

FIRST FLOOR 9-01-00
FLOOR JOINTS 14"
SUB FLOOR 3/4"
LOFT FLOOR 9-01-00

61-06-00

55-06-00

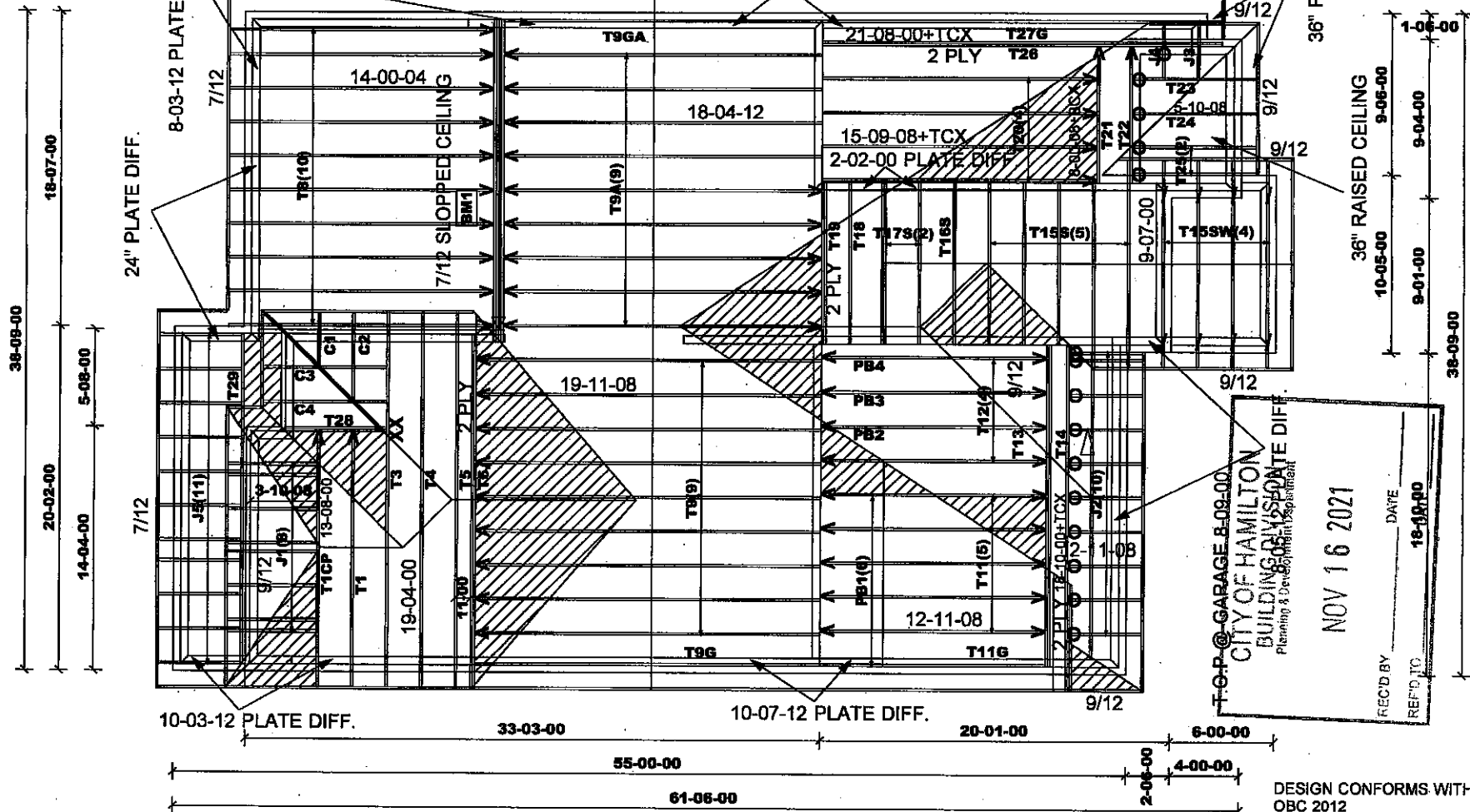
31-03-00

BM1 = 4 PLY 1 3/4" X 14" LVL 2.0E

10-03-12 PLATE DIFF.

ALL ROOF PITCHES ARE 6/12
UNLESS OTHERWISE NOTED

MIN R20 MUST BE
PROVIDED



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
H2.5T - (I)

DENOTES
CONV.
FRAMING



All conventional framing to conform with
Part 9 of O.B.C. 2012. Roof rafters that
cross over or meet trusses to be min. 2x4
SPF #2 @ 24" o/c with a vertical post to the
truss at each cross point. Vertical posts
longer than 6' to have lateral bracing so that
the distance between the post end points
and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH
OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL
PART: 9
Ss = 31.3 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf



Job Track: **52166**

Plan Log: **205172**

Layout ID: **421618**

Builder / Location:

GREENPARK HOMES / HAMILTON

Project: **RUSSELL GARDENS PH.4**

Date: 2021-09-21

Sales:

Rick DiCiano

Designer: LC

Model / Elevation:

RH-574 / 1

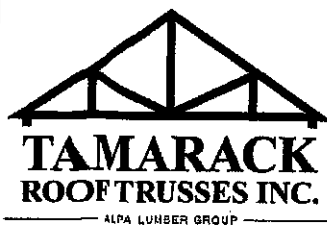
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Mitek ver 8.4.2.286

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST							
	Lumber Yard: TAMARACK LUMBER						Job Track: 52166	
	Builder: GREENPARK HOMES						PlanLog: 205172	
	Project: RUSSELL GARDENS PH.4						Layout ID: 421618	
	Location: HAMILTON						Ref #	
	Model: RH-574						Page: 1 of 4	
Lot #:						Date: 09-21-2021		
Elevation: 1						Designer:		
						Sales Rep: Rick DiCiano		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T1 Common	6 /12	13-08-00	4-07-00	2 x 4	1-03-08	1-02-00 1-02-00	55.36 35.00		
	1	T1CP Hip Girder	6 /12	13-08-00	4-05-02	2 x 4	1-03-08	1-02-00 1-02-00	55.2 35.00		
	1	T3 Hip Girder	6 /12	19-04-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	99.87 83.87		
	1	T4 Hip	6 /12	19-04-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	81.28 51.67		
	1	T5 Common	6 /12	19-04-00	6-00-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	80.01 51.00		
	1 2-ply	T6 Common Girder	6 /12	19-04-00	6-00-00	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	199.64 124.00		
	10	T8 Monopitch	7 /12 7 /12	14-00-04	9-05-09	2 x 4	1-03-08	1-03-07 1-06-10	564.51 355.00		
	9	T9 Roof Special	6 /12	19-11-08	6-06-10	2 x 4		1-08-04 7-00	670.18 427.50		
	9	T9A Roof Special	6 /12	18-04-12	6-06-10	2 x 4		1-08-04 1-05-15	665.7 433.50		
	1	T9G GABLE	6 /12	19-11-08	6-06-10	2 x 4		1-08-04 7-00	78.02 49.33		
	1	T9GA GABLE	6 /12	18-04-12	6-06-10	2 x 4		1-08-04 1-05-15	75.35 48.67		
	5	T11 Piggyback Base	6 /12	12-11-08	10-06-04	2 x 4	1-04-08	5-10-04 10-06-04	446.47 275.00		
	1	T11G GABLE	6 /12	12-11-08	10-06-04	2 x 4	1-04-08	5-10-04 10-06-04	96.37 80.67		
	4	T12 Flat	0 /12	12-11-08	10-06-04	2 x 4		10-06-04 10-06-04	360.28 213.33		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: RH-574
 Lot #:
 Elevation: 1

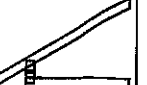
Job Track: 52166
 PlanLog: 205172
 Layout ID: 421618
 Ref #
 Page: 2 of 4
 Date: 09-21-2021
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T13 Half Hip Girder	9 / 12	18-10-00	10-09-00	2 x 6		1-06-04 10-09-00	292.11 173.33		
	1	T14 Half Hip Girder	9 / 12	18-10-00	10-00-00	2 x 4 2 x 6	1-03-08	1-06-04 10-00-00	145.69 89.67		
	5	T15S Scissor	9 / 12 6 / 12	9-07-00	5-01-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	224.84 148.33		
	4	T15SW Scissor	9 / 12 6 / 12	9-07-00	5-01-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	179.71 118.87		
	1	T16S Half Hip	9 / 12 6 / 12	9-07-00	6-05-02	2 x 4		1-06-04 6-05-02	56.12 37.87		
	2	T17S Scissor	9 / 12 6 / 12	9-07-00	7-11-04	2 x 4		1-06-04 7-02-00	117.7 77.33		
	1	T18 Roof Special	9 / 12	9-07-00	5-09-04	2 x 4 2 x 6		10-04 5-00-00	45.02 28.00		
	1 2-ply	T19 Roof Special Girder	9 / 12	9-07-00	5-09-04	2 x 4 2 x 6		1-06-12 5-00-00	109.68 69.33		
	4	T20 Monopitch	6 / 12	15-09-08	12-02-12	2 x 4		4-01-04 12-00-00	348.61 215.33		
	1	T21 Monopitch Girder	9 / 12	8-00-08	9-08-06	2 x 4 2 x 6		3-08-00 9-08-06	63.89 40.33		
	1	T22 Half Hip Girder	9 / 12	8-00-08	8-00-14	2 x 4 2 x 6		3-08-00 8-00-14	62.67 41.67		
	1	T23 Half Hip Girder	9 / 12	5-10-08	5-00-14	2 x 4 2 x 6	1-03-08	8-00 2-00-14	29.35 19.00		
	1	T24 Half Hip	9 / 12	5-10-08	6-06-14	2 x 4 2 x 6	1-03-08	8-00 3-06-14	32.41 20.83		
	2	T25 Monopitch	9 / 12	5-10-08	8-00-14	2 x 4 2 x 6	1-03-08	8-00 5-00-14	59.41 37.33		

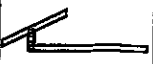
 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	DELIVERY SHIPLIST							
	Lumber Yard: TAMARACK LUMBER						Job Track: 52166	
	Builder: GREENPARK HOMES						PlanLog: 205172	
	Project: RUSSELL GARDENS PH.4						Layout ID: 421618	
	Location: HAMILTON						Ref #	
	Model: RH-574						Page: 3 of 4	
Lot #:						Date: 09-21-2021		
Elevation: 1						Designer:		
						Sales Rep: Rick DiCiano		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T26 Monopitch Girder	6 /12	21-08-00	12-02-12	2 x 4 2 x 6	1-03-08	1-02-00 12-00-00	284.52 173.67		
	1	T27G GABLE	6 /12	21-08-00	12-00-00	2 x 4	1-03-08	1-02-00 12-00-00	132.6 83.00		
	1 2-ply	T28 Half Hip Girder	6 /12	5-10-08	3-10-08	2 x 4 2 x 6		1-02-00 3-10-08	58.16 37.67		
	1	T29 Flat Girder	0 /12	19-04-00	2-11-10	2 x 4		2-11-10 2-11-10	74.93 49.17		
	6	PB1 Piggyback	6 /12	3-07-08	1-09-12	2 x 4		1-09-12	53.14 40.00		
	1	PB2 Piggyback	9 /12	12-11-08	1-05-06	2 x 4		5-00 1-06-00	41.15 26.33		
	1	PB3 Piggyback	9 /12	12-11-08	2-11-06	2 x 4		5-00 3-00-00	45.07 28.67		
	1	PB4 Piggyback	9 /12	12-11-08	4-05-06	2 x 4		5-00 4-06-00	47.04 30.00		
	8	J1 Jack-Open	9 /12	3-10-08	4-05-02	2 x 4	1-03-08	1-06-04 4-05-02	117.47 77.33		
	10	J2 Jack-Closed	6 /12	2-11-08	2-07-12	2 x 4	1-03-08	1-02-00 2-07-12	140.78 95.00		
	1	J3 Jack-Open	9 /12	1-10-08	2-08-14	2 x 4 2 x 6	1-03-08	8-00 2-00-14	8.57 6.00		
	1	J4 Jack-Open	9 /12	1-10-08	5-00-14	2 x 4 2 x 6	1-03-08	8-00 2-00-14	11.62 8.00		
	11	J5 Jack-Open	7 /12	3-05-08	3-01-05	2 x 4	1-03-08	11-07 2-11-10	120.69 80.67		
	1	C1 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	7.02 4.67		

 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREENPARK HOMES Project: RUSSELL GARDENS PH.4 Location: HAMILTON Model: RH-574 Lot #: Elevation: 1	Job Track: 52166 PlanLog: 205172 Layout ID: 421618 Ref # Page: 4 of 4 Date: 09-21-2021 Designer: Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	C2 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	9.57 6.00		
	1	C3 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	11.58 7.33		
	1	C4 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	14.13 8.67		

TOTAL # TRUSS= 125

TOTAL BFT OF ALL TRUSSES= 4102.34

BFT.

TOTAL WEIGHT OF ALL TRSSES 6472.91 LBS

HARDWARE

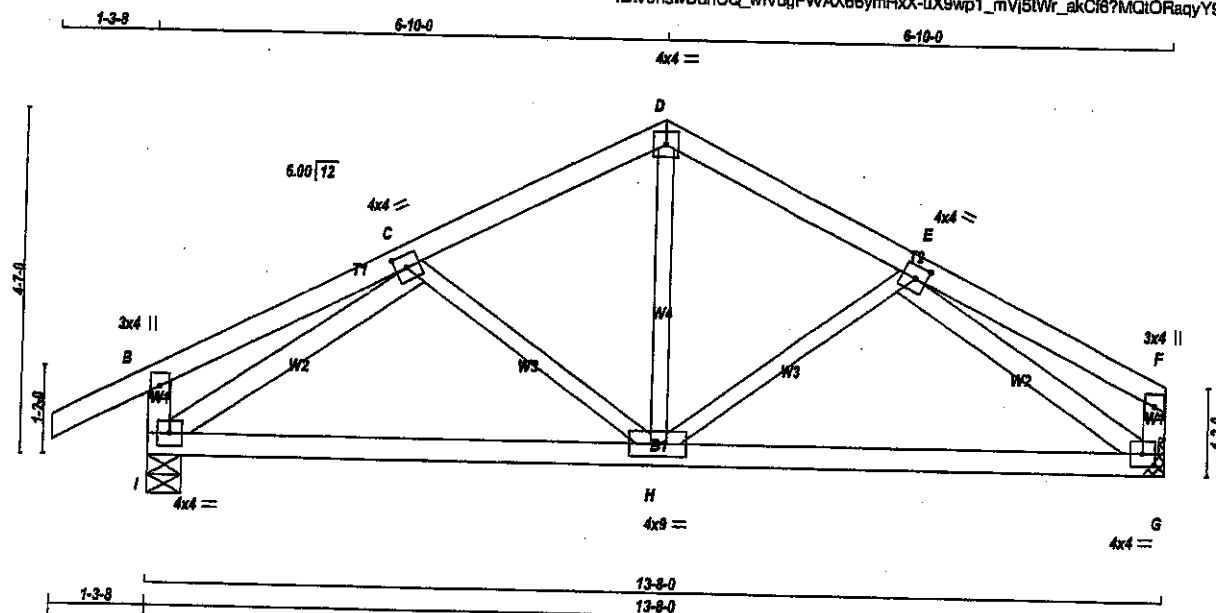
QTY	TYPE	MODEL	LENGTH
8	Hardware	H2.5T	
1	Hardware	HGUS26-2	
59	Hardware	LJS26DS	
15	Hardware	LUS24	
4	Rectangular EWP	1-3/4" x 14" VERSA-LAM® 2.0 3100 SP	20-00-00

TOTAL NUMBER OF ITEMS= 87

JOB NAME 421618	TRUSS NAME T1	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Rail Truss, Burlington

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Scale = 1:29.2

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
I - B	2x4 DRY	No.2	SPF		
G - F	2x4 DRY	No.2	SPF		
I - G	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					
I - C	2x4 DRY	No.2	SPF		
E - G	2x4 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-p	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
G	BMVW-t	MT20	4.0	4.0		
H	BMVW-t	MT20	4.0	9.0		
I	BMVW-t	MT20	4.0	4.0		

NOTES: (1)

- 1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
I	878	0	5-8	5-8
G	753	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	818	418 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0
G	533	350 / 0	0 / 0	0 / 0	0 / 0	183 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 MAX (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	H-D	0 / 383	0.09 (1)
B-C	0 / 15	-91.8	-91.8 0.16 (1)	10.00	H-E	-189 / 0	0.06 (1)
C-D	-715 / 0	-91.8	-91.8 0.13 (1)	6.25	C-H	-189 / 0	0.06 (1)
D-E	-715 / 0	-91.8	-91.8 0.13 (1)	6.25	I-C	-969 / 0	0.21 (1)
E-F	0 / 15	-91.8	-91.8 0.16 (1)	10.00	E-G	-969 / 0	0.21 (1)
I-B	-248 / 0	0.0	0.0 0.03 (1)	7.81			
G-F	-123 / 0	0.0	0.0 0.01 (1)	7.81			
I-H	0 / 776	-18.5	-18.5 0.31 (4)	10.00			
H-G	0 / 776	-18.5	-18.5 0.31 (4)	10.00			

TOTAL WEIGHT = 55 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.18/1.00 (B-C:1), BC=0.31/1.00 (H-I:4),
WB=0.21/1.00 (C-I:1), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1897	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

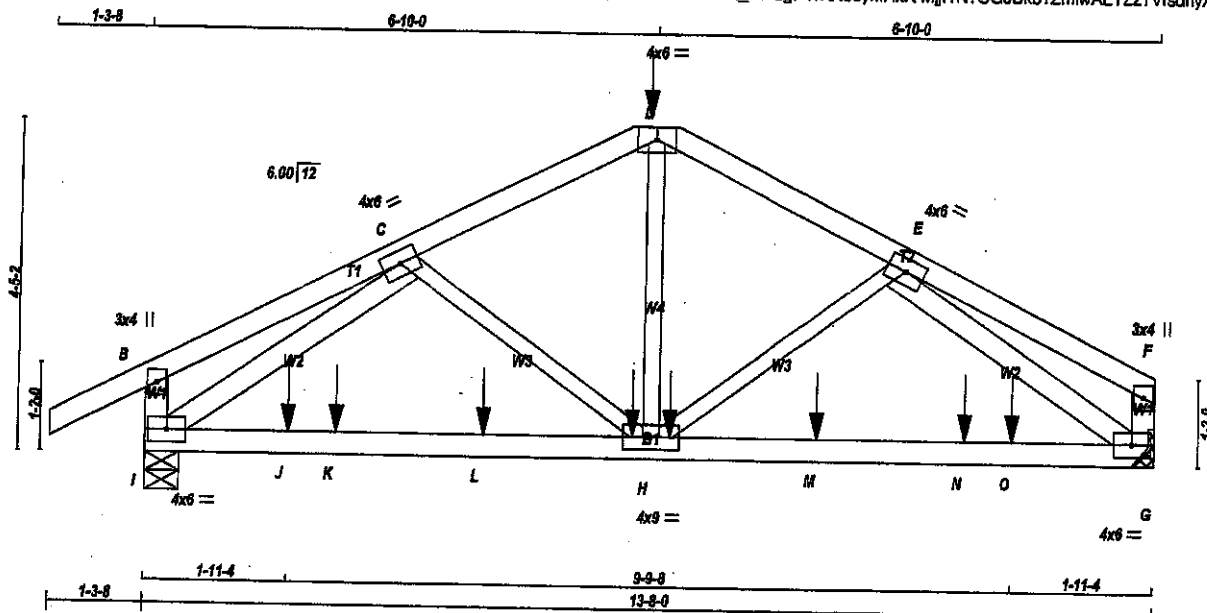
JSI GRIP= 0.81 (I) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 1.00)



Structural component only
DWG# T-2132640

JOB NAME 421618	TRUSS NAME T1CP	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID: v9nswDdnCQ_wlVdgPWAX66ymHxX-Mj11N7OG0Dk87ZmiWALYZz1VrsdhyX7mQFVyychZ6



Scale = 1:25

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	No.2	SPF		
I - B	2x4	DRY	No.2	SPF		
G - F	2x4	DRY	No.2	SPF		
I - G	2x4	DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF		
I - C	2x4	DRY	No.2	SPF		
E - G	2x4	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table in inches)	W	LEN	Y	X
JT TMV-p	MT20	3.0	4.0	
C TMWW-t	MT20	4.0	6.0	
D TTW-p	MT20	4.0	6.0	
E TMWW-t	MT20	4.0	6.0	
F TMV-p	MT20	3.0	4.0	
G BMVW1-t	MT20	4.0	6.0	
H BMVWW-t	MT20	4.0	9.0	
I BMVW1-t	MT20	4.0	6.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	BRG	BRG	BRG
JT	VERT	HORZ	VERT	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX
I	1361	0	1361	0	0	5-8	5-8	
G	1237	0	1237	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

1ST LOASE	MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	580	647 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0
G	874	578 / 0	0 / 0	0 / 0	0 / 0	286 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO		
A-B	0 / 28	-91.8	10.00	H-D	0 / 288	0.09 (4)
B-C	0 / 14	-91.8	10.00	H-E	-31 / 37	0.01 (4)
C-D	-1539 / 0	-91.8	5.06	C-H	-31 / 37	0.01 (4)
D-E	-1539 / 0	-91.8	5.06	I-C	-1735 / 0	0.39 (1)
E-F	0 / 14	-91.8	10.00	E-G	-1735 / 0	0.39 (1)
I-B	-250 / 0	0.0	7.81			
G-F	-126 / 0	0.0	7.81			
I-J	0 / 1388	-18.5	10.00			
J-K	0 / 1388	-18.5	10.00			
K-L	0 / 1388	-18.5	10.00			
L-H	0 / 1388	-18.5	10.00			
H-M	0 / 1388	-18.5	10.00			
M-N	0 / 1388	-18.5	10.00			
N-O	0 / 1388	-18.5	10.00			
O-G	0 / 1388	-18.5	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
D	6-10-0	-84	-84	FRONT	VERT	DEAD		C1
D	6-10-0	-127	-127	BACK	VERT	TOTAL		C1
D	6-10-0	-360	-360	FRONT	VERT	SNOW		C1
H	7-1-0	-14	-14	BACK	VERT	TOTAL		C1
J	1-11-4	-14	-14	BACK	VERT	TOTAL		C1
K	2-7-0	-14	-14	BACK	VERT	TOTAL		C1
L	4-7-0	-14	-14	BACK	VERT	TOTAL		C1
M	9-1-0	-14	-14	BACK	VERT	TOTAL		C1
N	11-1-0	-14	-14	BACK	VERT	TOTAL		C1
O	11-8-12	-14	-14	BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 55 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN/C

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.20/1.00 (C-D-1), BC=0.53/1.00 (H-K-4), WB=0.39/1.00 (C-E-1), SS=0.16/1.00 (G-H-4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

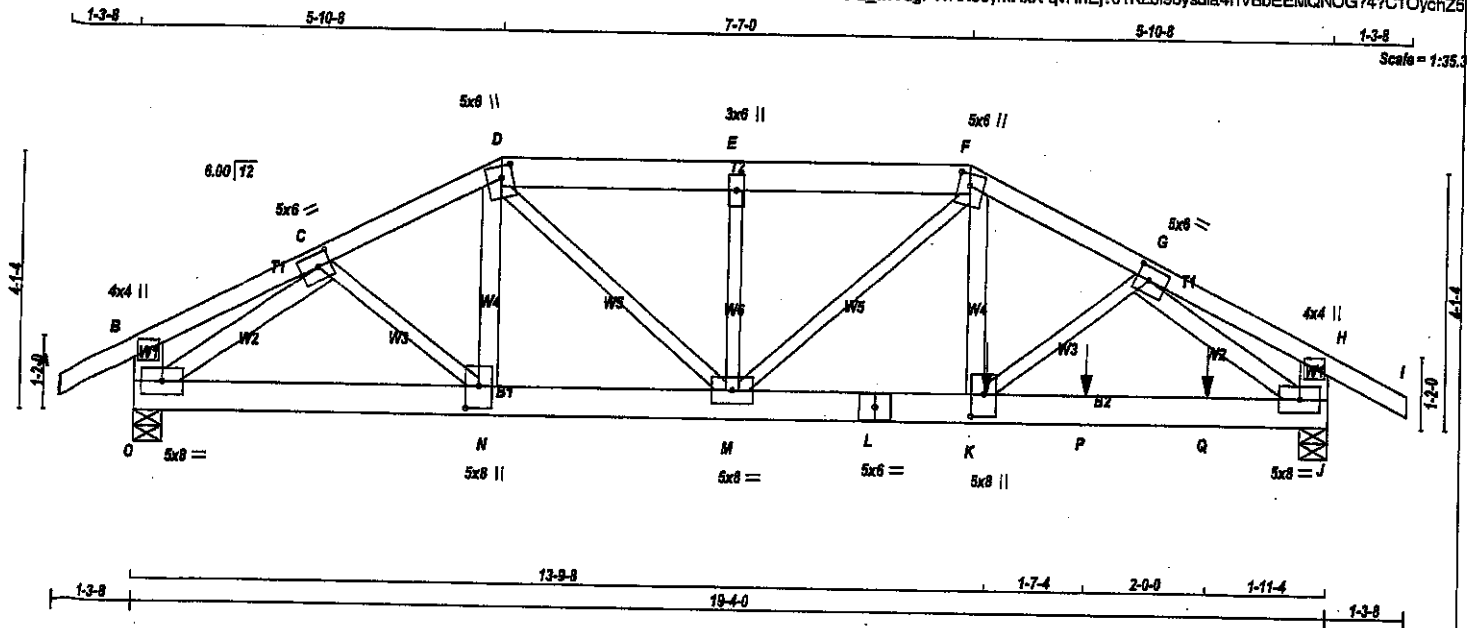
JSI GRIP = 0.83 (I) (INPUT = 0.90)
JSI METAL = 0.38 (E) (INPUT = 1.00)



Structural component only
DWG# T-2132641

JOB NAME 421618	TRUSS NAME T3	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Rd Truss, Burlington					

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ID:v9nswDdnCQ_wiVdgPWAX66ymHxX-qvHhEj?01KLb98ysdia4nVBbEEMQNOG74?C1Oych25



TOTAL WEIGHT = 100 LB

LUMBER	N. L. G. A.	RULES	SIZE	LUMBER	DESCR.
CHORDS					
A - D	2x4	DRY	No.2	SPF	
D - F	2x6	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
O - B	2x6	DRY	No.2	SPF	
J - H	2x6	DRY	No.2	SPF	
O - L	2x6	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
N - D	2x4	DRY	No.2	SPF	
K - F	2x4	DRY	No.2	SPF	
G - J	2x4	DRY	No.2	SPF	
O - C	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMVW-1	MT20	5.0	8.0	2.50	2.50
D	TTWW+m	MT20	5.0	8.0	2.25	2.25
E	TMVW-w	MT20	3.0	8.0		
F	TTWW+m	MT20	5.0	8.0	2.25	2.25
G	TMVW-1	MT20	5.0	8.0	2.50	2.50
H	TMV+p	MT20	4.0	4.0		
J	BMVW1-t	MT20	5.0	8.0		
K	BMVW+1	MT20	5.0	8.0	4.25	2.50
L	BS-t	MT20	5.0	8.0		
M	BMVW-1	MT20	5.0	8.0		
N	BMVW+1	MT20	5.0	8.0	4.25	2.50
O	BMVW1-t	MT20	5.0	8.0		

NOTES: (1)

1) Lateral brades to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HCRZ	DOWN	HORIZ
J	2256	0	2256	0
O	1628	0	1628	0

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1592	1065 / 0	0 / 0	0 / 0	0 / 0	527 / 0	0 / 0
O	1148	775 / 0	0 / 0	0 / 0	0 / 0	372 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.13 (1)	10.00	N-D	0 / 89	0.02 (4)
B-C	0 / 11	-91.8 -91.8	0.11 (1)	10.00	D-M	0 / 987	0.24 (1)
C-D	-2113 / 0	-91.8 -91.8	0.17 (1)	4.49	M-E	-442 / 0	0.11 (1)
D-E	-2598 / 0	-91.8 -91.8	0.13 (1)	5.02	M-F	-255 / 0	0.12 (1)
E-F	-2598 / 0	-91.8 -91.8	0.13 (1)	5.02	K-F	0 / 1321	0.23 (1)
F-G	-3161 / 0	-91.8 -91.8	0.25 (1)	3.70	K-G	0 / 382	0.09 (1)
G-H	0 / 9	-91.8 -91.8	0.11 (1)	10.00	G-J	-3189 / 0	0.55 (1)
H-I	0 / 28	-91.8 -91.8	0.13 (1)	10.00	C-N	0 / 192	0.04 (1)
O-B	-235 / 0	0.0 0.0	0.02 (1)	7.81	O-C	-2217 / 0	0.38 (1)
J-H	-241 / 0	0.0 0.0	0.02 (1)	7.81			
O-N	0 / 1783	-18.5 -18.5	0.28 (1)	10.00			
N-M	0 / 1859	-18.5 -18.5	0.27 (1)	10.00			
M-L	0 / 2780	-18.5 -18.5	0.40 (1)	10.00			
L-K	0 / 2780	-18.5 -18.5	0.40 (1)	10.00			
K-P	0 / 2536	-18.5 -18.5	0.43 (1)	10.00			
P-Q	0 / 2536	-18.5 -18.5	0.43 (1)	10.00			
Q-J	0 / 2536	-18.5 -18.5	0.43 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
K	13-9-8	-1019	-1019	—	BACK	VERT	TOTAL	C1
P	15-4-12	-21	-21	—	BACK	VERT	TOTAL	C1
Q	17-4-12	-20	-20	—	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.8	PSF
DL	= 8.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF CBC 2012 (2015 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.84")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.84")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.25/1.00 (F-G:1), BC=0.43/1.00 (J-K:1), WB=0.55/1.00 (G-J:1), SS=0.13/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	850 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

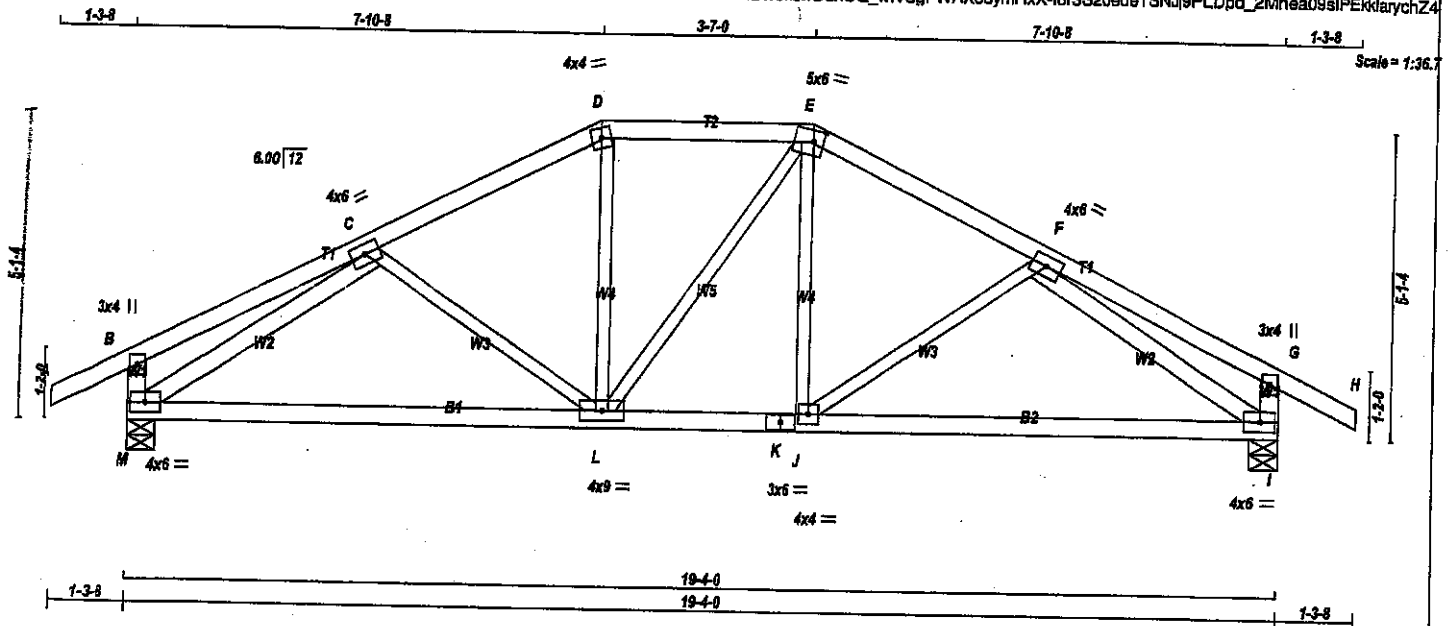
JSI GRIP = 0.89 (C) (INPUT = 0.90)
JSI METAL = 0.75 (G) (INPUT = 1.00)



Structural component only
DWG# T-2132642

JOB NAME 421618	TRUSS NAME T4	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Rd Truss, Burlington					

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TOTAL WEIGHT = 81 lb
(M/F)

LUMBER				
N. L. G. A. RULES	SIZE	DRY	DESCR.	SPF
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
M - C	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMVW-t	MT20	4.0	6.0
D	TTW-m	MT20	4.0	4.0
E	TTWW-m	MT20	5.0	6.0 2.25 2.00
F	TMVW-t	MT20	4.0	6.0
G	TMV+p	MT20	3.0	4.0
I	BMVW-t	MT20	4.0	8.0
J	BMVW-t	MT20	4.0	4.0
K	BS4	MT20	3.0	8.0
L	BMVW-t	MT20	4.0	9.0
M	BMVW-t	MT20	4.0	6.0

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
M	1190 0	1190 0	5-8	5-8	
I	1190 0	1190 0	5-8	5-8	

UNFACTORED REACTIONS					
JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS			
M	839	564 / 0	0 / 0	0 / 0	0 / 0
I	839	564 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING					
TOTAL LOAD CASES: (4)					
CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (LC)
FR-TO			FR-TO		
A-B	0 / 28	-91.8 -91.8 0.12 (1)	C-L	-187 / 0	0.08 (1)
B-C	0 / 18	-91.8 -91.8 0.22 (1)	D-E	0 / 224	0.05 (1)
C-D	-1193 / 0	-91.8 -91.8 0.18 (1)	E-F	0 / 0	0.00 (1)
D-E	-1055 / 0	-91.8 -91.8 0.16 (1)	F-G	0 / 223	0.05 (1)
E-F	-1193 / 0	-91.8 -91.8 0.18 (1)	G-H	-187 / 0	0.08 (1)
F-G	0 / 18	-91.8 -91.8 0.22 (1)	H-I	-1479 / 0	0.41 (1)
G-H	0 / 28	-91.8 -91.8 0.12 (1)	I-J	-1479 / 0	0.41 (1)
H-I	-266 / 0	0.0 0.0 0.03 (1)			
I-J	-266 / 0	0.0 0.0 0.03 (1)			
M-L	0 / 1202	-18.5 -18.5 0.34 (1)			
L-K	0 / 1054	-18.5 -18.5 0.31 (4)			
K-J	0 / 1054	-18.5 -18.5 0.31 (4)			
J-I	0 / 1202	-18.5 -18.5 0.34 (1)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 8.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/960 (0.64")
CALCULATED VERT. DEFL.(LL) = L/998 (0.03")
ALLOWABLE DEFL.(TL) = L/960 (0.64")
CALCULATED VERT. DEFL.(TL) = L/998 (0.15")

CSI: TC=0.22/1.00 (F-G:1), BC=0.34/1.00 (I-J:1), WB=0.41/1.00 (C-M:1), SS=0.15/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

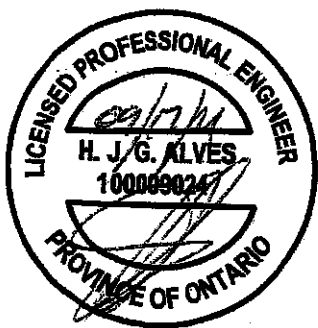
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PLJ)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

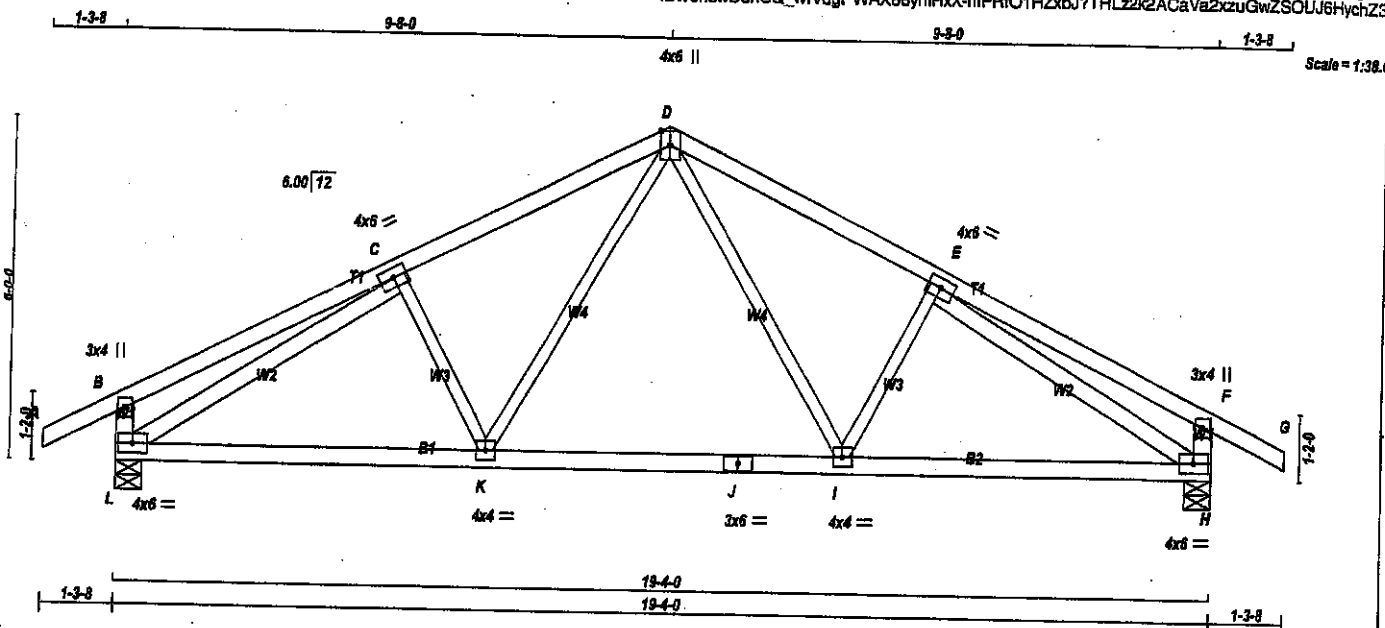
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.80 (K) (INPUT = 0.90)
JSI METAL = 0.54 (K) (INPUT = 1.00)



Structural component only
DWG# T-2132643



LUMBER			
N. L. & A.	RULES	SIZE	LUMBER
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
L - C	2x4	DRY	No.2
E - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMVW-t	MT20	4.0	8.0
D	TTWW+p	MT20	4.0	8.0
E	TMVW-t	MT20	4.0	8.0
F	TMV+p	MT20	3.0	4.0
H	BMVW1-t	MT20	4.0	8.0
I	BMVW-t	MT20	4.0	4.0
J	BS-t	MT20	3.0	6.0
K	BMVW-t	MT20	4.0	4.0
L	BMVW1-t	MT20	4.0	8.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	VERT	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
L	1180	0	1180	0	0	5-8	5-8	5-8	5-8
H	1180	0	1180	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	DEAD	SOIL	DEAD	SOIL
L	839	564 / 0	0 / 0	0 / 0	0 / 0	275 / 0	0 / 0	275 / 0	0 / 0	275 / 0	0 / 0
H	839	564 / 0	0 / 0	0 / 0	0 / 0	275 / 0	0 / 0	275 / 0	0 / 0	275 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0 / 28	-91.8	0.12 (1)	D-I	0 / 442	10.00	0.10 (1)
B-C	0 / 23	-91.8	0.34 (1)	E-H	-290 / 0	10.00	0.07 (1)
C-D	-1270 / 0	-91.8	0.29 (1)	K-D	0 / 442	5.40	0.10 (1)
D-E	-1270 / 0	-91.8	0.29 (1)	C-K	-290 / 0	5.40	0.07 (1)
E-F	0 / 23	-91.8	0.34 (1)	L-C	-1508 / 0	10.00	0.81 (1)
F-G	0 / 28	-91.8	0.12 (1)	E-H	-1508 / 0	10.00	0.81 (1)
L-B	-294 / 0	0.0	0.03 (1)				
H-F	-294 / 0	0.0	0.03 (1)				
L-K	0 / 1248	-18.5	0.29 (1)				
K-J	0 / 901	-18.5	0.25 (4)				
J-I	0 / 901	-18.5	0.25 (4)				
I-H	0 / 1248	-18.5	0.29 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.84")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.84")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.34/1.00 (E-F:1), BC=0.29/1.00 (K-L:1), WB=0.81/1.00 (E-H:1), SS=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

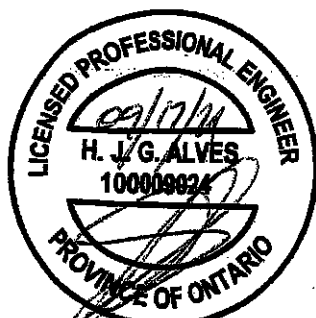
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

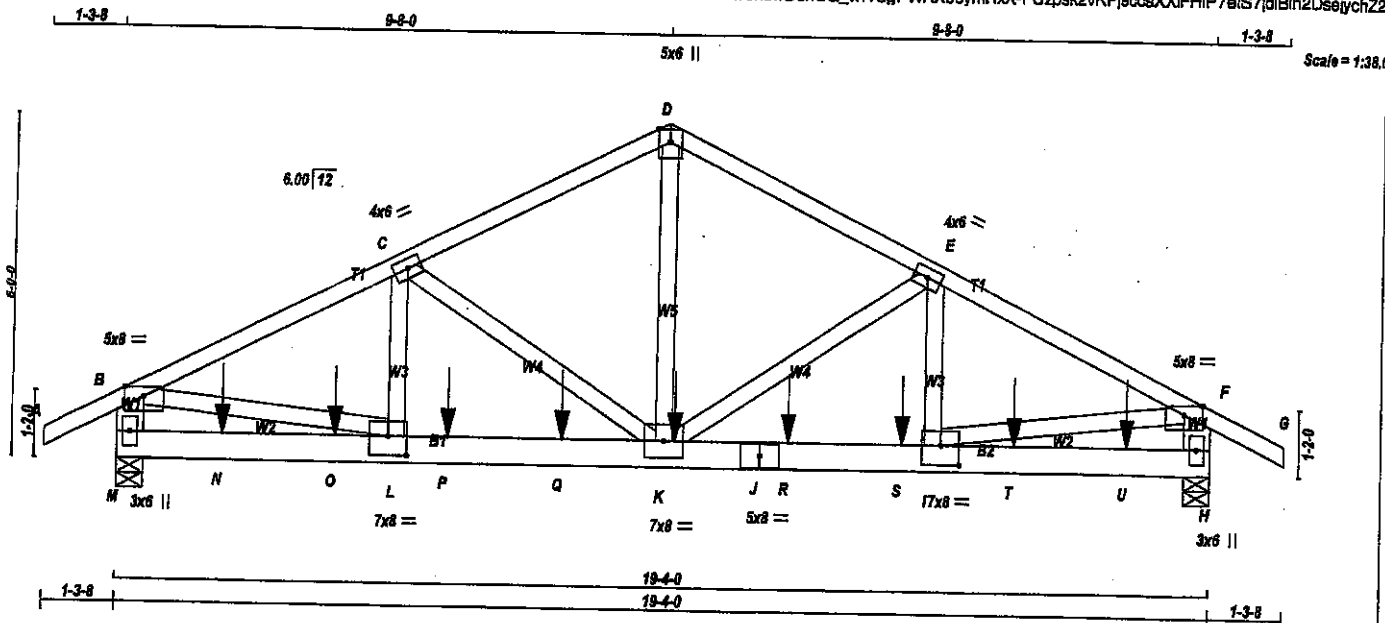
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (C) (INPUT = 0.90)
JSI METAL= 0.34 (E) (INPUT = 1.00)



JOB NAME 421518	TRUSS NAME T6	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Flat Truss, Burlington					

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ID:v9nswDdnCQ_wlVdqpWAX66ymHxX-FUzpsk2vKF8ccsXXIFH7eIS7jdBih2Dsejych22



TOTAL WEIGHT = 2 X 100 = 200 lb

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER	DESCR	
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS		2x4	DRY	No.2	SPF
EXCEPT					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1	12	TOP
D-G 1	12	TOP
M-B 2	12	TOP
H-F 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
J-M 2	12	SIDE(183.1)
J-H 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
K-D 1	6	SIDE(99.1)
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	DOWN	UP	DOWN
H 6142	0	0	5-8
M 5974	0	0	5-8

1ST LCASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT COMBINED	SNOW	LIVE	PERM. LIVE	
H 4393	2901 / 0	0 / 0	0 / 0	1432 / 0
M 4215	2822 / 0	0 / 0	0 / 0	1393 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO						FR-TO			
A-B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	K-D	0 / 5233	0.48 (1)	
B-C	-8372 / 0	-91.8	-91.8	0.51 (1)	3.09	K-E	-2300 / 0	0.45 (1)	
C-D	-6268 / 0	-91.8	-91.8	0.38 (1)	3.69	I-E	0 / 1731	0.15 (1)	
D-E	-6268 / 0	-91.8	-91.8	0.38 (1)	3.69	C-K	-2319 / 0	0.46 (1)	
E-F	-8355 / 0	-91.8	-91.8	0.51 (1)	3.09	L-C	0 / 1749	0.15 (1)	
F-G	0 / 28	-91.8	-91.8	0.07 (1)	10.00	B-L	0 / 7594	0.87 (1)	
M-B	-5243 / 0	0.0	0.0	0.18 (1)	6.38	I-F	0 / 7579	0.87 (1)	
H-F	-5233 / 0	0.0	0.0	0.18 (1)	6.39				
M-N	0 / 0	-18.5	-18.5	0.48 (1)	10.00				
N-O	0 / 0	-18.5	-18.5	0.48 (1)	10.00				
O-L	0 / 0	-18.5	-18.5	0.48 (1)	10.00				
L-P	0 / 7503	-18.5	-18.5	0.80 (1)	10.00				
P-Q	0 / 7503	-18.5	-18.5	0.80 (1)	10.00				
Q-K	0 / 7503	-18.5	-18.5	0.80 (1)	10.00				
K-J	0 / 7488	-18.5	-18.5	0.87 (1)	10.00				
J-R	0 / 7488	-18.5	-18.5	0.87 (1)	10.00				
R-S	0 / 7488	-18.5	-18.5	0.87 (1)	10.00				
S-I	0 / 7488	-18.5	-18.5	0.87 (1)	10.00				
I-T	0 / 0	-18.5	-18.5	0.49 (1)	10.00				
T-U	0 / 0	-18.5	-18.5	0.49 (1)	10.00				
U-H	0 / 0	-18.5	-18.5	0.49 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
K	9-10-4	-763	-763		FRONT	VERT	TOTAL		C1
N	1-10-4	-763	-763		FRONT	VERT	TOTAL		C1
O	3-10-4	-763	-763		FRONT	VERT	TOTAL		C1
P	5-10-4	-763	-763		FRONT	VERT	TOTAL		C1
Q	7-10-4	-763	-763		FRONT	VERT	TOTAL		C1
R	11-10-4	-763	-763		FRONT	VERT	TOTAL		C1
S	13-10-4	-763	-763		FRONT	VERT	TOTAL		C1
T	15-10-4	-763	-763		FRONT	VERT	TOTAL		C1
U	17-10-4	-763	-763		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.51/1.00 (B-C:1), BC=0.90/1.00 (K-L:1),
WB=0.87/1.00 (B-L:1), SS=0.52/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (I) (INPUT = 0.90)
JSI METAL = 0.93 (J) (INPUT = 1.00)



Structural component only
DWG# T-2132645

JOB NAME 421618	TRUSS NAME T6	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Rd Truss, Burlington

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ID:v9nswDdnCQ wIVdgPWAX86vmHxX-FUzask2vKF8ocsXXIFHlP7eiS7idBih2DseivchZ2

PLATES (All in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-W ₃	MT20	5.0	8.0	Edge	
C	TMV-W ₁	MT20	4.0	8.0		
D	TTW ₁ -p	MT20	5.0	6.0	Edge	
E	TMV-W ₁	MT20	4.0	8.0		
F	TMV-W ₃	MT20	5.0	8.0	Edge	
H	BMV ₁ -p	MT20	3.0	6.0		
I	BMV-W ₁	MT20	7.0	8.0	4.00	4.00
J	BS ₁	MT20	5.0	8.0		
K	BMV-W ₁	MT20	7.0	8.0		
L	BMV-W ₁	MT20	7.0	8.0	4.00	4.00
M	BMV ₁ -p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

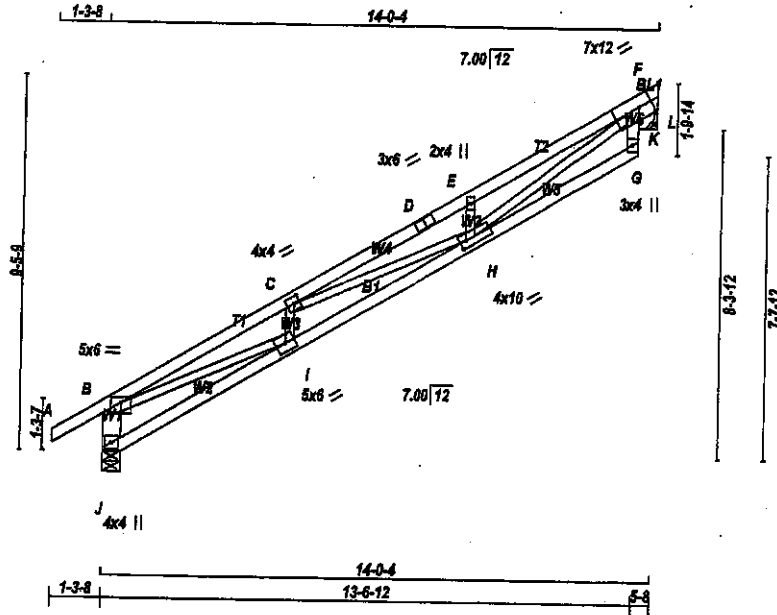
NOTES: (1)
1) Lateral bracing to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2132645 *gml*

JOB NAME 421618	TRUSS NAME T8	QUANTITY 10	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack: Red Truss, Burlington					

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ID:v9nswDdmCQ_wiVdgPWAX66ymHxX-igXB442X5Zr0EmRk5TmWFdgrBsaQMBqswizQAAYchZ1



Scale = 1/8" = 1'-0"

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
J - B	2x6	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	

BEARING BLOCKS	BL1	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-1	MT20	4.0	4.0	2.00	1.75
D	TS-1	MT20	3.0	8.0		
E	TMVW-w	MT20	2.0	4.0		
F	TMVWK1-1	MT20	7.0	12.0	Edge	4.00
G	BMV-p	MT20	3.0	4.0		
H	BMVW-1	MT20	4.0	10.0	2.00	1.75
I	BMVW-1	MT20	5.0	8.0		
J	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
J	890	0	0	5-8
L(F)	731	0	0	MECHANICAL

(** SEE "BEARING NOTE" **)

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L(F). MINIMUM BEARING LENGTH AT JOINT L(F) = 3'-8".

UNFACTORED REACTIONS

1ST LOASE	MAX/MIN. COMPONENT REACTIONS	JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	927	425 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	202 / 0	0 / 0
L(F)	517	339 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	178 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	WEBS	MAX. FACTORED	MEMB.	FORCE	MAX
	MEMB.	FORCE	(LBS)	(PLF)	CSI (LC)		MEMB.	FORCE	(LBS)	CSI (LC)
FR-TO						FR-TO				
J-B	-842 / 0	0.0	0.0	0.08 (1)	7.81	B-I	0 / 2237	0.50 (1)		
A-B	0 / 32	-91.8	-91.8	0.12 (1)	10.00	I-C	-408 / 0	0.08 (1)		
B-C	-2443 / 0	-91.8	-91.8	0.34 (1)	4.11	C-H	-83 / 0	0.03 (1)		
C-D	-2311 / 0	-91.8	-91.8	0.24 (1)	4.33	H-E	-383 / 0	0.08 (1)		
D-E	-2311 / 0	-91.8	-91.8	0.24 (1)	4.33	E-F	0 / 2305	0.52 (1)		
E-F	-2237 / 0	-91.8	-91.8	0.29 (1)	4.32	F-L	-797 / 0	0.05 (1)		
G-K	0 / 158	0.0	0.0	0.15 (1)	10.00	K-L	0 / 319	0.00 (1)		
K-F	0 / 158	0.0	0.0	0.15 (1)	10.00					
J-I	-1 / 3	-18.5	-18.5	0.10 (4)	10.00					
I-H	0 / 2374	-18.5	-18.5	0.49 (1)	10.00					
H-G	0 / 222	-18.5	-18.5	0.12 (1)	10.00					

TOTAL WEIGHT = 10 X 58 = 585 lb

(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 8.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 91.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.45")
CALCULATED VERT. DEFL. (LL) = $L/998$ (0.16")
ALLOWABLE DEFL. (TL) = $L/360$ (0.45")
CALCULATED VERT. DEFL. (TL) = $L/535$ (0.30")

CSI: TC=0.34/1.00 (B-C:1), BC=0.49/1.00 (H-I:1),
WB=0.52/1.00 (F-H:1), SS=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

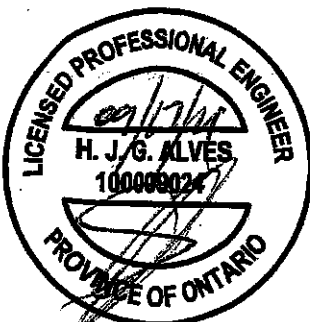
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	850	371
	1747	788
	1887	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

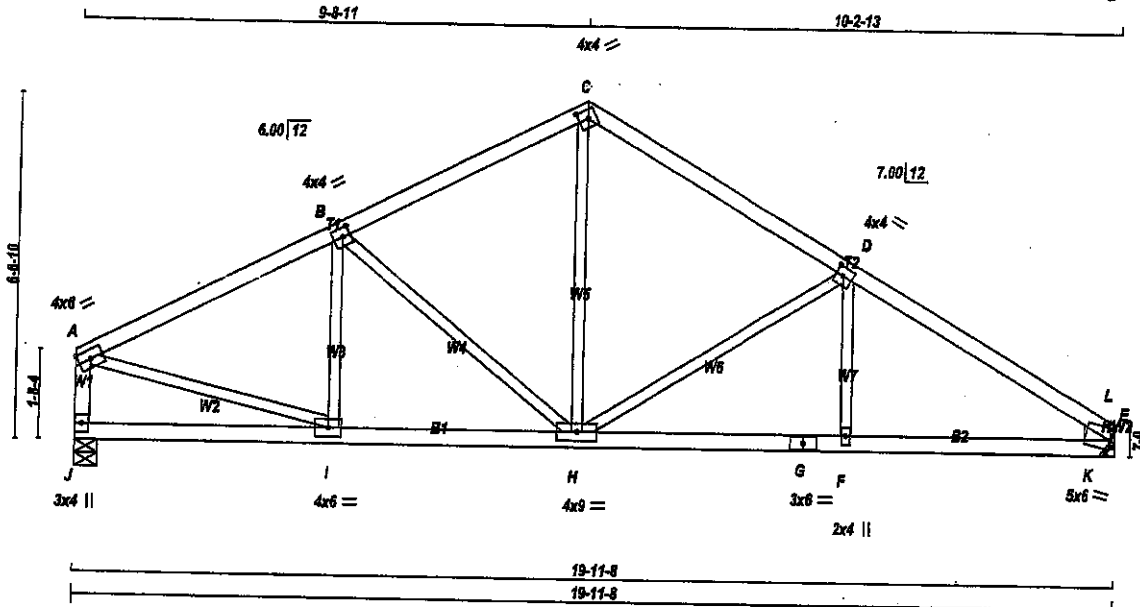
JSI GRIP = 0.87 (H) (INPUT = 0.90)
JSI METAL = 0.53 (I) (INPUT = 1.00)



Structural component only
DWG# T-2132646

JOB NAME 421618	TRUSS NAME T9	QUANTITY 9	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Rod Truss, Burlington					

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ID:v9newDdnCQ_wlVdgPWAX66ymHxX-igXB442X5Zr0EmRk5TmWfdgersd1MEAswizQAAYchZ1



Scale = 1:41.8

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - E	2x4	DRY	No.2	SPF		
J - A	2x4	DRY	No.2	SPF		
J - G	2x4	DRY	No.2	SPF		
G - E	2x4	DRY	No.2	SPF		
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		Edge
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TTW-t	MT20	4.0	4.0	2.00	2.25
D	TMVW-t	MT20	4.0	4.0	2.00	1.75
E	TMBH1-m	MT20	5.0	6.0		2.75 Edge
F	BMVW-w	MT20	2.0	4.0		
G	BS-t	MT20	3.0	6.0		
H	BMVW-w	MT20	4.0	9.0		
I	BMVW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
J	1100	0	1100	0	0
E	1100	0	1100	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED
J	778
E	778

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 MAX (PLF)	LC1 MAX (CSI (LC))	MAX. UNBRACED LENGTH (FT)	FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	LC1 MAX (PLF)	LC1 MAX (CSI (LC))
A-B	-1248 / 0	-91.8	-91.8	0.29 (1)	5.42	I-B	-224 / 2	0.06 (1)			
B-C	-1020 / 0	-91.8	-91.8	0.28 (1)	5.88	B-H	-320 / 0	0.20 (1)			
C-D	-1041 / 0	-91.8	-91.8	0.28 (1)	5.82	H-C	0 / 604	0.14 (1)			
D-L	-1559 / 0	-91.8	-91.8	0.26 (1)	5.02	H-D	-558 / 0	0.37 (1)			
L-E	-1824 / 0	-91.8	-91.8	0.20 (1)	4.77	F-D	0 / 99	0.03 (4)			
J-A	-1080 / 0	0.0	0.0	0.11 (1)	7.84	A-I	0 / 1180	0.27 (1)			
J-I	0 / 0	-18.5	-18.5	0.09 (4)	10.00	K-L	0 / 233	0.00 (1)			
I-H	0 / 1137	-18.5	-18.5	0.24 (1)	10.00						
H-G	0 / 1352	-18.5	-18.5	0.28 (1)	10.00						
G-F	0 / 1352	-18.5	-18.5	0.28 (1)	10.00						
F-K	0 / 1352	-18.5	-18.5	0.32 (1)	10.00						
K-E	0 / 1352	-18.5	-18.5	0.32 (1)	10.00						

TOTAL WEIGHT = 9 X 74 = 670 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.67")
CALCULATED VERT. DEFL. (LL) = L/989 (0.04")
ALLOWABLE DEFL. (TL) = L/360 (0.67")
CALCULATED VERT. DEFL. (TL) = L/989 (0.09")

CS: TC=0.29/1.00 (A-B:1), BC=0.32/1.00 (E-K:1), WB=0.37/1.00 (D-H:1), SS=0.20/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 660 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.44 (G) (INPUT = 1.00)



Structural component only
DWG# T-2132647



TOTAL WEIGHT = 9 X 74 = 666 lb

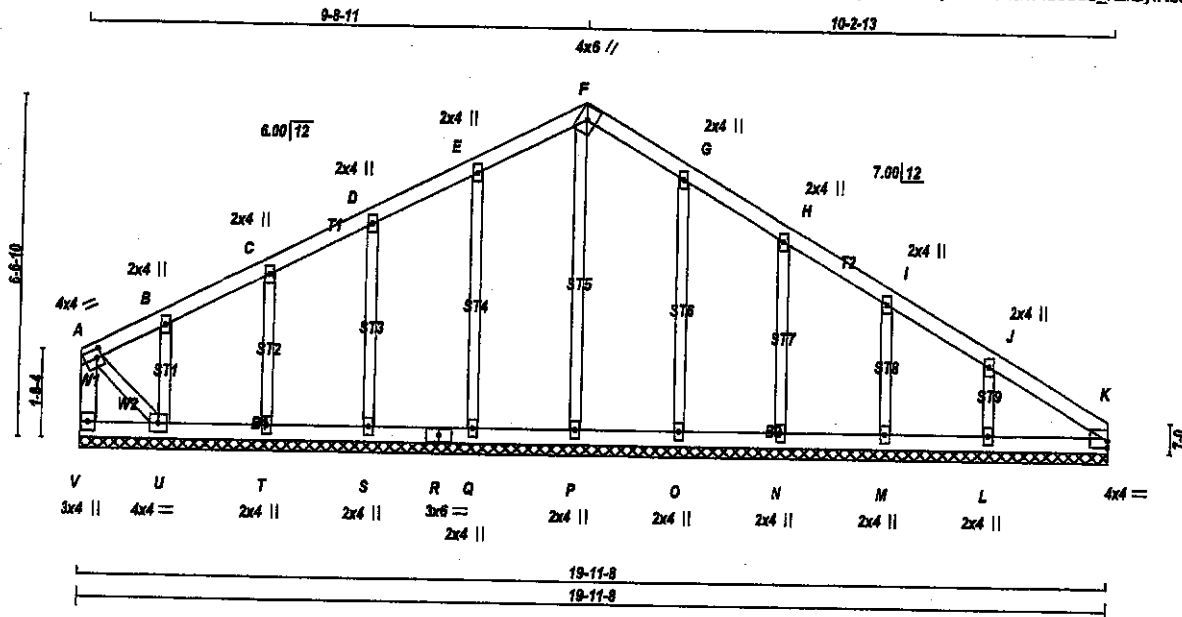
DRY: SEASONED LUMBER.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2

CHORDS				WEBB			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (I.C.)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (I.C.)
FR-TO		FROM TO			FR-TO		
A-B	-1125 / 0	-91.8	-91.8 0.29 (1)	5.84	J-B	-191 / 13	0.05 (1)
B-C	-869 / 0	-81.8	-91.8 0.28 (1)	6.22	B-H	-352 / 0	0.22 (1)
C-D	-888 / 0	-91.8	-91.8 0.28 (1)	6.18	I-C	0 / 461	0.10 (1)
D-E	-1092 / 0	-91.8	-91.8 0.25 (1)	5.76	I-D	-263 / 0	0.17 (1)
K-A	-974 / 0	0.0	0.0 0.10 (1)	7.81	G-D	-233 / 0	0.05 (1)
F-E	-987 / 0	0.0	0.0 0.10 (1)	7.81	A-J	0 / 1066	0.24 (1)
					G-E	0 / 1021	0.23 (1)
K-J	0 / 0	-18.5	-18.5 0.10 (4)	10.00			
J-I	0 / 1027	-18.5	-18.5 0.22 (1)	10.00			
I-H	0 / 970	-18.5	-18.5 0.21 (1)	10.00			
H-G	0 / 970	-18.5	-18.5 0.21 (1)	10.00			
G-F	0 / 0	-18.5	-18.5 0.08 (4)	10.00			

USI GRIP= 0.88 (E) (INPUT = 0.90)
USI METAL= 0.31 (E) (INPUT = 1.00)





Scale = 1:42

LUMBER	N. L. G. A.	RULES
CHORDS	SIZE	
V - A	2x4	DRY
A - F	2x4	DRY
F - K	2x4	DRY
V - R	2x4	DRY
R - K	2x4	DRY
ALL WEBS	2x3	DRY
ALL GABLE WEBS	2x3	DRY
DRY: SEASONED LUMBER.		
GABLE STUDS SPACED AT	2-0-0 OC.	

PLATES (table in inches)	W	LEN	Y	X
JT TYPE PLATES				
A TMVW-I MT20	4.0	4.0	2.00	1.25
B, C, D, E, G, H, I, J				
B TMVW-w MT20	2.0	4.0		
F TTW-h MT20	4.0	6.0		
K TMB1-I MT20	4.0	4.0	1.50	
L, M, N, O, P, Q, S, T				
L BMW1-w MT20	2.0	4.0		
R BS-I MT20	3.0	6.0		
U BMWVW-I MT20	4.0	4.0		
V BMW1-p MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
V-A	-91/0	0.0	0.0 0.01 (1)	P-F	-137/0	0.09 (1)	
A-B	-23/0	-91.8	-91.8 0.04 (1)	Q-E	-215/0	0.10 (1)	
B-C	-24/0	-91.8	-91.8 0.04 (1)	S-D	-174/0	0.05 (1)	
C-D	-21/0	-91.8	-91.8 0.04 (1)	T-C	-185/0	0.04 (1)	
D-E	-15/0	-91.8	-91.8 0.06 (1)	U-B	-177/0	0.03 (1)	
E-F	-23/0	-91.8	-91.8 0.06 (1)	O-G	-201/0	0.09 (1)	
F-G	-27/0	-91.8	-91.8 0.06 (1)	N-H	-180/0	0.05 (1)	
G-H	-20/0	-91.8	-91.8 0.05 (1)	M-I	-170/0	0.03 (1)	
H-I	-24/0	-91.8	-91.8 0.04 (1)	L-J	-216/0	0.03 (1)	
I-J	-35/0	-91.8	-91.8 0.06 (1)	A-U	0/38	0.01 (1)	
J-K	-32/0	-91.8	-91.8 0.06 (1)				
V-U	0/0	-18.5	-18.5 0.01 (4)				
U-T	0/22	-18.5	-18.5 0.02 (4)				
T-S	0/19	-18.5	-18.5 0.02 (4)				
S-R	0/16	-18.5	-18.5 0.02 (4)				
R-Q	0/16	-18.5	-18.5 0.02 (4)				
Q-P	0/13	-18.5	-18.5 0.02 (4)				
P-O	0/15	-18.5	-18.5 0.02 (4)				
O-N	0/18	-18.5	-18.5 0.02 (4)				
N-M	0/22	-18.5	-18.5 0.02 (4)				
M-L	0/26	-18.5	-18.5 0.02 (4)				
L-K	0/38	-18.5	-18.5 0.02 (4)				

TOTAL WEIGHT = 78 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (J-K:1), BC=0.02/1.00 (K-L:4), WB=0.10/1.00 (E-Q:1), SS=0.08/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

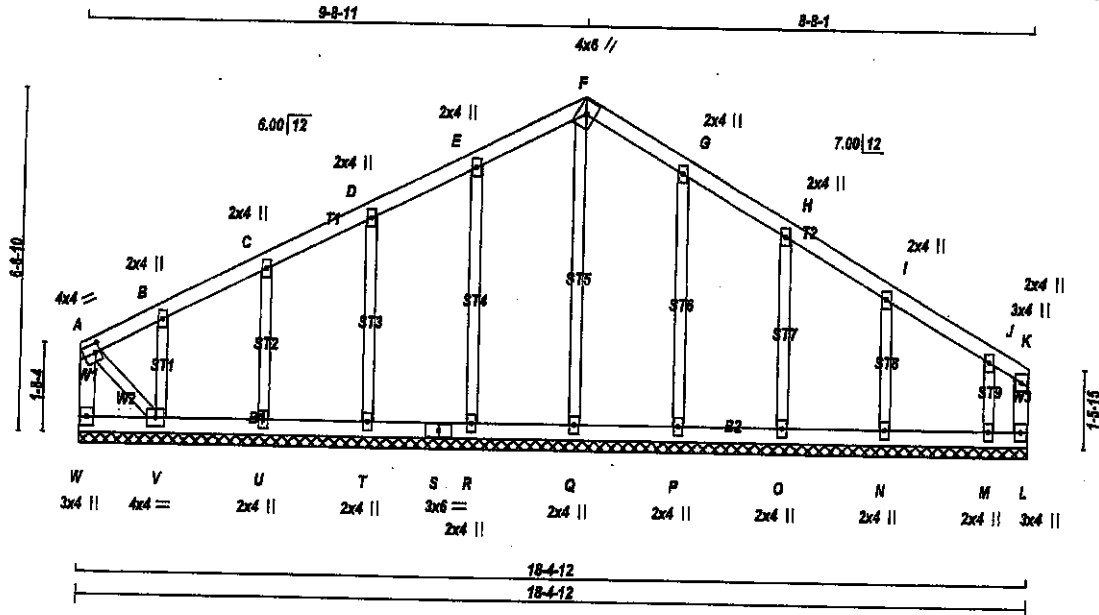
JSI GRIP = 0.85 (K) (INPUT = 0.90)
JSI METAL = 0.10 (J) (INPUT = 1.00)



Structural component only
DWG# T-2132649

JOB NAME 421618	TRUSS NAME T9GA	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Rd Truss, Burlington					

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ID:v9nswDdnCQ_wIVdgPWAX66ymHxX-7FCKI85PNUEb5EAlmbKDIrK3ITZfclB4nUychZ



Scale = 1:42.2

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
W - A	2x4	DRY	No.2	SPF		
A - F	2x4	DRY	No.2	SPF		
F - K	2x4	DRY	No.2	SPF		
L - K	2x4	DRY	No.2	SPF		
W - S	2x4	DRY	No.2	SPF		
S - L	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
ALL GABLE WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						
GABLE STUDS SPACED AT 2-0-0 OC.						

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 INCHES

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (D-E:1), BC=0.02/1.00 (N-O:4), WB=0.12/1.00 (F-Q:1), SS=0.03/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1967 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.82 (A) (INPUT = 0.90)
JSI METAL = 0.09 (G) (INPUT = 1.00)

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B, C, D, E, G, H, I, J						
B	TMVW-w	MT20	2.0	4.0		
F	TTW-h	MT20	4.0	6.0		
K	TMV-p	MT20	3.0	4.0		
L	BMV1+p	MT20	3.0	4.0		
M, N, O, P, Q, R, T, U						
M	BMV1+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	6.0		
V	BMVW1-t	MT20	4.0	4.0		
W	BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

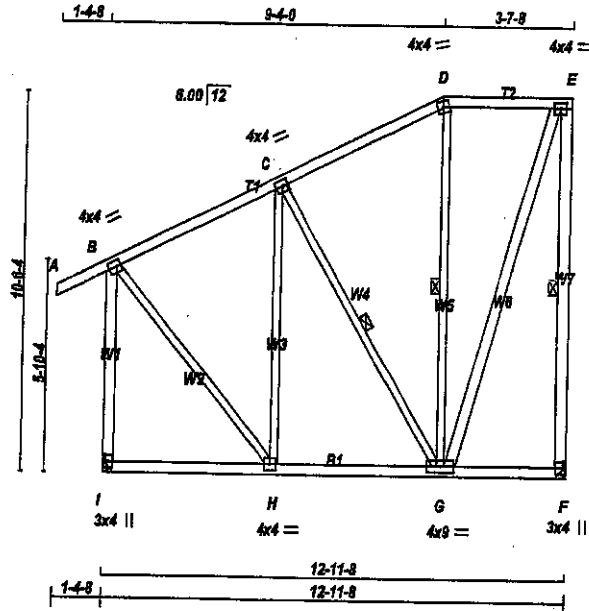
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
W-A	-45 / 0	0.0	0.0 0.00 (1)	7.81	Q-F	-173 / 0	0.12 (1)		
A-B	0 / 15	-91.8	-91.8 0.04 (1)	10.00	R-E	-216 / 0	0.10 (1)		
B-C	0 / 14	-91.8	-91.8 0.05 (1)	10.00	T-D	-174 / 0	0.05 (1)		
C-D	0 / 16	-91.8	-91.8 0.05 (1)	10.00	U-C	-185 / 0	0.04 (1)		
D-E	0 / 23	-91.8	-91.8 0.06 (1)	10.00	V-B	-177 / 0	0.03 (1)		
E-F	0 / 15	-91.8	-91.8 0.06 (1)	10.00	P-G	-203 / 0	0.09 (1)		
F-G	0 / 11	-91.8	-91.8 0.05 (1)	10.00	O-H	-178 / 0	0.05 (1)		
G-H	0 / 19	-91.8	-91.8 0.05 (1)	10.00	N-I	-189 / 0	0.03 (1)		
H-I	0 / 12	-91.8	-91.8 0.05 (1)	10.00	M-J	-143 / 0	0.02 (1)		
I-J	0 / 11	-91.8	-91.8 0.05 (1)	10.00	A-V	-8 / 0	0.00 (1)		
J-K	0 / 15	-91.8	-91.8 0.03 (1)	10.00					
L-K	-3 / 0	0.0	0.0 0.00 (1)	7.81					
W-V	0 / 0	-18.5	-18.5 0.01 (4)	10.00					
V-U	-11 / 0	-18.5	-18.5 0.01 (4)	8.25					
U-T	-15 / 0	-18.5	-18.5 0.01 (4)	8.25					
T-S	-18 / 0	-18.5	-18.5 0.01 (4)	8.25					
S-R	-18 / 0	-18.5	-18.5 0.01 (4)	8.25					
R-Q	-21 / 0	-18.5	-18.5 0.01 (4)	8.25					
Q-P	-18 / 0	-18.5	-18.5 0.01 (4)	8.25					
P-O	-15 / 0	-18.5	-18.5 0.01 (4)	8.25					
O-N	-12 / 0	-18.5	-18.5 0.02 (4)	8.25					
N-M	-7 / 0	-18.5	-18.5 0.02 (4)	10.00					
M-L	0 / 0	-18.5	-18.5 0.01 (4)	10.00					



Structural component only
DWG# T-2132650

JOB NAME 421618	TRUSS NAME T11	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Rod Truss, Burlington					

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Scale = 1:81.2

LUMBER	N. L. G. A.	RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF			
D - E	2x4	DRY	No.2	SPF			
F - E	2x4	DRY	No.2	SPF			
I - B	2x4	DRY	No.2	SPF			
I - F	2x4	DRY	No.2	SPF			
ALL WEBS EXCEPT G - E	2x3	DRY	No.2	SPF			
	2x4	DRY	No.2	SPF			

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	4.0	2.00	1.25
C	TMVW-1	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TMVW-1	MT20	4.0	4.0	2.00	1.75
F	BMV1-p	MT20	3.0	4.0		
G	BMVWW-1	MT20	4.0	9.0		
H	BMVWW-1	MT20	4.0	4.0		
I	BMV1-p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UP/LIFT
F	714	0	714	0
I	846	0	846	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, I. MINIMUM BEARING LENGTH AT JOINT F = 3-8, JOINT I = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED						
F	505	392 / 0	0 / 0	0 / 0	0 / 0	174 / 0	0 / 0
I	596	405 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
FR-TO												
A-B	0 / 30	-91.8	-91.8	0.13 (1)	10.00	H-C	-257 / 0	0.34 (1)				
B-C	-321 / 0	-91.8	-91.8	0.26 (1)	6.25	C-G	-248 / 0	0.15 (1)				
C-D	-228 / 0	-91.8	-91.8	0.26 (1)	6.25	G-D	-231 / 0	0.18 (1)				
D-E	-179 / 0	-91.8	-91.8	0.15 (1)	6.25	G-E	0 / 551	0.09 (1)				
E-F	-887 / 0	0.0	0.0	0.38 (1)	6.25	B-H	0 / 472	0.11 (1)				
I-B	-811 / 0	0.0	0.0	0.55 (1)	7.81							
I-H	0 / 0	-18.5	-18.5	0.10 (4)	10.00							
H-G	0 / 308	-18.5	-18.5	0.13 (4)	10.00							
G-F	0 / 0	-18.5	-18.5	0.06 (4)	10.00							

TOTAL WEIGHT = 5 X 89 = 445 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.43")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.43")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.55/1.00 (B-1), BC=0.13/1.00 (G-H-4), WB=0.34/1.00 (C-H-1), SS=0.19/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

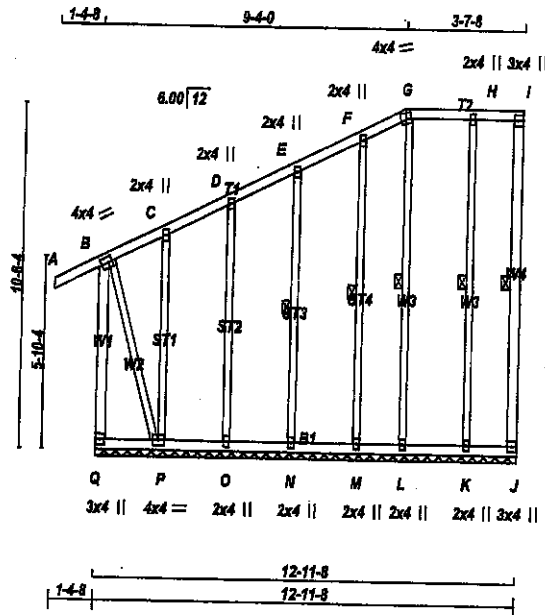
JSI GRIP = 0.84 (B) (INPUT = 0.90)
JSI METAL = 0.17 (B) (INPUT = 1.00)



Structural component only
DWG# T-2132651

JOB NAME 421618	TRUSS NAME T11G	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Rod Truss, Burlington					

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ID:v9nswDdnCQ_wlVdgPWAX66ymHxX-3eK47o6gv5UJXXKht0MhnygNkAtOu1Xgb3zgBrNychYy



Scale = 1:87.3

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
Q - B	2x4	DRY	No.2	DESCR.
A - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
Q - J	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" O.C.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-t	MT20	4.0	4.0 2.00 1.25
C, D, E, F, H				
C	TMVW-w	MT20	2.0	4.0
G	TTW-m	MT20	4.0	4.0
I	TMV-p	MT20	3.0	4.0
J	BMV1-p	MT20	3.0	4.0
K, L, M, N, O				
K	BMV1-w	MT20	2.0	4.0
P	BMVW1-t	MT20	4.0	4.0
Q	BMV1-p	MT20	3.0	4.0

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF J-I, E-N, F-M, G-L, H-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
Q-B	-264 / 0	0.0	0.0 0.19 (1)	P-C	-130 / 0	0.10 (1)	
A-B	0 / 30	-91.8	-91.8 0.13 (1)	O-D	-192 / 0	0.22 (1)	
B-C	-26 / 0	-91.8	-91.8 0.13 (1)	N-E	-185 / 0	0.09 (1)	
C-D	-2 / 0	-91.8	-91.8 0.04 (1)	M-F	-170 / 0	0.11 (1)	
D-E	-5 / 0	-91.8	-91.8 0.04 (1)	L-G	-128 / 0	0.10 (1)	
E-F	-4 / 0	-91.8	-91.8 0.04 (1)	K-H	-189 / 0	0.15 (1)	
F-G	-10 / 0	-91.8	-91.8 0.03 (1)	B-P	0 / 20	0.00 (1)	
G-H	0 / 0	-91.8	-91.8 0.05 (1)				
H-I	0 / 0	-91.8	-91.8 0.04 (1)				
J-I	-51 / 0	0.0	0.0 0.03 (1)				
Q-P	0 / 0	-18.5	-18.5 0.02 (4)				
P-O	0 / 6	-18.5	-18.5 0.02 (4)				
O-N	0 / 4	-18.5	-18.5 0.02 (4)				
N-M	0 / 2	-18.5	-18.5 0.02 (4)				
M-L	0 / 1	-18.5	-18.5 0.01 (4)				
L-K	0 / 0	-18.5	-18.5 0.01 (4)				
K-J	0 / 0	-18.5	-18.5 0.01 (4)				

TOTAL WEIGHT = 98 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19/1.00 (B-Q:1), BC=0.02/1.00 (O-P:4), WB=0.22/1.00 (D-O:1), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLJ) (PLJ)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

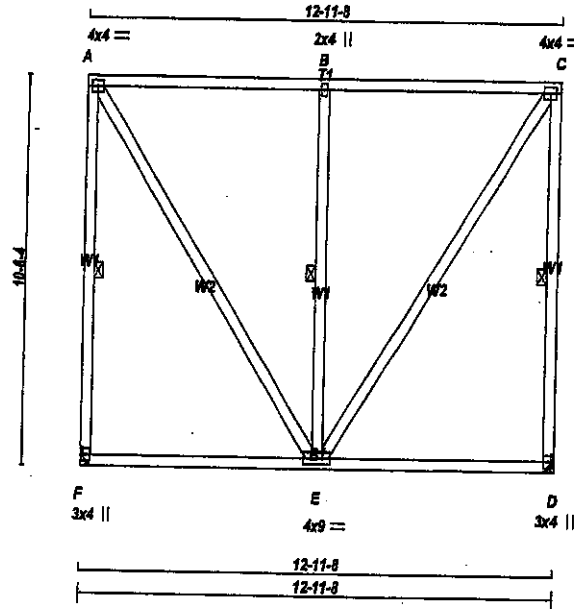
JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)



Structural component only
DWG# T-2132652

JOB NAME 421618	TRUSS NAME T12	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Flat Truss, Burlington					

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ID:v9nswDdnCQ_wlVdGFWAX66ymHxX-YquTK77igPcAyhvtRktwUuwodGhxmx0kldCkNpychYx



Scale = 1:59.7

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF		
A - C	2x4	DRY	No.2	SPF		
D - C	2x4	DRY	No.2	SPF		
F - D	2x4	DRY	No.2	SPF		
ALL WEBS	2x4	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	714	0	714	0
D	714	0	714	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 3-8, JOINT D = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. / MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
F	505	332 / 0	0 / 0	0 / 0	174 / 0	0 / 0
D	505	332 / 0	0 / 0	0 / 0	174 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-F, C-D, B-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			FACTORED				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. CSI (LC)	UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO				FR-TO				
F-A	-668 / 0	0.0	0.0	0.37 (1)	6.25	A-E	0 / 526	0.08 (1)		
A-B	-281 / 0	-91.8	-91.8	0.66 (1)	6.25	E-B	-741 / 0	0.41 (1)		
B-C	-281 / 0	-91.8	-91.8	0.66 (1)	6.25	E-C	0 / 526	0.08 (1)		
D-C	-668 / 0	0.0	0.0	0.37 (1)	6.25					
F-E	0 / 0	-18.5	-18.5	0.22 (4)	10.00					
E-D	0 / 0	-18.5	-18.5	0.22 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
ROT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OSC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CS1: TC=0.66/1.00 (A-B:1), BC=0.22/1.00 (E-F:4), WB=0.41/1.00 (B-E:1), SS=0.29/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1887 1873

PLATE PLACEMENT TOL = 0.250 Inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.52 (C) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

PLATES (table is in inches)

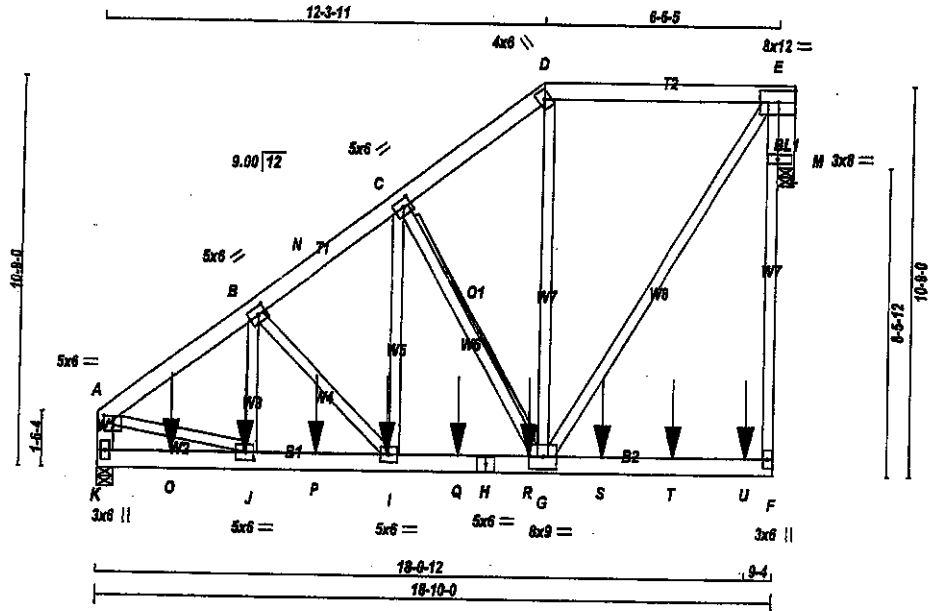
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0		
B	TMW-w	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
D	BMV1+p	MT20	3.0	4.0		
E	BMVWW-t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2132653



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - D	2x6	DRY	No.2
D - E	2x6	DRY	No.2
F - E	2x4	DRY	No.2
K - A	2x6	DRY	No.2
K - H	2x6	DRY	No.2
H - F	2x6	DRY	No.2
BEARING BLOCKS			
BL1	2x6	DRY	No.2
ALL WEBS EXCEPT			
	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D	2	12
D-E	2	12
K-A	2	12
E-F	1	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
K-H	2	12
H-F	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
B-J	1	6
C-I	1	6
2x4	1	6
2x6	2	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	M(E)	VERT	HORZ	DOWN	UP/LIFT
JT	M(E)	4580	0	4580	0
K		4609	0	4609	0

UNFACTORED REACTIONS

1ST LOASE		MAX/MIN. COMPONENT REACTIONS		PERM. LIVE		WIND	DEAD	SOIL
JT	M(E)	COMBINED	SNOW	LIVE	DOWN	UP/LIFT		
JT	M(E)	3229	2177 / 0	0 / 0	0 / 0	0 / 0	1052 / 0	0 / 0
K		3259	2139 / 0	0 / 0	0 / 0	0 / 0	1120 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M(E), K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-G

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
MEMB.	FR-TO				MEMB.		
A-B	-5040 / 0	-154.3	-110.1	0.14 (1)	J-B	0 / 1005	0.09 (1)
B-N	-4007 / 0	-110.1	-91.8	0.07 (1)	B-I	-1191 / 0	0.23 (1)
N-C	-4007 / 0	-91.8	-91.8	0.07 (1)	I-C	0 / 2278	0.20 (1)
C-D	-2570 / 0	-91.8	-91.8	0.08 (1)	C-G	-2177 / 0	0.82 (1)
D-E	-2132 / 0	-91.8	-91.8	0.19 (1)	G-D	0 / 1136	0.10 (1)
F-L	0 / 1017	0.0	0.0	0.72 (1)	A-J	0 / 4145	0.37 (1)
L-E	0 / 1017	0.0	0.0	0.72 (1)	G-E	0 / 3851	0.34 (1)
K-A	-4259 / 0	0.0	0.0	0.15 (1)	E-M	-4509 / 0	0.21 (1)
					L-M	0 / 514	0.00 (1)
K-O	0 / 0	-18.5	-18.5	0.25 (1)			
O-J	0 / 0	-18.5	-18.5	0.25 (1)			
J-P	0 / 4033	-18.5	-18.5	0.46 (1)			
P-I	0 / 4033	-18.5	-18.5	0.46 (1)			
I-Q	0 / 3207	-18.5	-18.5	0.74 (1)			
Q-H	0 / 3207	-18.5	-18.5	0.74 (1)			
H-R	0 / 3207	-18.5	-18.5	0.74 (1)			
R-G	0 / 3207	-18.5	-18.5	0.74 (1)			
G-S	0 / 102	-18.5	-18.5	0.53 (1)			
S-T	0 / 102	-18.5	-18.5	0.53 (1)			
T-U	0 / 102	-18.5	-18.5	0.53 (1)			
U-F	0 / 102	-18.5	-18.5	0.53 (1)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	FACE	DIR.	TYPE	HEEL	CONN.
JT	8-0-12	-581	-581	---	BACK	VERT	TOTAL	C1
I	4-0-12	-581	-581	---	BACK	VERT	TOTAL	C1
O	2-0-12	-581	-581	---	BACK	VERT	TOTAL	C1
P	8-0-12	-581	-581	---	BACK	VERT	TOTAL	C1
Q	10-0-12	-581	-581	---	BACK	VERT	TOTAL	C1
R	12-0-12	-490	-490	---	BACK	VERT	TOTAL	C1
S	14-0-12	-490	-490	---	BACK	VERT	TOTAL	C1
T	16-0-12	-490	-490	---	BACK	VERT	TOTAL	C1

TOTAL WEIGHT = 2 X 146 = 292 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 38.0	PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/899 (0.19")

CSI: TC=0.72/1.00 (F-L:1), BC=0.74/1.00 (G-I:1),
WB=0.82/1.00 (C-G:1), SS=0.44/1.00 (G-I:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (J) (INPUT = 0.90)
JSI METAL = 0.47 (A) (INPUT = 1.00)



JOB NAME 421618	TRUSS NAME T13	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Fri Sep 17 16:06:27 2021 Page 2
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PLATES (All in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV-W-p	MT20	5.0	6.0	1.25	3.25
B	TMW-WH	MT20	5.0	6.0	2.50	2.75
C	TMW-WH	MT20	5.0	6.0	2.50	2.75
D	TTW-h	MT20	4.0	6.0		
E	TMV-WKH	MT20	6.0	12.0		Edge
F	BMV-p	MT20	3.0	6.0		
G	BMW-WH	MT20	6.0	9.0		
H	BS-H	MT20	5.0	6.0		
I	BMW-WH	MT20	5.0	6.0		
J	BMW-WH	MT20	5.0	6.0	2.50	2.75
K	BMV-H-p	MT20	3.0	6.0		
L	KP-p	MT20	3.0	6.0	1.50	3.25

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES (1)

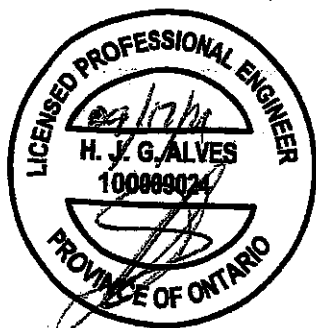
1) Lateral bracing to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
U	18-0-12	-492	-492	-	BACK	VERT	TOTAL	-	C1

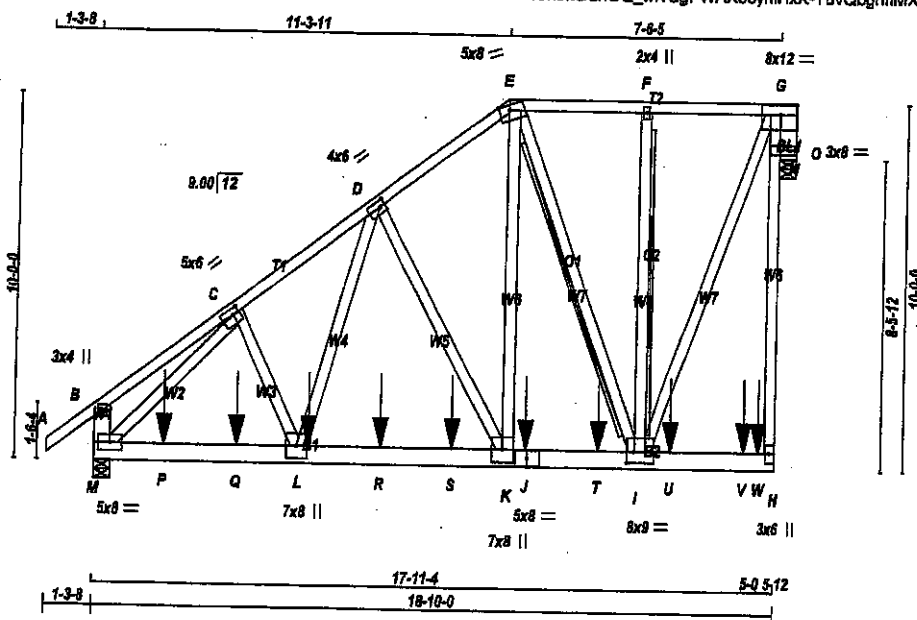
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2132654 *en*

JOB NAME 421618	TRUSS NAME T14	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Rod Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Sep 17 16:51:24 2021 Page 1 ID:v9nswDdnCQ_wfVdgPWAX66ymHbX-XyuvQbgnfIMXMeJSQUMHcmrfrnCv30sIO_W4Rygcun	



Scale = 1/8" = 1'-0"

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	SPF
A - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	2100F 1.8E	SPF	
M - B	2x6	DRY	No.2	SPF	
M - J	2x6	DRY	No.2	SPF	
J - H	2x6	DRY	No.2	SPF	
BEARING BLOCKS					
BL1	2x6	DRY	No.2	SPF	
ALL WEBS					
EXCEPT	2x4	DRY	No.2	SPF	
D - K	2x4	DRY	2100F 1.8E	SPF	
K - E	2x4	DRY	2100F 1.8E	SPF	
M - C	2x4	DRY	2100F 1.8E	SPF	
I - F	2x4	DRY	2100F 1.8E	SPF	

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0	2.00	2.00
D	TMVW-t	MT20	4.0	6.0	2.00	1.75
E	TMVW-m	MT20	5.0	8.0	1.75	3.25
F	TMVW-w	MT20	2.0	4.0		
G	TMVWK-t	MT20	8.0	12.0	3.25	5.50
H	BMV-p	MT20	3.0	8.0		
I	BMVW-p	MT20	8.0	8.0	3.50	2.50
J	BS-t	MT20	5.0	8.0		
K	BMVW-t	MT20	7.0	8.0		
L	BMVW-t	MT20	7.0	8.0		
M	BMVW-t	MT20	5.0	8.0		
N	KP-p	MT20	3.0	8.0	1.50	3.25

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

BEARINGS							
JT	FACTORED		MAXIMUM FACTORED			INPUT	REQD
	GROSS REACTION		GROSS REACTION			BRG	BRG
O(G)	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	2778	0	2778	0	0	5-8	5-8
M	2760	0	2760	0	0	5-8	5-8
							(** SEE "BEARING NOTE" **)

(** SEE "BEARING NOTE" **)

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
O(G)	2041	908 / 0	0 / 0	0 / 0	0 / 0	1138 / 0	0 / 0
M	2031	888 / 0	0 / 0	0 / 0	0 / 0	1143 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O(G), M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-I, E-I

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		W E B S		MAX. FACTORED	
	FORCE (LBS)	VERT. LOAD	LC1 MAX	LC1 (LC)	MEMB.	FORCE (LBS)	MAX	LC1 (LC)
FR-TO					FR-TO			
A-B	0 / 38	-91.8	-91.8	0.15 (1)	C-L	0 / 229	0.06 (4)	
B-C	0 / 18	-91.8	-91.8	0.18 (1)	L-D	0 / 1138	0.22 (4)	
C-D	-2630 / 0	-91.8	-91.8	0.38 (1)	D-K	-1007 / 0	0.57 (1)	
D-E	-1654 / 0	-91.8	-91.8	0.28 (1)	E-K	0 / 1722	0.15 (1)	
E-F	-935 / 0	-91.8	-91.8	0.25 (1)	M-C	-2931 / 0	0.81 (1)	
F-G	-935 / 0	-91.8	-91.8	0.25 (1)	I-F	-418 / 0	0.18 (1)	
H-N	0 / 422	0.0	0.0	0.82 (1)	E-I	-1083 / 0	0.79 (1)	
N-G	0 / 422	0.0	0.0	0.82 (1)	I-G	0 / 2388	0.46 (1)	
M-B	-269 / 0	0.0	0.0	0.02 (1)	G-O	-2816 / 0	0.23 (1)	
					N-O	0 / 483	0.00 (1)	
M-P	0 / 2026	-118.5	-118.5	0.79 (4)				
P-Q	0 / 2026	-118.5	-118.5	0.79 (4)				
Q-L	0 / 2026	-118.5	-118.5	0.79 (4)				
L-R	0 / 1790	-118.5	-118.5	0.77 (4)				
R-S	0 / 1790	-118.5	-118.5	0.77 (4)				
S-K	0 / 1790	-118.5	-118.5	0.77 (4)				
K-J	0 / 1330	-118.5	-118.5	0.43 (4)				
J-T	0 / 1330	-118.5	-118.5	0.43 (4)				
T-I	0 / 1330	-118.5	-118.5	0.43 (4)				
I-U	0 / 66	-118.5	-118.5	0.25 (4)				
U-V	0 / 66	-118.5	-118.5	0.25 (4)				
V-W	0 / 66	-118.5	-118.5	0.25 (4)				
W-H	0 / 66	-118.5	-118.5	0.25 (4)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
J	11-11.4	-101	-101	---	FRONT	VERT	TOTAL	C1
L	5-11.4	-101	-101	---	FRONT	VERT	TOTAL	C1
P	1-11.4	-101	-101	---	FRONT	VERT	TOTAL	C1
Q	3-11.4	-101	-101	---	FRONT	VERT	TOTAL	C1
R	7-11.4	-101	-101	---	FRONT	VERT	TOTAL	C1
S	9-11.4	-101	-101	---	FRONT	VERT	TOTAL	C1

TOTAL WEIGHT = 146 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.62/1.00 (H-N:1), BC=0.79/1.00 (L-M:4), WB=0.79/1.00 (E-I:1), SS=0.51/1.00 (L-M:4)

DOL LUMBER=0.92 NAIL=0.92 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

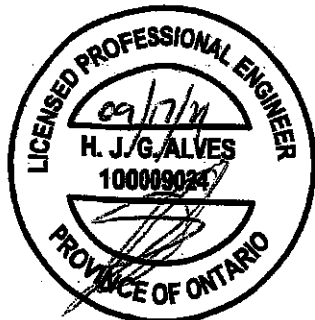
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(FSD) (FLJ) (PLJ)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.80)
JSI METAL= 0.71 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2132655 *in*

JOB NAME 421618	TRUSS NAME T14	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Post Truss, Burlington				Version 8.420 S Jan 21 2021 MITek Industries, Inc. Fri Sep 17 16:51:24 2021 Page 2 ID: v9nswDdhCO w/VdgPWAX66vmHxx-YuvQbanfIMXMeJ8QUMHcrrfIrNCv30siO W4Rvcgun	

SPECIFIED CONCENTRATED LOADS (LBS)

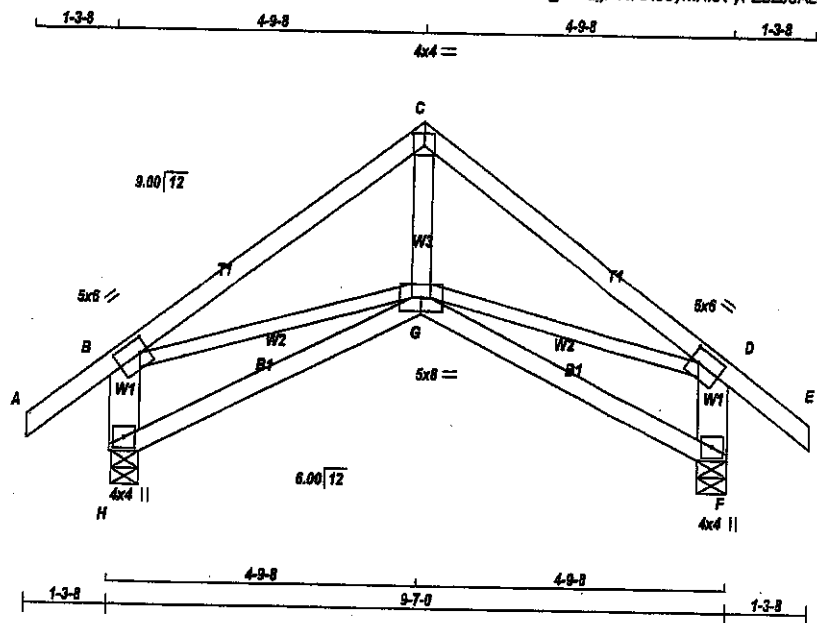
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	13-11-4	-101	-101	—	FRONT	VERT	TOTAL	—	C1
U	15-11-4	-101	-101	—	FRONT	VERT	TOTAL	—	C1
V	17-11-4	-101	-101	—	FRONT	VERT	TOTAL	—	C1
W	18-4-4	-104	-104	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2132655 *ML*



Scale = 1/32"

LUMBER	N. L. G. A.	RULES	SIZE	DRY	LUMBER	DESCR.
CHORDS	A - C	2x4	DRY	No.2	SPF	
	C - E	2x4	DRY	No.2	SPF	
	H - B	2x6	DRY	No.2	SPF	
	F - D	2x6	DRY	No.2	SPF	
	H - G	2x4	DRY	No.2	SPF	
	G - F	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	G - C	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT	TYPE	PLATES	W	LEN	Y	X
	B	TMVW-t	MT20	5.0	6.0	2.50	1.50
	C	TTW-p	MT20	4.0	4.0	2.25	2.00
	D	TMVW-t	MT20	5.0	6.0	2.50	1.50
	F	BMV1-p	MT20	4.0	4.0		
	G	BMVWV-p	MT20	5.0	3.0		
	H	BMV1-p	MT20	4.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP/LIFT	IN-SX
H	655	0	0	5-0
F	655	0	0	5-8

UNFACTORED REACTIONS	1ST CASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM.LIVE	
H	481	318 / 0	0 / 0	0 / 0	145 / 0
F	461	318 / 0	0 / 0	0 / 0	145 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO						FR-TO			
A-B	0 / 38	-91.8	-91.8	0.12 (1)	10.00	G-C	0 / 290	0.05 (1)	
B-C	-608 / 0	-91.8	-91.8	0.28 (1)	6.25	B-G	0 / 497	0.11 (1)	
C-D	-608 / 0	-91.8	-91.8	0.28 (1)	6.25	G-D	0 / 497	0.11 (1)	
D-E	0 / 38	-91.8	-91.8	0.12 (1)	10.00				
H-B	-811 / 0	0.0	0.0	0.04 (1)	7.81				
F-D	-811 / 0	0.0	0.0	0.04 (1)	7.81				
H-G	0 / 0	-18.5	-18.5	0.12 (4)	10.00				
G-F	0 / 0	-18.5	-18.5	0.12 (4)	10.00				

TOTAL WEIGHT = 5 X 45 = 225 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.28/1.00 (B-C:1), BC=0.12/1.00 (G-H:4), WB=0.11/1.00 (B-G:1), SS=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

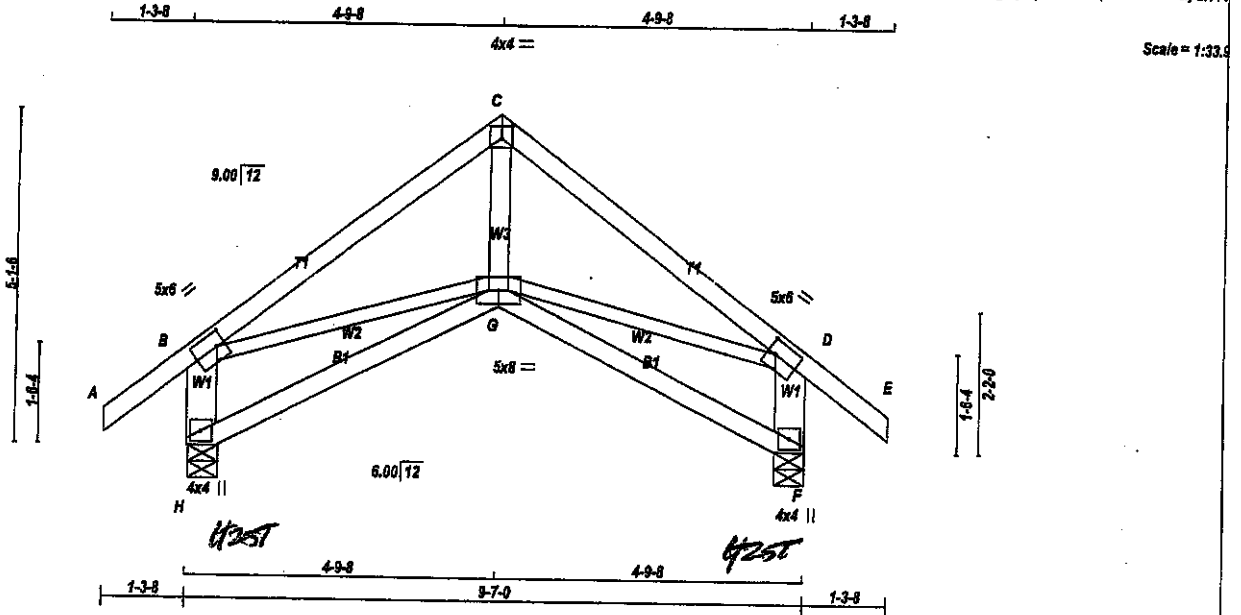
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (C) (INPUT = 0.90)
JSI METAL= 0.19 (B) (INPUT = 1.00)



JOB NAME 421618	TRUSS NAME T15SW	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Rd Truss, Burlington					

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Structural component only
DWG# T-2132658

JSI GRIP= 0.57 (F) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)



TOTAL WEIGHT = 2 X 59 = 118 lb

DRY; SEASONED LUMBER.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

BEARINGS

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G. E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBB			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	-704 / 0	-91.8	-91.8 0.22 (1)	6.25	A-F	0 / 586	0.13 (1)
B-C	-704 / 0	-91.8	-91.8 0.21 (1)	6.25	F-B	-452 / 0	0.06 (1)
C-D	0 / 0	-91.8	-91.8 0.02 (1)	10.00	F-C	0 / 840	0.19 (1)
G-A	-484 / 0	0.0	0.0 0.03 (1)	7.81	C-E	-472 / 0	0.54 (1)
E-D	-47 / 0	0.0	0.0 0.01 (1)	6.25			
G-F	0 / 0	-18.5	-18.5 0.12 (4)	10.00			
F-E	0 / 69	-18.5	-18.5 0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS-

TOP CH.	LL =	25.8	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.32")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.32")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.05")

CSI: TC=0.22/1.00 (A-B:1), BC=0.13/1.00 (E-F:4),
WB=0.64/1.00 (C-E:1), SSI=0.16/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

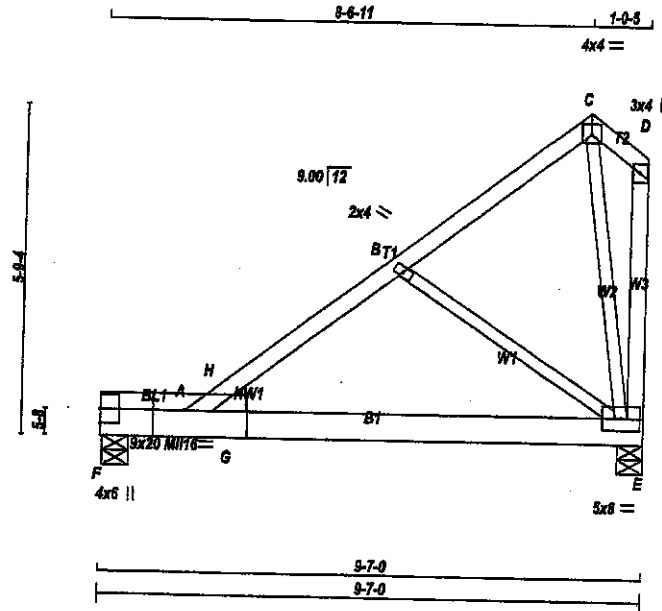
JSI GRIP= 0.83 (A) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)



Structural component only
DWG# T-2132659

JOB NAME 421618	TRUSS NAME T18	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Rod Truss, Burlington					

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Scale = 1/32"

LUMBER	N. L. G. A.	RULES
CHORDS	SIZE	
A - C	2x4	DRY
C - D	2x4	DRY
E - D	2x4	DRY
F - E	2x6	DRY

BEARING BLOCKS		
BL1	2x4	DRY
ALL WEBS	2x3	DRY
DRY: SEASONED LUMBER.		

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TBMHK	MI16	9.0	20.0	Edge	6.50
B	TTW-w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TTW-p	MT20	3.0	4.0		
E	BMVWW1-t	MT20	5.0	8.0		
F	BMKM1-t+m	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD	HEEL
JT	GROSS REACTION	GROSS REACTION	BRG	BRG	WEDGE
F	VERT	HORZ	DOWN	HORZ	UPLIFT
E	526	0	526	0	0
F	526	0	526	0	0

UNFACTORED REACTIONS							
1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	372	244 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0
E	372	244 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO								
A-H	-702 / 0	-91.8	-91.8	0.37 (1)	6.25	C-E	-125 / 0	0.08 (1)
H-B	-389 / 0	-91.8	-91.8	0.36 (1)	6.25	B-E	-435 / 0	0.17 (1)
B-C	-88 / 0	-91.8	-91.8	0.28 (1)	6.25	G-H	0 / 335	0.09 (1)
C-D	0 / 0	-91.8	-91.8	0.02 (1)	10.00			
E-D	-47 / 0	0.0	0.0	0.02 (1)	7.81			
F-A	0 / 0	-110.3	-110.3	0.27 (1)	10.00			
A-G	0 / 389	-18.5	-18.5	0.47 (1)	10.00			
G-E	0 / 389	-18.5	-18.5	0.31 (1)	10.00			

TOTAL WEIGHT = 45 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 33.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/899 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/597 (0.19")

CSI: TC=0.37/1.00 (A-H:1), BC=0.47/1.00 (A-G:1), WB=0.17/1.00 (B-E:1), SSI=0.22/1.00 (A-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

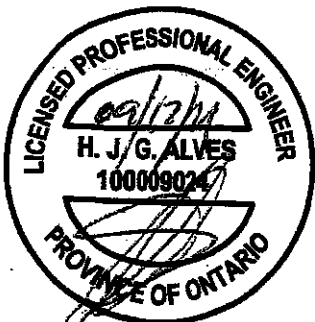
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)	(PL)
	MAX	MIN	MAX	MIN
MI16	438	302	2547	1256
MT20	650	371	1747	788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

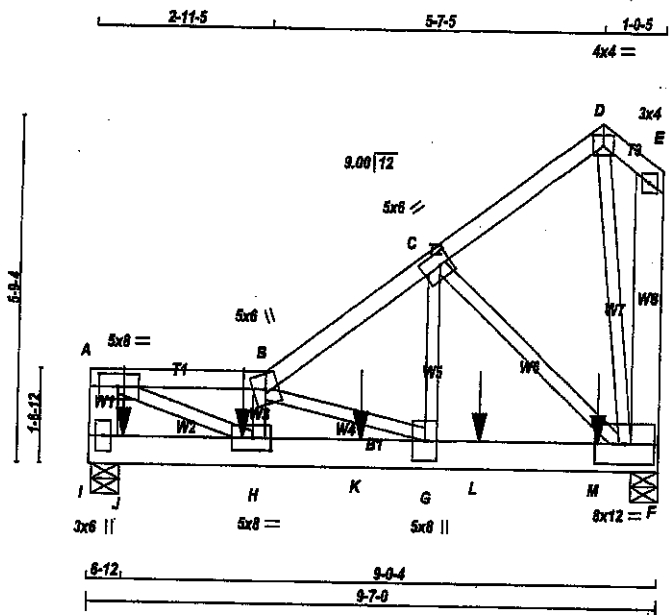


Structural component only
DWG# T-2132660

JOB NAME 421618	TRUSS NAME T19	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Fri Sep 17 16:08:32 2021 Page 1
ID:v9nswDdnCQ_wlVdgPWAX66ymHxX-M_FkbBC3GFMJgcM1o_Kl8Ax2hedAatdgZt2aTychYr



Scale = 1:36.8

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
I - A	2x6	DRY	No.2	SPF		
A - B	2x4	DRY	No.2	SPF		
B - D	2x4	DRY	No.2	SPF		
D - E	2x4	DRY	No.2	SPF		
E - F	2x6	DRY	No.2	SPF		
F - G	2x6	DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
I-A 2 12		TOP
F-E 2 12		TOP
A-B 1 12		TOP
B-D 1 12		TOP
D-E 1 12		TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
I-F 2 12		TOP
WEBS: (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW-t	MT20	5.0	8.0	2.50	3.75
B TTWW+m	MT20	5.0	8.0		
C TMWW-t	MT20	5.0	8.0	2.50	2.25
D TTW-p	MT20	4.0	4.0	2.25	2.00
E TMV-p	MT20	3.0	4.0		
F BMVWW-t	MT20	8.0	12.0		
G BMVWW-t	MT20	5.0	8.0		
H BMVWW-t	MT20	5.0	8.0	2.50	3.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ
JT	3111	0	3111	0
I	2928	0	2928	0
F	2928	0	2928	0

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	2198	1484 / 0	0 / 0	0 / 0	0 / 0	732 / 0	0 / 0
I	2088	1379 / 0	0 / 0	0 / 0	0 / 0	687 / 0	0 / 0
F	2088	1379 / 0	0 / 0	0 / 0	0 / 0	687 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.29 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
I-A	-2104 / 0	0.0	0.0 0.07 (1)	7.81	A-H	0 / 5180	0.64 (1)
A-B	-4779 / 0	-91.8	-91.8 0.15 (1)	4.29	H-B	-957 / 0	0.07 (1)
B-C	-2898 / 0	-91.8	-91.8 0.08 (1)	5.34	D-F	-135 / 0	0.03 (1)
C-D	-46 / 0	-91.8	-91.8 0.05 (1)	6.25	C-F	-3082 / 0	0.54 (1)
D-E	0 / 0	-91.8	-91.8 0.01 (1)	10.00	G-C	0 / 3376	0.42 (1)
F-E	-47 / 0	0.0	0.0 0.01 (1)	7.81	B-G	-2738 / 0	0.29 (1)
I-J	0 / 0	-18.5	-18.5 0.21 (1)	10.00			
J-H	0 / 0	-18.5	-18.5 0.21 (1)	10.00			
H-K	0 / 4884	-18.5	-18.5 0.59 (1)	10.00			
K-G	0 / 4884	-18.5	-18.5 0.59 (1)	10.00			
G-L	0 / 2337	-18.5	-18.5 0.44 (1)	10.00			
L-M	0 / 2337	-18.5	-18.5 0.44 (1)	10.00			
M-F	0 / 2337	-18.5	-18.5 0.44 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
H	2-6-12	-702	-702	TOP	VERT	TOTAL	---	C1
J	8-12	-708	-708	TOP	VERT	TOTAL	---	C1
K	4-6-12	-702	-702	TOP	VERT	TOTAL	---	C1
L	6-6-12	-702	-702	TOP	VERT	TOTAL	---	C1
M	8-6-12	-702	-702	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 55 = 110 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.32")
CALCULATED VERT. DEFL. (LL) = L/999 (0.05")
ALLOWABLE DEFL. (TL) = L/360 (0.32")
CALCULATED VERT. DEFL. (TL) = L/999 (0.09")

CSI: TC=0.15/1.00 (A-B:1), BC=0.59/1.00 (G-H:1), WB=0.64/1.00 (A-H:1), SS=0.42/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

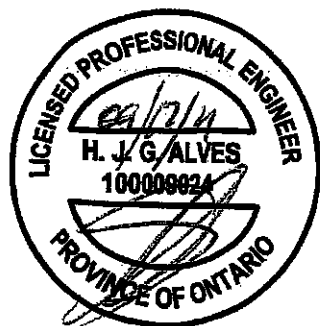
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PL)	SECTION (PL)
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1887 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.83 (H) (INPUT = 0.90)
JSI METAL = 0.46 (B) (INPUT = 1.00)



Structural component only
DWG# T-2132661

CONTINUED ON PAGE 2

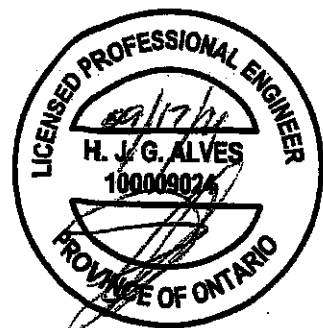
JOB NAME 421618	TRUSS NAME T19	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Rod Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Fri Sep 17 16:06:32 2021 Page 2
ID:v9nswDdnCQ wIVdgPWAX66ymHxX-M FkBC3GFMJgcM1o Kk8Ax2hedAatddZt2aTvchYr

PLATES (table in inches)					
JT TYPE	PLATES	W	LEN	Y	X
1 BMV 1-p	MT20	3.0	6.0		

NOTES: (1)
1) Lateral bracing to be a minimum of 2X4 SPF #2.

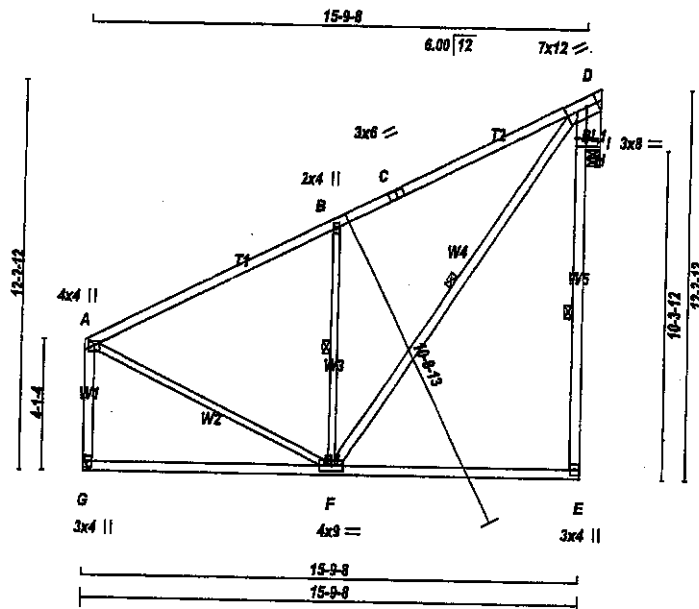


Structural component only
DWG# T-2132661 *3/12*

JOB NAME 421618	TRUSS NAME T20	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Rod Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Fri Sep 17 16:06:33 2021 Page 1
ID:v9nswDdnCQ_wfVdgPWAX86ymFhXK-qBp6oXCh1YUAmxDLIVZHMlyq52qv6NmVDec7vychYq



Scale = 1/64"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
G - A	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
BEARING BLOCKS					
BL1	2x8	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B	TMVW+w	MT20	2.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMVWK1-t	MT20	7.0	12.0	3.25	4.25
E	BMV+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	4.0	9.0		
G	BMV1+p	MT20	3.0	4.0		
H	KP-p	MT20	3.0	8.0	1.50	3.50

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
JT (D)	VERT	HORZ	DOWN	HORZ	UPLIFT IN-SX	IN-SX
	858	0	858	0	0	5-8
G	883	0	883	0	0	MECHANICAL
(** SEE "BEARING NOTE" **)						

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3'-8".

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
(D)	607	398 / 0	0 / 0	0 / 0	0 / 0	209 / 0	0 / 0
G	624	410 / 0	0 / 0	0 / 0	0 / 0	215 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) (D)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-H, B-F, D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	-598 / 0	-91.8 -91.8	0.78 (1)	5.83	F-B	-899 / 0	0.37 (1)
B-C	-667 / 0	-91.8 -91.8	0.78 (1)	5.54	F-D	0 / 973	0.18 (1)
C-D	-667 / 0	-91.8 -91.8	0.78 (1)	5.54	A-F	0 / 691	0.14 (1)
E-H	0 / 62	0.0 0.0	0.24 (1)	10.00	D-I	-868 / 0	0.06 (1)
H-D	0 / 62	0.0 0.0	0.24 (1)	10.00	H-I	0 / 129	0.00 (1)
G-A	-827 / 0	0.0 0.0	0.23 (1)	7.81			
G-F	0 / 0	-18.5 -18.5	0.33 (4)	10.00			
F-E	0 / 17	-18.5 -18.5	0.33 (4)	10.00			

TOTAL WEIGHT = 4 X 87 = 348 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 8.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2019, ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/980 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/980 (0.53")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.79/1.00 (B-D:1), BC=0.33/1.00 (E-F:4),
WB=0.37/1.00 (B-F:1), SS=0.32/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	850	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.84 (F) (INPUT = 0.90)

JSI METAL = 0.37 (B) (INPUT = 1.00)

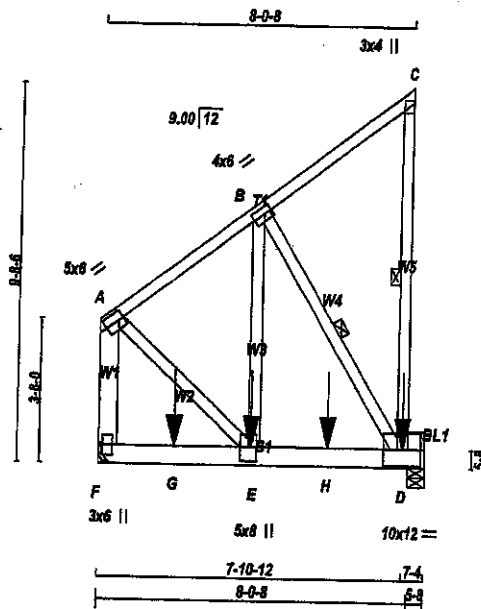


Structural component only
DWG# T-2132662

JOB NAME 421618	TRUSS NAME T21	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Rd Truss, Burlington

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Scale = 1:56.8

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
D - C	2x4	DRY	No.2	SPF		
F - A	2x6	DRY	No.2	SPF		
F - D	2x6	DRY	No.2	SPF		

BEARING BLOCKS	SIZE	DRY	LUMBER	DESCR.
BL1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	SIZE	DRY	LUMBER	DESCR.
	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.50	1.50
B	TMVW-t	MT20	4.0	6.0	2.00	2.25
C	TMV-p	MT20	3.0	4.0		
D	BMVWK1-t	MT20	10.0	12.0		
E	BMVW-t	MT20	5.0	8.0	4.25	2.50
F	BMV1-p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN
D	2625	0	2625
F	1726	0	1726

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3'-8".

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED
D	1853
F	1218

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-D, B-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO
A-B		-969 / 0	-91.8	-91.8	0.29 (1)	5.90	E-B	0 / 1435	0.25 (1)
B-C		-24 / 0	-91.8	-91.8	0.27 (1)	6.25	B-D	-1520 / 0	0.41 (1)
D-C		-142 / 0	0.0	0.0	0.08 (1)	6.25	A-E	0 / 1025	0.18 (1)
F-A		-1383 / 0	0.0	0.0	0.20 (1)	7.81			
F-G		0 / 0	-18.5	-18.5	0.46 (1)	10.00			
G-E		0 / 0	-18.5	-18.5	0.46 (1)	10.00			
E-H		0 / 797	-18.5	-18.5	0.53 (1)	10.00			
H-D		0 / 797	-18.5	-18.5	0.53 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	7-10-12	-617	-617		FRONT	VERT	TOTAL		C1
E	3-11-4	-610	-610		FRONT	VERT	TOTAL		C1
G	1-11-4	-610	-610		FRONT	VERT	TOTAL		C1
H	5-11-4	-610	-610		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 64 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.26")
CALCULATED VERT. DEFL. (LL) = L/999 (0.03")
ALLOWABLE DEFL. (TL) = L/360 (0.26")
CALCULATED VERT. DEFL. (TL) = L/999 (0.05")

CSI: TC=0.29/1.00 (A-B:1), BC=0.53/1.00 (D-E:1), WB=0.41/1.00 (B-D:1), SS=0.41/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

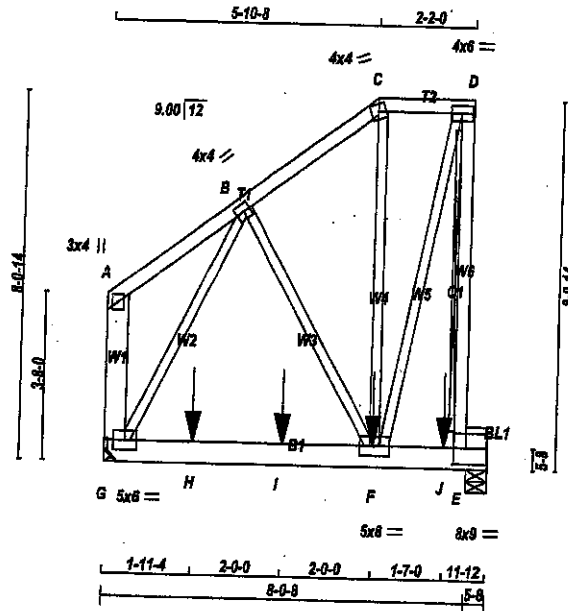
JSI GRIP = 0.85 (B) (INPUT = 0.90)
JSI METAL = 0.35 (E) (INPUT = 1.00)



Structural component only
DWG# T-2132663

JOB NAME 421618	TRUSS NAME T22	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:v9nswDdnCQ_wlVdgPWAX66ymHxX-nZxsDCEXZakuX45cT7X1MnoHXvj5N023NX5IBoychYo



Scale: 1/4"=1'

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2		
C - D	2x4	DRY	No.2		
E - D	2x4	DRY	No.2		
G - A	2x6	DRY	No.2		
G - E	2x6	DRY	No.2		

BEARING BLOCKS	BL1	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A - TMV+p	MT20	3.0	4.0				
B - TMVW-t	MT20	4.0	4.0	2.00	1.75		
C - TMV-m	MT20	4.0	4.0				
D - TMVW-t	MT20	4.0	4.0				
E - BMVK1-t	MT20	8.0	8.0	4.25	3.75		
F - BMVW-t	MT20	5.0	8.0				
G - BMVW1-t	MT20	5.0	6.0				

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	UPLIFT	IN-SX
E 1197	0	0	5-8
G 886	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT COMBINED							
E 842	574 / 0	0 / 0	0 / 0	0 / 0	288 / 0	0 / 0	0 / 0
G 824	421 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT D-E

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	UNBRAC LENGTH	FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
A-B	0 / 19	-91.8	-91.8	0.14 (1)	10.00	F-C	-10 / 27	0.01 (1)			
B-C	-340 / 0	-91.8	-91.8	0.14 (1)	6.25	F-D	0 / 1071	0.27 (1)			
C-D	-256 / 0	-91.8	-91.8	0.08 (1)	6.25	B-F	-30 / 18	0.02 (1)			
E-D	-1132 / 0	0.0	0.0	0.84 (1)	7.81	G-B	-568 / 0	0.38 (1)			
G-A	-105 / 0	0.0	0.0	0.02 (1)	7.81						
G-H	0 / 270	-18.5	-18.5	0.41 (1)	10.00						
H-I	0 / 270	-18.5	-18.5	0.41 (1)	10.00						
I-F	0 / 270	-18.5	-18.5	0.41 (1)	10.00						
F-J	-21 / 0	-18.5	-18.5	0.29 (1)	6.25						
J-E	-21 / 0	-18.5	-18.5	0.29 (1)	6.25						

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	5-11-4	-214	-214	---	BACK	VERT	TOTAL	---	C1
H	1-11-4	-195	-195	---	BACK	VERT	TOTAL	---	C1
I	3-11-4	-214	-214	---	BACK	VERT	TOTAL	---	C1
J	7-8-4	-219	-219	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 63 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.84/1.00 (D-E-1), BC=0.41/1.00 (F-G-1), WB=0.38/1.00 (B-G-1), SS=0.27/1.00 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

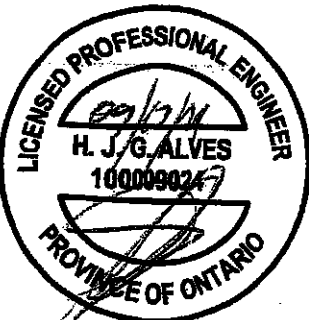
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.80)
JSI METAL= 0.24 (D) (INPUT = 1.00)



Structural component only
DWG# T-2132664

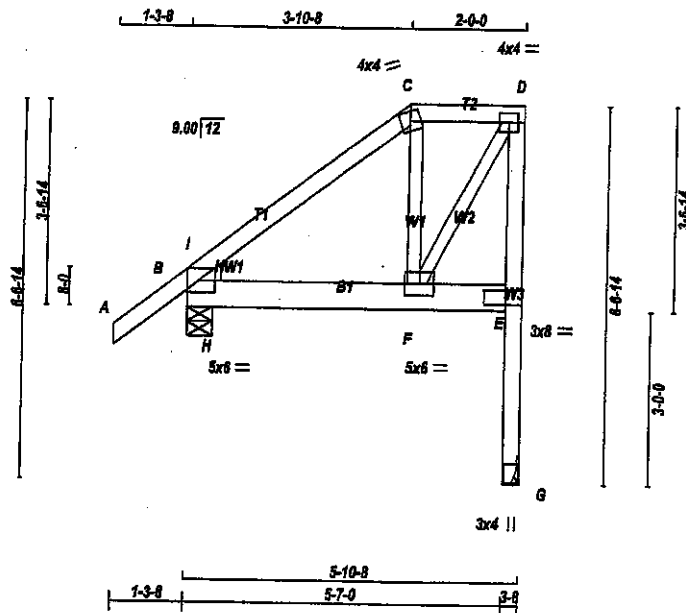
Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri Sep 17 16:06:36 2021 Page 1
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Structural component only
DWG# T-2132665

JOB NAME 421618	TRUSS NAME T24	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Rd Truss, Burlington					

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ID:v9nswDdnCQ_wVdgPWAX68ymHxX-FIVERYFZJTs9Efo1q3Gu_KdM1748YUcBbRG|EychYr



Scale = 1/32"

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
G - D	2x4	DRY	No.2
B - E	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBH1-I	MT20	5.0	8.0	Edge
B	TP-p	MT20	3.0	4.0	2.50
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BVMH	MT20	3.0	8.0	1.50 4.50
F	BMVW-t	MT20	5.0	8.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
G	324	0	324	0	0	MECHANICAL	
B	450	0	450	0	0	5-8	2x4 L

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-12.

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
G	229	150/0	0/0	0/0	0/0	79/0	0/0
B	316	221/0	0/0	0/0	0/0	95/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO				FR-TO			
A-B	0/31	-91.8	-91.8 0.12 (1)	F-C	-113/0	0.02 (1)	
B-I	-92/0	-91.8	-91.8 0.12 (1)	F-D	0/284	0.06 (1)	
I-C	-201/0	-91.8	-91.8 0.13 (1)	H-I	-312/0	0.00 (1)	
C-D	-145/0	-91.8	-91.8 0.06 (1)				
G-E	-324/0	0.0	0.0 0.07 (1)				
E-D	-335/0	0.0	0.0 0.07 (1)				
B-H	0/148	-18.5	-18.5 0.11 (1)				
H-F	0/148	-18.5	-18.5 0.11 (1)				
F-E	0/0	-18.5	-18.5 0.04 (1)				

TOTAL WEIGHT = 32 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OSC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.13/1.00 (C-H), BC=0.11/1.00 (F-H), WB=0.06/1.00 (D-F), SS=0.18/1.00 (B-H)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

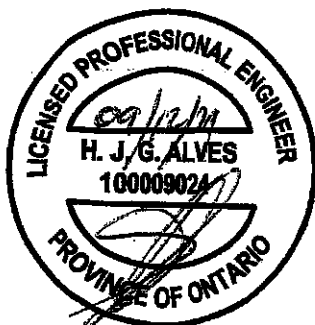
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION	(PSI)	(PLI)	(PLI)
MAX MIN MAX MIN MAX MIN			
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

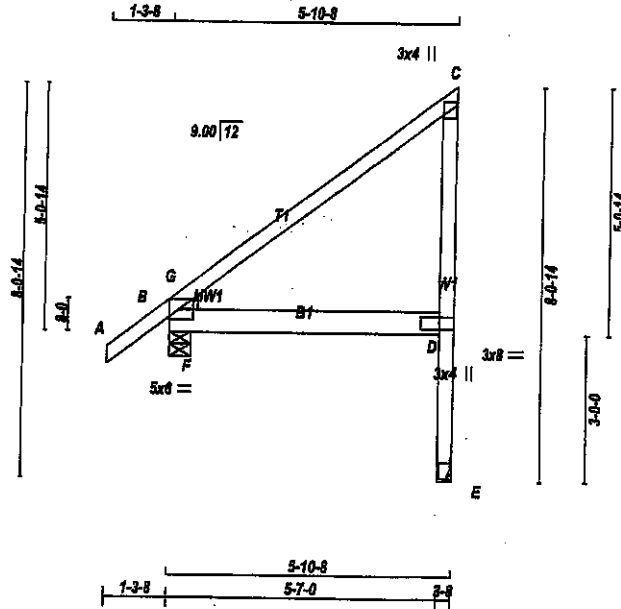
JSI GRIP = 0.31 (D) (INPUT = 0.90)
JSI METAL = 0.08 (D) (INPUT = 1.00)



Structural component only
DWG# T-2132666

JOB NAME 421618	TRUSS NAME T25	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Rod Truss, Burlington					

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ID:v9nswDdnCQ_wfVdgPWAX66ymHxX-ly2deuGB4n_cmNE?axaVRcUzIRJr77MqrapGhyhYm



Scale = 1/4" = 1'-0"

LUMBER	N. L. G. A.	RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
E - C	2x4	DRY	No.2	SPF		
B - D	2x8	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH-1	MT20	5.0	8.0	Edge		
B	TP-p	MT20	3.0	4.0	2.50	2.50	
C	TMV-p	MT20	3.0	4.0			
D	BVM-1	MT20	3.0	8.0	1.50	4.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG	HEEL WEDGE
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
E	324	0	324	0	0
B	450	0	450	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-12.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	229	150 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0
B	318	221 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO						FR-TO			
A-B	0 / 31	-91.8	-91.8	0.12 (1)	10.00	F-G	-795 / 0	0.00 (1)	10.00
B-G	0 / 387	-91.8	-91.8	0.36 (1)	10.00				
G-C	-24 / 0	-91.8	-91.8	0.32 (1)	8.25				
E-D	-324 / 0	0.0	0.0	0.07 (1)	7.81				
D-C	-209 / 0	0.0	0.0	0.10 (1)	7.81				
B-F	0 / 0	-18.5	-18.5	0.23 (1)	10.00				
F-D	0 / 0	-18.5	-18.5	0.23 (1)	10.00				

TOTAL WEIGHT = 2 X 30 = 59 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.03")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.08")

CSI: TC=0.36/1.00 (B-G:1), BC=0.23/1.00 (D-F:1), WB=0.00/1.00 (F-G:1), SS=0.48/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

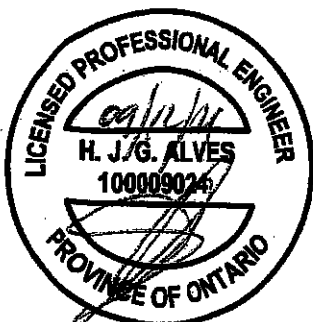
NAIL VALUES

PLATE GRIP (DRY) SHEAR	SECTION
(PSI)	(PLI)
MAX MIN MAX MIN	MAX MIN
MT20	650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

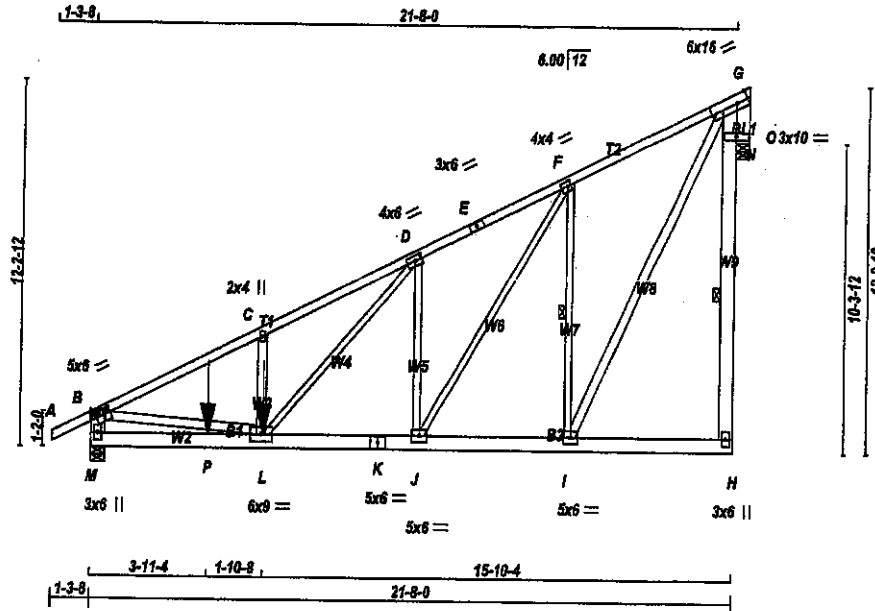
JSI GRIP = 0.21 (B) (INPUT = 0.90)
JSI METAL = 0.07 (C) (INPUT = 1.00)



Structural component only
DWG# T-2132667

JOB NAME 421618	TRUSS NAME T26	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1/4\"

TOTAL WEIGHT = 2 X 142 = 285 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x6	DRY	No.2
M - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
K - H	2x6	DRY	No.2

BEARING BLOCKS			
BL1	2x6	DRY	No.2
ALL WEBS 2x3 DRY No.2			
EXCEPT			
L - C	2x4	DRY	No.2
B - L	2x4	DRY	No.2
I - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-E	12	TOP
E-G	12	TOP
G-H	12	TOP
M-B	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
M-K	12	SIDE(183.1)
K-H	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	SIDE(282.4)
C-L	6	
2x4	6	
2x6	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
JT VERT	1792	0	0	5-8
O(G) HORIZ	0	1792	0	5-8
M	3296	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS						
JT COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
O(G)	1265	840/0	0/0	0/0	425/0	0/0	
M	2323	1572/0	0/0	0/0	750/0	0/0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O(G), M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-N, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	0/28	-91.8	-91.8 0.07 (1)	L-C	-560/0	0.05 (1)	
B-C	-4883/0	-91.8	-91.8 0.37 (1)	B-L	0/4242	0.38 (1)	
C-D	-4885/0	-91.8	-91.8 0.38 (1)	I-G	0/2291	0.20 (1)	
D-E	-2366/0	-91.8	-91.8 0.28 (1)	F-F	-1914/0	0.53 (1)	
E-F	-2366/0	-91.8	-91.8 0.28 (1)	J-F	0/2197	0.27 (1)	
F-G	-1169/0	-91.8	-91.8 0.23 (1)	L-D	-1907/0	0.67 (1)	
H-N	0/44	0.0	0.0 0.13 (1)	L-D	0/3235	0.40 (1)	
N-G	0/44	0.0	0.0 0.13 (1)	G-O	-1812/0	0.07 (1)	
M-B	-3006/0	0.0	0.0 0.11 (1)	N-O	0/269	0.00 (1)	
M-P	0/0	-18.5	-18.5 0.37 (1)				
P-L	0/0	-18.5	-18.5 0.37 (1)				
L-K	0/2125	-18.5	-18.5 0.23 (1)				
K-J	0/2125	-18.5	-18.5 0.23 (1)				
J-I	0/1025	-18.5	-18.5 0.10 (1)				
I-H	0/35	-18.5	-18.5 0.04 (4)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
L	5-9-12	-1203	-1203	-	BACK	VERT	TOTAL	-	C1
P	3-11-4	-610	-610	-	BACK	VERT	TOTAL	-	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 085-14

- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.72")

CALCULATED VERT. DEFL.(LL) = L/999 (0.09")

ALLOWABLE DEFL.(TL) = L/360 (0.72")

CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.37/1.00 (B-C:1), BC=0.37/1.00 (L-M:1),

WB=0.87/1.00 (D-J:1), SS=0.23/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.85 (B) (INPUT = 0.90)

JSI METAL = 0.55 (B) (INPUT = 1.00)



Structural component only
DWG# T-2132668

CONTINUED ON PAGE 2

JOB NAME 421618	TRUSS NAME T26	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Flat Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri Sep 17 16:06:35 2021 Page 2
ID:v9nswDdnCQ wIVdaPWAX66vmHxX-B8c?sEGar56TOXo88F5k PPw56ISalrV3VKNo7vchY

PLATES (size in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-WH	MT20	5.0	6.0	2.25	2.25
C	TMV-WH	MT20	2.0	4.0		
D	TMV-WH	MT20	4.0	6.0	2.00	2.50
E	TS-t	MT20	3.0	6.0		
F	TMV-WH	MT20	4.0	4.0	2.00	1.25
G	TMV-WH-t	MT20	6.0	16.0	Edge	5.00
H	BMV-p	MT20	3.0	6.0		
I	BMV-WH	MT20	5.0	6.0		
J	BMV-WH	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMV-WH-t	MT20	6.0	9.0		
M	BMV-t-p	MT20	3.0	6.0		
N	KP-p	MT20	3.0	10.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral bracing to be a minimum of 2X4 SPF #2.

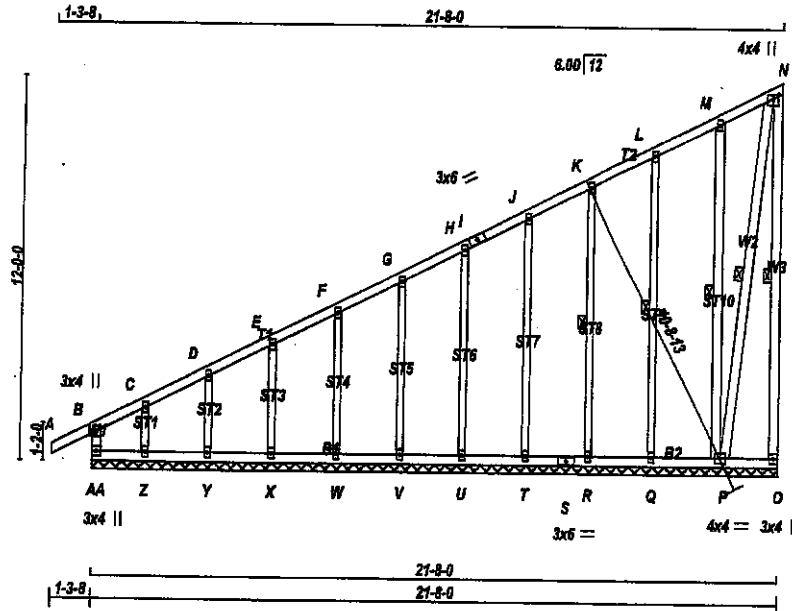


Structural component only
DWG# T-2132668 *W*

JOB NAME 421618	TRUSS NAME T27G	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Fri Sep 17 18:06:39 2021 Page 1
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Scale = 1/8" = 1'-0"

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
AA - B	2x4	DRY	No.2
A - I	2x4	DRY	No.2
I - N	2x4	DRY	No.2
O - N	2x4	DRY	No.2
AA - S	2x4	DRY	No.2
S - O	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
P - M	2x4	DRY	No.2
P - N	2x4	DRY	No.2
ALL GABLE WEBS EXCEPT	2x3	DRY	No.2
ST1	2x4	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2'-0" OC.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C, D, E, F, G, H, J, K, L, M				
C	TMW+w	MT20	2.0	4.0
I	TS-t	MT20	3.0	6.0
N	TMVW+p	MT20	4.0	4.0
O	BMV1+p	MT20	3.0	4.0
P	BMVW1+t	MT20	4.0	4.0
Q, R, T, U, V, W, X, Y, Z				
Q	BMV1+w	MT20	2.0	4.0
S	BS-t	MT20	3.0	6.0
AA	BMV1+p	MT20	3.0	4.0

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT BEARING JOINT O FOR 150 LBS. FACTORED UPLIFT BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S).

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF N-O, M-P, L-Q, K-R, N-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO	
AA-B	-246 / 0	0.0 0.0	0.02 (1)	P-M	-198 / 0
A-B	0 / 28	-91.8 -91.8	0.12 (1)	Q-L	-183 / 0
B-C	-22 / 0	-91.8 -91.8	0.11 (1)	R-K	-183 / 0
C-D	0 / 11	-91.8 -91.8	0.05 (1)	T-J	-183 / 0
D-E	0 / 8	-91.8 -91.8	0.05 (1)	U-H	-183 / 0
E-F	0 / 14	-91.8 -91.8	0.04 (1)	V-G	-182 / 0
F-G	0 / 16	-91.8 -91.8	0.04 (1)	W-F	-183 / 0
G-H	0 / 19	-91.8 -91.8	0.04 (1)	X-E	-178 / 0
H-I	0 / 21	-91.8 -91.8	0.04 (1)	Y-D	-199 / 0
I-J	0 / 22	-91.8 -91.8	0.05 (1)	Z-C	-100 / 0
J-K	0 / 24	-91.8 -91.8	0.05 (1)	P-N	-189 / 0
K-L	0 / 25	-91.8 -91.8	0.05 (1)		
L-M	0 / 26	-91.8 -91.8	0.05 (1)		
M-N	0 / 28	-91.8 -91.8	0.05 (1)		
O-N	0 / 102	0.0 0.0	0.02 (1)		
AA-Z	0 / 0	-18.5 -18.5	0.01 (4)		
Z-Y	-4 / 0	-18.5 -18.5	0.01 (4)		
Y-X	-9 / 0	-18.5 -18.5	0.01 (4)		
X-W	-12 / 0	-18.5 -18.5	0.01 (4)		
W-V	-15 / 0	-18.5 -18.5	0.01 (4)		
V-U	-17 / 0	-18.5 -18.5	0.01 (4)		
U-T	-18 / 0	-18.5 -18.5	0.01 (4)		
T-S	-20 / 0	-18.5 -18.5	0.01 (4)		
S-R	-20 / 0	-18.5 -18.5	0.01 (4)		
R-Q	-21 / 0	-18.5 -18.5	0.02 (4)		
Q-P	-23 / 0	-18.5 -18.5	0.02 (4)		
P-O	0 / 0	-18.5 -18.5	0.02 (4)		

TOTAL WEIGHT = 133 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (P-Q:4),
WB=0.22/1.00 (J-T:1), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

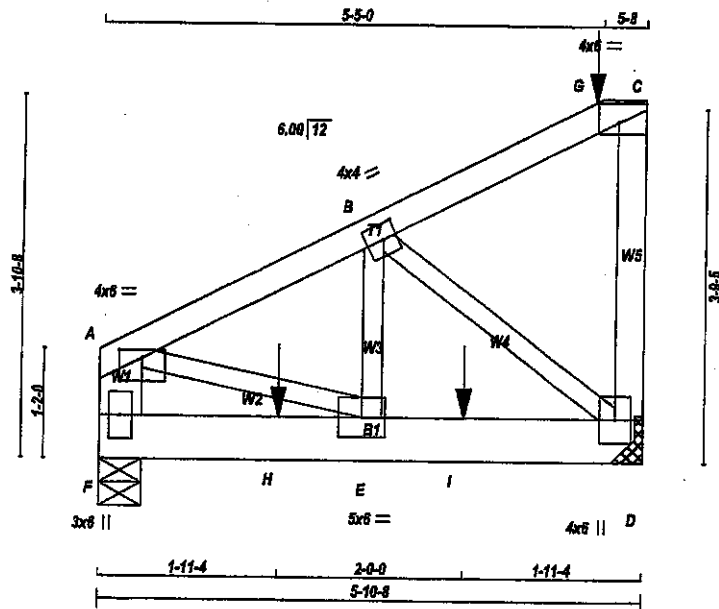
JSI GRIP= 0.47 (P) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)



Structural component only
DWG# T-2132669

JOB NAME 421618	TRUSS NAME T28	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Rd Truss, Burlington					

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ID:v9nswDdnCQ_wIVdgPWAX66ymHbX-7XkiGwI4NINBdrzaGg7C3qVKhWTF2JsoWppT7ychY



Scale = 1:23.8

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
D - C	2x4	DRY	No.2	SPF		
F - A	2x6	DRY	No.2	SPF		
F - D	2x6	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
A - C	1	12	SIDE (81.0)
C - D	1	12	SIDE (10.4)
F - A	2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
F - D	2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	8.0	1.00	8.00
B	TMVW-l	MT20	4.0	4.0	2.00	1.75
C	TMV-p	MT20	4.0	8.0	2.50	2.50
D	BMVW-l-p	MT20	4.0	8.0		
E	BMVW-l	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
D	1470	0	1470	0	5-8	5-8
F	1404	0	1404	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS		PERM. LIVE WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
D	1034	714 / 0	0 / 0	0 / 0	0 / 0	320 / 0	0 / 0
F	991	665 / 0	0 / 0	0 / 0	0 / 0	326 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS							
MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FACTORED FORCE (LBS)	MAX CSI (LC)				
FR-TO		FROM	TO		FR-TO						
A-B	-1430 / 0	-91.8	-91.8	0.08 (1)	8.25	A-E	0 / 1337	0.17 (1)			
B-G	0 / 27	-91.8	-91.8	0.11 (1)	8.25	E-B	0 / 1253	0.16 (1)			
G-C	0 / 27	-91.8	-91.8	0.11 (1)	8.25	B-D	-1627 / 0	0.19 (1)			
D-C	-332 / 0	0.0	0.0	0.04 (1)	7.81						
F-A	-1088 / 0	0.0	0.0	0.04 (1)	7.81						
F-H	0 / 0	-18.5	-18.5	0.21 (1)	10.00						
H-E	0 / 0	-18.5	-18.5	0.21 (1)	10.00						
E-I	0 / 1293	-18.5	-18.5	0.22 (1)	10.00						
I-D	0 / 1293	-18.5	-18.5	0.22 (1)	10.00						

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	5-5-0	-189	-189	---	BACK	VERT	TOTAL	---	C1
H	1-11-4	-859	-859	---	FRONT	VERT	TOTAL	---	C1
I	3-11-4	-518	-518	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TG=0.11/1.00 (B-C:1), BC=0.22/1.00 (D-E:1), WB=0.19/1.00 (B-D:1), SS=0.32/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.82 (B) (INPUT = 0.90)
JSI METAL = 0.29 (D) (INPUT = 1.00)



Structural component only
DWG# T-2132670

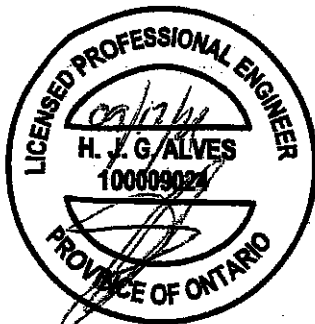
CONTINUED ON PAGE 2

JOB NAME 421618	TRUSS NAME T28	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Flat Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Fri Sep 17 18:06:40 2021 Page 2
 ID:v9nswDdnCQ wVdgPWAX66ymHxX-7XKdGw4NINBdrzaGa7C3dVKhwTF2JsoWppTY7ychY

PLATES (title in inches)
 JT TYPE PLATES W LEN Y X
 F BMV 1+1 MT20 3.0 6.0

NOTES- (i)
 1) Lateral bracing to be a minimum of 2X4 SPF #2.



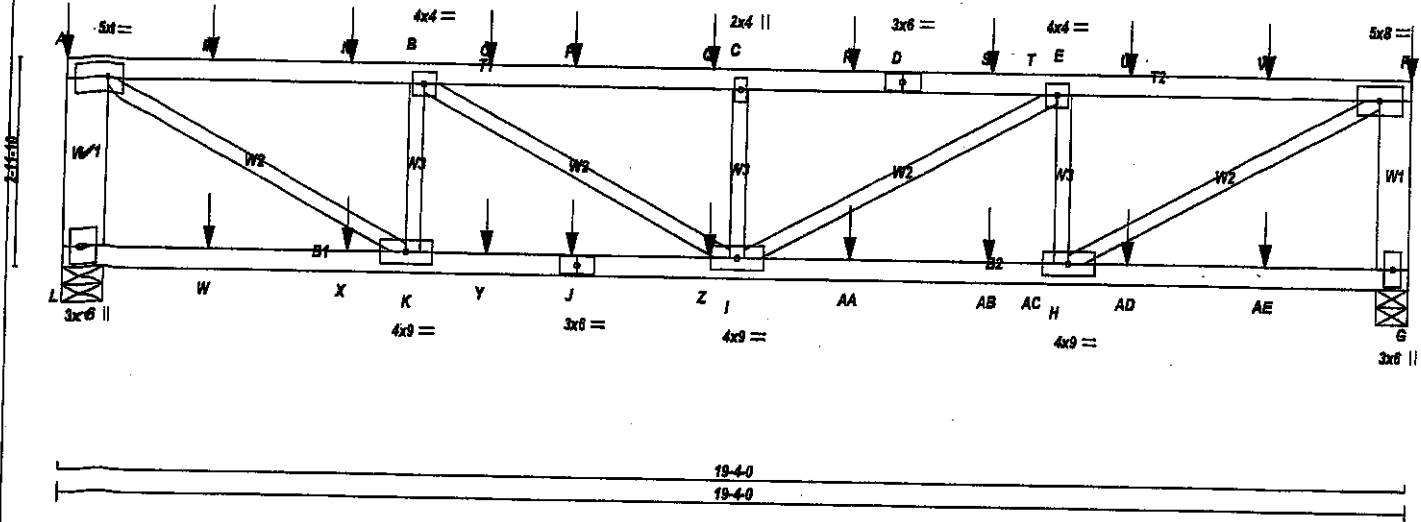
Structural component only
 DWG# T-2132670 *ml*

JOB NAME 421618	TRUSS NAME T29	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:yWjB8OQsuynxrGib5OKVayf0Ne-bj7UGJl80V2F?YmpNeRb21M5Jm0ng?xTY1PSychY

19-4-0

Scale = 1:31.4



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
L - A	2x6	DRY	No.2	SPF		
A - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	No.2	SPF		
G - F	2x6	DRY	No.2	SPF		
L - J	2x4	DRY	No.2	SPF		
J - G	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0		
B	TMVW-t	MT20	4.0	4.0		
C	TMVW-t	MT20	2.0	4.0		
D	TS-t	MT20	3.0	8.0		
E	TMVW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	8.0		
G	BMV1+p	MT20	3.0	6.0		
H	BMVW-t	MT20	4.0	9.0		
I	BMVW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	9.0		
L	BMV1+p	MT20	3.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORIZ	DOWN	HORIZ
JT	1341	0	0	5-8
L	1341	0	0	5-8
G	1551	0	0	5-8

UNFACTORED REACTIONS

	1ST LOASE	MAX/MIN COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	948	823 / 0	0 / 0	0 / 0	0 / 0	325 / 0	0 / 0
G	1097	721 / 0	0 / 0	0 / 0	0 / 0	376 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO						FR-TO			
L-A	-1296 / 0	0.0	0.0	0.13 (1)	7.81	H-F	0 / 2362	0.58 (1)	
A-M	-1919 / 0	-91.8	-91.8	0.49 (1)	4.28	A-K	0 / 2176	0.54 (1)	
M-N	-1919 / 0	-91.8	-91.8	0.49 (1)	4.28	H-E	-990 / 0	0.18 (1)	
N-B	-1919 / 0	-91.8	-91.8	0.49 (1)	4.28	K-B	-901 / 0	0.17 (1)	
B-O	-2448 / 0	-91.8	-91.8	0.53 (1)	3.78	I-E	0 / 421	0.10 (1)	
O-P	-2448 / 0	-91.8	-91.8	0.53 (1)	3.78	B-I	0 / 610	0.15 (1)	
P-Q	-2448 / 0	-91.8	-91.8	0.53 (1)	3.78	I-C	-421 / 0	0.08 (1)	
Q-C	-2448 / 0	-91.8	-91.8	0.53 (1)	3.78				
C-R	-2448 / 0	-91.8	-91.8	0.53 (1)	3.78				
R-D	-2448 / 0	-91.8	-91.8	0.53 (1)	3.78				
D-S	-2448 / 0	-91.8	-91.8	0.53 (1)	3.78				
S-T	-2448 / 0	-91.8	-91.8	0.53 (1)	3.78				
T-E	-2448 / 0	-91.8	-91.8	0.53 (1)	3.78				
E-U	-2083 / 0	-137.7	-137.7	0.82 (1)	3.89				
U-V	-2083 / 0	-137.7	-137.7	0.82 (1)	3.89				
V-F	-2083 / 0	-137.7	-137.7	0.82 (1)	3.89				
F-G	-1485 / 0	0.0	0.0	0.15 (1)	7.81				

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO						FR-TO			
L-W	0 / 0	-18.5	-18.5	0.14 (4)	10.00				
W-X	0 / 0	-18.5	-18.5	0.14 (4)	10.00				
X-K	0 / 0	-18.5	-18.5	0.14 (4)	10.00				
K-Y	0 / 1919	-18.5	-18.5	0.40 (1)	10.00				
Y-J	0 / 1919	-18.5	-18.5	0.40 (1)	10.00				
J-Z	0 / 1919	-18.5	-18.5	0.40 (1)	10.00				
Z-I	0 / 1919	-18.5	-18.5	0.40 (1)	10.00				
I-AA	0 / 2083	-18.5	-18.5	0.44 (1)	10.00				
AA-AB	0 / 2083	-18.5	-18.5	0.44 (1)	10.00				
AB-AC	0 / 2083	-18.5	-18.5	0.44 (1)	10.00				
AC-H	0 / 2083	-27.7	-27.7	0.44 (1)	10.00				
H-AD	0 / 0	-27.7	-27.7	0.18 (4)	10.00				
AD-AE	0 / 0	-27.7	-27.7	0.18 (4)	10.00				
AE-G	0 / 0	-27.7	-27.7	0.18 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
A	0-0	-48	-48	-	BACK	VERT	TOTAL	-	C1
F	19-4-0	-48	-48	-	BACK	VERT	TOTAL	-	C1
J	7-3-4	-7	-7	-	BACK	VERT	TOTAL	-	C1
M	2-0-12	-19	-19	-	BACK	VERT	TOTAL	-	C1
N	4-0-12	-19	-19	-	BACK	VERT	TOTAL	-	C1
O	6-0-12	-19	-19	-	BACK	VERT	TOTAL	-	C1
P	7-3-4	-19	-19	-	BACK	VERT	TOTAL	-	C1

TOTAL WEIGHT = 75 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.84/1.00 (C-E:1), BC=0.44/1.00 (H-t:1), WB=0.58/1.00 (F-t:1), SS=0.38/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLJ) (PLJ)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (K) (INPUT = 0.90)
JSI METAL = 0.67 (J) (INPUT = 1.00)



Structural component only
DWG# T-2132671

CONTINUED ON PAGE 2

JOB NAME 421618	TRUSS NAME T29	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Row Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Fri Sep 17 16:56:41 2021 Page 2
 ID: vWhB8OQsuvnXrGib5OKVavf0Ne-bj7UGJ80V2F?YmpNeRb21M5Jm0ng?xITY1PSychY

SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	8-3-4	-19	-19	—	BACK	VERT	TOTAL	—	C1
R	11-3-4	-19	-19	—	BACK	VERT	TOTAL	—	C1
S	13-3-4	-19	-19	—	BACK	VERT	TOTAL	—	C1
U	15-3-4	-19	-19	—	BACK	VERT	TOTAL	—	C1
V	17-3-4	-19	-19	—	BACK	VERT	TOTAL	—	C1
W	2-0-12	-7	-7	—	BACK	VERT	TOTAL	—	C1
X	4-0-12	-7	-7	—	BACK	VERT	TOTAL	—	C1
Y	6-0-12	-7	-7	—	BACK	VERT	TOTAL	—	C1
Z	9-3-4	-7	-7	—	BACK	VERT	TOTAL	—	C1
AA	11-3-4	-7	-7	—	BACK	VERT	TOTAL	—	C1
AB	13-3-4	-7	-7	—	BACK	VERT	TOTAL	—	C1
AD	15-3-4	-7	-7	—	BACK	VERT	TOTAL	—	C1
AE	17-3-4	-7	-7	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

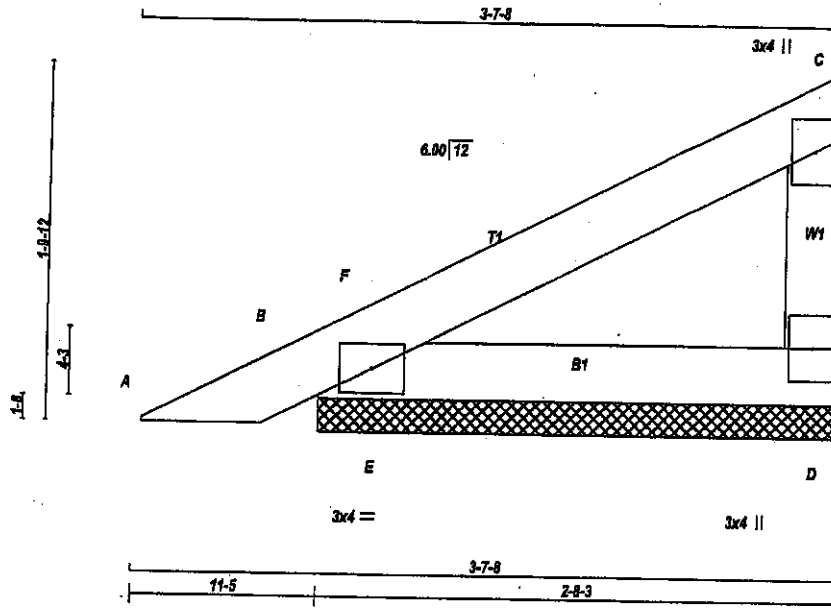
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
 DWG# T-2132671

JOB NAME 4216-18	TRUSS NAME PB1	QUANTITY 6	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER	N. L. G. A.	RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2				SPF
D - G	2x4	DRY	No.2				SPF
B - D	2x4	DRY	No.2				SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0			
C	TMV+p	MT20	3.0	4.0			
D	BMV1+p	MT20	3.0	4.0			

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
D	148	0	148	0
B	225	0	225	0

UNFACTORED REACTIONS	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
D	105	69 / 0
B	158	112 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)	MAX. FACTORED UNBRAC LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)
FR-TO	A-B	0 / 16	-91.8	-91.8	0.05 (5)	10.00	E-F	-117 / 2	0.00 (1)
B-F	-11 / 0	-91.8	-91.8	0.02 (4)	6.25				
F-C	0 / 2	-91.8	-91.8	0.08 (1)	10.00				
D-C	-105 / 0	0.0	0.0	0.01 (1)	7.81				
B-E	0 / 0	-18.5	-18.5	0.07 (1)	10.00				
E-D	0 / 0	-18.5	-18.5	0.07 (1)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 8 X 9 = 53 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.08/1.00 (C-F:1), BC=0.07/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.10/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)



Structural component only
DWG# T-2132636

Tamarack Rod Truss, Burlington

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ID:v9nswDdnCQ_wlVdgPWAX866ymHxX-y82AOLyWz5r9GYqBdnedwXLRcdwgUhUg4S1?udyChZ9

Scale = 1:22.6



TOTAL WEIGHT = 41 lb

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

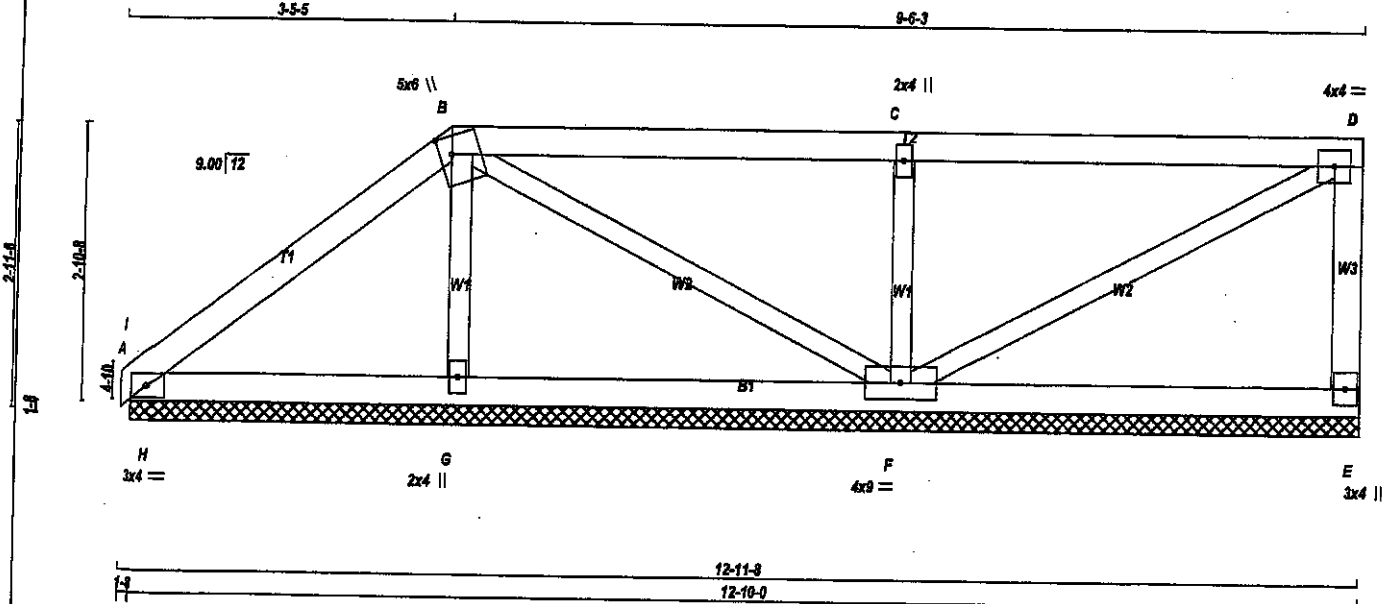


Structural component only
DWG# T-2132637.

JOB NAME 421618	TRUSS NAME PB3	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Rod Truss, Burlington					

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Scale = 1:22.5



TOTAL WEIGHT = 45 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - B	No.2	2x4	DRY	No.2	SPF	
B - D	No.2	2x4	DRY	No.2	SPF	
E - D	No.2	2x4	DRY	No.2	SPF	
A - E	No.2	2x4	DRY	No.2	SPF	
ALL WEBS	No.2	2x4	DRY	No.2	SPF	
DRY: SEASONED LUMBER.						

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMB1-I	MT20	3.0	4.0		
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMVW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMVWW1-l	MT20	4.0	9.0		
G	BMV1+w	MT20	2.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	202	0	202	0	12-10-0	12-10-0
A	225	0	225	0	12-10-0	12-10-0
G	314	0	314	0	12-10-0	12-10-0
F	674	0	674	0	12-10-0	12-10-0

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	143	93 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0
A	158	111 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
G	225	134 / 0	0 / 0	0 / 0	0 / 0	91 / 0	0 / 0
F	475	319 / 0	0 / 0	0 / 0	0 / 0	156 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, A, G, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED				WEBS			
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A-I	-21 / 0	-91.8	-91.8	0.05 (1)	8.25	G-B	-204 / 0	0.04 (1)	
I-B	-93 / 0	-91.8	-91.8	0.12 (1)	8.25	F-D	0 / 0	0.00 (1)	
B-C	0 / 1	-91.8	-91.8	0.35 (1)	10.00	B-F	-70 / 0	0.03 (1)	
C-D	0 / 0	-91.8	-91.8	0.35 (1)	10.00	F-C	-545 / 0	0.09 (1)	
E-D	-168 / 0	0.0	0.0	0.02 (1)	7.81	H-I	-280 / 0	0.00 (1)	
A-H	0 / 69	-18.5	-18.5	0.11 (1)	10.00				
H-G	0 / 69	-18.5	-18.5	0.11 (1)	10.00				
G-F	0 / 61	-18.5	-18.5	0.11 (4)	10.00				
F-E	0 / 0	-18.5	-18.5	0.10 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.35/1.00 (B-C:1), BC=0.11/1.00 (A-H:1), WB=0.09/1.00 (C-F:1), SS=0.22/1.00 (A-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.27 (C) (INPUT = 0.90)
JSI METAL= 0.11 (C) (INPUT = 1.00)

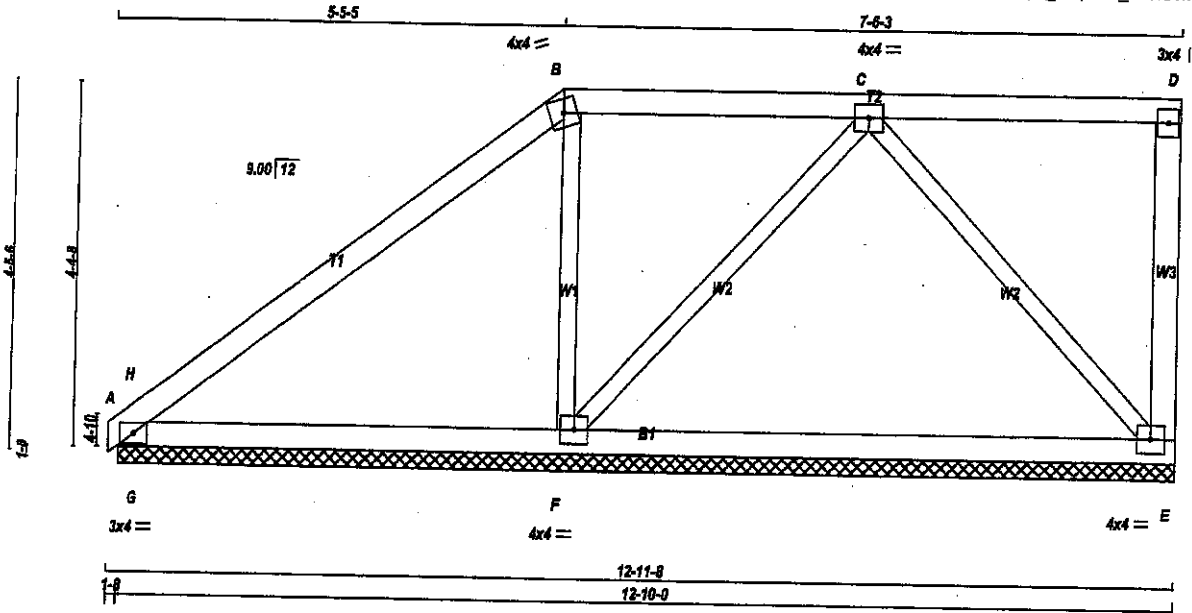


Structural component only
DWG# T-2132638

JOB NAME 421618	TRUSS NAME PB4	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:v9nswDdnCQ_wlVdgPWAX66ymHxX-uX9wp1_mvj5Wf_akCf6?M0ryRaJyZfzXmW5zWychZ7

Scale = 1:25.5



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF		
B - D	2x4	DRY	No.2	SPF		
E - D	2x4	DRY	No.2	SPF		
A - E	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMB1-1	MT20	3.0	4.0			
B	TTW-m	MT20	4.0	4.0			
C	TMWW-t	MT20	4.0	4.0			
D	TMV-p	MT20	3.0	4.0			
E	BMVW1-1	MT20	4.0	4.0			
F	BMVW1-1	MT20	4.0	4.0			

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	441	0	441	0	0	12-10-0	12-10-0
A	344	0	344	0	0	12-10-0	12-10-0
F	830	0	830	0	0	12-10-0	12-10-0

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	310	214/0	0/0	0/0	0/0	98/0	0/0
A	240	172/0	0/0	0/0	0/0	68/0	0/0
F	450	270/0	0/0	0/0	0/0	180/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, A, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
MEMB.	FR-TO	FR-TO	FR-TO	FR-TO	MEMB.	FR-TO	FR-TO
A-H	0/189	-91.8	-91.8 0.20 (1)	10.00	F-B	-261/0	0.07 (1)
H-B	-138/0	-91.8	-91.8 0.32 (1)	6.25	F-C	-226/0	0.11 (1)
B-C	-91/0	-91.8	-91.8 0.22 (1)	6.25	C-E	-354/0	0.18 (1)
C-D	0/0	-91.8	-91.8 0.21 (1)	10.00	G-H	-718/0	0.00 (1)
E-D	-133/0	0.0	0.0 0.04 (1)	7.81			
A-G	0/98	-18.5	-18.5 0.27 (1)	10.00			
G-F	0/98	-18.5	-18.5 0.27 (1)	10.00			
F-E	0/241	-18.5	-18.5 0.26 (4)	10.00			

TOTAL WEIGHT = 47 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (B-H:1), BC=0.27/1.00 (A-G:1), WB=0.18/1.00 (C-E:1), SS=0.55/1.00 (A-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

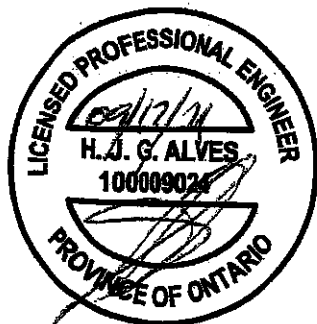
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

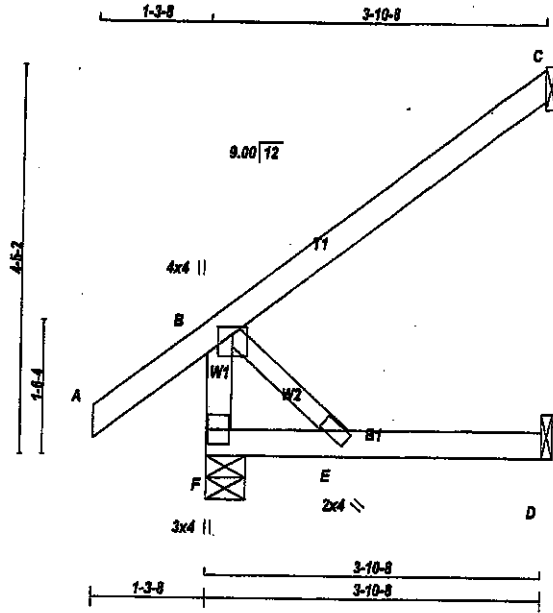
JSI GRIP= 0.28 (A) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)



Structural component only
DWG# T-2132639

JOB NAME 421618	TRUSS NAME J1	QUANTITY 8	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Rod Truss, Burlington					

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ID: v9nswDdnCQ_wVdgPWAX66ymHxX-bAEHLauN9ZDsAmyEqE1SDtdf2cEhpRlxwAJEDQychZE



Scale = 1:25.2

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
E	BMV+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
F	340	0	340	0	0	5-8	5-8	
C	178	0	178	0	0	1-8	1-8	
D	36	0	40	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
F	238	170 / 0	0 / 0	0 / 0	68 / 0	0 / 0	0 / 0
C	122	99 / 0	0 / 0	0 / 0	23 / 0	0 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (LBS)	LC1 MAX (PLF)	LC1 MAX (LC)	MEMB. FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FORCE (LBS)	MAX. CSI (LC)
F-B	-304 / 0	0.0	0.0	0.03 (1)	7.81	10.00	0 / 0	0.00 (1)
A-B	0 / 38	-91.8	-91.8	0.14 (5)	10.00			
B-C	0 / 0	-91.8	-91.8	0.23 (1)	10.00			
F-E	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
E-D	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 8 X 15 = 117 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.23/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

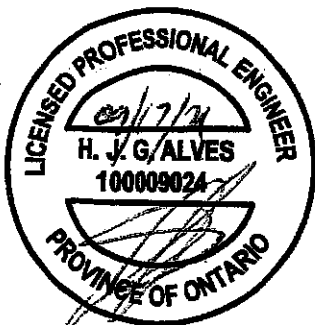
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

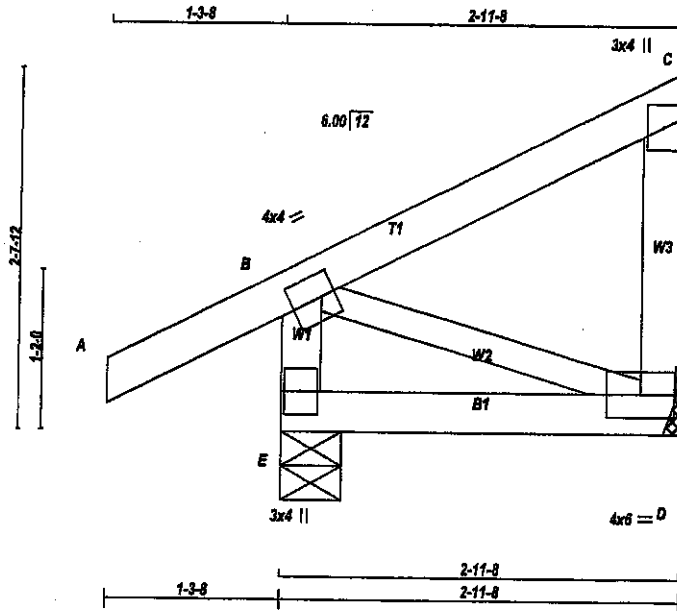
JSI GRIP = 0.23 (5) (INPUT = 0.90)
JSI METAL = 0.06 (5) (INPUT = 1.00)



Structural component only
DWG# T-2132631

JOB NAME 421618	TRUSS NAME J2	QUANTITY 10	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Field Truss, Burlington					

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ID:v9nswDdnCQ_wIVdgPWAX66ymHxX-3NotZ_v?wtLjowWQOyYim5ArK0bSYuy59q3nlsychZD



Scale = 1:16.2

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
ALL WEBS 2x3 DRY No.2 SPF					
DRY: SEASONED LUMBER.					

PLATES (Table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-1	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	288	0	288	0	5-8	5-8
D	183	0	183	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
E	201	145/0	0/0	0/0	0/0	58/0	0/0
D	115	78/0	0/0	0/0	0/0	40/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO			FR-TO			
E-B	-260/0	0.0	0.0	0.03 (1)	7.81	B-D	0/0	0.00 (1)	
A-B	0/28	-91.8	-91.8	0.13 (5)	10.00				
B-C	0/0	-91.8	-91.8	0.14 (1)	10.00				
D-C	-136/0	0.0	0.0	0.02 (1)	7.81				
E-D	0/0	-18.5	-18.5	0.05 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 10 X 14 = 141 lb

DESIGN CRITERIA

SPECIFIED LOADS:		
TOP CH.	LL	= 25.8 PSF
	DL	= 8.0 PSF
BOT CH.	LL	= 0.0 PSF
	DL	= 7.4 PSF
TOTAL LOAD = 39.0 PSF		

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD.

ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (B-C:1), BC=0.05/1.00 (D-E:4), WB=0.00/1.00 (B-D:1), SS=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(FS)	(PU)	(PL)
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

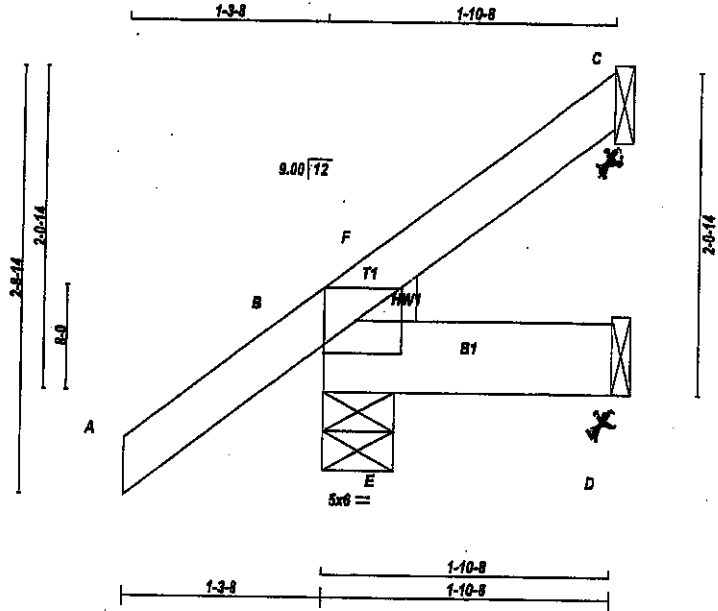
JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



Structural component only
DWG# T-2132632

JOB NAME 421618	TRUSS NAME J3	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1/4" = 1'-0"

TOTAL WEIGHT = 9 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
B - D	2x6	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMBH1-1	MT20	5.0	6.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
C	58	0	58	0	1-8	1-8	
B	276	0	276	0	5-8	5-8	2x4 L
D	-1	0	15	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	40	32 / -1	0 / 0	0 / 0	0 / 0	8 / 0	0 / 0
B	182	144 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0
D	1	0 / -25	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	FR-TO	CHORDS		WEBS	
		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)
A-B	0/31	-91.8	-91.8	0.12 (1)	10.00
B-F	-149/0	-91.8	-91.8	0.12 (5)	6.25
F-C	-4/0	-91.8	-91.8	0.03 (1)	10.00
B-E	0/0	-18.5	-18.5	0.04 (5)	10.00
E-D	0/0	-18.5	-18.5	0.04 (5)	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/IC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5), WB=0.00/1.00 (E-F:1), SS=0.09/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

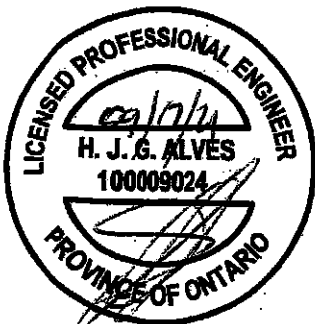
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

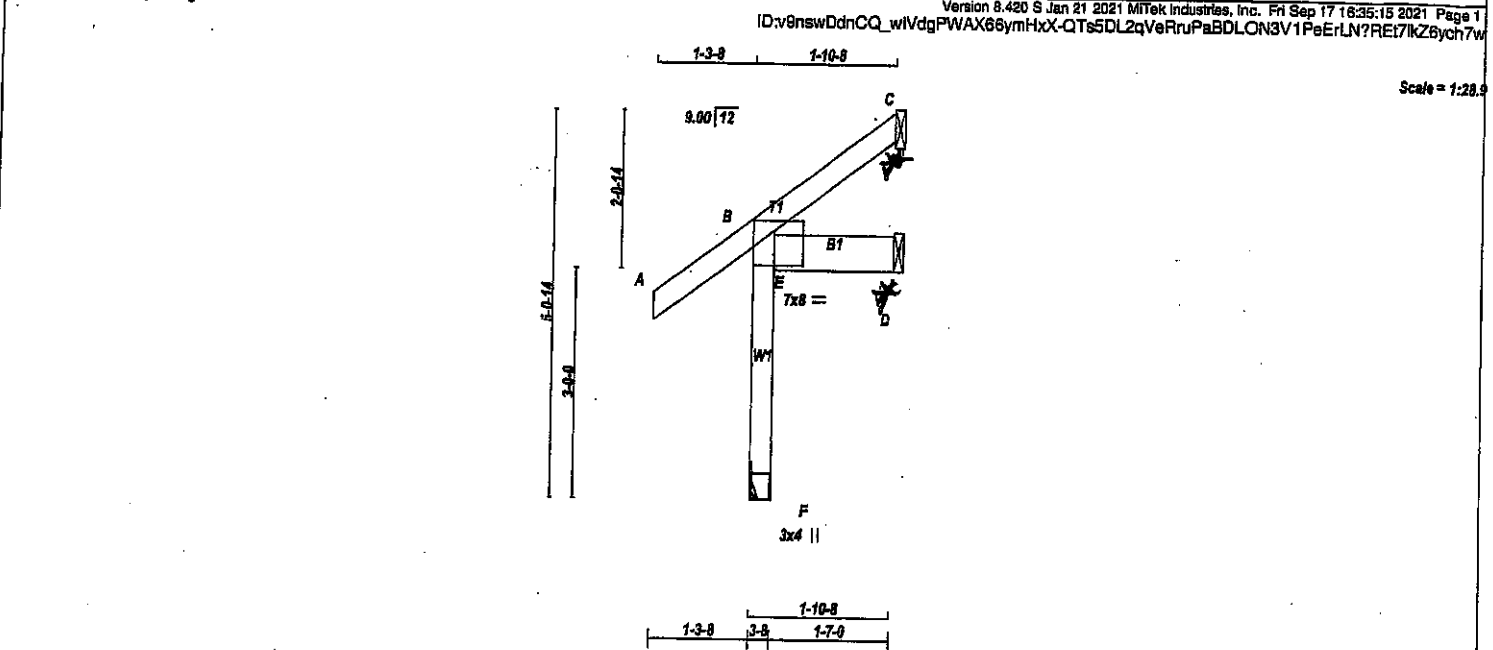
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.10 (B) (INPUT = 0.90)
JSI METAL= 0.02 (B) (INPUT = 1.00)



Structural component only
DWG# T-2132633

JOB NAME 421618	TRUSS NAME J4	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Rd Truss, Burlington		Version 8.420 S Jan 21 2021 Mittek Industries, Inc. Fri Sep 17 16:35:15 2021 Page 1			ID:v9nswDdnCQ_wlVdgPWAX66ymHxX-QTs5DL2qVeRruPaBDLON3V1PeErLN?REI7kZ6ych7w



Scale = 1:28.9

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
F - B	2x4	DRY	No.2	SPF		
E - D	2x6	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B						
E	TMBV1-1	MT20	7.0	8.0	4.75	4.50
E	TP+p	MT20	3.0	4.0	3.25	0.25

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UP
C	57	0	57	0
D	0	16	0	-17
F	276	0	276	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LOASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
C	39 31/-8 0/0 0/0 0/0 8/0 0/0
D	0 0/-17 0/0 0/0 0/0 11/0 0/0
F	192 144/0 0/0 0/0 0/0 48/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED UNBRACED LENGTH (FR-TO)
FR-TO				
A-B	0/38	-91.8	-91.8 0.12 (1)	10.00
B-C	-18/0	-91.8	-91.8 0.08 (1)	6.25
F-E	-276/0	0.0	0.0 0.08 (1)	7.81
E-B	-242/0	0.0	0.0 0.07 (5)	7.81
E-D	0/0	-18.5	-18.5 0.04 (5)	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:				
TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2015, ASC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), SC=0.04/1.00 (D-E:5), WS=0.00/1.00 (W-A:0), SSI=0.08/1.00 (A-B:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

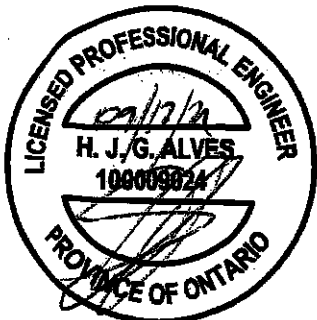
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL = 0.250 Inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.05 (E) (INPUT = 0.90)
JSI METAL= 0.04 (E) (INPUT = 1.00)



Structural component only
DWG# T-2132634



DRY; SEASONED LUMBER.

NOTES- (1)
1) Lateral braced to be a minimum of 2X4 SPF #2.

TOTAL WEIGHT = $11 \times 11 = 121$ lb

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP CH.	LL	= 25.6	PSF
	DL	= 8.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.19/1.00 (B-C:1), BC=0.05/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP (DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	850	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

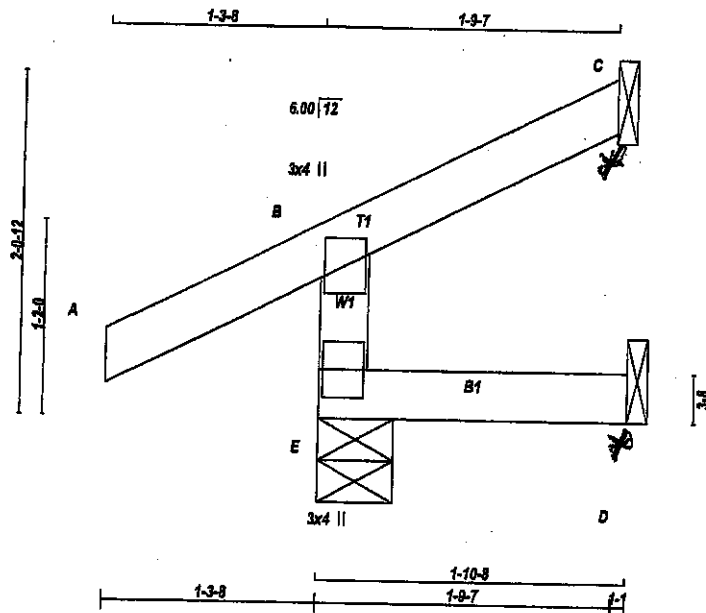
JSI GRIP= 0.13 (E) (INPUT = 0.80)
JSI METAL= 0.10 (B) (INPUT = 1.00)



JOB NAME 421618	TRUSS NAME C1	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID: v9nswDdnCQ_wlVdgPWAX66ymHxX-8bZ8jcsVseqlJJDf96UibF?AcPDhc4yVE05ZcsychZH



Scale = 1:13.3

LUMBER	N. L. G. A. RULES
CHORDS	SIZE
E - B	2x4 DRY No.2
A - C	2x4 DRY No.2
E - D	2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)	W	LEN	Y	X
JT TYPE	PLATES			
B TMV+p	MT20	3.0	4.0	
E BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	271	0	271	0	5-8	5-8
C	45	0	45	0	1-8	1-8
D	8	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX. MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	189	141 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	31	24 / -18	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	7	0 / -8	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED			MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED		
		VERT. LOAD (KIP)	LC1 (KIP)	MAX (KIP)			FORCE (LBS)	MAX (KIP)	
FR-TO		FROM TO	CS1 (KIP)	LC1 (KIP)		FR-TO		CS1 (KIP)	
E-B	-244 / 0	0.0	0.0	0.04 (5)	7.81				
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00				
B-C	-17 / 0	-91.8	-91.8	0.09 (1)	6.25				
E-D	0 / 0	-18.5	-18.5	0.04 (5)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 7 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/998 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/998 (0.00")

CSL TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5),
WB=0.00/1.00 (n/a:0), SSL=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

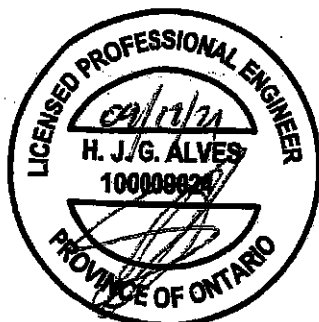
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

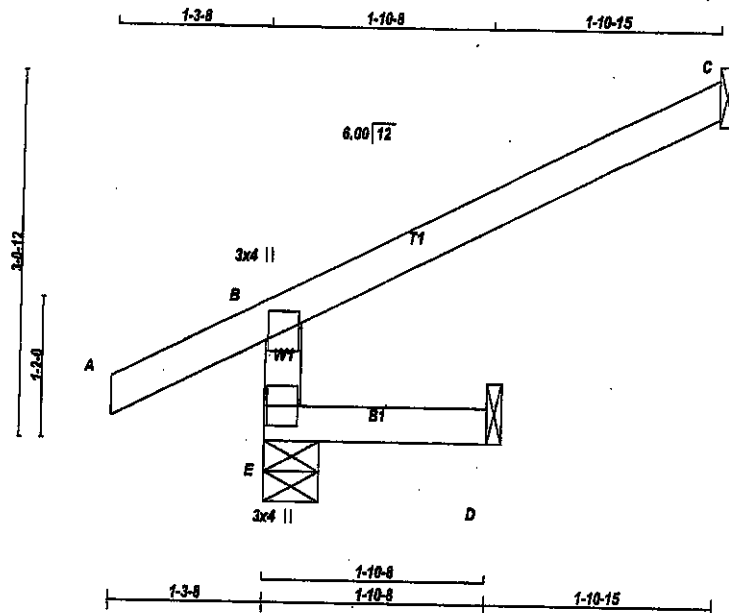
JSI GRIP = 0.10 (E) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2132627

JOB NAME 421618	TRUSS NAME C2	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Rd Truss, Burlington		TRUSS DESC.			

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Fri Sep 17 16:06:05 2021 Page 1
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Scale = 1:18.4

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF		
A - C	2x4	DRY	No.2	SPF		
E - D	2x4	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	381	0	361	0	5-8	5-8
C	130	0	130	0	1-8	1-8
D	16	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	250	180/0	0/0	0/0	0/0	80/0	0/0
C	90	73/0	0/0	0/0	0/0	17/0	0/0
D	12	0/0	0/0	0/0	0/0	12/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)	
FR-TO		FROM TO			FR-TO			
E-B	-342 / 0	0.0 0.0	0.01 (4)	7.81				
A-B	0 / 28	-91.8 -91.8	0.13 (5)	10.00				
B-C	-19 / 0	-91.8 -91.8	0.22 (1)	6.25				
E-D	0 / 0	-18.5 -18.5	0.02 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 10 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	38.0	PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CS1: TC=0.22/1.00 (B-C:1), SC=0.02/1.00 (D-E:4), WB=0.00/1.00 (N/A:0), SS=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

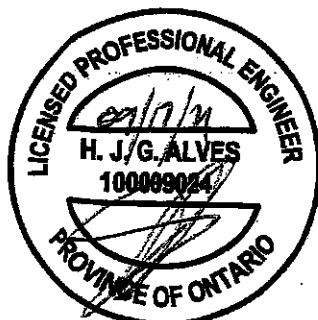
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
		1987 1873

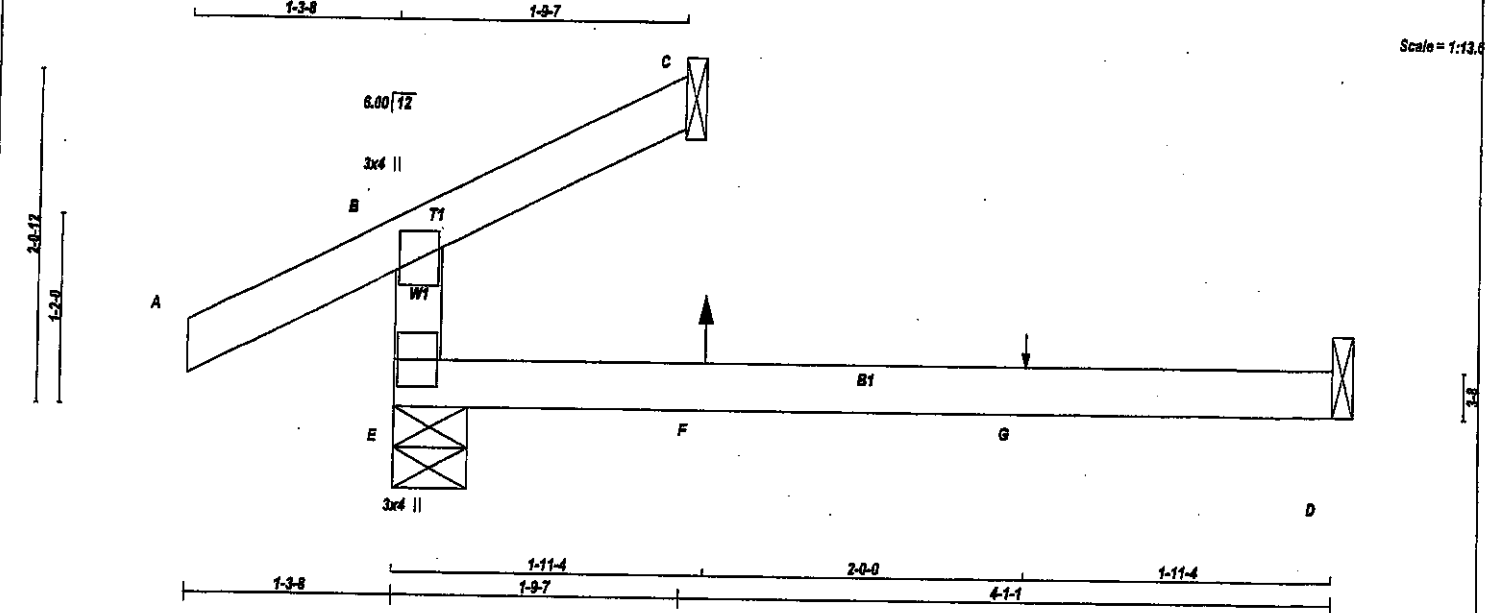
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.80)
JSI METAL= 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2132628



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

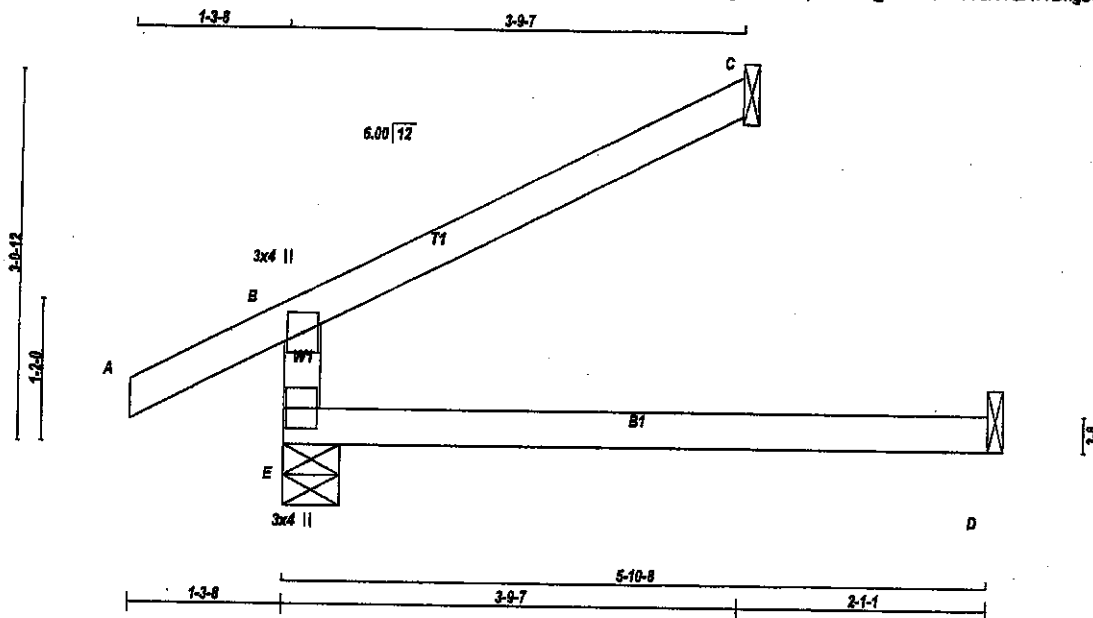
PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
NOTES: (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
E	284	0	284	0	0	5-8	5-8		
C	63	0	63	0	0	1-8	1-8		
D	44	0	52	0	0	1-8	1-8		
SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D									
UNFACTORED REACTIONS									
1ST CASE		MAX. MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
E	200	137 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0		
C	46	21 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0		
D	35	0 / 3	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0		
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (7)									
CHORDS				WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)		
FR-TO		FROM TO			FR-TO				
E-B	-227 / 0	0.0	0.0 0.11 (4)	7.81					
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00					
B-C	-9 / 9	-91.8	-91.8 0.08 (4)	10.00					
E-F	0 / 0	-18.5	-18.5 0.14 (4)	10.00					
F-G	0 / 0	-18.5	-18.5 0.14 (4)	10.00					
G-D	0 / 0	-18.5	-18.5 0.14 (4)	10.00					
SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	5	1	8	BACK	VERT	TOTAL	—	C1
G	3-11-4	1	1	—	BACK	VERT	TOTAL	—	C1
CONNECTION REQUIREMENTS									
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.									

TOTAL WEIGHT = 12 lb	
DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF	
SPACING = 24.0 IN. C/C	
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015	
THIS DESIGN COMPLIES WITH:	
- PART 9 OF CBC 2015, ABC 2015	
- PART 9 OF CBC 2012 (2019 AMENDMENT)	
- CSA 086-14	
- TPIC 2014	
DESIGN ASSUMPTIONS	
- OVERHANG NOT TO BE ALTERED OR CUT OFF.	
(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD	
ALLOWABLE DEFL.(LL) = L/360 (0.20")	
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")	
ALLOWABLE DEFL.(TL) = L/360 (0.20")	
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")	
CSI: TC=0.12/1.00 (A-B:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS=0.09/1.00 (A-B:1)	
DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10	
COMPANION LIVE LOAD FACTOR = 1.00	
AUTOSOLVE RIGHT HEEL ONLY	
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.	
NAIL VALUES	
PLATE GRIP(DRY)	SHEAR SECTION
(PSI)	(PLI)
MAX MIN	MAX MIN
MT20	650 371 1747 788 1987 1873
PLATE PLACEMENT TOL = 0.250 inches	
PLATE ROTATION TOL = 5.0 Deg.	
JSI GRIP= 0.09 (E) (INPUT = 0.90)	
JSI METAL= 0.06 (B) (INPUT = 1.00)	



Scale = 1/16.2



TOTAL WEIGHT = 14 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
E - B	2x4	No.2	DRY	No.2	SPF	SPF
A - C	2x4	No.2	DRY	No.2	SPF	SPF
E - D	2x4	No.2	DRY	No.2	SPF	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	406	0	405	0	5-8	5-8
C	130	0	130	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	285	190/0	0/0	0/0	0/0	98/0	0/0
C	90	73/0	0/0	0/0	0/0	17/0	0/0
D	36	0/0	0/0	0/0	0/0	36/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD				MAX. UNBRACED LENGTH FR-TO	MEMB.	WESS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
			FROM	TO	PLF	CSI (LC)					
FR-TO											
E-B		-342/0	0.0	0.0	0.13 (4)	7.81					
A-B		0/28	-91.8	-91.8	0.12 (1)	10.00					
B-C		-19/0	-91.8	-91.8	0.22 (1)	6.25					
E-D		0/0	-18.5	-18.5	0.13 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CSI: TC=0.22/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

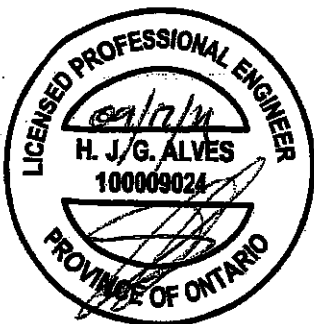
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)





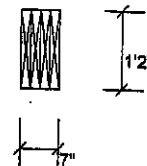
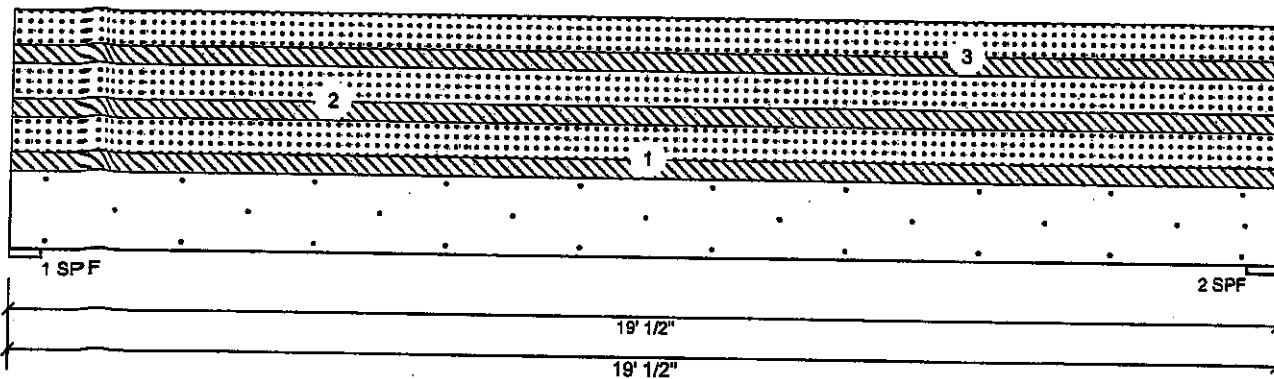
Client:
Project:
Address:

Date: 2021-09-17
Input by:
Job Name: 421231 -
Project #:

Page 1 of 2

BM1 West Fraser 2.0 LVL 1.750" X 14.000" 4-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Roof (Residential)
Plies:	4	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	380	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	Yes
Importance:	Normal - II	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions PATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind
1	Vertical	0	2477	4225	0
2	Vertical	0	2477	4225	0

Bearings and Factored Reactions

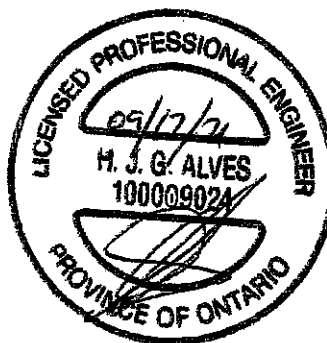
Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	Vert	40%	3096 / 6337	9433	L	1.25D+1.5S
2 - SPF	5.500"	Vert	40%	3096 / 6337	9433	L	1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	41250 ft-lb	9'6 1/4"	100447 ft-lb	0.411 (41%)	1.25D+1.5S	L
Unbraced	41250 ft-lb	9'6 1/4"	95889 ft-lb	0.430 (43%)	1.25D+1.5S	L
Shear	8915 lb	17 1/2"	32575 lb	0.274 (27%)	1.25D+1.5S	L
LL Defl inch	0.368 (L/596)	9'6 5/16"	0.608 (L/360)	0.604 (60%)	S	L
TL Defl inch	0.583 (L/375)	9'6 5/16"	0.608 (L/360)	0.959 (96%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of SDW22634 at 24" o.c. Maximum end distance not to exceed 12".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Simpson fasteners applied from a single side of the member use tip values where published.
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top loads must be supported equally by all plies.
- 7 Top must be laterally braced at end bearings.
- 8 Bottom must be laterally braced at end bearings.
- 9 Lateral slenderness ratio based on full section width.



DWG NO. TAM F-2132672
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		1-0-0	Top	13.4 PSF	0 PSF	25.6 PSF	0 PSF	
2	Uniform		7-1-0	Near Face	13.4 PSF	0 PSF	25.6 PSF	0 PSF	T8
3	Uniform		9-3-0	Far Face	13.4 PSF	0 PSF	25.6 PSF	0 PSF	T9A
	Self Weight				28 PLF				

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

West Fraser
(250) 991-5350
www.westfraser.com
CCMC: 12904-R

This design is valid until 2024-05-24



CSDI
DRAW
DESIGN
BUILD

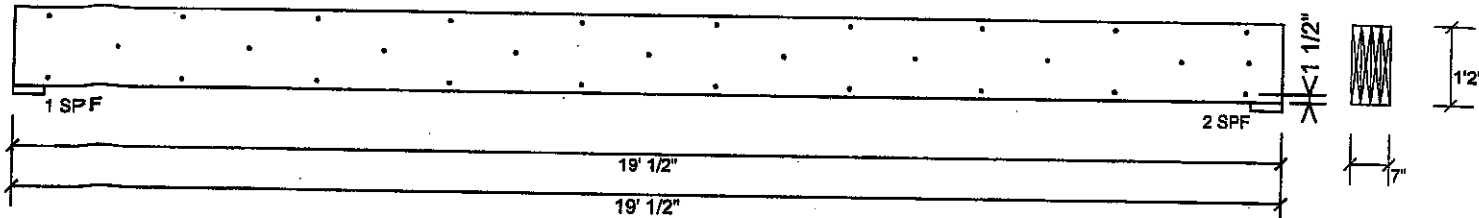


Client:
Project:
Address:

Date: 2021-09-17
Input by:
Job Name: 421231
Project #:

Page 2 of 2

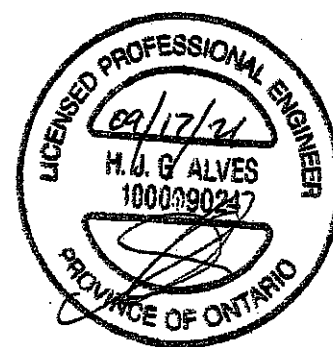
BM1 West Fraser 2.0 LVL 1.750" X 14.000" 4-Ply - PASSED Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of SDW22634 at 24" o.c.. Maximum end distance not to exceed 12".

Capacity	69.9 %
Load	382.6 PLF
Yield Limit per Foot	547.5 PLF
Yield Limit per Fastener	365.0 lb.
Yield Mode	Lookup
Edge Distance	1 1/2"
Min. End Distance	6"
Load Combination	1.25D+1.5S
Duration Factor	1.00



DWG NO. IAM T-2132672
STRUCTURAL
COMPONENT ONLY

Notes

Calculated Structural Design is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

West Fraser
(250) 991-5350
www.westfraser.com
CCMC: 12904-R

This design is valid until 2024-05-24



CSDI
DRAW
DESIGN
BUILD



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering tem and General Safety notes.

T-1900218

Feb 09, 2018

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

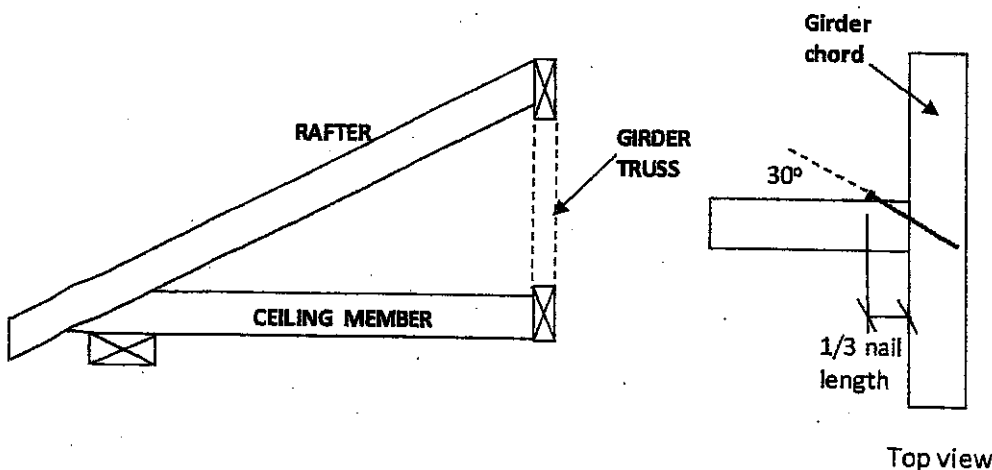
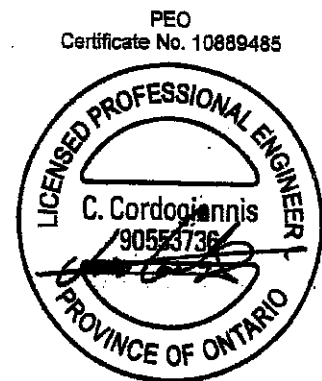


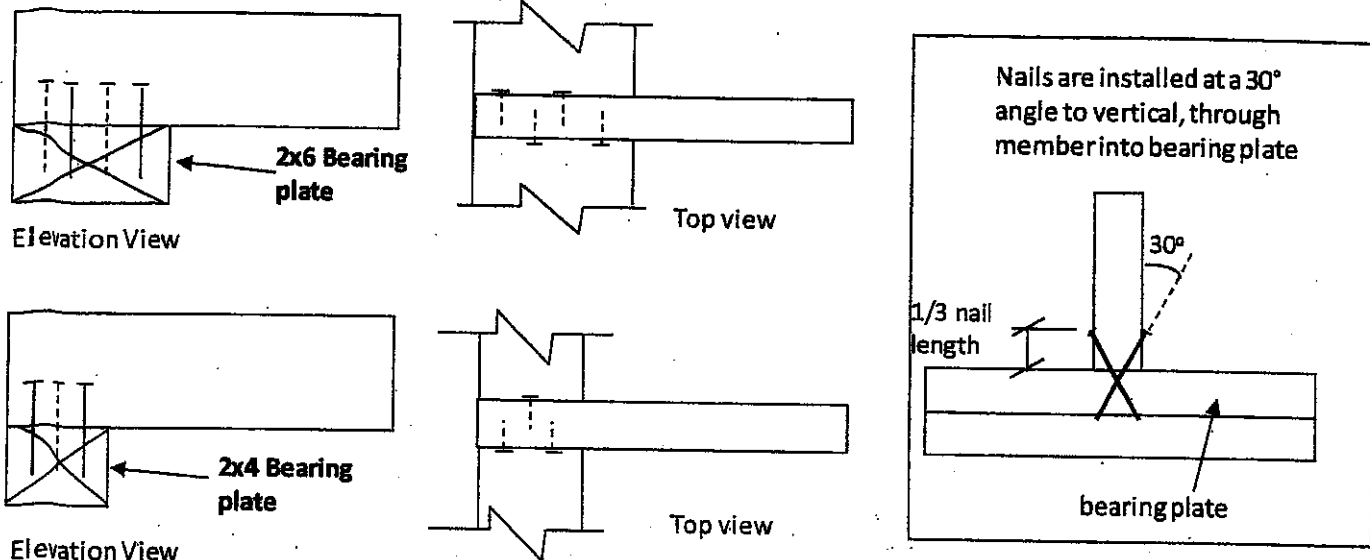
Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss



December 21, 2020

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_0 factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



LUL/LUS/LJS/HUS/HHUS/HGUS**SIMPSON**
Strong-Tie**Standard and Double-Shear Joist Hangers**

This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options Information on pp. 105–107.



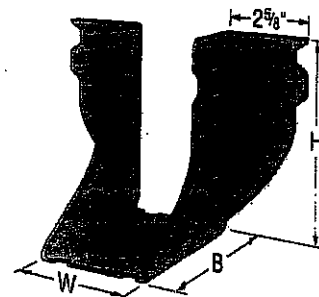
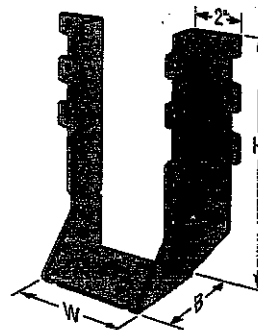
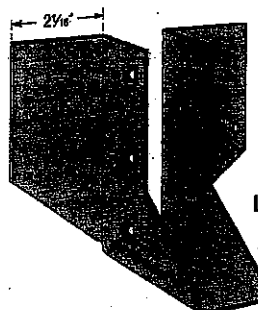
Double-Shear
Nailing
Top View



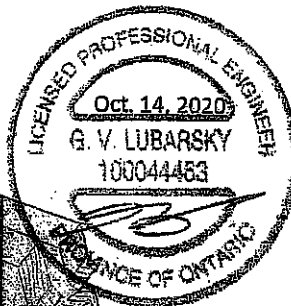
Double-Shear
Nailing
Side View;
Do not
bend tab



Dome Double-Shear
Nailing
Side View
(available on
some models)

**LUS28****LU26L****HUS210**
(HUS26, HUS28,
and HHUS similar)**HGUS28-2****HHUS210-2****LJS26DS**

Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

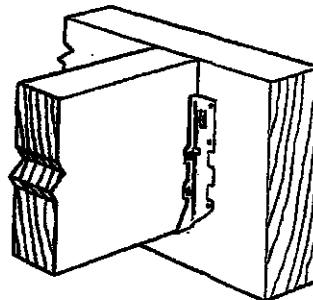
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

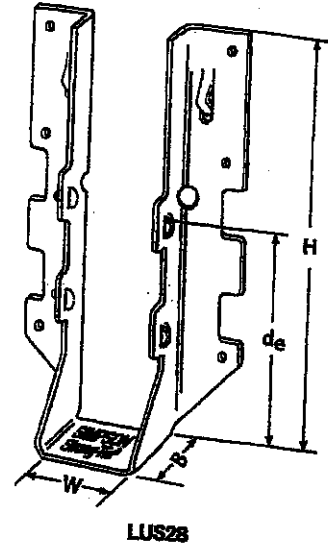
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

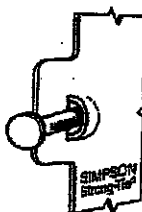


Typical LUS Installation

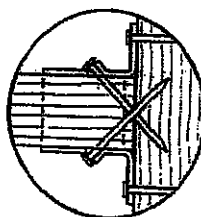


Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁ ¹		Face	Joist	D.F.-L		S.-P.-F	
									Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)	Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)
LUS24	18	1 1/8	3 1/4	1 1/4	1 1/8		(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/4	2	1 1/8		(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/8	4 1/4	1 1/4	3/4		(4) 10d	(4) 10d	1420	2170	1290	1830
LUS26-2	18	3 1/8	4 1/4	2	4		(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 1/8	4 1/4	2	3 1/4		(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 1/4	1 1/4	3/4		(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4		(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 1/8	6 1/4	2	3 1/4		(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/4	1 1/4	3/4		(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6		(8) 16d	(6) 16d	2580	4500	2320	3185
LUS210-3	18	4 1/8	8 1/4	2	5 1/4		(8) 16d	(6) 16d	2580	3345	2320	2375

1. d₂ is the distance from the seat of the hanger to the highest joist nail.



Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,680



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPEC LUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

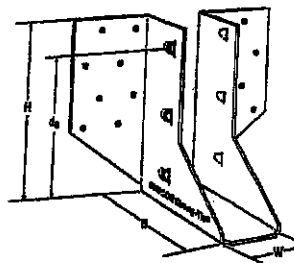
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

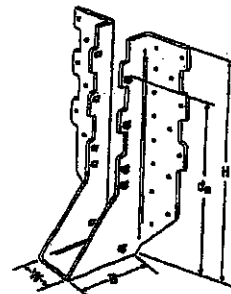
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

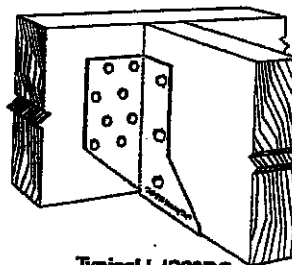
- See current catalogue for options



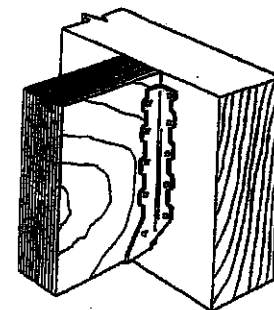
LJS26DS



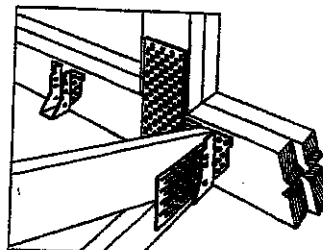
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



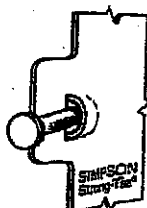
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

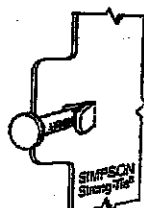
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁	Face	Joist	D.F.-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LJS26DS	18	1 1/8	5	3 1/2	4 1/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/4	3	3 1/8	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 1/8	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 1/2	3	7 3/8	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d₂ is the distance from the seat of the hanger to the highest joist nail.

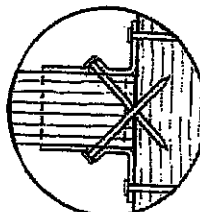


Double
Shear
Nailing
prevents
tabs
breaking
off
(available
on
some
models).

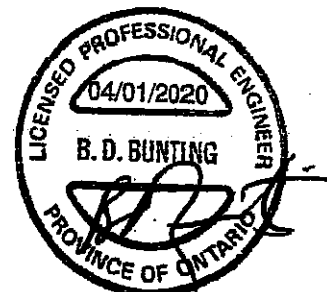
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend
tab
back.



Double
Shear
Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

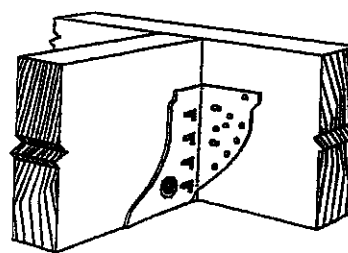
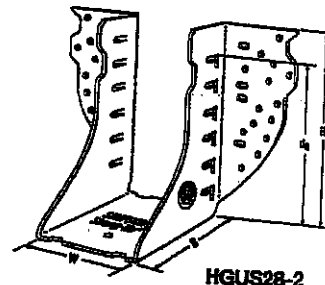
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

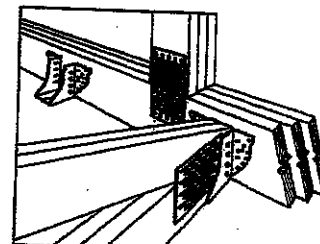
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



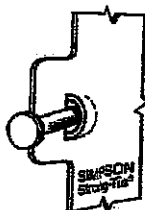
Typical HGUS Installation



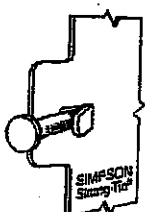
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁	Face	Joist		D.Fr-L		S-P-F	
									Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)	Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)
HGUS26	12	1 1/8	5 3/8	5	4 1/2	(20) 16d	(8) 16d		2685	6625	2685	5700
HGUS26-2	12	3 1/8	5 7/8	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/2	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 7/8	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d		3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS210	12	1 1/8	9 1/8	5	7 1/8	(46) 16d	(16) 16d		3535	11070	2510	8090
HGUS210-2	12	3 1/8	9 3/8	4	8 1/8	(46) 16d	(16) 16d		6840	14015	4855	10270
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d		6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 3/8	4	8 1/8	(46) 16d	(16) 16d		6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d		7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d		10130	16400	7195	11645

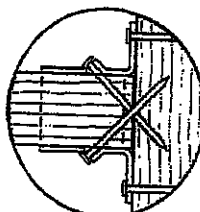
1. d₂ is the distance from the seat of the hanger to the highest joist nail.



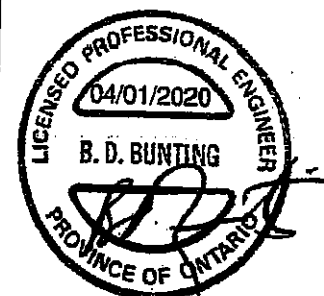
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

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T-SPECHGUS20 3/20 exp. 6/22

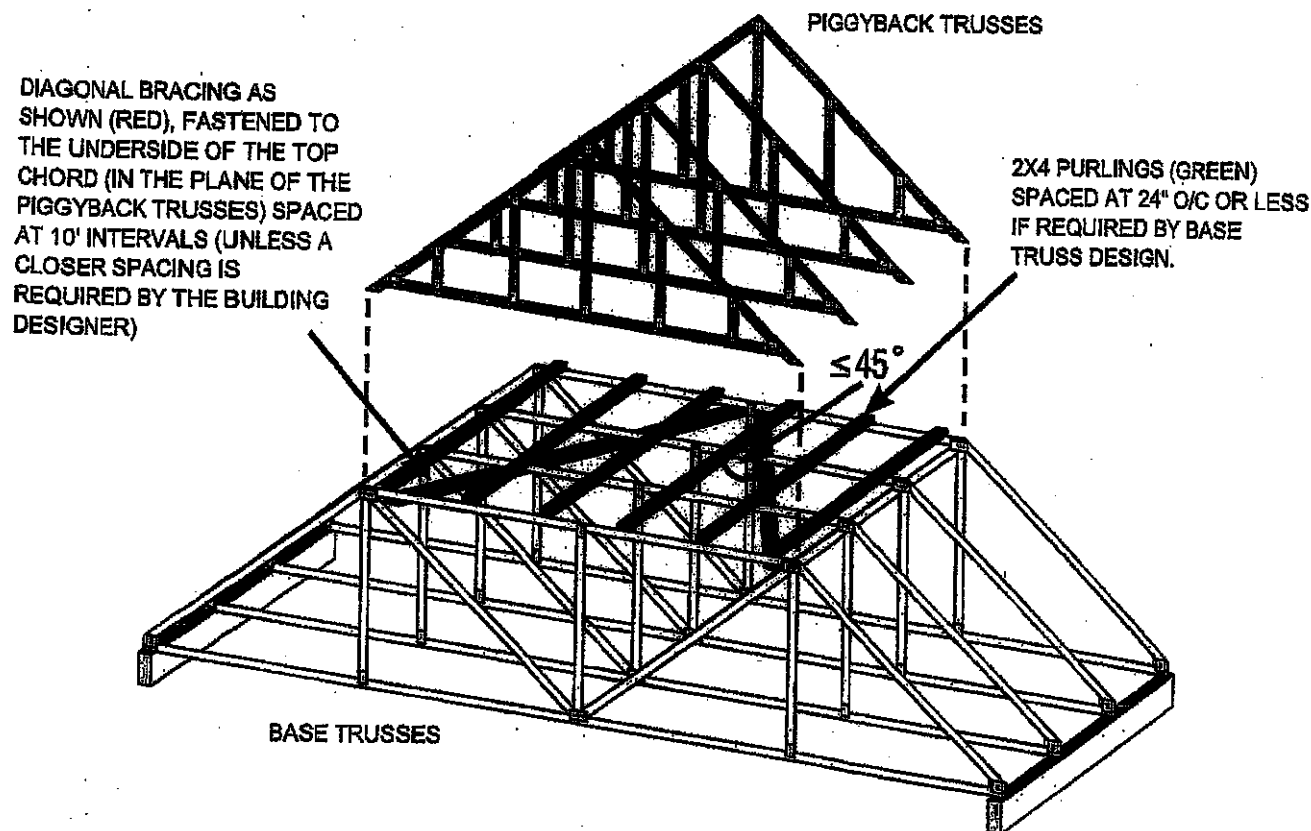
(800) 999-5099
strongtie.com

Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

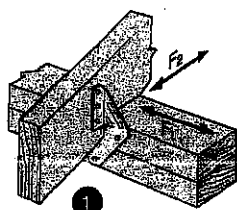
SKETCH FROM BCSI-CANADA 2013

Disclaimer:

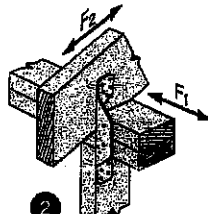
OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

H/TSP

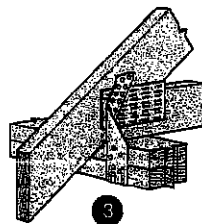
Seismic and Hurricane Ties (cont.)



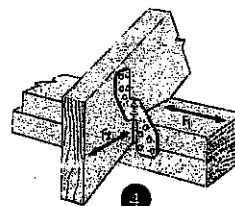
1 H1 Installation



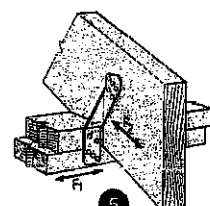
2 H2A Installation



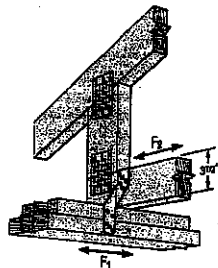
3 TSP Installation



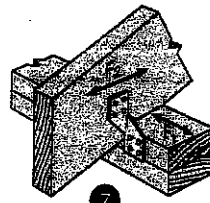
4 H2.5A Installation
(Nails into both top plates)



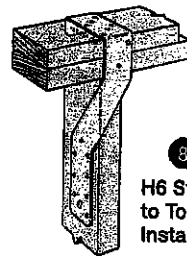
5 H2.5T Installation
(Nails into both top plates)



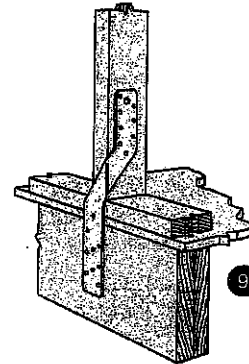
6 H2.5T Installation



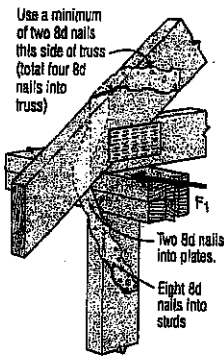
7 H3 Installation
(Nails into upper top plate)



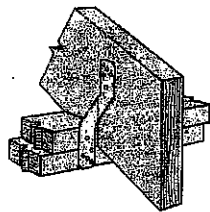
8 H6 Stud to Top Plate Installation



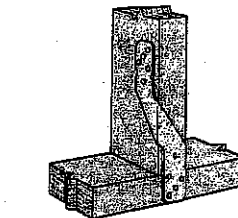
9 H6 Stud to Band Joist Installation



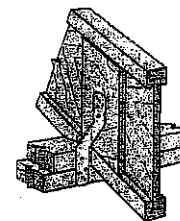
10 H7Z Installation



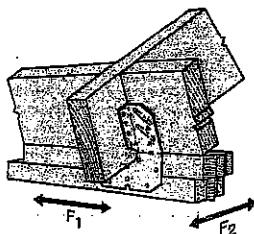
11 H8 Attaching Rafter to Double Top Plates



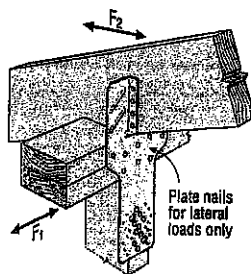
12 H8 attaching Stud to Sill
(4) 8d into plate, (5) 8d into stud



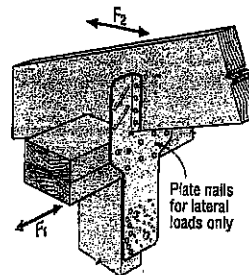
13 H8 attaching I-Joist to Double Top Plates



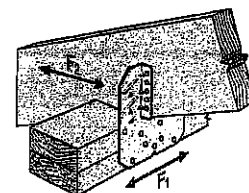
14 H10A Field-Bent Installation



15 H10S Installation

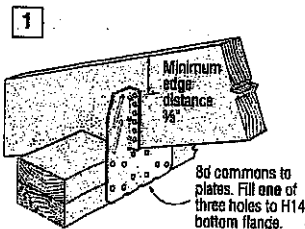


16 H10S Installation with Stud Offset

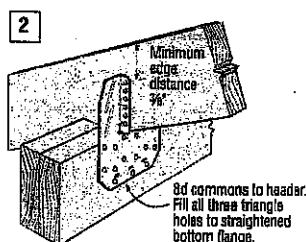


17 H10A Installation

H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

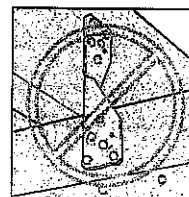


18 H14 Installation to Double Top Plates



19 H14 Installation to Double 2x Header

Avoid a Misinstallation



Do not make new holes or overdrive nails.

Straps and Ties

H/TSP

Seismic and Hurricane Ties

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

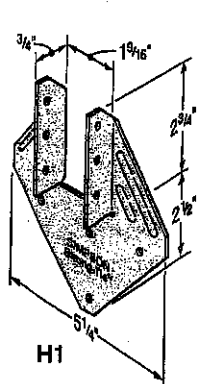
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

Material: See table

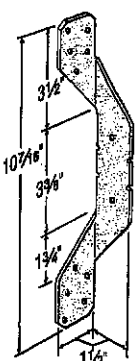
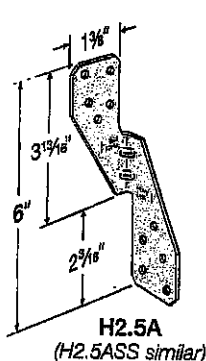
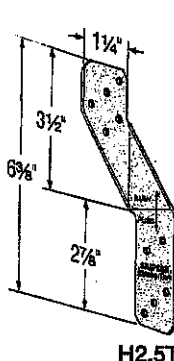
Finish: Galvanized. H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp. 20–24 or visit strongtie.com.

Installation:

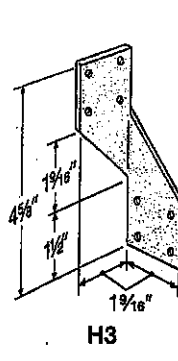
- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12; use H10A sloped loads for field bent installation.



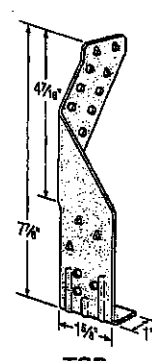
H1

H2A
(H2ASS similar)H2.5A
(H2.5ASS similar)

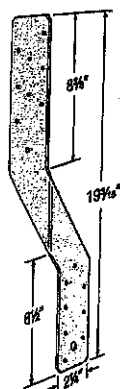
H2.5T



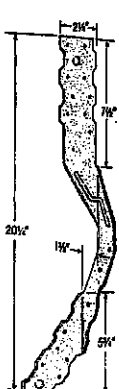
H3



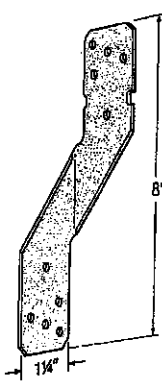
TSP



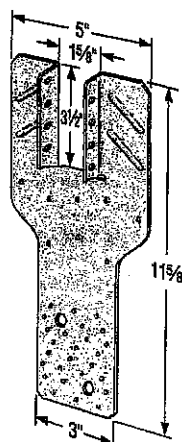
H6



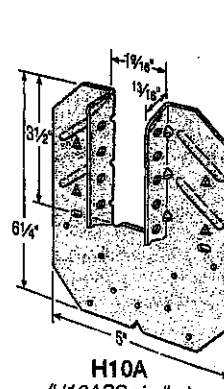
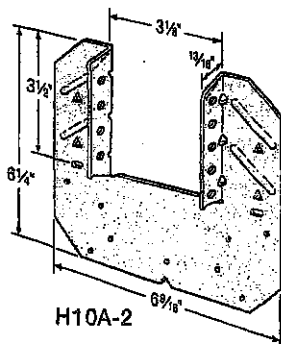
H7Z



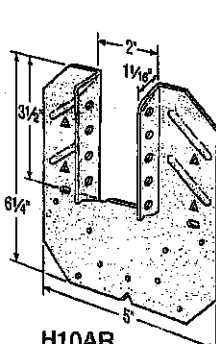
H8



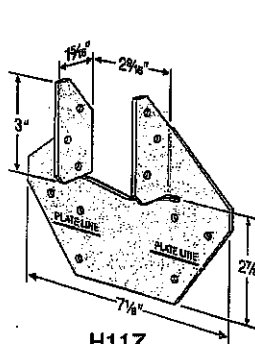
H10S

H10A
(H10ASS similar)

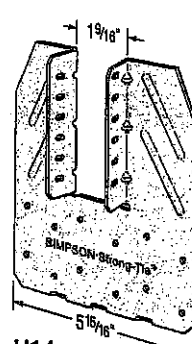
H10A-2



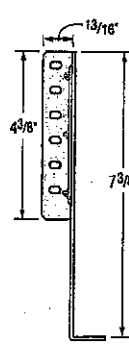
H10AR



H11Z



H14

H14
Profile

Straps and Ties

TECHNICAL BULLETIN

H - Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

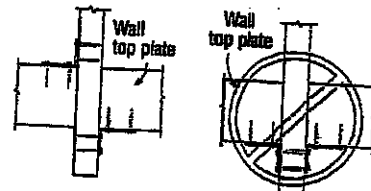
Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA O86-14
• Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners
• Nails: $8d = 0.131" \text{ dia.} \times 2\frac{1}{2}" \text{ long}$ common wire, $8d \times 1\frac{1}{2}" = 0.131" \times 1\frac{1}{2}" \text{ long}$, $10d \times 1\frac{1}{2}" = 0.146" \times 1\frac{1}{2}" \text{ long}$
• H1 can be installed with flanges facing outwards
• Hurricane ties do not replace solid blocking

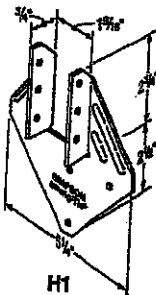
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0 .

Hurricane Tie Installations to Achieve Twice the Load (Top View)

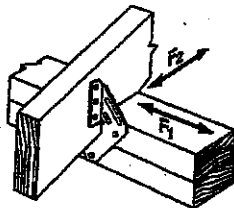


Install diagonally across from each other for minimum 2x truss.

Nailing into both sides of a single ply 2x truss may cause the wood to split.



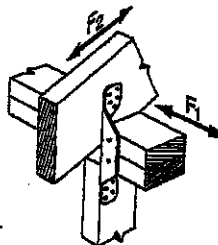
H1



H1 Installation



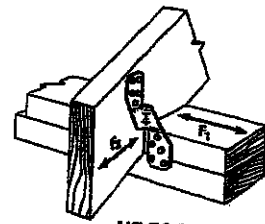
H2A



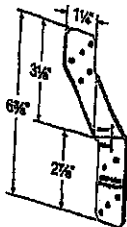
H2A Installation



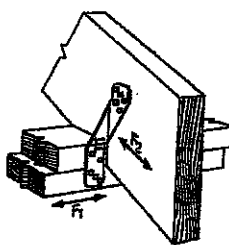
H2.5A



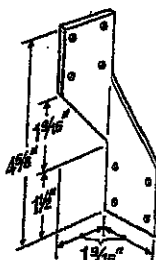
H2.5A Installation



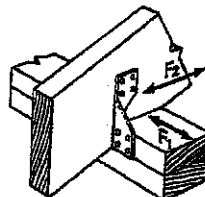
H2.5T



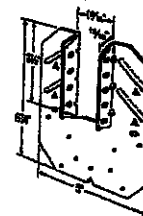
H2.5T Installation
(Nails into both top plates)



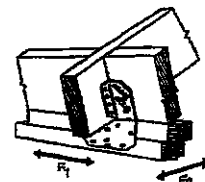
H3



H3 Installation



H10A



H10A Installation

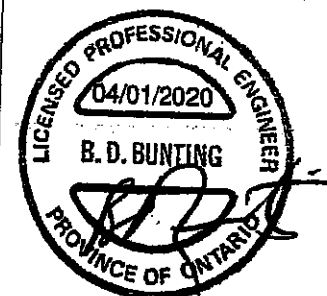
Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
		To Rafter	To Plates	To Studs	D-Fir-L			S-P-F		
					Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
H1	18	(6) 8d x 1 1/2"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1 1/2"	(2) 8d x 1 1/2"	(5) 8d x 1 1/2"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.

2. Factored resistances are for one anchor. A minimum rafter thickness of 2 1/2" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.

4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

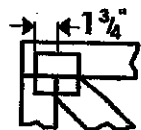
© 2020 Simpson Strong-Tie Company Inc.

T-SPECH20 3/20 exp. 6/22

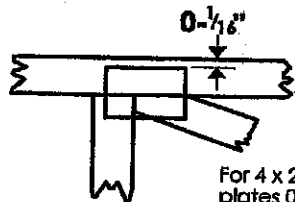
(800) 999-5099
strongtie.com

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

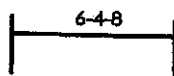


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

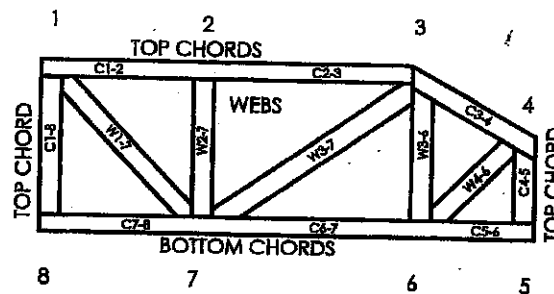
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm
(Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.