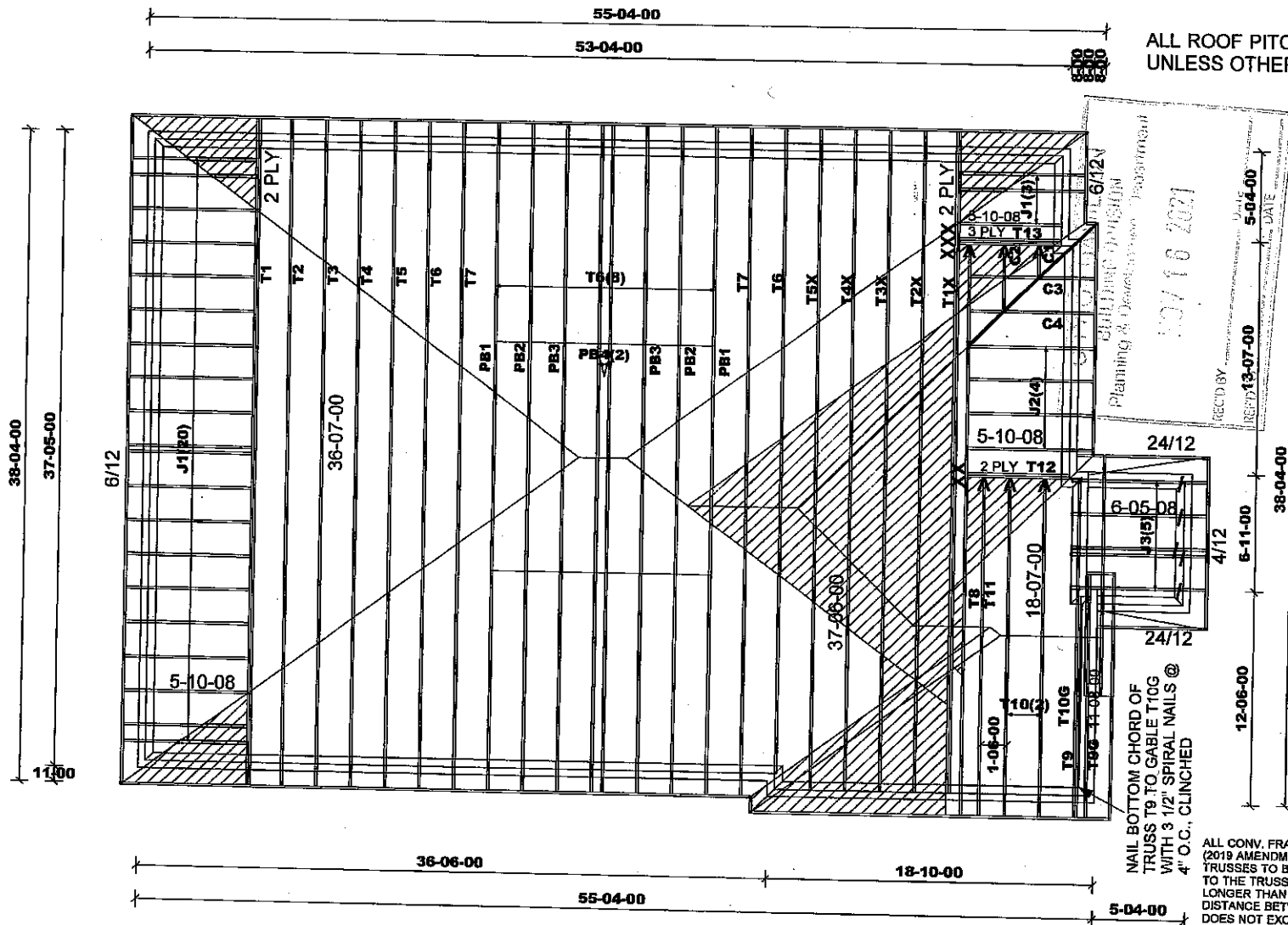




ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)
HGUS26-2 - (XX)
HGUS26-3 - (XXX)
H2.5T - (I)

DENOTES
CONV.
FRAMING

DESIGN CONFORMS WITH
OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL
PART: 9
Ss = 31.3 psf Sr = 8.4 psf

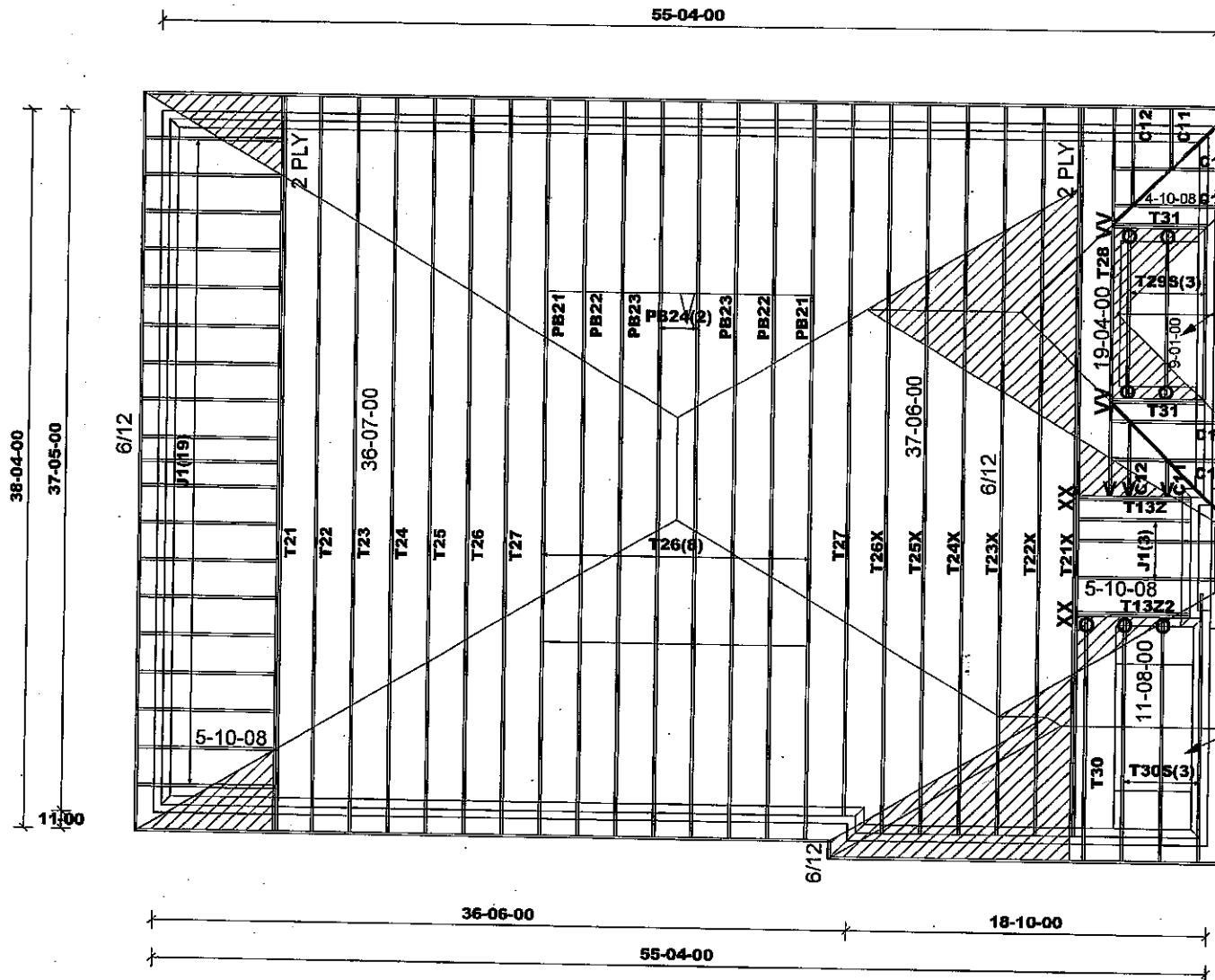
DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C.2012
(2019 AMENDMENT) ROOF RAFTERS THAT CROSS MEET OVER
TRUSSES TO BE 2X4 SPF @ 24"O.C. WITH A 2X4 VERT. POST
TO THE TRUSS UNDER NEATH AT EACH CROSS PT. VERT. POST
LONGER THAN 8' TO HAVE LATERAL BRACING SO THAT THE
DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING
DOES NOT EXCEED 6'

M13792



Job Track: 52166	Builder / Location: GREENPARK HOMES / HAMILTON	Model / Elevation: SPRINGFIELD 3S / EL.1
Plan Log: 204488	Project: RUSSELL GARDENS PH.4	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE
Layout ID: 417467	Date: 2021-03-23 Sales: Rick DiCiano Designer: LC	Mitek ver 8.4.2.286



ALL ROOF PITCHES ARE 10/12
UNLESS OTHERWISE NOTED

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2 - (VV)
HGUS26-2 - (XX)

DENOTES
CONV.
FRAMING



DESIGN CONFORMS WITH
OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL
PART: 9
Ss = 31.3 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C.2012
(2019 AMENDMENT) ROOF RAFTERS THAT CROSS MEET OVER
TRUSSES TO BE 2X4 SPF @ 24"O.C. WITH A 2X4 VERT. POST
TO THE TRUSS UNDER NEATH AT EACH CROSS PT. VERT. POST
LONGER THAN 6" TO HAVE LATERAL BRACING SO THAT THE
DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING
DOES NOT EXCEED 6"

M13792



Job Track: **52166**

Plan Log: **204488**

Layout ID: **417471**

Builder / Location:

GREENPARK HOMES / HAMILTON

Project: **RUSSELL GARDENS PH.4**

Date: **2021-03-23**

Sales:

Rick DiCiano

Designer: LC

Model / Elevation:

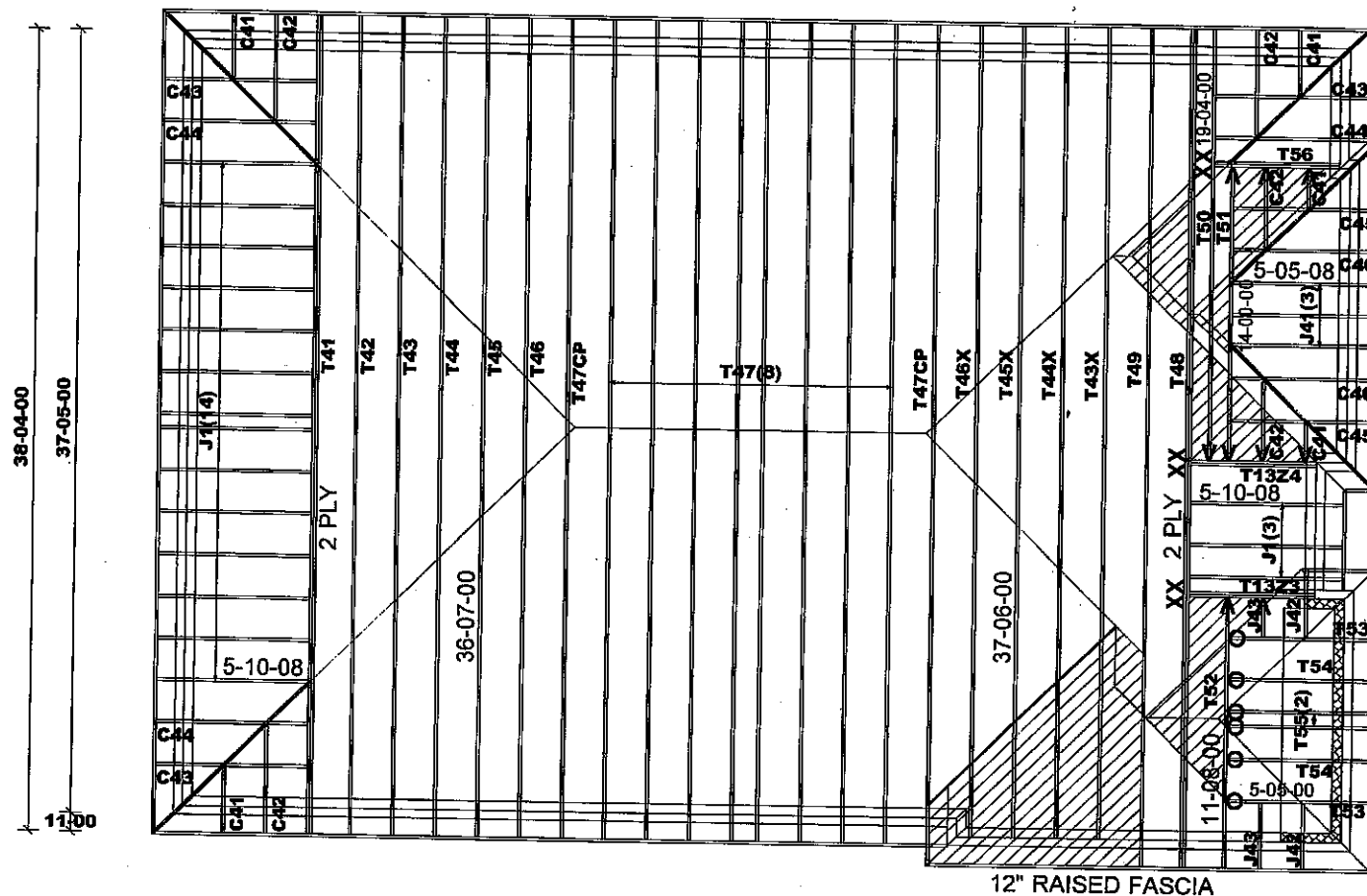
SPRINGFIELD 3S / EL.2

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED,
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TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER
PURPOSE.

Mile ver 8.4.2.288

55-04-00
54-10-00
6'-0"

ALL ROOF PITCHES ARE 6/12
UNLESS OTHERWISE NOTED



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES
CONV.
FRAMING



DESIGN CONFORMS WITH
OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL
PART: 9
Ss = 31.3 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C.2012
(2019 AMENDMENT) ROOF RAFTERS THAT CROSS MEET OVER
TRUSSES TO BE 2X4 SPF @ 24"O.C. WITH A 2X4 VERT. POST
TO THE TRUSS UNDER NEATH AT EACH CROSS PT. VERT. POST
LONGER THAN 6" TO HAVE LATERAL BRACING SO THAT THE
DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING
DOES NOT EXCEED 6"

M13792



Job Track: 52166

Plan Log: 204488

Layout ID: 417473

Builder / Location:

GREENPARK HOMES / HAMILTON

Project: RUSSELL GARDENS PH.4

Date: 2021-03-23

Sales: Rick DiCiano

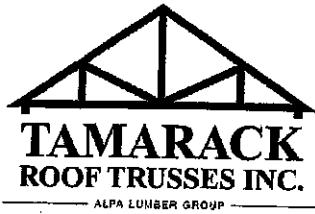
Designer: LC

Model / Elevation:

SPRINGFIELD 3S / EL.3

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED,
OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY
TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER
PURPOSE.

Mfilek ver 8.4.2.286



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: SPRINGFIELD 3S
 Lot #:
 Elevation: EL.1

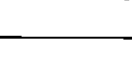
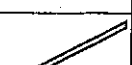
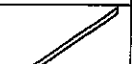
Job Track: 52166
 PlanLog: 204488
 Layout ID: 417467
 Ref #
 Page: 1 of 3
 Date: 03-23-2021
 Designer: Leo Chen
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	8/12	36-07-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	357.17 225.33		
	1 2-ply	T1X Hip Girder	8/12	37-06-00	4-01-04	2 x 4 2 x 6	1-03-08	1-04-13 9-08	352.82 221.33		
	1	T2 Hip	8/12	36-07-00	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	173.28 107.83		
	1	T2X Hip	8/12	37-06-00	5-01-04	2 x 4 2 x 6	1-03-08	1-04-13 9-08	176.26 107.83		
	1	T3 Hip	8/12	36-07-00	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	181.49 112.67		
	1	T3X Hip	8/12	37-06-00	6-01-04	2 x 4 2 x 6	1-03-08	1-04-13 9-08	182.26 111.67		
	1	T4 Hip	8/12	36-07-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	180.89 110.83		
	1	T4X Hip	8/12	37-06-00	7-01-04	2 x 4 2 x 6	1-03-08	1-04-13 9-08	181.67 109.83		
	1	T5 Hip	8/12	36-07-00	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	189.61 118.00		
	1	T5X Hip	8/12	37-06-00	8-01-04	2 x 4 2 x 6	1-03-08	1-04-13 9-08	193.44 119.83		
	10	T6 Hip	8/12	36-07-00	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	1922.37 1178.33		
	2	T7 Hip	8/12	36-07-00	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	397.66 246.00		
	1	T8 Hip Girder	8/12	32-02-00	5-03-13	2 x 4 2 x 6	1-03-08	1-04-13 1-04-13	170.84 105.33		
	1	T9 Common	8/12	11-08-00	5-03-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	54.59 34.33		

	DELIVERY SHIPLIST			
	Lumber Yard:	TAMARACK LUMBER	Job Track:	52166
	Builder:	GREENPARK HOMES	PlanLog:	204488
	Project:	RUSSELL GARDENS PH.4	Layout ID:	417467
	Location:	HAMILTON	Ref #	
	Model:	SPRINGFIELD 3S	Page:	2 of 3
	Lot #:		Date:	03-23-2021
	Elevation:	EL.1	Designer:	Leo Chen
		Sales Rep:	Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T9G GABLE	8 /12	11-08-00	5-03-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	48.09 30.83		
	2	T10 Common	8 /12	18-07-00	7-07-02	2 x 4	1-03-08	1-04-13 1-04-13	167.19 103.33		
	1	T10G GABLE	8 /12	18-07-00	7-07-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	82.52 51.83		
	1	T11 Hip	8 /12	18-07-00	7-00-04	2 x 4	1-03-08	1-04-13 1-04-13	87.99 56.17		
	1 2-ply	T12 Monopitch Girder	8 /12	5-10-08	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	65.45 41.33		
	1 3-ply	T13 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	87.59 56.50		
	2	PB1 Piggyback	8 /12	13-05-11	2-00-00	2 x 4			77.16 49.33		
	2	PB2 Piggyback	8 /12	13-05-11	3-00-00	2 x 4			77.14 49.67		
	2	PB3 Piggyback	8 /12	13-05-11	4-00-00	2 x 4			79.82 52.00		
	2	PB4 Piggyback	8 /12	13-05-11	4-05-14	2 x 4			81.16 52.00		
	23	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	415.78 268.33		
	4	J2 Jack-Open	8 /12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	76.76 46.67		
	5	J3 Jack-Partial	4 /12	6-05-08	2-10-11	2 x 4	1-03-08	3-15 2-05-12	95.15 60.83		
	1	C1 Jack-Open	8 /12	1-10-08	2-07-02	2 x 4	1-03-08	1-04-13 2-07-13	9.03 5.67		

 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREENPARK HOMES Project: RUSSELL GARDENS PH.4 Location: HAMILTON Model: SPRINGFIELD 3S Lot #: Elevation: EL.1	Job Track: 52166 PlanLog: 204488 Layout ID: 417467 Ref # Page: 3 of 3 Date: 03-23-2021 Designer: Leo Chen Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	C2 Jack-Open	8 /12	1-10-08	3-11-02	2 x 4	1-03-08 1-10-15	1-04-13 2-07-13	11.77 7.67		
	1	C3 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 4-01-01	1-04-13 2-07-02	13.59 8.33		
	1	C4 Jack-Open	8 /12	3-09-07	3-11-02	2 x 4	1-03-08 2-01-01	1-04-13 3-11-02	16.33 10.33		

TOTAL #TRUSS= 80

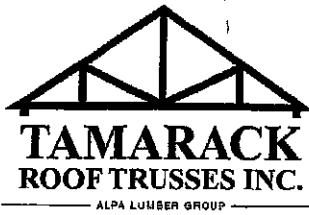
TOTAL BFT OF ALL TRUSSES= 3859.96 BFT.

TOTAL WEIGHT OF ALL TRSSES 6207.04 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
5	Hardware	H2.5T	
1	Hardware	HGUS26-2	
1	Hardware	HGUS26-3	
6	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 13



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: SPRINGFIELD 3S
 Lot #:
 Elevation: EL.2

Job Track: 52166
 PlanLog: 204488
 Layout ID: 417471
 Ref #
 Page: 1 of 3
 Date: 03-23-2021
 Designer: Leo Chen
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T13Z Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T13Z2 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T21 Hip Girder	10 /12	36-07-00	4-01-10	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	436.11 264.00		
	1 2-ply	T21X Hip Girder	10 /12	37-06-00	4-01-10	2 x 6	1-03-08	1-07-11 10-08	430.04 260.00		
	1	T22 Hip	10 /12	36-07-00	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	174.63 106.50		
	1	T22X Hip	10 /12	37-06-00	5-01-04	2 x 4 2 x 6	1-03-08	1-07-11 10-08	178.24 109.17		
	1	T23 Hip	10 /12	36-07-00	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	183.59 116.33		
	1	T23X Hip	10 /12	37-06-00	6-01-04	2 x 4 2 x 6	1-03-08	1-07-11 10-08	189.32 117.83		
	1	T24 Hip	10 /12	36-07-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	192.13 119.83		
	1	T24X Hip	10 /12	37-06-00	7-01-04	2 x 4 2 x 6	1-03-08	1-07-11 10-08	195.8 119.83		
	1	T25 Hip	10 /12	36-07-00	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	211.92 131.33		
	1	T25X Hip	10 /12	37-06-00	8-01-04	2 x 4 2 x 6	1-03-08	1-07-11 10-08	215.81 131.33		
	9	T26 Hip	10 /12	36-07-00	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	1877.62 1167.00		
	1	T26X Hip	10 /12	37-06-00	9-01-04	2 x 4 2 x 6	1-03-08	1-07-11 10-08	213.38 131.00		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: SPRINGFIELD 3S
 Lot #:
 Elevation: EL.2

Job Track: 52166
 Plan Log: 204488
 Layout ID: 417471
 Ref #
 Page: 2 of 3
 Date: 03-23-2021
 Designer: Leo Chen
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T27 Hip	10/12	36-07-00	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	441.72 274.67		
	1	T28 Hip Girder	10/12	19-04-00	5-08-07	2 x 4 2 x 6	1-03-08	1-07-11 1-07-11	97.19 60.83		
	3	T29S Scissor	10/12 6/12	9-01-00	5-06-05	2 x 4 2 x 6		1-08-15 1-08-15	143.6 97.00		
	1	T30 Common	10/12	11-08-00	6-06-00	2 x 4	1-03-08	1-07-11 1-07-11	49.68 31.67		
	3	T30S Roof Special	10/12 6/12	11-08-00	6-06-00	2 x 4	1-03-08	1-07-11 1-07-11	167.01 110.00		
	2 2-ply	T31 Monopitch Girder	10/12	4-10-08	5-08-07	2 x 4 2 x 6		1-07-11 5-08-07	109.66 70.00		
	2	PB21 Piggyback	10/12	18-08-01	2-00-00	2 x 4			113.79 73.33		
	2	PB22 Piggyback	10/12	18-08-01	3-00-00	2 x 4			117.53 74.33		
	2	PB23 Piggyback	10/12	18-08-01	4-00-00	2 x 4			122.33 80.00		
	2	PB24 Piggyback	10/12	18-08-01	5-00-00	2 x 4			121.74 76.67		
	22	J1 Jack-Open	6/12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	369.48 234.67		
	2	C11 Jack-Open	10/12	1-10-08	3-01-09	2 x 4	1-03-08	1-07-11 3-02-07	19.69 14.00		
	2	C12 Jack-Open	10/12	1-10-08	4-09-09	2 x 4	1-03-08 1-10-15	1-07-11 3-02-07	25.63 16.87		
	2	C13 Jack-Open	10/12	1-09-07	3-01-09	2 x 4	1-03-08 3-01-01	1-07-11 3-01-09	26.53 18.00		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP		DELIVERY SHIPLIST	
		Lumber Yard: TAMARACK LUMBER Builder: GREENPARK HOMES Project: RUSSELL GARDENS PH.4 Location: HAMILTON Model: SPRINGFIELD 3S Lot #: Elevation: EL.2	Job Track: 52166 PlanLog: 204488 Layout ID: 417471 Ref # Page: 3 of 3 Date: 03-23-2021 Designer: Leo Chen Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	C14 Jack-Open	10 /12	3-09-07	4-09-09	2 x 4	1-03-08 1-01-01	1-07-11 4-09-09	32.47 20.67		

TOTAL # TRUSS= 78 TOTAL BFT OF ALL TRUSSES= 4103 BFT. TOTAL WEIGHT OF ALL TRSSES 6573.42 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
3	Hardware	LJS26DS	
7	Hardware	LUS24	
2	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 14

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: SPRINGFIELD 3S
 Lot #:
 Elevation: EL.3

Job Track: 52166
 PlanLog: 204488
 Layout ID: 417473
 Ref #
 Page: 1 of 3
 Date: 03-23-2021
 Designer: Leo Chen
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1323 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T1324 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T41 Hip Girder	6 /12	36-07-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	384.08 236.00		
	1	T42 Hip	6 /12	36-07-00	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	171.77 106.83		
	1	T43 Hip	6 /12	36-07-00	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	172.82 106.67		
	1	T43X Hip	6 /12	37-06-00	6-01-04	2 x 4 2 x 6	1-03-08	1-02-00 8-08	169.97 103.83		
	1	T44 Hip	6 /12	36-07-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	171.67 105.17		
	1	T44X Hip	6 /12	37-06-00	7-01-04	2 x 4 2 x 6	1-03-08	1-02-00 8-08	187.91 101.83		
	1	T45 Hip	6 /12	36-07-00	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	178.27 110.00		
	1	T45X Hip	6 /12	37-06-00	8-01-04	2 x 4 2 x 6	1-03-08	1-02-00 8-08	173.7 105.50		
	1	T46 Hip	6 /12	36-07-00	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	186.4 114.67		
	1	T46X Hip	6 /12	37-06-00	9-01-04	2 x 4 2 x 6	1-03-08	1-02-00 8-08	180.52 109.67		
	8	T47 Common	6 /12	36-07-00	10-03-12	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1397.26 856.00		
	2	T47CP Hip	6 /12	36-07-00	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	351.55 216.67		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: SPRINGFIELD 3S
 Lot #:
 Elevation: EL.3

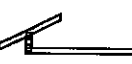
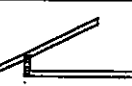
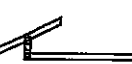
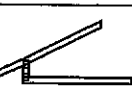
Job Track: 52166
 PlanLog: 204488
 Layout ID: 417473
 Ref #
 Page: 2 of 3
 Date: 03-23-2021
 Designer: Leo Chen
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T48 Hip Girder	6 / 12	37-06-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 2-02-00	425.63 262.00		
	1	T49 Hip	6 / 12	37-06-00	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 2-02-00	179.48 111.67		
	1	T50 Hip Girder	6 / 12	19-04-00	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 1-02-00	91.25 58.00		
	1	T51 Hip Girder	6 / 12	14-00-00	3-10-12	2 x 4		1-02-00 1-02-00	53.62 35.83		
	1	T52 Hip Girder	6 / 12	11-08-00	4-10-08	2 x 4 2 x 6	1-03-08	2-02-00 2-02-00	62.09 40.50		
	2	T53 Half Hip Girder	6 / 12	5-05-00	3-01-04	2 x 4	1-03-08	1-02-00 3-01-04	57.47 40.00		
	2	T54 Half Hip	6 / 12	5-05-00	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	65.59 44.67		
	2	T55 Monopitch	6 / 12	5-05-00	4-10-08	2 x 4	1-03-08	1-02-00 4-10-08	63.57 43.00		
	1 2-ply	T56 Half Hip Girder	6 / 12	5-10-08	3-08-08	2 x 4 2 x 6		1-02-00 3-08-08	61.95 39.33		
	17	J1 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	285.51 181.33		
	3	J41 Jack-Open	6 / 12	5-05-08	3-10-12	2 x 4	1-03-08	1-02-00 3-10-12	47.36 30.00		
	2	J42 Jack-Open	6 / 12	1-10-08	2-01-04	2 x 4	1-03-08	1-02-00 2-01-04	14.27 9.33		
	2	J43 Jack-Open	6 / 12	1-10-08	3-01-04	2 x 4	1-03-08	2-02-00 3-01-04	20.06 13.67		
	5	C41 Jack-Open	6 / 12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	35.1 23.33		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREENPARK HOMES Project: RUSSELL GARDENS PH.4 Location: HAMILTON Model: SPRINGFIELD 3S Lot #: Elevation: EL.3	Job Track: 52166 PlanLog: 204488 Layout ID: 417473 Ref # Page: 3 of 3 Date: 03-23-2021 Designer: Leo Chen Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	C42 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	47.85 30.00		
	3	C43 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	34.75 22.00		
	3	C44 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	42.4 26.00		
	2	C45 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 3-08-01	1-02-00 2-00-12	22.21 14.67		
	2	C46 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 1-08-01	1-02-00 3-00-12	27.31 17.33		

TOTAL # TRUSS= 83

TOTAL BFT OF ALL TRUSSES= 3390.84

BFT.

TOTAL WEIGHT OF ALL TRSSES 5460.3 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
9	Hardware	LJS26DS	
6	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 18

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Sat Mar 10 20:25:51 2021 Page 1
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1-3-8 0-0 4-0-11 4-0-11 5-1-12 5-5-14 11-8-10 5-5-14 18-3-8 5-5-14 24-10-5 5-5-14 31-5-4 32-5-5 36-7-0 37-10-8
1-3-8 1-1-2 1-1-2 1-3-8
Scale = 1:81



	TOTAL WEIGHT = 2 X 208 = 416 lb
--	---------------------------------

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

[illegible]

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 8.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL
LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.27")
ALLOWABLE DEFL.(TL)= L/360 (1.22")

CSI: TC=0.47/1.00 (F-H:1), BC=0.68/1.00 (R-T:1),
WB=0.41/1.00 (I-P:1), SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00
COMPANION LIVE LOAD FACTOR = 1.00
AUTOSOLVE HEELS OFF
TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR ORIGINAL CONTROL IN THE

TRUSS MANUFACTURING PLANT.

NAIL VALUES

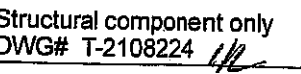
PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (H) (INPUT = 0.80)

JSI METAL = 0.88 (S) (INPUT = 1.00)



JOB NAME 417467	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MITek Industries, Inc. Sat Mar 20 10:25:51 2021 Page 2 ID: FnlJiePQJhgRPc08ysmZj2zZrcl-qGIIHepXQ ?QlKKhHX0MU607OGIU78bQ9s0lvzZ4IE	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMWW-t	MT20	5.0	6.0	2.50	2.75
E	TMWW-t	MT20	5.0	6.0		
F	TMWW-w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	8.0	2.50	2.75
J	TTWW+m	MT20	5.0	8.0	Edge	
K	TMVW-p	MT20	5.0	8.0	1.50	3.00
M	BMV1-p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMWW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.75
Q	BS-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.75
U	BMWW-t	MT20	5.0	6.0		
V	BMWW-t	MT20	5.0	6.0	2.50	2.50
W	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO			LENGTH FR-TO		
P-AS	0 / 5306	-18.5	-18.5	0.41 (1)	10.00		
AS-AT	0 / 5306	-18.5	-18.5	0.41 (1)	10.00		
AT-AU	0 / 5306	-18.5	-18.5	0.41 (1)	10.00		
AU-O	0 / 5306	-18.5	-18.5	0.41 (1)	10.00		
O-N	0 / 4287	-18.5	-18.5	0.30 (1)	10.00		
N-AV	0 / 0	-18.5	-18.5	0.04 (4)	10.00		
AV-AW	0 / 0	-18.5	-18.5	0.04 (4)	10.00		
AW-M	0 / 0	-18.5	-18.5	0.04 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-41	-41		FRONT	VERT	DEAD		C1
C	4-0-11	-128	-128		FRONT	VERT	TOTAL		C1
C	4-0-11	-175	-175		FRONT	VERT	SNOW		C1
F	18-1-7	-122	-122		FRONT	VERT	TOTAL		C1
F	18-5-9	-122	-122		FRONT	VERT	TOTAL		C1
J	32-6-6	-41	-41		FRONT	VERT	DEAD		C1
J	32-6-6	-128	-128		FRONT	VERT	TOTAL		C1
J	32-6-6	-175	-175		FRONT	VERT	SNOW		C1
N	32-5-9	-29	-29		FRONT	VERT	TOTAL		C1
Q	22-5-9	-29	-29		FRONT	VERT	TOTAL		C1
R	18-1-7	-29	-29		FRONT	VERT	TOTAL		C1
R	18-5-9	-29	-29		FRONT	VERT	TOTAL		C1
S	14-1-7	-29	-29		FRONT	VERT	TOTAL		C1
V	4-1-6	-29	-29		FRONT	VERT	TOTAL		C1
X	8-1-7	-122	-122		FRONT	VERT	TOTAL		C1
Y	8-1-7	-122	-122		FRONT	VERT	TOTAL		C1
Z	10-1-7	-122	-122		FRONT	VERT	TOTAL		C1
AA	12-1-7	-122	-122		FRONT	VERT	TOTAL		C1
AB	14-1-7	-122	-122		FRONT	VERT	TOTAL		C1
AC	16-1-7	-122	-122		FRONT	VERT	TOTAL		C1
AD	20-5-9	-122	-122		FRONT	VERT	TOTAL		C1
AE	22-5-9	-122	-122		FRONT	VERT	TOTAL		C1
AF	24-5-9	-122	-122		FRONT	VERT	TOTAL		C1
AG	26-5-9	-122	-122		FRONT	VERT	TOTAL		C1
AH	28-5-9	-122	-122		FRONT	VERT	TOTAL		C1
AI	30-5-9	-122	-122		FRONT	VERT	TOTAL		C1
AJ	1-1-7	-29	-29		FRONT	VERT	TOTAL		C1
AK	2-1-7	-29	-29		FRONT	VERT	TOTAL		C1
AL	6-1-7	-29	-29		FRONT	VERT	TOTAL		C1
AM	8-1-7	-29	-29		FRONT	VERT	TOTAL		C1
AN	10-1-7	-29	-29		FRONT	VERT	TOTAL		C1
AO	12-1-7	-29	-29		FRONT	VERT	TOTAL		C1
AP	16-1-7	-29	-29		FRONT	VERT	TOTAL		C1
AQ	20-5-9	-29	-29		FRONT	VERT	TOTAL		C1
AR	24-5-9	-29	-29		FRONT	VERT	TOTAL		C1
AS	26-5-9	-29	-29		FRONT	VERT	TOTAL		C1
AT	28-5-9	-29	-29		FRONT	VERT	TOTAL		C1
AU	30-5-9	-29	-29		FRONT	VERT	TOTAL		C1
AV	34-5-9	-29	-29		FRONT	VERT	TOTAL		C1
AW	35-5-9	-29	-29		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2108224 3/2

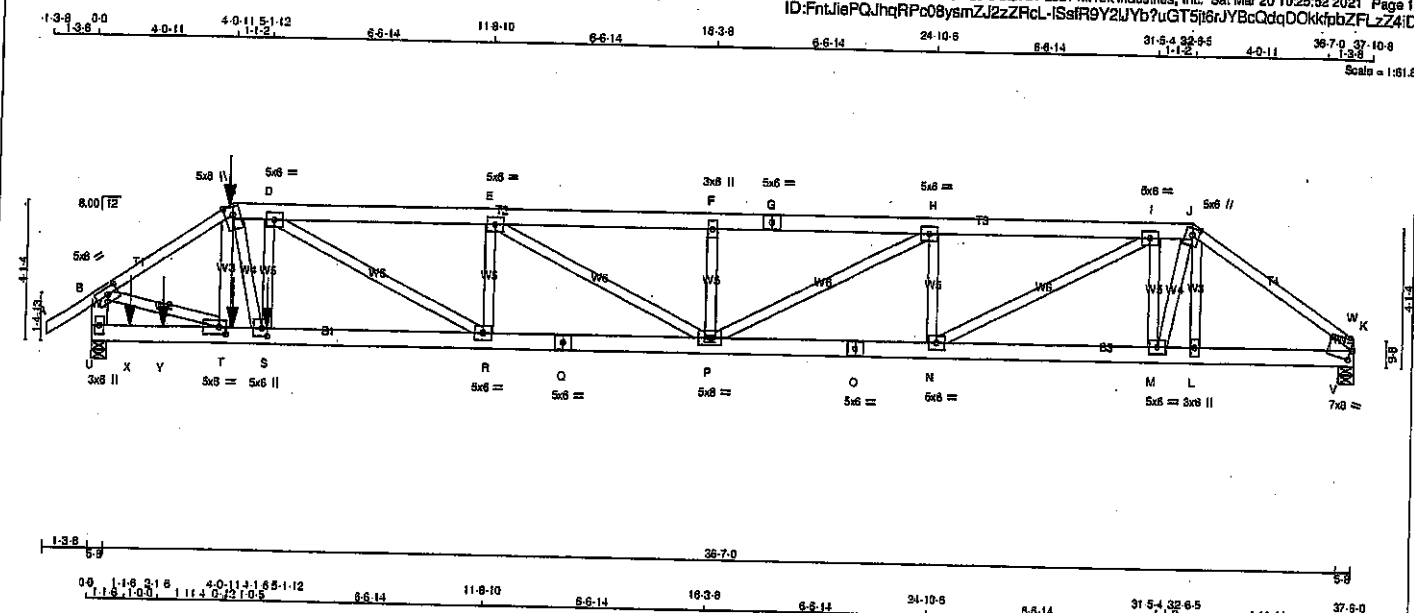
JOB NAME 417467	TRUSS NAME T1X	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID: FntJiePQJhqrPC08ysmZJ2zZrCL-IsatR9Y2JYb?uGT5j6rJYBcQdQOkfphZLZZ4ID

Scale = 1:61.8



CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - G	2x6	DRY	No.2
G - J	2x4	DRY	No.2
J - K	2x4	DRY	No.2
U - B	2x6	DRY	No.2
U - O	2x6	DRY	No.2
O - Q	2x6	DRY	No.2
Q - K	2x6	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE (81.0)
J-K	12	TOP
C-G	12	SIDE (61.0)
G-J	12	TOP
U-B	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
U-Q	12	SIDE (183.1)
Q-O	12	TOP
O-K	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	6	SIDE (861.9)
D-S	2	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG	HEEL WEDGE
JT	VERT 6195	DOWN 6195	0	5-8	5-8
U	HORIZ 2878	UPLIFT 2878	0	5-8	5-8
K					2x6 R

UNFACTORED REACTIONS

1ST CASE	MAX / MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
U	4373	2914 / 0 0 / 0 0 / 0 1439 / 0 0 / 0
K	1892	1250 / 0 0 / 0 0 / 0 642 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.29 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
MEMB.					MEMB.		
FR-TO		FROM TO			FR-TO		
A-B	0 / 35	-91.8 -91.8	0.07 (1)	10.00	T-C	-1262 / 0	0.11 (1)
B-C	-7825 / 0	-91.8 -91.8	0.40 (1)	3.29	L-J	-473 / 0	0.04 (1)
C-D	-7871 / 0	-91.8 -91.8	0.11 (1)	4.27	B-T	0 / 6587	0.58 (1)
D-E	-8869 / 0	-91.8 -91.8	0.31 (1)	3.88	S-D	-1085 / 0	0.09 (1)
E-F	-8571 / 0	-91.8 -91.8	0.26 (1)	4.00	C-S	0 / 5111	0.45 (1)
F-G	-8571 / 0	-91.8 -91.8	0.28 (1)	4.00	M-I	-2227 / 0	0.19 (1)
G-H	-8571 / 0	-91.8 -91.8	0.28 (1)	4.00	M-J	0 / 2729	0.24 (1)
H-I	-6835 / 0	-91.8 -91.8	0.20 (1)	4.43	N-I	0 / 3409	0.30 (1)
I-J	-3959 / 0	-91.8 -91.8	0.08 (1)	5.64	D-R	0 / 1257	0.11 (1)
J-W	-3824 / 0	-91.8 -91.8	0.32 (1)	4.52	N-H	-1541 / 0	0.13 (1)
W-K	-4828 / 0	-91.8 -91.8	0.31 (1)	4.02	R-E	-448 / 0	0.04 (1)
U-B	-6134 / 0	0.0 0.0	0.22 (1)	5.98	P-H	-454 / 0	0.18 (1)
U-X	0 / 0	-18.5 -18.5	0.04 (4)	10.00	P-F	-577 / 0	0.05 (1)
X-Y	0 / 0	-18.5 -18.5	0.04 (4)	10.00	V-W	0 / 1414	0.00 (1)
Y-T	0 / 0	-18.5 -18.5	0.04 (4)	10.00			
T-S	0 / 8413	-18.5 -18.5	0.50 (1)	10.00			
S-R	0 / 7889	-18.5 -18.5	0.60 (1)	10.00			
R-Q	0 / 8909	-18.5 -18.5	0.63 (1)	10.00			
Q-P	0 / 8988	-18.5 -18.5	0.63 (1)	10.00			
P-O	0 / 6935	-18.5 -18.5	0.50 (1)	10.00			
O-N	0 / 6935	-18.5 -18.5	0.50 (1)	10.00			
N-M	0 / 3964	-18.5 -18.5	0.29 (1)	10.00			
M-L	0 / 3177	-18.5 -18.5	0.33 (1)	10.00			
L-V	0 / 3158	-18.5 -18.5	0.43 (1)	10.00			
V-K	0 / 3158	-18.5 -18.5	0.43 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

LOC.	LOC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	4-0-11	-41	-41	FRONT	VERT	DEAD	---	C1
C	4-0-11	-128	-128	BACK	VERT	TOTAL	---	C1
G	4-0-11	-175	-175	FRONT	VERT	SNOW	---	C1
S	5-1-12	-2825	-2825	BACK	VERT	TOTAL	---	C1
T	4-1-6	-29	-29	BACK	VERT	TOTAL	---	C1
X	1-1-6	-29	-29	BACK	VERT	TOTAL	---	C1
Y	2-1-6	-29	-29	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.25")
CALCULATED VERT. DEFL.(LL) = U/999 (0.24")
ALLOWABLE DEFL.(TL) = L/360 (1.25")
CALCULATED VERT. DEFL.(TL) = U/999 (0.43")

CSI: TC=0.40/1.00 (B-C:1), BC=0.63/1.00 (P-R:1),
WB=0.58/1.00 (B-T:1), SS=0.38/1.00 (K-V:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1997 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (S) (INPUT = 0.90)
JSI METAL= 0.80 (Q) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2108225

JOB NAME 417467	TRUSS NAME T1X	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MTek Industries, Inc. Sat Mar 20 10:26:52 2021 Page 2
ID:FnlJiePQJhRPa08vsmZJ2zZRoL-ISsfR9Y2JYb7uGT5t6rJYBcQdgDCKkpbZFLzZ4ID

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	5.0	8.0	2.25 3.75
C	TTWW+m	MT20	5.0	8.0	Edge
D, E, H, I					
D	TMWW-t	MT20	5.0	6.0	
F	TMW+w	MT20	3.0	6.0	
G	TS-t	MT20	5.0	6.0	
J	TTWW+m	MT20	5.0	8.0	2.50 2.50
K	TMBH1-m	MT20	7.0	8.0	3.50 0.75
L	BMW+w	MT20	3.0	6.0	
M, N, R					
M	BMWW-t	MT20	5.0	6.0	
O	BS-t	MT20	5.0	8.0	
P	BMWWW-t	MT20	5.0	8.0	
Q	BS-t	MT20	5.0	6.0	
S	BMWW-t	MT20	5.0	6.0	2.75 2.00
T	BMWW-t	MT20	5.0	8.0	2.50 2.25
U	BMV1+p	MT20	3.0	8.0	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2108225 *in*

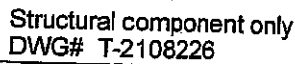
Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 10:25:53 2021 Page 1
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1.3-8 0.0
1.3-9 5-8-10 5-8-10 5-1-14 10-8-9 15-9-3 5-0-10 30-9-13 5-0-10 25-10-7 5-1-14 31-0-5 5-6-11 38-7-0 37-10-6
1.3-8
Scale = 1:51.4



Notes: (1)
Lateral braces to be a minimum of 2X4 SPF #2.

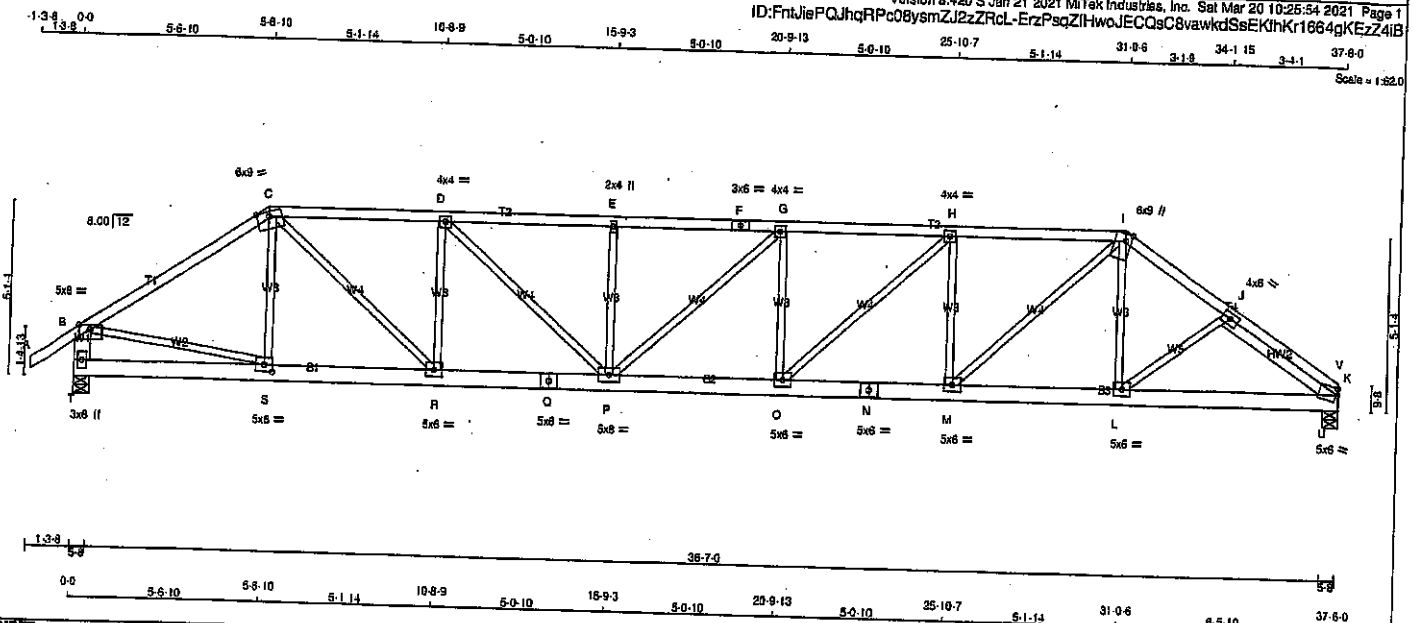
1. *Journal of the American Medical Association*, 1997; 277: 1033-1036.



JOB NAME 417467	TRUSS NAME T2X	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 Mittek Industries, Inc. Sat Mar 20 10:25:54 2021 Page 1
ID: FntJiePQJhqrPC08ysmZJ2zZrcl-ErzPsgZiHwJECQsC8vawkdSsEKthKr1664gKEzZ4iB

Scale = 1:32.0



LUMBER	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES				
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF

REINFORCING MEMBERS	SIZE	DRY	LUMBER	DESCR.
HW2	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	Edge	
C TTWW-m	MT20	6.0	9.0	Edge	
D, G, H					
D TMVW-t	MT20	4.0	4.0		
E TMVW-w	MT20	2.0	4.0		
F TS-t	MT20	3.0	6.0		
I TTWW+m	MT20	6.0	9.0	Edge	2.00
J TMVW-t	MT20	4.0	6.0		
K TMBW-t-m	MT20	5.0	6.0	2.25	0.50
L, M, O, R					
L BMVW-t	MT20	5.0	6.0		
N BS-t	MT20	5.0	6.0		
P BMVWV-t	MT20	5.0	8.0		
Q BS-t	MT20	5.0	6.0		
S BMVW-t	MT20	5.0	8.0	2.50	2.75
T BMV1-p	MT20	3.0	6.0		

Edges - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	GROSS REACTION		
K	HORZ	GROSS REACTION		
T	VERT	GROSS REACTION		
T	HORZ	GROSS REACTION		

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED				
K	SNOW				
T	LIVE				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORIZ. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO						FR-TO			
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	S-C	-255 / 0	0.09 (1)	
B-C	-2515 / 0	-91.8	-91.8	0.74 (1)	3.61	L-I	0 / 174	0.04 (4)	
C-D	-3395 / 0	-91.8	-91.8	0.58 (1)	3.33	L-J	-55 / 14	0.02 (1)	
D-E	-4020 / 0	-91.8	-91.8	0.66 (1)	3.00	B-S	0 / 2122	0.48 (1)	
E-F	-4020 / 0	-91.8	-91.8	0.55 (1)	3.14	M-I	0 / 1612	0.36 (1)	
F-G	-4020 / 0	-91.8	-91.8	0.55 (1)	3.14	C-R	0 / 1776	0.40 (1)	
G-H	-4020 / 0	-91.8	-91.8	0.60 (1)	3.25	R-D	-1105 / 0	0.40 (1)	
H-I	-3554 / 0	-91.8	-91.8	0.87 (1)	2.97	M-H	-1012 / 0	0.36 (1)	
I-J	-2855 / 0	-91.8	-91.8	0.21 (1)	3.96	O-H	0 / 724	0.18 (1)	
J-V	-1541 / 0	-91.8	-91.8	0.12 (1)	5.19	D-P	0 / 856	0.19 (1)	
V-K	-2313 / 0	-91.8	-91.8	0.12 (1)	4.41	O-G	-384 / 0	0.14 (1)	
T-B	-2147 / 0	0.0	0.0	0.14 (1)	6.94	P-E	-447 / 0	0.16 (1)	
T-S	0 / 0	-18.5	-18.5	0.08 (4)	10.00	P-G	-85 / 0	0.07 (1)	
S-R	0 / 2084	-18.5	-18.5	0.27 (1)	10.00	U-V	0 / 920	0.00 (1)	
R-Q	0 / 3395	-18.5	-18.5	0.46 (1)	10.00	U-J	-1499 / 0	0.32 (1)	
Q-P	0 / 3395	-18.5	-18.5	0.46 (1)	10.00				
P-O	0 / 4083	-18.5	-18.5	0.54 (1)	10.00				
O-N	0 / 3554	-18.5	-18.5	0.47 (1)	10.00				
N-M	0 / 3554	-18.5	-18.5	0.47 (1)	10.00				
M-L	0 / 2364	-18.5	-18.5	0.32 (1)	10.00				
L-U	0 / 2403	-18.5	-18.5	0.34 (1)	10.00				
U-K	0 / 1285	-18.5	-18.5	0.19 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. G/G

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.25")
CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
ALLOWABLE DEFL.(TL) = L/360 (1.25")
CALCULATED VERT. DEFL.(TL) = L/999 (0.43")

CSI: TC=0.74/1.00 (B-C:1), BC=0.54/1.00 (O-P:1), WB=0.48/1.00 (B-S:1), SS=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

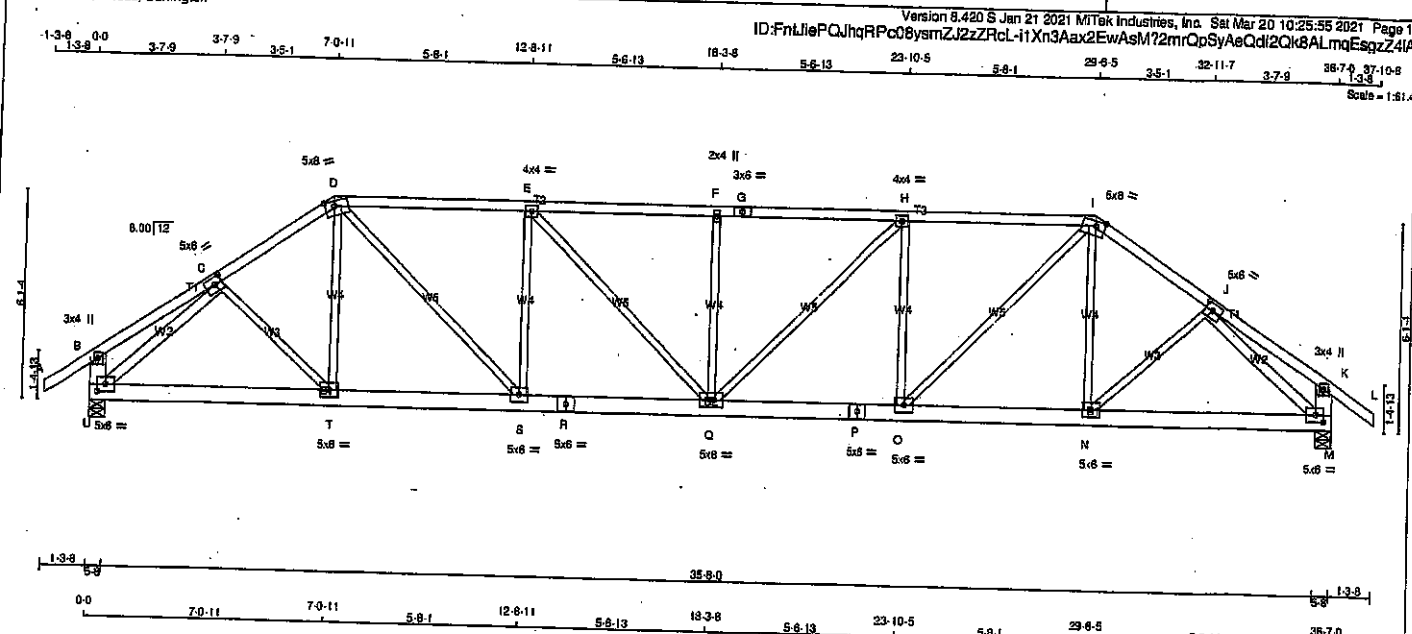
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (I) (INPUT = 0.90)
JSI METAL = 0.66 (N) (INPUT = 1.00)



Structural component only
DWG# T-2108227

JOB NAME 417467	TRUSS NAME T3	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 S Jan 21 2021 M/Tek Industries, Inc. Sat Mar 20 10:25:55 2021 Page 1	



CHORDS	SIZE	DRY	NO.2
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
U - C	2x4	DRY	No.2
J - M	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMV+p	MT20	3.0	4.0	
C TMWW-1	MT20	5.0	6.0	2.50 2.75
D TTWW-m	MT20	5.0	8.0	1.75 3.25
E TMWW-t	MT20	4.0	4.0	
F TMW+w	MT20	2.0	4.0	
G TS-1	MT20	3.0	6.0	
H TMWW-1	MT20	4.0	4.0	
I TTWW-m	MT20	5.0	8.0	1.75 3.25
J TMWW-1	MT20	5.0	6.0	2.50 2.75
K TMV+p	MT20	3.0	4.0	
M BMWW-1	MT20	5.0	6.0	2.50 2.75
N, O, S, T				
N BMWW-1	MT20	5.0	6.0	
P BS-1	MT20	5.0	6.0	
Q BMWWWW-1	MT20	5.0	8.0	
R BS-1	MT20	5.0	6.0	
U BMWW-1	MT20	5.0	6.0	2.50 2.75

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
U	2143	0	0	5-8
M	2143	0	0	5-8

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
U	1513	1006 / 0
M	1513	1006 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	UNBRAC	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX
FR-TO														
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	C-T	0 / 128	0.03 (4)						
B-C	0 / 19	-91.8	-91.8	0.17 (1)	10.00	T-D	0 / 94	0.03 (4)						
C-D	-2454 / 0	-91.8	-91.8	0.25 (1)	4.17	N-I	0 / 94	0.03 (4)						
D-E	-2976 / 0	-91.8	-91.8	0.88 (1)	3.44	N-J	0 / 128	0.03 (4)						
E-F	-3249 / 0	-91.8	-91.8	0.89 (1)	3.27	U-C	-2862 / 0	0.67 (1)						
F-G	-3249 / 0	-91.8	-91.8	0.89 (1)	3.27	J-M	-2862 / 0	0.67 (1)						
G-H	-3249 / 0	-91.8	-91.8	0.89 (1)	3.44	D-S	0 / 1352	0.30 (1)						
H-I	-2976 / 0	-91.8	-91.8	0.89 (1)	3.27	O-I	0 / 1352	0.30 (1)						
I-J	-2454 / 0	-91.8	-91.8	0.25 (1)	4.17	O-H	-883 / 0	0.47 (1)						
J-K	0 / 19	-91.8	-91.8	0.17 (1)	10.00	S-E	-883 / 0	0.47 (1)						
K-L	0 / 35	-91.8	-91.8	0.12 (1)	10.00	C-H	0 / 392	0.09 (1)						
U-B	-253 / 0	0.0	0.0	0.02 (1)	7.81	E-Q	0 / 392	0.09 (1)						
M-K	-253 / 0	0.0	0.0	0.02 (1)	7.81	Q-F	-469 / 0	0.26 (1)						
U-T	0 / 1930	-18.5	-18.5	0.28 (1)	10.00									
T-S	0 / 2023	-18.5	-18.5	0.29 (1)	10.00									
S-R	0 / 2976	-18.5	-18.5	0.40 (1)	10.00									
R-Q	0 / 2976	-18.5	-18.5	0.40 (1)	10.00									
Q-P	0 / 2976	-18.5	-18.5	0.40 (1)	10.00									
P-O	0 / 2976	-18.5	-18.5	0.40 (1)	10.00									
O-N	0 / 2023	-18.5	-18.5	0.29 (1)	10.00									
N-M	0 / 1930	-18.5	-18.5	0.28 (1)	10.00									

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.23")

CSI: TO=0.69/1.00 (F-H:1), BC=0.40/1.00 (C-Q:1), WB=0.67/1.00 (J-M:1), SS=0.24/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)

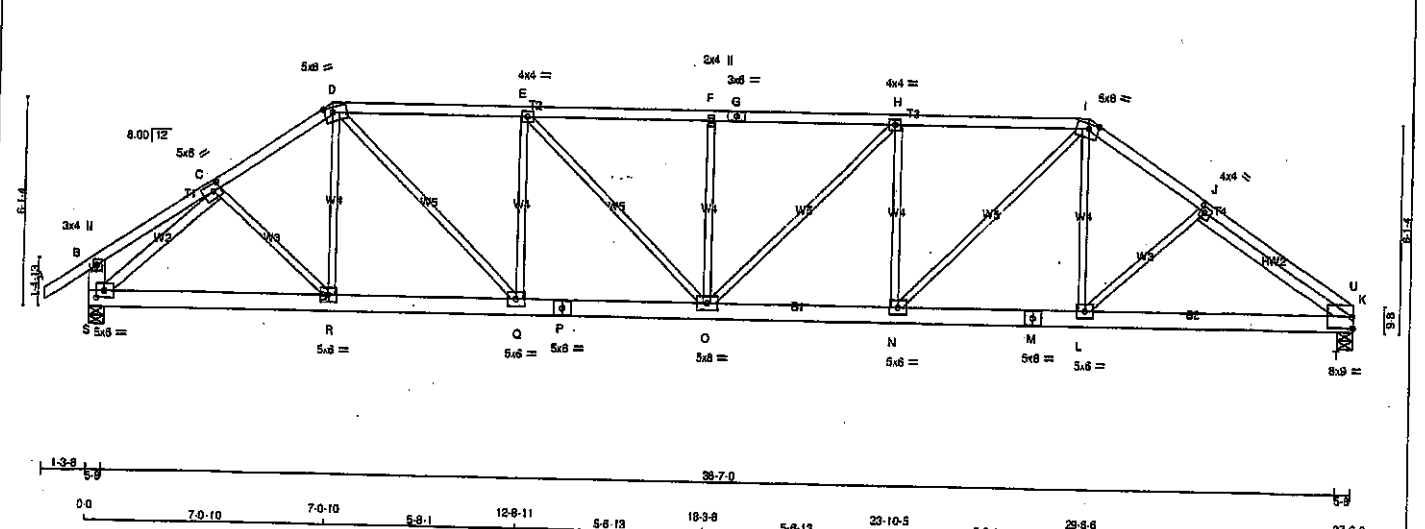
JSI METAL= 0.60 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108228

JOB NAME 417467	TRUSS NAME T3X	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 10:25:58 2021 Page 1
 ID: FntJlePQJhQPC08ysmZJ2zZPCL-AD5AHWbZpX21UVaEKZx279op12s9B6KaQZn06zZ419
 Scale = 1:31.5



LUMBER			
N. L. G. A. RULES	SIZE	DRY	NO.2
CHORDS			
A - D	2x4	DRY	No.2
D - I	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
S - B	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
M - K	2x6	DRY	No.2
REINFORCING MEMBERS			
HW2	2x4	DRY	No.2
ALL WEBS			
EXCEPT	2x3	DRY	No.2
S - C	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW-t	MT20	5.0	6.0	2.50 2.50
D	TTWV-m	MT20	5.0	8.0	1.75 3.00
E	TMVW-t	MT20	4.0	4.0	
F	TMVW-w	MT20	2.0	4.0	
G	TS-t	MT20	3.0	6.0	
H	TMVW-t	MT20	4.0	4.0	
I	TTWV-m	MT20	5.0	8.0	1.75 3.50
J	TMVW-t	MT20	4.0	4.0	2.00 1.75
K	TMVW-t	MT20	8.0	9.0	Edge
L, N, Q, R					
L	BMVW-t	MT20	5.0	6.0	
M	BS-t	MT20	5.0	6.0	
O	BMVW-t	MT20	5.0	6.0	
P	BS-t	MT20	5.0	6.0	
S	BMVW-t	MT20	5.0	6.0	2.50 2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
 1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
K	VERT 2067	DOWN 2067	0	5-8	5-8
S	HORZ 2193	UPLIFT 0	0	5-8	5-8

UNFACTORED REACTIONS							
JT	1ST CASE COMBINED	MAX SNOW	MIN LIVE	COMPONENT REACTIONS	PERM LIVE	WIND	DEAD
K	1462	959 / 0	0 / 0	0 / 0	0 / 0	502 / 0	0 / 0
S	1549	1030 / 0	0 / 0	0 / 0	0 / 0	519 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, S

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.17 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO				FR-TO			
A-B	0 / 35	-91.8	-91.8 0.12 (1)	C-R	0 / 138	0.03 (4)	
B-C	0 / 19	-91.8	-91.8 0.16 (1)	D-O	0 / 91	0.03 (4)	
C-D	-2529 / 0	-91.8	-91.8 0.26 (1)	E-I	0 / 289	0.07 (1)	
D-E	-3088 / 0	-91.8	-91.8 0.87 (1)	F-J	-2735 / 0	0.68 (1)	
E-F	-3410 / 0	-91.8	-91.8 0.71 (1)	G-K	0 / 1275	0.29 (1)	
F-G	-3410 / 0	-91.8	-91.8 0.71 (1)	H-L	0 / 1423	0.32 (1)	
G-H	-3410 / 0	-91.8	-91.8 0.71 (1)	I-M	-812 / 0	0.44 (1)	
H-I	-3185 / 0	-91.8	-91.8 0.99 (1)	J-N	-913 / 0	0.50 (1)	
I-J	-2768 / 0	-91.8	-91.8 0.27 (1)	O-S	0 / 323	0.07 (1)	
J-U	-1981 / 0	-91.8	-91.8 0.22 (1)	P-Q	0 / 463	0.10 (1)	
U-K	-2423 / 0	-91.8	-91.8 0.20 (1)	Q-R	-469 / 0	0.26 (1)	
S-B	-254 / 0	0.0	0.0 0.02 (1)	L-J	-197 / 0	0.08 (1)	
				T-U	0 / 844	0.00 (1)	
				T-J	-1328 / 0	0.52 (1)	
S-R	0 / 1983	-18.5	-18.5 0.29 (1)				
R-Q	0 / 2086	-18.5	-18.5 0.29 (1)				
Q-P	0 / 3088	-18.5	-18.5 0.41 (1)				
P-O	0 / 3088	-18.5	-18.5 0.41 (1)				
O-N	0 / 3185	-18.5	-18.5 0.43 (1)				
N-M	0 / 2287	-18.5	-18.5 0.33 (1)				
M-L	0 / 2287	-18.5	-18.5 0.33 (1)				
L-T	0 / 2424	-18.5	-18.5 0.36 (1)				
T-K	0 / 1400	-18.5	-18.5 0.18 (1)				

TOTAL WEIGHT = 132 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.25")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
 ALLOWABLE DEFL.(TL) = L/360 (1.25")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.32")

CS: TC=0.71/1.00 (F-H:1), BC=0.43/1.00 (N-O:1), WB=0.68/1.00 (C-S:1), SS=0.24/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747	788	1987

PLATE PLACEMENT TOL = 0.250 inches

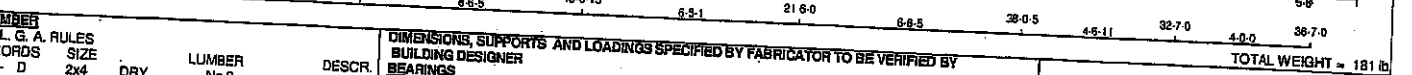
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (D) (INPUT = 0.90)
 JSI METAL = 0.82 (C) (INPUT = 1.00)



Structural component only
 DWG# T-2108229

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ID:FntJiePQJhRcP08ysmZJ2Z2ZrCL-ePfYUsoBZrAu5I9RIGTHYNFy?ROWuET04JKZzZ4H
-13.8 0.0 4.0-0 4.8-10 8.6-10 8.6-5 15.0-15 8.6-1 21.6-0 8.6-5 28.0-5 4.8-11 32.7-0 4.0-0 38.7-0 37.10-8 1.3-8
Scale = 1:251.4



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER		6-3		6-1		6-0		6-5	
DESCR.		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
SPF		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
SPF	L	2143	0	2143	0	0	5-8	5-8	
SPF	S	2143	0	2143	0	0	5-8	5-8	
UNFACTORED REACTIONS		1ST LCASE		MAX. MIN. COMPONENT REACTIONS					
JT		COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
SPF	L	1513	1008 / 0	0 / 0	0 / 0	0 / 0	507 / 0	0 / 0	
SPF	S	1513	1008 / 0	0 / 0	0 / 0	0 / 0	507 / 0	0 / 0	
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, S									

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-P.
END VERTICAL(S) MUST BE SHEATHED OR MAX. SPACING = 3.40 FT.

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.($L/16$) = $L/360$ (1.22")
CALCULATED VERT. DEFL.($L/1$) = $L/989$ (0.13")
ALLOWABLE DEFL.($T/16$) = $L/360$ (1.22")
CALCULATED VERT. DEFL.($T/1$) = $L/989$ (0.24")

SK: $TC=0.81/1.00$ (G-H:1), $BC=0.37/1.00$ (O-P:1),
 $NB=0.82/1.00$ (Q-S:1), $SB=0.28/1.00$ (G-H:1)

DL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

UTOSOLVEHEELS OFF

RUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
RUSS MANUFACTURING PLANT

IL VALUES

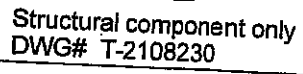
DATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
20	850 371	1747 788	1987 1873

PLACEMENT TOL. = 0.250 inches

ROTATION TOL. = 5.0 Deg.

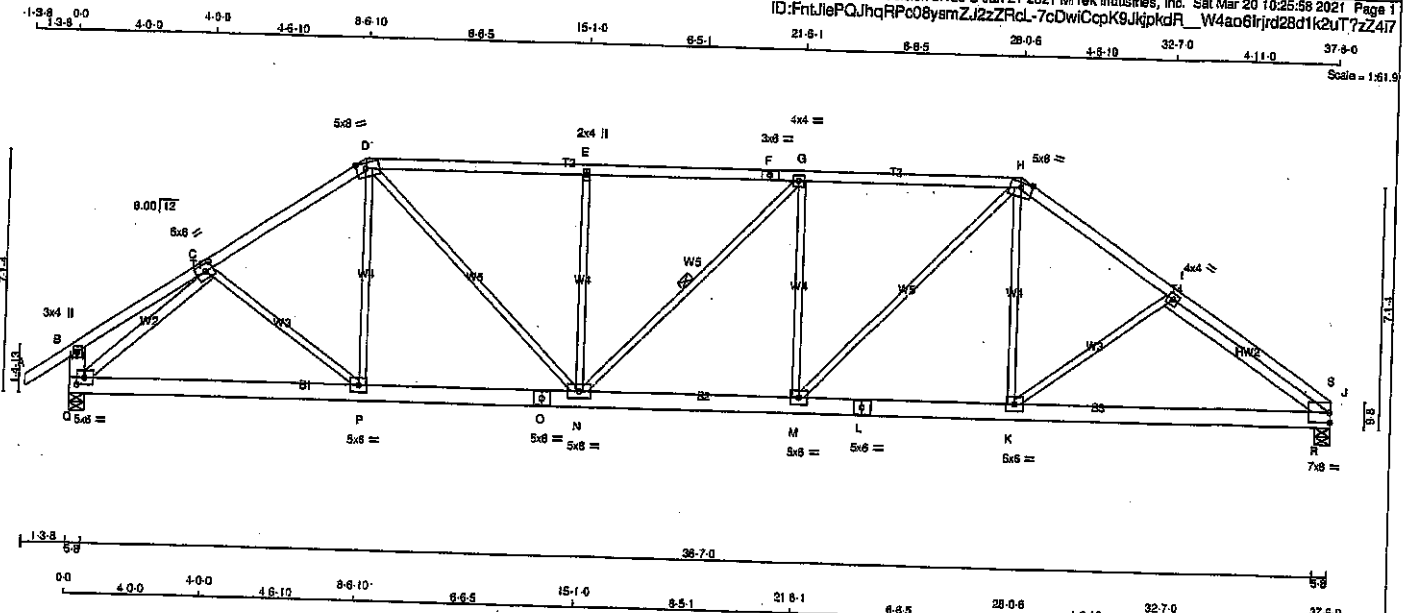
GRIP = 0.90 (C) (INPUT = 0.90)

METAL = 0.60 (C) (INPUT = 1.00)



JOB NAME 417467	TRUSS NAME T4X	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 ID: FntJlePQJhQRPc08yamZJ2zZPdL-7cDwiCpK9JkpkdR_W4ao6lrjd28d1k2uT7zZ417
 Scale = 1/8" = 1'-0"



LUMBER

N.L.C.A. RULES	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
Q - B	2x6	DRY	No.2
Q - O	2x6	DRY	No.2
O - L	2x6	DRY	No.2
L - J	2x6	DRY	No.2

REINFORCING MEMBERS
 HW2 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT
 Q - C 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	8.0	2.50	2.75
D	TTWW-m	MT20	5.0	8.0	2.00	3.25
E	TMWW-w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	8.0		Edge
I	TMWW-t	MT20	4.0	4.0		
J	TMWW-t	MT20	7.0	8.0		Edge
K, M, P						
K	BMWW-t	MT20	5.0	8.0		
L	BS-t	MT20	5.0	8.0		
N	BMWW-t	MT20	5.0	8.0		
O	BS-t	MT20	5.0	8.0		
Q	BMWW-t	MT20	5.0	8.0	2.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
 1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UPLIFT		
JT	2067	0	2067	0	5-8	5-8
Q	2193	0	2193	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW			
JT	1482	969 / 0	0 / 0	502 / 0	0 / 0
Q	1549	1030 / 0	0 / 0	519 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, Q

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.29 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD		MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
		FROM	TO				
FR-TO					FR-TO		
A-B	0 / 35	-91.8	-91.8	10.00	P-D	0 / 155	0.05 (4)
B-C	0 / 27	-91.8	-91.8	10.00	D-N	0 / 1111	0.25 (1)
C-D	-2489 / 0	-91.8	-91.8	10.00	N-E	-843 / 0	0.53 (1)
D-E	-2825 / 0	-91.8	-91.8	10.00	E-G	-76 / 0	0.05 (1)
E-F	-2825 / 0	-91.8	-91.8	10.00	M-G	-591 / 0	0.48 (1)
F-G	-2825 / 0	-91.8	-91.8	10.00	M-H	0 / 975	0.22 (1)
G-H	-2878 / 0	-91.8	-91.8	10.00	K-H	0 / 355	0.08 (1)
H-I	-2863 / 0	-91.8	-91.8	10.00	C-P	0 / 69	0.02 (4)
I-S	-1735 / 0	-91.8	-91.8	10.00	Q-C	-2760 / 0	0.84 (1)
S-J	-2530 / 0	-91.8	-91.8	10.00	K-I	-299 / 0	0.18 (1)
Q-B	-255 / 0	0.0	0.0	10.00	R-S	0 / 901	0.00 (1)
					R-I	-1257 / 0	0.59 (1)

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.8 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBCC 2015, ABC 2019
 - PART 9 OF NBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.25")
 CALCULATED VERT. DEFL.(LL) = 1/898 (0.13")
 ALLOWABLE DEFL.(TL) = L/360 (1.25")
 CALCULATED VERT. DEFL.(TL) = 1/999 (0.25")

CSI: TC=0.84/1.00 (G-H:1), BC=0.39/1.00 (M-N:1), WB=0.84/1.00 (C-Q:1), SS=0.29/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

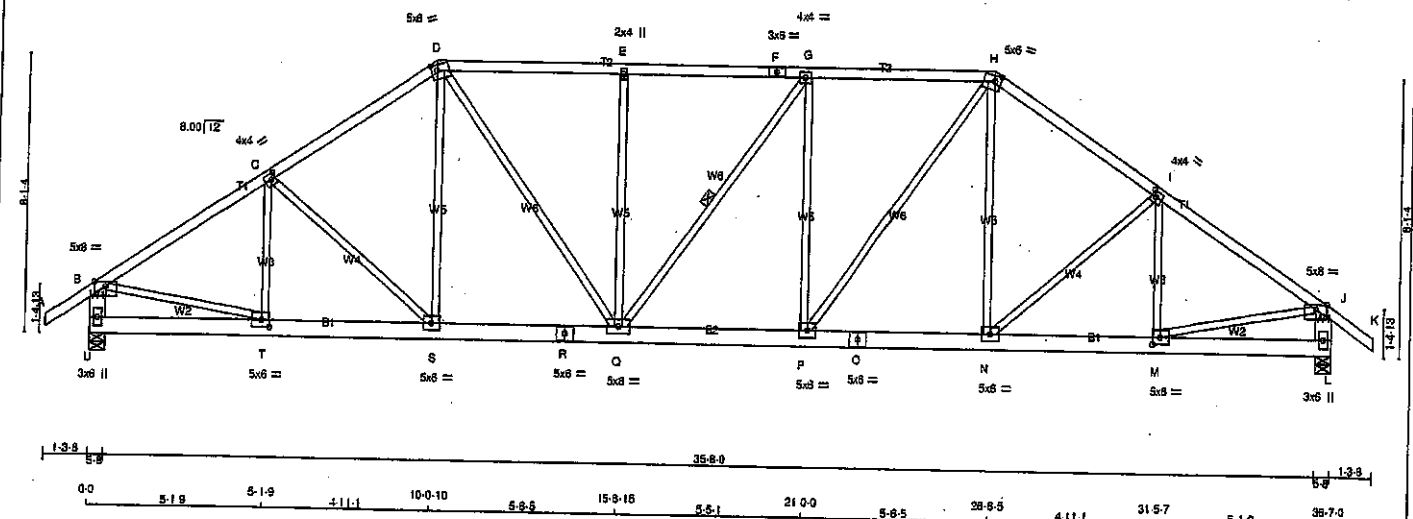
JSI GRIP = 0.88 (H) (INPUT = 0.90)
 JSI METAL = 0.82 (C) (INPUT = 1.00)



Structural component only
 DWG# T-2108231

JOB NAME 417467	TRUSS NAME T5	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID: FntJiePQJhQRPc08ymZJ2zRcl-bonIvYdR5SRblZp?hVidLOvF5IMYRmGCoR?RzZ4i6
Scale = 1:51.4



CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
U - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - O	2x6	DRY	No.2
O - L	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY, SEASONED LUMBER.

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge
C	TMVW-t	MT20	4.0	4.0	2.00 1.50
D	TTWW-m	MT20	5.0	8.0	2.00 2.00
E	TMVW-w	MT20	2.0	4.0	
F	TS-t	MT20	3.0	8.0	
G	TMVW-t	MT20	4.0	4.0	
H	TTWW-m	MT20	5.0	8.0	2.00 2.00
I	TMVW-t	MT20	4.0	4.0	2.00 1.50
J	TMVW-p	MT20	5.0	8.0	Edge
L	BMV-t	MT20	3.0	8.0	
M	BMVW-t	MT20	5.0	6.0	2.50 2.75
N, P, S					
N	BMVW-t	MT20	5.0	6.0	
O	SS-t	MT20	5.0	6.0	
Q	BMVW-t	MT20	5.0	8.0	
R	SS-t	MT20	5.0	6.0	
T	BMVW-t	MT20	5.0	6.0	2.50 2.75
U	BMV-t	MT20	3.0	8.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
U	2143	0	2143	0
L	2143	0	2143	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1513	1006 / 0	0 / 0	0 / 0	0 / 0	507 / 0	0 / 0
L	1513	1006 / 0	0 / 0	0 / 0	0 / 0	507 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO					FR-TO		
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	T-C	-332 / 0	0.11 (1)
B-C	-2457 / 0	-91.8	-91.8 0.40 (1)	4.05	C-S	-210 / 0	0.17 (1)
C-D	-2331 / 0	-91.8	-91.8 0.38 (1)	4.15	S-D	0 / 264	0.06 (1)
D-E	-2368 / 0	-91.8	-91.8 0.40 (1)	4.09	D-Q	0 / 778	0.18 (1)
E-F	-2368 / 0	-91.8	-91.8 0.37 (1)	4.13	Q-E	-542 / 0	0.65 (1)
F-G	-2368 / 0	-91.8	-91.8 0.40 (1)	4.09	P-G	-539 / 0	0.84 (1)
G-H	-2372 / 0	-91.8	-91.8 0.38 (1)	4.16	P-H	0 / 788	0.18 (1)
H-I	-2330 / 0	-91.8	-91.8 0.40 (1)	4.05	N-H	0 / 257	0.06 (1)
I-J	-2457 / 0	-91.8	-91.8 0.40 (1)	10.00	N-I	-212 / 0	0.17 (1)
J-K	0 / 35	-91.8	-91.8 0.12 (1)	7.02	M-I	-329 / 0	0.11 (1)
U-B	-2089 / 0	0.0	0.0 0.14 (1)	7.02	B-T	0 / 2107	0.47 (1)
L-J	-2090 / 0	0.0	0.0 0.14 (1)	7.02	M-J	0 / 2108	0.47 (1)
U-T	0 / 0	-18.5	-18.5 0.06 (4)	10.00			
T-S	0 / 2069	-18.5	-18.5 0.28 (1)	10.00			
S-R	0 / 1915	-18.5	-18.5 0.27 (1)	10.00			
R-Q	0 / 1915	-18.5	-18.5 0.27 (1)	10.00			
Q-P	0 / 2372	-18.5	-18.5 0.32 (1)	10.00			
P-O	0 / 1914	-18.5	-18.5 0.26 (1)	10.00			
O-N	0 / 1914	-18.5	-18.5 0.26 (1)	10.00			
N-M	0 / 2069	-18.5	-18.5 0.28 (1)	10.00			
M-L	0 / 0	-18.5	-18.5 0.06 (4)	10.00			

TOTAL WEIGHT = 190 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.22")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.11")
ALLOWABLE DEFL. (TL) = $L/360$ (1.22")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.21")

CSI: TC=0.40/1.00 (G-H:1), BC=0.32/1.00 (P-Q:1), WB=0.65/1.00 (E-Q:1), SS=0.24/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

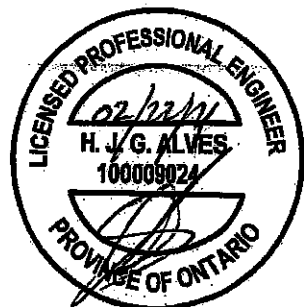
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

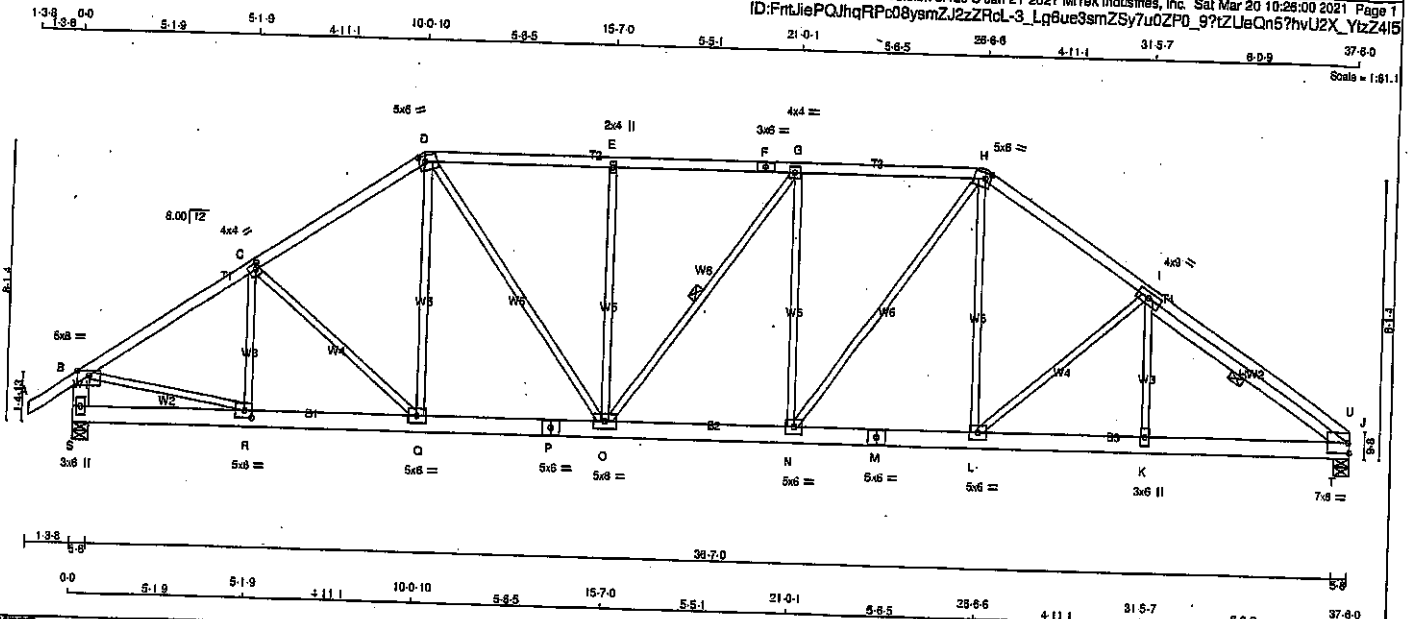
JSI GRIP = 0.87 (M) (INPUT = 0.90)
JSI METAL = 0.47 (M) (INPUT = 1.00)



Structural component only
DWG# T-2108232

JOB NAME 417467	TRUSS NAME T5X	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID: FntJiePQJhRPa08ysmZJ2zZRL-3_Lg8ue3smZSy7u0ZP0_9?zUeQn57hvU2X_YtzZ415



TOTAL WEIGHT = 193 lb

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - L	2x6	DRY	No.2
L - M	2x6	DRY	No.2
M - N	2x6	DRY	No.2
N - O	2x6	DRY	No.2
O - P	2x6	DRY	No.2
P - Q	2x6	DRY	No.2
Q - R	2x6	DRY	No.2
R - S	2x6	DRY	No.2
S - T	2x6	DRY	No.2
T - U	2x6	DRY	No.2
U - V	2x6	DRY	No.2
V - W	2x6	DRY	No.2
W - X	2x6	DRY	No.2
X - Y	2x6	DRY	No.2
Y - Z	2x6	DRY	No.2

REINFORCING MEMBERS	SIZE	LUMBER	DESCR.
HW2	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	8.0	Edge
C	TMWV-w	MT20	4.0	4.0	2.00 1.50
D	TMWV-m	MT20	5.0	8.0	2.00 2.00
E	TMWV-w	MT20	2.0	4.0	
F	TS-t	MT20	3.0	8.0	
G	TMWV-t	MT20	4.0	4.0	
H	TMWV-m	MT20	5.0	8.0	2.00 2.00
I	TMWV-w	MT20	4.0	4.0	
J	TMWV-t	MT20	7.0	8.0	Edge
K	TMWV-w	MT20	3.0	8.0	
L, N, O					
L	BMWV-t	MT20	5.0	8.0	
M	BS-t	MT20	5.0	8.0	
O	BMWV-w	MT20	5.0	8.0	
P	BS-t	MT20	5.0	8.0	
R	BMWV-t	MT20	5.0	8.0	2.50 2.75
S	BMV-t-p	MT20	3.0	8.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	UPLIFT	IN-SX
J	2057	0	5-8
S	2193	0	5-8

UNFACTORED REACTIONS

1ST LOSE	MAX/MIN COMPONENT REACTIONS	PERM LIVE	WIND	DEAD	SOIL
JT COMBINED	SNOW	LIVE			
J	1482	959 / 0	0 / 0	0 / 0	502 / 0
S	1549	1030 / 0	0 / 0	0 / 0	519 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS J, S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-O, I-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	W E B S	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO								
A-B	0 / 35							
B-C	-2527 / 0	-91.8	-91.8	0.12 (1)	10.00	R-C	-344 / 0	0.11 (1)
C-D	-2410 / 0	-91.8	-91.8	0.40 (1)	4.00	C-Q	-199 / 0	0.16 (1)
D-E	-2489 / 0	-91.8	-91.8	0.38 (1)	4.09	Q-D	0 / 258	0.06 (1)
E-F	-2489 / 0	-91.8	-91.8	0.41 (1)	4.01	D-O	0 / 839	0.19 (1)
F-G	-2489 / 0	-91.8	-91.8	0.38 (1)	4.05	O-E	-542 / 0	0.65 (1)
G-H	-2510 / 0	-91.8	-91.8	0.38 (1)	4.05	O-G	-71 / 0	0.04 (1)
H-I	-2533 / 0	-91.8	-91.8	0.41 (1)	3.98	N-G	-487 / 0	0.58 (1)
I-U	-1741 / 0	-91.8	-91.8	0.38 (1)	4.02	N-H	0 / 729	0.16 (1)
U-J	-2335 / 0	-91.8	-91.8	0.34 (1)	4.74	L-H	0 / 418	0.09 (1)
S-B	-2139 / 0	-91.8	-91.8	0.26 (1)	4.25	L-I	-473 / 0	0.38 (1)
		0.0	0.0	0.14 (1)	6.98	K-I	0 / 118	0.04 (4)
						B-R	0 / 2167	0.49 (1)
S-R	0 / 0	-18.5	-18.5	0.06 (4)	10.00	T-U	0 / 606	0.00 (1)
R-Q	0 / 2127	-18.5	-18.5	0.29 (1)	10.00	T-I	-1246 / 0	0.34 (1)
Q-P	0 / 1981	-18.5	-18.5	0.27 (1)	10.00			
P-O	0 / 1981	-18.5	-18.5	0.27 (1)	10.00			
O-N	0 / 2510	-18.5	-18.5	0.27 (1)	10.00			
N-M	0 / 2068	-18.5	-18.5	0.33 (1)	10.00			
M-L	0 / 2068	-18.5	-18.5	0.27 (1)	10.00			
L-K	0 / 2433	-18.5	-18.5	0.31 (1)	10.00			
K-T	0 / 2434	-18.5	-18.5	0.36 (1)	10.00			
T-J	0 / 1454	-18.5	-18.5	0.23 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.25")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.25")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.41/1.00 (G-H:1), BC=0.36/1.00 (K-T:1), WB=0.65/1.00 (O-C:1), SSI=0.24/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

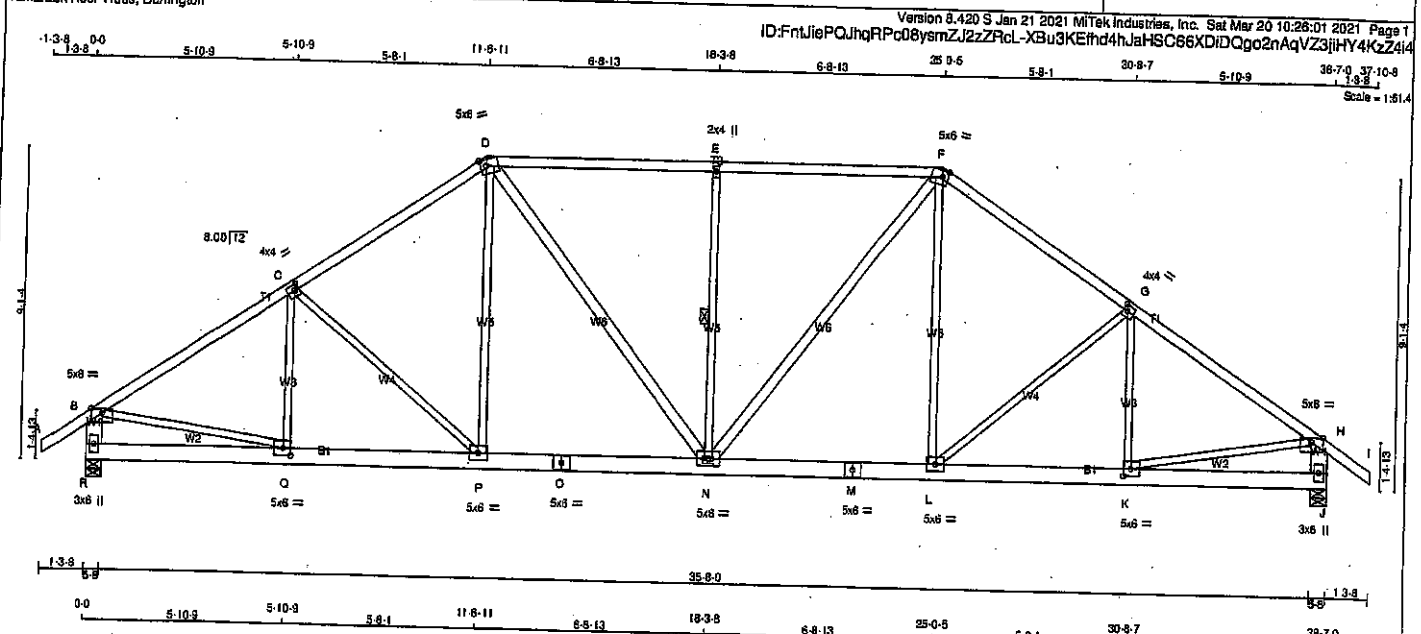
JSI GRIP=0.89 (R) (INPUT = 0.90)

JSI METAL=0.48 (R) (INPUT = 1.00)



Structural component only
DWG# T-2108233

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417467	T6	10	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington					



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
R - B	2x6	DRY	No.2	SPF	
J - H	2x6	DRY	No.2	SPF	
R - O	2x6	DRY	No.2	SPF	
O - M	2x6	DRY	No.2	SPF	
M - J	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
D - N	2x4	DRY	No.2	SPF	
N - F	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	Edge	
C	TMW-w	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	5.0	6.0	2.25	2.00
E	TMW-w	MT20	2.0	4.0		
F	TTW-m	MT20	5.0	6.0	2.25	2.00
G	TMW-w	MT20	4.0	4.0	2.00	1.50
H	TMW-p	MT20	5.0	8.0	Edge	
J	BMV1-p	MT20	3.0	6.0		
K	BMW-w	MT20	5.0	6.0	2.50	2.75
L	BMW-w	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMW-w	MT20	5.0	8.0		
O	BS-t	MT20	5.0	6.0		
P	BMW-w	MT20	5.0	6.0		
Q	BMW-w	MT20	5.0	6.0	2.50	2.75
R	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UPLIFT	IN-SX
R	2143	0	2143	0
J	2143	0	2143	0

UNFACTORED REACTIONS	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM. LIVE
R	1513	1006	0	0
J	1513	1006	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	UNBRACED LENGTH FR-TO	W E B S	MEMB.	MAX. FACTORED FORCE (LBS)	MAX
FR-TO	A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	Q-C	-262 / 5	0.10 (1)	
	B-C	-2486 / 0	-91.8	-91.8	0.53 (1)	3.88	C-P	-346 / 0	0.39 (1)	
	C-D	-2242 / 0	-91.8	-91.8	0.49 (1)	4.09	P-D	0 / 355	0.08 (1)	
	D-E	-2176 / 0	-91.8	-91.8	0.83 (1)	3.89	D-N	0 / 553	0.08 (1)	
	E-F	-2242 / 0	-91.8	-91.8	0.83 (1)	3.89	N-E	-759 / 0	0.39 (1)	
	F-G	-2242 / 0	-91.8	-91.8	0.49 (1)	4.09	N-F	0 / 553	0.09 (1)	
	G-H	-2486 / 0	-91.8	-91.8	0.53 (1)	3.88	L-F	0 / 355	0.08 (1)	
	H-I	0 / 35	-91.8	-91.8	0.12 (1)	10.00	L-G	-346 / 0	0.39 (1)	
	R-B	-2087 / 0	0.0	0.0	0.14 (1)	7.02	K-G	262 / 5	0.10 (1)	
	J-H	-2087 / 0	0.0	0.0	0.14 (1)	7.02	B-Q	0 / 2127	0.48 (1)	
							K-H	0 / 2127	0.48 (1)	
	R-Q	0 / 0	-18.5	-18.5	0.07 (4)	10.00				
	Q-P	0 / 2087	-18.5	-18.5	0.29 (1)	10.00				
	P-O	0 / 1838	-18.5	-18.5	0.28 (1)	10.00				
	O-N	0 / 1838	-18.5	-18.5	0.28 (1)	10.00				
	N-M	0 / 1838	-18.5	-18.5	0.28 (1)	10.00				
	M-L	0 / 1838	-18.5	-18.5	0.28 (1)	10.00				
	L-K	0 / 2097	-18.5	-18.5	0.29 (1)	10.00				
	K-J	0 / 0	-18.5	-18.5	0.07 (4)	10.00				

TOTAL WEIGHT = 10 X 192 = 1922 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.83/1.00 (D-E:1), BC=0.29/1.00 (K-L:1), WE=0.48/1.00 (H-K:1), SS=0.30/1.00 (D-E:1)

DO LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1967 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (Q) (INPUT = 0.90)

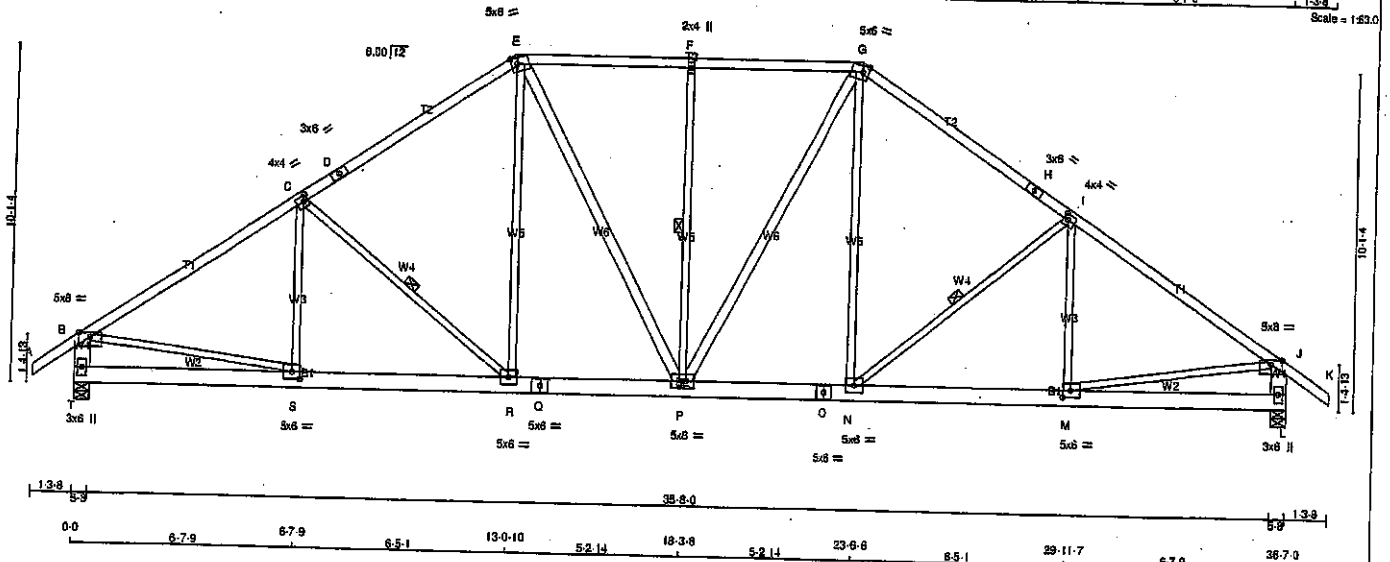
JSI METAL= 0.48 (Q) (INPUT = 1.00)



Structural component only
DWG# T-2108234

JOB NAME 417467	TRUSS NAME T7	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID: FntJiePQJhQRPc08ysmZJ2zZRcl-?NSRXZgKONPACQ10gp2SfQzJS7OZynCyM05cmzZ413
Scale = 1:20.0



CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
T - J	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0		2.00 1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0		2.25 2.00
F	TMVW-w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0		2.25 2.00
H	TS-t	MT20	3.0	6.0		
I	TMVW-t	MT20	4.0	4.0		2.00 1.50
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMVt-p	MT20	3.0	6.0		
M	BMVW-t	MT20	5.0	8.0		2.50 2.75
N	BMVW-t	MT20	5.0	8.0		
O	BS-t	MT20	5.0	8.0		
P	BMVW-w	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	8.0		
R	BMVW-t	MT20	5.0	8.0		
S	BMVW-t	MT20	5.0	8.0		2.50 2.75
T	BMVt-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
T	2143	0	2143	0
L	2143	0	2143	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	DEAD	SOIL
T	1513	1006 / 0	0 / 0	0 / 0	507 / 0
L	1513	1006 / 0	0 / 0	0 / 0	507 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-R, F-P, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO		
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	S-C	-198 / 43
B-C	-2499 / 0	-91.8	-91.8 0.71 (1)	3.66	C-R	-478 / 0
C-D	-2140 / 0	-91.8	-91.8 0.83 (1)	3.98	R-E	0 / 425
D-E	-2140 / 0	-91.8	-91.8 0.83 (1)	3.98	E-P	0 / 379
E-F	-1929 / 0	-91.8	-91.8 0.37 (1)	4.48	P-F	-584 / 0
F-G	-1929 / 0	-91.8	-91.8 0.71 (1)	3.66	N-G	0 / 425
G-H	-2140 / 0	-91.8	-91.8 0.83 (1)	3.98	N-I	-478 / 0
H-I	-2140 / 0	-91.8	-91.8 0.83 (1)	3.66	M-I	-188 / 43
I-J	-2499 / 0	-91.8	-91.8 0.71 (1)	3.66	S-S	0 / 2136
J-K	0 / 35	-91.8	-91.8 0.12 (1)	10.00	M-J	0 / 2136
T-B	-2084 / 0	0.0	0.0 0.14 (1)	7.03		
L-J	-2084 / 0	0.0	0.0 0.14 (1)	7.03		
T-S	0 / 0	-18.5	-18.5 0.08 (4)	10.00		
S-R	0 / 2112	-18.5	-18.5 0.29 (1)	10.00		
R-Q	0 / 1749	-18.5	-18.5 0.24 (1)	10.00		
Q-P	0 / 1749	-18.5	-18.5 0.24 (1)	10.00		
P-O	0 / 1749	-18.5	-18.5 0.24 (1)	10.00		
O-N	0 / 1749	-18.5	-18.5 0.24 (1)	10.00		
N-M	0 / 2112	-18.5	-18.5 0.29 (1)	10.00		
M-L	0 / 0	-18.5	-18.5 0.09 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OSC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/898 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.71/1.00 (B-C:1), BC=0.29/1.00 (R-S:1),
WB=0.48/1.00 (J-M:1), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

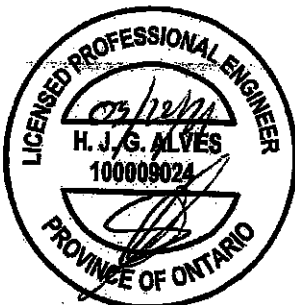
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

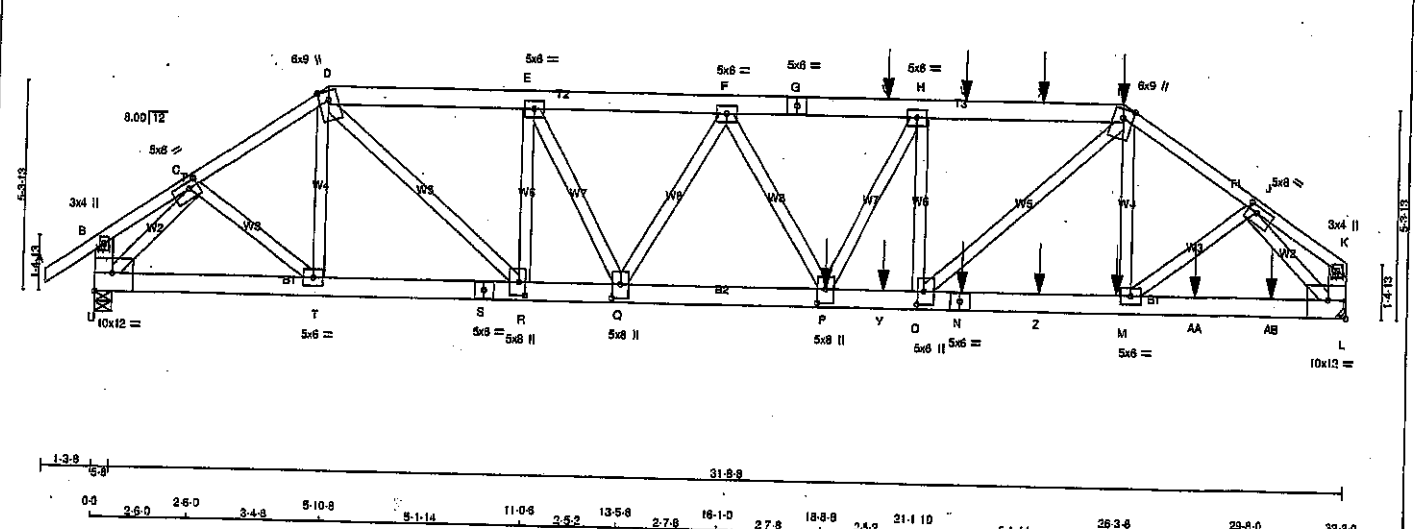
JSI GRIP = 0.84 (S) (INPUT = 0.90)
JSI METAL = 0.48 (M) (INPUT = 1.00)



Structural component only
DWG# T-2108235

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417467	T8	1	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington					

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 ID: FntJiePQJhRfPc0BysmZJ2zZRoL-TZ0pvgY9hX1pacbEXzhneV?xsQZIK2MB0m18CzZ412
 Scale = 1:53.5



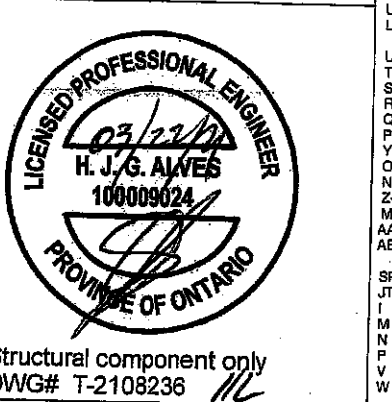
LUMBER			
N. L. & A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x6	DRY	No.2
G - I	2x6	DRY	No.2
I - K	2x4	DRY	No.2
U - B	2x6	DRY	No.2
L - K	2x6	DRY	No.2
U - S	2x6	DRY	2100F 1.8E
S - N	2x6	DRY	2100F 1.8E
N - L	2x6	DRY	2100F 1.8E
ALL WEBS EXCEPT	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0	
C	TMWW-t	MT20	5.0	8.0	2.00 2.75
D	TTWW+m	MT20	6.0	9.0	Edge
E, F, H					
E	TMWW-t	MT20	5.0	8.0	
G	TS-t	MT20	5.0	6.0	
I	TTWW+m	MT20	6.0	9.0	Edge
J	TMWW-t	MT20	5.0	8.0	2.00 2.75
K	TMV-p	MT20	3.0	4.0	
L	BMVW-t	MT20	10.0	12.0	5.50 Edge
M	BMVW-t	MT20	5.0	6.0	
N	BS-t	MT20	3.0	6.0	
O	BMVW-t	MT20	5.0	8.0	4.00 2.00
P	BMVW-t	MT20	5.0	8.0	4.25 2.50
Q	BMVW-t	MT20	5.0	8.0	4.25 2.50
R	BMVW-t	MT20	5.0	8.0	4.00 2.00
S	BS-t	MT20	5.0	6.0	
T	BMVW-t	MT20	5.0	6.0	
U	BMVW-t	MT20	10.0	12.0	5.50 Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
 1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
 DWG# T-2108236

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
VERT	HORIZ	VERT	HORIZ	IN-SX	IN-SX	IN-SX	IN-SX
JT 3890	0	3890	0	5-8	5-8		
L 3030	0	3030	0				

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 3-8.

UNFACTORED REACTIONS							
1ST LCASE	MAX. MIN. COMPONENT REACTIONS						
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
L 2750	1812.0	0.0	0.0	0.0	937.0	0.0	
U 2138	1430.0	0.0	0.0	0.0	708.0	0.0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.65 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0.35	-91.8 -91.8	0.14 (1)	10.00	T-D	-390.0	0.09 (1)
B-C	0.18	-91.8 -91.8	0.12 (1)	10.00	M-I	-398.30	0.11 (1)
C-D	-3722.0	-91.8 -91.8	0.38 (1)	3.31	U-C	-3779.0	0.60 (1)
D-E	-3249.0	-91.8 -91.8	0.36 (1)	3.54	C-T	0.879	0.12 (1)
E-F	-5949.0	-91.8 -91.8	0.38 (1)	3.33	J-L	-5073.0	0.80 (1)
F-G	-7144.0	-91.8 -91.8	0.81 (1)	2.78	M-J	0.1016	0.18 (1)
G-V	-7144.0	-91.8 -91.8	0.81 (1)	2.78	D-R	0.2851	0.52 (1)
V-H	-7144.0	-91.8 -91.8	0.81 (1)	2.78	R-E	-1947.0	0.53 (1)
H-W	-6618.0	-91.8 -91.8	0.85 (1)	2.83	O-I	0.3253	0.58 (1)
W-X	-6618.0	-91.8 -91.8	0.85 (1)	2.83	O-H	-2032.0	0.55 (1)
X-I	-6618.0	-91.8 -91.8	0.85 (1)	2.83	E-Q	0.1535	0.28 (1)
I-J	-5091.0	-91.8 -91.8	0.57 (1)	2.85	Q-F	-1503.0	0.55 (1)
J-K	0.15	-91.8 -91.8	0.10 (1)	10.00	F-P	0.1011	0.18 (1)
U-B	-203.0	0.0 0.0	0.01 (1)	7.81	P-H	0.1183	0.21 (1)
L-K	-82.0	0.0 0.0	0.01 (1)	7.81			
U-T	0.2580	-18.5 -18.5	0.15 (1)	10.00			
T-S	0.3107	-18.5 -18.5	0.18 (1)	10.00			
S-R	0.3107	-18.5 -18.5	0.18 (1)	10.00			
R-Q	0.5249	-18.5 -18.5	0.29 (1)	10.00			
Q-P	0.6663	-18.5 -18.5	0.42 (1)	10.00			
P-Y	0.6615	-18.5 -18.5	0.41 (1)	10.00			
Y-O	0.6615	-18.5 -18.5	0.41 (1)	10.00			
O-N	0.4254	-18.5 -18.5	0.28 (1)	10.00			
N-Z	0.4254	-18.5 -18.5	0.28 (1)	10.00			
Z-M	0.4254	-18.5 -18.5	0.28 (1)	10.00			
M-AA	0.3437	-18.5 -18.5	0.23 (1)	10.00			
AA-AB	0.3437	-18.5 -18.5	0.23 (1)	10.00			
AB-L	0.3437	-18.5 -18.5	0.23 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE
I	28-3-8	-369	-369		FRONT	VERT	TOTAL
M	28-2-12	-29	-29		FRONT	VERT	TOTAL
N	22-2-12	-29	-29		FRONT	VERT	TOTAL
P	18-8-4	-1385	-1385		FRONT	VERT	TOTAL
V	20-2-12	-122	-122		FRONT	VERT	TOTAL
W	22-2-12	-122	-122		FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 085-14
 - TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.07")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.22")
 ALLOWABLE DEFL.(TL) = L/360 (1.07")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.40")

CSI: TC=0.85/1.00 (H-I), BC=0.42/1.00 (P-Q), WB=0.80/1.00 (J-L-1), SSI=0.34/1.00 (H-I-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PU)	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747	788	1887

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)

JSI METAL= 0.94 (C) (INPUT = 1.00)

JOB NAME 417467	TRUSS NAME T8	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 10:26:03 2021 Page 2
 ID:FnlJiePQJhgRPc08ysmZJ2zZRd-TZ0plvay9hx1pacbEXZheV?xsQZIK2MB0mf8CzZ4i2

SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
X	24-2-12	-122	-122	---	FRONT	VERT	TOTAL	---	C1
Y	20-2-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
Z	24-2-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AA	28-2-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AB	30-2-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
 DWG# T-2108236 *HL*

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 10:26:03 2021 Page 1
ID:FntJliePQJhqPpC08ysmZJ2zZRcl-TZ0plvgY9hx1pacbEXZhnv71sPpIqM0M0mf8CzZ4j2



DRY: SEASONED LUMBER

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

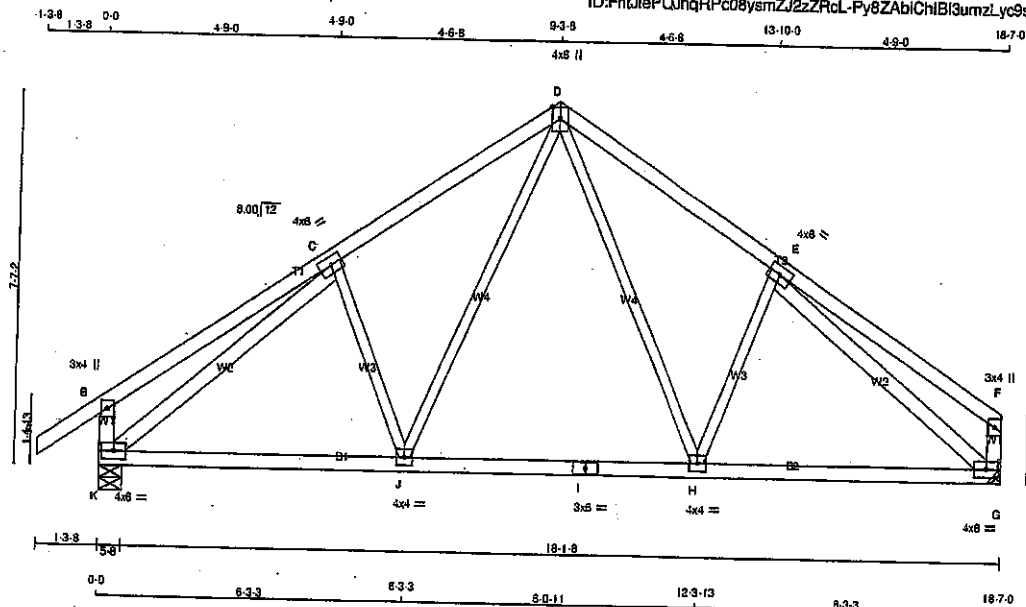
CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 35	-91.8	-91.8 0.14 (1)	10.00	D-I	0 / 687	0.17 (1)
B-C	0 / 16	-91.8	-91.8 0.12 (1)	10.00	J-E	0 / 76	0.03 (4)
C-D	-812 / 0	-91.8	-91.8 0.13 (1)	6.25	I-D	0 / 158	0.04 (1)
D-E	-1035 / 0	-91.8	-91.8 0.13 (1)	5.01	C-J	-59 / 40	0.02 (4)
E-F	0 / 14	-91.8	-91.8 0.11 (1)	10.00	K-C	-965 / 0	0.29 (1)
F-G	0 / 35	-91.8	-91.8 0.14 (1)	10.00	E-H	-1226 / 0	0.36 (1)
K-B	-230 / 0	0.0	0.0 0.03 (1)	7.81			
H-F	-234 / 0	0.0	0.0 0.03 (1)	7.81			
K-J	0 / 682	-18.5	-18.5 0.16 (1)	10.00			
J-L	0 / 807	-18.5	-18.5 0.43 (1)	10.00			
L-I	0 / 807	-156.5	-156.5 0.43 (1)	10.00			
I-H	0 / 849	-156.5	-156.5 0.47 (1)	10.00			

JSI GRIP= 0.84 (I) (INPUT = 0.90)
JSI METAL= 0.27 (C) (INPUT = 1.00)



JOB NAME 417467	TRUSS NAME T10	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Sat Mar 20 10:28:05 2021 Page 1
ID:FntUiePQhQRPc08ymZJ2zZRL-Py8ZAbiChIBi3umzLyc9s3bQb19?mHzeeKFID5zZ410



Scale = 1/4" = 1'-0"

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
K - C	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW+1	MT20	4.0	8.0		
D	TMWW+p	MT20	4.0	8.0	Edge	
E	TMWW+1	MT20	4.0	8.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW+1	MT20	4.0	8.0		
H	BMVW+1	MT20	4.0	8.0		
I	BS+1	MT20	3.0	6.0		
J	BMVW+1	MT20	4.0	8.0		
K	BMVW+1	MT20	4.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES:

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
K	1150	0	5-8	5-8
G	1024	0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

1ST CASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	811	546	0	0	0	285	0
K	724	475	0	0	0	249	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
FR-TO						FR-TO		
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	D-H	0 / 403	0.09 (1)
B-C	0 / 28	-91.8	-91.8	0.32 (1)	10.00	H-E	-267 / 0	0.09 (1)
C-D	-997 / 0	-91.8	-91.8	0.26 (1)	5.94	J-D	0 / 403	0.09 (1)
D-E	-997 / 0	-91.8	-91.8	0.26 (1)	5.94	C-J	-267 / 0	0.09 (1)
E-F	0 / 28	-91.8	-91.8	0.32 (1)	10.00	K-C	-1219 / 0	0.58 (1)
K-B	-298 / 0	0.0	0.0	0.03 (1)	7.81	E-G	-1219 / 0	0.58 (1)
G-F	-160 / 0	0.0	0.0	0.02 (1)	7.81			
K-J	0 / 806	-18.5	-18.5	0.23 (4)	10.00			
J-I	0 / 651	-18.5	-18.5	0.21 (4)	10.00			
I-H	0 / 651	-18.5	-18.5	0.21 (4)	10.00			
H-G	0 / 806	-18.5	-18.5	0.23 (4)	10.00			

TOTAL WEIGHT = 2 X 84 = 167 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.82")
CALCULATED VERT. DEFL. (LL) = L/999 (0.03")
ALLOWABLE DEFL. (TL) = L/360 (0.82")
CALCULATED VERT. DEFL. (TL) = L/999 (0.08")

CSI: TC=0.32/1.00 (B-C:1), BC=0.23/1.00 (J-K:4),
WB=0.59/1.00 (C-K:1), SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PLI)
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.80 (C) (INPUT = 0.80)
JSI METAL = 0.29 (C) (INPUT = 1.00)



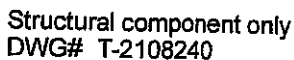
Structural component only
DWG# T-2108239

Tamarack Roof Truss, Burlington

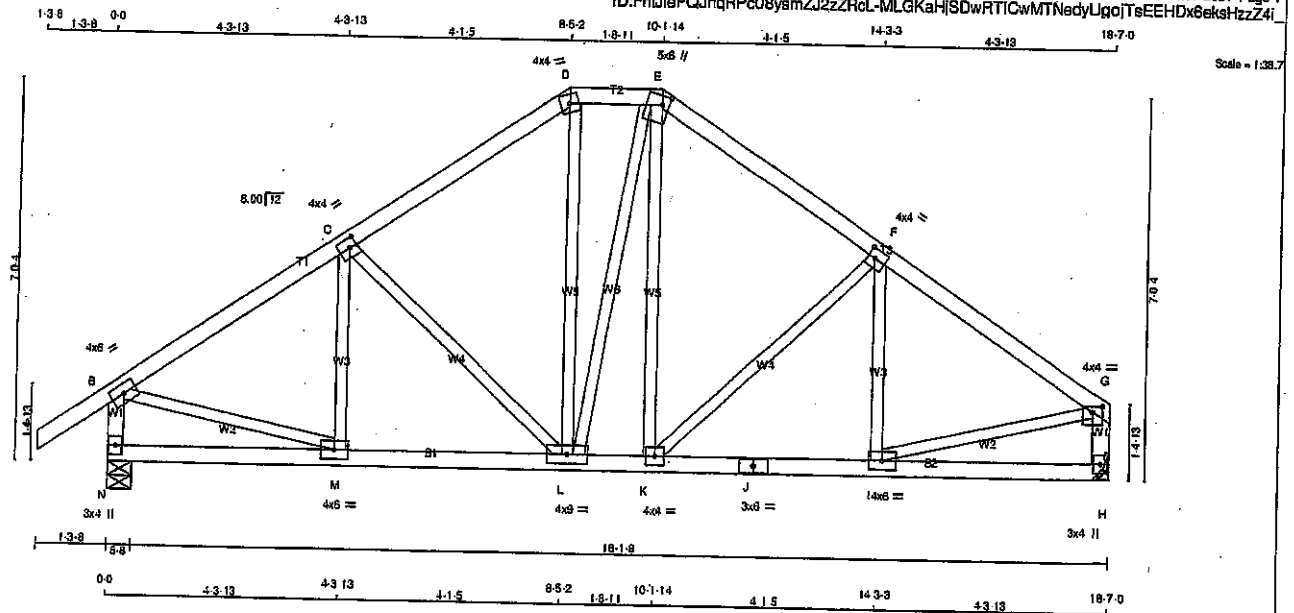


NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

JSI GRIP= 0.38 (K) (INPUT = 0.90)
JSI METAL= 0.10 (J) (INPUT = 1.00)



JOB NAME 417467	TRUSS NAME T11	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
K - L	2x4	DRY	No.2	SPF	
M - N	2x4	DRY	No.2	SPF	
O - P	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	

EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMW-t MT20	4.0	6.0		
C TMW-t MT20	4.0	4.0	2.00	1.50
D TTW-m MT20	4.0	4.0		
E TTW-t MT20	5.0	6.0	2.00	1.50
F TMW-t MT20	4.0	4.0	2.00	1.50
G TMW-p MT20	4.0	4.0	1.25	2.00
H BMV-t MT20	3.0	4.0		
I BMW-t MT20	4.0	6.0		
J BS-t MT20	3.0	6.0		
K BMW-t MT20	4.0	4.0		
L BMW-t MT20	4.0	4.0		
M BMW-t MT20	4.0	6.0		
N BMV-t MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
N	1150	0	0	5-8
H	1024	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT COMBINED				
N	811	548	0	0
H	724	475	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO		
A-B	0/35	-91.8	-91.8 0.12 (1)	10.00	M-C	-144/23
B-C	-1072/0	-91.8	-91.8 0.22 (1)	5.84	C-L	-288/0
C-D	-872/0	-91.8	-91.8 0.21 (1)	6.25	L-D	0/250
D-E	-708/0	-91.8	-91.8 0.04 (1)	6.25	E-E	0/9
E-F	-888/0	-91.8	-91.8 0.21 (1)	8.25	K-E	0/240
F-G	-1073/0	-91.8	-91.8 0.22 (1)	5.84	K-F	-282/0
G-H	-1118/0	0.0	0.0 0.11 (1)	7.51	I-F	-139/28
H-I	-991/0	0.0	0.0 0.10 (1)	7.81	B-M	0/941
N-M	0/0	-18.5	-18.5 0.08 (4)	10.00	I-G	0/941
M-L	0/912	-18.5	-18.5 0.18 (1)	10.00		
L-K	0/704	-18.5	-18.5 0.15 (1)	10.00		
K-J	0/913	-18.5	-18.5 0.18 (1)	10.00		
J-I	0/913	-18.5	-18.5 0.18 (1)	10.00		
I-H	0/0	-18.5	-18.5 0.08 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.22/1.00 (F-G:1), BC=0.18/1.00 (H-K:1), WB=0.21/1.00 (G-I:1), SS=0.18/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

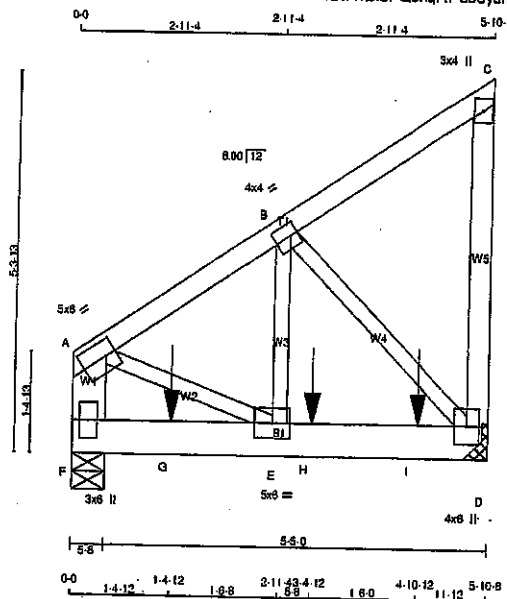
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (G) (INPUT = 0.90)
JSI METAL= 0.32 (J) (INPUT = 1.00)



Structural component only
DWG# T-2108241

JOB NAME 417467	TRUSS NAME T12	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - A	2x6	DRY	No.2
F - D	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1	12	TOP
C-D 1	12	TOP
F-A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	1.75
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMV-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
D	1983	0	1983	0	0
F	1583	0	1583	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	DEAD	SOIL
D	COMBINED	SNOW	LIVE	PERM. LIVE
D	1399	934	0	0
F	1188	793	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. (PLF)	MAX. CS1 (LC)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO	FROM TO					FR-TO		
A-B	-1406 / 0	-91.8	-91.8	0.06 (1)	6.25	E-B	0.1868	0.20 (1)
B-C	-14.7	-91.8	-91.8	0.06 (1)	6.25	B-D	-1677 / 0	0.24 (1)
D-C	-109 / 0	0.0	0.0	0.02 (1)	7.81	A-E	0 / 1248	0.15 (1)
F-A	-1298 / 0	0.0	0.0	0.05 (1)	7.81			
F-G	0 / 0	-18.5	-18.5	0.19 (1)	10.00			
G-E	0 / 0	-18.5	-18.5	0.19 (1)	10.00			
E-H	0 / 1182	-18.5	-18.5	0.32 (1)	10.00			
H-I	0 / 1182	-18.5	-18.5	0.32 (1)	10.00			
I-D	0 / 1182	-18.5	-18.5	0.32 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
G	1-4-12	-710	-710	---	BACK	VERT	TOTAL	---	C1
H	3-4-12	-710	-710	---	BACK	VERT	TOTAL	---	C1
I	4-10-12	-710	-710	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CS1: TC=0.08/1.00 (A-B:1), BC=0.32/1.00 (D-E:1), WB=0.24/1.00 (B-D:1), SS1=0.46/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

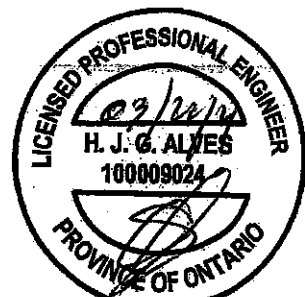
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

MAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.27 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108242

JOB NAME 417467	TRUSS NAME T12	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MITek Industries, Inc. Sat Mar 20 10:28:08 2021 Page 2 ID:FntUiePQJhgRPc08ysmZJ2zZReL-oXpiodk5 DZKwLVY149sUhd?tt9Lzk45KI TOpQzZ4hz		

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMW1+p	MT20	4.0	6.0		
E	BMW1-i	MT20	5.0	6.0		
F	BMW1+p	MT20	3.0	6.0		

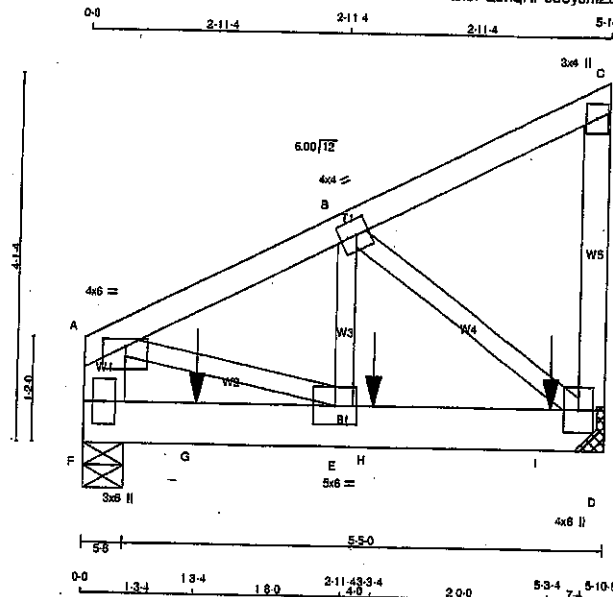
NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108242

JOB NAME 417467	TRUSS NAME T13	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1/2" = 1'-0"

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
D - C	2x4	DRY	No.2	SPF		
F - A	2x6	DRY	No.2	SPF		
F - D	2x6	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
A - C	1	12	TOP
C - D	1	12	TOP
F - A	2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
F - D	3	4	SIDE (1556.5)
WEBS : (0.122"x3") SPIRAL NAILS			
2x3	1	8	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	8.0	1.00	3.00
B	TMVW-l	MT20	4.0	4.0	2.00	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
D	4024	0	0	MECHANICAL
F	1058	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

	1ST LOASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	2839	1900 / 0	0 / 0	0 / 0	0 / 0	939 / 0	0 / 0
F	742	523 / 0	0 / 0	0 / 0	0 / 0	219 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. LOAD LC1	MAX. OSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. OSI (LC)
FR-TO		FROM	TO			FR-TO		
A-B	-1376 / 0	-91.8	-91.8	0.03 (1)	6.25	E-B	0 / 1251	0.09 (1)
B-C	-11 / 0	-91.8	-91.8	0.03 (1)	6.25	B-D	-1561 / 0	0.12 (1)
D-C	-111 / 0	0.0	0.0	0.01 (1)	7.81	A-E	0 / 1283	0.10 (1)
F-A	-1058 / 0	0.0	0.0	0.02 (1)	7.81			
F-G	0 / 0	-18.5	-18.5	0.11 (1)	10.00			
G-E	0 / 0	-18.5	-18.5	0.11 (1)	10.00			
E-H	0 / 1241	-18.5	-18.5	0.45 (1)	10.00			
H-I	0 / 1241	-18.5	-18.5	0.45 (1)	10.00			
I-D	0 / 1241	-18.5	-18.5	0.45 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-3-4	-178	-178	---	BACK	VERT	TOTAL	---	C1
H	3-3-4	-208	-208	---	BACK	VERT	TOTAL	---	C1
I	5-3-4	-2738	-2738	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 3 X 29 = 88 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.03/1.00 (A-B:1), BC=0.45/1.00 (D-E:1),
WB=0.12/1.00 (B-D:1), SS=0.60/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PS)	(PL)	(PL)
JT	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.54 (B) (INPUT = 0.90)
JSI METAL = 0.19 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108243 1/2

CONTINUED ON PAGE 2

JOB NAME 417467	TRUSS NAME T13	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW1	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



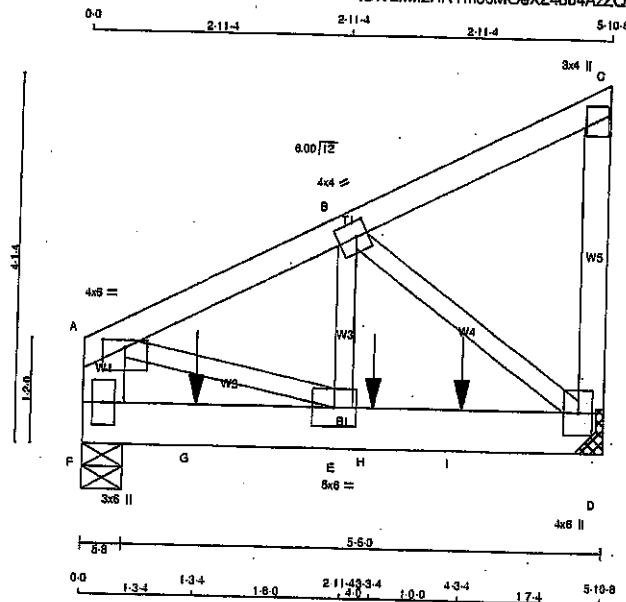
Structural component only
DWG# T-2108243 *in*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
417471	T13Z	1	2	GREENPARK HOMES		

Tamarack Roof Truss, Burlington

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Scale = 1/20.5

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - A	2x6	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1 12		TOP
C-D 1 12		TOP
F-A 2 12		TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2 12		SIDE (183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMVW-I	MT20	4.0	4.0	2.00	1.50
C	TMV-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
D	2170	0	2170	0	0	0
F	1271	0	1271	0	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE		
D	1527	1048	0	0	0	0
F	891	628	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	-1696 / 0	-91.8	-91.8	0.08 (1)	8.25	E-B	0' 1816 0.20 (1)		
B-C	-10 / 0	-91.8	-91.8	0.05 (1)	10.00	B-D	-1920 / 0 0.23 (1)		
D-C	-113 / 0	0.0	0.0	0.01 (1)	7.81	A-E	0' 1578 0.20 (1)		
F-A	-1278 / 0	0.0	0.0	0.05 (1)	7.81				
F-G	0 / 0	-18.5	-18.5	0.20 (1)	10.00				
G-E	0 / 0	-18.5	-18.5	0.20 (1)	10.00				
E-H	0 / 1526	-18.5	-18.5	0.82 (1)	10.00				
H-I	0 / 1526	-18.5	-18.5	0.82 (1)	10.00				
I-D	0 / 1526	-18.5	-18.5	0.82 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-3-4	-179	-179	---	FRONT	VERT	TOTAL	---	C1
H	3-4	-209	-209	---	FRONT	VERT	TOTAL	---	C1
I	4-3-4	-1572	-1572	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CSI: TC=0.06/1.00 (A-B:1), BC=0.82/1.00 (D-E:1), WB=0.23/1.00 (B-D:1), SSI=0.56/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (B) (INPUT = 0.90)

JSI METAL = 0.34 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108253 1/2

CONTINUED ON PAGE 2

JOB NAME 417471	TRUSS NAME T13Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MITek Industries, Inc. Sat Mar 20 10:33:47 2021 Page 2 ID:V2rMlzHRYm36MOeXZ46o4AzZQVV-ug3D7Zi?uFOGUj4mQ uVbUIS3u160JftC8CQJzZ4ao		

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMWV1+p	MT20	4.0	6.0		
E	BMWV1-i	MT20	5.0	6.0		
F	BMWV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

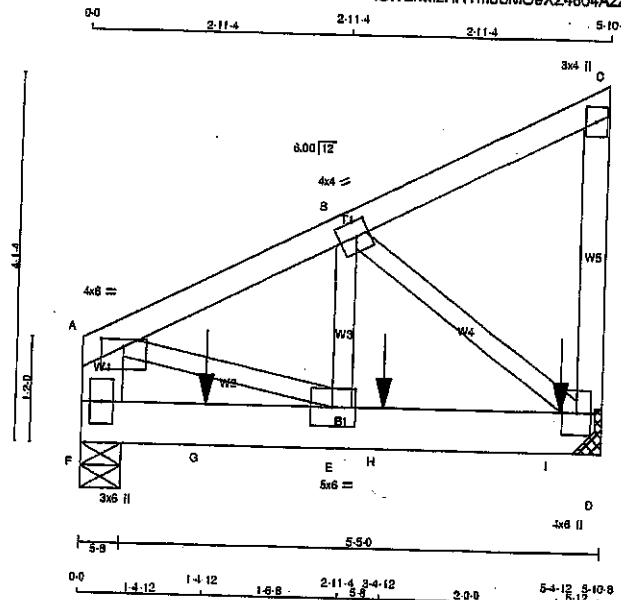


Structural component only
DWG# T-2108253 *W*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417471	T13Z2	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/2\"/>

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - D	12	SIDE(42.2)
F - A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE(122.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWV-t	MT20	4.0	4.0	2.00	1.75
C	TMV-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
D	1412	0	1412	0
F	1115	0	1115	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	996	688	0	0	0	0	328	0
F	786	528	0	0	0	0	258	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED MOMENT (LBS-FT)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED MOMENT (LBS-FT)
FR-TO								
A-B	-1080	0	-91.8	-91.8	0.06 (1)	6.25	E-B	0
B-C	-12	0	-91.8	-91.8	0.06 (1)	8.25	B-D	-1208
D-C	-109	0	0.0	0.0	0.01 (1)	7.81	A-E	0
F-A	-841	0	0.0	0.0	0.03 (1)	7.81		
F-G	0	0	-18.5	-18.5	0.13 (1)	10.00		
G-E	0	0	-18.5	-18.5	0.13 (1)	10.00		
E-H	0	959	-18.5	-18.5	0.16 (1)	10.00		
H-I	0	959	-18.5	-18.5	0.16 (1)	10.00		
I-D	0	959	-18.5	-18.5	0.16 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
G	1-4-12	-440	-440	---	BACK	VERT	TOTAL	---	C1
H	3-4-12	-440	-440	---	BACK	VERT	TOTAL	---	C1
I	5-4-12	-444	-444	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.08/1.00 (A-B:1), SC=0.16/1.00 (D-E:1),
WB=0.14/1.00 (B-D:1), SSI=0.24/1.00 (D-E:1)

DOL LUMBER-1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	650	371	1747
	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.58 (B) (INPUT = 0.90)
JSI METAL = 0.22 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108254

CONTINUED ON PAGE 2

JOB NAME 417471	TRUSS NAME T13Z2	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

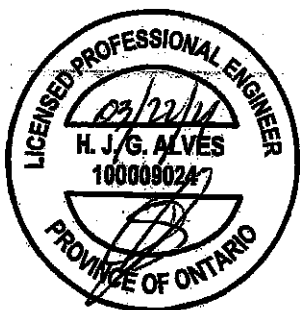
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW1+	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

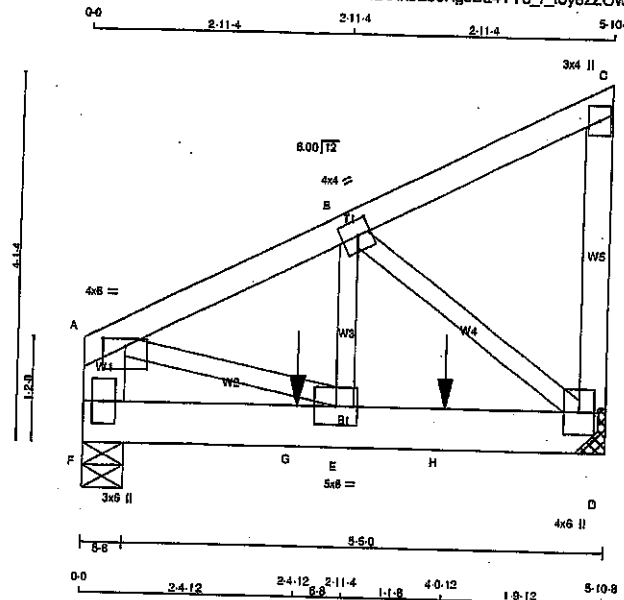


Structural component only
DWG# T-2108254 *ML*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417473	T13Z3	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/32

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - A	2x6	DRY No.2	SPF
F - D	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'x3') SPIRAL NAILS		
A - C	12	TOP
C - D	12	TOP
F - A	12	TOP
BOTTOM CHORDS : (0.122'x3') SPIRAL NAILS		
F - D	12	SIDE (0.0)
WEBS : (0.122'x3') SPIRAL NAILS		
2x3	1	5

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	8.0	1.00	3.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TMV-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
D	1448	0	0	0
F	931	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4'-0.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
D	1017	705 / 0	0 / 0	0 / 0	312 / 0	0 / 0
F	654	456 / 0	0 / 0	0 / 0	198 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	MAX. UNBRACED LENGTH	FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
A-B	-1243 / 0	-91.8	-91.8	0.06 (1)	6.25	6.25	E-B	0 / 1098	0.14 (1)		
B-C	-11 / 0	-91.8	-91.8	0.05 (1)	6.25	6.25	B-D	-1411 / 0	0.17 (1)		
D-C	-110 / 0	0.0	0.0	0.01 (1)	7.81	7.81	A-E	0 / 1180	0.14 (1)		
F-A	-987 / 0	0.0	0.0	0.03 (1)	7.81	7.81					
F-G	0 / 0	-18.5	-18.5	0.12 (1)	10.00	10.00					
G-E	0 / 0	-18.5	-18.5	0.12 (1)	10.00	10.00					
E-H	0 / 1122	-18.5	-18.5	0.38 (1)	10.00	10.00					
H-D	0 / 1122	-18.5	-18.5	0.38 (1)	10.00	10.00					

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-4-12	-178	-178		TOP	VERT	TOTAL		C1
H	4-0-12	-1035	-1035		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TC=0.06/1.00 (A-B:1), BC=0.38/1.00 (D-E:1), WB=0.17/1.00 (B-D:1), SS=0.35/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.71 (B) (INPUT = 0.90)
JSI METAL = 0.25 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108283

CONTINUED ON PAGE 2

JOB NAME 417473	TRUSS NAME T13Z3	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW1-l	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

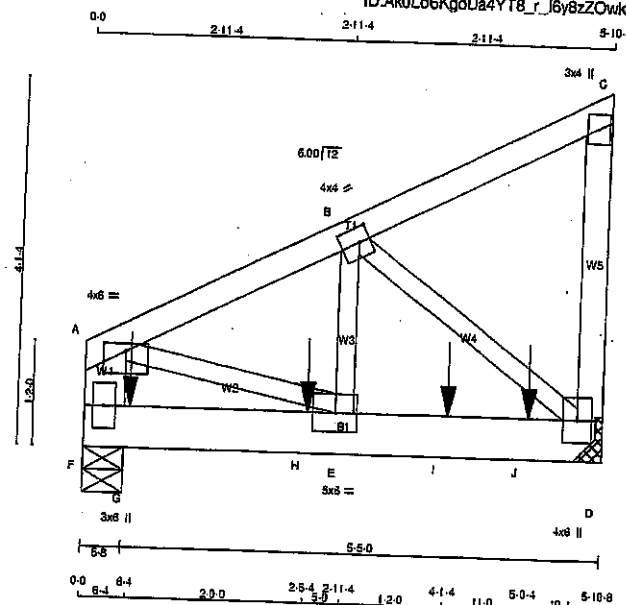


Structural component only
DWG# T-2108283 *2/1*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
417473	T13Z4	1	2	GREENPARK HOMES		

Tamarack Roof Truss, Burlington

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Scale = 1/20

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - A	2x6	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD/PLF
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1	12	TOP
C-D 1	12	TOP
F-A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)	W	LEN	Y	X
JT TYPE				
A TMVW-p	MT20	4.0	6.0	1.00 3.00
B TMVW-t	MT20	4.0	4.0	2.00 1.50
C TMV-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	IN-SX	IN-SX
D	2892 0	2892 0	0 0	0 0
F	1356 0	1356 0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT COMBINED							
D	1898 1280 0	0 0	0 0	0 0	0 0	818 0	0 0
F	951 868 0	0 0	0 0	0 0	0 0	283 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL FITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO					FR-TO		
A-B	-1659 0	-91.8 -91.8	0.05 (1)	6.25	E-B	0 1573	0.19 (1)
B-C	-10 0	-91.8 -91.8	0.05 (1)	10.00	B-D	-1877 0	0.22 (1)
D-C	-113 0	0.0 0.0	0.01 (1)	7.81	A-E	0 1543	0.19 (1)
F-A	-1253 0	0.0 0.0	0.04 (1)	7.81			
F-G	0 0	-18.5 -18.5	0.20 (1)	10.00			
G-H	0 0	-18.5 -18.5	0.20 (1)	10.00			
H-E	0 0	-18.5 -18.5	0.20 (1)	10.00			
E-I	0 1492	-18.5 -18.5	0.55 (1)	10.00			
I-J	0 1492	-18.5 -18.5	0.55 (1)	10.00			
J-D	0 1492	-18.5 -18.5	0.55 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	D/R	TYPE	HEEL	CONN.
G	8-4	-179	-179	---	TOP	VERT	TOTAL	---	C1
H	2-8-4	-239	-239	---	TOP	VERT	TOTAL	---	C1
I	4-1-4	-870	-870	---	FRONT	VERT	TOTAL	---	C1
J	5-0-4	-1103	-1103	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = 1/998 (0.03")

CSI: TC=0.06/1.00 (A-B:1), BC=0.55/1.00 (D-E:1), WB=0.22/1.00 (B-D:1), SSI=0.48/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.84 (B) (INPUT = 0.90)
JSI METAL = 0.34 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108284

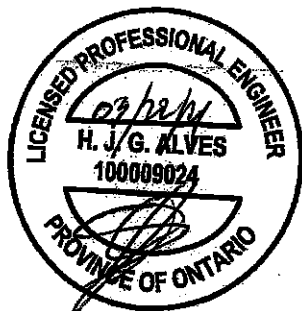
CONTINUED ON PAGE 2

JOB NAME 417473	TRUSS NAME T13Z4	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 10:58:21 2021 Page 2 ID: Ak0Lo6KgoDg4YT8 r l6y6zZOwk-kP7SUa7bl862Kpm2mYL9KJmM44q2NzccqXDC9tzZ4Dm		

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
D BMVW1+p	MT20	4.0	6.0		
E BMVW1+	MT20	5.0	6.0		
F BMV1+p	MT20	3.0	6.0		

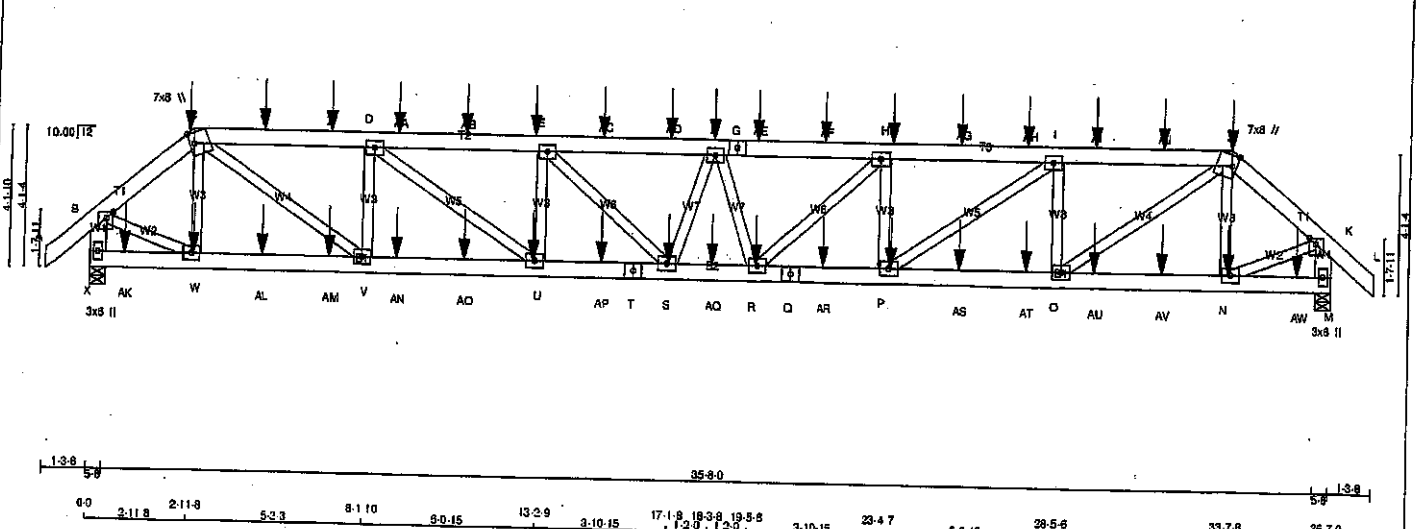
NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108284 2/2

JOB NAME 417471	TRUSS NAME T21	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 10:33:50 2021 Page 1
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Scale = 1:31.4



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - G	2x6	DRY	No.2
C - G	2x6	DRY	No.2
G - J	2x6	DRY	No.2
J - L	2x6	DRY	No.2
X - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
X - T	2x6	DRY	2100F 1.8E
T - Q	2x6	DRY	2100F 1.8E
Q - M	2x6	DRY	2100F 1.8E

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C 2 12		SIDE(122.0)
C-G 2 12		SIDE(183.1)
G-J 2 12		SIDE(183.1)
J-L 2 12		SIDE(122.0)
X-B 2 12		TOP
M-K 2 12		TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
X-T 2 12		SIDE(197.8)
T-Q 2 12		SIDE(0.0)
Q-M 2 12		SIDE(197.8)

WEBS: (0.122"x3") SPIRAL NAILS
2x4 1 6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD. BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
X	3604	0	3580	0	0	5-8	5-8	5-8	5-8
M	3580	0	3580	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST CASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
X	2547	1683	0	0	0
M	2528	1688	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS) X, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC. LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 42	-91.8	-91.8	0.04 (1)	10.00	W-C	-798 / 0	0.07 (1)
B-C	-3615 / 0	-91.8	-91.8	0.05 (1)	5.87	C-V	0 / 4038	0.36 (1)
C-Y	-8041 / 0	-91.8	-91.8	0.20 (1)	4.67	V-D	-2175 / 0	0.19 (1)
Y-Z	-8041 / 0	-91.8	-91.8	0.20 (1)	4.67	D-U	0 / 2370	0.21 (1)
Z-D	-8041 / 0	-91.8	-91.8	0.20 (1)	4.67	U-E	1234 / 0	0.11 (1)
D-AA	-7987 / 0	-91.8	-91.8	0.24 (1)	4.13	P-H	-1238 / 0	0.11 (1)
AA-AB	-7987 / 0	-91.8	-91.8	0.24 (1)	4.13	P-I	0 / 2378	0.21 (1)
AB-E	-7987 / 0	-91.8	-91.8	0.24 (1)	4.13	O-I	-2179 / 0	0.19 (1)
E-AC	-8515 / 0	-91.8	-91.8	0.23 (1)	4.03	O-J	0 / 4046	0.36 (1)
AC-AD	-8515 / 0	-91.8	-91.8	0.23 (1)	4.03	N-J	-789 / 0	0.07 (1)
AD-F	-8515 / 0	-91.8	-91.8	0.23 (1)	4.03	B-W	0 / 2933	0.26 (1)
F-G	-8513 / 0	-91.8	-91.8	0.23 (1)	4.03	N-K	0 / 2913	0.26 (1)
G-AE	-8513 / 0	-91.8	-91.8	0.23 (1)	4.03	E-S	0 / 756	0.07 (1)
AE-AF	-8513 / 0	-91.8	-91.8	0.23 (1)	4.03	S-F	-446 / 0	0.04 (1)
AF-H	-8513 / 0	-91.8	-91.8	0.23 (1)	4.03	F-R	-451 / 0	0.04 (1)
H-AG	-7960 / 0	-91.8	-91.8	0.24 (1)	4.13	R-H	0 / 762	0.07 (1)
AG-AH	-7980 / 0	-91.8	-91.8	0.24 (1)	4.13			
AH-I	-7980 / 0	-91.8	-91.8	0.24 (1)	4.13			
I-AJ	-6028 / 0	-91.8	-91.8	0.20 (1)	4.67			
AJ-AJ	-6028 / 0	-91.8	-91.8	0.20 (1)	4.67			
AJ-J	-6028 / 0	-91.8	-91.8	0.20 (1)	4.67			
J-K	-3589 / 0	-91.8	-91.8	0.05 (1)	5.88			
K-L	0 / 42	-91.8	-91.8	0.04 (1)	10.00			
X-B	-3591 / 0	0.0	0.0	0.13 (1)	7.40			
M-K	-3568 / 0	0.0	0.0	0.13 (1)	7.42			

X-AK	0 / 0	-18.5	-18.5	0.02 (4)	10.00
AK-W	0 / 0	-18.5	-18.5	0.02 (4)	10.00
W-AL	0 / 2729	-18.5	-18.5	0.09 (1)	10.00
AL-AM	0 / 2729	-18.5	-18.5	0.09 (1)	10.00
AM-V	0 / 2729	-18.5	-18.5	0.09 (1)	10.00
V-AN	0 / 6042	-18.5	-18.5	0.19 (1)	10.00
AN-AO	0 / 6042	-18.5	-18.5	0.19 (1)	10.00
AO-U	0 / 6042	-18.5	-18.5	0.19 (1)	10.00
U-AP	0 / 7987	-18.5	-18.5	0.24 (1)	10.00
AP-T	0 / 7987	-18.5	-18.5	0.24 (1)	10.00
T-S	0 / 7987	-18.5	-18.5	0.24 (1)	10.00
S-AQ	0 / 8858	-18.5	-18.5	0.26 (1)	10.00
AQ-R	0 / 8858	-18.5	-18.5	0.26 (1)	10.00
R-Q	0 / 7960	-18.5	-18.5	0.24 (1)	10.00
Q-AR	0 / 7980	-18.5	-18.5	0.24 (1)	10.00
AR-P	0 / 7980	-18.5	-18.5	0.24 (1)	10.00

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(56% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.22')
CALCULATED VERT. DEFL.(LL) = L/999 (0.19')
ALLOWABLE DEFL.(TL) = L/360 (1.22')
CALCULATED VERT. DEFL.(TL) = L/999 (0.35')

CSI: TC=0.24/1.00 (D-E-1), BC=0.26/1.00 (R-S-1), WB=0.38/1.00 (J-O-1), SSI=0.14/1.00 (J-J-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

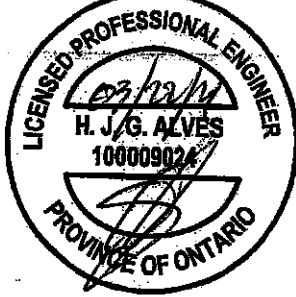
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.74 (T) (INPUT = 1.00)



Structural component only
DWG# T-2108255

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417471	T21	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:V2MizHRYm36MOeXZ46o4AzZQVV-IPkMmbKIAAmmlLpL58SCD8LwW6eSDeZ5ZAOs1ezZ4a

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	2.00	2.25
C	TTWW+m	MT20	7.0	8.0	3.75	1.50
D, E, F, H, I						
D	TMVW-t	MT20	5.0	8.0		
G	TS-t	MT20	5.0	8.0		
J	TTWW+m	MT20	7.0	8.0	3.75	1.50
K	TMVW+p	MT20	5.0	8.0	2.00	2.25
M	BMV1+p	MT20	3.0	6.0		
N, O, P, R, S, U, V, W						
N	BMVW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	8.0		
T	BS-t	MT20	5.0	8.0		
X	BMV1+p	MT20	3.0	6.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. MEMB. FORCE (LBS)	FACTORED UNBRACED LENGTH FR-TO	MAX. MEMB. FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM	TO				
P-AS	0 / 6029	-18.5	-18.5	0.19 (1)	10.00		
AS-AT	0 / 6029	-18.5	-18.5	0.19 (1)	10.00		
AT-O	0 / 6029	-18.5	-18.5	0.19 (1)	10.00		
O-AU	0 / 2710	-18.5	-18.5	0.09 (1)	10.00		
AU-AV	0 / 2710	-18.5	-18.5	0.09 (1)	10.00		
AV-N	0 / 2710	-18.5	-18.5	0.09 (1)	10.00		
N-AW	0 / 0	-18.5	-18.5	0.02 (4)	10.00		
AW-M	0 / 0	-18.5	-18.5	0.02 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	2-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-108	-108	---	FRONT	VERT	TOTAL	---	C1
O	2-11-8	-125	-125	---	FRONT	VERT	SNOW	---	C1
E	13-0-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
F	18-3-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
H	23-5-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
J	33-7-8	-82	-82	---	FRONT	VERT	TOTAL	---	C1
N	33-7-8	-155	-155	---	FRONT	VERT	SNOW	---	C1
P	33-6-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
R	23-6-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
S	19-6-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
U	17-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
W	13-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Y	3-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Z	5-0-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AA	7-0-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AB	9-0-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AC	11-0-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AD	13-0-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AE	15-0-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AF	17-0-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AG	19-6-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AH	21-6-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AI	23-6-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AJ	25-6-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AK	27-6-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AL	29-6-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AM	31-6-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AN	1-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AO	5-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AP	7-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AQ	9-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AR	11-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AS	13-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AT	15-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AU	17-0-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AV	19-6-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AW	21-6-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

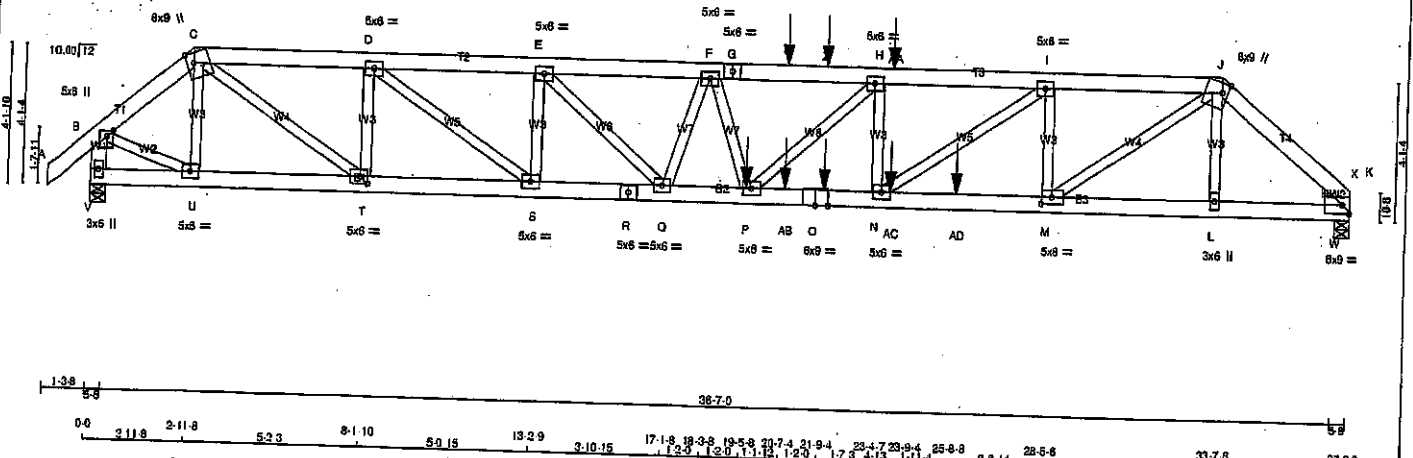
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2108255

JOB NAME 417471	TRUSS NAME T21X	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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 ID: V2rMzHRYm36MOeXZ46o4AzZQVv-molkxLWxUvdzVOXapzRiKu01W7Ry11Eoq7OZ4z4ak
 1-3.8 0-0 2-11.8 5-2.3 8-1.10 5-0.15 13-2.9 3-10.15 17-1.8 18-3.8 19-5.8 20-7.4 21-9.4 23-4.7 23-9.4 25-5.8 5-2.3 33-7.8 3-10.9 37-6.0
 Scale = 1:80.0



CHORDS	SIZE	LUMBER	DESCR.
A - C	2x8	DRY No.2	SPF
C - G	2x6	DRY No.2	SPF
G - J	2x6	DRY No.2	SPF
V - B	2x6	DRY No.2	SPF
V - R	2x6	DRY 2100F 1.8E	SPF
R - O	2x6	DRY 2100F 1.8E	SPF
O - K	2x6	DRY 2100F 1.8E	SPF

ALL WEBS 2x4 DRY No.2
 EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS			
A-C	2	12	TOP
C-G	2	12	TOP
G-J	2	12	TOP
J-K	2	12	TOP
V-B	2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS			
V-R	2	12	TOP
R-O	2	12	TOP
O-K	2	12	TOP
WEBS: (0.122"x3") SPIRAL NAILS			
2x4	1	6	TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
JT	3838	0	3838	0	5-8	5-8	
K	4384	0	4384	0	5-8	5-8	2x6 R

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
JT	2705	1821	0	0	0	885
K	3093	2072	0	0	0	1021

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.99 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD		MAX. UNBRACED LENGTH	FR-TO	WEBS		MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
			LC1	LC2			FR-TO	CS1 (LC)			
A-B	0	42	-91.8	-91.8	0.04 (1)	10.00	U-O	-918	0	0.08 (1)	10.00
B-C	-3893	0	-91.8	-91.8	0.08 (1)	5.70	L-J	-308	0	0.03 (1)	10.00
C-D	-7299	0	-91.8	-91.8	0.18 (1)	4.35	B-U	0	9159	0.28 (1)	10.00
D-E	-10767	0	-91.8	-91.8	0.29 (1)	3.59	C-T	0	5318	0.47 (1)	10.00
E-F	-12888	0	-91.8	-91.8	0.39 (1)	3.23	T-D	-2975	0	0.25 (1)	10.00
F-G	-13898	0	-91.8	-91.8	0.51 (1)	2.99	D-S	0	4270	0.58 (1)	10.00
G-Y	-13898	0	-91.8	-91.8	0.51 (1)	2.99	S-E	-2417	0	0.21 (1)	10.00
Y-Z	-13898	0	-91.8	-91.8	0.51 (1)	2.99	M-J	0	6583	0.25 (1)	10.00
Z-H	-13898	0	-91.8	-91.8	0.51 (1)	2.99	N-I	0	4221	0.37 (1)	10.00
H-AA	-13229	0	-91.8	-91.8	0.40 (1)	3.18	N-H	-1323	0	0.11 (1)	10.00
AA-I	-13229	0	-91.8	-91.8	0.40 (1)	3.18	E-Q	0	2888	0.26 (1)	10.00
I-J	-9801	0	-91.8	-91.8	0.28 (1)	4.80	Q-F	-1902	0	0.18 (1)	10.00
J-X	-5895	0	-91.8	-91.8	0.28 (1)	4.17	F-P	0	1280	0.11 (1)	10.00
X-K	-7330	0	-91.8	-91.8	0.29 (1)	7.22	P-H	0	919	0.08 (1)	10.00
V-B	-3847	0	0.0	0.0	0.14 (1)	10.00	W-X	0	1825	0.00 (1)	10.00
V-U	0	0	-18.5	-18.5	0.02 (1)	10.00					
U-T	0	2836	-18.5	-18.5	0.10 (1)	10.00					
T-S	0	7299	-18.5	-18.5	0.22 (1)	10.00					
S-R	0	10767	-18.5	-18.5	0.31 (1)	10.00					
R-Q	0	10767	-18.5	-18.5	0.31 (1)	10.00					
Q-P	0	13480	-18.5	-18.5	0.45 (1)	10.00					
P-AB	0	13229	-18.5	-18.5	0.45 (1)	10.00					
AB-O	0	13229	-18.5	-18.5	0.45 (1)	10.00					
O-N	0	13229	-18.5	-18.5	0.45 (1)	10.00					
N-AC	0	9800	-18.5	-18.5	0.53 (1)	10.00					
AC-AD	0	9800	-18.5	-18.5	0.53 (1)	10.00					
AD-M	0	9800	-18.5	-18.5	0.53 (1)	10.00					
M-L	0	4400	-18.5	-18.5	0.20 (1)	10.00					
L-W	0	4413	-18.5	-18.5	0.25 (1)	10.00					
W-K	0	4413	-18.5	-18.5	0.25 (1)	10.00					

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
O	21-9.4	-21	-21	---	BACK	VERT	---	C1
P	19-5.8	-1512	-1512	---	BACK	VERT	---	C1
Y	20-7.4	-76	-76	---	BACK	VERT	---	C1
Z	21-9.4	-76	-76	---	BACK	VERT	---	C1
AA	23-9.4	-76	-76	---	BACK	VERT	---	C1
AB	20-7.4	-21	-21	---	BACK	VERT	---	C1
AC	23-9.4	-21	-21	---	BACK	VERT	---	C1
AD	25-8.8	-981	-981	---	BACK	VERT	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
 DL = 8.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TRC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.25")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.31")
 ALLOWABLE DEFL.(TL) = L/360 (1.25")
 CALCULATED VERT. DEFL.(TL) = L/794 (0.57")

CSI: TC=0.51/1.00 (F-H:1), BC=0.53/1.00 (M-N:1), WB=0.58/1.00 (J-M:1), SS=0.51/1.00 (K-W:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

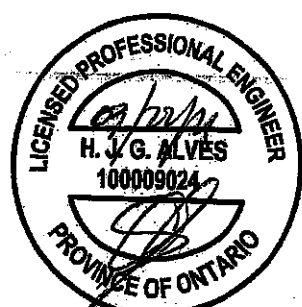
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (FS) (PL) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (M) (INPUT = 0.90)
 JSI METAL=0.98 (R) (INPUT = 1.00)



Structural component only
 DWG# T-2108256

CONTINUED ON PAGE 2

JOB NAME 417471	TRUSS NAME T21X	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	8.0	9.0	3.50	2.50
D, E, F, H, I						
D	TMVW-l	MT20	5.0	6.0		
G	TS-l	MT20	5.0	6.0		
J	TTWW+m	MT20	8.0	9.0	3.50	2.00
K	TMBH-l	MT20	6.0	9.0		Edge
L	BMVW+w	MT20	3.0	6.0		
M	BMVW-l	MT20	5.0	8.0	2.50	3.75
N, P, Q, S, U						
N	BMVW-l	MT20	5.0	6.0		
O	BS-l	MT20	6.0	9.0		
R	BS-l	MT20	5.0	6.0		
T	BMVW-l	MT20	5.0	6.0	2.50	2.50
V	BMV-l+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

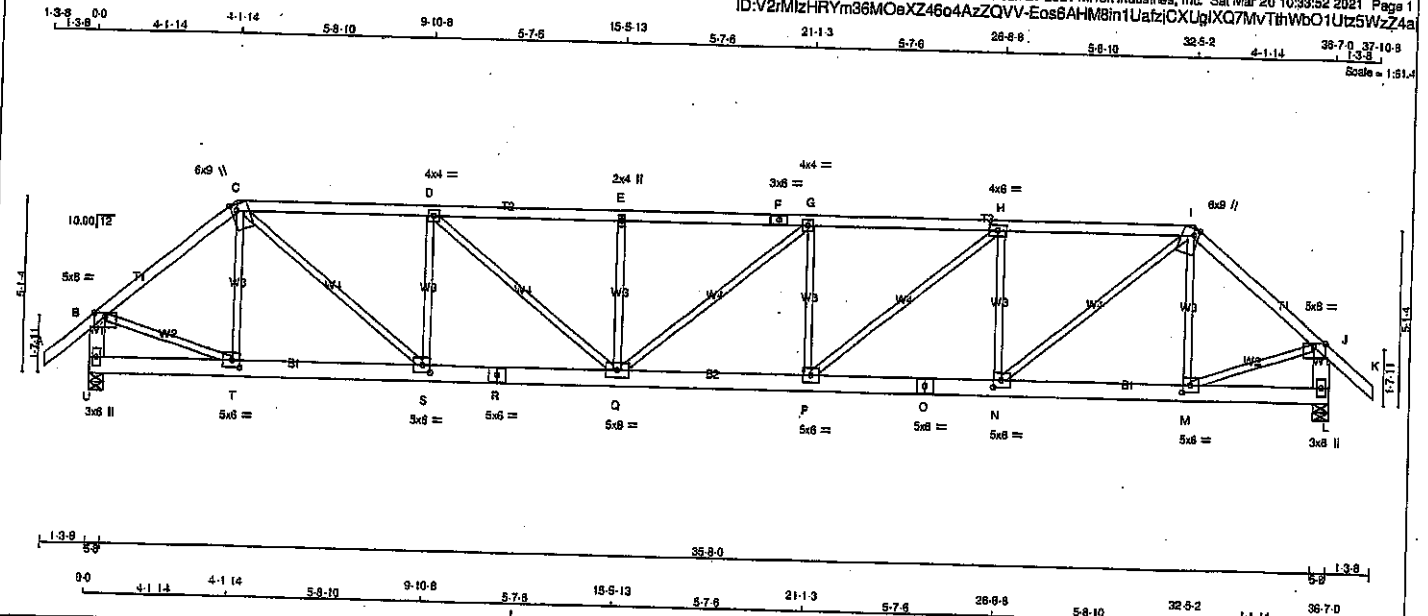


Structural component only
DWG# T-2108256

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
417471	T22	1	1	GREENPARK HOMES		

Tamarack Roof Truss, Burlington

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LUMBER

N. L. & A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - G	2x4 DRY	No.2	SPF
C - F	2x4 DRY	No.2	SPF
F - I	2x4 DRY	No.2	SPF
I - K	2x4 DRY	No.2	SPF
U - B	2x6 DRY	No.2	SPF
L - J	2x6 DRY	No.2	SPF
U - R	2x6 DRY	No.2	SPF
R - O	2x6 DRY	No.2	SPF
O - L	2x6 DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWV-p	MT20	5.0	6.0	Edge	
C TTWW-m	MT20	8.0	9.0	Edge	1.75
D TMWV-l	MT20	4.0	4.0		
E TMW-w	MT20	2.0	4.0		
F TS-l	MT20	3.0	6.0		
G TMWV-l	MT20	4.0	4.0		
H TMWV-l	MT20	4.0	6.0		
I TTWW-m	MT20	6.0	9.0	Edge	1.75
J TMWV-p	MT20	5.0	6.0	Edge	
L BMV1-p	MT20	3.0	6.0		
M, N, S, T					
M BMWV-l	MT20	5.0	6.0	2.50	2.75
O BS-l	MT20	5.0	6.0		
P BMWV-l	MT20	5.0	6.0		
Q BMWV-l	MT20	5.0	6.0		
R BS-l	MT20	5.0	6.0		
U BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	2144	0	2144	0	0	5-8	5-8
L	2144	0	2144	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE	MAX./MIN. COMPONENT REACTIONS	PERM.LIVE	WIND	DEAD	SOIL
U	COMBINED	SNOW	LIVE			
U	1514	1007.0	0.0	0.0	507.0	0.0
L	1514	1007.0	0.0	0.0	507.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	W E B S	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MEMB.	FORCE (LBS)
FR-TO			FR-TO	
A-B	0.41	-91.8 -91.8 0.13 (1)	T-C	-345.0 0.12 (1)
B-C	-2087.0	-91.8 -91.8 0.37 (1)	M-I	-343.0 0.12 (1)
C-D	-3138.0	-91.8 -91.8 0.69 (1)	B-T	0.1664 0.37 (1)
D-E	-3846.0	-91.8 -91.8 0.80 (1)	M-J	0.1664 0.37 (1)
E-F	-3846.0	-91.8 -91.8 0.85 (1)	N-I	0.2000 0.45 (1)
F-G	-3846.0	-91.8 -91.8 0.85 (1)	C-S	0.2003 0.45 (1)
G-H	-3854.0	-91.8 -91.8 0.79 (1)	N-H	-1179.0 0.42 (1)
H-I	-3138.0	-91.8 -91.8 0.37 (1)	S-D	-1172.0 0.42 (1)
I-J	-2088.0	-91.8 -91.8 0.13 (1)	P-H	0.939 0.21 (1)
J-K	0.41	0.0 0.0 0.14 (1)	D-Q	0.926 0.21 (1)
U-B	-2117.0	0.0 0.0 0.14 (1)	P-G	-496.0 0.18 (1)
L-J	-2118.0	0.0 0.0 0.14 (1)	Q-E	-496.0 0.18 (1)
U-T	0.0	-18.5 -18.5 0.05 (4)	Q-G	-10.0 0.01 (1)
T-S	0.1592	-18.5 -18.5 0.23 (1)		
S-R	0.3138	-18.5 -18.5 0.43 (1)		
R-Q	0.3138	-18.5 -18.5 0.43 (1)		
Q-P	0.3854	-18.5 -18.5 0.51 (1)		
P-O	0.3138	-18.5 -18.5 0.42 (1)		
O-N	0.3138	-18.5 -18.5 0.42 (1)		
N-M	0.1592	-18.5 -18.5 0.23 (1)		
M-L	0.0	-18.5 -18.5 0.05 (4)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = U/999 (0.22")
ALLOWABLE DEFL.(TL) = L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = U/999 (0.41")

CSI: TC=0.80/1.00 (D-E:1), BC=0.51/1.00 (P-Q:1),
WB=0.45/1.00 (C-S:1), SS=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1967 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

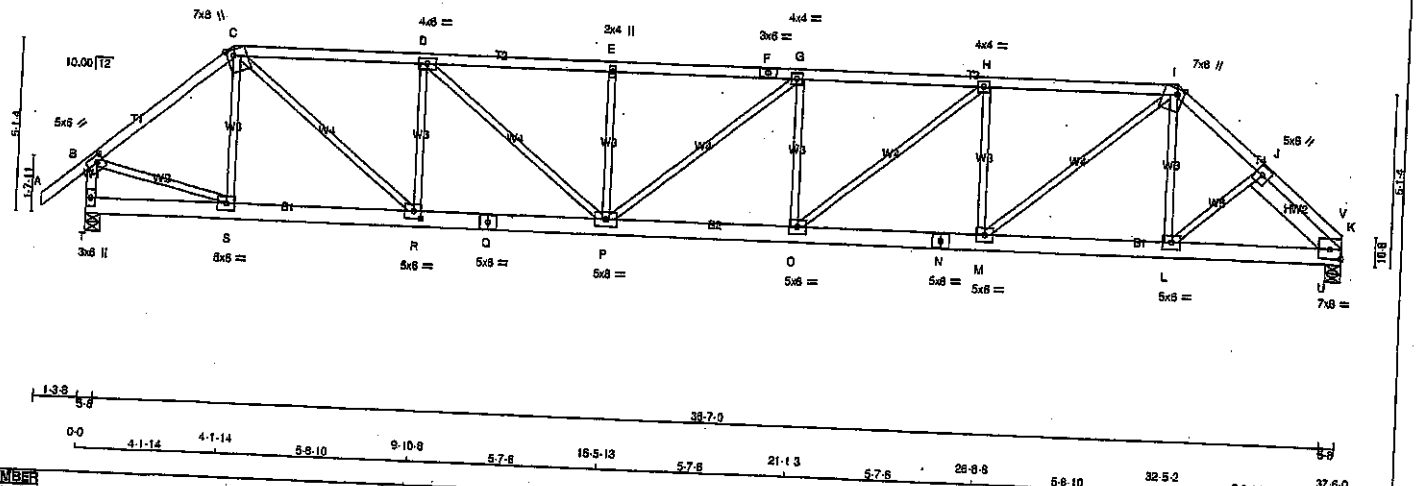
JSI GRIP = 0.89 (D) (INPUT = 0.90)
JSI METAL = 0.80 (R) (INPUT = 1.00)



Structural component only
DWG# T-2108257

JOB NAME 417471	TRUSS NAME T22X	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						

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ID:V2mIzHRYm38MOeXZ46c4AzZQVV-i_QuOdMmT59LCpYwME7vqIzHVJphQzZXG8cWdzzZ4a



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - F	2x4 DRY	No.2	SPF
F - I	2x4 DRY	No.2	SPF
I - K	2x4 DRY	No.2	SPF
T - B	2x4 DRY	No.2	SPF
T - Q	2x6 DRY	No.2	SPF
Q - N	2x6 DRY	No.2	SPF
N - K	2x6 DRY	No.2	SPF
REINFORCING MEMBERS			
HW2	2x6 DRY	No.2	SPF
ALL WEBS 2x3 DRY EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMWV-I	MT20	5.0	8.0	2.25 2.50
C TTWW+m	MT20	7.0	8.0	Edge 2.25
D TMWV-I	MT20	4.0	8.0	Edge 2.25
E TMWV-I	MT20	2.0	4.0	
F TS-I	MT20	3.0	6.0	
G TMWV-I	MT20	4.0	4.0	
H TMWV-I	MT20	4.0	4.0	
I TTWW+m	MT20	7.0	8.0	Edge 2.25
J TMWV-I	MT20	5.0	6.0	
K TMBMW-I	MT20	7.0	8.0	3.25 Edge
L M, O, S				
L BMWV-I	MT20	5.0	8.0	
N BS-I	MT20	5.0	6.0	
P BMWV-I	MT20	5.0	8.0	
Q BS-I	MT20	5.0	6.0	
R BMWV-I	MT20	5.0	8.0	2.50 2.75
T BMV-I	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	JT	GROSS REACTION	JT	GROSS REACTION	JT	GROSS REACTION
K	2067 0	K	2067 0	K	2067 0	K	2067 0
T	2194 0	T	2194 0	T	2194 0	T	2194 0

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS
K	1482	959 0 0 0 0 0 0 0	K	1482	959 0 0 0 0 0 0 0
T	1549	1030 0 0 0 0 0 0 0	T	1549	1030 0 0 0 0 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS K, T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (LBS)	MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (LBS)
FR-TO				FR-TO			
A-B	0 / 41	-91.8	-91.8 0.13 (1)	S-C	-359 / 0	0.13 (1)	
B-C	-2145 / 0	-91.8	-91.8 0.38 (1)	L-I	0 / 102	0.03 (4)	
C-D	-3243 / 0	-91.8	-91.8 0.70 (1)	B-S	0 / 1710	0.38 (1)	
D-E	-4011 / 0	-91.8	-91.8 0.82 (1)	M-I	0 / 1907	0.43 (1)	
E-F	-4011 / 0	-91.8	-91.8 0.88 (1)	C-R	0 / 2082	0.47 (1)	
F-G	-4011 / 0	-91.8	-91.8 0.88 (1)	M-H	-1128 / 0	0.40 (1)	
G-H	-4079 / 0	-91.8	-91.8 0.83 (1)	R-D	-1222 / 0	0.44 (1)	
H-I	-3419 / 0	-91.8	-91.8 0.73 (1)	O-H	0 / 893	0.19 (1)	
I-J	-2553 / 0	-91.8	-91.8 0.15 (1)	D-P	0 / 1004	0.23 (1)	
J-V	-1342 / 0	-91.8	-91.8 0.08 (1)	O-G	-445 / 0	0.16 (1)	
V-K	-2071 / 0	-91.8	-91.8 0.06 (1)	P-E	-498 / 0	0.18 (1)	
T-B	-2168 / 0	0.0	0.0 0.23 (1)	L-J	-99 / 0	0.09 (1)	
T-S	0 / 0	-18.5	-18.5 0.05 (4)	U-V	0 / 83	0.02 (1)	
S-R	0 / 1635	-18.5	-18.5 0.23 (1)	U-W	0 / 824	0.00 (1)	
R-Q	0 / 3243	-18.5	-18.5 0.44 (1)	U-J	-1365 / 0	0.14 (1)	
Q-P	0 / 3243	-18.5	-18.5 0.44 (1)				
P-O	0 / 4079	-18.5	-18.5 0.54 (1)				
O-N	0 / 3419	-18.5	-18.5 0.48 (1)				
N-M	0 / 3419	-18.5	-18.5 0.46 (1)				
M-L	0 / 1948	-18.5	-18.5 0.29 (1)				
L-U	0 / 1886	-18.5	-18.5 0.31 (1)				
U-K	0 / 1039	-18.5	-18.5 0.21 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.25")
CALCULATED VERT. DEFL. (LL) = L/999 (0.23")
ALLOWABLE DEFL. (TL) = L/360 (1.25")
CALCULATED VERT. DEFL. (TL) = L/999 (0.44")

CSK TC=0.83/1.00 (G-H:1), BC=0.54/1.00 (C-P:1), WB=0.47/1.00 (C-R:1), SS=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (R) (INPUT = 0.90)

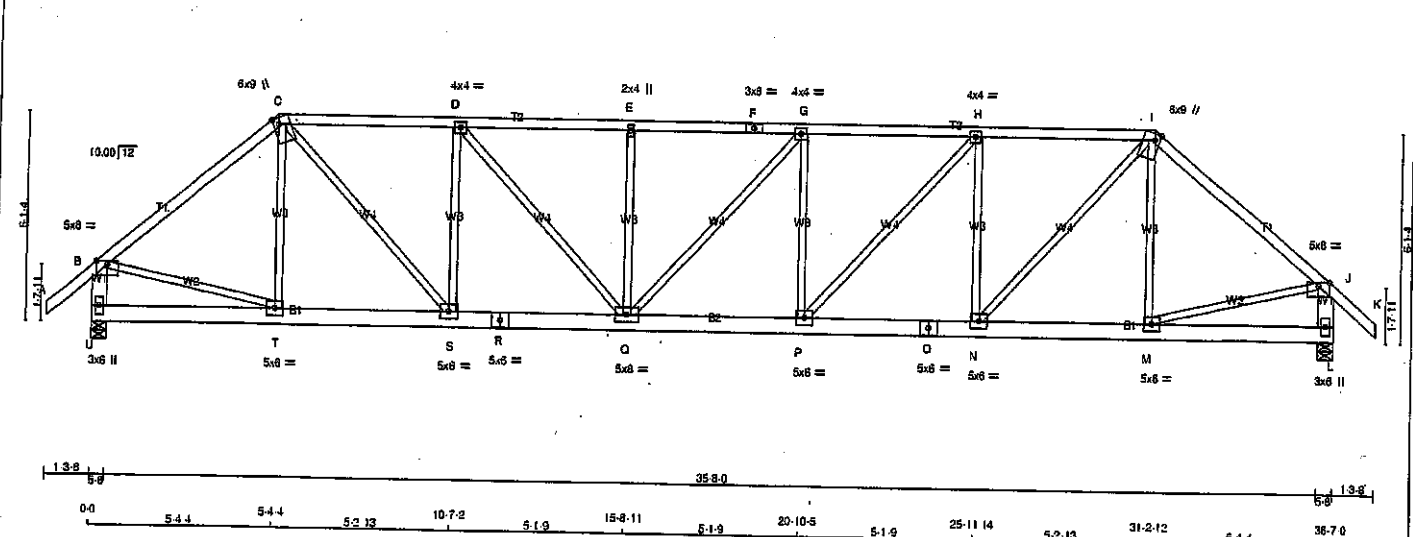
JSI METAL = 0.83 (N) (INPUT = 1.00)



Structural component only
DWG# T-2108258

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417471	T23	1	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID: V2mIzHRYm36MOeXZ46o4AzZQVW-BB_tbzNOEPHCqz76KyW8N9WV0IB9C9hUoM4APzZ4ah
13-8 0.0 5-4.4 5-4.4 5-2.13 10-7.2 5-1.9 15-8.11 5-1.9 20-10.5 5-1.9 25-11.14 5-2.13 31-2.12 5-4.4 36-7.0 37-18.8
Scale = 1:51.4



LUMBER	N, L, G, A, RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
I - K	2x4	DRY	No.2	SPF	
U - B	2x6	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	
U - R	2x6	DRY	No.2	SPF	
R - O	2x6	DRY	No.2	SPF	
O - L	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE	MT20	5.0	8.0	Edge
B TMVW-p	MT20	6.0	9.0	Edge 1.75
C TTWW+m	MT20	4.0	4.0	
D, G, H	MT20	2.0	4.0	
E TMVW-w	MT20	3.0	6.0	
F TS-i	MT20	6.0	9.0	Edge 1.75
I TTWW+m	MT20	5.0	8.0	Edge
J TMVW-p	MT20	3.0	6.0	
L BMV1+p	MT20	5.0	8.0	
M, N, P, S, T	MT20	5.0	8.0	
M BMVW-t	MT20	5.0	8.0	
O BS-i	MT20	5.0	8.0	
Q BMVW-w	MT20	5.0	8.0	
R BS-i	MT20	5.0	8.0	
U BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERY	DOWN	UPLIFT	IN-SX
U	2144	0	0	5-8
L	2144	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	1007.0	0.0	0.0	0.0	507.0	0.0
U	1514	1007.0	0.0	0.0	0.0	507.0	0.0
L	1514	1007.0	0.0	0.0	0.0	507.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO						FR-TO			
A-B	0.741	-91.8	-91.8	0.13 (1)	10.00	T-C	-251.0	0.14 (1)	
B-C	-2126.0	-91.8	-91.8	0.66 (1)	4.00	M-I	-249.0	0.14 (1)	
C-D	-2695.0	-91.8	-91.8	0.52 (1)	3.74	E-T	0.1670	0.36 (1)	
D-E	-3182.0	-91.8	-91.8	0.57 (1)	3.42	M-J	0.1670	0.36 (1)	
E-F	-3182.0	-91.8	-91.8	0.46 (1)	3.57	N-I	0.1580	0.36 (1)	
F-G	-3182.0	-91.8	-91.8	0.46 (1)	3.57	C-S	0.1583	0.36 (1)	
G-H	-3188.0	-91.8	-91.8	0.57 (1)	3.42	N-H	-1077.0	0.59 (1)	
H-I	-2693.0	-91.8	-91.8	0.51 (1)	3.75	S-D	-1071.0	0.58 (1)	
I-J	-2126.0	-91.8	-91.8	0.66 (1)	4.00	P-H	0.741	0.17 (1)	
J-K	0.741	-91.8	-91.8	0.13 (1)	10.00	D-Q	0.730	0.16 (1)	
U-B	-2102.0	0.0	0.0	0.14 (1)	7.00	P-G	-453.0	0.25 (1)	
L-J	-2102.0	0.0	0.0	0.14 (1)	7.00	Q-E	-455.0	0.25 (1)	
						Q-G	-9.0	0.01 (1)	
U-T	0.0	-18.5	-18.5	0.06 (4)	10.00				
T-S	0.1625	-18.5	-18.5	0.22 (1)	10.00				
S-R	0.2695	-18.5	-18.5	0.36 (1)	10.00				
R-Q	0.2695	-18.5	-18.5	0.36 (1)	10.00				
Q-P	0.3188	-18.5	-18.5	0.42 (1)	10.00				
P-O	0.2694	-18.5	-18.5	0.36 (1)	10.00				
O-N	0.2694	-18.5	-18.5	0.36 (1)	10.00				
N-M	0.1625	-18.5	-18.5	0.23 (1)	10.00				
M-L	0.0	-18.5	-18.5	0.06 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (1.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.66/1.00 (H-I), BC=0.42/1.00 (P-Q), WB=0.59/1.00 (H-N), SS=0.22/1.00 (C-D)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

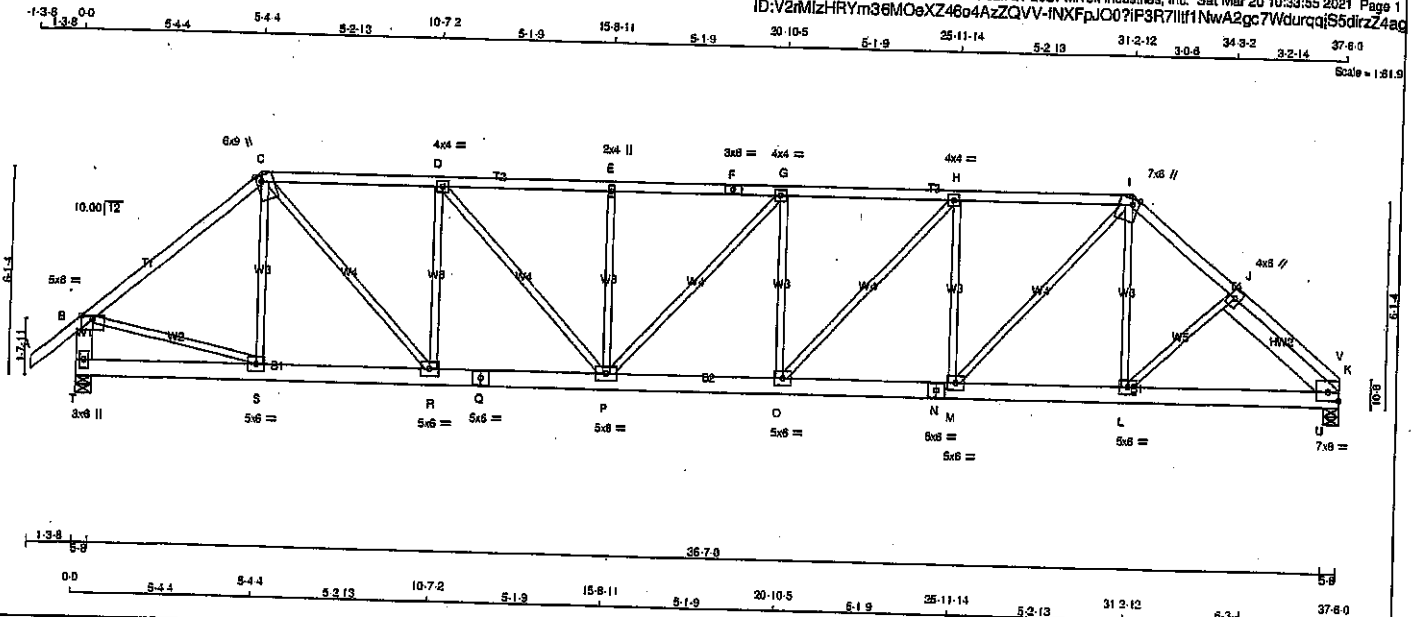
JSI GRIP = 0.79 (M) (INPUT = 0.90)
JSI METAL = 0.50 (R) (INPUT = 1.00)



Structural component only
DWG# T-2108259

JOB NAME 417471	TRUSS NAME T23X	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:V2nMlzHRYm36MOeXZ46o4AzZQVV-INXFPJ007IF3R7IIf1NwA2gc7WdurqqiS5dirZ4ag



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
T - B	2x6	DRY	No.2
T - Q	2x6	DRY	No.2
Q - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2

REINFORCING MEMBERS			
HW2	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-w	MT20	5.0	8.0	Edge	1.75
C	TTWW-m	MT20	6.0	9.0	Edge	1.75
D, G, H						
D	TMW-w	MT20	4.0	4.0		
E	TMW-w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
I	TTWW-m	MT20	7.0	8.0	Edge	2.25
J	TMW-w	MT20	4.0	6.0		
K	TMBMW-t	MT20	7.0	8.0	3.00	Edge
L, M, O, R, S						
L	BMW-w	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
P	BMW-w	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
T	BMV-t	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
K	2067	0	0	5-8	5-8
T	2194	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
K	1462	959	0	0	0	0	0
T	1549	1030	0	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO				FR-TO			
A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00	S-C	-261 / 0	0.14 (1)
B-C	-2188 / 0	-91.8	-91.8 0.87 (1)	3.94	L-I	0 / 177	0.04 (4)
C-D	-2788 / 0	-91.8	-91.8 0.53 (1)	2.68	L-J	-40 / 13	0.01 (1)
D-E	-3321 / 0	-91.8	-91.8 0.59 (1)	3.34	B-S	0 / 1719	0.39 (1)
E-F	-3321 / 0	-91.8	-91.8 0.48 (1)	3.49	M-I	0 / 1499	0.34 (1)
F-G	-3321 / 0	-91.8	-91.8 0.48 (1)	3.49	C-R	0 / 1650	0.37 (1)
G-H	-3372 / 0	-91.8	-91.8 0.59 (1)	3.32	M-H	-1027 / 0	0.56 (1)
H-I	-2921 / 0	-91.8	-91.8 0.54 (1)	3.59	R-D	-1120 / 0	0.61 (1)
I-J	-2505 / 0	-91.8	-91.8 0.18 (1)	4.22	O-H	0 / 875	0.15 (1)
J-V	-1408 / 0	-91.8	-91.8 0.13 (1)	5.36	D-P	0 / 798	0.18 (1)
V-K	-2152 / 0	-91.8	-91.8 0.08 (1)	4.59	O-G	-402 / 0	0.22 (1)
T-B	-2153 / 0	0.0	0.0 0.15 (1)	6.94	P-E	-455 / 0	0.25 (1)
					P-G	-77 / 0	0.08 (1)
					U-V	0 / 818	0.00 (1)
					U-J	-1288 / 0	0.20 (1)
T-S	0 / 0	-18.5	-18.5 0.06 (4)	10.00			
S-R	0 / 1673	-18.5	-18.5 0.23 (1)	10.00			
R-Q	0 / 2788	-18.5	-18.5 0.38 (1)	10.00			
Q-P	0 / 2788	-18.5	-18.5 0.38 (1)	10.00			
P-O	0 / 3372	-18.5	-18.5 0.45 (1)	10.00			
O-N	0 / 2922	-18.5	-18.5 0.38 (1)	10.00			
N-M	0 / 2922	-18.5	-18.5 0.38 (1)	10.00			
M-L	0 / 1908	-18.5	-18.5 0.28 (1)	10.00			
L-U	0 / 1933	-18.5	-18.5 0.30 (1)	10.00			
U-K	0 / 1090	-18.5	-18.5 0.19 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.25")
CALCULATED VERT. DEFL. (LL) = L/899 (0.17")
ALLOWABLE DEFL. (TL) = L/360 (1.25")
CALCULATED VERT. DEFL. (TL) = L/999 (0.32")

CSI: TC=0.67/1.00 (B-C:1), BC=0.45/1.00 (O-P:1), WB=0.81/1.00 (D-R:1), SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 950 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.82 (S) (INPUT = 0.90)

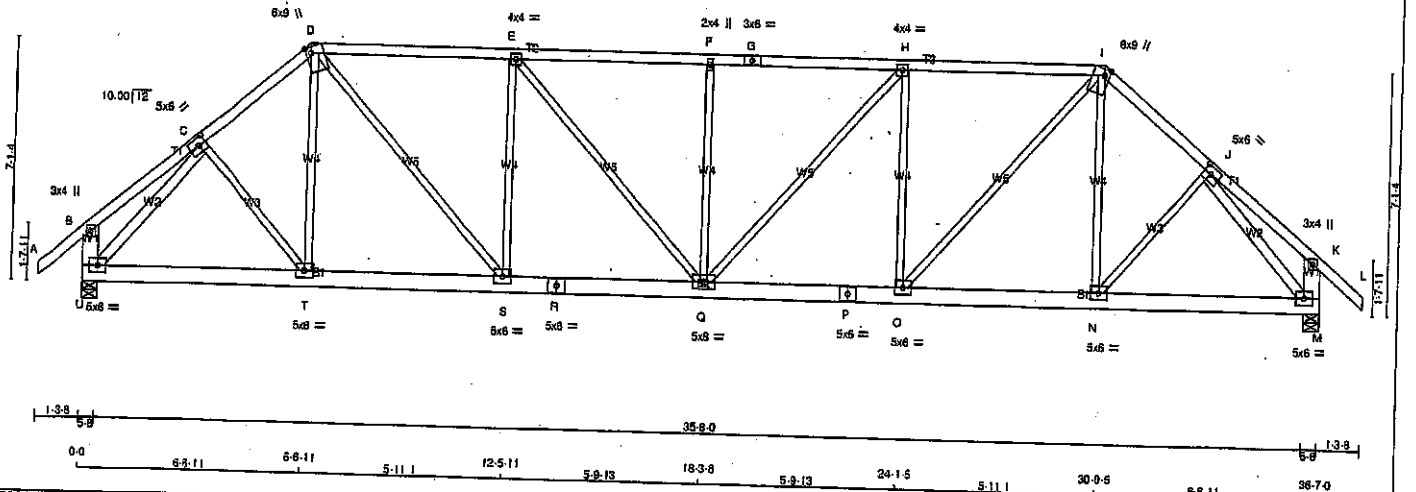
JSI METAL = 0.56 (J) (INPUT = 1.00)



Structural component only
DWG# T-2108260

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
417471	T24	1	1	GREENPARK HOMES		
Tamarack Roof Truss, Burlington						

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 ID:V2rMizHRYm36MOaXZ46o4AzZQVV-7Z5d0ePerm0Xw3GGVRNzC5Nbr1XivdG6zy6rBEIzZ46
 1-3.8 0-0 3-4.9 3-4.9 3-2.1 6-6.11 5-11.1 12-5.11 5-9.13 18-3.8 5-9.13 24-1.5 5-11.1 30-0.5 3-2.1 33-2.7 3-4.9 36-7.0 37.10.8 1-3.8
 Scale = 1:51.7



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
U - C	2x4	DRY	No.2
J - M	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMWW-t	MT20	5.0	6.0	2.50	2.50
D TTWW+m	MT20	8.0	9.0	Edge	1.75
E TMWW-t	MT20	4.0	4.0		
F TMW+w	MT20	2.0	4.0		
G TS-t	MT20	3.0	6.0		
H TMWW-t	MT20	4.0	4.0		
I TTWW+m	MT20	3.0	9.0	Edge	1.75
J TMWW-t	MT20	5.0	6.0	2.50	2.50
K TMV+p	MT20	3.0	4.0		
M BMVW-t	MT20	5.0	6.0		
N O, S, T					
N BMWW-t	MT20	5.0	6.0		
P BS-t	MT20	5.0	6.0		
Q BMWWW-t	MT20	5.0	6.0		
R BS-t	MT20	5.0	6.0		
U BMVW-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
U	2144	0	2144	0	5-8	5-8
M	2144	0	2144	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
U	1514	1007.0	0.0	0.0	0.0	507.0	0.0
M	1514	1007.0	0.0	0.0	0.0	507.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.49 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (L)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (L)	
FR-TO		FROM TO	CSI (L)	FR-TO		CSI (L)	
A-B	0 / 41	-91.8	-91.8 0.13 (1)	C-T	0 / 154	0.03 (1)	
B-C	0 / 20	-91.8	-91.8 0.14 (1)	D-T	0 / 79	0.03 (4)	
C-D	-2148 / 0	-91.8	-91.8 0.21 (1)	E-T	0 / 79	0.03 (4)	
D-E	-2517 / 0	-91.8	-91.8 0.66 (1)	F-T	0 / 154	0.03 (1)	
E-F	-2771 / 0	-91.8	-91.8 0.69 (1)	G-T	-2399 / 0	0.87 (1)	
F-G	-2771 / 0	-91.8	-91.8 0.69 (1)	H-T	-2399 / 0	0.87 (1)	
G-H	-2771 / 0	-91.8	-91.8 0.69 (1)	I-T	0 / 1341	0.30 (1)	
H-I	-2517 / 0	-91.8	-91.8 0.66 (1)	J-T	0 / 1341	0.30 (1)	
I-J	-2148 / 0	-91.8	-91.8 0.21 (1)	K-T	-900 / 0	0.74 (1)	
J-K	0 / 20	-91.8	-91.8 0.14 (1)	L-T	-900 / 0	0.74 (1)	
K-L	0 / 41	-91.8	-91.8 0.13 (1)	M-T	0 / 388	0.09 (1)	
U-B	-244 / 0	0.0	0.0 0.02 (1)	N-T	0 / 388	0.09 (1)	
M-K	-244 / 0	0.0	0.0 0.02 (1)	O-F	-491 / 0	0.40 (1)	
U-T	0 / 1528	-18.5	-18.5 0.23 (1)				
T-S	0 / 1630	-18.5	-18.5 0.24 (1)				
S-R	0 / 2517	-18.5	-18.5 0.34 (1)				
R-Q	0 / 2517	-18.5	-18.5 0.34 (1)				
Q-P	0 / 2517	-18.5	-18.5 0.34 (1)				
P-O	0 / 2517	-18.5	-18.5 0.34 (1)				
O-N	0 / 1630	-18.5	-18.5 0.24 (1)				
N-M	0 / 1528	-18.5	-18.5 0.23 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBC 2015, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 088-14
 - TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.22")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.13")
 ALLOWABLE DEFL. (TL) = L/360 (1.22")
 CALCULATED VERT. DEFL. (TL) = L/699 (0.24")
 CSI: TC=0.69/1.00 (F-H:1), BC=0.34/1.00 (O-S:1), WB=0.74/1.00 (E-S:1), SSI=0.26/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PS) (PL) (PL)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

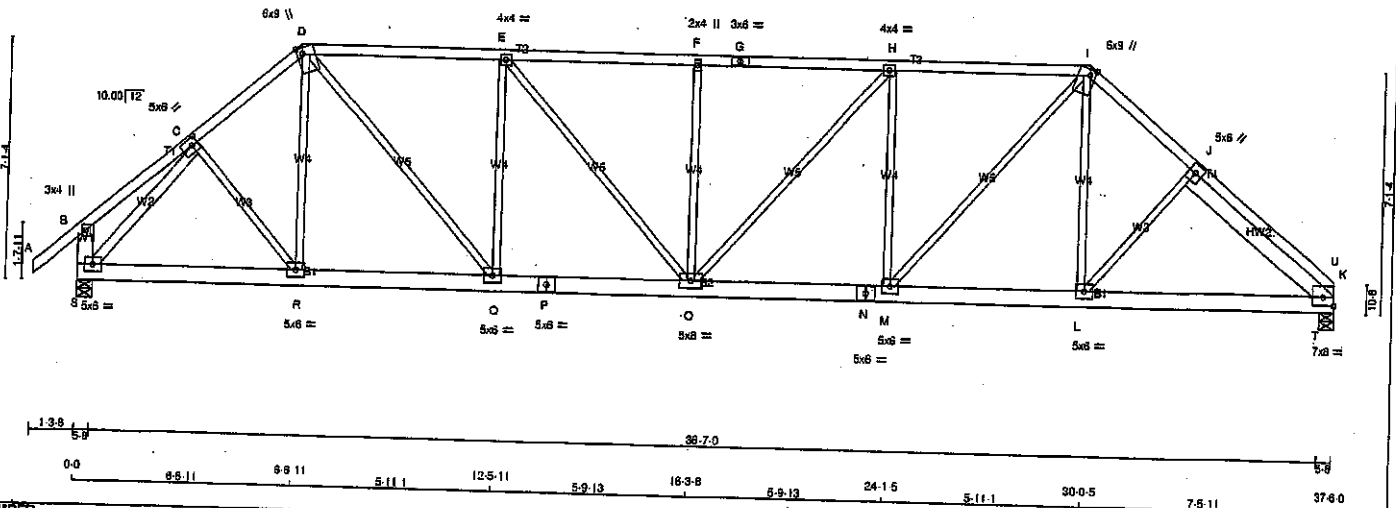
JSI GRIP = 0.89 (J) (INPUT = 0.90)
 JSI METAL = 0.54 (J) (INPUT = 1.00)



Structural component only
 DWG# T-2108261

JOB NAME 417471	TRUSS NAME T24X	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MTEK Industries, Inc. Sat Mar 20 10:33:56 2021 Page 1
 ID: V2mZmzHRYm36MOeXZ48o4AzZQVV-7Z5d0ePam0Xw3GGVRNzCnBqjX17dFTzy6rBeizZ4a
 Scale = 1:22.1



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
S - B	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2
REINFORCING MEMBERS			
HW2	2x6	DRY	No.2
ALL WEBS EXCEPT			
S - C	2x4	DRY	No.2

PLATES (Table in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMW-p	MT20	3.0	4.0
C	TMW-W-t	MT20	5.0	6.0
D	TTWW-m	MT20	6.0	9.0
E	TMW-W-t	MT20	4.0	4.0
F	TMW-w	MT20	2.0	4.0
G	TS-t	MT20	3.0	6.0
H	TMW-W-t	MT20	4.0	4.0
I	TTWW-m	MT20	6.0	9.0
J	TMW-W-t	MT20	5.0	6.0
K	TMBMW-t	MT20	7.0	8.0
L, M, Q, R				
N	BMW-W-t	MT20	5.0	6.0
O	BS-t	MT20	5.0	6.0
P	BMW-W-t	MT20	5.0	6.0
S	BMW-W-t	MT20	5.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)
 1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
FACTORED				MAXIMUM FACTORED			
GROSS REACTION				GROSS REACTION			
JT	VERT	HORZ		JT	VERT	HORZ	
K	2067	0		K	2067	0	
S	2194	0		S	2194	0	
UNFACTORED REACTIONS							
1ST CASE MAX. MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	1462	959	0	0	0	502	0
S	1549	1030	0	0	0	519	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS, K, S
 BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.40 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING							
TOTAL LOAD CASES: (4)							
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO				FR-TO			
A-B	0/41	-91.8	-91.8 0.13 (1)	C-R	0/166	0.04 (1)	
B-C	0/20	-91.8	-91.8 0.14 (1)	D-Q	0/76	0.03 (4)	
C-D	-2213/0	-91.8	-91.8 0.21 (1)	E-Q	0/1408	0.32 (1)	
D-E	-2611/0	-91.8	-91.8 0.67 (1)	F-Q	-950/0	0.78 (1)	
E-F	-2908/0	-91.8	-91.8 0.71 (1)	G-Q	0/455	0.10 (1)	
F-G	-2908/0	-91.8	-91.8 0.71 (1)	H-Q	-491/0	0.40 (1)	
G-H	-2908/0	-91.8	-91.8 0.71 (1)	I-Q	0/323	0.07 (1)	
H-I	-2697/0	-91.8	-91.8 0.68 (1)	J-Q	-850/0	0.70 (1)	
I-J	-2447/0	-91.8	-91.8 0.24 (1)	K-Q	0/1265	0.28 (1)	
J-U	-1508/0	-91.8	-91.8 0.19 (1)	L-Q	0/275	0.08 (1)	
U-K	-2219/0	-91.8	-91.8 0.14 (1)	M-Q	-151/0	0.07 (1)	
S-B	-244/0	0.0	0.0 0.02 (1)	N-Q	-2464/0	0.68 (1)	
S-R	0/1570	-18.5	-18.5 0.23 (1)	O-Q	0/737	0.00 (1)	
R-Q	0/1679	-18.5	-18.5 0.25 (1)	P-Q	-1152/0	0.31 (1)	
Q-P	0/2811	-18.5	-18.5 0.35 (1)				
P-O	0/2811	-18.5	-18.5 0.35 (1)				
O-N	0/2697	-18.5	-18.5 0.37 (1)				
N-M	0/2697	-18.5	-18.5 0.37 (1)				
M-L	0/1880	-18.5	-18.5 0.29 (1)				
L-T	0/1951	-18.5	-18.5 0.31 (1)				
T-K	0/1162	-18.5	-18.5 0.19 (1)				

TOTAL WEIGHT = 196 lb

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF
SPACING	= 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12
 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2018
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.25")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.14")
 ALLOWABLE DEFL. (TL) = L/360 (1.25")
 CALCULATED VERT. DEFL. (TL) = L/999 (0.25")

CSI: TC=0.71/1.00 (F-H); BC=0.37/1.00 (M-O); WB=0.78/1.00 (E-Q); SS=0.26/1.00 (H-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE GRIP (DRY)	SHEAR SECTION	(PSI)	(PL)	(PL)	
MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN	
MT20	650	371	1747	788	1987

PLATE PLACEMENT TOL = 0.250 inches

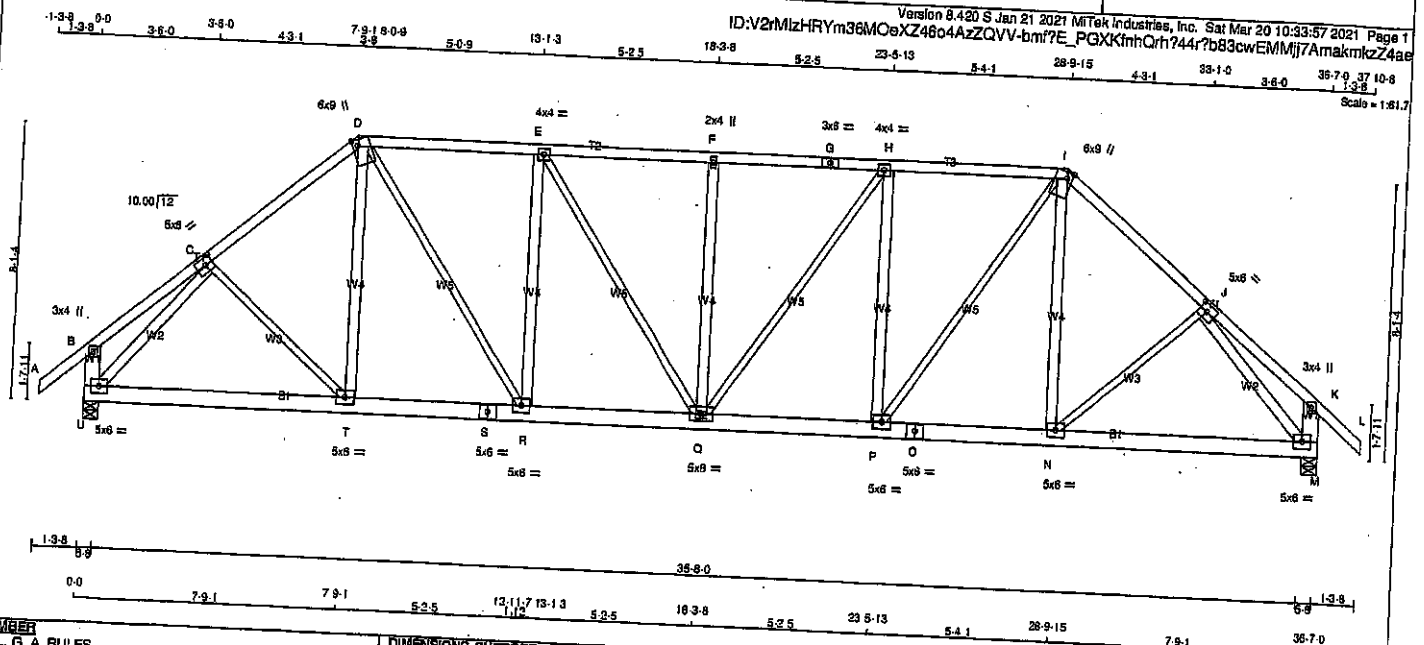
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (I) (INPUT = 0.90)
 JSI METAL = 0.55 (C) (INPUT = 1.00)



Structural component only
 DWG# T-2108262

JOB NAME 417471	TRUSS NAME T25	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			



N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
U - S	2x6	DRY	No.2
S - O	2x6	DRY	No.2
O - M	2x6	DRY	No.2

ALL WEBS	2x4	DRY	No.2
EXCEPT			
C - T	2x3	DRY	No.2
N - J	2x3	DRY	No.2
P - R	2x3	DRY	No.2
Q - H	2x3	DRY	No.2
E - Q	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV+p	MT20	3.0	4.0	
C TMVW-t	MT20	5.0	5.0	2.50 2.75
D TTWW+m	MT20	6.0	9.0	Edge 1.75
E TMVW-t	MT20	4.0	4.0	
F TS-t	MT20	2.0	4.0	
G TMVW-t	MT20	3.0	6.0	
H TTWW+m	MT20	4.0	4.0	
I TMVW-t	MT20	5.0	9.0	Edge 1.75
J TMVW-t	MT20	5.0	6.0	2.50 2.75
K TMV+p	MT20	3.0	4.0	
M BMVW-t	MT20	5.0	6.0	
N, P, R, T				
N BMVW-t	MT20	5.0	6.0	
O BS-t	MT20	5.0	6.0	
Q BMVW-t	MT20	5.0	6.0	
S BS-t	MT20	5.0	6.0	
U BMVW-t	MT20	5.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT VERT	2144	DOWN	2144	UPLIFT	5-8	IN-SX	5-8
U	2144	0	2144	0	5-8	IN-SX	5-8
M	2144	0	2144	0	5-8	IN-SX	5-8

UNFACTORED REACTIONS

1ST CASE		MAX MIN COMPONENT REACTIONS	
JT COMBINED		SNOW	LIVE
U	1514	1007.0	0.0
M	1514	1007.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO	
A-B	0.41	-91.8	10.00	C-T	0.85
B-C	0.31	-91.8	10.00	T-D	0.116
C-D	-2126.0	-91.8	4.37	N-I	0.116
D-E	-2230.0	-91.8	4.06	N-J	0.85
E-F	-2407.0	-91.8	3.92	U-C	-2411.0
F-G	-2407.0	-91.8	3.92	J-M	-2411.0
G-H	-2407.0	-91.8	4.06	D-R	0.1084
H-I	-2230.0	-91.8	4.37	P-H	-808.0
I-J	-2126.0	-91.8	10.00	R-E	-808.0
J-K	0.31	-91.8	10.00	Q-H	0.317
K-L	0.41	-91.8	7.81	E-Q	0.07 (1)
U-B	-234.0	0.0	0.02 (1)	Q-F	-436.0
M-K	-234.0	0.0	0.02 (1)		
U-T	0.1547	-18.5	10.00		
T-S	0.1513	-18.5	10.00		
S-R	0.1513	-18.5	10.00		
R-Q	0.2230	-18.5	10.00		
Q-P	0.2230	-18.5	10.00		
P-O	0.1513	-18.5	10.00		
O-N	0.1513	-18.5	10.00		
N-M	0.1547	-18.5	10.00		

TOTAL WEIGHT = 212 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/989 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TO=0.51/1.00 (E-F:1), BC=0.30/1.00 (Q-P:1), WB=0.71/1.00 (J-M:1), SS=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

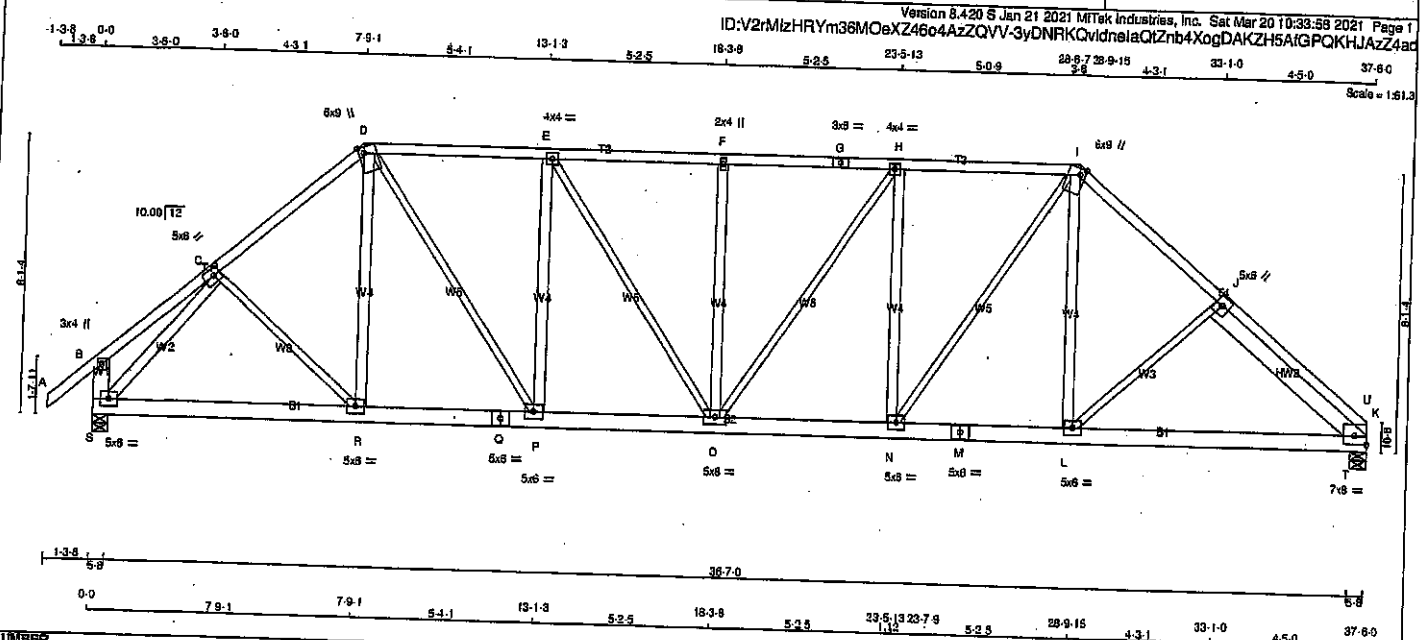
JSI GRIP = 0.89 (J) (INPUT = 0.80)

JSI METAL = 0.54 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108263

JOB NAME 417471	TRUSS NAME T25X	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						



LUMBER		
N. L. G. A. RULES	CHORDS	SIZE
A - D	2x4	DRY
D - G	2x4	DRY
G - I	2x4	DRY
I - K	2x4	DRY
S - B	2x6	DRY
S - Q	2x6	DRY
Q - M	2x6	DRY
M - K	2x6	DRY

REINFORCING MEMBERS		
HW2	2x6	DRY
ALL WEBS 2x3 DRY		
EXCEPT		
R - D	2x4	DRY
L - I	2x4	DRY
S - C	2x4	DRY
N - H	2x4	DRY
P - E	2x4	DRY
O - F	2x4	DRY

DRY, SEASONED LUMBER.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMW+P	MT20	3.0	4.0		
C TMW+V-1	MT20	5.0	6.0	2.50	2.50
D TTWW+m	MT20	6.0	8.0	Edge	1.75
E TMW+V-1	MT20	4.0	4.0		
F TMW+V-1	MT20	2.0	4.0		
G TS-1	MT20	3.0	6.0		
H TMW+V-1	MT20	4.0	4.0		
I TTWW+m	MT20	6.0	8.0	Edge	1.75
J TMW+V-1	MT20	5.0	6.0		
K TMBW+V-1	MT20	7.0	8.0	3.00	Edge
L, N, P, R					
L BMW+V-1	MT20	5.0	6.0		
M BS-1	MT20	5.0	6.0		
O BMW+V-1	MT20	5.0	6.0		
Q BS-1	MT20	5.0	6.0		
S BMW+V-1	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	GROSS REACTION	JT	GROSS REACTION	BRG	BRG	BRG	BRG
K	2087	0	2087	0	0	5-8	5-8
S	2194	0	2194	0	0	5-8	5-8

UNFACTORED REACTIONS		MAX/MIN COMPONENT REACTIONS	
JT	1ST CASE	JT	1ST CASE
K	1462	K	1462
S	1549	S	1549

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED	MEMB.	MAX. FACTORED	MEMB.	MAX. FACTORED
FR-TO	FORCE (LBS)	FR-TO	FORCE (LBS)	FR-TO	FORCE (LBS)
A-B	0/41	A-B	0/41	A-B	0/41
B-C	0/31	B-C	0/31	B-C	0/31
C-D	-2193/0	C-D	-2193/0	C-D	-2193/0
D-E	-2315/0	D-E	-2315/0	D-E	-2315/0
E-F	-2526/0	E-F	-2526/0	E-F	-2526/0
F-G	-2526/0	F-G	-2526/0	F-G	-2526/0
G-H	-2526/0	G-H	-2526/0	G-H	-2526/0
H-I	-2373/0	H-I	-2373/0	H-I	-2373/0
I-J	-2373/0	I-J	-2373/0	I-J	-2373/0
J-U	-1531/0	J-U	-1531/0	J-U	-1531/0
U-K	-2304/0	U-K	-2304/0	U-K	-2304/0
S-B	-234/0	S-B	-234/0	S-B	-234/0
S-R	0/1589	S-R	0/1589	S-R	0/1589
R-Q	0/1884	R-Q	0/1884	R-Q	0/1884
Q-P	0/1864	Q-P	0/1864	Q-P	0/1864
P-O	0/2316	P-O	0/2316	P-O	0/2316
O-N	0/2383	O-N	0/2383	O-N	0/2383
N-M	0/1805	N-M	0/1805	N-M	0/1805
M-L	0/1805	M-L	0/1805	M-L	0/1805
L-T	0/1805	L-T	0/1805	L-T	0/1805
T-K	0/1191	T-K	0/1191	T-K	0/1191

DESIGN CRITERIA

SPECIFIED LOADS:		
TOP CH.	LL	= 25.6 PSF
	DL	= 6.0 PSF
BOT CH.	LL	= 0.0 PSF
	DL	= 7.4 PSF
TOTAL LOAD = 39.0 PSF		

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00.12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.25")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.25")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")
CSI: TC=0.52/1.00 (F-H:1), BC=0.32/1.00 (N-O:1), WB=0.73/1.00 (C-S:1), SS=0.23/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE GRIP(DRY)	SHEAR	SECTION	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PSI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	798	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.56 (C) (INPUT = 1.00)

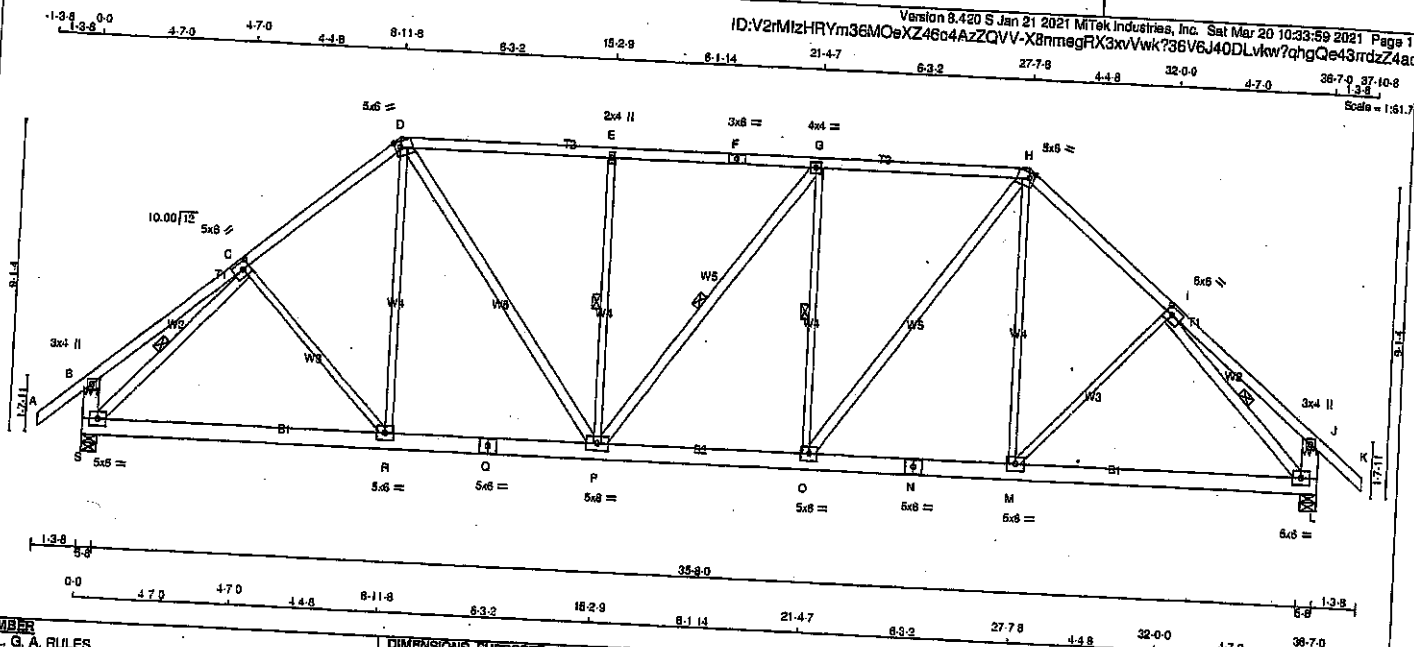


Structural component only
DWG# T-2108264

JOB NAME 417471	TRUSS NAME T26	QUANTITY 9	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 10:33:59 2021 Page 1
ID:V2rMtzHRYm36MOeXZ46o4AzZQVV-X8rmegRX3vVwk?36V6J40DLvkW?qhGQe43rdzZ4ac



TOTAL WEIGHT = 9 X 209 = 1878 LB

LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
S - C	2x6	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
D - P	2x4	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF
O - H	2x4	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

PLATE TYPE	PLATES	W	LEN	Y	X
B TMW-p	MT20	3.0	4.0		
C TMW-W-1	MT20	5.0	8.0	2.50	2.50
D TTW-W-m	MT20	5.0	8.0	2.00	1.75
E TMW-w	MT20	2.0	4.0		
F TS-1	MT20	3.0	6.0		
G TMW-W-1	MT20	4.0	4.0		
H TTW-W-m	MT20	5.0	8.0	2.00	1.75
I TMW-W-1	MT20	5.0	8.0	2.50	2.50
J TMW-p	MT20	3.0	4.0		
L BMWV-1	MT20	5.0	8.0		
M O-R	MT20	5.0	8.0		
N BMWV-1	MT20	5.0	8.0		
P BS-1	MT20	5.0	8.0		
Q BS-1	MT20	5.0	8.0		
S BMWV-1	MT20	5.0	8.0		

NOTES

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	GROSS REACTION		MAXIMUM FACTORED		INPUT	REQD
	VERT	HORZ	DOWN	HORZ		
JT	2144	0	2144	0	5-8	5-8
L	2144	0	2144	0	5-8	5-8
S	2144	0	2144	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX/MIN COMPONENT REACTIONS		PERM.LIVE	WIND	DEAD	SOIL
	SNOW	LIVE				
JT	1514	1007.0	0.0	0.0	507.0	0.0
L	1514	1007.0	0.0	0.0	507.0	0.0
S	1514	1007.0	0.0	0.0	507.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P, G-P, G-O, C-S, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)		MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PL)
		FROM	TO				
FR-TO							
A-B	0.41	-91.8	-91.8	10.00	R-D	0.212	0.08 (4)
B-C	0.31	-91.8	-91.8	10.00	D-P	0.878	0.14 (1)
C-D	-2091.0	-91.8	-91.8	4.30	P-E	-618.0	0.32 (1)
D-E	-2093.0	-91.8	-91.8	3.99	E-G	-71.0	0.00 (1)
E-F	-2094.0	-91.8	-91.8	3.97	G-H	-613.0	0.32 (1)
F-G	-2094.0	-91.8	-91.8	3.97	H-I	0.888	0.14 (1)
G-H	-2097.0	-91.8	-91.8	4.30	I-L	0.208	0.08 (4)
H-I	-2097.0	-91.8	-91.8	10.00	S-C	-54.38	0.04 (1)
I-J	-2090.0	-91.8	-91.8	10.00	M-L	-54.38	0.04 (1)
J-K	0.41	-91.8	-91.8	7.81	I-L	-2411.0	0.49 (1)
K-L	-283.0	0.0	0.0	7.81			
S-B	-283.0	0.0	0.0	7.81			
L-J	-283.0	0.0	0.0	7.81			
S-R	0.1516	-18.5	-18.5	10.00			
R-O	0.1581	-18.5	-18.5	10.00			
Q-P	0.1581	-18.5	-18.5	10.00			
P-O	0.2098	-18.5	-18.5	10.00			
O-N	0.1581	-18.5	-18.5	10.00			
N-M	0.1581	-18.5	-18.5	10.00			
M-L	0.1615	-18.5	-18.5	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TO=0.65/1.00 (G-H:1), BC=0.29/1.00 (O-P:1), WB=0.49/1.00 (C-S:1), SSI=0.27/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

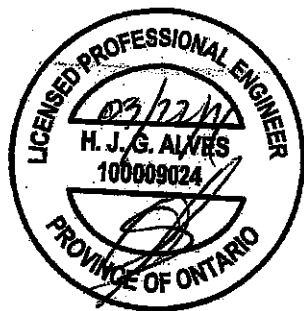
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

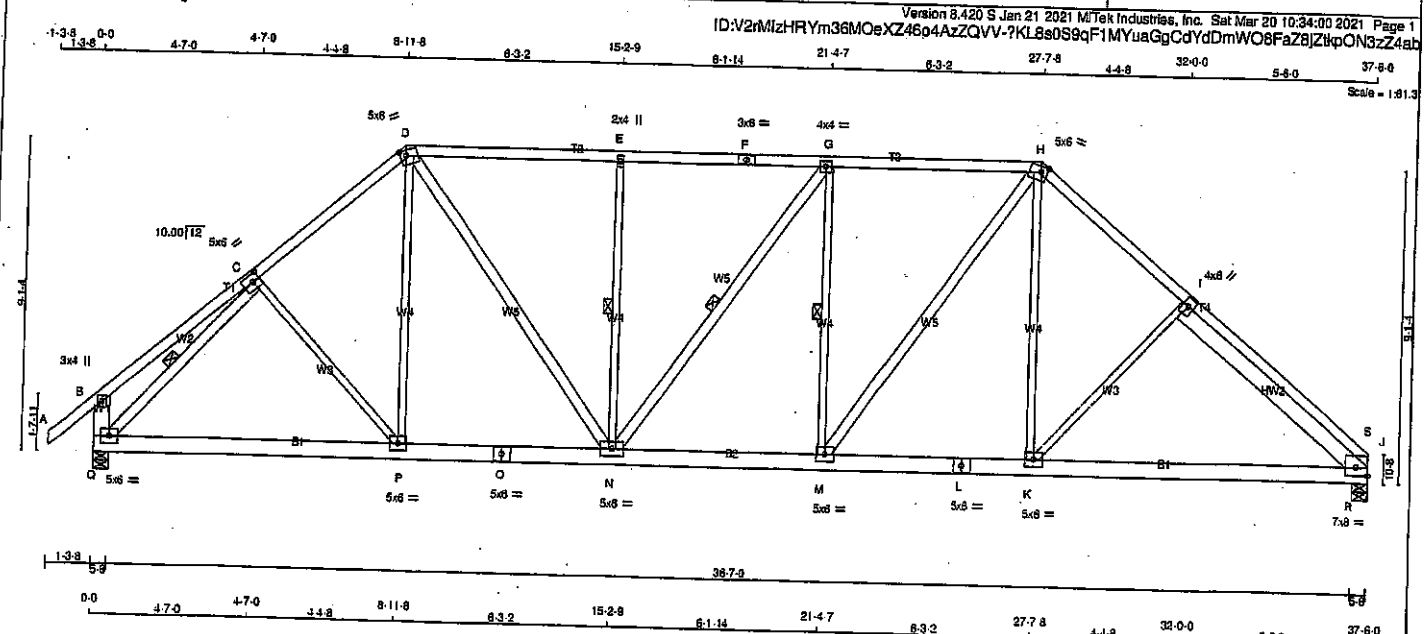
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (H) (INPUT = 0.90)
JSI METAL = 0.54 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108265

JOB NAME 417471	TRUSS NAME T26X	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



Version 8.420 S Jan 21 2021 M/Tek Industries, Inc. Sat Mar 20 10:34:00 2021 Page 1
ID:V2mMzHRYm36MOsXZ46p4AZQVV-7KL8s0S9qF1MYuaGgCdYdDmW08FaZ8JZkpON3zZ4ab
Scale = 1:12.5
TOTAL WEIGHT = 213 lb

LUMBER				REINFORCING MEMBERS				ALL WEBS EXCEPT				DRY: SEASONED LUMBER.			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.	SPF	NO.2	SPF	NO.2	SPF	NO.2	SPF	NO.2	SPF	NO.2	
A - D	2x4	DRY	No.2	SPF											
D - F	2x4	DRY	No.2	SPF											
F - H	2x4	DRY	No.2	SPF											
H - J	2x4	DRY	No.2	SPF											
Q - B	2x6	DRY	No.2	SPF											
O - O	2x6	DRY	No.2	SPF											
L - L	2x6	DRY	No.2	SPF											
L - J	2x6	DRY	No.2	SPF											
REINFORCING MEMBERS				ALL WEBS EXCEPT				DRY: SEASONED LUMBER.							
HWZ	2x6	DRY	No.2	SPF											
D - N	2x4	DRY	No.2	SPF											
N - G	2x4	DRY	No.2	SPF											
M - H	2x4	DRY	No.2	SPF											
Q - C	2x4	DRY	No.2	SPF											

BEARINGS									
FACTORED GROSS REACTION					MAXIMUM FACTORED GROSS REACTION				
JT	VERT	HORZ	DOWN	UPLIFT	BRG	IN-SX	BRG	IN-SX	REQRD
J	2067	0	2067	0	0	5-8	5-8	5-8	5-8
Q	2194	0	2194	0	0	5-8	5-8	5-8	5-8
UNFACTORED REACTIONS									
1ST CASE		MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
J	1462	959.0	0.0	0.0	0.0	502.0	0.0		
Q	1549	1030.0	0.0	0.0	0.0	519.0	0.0		
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, Q									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.86 FT.									
MAX. UNBRACED BOTTOM CHORD									

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF
SPACING = 24.0 IN. C/C	
LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12	
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015	

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF
SPACING = 24.0 IN C/C
LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015
THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPIC 2014
(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

TYPE PLATES					END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW					(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																								
TMV+p	MT20	3.0	4.0																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																															</

UNFACTORED REACTIONS				1ST CASE				MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	JT	COMBINED	SNOW	LIVE	JT	COMBINED	SNOW	LIVE	JT	COMBINED	SNOW	LIVE
JT	1462	959.0	0.0	JT	1462	959.0	0.0	JT	1462	959.0	0.0	JT	1462	959.0	0.0
J	1549	1030.0	0.0	J	1549	1030.0	0.0	J	1549	1030.0	0.0	J	1549	1030.0	0.0
Q	1549	1030.0	0.0	Q	1549	1030.0	0.0	Q	1549	1030.0	0.0	Q	1549	1030.0	0.0



PROFESSIONAL ENGINEER

Q-P	0 / 1661	-18.5	-18.5	0.29 (1)	10.00
P-O	0 / 1639	-18.5	-18.5	0.27 (1)	10.00
O-N	0 / 1639	-18.5	-18.5	0.27 (1)	10.00
N-M	0 / 2221	-18.5	-18.5	0.31 (1)	10.00
M-L	0 / 1744	-18.5	-18.5	0.30 (1)	10.00
L-K	0 / 1744	-18.5	-18.5	0.30 (1)	10.00
K-R	0 / 1938	-18.5	-18.5	0.33 (1)	10.00
R-J	0 / 1289	-18.5	-18.5	0.18 (4)	10.00

0 / 698

0.00 (1)

0.46 (1)

R-S

R-I

TRUSS MANUFACTURING PLANT.			
NAIL VALUES			
PLATE GRIP (DRY)	SHEAR	SECTION	
(FBI)	(F/L)	(F/L)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873
PLATE PLACEMENT TOL = 0.250 inches			

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMVW-i	MT20	5.0	6.0	2.50	2.50
D TTWW-m	MT20	5.0	6.0		1.75
E TMV+w	MT20	2.0	4.0		
F TS-i	MT20	3.0	6.0		
G TMVW-i	MT20	4.0	4.0		
H TTWW-m	MT20	5.0	6.0	2.00	2.00
I TMVW+i	MT20	4.0	6.0		
J TBMVW-i	MT20	7.0	8.0	2.75	Edge
K, M, P					
K BMVW-i	MT20	5.0	6.0		
L BS-i	MT20	5.0	6.0		
N BMVW-w	MT20	5.0	6.0		
O BS-i	MT20	5.0	6.0		
Q BMVW-i	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES

1. (1)

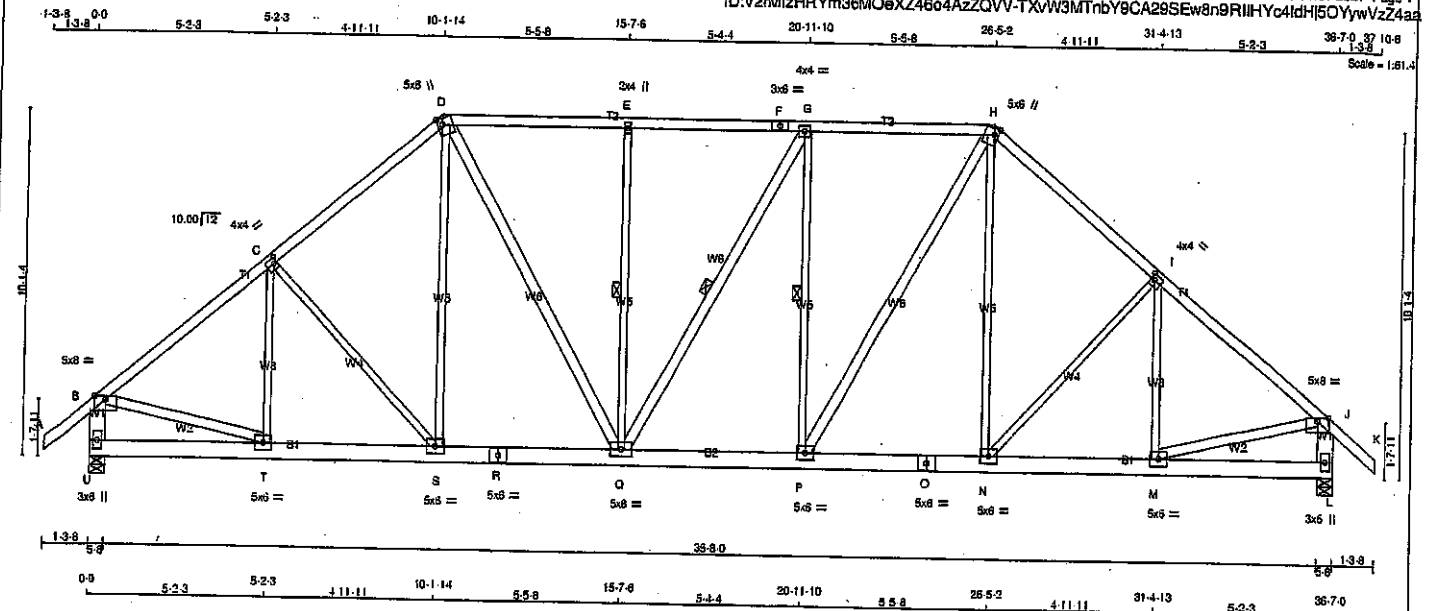
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2108266

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417471	T27	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Set Mar 20 10:34:01 2021 Page 1
ID:V2rMlzHRYm36MOeXZ46e4AZZQVV-TXvW3MTnbY9CA29SEw8n9RIHyc4IdHj50YyVzZ4as



N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x8	DRY	No.2	SPF
L - J	2x8	DRY	No.2	SPF
U - R	2x8	DRY	No.2	SPF
R - O	2x8	DRY	No.2	SPF
O - L	2x8	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - Q	2x4	DRY	No.2	SPF
Q - G	2x4	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF
B - T	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-1	MT20	4.0	4.0	2.00	1.25
D	TTWV-m	MT20	5.0	8.0	2.25	1.50
E	TMVW-m	MT20	2.0	4.0		
F	TS-1	MT20	3.0	6.0		
G	TMVW-1	MT20	4.0	4.0		
H	TTWV-m	MT20	5.0	8.0	2.25	1.50
I	TMVW-1	MT20	4.0	4.0	2.00	1.25
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1-p	MT20	3.0	8.0		
M, N, P, S, T						
M	BMVW-1	MT20	5.0	8.0		
O	BS-1	MT20	5.0	8.0		
Q	BMVW-1	MT20	5.0	8.0		
R	BS-1	MT20	5.0	8.0		
U	BMV1-p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)



Structural component only
DWG# T-2108267

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	GROSS REACTION	JT	GROSS REACTION	BRG	BRG	IN-SX	IN-SX
U	VERT 2144	DOWN 2144	0	0	5-8	5-8	5-8
L	VERT 2144	DOWN 2144	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS		MAX. MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	JT	COMBINED	SNOW	LIVE	JT	COMBINED	SNOW	LIVE
U	1514	1007	0	0	0	0	0	0	507	0	0
L	1514	1007	0	0	0	0	0	0	507	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, G-Q, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0.41	-91.8	-91.8	0.13 (1)	10.00	T-C	-308.0	0.16 (1)
B-C	-2139.0	-91.8	-91.8	0.40 (1)	4.30	C-S	-213.0	0.23 (1)
C-D	-2028.0	-91.8	-91.8	0.38 (1)	4.40	S-D	0.270	0.05 (1)
D-E	-1882.0	-91.8	-91.8	0.36 (1)	4.54	D-Q	0.720	0.12 (1)
E-F	-1883.0	-91.8	-91.8	0.33 (1)	4.58	Q-E	-534.0	0.36 (1)
F-G	-1883.0	-91.8	-91.8	0.33 (1)	4.58	Q-G	-9.0	0.01 (1)
G-H	-1887.0	-91.8	-91.8	0.36 (1)	4.54	P-G	-531.0	0.36 (1)
H-I	-2027.0	-91.8	-91.8	0.38 (1)	4.41	P-H	0.731	0.12 (1)
I-J	-2140.0	-91.8	-91.8	0.40 (1)	4.30	N-H	0.281	0.06 (1)
J-K	0.41	-91.8	-91.8	0.13 (1)	10.00	N-I	-214.0	0.24 (1)
U-B	-2095.0	0.0	0.0	0.14 (1)	7.01	M-I	-306.0	0.16 (1)
L-J	-2098.0	0.0	0.0	0.14 (1)	7.01	B-T	0.1716	0.28 (1)
						M-J	0.1717	0.28 (1)
U-T	0.0	-18.5	-18.5	0.05 (4)	10.00			
T-S	0.1670	-18.5	-18.5	0.23 (1)	10.00			
S-R	0.1530	-18.5	-18.5	0.21 (1)	10.00			
R-O	0.1530	-18.5	-18.5	0.21 (1)	10.00			
Q-P	0.1887	-18.5	-18.5	0.25 (1)	10.00			
P-O	0.1529	-18.5	-18.5	0.21 (1)	10.00			
O-N	0.1529	-18.5	-18.5	0.21 (1)	10.00			
N-M	0.1671	-18.5	-18.5	0.22 (1)	10.00			
M-L	0.0	-18.5	-18.5	0.05 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.40/1.00 (I-J-1), BC=0.25/1.00 (P-Q-1), WB=0.38/1.00 (E-Q-1), SS=0.23/1.00 (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

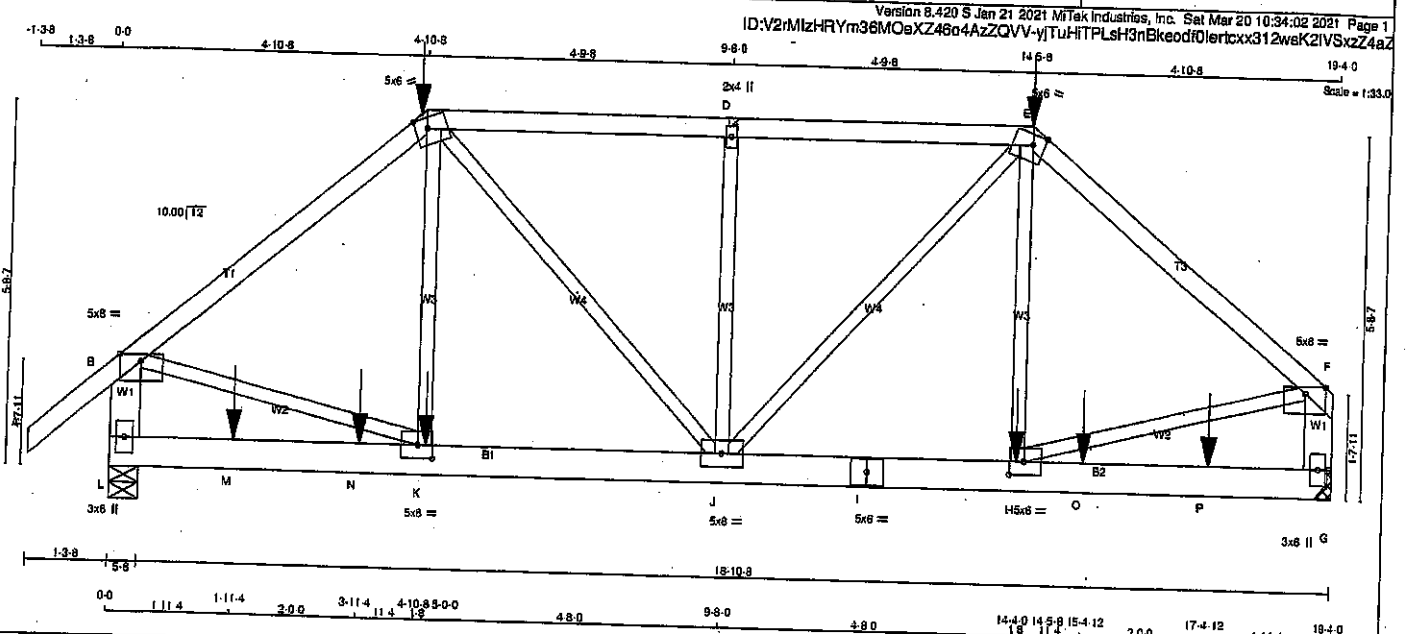
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (D) (INPUT = 0.90)

JSI METAL = 0.98 (M) (INPUT = 1.00)

JOB NAME 417471	TRUSS NAME T28	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			



CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - F	2x4	DRY No.2	SPF
F - B	2x6	DRY No.2	SPF
B - F	2x6	DRY No.2	SPF
G - I	2x8	DRY No.2	SPF
I - G	2x8	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY, SEASONED LUMBER.

PLATES (table in inches)	W	LEN	Y	X
JT TYPE PLATES				
S TMVW-p MT20	5.0	8.0	Edge	
C TMVW-m MT20	5.0	6.0	2.00	2.25
D TMVW-w MT20	2.0	4.0		
E TMVW-m MT20	5.0	6.0	2.00	2.25
F TMVW-p MT20	5.0	8.0	Edge	
G BMV1-p MT20	3.0	6.0		
H BMVW-t MT20	5.0	6.0	2.50	2.75
I BS-1 MT20	5.0	8.0		
J BMVW-t MT20	5.0	8.0		
K BMVW-t MT20	5.0	8.0	2.50	2.75
L BMV1-p MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
L	2376	0	2376	0	5-8	5-8
G	2249	0	2249	0	-MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LOAGE MAX/MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW				
L	1874	1134.0	0.0	0.0	540.0	0.0
G	1587	1063.0	0.0	0.0	523.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 MAX (PLF)		MAX. UNBRACED LENGTH FR-TO	WEBS MEMB. MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
	FR-TO	FROM	TO	CSI (LC)			
A-B	0 / 41	-91.8	-91.8	0.14 (1)	10.00	K-C	0 / 535
B-C	-2359 / 0	-91.8	-91.8	0.62 (1)	3.82	C-J	0 / 412
C-D	-2095 / 0	-91.8	-91.8	0.47 (1)	4.12	J-D	-523 / 0
D-E	-2095 / 0	-91.8	-91.8	0.47 (1)	4.12	E-E	0 / 412
E-F	-2359 / 0	-91.8	-91.8	0.62 (1)	3.82	H-E	0 / 535
L-B	-2309 / 0	0.0	0.0	0.17 (1)	6.71	B-K	0 / 1882
G-F	-2182 / 0	0.0	0.0	0.16 (1)	6.87	H-F	0 / 1882
L-M	0 / 0	-18.5	-18.5	0.10 (4)	10.00		
M-N	0 / 0	-18.5	-18.5	0.10 (4)	10.00		
N-K	0 / 0	-18.5	-18.5	0.10 (4)	10.00		
K-J	0 / 1819	-18.5	-18.5	0.27 (1)	10.00		
J-I	0 / 1819	-18.5	-18.5	0.27 (1)	10.00		
I-H	0 / 1819	-18.5	-18.5	0.27 (1)	10.00		
H-O	0 / 0	-18.5	-18.5	0.10 (4)	10.00		
O-P	0 / 0	-18.5	-18.5	0.10 (4)	10.00		
P-G	0 / 0	-18.5	-18.5	0.10 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-10.8	-188	-188		BACK	VERT	TOTAL		C1
E	14-5.8	-188	-188		BACK	VERT	TOTAL		C1
H	14-4.0	-603	-603		BACK	VERT	TOTAL		C1
K	5-0.0	-603	-603		BACK	VERT	TOTAL		C1
M	1-11.4	-21	-21		BACK	VERT	TOTAL		C1
N	3-11.4	-21	-21		BACK	VERT	TOTAL		C1
O	15-4.12	-21	-21		BACK	VERT	TOTAL		C1
P	17-4.12	-21	-21		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00.12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BOBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.84")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.84")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CS1: TC=0.62/1.00 (E-F:1), BC=0.27/1.00 (J-K:1), WB=0.48/1.00 (F-H:1), SS=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PL)	SECTION (PL)
PSI	PLI	PLI
MAX MIN	MAX MIN	MAX MIN
650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.83 (H) (INPUT = 0.90)
JSI METAL = 0.41 (K) (INPUT = 1.00)



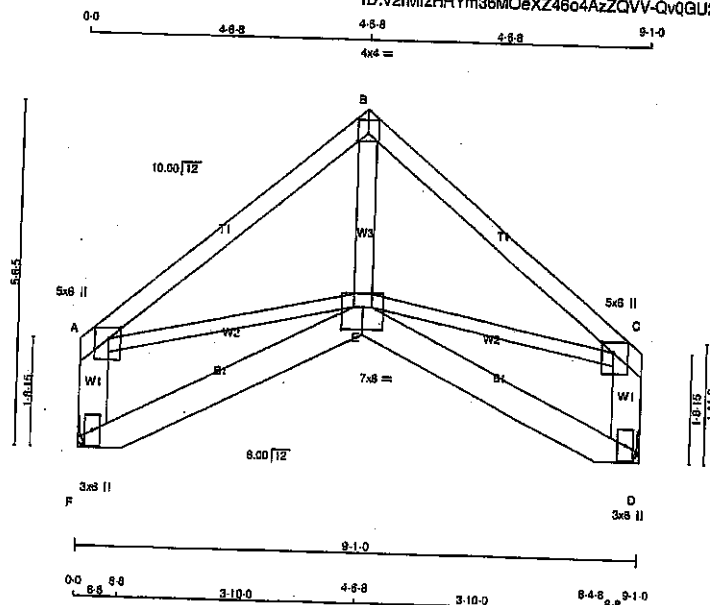
Structural component only
DWG# T-2108268

JOB NAME 417471	TRUSS NAME T29S	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/32



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY No.2	SPF
B - C	2x4	DRY No.2	SPF
F - A	2x6	DRY No.2	SPF
D - C	2x6	DRY No.2	SPF
F - E	2x6	DRY No.2	SPF
E - D	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT
E - B 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TTW+p	MT20	4.0	4.0	1.50	2.00
C	TMVW+p	MT20	5.0	6.0	2.00	2.25
D	BMV1+p	MT20	3.0	6.0		
E	BBVW+p	MT20	7.0	8.0	4.25	4.00
F	BMV1+p	MT20	3.0	6.0		

NOTES:

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
F	501	0	501	0	0	MECHANICAL
D	501	0	501	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LOASE		MAX. MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	354	232.0	0.0	0.0	0.0	122.0	0.0
D	354	232.0	0.0	0.0	0.0	122.0	0.0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	FR-TO	CHORDS		MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (L)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (L)
		FROM	TO					
A-B	-468 / 0	-91.8	-91.8	0.25 (1)	6.25	E-B	0 / 182	0.03 (4)
B-C	-468 / 0	-91.8	-91.8	0.25 (1)	6.25	A-E	0 / 363	0.08 (1)
F-A	-468 / 0	0.0	0.0	0.03 (1)	7.81	E-C	0 / 363	0.08 (1)
D-C	-468 / 0	0.0	0.0	0.03 (1)	7.81			
F-E	0 / 0	-18.5	-18.5	0.05 (4)	10.00			
E-D	0 / 0	-18.5	-18.5	0.05 (4)	10.00			

TOTAL WEIGHT = 3 X 46 = 144 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.30")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.30")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.25/1.00 (B-C:1), BC=0.05/1.00 (D-E:4), WB=0.08/1.00 (A-E:1), SSI=0.12/1.00 (B-C:1)

DOJ LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.82 (B) (INPUT = 0.90)

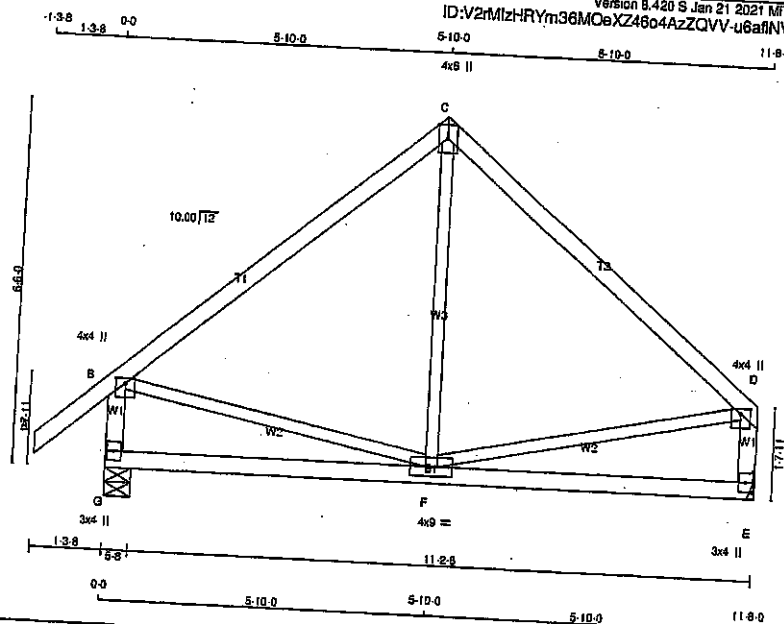
JSI METAL=0.13 (F) (INPUT = 1.00)



Structural component only
DWG# T-2108269

JOB NAME 417471	TRUSS NAME T30	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/32"

LUMBER			
N.L.G.A. RULES	SIZE	DRY	NO.2
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - B	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C	TTW+p	MT20	4.0	8.0	Edge
D	TMVW+p	MT20	4.0	4.0	1.00 2.00
E	BMV1+p	MT20	3.0	4.0	
F	BMVWW+	MT20	4.0	9.0	
G	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
G	770	0	770	0	0
E	643	0	643	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	542	369.0	0.0	0.0	0.0	173.0	0.0
E	455	288.0	0.0	0.0	0.0	159.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO				FR-TO			
A-B	0.741	-91.8	-91.8 0.13 (1)	F-C	-8.98	0.03 (4)	
B-C	-412.0	-91.8	-91.8 0.40 (1)	B-F	0.324	0.07 (1)	
C-D	-412.0	-91.8	-91.8 0.40 (1)	F-D	0.324	0.07 (1)	
G-B	-729.0	0.0	0.0 0.08 (1)				
E-D	-602.0	0.0	0.0 0.06 (1)				
G-F	0.0	-18.5	-18.5 0.18 (4)				
F-E	0.0	-18.5	-18.5 0.18 (4)				

TOTAL WEIGHT = 50 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2018
- PART 9 OF OSC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.39")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.39")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.40/1.00 (B-C:1), BC=0.18/1.00 (F-G:4), WB=0.07/1.00 (B-F:1), SS=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

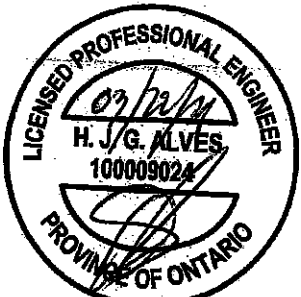
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

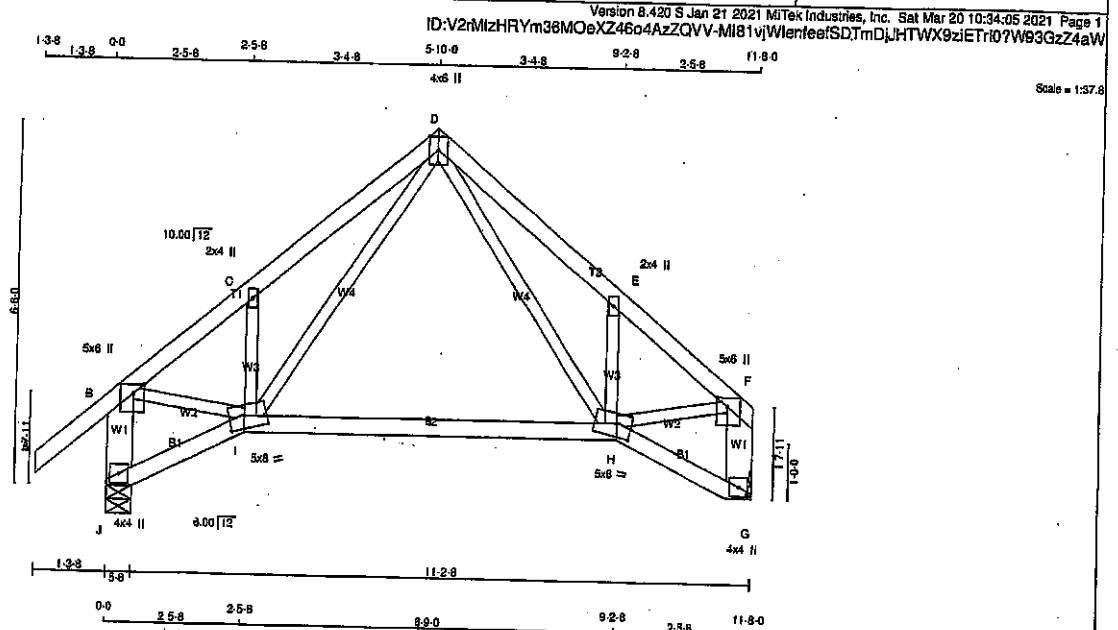
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.81 (D) (INPUT = 0.90)
JSI METAL = 0.17 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108270

JOB NAME 417471	TRUSS NAME T30S	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESOR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x6	DRY	No.2
G - F	2x6	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
H - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	W	LEN	Y X
B	TMVW+p	MT20	5.0	6.0 2.00 2.25
C	TMVW+w	MT20	2.0	4.0
D	TMVW+p	MT20	4.0	6.0 Edge
E	TMVW+w	MT20	2.0	4.0
F	TMVW+p	MT20	5.0	6.0 2.00 2.25
G	BMV1+p	MT20	4.0	4.0
H	BBVWW+m	MT20	5.0	8.0 2.75 3.25
I	BBVWW+m	MT20	5.0	8.0 2.75 3.25
J	BMV1+p	MT20	4.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	JT	VERT	JT	IN-SX	JT	IN-SX
J	770	J	770	J	5-8	J	5-8
G	643	G	643	G	0	G	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	542	369.0	0.0	0.0	0.0	173.0	0.0
G	455	298.0	0.0	0.0	0.0	158.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0.41	D-H	0.411
B-C	-717.0	E-H	-327.0
C-D	-758.0	I-D	0.411
D-E	-758.0	I-C	-327.0
E-F	-717.0	B-I	0.571
J-B	-747.0	H-F	0.571
G-F	-620.0		
J-I	0.0		
I-H	0.343		
H-G	0.0		

TOTAL WEIGHT = 3 X 58 = 167 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 8.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.13/1.00 (A-B:1), BC=0.27/1.00 (H-I:4), WB=0.13/1.00 (B-I:1), SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (FLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 768 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

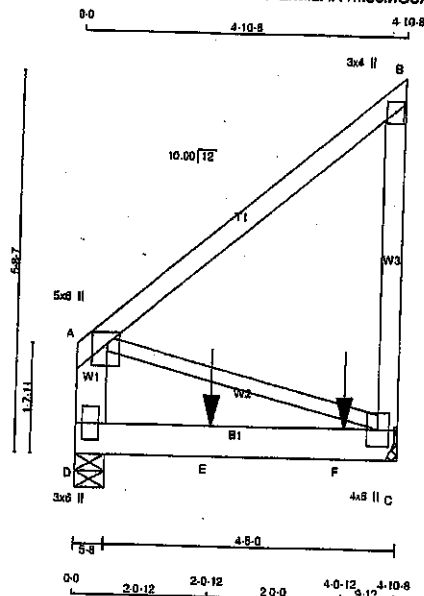
JSI GRIP= 0.47 (B) (INPUT = 0.90)
JSI METAL= 0.18 (E) (INPUT = 1.00)



Structural component only
DWG# T-2108271

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417471	T31	2	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:31.5

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N.L.G.A. RULES	A-B	2x4	DRY	No.2
	C-B	2x4	DRY	No.2
	D-A	2x6	DRY	No.2
	D-C	2x6	DRY	No.2
	ALL WEBS	2x3	DRY	No.2
			DRY: SEASONED LUMBER.	

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-B 1	12	TOP
B-C 1	12	TOP
D-A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D-C 2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
C	876	0	876	0	5-8	5-8
D	628	0	628	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 2-0.

UNFACTORED REACTIONS

JT	1ST LOASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	818	417.0	0.0	0.0	0.0	201.0	0.0
D	442	298.0	0.0	0.0	0.0	145.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	FORCE (LBS)	VERT. LOAD (PLF)	LOC1	MAX	LOC1	UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX
FR-TO								
A-B	0.0	-91.8	-91.8	0.20 (1)	10.00	A-C	0.0	0.00 (1)
C-B	-224.0	0.0	0.0	0.08 (1)	7.81			
D-A	-224.0	0.0	0.0	0.01 (1)	7.81			
D-E	0.0	-18.5	-18.5	0.29 (1)	10.00			
E-F	0.0	-18.5	-18.5	0.29 (1)	10.00			
F-C	0.0	-18.5	-18.5	0.29 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC	LOC1	MAX-	MAX+	FACE	DIR	TYPE	HEEL	CONN.
E	2-0-12	-339	-339		FRONT	VERT	TOTAL		C1
F	4-0-12	-341	-341		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 27 = 110 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NSCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.20/1.00 (A-B-1), BC=0.29/1.00 (C-D-1), WB=0.00/1.00 (A-C-1), SS=0.22/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
		788	1987
		1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.05 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



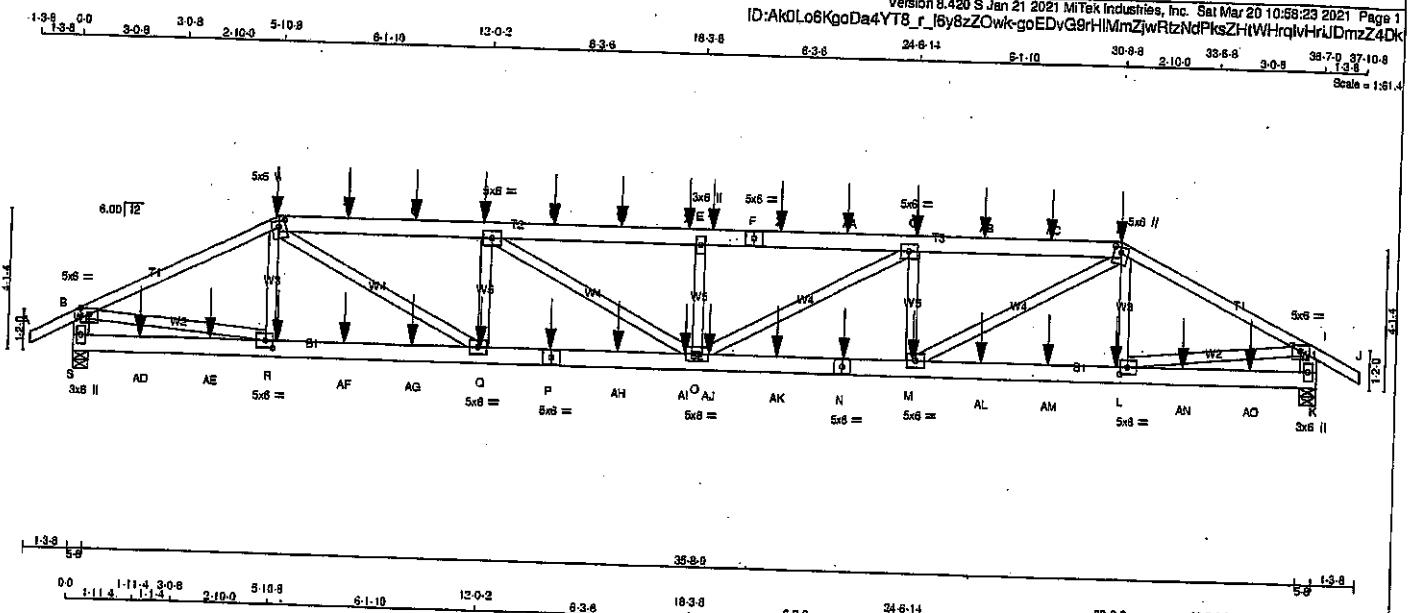
Structural component only
DWG# T-2108272

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
417473	T41	1	2	GREENPARK HOMES		

Tamarack Roof Truss, Burlington

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Scale = 1:51.4



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x6	DRY	No.2	SPF	
F - H	2x6	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
J - B	2x6	DRY	No.2	SPF	
B - I	2x6	DRY	No.2	SPF	
I - P	2x6	DRY	No.2	SPF	
P - N	2x6	DRY	No.2	SPF	
N - K	2x6	DRY	No.2	SPF	

ALL WEBS 2x4 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122'X3') SPIRAL NAILS			
A-C	1	12	
H-J	1	12	
C-F	2	12	
F-H	2	12	
S-B	2	12	
K-I	2	12	
BOTTOM CHORDS: (0.122'X3') SPIRAL NAILS			
S-P	2	12	
P-N	2	12	
N-K	2	12	
WEBS: (0.122'X3') SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX
S	3406	0	3406	0	0	5-8	5-8	5-8
K	3406	0	3406	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	2410	1571 : 0	0 : 0	0 : 0	0 : 0	839 : 0	0 : 0
K	2410	1571 : 0	0 : 0	0 : 0	0 : 0	839 : 0	0 : 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH (FT)	MEMB. FORCE (LBS)	MAX. FACTORED CSI (LC)		
FR-TO		FROM TO			FR-TO			
A-B	0' 28	-91.8	-91.8	0.07 (1)	10.00	R-C	-432' 12	0.04 (1)
B-C	-5208' 0	-91.8	-91.8	0.53 (1)	3.81	L-H	-432' 12	0.04 (1)
C-T	-7735' 0	-91.8	-91.8	0.29 (1)	4.13	B-R	0' 4711	0.42 (1)
T-U	-7735' 0	-91.8	-91.8	0.29 (1)	4.13	L-I	0' 4711	0.42 (1)
U-D	-7735' 0	-91.8	-91.8	0.29 (1)	4.13	M-H	0' 3555	0.31 (1)
D-V	-8731' 0	-91.8	-91.8	0.32 (1)	3.89	C-Q	0' 3555	0.31 (1)
V-W	-8731' 0	-91.8	-91.8	0.32 (1)	3.89	M-G	-1628' 0	0.14 (1)
W-X	-8731' 0	-91.8	-91.8	0.32 (1)	3.89	Q-D	-1628' 0	0.14 (1)
X-E	-8731' 0	-91.8	-91.8	0.32 (1)	3.89	O-G	0' 1152	0.10 (1)
E-Y	-8731' 0	-91.8	-91.8	0.32 (1)	3.89	D-O	0' 1152	0.10 (1)
Y-F	-8731' 0	-91.8	-91.8	0.32 (1)	3.89	O-E	-950' 0	0.08 (1)
F-Z	-8731' 0	-91.8	-91.8	0.32 (1)	3.89			
Z-AA	-8731' 0	-91.8	-91.8	0.32 (1)	3.89			
AA-G	-8731' 0	-91.8	-91.8	0.32 (1)	3.89			
G-AB	-7735' 0	-91.8	-91.8	0.29 (1)	4.13			
AB-AC	-7735' 0	-91.8	-91.8	0.29 (1)	4.13			
AC-H	-7735' 0	-91.8	-91.8	0.29 (1)	4.13			
H-I	-5208' 0	-91.8	-91.8	0.53 (1)	3.81			
I-J	0' 28	-91.8	-91.8	0.07 (1)	10.00			
S-B	-3333' 0	0.0	0.0	0.12 (1)	7.61			
K-I	-3333' 0	0.0	0.0	0.12 (1)	7.61			
S-AD	0' 0	-18.5	-18.5	0.06 (4)	10.00			
AD-AE	0' 0	-18.5	-18.5	0.06 (4)	10.00			
AE-R	0' 0	-18.5	-18.5	0.06 (4)	10.00			
R-AF	0' 4686	-18.5	-18.5	0.35 (1)	10.00			
AF-AG	0' 4686	-18.5	-18.5	0.35 (1)	10.00			
AG-Q	0' 4686	-18.5	-18.5	0.35 (1)	10.00			
Q-P	0' 7734	-18.5	-18.5	0.59 (1)	10.00			
P-AH	0' 7734	-18.5	-18.5	0.59 (1)	10.00			
AH-AI	0' 7734	-18.5	-18.5	0.59 (1)	10.00			
AI-O	0' 7734	-18.5	-18.5	0.59 (1)	10.00			
O-AJ	0' 7734	-18.5	-18.5	0.59 (1)	10.00			
AJ-AK	0' 7734	-18.5	-18.5	0.59 (1)	10.00			
AK-N	0' 7734	-18.5	-18.5	0.59 (1)	10.00			
N-M	0' 7734	-18.5	-18.5	0.59 (1)	10.00			
M-AL	0' 4686	-18.5	-18.5	0.35 (1)	10.00			
AL-AM	0' 4686	-18.5	-18.5	0.35 (1)	10.00			
AM-L	0' 4686	-18.5	-18.5	0.35 (1)	10.00			
L-AN	0' 0	-18.5	-18.5	0.06 (4)	10.00			
AN-AO	0' 0	-18.5	-18.5	0.06 (4)	10.00			
AO-K	0' 0	-18.5	-18.5	0.06 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ASB 2015
- PART 9 OF OBC 2012 (2015 AMENDMENT)
- CSA 086-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.22')
CALCULATED VERT. DEFL. (LL) = L/999 (0.22')
ALLOWABLE DEFL. (TL) = L/360 (1.22')
CALCULATED VERT. DEFL. (TL) = L/999 (0.42')

CSI: TC=0.53/1.00 (H=1), BC=0.56/1.00 (M=0.1), WB=0.42/1.00 (L=1), SSI=0.16/1.00 (D=E=1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.72 (F) (INPUT = 1.00)



Structural component only
DWG# T-2108285

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417473	T41	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Sat Mar 20 10:58:23 2021 Page 2
ID:AK0Lo6KqoDg4YT8 r l6v8zZOWk-goEDvG8rHlMmZlwRtzNdPksZHWtHrlyJDmzZ4Dk

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	3.00
C	TTWW-m	MT20	5.0	6.0	2.00	2.50
D	TMVW-l	MT20	5.0	6.0		
E	TMVW-w	MT20	3.0	6.0		
F	TS-l	MT20	5.0	6.0		
G	TMVW-l	MT20	5.0	6.0		
H	TTWW-m	MT20	5.0	6.0	2.00	2.50
I	TMVW-p	MT20	5.0	6.0	2.00	3.00
K	BMV1-p	MT20	3.0	6.0		
L	BMVW-l	MT20	5.0	6.0	2.50	2.75
M	BMVW-l	MT20	5.0	6.0		
N	BS-l	MT20	5.0	6.0		
O	BMVW-l	MT20	5.0	6.0		
P	BS-l	MT20	5.0	6.0		
Q	BMVW-l	MT20	5.0	6.0		
R	BMVW-l	MT20	5.0	6.0	2.50	2.75
S	BMV1-p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-253	-253		FRONT	VERT	TOTAL		C1
D	11-11-4	-76	-76		FRONT	VERT	TOTAL		C1
G	24-7-12	-76	-76		FRONT	VERT	TOTAL		C1
H	30-8-8	-253	-253		FRONT	VERT	TOTAL		C1
L	30-7-12	-21	-21		FRONT	VERT	TOTAL		C1
M	24-7-12	-21	-21		FRONT	VERT	TOTAL		C1
N	22-7-12	-21	-21		FRONT	VERT	TOTAL		C1
P	13-11-4	-21	-21		FRONT	VERT	TOTAL		C1
Q	11-11-4	-21	-21		FRONT	VERT	TOTAL		C1
R	5-11-4	-21	-21		FRONT	VERT	TOTAL		C1
T	7-11-4	-76	-76		FRONT	VERT	TOTAL		C1
U	9-11-4	-76	-76		FRONT	VERT	TOTAL		C1
V	13-11-4	-76	-76		FRONT	VERT	TOTAL		C1
W	15-11-4	-76	-76		FRONT	VERT	TOTAL		C1
X	17-11-4	-76	-76		FRONT	VERT	TOTAL		C1
Y	18-7-12	-76	-76		FRONT	VERT	TOTAL		C1
Z	20-7-12	-76	-76		FRONT	VERT	TOTAL		C1
AA	22-7-12	-76	-76		FRONT	VERT	TOTAL		C1
AB	26-7-12	-76	-76		FRONT	VERT	TOTAL		C1
AC	28-7-12	-76	-76		FRONT	VERT	TOTAL		C1
AD	1-11-4	-20	-20		FRONT	VERT	TOTAL		C1
AE	3-11-4	-21	-21		FRONT	VERT	TOTAL		C1
AF	7-11-4	-21	-21		FRONT	VERT	TOTAL		C1
AG	9-11-4	-21	-21		FRONT	VERT	TOTAL		C1
AH	15-11-4	-21	-21		FRONT	VERT	TOTAL		C1
AI	17-11-4	-21	-21		FRONT	VERT	TOTAL		C1
AJ	18-7-12	-21	-21		FRONT	VERT	TOTAL		C1
AK	20-7-12	-21	-21		FRONT	VERT	TOTAL		C1
AL	26-7-12	-21	-21		FRONT	VERT	TOTAL		C1
AM	28-7-12	-21	-21		FRONT	VERT	TOTAL		C1
AN	32-7-12	-21	-21		FRONT	VERT	TOTAL		C1
AO	34-7-12	-20	-20		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



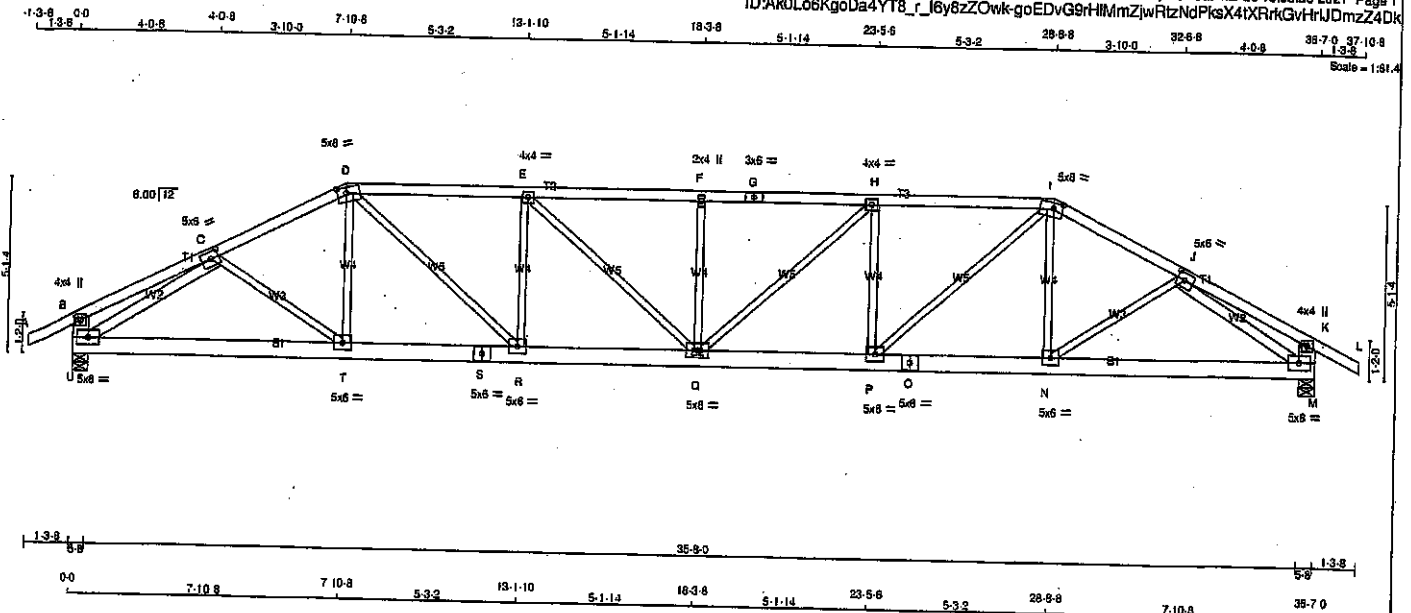
Structural component only
DWG# T-2108285

JOB NAME 417473	TRUSS NAME T42	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MTEK Industries, Inc. Sat Mar 20 10:58:23 2021 Page 1
ID:AK0Lo6KgoDa4YT8_r_l6y6zZOWk-goEDvG9rHmMzjwRtZNdPksX4tXRrkGvHrJDmzZ4Dk

Scale = 1:31.4



LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE		LUMBER	
A - D	2x4	DRY	No.2		
D - G	2x4	DRY	No.2		
G - I	2x4	DRY	No.2		
I - L	2x4	DRY	No.2		
U - B	2x6	DRY	No.2		
M - K	2x6	DRY	No.2		
U - S	2x6	DRY	No.2		
S - O	2x6	DRY	No.2		
O - M	2x6	DRY	No.2		

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
U - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY, SEASONED LUMBER.

PLATES (tables in inches)	PLATES	W	LEN	Y	X
BT	TMV+p	4.0	4.0		
C	TMVW-i	MT20	5.0	6.0	2.50 2.75
D	TMVW-m	MT20	5.0	8.0	2.00 3.25
E	TMVW-i	MT20	4.0	4.0	
F	TMVW-w	MT20	2.0	4.0	
G	TS-i	MT20	3.0	6.0	
H	TMVW-i	MT20	4.0	4.0	
I	TTVW-m	MT20	5.0	8.0	2.00 3.25
J	TMVW-i	MT20	6.0	8.0	2.50 2.75
K	TMVW-p	MT20	4.0	4.0	
M	BMVW-i	MT20	5.0	8.0	
N, P, R, T					
N	BMVW-i	MT20	5.0	6.0	
O	BS-i	MT20	5.0	6.0	
O	BMVW-i	MT20	5.0	8.0	
S	BS-i	MT20	5.0	6.0	
U	BMVW-i	MT20	5.0	8.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	5-8	5-8
U	HORIZ	HORIZ	5-8	5-8
M			5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1512	1005.0	0.0	0.0	0.0	508.0	0.0
M	1512	1005.0	0.0	0.0	0.0	508.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	FACTORED	WEBS	MAX. FACTORED	FACTORED	FACTORED
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MEMB.	FORCE	MAX	LC1
(LBS)	(PLF)	(LC)	(LC)	(LBS)	(LBS)	(LC)	(LC)
FR-TO		FROM	TO	FR-TO			
A-B	0.28	-91.8	-91.8 0.12 (1)	10.00	C-T	0.188	0.03 (4)
B-C	0.17	-91.8	-91.8 0.20 (1)	10.00	T-D	0.118	0.04 (4)
C-D	-2987.0	-91.8	-91.8 0.34 (1)	3.75	N-I	0.118	0.04 (4)
D-E	-3641.0	-91.8	-91.8 0.64 (1)	3.16	N-J	0.188	0.03 (4)
E-F	-3925.0	-91.8	-91.8 0.67 (1)	3.02	U-C	-3165.0	0.80 (1)
F-G	-3925.0	-91.8	-91.8 0.67 (1)	3.02	J-M	-3165.0	0.80 (1)
G-H	-3925.0	-91.8	-91.8 0.67 (1)	3.02	P-I	0.1323	0.30 (1)
H-I	-3641.0	-91.8	-91.8 0.84 (1)	3.16	D-R	0.1323	0.30 (1)
I-J	-2987.0	-91.8	-91.8 0.34 (1)	3.75	P-H	-800.0	0.29 (1)
J-K	0.17	-91.8	-91.8 0.20 (1)	10.00	R-E	-800.0	0.29 (1)
K-L	0.28	-91.8	-91.8 0.12 (1)	10.00	Q-H	0.7386	0.09 (1)
U-B	-269.0	0.0	0.0 0.02 (1)	7.81	Q-C	0.7386	0.09 (1)
M-K	-269.0	0.0	0.0 0.02 (1)	7.81	Q-F	-434.0	0.16 (1)
U-T	0.2599	-18.5	-18.5 0.38 (1)	10.00			
T-S	0.2657	-18.5	-18.5 0.36 (1)	10.00			
S-R	0.2657	-18.5	-18.5 0.36 (1)	10.00			
R-Q	0.3641	-18.5	-18.5 0.49 (1)	10.00			
Q-P	0.3641	-18.5	-18.5 0.49 (1)	10.00			
P-O	0.2657	-18.5	-18.5 0.36 (1)	10.00			
O-N	0.2657	-18.5	-18.5 0.36 (1)	10.00			
N-M	0.2599	-18.5	-18.5 0.38 (1)	10.00			

TOTAL WEIGHT = 172 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=		39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.22")
ALLOWABLE DEFL.(TL) = L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.40")

CSI TC=0.87/1.00 (E-F:1), BC=0.49/1.00 (Q-R:1),
WB=0.80/1.00 (J-M:1), SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1973

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

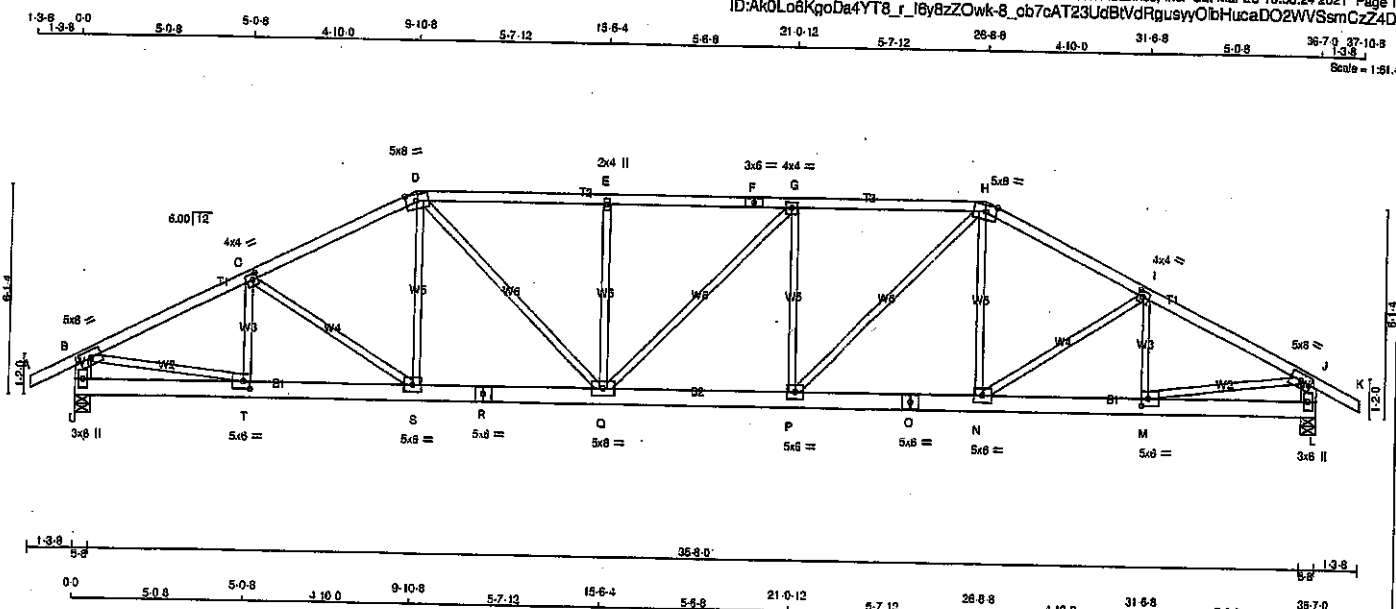
JSI GRIP= 0.86 (I) (INPUT = 0.90)
JSI METAL= 0.72 (J) (INPUT = 1.00)



Structural component only
DWG# T-2108286

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417473	T43	1	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington					

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ID: Akl0Lo6KgoDa4YT8_r_l6y8zzOwk-8_ob7cAT23UdBTdRguyyOibHucaDO2WWSmCzZ4D



LUMBER

N. L. G. A. RULES

CHORDS

SIZE

LUMBER

DESCR.

ALL WEBS

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE

PLATES

W

LEN

Y

X

N. P. S.

OBS-I

Q BMVW-I

R BS-I

T BMVW-I

U BMV-I+p

NOTES-

(1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED

GROSS REACTION

MAXIMUM FACTORED

INPUT

REQD

JT

U

L

UNFACTORED REACTIONS

1ST LCASE

JT

U

L

COMBINED

SNOW

LIVE

PERM. LIVE

WIND

DEAD

SOIL

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.

MAX. FACTORED

FORCE

VERT. LOAD

LC1

MAX

CS1 (LC)

UNBRAC

LENGTH

FR-TO

A-B

B-C

C-D

D-E

E-F

F-G

G-H

H-I

I-J

J-K

U-B

L-J

U-T

T-S

S-R

R-Q

Q-P

P-O

O-N

N-M

M-L

WEBS

MEMB.

MAX. FACTORED

FORCE

MAX

CS1 (LC)

FR-TO

T-C

C-S

S-D

D-Q

Q-E

E-G

G-H

H-I

I-H

N-H

N-I

M-I

B-T

M-J

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF

DL = 8.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019

- PART 9 OF CBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.22")

CALCULATED VERT. DEFL. (LL) = L/999 (0.18")

ALLOWABLE DEFL. (TL) = L/360 (1.22")

CALCULATED VERT. DEFL. (TL) = L/999 (0.33")

CSI: TG=0.49/1.00 (G-H:1), BC=0.43/1.00 (P-Q:1)

WB=0.62/1.00 (J-M:1), SS=0.24/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.80)

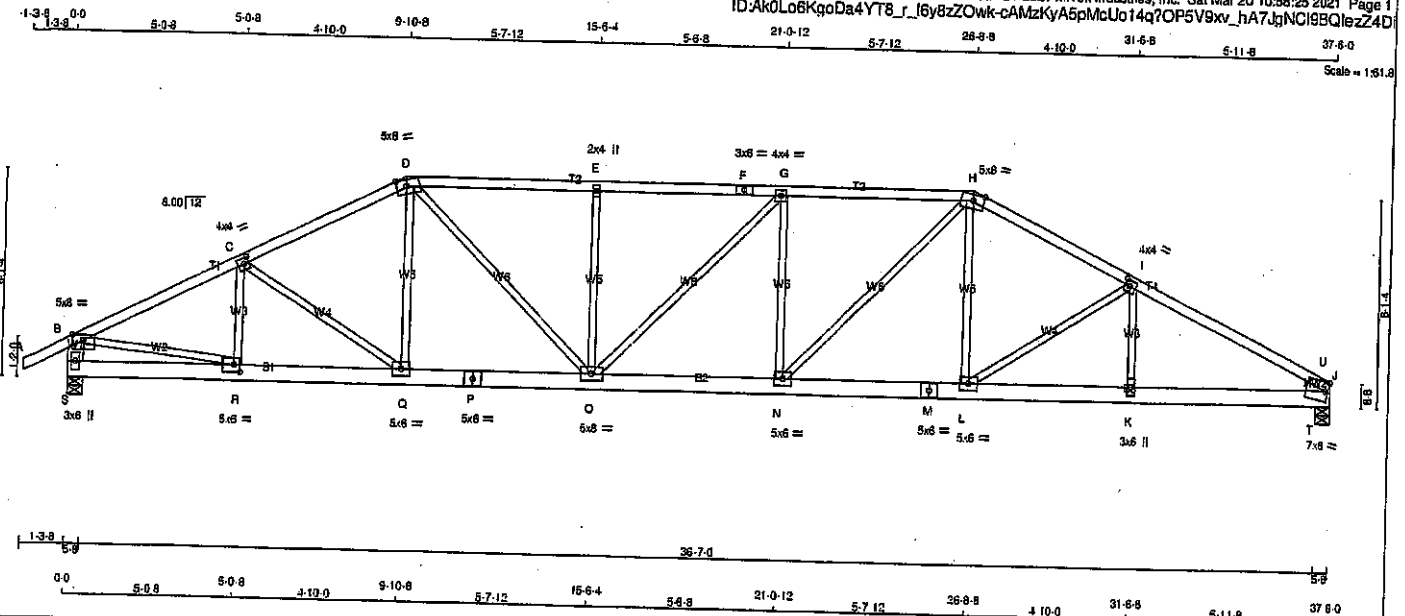
JSI METAL= 0.82 (M) (INPUT = 1.00)



Structural component only
DWG# T-2108287

JOB NAME 417473	TRUSS NAME T43X	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 6.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 10:58:25 2021 Page 1
ID:AK0Lo6KgoDa4YT8_r_16y8zzOwk-cAMzKyA5pMcUo14q?OP5V9xv_hA7JgNCI9BQleZ4D



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
S - B	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
M - J	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	8.0	Edge
C	TMVW-t	MT20	4.0	4.0	2.00 1.75
D	TTVW-m	MT20	5.0	8.0	2.25 3.50
E	TMVW-w	MT20	2.0	4.0	
F	TS-t	MT20	3.0	6.0	
G	TMVW-t	MT20	4.0	4.0	
H	TTVW-m	MT20	5.0	8.0	2.25 3.75
I	TMVW-t	MT20	4.0	4.0	2.00 1.75
J	TMBH-t	MT20	7.0	8.0	Edge 1.00
K	BMVW-w	MT20	3.0	6.0	
L, N, Q	BMVW-t	MT20	5.0	8.0	
M	BS-t	MT20	5.0	8.0	
O	BMVW-w	MT20	5.0	8.0	
P	BS-t	MT20	5.0	8.0	
R	BMVW-t	MT20	5.0	8.0	2.50 2.00
S	BMVW-p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG		HEEL WEDGE	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
S	2182	0	2182	0	0	5-8	5-8		
J	2087	0	2087	0	0	5-8	5-8	2x4 R	

UNFACTORED REACTIONS

1ST CASE		MAX/MIN COMPONENT REACTIONS		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW											
S	1548	1029 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	519 / 0	0 / 0	502 / 0	0 / 0	0 / 0	0 / 0
J	1482	959 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	519 / 0	0 / 0	502 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED (LC1)	MEMB.	FORCE (LBS)	MAX. FACTORED (LC1)	MAX. FACTORED (LC1)
FR-TO		FROM TO	CSI (LC)	FR-TO		FROM TO	CSI (LC)
A-B	0 / 28	-91.8	-91.8 0.12 (1)	R-C	-383 / 0	0.08 (1)	
B-C	-3095 / 0	-91.8	-91.8 0.42 (1)	C-Q	-181 / 0	0.11 (1)	
C-D	-2974 / 0	-91.8	-91.8 0.40 (1)	D-O	0 / 243	0.05 (1)	
D-E	-3327 / 0	-91.8	-91.8 0.51 (1)	O-Q	0 / 975	0.22 (1)	
E-F	-3327 / 0	-91.8	-91.8 0.47 (1)	Q-E	-555 / 0	0.30 (1)	
F-G	-3327 / 0	-91.8	-91.8 0.47 (1)	G-Q	-77 / 0	0.10 (1)	
G-H	-3380 / 0	-91.8	-91.8 0.51 (1)	H-Q	-503 / 0	0.27 (1)	
H-I	-8137 / 0	-91.8	-91.8 0.39 (1)	I-H	0 / 832	0.19 (1)	
I-U	-3507 / 0	-91.8	-91.8 0.52 (1)	U-H	0 / 409	0.09 (1)	
U-J	-4211 / 0	-91.8	-91.8 0.48 (1)	J-I	-411 / 0	0.25 (1)	
S-B	-2129 / 0	0.0	0.0 0.14 (1)	K-I	-97 / 43	0.02 (1)	
				B-R	0 / 2821	0.82 (1)	
				T-U	0 / 983	0.00 (1)	
S-R	0 / 0	-18.5	-18.5 0.07 (1)				
R-Q	0 / 2788	-18.5	-18.5 0.39 (1)				
Q-P	0 / 2642	-18.5	-18.5 0.36 (1)				
P-O	0 / 2642	-18.5	-18.5 0.38 (1)				
O-N	0 / 3380	-18.5	-18.5 0.45 (1)				
N-M	0 / 2798	-18.5	-18.5 0.39 (1)				
M-L	0 / 2798	-18.5	-18.5 0.39 (1)				
L-K	0 / 3130	-18.5	-18.5 0.53 (1)				
K-T	0 / 3130	-18.5	-18.5 0.67 (1)				
T-J	0 / 3130	-18.5	-18.5 0.67 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.25")
CALCULATED VERT. DEFL.(LL) = L/999 (0.20")
ALLOWABLE DEFL.(TL) = L/360 (1.25")
CALCULATED VERT. DEFL.(TL) = L/999 (0.37")

CSI: TC=0.52/1.00 (H-U:1), BC=0.67/1.00 (K-T:1),
WB=0.83/1.00 (B-R:1), SSI=0.49/1.00 (J-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

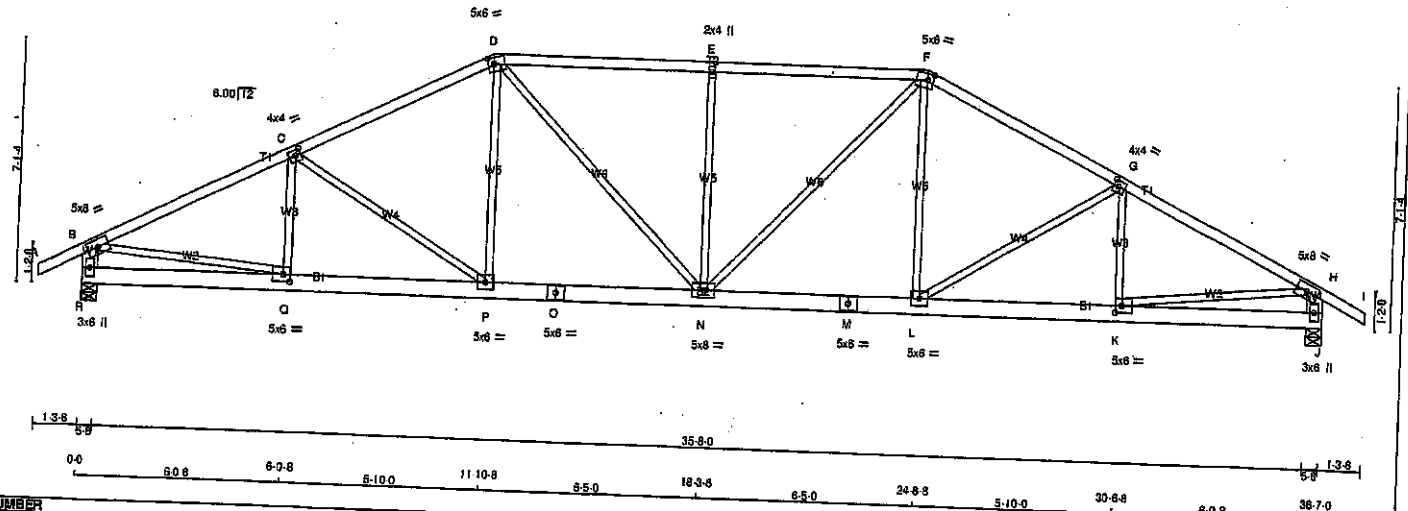
JSI GRIP = 0.88 (J) (INPUT = 0.90)
JSI METAL = 0.83 (R) (INPUT = 1.00)



Structural component only
DWG# T-2108288

JOB NAME 417473 Tamarack Roof Truss, Burlington	TRUSS NAME T44	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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ID:AK0Lo6KgoDa4YT8_r_l8y8zZOWk-4NwLXIBagkLQA0Z5wk1NU3_5bq27Kzpxzq5zZ4Dh
Scale = 1/8" = 1'-0"



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
F - I	2x4	DRY No.2
J - L	2x6	DRY No.2
M - O	2x6	DRY No.2
P - R	2x6	DRY No.2
S - T	2x6	DRY No.2
U - V	2x6	DRY No.2
W - X	2x6	DRY No.2
Y - Z	2x6	DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	BMVW-1	MT20	5.0	8.0		
B	BMVW-1	MT20	4.0	4.0	2.00	1.75
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTWW-m	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TTWW-m	MT20	5.0	6.0	2.25	1.75
G	BMVW-1	MT20	4.0	4.0	2.00	1.75
H	BMVW-1	MT20	5.0	6.0		
I	BMVW-1	MT20	3.0	6.0		
J	BMVW-1	MT20	5.0	6.0	2.50	2.25
K	BMVW-1	MT20	5.0	6.0		
L	BMVW-1	MT20	5.0	6.0		
M	BS-1	MT20	5.0	6.0		
N	BMVW-1	MT20	5.0	6.0		
O	BS-1	MT20	5.0	6.0		
P	BMVW-1	MT20	5.0	6.0		
Q	BMVW-1	MT20	5.0	6.0	2.50	2.25
R	BMV1+p	MT20	3.0	6.0		

NOTES

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
R	2141	0	2141	0	5-8	5-8
J	2141	0	2141	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX/MIN COMPONENT REACTIONS		DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND		
R	1512	1005.0	0.0	0.0	506.0	0.0
J	1512	1005.0	0.0	0.0	506.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	
R-TO		FROM	CS1 (LC)	FR-TO			
A-B	0/28	-91.8	-91.8 0.12 (1)	10.00	Q-C	-273/5	0.07 (1)
B-C	-3068/0	-91.8	-91.8 0.60 (1)	3.46	C-P	-420/0	0.39 (1)
C-D	-2729/0	-91.8	-91.8 0.54 (1)	3.70	P-D	0/371	0.08 (1)
D-E	-2809/0	-91.8	-91.8 0.63 (1)	3.51	D-N	0/566	0.13 (1)
E-F	-2809/0	-91.8	-91.8 0.63 (1)	3.51	N-E	-723/0	0.59 (1)
F-G	-2729/0	-91.8	-91.8 0.54 (1)	3.70	N-F	0/566	0.13 (1)
G-H	-3068/0	-91.8	-91.8 0.60 (1)	3.46	L-F	0/371	0.08 (1)
H-I	0/28	-91.8	-91.8 0.12 (1)	10.00	L-G	-420/0	0.39 (1)
I-J	-2077/0	0.0	0.0 0.13 (1)	7.03	K-G	-273/5	0.07 (1)
J-K	-2077/0	0.0	0.0 0.13 (1)	7.03	B-Q	0/2790	0.83 (1)
K-L	0/0	-18.5	-18.5 0.08 (4)	10.00	K-H	0/2790	0.83 (1)
L-M	0/2767	-18.5	-18.5 0.38 (1)	10.00			
M-N	0/2419	-18.5	-18.5 0.34 (1)	10.00			
N-O	0/2419	-18.5	-18.5 0.34 (1)	10.00			
O-P	0/2419	-18.5	-18.5 0.34 (1)	10.00			
P-Q	0/2767	-18.5	-18.5 0.38 (1)	10.00			
Q-R	0/0	-18.5	-18.5 0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.63/1.00 (D-E:1), BC=0.38/1.00 (K-L:1), WB=0.63/1.00 (H-K:1), GS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

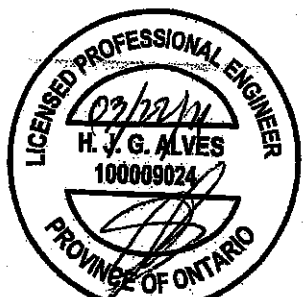
RUSS MANUFACTURING PLANT.						
AIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	{PSI}		{PLI}		{PLI}	
	MAX	MIN	MAX	MIN	MAX	MIN
T20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)

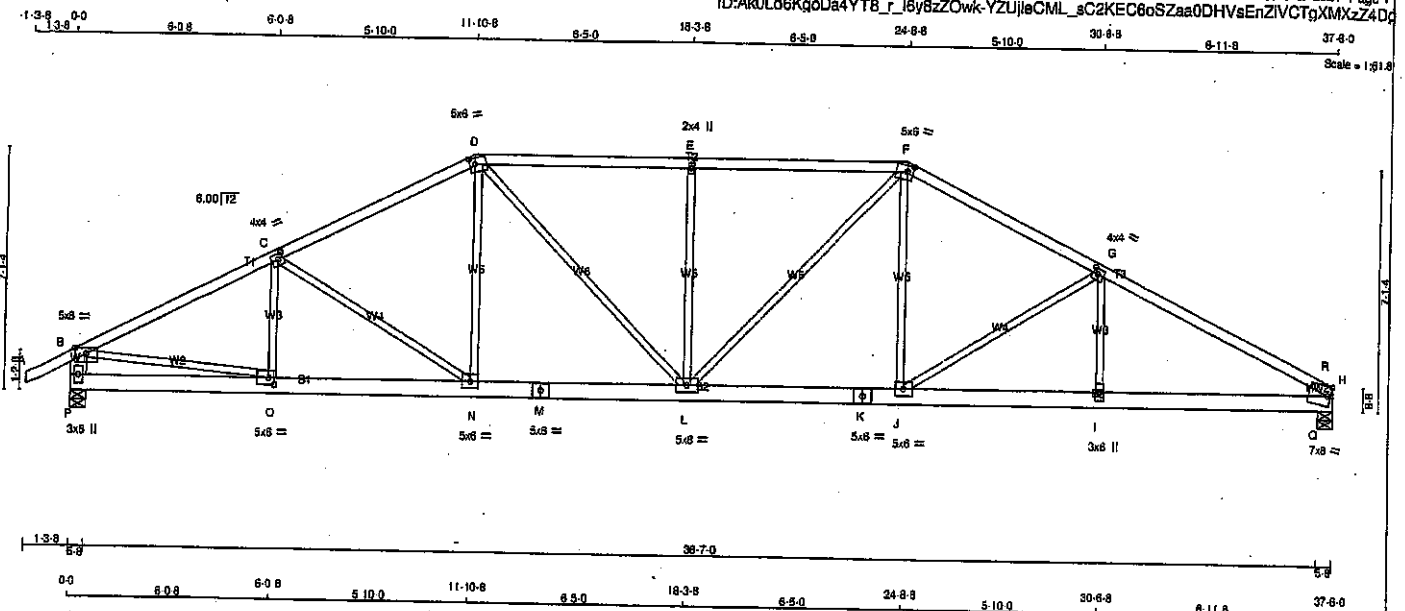
JSI METAL= 0.63 (Q) (INPUT = 1.00)



Structural component only
DWG# T-2108289

JOB NAME 417473	TRUSS NAME T44X	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID: Ak0Lo6KgoDa4YTB_r_j6y8zZOWk-YZUjleCML_sC2KEC6oSZaa0DHVsEnZIVCTgXMxZ4DQ



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
P - B	2x6	DRY	No.2
P - M	2x6	DRY	No.2
M - K	2x6	DRY	No.2
K - H	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY, SEASONED LUMBER.			

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	8.0 Edge
C	TMVW-l	MT20	4.0	4.0 2.00 1.75
D	TMVW-m	MT20	5.0	6.0 2.25 2.00
E	TMVW-w	MT20	2.0	4.0
F	TMVW-m	MT20	5.0	6.0 2.25 2.00
G	TMVW-l	MT20	4.0	4.0 2.00 1.75
H	TMBH-l	MT20	7.0	8.0 Edge 1.00
I	BMVW-w	MT20	3.0	6.0
J	BMVW-l	MT20	5.0	6.0
K	BS-l	MT20	5.0	6.0
L	BMVW-w	MT20	5.0	6.0
M	BS-l	MT20	5.0	6.0
N	BMVW-l	MT20	5.0	6.0
O	BMVW-l	MT20	5.0	6.0 2.50 2.00
P	BMV-l+p	MT20	3.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT VERT	HORZ	JT VERT	HORZ	UPLIFT	IN-SX	HEEL	WEDGE
P	2192	0	2192	0	5-8	5-8	
H	2067	0	2067	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1548	1029	0	0	0	519	0
H	1462	959	0	0	0	502	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO		FROM TO		FR-TO			
A-B	0/28	-91.8 -91.8 0.12 (1)	10.00	O-C	-284/1	0.07 (1)	
B-C	-9155/0	-91.8 -91.8 0.61 (1)	3.41	C-N	-407/0	0.38 (1)	
C-D	-2829/0	-91.8 -91.8 0.55 (1)	3.63	N-D	0/398	0.08 (1)	
D-E	-2945/0	-91.8 -91.8 0.64 (1)	3.42	D-L	0/632	0.14 (1)	
E-F	-2945/0	-91.8 -91.8 0.64 (1)	3.42	L-E	-722/0	0.59 (1)	
F-G	-2934/0	-91.8 -91.8 0.53 (1)	3.62	L-F	0/486	0.11 (1)	
G-R	-3494/0	-91.8 -91.8 0.66 (1)	3.28	J-F	0/511	0.11 (1)	
R-H	-4127/0	-91.8 -91.8 0.53 (1)	2.95	J-G	-619/0	0.58 (1)	
P-B	-2127/0	0.0 0.0 0.14 (1)	6.87	I-G	-19/85	0.03 (4)	
P-O	0/0	-18.5 -18.5 0.08 (4)	10.00	B-O	0/2870	0.65 (1)	
O-N	0/2847	-18.5 -18.5 0.39 (1)	10.00	Q-R	0/845	0.00 (1)	
N-M	0/2509	-18.5 -18.5 0.35 (1)	10.00				
M-L	0/2509	-18.5 -18.5 0.35 (1)	10.00				
L-K	0/2810	-18.5 -18.5 0.38 (1)	10.00				
K-J	0/2810	-18.5 -18.5 0.38 (1)	10.00				
J-I	0/3123	-18.5 -18.5 0.48 (1)	10.00				
I-Q	0/3123	-18.5 -18.5 0.62 (1)	10.00				
Q-H	0/3123	-18.5 -18.5 0.62 (1)	10.00				

TOTAL WEIGHT = 168 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.25")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.25")
CALCULATED VERT. DEFL.(TL) = L/999 (0.32")

CSI: TC=0.66/1.00 (G-R:1), SC=0.62/1.00 (I-Q:1), WS=0.65/1.00 (B-O:1), SSI=0.42/1.00 (H-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN MAX MIN MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

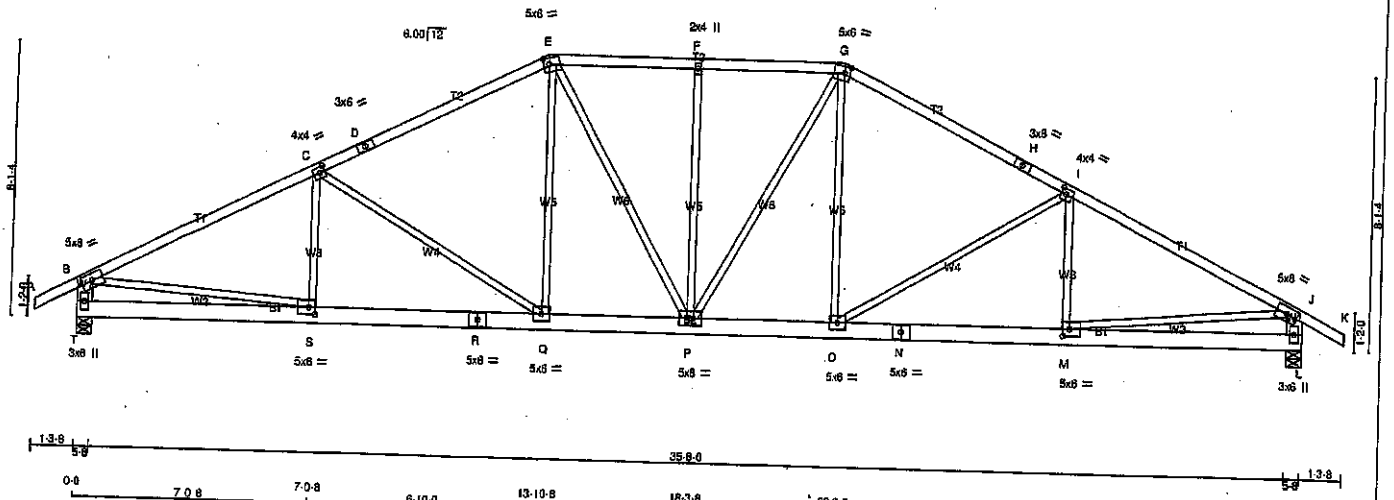
JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.65 (O) (INPUT = 1.00)



Structural component only
DWG# T-2108290

JOB NAME 417473	TRUSS NAME T45	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:AK0L6KgcDa4YT8_r_l6y8zZOWk-0128yzD_6H_3fUoPgWzo6oZLpFBWzBeR7Q4vzzZ4Df
13-8 0.0 7-0-8 7-0-8 6-10-0 13-10-8 4-5-0 18-3-8 4-5-0 22-8-8 5-10-0 29-8-8 7-0-8 36-7-0 37-10-8 13-8
Scale = 1:52.2



N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2	
D - E	2x4	DRY	No.2	
E - G	2x4	DRY	No.2	
G - H	2x4	DRY	No.2	
H - K	2x4	DRY	No.2	
L - B	2x6	DRY	No.2	
L - J	2x6	DRY	No.2	
T - R	2x6	DRY	No.2	
R - N	2x6	DRY	No.2	
N - L	2x6	DRY	No.2	

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	5.0	8.0		
C	TMVW-1	MT20	4.0	4.0	2.00	1.75
D	TS-1	MT20	3.0	8.0		
E	TTVW-m	MT20	5.0	6.0	2.25	2.00
F	TMVW-w	MT20	2.0	4.0		
G	TTVW-m	MT20	5.0	6.0	2.25	2.00
H	TS-1	MT20	3.0	8.0		
I	TMVW-1	MT20	4.0	4.0	2.00	1.75
J	TMVW-1	MT20	5.0	8.0		
L	BMV1-p	MT20	3.0	6.0		
M	BMVW-1	MT20	5.0	6.0	2.50	2.25
N	BS-1	MT20	5.0	6.0		
O	BMVW-1	MT20	5.0	6.0		
P	BMVW-1	MT20	5.0	6.0		
Q	BMVW-1	MT20	5.0	6.0		
R	BS-1	MT20	5.0	6.0		
S	BMVW-1	MT20	5.0	6.0	2.50	2.25
T	BMV1-p	MT20	3.0	6.0		

NOTES (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
T	2141	0	2141	0
L	2141	0	2141	0

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
T	1512	1005.0	0.0	0.0	0.0	508.0	0.0
L	1512	1005.0	0.0	0.0	0.0	508.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO			
A-B	0.28	-91.8	-91.8	0.12 (1)	S-C	-189.54	0.06 (1)	
B-C	-3087.0	-91.8	-91.8	0.86 (1)	C-Q	-633.0	0.89 (1)	
C-D	-2555.0	-91.8	-91.8	0.74 (1)	Q-E	0.467	0.11 (1)	
D-E	-2555.0	-91.8	-91.8	0.74 (1)	E-P	0.307	0.07 (1)	
E-F	-2411.0	-91.8	-91.8	0.29 (1)	P-F	-499.0	0.58 (1)	
F-G	-2411.0	-91.8	-91.8	0.29 (1)	P-G	0.307	0.07 (1)	
G-H	-2555.0	-91.8	-91.8	0.74 (1)	O-G	0.467	0.11 (1)	
H-I	-2555.0	-91.8	-91.8	0.74 (1)	O-I	-633.0	0.89 (1)	
I-J	-3087.0	-91.8	-91.8	0.86 (1)	M-I	-189.54	0.06 (1)	
J-K	0.28	-91.8	-91.8	0.12 (1)	B-S	0.2808	0.63 (1)	
T-B	-2075.0	0.0	0.0	0.13 (1)	M-J	0.2808	0.63 (1)	
L-J	-2075.0	0.0	0.0	0.13 (1)				
T-S	0.0	-18.5	-18.5	0.10 (4)				
S-R	0.2791	-18.5	-18.5	0.39 (1)				
R-Q	0.2791	-18.5	-18.5	0.39 (1)				
Q-P	0.2259	-18.5	-18.5	0.31 (1)				
P-O	0.2259	-18.5	-18.5	0.31 (1)				
O-N	0.2791	-18.5	-18.5	0.39 (1)				
N-M	0.2791	-18.5	-18.5	0.39 (1)				
M-L	0.0	-18.5	-18.5	0.10 (4)				

TOTAL WEIGHT = 178 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 8.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCO 2015, ABC 2015

- PART 9 OF NBCO 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.22")

CALCULATED VERT. DEFL. (LL) = L/999 (0.15")

ALLOWABLE DEFL. (TL) = L/360 (1.22")

CALCULATED VERT. DEFL. (TL) = L/999 (0.27")

CSI: TC=0.86/1.00 (I-J), BC=0.39/1.00 (Q-S), WB=0.89/1.00 (C-Q), SS=0.28/1.00 (I-J)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)

JSI METAL= 0.63 (S) (INPUT = 1.00)

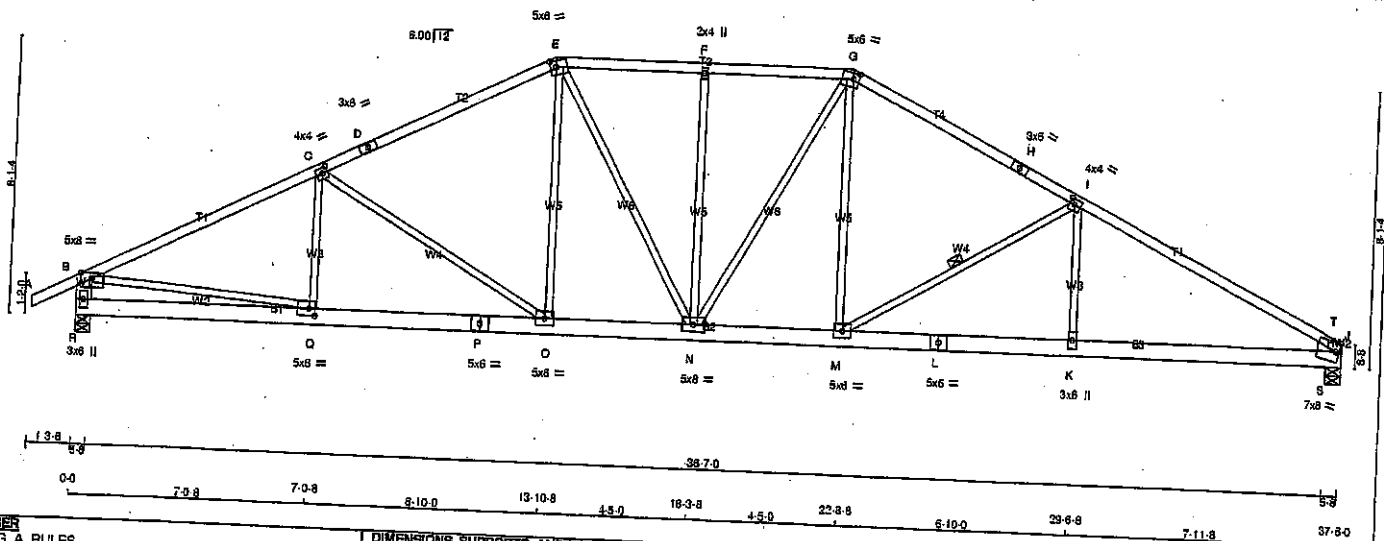


Structural component only
DWG# T-2108291

JOB NAME 417473	TRUSS NAME T45X	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						

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ID: Akl0L6KgoDa4YT8_r_l8y8zZowk-VycUAJEctb6wHbNbEDU1f76V7IZYFQgnfn9dROzZ4De

Scale = 1/8" = 1'-0"



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - N	2x4	DRY	No.2
N - O	2x4	DRY	No.2
O - P	2x4	DRY	No.2
P - Q	2x4	DRY	No.2
Q - R	2x4	DRY	No.2
R - S	2x4	DRY	No.2
S - T	2x4	DRY	No.2
T - U	2x4	DRY	No.2
U - V	2x4	DRY	No.2
V - W	2x4	DRY	No.2
W - X	2x4	DRY	No.2
X - Y	2x4	DRY	No.2
Y - Z	2x4	DRY	No.2
Z - A	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	8.0	Edge
C	TMWV-t	MT20	4.0	4.0	2.00 1.75
D	TS-t	MT20	3.0	6.0	
E	TMWV-m	MT20	5.0	6.0	2.25 2.00
F	TMWV-w	MT20	2.0	4.0	
G	TMWV-w	MT20	5.0	6.0	2.25 2.00
H	TS-t	MT20	3.0	6.0	
I	TMWV-t	MT20	4.0	4.0	2.00 1.75
J	TMWV-m	MT20	7.0	8.0	Edge 1.00
K	BMWV-w	MT20	3.0	6.0	
L	BS-t	MT20	5.0	6.0	
M	BMWV-t	MT20	5.0	6.0	
N	BMWV-w	MT20	5.0	6.0	
O	BMWV-t	MT20	5.0	6.0	
P	BS-t	MT20	5.0	6.0	
Q	BMWV-t	MT20	5.0	6.0	2.50 2.00
R	BMWV-p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX	HEEL WEDGE
	VERT	HORZ			
JT	2192	0	0	0	
R	2192	0	0	0	
J	2067	0	0	0	

UNFACTORED REACTIONS

1ST LOOSE	MAX. MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW				
JT	1548	1029	0	0	0	0
R	1482	959	0	0	0	0
J	1482	959	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.65 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF WM.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)		MAX. CSF (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	FACTORED FORCE (LBS)	MAX. CSF (LC)
			FROM	TO						
A-B	0	28	-11.8	-11.8	0.12 (1)	10.00	Q-C	-199	50	0.08 (1)
B-C	-3179	0	-11.8	-11.8	0.88 (1)	3.08	C-O	-623	0	0.87 (1)
C-D	-2856	0	-11.8	-11.8	0.78 (1)	3.45	O-E	0	463	0.10 (1)
D-E	-2856	0	-11.8	-11.8	0.78 (1)	3.45	E-N	0	363	0.08 (1)
E-F	-2530	0	-11.8	-11.8	0.30 (1)	4.08	N-F	-489	0	0.58 (1)
F-G	-2530	0	-11.8	-11.8	0.30 (1)	4.08	N-G	0	240	0.05 (1)
G-H	-2718	0	-11.8	-11.8	0.72 (1)	3.50	M-G	0	589	0.13 (1)
H-I	-2718	0	-11.8	-11.8	0.72 (1)	3.50	M-I	-820	0	0.37 (1)
I-T	-3483	0	-11.8	-11.8	0.30 (1)	3.11	K-I	0	127	0.04 (4)
T-J	-4008	0	-11.8	-11.8	0.59 (1)	2.91	B-Q	0	2891	0.65 (1)
R-B	-2125	0	0.0	0.0	0.14 (1)	6.97	S-T	0	681	0.00 (1)
R-Q	0	0	-18.5	-18.5	0.10 (4)	10.00				
Q-P	0	2873	-18.5	-18.5	0.40 (1)	10.00				
P-O	0	2873	-18.5	-18.5	0.40 (1)	10.00				
O-N	0	2350	-18.5	-18.5	0.32 (1)	10.00				
N-M	0	2411	-18.5	-18.5	0.35 (1)	10.00				
M-L	0	3100	-18.5	-18.5	0.46 (1)	10.00				
L-K	0	3100	-18.5	-18.5	0.46 (1)	10.00				
K-S	0	3100	-18.5	-18.5	0.57 (1)	10.00				
S-J	0	3100	-18.5	-18.5	0.57 (1)	10.00				

TOTAL WEIGHT = 174 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.5 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TRC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.25")
CALCULATED VERT. DEFL. (LL) = L/999 (0.16")
ALLOWABLE DEFL. (TL) = L/360 (1.25")
CALCULATED VERT. DEFL. (TL) = L/999 (0.31")

CSF: TC=0.90/1.00 (I-T), BC=0.57/1.00 (K-S), WB=0.87/1.00 (C-O), SS=0.41/1.00 (J-T)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (J) (INPUT = 0.90)
JSI METAL = 0.65 (Q) (INPUT = 1.00)

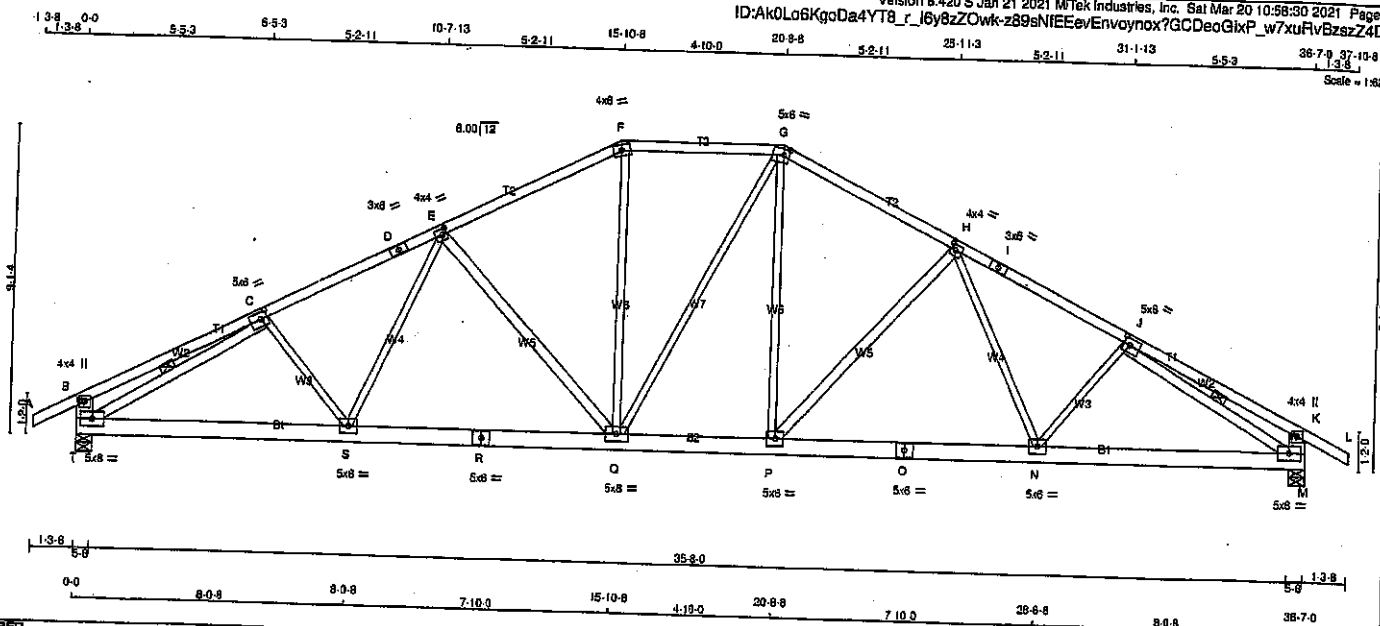


Structural component only
DWG# T-2108292

JOB NAME 417473	TRUSS NAME T46	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:AK0Lo6KqoDa4YT8_r_l6y8zZOWk-z89sNIEEvEnvocnoX?GCDeoGlxP_w7xuRvBzszZ4Dd

Scale = 1:82.2



N.L.G.A. RULES	CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
E - C	2x4	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV-p	MT20	4.0	4.0		
C TMWW-1	MT20	5.0	6.0	2.50	2.25
D TS-1	MT20	3.0	6.0		
E TMWW-1	MT20	4.0	4.0	2.00	1.50
F TTW-m	MT20	4.0	6.0		
G TTWW-m	MT20	5.0	6.0	2.25	2.00
H TMWW-1	MT20	4.0	4.0	2.00	1.50
I TS-1	MT20	3.0	6.0		
J TMWW-1	MT20	5.0	8.0		
K TMV-p	MT20	4.0	4.0	2.50	2.25
M BMWW-1	MT20	5.0	8.0		
N, P, S					
N BMWW-1	MT20	5.0	6.0		
O BS-1	MT20	5.0	6.0		
Q BMWW-1	MT20	5.0	8.0		
R BS-1	MT20	5.0	6.0		
T BMWW-1	MT20	5.0	8.0		

NOTES:

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	0	0
T	HORIZ	HORIZ	5-8	5-8
M	0	0	0	0

UNFACTORED REACTIONS

	1ST LOOSE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1512	1005 / 0	0 / 0	0 / 0	0 / 0	508 / 0	0 / 0
M	1512	1005 / 0	0 / 0	0 / 0	0 / 0	508 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-T, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO		
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	C-S	-124 / 32
B-C	0 / 20	-91.8	-91.8 0.32 (1)	10.00	E-Q	0 / 272
C-D	-2942 / 0	-91.8	-91.8 0.42 (1)	3.74	E-Q	-878 / 0
D-E	-2942 / 0	-91.8	-91.8 0.42 (1)	3.74	Q-G	0 / 830
E-F	-2348 / 0	-91.8	-91.8 0.38 (1)	4.14	Q-G	0 / 6
F-G	-2087 / 0	-91.8	-91.8 0.33 (1)	4.38	P-G	0 / 823
G-H	-2345 / 0	-91.8	-91.8 0.38 (1)	4.15	P-H	-882 / 0
H-I	-2944 / 0	-91.8	-91.8 0.42 (1)	3.74	H-N	0 / 277
I-J	-2944 / 0	-91.8	-91.8 0.42 (1)	3.74	N-J	-124 / 32
J-K	0 / 20	-91.8	-91.8 0.32 (1)	10.00	T-C	-3225 / 0
K-L	0 / 28	-91.8	-91.8 0.12 (1)	10.00	J-M	-3227 / 0
L-M	-324 / 0	0.0	0.0 0.02 (1)	7.81		
M-K	-324 / 0	0.0	0.0 0.02 (1)	7.81		
T-S	0 / 2710	-18.5	-18.5 0.40 (1)	10.00		
S-R	0 / 2526	-18.5	-18.5 0.35 (1)	10.00		
R-Q	0 / 2526	-18.5	-18.5 0.35 (1)	10.00		
Q-P	0 / 2064	-18.5	-18.5 0.30 (1)	10.00		
P-O	0 / 2525	-18.5	-18.5 0.35 (1)	10.00		
O-N	0 / 2525	-18.5	-18.5 0.35 (1)	10.00		
N-M	0 / 2711	-18.5	-18.5 0.40 (1)	10.00		

TOTAL WEIGHT = 186 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 8 OF CBC 2015, ABC 2015
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

ALLOWABLE DEFL.(LL) = L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSK: TC=0.42/1.00 (H-J:1), BC=0.40/1.00 (S-T:1), WB=0.67/1.00 (H-F:1), SS=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

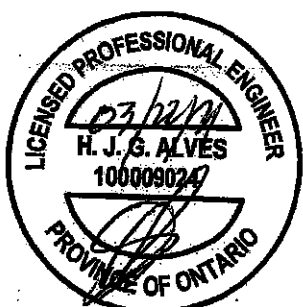
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.72 (J) (INPUT = 1.00)

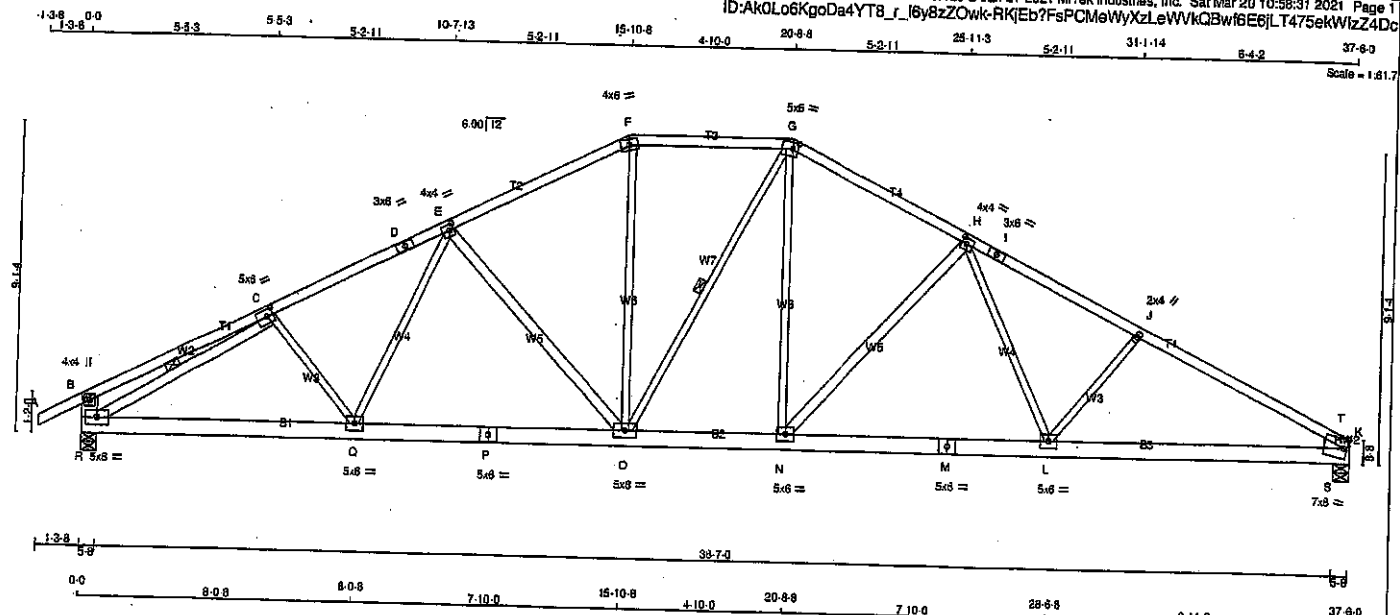


Structural component only
DWG# T-2108293

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
417473	T46X	1	1	GREENPARK HOMES		

Tamarack Roof Truss, Burlington

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ID:AK0Lo6KgoDa4YT8_r_l6y8ZOWk-RK[Eb?FsPCMeWyXzLeWwVkBwI6E6]LT475ekWizZ4Dc



CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF
D - F	2x4 DRY	No.2	SPF
F - G	2x4 DRY	No.2	SPF
G - I	2x4 DRY	No.2	SPF
I - K	2x4 DRY	No.2	SPF
R - B	2x6 DRY	No.2	SPF
R - P	2x6 DRY	No.2	SPF
P - M	2x6 DRY	No.2	SPF
M - K	2x6 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
EXCEPT			
E - O	2x4 DRY	No.2	SPF
N - H	2x4 DRY	No.2	SPF
R - C	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV4p	MT20	4.0	4.0		
C TMVW4t	MT20	5.0	6.0	2.50	2.25
D TS4	MT20	3.0	6.0		
E TMVW4t	MT20	4.0	4.0	2.00	1.50
F TTW4m	MT20	4.0	6.0		
G TTW4m	MT20	5.0	6.0	2.25	2.00
H TMVW4t	MT20	4.0	4.0	2.00	1.50
I TS4	MT20	3.0	6.0		
J TMVW4t	MT20	2.0	4.0		
K TMBH1-m	MT20	7.0	6.0	Edge	1.00
L N, Q					
L BMVW4t	MT20	5.0	6.0		
M BS4	MT20	5.0	6.0		
O BMVW4t	MT20	5.0	6.0		
P BS4	MT20	5.0	6.0		
R BMVW4t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

MEMB.	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
JT	2192	0	2067	0	5-8	5-8	2x4 R
R	2192	0	2067	0	5-8	5-8	
K	2067	0	2067	0	5-8	5-8	

UNFACTORED REACTIONS

MEMB.	1ST CASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM LIVE			
JT	1548	1029	0	0	0	519	0
R	1548	1029	0	0	0	502	0
K	1462	959	0	0	0	502	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-O, C-R

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	CS (LBS)	CS (LBS)			MAX. FACTORED FORCE (LBS)	MAX. CS (LBS)
FR-TO						FR-TO		
A-B	0.28	-91.8	-91.8	0.12 (1)	10.00	C-Q	-114.38	0.04 (1)
B-C	0.20	-91.8	-91.8	0.32 (1)	10.00	Q-E	0.263	0.08 (4)
C-D	-3037.0	-91.8	-91.8	0.43 (1)	3.89	E-O	-673.0	0.68 (1)
D-E	-3037.0	-91.8	-91.8	0.43 (1)	3.89	O-F	0.675	0.15 (1)
E-F	-2450.0	-91.8	-91.8	0.39 (1)	4.06	G-G	-62.0	0.04 (1)
F-G	-2179.0	-91.8	-91.8	0.34 (1)	4.30	N-G	0.748	0.17 (1)
G-H	-2484.0	-91.8	-91.8	0.40 (1)	4.03	H-H	-807.0	0.78 (1)
H-I	-3235.0	-91.8	-91.8	0.47 (1)	3.54	I-L	0.435	0.10 (1)
I-J	-3235.0	-91.8	-91.8	0.47 (1)	3.54	L-J	-408.0	0.13 (1)
J-T	-3540.0	-91.8	-91.8	0.63 (1)	3.23	R-C	-3318.0	0.63 (1)
T-K	-4147.0	-91.8	-91.8	0.56 (1)	2.92	S-T	0.831	0.00 (1)
R-B	-324.0	0.0	0.0	0.02 (1)	7.81			
R-Q	0.2788	-18.5	-18.5	0.41 (1)	10.00			
Q-P	0.2614	-18.5	-18.5	0.36 (1)	10.00			
P-O	0.2514	-18.5	-18.5	0.36 (1)	10.00			
O-N	0.2209	-18.5	-18.5	0.32 (1)	10.00			
N-M	0.2731	-18.5	-18.5	0.42 (1)	10.00			
M-L	0.2731	-18.5	-18.5	0.42 (1)	10.00			
L-S	0.3152	-18.5	-18.5	0.83 (1)	10.00			
S-K	0.3152	-18.5	-18.5	0.83 (1)	10.00			

TOTAL WEIGHT = 181 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (1.25")
CALCULATED VERT. DEFL.(LL) = L/959 (0.17")
ALLOWABLE DEFL.(TL) = L/380 (1.25")
CALCULATED VERT. DEFL.(TL) = L/959 (0.33")

CSI: TC=0.63/1.00 (J-T:1), BC=0.63/1.00 (L-S:1), WB=0.79/1.00 (H-N:1), SS=0.42/1.00 (K-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

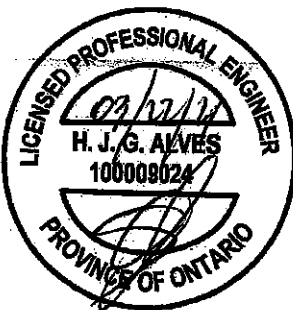
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

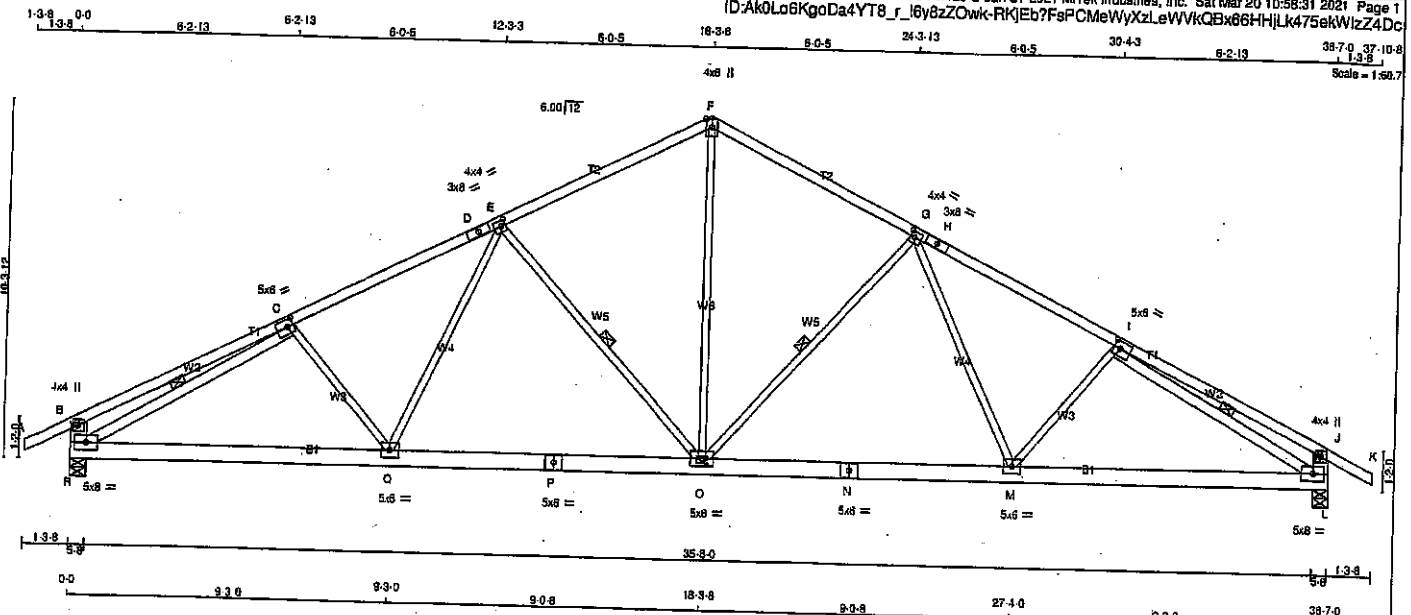
JSI GRIP = 0.90 (C) (INPUT = 0.90)
JSI METAL = 0.74 (C) (INPUT = 1.00)



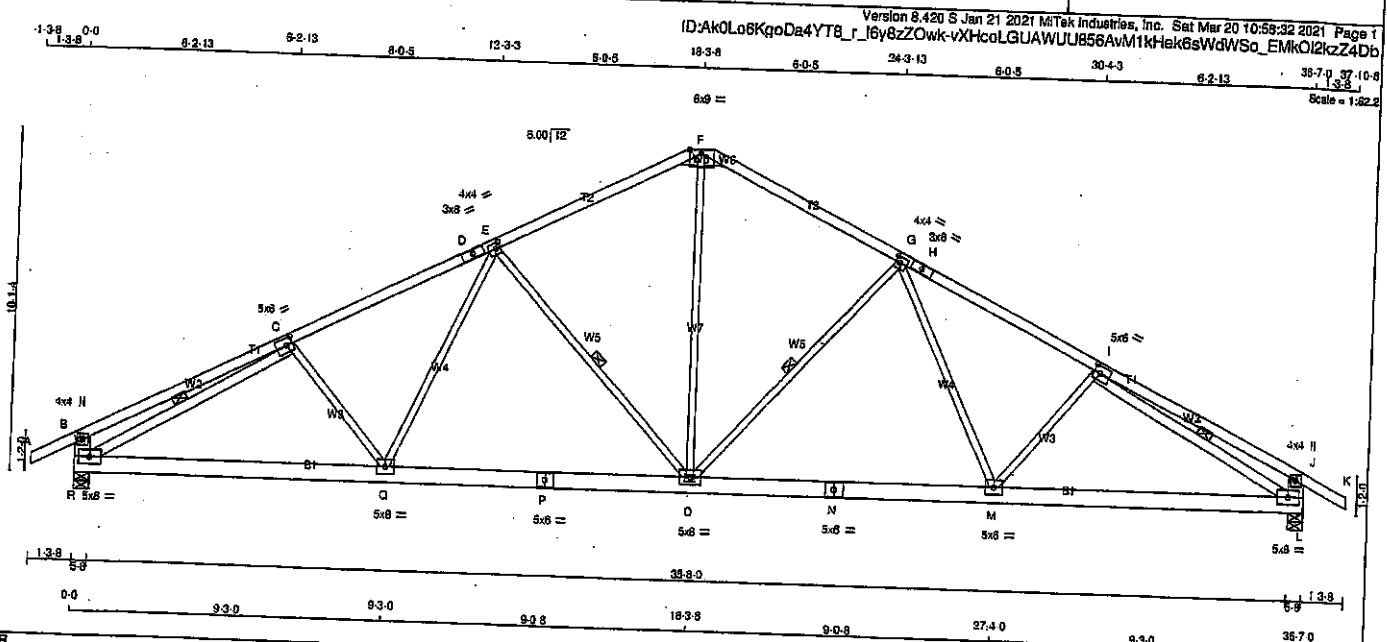
Structural component only
DWG# T-2108294

JOB NAME 417473	TRUSS NAME T47	QUANTITY 8	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 10:58:31 2021 Page 1
 (D:Ak0Lo6KgoDa4YT8_r_l6y8zzOwk-RKJeb?FsPCMeWyXzLeWVWkQBx86HHJLk475ekWizZ4Dc
 Scale = 1:50.7



JOB NAME 417473	TRUSS NAME T47CP	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



CHORDS	SIZE	DRY	NUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
R - P	2x6	DRY	No.2
P - N	2x6	DRY	No.2
N - L	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
R - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	4.0	4.0		
C TMWV-t	MT20	5.0	6.0	2.50	2.25
D TS-t	MT20	3.0	8.0		
E TMWV-t	MT20	4.0	4.0	2.00	1.50
F TMTMWV-t	MT20	6.0	9.0	1.25	4.50
G TMWV-t	MT20	4.0	4.0	2.00	1.50
H TS-t	MT20	3.0	8.0		
I TMWV-t	MT20	5.0	6.0	2.50	2.25
J TMV+p	MT20	4.0	4.0		
L BMWV-t	MT20	5.0	8.0		
M BMWV-t	MT20	5.0	6.0		
N BS-t	MT20	5.0	6.0		
O BMWV-t	MT20	5.0	8.0		
P BS-t	MT20	5.0	6.0		
Q BMWV-t	MT20	5.0	6.0		
R BMWV-t	MT20	5.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	RECORD BRG IN-SX
JT	VERT	HORZ	DOWN	HORZ
R	2141	0	2141	0
L	2141	0	2141	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS
R	COMBINED	SNOW	LIVE	PERM.LIVE
L	1512	1005.0	0.0	0.0
L	1512	1005.0	0.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-O, E-O, C-R, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
A-B	FR-TO	0.28	-91.8	-91.8	0.12 (1)	0-F	0.1428	0.32 (1)
B-C	FR-TO	0.24	-91.8	-91.8	0.44 (1)	0-G	-832.0	0.48 (1)
C-D	FR-TO	-2883.0	-91.8	-91.8	0.54 (1)	0-H	0.400	0.09 (1)
D-E	FR-TO	-2883.0	-91.8	-91.8	0.54 (1)	0-I	-248.2	0.10 (1)
E-F	FR-TO	-2117.0	-91.8	-91.8	0.48 (1)	0-J	-832.0	0.48 (1)
F-G	FR-TO	-2117.0	-91.8	-91.8	0.54 (1)	0-K	0.400	0.09 (1)
G-H	FR-TO	-2883.0	-91.8	-91.8	0.54 (1)	0-L	-248.2	0.10 (1)
H-I	FR-TO	-2883.0	-91.8	-91.8	0.54 (1)	0-M	-3227.0	0.77 (1)
I-J	FR-TO	0.24	-91.8	-91.8	0.44 (1)	0-N	0.1428	0.32 (1)
J-K	FR-TO	0.28	-91.8	-91.8	0.12 (1)	0-O	0.1428	0.32 (1)
K-L	FR-TO	-353.0	0.0	0.0	0.02 (1)	0-P	-832.0	0.48 (1)
L-M	FR-TO	-353.0	0.0	0.0	0.02 (1)	0-Q	-832.0	0.48 (1)
M-N	FR-TO	0.2735	-18.5	-18.5	0.43 (1)	0-R	-832.0	0.48 (1)
N-O	FR-TO	0.2419	-18.5	-18.5	0.37 (1)	0-S	-832.0	0.48 (1)
O-P	FR-TO	0.2419	-18.5	-18.5	0.37 (1)	0-T	-832.0	0.48 (1)
P-Q	FR-TO	0.2419	-18.5	-18.5	0.37 (1)	0-U	-832.0	0.48 (1)
Q-R	FR-TO	0.2419	-18.5	-18.5	0.37 (1)	0-V	-832.0	0.48 (1)
R-S	FR-TO	0.2735	-18.5	-18.5	0.43 (1)	0-W	-832.0	0.48 (1)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 089-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.27")

CSI: TC=0.54/1.00 (G-I), BC=0.43/1.00 (L-M), WB=0.77/1.00 (C-R), SI=0.23/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

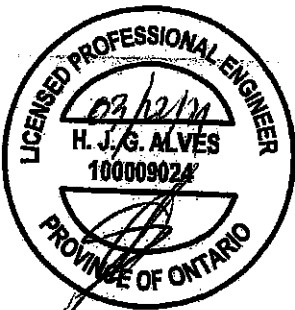
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 789 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

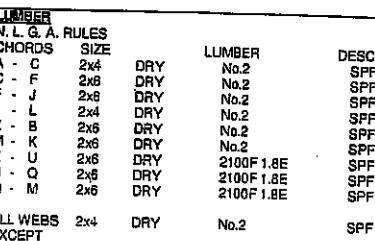
JSI GRIP= 0.87 (C) (INPUT = 0.90),
JSI METAL= 0.72 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108296

Tamarack Roof Truss, Burlington

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DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
A-C	1	12	TOP
J-L	1	12	TOP
C-F	2	12	TOP
F-J	2	12	TOP
X-B	2	12	SIDE(0.0)
M-K	2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
X-U	2	12	TOP
U-Q	2	12	SIDE(197.6)
Q-M	2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS			
S-G	1	3	SIDE(499.7)
S-H	2	4	SIDE(499.7)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD. BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
X	4084	0	4084	0	0	5-8	5-8
M	4812	0	4812	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
X	2880	1937 : 0	0 : 0	0 : 0	0 : 0	943 : 0	0 : 0
M	3392	2290 : 0	0 : 0	0 : 0	0 : 0	1102 : 0	0 : 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		WEBS	
MEMB.	FORCE	VERT. LOAD	CS1	MAX. UNBRAC	MEMB.	MAX. FORCE	MAX
	(LBS)	(PLF)	CS1 (LC)	LENGTH		(LBS)	CS1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	B-W	* 5806
B-C	-9421 / 0	-91.8	-91.8	0.63 (1)	3.41	W-C	-937 / 0
C-D	-7602 / 0	-91.8	-91.8	0.24 (1)	3.80	N-J	-2192 / 0
D-E	-12979 / 0	-91.8	-91.8	0.37 (1)	3.22	N-K	* 5110
E-F	-15234 / 0	-91.8	-91.8	0.51 (1)	2.86	T-E	-2340 / 0
F-G	-15234 / 0	-91.8	-91.8	0.51 (1)	2.86	P-H	-1759 / 0
G-Y	-9159 / 0	-91.8	-91.8	0.53 (1)	2.84	E-S	* 2959
Y-Z	-15159 / 0	-91.8	-91.8	0.53 (1)	2.84	S-G	0 / 319
Z-H	-15159 / 0	-91.8	-91.8	0.53 (1)	2.84	G-R	-810 / 0
H-AA	-13867 / 0	-91.8	-91.8	0.42 (1)	3.08	R-H	0 / 1696
AA-I	-13867 / 0	-91.8	-91.8	0.42 (1)	3.08	D-T	0 / 4143
I-J	-8667 / 0	-91.8	-91.8	0.25 (1)	3.77	C-V	0 / 5030
J-K	-5142 / 0	-91.8	-91.8	0.24 (1)	4.08	V-D	-3065 / 0
K-L	0 / 28	-91.8	-91.8	0.07 (1)	10.00	O-J	0 / 6631
X-B	-4018 / 0	0.0	0.0	0.14 (1)	7.10	P-I	0 / 5057
M-H	-4786 / 0	0.0	0.0	0.18 (1)	6.63	O-I	-3820 / 0

X-W	0.0	-18.5	-18.5	0.02 (1)	10.00
W-V	0 / 5784	-18.5	-18.5	0.18 (1)	10.00
V-U	0 / 9702	-18.5	-18.5	0.28 (1)	10.00
U-T	0 / 9702	-18.5	-18.5	0.28 (1)	10.00
T-S	0 / 12879	-18.5	-18.5	0.28 (1)	10.00
S-R	0 / 15166	-18.5	-18.5	0.45 (1)	10.00
R-AB	0 / 13867	-18.5	-18.5	0.50 (1)	10.00
AB-Q	0 / 13867	-18.5	-18.5	0.44 (1)	10.00
Q-P	0 / 13867	-18.5	-18.5	0.44 (1)	10.00
P-AC	0 / 9866	-18.5	-18.5	0.48 (1)	10.00
AC-AD	0 / 9866	-18.5	-18.5	0.48 (1)	10.00
AD-O	0 / 9866	-18.5	-18.5	0.48 (1)	10.00
O-N	0 / 4701	-18.5	-18.5	0.17 (1)	10.00
N-M	0/0	-18.5	-18.5	0.01 (1)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC.	LO1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	23-4-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
S	19-5-8	-1083	-1083	---	BACK	VERT	TOTAL	---	C1
Y	21-4-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
Z	23-4-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AA	24-10-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AB	21-4-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AC	21-4-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AD	25-8-8	-1002	-1002	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
-PART 9 OF BCBC 2018 , ABC 2019
-PART 9 OF OBC 2012 (2019 AMENDMENT)
-CSA 086-14
-TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (1.25")
CALCULATED VERT. DEFL.(LL) = $L/998$ (0.35")
ALLOWABLE DEFL.(TL)= $L/360$ (1.25")
CALCULATED VERT. DEFL.(TL) = $L/710$ (0.63")

CSI: TC=0.63/1.00 (B-C:1), BC=0.50/1.00 (R-S:1),
WB=0.59/1.00 (J-O:1), SSI=0.37/1.00 (O-P:1)

DOE LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873
III16	438	302	2547	1256	4283	1816

PLATE PLACEMENT TOL = 0.250 inches

LATE ROTATION TOL = 5.0 Deg.

SI GRIP= 0.89 (B) (INPUT = 0.90)
SI METAL= 0.88 (U) (INPUT = 1.00)



Structural component only
DWG# T-2108297 *11*

CONTINUED ON PAGE 2

JOB NAME 417473	TRUSS NAME T48	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						

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 ID: Ak0Lc6KqoDa4YT8 r l8y8zZOwk-rvPND1Hh7kCNPgy1n4CM3pRsJHwIMXo2iO6dzZ4DZ

PLATES (table is in inches)

PLATE	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	2.75
C	TTWW+m	MT20	5.0	8.0	Edge	
D, E, G, H						
D	TMWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.50
J	TTWW+m	MT20	5.0	8.0	2.75	2.25
K	TMVW-p	MT20	5.0	6.0	2.00	3.00
M	BMV1+p	MT20	3.0	6.0		
N, P, V						
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMWW-t	MT20	5.0	6.0	2.50	3.50
Q	BS-t	MT16	5.0	12.5	0.25	6.00
R	BMWW-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	8.0	4.25	2.50
T	BMWW-t	MT20	5.0	6.0		
U	BS-t	MT20	5.0	6.0		
W	BMWW-t	MT20	5.0	6.0	2.50	2.00
X	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

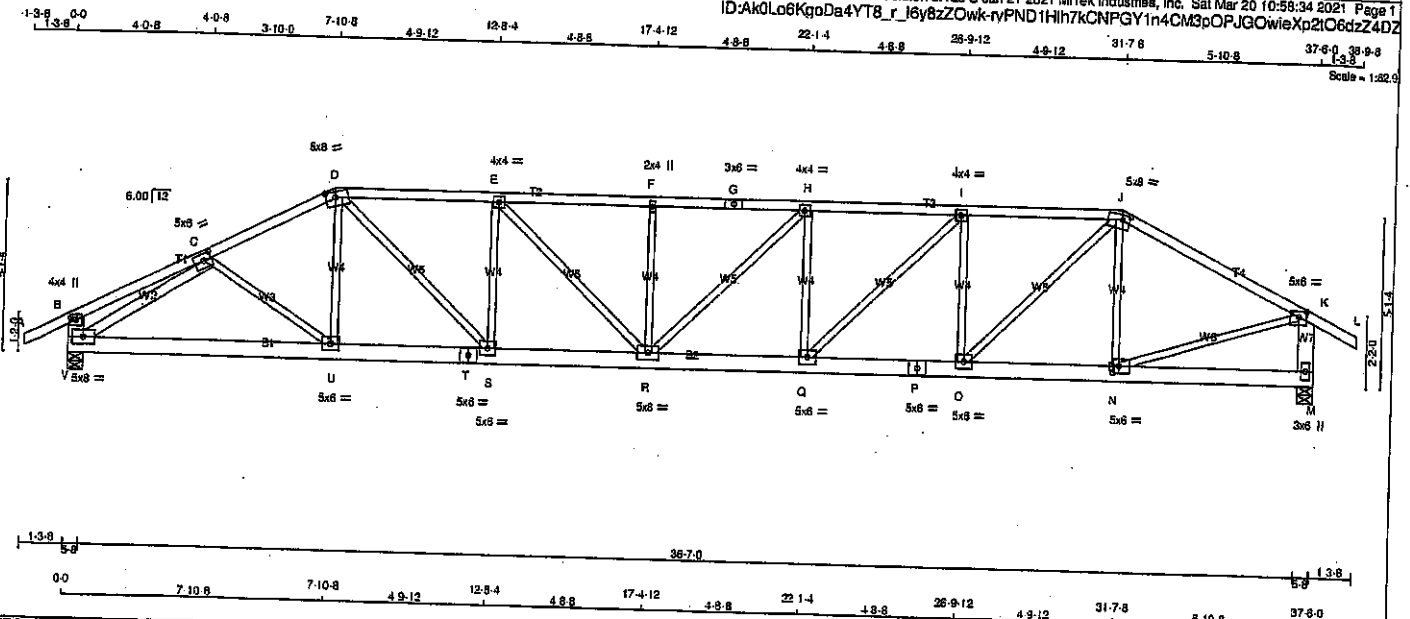


Structural component only
 DWG# T-2108297 *m*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
417473	T49	1	1	GREENPARK HOMES		

Tamarack Roof Truss, Burlington

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LUMBER

N.L.G.A. RULES

CHORDS SIZE

A - D 2x4 DRY No.2

D - G 2x4 DRY No.2

G - J 2x4 DRY No.2

J - L 2x4 DRY No.2

V - B 2x6 DRY No.2

M - K 2x6 DRY No.2

V - T 2x6 DRY No.2

T - P 2x6 DRY No.2

P - M 2x6 DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

V - C 2x4 DRY No.2

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

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SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION MAXIMUM FACTORED INPUT REQD

JT VERT HORZ DOWN HORZ UPLIFT IN-SX BRG BRG

V 2192 0 2192 0 0 5-8 5-8

M 2192 0 2192 0 0 5-8 5-8

UNFACTORED REACTIONS

1ST LCASE MAX/MIN COMPONENT REACTIONS

JT COMBINED SNOW LIVE PERM LIVE WIND DEAD SOIL

V 1548 1029 0 0 0 0 0 519 0 0 0

M 1548 1029 0 0 0 0 0 519 0 0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.04 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MAX. FACTORED FORCE (LBS)

FR-TO VERT. LOAD LC1 MAX. FACTORED FORCE (LBS)

A-B 0 28 -91.8 -91.8 0.12 (1) 10.00

B-C 0 16 -91.8 -91.8 0.20 (1) 10.00

C-D -3081 0 -91.8 -91.8 0.34 (1) 3.70

D-E -3710 0 -91.8 -91.8 0.54 (1) 3.23

E-F -4095 0 -91.8 -91.8 0.59 (1) 3.04

F-G -4095 0 -91.8 -91.8 0.50 (1) 3.15

G-H -4095 0 -91.8 -91.8 0.50 (1) 3.15

H-I -3995 0 -91.8 -91.8 0.57 (1) 3.09

I-J -3380 0 -91.8 -91.8 0.50 (1) 3.41

J-K -2452 0 -91.8 -91.8 0.79 (1) 3.53

K-L 0 28 -91.8 -91.8 0.12 (1) 10.00

L-M -270 0 0 0 0.02 (1) 7.81

M-K -2143 0 0 0 0.16 (1) 6.95

V-U 0 2870 -18.5 -18.5 0.39 (1) 10.00

U-T 0 2740 -18.5 -18.5 0.37 (1) 10.00

T-S 0 2740 -18.5 -18.5 0.37 (1) 10.00

S-R 0 3710 -18.5 -18.5 0.49 (1) 10.00

R-Q 0 3995 -18.5 -18.5 0.53 (1) 10.00

Q-P 0 3380 -18.5 -18.5 0.44 (1) 10.00

P-O 0 3380 -18.5 -18.5 0.44 (1) 10.00

O-N 0 2180 -18.5 -18.5 0.29 (1) 10.00

N-M 0 0 -18.5 -18.5 0.07 (4) 10.00

WEBS

MAX. FACTORED FORCE (LBS)

FR-TO VERT. LOAD LC1 MAX. FACTORED FORCE (LBS)

C-U 0 93 0.03 (4) 0.03 (4)

U-D 0 113 0.04 (4) 0.04 (4)

N-J -532 0 0.19 (1) 0.19 (1)

V-C -3253 0 0.82 (1) 0.82 (1)

N-K 0 2286 0.51 (1) 0.51 (1)

C-J 0 1681 0.38 (1) 0.38 (1)

D-S 0 1358 0.31 (1) 0.31 (1)

O-I -1103 0 0.40 (1) 0.40 (1)

S-E -875 0 0.31 (1) 0.31 (1)

Q-I 0 872 0.20 (1) 0.20 (1)

E-R 0 548 0.12 (1) 0.12 (1)

Q-H -524 0 0.19 (1) 0.19 (1)

R-F -417 0 0.15 (1) 0.15 (1)

R-H 0 144 0.08 (1) 0.08 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 5.0 PSF

BOT CH. LL = 5.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019

- PART 9 OF CBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.25")

CALCULATED VERT. DEFL. (LL) = L/999 (0.24")

ALLOWABLE DEFL. (TL) = L/360 (1.25")

CALCULATED VERT. DEFL. (TL) = L/999 (0.44")

OSI: TC=0.79/1.00 (JK:1), BC=0.53/1.00 (Q-R:1), WB=0.82/1.00 (C-V:1), SSI=0.21/1.00 (D-E:1)

DCL LUMBER=1.00 NAIL=1.00 LB BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1973

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (D) (INPUT = 0.90)

JSI METAL = 0.74 (C) (INPUT = 1.00)

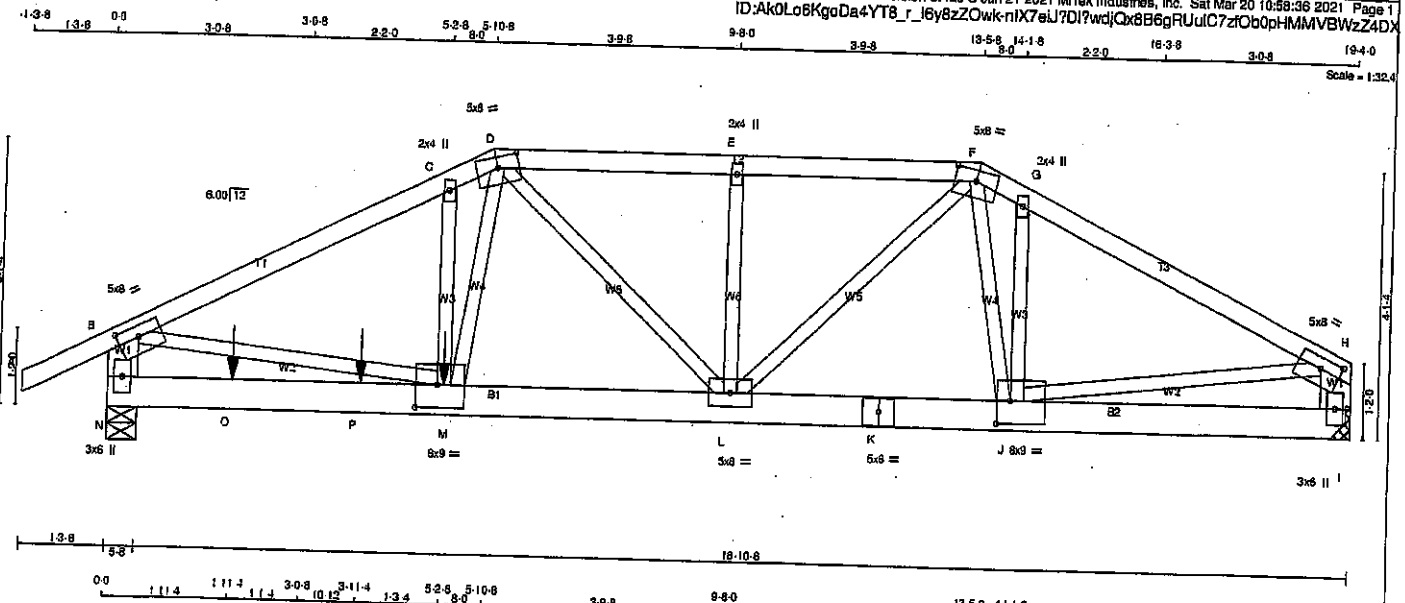


Structural component only
DWG# T-2108298

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417473	T50	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER					
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER		DESCR.	
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
N - B	2x6	DRY	No.2	SPF	
I - H	2x6	DRY	No.2	SPF	
N - K	2x6	DRY	No.2	SPF	
K - I	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-1	MT20	5.0	8.0	2.00	3.75
C, E, G					
C TMVW-w	MT20	2.0	4.0		
D TTWW-m	MT20	5.0	8.0	2.00	4.00
F TTWW-m	MT20	5.0	8.0	2.00	4.00
H TMVW-1	MT20	5.0	8.0	2.00	3.75
I BMV1+p	MT20	3.0	6.0		
J BMVWW-1	MT20	8.0	9.0	4.25	2.50
K BS-1	MT20	5.0	6.0		
L BMVWW-1	MT20	5.0	8.0		
M BMVWW-1	MT20	8.0	9.0	4.25	4.00
N BMV1+p	MT20	3.0	6.0		

NOTES-

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
N	2574	0	2574	0	0	5-8	5-8	5-8
I	1582	0	1582	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3'-8".

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
N	1816	1219	0	0	0	598	0
I	1117	743	0	0	0	373	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 28	-91.8	-91.8 0.13 (1)	10.00	L-E	-418 / 0	0.11 (1)
B-C	-3709 / 0	-91.8	-91.8 0.71 (1)	3.03	B-M	0 / 3365	0.83 (1)
C-D	-3786 / 0	-91.8	-91.8 0.41 (1)	3.17	M-C	-493 / 0	0.11 (1)
D-E	-2764 / 0	-91.8	-91.8 0.33 (1)	3.84	J-H	0 / 2085	0.52 (1)
E-F	-2764 / 0	-91.8	-91.8 0.33 (1)	3.84	J-G	-575 / 0	0.13 (1)
F-G	-2398 / 0	-91.8	-91.8 0.39 (1)	3.94	M-D	0 / 1942	0.48 (1)
G-H	-2289 / 0	-91.8	-91.8 0.47 (1)	4.03	F-J	0 / 348	0.09 (1)
H-B	-2480 / 0	0.0	0.0 0.17 (1)	8.52	D-L	-330 / 0	0.15 (1)
I-H	-1527 / 0	0.0	0.0 0.11 (1)	7.81	L-F	0 / 1106	0.27 (1)
N-O	0 / 0	-18.5	-18.5 0.13 (1)	10.00			
O-P	0 / 0	-18.5	-18.5 0.13 (1)	10.00			
P-M	0 / 0	-18.5	-18.5 0.13 (1)	10.00			
M-L	0 / 2999	-18.5	-18.5 0.48 (1)	10.00			
L-K	0 / 1979	-18.5	-18.5 0.29 (1)	10.00			
K-J	0 / 1979	-18.5	-18.5 0.29 (1)	10.00			
J-I	0 / 0	-18.5	-18.5 0.07 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	FACE	DIR.	TYPE	HEEL	CONN.
M	5-2-8	-1298	-1298	---	BACK	VERT	TOTAL	C1
O	1-11-4	-20	-20	---	BACK	VERT	TOTAL	C1
P	3-11-4	-21	-21	---	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

(1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6:00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CS1: TC=0.71/1.00 (B-C:1), BC=0.48/1.00 (L-M:1), WB=0.83/1.00 (B-M:1), SS=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 785

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (B) (INPUT = 0.90)
JSI METAL = 0.69 (H) (INPUT = 1.00)

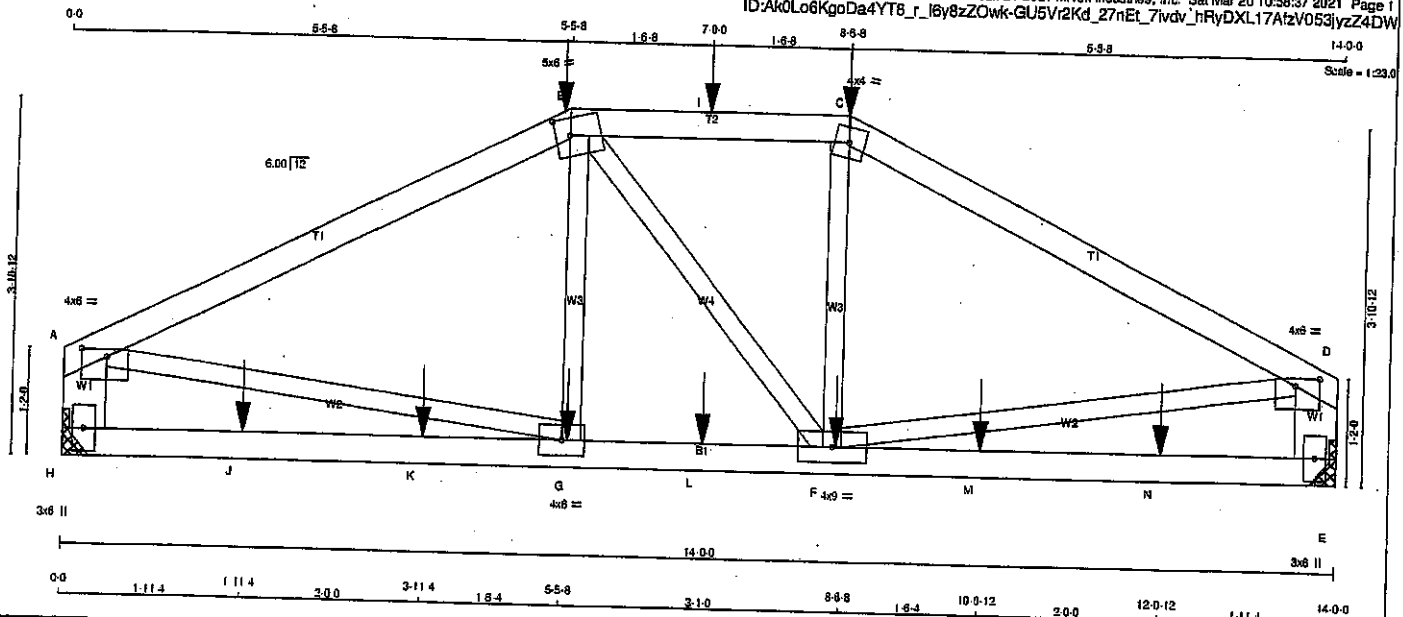


Structural component only
DWG# T-2108299

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417473	T51	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER

N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
H - A	2x6	DRY	No.2
E - D	2x6	DRY	No.2
H - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - B	MT20	4.0	5.0	1.00	3.25
B - C	MT20	5.0	6.0	2.25	2.00
C - D	MT20	4.0	4.0		
D - E	MT20	4.0	6.0	1.00	3.25
E - F	MT20	3.0	6.0		
F - G	MT20	4.0	6.0		
G - H	MT20	4.0	6.0		
H - E	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
H	1249	0	1249	0
E	1249	0	1249	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, E. MINIMUM BEARING LENGTH AT JOINT H = 3-8, JOINT E = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
H	COMBINED	SNOW	LIVE			
E	885	572.0	0.0	0.0	313.0	0.0
	885	572.0	0.0	0.0	313.0	0.0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
		FORCE	(PLF)								
FR-TO											
A-B	-1598.0		-91.8	-91.8	0.63	(1)	4.37	G-B	-74.01	0.03	(4)
B-I	-1428.0		-91.8	-91.8	0.31	(1)	5.05	B-F	0.3	0.00	(4)
I-C	-1428.0		-91.8	-91.8	0.31	(1)	5.05	F-C	-71.93	0.04	(4)
C-D	-1801.0		-91.8	-91.8	0.63	(1)	4.38	A-G	0.1445	0.36	(1)
H-A	-1187.0		0.0	0.0	0.08	(1)	7.81	F-D	0.1448	0.36	(1)
E-D	-1188.0		0.0	0.0	0.08	(1)	7.81				
H-J	0.0		-18.5	-18.5	0.22	(4)	10.00				
J-K	0.0		-18.5	-18.5	0.22	(4)	10.00				
K-G	0.0		-18.5	-18.5	0.22	(4)	10.00				
G-L	0.1428		-18.5	-18.5	0.34	(1)	10.00				
L-F	0.1428		-18.5	-18.5	0.34	(1)	10.00				
F-M	0.0		-18.5	-18.5	0.22	(4)	10.00				
M-N	0.0		-18.5	-18.5	0.22	(4)	10.00				
N-E	0.0		-18.5	-18.5	0.22	(4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
B	5-5-8	-242	-242		BACK	VERT	TOTAL		C1
C	8-8-8	-242	-242		BACK	VERT	TOTAL		C1
F	8-5-12	-19	-19		BACK	VERT	TOTAL		C1
G	5-6-4	-19	-19		BACK	VERT	TOTAL		C1
I	7-0-0	-66	-66		BACK	VERT	TOTAL		C1
J	1-11-4	-18	-18		BACK	VERT	TOTAL		C1
K	3-11-4	-19	-19		BACK	VERT	TOTAL		C1
L	7-0-0	-19	-19		BACK	VERT	TOTAL		C1
M	10-0-12	-19	-19		BACK	VERT	TOTAL		C1
N	12-0-12	-18	-18		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIG 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.63/1.00 (C-D:1), BC=0.34/1.00 (F-G:1),
WB=0.38/1.00 (D-F:1), SS=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

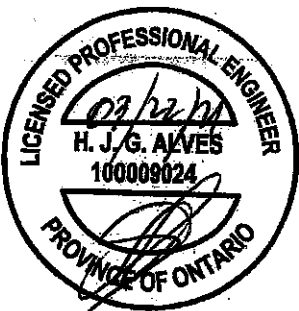
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650	371	1747 788 1887 1873

PLATE PLACEMENT TOL. = 0.250 inches

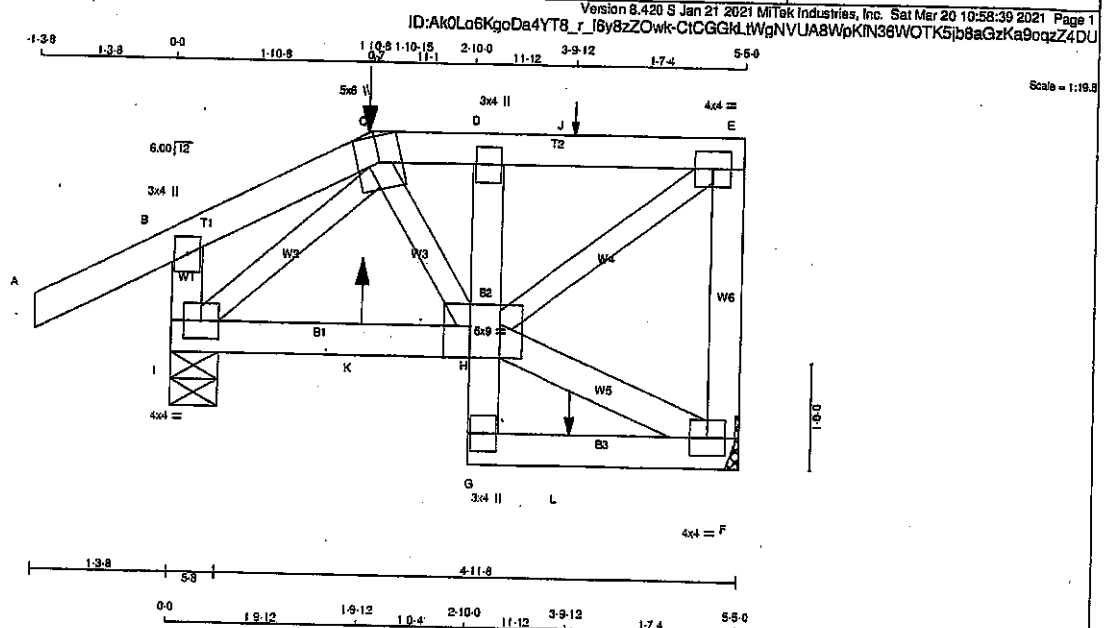
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (G) (INPUT = 0.90)
JSI METAL = 0.35 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108300

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417473	T53	2	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington					



LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
I - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
G - D	2x4	DRY	No.2
G - F	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
EXCEPT			
H - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+m	MT20	5.0	8.0		
D	TMV+p	MT20	3.0	4.0		
E	TMVW-l	MT20	4.0	4.0		
F	BMVW-l	MT20	4.0	4.0		
G	BMV+p	MT20	3.0	4.0		
H	BMVWVW-l	MT20	6.0	9.0	Edge	3.00
I	BMVW-l	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM	FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG	IN-SX
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
F	306	0	306	0	0
I	436	0	436	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	217	142.0	0.0	0.0	0.0	74.0	0.0
I	306	214.0	0.0	0.0	0.0	92.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
FR-TO			FROM	TO	UNBRAC	FR-TO			
A-B	0.728	-91.8	-91.8	0.13 (1)	10.00	H-F	-12.0	0.00 (1)	
B-C	0.0	-91.8	-91.8	0.06 (1)	10.00	H-E	0.306	0.08 (1)	
C-D	-292.0	-91.8	-91.8	0.08 (1)	6.25	C-H	0.83	0.02 (1)	
D-J	-246.0	-91.8	-91.8	0.08 (1)	6.25	I-C	-289.0	0.05 (1)	
J-E	-246.0	-91.8	-91.8	0.08 (1)	6.25				
F-E	-279.0	0.0	0.0	0.05 (1)	7.81				
I-B	-212.0	0.0	0.0	0.02 (1)	7.81				
K-H	0.7210	-18.5	-18.5	0.07 (4)	10.00				
G-H	0.728	-18.5	-18.5	0.07 (4)	10.00				
H-D	-212.0	0.0	0.0	0.02 (1)	10.00				
G-L	0.711	-18.5	-18.5	0.04 (4)	10.00				
L-F	0.713	-18.5	-18.5	0.04 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-10-15	-4	-4	-	FRONT	VERT	DEAD	-	C1
C	1-10-15	-1	-1	-	BACK	VERT	TOTAL	-	C1
C	1-10-15	-14	-14	-	FRONT	VERT	SNOW	-	C1
J	3-9-12	1	1	76	BACK	VERT	TOTAL	-	C1
K	1-9-12	4	4	8	BACK	VERT	TOTAL	-	C1
L	3-9-12	-0	-0	-	BACK	VERT	TOTAL	-	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 57 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDT'L. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TO=0.13/1.00 (A-B-1), BO=0.07/1.00 (H-I-4), WB=0.08/1.00 (E-H-1), SSI=0.11/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

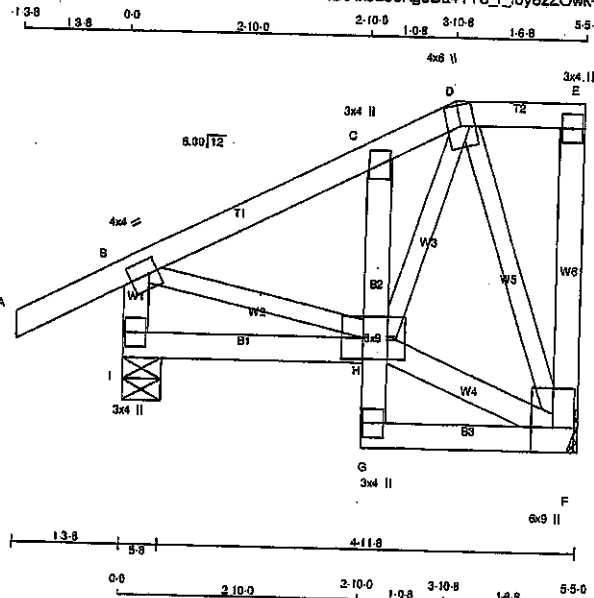
JSI GRIP= 0.30 (C) (INPUT = 0.90)
JSI METAL= 0.10 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108302

JOB NAME 417473	TRUSS NAME T54	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/4" = 1'-0"

LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
G - C	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
H - F	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	TMVW+m	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVWV1+p	MT20	6.0	9.0	Edge	
G	BMV+p	MT20	3.0	4.0		
H	BMVWVW-I	MT20	6.0	9.0	3.00	3.00
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
F	299	0	299	0	0	MECHANICAL	5-8	5-8
I	423	0	423	0	0	5-8	5-8	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1'-8".

UNFACTORED REACTIONS

JT	1ST LOSE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	211	139'0"	0'0"	0'0"	0'0"	73'0"	0'0"
I	297	208'0"	0'0"	0'0"	0'0"	89'0"	0'0"

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0'28	-91.8	-91.8 0.12 (1)	10.00	B-H	0'209	0.05 (1)
B-C	-215'0	-91.8	-91.8 0.10 (1)	6.25	H-F	0'87	0.01 (1)
C-D	-238'0	-91.8	-91.8 0.08 (1)	8.25	H-D	0'294	0.07 (1)
D-E	0'0	-91.8	-91.8 0.04 (1)	10.00	D-F	-258'0	0.07 (1)
F-E	-69'0	0.0	0.0 0.02 (1)	7.81			
I-B	-396'0	0.0	0.0 0.04 (1)	7.81			
I-H	0'0	-18.5	-18.5 0.05 (4)	10.00			
G-H	0'25	0.0	0.0 0.02 (1)	10.00			
H-C	-254'0	0.0	0.0 0.02 (1)	7.81			
G-F	0'14	-18.5	-18.5 0.03 (4)	10.00			

TOTAL WEIGHT = 2 X 33 = 66 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF OSC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.12/1.00 (A-B:1), BC=0.05/1.00 (H-I:4), WB=0.07/1.00 (D-H:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.33 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

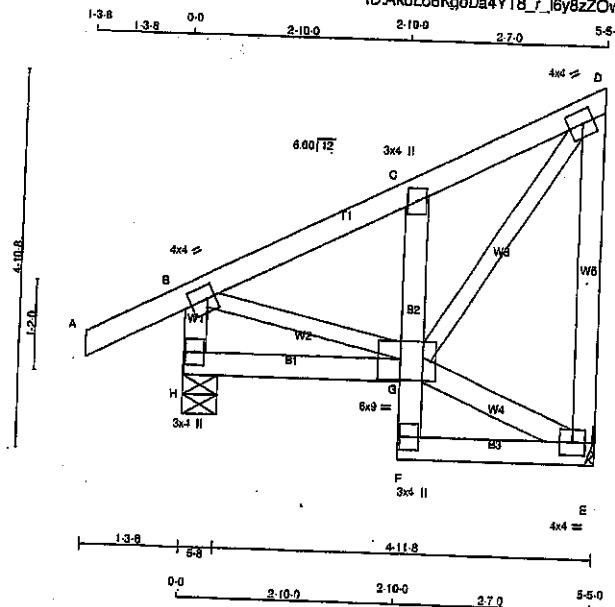


Structural component only
DWG# T-2108303

JOB NAME 417473	TRUSS NAME T55	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Famarack Roof Truss, Burlington

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Scale = 1/2" = 1'-0"

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT G - E	2x3	DRY	No.2	SPF
	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-p	MT20	3.0	4.0		
D	TMVW-t	MT20	4.0	4.0	2.00	1.75
E	BMVW-t	MT20	4.0	4.0		
F	BMVW-p	MT20	3.0	4.0		
G	BMVW-t	MT20	6.0	9.0	3.25	3.50
H	BMVW-p	MT20	3.0	4.0		

NOTES:

- (1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORZ	DOWN	HORZ
E 299	0	299	0
H 423	0	423	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-2.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS
COMBINED	SNOW	LIVE
E 211	139.0	0.0
H 297	208.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO							
A-B	0.128						
B-C	-220.0	-91.8	-91.8	0.12 (1)	10.00	B-G	0.215
C-D	-228.0	-91.8	-91.8	0.10 (1)	8.25	G-E	-17.0
E-D	-270.0	0.0	0.0	0.10 (1)	7.81	G-D	0.340
H-B	-398.0	0.0	0.0	0.04 (1)	7.81		
H-G	0.0	-18.5	-18.5	0.05 (4)	10.00		
F-G	0.25	0.0	0.0	0.02 (1)	10.00		
G-C	-297.0	0.0	0.0	0.02 (1)	7.81		
F-E	0.15	-18.5	-18.5	0.03 (4)	10.00		

TOTAL WEIGHT = 2 X 32 = 64 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.60")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.61")

CSI: TC=0.12/1.00 (A-B:1) ; BC=0.05/1.00 (G-H:4) ; WB=0.08/1.00 (D-G:1) ; SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

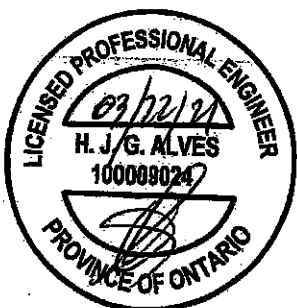
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 768 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

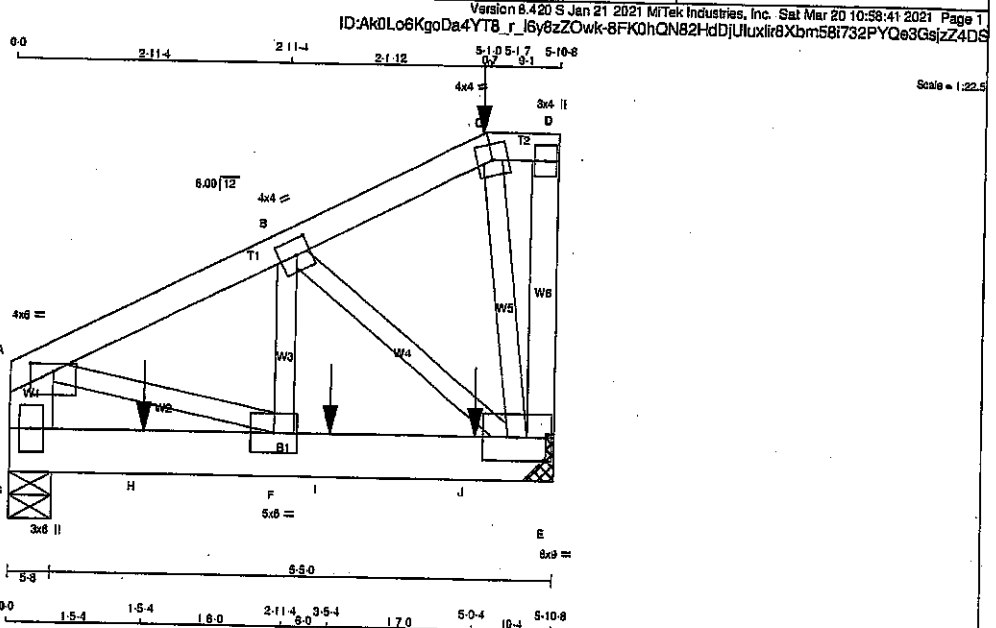
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.35 (D) (INPUT = 0.60)
JSI METAL= 0.11 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108304

JOB NAME 417473	TRUSS NAME T56	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
G - A	2x6	DRY	No.2
G - E	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(81.0)
C-D	12	SIDE(81.0)
D-E	12	TOP
G-A	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
G-E	12	SIDE(0.0)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	8.0	1.00	3.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TTW-m	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	JT	GROSS REACTION	BRG	BRG	BRG	BRG
E	1867 0	1867 0	1867 0	5-8	5-8	5-8	5-8
G	873 0	873 0	873 0	5-8	5-8	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 4-0.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	1313	906 0	0 0	0 0	0 0	408 0	0 0
G	811	437 0	0 0	0 0	0 0	174 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO	
A-B	-1005 0	-91.8 -91.8 0.06 (1)	6.25	A-F	0 638
B-C	-95 0	-91.8 -91.8 0.04 (1)	6.25	F-B	0 772
C-D	0 0	-91.8 -91.8 0.00 (1)	10.00	C-E	-388 0
E-D	-35 0	0.0 0.0 0.00 (1)	7.81	B-E	-1047 0
G-A	-808 0	0.0 0.0 0.03 (1)	7.81		
G-H	0 0	-18.5 -18.5 0.10 (1)	10.00		
H-F	0 0	-18.5 -18.5 0.10 (1)	10.00		
F-I	0 907	-18.5 -18.5 0.33 (1)	10.00		
I-J	0 907	-18.5 -18.5 0.33 (1)	10.00		
J-E	0 907	-18.5 -18.5 0.33 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-1-7	-179	-179	---	FRONT	VERT	TOTAL	---	C1
H	1-5-4	-177	-177	---	TOP	VERT	TOTAL	---	C1
I	3-5-4	-239	-239	---	TOP	VERT	TOTAL	---	C1
J	5-0-4	-871	-871	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 31 = 62 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.06/1.00 (A-B:1), BC=0.33/1.00 (E-F:1), WB=0.12/1.00 (B-E:1), SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.50 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108305 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417473	T56	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 10:58:41 2021 Page 2
ID: Ak0Lo6KooDe4YT8 r l6v8zZOwk-8FK0hQN82HdDlUluxlr8Xbm58i732PYQe3GajzZ4DS

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	TMV+p	MT20	3.0	4.0		
E	BMVWW1+	MT20	6.0	8.0		
F	BMVW1+	MT20	5.0	8.0		
G	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

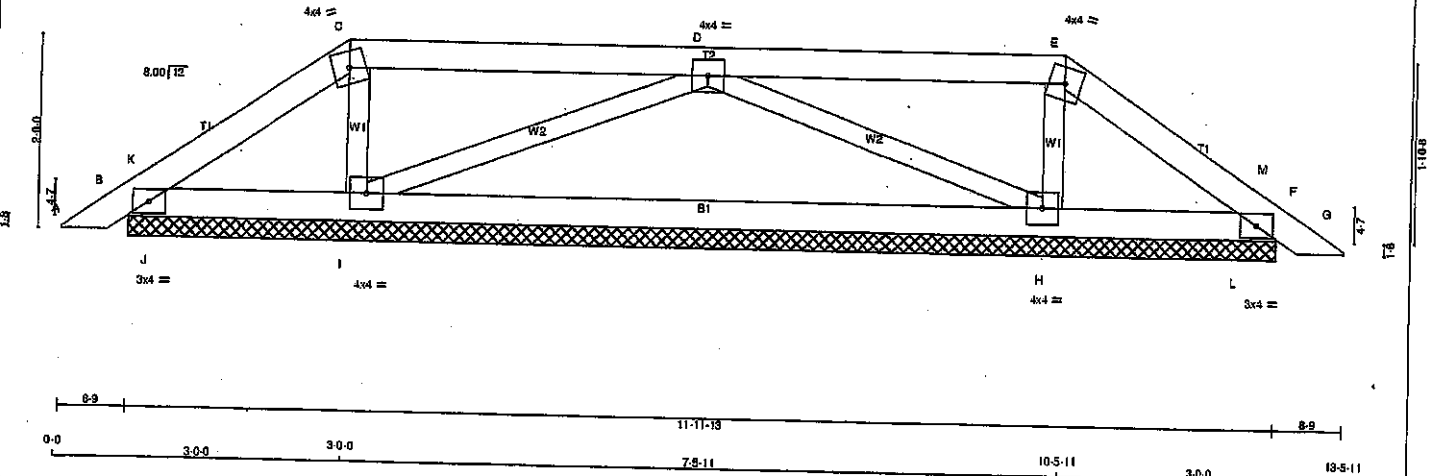
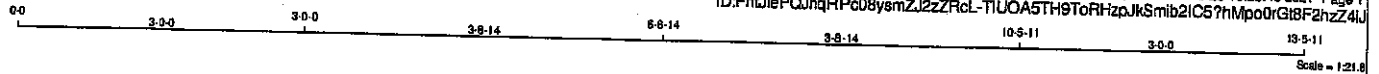


Structural component only
DWG# T-2108305 2/2

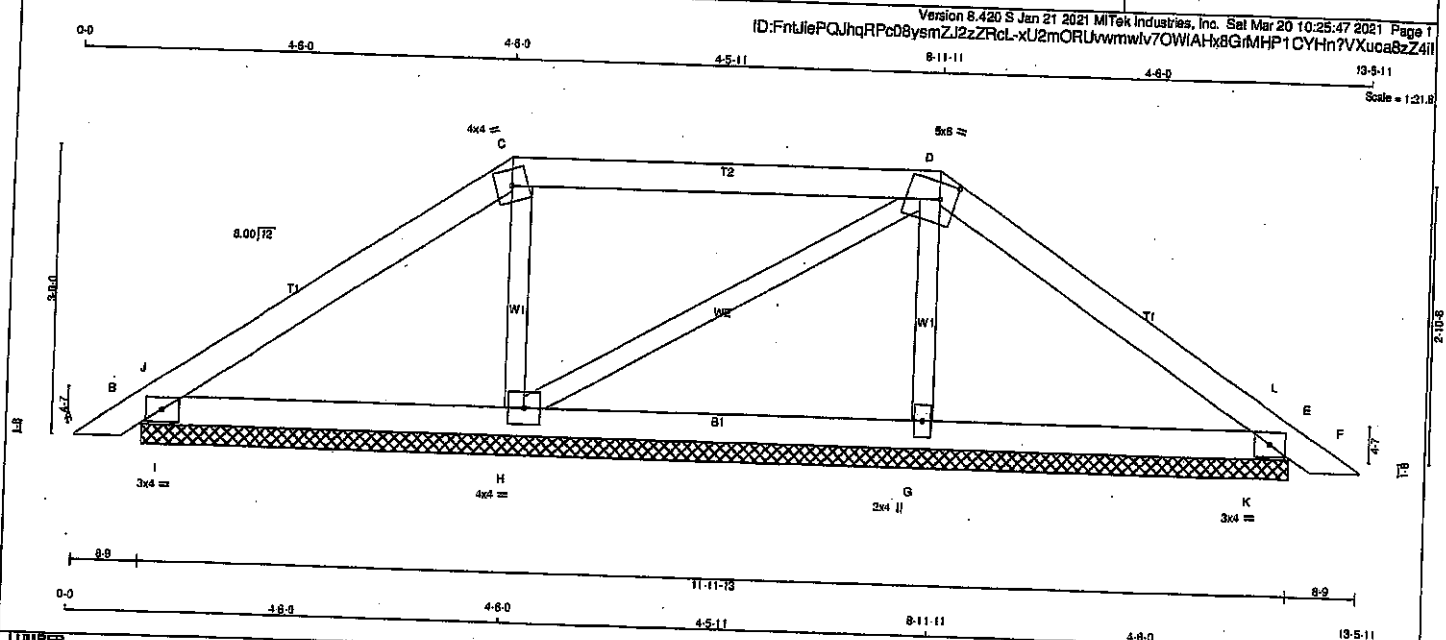
JOB NAME 417467	TRUSS NAME PB1	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Sat Mar 20 10:25:48 2021 Page 1
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417467	PB2	2	1	TRUSS DESC.		
Tamarack Roof Truss, Burlington						



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - O	2x4	DRY	No.2
D - F	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TTWV-m	MT20	5.0	6.0	2.00	2.00
E	TMB1-I	MT20	3.0	4.0		
G	BMW1-w	MT20	2.0	4.0		
H	BMWV1-I	MT20	4.0	4.0		

NOTES-

- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	315	0	315	0	11-11-13	11-11-13
E	335	0	335	0	11-11-13	11-11-13
H	422	0	422	0	11-11-13	11-11-13
G	367	0	367	0	11-11-13	11-11-13

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
B	220	158.0	0.0	0.0	0.0	83.0
E	234	189.0	0.0	0.0	0.0	65.0
H	300	192.0	0.0	0.0	0.0	108.0
G	262	161.0	0.0	0.0	0.0	101.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO	UNBRAC LENGTH	FR-TO		
A-B	0.15	-91.8	-91.8 0.03 (1)	10.00	H-C	-293.0 0.05 (1)	
B-J	-31.0	-91.8	-91.8 0.06 (1)	6.25	H-D	-35.0 0.01 (1)	
J-C	-112.0	-91.8	-91.8 0.16 (1)	6.25	G-D	-258.0 0.04 (1)	
C-D	-76.0	-91.8	-91.8 0.31 (1)	6.25	I-J	-308.0 0.00 (1)	
D-L	-147.0	-91.8	-91.8 0.16 (1)	6.25	K-L	-305.0 0.00 (1)	
L-E	-82.0	-91.8	-91.8 0.05 (1)	6.25			
E-F	0.15	-91.8	-91.8 0.03 (1)	10.00			
B-I	0.88	-18.5	-18.5 0.14 (1)	10.00			
I-H	0.88	-18.5	-18.5 0.14 (1)	10.00			
H-G	0.106	-18.5	-18.5 0.08 (1)	10.00			
G-K	0.116	-18.5	-18.5 0.14 (1)	10.00			
K-E	0.116	-18.5	-18.5 0.14 (1)	10.00			

TOTAL WEIGHT = 2 X 39 = 77 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.31/1.00 (C-D:1), BC=0.14/1.00 (G-K:1), WB=0.05/1.00 (C-H:1), SS=0.24/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

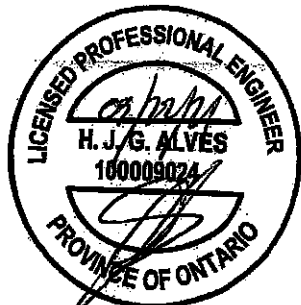
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
			788
			1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

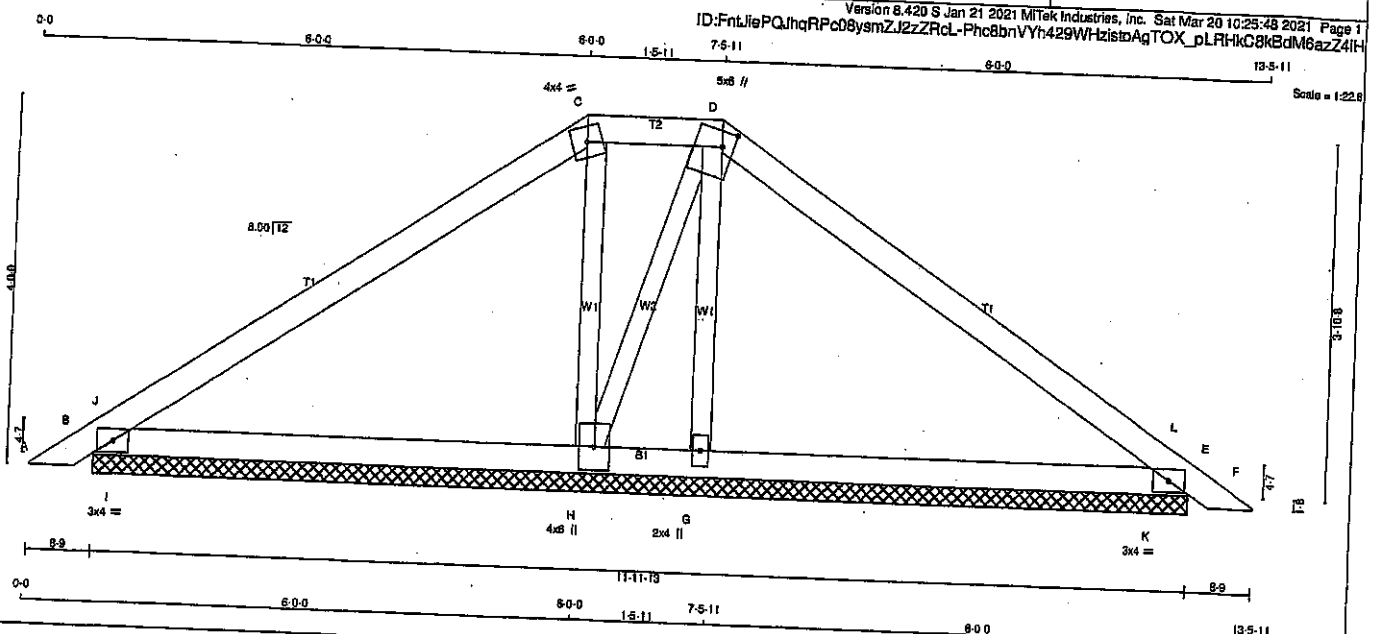
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (E) (INPUT = 0.90)
JSI METAL= 0.07 (E) (INPUT = 1.00)



Structural component only
DWG# T-2108221

JOB NAME 417467	TRUSS NAME PB3	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 10:25:48 2021 Page 1	



LUMBER					
N. L. G. A. RULES					
CHORDS	SIZE		LUMBER		DESCR
A - C	2x4	DRY	No.2		SPF
C - D	2x4	DRY	No.2		SPF
D - F	2x4	DRY	No.2		SPF
B - E	2x4	DRY	No.2		SPF
ALL WEBS 2x3 DRY			No.2		SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMB1-I	MT20	3.0	4.0			
C TTW-m	MT20	4.0	4.0			
D TTW-w	MT20	5.0	6.0	2.00	1.50	
E TMB1-I	MT20	3.0	4.0			
G BMW1-w	MT20	2.0	4.0			
H BMW1-w	MT20	4.0	6.0			

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	432	0	432	0	11-11-13	11-11-13
E	446	0	446	0	11-11-13	11-11-13
H	349	0	349	0	11-11-13	11-11-13
G	212	0	212	0	11-11-13	11-11-13

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS					
COMBINED		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	303	211.0	0.0	0.0	0.0	93.0	0.0
B	313	219.0	0.0	0.0	0.0	94.0	0.0
E	246	164.0	0.0	0.0	0.0	82.0	0.0
H	153	85.0	0.0	0.0	0.0	68.0	0.0
G							

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED (PLF)		MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
			VERT. LOAD	LC1 MAX				
A-B	0 / 15	-91.8	-91.8	0.03 (1)	10.00	H-C	-171 / 0	0.04 (1)
B-J	-58 / 32	-91.8	-91.8	0.15 (1)	8.25	H-D	-55 / 0	0.02 (1)
J-C	-196 / 0	-91.8	-91.8	0.32 (1)	6.25	G-D	-56 / 0	0.02 (1)
C-D	-148 / 0	-91.8	-91.8	0.03 (1)	6.25	I-J	-59 / 0	0.00 (1)
D-L	-221 / 0	-91.8	-91.8	0.32 (1)	6.25	K-L	-58 / 0	0.00 (1)
L-E	-62 / 5	-91.8	-91.8	0.15 (1)	6.25			
E-F	0 / 15	-91.8	-91.8	0.03 (1)	10.00			
B-I	0 / 153	-18.5	-18.5	0.27 (1)	10.00			
I-H	0 / 153	-18.5	-18.5	0.27 (1)	10.00			
H-G	0 / 171	-18.5	-18.5	0.19 (1)	10.00			
G-K	0 / 174	-18.5	-18.5	0.27 (1)	10.00			
K-E	0 / 174	-18.5	-18.5	0.27 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPI 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (D-L:1), BC=0.27/1.00 (G-K:1), WB=0.04/1.00 (C-H:1), SS=0.46/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

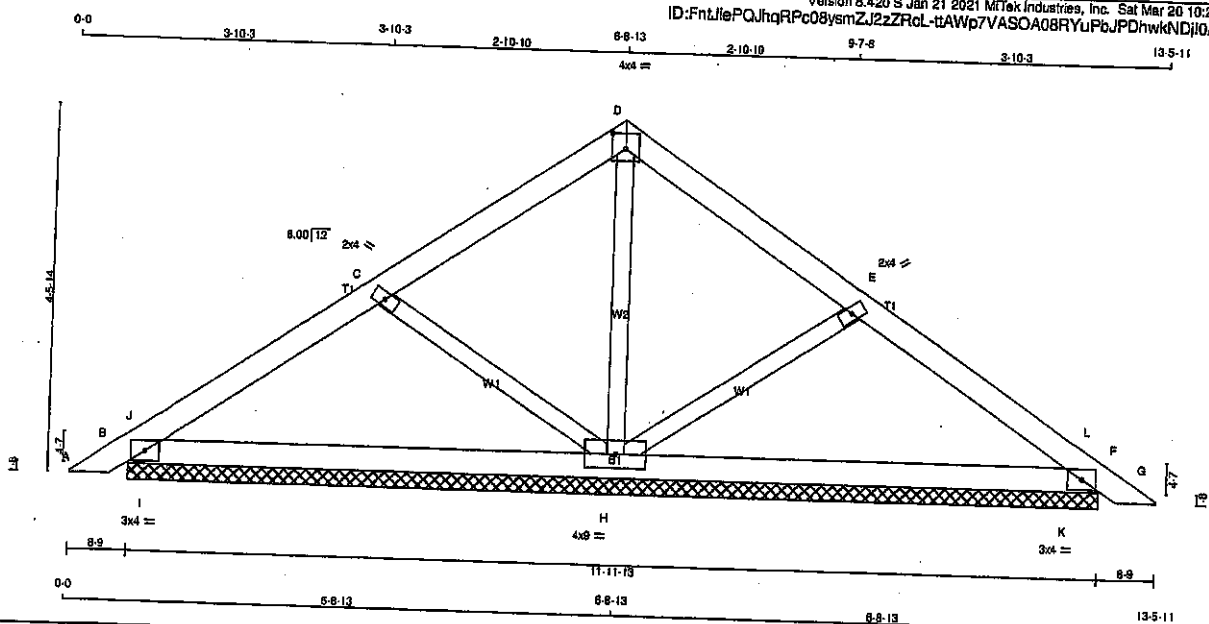
JSI GRIP= 0.37 (E) (INPUT = 0.90)
JSI METAL= 0.10 (E) (INPUT = 1.00)



Structural component only
DWG# T-2108222

JOB NAME 417467	TRUSS NAME PB4	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:25.0

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW-w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMW-w	MT20	2.0	4.0		
F	TMB1-J	MT20	3.0	4.0		
H	BMWW1-I	MT20	4.0	9.0		

NOTES: (1)
1) Lateral bracing to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	326	0	326	0	11-11-13	11-11-13
F	326	0	326	0	11-11-13	11-11-13
H	786	0	786	0	11-11-13	11-11-13

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX. MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	SNOW	LIVE	SNOW	LIVE				
B	230	156	0	0	0	0	73	0
F	230	156	0	0	0	0	73	0
H	556	366	0	0	0	0	190	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/15	-91.8 -91.8	0.03 (1)	10.00	H-D	-303/0	0.08 (1)
B-J	-288/0	-91.8 -91.8	0.08 (4)	6.25	H-E	-306/0	0.07 (1)
J-C	-187/0	-91.8 -91.8	0.14 (1)	6.25	C-H	-306/0	0.07 (1)
C-D	0/70	-91.8 -91.8	0.15 (1)	10.00	I-J	0/122	0.00 (1)
D-E	0/70	-91.8 -91.8	0.15 (1)	10.00	K-L	0/122	0.00 (1)
E-L	-187/0	-91.8 -91.8	0.14 (1)	6.25			
L-F	-288/0	-91.8 -91.8	0.08 (4)	6.25			
F-G	0/15	-91.8 -91.8	0.03 (1)	10.00			
B-I	0/177	-18.5 -18.5	0.06 (4)	10.00			
I-H	0/177	-18.5 -18.5	0.17 (4)	10.00			
H-K	0/177	-18.5 -18.5	0.17 (4)	10.00			
K-F	0/177	-18.5 -18.5	0.06 (4)	10.00			

TOTAL WEIGHT = 2 X 41 = 81 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL	= 6.0	PSF
BOT CH.	LL = 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (D-E:1), BC=0.17/1.00 (H-I:4), WB=0.08/1.00 (D-H:1), SS=0.11/1.00 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	850 371	1747 788
	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.24 (D) (INPUT = 0.90)
JSI METAL = 0.10 (C) (INPUT = 1.00)



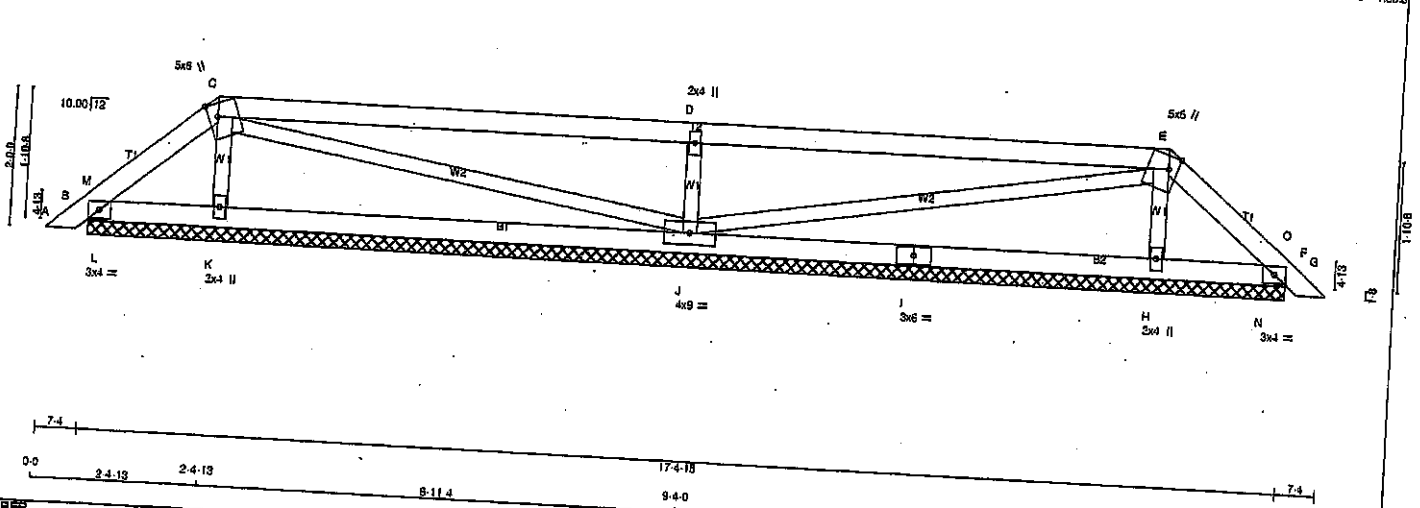
Structural component only
DWG# T-2108223

JOB NAME 417471	TRUSS NAME PB21	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:V2rMzHRYm36MOeXZ46o4AZZQVV-TFN5VYF7bK0ddQMBKfLozr5m4hq7p7sCBExYp_zZ4a

Scale = 1:20.3



LUMBER			
N, L, G, A, RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY, SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TMW+w	MT20	2.0	4.0	
E	TTWW+m	MT20	5.0	6.0	2.25 1.50
F	TMB1-I	MT20	3.0	4.0	
H	BMW1+w	MT20	3.0	4.0	
I	BS-I	MT20	4.0	9.0	
J	BMWW1-I	MT20	4.0	9.0	
K	BMW1+w	MT20	2.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	VERT	HORZ	GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
B	155	0	155	0	17-4-15 (5-5-37-4-15)	17-4-15 (5-5-37-4-15)
K	385	0	385	0	17-4-15 (5-5-37-4-15)	17-4-15 (5-5-37-4-15)
J	935	0	935	0	17-4-15 (5-5-37-4-15)	17-4-15 (5-5-37-4-15)
H	385	0	385	0	17-4-15 (5-5-37-4-15)	17-4-15 (5-5-37-4-15)
F	155	0	155	0	17-4-15 (5-5-37-4-15)	17-4-15 (5-5-37-4-15)

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	105	95.0	0.0	0.0	0.0	10.0	0.0
K	278	153.0	0.0	0.0	0.0	124.0	0.0
J	658	447.0	0.0	0.0	0.0	211.0	0.0
H	278	153.0	0.0	0.0	0.0	124.0	0.0
F	105	95.0	0.0	0.0	0.0	10.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO								
A-B	0.14	-91.8	-91.8	0.02 (1)	10.00	K-O	-258.0	0.04 (1)
B-M	-13.30	-91.8	-91.8	0.03 (4)	6.25	C-J	-18.0	0.02 (1)
M-C	-66.0	-91.8	-91.8	0.03 (1)	6.25	J-D	-785.0	0.12 (1)
C-D	-9.0	-91.8	-91.8	0.75 (1)	10.00	J-E	-18.0	0.02 (1)
D-E	-9.0	-91.8	-91.8	0.75 (1)	10.00	H-E	-258.0	0.04 (1)
E-O	-66.0	-91.8	-91.8	0.03 (1)	6.25	L-M	-154.0	0.00 (1)
O-F	-13.30	-91.8	-91.8	0.03 (4)	6.25	N-O	-154.0	0.00 (1)
F-G	0.14	-91.8	-91.8	0.02 (1)	10.00			
B-L	0.44	-18.5	-18.5	0.05 (1)	10.00			
L-K	0.44	-18.5	-18.5	0.13 (4)	10.00			
K-J	0.27	-18.5	-18.5	0.19 (4)	10.00			
J-I	0.27	-18.5	-18.5	0.19 (4)	10.00			
I-H	0.27	-18.5	-18.5	0.19 (4)	10.00			
H-N	0.44	-18.5	-18.5	0.13 (4)	10.00			
N-F	0.44	-18.5	-18.5	0.05 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.75/1.00 (C-D:1), BC=0.19/1.00 (H-J:4), WB=0.12/1.00 (O-J:1), SS=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

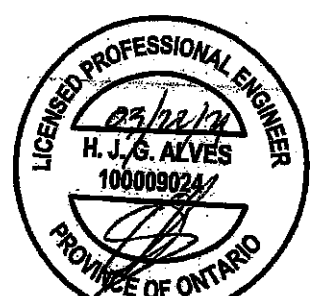
NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.58 (C) (INPUT = 0.90)

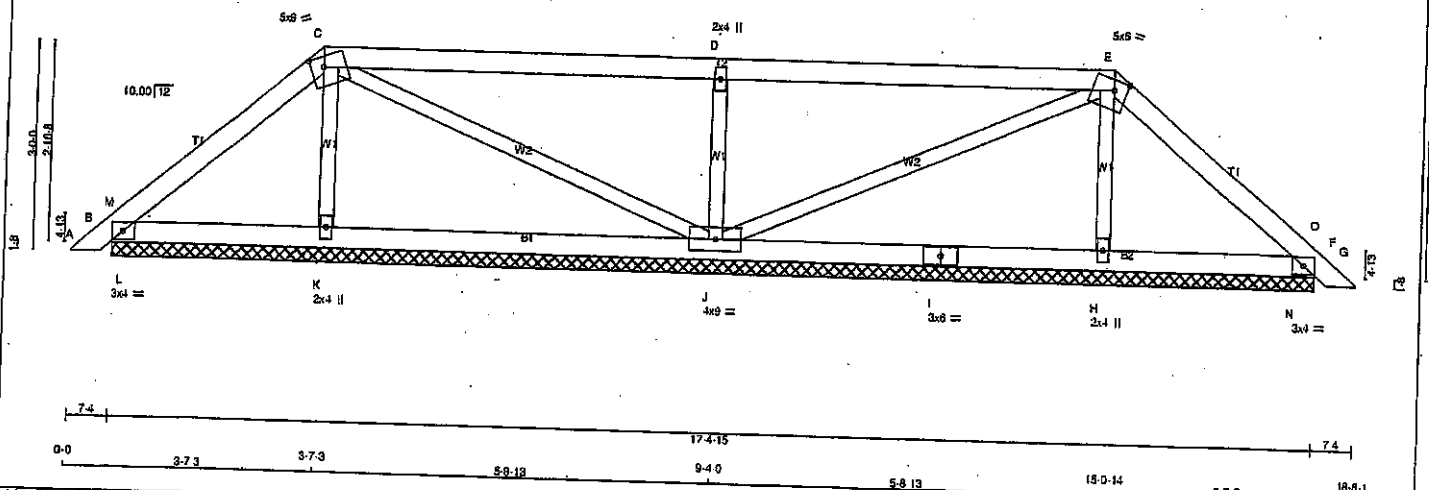
JSI METAL = 0.18 (I) (INPUT = 1.00)



Structural component only
DWG# T-2108249

JOB NAME 417471	TRUSS NAME PB22	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 5 Jan 21 2021 MITek Industries, Inc. Sat Mar 20 10:33:45 2021 Page 1	

ID:V2rMzHRYm36MOeXZ46o4AzZQVV-xSxTiuGIMe8UFaxNizs1W3e_Y5AGY88MQuG5LOzZ4ac
Scale = 1:30.2



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	
CHORDS			
A - C	2x4 DRY	No.2	
C - E	2x4 DRY	No.2	
E - G	2x4 DRY	No.2	
G - I	2x4 DRY	No.2	
I - F	2x4 DRY	No.2	
ALL WEBS	2x3 DRY	No.2	
DRY: SEASONED LUMBER.			

PLATES (tablets in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMB1-t	MT20	3.0	4.0		
C TTW-w	MT20	5.0	6.0	1.75	2.25
D TMB1-t	MT20	3.0	4.0		
E TTW-w	MT20	5.0	6.0	1.75	2.25
F TMB1-t	MT20	3.0	4.0		
G TTW-w	MT20	5.0	6.0	1.75	2.25
H TMB1-t	MT20	3.0	4.0		
I TTW-w	MT20	5.0	6.0	1.75	2.25
J TMB1-t	MT20	3.0	4.0		
K TTW-w	MT20	5.0	6.0	1.75	2.25

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	259	0	259	0	17.4-15 (5-5-37-4-15)	17.4-15 (5-5-37-4-15)
K	349	0	349	0	17.4-15 (5-5-37-4-15)	17.4-15 (5-5-37-4-15)
J	800	0	800	0	17.4-15 (5-5-37-4-15)	17.4-15 (5-5-37-4-15)
H	349	0	349	0	17.4-15 (5-5-37-4-15)	17.4-15 (5-5-37-4-15)
F	259	0	259	0	17.4-15 (5-5-37-4-15)	17.4-15 (5-5-37-4-15)

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
B	180	135 / 0	0 / 0	0 / 0	45 / 0	0 / 0	0 / 0
K	250	145 / 0	0 / 0	0 / 0	105 / 0	0 / 0	0 / 0
J	503	384 / 0	0 / 0	0 / 0	179 / 0	0 / 0	0 / 0
H	250	145 / 0	0 / 0	0 / 0	105 / 0	0 / 0	0 / 0
F	180	135 / 0	0 / 0	0 / 0	45 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 14	K-C	-230 / 0
B-M	-7 / 0	C-J	-48 / 0
M-C	-103 / 0	J-D	-657 / 0
C-D	-19 / 0	D-E	-48 / 0
D-E	-19 / 0	E-F	-230 / 0
E-F	-103 / 0	F-G	-266 / 0
F-G	-7 / 0		
B-L	0 / 73		
L-K	0 / 73		
K-J	0 / 83		
J-I	0 / 83		
I-H	0 / 63		
H-N	0 / 73		
N-F	0 / 73		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.51/1.00 (D-E:1), BC=0.18/1.00 (J-K:4), WB=0.11/1.00 (D-J:1), SS=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

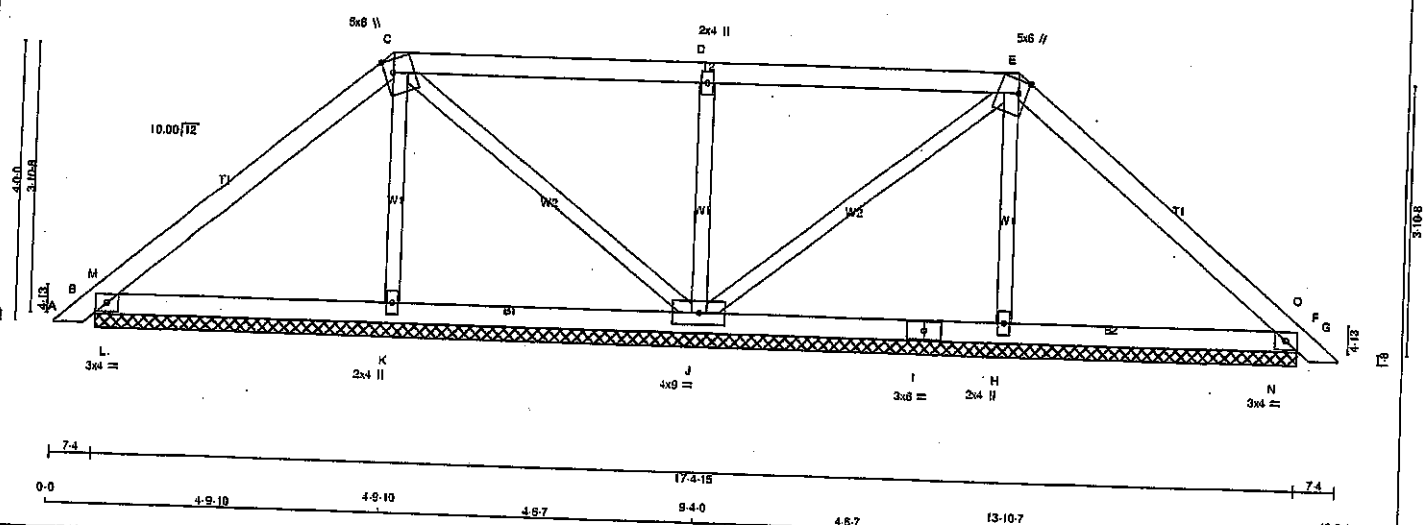
JSI GRIP = 0.74 (E) (INPUT = 0.90)
JSI METAL = 0.14 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108250

JOB NAME 417471	TRUSS NAME PB23	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 S Jan 21 2021 MTek Industries, Inc. Sat Mar 20 10:33:45 2021 Page 1	

ID:V2rMzHRYm36MOeXZ46o4AzZQVV-xSxTtuGIMe8UFaxNIZs1W3e2Z5AZYS7MCug5LQzZ4ac
Scale = 1:20.2



LUMBER
N. L. G. A. RULES
CHORDS SIZE
A - C 2x4 DRY No.2
C - E 2x4 DRY No.2
E - G 2x4 DRY No.2
B - I 2x4 DRY No.2
I - F 2x4 DRY No.2
ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DESCR.	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
B	358	0	358	0	17-4-15 (5-5-37-4-15)
K	311	0	311	0	17-4-15 (5-5-37-4-15)
J	676	0	676	0	17-4-15 (5-5-37-4-15)
H	311	0	311	0	17-4-15 (5-5-37-4-15)
F	358	0	358	0	17-4-15 (5-5-37-4-15)

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMB1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BS-I	MT20	3.0	6.0		
J	BMWW1-I	MT20	4.0	9.0		
K	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS	FACTORED	WEBS	FACTORED
FR-TO	MAX. FORCE (LBS)	VERT. LOAD (LBS)	MAX. FORCE (LBS)	MAX. FORCE (LBS)
A-B	0 / 14	-91.8	-91.8	0.02 (1)
B-M	-32 / 52	-91.8	-91.8	0.11 (1)
M-C	-157 / 0	-91.8	-91.8	0.20 (1)
C-D	-41 / 0	-91.8	-91.8	0.32 (1)
D-E	-41 / 0	-91.8	-91.8	0.20 (1)
E-O	-157 / 0	-91.8	-91.8	0.11 (1)
O-F	-32 / 52	-91.8	-91.8	0.02 (1)
F-G	0 / 14	-91.8	-91.8	0.02 (1)

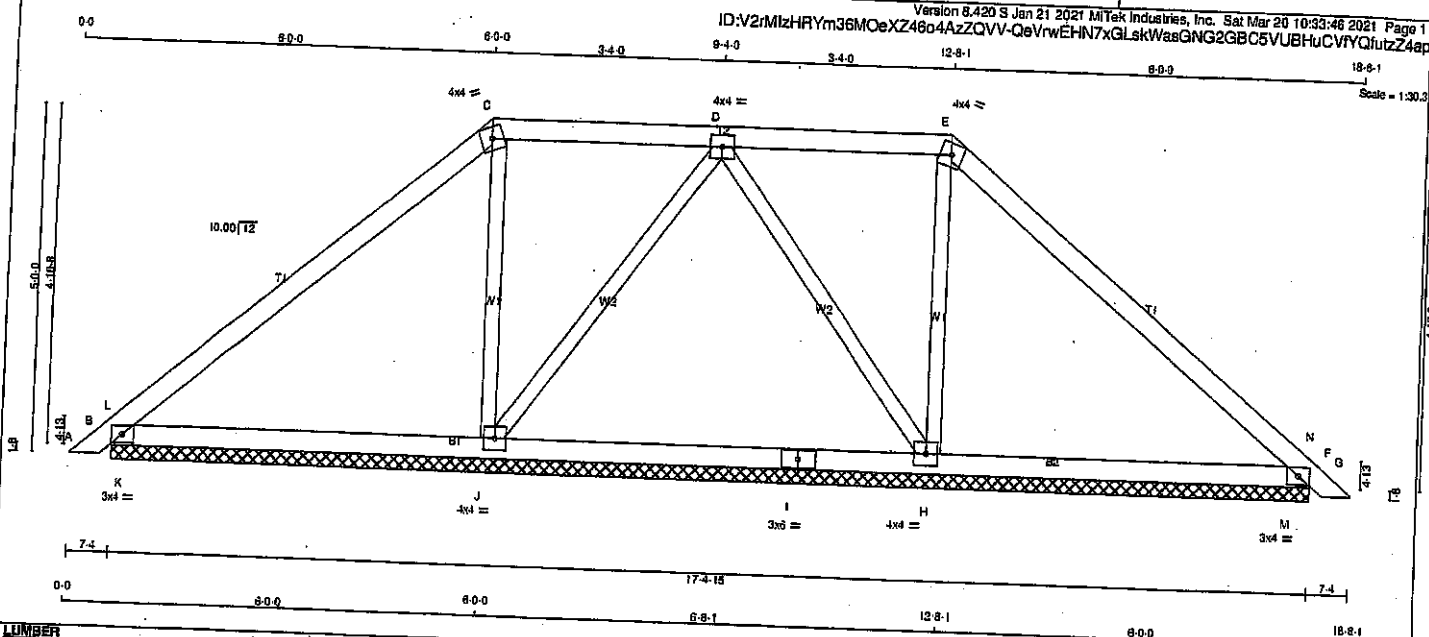
MEMB.	CHORDS	FACTORED	WEBS	FACTORED
FR-TO	MAX. FORCE (LBS)	VERT. LOAD (LBS)	MAX. FORCE (LBS)	MAX. FORCE (LBS)
B-L	0 / 112	-18.5	-18.5	0.17 (1)
L-K	0 / 112	-18.5	-18.5	0.17 (1)
K-J	0 / 106	-18.5	-18.5	0.11 (1)
J-I	0 / 106	-18.5	-18.5	0.11 (1)
I-H	0 / 106	-18.5	-18.5	0.11 (1)
H-N	0 / 112	-18.5	-18.5	0.17 (1)
N-F	0 / 112	-18.5	-18.5	0.17 (1)

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF
SPACING = 24.0 IN/C
LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015
THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014
(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD
CSI: TO=0.32/1.00 (D-E:1), BC=0.17/1.00 (H-N:1), WB=0.12/1.00 (D-J:1), SS=0.37/1.00 (B-L:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP = 0.56 (C) (INPUT = 0.90)
JSI METAL = 0.11 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108251

JOB NAME 417471	TRUSS NAME PB24	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 S Jan 21 2021 M/Tek Industries, Inc. Sat Mar 20 10:33:46 2021 Page 1	



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-1	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMWW-1	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMB1-1	MT20	3.0	4.0		
H	BMWW1-1	MT20	4.0	4.0		
I	BS-1	MT20	3.0	6.0		
J	BMWW1-1	MT20	4.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
JT	VERT	HORZ	UPLIFT	BRG
B	482	0	482	0
J	546	0	546	0
H	546	0	546	0
F	482	0	482	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	324	230 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0
J	388	242 / 0	0 / 0	0 / 0	0 / 0	146 / 0	0 / 0
H	388	242 / 0	0 / 0	0 / 0	0 / 0	146 / 0	0 / 0
F	324	230 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, J, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 14	-91.8	-91.8	0.02 (1)	10.00	J-C	-194 / 0	0.07 (1)
B-L	-35 / 157	-91.8	-91.8	0.20 (1)	6.25	H-E	-194 / 0	0.07 (1)
L-C	-223 / 0	-91.8	-91.8	0.33 (1)	6.25	J-D	-232 / 0	0.12 (1)
C-D	-154 / 0	-91.8	-91.8	0.17 (1)	6.25	D-H	-232 / 0	0.12 (1)
D-E	-154 / 0	-91.8	-91.8	0.17 (1)	6.25	K-L	-761 / 0	0.00 (1)
E-N	-223 / 0	-91.8	-91.8	0.33 (1)	6.25	M-N	-761 / 0	0.00 (1)
N-F	-35 / 157	-91.8	-91.8	0.20 (1)	6.25			
F-G	0 / 14	-91.8	-91.8	0.02 (1)	10.00			
B-K	0 / 159	-18.5	-18.5	0.28 (1)	10.00			
K-J	0 / 159	-18.5	-18.5	0.28 (1)	10.00			
J-I	0 / 288	-18.5	-18.5	0.18 (1)	10.00			
I-H	0 / 288	-18.5	-18.5	0.18 (1)	10.00			
H-M	0 / 159	-18.5	-18.5	0.28 (1)	10.00			
M-F	0 / 159	-18.5	-18.5	0.28 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, AEC 2015
- PART 9 OF CBC 2012 (2015 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1 TC=0.33/1.00 (E-N:1), BC=0.28/1.00 (H-M:1), WB=0.12/1.00 (D-H:1), SS=0.59/1.00 (B-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

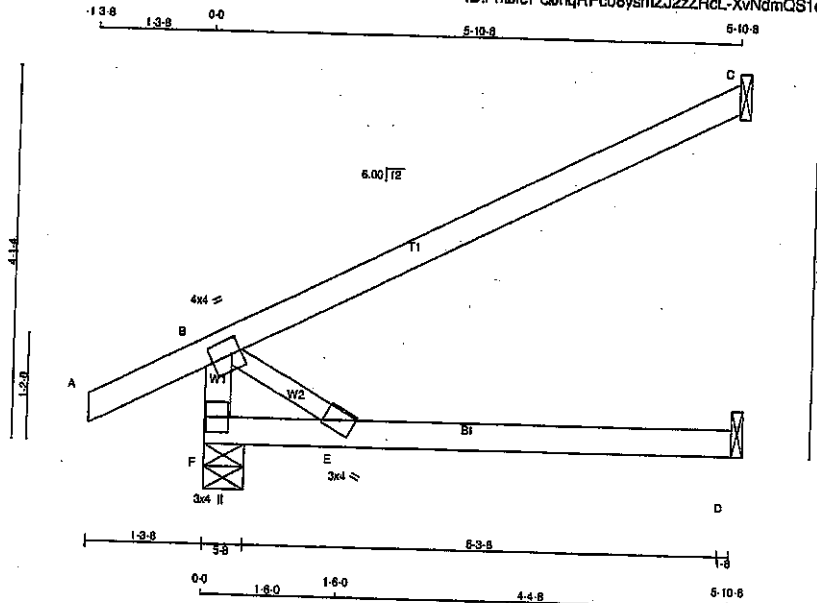
JSI GRIP= 0.62 (C) (INPUT = 0.90)
JSI METAL= 0.13 (I) (INPUT = 1.00)



Structural component only
DWG# T-2108252

JOB NAME 417467	TRUSS NAME J1	QUANTITY 23	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1/23.3

LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER.			

PLATES (tablets in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	4.0	4.0	2.00	1.25
E BMW-w	MT20	3.0	4.0		
F BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION	BRG	GROSS REACTION	BRG	BRG	IN-SX	BRG	IN-SX
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	5-8	5-8
F	448	0	448	0	0	1-8	1-8
C	270	0	270	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	315	220.0	0.0	0.0	0.0	95.0	0.0
C	185	150.0	0.0	0.0	0.0	35.0	0.0
D	43	0.0	0.0	0.0	0.0	43.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
F-B	-394.0	0.0	0.0 0.04 (1)	B-E	0.0	0.00 (1)	
A-B	0.28	-91.8	-91.8 0.12 (1)				
B-C	0.0	-91.8	-91.8 0.54 (1)				
F-E	0.0	-18.5	-18.5 0.14 (4)				
E-D	0.0	-18.5	-18.5 0.19 (4)				

TOTAL WEIGHT = 23 X 18 = 416 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.05")

CSI: TO=0.54/1.00 (B-C:1), BC=0.19/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.23 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)

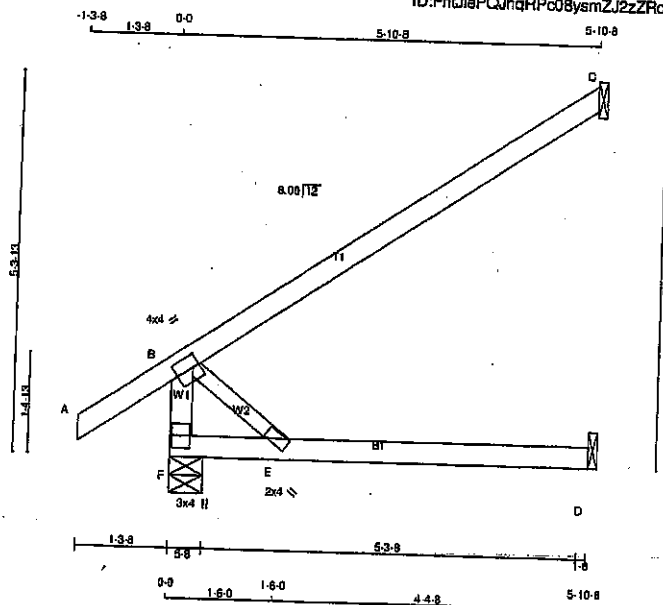


Structural component only
DWG# T-2108217

JOB NAME 417467	TRUSS NAME J2	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID: FntJlaPQJhQRPc08ysmZJ2zZRcl-76x7ziSIO8gbpE7AIFT3rmzGbl24N3I2DPWFz4IK

Scale = 1/2" = 1'-0"



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	
CHORDS			
F - B	2x4 DRY	No.2	
A - C	2x4 DRY	No.2	
F - D	2x4 DRY	No.2	
ALL WEBS	2x3 DRY	No.2	
DRY: SEASONED LUMBER.			

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-1	MT20	4.0	4.0	2.00 1.00
E	BMV-w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD " BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	450	0	450	0	5-8	5-8
C	270	0	270	0	1-8	1-8
D	54	0	61	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS					DEAD	SOIL
		SNOW	LIVE	PERM.LIVE	WIND			
F	318	221 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0	0 / 0
C	188	190 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PL)				MAX. UNBRACED LENGTH FR-TO	WEBS MEMB. MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
		FROM	TO	CS1 (LC)	MAX.			
F-B	-395 / 0	0.0	0.0	0.04 (1)	7.81	8-E	0 / 0	0.00 (1)
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00			
B-C	0 / 0	-91.8	-91.8	0.54 (1)	10.00			
F-E	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
E-D	0 / 0	-18.5	-18.5	0.19 (4)	10.00			

TOTAL WEIGHT = 4 X 19 = 77 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.54/1.00 (B-C:1), BC=0.19/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.24 (B) (INPUT = 0.50)
JSI METAL = 0.07 (B) (INPUT = 1.00)

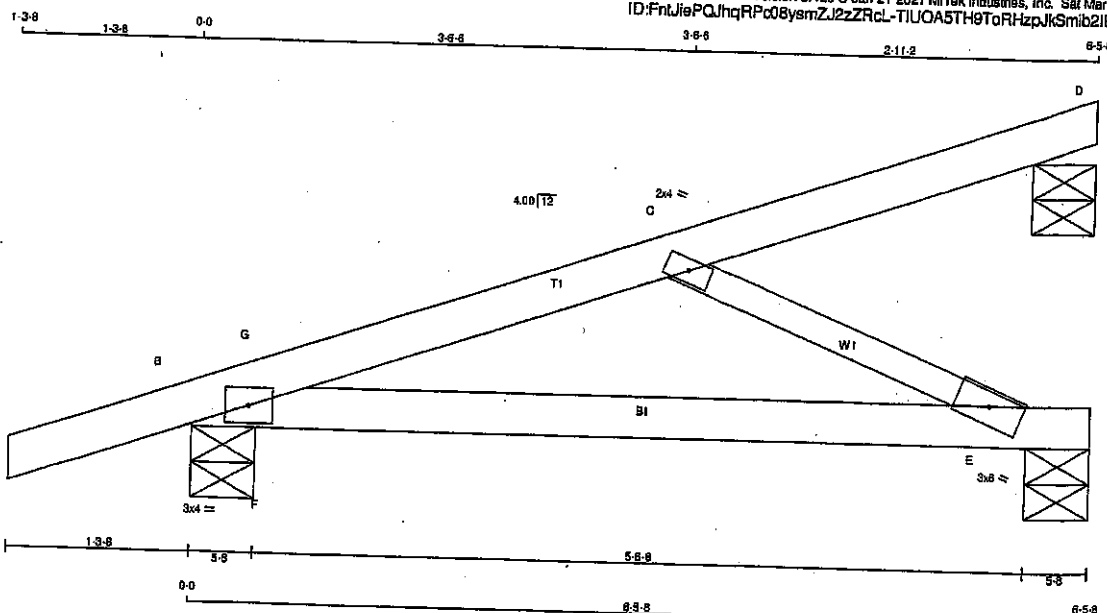


Structural component only
DWG# T-2108218

JOB NAME 417467	TRUSS NAME J3	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:15.0



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES	A - D	2x4	DRY	No.2
	B - E	2x4	DRY	No.2
ALL WEBS	2x8	DRY	No.2	SPF
DRY; SEASONED LUMBER.				

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMB1-4	MT20	3.0	4.0	
C TMW1+W	MT20	2.0	4.0	
E BMW1+W	MT20	3.0	6.0	

NOTES: (1)
1) Lateral bracing to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ UPLIFT		
D	99	0	99	0	5-8	5-8
B	447	0	447	0	5-8	5-8
E	275	0	275	0	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	88	58 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
B	313	220 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0
E	196	121 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD		MAX. CSI (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB. FORCE (LBS)	MAX. FACTORED CSI (LC)
		FROM	TO				
FR-TO							
A-B	0 / 18	-91.8	-91.8	0.11 (1)	10.00	C-E	-481 / 0
B-G	-493 / 0	-91.8	-91.8	0.09 (4)	6.25	F-G	0 / 104
G-C	-429 / 0	-91.8	-91.8	0.12 (1)	6.25		
C-D	-12 / 0	-91.8	-91.8	0.12 (1)	6.25		
B-F	0 / 425	-18.5	-18.5	0.09 (1)	10.00		
F-E	0 / 425	-18.5	-18.5	0.16 (1)	10.00		

TOTAL WEIGHT = 5 X 19 = 95 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 089-14
- TPIC 2014

(55% OF 81.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.04")

CSI: TC=0.12/1.00 (C-G:1), BC=0.18/1.00 (E-F:1), WB=0.09/1.00 (C-E:1), SS=0.14/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

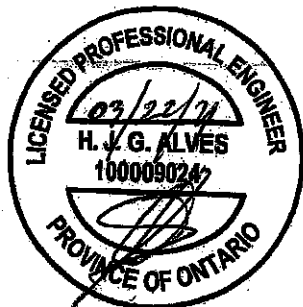
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PS) (PL) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (C) (INPUT = 0.90)
JSI METAL= 0.24 (C) (INPUT = 1.00)



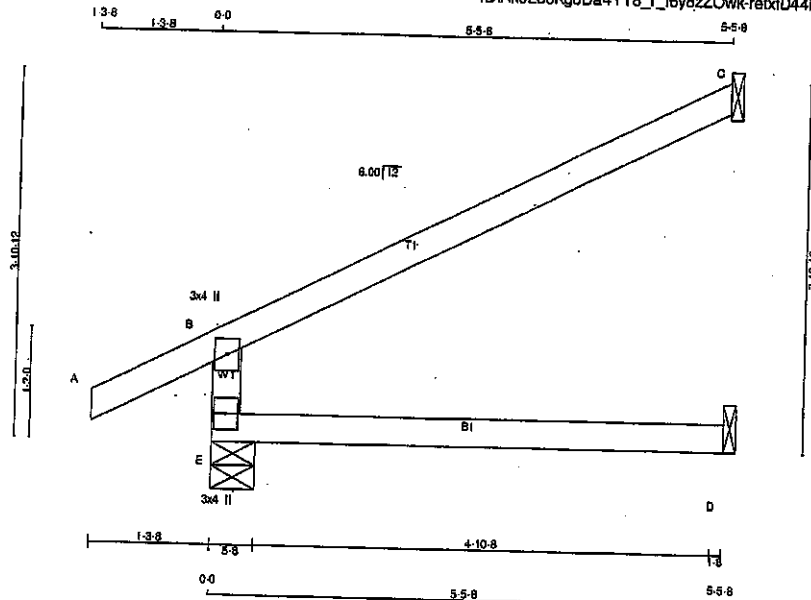
Structural component only
DWG# T-2108219

JOB NAME 417473	TRUSS NAME J41	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamareck Roof Truss, Burlington

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ID: Ak0Lo6KgoDa4YT8_r_l6y8zZOWk-retxID44lvbdr0THXIGDAToZotavRCJ0wvF708zZ4Dq

Scale = 1/2" = 1'-0"



LUMBER

N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	497	0	497	0	5-8	5-8
C	188	0	188	0	1-8	1-8
D	42	0	47	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT E C D	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	348	244 0	0 0	0 0	0 0	104 0	0 0
	129	105 0	0 0	0 0	0 0	25 0	0 0
	34	0 0	0 0	0 0	0 0	34 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. MAX. FORCE (LBS)	MAX. FACTORED MAX CSI (LC)
FR-TO		FROM TO			FR-TO	
E-B	-437.0	0.0	0.0	0.11 (4)		
A-B	0.28	-91.8	-91.8	0.12 (1)	10.00	7.81
B-C	-28.0	-91.8	-91.8	0.47 (1)	6.25	
E-D	0.0	-18.5	-18.5	0.11 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:		
TOP CH.	LL	= 25.6 PSF
	DL	= 6.0 PSF
BOT CH.	LL	= 0.0 PSF
	DL	= 7.4 PSF
TOTAL LOAD		= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.18")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.47/1.00 (B-C:1), BC=0.11/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

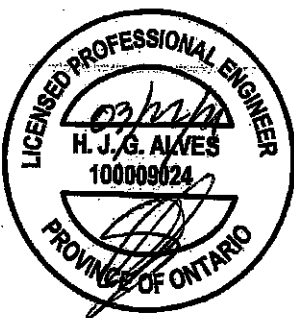
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
		1873	

PLATE PLACEMENT TOL. = 0.250 inches

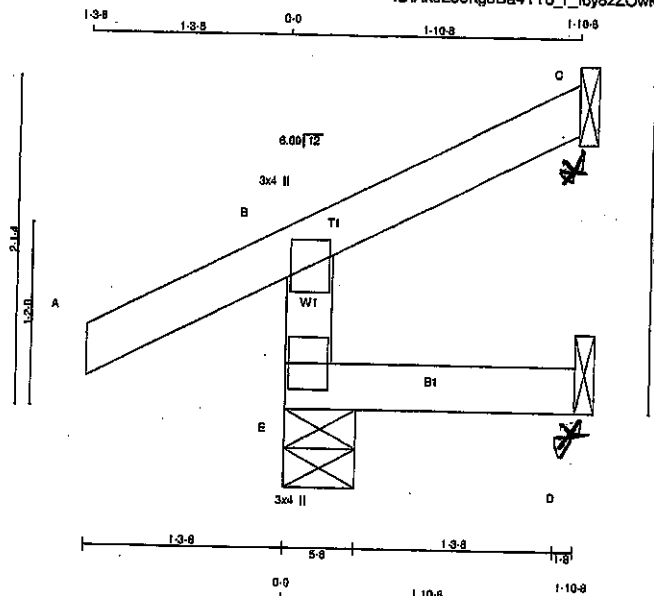
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.17 (E) (INPUT = 0.90)
JSI METAL = 0.12 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108280

JOB NAME 417473	TRUSS NAME J42	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 9 Jan 21 2021 MITak Industries, Inc. Sat Mar 20 10:58:18 2021 Page 1 ID: Ak0Lo6KgoDa4YT8_r_l6y8zZowk-JqRKs25itDjUTy2U5QnSih8qyaxIAFYA8Z?YYYzZ4Dp			
		Scale = 1:12.5			



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES

- (1) 1) Lateral braces to be a minimum of 2x4 SPF #2.

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	273	0	273	0	5-8	5-8
C	50	0	50	0	1-8	1-8
D	8	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LOASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	190	143 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	35	27 / -17	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	8	0 / -8	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	
E-B	-247 / 0	0.0	0.0
A-B	0 / 28	-91.8	-91.8
B-C	-18 / 0	-91.8	-91.8
E-D	0 / 0	-18.5	-18.5

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 7 = 14 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2015
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5), WB=0.00/1.00 (N/A:0), SSI=0.09/1.00 (A-B:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (E) (INPUT = 1.00)

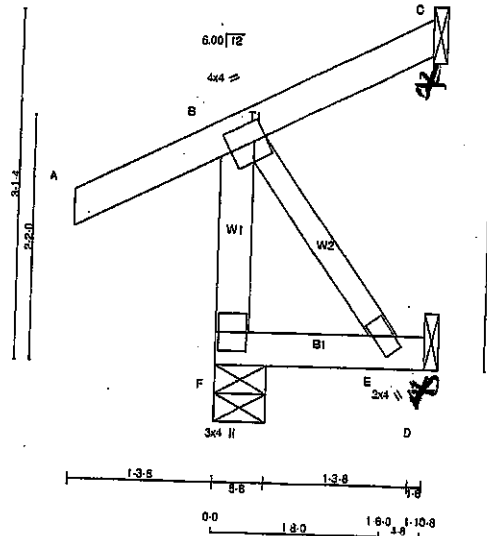


Structural component only
DWG# T-2108281

JOB NAME 417473	TRUSS NAME J43	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 10:58:19 2021 Page 1
ID: Ak0Lo6KqoDa4YT8_rJ8y8zZOWk-n07i3u6KDWL48cge7fhFuh7IGHpv8oJMDk647ZZ4D0

1-3.8 1-3.8 0-0 1-10.8 1-10.8



Scale = 1:12.7

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
F - B	2x4	DRY	No.2			SPF
A - C	2x4	DRY	No.2			SPF
F - D	2x4	DRY	No.2			SPF
ALL WEBS	2x3	DRY	No.2			SPF
DRY: SEASONED LUMBER.						

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN - Y	X
B	TMVW-1	MT20	4.0	4.0	2.00 1.25
E	BMVW-w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	DOWN	HORIZ	UPLIFT
			IN-SX	IN-SX
JT	273	0	0	0
F	41	0	41	0
C	17	0	19	0
D	17	0	19	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	190	143 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
F	28	23 / 25	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
C	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
A-B	-258 / 0	0.0	0.0	0.03 (1)	7.81	0.0	0.00 (1)		
B-C	0 / 28	-91.8	-91.8	0.12 (1)	10.00				
F-E	-20 / 0	-91.8	-91.8	0.11 (1)	6.25				
F-E	0 / 0	-18.5	-18.5	0.02 (4)	10.00				
E-D	0 / 0	-18.5	-18.5	0.01 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 10 = 20 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA-085-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSD) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 758 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.15 (B) (INPUT = 0.90)
JSI METAL = 0.04 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108282

Tamarack Roof Truss, Burlington

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ID:FntWjePQJhqRPc08ysmJ2zZRcl-6KhV7OP9Lw99ACxMxvAXu?bOm_Ok8Z467bRUNUzZ4j0



TOTAL WEIGHT = 9 lb

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	4.0	4.0	2.00	1.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

BEARINGS

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBB			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS† (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS† (LC)
FR-TO		FROM TO			FR-TO		
F-B	-256 / 0	0.0	0.0	0.03 (1)	7.81		
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	0 / 0	0.00 (1)
B-C	-27 / 0	-91.8	-91.8	0.12 (1)	6.25		
F-E	0 / 0	-18.5	-18.5	0.02 (4)	10.00		
E-D	0 / 0	-18.5	-18.5	0.01 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP CH.	LL =	25.6	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 8 OF BCBC 2018 , ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (E-F:4),
WB=0.00/1.00 (B-E:1), SS=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PLI)	(PLI)	(PLI)	(PLI)
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. $\pm$ 0.250 inches

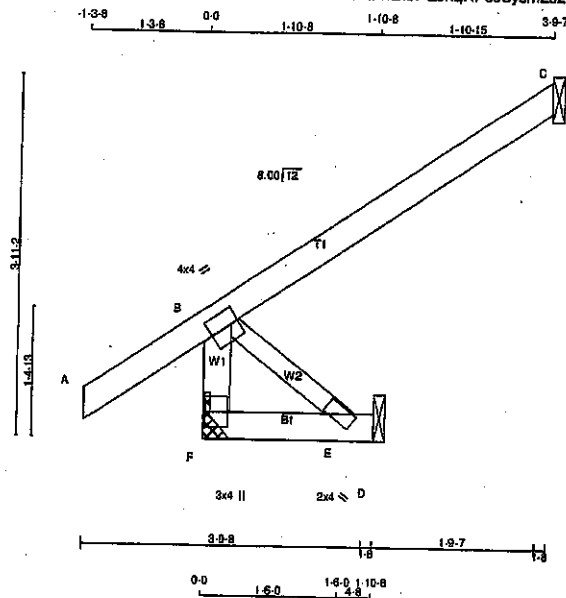
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108213

JOB NAME 417467	TRUSS NAME C2	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 10:25:42 2021 Page 1
ID: FntliePQJhqrPc08ysmZJ2zZRCf-bXFLkQn6EI0cMVYVdhmRC8XyOMzt0KGMFA2vwrZ4IN

Scale = 1/22.9

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	4.0	2.00 1.00
E	BMW-w	MT20	2.0	4.0	
F	BMV1-p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG. IN-SX		REQD BRG. IN-SX	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT			
F	317	0	317	0	0	0	MECHANICAL		
C	174	0	174	0	0	1-8		1-8	
D	17	0	19	0	0	1-8		1-8	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	220	187 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
C	120	97 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MAX. UNBRACED LENGTH (FT)		WEBS		MAX. FACTORED FORCE (LBS)		MAX. UNBRACED LENGTH (FT)	
MEMB.		FR-TO		FROM TO		FR-TO		MEMB.		FR-TO		FROM TO	
F-B	-300 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)					
A-B	0 / 35	-91.8	-91.8	0.13 (5)	10.00								
B-C	0 / 0	-91.8	-91.8	0.22 (1)	10.00								
F-E	0 / 0	-18.5	-18.5	0.02 (4)	10.00								
E-D	0 / 0	-18.5	-18.5	0.01 (4)	10.00								

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 12 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.00")

CSI: TC=0.22/1.00 (B-C:1), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

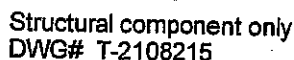
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.19 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



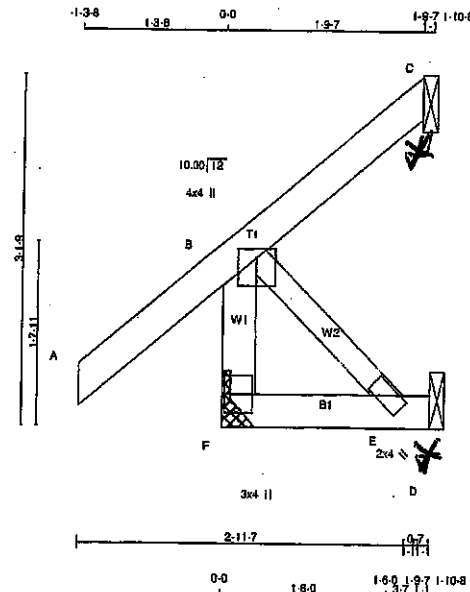
Structural component only
DWG# T-2108214

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 10:25:42 2021 Page 1
ID:FntJiePQJhqRPc08ysmJZj2ZRdL-bXFLkQn8EI0oMVVYdhmRC8ZUOKL0KGMFA2vwzZ4iN

ISI GRIP= 0.16 (B) (INPUT = 0.90)
ISI METAL= 0.05 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417471	C11	2	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 10:33:40 2021 Page 1
ID:V2rMizHRYm36MOeXZ48o4AzZQVV-bU7afACcX5WB8p2QV?Gsp?x?Fq4VotCfdGczKgDzZ4av



Scale = 1:18.6

TOTAL WEIGHT = 2 X 10 = 20 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	NO.2
F - B	2x4	DRY	No.2	
A - C	2x4	DRY	No.2	
F - D	2x4	DRY	No.2	
ALL WEBS	2x3	DRY	No.2	
DRY: SEASONED LUMBER				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	276	0	278	0
C	33	0	33	0
D	17	0	19	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	192	144 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0	0 / 0
C	23	18 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
FR-TO													
F-B		-258 / 0	0.0	0.0	0.03 (1)	7.81							
A-B		0 / 41	-91.8	-91.8	0.13 (1)	10.00							
B-C		-32 / 0	-91.8	-91.8	0.12 (1)	6.25							
F-E		0 / 0	-18.5	-18.5	0.02 (4)	10.00							
E-D		0 / 0	-18.5	-18.5	0.01 (4)	10.00							

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (E-F:1), WB=0.00/1.00 (B-E:1), SS=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX	MIN	MAX
MIN	MAX	MIN
MT20	850	371
	1747	768
	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



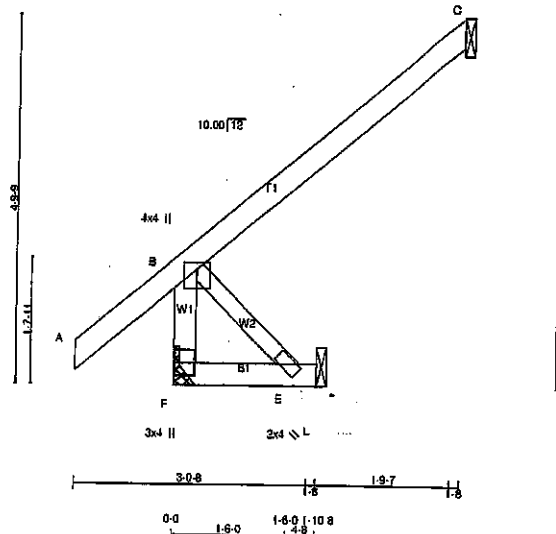
Structural component only
DWG# T-2108244

JOB NAME 417471	TRUSS NAME C12	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						

ID:V2nMzHRYm36MOeXZ48p4AZZQVV-bU7afACcX5WB3p2QV7Gsp7xDJ4VotCtdGczKgDzZ4av

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Scale = 1/27.4



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	318	0	318	0
C	174	0	174	0
D	17	0	19	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	221	168 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
C	120	97 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO					FR-TO		
F-B	-301 / 0	0.0	0.0	0.03 (1)	B-E	0 / 0	0.00 (1)
A-B	0 / 41	-91.8	-91.8	0.13 (5)			
B-C	0 / 0	-91.8	-91.8	0.22 (1)			
F-E	0 / 0	-18.5	-18.5	0.02 (4)			
E-D	0 / 0	-18.5	-18.5	0.01 (4)			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 13 = 26 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCG 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.22/1.00 (B-C:1), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

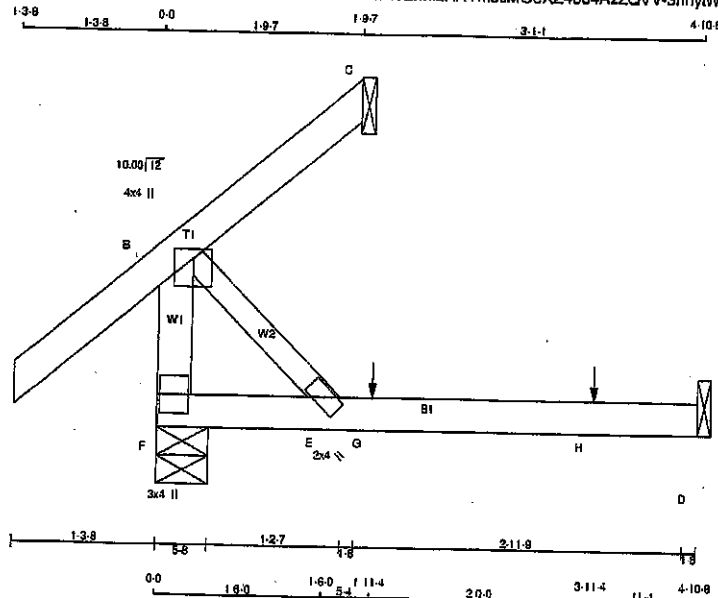
JSI GRIP = 0.22 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108245

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417471	C13	2	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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Scale = 1/16" = 1'-0"

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
E	BMV+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	JT	GROSS REACTION	BRG	BRG	BRG	BRG
F	VERT	F	VERT	DOWN	HORIZ	IN-SX	IN-SX
C	303	0	303	0	0	5-8	5-8
C	33	0	33	0	0	1-8	1-8
D	45	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	JT	COMBINED
F	214	144	0
C	23	18	0
D	36	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PL)	LC1 MAX	MEMB.	FORCE (LBS)	MAX	LC1 (L)
FR-TO		FROM	TO	FR-TO			
F-B	-258 / 0	0.0	0.0	0.03 (1)	7.81	0.0	0.00 (1)
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00		
B-C	-32 / 0	-91.8	-91.8	0.12 (1)	6.25		
F-E	0 / 0	-18.5	-18.5	0.11 (4)	10.00		
E-G	0 / 0	-18.5	-18.5	0.13 (4)	10.00		
G-H	0 / 0	-18.5	-18.5	0.13 (4)	10.00		
H-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11.4	1	1		FRONT	VERT	TOTAL		C1
H	3-11.4	1	1		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 13 = 27 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, ABC 2015
- PART 9 OF NBC 2012 (2015 AMENDMENT)
- CSA C88-14
- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/899$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/899$ (0.03")

CSI: TC=0.13/1.00 (A-B:1), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (B-E:1), SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES		PLATE GRIP(DRY)		SHEAR (PS)		SECTION (PL)	
MAX	MIN	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

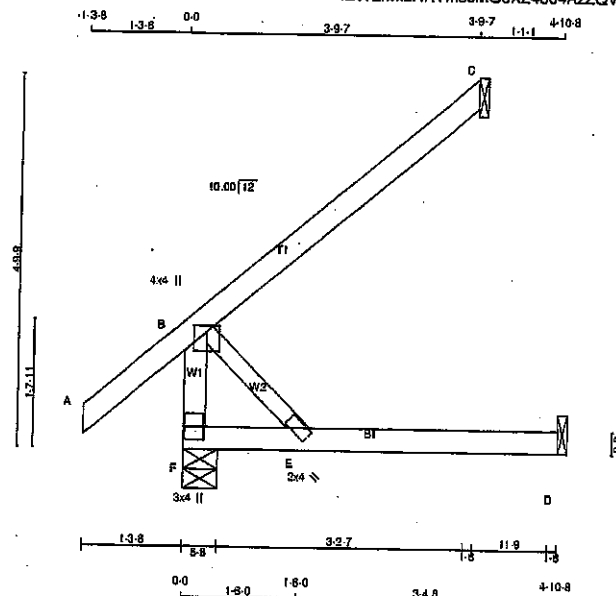
JSI GRIP = 0.19 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108246

JOB NAME 417471	TRUSS NAME C14	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1/2" = 1'

LUMBER			
N. L. & A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
F - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
F - D	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY, SEASONED LUMBER			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
E	BMVW+p	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	346	0	346	0	5-8	5-8
C	174	0	174	0	1-8	1-8
D	45	0	51	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
F	243	168	0	0	0	75	0
C	120	97	0	0	0	23	0
D	35	0	0	0	0	38	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FACTORED	FORCE	VERT. LOAD	LC1 MAX	MAX. MEMB.	FORCE	MAX. FACTORED	LC1 MAX
FR-TO		(LBS)	(PLF)	CSI (LC)	UNBRAC LENGTH	FR-TO	(LBS)	CSI (LC)
F-B	-301	0	0.0	0.0 0.03 (1)	7.81	B-E	0	0.00 (1)
A-B	0	41	-91.8	-91.8 0.13 (1)	10.00			
B-C	0	0	-91.8	-91.8 0.22 (1)	10.00			
F-E	0	0	-18.5	-18.5 0.11 (4)	10.00			
E-D	0	0	-18.5	-18.5 0.13 (4)	10.00			

TOTAL WEIGHT = 2 X 16 = 32 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.22/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

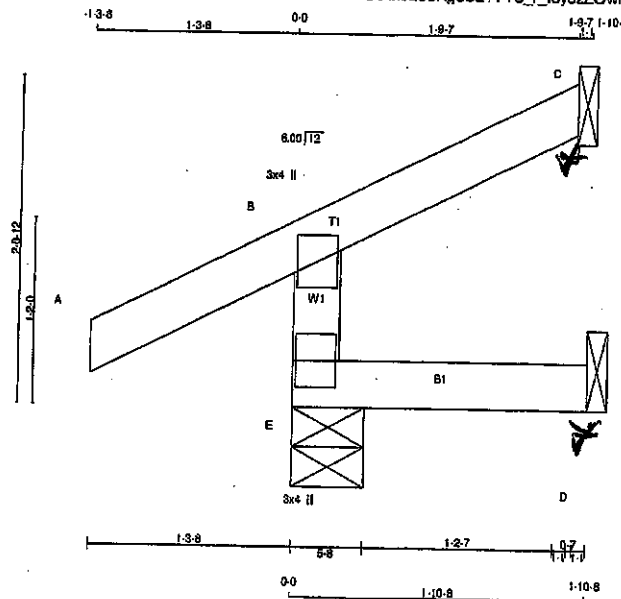


Structural component only
DWG# T-2108247

Tamarack Roof Truss, Burlington

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Scale = 1:13.3



TOTAL WEIGHT = 5 X 7 = 35 lb

Structural component only
DWG# T-2108273

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 10:58:12 2021 Page 1
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DRY: SEASONED LUMBER

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

TOTAL WEIGHT = $5 \times 10 = 48$ lb

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C. D.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

TOTAL LOAD CASES: (5)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

SPECIFIED LOADS:			
TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD =		39.0	PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIC ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.22/1.00 (B-C:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

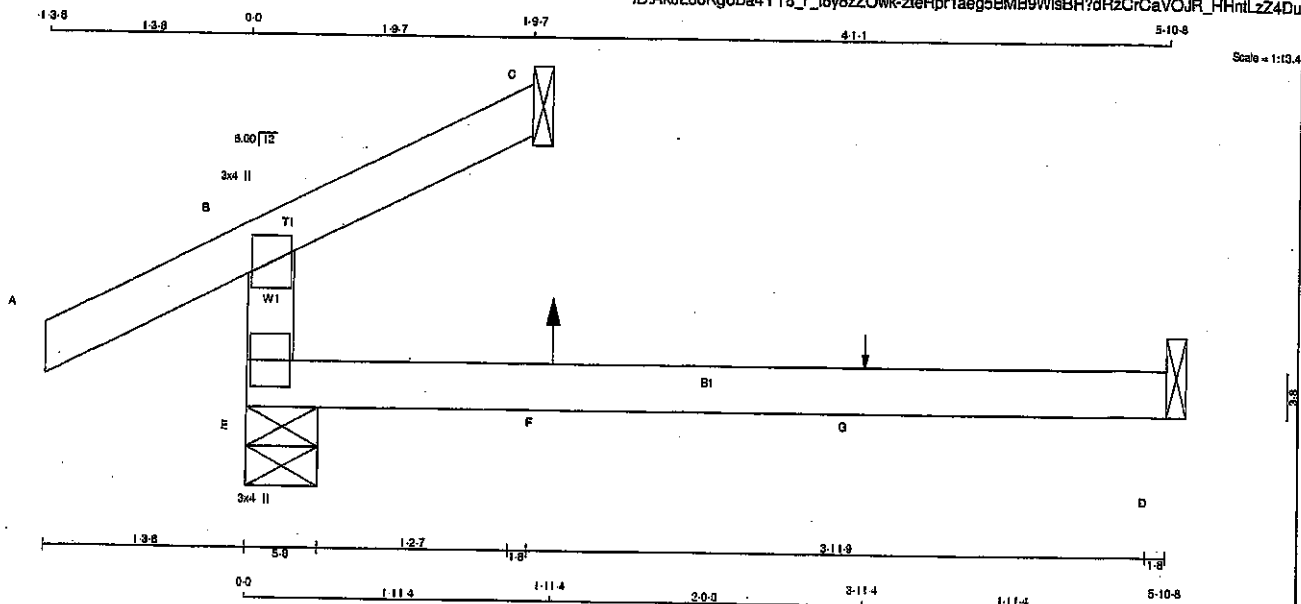
JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108274

JOB NAME 417473	TRUSS NAME C43	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 10:58:13 2021 Page 1
ID: A00Lo6KgoDa4YT8_r_15y8zZOWk-zleRpr1aeg5BMB9WIsBH?dRzCrCaVOJR_HHnLzZ4Du



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
E - B 2x4 DRY No.2
A - C 2x4 DRY No.2
E - D 2x4 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMV+p MT20 3.0 4.0
E BMV1+p MT20 3.0 4.0

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQ'D BRG
	VERT	HORZ	DOWN	HORZ		
E	284	0	284	0	5-8	5-8
C	63	0	63	0	1-8	1-8
D	44	0	52	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOOSE COMBINED		MAX./MIN. COMPONENT REACTIONS		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	VERT	HORZ	DOWN	HORZ						
E	200	137.0	0.0	0.0	0.0	0.0	0.0	62.0	0.0	0.0
C	48	21.0	0.0	0.0	0.0	0.0	0.0	25.0	0.0	0.0
D	35	0.3	0.0	0.0	0.0	0.0	0.0	37.0	0.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. MAX. FORCE (LBS)	MAX. CSI (LC)	MAX. FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO						
E-B	-227.0	0.0	0.0	0.11 (4)	7.81			
A-B	0.28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-9.9	-91.8	-91.8	0.08 (4)	10.00			
E-F	0.0	-18.5	-18.5	0.14 (4)	10.00			
F-G	0.0	-18.5	-18.5	0.14 (4)	10.00			
G-D	0.0	-18.5	-18.5	0.14 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11.4	5	1	8	FRONT	VERT	TOTAL	--	C1
G	3-11.4	1	1	--	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CS: TC=0.12/1.00 (A-B:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

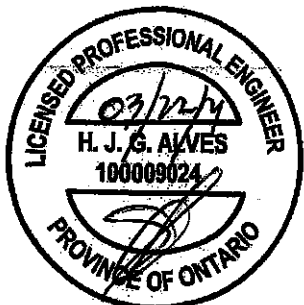
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1887 1873

PLATE PLACEMENT TOL. = 0.250 inches

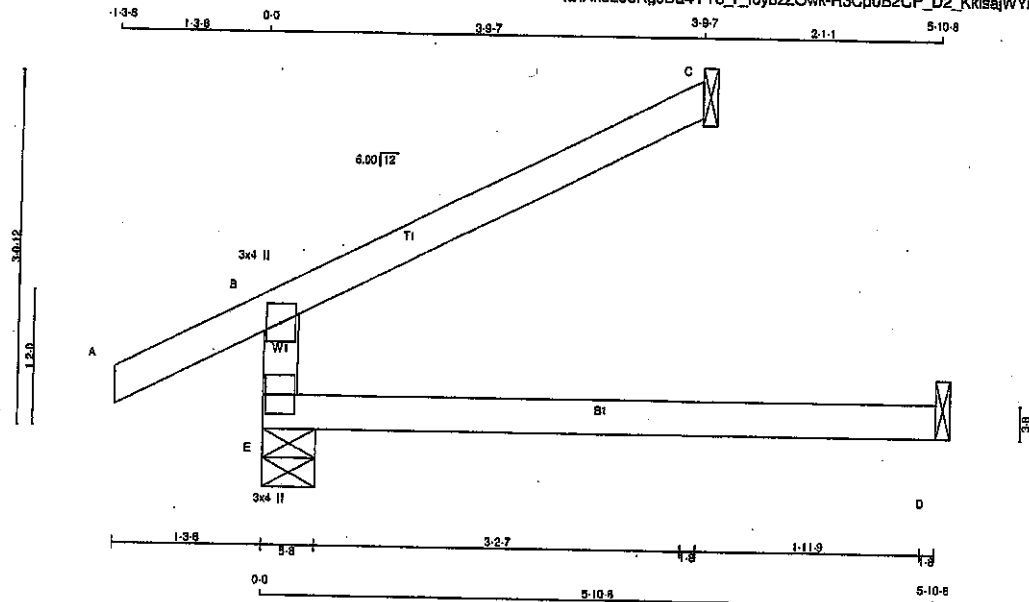
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108275

JOB NAME 417473	TRUSS NAME C44	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 5 Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 10:58:14 2021 Page 1 ID: Ak0L06KgoDa4YT8_r_l6y8ZOWk-R3Cp0B2CP_D2_KkisaWYr_6KFXvErZaDx1LPnzZ4D	



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
JT	405	0	405	0	5-8	5-8
E	130	0	130	0	1-8	1-8
C	45	0	50	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
JT	286	190.0	0.0	0.0	0.0	96.0
E	90	73.0	0.0	0.0	0.0	17.0
C	38	0.0	0.0	0.0	0.0	36.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
			FROM	TO					
E-B		-342.0	0.0	0.0	0.13 (4)	7.81			
A-B		0.28	-91.8	-91.8	0.12 (1)	10.00			
B-C		-19.0	-91.8	-91.8	0.22 (1)	6.25			
E-D		0.0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.22/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.14 (E) (INPUT = 0.90)
JSI METAL = 0.09 (B) (INPUT = 1.00)

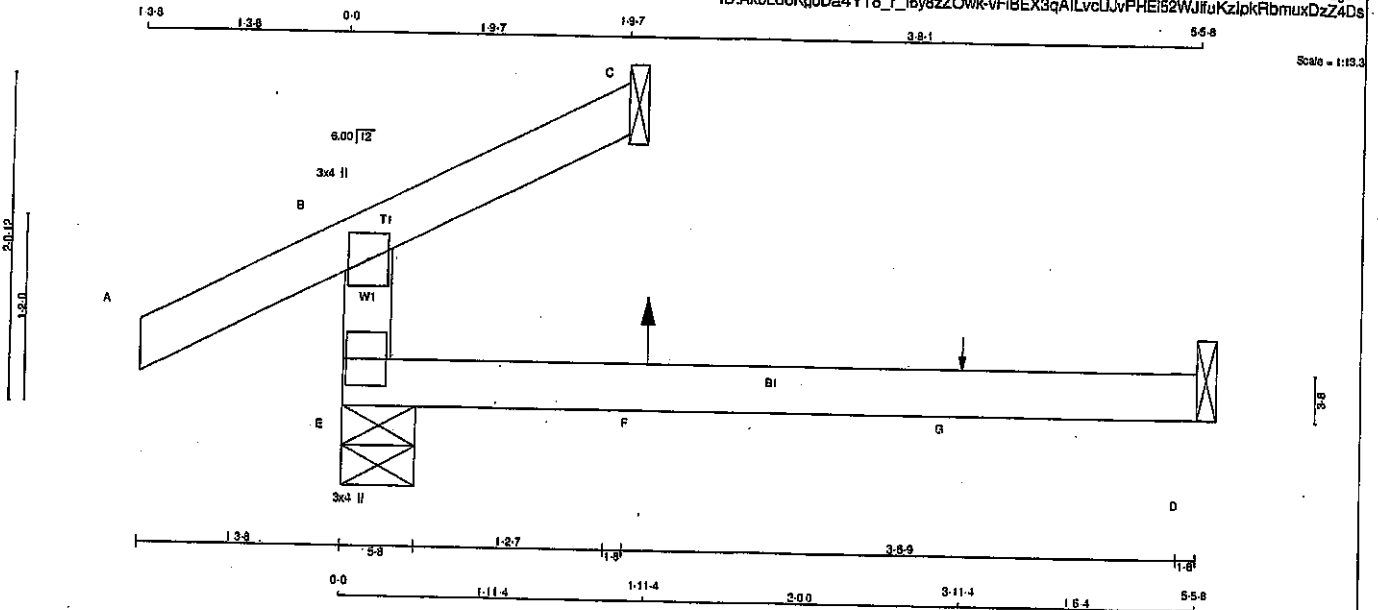


Structural component only
DWG# T-2108276

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417473	C45	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 10:58:15 2021 Page 1
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Scale = 1:12.5

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	283	0	283	0	5-8	5-8
C	59	0	59	0	1-8	1-8
D	40	0	49	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	199	137.0	0.0	0.0	0.0	62.0	0.0
C	43	21.0	0.0	0.0	0.0	22.0	0.0
D	33	0.3	0.0	0.0	0.0	35.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)
		FROM TO	CS1 (LC)		
E-B	-231.0	0.0	0.0	0.09 (4)	7.81
A-B	0.28	-91.8	-91.8	0.12 (1)	10.00
B-C	-11.7	-91.8	-91.8	0.06 (4)	6.25
E-F	0.0	-18.5	-18.5	0.12 (4)	10.00
F-G	0.0	-18.5	-18.5	0.12 (4)	10.00
G-D	0.0	-18.5	-18.5	0.12 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11.4	5	1	8	BACK	VERT	TOTAL	C1
G	3-11.4	1	1	--	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 11 = 22 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/699$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/699$ (0.03")

CS: TO=0.12/1.00 (A-B:1), BC=0.12/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(FBI)	(PLI)	(PLI)	
MT20	MAX MIN	MAX MIN	MAX MIN
	850	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

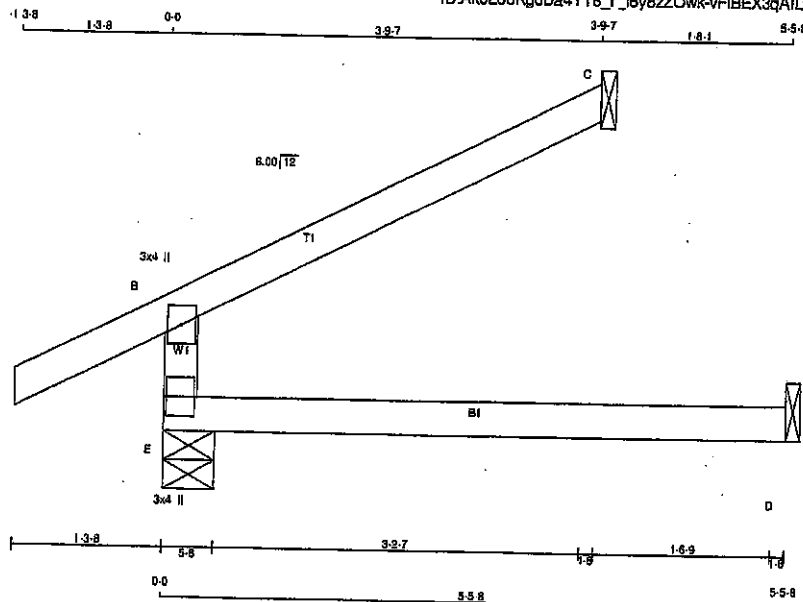
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.09 (E) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108277

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417473	C46	2	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER

N. L. G. A. RULES

CHORDS SIZE

E - B 2x4 DRY

A - C 2x4 DRY

E - D 2x4 DRY

LUMBER

No.2

No.2

No.2

DESCR.

SPF

SPF

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE PLATES W LEN Y X

B TMV+P MT20 3.0 4.0

E BMV1+P MT20 3.0 4.0

NOTES

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
E	401	0	401	0
C	130	0	130	0
D	42	0	47	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	232	190	0	0	0	0	0
C	90	73	0	0	0	17	0
D	34	0	0	0	0	34	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

	CHORDS	FACTORED	WEBS	FACTORED
	MEMB.	FORCE	VERT. LOAD	MEMB.
	(LBS)	(PLF)	CSI (LC)	FORCE
				(LBS)
FR-TO		FROM TO	UNBRACED LENGTH	FR-TO
E-B	-342.0	0.0	0.0	7.81
A-B	0.20	-91.8	-91.8	10.00
B-C	-19.0	-91.8	-91.8	6.25
E-D	0.0	-18.5	-18.5	10.00

TOTAL WEIGHT = 2 X 14 = 27 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2015

- PART 9 OF CBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")

CALCULATED VERT. DEFL.(LL) = L/999 (0.00")

ALLOWABLE DEFL.(TL) = L/360 (0.19")

CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.22/1.00 (B-C:1), BC=0.11/1.00 (D-E:4), WB=0.00/1.00 (W:0), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (FS) (PL) (PL)

MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JST GRIP = 0.14 (E) (INPUT = 0.80)

JST METAL = 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108278



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218

Feb 09, 2018

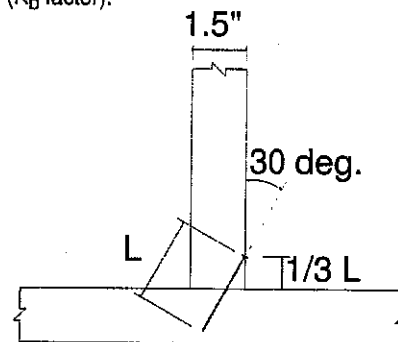
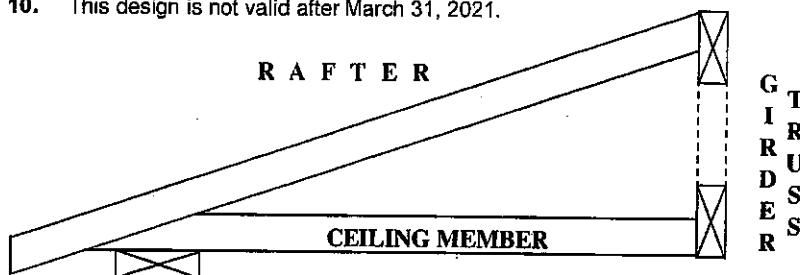
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.



TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

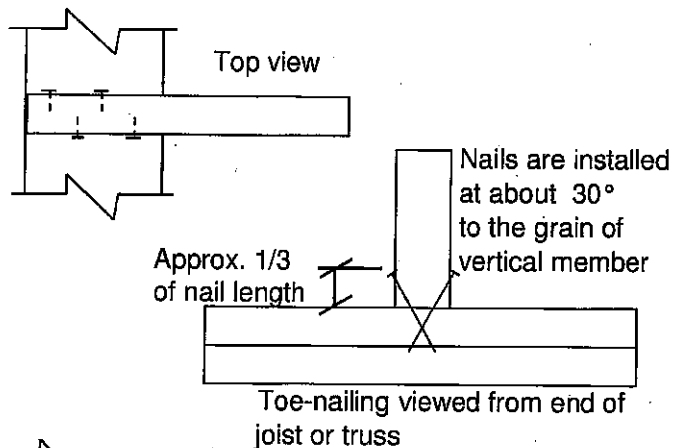
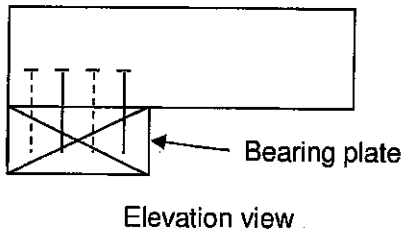
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

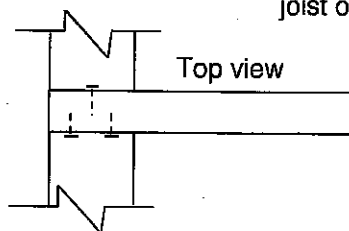
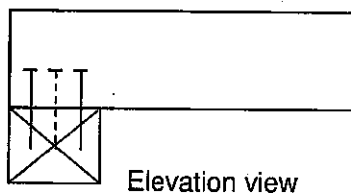
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



December 2, 2019

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

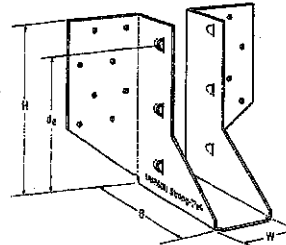
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

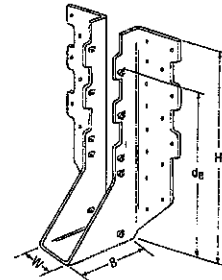
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

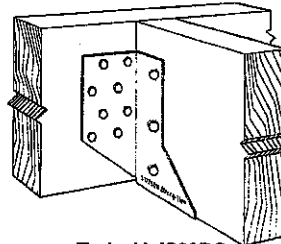
- See current catalogue for options



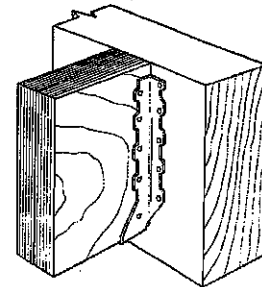
LJS26DS



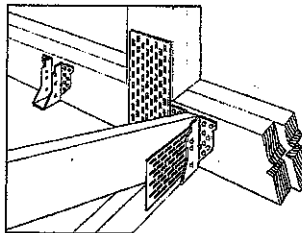
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



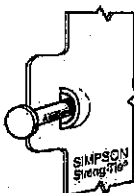
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

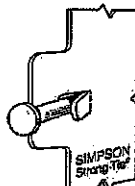
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LJS26DS	18	1⅞	5	3½	4⅞	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅞	5⅞	3	3⅞⅞	(14) 16d	(6) 16d	2705	4940	2085	3875
HUS28	16	1⅞	7⅞	3	6⅞	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅞	9⅞	3	7⅞⅞	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1⅞	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

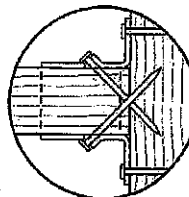


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

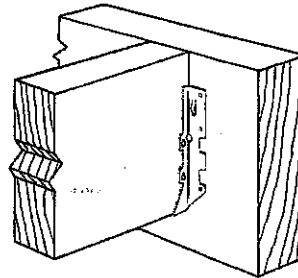
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

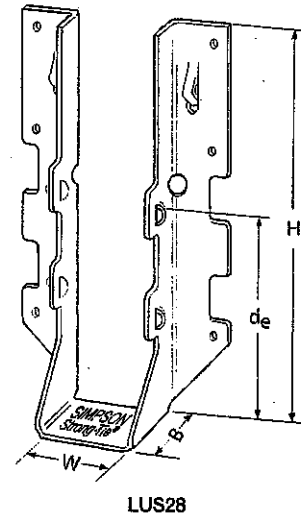
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



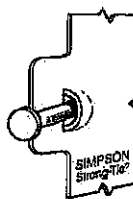
Typical LUS Installation



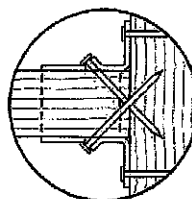
LUS28

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _p =1.15)	Normal (K _p =1.00)	Uplift (K _p =1.15)	Normal (K _p =1.00)
LUS24	18	1½	3½	1¾	1½	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1½	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1½	4¾	1¾	3½	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1½	6¾	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1½	7½	1¾	3½	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4¾	8½	2	5½	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing
Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC LUS20 3/20 exp. 6/22

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TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

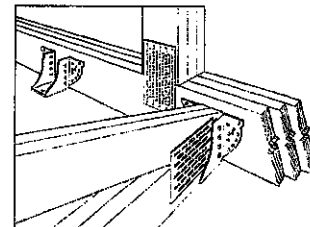
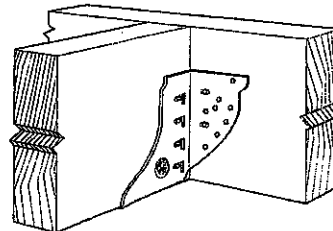
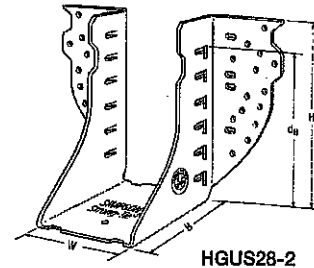
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

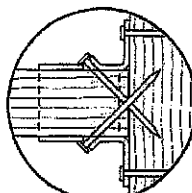
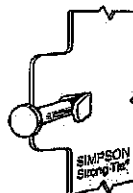
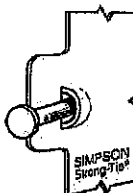
Options:

- See current catalogue for options



Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HGUS26	12	1 1/8	5 3/8	5	4 5/8	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 3/8	5 7/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 3/8	5 7/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 3/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 3/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1 1/8	9 1/8	5	7 7/8	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3 3/8	9 3/8	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 3/8	9 3/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 3/8	10 3/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 3/8	12 3/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_g is the distance from the seat of the hanger to the highest joist nail.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC HGUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

H – Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA O86-14

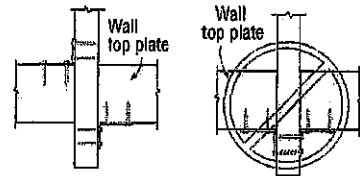
- Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

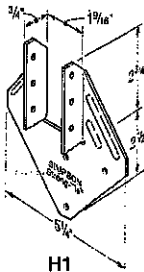
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

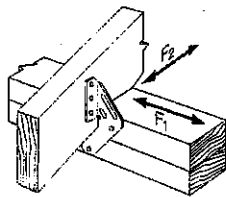


Install diagonally across from each other for minimum 2x truss.

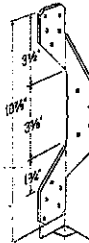
Nailing into both sides of a single ply 2x truss may cause the wood to split.



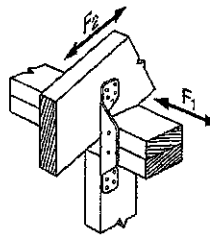
H1



H1 Installation



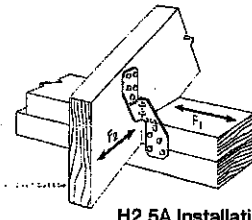
H2A



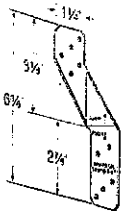
H2A Installation



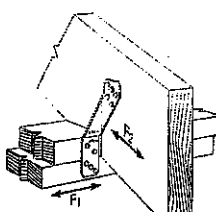
H2.5A



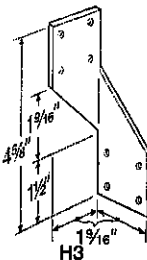
H2.5A Installation



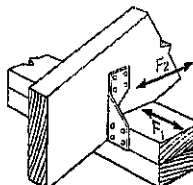
H2.5T



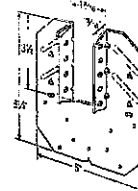
H2.5T Installation
(Nails into both top plates)



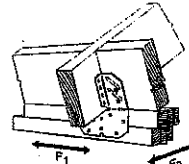
H3



H3 Installation



H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
						(K ₀ =1.15)			(K ₀ =1.15)	
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	585	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.

2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.

4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECH20 3/20 exp. 6/22

(800) 999-5099
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ONTARIO WOOD TRUSS
FABRICATORS ASSOCIATION

TECH-NOTES

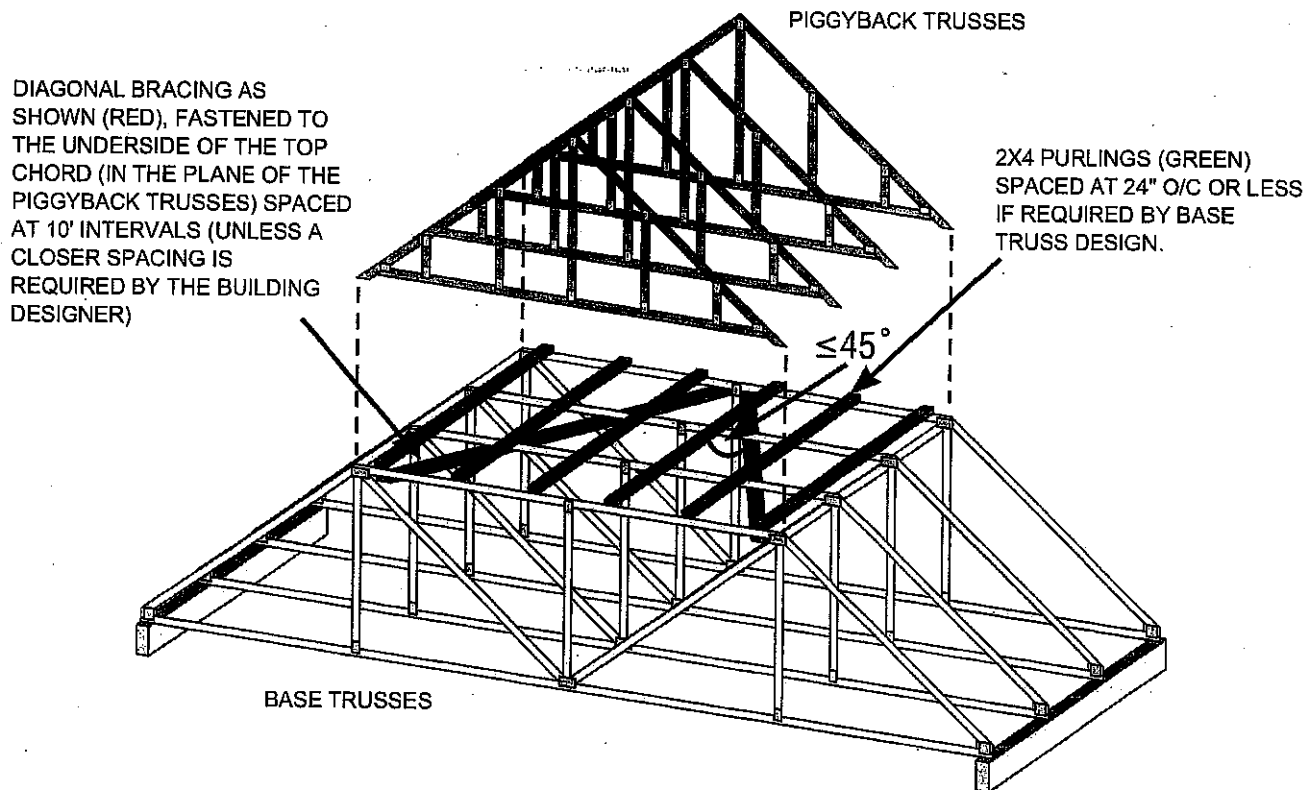
TN 15-001 Piggyback Bracing

Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

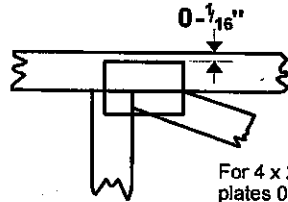
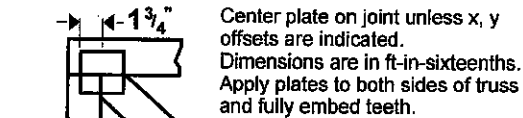
SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

Symbols

PLATE LOCATION AND ORIENTATION



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

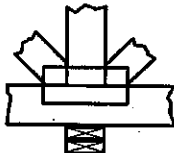
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



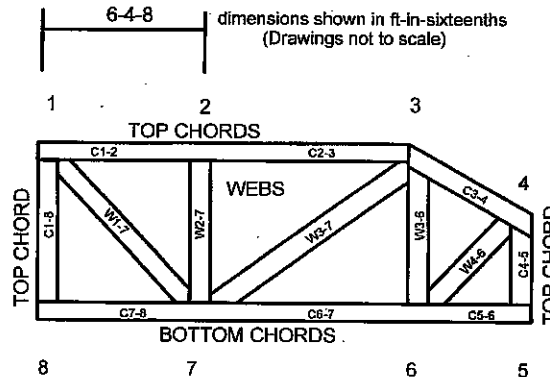
BEARING



Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR-1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.