 2.00
L TMVW-t	MT20	4.0	4.0	2.00 1.00
N BMV1-p	MT20	3.0	4.0	
O BMWV1-t	MT20	4.0	4.0	
P, Q, S, T, U, V, W				
P BMW1-w	MT20	2.0	4.0	
R BS-t	MT20	3.0	6.0	
X BMWV1-t	MT20	4.0	4.0	
Y BMV1-p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
Y-B	-278 / 0	0.0	0.03 (1)	T-G	-140 / 0	0.15 (1)	
A-B	0 / 35	-91.8	-91.8 0.12 (1)	U-F	-209 / 0	0.14 (1)	
B-C	-60 / 0	-91.8	-91.8 0.12 (1)	V-E	-173 / 0	0.08 (1)	
C-D	-18 / 0	-91.8	-91.8 0.05 (1)	W-D	-197 / 0	0.04 (1)	
D-E	-21 / 0	-91.8	-91.8 0.05 (1)	X-C	-85 / 0	0.01 (1)	
E-F	-12 / 0	-91.8	-91.8 0.05 (1)	S-H	-209 / 0	0.14 (1)	
F-G	-22 / 0	-91.8	-91.8 0.05 (1)	Q-I	-173 / 0	0.06 (1)	
G-H	-22 / 0	-91.8	-91.8 0.05 (1)	P-J	-197 / 0	0.04 (1)	
H-I	-12 / 0	-91.8	-91.8 0.05 (1)	O-K	-85 / 0	0.01 (1)	
I-J	-21 / 0	-91.8	-91.8 0.05 (1)	S-X	0 / 30	0.01 (1)	
J-K	-18 / 0	-91.8	-91.8 0.05 (1)	O-L	0 / 30	0.01 (1)	
K-L	-60 / 0	-91.8	-91.8 0.12 (1)				
L-M	0 / 35	-91.8	-91.8 0.12 (1)				
N-L	-278 / 0	0.0	0.03 (1)				
Y-X	0 / 0	-18.5	-18.5 0.01 (4)				
X-W	0 / 21	-18.5	-18.5 0.02 (4)				
W-V	0 / 16	-18.5	-18.5 0.02 (4)				
V-U	0 / 13	-18.5	-18.5 0.02 (4)				
U-T	0 / 10	-18.5	-18.5 0.01 (4)				
T-S	0 / 10	-18.5	-18.5 0.01 (4)				
S-R	0 / 13	-18.5	-18.5 0.02 (4)				
R-Q	0 / 13	-18.5	-18.5 0.02 (4)				
Q-P	0 / 16	-18.5	-18.5 0.02 (4)				
P-O	0 / 21	-18.5	-18.5 0.02 (4)				
O-N	0 / 0	-18.5	-18.5 0.01 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (L-M:1), BC=0.02/1.00 (O-P:M), WS=0.15/1.00 (G-T:1), SS=0.08/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

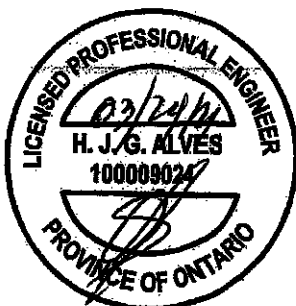
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

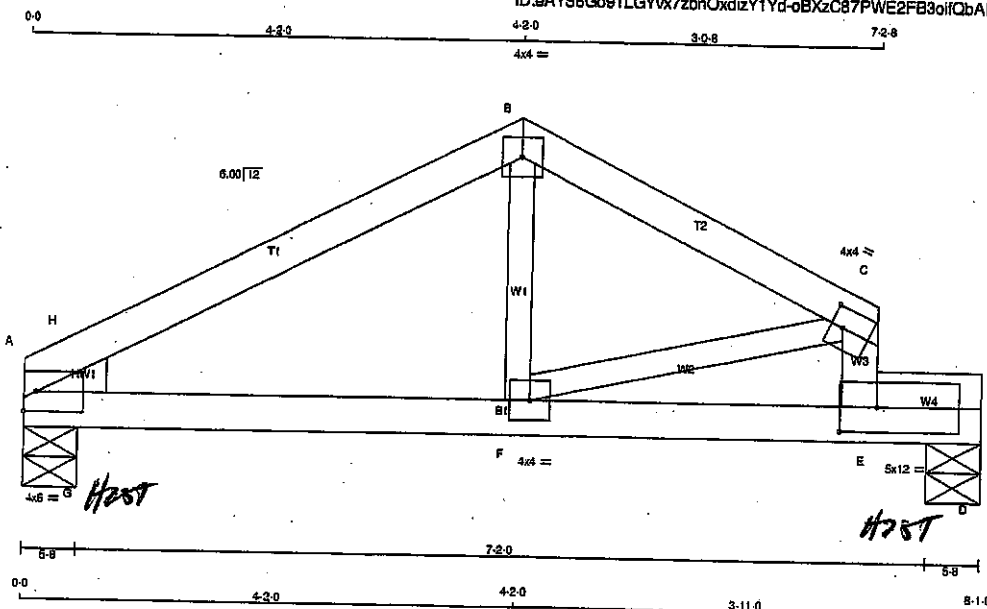
JSI GRIP = 0.67 (B) (INPUT = 0.90)
JSI METAL = 0.11 (H) (INPUT = 1.00)



Structural component only
DWG# T-2108976

JOB NAME 417565	TRUSS NAME T34W	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:17.7

TOTAL WEIGHT = 2 X 25 = 50 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
E - C	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TM9H14	MT20	4.0	6.0			Edge
B	TMV-2	MT20	4.0	4.0			
C	TMVW-1	MT20	4.0	4.0	2.00	1.25	
E	BMVW-1	MT20	5.0	12.0	2.50	3.75	
F	BMVW-1	MT20	4.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
END VERTICALS ARE NOT EXPOSED TO WIND.

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD.	HEEL
JT	GROSS REACTION	GROSS REACTION	BRG	BRG	WEDGE
	VERT	HCRZ	DOWN	HCRZ	UPLIFT
D	370	0	370	0	-227
A	441	0	441	88	-316

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 227 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT A FOR 316 LBS FACTORED UPLIFT

PROVIDE FOR 88 LBS FACTORED HORIZONTAL REACTION AT JOINT A

UNFACTORED REACTIONS	1ST LOOSE	MAX/MIN COMPONENT REACTIONS	JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
D	283	164 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0
A	312	204 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0

HORIZONTAL REACTIONS
A - 0 / 0 0 / 0 0 / 0 83 / -27 0 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, A

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (12)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF) CSI (LC)	MEMB.	FORCE (LBS) MAX CSI (LC)
FR-TO		FROM TO	LENGTH FR-TO	
A-H	-488 / 285	-91.8 -91.8 0.12 (1)	6.25	F-B -191 / 142 0.03 (12)
H-B	-488 / 346	-91.8 -91.8 0.18 (7)	6.25	F-C -315 / 441 0.10 (1)
B-C	-478 / 373	-91.8 -91.8 0.15 (1)	6.25	G-H -213 / 110 0.00 (1)
E-C	-467 / 291	0.0 0.0 0.05 (1)	7.81	
A-G	-304 / 426	-18.5 -18.5 0.17 (1)	6.25	
G-F	-304 / 426	-18.5 -18.5 0.22 (1)	6.25	
F-E	0 / 0	-18.5 -18.5 0.43 (1)	10.00	
E-D	0 / 0	-18.5 -18.5 0.18 (1)	10.00	

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (15-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe}, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015.

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.27")
CALCULATED VERT. DEFL. (LL) = L/999 (0.04")
ALLOWABLE DEFL. (TL) = L/360 (0.27")
CALCULATED VERT. DEFL. (TL) = L/999 (0.07")

CSI: TC=0.18/1.00 (B-H:7), BC=0.43/1.00 (E-F:1), WB=0.10/1.00 (C-F:1), SS=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

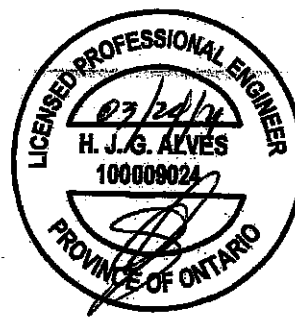
NAIL VALUES	PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)	
MAX MIN	MAX MIN	MAX MIN	MAX MIN	
MT20	850	371	1747	789 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.72 (C) (INPUT = 0.90)

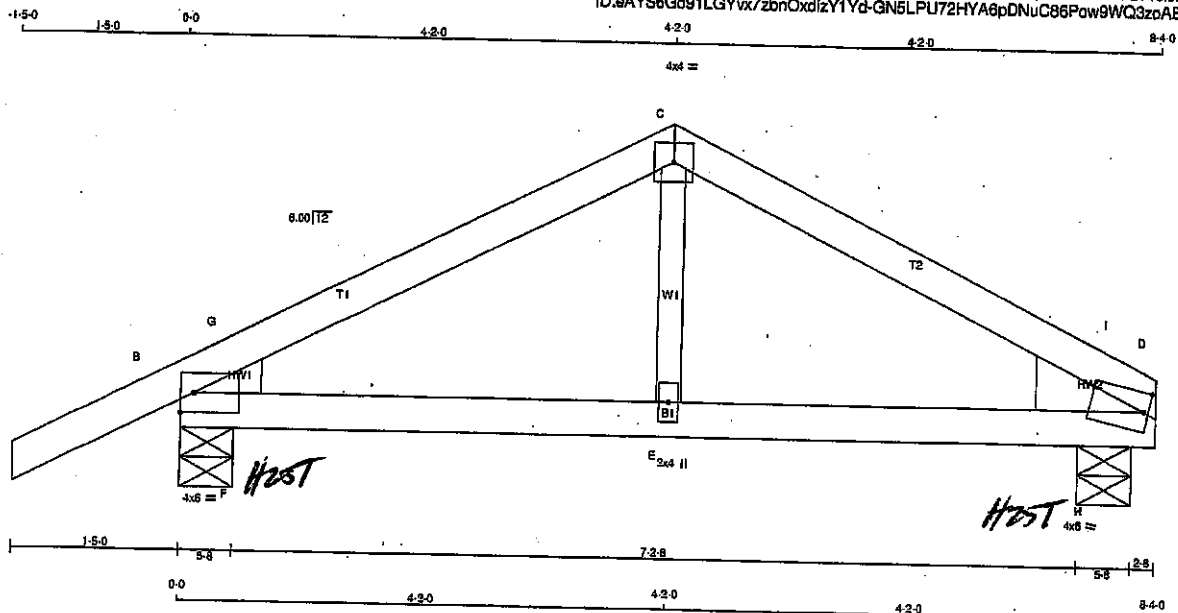
JSI METAL = 0.18 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108990

JOB NAME 417565	TRUSS NAME T35W	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:AYS6Go91LGYvx7zbnOxdizY1Yd-GN5LPU72HYA6pDNUC86Pow9WQ3zoAEV12z8jZ1zXgss



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBH1-I	MT20	4.0	6.0	Edge
C	TTW-p	MT20	4.0	4.0	
D	TMBH1-m	MT20	4.0	6.0	2.00 0.50
E	BNW-w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
B	595	0	595	80	-386	5-8	2x4 L
D	459	0	459	0	-296	5-8	2x6 R

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 386 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 296 LBS. FACTORED UPLIFT

PROVIDE FOR 80 LBS. FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
B	418	289.0	0.0	0.0	0.0	129.0	0.0
D	325	213.0	0.0	0.0	0.0	112.0	0.0

HORIZONTAL REACTIONS

B	0.0	0.0	0.0	57.1	32	0.0	0.0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO				
A-B	0 / 21	-91.8	-91.8	10.00
B-G	-520 / 311	-91.8	-91.8	6.25
G-C	-500 / 358	-91.8	-91.8	6.25
C-I	-500 / 382	-91.8	-91.8	6.25
I-D	-520 / 380	-91.8	-91.8	6.25
B-F	-284 / 439	-18.5	-18.5	6.25
F-E	-284 / 439	-18.5	-18.5	6.25
E-H	-284 / 439	-18.5	-18.5	6.25
H-D	-284 / 439	-18.5	-18.5	6.25

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.8 } PSF AT (15.0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe}, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 8.0 PSF AND 7.4 PSF RESPECTIVELY.

TOTAL WEIGHT = 2 X 26 = 52 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. GC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.28")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.28")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TC=0.17/1.00 (C-G:7), BC=0.17/1.00 (E-H:1), WB=0.03/1.00 (C-E:12), SSI=0.13/1.00 (C-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSD) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.71 (B) (INPUT = 0.90)
JSI METAL = 0.12 (D) (INPUT = 1.00)

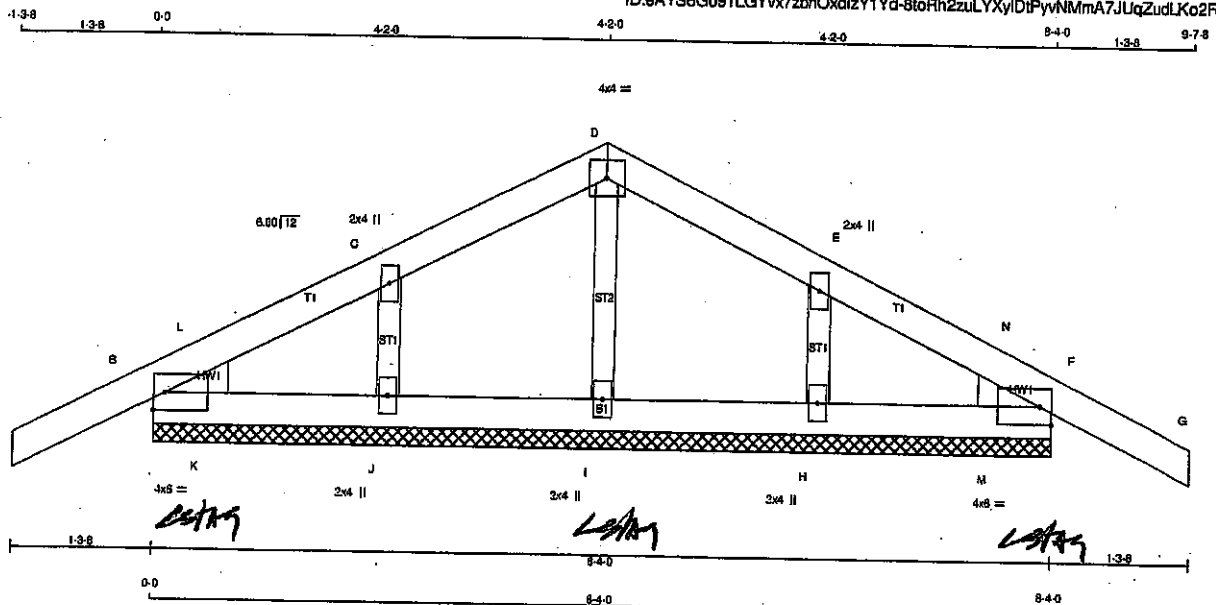


Structural component only
DWG# T-2108991

JOB NAME 417565	TRUSS NAME G35W	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 6.420 S Jan 21 2021 M/Tek Industries, Inc. Wed Mar 24 18:39:03 2021 Page 1
ID: eAYS6Go91LGYvx7zbnOxdizY1Yd-StoRh2zuLYXyIDtPyNMMA7JUqZudLKo2RVXbXzXgsm



TOTAL WEIGHT = 28 lb

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
A - D	2x4	DRY	No.2		SPF
D - G	2x4	DRY	No.2		SPF
B - F	2x4	DRY	No.2		SPF
ALL WEBS		2x3	DRY	No.2	
ALL GABLE WEBS					SPF
		2x3	DRY	No.2	
DRY: SEASONED LUMBER.					

GABLE STUDS SPACED AT 2'-0" O.C.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	6.0		Edge
C	TMW-w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TMW-w	MT20	2.0	4.0		
F	TMBH1-I	MT20	4.0	6.0		Edge
H, I, J						
H	BMW1-w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL

WEDGE

2x4 L

2x4 R

PROVIDE ANCHORAGE AT ALL BEARING JOINTS FOR 150 LBS FACTORED UPLIFT.

PROVIDE FOR 87 LBS FACTORED HORIZONTAL REACTION AT JOINT B

HORIZONTAL REACTIONS

1ST LCASE MAX. MIN. COMPONENT REACTIONS

	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	---	0/0	0/0	0/0	48/48	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO						FR-TO		
A-B	0/19	-91.8	-91.8	0.12 (1)	10.00	I-D	-110/0	0.02 (1)
B-L	-89/41	-91.8	-91.8	0.01 (1)	6.25	J-C	-225/158	0.03 (7)
L-C	-51/60	-91.8	-91.8	0.06 (1)	6.25	H-E	-224/158	0.03 (8)
C-D	-60/102	-91.8	-91.8	0.06 (1)	6.25	K-L	-34/59	0.00 (1)
D-E	-38/91	-91.8	-91.8	0.06 (1)	6.25	M-N	-33/54	0.00 (1)
E-N	-46/37	-91.8	-91.8	0.06 (1)	6.25			
N-F	-53/15	-91.8	-91.8	0.01 (1)	6.25			
F-G	0/19	-91.8	-91.8	0.12 (1)	10.00			
B-K	0/58	-18.5	-18.5	0.03 (1)	10.00			
K-J	0/58	-18.5	-18.5	0.03 (1)	10.00			
J-I	0/60	-18.5	-18.5	0.02 (1)	10.00			
I-H	0/60	-18.5	-18.5	0.02 (1)	10.00			
H-M	0/57	-18.5	-18.5	0.03 (1)	10.00			
M-F	0/57	-18.5	-18.5	0.03 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (8.6) PSF AT (15'-0") FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe}, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0'-0") FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2016, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (A-B:1), BC=0.03/1.00 (J-K:1), WB=0.03/1.00 (C-J:7), SS=0.09/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

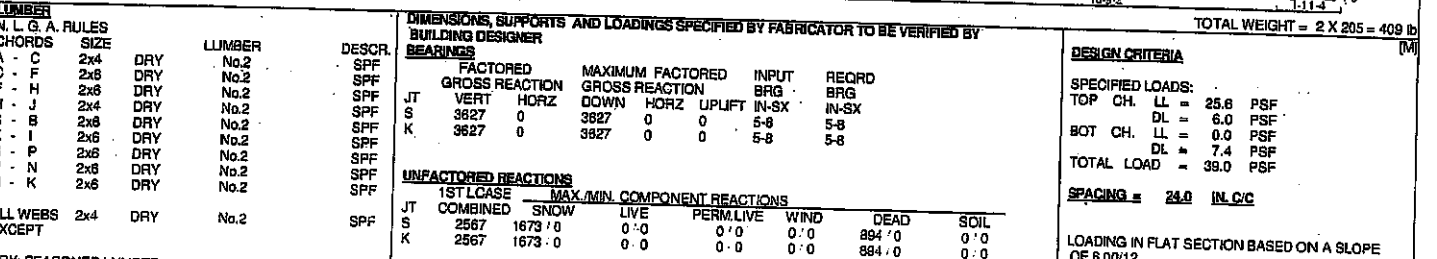
JSI GRIP= 0.25 (F) (INPUT = 0.90)

JSI METAL= 0.09 (E) (INPUT = 1.00)



Structural component only
DWG# T-2108977

Version 8.420 S Jan 21 2021 MTek Industries, Inc. Wed Mar 24 17:18:21 2021 Page 1
ID: eAYS6Go81LGYvx7zbnOxdIzY1Yd-6WHsaKWz12dOeb8b47gJLrNASMC26hY5HhJMsZxgHW
-1-3-8 0-0 5-10-8 5-10-8 6-9-14 12-9-6 6-11-10 19-9-0 6-11-10 26-7-10 6-9-14 33-5-0 40-7-8
1-3-8 5-10-8 1-3-8
Scale = 1:56.8



TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

AF	0.0	-18.5	-18.5	0.06 (4)	10.00
AG	0.0	-18.5	-18.5	0.06 (4)	10.00
AH	0.0	-18.5	-18.5	0.08 (4)	10.00
AR	0.5045	-18.5	-18.5	0.38 (1)	10.00
AI	0.5045	-18.5	-18.5	0.38 (1)	10.00
AJ	0.5045	-18.5	-18.5	0.38 (1)	10.00
AK	0.5045	-18.5	-18.5	0.38 (1)	10.00
Q	0.5045	-18.5	-18.5	0.38 (1)	10.00
QK	0.5045	-18.5	-18.5	0.38 (1)	10.00
P	0.8747	-18.5	-18.5	0.84 (1)	10.00
PL	0.8747	-18.5	-18.5	0.84 (1)	10.00
LO	0.8747	-18.5	-18.5	0.84 (1)	10.00
OM	0.8747	-18.5	-18.5	0.84 (1)	10.00
N	0.8747	-18.5	-18.5	0.84 (1)	10.00
NN	0.8747	-18.5	-18.5	0.84 (1)	10.00
N	0.8747	-18.5	-18.5	0.84 (1)	10.00
MO	0.8747	-18.5	-18.5	0.84 (1)	10.00
I	0.5045	-18.5	-18.5	0.38 (1)	10.00
IQ	0.5045	-18.5	-18.5	0.38 (1)	10.00
AP	0.5045	-18.5	-18.5	0.38 (1)	10.00
QK	0.5045	-18.5	-18.5	0.38 (1)	10.00

Structural component only
DWG# T-2109000 1/2

JOB NAME 417566	TRUSS NAME T40	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mtek Industries, Inc. Wed Mar 24 17:18:21 2021 Page 2 ID:AYS8G91LGYx7zbnOxdizY1Yd-6WHsaKWz12dOeb6b47qJLRnASMC29hYsHihJMsZxqHw	

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	6.0	2.00 3.00
C	TTWW-m	MT20	5.0	8.0	Edge
D	TMVW-t	MT20	5.0	8.0	
E	TMVW-w	MT20	3.0	8.0	
F	TS-t	MT20	5.0	6.0	
G	TMVW-t	MT20	5.0	6.0	
H	TTWW-m	MT20	5.0	8.0	Edge
I	TMVW-p	MT20	5.0	6.0	2.00 3.00
K	BMV1-p	MT20	3.0	6.0	
L	BMVW-t	MT20	5.0	6.0	2.50 2.50
M	BMVW-t	MT20	5.0	6.0	
N	BS-t	MT20	5.0	8.0	
O	BMVW-t	MT20	5.0	8.0	
P	BS-t	MT20	5.0	8.0	
Q	BMVW-t	MT20	5.0	6.0	
R	BMVW-t	MT20	5.0	6.0	2.50 2.50
S	BMV1-p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED CS (LBS)
FR-TO		FROM	TO
AQ-L	0/5045	-18.5	-18.5 0.38 (1) 10.00
L-AR	0/0	-18.5	-18.5 0.08 (4) 10.00
AR-AS	0/0	-18.5	-18.5 0.08 (4) 10.00
AS-K	0/0	-18.5	-18.5 0.08 (4) 10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC	LOC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-254	-254	-	FRONT	VERT	TOTAL	-	C1
E	19-8-0	-76	-76	-	FRONT	VERT	TOTAL	-	C1
H	33-5-8	-254	-254	-	FRONT	VERT	TOTAL	-	C1
L	33-4-12	-21	-21	-	FRONT	VERT	TOTAL	-	C1
N	23-4-12	-21	-21	-	FRONT	VERT	TOTAL	-	C1
O	19-8-0	-21	-21	-	FRONT	VERT	TOTAL	-	C1
P	15-11-4	-21	-21	-	FRONT	VERT	TOTAL	-	C1
R	5-11-4	-21	-21	-	FRONT	VERT	TOTAL	-	C1
T	7-11-4	-76	-76	-	FRONT	VERT	TOTAL	-	C1
U	9-11-4	-76	-76	-	FRONT	VERT	TOTAL	-	C1
V	11-11-4	-76	-76	-	FRONT	VERT	TOTAL	-	C1
W	13-11-4	-76	-76	-	FRONT	VERT	TOTAL	-	C1
X	15-11-4	-76	-76	-	FRONT	VERT	TOTAL	-	C1
Y	17-11-4	-76	-76	-	FRONT	VERT	TOTAL	-	C1
Z	21-4-12	-76	-76	-	FRONT	VERT	TOTAL	-	C1
AA	23-4-12	-76	-76	-	FRONT	VERT	TOTAL	-	C1
AB	25-4-12	-76	-76	-	FRONT	VERT	TOTAL	-	C1
AC	27-4-12	-76	-76	-	FRONT	VERT	TOTAL	-	C1
AD	29-4-12	-76	-76	-	FRONT	VERT	TOTAL	-	C1
AE	31-4-12	-76	-76	-	FRONT	VERT	TOTAL	-	C1
AF	1-11-4	-20	-20	-	FRONT	VERT	TOTAL	-	C1
AG	3-11-4	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AH	7-11-4	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AI	9-11-4	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AJ	11-11-4	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AK	13-11-4	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AL	17-11-4	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AM	21-4-12	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AN	25-4-12	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AO	27-4-12	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AP	29-4-12	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AQ	31-4-12	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AR	35-4-12	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AS	37-4-12	-20	-20	-	FRONT	VERT	TOTAL	-	C1

CONNECTION REQUIREMENTS

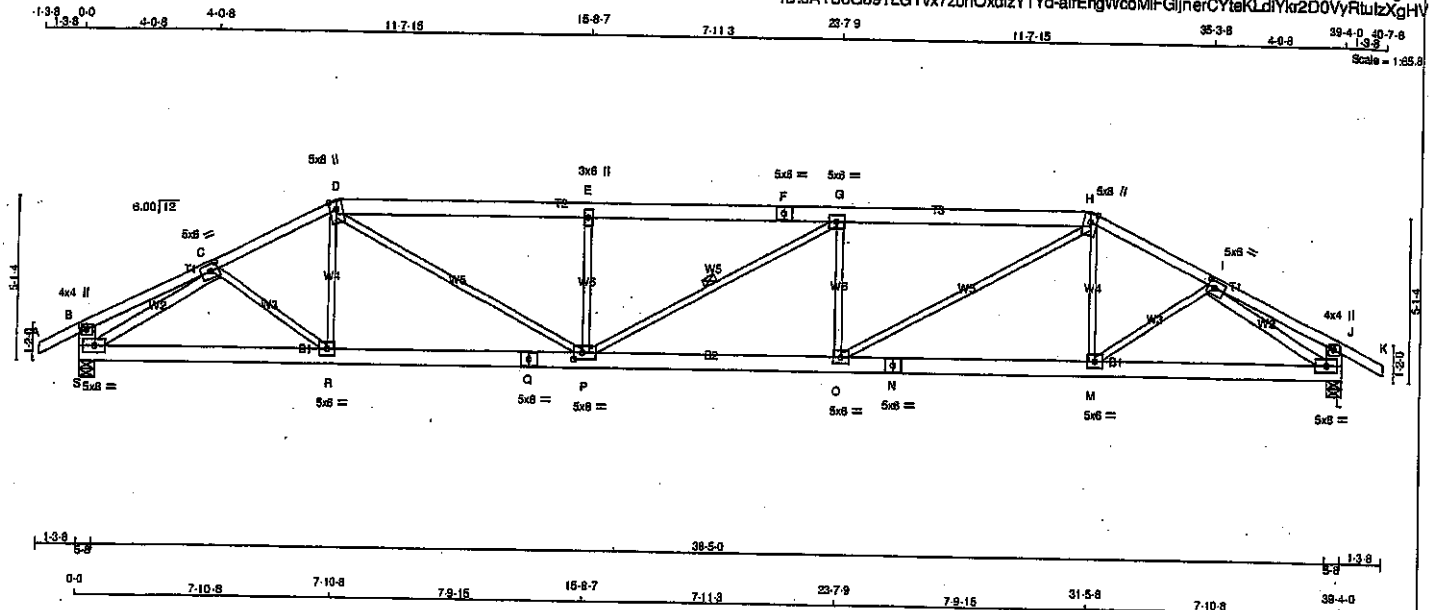
1) C1: A SUITABLE HANGER MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2109000

JOB NAME 417566	TRUSS NAME T41	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 5 Jan 21 2021 MTEK Industries, Inc. Wed Mar 24 17:18:22 2021 Page 1
ID:AYS6Go91LGYvx7zbnOxdizY1Yd-airEngWcoMIFGJlnerCYteKldYkr2D0VyRtutzXgHv



TOTAL WEIGHT = 191 lb

LUMBER						
N. L. G. A. RULES						
CHORDS		SIZE		LUMBER		DESCR
A - D	2x4	DRY		No.2		SPF
D - F	2x6	DRY		No.2		SPF
F - H	2x6	DRY		No.2		SPF
H - K	2x4	DRY		No.2		SPF
S - B	2x6	DRY		No.2		SPF
L - J	2x6	DRY		No.2		SPF
S - C	2x6	DRY		No.2		SPF
Q - N	2x6	DRY		No.2		SPF
N - L	2x6	DRY		No.2		SPF
ALL WEBS EXCEPT				No.2		SPF
S - C	2x4	DRY		No.2		SPF
I - L	2x4	DRY		No.2		SPF
DRY: SEASONED LUMBER.						

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV-w	MT20	4.0	4.0		
C TMVW-t	MT20	5.0	6.0	2.50	2.50
D TTWW-m	MT20	5.0	8.0	Edge	
E TMV-w	MT20	3.0	6.0		
F TS-t	MT20	5.0	6.0		
G TMVW-t	MT20	5.0	6.0		
H TTWW-m	MT20	5.0	8.0	Edge	
I TMVW-t	MT20	5.0	6.0	2.50	2.50
J TMV-w	MT20	4.0	4.0		
L BMVW-t	MT20	5.0	8.0		
M, O, R					
M BMVW-t	MT20	5.0	6.0		
N BS-t	MT20	5.0	6.0		
P BMVW-t	MT20	5.0	8.0	2.50	3.00
Q BS-t	MT20	5.0	6.0		
S BMVW-t	MT20	5.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES:

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
S	2293	0	2293	0	5-8	5-8
L	2293	0	2293	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
S	1619	1078 / 0	0 / 0	0 / 0	0 / 0	548 / 0	0 / 0
L	1619	1078 / 0	0 / 0	0 / 0	0 / 0	548 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL FITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO						FR-TO		
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	C-R	0 / 143	0.04 (4)
B-C	0 / 16	-91.8	-91.8	0.20 (1)	10.00	R-D	0 / 117	0.04 (4)
C-D	-3292 / 0	-91.8	-91.8	0.36 (1)	3.58	M-H	0 / 117	0.04 (4)
D-E	-4516 / 0	-91.8	-91.8	0.53 (1)	3.67	M-I	0 / 143	0.04 (4)
E-F	-4516 / 0	-91.8	-91.8	0.54 (1)	3.66	S-C	-3439 / 0	0.87 (1)
F-G	-4516 / 0	-91.8	-91.8	0.54 (1)	3.66	I-L	-3439 / 0	0.87 (1)
G-H	-4520 / 0	-91.8	-91.8	0.53 (1)	3.66	C-H	0 / 1943	0.41 (1)
H-I	-3291 / 0	-91.8	-91.8	0.36 (1)	3.58	D-P	0 / 1838	0.41 (1)
I-J	0 / 16	-91.8	-91.8	0.20 (1)	10.00	O-G	-805 / 0	0.27 (1)
J-K	0 / 28	-91.8	-91.8	0.12 (1)	10.00	P-E	-804 / 0	0.27 (1)
S-B	-270 / 0	0.0	0.0	0.02 (1)	7.81	P-G	-5 / 0	0.00 (1)
L-J	-270 / 0	0.0	0.0	0.02 (1)	7.81			
S-R	0 / 2823	-18.5	-18.5	0.40 (1)	10.00			
R-Q	0 / 2936	-18.5	-18.5	0.41 (1)	10.00			
Q-P	0 / 2936	-18.5	-18.5	0.41 (1)	10.00			
P-O	0 / 4520	-18.5	-18.5	0.81 (1)	10.00			
O-N	0 / 2937	-18.5	-18.5	0.41 (1)	10.00			
N-M	0 / 2937	-18.5	-18.5	0.41 (1)	10.00			
M-L	0 / 2823	-18.5	-18.5	0.40 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.31")
CALCULATED VERT. DEFL.(LL) = L/999 (0.25")
ALLOWABLE DEFL.(TL) = L/360 (1.31")
CALCULATED VERT. DEFL.(TL) = L/999 (0.47")

CSI: TC=0.54/1.00 (E-G:1), BC=0.51/1.00 (O-P:1), WB=0.87/1.00 (C-S:1), SS=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

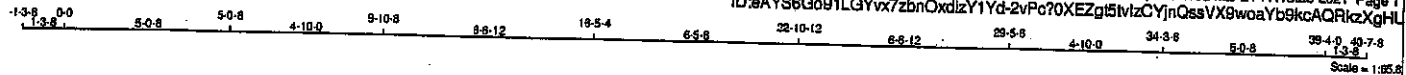
JSI GRIP = 0.88 (C) (INPUT = 0.50)
JSI METAL = 0.79 (C) (INPUT = 1.00)



Structural component only
DWG# T-2109001

JOB NAME 417566	TRUSS NAME T42	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:AYS6Go91LGYvx7zbnOxdizY1Yd-2vPc70XEZg15ivIzCYjnQssVX9woaYb9kcAQRkzXgHL



LUMBER					
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER		DESCR.	
A - D	2x4	DRY	No.2	SPF	
D - F	2x6	DRY	No.2	SPF	
F - H	2x6	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
K - B	2x6	DRY	No.2	SPF	
B - J	2x6	DRY	No.2	SPF	
J - R	2x6	DRY	No.2	SPF	
R - O	2x6	DRY	No.2	SPF	
O - L	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
RY: SEASONED LUMBER					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	9.0	1.00	4.75
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTWV+m	MT20	5.0	6.0	2.25	2.25
E	TMVW-w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMVW-t	MT20	5.0	6.0		
H	TTWV+m	MT20	5.0	6.0	2.25	2.25
I	TMVW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-p	MT20	4.0	9.0	1.00	4.75
L	BMV-t	MT20	3.0	6.0		
M	BMVW-t	MT20	5.0	6.0	2.50	3.75
N, P, S						
N	BMVW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
Q	BSW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
T	BMVW-t	MT20	5.0	6.0	2.50	3.75
U	BMV-t	MT20	3.0	6.0		

NOTES:

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UP	IN-SX
U	2293	0	0	5-8
L	2293	0	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1619	1076 / 0	0 / 0	0 / 0	0 / 0	543 / 0	0 / 0
L	1619	1076 / 0	0 / 0	0 / 0	0 / 0	543 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	T-C	-415 / 0	0.09 (1)
B-C	-3254 / 0	-91.8	-91.8 0.59 (1)	3.38	C-S	-121 / 0	0.07 (1)
C-D	-3189 / 0	-91.8	-91.8 0.56 (1)	3.42	S-D	0 / 220	0.05 (4)
D-E	-3737 / 0	-91.8	-91.8 0.33 (1)	4.17	D-Q	0 / 1191	0.27 (1)
E-F	-3736 / 0	-91.8	-91.8 0.33 (1)	4.17	Q-E	-665 / 0	0.34 (1)
F-G	-3736 / 0	-91.8	-91.8 0.33 (1)	4.17	Q-G	-8 / 0	0.00 (1)
G-H	-3742 / 0	-91.8	-91.8 0.34 (1)	4.16	P-G	-867 / 0	0.34 (1)
H-I	-3189 / 0	-91.8	-91.8 0.56 (1)	3.38	N-H	0 / 1188	0.27 (1)
I-J	-3265 / 0	-91.8	-91.8 0.59 (1)	3.42	P-H	0 / 217	0.05 (4)
J-K	0 / 28	-91.8	-91.8 0.12 (1)	10.00	N-I	-122 / 0	0.07 (1)
U-B	-2228 / 0	0.0	0.0 0.14 (1)	6.85	M-I	-414 / 0	0.09 (1)
L-J	-2228 / 0	0.0	0.0 0.14 (1)	6.85	B-T	0 / 2974	0.87 (1)
U-T	0 / 0	-18.5	-18.5 0.08 (1)	10.00	M-J	0 / 2974	0.67 (1)
T-S	0 / 2940	-18.5	-18.5 0.41 (1)	10.00			
S-R	0 / 2835	-18.5	-18.5 0.39 (1)	10.00			
R-O	0 / 2835	-18.5	-18.5 0.39 (1)	10.00			
Q-P	0 / 3741	-18.5	-18.5 0.49 (1)	10.00			
P-O	0 / 2835	-18.5	-18.5 0.38 (1)	10.00			
O-N	0 / 2835	-18.5	-18.5 0.38 (1)	10.00			
N-M	0 / 2840	-18.5	-18.5 0.41 (1)	10.00			
M-L	0 / 0	-18.5	-18.5 0.08 (1)	10.00			

TOTAL WEIGHT = 195 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	8.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/960$ (1.31")
CALCULATED VERT. DEFL.(LL) = $L/989$ (0.20")
ALLOWABLE DEFL.(TL) = $L/960$ (1.31")
CALCULATED VERT. DEFL.(TL) = $L/989$ (0.37")

CSI: TC=0.59/1.00 (I-J:1), BC=0.49/1.00 (P-Q:1), WB=0.67/1.00 (J-M:1), SS=0.21/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

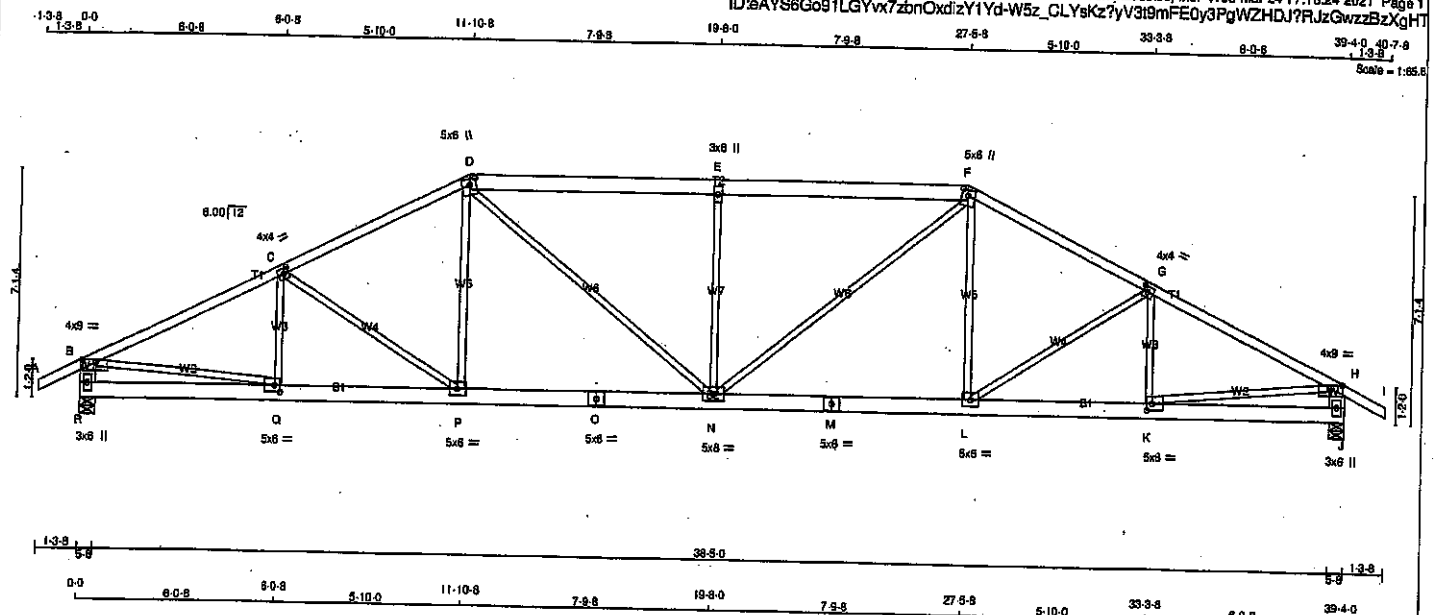
JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.54 (J) (INPUT = 1.00)



Structural component only
DWG# T-2109002

JOB NAME 417566	TRUSS NAME T43	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 191 lb
[M/F]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x8	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	9.0	1.00	4.75
C	TMVW-w	MT20	4.0	4.0	2.00	1.75
D	TMVW+m	MT20	5.0	6.0	2.25	2.25
E	TMVW-w	MT20	3.0	6.0		
F	TMVW+m	MT20	5.0	6.0	2.25	2.25
G	TMVW-w	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	4.0	9.0	1.00	4.75
J	BMV1-p	MT20	3.0	6.0		
K	BMVW-t	MT20	5.0	6.0	2.50	2.00
L	BMVW-t	MT20	5.0	6.0		
M	BS-1	MT20	5.0	6.0		
N	BMVW-w	MT20	5.0	6.0		
O	BS-1	MT20	5.0	6.0		
P	BMVW-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.00
R	BMV1-p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
R	2293	0	5-8	5-8
J	2293	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1619	1076	0	0	0	543	0
J	1619	1076	0	0	0	543	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	UNBRAC	W E B S	MAX. FACTORED	MAX
MEMB.	FORCE	(PLF)	(LBS)	(LBS)	(LBS)	LENGTH	MEMB.	FORCE	CSI (LC)
FR-TO							FR-TO		
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00		C-C	-314 / 0	0.08 (1)
B-C	-3331 / 0	-91.8	-91.8	0.64 (1)	3.31		C-P	-354 / 0	0.31 (1)
C-D	-3051 / 0	-91.8	-91.8	0.58 (1)	3.49		P-D	0 / 353	0.08 (1)
D-E	-3312 / 0	-91.8	-91.8	0.40 (1)	4.29		D-N	0 / 800	0.18 (1)
E-F	-3312 / 0	-91.8	-91.8	0.40 (1)	4.29		N-E	-906 / 0	0.89 (1)
F-G	-3051 / 0	-91.8	-91.8	0.58 (1)	3.49		N-F	0 / 800	0.18 (1)
G-H	-3331 / 0	-91.8	-91.8	0.64 (1)	3.31		L-F	0 / 353	0.08 (1)
H-I	0 / 28	-91.8	-91.8	0.12 (1)	10.00		L-G	-354 / 0	0.31 (1)
R-B	-2227 / 0	0.0	0.0	0.14 (1)	6.85		K-G	-314 / 0	0.08 (1)
J-H	-2227 / 0	0.0	0.0	0.14 (1)	6.85		B-Q	0 / 3029	0.68 (1)
							K-H	0 / 3029	0.68 (1)
R-Q	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
O-P	0 / 3004	-18.5	-18.5	0.42 (1)	10.00				
P-O	0 / 2704	-18.5	-18.5	0.38 (1)	10.00				
D-N	0 / 2704	-18.5	-18.5	0.38 (1)	10.00				
N-M	0 / 2704	-18.5	-18.5	0.38 (1)	10.00				
M-L	0 / 2704	-18.5	-18.5	0.38 (1)	10.00				
L-K	0 / 3004	-18.5	-18.5	0.42 (1)	10.00				
K-J	0 / 0	-18.5	-18.5	0.08 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(85 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.31")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (1.31")
CALCULATED VERT. DEFL.(TL) = L/999 (0.33")

CSI: TC=0.64/1.00 (G-H:1), BC=0.42/1.00 (K-L:1), WB=0.69/1.00 (E-N:1), SS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (C) (INPUT = 0.90)
JSI METAL = 0.68 (C) (INPUT = 1.00)

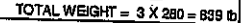


Structural Component only
DWG# T-2109003

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ID:sAYS6Go91LGYvxZbnOxdizY1Yd-HXMPHZU5H7p7CRMJzFVHyuZW12TqSBwFXvdzXgHS

1-3.8 0.0 4.7-10 4.7-10 4.5-2 9.0-12 4.9-12 13-10-8 5.0-8 19.0-0 5.3-8 25.5-8 4.9-12 30.3-4 4.5-2 34.8-8 4.7-10 39.4-0 40.7-8 1.3-8

Scale = 1.68.5



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
<u>BEARINGS</u>									
JT	FACTORED		MAXIMUM FACTORED		INPUT BRG	REQD		BRG	IN-SX
	GROSS	REACTION	GROSS	REACTION		IN-SX	IN-SX		
B	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
JT	4874	0	4874	0	0	5-8	5-8	5-8	5-8
B	12453	0	12453	0	0	5-8	5-8	5-8	5-8

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.13 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
 2x6 DRY SPF No.2 T-BRACE AT H-R, I-P
 FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
LOADING
 TOTAL LOAD CASES: (4)

Z	-10044.0	-91.8	-91.8	0.20 (1)	3.19	O-Y	0 / 10058	0.54 (1)
V						O-K	-1074.0	0.11 (1)
U	-14341.0	-91.8	-91.8	0.25 (1)	3.81	N-K	0 / 3545	0.19 (1)
M	0.4	-91.8	-91.8	0.02 (1)	10.00	W-X	0 / 2948	0.00 (1)
						W-C	-4498.0	0.32 (1)
W	0 / 3428	-18.5	-18.5	0.15 (1)	10.00	K-Y	-12826.0	0.90 (1)
V	0 / 7048	-18.5	-18.5	0.27 (1)	10.00	Y-Z	0 / 6343	0.00 (1)
U	0 / 7046	-18.5	-18.5	0.26 (1)	10.00			
T	0 / 7496	-18.5	-18.5	0.28 (1)	10.00			
S	0 / 7322	-18.5	-18.5	0.25 (1)	10.00			
R	0 / 7322	-18.5	-18.5	0.25 (1)	10.00			
Q	0 / 11266	-18.5	-18.5	0.37 (1)	10.00			
P	0 / 11266	-18.5	-18.5	0.37 (1)	10.00			
O	0 / 18047	-18.5	-18.5	0.65 (1)	10.00			
NA	0 / 18935	-18.5	-18.5	0.89 (1)	10.00			
AB	0 / 18935	-18.5	-18.5	0.89 (1)	10.00			
N	0 / 18935	-18.5	-18.5	0.89 (1)	10.00			
Y	0 / 19028	-18.5	-18.5	0.72 (1)	10.00			
L	0 / 8887	-18.5	-18.5	0.39 (1)	10.00			

SPECIFIC CONCENTRATED LOADS (LBS)								
LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
30-3-4	-6227	-6227	--	BACK	VERT	TOTAL	--	C1
32-4-12	-775	-775	--	BACK	VERT	TOTAL	--	C1
34-4-12	-1991	-1991	--	BACK	VERT	TOTAL	--	C1

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417566	T44	1	3	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Mar 24 17:18:25 2021 Page 2
ID: eAYS6G091LGYvx7zbnOxdizY1Yd- HXMPH7U5H7p7CRMJziFVHvuZzW12TqSBwfXVdzXqH5

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

PLATES (table is in inches)

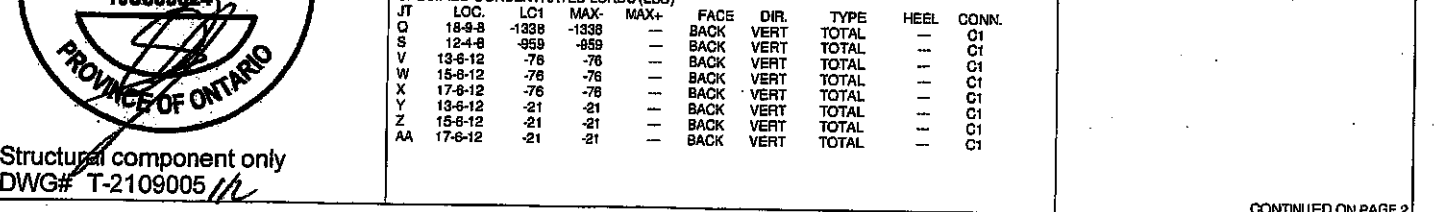
JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-m	MT20	10.0	12.0	4.75	0.25
C	TMWWW-t	MT20	8.0	12.0	3.00	4.50
D	TS-t	MT20	5.0	8.0		
E	TMW-w	MT20	5.0	8.0	4.00	1.25
F	TTWW-m	MT20	8.0	9.0	3.00	3.00
G	TMW-w	MT20	3.0	6.0		
H	TTWW-m	MT20	8.0	9.0	3.00	3.00
I	TMWWW-t	MT20	5.0	8.0	4.00	1.25
J	TS-t	MT20	5.0	8.0		
K	TMWWW-t	MT20	6.0	12.0	3.00	4.50
L	TMBMW1-m	MT20	10.0	12.0	2.50	1.50
N	BMW-w	MT20	3.0	10.0		
O	BMWW-t	MT20	7.0	12.0		
P	BMWW-t	MT20	6.0	8.0	4.50	2.75
Q	BS-t	MT20	6.0	9.0		
R	BMWW-t	MT20	8.0	9.0		
S	BS-t	MT20	6.0	9.0		
T	BMWW-t	MT20	6.0	9.0	4.50	2.75
U	BMWW-t	MT20	7.0	12.0		
V	BMW-w	MT20	3.0	10.0		

NOTES (1)

- 1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural componeht only
DWG# T-2109004 *20*



JOB NAME 417566	TRUSS NAME T45	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MITek Industries, Inc. Wed Mar 24 17:18:26 2021 Page 2 ID:AYS6Gc91LGYvx7zbnOxdizY1Yd-SU4ld1a6sbFqkM0YtoGU2UU VNschnwhcQaP413zXqHf	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0		
B	TTWW-m	MT20	8.0	9.0	Edge	
C	TMWW-t	MT20	5.0	6.0		
D	TMW-w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	8.0		
F	TMWW-t	MT20	5.0	8.0		
G	TTWW-m	MT20	5.0	8.0	Edge	
H	TTWW-m	MT20	5.0	6.0	2.50	2.75
I	TTW-m	MT20	4.0	6.0		
J	TMVW-p	MT20	5.0	6.0	2.00	3.00
L	BMV1-p	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	12.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.75
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	6.0	9.0		
R	BS-t	MT20	6.0	9.0		
S	BMWW-t	MT20	5.0	8.0	2.50	3.75
T	BMWW-t	MT20	5.0	8.0	2.50	3.75
U	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

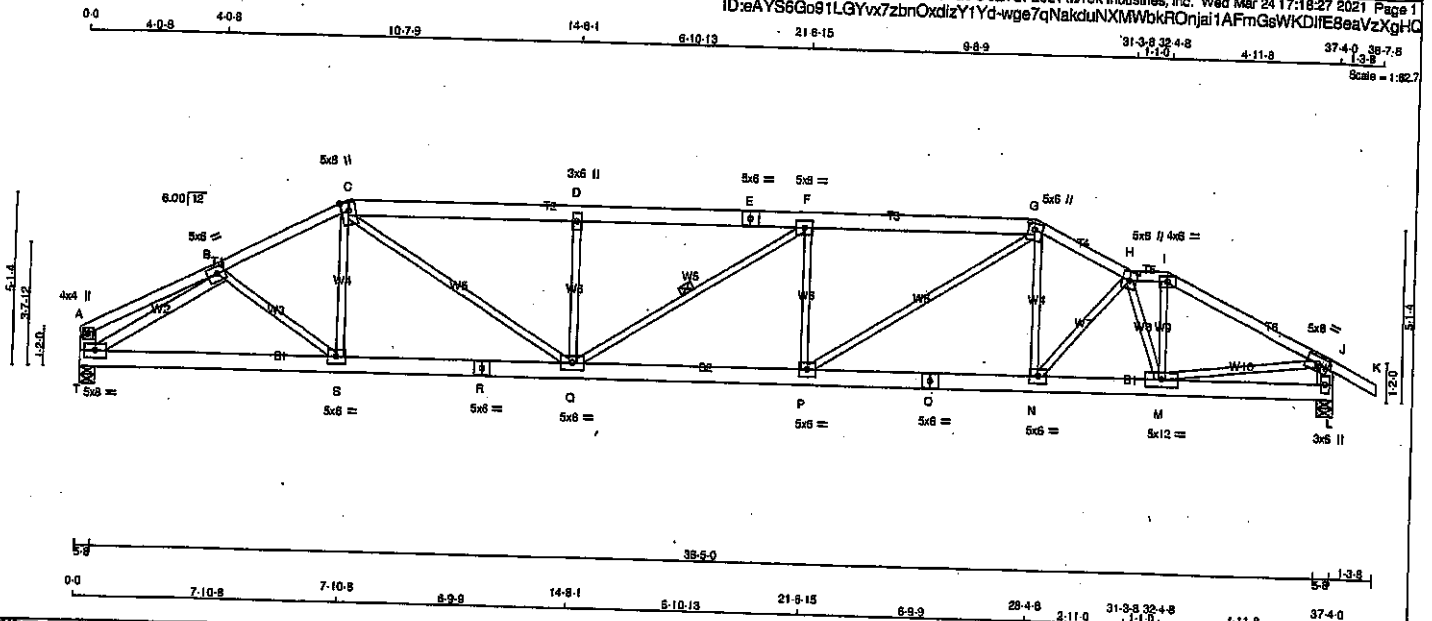


Structural component only
DWG# T-2109005 *W*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
417566	T46	1	1	GREENPARK HOMES		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mtek Industries, Inc. Wed Mar 24 17:18:27 2021 Page 1
ID:AYS6G091LGYv7zbnOxdizY1Yd-wge7qNakduNXMWbKRONjai1AFmGsWKDIE8eaVzXgH-Q



LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
T - A	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
T - B	2x4	DRY	No.2.	SPF
DRY: SEASONED LUMBER.				

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	4.0	4.0		
B	TMWW-t	MT20	5.0	6.0	2.50	2.75
C	TTWW+m	MT20	5.0	8.0		Edge
D	TMW-w	MT20	3.0	8.0		
E	TS-t	MT20	5.0	6.0		
F	TMWW-t	MT20	5.0	6.0		
G	TTWW+m	MT20	5.0	6.0	2.25	2.25
H	TTWW+m	MT20	5.0	6.0	3.25	2.50
I	TTW-t	MT20	4.0	6.0		
J	TMW-t	MT20	5.0	8.0		
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	12.0		
N, P, S						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	8.0		
T	BMWW-t	MT20	5.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
T	2058	0	2058	0
L	2183	0	2183	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
T	COMBINED	SNOW	LIVE	WIND	DEAD	SOIL
L	1455	955 / 0	0 / 0	0 / 0	500 / 0	0 / 0
L	1541	1025 / 0	0 / 0	0 / 0	517 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 16	-91.8	-91.8 0.20 (1)	10.00	B-S	0 / 109	
B-C	-3092 / 0	-91.8	-91.8 0.34 (1)	3.70	S-C	0 / 115	
C-D	-4026 / 0	-91.8	-91.8 0.37 (1)	4.01	N-G	0 / 336	
D-E	-4026 / 0	-91.8	-91.8 0.38 (1)	4.00	N-H	-253 / 0	
E-F	-4026 / 0	-91.8	-91.8 0.38 (1)	4.00	M-I	0 / 1058	
F-G	-4121 / 0	-91.8	-91.8 0.38 (1)	3.63	T-B	-3243 / 0	
G-H	-3354 / 0	-91.8	-91.8 0.26 (1)	3.98	M-J	0 / 2734	
H-I	-2755 / 0	-91.8	-91.8 0.10 (1)	4.13	H-M	-1502 / 0	
I-J	-3024 / 0	-91.8	-91.8 0.61 (1)	3.45	P-Q	0 / 1353	
J-K	0 / 28	-91.8	-91.8 0.12 (1)	10.00	C-Q	0 / 1550	
T-A	-145 / 0	0.0	0.0 0.01 (1)	7.81	P-F	-641 / 0	
L-J	-2117 / 0	0.0	0.0 0.14 (1)	6.99	Q-D	-700 / 0	
					Q-F	-114 / 0	
T-S	0 / 2663	-18.5	-18.5 0.38 (1)	10.00			
S-R	0 / 2749	-18.5	-18.5 0.38 (1)	10.00			
R-Q	0 / 2749	-18.5	-18.5 0.38 (1)	10.00			
Q-P	0 / 4120	-18.5	-18.5 0.55 (1)	10.00			
P-O	0 / 3006	-18.5	-18.5 0.40 (1)	10.00			
O-N	0 / 3008	-18.5	-18.5 0.40 (1)	10.00			
N-M	0 / 3177	-18.5	-18.5 0.44 (1)	10.00			
M-L	0 / 0	-18.5	-18.5 0.08 (1)	10.00			



Structural component only
DWG# T-2109006

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF CBC 2012 (2015 AMENDMENT)
- CSA 086-14
- TP10 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.24")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.24")
CALCULATED VERT. DEFL.(TL) = L/999 (0.40")

CSI: TC=0.51/1.00 (I-J:1), BC=0.55/1.00 (P-Q:1), WB=0.82/1.00 (B-T:1), SS=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

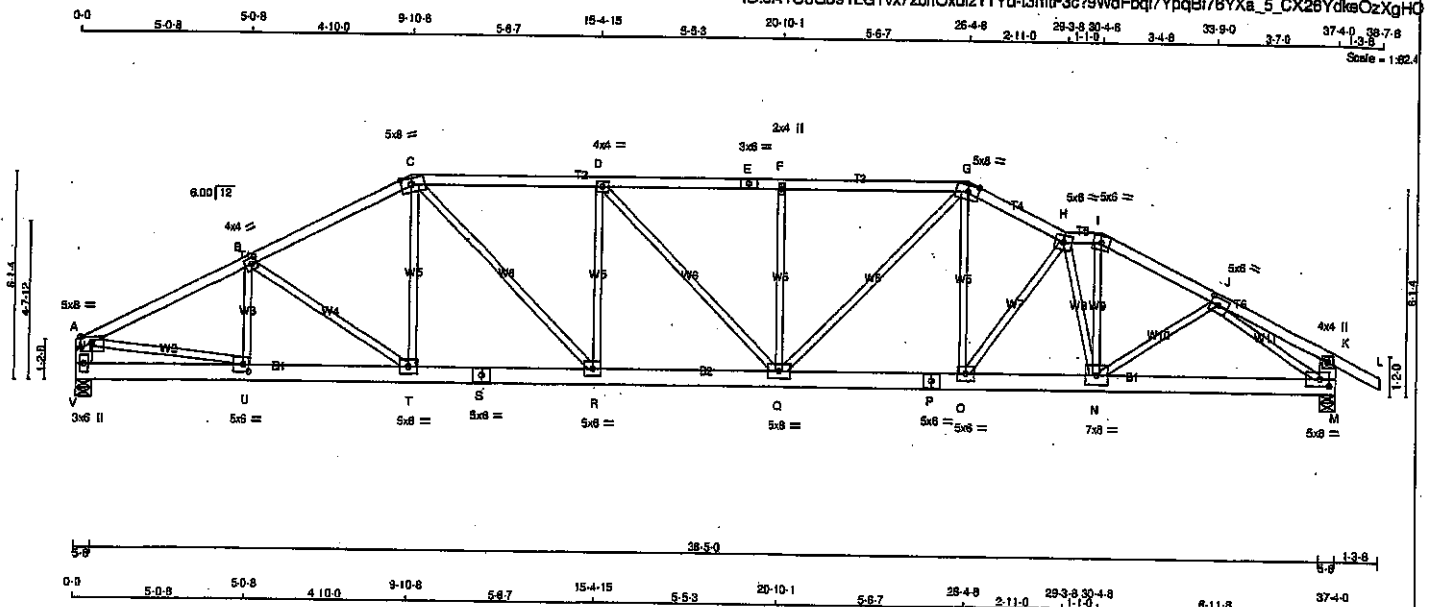
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (I) (INPUT = 0.90)
JSI METAL = 0.74 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417566	T47	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: eAYS6Go91LGYvx7zbnOxdizY1Yd-13mtF3c79WdFbq7YpqBf76YXa_5_CX28YdksOzXgHO



TOTAL WEIGHT = 180 lb

LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - A	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TTWW-m	MT20	5.0	8.0	2.25	3.50
D	TMVW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMVW-w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	2.25	3.75
H	TTWW-m	MT20	5.0	8.0		
I	TTWW-m	MT20	5.0	6.0		
J	TMVW-t	MT20	5.0	6.0	2.50	2.75
K	TMV-p	MT20	4.0	4.0		
M	BMVW-t	MT20	5.0	8.0	2.50	3.25
N	BMVWW-t	MT20	7.0	8.0		
O	R-T					
Q	BMVW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMVWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
U	BMVW-t	MT20	5.0	8.0	2.50	2.00
V	BMV-t-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REQRD	
JT	VERT	JT	DOWN	JT	BRG
V	2058	V	2058	V	5-8
M	2183	M	2183	M	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1455	855	0	0	0	500	0
M	1541	1025	0	0	0	517	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM TO		FR-TO			
A-B	-3080 / 0	-91.8 -91.8	0.42 (1)	U-B	-378 / 0	0.08 (1)	
B-C	-2855 / 0	-91.8 -91.8	0.39 (1)	B-T	-186 / 0	0.11 (1)	
C-D	-3298 / 0	-91.8 -91.8	0.49 (1)	T-C	0 / 238	0.05 (1)	
D-E	-3351 / 0	-91.8 -91.8	0.46 (1)	C-R	0 / 969	0.22 (1)	
E-F	-3351 / 0	-91.8 -91.8	0.45 (1)	R-D	-603 / 0	0.33 (1)	
F-G	-3351 / 0	-91.8 -91.8	0.49 (1)	D-Q	0 / 77	0.02 (1)	
G-H	-8114 / 0	-91.8 -91.8	0.20 (1)	Q-F	-543 / 0	0.30 (1)	
H-I	-2762 / 0	-91.8 -91.8	0.09 (1)	F-G	0 / 807	0.18 (1)	
I-J	-3071 / 0	-91.8 -91.8	0.23 (1)	G-O	0 / 475	0.11 (1)	
J-K	0 / 12	-91.8 -91.8	0.13 (1)	O-I	0 / 1169	0.26 (1)	
K-L	0 / 28	-91.8 -91.8	0.12 (1)	I-N	0 / 194	0.04 (1)	
V-A	-1996 / 0	0.0 0.0	0.13 (1)	A-U	0 / 2808	0.83 (1)	
M-K	-258 / 0	0.0 0.0	0.02 (1)	U-M	-3175 / 0	0.90 (1)	
V-U	0 / 0	-18.5 -18.5	0.07 (1)	M-H	-1220 / 0	0.38 (1)	
U-T	0 / 2775	-18.5 -18.5	0.39 (1)	H-O	-457 / 0	0.20 (1)	
T-S	0 / 2625	-18.5 -18.5	0.35 (1)				
S-R	0 / 2625	-18.5 -18.5	0.35 (1)				
R-Q	0 / 3258	-18.5 -18.5	0.44 (1)				
Q-P	0 / 2790	-18.5 -18.5	0.37 (1)				
P-O	0 / 2790	-18.5 -18.5	0.37 (1)				
O-N	0 / 3044	-18.5 -18.5	0.38 (1)				
N-M	0 / 2575	-18.5 -18.5	0.38 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.24")
CALCULATED VERT. DEFL. (LL) = L/999 (0.19")
ALLOWABLE DEFL. (TL) = L/360 (1.24")
CALCULATED VERT. DEFL. (TL) = L/999 (0.35")

CSI: TC=0.49/1.00 (F-G:1), BC=0.44/1.00 (Q-R:1), WB=0.90/1.00 (J-M:1), SS=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

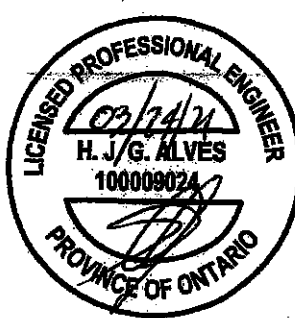
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	850 371	1747 788 1987 1879

PLATE PLACEMENT TOL = 0.260 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.73 (J) (INPUT = 1.00)



Structural component only
DWG# T-2109007

Structural component only
DWG# T-2109008

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417566	T48	1	3	GREENPARK HOMES	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Wed Mar 24 17:18:30 2021 Page 2
ID:AYS6Go91LGYvz7znOxdizY1Yd-LFKFTPadwpl6D KJ6WLQCKfmi K7im6BLCNIQgzXqfH

PLATES (table is in inches)

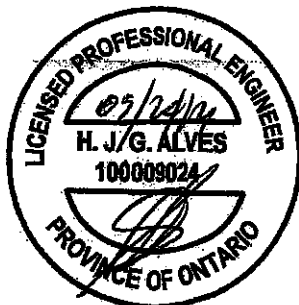
JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-m	MT20	10.0	12.0	4.75	1.50
B, D, G, I	BMWW-t	MT20	5.0	6.0		
C	TTWWW-m	MT20	8.0	9.0	3.25	4.25
E	BMW-w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	9.0		
H	TTWWW-m	MT20	8.0	9.0	3.25	4.25
J	TMBMW1-m	MT20	10.0	12.0	4.75	1.50
K	BMWW-t	MT20	8.0	9.0		
L	BMWW-t	MT20	8.0	12.0		
M	BMW-w	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	6.0	9.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	8.0	9.0		
R	BMWW-t	MT20	5.0	6.0		
S	BMW-w	MT20	3.0	6.0		
T	BMWW-t	MT20	8.0	12.0		
U	BMWW-t	MT20	8.0	9.0		

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

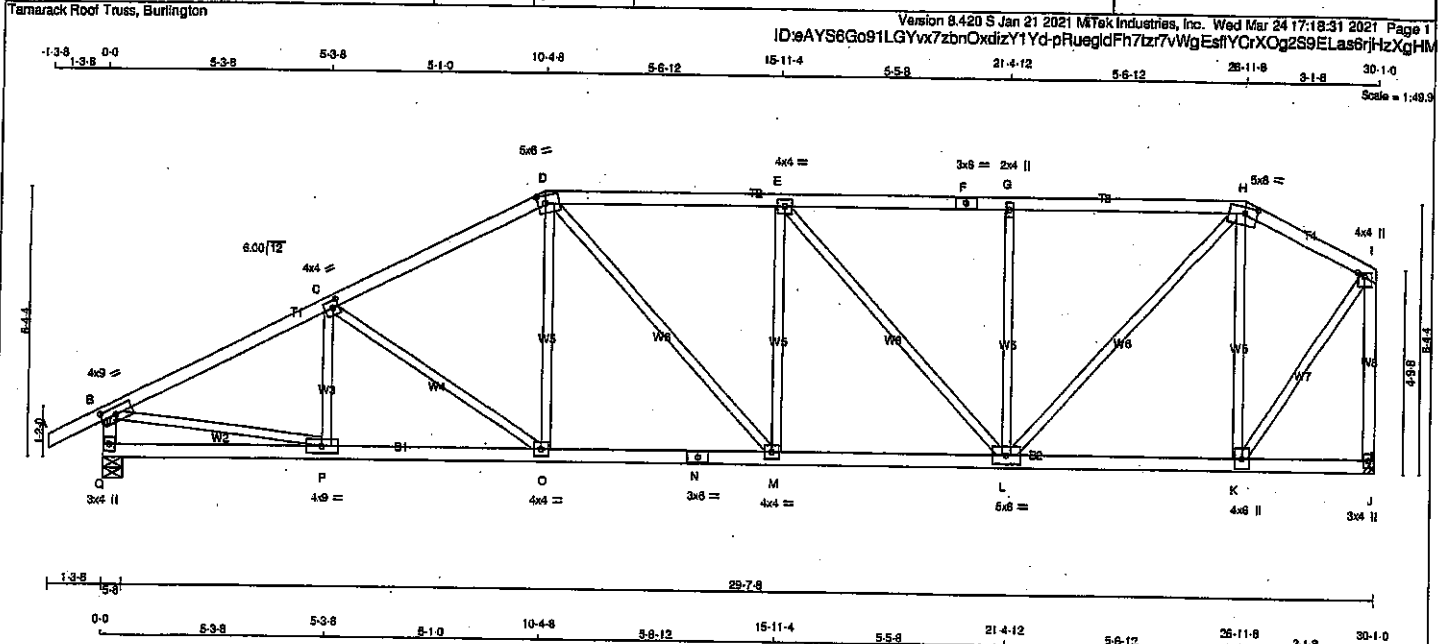
NOTES: (1)

- 1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2109008 *m*

JOB NAME 417566	TRUSS NAME T49	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESO.		Version 8.420 S Jan 21 2021 M/Tek Industries, Inc. Wed Mar 24 17:18:31 2021 Page 1	



LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - I	2x4	DRY	No.2	SPF	
Q - B	2x4	DRY	No.2	SPF	
J - I	2x4	DRY	No.2	SPF	
Q - N	2x4	DRY	No.2	SPF	
N - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWV-t	MT20	4.0	9.0	2.00 4.00
C	TMWV-t	MT20	4.0	4.0	2.00 1.75
D	TTWV-m	MT20	5.0	6.0	2.25 2.00
E	TMWV-t	MT20	4.0	4.0	
F	TS-t	MT20	3.0	6.0	
G	TMW-w	MT20	2.0	4.0	
H	TTWV-m	MT20	5.0	8.0	1.75 3.50
I	TMWV-p	MT20	4.0	4.0	1.25 2.00
J	BMV1-p	MT20	3.0	4.0	
K	BMWV-t	MT20	4.0	8.0	
L	BMWV-t	MT20	5.0	8.0	
M	BMWV-t	MT20	4.0	4.0	
N	BS-t	MT20	3.0	6.0	
O	BMWV-t	MT20	4.0	4.0	
P	BMWV-t	MT20	4.0	4.0	
Q	BMV1-p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
JT 1783	0	1783	0	0	5-8	5-8	
Q 1658	0	1658	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

1ST LOASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	1258	839 / 0	0 / 0	0 / 0	0 / 0	419 / 0	0 / 0
J	1173	770 / 0	0 / 0	0 / 0	0 / 0	403 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	P-C	-250 / 0	0.06 (1)
B-C	-2393 / 0	-91.8 -91.8	0.55 (1)	3.90	C-D	-347 / 0	0.24 (1)
C-D	-2111 / 0	-91.8 -91.8	0.51 (1)	4.13	D-E	0 / 297	0.07 (1)
D-E	-2101 / 0	-91.8 -91.8	0.49 (1)	4.17	E-F	0 / 342	0.08 (1)
E-F	-1740 / 0	-91.8 -91.8	0.46 (1)	4.53	F-G	-151 / 36	0.10 (1)
F-G	-1740 / 0	-91.8 -91.8	0.46 (1)	4.53	G-H	-540 / 0	0.75 (1)
G-H	-1740 / 0	-91.8 -91.8	0.45 (1)	4.54	H-I	-550 / 0	0.38 (1)
H-I	-887 / 0	-91.8 -91.8	0.18 (1)	8.25	I-J	0 / 1426	0.32 (1)
I-J	-1739 / 0	0.0 0.0	0.18 (1)	8.32	J-K	-1005 / 0	0.65 (1)
J-K	-1641 / 0	0.0 0.0	0.65 (1)	6.47	K-L	0 / 1365	0.49 (1)
					L-M	0 / 1365	0.30 (1)
Q-P	0 / 0	-18.5 -18.5	0.11 (4)	10.00			
P-O	0 / 2152	-18.5 -18.5	0.40 (1)	10.00			
O-N	0 / 1870	-18.5 -18.5	0.36 (1)	10.00			
N-M	0 / 1870	-18.5 -18.5	0.36 (1)	10.00			
M-L	0 / 2101	-18.5 -18.5	0.39 (1)	10.00			
L-K	0 / 775	-18.5 -18.5	0.18 (1)	10.00			
K-J	0 / 0	-18.5 -18.5	0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.00")
CALCULATED VERT. DEFL. (LL) = L/999 (0.10")
ALLOWABLE DEFL. (TL) = L/360 (1.00")
CALCULATED VERT. DEFL. (TL) = L/999 (0.10")

CSI: TC=0.65/1.00 (I-J-I), EC=0.40/1.00 (O-P-I), WB=0.75/1.00 (E-L-I), SSI=0.24/1.00 (D-E-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1997 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

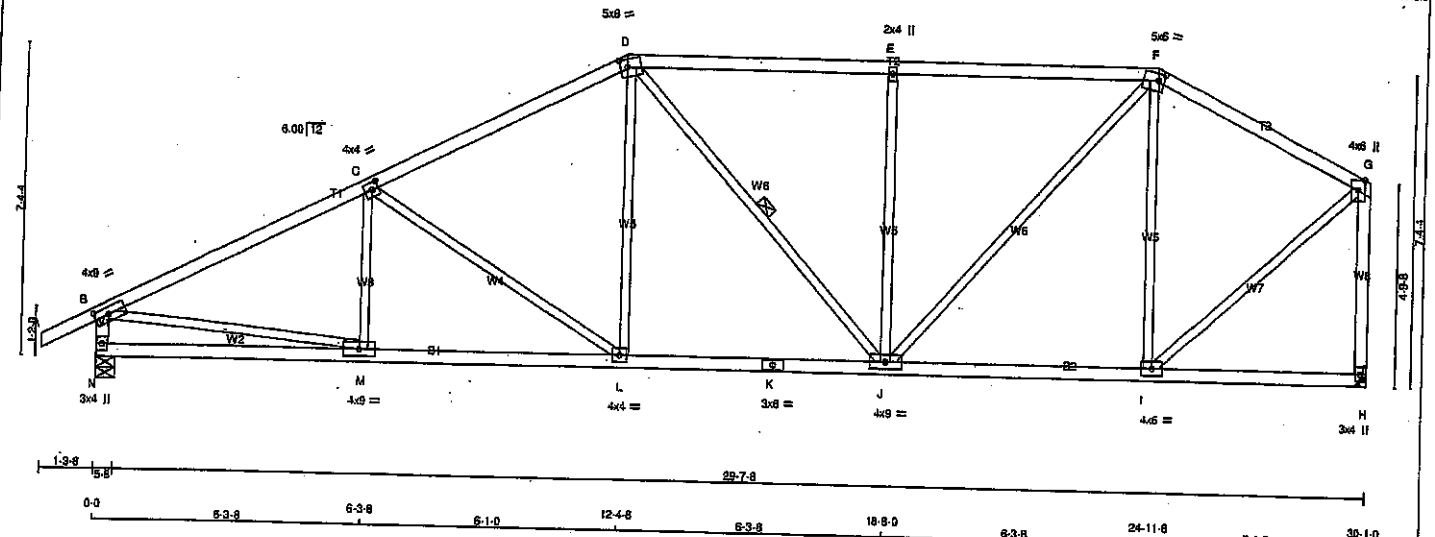
JSI GRIP = 0.90 (H) (INPUT = 0.90)
JSI METAL = 0.58 (N) (INPUT = 1.00)



Structural component only
DWG# T-2109009

JOB NAME 417566	TRUSS NAME T50	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:AYS6G091LGYvx7zbnOxdizY1Yd-HeS014eISR?gSHUIExNvHik0On0vBcxUoWSPFzXgHL
Scale = 1:49.9



TOTAL WEIGHT = 129 lb
[MIF]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DRY
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	9.0	2.00	4.00
C	TMVW-1	MT20	4.0	4.0	2.00	1.75
D	TTVW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTVW-m	MT20	5.0	6.0	2.00	1.75
G	TMVW-p	MT20	4.0	6.0	Edge	
H	BMV1-p	MT20	3.0	4.0		
I	BMVW-1	MT20	4.0	6.0		
J	BMVW-1	MT20	4.0	9.0		
K	BS-1	MT20	3.0	6.0		
L	BMVW-1	MT20	4.0	4.0		
M	BMVW-1	MT20	4.0	9.0		
N	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	1783	0	1783	0
N	HORIZ	0	1783	0	0
H	UPLIFT	1658	0	1658	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	DEAD	SOIL
N	COMBINED	839'0	0'0	0'0
H	SNOW	770'0	0'0	0'0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 28	M-C	-175 / 43
B-C	-2405 / 0	C-L	-551 / 0
C-D	-1950 / 0	L-D	0 / 416
D-E	-1732 / 0	D-J	-9 / 18
E-F	-1732 / 0	J-E	-712 / 0
F-G	-1142 / 0	E-F	0 / 1086
N-B	-1733 / 0	F-I	-755 / 0
H-G	-1622 / 0	B-M	0 / 2198
		I-G	0 / 1344
N-M	0 / 0		
M-L	0 / 2178		
L-K	0 / 1722		
K-J	0 / 1722		
J-I	0 / 1009		
I-H	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(65% OF 91.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.00")
CALCULATED VERT. DEFL. (LL) = L/999 (0.09")
ALLOWABLE DEFL. (TL) = L/360 (1.00")
CALCULATED VERT. DEFL. (TL) = L/999 (0.18")

CSI: TO=0.64/1.00 (G-H:1), BC=0.43/1.00 (L-M:1), WB=0.73/1.00 (F-I:1), SS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

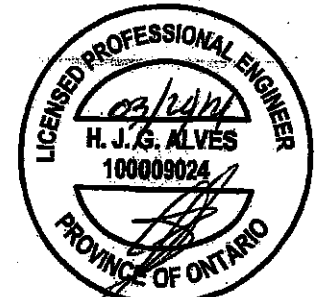
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (F) (INPUT = 0.90)
JSI METAL = 0.58 (G) (INPUT = 1.00)

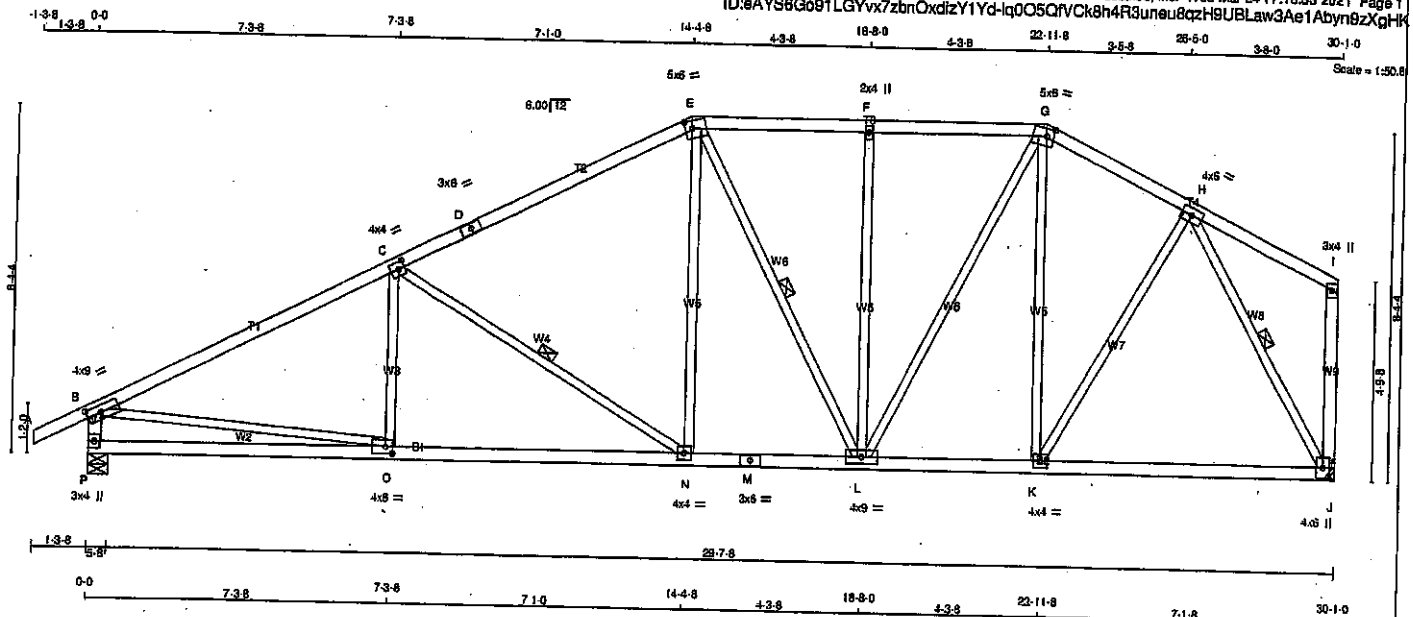


Structural component only
DWG# T-2109010

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417566	T51	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 139 lb
[M/F]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
E - G	2x4 DRY	No.2	SPF		
H - I	2x4 DRY	No.2	SPF		
J - L	2x4 DRY	No.2	SPF		
M - N	2x4 DRY	No.2	SPF		
O - P	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	2.00	4.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMVW-w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TMVW-t	MT20	4.0	6.0		
I	TMV-p	MT20	3.0	4.0		
J	BMVW1-p	MT20	4.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMVW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMVW-t	MT20	4.0	6.0	2.00	2.00
P	BMV1-p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
P	1783	0	1783	0
J	1658	0	1658	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3.8.

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1258	839 / 0	0 / 0	0 / 0	0 / 0	419 / 0	0 / 0
J	1173	770 / 0	0 / 0	0 / 0	0 / 0	403 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-N, E-L, H-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	W E B S	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MAX. UNBRACED LENGTH	FORCE
	(LBS)	(PLF)	CS1 (LC)	(LBS)
FR-TO	FROM TO			
A-B	0 / 28	-91.8	-91.8	10.00
B-C	-2402 / 0	-91.8	-91.8	3.50
C-D	-1766 / 0	-91.8	-91.8	4.11
D-E	-1766 / 0	-91.8	-91.8	4.11
E-F	-1481 / 0	-91.8	-91.8	5.15
F-G	-1481 / 0	-91.8	-91.8	5.15
G-H	-1289 / 0	-91.8	-91.8	5.52
H-I	0 / 17	-91.8	-91.8	10.00
P-B	-1727 / 0	0.0	0.0	6.33
J-I	-127 / 0	0.0	0.0	7.81
P-O	0 / 0	-18.5	-18.5	10.00
O-N	0 / 2180	-18.5	-18.5	10.00
N-M	0 / 1553	-18.5	-18.5	10.00
M-L	0 / 1553	-18.5	-18.5	10.00
L-K	0 / 1132	-18.5	-18.5	10.00
K-J	0 / 845	-18.5	-18.5	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL	LD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF CBC 2012 (2015 AMENDMENT)
- CSA 086-14
- TPIC-2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.00")
CALCULATED VERT. DEFL. (LL) = L/999 (0.09")
ALLOWABLE DEFL. (TL) = L/360 (1.00")
CALCULATED VERT. DEFL. (TL) = L/999 (0.20")

CS1: TC=0.81/1.00 (B-C:1), BC=0.48/1.00 (N-O:1), WB=0.68/1.00 (F-L:1), SS1=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

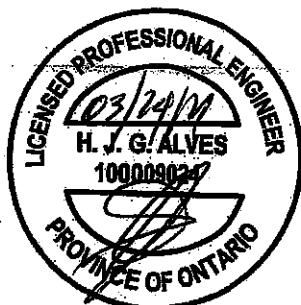
PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	850	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

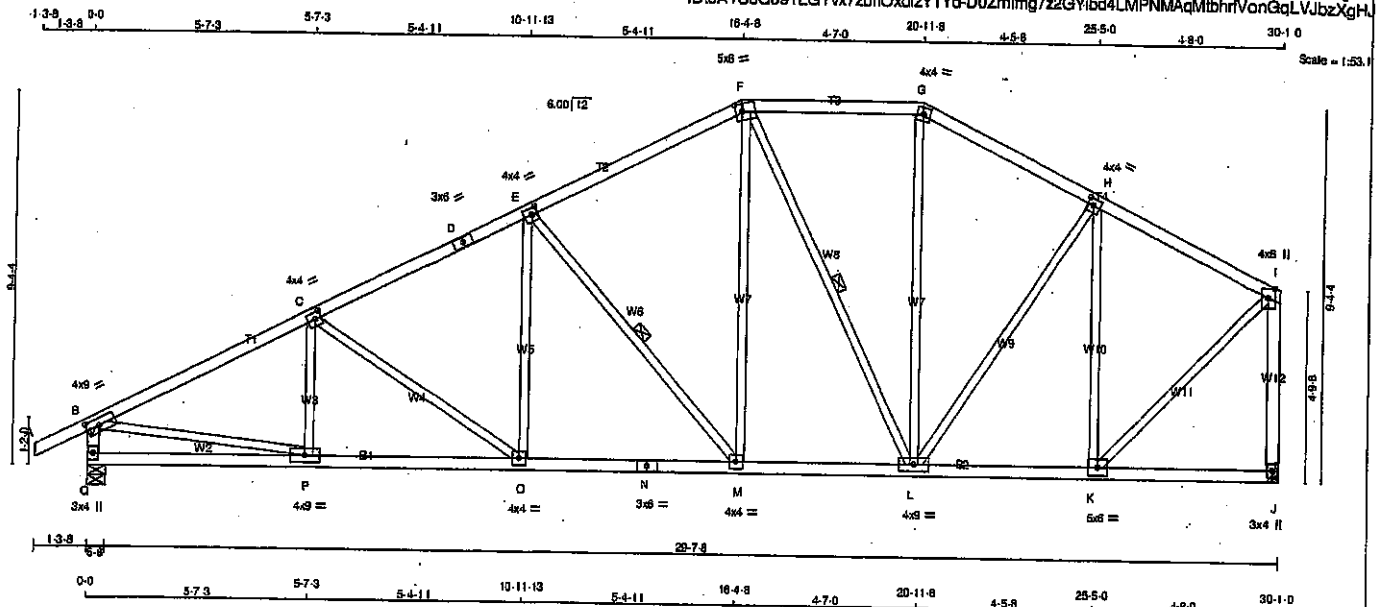
JSI GRIP= 0.89 (O) (INPUT = 0.90)

JSI METAL= 0.50 (O) (INPUT = 1.00)



Structural component only
DWG# T-2109011

JOB NAME 417566	TRUSS NAME T52	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
J - I	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

PLATES (table in inches)	TYPE	PLATES	W	LEN	Y	X
JT	TMVW-t	MT20	4.0	9.0	2.00	4.00
B	TMVW-t	MT20	4.0	9.0	2.00	1.75
C	TS-t	MT20	3.0	8.0		
D	TTW-m	MT20	5.0	8.0	2.25	2.00
F	TTW-m	MT20	4.0	4.0		
G	TMVW-p	MT20	4.0	6.0	Edge	
I	BMV1-p	MT20	3.0	4.0		
J	BMVW-t	MT20	5.0	6.0		
K	BMVW-t	MT20	4.0	9.0		
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BS-t	MT20	4.0	4.0		
O	BMVW-t	MT20	4.0	9.0		
P	BMVW-t	MT20	4.0	9.0		
Q	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	JT VERT	IN-SX	IN-SX
Q 1783	0	1783	0
J 1658	0	1658	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	1256	839 / 0	0 / 0	0 / 0	0 / 0	419 / 0	0 / 0
J	1173	770 / 0	0 / 0	0 / 0	0 / 0	403 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.09 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M, F-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	P-C	-222 / 14	0.05 (1)	
B-C	-2379 / 0	-91.8	-91.8	0.44 (1)	4.06	C-O	-337 / 0	0.26 (1)	
C-D	-2088 / 0	-91.8	-91.8	0.35 (1)	4.38	O-E	0 / 292	0.07 (1)	
D-E	-2088 / 0	-91.8	-91.8	0.35 (1)	4.38	E-M	-771 / 0	0.37 (1)	
E-F	-1548 / 0	-91.8	-91.8	0.34 (1)	4.94	M-F	0 / 675	0.15 (1)	
F-G	-1173 / 0	-91.8	-91.8	0.26 (1)	5.58	F-L	-447 / 0	0.38 (1)	
G-H	-1332 / 0	-91.8	-91.8	0.26 (1)	5.74	L-G	0 / 224	0.05 (1)	
H-I	-1094 / 0	-91.8	-91.8	0.26 (1)	5.74	L-H	0 / 309	0.07 (1)	
Q-B	-1737 / 0	0.0	0.0	0.18 (1)	6.32	K-H	-864 / 0	0.77 (1)	
J-I	-1623 / 0	0.0	0.0	0.65 (1)	6.49	B-P	0 / 2172	0.49 (1)	
Q-P	0 / 0	-18.5	-18.5	0.12 (4)	10.00	K-I	0 / 1384	0.31 (1)	
P-O	0 / 2147	-18.5	-18.5	0.40 (1)	10.00				
O-N	0 / 1868	-18.5	-18.5	0.35 (1)	10.00				
N-M	0 / 1868	-18.5	-18.5	0.35 (1)	10.00				
M-L	0 / 1371	-18.5	-18.5	0.27 (1)	10.00				
L-K	0 / 998	-18.5	-18.5	0.21 (1)	10.00				
K-J	0 / 0	-18.5	-18.5	0.10 (4)	10.00				

TOTAL WEIGHT = 142 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 6.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBCG 2015, ABC 2019
- PART 9 OF CBCG 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.00")
CALCULATED VERT. DEFL. (LL) = L/999 (0.09")
ALLOWABLE DEFL. (TL) = L/360 (1.00")
CALCULATED VERT. DEFL. (TL) = L/999 (0.17")

CSI: TC=0.65/1.00 (I-I), BC=0.40/1.00 (O-P), WS=0.77/1.00 (H-K), SS=0.21/1.00 (B-C)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

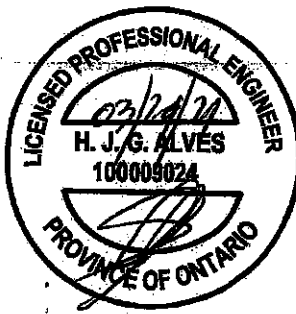
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
		788	1997
		1873	

PLATE PLACEMENT TOL = 0.250 inches

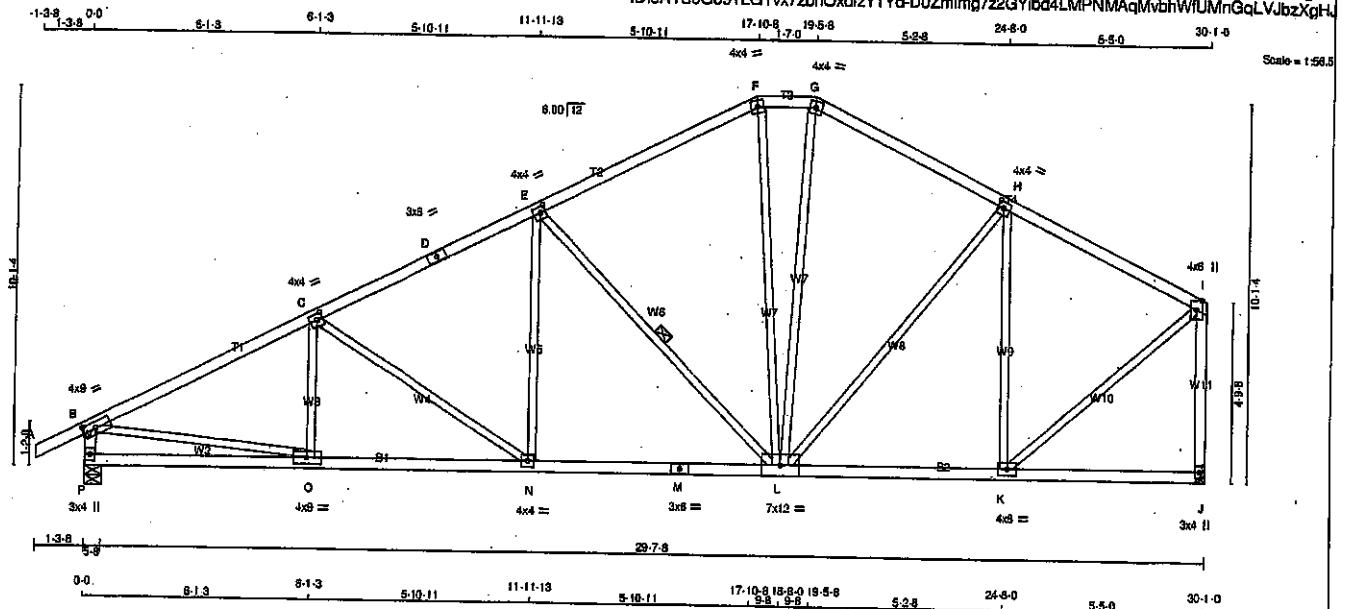
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.87 (B) (INPUT = 0.90)
JSI METAL = 0.81 (N) (INPUT = 1.00)



Structural component only
DWG# T-2109012

JOB NAME 417566	TRUSS NAME T53	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
J - B	2x4	DRY	No.2
P - I	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	9.0	2.00 4.00
C, E, H					
C	TMVW-t	MT20	4.0	4.0	2.00 1.75
D	TS-t	MT20	3.0	6.0	
F	TTW-m	MT20	4.0	4.0	
G	TTW-m	MT20	4.0	4.0	
I	TMVW-p	MT20	4.0	6.0	Edge
J	BMV1-p	MT20	3.0	4.0	
K	BMVW-t	MT20	4.0	6.0	
L	BMVWW-t	MT20	7.0	12.0	
M	BS-t	MT20	3.0	6.0	
N	BMVW-t	MT20	4.0	4.0	
O	BMVW-t	MT20	4.0	9.0	
P	BMV1-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
P	1783	0	1783	0	0	5-8	5-8		
J	1658	0	1658	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1258	839 / 0	0 / 0	0 / 0	0 / 0	419 / 0	0 / 0
J	1173	770 / 0	0 / 0	0 / 0	0 / 0	403 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3-96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	CSI (LC)	FR-TO			CSI (LC)
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	C-C	-188 / 33	0.05 (1)
B-C	-2387 / 0	-91.8	-91.8 0.62 (1)	3.96	C-N	-428 / 0	0.41 (1)
C-D	-2011 / 0	-91.8	-91.8 0.42 (1)	4.38	N-E	0 / 353	0.08 (1)
D-E	-2011 / 0	-91.8	-91.8 0.42 (1)	4.38	E-L	-899 / 0	0.57 (1)
E-F	-1341 / 0	-91.8	-91.8 0.40 (1)	5.13	L-H	0 / 155	0.03 (1)
F-G	-1202 / 0	-91.8	-91.8 0.04 (1)	5.80	K-H	-778 / 0	0.79 (1)
G-H	-1341 / 0	-91.8	-91.8 0.36 (1)	5.18	H-O	0 / 2177	0.49 (1)
H-I	-1178 / 0	-91.8	-91.8 0.35 (1)	5.46	K-I	0 / 1396	0.31 (1)
P-B	-1734 / 0	0.0	0.0 0.18 (1)	6.32	F-L	0 / 302	0.07 (1)
J-I	-1618 / 0	0.0	0.0 0.64 (1)	6.50	L-G	0 / 339	0.08 (1)
P-O	0 / 0	-18.5	-18.5 0.14 (4)	10.00			
O-N	0 / 2156	-18.5	-18.5 0.42 (1)	10.00			
N-M	0 / 1789	-18.5	-18.5 0.36 (1)	10.00			
M-L	0 / 1789	-18.5	-18.5 0.36 (1)	10.00			
L-K	0 / 1075	-18.5	-18.5 0.26 (1)	10.00			
K-J	0 / 0	-18.5	-18.5 0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.00")
CALCULATED VERT. DEFL. (LL) = L/999 (0.08")
ALLOWABLE DEFL. (TL) = L/360 (1.00")
CALCULATED VERT. DEFL. (TL) = L/999 (0.18")

CSI: TC=0.64/1.00 (H-J), BC=0.42/1.00 (N-O), WB=0.79/1.00 (H-K), SS=0.23/1.00 (B-C)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

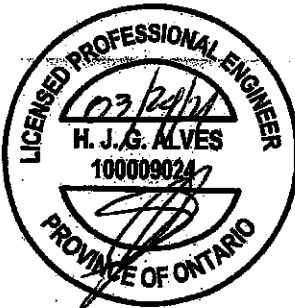
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

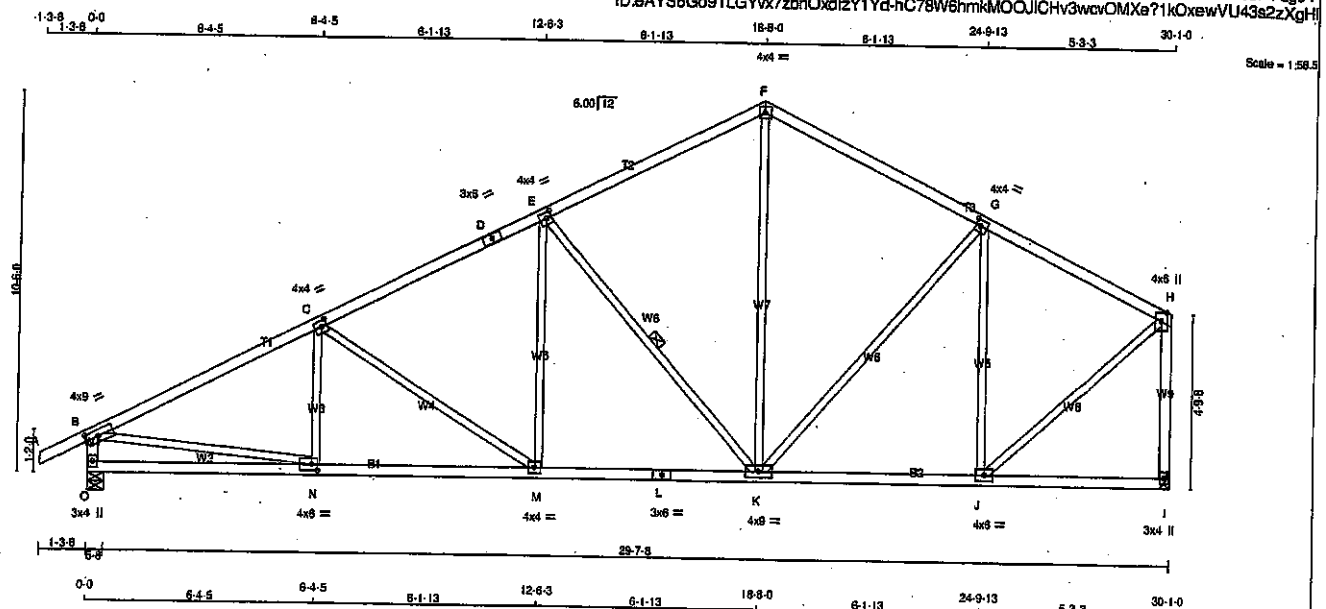
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (I) (INPUT = 0.90)
JSI METAL = 0.61 (M) (INPUT = 1.00)



Structural component only
DWG# T-2109013

JOB NAME 417566	TRUSS NAME T54	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-1	MT20	4.0	9.0	2.00	4.00
C E-G					
C TMVW-1	MT20	4.0	4.0	2.00	1.75
D TS-1	MT20	3.0	6.0		
F TTW-p	MT20	4.0	4.0		
H TMVW-p	MT20	4.0	6.0	Edge	
I BMV1-p	MT20	3.0	4.0		
J BMVW-1	MT20	4.0	6.0		
K BMVW-1	MT20	4.0	9.0		
L BS-1	MT20	3.0	6.0		
M BMVW-1	MT20	4.0	4.0		
N BMVW-1	MT20	4.0	6.0	2.00	2.00
O BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
O	1783	0	1783	0
I	1858	0	1858	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
O	1258 839 0 0 0 0 0 0 0 0
I	1173 770 0 0 0 0 0 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0/28	-91.8	-91.8 0.12 (1)	10.00	N-C	-163/46	0.05 (1)
B-C	-2389/0	-91.8	-91.8 0.57 (1)	3.90	C-M	-479/0	0.51 (1)
C-D	-1996/0	-91.8	-91.8 0.45 (1)	4.38	M-E	0/374	0.08 (1)
D-E	-1996/0	-91.8	-91.8 0.45 (1)	4.38	E-K	-916/0	0.57 (1)
E-F	-1320/0	-91.8	-91.8 0.44 (1)	5.10	K-G	0/708	0.16 (1)
F-G	-1322/0	-91.8	-91.8 0.45 (1)	5.08	G-K	0/133	0.03 (1)
G-H	-1167/0	-91.8	-91.8 0.41 (1)	5.38	J-N	-798/0	0.79 (1)
O-B	-1733/0	0.0	0.0 0.18 (1)	8.32	B-N	0/2178	0.49 (1)
I-H	-1620/0	0.0	0.0 0.64 (1)	5.50	J-H	0/1408	0.32 (1)
O-N	0/0	-18.5	-18.5 0.16 (4)	10.00			
N-M	0/2158	-18.5	-18.5 0.42 (1)	10.00			
M-L	0/1758	-18.5	-18.5 0.36 (1)	10.00			
L-K	0/1758	-18.5	-18.5 0.36 (1)	10.00			
K-J	0/1072	-18.5	-18.5 0.25 (1)	10.00			
J-I	0/0	-18.5	-18.5 0.14 (4)	10.00			

TOTAL WEIGHT = 3 X 133 = 400 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/960 (1.00")
CALCULATED VERT. DEFL. (LL) = L/999 (0.09")
ALLOWABLE DEFL. (TL) = L/960 (1.00")
CALCULATED VERT. DEFL. (TL) = L/999 (0.18")

CSI: TO=0.64/1.00 (H=1), BC=0.42/1.00 (M=N=1), WB=0.79/1.00 (G=J=1), SS=0.24/1.00 (B=C=1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 786 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

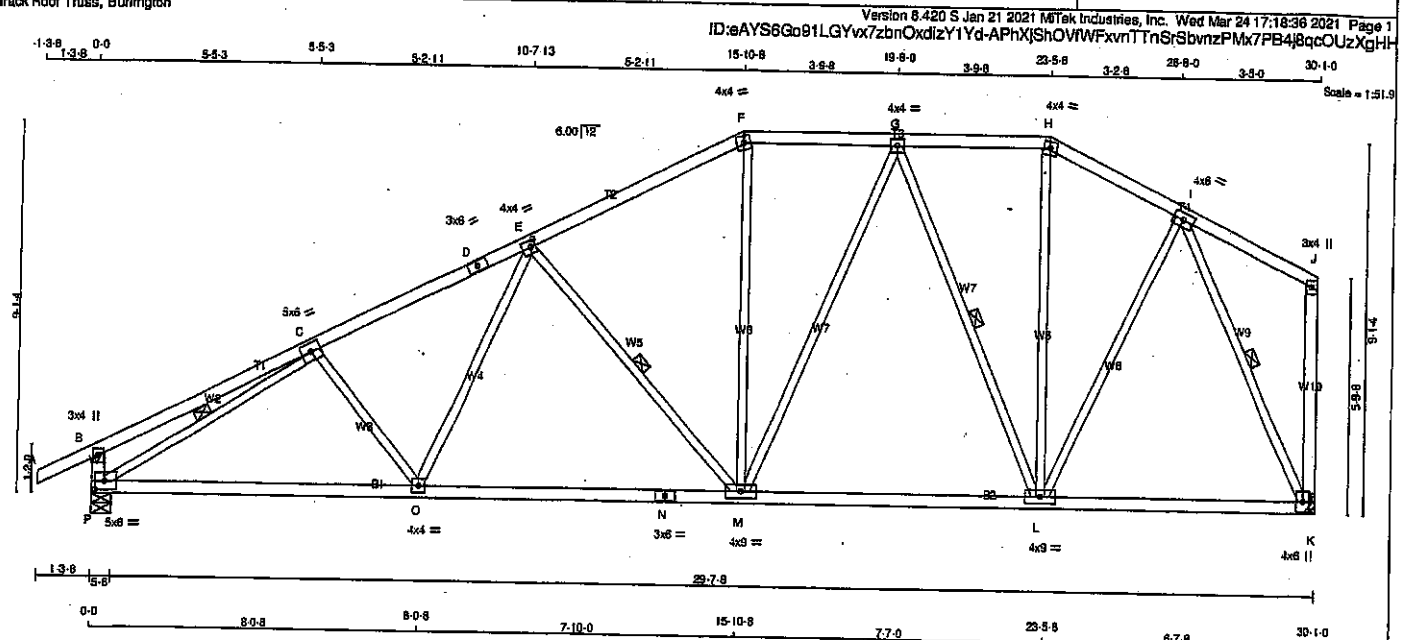
JSI GRIP = 0.90 (N) (INPUT = 0.90)

JSI METAL = 0.59 (H) (INPUT = 1.00)



Structural component only
DWG# T-2109014

JOB NAME 417566	TRUSS NAME T55	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
K - L	2x4	DRY	No.2	SPF
L - M	2x4	DRY	No.2	SPF
M - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
C	TMW-w	MT20	5.0	6.0		
D	TS-t	MT20	3.0	6.0		
E	TMW-w	MT20	4.0	4.0	2.00	1.50
F	TTW-m	MT20	4.0	4.0		
G	TMW-w	MT20	4.0	4.0		
H	TTW-m	MT20	4.0	4.0		
I	TMW-w	MT20	4.0	6.0		
J	TMV-p	MT20	3.0	4.0		
K	BMVW-i	MT20	4.0	6.0		
L	BMVW-w	MT20	4.0	9.0		
M	BMVW-w	MT20	4.0	9.0		
N	BS-t	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	5.0	6.0	2.50	2.50

NOTES - (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
P	1783	0	1783	0
K	1658	0	1658	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 3'-8".

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
P	1258	839	0	0	0	419
K	1173	770	0	0	0	403

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M, G-L, O-P, I-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	C-O	-173 / 15	0.06 (1)
B-C	0 / 20	-91.8	-91.8 0.32 (1)	10.00	O-E	0 / 328	0.07 (1)
C-D	-2244 / 0	-91.8	-91.8 0.35 (1)	4.26	E-M	-714 / 0	0.32 (1)
D-E	-2244 / 0	-91.8	-91.8 0.35 (1)	4.26	M-F	0 / 380	0.09 (1)
E-F	-1604 / 0	-91.8	-91.8 0.32 (1)	4.69	F-G	0 / 323	0.07 (1)
F-G	-1419 / 0	-91.8	-91.8 0.18 (1)	5.30	G-L	-781 / 0	0.51 (1)
G-H	-993 / 0	-91.8	-91.8 0.17 (1)	6.07	L-H	0 / 245	0.08 (1)
H-I	-1123 / 0	-91.8	-91.8 0.13 (1)	5.85	I-K	0 / 686	0.15 (1)
I-J	0 / 15	-91.8	-91.8 0.15 (1)	10.00	P-C	-2533 / 0	0.71 (1)
P-B	-325 / 0	0.0	0.0 0.03 (1)	7.81	I-K	-1844 / 0	0.67 (1)
K-J	-121 / 0	0.0	0.0 0.08 (1)	7.81			
P-O	0 / 2113	-18.5	-18.5 0.49 (1)	10.00			
O-N	0 / 1891	-18.5	-18.5 0.45 (1)	10.00			
N-M	0 / 1881	-18.5	-18.5 0.45 (1)	10.00			
M-L	0 / 1294	-18.5	-18.5 0.33 (1)	10.00			
L-K	0 / 697	-18.5	-18.5 0.27 (4)	10.00			

TOTAL WEIGHT = 142 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.00")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.09")
ALLOWABLE DEFL. (TL) = L/360 (1.00")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.19")

CSI: TC=0.35/1.00 (C-E:1), BC=0.49/1.00 (O-P:1), WB=0.71/1.00 (C-P:1), SI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	850 371	1747 788
	1987 1873	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

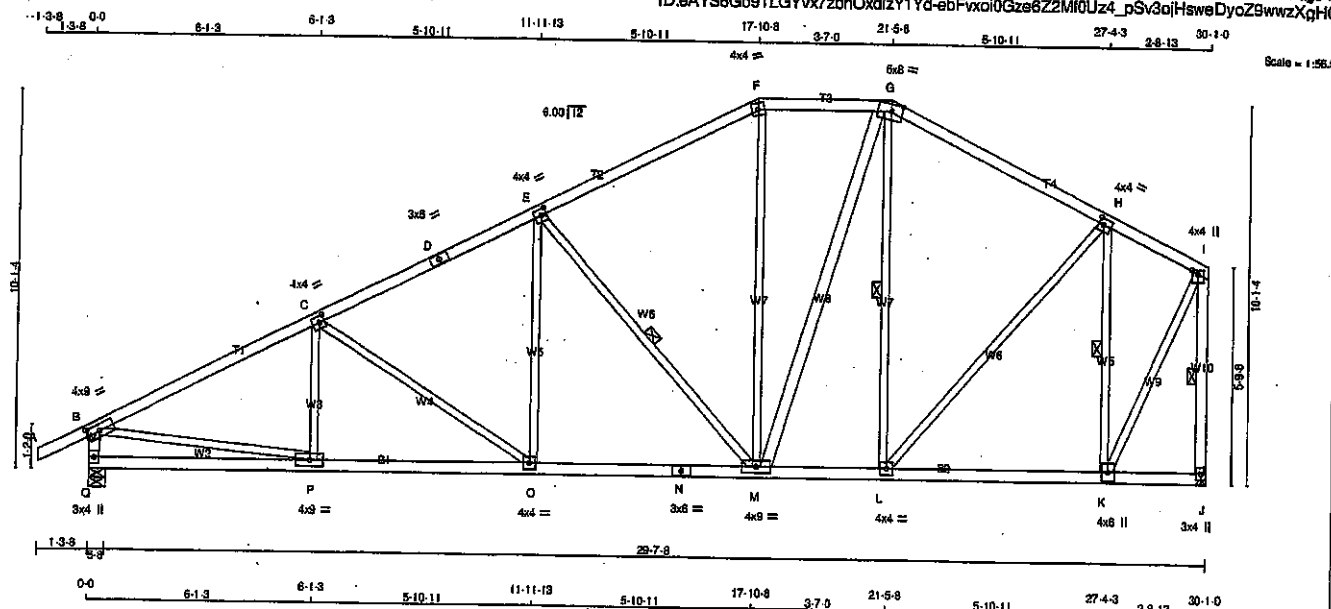
JSI GRIP = 0.90 (C) (INPUT = 0.90)
JSI METAL = 0.57 (N) (INPUT = 1.00)



Structural component only
DWG# T-2109015

JOB NAME 417566	TRUSS NAME T56	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 151 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	No.2	SPF		
F - G	2x4	DRY	No.2	SPF		
G - I	2x4	DRY	No.2	SPF		
Q - J	2x4	DRY	No.2	SPF		
J - I	2x4	DRY	No.2	SPF		
Q - N	2x4	DRY	No.2	SPF		
N - J	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						
M - G	2x4	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+	MT20	4.0	9.0	2.00	4.00
C TMVW+	MT20	4.0	4.0	2.00	1.75
D TS+	MT20	3.0	6.0		
E TMVW+	MT20	4.0	4.0	2.00	1.75
F TTW+	MT20	4.0	4.0		
G TTWV+	MT20	5.0	8.0	2.25	3.50
H TMVW+	MT20	4.0	4.0	2.00	1.50
I TMVW+	MT20	4.0	4.0	1.25	2.00
J BMV1+	MT20	3.0	4.0		
K BMVW+	MT20	4.0	8.0		
L BMVW+	MT20	4.0	4.0		
M BMVW+	MT20	4.0	9.0		
N BS+	MT20	3.0	6.0		
O BMVW+	MT20	4.0	4.0		
P BMVW+	MT20	4.0	9.0		
Q BMV1+	MT20	3.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	UPLIFT
Q	1793	0	1793	0
J	1658	0	1658	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	1258	899 / 0	0 / 0	0 / 0	0 / 0	419 / 0	0 / 0
J	1173	770 / 0	0 / 0	0 / 0	0 / 0	403 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M, G-L, H-K, I-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO									
A-B	0/28								
B-C	-2387/0	-91.8	-91.8	0.12 (1)	10.00	P-C	-185/34	0.05 (1)	
C-D	-2011/0	-91.8	-91.8	0.52 (1)	3.96	C-O	-429/0	0.41 (1)	
D-E	-2011/0	-91.8	-91.8	0.42 (1)	4.38	O-E	-0/351	0.08 (1)	
E-F	-1392/0	-91.8	-91.8	0.42 (1)	4.38	E-M	-874/0	0.50 (1)	
F-G	-1225/0	-91.8	-91.8	0.40 (1)	5.05	M-F	0/231	0.05 (1)	
G-H	-1198/0	-91.8	-91.8	0.17 (1)	5.62	M-G	0/538	0.09 (1)	
H-I	-679/0	-91.8	-91.8	0.30 (1)	6.25	L-G	-367/0	0.26 (1)	
Q-B	-1734/0	0.0	0.0	0.18 (1)	6.32	L-H	0/607	0.14 (1)	
J-I	-1847/0	0.0	0.0	0.28 (1)	5.16	K-H	-1207/0	0.39 (1)	
Q-P	0/0	-18.5	-18.5	0.15 (4)	10.00	B-P	0/2177	0.49 (1)	
P-O	0/2166	-18.5	-18.5	0.42 (1)	10.00	K-I	0/1454	0.33 (1)	
O-N	0/1798	-18.5	-18.5	0.36 (1)	10.00				
N-M	0/1798	-18.5	-18.5	0.36 (1)	10.00				
M-L	0/1045	-18.5	-18.5	0.23 (1)	10.00				
L-K	0/849	-18.5	-18.5	0.18 (1)	10.00				
K-J	0/0	-18.5	-18.5	0.11 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2015, ABC 2019
- PART 8 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.52/1.00 (B-C:1), BC=0.42/1.00 (O-P:1), WB=0.50/1.00 (E-M:1), SS=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 OOMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

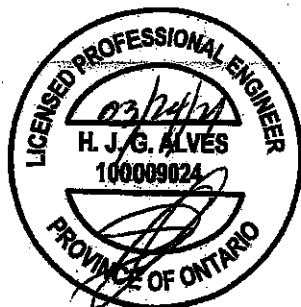
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

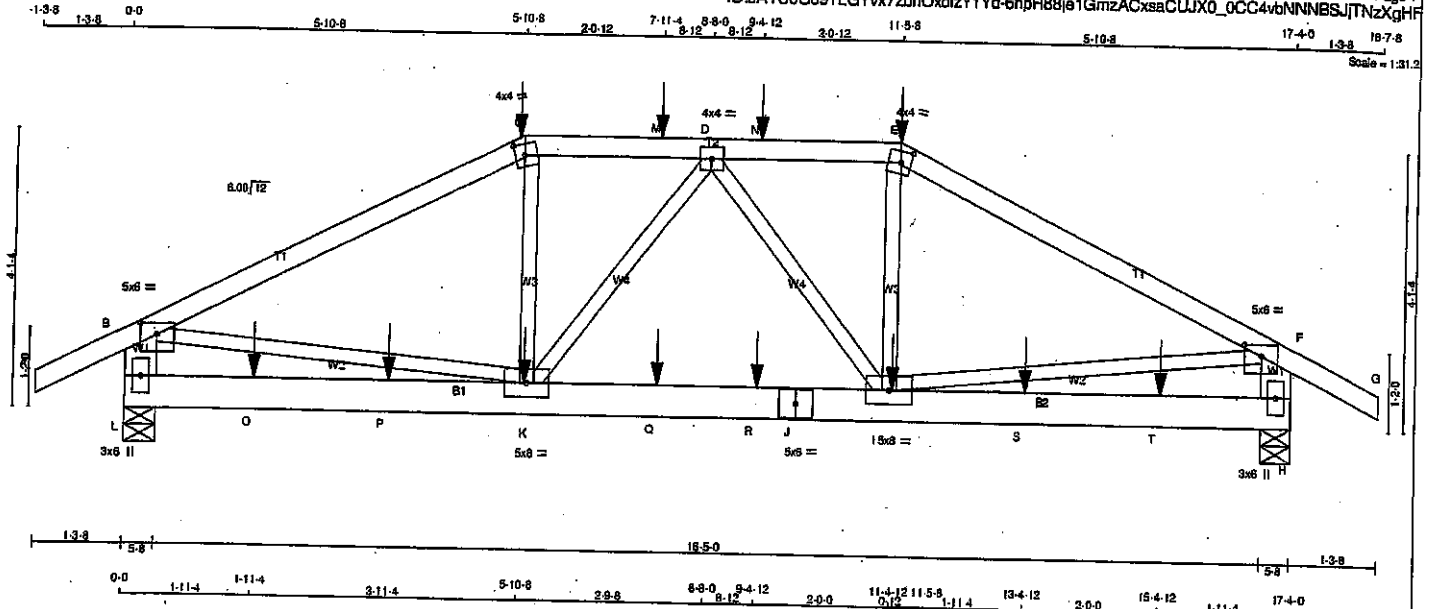
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (1) (INPUT = 0.90)
JSI METAL = 0.60 (4) (INPUT = 1.00)



Structural component only
DWG# T-2109016

JOB NAME 417566	TRUSS NAME T57	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	
Version 8.420 S Jan 21 2021 MTEK Industries, Inc. Wed Mar 24 17:18:38 2021 Page 1					
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Scale = 1:31.2					



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
L - B	2x6	DRY No.2	SPF
H - F	2x6	DRY No.2	SPF
L - J	2x6	DRY No.2	SPF
J - H	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

PLATES (tablets in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	6.0	2.00	3.00
C TTW-m	MT20	4.0	4.0	2.00	1.75
D TMVW-t	MT20	4.0	4.0		
E TTW-m	MT20	4.0	4.0	2.00	1.75
F TMVW-p	MT20	5.0	6.0	2.00	3.00
H BMV1-p	MT20	3.0	6.0		
I BMVW-t	MT20	5.0	6.0		
J BS-t	MT20	5.0	6.0		
K BMVW-t	MT20	5.0	6.0		
L BMV1-p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	JT	VERT	BRG	IN-SX	BRG	IN-SX
L	1682	0	1682	0	0	5-8	5-8
H	1682	0	1682	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	JT	COMBINED	JT	COMBINED	JT	COMBINED	JT	COMBINED
L	1175	772 / 0	0 / 0	0 / 0	0 / 0	404 / 0	0 / 0	0 / 0	0 / 0
H	1175	772 / 0	0 / 0	0 / 0	0 / 0	404 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. (PLF)	MAX. (LBS)	MEMB.	FORCE (LBS)	VERT. (PLF)	MAX. (LBS)
FR-TO				FR-TO			
A-B	0 / 28	-91.8	-91.8 0.13 (1)	K-C	0 / 184	0.07 (4)	0.07 (4)
B-C	-2118 / 0	-91.8	-91.8 0.82 (1)	L-E	0 / 184	0.07 (4)	0.07 (4)
C-M	-1902 / 0	-91.8	-91.8 0.24 (1)	B-K	0 / 1908	0.47 (1)	0.47 (1)
M-D	-1902 / 0	-91.8	-91.8 0.24 (1)	F-F	0 / 1908	0.47 (1)	0.47 (1)
D-N	-1902 / 0	-91.8	-91.8 0.24 (1)	K-D	-303 / 0	0.11 (1)	0.11 (1)
N-E	-1902 / 0	-91.8	-91.8 0.24 (1)	D-I	-304 / 0	0.11 (1)	0.11 (1)
E-F	-2118 / 0	-91.8	-91.8 0.82 (1)				
F-G	0 / 28	-91.8	-91.8 0.13 (1)				
L-B	-1586 / 0	0.0	0.0 0.11 (1)				
H-F	-1586 / 0	0.0	0.0 0.11 (1)				
L-O	0 / 0	-18.5	-18.5 0.13 (4)				
O-P	0 / 0	-18.5	-18.5 0.13 (4)				
P-K	0 / 0	-18.5	-18.5 0.13 (4)				
K-Q	0 / 2078	-18.5	-18.5 0.33 (1)				
Q-R	0 / 2078	-18.5	-18.5 0.33 (1)				
R-J	0 / 2078	-18.5	-18.5 0.33 (1)				
J-I	0 / 2078	-18.5	-18.5 0.33 (1)				
I-S	0 / 0	-18.5	-18.5 0.13 (4)				
S-T	0 / 0	-18.5	-18.5 0.13 (4)				
T-H	0 / 0	-18.5	-18.5 0.13 (4)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-254	-254	FRONT	VERT	TOTAL		C1
E	11-6-8	-254	-254	FRONT	VERT	TOTAL		C1
I	11-4-12	-21	-21	FRONT	VERT	TOTAL		C1
K	5-11-4	-21	-21	FRONT	VERT	TOTAL		C1
M	7-11-4	-76	-76	FRONT	VERT	TOTAL		C1
N	9-4-12	-76	-76	FRONT	VERT	TOTAL		C1
O	1-11-4	-20	-20	FRONT	VERT	TOTAL		C1
P	3-11-4	-21	-21	FRONT	VERT	TOTAL		C1
Q	7-11-4	-21	-21	FRONT	VERT	TOTAL		C1
R	9-4-12	-21	-21	FRONT	VERT	TOTAL		C1
S	13-4-12	-21	-21	FRONT	VERT	TOTAL		C1
T	15-4-12	-20	-20	FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.82/1.00 (B-C:1), BC=0.33/1.00 (I-K:1), WB=0.47/1.00 (B-K:1), SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (F) (INPUT = 0.90)

JSI METAL = 0.43 (F) (INPUT = 1.00)



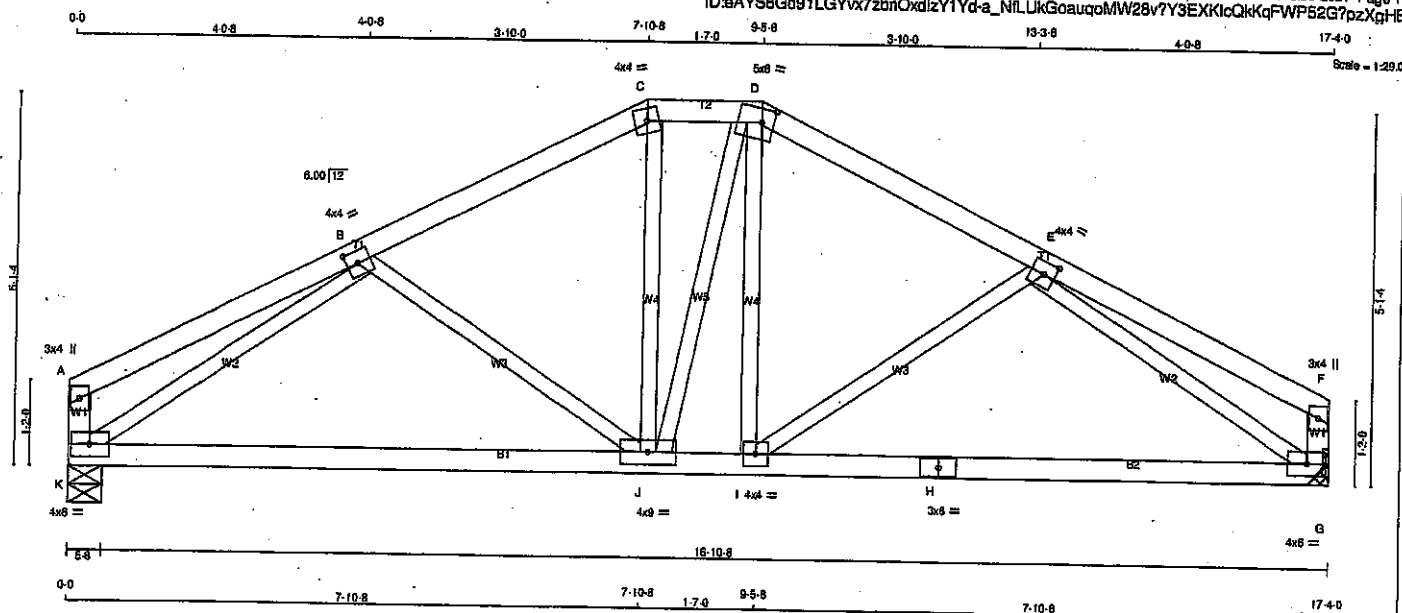
Structural component only
DWG# T-2109017

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417566	T58	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	No.2	SPF		
K - A	2x4	DRY	No.2	SPF		
G - F	2x4	DRY	No.2	SPF		
K - H	2x4	DRY	No.2	SPF		
H - G	2x4	DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - TMV-p	MT20	3.0	4.0		
B - TMWW-t	MT20	4.0	4.0	2.00	1.75
C - TTW-m	MT20	4.0	4.0		
D - TTWW-m	MT20	6.0	6.0	2.25	2.00
E - TMV-p	MT20	4.0	4.0	2.00	1.75
F - TMV-p	MT20	3.0	4.0		
G - BMVW-t	MT20	4.0	6.0		
H - BS-t	MT20	3.0	6.0		
I - BMVW-t	MT20	4.0	4.0		
J - BMVW-t	MT20	4.0	9.0		
K - BMVW-t	MT20	4.0	6.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UPLIFT	IN-SX
K	956	0	0	5-8
G	956	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	676	443 / 0	0 / 0	0 / 0	0 / 0	232 / 0	0 / 0
G	676	443 / 0	0 / 0	0 / 0	0 / 0	232 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	UNBRACED	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	(PLF)			CS1 (LC)	LENGTH		FORCE (LBS)
FR-TO		FROM	TO				FR-TO	
A-B	0 / 18	-91.8	-91.8	0.22 (1)	10.00	B-J	-218 / 0	0.09 (1)
B-C	-995 / 0	-91.8	-91.8	0.18 (1)	6.08	J-C	0 / 228	0.05 (1)
C-D	-878 / 0	-91.8	-91.8	0.04 (1)	6.25	J-D	0 / 4	0.00 (1)
D-E	-994 / 0	-91.8	-91.8	0.18 (1)	6.08	I-D	0 / 222	0.05 (1)
E-F	0 / 18	-91.8	-91.8	0.22 (1)	10.00	I-E	-219 / 0	0.09 (1)
K-A	-141 / 0	0.0	0.0	0.01 (1)	7.81	K-B	-1294 / 0	0.50 (1)
G-F	-141 / 0	0.0	0.0	0.01 (1)	7.81	E-G	-1293 / 0	0.50 (1)
K-J	0 / 1052	-18.5	-18.5	0.35 (4)	10.00			
J-I	0 / 877	-18.5	-18.5	0.35 (4)	10.00			
I-H	0 / 1051	-18.5	-18.5	0.35 (4)	10.00			
H-G	0 / 1051	-18.5	-18.5	0.35 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
BOT CH. LL	= 6.0	PSF
DL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.58")
CALCULATED VERT. DEFL. (LL) = L/899 (0.03")
ALLOWABLE DEFL. (TL) = L/360 (0.58")
CALCULATED VERT. DEFL. (TL) = L/999 (0.13")

CS1: TC=0.22/1.00 (E-F:1), BC=0.35/1.00 (J-K:4), WB=0.50/1.00 (B-K:1), SS1=0.15/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

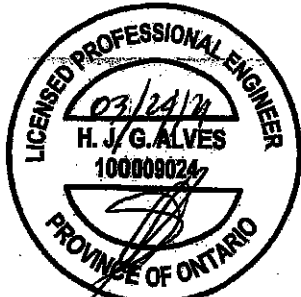
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1997 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.45 (H) (INPUT = 1.00)

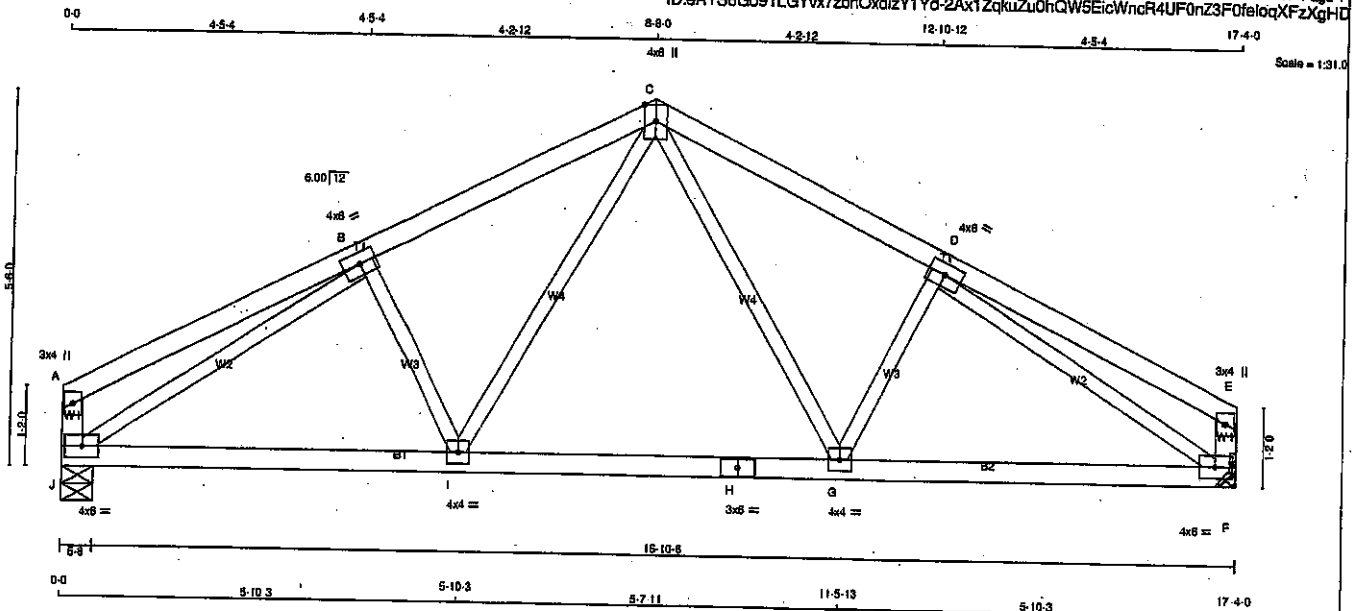


Structural component only
DWG# T-2109018

JOB NAME 417566	TRUSS NAME T59	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MTek Industries, Inc. Wed Mar 24 17:18:40 2021 Page 1
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TOTAL WEIGHT = 66 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4	DRY No.2
C - A	2x4	DRY No.2
J - E	2x4	DRY No.2
J - H	2x4	DRY No.2
H - F	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMVW-t	MT20	4.0	6.0		
C	TTWW+p	MT20	4.0	6.0		Edge
D	TMVW-t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	4.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMVW-t	MT20	4.0	4.0		
J	BMVW-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT	IN-SX
J	956	0	0	5-8
F	956	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL
J	876 443 / 0 0 / 0 0 / 0 0 / 0 232 / 0 0 / 0
F	876 443 / 0 0 / 0 0 / 0 0 / 0 232 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PL)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
A-B	0 / 20	-91.8	-91.8	0.27 (1)	10.00	C-G	0 / 380	0.09 (1)
B-C	-1110 / 0	-91.8	-91.8	0.22 (1)	5.78	G-D	-244 / 0	0.05 (1)
C-D	-1110 / 0	-91.8	-91.8	0.22 (1)	5.78	D-E	0 / 380	0.09 (1)
D-E	0 / 20	-91.8	-91.8	0.27 (1)	10.00	E-F	-244 / 0	0.05 (1)
J-A	-154 / 0	0.0	0.0	0.02 (1)	7.81	F-I	-1322 / 0	0.59 (1)
F-E	-154 / 0	0.0	0.0	0.02 (1)	7.81	I-H	-1322 / 0	0.59 (1)
J-I	0 / 1084	-18.5	-18.5	0.25 (1)	10.00	H-G	0 / 793	-18.5
I-H	0 / 793	-18.5	-18.5	0.20 (4)	10.00	G-F	0 / 1084	-18.5
H-G	0 / 793	-18.5	-18.5	0.20 (4)	10.00			
G-F	0 / 1084	-18.5	-18.5	0.25 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=		39.0	PSF

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.58")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.04")
ALLOWABLE DEFL. (TL) = $L/360$ (0.58")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.07")

CSI: TC=0.27/1.00 (A-B:1), BC=0.25/1.00 (I-J:1), WB=0.59/1.00 (D-F:1), SS=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
MT20	650	371	1747
	798	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

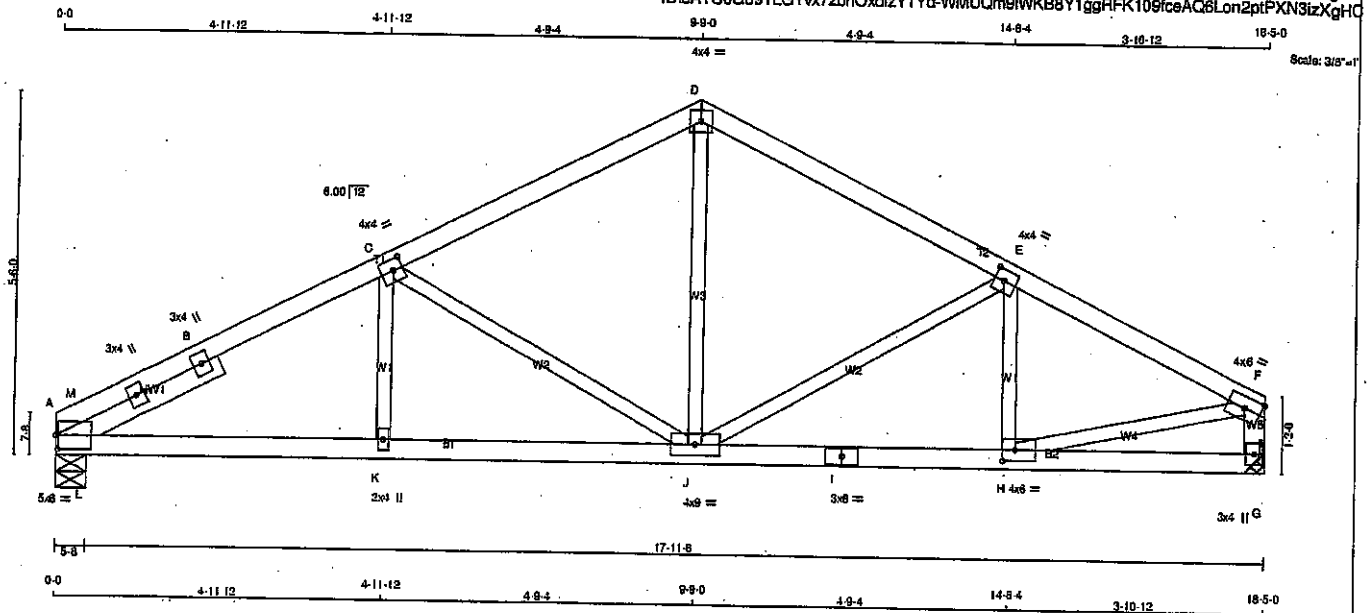
JSI GRIP = 0.78 (B) (INPUT = 0.90)

JSI METAL = 0.30 (B) (INPUT = 1.00)



Structural component only
DWG# T-2109019

JOB NAME 417566	TRUSS NAME T60	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
A - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
REINFORCING MEMBERS			
HW1	2x4	DRY	No.2
ALL WEBS 2x3 DRY EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMBMR1-I	MT20	5.0	8.0	2.50 0.25
A	RT-t	MT20	3.0	4.0	
A	RT-t	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	4.0	2.00 1.75
D	TTW-p	MT20	4.0	4.0	
E	TMWW-t	MT20	4.0	4.0	2.00 1.75
F	TMWW-t	MT20	4.0	8.0	Edge
G	BMV1-p	MT20	3.0	4.0	
H	BMWW-t	MT20	4.0	8.0	2.00 2.25
I	BS-t	MT20	3.0	8.0	
J	BMWW-t	MT20	4.0	9.0	
K	BMW-w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
A	1015	0	1015	0	0
G	1015	0	1015	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
A	718	471	0	0	0	247
G	718	471	0	0	0	247

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

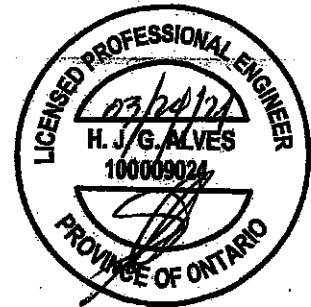
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO	CSI (LC)	FR-TO		FROM TO	CSI (LC)
A-M	-1357 / 0	-91.8 -91.8	0.02 (1)	5.57	K-C	0 / 84	0.03 (4)
M-S	-1243 / 0	-91.8 -91.8	0.20 (1)	5.55	C-J	-551 / 0	0.29 (1)
B-C	-1243 / 0	-91.8 -91.8	0.20 (1)	5.55	J-D	0 / 548	0.12 (1)
C-D	-1028 / 0	-91.8 -91.8	0.28 (1)	5.88	E-E	-329 / 0	0.17 (1)
D-E	-1031 / 0	-91.8 -91.8	0.24 (1)	5.89	H-E	-184 / 7	0.03 (1)
E-F	-1308 / 0	-91.8 -91.8	0.23 (1)	5.40	H-F	0 / 1217	0.27 (1)
G-F	-983 / 0	0.0 0.0	0.10 (1)	7.81	L-M	0 / 488	0.00 (1)
A-L	0 / 887	-18.5 -18.5	0.17 (1)	10.00	L-B	-337 / 0	0.08 (1)
L-K	0 / 1380	-18.5 -18.5	0.28 (1)	10.00			
K-J	0 / 1380	-18.5 -18.5	0.28 (1)	10.00			
J-I	0 / 1189	-18.5 -18.5	0.25 (1)	10.00			
I-H	0 / 1189	-18.5 -18.5	0.25 (1)	10.00			
H-G	0 / 0	-18.5 -18.5	0.07 (4)	10.00			



Structural component only
DWG# T-2109020

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.28/1.00 (C-D:1), BC=0.28/1.00 (J-K:1), WS=0.29/1.00 (C-J:1), SS=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

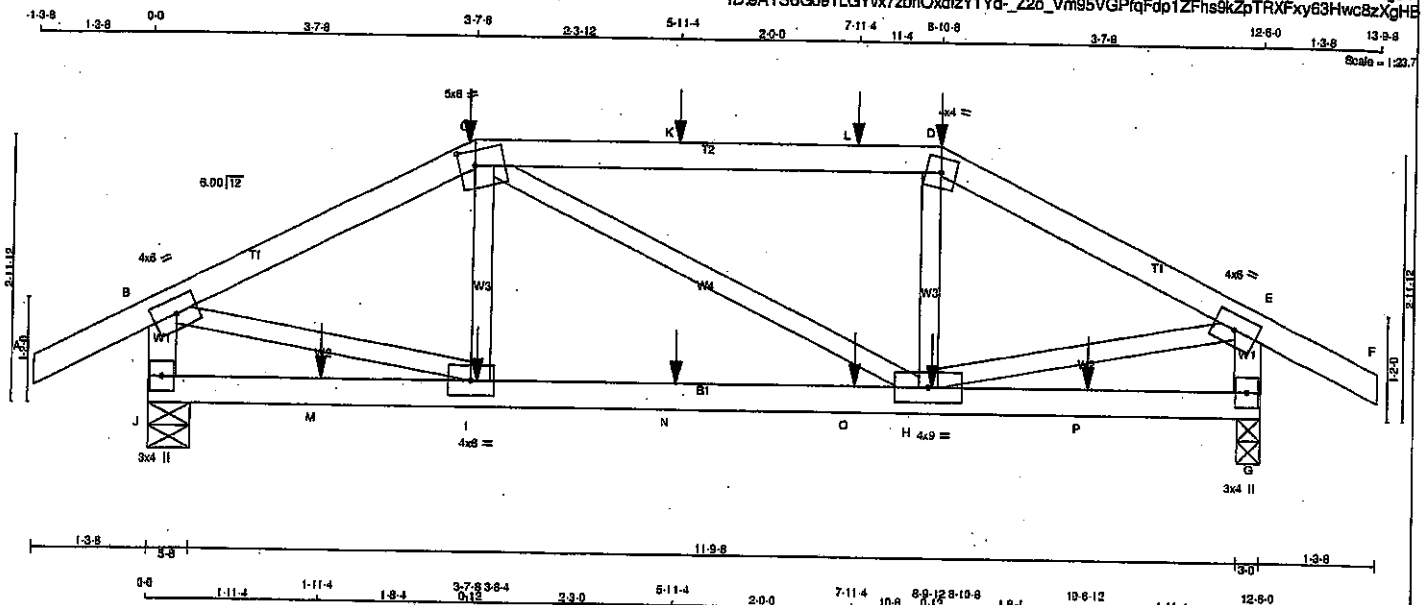
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.64 (F) (INPUT = 0.90)
JSI METAL=0.49 (A) (INPUT = 1.00)

JOB NAME 417566	TRUSS NAME T61	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Mar 24 17:18:42 2021 Page 1 ID: aYS6G091LGYvx7zbnOxdizY1Yd_-Z2o_Vm95VGPfqFdp1ZFhs9kZpTRXFxy63Hwc8zXgHE			



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
I - J	2x4	DRY	No.2
J - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - N	2x4	DRY	No.2
N - O	2x4	DRY	No.2
O - P	2x4	DRY	No.2
P - Q	2x4	DRY	No.2
Q - R	2x4	DRY	No.2
R - S	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	4.0	8.0		
C	TTWw-m	MT20	5.0	8.0	2.00	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMVW-I	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-I	MT20	4.0	9.0		
I	BMVWW-I	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORIZ UPLIFT
JT	1009	0	1009	0
G	1019	0	1019	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	712	477.0	0.0	0.0	0.0	235.0	0.0
G	719	482.0	0.0	0.0	0.0	237.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO				FR-TO			
A-B	0.128	-91.8	-91.8 0.13 (1)	I-C	-108.150	0.02 (1)	
B-C	-1054.0	-91.8	-91.8 0.25 (1)	C-H	0.119	0.00 (1)	
C-K	-954.0	-91.8	-91.8 0.60 (1)	H-D	-108.156	0.02 (4)	
K-L	-954.0	-91.8	-91.8 0.60 (1)	B-I	0.966	0.24 (1)	
L-D	-954.0	-91.8	-91.8 0.60 (1)	H-E	0.987	0.24 (1)	
D-E	-1078.0	-91.8	-91.8 0.25 (1)				
E-F	0.128	-91.8	-91.8 0.13 (1)				
J-B	-990.0	0.0	0.0 0.11 (1)				
G-E	-989.0	0.0	0.0 0.11 (1)				
J-M	0.0	-18.5	-18.5 0.10 (4)				
M-I	0.0	-18.5	-18.5 0.10 (4)				
I-N	0.937	-18.5	-18.5 0.22 (1)				
N-O	0.937	-18.5	-18.5 0.22 (1)				
O-H	0.937	-18.5	-18.5 0.22 (1)				
H-P	0.0	-18.5	-18.5 0.11 (4)				
P-G	0.0	-18.5	-18.5 0.11 (4)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	3-7-8	-97	-97		BACK	VERT	TOTAL		C1
D	8-10-8	-97	-97		BACK	VERT	TOTAL		C1
H	8-9-12	-8	-8		BACK	VERT	TOTAL		C1
I	3-8-4	-8	-8		BACK	VERT	TOTAL		C1
K	5-11-4	-23	-23		BACK	VERT	TOTAL		C1
L	7-11-4	-25	-25		BACK	VERT	TOTAL		C1
M	1-11-4	-5	-5		BACK	VERT	TOTAL		C1
N	5-11-4	-8	-8		BACK	VERT	TOTAL		C1
O	7-11-4	-8	-8		BACK	VERT	TOTAL		C1
P	10-6-12	-5	-5		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.05")

CS1: TC=0.80/1.00 (C-D:1), BC=0.22/1.00 (H-I:1), WS=0.24/1.00 (E-H:1), SS=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

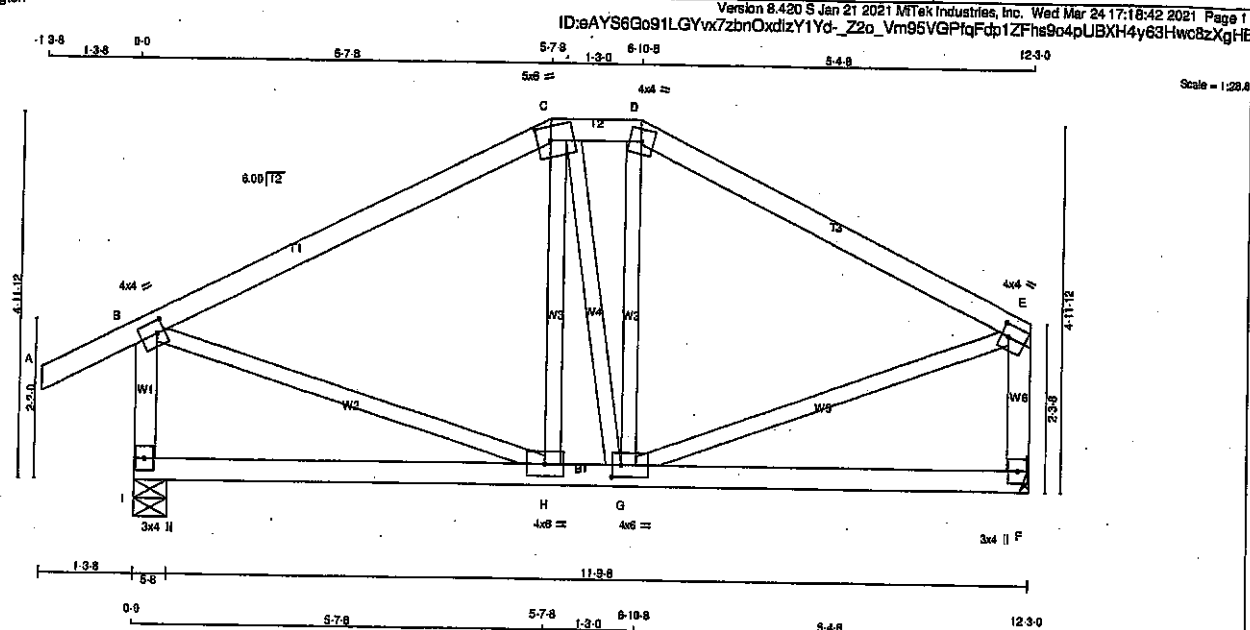
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.62 (I) (INPUT = 0.90)
JSI METAL= 0.27 (E) (INPUT = 1.00)



Structural component only
DWG# T-2109021

JOB NAME 417566	TRUSS NAME T62	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 MTEK Industries, Inc. Wed Mar 24 17:18:42 2021 Page 1			



TOTAL WEIGHT = 55 lb

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-I	MT20	4.0	4.0	2.00	1.25
C TTWW-m	MT20	5.0	3.0	2.25	2.25
D TTW-m	MT20	4.0	4.0		
E TMVW-I	MT20	4.0	4.0	2.00	1.25
F BMV1+p	MT20	3.0	4.0		
G BMVWV-I	MT20	4.0	6.0	2.00	1.50
H BMVW-I	MT20	4.0	6.0		
I BMV1+p	MT20	3.0	4.0		

NOTES:

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
I	800	0	0	5-8
F	675	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LOASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	583	383	0	0	0	180	0
F	478	313	0	0	0	164	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. GSI (LC)	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. GSI (LC)
FR-TO					FR-TO		
A-B	0/28	-91.8	-91.8	0.12 (1)	H-C	-66/34	0.02 (1)
B-C	-507/0	-91.8	-91.8	0.38 (1)	C-G	-22/0	0.01 (1)
C-D	-446/0	-91.8	-91.8	0.02 (1)	G-D	-79/25	0.03 (1)
D-E	-501/0	-91.8	-91.8	0.34 (1)	B-H	0/478	0.11 (1)
E-F	-757/0	0.0	0.0	0.09 (1)	G-E	0/478	0.11 (1)
F-G	-633/0	0.0	0.0	0.08 (1)			
I-H	0/0	-18.5	-18.5	0.14 (4)			
H-G	0/451	-18.5	-18.5	0.18 (4)			
G-F	0/0	-18.5	-18.5	0.12 (4)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2019, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = 1/360 (0.41")
CALCULATED VERT. DEF. (LL) = 1/999 (0.01")
ALLOWABLE DEF. (TL) = 1/360 (0.41")
CALCULATED VERT. DEF. (TL) = 1/999 (0.03")

CSI: TC=0.38/1.00 (B-C:1), BC=0.18/1.00 (G-H:4), WB=0.11/1.00 (E-G:1), SS=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

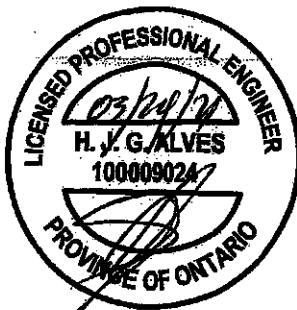
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

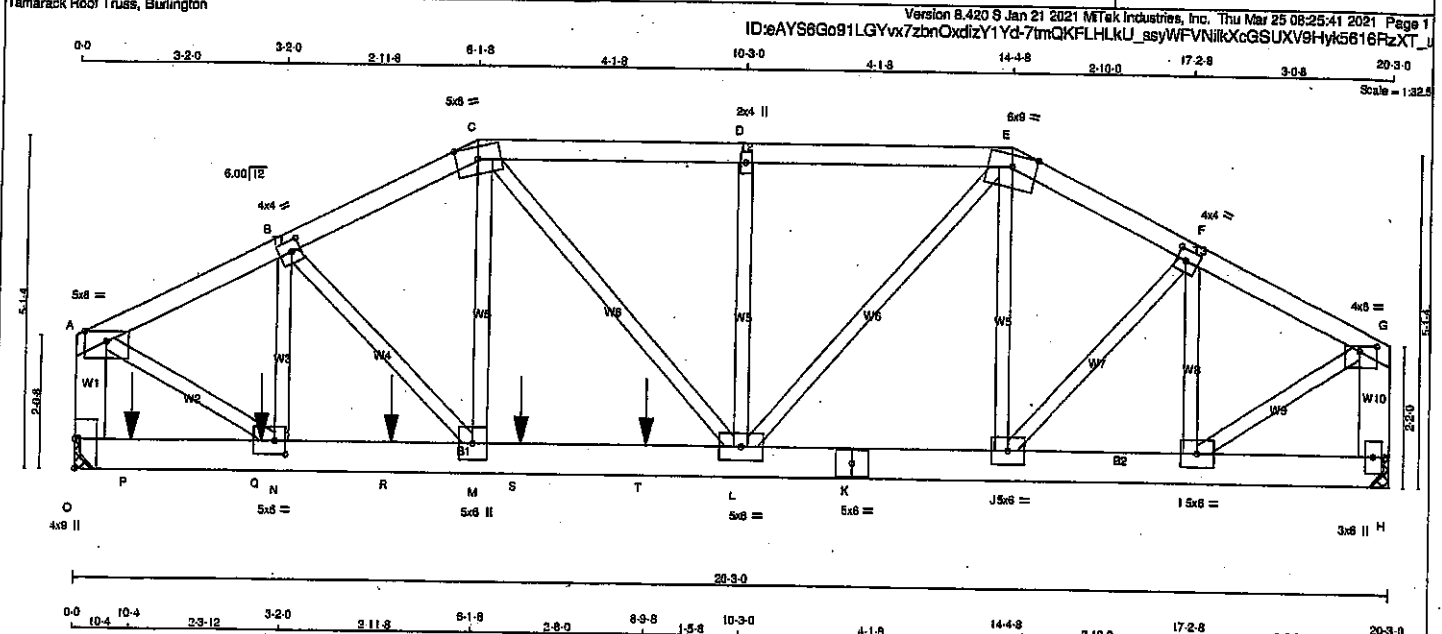
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.73 (B) (INPUT = 0.90)
JSI METAL = 0.23 (B) (INPUT = 1.00)



Structural component only
DWG# T-2109022

JOB NAME 417566	TRUSS NAME T63	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
O - A	2x6	DRY	No.2	SPF	
H - G	2x6	DRY	No.2	SPF	
O - K	2x6	DRY	1650F 1.5E	SPF	
K - H	2x6	DRY	1650F 1.5E	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table in inches)	W	LEN	Y	X
A - TMVW-p	MT20	5.0	8.0	Edge
B - TMVW-t	MT20	4.0	4.0	2.00 1.75
C - TMVW-m	MT20	5.0	8.0	Edge
D - TMVW-w	MT20	2.0	4.0	
E - TMVW-m	MT20	6.0	9.0	Edge
F - TMVW-t	MT20	4.0	4.0	2.00 1.75
G - TMVW-p	MT20	4.0	8.0	1.00 3.25
H - BMV1-p	MT20	3.0	6.0	
I - BMVW-t	MT20	5.0	8.0	
J - BMVW-t	MT20	5.0	6.0	
K - BS-t	MT20	5.0	6.0	
L - BMVW-w	MT20	5.0	8.0	
M - BMVW-t	MT20	5.0	6.0	
N - BMVW-t	MT20	5.0	6.0	2.50 2.00
O - BMV1-t	MT20	4.0	9.0	5.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	2848	2848	0	0
O	1927	1927	0	0
H	1927	1927	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT O, H. MINIMUM BEARING LENGTH AT JOINT O = 3-8, JOINT H = 3-8.

UNFACTORED REACTIONS	1ST LOASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
O	2006	1360 / 0 0 / 0 0 / 0 646 / 0 0 / 0
H	1360	911 / 0 0 / 0 0 / 0 0 / 0 449 / 0 0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO					FR-TO		
A-B	-2872 / 0	-91.8	-91.8 0.22 (1)	4.02	N-B	-867 / 0	0.19 (1)
B-C	-3078 / 0	-91.8	-91.8 0.25 (1)	3.75	B-M	0 / 494	0.12 (1)
C-D	-3023 / 0	-91.8	-91.8 0.40 (1)	3.60	M-C	0 / 807	0.20 (1)
D-E	-3023 / 0	-91.8	-91.8 0.40 (1)	3.60	C-L	0 / 405	0.10 (1)
E-F	-2241 / 0	-91.8	-91.8 0.20 (1)	4.35	L-D	-445 / 0	0.17 (1)
F-G	-1827 / 0	-91.8	-91.8 0.18 (1)	4.75	L-E	0 / 1585	0.39 (1)
G-A	-2571 / 0	0.0	0.0 0.20 (1)	6.43	J-E	-449 / 0	0.17 (1)
H-G	-1894 / 0	0.0	0.0 0.15 (1)	7.26	J-F	0 / 518	0.13 (1)
					F-G	-892 / 0	0.20 (1)
O-P	0 / 0	-18.5	-18.5 0.12 (1)	10.00	A-N	0 / 2705	0.67 (1)
P-Q	0 / 0	-18.5	-18.5 0.12 (1)	10.00	I-G	0 / 1905	0.47 (1)
Q-N	0 / 0	-18.5	-18.5 0.12 (1)	10.00			
N-R	0 / 2399	-18.5	-18.5 0.44 (1)	10.00			
R-M	0 / 2399	-18.5	-18.5 0.44 (1)	10.00			
M-S	0 / 2757	-18.5	-18.5 0.70 (1)	10.00			
S-T	0 / 2757	-18.5	-18.5 0.70 (1)	10.00			
T-L	0 / 2757	-18.5	-18.5 0.70 (1)	10.00			
L-K	0 / 1980	-18.5	-18.5 0.35 (1)	10.00			
K-J	0 / 1980	-18.5	-18.5 0.35 (1)	10.00			
J-I	0 / 1645	-18.5	-18.5 0.17 (1)	10.00			
I-H	0 / 0	-18.5	-18.5 0.02 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	10-4	-205	-205	-	-	FRONT	VERT	TOTAL	-	C1
Q	2-10-4	-214	-214	-	-	FRONT	VERT	TOTAL	-	C1
R	4-10-4	-214	-214	-	-	FRONT	VERT	TOTAL	-	C1
S	8-10-4	-214	-214	-	-	FRONT	VERT	TOTAL	-	C1
T	8-9-8	-939	-939	-	-	FRONT	VERT	TOTAL	-	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 0.0 PSF
TOTAL LOAD = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/360 (0.67")
CALCULATED VERT. DEF. (LL) = L/999 (0.09")
ALLOWABLE DEF. (TL) = L/360 (0.67")
CALCULATED VERT. DEF. (TL) = L/999 (0.17")

CSI: TC=0.40/1.00 (D-E:1), BC=0.70/1.00 (L-M:1), WB=0.67/1.00 (A-N:1), SS=0.60/1.00 (L-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

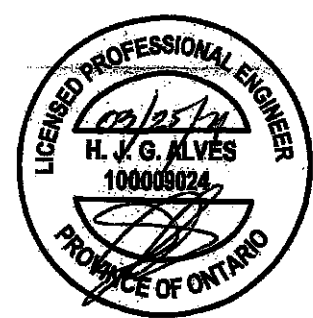
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

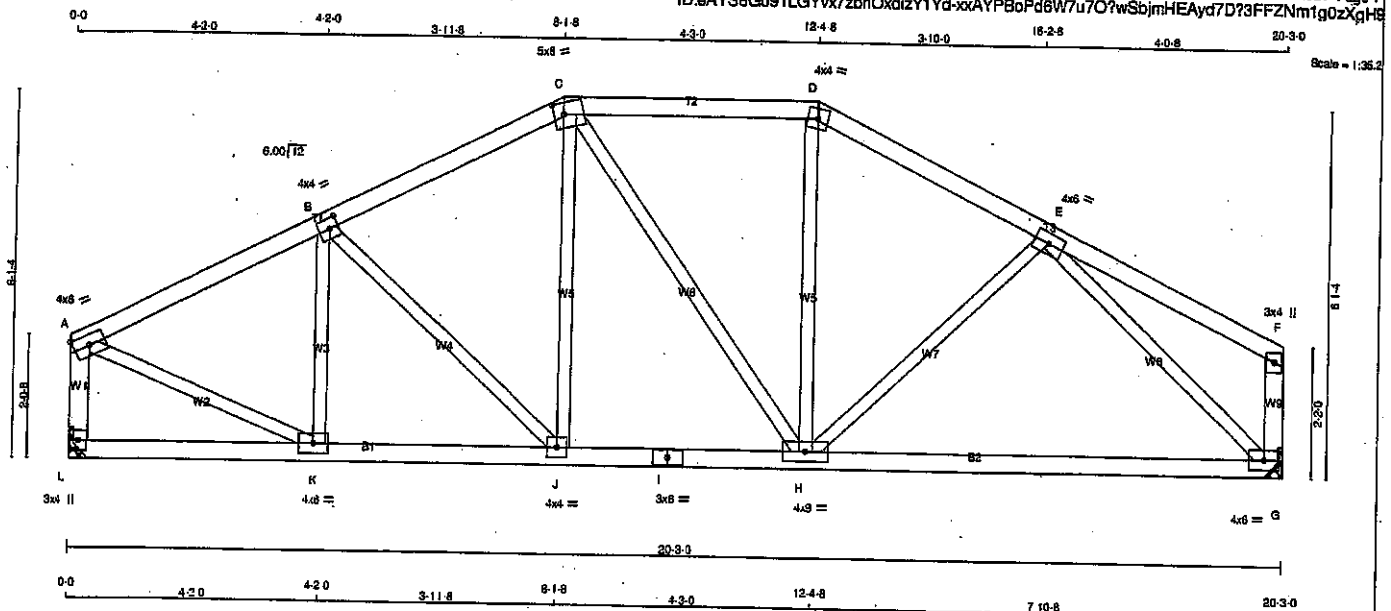
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.55 (N) (INPUT = 1.00)



Structural component only
DWG# T-2109023

JOB NAME 417566	TRUSS NAME T64	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
L - A	2x4	DRY	No.2
Q - F	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	Edge
B	TMVW-t	MT20	4.0	4.0	2.00 1.75
C	TTWV-m	MT20	5.0	6.0	2.25 2.00
D	TTW-m	MT20	4.0	4.0	
E	TMVW-t	MT20	4.0	6.0	
F	TMV-p	MT20	3.0	4.0	
G	BMVW-t	MT20	4.0	6.0	
H	BMVW-t	MT20	4.0	9.0	
I	BS-t	MT20	3.0	6.0	
J	BMVW-t	MT20	4.0	4.0	
K	BMVW-t	MT20	4.0	6.0	
L	BMV-t	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT HORZ	DOWN HORZ UPLIFT	IN-SX	IN-SX
L 1116 0	1116 0	0	MECHANICAL
G 1116 0	1116 0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L, G. MINIMUM BEARING LENGTH AT JOINT L = 3-8, JOINT G = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX/MIN COMPONENT REACTIONS						
JT COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
L 789	518.0	0.0	0.0	0.0	271.0	0.0	
G 789	518.0	0.0	0.0	0.0	271.0	0.0	

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO		FROM TO		FR-TO			
A-B	-1123 / 0	-91.8 -91.8 0.20 (1)	5.76	K-B	-336 / 0	0.09 (1)	
B-C	-1085 / 0	-91.8 -91.8 0.20 (1)	5.88	B-J	-121 / 0	0.06 (1)	
C-D	-944 / 0	-91.8 -91.8 0.22 (1)	6.12	J-C	0 / 125	0.03 (1)	
D-E	-1072 / 0	-91.8 -91.8 0.18 (1)	5.88	C-H	0 / 31	0.01 (4)	
E-F	0 / 18	-91.8 -91.8 0.22 (1)	10.00	H-D	0 / 138	0.04 (4)	
L-A	-1084 / 0	0.0 0.0 0.12 (1)	7.56	H-E	0 / 56	0.02 (4)	
G-F	-139 / 0	0.0 0.0 0.02 (1)	7.81	A-K	0 / 1106	0.25 (1)	
				E-G	-1314 / 0	0.84 (1)	
L-K	0 / 0	-18.5 -18.5 0.08 (4)	10.00				
K-J	0 / 1022	-18.5 -18.5 0.20 (1)	10.00				
J-I	0 / 936	-18.5 -18.5 0.33 (4)	10.00				
I-H	0 / 936	-18.5 -18.5 0.33 (4)	10.00				
H-G	0 / 942	-18.5 -18.5 0.33 (4)	10.00				

TOTAL WEIGHT = 66 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.70 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (0.87")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.03")
ALLOWABLE DEFL. (TL) = 1/360 (0.87")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.13")

CSI: TC=0.22/1.00 (E-F:1), BC=0.33/1.00 (H-J:4), WB=0.64/1.00 (E-G:1), SSI=0.16/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MT20	650	371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

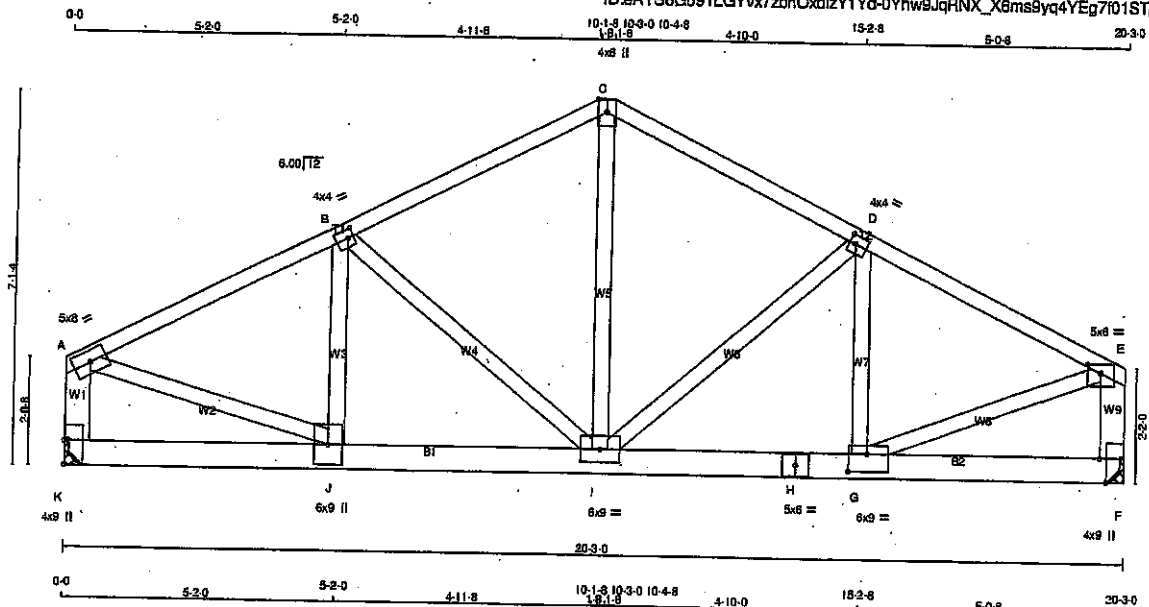
JSI GRIP= 0.71 (A) (INPUT = 0.90)
JSI METAL= 0.29 (A) (INPUT = 1.00)



Structural component only
DWG# T-2109024

JOB NAME 417566	TRUSS NAME T65	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID: AYS6G091LGYvx7ZbnOxdizY1Yd-0Yhw9JqRNX_X8ms9yq4YEg7f01STpq30kP2wozXSyz



TOTAL WEIGHT = 3 X 109 = 327 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - E	2x4 DRY	No.2	SPF		
K - A	2x6 DRY	No.2	SPF		
F - E	2x6 DRY	No.2	SPF		
K - H	2x6 DRY	No.2	SPF		
H - F	2x6 DRY	No.2	SPF		
ALL WEBS	2x4 DRY	No.2	SPF		
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	TOP
C - E 1	12	TOP
K - A 2	12	TOP
F - E 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
K - H 2	5	SIDE(507.6)
H - F 2	5	SIDE(507.6)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INRUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UPLIFT
K	8826	0	8826	0
F	8826	0	8826	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K, F. MINIMUM BEARING LENGTH AT JOINT K = 4'-0", JOINT F = 4'-0".

UNFACTORED REACTIONS

1ST LC CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED
K	6241 4098 / 0 0 / 0 0 / 0 0 2145 / 0 0 / 0
F	6241 4098 / 0 0 / 0 0 / 0 0 2145 / 0 0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO					FR-TO		
A-B	-9504 / 0	-91.8	-91.8 0.29 (1)	3.77	J-B	0 / 1723	0.09 (1)
B-C	-7631 / 0	-91.8	-91.8 0.22 (1)	4.19	S-I	-2225 / 0	0.41 (1)
C-D	-7630 / 0	-91.8	-91.8 0.21 (1)	4.19	I-D	-1893 / 0	0.34 (1)
D-E	-9193 / 0	-91.8	-91.8 0.26 (1)	3.94	G-D	0 / 1413	0.08 (1)
K-A	-7173 / 0	0.0	0.0 0.17 (1)	6.66	A-J	0 / 8941	0.48 (1)
F-E	-7209 / 0	0.0	0.0 0.18 (1)	6.65	G-E	0 / 8734	0.47 (1)
					I-C	0 / 8432	0.34 (1)
K-J	0 / 0	-779.9	-779.9 0.42 (1)	10.00			
J-I	0 / 8518	-779.9	-779.9 0.75 (1)	10.00			
I-H	0 / 8239	-779.9	-779.9 0.72 (1)	10.00			
H-G	0 / 8239	-779.9	-779.9 0.72 (1)	10.00			
G-F	0 / 0	-779.9	-779.9 0.39 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN CC

GIRDER TYPE: CSidGirder
START DISTANCE = 0-0
START SPAN CARRIED = 30-1-0
END DISTANCE = 20-3-0
END SPAN CARRIED = 30-1-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSI.
(DEFINED BY USER)

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = L/360 (0.68")
CALCULATED VERT. DEFL (LL) = L/999 (0.08")
ALLOWABLE DEFL (TL) = L/360 (0.68")
CALCULATED VERT. DEFL (TL) = L/999 (0.15")

CSI: TC=0.29/1.00 (A-B:1), BC=0.75/1.00 (I-J:1), WB=0.48/1.00 (A-J:1), SS=0.48/1.00 (J-K:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

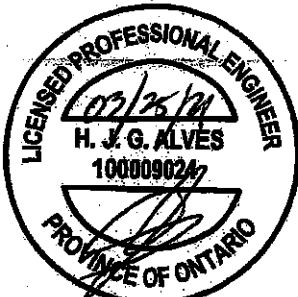
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.85 (J) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2109025 *in*

JOB NAME 417566	TRUSS NAME T65	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID: eAYS6Go91LGYvx7zbnOxdizY1Yd-0Yhw9JgRNX X8ms9va4YEg7f01STqg30kP2wozXSyz

PLATES (table is in inches)

PLATE	TYPE	PLATES	W	LEN	Y	X
B	TMWW-1	MT20	4.0	4.0	2.00	1.25
C	TTW-p	MT20	4.0	6.0	Edge	
D	TMWW-1	MT20	4.0	4.0	2.00	1.25
E	TMWW-p	MT20	5.0	6.0	2.00	3.00
F	BMV1-t	MT20	4.0	9.0	Edge	1.50
G	BMWW-4	MT20	6.0	9.0	4.00	4.25
H	BS-t	MT20	5.0	6.0		
I	BMWW-1	MT20	6.0	9.0		
J	BMWW-t	MT20	6.0	9.0		
K	BMV1-t	MT20	4.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

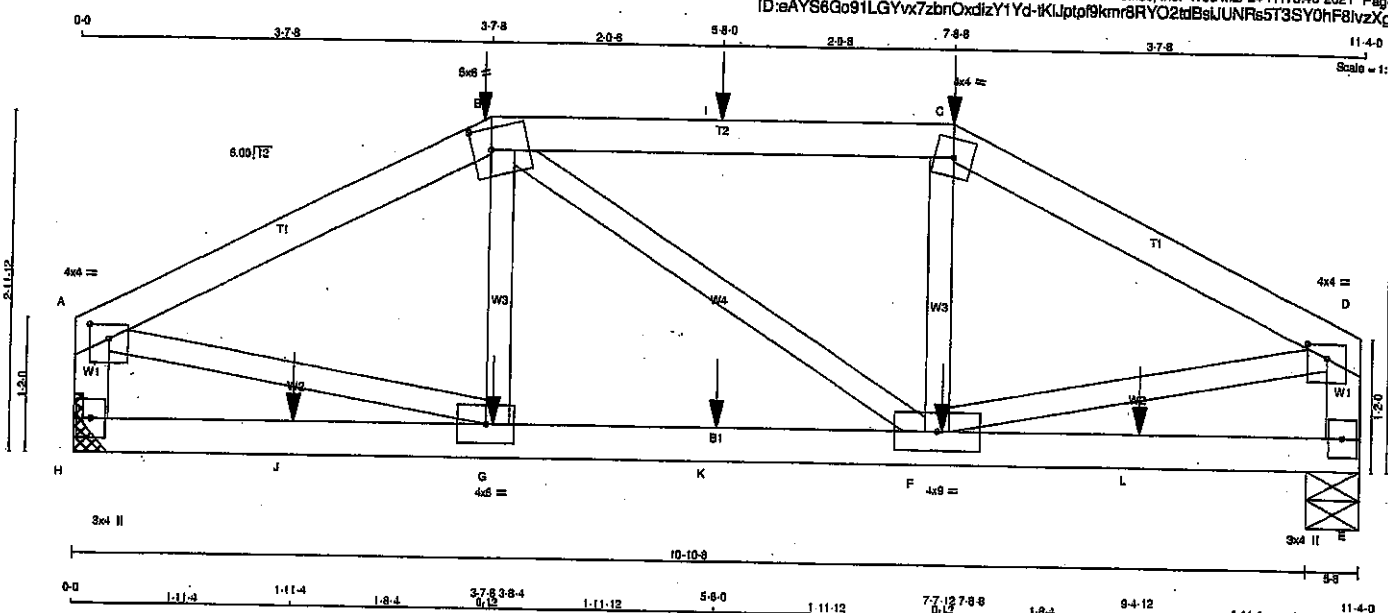


Structural component only
DWG# T-2109025 8/2

JOB NAME 417566	TRUSS NAME T66	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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Scale = 1:18.5



LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS			
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
H - A	2x4	DRY	No.2
E - D	2x4	DRY	No.2
H - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-p	MT20	4.0	4.0	1.50 2.00
B	TTWV-m	MT20	5.0	6.0	2.25 2.00
C	TTW-m	MT20	4.0	4.0	
D	TMVW-p	MT20	4.0	4.0	1.50 2.00
E	BMV1-p	MT20	3.0	4.0	
F	BMVWW-I	MT20	4.0	9.0	
G	BMVWW-I	MT20	4.0	6.0	
H	BMV1-p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT RECD
	VERT	HORZ	BRG
JT			IN-SX
H	802	0	MECHANICAL
E	802	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST LC CASE MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
H	588	370 / 0	0 / 0	0 / 0	0 / 0	198 / 0
E	588	370 / 0	0 / 0	0 / 0	0 / 0	198 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PL)	MAX. UNBRACED LENGTH (LC)
FR-TO			
A-B	-923 / 0	-91.8	-91.8 0.24 (1)
B-I	-823 / 0	-91.8	-91.8 0.38 (1)
I-C	-823 / 0	-91.8	-91.8 0.38 (1)
C-D	-923 / 0	-91.8	-91.8 0.24 (1)
H-A	-771 / 0	0.0	0.0 0.09 (1)
E-D	-789 / 0	0.0	0.0 0.09 (1)
H-J	0 / 0	-18.5	-18.5 0.07 (4)
J-G	0 / 0	-18.5	-18.5 0.07 (4)
G-K	0 / 821	-18.5	-18.5 0.18 (1)
K-F	0 / 821	-18.5	-18.5 0.18 (1)
F-L	0 / 0	-18.5	-18.5 0.08 (4)
L-E	0 / 0	-18.5	-18.5 0.08 (4)

SUPPORTED CONCENTRATED LOADS (LBS)			
JT	LOC.	LC1	MAX.
B	3-7-8	-97	-97
C	7-8-8	-97	-97
F	7-7-12	-8	-8
G	3-8-4	-8	-8
I	5-8-0	-23	-23
J	1-11-4	-5	-5
K	5-8-0	-8	-8
L	9-4-12	-5	-5

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.36/1.00 (B-C:1), BC=0.18/1.00 (F-G:1), WB=0.21/1.00 (D-F:1), SS=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

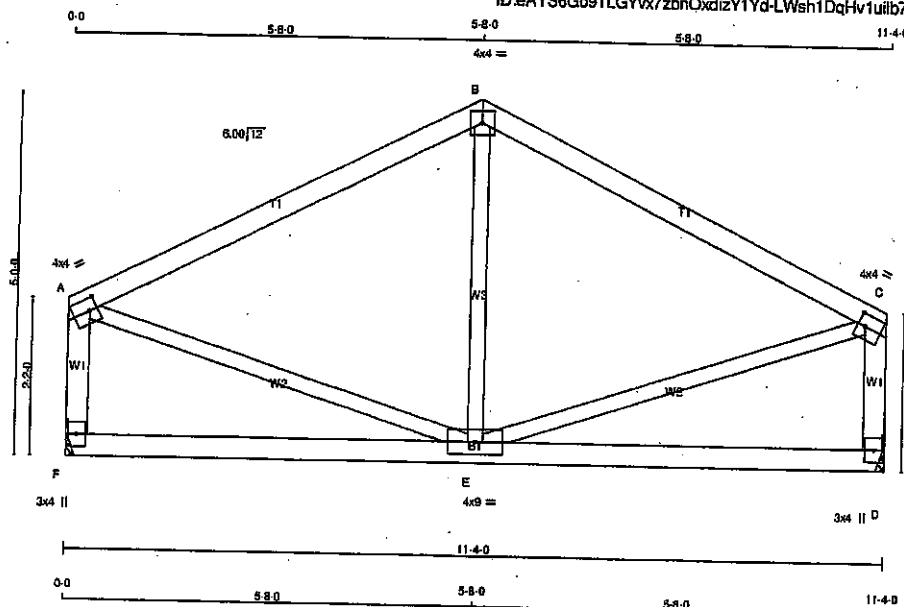
JSI GRIP = 0.82 (D) (INPUT = 0.90)
JSI METAL = 0.28 (D) (INPUT = 1.00)



Structural component only
DWG# T-2109026

JOB NAME 417566	TRUSS NAME T67	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF		
B - C	2x4	DRY	No.2	SPF		
F - A	2x4	DRY	No.2	SPF		
D - C	2x4	DRY	No.2	SPF		
F - D	2x4	DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - TMVW-t	MT20	4.0	4.0	2.00	1.25
B - TTW-p	MT20	4.0	4.0		
C - TMVW-t	MT20	4.0	4.0	2.00	1.25
D - BMV1+p	MT20	3.0	4.0		
E - BMVW-t	MT20	4.0	9.0		
F - BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORIZ
F	625	0	625	0
D	625	0	625	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	442	290	0	0	0	152	0
D	442	290	0	0	0	152	0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	-438 / 0	-91.8 -91.8 0.38 (1)	E-B	-128 / 50
B-C	-438 / 0	-91.8 -91.8 0.38 (1)	A-E	0 / 412
F-A	-584 / 0	0.0 0.0 0.07 (1)	E-C	0 / 412
D-C	-584 / 0	0.0 0.0 0.07 (1)		
F-E	0 / 0	-18.5 -18.5 0.17 (4)		
E-D	0 / 0	-18.5 -18.5 0.17 (4)		

TOTAL WEIGHT = 44 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/360 (0.38")
CALCULATED VERT. DEF. (LL) = L/999 (0.01")
ALLOWABLE DEF. (TL) = L/360 (0.38")
CALCULATED VERT. DEF. (TL) = L/999 (0.03")

CSI: TC=0.38/1.00 (B-C:1), BC=0.17/1.00 (E-F:4),
WB=0.09/1.00 (A-E:1), SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (A) (INPUT = 0.90)

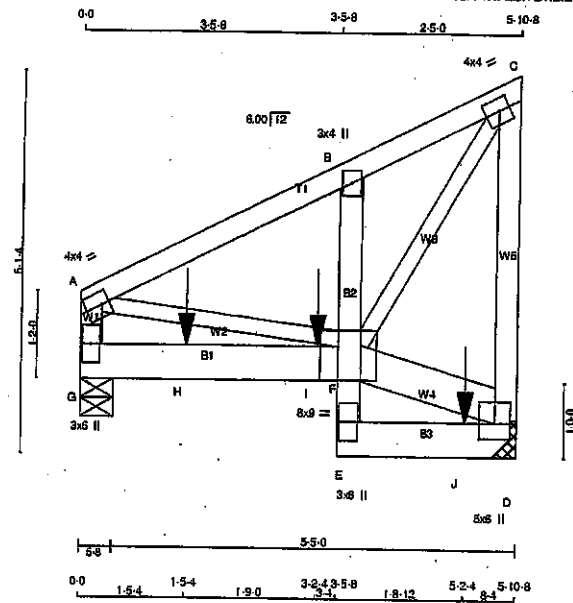
JSI METAL = 0.19 (A) (INPUT = 1.00)



Structural component only
DWG# T-2109027

JOB NAME 417566	TRUSS NAME T68S	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1/28.8

TOTAL WEIGHT = 2 X 37 = 73 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
G - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT F - D	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - D	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
G - F	12	SIDE (0.0)
E - D	12	SIDE (0.0)
B - E	12	TOP
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	6	
2x6	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT
G	950	0	950	0	5-8
D	1353	0	1353	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	688	480 / 0	0 / 0	0 / 0	0 / 0	207 / 0	0 / 0
D	954	642 / 0	0 / 0	0 / 0	0 / 0	312 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
G-A	-871 / 0	A-F	0 / 759
A-B	-820 / 0	F-D	-58 / 0
B-C	-812 / 0	F-C	0 / 1367
D-C	-893 / 0		
G-H	0 / 0		
H-I	0 / 0		
I-F	0 / 0		
E-F	0 / 205		
F-B	-311 / 0		
E-J	0 / 53		
J-D	0 / 53		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
H	1-5-4	-177	-177	---	TOP	VERT	TOTAL	C1
I	3-2-4	-557	-557	---	FRONT	VERT	TOTAL	C1
J	5-2-4	-429	-429	---	FRONT	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:		
TOP CH.	LL	= 25.6 PSF
	DL	= 6.0 PSF
BOT CH.	LL	= 0.0 PSF
	DL	= 7.4 PSF
TOTAL LOAD		= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TO=0.18/1.00 (C-0.1), BC=0.14/1.00 (F-G-1), WB=0.17/1.00 (C-F-1), SS=0.28/1.00 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

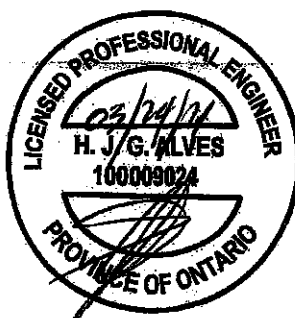
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (C) (INPUT = 0.90)
JSI METAL= 0.16 (C) (INPUT = 1.00)



Structural component only
DWG# T-2109028

JOB NAME 417566	TRUSS NAME T68S	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

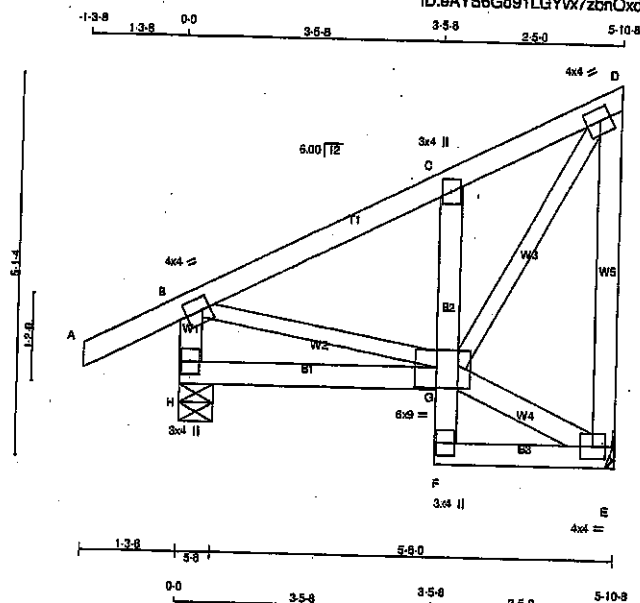
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TMV+p	MT20	3.0	4.0		
C	TMVW-l	MT20	4.0	4.0	2.00	1.75
D	BMVW1+p	MT20	5.0	6.0	2.50	2.50
E	BMV+p	MT20	3.0	6.0		
F	BMVWW-l	MT20	8.0	9.0	5.50	2.75
G	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2109028 *42*

JOB NAME 417566	TRUSS NAME T69S	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



TOTAL WEIGHT = 34 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
H - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
H - G	2x4	DRY	No.2
F - C	2x4	DRY	No.2
F - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
G - E	2x4	DRY	No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00 1.25
C	TMV+p	MT20	3.0	4.0	
D	TMVW-t	MT20	4.0	4.0	2.00 1.75
E	BMVW-t	MT20	4.0	4.0	
F	BMV+p	MT20	3.0	4.0	
G	BMVWVW-t	MT20	6.0	9.0	3.25 3.50
H	BMV1+p	MT20	3.0	4.0	

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT	IN-SX
H	448	0	448	0
E	324	0	324	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	315	220 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
E	229	130 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM	TO		FR-TO		
H-B	-415 / 0	0.0	0.0	0.04 (1)	7.81	B-G	0 / 218
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	G-E	-17 / 0
B-C	-224 / 0	-91.8	-91.8	0.14 (1)	8.25	G-D	0 / 385
C-D	-240 / 0	-91.8	-91.8	0.14 (1)	8.25		
E-D	-296 / 0	0.0	0.0	0.13 (1)	7.81		
H-G	0 / 0	-18.5	-18.5	0.07 (4)	10.00		
F-G	0 / 24	0.0	0.0	0.02 (1)	10.00		
G-C	-334 / 0	0.0	0.0	0.02 (1)	7.81		
F-E	0 / 16	-18.5	-18.5	0.03 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 8.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPO 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.14/1.00 (B-C:1), BC=0.07/1.00 (G-H:4), WB=0.09/1.00 (D-G:1), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (D) (INPUT = 0.80)
JSI METAL= 0.12 (B) (INPUT = 1.00)

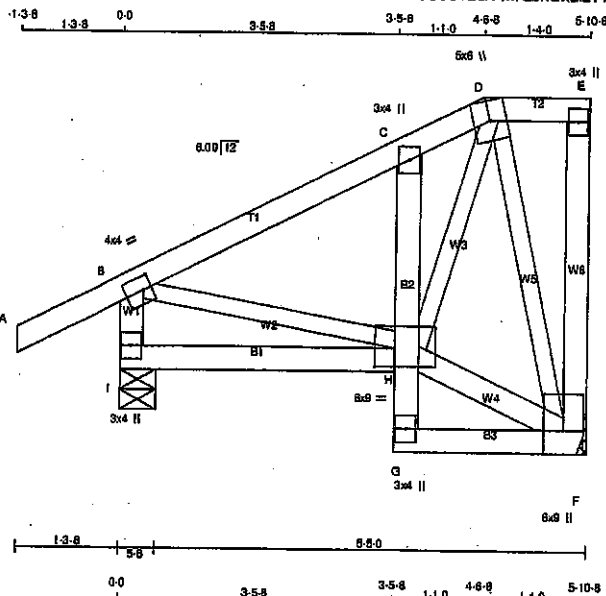


Structural component only
DWG# T-2109029

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417566	T70S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/25.0

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
G - C	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	TTWW+m	MT20	5.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVWW1+p	MT20	6.0	9.0	Edge	
G	BMV+p	MT20	3.0	4.0		
H	BVMWWW-I	MT20	6.0	9.0	2.75	3.00
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	324	0	324	0	0	0
I	448	0	448	0	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LC CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERMALIVE			
F	229	150	0	0	0	79	0
I	315	220	0	0	0	95	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED			MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
		VERT. LOAD (PLF)	LC1 CSI (LC)	MAX. UNBRAC LENGTH					
FR-TO		FROM	TO		FR-TO				
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	H-F	0 / 77	0.01 (1)	
B-C	-223 / 0	-91.8	-91.8	0.14 (1)	8.25	H-D	0 / 376	0.08 (1)	
C-D	-262 / 0	-91.8	-91.8	0.13 (1)	6.25	B-H	0 / 217	0.05 (1)	
D-E	0 / 0	-91.8	-91.8	0.03 (1)	10.00	D-F	-287 / 0	0.08 (1)	
F-E	-80 / 0	0.0	0.0	0.02 (1)	7.81				
I-B	-415 / 0	0.0	0.0	0.04 (1)	7.81				
I-H	0 / 0	-18.5	-18.5	0.07 (4)	10.00				
G-H	0 / 24	0.0	0.0	0.02 (1)	10.00				
H-C	-324 / 0	0.0	0.0	0.02 (1)	7.81				
G-F	0 / 16	-18.5	-18.5	0.03 (4)	10.00				

TOTAL WEIGHT = 35 b

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBCO 2015, ABC 2019
- PART 9 OF CBCO 2012 (2019 AMENDMENT)
- CSA 089-14
- TPIC 2014

(85 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.14/1.00 (B-C:1), BC=0.07/1.00 (H-I:4), WS=0.08/1.00 (D-H:1), SS=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	.650 .371	1747 788 .1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

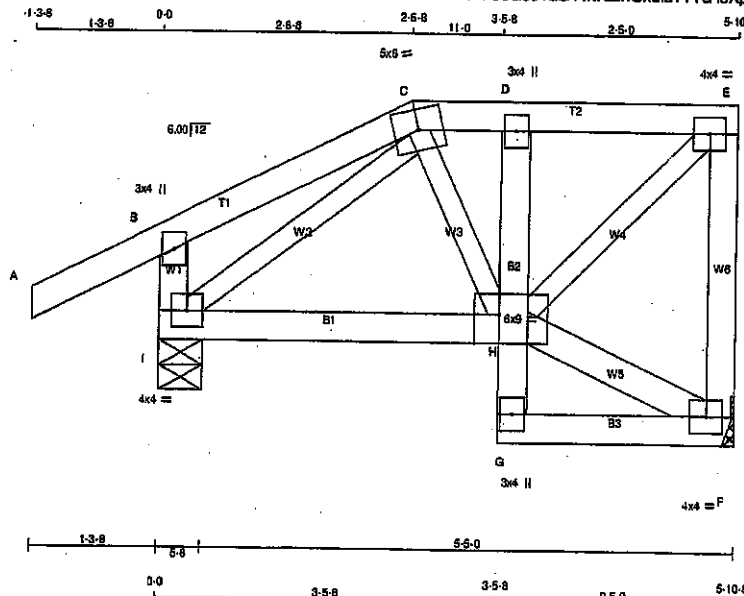
JSI GRIP = 0.35 (B) (INPUT = 0.90)
JSI METAL = 0.12 (B) (INPUT = 1.00)



Structural component only
DWG# T-2109030

JOB NAME	TRUSS NAME	QUANTITY	PLY.	JOB DESC.	DRWG NO.
417566	T71S	1	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/2" = 1'-0"

TOTAL WEIGHT = 31 b

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2		
C - E	2x4	DRY	No.2		
F - E	2x4	DRY	No.2		
I - B	2x4	DRY	No.2		
I - H	2x4	DRY	No.2		
G - D	2x4	DRY	No.2		
G - F	2x4	DRY	No.2		
ALL WEBS	2x3	DRY	No.2		
EXCEPT	H - F	2x4	DRY	No.2	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B - TMV+p	MT20	3.0	4.0			
C - TTWW-m	MT20	5.0	6.0			
D - TMV+p	MT20	3.0	4.0			
E - TMVW-t	MT20	4.0	4.0			
F - BMVW-t	MT20	4.0	4.0			
G - BMV+p	MT20	3.0	4.0			
H - BMVWW-t	MT20	6.0	9.0	Edge	3.00	
I - BMVW-t	MT20	4.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
F	324	0	324	0	0	0
I	448	0	448	0	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LOASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	229	150 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0
F	315	220 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	C-H	0 / 42	0.01 (4)
B-C	0 / 0	-91.8	-91.8 0.10 (1)	10.00	H-F	-11 / 0	0.00 (1)
C-D	-225 / 0	-91.8	-91.8 0.04 (1)	6.25	I-E	0 / 304	0.07 (1)
D-E	-221 / 0	-91.8	-91.8 0.06 (1)	6.25	I-C	-269 / 0	0.05 (1)
E-F	-299 / 0	0.0	0.0 0.06 (1)	7.81			
I-B	-243 / 0	0.0	0.0 0.02 (1)	7.81			
I-H	0 / 207	-18.5	-18.5 0.09 (4)	10.00			
G-H	0 / 24	0.0	0.0 0.02 (1)	10.00			
H-D	-197 / 0	0.0	0.0 0.01 (1)	7.81			
G-F	0 / 10	-18.5	-18.5 0.03 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TO=0.12/1.00 (A-B:1), BC=0.09/1.00 (H-I:4), WB=0.07/1.00 (E-H:1), SI=0.09/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSD) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

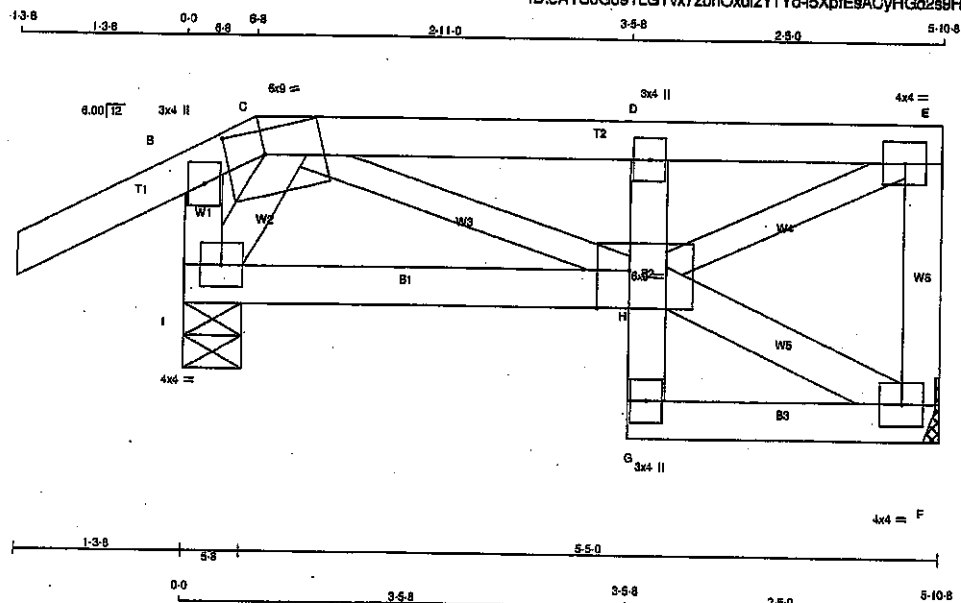
JSI GRIP= 0.30 (E) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)



Structural component only
DWG# T-2109031

JOB NAME 417566	TRUSS NAME T72S	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:18.3

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
F - H	2x4 DRY	No.2	SPF
I - B	2x4 DRY	No.2	SPF
J - H	2x4 DRY	No.2	SPF
G - D	2x4 DRY	No.2	SPF
G - F	2x4 DRY	No.2	SPF
ALL WEBS	2x4 DRY	No.2	SPF
EXCEPT			
H - E	2x3 DRY	No.2	SPF
C - H	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0	
C	TTWW-m	MT20	6.0	9.0	2.25 3.50
D	TMV-p	MT20	3.0	4.0	
E	TMVW-t	MT20	4.0	4.0	
F	BMVW-t	MT20	4.0	4.0	
G	BMV-p	MT20	3.0	4.0	
H	BMVWW-t	MT20	6.0	9.0	Edge 3.00
I	BMVW-t	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	JT	GROSS REACTION	BRG	BRG	BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	309	0	309	0	0	MECHANICAL	MECHANICAL
I	463	0	463	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	219	142 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
I	325	228 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8 0.12 (1)	10.00	H-F	-27 / 0	0.00 (1)	0.00 (1)
B-C	-68 / 0	-91.8 -91.8 0.11 (1)	6.25	H-E	0 / 448	0.10 (1)	0.10 (1)
C-D	-421 / 0	-91.8 -91.8 0.11 (1)	6.25	I-C	-148 / 0	0.02 (1)	0.02 (1)
D-E	-400 / 0	-91.8 -91.8 0.09 (1)	6.25	C-H	0 / 379	0.09 (1)	0.09 (1)
F-E	-278 / 0	0.0 0.0 0.03 (1)	7.81				
I-B	-298 / 0	0.0 0.0 0.03 (1)	7.81				
I-H	0 / 66	-18.5 -18.5 0.08 (4)	10.00				
G-H	0 / 24	0.0 0.0 0.04 (1)	10.00				
H-D	-292 / 0	0.0 0.0 0.03 (1)	7.81				
G-F	0 / 25	-18.5 -18.5 0.03 (4)	10.00				

TOTAL WEIGHT = 28 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.12/1.00 (A-B:1), BC=0.08/1.00 (H:4), WB=0.10/1.00 (E-H:1), SS=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

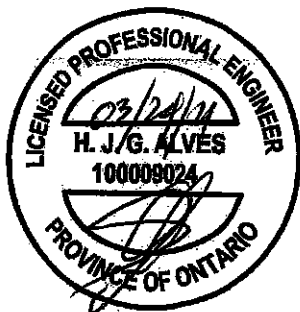
NAIL VALUES			
PLATE	GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 768	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (E) (INPUT = 0.90)

JSI METAL= 0.14 (E) (INPUT = 1.00)

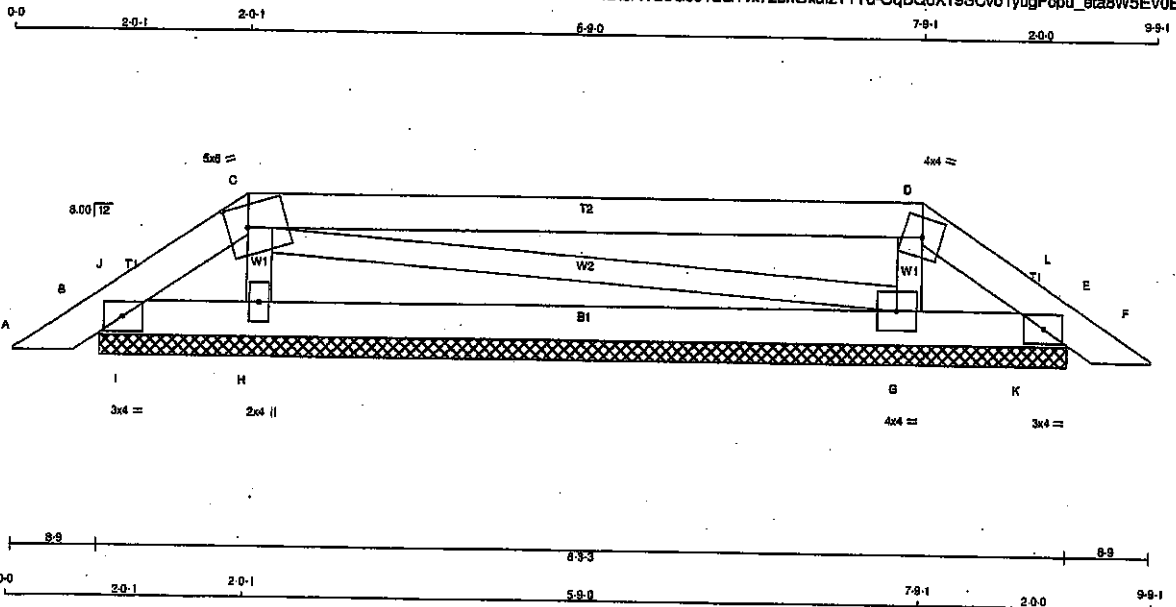


Structural component only
DWG# T-2109032

JOB NAME 417564	TRUSS NAME PB1	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:18.0

LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS 2x3 DRY			
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	132	0	132	0	8-3-3	8-3-3
E	129	0	129	0	8-3-3	8-3-3
H	382	0	382	0	8-3-3	8-3-3
G	386	0	386	0	8-3-3	8-3-3

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.52/1.00 (C-D:1), 80=0.10/1.00 (H-I:4), WB=0.04/1.00 (D-G:1), SS=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.21 (D) (INPUT = 0.90)
JSI METAL= 0.08 (C) (INPUT = 1.00)

UNFACTORED REACTIONS

1ST LC CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SCIL
B	89	85.0	0.0	0.0	0.0	4.0	0.0
E	87	83.0	0.0	0.0	0.0	4.0	0.0
H	274	159.0	0.0	0.0	0.0	115.0	0.0
G	276	161.0	0.0	0.0	0.0	115.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 15	-91.8 -91.8 0.03 (1)	10.00	H-C	-267 / 0	0.04 (1)	
B-J	-38 / 15	-91.8 -91.8 0.03 (4)	6.25	C-G	-4 / 0	0.00 (1)	
J-C	-62 / 0	-91.8 -91.8 0.02 (4)	6.25	G-D	-270 / 0	0.04 (1)	
C-D	-11 / 0	-91.8 -91.8 0.02 (1)	6.25	I-J	-95 / 0	0.00 (1)	
D-L	-57 / 0	-91.8 -91.8 0.02 (4)	6.25	K-L	-98 / 0	0.00 (1)	
L-E	-33 / 16	-91.8 -91.8 0.03 (4)	6.25				
E-F	0 / 15	-91.8 -91.8 0.03 (1)	10.00				
B-I	0 / 46	-18.5 -18.5 0.03 (1)	10.00				
I-H	0 / 48	-18.5 -18.5 0.10 (4)	10.00				
H-G	0 / 15	-18.5 -18.5 0.10 (4)	10.00				
G-K	0 / 42	-18.5 -18.5 0.10 (4)	10.00				
K-E	0 / 42	-18.5 -18.5 0.03 (1)	10.00				

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-I	MT20	4.0	4.0		
H	BMW1-w	MT20	2.0	4.0		

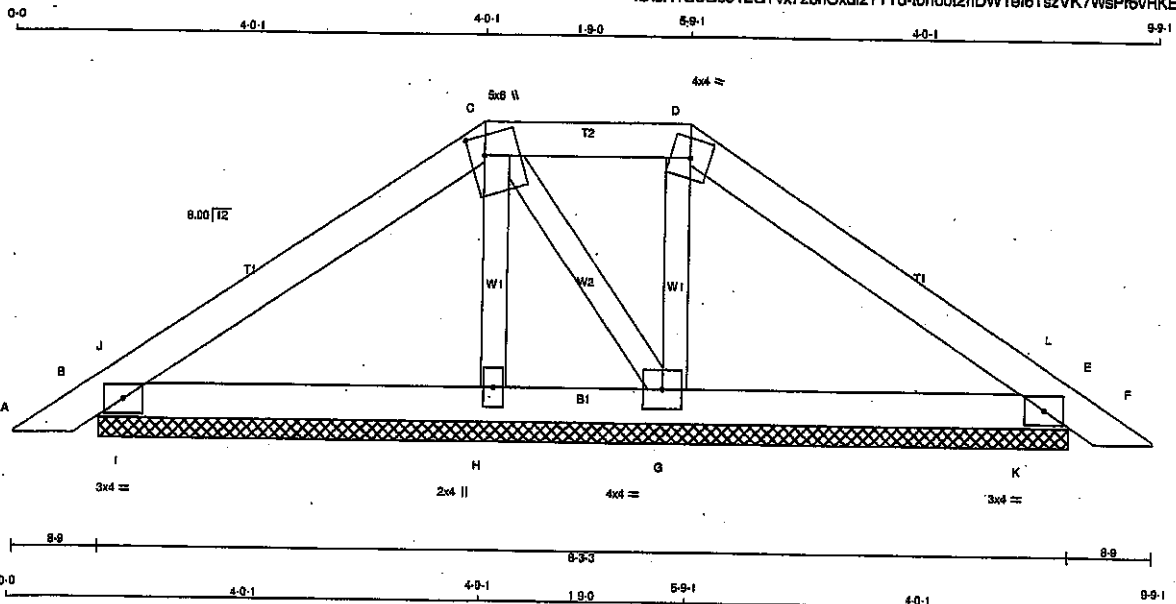
NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108942

JOB NAME 417564	TRUSS NAME PB2	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1/16.0

TOTAL WEIGHT = 2 X 27 = 54 lb

LUMBER	DESQR.
N. L. G. A. RULES	
CHORDS SIZE	
A - C 2x4 DRY No.2	SPF
C - D 2x4 DRY No.2	SPF
D - F 2x4 DRY No.2	SPF
B - E 2x4 DRY No.2	SPF
ALL WEBS 2x3 DRY No.2	SPF
DRY: SEASONED LUMBER.	

PLATES (tablets in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMB1-J MT20	3.0	4.0		
C TTW+m MT20	5.0	6.0	2.00	1.50
D TTW-m MT20	4.0	4.0		
E TMB1-J MT20	3.0	4.0		
G BMW1-J MT20	4.0	4.0		
H BMW1-w MT20	2.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT DOWN	DOWN	IN-SX	IN-SX
B	230 0	230 0	8-3-3	8-3-3
E	273 0	273 0	8-3-3	8-3-3
H	188 0	188 0	8-3-3	8-3-3
G	278 0	278 0	8-3-3	8-3-3

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL	
B 203 144 0 0 0 0 59 0 0 0	
E 191 135 0 0 0 0 57 0 0 0	
H 135 80 0 0 0 0 55 0 0 0	
G 197 130 0 0 0 0 56 0 0 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO						FR-TO		
A-B	0 / 15	-91.8	-91.8	0.03 (1)	10.00	H-C	-111 / 0	0.02 (1)
B-J	-57 / 0	-91.8	-91.8	0.04 (1)	6.25	C-G	-45 / 0	0.01 (1)
J-C	-112 / 0	-91.8	-91.8	0.12 (1)	6.25	G-D	-164 / 0	0.03 (1)
C-D	-56 / 0	-91.8	-91.8	0.05 (1)	6.25	D-J	-235 / 0	0.00 (1)
D-L	-82 / 0	-91.8	-91.8	0.12 (1)	6.25	K-L	-238 / 0	0.00 (1)
L-E	-31 / 0	-91.8	-91.8	0.04 (1)	6.25			
E-F	0 / 15	-91.8	-91.8	0.03 (1)	10.00			
B-I	0 / 28	-18.5	-18.5	0.11 (1)	10.00			
H-G	0 / 28	-18.5	-18.5	0.11 (1)	10.00			
G-K	0 / 64	-18.5	-18.5	0.07 (1)	10.00			
K-E	0 / 64	-18.5	-18.5	0.11 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (D-L:1), SC=0.11/1.00 (B-L:1), WB=0.03/1.00 (D-G:1), SS=0.18/1.00 (E-K:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

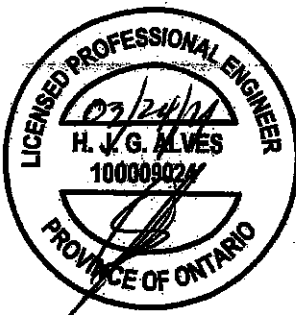
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1997 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

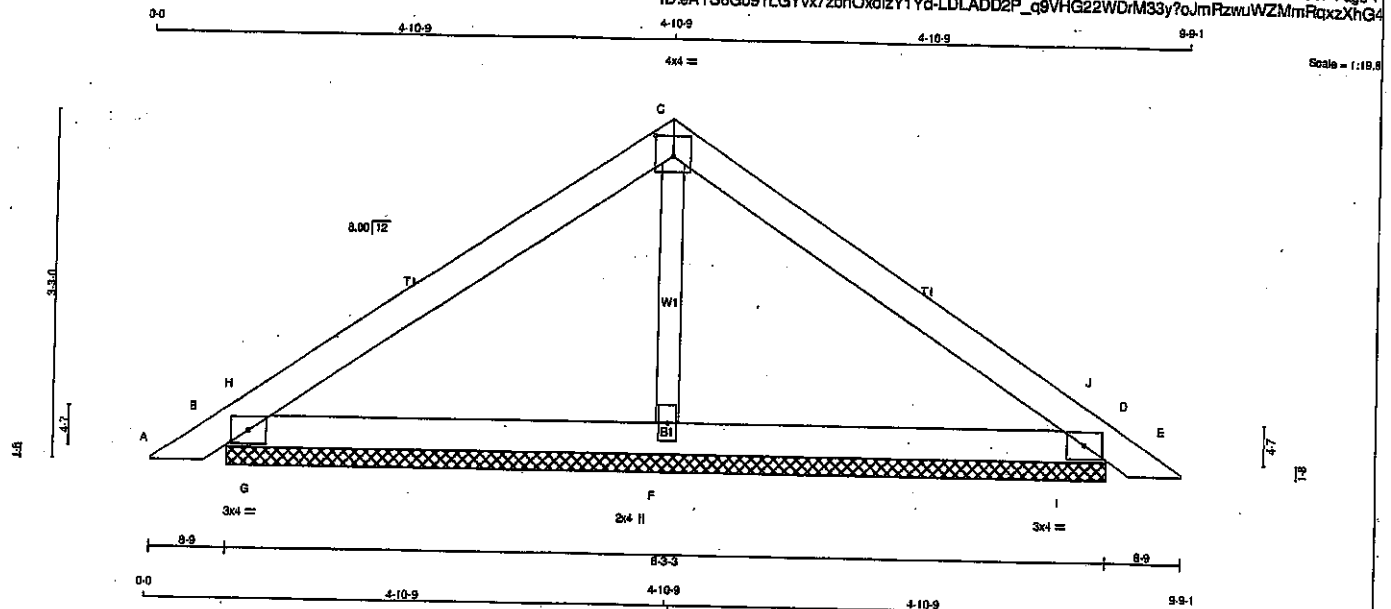
JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108943

JOB NAME 417564	TRUSS NAME PB3	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 3 X 25 = 75 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-p	MT20	4.0	4.0	2.25 2.00
D	TMB1-I	MT20	3.0	4.0	
F	BMW1-w	MT20	2.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	344	0	344	0	8-3-3	8-3-3
D	344	0	344	0	8-3-3	8-3-3
F	341	0	341	0	8-3-3	8-3-3

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND PERM/LIVE	DEAD	SOIL
	SNOW	LIVE	SNOW	LIVE			
B	241	172 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
D	241	172 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
F	244	145 / 0	0 / 0	0 / 0	0 / 0	99 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 15	-91.8	-91.8 0.03 (1)	10.00	F-C	-155 / 0	0.03 (1)
B-H	-34 / 0	-91.8	-91.8 0.09 (1)	8.25	G-H	-411 / 0	0.00 (1)
H-C	-155 / 0	-91.8	-91.8 0.18 (1)	6.25	I-J	-411 / 0	0.00 (1)
C-J	-155 / 0	-91.8	-91.8 0.19 (1)	6.25			
J-D	-34 / 0	-91.8	-91.8 0.09 (1)	6.25			
D-E	0 / 15	-91.8	-91.8 0.03 (1)	10.00			
B-G	0 / 120	-18.5	-18.5 0.18 (1)	10.00			
G-F	0 / 120	-18.5	-18.5 0.18 (1)	10.00			
F-I	0 / 120	-18.5	-18.5 0.18 (1)	10.00			
I-D	0 / 120	-18.5	-18.5 0.18 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA D86-14
- TPIG 2014

(55% OF 21.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.19/1.00 (C-J:1), BC=0.18/1.00 (F-I:1), WB=0.09/1.00 (C-F:1), SS=0.31/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

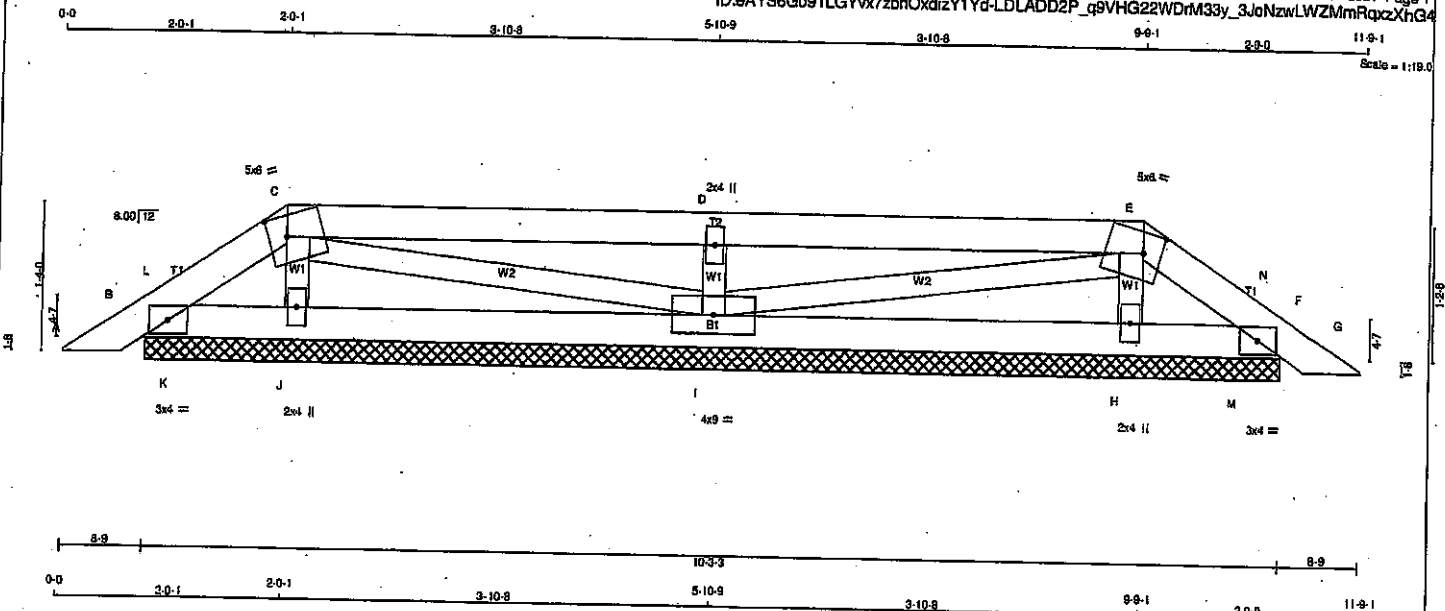
JSI GRIP= 0.29 (D) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108944

JOB NAME 417564	TRUSS NAME PB4	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MTek Industries, Inc. Wed Mar 24 16:11:37 2021 Page 1
ID: 3AYS6Go91LGYvx7zbnOxdizY1Yd-LDADD2P_g9VHG22WDrM33y_3JoNzwlWZMmRqzXhG4



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
B - F	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMB1-J	MT20	3.0	4.0		
C TTWW-m	MT20	5.0	6.0	2.25	2.00
D TMW-w	MT20	2.0	4.0		
E TTWW-m	MT20	5.0	6.0	2.25	2.00
F TMB1-I	MT20	3.0	4.0		
H BMW1-w	MT20	2.0	4.0		
I BMW1-w	MT20	4.0	9.0		
J BMW1-w	MT20	2.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	137	0	137	0	10-3-3	10-3-3
F	137	0	137	0	10-3-3	10-3-3
J	228	0	228	0	10-3-3	10-3-3
I	520	0	520	0	10-3-3	10-3-3
H	228	0	228	0	10-3-3	10-3-3

UNFACTORED REACTIONS

JT	COMBINED	MAX. MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	94	78 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
F	94	78 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
J	163	95 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
I	366	249 / 0	0 / 0	0 / 0	0 / 0	117 / 0	0 / 0
H	163	95 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS (B, F, J, I, H)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	FACTORED FORCE (LBS)	CHORDS			MEMB.	FACTORED FORCE (LBS)	WEBS		
		MAX. FACTORED	FACTORED	VERT. LOAD LC1 MAX			MAX. FACTORED	MAX. FACTORED	MAX. FACTORED
FR-TO					FR-TO				
A-B	0 / 15				J-C	-154 / 0			0.02 (1)
B-L	-32 / 0	-91.8	-91.8	0.03 (1)	C-I	-9 / 0			0.00 (1)
L-C	-36 / 0	-91.8	-91.8	0.01 (4)	I-D	-444 / 0			0.08 (1)
C-D	-2 / 0	-91.8	-91.8	0.23 (1)	D-E	-9 / 0			0.00 (1)
D-E	-2 / 0	-91.8	-91.8	0.23 (1)	E-H	-154 / 0			0.02 (1)
E-H	-36 / 0	-91.8	-91.8	0.01 (1)	H-L	-64 / 0			0.00 (1)
H-L	-32 / 0	-91.8	-91.8	0.01 (4)	L-M	-64 / 0			0.00 (1)
F-G	0 / 15	-91.8	-91.8	0.03 (1)	M-N				
B-K	0 / 28	-18.5	-18.5	0.02 (1)					
K-J	0 / 28	-18.5	-18.5	0.04 (4)					
J-I	0 / 11	-18.5	-18.5	0.06 (4)					
I-H	0 / 11	-18.5	-18.5	0.06 (4)					
H-M	0 / 28	-18.5	-18.5	0.04 (4)					
M-F	0 / 28	-18.5	-18.5	0.02 (1)					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCO 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (C-D:1), BC=0.08/1.00 (I-J:4), WB=0.06/1.00 (D-E:1), SS=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PS)	SECTION (PLI)
MT20	850	371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.22 (D) (INPUT = 0.80)

JSI METAL = 0.09 (D) (INPUT = 1.00)

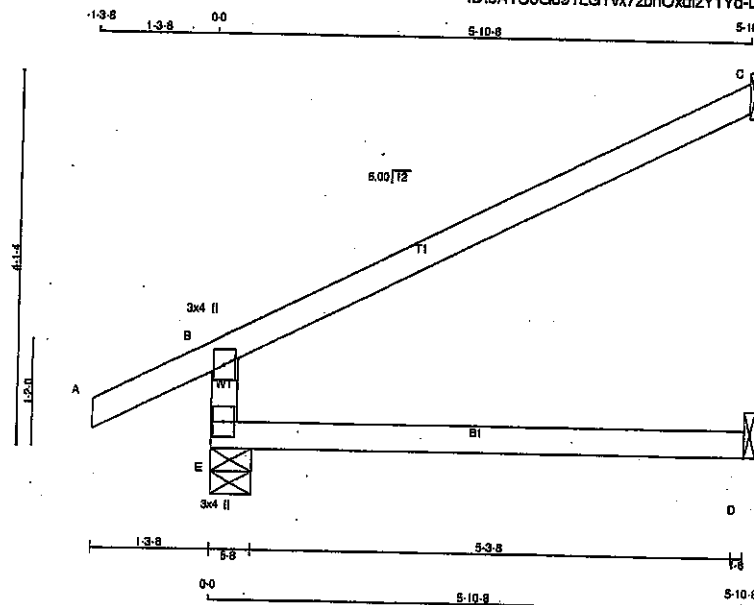


Structural component only
DWG# T-2108945

JOB NAME 417566	TRUSS NAME J1	QUANTITY 22	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1/23.3



TOTAL WEIGHT = 22 X 17 = 399 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	525	0	525	0	5-8	5-8
C	202	0	202	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	369	257.0	0.0	0.0	0.0	111.0	0.0
C	139	113.0	0.0	0.0	0.0	26.0	0.0
D	36	0.0	0.0	0.0	0.0	36.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		W E B S	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. FACTORED CS1 (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB. MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO						
E-B	-481.0	0.0	0.0	0.13 (4)	7.81	
A-B	0.28	-91.8	-91.8	0.12 (1)	10.00	
B-C	-30.0	-91.8	-91.8	0.54 (1)	6.25	
E-D	0.0	-18.5	-18.5	0.13 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BOBC 2015, ABC 2015
- PART 9 OF OBC 2012 (2015 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CS1: TC=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (A:0), SS1=0.24/1.00 (B-C:1).

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 768 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

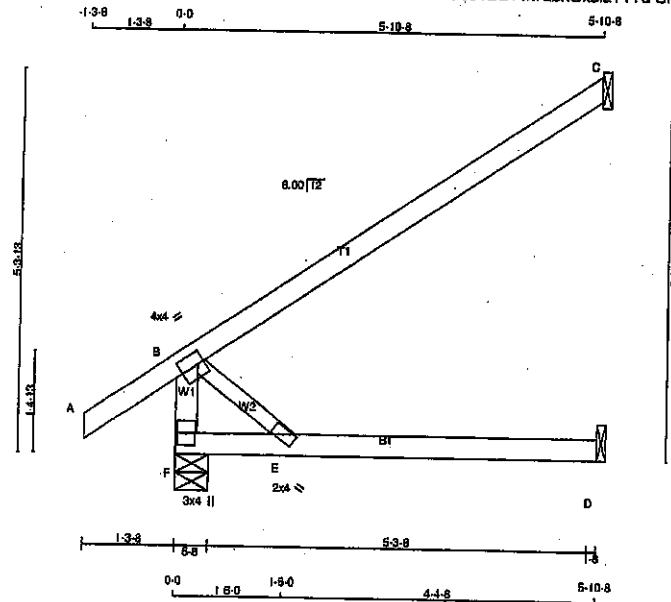
JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108997

JOB NAME 417564	TRUSS NAME J1	QUANTITY 14	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/25.4

TOTAL WEIGHT = 14 X 19 = 269 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - 8	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW-I	MT20	4.0	4.0	2.00	1.00
E BMW-w	MT20	2.0	4.0		
F BMV1-p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
JT	450	0	450	0	5-8	5-8		
F	270	0	270	0	1-8	1-8		
C	270	0	270	0	1-8	1-8		
D	54	0	54	0	1-8	1-8		

SEE MITEK STANDARD DETAIL 697791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LOASE	MAX. MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	316	221.0	0.0	0.0	0.0	95.0	0.0
F	186	150.0	0.0	0.0	0.0	35.0	0.0
C	186	150.0	0.0	0.0	0.0	35.0	0.0
D	43	0.0	0.0	0.0	0.0	43.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
F-B	-395.0	0.0	0.0 0.04 (1)	B-E	0.0	0.00 (1)	
A-B	0.0	-91.8	-91.8 0.12 (1)				
B-C	0.0	-91.8	-91.8 0.54 (1)				
F-E	0.0	-18.5	-18.5 0.14 (4)				
E-D	0.0	-18.5	-18.5 0.19 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = L/360 (0.20")
CALCULATED VERT. DEFL (LL) = L/999 (0.00")
ALLOWABLE DEFL (TL) = L/360 (0.20")
CALCULATED VERT. DEFL (TL) = L/999 (0.05")

CSI: TC=0.54/1.00 (B-C:1), BC=0.19/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

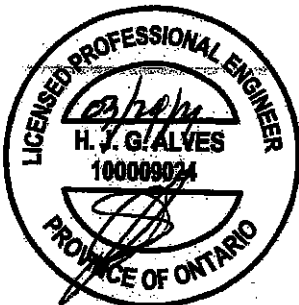
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.24 (B) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)

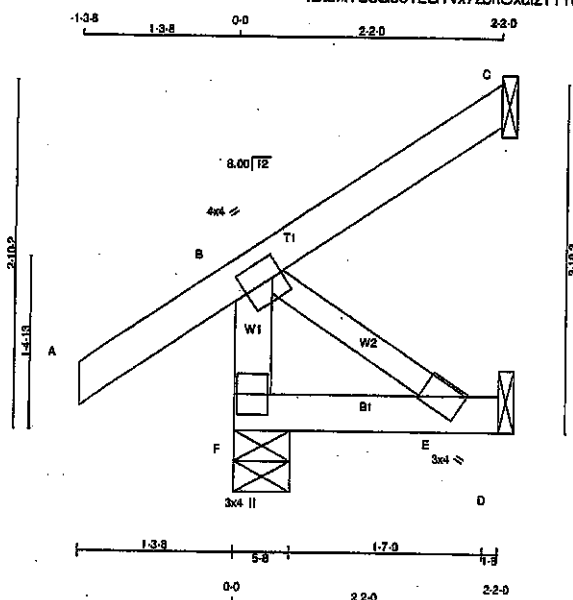


Structural component only
DWG# T-2108940

JOB NAME 417564	TRUSS NAME J2	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:17.3

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES	F - B	2x4 DRY	No.2	SPF
	A - C	2x4 DRY	No.2	SPF
	F - D	2x4 DRY	No.2	SPF
	ALL WEBS	2x3 DRY	No.2	SPF
	DRY: SEASONED LUMBER.			

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMV-w	MT20	3.0	4.0		
F	BMV-t+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	245	0	245	0	5-8	5-8
C	99	0	99	0	1-8	1-8
D	20	0	22	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	171	128.0	0.0	0.0	0.0	46.0	0.0
C	68	55.0	0.0	0.0	0.0	13.0	0.0
D	15	0.0	0.0	0.0	0.0	16.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS		MAX. UNBRACED LENGTH
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	VERT. LOAD (PLF)	CS1 (LC)			MAX. FORCE (LBS)	CS1 (LC)	
F-B	-225.0	0.0	0.0	0.02 (1)	7.81	B-E	0.0	0.00 (1)	
A-B	0.35	-91.8	-91.8	0.12 (5)	10.00				
B-C	0.0	-91.8	-91.8	0.07 (1)	10.00				
F-E	0.0	-18.5	-18.5	0.03 (4)	10.00				
E-D	0.0	-18.5	-18.5	0.02 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 10 = 40 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABO 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/960$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CS1: TC=0.12/1.00 (A-B-S), BC=0.03/1.00 (E-F-4), WB=0.00/1.00 (B-E-1), SS1=0.08/1.00 (A-B-S)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

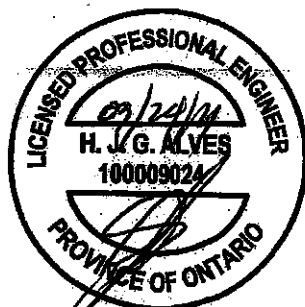
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLJ) (PLI)
MAX MIN MAX MIN
MT20 650 371 1747 788 1997 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.14 (B) (INPUT = 0.90)
JSI METAL = 0.04 (B) (INPUT = 1.00)

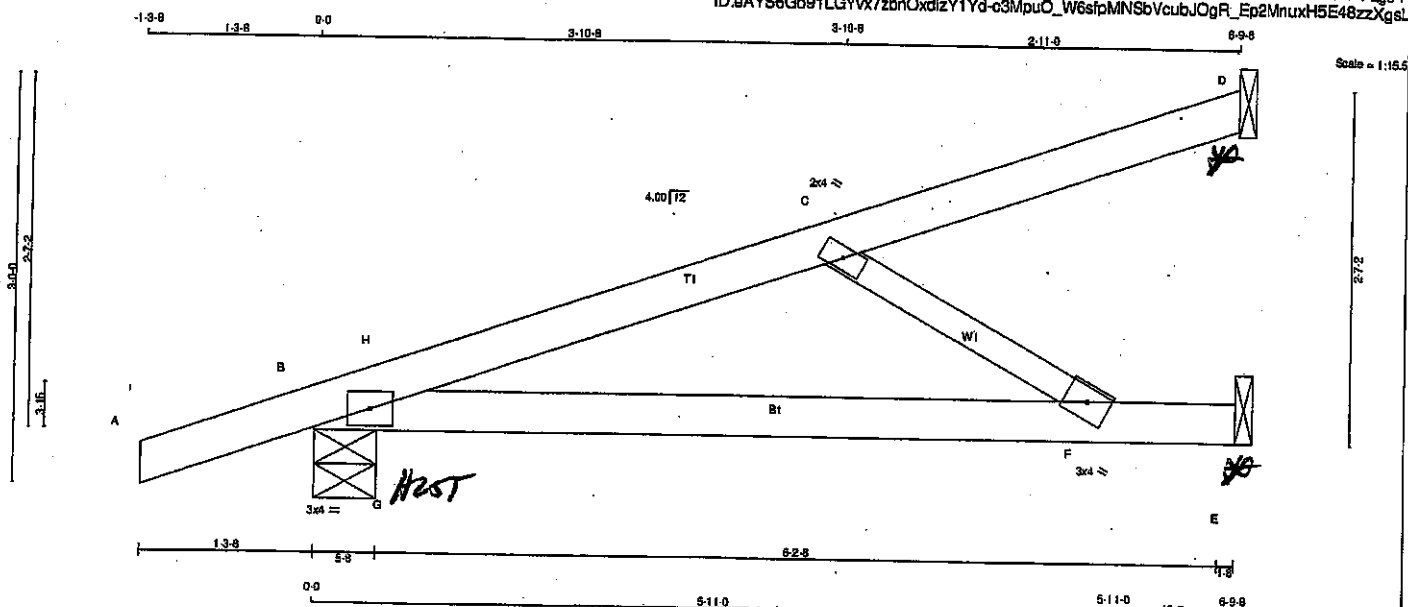


Structural component only
DWG# T-2108941

JOB NAME 417565	TRUSS NAME J30W	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Mar 24 18:39:04 2021 Page 1
ID: aYS6Go91LGYvx7zbnOxdizY1Yd-c3MpuO_W6sfpMNSbVcubJOG_R_Ep2MnuxH5E48zzXgSL



<u>LUMBER</u>			
N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
A - D	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
F	BMW+w	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
D	110	0	119	0	-81	1-8
B	497	0	497	165	-322	5-8
E	255	0	255	0	-240	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) D, E

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 322 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT E FOR 240 LBS. FACTORED UPLIFT

PROVIDE FOR 165 LBS. FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	COMBINED	MAX. MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	83	85/0	0/0	0/0	0/-59	18/0	0/0
B	349	242/0	0/0	0/0	0/-299	107/0	0/0
E	182	109/0	0/0	0/0	0/-219	73/0	0/0

HORIZONTAL REACTIONS

JT	VERT	HORZ	WIND	DEAD	SOIL
B	0/0	0/0	118/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 18	-91.8	-91.8	0.11 (1)	10.00		
B-H	-460 / 218	-91.8	-91.8	0.24 (5)	6.25	C-F	-427 / 299
H-C	-350 / 137	-91.8	-91.8	0.26 (1)	6.25	G-H	-242 / 140
C-D	-31 / 0	-91.8	-91.8	0.09 (1)	6.25		
B-G	-244 / 351	-18.5	-18.5	0.09 (12)	6.25		
G-F	-244 / 351	-18.5	-18.5	0.42 (1)	8.25		
F-E	0 / 0	-18.5	-18.5	0.36 (1)	10.00		

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.8 } PSF AT { 15-0-0 } FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_p , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST { 0-0 } FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.23")
CALCULATED VERT. DEFL. (LL) = $L/463$ (0.18")
ALLOWABLE DEFL. (TL) = $L/360$ (0.23")
CALCULATED VERT. DEFL. (TL) = $L/397$ (0.21")

CSI: $TC=0.26/1.00$ (C-H:1), $BC=0.42/1.00$ (F-G:1), $WB=0.07/1.00$ (C-F:1), $SSI=0.20/1.00$ (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSD) (PLJ) (PLJ)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 789 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

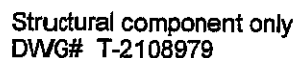
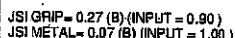
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.32 (C) (INPUT = 0.50)
JSI METAL = 0.21 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108978

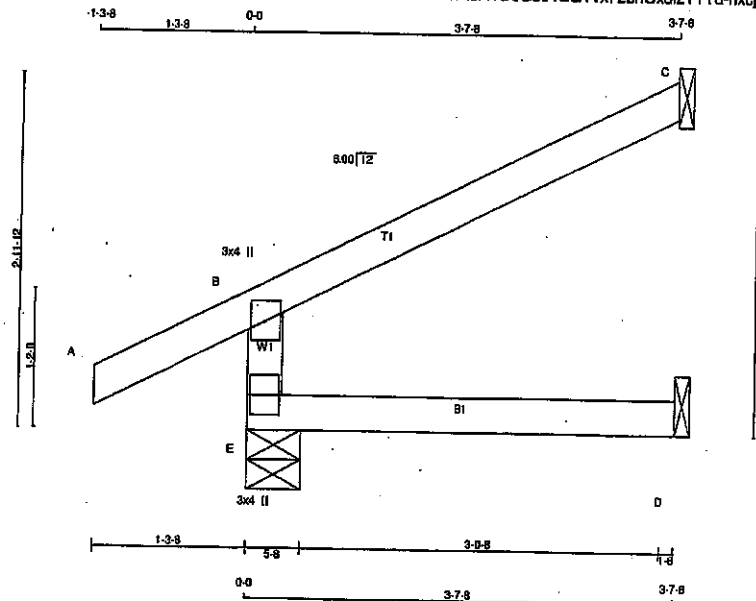
Tamarack Roof Truss, Burlington



JOB NAME 417566	TRUSS NAME J40	QUANTITY 7	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:17.5



LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF
E - B	2x4	DRY	No.2	SPF	SPF
A - C	2x4	DRY	No.2	SPF	SPF
E - D	2x4	DRY	No.2	SPF	SPF

DRY, SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS				MAXIMUM FACTORED GROSS REACTION				INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	5-8	5-8	1-8	1-8
E	371	0	371	0	0	5-8	5-8	5-8	5-8	1-8	1-8
C	125	0	125	0	0	1-8	1-8	1-8	1-8	1-8	1-8
D	29	0	32	0	0	1-8	1-8	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS				DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	DEAD	SOIL
E	259	185.0	0.0	0.0	0.0	74.0	0.0	0.0	0.0
C	86	70.0	0.0	0.0	0.0	16.0	0.0	0.0	0.0
D	23	0.0	0.0	0.0	0.0	23.0	0.0	0.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		MAX. FACTORED FORCE				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	FR-TO	MEMB.	FORCE (LBS)	MAX. CSI (LC)	FR-TO
E-B	-332.0	0.0	0.0	0.04 (4)	7.81	E-B	0.0	0.0	0.04 (4)
A-B	0.0	-91.8	-91.8	0.13 (5)	10.00	A-B	-91.8	-91.8	0.13 (5)
B-C	-19.0	-91.8	-91.8	0.20 (1)	6.25	B-C	-91.8	-91.8	0.20 (1)
E-D	0.0	-18.5	-18.5	0.05 (4)	10.00	E-D	-18.5	-18.5	0.05 (4)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 7 X 11 = 80 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.20/1.00 (B-C:1), BC=0.05/1.00 (D-E:4), WB=0.00/1.00 (W-B:0), SS=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	850 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.13 (E) (INPUT = 0.90)

JSI METAL = 0.09 (B) (INPUT = 1.00)

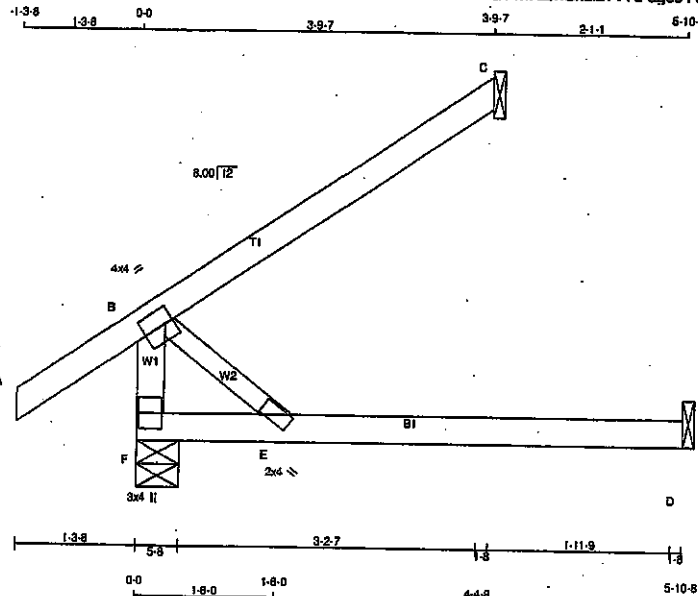


Structural component only
DWG# T-2108998

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417564	C1	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/22.7

TOTAL WEIGHT = 2 X 16 = 33 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DESCR.

SPF

SPF

SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECD BRG
	VERT	HORZ	DOWN	HORZ		
F	354	0	354	0	5-8	5-8
C	174	0	174	0	1-8	1-8
D	54	0	51	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97781H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN COMPONENT REACTIONS					
		COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
F	250	167.0	0.0	0.0	0.0	83.0	0.0
C	120	97.0	0.0	0.0	0.0	23.0	0.0
D	43	0.0	0.0	0.0	0.0	43.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)		MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB. FR-TO	WEBS MAX. FACTORED FORCE (LBS)	
		FROM	TO				FR-TO	MAX. CSI (LC)
F-B	-300.0	0.0	0.0	0.03 (1)	7.81	B-E	0.0	0.00 (1)
A-B	0.35	-91.8	-91.8	0.12 (1)	10.00			
B-C	0.0	-91.8	-91.8	0.22 (1)	10.00			
F-E	0.0	-18.5	-18.5	0.14 (4)	10.00			
E-D	0.0	-18.5	-18.5	0.19 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2015 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.05")

CSI: TO=0.22/1.00 (B-C:1), BC=0.19/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.19 (B) (INPUT=0.90)
JSI METAL=0.06 (B) (INPUT=1.00)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TN/VW+I	MT20	4.0	4.0	2.00	1.00
F	BN/VW+I	MT20	2.0	4.0		
F	BM/V+I	MT20	3.0	4.0		

NOTES- (1)

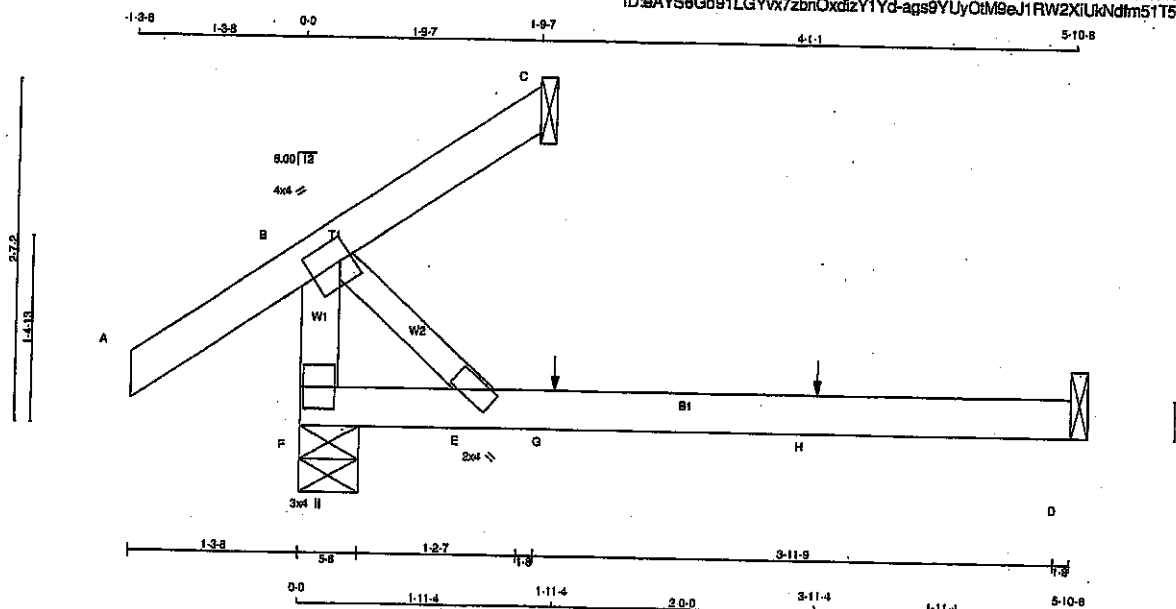
- 1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2109036

JOB NAME 417564	TRUSS NAME C2	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID: aAYS8G091LG YvX7zbnOxdzY1Yd-ags9YUyOtM9eJ1RW2XUkNdfm51T5KK6q0Y0zXhG0



LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS 2x3 DRY			
DRY: SEASONED LUMBER.			

PLATES (tablets in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+	MT20	4.0	4.0	2.00 1.00
E	BMV+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
F	311	0	311	0	0	0	5-8	5-8	
C	34	0	34	0	0	1-8	1-8		
D	54	0	54	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	220	143 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
C	23	19 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO			FR-TO			
F-B	-256 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)
A-B	0 / 35	-91.8	-91.8	0.13 (1)	10.00			
B-C	-27 / 0	-91.8	-91.8	0.12 (1)	6.25			
F-E	0 / 0	-18.5	-18.5	0.13 (4)	10.00			
E-G	0 / 0	-18.5	-18.5	0.19 (4)	10.00			
G-H	0 / 0	-18.5	-18.5	0.19 (4)	10.00			
H-D	0 / 0	-18.5	-18.5	0.19 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	--	FRONT	VERT	TOTAL	--	C1
H	3-11-4	1	1	--	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 13 = 27 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.05")

CSI: TC=0.13/1.00 (A-B:1), BC=0.19/1.00 (D-E:4),
WB=0.00/1.00 (B-E:1), SSI=0.09/1.00 (B-C:1)

DOL LUMBER=0.98 NAIL=0.98 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

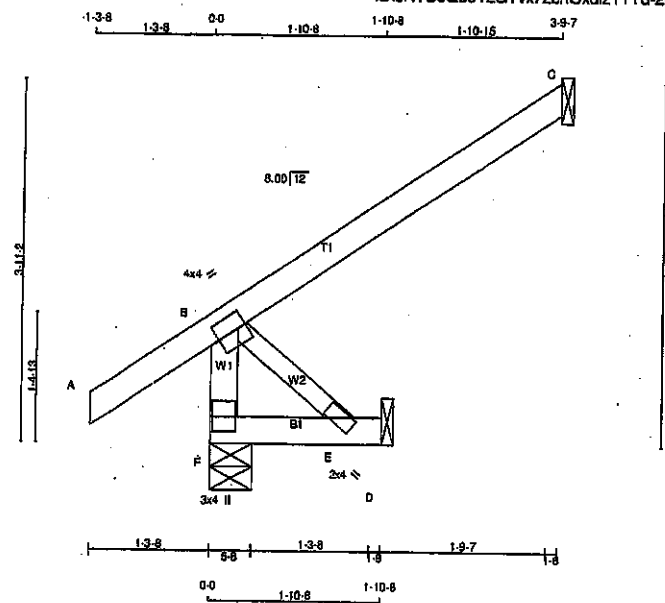
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.16 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108935

JOB NAME 417564	TRUSS NAME C3	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
F - B	2x4	DRY	No.2		SPF
A - C	2x4	DRY	No.2		SPF
F - D	2x4	DRY	No.2		SPF
ALL WEBS		2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMV-w	MT20	2.0	4.0		
F	BMV1-p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	317	0	317	0	5-8	5-8
C	174	0	174	0	1-8	1-8
D	17	0	19	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
		COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
F	220	167 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
C	120	97 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS		WEBS	
		FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
F-B	-300 / 0	0.0	0.03 (1)	7.81	0 / 0
A-B	0 / 35	-91.8	-91.8 0.13 (5)	10.00	
B-C	0 / 0	-91.8	-91.8 0.22 (1)	10.00	
F-E	0 / 0	-18.5	-18.5 0.02 (4)	10.00	
E-D	0 / 0	-18.5	-18.5 0.01 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 12 = 23 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2015 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/980 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.22/1.00 (B-C:1), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1887 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.19 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)

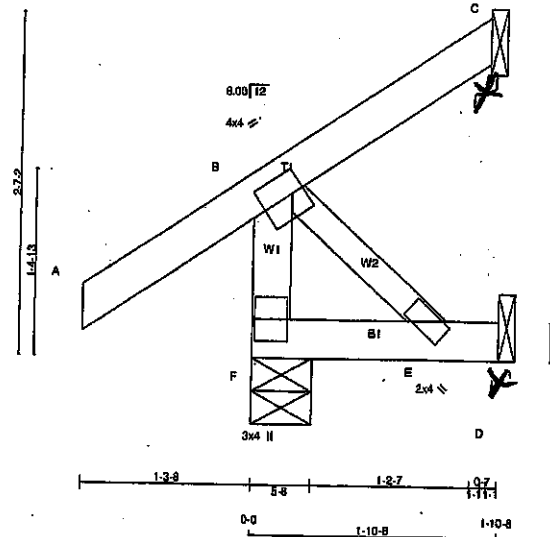


Structural component only
DWG# T-2108936

JOB NAME 417564	TRUSS NAME C4	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale: 3/4"=1'



TOTAL WEIGHT = 4 X 9 = 36 lb

LUMBER			
N. L. & A. RULES	SIZE	LUMBER	
CHORDS			
F - B	2x4 DRY	No.2	
A - C	2x4 DRY	No.2	
F - D	2x4 DRY	No.2	
ALL WEBS	2x3 DRY	No.2	
DRY: SEASONED LUMBER.			

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX		REQD BRG IN-SX	
	VERT	HORZ	DOWN	HORZ	UPLIFT			
F	274	0	274	0	0	5-8	5-8	
C	34	0	34	0	-39	1-8	1-8	
D	17	0	19	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE				
F	190	143 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	23	19 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MAX. UNBRACED LENGTH (L)		WEBS MAX. FACTORED FORCE (LBS)	
	FR-TO		FROM TO	CSI (L)	FR-TO		FR-TO	
F-B	-258 / 0		0.0	0.0	0.03 (1)		0 / 0	
A-B	0 / 35		-91.8	-91.8	0.12 (1)		0 / 0	
B-C	-27 / 0		-91.8	-91.8	0.12 (1)		0.00 (1)	
F-E	0 / 0		-18.5	-18.5	0.02 (4)			
E-D	0 / 0		-18.5	-18.5	0.02 (4)			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1997 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

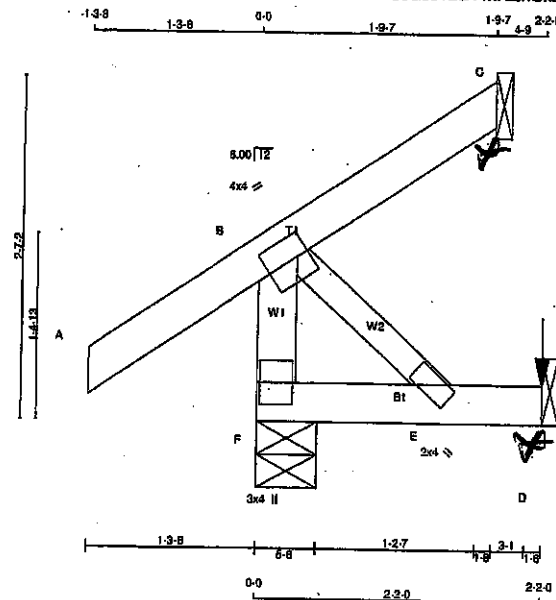
JSI GRIP = 0.16 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108937

JOB NAME 417564	TRUSS NAME C5	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER			
N. L. & A. RULES	SIZE	DRY	LUMBER
CHORDS			
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	4.0	4.0	2.00	1.00
E BMV-w	MT20	2.0	4.0		
F BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	276	0	276	0	5-8	5-8
C	34	0	34	0	1-8	1-8
D	28	0	29	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
F	192	143.0	0.0	0.0	0.0	50.0	0.0
C	23	18.27	0.0	0.0	0.0	4.0	0.0
D	21	0.0	0.0	0.0	0.0	21.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
	MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1 MAX (LBS)	MAX. UNBRACED LENGTH (LBS)	FR-TO	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH (LBS)	FR-TO
F-B		-256.0	0.0	0.0	0.03 (1)	7.81	B-E	0.0	0.00 (1)	
A-B		0.35	-91.8	-91.8	0.13 (5)	10.00				
B-C		-27.0	-91.8	-91.8	0.13 (5)	6.25				
F-E		0.0	-18.5	-18.5	0.03 (4)	10.00				
E-D		0.0	-18.5	-18.5	0.02 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	2-2-0	-5	-5	-	FRONT	VERT	TOTAL	-	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 9 = 18 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.13/1.00 (A-B-5), BC=0.03/1.00 (E-F-4), WB=0.00/1.00 (B-E-1), SSI=0.09/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	850 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

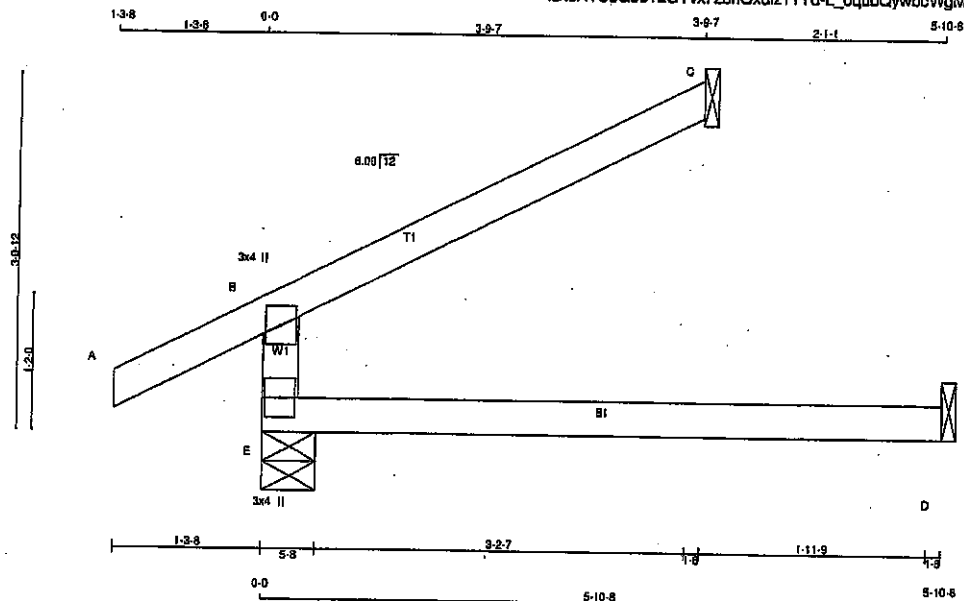
JSI GRIP = 0.16 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108938

JOB NAME 417566	TRUSS NAME C40	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY, SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y
B	TMV+p	MT20	3.0	4.0	X
E	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	405	0	405	0	5-8	5-8
C	130	0	130	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		PERM. LIVE WIND	DEAD	SOIL
	SNOW	LIVE	SNOW	LIVE			
E	286	190.0	0.0	0.0	0.0	96.0	0.0
C	90	73.0	0.0	0.0	0.0	17.0	0.0
D	36	0.0	0.0	0.0	0.0	36.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX. CS1 (LC)	MAX. FORCE (LBS)	MAX. CS1 (LC)
FR-TO								
E-B	-342.0	0.0	0.0	0.13 (4)	7.81			
A-B	0.28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-19.0	-91.8	-91.8	0.22 (1)	6.25			
E-D	0.0	-18.5	-18.5	0.13 (4)	10.00			

TOTAL WEIGHT = 4 X 14 = 57 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.20")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL) = 1/360 (0.20")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.03")

CSI: TC=0.22/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

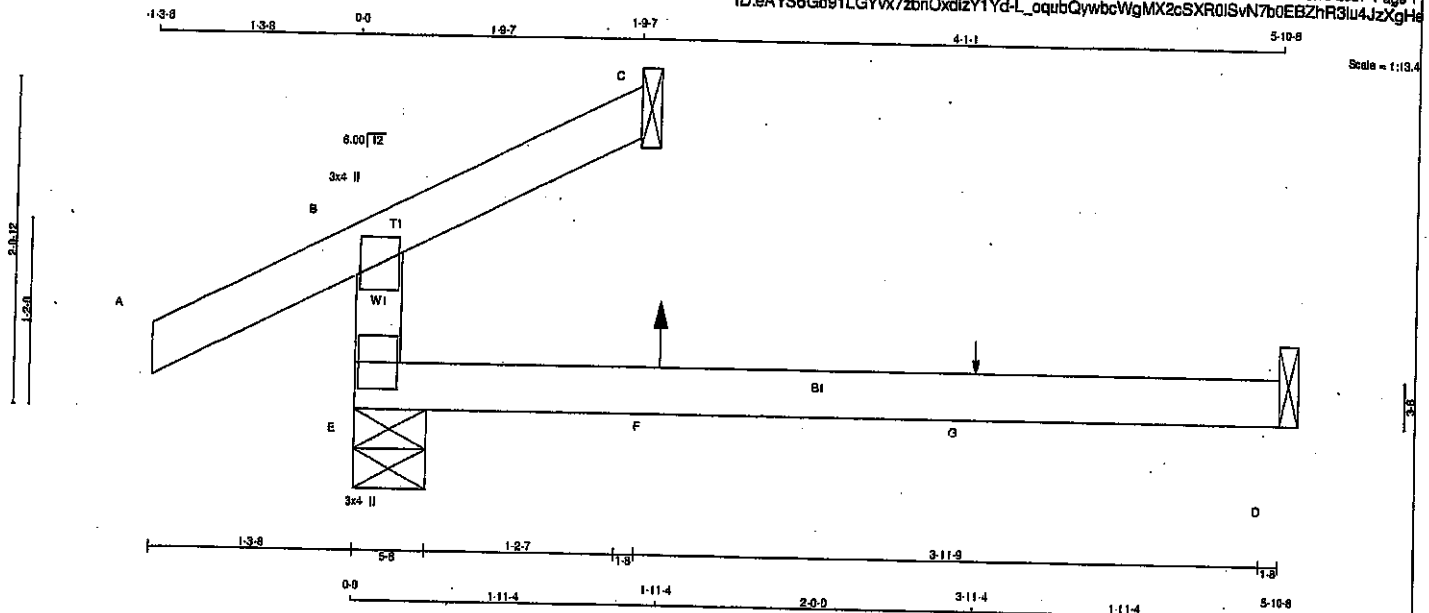
JSI GRIP = 0.14 (E) (INPUT = 0.90)
JSI METAL = 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108992

JOB NAME 417566	TRUSS NAME C41	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:eAYS6G091LGYvx7zbnOxdizY1Yd-L_qubQywbWgMX2cSXR0ISvN7b0EBZhR3lu4JzXgHe



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2	
A - C	2x4	DRY	No.2	
E - D	2x4	DRY	No.2	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	PLATES	W	LEN	Y	X
JT TYPE	MT20	3.0	4.0		
B	MT20	3.0	4.0		
E	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	284	0	284	0	5-8	5-8
C	63	0	63	0	1-8	1-8
D	44	0	52	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	200	137.0	0.0	0.0	0.0	62.0	0.0
C	46	21.0	0.0	0.0	0.0	25.0	0.0
D	35	0.0	0.0	0.0	0.0	37.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

FR-TO	CHORDS				WEBS			
	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. CSF (LC)	UNBRACED LENGTH FR-TO	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. CSF (LC)	UNBRACED LENGTH FR-TO
E-B	-227.0	0.0	0.0	11.4	7.81			
A-B	0.0	-91.8	-91.8	12.1	10.00			
B-C	-9.9	-91.8	-91.8	0.8	10.00			
E-F	0.0	-18.5	-18.5	0.14	10.00			
F-G	0.0	-18.5	-18.5	0.14	10.00			
G-D	0.0	-18.5	-18.5	0.14	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11.4	5	1	8	FRONT	VERT	TOTAL	-	C1
G	3-11.4	1	1	-	FRONT	VERT	TOTAL	-	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 12 = 48 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OSC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.04")

CSF: TC=0.12/1.00 (A-B:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS=0.09/1.00 (A-B:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

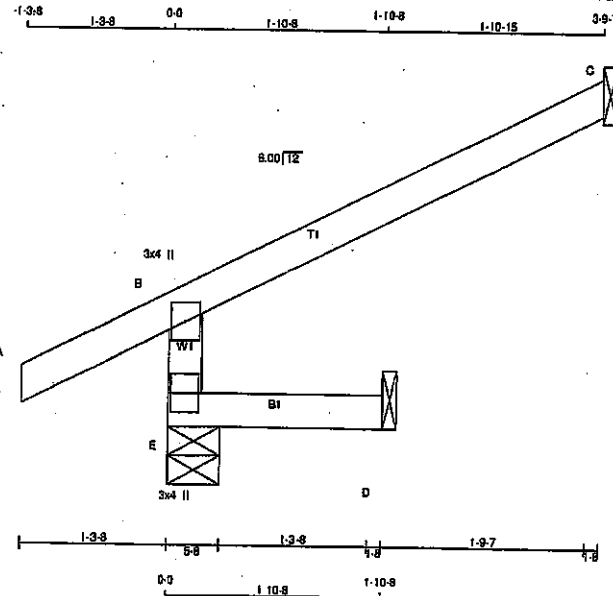


Structural component only
DWG# T-2108993

JOB NAME 417566	TRUSS NAME C42	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/16"

TOTAL WEIGHT = 4 X 10 = 38 lb

LUMBER			
N.L.G.A. RULES	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
E BMV1+p	MT20	3.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	361	0	361	0	5-8	5-8
C	130	0	130	0	1-8	1-8
D	16	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX/MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM.LIVE	WIND	DEAD
E	250	190 / 0	0 / 0	0 / 0	0 / 0	60 / 0
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		W E B S	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO						
E-B	-342 / 0	0.0	0.0	0.01 (4)	7.81	
A-B	0 / 28	-91.8	-91.8	0.13 (5)	10.00	
B-C	-19 / 0	-91.8	-91.8	0.22 (1)	8.25	
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.9 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.22/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

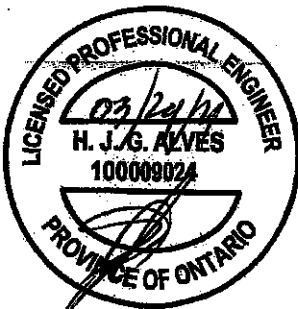
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PLJ) (PLJ)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

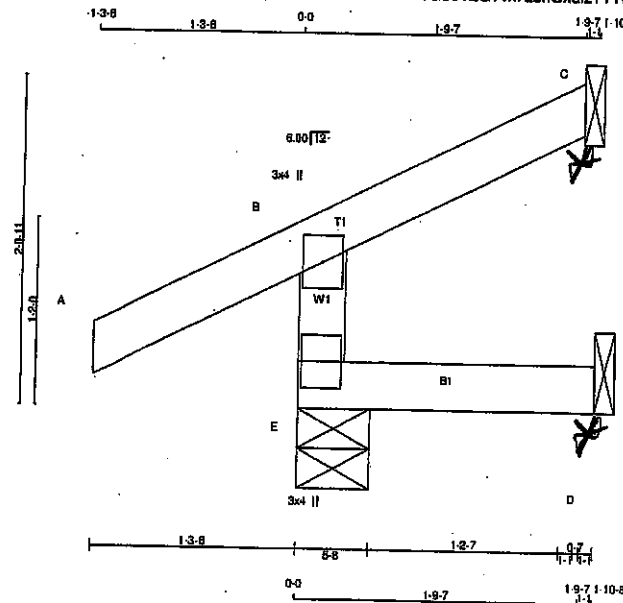
JSI GRIP= 0.14 (E) (INPUT = 0.80)
JSI METAL= 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108994

JOB NAME 417566	TRUSS NAME C43	QUANTITY 8	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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Scale = 1:13.3

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT		VERT	HORZ	DOWN	HORZ
E		271	0	271	0
C		46	0	45	0
D		8	0	17	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	188	141 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	31	24 / -18	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	7	0 / -8	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
MEMB.									
FR-TO									
E-B	-244 / 0	0.0	0.0	0.04 (5)	7.81				
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00				
B-C	-17 / 0	-91.8	-91.8	0.08 (1)	8.25				
E-D	0 / 0	-18.5	-18.5	0.04 (5)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 8 X 7 = 56 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5), WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

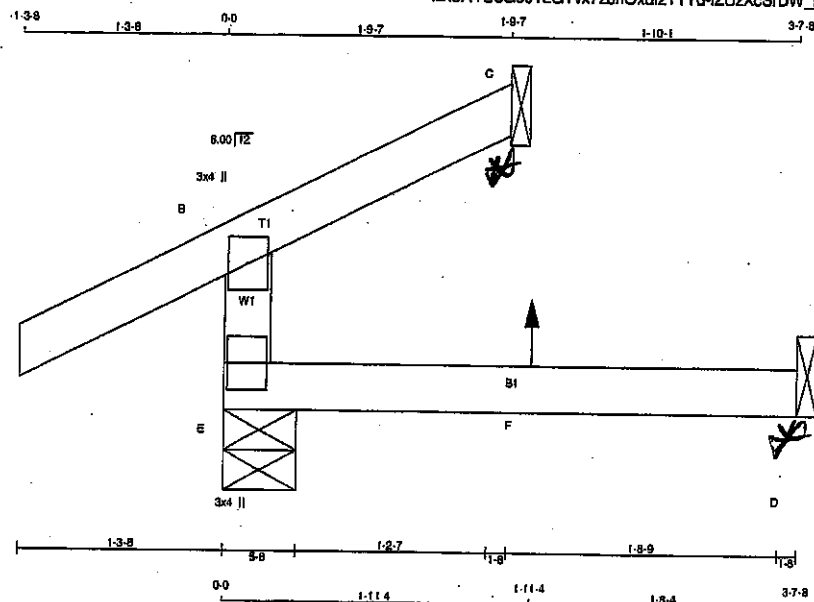
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.10 (E) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108995

JOB NAME 417566	TRUSS NAME C44	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2		
A - C	2x4	DRY	No.2		
E - D	2x4	DRY	No.2		

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	277	0	277	0	5-8	5-8
C	48	0	48	0	1-8	1-8
D	23	0	33	0	1-8	1-8

SEE MTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
E	194	138	0	0	0	57	0
C	34	22	0	0	0	11	0
D	20	0	0	0	0	23	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (9)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO		FR-TO			
E-B	-241 / 0	0.0	0.0	0.05 (5)	7.81		
A-B	0 / 28	-91.8	-91.8	0.13 (5)	10.00		
B-C	-16 / 0	-91.8	-91.8	0.09 (6)	6.25		
E-F	0 / 0	-18.5	-18.5	0.05 (4)	10.00		
F-D	0 / 0	-18.5	-18.5	0.05 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11.4	5	1	8	BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	8.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.13/1.00 (A-B-5), BC=0.05/1.00 (D-E-4),
WB=0.00/1.00 (n/a:0), SSI=0.10/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.10 (E) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108996



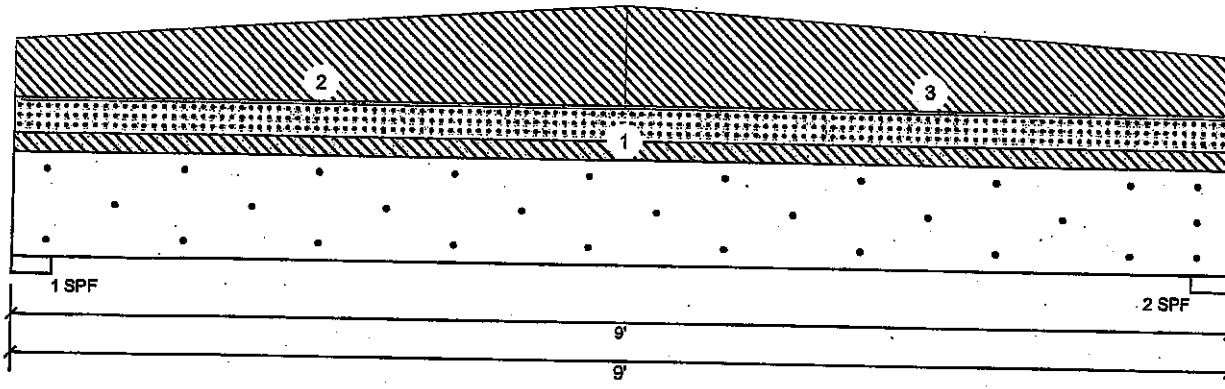
Client:
Project:
Address:

Date: 2021-03-24
Input by:
Job Name: 417565
Project #:

Page 1 of 4

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type: Girder
Plies: 2
Moisture Condition: Dry
Deflection LL: 360
Deflection TL: 360
Importance: Normal - II

Application: Roof (Residential)
Slope: 0/12
Design Method: LSD
Building Code: NBCC 2015
Load Sharing: No
Deck: Not Checked
Vibration: Not Checked

Unfactored Reactions PATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	407	288	0
2	0	407	288	0

Bearings and Factored Reactions

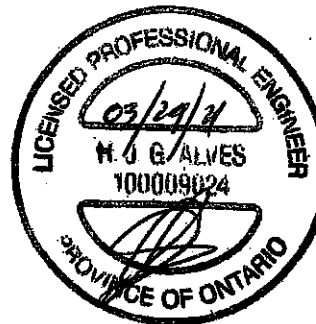
Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	16%	509 / 432	941 L	1.25D+1.5S
2 - SPF	3.500"	16%	509 / 432	941 L	1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1970 ft-lb	4'6"	5556 ft-lb	0.355 (35%)	1.25D+1.5S	L
Unbraced	1970 ft-lb	4'6"	5451 ft-lb	0.361 (36%)	1.25D+1.5S	L
Shear	897 lb	8'	3665 lb	0.245 (24%)	1.25D+1.5S	L
LL Defl inch	0.028 (L/3645)	4'6"	0.285 (L/360)	0.100 (10%)	S	L
TL Defl inch	0.070 (L/1465)	4'6"	0.285 (L/360)	0.250 (25%)	D+S	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.



DWG NO. TAM T-2109033
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		2-6-0	Near Face	13.4 PSF	0 PSF	25.6 PSF	0 PSF	
2	Tapered Start	0-0-0		Far Face	42 PLF	0 PLF	0 PLF	0 PLF	
	End	4-6-0			72 PLF	0 PLF	0 PLF	0 PLF	
3	Tapered Start	4-6-0		Far Face	72 PLF	0 PLF	0 PLF	0 PLF	
	End	9-0-0			42 PLF	0 PLF	0 PLF	0 PLF	

Manufacturer Info

Tamarack roof trusses
3269 North Service rd,
Ontario



This design is valid until 2023-11-27



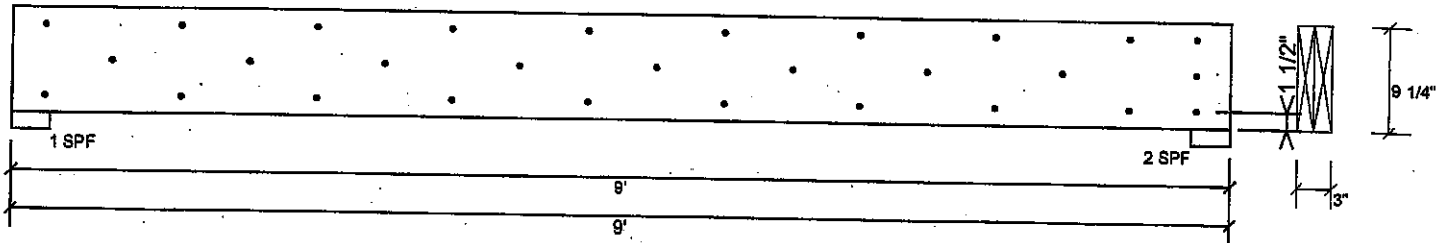
Client:
Project:
Address:

Date: 2021-03-24
Input by:
Job Name: 417565
Project #:

Page 2 of 4

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

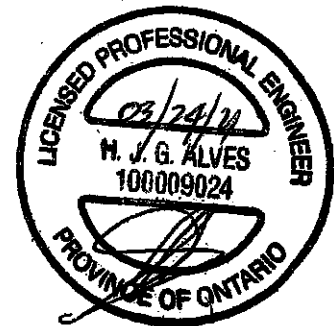
Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	22.8 %
Load	50.4 PLF
Yield Limit per Foot	221.0 PLF
Yield Limit per Fastener	73.7 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.4D
Duration Factor	0.65



DWG NO. TAM T-2109033
STRUCTURAL
COMPONENT ONLY 42

Manufacturer Info

Tamarack roof trusses
3269 North Service rd,
Ontario



This design is valid until 2023-11-27



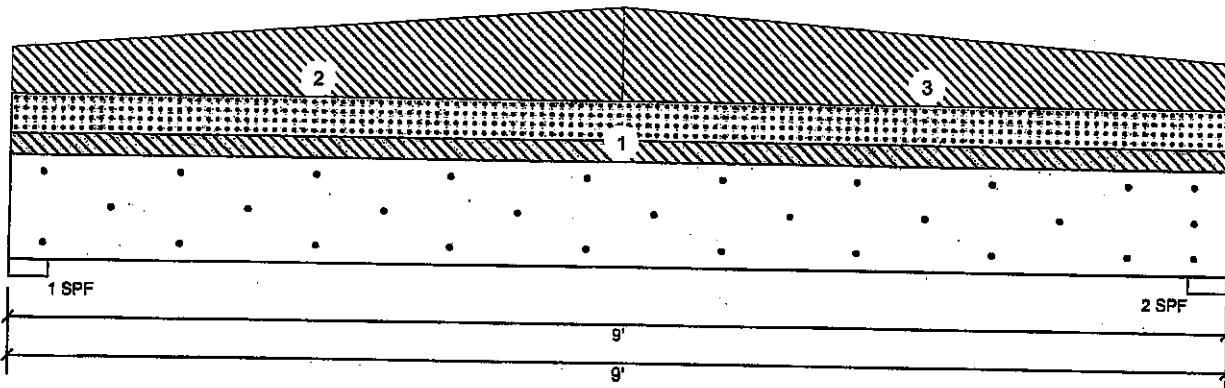
Client:
Project:
Address:

Date: 2021-03-24
Input by:
Job Name: 417565
Project #:

Page 3 of 4

B2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type: Girder
Piles: 2
Moisture Condition: Dry
Deflection LL: 360
Deflection TL: 360
Importance: Normal - II

Application: Roof (Residential)
Slope: 0/12
Design Method: LSD
Building Code: NBCC 2015
Load Sharing: No
Deck: Not Checked
Vibration: Not Checked

Unfactored Reactions PATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	353	288	0
2	0	353	288	0

Bearings and Factored Reactions

Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	14%	442 / 432	874 L		1.25D+1.5S
2 - SPF	3.500"	14%	442 / 432	874 L		1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1833 ft-lb	4'6"	5797 ft-lb	0.316 (32%)	1.25D+1.5S	L
Unbraced	1833 ft-lb	4'6"	5678 ft-lb	0.323 (32%)	1.25D+1.5S	L
Shear	833 lb	1'	3824 lb	0.218 (22%)	1.25D+1.5S	L
LL Defl Inch	0.028 (L/3645)	4'6"	0.285 (L/360)	0.100 (10%)	S	L
TL Defl Inch	0.085 (L/1584)	4'6"	0.285 (L/360)	0.230 (23%)	D+S	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.



DWG NO. TAM T-210903A
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		2-6-0	Near Face	13.4 PSF	0 PSF	25.6 PSF	0 PSF	
2	Tapered Start	0-0-0		Far Face	30 PLF	0 PLF	0 PLF	0 PLF	
	End	4-6-0			60 PLF	0 PLF	0 PLF	0 PLF	
3	Tapered Start	4-6-0		Far Face	60 PLF	0 PLF	0 PLF	0 PLF	
	End	9-0-0			30 PLF	0 PLF	0 PLF	0 PLF	

Manufacturer Info

Tamarack roof trusses
3269 North Service rd,
Ontario



This design is valid until 2023-11-27



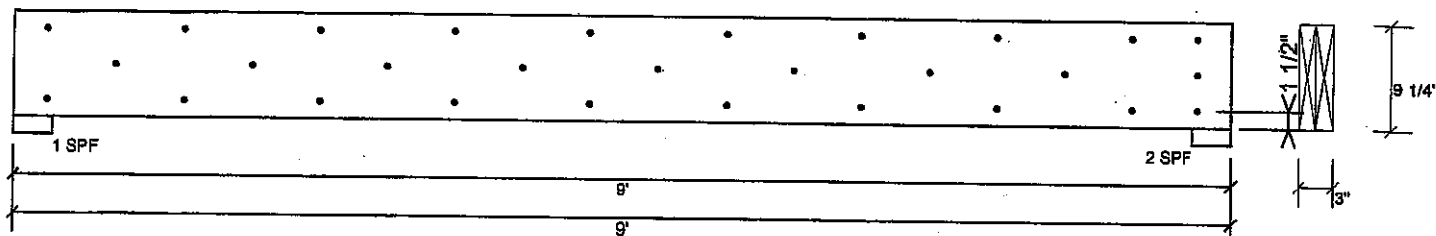
Client:
Project:
Address:

Date: 2021-03-24
Input by:
Job Name: 417565
Project #:

Page 4 of 4

B2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	21.1 %
Load	68.9 PLF
Yield Limit per Foot	326.4 PLF
Yield Limit per Fastener	108.8 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S
Duration Factor	0.96



DWG NO. TAM F-210902A
STRUCTURAL
COMPONENT ONLY

Manufacturer Info

Tamarack roof trusses
3269 North Service rd,
Ontario



This design is valid until 2023-11-27



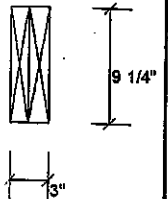
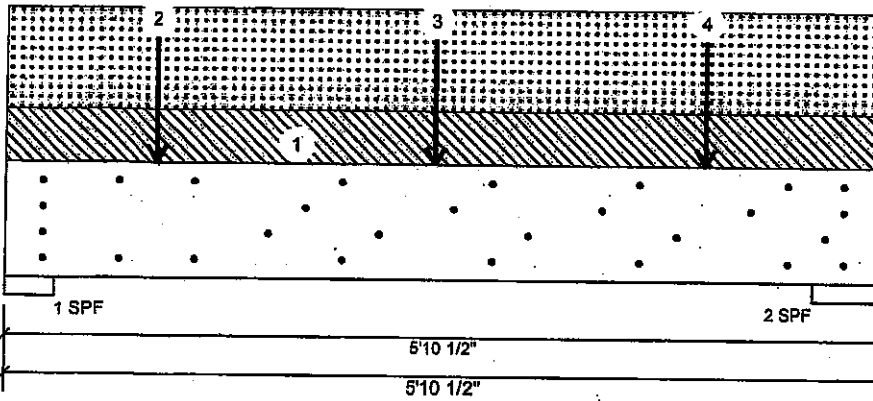
Client:
Project:
Address:

Date: 2021-03-24
Input by:
Job Name: 417566
Project #:

Page 1 of 2

B3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type: Girder
Piles: 2
Moisture Condition: Dry
Deflection LL: 360
Deflection TL: 360
Importance: Normal - II

Application: Roof (Residential)
Slope: 0/12
Design Method: LSD
Building Code: NBCC 2015
Load Sharing: No
Deck: Not Checked
Vibration: Not Checked

Unfactored Reactions PATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	319	665	0
2	0	224	491	0

Bearings and Factored Reactions

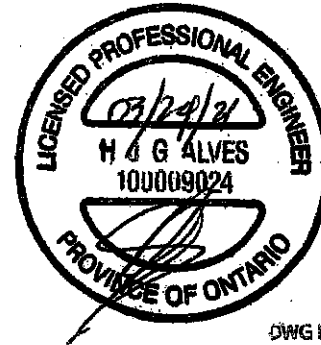
Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	4.000"	19%	399 / 998	1397 L	1.25D+1.5S
2 - SPF	5.500"	10%	280 / 736	1016 L	1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1913 ft-lb	2'10 1/2"	6039 ft-lb	0.317 (32%)	1.25D+1.5S	L
Unbraced	1913 ft-lb	2'10 1/2"	5989 ft-lb	0.319 (32%)	1.25D+1.5S	L
Shear	1382 lb	1' 1/2"	3984 lb	0.347 (35%)	1.25D+1.5S	L
LL Defl inch	0.015 (L/4289)	2'10 1/16"	0.174 (L/360)	0.080 (8%)	S	L
TL Defl inch	0.022 (L/2888)	2'10 1/16"	0.174 (L/360)	0.120 (12%)	D+S	L

Design Notes

- 1 Fasten all plies using 4 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top braced at bearings.
- 7 Bottom braced at bearings.
- 8 Lateral slenderness ratio based on full section width.



DWG NO. TAM 7-2109035
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		1-0-0	Near Face	13.4 PSF	0 PSF	25.6 PSF	0 PSF	
2	Point		1-0-0	Far Face	180 lb	0 lb	383 lb	0 lb	162
3	Point		2-10-8	Top	237 lb	0 lb	482 lb	0 lb	161
4	Point		4-8-4	Top	47 lb	0 lb	141 lb	0 lb	c43

Manufacturer Info

Tamarack roof trusses
3269 North Service rd,
Ontario



This design is valid until 2023-11-27



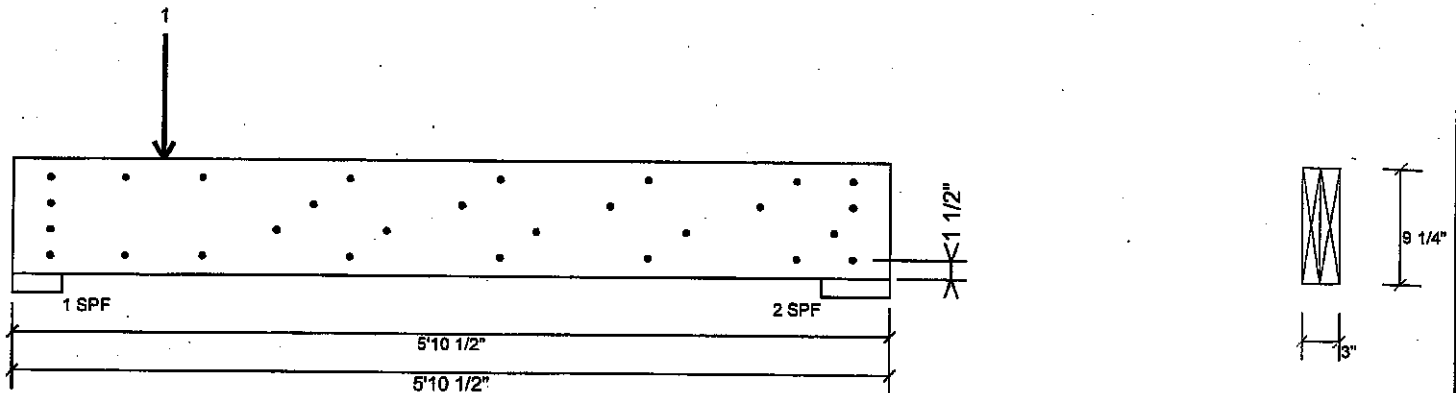
Client:
Project:
Address:

Date: 2021-03-24
Input by:
Job Name: 417566
Project #:

Page 2 of 2

B3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 4 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6"

Capacity	6.1 %
Load	27.6 PLF
Yield Limit per Foot	453.4 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S
Duration Factor	1.00



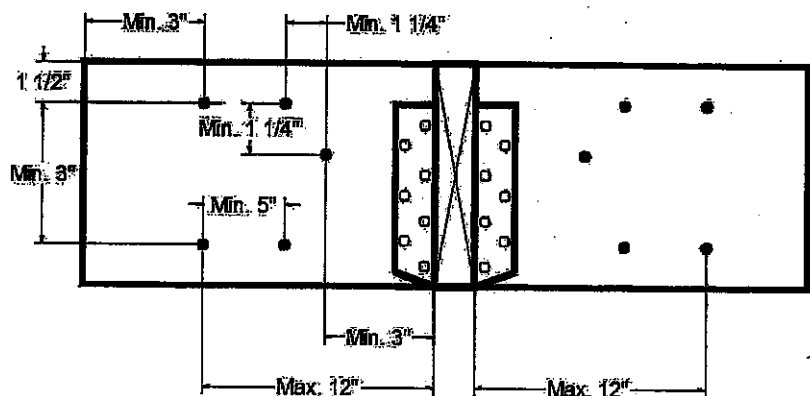
DWG NO. TAM F-2109035
STRUCTURAL
COMPONENT ONLY 7/2

Concentrated Load

Fasten at concentrated side load at 1-0-0 with a minimum of (4) - Pneumatic Gun Nail (.120x3.25") in the pattern shown.

Capacity	88.2 %
Load	399.8 lb.
Total Yield Limit	453.4 lb.
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Load Combination	1.25D+1.5S
Duration Factor	1.00

Min/Max fastener distances for Concentrated Side Loads



Manufacturer Info

Tamarack roof trusses
3269 North Service rd,
Ontario



This design is valid until 2023-11-27



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800213

Feb 09, 2018

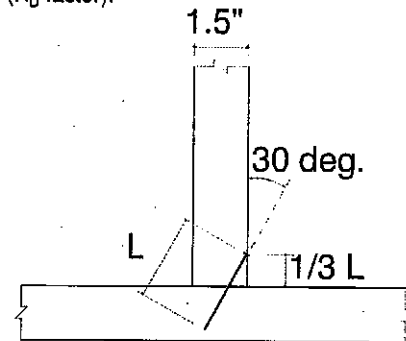
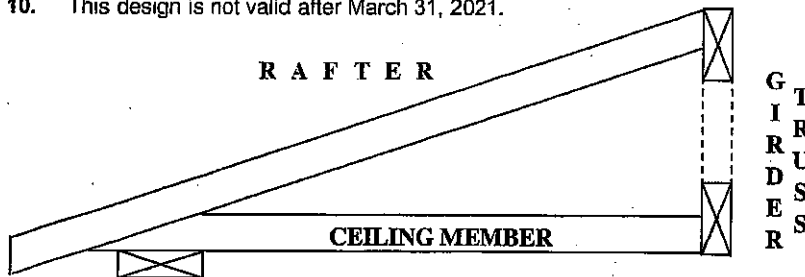
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.



TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

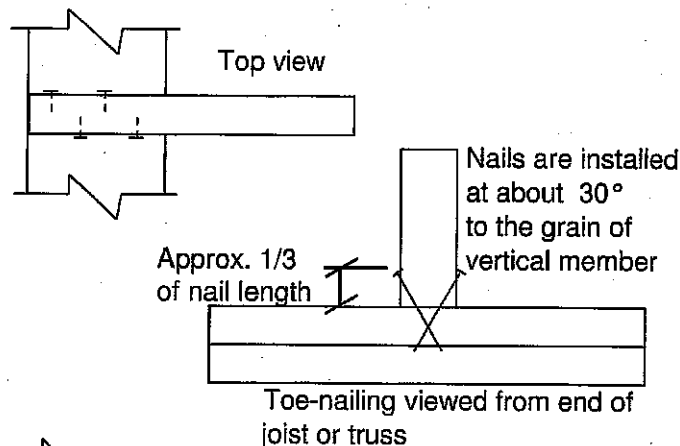
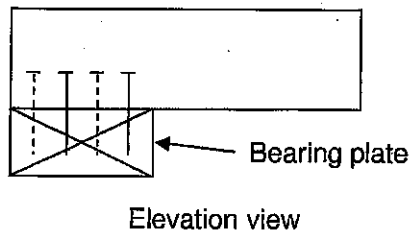
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

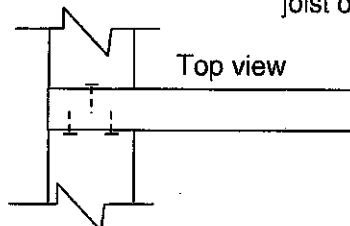
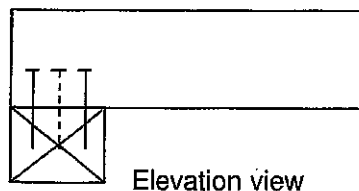
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



December 2, 2019

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

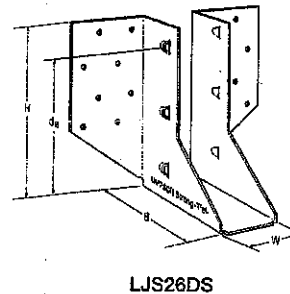
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

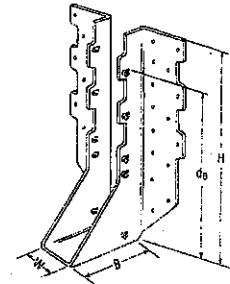
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

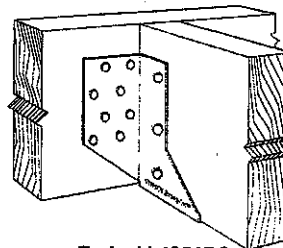
- See current catalogue for options



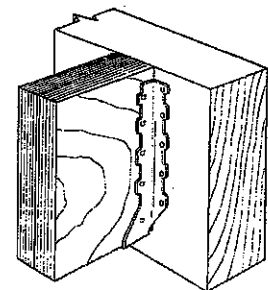
LJS26DS



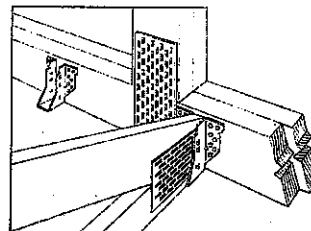
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



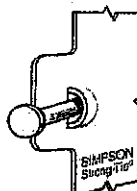
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

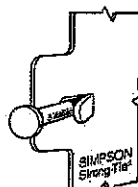
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _s ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LJS26DS	18	1½	5	3½	¾	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1½	5½	3	3½/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1½	7¾	3	6¾/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1½	9¾	3	7¾/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1½	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_s is the distance from the seat of the hanger to the highest joist nail.

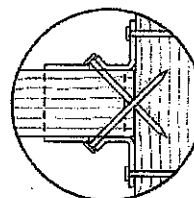


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

LUS – Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

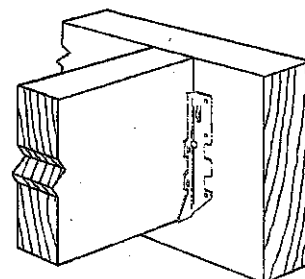
Finish: G90 galvanized

Design:

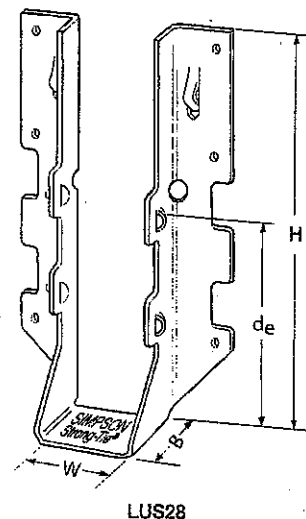
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

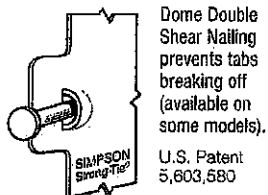


Typical LUS Installation

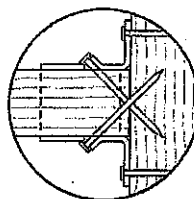


Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _a ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LUS24	18	1 1/8	3 1/8	1 3/4	1 1/8	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 1/8	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/8	4 3/8	1 3/4	3 3/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 3/8	4 3/8	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 3/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 3/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 3/8	8 3/8	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_a is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC LUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HGUS – Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

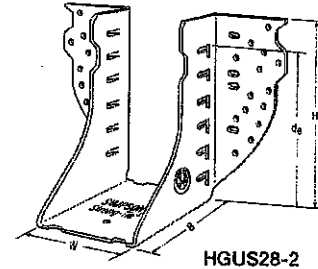
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

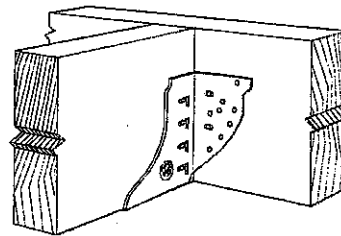
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

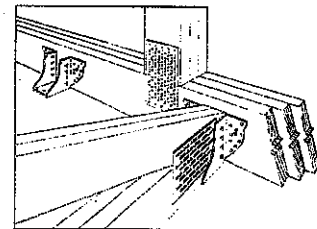
- See current catalogue for options



HGUS28-2



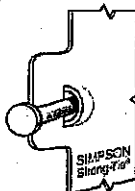
Typical HGUS Installation



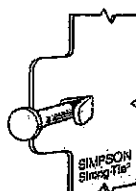
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HGUS26	12	1 1/8	5 3/8	5	4 3/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 1/16	5 7/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 1/16	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 1/16	5 7/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 1/16	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 1/16	7 1/4	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 1/16	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1 3/8	9 1/8	5	7 7/8	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3 3/16	9 3/8	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 1/16	9 1/4	4	8 3/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 1/16	9 3/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/16	10 3/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/16	12 3/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

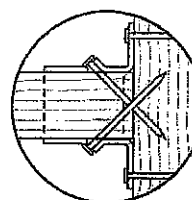
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

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T-SPECHGUS20 3/20 exp. 6/22

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H – Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

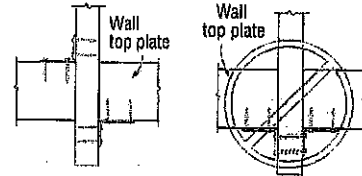
Design: • Factored resistances are in accordance with CSA O86-14
• Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

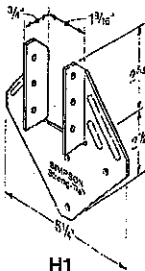
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

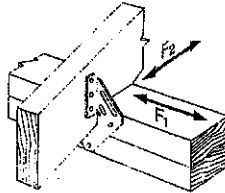


Install diagonally across from each other for minimum 2x truss.

Nailing into both sides of a single ply 2x truss may cause the wood to split.



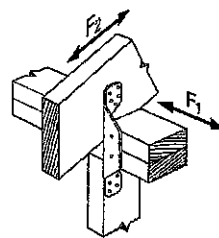
H1



H1 Installation



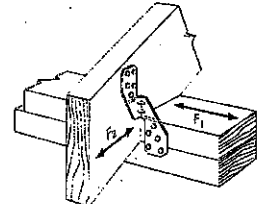
H2A



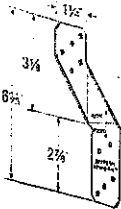
H2A Installation



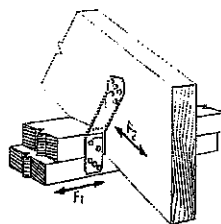
H2.5A



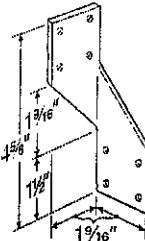
H2.5A Installation



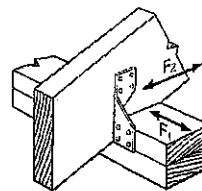
H2.5T



H2.5T Installation
(Nails into both top plates)



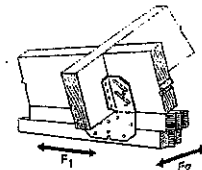
H3



H3 Installation



H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
						(K ₀ =1.15)			(K ₀ =1.15)	
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquakes or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



LIMIT
STATES
DESIGN

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T-SPECH20 3/20 exp. 6/22



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strongtie.com

TC – Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

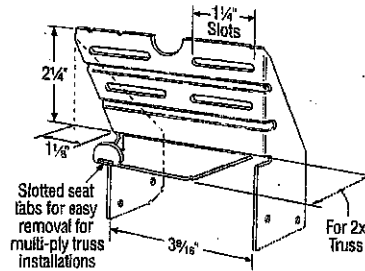
Design: Factored resistances are in accordance with CSA 086-14

Installation:

- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

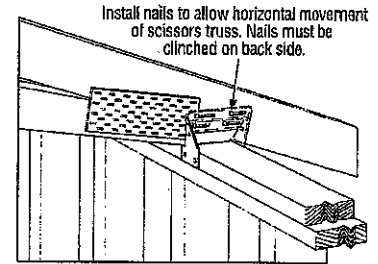
Optional TC Installation:

- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.

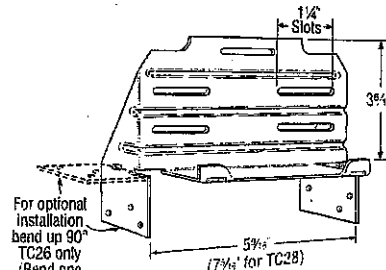


TC24

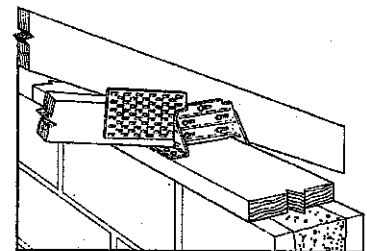
U.S. Patent 4,932,173



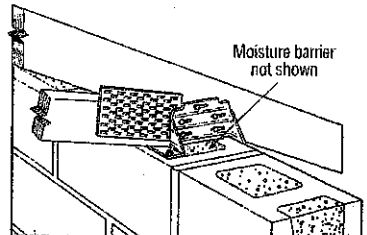
Typical TC24 Installation



TC26
(TC28 Similar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K ₀ =1.15)	Uplift (K ₀ =1.15)
			lb.	lb.
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K ₀ =1.15)	Uplift (K ₀ =1.15)
			lb.	lb.
TC26	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) – ¾" x 2¼" Titen screws has a factored uplift resistance of 275 lb.

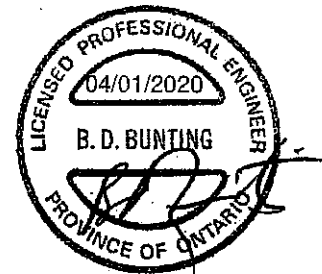


**LIMIT
STATES
DESIGN**

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HHUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

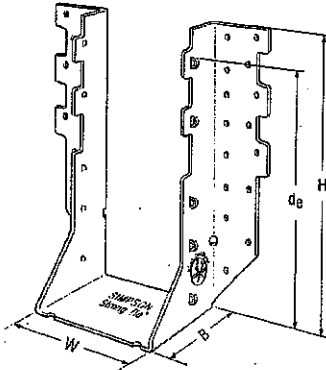
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

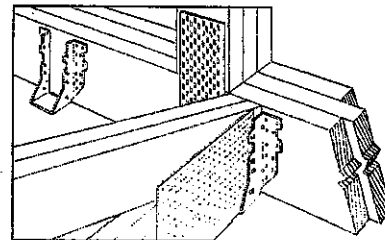
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

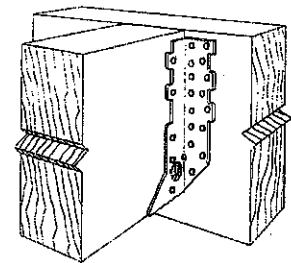
- See current catalogue for options



HHUS410



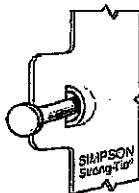
Typical HHUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)



Typical HHUS Installation

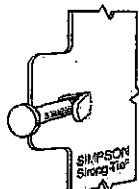
Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist		D.Fir-L		S-P-F	
									Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HHUS26-2	14	3 9/16	5 19/16	3	3 15/16	(14) 16d	(6) 16d		2850	7335	2065	5205
HHUS28-2	14	3 9/16	7 1/2	3	6 5/32	(22) 16d	(8) 16d		3765	8940	2675	6345
HHUS210-2	14	3 9/16	9 3/32	3	8	(30) 16d	(10) 16d		4670	9660	4235	7000
HHUS210-3	14	4 1/16	9	3	7 15/16	(30) 16d	(10) 16d		4670	9670	4235	6865
HHUS210-4	14	6 1/8	8 29/32	3	7 27/32	(30) 16d	(10) 16d		4670	10155	4235	7210
HHUS46	14	3 9/16	5 19/32	3	3 15/16	(14) 16d	(6) 16d		2540	7335	2065	5205
HHUS48	14	3 9/16	7 1/8	3	6 1/4	(22) 16d	(8) 16d		3765	8940	2675	6345
HHUS410	14	3 9/16	9	3	8	(30) 16d	(10) 16d		4670	9855	4235	7000
HHUS5.50/10	14	5 1/2	9	3	8	(30) 16d	(10) 16d		4670	10155	4235	7210
HHUS7.25/10	14	7 1/4	9	3 3/16	7 29/32	(30) 16d	(10) 16d		4670	10155	3370	7210

1. d_e is the distance from the seat of the hanger to the highest joist nail.

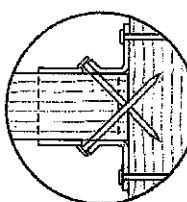


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

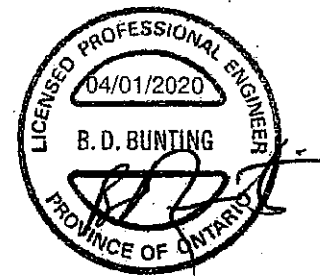
U.S. Patent 5,803,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

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T-SPECHHUS20 3/20 exp. 6/22

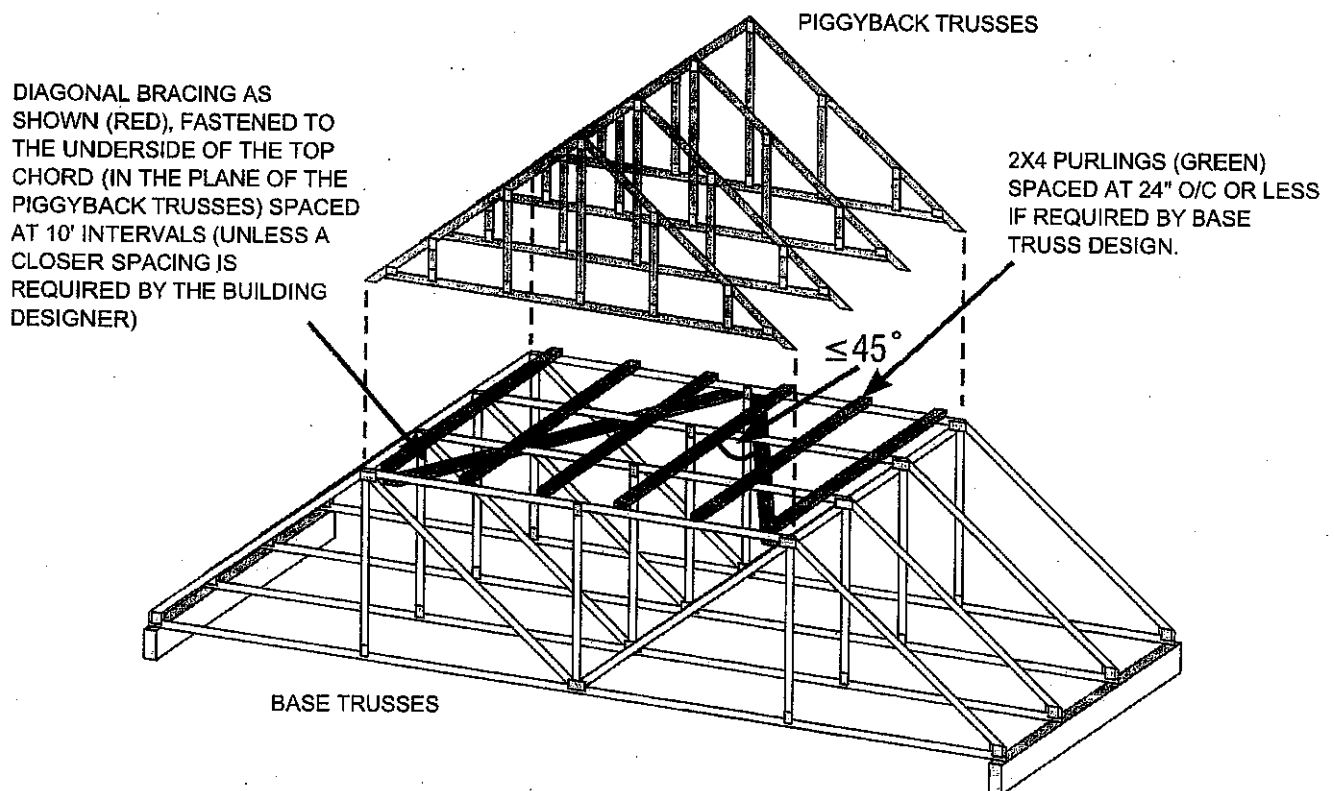
(800) 999-5099
strongtie.com

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

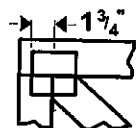
SKETCH FROM BCSI-CANADA 2013

Disclaimer:

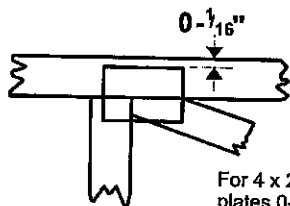
OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4

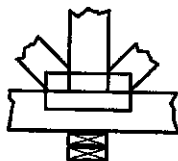
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

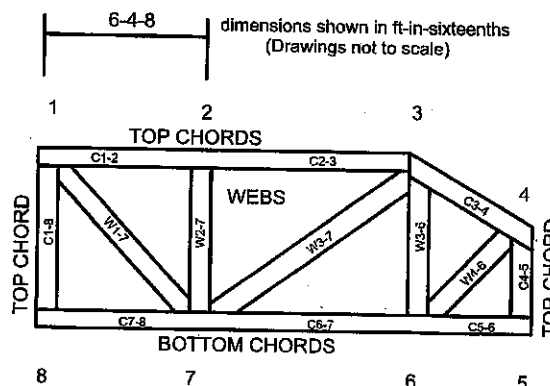


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal-Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.